# **BUILDING LEBRETON FLATS**

Transportation Impact Assessment Ottawa, Ontario

July 21, 2022

Presented to:

**National Capital Commission** 202 – 40 Elgin Street Ottawa, ON K1P 1C7

Presented by:

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MORRISON HERSHFIELD



FINAL REPORT

# **LeBreton Flats**

**Transportation Impact Assessment** 

Ottawa, Ontario

Presented to:

National Capital Commission 202-40 Elgin Street, Ottawa ON, K1P 1C7



# **Certification Form for TIA Study PM**

### **TIA Plan Reports**

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

#### CERTIFICATION



I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;



I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;



I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and



I am either a licensed<sup>1</sup> or registered<sup>2</sup> professional in good standing, whose field of expertise



is either transportation engineering



or transportation planning.

<sup>1,2</sup> License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

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Dated at	Ottawa	th	<sub>is</sub> 15	day of August	, 20 22
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Professio	onal title: Tran	nsportation Er	ngineer		

Signature of individual certifier that s/he meets the above criteria



# Stamp



#### 1. PLANNING RATIONALE EXECUTIVE SUMMARY

Ottawa's new Official Plan and updated Transportation Master Plan will guide Ottawa to become the most liveable mid-sized city in North America. The City's vision for walkable 15-minute neighbourhoods can be realized at LeBreton Flats, which is poised to become an exemplar of the model the new Official Plan (OP) presents if the area is able to be developed as envisioned in the LeBreton Flats Master Concept Plan (MCP). However, the outdated intent to divide LeBreton Flats with a new arterial road, the Preston Street Extension, would severely compromise the success of the redevelopment LeBreton Flats. Instead, the NCC's MCP proposes a focus on active mobility and places for people by providing a bridge in the Preston corridor for active mobility only, connected to a local street network where pedestrians are prioritized.

Urban planning has evolved significantly since the Preston Street Extension was originally conceived in the 1970s and incorporated into the previous plan for LeBreton Flats in 1997. Taking notes from the City's current OP and the new OP's Five Big Moves, the LeBreton Flats MCP proposes a new community that is unlike any other in Ottawa (or Canada) today, and will become a leader for Canadian urbanism. The MCP is a blueprint for a new type of community, built on principles of sustainability, accessibility, and community wellbeing. Through the MCP, LeBreton Flats is poised to deliver on the ambitions the City has articulated in its OP. Both the OP and the MCP posit a new approach to urban planning in Ottawa, one which reflects the global best practice of declining to cede precious urban space to automobiles and instead prioritizing space and investment in places for people.

Key to the MCP's success is the prioritization of active modes by creating a safe, segregated and direct pedestrian and cycling network that offers the most convenient and direct routes to people who use transit and active modes. At the same time, the success of this vision relies on the discouragement of fast and facilitated automobile movement through the plan area. The embedded incentives and disincentives in this filtered permeability approach are key to enable the outcomes envisioned in the MCP and the OP. The MCP was developed to promote a true 15-minute neighbourhood that is safe and accessible for everyone, provides attractive and efficient active mobility options, discourages local automobile trips and offers a pleasant, quiet and safe environment for people of all ages and abilities.

The NCC's application to amend to the Official Plan to remove the proposed Preston Street Extension in favour of an active-modes bridge supports provincial objectives and City of Ottawa planning policies and priorities as detailed in the following policy documents:

- Ontario Planning Act (1990)
- Ontario Provincial Policy Statement (2020)
- City of Ottawa Official Plan (2013), including Central Area Secondary Plan
- City of Ottawa new Official Plan (2021), including the West Downtown Core Secondary Plan
- City of Ottawa Transportation Master Plan (2013)
- City of Ottawa updated Transportation Master Plan policy direction (2022)

Approval of the requested amendment represents good planning and is essential in order to:

- Make bold and progressive infrastructure decisions that help to achieve stated City policy objectives;
- Support the sustainable transportation infrastructure investments of the City within the area:
- Increase the share of trips by sustainable modes of transportation to meet the City's climate change goals;
- Creating a healthier and more equitable and inclusive city, where anyone can get to work, to school and to daily activities without needing a private vehicle;
- Build a truly visionary and inspiring new community at LeBreton Flats;
- Capitalize on the vast park space and recreational potential of LeBreton Flats; and
- Make decisions today that align with the trajectory of urban development and support the future of Ottawa.

#### 2. TIA EXECUTIVE SUMMARY

LeBreton Flats is a large and unique site in the heart of the Nation's Capital. In many respects, the site is truly a rare one-of-a-kind gem. The site is located just 1.5 kilometres west of the Capital's Parliamentary Precinct and central business district, and is anchored by two LRT stations, along with aqueduct water features and access to the Ottawa River. The future community of LeBreton Flats has the potential to be a showcase for urban development in Canada.

A complete understanding of the transportation needs and implications of the site is necessary to guide and inform the movement from vision to reality. The Transportation Impact Assessment (TIA) has been prepared in accordance with the City of Ottawa's 2017 TIA Guidelines, as required by the City of Ottawa in support of an application to amend the Official Plan. Many high-level assumptions and findings are documented within the report, as are the details that are important to transportation professionals. The TIA report aims to provide the necessary analysis and insight at this stage of the planning process, but certainly will not be the last transportation analysis. Detailed TIA studies will be prepared each future development phase within the LeBreton Flats Master Concept Plan area, as details and specifics of such developments become known during implementation.

The foundation of the TIA is an analysis of the trips expected to be generated from the developments proposed in the Master Concept Plan. The Master Concept Plan provides a vision for a future community, with a likely range of development yield scenarios. To help decision makers assess the potential traffic impacts of the Master Concept Plan, four land use scenarios were reviewed and **Scenario 4 was selected for evaluation**, as it is the most ambitious development scenario, with the highest predicted trip generation. The City's TRANS Trip Generation Manual was used to calculate residential trips, with the ITE Trip Generation Manual used for all other trips. The LeBreton Flats development is expected to generate approximately **4,800 person trips in the weekday morning peak hour**, and **8,100 person trips in the weekday afternoon peak hour**. Aggressive mode share splits are targeted, with **15% auto driver**, **5% auto passenger**, **60% transit and 20% walking and cycling**. These mode share targets are comparable to those in similar Transit Oriented Developments in Ottawa, such as 900 Albert Street, the Zibi development, and Wateridge Village.

The LeBreton Flats Master Concept Plan proposes the elimination of the planned vehicular connection of Preston Street between Albert Street and the Sir John A. Macdonald Parkway. This connecting roadway has been part of previous versions of the City of Ottawa Official Plan and Transportation Master Plan but there was has been no plans or studies to date initiating this as a future City project. Modelling and analysis in the TIA shows that **the drawbacks of the Preston extension far outweigh the benefits**. Constructing the Preston extension would divert additional traffic to the area, worsening traffic conditions around LeBreton Flats. The Preston extension would draw traffic away from roads that are well beyond the influence area of LeBreton Flats and result in an increase in traffic in both directions on Preston Street, Rochester Street and Booth Street. In addition, the removal of the Preston extension avoids the need to designate Wellington Street as a Truck Route, which is consistent with the prohibition of heavy vehicles on the SJAMP, and increases accessibility to the LRT stations to ensure the achievement of the high transit mode share targets set as part of the development. For these reasons, the deletion of the Preston vehicular extension from the City's Official Plan is recommended.

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## 1. INTRODUCTION

LeBreton Flats is a large and unique site in the heart of the Nation's Capital. In many respects, the site is truly a rare one-of-a-kind gem. The site is approximately 29 hectares in size and is located just 1.5 kilometres west of the Capital's Parliamentary Precinct and central business district. The site is anchored by two LRT stations at Pimisi and Bayview, aqueduct water features, and Nepean Inlet, with access to the Ottawa River. The future community of LeBreton Flats has the potential to be a showcase for future urban development in Canada. As with any urban development of this caliber, along with its enormous potential comes significant challenges. Understanding the value of the site as well as the nature and significance of the challenges facing its development is necessary. Failure to do so may unreasonably deem some challenges as development constraints and, in doing so, sadly miss the opportunity to undertake proper trade-offs analysis and unnecessarily compromise the full potential of the site.

A complete understanding of the transportation needs and implications of the site is necessary to guide and inform the movement from vision to reality. This report aims to provide the necessary analysis and insight, but certainly will not be the last. Our world continues to change and preparing this report during the pandemic highlights the fact that we could very well be embarking on a new era in transportation, which will require us to revisit our past assumptions about travel needs and expectations. Regardless, as required by the City's TIA guidelines, this report uses past experience to predict future outcomes. There are many high-level assumptions and findings, which are documented within the report, as are the details that are important to transportation professionals.

The immediate surrounding roadway network, consisting of Albert Street, Booth Street, and Wellington Street exhibit varying degrees of congestion today. Expanding the capacity of these roadways is not foreseen, as LeBreton Flats and the roads that surrounds it fall within the City's Downtown Core (refer to **Figure 1**). The City of Ottawa Transportation Master Plan and New Official Plan do not support roadway expansion in this constrained urban area of the City. Therefore, additional roadway capacity has not been proposed as part of this development, other than new local roads provided as part of the development access/egress. Providing a supportive environment for pedestrians and cyclists will improve the capacity of the active transportation network and help to improve active mode share.



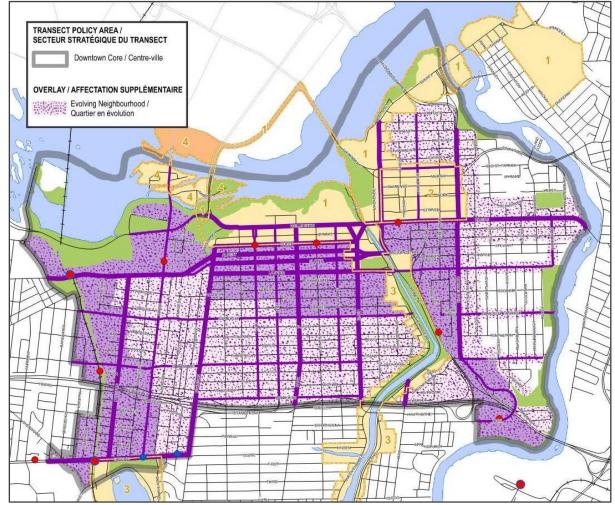


Figure 1: Downtown Core Transect Policy Area (Source: City of Ottawa Official Plan 2021)

This report has been prepared in accordance with the City of Ottawa's 2017 Transportation Impact Assessment Guidelines, as required by the City of Ottawa in support of the Master Concept Planning process. Additionally, it is acknowledged that detailed TIA studies will be prepared at a later date in the future, for each individual development phase associated with the LeBreton Flats lands, as details and specifics of such developments become more known closer to implementation time.

In addition to the above, the following should also be noted:

- Baseline Conditions: Study area intersections and roadways surrounding LeBreton Flats have been influenced by the LRT construction activities (e.g., transitway detours, the construction of Booth Street over the LRT corridor, etc.). With respect to the timelines associated with the Master Concept Planning process, City Staff agreed to using historical traffic count data from the year 2014, as this is a time that likely best represents normal travel patterns and volumes. It should be noted that LRT opening delays and the COVID-19 pandemic during the spring of 2020 further complicated any potential efforts to collect more recent traffic data that could be viewed as being representative of "typical" conditions.
- Network Modifications: The LeBreton Flats Master Concept Plan proposes the elimination of the planned vehicular connection of Preston Street between Albert Street and the Sir John A. Macdonald Parkway. This connecting roadway has been part of previous versions of the City



of Ottawa Official Plan and Transportation Master Plan but has not been scheduled to be built. The Planning Rationale supporting the removal of the Preston Street extension is submitted under separate cover. The implications of the removal of the Preston Street extension from the future road network are explored in *Section 4.4* of this report. This was informed by regional level modelling using the City's EMME/3 Travel Demand Model and an assessment of operational impacts on the surrounding road network.

- Mode Share Targets: LeBreton Flats currently has exceptional active transportation facilities, and the Master Concept Plan will build on this by creating world-class facilities to support active transportation and transit modes. Future residents and businesses that will call LeBreton Flats home, will be exceptionally well located geographically and supported by the existing transportation system to easily access Ottawa and Gatineau's downtown cores, and some of the other great amenities the Nation's Capital has to offer. As such, and as detailed in this report, it is reasonable to expect an aggressive reduction in the degree to which private vehicles are relied upon. The mode share targets set in this TIS are comparable to those of similar Transit Oriented Developments, including 900 Albert Street, the Zibi development, and Wateridge Village.
- Trip Generation: The foundation of the analysis in this report is the trip generation expected to be realized from the developments proposed in the Master Concept Plan. The Master Concept Plan provides a vision for a future community, with a likely range of development yield scenarios. To help decision makers assess the potential traffic impacts of the Master Concept Plan, the scenario that results in the highest predicted trip generation has been evaluated. It is likely that the proposed development will evolve over time, at which point, updated traffic studies will be completed with more precise predictions.
- Preston Street Extension: Given that the Preston extension from Albert Street to Wellington Street has been part of the City's Transportation Master Plan and Official Plan for many years, additional analysis was undertaken to justify its proposed removal from the future road network. The removal of the Preston extension from the Official Plan has many benefits, explored in Section 4.4, including maximizing the accessibility to the LRT stations by minimizing walking distances; this ensures that the high transit mode share target set for the development is in fact achieved.



# 2. STEP 1 - SCREENING FORM

As required by the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines, a Screening Form was completed for the proposed development (described below in **Section 3.1**). The Screening Form triggered the trip generation, location and safety criteria outlined in the City's TIA Step 1 – Screening Form. Since all triggers were met, a formal TIA (i.e., TIA encompassing Steps 1 through 5) is required to accompany the development application. The Screening Form is provided in **Appendix A**.



## 3. STEP 2 – SCOPING

# 3.1 Existing and Planned Conditions

#### 3.1.1 Description of Proposed Development

The subject development lands (i.e., LeBreton Flats) are generally situated within the area bound by Booth Street to the east, Wellington Street / Ottawa River Parkway to the north, Albert Street to the south and the Trillium Pathway to the west. Several development scenarios were provided, and the scenario that is likely to result in the highest trip generation has been evaluated.

Based on the information provided, the proposed redevelopment of LeBreton Flats is planned to include a mix of high-density residential, office and retail type land uses, as well as approximately 12.7 hectares of parks and open spaces. It should also be noted that the Master Concept Plan includes an option to host a new major event centre. Given the size of LeBreton Flats, market demand will ultimately dictate the rate of development.

The Master Concept Plan depicts thirteen access points, including six access points to Albert Street, five access points to Wellington Street, and two access points to Booth Street. All new internal streets within LeBreton Flats are intended to be designed to be slow speed and relatively narrow shared spaces (e.g., no raised curbs). Almost all parking will be provided in underground lots with access/egress located near the edge of the Flats.

Internal multi-use pathways will be provided to support active mobility, which will enhance access to parks, provide connectivity between on-site facilities, and will be fully integrated with the Capital Pathway network and the City's extensive pedestrian/cycling network. This active network will also include two new multi-use pathway structures over Ottawa's LRT Confederation Line that will provide convenient and direct access to the highest order public transit via existing LRT stations at Pimisi and Bayview, as well as regular OC Transpo bus service provided along Booth Street and Albert Street.

The local context of the subject site is provided in **Figure 2**, the proposed Master Concept Plan is provided in **Figure 3**, and the proposed development phasing is provided in **Figure 4**.





Figure 2: Local Context





**Figure 3: Proposed Master Concept Plan** 





Figure 4: Development Parcels & Phasing



#### 3.1.2 Existing Conditions

The transportation network surrounding LeBreton Flats has undergone significant changes over the past several years, mainly because of LRT construction that required temporary detouring of transitway bus traffic onto study area roadways. With respect to the City's TIA Guidelines, the following describes the study area network as it is in its current capacity.

#### Area Road Network

Wellington Street is a four-lane Arterial roadway (i.e., a two-lane per direction) with sidewalks on both sides, that extends from Sussex Drive in the east to Vimy Place in the west. Beyond Sussex Drive and Vimy Place, Wellington Street continues as Rideau Street and the Sir John A. Macdonald Parkway, respectively. Within the vicinity of the subject site, the speed limit is 60 km/h and on-street parking is provided along both sides of the roadway between Booth Street and Vimy Place.

Bay Street is a two-lane, one-way northbound Local roadway with a bike lane and sidewalks on both sides, located within the vicinity of the subject development. It extends from Catherine Street in the south to Wellington Street in the north. The cycling facility on Bay Street is in the process of being upgraded to provide uni-directional northbound and southbound cycle tracks between Laurier Avenue and Wellington Street. Within the vicinity of the subject site, the posted speed limit is 50 km/h and onstreet parking is permitted on the west side of the roadway between Catherine Street and Laurier Avenue.

Lyon Street North is a three-lane, one-way southbound Arterial roadway with sidewalks on both sides, located within the vicinity of the subject development. It extends from Highway 417 in the south to Wellington Street in the north. South of Somerset Street, this roadway is reduced to two lanes. Within the vicinity of the subject site, the posted speed limit is 50 km/h and on-street parking is permitted on the west side of the roadway between Slater Street and Catherine Street. There is a southbound bike lane on the segment south of Albert Street.

Albert Street is a five-lane Arterial roadway (i.e., two eastbound lanes and three westbound travel lanes, with east and westbound shoulder lanes reserved for transit only) along the southern frontage of the subject site. This roadway continues as Mackenzie King Bridge east of Elgin Street and as Scott Street west of Bayview Station Road. East of Empress Avenue, two-way traffic on Albert Street is split into two one-way roadways (i.e., eastbound, Albert Street continues one-way as Slater Street, between Bronson Avenue and Elgin Street; and westbound, Albert Street operates as one-way between Bronson Avenue and Elgin Street). There is a bidirectional multi-use pathway on the north side of Albert Street from Commissioner Street in the east to Bayview Station. Within the vicinity of the subject site, the posted speed limit is 50 km/h and Albert Street is designated as a Full Loads truck route with respect to the City's Urban Truck Routes network.

Slater Street is a three-lane, one-way Arterial roadway within the vicinity of the subject site. It develops/merges with Albert Street and the Mackenzie King Bridge at Empress Avenue in the west and Elgin Street in the east, respectively. Within the vicinity of the subject site, the posted speed limit is 50 km/h, and on street parking is permitted on both sides of the roadway during nonpeak periods and weekends (with the exception of along the southern frontage of the subject site, parking is prohibited), and Slater Street is designated as a Full Loads truck route with respect to the City's Urban Truck Routes network.

Bronson Avenue is a four-lane Arterial roadway (i.e., two travel lanes per direction) with sidewalks on both sides, located within the vicinity of the subject development. It extends from Sparks Street in the north and continues as the Airport Parkway, south of the Rideau River. Within the vicinity of the subject



site, the posted speed limit is 50 km/h and it is designated as a Full Loads truck route south of Albert Street with respect to the City's Urban Truck Routes network.

Booth Street is a four-lane Arterial roadway (i.e., two travel lanes per direction), which passes through the subject development site. It extends from Carling Avenue in the south, crossing the Confederation Line LRT tracks as a bridge within the subject site, and continues north into Gatineau, where it becomes Eddy Street. Booth Street is designated as a Restricted Loads truck route with respect to the City's Urban Truck Routes network. Within the vicinity of the subject site, the posted speed limit is 50 km/h. There are raised cycle tracks and sidewalks on both sides of the roadway within the subject site area. However, south of Albert Street, Booth Street is reduced to a two-lane Major Collector Road with a posted speed limit of 40 km/h. Booth Street, south of Albert Street, supports residential land uses on both sides, sidewalks on both sides, with a narrow set-back and on-street parking on the west side of the roadway. This section of Booth Street is not part of the City's Urban Truck Routes network, and significant efforts have been undertaken to preserve the residential nature of this section of the road, including turning restrictions, speed humps and other traffic calming measures (refer to Section 3.1.2 for more details).

**Sir John A. Macdonald Parkway** is a four-lane federally owned divided Parkway (i.e., two travel lanes per direction) within the vicinity of the subject development. It extends from Vimy Place in the east and continues west where it merges into Carling Avenue (near the Lincoln Fields transit station). Within the vicinity of the subject site, the posted speed limit is 60 km/h and on-street parking is not permitted. A multi-use pathway runs along the Ottawa River parallel to the Parkway.

Scott Street is a four-lane Arterial roadway (i.e., two travel lanes per direction) within the vicinity of the subject development. It extends from Churchill Avenue in the west and continues as Albert Street, east of Bayview Station Road. Within the vicinity of the subject site, the posted speed limit is 50 km/h and it is designated as a Full Loads truck route with respect to the City's Urban Truck Routes network. It features a multi-use pathway along its north side, from Bayview Station Road to Churchill Avenue, with a sidewalk and bike lane along its south side.

**Bayview Station Road** is a two-lane Collector roadway (i.e., one travel lane per direction) with sidewalks within the vicinity of the development. It extends between Albert Street in the south and Burnside Avenue in the north. The posted speed limit is 50 km/h and on-street parking is permitted on both sides of the roadway.

Slidell Street is a two-lane Collector roadway (i.e., one travel lane per direction) with one discontinuous sidewalk within the vicinity of the subject development. It extends between Burnside Avenue in the south and the Sir John A. Macdonald Parkway in the north, where it continues north as Onigam Street. The posted speed limit is 40 km/h and on-street parking is prohibited.

Preston Street is two-lane Arterial roadway (i.e., one travel lane per direction) within the vicinity of the subject development. It extends between Albert Street in the north and Queen Elizabeth Driveway in the south. Within the vicinity of the subject site, there are sidewalks on both sides, the posted speed limit is 50 km/h and on-street parking is permitted on the east side of the roadway only, and it is designated as a Full Loads truck route with respect to the City's Urban Truck Routes network.

City Centre Avenue is two-lane Local roadway (i.e., one travel lane per direction) with partial sidewalks within the vicinity of the subject development. It extends between Albert Street in the north and Somerset Street in the south. Within the vicinity of the subject site, the posted speed limit is 50 km/h and on-street parking is permitted on both sides of the roadway.

Parkdale Avenue is a two-lane Arterial roadway (i.e., one travel lanes per direction) within the vicinity of the subject development. It extends between Carling Avenue in the south and the Sir John A.



Macdonald Parkway in the north. The posted speed limit is 50 km/h within the vicinity of the subject site and there are sidewalks on both sides.

Vimy Place is a private two-lane Local roadway (i.e., one travel lanes per direction). It extends between the Sir John A. Macdonald Parkway and Booth Street. The posted speed limit is 40km/h and on-street parking is permitted on the south side of the roadway, along the Canadian War Museum frontage.

#### Study Area Intersections

Wellington/Portage Bridge - The Wellington/Portage Bridge intersection is a signalized, three-legged intersection. The north approach (Portage Bridge) consists of three left turn lanes (including one bus/taxi/HOV lane) and one channelized right-turn lane. The west approach (Wellington Street) consists of three right-turn lanes, and two left turn lanes. The east approach (Wellington Street) consists of two left-turn lanes, and three right-turn lanes (including one transit exclusive lane).

There are no prohibited vehicular movements at this intersection. There is a separate active-modes network at a lower level below the road network.

Booth/Chaudière - The Booth/Chaudière intersection is a signalized, three-legged intersection. The west approach (Chaudière) consists of one shared left-right turn lane exiting from privately-owned Zibi development lands. The north approach (Booth Street) consists of one shared through-right lane and one exclusive through lane. The south approach (Booth Street) consists of one left-turn lane and two through lanes.

There are no prohibited movements at this intersection. As part of the Zibi development project, this intersection is being reconstructed to accommodate a shared through-right lane and a short left-turn lane on all approaches.

Booth/War Museum - The Booth/War Museum intersection is a signalized three-legged intersection. The south approach (Booth Street) consists of one left turn lane and two through lanes. The north approach (Booth Street) consists of two shared lanes for all movements. The west approach (War Museum) consists of one shared lane for all movements. The primary function of this intersection is to provide signalized crossing for users of the Ottawa River Pathway MUP.

**Booth/Wellington** - The Booth/Wellington intersection is a signalized four-legged intersection. The south approach (Booth Street) consists of one shared right turn-through lane and one through lane. The north approach (Booth Street) consists of two through lanes, one left turn lane, and one right turn lane. The east approach (Wellington Street) consists of one right turn lane and two through lanes. The west approach (Wellington Street) consists of two through lanes. This intersection was recently reconstructed as a protected intersection with cycling lanes separated from vehicular traffic.

Left and right turns are prohibited at the west approach. Left turns are prohibited at the east approach except on Sundays from 7am-1pm in order to facilitate Sunday closures of the SJAM. Left turns are prohibited at the south approach.

Albert/Booth - The Albert/Booth intersection is a signalized four-legged intersection. The north approach (Booth Street) consists of one through lane, one left turn and one right turn lane. The south approach (Booth Street) consists of one shared left-through lane and one shared through-right lane. The east approach (Albert Street) consists of one left turn lane, one right turn lane and three through lanes, including one transit exclusive lane. The west approach (Albert Street) consists of one left turn lane, one through lane and one transit exclusive through lane, which acts as a right turn lane for non-transit vehicles.



Left turns are prohibited at the east approach during 7-9AM and 3:30 - 5:30PM on weekdays. Right-Turn-On-Red movements are prohibited from 7AM-9PM on weekdays for the north and east approaches. Through traffic is prohibited from 11PM to 6AM on the north approach.

Trucks are directed to turn left or right on the north approach. Signage indicates that trucks and buses are prohibited from traveling southbound on Booth Street from the Booth/Albert intersection.

Albert/Preston - The Albert/Preston intersection is a signalized, three-legged intersection. The south approach (Preston Street) consists of one left turn, and one right turn lane. The west approach (Albert Street) consists of one through lane and one transit exclusive through lane, which acts as a right turn lane for non-transit vehicles. The east approach (Albert Street) consists of three through lanes, including one transit exclusive lane, and one left turn lane.

There are no prohibited movements at this intersection.

Wellington/Vimy - The Wellington/Vimy intersection is a signalized, three-legged intersection. The north approach (Vimy PI) consists of one shared lane for all movements. The west approach (Sir John A. Macdonald Parkway) consists of two through lanes, and one left turn lane. The east approach (Wellington Street) consists of one through lane and one shared through-right lane.

There are no prohibited movements at this intersection.

Sir John A. Macdonald/Slidell - The Sir John A. Macdonald/Slidell intersection is a signalized, four-legged, intersection. The north approach (Onigam Street) consists of one shared through-right lane. The south approach (Slidell Street) consists of one through lane. The west and east approaches (Sir John A. Macdonald Parkway) each consists of one shared left-through lane and one shared through-right lane.

Left turns and right turns are prohibited at the west and east approaches from 7-9AM and 4-6PM. Left turns are prohibited at the north approach. Additionally, both left and right turns are prohibited at the south approach.

**Sir John A. Macdonald/Parkdale** - The Sir John A. Macdonald/Parkdale is an unsignalized interchange connecting Sir John A. Macdonald Parkway and Parkdale Avenue. Two through lanes are maintained in each direction on Sir John A. Macdonald Parkway through the interchange. There are no ramp terminal intersections since all possible movements are accommodated through free-flowing merge and diverge ramps.

Albert/City Centre - The Albert/City Centre intersection is a signalized four-legged intersection. The south approach (City Centre Avenue) consists of one left turn lane and one shared through-right lane. The north approach (OC Transpo Access) consists of one shared lane for all bus movements. The east approach (Albert Street) consists of one left turn lane, two through lanes (including one transit exclusive lane), and a transit exclusive right turn lane. The west approach (Albert Street) consists of one transit exclusive left turn lane and two through lanes (including one transit exclusive lane that facilitates right-turn movements for non-transit vehicles).

Non-transit vehicles are prohibited from entering the north approach of the intersection.

Albert/Bayview Station - The Albert/Bayview Station intersection is a signalized four-legged intersection. The south approach (Bayview Station Road) consists of one through lane, one left turn lane, and one channelized right turn lane. The north approach (Bayview Station Road) consists of one shared through-right lane and one left turn lane. The east approach (Albert Street) consists of one left turn lane, one through lane, and one transit exclusive through lane that facilitates right-turn movements for non-transit vehicles. The west approach (Scott Street) consists of one shared left turn-through lane and one transit exclusive through lane that facilitates right-turn movements for non-transit vehicles.



There are no prohibited movements at this intersection.

Scott/Parkdale - The Scott/Parkdale intersection is a signalized four-legged intersection. The north approach (Parkdale Avenue) consists of one shared right turn-through lane, and one left turn lane. The south approach (Parkdale Avenue) consists of one shared right turn-through lane, and one left turn lane. The east approach (Scott Street) consists of two through lanes (including one transit exclusive lane that facilitates right-turn movements for non-transit vehicles), and one left turn lane. The west approach (Scott Street) consists of two through lanes (including one transit exclusive lane that facilitates right-turn movements for non-transit vehicles), and one left turn lane.

There are no prohibited movements at this intersection.

#### Existing Driveways to Adjacent Developments

There are 19 driveways that fall within a 200m boundary of the site. These exclude driveways that only serve a single private dwelling.

- 12 driveways are located near the south perimeter of the site
  - 1 driveway on Empress Avenue that is 40m south of Albert Street, connecting to a seniors' centre and spiritual centre parking lot.
  - 3 driveways on Booth Street. Two of which are approximately 50m south of Albert Street, connecting to office buildings and a townhouse complex. The third driveway is approximately 90m south of Albert Street, connecting to a separate townhouse complex.
  - 3 driveways on Rochester Street, all located at the cul-de-sac at the north end of the street, connecting to townhouse complexes.
  - 3 driveways on Primrose Avenue. Two are located 40m east of the intersection while the remaining driveway is located 100m west of the intersection. All driveways provide connections to separate townhouse complexes; and,
  - 2 driveways on City Centre Avenue, located approximately 50m and 150m south of Albert Street. Both driveways provide connections to an office and retail complex.
- 6 driveways are located near the east perimeter of the site
  - 4 driveways on Lett Street, ranging from approximately 70m south of Wellington Street to approximately 220m south of Wellington Street. All four driveways connect to apartment complexes.
  - 1 driveway is located on Fleet Street, approximately 50m east of Booth Street, providing connection to an apartment show room/office, and
  - 1 driveway is located on Lloyd Street, approximately 90m south of Fleet Street. This
    driveway provides connection to a surface parking lot.
- 1 driveway is located near the north perimeter of the site
  - This driveway is located on Vimy Place, approximately 260m west of Booth Street, serving the parking lot of the Canadian War Museum.

#### Pedestrian/Cycling Network

The pedestrian network in the vicinity of the site is well developed and offers a number of convenient and scenic routes, such as the expansive Capital Pathway and Trans Canada Trail (along the Ottawa River), the Trillium Pathway (along the Trillium LRT line), and the aforementioned multi-use pathway along the north side of Albert Street / Scott Street, all of which are in close proximity to LeBreton Flats and will have direct connectivity to the development.



Sidewalks are also provided along both sides of study area roadways, in most cases. Exceptions can be found on select local streets accommodating low vehicle speeds, where sidewalks are either reduced to one side only or terminate midblock, such as City Centre Avenue. It should also be noted that Sir John A. Macdonald Parkway and portions of Slater Street do not have sidewalks (e.g., currently, Slater Street between Empress Street and Bronson Avenue has little to no sidewalks available for pedestrians).

With regard to cycling facilities, the study area is bisected by two cycling spine routes (Albert Street and Booth Street) as defined by the Ottawa Cycling Plan 2013; additional spine routes in the area include Wellington Street from the Portage Bridge to Vimy Place, and Slater Street east of the split with Albert Street. Additionally, the study area is surrounded by various pathway networks (NCC Capital Pathway, Trillium Pathway, and Albert Street multi-use pathway). The existing multi-use path/cycling network within the vicinity of the subject site, as sourced from GeoOttawa, is shown in the following Figure 5.



Figure 5: Existing Multi-Use Path/Cycling Network

As shown in Figure 5, there are currently multi-use pathways directly adjacent to LeBreton Flats along Albert Street, which feed directly into bike lanes on Scott Street to the west, and dedicated cycle tracks on Laurier Avenue to the east. Based on field observations and local area knowledge, cycling activity is considered to be high within the vicinity of the subject development lands.



#### **Transit Network**

OC Transpo currently provides the highest order transit service through the heart of LeBreton Flats. The site will benefit from direct access to both of OC Transpo's O-Train Lines: Confederation Line and Trillium Line. The Bayview LRT Station is located along the western limit of LeBreton Flats, which serves as a transfer station between the east-west Confederation Line (Line 1) and north-south Trillium Line (Line 2). The Pimisi LRT Station is located closer to the eastern limit of LeBreton Flats and provides service for the east-west Confederation Line (Line 1).

Additionally, 11 OC Transpo bus stops are located within walking distance to/from LeBreton Flats. The following **Table 1** summarizes existing stops, their associated routes and direction of travel. In addition to OC Transpo, STO also provides service between downtown Ottawa and Hull. STO provides service through the study area via Portage Bridge and Wellington Street; however, there are currently no stops within a reasonable walking distance to/from the subject development site.

**Table 1: Transit Information** 

Stop #	Location	Route Identifier	Direction	
#0433	120m north of Booth/Wellington	61, 63, 66, 75, 85	Southbound	
#1877	Immediately south of Booth/Wellington	61, 63, 66, 75, 85	Southbound	
#1876	Immediately south of Booth/Wellington	61, 63, 66, 75, 85	Northbound	
#2371	Immediately south of Preston/Albert	85	Southbound	
#2392	Immediately west of Albert/Empress	16,57,61,75	Westbound	
#2396	Immediately east of Albert/Empress	16,57,61,75	Eastbound	
#3010	Pimisi LRT Station	Confederation Line	East/Westbound	
#3010A	Pimisi Station, Upper Level	61, 63, 66, 75, 85	Northbound	
#3010B	Pimisi Station, Upper Level	61, 63, 66, 75, 85	Southbound	
#3010C	Immediately west of Booth/Albert	16,57,61,75	Westbound	
#3010D	Immediately east of Booth/Albert	16,57,61,75	Eastbound	
#3060	Bayview LRT Station	Confederation & Trillium Line	East/Westbound & Southbound	
#3060A	150m west of City Centre/Albert	16,57,61,63,66,75	Westbound	
#5684	100m east of Preston/Albert	16,57,61,75,85	Eastbound	
#5722	120m north of Booth/Wellington	61, 63, 66, 75, 85	Northbound	
#6659	70m west of Preston/Albert	16,57,61,75	Westbound	
#8005	Immediately south of Preston/Albert	85	Northbound	
#8048	Immediately east of City Centre/Albert	16,57,61,75	Eastbound	



The following **Figure 6** depicts the OC Transpo routes within the vicinity of the LeBreton Flats, and **Table 2** provides additional information with respect OC Transpo service identified in Table 1.



Figure 6: Transit Routes Within Study Area (Source: OC Transpo System Map)

**Table 2: OC Transpo Route Information** 

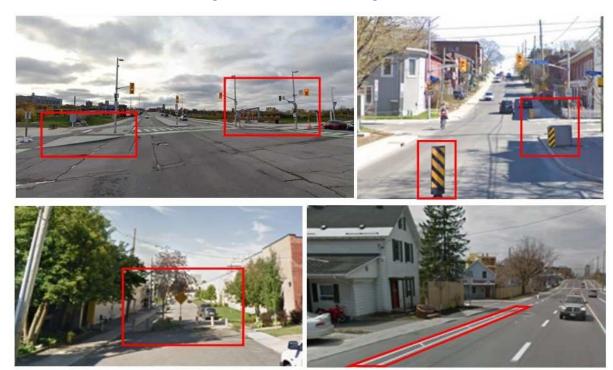
Route	Origin/Destination	Service Type	Peak Hour Headway	
1	Confederation Line (Tunney's Pasture ↔ Blair)	LRT	5 min	
2	Trillium Line (Bayview ↔ Greenboro)	LRT	12 min	
16	Tunney's Pasture/Westboro ↔ Main Street	Local	30 min	
57	Tunney's Pasture ↔ Bells Corners	Rapid & Night Route	15 min (20 -30 min on Night Route)	
61	Tunney's Pasture/Gatineau ↔ Stittsville	Rapid & Night Route	5 min, (30 min on Night Route)	
63	Briarbrook ↔ Tunney's Pasture	Rapid	15 min	
66	Gatineau ↔ Kanata-Solandt	Limited Local	15 min, AM-out/PM- inbound only	
75	Barrhaven Centre ↔ Tunney's Pasture/Gatineau	Rapid & Night Route	10 min, (20 -30 min on Night Route)	
85	Gatineau ↔ Bayshore	Frequent	15 min	



## Area Traffic Management Measures

The following **Figure 7** highlights the various area traffic management measures implemented within the vicinity of LeBreton Flats. The top left corner of the figure shows bulb-outs, deflectors, and turning restrictions on Wellington/Booth. The top right corner of the figure shows bulb-outs, planter, and vertical centreline treatments on Booth Street, south of the Booth/Albert intersection. It should also be noted that there are speed humps on Booth between Albert Street and Primrose Avenue. The bottom left corner shows on-street plazas/vehicle access closure on Elm St. W (vehicle access closures are also present on Spruce St. W). The bottom right corner shows road dieting measures on Scott Street in the form of a bike lane with buffer.

Figure 7: Area Traffic Management





#### Peak Hour Travel Demands

For the purpose of this assessment and based on the initial study, the following study area intersections have been identified for intersection capacity analysis (traffic count date included in parentheses):

- Portage Bridge/Wellington (June 2014)
- Booth/Chaudière
- Booth/Wellington (May 2013)
- Booth/Albert (April 2014)
- Booth/War Museum (July 2013)
- Albert/Preston (April 2014)
- Vimy/Wellington (January 2020)
- Slidell/Sir John A. Macdonald Parkway (April 2017)
- Albert-Scott/Bayview Station (April 2014)
- Albert/City Centre (April 2014)
- Parkdale/Sir John A. Macdonald Parkway (February 2020)
- Parkdale/Scott (April 2014)

It is noted in the City's 2013 TMP that reliance on vehicles to enter and exit the downtown has been diminishing for some time now. **Figure 8** below, from the TMP, illustrates this graphically. The exhibit makes it clear that between 1986 and 2011, the number of vehicles arriving downtown in the morning peak period has decreased while the number of people arriving downtown has increased. An Origin-Destination Travel Survey that was planned for 2021 has been delayed due to the COVID pandemic, but the trend is expected to have continued from 2011 onwards due to the increased residential development in the downtown, improved transit service, and limited roadway capacity to enter the downtown.

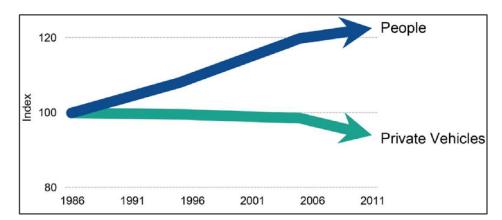


Figure 8: Trips Entering the Downtown Core, Morning Peak Period, 1986 - 2011

According to the City of Ottawa's New Official Plan (2021), LeBreton Flats falls within the Downtown Core Transect (formerly known as Ottawa's Central Area in older versions of the Official Plan), which encompasses a large portion of the downtown area. Therefore, for the purposes of this study, no background traffic growth (i.e., background traffic growth of 0%) was assumed.



In addition to the lack of growth in background traffic, the study area roadways have been impacted by LRT related construction activities for a considerable time (2015-2020) which reduced the attractiveness of relying on private vehicles and prompted some to change their trip time, forego their trips, or change routes/destinations in an effort to avoid congestion. Additionally, the COVID-19 crisis that started in March 2020 further impacted travel patterns, making more recent traffic counts post LRT implementation not beneficial or representative of "typical" conditions. Therefore, and as agreed to by City Staff, historical traffic count data from the year 2014 (where available) was used for analysis purposes. It should be noted that due to certain data gaps (i.e., not every study area intersection was counted during the year 2014), a volume balancing exercise was conducted (i.e., traffic volumes were appropriately adjusted to minimize large volume imbalances between study area intersections).

The following **Figure 9** depicts observed weekday morning and afternoon peak hour vehicle volumes at the study area intersections and **Figure 10** illustrates pedestrian and cyclist volumes over the same peak hour periods. It should be noted that two of the counts were taken during winter, and six of the counts were taken in early spring, which may result in artificially lower cycling volumes due to poor cycling conditions. Additionally, City staff indicate that cycling volumes have greatly increased since 2014, which means cycling volumes below may be underreported for current conditions. Detailed traffic count data is included in **Appendix B**.

### **Existing Road Safety Conditions**

Available collision data for the years 2015 – 2019 was obtained from the City of Ottawa's Open Data Catalogue and provided in **Appendix C**. The collision data includes all collisions occurring at the intersections and the roadway segments within the area surrounding the subject development site, including intersections and segments along Albert Street, Booth Street, Parkdale Avenue, Scott Street, Wellington Street and Sir John A. Macdonald Parkway.

Based on the most recent available historical collision data, the 5-year total number of recorded collisions within the study area is 552. Most collisions within the study area (441 incidents or 80%) resulted in property damage only, and the remaining collisions result in either personal injuries (109 incidents or 20%) or fatalities (2 incidents or <1%). Both fatalities occurred outside the development area, at the intersection of Sir John A Macdonald Parkway (SJAMP) with Slidell. The most frequent types of collisions, as cited by police, were rear ends (217 incidents or 39%) and sideswipes (100 incidents or 18%).

It is noteworthy that within the five years of recorded collision data, there were 10 collisions involving pedestrians. Fortunately, all the reported collisions involving pedestrians were non-fatal; however, personal injuries were reported.

There were 20 collisions involving cyclists within the five years of recorded data, 19 of which were at intersections and 1 which was on a roadway segment. It is notable that 4 of the 20 collisions occurred at the intersection of Albert Street and Booth Street.



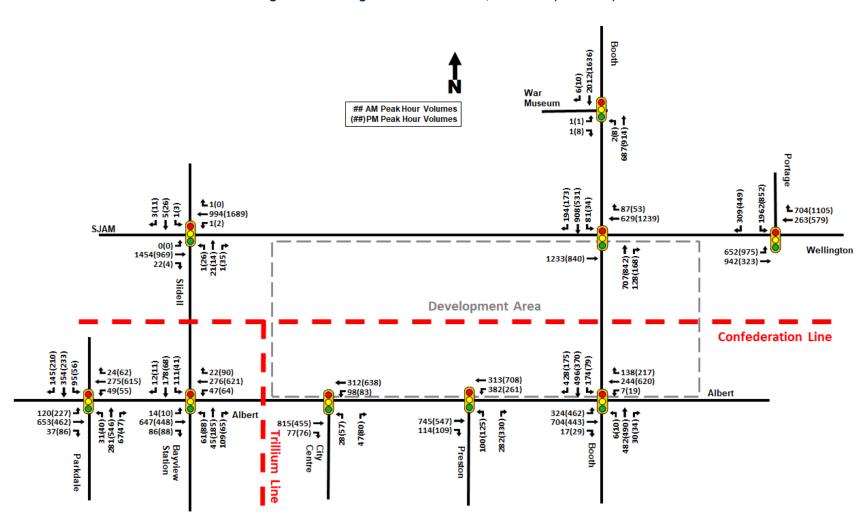


Figure 9: Turning Movement Counts, AM Peak (PM Peak)



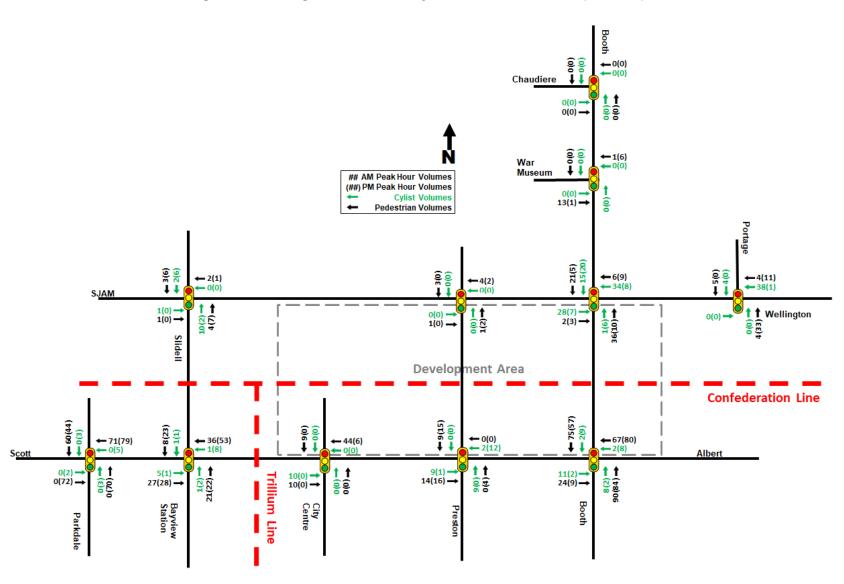


Figure 10: Existing Pedestrian and Cyclist Volumes, AM Peak (PM Peak)



#### 3.1.3 Planned Conditions

#### **Active Transportation Projects**

Cycling projects underway or planned in the area include:

- Approximately one kilometer of multi-use pathway was recently built in LeBreton Flats. This
  pathway links existing multi-use pathways at Pimisi LRT Station with the Trillium Pathway and
  the Ottawa River Pathway.
- Uni-directional cycle tracks on Booth Street north of Wellington Street, providing connectivity between Wellington Street and the Ottawa River Pathway. These cycle tracks will connect to the cycling facilities being provided across the Chaudière Crossing as part of the Zibi development, which in turn will connect to Gatineau and NCC cycling facilities on the Quebec side of the Ottawa River.
- Uni-directional cycle tracks on Bay Street, from Wellington Street to Laurier Avenue, providing connectivity between Wellington Street and the Laurier Avenue bike lanes.
- A segregated bike facility on Wellington Street providing connectivity between Portage Bridge and Mackenzie Avenue.
- Uni-directional cycle tracks along Albert as part of various improvement projects along Albert and Slater, extending from Empress Avenue in the west to the MacKenzie King Bridge in the east.
- A multi-use pathway across the Chief William Commanda Bridge (formerly the Prince of Wales Bridge), as part of the Chief William Commanda Bridge rehabilitation project.



#### **Transit Projects**

With the completion of Ottawa's Confederation LRT line in 2019, there are no proposed or ongoing transit projects within the vicinity of the site identified in the City of Ottawa's Transportation Master Plan (TMP). In the coming years, the transit only lanes along Albert Street that were used for transit detours during LRT construction will be decommissioned and returned to general traffic use/active modes. New roadway cross-sections are being proposed as part of a new functional design for the Albert Street corridor. Construction work for the Stage 2 LRT extension of the Confederation Line is ongoing at the time of this study; while no construction on Stage 2 is located within the study area, the extension of the line will increase the usage of the Confederation Line, which bisects the LeBreton Flats site.

The City of Gatineau has recently released plans for a tramway connecting the growing area of Aylmer to downtown Ottawa, including potential connections to the Confederation Line. The system would traverse the Portage Bridge into Ottawa, likely replacing the existing bus-only lanes on the Portage Bridge. The tramway would terminate near Elgin Street, with an alignment either along Wellington Street or a tunnel under Sparks Street. The City of Ottawa has shown a preference for the Sparks Street alignment, while the NCC has shown a preference for the Wellington Street alignment. The closest the West Gatineau Tramway would be to LeBreton Flats is at the intersection of Wellington/Portage Bridge, which is approximately 270m from the northeast corner of the development site. There are currently no projections for OC Transpo and STO ridership changes, although it can be expected that there may be fewer trips on bus routes crossing into Gatineau on Booth Street, such as OC Transpo Route 85. Additionally, there have been recent indications from the NCC that a "Downtown Transit Loop" be implemented, connecting the downtowns of Ottawa and Gatineau. **Figure 11** below shows all existing and proposed rapid transit networks in the downtown area.

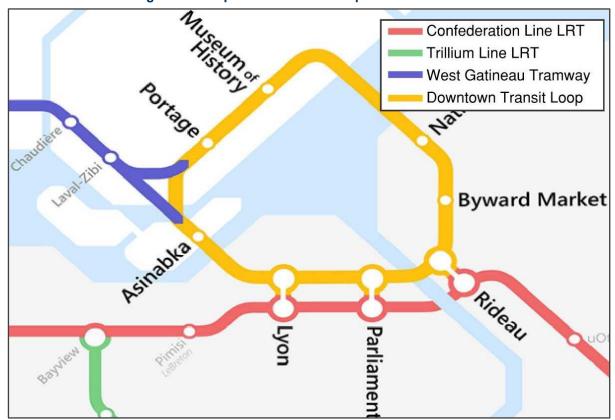


Figure 11: Proposed Downtown Rapid Transit Network



#### **Road Projects**

Referencing the City of Ottawa's Construction and Infrastructure Projects website, construction is anticipated to impact the following roadways within the study area. These construction projects may relate to road resurfacing, watermains, sewers, multi-use pathways, and bike facilities, which are all opportunities to change roadway characteristics/functionality:

- This year (2022-2023)
  - Re-alignment of Albert Street and Slater Street, between Empress Avenue to Bay Street, as well as construction on Queen Street
  - Wellington Street resurfacing, from Booth to O'Connor
  - Bay Street cycling facilities
  - Scott Street Transitway renewal, from Empress to Bayview Station Road
- 2-5 years
  - Scott Street streetscaping, from Empress Avenue to Bayview Station
  - Road, sewer and water on City Centre Avenue, and Elm Street between Albert and Preston
  - Albert Street streetscaping, from Booth Street to City Centre Avenue
- 5+ years (or construction start yet to be determined)
  - Albert Street and Slater Street, Bay Street to Elgin Street

## Other Area Development

Planned developments within the study area have been identified using the City's Development Application Search Tool. The following **Table 3** below summarizes planned and active developments within the vicinity of the subject development lands.

**Table 3: Area Development** 

Location	Description	Size	Туре
3-4 Booth	Zibi Project, Chaudière and Albert Islands Redevelopment	(Ottawa Sector) - 1,202 condo units - 51,954 ft² retail - 184,045 ft² office - 160 suite hotel	Mixed-use community
133 Booth	East LeBreton Flats Redevelopment	<ul> <li>592 residential units</li> <li>5,190 ft<sup>2</sup> daycare</li> <li>3,265 ft<sup>2</sup> ground floor commercial</li> </ul>	Mixed-use community
900 Albert	Three high-rise residential buildings with commercial	<ul> <li>- 1,232 condo units</li> <li>- 150 suite hotel</li> <li>- 128,370 ft² retail</li> <li>- 197,324 ft² office</li> </ul>	Mixed-use residential buildings

It should be noted that the projected impact of the developments summarized in Table 3 are included in the subsequent analysis.



# 3.2 Study Area and Time Periods

#### 3.2.1 Study Area

The following study area intersections were agreed to be assessed through discussions with City staff:

- Portage Bridge/Wellington
- Booth/Chaudière
- Booth/Wellington
- Booth/Albert
- Booth/War Museum
- Albert/Preston
- Vimy/Wellington
- Slidell/ Sir John A. Macdonald Parkway
- Bayview Station/Albert
- Albert/City Centre
- Parkdale/Scott

The defined study area is considered to be relatively large and should capture the majority of the projected traffic generated by the proposed development lands. Traffic impacts outside the defined study area should be relatively small. However, shifts in demand may occur outside of the study area due to the currently saturated road network. Such changes in travel behaviour may be captured by the City's regional macroscopic transportation demand model, which is currently being updated to help assess future infrastructure needs.

#### 3.2.2 Time Periods

Given the surrounding road network (e.g., Albert Street, Wellington Street) typically experience the heaviest traffic volumes during the weekday morning and afternoon peak hours, this assessment considered weekday morning and afternoon peak hours for analysis purposes only.

#### 3.2.3 Horizon Years

As noted in the TIA Guidelines, when a development will proceed in phases, TIA analysis must be completed for each development phase. Due to the scope of the development, it is difficult to select an exact year for full build-out of each phase, however through discussions with the NCC and O2 Planning + Design (consultants of the Master Concept Plan), the following horizons were agreed to for assessment.

2030: Phase 1 build-out2040: Phase 2 build-out

2050: Phase 3 build-out

It is noted in the TIA Guidelines that a "build-out plus five years horizon" is also required. It is also noted that the City may waive the need to analyze a "build-out plus five years horizon". It is proposed here that due to the numerous stages to this development along with the lack of background traffic growth, that no "build-out plus five years horizon" be required.



# 3.3 Exemptions Review

Given the size and nature of the proposed development lands, and following discussion with City Staff, the following TIA analysis modules have been exempted from this TIA analysis: Modules 4.1, 4.2, 4.3 and 4.4. It is our understanding that the City will request that these modules be included in future development applications for individual parcels of land. The following **Table 4** summarizes the modules that were considered for exemption.

**Table 4: Module Exemption Review** 

		•	
Module	Element	Exemption Criteria	Exemption Status
Design Review			
4.1 Development	4.1.2 Circulation and Access	Required for Site Plans	Exempt
Design	4.1.3 New Street Network	Required for Plans of Subdivision	Exempt
	4.2.1 Parking Supply	Required for Site Plans	Exempt
4.2 Parking	4.2.2 Spillover Parking	Required for Site Plans where parking supply will be 15% below unconstrained demand	Exempt
4.3 Boundary Streets	Mobility Road Safety Neighbourhood Traffic Management		Exempt
4.4 Access Intersections	4.4.1 Location and Design of Access 4.4.2 Intersection Control 4.4.3 Intersection Design	Exempt through discussions with City has noted above.	Exempt
Network Impact			
4.5 Transportation Demand Management	All Elements	Not required for non-residential Site Plans expected to have fewer than 60 employees and/or students on location at any given time	Not Exempt
4.6 Neighbourhood Traffic Management	All Elements	Required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Not Exempt
4.8 Network Concept	All Elements	Required when development is projected to generate more than 200 person-trips during the peak hour, in excess of the equivalent volume permitted by the established zoning	Not Exempt



# 4. STEP 3 – FORECASTING

# 4.1 Development-Generated Travel Demand

### 4.1.1 Land Use Scenarios

As previously described, the Master Concept Plan of LeBreton Flats is planned to include a mix of high-density residential, office, retail and hotel type land uses, as well as approximately 12.7 hectares of parks and open spaces. It should also be noted that the current Master Concept Plan includes an option to host a major event centre. Based on a land use planning exercise by O2 Planning + Design, four potential development scenarios were envisioned for LeBreton Flats – all of which are summarized in **Table 5** below.

**Table 5: Potential Development Scenarios** 

Scenario	Description	Townhome (units)	Mid- Rise (units)	High-Rise (units)	Retail (ft²)	Office (ft²)	Hotel (ft²)
1	Major Event Centre & Mixed-use	379	1,076	2,626	195,382	523,126	216,418
2	Mixed-use Only	473	1,242	2,735	183,617	535,483	85,638
3	Major Event Centre & Predominantly Office	95	1,174	3,069	194,866	949,378	230,950
4	Major Event Centre & Highest Density	301	1397	2419	261,035	508,734	154,419

Of the four scenarios listed above, Scenario 4 has the highest density and therefore, is considered to exhibit the highest potential trip generation. As such, the **subsequent analysis will only consider Scenario 4**, **as it reflects the "worst case scenario" from a trip generation perspective**. That being said, Scenario 4 is considered to be the most ambitious development scenario.

For analysis purposes, the Master Concept Plan has been assumed to be built-out in the following phases:



### Phase 1: Early Stages (approximately 2022-2030)

#### **Land Sales + Development**

- Albert District (east of Booth Street, North of Albert Street) {parcels A9, A10}
- Flats North (+associated new streets/lanes) {parcels F1, F2, F3, F8}
- Albert District West: Major events centre development (major event centre site)
   {parcels A1, A2, A3, A4}
  - o If no major event centre is developed (or other special uses): NCC will proceed with Alternate Site option (+associated streets)

### Infrastructure & Open Space Investments

- Cave Creek Sewer
- Urban Playground
- Inlet area
- Ph1. City Park (East)
- Preston (between Albert & LRT)
- Preston Pedestrian/Bike Bridge
- Connecting pathway to Bayview station
- Potential for limited improvements for interim uses in the Aqueduct District

### Phase 2: Middle Stages (2030-2040)

### Land Sales + Development

- Flats South (+associated streets) {F4, F9, F10, F11, F12}
- Albert District West, between Preston and Broad (+associated streets) (A5, A6)

### **Infrastructure & Open Space Investments**

- Covered Aqueduct enhancements
- Aqueduct District Open Spaces
- Ph2. City Park (west)
- Capital Park
- City Centre Pedestrian/Bike Bridge

### Phase 3: Later Stages (2040+)

### Land Sales + Development

- Aqueduct District (AD1, AD2, AD3, AD4, AD5)
- Albert District East (south of Albert Street) {A11, A12}
- Albert District West, between Broad and Booth (+associated streets) (A7, A8)
- Flats (Pindigen Park Site) {F5, F6, F7}

The following **Table 6** summarizes the size and type of land uses for each development block per planned phase of development for Scenario 4.



Table 6: Scenario 4 Land Use Build-out by Block/Phase

				nd Use		
Block	Low- Rise Housing (units)	Mid-Rise Housing (units)	High- Rise Housing (units)	Shopping Center (ft²)	General Office (ft²)	Hotel (ft²)
Phase 1 (2022-2030)						
A1	-	120	364	14,951	-	-
A2-4 (Major Event Centre)	-	-	257	25,510	-	101,719 (201 rooms)
A9	-	59	128	10,333	-	-
A10	-	114	256	20,333	-	-
F1	74	-	-	-	-	-
F2	56	-	-	-	1	-
F3	76	-	-	-	-	-
F8	-	86	-	9,515	-	52,700 (104 rooms)
Phase 1 Total	206	379	1005	80,643	-	154,419 (305 rooms)
Phase 2 (2030-2040)						
A5	10	132	81	-	-	-
A6	10	145	122	9,020	-	-
A11	0	178	61	11,259	-	-
A12	0	60	41	6,458	-	-
F9	14	81	94	5,533	-	-
F10	14	86	135	5,877	-	-
Phase 2 Total	48	682	534	38,147	ı	-
Phase 3 (2040+)						
A7	-	-	150	21,905	144,139	-
A8	-	-	230	30,257	117,563	-
F4	47	-	-	-	-	-
F5	1	56	95	-	-	-
F6	-	49	68	-	-	-
F7	-	55	-	-	-	-
F11	-	110	175	30,785	-	-
F12	-	66	162	19,289	-	-
AD1-5	-	-	-	40,009	247,032	-
Phase 3 Total	47	336	880	142,245	508,734	-
Total	301	1397	2419	261,035	508,734	154,419 (305 rooms)

# 4.1.2 Trip Generation

For the purpose of this assessment, projected residential site-generated traffic was estimated using the City of Ottawa TRANS Trip Generation Manual (2020). Projected retail, office and hotel traffic was estimated using the trip generation rates from the 10th Edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual. This method of predicting trip generation is considered industry best practice, is the method required as part of a formal Traffic Impact Assessment Study for the City of Ottawa, and is the method agreed to specifically for this project through discussions with the City of Ottawa.



For the purposes of this TIA analysis, it is worth noting that Table 6 above reflects that the major events centre is not currently anticipated to be constructed, with those parcels (A2-4) being assumed to consist of high-rise residential units and commercial space. If a major events centre were to be constructed on-site in the future, it is anticipated that the majority of trips generated by special events will occur outside commuter peak hours (when spare network capacity is available). For example, the Ottawa Senators 2019-2020 regular season schedule included only 25 home games that are scheduled during the week, and game start normally occurs at 7:30PM (i.e., outside normal commuter peak hours). In addition, other special events hosted at major event centres (e.g., concerts, other sports events, etc.) are often scheduled on a weekend and/or outside regular commuter peak hours, when spare network capacity is available. However, OC Transpo has noted that a major event centre with nearby on-site amenities has the potential to draw trips to site during the PM peak hour through pre-event activities. This is based on observations from events at Lansdowne Park, where there tend to be pre-event activities and PM peak hour generated transit trips associated with said activities. It should also be noted that if a major event centre is constructed within LeBreton Flats, it is anticipated that trips made to/from special events will be predominately made by transit or active modes, as is the case with urban major event centres located in the urban cores of other major cities. There will still be traffic impacts, but they will be significantly mitigated by the proximity to transit and connections to the surrounding network for active modes.

Similar to Ottawa's Lansdowne Park and downtown major event centres in other cities, it is anticipated that an aggressive Transportation Demand Management (TDM) plan will be implemented for any special event scheduled at LeBreton Flats (e.g., transit passes included with ticket purchase, offsite park and ride shuttle service, free secure bike storage/valet, etc.), which will mitigate the reliance on the private automobile. Therefore, due to the expected focus on TDM measures and concentration of trips outside of peak hours, for the purpose of this assessment, site trip generation for a major event centre was not considered. However, it should be noted that if a development application is submitted for the construction of a major event centre (or any other type of development) within LeBreton Flats, a formal TIA will be prepared for each development application (e.g. Site Plan, Plan of Subdivision, Zoning, etc.), which will include a TDM plan that will outline commitments to strategies that will reduce the reliance on the private automobile, as well as provide trip generation calculations for trips related to the events centre, including trips generated by pre-event activities nearby.

Based on the foregoing and the information provided, the following **Table 7** summarizes appropriate vehicle trip generation rates for estimating projected site-generated traffic by land use. It should be noted that the first listed equation is an average person trip generation rate, and the second equation is a "line of best fit" equation that more accurately represents the trend of person trip generation based on land use size. Typical industry practice is to use the "line of best fit" equation for site-generated traffic projections, if available.



**Table 7: Trip Generation Rates** 

Land Use	Land Use Code (TRANS / ITE)	AM Peak Hour	PM Peak Hour
Low-Rise Multi-family Housing (X = Units)	TRANS Multi-Unit (Low-Rise)	T = 0.68(X)	T = 0.70(X)
Mid-Rise Multi-family Housing (X = Dwelling Units)	TRANS Multi-Unit (High-Rise)	T = 0.4(X)	T = 0.4(X)
High-Rise Multi-family Housing (X = Dwelling Units)	TRANS Multi-Unit (High-Rise)	1 - 0.4(^)	1 - 0.4(A)
Shopping Center (X = 1,000 ft <sup>2</sup> GFA)	ITE 820 General Urban/Suburban	T = 5.03(X); or $Ln(T) = 0.86(X) + 2.53$	T = 7.49(X); or $Ln(T) = 0.66(X) + 4.04$
General Office Building (X = 1,000 ft <sup>2</sup> GFA)	ITE 710 General Urban/Suburban	T = 1.25(X); or T = 1.23(X) + 6.01	T = 1.35(X); or T = 1.32(X) + 6.07
Hotel (X = Rooms)	ITE 310 General Urban/Suburban	T = 0.47(X); or $T = 0.5(X) - 5.34$	T = 0.60(X); or T = 0.75(X) - 26.02

Note: T = Average Person Trip Ends

With respect to TRANS residential trip generation rates, the TRANS Trip Generation Manual provides a person trip rate for the AM and PM peak periods. Adjustment factors are also provided in the TRANS Trip Generation Manual to convert the person peak period trip rates into vehicular, transit, cycling and walking peak hour trip rates.

With respect to ITE Trip Generation rates, the data used to develop these rates in the 10<sup>th</sup> Edition of the Trip Generation Manual provides person trips for certain development types, including Shopping Center (ITE Land Use Code 820) and General Office Building (ITE Land Use Code 710). These person trips were calculated for each land use, and then broken down into trips for different modes (vehicle, transit, cycling and walking) by using the mode split agreed upon with the City for this development (refer to Travel Mode Shares below).

The Hotel Land Use Code (ITE Code 310) only includes vehicular trip generation, with the data collection surveys used to develop the trip generation typically conducted in highly suburban locations with limited access to transit and dedicated non-motorized facilities (e.g., sidewalks, bike lanes, etc. are generally limited). To properly consider the multi-modal trips generated by the Hotel land use, projected site-generated traffic (estimated using ITE trip generation rates) is converted to projected site-generated person trips. To convert projected ITE vehicle trips to person trips, an auto occupancy factor and non-auto trip factor is applied to the ITE trip generation rates. According to the City's TIA Guidelines, and based on available American Census data, the typical modal share of non-auto person trips is approximately 10% and the typical auto occupancy is 1.15. When combined/solving for "person trips" (i.e., Persons = 1.15xAutos + 0.10xPersons), a factor of 1.28 is used to convert vehicle trips to person trips. These person trips are then broken down into trips for different modes (vehicle, transit, cycling and walking) by using the mode split agreed upon with the City for this development (refer to Travel Mode Shares below).



The following **Table 8** and **Table 9** summarizes the resulting projected two-way person site trip generation for each phase of development, by development block and by land use type, respectively.

Table 8: Projected Site Person Trip Generation by Block / Parcel

Disale	А	M Peak Ho	ur	Р	M Peak Hou	ur
Block	In	Out	Total	ln	Out	Total
Phase 1 (2022-2030)						
A1	130	193	323	282	250	532
A2-4 (Major Event Centre)	214	214	428	394	350	744
A9	74	95	169	176	164	340
A10	136	179	315	293	269	562
F1	15	35	50	29	22	51
F2	11	27	38	22	17	39
F3	15	36	51	30	23	53
F8	93	88	181	185	167	352
Phase 1 Total	688	867	1555	1411	1262	2673
Phase 2 (2030-2040)						
A5	28	64	92	53	38	91
A6	80	117	197	187	169	356
A11	84	112	196	196	180	376
A12	45	57	102	121	115	236
F9	55	79	134	134	121	255
F10	62	93	155	149	132	281
Phase 2 Total	354	522	876	840	755	1595
Phase 3 (2040+)						
A7	299	155	454	297	422	719
A8	310	199	509	362	464	826
F4	10	22	32	18	15	33
F5	19	41	60	35	25	60
F6	14	33	47	27	19	46
F7	7	15	22	13	9	22
F11	165	188	353	339	320	659
F12	114	137	251	253	239	492
AD1-5	460	191	651	392	595	987
Phase 3 Total	1398	981	2379	1736	2108	3844
Total Person Trips	2440	2370	4810	3987	4125	8112



Table 9: Projected Site Person Trip Generation by Land Use Type

Dlack	Α	M Peak Ho	ur	Р	M Peak Hou	ır
Block	ln	Out	Total	In	Out	Total
Phase 1 (2022-2030)						
Residential	213	480	693	400	291	691
Retail	368	312	680	878	878	1756
Office	0	0	0	0	0	0
Hotel	107	75	182	133	93	226
Phase 1 Total	688	867	1555	1411	1262	2673
Phase 2 (2030-2040)						
Residential	160	357	517	299	214	513
Retail	194	165	359	541	541	1082
Office	0	0	0	0	0	0
Hotel	0	0	0	0	0	0
Phase 2 Total	354	522	876	840	755	1595
Phase 3 (2040+)						
Residential	162	356	518	298	216	514
Retail	600	513	1113	1287	1287	2574
Office	636	112	748	151	605	756
Hotel	0	0	0	0	0	0
Phase 3 Total	1398	981	2379	1736	2108	3844
Total Person Trips	2440	2370	4810	3987	4125	8112

As shown in Tables 8 and 9, the full build-out of LeBreton Flats is ultimately projected to generate an approximate two-way total of 4,810 and 8,110 person trips per hour during weekday morning and afternoon peak hours, respectively.

It should be noted that a percentage of projected site-generated trips can be attributed to 'pass-by' traffic (i.e., a quick stopover at LeBreton Flats on someone's normal daily commute), which does not impact overall network capacity, as a 'pass-by' trip is traffic already using the adjacent transportation network. Additionally, a percentage of projected site-generated trips could theoretically be further reduced, as a certain percentage of trips will be 'internal' trips (i.e., originate from and be destined to LeBreton Flats, such as individuals who live, work and shop all within LeBreton Flats). A high-level estimate of internal trip capture rate was calculated using the methodology outlined in the *National Cooperative Highway Research Program (NCHRP) Report 684 – Enhancing Internal Trip Capture Estimate for Mixed-Use Developments*. The calculation showed that a person trip reduction for LeBreton Flats due to internal capture could range from 5% to 13% (approximately 260 to 1060 trips in the peak hours).

Given that these potential reductions to projected site-generated trips will largely impact walking/cycling trips (because these are the likely mode choices for internal trips at LeBreton Flats), these reductions were not considered in the subsequent analysis, in order to provide a conservative estimate in this higher-level study. It is recommended that future TIAs for individual parcels of land take into account internal trip generation for their site-specific studies.

### 4.1.3 Travel Mode Shares

In order to determine the number of person trips arriving/departing by travel mode, total projected person trips are subdivided by mode share values, derived from the 2011 TRANS National Capital Region (NCR) Origin-Destination (OD) survey data, the nature/context of the proposed development and local area knowledge. Key factors that are taken into consideration, beyond NCR OD survey data, include: proximity and quality of transit, pedestrian and cycling facilities, purpose of trips, etc.



Based on discussions with City Staff and remaining consistent with assumptions used for TIA studies prepared for other area development sites, such as 900 Albert Street, the Zibi development, and Wateridge Village. LeBreton Flats is considered to be a Transit Oriented Development (TOD) site, given its proximity/connectivity to the highest order transit service. As such, the following summarizes the projected modal split of site-generated traffic for the subject development:

• 15% Auto Driver

5% Auto Passenger

• 60% Transit

20% Walking and Cycling

Based on the foregoing, the resulting projected vehicle, transit, and active transportation trips generated by the proposed development are summarized in **Table 10**, **Table 11**, and **Table 12**, respectively. It is worth noting that the actual transit mode share will differ by parcel, depending on the distance from the LRT station. However, for simplicity a blended mode share was carried for the entire site.

**Table 10: Projected Site Vehicle Trip Generation** 

Disale	А	M Peak Hou	ur	Р	M Peak Ho	ur
Block	In	Out	Total	ln	Out	Total
Phase 1 (2022-2030)						
A1	20	29	48	43	38	80
A2-4 (Major Event Centre)	33	32	64	59	52	111
A9	11	14	25	26	25	51
A10	21	27	47	44	40	84
F1	2	5	7	4	3	7
F2	2	4	6	3	3	6
F3	2	5	7	5	3	8
F8	14	13	27	28	25	53
Phase 1 Total	105	129	231	212	189	400
Phase 2 (2030-2040)						
A5	4	10	14	8	6	14
A6	12	18	29	28	25	54
A11	12	17	29	29	27	56
A12	7	8	15	18	18	35
F9	9	12	20	20	18	38
F10	10	14	24	23	20	43
Phase 2 Total	54	79	131	126	114	240
Phase 3 (2040+)						
A7	45	23	69	45	64	108
A8	46	29	75	55	70	124
F4	2	3	5	3	2	5
F5	3 2	6	9	5	4	9
F6	2	5	7	4	3	7
F7	1	2	3	2	1	3
F11	24	29	53	51	48	99
F12	17	20	37	38	36	74
AD1-5	69	29	98	59	89	148
Phase 3 Total	209	146	356	262	317	577
Total 'New' Vehicle Trips	368	354	718	600	620	1217



As shown in Table 10, the full build-out of LeBreton Flats is projected to generate approximate two-way vehicle volumes of 720 veh/h and 1,220 veh/h during weekday morning and afternoon peak hours, respectively.

**Table 11: Projected Site Transit Trip Generation** 

Dlask	Al	M Peak Hou	ır	Р	M Peak Ho	ur
Block	In	Out	Total	ln	Out	Total
Phase 1 (2022-2030)						
A1	78	115	193	169	150	319
A2-4 (Major Event Centre)	128	129	257	229	218	446
A9	45	57	101	106	99	205
A10	82	107	189	176	162	338
F1	9	21	30	17	13	30
F2	7	16	23	13	10	23
F3	9	22	31	18	14	32
F8	56	53	109	108	104	211
Phase 1 Total	414	520	933	836	770	1604
Phase 2 (2030-2040)						
A5	17	38	55	32	23	55
A6	48	70	118	112	101	213
A11	50	68	118	118	108	225
A12	27	34	61	73	69	142
F9	33	47	80	81	73	154
F10	38	56	94	89	79	168
Phase 2 Total	213	313	526	505	453	957
Phase 3 (2040+)						
A7	179	93	272	178	254	432
A8	185	119	305	217	279	496
F4	6	13	19	11	9	20
F5	11	25	36	21	15	36
F6	8	20	28	16	11	27
F7	4	9	13	8	5	13
F11	99	113	212	204	192	396
F12	69	82	151	152	144	295
AD1-5	276	115	391	235	357	592
Phase 3 Total	837	589	1427	1042	1266	2307
Total 'New' Transit Trips	1464	1422	2886	2383	2489	4868

As shown in Table 11, the full build-out of LeBreton Flats is projected to generate approximate two-way transit trip volumes of 2,890 trips/h and 4,870 trips/h during weekday morning and afternoon peak hours, respectively.



**Table 12: Projected Site Active Trip Generation** 

	A	M Peak Hou	ır	Р	M Peak Ho	ur
Block	In	Out	Total	ln	Out	Total
Phase 1 (2022-2030)						
A1	27	39	66	58	51	109
A2-4 (Major Event Centre)	45	43	88	76	73	149
A9	16	19	35	36	34	70
A10	29	37	66	59	54	113
F1	3	7	10	6	5	11
F2	3	6	9	5	4	9
F3	3	7	10	7	5	12
F8	19	18	37	36	35	71
Phase 1 Total	145	176	321	283	261	544
Phase 2 (2030-2040)						
A5	6	14	20	11	8	19
A6	16	24	40	38	34	72
A11	17	23	40	39	36	75
A12	10	11	21	25	24	49
F9	13	16	29	27	24	51
F10	14	19	33	31	27	58
Phase 2 Total	76	107	183	171	153	324
Phase 3 (2040+)						
A7	60	32	92	60	86	146
A8	62	39	101	74	94	168
F4	3	5	8	4	3	7
F5	4	9	13	7	6	13
F6	3	7	10	6	4	10
F7	2	3	5	3	2	5
F11	32	39	71	69	65	134
F12	23	28	51	51	48	99
AD1-5	92	39	131	78	118	196
Phase 3 Total	281	201	482	352	426	778
Total 'New' Active Trips	502	484	986	806	840	1646

As shown in Table 12, the full build-out of LeBreton Flats is projected to generate approximate two-way active trip volumes of 990 trips/h and 1,650 trips/h during weekday morning and afternoon peak hours, respectively.

It should be noted that given most transit trips begin or end as an active mode, it can be expected that approximately 3,880 trips/h and 6,520 trips/h will be made to/from/within LeBreton Flats as an active mode during weekday morning and afternoon peak hours, respectively. Given this relatively high projected volume of site-generated trips made by active modes, special consideration should be given to sidewalk/pathway capacity during design. Additional discussion on proposed roadway cross sections is provided in the subsequent **Step 4 - Analysis** section.



# 4.1.4 Trip Distribution and Assignment

The projected distribution of site-generated vehicular traffic was derived based on existing travel patterns, the site's connections to/from the surrounding road network, and local area knowledge. (e.g., the location and proximity of other employment areas, residential communities, entertainment, etc.). For analysis purposes, the following approximate distribution of projected site-generated traffic was assumed, which is consistent with data from the most recent 2011 TRANS Origin-Destination (OD) travel survey (i.e., "existing travel patterns"), consistent with the assumptions used for TIA studies prepared for other area development sites (e.g., Zibi, 900 Albert, etc.), and has been agreed to with the City of Ottawa for use in this study.

### **Departure**

- 60% to/from the east via Wellington Street and Albert Street
- 15% to/from the west via Sir John A. Macdonald Parkway and Albert Street
- 5% to/from the north via Chaudière Crossing and Portage Bridge
- 20% to/from the south via Booth Street and Preston Street.

#### **Arrival**

- 40% to/from the east via Wellington Street and Albert Street
- 15% to/from the west via Sir John A. Macdonald Parkway and Albert Street
- 10% to/from the north via Chaudière Crossing and Portage Bridge
- 35% to/from the south via Booth Street and Preston Street.

Based on the above assumed distribution, projected site-generated traffic was assigned to the study area network, which is depicted as the following **Figure 12** to **Figure 15**. Site traffic was assigned individually according to each development parcel; this was done to account for the unique situation for parcels north of the Confederation Line (i.e., Flats District), where the turning restrictions at Booth Street make access to these parcels difficult.

It should be noted that given size of the study area network and the number of study area intersections, each phase of site-generated traffic is subsequently depicted as two separate figures. The first figure for each phase depicts the assignment of site-generated traffic to the greater study area network, and the second figure for each phase depicts the assignment of site-generated traffic to site driveway connections and the immediate road network surrounding the subject development lands.



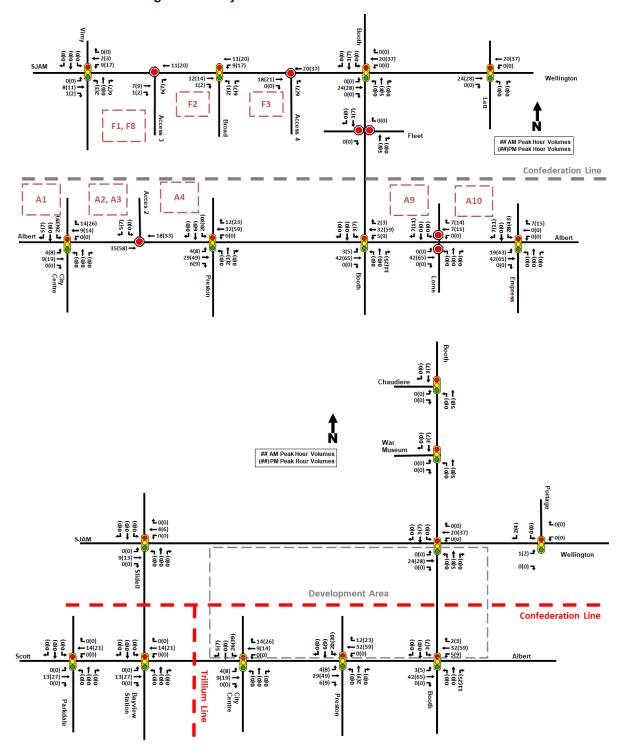


Figure 12: Projected Site-Generated Traffic - Phase 1



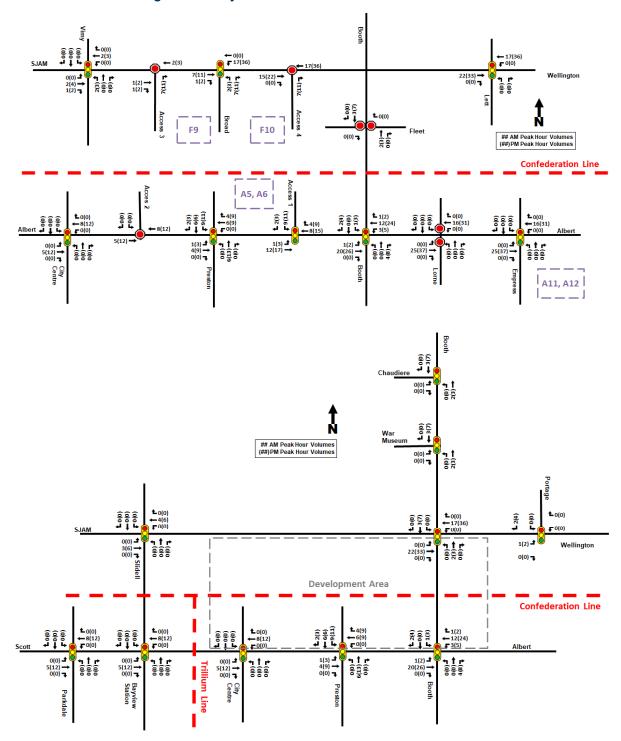


Figure 13: Projected Site-Generated Traffic - Phase 2



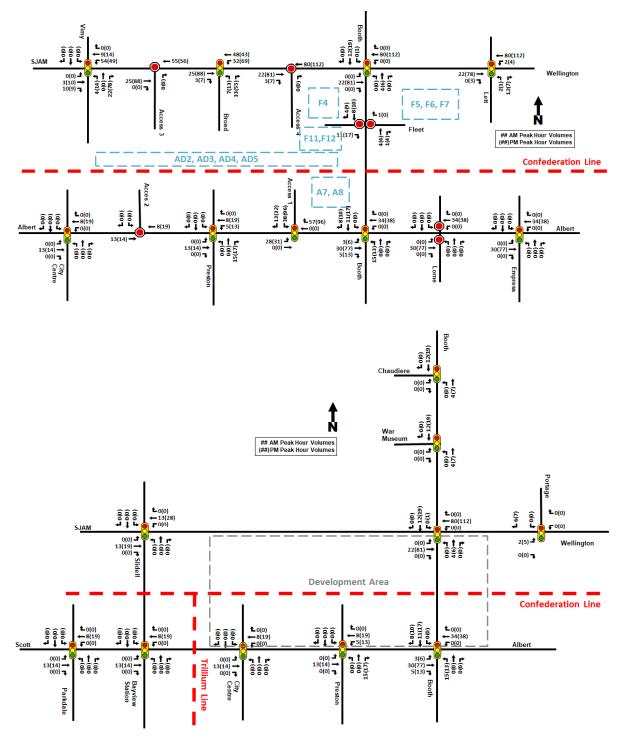


Figure 14: Projected Site-Generated Traffic - Phase 3



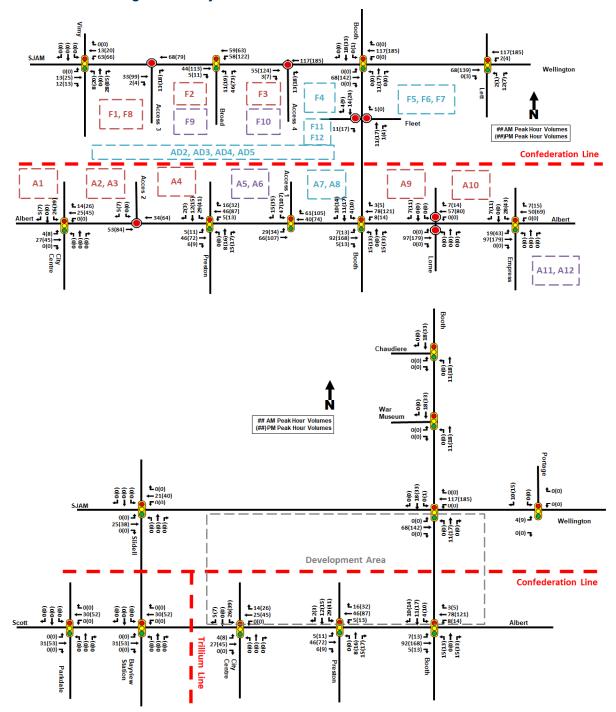


Figure 15: Projected Site-Generated Traffic - Full Build-Out



# 4.2 Background Network Travel Demands

### 4.2.1 Transportation Network Plans

As previously mentioned in Step 2 - Scoping, the current transit-only lanes along Albert Street will be decommissioned and returned to general traffic use and/or active modes. New roadway cross-sections are currently being considered by the City of Ottawa as part of a new functional design for the Albert Street corridor, which will be discussed in the subsequent Step 4 - Analysis section.

As part of the Zibi development, the vehicular capacity of the Booth-Eddy Street corridor has been reduced and has been rededicated to cycling/pedestrian facilities. This has resulted in a single vehicular travel lane in each direction across the Chaudière Crossing.

As part of the LeBreton Flats Master Concept Plan, it is proposed that a bridge dedicated to serving pedestrians and cyclists only be extended over the Confederation Line in the Preston Street corridor between Albert Street and the SJAM/Wellington Street. Although this link currently does not exist, it has been identified in the City's current and previous Transportation Master Plans as a new arterial roadway link to serve all travel modes.

As noted in Section 3.1.3, the City of Gatineau has released plans for a tramway connecting the growing area of Aylmer to downtown Ottawa, via the Portage Bridge. The closest the West Gatineau Tramway would be to LeBreton Flats is at the intersection of Wellington/Portage Bridge, which is approximately 270m from the northeast corner of the development site. Although there are currently no projections for OC Transpo and STO ridership changes, it can be expected that there may be fewer trips on bus routes crossing into Gatineau on Booth Street, such as OC Transpo Route 85.

With the exception of a new interprovincial bridge between Ottawa and Gatineau, these future transportation network plans have been included/assumed in the subsequent analysis.

### 4.2.2 Background Growth and Traffic Volume Balancing

Due to certain data gaps (i.e., not every study area intersection was counted during the year 2014), a volume balancing exercise was conducted, which resulted in the following modifications to peak hour vehicular volumes at study area intersections (note, the following negative values indicate veh/h that were removed, and positive values indicate veh/h that were added):

Booth/War Museum<sup>1</sup>: SB [-730(AM), -810(PM)]

Booth/Wellington: NB [-10(AM)]

Albert/Booth: SB [-90(AM)]; WB [+70(PM)]

Albert/Preston: EB (-20(PM)]

Albert/City Centre: WB [+10(PM)]

SJAM/Slidell: WB [-50(AM), -30(PM)]

Wellington/Vimy: EB [+20(AM)]; WB [-40(PM)]

Wellington/Lett: EB [+180(AM)]; WB [-130(AM), -70(PM)]

Wellington/Portage: EB [-140 (AM), -240(PM)]

<sup>&</sup>lt;sup>1</sup> It is noted that the turning movement count at Booth Street / War Museum on July 18, 2013, was flagged as an anomaly in the Zibi development 2014 TIS. Because of this, the Booth Street corridor was balanced according to the counts at its intersections with Wellington Street and Albert Street. The discrepancy between Booth Street / Wellington Street and Booth Street / War Museum was fully addressed in the adjustments at the Booth Street / War Museum intersection



Based on the foregoing volume balancing assumptions, **Figure 16** on the following page depicts the resulting baseline existing conditions.

As previously mentioned in Section 3.1.2, Ottawa's downtown arterial network is generally accepted to operate at capacity during peak hours; additionally, the City's TMP notes that the number of cars arriving downtown in the morning peak period has been decreasing since 1986. Therefore, background traffic volumes have exhibited negligible growth.

In addition to negligible background traffic growth, study area roadways have been impacted by the extended LRT related construction which have prompted some travelers to forego tips altogether, make different mode choices, take different travel routes, or change trip times to avoid increased congestion brought by detours. Therefore, and as agreed to by City Staff, historical traffic count data from the year 2014 (where available) was used for analysis purposes and zero background growth (i.e., background growth rate of 0%) was applied.

# 4.2.3 Current and Anticipated Area Developments

Using the City's online Development Application Tool, planned developments including 900 Albert, East Flats and Zibi were identified to have impacts on the study area. As such, the projected site-generated traffic from these developments was included in the subsequent analysis. Excerpts from the TIA study reports for 900 Albert, LeBreton East Flats and Zibi are included as **Appendix D**, depicting projected site-generated traffic for these developments. Trips generated by these developments were carried through all study area intersections for this report, regardless of where the study area terminated for each individual development.

Given that the TIA studies prepared for the identified area developments did not include some of the intersections located within the LeBreton study area, projected site-generated traffic from such area developments was appropriately distributed/assigned throughout the LeBreton study area as described in Step 2 - Scoping. The resulting assignment of projected site-generated traffic from other area developments is depicted in **Figure 17**, while **Figure 18** depicts the total background traffic volumes for this analysis, including existing conditions, background growth (0%) and traffic volumes from other area developments.



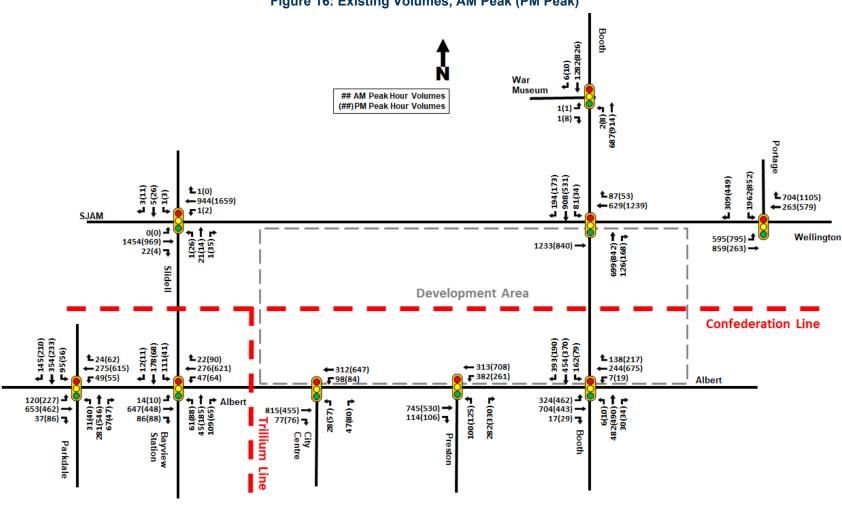


Figure 16: Existing Volumes, AM Peak (PM Peak)



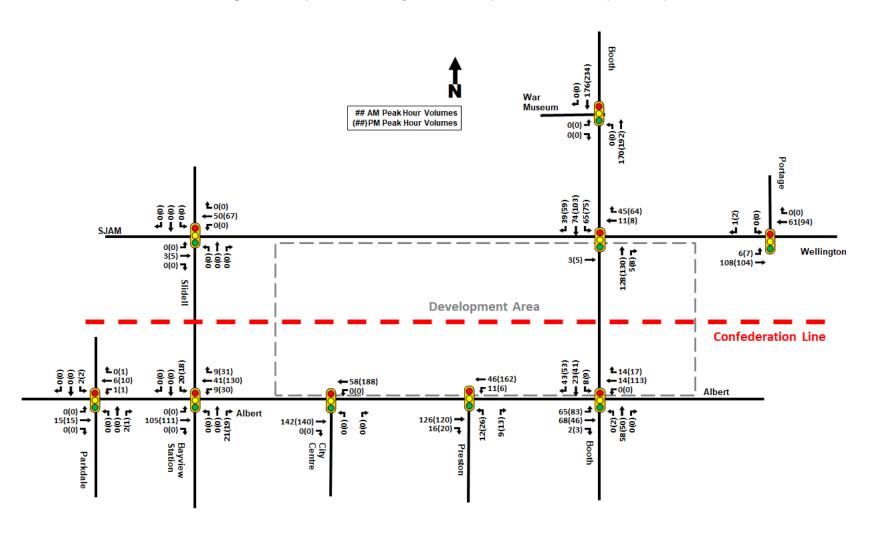


Figure 17: Trips Generated by Area Developments, AM Peak (PM Peak)



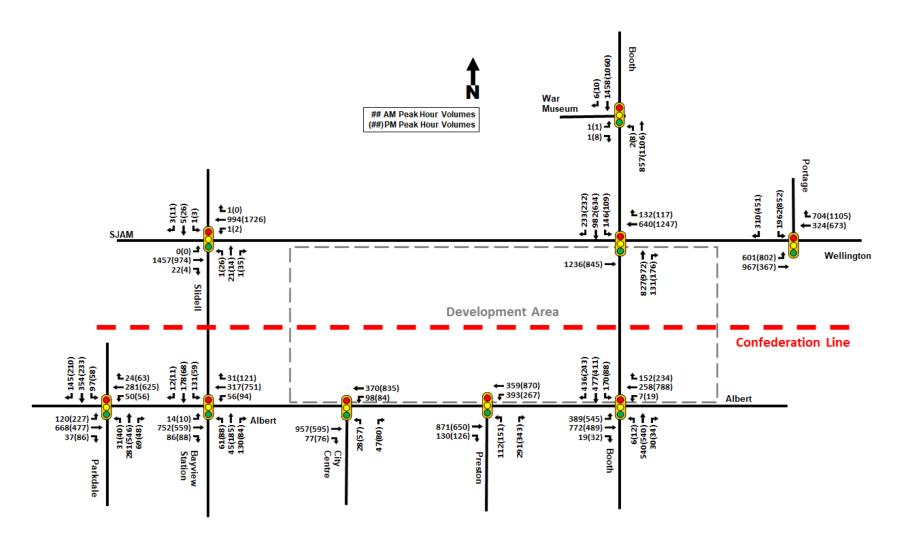


Figure 18: Future Background Turning Movement Volumes, AM Peak (PM Peak)



#### 4.3 Demand Rationalization

The following section summarizes the study area intersection capacity analysis for Existing, Future Background and Future Total Volume scenarios. For analysis purposes, the Existing Conditions scenario is considered to be 2022, the Future Background scenario is considered to be 2030 and the Future Total Volume scenario is 2030 for Phase 1, 2040 for Phase 2 and 2050 for Phase 3.

Using the intersection capacity analysis software Synchro (v10), study area intersections were assessed in terms of vehicle delay, volume-to-capacity ratio (v/c) and the corresponding Level of Service (LOS). It should be noted that the overall performance of a signalized intersection is calculated as a weighted v/c ratio and assigned a corresponding LOS, with critical movements assigned a LOS based on their respective v/c ratio. The overall performance of an unsignalized intersection is a LOS output from Synchro, which is based on an Intersection Capacity Utilization (ICU) method, and critical movements are assigned a LOS based on delay. **Table 13** shows the vehicular level of service that corresponds to each v/c ratio.

Level of Service	Volume to Capacity Ratio
А	0 to 0.60
В	0.61 to 0.70
С	0.71 to 0.80
D	0.81 to 0.90
E	0.91 to 1.00
F	> 1.00

Table 13: Level of Service vs. V/C Ratio

Given the number of study area intersections, the general proximity between intersections/driveways within the study area, the level of existing network saturation, and the level of variability with respect to developing a Master Concept Plan for a large area such as LeBreton Flats, estimated 95th percentile queues at study area intersections were not explicitly assessed as part of this TIA study report. For the purpose of this assessment, study area intersections with a LOS at, or over capacity (i.e., LOS 'E' or 'F') and long delays (i.e., delays greater than 35s), it is reasonable to conclude that 95th percentile queues are also problematic (e.g. problematic queues spill back and block driveways and/or adjacent intersections, extend beyond provided turn lane storage, etc.) and therefore, provide limited to no additional value for analysis/decision purposes.

The City of Ottawa follows a Multi-Modal Level of Service (MMLOS) policy, which evaluates all modes of transportation, including pedestrians, cyclists, transit, trucks, and vehicles. The MMLOS analysis allows for trade-offs between the different modes of transportation, prioritizing different modes depending on the location within the City. The City's MMLOS Guidelines define the LOS targets for each mode of transportation based on the Official Plan Designation / Policy Area, as well as roadway classification, cycling facilities, transit priority and truck route status. These targets are presented in **Table 14** for each major roadway within the study area.



Table 14: Minimum Desirable MMLOS Targets by Official Plan Policy/Designation & Road Classification (Source: City of Ottawa MMLOS Guidelines)

OP Designation (Roadway Characteristics)	Street	Pedestrian LOS	Bike LOS	Transit LOS	Truck LOS	Auto LOS
within 600m of Rapid Transit Station (Arterial, Cross-Town Bikeway, Truck Route)	Albert/Scott	Α	А	D	D	Е
within 600m of Rapid Transit Station (Arterial, Spine Route, Truck Route)	Booth	Α	С	D	D	Е
Central Area (Arterial, Spine Route, No Trucks)	Wellington	А	С	D	E	Е

Due to the central location of the study area, all roadways and intersections within the study area have a Pedestrian LOS (PLOS) target of LOS 'A', and an Auto LOS target of LOS 'E', indicating the focus on pedestrians. The cross-town bikeway along Albert Street and Scott Street requires that that corridor meet the Bike LOS (BLOS) target of LOS 'A', while Booth Street and Wellington Street both have BLOS targets of LOS 'C'. There are no plans for transit priority above and beyond isolated measures, therefore the Transit LOS (TLOS) target is LOS 'D' for all intersections. Wellington Street is the only major roadway not designated as a truck route; therefore, its Truck LOS (TrLOS) target is LOS 'E', with the rest of the study area targeting LOS 'D'.

### 4.3.1 Existing and Future Background Conditions

Based on existing volumes depicted in Figure 14 and existing signal timing plans provided by the City, the following **Table 15** summarizes the existing performance of study area intersections. Detailed Synchro output data for Existing and Future Background Conditions are provided in **Appendix E**.

**Table 15: Study Area Intersection Operations - Existing Conditions** 

		Overall			Critical M	ovement	
Intersections	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS
Booth & Chaudière	2 (1)	0.75 (0.54)	C (A)	SBTR	3 (1)	0.75 (0.49)	C (A)
Booth & War Museum	4 (5)	0.44 (0.31)	A (A)	SBTR	7 (5)	0.44 (0.29)	A (A)
Booth & Wellington	33 (31)	0.86 (0.85)	D (D)	EBT	34 (27)	0.90 (0.61)	D (B)
Booth & Albert	37 (40)	0.83 (0.80)	D (C)	EBL	28 (60)	0.66 (0.98)	B (E)
Albert & Empress	4 (4)	0.25 (0.33)	A (A)	WBLT	2 (2)	0.13 (0.33)	A (A)
Albert & Preston	29 (14)	0.86 (0.57)	D (A)	EBT	41 (15)	0.93 (0.51)	E (A)
Albert & City Centre	8 (9)	0.53 (0.49)	A (A)	EBT	8 (5)	0.58 (0.34)	A (A)
Albert/Scott & Bayview	16 (19)	0.61 (0.53)	B (A)	EBTR	8 (19)	0.65 (0.48)	B (A)
Scott & Parkdale	29 (56)	0.79 (0.95)	C (E)	WBT	15 ( <mark>126</mark> )	0.33 (1.18)	A (F)
SJAMP & Slidell	3 (7)	0.50 (0.69)	A (B)	WBT	3 (8)	0.34 (0.69)	A (B)
Wellington/SJAMP & Vimy	2 (3)	0.46 (0.51)	A (A)	WBT	2 (4)	0.30 (0.52)	A (A)
Wellington & Lett	15 (4)	0.52 (0.40)	A (A)	EBTR	20 (2)	0.54 (0.36)	A (A)
Wellington & Portage	111 (39)	1.21 (0.83)	<b>F</b> (D)	SBL	<b>230</b> (53)	1.44 (0.88)	<b>F</b> (D)



As shown in Table 15, the intersection of Wellington Street at Portage Bridge is operating over capacity in the weekday morning peak hour, with a LOS 'F'. This is mainly driven by the southbound left turn volume, which has a v/c ratio of 1.44. All other movements at this intersection operate with acceptable LOS. There is minimal opportunity for improvement in LOS for the southbound left turn movement, as reassigning green time from other conflicting movements is not possible as it either violates the minimum green time (i.e., pedestrian crossing time) or it results in further deterioration in overall intersection operations. When the future West Gatineau Tramway is in place across the Portage Bridge it may encourage a shift in mode of transportation to transit, reducing the volume of vehicles crossing the bridge from Gatineau and improving the LOS of the intersection.

The intersection of Scott Street at Parkdale Avenue is approaching capacity (LOS 'E') in the weekday afternoon peak hour. The westbound through movement has a v/c ratio of 1.18 in the PM peak hour. Changes along Scott Street to provide transit priority in curbside lanes has resulted in a single through lane in each direction at this intersection, increasing the v/c ratio for this movement. Optimization of the signal timing at this intersection would reduce the v/c ratio of the westbound through movement to 1.01, at the expense of the northbound through movement, which would increase in v/c ratio from 0.95 to 1.08.

The following **Table 16** summarizes the projected study area intersection performance based on Future Background volumes, assuming no significant changes to existing signal timing plans (i.e., slight tweaks to optimize phases, but not cycle lengths). Future Background volumes were derived by summing together existing traffic volumes and projected site-generated traffic from the other area developments (i.e., summing volumes together from Figure 14 and Figure 15, resulting in Figure 16). Given an annual background traffic growth rate was assumed to be zero and assuming other area development will be fully built-out by the year 2030, Table 16 summarizes the study area intersection performance for all the Future Background scenario.

Table 16: Study Area Intersection Operations – Future Background Conditions

Intersections	Overall			Critical Movement			
	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS
Booth & Chaudière	13 (12)	0.91 (0.76)	E (C)	SBTR	18 (12)	0.93 (0.74)	E (C)
Booth & War Museum	5 (6)	0.50 (0.38)	A (A)	SBTR	8 (6)	0.50 (0.37)	A (A)
Booth & Wellington	46 (38)	0.93 (0.95)	E (E)	NBTR	87 (50)	0.98 (1.01)	E(F)
Booth & Albert	42 (53)	0.92 (0.93)	E (E)	EBL	34 (139)	0.80 (1.22)	C(F)
Albert & Empress	1 (4)	0.26 (0.38)	A (A)	WBLT	2 (3)	0.14 (0.38)	A (A)
Albert & Preston	42 (20)	0.95 (0.72)	E (C)	EBT	63 (27)	1.03 (0.74)	<b>F</b> (C)
Albert & City Centre	10 (8)	0.63 (0.61)	B (B)	EBT	11 (6)	0.68 (0.44)	B (A)
Albert/Scott & Bayview	18 (17)	0.68 (0.61)	B (B)	EBTR	11 (12)	0.74 (0.57)	C (A)
Scott & Parkdale	30 (59)	0.80 (1.01)	C (F)	WBT	14 (134)	0.34 (1.20)	A (F)
SJAMP & Slidell	3 (8)	0.50 (0.72)	A (C)	WBT	3 (9)	0.36 (0.72)	A (C)
Wellington/SJAMP & Vimy	2 (3)	0.46 (0.54)	A (A)	WBT	2 (4)	0.32 (0.55)	A (A)
Wellington & Lett	21 (5)	0.59 (0.46)	A (A)	EBTR	28 (3)	0.61 (0.42)	B (A)
Wellington & Portage	109 (40)	1.21 (0.87)	<b>F</b> (D)	SBL	<b>232</b> (53)	1.45 (0.88)	<b>F</b> (D)



As expected, delays and v/c ratios increase within the study area due to an increase in future background traffic. The intersection of Wellington Street at Portage Bridge, which was over capacity in the morning peak hour for Existing Conditions, continues to be over capacity in the Future Background Conditions. The intersection of Scott Street at Parkdale Street is expected to be over capacity in the PM peak hour, with a v/c ratio of 1.01. The westbound through movement has increased to a v/c ratio of 1.20 in the PM peak hour, and the northbound through v/c ratio is at 0.95. As identified in the Existing Conditions, it is possible to optimize the signal timing at this intersection to improve the westbound through at the expense of the northbound through, however that optimization has not been undertaken here.

Numerous intersections that operated acceptably in the Existing Conditions are approaching capacity in the Future Background Conditions, including the intersections of Booth Street at Chaudière (AM peak), Booth Street at Wellington Street (AM and PM peaks), Booth Street at Albert Street (AM and PM peaks) and Albert Street at Preston Street (AM peak).

- The southbound through movement at Booth Street and Chaudière is the heaviest movement in the AM peak hour, approaching capacity with a v/c ratio of 0.93. With single lanes on each approach there are minimal opportunities to improve the operations of the southbound through movement at this intersection.
- The northbound through/right movement is the heaviest movement at Booth Street and Wellington Street in both peak hours: it is approaching capacity in the AM peak hour (v/c ratio of 0.98) and over capacity in the PM peak hour (v/c ratio of 1.01). The southbound left turn movement is also approaching capacity in the PM peak hour, with a v/c ratio of 0.99. Similar to the intersection of Scott Street at Parkdale Avenue, it is possible to optimize the signal timing to improve intersection operations, however it increases the delay on the heaviest volume movements (eastbound through in AM peak, westbound through in PM peak). Therefore, the optimization has not been included in this assessment, but it is recommended that this intersection be monitored moving forward.
- The eastbound through movement is approaching capacity in the AM peak hour at the intersection of Booth Street and Albert Street, with a v/c ratio of 0.98. This is mainly due to there only being a single eastbound through lane on this approach, whereas there are two through lanes for the opposite direction. It is our understanding that the City intends to redesignate the bus lanes on Albert Street as general traffic lanes, which may open up additional green time at the intersection for other movements and improving the LOS. Additionally, the eastbound left turn movement is over capacity in the PM peak hour at this intersection, with a v/c ratio of 1.22, due to the volume of vehicles making this movement approaching 550 vehicles per hour. Generally, the City recommends a second left turn lane be considered at 300 vehicles, however widening the intersection to accommodate a second left turn lane would run contradictory to the focus on active transportation for this corridor.
- The eastbound through movement is over capacity in the AM peak hour at the intersection of Albert Street at Preston Street, with a v/c ratio of 1.03. Similar to the eastbound approach at Booth Street and Albert Street, this is mainly due to the single eastbound through lane on this approach. Adjustments to the signal timing are possible to reduce the v/c ratio to 1.00, but at the expect of the westbound through movement, which would increase from a v/c ratio of 0.95 to a v/c ratio of 1.01.



# **Potential Mitigation Measures**

Notwithstanding the exemplary existing and planned measures to accommodate and promote active/sustainable modes of transportation within the study area, the following are potential measures to improve the performance of study area intersections operating at, or over capacity from a vehicular operations perspective only. In some cases, these potential mitigation measures may contradict with policy direction, decisions or investments in infrastructure, and should not be considered requirements as conditions of development approval unless otherwise stipulated by the City. Therefore, mitigation measures have been separated into two groups – a primary group of preferred measures that supports the City's TMP by improving conditions for all modes of transportations, and a secondary group of alternatives that improves operating conditions for vehicles only, with potential negative impacts on other modes of transportation.

It should be noted that although the network modifications listed below are all technically possible, they may not be feasible due to physical/economical constraints and/or they may not satisfy or support policy/political/planning objectives. Therefore, the possible measures to improve the performance of study area intersections are only provided for information/decision making purposes only. If any of these possible measures are deemed to be desirable, further analysis may be required to support their justification.

# Group A - Preferred Mitigation Measures

The following mitigation measures are the most preferred due to their prioritization for all modes of transportation, not just motorized modes. Further improvements to vehicular LOS may be observed as trips are shifted to alternative modes of transportation or alternative corridors as major projects within the National Capital Region are completed.

# **Transit Projects**

- When the future West Gatineau Tramway is in place across the Portage Bridge in 2028, trips across
  the Portage Bridge may be shifted away from the vehicular mode and towards the transit mode. It
  is recommended that the City monitor traffic volumes at the intersection of Wellington Street and
  Portage Bridge and respond to a reduction in vehicular traffic accordingly.
- Confederation Line Stage 2 LRT (with improved reliability extension drawing additional trips when open in 2026)

### **Active Mobility Projects**

Projects that may reduce the vehicular demand in the study area include:

- Improvements to cycling facilities throughout LeBreton Flats and along Albert Street into downtown (mode shift to cycling)
- The Chief William Commanda multi-use pathway interprovincial bridge (mode shift to cycling and walking)
- These measures would improve operations at most intersections in the study area, with a specific benefit to Booth Street at Albert Street, Albert Street at Preston Street and Parkdale Avenue at Scott Street.



# Group B – Alternative Mitigation Measures

The following mitigation measures are less preferred due to their prioritization for vehicular modes only, and not benefitting active transportation and transit modes.

### **Ottawa River Sixth Crossing**

• The NCC recently completed a Long-Term Integrated Crossings Plan; a potential future additional crossing of the Ottawa River may result in a vehicular shift to alternative corridors.

### **Wellington Street at Portage Bridge**

- Re-designate southbound through and westbound through HOV lanes for general purpose traffic.
   This would prioritize single occupancy vehicles over high occupancy vehicles, including buses and carpool vehicles, running contrary to the City's stated goals for reducing vehicular traffic.
- Widen both the Portage Bridge and Wellington Street to accommodate additional southbound and westbound through lanes. The cost would likely make this measure unfeasible, would potentially induce demand resulting in a shift from active modes to vehicles, and would restrict the ability to provide the West Gatineau Tramway connection across the Portage Bridge in the future.

#### **Booth Street at Albert Street**

- Re-designating the eastbound through bus lane as a general traffic lane would maintain the same roadway width and improve vehicular operations. However, the removal of transit priority on an arterial corridor in favour of vehicular lanes runs contrary to the City's stated goals of prioritizing transit over general traffic.
- Redesign Albert Street to accommodate dual eastbound left-turn lanes. This would increase the footprint of the intersection, forcing pedestrians and cyclists to cross a wider intersection and reducing their respective LOS.

#### **Preston Street at Albert Street**

Redesign Albert Street to accommodate dual westbound left-turn lanes and widen Preston Street
to accommodate dual receiving lanes. This would increase the footprint of the intersection, forcing
pedestrians and cyclists to cross a wider intersection and reducing their respective LOS.

#### Parkdale Avenue at Scott Street

• Widen Scott Street from two to four lanes (i.e., two travel lanes per direction for general purpose traffic). This would increase the footprint of the intersection, forcing pedestrians and cyclists to cross a wider intersection and reducing their respective LOS.

# 4.3.2 Total Projected Conditions

The following section summarizes the study area intersection capacity analysis for total projected volume scenarios for the 2030, 2040 and 2050 horizon years. Total projected volumes depicted in Figure 19, Figure 20 and Figure 21 were derived by superimposing LeBreton Flats site-generated traffic volumes onto projected background traffic volumes. It should be noted that given the size of the study area network and the number of study area intersections, each horizon year is subsequently depicted as two separate figures. The first figure for each horizon year depicts the total projected traffic volumes for site driveway connections / immediate road network surrounding the subject development lands and the second figure depicts the total projected traffic volumes for the greater study area network.



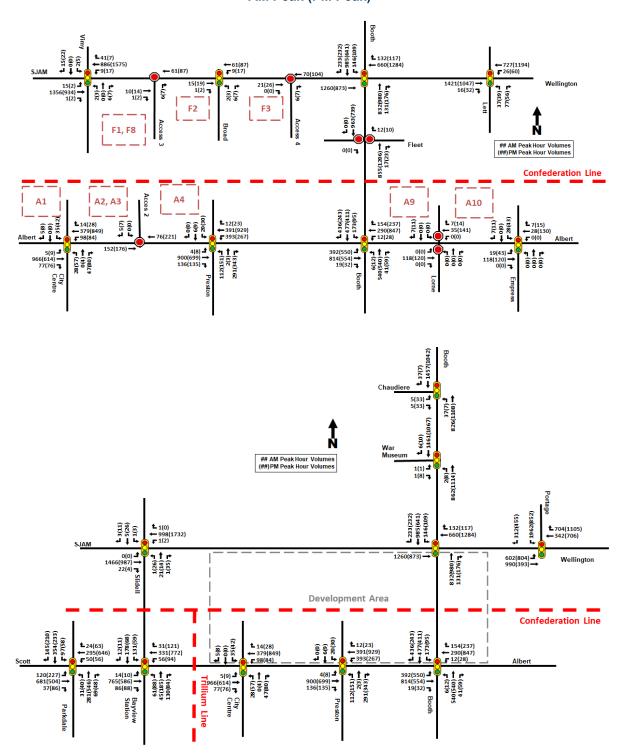


Figure 19: Total Projected Traffic Volumes – 2030 (Phase One),

AM Peak (PM Peak)



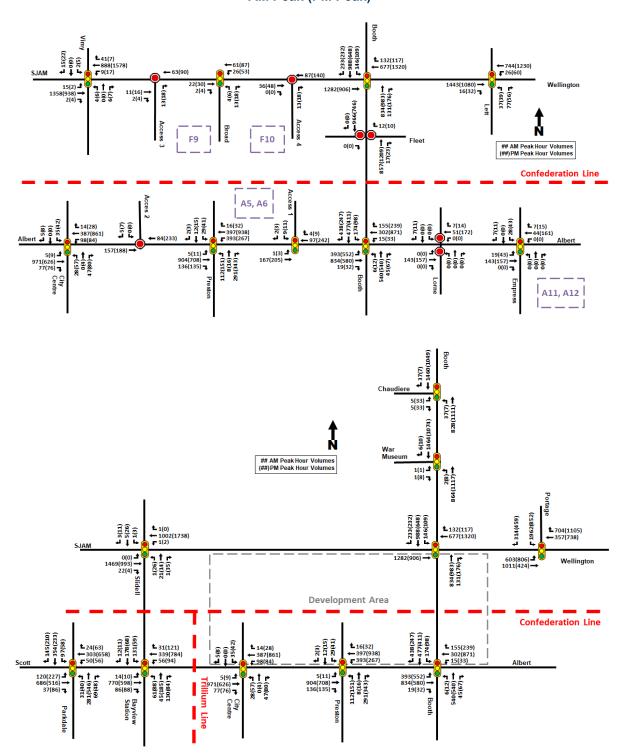


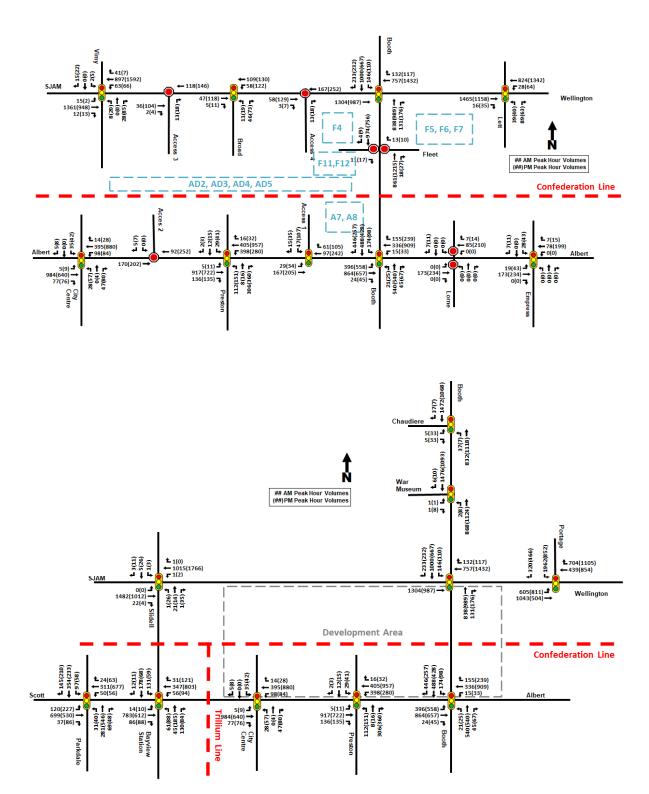
Figure 20: Total Projected Traffic Volumes – 2040 (Phase One & Two),

AM Peak (PM Peak)



Figure 21: Total Projected Traffic Volumes – 2050 (Phase One, Two & Three),

AM Peak (PM Peak)





Similar to existing baseline and background conditions, total projected conditions were assessed using the intersection capacity analysis software Synchro (v10) and using the same metrics such as v/c and delay. The following network modifications were included in the analysis (i.e., existing signal timing plans were not modified, unless otherwise specified below) for the three horizon scenarios, based on the changes to the road network shown in the Master Concept Plan:

### **Network Modifications from Master Concept Plan**

#### **Preston Street at Albert Street**

Modified to include eastbound left-turn lane with 30m of storage.

#### **Albert Street at Lorne Avenue**

 Modified to include north leg with stop control on the minor approach, permitting right-in/rightout movements only.

### **Albert Street at Empress Avenue**

Modified to include north leg, actuated-coordinated signal control with a 120s signal cycle.

#### **Booth Street at Fleet Street**

 Modified to include west leg with stop control on the minor approach, permitting right-in/rightout movements only.

### **Wellington Street at Broad Street**

 Modified to include south leg, actuated-coordinated signal control with a 95s AM, 120s PM signal cycles and a fully protected westbound left-turn phase.

# New Intersections from Master Concept Plan

#### **Albert Street at Access 1**

Actuated-coordinated signal control with a 120s signal cycle during AM and PM peaks.

#### **Albert Street at Access 2**

• Right-in/right-out with stop control on the minor approach only.

### **Wellington Street at Access 3**

Right-in/right-out with stop control on the minor approach only.

### **Wellington Street at Access 4**

Right-in/right-out with stop control on the minor approach only.

Operational analysis for all key intersections was conducted based on total projected volumes depicted in Figures 17-19, existing signal timing plans and the previously described network modifications. **Tables 17 - 19** summarizes the projected performance of study area intersections for the 2030, 2040 and 2050 horizon years. Detailed Synchro output data for total projected conditions is provided as **Appendix F.** 



Table 17: Study Area Intersection Operations - 2030 Phase 1 Total Projected Conditions

Intersections	Overall			Critical Movement			
	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS
Booth & Chaudière	13 (13)	0.91 (0.77)	E (C)	SBTR	18 (12)	0.93 (0.75)	E (C)
Booth & War Museum	1 (2)	0.47 (0.36)	A (A)	SBTR	2 (2)	0.47 (0.35)	A (A)
Booth & Wellington	49 (39)	0.95 (0.95)	E (E)	NBTR	<b>97</b> (61)	1.01 (1.03)	F (F)
Booth & Albert	53 (66)	0.93 (0.99)	E (E)	EBL	39 ( <mark>165</mark> )	0.83 (1.28)	D (F)
Albert & Empress	4 (4)	0.29 (0.38)	A (A)	WBLTR	3 (4)	0.15 (0.40)	A (A)
Albert & Preston	59 (40)	1.01 (0.81)	<b>F</b> (D)	EBT	89 (92)	1.07 (0.91)	F(E)
Albert & City Centre	10 (10)	0.63 (0.62)	B (B)	EBT	11 (6)	0.69 (0.46)	B (A)
Albert/Scott & Bayview Station	18 (20)	0.69 (0.63)	B (B)	EBTR	11 (22)	0.75 (0.60)	C (A)
Scott & Parkdale	30 (63)	0.81 (1.03)	D (F)	WBT	14 ( <mark>150</mark> )	0.36 (1.24)	A (F)
SJAMP & Slidell	3 (8)	0.50 (0.72)	A (C)	WBT	3 (9)	0.36 (0.72)	A (C)
Wellington/SJAMP & Vimy	2 (4)	0.45 (0.54)	A (A)	WBTR	2 (5)	0.31 (0.55)	A (A)
Wellington & Lett	20 (5)	0.60 (0.47)	A (A)	EBTR	27 (3)	0.62 (0.43)	B (A)
Wellington & Portage	109 (40)	1.22 (0.88)	<b>F</b> (D)	SBL	<b>234</b> (53)	<b>1.45</b> (0.88)	<b>F</b> (D)
Wellington & Broad	4 (3)	0.42 (0.48)	A (A)	WBT	3 (4)	0.29 (0.48)	A (A)

As shown in Table 17, the Wellington Street at Portage Bridge and Scott Street at Parkdale Avenue intersections are projected to continue operating over capacity during weekday morning or afternoon peak hours. In addition, the Albert Street at Preston Street intersection is now over capacity with a v/c ratio of 1.01, increasing from a v/c ratio of 0.95 in the Future Background Conditions. The main reason for this is the addition of a fourth leg (i.e., north leg) to the intersection as part of the LeBreton Flats development.

The only other noticeable change to intersection operations is the northbound through movement at the intersection of Booth Street at Wellington Street, which increases from a v/c ratio of 0.98 to 1.01 due to growth from the LeBreton Flats development. All other study area intersections are projected to continue operating similar to future background conditions, with only minor increases in volumes and delays.



Table 18: Study Area Intersection Operations - 2040 Phase 2 Total Projected Conditions

Intersections	Overall			Critical Movement			
	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS
Booth & Chaudière	13 (13)	0.91 (0.77)	E (C)	SBTR	18 (12)	0.93 (0.75)	E (C)
Booth & War Museum	1 (2)	0.47 (0.36)	A (A)	SBTR	2 (2)	0.47 (0.35)	A (A)
Booth & Wellington	51 (55)	0.96 (0.97)	E (E)	NBTR	97 (62)	1.01 (1.04)	F (F)
Booth & Albert	56 (67)	0.98 (1.00)	E (E)	EBL	45 (177)	0.85 (1.31)	D (F)
Albert & Empress	4 (5)	0.30 (0.42)	A (A)	WBLTR	3 (8)	0.16 (0.45)	A (A)
Albert & Preston	55 (29)	1.01 (0.83)	<b>F</b> (D)	EBT	79 (50)	1.07 (0.93)	F(E)
Albert & City Centre	11 (9)	0.63 (0.63)	B (B)	EBT	12 (6)	0.69 (0.47)	B (A)
Albert/Scott & Bayview Station	18 (20)	0.69 (0.64)	B (B)	EBTR	11 (22)	0.76 (0.61)	C (B)
Scott & Parkdale	30 (65)	0.81 (1.04)	D (F)	WBT	14 ( <mark>159</mark> )	0.37 (1.26)	A (F)
SJAMP & Slidell	3 (8)	0.50 (0.72)	A (C)	WBT	3 (9)	0.36 (0.72)	A (C)
Wellington/SJAMP & Vimy	3 (3)	0.45 (0.53)	A (A)	WBTR	4 (3)	0.32 (0.54)	A (A)
Wellington & Lett	22 (9)	0.61 (0.47)	B (A)	EBTR	31 (10)	0.63 (0.49)	B (A)
Wellington & Portage	<b>139</b> (46)	1.30 (0.86)	<b>F</b> (D)	SBL	309 (64)	1.62 (0.93)	F(E)
Wellington & Broad	6 (6)	0.45 (0.50)	A (A)	WBT	5 (7)	0.30 (0.51)	A (A)
Albert & Access 1	8 (7)	0.77 (0.71)	C (C)	EBT	12 (13)	0.78 (0.72)	C (C)

As shown in Table 18, study area intersections are projected to continue operating similar in the year 2040 when compared to the projected conditions for the 2030 horizon year. With the exception of previously identified problematic intersections, all study area intersections are projected to operate acceptably, at LOS 'E' or better.



Table 19: Study Area Intersection Operations - 2050 Phase 3 Total Projected Conditions

Intersections	Overall			Critical Movement			
	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS
Booth & Chaudière	14 (14)	0.93 (0.79)	E (C)	SBTR	21 (13)	0.95 (0.77)	E (C)
Booth & War Museum	1 (2)	0.48 (0.37)	A (A)	SBTR	2 (2)	0.48 (0.36)	A (A)
Booth & Wellington	51 (60)	0.97 (0.99)	E (E)	NBTR	97 (77)	1.03 (1.07)	F (F)
Booth & Albert	59 (74)	0.97 (1.04)	E(F)	EBL	54 (204)	0.90 (1.35)	D (F)
Albert & Empress	4 (6)	0.30 (0.43)	A (A)	WBLTR	3 (8)	0.16 (0.46)	A (A)
Albert & Preston	57 (30)	1.03 (0.86)	<b>F</b> (D)	EBT	85 (51)	1.09 (0.95)	F(E)
Albert & City Centre	11 (9)	0.64 (0.63)	B (B)	EBT	12 (6)	0.70 (0.47)	B (A)
Albert/Scott & Bayview Station	17 (20)	0.69 (0.64)	B (B)	EBTR	11 (22)	0.75 (0.60)	C (A)
Scott & Parkdale	28 (69)	0.78 (1.06)	C (F)	WBT	14 (175)	0.36 (1.30)	A (F)
SJAMP & Slidell	3 (8)	0.51 (0.74)	A (C)	WBT	3 (9)	0.36 (0.74)	A (C)
Wellington/SJAMP & Vimy	3 (3)	0.46 (0.54)	A (A)	WBTR	4 (3)	0.32 (0.55)	A (A)
Wellington & Lett	23 (9)	0.61 (0.48)	B (A)	EBTR	32 (11)	0.63 (0.50)	B (A)
Wellington & Portage	142 (47)	1.31 (0.87)	<b>F</b> (D)	SBL	317 (65)	1.63 (0.94)	F(E)
Wellington & Broad	6 (6)	0.46 (0.51)	A (A)	WBT	5 (7)	0.31 (0.52)	A (A)
Albert & Access 1	9 (8)	0.79 (0.72)	C (C)	EBT	12 (14)	0.80 (0.73)	C (C)

As shown in Table 19, operational conditions at key study area intersections in the year 2050 are expected to be slightly worse than those in the 2040 horizon year. The following intersections operated acceptably in the 2040 horizon year and will continue to operate acceptably in the 2050 horizon year:

- Booth Street and Chaudière
- Booth Street and War Museum
- Booth Street and Wellington Street
  - The northbound through/right movement is over capacity in both peak hours (v/c ratio 1.03 in AM peak, 1.07 in PM peak). This is consistent with the 2040 horizon year (v/c ratio 1.01 in AM peak, 1.04 in PM peak), albeit with slightly higher v/c ratios.
- Albert Street and Empress Avenue
- Albert Street and City Centre Avenue
- Albert Street / Scott Street and Bayview Station Road
- Sir John A. Macdonald Parkway and Slidell Street
- Wellington Street / Sir John A. Macdonald Parkway and Vimy Place
- Wellington Street and Lett Street
- Wellington Street and Broad Street
- Albert Street and Access 1



The following intersections were over capacity in at least one peak hour in the 2040 horizon year, and continue to be over capacity in at least one peak hour in the 2050 horizon year, with minimal increase to the v/c ratio:

- Albert Street and Preston Street (AM peak hour)
- Scott Street and Parkdale Avenue (PM peak hour)
- Wellington Street and Portage Bridge (AM peak hour)

The intersection of Booth Street at Albert Street operates with a LOS 'E' in both peak hours in the 2040 horizon year. In the 2050 horizon year, the PM peak hour operates with a LOS 'F' in the PM peak hour. While it is noted that this results in an unacceptable LOS for the overall intersection, it is worth noting that the actual change in v/c ratio is an increase from 1.00 in 2040 to 1.04 in 2050, a small increase that results in the LOS 'F'. It is difficult to make signal timing adjustments to improve operations at this intersection, given the high traffic volumes in all directions. Any changes to signal timing to accommodate one direction or movement will result in another movement being over capacity.

# 4.3.3 Adjustments to Travel Demand

Adjusting modal splits away from projected auto trips further is difficult to justify, as certain individuals will ultimately be required to travel by vehicle for one reason or another (e.g., distance between origin/destination is too great, travel is a requirement for employment, physical disabilities limit travel options, etc.). Additionally, adjusting the auto modal share for site-generated traffic much lower will have a negligible effect on the performance of study area network.

With the opening of the Confederation LRT line and the coming expansion of both the Confederation LRT line and the Trillium LRT line, it is anticipated that there will be an increased number of transit users, which is likely to alleviate the vehicular demand on study area intersections. As noted above, there is also the future West Gatineau Tramway and downtown transit loops project that has the potential to reduce interprovincial vehicular travel, including along the Booth Street corridor. Furthermore, with the planned improvements to active transportation facilities as identified in Section 3.1.3 and as proposed active transportation facilities as part of the Master Concept Plan, there may be a shift to more active modes in the study area in the future. In addition to a shift to alternative modes, peak network demand may also be further spread beyond peak hours with individuals able to modify their working hours (e.g., individuals choosing to leave for work earlier or later to avoid the most congested network conditions) or working remotely (or telecommuting) from their homes.

Telecommuting has become a more common trend since the start of the COVID-19 pandemic in March 2020. A Statistics Canada report indicates that as of May 2020 almost one-third (32.6%) of businesses had 10% or more of their workforce telecommuting. This is a significant increase over the numbers from February 2020 that indicated only 16.6% of businesses had 10% or more of their workforce telecommuting. The report also indicates that close to one quarter (22.5%) of businesses expect that 10% or more of their workforce will continue to telecommute once the COVID-19 pandemic is over. Rapid technological advancement in network security, accessibility and remote monitoring holds much promise in addressing long standing concerns with telecommuting. A paper presented at the Transportation Association of Canada (TAC) conference in 2004 titled Development of Modal Share Targets for Ottawa's Transportation Master Plan estimated that 5% of Ottawa workers telecommute. The same paper estimated that in the future this number could grow from 5% to 8%.

Based on the foregoing, no adjustments to background or site-generated network demand were considered for the purposes of this TIA study. However, it should be noted that new traffic data will be collected for each development application related to LeBreton Flats to feed into TIA studies for each application. The updated data collected with each study should more accurately reflect the benefits of Ottawa's new LRT service, which may potentially alleviate vehicular demand on study area intersections.



### 4.4 Preston Street Extension

The LeBreton Flats Master Concept Plan proposes a shift in function of the planned Preston Street extension and bridge between Albert Street and the SJAMP/Wellington Street from a vehicular focus to an active transportation focus more in line with the City's new Transportation Master Plan. The Preston Street arterial extension has been previously identified in the City of Ottawa's Official Plan and Transportation Master Plan. The Master Concept Plan proposes to replace this planned roadway, including a vehicular bridge, with an active transportation bridge. This is addressed in the Planning Rationale, detailing the policy alignment and qualitative considerations for an active-modes Preston extension to support this change.

Through discussions with the City, it was agreed that an analysis of the removal of the Preston extension and bridge should be included in this TIA, which would normally beyond the scope of work for a TIA as there are regional implications that would require more advanced modelling and scenario testing.

A list of high-level concerns from the City regarding the potential removal of the Preston Street extension are provided below. The balance of the following sections aims to address the stated concerns from the City of Ottawa with the removal of the Preston extension. This has been informed and grounded using various methodologies including: a network assessment using the City's EMME Travel Demand Model; a screenline analysis; intersection operations analysis; and a review of impacts to pedestrians, cyclists, heavy vehicles, transit, and emergency vehicles.

#### 4.4.1 EMME Models

Through discussions with City of Ottawa staff, a number of modelling scenarios were developed and tested. All scenarios for the EMME models:

- Used the City's 2031 Network Concept Plan per the City's 2013 Transportation Master Plan, which assumes a higher mode share for car trips than the current EMME model in development at the City for the new TMP. This will result in a more conservative analysis, as a greater number of trips will be assigned to the vehicular mode.
- Assumed full build-out of the LeBreton Flats lands even though the full buildout is not expected to occur until the year 2050.
- Assumed that the Tunney's Pasture Complex will be redeveloped to add 3,718 persons and increase employment from 11,440 jobs to 13,091. The redevelopment of the Tunney's Pasture Complex, a federal government office complex located approximately 2.0 km to the west, is the largest planned adjacent development that does not yet have a TIA completed (as previously mentioned, 900 Albert Street and Zibi have TIAs already completed). It is expected that at its ultimate completion, the redevelopment will employee somewhere between 22,000 and 25,000² people.

<sup>&</sup>lt;sup>2</sup> https://www.tpsgc-pwgsc.gc.ca/biens-property/construction/tunneyspasture-eng.html





A description of each scenario along with associated assumptions is presented below

- Scenario 1 (Baseline Scenario) 2013 TMP Network Concept model, without Preston extension, with the following changes made to the network:
  - Barrhaven LRT
  - Kanata LRT
  - Baseline BRT
  - Blackburn Hamlet Bypass
  - New Gatineau Road Network
  - New STO Routes
  - West Gatineau Tramway
  - Discussions were held regarding the potential inclusion of a sixth crossing of the Ottawa River between Ottawa and Gatineau, however the City staff maintained that the analysis of the Preston extension should be based on currently planned projects. With no specific timeline, location or budget, the sixth crossing does not fall into this category.
- Scenario 2A Baseline scenario (no Preston Street extension), with all turning movements
  permitted at the intersection of Booth Street / Wellington Street (i.e., northbound left,
  eastbound left, eastbound right and westbound left turns permitted).
- Scenario 2B Baseline scenario (no Preston Street extension), with northbound left turns permitted at the intersection of Booth Street / Wellington Street.
- Scenario 3 Baseline scenario, Preston Street extension in place, current turning movement restrictions in place at Booth Street / Wellington Street.

The full EMME models for the Preston extension analysis are included in **Appendix G**, with snapshots provided below from each scenario, including an image showing the volume of traffic diverted to Preston Street from other corridors when the extension is in place.



## 4.4.2 Local Impacts

Figures 22 and 23 below show the volume to capacity ratio (v/c ratio) for Scenario 1 (without Preston Street extension) and Scenario 3 (with Preston extension) projected by the EMME model. An acceptable v/c ratio is shown in green, approaching capacity is shown in orange, and over capacity is shown in red. Some takeaways from these figures are noted below:

Figure 22: Scenario 1 (w/o extension) v/c Ratio



- It is notable that the volume relief in the model for the westbound SJAMP is for a section already operating well (i.e., west of Preston Street).
- The addition of the Preston extension appears to further deteriorate operations on Wellington Street east of Preston Street (i.e., between Preston Street and Booth Street).
- The Preston extension does not appear to relieve Booth Street south of Albert Street.

The model shows that the Preston extension seems to draw traffic away from roads that are well beyond the influence area of LeBreton Flats. This exhibits the potential for the Preston extension to create induced demand, by providing a more attractive vehicular facility and attracting trips away from other modes of transportation such as transit and active modes. which runs contrary to the City's new Transportation Master Plan.



Figure 23: Scenario 3 (with extension) v/c Ratio



### 4.4.3 Traffic Diversion

**Figures 24** and **25** display the volume of diverted traffic from the EMME model when the Preston extension is in place. An increase in volume is shown in red, while a decrease in volume is shown in green. Figure 24 focuses on the area around LeBreton Flats, while Figure 25 shows the larger road network. Some takeaways from the modelling demonstrated in these figures are noted below:

- A significant reduction in volumes to/from the SJAMP west of Preston Street is observed (-320 vph westbound, -60 vph eastbound) in the modelling with the introduction of the Preston extension. This reduction in volumes appears to extend further west than originally anticipated, as there are vehicle reductions as far west as Pinecrest Road and Richmond Road.
- There are minimal changes in downtown traffic volumes (i.e., east of Bronson Avenue) from the introduction of Preston Street into the model.
- The traffic increase shown in the model on Preston Street is well in excess of the traffic reduction observed on Booth Street (+280 vph northbound, +415 vph southbound), resulting in an overall net increase in traffic within the study area.
- The increase of traffic shown in the model in both directions on Preston Street, Rochester Street and Booth Street (+160 vph northbound, +220 vph southbound) would impact local residents living in the Centretown West neighbourhood. This modelled increase in traffic is likely due to an increase in commuters connecting between Gatineau and Highway 417.
- The model shows that westbound vehicles destined to the Tunney's Pasture complex shift from the SJAMP (-150 vph) to Albert Street (+70 vph) with the Preston extension in place.
- Between Carling Avenue and Baseline Road, the model shows that the Preston extension reduces southbound traffic on Fisher Avenue (-30 vph) and increases traffic on Prince of Wales Drive (+50 vph).
- The model shows that the Preston extension reduces westbound traffic on SJAMP (-100 vph) and increases traffic on Highway 417 (+60 vph) between Rochester Street and Pinecrest Road.



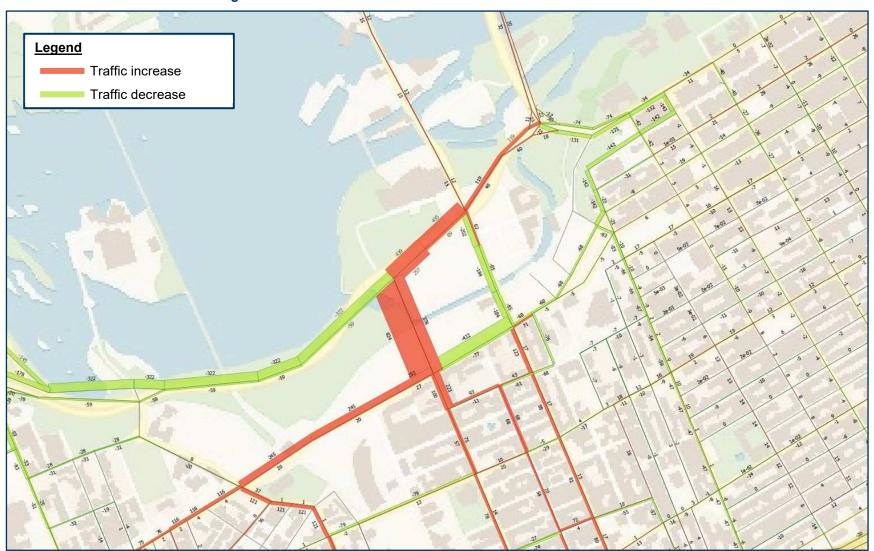


Figure 24: Preston Extension Traffic Diversion – LeBreton Flats Area



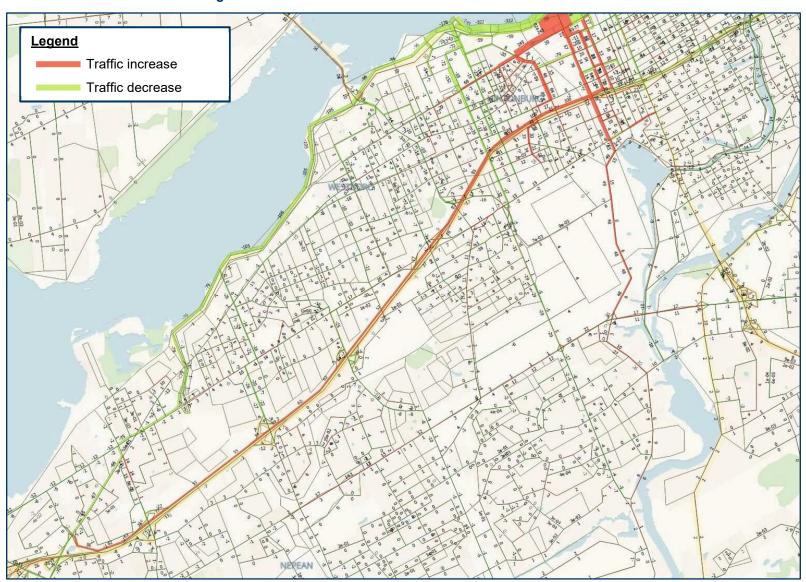


Figure 25: Preston Extension Traffic Diversion – Road Network



## 4.4.4 Screenline Analysis

It is notable that the model appears to show an overall increase in traffic volumes in and around LeBreton Flats with the Preston extension in place. This would indicate that the implementation of the Preston extension diverts traffic away from other corridors by drawing additional traffic from outside of the study area that was not previously in the area. This can be confirmed by the use of a Screenline Analysis, as shown in Table 20 below. For this analysis, the selected screenline is the general alignment of the Confederation Line LRT, just north of Albert Street.

Table 20: Preston Extension – Screenline Analysis (Confederation Line)<sup>3</sup>

Otro at	On ation	No	orthbound		Southbound			
Street	Section	Scen 1	Scen 3	Diff	Scen 1	Scen 3	Diff	
Parkdale	Lyndale - Scott	423	398	-25	529	478	-51	
Bayview Station	Burnside - Scott	65	73	+8	442	422	-20	
Preston	SJAMP – Albert	0	378	+378	0	824	+824	
Booth	Fleet – Albert	1147	1062	-85	1324	1140	-184	
Bronson	Queen – Albert	951	932	-19	494	353	-141	
Вау	Queen – Albert	288	341	+53	ı	-	-	
Lyon	Queen – Albert	-	-	-	562	553	-9	
Kent	Queen – Albert	323	296	-27	ı	-	-	
Bank	Queen – Albert	34	34	0	176	172	-4	
Total	-	3231	3514	+283	3527	3942	+415	

While the model shows an obvious increase in traffic volumes on Preston Street, one of the general ideas for the extension is to divert vehicles off of Booth Street, spreading traffic across both roadways. As shown in the City's EMME model this is not the case, as the reduction in vehicles on Booth Street and other parallel corridors would not balance out the increase in traffic on Preston Street caused by the Preston extension. This indicates that the Preston extension would draw in additional traffic that was not previously in the area, increasing the amount of traffic in the area. This can be further confirmed by using a screenline further south, just north of Somerset Street aligning with Spruce Street.

<sup>&</sup>lt;sup>3</sup> It should be noted that as shown in Figure 22, there is a decrease in traffic volumes for Scenario 3 on the Sir John A. Macdonald Parkway west of the Preston extension. It is likely that these vehicles would eventually cross the designated screenline at a point further west, and therefore are notably absent from the analysis. However even when added to the above numbers, we still see an increase in traffic volumes across the screenline due to the Preston extension (+249 vph vehicles northbound, +144 vehicles vph southbound).



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**Northbound Southbound** Street Section Diff Scen 1 Scen 3 Scen 1 Scen 3 Diff **Bayview Station** Burnside - Scott +1 365 487 +122 153 154 Preston SJAMP - Albert 537 612 +75 488 546 +58 277 344 +67 248 +67 Rochester 315 Fleet - Albert 804 821 +17 670 758 +88 Booth -9 Bronson Queen - Albert 1161 1152 613 554 -59 +151 2932 3083 2384 2660 **Total** +276

Table 21: Preston Extension – Screenline Analysis (Spruce Street)

It is worth noting that for this screenline, the model shows that north-south traffic would increase on Preston Street, Rochester Street and Booth Street. This further confirms that while the implementation of the Preston extension would divert some traffic away from Booth Street between Wellington Street and Albert Street, it would attract additional traffic to Booth Street between Wellington Street and Albert Street, as well as Preston Street, Rochester Street and Bayview Station Road. This section of Booth Street already features traffic calming measures to address the heavy traffic volumes and speeds, and drawing additional traffic to the neighbourhood would likely not be welcomed by the surrounding residential neighbourhood.

#### 4.4.5 Flats District Access

City of Ottawa comment: The Flats District that is north of the Confederation Line and west of Booth Street makes up approximately 33% of all the site's peak hour trip generation and as currently proposed cannot be accessed from the south via Booth Street. This access issue may cause deliveries, taxis, and other necessary motor vehicle trips to use Parkdale Avenue (~2 km west) or Bay Street (~1 km east) to reach the development. The Preston extension would alleviate this concern.

The Flats District is the area of LeBreton Flats west of Booth Street and north of the Confederation Line. Booth Street would not be able to be used for vehicles to access the Flats District from the south due to the northbound left turn restriction in place at Booth Street and Wellington Street, unless the turn restriction was removed/changed. As noted by the City, with the Preston extension in place, vehicles from the south would be able to use Preston Street to access Wellington Street and enter the Flats District; however, it is important to note that due to the site grades and the type of bridge infrastructure that would be required in order to provide the Preston extension, it would not be possible for vehicles to directly access future local streets within the Flats District from Preston Street.

The City's concern that 33% of all vehicular trips generated by the LeBreton Flats development would not be able to enter and exit the Flats District is somewhat misguided; this figure assumes that all vehicular trips into and out of the Flats District would not be able to connect with the Flats District. However, access and egress to the Flats District would be possible for all vehicular trips from the north, east and west – only inbound vehicular trips from the south would be unable to access the Flats District, due to the existing northbound left turn restriction at Booth Street and Wellington Street. These trips (i.e., inbound trips from the south) amount to only 14% of all inbound vehicular trips (3% of all person trips) to LeBreton Flats, approximately 50 vph in the AM peak hour and 80 vph in the PM peak hour. Additionally, if this access remains a concern, there is potential for the removal of the northbound left turn restriction at the intersection of Booth Street and Wellington Street.



The diverted traffic volumes shown above in Figure 24 provide some insight on access routes into the Flats District for vehicles from the south without the Preston extension in place. It appears that the model shows the routes of choice to access the Flats District from the south would be:

- Via Parkdale Avenue / SJAMP (approximately 40%)
- Via Kent Street / Bank Street / Wellington Street (approximately 30%)
- Via Bay Street / Bronson Avenue / Wellington Street (approximately 30%)

Kent Street would be a strong contender for accessing the Flats District from the south as it is a northbound street with an off-ramp access from Highway 417 and has some connectivity further south via Bank Street. Bay Street would be another contender for access from the south, due to its connection to Bronson Avenue, which has access to Highway 417 and further south of the highway.

Bayswater Avenue / Bayview Station Road / Slidell Street could be a contender for access to the Flats District if the northbound right turn restrictions at the intersection of Slidell Street and SJAMP were removed. This scenario was modelled in EMME to review the potential upstream impacts from this change. The results of the model show the following changes to traffic volumes in the area:

- Increase of 50 vph northbound on Bayswater Avenue between Carling Avenue and Gladstone Avenue. This section of Bayswater Avenue is classified as a Local Street.
- Increase of 70 vph northbound on Bayswater Avenue / Bayview Station Road between Gladstone Avenue and Albert Street / Scott Street. This section of Bayswater Avenue / Bayview Station Road is classified as a Collector Street.
- Increase of 140 vph northbound on Bayview Station Road north of Albert Street / Scott Street. This section of Bayview Station Road is classified as a Collector Street.
- Increase of 110 vph northbound through Mechanicsville neighbourhood; all streets classified as Local Streets.
- Decrease of 200 vph on Parkdale Avenue north of Scott Street. Parkdale Avenue is classified as an Arterial Street on this section.

Given that the impacts this change would present in terms of an increase in traffic volumes on lower order facilities, specifically south of Somerset Street where there are already traffic calming measures in place, the removal of the northbound right turn restriction from Slidell Street onto the SJAMP is not recommended. However, it is worth noting that this movement could be permitted for other traffic, such as transit or emergency vehicles, if desired.

Without the Preston extension, 97% of all person trips would have direct access to the Flats District, with only 3% of person trips (and 14% of vehicular trips) requiring the use of an alternative corridor such as Parkdale Avenue, Kent Street or Bay Street. Given the strong connection provided to the Flats District for all non-vehicular modes, access to the Flats District is not felt to be a strong concern with the removal of the Preston extension.



# 4.4.6 Intersection Operations

City of Ottawa comment: The increase in network capacity from the addition of 400 m of roadway in the Preston Street corridor would be nominal and would be limited by area bottle necks (e.g., Chaudière interprovincial crossing is and will continue to operate at capacity).

The City's EMME models identified in Section 4.4.1 were used as the basis for developing Scenario 3 (Preston extension) turning movement volumes that are used in the Synchro analysis below. The changes in traffic volume on each corridor were manually adjusted since the EMME model tends to overrepresent traffic volumes when compared against turning movement counts (this is consistent across both the existing EMME models and the future EMME models). Once the adjustment was made, traffic was diverted as outlined in Figure 22, with adjustments made to the turning movement volumes as necessary. Since the EMME model is for the morning peak hour only, this analysis was undertaken only for the AM peak hour. Figure 26 below shows the adjustments made to Scenario 1 turning movement volumes, while Figure 27 shows the Scenario 3 turning movement volumes.

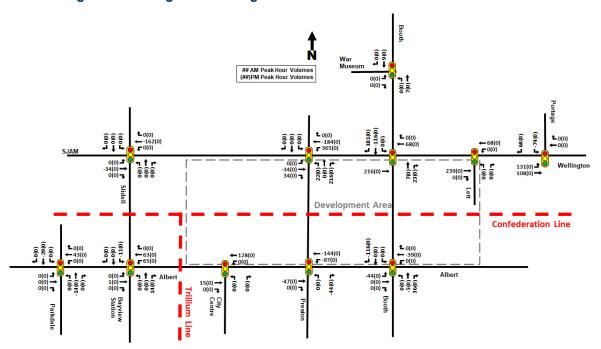


Figure 26: Changes to Turning Movement Volumes from Preston Extension



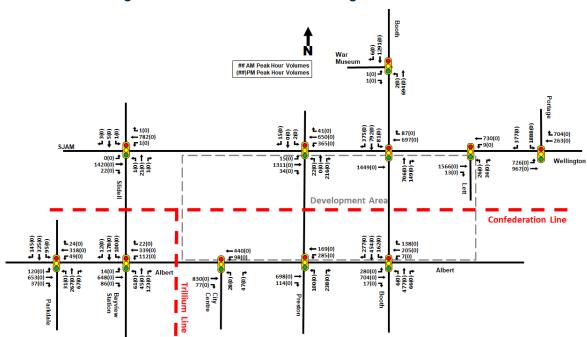


Figure 27: Preston Extension –Turning Movement Volumes

The following **Table 22** summarizes the projected study area intersection performance based on the Preston extension - Future Background volumes, assuming no significant changes to signal timing plans from the Future Background conditions (i.e., slight tweaks to optimize phases, but not cycle lengths). As with the traffic volumes, only the AM peak hour was analyzed, given that the EMME model only focuses on the morning peak hour.

Table 22: Study Area Intersection Operations – Preston Extension

		Overall		Critical Movement				
Intersections	Delay (s)	v/c Ratio	v/c LOS	Mvmt	Delay (s)	v/c Ratio	v/c LOS	
Wellington & Portage	122	1.24	F	SBL	278	1.54	F	
Booth & Chaudière	19	0.92	Е	SBTR	27	0.95	Е	
Booth & Wellington	34	0.88	D	EBT	37	0.94	Е	
Booth & Albert	41	0.70	D	EBL	78	0.87	D	
Preston & Albert	62	1.01	F	WBL	103	1.05	F	
Wellington/SJAMP & Vimy/Preston	50	0.94	E	EBTR	85	1.10	F	
Slidell & SJAMP	3	0.42	Α	EBLTR	3	0.50	Α	
Bayview Station & Scott/Albert	19	0.58	А	SBT	50	0.77	С	
Parkdale & Scott	23	0.71	С	EBT	27	0.77	С	
Booth & War Museum	4	0.45	А	SBTR	7	0.45	А	
City Centre & Albert	7	0.53	Α	EBT	7	0.59	Α	
Wellington & Lett	21	0.65	В	EBTR	29	0.67	В	



The intersection of Wellington Street at Portage Bridge continues to operate poorly in this scenario, consistent with all other analysis. The intersection of Preston Street at Albert Street is over capacity, which is consistent with the Future Background Scenario presented in Table 16. It is worth noting that the removal of the Preston extension does not noticeably impact the level of service of any intersections outside of the four key intersections identified below.

The following tables focus on the four key intersections that are most impact by the Preston extension: Booth Street at Wellington Street, Booth Street at Albert Street, Preston Street at Sir John A. Macdonald Parkway and Preston Street at Albert Street. The benefits and impacts of each scenario are summarized after each table.

Table 23: Booth Street at Wellington Street - Scenario 1 vs Scenario 3

Intersections	Movements	Delay (s)	v/c Ratio	v/c LOS
	EBT	61	0.93	E
	WBT	31	0.54	Α
	WBR	24	0.23	Α
Scenario 1 – No Preston Extension	NBTR	48	0.88	D
Scenario I – No Prestori Extension	SBL	119	0.71	С
	SBT	21	0.66	В
	SBR	10	0.32	Α
	Overall	42	0.90	D
	EBT	53	0.94	E
	WBT	27	0.45	Α
	WBR	21	0.13	Α
Scenario 3 – Preston Extension	NBTR	40	0.82	D
Scenario 3 – Prestori Extension	SBL	123	0.40	Α
	SBT	28	0.58	Α
	SBR	22	0.54	Α
	Overall	39	0.88	D

The overall intersection of Booth Street at Wellington Street operates with a LOS 'D' in both scenarios and no failing movements. Both scenarios have one movement approaching capacity with the northbound through/right in Scenario 1 (No Preston extension) at LOS 'E' and the eastbound through in Scenario 3 (Preston extension) at LOS 'E'. The eastbound through movement operates worse in Scenario 1 as there is less east-west green time due to the need to accommodate additional southbound right turn traffic bound for Preston Street. This intersection operates comparably in both scenarios.



Table 24: Booth Street at Albert Street - Scenario 1 vs Scenario 3

Intersections	Movements	Delay (s)	v/c Ratio	v/c LOS
	EBL	21	0.65	В
	EBT	84	0.90	D
	EBR	0	0.03	А
	WBL	36	0.16	Α
	WBT	33	0.35	А
Scenario 1 – No Preston Extension	WBR	35	0.36	А
	NBLTR	59	0.89	D
	SBL	70	0.88	D
	SBT	47	0.81	D
	SBR	54	0.87	D
	Overall	55	0.89	D
	EBL	80	0.87	D
	EBT	24	0.79	С
	EBR	0	0.02	Α
	WBL	33	0.04	Α
	WBT	34	0.22	Α
Scenario 3 – Preston Extension	WBR	37	0.33	Α
	NBLTR	49	0.76	С
	SBL	81	0.82	D
	SBT	36	0.68	В
	SBR	30	0.49	Α
	Overall	41	0.70	В

The intersection of Booth Street at Albert Street operates well in both scenarios, with a LOS 'D' in Scenario 1 and a LOS 'B' in Scenario 3. Scenario 1 has the eastbound left turn movement failing (v/c = 1.39) and the eastbound through movement approaching capacity (v/c = 0.95). Neither movement is an issue in Scenario 3, as the Preston extension reduces the volume of the eastbound left turn movement, allowing for additional time to be allotted to other movements. While this intersection operates well in both scenarios, Scenario 3 is favoured here from a traffic operations perspective.



Table 25: Preston Street at Albert Street - Scenario 1 vs Scenario 3

Intersections	Movements	Delay (s)	v/c Ratio	v/c LOS
	EBL	18	0.01	Α
	EBT	83	1.08	F
	EBR	9	0.19	Α
	WBL	66	0.94	E
Scenario 1 – No Preston Extension	WBTR	5	0.17	Α
	NBLT	59	0.61	В
	NBR	11	0.63	В
	SBLTR	43	0.22	Α
	Overall	50	0.96	E
	EBL	20	0.15	Α
	EBT	75	1.04	F
	EBR	6	0.18	Α
	WBL	103	1.05	F
Scenario 3 – Preston Extension	WBTR	15	0.10	Α
Scenario 3 – Prestori Exterision	NBL	143	1.04	F
	NBTR	34	0.67	В
	SBL	37	0.24	Α
	SBTR	64	0.93	E
	Overall	62	1.01	F

The overall intersection operations at Preston Street and Albert Street are both LOS 'F', with delays of 72 seconds for Scenario 1 and 62 seconds for Scenario 3. Scenario 1 fails due to the heavy opposing movements of the eastbound through (v/c ratio = 1.02) and westbound left (v/c ratio = 1.41), with all other movements operating acceptably. The Preston extension allows for the redistribution of westbound left trips to the southbound trips. Scenario 3 fails due to numerous heavy volume movements, including eastbound through (v/c ratio = 1.04), westbound left (v/c ratio = 1.04) and southbound through/right (v/c ratio = 0.93). The operations at this intersection are so similar that there is not a clear preference for either scenario from a traffic operations perspective.



Table 26: Preston Street at Sir John A. Macdonald Parkway – Scenario 1 vs Scenario 3

Intersections	Movements	Delay (s)	v/c Ratio	v/c LOS
	EBL	3	0.04	Α
	EBTR	4	0.50	Α
	WBL	7	0.26	Α
Scenario 1 – No Preston Extension	WBTR	3	0.34	Α
	NBLTR	17	0.17	Α
	SBLTR	7	0.08	А
	Overall	3	0.48	Α
	EBL	20	0.06	Α
	EBTR	85	1.10	F
	WBL	40	0.64	В
Connerio 2 Proston Futoncian	WBTR	3	0.28	Α
Scenario 3 – Preston Extension	NBL	42	0.17	Α
	NBTR	4	0.48	А
	SBLR	1	0.07	Α
	Overall	50	0.94	E

While the intersection of Preston Street at SJAMP operates acceptably in both scenarios, it is approaching capacity in Scenario 3 (v/c ratio = 0.94). Due to the introduction the south leg of the intersection, there is inadequate green time to serve the east-west movement, resulting in a v/c ratio of 1.10 for the eastbound through/right movement. From the intersection operations analysis we can see that the Preston extension causes a deterioration in intersection operations on the Preston Street corridor that do not justify the marginal improvement in intersection operations along Booth Street corridor.

#### 4.4.7 Network Modifications

Due to the limited number of north-south connections between Wellington Street / SJAMP and Albert Street, there appear to be limited opportunities for network modifications that would improve vehicular capacity while still prioritizing non-vehicular modes of transportation. An example of this is the aforementioned permitting of northbound right turns at the intersection of Slidell Street at SJAMP, which would result in an increase of cut-through traffic through the residential section of Bayswater Avenue. Bayswater Avenue is classified as a local street from Carling Avenue to Gladstone Street. Furthermore, due to requests from residents, speed humps and flex posts were implemented in recent years as traffic calming measures. These two factors show that while allowing the northbound right turn at Slidell Street / SJAMP may improve traffic operations in the study area, there are other reasons that it wouldn't be an acceptable network modification solution.

The only modification that has merit at this time is permitting the northbound left turn movement at the Booth Street and Wellington Street intersection, making it accessible to all traffic. This movement was previously permitted to transit only, but with the recent reconstruction of the intersection it has been removed.

The City provided an EMME model for the scenario with the northbound left turn being implemented at the intersection of Booth Street and Wellington Street, which indicated that 112 vehicles would make a northbound left turn movement during the AM peak hour. This has been modelled in Synchro software and is compared against the default Scenario 1 in the table below.



Table 27: Booth Street at Wellington Street – Permitted Northbound Left Turn

Scenario	Movements	Delay (s)	v/c Ratio	v/c LOS
	EBT	61	0.93	E
	WBT	31	0.54	Α
	WBR	24	0.23	Α
No Northbound Loft Turn	NBTR	48	0.88	D
No Northbound Left Turn	SBL	119	0.71	С
	SBT	21	0.66	В
	SBR	10	0.32	Α
	Overall	42	0.90	D
	EBT	87	0.95	E
	WBT	30	0.55	Α
	WBR	23	0.22	Α
	NBL	41	0.69	В
Northbound Left Turn Permitted	NBTR	97	1.03	F
	SBL	131	0.90	D
	SBT	77	1.06	F
	SBR	15	0.46	Α
	Overall	72	0.98	E

Implementing a northbound left turn movement at the intersection of Booth Street / Wellington Street increases the delay and v/c ratio for most movements at the intersection, as well as for the overall intersection as a whole. The northbound through/right and southbound through movements are over capacity, and the intersection is approaching capacity. This indicates that a northbound left turn movement is not recommended at this intersection and should only be considered if the City continues to be concerned with the lack of vehicular access to the Flats District from the south.

The other potential mitigation measure is the provision of a sixth crossing of the Ottawa River. The NCC recently completed a Long-Term Integrated Crossings Plan; some key items that may benefit the transportation network around LeBreton Flats are highlighted below:

- There are limited opportunities to enhance vehicular capacity of existing crossings in the central core, however increasing people-moving capacity through sustainable transportation initiatives is possible.
- A new eastern crossing shows a higher potential to divert truck traffic away from the downtown crossings than a western one (15% diversion vs 8% diversion).
- A truck tunnel from Highway 417 to Macdonald-Cartier Bridge diverts the greatest volume of trucks from the downtown crossings (35%).
- The planned West Gatineau Tramway and downtown transit loop (shown previously in Figure 11) has significant potential to meet the needs of interprovincial travel. It is worth noting that the EMME model used in this analysis includes only the West Gatineau Tramway, and not the downtown transit loop. The West Gatineau Tramway is assumed to not lead to a reduction in vehicle lanes, as it would be accommodated through the addition of a new bridge crossing or replacement of the bus lanes currently on the Portage Bridge.



## 4.4.8 Pedestrian and Cycling Safety

City of Ottawa comment: The intersection of Booth/Albert has a lot of responsibilities to transit, pedestrians and cyclists due to its proximity to OC Transpo's LRT Pimisi Station. The ability to shift freight transport away from the Albert Street / Booth Street intersection will reduce the number of turning trucks at both the Albert Street / Booth Street and Preston Street / Albert Street intersections, which would improve the pedestrian experience and more importantly, pedestrian safety at these two intersections. Truck turning would still be required at the Preston Street / Wellington Street and Wellington Street / Booth Street intersections; however, the Wellington Street corridor does not have the physical constraints, nor the pedestrian volumes that the Albert Street corridor does and would. Conflicts between pedestrians and turning trucks (as currently seen on the northern leg of the Albert Street / Booth Street intersection) are typically unwelcomed from a road safety perspective, and should be considered carefully, with the understanding that both corridors are not without their risks.

Concerns with the mixing of heavy left turn movements (including heavy trucks) and pedestrians/cyclists at the intersection of Albert Street and Booth Street are justified. It is worth noting that diverting traffic and heavy trucks to the Preston extension helps alleviate safety issues in the Booth Street corridor, however it shifts the problem to the Preston Street corridor.



Figure 28: Existing Cycling Facilities at Booth Street / Albert Street

The Master Concept Plan proposes multi-use pathways on both sides of the aqueduct (i.e., north of the Confederation Line) as shown in **Figure 29**, which will permit pedestrians and cyclists to avoid using Albert Street if desired. The proposed MUPs will be attractive to trips originating or destined to LeBreton Flats, as well as cut through trips, such as active transportation users along the Ottawa River Pathway, or to and from downtown. At the time of this report, the multi-use pathway on the north side of the Confederation Line (south of the aqueduct) has been recently built between Pimisi and Bayview LRT stations with a connection to the Ottawa River Pathway and since its opening in December 2021, it has received more than 12,000 trips by pedestrians and cyclists This will reduce the reliance of active transportation users on the Albert Street corridor, reducing vehicular conflicts with pedestrians and cyclists. Pedestrians and cyclists travelling north-south will still need to cross the Albert Street corridor (i.e., to/from Gatineau via the Chaudière Crossing). Further improvements to pedestrian and cyclist safety at the intersection of Albert Street/Booth Street could be observed with the implementation of a crossride on the north side of the intersection along with a fully protected eastbound left turn phase to avoid vehicular-pedestrian/cyclist conflicts.





Figure 29: Proposed Multi-Use Pathways



## 4.4.9 Freight Movements

City of Ottawa comment: Preston Street is a designated truck route, which provides freight transportation (e.g., between Highway 417 and Quebec). There are only two Interprovincial freight crossings in the region, and neither can support the addition of the other's load. At a future date, if freight can be relocated to another crossing location, this issue may be significantly mitigated. Note that the cities of Ottawa and Gatineau and the NCC recognize the less-than-ideal freight crossing routes and have been actively planning alternatives.

The utility of designating Preston extension as a Truck Route is compromised by the fact that Wellington Street is not currently designated as a Truck Route between Preston Street and Booth Street in the City of Ottawa's Truck Route Network. This is reflected in Figure 30, showing the current and planned truck route network. Additionally, utilizing the Preston extension to connect freight to the Chaudière Crossing would require the reconstruction of the intersection of Booth Street and Wellington Street to provide an eastbound left turn lane that would accommodate heavy vehicles— a movement which is currently prohibited.

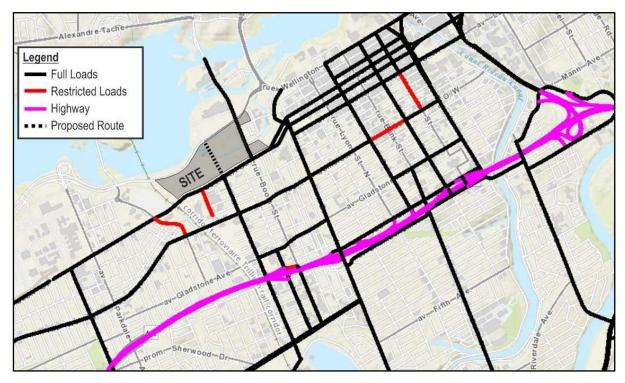


Figure 30: City of Ottawa Truck Route Network

It is worth noting that the NCC's study for the *Long-Term Integrated Interprovincial Crossings Plan for Canada's Capital Region* indicates that a new crossing of the Ottawa River east of downtown has the potential to divert 15% of truck traffic away from the downtown core, while a new crossing west of downtown has the potential to divert 8% of truck traffic away. A truck tunnel connecting Highway 417 to Macdonald-Cartier Bridge has the highest potential for diverting trucks away from downtown, at 35% diversion. The merit of making a significant and potentially contentious change to the truck route network to capitalize on the Preston extension is questionable if there is going to be fewer trucks in the Core Area in the future.



### 4.4.10 Transit Services

City of Ottawa comment: The Preston extension would allow for more flexible routing and staging of transit routes, such as looping around Preston-Wellington-Booth to end a bus route at LeBreton Flats without having to cross to Gatineau. This routing may also help with staging for large events such as Bluesfest or for other events at the potential events centre.

The Preston extension is not a preferred alternative to Booth Street for fixed (i.e., standard day to day) transit routing. The potential benefits of transit routing onto Preston extension cannot be realized for the following reasons:

- There is no connectivity to Pimisi or Bayview LRT Stations, and connectivity from any Preston
  extension to LRT stations would be difficult due to the structure required to cross the aqueduct
  and the Confederation Line.
- There are no transit priority facilities on Wellington Street in the study area.
- There are no eastbound right or left turn lanes onto Booth Street from Wellington Street eastbound to allow buses to continue along the Booth Street corridor.

It is worth noting that the Preston extension would provide value for operational flexibility and redundancy during temporary transit routing (i.e., event or incident driven routing changes), but this redundancy could be replicated by allowing transit to use Slidell Street and the SJAMP during major events or incidents.

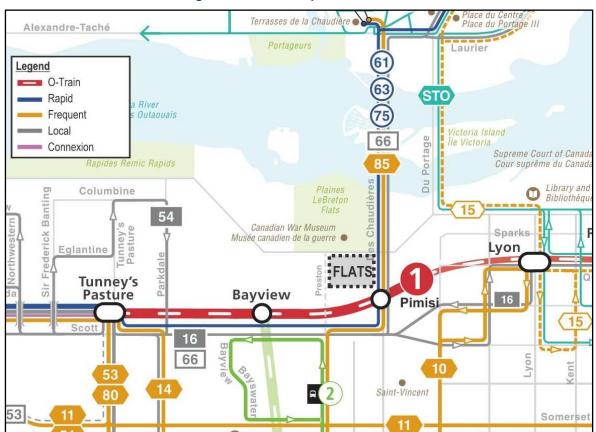


Figure 31: OC Transpo Route Network



# 4.4.11 Emergency Services

**City of Ottawa comment:** The City has mandated response time requirements. If adequate response times cannot be met, additional resources would be required.

A high-level review of nearby emergency services, as shown in **Figure 32**, indicates that all emergency services are less than 15 minutes away. If additional access for emergency vehicles to LeBreton Flats, or more specifically the Flats District, is a concern then it should be noted that **emergency vehicles would not have to abide by turning restrictions at intersections**, such as those at Booth Street, Wellington Street and Slidell Street. Additionally, **the City could choose to provide emergency services on-site at LeBreton Flats**.

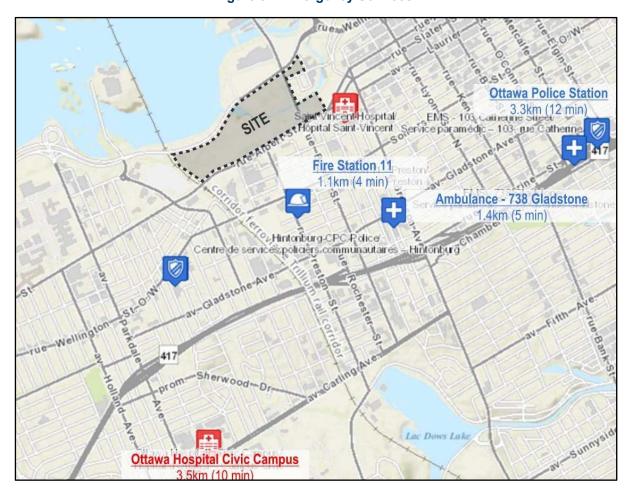


Figure 32: Emergency Services



## 4.4.12 Additional City Concerns with Removal of Preston Extension

The following are a list of additional concerns regarding the potential removal of the proposed Preston extension provided by City of Ottawa staff (in italics) that may require further study above and beyond the scope of this TIA.

**Past Commitments**: The City has made past commitments to stakeholders, including those involved in the East Flats development, regarding the proposed extension of Preston Street.

It is felt by the study team that the analysis contained within this TIA study indicate that there will be minimal impacts on adjacent stakeholders due to the removal of the Preston extension from the OP and TMP, and that there are more benefits to stakeholders with the removal of the proposed Preston extension than there are negative impacts.

Network Resilience - The arterial road network is a critical component within a transportation system. An arterial network not only supports travel to/from the development area, but it also supports through traffic unrelated to the development. Due to the specific geographical constraints of LeBreton Flats, as well as the existing surrounding arterial network, Booth Street is currently the only arterial connecting Wellington Street and Albert Street. If Booth Street becomes disabled, in either the existing or future conditions, for reasons such as a collision, construction, or general maintenance, traffic accessing Wellington Street would need to detour through downtown or to Parkdale Avenue, adding 2 to 5 km of additional travel on routes that already experience at, or near capacity conditions. This would not only have an impact on daily commuters, but would potentially also impact emergency response time, supply chain efficiency and construction vehicles that require access to/from and through LeBreton Flats. The proposed Preston extension is intended to provide network redundancy within this area, which allows for some added network resilience.

It is important to note that the above scenario regarding potential impacts due to Booth Street becoming disabled is an existing condition, as there hasn't previously been an alternative corridor to provide network resiliency for Booth Street. The LeBreton Flats development will be maintaining the status quo and will not be removing network resiliency.

**Greenhouse Gases** - Any potential increase in pollution due to the addition of approximately 400 m of roadway, including construction of a structure over the Confederation Line and aqueduct, may be nullified by the additional travel distance required to use alternate routes.

It is felt by the study team that the analysis contained within this TIA study indicate that the removal of the proposed Preston extension will not require alternate routes for the majority of vehicles accessing the development, with the exception of 3% of all person trips, as identified in Section 4.4.5. Furthermore, the active transportation connections proposed as part of the Master Concept Plan in lieu of the Preston extension provide improved facilities for users of active modes of transportation.



#### 4.4.13 Additional Benefits of the Removal of the Preston Extension

The following are high-level concerns that the LeBreton Flats study team have identified with the proposed Preston extension, which need to be taken into consideration and discussed alongside the concerns identified by the City.

Construction Cost - Previous plans for the proposed Preston extension did not detail how the sectional and grading complexities could be accommodated, from either a design or financial perspective. Due to the position of the Confederation Line, a Preston Street arterial bridge would have to be over 150 metres in length, which would be twice the length of the Booth Street bridge. The cost to the City of constructing such a large arterial bridge would be significant (estimated somewhere in the order of \$35 million in 2020 dollars by O2 Planning + Design), which is a significant increase compared to the \$14.2 million cost estimated for the Preston extension carried in the City's Development Charge Background Study.

Connectivity within LeBreton Flats - A Preston Street arterial bridge would reduce road and pathway connectivity within LeBreton Flats. Due to the elevated nature of the bridge, new internal roads and paths within LeBreton Flats could not be sloped to connect with the bridge (refer to Section 4 of the Planning Rationale for further details). The construction of a Preston extension could cause the open aqueduct to be isolated behind a retaining wall or steep embankment. Designing the bridge structure in a way that maintains maintenance vehicle access to this essential infrastructure would likely require a longer bridge span with higher clearance than previously anticipated. This reality would impose significant additional infrastructure costs and cause significant detrimental impacts to the public realm and desirability of the neighbourhood adjacent to this critical infrastructure, as a large vehicle access route would have to snake through the area, compromising the ability to achieve a pleasant, safe, and human-scaled urban condition.

While pathways would still be able to get beneath a bridge, they would be constricted to the opening beneath the bridge. There would be no connections between the west end of the Flats and the City's municipal park, making the space between the inlet and the Preston extension very isolated. Retaining walls would separate the municipal park from the Flats and Aqueduct Districts, reducing the internal connectivity of the Flats since the Preston extension would not be able to connect to the internal roads The road network would have to be redesigned to allow for adequate access to the Waterworks facility, the municipal park and LeBreton Place.

**Reduction in Greenspace** - Additional roads in the development would be required to create vehicle access to the Parks District and LeBreton Place for maintenance and accessible parking, reducing the amount of greenspace provided in LeBreton Flats.

Reduction in Density - It is expected that the bridge would require extensive retaining walls along lands slated as part of the LeBreton Flats development area. This would significantly reduce development potential (by approximately 2,000 m² of land) and compromise the ability of the district to meet the density targets outlined in the new Official Plan.

**Filtered Permeability** - The inclusion of a pedestrian and cycling bridge in place of the potential Preston Street bridge is a prime example of the principle of filtered permeability in action in a local context, benefitting users who choose active transportation modes over those who choose the vehicular mode.

Public Realm Design – The additional space provided by the removal of the vehicular bridge will contribute to more appealing public spaces in LeBreton Flats. This will allow for an improved design of the public realm, including safety improvements such as Crime Prevention Through Environmental Design (CPTED) and better lighting, which support the City's 2021 Woman and Gender Equity Strategy. Additional details on design of the public realm are available in the Planning Rationale.



#### 4.4.14 Preston Street Extension Conclusions

Modelling and analysis show that the drawbacks of the Preston extension far outweigh the benefits. Constructing the Preston extension would divert and potentially induce additional traffic to the area, worsening traffic conditions around LeBreton Flats. The Preston extension would draw traffic away from roads that are well beyond the influence area of LeBreton Flats (i.e., as far west as Pinecrest Road), but would not relieve traffic on Booth Street south of Albert Street. It would also result in an increase in traffic in both directions on Preston Street, Rochester Street and Booth Street and cause a deterioration in intersection operations on the Preston Street corridor (including one intersection operating with a LOS 'E' and one with a LOS 'F') that are more significant than the marginal improvement in intersection operations along the Booth Street corridor. In addition, the removal of the Preston extension avoids the need to designate Wellington Street as a Truck Route, which is consistent with the prohibition of heavy vehicles on the SJAMP, and increases accessibility to the LRT stations to ensure the achievement of the high transit mode share targets set as part of the development. For these reasons, the deletion of the Preston vehicular extension from the City's Official Plan is recommended.

# 4.5 Step 3 Findings

The main objectives of Step 3 are to estimate projected site-generated trips based on the proposed development; identify potential impacts site-generated trips will impose on the surrounding transportation network; and to identify any potential modifications that would be required to achieve an acceptable Level of Service (LOS) for the surrounding transportation network. Based on the foregoing, the following findings are offered:

- A total of four potential development scenarios were envisioned for LeBreton Flats and based on an analysis of projected site trip generation, the ultimate build-out of Scenario 4 was projected to generate the most site-generated traffic, with an estimated two-way total of 4,810 and 8,112 person trips/h during weekday morning and afternoon peak hours, respectively.
- Based on discussions with City staff and to remain consistent with assumptions used for TIA studies prepared for other area development sites, the projected modal split of site-generated traffic for the subject development was assumed to be 15% auto driver; 5% auto passenger; 60% transit; and 20% walk/cycling.
- The resulting projected site-generated two-way vehicle volumes is approximately 718 veh/h during the AM and 1,211 veh/h during the PM.
- The resulting projected site-generated two-way transit trip volumes is approximately 2,886 trips/h during the AM and 4,868 trips/h during the PM.
- The resulting projected site-generated two-way trip volumes by active mode is approximately 986 trips/h during the AM and 1,646 trips/h during the PM.
- Given most transit trips begin or end as an active mode, it can be expected that approximately 3,872 trips/h and 6,514 trips/h will be made to/from/within LeBreton Flats as an active mode during weekday morning and afternoon peak hours, respectively.
  - The concept design for the site has maximized the width of pedestrian and cycling facilities wherever possible in order to accommodate the high volume of active trips. Additionally, Synchro analysis for the intersections has assumed that pedestrian phases are called every cycle at all intersections around the study area. Section 5.6 below addresses additional improvements that can be made to City of Ottawa facilities



to improve the pedestrian and cycling LOS, such as leading pedestrian intervals and No Right Turn on Red at signalized intersections.

- Background traffic volumes have exhibited limited growth, as Ottawa's downtown arterial road
  network generally operates at capacity during peak hours. However, projected site-generated
  traffic from planned area developments were explicitly accounted for in the analysis of future
  conditions.
- Historical traffic count data from the year 2014 was used for analysis purposes, given recent network impacts related to LRT construction.
- A study area intersection performance assessment revealed that the Wellington Street / Portage Bridge intersection is currently operating over capacity during weekday morning peak hours. The Scott Street / Parkdale Avenue intersection is approaching capacity in the afternoon peak hour.
- Possible measures to improve the performance of study area intersections while prioritizing
  active modes include the construction of the West Gatineau Tramway, re-designating bus
  lanes as general traffic lanes, Stage 2 LRT extension and improvements to cycling facilities on
  Albert Street / Scott Street.

The Preston extension results in an increase in traffic in both directions on Preston Street, Rochester Street and Booth Street and causes a deterioration in intersection operations on the Preston Street corridor that do not justify the marginal improvement in intersection operations along Booth Street corridor. The potential impacts associated with the removal of the Preston extension are acceptable.

The results of this analysis indicate that there will be traffic challenges at some study area intersections, including Booth Street at Chaudière Crossing, Booth Street at Wellington Street, Booth Street at Albert Street, Albert Street at Preston Street, Scott Street at Parkdale Avenue and Wellington Street at Portage Bridge. Of these intersections, Booth Street at Albert Street, Albert Street at Preston Street, Scott Street at Parkdale Avenue and Wellington Street at Portage Bridge are over capacity, the others are approaching capacity. It will ultimately be up to the City of Ottawa to determine if the projected incremental changes in the performance of the road network will be acceptable, especially when considering that the Wellington Street at Portage Bridge is already over capacity in the Existing Conditions.

It is important to note that not all decisions need to be made at this time due to the size of the LeBreton Flats development (e.g., the City could consider development applications for phases within LeBreton Flats regardless of the status of the Preston extension in the Official Plan), as there will be ample opportunities for refinement to the transportation analysis as each parcel of land is developed and undergoes its own TIA process, including submission for approval. It should also be noted that given the significant timelines for the ultimate build-out of this project, it is important to recognize that travel patterns will change as projects like the Stage 2 Confederation Line LRT extension, West Gatineau Tramway, downtown transit loop and Ottawa River Sixth Crossing are designed and constructed, as well as City of Ottawa guidelines and targets.



# 5. STEP 4 – ANALYSIS

# 5.1 Exempted Modules

As noted in Section 3.3, the following modules have been exempted from this TIA after discussions with the City: 4.1 Development Design, 4.2 Parking, 4.3 Boundary Streets, 4.4 Access Intersections. These modules will be submitted in the future as part of the TIA analysis for individual development parcels.

# 5.2 Transportation Demand Management

#### 5.2.1 Context for TDM

The proposed mode share of the development, as outlined in Section 4.1.1, is 15% auto driver, 5% auto passenger, 60% transit, and 20% walking and cycling. Comparatively, the mode share in the City's EMME model for TAZ 300 which is mostly made up of the LeBreton Flats development, is 42% auto driver, 10% auto passenger, 39% transit, and 9% walking and cycling. Through discussions with the City, it was agreed that the model is underrepresenting the potential level of transit usage in TAZ 300, especially for trips arriving to TAZ 300, which are shown as only 28% transit in the model.

With the LeBreton Flats location just west of downtown, it falls under the "Central Area" definition of the Official Plan, but with two LRT stations located within the site, it can also be considered a transit-oriented development (TOD). This allows the development to place a greater emphasis on non-auto modes, as there are no minimum parking requirements for the development. The ultimate decision for providing parking is up to each individual developer, however the Planning Rationale makes numerous mentions of a desire for minimal parking, and where required, implementing shared parking between land uses.

# 5.2.2 Need and Opportunity

It is clear that to meet the above noted mode share targets that an aggressive TDM program is required. The following are three key points to consider for the development of the TDM program for LeBreton Flats.

- Other similar Transit-Oriented Developments in the City have had similar targets to what is being proposed for LeBreton Flats. Those developments are listed below along with a highlevel summary of the proposed TDM measures for each development:
  - 900 Albert Street 25 to 30% auto driver, 5 to 10% auto passenger, 45 to 55% transit, 15% active.
    - Enhanced sidewalks and lighting, ride-sharing programs, carpool incentives, preferential parking for hybrid/electric vehicles, on-site transit information booth, subsidized transit passes; additional shelter area for transit users; onsite change rooms/shower facilities.
  - Zibi 25 to 30% auto driver, 5% auto passenger, 45 to 55% transit, 20% active.
    - o Small development blocks with frequent intersections, pedestrian streets and woonerfs, secure bicycle parking, parking minimums with shared parking between buildings/land uses, car sharing programs/facilities, provide information/material to future residents and employees to educate them on sustainability objectives.



- CFB Wateridge Development 45 to 50% auto driver, 10% auto passenger, 30 to 35% transit, 20% active.
  - Ride-sharing programs, carpool incentives, preferential parking for hybrid vehicles, on-site transit information booth, on-site change rooms/shower facilities
- 2. The City's continuous monitoring and interest of these types of developments as they are built confirms that the mode share targets are quite favorable compared to the rest of the City, but do fall short of the TOD targets.
- 3. Committing to an aggressive TDM program is necessary and prudent, with the recognition and understanding that some TDM measures will be attractive and effective from the outset, while others will become more attractive as the development progresses and nears completion.

The main opportunity for the LeBreton Flats lands is that the NCC is a willing and committed landowner, willing to put forth an attractive and aggressive TDM plan that will help to create the vision for LeBreton Flats being presented in this and other reports. Other opportunities to be considered as part of the LeBreton Flats development are:

- The NCC is committed to working with OC Transpo to pursue strategies that boost transit
  mode share to and from LeBreton flats, including methods to encourage/incentivize developers
  and future residents to use transit. This would provide a great jump-start on encouraging transit
  usage and could be supported by transit fare incentives for non-residential developments at
  LeBreton Flats.
- According to Section 101 of the City's Zoning By-law, no off-street motor vehicle parking is
  required to be provided on the entire site, given the proximity of the development to LRT
  stations.
  - According to Section 103 of the City's Zoning By-law, there is a maximum number of motor vehicle parking permitted at the LeBreton Flats site, due to its proximity to LRT stations. These numbers equivalent to 1.5 parking spaces per dwelling unit and 1.0 per 100m² of GFA for office land uses and retail stores. This would translate to a maximum allowable number of parking spaces on-site of approximately 7,000.
  - This is significantly higher than the number of vehicular trips expected to be generated by the site (approximately 1930 entering and exiting during the AM and PM peak hours) indicating that it is important that maximum parking provisions on-site be more stringent than those outlined in the City's Zoning By-law.
- The minimum number of bicycle parking spaces as required by Section 111 of the City's Zoning By-law are 0.5 per dwelling unit and 1 per 250m2 GFA for an office or retail store. This would result in approximately 2,400 bike parking spaces on-site.
  - Given that 15% of trips are expected to be made by auto drivers, and 20% of trips are expected to be made by active modes, it would be worthwhile to provide an equivalent or greater number of bicycle parking spaces on-site when compared to vehicular parking spaces.



## 5.2.3 TDM Program

According to the City's TIA Guidelines, an analysis of Transportation Demand Management (TDM) measures is required when a proposed development is projected to have more than 60 employees onsite at any given time. It is understood that the City generally prefers a post-occupancy TDM program be in place ahead of site plan approval; however, with different parcels of land likely to have different owners or developers, it is difficult to project which TDM measures will be used by each owner. The proposed design of the LeBreton Flats site encourages active modes of transportation as much as possible, as outlined in detail in this TIA, by using measures such as filtered permeability, numerous multi-use pathways and sidewalks, and woonerf or slow streets design.

It is expected that a TDM strategy will be established for each individual development application at the time of development approval. Given that this TIA is for the entire site, and that individual TIAs will be required for each individual development, it is recommended that the City take a closer review of TDM programs at that stage of the planning process. Many of the TDM programs are specifically related to operations of a specific company or developer, such as offering discounted transit passes or flexible working hours, which cannot be captured in this TIA. That being said, some potential TDM-supportive measures that can be considered for LeBreton Flats are listed below:

- Travel Surveys The NCC could commission travel surveys / monitoring programs to be undertaken at intervals throughout the development of LeBreton Flats in order to gauge the mode share and make adjustments to requirements accordingly. For example, such surveys could be undertaken at 20% completion intervals (i.e., a 20-year development would be undertaken every 4 years).
- Enhanced Public Transit Service Given the existing presence of OC Transpo routes on Booth Street, Albert Street and Preston Street, as well as the Confederation and Trillium Lines, it is expected that OC Transpo will be monitoring transit usage in and around LeBreton Flats throughout the development process. Section 5.4 of this TIA provides additional discussion on transit capacity in and around LeBreton Flats.
- On-Site Amenities The mixed-use nature of the LeBreton Flats development suggests that
  a variety of amenities and services will be available on-site, which will reduce need for and
  dependency on personal vehicles.
- Parking-Related Strategies The following are some TDM measures specifically related to vehicular parking management.
  - A maximum limit on parking supply (either a per unit rate or maximum stalls per development) more aggressive than the City's Zoning By-law.
  - Charge for all parking (i.e., short-term, and long-term parking), with short-term parking being charged at a higher parking rate.
  - Provide carpool and carshare vehicles with discounts on parking costs and/or provide more of them with more convenient parking locations.
  - Unbundle parking cost from commercial/office lease rates, residential purchase prices and monthly rent. Alternatively, the NCC (or another entity, such as a private company) could maintain control of all parking on-site.
- In addition to the above, there are numerous TDM measures that can be included as a requirement for each individual development as part of the procurement process. These measures tend to be physical measures that would have to be constructed / installed as part of each development. They include:



- Displaying local area maps with walking/cycling access routes, key destinations, transit schedules and route maps at major entrances.
- Provide real-time transit arrival information display at entrances to buildings in LeBreton Flats.
- o **Install on-site bikeshare stations** for use by commuters and visitors.
- Generous provisions for secure bike parking.
- Minimum sidewalk widths above and beyond City standards.
- Curb management accommodation (e.g., percentage of curb space dedicated to pick-up/drop-off activity).
- Minimum bicycle parking provisions that are higher than the City standard (e.g., 2+ bike parking stalls per residential unit)
- Mandating bicycle maintenance and repair facilities and end-of-trip amenities (e.g., showers and change rooms).

The formal TDM Checklist, provided by the City, has been attached as **Appendix H** and is filled out for measures that may be applicable to the LeBreton Flats site. It is worth reiterating that it is difficult to project which specific measures will be utilized by individual developments.

# 5.3 Neighbourhood Traffic Management

With respect to the City's TIA guidelines, this module reviews significant access routes to the development and identifies any required neighbourhood traffic management (NTM) measures to mitigate impacts on collector and local roads.

## 5.3.1 Adjacent Neighbourhoods

Given projected traffic volume on Wellington Street, Booth Street, Albert Street and SJAMP are currently, and are anticipated to continue to exceed the major arterial capacity thresholds (i.e. 600 veh/h per lane during peak hours), the City's TIA Guidelines requires a review of potential neighbourhood traffic management strategies for the adjacent neighbourhoods, including West Centretown (generally bounded by Albert Street to the north, Carling Avenue to the south, Bronson Avenue to the east and the Trillium Line to the west), Centretown (generally bounded by the Ottawa River to the north, Highway 417 to the south, the Rideau Canal to the east and Bronson Avenue to the west) and Hintonburg (generally bounded by the Ottawa River to the north, Highway 417 to the south, the Trillium Line to the east and Parkdale Avenue to the west).

The Master Concept Plan carefully and deliberately minimizes the need for neighbourhood traffic management strategies within the LeBreton Flats site. The residential neighbourhoods south of the development site will feel some additional pressure from the additional traffic generated by vehicles to/from LeBreton Flats, especially if the Preston extension is implemented. These neighbourhoods already experience streets with long queues of traffic during peak hours and have existing area traffic management measures in place to reduce the potential for cut-through traffic. There is the potential for peak period spreading, which means that the queues of traffic will start earlier and/or finish later in the day, albeit with less pronounced peaks in traffic. Surrounding residential streets are for the most part already protected against cut-through traffic issues as outlined in Section 3.1.2.

The arterial roads surrounding the development site are the most likely to experience off peak speeding due to their alignment and width. Currently, the primary function of these roads is mobility, and therefore, the design elements prioritize the efficient movement of motor vehicles. For example, intersections need to facilitate truck turning, which can result in wider crossing distances for pedestrians. Some intersections require vehicle turning lanes, which increase the crossing distances



for pedestrians. That being said, there are many strategies that can be employed to promote the comfort and security of other road users. Albert Street is expected to undergo significant design changes that will include segregated facilities for pedestrians and cyclists.

#### 5.4 Transit

With respect to the City's TIA Guidelines, this module reviews the potential impacts on existing and planned transit networks and service to ensure that level of service is not unacceptably impacted.

## 5.4.1 Route Capacity

The transit routes that serve the subject site were previously summarized in Table 2. It is expected that 60 percent of the trips generated by the site will be accommodated by transit, and that the majority of transit usage for people accessing the development site will be completed by LRT (either Confederation Line or Trillium Line). It is expected that Bayview Station on the western edge of the development site will service the Park District and the western portion of the Albert District, including the major event centre (if constructed). The Flats District and Aqueduct District will be well served by Pimisi Station on Booth Street.

Based upon the analysis provided in Step 3, and summarized in the table below, it is expected that the number of transit trips generated during each of the three phases of the LeBreton Flats development will range from 1,500 to 3,700 additional transit trips in both peak hours. This will result in an approximate total of 7,750 additional transit passengers generated by the LeBreton Flats development during the peak hours.

**AM Peak Hour PM Peak Hour** Total **Block** Peak In Out **Total** In Out **Total** Hour 520 836 770 1604 Phase 1 Total 414 933 2537 Phase 2 Total 213 526 505 453 957 313 1483 Phase 3 Total 589 1427 1042 1266 2307 837 3734 **Total 'New' Transit Trips** 1464 1422 2886 2383 2489 4868 7754

Table 28: Peak Hour Transit Trips by Development Phase

Using information from the City of Ottawa's EMME model, the following breakdown was calculated for transit users around the study area in order to assume a distribution to various LRT or bus routes. It should be noted that the numbers in **Table 29** include all transit users, not just those from LeBreton Flats.

**Eastbound** Westbound **Transit Mode** Volume Percentage Volume **Percentage** LRT 28,146 97% 9557 95% Bus 930 3% 543 5% Total 29,076 100% 10,100 100%

Table 29: EMME 2031 Transit Trip Distribution - AM Peak Hour

The data provided in Table 29 indicates that of all eastbound transit trips in the AM peak hour, 97% are made by LRT and 3% are made by bus. 95% of all westbound transit trips are made by LRT and 5% are made by bus. Of all LRT trips, 67% are headed eastbound and 33% are headed westbound, while bus trips see 65% of trips headed eastbound and 35% of trips headed westbound. This information allows for the assignment of transit trips to stations and bus stops in the area as shown in **Table 30** and **Figure 33** below.



Table 30: Projected LeBreton Flats Transit Trip Distribution – Full Buildout

			AM Pea	ak Hour					PM Pea	ak Hour		
Block		In			Out			In			Out	
	Total	EB	WB	Total	EB	WB	Total	EB	WB	Total	EB	WB
A1-4 (Major Events Centre)	206	138	68	244	163	81	398	132	266	368	122	246
LRT (Bayview)	200	134	66	237	159	78	378	125	253	350	115	235
Bus	6	4	2	7	4	3	20	7	13	18	7	11
A9-10	127	85	42	164	110	52	282	93	189	261	87	174
LRT (Pimisi)	123	82	41	159	107	52	268	88	180	248	82	166
Bus	4	3	1	5	3	2	14	5	9	13	5	8
Flats District (F1-4, 8-12, AD 1-5)	605	404	201	538	360	178	928	308	620	995	330	665
LRT (Pimisi)	587	393	194	522	350	172	882	291	591	945	312	633
Bus	18	11	7	16	10	6	46	17	29	50	18	32
A5-6	65	43	22	108	72	36	144	48	96	124	41	83
LRT (Pimisi)	63	42	21	105	70	35	137	45	92	118	39	79
Bus	2	1	1	3	2	1	7	3	4	6	2	4
A11-12	77	51	26	102	68	34	191	64	127	177	58	119
LRT (Pimisi)	75	50	25	99	66	33	181	60	121	168	55	113
Bus	2	1	1	3	2	1	10	4	6	9	3	6
A7-8	364	244	120	212	142	70	395	131	264	533	177	356
LRT (Pimisi)	353	237	116	206	138	68	375	124	251	506	167	339
Bus	11	7	4	6	4	2	20	7	13	27	10	17
F5-7	23	16	7	54	36	18	45	15	30	31	11	20
LRT (Pimisi)	22	15	7	51	34	17	44	15	29	29	10	19
Bus	1	1	0	3	2	1	1	0	1	2	1	1
LRT	1300	871	429	1379	924	455	2265	748	1517	2364	780	1584
Bus	41	25	16	43	27	16	118	43	75	125	46	79



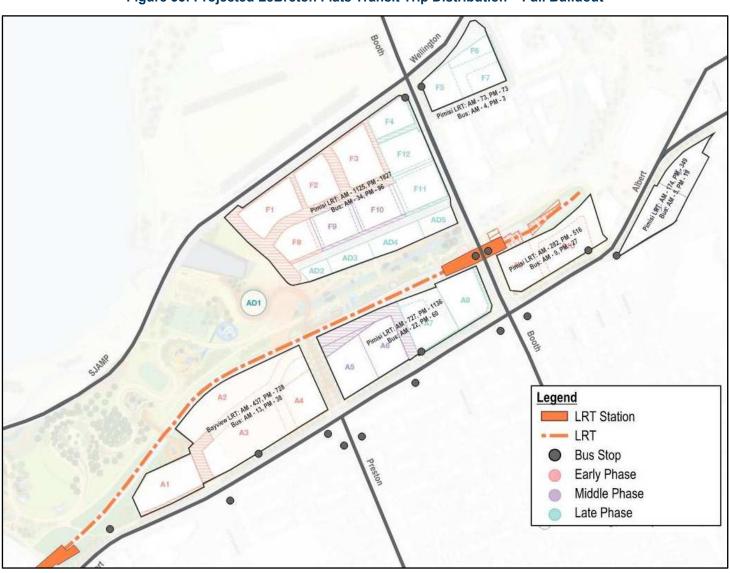


Figure 33: Projected LeBreton Flats Transit Trip Distribution – Full Buildout



The full build-out of the LeBreton Flats development is expected to generate approximately 2,680 LRT trips in the AM peak hour and 4,630 trips in the PM peak hour. These trips are weighted slightly more towards trips leaving LeBreton Flats than trips entering LeBreton Flats. It is important to note that not all new riders will be on the LRT at the same time. For example, in the morning peak hour at LeBreton Flats there will be 871 new eastbound riders boarding the LRT, and 924 new eastbound riders departing the LRT. Therefore, the net increase in LRT riders is not 1,785 riders, it is somewhere between 871 and 924 riders depending which section of the LRT is reviewed. With the current LRT capacity of 10,700 passengers per hour one way, the trips generated by LeBreton Flats would represent approximately 9% of eastbound and 4% of westbound capacity in the morning, and 7% of eastbound and 15% of westbound capacity in the afternoon. It is worth noting the City is expecting an increase in planned capacity of the LRT to 36,000 passengers per hour by 2031, and 48,000 passengers per hour at ultimate build out<sup>4</sup>, and that at the time of the development of the Confederation Line Environmental Assessment the LeBreton Flats redevelopment was a known entity and our understanding is that it was included in the development of the planned future LRT capacity. The City's 2031 EMME model projects 28,146 eastbound passengers on the LRT in the morning peak hour, which includes riders from LeBreton Flats, With a capacity of 36,000 passengers per hour, 28,146 passengers would be at 78% capacity, indicating the LRT can comfortably accommodate the increases in passengers from the full build-out of the LeBreton Flats development.

The full build-out of the LeBreton Flats development is expected to generate approximately 80 bus trips in the AM peak hour and 240 in the PM peak hour. These trips are split fairly even between trips into LeBreton Flats and trips out of LeBreton Flats. Assuming a similar transit plan and bus routings to the existing plan shown in Table 2, it can be expected that the additional trips to buses will be distributed as follows:

- Eastbound AM (includes buses to Gatineau): 32 buses per hour = 1 new rider per bus.
- Westbound AM (includes buses from Gatineau): 47 buses per hour = 1 new rider per 3 buses.
- Eastbound PM: 47 buses per hour = 1 new rider per bus.
- Westbound PM: 32 buses per hour = 2 new riders per bus.

When compared against the City's EMME model, the expected increase in transit customers due to the full build-out of the LeBreton Flats development represents a less than 3% increase in bus passengers.

### **5.4.2 Transit Priority**

Given that the fully grade separated Confederation Line bisects the LeBreton Flats development lands transit travel times should be unimpeded. Additionally, both the Trillium Line and the proposed West Gatineau Tramway are approximately a 10-minute walk from the centre of the LeBreton Flats development lands. Therefore, additional bus transit priority measures are not required as part of this study.



<sup>&</sup>lt;sup>4</sup> https://www.octranspo.com/en/ready-for-rail/o\_train\_confederation\_line\_system\_faqs

# 5.5 Review of Network Concept

With respect to the City's TIA Guidelines, this module determines if changes to the Transportation Master Plan (TMP) concepts for auto or transit networks are required to accommodate the development-generated travel demands.

The purpose of this section of the TIA is to outline any changes to the existing or planned transportation network that are required due to added traffic from a new development. It is important to recognize that the existing arterial road network, serving the area of LeBreton Flats, is generally approaching or over capacity during the peak periods. The LeBreton Master Concept Plan proposes a plan that will rely heavily on active modes and the transit network to service the community's transportation needs. Nonetheless, there will be an additional automotive burden placed on the surrounding arterial network as a result of the proposed development.

Creating additional roadway capacity within the central area of Ottawa is not considered a priority for the Transportation Master Plan, nor is it practical in a constrained urban environment such as in the vicinity of LeBreton Flats. Therefore, the assumption is that additional roadway capacity will not be provided as part of this development. That being said, the addition of a vibrant central urban community as proposed in the Master Concept Plan will increase the City's active and transit mode share statistics, helping to achieve TMP mode share objectives. Further, the Master Concept Plan supports the City's objectives of increasing the number of roadways that can be defined as "Complete Streets". Providing a supportive environment for pedestrians and cyclists will improve the capacity of the active transportation network and help to improve active mode share.

The Master Concept Plan does deviate from the TMP in that it has eliminated the proposed Preston Street extension between Albert Street and Wellington Street for auto modes. The link is proposed to remain for active modes only. This deviation is described in detail in the Planning Rationale (prepared by O2 Planning + Design and submitted under separate cover). From a transportation perspective, the elimination of this link has several implications which are explored in Section 4.4.

# 5.6 Intersection Design

This module determines the design elements of study area intersections required to accommodate the proposed development, consistent with the City's Complete Streets philosophy and MMLOS practices.

### **5.6.1 Intersection Control**

All study area intersections are currently traffic signal controlled and are more or less fully built out. Based on the City's policies, goals and objectives, additional road widenings or intersection control is not supported. However, several area intersections will continue to operate over capacity. The following are some possible measures, previously identified in Section 4.3.2. that may provide a reduction in vehicular demand at these intersections, without requiring additional roadway infrastructure:



## Potential Mitigation Measures that Prioritize all Modes of Transportation

### **Transit Projects**

- When the future West Gatineau Tramway is in place across the Portage Bridge in 2028, trips across
  the Portage Bridge may be shifted away from the vehicular mode and towards the transit mode. It
  is recommended that the City monitor traffic volumes at the intersection of Wellington Street and
  Portage Bridge and respond to a reduction in vehicular traffic accordingly.
- Confederation Line Stage 2 LRT (with improved reliability extension drawing additional trips when open in 2026)

### **Active Mobility Projects**

Further improvements to vehicular LOS may be observed as trips are shifted to alternative modes of transportation or alternative corridors as major projects within the National Capital Region are completed. Projects that may reduce the vehicular demand in the study area include:

- Improvements to cycling facilities within LeBreton Flats and along Albert Street into downtown (mode shift to cycling). This would improve operations at most intersections in the study area, with a specific benefit to Booth Street at Albert Street, Albert Street at Preston Street and Parkdale Avenue at Scott Street.
- Construction of the Chief William Commanda multi-use pathway interprovincial bridge (mode shift to walking and cycling), which will provide an attractive alternative route for pedestrians and cyclists to travel between Ottawa and Gatineau.

### 5.6.2 Intersection Design

Intersection details are typically not part of master concept plans; however, it is expected that connections to the boundary road network will be designed to the latest standards/guidelines (e.g., adequate turning radii will be provided for trucks, sufficiently long driveway clear throat lengths will be provided, etc.). Intersections are shown to be located at appropriate distances from existing intersections, and signalization is suggested at a minimal number of locations to provide for protected movements to/from the LeBreton Flats development. The approximate location and design of new driveway connections will be refined during the development application process. Nevertheless, the following is a MMLOS analysis for the planned signalized access intersections to/from LeBreton Flats.

## Intersection MMLOS Summary

A Multi-Modal Level of Service (MMLOS) assessment was conducted for the subject site's boundary intersections, to gauge the extent of risk, comfort and stress for active modes and gauge the extent of impedance, delay and reliability for trucks/buses. **Table 31** provides an MMLOS summary for existing conditions for all modes, including Pedestrian (PLOS), Bike (BLOS), Transit (TLOS) and Truck (TrLOS) at signalized intersections. Target MMLOS values were identified in Table 14 and are identified at the bottom of each street in the table. **Table 32** summarizes the projected intersection MMLOS with planned network improvements, as outlined in Section 3.1.3. The detailed assessment is included as **Appendix I**.

One important note regarding the PLOS and BLOS is that this review focuses on existing city streets, including Wellington Street and Albert Street. Therefore, it does not accurately reflect the robust segregated pathway network that is included as part of the Master Concept Plan, as shown in Figure 29. This pathway network will allow active transportation users to avoid travelling on busy vehicular corridors such as Wellington Street and Booth Street, providing them with a level of risk, comfort and stress that would be comparable to a LOS 'A'.



Table 31: Intersection MMLOS - Existing LOS

Main Street	Cross Street	PLOS	BLOS	TLOS	TkLOS	AutoLOS
	Vimy Place	D	F	-	-	Α
Wallington	Booth	D	Α	F	D	D
Wellington	Lett	F	F	-	-	Α
	Target	А	С	D	D	Е
	Booth	F	F	F	F	D
Albort	Preston	Е	F	Е	F	D
Albert	City Centre	F	F	В	Е	Α
	Target	Α	Α	С	D	E

Table 32: Intersection MMLOS - Projected LOS

Main Street	Cross Street	PLOS	BLOS	TLOS	TkLOS	AutoLOS
	Vimy Place	D	F	-	-	Α
	Broad	F	F	-	-	Α
Wellington	Booth	F	Α	F	D	Е
	Lett	F	F	-	-	В
	Target	А	С	D	D	E
	Empress	F	В	F	Ī	А
	Booth	F	F	F	F	F
Albert	Access 1	F	F	F	-	С
Albert	Preston	Е	F	Е	F	F
	City Centre	E	В	С	E	В
	Target	А	А	С	D	Е

As shown in Table 31, outside of AutoLOS, study area intersections currently do not meet LOS targets with a few exceptions. Takeaways regarding the Intersection MMLOS for the Future Conditions are noted below.

### **Pedestrian LOS**

- It will be difficult to improve Pedestrian LOS at intersections without reducing the number of vehicular lanes. If Albert Street is reconstructed to remove bus lanes in the future, this will have a positive impact on the PLOS.
  - The removal of the existing parking lanes on Wellington Street between Vimy Place and Portage Bridge would also benefit pedestrians as it reduces their crossing distance.
- The only existing intersections without zebra stripe hi-visibility markings are Wellington Street
  at Vimy Place and at Broad Street. It is recommended that this be rectified once Vimy Place
  and Broad Street are extended to the south side of Wellington Street as part of the
  development.
- Any new intersections, such as Albert Street at Access 1, should implement zebra stripe hi-vis markings.
- The implementation of leading pedestrian intervals (LPIs) at intersections that operate well is recommended, such as at Booth Street at Chaudière, Wellington Street at Vimy Place, Albert



- Street / Scott Street at Bayview Station Road, Albert Street at City Centre Avenue and Wellington Street at Lett Street.
- The implementation of No Right-Turn-on-Red provisions at intersection that operate well is also recommended.
- As an example, if all the above recommendations (i.e., remove parking lanes, zebra stripe markings, leading pedestrian intervals and no RTOR) are implemented at the intersection of Wellington Street / Lett Street, the PLOS will improve from LOS 'F' to LOS 'D'.
- As noted above, a robust network of multi-use pathways is proposed as part of the Master Concept Plan, including a recently-built east-west pathway along the north side of the Confederation Line. This pathway provides pedestrians with a more comfortable, safer route through the area, and will ultimately connect to facilities further east on Wellington Street and Albert Street.

# Bicycle LOS

- If protected intersections are provided along Albert Street, the Bike LOS will improve from LOS 'F' to LOS 'B'. Although this will not meet the target LOS 'A', this is a significant improvement over existing conditions.
- Although not scored in the MMLOS, where protected intersections are provided, and intersection operations allow for it, leading bike intervals should be provided alongside the LPIs.
- As noted above, a robust network of multi-use pathways is proposed as part of the Master Concept Plan, including a recently built east-west pathway along the north side of the Confederation Line. This pathway provides cyclists with a more comfortable, safer route through the area, and will ultimately connect to facilities further east on Wellington Street and Albert Street.

### **Transit LOS**

- Transit LOS is projected to worsen due to the removal of temporary dedicated transit facilities along Albert Street. However, LRT access along this corridor should help supersede the need for improvements to increase transit LOS targets.
- Failing TLOS is mainly attributed to vehicle movements experiencing long delays, which impact bus travel time/reliability.
- Transit queue jump phases may be possible along Albert Street at locations where an auxiliary right-turn lane is provided. This lane could also function as a queue jump lane, using a cigar signal which would allow buses to jump to the head of the vehicular queue.

## **Truck LOS**

- Truck LOS is generally guided by corner radii and the number of receiving lanes.
- As corner radii increases truck LOS improves; however, this negatively impacts the
  pedestrian LOS. As the pedestrian target LOS is higher for this study area the pedestrian LOS
  should take precedence in this case, unless compromising the accommodation for trucks will
  result in a reduction in safety for pedestrians.
- The Booth Street / Albert Street and Preston Street / Albert Street intersections are locations
  with high truck turning volumes due to the designated truck routing through the area. It would
  not be reasonable to reduce the curb radii at these intersections to improve the PLOS, as this
  would result in trucks driving over the curb and potentially putting pedestrians at significant



- safety risk (e.g., the high pedestrian/truck Waller-Rideau-King Edward corridor is well known to City staff as a problematic truck route through Ottawa's dense urban core).
- Long-term improvements to this Truck LOS in this area may be possible with the implementation of a sixth crossing of the Ottawa River, allowing the full removal of truck routing across Booth Street / Chaudière Crossing.



#### 6. CONCLUSION AND RECOMMENDATION

The future community of LeBreton Flats has the potential to be a showcase for future urban development in Canada. As with any urban development of this caliber, there is both enormous potential and significant challenges. It is important to understand the value of the site, as failure to do so may unreasonably deem some elements as challenges and miss the opportunity to undertake proper trade-off analysis, therefore unnecessarily compromising the full potential of the site.

This Transportation Impact Assessment followed the City of Ottawa TIA Guidelines to assess and evaluate the potential benefits and impacts that are anticipated to City of Ottawa roadways as part of the Building LeBreton Flats development.

LeBreton Flats development is anticipated to generate approximately 4,800 person trips in the weekday morning peak hour, and 8,100 person trips in the weekday afternoon peak hour. The development is targeting aggressive modal splits for site generated traffic, including 15% auto driver trips, 5% auto passenger, 60% transit trips and 20% active transportation trips. This results in an expected increase in peak hour vehicle traffic onto adjacent roadways in the order of 700 vehicles per hour in the morning and 1200 vehicles per hour in the afternoon.

Potential measures that may improve the performance of study area intersections while prioritizing active modes include the construction of the West Gatineau Tramway, re-designating bus lanes as general traffic lanes, completion of the Stage 2 LRT extension and improvements to cycling facilities on Albert Street / Scott Street. The proposed Preston Street extension from the City's Transportation Master Plan is expected to result in an increase in traffic in both directions on Preston Street, Rochester Street and Booth Street and causes a deterioration in intersection operations on the Preston Street corridor that do not justify the marginal improvement in intersection operations along Booth Street corridor. The drawbacks of the Preston extension far outweigh the benefits, therefore the deletion of the Preston vehicular extension from the City's Official Plan is recommended.

It is important to note that not all decisions related to this development need to be made at this time due to the size of the LeBreton Flats development (e.g., "conditional approval" can be offered), as there will be ample opportunities for refinement to the transportation analysis as each parcel of land is developed and undergoes its own TIA process, including submission for approval. It should also be noted that given the significant timelines for the ultimate build-out of this project, it is important to recognize that travel patterns will change as projects like the Stage 2 Confederation Line LRT extension, West Gatineau Tramway and the downtown transit loop (and potentially other projects, such as a sixth crossing of the Ottawa River) are designed and constructed.

While it is difficult to provide a detailed TDM Implementation Program at this time given that this TIA is for the entire site, it is recommended that specific TDM initiatives be given further consideration as each development phase or site move forward. That being said, potential TDM measures that can be implemented across LeBreton Flats have been identified in Section 5.2 and are recommended for consideration, which includes some physical measures, travel surveys, and monitoring programs.



# **APPENDIX A: City of Ottawa TIA – Screening Form**





#### City of Ottawa 2017 TIA Guidelines Screening Form

#### 1. Description of Proposed Development

Municipal Address	
Description of Location	LeBreton Flats
Land Use Classification	Residential, commercial, office, hotel
Development Size (units)	4117 residential units
Development Size (m²)	24,250m² commercial, 47,265m² office, 14,345m² hotel
Number of Accesses and Locations	10 new roadway accesses
Phase of Development	Phases 1 - 3
Buildout Year	2030 (Phase 1), 2040 (Phase 2), 2050 (Phase 3)

If available, please attach a sketch of the development or site plan to this form.

#### 2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
Townhomes or apartments	90 units
Office	3,500 m <sup>2</sup>
Industrial	5,000 m <sup>2</sup>
Fast-food restaurant or coffee shop	100 m²
Destination retail	1,000 m <sup>2</sup>
Gas station or convenience market	75 m²

<sup>\*</sup> If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, <u>the Trip Generation</u> <u>Trigger is satisfied.</u>

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#### 3. Location Triggers

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	X	
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		

<sup>\*</sup>DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

#### 4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		X
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	X	
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	X	
Is the proposed driveway within auxiliary lanes of an intersection?	X	
Does the proposed driveway make use of an existing median break that serves an existing site?	X	
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	X	
Does the development include a drive-thru facility?		X

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

#### 5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?	X	
Does the development satisfy the Location Trigger?	X	
Does the development satisfy the Safety Trigger?	X	

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If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, the TIA Study must continue into the next stage (Screening and Scoping).

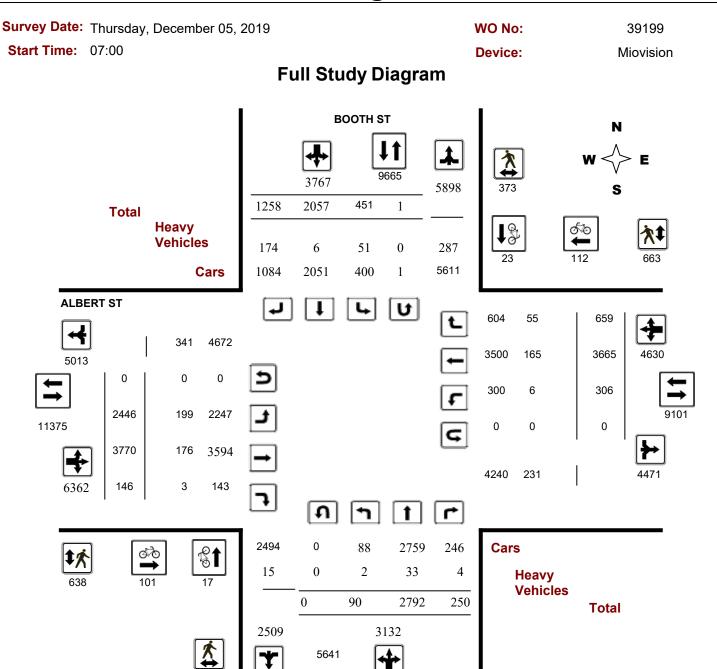
# **APPENDIX B: City of Ottawa - Traffic Count and Signal Timing Data**





### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**



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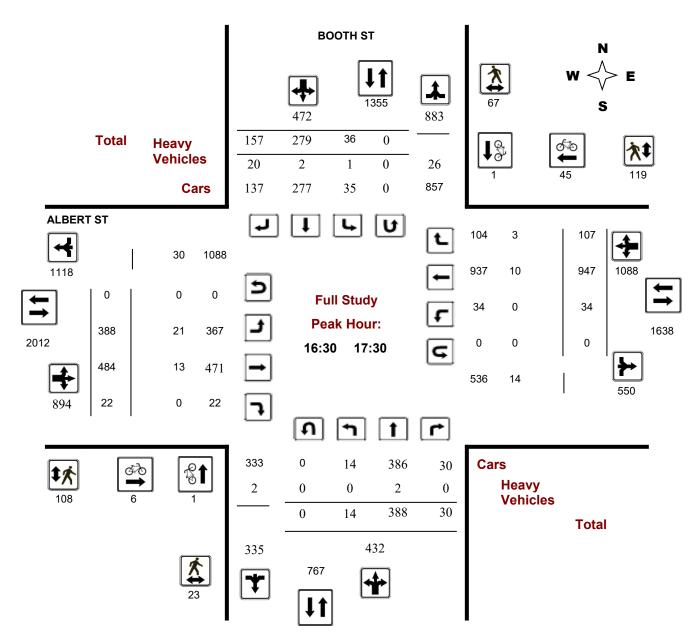
### **Turning Movement Count - Study Results**

#### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

### **Full Study Peak Hour Diagram**

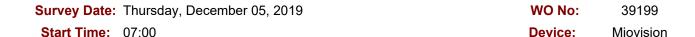


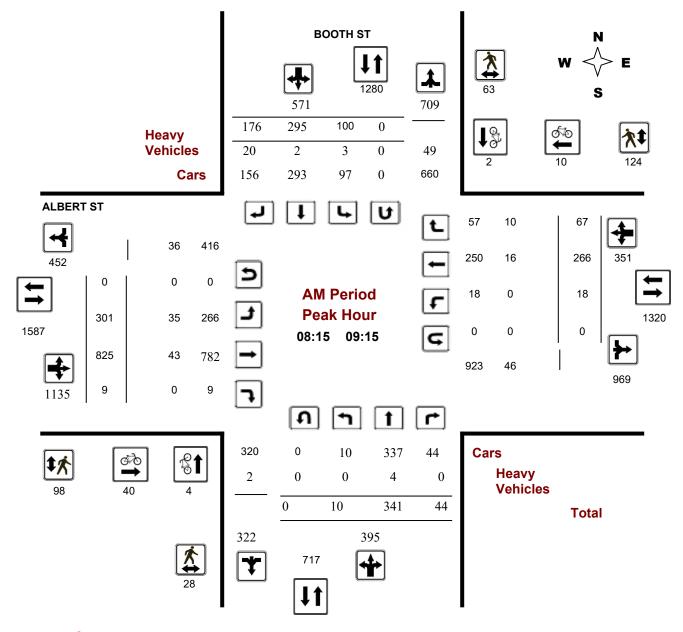
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### **Turning Movement Count - Peak Hour Diagram**

### **ALBERT ST @ BOOTH ST**





**Comments** 

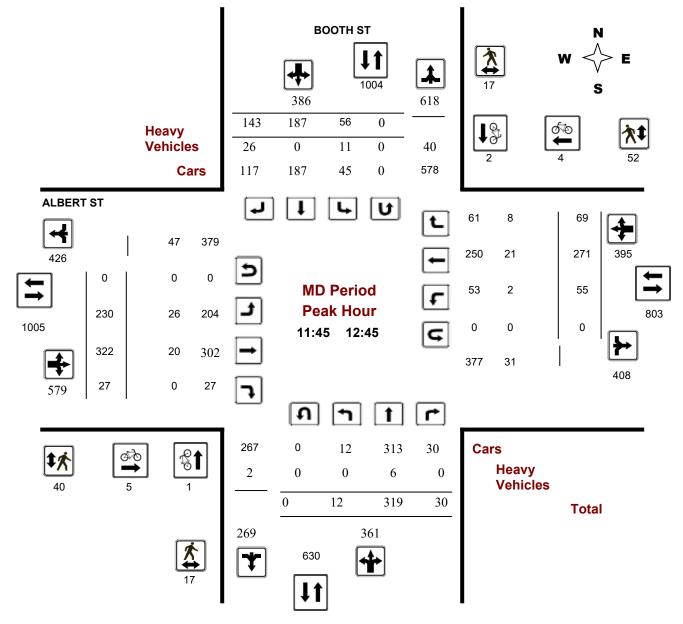
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### **Turning Movement Count - Peak Hour Diagram**

### **ALBERT ST @ BOOTH ST**





**Comments** 

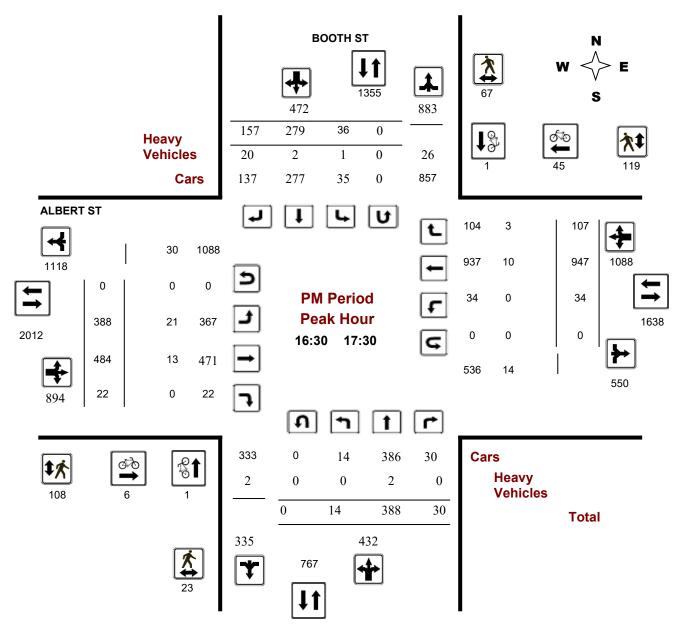
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### **Turning Movement Count - Peak Hour Diagram**

### **ALBERT ST @ BOOTH ST**





**Comments** 

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#### **Turning Movement Count - Study Results**

#### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Thursday, December 05, 2019 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 1

Eastbound: 0 Westbound: 0

1.00

BOOTH ST ALBERT ST

	BOOTH 21										ALDERI 31									
	No	rthbou	nd		So	uthbou	und			Е	astbou	ınd		٧	/estbo	und	<u>.</u>			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total	
07:00 08:00	9	323	24	356	71	334	173	578	934	234	484	6	724	6	213	51	270	994	1928	
08:00 09:00	9	343	42	394	92	311	167	570	964	296	814	10	1120	16	265	53	334	1454	2418	
09:00 10:00	7	284	48	339	75	277	176	528	867	268	653	15	936	31	233	60	324	1260	2127	
11:30 12:30	10	311	27	348	46	182	154	382	730	229	315	26	570	58	290	67	415	985	1715	
12:30 13:30	12	323	39	374	47	167	123	337	711	230	268	23	521	49	219	77	345	866	1577	
15:00 16:00	12	447	16	475	37	240	160	437	912	421	355	18	794	67	704	129	900	1694	2606	
16:00 17:00	15	367	31	413	44	282	143	469	882	381	409	27	817	44	949	121	1114	1931	2813	
17:00 18:00	16	394	23	433	39	264	162	465	898	387	472	21	880	35	792	101	928	1808	2706	
Sub Total	90	2792	250	3132	451	2057	1258	3766	6898	2446	3770	146	6362	306	3665	659	4630	10992	17890	
U Turns				0				1	1				0				0	0	1	
Total	90	2792	250	3132	451	2057	1258	3767	6899	2446	3770	146	6362	306	3665	659	4630	10992	17891	
EQ 12Hr	125	3881	348	4353	627	2859	1749	5236	9590	3400	5240	203	8843	425	5094	916	6436	15279	24868	
Note: These v	alues a	re calcu	lated by	y multiply	ying the	totals b	y the a	ppropriat	e expans	sion fac	tor.			1.39						
AVG 12Hr	118	3658	328	4103	591	2695	1648	4935	9590	3204	4939	191	8334	401	4801	863	6065	15279	24868	
Note: These v	olumes	are cald	culated	by multi	plying t	he Equiv	valent 1	2 hr. tota	ls by the	AADT	factor.			1						
AVG 24Hr	154	4791	429	5375	774	3530	2159	6465	11840	4198	6470	251	10918	525	6290	1131	7946	18864	30704	
Note: These v	olumes	are cal	culated	by multi	plying tl	he Avera	age Dai	ly 12 hr.	totals by	12 to 2	4 expans	sion fac	ctor.	1.31						

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

### **Full Study 15 Minute Increments**

BOOTH ST ALBERT ST

		N	orthbou	ınd		Sc	outhbou	nd		Eastbound					Westbound					
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	3	79	5	87	13	92	57	162	8	58	78	2	138	3	43	10	56	8	443
07:15	07:30	1	73	5	79	17	79	38	134	11	54	106	2	162	2	53	13	68	11	443
07:30	07:45	4	93	6	103	20	91	39	150	8	55	119	0	174	0	50	11	61	8	488
07:45	08:00	1	78	8	87	21	72	39	132	7	67	181	2	250	1	67	17	85	7	554
08:00	08:15	1	86	11	98	15	88	38	141	6	69	187	1	257	4	54	11	69	6	565
08:15	08:30	2	69	9	80	28	81	46	155	4	71	218	3	292	5	60	12	77	4	604
08:30	08:45	5	103	12	120	25	68	45	138	7	80	206	3	289	3	73	13	89	7	636
08:45	09:00	1	85	10	96	24	74	38	136	9	76	203	3	282	4	78	17	99	9	613
09:00	09:15	2	84	13	99	23	72	47	142	9	74	198	0	272	6	55	25	86	9	599
09:15	09:30	2	75	13	90	18	67	39	124	13	49	166	1	216	8	58	14	80	13	510
09:30	09:45	1	74	9	84	16	72	49	138	13	65	147	7	219	10	68	11	89	13	530
09:45	10:00	2	51	13	66	18	66	41	125	13	80	142	7	229	7	52	10	69	13	489
11:30	11:45	0	89	8	97	8	42	39	89	7	60	69	5	134	11	68	18	97	7	417
11:45	12:00	4	64	7	75	17	51	46	114	12	50	81	12	143	13	88	15	116	12	448
12:00	12:15	4	85	5	94	15	46	35	96	15	64	83	8	155	11	67	14	92	15	437
12:15	12:30	2	73	7	82	6	43	34	83	8	55	82	1	138	23	67	20	110	8	413
12:30	12:45	2	97	11	110	18	47	28	93	8	61	76	6	143	8	49	20	77	8	423
12:45	13:00	3	79	8	90	8	42	28	78	7	58	64	6	128	13	51	20	84	7	380
13:00	13:15	5	81	8	94	11	36	34	81	9	58	63	6	127	15	71	21	107	9	409
13:15	13:30	2	66	12	80	10	42	33	85	9	53	65	5	123	13	48	16	77	9	365
15:00	15:15	6	117	3	126	10	52	39	101	10	108	68	4	180	31	127	31	189	10	596
15:15	15:30	4	108	5	117	7	56	38	101	12	112	85	3	200	11	161	31	203	12	621
15:30	15:45	0	110	4	114	10	79	43	132	8	96	106	6	208	15	198	37	250	8	704
15:45	16:00	2	112	4	118	10	53	40	103	11	105	96	5	206	10	218	30	258	11	685
16:00	16:15	6	96	4	106	11	75	40	126	10	89	93	7	189	10	207	32	249	10	670
16:15	16:30	5	88	3	96	10	70	23	103	4	99	101	8	208	16	247	33	296	4	703
16:30	16:45	3	90	14	107	13	76	41	130	6	91	98	5	194	6	249	27	282	6	713
16:45	17:00	1	93	10	104	10	61	39	110	10	102	117	7	226	12	246	29	287	10	727
17:00	17:15	4	105	2	111	7	77	38	122	5	92	136	6	234	11	225	21	257	5	724
17:15	17:30	6	100	4	110	6	65	39	110	4	103	133	4	240	5	227	30	262	4	722
17:30	17:45	1	102	6	109	13	65	45	123	4	92	90	6	188	10	177	24	211	4	631
17:45	18:00	5	87	11	103	13	57	40	110	3	100	113	5	218	9	163	26	198	3	629
Total:		90	2792	250	3132	451	2057	1258	3767	270	2446	3770	146	6362	306	3665	659	4630	270	17,891

Note: U-Turns are included in Totals.

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

### **Full Study Cyclist Volume**

#### BOOTH ST ALBERT ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total		
07:00 07:15	0	0	0	0	1	1	1		
07:15 07:30	1	0	1	2	0	2	3		
07:30 07:45	1	3	4	13	2	15	19		
07:45 08:00	3	3	6	6	0	6	12		
08:00 08:15	1	0	1	10	0	10	11		
08:15 08:30	1	0	1	10	2	12	13		
08:30 08:45	2	1	3	10	3	13	16		
08:45 09:00	0	0	0	8	3	11	11		
09:00 09:15	1	1	2	12	2	14	16		
09:15 09:30	0	1	1	4	0	4	5		
09:30 09:45	1	1	2	2	2	4	6		
09:45 10:00	0	0	0	1	0	1	1		
11:30 11:45	0	0	0	0	0	0	0		
11:45 12:00	0	0	0	1	0	1	1		
12:00 12:15	0	0	0	1	1	2	2		
12:15 12:30	0	2	2	2	1	3	5		
12:30 12:45	1	0	1	1	2	3	4		
12:45 13:00	0	0	0	0	2	2	2		
13:00 13:15	1	1	2	1	1	2	4		
13:15 13:30	0	0	0	1	2	3	3		
15:00 15:15	0	3	3	0	2	2	5		
15:15 15:30	0	0	0	0	6	6	6		
15:30 15:45	0	0	0	2	4	6	6		
15:45 16:00	0	2	2	2	3	5	7		
16:00 16:15	1	0	1	4	6	10	11		
16:15 16:30	2	3	5	1	8	9	14		
16:30 16:45	0	0	0	4	5	9	9		
16:45 17:00	1	0	1	1	13	14	15		
17:00 17:15	0	0	0	0	12	12	12		
17:15 17:30	0	1	1	1	15	16	17		
17:30 17:45	0	1	1	0	10	10	11		
17:45 18:00	0	0		1	4	5	5		
Total	17	23	40	101	112	213	253		

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

# Full Study Pedestrian Volume BOOTH ST ALBERT ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	6	7	12	12	24	31
07:15 07:30	3	8	11	13	17	30	41
07:30 07:45	8	13	21	27	21	48	69
07:45 08:00	9	15	24	23	28	51	75
08:00 08:15	6	19	25	28	35	63	88
08:15 08:30	13	21	34	28	32	60	94
08:30 08:45	6	15	21	27	32	59	80
08:45 09:00	5	17	22	27	38	65	87
09:00 09:15	4	10	14	16	22	38	52
09:15 09:30	7	6	13	14	12	26	39
09:30 09:45	6	3	9	12	17	29	38
09:45 10:00	4	8	12	15	11	26	38
11:30 11:45	4	3	7	7	6	13	20
11:45 12:00	6	2	8	10	13	23	31
12:00 12:15	4	1	5	9	11	20	25
12:15 12:30	4	8	12	8	18	26	38
12:30 12:45	3	6	9	13	10	23	32
12:45 13:00	2	6	8	9	9	18	26
13:00 13:15	6	7	13	25	9	34	47
13:15 13:30	2	4	6	7	11	18	24
15:00 15:15	6	10	16	20	17	37	53
15:15 15:30	5	15	20	25	15	40	60
15:30 15:45	5	7	12	18	26	44	56
15:45 16:00	8	14	22	23	20	43	65
16:00 16:15	4	20	24	33	25	58	82
16:15 16:30	4	27	31	27	26	53	84
16:30 16:45	2	16	18	26	30	56	74
16:45 17:00	2	16	18	18	34	52	70
17:00 17:15	12	14	26	40	22	62	88
17:15 17:30	7	21	28	24	33	57	85
17:30 17:45	4	17	21	30	34	64	85
17:45 18:00	3	18	21	24	17	41	62
Total	165	373	538	638	663	1301	1839

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

### **Full Study Heavy Vehicles**

BOOTH ST ALBERT ST

	N	orthbo	und		Sc	uthbou	nd		Eastbound					Westbound					
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	2	0	2	1	0	5	6	8	6	5	0	11	0	4	1	5	16	24
07:15 07:30	0	1	0	1	4	0	6	10	11	5	7	0	12	0	6	1	7	19	30
07:30 07:45	0	0	0	0	5	0	3	8	8	5	7	0	12	0	5	3	8	20	28
07:45 08:00	0	0	0	0	2	0	5	7	7	8	6	0	14	0	5	3	8	22	29
08:00 08:15	1	0	0	1	2	0	3	5	6	4	11	0	15	0	3	2	5	20	26
08:15 08:30	0	1	0	1	0	1	2	3	4	7	7	0	14	0	4	1	5	19	23
08:30 08:45	0	1	0	1	2	0	4	6	7	4	22	0	26	0	4	1	5	31	38
08:45 09:00	0	1	0	1	0	1	7	8	9	11	6	0	17	0	5	4	9	26	35
09:00 09:15	0	1	0	1	1	0	7	8	9	13	8	0	21	0	3	4	7	28	37
09:15 09:30	0	3	0	3	3	0	7	10	13	11	7	1	19	0	6	3	9	28	41
09:30 09:45	0	2	0	2	0	0	11	11	13	8	5	0	13	0	7	1	8	21	34
09:45 10:00	0	3	1	4	3	0	6	9	13	9	7	0	16	0	5	1	6	22	35
11:30 11:45	0	1	0	1	1	0	5	6	7	10	4	1	15	0	5	4	9	24	31
11:45   12:00	0	1	0	1	5	0	6	11	12	7	6	0	13	0	11	1	12	25	37
12:00 12:15	0	1	0	1	4	0	10	14	15	8	3	0	11	1	4	3	8	19	34
12:15   12:30	0	1	0	1	0	0	7	7	8	6	6	0	12	1	1	2	4	16	24
12:30 12:45	0	3	0	3	2	0	3	5	8	5	5	0	10	0	5	2	7	17	25
12:45   13:00	0	0	0	0	1	1	5	7	7	5	5	1	11	0	10	5	15	26	33
13:00 13:15	0	0	1	1	5	0	3	8	9	5	6	0	11	0	4	1	5	16	25
13:15   13:30	0	0	1	1	2	0	6	8	9	1	6	0	7	1	4	3	8	15	24
15:00 15:15	0	1	0	1	1	0	8	9	10	3	6	0	9	0	9	1	10	19	29
15:15   15:30	1	4	0	5	1	0	6	7	12	4	2	0	6	0	5	3	8	14	26
15:30 15:45	0	1	0	1	1	0	6	7	8	5	3	0	8	1	13	1	15	23	31
15:45 16:00	0	1	1	2	2	0	7	9	11	9	5	0	14	1	8	0	9	23	34
16:00 16:15	0	1	0	1	1	0	8	9	10	6	2	0	8	0	8	0	8	16	26
16:15 16:30	0	0	0	0	0	0	4	4	4	5	1	0	6	1	6	0	7	13	17
16:30 16:45	0	0	0	0	1	1	4	6	6	5	1	0	6	0	4	1	5	11	17
16:45 17:00	0	1	0	1	0	1	8	9	10	6	5	0	11	0	2	1	3	14	24
17:00 17:15	0	1	0	1	0	0	4	4	5	5	4	0	9	0	2	1	3	12	17
17:15 17:30	0	0	0	0	0	0	4	4	4	5	3	0	8	0	2	0	2	10	14
17:30 17:45	0	1	0	1	1	0	2	3	4	4	3	0	7	0	2	0	2	9	13
17:45 18:00	0	0	0	0	0	1	2	3	3	4	2	0	6	0	3	1	4	10	13
Total: None	2	33	4	39	51	6	174	231	270	199	176	3	378	6	165	55	226	604	874

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ BOOTH ST**

Survey Date: Thursday, December 05, 2019 WO No: 39199

Start Time: 07:00 Device: Miovision

# Full Study 15 Minute U-Turn Total BOOTH ST ALBERT ST

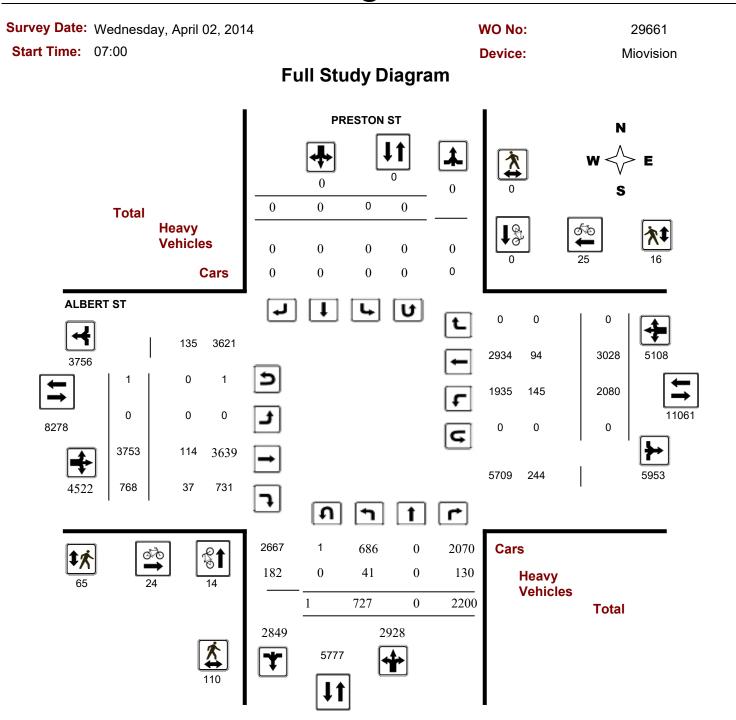
Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	1	0	0	1
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	1	0	0	1

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### **Turning Movement Count - Study Results**

#### **ALBERT ST @ PRESTON ST**



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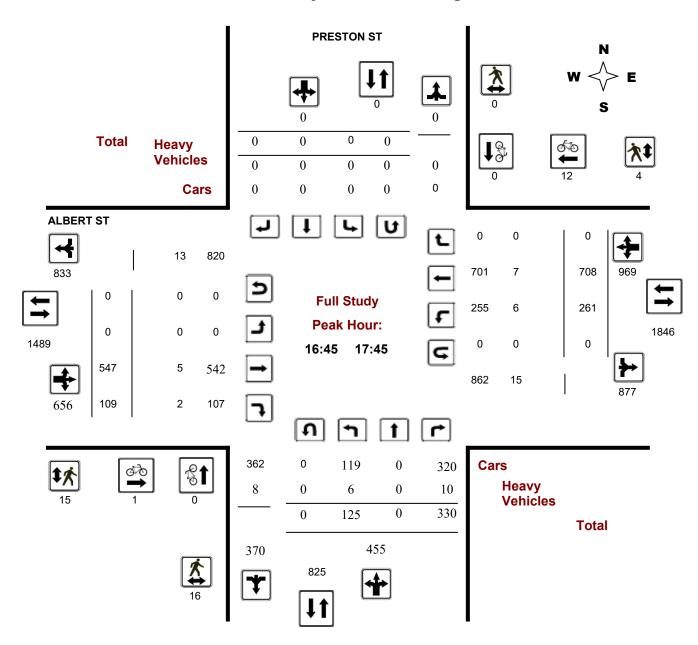
#### **Turning Movement Count - Study Results**

#### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

### **Full Study Peak Hour Diagram**



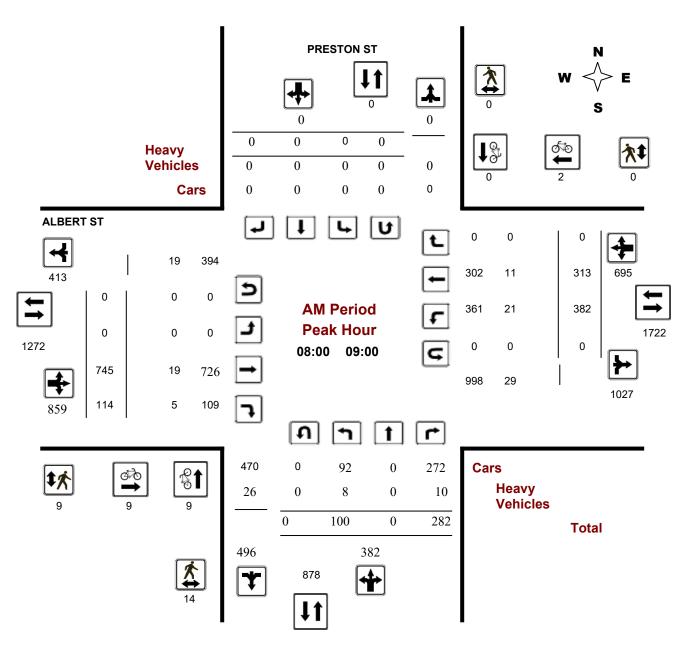
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### **Turning Movement Count - Peak Hour Diagram**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661
Start Time: 07:00 Device: Miovision



**Comments** 

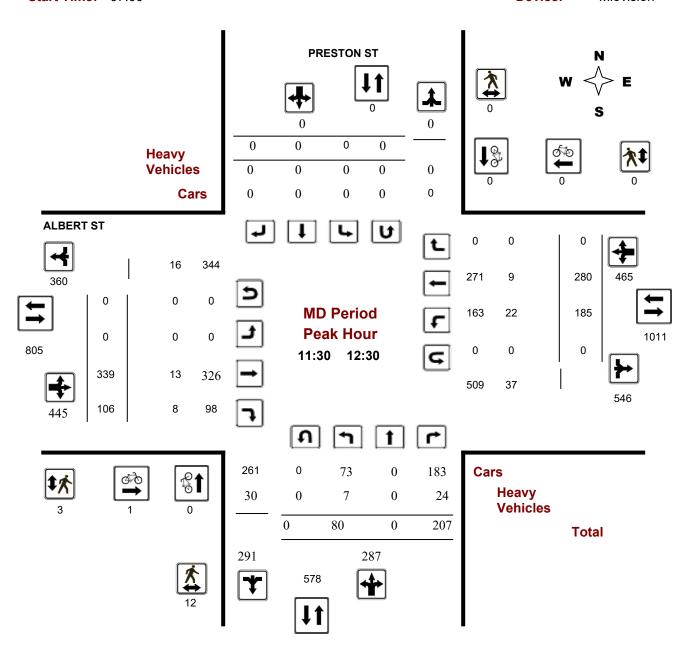
2020-Mar-27 Page 1 of 3



### **Turning Movement Count - Peak Hour Diagram**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661
Start Time: 07:00 Device: Miovision



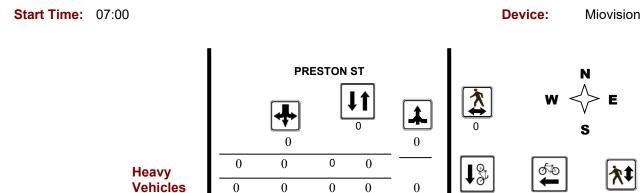
**Comments** 

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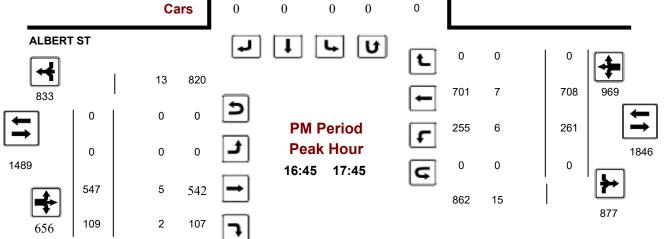
### **Turning Movement Count - Peak Hour Diagram**

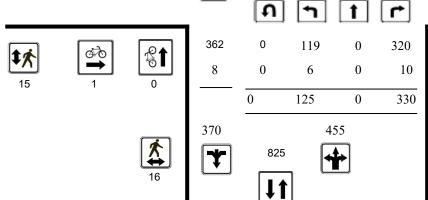
### **ALBERT ST @ PRESTON ST**



**Vehicles** Cars

Survey Date: Wednesday, April 02, 2014





Cars Heavy **Vehicles Total** 

WO No:

29661

**Comments** 

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#### **Turning Movement Count - Study Results**

# **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Wednesday, April 02, 2014 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: 0 .90

Eastbound: 1 Westbound: 0

	PRESTON ST											Al	BERT	ST					
	Nor	thbou	nd		Sou	uthbou	ınd			Е	astbou	und		٧	Vestbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	54	0	245	299	0	0	0	0	299	0	450	69	519	390	277	0	667	1186	1485
08:00 09:00	100	0	282	382	0	0	0	0	382	0	745	114	859	382	313	0	695	1554	1936
09:00 10:00	82	0	198	280	0	0	0	0	280	0	416	81	497	228	263	0	491	988	1268
11:30 12:30	80	0	207	287	0	0	0	0	287	0	339	106	445	185	280	0	465	910	1197
12:30 13:30	72	0	190	262	0	0	0	0	262	0	317	82	399	166	239	0	405	804	1066
15:00 16:00	113	0	377	490	0	0	0	0	490	0	458	97	555	223	406	0	629	1184	1674
16:00 17:00	112	0	370	482	0	0	0	0	482	0	548	104	652	247	639	0	886	1538	2020
17:00 18:00	114	0	331	445	0	0	0	0	445	0	480	115	595	259	611	0	870	1465	1910
Sub Total	727	0	2200	2927	0	0	0	0	2927	0	3753	768	4521	2080	3028	0	5108	9629	12556
U Turns				1				0	1				1				0	1	2
Total	727	0	2200	2928	0	0	0	0	2928	0	3753	768	4522	2080	3028	0	5108	9630	12558
EQ 12Hr	1011	0	3058	4070	0	0	0	0	4070	0	5217	1068	6286	2891	4209	0	7100	13386	17456
Note: These	values ar	e calcu	ılated by	y multiply	ing the	totals b	y the a <sub>l</sub>	opropriate	e expans	ion fact	tor.			1.39					
AVG 12Hr	857	0	2594	3452	0	0	0	0	3663	0	4425	905	5331	2452	3570	0	6022	12047	15710
Note: These	volumes	are cal	culated	by multip	lying th	e Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			0.9					
AVG 24Hr	1123	0	3398	4522	0	0	0	0	4522	0	5796	1186	6984	3213	4677	0	7889	14873	19395
Note: These	volumes	are cal	culated	by multip	olying th	e Avera	ige Dai	ly 12 hr. 1	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

### **Full Study 15 Minute Increments**

PRESTON ST ALBERT ST

	Northbound		und	Southbound					Eastbound			Westbound								
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	15	0	56	71	0	0	0	0	5	0	69	15	84	90	51	0	141	5	296
07:15	07:30	12	0	48	60	0	0	0	0	3	0	88	13	101	113	67	0	180	3	341
07:30	07:45	12	0	78	90	0	0	0	0	7	0	137	15	152	100	88	0	188	7	430
07:45	08:00	15	0	63	78	0	0	0	0	5	0	156	26	182	87	71	0	158	5	418
08:00	08:15	26	0	76	102	0	0	0	0	2	0	176	20	196	111	78	0	189	2	487
08:15	08:30	20	0	68	88	0	0	0	0	6	0	218	28	246	101	64	0	165	6	499
08:30	08:45	30	0	74	104	0	0	0	0	6	0	177	30	207	94	73	0	167	6	478
08:45	09:00	24	0	64	88	0	0	0	0	4	0	174	36	210	76	98	0	174	4	472
09:00	09:15	27	0	52	79	0	0	0	0	4	0	129	18	147	80	82	0	162	4	388
09:15	09:30	24	0	58	82	0	0	0	0	2	0	120	29	149	57	70	0	127	2	358
09:30	09:45	13	0	45	58	0	0	0	0	6	0	91	18	109	48	58	0	106	6	273
09:45	10:00	18	0	43	61	0	0	0	0	4	0	76	16	92	43	53	0	96	4	249
11:30	11:45	28	0	50	78	0	0	0	0	11	0	94	19	113	40	76	0	116	11	307
11:45	12:00	18	0	64	82	0	0	0	0	10	0	74	39	113	53	64	0	117	10	312
12:00	12:15	18	0	47	65	0	0	0	0	3	0	88	27	115	39	69	0	108	3	288
12:15	12:30	16	0	46	62	0	0	0	0	7	0	83	21	104	53	71	0	124	7	290
12:30	12:45	19	0	42	61	0	0	0	0	10	0	70	23	93	37	64	0	101	10	255
12:45	13:00	21	0	49	71	0	0	0	0	4	0	66	21	87	49	63	0	112	4	270
13:00	13:15	15	0	58	73	0	0	0	0	5	0	84	19	103	42	54	0	96	5	272
13:15	13:30	17	0	41	58	0	0	0	0	6	0	97	19	116	38	58	0	96	6	270
15:00	15:15	27	0	99	126	0	0	0	0	8	0	97	21	118	52	95	0	147	8	391
15:15	15:30	20	0	96	116	0	0	0	0	8	0	102	25	128	51	100	0	151	8	395
15:30	15:45	22	0	93	115	0	0	0	0	3	0	129	18	147	50	103	0	153	3	415
15:45	16:00	44	0	89	133	0	0	0	0	7	0	130	33	163	70	108	0	178	7	474
16:00	16:15	29	0	89	118	0	0	0	0	5	0	134	33	167	57	133	0	190	5	475
16:15	16:30	22	0	117	139	0	0	0	0	7	0	137	30	167	71	157	0	228	7	534
16:30	16:45	33	0	89	122	0	0	0	0	4	0	117	23	140	56	162	0	218	4	480
16:45	17:00	28	0	75	103	0	0	0	0	4	0	160	18	178	63	187	0	250	4	531
17:00	17:15	37	0	96	133	0	0	0	0	4	0	134	41	175	66	155	0	221	4	529
17:15	17:30	34	0	72	106	0	0	0	0	5	0	129	22	151	71	190	0	261	5	518
17:30	17:45	26	0	87	113	0	0	0	0	3	0	124	28	152	61	176	0	237	3	502
17:45	18:00	17	0	76	93	0	0	0	0	3	0	93	24	117	61	90	0	151	3	361
Total:		727	0	2200	2928	0	0	0	0	171	0	3753	768	4522	2080	3028	0	5108	171	12,558

Note: U-Turns are included in Totals.

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

### **Full Study Cyclist Volume**

#### PRESTON ST ALBERT ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	0	1	1	1
07:15 07:30	1	0	1	0	0	0	1
07:30 07:45	0	0	0	2	0	2	2
07:45 08:00	1	0	1	3	0	3	4
08:00 08:15	4	0	4	5	0	5	9
08:15 08:30	1	0	1	2	1	3	4
08:30 08:45	2	0	2	1	1	2	4
08:45 09:00	2	0	2	1	0	1	3
09:00 09:15	0	0	0	2	0	2	2
09:15 09:30	0	0	0	2	0	2	2
09:30 09:45	1	0	1	1	0	1	2
09:45 10:00	0	0	0	1	0	1	1
11:30 11:45	0	0	0	1	0	1	1
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	1	1	1
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	1	1	1
15:30 15:45	0	0	0	0	1	1	1
15:45 16:00	1	0	1	1	0	1	2
16:00 16:15	0	0	0	0	1	1	1
16:15 16:30	0	0	0	1	0	1	1
16:30 16:45	1	0	1	0	3	3	4
16:45 17:00	0	0	0	0	2	2	2
17:00 17:15	0	0	0	1	1	2	2
17:15 17:30	0	0	0	0	8	8	8
17:30 17:45	0	0	0	0	1	1	1
17:45 18:00	0	0	0	0	3	3	3
Total	14	0	14	24	25	49	63

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

# **Full Study Pedestrian Volume**

PRESTON ST ALBERT ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	0	1	1	0	1	2
07:15 07:30	3	0	3	4	0	4	7
07:30 07:45	3	0	3	5	0	5	8
07:45 08:00	3	0	3	1	0	1	4
08:00 08:15	4	0	4	2	0	2	6
08:15 08:30	3	0	3	3	0	3	6
08:30 08:45	5	0	5	4	0	4	9
08:45 09:00	2	0	2	0	0	0	2
09:00 09:15	7	0	7	0	0	0	7
09:15 09:30	4	0	4	1	0	1	5
09:30 09:45	3	0	3	2	0	2	5
09:45 10:00	5	0	5	2	0	2	7
11:30 11:45	5	0	5	0	0	0	5
11:45 12:00	3	0	3	1	0	1	4
12:00 12:15	2	0	2	2	0	2	4
12:15 12:30	2	0	2	0	0	0	2
12:30 12:45	2	0	2	0	0	0	2
12:45 13:00	2	0	2	5	0	5	7
13:00 13:15	2	0	2	1	2	3	5
13:15 13:30	0	0	0	0	2	2	2
15:00 15:15	4	0	4	2	0	2	6
15:15 15:30	3	0	3	5	0	5	8
15:30 15:45	3	0	3	2	1	3	6
15:45 16:00	0	0	0	4	1	5	5
16:00 16:15	3	0	3	0	2	2	5
16:15 16:30	12	0	12	0	3	3	15
16:30 16:45	6	0	6	0	1	1	7
16:45 17:00	8	0	8	0	1	1	9
17:00 17:15	3	0	3	6	0	6	9
17:15 17:30	3	0	3	2	1	3	6
17:30 17:45	2	0	2	7	2	9	11
17:45 18:00	2	0	2	3	0	3	5
Total	110	0	110	65	16	81	191

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

### **Full Study Heavy Vehicles**

PRESTON ST ALBERT ST

	Northbound Southbound Eastbound					Westbound													
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	2	0	3	5	0	0	0	0	5	0	2	0	2	2	2	0	4	6	11
07:15 07:30	0	0	3	3	0	0	0	0	3	0	1	2	3	7	2	0	9	12	15
07:30 07:45	2	0	5	7	0	0	0	0	7	0	6	0	6	4	6	0	10	16	23
07:45 08:00	1	0	4	5	0	0	0	0	5	0	9	2	11	5	3	0	8	19	24
08:00 08:15	1	0	1	2	0	0	0	0	2	0	6	0	6	5	3	0	8	14	16
08:15 08:30	1	0	5	6	0	0	0	0	6	0	3	3	6	6	2	0	8	14	20
08:30 08:45	5	0	1	6	0	0	0	0	6	0	3	0	3	6	2	0	8	11	17
08:45 09:00	1	0	3	4	0	0	0	0	4	0	7	2	9	4	4	0	8	17	21
09:00 09:15	1	0	3	4	0	0	0	0	4	0	6	3	9	8	3	0	11	20	24
09:15 09:30	0	0	2	2	0	0	0	0	2	0	9	3	12	7	7	0	14	26	28
09:30 09:45	1	0	5	6	0	0	0	0	6	0	2	1	3	7	3	0	10	13	19
09:45 10:00	0	0	4	4	0	0	0	0	4	0	5	0	5	3	6	0	9	14	18
11:30 11:45	2	0	9	11	0	0	0	0	11	0	3	1	4	5	1	0	6	10	21
11:45 12:00	2	0	8	10	0	0	0	0	10	0	1	4	5	6	5	0	11	16	26
12:00 12:15	2	0	1	3	0	0	0	0	3	0	5	1	6	4	1	0	5	11	14
12:15 12:30	1	0	6	7	0	0	0	0	7	0	4	2	6	7	2	0	9	15	22
12:30 12:45	4	0	6	10	0	0	0	0	10	0	3	2	5	5	3	0	8	13	23
12:45 13:00	0	0	4	4	0	0	0	0	4	0	2	1	3	9	4	0	13	16	20
13:00 13:15	0	0	5	5	0	0	0	0	5	0	2	3	5	8	4	0	12	17	22
13:15   13:30	0	0	6	6	0	0	0	0	6	0	7	1	8	4	3	0	7	15	21
15:00 15:15	2	0	6	8	0	0	0	0	8	0	2	0	2	2	3	0	5	7	15
15:15 15:30	2	0	6	8	0	0	0	0	8	0	5	0	5	4	2	0	6	11	19
15:30 15:45	1	0	2	3	0	0	0	0	3	0	3	0	3	3	5	0	8	11	14
15:45 16:00	3	0	4	7	0	0	0	0	7	0	5	0	5	6	4	0	10	15	22
16:00 16:15	0	0	5	5	0	0	0	0	5	0	2	0	2	4	0	0	4	6	11
16:15 16:30	0	0	7	7	0	0	0	0	7	0	2	2	4	5	4	0	9	13	20
16:30 16:45	1	0	3	4	0	0	0	0	4	0	1	2	3	1	2	0	3	6	10
16:45 17:00	3	0	1	4	0	0	0	0	4	0	2	1	3	2	3	0	5	8	12
17:00 17:15	1	0	3	4	0	0	0	0	4	0	1	1	2	2	2	0	4	6	10
17:15 17:30	1	0	4	5	0	0	0	0	5	0	1	0	1	1	1	0	2	3	8
17:30 17:45	1	0	2	3	0	0	0	0	3	0	1	0	1	1	1	0	2	3	6
17:45 18:00	0	0	3	3	0	0	0	0	3	0	3	0	3	2	1	0	3	6	9
Total: None	41	0	130	171	0	0	0	0	171	0	114	37	151	145	94	0	239	390	561

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### **Turning Movement Count - Study Results**

### **ALBERT ST @ PRESTON ST**

Survey Date: Wednesday, April 02, 2014 WO No: 29661

Start Time: 07:00 Device: Miovision

# Full Study 15 Minute U-Turn Total PRESTON ST ALBERT ST

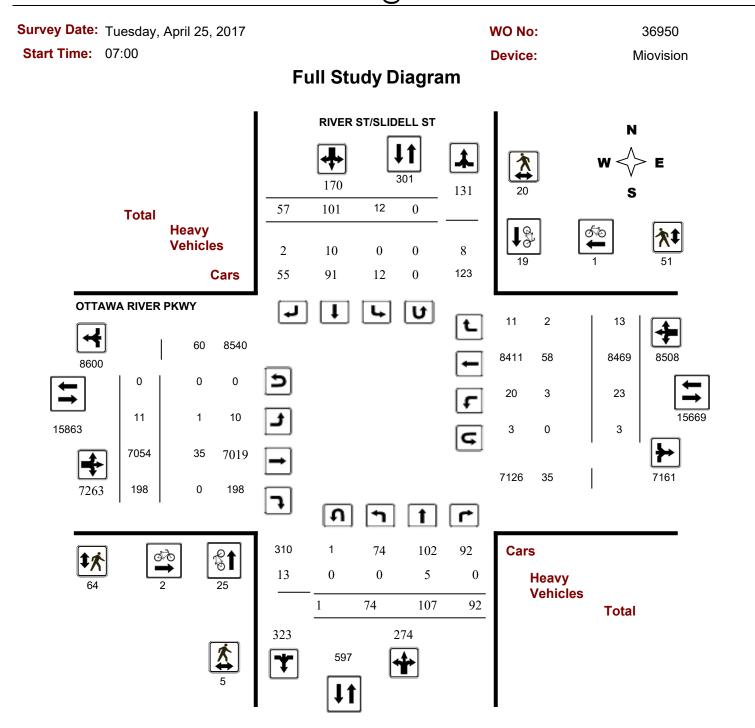
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	1	0	0	0	1
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	1	0	1
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	1	0	1	0	2

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### **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST



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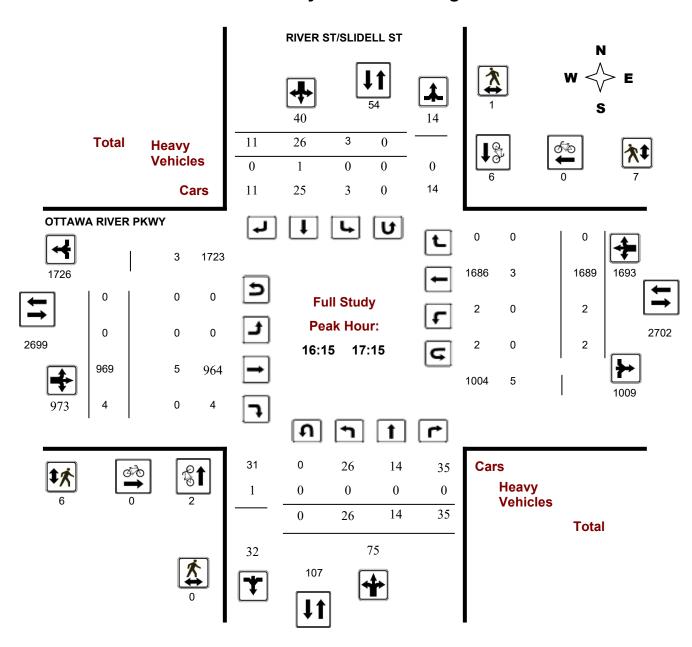


### **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950
Start Time: 07:00 Device: Miovision

#### **Full Study Peak Hour Diagram**

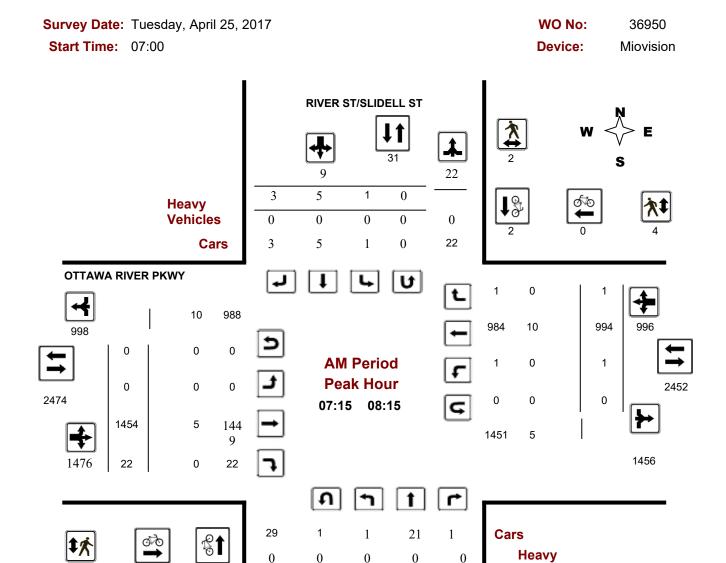


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### **Turning Movement Count - Peak Hour Diagram**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST



**Vehicles** 

**Total** 

Comments

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1

53

29

21

24

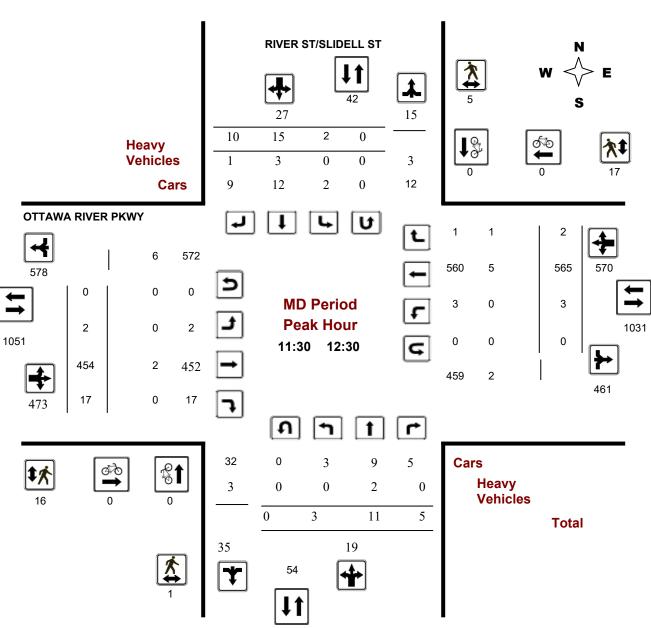
1



### **Turning Movement Count - Peak Hour Diagram**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date:Tuesday, April 25, 2017WO No:36950Start Time:07:00Device:Miovision



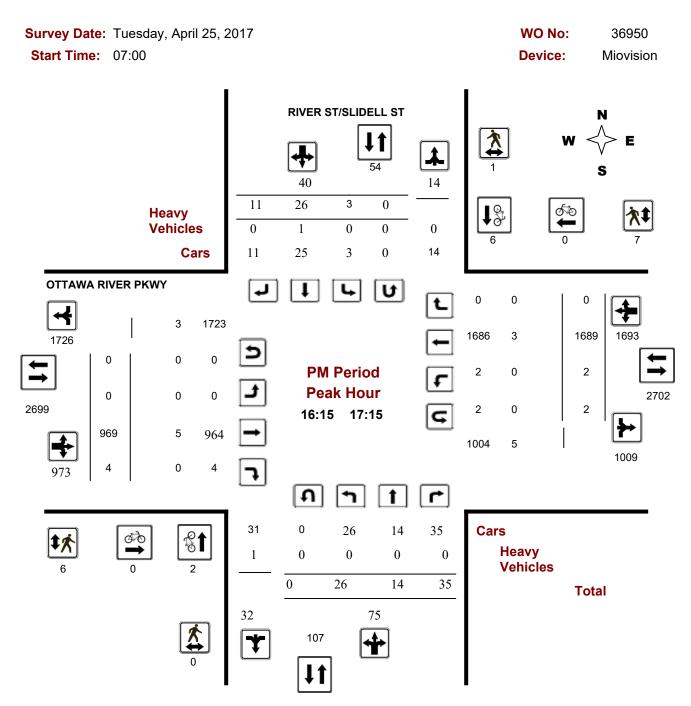
**Comments** 

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### **Turning Movement Count - Peak Hour Diagram**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST



**Comments** 

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### **Turning Movement Count - Study Results**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Tuesday, April 25, 2017 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: 0 .90

Eastbound: 0 Westbound: 3

		RI	VER S	ST/SLID	ELL S	ST			OTTAWA RIVER PKWY										
	Nor	thbou	nd		So	uthbou	nd			Е	astbou	ınd		٧	Vestbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	1	22	0	23	1	4	2	7	30	0	1315	21	1336	0	986	1	987	2323	2353
08:00 09:00	0	28	4	32	0	8	4	12	44	1	1401	39	1441	1	909	1	911	2352	2396
09:00 10:00	2	15	2	19	0	14	5	19	38	2	698	80	780	3	693	5	701	1481	1519
11:30 12:30	3	11	5	19	2	15	10	27	46	2	454	17	473	3	565	2	570	1043	1089
12:30 13:30	4	7	7	18	3	11	6	20	38	6	418	20	444	10	529	1	540	984	1022
15:00 16:00	10	2	22	34	3	9	10	22	56	0	993	12	1005	2	1502	0	1504	2509	2565
16:00 17:00	27	15	38	80	3	24	11	38	118	0	954	3	957	0	1684	0	1684	2641	2759
17:00 18:00	27	7	14	48	0	16	9	25	73	0	821	6	827	4	1601	3	1608	2435	2508
Sub Total	74	107	92	273	12	101	57	170	443	11	7054	198	7263	23	8469	13	8505	15768	16211
U Turns				1				0	1				0				3	3	4
Total	74	107	92	274	12	101	57	170	444	11	7054	198	7263	23	8469	13	8508	15771	16215
EQ 12Hr	103	149	128	381	17	140	79	236	617	15	9805	275	10096	32	11772	18	11826	21922	22539
Note: These \	/alues ar	e calcu	lated by	/ multiply	ing the	totals b	y the ap	opropriate	expans	ion fac	tor.			1.39					
AVG 12Hr	87	126	108	323	14	119	67	200	555	13	8317	233	8563	27	9985	15	10031	19730	20285
Note: These \	olumes/	are calc	culated	by multip	olying th	ne Equiv	alent 1	2 hr. total	ls by the	AADT	factor.			0.9					
AVG 24Hr	114	165	142	423	19	156	88	263	686	17	10895	306	11218	36	13080	20	13141	24359	25045
Note: These \	olumes/	are calc	culated	by multip	olying th	ne Avera	ige Dail	y 12 hr. t	otals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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### **Turning Movement Count - Study Results**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

### **Full Study 15 Minute Increments**

#### **RIVER ST/SLIDELL ST**

#### OTTAWA RIVER PKWY

		Northbound Southbound Eastbound				Westbound														
Time Pe	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	10	0	10	0	0	0	0	1	0	268	8	276	0	246	0	246	1	532
07:15	07:30	1	2	0	3	1	0	1	2	0	0	360	3	363	0	234	1	235	0	603
07:30	07:45	0	2	0	3	0	1	1	2	0	0	312	8	320	0	271	0	271	0	596
07:45	08:00	0	8	0	8	0	3	0	3	0	0	375	2	377	0	235	0	235	0	623
08:00	08:15	0	9	1	10	0	1	1	2	0	0	407	9	416	1	254	0	255	0	683
08:15	08:30	0	11	1	12	0	3	1	4	0	0	338	8	346	0	221	0	221	0	583
08:30	08:45	0	2	0	2	0	2	2	4	0	0	351	9	360	0	193	0	193	0	559
08:45	09:00	0	6	2	8	0	2	0	2	2	1	305	13	319	0	241	1	242	2	571
09:00	09:15	1	3	1	5	0	4	2	6	0	0	249	27	276	1	241	1	243	0	530
09:15	09:30	0	5	0	5	0	2	1	3	1	0	170	24	194	2	171	2	175	1	377
09:30	09:45	0	4	1	5	0	4	1	5	0	1	154	17	172	0	135	0	135	0	317
09:45 1	10:00	1	3	0	4	0	4	1	5	2	1	125	12	138	0	146	2	148	2	295
11:30 1	11:45	1	4	2	7	0	4	3	7	1	0	120	5	125	1	126	0	127	1	266
11:45 1	12:00	0	0	0	0	0	3	5	8	0	2	117	3	122	0	138	0	138	0	268
12:00 1	12:15	1	4	1	6	0	6	1	7	4	0	121	5	126	2	149	2	153	4	292
12:15 1	12:30	1	3	2	6	2	2	1	5	1	0	96	4	100	0	152	0	152	1	263
	12:45	1	0	2	3	1	1	1	3	1	2	112	7	121	3	132	0	135	1	262
12:45 1	13:00	1	1	0	2	1	8	3	12	1	2	92	6	100	2	130	0	132	1	246
13:00 1	13:15	1	3	2	6	1	2	0	3	0	1	100	4	105	1	137	0	138	0	252
	13:30	1	3	3	7	0	0	2	2	0	1	114	3	118	4	130	1	136	0	263
_	15:15	1	1	4	6	3	3	2	8	1	0	262	5	267	1	309	0	310	1	591
	15:30	1	0	8	9	0	5	3	8	1	0	235	1	236	0	424	0	424	1	677
	15:45	4	0	6	10	0	1	1	2	0	0	241	3	244	0	370	0	370	0	626
	16:00	4	1	4	9	0	0	4	4	0	0	255	3	258	1	399	0	400	0	671
	16:15	9	5	9	23	0	1	2	3	0	0	213	0	213	0	450	0	450	0	689
_	16:30	6	3	9	18	3	6	4	13	1	0	255	2	257	0	412	0	412	1	700
	16:45	6	4	13	23	0	12	5	17	0	0	245	0	245	0	432	0	432	0	717
	17:00	6	3	7	16	0	5	0	5	0	0	241	1	242	0	390	0	391	0	654
	17:15	8	4	6	18	0	3	2	5	0	0	228	1	229	2	455	0	458	0	710
	17:30	11	1	4	16	0	8	2	10	0	0	221	4	225	1	388	1	390	0	641
_	17:45	4	0	1	5	0	4	4	8	0	0	209	0	209	0	420	1	421	0	643
	18:00	4	2	3	9	0	1	1	2	0	0	163	1	164	1	338	1	340	0	515
Total:		74	107	92	274	12	101	57	170	17	11	7054	198	7263	23	8469	13	8508	17	16,215

Note: U-Turns are included in Totals.

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### **Turning Movement Count - Study Results**

### OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

### **Full Study Cyclist Volume**

#### RIVER ST/SLIDELL ST OTTAWA RIVER PKWY

		LIK O 170LIDEL			_		
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	1	1	0	0	0	1
07:15 07:30	1	1	2	0	0	0	2
07:30 07:45	2	1	3	1	0	1	4
07:45 08:00	3	0	3	0	0	0	3
08:00 08:15	4	0	4	0	0	0	4
08:15 08:30	1	0	1	0	0	0	1
08:30 08:45	1	0	1	0	0	0	1
08:45 09:00	1	0	1	0	0	0	1
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	2	0	2	0	0	0	2
09:30 09:45	2	0	2	0	0	0	2
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	1	0	1	1
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	2	2	0	0	0	2
15:15 15:30	1	0	1	0	0	0	1
15:30 15:45	0	1	1	0	1	1	2
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	1	3	4	0	0	0	4
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	2	2	0	0	0	2
16:45 17:00	1	3	4	0	0	0	4
17:00 17:15	1	1	2	0	0	0	2
17:15 17:30	1	0	1	0	0	0	1
17:30 17:45	1	1	2	0	0	0	2
17:45 18:00	2	3	5	0	0	0	5
Total	25	19	44	2	1	3	47

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# **Turning Movement Count - Study Results**

# OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

# **Full Study Pedestrian Volume**

RIVER ST/SLIDELL ST

**OTTAWA RIVER PKWY** 

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	2	0	2	2
07:15 07:30	0	0	0	0	2	2	2
07:30 07:45	0	2	2	2	0	2	4
07:45 08:00	1	0	1	1	2	3	4
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	1	1	2	1	3	4
08:30 08:45	0	0	0	3	3	6	6
08:45 09:00	0	0	0	1	0	1	1
09:00 09:15	2	0	2	0	2	2	4
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	1	1	2	2
09:45 10:00	0	0	0	0	3	3	3
11:30 11:45	0	0	0	1	3	4	4
11:45 12:00	0	2	2	3	4	7	9
12:00 12:15	1	2	3	3	6	9	12
12:15 12:30	0	1	1	9	4	13	14
12:30 12:45	0	4	4	7	4	11	15
12:45 13:00	0	3	3	9	0	9	12
13:00 13:15	0	2	2	4	1	5	7
13:15 13:30	0	1	1	3	0	3	4
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	2	2	2
15:30 15:45	0	0	0	2	1	3	3
15:45 16:00	0	0	0	0	1	1	1
16:00 16:15	0	1	1	1	0	1	2
16:15 16:30	0	0	0	2	2	4	4
16:30 16:45	0	1	1	1	2	3	4
16:45 17:00	0	0	0	0	1	1	1
17:00 17:15	0	0	0	3	2	5	5
17:15 17:30	0	0	0	2	0	2	2
17:30 17:45	1	0	1	1	1	2	3
17:45 18:00	0	0	0	1	3	4	4
Total	5	20	25	64	51	115	140

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# **Turning Movement Count - Study Results**

# OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

# **Full Study Heavy Vehicles**

#### **RIVER ST/SLIDELL ST**

#### **OTTAWA RIVER PKWY**

	1	Northbo	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Period	d LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:1	5 0	1	0	1	0	0	0	0	1	0	0	0	0	0	3	0	3	3	4
07:15 07:3	0 0	0	0	0	0	0	0	0	0	0	3	0	3	0	2	0	2	5	5
07:30 07:4	5 0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
07:45 08:0	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	3	3
08:00 08:1	5 0	0	0	0	0	0	0	0	0	0	1	0	1	0	4	0	4	5	5
08:15 08:3	0 0	0	0	0	0	0	0	0	0	0	2	0	2	0	3	0	3	5	5
08:30 08:4	5 0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
08:45 09:0	0 0	1	0	1	0	1	0	1	2	1	3	0	4	0	4	0	4	8	10
09:00 09:1	5 0	0	0	0	0	0	0	0	0	0	3	0	3	0	4	0	4	7	7
09:15 09:3	0 0	1	0	1	0	0	0	0	1	0	2	0	2	1	2	0	3	5	6
09:30 09:4	5 0	0	0	0	0	0	0	0	0	0	5	0	5	0	3	0	3	8	8
09:45 10:0	0 0	0	0	0	0	1	1	2	2	0	0	0	0	0	6	1	7	7	9
11:30 11:4	5 0	0	0	0	0	1	0	1	1	0	0	0	0	0	1	0	1	1	2
11:45 12:0	0 0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	0	2	4	4
12:00 12:1	5 0	2	0	2	0	2	0	2	4	0	0	0	0	0	2	1	3	3	7
12:15 12:3	0 0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	0	0	0	1
12:30 12:4	5 0	0	0	0	0	1	0	1	1	0	1	0	1	1	2	0	3	4	5
12:45 13:0	0 0	0	0	0	0	1	0	1	1	0	1	0	1	0	0	0	0	1	2
13:00 13:1	5 0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
13:15   13:3	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00 15:1	5 0	0	0	0	0	1	0	1	1	0	0	0	0	0	1	0	1	1	2
15:15 15:3	0 0	0	0	0	0	1	0	1	1	0	0	0	0	0	1	0	1	1	2
15:30 15:4	5 0	0	0	0	0	0	0	0	0	0	2	0	2	0	4	0	4	6	6
15:45 16:0	0 0	0	0	0	0	0	0	0	0	0	2	0	2	0	4	0	4	6	6
16:00 16:1	5 0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
16:15 16:3	0 0	0	0	0	0	1	0	1	1	0	1	0	1	0	0	0	0	1	2
16:30 16:4	5 0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
16:45 17:0	0 0	0	0	0	0	0	0	0	0	0	2	0	2	0	1	0	1	3	3
17:00 17:1	5 0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
17:15 17:3	0 0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
17:30 17:4	5 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45 18:0	0 0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	2	2	2
Total: Non	e 0	5	0	5	0	10	2	12	17	1	35	0	36	3	58	2	63	99	116

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# **Turning Movement Count - Study Results**

# OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST

Survey Date: Tuesday, April 25, 2017 WO No: 36950

Start Time: 07:00 Device: Miovision

# **Full Study 15 Minute U-Turn Total**

RIVER ST/SLIDELL ST OTTAWA RIVER PKWY

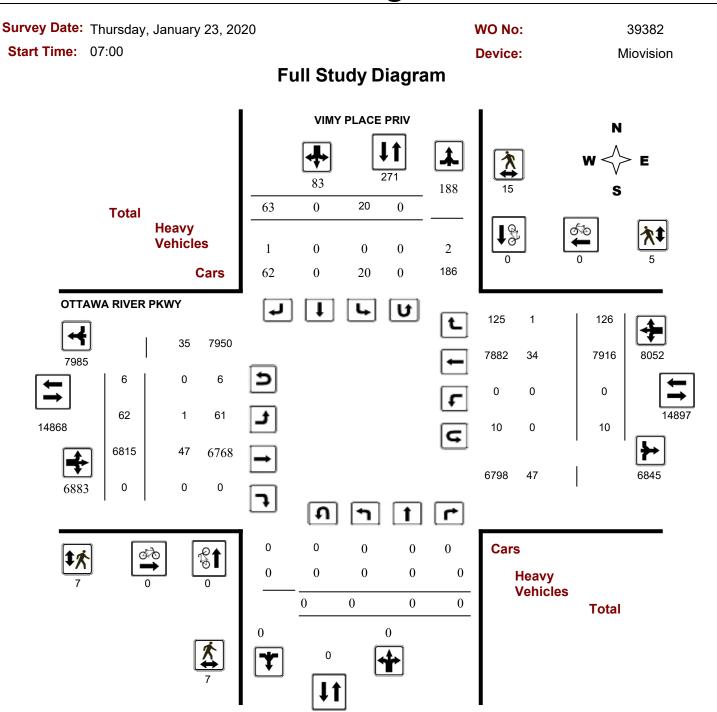
Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	1	0	0	0	1
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	1	1
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	1	1
17:00	17:15	0	0	0	1	1
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	1	0	0	3	4

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### **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV



5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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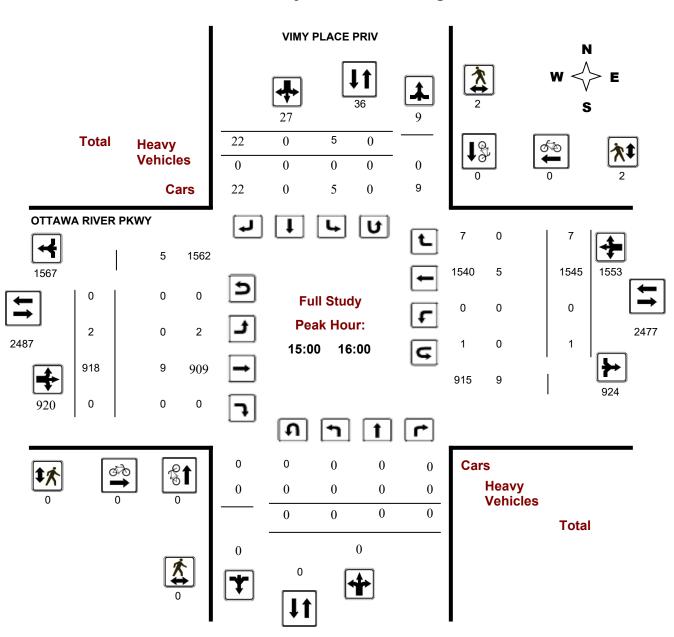
### **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

#### **Full Study Peak Hour Diagram**



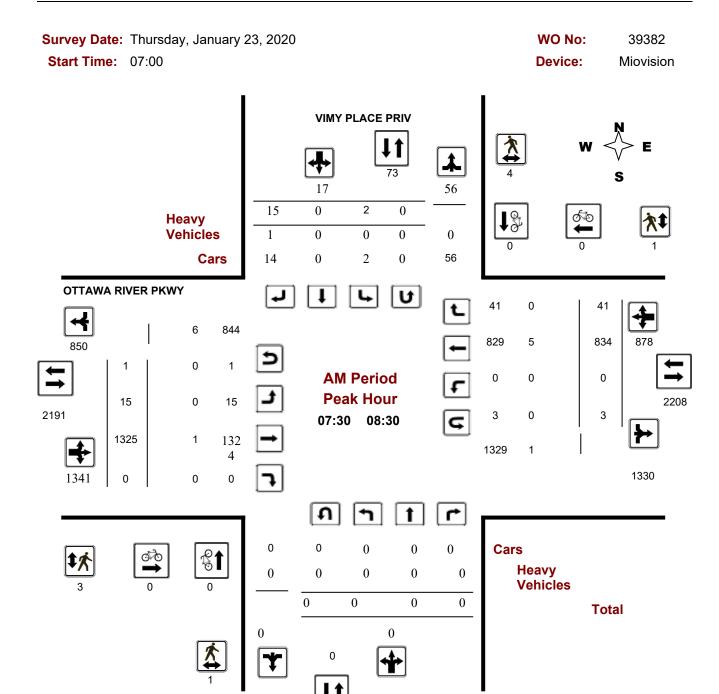
5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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### **Turning Movement Count - Peak Hour Diagram**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV



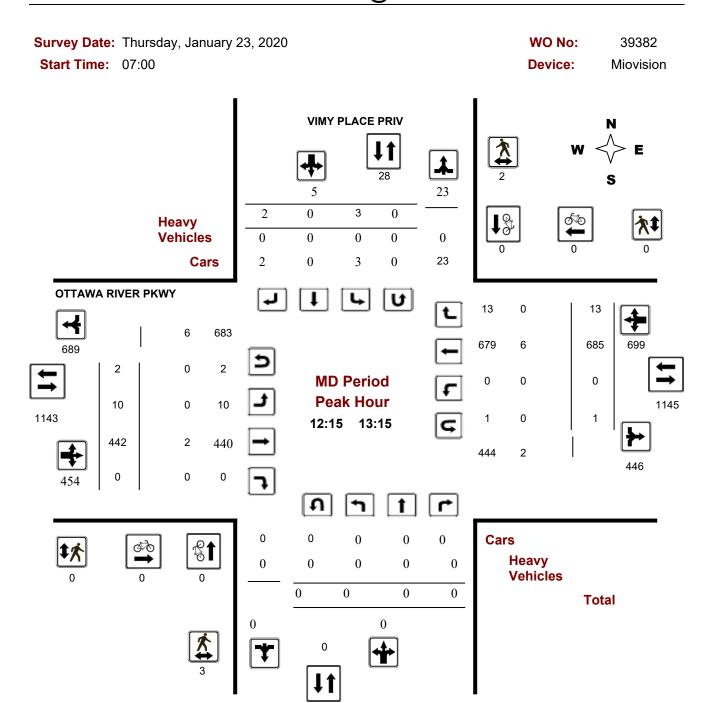
Comments 5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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### **Turning Movement Count - Peak Hour Diagram**

# OTTAWA RIVER PKWY @ VIMY PLACE PRIV



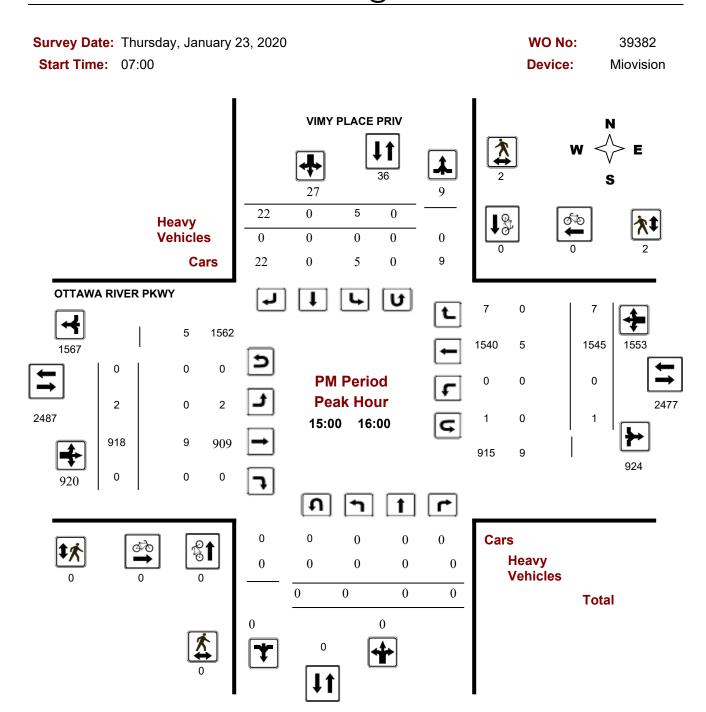
**Comments** 5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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### **Turning Movement Count - Peak Hour Diagram**

# OTTAWA RIVER PKWY @ VIMY PLACE PRIV



Comments 5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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#### **Turning Movement Count - Study Results**

### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Thursday, January 23, 2020 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0

1.00

Eastbound: 6 Westbound: 10

		\	/IMY I	PLACE	PRIV						07	ΓΤΑW	'A RIV	ER Pk	(WY				
	Nor	thbou	nd		Sou	uthbou	ınd			Е	astbou	ınd		٧	Vestbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	0	0	0	3	0	12	15	15	11	1280	0	1291	0	830	26	856	2147	2162
08:00 09:00	0	0	0	0	0	0	7	7	7	21	1338	0	1359	0	780	38	818	2177	2184
09:00 10:00	0	0	0	0	0	0	4	4	4	6	904	0	910	0	523	11	534	1444	1448
11:30 12:30	0	0	0	0	4	0	2	6	6	3	403	0	406	0	613	4	617	1023	1029
12:30 13:30	0	0	0	0	1	0	2	3	3	12	414	0	426	0	660	15	675	1101	1104
15:00 16:00	0	0	0	0	5	0	22	27	27	2	918	0	920	0	1545	7	1552	2472	2499
16:00 17:00	0	0	0	0	5	0	7	12	12	3	823	0	826	0	1637	6	1643	2469	2481
17:00 18:00	0	0	0	0	2	0	7	9	9	4	735	0	739	0	1328	19	1347	2086	2095
Sub Total	0	0	0	0	20	0	63	83	83	62	6815	0	6877	0	7916	126	8042	14919	15002
U Turns				0				0	0				6				10	16	16
Total	0	0	0	0	20	0	63	83	83	62	6815	0	6883	0	7916	126	8052	14935	15018
EQ 12Hr	0	0	0	0	28	0	88	115	115	86	9473	0	9567	0	11003	175	11192	20760	20875
Note: These v	alues ar	e calcu	lated by	/ multiply	ing the	totals b	y the a	opropriate	expans	ion fac	tor.			1.39					
AVG 12Hr	0	0	0	0	26	0	83	109	115	81	8928	0	9017	0	10370	165	10548	20760	20875
Note: These v	olumes	are calc	culated	by multip	lying th	e Equiv	alent 1	2 hr. total	s by the	AADT	factor.			1					
AVG 24Hr	0	0	0	0	34	0	108	142	142	106	11695	0	11812	0	13585	216	13818	25630	25772
Note: These v	olumes	are calc	culated	bv multir	olvina th	e Avera	age Dail	v 12 hr. t	otals by	12 to 2	4 expans	sion fac	ctor.	1.31					

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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# **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

# **Full Study 15 Minute Increments**

**VIMY PLACE PRIV** 

**OTTAWA RIVER PKWY** 

		No	orthbou	und		Sc	uthbou	ınd			Е	astbour	nd		W	estboun	d			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	0	0	0	0	0	0	0	0	0	308	0	308	0	194	5	199	0	507
07:15	07:30	0	0	0	0	1	0	1	2	0	3	321	0	325	0	215	5	221	0	548
07:30	07:45	0	0	0	0	1	0	4	5	1	4	330	0	334	0	216	9	226	1	565
07:45	08:00	0	0	0	0	1	0	7	8	0	4	321	0	326	0	205	7	212	0	546
08:00	08:15	0	0	0	0	0	0	4	4	0	2	337	0	339	0	214	11	226	0	569
08:15	08:30	0	0	0	0	0	0	0	0	0	5	337	0	342	0	199	14	214	0	556
08:30	08:45	0	0	0	0	0	0	1	1	0	8	348	0	356	0	195	8	203	0	560
08:45	09:00	0	0	0	0	0	0	2	2	0	6	316	0	322	0	172	5	177	0	501
09:00	09:15	0	0	0	0	0	0	2	2	0	3	291	0	294	0	142	4	146	0	442
09:15	09:30	0	0	0	0	0	0	1	1	0	1	258	0	259	0	117	2	120	0	380
09:30	09:45	0	0	0	0	0	0	0	0	0	0	178	0	178	0	150	3	154	0	332
09:45	10:00	0	0	0	0	0	0	1	1	0	2	177	0	179	0	114	2	117	0	297
11:30	11:45	0	0	0	0	1	0	1	2	0	1	94	0	95	0	123	0	123	0	220
11:45	12:00	0	0	0	0	0	0	0	0	0	0	107	0	108	0	150	1	152	0	260
12:00	12:15	0	0	0	0	1	0	1	2	0	1	96	0	97	0	148	3	151	0	250
12:15	12:30	0	0	0	0	2	0	0	2	0	1	106	0	107	0	192	0	192	0	301
12:30	12:45	0	0	0	0	0	0	1	1	0	5	119	0	125	0	152	1	153	0	279
12:45	13:00	0	0	0	0	1	0	1	2	0	2	103	0	106	0	145	2	147	0	255
13:00	13:15	0	0	0	0	0	0	0	0	0	2	114	0	116	0	196	10	207	0	323
13:15	13:30	0	0	0	0	0	0	0	0	0	3	78	0	81	0	167	2	169	0	250
15:00	15:15	0	0	0	0	2	0	5	7	0	0	250	0	250	0	392	2	395	0	652
15:15	15:30	0	0	0	0	3	0	9	12	0	1	221	0	222	0	459	1	460	0	694
15:30	15:45	0	0	0	0	0	0	6	6	0	1	222	0	223	0	318	1	319	0	548
15:45	16:00	0	0	0	0	0	0	2	2	0	0	225	0	225	0	376	3	379	0	606
16:00	16:15	0	0	0	0	1	0	0	1	0	0	191	0	192	0	399	2	401	0	594
16:15	16:30	0	0	0	0	3	0	4	7	0	0	212	0	212	0	438	2	440	0	659
16:30	16:45	0	0	0	0	1	0	2	3	0	0	193	0	193	0	400	1	401	0	597
16:45	17:00	0	0	0	0	0	0	1	1	0	3	227	0	230	0	400	1	401	0	632
17:00	17:15	0	0	0	0	1	0	4	5	0	1	209	0	210	0	389	7	396	0	611
17:15	17:30	0	0	0	0	1	0	1	2	0	1	195	0	196	0	361	6	367	0	565
17:30	17:45	0	0	0	0	0	0	1	1	0	1	169	0	170	0	324	3	327	0	498
17:45	18:00	0	0	0	0	0	0	1	1	0	1	162	0	163	0	254	3	257	0	421
Total:		0	0	0	0	20	0	63	83	1	62	6815	0	6883	0	7916	126	8052	1	15,018

Note: U-Turns are included in Totals.

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# **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

# **Full Study Cyclist Volume**

#### VIMY PLACE PRIV OTTAWA RIVER PKWY

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	 Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0

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# **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

# **Full Study Pedestrian Volume**

**VIMY PLACE PRIV** 

**OTTAWA RIVER PKWY** 

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	1	3	4	3	1	4	8
08:15 08:30	0	1	1	0	0	0	1
08:30 08:45	0	2	2	0	0	0	2
08:45 09:00	0	1	1	0	0	0	1
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	1	1	0	0	0	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	1	1	1
12:00 12:15	2	2	4	2	0	2	6
12:15 12:30	0	1	1	0	0	0	1
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	2	0	2	0	0	0	2
13:00 13:15	1	1	2	0	0	0	2
13:15 13:30	0	0	0	1	1	2	2
15:00 15:15	0	1	1	0	0	0	1
15:15 15:30	0	1	1	0	2	2	3
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	1	0	1	1	0	1	2
17:30 17:45	0	1	1	0	0	0	1
17:45 18:00	0	0	0	0	0	0	0
Total	7	15	22	7	5	12	34

5472221 - THU JAN 23, 2020 - 8HRS - LORETTA

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# **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

# **Full Study Heavy Vehicles**

#### VIMY PLACE PRIV

#### **OTTAWA RIVER PKWY**

	N	orthbou	und		Sc	uthbou	ınd			E	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
07:15 07:30	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
07:30 07:45	0	0	0	0	0	0	1	1	1	0	0	0	0	0	3	0	3	3	4
07:45 08:00	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
08:00 08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
08:15 08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2	2	2
08:45 09:00	0	0	0	0	0	0	0	0	0	0	3	0	3	0	2	0	2	5	5
09:00 09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0	0	0	0	5	0	5	0	0	0	0	5	5
09:30 09:45	0	0	0	0	0	0	0	0	0	0	3	0	3	0	0	0	0	3	3
09:45 10:00	0	0	0	0	0	0	0	0	0	0	3	0	3	0	1	0	1	4	4
11:30 11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
11:45 12:00	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
12:00 12:15	0	0	0	0	0	0	0	0	0	1	0	0	1	0	2	0	2	3	3
12:15 12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	2
12:30 12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	3	3
12:45 13:00	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
13:00 13:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
13:15 13:30	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
15:00 15:15	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
15:15 15:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	1	0	1	3	3
15:30 15:45	0	0	0	0	0	0	0	0	0	0	3	0	3	0	2	0	2	5	5
15:45 16:00	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	0	2	4	4
16:00 16:15	0	0	0	0	0	0	0	0	0	0	3	0	3	0	2	0	2	5	5
16:15 16:30	0	0	0	0	0	0	0	0	0	0	3	0	3	0	1	0	1	4	4
16:30 16:45	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
16:45 17:00	0	0	0	0	0	0	0	0	0	0	3	0	3	0	3	0	3	6	6
17:00 17:15	0	0	0	0	0	0	0	0	0	0	3	0	3	0	1	0	1	4	4
17:15 17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
17:45 18:00	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
Total: None	0	0	0	0	0	0	1	1	1	1	47	0	48	0	34	1	35	83	84

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# **Turning Movement Count - Study Results**

#### OTTAWA RIVER PKWY @ VIMY PLACE PRIV

Survey Date: Thursday, January 23, 2020 WO No: 39382

Start Time: 07:00 Device: Miovision

# **Full Study 15 Minute U-Turn Total**

VIMY PLACE PRIV OTTAWA RIVER PKWY

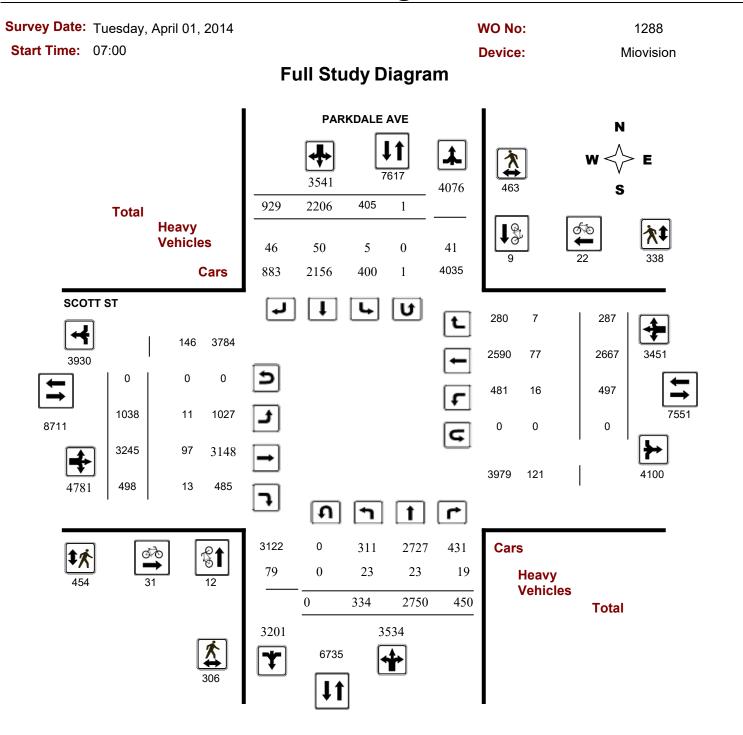
Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	1	1	2
07:30	07:45	0	0	0	1	1
07:45	08:00	0	0	1	0	1
08:00	08:15	0	0	0	1	1
08:15	08:30	0	0	0	1	1
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	1	1
09:30	09:45	0	0	0	1	1
09:45	10:00	0	0	0	1	1
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	1	1	2
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	1	0	1
12:45	13:00	0	0	1	0	1
13:00	13:15	0	0	0	1	1
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	1	1
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	1	0	1
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	0	0	6	10	16

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### **Turning Movement Count - Study Results**

#### **PARKDALE AVE @ SCOTT ST**



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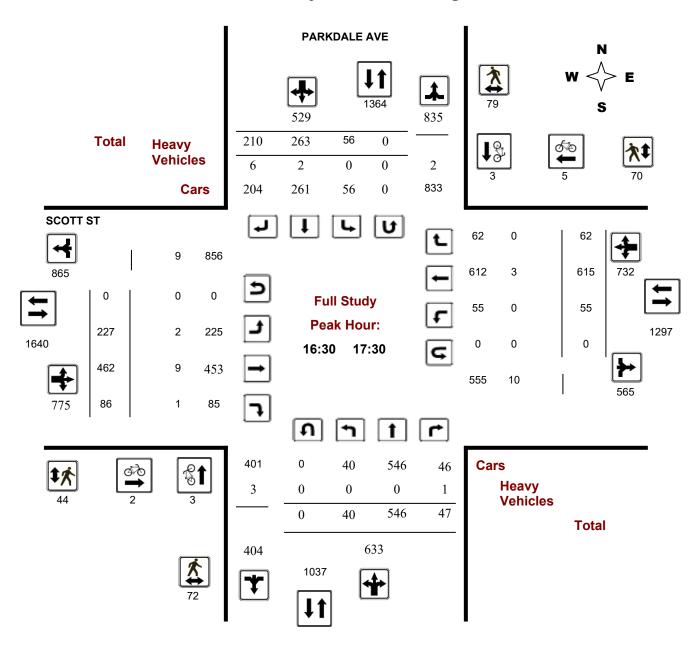
#### **Turning Movement Count - Study Results**

#### **PARKDALE AVE @ SCOTT ST**

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

#### **Full Study Peak Hour Diagram**



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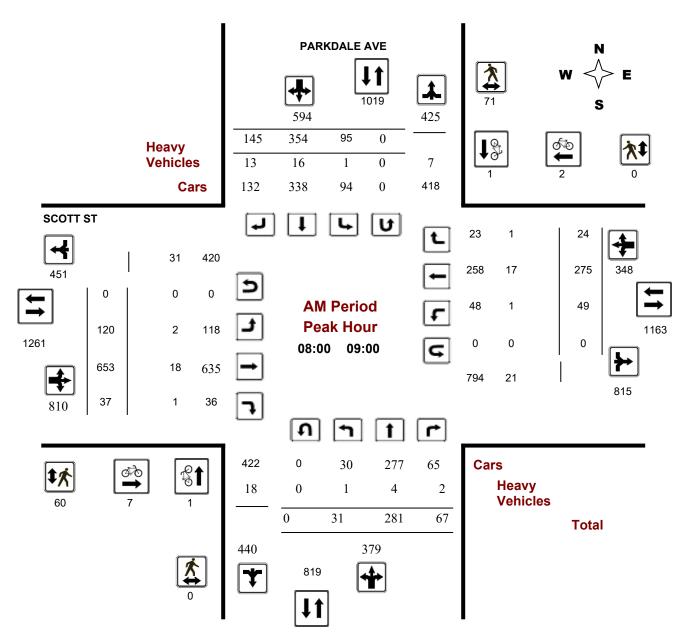


### **Turning Movement Count - Peak Hour Diagram**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision



**Comments** 

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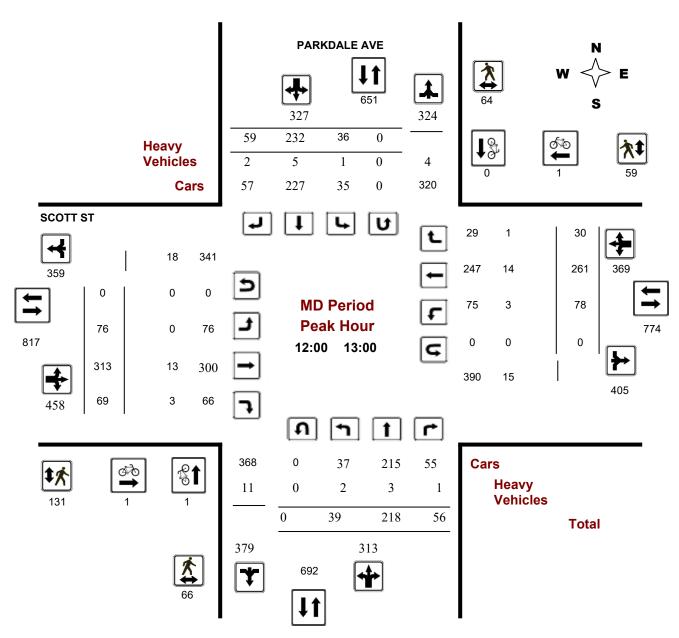


### **Turning Movement Count - Peak Hour Diagram**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision



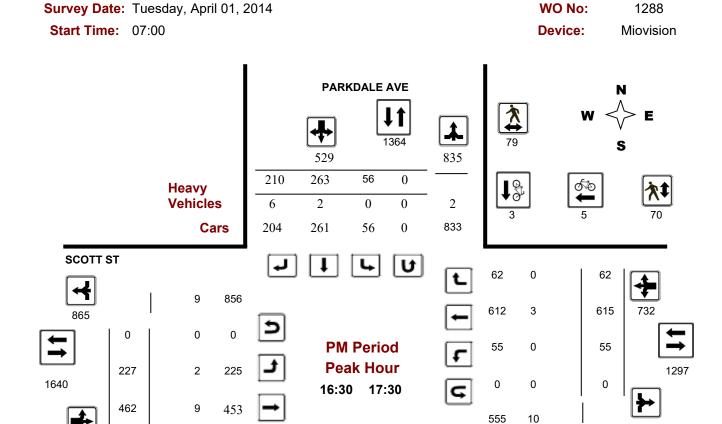
**Comments** 

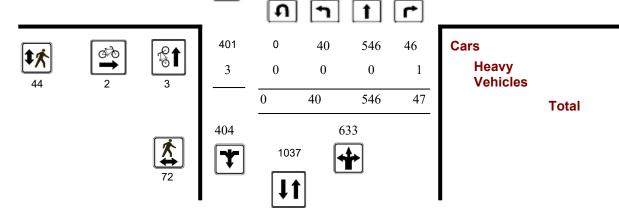
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### **Turning Movement Count - Peak Hour Diagram**

# **PARKDALE AVE @ SCOTT ST**





565

Comments

86

1

85

7

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#### **Turning Movement Count - Study Results**

#### PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Tuesday, April 01, 2014 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 1 .90

Eastbound: 0 Westbound: 0

			PAR	KDALE	AVE							S	COTT	ST					
	No	rthbou	nd		So	uthbou	und			Е	astbou	ınd		٧	√estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	30	227	47	304	63	381	139	583	887	79	356	44	479	59	198	21	278	757	1644
08:00 09:00	31	281	67	379	95	354	145	594	973	120	653	37	810	49	275	24	348	1158	2131
09:00 10:00	53	216	61	330	34	264	89	387	717	58	377	54	489	63	221	24	308	797	1514
11:30 12:30	41	194	60	295	43	240	75	358	653	66	316	68	450	55	240	33	328	778	1431
12:30 13:30	48	235	54	337	35	218	55	308	645	70	289	67	426	79	256	31	366	792	1437
15:00 16:00	53	508	63	624	34	244	106	384	1008	202	367	72	641	50	349	36	435	1076	2084
16:00 17:00	34	590	47	671	55	237	166	458	1129	238	461	65	764	52	550	65	667	1431	2560
17:00 18:00	44	499	51	594	46	268	154	468	1062	205	426	91	722	90	578	53	721	1443	2505
Sub Total	334	2750	450	3534	405	2206	929	3540	7074	1038	3245	498	4781	497	2667	287	3451	8232	15306
U Turns				0				1	1				0				0	0	1
Total	334	2750	450	3534	405	2206	929	3541	7075	1038	3245	498	4781	497	2667	287	3451	8232	15307
EQ 12Hr	464	3822	626	4912	563	3066	1291	4922	9834	1443	4511	692	6646	691	3707	399	4797	11442	21277
Note: These	values a	re calcu	lated by	y multiply	ing the	totals b	y the a	opropriat	e expans	sion fac	tor.			1.39					
AVG 12Hr	394	3242	531	4167	477	2601	1095	4175	8851	1224	3826	587	5637	586	3144	338	4069	10298	19149
Note: These	volumes	are calc	culated	by multi	olying t	he Equiv	valent 1	2 hr. tota	als by the	AADT	factor.			0.9					
AVG 24Hr	516	4247	695	5458	626	3407	1435	5469	10927	1603	5012	769	7384	768	4119	443	5330	12714	23641
Note: These	volumes	are calc	culated	by multi <sub>l</sub>	olying t	he Avera	age Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

# **Full Study 15 Minute Increments**

#### **PARKDALE AVE**

SCOTT ST

		No	orthbou	und		Sc	uthbou	nd			Е	astbour	nd		We	estbour	nd			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	2	75	6	83	14	94	30	138	3	13	58	8	79	11	43	4	58	3	358
07:15	07:30	9	49	11	69	11	102	31	144	7	16	76	11	103	17	41	4	62	7	378
07:30	07:45	4	52	12	68	16	93	36	145	4	25	97	12	134	15	58	9	82	4	429
07:45	08:00	15	51	18	84	22	92	42	156	4	25	125	13	163	16	56	4	76	4	479
08:00	08:15	11	77	11	99	31	80	27	138	7	30	171	5	206	14	72	6	92	7	535
08:15	08:30	9	67	12	88	16	92	36	144	5	29	166	7	202	7	61	4	72	5	506
08:30	08:45	6	63	23	92	23	96	38	157	13	37	149	11	197	12	65	7	84	13	530
08:45	09:00	5	74	21	100	25	86	44	155	12	24	167	14	205	16	77	7	100	12	560
09:00	09:15	15	72	14	101	12	69	28	109	5	19	132	13	164	13	67	11	91	5	465
09:15	09:30	16	65	22	103	7	78	31	116	13	18	92	9	119	12	59	6	77	13	415
09:30	09:45	12	28	9	49	10	77	17	104	3	13	94	19	126	22	47	3	72	3	351
09:45	10:00	10	51	16	77	5	40	13	58	5	8	59	13	80	16	48	4	68	5	283
11:30	11:45	16	43	16	75	10	60	24	95	5	15	84	14	113	14	58	5	77	5	360
11:45	12:00	10	54	13	77	17	60	20	97	5	14	78	13	105	10	58	17	85	5	364
12:00	12:15	11	47	13	71	11	61	11	83	4	13	93	21	127	17	64	6	87	4	368
12:15	12:30	4	50	18	72	5	59	20	84	3	24	61	20	105	14	60	5	79	3	340
12:30	12:45	12	64	12	88	10	52	16	78	5	20	67	11	98	26	67	11	104	5	368
12:45	13:00	12	57	13	82	10	60	12	82	2	19	92	17	128	21	70	8	99	2	391
13:00	13:15	11	63	11	85	6	54	12	72	5	11	70	19	100	18	64	8	90	5	347
13:15	13:30	13	51	18	82	9	52	15	76	8	20	60	20	100	14	55	4	73	8	331
15:00	15:15	14	109	12	135	7	72	20	99	5	31	76	19	126	12	64	5	81	5	441
15:15	15:30	14	130	14	158	5	59	28	92	9	61	96	27	184	15	73	10	98	9	532
15:30	15:45	14	138	18	170	12	61	31	104	5	48	97	9	154	9	100	9	118	5	546
15:45	16:00	11	131	19	161	10	52	27	89	6	62	98	17	177	14	112	12	138	6	565
16:00	16:15	10	149	11	170	15	55	23	93	2	63	123	13	199	22	130	12	164	2	626
16:15	16:30	7	156	13	176	7	56	35	98	4	59	108	14	181	13	137	25	175	4	630
16:30	16:45	7	158	15	180	16	63	49	128	2	58	131	20	209	6	134	17	157	2	674
16:45	17:00	10	127	8	145	17	63	59	139	3	58	99	18	175	11	149	11	171	3	630
17:00	17:15	13	125	16	154	13	79	53	145	3	50	138	23	211	19	169	22	210	3	720
17:15	17:30	10	136	8	154	10	58	49	117	1	61	94	25	180	19	163	12	194	1	645
17:30	17:45	15	127	12	154	12	77	30	119	6	46	93	24	163	32	122	6	160	6	596
17:45	18:00	6	111	15	132	11	54	22	87	2	48	101	19	168	20	124	13	157	2	544
Total:		334	2750	450	3534	405	2206	929	3541	166	1038	3245	498	4781	497	2667	287	3451	166	15,307

Note: U-Turns are included in Totals.

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

# **Full Study Cyclist Volume**

#### PARKDALE AVE SCOTT ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	 Grand Total
07:00 07:15	0	0	0	3	0	3	3
07:15 07:30	0	0	0	1	0	1	1
07:30 07:45	2	0	2	2	1	3	5
07:45 08:00	0	0	0	2	0	2	2
08:00 08:15	0	1	1	2	1	3	4
08:15 08:30	1	0	1	2	0	2	3
08:30 08:45	0	0	0	1	0	1	1
08:45 09:00	0	0	0	2	1	3	3
09:00 09:15	0	0	0	3	0	3	3
09:15 09:30	0	0	0	0	1	1	1
09:30 09:45	0	0	0	1	0	1	1
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	1	0	1	0	0	0	1
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	1	1	2	2
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	1	0	1	1	0	1	2
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	1	0	1	1
15:15 15:30	0	1	1	1	0	1	2
15:30 15:45	0	1	1	1	0	1	2
15:45 16:00	0	1	1	0	0	0	1
16:00 16:15	2	1	3	2	0	2	5
16:15 16:30	2	1	3	2	1	3	6
16:30 16:45	2	0	2	0	2	2	4
16:45 17:00	0	0	0	0	2	2	2
17:00 17:15	0	1	1	0	1	1	2
17:15 17:30	1	2	3	2	0	2	5
17:30 17:45	0	0	0	1	2	3	3
17:45 18:00	0	0	0	0	9	9	9
Total	12	9	21	31	22	53	74

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

**Start Time:** 07:00 Device: Miovision

#### **Full Study Pedestrian Volume SCOTT ST**

**PARKDALE AVE** 

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	7	13	20	12	6	18	38
07:15 07:30	4	14	18	8	12	20	38
07:30 07:45	8	18	26	9	12	21	47
07:45 08:00	18	17	35	20	10	30	65
08:00 08:15	0	23	23	11	0	11	34
08:15 08:30	0	10	10	16	0	16	26
08:30 08:45	0	18	18	19	0	19	37
08:45 09:00	0	20	20	14	0	14	34
09:00 09:15	1	1	2	8	0	8	10
09:15 09:30	2	6	8	11	0	11	19
09:30 09:45	2	4	6	11	9	20	26
09:45 10:00	3	2	5	8	6	14	19
11:30 11:45	6	14	20	8	13	21	41
11:45 12:00	9	11	20	12	13	25	45
12:00 12:15	15	28	43	32	22	54	97
12:15 12:30	19	17	36	33	15	48	84
12:30 12:45	15	12	27	29	12	41	68
12:45 13:00	17	7	24	37	10	47	71
13:00 13:15	6	11	17	10	8	18	35
13:15 13:30	9	6	15	16	10	26	41
15:00 15:15	1	4	5	10	9	19	24
15:15 15:30	12	13	25	15	11	26	51
15:30 15:45	13	21	34	15	23	38	72
15:45 16:00	9	18	27	10	15	25	52
16:00 16:15	15	16	31	12	15	27	58
16:15 16:30	17	21	38	14	10	24	62
16:30 16:45	16	17	33	14	18	32	65
16:45 17:00	16	22	38	10	22	32	70
17:00 17:15	18	21	39	14	15	29	68
17:15 17:30	22	19	41	6	15	21	62
17:30 17:45	7	14	21	5	10	15	36
17:45 18:00	19	25	44	5	17	22	66
Total	306	463	769	454	338	792	1561

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

# **Full Study Heavy Vehicles**

#### PARKDALE AVE SCOTT ST

	١	Northbo	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Period	d LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:1	5 0	1	0	1	0	0	2	2	3	1	3	1	5	0	2	0	2	7	10
07:15 07:3	0 1	0	0	1	1	1	4	6	7	0	3	1	4	0	3	0	3	7	14
07:30 07:4	5 0	1	0	1	1	1	1	3	4	2	3	0	5	1	5	0	6	11	15
07:45 08:0	0 1	0	1	2	0	1	1	2	4	0	2	0	2	0	2	0	2	4	8
08:00 08:1	5 1	2	0	3	0	0	4	4	7	1	6	0	7	1	6	0	7	14	21
08:15 08:3	0 0	1	0	1	0	1	3	4	5	0	2	0	2	0	3	0	3	5	10
08:30 08:4	5 0	1	2	3	0	7	3	10	13	1	4	0	5	0	4	0	4	9	22
08:45 09:0	0 0	0	0	0	1	8	3	12	12	0	6	1	7	0	4	1	5	12	24
09:00 09:1	5 0	1	1	2	0	2	1	3	5	1	3	0	4	0	2	0	2	6	11
09:15 09:3	0 0	1	4	5	0	8	0	8	13	0	4	0	4	1	1	2	4	8	21
09:30 09:4	5 0	1	0	1	0	2	0	2	3	0	6	1	7	3	3	0	6	13	16
09:45 10:0	0 2	1	2	5	0	0	0	0	5	0	2	1	3	0	2	0	2	5	10
11:30 11:4	5 0	3	0	3	0	1	1	2	5	0	1	1	2	2	2	0	4	6	11
11:45 12:0	0 0	1	2	3	0	2	0	2	5	1	2	0	3	1	2	0	3	6	11
12:00 12:1	5 0	0	0	0	0	3	1	4	4	0	3	1	4	0	3	0	3	7	11
12:15 12:3	0 0	1	1	2	0	1	0	1	3	0	3	1	4	1	6	0	7	11	14
12:30 12:4	5 2	1	0	3	0	1	1	2	5	0	4	1	5	0	3	1	4	9	14
12:45   13:0	0 0	1	0	1	1	0	0	1	2	0	3	0	3	2	2	0	4	7	9
13:00 13:1	5 1	0	1	2	1	1	1	3	5	0	4	1	5	1	1	0	2	7	12
13:15   13:3	0 1	2	1	4	0	4	0	4	8	0	2	0	2	1	2	1	4	6	14
15:00 15:1	5 4	0	0	4	0	0	1	1	5	0	3	0	3	0	5	0	5	8	13
15:15 15:3	0 3	2	0	5	0	1	3	4	9	2	3	0	5	0	3	2	5	10	19
15:30 15:4	5 2	0	1	3	0	1	1	2	5	0	3	0	3	0	2	0	2	5	10
15:45 16:0	0 3	0	1	4	0	0	2	2	6	0	5	2	7	1	3	0	4	11	17
16:00 16:1		0	0	1	0	1	0	1	2	0	3	0	3	1	0	0	1	4	6
16:15 16:3	_	0	0	1	0	0	3	3	4	0	1	0	1	0	0	0	0	1	5
16:30 16:4		0	0	0	0	1	1	2	2	1	3	1	5	0	0	0	0	5	7
16:45 17:0	0 0	0	0	0	0	1	2	3	3	0	2	0	2	0	0	0	0	2	5
17:00 17:1	_	0	1	1	0	0	2	2	3	1	3	0	4	0	0	0	0	4	7
17:15 17:3	_	0	0	0	0	0	1	1	1	0	1	0	1	0	3	0	3	4	5
17:30 17:4	5 0	2	1	3	0	1	2	3	6	0	3	0	3	0	2	0	2	5	11
17:45 18:0	0 0	0	0	0	0	0	2	2	2	0	1	0	1	0	1	0	1	2	4
Total: Non	e 23	23	19	65	5	50	46	101	166	11	97	13	121	16	77	7	100	221	387

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SCOTT ST

Survey Date: Tuesday, April 01, 2014 WO No: 1288

Start Time: 07:00 Device: Miovision

# Full Study 15 Minute U-Turn Total PARKDALE AVE SCOTT ST

Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	1	0	0	1
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	1	0	0	1

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#### **Turning Movement Count - Study Results**

# PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

Survey Date: Wednesday, February 26, 2020 WO No: **Start Time:** 07:00 **Device:** Miovision **Full Study Diagram** PARKDALE AVE **Total** Heavy **Vehicles** Cars **SJAM RAMPS** U Ð Cars Heavy **Vehicles Total** 

PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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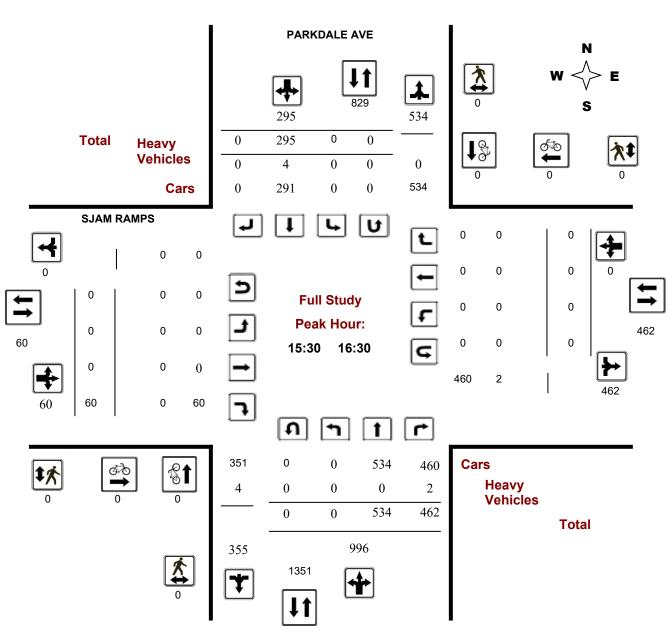
#### **Turning Movement Count - Study Results**

### PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

Survey Date: Wednesday, February 26, 2020 WO No: 39635

Start Time: 07:00 Device: Miovision

### **Full Study Peak Hour Diagram**



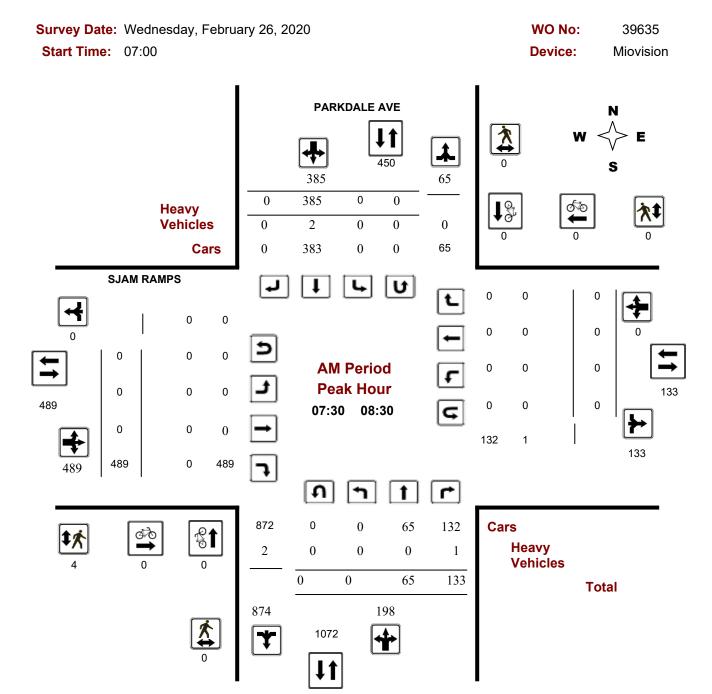
PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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# **Turning Movement Count - Peak Hour Diagram**

#### PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS



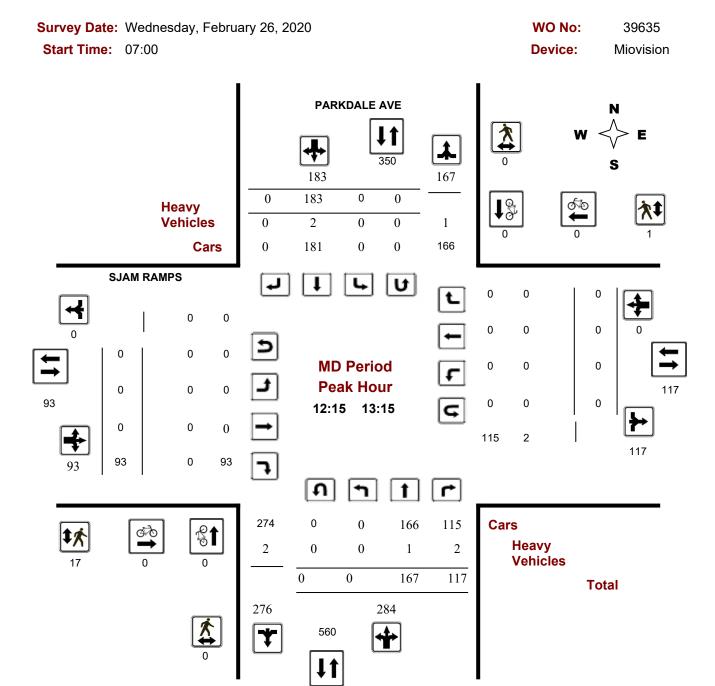
Comments PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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# **Turning Movement Count - Peak Hour Diagram**

#### PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS



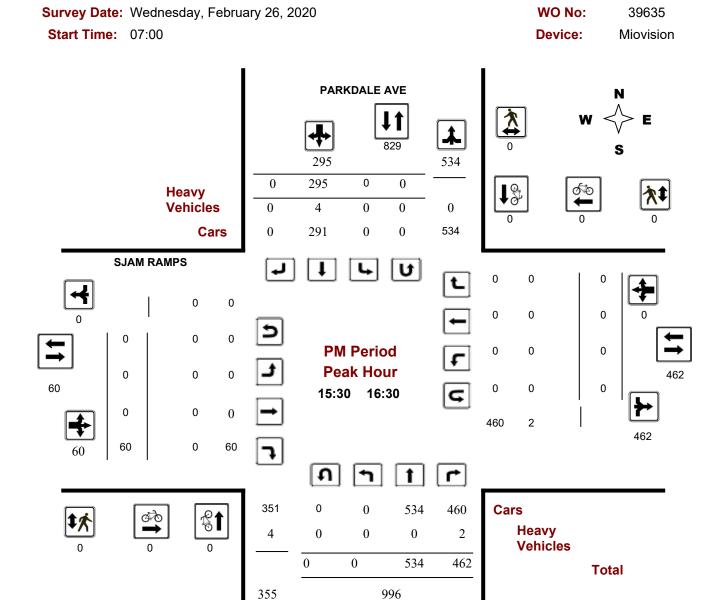
Comments PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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# **Turning Movement Count - Peak Hour Diagram**

#### PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS



Comments PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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#### **Turning Movement Count - Study Results**

# PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

Survey Date: Wednesday, February 26, 2020 WO No: 39635

Start Time: 07:00 Device: Miovision

**Full Study Summary (8 HR Standard)** 

Survey Date: Wednesday, February 26, Total Observed U-Turns AADT Factor

2020 Northbound: 0 Southbound: 0

Eastbound: 0 Westbound: 0 1.00

PARKDALE AVE SJAM RAMPS

	No	rthbou	ınd		So	uthbou	nd			Ea	astbou	ınd		W	estbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Tota
07:00 08:00	0	61	109	170	0	369	0	369	539	0	0	484	484	0	0	0	0	484	1023
08:00 09:00	0	74	131	205	0	385	0	385	590	0	0	415	415	0	0	0	0	415	1005
09:00 10:00	0	65	101	166	0	230	0	230	396	0	0	205	205	0	0	0	0	205	601
11:30 12:30	0	130	134	264	0	168	0	168	432	0	0	96	96	0	0	0	0	96	528
12:30 13:30	0	177	120	297	0	158	0	158	455	0	0	90	90	0	0	0	0	90	545
15:00 16:00	0	582	473	1055	0	212	0	212	1267	0	0	54	54	0	0	0	0	54	1321
16:00 17:00	0	480	420	900	0	347	0	347	1247	0	0	59	59	0	0	0	0	59	1306
17:00 18:00	0	416	264	680	0	224	0	224	904	0	0	73	73	0	0	0	0	73	977
Sub Total	0	1985	1752	3737	0	2093	0	2093	5830	0	0	1476	1476	0	0	0	0	1476	7306
U Turns				0				0	0				0				0	0	0
Total	0	1985	1752	3737	0	2093	0	2093	5830	0	0	1476	1476	0	0	0	0	1476	7306
EQ 12Hr	0	2759	2435	5194	0	2909	0	2909	8104	0	0	2052	2052	0	0	0	0	2052	10155
Note: These va	alues a	re calcu	ılated by	y multiply	ing the	totals b	y the ap	opropriat	e expansi	ion facto	or.			1.39					
AVG 12Hr	0	2600	2295	4895	0	2742	0	2742	8104	0	0	1934	1934	0	0	0	0	2052	10155
Note: These v	olumes	are cal	culated	by multip	olying t	he Equiv	alent 1	2 hr. tota	ls by the	AADT fa	actor.			1					
AVG 24Hr	0	3406	3007	6413	0	3592	0	3592	10005	0	0	2533	2533	0	0	0	0	2533	12538

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. 1.31

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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**PARKDALE AVE** 

#### **Transportation Services - Traffic Services**

#### **Turning Movement Count - Study Results**

#### PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

Survey Date: Wednesday, February 26, 2020 WO No: 39635

Start Time: 07:00 Device: Miovision

# Full Study 15 Minute Increments SJAM RAMPS

Northbound Eastbound Westbound Southbound s STR W **STR** Grand Ε **Time Period** LT ST LT ST RT LT ST RT LT ST RT TOT TOT TOT TOT TOT TOT **Total** 07:00 07:15 n 07:15 07:30 07:30 07:45 07:45 08:00 08:00 08:15 08:15 08:30 n 08:45 08:30 08:45 09:00 09:15 09:00 09:15 09:30 09:30 09:45 09:45 10:00 11:30 11:45 O n 11:45 12:00 12:00 12:15 12:15 12:30 12:30 12:45 12:45 13:00 13:00 13:15 O 13:15 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 16:00 16:15 n 16:15 16:30 16:30 16:45 16:45 17:00 17:00 17:15 17:30 17:15 n 17:30 17:45 n n 17:45 18:00 7,306 Total:

Note: U-Turns are included in Totals.

March 27, 2020 Page 4 of 8



# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

**Survey Date:** Wednesday, February 26, 2020 **WO No:** 39635

Start Time: 07:00 Device: Miovision

# **Full Study Cyclist Volume**

PARKDALE AVE SJAM RAMPS

				Married Egyphorund 194 (1 1 0) (T. )						
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total			
07:00 07:15	0	0	0	0	0	0	0			
07:15 07:30	0	1	1	0	0	0	1			
07:30 07:45	0	0	0	0	0	0	0			
07:45 08:00	0	0	0	0	0	0	0			
08:00 08:15	0	0	0	0	0	0	0			
08:15 08:30	0	0	0	0	0	0	0			
08:30 08:45	0	0	0	0	0	0	0			
08:45 09:00	0	0	0	0	0	0	0			
09:00 09:15	0	0	0	0	0	0	0			
09:15 09:30	0	0	0	0	0	0	0			
09:30 09:45	0	0	0	0	0	0	0			
09:45 10:00	0	0	0	0	0	0	0			
11:30 11:45	0	0	0	0	0	0	0			
11:45 12:00	0	0	0	0	0	0	0			
12:00 12:15	0	0	0	0	0	0	0			
12:15 12:30	0	0	0	0	0	0	0			
12:30 12:45	0	0	0	0	0	0	0			
12:45 13:00	0	0	0	0	0	0	0			
13:00 13:15	0	0	0	0	0	0	0			
13:15 13:30	0	0	0	0	0	0	0			
15:00 15:15	0	0	0	0	0	0	0			
15:15 15:30	0	0	0	0	0	0	0			
15:30 15:45	0	0	0	0	0	0	0			
15:45 16:00	0	0	0	0	0	0	0			
16:00 16:15	0	0	0	0	0	0	0			
16:15 16:30	0	0	0	0	0	0	0			
16:30 16:45	0	0	0	0	0	0	0			
16:45 17:00	0	0	0	0	0	0	0			
17:00 17:15	0	0	0	0	0	0	0			
17:15 17:30	0	0	0	0	0	0	0			
17:30 17:45	0	0	0	0	0	0	0			
17:45 18:00	0	0	0	0	0	0	0			
Total	0	1	1	0	0	0	1			

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

**Survey Date:** Wednesday, February 26, 2020 **WO No:** 39635

Start Time: 07:00 Device: Miovision

# Full Study Pedestrian Volume PARKDALE AVE SJAM RAMPS

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	2	0	2	2
08:00 08:15	0	0	0	2	0	2	2
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	1	0	1	1
12:00 12:15	0	0	0	3	0	3	3
12:15 12:30	0	0	0	4	0	4	4
12:30 12:45	0	0	0	9	1	10	10
12:45 13:00	0	0	0	4	0	4	4
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	1	0	1	1
15:15 15:30	0	0	0	1	0	1	1
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	1	0	1	1
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0
Total	0	0	0	28	1	29	29

PARKDALE AVE @ SJAM RAMPS - FEB 26 2020 - 8HR

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# **Turning Movement Count - Study Results**

# X 2 B Geo\_ID DO NOT APPROVE @ X 2 B Geo\_ID DO NOT APPROVE

Survey Date: Wednesday, February 26, 2020 WO No: 39635

Start Time: 07:00 Device: Miovision

# **Full Study Heavy Vehicles**

#### **PARKDALE AVE**

SJAM RAMPS

	No	orthbou	und		Sc	uthbou	ınd			E	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	0	2	2	0	1	0	1	3	0	0	0	0	0	0	0	0	0	3
07:15 07:30	0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
07:30 07:45	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
07:45 08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
08:15 08:30	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
08:30 08:45	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
08:45 09:00	0	0	0	0	0	2	0	2	2	0	0	2	2	0	0	0	0	2	4
09:00 09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
09:45 10:00	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
11:30 11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45   12:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 12:30	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
12:30 12:45	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
12:45   13:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
13:00 13:15	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
13:15   13:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00 15:15	0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
15:15 15:30	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
15:30 15:45	0	0	1	1	0	3	0	3	4	0	0	0	0	0	0	0	0	0	4
15:45 16:00	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
16:00 16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
16:30 16:45	0	1	1	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
16:45 17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
17:15 17:30	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
17:30 17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45 18:00	0	0	1	1	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
Total: None	0	4	11	15	0	16	0	16	31	0	0	2	2	0	0	0	0	2	33

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# **Turning Movement Count - Study Results**

# PARKDALE AVE @ SIR JOHN A. MACDONALD PKWY RAMPS

Survey Date: Wednesday, February 26, 2020 WO No: 39635

Start Time: 07:00 Device: Miovision

# Full Study 15 Minute U-Turn Total PARKDALE AVE SJAM RAMPS

Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	0	0	0	0

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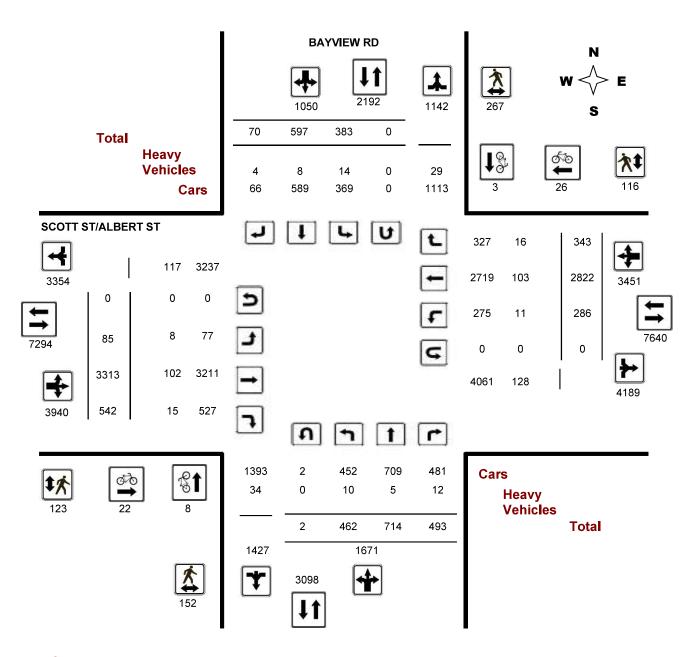


### **Turning Movement Count - Full Study Diagram**

# BAYVIEW RD @ SCOTT ST/ALBERT ST

Survey Date: Tuesday, April 01, 2014 WO#: 1292

**Device:** Miovision



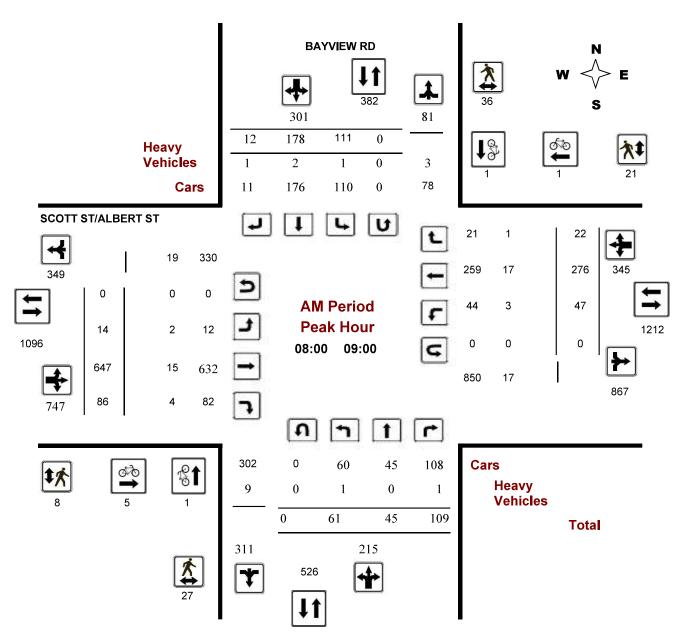
Comments



### **Turning Movement Count - Peak Hour Diagram**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**



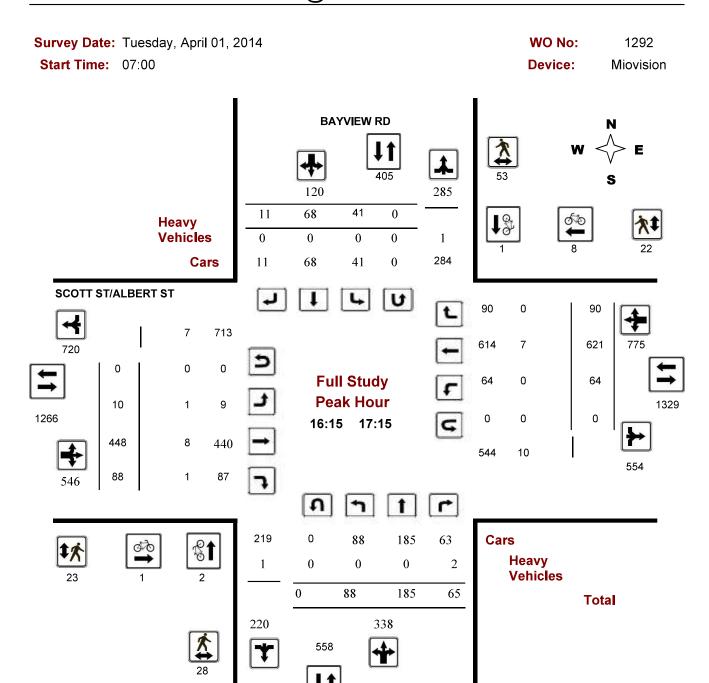


**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**



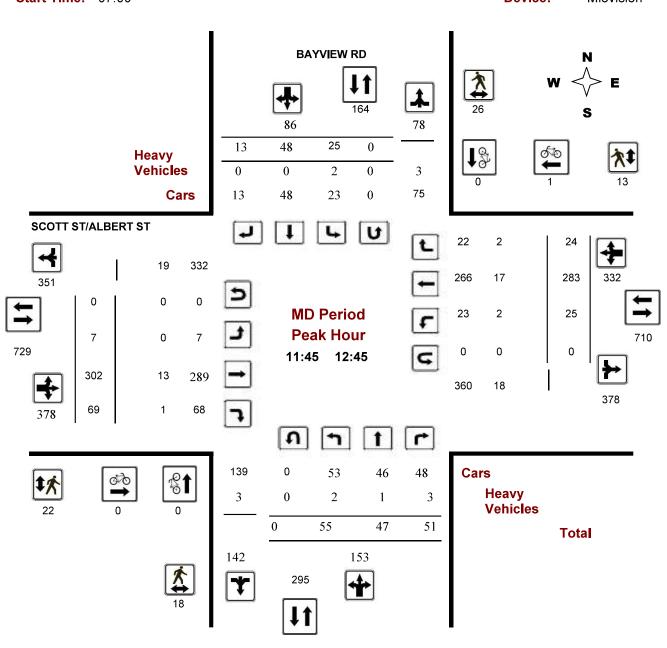
**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**

Survey Date: Tuesday, April 01, 2014 WO No: 1292
Start Time: 07:00 Device: Miovision

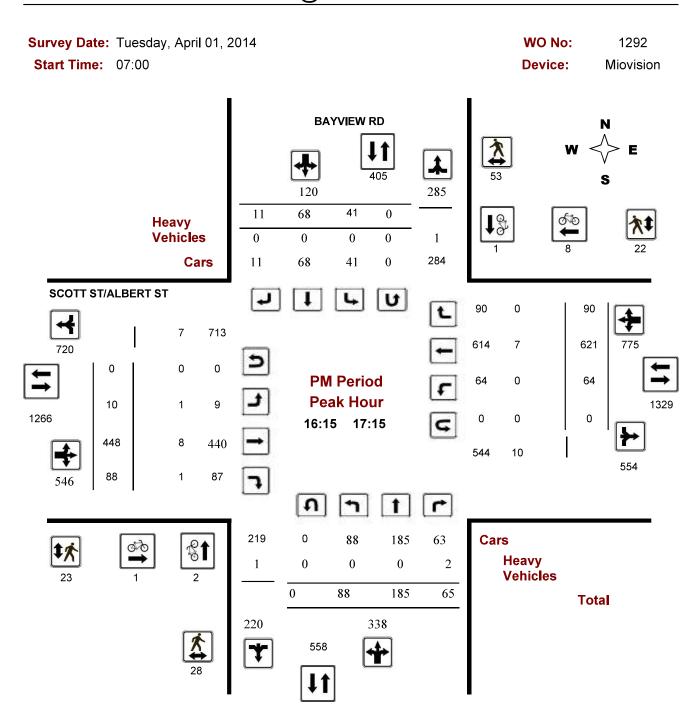


**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**



**Comments** 



**Work Order** 

1292

# **Turning Movement Count - Full Study Summary Report**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**

Survey Date: Tuesday, April 01, 2014

#### **Total Observed U-Turns**

**AADT Factor** 

Northbound: 2

Southbound: 0

.90

Eastbound:

0 Westbound: 0

#### **Full Study**

			В	AYVIE	W RD						;	SCOT	T ST/A	LBER	TST				
-	١	orthbo	ound		S	Southb	ound		_		Eastbo	ound			Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	29	34	58	121	78	101	3	182	303	10	420	47	477	25	227	21	273	750	1053
08:00 09:00	61	45	109	215	111	178	12	301	516	14	647	86	747	47	276	22	345	1092	1608
09:00 10:00	34	28	50	112	48	64	9	121	233	12	384	45	441	36	222	29	287	728	961
11:30 12:30	49	43	54	146	32	53	11	96	242	7	318	72	397	20	268	17	305	702	944
12:30 13:30	55	37	48	140	28	40	14	82	222	19	278	64	361	23	269	24	316	677	899
15:00 16:00	72	185	45	302	20	54	6	80	382	6	391	71	468	22	364	76	462	930	1312
16:00 17:00	91	212	59	362	36	60	10	106	468	12	449	87	548	57	575	98	730	1278	1746
17:00 18:00	71	130	70	271	30	47	5	82	353	5	426	70	501	56	621	56	733	1234	1587
Sub Total	462	714	493	1669	383	597	70	1050	2719	85	3313	542	3940	286	2822	343	3451	7391	10110
U Turns				2				0	2				0				0	0	2
Total	462	714	493	1671	383	597	70	1050	2721	85	3313	542	3940	286	2822	343	3451	7391	10112
EQ 12Hr	642	992	685	2323	532	830	97	1460	3783	118	4605	753	5477	398	3923	477	4797	10274	14057
Note: These	va <b>l</b> ues a	re ca <b>l</b> cul	lated by	/ multiply	ing the	totals b	y the ap	propriat	e expans	ion fact	tor.		1	.39					
AVG 12Hr	578	893	617	2090	479	747	88	1314	3404	106	4145	678	4929	358	3530	429	4317	9246	12650
Note: These	vo <b>l</b> umes	are calc	culated	by mu <b>l</b> tip	olying th	ne Equiv	alent 12	2 hr. tota	<b>I</b> s by the	AADT	factor.			90					
AVG 24Hr	757	1170	808	2738	628	978	115	1721	4459	139	5429	888	6457	469	4625	562	5656	12113	16572
Note: These	volumes	are calc	culated	by mu <b>l</b> tip	olying th	ne Avera	ige Dail	y 12 hr.	totals by	12 to 2	4 expan	sion fac	tor.	1.31					

#### Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



W.O.

1292

# **Turning Movement Count - 15 Minute Summary Report**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**

Survey Date: Tuesday, April 01, 2014

**Total Observed U-Turns** 

 $\begin{array}{cccc} \text{Northbound:} & 2 & & \text{Southbound:} & 0 \\ \text{Eastbound:} & 0 & & \text{Westbound:} & 0 \\ \end{array}$ 

**BAYVIEW RD** 

#### **SCOTT ST/ALBERT ST**

					VIEVV						_		113	IALD						
		N	orthbo	und	N	Soi	uthbour	nd	s	STR	Ea	stbound		Е	vve	stbound	d	w	STR	Grand
Time F	Period	LT	ST	RT	TOT	LT	ST	RT	TOT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	TOT	Total
07:00	07:15	5	5	6	16	11	12	0	23	39	1	71	9	81	3	43	7	53	134	173
07:15	07:30	8	6	13	27	21	32	0	53	80	4	78	15	97	8	56	3	67	164	244
07:30	07:45	9	11	25	47	23	32	1	56	103	1	119	13	133	9	51	6	66	199	302
07:45	08:00	7	12	14	33	23	25	2	50	83	4	152	10	166	5	77	5	87	253	336
08:00	08:15	14	10	33	57	32	40	3	75	132	3	149	22	174	15	65	7	87	261	393
08:15	08:30	10	10	35	55	27	55	1	83	138	3	155	29	187	7	48	5	60	247	385
08:30	08:45	15	10	24	49	29	47	3	79	128	3	175	13	191	11	78	7	96	287	415
08:45	09:00	22	15	17	54	23	36	5	64	118	5	168	22	195	14	85	3	102	297	415
09:00	09:15	13	12	15	40	8	24	3	35	75	3	130	11	144	13	70	7	90	234	309
09:15	09:30	11	7	16	34	18	15	0	33	67	4	102	10	116	8	54	9	71	187	254
09:30	09:45	4	6	11	21	12	11	2	25	46	3	76	14	93	7	51	9	67	160	206
09:45	10:00	6	3	8	17	10	14	4	28	45	2	76	10	88	8	47	4	59	147	192
11:30	11:45	10	6	19	35	15	10	2	27	62	2	89	19	110	2	65	3	70	180	242
11:45	12:00	13	12	18	43	6	15	5	26	69	2	82	25	109	7	65	5	77	186	255
12:00	12:15	11	12	10	33	5	15	2	22	55	2	81	20	103	4	77	8	89	192	247
12:15	12:30	15	13	7	35	6	13	2	21	56	1	66	8	75	7	61	1	69	144	200
12:30	12:45	16	10	16	42	8	5	4	17	59	2	73	16	91	7	80	10	97	188	247
12:45	13:00	10	14	12	36	4	9	2	15	51	7	75	22	104	6	72	4	82	186	237
13:00	13:15	13	8	9	30	8	13	3	24	54	6	65	15	86	3	62	6	71	157	211
13:15	13:30	16	5	11	32	8	13	5	26	58	4	65	11	80	7	55	4	66	146	204
15:00	15:15	16	37	11	64	3	19	2	24	88	0	81	17	98	6	66	12	84	182	270
15:15	15:30	12	44	4	60	4	11	1	16	76	1	102	17	120	5	81	15	101	221	297
15:30	15:45	30	50	15	95	5	13	2	20	115	3	99	25	127	8	94	14	116	243	358
15:45	16:00	14	54	15	83	8	11	1	20	103	2	109	12	123	3	123	35	161	284	387
16:00	16:15	24	69	13	106	9	13	2	24	130	3	122	21	146	10	127	25	162	308	438
16:15	16:30	20	50	16	86	7	13	3	23	109	6	101	25	132	11	151	36	198	330	439
16:30	16:45	23	44	14	81	10	17	2	29	110	3	113	23	139	19	130	15	164	303	413
16:45	17:00	24	49	16	89	10	17	3	30	119	0	113	18	131	17	167	22	206	337	456
17:00	17:15	21	42	19	82	14	21	3	38	120	1	121	22	144	17	173	17	207	351	471
17:15	17:30	22	41	13	76	6	10	0	16	92	0	95	21	116	12	163	18	193	309	401
17:30	17:45	15	29	21	65	5	8	1	14	79	1	103	16	120	13	152	11	176	296	375
17:45	18:00	13	18	17	48	5	8	1	14	62	3	107	11	121	14	133	10	157	278	340
TOTAL	.: 4	462	714	493	1671	383	597	70	1050	2721	85	3313	542	3940	286	2822	34	3 <b>34</b> !	51 7391	10112

Note: U-Turns are included in Totals.

Comment:



Work Order 1292

**Turning Movement Count - Pedestrian Volume Report** 

#### BAYVIEW RD @ SCOTT ST/ALBERT ST

<b>Count Date</b>	: Tuesday, Ap	ril 01, 2014				Start Time:	07:00
Time Period (	NB Approach E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	4	5	2	0	2	7
07:15 07:30	1	8	9	2	4	6	15
07:30 07:45	2	14	16	8	0	8	24
07:45 08:00	9	12	21	4	5	9	30
07:00 08:00	13	38	51	16	9	25	76
08:00 08:15	7	8	15	3	5	8	23
08:15 08:30	5	12	17	4	4	8	25
08:30 08:45	9	11	20	0	9	9	29
08:45 09:00	6	5	11	1	3	4	15
08:00 09:00	27	36	63	8	21	29	92
09:00 09:15	5	6	11	4	4	8	19
09:15 09:30	3	5	8	0	3	3	11
09:30 09:45	3	2	5	1	4	5	10
09:45 10:00	4	1	5	1	1	2	7
09:00 10:00	15	14	29	6	12	18	47
11:30 11:45	1	6	7	2	1	3	10
11:45 12:00	0	5	5	4	1	5	10
12:00 12:15	3	10	13	2	5	7	20
12:15 12:30	8	6	14	13	2	15	29
11:30 12:30	12	27	39	21	9	30	69
12:30 12:45	7	5	12	3	5	8	20
12:45 13:00	1	2	3	5	4	9	12
13:00 13:15	2	2	4	4	2	6	10
13:15 13:30	4	7	11	2	1	3	14
12:30 13:30	14	16	30	14	12	26	56
15:00 15:15	7	2	9	2	3	5	14
15:15 15:30	2	6	8	1	2	3	11
15:30 15:45	2	5	7	4	3	7	14
15:45 16:00	6	9	15	0	4	4	19
15:00 16:00	17	22	39	7	12	19	58
16:00 16:15	2	16	18	6	0	6	24
16:15 16:30	7	14	21	9	4	13	34
16:30 16:45	8	12	20	5	4	9	29
16:45 17:00	6	13	19	4	7	11	30
16:00 17:00	23	55	78	24	15	39	117
17:00 17:15	7	14	21	5	7	12	33
17:15 17:30	5	16	21	8	3	11	32
17:30 17:45	12	20	32	5	8	13	45
17:45 18:00	7	9	16	9	8	17	33
17:00 18:00	31	59	90	27	26	53	143
Total	152	267	419	123	116	239	658

Comment:



# **Turning Movement Count - Cyclist Volume Report**

Work Order

1292

#### **BAYVIEW RD @ SCOTT ST/ALBERT ST**

Count Date: Tuesday, April 01, 2014 Start Time: 07:00

**BAYVIEW RD** 

#### **SCOTT ST/ALBERT ST**

<del>_</del>							
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	0	0	6	1	7	7
08:00 09:00	1	1	2	5	1	6	8
09:00 10:00	0	0	0	5	0	5	5
11:30 12:30	1	0	1	0	0	0	1
12:30 13:30	0	1	1	0	1	1	2
15:00 16:00	2	0	2	2	5	7	9
16:00 17:00	2	1	3	1	9	10	13
17:00 18:00	2	0	2	3	9	12	14
Total	8	3	11	22	26	48	59

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.



W.O.

1292

# **Turning Movement Count - Heavy Vehicle Report**

# **BAYVIEW RD @ SCOTT ST/ALBERT ST**

Survey Date: Tuesday, April 01, 2014

BAYVIEW RD SCOTT ST/ALBERT ST

		Northb	ound			Southb	ound	_			Eastb	ound		1	Nestbo	ound				
Time P	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	1	1	1	3	2	1	0	3	6	1	12	3	16	1	14	5	20	36	42
08:00	09:00	1	0	1	2	1	2	1	4	6	2	15	4	21	3	17	1	21	42	48
09:00	10:00	2	0	2	4	6	0	1	7	11	1	18	2	21	1	12	4	17	38	49
11:30	12:30	3	2	4	9	2	1	0	3	12	1	10	1	12	1	17	0	18	30	42
12:30	13:30	1	2	2	5	3	2	2	7	12	2	15	2	19	1	16	4	21	40	52
15:00	16:00	2	0	0	2	0	1	0	1	3	0	16	2	18	1	14	2	17	35	38
16:00	17:00	0	0	2	2	0	0	0	0	2	1	10	0	11	0	4	0	4	15	17
17:00	18:00	0	0	0	0	0	1	0	1	1	0	6	1	7	3	9	0	12	19	20
Sub T	otal	10	5	12	27	14	8	4	26	53	8	102	15	125	11	103	16	130	255	308
U-Turns	s (Heav	y Veh	icles)		0				0	0				0				0	0	0
Tota	al	10	5	12	0	14	8	4	26	53	8	102	15	125	11	103	16	130	255	308

Heavy Vehicles are vehicles having one rear axle with four or more wheels, or having two or more rear axles. These vehicles include most O.C. Transpo, school and inter-city buses. Further, they ARE included in the Turning Movement Count Summary.

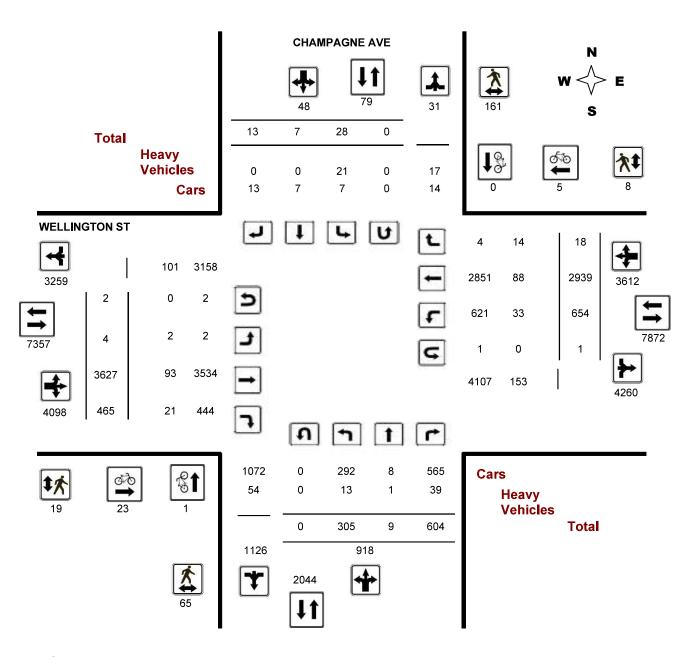


# **Turning Movement Count - Full Study Diagram**

#### **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014 WO#: 29660

**Device:** Miovision



Comments



### **Turning Movement Count - Peak Hour Diagram**

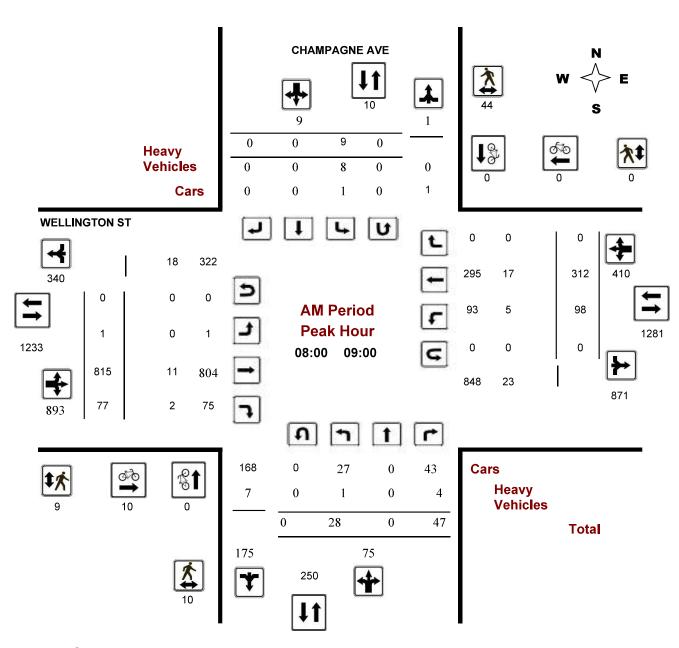
#### **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

Start Time: 07:00

WO No: 29660

Device: Miovision



**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

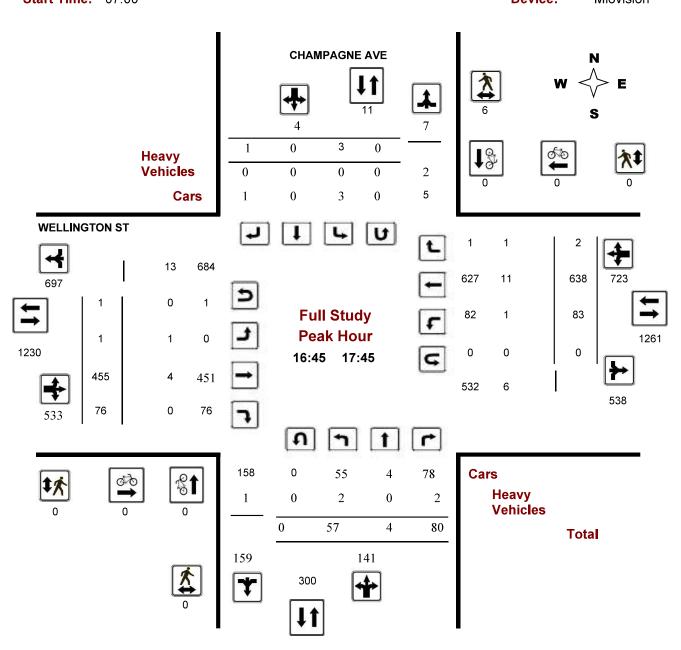
#### **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

Start Time: 07:00

WO No: 29660

Device: Miovision



**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

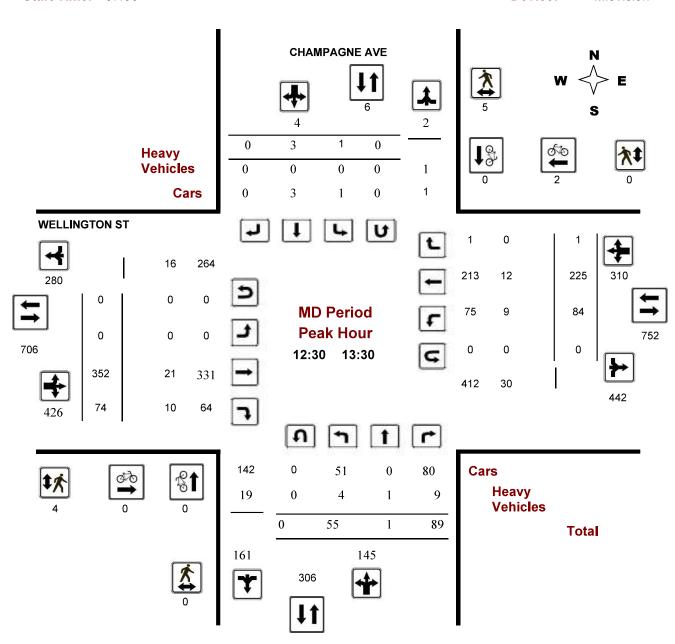
#### **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

Start Time: 07:00

WO No: 29660

Device: Miovision



**Comments** 



### **Turning Movement Count - Peak Hour Diagram**

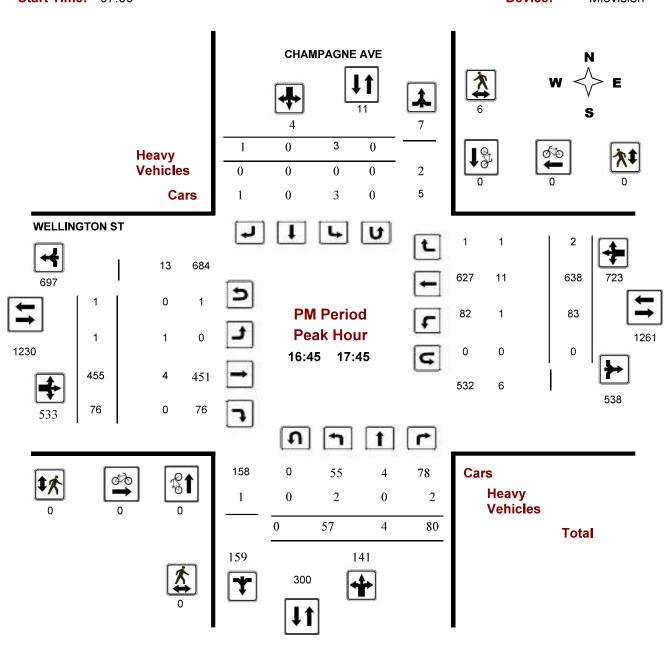
#### **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

Start Time: 07:00

WO No: 29660

Device: Miovision



**Comments** 



**Work Order** 29660

# **Turning Movement Count - Full Study Summary Report**

# **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

#### **Total Observed U-Turns**

**AADT Factor** 

Northbound: Eastbound: 2 Southbound: 0 Westbound: 1 .90

Full Study

								F	ull Stu	ıdy									
			CHA	MPAG	NE AV	/E						WE	LLING	NOT	ST				
_	N	lorthb	ound		S	outhbo	ound				Eastbo	ound			Westbo	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT_	Grand Total
07:00 08:00	7	0	33	40	8	0	0	8	48	0	466	50	516	68	253	3	324	840	888
08:00 09:00	28	0	47	75	9	0	0	9	84	1	815	77	893	98	312	0	410	1303	1387
09:00 10:00	25	0	66	91	4	1	0	5	96	0	423	54	477	100	256	2	358	835	931
11:30 12:30	43	0	111	154	0	0	0	0	154	0	320	53	373	98	240	0	338	711	865
12:30 13:30	55	1	89	145	1	3	0	4	149	0	352	74	426	84	225	1	310	736	885
15:00 16:00	44	1	92	137	1	3	11	15	152	1	452	28	481	66	436	9	511	992	1144
16:00 17:00	57	4	90	151	2	0	1	3	154	1	347	52	400	48	647	2	697	1097	1251
17:00 18:00	46	3	76	125	3	0	1	4	129	1	452	77	530	92	570	1	663	1193	1322
Sub Total	305	9	604	918	28	7	13	48	966	4	3627	465	4096	654	2939	18	3611	7707	8673
U Turns				0				0	0				2				1	3	3
Total	305	9	604	918	28	7	13	48	966	4	3627	465	4098	654	2939	18	3612	7710	8676
EQ 12Hr	424	13	840	1276	39	10	18	67	1343	6	5042	646	5696	909	4085	25	5021	10717	12060
Note: These	va <b>l</b> ues ar	e calcu	lated by	y mu <b>l</b> tiply	ing the	totals by	y the ap	propriat	e expans	ion fac	tor.		,	1.39					
AVG 12Hr	382	11	756	1148	35	9	16	60	1208	5	4537	582	5127	818	3677	23	4519	9646	10854
Note: These	vo <b>l</b> umes	are cal	culated	by multip	lying th	e Equiv	alent 1	2 hr. tota	Is by the	AADT	factor.			.90					
AVG 24Hr	500	15	990	1504	46	11	21	79	1583	7	5944	762	6716	1072	4816	29	5919	12635	14218
Note: These	volumes	are calc	culated	by multip	lying th	e Avera	ige Dail	y 12 hr.	tota <b>l</b> s by	12 to 2	4 expan	sion fac	tor.	1.31					

#### Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



W.O.

29660

# **Turning Movement Count - 15 Minute Summary Report**

# **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

**Total Observed U-Turns** 

#### **CHAMPAGNE AVE**

#### **WELLINGTON ST**

			C	HAII	PAGNI	EAV	E					VV	ELLII	NGIO	N S I					
		No	orthbou	und		So	uthbour	nd			Ea	stbound			We	stbound				
Time Per	riod _	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07	7:15	2	0	8	10	1	0	0	1	11	0	67	10	77	12	50	2	64	141	152
07:15 07	7:30	3	0	4	7	2	0	0	2	9	0	95	13	108	21	55	0	76	184	193
07:30 07	7:45	1	0	9	10	2	0	0	2	12	0	144	14	158	18	75	1	94	252	264
07:45 08	8:00	1	0	12	13	3	0	0	3	16	0	160	13	173	17	73	0	90	263	279
08:00 08	8:15	6	0	12	18	2	0	0	2	20	1	196	14	211	21	78	0	99	310	330
08:15 08	8:30	6	0	5	11	1	0	0	1	12	0	225	20	245	27	72	0	99	344	356
08:30 08	8:45	11	0	16	27	2	0	0	2	29	0	190	22	212	24	66	0	90	302	331
08:45 09	9:00	5	0	14	19	4	0	0	4	23	0	204	21	225	26	96	0	122	347	370
09:00 09	9:15	11	0	19	30	2	0	0	2	32	0	125	21	146	31	77	1	109	255	287
09:15 09	9:30	5	0	17	22	2	0	0	2	24	0	128	14	142	27	73	1	101	243	267
09:30 09	9:45	4	0	17	21	0	0	0	0	21	0	90	6	96	22	57	0	79	175	196
09:45 10	0:00	5	0	13	18	0	1	0	1	19	0	80	13	93	20	49	0	69	162	181
11:30 1	1:45	11	0	22	33	0	0	0	0	33	0	77	7	84	30	70	0	101	185	218
11:45 12	2:00	11	0	32	43	0	0	0	0	43	0	85	20	105	30	51	0	81	186	229
12:00 12	2:15	13	0	34	47	0	0	0	0	47	0	73	10	83	19	60	0	79	162	209
12:15 12	2:30	8	0	23	31	0	0	0	0	31	0	85	16	101	19	59	0	78	179	210
12:30 12	2:45	20	0	21	41	0	3	0	3	44	0	75	14	89	22	59	0	81	170	214
12:45 13	3:00	12	0	21	33	0	0	0	0	33	0	62	32	94	23	60	0	83	177	210
13:00 13	3:15	11	0	23	34	0	0	0	0	34	0	78	15	93	17	42	1	60	153	187
13:15 13	3:30	12	1	24	37	1	0	0	1	38	0	137	13	150	22	64	0	86	236	274
15:00 1	5:15	14	1	27	42	0	3	5	8	50	0	108	5	113	26	94	1	121	234	284
15:15 1	5:30	13	0	16	29	0	0	2	2	31	0	94	9	103	15	94	2	111	214	245
15:30 1	5:45	9	0	29	38	0	0	4	4	42	0	117	7	124	9	117	3	129	253	295
15:45 16	6:00	8	0	20	28	1	0	0	1	29	1	133	7	141	16	131	3	150	291	320
16:00 16	6:15	9	0	22	31	0	0	0	0	31	0	99	13	113	7	135	0	142	255	286
16:15 16	6:30	15	2	23	40	2	0	0	2	42	0	87	14	101	16	161	0	177	278	320
16:30 16	6:45	19	1	26	46	0	0	1	1	47	1	68	11	80	9	167	1	177	257	304
16:45 17	7:00	14	1	19	34	0	0	0	0	34	0	93	14	107	16	184	1	201	308	342
17:00 17	7:15	19	3	28	50	2	0	1	3	53	1	131	24	157	16	159	0	175	332	385
17:15 17	7:30	13	0	19	32	0	0	0	0	32	0	108	18	126	25	152	0	177	303	335
17:30 17		11	0	14	25	1	0	0	1	26	0	123	20	143	26	143	1	170	313	339
17:45 18	8:00	3	0	15	18	0	0	0	0	18	0	90	15	105	25	116	0	141	246	264
TOTAL:	3	05	9	604	918	28	7	13	48	966	4	3627	465	4098	654	2939	18	361	12 7710	8676

Note: U-Turns are included in Totals.

Comment:



Work Order 

#### **Turning Movement Count - Pedestrian Volume Report**

#### **CHAMPAGNE AVE @ WELLINGTON ST** Count Date: Wednesday, April 02, 2014 **Start Time:** 07:00 **NB** Approach SB Approach EB Approach WB Approach Time Period Total **Grand Total Total** (E or W Crossing) (E or W Crossing) (N or S Crossing) (N or S Crossing) 07:00 07:15 07:15 07:30 07:30 07:45 07:45 08:00 07:00 08:00 08:00 08:15 08:15 08:30 08:30 08:45 08:45 09:00 08:00 09:00 09:00 09:15 09:15 09:30 09:30 09:45 09:45 10:00 09:00 10:00 11:30 11:45 11:45 12:00 12:00 12:15 12:15 12:30 11:30 12:30 12:30 12:45 12:45 13:00 13:00 13:15 13:15 13:30 12:30 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 15:00 16:00 16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00 16:00 17:00 17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00

Comment:

17:00 18:00

Total .....

2016-Sep-13 Page 1 of 1



# **Turning Movement Count - Cyclist Volume Report**

Work Order 29660

#### **CHAMPAGNE AVE @ WELLINGTON ST**

Count Date: Wednesday, April 02, 2014

Start Time: 07:00

#### CHAMPAGNE AVE

#### **WELLINGTON ST**

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	0	0	7	1	8	8
08:00 09:00	0	0	0	10	0	10	10
09:00 10:00	1	0	1	5	0	5	6
11:30 12:30	0	0	0	1	0	1	1
12:30 13:30	0	0	0	0	2	2	2
15:00 16:00	0	0	0	0	2	2	2
16:00 17:00	0	0	0	0	0	0	0
17:00 18:00	0	0	0	0	0	0	0
Total	1	0	1	23	5	28	29

Comment:



**W.O.** 29660

# **Turning Movement Count - Heavy Vehicle Report**

# **CHAMPAGNE AVE @ WELLINGTON ST**

Survey Date: Wednesday, April 02, 2014

CHAMPAGNE AVE WELLINGTON ST

		Northb	ound			Southb	ound				Eastb	ound		1	Vestbo	ound				
Time P	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	0	0	6	6	7	0	0	7	13	0	6	1	7	3	11	2	16	23	36
08:00	09:00	1	0	4	5	8	0	0	8	13	0	11	2	13	5	17	0	22	35	48
09:00	10:00	2	0	5	7	4	0	0	4	11	0	21	1	22	4	15	2	21	43	54
11:30	12:30	1	0	9	10	0	0	0	0	10	0	9	4	13	7	7	0	14	27	37
12:30	13:30	4	1	9	14	0	0	0	0	14	0	21	10	31	9	12	0	21	52	66
15:00	16:00	1	0	2	3	1	0	0	1	4	1	14	2	17	4	11	8	23	40	44
16:00	17:00	2	0	2	4	1	0	0	1	5	0	6	1	7	1	4	1	6	13	18
17:00	18:00	2	0	2	4	0	0	0	0	4	1	5	0	6	0	11	1	12	18	22
Sub T	otal	13	1	39	53	21	0	0	21	74	2	93	21	116	33	88	14	135	251	325
U-Turns	(Heav	y Veh	icles)		0				0	0				0				0	0	0
Tota	al	13	1	39	0	21	0	0	21	74	2	93	21	116	33	88	14	135	251	325

Heavy Vehicles are vehicles having one rear axle with four or more wheels, or having two or more rear axles. These vehicles include most O.C. Transpo, school and inter-city buses. Further, they ARE included in the Turning Movement Count Summary.



#### **BOOTH ST and WAR MUSEUM**

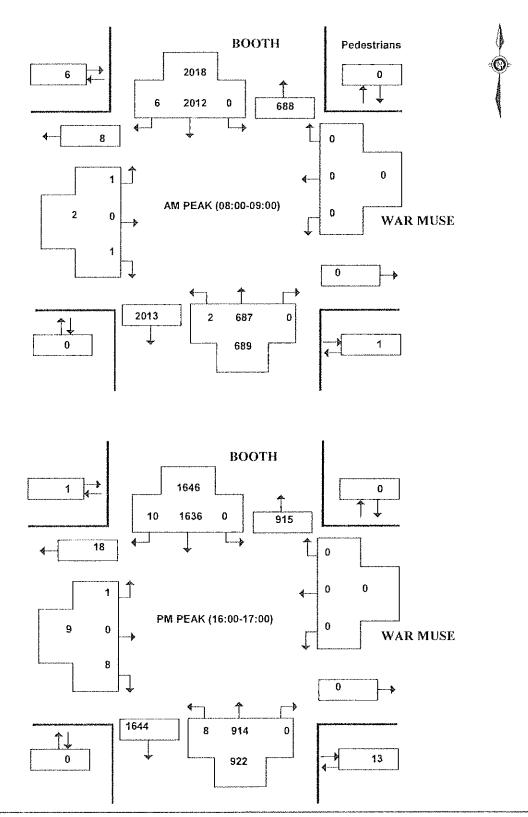
(ULRS Listing BOOTH & WAR MUSE)

Survey Date: Thursday 18 July 2013

Conditions: DRY Start Time: 0700 **Total Observed U-Turns** 

Northbound: 0 Southbound: 0 Eastbound:

0 Westbound: 0 AADT Factor Thursday in July is



Approved by: MO Printed on: 06/08/2013





#### BOOTH ST and WELLINGTON ST /OT. R.

(ULRS Listing BOOTH & WELLINGT)

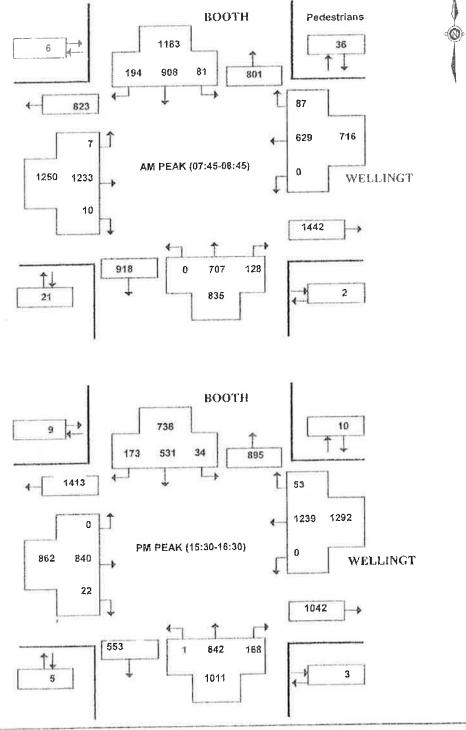
Survey Date: Friday 10 May 2013

dry Conditions: 0700 Start Time:

Total Observed U-Turns

Northbound: Eastbound:

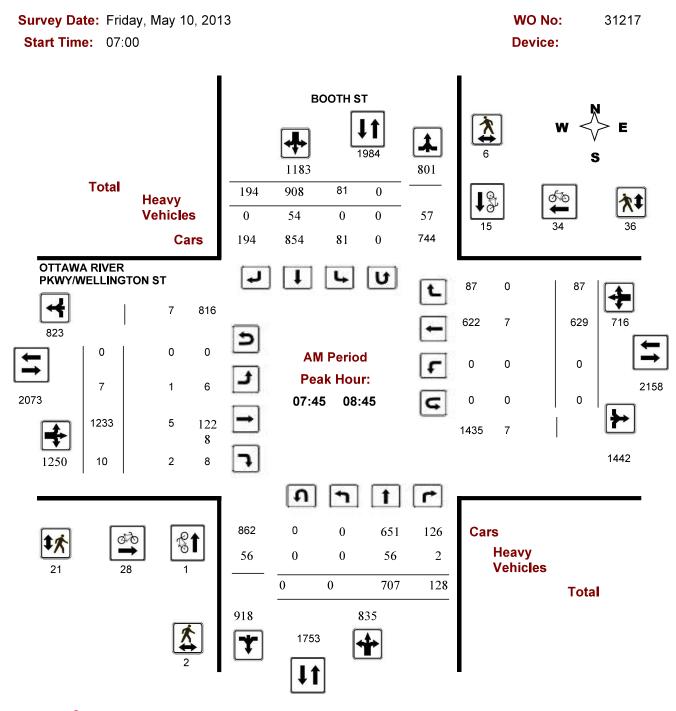
O Southbound: 0 Westbound: 0 AADT Factor Priday in May is





### **Turning Movement Count - Full Study Peak Hour Diagram**

# **BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST**



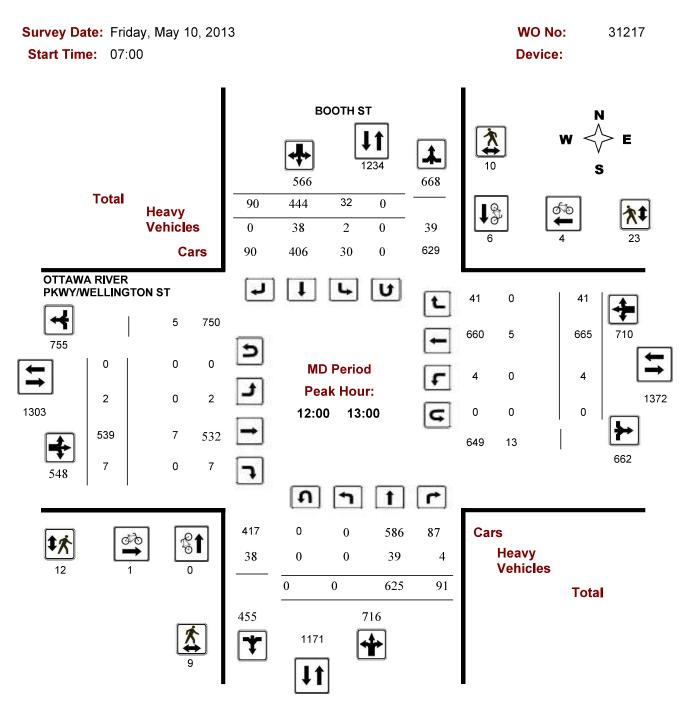
**Comments** 

2016-Jan-29 Page 1 of 3



### **Turning Movement Count - Full Study Peak Hour Diagram**

# **BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST**



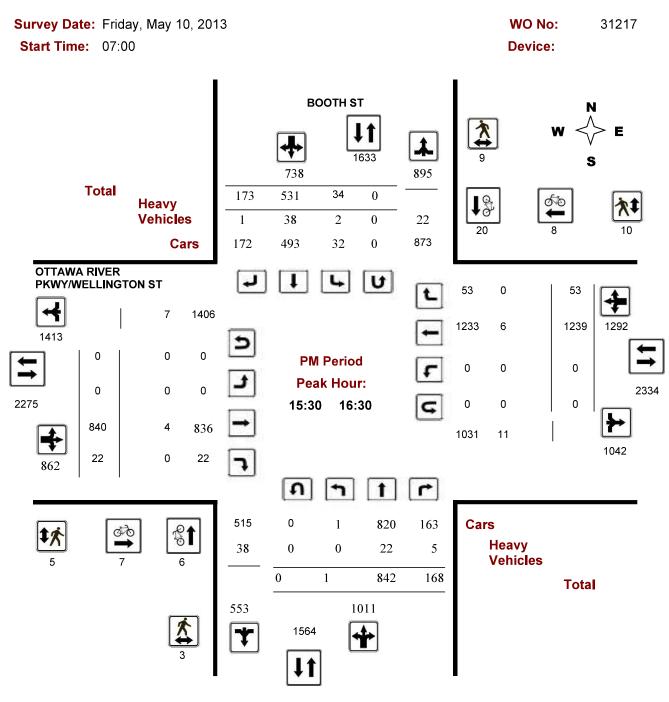
**Comments** 

2016-Jan-29 Page 2 of 3



### **Turning Movement Count - Full Study Peak Hour Diagram**

# **BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST**



**Comments** 

2016-Jan-29 Page 3 of 3

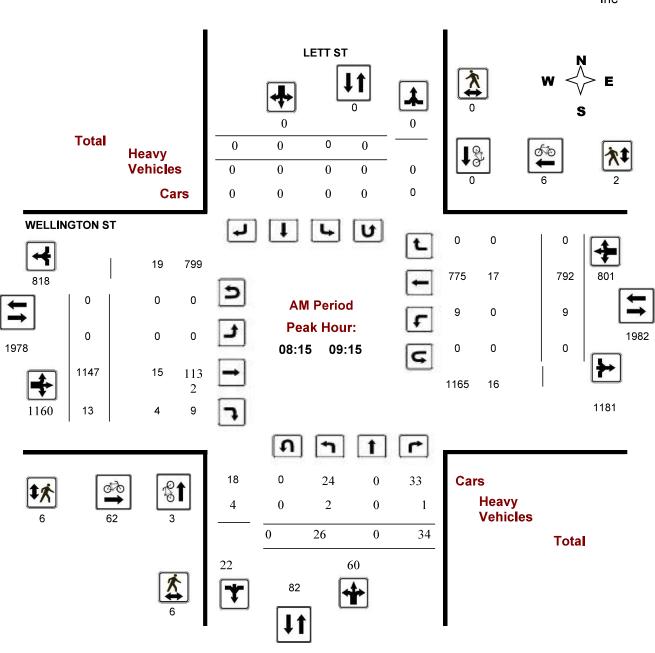


### **Turning Movement Count - Full Study Peak Hour Diagram**

# **WELLINGTON ST @ LETT ST**

Survey Date: Monday, August 17, 2015 WO No: 35251
Start Time: 07:00 Device: Jamar

Technologies, Inc



**Comments** 

2016-Feb-03 Page 1 of 3

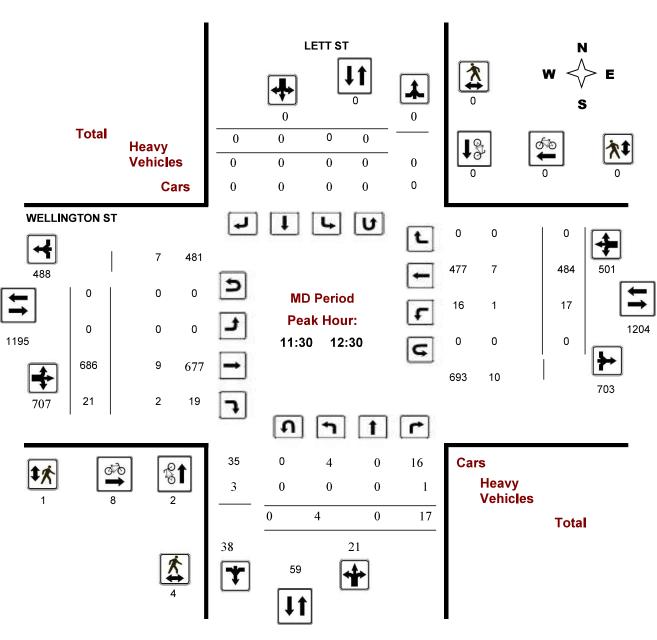


### **Turning Movement Count - Full Study Peak Hour Diagram**

# **WELLINGTON ST @ LETT ST**

Survey Date: Monday, August 17, 2015 WO No: 35251
Start Time: 07:00 Device: Jamar

Technologies, Inc



**Comments** 

2016-Feb-03 Page 2 of 3



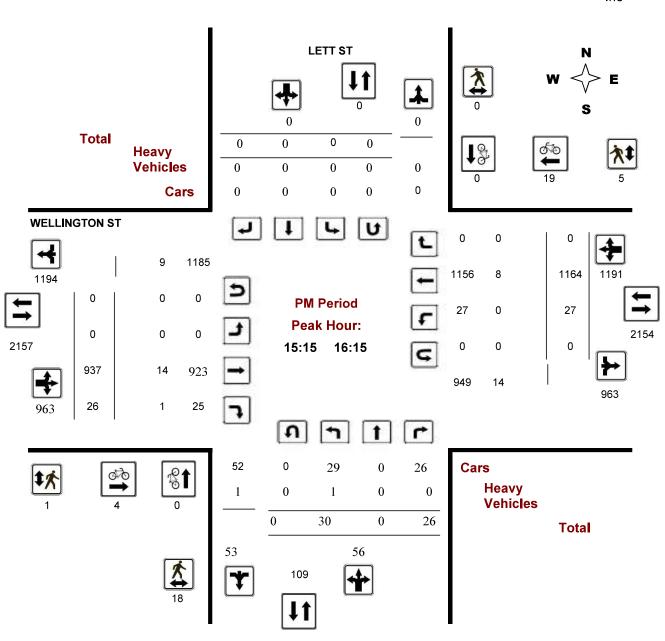
### **Turning Movement Count - Full Study Peak Hour Diagram**

# **WELLINGTON ST @ LETT ST**

Survey Date: Monday, August 17, 2015 WO No: 35251 **Start Time:** 07:00

Jamar Device: Technologies,

Inc



**Comments** 

2016-Feb-03 Page 3 of 3



### **Turning Movement Count - Full Study Peak Hour Diagram**

# OTTAWA RIVER PKWY @ PORTAGE BRIDGE

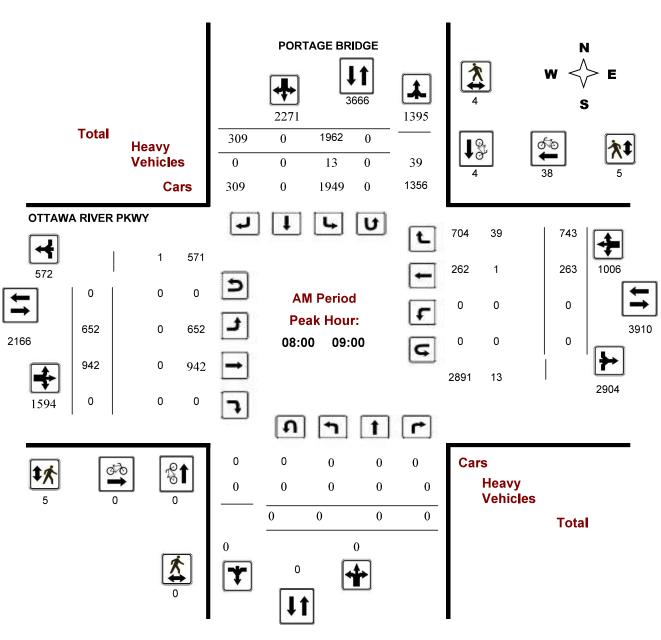
Survey Date: Wednesday, June 11, 2014

**Start Time:** 07:00

WO No: 29831

Device: Jamar

Technologies, Inc



**Comments** 

2016-Feb-03 Page 1 of 3



### **Turning Movement Count - Full Study Peak Hour Diagram**

# OTTAWA RIVER PKWY @ PORTAGE BRIDGE

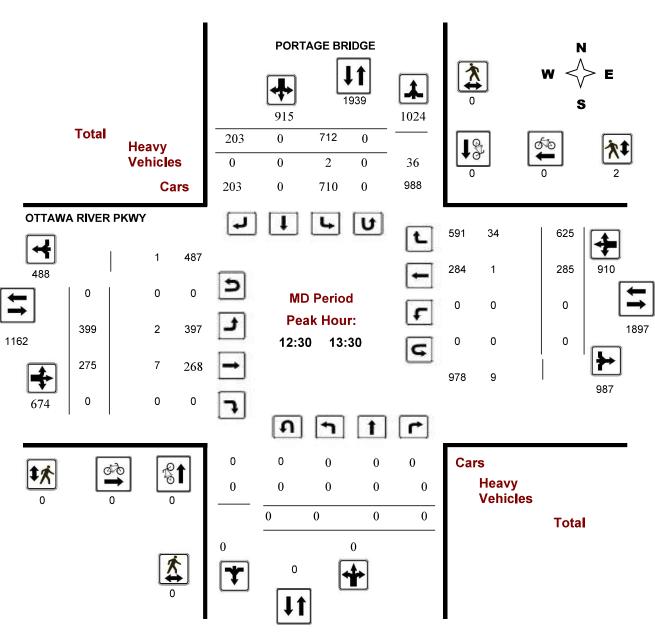
Survey Date: Wednesday, June 11, 2014

Start Time: 07:00

WO No: 29831

Device: Jamar

Technologies, Inc



**Comments** 

2016-Feb-03 Page 2 of 3



### **Turning Movement Count - Full Study Peak Hour Diagram**

# **OTTAWA RIVER PKWY @ PORTAGE BRIDGE**

Survey Date: Wednesday, June 11, 2014

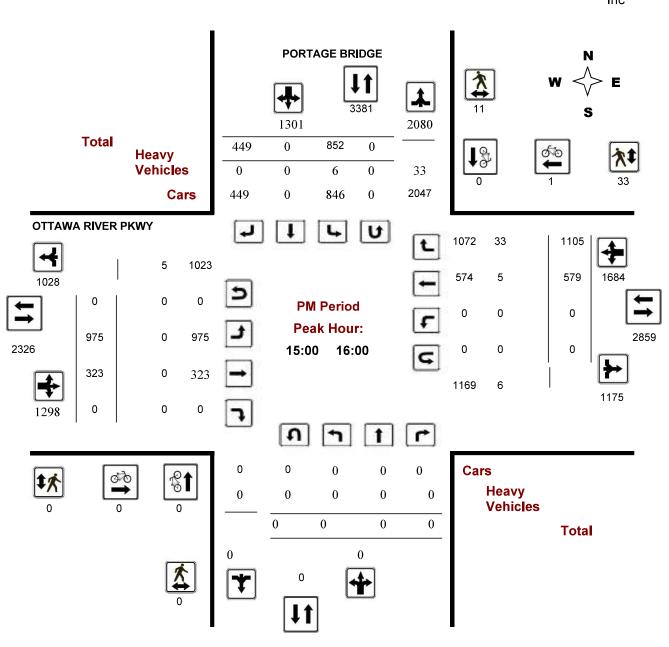
Start Time: 07:00

**WO No:** 29831

Device:

Jamar Technologies,

nnologies Inc



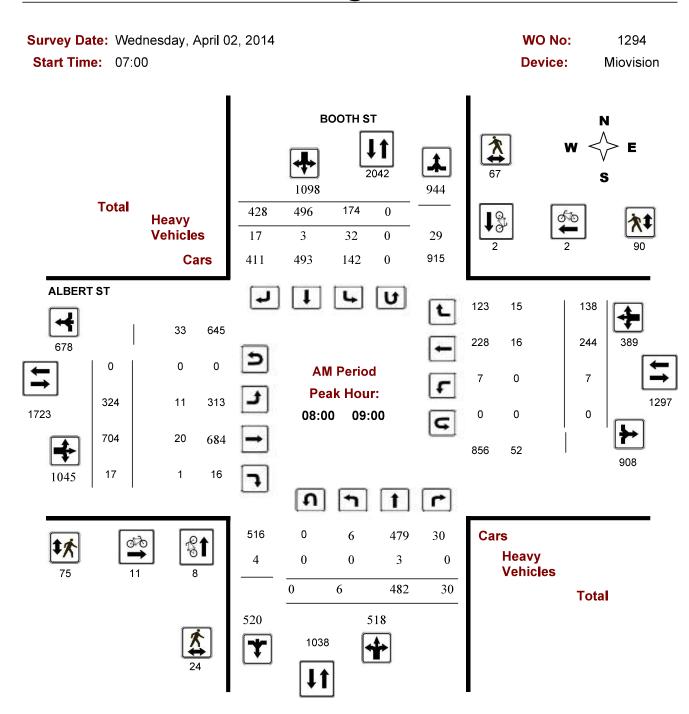
**Comments** 

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### **Turning Movement Count - Full Study Peak Hour Diagram**

# **ALBERT ST @ BOOTH ST**



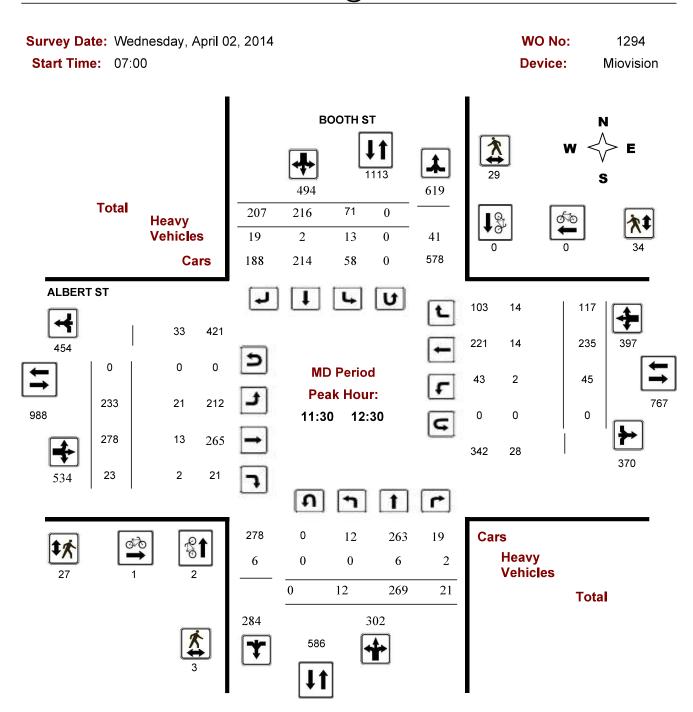
**Comments** 

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### **Turning Movement Count - Full Study Peak Hour Diagram**

# **ALBERT ST @ BOOTH ST**



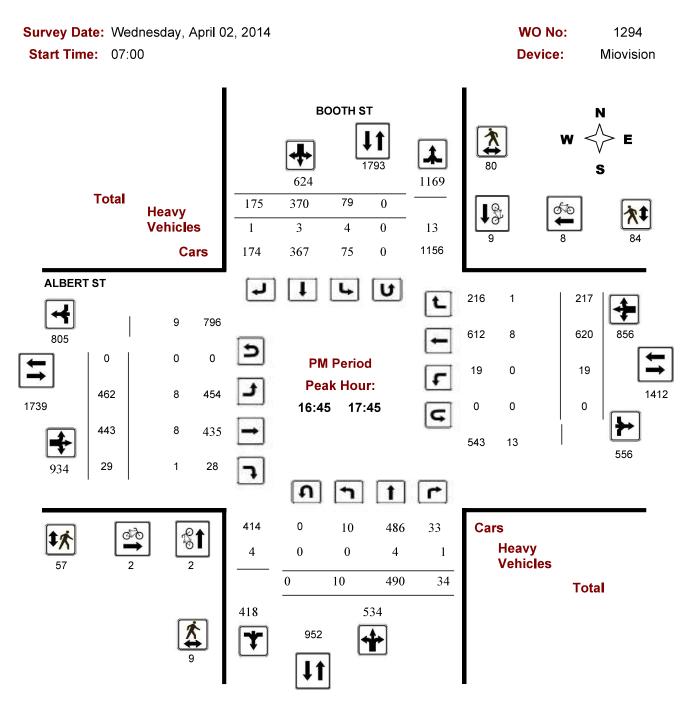
**Comments** 

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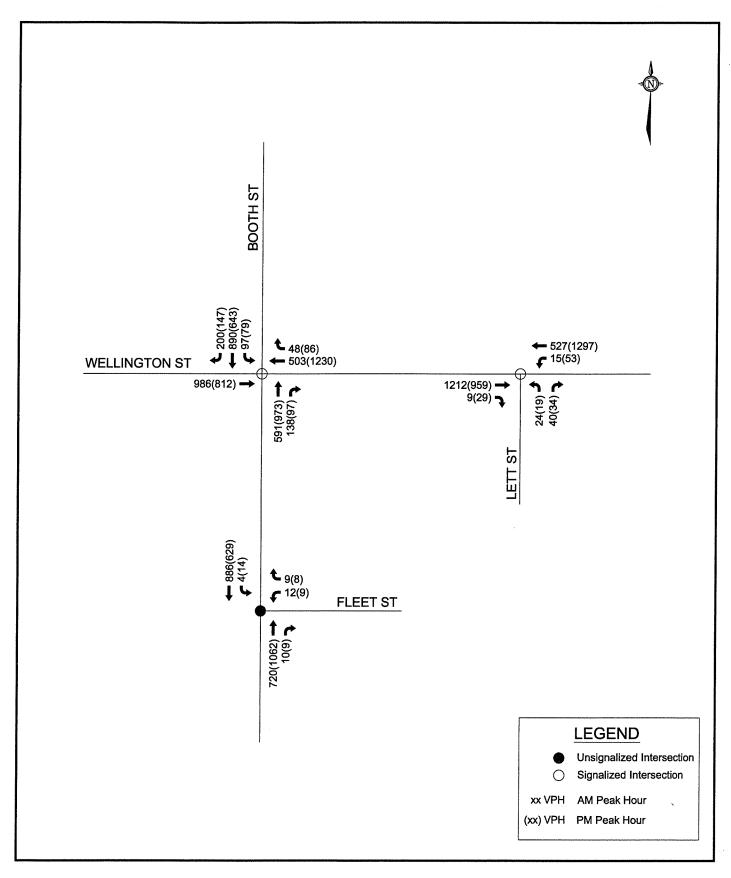
### **Turning Movement Count - Full Study Peak Hour Diagram**

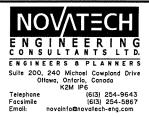
# **ALBERT ST @ BOOTH ST**



**Comments** 

2016-Jan-29 Page 3 of 3





LEBRETON PHASE III, 300 LETT STREET

2013 TOTAL TRAFFIC

105006

APRIL 2011 **F** 

FIGURE 7

#### **Traffic Signal Timing**

City of Ottawa, Transportation Services Department

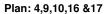
#### **Traffic Signal Operations Unit**

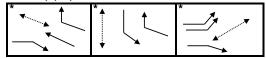
Intersection:	Main:	Wellington	Side:	Portage Bridge
Controller:	MS-320	0	TSD:	5474
Author:	Matthew	Anderson	Date:	06-May-2020

#### Existing Timing Plans<sup>†</sup>

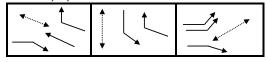
	Plan								Ped Min	imum Ti	me
	Night 4	Evening 9	Weekend AM 10	Morning 16	Weekend 17	AM Peak	Off Peak	PM Peak	Walk	DW	A+R
Cycle	Free	Free	Free	Free	Free	Free	Free	Free			
Offset	Х	Х	Χ	Χ	Χ	Χ	Χ	Χ			
EB Thru	max=26.5	max=27.5	max=36.5	max=27.5	max=36.5	max=27.5	max=36.5	max=36.5	-	-	3.3+3.2
WB Thru	max=26.5	max=27.5	max=36.5	max=27.5	max=36.5	max=27.5	max=36.5	max=36.5	7	13	3.3+3.2
WB Right (fp)	max=26.1	max=31.1	max=41.1	max=51.1	max=31.1	max=51.1	44.1	44.1	-		3.3+2.8
SB Thru	max=26.1	max=31.1	max=41.1	max=58.1	max=31.1	max=51.1	44.1	44.1	26	12	3.3+2.8
EB Left (fp)	max=20.8	max=26.8	max=33.8	max=40.8	max=45.8	42.8	42.8	max=55.8	25	12	3.3+2.5

#### Phasing Sequence<sup>‡</sup>





#### Plan: 28,29,30



#### Notes:

- 1) For all plans except 28,29,30, the EW thru movements have minimum recalls of 10 seconds green. There are no ped recalls
  2) For all plans except 28,29,30 the maximum splits provided will be extended if the pedestrian phases are actuated
- to satisfy the walk and flashing-don't-walk intervals.

#### **Schedule**

#### Weekday

Time	Plan
0:00	4
6:30	16
7:00	28
9:30	29
14:30	30
17:00	9
22:30	4

#### Weekend

Time	Plan
0:15	4
7:00	10
10:00	17
22:00	4

#### Notes

Asterisk (\*) Indicates actuated phase (fp): Fully Protected Left Turn

**←**······►

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

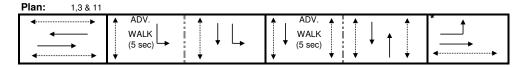
#### **Traffic Signal Operations Unit**

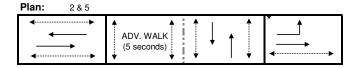
Intersection:	Main:	Albert	Side:	Booth
Controller:	ATC-3		TSD:	5465
Author:	Matthew	Anderson	Date:	09-Aug-2021

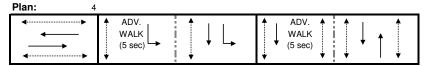
## Existing Timing Plans<sup>†</sup>

	Plan	an Ped Minimum Time									
	AM Peak	Off Peak	PM Peak	Night	Weekend	AM Heavy	Walk	DW	A+R		
	1	2	3	4	5	11					
Cycle	120	85	120	85	85	120					
Offset	104	38	9	3	38	104					
EB Thru	55	50	70	37	50	67	7	23	3.3+3.2		
WB Thru	37	37	41	37	37	36	7	23	3.3+3.2		
SB Left	25	-	12	13	1	15	1	1	3.3+3.2		
NB Thru	40	35	38	35	35	38	7	21	3.3+3.2		
SB Thru	65	35	50	48	35	53	7	21	3.3+3.2		
EB Left	18	13	29	-	13	31	-	-	3.3+3.2		

#### Phasing Sequence<sup>‡</sup>







#### Notes:

- 1) Plans 1, 3, and 11, have an alternative walk time of 10 seconds for the NS thru movements.
- 2) The SB thru movement is prohibited from 11:00pm to 6:00am.
- 3) The SB and WB right turn on red is prohibited on weekdays from 7:00am to 9:00pm.
  4) The WB left turn is prohibited on weekdays from 7:00am to 9:00am, and 3:30pm to 5:30pm with bicycles excepted

#### Schedule

Weekday						
Time	Plan					
0:15	4					
6:00	1					
8:00	11					
9:30	2					
15:00	3					
18:30	2					

Saturda	у
Time	Plan
0:15	4
6:00	2
12:00	5
18:00	2
23:00	4

Sunday	
Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
23:00	4

#### **Notes**

23:00

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

City of Ottawa, Transportation Services Department

#### **Traffic Signal Operations Unit**

Intersection: Main: Albert / Slater Side: Empress

Controller: ATC3 TSD: 5658

Author: Matthew Anderson Date: 09-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

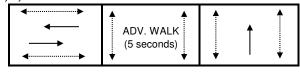
#### Plan

#### **Ped Minimum Time**

	AM Peak	Off Peak	PM Peak	Night 4	Weekend 5	Walk	DW	A+R
Cycle	120	75	120	70	75			
Offset	87	6	87	Х	6			
EB Thru	81	36	81	31	36	7	15	3.3+3.8
WB Thru	81	36	81	31	36	7	15	3.3+3.8
NB Thru	39	39	39	39	39	10	23	3.3+3.0
SB Thru	39	39	39	39	39	10	23	3.3+3.0

## Phasing Sequence<sup>‡</sup>

Plan: 1, 2, 3, 5



## Plan: 4



## **Schedule**

## Weekday

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:30	2
22:30	4

#### Weekend

Time	Plan
0:15	4
8:00	2
22:00	4

#### **Notes**

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

City of Ottawa, Transportation Services Department

## **Traffic Signal Operations Unit**

Intersection: Main: **SJAM** Side: Vimy

**Controller:** ATC 3 TSD: 6570

Author: Matthew Anderson Date: 11-Aug-2021

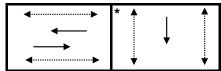
## **Existing Timing Plans**<sup>†</sup>

#### **Ped Minimum Time** Plan

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	95	80	120	80	80			
Offset	59	10	37	64	10			
EB Thru	62	47	87	47	47	-	-	3.7+2.3
WB Thru	62	47	87	47	47	20	11	3.7+2.3
SB Thru	33	33	33	33	33	7	19	3.3+3.0

## Phasing Sequence<sup>‡</sup>

Plan: All



## **Schedule**

## Weekday

Time	Plan
Tillle	гіан
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
22:00	4

## Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

## **Notes**

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase (fp): Fully Protected Left Turn

**◄**······

Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

City of Ottawa, Transportation Services Department

## **Traffic Signal Operations Unit**

Intersection: Main: Wellington Side: Lett

Controller: MS-3200 TSD: 6565

Author: Matthew Anderson Date: 11-Aug-2021

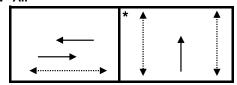
## **Existing Timing Plans**<sup>†</sup>

## Plan Ped Minimum Time

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	95	80	120	70	80			
Offset	60	Χ	27	Χ	Χ			
EB Thru	61	46	86	36	46	15	9	3.7+2.1
WB Thru	61	46	86	36	46	-	-	3.7+2.1
NB Thru	34	34	34	34	34	7	21	3.3+2.6

## Phasing Sequence<sup>‡</sup>

Plan: All



## **Schedule**

## Weekday

,	
Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
22:00	4

## Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

## **Notes**

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

## **Traffic Signal Operations Unit**

Intersection: Main: Albert Side: City Centre

Controller: ATC 3 TSD: 5661

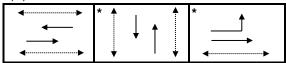
Author: Matthew Anderson Date: 11-Aug-2021

## **Existing Timing Plans<sup>†</sup>**

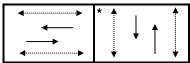
Plan						Ped Min	imum T	ime
	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	120	85	120	65	85			
Offset	80	55	80	Х	55			
EB Thru	84	56	84	35	56	7	14	3.3+3.0
WB Thru	67	45	67	35	45	7	14	3.3+3.0
NB Thru	36	29	36	30	29	7	16	3.3+3.0
SB Thru	36	29	36	30	29	7	16	3.3+3.0
EB Left	17	11	17	-	11	-	-	3.3+3.0

## Phasing Sequence<sup>‡</sup>

Plan: 1, 2, 3 & 5



Plan: 4



## **Schedule**

## Weekday

,	
Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:30	2
22:00	4

#### Weekend

#### **Notes**

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

## **Traffic Signal Operations Unit**

Intersection: Perley Booth Side: Main: TSD: Controller: ATC3 5461 Author: Matthew Anderson Date: 09-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

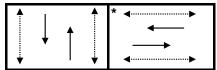
## Plan

#### **Ped Minimum Time**

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	75	80	75	70	80			
Offset	47	Χ	10	Χ	X			
NB Thru	53	58	53	48	58	15	15	3.3+2.6
SB Thru	53	58	53	48	58	15	15	3.3+2.6
EB Thru	22	22	22	22	22	7	9	3.0+3.2
WB Thru	22	22	22	22	22	7	9	3.0+3.2

## Phasing Sequence<sup>‡</sup>





## **Schedule**

## Weekday

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
22:00	4

## Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

#### **Notes**

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

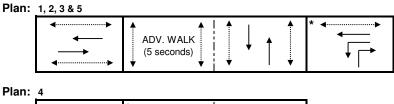
#### **Traffic Signal Operations Unit**

Intersection:	Main:	Albert	Side:	Preston
Controller:	Ms 3200	1	TSD:	5009
Author:	Matthew Anderson		Date:	11-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

Plan							<b>Ped Min</b>	imum T	ime
		AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
		1	2	3	4	5			
	Cycle	120	80	120	70	80			
	Offset	55	75	65	Х	75			
	EB Thru	65	35	61	41	35	7	18	3.3+3.5
	WB Thru	90	48	77	41	48	7	18	3.3+3.5
	NB Thru	30	32	43	29	32	7	16	3.3+3.0
	NB Thru	30	32	43	29	32	7	16	3.3+3.0
	WB Left	25	13	16	-	13	-	-	3.3+2.9
	NB Right	25	13	16	-	13	-	-	3.3+2.9

## Phasing Sequence<sup>‡</sup>



ADV. WALK (5 seconds)

Notes: 1) The NB right turn is prohibited on red, weekdays between 700-1900

## Schedule

Weekda	ay
Time	

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:30	2
22:00	4

#### Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

#### Notes

Asterisk (\*) Indicates actuated phase (fp): Fully Protected Left Turn

Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

#### **Traffic Signal Operations Unit**

Intersection: Main: Albert / Scott Side: Bayview Station

Controller: MS 3200 TSD: 5613

Author: Matthew Anderson Date: 11-Aug-2021

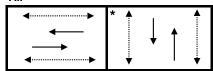
## **Existing Timing Plans**<sup>†</sup>

## Plan Ped Minimum Time

	Early AM	Off Peak	PM Peak	Night	Weekend	AM Peak	Walk	DW	A+R
	1	2	3	4	5	21			
Cycle	95	65	100	70	65	100			
Offset	40	54	65	Х	54	40			
EB Thru	63	33	68	38	33	68	7	19	3.3+3.2
WB Thru	63	33	68	38	33	68	7	19	3.3+3.2
NB Thru	32	32	32	32	32	32	7	19	3.3+3.1
SB Thru	32	32	32	32	32	32	7	19	3.3+3.1

## Phasing Sequence<sup>‡</sup>

Plan: All



## **Schedule**

## Weekday

Time	Plan
0:15	4
6:30	1
7:45	21
9:30	2
15:00	3
18:30	2
22:30	4

#### Saturday

Time	Plan				
0:15	4				
6:30	2				
9:00	5				
18:30	2				
22:30	4				

#### Sunday

,	
Time	Plan
0:15	4
6:30	2
9:00	5
18:00	2
22:30	4

### **Notes**

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

← Pedestri

Pedestrian signal

City of Ottawa, Transportation Services Department

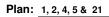
#### **Traffic Signal Operations Unit**

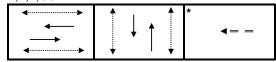
Intersection:	Main: Scott	Side: Parkdale
Controller:	ATC 3	TSD: 5310
Author:	Matthew Anderson	<b>Date:</b> 11-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

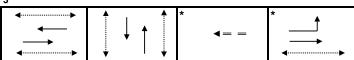
	Plan						Ped Min	imum T	ime
	AM Peak	Off Peak	PM Peak	Night	Weekend	AM Heavy	Walk	DW	A+R
	1	2	3	4	5	21			
Cycle	95	75	100	70	75	100			
Offset	91	33	8	61	33	91			
EB Thru	46	34	49	29	34	51	7	15	3.3+2.8
WB Thru	46	34	34	29	34	51	7	15	3.3+2.8
NB Thru	43	35	45	35	35	43	10	19	3.0+3.3
SB Thru	43	35	45	35	35	43	10	19	3.0+3.3
WB Bus	6	6	6	6	6	6	-	-	0.0+2.0
EB Left	-	-	15	-	-	-	-	-	3.3+2.3

## Phasing Sequence<sup>‡</sup>









## Schedule

## Weekday

Time	Plan
0:15	4
6:30	1
7:45	21
9:30	2
15:00	3
18:30	2
22:30	4

### Saturday

Time	Plan		
0:15	4		
6:30	2		
9:00	5		
18:30	2		
22:30	4		

### Sunday

Time	Plan
0:15	4
6:30	2
9:00	5
18:00	2
22:30	4

#### Notes

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

Transit signal

Cost is \$59.96 (\$53.06 + HST)

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

#### **Traffic Signal Operations Unit**

 Intersection:
 Main:
 SJAM
 side:
 Slidell

 Controller:
 ATC 3
 TSD:
 5890

 Author:
 Matthew Anderson
 Date:
 11-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

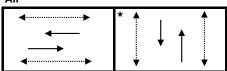
#### Plan

## **Ped Minimum Time**

	AM Peak	Off Peak	PM Peak	Night	Walk	DW	A+R
	1	2	3	4			
Cycle	95	Free	95	Free			
Offset	0	Х	0	X			
EB Thru	61	max = 53.5	61	max = 54.5	15	10	3.7+1.8
ED IIIIU	01	IIIax = 55.5	01	111ax = 54.5	15	10	3.7+1.0
WB Thru	61	max = 53.5	61	max = 54.5	15	10	3.7+1.8
NB Thru	34	max = 31.3	34	max = 31.3	7	20	3.3+3.0
SB Thru	34	max = 31.3	34	max = 31.3	7	20	3.3+3.0

## Phasing Sequence<sup>‡</sup>

Plan: All



Notes: 1) Plans 2 & 4, have a max and ped recall on the EW movements

2) Plans 1 & 3, have a ped recall on the EW movements

## **Schedule**

## Weekday

Time	Plan
0:15	4
6:30	1
9:00	2
15:00	3
18:30	2
21:00	4

## Weekend

Time	Plan
0:10	4
7:00	2
19:00	4

#### Notes

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

#### **Traffic Signal Operations Unit**

Intersection: SJAM / Wellington Booth Side: Main: Controller: MS 3200 TSD: 6567

Author: Matthew Anderson Date: 09-Aug-2021

## Existing Timing Plans<sup>†</sup>

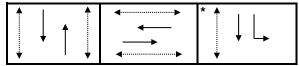
#### Plan

#### **Ped Minimum Time**

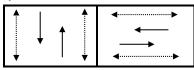
	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	95	85	120	75	85			
Offset	31	58	3	23	58			
NB Thru	35	35	48	35	35	10	15	3.3+3.5
SB Thru	35	35	48	35	35	10	15	3.3+3.6
EB Thru	48	38	60	40	38	10	19	3.7+3.1
WB Thru	48	38	60	40	38	10	19	3.7+3.1
SB Left	12	12	12	-	12	-	-	3.3+3.5

## Phasing Sequence<sup>‡</sup>

Plan: 1,2,3,5



Plan: 4



## **Schedule**

## Weekday

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
23:45	4

## Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

#### **Notes**

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

City of Ottawa, Transportation Services Department

## **Traffic Signal Operations Unit**

Intersection: Main: Booth Side: War museum

Controller: MS 3200 TSD: 6564

Author: Matthew Anderson Date: 09-Aug-2021

## **Existing Timing Plans**<sup>†</sup>

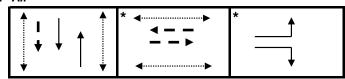
## Plan

## **Ped Minimum Time**

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	95	85	95	85	95			
Offset	47	X	10	X	Х			
NB Thru	49	39	49	39	49	20	5	3.3+2.6
SB Thru	49	39	49	39	49	20	5	3.3+2.6
EW Bike	29	29	29	29	29	7	15	3.0+4.0
EB Exit (fp)	17	17	17	17	17	-	-	3.3+2.9

## Phasing Sequence<sup>‡</sup>

Plan: All



## **Schedule**

## Weekday

Time	Plan
0:15	4
6:00	1
9:30	2
15:00	3
18:00	2
22:00	4

## Weekend

Time	Plan
0:15	4
8:00	2
12:00	5
18:00	2
22:00	4

#### notes

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn



Cost is \$58.78 (\$52.02 + HST)

<sup>†:</sup> Time for each direction includes amber and all red intervals

<sup>‡:</sup> Start of first phase should be used as reference point for offset

# **APPENDIX C: City of Ottawa - Collision Data**



STUDY AREA YE	EAR DATE TIME COLLISION II	D. LOCATION			LONGI	TUDE LATITUDE ENVIR	DONMENT	LIGHT	SURFACE CONDITION	TRAFFIC CONTROL	TRAFFIC CONTROL CONDITION	COLLISION CLASSIFICATION	IMPACT TYPE	NO OF PEDESTRIANS FID
Y Y	2015 2015/07/05 04:00:00+00 1899/12/31 22:34:00+00 15-7406	ALBERT ST @ BRONSON AVE	36	6739.7188 503	31015 -75.70	0859528 45.41632843 01 - C	llear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	02 - Angle	0 7191
Υ	2015 2015/12/14 05:00:00+00 1900/01/01 01:55:00+00 15-12926	ALBERT ST @ BRONSON AVE				0861053 45.41635132 02 - R			02 - Wet	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	02 - Angle	0 12464
Y	2015 2015/12/08 05:00:00+00 1899/12/31 20:16:00+00 15-12682 2015 2015/01/05 05:00:00+00 1899/12/31 13:33:00+00 15-134	ALBERT ST @ COMMISSIONER ST ALBERT ST @ EMPRESS AVE	36	6713.6875 503	30970 -75.7	7089386 45.41592789 02 - R 1071625 45.41370773 06 - S	Rain	01 - Daylight 01 - Daylight	02 - Wet	02 - Stop sign 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 02 - Non-fatal injury	02 - Angle 03 - Rear end	0 12980 0 234
Y	2015 2015/01/16 05:00:00+00 1899/12/31 19:29:00+00 15:853	ALBERT ST @ EMPRESS AVE	36	6576.6563 5030	720.5 -75.71	1071625 45.41369629 01 - C	Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	99 - Other	0 653
Υ	2015 2015/02/20 05:00:00+00 1899/12/31 11:16:00+00 15-2852	ALBERT ST @ EMPRESS AVE				1075439 45.41368866 01 - C		07 - Dark	02 - Wet	01 - Traffic signal	01 - Functioning	03 - P.D. only	07 - SMV other	0 2656
Y	2015 2015/01/29 05:00:00+00 1899/12/31 13:55:00+00 15:1510 2015 2015/02/04 05:00:00+00 1899/12/31 15:00:00+00 15:1958	ALBERT ST @ PRESTON ST ALBERT ST @ PRESTON ST				1607208 45.41143799 01 - C		01 - Daylight 01 - Daylight	06 - Ice	01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 02 - Non-fatal injury	03 - Rear end 03 - Rear end	0 1329 0 1693
Y	2015 2015/02/04 05:00:00+00 1899/12/31 15:00:00+00 15-1958 2015 2015/02/17 05:00:00+00 1899/12/31 13:52:00+00 15-2700	ALBERT ST @ PRESTON ST				1608734 45.41144562 03 - S 1607208 45.41143799 01 - C		01 - Daylight 01 - Daylight			01 - Functioning 01 - Functioning	02 - Non-ratal injury 03 - P.D. only	03 - Rear end	0 2500
Υ	2015 2015/03/03 05:00:00+00 1899/12/31 21:15:00+00 15-3451	ALBERT ST @ PRESTON ST	36	6158.6875 503	30465 -75.71	1608734 45.41143036 03 - S	inow		05 - Packed snow		01 - Functioning	03 - P.D. only	03 - Rear end	0 3949
Y	2015 2015/07/19 04:00:00+00 1899/12/31 22:55:00+00 15-7872 2015 2015/07/10 04:00:00+00 1899/12/31 23:30:00+00 15-7596	ALBERT ST @ PRESTON ST ALBERT ST @ PRESTON ST				1607208 45.41143036 02 - R 1605682 45.41141129 01 - C		01 - Daylight	02 - Wet	01 - Traffic signal	01 - Functioning 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	02 - Angle 03 - Rear end	0 7320 0 7443
Y	2015 2015/07/10 04:00:00+00 1899/12/31 23:30:00+00 15-7596 2015 2015/10/29 04:00:00+00 1899/12/31 21:25:00+00 15-11234	ALBERT ST @ PRESTON ST				1605682 45.41141129 01 - C		01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Kear end 05 - Turning movement	0 7443
Y	2015 2015/12/21 05:00:00+00 1899/12/31 12:20:00+00 15-13146	ALBERT ST @ PRESTON ST	36	6160.6875 503	30464 -75.71	1606445 45.41141891 01 - C	llear	03 - Dawn	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	01 - Approaching	0 13292
Υ	2015 2015/05/27 04:00:00+00 1899/12/31 21:30:00+00 15-13635	ALBERT ST @ PRESTON ST				1605682 45.41141129 00 - U		01 - Daylight	00 - Unknown	01 - Traffic signal	00 - Unknown	03 - P.D. only	04 - Sideswipe	0 13643
Y	2015 2015/07/09 04:00:00+00 1899/12/31 19:32:00+00 15-7540 2015 2015/07/05 04:00:00+00 1899/12/31 05:44:00+00 15-7394	ALBERT ST btwn Continuation of ALBERT ST & BOOTH ST ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST	36			1372986 45.41239166 01 - C 7144165 45.41214371 01 - C		01 - Daylight 07 - Dark	01 - Dry 01 - Dry	10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	04 - Sideswipe 07 - SMV other	0 7384 0 7100
Ý	2015 2015/08/23 04:00:00+00 1900/01/01 02:16:00+00 15-8954	ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST		66179.375 503	30476 -75.71	1582794 45.41152954 01 - C	llear	07 - Dark	01 - Dry	10 - No control		02 - Non-fatal injury	05 - Turning movement	0 8691
Υ	2015 2015/10/20 04:00:00+00 1899/12/31 21:56:00+00 15-10904	ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST	36	6178.4063 503	30476 -75.71	1583557 45.41152573 01 - C	Clear	01 - Daylight		10 - No control		03 - P.D. only	04 - Sideswipe	0 10122
Y	2015 2015/12/29 05:00:00+00 1899/12/31 19:54:00+00 15-13391 2015 2015/01/07 05:00:00+00 1899/12/31 22:20:00+00 15-305	ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (2) BAYVIEW RD @ SCOTT ST/ALBERT ST				1988678 45.41001129 03 - S 2431183 45.40794373 01 - C		01 - Daylight 05 - Dusk	03 - Loose snow 05 - Packed snow	10 - No control 01 - Traffic signal	01 - Functioning	03 - P.D. only 03 - P.D. only	07 - SMV other 05 - Turning movement	0 13989 0 407
Y	2015 2015/02/13 05:00:00+00 1899/12/31 14:25:00+00 15-303	BAYVIEW RD @ SCOTT ST/ALBERT ST	30			2429657 45.40794373 01 - C		01 - Daylight			01 - Functioning	03 - P.D. only	04 - Sideswipe	0 2575
Υ	2015 2015/03/15 04:00:00+00 1899/12/31 21:49:00+00 15-3848	BAYVIEW RD @ SCOTT ST/ALBERT ST		5518.0625 503	30071 -75.72	2431946 45.40794373 01 - C	llear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 3757
Y	2015 2015/07/02 04:00:00+00 1900/01/01 00:46:00+00 15-7334 2015 2015/06/29 04:00:00+00 1899/12/31 15:53:00+00 15-7184	BAYVIEW RD @ SCOTT ST/ALBERT ST BAYVIEW RD @ SCOTT ST/ALBERT ST				2428894 45.40793228 01 - C 7243042 45.4079361 01 - C		01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	02 - Non-fatal injury 02 - Non-fatal injury	07 - SMV other 02 - Angle	1 7040 0 7618
Y	2015 2015/08/28 04:00:00+00 1899/12/31 15:53:00+00 15:7184 2015 2015/08/28 04:00:00+00 1899/12/31 12:30:00+00 15:9120	BAYVIEW RD @ SCOTT ST/ALBERT ST				7243042 45.40795135 01 - C		01 - Daylight 01 - Daylight			01 - Functioning 01 - Functioning	03 - P.D. only	05 - Turning movement	0 9681
Y	2015 2015/03/21 04:00:00+00 1899/12/31 22:51:00+00 15-4000	BOOTH ST @ 148 N OF MIDDLE ST/E.B.EDDY S	36	6035.5313 503	31415 -75.71	1754456 45.41999054 06 - S	itrong wind	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 3698
Υ	2015 2015/07/21 04:00:00+00 1899/12/31 22:45:00+00 15-7932	BOOTH ST @ 148 N OF MIDDLE ST/E.B.EDDY S		366036.25 5031	415.5 -75.71	1753693 45.41999435 01 - C	Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	03 - Rear end	0 7463
Y	2015 2015/01/05 05:00:00+00 1899/12/31 19:08:00+00 15:157 2015 2015/01/21 05:00:00+00 1900/01/01 00:19:00+00 15:1129	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				7146759 45.41629028 01 - C 1468353 45.41628265 01 - C	lear lear	01 - Daylight 07 - Dark	02 - Wet	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 07 - SMV other	0 257 0 1450
Ý	2015 2015/01/28 05:00:00+00 1899/12/31 15:07:00+00 15-1437	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	36	6262.9375 5031	.007.5 -75.71	1469116 45.41630554 01 - C	lear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	0 1937
Υ	2015 2015/02/12 05:00:00+00 1899/12/31 15:10:00+00 15-2422	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				7146759 45.41630554 01 - C	Clear	01 - Daylight	04 - Slush	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 2521
Y	2015 2015/02/13 05:00:00+00 1899/12/31 18:13:00+00 15:2490 2015 2015/03/22 04:00:00+00 1899/12/31 13:52:00+00 15:4013	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				1466064 45.41629028 01 - C 7146759 45.41628265 03 - S		01 - Daylight	01 - Dry 03 - Loose snow		01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	0 2589 0 4642
Y	2015 2015/05/28 04:00:00+00 1899/12/31 13:52:00+00 15-4013 2015 2015/05/28 04:00:00+00 1899/12/31 21:14:00+00 15-6074	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				1468353 45.41630936 01 - C		01 - Daylight 01 - Daylight		01 - Traffic signal	01 - Functioning 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 6191
Y	2015 2015/07/20 04:00:00+00 1899/12/31 21:15:00+00 15-7895	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	36	6264.3125 5031	.006.5 -75.7	7146759 45.41629791 01 - C	Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 7343
Y	2015 2015/08/01 04:00:00+00 1899/12/31 06:38:00+00 15-8276	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				1468353 45.41630173 01 - C			01 - Dry		01 - Functioning	03 - P.D. only	05 - Turning movement	0 8085
Y	2015 2015/08/18 04:00:00+00 1899/12/31 13:20:00+00 15-8819 2015 2015/09/03 04:00:00+00 1899/12/31 14:48:00+00 15-9297	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST  BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	30			1468353 45.41627884 01 - C 7146759 45.4162941 01 - C		01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	00 - Unknown 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	03 - Rear end 04 - Sideswipe	0 8917 0 9571
Y	2015 2015/11/05 05:00:00+00 1899/12/31 21:51:00+00 15-11488	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				7146759 45.41628265 01 - 0			01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	0 11202
Υ	2015 2015/11/17 05:00:00+00 1900/01/01 01:00:00+00 15-11922	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST		366264.25 503	31006 -75.7	7146759 45.41629028 01 - 0	lear	07 - Dark	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	05 - Turning movement	0 11685
Y	2015 2015/11/26 05:00:00+00 1900/01/01 04:43:00+00 15-12279 2015 2015/12/01 05:00:00+00 1899/12/31 10:40:00+00 15-12432	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				1469116 45.41629028 01 - C 7146759 45.41628265 01 - C		07 - Dark 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	00 - Unknown 01 - Functioning	03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	0 12414 0 12490
Y	2015 2015/10/14 04:00:00+00 1899/12/31 22:09:00+00 15-13721	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST				1465302 45.41632843 01 - C		01 - Daylight	01 - Dry	01 - Traffic signal	00 - Unknown	02 - Non-fatal injury	03 - Rear end	0 13724
Υ	2015 2015/10/02 04:00:00+00 1899/12/31 13:00:00+00 15-13734	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	36	6263.0625 503	31004 -75.71	1469116 45.41627121 01 - C	Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 13737
Y	2015 2015/02/20 05:00:00+00 1899/12/31 23:15:00+00 15-2913	BOOTH ST btwn 148 N OF MIDDLE ST/E.B. EDDY S & MIDDLE ST				7170105 45.41937256 01 - C			02 - Wet	10 - No control		03 - P.D. only	07 - SMV other	0 2717
Y	2015 2015/08/27 04:00:00+00 1899/12/3114:09:00+00 15:9081 2015 2015/05/12 04:00:00+00 1899/12/3113:19:00+00 15:5527	BOOTH ST btwn 148 N OF MIDDLE ST/E.B. EDDY S & MIDDLE ST BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV				1668243 45.41899109 01 - C 1575928 45.41806412 02 - R		01 - Daylight 01 - Daylight		10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	04 - Sideswipe 07 - SMV other	0 9733 0 5589
Ý	2015 2015/09/17 04:00:00+00 1899/12/31 18:55:00+00 15:9802	BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV				1600342 45.41834259 01 - C		01 - Daylight	01 - Dry	10 - No control		03 - P.D. only	03 - Rear end	0 9668
Υ	2015 2015/12/15 05:00:00+00 1899/12/31 13:40:00+00 15-13769	BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N				1809387 45.4209137 02 - R	Rain	01 - Daylight	02 - Wet	10 - No control		03 - P.D. only	03 - Rear end	0 13771
Y	2015 2015/02/23 05:00:00+00 1900/01/01 04:34:00+00 15-3101 2015 2015/06/24 04:00:00+00 1899/12/31 12:40:00+00 15-7014	BOOTH ST btwn VIMY PLACE PRIV & OTTAWA RIVER PKWY BOOTH ST btwn VIMY PLACE PRIV & OTTAWA RIVER PKWY				1485901 45.4166336101-0 1488953 45.4166717501-0		07 - Dark 01 - Daylight	06 - Ice	10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	0 3499 0 7244
Y	2015 2015/06/24 04:00:00+00 1899/12/31 12:40:00+00 15:7014 2015 2015/09/14 04:00:00+00 1899/12/31 21:26:00+00 15:9672	BOOTH ST DIWN VIMY PLACE PRIV & OTTAWA RIVER PRWY  BOOTH ST DIWN VIMY PLACE PRIV & OTTAWA RIVER PKWY				1485901 45.4166832 01 - C		01 - Daylight 01 - Daylight	01 - Dry	10 - No control		03 - P.D. only	03 - Rear end	0 7244
Y	2015 2015/01/20 05:00:00+00 1899/12/31 15:15:00+00 15-1023	BRONSON AVE @ COMMISSIONER ST/SLATER ST	36	6768.5938 5030	952.5 -75.70	0823669 45.41576385 01 - C	lear	01 - Daylight	01 - Dry		01 - Functioning	02 - Non-fatal injury	02 - Angle	0 1243
Y	2015 2015/03/08 05:00:00+00 1899/12/31 22:47:00+00 15:3607 2015 2015/03/23 04:00:00+00 1899/12/31 14:58:00+00 15:4035	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST				0825195 45.41570663 01 - C 0822144 45.41573334 01 - C		01 - Daylight		01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 02 - Non-fatal injury	04 - Sideswipe	0 3110 0 4765
Y	2015 2015/03/23 04:00:00+00 1899/12/31 14:58:00+00 15-4035 2015 2015/03/23 04:00:00+00 1899/12/31 23:37:00+00 15-4062	BRONSON AVE @ COMMISSIONER ST/SLATER ST  BRONSON AVE @ COMMISSIONER ST/SLATER ST				J822144 45.41573334 01 - C J823669 45.41572571 01 - C		01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	05 - Turning movement 05 - Turning movement	0 4765
Y	2015 2015/06/19 04:00:00+00 1899/12/31 13:05:00+00 15-6828	BRONSON AVE @ COMMISSIONER ST/SLATER ST	36	6768.7188 503	30949 -75.70	0823669 45.41573334 01 - C	llear	01 - Daylight		01 - Traffic signal	00 - Unknown	02 - Non-fatal injury	04 - Sideswipe	0 6027
Υ	2015 2015/06/19 04:00:00+00 1899/12/31 22:47:00+00 15-6867	BRONSON AVE @ COMMISSIONER ST/SLATER ST				0824432 45.41573715 01 - C		01 - Daylight		01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 6066
Y	2015 2015/07/14 04:00:00+00 1900/01/01 03:04:00+00 15-7715 2015 2015/10/28 04:00:00+00 1899/12/31 19:50:00+00 15-11179	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST	36	6769.4375 503	30948 -75.70	0822906 45.41572571 01 - C 0822906 45.41571808 02 - R	llear	07 - Dark 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	04 - Sideswipe 05 - Turning movement	0 7226 0 11538
Y	2015 2015/10/28 04:00:00+00 1899/12/31 19:50:00+00 15-111/9 2015 2015/11/27 05:00:00+00 1899/12/31 11:24:00+00 15-12282	BRONSON AVE @ COMMISSIONER ST/SLATER ST  BRONSON AVE @ COMMISSIONER ST/SLATER ST	36	6769.4375 5030	1947.5 -75.70 1948.5 -75.70	3822906 45.41571808 02 - K 3822906 45.41572952 01 - C	lear	01 - Daylight 03 - Dawn	01 - Dry	01 - Traffic signal	01 - Functioning 01 - Functioning	02 - Non-fatal injury	05 - Turning movement 05 - Turning movement	0 11538
Υ	2015 2015/12/06 05:00:00+00 1899/12/31 23:10:00+00 15-12626	BRONSON AVE @ COMMISSIONER ST/SLATER ST				0822906 45.41573715 01 - C			01 - Dry	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	03 - Rear end	0 12741
Y	2015 2015/02/04 05:00:00+00 1899/12/31 20:16:00+00 15-1998	OTTAWA RIVER PKWY @ PORTAGE BRIDGE				1142578 45.41904831 03 - S		01 - Daylight	03 - Loose snow	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 1733
Y	2015 2015/02/25 05:00:00+00 1899/12/31 19:29:00+00 15-3221 2015 2015/06/07 04:00:00+00 1899/12/31 08:55:00+00 15-13641	OTTAWA RIVER PKWY @ PORTAGE BRIDGE OTTAWA RIVER PKWY @ PORTAGE BRIDGE				1141815 45.41904831 01 - C 1141052 45.41906357 01 - C		01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	04 - Sideswipe 07 - SMV other	0 3219 0 13649
Ý	2015 2015/12/30 05:00:00+00 1900/01/01 03:06:00+00 15-13442	OTTAWA RIVER PKWY @ PORTAGE BRIDGE	36	6516.2188 5031	314.5 -75.71	1141815 45.41904449 03 - S	now	07 - Dark	04 - Slush	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	03 - Rear end	0 13851
Y	2015 2015/01/26 05:00:00+00 1899/12/31 13:52:00+00 15-13516	OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST				2888947 45.41072845 01 - C		01 - Daylight		01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 13018
Y	2015 2015/02/21 05:00:00+00 1899/12/31 18:26:00+00 15-13561 2015 2015/04/28 04:00:00+00 1899/12/31 20:55:00+00 15-13618	OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST	3	65157.625 503 5158.4275 5020	30376 -75.72 1279 5 -75 72	2888947 45.41071701 00 - U 2888184 45.41075134 01 - C	Jnknown	01 - Daylight 01 - Daylight	03 - Loose snow	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 02 - Angle	0 13061 0 13542
Y	2015 2015/10/28 04:00:00+00 1899/12/31 12:00:00+00 15-13156	OTTAWA RIVER PKWY @ VIMY PLACE PRIV	36	5974.8438 5030	797.5 -75.71	1839905 45.41444016 01 - C	lear	03 - Dayiigiit	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	0 11515
Υ	2015 2015/12/11 05:00:00+00 1899/12/31 21:16:00+00 15-12813	OTTAWA RIVER PKWY @ VIMY PLACE PRIV	36	5974.8125 5030	797.5 -75.71	1839905 45.41444016 02 - R	Rain	05 - Dusk	02 - Wet		01 - Functioning	03 - P.D. only	05 - Turning movement	0 12285
Y	2015 2015/01/14 05:00:00+00 1899/12/31 14:22:00+00 15-13507 2015 2015/12/22 05:00:00+00 1899/12/31 21:45:00+00 15-13216	OTTAWA RIVER PKWY @ VIMY PLACE PRIV OTTAWA RIVER PKWY @ VIMY PLACE PRIV				1840668 45.41444016 01 - C 1839142 45.41444397 02 - R	llear lain	01 - Daylight 05 - Dusk	02 - Wet 02 - Wet	01 - Traffic signal 01 - Traffic signal	01 - Functioning 00 - Unknown	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	0 13007 0 13358
Y	2015 2015/01/15 05:00:00+00 1899/12/31 16:15:00+00 15-13503	OTTAWA RIVER PKWY @ VIMY PLACE PRIV	36	5971.9688 5030	801.5 -75.71	1843719 45.4144783 01 - C	lear	01 - Daylight			01 - Functioning	03 - P.D. only	99 - Other	0 13338
Υ	2015 2015/01/16 05:00:00+00 1899/12/31 11:45:00+00 15-906	OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV	36	5947.7188 5030	781.5 -75.71	1874237 45.4142952 03 - S	inow		05 - Packed snow	10 - No control		03 - P.D. only	04 - Sideswipe	0 906
Y	2015 2015/03/12 04:00:00+00 1900/01/01 00:59:00+00 15-3739	OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV				1884155 45.41427612 01 - C			01 - Dry	10 - No control		02 - Non-fatal injury	03 - Rear end	0 3845
Y Y	2015 2015/01/13 05:00:00+00 1899/12/31 23:20:00+00 15-13523 2015 2015/02/14 05:00:00+00 1899/12/31 14:15:00+00 15-13551	OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV				1866608 45.4143219 03 - S 7192688 45.41383362 03 - S			05 - Packed snow 03 - Loose snow	10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	07 - SMV other 99 - Other	0 13025 0 13052
Y	2015 2015/01/28 05:00:00+00 1899/12/31 15:15:00+00 15-1441	OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST	36	6254.0625 5030	1999.5 -75.7	7148056 45.41623306 01 - 0	lear	01 - Daylight	01 - Dry	10 - No control		02 - Non-fatal injury	03 - Rear end	0 1941
Υ	2015 2015/02/13 05:00:00+00 1899/12/31 11:43:00+00 15-2458	OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST				1578979 45.4157486 03 - S	inow	03 - Dawn	06 - Ice	10 - No control		03 - P.D. only	07 - SMV other	0 2557
Y	2015 2015/06/02 04:00:00+00 1899/12/31 19:17:00+00 15-13637 2015 2015/10/15 04:00:00+00 1899/12/31 05:00:00+00 15-13725	OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST	36	6194.0625 503 6234.2188 5030	30956 -75.71 1985 5 -75.71	1557617 45.41584396 01 - C	lear lear	01 - Daylight 00 - Unknown	01 - Dry	10 - No control		03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	0 13645 0 13728
Y	2015 2015/06/26 04:00:00+00 1899/12/31 18:34:00+00 15-7099	OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE		364682.75 503	30333 -75.73	3496246 45.41036987 01 - C	Clear	01 - Daylight	01 - Dry	03 - Yield sign	01 - Functioning	02 - Non-fatal injury	03 - Rear end	0 7724
Υ	2015 2015/02/12 05:00:00+00 1899/12/31 11:31:00+00 15-13550	OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE		364683.5 5030	331.5 -75.73	3495483 45.41035843 03 - S	inow	03 - Dawn	02 - Wet	03 - Yield sign	01 - Functioning	03 - P.D. only	03 - Rear end	0 13051
Y	2015 2015/10/26 04:00:00+00 1899/12/31 13:49:00+00 15-13728 2015 2015/09/28 04:00:00+00 1899/12/31 20:40:00+00 15-13731	OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE OTTAWA RIVER PKWY WB @ PARKDALE AVE WB ON RAMP	24			3495483 45.41036224 01 - C 3551941 45.41159439 02 - R			01 - Dry	03 - Yield sign	01 - Functioning 00 - Unknown	03 - P.D. only 03 - P.D. only	03 - Rear end 07 - SMV other	0 13731 0 13734
Y	2015 2015/09/28 04:00:00+00 1899/12/31 20:40:00+00 15-13/31 2015 2015/03/14 04:00:00+00 1899/12/31 07:05:00+00 15-13751	OTTAWA RIVER PKWY WB @ PARKDALE AVE WB ON RAMP OTTAWA RIVER PKWY WB btwn RAMP & RIVER ST				3551941 45.41159439 U2 - R 3023987 45.41060257 O1 - C		01 - Daylight 07 - Dark	02 - Wet 00 - Unknown	03 - Yield sign 10 - No control	oo - onknown	03 - P.D. only 03 - P.D. only	07 - SMV other 04 - Sideswipe	0 13734 0 13753
Y	2015 2015/09/23 04:00:00+00 1899/12/31 20:49:00+00 15-9963	PARKDALE AVE @ BURNSIDE AVE		64788.375 5030	097.5 -75.73	3364258 45.40824127 01 - C	llear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 9397
Y	2015 2015/02/27 05:00:00+00 1899/12/31 13:42:00+00 15-3277	PARKDALE AVE @ SCOTT ST				3162842 45.40488815 01 - C		01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	07 - SMV other	1 3275
Y	2015 2015/05/21 04:00:00+00 1899/12/31 13:35:00+00 15-5807 2015 2015/07/05 04:00:00+00 1899/12/31 22:47:00+00 15-7407	PARKDALE AVE @ SCOTT ST PARKDALE AVE @ SCOTT ST				3162842 45.40488052 01 - C 3161316 45.40487671 01 - C		01 - Daylight 01 - Daylight	01 - Dry 01 - Dry		01 - Functioning 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	02 - Angle 03 - Rear end	0 5128 0 7206
Y	2015 2015/11/03 05:00:00+00 1899/12/31 20:00:00+00 15-11402	PARKDALE AVE @ SCOTT ST	36	4949.9375 5029	726.5 -75.73	3162079 45.40488815 01 - C	llear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	05 - Turning movement	0 11858
Υ	2015 2015/11/26 05:00:00+00 1900/01/01 03:35:00+00 15-12278	PARKDALE AVE @ SCOTT ST	36	4949.1875 5029	726.5 -75.73	3162842 45.40488815 01 - C	Clear	07 - Dark	02 - Wet	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	0 12413
Y	2015 2015/07/17 04:00:00+00 1899/12/31 22:42:00+00 15-13670 2015 2015/06/12 04:00:00+00 1899/12/31 20:30:00+00 15-13647	PARKDALE AVE EB ON RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PAR PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA				3432922 45.41088486 02 - R 3526001 45.41266632 02 - R		01 - Daylight 01 - Daylight	02 - Wet	10 - No control 10 - No control		02 - Non-fatal injury 02 - Non-fatal injury	07 - SMV other 01 - Approaching	0 13676 0 13654
Y	2015 2015/06/12 04:00:00+00 1899/12/31 20:30:00+00 15-13647 2015 2015/07/12 04:00:00+00 1899/12/31 20:00:00+00 15-13669	PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY blwn OTTAWA RIVER PKWY & PA PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY blwn OTTAWA RIVER PKWY & PA				3526001 45.41266632 02 - R 3525238 45.41085434 01 - C		01 - Daylight 01 - Daylight		10 - No control 10 - No control		02 - Non-fatal injury 03 - P.D. only	01 - Approaching 03 - Rear end	0 13654 0 13675
Υ	2015 2015/09/12 04:00:00+00 1899/12/31 19:10:00+00 15-13694	PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA	36	4645.5625 5030	398.5 -75.73	3542786 45.41096115 02 - R	Rain	01 - Daylight	02 - Wet	10 - No control		03 - P.D. only	07 - SMV other	0 13700
Y	2015 2015/07/17 04:00:00+00 1899/12/31 20:00:00+00 15-13590 2015 2015/01/08 05:00:00+00 1900/01/01 02:15:00+00 15-13499	PARKDALE AVE WB ON RAMP/OTTAWA RIVER PKWY btwn PARKDALE AVE & OTTAWA R PARKDALE AVE WB ON RAMP/OTTAWA RIVER PKWY btwn PARKDALE AVE & OTTAWA R				3581696 45.41210175 02 - R 3537445 45.41164398 03 - S	Rain	01 - Daylight 07 - Dark	02 - Wet	10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	07 - SMV other 07 - SMV other	0 13089 0 13907
Y Y	2015 2015/01/08 05:00:00+00 1900/01/01 02:15:00+00 15-13499 2015 2015/11/23 05:00:00+00 1899/12/31 20:42:00+00 15-12151	PARKDALE AVE WB ON RAMP/OTTAWA RIVER PKWY blwn PARKDALE AVE & OTTAWA R SCOTT ST blwn HINCHEY AVE & CARRUTHERS AVE				3537445 45.41164398 03 - S 7297821 45.40565109 01 - C		07 - Dark 01 - Daylight		10 - No control 10 - No control		03 - P.D. only 03 - P.D. only	07 - SMV other 02 - Angle	0 13907 0 12214
Υ	2015 2015/05/18 04:00:00+00 1900/01/01 00:34:00+00 15-5722	SCOTT ST btwn PARKDALE AVE & PINEHURST AVE	36	4963.4063 502	29732 -75.73	3145294 45.40493774 01 - C	Clear	01 - Daylight	01 - Dry	10 - No control		02 - Non-fatal injury	02 - Angle	0 5247
Y	2015 2015/01/14 05:00:00+00 1899/12/31 20:28:00+00 15-725	WELLINGTON ST @ LETT ST	36	6355.4375 503	31110 -75.71	1349335 45.41721725 01 - C	Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	0 525
Y	2015 2015/01/29 05:00:00+00 1899/12/31 16:15:00+00 15-1498 2015 2015/10/05 04:00:00+00 1899/12/31 13:50:00+00 15-10363	WELLINGTON ST @ LETT ST WELLINGTON ST @ LETT ST				1352386 45.41722488 01 - C 1350098 45.41723251 01 - C		01 - Daylight 01 - Daylight	U1 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	U1 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 02 - Angle	0 1998 0 10484
			-					,						2 20404

STUDY ARE	YEAR DATE TIME COLLISION_I 2016 2016/08/04 04:00:00+00 1899/12/31 22:03:00+00 16-7298	ID LOCATION 105 S OF COMMISSIONER ST @ ALBERT ST	X Y LONGITUDE LATITUDE ENVIRONMENT 366666.2813 5030879.5 -75.70955658 45.41511536 01 - Clear	LIGHT 01 - Daylight	SURFACE_CONDITION 01 - Dry	TRAFFIC_CONTROL TRAFFIC_CONTROL_CONDIT 01 - Traffic signal 01 - Functioning	ION COLLISION_CLASSIFICATION  03 - P.D. only	ON IMPACT_TYPE 04 - Sideswipe	NO_OF_PEDESTRIANS FID 0 7990
Ÿ	2016 2016/01/15 05:00:00+00 1899/12/31 21:30:00+00 16-631	ALBERT ST @ BOOTH ST	366428.0313 5030626.5 -75.71263123 45.41286087 01 - Clear	01 - Daylight	02 - Wet	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 831
Y	2016 2016/02/17 05:00:00+00 1900/01/01 00:31:00+00 16:1906 2016 2016/02/25 05:00:00+00 1900/01/01 00:28:00+00 16:2363	ALBERT ST @ BOOTH ST ALBERT ST @ BOOTH ST	366429.0938 5030626.5 -75.71261597 45.41286087 01 - Clear 366427.5938 5030626.5 -75.71263123 45.41286087 01 - Clear	07 - Dark 07 - Dark	03 - Loose snow 02 - Wet	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 03 - Rear end	0 1711 0 2800
Y	2016 2016/07/16 04:00:00+00 1899/12/31 15:21:00+00 16:6736	ALBERT ST @ BOOTH ST	366428 5030626 5 -75 71263123 45 41286087 01 - Clear	01 - Daylight	02 - Wet 01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswine	0 6133
Υ	2016 2016/07/06 04:00:00+00 1899/12/31 12:25:00+00 16-6429 2016 2016/07/10 04:00:00+00 1899/12/31 16:59:00+00 16-6550	ALBERT ST @ ROOTH ST	366428.2813 5030625 -75.7126236 45.41284943 01 - Clear 366429.6563 5030625.5 -75.71260834 45.41285324 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 04 - Sideswipe	0 6483 0 6943
Y	2016 2016/07/10 04:00:00+00 1899/12/31 16:59:00+00 16:6550 2016 2016/08/17 04:00:00+00 1899/12/31 06:56:00+00 16:7633	ALBERT ST @ BOOTH ST ALBERT ST @ BOOTH ST	366429.6563 5030625.5 -75.71260834 45.41285324 01 - Clear 366428.6875 5030628.5 -75.7126236 45.41287994 01 - Clear	01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	04 - Sideswipe 07 - SMV other	0 6943 0 7337
Y	2016 2016/09/18 04:00:00+00 1899/12/31 15:29:00+00 16:8585	ALBERT ST @ BOOTH ST	366429.25 5030627 -75.71261597 45.41286469 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 8398
Υ	2016 2016/11/16 05:00:00+00 1900/01/01 04:30:00+00 16-10577	ALBERT ST @ BOOTH ST	366427.5938 5030627.5 -75.71263123 45.41287231 01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	02 - Angle	0 10639
Y	2016 2016/12/17 05:00:00+00 1899/12/31 17:20:00+00 16-11948 2016 2016/11/29 05:00:00+00 1899/12/31 11:39:00+00 16-11077	ALBERT ST @ BOOTH ST ALBERT ST @ BOOTH ST	366428.4688 5030626.5 -75.7126236 45.41286087 03 - Snow 366429.5313 5030625.5 -75.71260834 45.41284943 03 - Snow	01 - Daylight 07 - Dark	03 - Loose snow 02 - Wet	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 02 - Angle	0 11265 0 11691
Y	2016 2016/02/16 05:00:00+00 1899/12/31 19:53:00+00 16-1782	ALBERT ST @ BRONSON AVE	366741.0938 5031017.5 -75.70858002 45.41635513 03 - Snow	01 - Daylight	03 - Loose snow	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	01 - Approaching	0 1982
Y	2016 2016/07/31 04:00:00+00 1899/12/31 19:55:00+00 16:7187 2016 2016/09/03 04:00:00+00 1900/01/01 02:47:00+00 16:8161	ALBERT ST @ BRONSON AVE ALBERT ST @ BRONSON AVE	366739.4375 5031017 -75.70860291 45.4163475 01 - Clear 366739.0313 5031016.5 -75.70861053 45.41633987 01 - Clear	01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 02 - Angle	0 7473 0 8175
Y	2016 2016/10/11 04:00:00+00 1999/12/31 19:34:00+00 16-9288	ALBERT ST @ BRONSON AVE	366739.0313 5031016.5 -/5.70861053 45.41633987 01 - Clear 366739.0313 5031017 -75.70861053 45.41634369 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswipe	0 9787
Υ	2016 2016/10/05 04:00:00+00 1899/12/31 12:47:00+00 16:9098	ALBERT ST @ COMMISSIONER ST	366713.9063 5030971.5 -75.70893097 45.41593933 01 - Clear	01 - Daylight	01 - Dry	02 - Stop sign 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 9297
Y	2016 2016/06/14 04:00:00+00 1899/12/31 23:41:00+00 16:5718 2016 2016/06/30 04:00:00+00 1899/12/31 15:45:00+00 16:6251	ALBERT ST @ EMPRESS AVE ALBERT ST @ PERKINS ST	366577.375 5030720.5 -75.71070862 45.41369247 01 - Clear 366534.5625 5030695 -75.71125793 45.41346741 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 02 - Stop sign 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	0 5479 0 6045
Y	2016 2016/01/04 05:00:00+00 1899/12/31 23:13:00+00 16:95	ALBERT ST @ PRESTON ST	366161.0625 5030464.5 -75.71605682 45.41142654 01 - Clear	07 - Dark	03 - Loose snow	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 95
Y	2016 2016/02/15 05:00:00+00 1899/12/31 17:15:00+00 16:1731 2016 2016/02/19 05:00:00+00 1899/12/31 15:51:00+00 16:2077	ALBERT ST @ PRESTON ST ALBERT ST @ PRESTON ST	366160.3438 5030464.5 -75.71607208 45.41142654 01 - Clear 366161.1875 5030464.5 -75.71605682 45.41142654 01 - Clear	01 - Daylight 01 - Daylight	06 - Ice 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	0 1931 0 2877
Y	2016 2016/03/21 04:00:00+00 1899/12/31 14:00:00+00 16:3140	ALBERT ST @ PRESTON ST	366160.3125 5030464.5 -75.71607208 45.41142654 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 3039
Υ	2016 2016/06/10 04:00:00+00 1899/12/31 15:44:00+00 16:5596	ALBERT ST @ PRESTON ST	366161 5030464.5 -75.71606445 45.41142654 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswipe	0 5213
Y	2016 2016/07/07 04:00:00+00 1899/12/31 20:46:00+00 16:6478 2016 2016/08/24 04:00:00+00 1899/12/31 09:54:00+00 16:7834	ALBERT ST @ PRESTON ST	366159.9063 5030465 -75.71607208 45.41143036 01 - Clear 366161.0625 5030465 5 -75.71605682 45.41142654 01 - Clear	01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 02 - Non-fatal injury	03 - Rear end 02 - Angle	0 6533 0 7305
Y	2016 2016/08/01 04:00:00+00 1899/12/31 15:37:00+00 16-7196	ALBERT ST @ PRESTON ST	366160.1875 5030464.5 -75.71607208 45.41142654 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	04 - Sideswipe	0 7482
Y	2016 2016/09/23 04:00:00+00 1899/12/31 12:36:00+00 16:8760 2016 2016/09/14 04:00:00+00 1899/12/31 17:24:00+00 16:8460	ALBERT ST @ PRESTON ST ALBERT ST @ PRESTON ST	366160.7813 5030464.5 -75.71606445 45.41142654 02 - Rain 366161.0625 5030465 -75.71605682 45.41143036 01 - Clear	01 - Daylight 01 - Daylight	02 - Wet 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 04 - Sideswipe	0 8761 0 8950
Y	2016 2016/05/13 04:00:00+00 1999/12/31 17:24:00+00 16-8460 2016 2016/05/13 04:00:00+00 1900/01/01 02:33:00+00 16-4717	ALBERT ST btwn CITY CENTRE AVE & PRESTON ST	36616.4063 5030437.5 -75.71663666 45.41119003 01 - Clear	07 - Dark	01 - Dry 01 - Dry	10 - No control	02 - Non-fatal injury	05 - Turning movement	0 4262
Υ	2016 2016/08/05 04:00:00+00 1899/12/31 16:40:00+00 16-7318	ALBERT ST htmn PERKINS ST & TRANSIT	366556 4063 5030707 5 -75 71097565 45 41357803 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	10 - No control 10 - No control	03 - P.D. only	04 - Sideswine	0 7200 0 6709
Y	2016 2016/07/21 04:00:00+00 1899/12/31 12:05:00+00 16:6864 2016 2016/10/21 04:00:00+00 1899/12/31 16:49:00+00 16:9657	ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (2) ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (2)	365873.8125 5030307.5 -75.71974945 45.41003799 01 - Clear 365944.5938 5030340 -75.71884155 45.4103241 02 - Rain	01 - Daylight 01 - Daylight	01 - Dry 02 - Wet	10 - No control 10 - No control	03 - P.D. only 02 - Non-fatal injury	04 - Sideswipe 01 - Approaching	0 6709 0 9460
Y	2016 2016/02/10 05:00:00+00 1899/12/31 13:11:00+00 16:1493	BAYVIEW RD @ SCOTT ST/ALBERT ST	365518.8438 5030072 -75.72431183 45.40794754 03 - Snow	01 - Daylight	04 - Slush	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 1893
Υ	2016 2016/03/02 05:00:00+00 1899/12/31 23:24:00+00 16-2609 2016 2016/06/16 04:00:00+00 1899/12/31 13:38:00+00 16-5760	BAYVIEW RD @ SCOTT ST/ALBERT ST RAYVIEW RD @ SCOTT ST/ALBERT ST	365520.5313 5030072 -75.72428894 45.40795135 01 - Clear 365519.0313 5030071 5 -75.72431183 45.40794754 01 - Clear	05 - Dusk	02 - Wet	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	02 - Non-fatal injury 02 - Non-fatal injury	07 - SMV other	1 2234 0 5868
Y	2016 2016/06/16 04:00:00+00 1899/12/31 13:38:00+00 16:5760 2016 2016/08/18 04:00:00+00 1899/12/31 12:50:00+00 16:7677	BAYVIEW RD @ SCOTT ST/ALBERT ST  BAYVIEW RD @ SCOTT ST/ALBERT ST	365518.125 5030071.5 -75.72431183 45.40794754 01 - Clear 365518.125 5030072 -75.72431946 45.40794754 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	02 - Non-fatal injury 03 - P.D. only	05 - Turning movement 04 - Sideswipe	0 5868 0 7389
Y	2016 2016/08/02 04:00:00+00 1900/01/01 00:13:00+00 16-7236	BAYVIEW RD @ SCOTT ST/ALBERT ST	365519.1563 5030071.5 -75.72431183 45.40794754 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 7588
Y	2016 2016/09/15 04:00:00+00 1899/12/31 18:14:00+00 16:8487 2016 2016/12/15 05:00:00+00 1899/12/31 22:07:00+00 16:11845	BAYVIEW RD @ SCOTT ST/ALBERT ST BAYVIEW RD @ SCOTT ST/ALBERT ST	365518.8125 5030072 -75.72431183 45.40794754 01 - Clear 365518.8125 5030071.5 -75.72431183 45.40794373 01 - Clear	01 - Daylight 07 - Dark	01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 05 - Turning movement	0 8977 0 11466
Y	2016 2016/12/15 05:00:00+00 1899/12/31 22:07:00+00 16-11845 2016 2016/12/20 05:00:00+00 1899/12/31 14:07:00+00 16-12093	BAYVIEW RD @ SCOTT ST/ALBERT ST	365519.875 5030071 -75.72429657 45.40794373 01 - Clear	01 - Daylight	03 - Loose snow 04 - Slush	01 - Traffic signal 01 - Functioning	03 - P.D. only	02 - Angle	0 11466 0 12809
Υ	2016 2016/04/04 04:00:00+00 1899/12/31 14:18:00+00 16:3501	BOOTH ST @ 148 N OF MIDDLE ST/E.B.EDDY S	366034.5 5031417 -75.71755981 45.4200058 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 3915
Y	2016 2016/07/07 04:00:00+00 1899/12/31 20:05:00+00 16:6477 2016 2016/09/15 04:00:00+00 1899/12/31 14:10:00+00 16:8481	BOOTH ST @ 208 N OF MIDDLE ST/E.B. EDDY N  BOOTH ST @ FLEET ST	366010.4063 5031471.5 -75.71785736 45.42050171 01 - Clear 366296.4688 5030935 -75.71427155 45.41564941 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 02 - Stop sign 01 - Functioning	03 - P.D. only 03 - P.D. only	04 - Sideswipe 05 - Turning movement	0 6532 0 8971
Y	2016 2016/04/07 04:00:00+00 1899/12/31 18:44:00+00 16:3669	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366265.375 5031006.5 -75.71466064 45.4162941 03 - Snow	01 - Daylight	03 - Loose snow	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 3182
Y	2016 2016/03/25 04:00:00+00 1900/01/01 03:21:00+00 16:3288 2016 2016/05/13 04:00:00+00 1899/12/31 23:03:00+00 16:4706	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366262.8438 5031006 -75.71469116 45.41629028 01 - Clear 366264.5 5031007.5 -75.71466827 45.41630173 01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswipe	0 3805 0 4127
Y	2016 2016/05/13 04:00:00+00 1899/12/31 23:03:00+00 16-4/06 2016 2016/06/24 04:00:00+00 1899/12/31 18:42:00+00 16-6061	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.1875 5031007 -75.7146759 45.41630173 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	0 6412
Υ	2016 2016/09/04 04:00:00+00 1899/12/31 18:30:00+00 16:8170	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.8125 5031007 -75.71466827 45.41629791 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 8183
Y	2016 2016/10/03 04:00:00+00 1899/12/31 17:15:00+00 16:9042 2016 2016/12/06 05:00:00+00 1899/12/31 13:45:00+00 16:11362	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.4375 5031008 -75.71466827 45.41630554 01 - Clear 366264.0313 5031006 -75.7146759 45.41679028 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 02 - Wet	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 04 - Sideswine	0 9239
Y	2016 2016/01/15 05:00:00+00 1899/12/31 23:43:00+00 16-12663	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366263.9375 5031005 -75.7146759 45.41628265 01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswipe	0 12047
Y	2016 2016/09/04 04:00:00+00 1899/12/31 18:30:00+00 16:12537	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366263.7188 5031008 -75.71468353 45.41630554 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	03 - Rear end	0 12651
Y	2016 2016/08/19 04:00:00+00 1899/12/31 22:11:00+00 16-12550 2016 2016/01/19 05:00:00+00 1899/12/31 13:05:00+00 16-754	BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV	366264.4063 5031007 -75.7146759 45.41629791 01 - Clear 366129.8438 5031271.5 -75.716362 45.41868973 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 02 - Wet	01 - Traffic signal 01 - Functioning 10 - No control	03 - P.D. only 03 - P.D. only	03 - Rear end 07 - SMV other	0 12664 0 154
Υ	2016 2016/02/15 04:00:00±00 1999/12/21 20:10:00±00 16:2001	ROOTH ST bown MIDDLE ST & VIMY PLACE PRIV	366228.7813 5031102 -75.71511841 45.41715622 02 - Rain 366267.5 5030999 -75.71463013 45.41622925 01 - Clear	01 - Daylight 01 - Daylight	02 - Wet	10 - No control 10 - No control	03 - P.D. only 03 - P.D. only	03 - Rear end	0 2990 0 5872
Y	2016 2016/06/16 04:00:00+00 1899/12/31 16:45:00+00 16:5764 2016 2016/08/22 04:00:00+00 1899/12/31 20:37:00+00 16:7790	BOOTH ST btwn OTTAWA RIVER PKWY & FLEET ST BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N	366267.5 5030999 -75.71463013 45.41622925 01 - Clear 365990.5 5031513.5 -75.71810913 45.42087936 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 02 - Non-fatal injury	03 - Rear end 05 - Turning movement	0 5872 0 7083
Ÿ	2016 2016/11/04 04:00:00+00 1899/12/31 11:22:00+00 16-10138	BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N	366006.6563 5031480 -75.71790314 45.420578 01 - Clear	07 - Dark	01 - Dry	10 - No control	03 - P.D. only	04 - Sideswipe	0 10777
Y	2016 2016/12/24 05:00:00+00 1899/12/31 12:03:00+00 16:12594 2016 2016/02/01 05:00:00+00 1899/12/31 18:35:00+00 16:1238	BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N RRONSON AVE & COMMISSIONER ST/SI ATER ST	365983.9375 5031527.5 -75.71819305 45.42100906 03 - Snow 366768.9375 5030949 -75.70823669 45.41572952 01 - Clear	07 - Dark 01 - Daylight	02 - Wet	10 - No control 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	07 - SMV other	0 12709 0 1447
Y	2016 2016/02/21 05:00:00+00 1899/12/31 18:35:00+00 16-1238 2016 2016/02/22 05:00:00+00 1899/12/31 13:02:00+00 16-2185	BRONSON AVE @ COMMISSIONER ST/SLATER ST  BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.8125 5030949 -75.70823669 45.41572952 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only	02 - Angle 04 - Sideswipe	0 1447
Υ	2016 2016/04/12 04:00:00+00 1899/12/31 15:16:00+00 16:3804	BRONSON AVE @ COMMISSIONER ST/SLATER ST	366768.4063 5030949 -75.70824432 45.41572952 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 3450
Y	2016 2016/05/23 04:00:00+00 1899/12/31 20:22:00+00 16-4988 2016 2016/05/26 04:00:00+00 1899/12/31 19:39:00+00 16-5105	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST	366767.7813 5030949.5 -75.70825195 45.41573334 01 - Clear 366768.5 5030949 -75.70823669 45.41573334 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 02 - Non-fatal injury	05 - Turning movement 05 - Turning movement	0 4910 0 5292
Ý	2016 2016/06/05 04:00:00+00 1899/12/31 21:22:00+00 16:5456	BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.0938 5030948 -75.70822906 45.41572189 01 - Clear	01 - Daylight	02 - Wet	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 5705
Υ	2016 2016/06/07 04:00:00+00 1899/12/31 21:16:00+00 16:5504 2016 2016/06/16 04:00:00+00 1899/12/31 18:20:00+00 16:5769	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.0938 5030948.5 -75.70822906 45.41572952 01 - Clear 366768.3125 5030948 -75.70824432 45.41572189 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	02 - Angle	0 5752 0 5877
Y	2016 2016/06/16 04:00:00+00 1899/12/31 18:20:00+00 16:5769 2016 2016/07/22 04:00:00+00 1899/12/31 17:31:00+00 16:6911	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST	366768.3125 5030948 -75.70824432 45.41572189 01 - Clear 366769.3438 5030949.5 -75.70822906 45.41573334 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 05 - Turning movement	0 5877 0 6225
Υ	2016 2016/07/20 04:00:00+00 1899/12/31 19:29:00+00 16-6847	BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.3438 5030948 -75.70822906 45.41572571 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	02 - Angle	0 6692
Y	2016 2016/09/18 04:00:00+00 1899/12/31 16:44:00+00 16:8587 2016 2016/09/15 04:00:00+00 1899/12/31 23:50:00+00 16:8503	BRONSON AVE @ COMMISSIONER ST/SLATER ST BRONSON AVE @ COMMISSIONER ST/SLATER ST	366767.5938 5030948 -75.70825195 45.41572189 01 - Clear 366767.5938 5030949 -75.70825195 45.41572952 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 04 - Sideswipe	0 8400 0 8993
Y	2016 2016/10/11 04:00:00+00 1899/12/31 15:07:00+00 16-9280	BRONSON AVE @ COMMISSIONER ST/SLATER ST	366770.125 5030948.5 -75.70822144 45.41572571 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 9779
Υ	2016 2016/10/06 04:00:00+00 1899/12/31 19:01:00+00 16:9164	BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.375 5030949 -75.70822906 45.41572952 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	05 - Turning movement	0 9953
Y	2016 2016/11/21 05:00:00+00 1900/01/01 03:01:00+00 16:10795 2016 2016/01/04 05:00:00+00 1900/01/01 00:48:00+00 16:96	BRONSON AVE @ COMMISSIONER ST/SLATER ST OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366769.4063 5030947.5 -75.70822906 45.41571808 03 - Snow 366516.75 5031315 -75.71141052 45.41904831 03 - Snow	07 - Dark 07 - Dark	03 - Loose snow 03 - Loose snow	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	07 - SMV other 04 - Sideswipe	0 10752 0 96
Υ	2016 2016/07/09 04:00:00+00 1899/12/31 22:39:00+00 16-6536	OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366515.7188 5031314.5 -75.71142578 45.41904449 02 - Rain	01 - Daylight	02 - Wet	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 6929
Y	2016 2016/08/25 04:00:00+00 1899/12/31 17:10:00+00 16:12568 2016 2016/12/29 05:00:00+00 1899/12/31 18:32:00+00 16:12729	OTTAWA RIVER PKWY @ PORTAGE BRIDGE OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST	366516.2188 5031314.5 -75.71141815 45.41904449 01 - Clear 365159.2188 5030375.5 -75.72886658 45.4107132 03 - Snow	01 - Daylight 01 - Daylight	01 - Dry 03 - Loose snow	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 01 - Fatal injury	04 - Sideswipe 03 - Rear end	0 12683 0 12114
Y	2016 2016/07/17 04:00:00+00 1899/12/31 19:55:00+00 16-12578	OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST	365153.4688 5030375 -75.72894287 45.41070938 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	02 - Angle	0 12693
Y	2016 2016/10/30 04:00:00+00 1899/12/31 21:00:00+00 16-12602 2016 2016/06/25 04:00:00+00 1899/12/31 15:56:00+00 16-6090	OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST OTTAWA RIVER PKWY @ VIMY PLACE PRIV	365156.9063 5030377.5 -75.72889709 45.41073227 01 - Clear 365975.2188 5030800 -75.71839142 45.41445923 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	02 - Angle 03 - Rear end	0 12717 0 6442
Y	2016 2016/06/25 04:00:00+00 1899/12/31 15:56:00+00 16-6090 2016 2016/09/01 04:00:00+00 1899/12/31 10:36:00+00 16-8059	OTTAWA RIVER PKWY @ VIMY PLACE PRIV	365975.2188 5030800 -75.71839142 45.41443923 01 - Clear 365973.8125 5030797.5 -75.71841431 45.41443634 01 - Clear	07 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	04 - Sideswipe	0 644Z 0 8568
Υ	2016 2016/01/13 05:00:00+00 1899/12/31 21:09:00+00 16-12656	OTTAWA RIVER PKWY @ VIMY PLACE PRIV	365975.9063 5030797.5 -75.71838379 45.41444016 01 - Clear	01 - Daylight	02 - Wet	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 12041
Y	2016 2016/12/31 05:00:00+00 1899/12/31 18:50:00+00 16:12470 2016 2016/03/02 05:00:00+00 1899/12/31 16:05:00+00 16:12683	OTTAWA RIVER PKWY @ VIMY PLACE PRIV OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV	365975.1875 5030798 -75.71839142 45.41444397 03 - Snow 365723.8438 5030498 5 -75.72164154 45.41176987 03 - Snow	01 - Daylight 01 - Daylight	03 - Loose snow 06 - Ice	01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 07 - SMV other	0 12956 0 12067
Y	2016 2016/03/02 05:00:00+00 1899/12/31 16:05:00+00 16-12683 2016 2016/04/20 04:00:00+00 1899/12/31 21:17:00+00 16-12714	OTTAWA RIVER PKWY BIWN TRANSITWAY - TIME POINT & VIMY PLACE PRIV	365/23.8438 5030/498.5 -/5.72164154 45.411/6987 03 - 5now 365949.8438 5030/780 -75.71871948 45.41428375 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	10 - No control	02 - Non-fatal injury	07 - SMV other	0 12097
Υ	2016 2016/09/26 04:00:00+00 1899/12/31 22:10:00+00 16-12512	OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV	365934.4063 5030778.5 -75.71891785 45.41426849 01 - Clear	01 - Daylight	01 - Dry	10 - No control	03 - P.D. only	03 - Rear end	0 12998
Y	2016 2016/06/13 04:00:00+00 1899/12/31 20:20:00+00 16:5683 2016 2016/07/13 04:00:00+00 1899/12/31 20:00:00+00 16:6640	OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST	366250.2813 5030996 -75.71485138 45.41620255 01 - Clear 366260.1875 5031004 -75.71472931 45.41627502 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 02 - Non-fatal injury	04 - Sideswipe 03 - Rear end	0 5417 0 6270
Ÿ	2016 2016/04/27 04:00:00+00 1899/12/31 08:20:00+00 16-12754	OTTAWA RIVER PKWY WB btwn RAMP & RIVER ST	365064.125 5030356 -75.73008728 45.41054916 01 - Clear	07 - Dark	01 - Dry	10 - No control	03 - P.D. only	07 - SMV other	0 12139
Y	2016 2016/07/06 04:00:00+00 1899/12/31 21:03:00+00 16-6444 2016 2016/07/23 04:00:00+00 1899/12/31 15:33:00+00 16-6942	PARKDALE AVE @ SCOTT ST PARKDALE AVE @ SCOTT ST	364949.3125 5029726.5 -75.73162842 45.40489197 01 - Clear 364949.3125 5029726 -75.73162842 45.40488434 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only	07 - SMV other	0 6498 0 6573
Y	2016 2016/07/23 04:00:00+00 1899/12/31 15:33:00+00 16-6942 2016 2016/11/14 05:00:00+00 1899/12/31 22:31:00+00 16-10483	PARKDALE AVE @ SCOTT ST	364951.375 5029726 -75.73160553 45.40488815 01 - Clear	01 - Daylight 07 - Dark	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	05 - Turning movement 03 - Rear end	0 10523
Υ	2016 2016/12/06 05:00:00+00 1899/12/31 23:22:00+00 16-11387	PARKDALE AVE @ SCOTT ST	364948.5938 5029726.5 -75.73163605 45.40489197 01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal 01 - Functioning	02 - Non-fatal injury	05 - Turning movement	0 11601
Y	2016 2016/03/16 04:00:00+00 1899/12/31 15:00:00+00 16:3007 2016 2016/04/14 04:00:00+00 1899/12/31 13:34:00+00 16:3855	PARKDALE AVE bitwin BURNSIDE AVE & LYNDALE AVE PARKDALE AVE bitwin LYNDALE AVE & SCOTT ST	364825.1875 5030016.5 -75.73317719 45.40750885 01 - Clear 364918.5 5029798.5 -75.73201752 45.40554428 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 03 - P.D. only	02 - Angle 02 - Angle	0 3321 0 3582
Ÿ	2016 2016/06/22 04:00:00+00 1899/12/31 23:37:00+00 16-6008	PARKDALE AVE bitwn LYNDALE AVE & SCOTT ST PARKDALE AVE bitwn LYNDALE AVE & SCOTT ST PARKDALE AVE bitwn LYNDALE AVE & SCOTT ST	364928.7188 5029773.5 -75.73188782 45.40531921 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 03 - P.D. only	02 - Angle 02 - Angle	0 3582 0 6143
Y	2016 2016/12/06 05:00:00+00 1899/12/31 13:19:00+00 16:11360	PARKDALE AVE btwn LYNDALE AVE & SCOTT ST	364936.1563 5029753 -75.73179626 45.40513229 01 - Clear	01 - Daylight	02 - Wet	10 - No control	03 - P.D. only	02 - Angle	0 11574
Y	2016 2016/06/14 04:00:00+00 1899/12/31 21:36:00+00 16-5712 2016 2016/06/01 04:00:00+00 1899/12/31 19:45:00+00 16-5357	PARKDALE AVE bown TO BE DETERMINED & EMMERSON AVE PARKDALE AVE bown TO BE DETERMINED & EMMERSON AVE	364706.875 5030289 -75.73465729 45.40997314 01 - Clear 364706.5313 5030288 -75.73466492 45.40996552 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 03 - P.D. only	03 - Rear end 06 - SMV unattended vehicle	0 5352 0 5781
Y	2016 2016/07/30 04:00:00+00 1899/12/31 15:10:00+00 16-12730	PARKDALE AVE EB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA	364585.125 5030405.5 -75.73619843 45.41103363 01 - Clear	01 - Daylight	01 - Dry	10 - No control	03 - P.D. only	03 - Rear end	0 12115
Y	2016 2016/01/04 05:00:00+00 1899/12/31 16:00:00+00 16:12658 2016 2016/06/06 04:00:00+00 1899/12/31 12:20:00+00 16:12726	PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA	364830.9375 5030402.5 -75.73306274 45.41098404 01 - Clear 364818.4375 5030412 -75.73321533 45.41107178 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control 10 - No control	03 - P.D. only 03 - P.D. only	07 - SMV other 04 - Sideswipe	0 12042 0 12111
Ÿ	2016 2016/12/09 05:00:00+00 1899/12/31 13:30:00+00 16-12589	PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA	364688.3438 5030584 -75.73485565 45.41262817 01 - Clear	01 - Daylight	06 - Ice	10 - No control	03 - P.D. only	03 - Rear end	0 12704
Y	2016 2016/05/20 04:00:00+00 1899/12/31 22:05:00+00 16-4915 2016 2016/04/04 04:00:00+00 1899/12/31 17:24:00+00 16-3505	SLATER ST btwn ALBERT ST & COMMISSIONER ST WELLINGTON ST @ LETT ST	366737.6875 5030913 -75.70864105 45.41540909 01 - Clear 366354.5 5031111 -75.71350861 45.41722488 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	10 - No control	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	0 4358 0 3114
Y	2016 2016/04/04 04:00:00+00 1899/12/31 17:24:00+00 16:3505 2016 2016/04/06 04:00:00+00 1899/12/31 22:37:00+00 16:3595	WELLINGTON ST @ LETT ST WELLINGTON ST @ LETT ST	366354.5 5031111 -75.71350861 45.41722488 01 - Clear 366355.6563 5031111 -75.71349335 45.4172287 03 - Snow	01 - Daylight 01 - Daylight	01 - Dry 04 - Slush	01 - Traffic signal 01 - Functioning	03 - P.D. only 03 - P.D. only	03 - Rear end 02 - Angle	0 3114 0 3725
Υ	2016 2016/08/10 04:00:00+00 1899/12/31 12:41:00+00 16-7423	WELLINGTON ST @ LETT ST	366354.8125 5031111 -75.71350098 45.41722488 01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal 01 - Functioning	03 - P.D. only	03 - Rear end	0 7199
Y	2016 2016/09/19 04:00:00+00 1899/12/31 20:51:00+00 16:8627 2016 2016/06/13 04:00:00+00 1899/12/31 20:00:00+00 16:5680	WELLINGTON ST @ LETT ST WELLINGTON ST btwn OTTAWA RIVER PKWY & TO BE DETERMINED	366355.2188 5031111.5 -75.71350098 45.41723251 01 - Clear 366275.3438 5031015 -75.71453094 45.41637039 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry 01 - Dry	01 - Traffic signal 01 - Functioning 10 - No control	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	0 8017 0 5414
•		A 10 DE DETERMINED					amy		0 3424

Study A	rea Year R	ecord Location	х ү	Longitude	Latitude	Date T	ime Environment	Road Surface	Traffic Control	Collision Location	Light	Collision Classification	Impact type	FID
Y	2017	150 ALBERT ST @ BOOTH ST	366428.3438 5030626	-75.7126236	45.41285706	1.49638E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	07 - SMV other	51
Y	2017	151 ALBERT ST @ BOOTH ST	366429.0313 5030626.5				-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related		03 - P.D. only	04 - Sideswipe	52
Y	2017 2017	152 ALBERT ST @ BOOTH ST 153 ALBERT ST @ BOOTH ST	366427.6875 5030626 366429.1563 5030625.5			1.49975E+12 1.50381E+12	-2.209E+12 01 - Clear -2.20897E+12 01 - Clear	02 - Wet 01 - Dry	01 - Traffic signal 01 - Traffic signal	03 - At intersection 03 - At intersection	01 - Daylight 07 - Dark	02 - Non-fatal injury 03 - P.D. only	05 - Turning movement 05 - Turning movement	53 54
Y	2017	154 ALBERT ST @ BOOTH ST	366430.375 5030626.5				-2.20897E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	07 - Dark	03 - P.D. only	05 - Turning movement	55
Y	2017	155 ALBERT ST @ BOOTH ST	366427.7813 5030627				-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	56
Y	2017	156 ALBERT ST @ BOOTH ST	366429.625 5030627.5 366429.6875 5030626.5	-75.71260834	45.4128685	1.50536E+12 1.50942E+12	-2.20903E+12 01 - Clear -2.209E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	03 - At intersection 02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	02 - Angle 04 - Sideswipe	57
Y	2017	158 ALBERT ST @ BOOTH ST	366428.6875 5030627				-2.209E+12 01 - Clear -2.20902E+12 03 - Snow	05 - Packed snow	01 - Traffic signal	02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only	03 - Rear end	58 59
Y	2017	159 ALBERT ST @ BOOTH ST	366427.6875 5030628	-75.71263123	45.41287231	1.51089E+12	-2.20898E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	07 - Dark	03 - P.D. only	03 - Rear end	60
Y	2017	160 ALBERT ST @ BOOTH ST	366429.1563 5030625.5			1.51262E+12	-2.20898E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	07 - Dark	02 - Non-fatal injury	05 - Turning movement	61
Y	2017	161 ALBERT ST @ BOOTH ST 162 ALBERT ST @ BOOTH ST	366429.4375 5030627.5 366428.6875 5030626.5			1.4861E+12	-2.209E+12 01 - Clear -2.20902E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 03 - At intersection	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	62 63
Ÿ	2017	163 ALBERT ST @ BOOTH ST	366429.4375 5030625.5	-75.71260834	45.41284943		-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	64
Y	2017	164 ALBERT ST @ BOOTH ST	366427.6875 5030627				-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	65
Y	2017 2017	165 ALBERT ST @ BOOTH ST 166 ALBERT ST @ BOOTH ST	366428.3438 5030628 366429.4375 5030627.5	-75.7126236		1.48946E+12	-2.20901E+12 03 - Snow -2.20901E+12 01 - Clear	03 - Loose snow	01 - Traffic signal 01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only 03 - P.D. only	05 - Turning movement 03 - Rear end	66 67
Y	2017	167 ALBERT ST @ BOOTH ST	366429.4375 5030627.5 366429.1563 5030626				-2.20901E+12 01 - Clear -2.20901E+12 03 - Snow	01 - Dry 03 - Loose snow	01 - Traffic signal	02 - Intersection related 03 - At intersection	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end	68
Y	2017	169 ALBERT ST @ BRONSON AVE		-75.70859528			-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related		03 - P.D. only	05 - Turning movement	70 71
Y	2017	170 ALBERT ST @ COMMISSIONER ST		-75.70893097			-2.209E+12 01 - Clear	01 - Dry	02 - Stop sign	03 - At intersection		02 - Non-fatal injury	05 - Turning movement	71
Y	2017 2017	171 ALBERT ST @ COMMISSIONER ST 172 ALBERT ST @ COMMISSIONER ST		-75.70892334 -75.70893097			-2.20901E+12 01 - Clear -2.20902E+12 01 - Clear	01 - Dry 01 - Dry	02 - Stop sign 02 - Stop sign	03 - At intersection 02 - Intersection related		03 - P.D. only 03 - P.D. only	04 - Sideswipe 02 - Angle	72 73
Y	2017	173 ALBERT ST @ COMMISSIONER ST	366714.6563 5030973	-75.70892334	45.41595078	1.51401E+12	-2.20901E+12 03 - Snow	03 - Loose snow	02 - Stop sign	03 - At intersection	01 - Daylight	03 - P.D. only	04 - Sideswipe	74
Y	2017	185 ALBERT ST @ EMPRESS AVE	366576.6875 5030720.5			1.50579E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	04 - Sideswipe	86
Y	2017 2017	186 ALBERT ST @ EMPRESS AVE 187 ALBERT ST @ EMPRESS AVE	366577.1875 5030720.5 366577.4375 5030721				-2.20901E+12 01 - Clear -2.20901E+12 02 - Rain	01 - Dry 02 - Wet	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 02 - Intersection related		03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	87 88
Ý	2017	188 ALBERT ST @ EMPRESS AVE		-75.71070802			-2.20902E+12 02 - Kalli -2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	04 - Sideswipe	89
Y	2017	205 ALBERT ST @ PERKINS ST	366534.0313 5030697	-75.71126556	45.41348648		-2.20902E+12 01 - Clear	01 - Dry	02 - Stop sign	03 - At intersection	01 - Daylight	03 - P.D. only	07 - SMV other	205
Y	2017	206 ALBERT ST @ PRESTON ST 207 ALBERT ST @ PRESTON ST	366160.2188 5030466 366161.0313 5030466	-75.71607208 -75.71605682	45.41143799		-2.20899E+12 01 - Clear -2.209E+12 02 - Rain	01 - Dry 02 - Wet	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 02 - Intersection related	01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	206 207
Y	2017	207 ALBERT ST @ PRESTON ST 208 ALBERT ST @ PRESTON ST		-75.71605682			-2.209E+12 02 - Rain -2.20897E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight 07 - Dark	03 - P.D. only	03 - Rear end	207
Y	2017	209 ALBERT ST @ PRESTON ST	366159.375 5030466	-75.71607971	45.4114418	1.50666E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	02 - Non-fatal injury	03 - Rear end	209
Y	2017	210 ALBERT ST @ PRESTON ST	366162.4063 5030466.5				-2.20898E+12 03 - Snow	03 - Loose snow	01 - Traffic signal	02 - Intersection related	07 - Dark	03 - P.D. only	03 - Rear end	210
Y	2017 2017	211 ALBERT ST @ PRESTON ST 219 ALBERT ST btwn CITY CENTRE AVE & PRESTON ST		-75.71605682 -75.71642303			-2.20899E+12 01 - Clear -2.20901E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 10 - No control	02 - Intersection related 01 - Non intersection	01 - Daylight 01 - Daylight	02 - Non-fatal injury 03 - P.D. only	03 - Rear end 04 - Sideswipe	211 219
Y	2017	222 ALBERT ST btwn LORNE AVE & PERKINS ST	366519.3438 5030687	-75.7114563	45.41339493	1.48765E+12	-2.20899E+12 02 - Rain	02 - Wet	10 - No control	01 - Non intersection	05 - Dusk	03 - P.D. only	04 - Sideswipe	222
Y	2017	226 ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST	366252.4375 5030526				-2.209E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection		02 - Non-fatal injury	07 - SMV other	226
Y	2017 2017	227 ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (1) 228 ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (1)	365728.5625 5030212.5 365654.5313 5030150.5				-2.20903E+12 01 - Clear -2.20902E+12 05 - Drifting Snow	01 - Dry 03 - Loose snow	10 - No control 10 - No control	01 - Non intersection 01 - Non intersection	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	227 228
Ý	2017	1567 BAYVIEW RD @ SCOTT ST/ALBERT ST	365520.25 5030070.5				-2.20899E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	1795
Y	2017	1568 BAYVIEW RD @ SCOTT ST/ALBERT ST	365519.9688 5030071.5	-75.72429657	45.40794373		-2.20898E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	07 - Dark	03 - P.D. only	02 - Angle	1796
Y		1569 BAYVIEW RD @ SCOTT ST/ALBERT ST 1570 BAYVIEW RD @ SCOTT ST/ALBERT ST	365520.4688 5030072 365520.0625 5030071.5				-2.20901E+12 01 - Clear -2.20899E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	03 - At intersection	01 - Daylight 07 - Dark	02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 05 - Turning movement	1797
Ϋ́Υ		15/0 BAYVIEW RD @ SCOTT ST/ALBERT ST 1571 BAYVIEW RD @ SCOTT ST/ALBERT ST	365518.5625 5030071.5			1.49007E+12 1.51392E+12	-2.20899E+12 01 - Clear -2.209E+12 01 - Clear	03 - Loose snow	01 - Traffic signal	03 - At intersection	07 - Dark 01 - Daylight	02 - Non-tatal injury 03 - P.D. only	05 - Turning movement 05 - Turning movement	1798 1799
Y	2017	2052 BOOTH ST @ MIDDLE ST	366114.375 5031294	-75.71655273	45.41889191	1.49914E+12	-2.20902E+12 01 - Clear	01 - Dry	02 - Stop sign	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	2854
Y	2017	2053 BOOTH ST @ MIDDLE ST	366115.3125 5031291.5			1.51323E+12	-2.20899E+12 03 - Snow	04 - Slush	02 - Stop sign	03 - At intersection	07 - Dark	03 - P.D. only	05 - Turning movement	2855
v	2017	2054 BOOTH ST @ MIDDLE ST 2059 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366115.5313 5031291.5 366264.5938 5031007.5	-75.71653748 -75.71466827	45.41887283 45.41630173	1.50994E+12 1.49776F±12	-2.209E+12 01 - Clear -2.20901E+12 01 - Clear	01 - Dry 01 - Dry	02 - Stop sign 01 - Traffic signal	02 - Intersection related 02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	2856
Ÿ	2017	2060 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.5625 5031005.5	-75.71466827	45.41628265	1.4937E+12	-2.209E+12 02 - Rain	02 - Wet	01 - Traffic signal	03 - At intersection	01 - Daylight	02 - Non-fatal injury	05 - Turning movement	2861 2862
Y		2061 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366265.5313 5031005.5			1.49914E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related		03 - P.D. only	04 - Sideswipe	2863
Y	2017 2017	2062 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST 2063 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366263.7813 5031006.5 366263.7813 5031006				-2.20899E+12 01 - Clear -2.20901E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 03 - At intersection	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end 05 - Turning movement	2864 2865
Ÿ		2064 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366265.25 5031005.5				-2.20899E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	2866
Y	2017	2065 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366263.9375 5031005.5	-75.7146759	45.41628265	1.50864E+12	-2.20903E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	07 - SMV other	2867
Y	2017 2017	2066 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST 2067 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.5938 5031007.5 366263.25 5031008				-2.209E+12 01 - Clear -2.20902E+12 03 - Snow	01 - Dry 02 - Wet	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 03 - At intersection	05 - Dusk 01 - Daylight	03 - P.D. only 02 - Non-fatal injury	03 - Rear end 02 - Angle	2868 2869
Y	2017	2068 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.5938 5031007.5			1.51107E+12	-2.20902E+12 03 - Show -2.20902E+12 01 - Clear	02 - Wet 01 - Dry	01 - Traffic signal	03 - At Intersection		02 - Non-ratar injury 03 - P.D. only	02 - Angle 02 - Angle	2870
Y	2017	2069 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366262.5938 5031008	-75.71469879	45.41630936	1.51141E+12	-2.20899E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	07 - Dark	03 - P.D. only	05 - Turning movement	2871
Y		2070 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST 2071 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.7188 5031006.5 366264.3438 5031007.5				-2.20902E+12 01 - Clear -2.20899E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal	02 - Intersection related 03 - At intersection	01 - Daylight 07 - Dark	03 - P.D. only 03 - P.D. only	03 - Rear end	2872 2873
Y		2071 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.3438 5031007.5 366264.0313 5031006				-2.20899E+12 01 - Clear -2.20899E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	03 - At intersection 03 - At intersection	07 - Dark 05 - Dusk	03 - P.D. only 03 - P.D. only	02 - Angle 04 - Sideswipe	2873
Y		2073 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366262.5625 5031006.5	-75.71469879	45.41629791	1.48757E+12	-2.20903E+12 01 - Clear	02 - Wet	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	04 - Sideswipe	2875
Y	2017	2074 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST	366264.6563 5031008.5				-2.20902E+12 03 - Snow	05 - Packed snow	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	04 - Sideswipe	2876
v		2075 BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST 2097 BOOTH ST btwn 148 N OF MIDDLE ST/E.B. EDDY S & MIDDLE ST	366264.25 5031006.5 366054.2813 5031376				-2.209E+12 01 - Clear -2.2089F+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 10 - No control	02 - Intersection related 01 - Non intersection	01 - Daylight 07 - Dark	03 - P.D. only 03 - P.D. only	03 - Rear end 07 - SMV other	2877 2899
Y	2017	2101 BOOTH ST btwn FLEET ST & END	366345.75 5030822.5				-2.20899E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection	07 - Dark	03 - P.D. only	04 - Sideswipe	2019
Y	2017	2104 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV	366223.7813 5031111				-2.20899E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection		03 - P.D. only	03 - Rear end	2120
Y	2017 2017	2105 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV 2106 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV		-75.71620941 -75.71556091		1.51288E+12	-2.20906E+12 03 - Snow -2.20903E+12 02 - Rain	05 - Packed snow 02 - Wet	10 - No control 10 - No control	01 - Non intersection 04 - At/near private drive	07 - Dark 01 - Daylight	02 - Non-fatal injury 03 - P.D. only	01 - Approaching 02 - Angle	2124 2128
Ý		2107 BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N		-75.71799469		1.48368E+12	-2.20901E+12 03 - Snow	06 - Ice	10 - No control	01 - Non intersection	01 - Daylight	03 - P.D. only	03 - Rear end	2132
Y		2108 BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N	365991.8125 5031515				-2.20905E+12 02 - Rain	02 - Wet	10 - No control	07 - Overpass or bridge	07 - Dark	03 - P.D. only	07 - SMV other	2133
Y		2349 BRONSON AVE @ COMMISSIONER ST/SLATER ST 2350 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.25 5030948.5 366767.6875 5030950	-75.70822906 -75.70825195			-2.20903E+12 01 - Clear -2.20898E+12 01 - Clear	01 - Dry 02 - Wet	01 - Traffic signal 01 - Traffic signal	03 - At intersection 02 - Intersection related	01 - Daylight 07 - Dark	03 - P.D. only 03 - P.D. only	01 - Approaching 05 - Turning movement	2493 2494
Ý	2017	2351 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366768.9375 5030948.5	-75.70823669	45.41572571	1.50623E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Dark 01 - Daylight	03 - P.D. only	04 - Sideswipe	2495
Y	2017	2352 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366768.7188 5030949	-75.70823669	45.41572952	1.50683E+12	-2.20901E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	06 - SMV unattended vehicle	2496
Y		2353 BRONSON AVE @ COMMISSIONER ST/SLATER ST 2354 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.25 5030949 366768.8125 5030948.5				-2.20901E+12 01 - Clear -2.2089F+12 03 - Snow	01 - Dry 03 - Loose snow	01 - Traffic signal 01 - Traffic signal	03 - At intersection 02 - Intersection related	01 - Daylight 07 - Dark	03 - P.D. only	05 - Turning movement 07 - SMV other	2497 2498
Ÿ	2017	2355 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366770.375 5030948	-75.70821381	45.41572189	1.51176E+12	-2.20902E+12 01 - Clear	02 - Wet	01 - Traffic signal	03 - At intersection	01 - Daylight	02 - Non-fatal injury	02 - Angle	2499
Y		2356 BRONSON AVE @ COMMISSIONER ST/SLATER ST		-75.70822144			-2.20901E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	05 - Turning movement	2500
Y	2017 2017	2357 BRONSON AVE @ COMMISSIONER ST/SLATER ST 2358 BRONSON AVE @ COMMISSIONER ST/SLATER ST	366769.7188 5030948.5 366768.2813 5030948				-2.20903E+12 03 - Snow -2.20899E+12 01 - Clear	03 - Loose snow 01 - Dry	01 - Traffic signal 01 - Traffic signal	03 - At intersection 03 - At intersection	01 - Daylight 07 - Dark	03 - P.D. only 03 - P.D. only	02 - Angle 05 - Turning movement	2501 2502
Y	2017	2359 BRONSON AVE @ COMMISSIONER ST/SLATER ST		-75.70822144		1.48852E+12	-2.20899E+12 01 - Clear -2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	07 - Dark 01 - Daylight	03 - P.D. only	04 - Sideswipe	2502
Y	2017	2360 BRONSON AVE @ COMMISSIONER ST/SLATER ST		-75.70822144			-2.20897E+12 03 - Snow	05 - Packed snow	01 - Traffic signal	02 - Intersection related	07 - Dark	03 - P.D. only	03 - Rear end	2504
Y	2017	2361 BRONSON AVE @ COMMISSIONER ST/SLATER ST 11144 OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366769.0625 5030949 366516.5313 5031314.5	-75.70822906		1.49128E+12	-2.209E+12 02 - Rain	02 - Wet 01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end	2505
Y		11144 OTTAWA RIVER PKWY @ PORTAGE BRIDGE 11145 OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366516.5313 5031314.5 366515.9688 5031314.5			1.49551E+12 1.49491E+12	-2.20899E+12 01 - Clear -2.209E+12 01 - Clear	01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	02 - Angle 99 - Other	11640 11641
Υ	2017	11146 OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366516.2813 5031313.5	-75.71141815	45.41903687	1.49016E+12	-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	02 - Non-fatal injury	03 - Rear end	11642
Y		11147 OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366516.0938 5031313	-75.71141815	45.41903305	1.49111E+12	-2.20898E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	07 - Dark	03 - P.D. only	04 - Sideswipe	11643
Y		11148 OTTAWA RIVER PKWY @ PORTAGE BRIDGE 11149 OTTAWA RIVER PKWY @ PORTAGE BRIDGE	366516.2188 5031314.5 366516.0313 5031316.5				-2.20902E+12 01 - Clear -2.20899E+12 02 - Rain	02 - Wet 02 - Wet	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	11644 11645
Ý	2017	11155 OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST	365156.7188 5030377.5	-75.72890472	45.41073227	1.50096E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	11651
Y		11156 OTTAWA RIVER PKWY @ VIMY PLACE PRIV	365977 5030795.5				-2.20903E+12 01 - Clear	02 - Wet	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	11652
Y		11157 OTTAWA RIVER PKWY @ VIMY PLACE PRIV 11158 OTTAWA RIVER PKWY @ VIMY PLACE PRIV	365975.75 5030796.5 365974.1875 5030798.5			1.48904E+12 1.51444E+12	-2.209E+12 01 - Clear -2.209E+12 01 - Clear	01 - Dry 06 - Ice	01 - Traffic signal 01 - Traffic signal	02 - Intersection related 02 - Intersection related	01 - Daylight 01 - Daylight	03 - P.D. only 02 - Non-fatal injury	03 - Rear end 03 - Rear end	11653 11654
Ÿ		11158 OTTAWA RIVER PRWY @ VIMY PLACE PRIV 11159 OTTAWA RIVER PRWY @ WELLINGTON ST EB	365974.1875 5030798.5 366690.75 5031277.5				-2.20901E+12 01 - Clear -2.20901E+12 01 - Clear	01 - Dry	02 - Stop sign	02 - Intersection related 02 - Intersection related		03 - P.D. only	03 - Rear end	11654
Υ		11163 OTTAWA RIVER PKWY btwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV	365766.3125 5030525.5				-2.20901E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection		03 - P.D. only	07 - SMV other	11659
Y		11164 OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST	366093.75 5030885			1.50657E+12	-2.209E+12 00 - Unknown	01 - Dry	10 - No control	01 - Non intersection	01 - Daylight	03 - P.D. only	03 - Rear end	11660
Ϋ́Υ		11165 OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST 11171 OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE	366227.75 5030980.5 364685.0938 5030331				-2.20898E+12 02 - Rain -2.20901E+12 02 - Rain	02 - Wet 02 - Wet	10 - No control 03 - Yield sign	01 - Non intersection 02 - Intersection related	07 - Dark 01 - Daylight	03 - P.D. only 02 - Non-fatal injury	04 - Sideswipe 07 - SMV other	11661 11667
Υ	2017	11172 OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE	364683.0938 5030331.5	-75.73495483	45.41035843	1.51297E+12	-2.20903E+12 01 - Clear	01 - Dry	03 - Yield sign	03 - At intersection	01 - Daylight	03 - P.D. only	03 - Rear end	11668
Y		11173 OTTAWA RIVER PKWY ONR-EB @ PARKDALE AVE	364692.625 5030334.5				-2.20901E+12 01 - Clear	01 - Dry	10 - No control		01 - Daylight		04 - Sideswipe	11669
Ϋ́Υ	2017 2017	11174 OTTAWA RIVER PKWY WB @ PARKDALE AVE WB ON RAMP 11264 PARKDALE AVE @ SCOTT ST	364638.25 5030469 364950.0313 5029728	-/5./3551178 -75.73162079	45.41159821 45.40490341	1.49128E+12 1.49551F+12	-2.209E+12 01 - Clear -2.209E+12 01 - Clear	01 - Dry 01 - Dry	03 - Yield sign 01 - Traffic signal	02 - Intersection related 03 - At intersection	01 - Daylight 01 - Daylight	03 - P.D. only 03 - P.D. only	04 - Sideswipe 07 - SMV other	11670 11190
			3023/20								uyngnt			

Y	2017	11265 PARKDALE AVE @ SCOTT ST	364950.0313 502972	7 -75.73162079	45.4048996	1.49352E+12	-2.20901E+12 02 - Rain	02 - Wet	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	04 - Sideswipe	11191
Y	2017	11266 PARKDALE AVE @ SCOTT ST	364948.6875 5029726.5	-75.73163605	45.40489197	1.49923E+12	-2.20903E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	03 - Rear end	11192
Y	2017	11267 PARKDALE AVE @ SCOTT ST	364949.7188 5029726	-75.73162842	45.40488434	1.4988E+12	-2.20905E+12 01 - Clear	02 - Wet	01 - Traffic signal	03 - At intersection	07 - Dark	03 - P.D. only	02 - Angle	11193
Y	2017	11268 PARKDALE AVE @ SCOTT ST	364948.4375 502972	7 -75.73164368	45.40489578	1.50173E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	05 - Turning movement	11194
Y	2017	11269 PARKDALE AVE @ SCOTT ST	364950.7188 5029726.5	-75.73161316	45.40489197	1.48791E+12	-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	03 - At intersection	01 - Daylight	03 - P.D. only	02 - Angle	11195
Y	2017	11270 PARKDALE AVE @ SCOTT ST	364950.0313 5029726	-75.73162079	45.40488434	1.4886E+12	-2.20899E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	05 - Dusk	03 - P.D. only	03 - Rear end	11196
Y	2017	11295 PARKDALE AVE btwn TO BE DETERMINED & EMMERSON AVE	364709.4375 5030285	-75.73462677	45.409935	1.49888E+12	-2.209E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection	01 - Daylight	02 - Non-fatal injury	05 - Turning movement	11221
Y	2017	11296 PARKDALE AVE WB OFF RAMP/OTTAWA RIVER PKWY btwn OTTAWA RIVER PKWY & PA	364694.125 5030582	-75.73478699	45.41261292	1.49007E+12	-2.20903E+12 03 - Snow	05 - Packed snow	10 - No control	01 - Non intersection	01 - Daylight	03 - P.D. only	03 - Rear end	11222
Y	2017	11297 PARKDALE AVE WB ON RAMP/OTTAWA RIVER PKWY btwn PARKDALE AVE & OTTAWA R	364662 5030406.5	-75.73521423	45.41103363	1.49396E+12	-2.209E+12 02 - Rain	02 - Wet	10 - No control	01 - Non intersection	01 - Daylight	02 - Non-fatal injury	03 - Rear end	11223
Y	2017	12641 SCOTT ST @ STIRLING AVE	365212.5938 5029884	-75.7282486	45.40628815	1.51366E+12	-2.20903E+12 03 - Snow	03 - Loose snow	02 - Stop sign	02 - Intersection related	03 - Dawn	03 - P.D. only	02 - Angle	12802
Y	2017	12645 SCOTT ST btwn PARKDALE AVE & PINEHURST AVE	364974.7813 5029740.5	-75.73130035	45.40501785	1.4956E+12	-2.20901E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection	01 - Daylight	03 - P.D. only	03 - Rear end	12806
Y	2017	12646 SCOTT ST btwn PINHEY ST & MERTON ST	365309.3125 5029944.5	-75.727005	45.4068222	1.5096E+12	-2.209E+12 01 - Clear	02 - Wet	10 - No control	01 - Non intersection	01 - Daylight	02 - Non-fatal injury	03 - Rear end	12807
Y	2017	12647 SCOTT ST btwn STIRLING AVE & PINHEY ST	365264.0938 5029915	-75.72758484	45.40656281	1.51089E+12	-2.209E+12 01 - Clear	01 - Dry	10 - No control	01 - Non intersection	07 - Dark	03 - P.D. only	04 - Sideswipe	12808
Y	2017	13996 WELLINGTON ST @ LETT ST	366355.2813 5031112	2 -75.71350098	45.41723633	1.49638E+12	-2.209E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	03 - P.D. only	03 - Rear end	13828
Y	2017	13997 WELLINGTON ST @ LETT ST	366354.125 5031111.5	-75.71351624	45.4172287	1.50372E+12	-2.20902E+12 01 - Clear	01 - Dry	01 - Traffic signal	02 - Intersection related	01 - Daylight	02 - Non-fatal injury	03 - Rear end	13829
Y	2017	14007 WELLINGTON ST btwn OTTAWA RIVER PKWY & TO BE DETERMINED	366357.25 5031113.5	-75.71347046	45.41725159	1.49041E+12	-2.20898E+12 01 - Clear	02 - Wet	10 - No control	01 - Non intersection	07 - Dark	03 - P.D. only	07 - SMV other	14269

9	TUDY AREA YEAR DATE	ANOM_ID T		GEO_ID ACCIDENT_LOCATION	CLASS_OF_ACCIDENT		ENVIRONMENT	LIGHT	ROAD_SURFACE_CONDITION		TRAFFIC_CONTROL_CONDITION NO_OF_P		Y LONGITUDE LATITUDE	
,			8:35:00 AM ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related		04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		428.3448 5030626.567 -75.71262535 45.41286137	
,	2018 2018/04	04/10 00:00:00+00 18-3579	7:21:00 AM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 3664	427.6479 5030625.87 -75.71263434 45.41285516	3514
,			6:06:00 AM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection		05 - Turning movement	01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning		428.6235 5030626.715 -75.71262177 45.41286268	
,			9:51:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	02 - Non-fatal injury		01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning		428.6933 5030626.566 -75.7126209 45.41286133	
,			4:30:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		428.3247 5030626.736 -75.71262558 45.41286289	
,			5:52:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection		05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		428.6927 5030626.567 -75.7126209 45.41286134	
,			12:45:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight		01 - Traffic signal	02 - Not functioning		428.6934 5030626.566 -75.7126209 45.41286133	
,			2:58:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related		03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		428.7812 5030626.688 -75.71261976 45.41286242	
,			5:00:00 PM ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection		05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		6428.693 5030626.567 -75.7126209 45.41286134	
١			7:47:00 AM ALBERT ST @ BOOTH ST (0002162)		02 - Non-fatal injury		02 - Rain	01 - Daylight		01 - Traffic signal	01 - Functioning		428.6931 5030626.567 -75.7126209 45.41286134	
,			4:02:00 PM ALBERT ST @ BRONSON AVE (0002160)		03 - P.D. only	04 - Sideswipe	02 - Rain	01 - Daylight		01 - Traffic signal	01 - Functioning		739.7045 5031016.575 -75.70859873 45.41634306	
,			6:30:00 PM ALBERT ST @ EMPRESS AVE (0010851)	10851 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	00 - Unknown		576.8856 5030720.754 -75.71071594 45.41369574	
,			4:05:00 PM ALBERT ST @ PERKINS ST (0002220)	2220 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		02 - Stop sign	01 - Functioning		535.6983 5030695.574 -75.71124527 45.41347281	
,			7:41:00 PM ALBERT ST @ PRESTON ST (0002217)	2217 03 - At intersection	03 - P.D. only	02 - Angle	03 - Snow		03 - Loose snow	01 - Traffic signal	01 - Functioning		160.3486 5030465.965 -75.71606899 45.41143986	
,			6:41:00 PM ALBERT ST @ PRESTON ST (0002217)			07 - SMV other	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		160.6974 5030464.571 -75.7160647 45.41142729	
,			1:24:00 PM ALBERT ST @ PRESTON ST (0002217)	2217 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		160.7063 5030464.569 -75.71606459 45.41142728	
,			4:51:00 PM ALBERT ST @ PRESTON ST (0002217)	2217 03 - At intersection	03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		160.6781 5030464.577 -75.71606495 45.41142734	
,			6:15:00 PM ALBERT ST @ PRESTON ST (0002217)	2217 03 - At intersection		02 - Angle	01 - Clear	07 - Dark		01 - Traffic signal	01 - Functioning		160.6966 5030464.571 -75.71606471 45.41142729	
,			2:35:00 PM ALBERT ST @ PRESTON ST (0002217)	2217 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		160.7182 5030464.54 -75.71606444 45.41142701	
,			I0:16:00 AM ALBERT ST btwn COMMISSIONER ST & BRONSON AVE (3ZA337)	3ZA337 01 - Non intersection	03 - P.D. only	04 - Sideswipe	02 - Rain	01 - Daylight		10 - No control			66735.93 5031011.558 -75.70864758 45.41629825	
,			1:48:00 AM ALBERT ST btwn Continuation of ALBERT ST & BOOTH ST (_3ZA2G2)	3ZA2G2	03 - P.D. only	03 - Rear end	01 - Clear		01 - Dry	10 - No control			421.7915 5030622.282 -75.7127096 45.41282339	
,			7:09:00 PM ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST (3ZA33O)	3ZA330 01 - Non intersection	03 - P.D. only	99 - Other	01 - Clear		01 - Dry	10 - No control			6228.677 5030508.138 -75.71519086 45.41181333	
,			3:33:00 AM ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (2) (3ZA32WB)	3ZA32WB 01 - Non intersection	03 - P.D. only	07 - SMV other	01 - Clear		02 - Wet	10 - No control			912.9405 5030324.138 -75.71924712 45.41018539	
,			4:30:00 PM ALBERT ST btwn SCOTT ST & CITY CENTRE AVE (2) (3ZA32WB)	3ZA32WB 01 - Non intersection	03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight		10 - No control			941.5853 5030338.003 -75.71887948 45.41030764	
,			11:22:00 PM BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 03 - At intersection	03 - P.D. only	05 - Turning movement	01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning		519.0769 5030071.525 -75.72430961 45.40794672	
,			1:48:00 PM BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 03 - At intersection	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		519.0369 5030071.564 -75.72431011 45.40794708	
,			12:42:00 PM BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		519.1445 5030071.453 -75.72430875 45.40794607	
,			8:54:00 AM BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 03 - At intersection	03 - P.D. only	05 - Turning movement	02 - Rain	01 - Daylight		01 - Traffic signal	01 - Functioning		519.0372 5030071.565 -75.72431011 45.40794709	
,			3:22:00 PM BOOTH ST @ 148 N OF MIDDLE ST/E.B.EDDY S (0012189)	12189 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		035.5926 5031416.536 -75.71754458 45.42000377	
,			8:10:00 PM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 03 - At intersection	03 - P.D. only	02 - Angle	03 - Snow		05 - Packed snow	01 - Traffic signal	01 - Functioning		263.2117 5031005.991 -75.7146876 45.41628984	
,		04/23 00:00:00+00 18-3931	6:20:00 PM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		264.2571 5031006.688 -75.71467416 45.41629603	
,			5:57:00 PM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 02 - Intersection related	02 - Non-fatal injury	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		264.6054 5031007.385 -75.71466962 45.41630226	
,	2018 2018/07	07/04 00:00:00+00 18-6169 1	11:43:00 AM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 03 - At intersection	03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		6264.301 5031006.754 -75.71467359 45.41629661	
,			6:39:00 AM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow		03 - Loose snow	01 - Traffic signal	01 - Functioning		264.2082 5031006.528 -75.7146748 45.41629459	
,			L0:32:00 PM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 03 - At intersection	03 - P.D. only	05 - Turning movement	03 - Snow		05 - Packed snow	01 - Traffic signal	01 - Functioning		6264.455 5031006.711 -75.71467163 45.41629621	
,			3:02:00 AM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 03 - At intersection	03 - P.D. only	07 - SMV other	03 - Snow		04 - Slush	01 - Traffic signal	01 - Functioning		6264.257 5031006.663 -75.71467416 45.4162958	
,			5:01:00 PM BOOTH ST @ OTTAWA RIVER PKWY/WELLINGTON ST (0012252)	12252 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal	01 - Functioning	0 3663	264.2599 5031006.689 -75.71467412 45.41629603	3 12609
,			5:30:00 AM BOOTH ST btwn 148 N OF MIDDLE ST/E.B. EDDY S & MIDDLE ST (3ZBPMPC)	3ZBPMPC 07 - Overpass or bridge	03 - P.D. only	07 - SMV other	02 - Rain		02 - Wet	10 - No control			075.1822 5031344.406 -75.71704773 45.4193513	
,			I0:00:00 AM BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV (_SVHI3U)	SVHJ3U 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		10 - No control			138.2042 5031259.619 -75.71625309 45.41858289	
,			I0:30:00 AM BOOTH ST btwn PROVINCIAL BOUNDARY & 208 N OF MIDDLE ST/E.B. EDDY N (3ZBPMPA)	3ZBPMPA 01 - Non intersection	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		10 - No control			001.6988 5031489.73 -75.71796854 45.42066532	
,			7:13:00 AM BOOTH ST btwn VIMY PLACE PRIV & OTTAWA RIVER PKWY (SVHJ2X)	SVHJ2X 01 - Non intersection	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		10 - No control			256.8775 5031025.669 -75.71476607 45.41646746	
,			7:48:00 PM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related	03 - P.D. only	04 - Sideswipe	02 - Rain	07 - Dark		01 - Traffic signal	01 - Functioning		769.0538 5030949.961 -75.70823214 45.41574109	
,			8:40:00 AM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow		03 - Loose snow	01 - Traffic signal	01 - Functioning		768.7058 5030948.568 -75.70823677 45.41572858	
,			9:51:00 AM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related	03 - P.D. only	04 - Sideswipe	03 - Snow		03 - Loose snow	01 - Traffic signal	01 - Functioning		768.7054 5030948.568 -75.70823677 45.41572858	
,			4:30:00 PM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		768.6969 5030948.412 -75.7082369 45.41572718	
,			6:31:00 PM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		768.7054 5030948.568 -75.70823677 45.41572858	
,			2:44:00 AM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection	02 - Non-fatal injury		01 - Clear	07 - Dark		01 - Traffic signal	01 - Functioning		768.6848 5030948.633 -75.70823703 45.41572917	
,			12:42:00 PM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection		05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		768.6369 5030948.822 -75.70823761 45.41573087	
,			11:20:00 AM BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		768.6327 5030948.555 -75.7082377 45.41572847	
,			12:30:00 PM OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	5099 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		5156.696 5030377.639 -75.72890114 45.41073241	
,			2:00:00 PM OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	5099 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow	01 - Daylight		01 - Traffic signal	01 - Functioning		156.8347 5030377.421 -75.7288994 45.41073043	
,			11:30:00 PM OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	5099 03 - At intersection	03 - P.D. only	05 - Turning movement	01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning		156.7038 5030377.572 -75.72890105 45.41073181	
,			6:45:00 AM OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	5099 03 - At intersection	01 - Fatal injury	02 - Angle	99 - Other	07 - Dark	01 - Dry	01 - Traffic signal	01 - Functioning		156.7032 5030377.572 -75.72890106 45.41073181	
,			7:35:00 AM OTTAWA RIVER PKWY blwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV (5VHJ2QA)	SVHJ2QA 01 - Non intersection		07 - SMV other	00 - Unknown		06 - Ice	10 - No control			913.5887 5030742.975 -75.71918691 45.41395392	
,			4:00:00 PM OTTAWA RIVER PKWY blwn TRANSITWAY - TIME POINT & VIMY PLACE PRIV (SVHJ2QA)	SVHJ2QA 01 - Non intersection	03 - P.D. only	04 - Sideswipe	00 - Unknown		00 - Unknown	10 - No control			650.0956 5030463.838 -75.72258756 45.41146528	
,			4:20:00 PM OTTAWA RIVER PKWY blwn VIMY PLACE PRIV & BOOTH ST (_SVHJ2QB)	SVHJ2QB 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight	01 - Dry	10 - No control			6171.553 5030939.441 -75.7158669 45.41569909	
,			3:35:00 PM OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST (5VHJ2QB)	SVHJ2QB 01 - Non intersection	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		10 - No control			003.7567 5030818.227 -75.71802565 45.41462314	
,			6:18:00 PM OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST (_SVHJ2QB)	SVHJ2QB 01 - Non intersection	03 - P.D. only	03 - Rear end	02 - Rain	05 - Dusk		10 - No control			201.7542 5030961.596 -75.7154783 45.41589578	
,			12:45:00 PM OTTAWA RIVER PKWY EB btwn RAMP & SLIDELL ST (_3ZA24L)	3ZA24L 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		10 - No control			841.2972 5030362.706 -75.73293204 45.41062517	
,			12:48:00 PM OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE (0011241)	11241 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		03 - Yield sign	01 - Functioning		684.0285 5030331.417 -75.73494487 45.41035712	
,			6:45:00 AM OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE (0011241)	11241 02 - Intersection related		03 - Rear end	01 - Clear		01 - Dry	03 - Yield sign	01 - Functioning		4684.068 5030331.424 -75.73494437 45.41035718	
,			5:30:00 PM OTTAWA RIVER PKWY WB btwn RAMP & RIVER ST (_3ZA24N)	3ZA24N 01 - Non intersection	02 - Non-fatal injury		01 - Clear	01 - Daylight		10 - No control			861.6894 5030385.3 -75.73266879 45.41082672	
,			6:50:00 AM OTTAWA RIVER PKWY WB btwn RAMP & RIVER ST (3ZA24N)	3ZA24N 01 - Non intersection	03 - P.D. only	03 - Rear end	03 - Snow	01 - Daylight		10 - No control			947.2127 5030355.422 -75.73157991 45.41055055	
,			3:05:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related		04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.7034 5029726.559 -75.73162481 45.40489197	
)			5:56:00 PM PARKDALE AVE @ SCOTT ST (0002213)		03 - P.D. only	03 - Rear end	02 - Rain	01 - Daylight		01 - Traffic signal	01 - Functioning		4950.052 5029726.56 -75.73162036 45.40489194	
,			7:46:00 AM PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.7034 5029726.56 -75.73162481 45.40489197	
,			8:45:00 AM PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.7034 5029726.561 -75.73162481 45.40489198	
,			1:55:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 03 - At intersection		05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.7032 5029726.56 -75.73162482 45.40489197	
,			2:43:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 03 - At intersection		05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.6268 5029726.598 -75.73162579 45.40489232	
,			5:07:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning		949.7347 5029726.48 -75.73162442 45.40489125	
,			6:30:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related		03 - Rear end	01 - Clear	07 - Dark		01 - Traffic signal	01 - Functioning		949.4987 5029726.643 -75.73162742 45.40489274	
,			6:35:00 PM PARKDALE AVE @ SCOTT ST (0002213)	2213 03 - At intersection	03 - P.D. only	01 - Approaching	01 - Clear		02 - Wet	01 - Traffic signal	01 - Functioning		64949.65 5029726.675 -75.73162548 45.40489301	
,			2:45:00 PM PARKDALE AVE btwn BURNSIDE AVE & LYNDALE AVE (3ZA32H)	3ZA32H 04 - At/near private drive		02 - Angle	01 - Clear	01 - Daylight		10 - No control			792.9333 5030088.056 -75.73358329 45.40815809	
,		03/20 00:00:00+00 18-3141	4:15:00 PM PARKDALE AVE btwn BURNSIDE AVE & LYNDALE AVE (3ZA32H)	3ZA32H 01 - Non intersection		03 - Rear end	01 - Clear	01 - Daylight		10 - No control			790.7691 5030094.278 -75.73361018 45.40821426	
,			12:00:00 PM PARKDALE AVE btwn EMMERSON AVE & COLOMBINE DRWY (3ZA326)	3ZA326 01 - Non intersection	03 - P.D. only	06 - SMV unattended vehicle			03 - Loose snow	10 - No control			725.5097 5030246.092 -75.73442535 45.40958583	
,			7:50:00 AM PARKDALE AVE btwn EMMERSON AVE & COLOMBINE DRWY (_3ZA326)	3ZA326	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		10 - No control			726.8039 5030243.532 -75.73440912 45.40956269	
,			2:30:00 PM WELLINGTON ST btwn BOOTH ST & END (3ZA2GX)	3ZA2GX 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		10 - No control			503.3738 5030787.431 -75.71164672 45.41430217	
,			4:53:00 PM WELLINGTON ST btwn OTTAWA RIVER PKWY & TO BE DETERMINED (SVHJWJ)	SVHJWJ 01 - Non intersection	03 - P.D. only	06 - SMV unattended vehicle	01 - Clear	01 - Daylight		10 - No control			345.6769 5031093.295 -75.71362314 45.41706813	
,			6:52:00 AM WELLINGTON ST btwn TURN LANE & Continuation of WELLINGTON ST (_3ZBPOI)	3ZBPOI 01 - Non intersection	03 - P.D. only	03 - Rear end	02 - Rain	07 - Dark		10 - No control			685.2788 5031294.64 -75.70925913 45.41884983	
,			9:02:00 AM WELLINGTON ST btwn TURN LANE & TO BE DETERMINED (3ZA2L9)	3ZA2L9 07 - Overpass or bridge		03 - Rear end	03 - Snow	01 - Daylight		10 - No control			611.6676 5031284.853 -75.71020084 45.41876827	
,	2018 2018/09	09/18 00:00:00+00 18-8446	6:03:00 PM WELLINGTON ST btwn TURN LANE & TO BE DETERMINED (3ZA2L9)	3ZA2L9 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	U1 - Daylight	00 - Unknown	10 - No control		0 3666	687.0988 5031277.16 -75.70923808 45.41869238	8849

Study Are	a Year Accident Date Accident Time Location	Geo ID Accident Location	Classification	Initial Impact Type	Environment	Light	Road Surface	Traffic Control	Traffic_Control_Condition Number_of_Pedestrians	X Y Latitude Longitude FID
Y	2019 2019/07/19 10:56:00+00 2019/07/19 10:56:00+00 105 S OF COMMISSIONER ST @ ALBERT ST (0016880)	16880 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight	01 - Dry	13 - MPS	01 - Functioning	0 366666.2747 5030879.415 45.41511543 -75.70955407 7631
Υ	2019 2019/01/11 14:25:00+00 2019/01/11 14:25:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	0 366428.7192 5030626.722 45.41286273 -75.71262055 815
Y	2019 2019/01/14 16:14:00+00 2019/01/14 16:14:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/01/29 19:45:00+00 2019/01/29 19:45:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection 2162 02 - Intersection related	03 - P.D. only 03 - P.D. only	05 - Turning movement 07 - SMV other	01 - Clear 03 - Snow	01 - Daylight 07 - Dark	02 - Wet 04 - Slush	01 - Traffic signal 01 - Traffic signal		0 366428.7517 5030626.628 45.41286188 -75.71262014 963 0 366428.7587 5030626.706 45.41286258 -75.71262004 1826
Ý	2019 2019/02/25 12:36:00+00 2019/02/25 12:36:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 366428.6932 5030626.567 45.41286134 -75.7126209 2049
Υ	2019 2019/02/14 19:30:00+00 2019/02/14 19:30:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	05 - Turning movement	03 - Snow		04 - Slush	01 - Traffic signal	01 - Functioning	0 366428.6818 5030626.696 45.4128625 -75.71262103 2396
Y	2019 2019/02/11 14:03:00+00 2019/02/11 14:03:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/03/13 20:22:00+00 2019/03/13 20:22:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection 2162 02 - Intersection related	02 - Non-fatal injury 03 - P.D. only	02 - Angle 04 - Sideswipe	01 - Clear 03 - Snow		06 - Ice 03 - Loose snow	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366428.6929 5030626.567 45.41286134 -75.7126209 2715 0 366428.5996 5030626.621 45.41286184 -75.71262209 3743
Ý	2019 2019/03/13 17:15:00+00 2019/03/13 17:15:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow		04 - Slush	01 - Traffic signal	01 - Functioning	0 366428.6928 5030626.567 45.41286134 -75.7126209 3976
Υ	2019 2019/04/10 07:00:00+00 2019/04/10 07:00:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 366428.6596 5030626.503 45.41286076 -75.71262134 4084
Y	2019 2019/04/06 22:18:00+00 2019/04/06 22:18:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/04/12 21:40:00+00 2019/04/12 21:40:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection 2162 02 - Intersection related	02 - Non-fatal injury 03 - P.D. only	07 - SMV other 03 - Rear end	01 - Clear 01 - Clear		01 - Dry 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	1 366428.793 5030626.518 45.41286089 -75.71261963 4457 0 366428.7026 5030626.698 45.41286252 -75.71262076 4664
Ý	2019 2019/05/21 08:30:00+00 2019/05/21 08:30:00+00 ALBERT ST @ BOOTH ST (0002102)	2162 03 - At intersection	02 - Non-fatal injury		01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning	0 366428.7056 5030626.587 45.41286152 -75.71262074 5533
Υ	2019 2019/05/14 09:33:00+00 2019/05/14 09:33:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	04 - Sideswipe	02 - Rain	01 - Daylight	02 - Wet	01 - Traffic signal	01 - Functioning	0 366428.6454 5030626.755 45.41286303 -75.71262148 5620 0 366428.7653 5030626.475 45.4128605 -75.71261999 5782
Y	2019 2019/05/17 14:45:00+00 2019/05/17 14:45:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/07/14 14:53:00+00 2019/07/14 14:53:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related 2162 03 - At intersection	03 - P.D. only 02 - Non-fatal injury	03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366428.7653 5030626.475 45.4128605 -75.71261999 5782 0 366428.6593 5030626.515 45.41286088 -75.71262134 7099
Y	2019 2019/07/14 14:53:00+00 2019/07/14 14:53:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/08/30 13:00:00+00 2019/08/30 13:00:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection 2162 03 - At intersection	03 - P.D. only	02 - Angle 02 - Angle	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366428.5725 5030626.691 45.41286286 -75.71262242 9850
Y	2019 2019/10/16 15:08:00+00 2019/10/16 15:08:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight 07 - Dark	01 - Dry	01 - Traffic signal	00 - Unknown	0 366428.6226 5030626.641 45.41286201 -75.71262179 10674
Υ	2019 2019/11/20 23:34:00+00 2019/11/20 23:34:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	07 - Dark	02 - Wet	01 - Traffic signal	01 - Functioning	0 366428.8509 5030626.299 45.41285892 -75.71261892 12312
Y	2019 2019/11/19 09:43:00+00 2019/11/19 09:43:00+00 ALBERT ST @ BOOTH ST (0002162) 2019 2019/11/14 09:00:00+00 2019/11/14 09:00:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection 2162 02 - Intersection related	02 - Non-fatal injury 03 - P.D. only	07 - SMV other 03 - Rear end	01 - Clear 03 - Snow	01 - Daylight 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	1 366428.6845 5030626.586 45.41286151 -75.71262101 12430 0 366430.5388 5030628.093 45.41287491 -75.71259713 12967
Ý	2019 2019/12/14 17:23:00+00 2019/12/14 17:23:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 03 - At intersection	02 - Non-fatal injury		02 - Rain	07 - Dark	02 - Wet	01 - Traffic signal	01 - Functioning	1 366428.8187 5030626.661 45.41286218 -75.71261928 13679
Υ	2019 2019/12/05 19:00:00+00 2019/12/05 19:00:00+00 ALBERT ST @ BOOTH ST (0002162)	2162 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear		03 - Loose snow	01 - Traffic signal	01 - Functioning	0 366428.5005 5030626.394 45.4128598 -75.71262338 13922
Y	2019 2019/02/12 12:23:00+00 2019/02/12 12:23:00+00 ALBERT ST @ BRONSON AVE (0002160) 2019 2019/04/14 12:58:00+00 2019/04/14 12:58:00+00 ALBERT ST @ BRONSON AVE (0002160)	2160 02 - Intersection related 2160 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366739.8248 5031016.425 45.41634169 -75.70859721 2758 0 366739.6031 5031016.897 45.41634597 -75.70859998 4781
Ý	2019 2019/07/14 16:10:00+00 2019/07/14 16:10:00+00 ALBERT ST @ BRONSON AVE (0002160)	2160 02 - Intersection related	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 366739.744 5031016.629 45.41634355 -75.70859832 7104
Υ	2019 2019/02/12 18:00:00+00 2019/02/12 18:00:00+00 ALBERT ST @ CITY CENTRE AVE (0006346)	6346 03 - At intersection	03 - P.D. only	02 - Angle	03 - Snow	05 - Dusk	03 - Loose snow	01 - Traffic signal	01 - Functioning	0 365955.5448 5030344.518 45.41036504 -75.71870035 2774
Y	2019 2019/08/06 11:22:00+00 2019/08/06 11:22:00+00 ALBERT ST @ CITY CENTRE AVE (0006346) 2019 2019/08/29 15:49:00+00 2019/08/29 15:49:00+00 ALBERT ST @ CITY CENTRE AVE (0006346)	6346 02 - Intersection related 6346 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 365955.4853 5030344.657 45.4103663 -75.71870109 8688 0 365955.6605 5030344.664 45.41036635 -75.71869886 9220
Y	2019 2019/08/29 15:49:00+00 2019/08/29 15:49:00+00 ALBERT ST @ CHIY CENTRE AVE (0006346) 2019 2019/06/13 20:01:00+00 2019/06/13 20:01:00+00 ALBERT ST @ COMMISSIONER ST (0002171)	2171 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 07 - SMV other	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		02 - Stop sign	01 - Functioning 01 - Functioning	0 365955.6605 5030344.664 45.41036635 -/5./1869886 9220 0 366713.5301 5030971.623 45.41594091 -75.70893877 6465
Y	2019 2019/06/12 22:03:00+00 2019/06/12 22:03:00+00 ALBERT ST @ COMMISSIONER ST (0002171)	2171 03 - At intersection	03 - P.D. only	02 - Angle	01 - Clear	07 - Dark	01 - Dry	02 - Stop sign	01 - Functioning	0 366713.6186 5030971.612 45.4159408 -75.70893764 6688
Υ	2019 2019/04/26 20:11:00+00 2019/04/26 20:11:00+00 ALBERT ST @ EMPRESS AVE (0010851)	10851 02 - Intersection related	03 - P.D. only	04 - Sideswipe	02 - Rain	05 - Dusk	02 - Wet	01 - Traffic signal	01 - Functioning	0 366576.5109 5030720.742 45.41369567 -75.71072073 4166
Y	2019 2019/05/10 15:36:00+00 2019/05/10 15:36:00+00 ALBERT ST @ EMPRESS AVE (0010851) 2019 2019/06/05 15:13:00+00 2019/06/05 15:13:00+00 ALBERT ST @ EMPRESS AVE (0010851)	10851 02 - Intersection related 10851 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366576.792 5030720.592 45.41369429 -75.71071716 5079 0 366576.8112 5030720.473 45.41369322 -75.71071693 6235
Ý	2019 2019/10/15 09:38:00+00 2019/10/15 09:38:00+00 ALBERT ST @ EMPRESS AVE (0010851)	10851 03 - At intersection	02 - Non-fatal injury		01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	1 366576.6376 5030720.594 45.41369432 -75.71071913 10345
Υ	2019 2019/07/26 10:24:00+00 2019/07/26 10:24:00+00 ALBERT ST @ PERKINS ST (0002220)	2220 02 - Intersection related	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight	01 - Dry	02 - Stop sign	01 - Functioning	0 366535.7477 5030695.577 45.41347283 -75.71124464 8924
Y	2019 2019/01/03 22:00:00+00 2019/01/03 22:00:00+00 ALBERT ST @ PRESTON ST (0002217) 2019 2019/01/29 16:14:00+00 2019/01/29 16:14:00+00 ALBERT ST @ PRESTON ST (0002217)		03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	03 - Snow 03 - Snow	07 - Dark 01 - Davlight	02 - Wet	01 - Traffic signal	01 - Functioning 01 - Functioning	0 366160.7515 5030464.526 45.41142689 -75.71606402 328 0 366160.5651 5030464.683 45.41142831 -75.71606638 1634
Y	2019/01/29 16:14:00+00 2019/01/29 16:14:00+00 ALBERT ST @ PRESTON ST (0002217) 2019 2019/02/18 16:57:00+00 2019/02/18 16:57:00+00 ALBERT ST @ PRESTON ST (0002217)	2217 02 - Intersection related 2217 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	03 - Snow 01 - Clear		04 - Slush 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366160.3794 5030464.573 45.41142831 -75.71606877 2279
Υ	2019 2019/02/15 19:58:00+00 2019/02/15 19:58:00+00 ALBERT ST @ PRESTON ST (0002217)	2217 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	07 - Dark	01 - Dry	01 - Traffic signal	01 - Functioning	0 366160.5798 5030464.589 45.41142746 -75.7160662 2852
Y	2019 2019/04/06 15:29:00+00 2019/04/06 15:29:00+00 ALBERT ST @ PRESTON ST (0002217) 2019 2019/07/03 11:30:00+00 2019/07/03 11:30:00+00 ALBERT ST @ PRESTON ST (0002217)	2217 02 - Intersection related 2217 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal	01 - Functioning	0 366160.7718 5030464.609 45.41142762 -75.71606375 4451 0 366160.7424 5030464.521 45.41142684 -75.71606413 7451
Y	2019 2019/07/03 11:30:00+00 2019/07/03 11:30:00+00 ALBERT ST (by PRESTON ST (0002217) 2019 2019/03/13 18:43:00+00 2019/03/13 18:43:00+00 ALBERT ST btwn BRICKHILL ST & COMMISSIONER ST ( 3ZA2GZ)	3ZAZGZ 01 - Non intersection	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	01 - Clear 03 - Snow		01 - Dry 04 - Slush	01 - Traffic signal 10 - No control	01 - Functioning	0 366710.4099 5030965.335 45.41588461 -75.70897942 3992
Y	2019 2019/04/05 01:04:00+00 2019/04/05 01:04:00+00 ALBERT ST btwn CITY CENTRE AVE & PRESTON ST (3ZA32G)	3ZA32G 01 - Non intersection	03 - P.D. only	07 - SMV other	01 - Clear		01 - Dry	10 - No control		0 365978.6298 5030356.251 45.4104686 -75.718404 4402
Y	2019 2019/02/21 17:10:00+00 2019/02/21 17:10:00+00 ALBERT ST btwn Continuation of ALBERT ST & BOOTH ST (3ZA2G2)	3ZA2G2 01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear		01 - Dry	10 - No control		0 366353.5011 5030579.711 45.41244636 -75.71358733 2168
Y	2019 2019/11/30 11:00:00+00 2019/11/30 11:00:00+00 ALBERT ST btwn PRESTON ST & Continuation of ALBERT ST (3ZA330 2019 2019/01/16 17:40:00+00 2019/01/16 17:40:00+00 ALBERT ST btwn SLATER ST & BRICKHILL ST (3ZBOG9)	0)3ZA330	03 - P.D. only 03 - P.D. only	04 - Sideswipe 03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 07 - Dark	01 - Dry 02 - Wet	10 - No control 10 - No control		0 366238.912 5030514.909 45.41187336 -75.71505926 12527 0 366613.1856 5030775.724 45.41418714 -75.71024531 663
Ý	2019 2019/04/23 18:19:00+00 2019/04/23 18:19:00+00 BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 03 - At intersection	02 - Non-fatal injury	05 - Turning movement	02 - Rain	01 - Daylight		01 - Traffic signal	01 - Functioning	0 365518.9208 5030071.735 45.40794863 -75.72431157 4912
Υ	2019 2019/12/03 10:35:00+00 2019/12/03 10:35:00+00 BAYVIEW RD @ SCOTT ST/ALBERT ST (0005646)	5646 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	0 365519.0592 5030071.462 45.40794616 -75.72430984 13808
Y	2019 2019/01/01 14:10:00+00 2019/01/01 14:10:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (001: 2019 2019 2019 2019 2019 2019 2019 2019	12: 12252 03 - At intersection 12: 12252 02 - Intersection related	03 - P.D. only 02 - Non-fatal injury	02 - Angle 03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366264.2684 5031006.876 45.41629772 -75.71467399 131 0 366264.1644 5031006.65 45.41629569 -75.71467535 3392
Y	2019 2019/03/21 09:20:00+00 2019/03/21 09:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PRWY/WELLINGTON ST (0012 2019 2019/05/28 06:20:00+00 2019/05/28 06:20:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PRWY/WELLINGTON ST (0012	12: 12252 U2 - Intersection related 12: 12252 02 - Intersection related	02 - Non-tatai injury 03 - P.D. only	04 - Sideswipe	01 - Clear 02 - Rain	01 - Daylight 01 - Daylight		01 - Traffic signal	01 - Functioning 01 - Functioning	0 366264.2234 5031006.83 45.41629734 -75.71467457 5744
Y	2019 2019/08/21 17:45:00+00 2019/08/21 17:45:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012	12: 12252 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 366264.3879 5031006.732 45.41629641 -75.71467248 8030
Υ	2019 2019/08/22 19:12:00+00 2019/08/22 19:12:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012)		03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	0 366264.2384 5031006.753 45.41629661 -75.71467439 8076
Y	2019 2019/08/27 08:45:00+00 2019/08/27 08:45:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019/08/24 07:29:00+00 2019/08/24 07:29:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019/08/24 07:29:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019/08/24 07:29:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019 2019 2019 2019 2019 2019	12: 12252 02 - Intersection related 12: 12252 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366264.1485 5031006.726 45.41629637 -75.71467554 9154 0 366264.2451 5031006.678 45.41629593 -75.71467431 9919
Ý	2019 2019/08/27 06:50:00+00 2019/08/27 06:50:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012		03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	0 366264.2078 5031006.748 45.41629657 -75.71467478 9997
Υ	2019 2019/08/27 08:38:00+00 2019/08/27 08:38:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012		03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 366264.209 5031006.748 45.41629657 -75.71467476 10000
Y	2019 2019/10/19 06:56:00+00 2019/10/19 06:56:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019/06/04 10:30:00+00 2019/06/04 10:30:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012	12: 12252 03 - At intersection 12: 12252 02 - Intersection related	02 - Non-fatal injury 03 - P.D. only	02 - Angle 03 - Rear end	01 - Clear 01 - Clear	03 - Dawn 01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning 01 - Functioning	0 366264.0645 5031006.769 45.41629677 -75.71467661 11676 0 366264.1332 5031006.688 45.41629604 -75.71467574 14001
Y	2019 2019/06/04 10:30:00+00 2019/06/04 10:30:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PRWY/WELLINGTON ST (0012 2019 2019/11/27 14:00:00+00 2019/11/27 14:00:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PRWY/WELLINGTON ST (0012		03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366264.3611 5031006.876 45.4162977 -75.7146728 14010
Ý	2019 2019/04/14 17:10:00+00 2019/04/14 17:10:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012	12: 12252 02 - Intersection related	03 - P.D. only	03 - Rear end	02 - Rain	01 - Daylight	02 - Wet	01 - Traffic signal	01 - Functioning	0 366264.2745 5031006.601 45.41629524 -75.71467395 14285
Υ	2019 2019/12/30 17:51:00+00 2019/12/30 17:51:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012)	22: 12252 02 - Intersection related	02 - Non-fatal injury	03 - Rear end	04 - Freezing Rain	07 - Dark	04 - Slush	01 - Traffic signal	01 - Functioning	0 366264.9567 5031006.536 45.41629459 -75.71466524 14391
Y	2019 2019/12/21 13:34:00+00 2019/12/21 13:34:00+00 BOOTH ST @ SIR JOHN A. MACDONALD PKWY/WELLINGTON ST (0012 2019 2019/02/21 23:00:00+00 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV (_5VHJ3U)	12: 12252 02 - Intersection related 5VHJ3U 01 - Non intersection	03 - P.D. only 03 - P.D. only	05 - Turning movement 04 - Sideswipe	01 - Clear 01 - Clear		01 - Dry 02 - Wet	01 - Traffic signal 10 - No control	01 - Functioning	0 366264.0356 5031005.6 45.41628625 -75.71467712 14934 0 366166.189 5031222.885 45.41824991 -75.71590013 2124
Ý	2019 2019/02/24 07:02:00+00 2019/02/24 07:02:00+00 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV (SVHJ3U)	5VHJ3U 01 - Non intersection	03 - P.D. only	07 - SMV other	04 - Freezing Rain		06 - Ice	10 - No control		0 366228.8533 5031095.914 45.41710196 -75.71511534 2193
Υ	2019 2019/12/21 12:00:00+00 2019/12/21 12:00:00+00 BOOTH ST btwn MIDDLE ST & VIMY PLACE PRIV (5VHJ3U)	5VHJ3U 07 - Overpass or bridge	02 - Non-fatal injury		01 - Clear	01 - Daylight		10 - No control		0 366129.92 5031270.151 45.41867838 -75.71635762 14932
Y	2019 2019/04/22 07:00:00+00 2019/04/22 07:00:00+00 BOOTH ST btwn OTTAWA RIVER PKWY & FLEET ST (SVHJ61) 2019 2019/09/17 06:30:00+00 2019/09/17 06:30:00+00 BOOTH ST btwn OTTAWA RIVER PKWY & FLEET ST (_SVHJ61)	5VHJ61 01 - Non intersection 5VHJ61 01 - Non intersection	03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	01 - Clear 01 - Clear	01 - Daylight 03 - Dawn	01 - Dry 01 - Dry	10 - No control 10 - No control		0 366286.4937 5030956.785 45.41584506 -75.71439629 4541 0 366281.7139 5030967.471 45.41594163 -75.71445603 9027
y y	2019 2019/10/27 10:50:00+00 2019/10/27 10:50:00+00 BOOTH ST blum VIMY PLACE PRIV & OTTAWA RIVER PKWY (_5VHJ2)		03 - P.D. only	03 - Rear end	01 - Clear 01 - Clear		01 - Dry	10 - No control		0 366258.6417 5031020.957 45.41642491 -75.71474412 11919
Y	2019 2019/04/10 15:35:00+00 2019/04/10 15:35:00+00 BOOTH ST btwn VIMY PLACE PRIV & OTTAWA RIVER PKWY (5VHJ2)	() _5VHJ2X 01 - Non intersection	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight	01 - Dry	10 - No control		0 366242.2096 5031063.401 45.41680825 -75.71494876 14281
Y	2019 2019/03/14 06:01:00+00 2019/03/14 06:01:00+00 BOOTH ST blwn WELLINGTON ST & ALBERT ST (_3ZA2G1)	3ZA2G1	03 - P.D. only	04 - Sideswipe	01 - Clear		04 - Slush	10 - No control		0 366411.3301 5030661.251 45.41317495 -75.71283837 3752
Y V	2019 2019/01/18 10:50:00+00 2019/01/18 10:50:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679) 2019 2019/01/20 15:14:00+00 2019/01/20 15:14:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related 1679 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	03 - Snow 03 - Snow	01 - Daylight 01 - Daylight		01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366768.7904 5030948.362 45.41572672 -75.70823571 286 0 366768.4681 5030948.622 45.41572909 -75.7082398 402
Ý	2019 2019/01/20 15:31:00+00 2019/01/20 15:31:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow	01 - Daylight	05 - Packed snow	01 - Traffic signal		0 366768.7211 5030948.706 45.41572982 -75.70823655 415
Y	2019 2019/01/27 10:08:00+00 2019/01/27 10:08:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 03 - At intersection	03 - P.D. only	02 - Angle	03 - Snow		03 - Loose snow	01 - Traffic signal	01 - Functioning	0 366768.8223 5030948.486 45.41572784 -75.70823529 1261
Y	2019 2019/02/02 12:30:00+00 2019/02/02 12:30:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679) 2019 2019/02/25 08:55:00+00 2019/02/25 08:55:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related 1679 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	03 - Snow 01 - Clear	01 - Daylight 01 - Daylight	03 - Loose snow	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366768.4342 5030948.497 45.41572797 -75.70824024 1624 0 366768.8385 5030948.726 45.41572999 -75.70823505 2038
Ÿ	2019 2019/02/13 09:55:00+00 2019/02/13 09:55:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)	1679 02 - Intersection related	03 - P.D. only	03 - Rear end	03 - Snow	01 - Daylight 01 - Daylight	04 - Slush	01 - Traffic signal	01 - Functioning	0 366768.7056 5030948.567 45.41572858 -75.70823677 2792
Υ	2019 2019/03/07 15:14:00+00 2019/03/07 15:14:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679)		03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight		01 - Traffic signal	01 - Functioning	0 366768.7315 5030948.438 45.41572741 -75.70823645 3047
Y	2019 2019/05/17 08:30:00+00 2019/05/17 08:30:00+00 BRONSON AVE @ COMMISSIONER ST/SLATER ST (0001679) 2019 2019/02/21 07:57:00+00 2019/02/21 07:57:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	1679 03 - At intersection 9072 02 - Intersection related	03 - P.D. only 02 - Non-fatal injury	05 - Turning movement 03 - Rear end	01 - Clear 03 - Snow	01 - Daylight		01 - Traffic signal	01 - Functioning 01 - Functioning	0 366768.6877 5030948.577 45.41572867 -75.708237 5865
Ϋ́	2019 2019/02/21 07:57:00+00 2019/02/21 07:57:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072) 2019 2019/03/27 21:30:00+00 2019/03/27 21:30:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)		02 - Non-ratar injury 03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight 07 - Dark	04 - Siusn 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366516.3123 5031314.647 45.41904476 -75.7114154 2248 0 366516.0839 5031314.712 45.41904537 -75.71141831 3286
Υ	2019 2019/03/21 21:26:00+00 2019/03/21 21:26:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	9072 02 - Intersection related	03 - P.D. only	03 - Rear end	02 - Rain		02 - Wet	01 - Traffic signal	01 - Functioning	0 366516.1176 5031314.474 45.41904323 -75.7114179 3415
Υ	2019 2019/04/18 11:29:00+00 2019/04/18 11:29:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	9072 02 - Intersection related	03 - P.D. only 03 - P.D. only	04 - Sideswipe	02 - Rain	01 - Daylight		01 - Traffic signal	00 - Unknown	0 366516.1941 5031314.834 45.41904645 -75.71141688 4484
Y	2019 2019/08/12 18:00:00+00 2019/08/12 18:00:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072) 2019 2019/10/11 22:47:00+00 2019/10/11 22:47:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	9072 02 - Intersection related 9072 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 05 - Turning movement	02 - Rain 01 - Clear	01 - Daylight 07 - Dark	02 - Wet 01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 366516.0216 5031314.867 45.41904677 -75.71141908 8647 0 366516.069 5031314.476 45.41904324 -75.71141853 10715
Ý	2019 2019/06/11 12:55:00+00 2019/06/11 12:55:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	9072 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear		01 - Dry	01 - Traffic signal	01 - Functioning	0 366516.0466 5031314.671 45.419045 -75.71141879 14023
Y	2019 2019/01/21 12:01:00+00 2019/01/21 12:01:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072)	9072 02 - Intersection related	03 - P.D. only	07 - SMV other	01 - Clear	01 - Daylight	05 - Packed snow	01 - Traffic signal	01 - Functioning	0 366516.2708 5031314.58 45.41904416 -75.71141593 14738
Y	2019 2019/08/22 06:53:00+00 2019/08/22 06:53:00+00 OTTAWA RIVER PKWY @ PORTAGE BRIDGE (0009072) 2019 2019/03/13 17:30:00+00 2019/03/13 17:30:00+00 OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	9072 02 - Intersection related 5099 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	01 - Clear 03 - Snow	01 - Daylight 05 - Dusk	01 - Dry 05 - Packed snow	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 36516.263 5031314.444 45.41904294 -75.71141605 14878 0 365156.8235 5030377.62 45.41073223 -75.72889952 4717
Ÿ	2019 2019/03/13 17:30:00+00 2019/03/13 17:30:00+00 OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099) 2019 2019/08/16 07:46:00+00 2019/08/16 07:46:00+00 OTTAWA RIVER PKWY @ RIVER ST/SLIDELL ST (0005099)	5099 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end	01 - Clear		01 - Packed snow 01 - Dry	01 - Traffic signal	01 - Functioning 01 - Functioning	0 365156.4237 5030377.635 45.41073223 -75.72889952 4717 0 365156.4237 5030377.635 45.4107324 -75.72890462 14867
Υ	2019 2019/09/30 17:30:00+00 2019/09/30 17:30:00+00 OTTAWA RIVER PKWY @ VIMY PLACE PRIV (0012251)	12251 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 365973.9275 5030798.141 45.41444502 -75.71840922 10471
Y	2019 2019/03/17 16:15:00+00 2019/03/17 16:15:00+00 OTTAWA RIVER PKWY @ VIMY PLACE PRIV (0012251)	12251 02 - Intersection related	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 365974.0473 5030798.043 45.41444412 -75.71840771 14009
Y	2019 2019/07/23 08:05:00+00 2019/07/23 08:05:00+00 OTTAWA RIVER PKWY @ VIMY PLACE PRIV (0012251) 2019 2019/05/03 10:28:00+00 2019/05/03 10:28:00+00 OTTAWA RIVER PKWY btwn VIMY PLACE PRIV & BOOTH ST (5VHJ20	12251 02 - Intersection related QE 5VHJ2QB 01 - Non intersection	03 - P.D. only 03 - P.D. only	03 - Rear end 04 - Sideswipe	01 - Clear 02 - Rain	01 - Daylight 01 - Daylight	01 - Dry 02 - Wet	01 - Traffic signal 10 - No control	01 - Functioning	0 365974.0957 5030798.06 45.41444427 -75.71840708 14359 0 366071.725 5030867.807 45.4150633 -75.71715117 14308
Y	2019 2019/10/18 11:53:00+00 2019/10/18 11:53:00+00 OTTAWA RIVER PKWY OFFR-EB @ PARKDALE AVE (0011241)	11241 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight	01 - Dry	03 - Yield sign	01 - Functioning	0 364684.0505 5030331.791 45.41036048 -75.73494455 11650
Y	2019 2019/06/20 12:30:00+00 2019/06/20 12:30:00+00 OTTAWA RIVER PKWY WB @ PARKDALE AVE WB ON RAMP (0011376		03 - P.D. only	07 - SMV other	02 - Rain	01 - Daylight	02 - Wet	03 - Yield sign	01 - Functioning	0 364638.3144 5030469.151 45.41160033 -75.73551213 14048
Y	2019 2019/02/06 08:39:00+00 2019/02/06 08:39:00+00 PARKDALE AVE @ SCOTT ST (0002213) 2019 2019/02/22 15:30:00+00 2019/02/22 15:30:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related 2213 02 - Intersection related	03 - P.D. only 03 - P.D. only	03 - Rear end 03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	03 - Loose snow 02 - Wet	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 364949.7217 5029726.658 45.40489285 -75.73162457 1722 0 364949.6092 5029726.317 45.40488979 -75.73162605 2141
Ý	2019 2019/03/30 13:46:00+00 2019/03/30 13:46:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related 2213 03 - At intersection	02 - Non-fatal injury	05 - Turning movement	02 - Rain	01 - Daylight 01 - Daylight		01 - Traffic signal	01 - Functioning 01 - Functioning	0 364949.5477 5029726.714 45.40489337 -75.73162678 3357
Y	2019 2019/04/16 09:15:00+00 2019/04/16 09:15:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 364949.7023 5029726.608 45.4048924 -75.73162482 4599
Y	2019 2019/07/14 08:25:00+00 2019/07/14 08:25:00+00 PARKDALE AVE @ SCOTT ST (0002213) 2019 2019/10/08 11:00:00+00 2019/10/08 11:00:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 03 - At intersection 2213 02 - Intersection related	03 - P.D. only	02 - Angle 03 - Rear end	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	01 - Dry	01 - Traffic signal 01 - Traffic signal	01 - Functioning 01 - Functioning	0 364949.7335 5029726.379 45.40489034 -75.73162445 7087 0 364949.6519 5029726.811 45.40489424 -75.73162544 10559
Y	2019 2019/10/31 09:11:00+00 2019/10/31 09:11:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 03 - At intersection	03 - P.D. only	03 - Rear end 02 - Angle	01 - Clear 02 - Rain	01 - Daylight 01 - Daylight	02 - Diy	01 - Traffic signal	01 - Functioning	0 364949.8011 5029726.739 45.40489357 -75.73162354 11477
Υ	2019 2019/11/22 15:16:00+00 2019/11/22 15:16:00+00 PARKDALE AVE @ SCOTT ST (0002213)	2213 02 - Intersection related	03 - P.D. only	05 - Turning movement	01 - Clear	01 - Daylight	01 - Dry	01 - Traffic signal	01 - Functioning	0 364949.8214 5029726.408 45.40489059 -75.73162333 12180

Y	2019 2019/01/16 00:00:00:00+00 2019/01/16 00:00:00+00 PARKDALE AVE btwn BURNSIDE AVE & LYNDALE AVE (3ZA32H)	3ZA32H	01 - Non intersection	03 - P.D. only	06 - SMV unattended vehicle	03 - Snow	00 - Unknown C	04 - Slush	10 - No control		0 364853.56 5029948.291 45.40689531 -75.73282587 633
Y	2019 2019/01/24 22:00:00+00 2019/01/24 22:00:00+00 PARKDALE AVE btwn BURNSIDE AVE & LYNDALE AVE (3ZA32H)	3ZA32H	01 - Non intersection	03 - P.D. only	06 - SMV unattended vehicle	01 - Clear	07 - Dark C	01 - Dry	10 - No control		0 364793.6364 5030086.203 45.40814135 -75.73357453 1418
Y	2019 2019/07/18 12:57:00+00 2019/07/18 12:57:00+00 PARKDALE AVE btwn LYNDALE AVE & SCOTT ST (_3ZA31W)	3ZA31W	04 - At/near private drive	03 - P.D. only	02 - Angle	01 - Clear	01 - Daylight C	01 - Dry	10 - No control		0 364877.7554 5029892.617 45.40639229 -75.73252359 7600
Y	2019 2019/08/06 10:25:00+00 2019/08/06 10:25:00+00 PARKDALE AVE btwn LYNDALE AVE & SCOTT ST (_3ZA31W)	3ZA31W	04 - At/near private drive	02 - Non-fatal injury	07 - SMV other	01 - Clear	01 - Daylight C	01 - Dry	10 - No control		1 364916.979 5029801.718 45.40557103 -75.73203365 8687
Y	2019 2019/02/02 13:42:00+00 2019/02/02 13:42:00+00 SCOTT ST btwn HILDA ST & BAYVIEW RD (3ZA32U)	3ZA32U	01 - Non intersection	03 - P.D. only	04 - Sideswipe	03 - Snow	01 - Daylight C	03 - Loose snow	10 - No control		0 365514.9775 5030069.363 45.40792763 -75.72436224 1610
Y	2019 2019/12/31 06:30:00+00 2019/12/31 06:30:00+00 SCOTT ST btwn PARKDALE AVE & PINEHURST AVE (3ZA31U)	3ZA31U	04 - At/near private drive	03 - P.D. only	02 - Angle	03 - Snow	07 - Dark 0	03 - Loose snow	10 - No control		0 364962.4684 5029734.056 45.40495832 -75.73146085 14424
Y	2019 2019/06/25 16:50:00+00 2019/06/25 16:50:00+00 WELLINGTON ST @ LETT ST (0012269)	1226	9 02 - Intersection related	03 - P.D. only	03 - Rear end	01 - Clear	01 - Daylight C	01 - Dry	01 - Traffic signal	01 - Functioning	0 366355.3951 5031111.272 45.41722903 -75.71349674 6753
Y	2019 2019/07/17 10:36:00+00 2019/07/17 10:36:00+00 WELLINGTON ST btwn TURN LANE & TO BE DETERMINED (3ZA2L9)	3ZA2L9	01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight C	01 - Dry	10 - No control		0 366474.49 5031272.137 45.41866596 -75.71195505 7546
Y	2019 2019/05/21 08:05:00+00 2019/05/21 08:05:00+00 WELLINGTON ST btwn TURN LANE & TO BE DETERMINED (3ZA2L9)	3ZA2L9	01 - Non intersection	03 - P.D. only	04 - Sideswipe	01 - Clear	01 - Daylight C	01 - Dry	10 - No control		0 366630.6264 5031280.427 45.41872677 -75.70995917 14333

## **APPENDIX D: Adjacent Development TIAs**



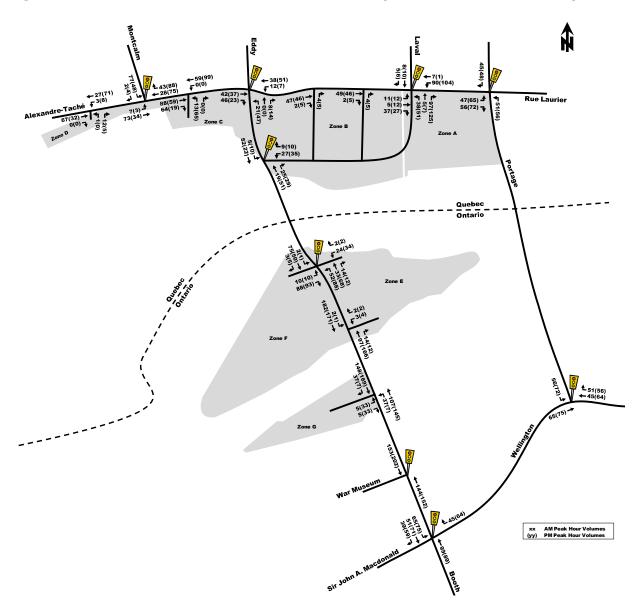


Figure 12: 'New' Site-Generated Traffic Volumes (Ottawa + Gatineau Sites)

## 5.4 Other Planned/Potential Development in the Vicinity of the Study Area

The Domtar lands are not the only site in this area of Gatineau and Ottawa that has development/redevelopment potential. The following Table 8 summarizes development sites identified by the Cities of Gatineau and Ottawa, as well their assumed development yield. Also included in this table are the assumptions used to estimate vehicle trips and the resultant peak hour vehicle trips. These being approximately 1000 vph two-way total during both peak hours for the Gatineau sites, and approximately 1300 vph two-way total during both peak hours for the Ottawa sites. Combining both provinces, the two-way peak hour total is approximately 2200 vph to 2400 vph.



Figure 9: Site Generated Traffic Volumes

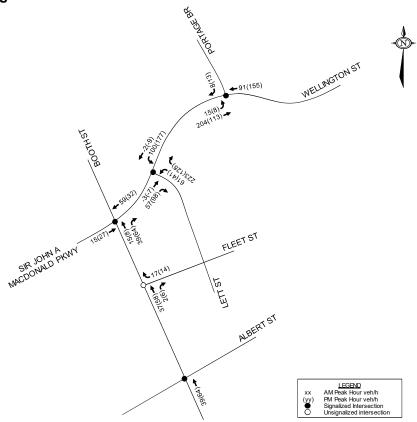
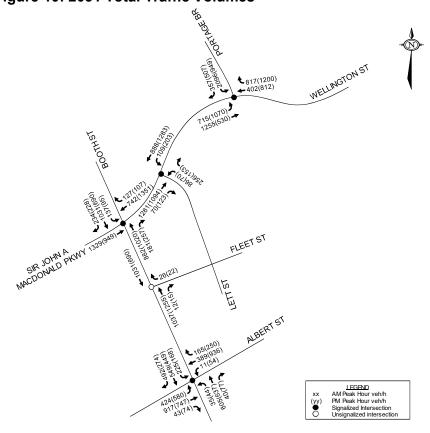


Figure 10: 2031 Total Traffic Volumes



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## **PARSONS**

Albert Street
(Neilington)

Activities

April 1990

Ap

Figure 11: New and Pass-by Site-Generated Traffic Volumes - 2025

## **FUTURE TRAFFIC OPERATIONS**

## PROJECTED CONDITIONS AT FULL SITE DEVELOPMENT

The total projected volumes associated with the proposed development were derived by superimposing new and pass-by site-generated traffic volumes (Figure 10 and 11) onto projected background traffic volumes (Figure 7 and 8). The resulting total projected volumes for the horizon years 2020 and 2025 are illustrated as Figure 12 and 13, respectively.

## As 1890|(373)

## As 1890|(

Figure 12: Total Projected Peak Hour Traffic Volumes - 2020

# APPENDIX E: Existing and Future Background Conditions - Synchro Outputs



Synchro Modelling Outputs – Existing Conditions



	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b> ^	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	595	859	263	704	1962	309
Future Volume (vph)	595	859	263	704	1962	309
Satd. Flow (prot)	3317	4914	3420	2693	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3301	4914	3420	2693	3272	1501
Satd. Flow (RTOR)			0.120			68
Lane Group Flow (vph)	626	904	277	741	2065	325
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	18	1	,
Permitted Phases						1
Detector Phase	7	4	8	18	1	1
Switch Phase		*				•
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		45.1	45.1
Total Split (s)	42.8	70.3	27.5		51.1	51.1
Total Split (%)	35.3%	57.9%	22.7%		42.1%	42.1%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		-2.1	0.0
Total Lost Time (s)	5.8	6.5	6.5		4.0	6.1
Lead/Lag	Lead	0.0	Lag			<b>V.</b> 1
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Min		Max	Max
Act Effct Green (s)	26.0	50.7	18.9	70.7	47.4	45.3
Actuated g/C Ratio	0.24	0.47	0.17	0.65	0.44	0.42
v/c Ratio	0.79	0.39	0.47	0.42	1.44	0.49
Control Delay	46.6	19.1	44.0	10.8	230.0	22.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.6	19.1	44.0	10.8	230.0	22.6
LOS	70.0 D	В	D	В	200.0 F	C
Approach Delay		30.4	19.9		201.8	<u> </u>
Approach LOS		C	В		201.0 F	
Queue Length 50th (m)	69.3	46.5	29.3	40.8	~331.2	42.0
Queue Length 95th (m)	89.3	56.5	47.6	71.7		80.2
Internal Link Dist (m)	00.0	234.2	291.0	, , , ,	48.7	50.2
Turn Bay Length (m)	150.0	201.2	201.0		10.1	30.0
Base Capacity (vph)	1136	2903	665	1738	1433	665
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.55	0.31	0.42	0.43	1.44	0.49
reduced v/o realio	0.00	0.01	0.72	0.70	1.77	0.73

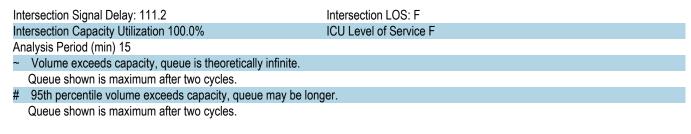
## Intersection Summary

Cycle Length: 121.4

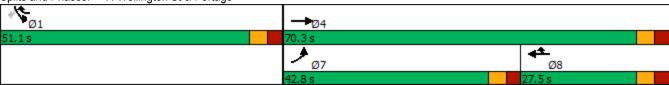
Actuated Cycle Length: 108.7

Natural Cycle: 145

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.44



Splits and Phases: 7: Wellington St & Portage



	۶	<b>→</b>	$\rightarrow$	•	←	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ <sub>a</sub>		ሻ	f)		ሻ	4		ሻ	1>	
Traffic Volume (vph)	0	0	0	0	0	0	0	688	0	0	1283	(
Future Volume (vph)	0	0	0	0	0	0	0	688	0	0	1283	C
Satd. Flow (prot)	1800	1800	0	1800	1800	0	1800	1800	0	1800	1800	C
Flt Permitted												
Satd. Flow (perm)	1800	1800	0	1800	1800	0	1800	1800	0	1800	1800	C
Satd. Flow (RTOR)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	724	0	0	1351	C
Turn Type	Perm			Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)								75.0			75.0	
Actuated g/C Ratio								1.00			1.00	
v/c Ratio								0.40			0.75	
Control Delay								0.7			2.9	
Queue Delay								0.0			0.0	
Total Delay								0.7			2.9	
LOS								Α			Α	
Approach Delay								0.7			2.9	
Approach LOS								Α			Α	
Queue Length 50th (m)								0.0			0.0	
Queue Length 95th (m)								0.0			0.0	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)												
Base Capacity (vph)								1800			1800	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.40			0.75	
Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 0 (0%), Referenced	to phase 2	:NBTL and	l 6:SBTL	, Start of	Green							

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.75		
Intersection Signal Delay: 2.1	Intersection LOS: A	
Intersection Capacity Utilization 76.2%	ICU Level of Service D	
Analysis Period (min) 15		
Splits and Phases: 10: Booth St & Chaudiere		
Ø2 (R)		<b>♣</b> <sub>04</sub>
53 s		22 s
<b>∮</b> Ø6 (R)		<b>▼</b> Ø8
53 c		22 s

	۶	-	$\rightarrow$	•	•	•	•	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>♦</b> ₽		ř	<b>十</b> 十	7
Traffic Volume (vph)	0	1233	0	0	629	87	0	699	126	81	908	194
Future Volume (vph)	0	1233	0	0	629	87	0	699	126	81	908	194
Satd. Flow (prot)	0	3420	0	0	3386	1530	0	3095	0	1710	3226	1530
Flt Permitted										0.132		
Satd. Flow (perm)	0	3420	0	0	3386	1478	0	3095	0	236	3226	1465
Satd. Flow (RTOR)								22				141
Lane Group Flow (vph)	0	1298	0	0	662	92	0	869	0	85	956	204
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase												
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.8		12.0	31.9	31.9
Total Split (s)		48.0			48.0	48.0		35.0		12.0	47.0	47.0
Total Split (%)		50.5%			50.5%	50.5%		36.8%		12.6%	49.5%	49.5%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	-2.8		0.0		0.0	0.0	-2.9
Total Lost Time (s)		6.8			6.8	4.0		6.8		6.8	6.9	4.0
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		40.0			40.0	42.8		31.7		41.4	41.3	44.2
Actuated g/C Ratio		0.42			0.42	0.45		0.33		0.44	0.43	0.47
v/c Ratio		0.90			0.46	0.14		0.83		0.45	0.68	0.27
Control Delay		32.6			29.4	22.9		38.4		32.4	30.7	12.1
Queue Delay		8.0			0.0	0.0		2.7		3.7	0.0	0.0
Total Delay		33.4			29.4	22.9		41.1		36.1	30.7	12.1
LOS		С			С	С		D		D	С	В
Approach Delay		33.4			28.6			41.1			28.1	
Approach LOS		С			С			D			С	
Queue Length 50th (m)		117.1			62.7	13.3		83.6		9.7	77.7	6.7
Queue Length 95th (m)	;	#151.9			83.4	27.7		#123.1		33.7	126.9	49.1
Internal Link Dist (m)		331.0			128.4			47.8			192.9	
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1483			1468	684		1046		188	1403	757
Starvation Cap Reductn		0			0	0		0		0	0	0
Spillback Cap Reductn		46			0	0		92		50	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.90			0.45	0.13		0.91		0.62	0.68	0.27

## Intersection Summary

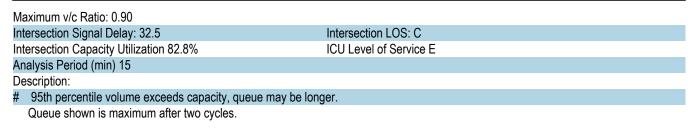
Cycle Length: 95

Actuated Cycle Length: 95

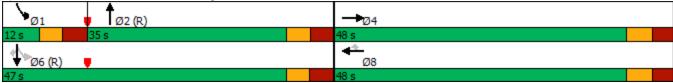
Offset: 31 (33%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated



Splits and Phases: 11: Booth St & Wellington St



	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>†</b>	7	ሻ	<b>^</b>	7		4TÞ		ሻ	<b>†</b>	7
Traffic Volume (vph)	324	704	17	7	244	138	6	482	30	162	454	393
Future Volume (vph)	324	704	17	7	244	138	6	482	30	162	454	393
Satd. Flow (prot)	1660	1748	1443	1710	3196	1378	0	3329	0	1425	1782	1471
Flt Permitted	0.493			0.239				0.948		0.215		
Satd. Flow (perm)	819	1748	1377	428	3196	1251	0	3157	0	310	1782	1313
Satd. Flow (RTOR)			91					5				
Lane Group Flow (vph)	341	741	18	7	257	145	0	545	0	171	478	414
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		5.0	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	18.0	55.0	55.0	37.0	37.0	37.0	40.0	40.0		25.0	65.0	65.0
Total Split (%)	15.0%	45.8%	45.8%	30.8%	30.8%	30.8%	33.3%	33.3%		20.8%	54.2%	54.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	None	C-Min	C-Min	C-Max	C-Max	C-Max	Min	Min		None	None	None
Act Effct Green (s)	58.3	58.3	60.8	34.8	34.8	34.8		27.2		48.7	48.7	48.7
Actuated g/C Ratio	0.49	0.49	0.51	0.29	0.29	0.29		0.23		0.41	0.41	0.41
v/c Ratio	0.66	0.87	0.02	0.06	0.28	0.40		0.76		0.65	0.66	0.78
Control Delay	27.4	34.4	0.1	37.9	36.7	41.9		49.9		34.6	33.1	41.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	27.4	34.4	0.1	37.9	36.7	41.9		49.9		34.6	33.1	41.2
LOS	С	С	Α	D	D	D		D		С	С	D
Approach Delay		31.6			38.5			49.9			36.5	
Approach LOS		С			D			D			D	
Queue Length 50th (m)	40.8	95.0	0.0	1.3	27.1	30.2		65.6		27.8	93.3	86.4
Queue Length 95th (m)	m56.3 r	n#235.7	m0.0	5.7	41.8	54.6		82.1		40.0	116.1	114.4
Internal Link Dist (m)		285.1			168.5			37.2			86.5	
Turn Bay Length (m)	160.0		40.0	40.0		95.0						120.0
Base Capacity (vph)	517	849	743	123	926	362		884		297	868	640
Starvation Cap Reductn	0	0	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	0.66	0.87	0.02	0.06	0.28	0.40		0.62		0.58	0.55	0.65

## Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

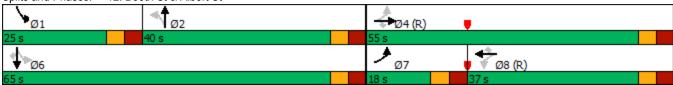
Offset: 104 (87%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.87
Intersection Signal Delay: 37.4
Intersection LOS: D
Intersection Capacity Utilization 117.3%
ICU Level of Service H
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	<b>→</b>	•	•	•	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	7	ሻ	<b>^</b>	ሻ	7
Traffic Volume (vph)	745	114	382	313	100	282
Future Volume (vph)	745	114	382	313	100	282
Satd. Flow (prot)	1748	1471	1629	3288	1583	1471
Flt Permitted			0.099	0_00	0.950	
Satd. Flow (perm)	1748	1384	170	3288	1561	1417
Satd. Flow (RTOR)		28				297
Lane Group Flow (vph)	784	120	402	329	105	297
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4		3	8	2	
Permitted Phases		4	8	_		2
Detector Phase	4	4	3	8	2	2
Switch Phase	•			_	_	_
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	31.8	31.8	11.2	31.8	29.3	29.3
Total Split (s)	65.0	65.0	25.0	90.0	30.0	30.0
Total Split (%)	54.2%	54.2%	20.8%	75.0%	25.0%	25.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	3.5	3.5	2.9	3.5	3.0	3.0
Lost Time Adjust (s)	0.0	-2.8	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	4.0	6.2	6.8	6.3	6.3
Lead/Lag	Lag	Lag	Lead	0.0	0.0	0.0
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Act Effct Green (s)	58.2	61.0	93.8	93.2	13.7	13.7
Actuated g/C Ratio	0.48	0.51	0.78	0.78	0.11	0.11
v/c Ratio	0.93	0.17	0.83	0.13	0.58	0.70
Control Delay	40.1	7.6	35.1	1.5	62.9	14.8
Queue Delay	0.3	0.0	0.0	0.0	0.0	0.0
Total Delay	40.3	7.6	35.1	1.5	62.9	14.8
LOS	D	Α.	D	Α	02.5 E	В
Approach Delay	36.0		<u> </u>	20.0	27.4	U
Approach LOS	30.0 D			20.0 B	C C	
Queue Length 50th (m)	176.4	9.9	52.1	3.3	25.2	0.0
Queue Length 95th (m)	#263.0	4.8	#138.1	5.7	42.3	27.2
Internal Link Dist (m)	78.2	4.0	#130.1	285.1	54.7	21.2
Turn Bay Length (m)	10.2	16.0	90.0	200.1	J <del>4</del> .1	
Base Capacity (vph)	847	717	483	2554	312	518
Starvation Cap Reductn	3			2554		0
		0	0		0	0
Spillback Cap Reductn	0		0	0	-	0
Storage Cap Reductn	0 03	0 17	0	0 12	0 24	
Reduced v/c Ratio	0.93	0.17	0.83	0.13	0.34	0.57

## Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

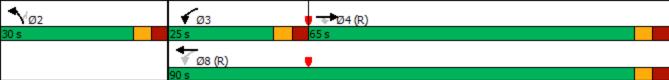
Offset: 80 (67%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 120

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.93
Intersection Signal Delay: 28.5 Intersection LOS: C
Intersection Capacity Utilization 88.1% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 13: Preston St & Albert St



# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ħ	<b>^</b>	<b>^</b>		¥	
Traffic Volume (vph)	15	1345	834	41	2	15
Future Volume (vph)	15	1345	834	41	2	15
Satd. Flow (prot)	1710	3420	3360	0	1462	0
Flt Permitted	0.310				0.994	
Satd. Flow (perm)	557	3420	3360	0	1462	0
Satd. Flow (RTOR)			9		16	
Lane Group Flow (vph)	16	1416	921	0	18	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases	. 5.111	4	8		6	
Permitted Phases	4	•				
Detector Phase	4	4	8		6	
Switch Phase		7	- 0		0	
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	37.0	37.0	37.0		32.3	
Total Split (s)	62.0	62.0	62.0		33.0	
Total Split (%)	65.3%	65.3%	65.3%		34.7%	
Yellow Time (s)	3.7	3.7	3.7		3.3	
All-Red Time (s)	2.3	2.3	2.3		3.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.3	
Lead/Lag	0.0	0.0	0.0		0.3	
· ·						
Lead-Lag Optimize? Recall Mode	C May	C-Max	C-Max		None	
	C-Max					
Act Effct Green (s)	86.1	86.1	86.1		10.0	
Actuated g/C Ratio	0.91	0.91	0.91		0.11	
v/c Ratio	0.03	0.46	0.30		0.11	
Control Delay	2.4	2.8	1.1		20.9	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	2.4	2.8	1.1		20.9	
LOS	А	A	A		С	
Approach Delay		2.8	1.1		20.9	
Approach LOS		Α	Α		С	
Queue Length 50th (m)	0.0	0.0	3.3		0.4	
Queue Length 95th (m)	2.0	58.5	5.7		7.0	
Internal Link Dist (m)		651.1	331.0		21.1	
Turn Bay Length (m)	40.0					
Base Capacity (vph)	505	3099	3045		422	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.03	0.46	0.30		0.04	
Intersection Summary						

#### Intersection Summar

Cycle Length: 95

Actuated Cycle Length: 95

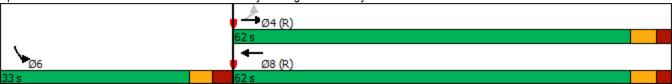
Offset: 59 (62%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 70

# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.46
Intersection Signal Delay: 2.3
Intersection Capacity Utilization 59.1%
ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>/</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>^</b>			f)	
Traffic Volume (vph)	0	1454	22	1	944	1	1	21	1	1	5	3
Future Volume (vph)	0	1454	22	1	944	1	1	21	1	1	5	
Satd. Flow (prot)	0	3412	0	0	3386	0	0	1784	0	0	1699	0
Flt Permitted					0.954			0.984			0.956	
Satd. Flow (perm)	0	3412	0	0	3230	0	0	1759	0	0	1634	0
Satd. Flow (RTOR)		3						1			3	
Lane Group Flow (vph)	0	1554	0	0	996	0	0	24	0	0	9	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		72.2			72.2			10.1			10.1	
Actuated g/C Ratio		0.90			0.90			0.13			0.13	
v/c Ratio		0.51			0.34			0.11			0.04	
Control Delay		3.3			2.4			33.2			28.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		3.3			2.4			33.2			28.7	
LOS		A			A			C			C	
Approach Delay		3.3			2.4			33.2			28.7	
Approach LOS		A			A			C			С	
Queue Length 50th (m)		0.0			0.0			3.1			0.8	
Queue Length 95th (m)		69.2			35.8			10.4			5.2	
Internal Link Dist (m)		121.2			288.5			29.2			167.1	
Turn Bay Length (m)		2007			0004			C40			FC0	
Base Capacity (vph)		3067			2904			610			569	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0.51			0 24			0.04			0	
Reduced v/c Ratio		0.51			0.34			0.04			0.02	

Cycle Length: 95

Actuated Cycle Length: 80.3

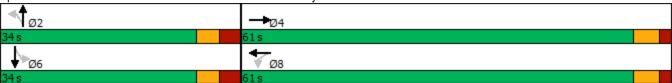
Natural Cycle: 75

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.51

Intersection Signal Delay: 3.3 Intersection LOS: A
Intersection Capacity Utilization 63.1% ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		J.	<b>†</b>	7	¥	<b>†</b>	7	¥	<del>(</del> Î	
Traffic Volume (vph)	14	647	86	47	276	22	61	45	109	111	178	12
Future Volume (vph)	14	647	86	47	276	22	61	45	109	111	178	12
Satd. Flow (prot)	1500	1712	0	1613	1698	1457	1676	1800	1515	1693	1752	0
Flt Permitted	0.582			0.282			0.499			0.726		
Satd. Flow (perm)	877	1712	0	474	1698	1328	871	1800	1420	1243	1752	0
Satd. Flow (RTOR)		12				37			115		3	
Lane Group Flow (vph)	15	772	0	49	291	23	64	47	115	117	200	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?	0.14	0.14		O.M.	0.14	O.M.	NI	NI	NI.	NI.	Maria	
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	68.9	68.9		68.9	68.9	68.9	18.2	18.2	18.2	18.2	18.2	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.02	0.65		0.15 8.5	0.25	0.02	0.41	0.14 32.5	0.33	0.52 43.7	0.62 44.8	
Control Delay	0.0	7.6 0.0		0.0	7.5 0.0	0.0	41.7 0.0	0.0	0.4	0.0	0.0	
Queue Delay Total Delay	3.0	7.6		8.5	7.5	1.4	41.7	32.5	8.4	43.7	44.8	
LOS	3.0 A	7.6 A		6.5 A	7.5 A	1.4 A	41.7 D	32.5 C	0.4 A	43. <i>1</i>	44.0 D	
Approach Delay	A	7.5		A	7.2	A	U	22.9	A	U	44.4	
Approach LOS		7.5 A			7.2 A			22.9 C			44.4 D	
Queue Length 50th (m)	0.2	9.0		2.8	18.3	0.0	11.9	8.3	0.0	22.3	38.2	
Queue Length 95th (m)	m0.7	187.3		9.7	38.7	1.9	23.2	16.9	13.7	37.0	56.3	
Internal Link Dist (m)	1110.7	635.7		3.1	497.2	1.3	20.2	83.8	13.7	37.0	141.3	
Turn Bay Length (m)	45.0	055.7		65.0	431.2	40.0	50.0	05.0	20.0	45.0	141.5	
Base Capacity (vph)	604	1183		326	1170	926	222	460	449	318	450	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.02	0.65		0.15	0.25	0.02	0.29	0.10	0.26	0.37	0.44	
Nouvoed vio Natio	0.02	0.00		0.13	0.23	0.02	0.23	0.10	0.20	0.51	0.44	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 40 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 75

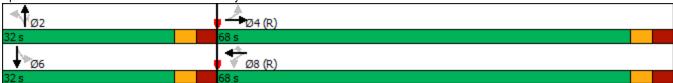
Maximum v/c Ratio: 0.65

Intersection Signal Delay: 16.4	Intersection LOS: B	
Intersection Capacity Utilization 81.2%	ICU Level of Service D	
Analysis Daried (min) 15		

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Scott St/Albert St & Bayview Station Rd



	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	<b>/</b>	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	*	<b>↑</b>	7	ሻ	f)		ች	f)	
Traffic Volume (vph)	120	653	37	49	275	24	31	281	67	95	354	145
Future Volume (vph)	120	653	37	49	275	24	31	281	67	95	354	145
Satd. Flow (prot)	1676	1748	1485	1676	1698	1471	1660	1724	0	1693	1564	0
Flt Permitted	0.545			0.212			0.215			0.396		
Satd. Flow (perm)	868	1748	1485	374	1698	1209	363	1724	0	706	1564	0
Satd. Flow (RTOR)			58			58		14			23	
Lane Group Flow (vph)	126	687	39	52	289	25	33	367	0	100	526	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	28.1	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	43.0	43.0		43.0	43.0	
Total Split (%)	51.0%	51.0%	51.0%	51.0%	51.0%	51.0%	43.0%	43.0%		43.0%	43.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	0.0	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	4.0	6.1	6.1	4.0	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	50.9	50.9	53.0	50.9	50.9	53.0	36.7	36.7		36.7	36.7	
Actuated g/C Ratio	0.51	0.51	0.53	0.51	0.51	0.53	0.37	0.37		0.37	0.37	
v/c Ratio	0.29	0.77	0.05	0.27	0.33	0.04	0.25	0.57		0.39	0.89	
Control Delay	16.3	27.2	1.8	16.3	14.5	0.6	28.2	28.6		28.9	48.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	16.3	27.2	1.8	16.3	14.5	0.6	28.2	28.6		28.9	48.5	
LOS	В	С	Α	В	В	Α	С	С		С	D	
Approach Delay		24.4			13.8			28.6			45.3	
Approach LOS		С			В			С			D	
Queue Length 50th (m)	14.1	108.5	0.0	7.1	40.9	0.1	4.6	56.9		14.8	96.1	
Queue Length 95th (m)	27.1	159.0	3.1	17.7	28.7	m0.6	13.3	87.0		30.5		
Internal Link Dist (m)		207.0			635.7			70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	441	889	814	190	864	668	133	641		259	588	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.29	0.77	0.05	0.27	0.33	0.04	0.25	0.57		0.39	0.89	

Cycle Length: 100

Actuated Cycle Length: 100

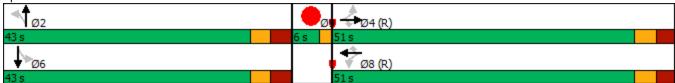
Offset: 91 (91%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Lane Group	Ø9
Lane Configurations	20
Traffic Volume (vph)	
Future Volume (vph)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Satd. Flow (RTOR)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	9
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	4.0
Minimum Split (s)	6.0
Total Split (s)	6.0
Total Split (%)	6%
Yellow Time (s)	2.0
All-Red Time (s)	0.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	None
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Maximum v/c Ratio: 0.89
Intersection Signal Delay: 29.3
Intersection LOS: C
Intersection Capacity Utilization 103.9%
ICU Level of Service G
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	•	•	4	<b>†</b>	<b>↓</b>	4			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9		
Lane Configurations	W		ሻ	<b>^</b>	<b>↑</b> ↑				
Traffic Volume (vph)	1	1	2	687	1282	6			
Future Volume (vph)	1	1	2	687	1282	6			
Satd. Flow (prot)	1600	0	1710	3420	3417	0			
Flt Permitted	0.976		0.170						
Satd. Flow (perm)	1598	0	306	3420	3417	0			
Satd. Flow (RTOR)	1				1	-			
Lane Group Flow (vph)	2	0	2	723	1355	0			
Turn Type	Prot		Perm	NA	NA	-			
Protected Phases	4			2	6		9		
Permitted Phases	•		2	_	•				
Detector Phase	4		2	2	6				
Switch Phase	-		_	_					
Minimum Initial (s)	7.8		10.0	10.0	10.0		10.0		
Minimum Split (s)	14.0		30.9	30.9	30.9		29.0		
Total Split (s)	17.0		49.0	49.0	49.0		29.0		
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%		
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0		
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0		
Lost Time Adjust (s)	0.0		0.0	0.0	0.0		1.0		
Total Lost Time (s)	6.2		5.9	5.9	5.9				
Lead/Lag	V.E		0.0	0.0	0.0				
Lead-Lag Optimize?									
Recall Mode	None		C-Max	C-Max	C-Max		None		
Act Effct Green (s)	7.8		85.2	85.2	85.2				
Actuated g/C Ratio	0.08		0.90	0.90	0.90				
v/c Ratio	0.02		0.01	0.24	0.44				
Control Delay	35.0		0.5	0.4	6.4				
Queue Delay	0.0		0.0	0.0	0.0				
Total Delay	35.0		0.5	0.4	6.4				
LOS	C		A	A	A				
Approach Delay	35.0		, (	0.4	6.4				
Approach LOS	C			Α	Α				
Queue Length 50th (m)	0.2		0.0	0.0	0.0				
Queue Length 95th (m)	2.4		m0.0	m3.2	143.1				
Internal Link Dist (m)	148.9		1110.0	192.9	188.6				
Turn Bay Length (m)	1-10.0		35.0	102.0	100.0				
Base Capacity (vph)	182		274	3068	3065				
Starvation Cap Reductn	0		0	0	0				
Spillback Cap Reductn	0		0	0	0				
Storage Cap Reductn	0		0	0	0				
Reduced v/c Ratio	0.01		0.01	0.24	0.44				
	0.01		0.01	0.24	0.44				
Intersection Summary									
Cycle Length: 95									

Actuated Cycle Length: 95

Offset: 47 (49%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

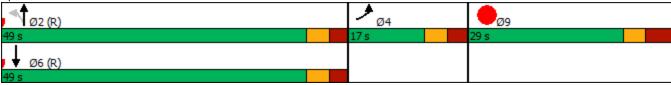
Natural Cycle: 80

Maximum v/c Ratio: 0.44

Intersection Signal Delay: 4.3	Intersection LOS: A	
Intersection Capacity Utilization 54.2%	ICU Level of Service A	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 19: Booth St & War Museum



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>†</b>	7	ሻ	<b>†</b>	7	ሻ	f)			4	
Traffic Volume (vph)	0	815	77	98	312	0	28	0	47	0	0	0
Future Volume (vph)	0	815	77	98	312	0	28	0	47	0	0	0
Satd. Flow (prot)	1800	1782	1485	1629	1714	1800	1644	1404	0	0	1800	0
Flt Permitted				0.291			0.950					
Satd. Flow (perm)	1800	1782	1406	496	1714	1800	1606	1404	0	0	1800	0
Satd. Flow (RTOR)			62					184				
Lane Group Flow (vph)	0	858	81	103	328	0	29	49	0	0	0	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA				
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	12.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)		99.3	101.2	99.3	99.3		12.6	12.6				
Actuated g/C Ratio		0.83	0.84	0.83	0.83		0.10	0.10				
v/c Ratio		0.58	0.07	0.25	0.23		0.17	0.16				
Control Delay		7.4	1.4	10.2	7.3		49.1	1.1				
Queue Delay		0.1	0.0	0.0	0.0		0.0	0.0				
Total Delay		7.5	1.4	10.2	7.3		49.1	1.1				
LOS		Α	Α	В	Α		D	Α				
Approach Delay		7.0			8.0			18.9				
Approach LOS		Α			Α			В				
Queue Length 50th (m)		61.1	0.6	8.8	22.0		6.8	0.0				
Queue Length 95th (m)		153.6	5.4	32.9	56.4		14.8	0.0				
Internal Link Dist (m)		497.2			115.1			178.8			41.9	
Turn Bay Length (m)			50.0	35.0			30.0					
Base Capacity (vph)		1475	1195	410	1418		397	485				
Starvation Cap Reductn		0	0	0	0		0	0				
Spillback Cap Reductn		73	0	0	0		0	7				
Storage Cap Reductn		0	0	0	0		0	0				
Reduced v/c Ratio		0.61	0.07	0.25	0.23		0.07	0.10				

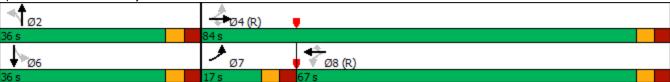
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 80

Maximum v/c Ratio: 0.58	
Intersection Signal Delay: 7.9	Intersection LOS: A
Intersection Capacity Utilization 80.5%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 20: City Center Ave & Albert St



	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		*	<b>^</b>	¥#	
Traffic Volume (vph)	1327	13	9	662	26	34
Future Volume (vph)	1327	13	9	662	26	34
Satd. Flow (prot)	3370	0	1710	3320	1533	0
Flt Permitted			0.162		0.979	
Satd. Flow (perm)	3370	0	291	3320	1529	0
Satd. Flow (RTOR)	2				36	
Lane Group Flow (vph)	1411	0	9	697	63	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	1.2		10.0	10.0	10.0	
Minimum Split (s)	30.0		30.0	30.0	33.9	
Total Split (s)	61.0		61.0	61.0	34.0	
Total Split (%)	64.2%	(	64.2%	64.2%	35.8%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	(	C-Max	C-Max	None	
Act Effct Green (s)	74.0		74.0	74.0	13.6	
Actuated g/C Ratio	0.78		0.78	0.78	0.14	
v/c Ratio	0.54		0.04	0.27	0.25	
Control Delay	18.4		6.6	5.1	19.6	
Queue Delay	1.2		0.0	0.0	0.0	
Total Delay	19.7		6.6	5.1	19.6	
LOS	В		Α	Α	В	
Approach Delay	19.7			5.1	19.6	
Approach LOS	В			Α	В	
Queue Length 50th (m)	128.3		0.3	16.1	4.8	
Queue Length 95th (m)	161.5		3.0	46.3	13.4	
Internal Link Dist (m)	128.4			234.2	63.1	
Turn Bay Length (m)			65.0			
Base Capacity (vph)	2626		226	2587	478	
Starvation Cap Reductn	918		0	0	0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.83		0.04	0.27	0.13	
Intersection Summary						

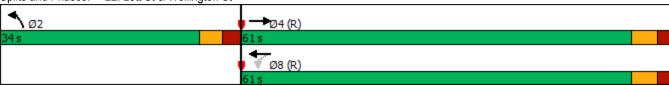
Cycle Length: 95

Actuated Cycle Length: 95

Offset: 23 (24%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.54		
Intersection Signal Delay: 15.0	Intersection LOS: B	
Intersection Capacity Utilization 58.2%	ICU Level of Service B	
Analysis Period (min) 15		
Splits and Phases: 22: Lett St & Wellington St		



	-	$\rightarrow$	•	<b>←</b>	<b>~</b>	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			414	W	
Traffic Volume (vph)	888	8	5	376	8	11
Future Volume (vph)	888	8	5	376	8	11
Satd. Flow (prot)	3417	0	0	3417	1621	0
Flt Permitted		•	•	0.945	0.980	-
Satd. Flow (perm)	3417	0	0	3232	1621	0
Satd. Flow (RTOR)	1				12	
Lane Group Flow (vph)	943	0	0	401	20	0
Turn Type	NA		Perm	NA	Perm	
Protected Phases	4			8		
Permitted Phases			8		2	
Detector Phase	4		8	8	2	
Switch Phase	•				<del>-</del>	
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	40.1		40.1	40.1	28.3	
Total Split (s)	81.0		81.0	81.0	39.0	
Total Split (%)	67.5%		67.5%	67.5%	32.5%	
Yellow Time (s)	3.3		3.3	3.3	3.3	
All-Red Time (s)	3.8		3.8	3.8	3.0	
Lost Time Adjust (s)	0.0		0.0	-3.1	-2.3	
Total Lost Time (s)	7.1			4.0	4.0	
Lead/Lag	7.1			1.0	1.0	
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	110.6		J	111.9	12.3	
Actuated g/C Ratio	0.92			0.93	0.10	
v/c Ratio	0.30			0.13	0.10	
Control Delay	4.2			1.1	31.5	
Queue Delay	0.0			0.0	0.0	
Total Delay	4.2			1.1	31.5	
LOS	A.2			A	C	
Approach Delay	4.2			1.1	31.5	
Approach LOS	4.2 A			Α	01.5 C	
Queue Length 50th (m)	0.0			0.0	1.8	
Queue Length 95th (m)	m85.5			10.4	9.7	
Internal Link Dist (m)	168.5			265.3	49.5	
Turn Bay Length (m)	100.0			200.0	70.0	
Base Capacity (vph)	3150			3013	481	
Starvation Cap Reductn	0			0	0	
Spillback Cap Reductn	0			0	0	
Storage Cap Reductn	0			0	0	
Reduced v/c Ratio	0.30			0.13	0.04	
	0.00			0.10	0.04	
Intersection Summary						

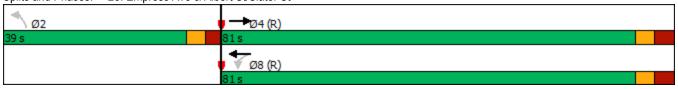
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 39 (33%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.30
Intersection Signal Delay: 3.7
Intersection Capacity Utilization 43.8%
ICU Level of Service A
Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St/Slater St



	•	•	<b>†</b>	~	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>∱</b> }			<b>^</b>
Traffic Volume (vph)	0	9	720	10	0	886
Future Volume (vph)	0	9	720	10	0	886
Satd. Flow (prot)	0	1557	3413	0	0	3420
Flt Permitted						
Satd. Flow (perm)	0	1557	3413	0	0	3420
Lane Group Flow (vph)	0	9	769	0	0	933
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 31.3%			ICI	U Level c	of Service
Analysis Period (min) 15						

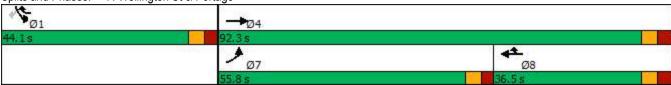
	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	795	263	579	1105	852	449
Future Volume (vph)	795	263	579	1105	852	449
Satd. Flow (prot)	3317	4914	3386	2614	3285	1530
Flt Permitted	0.950				0.950	,,,,,
Satd. Flow (perm)	3285	4914	3386	2614	3168	1530
Satd. Flow (RTOR)	0_00					178
Lane Group Flow (vph)	837	277	609	1163	897	473
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	81	1	
Permitted Phases	•	•		•	•	1
Detector Phase	7	4	8	8 1	1	1
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
Total Split (s)	55.8	92.3	36.5		44.1	44.1
Total Split (%)	40.9%	67.7%	26.8%		32.3%	32.3%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.8	6.5	6.5		6.1	6.1
Lead/Lag	Lead	0.0	Lag		0.1	0.1
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Max		Max	Max
Act Effct Green (s)	36.1	72.0	30.1	74.3	38.1	38.1
Actuated g/C Ratio	0.29	0.59	0.25	0.61	0.31	0.31
v/c Ratio	0.86	0.10	0.73	0.74	0.88	0.79
Control Delay	50.4	11.0	49.7	22.0	52.1	35.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.4	11.0	49.7	22.0	52.1	35.4
LOS	D	В	D	C	D	D
Approach Delay	<u> </u>	40.6	31.5	U	46.3	
Approach LOS		70.0 D	C C		70.5 D	
Queue Length 50th (m)	103.0	10.6	75.4	113.4	111.5	71.5
Queue Length 95th (m)	127.5	14.8	106.8	174.7		
Internal Link Dist (m)	141.0	240.3	292.7	177.7	47.6	π 1 <b>-1</b> 0.0
Turn Bay Length (m)	150.0	270.0	LUL.I		77.0	30.0
Base Capacity (vph)	1354	3443	829	1582	1019	597
Starvation Cap Reductn	0	0	029	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.08	0.73	0.74	0.88	0.79
Nouvoeu vio Natio	0.02	0.00	0.13	0.74	0.00	0.13

Cycle Length: 136.4 Actuated Cycle Length: 122.8

Natural Cycle: 115 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.88

Intersection Signal Delay: 38.7 Intersection LOS: D
Intersection Capacity Utilization 81.8% ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 7: Wellington St & Portage



	۶	<b>→</b>	•	•	<b>←</b>	•	4	†	/	<b>/</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>		ሻ	ĵ.		ሻ	1>		ሻ	f)	
Traffic Volume (vph)	0	0	0	0	0	0	0	915	0	0	834	0
Future Volume (vph)	0	0	0	0	0	0	0	915	0	0	834	0
Satd. Flow (prot)	1800	1800	0	1800	1800	0	1800	1800	0	1800	1800	0
Flt Permitted												
Satd. Flow (perm)	1800	1800	0	1800	1800	0	1800	1800	0	1800	1800	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	963	0	0	878	0
Turn Type	Perm	•	-	Perm	-	•	Perm	NA	-	Perm	NA	
Protected Phases		4		. •	8			2			6	
Permitted Phases	4	•		8			2	_		6	•	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	•	•					_	_			· ·	
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
( )	0.2	0.2		0.2	0.2		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize? Recall Mode	None	None		Mono	None		C-Max	C-Max		C-Max	C-Max	
	ivone	None		None	ivone		C-IVIAX			C-IVIAX		
Act Effet Green (s)								75.0			75.0	
Actuated g/C Ratio								1.00			1.00	
v/c Ratio								0.54			0.49	
Control Delay								1.1			0.9	
Queue Delay								0.0			0.0	
Total Delay								1.1			0.9	
LOS								A			A	
Approach Delay								1.1			0.9	
Approach LOS								A			A	
Queue Length 50th (m)								0.0			0.0	
Queue Length 95th (m)		22.2			10.0			0.0			0.0	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)												
Base Capacity (vph)								1800			1800	
Starvation Cap Reductn								0			0	
Spillback Cap Reductn								0			0	
Storage Cap Reductn								0			0	
Reduced v/c Ratio								0.54			0.49	
Intersection Summary												
Cycle Length: 75												
Actuated Cycle Length: 75												

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 75

Maximum v/c Ratio: 0.54							
Intersection Signal Delay: 1.1	Intersection LOS: A						
Intersection Capacity Utilization 55.8%	ICU Level of Service B						
Analysis Period (min) 15							
Splits and Phases: 10: Booth St & Chaudiere	<b>1</b> 04						
53 s	22 s						
Ø6 (R)	▼ø8						

	۶	<b>→</b>	$\searrow$	•	<b>←</b>	•	1	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>↑</b> ↑		ች	<b>^</b>	7
Traffic Volume (vph)	0	840	0	0	1239	53	0	842	168	34	531	173
Future Volume (vph)	0	840	0	0	1239	53	0	842	168	34	531	173
Satd. Flow (prot)	0	3420	0	0	3420	1530	0	3221	0	1613	3196	1515
Flt Permitted										0.096		
Satd. Flow (perm)	0	3420	0	0	3420	1480	0	3221	0	163	3196	1471
Satd. Flow (RTOR)								21				35
Lane Group Flow (vph)	0	884	0	0	1304	56	0	1063	0	36	559	182
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase												
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.8		12.0	31.9	31.9
Total Split (s)		60.0			60.0	60.0		48.0		12.0	60.0	60.0
Total Split (%)	;	50.0%			50.0%	50.0%		40.0%		10.0%	50.0%	50.0%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)		6.8			6.8	6.8		6.8		6.8	6.9	6.9
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		50.8			50.8	50.8		48.2		55.6	55.5	55.5
Actuated g/C Ratio		0.42			0.42	0.42		0.40		0.46	0.46	0.46
v/c Ratio		0.61			0.90	0.09		0.81		0.25	0.38	0.26
Control Delay		26.6			37.9	15.6		33.1		23.2	22.5	17.6
Queue Delay		0.0			1.1	0.0		0.0		0.0	0.0	0.0
Total Delay		26.6			39.0	15.6		33.1		23.2	22.5	17.6
LOS		С			D	В		С		С	С	В
Approach Delay		26.6			38.0			33.1			21.3	
Approach LOS		С			D			С			С	
Queue Length 50th (m)		85.7			152.1	8.1		126.1		4.9	47.8	22.0
Queue Length 95th (m)		105.7			183.3	11.7	n	n#163.3		11.5	63.2	39.2
Internal Link Dist (m)		331.0			119.3			51.3			192.9	
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1516			1516	656		1307		143	1477	698
Starvation Cap Reductn		0			70	0		0		0	0	0
Spillback Cap Reductn		0			0	0		0		0	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.58			0.90	0.09		0.81		0.25	0.38	0.26

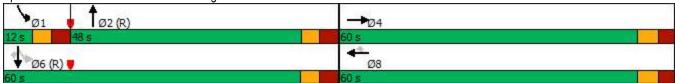
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 3 (3%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.90
Intersection Signal Delay: 31.1 Intersection LOS: C
Intersection Capacity Utilization 77.9% ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Booth St & Wellington St



	٠	<b>→</b>	•	•	<b>←</b>	*	1	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b></b>	7	*	<b>^</b>	7		4TÞ		ሻ	<b></b>	7
Traffic Volume (vph)	462	443	29	19	675	217	10	490	34	79	370	190
Future Volume (vph)	462	443	29	19	675	217	10	490	34	79	370	190
Satd. Flow (prot)	1676	1765	1485	1710	3386	1530	0	3320	0	1629	1782	1515
Flt Permitted	0.163			0.495				0.942		0.207		
Satd. Flow (perm)	281	1765	1451	886	3386	1360	0	3128	0	343	1782	1381
Satd. Flow (RTOR)			91					6				
Lane Group Flow (vph)	486	466	31	20	711	228	0	563	0	83	389	200
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		4.5	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	29.0	70.0	70.0	41.0	41.0	41.0	38.0	38.0		12.0	50.0	50.0
Total Split (%)	24.2%	58.3%	58.3%	34.2%	34.2%	34.2%	31.7%	31.7%		10.0%	41.7%	41.7%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		None	Min	Min
Act Effct Green (s)	69.6	69.6	72.1	34.5	34.5	34.5		27.8		37.4	37.4	37.4
Actuated g/C Ratio	0.58	0.58	0.60	0.29	0.29	0.29		0.23		0.31	0.31	0.31
v/c Ratio	0.98	0.46	0.03	0.08	0.73	0.58		0.77		0.50	0.70	0.47
Control Delay	59.2	13.1	0.1	32.9	42.3	42.4		50.3		32.4	37.1	29.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	59.2	13.1	0.1	32.9	42.3	42.4		50.3		32.4	37.1	29.5
LOS	E	В	Α	С	D	D		D		С	D	С
Approach Delay		35.5			42.1			50.3			34.3	
Approach LOS		D			D	40.0		D			С	4= 0
Queue Length 50th (m)	~107.7	72.3	0.0	3.6	72.1	43.9		68.2		17.1	97.4	45.9
Queue Length 95th (m)	#185.2	93.8	m0.2	10.2	86.9	63.2		86.1		31.9	133.1	72.6
Internal Link Dist (m)	400.0	285.1	40.0	40.0	168.5	05.0		37.2			83.1	400.0
Turn Bay Length (m)	160.0	4000	40.0	40.0	070	95.0		005		400	0.45	120.0
Base Capacity (vph)	495	1023	908	254	973	391		825		166	645	500
Starvation Cap Reductn	0	0	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	0.98	0.46	0.03	0.08	0.73	0.58		0.68		0.50	0.60	0.40

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 9 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

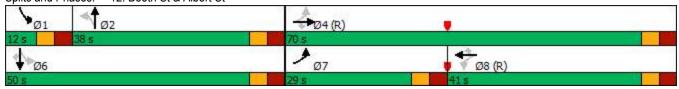
Maximum v/c Ratio: 0.98
Intersection Signal Delay: 39.8
Intersection LOS: D
Intersection Capacity Utilization 119.5%
ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	7	ሻ	<b>^</b>	ሻ	7
Traffic Volume (vph)	530	106	261	708	125	330
Future Volume (vph)	530	106	261	708	125	330
Satd. Flow (prot)	1782	1500	1676	3386	1629	1485
Flt Permitted	1702	1000	0.339	0000	0.950	1100
Satd. Flow (perm)	1782	1412	598	3386	1590	1441
Satd. Flow (RTOR)	1102	34	- 555	- 5555	1300	334
Lane Group Flow (vph)	558	112	275	745	132	347
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4	1 01111	3	8	2	1 01111
Permitted Phases		4	8	U		2
Detector Phase	4	4	3	8	2	2
Switch Phase	7	7	J			
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	31.8	31.8	11.2	31.8	29.3	29.3
Total Split (s)	61.0	61.0	16.0	77.0	43.0	43.0
Total Split (%)	50.8%	50.8%	13.3%	64.2%	35.8%	35.8%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
\ /	3.5	3.5	2.9	3.5	3.0	3.0
All-Red Time (s)	0.0	-2.8	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.8		6.2		6.3	
Total Lost Time (s)		4.0		6.8	0.3	6.3
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes	O M	N 4 !	N 4:
Recall Mode	C-Max	C-Max	None	C-Max	Min	Min
Act Effct Green (s)	73.4	76.2	92.2	91.6	15.3	15.3
Actuated g/C Ratio	0.61	0.64	0.77	0.76	0.13	0.13
v/c Ratio	0.51	0.12	0.49	0.29	0.64	0.73
Control Delay	14.5	5.6	8.9	7.4	63.0	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	5.6	8.9	7.4	63.0	15.6
LOS	В	Α	Α	A	Е	В
Approach Delay	13.0			7.8	28.6	
Approach LOS	В			Α	С	
Queue Length 50th (m)	73.4	2.6	31.3	55.7	31.6	2.9
Queue Length 95th (m)	132.3	12.1	m70.5	92.6	50.3	33.1
Internal Link Dist (m)	82.0			285.1	54.7	
Turn Bay Length (m)		16.0	90.0			
Base Capacity (vph)	1089	908	569	2584	498	672
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	98	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.51	0.12	0.48	0.30	0.27	0.52

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 81 (68%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 80

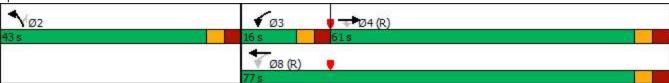
Maximum v/c Ratio: 0.73

Intersection Signal Delay: 14.0	Intersection LOS: B
Intersection Capacity Utilization 70.5%	ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	•	-	←	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	<b>^</b>	<b>^</b>		W	
Traffic Volume (vph)	2	918	1505	7	5	22
Future Volume (vph)	2	918	1505	7	5	22
Satd. Flow (prot)	1710	3386	3416	0	1586	0
Flt Permitted	0.143				0.991	
Satd. Flow (perm)	257	3386	3416	0	1585	0
Satd. Flow (RTOR)			1		23	
Lane Group Flow (vph)	2	966	1591	0	28	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases		4	8		6	
Permitted Phases	4					
Detector Phase	4	4	8		6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	37.0	37.0	37.0		32.3	
Total Split (s)	87.0	87.0	87.0		33.0	
Total Split (%)	72.5%	72.5%	72.5%		27.5%	
Yellow Time (s)	3.7	3.7	3.7		3.3	
All-Red Time (s)	2.3	2.3	2.3		3.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.3	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Max		None	
Act Effct Green (s)	106.6	106.6	106.6		10.0	
Actuated g/C Ratio	0.89	0.89	0.89		0.08	
v/c Ratio	0.01	0.32	0.52		0.18	
Control Delay	2.0	2.3	3.5		25.8	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	2.0	2.3	3.5		25.8	
LOS	A	A	Α		С	
Approach Delay		2.3	3.5		25.8	
Approach LOS		A	Α		С	
Queue Length 50th (m)	0.1	25.8	27.2		1.2	
Queue Length 95th (m)	0.6	32.0	49.8		10.7	
Internal Link Dist (m)		651.1	331.0		21.1	
Turn Bay Length (m)	40.0					
Base Capacity (vph)	228	3008	3035		370	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.01	0.32	0.52		0.08	
Intersection Summary						

Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

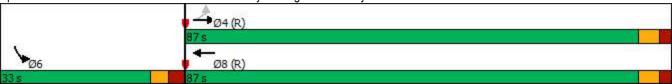
Offset: 37 (31%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 75

14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.52
Intersection Signal Delay: 3.3
Intersection Capacity Utilization 62.7%
ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	1	<b>†</b>	<b>/</b>	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>†</b>			f.	
Traffic Volume (vph)	0	969	4	2	1659	0	26	14	35	3	26	11
Future Volume (vph)	0	969	4	2	1659	0	26	14	35	3	26	11
Satd. Flow (prot)	0	3383	0	0	3420	0	0	1642	0	0	1669	0
Flt Permitted					0.954			0.870			0.969	
Satd. Flow (perm)	0	3383	0	0	3263	0	0	1450	0	0	1623	0
Satd. Flow (RTOR)		1						37			12	
Lane Group Flow (vph)	0	1024	0	0	1748	0	0	79	0	0	42	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		61.7			61.7			10.3			10.3	
Actuated g/C Ratio		0.78			0.78			0.13			0.13	
v/c Ratio		0.39			0.69			0.36			0.19	
Control Delay		4.4			7.8			24.0			25.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		4.4			7.8			24.0			25.9	
LOS		Α			A			С			С	
Approach Delay		4.4			7.8			24.0			25.9	
Approach LOS		A			A			С			С	
Queue Length 50th (m)		27.1			69.6			5.9			4.2	
Queue Length 95th (m)		39.1			102.2			18.3			13.1	
Internal Link Dist (m)		121.2			288.5			33.4			167.1	
Turn Bay Length (m)		2224			0540			500			570	
Base Capacity (vph)		2634			2540			532			576	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.39			0.69			0.15			0.07	

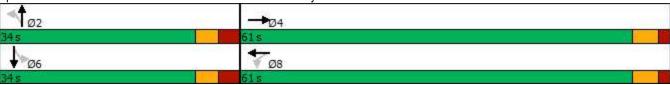
Cycle Length: 95

Actuated Cycle Length: 79.3

Natural Cycle: 90 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.69

Intersection Signal Delay: 7.3 Intersection LOS: A
Intersection Capacity Utilization 73.6% ICU Level of Service D
Analysis Period (min) 15

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	٠	<b>→</b>	$\rightarrow$	•	•	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	î»		7	<b>†</b>	7	7	<b>^</b>	7	7	£	_
Traffic Volume (vph)	10	448	88	64	621	90	88	185	65	41	68	11
Future Volume (vph)	10	448	88	64	621	90	88	185	65	41	68	11
Satd. Flow (prot)	1555	1703	0	1710	1782	1530	1710	1800	1485	1710	1745	0
Flt Permitted	0.349			0.402			0.702			0.509		
Satd. Flow (perm)	555	1703	0	710	1782	1340	1212	1800	1388	887	1745	0
Satd. Flow (RTOR)		18				87			61		8	
Lane Group Flow (vph)	11	565	0	67	654	95	93	195	68	43	84	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	-2.5	0.0	0.0	-2.4	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	4.0	6.4	6.4	4.0	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	69.1	69.1		69.1	69.1	71.6	18.0	18.0	20.4	18.0	18.0	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.72	0.18	0.18	0.20	0.18	0.18	
v/c Ratio	0.03	0.48		0.14	0.53	0.10	0.43	0.60	0.21	0.27	0.26	
Control Delay	11.6	19.0		7.7	10.7	2.0	40.6	44.5	10.4	37.0	31.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.6	19.0		7.7	10.7	2.0	40.6	44.5	10.4	37.0	31.7	
LOS	В	В		Α	В	Α	D	D	В	D	С	
Approach Delay		18.9			9.4			37.0			33.5	
Approach LOS		В			Α			D			С	
Queue Length 50th (m)	1.2	92.4		3.8	53.0	0.4	17.5	37.9	1.2	7.8	13.8	
Queue Length 95th (m)	m2.5	m132.0		11.6	106.6	5.9	30.1	55.2	11.4	16.7	25.0	
Internal Link Dist (m)		635.7			497.2			83.8			130.2	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	383	1182		490	1231	984	310	460	432	227	452	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.03	0.48		0.14	0.53	0.10	0.30	0.42	0.16	0.19	0.19	

Cycle Length: 100 Actuated Cycle Length: 100

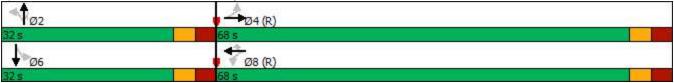
Offset: 65 (65%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 65

Maximum v/c Ratio: 0.60
Intersection Signal Delay: 19.2
Intersection Capacity Utilization 88.4%
ICU Level of Service E
Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Scott St/Albert St & Bayview Station Rd



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ሻ	<b>†</b>	7	Ţ	f)		7	ĵ.	
Traffic Volume (vph)	227	462	86	55	615	62	40	546	47	56	233	210
Future Volume (vph)	227	462	86	55	615	62	40	546	47	56	233	210
Satd. Flow (prot)	1693	1765	1515	1710	1800	1530	1710	1756	0	1710	1565	0
Flt Permitted	0.118			0.486			0.290			0.121		
Satd. Flow (perm)	210	1765	1240	802	1800	1222	507	1756	0	212	1565	0
Satd. Flow (RTOR)			81			119		5			53	
Lane Group Flow (vph)	239	486	91	58	647	65	42	624	0	59	466	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	10.6	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	15.0	49.0	49.0	34.0	34.0	34.0	45.0	45.0		45.0	45.0	
Total Split (%)	15.0%	49.0%	49.0%	34.0%	34.0%	34.0%	45.0%	45.0%		45.0%	45.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.3	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	-2.1	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	6.1	4.0	6.1	4.0	4.0	6.3	6.3		6.3	6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		Min	Min	
Act Effct Green (s)	50.6	50.1	52.2	28.5	30.6	30.6	37.5	37.5		37.5	37.5	
Actuated g/C Ratio	0.51	0.50	0.52	0.28	0.31	0.31	0.38	0.38		0.38	0.38	
v/c Ratio	0.69	0.55	0.13	0.25	1.18	0.14	0.22	0.95		0.75	0.75	
Control Delay	31.9	20.5	4.1	28.3	125.3	1.5	24.4	54.7		79.5	32.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	31.9	20.5	4.1	28.3	125.3	1.5	24.4	54.7		79.5	32.6	
LOS	С	С	Α	С	F	Α	С	D		Е	С	
Approach Delay		22.0			107.5			52.8			37.8	
Approach LOS		С			F			D			D	
Queue Length 50th (m)	30.1	67.2	1.0	6.9	~162.9	0.0	5.6	118.0		10.1	71.3	
Queue Length 95th (m)	#64.1	99.3	8.9	m14.1	#227.4	m0.6	14.4	#189.2		#34.6	111.4	
Internal Link Dist (m)		92.9			635.7		_	70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	345	885	686	228	550	455	196	682		82	638	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.69	0.55	0.13	0.25	1.18	0.14	0.21	0.91		0.72	0.73	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 8 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 110

Lane Group	Ø9
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Satd. Flow (RTOR)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	9
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	2.0
Minimum Split (s)	6.0
Total Split (s)	6.0
Total Split (%)	6%
Yellow Time (s)	2.0
All-Red Time (s)	0.0
Lost Time Adjust (s)	<u> </u>
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode	None
Act Effct Green (s)	INUITE
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

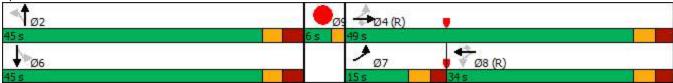
Maximum v/c Ratio: 1.18
Intersection Signal Delay: 56.1 Intersection LOS: E
Intersection Capacity Utilization 108.0% ICU Level of Service G
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	•	•	1	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	¥		ሻ	<b>^</b>	<b>†</b>	02.1	~~	
Traffic Volume (vph)	1	8	8	914	826	10		
Future Volume (vph)	1	8	8	914	826	10		
Satd. Flow (prot)	1554	0	1710	3420	3413	0		
Flt Permitted	0.994	-	0.310					
Satd. Flow (perm)	1551	0	558	3420	3413	0		
Satd. Flow (RTOR)	8	-			2			
Lane Group Flow (vph)	9	0	8	962	880	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	3.8		10.0	10.0	10.0		10.0	
Minimum Split (s)	10.0		30.9	30.9	30.9		29.0	
Total Split (s)	17.0		49.0	49.0	49.0		29.0	
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.9	5.9	5.9			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	5.9		85.4	85.4	85.4			
Actuated g/C Ratio	0.06		0.90	0.90	0.90			
v/c Ratio	0.09		0.02	0.31	0.29			
Control Delay	26.9		7.0	5.0	4.8			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	26.9		7.0	5.0	4.8			
LOS	С		Α	Α	Α			
Approach Delay	26.9			5.0	4.8			
Approach LOS	С			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	5.0		3.3	84.9	75.5			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	183		502	3074	3068			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.05		0.02	0.31	0.29			
Intono attan O								

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 10 (11%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.31								
Intersection Signal Delay: 5.0	Intersection LOS: A							
Intersection Capacity Utilization 40.1%	ICU Level of Service A							
Analysis Period (min) 15								
Splits and Phases: 19: Booth St & War Museum	<b>≯</b> <sub>Ø4</sub>	<b>●</b> Ø9						
49 s	17 s	29 s						

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ň	<b>†</b>	7	¥	f)			4	
Traffic Volume (vph)	1	455	76	84	647	0	57	4	80	3	0	0
Future Volume (vph)	1	455	76	84	647	0	57	4	80	3	0	0
Satd. Flow (prot)	855	1782	1530	1693	1765	1200	1644	1500	0	0	1710	0
Flt Permitted	0.319			0.490			0.756				0.700	
Satd. Flow (perm)	287	1782	1530	873	1765	1200	1308	1500	0	0	1260	0
Satd. Flow (RTOR)			80					84				
Lane Group Flow (vph)	1	479	80	88	681	0	60	88	0	0	3	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	16.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	95.5	95.5	97.8	92.2	92.2		11.9	11.9			11.2	
Actuated g/C Ratio	0.80	0.80	0.82	0.77	0.77		0.10	0.10			0.09	
v/c Ratio	0.00	0.34	0.06	0.13	0.50		0.46	0.39			0.03	
Control Delay	3.0	4.4	0.7	1.9	7.9		62.2	16.5			48.3	
Queue Delay	0.0	0.0	0.0	0.0	0.1		0.0	0.0			0.0	
Total Delay	3.0	4.4	0.7	1.9	8.0		62.2	16.5			48.3	
LOS	А	Α	Α	Α	Α		Е	В			D	
Approach Delay		3.8			7.3			35.0			48.3	
Approach LOS		Α			Α			D			D	
Queue Length 50th (m)	0.1	25.5	0.0	0.2	40.1		14.4	0.9			0.7	
Queue Length 95th (m)	0.5	46.0	3.0	6.6	211.5		28.1	16.5			3.8	
Internal Link Dist (m)		497.2			111.3			178.8			36.6	
Turn Bay Length (m)	30.0		50.0	35.0			30.0					
Base Capacity (vph)	278	1418	1261	670	1356		323	434			311	
Starvation Cap Reductn	0	0	0	0	65		0	0			0	
Spillback Cap Reductn	0	0	0	0	0		0	0			0	
Storage Cap Reductn	0	0	0	0	0		0	0			0	
Reduced v/c Ratio	0.00	0.34	0.06	0.13	0.53		0.19	0.20			0.01	

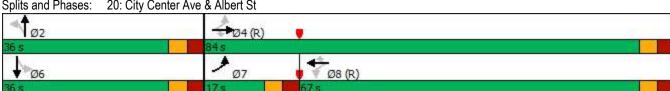
Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.50	
Intersection Signal Delay: 8.8	Intersection LOS: A
Intersection Capacity Utilization 68.4%	ICU Level of Service C
Analysis Period (min) 15	
Splits and Phases: 20: City Contar Ava & Albert St	



	<b>→</b>	•	•	•	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> Ъ		ች	<b>^</b>	W	
Traffic Volume (vph)	937	26	27	1094	30	26
Future Volume (vph)	937	26	27	1094	30	26
Satd. Flow (prot)	3364	0	1710	3386	1605	0
Flt Permitted			0.271		0.974	
Satd. Flow (perm)	3364	0	484	3386	1604	0
Satd. Flow (RTOR)	5				27	
Lane Group Flow (vph)	1013	0	28	1152	59	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		22.0	22.0	33.9	
Total Split (s)	86.0		86.0	86.0	34.0	
Total Split (%)	71.7%		71.7%	71.7%	28.3%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	99.0		99.0	99.0	13.6	
Actuated g/C Ratio	0.82		0.82	0.82	0.11	
v/c Ratio	0.36		0.07	0.41	0.29	
Control Delay	1.6		4.8	4.9	31.2	
Queue Delay	0.1		0.0	0.1	0.0	
Total Delay	1.6		4.8	5.1	31.2	
LOS	A		A	A	C	
Approach Delay	1.6		, ,	5.1	31.2	
Approach LOS	A			A	C	
Queue Length 50th (m)	7.5		1.1	32.0	7.6	
Queue Length 95th (m)	19.3		5.9	84.6	17.8	
Internal Link Dist (m)	119.3		0.0	240.3	83.4	
Turn Bay Length (m)	. 10.0		65.0	0.0	50.1	
Base Capacity (vph)	2777		399	2794	396	
Starvation Cap Reductn	454		0	0	0	
Spillback Cap Reductn	0		0	605	4	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.44		0.07	0.53	0.15	
	0.11		0.01	0.00	0.10	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 27 (23%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 65

Maximum v/c Ratio: 0.41	
Intersection Signal Delay: 4.2	Intersection LOS: A
Intersection Capacity Utilization 52.3%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 22: Lett St & Wellington St



	<b>→</b>	$\rightarrow$	•	<b>←</b>	<b>~</b>	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			41	¥	
Traffic Volume (vph)	540	16	11	891	9	14
Future Volume (vph)	540	16	11	891	9	14
Satd. Flow (prot)	3406	0	0	3417	1619	0
Flt Permitted	3100			0.946	0.982	
Satd. Flow (perm)	3406	0	0	3235	1619	0
Satd. Flow (RTOR)	5			0200	15	
Lane Group Flow (vph)	585	0	0	950	24	0
Turn Type	NA	- 0	Perm	NA	Perm	- 0
Protected Phases	4		i Giiii	8	i Giiii	
Permitted Phases	4		9	U	2	
	4		8	8	2	
Detector Phase	4		ď	Ŏ		
Switch Phase	40.0		10.0	40.0	10.0	
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	40.1		40.1	40.1	28.3	
Total Split (s)	81.0		81.0	81.0	39.0	
Total Split (%)	67.5%		67.5%	67.5%	32.5%	
Yellow Time (s)	3.3		3.3	3.3	3.3	
All-Red Time (s)	3.8		3.8	3.8	3.0	
Lost Time Adjust (s)	0.0			-3.1	-2.3	
Total Lost Time (s)	7.1			4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	106.0			107.8	12.3	
Actuated g/C Ratio	0.88			0.90	0.10	
v/c Ratio	0.19			0.33	0.13	
Control Delay	6.6			2.0	30.0	
Queue Delay	0.0			0.0	0.0	
Total Delay	6.6			2.0	30.0	
LOS	A			A	С	
Approach Delay	6.6			2.0	30.0	
Approach LOS	A			A	C	
Queue Length 50th (m)	42.4			22.5	2.1	
Queue Length 95th (m)	60.9			28.1	10.7	
Internal Link Dist (m)	168.5			265.3	48.5	
Turn Bay Length (m)	100.0			200.0	70.0	
Base Capacity (vph)	3008			2906	482	
Starvation Cap Reductn	0			2300	0	
Spillback Cap Reductn	0			0	0	
	0			0	0	
Storage Cap Reductn						
Reduced v/c Ratio	0.19			0.33	0.05	
Intersection Summary						

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 39 (33%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.33							
Intersection Signal Delay: 4.1		Intersection LOS: A					
Intersection Capacity Utilization 4	9.2%	ICU Level of Service A					
Analysis Period (min) 15							
Splits and Phases: 23: Empres	s Ave & Albert St/Slater St						
39 s	81s						
	Ø8 (R)						

	•	•	<b>†</b>	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>↑</b> ↑			<b>^</b>
Traffic Volume (vph)	0	8	1062	9	0	629
Future Volume (vph)	0	8	1062	9	0	629
Satd. Flow (prot)	0	1557	3417	0	0	3420
Flt Permitted						
Satd. Flow (perm)	0	1557	3417	0	0	3420
Lane Group Flow (vph)	0	8	1127	0	0	662
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized					<u> </u>	
Intersection Capacity Utiliza	tion 41.3%			ICI	U Level c	of Service
Analysis Period (min) 15						

**Synchro Modelling Outputs – Future Background Conditions** 



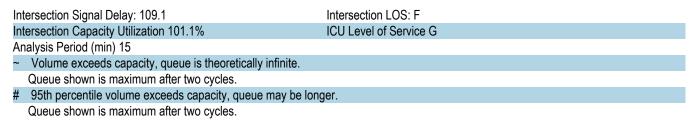
	•	<b>→</b>	<b>←</b>	1	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	601	967	324	704	1962	310
Future Volume (vph)	601	967	324	704	1962	310
Satd. Flow (prot)	3317	4914	3420	2693	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3302	4914	3420	2693	3272	1501
Satd. Flow (RTOR)						69
Lane Group Flow (vph)	633	1018	341	741	2065	326
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	18	1	
Permitted Phases						1
Detector Phase	7	4	8	18	1	1
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		45.1	45.1
Total Split (s)	42.8	70.3	27.5		51.1	51.1
Total Split (%)	35.3%	57.9%	22.7%		42.1%	42.1%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		-2.1	0.0
Total Lost Time (s)	5.8	6.5	6.5		4.0	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Min		Max	Max
Act Effct Green (s)	26.2	50.9	18.9	70.7	47.4	45.3
Actuated g/C Ratio	0.24	0.47	0.17	0.65	0.44	0.42
v/c Ratio	0.79	0.44	0.58	0.42	1.45	0.49
Control Delay	46.8	19.8	46.3	10.9	231.5	22.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.8	19.8	46.3	10.9	231.5	22.6
LOS	D	В	D	В	F	С
Approach Delay		30.1	22.1		203.0	
Approach LOS		С	С		F	
Queue Length 50th (m)	70.3	53.9	36.8	41.1	~332.2	42.1
Queue Length 95th (m)	90.4	64.7	58.0	71.7	#427.2	80.5
Internal Link Dist (m)		234.2	291.0		48.7	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1134	2897	663	1734	1429	664
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.56	0.35	0.51	0.43	1.45	0.49
rtoduood v/o rtatio	0.00	0.00	0.01	0.70	1.70	0.40

Cycle Length: 121.4

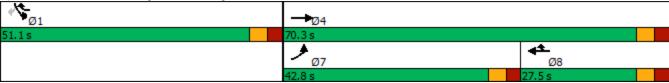
Actuated Cycle Length: 108.9

Natural Cycle: 145

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.45



Splits and Phases: 7: Wellington St & Portage



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	~	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		*	f)		7	f)		7	ĵ.	
Traffic Volume (vph)	5	0	5	0	0	0	37	821	0	0	1454	37
Future Volume (vph)	5	0	5	0	0	0	37	821	0	0	1454	37
Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1793	0
Flt Permitted							0.066					
Satd. Flow (perm)	1800	1530	0	1800	1800	0	119	1800	0	1800	1793	0
Satd. Flow (RTOR)		42									3	
Lane Group Flow (vph)	5	5	0	0	0	0	39	864	0	0	1570	0
Turn Type	Perm	NA		Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	-2.2		0.0	-2.2		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	4.0		6.2	4.0		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)	10.0	12.2					70.6	70.6			70.6	
Actuated g/C Ratio	0.13	0.16					0.94	0.94			0.94	
v/c Ratio	0.02	0.02					0.35	0.51			0.93	
Control Delay	28.6	0.2					15.8	3.4			17.4	
Queue Delay	0.0	0.0					0.0	0.0			0.0	
Total Delay	28.6	0.2					15.8	3.4			17.4	
LOS	С	Α					В	Α			В	
Approach Delay		14.4						3.9			17.4	
Approach LOS		В						Α			В	
Queue Length 50th (m)	0.7	0.0					0.0	0.0			0.0	
Queue Length 95th (m)	3.6	0.0					#18.0	96.5			#379.6	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)	30.0						30.0					
Base Capacity (vph)	379	399					112	1694			1687	
Starvation Cap Reductn	0	0					0	0			0	
Spillback Cap Reductn	0	0					0	0			0	
Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio	0.01	0.01					0.35	0.51			0.93	

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 150

Maximum v/c Ratio: 0.93
Intersection Signal Delay: 12.5
Intersection Capacity Utilization 99.7%
ICU Level of Service F
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere

Ø2 (R)	<u>≯</u> <sub>04</sub>	
53 s	22 s	
● Ø6 (R)	<b>▼</b> Ø8	
53 s	22 s	

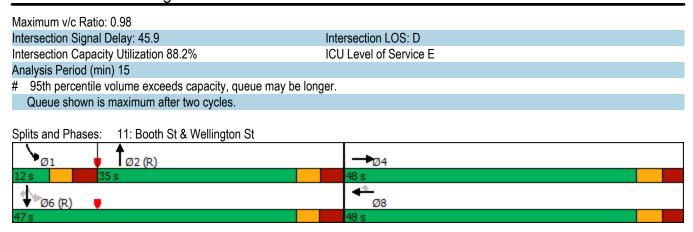
	۶	<b>→</b>	•	•	<b>←</b>	*	4	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>ተ</b> ኈ		ች	<b>^</b>	7
Traffic Volume (vph)	0	1236	0	0	640	132	0	827	131	146	982	233
Future Volume (vph)	0	1236	0	0	640	132	0	827	131	146	982	233
Satd. Flow (prot)	0	3420	0	0	3386	1530	0	3101	0	1710	3226	1530
Flt Permitted										0.114		
Satd. Flow (perm)	0	3420	0	0	3386	1476	0	3101	0	205	3226	1464
Satd. Flow (RTOR)								20				135
Lane Group Flow (vph)	0	1301	0	0	674	139	0	1009	0	154	1034	245
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase												
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.8		12.0	31.9	31.9
Total Split (s)		48.0			48.0	48.0		35.0		12.0	47.0	47.0
Total Split (%)	5	0.5%			50.5%	50.5%		36.8%		12.6%	49.5%	49.5%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	0.0		-2.8		0.0	0.0	0.0
Total Lost Time (s)		6.8			6.8	6.8		4.0		6.8	6.9	6.9
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		40.0			40.0	40.0		31.1		41.4	41.3	41.3
Actuated g/C Ratio		0.42			0.42	0.42		0.33		0.44	0.43	0.43
v/c Ratio		0.90			0.47	0.22		0.98		0.81	0.74	0.34
Control Delay		32.8			29.9	25.5		56.0		56.5	32.1	15.8
Queue Delay		0.9			0.0	0.0		30.3		57.7	0.0	0.0
Total Delay		33.6			29.9	25.5		86.3		114.2	32.1	15.8
LOS		С			С	С		F		F	С	В
Approach Delay		33.6			29.1			86.3			38.2	
Approach LOS		С			С	212		F		40.4	D	40.0
Queue Length 50th (m)		17.5			62.9	21.9		99.2		18.4	87.0	12.9
Queue Length 95th (m)		152.8			83.6	39.4		#145.2		#54.7	136.0	63.0
Internal Link Dist (m)	į	31.0			128.4	=0.0		47.8		445.0	192.9	
Turn Bay Length (m)		4.400			4.400	70.0		4000		145.0	4.400	55.0
Base Capacity (vph)		1483			1468	640		1029		189	1403	712
Starvation Cap Reductn		0			0	0		0		0	0	0
Spillback Cap Reductn		47			0	0		93		48	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.91			0.46	0.22		1.08		1.09	0.74	0.34

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 31 (33%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90



	•	<b>→</b>	•	•	•	•	1	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	*	<b>†</b> †	7		€î∌		, j	<b>†</b>	7
Traffic Volume (vph)	389	772	19	7	258	152	6	540	30	170	477	436
Future Volume (vph)	389	772	19	7	258	152	6	540	30	170	477	436
Satd. Flow (prot)	1660	1748	1443	1710	3196	1378	0	3338	0	1425	1782	1471
Flt Permitted	0.462			0.132				0.949		0.185		
Satd. Flow (perm)	769	1748	1377	238	3196	1251	0	3166	0	268	1782	1313
Satd. Flow (RTOR)			91					5				
Lane Group Flow (vph)	409	813	20	7	272	160	0	606	0	179	502	459
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		5.0	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	18.0	55.0	55.0	37.0	37.0	37.0	40.0	40.0		25.0	65.0	65.0
Total Split (%)	15.0%	45.8%	45.8%	30.8%	30.8%	30.8%	33.3%	33.3%		20.8%	54.2%	54.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	None	C-Min	C-Min	C-Max	C-Max	C-Max	Min	Min		None	None	None
Act Effct Green (s)	56.8	56.8	59.3	30.5	30.5	30.5		28.5		50.2	50.2	50.2
Actuated g/C Ratio	0.47	0.47	0.49	0.25	0.25	0.25		0.24		0.42	0.42	0.42
v/c Ratio	0.80	0.98	0.03	0.12	0.33	0.50		0.80		0.69	0.67	0.84
Control Delay	33.9	48.3	0.1	38.9	36.7	43.5		51.2		36.6	32.5	44.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	33.9	48.3	0.1	38.9	36.7	43.5		51.2		36.6	32.5	44.9
LOS	С	D	Α	D	D	D		D		D	С	D
Approach Delay		42.8			39.2			51.2			38.1	
Approach LOS		D			D			D			D	
Queue Length 50th (m)	55.2	116.8	0.0	1.4	29.1	34.1		74.6		29.1	99.1	100.1
Queue Length 95th (m)	m#73.8 r	m#244.7	m0.0	4.7	34.3	57.1		91.1		41.3	121.4	131.0
Internal Link Dist (m)		285.1			168.5			37.2			86.5	
Turn Bay Length (m)	160.0		40.0	40.0		95.0						120.0
Base Capacity (vph)	511	827	726	60	812	317		887		290	868	640
Starvation Cap Reductn	0	0	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	0.80	0.98	0.03	0.12	0.33	0.50		0.68		0.62	0.58	0.72

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 104 (87%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 95

Maximum v/c Ratio: 0.98
Intersection Signal Delay: 42.3 Intersection LOS: D
Intersection Capacity Utilization 122.4% ICU Level of Service H
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	<b>→</b>	•	•	•	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b></b>	7	ች	<b>^</b>	ሻ	7
Traffic Volume (vph)	871	130	393	359	112	291
Future Volume (vph)	871	130	393	359	112	291
Satd. Flow (prot)	1748	1471	1629	3288	1583	1471
Flt Permitted			0.062	0200	0.950	
Satd. Flow (perm)	1748	1384	106	3288	1561	1417
Satd. Flow (RTOR)		27		0200		306
Lane Group Flow (vph)	917	137	414	378	118	306
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4	7 51111	3	8	2	1 31111
Permitted Phases	7	4	8	- 0		2
Detector Phase	4	4	3	8	2	2
Switch Phase	4	4	J	0		
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0
	31.8	31.8	11.2	31.8	29.3	29.3
Minimum Split (s)						
Total Split (s)	65.0	65.0	25.0	90.0	30.0	30.0
Total Split (%)	54.2%	54.2%	20.8%	75.0%	25.0%	25.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	3.5	3.5	2.9	3.5	3.0	3.0
Lost Time Adjust (s)	-2.8	-2.8	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	4.0	6.2	6.8	6.3	6.3
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Act Effct Green (s)	61.0	61.0	93.0	92.4	14.5	14.5
Actuated g/C Ratio	0.51	0.51	0.78	0.77	0.12	0.12
v/c Ratio	1.03	0.19	0.95	0.15	0.62	0.70
Control Delay	60.2	8.7	57.0	1.8	63.6	14.1
Queue Delay	2.8	0.0	0.0	0.0	0.0	0.0
Total Delay	63.0	8.7	57.0	1.8	63.6	14.1
LOS	E	A	E	A	E	В
Approach Delay	55.9	,,	_	30.6	27.9	
Approach LOS	E			C	C C	
Queue Length 50th (m)	~244.5	10.5	72.5	3.5	28.3	0.0
Queue Length 95th (m)	#321.4	m7.5		8.7	46.3	27.5
	78.2	1117.5	#107.4	285.1	54.7	21.5
Internal Link Dist (m)	10.2	10.0	00.0	200.1	54.7	
Turn Bay Length (m)	000	16.0	90.0	0504	240	505
Base Capacity (vph)	888	716	437	2531	312	525
Starvation Cap Reductn	7	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.04	0.19	0.95	0.15	0.38	0.58

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 150

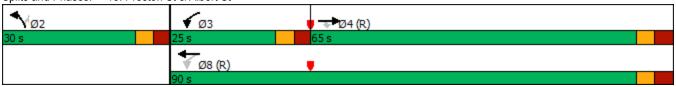
Maximum v/c Ratio: 1.03
Intersection Signal Delay: 41.9
Intersection LOS: D
Intersection Capacity Utilization 93.5%
ICU Level of Service F
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	۶	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	<b>^</b>	<b>^</b>		W	
Traffic Volume (vph)	15	1348	884	41	2	15
Future Volume (vph)	15	1348	884	41	2	15
Satd. Flow (prot)	1710	3420	3360	0	1462	0
Flt Permitted	0.292				0.994	
Satd. Flow (perm)	525	3420	3360	0	1462	0
Satd. Flow (RTOR)			8		16	
Lane Group Flow (vph)	16	1419	974	0	18	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases		4	8		6	
Permitted Phases	4					
Detector Phase	4	4	8		6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	37.0	37.0	37.0		32.3	
Total Split (s)	62.0	62.0	62.0		33.0	
Total Split (%)	65.3%	65.3%	65.3%		34.7%	
Yellow Time (s)	3.7	3.7	3.7		3.3	
All-Red Time (s)	2.3	2.3	2.3		3.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.3	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Max		None	
Act Effct Green (s)	86.1	86.1	86.1		10.0	
Actuated g/C Ratio	0.91	0.91	0.91		0.11	
v/c Ratio	0.03	0.46	0.32		0.11	
Control Delay	2.4	2.8	1.1		20.9	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	2.4	2.8	1.1		20.9	
LOS	Α	Α	Α		С	
Approach Delay		2.8	1.1		20.9	
Approach LOS		Α	Α		С	
Queue Length 50th (m)	0.0	0.0	3.0		0.4	
Queue Length 95th (m)	2.1	58.8	6.8		7.0	
Internal Link Dist (m)		651.1	331.0		21.1	
Turn Bay Length (m)	40.0					
Base Capacity (vph)	475	3099	3045		422	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.03	0.46	0.32		0.04	
Intersection Summary						

### Intersection Summary

Cycle Length: 95

Actuated Cycle Length: 95

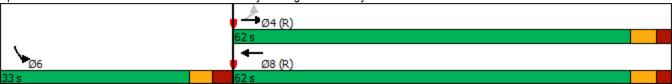
Offset: 59 (62%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 70

14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.46
Intersection Signal Delay: 2.3
Intersection Capacity Utilization 59.2%
Analysis Period (min) 15
Intersection LOS: A
ICU Level of Service B

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>†</b>			f)	
Traffic Volume (vph)	0	1457	22	1	994	1	1	21	1	1	5	3
Future Volume (vph)	0	1457	22	1	994	1	1	21	1	1	5	
Satd. Flow (prot)	0	3412	0	0	3386	0	0	1784	0	0	1699	0
Flt Permitted					0.954			0.984			0.956	
Satd. Flow (perm)	0	3412	0	0	3230	0	0	1759	0	0	1634	0
Satd. Flow (RTOR)		3						1			3	
Lane Group Flow (vph)	0	1557	0	0	1048	0	0	24	0	0	9	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		72.2			72.2			10.1			10.1	
Actuated g/C Ratio		0.90			0.90			0.13			0.13	
v/c Ratio		0.51			0.36			0.11			0.04	
Control Delay		3.3			2.5			33.2			28.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		3.3			2.5			33.2			28.7	
LOS		Α			Α			С			С	
Approach Delay		3.3			2.5			33.2			28.7	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)		0.0			0.0			3.1			8.0	
Queue Length 95th (m)		69.5			38.3			10.4			5.2	
Internal Link Dist (m)		192.5			288.5			29.2			167.1	
Turn Bay Length (m)												
Base Capacity (vph)		3067			2904			610			569	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.51			0.36			0.04			0.02	
Internación Comercia												

Cycle Length: 95

Actuated Cycle Length: 80.3

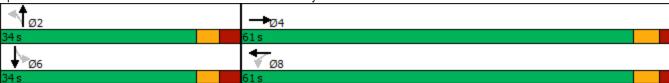
Natural Cycle: 75

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.51

Intersection Signal Delay: 3.3 Intersection LOS: A
Intersection Capacity Utilization 63.2% ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, j	ĵ»		7	<b>†</b>	7	7	<b>†</b>	7	7	<b>^</b>	
Traffic Volume (vph)	14	752	86	56	317	31	61	45	130	131	178	12
Future Volume (vph)	14	752	86	56	317	31	61	45	130	131	178	12
Satd. Flow (prot)	1500	1720	0	1613	1698	1457	1676	1800	1515	1693	1752	0
Flt Permitted	0.555			0.222			0.499			0.726		
Satd. Flow (perm)	840	1720	0	377	1698	1328	871	1800	1420	1243	1752	0
Satd. Flow (RTOR)		11				37			137		3	
Lane Group Flow (vph)	15	883	0	59	334	33	64	47	137	138	200	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	68.9	68.9		68.9	68.9	68.9	18.2	18.2	18.2	18.2	18.2	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.03	0.74		0.23	0.29	0.04	0.41	0.14	0.37	0.61	0.62	
Control Delay	3.2	10.5		10.3	7.8	2.3	41.7	32.5	8.4	48.1	44.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.2	10.5		10.3	7.8	2.3	41.7	32.5	8.4	48.1	44.8	
LOS	Α	В		В	Α	Α	D	С	Α	D	D	
Approach Delay		10.4			7.7			21.6			46.1	
Approach LOS		В			Α			С			D	
Queue Length 50th (m)	0.2	12.8		3.6	21.6	0.0	11.9	8.3	0.0	26.8	38.2	
Queue Length 95th (m)	m0.7	213.6		12.8	45.2	3.2	23.2	16.9	14.9	43.1	56.3	
Internal Link Dist (m)		635.7			497.2			83.8			141.3	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	579	1188		260	1170	926	222	460	465	318	450	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.03	0.74		0.23	0.29	0.04	0.29	0.10	0.29	0.43	0.44	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 40 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

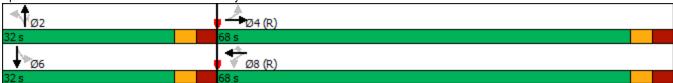
Natural Cycle: 90

Maximum v/c Ratio: 0.74

Intersection Signal Delay: 17.6	Intersection LOS: B	
Intersection Capacity Utilization 88.2%	ICU Level of Service E	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Scott St/Albert St & Bayview Station Rd



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ň	<b>†</b>	7	¥	f)		J.	f)	
Traffic Volume (vph)	120	668	37	50	281	24	31	281	69	97	354	145
Future Volume (vph)	120	668	37	50	281	24	31	281	69	97	354	145
Satd. Flow (prot)	1676	1748	1485	1676	1698	1471	1660	1722	0	1693	1564	0
Flt Permitted	0.539			0.200			0.215			0.394		
Satd. Flow (perm)	860	1748	1485	353	1698	1209	363	1722	0	702	1564	0
Satd. Flow (RTOR)			58			58		14			23	
Lane Group Flow (vph)	126	703	39	53	296	25	33	369	0	102	526	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	28.1	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	43.0	43.0		43.0	43.0	
Total Split (%)	51.0%	51.0%	51.0%	51.0%	51.0%	51.0%	43.0%	43.0%		43.0%	43.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	0.0	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	4.0	6.1	6.1	4.0	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	50.9	50.9	53.0	50.9	50.9	53.0	36.7	36.7		36.7	36.7	
Actuated g/C Ratio	0.51	0.51	0.53	0.51	0.51	0.53	0.37	0.37		0.37	0.37	
v/c Ratio	0.29	0.79	0.05	0.30	0.34	0.04	0.25	0.58		0.40	0.89	
Control Delay	16.4	28.2	1.8	16.5	13.9	0.4	28.2	28.7		29.3	48.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	16.4	28.2	1.8	16.5	13.9	0.4	28.2	28.7		29.3	48.5	
LOS	В	С	Α	В	В	Α	С	C		С	D	
Approach Delay		25.3			13.3			28.7			45.3	
Approach LOS	440	C	0.0	7.0	В	0.4	4.0	C		45.0	D	
Queue Length 50th (m)	14.2	112.7	0.0	7.2	41.3	0.1	4.6	57.4		15.2	96.1	
Queue Length 95th (m)	27.1	165.6	3.1	18.5	27.4	m0.4	13.3	87.8		31.3	#160.9	
Internal Link Dist (m)	55.0	207.0	45.0	05.0	635.7	05.0	55.0	70.7		50.0	630.1	
Turn Bay Length (m)	55.0	000	45.0	65.0	004	35.0	55.0	0.40		50.0	500	
Base Capacity (vph)	437	889	814	179	864	668	133	640		257	588	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.29	0.79	0.05	0.30	0.34	0.04	0.25	0.58		0.40	0.89	

Cycle Length: 100 Actuated Cycle Length: 100

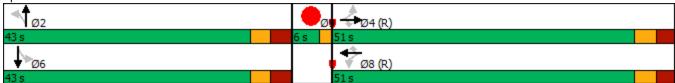
Offset: 91 (91%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Lane Configurations  Traffic Volume (vph)  Future Volume (vph)  Satd. Flow (prot)  Fit Permitted  Satd. Flow (perm)  Satd. Flow (RTOR)  Lane Group Flow (vph)  Turn Type  Protected Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (%)  Yellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Tum Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio  Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Satd. Flow (prot) FIt Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Source of time (s) 1.0 Lost Time (s) 1.0 Lost Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) 1.0 Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvatino Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Ype Internal Park (m) Internal Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Sellow Time (s) All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Solution Split (s				
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Protected Phases  Permitted Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (s)  Total Split (%)  Sylellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (yph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Cotal Split (%) Sellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lost Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 55th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		9		
Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Minimum Initial (s)	2.0		
Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		6.0		
Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio		3.0		
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS  Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		None		
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		NOTIC		
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	. ,			
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	•			
LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)			
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary				
	Intersection Summary			

Maximum v/c Ratio: 0.89
Intersection Signal Delay: 29.5
Intersection Capacity Utilization 104.7%
ICU Level of Service G
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	•	$\rightarrow$	1	<b>†</b>	ļ	4			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9		
Lane Configurations	¥		ች	<b>^</b>	<b>↑</b> ↑				
Traffic Volume (vph)	1	1	2	857	1458	6			
Future Volume (vph)	1	1	2	857	1458	6			
Satd. Flow (prot)	1600	0	1710	3420	3417	0			
Flt Permitted	0.976		0.131						
Satd. Flow (perm)	1598	0	236	3420	3417	0			
Satd. Flow (RTOR)	1				1				
Lane Group Flow (vph)	2	0	2	902	1541	0			
Turn Type	Prot		Perm	NA	NA				
Protected Phases	4			2	6		9		
Permitted Phases			2						
Detector Phase	4		2	2	6				
Switch Phase									
Minimum Initial (s)	7.8		10.0	10.0	10.0		10.0		
Minimum Split (s)	14.0		30.9	30.9	30.9		29.0		
Total Split (s)	17.0		49.0	49.0	49.0		29.0		
Total Split (%)	17.9%	ļ	51.6%	51.6%	51.6%		31%		
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0		
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0		
Lost Time Adjust (s)	0.0		0.0	0.0	0.0				
Total Lost Time (s)	6.2		5.9	5.9	5.9				
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	(	C-Max	C-Max	C-Max		None		
Act Effct Green (s)	7.8		85.2	85.2	85.2				
Actuated g/C Ratio	0.08		0.90	0.90	0.90				
v/c Ratio	0.02		0.01	0.29	0.50				
Control Delay	35.0		2.0	0.8	7.3				
Queue Delay	0.0		0.0	0.0	0.0				
Total Delay	35.0		2.0	0.8	7.3				
LOS	С		A	Α	A				
Approach Delay	35.0			0.8	7.3				
Approach LOS	С			Α	Α				
Queue Length 50th (m)	0.2		0.0	0.0	0.0				
Queue Length 95th (m)	2.4		m0.0	m7.0	#195.3				
Internal Link Dist (m)	148.9			192.9	188.6				
Turn Bay Length (m)			35.0						
Base Capacity (vph)	182		212	3068	3065				
Starvation Cap Reductn	0		0	0	0				
Spillback Cap Reductn	0		0	0	0				
Storage Cap Reductn	0		0	0	0				
Reduced v/c Ratio	0.01		0.01	0.29	0.50				
Intersection Summary									

Cycle Length: 95

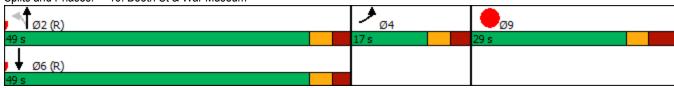
Actuated Cycle Length: 95

Offset: 47 (49%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.50
Intersection Signal Delay: 5.0
Intersection Capacity Utilization 59.3%
ICU Level of Service B
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 19: Booth St & War Museum



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	ሻ	<b>†</b>	7	ሻ	f)			4	
Traffic Volume (vph)	0	957	77	98	370	0	28	0	47	0	0	0
Future Volume (vph)	0	957	77	98	370	0	28	0	47	0	0	0
Satd. Flow (prot)	1800	1782	1485	1629	1714	1800	1644	1404	0	0	1800	0
Flt Permitted				0.228			0.950					
Satd. Flow (perm)	1800	1782	1406	391	1714	1800	1606	1404	0	0	1800	0
Satd. Flow (RTOR)			52					135				
Lane Group Flow (vph)	0	1007	81	103	389	0	29	49	0	0	0	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA				
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	12.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)		99.3	101.2	99.3	99.3		12.6	12.6				
Actuated g/C Ratio		0.83	0.84	0.83	0.83		0.10	0.10				
v/c Ratio		0.68	0.07	0.32	0.27		0.17	0.18				
Control Delay		9.7	1.6	12.3	7.3		49.1	1.5				
Queue Delay		0.6	0.0	0.0	0.0		0.0	0.0				
Total Delay		10.4	1.6	12.3	7.3		49.1	1.5				
LOS		В	Α	В	Α		D	Α				
Approach Delay		9.7			8.3			19.2				
Approach LOS		Α			Α			В				
Queue Length 50th (m)		85.6	1.0	12.6	35.9		6.8	0.0				
Queue Length 95th (m)		221.3	6.1	36.8	62.5		14.8	0.0				
Internal Link Dist (m)		497.2			115.1			178.8			41.9	
Turn Bay Length (m)			50.0	35.0			30.0					
Base Capacity (vph)		1475	1193	323	1418		397	449				
Starvation Cap Reductn		0	0	0	0		0	0				
Spillback Cap Reductn		177	0	0	0		0	12				
Storage Cap Reductn		0	0	0	0		0	0				
Reduced v/c Ratio		0.78	0.07	0.32	0.27		0.07	0.11				

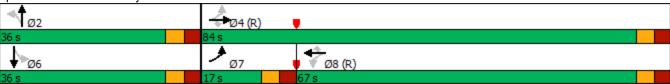
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Intersection Signal Delay: 9.7 Intersection LOS: A Intersection Capacity Utilization 88.4% ICU Level of Service E	Maximum v/c Ratio: 0.68		
<u> </u>	Intersection Signal Delay: 9.7	Intersection LOS: A	
	Intersection Capacity Utilization 88.4%	ICU Level of Service E	
Analysis Period (min) 15	Analysis Period (min) 15		

Splits and Phases: 20: City Center Ave & Albert St



	<b>→</b>	•	•	←	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	,	ኘ	<b>^</b>	¥	.,,,,,
Traffic Volume (vph)	1397	16	26	707	37	77
Future Volume (vph)	1397	16	26	707	37	77
Satd. Flow (prot)	3365	0	1710	3320	1522	0
Flt Permitted			0.139		0.984	
Satd. Flow (perm)	3365	0	250	3320	1519	0
Satd. Flow (RTOR)	2				35	
Lane Group Flow (vph)	1488	0	27	744	120	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		30.0	30.0	33.9	
Total Split (s)	61.0		61.0	61.0	34.0	
Total Split (%)	64.2%		64.2%	64.2%	35.8%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	68.9		68.9	68.9	14.4	
Actuated g/C Ratio	0.73		0.73	0.73	0.15	
v/c Ratio	0.61		0.15	0.31	0.46	
Control Delay	21.2		8.8	5.9	30.0	
Queue Delay	6.5		0.0	0.0	0.0	
Total Delay	27.7		8.8	5.9	30.0	
LOS	С		Α	Α	С	
Approach Delay	27.7			6.0	30.0	
Approach LOS	С			Α	С	
Queue Length 50th (m)	133.4		1.1	18.4	15.7	
Queue Length 95th (m)	m166.5		7.6	50.0	26.7	
Internal Link Dist (m)	128.4			234.2	63.1	
Turn Bay Length (m)			65.0			
Base Capacity (vph)	2442		181	2409	474	
Starvation Cap Reductn	901		0	0	0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.97		0.15	0.31	0.25	
Intersection Summary						

Cycle Length: 95

Actuated Cycle Length: 95

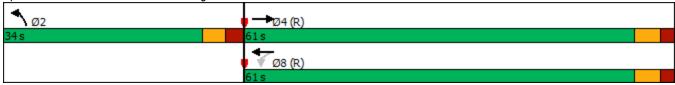
Offset: 23 (24%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 75

Maximum v/c Ratio: 0.61						
Intersection Signal Delay: 20.8	Intersection LOS: C					
Intersection Capacity Utilization 60.4%	ICU Level of Service B					
Analysis Period (min) 15						
V.1 ( 050 (1 1 1						

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	-	•	•	•	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			414	¥	
Traffic Volume (vph)	964	8	5	404	8	11
Future Volume (vph)	964	8	5	404	8	11
Satd. Flow (prot)	3417	0	0	3417	1621	0
Flt Permitted				0.945	0.980	
Satd. Flow (perm)	3417	0	0	3232	1621	0
Satd. Flow (RTOR)	1				12	
Lane Group Flow (vph)	1023	0	0	430	20	0
Turn Type	NA		Perm	NA	Perm	
Protected Phases	4			8		
Permitted Phases			8		2	
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	40.1		40.1	40.1	28.3	
Total Split (s)	81.0		81.0	81.0	39.0	
Total Split (%)	67.5%		67.5%	67.5%	32.5%	
Yellow Time (s)	3.3		3.3	3.3	3.3	
All-Red Time (s)	3.8		3.8	3.8	3.0	
Lost Time Adjust (s)	0.0			-3.1	-2.3	
Total Lost Time (s)	7.1			4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	110.6			111.9	12.3	
Actuated g/C Ratio	0.92			0.93	0.10	
v/c Ratio	0.32			0.14	0.11	
Control Delay	0.4			1.1	31.5	
Queue Delay	0.0			0.0	0.0	
Total Delay	0.5			1.1	31.5	
LOS	A			Α	С	
Approach Delay	0.5			1.1	31.5	
Approach LOS	A			Α	С	
Queue Length 50th (m)	0.0			0.0	1.8	
Queue Length 95th (m)	m5.4			11.2	9.7	
Internal Link Dist (m)	168.5			265.3	32.8	
Turn Bay Length (m)						
Base Capacity (vph)	3150			3013	481	
Starvation Cap Reductn	387			0	0	
Spillback Cap Reductn	0			0	0	
Storage Cap Reductn	0			0	0	
Reduced v/c Ratio	0.37			0.14	0.04	
Intersection Summary						

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

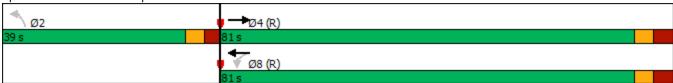
Natural Cycle: 70

Maximum v/c Ratio: 0.32

Intersection Signal Delay: 1.1	Intersection LOS: A
Intersection Capacity Utilization 46.0%	ICU Level of Service A
Analysis Period (min) 15	

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St/Slater St



	•	•	<b>†</b>	/	-	.↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>↑</b> ↑			<b>^</b>
Traffic Volume (vph)	0	12	850	17	0	960
Future Volume (vph)	0	12	850	17	0	960
Satd. Flow (prot)	0	1557	3410	0	0	3420
Flt Permitted						
Satd. Flow (perm)	0	1557	3410	0	0	3420
Lane Group Flow (vph)	0	13	913	0	0	1011
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 35.4%			ICI	U Level o	f Service
Analysis Period (min) 15						

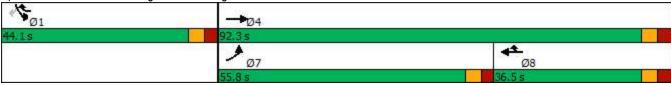
	•	<b>→</b>	<b>←</b>	•	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	11	ሻሻ	7
Traffic Volume (vph)	802	367	673	1105	852	451
Future Volume (vph)	802	367	673	1105	852	451
Satd. Flow (prot)	3317	4914	3386	2614	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3289	4914	3386	2614	3168	1530
Satd. Flow (RTOR)						179
Lane Group Flow (vph)	844	386	708	1163	897	475
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	8 1	1	
Permitted Phases						1
Detector Phase	7	4	8	8 1	1	1
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
Total Split (s)	55.8	92.3	36.5		44.1	44.1
Total Split (%)	40.9%	67.7%	26.8%		32.3%	32.3%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.8	6.5	6.5		6.1	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Max		Max	Max
Act Effct Green (s)	36.5	72.4	30.1	74.3	38.1	38.1
Actuated g/C Ratio	0.30	0.59	0.24	0.60	0.31	0.31
v/c Ratio	0.86	0.13	0.86	0.74	0.88	0.80
Control Delay	50.4	11.3	56.5	22.2	52.5	35.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.4	11.3	56.5	22.2	52.5	35.8
LOS	D	В	Е	С	D	D
Approach Delay		38.2	35.2		46.7	
Approach LOS		D	D		D	
Queue Length 50th (m)	104.2	15.2	91.3	114.2	111.9	72.2
Queue Length 95th (m)	128.6	20.1	#139.3	176.0	#168.6	#142.1
Internal Link Dist (m)		240.3	292.7		47.6	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1351	3434	827	1578	1017	597
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.11	0.86	0.74	0.88	0.80
	0.02	J. 1 1	0.00	J., F	3.00	3.00

Cycle Length: 136.4 Actuated Cycle Length: 123.1

Natural Cycle: 115 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.88

Intersection Signal Delay: 39.5 Intersection LOS: D
Intersection Capacity Utilization 84.8% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 7: Wellington St & Portage



Bane Group   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR		•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	$\triangleleft$	<b>†</b>	~	<b>&gt;</b>	ļ	4
Traffic Volume (vph) 33 0 33 0 0 0 7 1100 0 0 1035 7 Future Volume (vph) 33 0 33 0 0 0 0 7 1100 0 0 1035 7 Future Volume (vph) 33 0 33 0 0 0 0 7 1100 0 0 1035 7 Satal. Flow (prot) 1710 1530 0 1800 1800 0 170 1800 0 1800 1798 0 FIL Permitted 0 7.75 0.167 Satal. Flow (prot) 1363 1530 0 1800 1800 0 301 1800 0 1800 1798 0 Satal. Flow (RTOR) 110	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (vph)	Lane Configurations	Ť	f)		*	f)		7	f)		7	î»	
Future Volume (vph) 33 0 33 0 0 0 0 7 1100 0 0 1035 7 Satd. Flow (prot) 1710 1530 0 1800 1800 0 1710 1800 0 1800 1798 0 FIF Permitted 0.757	Traffic Volume (vph)		0		0	0	0	7	1100	0	0	1035	7
Fit Permitted	Future Volume (vph)	33	0	33	0	0	0	7	1100	0		1035	7
Satd. Flow (perm)   1363   1530   0   1800   1800   0   301   1800   0   1800   1798   0	Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1798	0
Satd. Flow (RTOR)	Flt Permitted												
Lane Group Flow (vph)   35   35   0   0   0   0   7   1158   0   0   1096   0     Turn Type	Satd. Flow (perm)	1363		0	1800	1800	0	301	1800	0	1800	1798	0
Turn Type         Perm         NA         Perm         NA         Perm         NA           Protected Phases         4         8         2         6           Permitted Phases         4         8         2         6           Detector Phase         4         4         8         8         2         2         6           Switch Phase         8         1         10.0 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1</td><td></td></td<>												1	
Protected Phases	, , ,			0		0	0			0			0
Permitted Phases		Perm			Perm			Perm			Perm		
Detector Phase   4			4			8			2			6	
Switch Phase   Minimum Initial (s)	Permitted Phases												
Minimum Initial (s)         10.0         13.0         53.0         20.0 </td <td>Detector Phase</td> <td>4</td> <td>4</td> <td></td> <td>8</td> <td>8</td> <td></td> <td>2</td> <td>2</td> <td></td> <td>6</td> <td>6</td> <td></td>	Detector Phase	4	4		8	8		2	2		6	6	
Minimum Split (s)         22.0         22.0         22.0         22.0         22.0         53.0         53.0         53.0         53.0           Total Split (s)         22.0         22.0         22.0         22.0         53.2         22.2         26.2         26.2         26.2	Switch Phase												
Total Split (s)         22.0         22.0         22.0         22.0         22.0         53.0         53.0         53.0           Total Split (%)         29.3%         29.3%         29.3%         70.7%         70.7%         70.7%         70.7%           Yellow Time (s)         3.0         3.0         3.0         3.3         3.3         3.3         3.3           All-Red Time (s)         3.2         3.2         3.2         2.6         2.6         2.6         2.6           Lost Time Adjust (s)         0.0         -2.2         0.0	Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0			10.0	10.0	
Total Split (%)         29.3%         29.3%         29.3%         29.3%         70.7%         70.7%         70.7%         70.7%           Yellow Time (s)         3.0         3.0         3.0         3.0         3.3         3.5         5.9         5.9         5.9         5.9         5.9	Minimum Split (s)	22.0			22.0			53.0					
Yellow Time (s)         3.0         3.0         3.0         3.0         3.3         3.5         5.9         5.9         5.9         5.9         5.9	Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0			53.0	
All-Red Time (s)         3.2         3.2         3.2         3.2         2.6         2.6         2.6         2.6           Lost Time Adjust (s)         0.0         -2.2         0.0 <td< td=""><td>Total Split (%)</td><td></td><td>29.3%</td><td></td><td>29.3%</td><td>29.3%</td><td></td><td>70.7%</td><td></td><td></td><td>70.7%</td><td>70.7%</td><td></td></td<>	Total Split (%)		29.3%		29.3%	29.3%		70.7%			70.7%	70.7%	
Lost Time Adjust (s)   0.0   -2.2   0.0   0.0   0.0   0.0   0.0   0.0   0.0     Total Lost Time (s)   6.2   4.0   6.2   6.2   5.9   5.9   5.9   5.9     Lead/Lag     Lead-Lag Optimize?     Recall Mode   None   None   None   None   C-Max   C-Max   C-Max     Act Effct Green (s)   10.0   12.2   61.7   61.7   61.7     Actuated g/C Ratio   0.13   0.16   0.82   0.82   0.82     Vic Ratio   0.19   0.10   0.03   0.78   0.74     Control Delay   31.9   0.6   3.7   13.0   11.2     Queue Delay   0.0   0.0   0.0   0.0     Total Delay   31.9   0.6   3.7   13.0   11.2     LOS   C   A   A   B   B     Approach Delay   16.2   12.9   11.2     Approach LOS   B   B   B     Queue Length 50th (m)   4.7   0.0   0.3   11.64   100.4     Queue Length 95th (m)   12.9   0.0   1.4   #239.2   #219.3     Internal Link Dist (m)   60.2   43.3   76.9   31.3     Turn Bay Length (m)   30.0     Base Capacity (vph)   287   450   247   1481   1480     Starvation Cap Reductn   0 0 0 0 0 0 0	Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
Total Lost Time (s)   6.2   4.0   6.2   6.2   5.9   5.9   5.9   5.9	All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lead/Lag       Lead-Lag Optimize?         Recall Mode       None       None       None       None       C-Max       0.02       0.03       0.74       C-Max       C-Max       0.74       C-Max       0.03       11.2       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       1.0       0.0       1.0       1.2       9.0       0.0	Lost Time Adjust (s)	0.0	-2.2		0.0	0.0		0.0	0.0		0.0	0.0	
Lead-Lag Optimize?         Recall Mode         None         None         None         None         C-Max         C-Max </td <td>Total Lost Time (s)</td> <td>6.2</td> <td>4.0</td> <td></td> <td>6.2</td> <td>6.2</td> <td></td> <td>5.9</td> <td>5.9</td> <td></td> <td>5.9</td> <td>5.9</td> <td></td>	Total Lost Time (s)	6.2	4.0		6.2	6.2		5.9	5.9		5.9	5.9	
Recall Mode         None         None         None         None         C-Max         C-Max         C-Max           Act Effct Green (s)         10.0         12.2         61.7         61.7         61.7           Actuated g/C Ratio         0.13         0.16         0.82         0.82         0.82           v/c Ratio         0.19         0.10         0.03         0.78         0.74           Control Delay         31.9         0.6         3.7         13.0         11.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         31.9         0.6         3.7         13.0         11.2           LOS         C         A         A         B         B           Approach Delay         16.2         12.9         11.2           Approach LOS         B         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         116.4         100.4           Queue Length 95th (m)         12.9         0.0         1.4         #239.2         #219.3           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)	Lead/Lag												
Act Effct Green (s)       10.0       12.2       61.7       61.7       61.7         Actuated g/C Ratio       0.13       0.16       0.82       0.82       0.82         v/c Ratio       0.19       0.10       0.03       0.78       0.74         Control Delay       31.9       0.6       3.7       13.0       11.2         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.6       3.7       13.0       11.2         LOS       C       A       A       B       B         Approach Delay       16.2       12.9       11.2         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0       0	Lead-Lag Optimize?												
Actuated g/C Ratio       0.13       0.16       0.82       0.82       0.82         v/c Ratio       0.19       0.10       0.03       0.78       0.74         Control Delay       31.9       0.6       3.7       13.0       11.2         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.6       3.7       13.0       11.2         LOS       C       A       A       B       B         Approach Delay       16.2       12.9       11.2         Approach LOS       B       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	Recall Mode	None			None	None		C-Max			C-Max		
v/c Ratio         0.19         0.10         0.03         0.78         0.74           Control Delay         31.9         0.6         3.7         13.0         11.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         31.9         0.6         3.7         13.0         11.2           LOS         C         A         A         B         B           Approach Delay         16.2         12.9         11.2           Approach LOS         B         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         116.4         100.4           Queue Length 95th (m)         12.9         0.0         1.4         #239.2         #219.3           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         450         247         1481         1480           Starvation Cap Reductn         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0	Act Effct Green (s)												
Control Delay       31.9       0.6       3.7       13.0       11.2         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.6       3.7       13.0       11.2         LOS       C       A       A       B       B         Approach Delay       16.2       12.9       11.2         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	Actuated g/C Ratio												
Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.6       3.7       13.0       11.2         LOS       C       A       A       B       B         Approach Delay       16.2       12.9       11.2         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	v/c Ratio												
Total Delay         31.9         0.6         3.7         13.0         11.2           LOS         C         A         A         B         B           Approach Delay         16.2         12.9         11.2           Approach LOS         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         116.4         100.4           Queue Length 95th (m)         12.9         0.0         1.4         #239.2         #219.3           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         450         247         1481         1480           Starvation Cap Reductn         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0	Control Delay												
LOS         C         A         B         B           Approach Delay         16.2         12.9         11.2           Approach LOS         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         116.4         100.4           Queue Length 95th (m)         12.9         0.0         1.4         #239.2         #219.3           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         450         247         1481         1480           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0	Queue Delay												
Approach Delay       16.2       12.9       11.2         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Total Delay	31.9	0.6					3.7	13.0			11.2	
Approach LOS B B B B Queue Length 50th (m) 4.7 0.0 0.3 116.4 100.4 100.4 Queue Length 95th (m) 12.9 0.0 1.4 #239.2 #219.3 Internal Link Dist (m) 60.2 43.3 76.9 31.3 Turn Bay Length (m) 30.0 30.0 Base Capacity (vph) 287 450 247 1481 1480 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0	LOS	С						Α					
Queue Length 50th (m)       4.7       0.0       0.3       116.4       100.4         Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Approach Delay								12.9			11.2	
Queue Length 95th (m)       12.9       0.0       1.4       #239.2       #219.3         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0													
Internal Link Dist (m)     60.2     43.3     76.9     31.3       Turn Bay Length (m)     30.0     30.0       Base Capacity (vph)     287     450     247     1481     1480       Starvation Cap Reductn     0     0     0     0       Spillback Cap Reductn     0     0     0     0		4.7						0.3					
Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Queue Length 95th (m)	12.9	0.0					1.4	#239.2			#219.3	
Base Capacity (vph)       287       450       247       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0	Turn Bay Length (m)							30.0					
Spillback Cap Reductn 0 0 0 0		287							1481			1480	
	Starvation Cap Reductn	0						0	0			0	
		0	0					0	0			0	
Storage Cap Reductn 0 0 0 0 0	Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio 0.12 0.08 0.03 0.78 0.74	Reduced v/c Ratio	0.12	0.08					0.03	0.78			0.74	

Cycle Length: 75

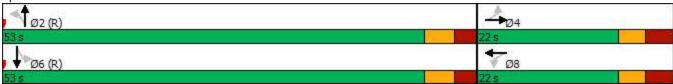
Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.78
Intersection Signal Delay: 12.2
Intersection Capacity Utilization 77.7%
ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere



	۶	<b>→</b>	$\rightarrow$	•	•	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>∱</b> }		ሻ	<b>^</b>	7
Traffic Volume (vph)	0	845	0	0	1247	117	0	972	176	146	982	233
Future Volume (vph)	0	845	0	0	1247	117	0	972	176	146	982	233
Satd. Flow (prot)	0	3420	0	0	3420	1530	0	3229	0	1613	3196	1515
Flt Permitted										0.084		
Satd. Flow (perm)	0	3420	0	0	3420	1480	0	3229	0	143	3196	1471
Satd. Flow (RTOR)								19				35
Lane Group Flow (vph)	0	889	0	0	1313	123	0	1208	0	154	1034	245
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase												
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.8		12.0	31.9	31.9
Total Split (s)		60.0			60.0	60.0		48.0		12.0	60.0	60.0
Total Split (%)		50.0%			50.0%	50.0%		40.0%		10.0%	50.0%	50.0%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	0.0		-2.7		0.0	0.0	0.0
Total Lost Time (s)		6.8			6.8	6.8		4.1		6.8	6.9	6.9
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		51.1			51.1	51.1		43.9		55.3	55.2	55.2
Actuated g/C Ratio		0.43			0.43	0.43		0.37		0.46	0.46	0.46
v/c Ratio		0.61			0.90	0.20		1.01		0.99	0.70	0.35
Control Delay		26.5			37.2	17.4		51.2		101.2	29.6	20.0
Queue Delay		0.0			1.4	0.0		0.0		0.0	0.0	0.0
Total Delay		26.5			38.6	17.4		51.2		101.2	29.6	20.0
LOS		С			D	В		D		F	С	С
Approach Delay		26.5			36.8			51.2			35.6	
Approach LOS		С			D			D			D	
Queue Length 50th (m)		85.4			152.3	18.2		~141.6		~31.7	108.8	33.5
Queue Length 95th (m)		106.4			185.5	24.4	n	n#172.8		#76.2	135.4	54.7
Internal Link Dist (m)		331.0			119.3			51.3			192.9	_
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1516			1516	656		1193		155	1470	695
Starvation Cap Reductn		0			77	0		0		0	0	0
Spillback Cap Reductn		0			0	0		0		0	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.59			0.91	0.19		1.01		0.99	0.70	0.35

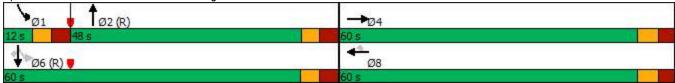
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 3 (3%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 100

Maximum v/c Ratio: 1.01 Intersection LOS: D Intersection Signal Delay: 38.1 Intersection Capacity Utilization 94.1% ICU Level of Service F Analysis Period (min) 15 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Booth St & Wellington St



	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ሻ	<b>^</b>	7		4îb		ሻ	<b>†</b>	7
Traffic Volume (vph)	545	489	32	19	788	234	12	540	34	88	411	243
Future Volume (vph)	545	489	32	19	788	234	12	540	34	88	411	243
Satd. Flow (prot)	1676	1765	1485	1710	3386	1530	0	3326	0	1629	1782	1515
Flt Permitted	0.109			0.474				0.939		0.162		
Satd. Flow (perm)	189	1765	1451	849	3386	1359	0	3124	0	270	1782	1380
Satd. Flow (RTOR)			91					5				
Lane Group Flow (vph)	574	515	34	20	829	246	0	617	0	93	433	256
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		4.5	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	36.0	74.0	74.0	38.0	38.0	38.0	34.5	34.5		11.5	46.0	46.0
Total Split (%)	30.0%	61.7%	61.7%	31.7%	31.7%	31.7%	28.8%	28.8%		9.6%	38.3%	38.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	-2.5	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	4.0	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		None	Min	Min
Act Effct Green (s)	71.1	68.6	71.1	31.5	31.5	31.5		26.9		38.4	38.4	38.4
Actuated g/C Ratio	0.59	0.57	0.59	0.26	0.26	0.26		0.22		0.32	0.32	0.32
v/c Ratio	1.10	0.51	0.04	0.09	0.93	0.69		0.88		0.65	0.76	0.58
Control Delay	95.4	9.6	0.1	37.8	60.7	51.4		59.1		31.5	29.1	23.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	95.4	9.6	0.1	37.8	60.7	51.4		59.1		31.5	29.1	23.0
LOS	F	Α	Α	D	Е	D		E		С	С	С
Approach Delay		53.1			58.2			59.1			27.4	
Approach LOS		D			E			E			С	
Queue Length 50th (m)	~146.0	35.7	0.0	3.8	87.0	49.0		76.6		16.3	111.5	61.4
Queue Length 95th (m)	#218.0	45.1	m0.0	m11.2	#139.3	73.3		#105.6		m19.8	148.8	95.0
Internal Link Dist (m)		285.1			168.5			37.2			83.1	
Turn Bay Length (m)	160.0		40.0	40.0		95.0						120.0
Base Capacity (vph)	521	1008	896	222	888	356		732		143	586	454
Starvation Cap Reductn	0	0	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	1.10	0.51	0.04	0.09	0.93	0.69		0.84		0.65	0.74	0.56

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 9 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 125

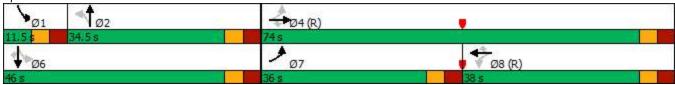
Maximum v/c Ratio: 1.10
Intersection Signal Delay: 50.1 Intersection LOS: D
Intersection Capacity Utilization 122.7% ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	<b>→</b>	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	7	ሻ	<b>^</b>	ሻ	7
Traffic Volume (vph)	650	126	267	870	151	343
Future Volume (vph)	650	126	267	870	151	343
Satd. Flow (prot)	1782	1500	1676	3386	1629	1485
Flt Permitted	1702	1000	0.201	0000	0.950	1700
Satd. Flow (perm)	1782	1412	355	3386	1590	1441
Satd. Flow (RTOR)	1102	33	000	0000	1000	281
Lane Group Flow (vph)	684	133	281	916	159	361
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	4	1 Cilli	3	8	2	1 Cilli
Permitted Phases	7	4	8	U	2	2
Detector Phase	4	4	3	8	2	2
Switch Phase	4	4	J	0		
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	31.8	31.8	11.2	31.8	29.3	29.3
. , ,	61.0	61.0	16.0	77.0	43.0	43.0
Total Split (s)	50.8%	50.8%	13.3%	64.2%	35.8%	35.8%
Total Split (%) Yellow Time (s)	3.3	3.3	3.3	3.3	33.6%	33.6%
\ /			2.9			
All-Red Time (s)	3.5	3.5		3.5	3.0	3.0
Lost Time Adjust (s)	0.0	-2.8	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	4.0	6.2	6.8	6.3	6.3
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes	0.14	N 41	N 4"
Recall Mode	C-Max	C-Max	None	C-Max	Min	Min
Act Effct Green (s)	62.5	65.3	89.7	89.1	17.8	17.8
Actuated g/C Ratio	0.52	0.54	0.75	0.74	0.15	0.15
v/c Ratio	0.74	0.17	0.57	0.36	0.66	0.80
Control Delay	26.2	8.5	11.5	10.5	60.2	25.0
Queue Delay	0.1	0.0	0.0	0.0	0.0	0.0
Total Delay	26.3	8.5	11.5	10.5	60.2	25.0
LOS	С	Α	В	В	Е	С
Approach Delay	23.4			10.8	35.8	
Approach LOS	С			В	D	
Queue Length 50th (m)	122.5	5.7	30.6	93.3	38.0	18.3
Queue Length 95th (m)	#219.2	22.5	m63.7	m112.9	55.8	51.6
Internal Link Dist (m)	82.0			285.1	54.7	
Turn Bay Length (m)		16.0	90.0			
Base Capacity (vph)	928	783	489	2513	498	635
Starvation Cap Reductn	16	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.75	0.17	0.57	0.36	0.32	0.57

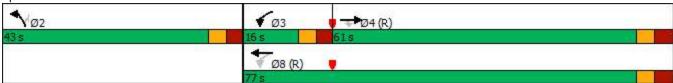
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 81 (68%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.80 Intersection Signal Delay: 20.0 Intersection LOS: B Intersection Capacity Utilization 77.9% ICU Level of Service D Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



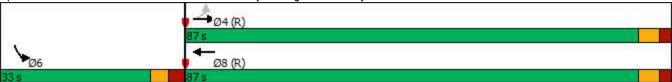
	•	-	←	•	<b>&gt;</b>	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	<b>†</b> †	<b>^</b>		¥		
Traffic Volume (vph)	2	923	1572	7	5	22	
Future Volume (vph)	2	923	1572	7	5	22	
Satd. Flow (prot)	1710	3386	3416	0	1586	0	
Flt Permitted	0.131				0.991		
Satd. Flow (perm)	236	3386	3416	0	1585	0	
Satd. Flow (RTOR)			1		23		
Lane Group Flow (vph)	2	972	1662	0	28	0	
Turn Type	Perm	NA	NA		Prot		
Protected Phases		4	8		6		
Permitted Phases	4						
Detector Phase	4	4	8		6		
Switch Phase							
Minimum Initial (s)	10.0	10.0	10.0		10.0		
Minimum Split (s)	37.0	37.0	37.0		32.3		
Total Split (s)	87.0	87.0	87.0		33.0		
Total Split (%)	72.5%	72.5%	72.5%		27.5%		
Yellow Time (s)	3.7	3.7	3.7		3.3		
All-Red Time (s)	2.3	2.3	2.3		3.0		
Lost Time Adjust (s)	0.0	0.0	0.0		0.0		
Total Lost Time (s)	6.0	6.0	6.0		6.3		
Lead/Lag							
Lead-Lag Optimize?							
Recall Mode	C-Max	C-Max	C-Max		None		
Act Effct Green (s)	106.6	106.6	106.6		10.0		
Actuated g/C Ratio	0.89	0.89	0.89		0.08		
v/c Ratio	0.01	0.32	0.55		0.18		
Control Delay	2.0	2.3	3.6		25.8		
Queue Delay	0.0	0.0	0.0		0.0		
Total Delay	2.0	2.3	3.6		25.8		
LOS	Α	Α	Α		С		
Approach Delay		2.3	3.6		25.8		
Approach LOS		Α	Α		С		
Queue Length 50th (m)	0.1	26.0	32.4		1.2		
Queue Length 95th (m)	0.6	32.3	52.6		10.7		
Internal Link Dist (m)		651.1	331.0		21.1		
Turn Bay Length (m)	40.0						
Base Capacity (vph)	210	3008	3035		370		
Starvation Cap Reductn	0	0	0		0		
Spillback Cap Reductn	0	0	0		0		
Storage Cap Reductn	0	0	0		0		
Reduced v/c Ratio	0.01	0.32	0.55		0.08		
Intersection Summary							
Cycle Length: 120							
Actuated Cycle Length: 120							

Offset: 37 (31%), Referenced to phase 4:EBTL and 8:WBT, Start of Green Natural Cycle: 80

## 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.55
Intersection Signal Delay: 3.4
Intersection Capacity Utilization 64.7%
ICU Level of Service C
Analysis Period (min) 15

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>†</b>			ĵ.	
Traffic Volume (vph)	0	974	4	2	1726	0	26	14	35	3	26	11
Future Volume (vph)	0	974	4	2	1726	0	26	14	35	3	26	11
Satd. Flow (prot)	0	3383	0	0	3420	0	0	1642	0	0	1669	0
Flt Permitted					0.954			0.870			0.969	
Satd. Flow (perm)	0	3383	0	0	3263	0	0	1450	0	0	1623	0
Satd. Flow (RTOR)		1						37			12	
Lane Group Flow (vph)	0	1029	0	0	1819	0	0	79	0	0	42	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		61.7			61.7			10.3			10.3	
Actuated g/C Ratio		0.78			0.78			0.13			0.13	
v/c Ratio		0.39			0.72			0.36			0.19	
Control Delay		4.4			8.3			24.0			25.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		4.4			8.3			24.0			25.9	
LOS		Α			Α			С			С	
Approach Delay		4.4			8.3			24.0			25.9	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)		27.3			76.0			5.9			4.2	
Queue Length 95th (m)		39.2			112.2			18.3			13.1	
Internal Link Dist (m)		192.5			288.5			38.4			167.1	
Turn Bay Length (m)												
Base Capacity (vph)		2634			2540			532			576	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.39			0.72			0.15			0.07	
l												

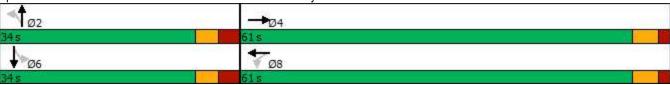
Cycle Length: 95

Actuated Cycle Length: 79.3

Natural Cycle: 90

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.72 Intersection Signal Delay: 7.6 Intersection LOS: A
Intersection Capacity Utilization 75.5% ICU Level of Service D
Analysis Period (min) 15

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	٠	<b>→</b>	$\rightarrow$	•	•	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, j	f)		J.	<b>†</b>	7	¥	<b>†</b>	7	¥	£	
Traffic Volume (vph)	10	559	88	94	751	121	88	185	84	59	68	11
Future Volume (vph)	10	559	88	94	751	121	88	185	84	59	68	11
Satd. Flow (prot)	1555	1714	0	1710	1782	1530	1710	1800	1485	1710	1745	0
Flt Permitted	0.272			0.334			0.702			0.509		
Satd. Flow (perm)	437	1714	0	593	1782	1340	1212	1800	1388	887	1745	0
Satd. Flow (RTOR)		15				90			76		8	
Lane Group Flow (vph)	11	681	0	99	791	127	93	195	88	62	84	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	69.1	69.1		69.1	69.1	69.1	18.0	18.0	18.0	18.0	18.0	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.04	0.57		0.24	0.64	0.13	0.43	0.60	0.28	0.39	0.26	
Control Delay	11.4	20.4		9.1	13.1	3.0	40.6	44.5	11.7	41.1	31.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.4	20.4		9.1	13.1	3.0	40.6	44.5	11.7	41.1	31.7	
LOS	В	С		Α	В	Α	D	D	В	D	С	
Approach Delay		20.3			11.4			35.9			35.7	
Approach LOS		С			В			D			D	
Queue Length 50th (m)	1.1	114.1		6.1	72.9	2.0	17.5	37.9	2.1	11.6	13.8	
Queue Length 95th (m)	m2.5	m158.8		17.9	148.3	9.7	30.1	55.2	14.2	22.5	25.0	
Internal Link Dist (m)		635.7			497.2			83.8			130.2	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	302	1189		410	1231	953	310	460	411	227	452	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.04	0.57		0.24	0.64	0.13	0.30	0.42	0.21	0.27	0.19	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 65 (65%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 75

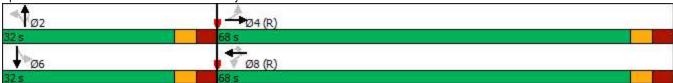
Maximum v/c Ratio: 0.64

Intersection Signal Delay: 19.9	Intersection LOS: B
Intersection Capacity Utilization 95.7%	ICU Level of Service F

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Scott St/Albert St & Bayview Station Rd



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	Ť	<b>†</b>	7	J.	f)		J.	f)	
Traffic Volume (vph)	227	477	86	56	625	63	40	546	48	58	233	210
Future Volume (vph)	227	477	86	56	625	63	40	546	48	58	233	210
Satd. Flow (prot)	1693	1765	1515	1710	1800	1530	1710	1755	0	1710	1565	0
Flt Permitted	0.117			0.479			0.290			0.119		
Satd. Flow (perm)	209	1765	1240	794	1800	1222	507	1755	0	208	1565	0
Satd. Flow (RTOR)			79			119		5			53	
Lane Group Flow (vph)	239	502	91	59	658	66	42	626	0	61	466	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	10.6	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	15.0	49.0	49.0	34.0	34.0	34.0	45.0	45.0		45.0	45.0	
Total Split (%)	15.0%	49.0%	49.0%	34.0%	34.0%	34.0%	45.0%	45.0%		45.0%	45.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.3	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	-2.1	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	6.1	4.0	6.1	4.0	4.0	6.3	6.3		6.3	6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		Min	Min	
Act Effct Green (s)	50.6	50.1	52.2	28.4	30.5	30.5	37.5	37.5		37.5	37.5	
Actuated g/C Ratio	0.51	0.50	0.52	0.28	0.30	0.30	0.38	0.38		0.38	0.38	
v/c Ratio	0.70	0.57	0.13	0.26	1.20	0.15	0.22	0.95		0.78	0.75	
Control Delay	32.2	21.0	4.3	31.3	133.5	2.9	24.4	55.0		87.8	32.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	32.2	21.0	4.3	31.3	133.5	2.9	24.4	55.0		87.8	32.5	
LOS	С	С	Α	С	F	Α	С	Е		F	С	
Approach Delay		22.4			114.8			53.1			38.9	
Approach LOS		С			F			D			D	
Queue Length 50th (m)	30.2	70.3	1.2	6.7	~167.7	0.1	5.6	118.6		10.6	71.3	
Queue Length 95th (m)	#64.3	103.4	9.1	m13.3	#233.5	m1.9	14.4	#190.2		#35.8	111.4	
Internal Link Dist (m)		92.9			635.7		_	70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	343	884	685	225	550	455	196	682		80	638	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.70	0.57	0.13	0.26	1.20	0.15	0.21	0.92		0.76	0.73	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 8 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 110

Lane Configurations  Traffic Volume (vph)  Future Volume (vph)  Satd. Flow (prot)  Fit Permitted  Satd. Flow (perm)  Satd. Flow (RTOR)  Lane Group Flow (vph)  Turn Type  Protected Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (%)  Yellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Tum Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio  Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Satd. Flow (prot) FIt Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vph) Satd. Flow (prot) Fit Permitted Satd. Flow (prom) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Solution Split (s) So				
Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Source Substited Split (s) 6.0 Total Split (s) 7 Total Cost Time (s) 7 Lead/Lag 8 Lead/Lag 9 Lead/Lag 9 Lead/Lag 9 Lead/Lag 9 Lead-Lag Optimize? Recall Mode 8 Control Delay 10 Recall Mode 9 Rone Act Effct Green (s) 8 Actuated g/C Ratio 7 V/c Ratio 7 V/c Ratio 7 Control Delay 10 Roueue Delay 10 Total Delay 10 Roueue Length 50th (m) 10 Roueue Length 95th (m) 10 Roueue L				
Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Ype Internal Park (m) Internal Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Sellow Time (s) All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Solution Split (s				
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Minimum				
Protected Phases  Permitted Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (s)  Total Split (%)  Sylellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (yph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Cotal Split (%) Sellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lost Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 55th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		9		
Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Tum Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Minimum Split (s) 6.0  Total Split (s) 6.0  Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio	Switch Phase			
Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Minimum Initial (s)	2.0		
Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		6.0		
Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio		3.0		
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS  Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		None		
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		NOTIC		
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	. ,			
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	•			
LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)			
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary				
	Intersection Summary			

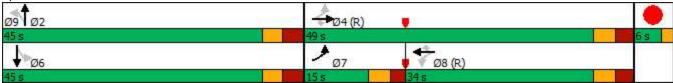
Maximum v/c Ratio: 1.20
Intersection Signal Delay: 58.5
Intersection Capacity Utilization 108.6%
ICU Level of Service G
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	•	•	•	<b>†</b>	ţ	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	N/		*	<b>^</b>	<b>†</b> 1>	02.11	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Traffic Volume (vph)	1	8	8	1106	1060	10		
Future Volume (vph)	1	8	8	1106	1060	10		
Satd. Flow (prot)	1554	0	1710	3420	3417	0		
Flt Permitted	0.994		0.230					
Satd. Flow (perm)	1551	0	414	3420	3417	0		
Satd. Flow (RTOR)	8				1			
Lane Group Flow (vph)	9	0	8	1164	1127	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	3.8		10.0	10.0	10.0		10.0	
Minimum Split (s)	10.0		30.9	30.9	30.9		29.0	
Total Split (s)	17.0		49.0	49.0	49.0		29.0	
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.9	5.9	5.9			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None	(	C-Max	C-Max	C-Max		None	
Act Effct Green (s)	5.9		85.4	85.4	85.4			
Actuated g/C Ratio	0.06		0.90	0.90	0.90			
v/c Ratio	0.09		0.02	0.38	0.37			
Control Delay	26.9		7.2	5.5	5.4			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	26.9		7.2	5.5	5.4			
LOS	С		Α	Α	Α			
Approach Delay	26.9			5.5	5.4			
Approach LOS	С			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	5.0		3.4	110.7	105.6			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	183		372	3074	3072			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.05		0.02	0.38	0.37			
Intersection Summary								

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 10 (11%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.38			
Intersection Signal Delay: 5.5	Intersection LOS: A		
Intersection Capacity Utilization 45.7%	ICU Level of Service A		
Analysis Period (min) 15			
Splits and Phases: 19: Booth St & War Museum	<b>≯</b> <sub>Ø4</sub>	<b>●</b> Ø9	
49 s	17 s	29 s	
₩ Ø6 (R)			
49 s			

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	<b>†</b>	7	7	<b>†</b>	7	7	f)			4	
Traffic Volume (vph)	0	595	76	84	835	0	57	0	80	0	0	0
Future Volume (vph)	0	595	76	84	835	0	57	0	80	0	0	0
Satd. Flow (prot)	900	1782	1530	1693	1765	1200	1644	1485	0	0	1800	0
Flt Permitted				0.397			0.950					
Satd. Flow (perm)	900	1782	1530	708	1765	1200	1644	1485	0	0	1800	0
Satd. Flow (RTOR)			80					311				
Lane Group Flow (vph)	0	626	80	88	879	0	60	84	0	0	0	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA				
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	16.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)		95.5	97.8	95.5	95.5		11.9	11.9				
Actuated g/C Ratio		0.80	0.82	0.80	0.80		0.10	0.10				
v/c Ratio		0.44	0.06	0.16	0.63		0.37	0.20				
Control Delay		5.2	0.7	1.0	9.1		56.4	1.0				
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0				
Total Delay		5.3	0.7	1.0	9.1		56.4	1.0				
LOS		Α	Α	Α	Α		E	Α				
Approach Delay		4.7			8.4			24.1				
Approach LOS		Α			Α			С				
Queue Length 50th (m)		37.6	0.0	0.2	85.8		14.3	0.0				
Queue Length 95th (m)		67.0	3.0	2.0	88.7		27.5	0.0				
Internal Link Dist (m)		497.2			111.3			178.8			36.6	
Turn Bay Length (m)			50.0	35.0			30.0					
Base Capacity (vph)		1418	1261	563	1404		406	601				
Starvation Cap Reductn		0	0	0	0		0	0				
Spillback Cap Reductn		49	0	0	0		0	8				
Storage Cap Reductn		0	0	0	0		0	0				
Reduced v/c Ratio		0.46	0.06	0.16	0.63		0.15	0.14				

Cycle Length: 120

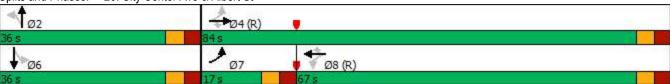
Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 100

Maximum v/c Ratio: 0.63		
Intersection Signal Delay: 8.2	Intersection LOS: A	
Intersection Capacity Utilization 78.8%	ICU Level of Service D	
Analysis Period (min) 15		

Splits and Phases: 20: City Center Ave & Albert St



	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> \$		*	<b>^</b>	W	
Traffic Volume (vph)	1019	32	60	1157	39	56
Future Volume (vph)	1019	32	60	1157	39	56
Satd. Flow (prot)	3359	0	1710	3386	1586	0
Flt Permitted			0.240		0.980	
Satd. Flow (perm)	3359	0	429	3386	1585	0
Satd. Flow (RTOR)	6				56	
Lane Group Flow (vph)	1107	0	63	1218	100	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		22.0	22.0	33.9	
Total Split (s)	86.0		86.0	86.0	34.0	
Total Split (%)	71.7%		71.7%	71.7%	28.3%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	94.5		94.5	94.5	13.8	
Actuated g/C Ratio	0.79		0.79	0.79	0.12	
v/c Ratio	0.42		0.19	0.46	0.43	
Control Delay	2.5		6.2	5.7	28.1	
Queue Delay	0.1		0.0	0.2	0.0	
Total Delay	2.6		6.2	5.9	28.1	
LOS	A		Α	А	С	
Approach Delay	2.6			5.9	28.1	
Approach LOS	A			Α	С	
Queue Length 50th (m)	16.4		2.6	35.0	10.5	
Queue Length 95th (m)	m32.8		12.8	91.6	23.8	
Internal Link Dist (m)	119.3			240.3	83.4	
Turn Bay Length (m)			65.0			
Base Capacity (vph)	2645		337	2665	414	
Starvation Cap Reductn	476		0	0	0	
Spillback Cap Reductn	0		0	596	9	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.51		0.19	0.59	0.25	
Intersection Summary						

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 27 (23%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 65

Maximum v/c Ratio: 0.46

Intersection Signal Delay: 5.3	Intersection LOS: A
Intersection Capacity Utilization 64.4%	ICU Level of Service C
–	

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	<b>→</b>	$\rightarrow$	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			41	W	
Traffic Volume (vph)	595	16	11	1021	9	14
Future Volume (vph)	595	16	11	1021	9	14
Satd. Flow (prot)	3406	0	0	3417	1619	0
Flt Permitted				0.946	0.982	
Satd. Flow (perm)	3406	0	0	3235	1619	0
Satd. Flow (RTOR)	4				15	
Lane Group Flow (vph)	643	0	0	1087	24	0
Turn Type	NA		Perm	NA	Perm	
Protected Phases				8		
Permitted Phases	4		8		2	
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	40.1		40.1	40.1	28.3	
Total Split (s)	81.0		81.0	81.0	39.0	
Total Split (%)	67.5%		67.5%	67.5%	32.5%	
Yellow Time (s)	3.3		3.3	3.3	3.3	
All-Red Time (s)	3.8		3.8	3.8	3.0	
Lost Time Adjust (s)	0.0		0.0	-3.1	-2.3	
Total Lost Time (s)	7.1			4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	106.0		J	107.8	12.3	
Actuated g/C Ratio	0.88			0.90	0.10	
v/c Ratio	0.21			0.37	0.13	
Control Delay	4.9			2.2	30.0	
Queue Delay	0.0			0.0	0.0	
Total Delay	4.9			2.2	30.0	
LOS	A.5			Α.2	C	
Approach Delay	4.9			2.2	30.0	
Approach LOS	4.5 A			Α.2	C	
Queue Length 50th (m)	41.5			27.4	2.1	
Queue Length 95th (m)	m39.1			34.0	10.7	
Internal Link Dist (m)	168.5			265.3	36.3	
Turn Bay Length (m)	100.0			200.0	00.0	
Base Capacity (vph)	3008			2906	482	
Starvation Cap Reductn	0			0	0	
Spillback Cap Reductn	0			0	0	
Storage Cap Reductn	0			0	0	
Reduced v/c Ratio	0.21			0.37	0.05	
	0.21			0.01	0.00	
Intersection Summary						

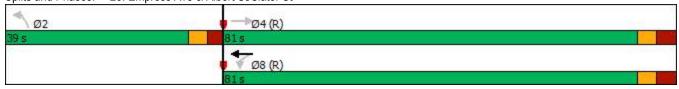
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 39 (33%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 70

Maximum v/c Ratio: 0.37		
Intersection Signal Delay: 3.5	Intersection LOS: A	
Intersection Capacity Utilization 53.0%	ICU Level of Service A	
Analysis Period (min) 15		
m Volume for 95th percentile queue is metered by upst	ream signal.	

Splits and Phases: 23: Empress Ave & Albert St/Slater St



	•	•	<b>†</b>	<b>/</b>	-	.↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	₩ #	<b>↑</b> ₽	INDIX	JDL	<u>↑</u>
Traffic Volume (vph)	0	10	1198	23	0	732
Future Volume (vph)	0	10	1198	23	0	732
Satd. Flow (prot)	0	1557	3410	0	0	3420
Flt Permitted						
Satd. Flow (perm)	0	1557	3410	0	0	3420
Lane Group Flow (vph)	0	11	1285	0	0	771
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 45.7%			ICI	U Level c	f Service
Analysis Period (min) 15						

# **APPENDIX F: Future Total Conditions - Synchro Outputs**



Synchro Modelling Outputs - Phase One (2030) Conditions



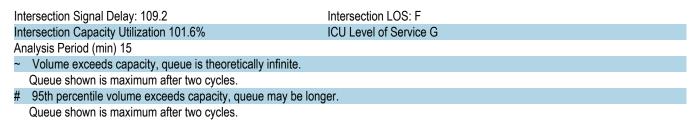
	۶	<b>→</b>	<b>—</b>	4	<b>\</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b> ^	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	602	990	342	704	1962	312
Future Volume (vph)	602	990	342	704	1962	312
Satd. Flow (prot)	3317	4914	3420	2693	3285	1530
Flt Permitted	0.950		120		0.950	. 500
Satd. Flow (perm)	3303	4914	3420	2693	3272	1501
Satd. Flow (RTOR)			120		<b></b>	69
Lane Group Flow (vph)	634	1042	360	741	2065	328
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	18	1	1 31111
Permitted Phases		7	- 0	10		1
Detector Phase	7	4	8	18	1	1
Switch Phase	1	7	U	10		
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
	42.8	70.3	27.5		51.1	51.1
Total Split (s)		57.9%	22.7%			42.1%
Total Split (%)	35.3% 3.3	3.3	3.3		42.1% 3.3	3.3
Yellow Time (s)						
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		-2.1	0.0
Total Lost Time (s)	5.8	6.5	6.5		4.0	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Min	74.0	Max	Max
Act Effct Green (s)	26.3	51.2	19.1	71.0	47.4	45.3
Actuated g/C Ratio	0.24	0.47	0.17	0.65	0.43	0.41
v/c Ratio	0.80	0.45	0.60	0.42	1.45	0.50
Control Delay	46.9	19.9	46.9	10.9	233.3	22.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.9	19.9	46.9	10.9	233.3	22.8
LOS	D	В	D	В	F	С
Approach Delay		30.1	22.7		204.5	
Approach LOS		С	С		F	
Queue Length 50th (m)	70.4	55.6	39.2		~332.7	42.6
Queue Length 95th (m)	90.6	66.5	61.1	71.7		81.2
Internal Link Dist (m)		244.7	295.5		47.9	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1130	2889	661	1729	1425	662
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.56	0.36	0.54	0.43	1.45	0.50

Cycle Length: 121.4

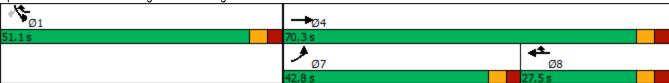
Actuated Cycle Length: 109.2

Natural Cycle: 145

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.45



Splits and Phases: 7: Wellington St & Portage



	ၨ	_	←	•	<b>\</b>	1	
		_				-	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		<b>↑</b>	<b>^</b>	7		7	
Traffic Volume (vph)	0	812	489	0	0	5	
Future Volume (vph)	0	812	489	0	0	5	
Satd. Flow (prot)	0	1800	1800	1800	0	1557	
Flt Permitted							
Satd. Flow (perm)	0	1800	1800	1800	0	1557	
Lane Group Flow (vph)	0	855	515	0	0	5	
Sign Control		Free	Free		Stop		
Intersection Summary							
Control Type: Unsignalized							
Intersection Capacity Utilization 48.4%			ICU Level of Service A				
Analysis Period (min) 15							

Lane Configurations         T         F         F         T         F	EDD WELL WEDT WEDT WELL WELL OFF ODE
Traffic Volume (vph) 5 0 5 0 0 0 37 826 0 0 1457 3	EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
	ካ ቱ ካ ቱ ካ ቱ
Future \(\delta\text{lump}\) \(\delta\text{lump}\text{lump}\) \(\delta\text{lump}\text{lump}\) \(\delta\text{lump}l	
	5 0 0 0 37 826 0 0 1457 37
Satd. Flow (prot) 1710 1530 0 1800 1800 0 1710 1800 0 1800 1793	0 1800 1800 0 1710 1800 0 1800 1793 0
Flt Permitted 0.066	
	0 1800 1800 0 119 1800 0 1800 1793 0
Satd. Flow (RTOR) 42 3	
Turn Type Perm NA Perm Perm NA Perm NA	
Protected Phases 4 8 2 6	
Permitted Phases 4 8 2 6	
Detector Phase 4 4 8 8 2 2 6 6	8 8 2 2 6 6
Switch Phase	
Minimum Initial (s) 10.0 10.0 10.0 10.0 10.0 10.0 10.0	
Minimum Split (s) 22.0 22.0 22.0 22.0 35.9 35.9 35.9	
Total Split (s) 22.0 22.0 22.0 53.0 53.0 53.0 53.0	
Total Split (%) 29.3% 29.3% 29.3% 70.7% 70.7% 70.7% 70.7%	
Yellow Time (s) 3.0 3.0 3.0 3.3 3.3 3.3	
All-Red Time (s) 3.2 3.2 3.2 2.6 2.6 2.6 2.6	
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Total Lost Time (s) 6.2 6.2 6.2 5.9 5.9 5.9	6.2 6.2 5.9 5.9 5.9
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode None None None C-Max C-Max C-Max C-Max	
Act Effct Green (s) 10.0 10.0 70.6 70.6 70.6	
Actuated g/C Ratio 0.13 0.13 0.94 0.94 0.94 0.94	
v/c Ratio 0.02 0.02 0.35 0.51 0.93	
Control Delay 28.6 0.2 15.8 3.4 17.6	
Queue Delay 0.0 0.0 0.0 0.0 0.0	
Total Delay 28.6 0.2 15.8 3.4 17.6	
LOS C A B A B	
Approach Delay 14.4 3.9 17.6	
Approach LOS B A B	
Queue Length 50th (m) 0.7 0.0 0.0 0.0 0.0	
Queue Length 95th (m) 3.6 0.0 #18.0 98.0 #380.6	
Internal Link Dist (m) 60.2 43.3 76.9 31.3	
Turn Bay Length (m) 30.0 30.0	
Base Capacity (vph) 379 355 112 1694 1687	
Starvation Cap Reductn 0 0 0 0	
Spillback Cap Reductn 0 0 0 0	
Storage Cap Reductn 0 0 0 0	
Reduced v/c Ratio 0.01 0.01 0.35 0.51 0.93	0.35 0.51 0.93

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 150

Maximum v/c Ratio: 0.93 Intersection LOS: B Intersection Signal Delay: 12.6 Intersection Capacity Utilization 101.7% ICU Level of Service G Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 10: Booth St & Chaudiere

ø <sub>2 (R)</sub>		<u>≯</u> <sub>04</sub>	
53 s		22 s	
<b>∮</b> Ø6 (R)	Ī	<b>▼</b> Ø8	
53 s		22 s	

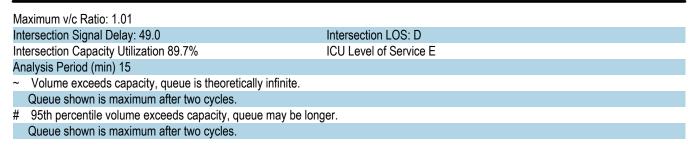
	۶	<b>→</b>	$\rightarrow$	•	•	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>∱</b> }		ሻ	<b>^</b>	7
Traffic Volume (vph)	0	1260	0	0	660	132	0	832	131	146	985	233
Future Volume (vph)	0	1260	0	0	660	132	0	832	131	146	985	233
Satd. Flow (prot)	0	3420	0	0	3386	1530	0	3104	0	1710	3226	1530
Flt Permitted										0.115		
Satd. Flow (perm)	0	3420	0	0	3386	1476	0	3104	0	207	3226	1464
Satd. Flow (RTOR)								19				127
Lane Group Flow (vph)	0	1326	0	0	695	139	0	1014	0	154	1037	245
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase												
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.9		12.0	37.9	37.9
Total Split (s)		48.0			48.0	48.0		35.0		12.0	47.0	47.0
Total Split (%)		50.5%			50.5%	50.5%		36.8%		12.6%	49.5%	49.5%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	0.0		-2.1		0.0	0.0	0.0
Total Lost Time (s)		6.8			6.8	6.8		4.7		6.8	6.9	6.9
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		40.3			40.3	40.3		30.3		41.1	41.0	41.0
Actuated g/C Ratio		0.42			0.42	0.42		0.32		0.43	0.43	0.43
v/c Ratio		0.92			0.48	0.22		1.01		0.83	0.74	0.35
Control Delay		41.2			27.3	22.9		64.2		55.3	25.6	9.6
Queue Delay		3.4			0.0	0.0		32.2		62.0	0.0	0.0
Total Delay		44.5			27.3	22.9		96.4		117.3	25.6	9.6
LOS		D			C	С		F		F	C	Α
Approach Delay		44.5			26.5			96.4			32.7	
Approach LOS		D			C	00.0		F		40.4	C	440
Queue Length 50th (m)		121.2			62.2	20.9		~103.1		18.4	87.5	14.0
Queue Length 95th (m)		#174.5			83.1	37.5		#148.6		#54.4	81.8	24.5
Internal Link Dist (m)		67.5			116.7	70.0		55.5		145.0	192.9	EE 0
Turn Bay Length (m)		1402			1460			1000			1202	55.0
Base Capacity (vph)		1483			1468	640		1002		186	1393	704
Starvation Cap Reducts		94			0	0		0		0 62	0	0
Spillback Cap Reductn		51 0			0	0		98		0	0	0
Storage Cap Reductn Reduced v/c Ratio					0 47	0 22		1 12				
Reduced V/C Katlo		0.95			0.47	0.22		1.12		1.24	0.74	0.35

Cycle Length: 95

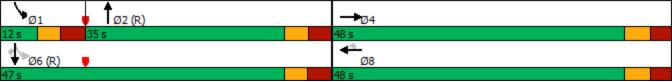
Actuated Cycle Length: 95

Offset: 31 (33%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90



Splits and Phases: 11: Booth St & Wellington St



	•	<b>→</b>	•	•	<b>←</b>	*	1	†	/	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	ሻ	<b>^</b>	7		€Î}		7	<b>1</b>	7
Traffic Volume (vph)	392	814	19	12	290	154	6	540	41	173	477	436
Future Volume (vph)	392	814	19	12	290	154	6	540	41	173	477	436
Satd. Flow (prot)	1660	1748	1443	1710	3196	1378	0	3324	0	1425	1782	1471
Flt Permitted	0.432			0.131				0.949		0.176		
Satd. Flow (perm)	721	1748	1377	236	3196	1251	0	3153	0	255	1782	1313
Satd. Flow (RTOR)			91					6				
Lane Group Flow (vph)	413	857	20	13	305	162	0	617	0	182	502	459
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		5.0	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	18.0	55.0	55.0	37.0	37.0	37.0	40.0	40.0		25.0	65.0	65.0
Total Split (%)	15.0%	45.8%	45.8%	30.8%	30.8%	30.8%	33.3%	33.3%		20.8%	54.2%	54.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	-2.5	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	4.0	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	None
Act Effct Green (s)	56.8	59.3	59.3	30.5	30.5	30.5		28.3		50.2	50.2	50.2
Actuated g/C Ratio	0.47	0.49	0.49	0.25	0.25	0.25		0.24		0.42	0.42	0.42
v/c Ratio	0.83	0.99	0.03	0.22	0.38	0.51		0.82		0.71	0.67	0.84
Control Delay	38.1	52.1	0.1	46.2	38.2	44.3		52.7		38.3	32.4	44.7
Queue Delay	0.0	36.5	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	38.1	88.6	0.1	46.2	38.2	44.3		52.7		38.3	32.4	44.7
LOS	D	F	Α	D	D	D		D		D	С	D
Approach Delay		71.1			40.5			52.7			38.3	
Approach LOS		Е			D			D			D	
Queue Length 50th (m)	48.6	~187.5	0.0	2.2	29.2	29.7		75.9		29.2	97.9	98.8
Queue Length 95th (m)	#170.7		m0.0	8.1	42.3	48.4		93.0		44.0	121.4	131.0
Internal Link Dist (m)		141.9			62.1			37.2			83.1	
Turn Bay Length (m)	160.0		40.0	40.0								120.0
Base Capacity (vph)	496	863	726	59	812	317		884		287	868	640
Starvation Cap Reductn	0	98	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	0.83	1.12	0.03	0.22	0.38	0.51		0.70		0.63	0.58	0.72

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 2 (2%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 105

Maximum v/c Ratio: 0.99
Intersection Signal Delay: 53.1 Intersection LOS: D
Intersection Capacity Utilization 122.7% ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	<b>†</b>	7	J.	<b>↑</b> 1>			र्स	7		4	
Traffic Volume (vph)	4	900	136	393	391	12	112	2	291	20	6	0
Future Volume (vph)	4	900	136	393	391	12	112	2	291	20	6	0
Satd. Flow (prot)	1710	1748	1471	1629	3274	0	0	1590	1471	0	1733	0
Flt Permitted	0.504			0.062				0.710			0.737	
Satd. Flow (perm)	907	1748	1384	106	3274	0	0	1169	1417	0	1327	0
Satd. Flow (RTOR)			86		6				306			
Lane Group Flow (vph)	4	947	143	414	425	0	0	120	306	0	27	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	65.0	65.0	65.0	25.0	90.0		30.0	30.0	30.0	30.0	30.0	
Total Split (%)	54.2%	54.2%	54.2%	20.8%	75.0%		25.0%	25.0%	25.0%	25.0%	25.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	-2.8	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	4.0	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	Min	C-Min		None	None	None	None	None	
Act Effct Green (s)	58.2	61.0	61.0	90.3	89.7			17.2	17.2		16.4	
Actuated g/C Ratio	0.48	0.51	0.51	0.75	0.75			0.14	0.14		0.14	
v/c Ratio	0.01	1.07	0.19	1.03	0.17			0.72	0.66		0.15	
Control Delay	13.5	74.2	4.4	91.3	7.9			71.2	12.0		44.5	
Queue Delay	0.0	14.6	0.0	0.0	0.0			0.0	0.3		0.0	
Total Delay	13.5	88.8	4.4	91.3	7.9			71.2	12.4		44.5	
LOS	В	F	Α	F	Α			E	В		D	
Approach Delay		77.5			49.1			28.9			44.5	
Approach LOS	0.4	E	4.4	04.0	D			C	0.0		D	
Queue Length 50th (m)	0.4	~260.3	4.4	~94.0	21.6			28.7	0.0		5.9	
Queue Length 95th (m)	m0.7	#338.4	m5.6	#181.6	30.1			47.6	26.1		14.2	
Internal Link Dist (m)	20.0	88.5	16.0	00.0	119.1			54.7			65.4	
Turn Bay Length (m)	30.0	000	16.0	90.0	0447			020	F0F		000	
Base Capacity (vph)	439	888	745	400	2447			230	525		262	
Starvation Cap Reducts	0	12	0	0	0			0	0		0	
Spillback Cap Reductn	0	183	0	0	0			0	32		0	
Storage Cap Reductn	0 01	1 24	0 10	1.03	0 17			0.53	0 63		0 10	
Reduced v/c Ratio	0.01	1.34	0.19	1.03	0.17			0.52	0.62		0.10	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 150

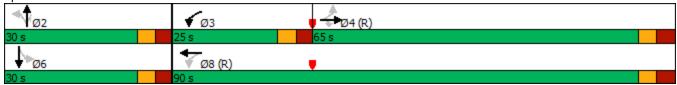
Maximum v/c Ratio: 1.07
Intersection Signal Delay: 58.4 Intersection LOS: E
Intersection Capacity Utilization 97.9% ICU Level of Service F
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	~	<b>&gt;</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> β		7	<b>↑</b> ↑			4			4	
Traffic Volume (vph)	15	1356	1	9	886	4	2	0	6	15	0	2
Future Volume (vph)	15	1356	1	9	886	4	2	0	6	15	0	2
Satd. Flow (prot)	1710	3420	0	1710	3383	0	0	1583	0	0	1681	0
Flt Permitted	0.305			0.173				0.909			0.944	
Satd. Flow (perm)	548	3420	0	311	3383	0	0	1455	0	0	1656	0
Satd. Flow (RTOR)					1			34			34	
Lane Group Flow (vph)	16	1428	0	9	937	0	0	8	0	0	18	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		37.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	62.0	62.0		62.0	62.0		33.0	33.0		33.0	33.0	
Total Split (%)	65.3%	65.3%		65.3%	65.3%		34.7%	34.7%		34.7%	34.7%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	86.1	86.1		86.1	86.1			10.0			10.0	
Actuated g/C Ratio	0.91	0.91		0.91	0.91			0.11			0.11	
v/c Ratio	0.03	0.46		0.03	0.31			0.04			0.09	
Control Delay	2.4	2.6		3.2	2.0			0.5			7.1	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	2.4	2.6		3.2	2.0			0.5			7.1	
LOS	А	Α		Α	Α			Α			Α	
Approach Delay		2.6			2.0			0.5			7.1	
Approach LOS		Α			Α			Α			Α	
Queue Length 50th (m)	0.0	0.0		0.0	0.0			0.0			0.0	
Queue Length 95th (m)	m1.3	59.6		m1.7	39.3			0.0			3.7	
Internal Link Dist (m)		651.1			64.4			41.8			21.1	
Turn Bay Length (m)	40.0			50.0								
Base Capacity (vph)	496	3099		281	3065			433			489	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.03	0.46		0.03	0.31			0.02			0.04	
Internation Comment												

Intersection Summary

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 59 (62%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

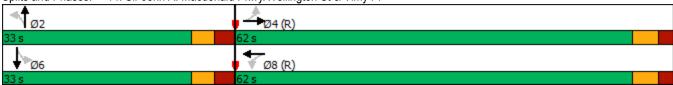
Natural Cycle: 70

## 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.46
Intersection Signal Delay: 2.4
Intersection Capacity Utilization 59.5%
Analysis Period (min) 15
Intersection LOS: A
ICU Level of Service B

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	۶	<b>→</b>	$\rightarrow$	•	←	•	1	<b>†</b>	<b>/</b>	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>1</b>			f)	
Traffic Volume (vph)	0	1466	22	1	998	1	1	21	1	1	5	3
Future Volume (vph)	0	1466	22	1	998	1	1	21	1	1	5	3
Satd. Flow (prot)	0	3412	0	0	3386	0	0	1784	0	0	1699	0
Flt Permitted					0.954			0.984			0.956	
Satd. Flow (perm)	0	3412	0	0	3230	0	0	1759	0	0	1634	0
Satd. Flow (RTOR)		3						1			3	
Lane Group Flow (vph)	0	1566	0	0	1053	0	0	24	0	0	9	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)		86.3			86.3			10.0			10.0	
Actuated g/C Ratio		0.91			0.91			0.11			0.11	
v/c Ratio		0.51			0.36			0.13			0.05	
Control Delay		3.0			2.2			39.3			33.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		3.0			2.2			39.3			33.4	
LOS		Α			Α			D			С	
Approach Delay		3.0			2.2			39.3			33.4	
Approach LOS		Α			Α			D			С	
Queue Length 50th (m)		0.0			3.5			4.1			1.1	
Queue Length 95th (m)		67.9			50.3			12.1			5.8	
Internal Link Dist (m)		176.2			288.5			38.4			167.1	
Turn Bay Length (m)												
Base Capacity (vph)		3099			2933			513			478	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.51			0.36			0.05			0.02	

Cycle Length: 95

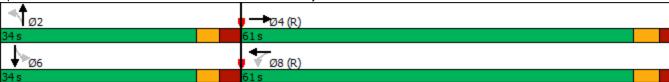
Actuated Cycle Length: 95

Offset: 34 (36%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 80

Maximum v/c Ratio: 0.51
Intersection Signal Delay: 3.1
Intersection Capacity Utilization 63.5%
ICU Level of Service B
Analysis Period (min) 15





# 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

	•	-	$\rightarrow$	•	•	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	f)		7	<b>†</b>	7	¥	<b>†</b>	7	J.	f)	
Traffic Volume (vph)	14	765	86	56	331	30	61	45	130	131	178	12
Future Volume (vph)	14	765	86	56	331	30	61	45	130	131	178	12
Satd. Flow (prot)	1500	1720	0	1613	1698	1457	1676	1800	1515	1693	1752	0
Flt Permitted	0.545			0.215			0.499			0.726		
Satd. Flow (perm)	826	1720	0	365	1698	1328	871	1800	1420	1243	1752	0
Satd. Flow (RTOR)		11				37			137		3	
Lane Group Flow (vph)	15	896	0	59	348	32	64	47	137	138	200	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	68.9	68.9		68.9	68.9	68.9	18.2	18.2	18.2	18.2	18.2	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.03	0.75		0.24	0.30	0.03	0.41	0.14	0.37	0.61	0.62	
Control Delay	3.2	10.7		10.6	7.9	2.3	41.7	32.5	8.4	48.1	44.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.2	10.7		10.6	7.9	2.3	41.7	32.5	8.4	48.1	44.8	
LOS	Α	В		В	Α	Α	D	С	Α	D	D	
Approach Delay		10.6			7.8			21.6			46.1	
Approach LOS		В			Α			С			D	
Queue Length 50th (m)	0.2	12.7		3.7	22.8	0.0	11.9	8.3	0.0	26.8	38.2	
Queue Length 95th (m)	m0.6	#219.1		13.0	47.3	3.1	23.2	16.9	14.9	43.1	56.3	
Internal Link Dist (m)		635.7			497.2			83.8			171.5	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	569	1188		251	1170	926	222	460	465	318	450	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.03	0.75		0.24	0.30	0.03	0.29	0.10	0.29	0.43	0.44	

Intersection Summary

Cycle Length: 100

Actuated Cycle Length: 100

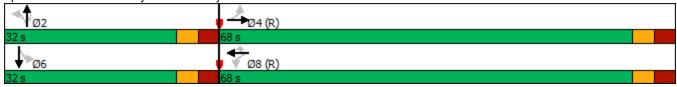
Offset: 40 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

### 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

Maximum v/c Ratio: 0.75
Intersection Signal Delay: 17.6 Intersection LOS: B
Intersection Capacity Utilization 88.4% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u></u>	7	*	<b>†</b>	7	*	f)		7	f)	
Traffic Volume (vph)	120	681	37	50	295	24	31	281	69	97	354	145
Future Volume (vph)	120	681	37	50	295	24	31	281	69	97	354	145
Satd. Flow (prot)	1676	1748	1485	1676	1698	1471	1660	1722	0	1693	1564	0
Flt Permitted	0.525			0.189			0.215			0.394		
Satd. Flow (perm)	841	1748	1485	334	1698	1209	363	1722	0	702	1564	0
Satd. Flow (RTOR)			58			58		14			23	
Lane Group Flow (vph)	126	717	39	53	311	25	33	369	0	102	526	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	28.1	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	43.0	43.0		43.0	43.0	
Total Split (%)	51.0%	51.0%	51.0%	51.0%	51.0%	51.0%	43.0%	43.0%		43.0%	43.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	0.0	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	4.0	6.1	6.1	4.0	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	50.9	50.9	53.0	50.9	50.9	53.0	36.7	36.7		36.7	36.7	
Actuated g/C Ratio	0.51	0.51	0.53	0.51	0.51	0.53	0.37	0.37		0.37	0.37	
v/c Ratio	0.29	0.81	0.05	0.31	0.36	0.04	0.25	0.58		0.40	0.89	
Control Delay	16.6	29.2	1.8	17.2	14.0	0.4	28.2	28.7		29.3	48.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	16.6	29.2	1.8	17.2	14.0	0.4	28.2	28.7		29.3	48.5	
LOS	В	С	Α	В	В	Α	С	С		С	D	
Approach Delay		26.2			13.5			28.7			45.3	
Approach LOS		С			В			С			D	
Queue Length 50th (m)	14.2	116.6	0.0	7.2	43.6	0.1	4.6	57.4		15.2	96.1	
Queue Length 95th (m)	27.3	171.0	3.1	18.7	28.1	m0.4	13.3	87.8		31.3	#160.9	
Internal Link Dist (m)		92.9			635.7			70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	428	889	814	170	864	668	133	640		257	588	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.29	0.81	0.05	0.31	0.36	0.04	0.25	0.58		0.40	0.89	

Cycle Length: 100 Actuated Cycle Length: 100

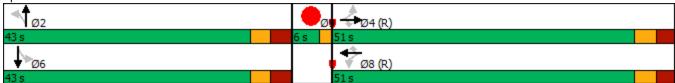
Offset: 91 (91%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Lane Configurations  Traffic Volume (vph)  Future Volume (vph)  Satd. Flow (prot)  Fit Permitted  Satd. Flow (perm)  Satd. Flow (RTOR)  Lane Group Flow (vph)  Turn Type  Protected Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (%)  Yellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Tum Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio  Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Satd. Flow (prot) FIt Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vph) Satd. Flow (prot) Fit Permitted Satd. Flow (prom) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Solution Split (s) So				
Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Source Substited Split (s) 6.0 Total Split (s) 7.0 Total Cost Time (s) 7.0 Lead/Lag 8.0 Lead/Lag 9.0 Le				
Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Ype Internal Park (m) Internal Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Sellow Time (s) All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Solution Split (s				
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Minimum				
Protected Phases  Permitted Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (s)  Total Split (%)  Sylellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (yph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Cotal Split (%) Sellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lost Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 55th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		9		
Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Minimum Split (s) 6.0  Total Split (s) 6.0  Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio	Switch Phase			
Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Minimum Initial (s)	2.0		
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Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
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Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio		0.0		
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Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
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Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		NOTIC		
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	. ,			
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	•			
LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)			
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary				
	Intersection Summary			

Maximum v/c Ratio: 0.89
Intersection Signal Delay: 29.7
Intersection Capacity Utilization 105.5%
ICU Level of Service G
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	-	•	•	<b>←</b>	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		7				
Traffic Volume (vph)	0	0	0	0	0	134
Future Volume (vph)	0	0	0	0	0	134
Satd. Flow (prot)	0	1800	0	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1800	0	0	0	0
Lane Group Flow (vph)	0	0	0	0	141	0
Sign Control	Free			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion Err%			IC	U Level c	of Service
Analysis Period (min) 15						

	•	•	<b>1</b>	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	N/		*	<b>^</b>	<b>↑</b> ↑		10.0	
Traffic Volume (vph)	1	1	2	862	1461	6		
Future Volume (vph)	1	1	2	862	1461	6		
Satd. Flow (prot)	1600	0	1710	3420	3417	0		
Flt Permitted	0.976		0.155					
Satd. Flow (perm)	1598	0	279	3420	3417	0		
Satd. Flow (RTOR)	1				1			
Lane Group Flow (vph)	2	0	2	907	1544	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	6.3		10.0	10.0	10.0		10.0	
Minimum Split (s)	14.0		30.7	30.7	30.9		29.0	
Total Split (s)	17.0		49.0	49.0	49.0		29.0	
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.4	2.4	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.7	5.7	5.9			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	6.3		91.4	91.4	91.3			
Actuated g/C Ratio	0.07		0.96	0.96	0.96			
v/c Ratio	0.02		0.01	0.28	0.47			
Control Delay	36.0		0.5	0.3	1.4			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	36.0		0.5	0.3	1.4			
LOS	D		Α	Α	Α			
Approach Delay	36.0			0.3	1.4			
Approach LOS	D			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	2.4		m0.0	m5.2	51.7			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	182		268	3289	3285			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.01		0.01	0.28	0.47			
Intersection Summary								

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 47 (49%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

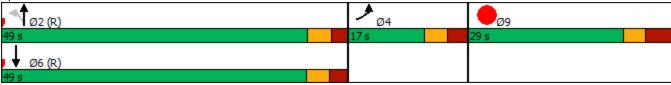
Natural Cycle: 90

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 1.0	Intersection LOS: A	
Intersection Capacity Utilization 58.2%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 19: Booth St & War Museum



	•	<b>→</b>	•	•	•	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	7	<b>†</b>	7	7	£			4	
Traffic Volume (vph)	5	966	77	98	379	14	28	0	47	35	0	5
Future Volume (vph)	5	966	77	98	379	14	28	0	47	35	0	5
Satd. Flow (prot)	1710	1782	1485	1629	1714	1530	1644	1404	0	0	946	0
Flt Permitted	0.479			0.242			0.730				0.716	
Satd. Flow (perm)	825	1782	1406	415	1714	1311	1235	1404	0	0	707	0
Satd. Flow (RTOR)			52			87		132			87	
Lane Group Flow (vph)	5	1017	81	103	399	15	29	49	0	0	42	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	98.1	99.3	101.2	96.9	96.9	98.7	12.6	12.6			12.6	
Actuated g/C Ratio	0.82	0.83	0.84	0.81	0.81	0.82	0.10	0.10			0.10	
v/c Ratio	0.01	0.69	0.07	0.31	0.29	0.01	0.22	0.18			0.28	
Control Delay	4.0	9.9	1.6	10.7	5.7	0.1	51.3	1.5			4.7	
Queue Delay	0.0	1.1	0.0	0.0	0.0	0.0	0.0	0.0			0.1	
Total Delay	4.0	11.0	1.6	10.7	5.7	0.1	51.3	1.6			4.7	
LOS	А	В	Α	В	Α	Α	D	Α			Α	
Approach Delay		10.3			6.5			20.1			4.7	
Approach LOS		В			Α			С			Α	
Queue Length 50th (m)	0.3	87.6	1.0	0.3	1.1	0.0	6.9	0.0			0.0	
Queue Length 95th (m)	1.5	226.9	6.1	45.1	110.7	m0.2	15.0	0.0			0.6	
Internal Link Dist (m)		497.2			104.8			178.8			41.9	
Turn Bay Length (m)	30.0		50.0	35.0		50.0	30.0					
Base Capacity (vph)	753	1475	1193	334	1384	1093	305	446			240	
Starvation Cap Reductn	0	0	0	0	0	0	0	0			0	
Spillback Cap Reductn	0	229	0	0	0	0	0	16			10	
Storage Cap Reductn	0	0	0	0	0	0	0	0			0	
Reduced v/c Ratio	0.01	0.82	0.07	0.31	0.29	0.01	0.10	0.11			0.18	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

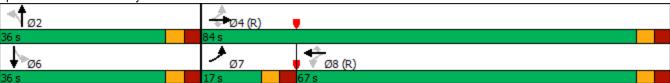
Natural Cycle: 90

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 9.5	Intersection LOS: A
Intersection Capacity Utilization 89.5%	ICU Level of Service E
Analysis Period (min) 15	

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: City Centre Ave & Albert St



	۶	-	←	•	<b>&gt;</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	<u> </u>	<b>†</b>	1151	¥	ODIT
Traffic Volume (vph)	0	1200	732	0	0	0
Future Volume (vph)	0	1200	732	0	0	0
Satd. Flow (prot)	1800	1748	3226	0	1800	0
Flt Permitted	1000	1740	JZZ0	U	1000	0
Satd. Flow (perm)	1800	1748	3226	0	1800	0
Satd. Flow (RTOR)	1000	1740	3220	U	1000	U
Lane Group Flow (vph)	0	1263	771	0	0	0
, , ,	Perm	NA	NA	U	Prot	U
Turn Type Protected Phases	Pellii	1NA 4	NA 8		6	
	4	4	Ö		b	
Permitted Phases	4	4	0		0	
Detector Phase	4	4	8		6	
Switch Phase	10.5	400	400		40.0	
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	22.0	22.0	22.0		28.7	
Total Split (s)	91.3	91.3	91.3		28.7	
Total Split (%)	76.1%	76.1%	76.1%		23.9%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.4	1.4	1.4		1.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	4.7	4.7	4.7		4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Min		None	
Act Effct Green (s)		113.5	113.5			
Actuated g/C Ratio		0.95	0.95			
v/c Ratio		0.76	0.25			
Control Delay		15.0	1.6			
Queue Delay		2.3	0.0			
Total Delay		17.3	1.7			
LOS		В	A			
Approach Delay		17.3	1.7			
Approach LOS		В	Α			
Queue Length 50th (m)		67.6	0.1			
Queue Length 95th (m)		m328.9	27.8			
Internal Link Dist (m)		119.1	141.9		77.8	
Turn Bay Length (m)		113.1	1-1.3		11.0	
Base Capacity (vph)		1653	3050			
		159	275			
Starvation Cap Reductn Spillback Cap Reductn		255				
•			0			
Storage Cap Reductn		0 00	0 20			
Reduced v/c Ratio		0.90	0.28			
Intersection Summary						
Cycle Length: 120						
Actuated Cycle Length: 120	0					
Offset: 0 (0%) Peteroneed		EDTL on	4 0 · / V/DT	Start of C	roon	

2030 LeBreton Flats 8:00 am 02-04-2020 JF

Control Type: Actuated-Coordinated

Natural Cycle: 110

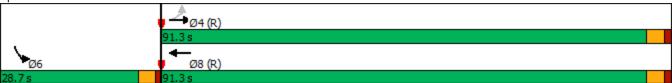
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 11.4	Intersection LOS: B	
Intersection Capacity Utilization 70.6%	ICU Level of Service C	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 21: Albert St & Access 1



	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> \$		*	<b>^</b>	W	
Traffic Volume (vph)	1421	16	26	727	37	77
Future Volume (vph)	1421	16	26	727	37	77
Satd. Flow (prot)	3365	0	1710	3320	1522	0
Flt Permitted			0.134		0.984	
Satd. Flow (perm)	3365	0	241	3320	1519	0
Satd. Flow (RTOR)	2				33	
Lane Group Flow (vph)	1513	0	27	765	120	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	1	
Permitted Phases			8			
Detector Phase	4		8	8	1	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		22.0	22.0	33.9	
Total Split (s)	61.0		61.0	61.0	34.0	
Total Split (%)	64.2%		64.2%	64.2%	35.8%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	68.9		68.9	68.9	14.4	
Actuated g/C Ratio	0.73		0.73	0.73	0.15	
v/c Ratio	0.62		0.16	0.32	0.47	
Control Delay	18.8		9.0	6.0	30.6	
Queue Delay	8.1		0.0	0.0	0.0	
Total Delay	26.9		9.0	6.0	30.6	
LOS	С		Α	А	С	
Approach Delay	26.9			6.1	30.6	
Approach LOS	С			Α	С	
Queue Length 50th (m)	118.7		1.1	19.3	16.0	
Queue Length 95th (m)	m150.0		7.7	51.8	27.0	
Internal Link Dist (m)	116.7			244.7	48.8	
Turn Bay Length (m)			70.0			
Base Capacity (vph)	2440		174	2407	473	
Starvation Cap Reductn	894		0	0	0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.98		0.16	0.32	0.25	
Intersection Summary						

Cycle Length: 95

Actuated Cycle Length: 95

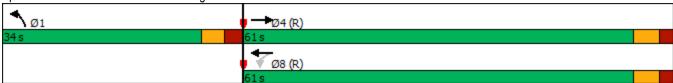
Offset: 27 (28%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 75

Maximum v/c Ratio: 0.62		
Intersection Signal Delay: 20.3	Intersection LOS: C	
Intersection Capacity Utilization 61.1%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	۶	-	$\rightarrow$	•	←	•	4	<b>†</b>	<b>/</b>	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> ∱			414			4			4	
Traffic Volume (vph)	19	1006	8	5	404	7	8	0	11	28	0	7
Future Volume (vph)	19	1006	8	5	404	7	8	0	11	28	0	7
Satd. Flow (prot)	1710	3417	0	0	3410	0	0	1621	0	0	1685	0
Flt Permitted	0.498				0.945			0.884			0.754	
Satd. Flow (perm)	896	3417	0	0	3225	0	0	1462	0	0	1322	0
Satd. Flow (RTOR)		1			3			15			15	
Lane Group Flow (vph)	20	1067	0	0	437	0	0	20	0	0	36	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6		30.5	30.5		30.5	30.5	
Total Split (s)	81.0	81.0		81.0	81.0		39.0	39.0		39.0	39.0	
Total Split (%)	67.5%	67.5%		67.5%	67.5%		32.5%	32.5%		32.5%	32.5%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.6	4.6			4.6			4.3			4.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	105.7	105.7			105.7			13.0			13.0	
Actuated g/C Ratio	0.88	0.88			0.88			0.11			0.11	
v/c Ratio	0.03	0.35			0.15			0.12			0.23	
Control Delay	1.5	2.2			2.4			24.4			34.3	
Queue Delay	0.0	0.3			0.0			0.0			0.0	
Total Delay	1.5	2.5			2.4			24.4			34.3	
LOS	А	Α			Α			С			С	
Approach Delay		2.5			2.4			24.4			34.3	
Approach LOS	•	A			A			С			С	
Queue Length 50th (m)	0.1	7.2			7.5			1.2			4.9	
Queue Length 95th (m)	m1.0	m62.8			22.1			7.9			13.7	
Internal Link Dist (m)		82.5			218.4			58.0			17.2	
Turn Bay Length (m)	70.0	2222			2212			400				
Base Capacity (vph)	788	3008			2840			433			392	
Starvation Cap Reductn	0	1137			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.03	0.57			0.15			0.05			0.09	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

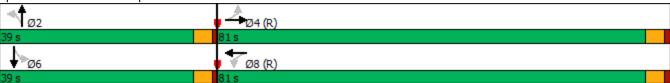
Natural Cycle: 60

Maximum v/c Ratio: 0.35

Intersection Signal Delay: 3.5	Intersection LOS: A	
Intersection Capacity Utilization 45.4%	ICU Level of Service A	
Analysis Pariod (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	ၨ	<b>→</b>	•	•	•	•	4	<b>†</b>	<i>&gt;</i>	<b>\</b>	<b>↓</b>	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>∱</b> }				7			7
Traffic Volume (vph)	0	1014	0	0	424	7	0	0	0	0	0	7
Future Volume (vph)	0	1014	0	0	424	7	0	0	0	0	0	7
Satd. Flow (prot)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Flt Permitted												
Satd. Flow (perm)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Lane Group Flow (vph)	0	1067	0	0	453	0	0	0	0	0	0	7
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	on 32.9%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

	ၨ	<b>→</b>	•	•	•	•	4	<b>†</b>	~	<b>\</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		<b>↑</b> ↑			<b>↑</b> ↑	
Traffic Volume (vph)	0	0	8	0	0	12	0	855	17	0	963	0
Future Volume (vph)	0	0	8	0	0	12	0	855	17	0	963	0
Satd. Flow (prot)	0	0	1557	0	0	1557	0	3410	0	0	3420	0
Flt Permitted												
Satd. Flow (perm)	0	0	1557	0	0	1557	0	3410	0	0	3420	0
Lane Group Flow (vph)	0	0	8	0	0	13	0	918	0	0	1014	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizati	ion 38.1%			IC	U Level o	of Service	Α					
Analysis Period (min) 15												

	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>↑</b> 1>			<b>^</b>		7
Traffic Volume (vph)	1271	0	0	893	0	6
Future Volume (vph)	1271	0	0	893	0	6
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Lane Group Flow (vph)	1338	0	0	940	0	6
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 47.1%			IC	U Level c	of Service A
Analysis Period (min) 15						

	-	•	$\checkmark$	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		ሻ	<b>^</b>	¥	
Traffic Volume (vph)	1265	1	9	884	2	6
Future Volume (vph)	1265	1	9	884	2	6
Satd. Flow (prot)	3420	0	1710	3386	1599	0
Flt Permitted			0.950		0.988	
Satd. Flow (perm)	3420	0	1710	3386	1599	0
Satd. Flow (RTOR)					6	
Lane Group Flow (vph)	1333	0	9	931	8	0
Turn Type	NA		Prot	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases						
Detector Phase	4		3	8	2	
Switch Phase						
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	37.0		11.0	37.0	32.3	
Total Split (s)	47.0		15.0	62.0	33.0	
Total Split (%)	49.5%	1	5.8%	65.3%	34.7%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.3		2.3	2.3	3.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	6.0		6.0	6.0	6.3	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	87.9		6.1	90.5	10.0	
Actuated g/C Ratio	0.93		0.06	0.95	0.11	
v/c Ratio	0.42		0.08	0.29	0.05	
Control Delay	4.2		51.2	2.4	26.4	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	4.2		51.2	2.4	26.4	
LOS	A		D	A	С	
Approach Delay	4.2			2.8	26.4	
Approach LOS	Α			A	С	
Queue Length 50th (m)	0.0		1.8	1.2	0.4	
Queue Length 95th (m)	136.3		m2.8	94.2	4.8	
Internal Link Dist (m)	77.2			49.9	32.0	
Turn Bay Length (m)			60.0			
Base Capacity (vph)	3164		162	3227	453	
Starvation Cap Reductn	104		0	0	0	
Spillback Cap Reductn	280		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.46		0.06	0.29	0.02	
_						

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 22 (23%), Referenced to phase 4:EBT and 8:WBT, Start of Green

Natural Cycle: 85

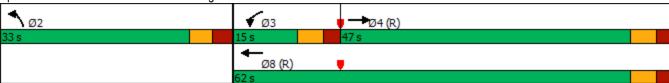
Maximum v/c Ratio: 0.42

Intersection Signal Delay: 3.7 Intersection LOS: A Intersection Capacity Utilization 55.5% ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 27: Broad St & Wellington St



	-	•	•	<b>←</b>	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> ∱			<b>^</b>		7
Traffic Volume (vph)	1260	1	0	884	0	5
Future Volume (vph)	1260	1	0	884	0	5
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Lane Group Flow (vph)	1327	0	0	931	0	5
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalized	t					
Intersection Capacity Utiliz	ation 46.8%			IC	U Level c	of Service A
Analysis Period (min) 15						

	۶	<b>→</b>	<b>—</b>	4	<b>\</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	11	ሻሻ	7
Traffic Volume (vph)	804	393	706	1105	852	455
Future Volume (vph)	804	393	706	1105	852	455
Satd. Flow (prot)	3317	4914	3386	2614	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3291	4914	3386	2614	3168	1530
Satd. Flow (RTOR)						180
Lane Group Flow (vph)	846	414	743	1163	897	479
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	81	1	
Permitted Phases						1
Detector Phase	7	4	8	8 1	1	1
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
Total Split (s)	55.8	92.3	36.5		44.1	44.1
Total Split (%)	40.9%	67.7%	26.8%		32.3%	32.3%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.8	6.5	6.5		6.1	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Min	Min	Max		Max	Max
Act Effct Green (s)	36.6	72.5	30.1	74.3	38.1	38.1
Actuated g/C Ratio	0.30	0.59	0.24	0.60	0.31	0.31
v/c Ratio	0.86	0.14	0.90	0.74	0.88	0.80
Control Delay	50.5	11.4	60.7	22.3	52.6	36.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.5	11.4	60.7	22.3	52.6	36.3
LOS	D	В	Е	С	D	D
Approach Delay		37.6	37.3		46.9	
Approach LOS		D	D		D	
Queue Length 50th (m)	104.6	16.4	97.2	114.4	112.0	73.2
Queue Length 95th (m)	129.2	21.5	#150.3	176.4	#168.8	#144.2
Internal Link Dist (m)		245.5	293.2		634.3	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1350	3431	826	1576	1016	597
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.63	0.12	0.90	0.74	0.88	0.80

Cycle Length: 136.4 Actuated Cycle Length: 123.2

Natural Cycle: 115 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.90

Intersection Signal Delay: 40.3 Intersection LOS: D
Intersection Capacity Utilization 85.8% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 7: Wellington St & Portage



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		_				-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<b>↑</b>	<b>↑</b>	7		7
Traffic Volume (vph)	0	812	1054	0	0	7
Future Volume (vph)	0	812	1054	0	0	7
Satd. Flow (prot)	0	1800	1800	1800	0	1557
Flt Permitted						
Satd. Flow (perm)	0	1800	1800	1800	0	1557
Lane Group Flow (vph)	0	855	1109	0	0	7
Sign Control		Free	Free		Stop	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization	on 68.6%			IC	U Level c	of Service (
Analysis Period (min) 15						

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>^}</b>		ሻ	f)		*	f)		ሻ	f.	
Traffic Volume (vph)	33	0	33	0	0	0	7	1108	0	0	1042	7
Future Volume (vph)	33	0	33	0	0	0	7	1108	0	0	1042	7
Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1798	0
Flt Permitted	0.757						0.164					
Satd. Flow (perm)	1363	1530	0	1800	1800	0	295	1800	0	1800	1798	0
Satd. Flow (RTOR)		108									1	
Lane Group Flow (vph)	35	35	0	0	0	0	7	1166	0	0	1104	0
Turn Type	Perm	NA		Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		35.9	35.9		35.9	35.9	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)	10.0	10.0					61.7	61.7			61.7	
Actuated g/C Ratio	0.13	0.13					0.82	0.82			0.82	
v/c Ratio	0.19	0.12					0.03	0.79			0.75	
Control Delay	31.9	0.8					3.7	13.3			11.4	
Queue Delay	0.0	0.0					0.0	0.0			0.0	
Total Delay	31.9	0.8					3.7	13.3			11.4	
LOS	С	Α					Α	В			В	
Approach Delay		16.3						13.2			11.4	
Approach LOS		В						В			В	
Queue Length 50th (m)	4.7	0.0					0.3	118.8			102.4	
Queue Length 95th (m)	12.9	0.0					1.4	#242.0			#222.2	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)	30.0						30.0					
Base Capacity (vph)	287	407					243	1481			1480	
Starvation Cap Reductn	0	0					0	0			0	
Spillback Cap Reductn	0	0					0	0			0	
Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio	0.12	0.09					0.03	0.79			0.75	

Cycle Length: 75

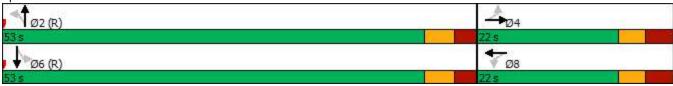
Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.79 Intersection Signal Delay: 12.5 Intersection LOS: B Intersection Capacity Utilization 80.0% ICU Level of Service D Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere



	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	<b>1</b>	<b>†</b>	/	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		ħβ		ች	<b>^</b>	7
Traffic Volume (vph)	0	873	0	0	1284	117	0	980	176	109	641	232
Future Volume (vph)	0	873	0	0	1284	117	0	980	176	109	641	232
Satd. Flow (prot)	0	3420	0	0	3420	1530	0	3229	0	1613	3196	1515
Flt Permitted										0.084		
Satd. Flow (perm)	0	3420	0	0	3420	1489	0	3229	0	143	3196	1471
Satd. Flow (RTOR)								19				35
Lane Group Flow (vph)	0	919	0	0	1352	123	0	1217	0	115	675	244
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases					_	8		_		6		6
Detector Phase		4			8	8		2		1	6	6
Switch Phase		100			40.0	100		40.0			40.0	40.0
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.8		12.0	31.9	31.9
Total Split (s)		60.0			60.0	60.0		48.0		12.0	60.0	60.0
Total Split (%)	•	50.0%			50.0%	50.0%		40.0%		10.0%	50.0%	50.0%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5 -2.1		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0 6.8	-2.8		-2.1 4.7		0.0 6.8	0.0 6.9	0.0 6.9
Total Lost Time (s)		6.8			0.0	4.0					6.9	6.9
Lead/Lag								Lag Yes		Lead Yes		
Lead-Lag Optimize? Recall Mode		Max			Max	Max		C-Min		None	C-Min	C-Min
Act Effct Green (s)		53.2			53.2	56.0		43.3		53.2	53.1	53.1
Actuated g/C Ratio		0.44			0.44	0.47		0.36		0.44	0.44	0.44
v/c Ratio		0.44			0.89	0.47		1.03		0.44	0.44	0.44
Control Delay		25.5			35.4	14.9		60.5		83.8	25.1	20.7
Queue Delay		0.4			1.7	0.0		0.0		0.0	0.0	0.0
Total Delay		26.0			37.0	14.9		60.5		83.8	25.1	20.7
LOS		C			D	В		E		F	C	C
Approach Delay		26.0			35.2			60.5		•	30.6	
Approach LOS		С			D			E			С	
Queue Length 50th (m)		89.3			159.6	17.6		~86.4		16.7	61.0	33.3
Queue Length 95th (m)		122.5			194.1	24.8		m#83.7		#51.1	78.4	54.7
Internal Link Dist (m)		72.0			117.1			52.2			192.9	
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1516			1516	694		1177		127	1414	670
Starvation Cap Reductn		213			65	0		0		0	0	0
Spillback Cap Reductn		0			0	0		0		0	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.71			0.93	0.18		1.03		0.91	0.48	0.36

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 3 (3%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 100

Maximum v/c Ratio: 1.03
Intersection Signal Delay: 39.0 Intersection LOS: D
Intersection Capacity Utilization 93.8% ICU Level of Service F
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

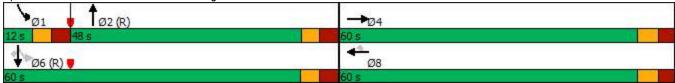
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Booth St & Wellington St



	•	<b>→</b>	•	•	•	•	1	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	<u></u>	7	Ť	<b>^</b>	7		۔}		ሻ	<u></u>	7
Traffic Volume (vph)	550	554	32	28	847	237	12	540	59	95	411	243
Future Volume (vph)	550	554	32	28	847	237	12	540	59	95	411	243
Satd. Flow (prot)	1676	1765	1485	1710	3386	1530	0	3291	0	1629	1782	1515
Flt Permitted	0.104			0.445				0.940		0.161		
Satd. Flow (perm)	181	1765	1451	797	3386	1360	0	3095	0	268	1782	1381
Satd. Flow (RTOR)			91					9				
Lane Group Flow (vph)	579	583	34	29	892	249	0	643	0	100	433	256
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		4.5	10.0	10.0
Minimum Split (s)	16.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.0	34.5	34.5
Total Split (s)	29.0	70.0	70.0	41.0	41.0	41.0	38.0	38.0		12.0	50.0	50.0
Total Split (%)	24.2%	58.3%	58.3%	34.2%	34.2%	34.2%	31.7%	31.7%		10.0%	41.7%	41.7%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	-2.5	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	4.0	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	C-Max	C-Max	C-Max	Min	Min	Min	Min	Min		None	Min	Min
Act Effct Green (s)	69.0	66.5	69.0	34.5	34.5	34.5		28.5		40.5	40.5	40.5
Actuated g/C Ratio	0.58	0.55	0.58	0.29	0.29	0.29		0.24		0.34	0.34	0.34
v/c Ratio	1.28	0.60	0.04	0.13	0.92	0.64		0.87		0.66	0.72	0.55
Control Delay	164.4	22.7	0.1	29.4	53.0	42.3		56.0		66.2	60.8	55.1
Queue Delay	0.0	1.5	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	164.4	24.3	0.1	29.4	53.0	42.3		56.0		66.2	60.8	55.1
LOS	F	С	Α	С	D	D		Е		Е	Е	Е
Approach Delay		91.4			50.2			56.0			59.6	
Approach LOS		F			D			Е			Е	
Queue Length 50th (m)	~168.4	139.3	0.0	5.2	112.6	54.2		78.8		25.4	113.0	66.2
Queue Length 95th (m)	#248.0	110.5	m0.0	11.1	#151.8	83.7		100.0		#46.8	146.8	93.9
Internal Link Dist (m)		138.6			62.5			37.2			83.1	
Turn Bay Length (m)	155.0		40.0	40.0								120.0
Base Capacity (vph)	452	977	872	229	973	391		819		152	645	500
Starvation Cap Reductn	0	219	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	1.28	0.77	0.04	0.13	0.92	0.64		0.79		0.66	0.67	0.51

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 9 (8%), Referenced to phase 4:EBTL and 7:EBL, Start of Green

Natural Cycle: 130

Maximum v/c Ratio: 1.28
Intersection Signal Delay: 66.1 Intersection LOS: E
Intersection Capacity Utilization 123.0% ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

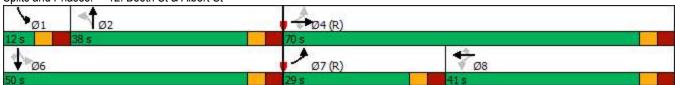
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



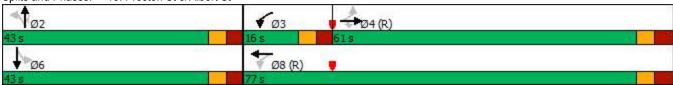
	۶	<b>→</b>	•	•	←	•	4	<b>†</b>	/	-	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	ሻ	<b>↑</b> ↑			4	7		4	
Traffic Volume (vph)	8	699	135	267	929	23	151	3	343	30	9	0
Future Volume (vph)	8	699	135	267	929	23	151	3	343	30	9	0
Satd. Flow (prot)	1710	1782	1500	1676	3371	0	0	1635	1485	0	1732	0
Flt Permitted	0.286			0.102				0.700			0.690	
Satd. Flow (perm)	515	1782	1412	180	3371	0	0	1175	1441	0	1234	0
Satd. Flow (RTOR)			86		3				264			
Lane Group Flow (vph)	8	736	142	281	1002	0	0	162	361	0	41	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	61.0	61.0	61.0	16.0	77.0		43.0	43.0	43.0	43.0	43.0	
Total Split (%)	50.8%	50.8%	50.8%	13.3%	64.2%		35.8%	35.8%	35.8%	35.8%	35.8%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	6.8	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	None	C-Max		None	None	None	None	None	
Act Effct Green (s)	54.5	54.5	57.3	85.4	84.8			22.1	22.1		21.5	
Actuated g/C Ratio	0.45	0.45	0.48	0.71	0.71			0.18	0.18		0.18	
v/c Ratio	0.03	0.91	0.20	0.66	0.42			0.75	0.75		0.19	
Control Delay	17.1	44.1	6.1	57.3	4.0			66.2	22.5		40.2	
Queue Delay	0.0	46.9	0.0	0.0	0.3			0.0	0.5		0.0	
Total Delay	17.1	91.1	6.1	57.3	4.2			66.2	23.0		40.2	
LOS	В	F	A	Е	Α			Е	С		D	
Approach Delay		76.8			15.9			36.4			40.2	
Approach LOS		Е			В			D			D	
Queue Length 50th (m)	0.8	167.2	3.9	50.0	9.8			38.5	21.8		8.7	
Queue Length 95th (m)	m2.3	#248.4	11.0	#102.7	38.7			57.9	53.8		17.6	
Internal Link Dist (m)		84.9			122.5			54.7			49.4	
Turn Bay Length (m)	30.0		16.0	90.0								
Base Capacity (vph)	233	809	718	428	2383			359	623		377	
Starvation Cap Reductn	0	0	0	0	655			0	0		0	
Spillback Cap Reductn	0	160	0	0	190			0	60		27	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.03	1.13	0.20	0.66	0.58			0.45	0.64		0.12	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 65 (54%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green Natural Cycle: 90

Maximum v/c Ratio: 0.91
Intersection Signal Delay: 39.9
Intersection Capacity Utilization 90.4%
ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



	۶	-	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> ∱		7	<b>∱</b> }			4			4	
Traffic Volume (vph)	2	934	2	17	1575	7	1	0	3	22	0	5
Future Volume (vph)	2	934	2	17	1575	7	1	0	3	22	0	5
Satd. Flow (prot)	1710	3386	0	1710	3416	0	0	1581	0	0	1688	0
Flt Permitted	0.130			0.287				0.906			0.762	
Satd. Flow (perm)	234	3386	0	517	3416	0	0	1450	0	0	1336	0
Satd. Flow (RTOR)					1			27			27	
Lane Group Flow (vph)	2	985	0	18	1665	0	0	4	0	0	28	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		37.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	87.0	87.0		87.0	87.0		33.0	33.0		33.0	33.0	
Total Split (%)	72.5%	72.5%		72.5%	72.5%		27.5%	27.5%		27.5%	27.5%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	106.6	106.6		106.6	106.6			10.0			10.0	
Actuated g/C Ratio	0.89	0.89		0.89	0.89			0.08			0.08	
v/c Ratio	0.01	0.33		0.04	0.55			0.03			0.21	
Control Delay	2.0	2.3		2.6	4.5			0.2			22.4	
Queue Delay	0.0	0.0		0.0	0.1			0.0			0.0	
Total Delay	2.0	2.3		2.6	4.6			0.2			22.4	
LOS	Α	Α		Α	Α			Α			С	
Approach Delay		2.3			4.6			0.3			22.4	
Approach LOS		Α			Α			Α			С	
Queue Length 50th (m)	0.1	26.5		0.4	31.8			0.0			0.2	
Queue Length 95th (m)	0.6	32.8		m2.2	143.9			0.0			9.8	
Internal Link Dist (m)		651.1			66.3			44.5			21.1	
Turn Bay Length (m)	40.0			50.0								
Base Capacity (vph)	208	3008		459	3035			343			318	
Starvation Cap Reductn	0	0		0	233			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.01	0.33		0.04	0.59			0.01			0.09	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 37 (31%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

2030 LeBreton Flats 4:00 pm 02-04-2020 JF

# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

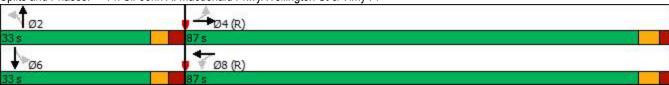
Maximum v/c Ratio: 0.55

Intersection Signal Delay: 3.9 Intersection LOS: A Intersection Capacity Utilization 65.6% ICU Level of Service C

Analysis Period (min) 15

Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>†</b>			f)	
Traffic Volume (vph)	0	987	4	2	1732	0	26	14	35	3	26	11
Future Volume (vph)	0	987	4	2	1732	0	26	14	35	3	26	11
Satd. Flow (prot)	0	3383	0	0	3420	0	0	1642	0	0	1669	0
Flt Permitted					0.954			0.870			0.969	
Satd. Flow (perm)	0	3383	0	0	3263	0	0	1450	0	0	1623	0
Satd. Flow (RTOR)		1						37			12	
Lane Group Flow (vph)	0	1043	0	0	1825	0	0	79	0	0	42	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		61.7			61.7			10.3			10.3	
Actuated g/C Ratio		0.78			0.78			0.13			0.13	
v/c Ratio		0.40			0.72			0.36			0.19	
Control Delay		4.4			8.4			24.0			25.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		4.4			8.4			24.0			25.9	
LOS		Α			Α			С			С	
Approach Delay		4.4			8.4			24.0			25.9	
Approach LOS		Α			A			С			С	
Queue Length 50th (m)		27.8			76.7			5.9			4.2	
Queue Length 95th (m)		40.1			112.7			18.3			13.1	
Internal Link Dist (m)		176.2			288.5			38.4			167.1	
Turn Bay Length (m)		2221			0=10							
Base Capacity (vph)		2634			2540			532			576	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.40			0.72			0.15			0.07	
Intersection Summary												

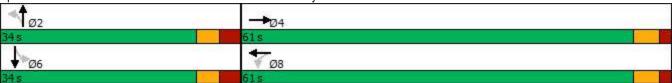
Cycle Length: 95

Actuated Cycle Length: 79.3

Natural Cycle: 90 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.72

Intersection Signal Delay: 7.7 Intersection LOS: A Intersection Capacity Utilization 75.7% ICU Level of Service D Analysis Period (min) 15

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>^</b>		ሻ	<b>†</b>	7	ሻ	<b>†</b>	7	ሻ	f)	
Traffic Volume (vph)	10	586	88	84	772	121	88	185	84	59	68	11
Future Volume (vph)	10	586	88	84	772	121	88	185	84	59	68	11
Satd. Flow (prot)	1555	1715	0	1710	1782	1530	1710	1800	1485	1710	1745	0
Flt Permitted	0.261			0.317			0.702			0.509		
Satd. Flow (perm)	420	1715	0	563	1782	1340	1212	1800	1388	887	1745	0
Satd. Flow (RTOR)		14				88			76		8	
Lane Group Flow (vph)	11	710	0	88	813	127	93	195	88	62	84	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	69.1	69.1		69.1	69.1	69.1	18.0	18.0	18.0	18.0	18.0	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.04	0.60		0.23	0.66	0.13	0.43	0.60	0.28	0.39	0.26	
Control Delay	11.0	21.1		9.0	13.5	3.0	40.6	44.5	11.7	41.1	31.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.0	21.1		9.0	13.5	3.0	40.6	44.5	11.7	41.1	31.7	
LOS	В	С		Α	В	Α	D	D	В	D	C	
Approach Delay		21.0			11.9			35.9			35.7	
Approach LOS	4.0	C			В	0.4	47.5	D	0.4	44.0	D	
Queue Length 50th (m)	1.0	123.9		5.3	76.5	2.1	17.5	37.9	2.1	11.6	13.8	
Queue Length 95th (m)	m2.2	m168.6		16.3	156.4	9.9	30.1	55.2	14.2	22.5	25.0	
Internal Link Dist (m)	45.0	635.7		50.0	497.2	40.0	50.0	83.8	00.0	45.0	171.5	
Turn Bay Length (m)	45.0	4400		50.0	1001	40.0	50.0	400	20.0	45.0	450	
Base Capacity (vph)	290	1189		388	1231	952	310	460	411	227	452	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.04	0.60		0.23	0.66	0.13	0.30	0.42	0.21	0.27	0.19	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 65 (65%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

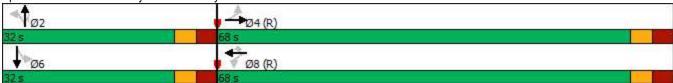
2030 LeBreton Flats 4:00 pm 02-04-2020 JF

## 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

Maximum v/c Ratio: 0.66
Intersection Signal Delay: 20.3
Intersection Capacity Utilization 96.8%
ICU Level of Service F
Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



	•	<b>→</b>	•	•	←	•	•	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>1</b>	7	ሻ	<b>1</b>	7	ሻ	₽		*	1>	
Traffic Volume (vph)	227	504	86	56	646	63	40	546	48	58	233	210
Future Volume (vph)	227	504	86	56	646	63	40	546	48	58	233	210
Satd. Flow (prot)	1693	1765	1515	1710	1800	1530	1710	1755	0	1710	1565	0
Flt Permitted	0.117			0.467			0.290			0.119		
Satd. Flow (perm)	209	1765	1240	779	1800	1222	507	1755	0	208	1565	0
Satd. Flow (RTOR)			74			119		5			53	
Lane Group Flow (vph)	239	531	91	59	680	66	42	626	0	61	466	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	10.6	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	15.0	49.0	49.0	34.0	34.0	34.0	45.0	45.0		45.0	45.0	
Total Split (%)	15.0%	49.0%	49.0%	34.0%	34.0%	34.0%	45.0%	45.0%		45.0%	45.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.3	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	-2.1	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	6.1	4.0	6.1	4.0	4.0	6.3	6.3		6.3	6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		Min	Min	
Act Effct Green (s)	50.6	50.1	52.2	28.4	30.5	30.5	37.5	37.5		37.5	37.5	
Actuated g/C Ratio	0.51	0.50	0.52	0.28	0.30	0.30	0.38	0.38		0.38	0.38	
v/c Ratio	0.70	0.60	0.13	0.27	1.24	0.15	0.22	0.95		0.78	0.75	
Control Delay	32.2	21.8	4.7	31.8	149.4	3.0	24.4	55.0		87.8	32.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	32.2	21.8	4.7	31.8	149.4	3.0	24.4	55.0		87.8	32.5	
LOS	С	С	Α	С	F	Α	С	Е		F	С	
Approach Delay		22.9			128.8			53.1			38.9	
Approach LOS	00.0	С	4 =	0.0	F	0.4		D		40.0	D	
Queue Length 50th (m)	30.2	76.2	1.7	6.6	~177.3	0.1	5.6	118.6		10.6	71.3	
Queue Length 95th (m)	#64.3	111.5	9.6	m13.0	#243.6	m1.8	14.4			#35.8	111.4	
Internal Link Dist (m)	0	92.9	45.0	05.0	635.7	25.0	0	70.7		=0.0	630.1	
Turn Bay Length (m)	55.0	004	45.0	65.0	550	35.0	55.0	000		50.0	000	
Base Capacity (vph)	343	884	682	221	550	455	196	682		80	638	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.70	0.60	0.13	0.27	1.24	0.15	0.21	0.92		0.76	0.73	

Cycle Length: 100 Actuated Cycle Length: 100

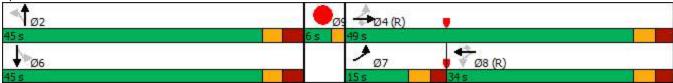
Offset: 8 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 110

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	9	
Detector Phase		
Switch Phase		
	2.0	
Minimum Initial (s)	2.0	
Minimum Split (s)	6.0	
Total Split (s)	6.0	
Total Split (%)	6%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		

Maximum v/c Ratio: 1.24 Intersection Signal Delay: 62.7 Intersection LOS: E Intersection Capacity Utilization 109.8% ICU Level of Service H Analysis Period (min) 15 Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	-	•	•	<b>←</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		7				
Traffic Volume (vph)	0	0	0	0	0	464
Future Volume (vph)	0	0	0	0	0	464
Satd. Flow (prot)	0	1800	0	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1800	0	0	0	0
Lane Group Flow (vph)	0	0	0	0	488	0
Sign Control	Free			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion Err%			IC	U Level c	f Service I
Analysis Period (min) 15						

	•	•	•	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	W		ች	<b>^</b>	<b>†</b> 1>	02.1	~~	
Traffic Volume (vph)	1	8	8	1114	1067	10		
Future Volume (vph)	1	8	8	1114	1067	10		
Satd. Flow (prot)	1554	0	1710	3420	3417	0		
Flt Permitted	0.994	•	0.247	0.20	•			
Satd. Flow (perm)	1551	0	445	3420	3417	0		
Satd. Flow (RTOR)	8	•		0.20	1			
Lane Group Flow (vph)	9	0	8	1173	1134	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	10.0		10.0	10.0	10.0		22.0	
Minimum Split (s)	17.0		30.9	30.9	30.9		29.0	
Total Split (s)	17.0		49.0	49.0	49.0		29.0	
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.9	5.9	5.9			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	10.0		90.6	90.6	90.6			
Actuated g/C Ratio	0.11		0.95	0.95	0.95			
v/c Ratio	0.05		0.02	0.36	0.35			
Control Delay	23.9		1.6	1.4	1.4			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	23.9		1.6	1.4	1.4			
LOS	С		Α	Α	Α			
Approach Delay	23.9			1.4	1.4			
Approach LOS	С			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	4.8		1.3	42.9	40.8			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	183		424	3261	3258			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.05		0.02	0.36	0.35			
Internation Comment								

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 10 (11%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 80

Intersection Signal Delay: 1.5 Intersection Capacity Utilization 50.9%	Intersection LOS: A ICU Level of Service A		
Analysis Period (min) 15	100 Level of Gervice A		
Splits and Phases: 19: Booth St & War Museum			
4			
Ø2 (R)	Ø4	Ø9	
9 g2 (R)	Ø4	Ø9 29 s	
Ø2 (R)  49.s  Ø6 (R)	04 17.s	Ø9 29.s	

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	7	<b>†</b>	7	ሻ	f)			4	
Traffic Volume (vph)	9	614	76	84	849	28	57	4	80	42	0	8
Future Volume (vph)	9	614	76	84	849	28	57	4	80	42	0	8
Satd. Flow (prot)	855	1782	1530	1693	1765	1020	1644	1500	0	0	1690	0
Flt Permitted	0.217			0.419			0.788				0.698	
Satd. Flow (perm)	195	1782	1530	747	1765	982	1364	1500	0	0	1230	0
Satd. Flow (RTOR)			80			87		84			87	
Lane Group Flow (vph)	9	646	80	88	894	29	60	88	0	0	52	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	16.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	95.3	95.3	97.6	92.0	92.0	94.3	12.1	12.1			12.1	
Actuated g/C Ratio	0.79	0.79	0.81	0.77	0.77	0.79	0.10	0.10			0.10	
v/c Ratio	0.04	0.46	0.06	0.15	0.66	0.04	0.44	0.39			0.26	
Control Delay	3.6	5.4	0.7	3.8	10.8	0.1	60.2	16.3			5.9	
Queue Delay	0.0	0.1	0.0	0.0	0.2	0.0	0.0	0.0			0.0	
Total Delay	3.6	5.5	0.7	3.8	11.0	0.1	60.2	16.4			5.9	
LOS	Α	A	Α	Α	В	Α	E	В			A	
Approach Delay		5.0			10.0			34.1			5.9	
Approach LOS	0.4	Α	0.0	2.0	В	0.0	44.4	С			A	
Queue Length 50th (m)	0.4	39.9	0.0	3.2	33.7	0.0	14.4	0.9			0.0	
Queue Length 95th (m)	1.7	71.7	3.1	9.5	278.6	m0.0	27.8	16.4			4.0	
Internal Link Dist (m)	20.0	497.2	F0 0	25.0	108.4	FO 0	20.0	178.8			36.6	
Turn Bay Length (m)	30.0	4445	50.0	35.0	4050	50.0	30.0	40.4			200	
Base Capacity (vph)	213	1415	1259	573	1353	790	337	434			369	
Starvation Cap Reductn	0	0	0	0	61	0	0	0			0	
Spillback Cap Reductn	0	89	0	0	0	0	0	4			4	
Storage Cap Reductn	0	0 40	0 06	0 15	0 60	0 04	0 10	0 20			0	
Reduced v/c Ratio	0.04	0.49	0.06	0.15	0.69	0.04	0.18	0.20			0.14	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 100

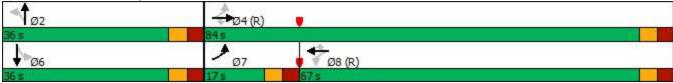
Maximum v/c Ratio: 0.66

Intersection Signal Delay: 9.8 Intersection LOS: A Intersection Capacity Utilization 80.9% ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: City Centre Ave & Albert St



	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ኝ	<b>†</b>	<b>↑</b> ↑		W	
Traffic Volume (vph)	0	1122	1102	0	0	0
Future Volume (vph)	0	1122	1102	0	0	0
Satd. Flow (prot)	1800	1765	3386	0	1800	0
Flt Permitted						
Satd. Flow (perm)	1800	1765	3386	0	1800	0
Satd. Flow (RTOR)						
Lane Group Flow (vph)	0	1181	1160	0	0	0
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	7	4	8		6	
Permitted Phases	4					
Detector Phase	7	4	8		6	
Switch Phase						
Minimum Initial (s)	5.0	10.0	10.0		10.0	
Minimum Split (s)	10.0	22.0	22.0		20.0	
Total Split (s)	15.0	90.0	75.0		30.0	
Total Split (%)	12.5%	75.0%	62.5%		25.0%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.7	1.7	1.7		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	5.0	5.0	5.0		5.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	C-Max	C-Min		None	
Act Effct Green (s)		115.6	115.6			
Actuated g/C Ratio		0.96	0.96			
v/c Ratio		0.69	0.36			
Control Delay		10.3	1.2			
Queue Delay		0.3	0.0			
Total Delay		10.6	1.2			
LOS		В	Α			
Approach Delay		10.6	1.2			
Approach LOS		В	Α			
Queue Length 50th (m)		168.0	0.1			
Queue Length 95th (m)		m275.7	m30.1			
Internal Link Dist (m)		122.5	138.6		61.4	
Turn Bay Length (m)						
Base Capacity (vph)		1700	3262			
Starvation Cap Reductn		131	261			
Spillback Cap Reductn		78	0			
Storage Cap Reductn		0	0			
Reduced v/c Ratio		0.75	0.39			
Intersection Summary						

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 90

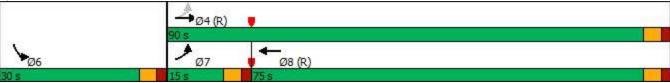
Maximum v/c Ratio: 0.69

Intersection Signal Delay: 6.0 Intersection LOS: A ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 21: Albert St & Access 1



	-	•	•	•	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>↑</b> ↑		ች	<b>^</b>	W	
Traffic Volume (vph)	1047	32	60	1194	39	56
Future Volume (vph)	1047	32	60	1194	39	56
Satd. Flow (prot)	3363	0	1710	3386	1586	0
Flt Permitted			0.232		0.980	
Satd. Flow (perm)	3363	0	415	3386	1585	0
Satd. Flow (RTOR)	5				56	
Lane Group Flow (vph)	1136	0	63	1257	100	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		22.0	22.0	33.8	
Total Split (s)	66.0		86.0	86.0	34.0	
Total Split (%)	55.0%		71.7%	71.7%	28.3%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	1.9		1.9	1.9	2.5	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.6		5.6	5.6	5.8	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	94.8		94.8	94.8	13.8	
Actuated g/C Ratio	0.79		0.79	0.79	0.12	
v/c Ratio	0.43		0.19	0.47	0.43	
Control Delay	1.9		6.2	5.7	28.1	
Queue Delay	0.1		0.0	0.3	0.0	
Total Delay	2.1		6.2	6.0	28.1	
LOS	Α		Α	A	C	
Approach Delay	2.1			6.0	28.1	
Approach LOS	Α			A	C	
Queue Length 50th (m)	4.4		2.5	36.1	10.5	
Queue Length 95th (m)	m22.4		12.8	95.4	23.8	
Internal Link Dist (m)	117.1			245.5	41.8	
Turn Bay Length (m)			70.0	0.0	. 1.0	
Base Capacity (vph)	2657		327	2674	415	
Starvation Cap Reductn	527		0	0	0	
Spillback Cap Reductn	0		0	662	10	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.53		0.19	0.62	0.25	
	0.00		0.10	0.02	U.EU	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 23 (19%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 65

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 5.1	Intersection LOS: A	
Intersection Capacity Utilization 64.8%	ICU Level of Service C	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	<b>/</b>	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> }			<b>€</b> 1}			4			4	
Traffic Volume (vph)	43	660	16	11	1021	15	9	0	14	43	0	11
Future Volume (vph)	43	660	16	11	1021	15	9	0	14	43	0	11
Satd. Flow (prot)	1710	3406	0	0	3410	0	0	1619	0	0	1683	0
Flt Permitted	0.246				0.945			0.894			0.755	
Satd. Flow (perm)	443	3406	0	0	3225	0	0	1474	0	0	1321	0
Satd. Flow (RTOR)		4			2			15			15	
Lane Group Flow (vph)	45	712	0	0	1103	0	0	24	0	0	57	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		30.5	30.5		30.5	30.5	
Total Split (s)	81.0	81.0		81.0	81.0		39.0	39.0		39.0	39.0	
Total Split (%)	67.5%	67.5%		67.5%	67.5%		32.5%	32.5%		32.5%	32.5%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.6	4.6			4.6			4.3			4.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Min	C-Min		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	101.7	101.7			101.7			13.2			13.2	
Actuated g/C Ratio	0.85	0.85			0.85			0.11			0.11	
v/c Ratio	0.12	0.25			0.40			0.14			0.36	
Control Delay	2.0	1.3			3.8			26.9			42.8	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	2.0	1.3			3.8			26.9			42.8	
LOS	Α	Α			Α			С			D	
Approach Delay		1.3			3.8			26.9			42.8	
Approach LOS		Α			Α			С			D	
Queue Length 50th (m)	0.7	6.0			25.3			2.1			10.0	
Queue Length 95th (m)	m2.5	16.2			67.5			9.4			21.0	
Internal Link Dist (m)		82.0			212.6			72.6			45.7	
Turn Bay Length (m)	75.0											
Base Capacity (vph)	375	2887			2733			436			392	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.12	0.25			0.40			0.06			0.15	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 60

Maximum v/c Ratio: 0.40
Intersection Signal Delay: 4.3
Intersection Capacity Utilization 54.2%
ICU Level of Service A

Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	<b>≯</b>	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	~	<b>\</b>	<b>↓</b>	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>∱</b> }				7			7
Traffic Volume (vph)	0	676	0	0	1052	14	0	0	0	0	0	11
Future Volume (vph)	0	676	0	0	1052	14	0	0	0	0	0	11
Satd. Flow (prot)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Flt Permitted												
Satd. Flow (perm)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Lane Group Flow (vph)	0	712	0	0	1122	0	0	0	0	0	0	12
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	on 41.2%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

	ၨ	<b>→</b>	•	•	•	•	•	<b>†</b>	<b>/</b>	<b>\</b>	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		<b>ተ</b> ኈ			<b>∱</b> }	
Traffic Volume (vph)	0	0	0	0	0	10	0	1206	23	0	739	0
Future Volume (vph)	0	0	0	0	0	10	0	1206	23	0	739	0
Satd. Flow (prot)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Flt Permitted												
Satd. Flow (perm)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Lane Group Flow (vph)	0	0	0	0	0	11	0	1293	0	0	778	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	on 46.0%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

	-	•	•	<b>←</b>	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>↑</b> ↑			<b>^</b>		7
Traffic Volume (vph)	888	0	0	1516	0	7
Future Volume (vph)	888	0	0	1516	0	7
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Lane Group Flow (vph)	935	0	0	1596	0	7
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 47.6%			IC	U Level o	of Service A
Analysis Period (min) 15						

	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		ሻ	<b>^</b>	¥	
Traffic Volume (vph)	881	2	17	1499	3	7
Future Volume (vph)	881	2	17	1499	3	7
Satd. Flow (prot)	3386	0	1710	3420	1605	0
Flt Permitted			0.950		0.985	
Satd. Flow (perm)	3386	0	1710	3420	1605	0
Satd. Flow (RTOR)					7	
Lane Group Flow (vph)	929	0	18	1578	10	0
Turn Type	NA		Prot	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases						
Detector Phase	4		3	8	2	
Switch Phase						
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	37.0		11.0	37.0	32.3	
Total Split (s)	72.0		15.0	87.0	33.0	
Total Split (%)	60.0%		12.5%	72.5%	27.5%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.3		2.3	2.3	3.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	6.0		6.0	6.0	6.3	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	108.7		6.9	115.5	10.0	
Actuated g/C Ratio	0.91		0.06	0.96	0.08	
v/c Ratio	0.30		0.19	0.48	0.07	
Control Delay	2.7		39.3	3.2	33.9	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	2.7		39.3	3.2	33.9	
LOS	A		D	A	C	
Approach Delay	2.7		_	3.6	33.9	
Approach LOS	Α			A	C	
Queue Length 50th (m)	0.0		3.8	0.0	0.7	
Queue Length 95th (m)	51.4		m5.7	121.2	6.4	
Internal Link Dist (m)	71.8			49.0	28.0	
Turn Bay Length (m)			60.0			
Base Capacity (vph)	3067		128	3293	362	
Starvation Cap Reductn	129		0	255	0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.32		0.14	0.52	0.03	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBT and 8:WBT, Start of Green

Natural Cycle: 85

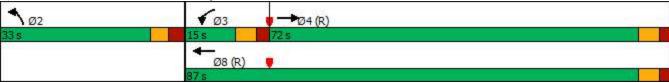
Maximum v/c Ratio: 0.48

Intersection Signal Delay: 3.4 Intersection LOS: A Intersection Capacity Utilization 62.3% ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 27: Broad St & Wellington St

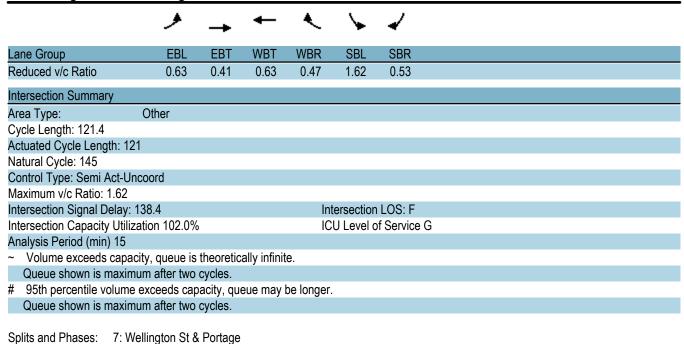


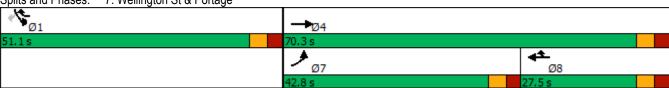
	-	•	•	<b>←</b>	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> ∱			<b>^</b>		7
Traffic Volume (vph)	876	2	0	1499	0	7
Future Volume (vph)	876	2	0	1499	0	7
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Lane Group Flow (vph)	924	0	0	1578	0	7
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalized	t					
Intersection Capacity Utiliz	ation 47.1%			IC	U Level o	of Service A
Analysis Period (min) 15						

Synchro Modelling Outputs – Phase Two (2040) Conditions



	•	<b>→</b>	←	•	-	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ			WDK 777	<u> </u>	JDK 7
Traffic Volume (vph)	<b>11</b> 603	<b>↑↑↑</b> 1011	<b>↑↑</b> 357	704	<b>1962</b>	314
Future Volume (vph)	603	1011	357	704	1962	314
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	150.0	1000	1000	0.0	0.0	30.0
	150.0			2	2	30.0
Storage Lanes				2	7.5	ı
Taper Length (m)	7.5	4014	3420	ეგიე	3285	1530
Satd. Flow (prot)	3317	4914	3420	2693		1030
Flt Permitted	0.950	1011	2/100	2602	0.950	1501
Satd. Flow (perm)	3303	4914	3420	2693	3272	1501
Right Turn on Red				No		Yes
Satd. Flow (RTOR)		00	50			72
Link Speed (k/h)		60	50		50	
Link Distance (m)		270.9	323.2		63.9	
Travel Time (s)		16.3	23.3		4.6	
Lane Group Flow (vph)	635	1064	376	741	2065	331
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	18	1	
Permitted Phases						1
Detector Phase	7	4	8	18	1	1
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	16.5	26.5		45.1	45.1
Total Split (s)	42.8	70.3	27.5		51.1	51.1
Total Split (%)	35.3%	57.9%	22.7%		42.1%	42.1%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		-2.1	-2.1
Total Lost Time (s)	5.8	6.5	6.5		4.0	4.0
Lead/Lag	Lead	0.0	Lag		+.∪	7.0
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Max	Min	Min		Max	Max
Act Effct Green (s)		Min	20.6	72.1		47.1
	37.0	63.4			47.1	
Actuated g/C Ratio	0.31	0.52	0.17	0.60	0.39	0.39
v/c Ratio	0.63	0.41	0.65	0.46	1.62	0.53
Control Delay	39.4	18.1	52.6	14.8	308.9	25.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	39.4	18.1	52.6	14.8	308.9	25.5
LOS	D	В	D	В	F	С
Approach Delay		26.1	27.5		269.8	
Approach LOS		С	С		F	
Queue Length 50th (m)	70.5	57.1	46.6	55.4	~384.1	49.2
Queue Length 95th (m)	90.8	68.1	63.8	71.7	#427.2	79.1
Internal Link Dist (m)		246.9	299.2		39.9	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1013	2591	593	1586	1278	628
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
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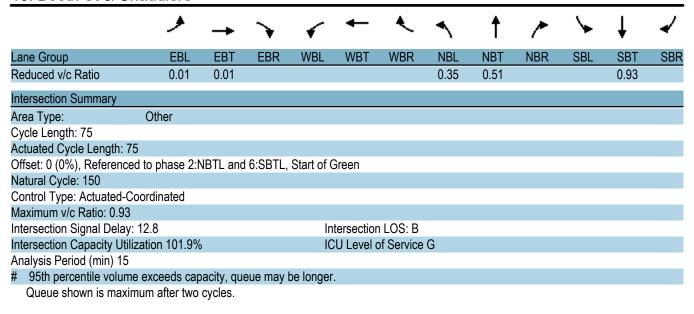




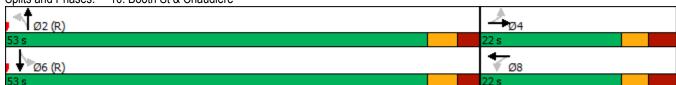
	•	-	←	•	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<b>†</b>	<b>†</b>	7		7
Traffic Volume (vph)	0	1016	497	0	0	5
Future Volume (vph)	0	1016	497	0	0	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	0	1800	1800	1800	0	1557
Flt Permitted						
Satd. Flow (perm)	0	1800	1800	1800	0	1557
Link Speed (k/h)		50	50		50	
Link Distance (m)		136.4	104.9		117.4	
Travel Time (s)		9.8	7.6		8.5	
Lane Group Flow (vph)	0	1069	523	0	0	5
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Util	ization 59.8%			IC	CU Level o	of Service

Analysis Period (min) 15

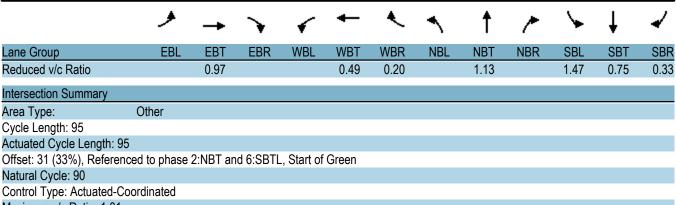
	۶	-	•	•	<b>←</b>	•	•	†	<i>&gt;</i>	<b>&gt;</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	f)		, j	f)		ř	f)		Ţ	f)	
Traffic Volume (vph)	5	0	5	0	0	0	37	828	0	0	1460	37
Future Volume (vph)	5	0	5	0	0	0	37	828	0	0	1460	37
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	30.0		0.0	30.0		0.0	30.0		0.0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1793	0
Flt Permitted							0.066					
Satd. Flow (perm)	1800	1530	0	1800	1800	0	119	1800	0	1800	1793	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		42									3	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		84.2			67.3			100.9			55.3	
Travel Time (s)		6.1			4.8			7.3			4.0	
Lane Group Flow (vph)	5	5	0	0	0	0	39	872	0	0	1576	0
Turn Type	Perm	NA		Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		35.9	35.9		35.9	35.9	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)	10.0	10.0					70.6	70.6			70.6	
Actuated g/C Ratio	0.13	0.13					0.94	0.94			0.94	
v/c Ratio	0.02	0.02					0.35	0.51			0.93	
Control Delay	28.6	0.2					15.8	3.4			17.8	
Queue Delay	0.0	0.0					0.0	0.0			0.0	
Total Delay	28.6	0.2					15.8	3.4			17.8	
LOS	С	Α					В	Α			В	
Approach Delay		14.4						3.9			17.8	
Approach LOS		В						Α			В	
Queue Length 50th (m)	0.7	0.0					0.0	0.0			0.0	
Queue Length 95th (m)	3.6	0.0					#18.0	98.8			#381.3	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)	30.0						30.0					
Base Capacity (vph)	379	355					112	1694			1687	
Starvation Cap Reductn	0	0					0	0			0	
Spillback Cap Reductn	0	0					0	0			0	
Storage Cap Reductn	0	0					0	0			0	



Splits and Phases: 10: Booth St & Chaudiere



	۶	-	•	•	<b>←</b>	•	•	†	~	<b>/</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>∱</b> }		ሻ	<b>^</b>	7
Traffic Volume (vph)	0	1282	0	0	677	132	0	834	131	146	988	233
Future Volume (vph)	0	1282	0	0	677	132	0	834	131	146	988	233
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		70.0	35.0		0.0	145.0		55.0
Storage Lanes	0		0	0		1	0		0	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	0	3420	0	0	3386	1530	0	3104	0	1710	3226	1530
Flt Permitted										0.114		
Satd. Flow (perm)	0	3420	0	0	3386	1478	0	3104	0	205	3226	1465
Right Turn on Red			Yes			No			Yes			Yes
Satd. Flow (RTOR)								19				120
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		91.5			140.7			79.5			216.9	
Travel Time (s)		5.5			8.4			5.7			15.6	
Lane Group Flow (vph)	0	1349	0	0	713	139	0	1016	0	154	1040	245
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases					-	8				6	-	6
Detector Phase		4			8	8		2		1	6	6
Switch Phase		•						_		•		
Minimum Initial (s)		10.0			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)		35.8			35.8	35.8		31.9		12.0	37.8	37.8
Total Split (s)		48.0			48.0	48.0		35.0		12.0	47.0	47.0
Total Split (%)		50.5%			50.5%	50.5%		36.8%		12.6%	49.5%	49.5%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.5	3.5
Lost Time Adjust (s)		0.0			0.0	-2.8		-2.1		0.0	0.0	-2.8
Total Lost Time (s)		6.8			6.8	4.0		4.7		6.8	6.8	4.0
Lead/Lag		0.0			0.0			Lag		Lead	0.0	
Lead-Lag Optimize?								Yes		Yes		
Recall Mode		Min			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)		40.5			40.5	43.3		30.3		40.9	40.9	43.7
Actuated g/C Ratio		0.43			0.43	0.46		0.32		0.43	0.43	0.46
v/c Ratio		0.93			0.49	0.21		1.01		0.85	0.75	0.33
Control Delay		46.5			27.5	22.6		64.6		59.0	25.5	9.0
Queue Delay		4.6			0.0	0.0		31.7		64.1	0.0	0.0
Total Delay		51.1			27.5	22.6		96.3		123.1	25.5	9.0
LOS		D			C	C		F		F	C	A
Approach Delay		51.1			26.7			96.3		•	33.2	, ,
Approach LOS		D			C			F			C	
Queue Length 50th (m)		124.7			64.5	20.9		~103.7		18.4	87.6	14.0
Queue Length 95th (m)		#179.3			85.6	37.5		#149.3		#54.7	67.0	23.7
Internal Link Dist (m)		67.5			116.7	07.0		55.5		7/01.7	192.9	20.1
Turn Bay Length (m)		01.0			110.7	70.0		00.0		145.0	102.0	55.0
Base Capacity (vph)		1483			1468	684		1002		182	1389	738
Starvation Cap Reductn		93			0	004		0		0	0	0
Spillback Cap Reductn		58			0	0		100		77	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Olorage Oap Neducin		<u> </u>			U	U		U				



Maximum v/c Ratio: 1.01 Intersection Signal Delay: 51.0

Intersection LOS: D Intersection Capacity Utilization 90.4% ICU Level of Service E

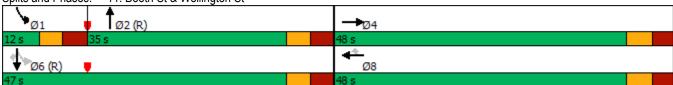
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 11: Booth St & Wellington St



	۶	<b>→</b>	•	•	<b>—</b>	•	•	†	/	<b>/</b>	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ሻ	<b>^</b>	7		4T <del>)</del>		ሻ	<b>1</b>	7
Traffic Volume (vph)	393	834	19	15	302	155	6	540	45	174	477	438
Future Volume (vph)	393	834	19	15	302	155	6	540	45	174	477	438
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	160.0		40.0	40.0		0.0	0.0		0.0	0.0		120.0
Storage Lanes	1		1	1		1	0		0	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1660	1748	1443	1710	3196	1378	0	3319	0	1425	1782	1471
Flt Permitted	0.420			0.131				0.949		0.174		
Satd. Flow (perm)	702	1748	1377	236	3196	1251	0	3148	0	253	1782	1313
Right Turn on Red			Yes			No			Yes			No
Satd. Flow (RTOR)			91					7				
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		165.9			86.1			61.2			107.1	
Travel Time (s)		11.9			6.2			4.4			7.7	
Lane Group Flow (vph)	414	878	20	16	318	163	0	621	0	183	502	461
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		5.0	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	18.0	55.0	55.0	37.0	37.0	37.0	40.0	40.0		25.0	65.0	65.0
Total Split (%)	15.0%	45.8%	45.8%	30.8%	30.8%	30.8%	33.3%	33.3%		20.8%	54.2%	54.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	None
Act Effct Green (s)	56.7	56.7	59.2	30.5	30.5	30.5		28.4		50.3	50.3	50.3
Actuated g/C Ratio	0.47	0.47	0.49	0.25	0.25	0.25		0.24		0.42	0.42	0.42
v/c Ratio	0.85	1.06	0.03	0.27	0.39	0.51		0.83		0.71	0.67	0.84
Control Delay	44.8	82.2	0.1	51.1	39.1	45.2		52.8		38.6	32.3	44.8
Queue Delay	0.0	15.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	44.8	97.2	0.1	51.1	39.1	45.2		52.8		38.6	32.3	44.8
LOS	D	F	Α	D	D	D		D		D	С	D
Approach Delay		79.2			41.5			52.8			38.3	
Approach LOS		Е			D			D			D	
Queue Length 50th (m)	71.3	~242.9	0.0	2.9	30.4	29.7		76.3		29.3	97.8	99.3
Queue Length 95th (m)	#180.1	#349.6	0.0	9.2	45.2	50.3		93.5		44.5	121.4	131.9
Internal Link Dist (m)		141.9			62.1			37.2			83.1	
Turn Bay Length (m)	160.0		40.0	40.0								120.0
Base Capacity (vph)	488	825	725	59	812	317		883		286	868	640
Starvation Cap Reductn	0	126	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0

	۶	<b>→</b>	•	•	•	•	4	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Reduced v/c Ratio	0.85	1.26	0.03	0.27	0.39	0.51		0.70		0.64	0.58	0.72

Intersection Summary

Area Type: Other

Cycle Length: 120
Actuated Cycle Length: 120

Offset: 2 (2%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.06 Intersection Signal Delay: 56.3 Intersection Capacity Utilization 125.9%

Intersection LOS: E
ICU Level of Service H

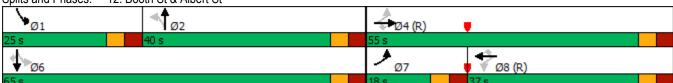
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

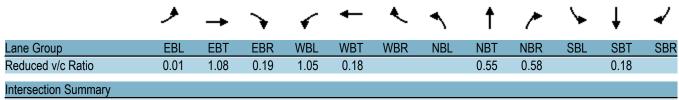
# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 12: Booth St & Albert St



	۶	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	/	<b>/</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<u></u>	7	ř	<b>↑</b> ↑			4	7		4	
Traffic Volume (vph)	5	904	136	393	397	16	112	8	291	29	12	2
Future Volume (vph)	5	904	136	393	397	16	112	8	291	29	12	2
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		16.0	90.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1710	1748	1471	1629	3271	0	0	1599	1471	0	1728	0
Flt Permitted	0.499			0.062				0.706			0.724	
Satd. Flow (perm)	898	1748	1384	106	3271	0	0	1168	1417	0	1294	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			86		8				306		2	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		104.9			143.1			78.7			89.4	
Travel Time (s)		7.6			10.3			5.7			6.4	
Lane Group Flow (vph)	5	952	143	414	435	0	0	126	306	0	46	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	65.0	65.0	65.0	25.0	90.0		30.0	30.0	30.0	30.0	30.0	
Total Split (%)	54.2%	54.2%	54.2%	20.8%	75.0%		25.0%	25.0%	25.0%	25.0%	25.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	-2.8	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	4.0	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	Min	C-Min		None	None	None	None	None	
Act Effct Green (s)	58.2	61.0	61.0	89.8	89.2			17.7	17.7		17.6	
Actuated g/C Ratio	0.48	0.51	0.51	0.75	0.74			0.15	0.15		0.15	
v/c Ratio	0.01	1.07	0.19	1.05	0.18			0.73	0.65		0.24	
Control Delay	19.4	75.5	7.8	95.8	5.1			72.2	11.8		44.5	
Queue Delay	0.0	3.4	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	19.4	78.9	7.8	95.8	5.1			72.2	11.8		44.5	
LOS	В	Е	Α	F	Α			Е	В		D	
Approach Delay		69.4			49.4			29.4			44.5	
Approach LOS		Е			D			С			D	
Queue Length 50th (m)	0.4	~262.7	4.8	~98.6	14.5			30.1	0.0		9.7	
Queue Length 95th (m)	m1.2	#340.3	m18.7	#183.0	24.0			49.6	26.1		20.4	
Internal Link Dist (m)		80.9			119.1			54.7			65.4	
Turn Bay Length (m)	30.0		16.0	90.0								
Base Capacity (vph)	435	888	745	394	2434			230	525		257	
Starvation Cap Reductn	0	7	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	



Area Type: Other

Cycle Length: 120
Actuated Cycle Length: 120

Offset: 55 (46%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 150

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07 Intersection Signal Delay: 54.8 Intersection Capacity Utilization 98.7%

Intersection LOS: D
ICU Level of Service F

Analysis Period (min) 15

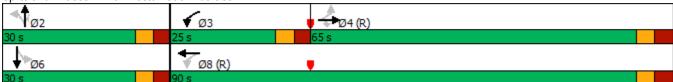
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

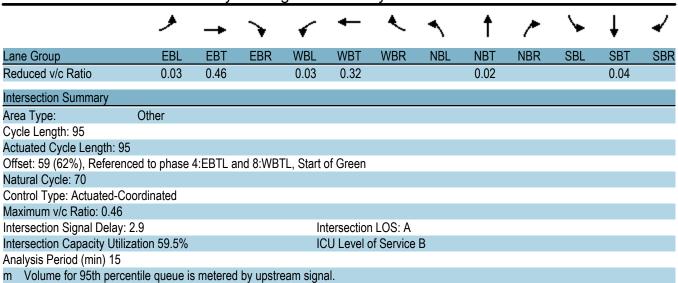
Splits and Phases: 13: Preston St & Albert St



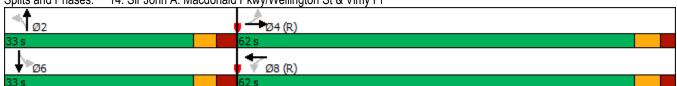
# Lanes, Volumes, Timings 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>↑</b> ↑		Ť	<b>↑</b> ↑			4			4	
Traffic Volume (vph)	15	1358	2	9	888	41	4	0	6	2	0	15
Future Volume (vph)	15	1358	2	9	888	41	4	0	6	2	0	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0		0.0	50.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1710	3420	0	1710	3360	0	0	1608	0	0	1462	0
Flt Permitted	0.291			0.172				0.861			0.958	
Satd. Flow (perm)	523	3420	0	310	3360	0	0	1411	0	0	1409	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					8			34			34	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		675.1			88.4			65.8			45.1	
Travel Time (s)		40.5			5.3			4.7			3.2	
Lane Group Flow (vph)	16	1431	0	9	978	0	0	10	0	0	18	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		37.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	62.0	62.0		62.0	62.0		33.0	33.0		33.0	33.0	
Total Split (%)	65.3%	65.3%		65.3%	65.3%		34.7%	34.7%		34.7%	34.7%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	86.1	86.1		86.1	86.1			10.0			10.0	
Actuated g/C Ratio	0.91	0.91		0.91	0.91			0.11			0.11	
v/c Ratio	0.03	0.46		0.03	0.32			0.06			0.10	
Control Delay	2.5	2.6		4.0	3.1			1.2			7.4	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	2.5	2.6		4.0	3.1			1.2			7.4	
LOS	Α	Α		Α	Α			Α			Α	
Approach Delay		2.6			3.1			1.2			7.4	
Approach LOS		Α			Α			Α			Α	
Queue Length 50th (m)	0.0	0.0		0.0	8.0			0.0			0.0	
Queue Length 95th (m)	m1.2	59.7		m1.6	41.7			0.7			3.7	
Internal Link Dist (m)		651.1			64.4			41.8			21.1	
Turn Bay Length (m)	40.0			50.0								
Base Capacity (vph)	474	3099		281	3045			421			420	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	

## 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



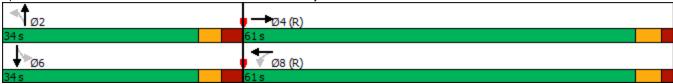
Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>†</b> †			<b>^</b>			<b>†</b>			f)	
Traffic Volume (vph)	0	1469	22	1	1002	1	1	21	1	1	5	3
Future Volume (vph)	0	1469	22	1	1002	1	1	21	1	1	5	3
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	0	3412	0	0	3386	0	0	1784	0	0	1699	0
Flt Permitted					0.954			0.984			0.956	
Satd. Flow (perm)	0	3412	0	0	3230	0	0	1759	0	0	1634	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3						1			3	
Link Speed (k/h)		60			60			50			50	
Link Distance (m)		175.1			312.5			62.4			191.1	
Travel Time (s)		10.5			18.8			4.5			13.8	
Lane Group Flow (vph)	0	1569	0	0	1057	0	0	24	0	0	9	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)		86.3			86.3			10.0			10.0	
Actuated g/C Ratio		0.91			0.91			0.11			0.11	
v/c Ratio		0.51			0.36			0.13			0.05	
Control Delay		3.0			2.3			39.3			33.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		3.0			2.3			39.3			33.4	
LOS		Α			Α			D			С	
Approach Delay		3.0			2.3			39.3			33.4	
Approach LOS		Α			Α			D			С	
Queue Length 50th (m)		0.0			1.6			4.1			1.1	
Queue Length 95th (m)		68.2			51.1			12.1			5.8	
Internal Link Dist (m)		151.1			288.5			38.4			167.1	
Turn Bay Length (m)												
Base Capacity (vph)		3099			2933			513			478	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.51			0.36			0.05			0.02	
Intersection Summary												

Area Type: Other Cycle Length: 95 Actuated Cycle Length: 95 Offset: 34 (36%), Referenced to phase 4:EBT and 8:WBTL, Start of Green Natural Cycle: 80 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.51 Intersection Signal Delay: 3.1 Intersection LOS: A Intersection Capacity Utilization 63.5% ICU Level of Service B Analysis Period (min) 15

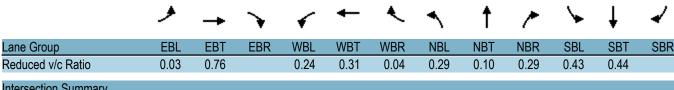
Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



## 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1•		ሻ	<b>1</b>	7	ሻ	<b>†</b>	7	ሻ	f)	
Traffic Volume (vph)	14	770	86	56	339	31	61	45	130	131	178	12
Future Volume (vph)	14	770	86	56	339	31	61	45	130	131	178	12
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	50.0		40.0	50.0		20.0	45.0		0.0
Storage Lanes	1		0	1		1	1		1	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1500	1720	0	1613	1698	1457	1676	1800	1515	1693	1752	0
Flt Permitted	0.538			0.212			0.499			0.726		
Satd. Flow (perm)	816	1720	0	360	1698	1328	871	1800	1420	1243	1752	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10				37			137		3	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		659.7			521.2			107.8			195.5	
Travel Time (s)		47.5			37.5			7.8			14.1	
Lane Group Flow (vph)	15	902	0	59	357	33	64	47	137	138	200	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	68.9	68.9		68.9	68.9	68.9	18.2	18.2	18.2	18.2	18.2	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.03	0.76		0.24	0.31	0.04	0.41	0.14	0.37	0.61	0.62	
Control Delay	3.2	10.9		10.6	8.0	2.3	41.7	32.5	8.4	48.1	44.8	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.2	10.9		10.6	8.0	2.3	41.7	32.5	8.4	48.1	44.8	
LOS	Α	В		В	Α	Α	D	С	Α	D	D	
Approach Delay		10.8			7.9			21.6			46.1	
Approach LOS		В			Α			С			D	
Queue Length 50th (m)	0.2	12.8		3.7	23.5	0.0	11.9	8.3	0.0	26.8	38.2	
Queue Length 95th (m)	m0.6	#225.9		13.1	48.7	3.2	23.2	16.9	14.9	43.1	56.3	
Internal Link Dist (m)		635.7			497.2			83.8			171.5	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	562	1188		248	1170	926	222	460	465	318	450	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	

### 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



#### Intersection Summary

Area Type: Other

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 40 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76 Intersection Signal Delay: 17.6 Intersection Capacity Utilization 88.7%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

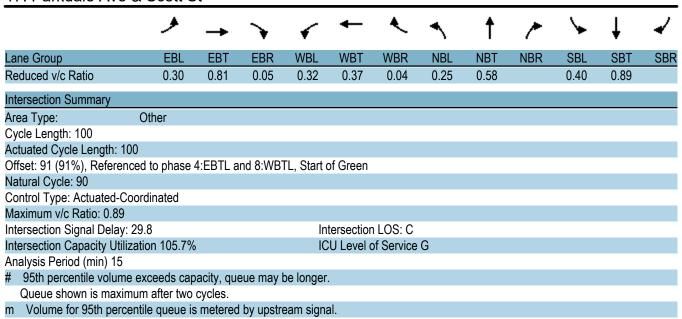
m Volume for 95th percentile queue is metered by upstream signal.

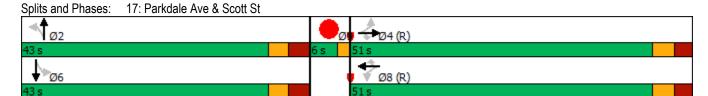
Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b></b>	7	*	<u></u>	7	ħ	f)		, j	f)	
Traffic Volume (vph)	120	686	37	50	303	24	31	281	69	97	354	145
Future Volume (vph)	120	686	37	50	303	24	31	281	69	97	354	145
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	55.0		45.0	65.0		35.0	55.0		0.0	50.0		0.0
Storage Lanes	1		1	1		1	1		0	1		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1676	1748	1485	1676	1698	1471	1660	1722	0	1693	1564	0
Flt Permitted	0.517			0.185			0.215			0.394		
Satd. Flow (perm)	830	1748	1485	326	1698	1209	363	1722	0	702	1564	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			58			58		14			23	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		116.9			659.7			94.7			654.1	
Travel Time (s)		8.4			47.5			6.8			47.1	
Lane Group Flow (vph)	126	722	39	53	319	25	33	369	0	102	526	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase				_	-						-	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	28.1	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	43.0	43.0		43.0	43.0	
Total Split (%)	51.0%	51.0%	51.0%	51.0%	51.0%	51.0%	43.0%	43.0%		43.0%	43.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	0.0	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	4.0	6.1	6.1	4.0	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	50.9	50.9	53.0	50.9	50.9	53.0	36.7	36.7		36.7	36.7	
Actuated g/C Ratio	0.51	0.51	0.53	0.51	0.51	0.53	0.37	0.37		0.37	0.37	
v/c Ratio	0.30	0.81	0.05	0.32	0.37	0.04	0.25	0.58		0.40	0.89	
Control Delay	16.7	29.6	1.8	17.5	14.0	0.4	28.2	28.7		29.3	48.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	16.7	29.6	1.8	17.5	14.0	0.4	28.2	28.7		29.3	48.5	
LOS	В	С	Α	В	В	Α	С	С		С	D	
Approach Delay		26.5			13.6			28.7			45.3	
Approach LOS		С			В			С			D	
Queue Length 50th (m)	14.2	117.9	0.0	7.3	44.8	0.1	4.6	57.4		15.2	96.1	
Queue Length 95th (m)	27.5	173.0	3.1	18.7	28.3	m0.3	13.3	87.8		31.3	#160.9	
Internal Link Dist (m)		92.9	<u> </u>		635.7			70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	422	889	814	165	864	668	133	640		257	588	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
2.2.490 040 110440111	3	<u> </u>	<u> </u>	3	<u> </u>			ŭ		<u> </u>	ŭ	

Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (m) Storage Lanes Taper Length (m) Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Right Turn on Red Satd. Flow (RTOR) Link Speed (k/h) Link Distance (m) Travel Time (s) Lane Group Flow (vph) Turn Type Protected Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (w) Yellow Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay Los Approach LOS Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn	Lane Group	Ø9
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Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn		
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn		
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Spillback Cap Reductn		
Storage Cap Reductn		
	Storage Cap Reductn	





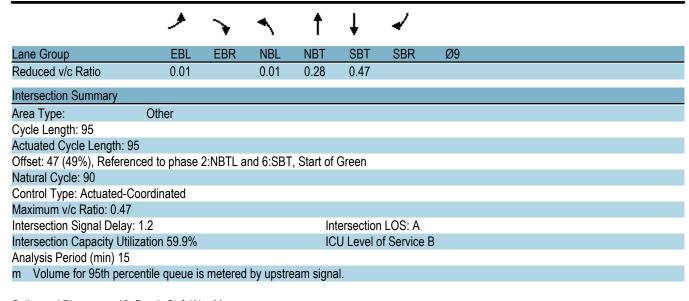
Lane Group	Ø9		
Reduced v/c Ratio			
Intersection Summary			

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		7				
Traffic Volume (vph)	0	0	0	0	0	140
Future Volume (vph)	0	0	0	0	0	140
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	0	1800	0	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1800	0	0	0	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	107.5			65.8	654.1	
Travel Time (s)	7.7			4.7	47.1	
Lane Group Flow (vph)	0	0	0	0	147	0
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					

ICU Level of Service H

Intersection Capacity Utilization Err%
Analysis Period (min) 15

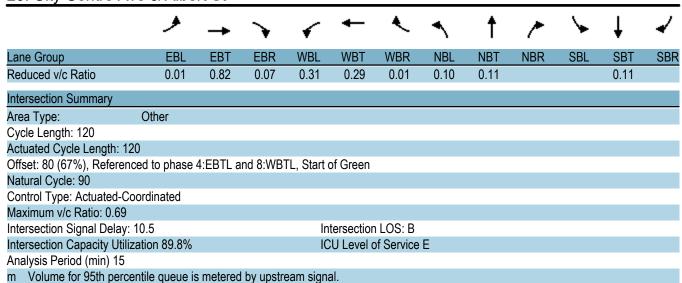
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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9		
Lane Configurations	¥		ች	<b>^</b>	<b>↑</b> ⊅				
Traffic Volume (vph)	1	1	2	864	1464	6			
Future Volume (vph)	1	1	2	864	1464	6			
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800			
Storage Length (m)	0.0	0.0	35.0	1000	1000	0.0			
Storage Lanes	1	0.0	1			0.0			
Taper Length (m)	7.5	U	7.5			U			
Satd. Flow (prot)	1600	0	1710	3420	3417	0			
Flt Permitted	0.976	U	0.154	3420	J <del>-1</del> 11	U			
Satd. Flow (perm)	1598	0	277	3420	3417	0			
Right Turn on Red	1590	Yes	211	3420	3417	Yes			
	1	168			1	168			
Satd. Flow (RTOR)				F0					
Link Speed (k/h)	50			50	50				
Link Distance (m)	172.9			216.9	212.6				
Travel Time (s)	12.4	^		15.6	15.3	^			
Lane Group Flow (vph)	2	0	_ 2	909	1547	0			
Turn Type	Prot		Perm	NA	NA				
Protected Phases	4		_	2	6		9		
Permitted Phases			2						
Detector Phase	4		2	2	6				
Switch Phase									
Minimum Initial (s)	8.3		10.0	10.0	10.0		10.0		
Minimum Split (s)	14.5		30.7	30.7	30.9		29.0		
Total Split (s)	17.0		49.0	49.0	49.0		29.0		
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%		
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0		
All-Red Time (s)	2.9		2.4	2.4	2.6		4.0		
Lost Time Adjust (s)	0.0		0.0	0.0	0.0				
Total Lost Time (s)	6.2		5.7	5.7	5.9				
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None		C-Max	C-Max	C-Max		None		
Act Effct Green (s)	8.3		91.0	91.0	90.9				
Actuated g/C Ratio	0.09		0.96	0.96	0.96				
v/c Ratio	0.01		0.01	0.28	0.47				
Control Delay	34.5		0.5	0.3	1.6				
Queue Delay	0.0		0.0	0.0	0.0				
Total Delay	34.5		0.5	0.3	1.6				
LOS	C		A	A	A				
Approach Delay	34.5		, , , , , , , , , , , , , , , , , , ,	0.3	1.6				
Approach LOS	C			A	A				
Queue Length 50th (m)	0.2		0.0	0.0	0.0				
Queue Length 95th (m)	2.4		m0.0	m5.4	60.5				
Internal Link Dist (m)	148.9		1110.0	192.9	188.6				
Turn Bay Length (m)	170.0		35.0	102.0	100.0				
Base Capacity (vph)	182		265	3274	3270				
Starvation Cap Reductn			203						
	0			0	0				
Spillback Cap Reductn	0		0	0	0				
Storage Cap Reductn	0		0	0	0				



Splits and Phases: 19: Booth St & War Museum



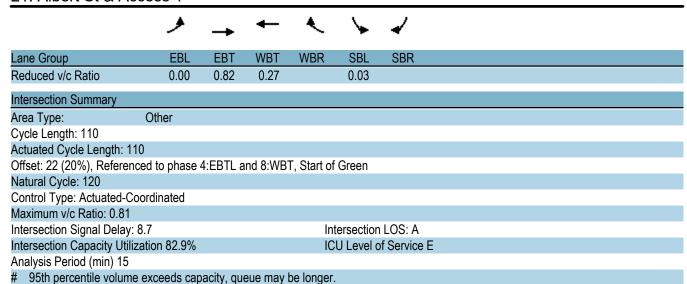
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	<b></b>	7	*	<b>†</b>	7	ň	f)			4	
Traffic Volume (vph)	5	971	77	98	387	14	28	0	47	35	0	5
Future Volume (vph)	5	971	77	98	387	14	28	0	47	35	0	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		50.0	35.0		50.0	30.0		0.0	0.0		0.0
Storage Lanes	1		1	1		1	1		0	0		0
Taper Length (m)	7.5			7.5			7.5			7.5		
Satd. Flow (prot)	1710	1782	1485	1629	1714	1530	1644	1404	0	0	1688	0
Flt Permitted	0.474			0.240			0.730				0.716	
Satd. Flow (perm)	817	1782	1406	411	1714	1311	1235	1404	0	0	1262	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			52			87		130			87	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		521.2			136.4			202.8			65.9	
Travel Time (s)		37.5			9.8			14.6			4.7	
Lane Group Flow (vph)	5	1022	81	103	407	15	29	49	0	0	42	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	98.1	99.3	101.2	96.9	96.9	98.7	12.6	12.6			12.6	
Actuated g/C Ratio	0.82	0.83	0.84	0.81	0.81	0.82	0.10	0.10			0.10	
v/c Ratio	0.01	0.69	0.07	0.31	0.29	0.01	0.22	0.19			0.20	
Control Delay	4.0	10.0	1.6	14.0	9.2	0.9	51.3	1.6			2.3	
Queue Delay	0.0	1.1	0.0	0.0	0.0	0.0	0.0	0.0			0.0	
Total Delay	4.0	11.1	1.6	14.0	9.2	0.9	51.3	1.6			2.3	
LOS	Α	В	Α	В	Α	Α	D	Α			Α	
Approach Delay		10.4			9.9			20.1			2.3	
Approach LOS		В			Α			С			Α	
Queue Length 50th (m)	0.3	88.5	1.0	8.4	44.7	0.0	6.9	0.0			0.0	
Queue Length 95th (m)	1.5	229.4	6.1	36.3	96.9	m0.4	15.0	0.0			0.6	
Internal Link Dist (m)		497.2			112.4			178.8			41.9	
Turn Bay Length (m)	30.0		50.0	35.0		50.0	30.0					
Base Capacity (vph)	747	1475	1193	331	1384	1093	305	445			377	
Starvation Cap Reductn	0	0	0	0	0	0	0	0			0	
Spillback Cap Reductn	0	228	0	0	0	0	0	16			11	
Storage Cap Reductn	0	0	0	0	0	0	0	0			0	



Splits and Phases: 20: City Centre Ave & Albert St

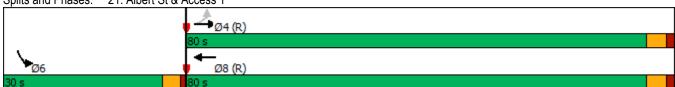


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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	<u> </u>	<b>↑</b> ↑		¥	U DI K
Traffic Volume (vph)	1	1212	740	4	9	2
Future Volume (vph)	1	1212	740	4	9	2
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0	1000	1000	0.0	0.0	0.0
Storage Lanes	1			0.0	1	0.0
Taper Length (m)	7.5				7.5	U
Satd. Flow (prot)	1710	1748	3194	0	1687	0
Flt Permitted	0.355	1740	3194	U	0.961	U
	639	1748	3194	0	1687	0
Satd. Flow (perm)	039	1748	3194		1007	
Right Turn on Red			4	Yes		Yes
Satd. Flow (RTOR)			1		2	
Link Speed (k/h)		50	50		50	
Link Distance (m)		143.1	165.9		101.8	
Travel Time (s)		10.3	11.9		7.3	
Lane Group Flow (vph)	1	1276	783	0	11	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases		4	8		6	
Permitted Phases	4					
Detector Phase	4	4	8		6	
Switch Phase			-		•	
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	22.0	22.0	22.0		28.7	
Total Split (s)	80.0	80.0	80.0		30.0	
Total Split (%)	72.7%	72.7%	72.7%		27.3%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.4	1.4	1.4		1.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	4.7	4.7	4.7		4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Min		None	
Act Effct Green (s)	99.7	99.7	99.7		12.8	
Actuated g/C Ratio	0.91	0.91	0.91		0.12	
v/c Ratio	0.00	0.81	0.27		0.06	
Control Delay	3.0	12.2	2.5		36.0	
Queue Delay	0.0	0.2	0.0		0.0	
Total Delay	3.0	12.4	2.5		36.0	
LOS	A	В	Α		D	
Approach Delay	•	12.3	2.5		36.0	
Approach LOS		В	A		D	
Queue Length 50th (m)	0.0	0.0	0.0		1.9	
Queue Length 95th (m)	0.5	#394.4	41.6		6.5	
Internal Link Dist (m)	0.5	119.1	141.9		77.8	
,	30.0	113.1	141.3		11.0	
Turn Bay Length (m)		1501	2005		400	
Base Capacity (vph)	579	1584	2895		400	
Starvation Cap Reductn	0	30	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	

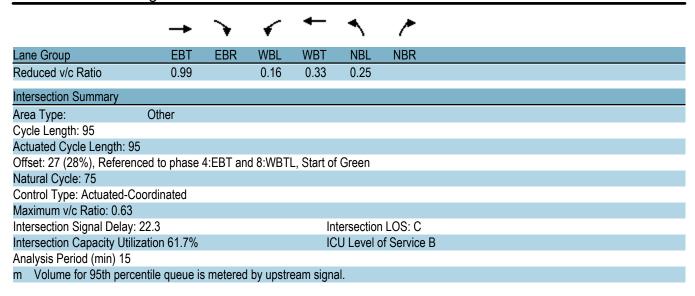


Splits and Phases: 21: Albert St & Access 1

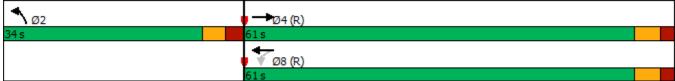
Queue shown is maximum after two cycles.



	<b>→</b>	$\rightarrow$	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	LBIX	**************************************	<b>†</b>	¥	HOIN
Traffic Volume (vph)	1443	16	26	744	37	77
Future Volume (vph)	1443	16	26	744	37	77
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	0.0	70.0	1000	0.0	0.0
Storage Lanes		0.0	1		1	0.0
Taper Length (m)		U	7.5		7.5	U
	3366	0	1710	3320	1522	0
Satd. Flow (prot)	3300	U		3320		U
Flt Permitted	2200	0	0.130	2200	0.984	0
Satd. Flow (perm)	3366	0	234	3320	1519	0
Right Turn on Red		Yes			2.1	Yes
Satd. Flow (RTOR)	2				31	
Link Speed (k/h)	60			60	50	
Link Distance (m)	140.7			270.9	72.8	
Travel Time (s)	8.4			16.3	5.2	
Lane Group Flow (vph)	1536	0	27	783	120	0
Turn Type	NA		Perm	NA	Prot	
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phase	4		8	8	2	
Switch Phase						
Minimum Initial (s)	10.0		10.0	10.0	10.0	
Minimum Split (s)	30.0		22.0	22.0	33.9	
Total Split (s)	61.0		61.0	61.0	34.0	
Total Split (%)	64.2%		64.2%	64.2%	35.8%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.1		2.1	2.1	2.6	
. ,						
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.8		5.8	5.8	5.9	
Lead/Lag						
Lead-Lag Optimize?	0.17		0	0		
Recall Mode	C-Max		C-Max	C-Max	None	
Act Effct Green (s)	68.9		68.9	68.9	14.4	
Actuated g/C Ratio	0.73		0.73	0.73	0.15	
v/c Ratio	0.63		0.16	0.33	0.47	
Control Delay	19.1		9.3	6.1	31.3	
Queue Delay	11.0		0.0	0.0	0.0	
Total Delay	30.1		9.3	6.1	31.3	
LOS	С		Α	Α	С	
Approach Delay	30.1			6.2	31.3	
Approach LOS	C			A	С	
Queue Length 50th (m)	121.4		1.1	20.1	16.4	
Queue Length 95th (m)	m150.0		7.8	53.2	27.4	
Internal Link Dist (m)	116.7		1.0	246.9	48.8	
Turn Bay Length (m)	110.7		70.0	270.0	₹0.0	
Base Capacity (vph)	2440		169	2406	472	
			0	2400		
Starvation Cap Reductn	896				0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	



Splits and Phases: 22: Lett St & Wellington St



	۶	<b>→</b>	•	•	<b>←</b>	•	4	†	~	<b>/</b>	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>↑</b> ↑			4T>			4			4	
Traffic Volume (vph)	19	1031	8	5	420	7	8	0	11	28	0	7
Future Volume (vph)	19	1031	8	5	420	7	8	0	11	28	0	7
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	1710	3318	0	0	3192	0	0	1621	0	0	1685	0
Flt Permitted	0.490				0.945			0.884			0.754	
Satd. Flow (perm)	882	3318	0	0	3020	0	0	1462	0	0	1322	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			2			15			15	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		106.5			242.4			82.0			41.2	
Travel Time (s)		7.7			17.5			5.9			3.0	
Lane Group Flow (vph)	20	1093	0	0	454	0	0	20	0	0	36	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6		30.5	30.5		30.5	30.5	
Total Split (s)	81.0	81.0		81.0	81.0		39.0	39.0		39.0	39.0	
Total Split (%)	67.5%	67.5%		67.5%	67.5%		32.5%	32.5%		32.5%	32.5%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.6	4.6			4.6			4.3			4.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	105.7	105.7			105.7			13.0			13.0	
Actuated g/C Ratio	0.88	0.88			0.88			0.11			0.11	
v/c Ratio	0.03	0.37			0.17			0.12			0.23	
Control Delay	1.5	2.7			2.5			24.4			34.3	
Queue Delay	0.0	0.3			0.0			0.0			0.0	
Total Delay	1.5	3.0			2.5			24.4			34.3	
LOS	Α	Α			Α			С			С	
Approach Delay		3.0			2.5			24.4			34.3	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	0.1	8.1			8.0			1.2			4.9	
Queue Length 95th (m)	m1.0	m69.1			23.4			7.9			13.7	
Internal Link Dist (m)		82.5			218.4			58.0			17.2	
Turn Bay Length (m)												
Base Capacity (vph)	776	2921			2659			433			392	
Starvation Cap Reductn	0	1093			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.03	0.60			0.17			0.05			0.09	
Intersection Summary												

Area Type:	Other	
Cycle Length: 120		
Actuated Cycle Length: 12	0	
Offset: 22 (18%), Reference	ced to phase 4:EBTL and 8:WBTL, St	tart of Green
Natural Cycle: 60		
Control Type: Actuated-Co	ordinated	
Maximum v/c Ratio: 0.37		
Intersection Signal Delay:	3.8	Intersection LOS: A
Intersection Capacity Utiliz	ation 46.1%	ICU Level of Service A
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	•	<b>→</b>	•	•	-	4
					0.71	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			<b>ተ</b> ኈ			7
Traffic Volume (vph)	0	1039	440	7	0	7
Future Volume (vph)	0	1039	440	7	0	7
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	0	3420	3413	0	0	1557
Flt Permitted						
Satd. Flow (perm)	0	3420	3413	0	0	1557
Link Speed (k/h)		50	50		50	
Link Distance (m)		86.1	106.5		54.7	
Travel Time (s)		6.2	7.7		3.9	
Lane Group Flow (vph)	0	1094	470	0	0	7
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 33.6%			IC	U Level o	of Service

	•	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	~	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		<b>↑</b> ↑			<b>∱</b> β	
Traffic Volume (vph)	0	0	0	0	0	12	0	857	17	0	966	0
Future Volume (vph)	0	0	0	0	0	12	0	857	17	0	966	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Flt Permitted												
Satd. Flow (perm)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		65.8			86.3			217.2			79.5	
Travel Time (s)		4.7			6.2			15.6			5.7	
Lane Group Flow (vph)	0	0	0	0	0	13	0	920	0	0	1017	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type: Other

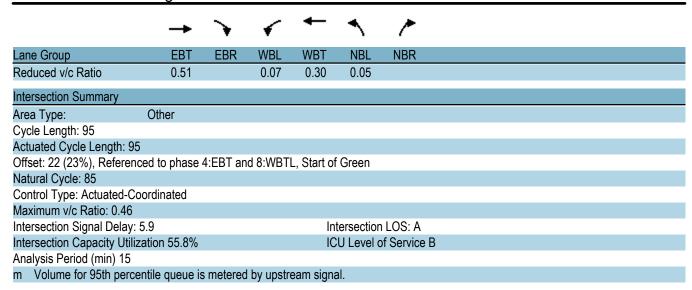
Control Type: Unsignalized Intersection Capacity Utilization 35.6% ICU Level of Service A

Analysis Period (min) 15

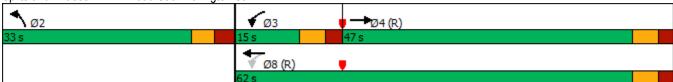
	-	•	•	•	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> ∱			<b>^</b>		7
Traffic Volume (vph)	1286	0	0	910	0	13
Future Volume (vph)	1286	0	0	910	0	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Link Speed (k/h)	60			60	50	
Link Distance (m)	73.9			91.5	39.3	
Travel Time (s)	4.4			5.5	2.8	
Lane Group Flow (vph)	1354	0	0	958	0	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 47.5%			IC	U Level o	of Service

Analysis Period (min) 15

	<b>→</b>	$\rightarrow$	•	←	4	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	LDIN	YVDL	<b>†</b>	NDL W	HUIT
Traffic Volume (vph)	1272	2	26	884	<b>T</b> 4	13
Future Volume (vph)	1272	2	26	884	4	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	1000	0.0	60.0	1000	0.0	0.0
		0.0	1		1	0.0
Storage Lanes		U			7.5	U
Taper Length (m)	2400	^	7.5	2400		٥
Satd. Flow (prot)	3420	0	1710	3420	1593	0
FIt Permitted	0.400	•	0.161	0.400	0.989	•
Satd. Flow (perm)	3420	0	290	3420	1593	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)					14	
Link Speed (k/h)	60			60	50	
Link Distance (m)	101.2			73.9	56.0	
Travel Time (s)	6.1			4.4	4.0	
Lane Group Flow (vph)	1341	0	27	931	18	0
Turn Type	NA		pm+pt	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases			8			
Detector Phase	4		3	8	2	
Switch Phase	7		J	- 0	L	
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	37.0		11.0	37.0	32.3	
				62.0	33.0	
Total Split (s)	47.0		15.0			
Total Split (%)	49.5%		15.8%	65.3%	34.7%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.3		2.3	2.3	3.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	6.0		6.0	6.0	6.3	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	81.2		82.5	86.1	10.0	
Actuated g/C Ratio	0.85		0.87	0.91	0.11	
v/c Ratio	0.46		0.08	0.30	0.10	
Control Delay	7.0		2.0	4.1	22.9	
Queue Delay	0.1		0.0	0.0	0.1	
Total Delay	7.0		2.0	4.1	22.9	
LOS	7.0 A			4.1 A	22.9 C	
			Α			
Approach Delay	7.0			4.0	22.9	
Approach LOS	A		0.4	A	C	
Queue Length 50th (m)	0.0		0.1	0.0	0.7	
Queue Length 95th (m)	137.0		m1.9	95.8	7.4	
Internal Link Dist (m)	77.2			49.9	32.0	
Turn Bay Length (m)			60.0			
Base Capacity (vph)	2923		386	3099	457	
Starvation Cap Reductn	113		0	0	0	
Spillback Cap Reductn	291		0	0	129	
Storage Cap Reductn	0		0	0	0	



Splits and Phases: 27: Broad St & Wellington St



	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> }			<b>^</b>		7
Traffic Volume (vph)	1261	2	0	886	0	13
Future Volume (vph)	1261	2	0	886	0	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Satd. Flow (prot)	3420	0	0	3420	0	1557
Flt Permitted						
Satd. Flow (perm)	3420	0	0	3420	0	1557
Link Speed (k/h)	60			60	50	
Link Distance (m)	88.4			101.2	49.3	
Travel Time (s)	5.3			6.1	3.5	
Lane Group Flow (vph)	1329	0	0	933	0	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utiliz	tersection Capacity Utilization 46.9%				U Level o	of Service

Analysis Period (min) 15

	_	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	806	424	738	1105	852	459
Future Volume (vph)	806	424	738	1105	852	459
Satd. Flow (prot)	3317	4914	3386	2614	3285	1530
Flt Permitted	0.950				0.950	,,,,,
Satd. Flow (perm)	3292	4914	3386	2614	3168	1530
Satd. Flow (RTOR)	0202	1011	0000	2011	0.00	182
Lane Group Flow (vph)	848	446	777	1163	897	483
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	81	1	1 01111
Permitted Phases	1	7	U	0 1		1
Detector Phase	7	4	8	8 1	1	1
Switch Phase	- 1	4	0	0 1		
	10.0	10.0	10.0		10.0	10.0
Minimum Initial (s)						
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
Total Split (s)	55.8	92.3	36.5		44.1	44.1
Total Split (%)	40.9%	67.7%	26.8%		32.3%	32.3%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	-2.5		-2.1	0.0
Total Lost Time (s)	5.8	6.5	4.0		4.0	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Max	Min	Min		None	None
Act Effct Green (s)	50.0	85.8	32.5	74.1	40.1	38.0
Actuated g/C Ratio	0.37	0.63	0.24	0.54	0.29	0.28
v/c Ratio	0.70	0.14	0.96	0.82	0.93	0.87
Control Delay	40.5	10.5	75.3	31.7	63.2	45.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.5	10.5	75.3	31.7	63.2	45.8
LOS	40.5 D	10.5 B	75.5 E	31.7 C	63.2 E	45.6 D
	U			C		U
Approach Delay		30.1	49.1		57.1	
Approach LOS	4040	C	D	440.4	E	00.0
Queue Length 50th (m)	104.9	17.8	115.4	149.1	127.6	86.8
Queue Length 95th (m)	129.0		#157.5	185.6	#167.5	#150.9
Internal Link Dist (m)		245.9	289.7		38.9	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1215	3091	806	1420	965	557
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.70	0.14	0.96	0.82	0.93	0.87

Cycle Length: 136.4

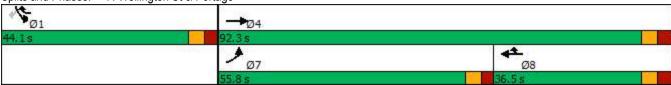
Actuated Cycle Length: 136.4

Natural Cycle: 115 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.96

Intersection Signal Delay: 46.2 Intersection LOS: D
Intersection Capacity Utilization 83.0% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.

Splits and Phases: 7: Wellington St & Portage

Queue shown is maximum after two cycles.



	ၨ	<b>→</b>	←	•	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<u></u>	<u> </u>	7	UDL	7
Traffic Volume (vph)	0	824	1066	0	0	7
Future Volume (vph)	0	824	1066	0	0	7
Satd. Flow (prot)	0	1800	1800	1800	0	1557
Flt Permitted						
Satd. Flow (perm)	0	1800	1800	1800	0	1557
Lane Group Flow (vph)	0	867	1122	0	0	7
Sign Control		Free	Free		Stop	
Intersection Summary						
Control Type: Unsignalized	_			_	_	_
Intersection Capacity Utilization	on 69.2%			IC	U Level c	of Service
Analysis Period (min) 15						

Bane Group   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR		•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ţ	4
Traffic Volume (vph) 33 0 33 0 0 0 0 7 1111 0 0 1049 7 Future Volume (vph) 33 0 33 0 0 0 0 7 1111 0 0 0 1049 7 Satat. Flow (prot) 1710 1530 0 1800 1800 0 1700 1800 0 1798 0 FIL Permitted 0 0.757 0.160 Satat. Flow (prot) 1363 1530 0 1800 1800 0 288 1800 0 1800 1798 0 Satat. Flow (RTOR) 106	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (vph)	Lane Configurations	Ť	f)		Ţ	f)		7	f)		7	î»	
Future Volume (vph) 33 0 33 0 0 0 0 7 11111 0 0 0 1049 7 Satd. Flow (prot) 1710 1530 0 1800 1800 0 1710 1800 0 1800 1798 0 FIF Permitted 0.757 0.160 0	Traffic Volume (vph)		0		0	0	0	7	1111	0	0	1049	7
Fit Permitted 0.757	Future Volume (vph)	33	0	33	0	0	0	7	1111	0		1049	7
Satd. Flow (perm)   1363   1530   0   1800   1800   0   288   1800   0   1800   1798   0	Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1798	0
Satd. Flow (RTOR)	Flt Permitted												
Lane Group Flow (vph)   35   35   0   0   0   0   7   1169   0   0   1111   0     Turn Type	Satd. Flow (perm)	1363		0	1800	1800	0	288	1800	0	1800	1798	0
Turn Type         Perm         NA         Perm         NA         Perm         NA           Protected Phases         4         8         2         6           Permitted Phases         4         8         2         6           Detector Phase         4         4         8         8         2         2         6           Switch Phase         8         1         10.0 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>													
Protected Phases	, , ,			0		0	0			0			0
Permitted Phases		Perm			Perm			Perm			Perm		
Detector Phase   4			4			8			2			6	
Switch Phase   Minimum Initial (s)	Permitted Phases												
Minimum Initial (s)         10.0         35.9         35.0         53.0         26.0 </td <td>Detector Phase</td> <td>4</td> <td>4</td> <td></td> <td>8</td> <td>8</td> <td></td> <td>2</td> <td>2</td> <td></td> <td>6</td> <td>6</td> <td></td>	Detector Phase	4	4		8	8		2	2		6	6	
Minimum Split (s)         22.0         22.0         22.0         22.0         22.0         35.9         35.9         35.9         35.9         35.9         35.9         35.0         53.0         26.2         26.2         26.2 <td>Switch Phase</td> <td></td>	Switch Phase												
Total Split (s)         22.0         22.0         22.0         22.0         22.0         53.0         53.0         53.0           Total Split (%)         29.3%         29.3%         29.3%         70.7%         70.7%         70.7%         70.7%           Yellow Time (s)         3.0         3.0         3.0         3.3         3.3         3.3         3.3           All-Red Time (s)         3.2         3.2         3.2         2.6         2.6         2.6         2.6           Lost Time Adjust (s)         0.0	Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0			10.0	10.0	
Total Split (%)         29.3%         29.3%         29.3%         29.3%         70.7%         70.7%         70.7%           Yellow Time (s)         3.0         3.0         3.0         3.0         3.3         3.5         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9         5.9	Minimum Split (s)	22.0			22.0			35.9					
Yellow Time (s)         3.0         3.0         3.0         3.0         3.3         3.4         3.1         3.1         3.1         3.3         3.1	Total Split (s)	22.0			22.0	22.0		53.0	53.0			53.0	
All-Red Time (s)         3.2         3.2         3.2         3.2         2.6         2.6         2.6         2.6           Lost Time Adjust (s)         0.0	Total Split (%)		29.3%		29.3%	29.3%		70.7%			70.7%	70.7%	
Lost Time Adjust (s)   0.0	Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
Total Lost Time (s)         6.2         6.2         6.2         6.2         5.9         5.9         5.9         5.9           Lead/Lag         Lead-Lag Optimize?         Recall Mode         None         None         None         None         C-Max         0.82	All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lead/Lag         Lead-Lag Optimize?           Recall Mode         None         None         None         C-Max         11.6         C-Max         C-Max         C-Max         C-Max         C-Max         C-Max         C-Max         C-Max         11.6         C-Max         C-Max         C-Max         C-Max         11.6         A         B         B         B         B         B         B         B         B         B         B <td>Lost Time Adjust (s)</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td>	Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Lead-Lag Optimize?         Recall Mode         None         None         None         None         C-Max         C-Max </td <td>Total Lost Time (s)</td> <td>6.2</td> <td>6.2</td> <td></td> <td>6.2</td> <td>6.2</td> <td></td> <td>5.9</td> <td>5.9</td> <td></td> <td>5.9</td> <td>5.9</td> <td></td>	Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
Recall Mode         None         None         None         None         C-Max         C-Max         C-Max           Act Effct Green (s)         10.0         10.0         61.7         61.7         61.7           Actuated g/C Ratio         0.13         0.13         0.82         0.82         0.82           v/c Ratio         0.19         0.12         0.03         0.79         0.75           Control Delay         31.9         0.8         3.9         13.4         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         31.9         0.8         3.9         13.4         11.6           LOS         C         A         A         B         B           Approach Delay         16.3         13.3         11.6         11.6           Approach LOS         B         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         119.6         104.3           Queue Length 95th (m)         12.9         0.0         1.4         #243.0         #224.5           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Tu	Lead/Lag												
Act Effct Green (s)       10.0       10.0       61.7       61.7       61.7         Actuated g/C Ratio       0.13       0.13       0.82       0.82       0.82         v/c Ratio       0.19       0.12       0.03       0.79       0.75         Control Delay       31.9       0.8       3.9       13.4       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.8       3.9       13.4       11.6         LOS       C       A       A       B       B         Approach Delay       16.3       13.3       11.6         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0       0	Lead-Lag Optimize?												
Actuated g/C Ratio       0.13       0.13       0.82       0.82       0.82         v/c Ratio       0.19       0.12       0.03       0.79       0.75         Control Delay       31.9       0.8       3.9       13.4       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.8       3.9       13.4       11.6         LOS       C       A       A       B       B         Approach Delay       16.3       13.3       11.6         Approach LOS       B       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	Recall Mode	None			None	None		C-Max			C-Max		
v/c Ratio         0.19         0.12         0.03         0.79         0.75           Control Delay         31.9         0.8         3.9         13.4         11.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         31.9         0.8         3.9         13.4         11.6           LOS         C         A         A         B         B           Approach Delay         16.3         13.3         11.6         B           Approach LOS         B         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         119.6         104.3           Queue Length 95th (m)         12.9         0.0         1.4         #243.0         #224.5           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         405         237         1481         1480           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0	Act Effct Green (s)												
Control Delay       31.9       0.8       3.9       13.4       11.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.8       3.9       13.4       11.6         LOS       C       A       A       B       B         Approach Delay       16.3       13.3       11.6         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	Actuated g/C Ratio												
Queue Delay       0.0       0.0       0.0       0.0         Total Delay       31.9       0.8       3.9       13.4       11.6         LOS       C       A       A       B       B         Approach Delay       16.3       13.3       11.6         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0	v/c Ratio												
Total Delay         31.9         0.8         3.9         13.4         11.6           LOS         C         A         A         B         B           Approach Delay         16.3         13.3         11.6           Approach LOS         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         119.6         104.3           Queue Length 95th (m)         12.9         0.0         1.4         #243.0         #224.5           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         405         237         1481         1480           Starvation Cap Reductn         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0	Control Delay												
LOS         C         A         B         B           Approach Delay         16.3         13.3         11.6           Approach LOS         B         B         B           Queue Length 50th (m)         4.7         0.0         0.3         119.6         104.3           Queue Length 95th (m)         12.9         0.0         1.4         #243.0         #224.5           Internal Link Dist (m)         60.2         43.3         76.9         31.3           Turn Bay Length (m)         30.0         30.0           Base Capacity (vph)         287         405         237         1481         1480           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0	Queue Delay												
Approach Delay       16.3       13.3       11.6         Approach LOS       B       B       B         Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Total Delay	31.9	8.0					3.9	13.4			11.6	
Approach LOS B B B B Queue Length 50th (m) 4.7 0.0 0.3 119.6 104.3 Queue Length 95th (m) 12.9 0.0 1.4 #243.0 #224.5 Internal Link Dist (m) 60.2 43.3 76.9 31.3 Turn Bay Length (m) 30.0 30.0 Base Capacity (vph) 287 405 237 1481 1480 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0	LOS	С						Α					
Queue Length 50th (m)       4.7       0.0       0.3       119.6       104.3         Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Approach Delay								13.3			11.6	
Queue Length 95th (m)       12.9       0.0       1.4       #243.0       #224.5         Internal Link Dist (m)       60.2       43.3       76.9       31.3         Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0													
Internal Link Dist (m)     60.2     43.3     76.9     31.3       Turn Bay Length (m)     30.0     30.0       Base Capacity (vph)     287     405     237     1481     1480       Starvation Cap Reductn     0     0     0     0       Spillback Cap Reductn     0     0     0     0		4.7						0.3					
Turn Bay Length (m)       30.0       30.0         Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Queue Length 95th (m)	12.9	0.0					1.4	#243.0			#224.5	
Base Capacity (vph)       287       405       237       1481       1480         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0	Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0	Turn Bay Length (m)							30.0					
Spillback Cap Reductn 0 0 0 0		287							1481			1480	
	Starvation Cap Reductn	0						0	0			0	
		0	0					0	0			0	
Storage Cap Reductn 0 0 0 0	Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio 0.12 0.09 0.03 0.79 0.75	Reduced v/c Ratio	0.12	0.09					0.03	0.79			0.75	

Cycle Length: 75

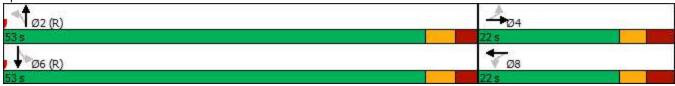
Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.79
Intersection Signal Delay: 12.6 Intersection LOS: B
Intersection Capacity Utilization 80.1% ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>∱</b> }		ሻ	<b>^</b>	7
Traffic Volume (vph)	0	906	0	0	1320	117	0	983	176	109	648	232
Future Volume (vph)	0	906	0	0	1320	117	0	983	176	109	648	232
Satd. Flow (prot)	0	3420	0	0	3420	1530	0	3229	0	1613	3196	1515
Flt Permitted										0.084		
Satd. Flow (perm)	0	3420	0	0	3420	1489	0	3229	0	143	3196	1472
Satd. Flow (RTOR)								19				35
Lane Group Flow (vph)	0	954	0	0	1389	123	0	1220	0	115	682	244
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Minimum Split (s)		35.8			37.4	37.4		31.8		12.0	31.9	31.9
Total Split (s)		60.0			60.0	60.0		48.0		12.0	60.0	60.0
Total Split (%)		50.0%			50.0%	50.0%		40.0%		10.0%	50.0%	50.0%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	-2.8		-2.1		0.0	0.0	-2.9
Total Lost Time (s)		6.8			6.8	4.0		4.7		6.8	6.9	4.0
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Act Effct Green (s)		53.2			53.2	56.0		43.3		53.2	53.1	56.0
Actuated g/C Ratio		0.44			0.44	0.47		0.36		0.44	0.44	0.47
v/c Ratio		0.63			0.92	0.18		1.04		0.91	0.48	0.35
Control Delay		24.2			46.7	22.7		61.2		83.8	25.1	18.9
Queue Delay		0.5			46.0	0.0		0.0		0.0	0.0	0.0
Total Delay		24.7			92.7	22.7		61.2		83.8	25.1	18.9
LOS		С			F	С		Е		F	С	В
Approach Delay		24.7			87.0			61.2			30.2	
Approach LOS		С			F			Ε			С	
Queue Length 50th (m)		100.5			190.3	20.8		~91.3		16.7	61.7	31.7
Queue Length 95th (m)		125.2			#220.7	35.9		m#82.7		#51.1	79.1	52.1
Internal Link Dist (m)		72.0			117.1			52.2			192.9	
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1516			1516	694		1177		127	1414	705
Starvation Cap Reductn		196			331	0		0		0	0	0
Spillback Cap Reductn		0			0	0		0		0	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.72			1.17	0.18		1.04		0.91	0.48	0.35

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 3 (3%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 105 Control Type: Pretimed Maximum v/c Ratio: 1.04 Intersection Signal Delay: 55.3

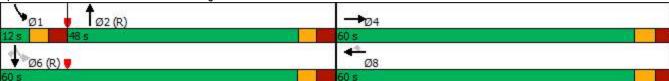
Intersection LOS: E ICU Level of Service F

Analysis Period (min) 15

Intersection Capacity Utilization 94.9%

- Volume exceeds capacity, queue is theoretically infinite.
  - Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
  - Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Booth St & Wellington St



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	<u></u>	7	7	<b>^</b>	7		€î∌		, j	<u></u>	7
Traffic Volume (vph)	552	580	32	33	871	239	12	540	67	98	411	247
Future Volume (vph)	552	580	32	33	871	239	12	540	67	98	411	247
Satd. Flow (prot)	1676	1765	1485	1710	3386	1530	0	3279	0	1629	1782	1515
Flt Permitted	0.098			0.433				0.940		0.158		
Satd. Flow (perm)	171	1765	1451	776	3386	1360	0	3083	0	263	1782	1381
Satd. Flow (RTOR)			91					11				
Lane Group Flow (vph)	581	611	34	35	917	252	0	652	0	103	433	260
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		4.5	10.0	10.0
Minimum Split (s)	16.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.0	34.5	34.5
Total Split (s)	29.0	70.0	70.0	41.0	41.0	41.0	38.0	38.0		12.0	50.0	50.0
Total Split (%)	24.2%	58.3%	58.3%	34.2%	34.2%	34.2%	31.7%	31.7%		10.0%	41.7%	41.7%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	-2.5	0.0	-2.5	0.0	0.0	-2.5		0.0		0.0	0.0	-2.5
Total Lost Time (s)	4.0	6.5	4.0	6.5	6.5	4.0		6.5		6.5	6.5	4.0
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	C-Max	C-Max	C-Max	None	None	None	None	None		None	None	None
Act Effct Green (s)	68.7	66.2	68.7	34.5	34.5	37.0		28.8		40.8	40.8	43.3
Actuated g/C Ratio	0.57	0.55	0.57	0.29	0.29	0.31		0.24		0.34	0.34	0.36
v/c Ratio	1.31	0.63	0.04	0.16	0.94	0.60		0.87		0.68	0.72	0.52
Control Delay	176.6	22.5	0.1	25.9	53.1	34.9		56.3		68.1	60.5	54.3
Queue Delay	0.0	1.9	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	176.6	24.4	0.1	25.9	53.1	34.9		56.3		68.1	60.5	54.3
LOS	F	С	Α	С	D	С		Е		Е	Е	D
Approach Delay		95.8			48.5			56.3			59.5	
Approach LOS		F			D			Е			Е	
Queue Length 50th (m)	~173.2	148.6	0.0	6.4	117.0	52.8		79.7		26.3	112.9	67.0
Queue Length 95th (m)	#251.3	122.2	m0.0	m10.8	#159.1	82.6		101.6		#49.3	146.7	94.9
Internal Link Dist (m)		138.6			62.5			37.2			83.1	
Turn Bay Length (m)	160.0		40.0	40.0								120.0
Base Capacity (vph)	445	973	869	223	973	419		817		151	645	529
Starvation Cap Reductn	0	211	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	1.31	0.80	0.04	0.16	0.94	0.60		0.80		0.68	0.67	0.49

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 9 (8%), Referenced to phase 4:EBTL and 7:EBL, Start of Green

Natural Cycle: 130

Maximum v/c Ratio: 1.31
Intersection Signal Delay: 67.0 Intersection LOS: E
Intersection Capacity Utilization 123.6% ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

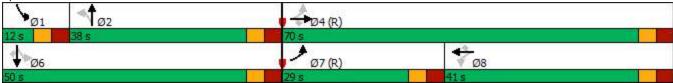
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



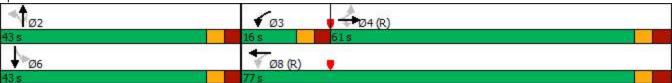
	۶	<b>→</b>	•	•	←	•	4	<b>†</b>	<b>/</b>	-	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	ሻ	<b>↑</b> ↑			4	7		4	
Traffic Volume (vph)	11	708	135	267	938	32	151	16	343	41	15	3
Future Volume (vph)	11	708	135	267	938	32	151	16	343	41	15	3
Satd. Flow (prot)	1710	1782	1500	1676	3367	0	0	1648	1485	0	1724	0
Flt Permitted	0.281			0.092				0.737			0.634	
Satd. Flow (perm)	506	1782	1412	162	3367	0	0	1245	1441	0	1125	0
Satd. Flow (RTOR)			86		5				262		2	
Lane Group Flow (vph)	12	745	142	281	1021	0	0	176	361	0	62	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	61.0	61.0	61.0	16.0	77.0		43.0	43.0	43.0	43.0	43.0	
Total Split (%)	50.8%	50.8%	50.8%	13.3%	64.2%		35.8%	35.8%	35.8%	35.8%	35.8%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	6.8	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	None	C-Max		None	None	None	None	None	
Act Effct Green (s)	54.2	54.2	57.0	84.2	83.6			23.3	23.3		22.6	
Actuated g/C Ratio	0.45	0.45	0.48	0.70	0.70			0.19	0.19		0.19	
v/c Ratio	0.05	0.93	0.20	0.69	0.44			0.73	0.74		0.29	
Control Delay	17.1	46.6	6.1	61.2	2.7			61.5	21.4		40.8	
Queue Delay	0.0	3.7	0.0	0.0	0.2			0.0	1.2		0.3	
Total Delay	17.1	50.3	6.1	61.2	2.9			61.5	22.6		41.1	
LOS	В	D	Α	Е	Α			E	С		D	
Approach Delay		42.9			15.5			35.4			41.1	
Approach LOS		D			В			D			D	
Queue Length 50th (m)	1.2	170.8	3.9	52.5	10.3			41.3	22.0		12.8	
Queue Length 95th (m)	m3.3	#254.1	11.1	#113.6	23.3			60.9	53.4		23.8	
Internal Link Dist (m)		83.1			122.5			54.7			49.4	
Turn Bay Length (m)	30.0		16.0	90.0								
Base Capacity (vph)	228	804	715	405	2346			380	622		345	
Starvation Cap Reductn	0	0	0	0	529			0	0		0	
Spillback Cap Reductn	0	29	0	0	141			0	107		76	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.05	0.96	0.20	0.69	0.56			0.46	0.70		0.23	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 65 (54%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green Natural Cycle: 90

Maximum v/c Ratio: 0.93
Intersection Signal Delay: 28.6 Intersection LOS: C
Intersection Capacity Utilization 90.9% ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



# 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

	٠	-	$\rightarrow$	•	←	*	1	<b>†</b>	<b>/</b>	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>↑</b> ↑		7	<b>↑</b> ↑			4			4	
Traffic Volume (vph)	2	938	4	17	1578	7	6	0	7	5	0	22
Future Volume (vph)	2	938	4	17	1578	7	6	0	7	5	0	22
Satd. Flow (prot)	1710	3383	0	1710	3416	0	0	1617	0	0	1586	0
Flt Permitted	0.141			0.260				0.846			0.938	
Satd. Flow (perm)	254	3383	0	468	3416	0	0	1400	0	0	1500	0
Satd. Flow (RTOR)		1			1			82			82	
Lane Group Flow (vph)	2	991	0	18	1668	0	0	13	0	0	28	0
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		3	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		5.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		11.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	72.0	72.0		15.0	87.0		33.0	33.0		33.0	33.0	
Total Split (%)	60.0%	60.0%		12.5%	72.5%		27.5%	27.5%		27.5%	27.5%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0			-2.3			-2.3	
Total Lost Time (s)	4.0	4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Act Effct Green (s)	103.1	103.1		106.2	107.8			12.3			12.3	
Actuated g/C Ratio	0.86	0.86		0.88	0.90			0.10			0.10	
v/c Ratio	0.01	0.34		0.04	0.54			0.06			0.12	
Control Delay	4.5	3.8		0.4	2.2			0.5			1.1	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	4.5	3.8		0.4	2.3			0.5			1.1	
LOS	A	Α		Α	Α			Α			Α	
Approach Delay		3.8			2.2			0.5			1.1	
Approach LOS		Α			Α			Α			Α	
Queue Length 50th (m)	0.1	23.4		0.4	1.8			0.0			0.0	
Queue Length 95th (m)	0.9	53.4		m0.1	2.1			0.0			0.0	
Internal Link Dist (m)		651.1			66.3			44.5			21.1	
Turn Bay Length (m)	40.0			50.0								
Base Capacity (vph)	218	2905		528	3069			400			424	
Starvation Cap Reductn	0	0		0	107			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.01	0.34		0.03	0.56			0.03			0.07	

Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 37 (31%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 85

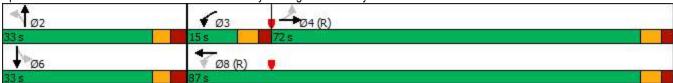
## 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl

Maximum v/c Ratio: 0.54
Intersection Signal Delay: 2.8
Intersection Capacity Utilization 62.1%
ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 14: Sir John A. Macdonald Pkwy/Wellington St & Vimy Pl



	۶	-	$\rightarrow$	•	←	*	1	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>1</b>			f)	
Traffic Volume (vph)	0	993	4	2	1738	0	26	14	35	3	26	11
Future Volume (vph)	0	993	4	2	1738	0	26	14	35	3	26	11
Satd. Flow (prot)	0	3383	0	0	3420	0	0	1642	0	0	1669	0
FIt Permitted					0.954			0.870			0.969	
Satd. Flow (perm)	0	3383	0	0	3263	0	0	1450	0	0	1623	0
Satd. Flow (RTOR)		1						37			12	
Lane Group Flow (vph)	0	1049	0	0	1831	0	0	79	0	0	42	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		Max		Max	Max		None	None		None	None	
Act Effct Green (s)		61.7			61.7			10.3			10.3	
Actuated g/C Ratio		0.78			0.78			0.13			0.13	
v/c Ratio		0.40			0.72			0.36			0.19	
Control Delay		4.5			8.4			24.0			25.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		4.5			8.4			24.0			25.9	
LOS		Α			A			C			C	
Approach Delay		4.5			8.4			24.0			25.9	
Approach LOS		Α			A			С			C	
Queue Length 50th (m)		28.1			77.2			5.9			4.2	
Queue Length 95th (m)		40.4			113.8			18.3			13.1	
Internal Link Dist (m)		151.1			288.5			38.4			167.1	
Turn Bay Length (m)		2624			2540			E22			E76	
Base Capacity (vph)		2634			2540			532			576	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn Reduced v/c Ratio		0.40			0.72			0.15			0.07	
Neudoed V/C Ratio		0.40			0.72			0.10			0.07	

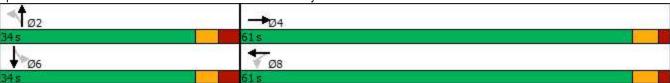
Cycle Length: 95

Actuated Cycle Length: 79.3

Natural Cycle: 90 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.72



Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



# 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		7	<b>†</b>	7	7	<b>†</b>	7	7	f)	
Traffic Volume (vph)	10	598	88	94	784	121	88	185	84	59	68	11
Future Volume (vph)	10	598	88	94	784	121	88	185	84	59	68	11
Satd. Flow (prot)	1555	1717	0	1710	1782	1530	1710	1800	1485	1710	1745	0
Flt Permitted	0.254			0.311			0.702			0.509		
Satd. Flow (perm)	416	1717	0	553	1782	1455	1212	1800	1388	887	1745	0
Satd. Flow (RTOR)		14				108			79		8	
Lane Group Flow (vph)	11	722	0	99	825	127	93	195	88	62	84	0
Turn Type	Perm	NA		Perm	NA	Free	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		Free	2		2	6		
Detector Phase	4	4		8	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5		31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0		32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%		32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	-2.4	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5		6.4	6.4	4.0	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None	None	None	None	
Act Effct Green (s)	69.1	69.1		69.1	69.1	100.0	18.0	18.0	20.4	18.0	18.0	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	1.00	0.18	0.18	0.20	0.18	0.18	
v/c Ratio	0.04	0.61		0.26	0.67	0.09	0.43	0.60	0.26	0.39	0.26	
Control Delay	10.8	21.4		9.5	13.8	0.1	40.6	44.5	10.0	41.1	31.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.8	21.4		9.5	13.8	0.1	40.6	44.5	10.0	41.1	31.7	
LOS	В	С		Α	В	Α	D	D	В	D	С	
Approach Delay		21.2			11.8			35.5			35.7	
Approach LOS		С			В			D		44.0	D	
Queue Length 50th (m)	1.0	127.9		6.2	78.8	0.0	17.5	37.9	1.5	11.6	13.8	
Queue Length 95th (m)	m2.2	m172.2		18.5	160.8	0.0	30.1	55.2	13.2	22.5	25.0	
Internal Link Dist (m)		635.7			497.2			83.8			171.5	
Turn Bay Length (m)	45.0			50.0		40.0	50.0		20.0	45.0		
Base Capacity (vph)	287	1190		382	1231	1455	310	460	445	227	452	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.04	0.61		0.26	0.67	0.09	0.30	0.42	0.20	0.27	0.19	

Intersection Summary

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 65 (65%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

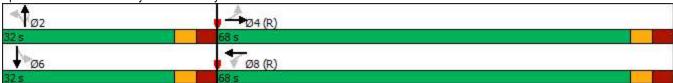
Natural Cycle: 80

### 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

Maximum v/c Ratio: 0.67
Intersection Signal Delay: 20.2
Intersection Capacity Utilization 97.5%
ICU Level of Service F
Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



	•	<b>→</b>	•	•	←	•	4	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b></b>	7	ች	<b>↑</b>	7	ሻ	₽		*	₽	
Traffic Volume (vph)	227	516	86	56	658	63	40	546	48	58	233	210
Future Volume (vph)	227	516	86	56	658	63	40	546	48	58	233	210
Satd. Flow (prot)	1693	1765	1515	1710	1800	1530	1710	1755	0	1710	1565	0
Flt Permitted	0.117			0.461			0.290			0.119		
Satd. Flow (perm)	209	1765	1240	771	1800	1222	507	1755	0	208	1565	0
Satd. Flow (RTOR)			73			119		5			53	
Lane Group Flow (vph)	239	543	91	59	693	66	42	626	0	61	466	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	10.6	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	15.0	49.0	49.0	34.0	34.0	34.0	45.0	45.0		45.0	45.0	
Total Split (%)	15.0%	49.0%	49.0%	34.0%	34.0%	34.0%	45.0%	45.0%		45.0%	45.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.3	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	-2.1	0.0	-2.1	-2.1	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.6	6.1	4.0	6.1	4.0	4.0	6.3	6.3		6.3	6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		Min	Min	
Act Effct Green (s)	50.6	50.1	52.2	28.4	30.5	30.5	37.5	37.5		37.5	37.5	
Actuated g/C Ratio	0.51	0.50	0.52	0.28	0.30	0.30	0.38	0.38		0.38	0.38	
v/c Ratio	0.70	0.61	0.13	0.27	1.26	0.15	0.22	0.95		0.78	0.75	
Control Delay	32.2	22.2	4.8	32.0	159.0	3.1	24.4	55.0		87.8	32.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	32.2	22.2	4.8	32.0	159.0	3.1	24.4	55.0		87.8	32.5	
LOS	С	С	Α	С	F	Α	С	Е		F	С	
Approach Delay		23.1			137.3			53.1			38.9	
Approach LOS		С			F			D			D	
Queue Length 50th (m)	30.2	78.7	1.8	6.6	~182.8	0.1	5.6	118.6		10.6	71.3	
Queue Length 95th (m)	#64.3	115.3	9.7	m13.0	#250.2	m1.8	14.4	#190.2		#35.8	111.4	
Internal Link Dist (m)		92.9			635.7			70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0	222	
Base Capacity (vph)	343	884	682	219	550	455	196	682		80	638	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.70	0.61	0.13	0.27	1.26	0.15	0.21	0.92		0.76	0.73	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 8 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 120

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	<b>J</b>	
Detector Phase		
Switch Phase		
Minimum Initial (s)	4.0	
Minimum Split (s)	6.0	
Total Split (s)	6.0	
	6%	
Total Split (%)	2.0	
Yellow Time (s)		
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

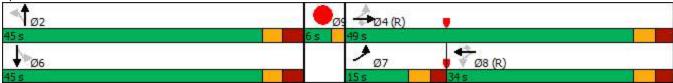
Maximum v/c Ratio: 1.26
Intersection Signal Delay: 65.3
Intersection Capacity Utilization 110.5%
ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	-	•	•	←	4	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		7				
Traffic Volume (vph)	0	0	0	0	0	468
Future Volume (vph)	0	0	0	0	0	468
Satd. Flow (prot)	0	1800	0	0	0	0
Flt Permitted						
Satd. Flow (perm)	0	1800	0	0	0	0
Lane Group Flow (vph)	0	0	0	0	493	0
Sign Control	Free			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion Err%			IC	U Level c	f Service I
Analysis Period (min) 15						

	۶	•	1	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	¥		ች	<b>^</b>	<b>†</b> 1>		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Traffic Volume (vph)	1	8	8	1117	1074	10		
Future Volume (vph)	1	8	8	1117	1074	10		
Satd. Flow (prot)	1554	0	1710	3420	3417	0		
Flt Permitted	0.994		0.245					
Satd. Flow (perm)	1552	0	441	3420	3417	0		
Satd. Flow (RTOR)	8				1			
Lane Group Flow (vph)	9	0	8	1176	1142	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	10.0		10.0	10.0	10.0		22.0	
Minimum Split (s)	17.0		30.9	30.9	30.9		29.0	
Total Split (s)	17.0		46.0	46.0	46.0		29.0	
Total Split (%)	18.5%		50.0%	50.0%	50.0%		32%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.9	5.9	5.9			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	10.0		87.6	87.6	87.6			
Actuated g/C Ratio	0.11		0.95	0.95	0.95			
v/c Ratio	0.05		0.02	0.36	0.35			
Control Delay	23.0		1.6	1.4	1.4			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	23.0		1.6	1.4	1.4			
LOS	С		Α	Α	Α			
Approach Delay	23.0			1.4	1.4			
Approach LOS	С			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	4.8		1.3	43.3	41.4			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	189		420	3256	3253			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.05		0.02	0.36	0.35			
Internation Comment								

Cycle Length: 92

Actuated Cycle Length: 92

Offset: 10 (11%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 80

Maximum v/c Ratio: 0.36									
Intersection Signal Delay: 1.5	Intersection LOS: A								
Intersection Capacity Utilization 51.0%	ICU Level of Service A	ICU Level of Service A							
Analysis Period (min) 15									
Splits and Phases: 19: Booth St & War Museum	<b>≯</b> <sub>04</sub>	<b>O</b> Ø9							
46 s	17 s	29 s							
▼ Ø6 (R)									
46 s									

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ň	<b>†</b>	7	¥	f)			4	
Traffic Volume (vph)	9	626	76	84	861	28	57	4	80	42	0	8
Future Volume (vph)	9	626	76	84	861	28	57	4	80	42	0	8
Satd. Flow (prot)	1710	1782	1530	1693	1765	1530	1644	1500	0	0	1690	0
Flt Permitted	0.211			0.414			0.788				0.698	
Satd. Flow (perm)	380	1782	1530	738	1765	1474	1364	1500	0	0	1230	0
Satd. Flow (RTOR)			79			87		84			87	
Lane Group Flow (vph)	9	659	80	88	906	29	60	88	0	0	52	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	16.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.3	0.0	0.0	-2.3	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	4.0	6.3	6.3	4.0	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	95.3	95.3	97.6	92.0	92.0	94.3	12.1	12.1			12.1	
Actuated g/C Ratio	0.79	0.79	0.81	0.77	0.77	0.79	0.10	0.10			0.10	
v/c Ratio	0.02	0.47	0.06	0.16	0.67	0.02	0.44	0.39			0.26	
Control Delay	3.2	5.5	0.8	3.5	9.6	0.1	60.2	16.3			5.9	
Queue Delay	0.0	0.1	0.0	0.0	0.2	0.0	0.0	0.0			0.0	
Total Delay	3.2	5.6	0.8	3.5	9.8	0.1	60.2	16.4			5.9	
LOS	Α	Α	Α	Α	Α	Α	Е	В			Α	
Approach Delay		5.1			9.0			34.1			5.9	
Approach LOS		Α			Α			С			Α	
Queue Length 50th (m)	0.4	41.2	0.0	3.1	34.1	0.0	14.4	0.9			0.0	
Queue Length 95th (m)	1.7	73.8	3.2	6.9	272.2	m0.0	27.8	16.4			4.0	
Internal Link Dist (m)		497.2			110.2			178.8			36.6	
Turn Bay Length (m)	30.0		50.0	35.0		50.0	30.0					
Base Capacity (vph)	420	1415	1259	566	1353	1177	337	434			369	
Starvation Cap Reductn	0	0	0	0	59	0	0	0			0	
Spillback Cap Reductn	0	95	0	0	0	0	0	4			4	
Storage Cap Reductn	0	0	0	0	0	0	0	0			0	
Reduced v/c Ratio	0.02	0.50	0.06	0.16	0.70	0.02	0.18	0.20			0.14	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

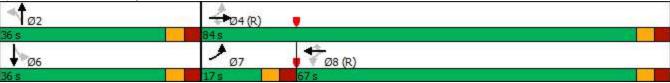
Natural Cycle: 100

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 9.3	Intersection LOS: A	
Intersection Capacity Utilization 81.6%	ICU Level of Service D	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: City Centre Ave & Albert St



	•	<b>→</b>	<b>←</b>	•	<b>/</b>	✓
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	<b>†</b>	<b>∱</b> ∱		¥	
Traffic Volume (vph)	3	1139	1117	9	11	3
Future Volume (vph)	3	1139	1117	9	11	3
Satd. Flow (prot)	1710	1765	3383	0	1685	0
Flt Permitted	0.232				0.962	
Satd. Flow (perm)	418	1765	3383	0	1685	0
Satd. Flow (RTOR)			2		3	
Lane Group Flow (vph)	3	1199	1185	0	15	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases		4	8		6	
Permitted Phases	4					
Detector Phase	4	4	8		6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	22.0	22.0	22.0		20.0	
Total Split (s)	90.0	90.0	90.0		30.0	
Total Split (%)	75.0%	75.0%	75.0%		25.0%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.7	1.7	1.7		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	5.0	5.0	5.0		5.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Min		None	
Act Effct Green (s)	111.6	111.6	111.6		10.4	
Actuated g/C Ratio	0.93	0.93	0.93		0.09	
v/c Ratio	0.01	0.73	0.38		0.10	
Control Delay	3.0	12.6	1.5		44.7	
Queue Delay	0.0	0.4	0.0		0.0	
Total Delay	3.0	13.0	1.5		44.7	
LOS	A	В	Α		D	
Approach Delay		13.0	1.5		44.7	
Approach LOS		В	Α		D	
Queue Length 50th (m)	0.0	113.1	0.0		2.8	
Queue Length 95th (m)	m0.3	m218.6	m20.3		9.8	
Internal Link Dist (m)		122.5	138.6		61.4	
Turn Bay Length (m)	30.0					
Base Capacity (vph)	389	1641	3146		353	
Starvation Cap Reductn	0	112	213		0	
Spillback Cap Reductn	0	119	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.01	0.79	0.40		0.04	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 7.5	Intersection LOS: A
Intersection Capacity Utilization 79.9%	ICU Level of Service D
–	

Analysis Period (min) 15

Volume for 95th percentile queue is metered by upstream signal.

21: Albert St & Access 1 Splits and Phases:



	-	•	•	•	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		ች	<b>^</b>	¥	
Traffic Volume (vph)	1080	32	60	1230	39	56
Future Volume (vph)	1080	32	60	1230	39	56
Satd. Flow (prot)	3363	0	1710	3386	1586	0
Flt Permitted			0.190		0.980	
Satd. Flow (perm)	3363	0	342	3386	1585	0
Satd. Flow (RTOR)	4				56	
Lane Group Flow (vph)	1171	0	63	1295	100	0
Turn Type	NA		pm+pt	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases			8			
Detector Phase	4		3	8	2	
Switch Phase						
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	30.0		14.0	22.0	33.8	
Total Split (s)	66.0		20.0	86.0	34.0	
Total Split (%)	55.0%		16.7%	71.7%	28.3%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	1.9		1.9	1.9	2.5	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.6		5.6	5.6	5.8	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	84.8		94.8	94.8	13.8	
Actuated g/C Ratio	0.71		0.79	0.79	0.12	
v/c Ratio	0.49		0.18	0.48	0.43	
Control Delay	9.5		5.1	5.9	28.1	
Queue Delay	0.5		0.0	1.0	0.0	
Total Delay	9.9		5.1	6.8	28.2	
LOS	A		A	A	C	
Approach Delay	9.9			6.7	28.2	
Approach LOS	A			A	C	
Queue Length 50th (m)	35.8		2.3	37.7	10.5	
Queue Length 95th (m)	m84.2		9.8	99.9	23.8	
Internal Link Dist (m)	117.1		0.0	245.9	41.8	
Turn Bay Length (m)			70.0			
Base Capacity (vph)	2378		434	2674	415	
Starvation Cap Reductn	666		0	0	0	
Spillback Cap Reductn	0		0	1014	16	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.68		0.15	0.78	0.25	
	0.00					

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 23 (19%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 80

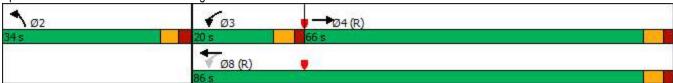
Maximum v/c Ratio: 0.49

Intersection Signal Delay: 9.0 Intersection LOS: A Intersection Capacity Utilization 61.6% ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	۶	-	$\rightarrow$	•	←	•	1	<b>†</b>	<b>/</b>	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>∱</b> ∱			414			4			4	
Traffic Volume (vph)	43	697	16	11	1052	15	9	0	14	43	0	11
Future Volume (vph)	43	697	16	11	1052	15	9	0	14	43	0	11
Satd. Flow (prot)	1710	3344	0	0	3377	0	0	1619	0	0	1683	0
Flt Permitted	0.212				0.945			0.893			0.755	
Satd. Flow (perm)	382	3344	0	0	3194	0	0	1472	0	0	1321	0
Satd. Flow (RTOR)		4			2			56			56	
Lane Group Flow (vph)	45	751	0	0	1135	0	0	24	0	0	57	0
Turn Type	pm+pt	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	9.6	22.0		22.0	22.0		30.5	30.5		30.5	30.5	
Total Split (s)	15.0	80.0		65.0	65.0		40.0	40.0		40.0	40.0	
Total Split (%)	12.5%	66.7%		54.2%	54.2%		33.3%	33.3%		33.3%	33.3%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.6	4.6			4.6			4.3			4.3	
Lead/Lag	Lead			Lag	Lag							
Lead-Lag Optimize?	Yes			Yes	Yes							
Recall Mode	None	C-Min		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	101.0	101.9			93.1			13.0			13.0	
Actuated g/C Ratio	0.84	0.85			0.78			0.11			0.11	
v/c Ratio	0.12	0.26			0.46			0.11			0.30	
Control Delay	2.3	1.7			7.7			1.7			15.6	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	2.3	1.7			7.7			1.7			15.6	
LOS	A	Α			Α			Α			В	
Approach Delay		1.7			7.7			1.7			15.6	
Approach LOS		Α			Α			Α			В	
Queue Length 50th (m)	0.7	10.2			48.3			0.0			0.2	
Queue Length 95th (m)	m2.7	20.3			101.4			1.0			11.8	
Internal Link Dist (m)		82.0			212.6			72.6			45.7	
Turn Bay Length (m)	75.0	2222			2.4=2						100	
Base Capacity (vph)	436	2839			2479			477			432	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.10	0.26			0.46			0.05			0.13	

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

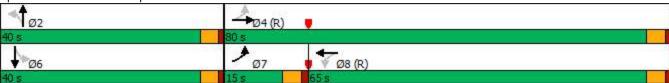
Natural Cycle: 75

Maximum v/c Ratio: 0.46

maximum v/o radio. o. ro		
Intersection Signal Delay: 5.5	Intersection LOS: A	
Intersection Capacity Utilization 55.1%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	•	<b>→</b>	•	•	•	•	•	<b>†</b>	~	<b>\</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ħβ			<b>∱</b> }				7			7
Traffic Volume (vph)	0	713	0	0	1083	14	0	0	0	0	0	11
Future Volume (vph)	0	713	0	0	1083	14	0	0	0	0	0	11
Satd. Flow (prot)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Flt Permitted												
Satd. Flow (perm)	0	3420	0	0	3413	0	0	0	1800	0	0	1557
Lane Group Flow (vph)	0	751	0	0	1155	0	0	0	0	0	0	12
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	n 42.1%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

	۶	<b>→</b>	•	•	•	•	•	<b>†</b>	~	<b>\</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		ħβ			ħβ	
Traffic Volume (vph)	0	0	0	0	0	10	0	1209	23	0	746	0
Future Volume (vph)	0	0	0	0	0	10	0	1209	23	0	746	0
Satd. Flow (prot)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Flt Permitted												
Satd. Flow (perm)	0	0	1800	0	0	1557	0	3410	0	0	3420	0
Lane Group Flow (vph)	0	0	0	0	0	11	0	1297	0	0	785	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 46.0%			IC	U Level o	of Service	Α					
Analysis Period (min) 15												

	-	•	•	<b>←</b>	1	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>∱</b> Ъ			<b>^</b>		7	
Traffic Volume (vph)	910	0	0	1552	0	18	
Future Volume (vph)	910	0	0	1552	0	18	
Satd. Flow (prot)	3420	0	0	3420	0	1557	
Flt Permitted							
Satd. Flow (perm)	3420	0	0	3420	0	1557	
Lane Group Flow (vph)	958	0	0	1634	0	19	
Sign Control	Free			Free	Stop		
Intersection Summary							
Control Type: Unsignalized	d						
Intersection Capacity Utilization 48.6%				ICU Level of Service A			
Analysis Period (min) 15							

	-	<b>→</b> ✓	•	1	/
Lane Group	EBT	EBR WBI	WBT	NBL	NBR
Lane Configurations	<b>†</b>	1		¥	
Traffic Volume (vph)	892	4 50		6	18
Future Volume (vph)	892	4 50		6	18
Satd. Flow (prot)	3417	0 1710		1595	0
Flt Permitted		0.27		0.988	
Satd. Flow (perm)	3417	0 488		1595	0
Satd. Flow (RTOR)	1		0.20	19	
Lane Group Flow (vph)	943	0 56	5 1578	25	0
Turn Type	NA	pm+p		Prot	
Protected Phases	4		3 8	2	
Permitted Phases	т т		3	L	
Detector Phase	4		8 8	2	
Switch Phase			,	L	
Minimum Initial (s)	10.0	5.0	10.0	10.0	
Minimum Split (s)	37.0	11.0		32.3	
Total Split (s)	72.0	15.0		33.0	
Total Split (%)	60.0%	12.5%		27.5%	
Yellow Time (s)	3.7	3.7		3.3	
All-Red Time (s)	2.3	2.3		3.0	
Lost Time Adjust (s)	-2.0	-2.0		-2.3	
	4.0	-2.0 4.0		4.0	
Total Lost Time (s)				4.0	
Lead/Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes		Ness	
Recall Mode	C-Max	None		None	
Act Effct Green (s)	97.3	106.2		12.3	
Actuated g/C Ratio	0.81	0.88		0.10	
v/c Ratio	0.34	0.1		0.14	
Control Delay	4.0	3.5		26.1	
Queue Delay	0.1	0.0		0.0	
Total Delay	4.1	3.5		26.1	
LOS	A	ŀ		С	
Approach Delay	4.1		6.3	26.1	
Approach LOS	A		Α	С	
Queue Length 50th (m)	29.0	4.0		1.4	
Queue Length 95th (m)	33.8	m4.6	m108.2	10.2	
Internal Link Dist (m)	71.8		49.0	28.0	
Turn Bay Length (m)		60.0			
Base Capacity (vph)	2769	543		399	
Starvation Cap Reductn	408		742	0	
Spillback Cap Reductn	0		0	0	
Storage Cap Reductn	0	(		0	
Reduced v/c Ratio	0.40	0.10	0.68	0.06	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 85

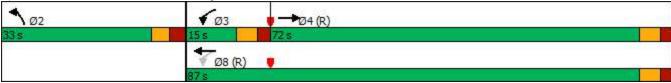
Maximum v/c Ratio: 0.51

Intersection Signal Delay: 5.7 Intersection LOS: A Intersection Capacity Utilization 58.7% ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 27: Broad St & Wellington St



	-	•	•	<b>←</b>	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> }			<b>^</b>		7
Traffic Volume (vph)	878	4	0	1502	0	18
Future Volume (vph)	878	4	0	1502	0	18
Satd. Flow (prot)	3417	0	0	3420	0	1557
FIt Permitted						
Satd. Flow (perm)	3417	0	0	3420	0	1557
Lane Group Flow (vph)	928	0	0	1581	0	19
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 47.2%			IC	U Level o	of Service A
Analysis Period (min) 15						

Synchro Modelling Outputs – Phase Three (2050) Conditions



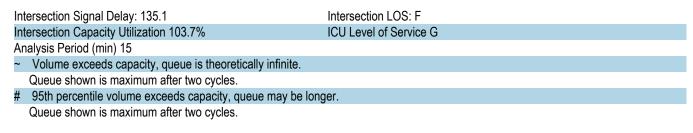
	•	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b>	<b>^</b>	77	ሻሻ	7
Traffic Volume (vph)	605	1043	439	704	1962	320
Future Volume (vph)	605	1043	439	704	1962	320
Satd. Flow (prot)	3317	4914	3420	2693	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3317	4914	3420	2693	3285	1530
Satd. Flow (RTOR)						71
Lane Group Flow (vph)	637	1098	462	741	2065	337
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	18	1	. 91111
Permitted Phases	-	<b>T</b>		1 0		1
Detector Phase	7	4	8	18	1	1
Switch Phase	1	4	U	10		
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
( )	42.8	16.5	26.5		45.1	45.1
Minimum Split (s)						
Total Split (s)	42.8	70.3	26.5		51.1	51.1
Total Split (%)	35.3%	57.9%	21.8%		42.1%	42.1%
Yellow Time (s)	3.3	3.3	3.3		3.3	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		-2.1	0.0
Total Lost Time (s)	5.8	6.5	6.5		4.0	6.1
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	Max	Min	Min		Max	Max
Act Effct Green (s)	37.0	62.6	19.8	71.3	47.1	45.0
Actuated g/C Ratio	0.31	0.52	0.16	0.59	0.39	0.37
v/c Ratio	0.62	0.43	0.82	0.46	1.60	0.55
Control Delay	38.9	18.4	61.5	14.9	304.0	26.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.9	18.4	61.5	14.9	304.0	26.9
LOS	D	В	E	В	F	C
Approach Delay		25.9	32.8		265.1	
Approach LOS		23.3 C	02.0 C		200.1	
Queue Length 50th (m)	69.9	59.4	59.0	55.4	~379.4	51.4
	90.1	70.9	#83.3	71.7		82.1
Queue Length 95th (m)	90.1	246.9	#os.s 299.2	11.1	39.9	02.1
Internal Link Dist (m)	150.0	240.9	233.2		39.9	20.0
Turn Bay Length (m)	150.0	0007	F07	1407	1007	30.0
Base Capacity (vph)	1021	2607	597	1487	1287	617
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.42	0.77	0.50	1.60	0.55

Cycle Length: 121.4

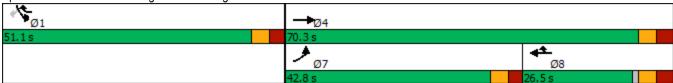
Actuated Cycle Length: 120.2

Natural Cycle: 145

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.60



Splits and Phases: 7: Wellington St & Portage



	ၨ	<b>-</b>	←	•	<b>\</b>	4
						-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<b>•</b>	<b>↑</b>	7		7
Traffic Volume (vph)	0	1016	497	0	0	5
Future Volume (vph)	0	1016	497	0	0	5
Satd. Flow (prot)	0	1765	1765	1765	0	1526
Flt Permitted						
Satd. Flow (perm)	0	1765	1765	1765	0	1526
Lane Group Flow (vph)	0	1069	523	0	0	5
Sign Control		Free	Free		Stop	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization	on 59.8%			IC	U Level c	of Service I
Analysis Period (min) 15						

	۶	-	$\rightarrow$	•	←	•	1	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		ሻ	f)		ሻ	f.		ሻ	f.	
Traffic Volume (vph)	5	0	5	0	0	0	37	832	0	0	1472	37
Future Volume (vph)	5	0	5	0	0	0	37	832	0	0	1472	37
Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1793	0
Flt Permitted							0.066					
Satd. Flow (perm)	1800	1530	0	1800	1800	0	119	1800	0	1800	1793	0
Satd. Flow (RTOR)		42									3	
Lane Group Flow (vph)	5	5	0	0	0	0	39	876	0	0	1588	0
Turn Type	Perm	NA		Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		35.9	35.9		35.9	35.9	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.9	5.9		5.9	5.9	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)	10.0	10.0					70.6	70.6			70.6	
Actuated g/C Ratio	0.13	0.13					0.94	0.94			0.94	
v/c Ratio	0.02	0.02					0.35	0.52			0.94	
Control Delay	28.6	0.2					15.8	3.4			18.7	
Queue Delay	0.0	0.0					0.0	0.0			0.0	
Total Delay	28.6	0.2					15.8	3.4			18.7	
LOS	С	Α					В	Α			В	
Approach Delay		14.4						4.0			18.7	
Approach LOS		В						Α			В	
Queue Length 50th (m)	0.7	0.0					0.0	0.0			0.0	
Queue Length 95th (m)	3.6	0.0					#18.0	99.6			#385.5	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)	30.0						30.0					
Base Capacity (vph)	379	355					112	1694			1687	
Starvation Cap Reductn	0	0					0	0			0	
Spillback Cap Reductn	0	0					0	0			0	
Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio	0.01	0.01					0.35	0.52			0.94	

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 150

Maximum v/c Ratio: 0.94
Intersection Signal Delay: 13.3
Intersection LOS: B
Intersection Capacity Utilization 102.6%
ICU Level of Service G
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere

ø <sub>2 (R)</sub>		
53 s	22 s	
● Ø6 (R)	<b>★</b> Ø8	
53 s	22 s	

# 11: Booth St & Sir John A. Macdonald Pkwy/Wellington St

	<i>→</i> –	• •	•	•	•	1	<b>†</b>	_	-	<b>↓</b>	4
Lane Group	EBL EB	T EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<b>↑</b>	<b>†</b>		<b>^</b>	7		<b>∱</b> %		*	<b>^</b>	7
Traffic Volume (vph)	0 130		0	757	132	0	838	131	146	1000	233
Future Volume (vph)	0 130	4 0	0	757	132	0	838	131	146	1000	233
Satd. Flow (prot)	0 342	0 0	0	3386	1530	0	3127	0	1710	3226	1530
Flt Permitted									0.114		
Satd. Flow (perm)	0 342	0 0	0	3386	1530	0	3127	0	205	3226	1530
Satd. Flow (RTOR)							19				95
Lane Group Flow (vph)	0 137	3 0	0	797	139	0	1020	0	154	1053	245
Turn Type	N	A		NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4		8			2		1	6	
Permitted Phases					8				6		6
Detector Phase		4		8	8		2		1	6	6
Switch Phase											
Minimum Initial (s)	10			10.0	10.0		10.0		5.2	10.0	10.0
Minimum Split (s)	35			35.8	35.8		31.9		12.0	37.8	37.8
Total Split (s)	48			48.0	48.0		35.0		12.0	47.0	47.0
Total Split (%)	50.5			50.5%	50.5%		36.8%		12.6%	49.5%	49.5%
Yellow Time (s)	3			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)	3			3.1	3.1		3.5		3.5	3.5	3.5
Lost Time Adjust (s)	0			0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6	8		6.8	6.8		6.8		6.8	6.8	6.8
Lead/Lag							Lag		Lead		
Lead-Lag Optimize?							Yes		Yes		
Recall Mode	M			Min	Min		C-Max		None	C-Min	C-Min
Act Effct Green (s)	40			40.6	40.6		28.2		40.8	40.8	40.8
Actuated g/C Ratio	0.4			0.43	0.43		0.30		0.43	0.43	0.43
v/c Ratio	0.9			0.55	0.21		1.08		0.86	0.76	0.35
Control Delay	55			28.5	22.3		87.5		61.3	26.0	11.9
Queue Delay	30			0.8	0.0		7.2		68.2	0.0	0.0
Total Delay	85			29.3	22.3		94.6		129.5	26.0	11.9
LOS	0.5	F		С	С		F		F	C	В
Approach Delay	85			28.2			94.6			34.6	
Approach LOS	450	F		C	00.0		F		40.4	С	40.0
Queue Length 50th (m)	152			74.3	20.8		~115.8		18.4	89.2	18.0
Queue Length 95th (m)	#184			97.9	37.4		#157.0		#54.7	67.7	27.8
Internal Link Dist (m)	67	5		116.7	70.0		55.5		4.45.0	192.9	0
Turn Bay Length (m)	4.40	•		4.400	70.0		0.14		145.0	1001	55.0
Base Capacity (vph)	148			1468	663		941		179	1384	710
Starvation Cap Reductn	19			366	0		0		0	0	0
Spillback Cap Reductn	6	5		0	0		112		95	0	0
Storage Cap Reductn		0		0	0		0		0	0	0
Reduced v/c Ratio	1.0	6		0.72	0.21		1.23		1.83	0.76	0.35

Intersection Summary

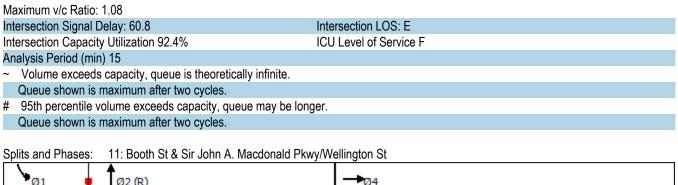
Cycle Length: 95

Actuated Cycle Length: 95

Offset: 31 (33%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 100

## 11: Booth St & Sir John A. Macdonald Pkwy/Wellington St



Ø6 (R) Ø8

	•	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>†</b>	7	ሻ	<b>^</b>	7		€î∌		ሻ	<b>†</b>	7
Traffic Volume (vph)	396	864	24	15	336	155	21	540	45	174	488	446
Future Volume (vph)	396	864	24	15	336	155	21	540	45	174	488	446
Satd. Flow (prot)	1660	1748	1443	1710	3196	1378	0	3314	0	1425	1782	1471
Flt Permitted	0.389			0.131				0.919		0.173		
Satd. Flow (perm)	652	1748	1377	236	3196	1251	0	3046	0	251	1782	1313
Satd. Flow (RTOR)			91					7				
Lane Group Flow (vph)	417	909	25	16	354	163	0	637	0	183	514	469
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		5.0	10.0	10.0
Minimum Split (s)	11.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.5	34.5	34.5
Total Split (s)	18.0	55.0	55.0	37.0	37.0	37.0	40.0	40.0		25.0	65.0	65.0
Total Split (%)	15.0%	45.8%	45.8%	30.8%	30.8%	30.8%	33.3%	33.3%		20.8%	54.2%	54.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	0.0	0.0	-2.5	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	4.0	6.5	6.5	6.5		6.5		6.5	6.5	6.5
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	Max	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	None
Act Effct Green (s)	55.8	55.8	58.3	30.5	30.5	30.5		29.4		51.2	51.2	51.2
Actuated g/C Ratio	0.46	0.46	0.49	0.25	0.25	0.25		0.24		0.43	0.43	0.43
v/c Ratio	0.90	1.12	0.03	0.27	0.44	0.51		0.85		0.71	0.68	0.84
Control Delay	53.3	101.4	0.1	53.9	41.6	47.4		53.8		38.2	31.9	44.2
Queue Delay	0.0	0.9	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	53.3	102.3	0.1	53.9	41.6	47.4		53.8		38.2	31.9	44.2
LOS	D	F	Α	D	D	D		D		D	С	D
Approach Delay		85.3			43.7			53.8			37.9	
Approach LOS		F			D			D			D	
Queue Length 50th (m)	73.3	~263.0	0.0	3.0	34.7	30.6		78.2		28.8	99.2	100.2
Queue Length 95th (m)	#186.2	#366.1	0.0	10.5	54.7	55.9		97.5		44.7	125.0	135.6
Internal Link Dist (m)		141.9			62.1			37.2			83.1	
Turn Bay Length (m)	160.0		40.0	40.0								120.0
Base Capacity (vph)	461	812	715	59	812	317		855		288	868	640
Starvation Cap Reductn	0	116	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	0.90	1.31	0.03	0.27	0.44	0.51		0.75		0.64	0.59	0.73

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Maximum v/c Ratio: 1.12
Intersection Signal Delay: 58.9
Intersection Capacity Utilization 128.2%
ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 12: Booth St & Albert St



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	<b>†</b>	7	J.	<b>↑</b> 1>			4	7		4	
Traffic Volume (vph)	5	917	136	398	405	16	112	8	306	29	12	2
Future Volume (vph)	5	917	136	398	405	16	112	8	306	29	12	2
Satd. Flow (prot)	1710	1748	1471	1629	3271	0	0	1599	1471	0	1728	0
Flt Permitted	0.495			0.062				0.706			0.724	
Satd. Flow (perm)	891	1748	1384	106	3271	0	0	1168	1417	0	1294	0
Satd. Flow (RTOR)			86		8				322		2	
Lane Group Flow (vph)	5	965	143	419	443	0	0	126	322	0	46	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	65.0	65.0	65.0	25.0	90.0		30.0	30.0	30.0	30.0	30.0	
Total Split (%)	54.2%	54.2%	54.2%	20.8%	75.0%		25.0%	25.0%	25.0%	25.0%	25.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	-2.8	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	4.0	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	Min	C-Min		None	None	None	None	None	
Act Effct Green (s)	58.2	61.0	61.0	89.8	89.2			17.7	17.7		17.6	
Actuated g/C Ratio	0.48	0.51	0.51	0.75	0.74			0.15	0.15		0.15	
v/c Ratio	0.01	1.09	0.19	1.06	0.18			0.73	0.67		0.24	
Control Delay	19.4	80.7	7.8	99.5	5.1			72.2	11.9		44.5	
Queue Delay	0.0	3.5	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	19.4	84.2	7.8	99.5	5.1			72.2	11.9		44.5	
LOS	В	F	А	F	Α			E	В		D	
Approach Delay		74.1			51.0			28.9			44.5	
Approach LOS	0.4	E	4.0	404.0	D			C	0.0		D	
Queue Length 50th (m)	0.4	~269.5	4.9	~101.2	14.8			30.1	0.0		9.7	
Queue Length 95th (m)	m1.1	#347.8	m18.5	#186.2	24.4			49.6	26.9		20.4	
Internal Link Dist (m)	20.0	80.9	40.0	00.0	119.1			54.7			65.4	
Turn Bay Length (m)	30.0	000	16.0	90.0	0404			020	<b>F</b> 20		057	
Base Capacity (vph)	432	888	745	394	2434			230	538		257	
Starvation Cap Reducts	0	7	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0 01	1 10	0 10	1.06	0 10			0 55	0.60		0 10	
Reduced v/c Ratio	0.01	1.10	0.19	1.06	0.18			0.55	0.60		0.18	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 55 (46%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

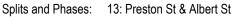
Natural Cycle: 150

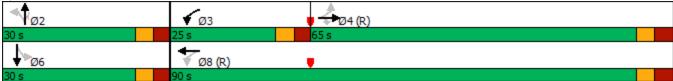
Maximum v/c Ratio: 1.09
Intersection Signal Delay: 57.3
Intersection LOS: E
Intersection Capacity Utilization 99.8%
ICU Level of Service F
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





# 14: Preston St/Vimy PI & Sir John A. Macdonald Pkwy

	۶	-	$\rightarrow$	•	←	•	4	<b>†</b>	<b>/</b>	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> }		ች	<b>↑</b> Ъ			4			4	
Traffic Volume (vph)	15	1361	12	63	897	41	8	0	28	2	0	15
Future Volume (vph)	15	1361	12	63	897	41	8	0	28	2	0	15
Satd. Flow (prot)	1710	3417	0	1710	3364	0	0	1591	0	0	1482	0
Flt Permitted	0.285			0.165				0.920			0.954	
Satd. Flow (perm)	513	3417	0	297	3364	0	0	1480	0	0	1423	0
Satd. Flow (RTOR)		2			8			34			34	
Lane Group Flow (vph)	16	1446	0	66	987	0	0	37	0	0	18	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		37.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	62.0	62.0		62.0	62.0		33.0	33.0		33.0	33.0	
Total Split (%)	65.3%	65.3%		65.3%	65.3%		34.7%	34.7%		34.7%	34.7%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	81.6	81.6		81.6	81.6			10.0			10.0	
Actuated g/C Ratio	0.86	0.86		0.86	0.86			0.11			0.11	
v/c Ratio	0.04	0.49		0.26	0.34			0.20			0.10	
Control Delay	2.9	3.6		8.0	3.6			17.7			7.4	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	2.9	3.6		8.0	3.6			17.7			7.4	
LOS	А	Α		Α	Α			В			A	
Approach Delay		3.6			3.9			17.7			7.4	
Approach LOS		A			A			В			Α	
Queue Length 50th (m)	0.6	47.8		2.2	17.4			0.5			0.0	
Queue Length 95th (m)	m1.2	60.7		11.7	43.2			10.0			3.7	
Internal Link Dist (m)	40.0	651.1		=0.0	64.4			41.8			21.1	
Turn Bay Length (m)	40.0	2222		50.0	2221						10.1	
Base Capacity (vph)	440	2936		255	2891			440			424	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.04	0.49		0.26	0.34			0.08			0.04	

Intersection Summary

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 59 (62%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

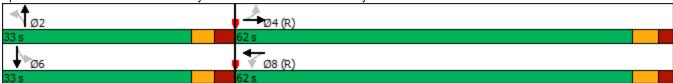
Natural Cycle: 80

## 14: Preston St/Vimy PI & Sir John A. Macdonald Pkwy

Maximum v/c Ratio: 0.49
Intersection Signal Delay: 4.0
Intersection Capacity Utilization 72.0%
Intersection Capacity Utilization 72.0%
ICU Level of Service C
Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 14: Preston St/Vimy Pl & Sir John A. Macdonald Pkwy



	۶	<b>→</b>	$\searrow$	•	←	•	4	<b>†</b>	<b>/</b>	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>			<b>1</b>			f)	
Traffic Volume (vph)	0	1482	22	1	1015	1	1	21	1	1	5	3
Future Volume (vph)	0	1482	22	1	1015	1	1	21	1	1	5	3
Satd. Flow (prot)	0	3413	0	0	3386	0	0	1786	0	0	1709	0
Flt Permitted					0.954			0.984			0.956	
Satd. Flow (perm)	0	3413	0	0	3230	0	0	1761	0	0	1643	0
Satd. Flow (RTOR)		3						1			3	
Lane Group Flow (vph)	0	1583	0	0	1070	0	0	24	0	0	9	0
Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases				8			2			6		
Detector Phase		4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)		30.5		30.5	30.5		33.3	33.3		33.3	33.3	
Total Split (s)		61.0		61.0	61.0		34.0	34.0		34.0	34.0	
Total Split (%)		64.2%		64.2%	64.2%		35.8%	35.8%		35.8%	35.8%	
Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)		1.8		1.8	1.8		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			6.3			6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode		C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)		86.3			86.3			10.0			10.0	
Actuated g/C Ratio		0.91			0.91			0.11			0.11	
v/c Ratio		0.51			0.36			0.13			0.05	
Control Delay		3.0			2.2			39.3			33.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		3.0			2.2			39.3			33.4	
LOS		Α			Α			D			С	
Approach Delay		3.0			2.2			39.3			33.4	
Approach LOS		Α			Α			D			С	
Queue Length 50th (m)		0.0			0.1			4.1			1.1	
Queue Length 95th (m)		69.2			53.5			12.1			5.7	
Internal Link Dist (m)		354.8			288.5			38.4			167.1	
Turn Bay Length (m)												
Base Capacity (vph)		3100			2933			514			481	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.51			0.36			0.05			0.02	

Cycle Length: 95

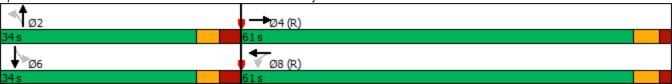
Actuated Cycle Length: 95

Offset: 34 (36%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 80

Maximum v/c Ratio: 0.51
Intersection Signal Delay: 3.1
Intersection Capacity Utilization 62.1%
Analysis Period (min) 15
Intersection LOS: A
ICU Level of Service B

Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	¥	ĵ»		J.	<b>†</b>	7	¥	<b>†</b>	7	¥	f)	
Traffic Volume (vph)	14	783	86	56	347	31	61	45	130	131	178	12
Future Volume (vph)	14	783	86	56	347	31	61	45	130	131	178	12
Satd. Flow (prot)	1500	1733	0	1613	1698	1457	1676	1800	1515	1693	1756	0
Flt Permitted	0.533			0.205			0.499			0.726		
Satd. Flow (perm)	842	1733	0	348	1698	1457	881	1800	1515	1294	1756	0
Satd. Flow (RTOR)		10				37			137		3	
Lane Group Flow (vph)	15	915	0	59	365	33	64	47	137	138	200	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.5	6.5		6.5	6.5	6.5	6.4	6.4	6.4	6.4	6.4	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	68.9	68.9		68.9	68.9	68.9	18.2	18.2	18.2	18.2	18.2	
Actuated g/C Ratio	0.69	0.69		0.69	0.69	0.69	0.18	0.18	0.18	0.18	0.18	
v/c Ratio	0.03	0.76		0.25	0.31	0.03	0.40	0.14	0.35	0.59	0.62	
Control Delay	3.1	10.8		10.9	8.0	2.3	41.4	32.5	8.0	46.5	44.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	3.1	10.8		10.9	8.0	2.3	41.4	32.5	8.0	46.5	44.7	
LOS	A	B		В	A	Α	D	C	Α	D	D	
Approach Delay		10.7			8.0			21.3			45.4	
Approach LOS	0.0	10.0		2.7	A 24.2	0.0	11.0	С	0.0	20.0	D	
Queue Length 50th (m)	0.2	12.8		3.7		0.0	11.9	8.3	0.0	26.6	38.2	
Queue Length 95th (m)	m0.6	#231.8		13.3	50.1	3.2	23.2	16.9	14.9	42.7	56.3	
Internal Link Dist (m)	4E 0	635.7		E0.0	497.2	40.0	50.0	83.8	20.0	4E 0	171.5	
Turn Bay Length (m)	45.0 580	1197		50.0 240	1170	40.0 1015	225	460	20.0 489	45.0 331	451	
Base Capacity (vph)												
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn Storage Cap Reductn	0	0					0			0	0	
Reduced v/c Ratio	0.03			0.25	0.31	0.03	0.28	0 10	0.28	0 0.42	0.44	
Neudled V/C Natio	0.03	0.76		0.23	0.31	0.03	0.20	0.10	0.20	0.42	0.44	

Cycle Length: 100 Actuated Cycle Length: 100

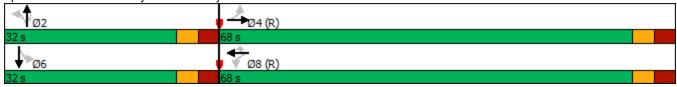
Offset: 40 (40%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

## 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

Maximum v/c Ratio: 0.76
Intersection Signal Delay: 17.4
Intersection LOS: B
Intersection Capacity Utilization 84.2%
ICU Level of Service E
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



	•	<b>→</b>	•	•	•	•	4	<b>†</b>	<b>/</b>	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>†</b>	7	7	<b>†</b>	7	7	£		7	eĵ.	
Traffic Volume (vph)	120	699	37	50	311	24	31	281	69	97	354	145
Future Volume (vph)	120	699	37	50	311	24	31	281	69	97	354	145
Satd. Flow (prot)	1676	1748	1485	1676	1698	1471	1660	1722	0	1693	1621	0
Flt Permitted	0.510			0.175			0.215			0.394		
Satd. Flow (perm)	900	1748	1485	309	1698	1471	376	1722	0	702	1621	0
Satd. Flow (RTOR)			58			58		14			23	
Lane Group Flow (vph)	126	736	39	53	327	25	33	369	0	102	526	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	28.1	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	43.0	43.0		43.0	43.0	
Total Split (%)	51.0%	51.0%	51.0%	51.0%	51.0%	51.0%	43.0%	43.0%		43.0%	43.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.8	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.1	6.1	6.1	6.1	6.1	6.1	6.3	6.3		6.3	6.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max		Max	Max	
Act Effct Green (s)	50.9	50.9	50.9	50.9	50.9	50.9	36.7	36.7		36.7	36.7	
Actuated g/C Ratio	0.51	0.51	0.51	0.51	0.51	0.51	0.37	0.37		0.37	0.37	
v/c Ratio	0.28	0.83	0.05	0.34	0.38	0.03	0.24	0.58		0.40	0.86	
Control Delay	16.1	30.7	1.9	18.3	14.1	0.3	27.7	28.7		29.3	44.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	16.1	30.7	1.9	18.3	14.1	0.3	27.7	28.7		29.3	44.3	
LOS	В	С	Α	В	В	Α	С	С		С	D	
Approach Delay		27.4			13.8			28.7			41.8	
Approach LOS		С			В			С			D	
Queue Length 50th (m)	14.0	121.9	0.0	7.3	46.0	0.1	4.6	57.4		15.2	94.3	
Queue Length 95th (m)	26.8	#185.4	3.2	18.8	28.6	m0.3	13.2	87.8		31.3	#156.3	
Internal Link Dist (m)	_	92.9			635.7		_	70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	458	889	784	157	864	777	137	640		257	609	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.28	0.83	0.05	0.34	0.38	0.03	0.24	0.58		0.40	0.86	

Cycle Length: 100 Actuated Cycle Length: 100

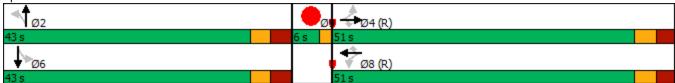
Offset: 91 (91%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Lane Configurations  Traffic Volume (vph)  Future Volume (vph)  Satd. Flow (prot)  Fit Permitted  Satd. Flow (perm)  Satd. Flow (RTOR)  Lane Group Flow (vph)  Turn Type  Protected Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (%)  Yellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Tum Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio  Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Satd. Flow (prot) FIt Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vph) Satd. Flow (prot) Fit Permitted Satd. Flow (prom) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Solution Split (s) So				
Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Source of time (s) 1.0 Lost Time (s) 1.0 Lost Time (s) 1.0 Lost Time Adjust (s) Total Lost Time (s) 1.0 Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvatino Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Ype Internal Park (m) Internal Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Sellow Time (s) All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Solution Split (s				
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Minimum				
Protected Phases  Permitted Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (s)  Total Split (%)  Sylellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (yph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Cotal Split (%) Sellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lost Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 55th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		9		
Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Minimum Split (s) 6.0  Total Split (s) 6.0  Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio	Switch Phase			
Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Minimum Initial (s)	2.0		
Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		6.0		
Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio		3.0		
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS  Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		None		
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		NOTIC		
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	. ,			
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	•			
LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)			
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary				
	Intersection Summary			

Maximum v/c Ratio: 0.86
Intersection Signal Delay: 29.1 Intersection LOS: C
Intersection Capacity Utilization 105.2% ICU Level of Service G
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



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Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	SWL	SWR	
Lane Configurations		<b>†</b>			<b>†</b>			7			
Traffic Volume (vph)	0	65	140	0	385	0	0	0	0	0	
Future Volume (vph)	0	65	140	0	385	0	0	0	0	0	
Satd. Flow (prot)	0	1602	0	0	1765	0	0	1765	0	0	
Flt Permitted											
Satd. Flow (perm)	0	1602	0	0	1765	0	0	1765	0	0	
Lane Group Flow (vph)	0	215	0	0	405	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilizati	ion 31.4%			IC	U Level c	of Service	Α				
Analysis Period (min) 15											

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	W		ሻ	<b>^</b>	<b>†</b> Þ			_
Traffic Volume (vph)	1	1	2	868	1476	6		
Future Volume (vph)	1	1	2	868	1476	6		
Satd. Flow (prot)	1637	0	1710	3420	3417	0		
Flt Permitted	0.976		0.152					
Satd. Flow (perm)	1637	0	274	3420	3417	0		
Satd. Flow (RTOR)	1				1			
Lane Group Flow (vph)	2	0	2	914	1560	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	8.3		10.0	10.0	10.0		10.0	
Minimum Split (s)	14.5		30.7	30.7	30.9		29.0	
Total Split (s)	17.0		49.0	49.0	49.0		29.0	
Total Split (%)	17.9%		51.6%	51.6%	51.6%		31%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.4	2.4	2.6		4.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.2		5.7	5.7	5.9			
Lead/Lag Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	8.3		91.0	91.0	90.9		INUITE	
Actuated g/C Ratio	0.09		0.96	0.96	0.96			
v/c Ratio	0.03		0.01	0.30	0.48			
Control Delay	34.5		0.5	0.20	1.7			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	34.5		0.5	0.3	1.7			
LOS	C C		A	A	Α			
Approach Delay	34.5		, ,	0.3	1.7			
Approach LOS	C			A	A			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	2.4		m0.0	m4.9	61.5			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	186		262	3274	3270			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.01		0.01	0.28	0.48			
Intersection Summary								
Cycle Length: 95								

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 47 (49%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

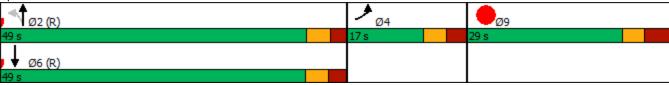
Natural Cycle: 90

Maximum v/c Ratio: 0.48

maximum v/o reaco. o. ro		
Intersection Signal Delay: 1.2	Intersection LOS: A	
Intersection Capacity Utilization 60.3%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 19: Booth St & War Museum



	•	-	•	•	•	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	7	<b>†</b>	7	7	£			4	
Traffic Volume (vph)	5	984	77	98	395	14	28	0	47	35	0	5
Future Volume (vph)	5	984	77	98	395	14	28	0	47	35	0	5
Satd. Flow (prot)	1710	1782	1485	1629	1714	1530	1644	1361	0	0	1697	0
Flt Permitted	0.469			0.234			0.730				0.716	
Satd. Flow (perm)	844	1782	1485	401	1714	1530	1263	1361	0	0	1257	0
Satd. Flow (RTOR)			48			87		132			87	
Lane Group Flow (vph)	5	1036	81	103	416	15	29	49	0	0	42	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			0.0	
Total Lost Time (s)	6.3	6.3	6.3	6.3	6.3	6.3	6.3	6.3			6.3	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	98.1	99.3	99.3	96.9	96.9	96.9	12.6	12.6			12.6	
Actuated g/C Ratio	0.82	0.83	0.83	0.81	0.81	0.81	0.10	0.10			0.10	
v/c Ratio	0.01	0.70	0.07	0.32	0.30	0.01	0.22	0.19			0.20	
Control Delay	4.0	10.3	2.0	14.2	9.1	0.8	51.0	1.6			2.3	
Queue Delay	0.0	1.4	0.0	0.0	0.0	0.0	0.0	0.0			0.0	
Total Delay	4.0	11.8	2.0	14.2	9.1	0.8	51.0	1.6			2.4	
LOS	А	В	Α	В	Α	Α	D	Α			Α	
Approach Delay		11.0			9.9			20.0			2.4	
Approach LOS		В			Α			В			Α	
Queue Length 50th (m)	0.3	91.5	1.3	8.3	45.5	0.0	6.9	0.0			0.0	
Queue Length 95th (m)	1.5	237.9	6.9	36.8	99.1	m0.4	15.0	0.0			0.6	
Internal Link Dist (m)		497.2			112.4			178.8			41.9	
Turn Bay Length (m)	30.0		50.0	35.0		50.0	30.0					
Base Capacity (vph)	766	1475	1237	323	1384	1252	312	436			376	
Starvation Cap Reductn	0	0	0	0	0	0	0	0			0	
Spillback Cap Reductn	0	247	0	0	0	0	0	18			12	
Storage Cap Reductn	0	0	0	0	0	0	0	0			0	
Reduced v/c Ratio	0.01	0.84	0.07	0.32	0.30	0.01	0.09	0.12			0.12	

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

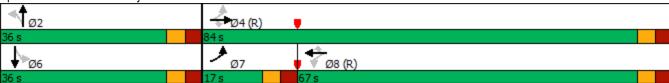
Natural Cycle: 90

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 10.9	Intersection LOS: B	
Intersection Capacity Utilization 88.4%	ICU Level of Service E	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: City Centre Ave & Albert St



	•	<b>→</b>	<b>+</b>	•	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	<b></b>	<b>↑</b> ↑		W	
Traffic Volume (vph)	29	1212	740	61	47	15
Future Volume (vph)	29	1212	740	61	47	15
Satd. Flow (prot)	1710	1748	3177	0	1672	0
Flt Permitted	0.329				0.964	
Satd. Flow (perm)	592	1748	3177	0	1655	0
Satd. Flow (RTOR)			17		14	
Lane Group Flow (vph)	31	1276	843	0	65	0
Turn Type	Perm	NA	NA		Prot	
Protected Phases		4	8		6	
Permitted Phases	4					
Detector Phase	4	4	8		6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	22.0	22.0	22.0		28.7	
Total Split (s)	80.0	80.0	80.0		30.0	
Total Split (%)	72.7%	72.7%	72.7%		27.3%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.4	1.4	1.4		1.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	4.7	4.7	4.7		4.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	C-Max	C-Max	C-Min		None	
Act Effct Green (s)	92.2	92.2	92.2		12.9	
Actuated g/C Ratio	0.84	0.84	0.84		0.12	
v/c Ratio	0.06	0.87	0.32		0.31	
Control Delay	3.6	17.2	3.4		38.2	
Queue Delay	0.0	15.9	0.2		0.0	
Total Delay	3.6	33.1	3.6		38.2	
LOS	Α	С	Α		D	
Approach Delay		32.4	3.6		38.2	
Approach LOS		С	Α		D	
Queue Length 50th (m)	0.9	134.0	16.6		11.0	
Queue Length 95th (m)	5.1	#394.4	45.0		21.5	
Internal Link Dist (m)		119.1	141.9		77.8	
Turn Bay Length (m)	30.0					
Base Capacity (vph)	496	1464	2665		405	
Starvation Cap Reductn	0	209	996		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.06	1.02	0.51		0.16	

Cycle Length: 110 Actuated Cycle Length: 110

Offset: 22 (20%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 120

Maximum v/c Ratio: 0.87 Intersection LOS: C Intersection Signal Delay: 21.6 Intersection Capacity Utilization 83.3% ICU Level of Service E Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 21: Albert St & Access 1 <sup>9</sup>Ø4 (R)

	-	*	€	•	1		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>∱</b> 1>			<b>^</b>	W		
Traffic Volume (vph)	1465	16	28	824	39	89	
Future Volume (vph)	1465	16	28	824	39	89	
Satd. Flow (prot)	3368	0	1710	3320	1537	0	
Flt Permitted		C	).124		0.985		
Satd. Flow (perm)	3368	0	223	3320	1537	0	
Satd. Flow (RTOR)	2				30		
Lane Group Flow (vph)	1559	0	29	867	135	0	
Turn Type	NA	F	Perm	NA	Prot		
Protected Phases	4			8	2		
Permitted Phases			8				
Detector Phase	4		8	8	2		
Switch Phase							
Minimum Initial (s)	10.0		10.0	10.0	10.0		
Minimum Split (s)	30.0		22.0	22.0	33.9		
Total Split (s)	61.0		61.0	61.0	34.0		
Total Split (%)	64.2%	64	4.2%	64.2%	35.8%		
Yellow Time (s)	3.7		3.7	3.7	3.3		
All-Red Time (s)	2.1		2.1	2.1	2.6		
Lost Time Adjust (s)	0.0		0.0	0.0	0.0		
Total Lost Time (s)	5.8		5.8	5.8	5.9		
Lead/Lag							
Lead-Lag Optimize?							
Recall Mode	C-Max	C-	-Max	C-Max	None		
Act Effct Green (s)	68.4		68.4	68.4	14.9		
Actuated g/C Ratio	0.72		0.72	0.72	0.16		
v/c Ratio	0.64		0.18	0.36	0.51		
Control Delay	19.7		10.0	6.5	33.5		
Queue Delay	15.1		0.0	0.0	0.0		
Total Delay	34.9		10.0	6.5	33.5		
LOS	С		В	Α	С		
Approach Delay	34.9			6.6	33.5		
Approach LOS	С			Α	С		
Queue Length 50th (m)	124.4		1.3	24.3	19.3		
Queue Length 95th (m)	m150.0		8.7	60.5	31.1		
Internal Link Dist (m)	116.7			246.9	48.8		
Turn Bay Length (m)			70.0				
Base Capacity (vph)	2427		160	2391	475		
Starvation Cap Reductn	886		0	0	0		
Spillback Cap Reductn	0		0	0	0		
Storage Cap Reductn	0		0	0	0		
Reduced v/c Ratio	1.01		0.18	0.36	0.28		

Cycle Length: 95

Actuated Cycle Length: 95

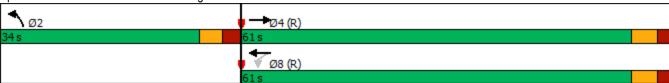
Offset: 27 (28%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 75

Maximum v/c Ratio: 0.64		
Intersection Signal Delay: 25.0	Intersection LOS: C	
Intersection Capacity Utilization 61.4%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>∱</b> ⊅			<b>€</b> 1₽			4			4	
Traffic Volume (vph)	19	1061	8	5	454	7	8	0	11	28	0	7 7
Future Volume (vph)	19	1061	8	5	454	7	8	0	11	28	0	
Satd. Flow (prot)	1710	3318	0	0	3313	0	0	1621	0	0	1685	0
Flt Permitted	0.473				0.946			0.884			0.754	
Satd. Flow (perm)	851	3318	0	0	3137	0	0	1462	0	0	1322	0
Satd. Flow (RTOR)		1			2			15			15	
Lane Group Flow (vph)	20	1125	0	0	490	0	0	20	0	0	36	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.6	22.6		22.6	22.6		30.5	30.5		30.5	30.5	
Total Split (s)	81.0	81.0		81.0	81.0		39.0	39.0		39.0	39.0	
Total Split (%)	67.5%	67.5%		67.5%	67.5%		32.5%	32.5%		32.5%	32.5%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.6	4.6			4.6			4.3			4.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	105.7	105.7			105.7			13.0			13.0	
Actuated g/C Ratio	0.88	0.88			0.88			0.11			0.11	
v/c Ratio	0.03	0.39			0.18			0.12			0.23	
Control Delay	1.8	3.2			2.5			24.4			34.3	
Queue Delay	0.0	0.4			0.0			0.0			0.0	
Total Delay	1.8	3.6			2.5			24.4			34.3	
LOS	Α	Α			Α			С			С	
Approach Delay		3.5			2.5			24.4			34.3	
Approach LOS		Α			Α			С			С	
Queue Length 50th (m)	0.2	13.9			8.7			1.2			4.9	
Queue Length 95th (m)	m1.2	m80.2			25.1			7.9			13.7	
Internal Link Dist (m)		82.5			218.4			58.0			17.2	
Turn Bay Length (m)												
Base Capacity (vph)	749	2921			2762			433			392	
Starvation Cap Reductn	0	1108			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.03	0.62			0.18			0.05			0.09	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 60

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 4.1	Intersection LOS: A	
Intersection Capacity Utilization 47.0%	ICU Level of Service A	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	ၨ	<b>-</b>	←	•	<b>\</b>	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		<b>^</b>	<b>∱</b> ∱			7	
Traffic Volume (vph)	0	1039	440	7	0	7	
Future Volume (vph)	0	1039	440	7	0	7	
Satd. Flow (prot)	0	3353	3346	0	0	1526	
Flt Permitted							
Satd. Flow (perm)	0	3353	3346	0	0	1526	
Lane Group Flow (vph)	0	1094	470	0	0	7	
Sign Control		Free	Free		Stop		
Intersection Summary							
Control Type: Unsignalized							
Intersection Capacity Utilization 33.6%				ICU Level of Service A			
Analysis Period (min) 15							

	•	<b>→</b>	•	•	•	•	•	<b>†</b>	~	<b>\</b>	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		<b>↑</b> ↑			<b>↑</b> ↑	
Traffic Volume (vph)	0	0	0	0	0	12	0	857	17	0	966	0
Future Volume (vph)	0	0	0	0	0	12	0	857	17	0	966	0
Satd. Flow (prot)	0	0	1765	0	0	1526	0	3343	0	0	3353	0
Flt Permitted												
Satd. Flow (perm)	0	0	1765	0	0	1526	0	3343	0	0	3353	0
Lane Group Flow (vph)	0	0	0	0	0	13	0	920	0	0	1017	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 35.6%			IC	U Level o	of Service	Α					
Analysis Period (min) 15												

	<b>→</b>	•	•	•	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b> β			<b>^</b>		7
Traffic Volume (vph)	1286	0	0	910	0	13
Future Volume (vph)	1286	0	0	910	0	13
Satd. Flow (prot)	3353	0	0	3353	0	1526
Flt Permitted						
Satd. Flow (perm)	3353	0	0	3353	0	1526
Lane Group Flow (vph)	1354	0	0	958	0	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 47.5%			IC	U Level o	of Service
Analysis Period (min) 15						

	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		ሻ	<b>^</b>	¥	
Traffic Volume (vph)	1297	5	58	932	11	46
Future Volume (vph)	1297	5	58	932	11	46
Satd. Flow (prot)	3417	0	1710	3420	1590	0
Flt Permitted			0.138		0.990	
Satd. Flow (perm)	3417	0	248	3420	1590	0
Satd. Flow (RTOR)					48	
Lane Group Flow (vph)	1370	0	61	981	60	0
Turn Type	NA		pm+pt	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases	•		8			
Detector Phase	4		3	8	2	
Switch Phase	•					
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	37.0		11.0	37.0	32.3	
Total Split (s)	47.0		15.0	62.0	33.0	
Total Split (%)	49.5%		15.8%	65.3%	34.7%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	2.3		2.3	2.3	3.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	6.0		6.0	6.0	6.3	
Lead/Lag	Lag		Lead	0.0	0.0	
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	67.2		76.0	77.2	10.0	
Actuated g/C Ratio	0.71		0.80	0.81	0.11	
v/c Ratio	0.57		0.00	0.35	0.11	
Control Delay	12.6		3.7	6.4	19.4	
Queue Delay	0.2		0.0	0.4	10.6	
Total Delay	12.8		3.7	6.6	30.0	
LOS	12.0 B		3. <i>1</i>	0.0 A	30.0 C	
Approach Delay	12.8		Λ.	6.5	30.0	
Approach LOS	12.0 B			0.5 A	30.0 C	
Queue Length 50th (m)	100.6		2.1	80.6	2.1	
Queue Length 95th (m)	138.3					
Internal Link Dist (m)	77.2		m3.6	97.5 49.9	14.0 32.0	
Turn Bay Length (m)	11.2		60.0	43.3	32.0	
Base Capacity (vph)	2415		336	2778	481	
Starvation Cap Reductn	327			980		
Spillback Cap Reductn	327		0		0 390	
			0	0		
Storage Cap Reductn	0 66		0 10	0.55	0	
Reduced v/c Ratio	0.66		0.18	0.55	0.66	

Cycle Length: 95

Actuated Cycle Length: 95

Offset: 22 (23%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

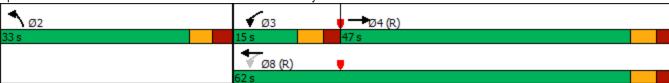
Natural Cycle: 85

Maximum v/c Ratio: 0.57

Intersection Signal Delay: 10.6	Intersection LOS: B	
Intersection Capacity Utilization 65.8%	ICU Level of Service C	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 27: Broad St & Sir John A. Macdonald Pkwy



	-	•	•	•	4	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>↑</b> 1>			<b>^</b>		7
Traffic Volume (vph)	1261	2	0	886	0	13
Future Volume (vph)	1261	2	0	886	0	13
Satd. Flow (prot)	3353	0	0	3353	0	1526
Flt Permitted						
Satd. Flow (perm)	3353	0	0	3353	0	1526
Lane Group Flow (vph)	1329	0	0	933	0	14
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalized	d					
Intersection Capacity Utiliz	ation 46.9%			IC	U Level c	of Service
Analysis Period (min) 15						

	>	-	7	*	<b>←</b>	*_	<b>\</b>	1	7	/	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SER	NEL	NER	
Lane Configurations		<b>†</b> †			<b>^</b>					7	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Satd. Flow (prot)	0	3353	0	0	3353	0	0	0	0	1765	
Flt Permitted											
Satd. Flow (perm)	0	3353	0	0	3353	0	0	0	0	1765	
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilization	n 13.3%			IC	U Level o	of Service	Α				
Analysis Period (min) 15											

	ሻ	Æ	$\mathbf{x}$	7	€	×
Lane Group	NBL	NBR	SET	SER	NWL	NWT
Lane Configurations			<b>↑</b>			<b>†</b>
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0
Satd. Flow (prot)	0	0	1765	0	0	1765
Flt Permitted						
Satd. Flow (perm)	0	0	1765	0	0	1765
Lane Group Flow (vph)	0	0	0	0	0	0
Sign Control	Free		Free			Free
Intersection Summary						
Control Type: Unsignalized			_	_		
Intersection Capacity Utiliza	tion 31.4%			IC	U Level o	of Service
Analysis Period (min) 15						

	_#	<b>→</b>	<b>-</b> *	4	←	€_	4	₹	4	1	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NWL	NWR	SWL	SWR	
Lane Configurations		<b>^</b>			44				ሻ	•	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Satd. Flow (prot)	0	3353	0	0	3353	0	0	0	1765	0	
FIt Permitted											
Satd. Flow (perm)	0	3353	0	0	3353	0	0	0	1765	0	
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilization	on 13.3%			IC	U Level o	of Service	Α				
Analysis Period (min) 15											

	۶	<b>→</b>	•	4	<b>/</b>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻሻ	<b>^</b> ^	<b>^</b>	11	ሻሻ	7
Traffic Volume (vph)	811	504	854	1105	852	466
Future Volume (vph)	811	504	854	1105	852	466
Satd. Flow (prot)	3317	4914	3386	2614	3285	1530
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	3317	4914	3386	2614	3285	1530
Satd. Flow (RTOR)	• • • • • • • • • • • • • • • • • • • •				0200	189
Lane Group Flow (vph)	854	531	899	1163	897	491
Turn Type	Prot	NA	NA	pt+ov	Prot	Perm
Protected Phases	7	4	8	81	1	. 51111
Permitted Phases	-	-т		- U		1
Detector Phase	7	4	8	81	1	1
Switch Phase	-	7	- 0	0 1		
Minimum Initial (s)	10.0	10.0	10.0		10.0	10.0
Minimum Split (s)	42.8	26.5	26.5		44.1	44.1
Total Split (s)	55.8	92.3	36.5		44.1	44.1
Total Split (%)	40.9%	67.7%	26.8%		32.3%	32.3%
Yellow Time (s)	3.3	3.3	3.3		32.3%	3.3
All-Red Time (s)	2.5	3.2	3.2		2.8	2.8
. ,	-1.8	-2.5	-2.5		-2.1	-2.1
Lost Time Adjust (s)	4.0	-2.5 4.0	-2.5 4.0		-2.1 4.0	-2.1 4.0
Total Lost Time (s)		4.0			4.0	4.0
Lead/Lag	Lead Yes		Lag Yes			
Lead-Lag Optimize?	Max	Min			None	Mono
Recall Mode		Min	Min	76.6	None	None
Act Effct Green (s)	51.8	88.3	32.5	76.6	40.1	40.1
Actuated g/C Ratio	0.38	0.65	0.24	0.56	0.29	0.29
v/c Ratio	0.68	0.17	1.12	0.79	0.93	0.84
Control Delay	38.7	9.7	115.5	28.7	63.2	41.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.7	9.7	115.5	28.7	63.2	41.6
LOS	D	Α	F	С	E	D
Approach Delay		27.5	66.5		55.6	
Approach LOS		С	E		Е	_
Queue Length 50th (m)	103.6	20.4	~153.5	142.5	127.6	85.5
Queue Length 95th (m)	127.3	25.8	#196.3	177.3	#167.5	#147.2
Internal Link Dist (m)		245.9	286.9		42.1	
Turn Bay Length (m)	150.0					30.0
Base Capacity (vph)	1259	3181	806	1467	965	583
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.17	1.12	0.79	0.93	0.84

Cycle Length: 136.4 Actuated Cycle Length: 136.4

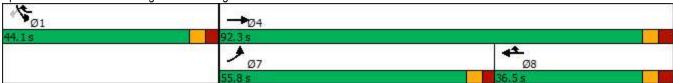
Natural Cycle: 125 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.12

Intersection Signal Delay: 52.2 Intersection LOS: D
Intersection Capacity Utilization 85.0% ICU Level of Service E
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 7: Wellington St & Portage



	ၨ	<b>→</b>	<b>←</b>	•	<b>\</b>	4
Lana Craun	EDI	FDT	WDT	WDD	CDI	SBR
Lane Group	EBL	EBT	WBT	WBR	SBL	SDK
Lane Configurations				7		7
Traffic Volume (vph)	0	838	1085	0	0	7
Future Volume (vph)	0	838	1085	0	0	7
Satd. Flow (prot)	0	1765	1765	1765	0	1526
Flt Permitted						
Satd. Flow (perm)	0	1765	1765	1765	0	1526
Lane Group Flow (vph)	0	882	1142	0	0	7
Sign Control		Free	Free		Stop	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization	n 70.3%			IC	U Level c	f Service (
Analysis Period (min) 15						

	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	$\blacktriangleleft$	<b>†</b>	/	<b>&gt;</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	£		ሻ	₽		7	<b>₽</b>		7	1>	
Traffic Volume (vph)	33	0	33	0	0	0	7	1118	0	0	1068	7
Future Volume (vph)	33	0	33	0	0	0	7	1118	0	0	1068	7
Satd. Flow (prot)	1710	1530	0	1800	1800	0	1710	1800	0	1800	1798	0
Flt Permitted	0.757						0.160					
Satd. Flow (perm)	1363	1530	0	1800	1800	0	288	1800	0	1800	1798	0
Satd. Flow (RTOR)		112									1	
Lane Group Flow (vph)	35	35	0	0	0	0	7	1177	0	0	1131	0
Turn Type	Perm	NA		Perm			Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.0	22.0		22.0	22.0		35.9	35.9		35.9	35.9	
Total Split (s)	22.0	22.0		22.0	22.0		53.0	53.0		53.0	53.0	
Total Split (%)	29.3%	29.3%		29.3%	29.3%		70.7%	70.7%		70.7%	70.7%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2		2.6	2.6		2.6	2.6	
Lost Time Adjust (s)	-2.2	-2.2		-2.2	-2.2		-1.9	-1.9		-1.9	-1.9	
Total Lost Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Act Effct Green (s)	12.2	12.2					62.9	62.9			62.9	
Actuated g/C Ratio	0.16	0.16					0.84	0.84			0.84	
v/c Ratio	0.16	0.10					0.03	0.78			0.75	
Control Delay	29.1	0.6					3.1	11.8			10.5	
Queue Delay	0.0	0.0					0.0	0.0			0.0	
Total Delay	29.1	0.6					3.1	11.8			10.5	
LOS	С	Α					Α	В			В	
Approach Delay		14.8						11.8			10.5	
Approach LOS		В						В			В	
Queue Length 50th (m)	4.5	0.0					0.2	107.6			96.6	
Queue Length 95th (m)	12.4	0.0					1.2	#238.3			#223.7	
Internal Link Dist (m)		60.2			43.3			76.9			31.3	
Turn Bay Length (m)	30.0						30.0					
Base Capacity (vph)	327	452					241	1508			1507	
Starvation Cap Reductn	0	0					0	0			0	
Spillback Cap Reductn	0	0					0	0			0	
Storage Cap Reductn	0	0					0	0			0	
Reduced v/c Ratio	0.11	0.08					0.03	0.78			0.75	
lt												

Cycle Length: 75

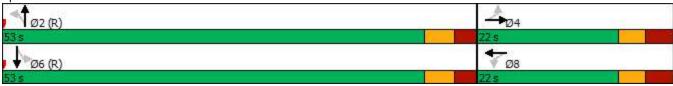
Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Maximum v/c Ratio: 0.78
Intersection Signal Delay: 11.3
Intersection LOS: B
Intersection Capacity Utilization 77.1%
ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 10: Booth St & Chaudiere



## 11: Booth St & Sir John A. Macdonald Pkwy/Wellington St

	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	-	ţ	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>			<b>^</b>	7		<b>∱</b> ∱		ሻ	<b>^</b>	7
Traffic Volume (vph)	0	987	0	0	1432	117	0	989	176	110	667	232
Future Volume (vph)	0	987	0	0	1432	117	0	989	176	110	667	232
Satd. Flow (prot)	0	3420	0	0	3420	1530	0	3244	0	1613	3196	1515
Flt Permitted										0.084		
Satd. Flow (perm)	0	3420	0	0	3420	1530	0	3244	0	143	3196	1515
Satd. Flow (RTOR)								19				35
Lane Group Flow (vph)	0	1039	0	0	1507	123	0	1226	0	116	702	244
Turn Type		NA			NA	Perm		NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases						8				6		6
Minimum Split (s)		35.8			37.4	37.4		31.8		12.0	31.9	31.9
Total Split (s)		60.0			60.0	60.0		48.0		12.0	60.0	60.0
Total Split (%)		50.0%			50.0%	50.0%		40.0%		10.0%	50.0%	50.0%
Yellow Time (s)		3.7			3.7	3.7		3.3		3.3	3.3	3.3
All-Red Time (s)		3.1			3.1	3.1		3.5		3.5	3.6	3.6
Lost Time Adjust (s)		0.0			0.0	-2.8		-2.1		0.0	0.0	-2.9
Total Lost Time (s)		6.8			6.8	4.0		4.7		6.8	6.9	4.0
Lead/Lag								Lag		Lead		
Lead-Lag Optimize?								Yes		Yes		
Act Effct Green (s)		53.2			53.2	56.0		43.3		53.2	53.1	56.0
Actuated g/C Ratio		0.44			0.44	0.47		0.36		0.44	0.44	0.47
v/c Ratio		0.69			0.99	0.17		1.04		0.91	0.50	0.34
Control Delay		23.5			58.6	21.8		56.7		85.6	25.4	18.7
Queue Delay		0.6			38.0	0.0		0.0		0.0	0.0	0.0
Total Delay		24.1			96.7	21.8		56.7		85.6	25.4	18.7
LOS		С			F	С		Е		F	С	В
Approach Delay		24.1			91.0			56.7			30.4	
Approach LOS		С			F			Е			С	
Queue Length 50th (m)		113.4			207.8	20.6		~98.2		16.9	64.1	31.6
Queue Length 95th (m)		140.5			#253.4	35.5		m#80.3		#51.6	82.1	51.6
Internal Link Dist (m)		72.0			117.1			52.2			192.9	
Turn Bay Length (m)						70.0				145.0		55.0
Base Capacity (vph)		1516			1516	714		1182		127	1414	725
Starvation Cap Reductn		176			275	0		0		0	0	0
Spillback Cap Reductn		0			0	0		0		0	0	0
Storage Cap Reductn		0			0	0		0		0	0	0
Reduced v/c Ratio		0.78			1.21	0.17		1.04		0.91	0.50	0.34

### Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 3 (3%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 115 Control Type: Pretimed Maximum v/c Ratio: 1.04 Intersection Signal Delay: 55.5

Intersection LOS: E ICU Level of Service F

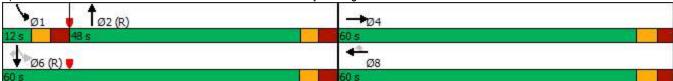
Analysis Period (min) 15

Intersection Capacity Utilization 98.2%

### 11: Booth St & Sir John A. Macdonald Pkwy/Wellington St

- ~ Volume exceeds capacity, queue is theoretically infinite.
  - Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
  - Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 11: Booth St & Sir John A. Macdonald Pkwy/Wellington St



	٠	<b>→</b>	•	•	←	*	1	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	ሻ	<b>^</b>	7		4Te		ሻ	<b></b>	7
Traffic Volume (vph)	558	657	45	33	909	239	25	540	67	98	428	257
Future Volume (vph)	558	657	45	33	909	239	25	540	67	98	428	257
Satd. Flow (prot)	1676	1765	1485	1710	3386	1530	0	3277	0	1629	1782	1515
Flt Permitted	0.098			0.402				0.903		0.158		
Satd. Flow (perm)	173	1765	1451	721	3386	1360	0	2961	0	264	1782	1381
Satd. Flow (RTOR)			91					10				
Lane Group Flow (vph)	587	692	47	35	957	252	0	665	0	103	451	271
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm
Protected Phases	7	4			8			2		1	6	
Permitted Phases	4		4	8		8	2			6		6
Detector Phase	7	4	4	8	8	8	2	2		1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		4.5	10.0	10.0
Minimum Split (s)	16.5	36.5	36.5	36.5	36.5	36.5	34.5	34.5		11.0	34.5	34.5
Total Split (s)	29.0	70.0	70.0	41.0	41.0	41.0	38.0	38.0		12.0	50.0	50.0
Total Split (%)	24.2%	58.3%	58.3%	34.2%	34.2%	34.2%	31.7%	31.7%		10.0%	41.7%	41.7%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.2	3.2		3.2	3.2	3.2
Lost Time Adjust (s)	-2.5	0.0	-2.5	0.0	0.0	-2.5		0.0		0.0	0.0	-2.5
Total Lost Time (s)	4.0	6.5	4.0	6.5	6.5	4.0		6.5		6.5	6.5	4.0
Lead/Lag	Lead			Lag	Lag	Lag	Lag	Lag		Lead		
Lead-Lag Optimize?	Yes			Yes	Yes	Yes	Yes	Yes		Yes		
Recall Mode	C-Max	C-Max	C-Max	None	None	None	None	None		None	None	None
Act Effct Green (s)	67.9	65.4	67.9	34.5	34.5	37.0		29.6		41.6	41.6	44.1
Actuated g/C Ratio	0.57	0.54	0.57	0.29	0.29	0.31		0.25		0.35	0.35	0.37
v/c Ratio	1.35	0.72	0.05	0.17	0.98	0.60		0.90		0.67	0.73	0.53
Control Delay	203.3	26.4	0.3	26.2	61.0	34.9		59.5		66.5	60.9	54.2
Queue Delay	0.0	10.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	0.0
Total Delay	203.3	36.4	0.3	26.2	61.0	34.9		59.5		66.5	60.9	54.2
LOS	F	D	Α	С	E	С		E		E	E	D
Approach Delay		109.0			54.8			59.5			59.4	
Approach LOS	100.0	F			D	-0.4		Е		0=0	E	22 =
Queue Length 50th (m)	~183.3	127.7	0.0	6.5	124.1	53.1		81.4		25.9	117.3	69.5
Queue Length 95th (m)	#255.7	178.3	0.8	m10.7	#170.0	82.6		#111.6		#49.1	152.0	98.0
Internal Link Dist (m)	400.0	138.6	40.0	40.0	62.5			37.2			83.1	400.0
Turn Bay Length (m)	160.0	004	40.0	40.0	070	110		704		4=4	0.45	120.0
Base Capacity (vph)	434	961	860	207	973	419		784		154	645	529
Starvation Cap Reductn	0	242	0	0	0	0		0		0	0	0
Spillback Cap Reductn	0	0	0	0	0	0		0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0		0		0	0	0
Reduced v/c Ratio	1.35	0.96	0.05	0.17	0.98	0.60		0.85		0.67	0.70	0.51

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 9 (8%), Referenced to phase 4:EBTL and 7:EBL, Start of Green

Natural Cycle: 120

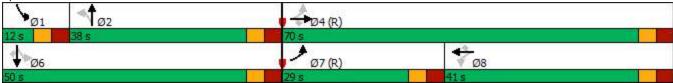
Maximum v/c Ratio: 1.35
Intersection Signal Delay: 74.2
Intersection Copacity Utilization 125.6%
ICU Level of Service H
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 12: Booth St & Albert St



	۶	<b>→</b>	•	•	←	•	4	<b>†</b>	<b>/</b>	-	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>1</b>	7	7	<b>↑</b> ↑			4	7		4	
Traffic Volume (vph)	11	722	135	280	957	32	151	16	360	41	15	3
Future Volume (vph)	11	722	135	280	957	32	151	16	360	41	15	3
Satd. Flow (prot)	1710	1782	1500	1676	3367	0	0	1648	1485	0	1724	0
Flt Permitted	0.275			0.081				0.737			0.634	
Satd. Flow (perm)	495	1782	1412	143	3367	0	0	1245	1441	0	1125	0
Satd. Flow (RTOR)			86		5				258		2	
Lane Group Flow (vph)	12	760	142	295	1041	0	0	176	379	0	62	0
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Detector Phase	4	4	4	3	8		2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0		10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	31.8	31.8	31.8	11.2	31.8		29.3	29.3	29.3	29.3	29.3	
Total Split (s)	61.0	61.0	61.0	16.0	77.0		43.0	43.0	43.0	43.0	43.0	
Total Split (%)	50.8%	50.8%	50.8%	13.3%	64.2%		35.8%	35.8%	35.8%	35.8%	35.8%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.5	3.5	3.5	2.9	3.5		3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	-2.8	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.8	6.8	4.0	6.2	6.8			6.3	6.3		6.3	
Lead/Lag	Lag	Lag	Lag	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Recall Mode	C-Max	C-Max	C-Max	None	C-Min		None	None	None	None	None	
Act Effct Green (s)	54.2	54.2	57.0	84.2	83.6			23.3	23.3		22.6	
Actuated g/C Ratio	0.45	0.45	0.48	0.70	0.70			0.19	0.19		0.19	
v/c Ratio	0.05	0.95	0.20	0.74	0.44			0.73	0.78		0.29	
Control Delay	17.8	50.2	6.4	41.2	9.6			61.5	25.3		40.8	
Queue Delay	0.0	0.0	0.0	0.0	0.8			0.0	0.0		0.0	
Total Delay	17.8	50.2	6.4	41.2	10.4			61.5	25.3		40.8	
LOS	В	D	Α	D	В			E	С		D	
Approach Delay		42.9			17.2			36.7			40.8	
Approach LOS		D			В			D			D	
Queue Length 50th (m)	1.3	176.7	4.2	48.7	53.3			41.3	28.5		12.8	
Queue Length 95th (m)	m3.4	#262.1	11.8	#123.8	87.1			60.9	61.0		23.8	
Internal Link Dist (m)		83.6			122.5			54.7			49.4	
Turn Bay Length (m)	30.0		16.0	90.0								
Base Capacity (vph)	223	804	715	396	2346			380	619		345	
Starvation Cap Reductn	0	0	0	0	908			0	0		0	
Spillback Cap Reductn	0	0	0	0	227			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.05	0.95	0.20	0.74	0.72			0.46	0.61		0.18	

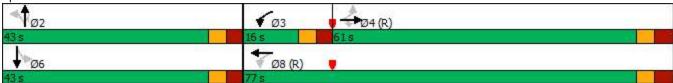
Cycle Length: 120 Actuated Cycle Length: 120

Offset: 65 (54%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 100

Maximum v/c Ratio: 0.95
Intersection Signal Delay: 29.7 Intersection LOS: C
Intersection Capacity Utilization 92.8% ICU Level of Service F
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Preston St & Albert St



# 14: Preston St/Vimy PI & Sir John A. Macdonald Pkwy

	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>∱</b> ∱		7	<b>∱</b> ∱			4			4	
Traffic Volume (vph)	2	948	13	66	1592	7	10	0	34	5	0	22
Future Volume (vph)	2	948	13	66	1592	7	10	0	34	5	0	22
Satd. Flow (prot)	1710	3380	0	1710	3417	0	0	1595	0	0	1586	0
Flt Permitted	0.140			0.246				0.920			0.948	
Satd. Flow (perm)	252	3380	0	443	3417	0	0	1485	0	0	1517	0
Satd. Flow (RTOR)		2			1			82			82	
Lane Group Flow (vph)	2	1012	0	69	1683	0	0	47	0	0	28	0
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		3	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		5.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	37.0	37.0		11.0	37.0		32.3	32.3		32.3	32.3	
Total Split (s)	72.0	72.0		15.0	87.0		33.0	33.0		33.0	33.0	
Total Split (%)	60.0%	60.0%		12.5%	72.5%		27.5%	27.5%		27.5%	27.5%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0			-2.3			-2.3	
Total Lost Time (s)	4.0	4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Act Effct Green (s)	93.8	93.8		103.0	103.8			12.3			12.3	
Actuated g/C Ratio	0.78	0.78		0.86	0.86			0.10			0.10	
v/c Ratio	0.01	0.38		0.15	0.57			0.21			0.12	
Control Delay	5.0	6.0		8.0	2.4			4.8			1.1	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	5.0	6.0		8.0	2.4			4.8			1.1	
LOS	Α	Α		Α	Α			Α			Α	
Approach Delay		6.0			2.4			4.8			1.1	
Approach LOS		Α			Α			Α			Α	
Queue Length 50th (m)	0.1	44.9		0.0	2.6			0.0			0.0	
Queue Length 95th (m)	0.9	57.2		m0.0	3.8			3.7			0.0	
Internal Link Dist (m)		651.1			66.3			44.5			21.1	
Turn Bay Length (m)	40.0			50.0								
Base Capacity (vph)	196	2641		496	2955			421			428	
Starvation Cap Reductn	0	0		0	114			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.01	0.38		0.14	0.59			0.11			0.07	

Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 37 (31%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 85

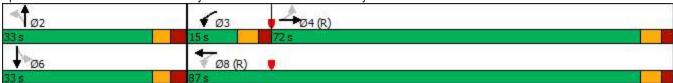
### 14: Preston St/Vimy PI & Sir John A. Macdonald Pkwy

Maximum v/c Ratio: 0.57
Intersection Signal Delay: 3.7
Intersection Capacity Utilization 72.9%
ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 14: Preston St/Vimy Pl & Sir John A. Macdonald Pkwy

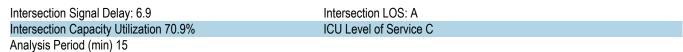


Lane Group		۶	-	$\rightarrow$	•	←	*	1	<b>†</b>	<b>/</b>	-	ţ	1
Lane Configurations	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)			44			44			<b>*</b>			ĵ.	
Satis   Flow (prot)   0   3383   0   0   3420   0   0   1658   0   0   1680   0		0		4	2		0	26		35	3		11
Fit Permitted   0.954   0.874   0.976	Future Volume (vph)	0	1012	4	2	1766	0	26	14	35	3	26	11
Satid. Flow (perm)	Satd. Flow (prot)	0	3383	0	0	3420	0	0	1658	0	0	1680	0
Satid. Flow (RTOR)	FIt Permitted					0.954			0.874			0.976	
Lane Group Flow (vph)	Satd. Flow (perm)	0	3383	0	0	3263	0	0	1474	0	0	1646	0
Tum Type	Satd. Flow (RTOR)		1						37				
Protected Phases   4	Lane Group Flow (vph)	0	1069	0	0	1861	0	0	79	0	0	42	0
Permitted Phases   8	Turn Type		NA		Perm	NA		Perm	NA		Perm	NA	
Detector Phase   Switch Phase   Sw	Protected Phases		4			8			2			6	
Switch Phase   Minimum Initial (s)	Permitted Phases												
Minimum Initial (s)         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         Minimum Spit (s)         30.5         30.5         30.5         30.5         33.3         33.3         33.3         33.3         33.3         33.3         33.3         33.3         33.3         34.0         35.8%	Detector Phase		4		8	8		2	2		6	6	
Minimum Split (s)         30.5         30.5         30.5         33.3         33.8         35.8%	Switch Phase												
Total Split (s)         61.0         61.0         61.0         34.0         34.0         34.0         34.0           Total Split (%)         64.2%         64.2%         64.2%         35.8%         35.8%         35.8%         35.8%           Yellow Time (s)         3.7         3.7         3.7         3.3         3.3         3.3         3.3           All-Red Time (s)         1.8         1.8         1.8         3.0         3.0         3.0         3.0           Lost Time Adjust (s)         -1.5         -1.5         -2.3         -2.3         -2.3         -2.3           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lead-Lag Optimize?         8         8         8         8         None         None         None         None         None         Aone         Ac         Lo         Lo </td <td>Minimum Initial (s)</td> <td></td> <td>10.0</td> <td></td> <td>10.0</td> <td>10.0</td> <td></td> <td>10.0</td> <td>10.0</td> <td></td> <td>10.0</td> <td>10.0</td> <td></td>	Minimum Initial (s)		10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Total Splitt (%) 64.2% 64.2% 64.2% 35.8% 35.8% 35.8% 35.8% Yellow Time (s) 3.7 3.7 3.7 3.3 3.3 3.3 3.3 3.3 3.3 3.3	Minimum Split (s)							33.3					
Yellow Time (s)         3.7         3.7         3.7         3.3         3.3         3.3         3.3           All-Red Time (s)         1.8         1.8         1.8         3.0         3.0         3.0         3.0           Lost Time Adjust (s)         -1.5         -1.5         -2.3         -2.3         -2.3           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lead/Lag         Well Common	Total Split (s)		61.0		61.0	61.0		34.0	34.0			34.0	
All-Red Time (s) 1.8 1.8 1.8 3.0 3.0 3.0 3.0 3.0 Lost Time Adjust (s) -1.5 -1.5 -1.5 -2.3 -2.3 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lead-Lag Optimize?  Recall Mode Max Max Max None None None None Act Effet Green (s) 63.0 63.0 12.6 12.6 Actuated g/C Ratio 0.79 0.79 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.10 0.72 0.30 0.15 0.15 0.15 0.10 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Total Split (%)		64.2%					35.8%				35.8%	
Lost Time Adjust (s)	Yellow Time (s)		3.7		3.7	3.7		3.3	3.3		3.3	3.3	
Total Lost Time (s)         4.0         4.0         4.0           Lead/Lag         Lead-Lag Optimize?           Recall Mode         Max         Max         Max         None         None         None           Act Effet Green (s)         63.0         63.0         12.6         12.6           Actuated g/C Ratio         0.79         0.79         0.16         0.16           v/c Ratio         0.40         0.72         0.30         0.15           Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           LOS         A         A         A         C         C           Approach LOS         A         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6	All-Red Time (s)				1.8			3.0			3.0		
Lead/Lag         Lead-Lag Optimize?           Recall Mode         Max         Max         Max         None         None         None           Act Effet Green (s)         63.0         63.0         12.6         12.6           Actuated g/C Ratio         0.79         0.79         0.16         0.16           v/c Ratio         0.40         0.72         0.30         0.15           Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (vph)         2685         2590         582 </td <td>Lost Time Adjust (s)</td> <td></td> <td>-1.5</td> <td></td> <td></td> <td>-1.5</td> <td></td> <td></td> <td>-2.3</td> <td></td> <td></td> <td>-2.3</td> <td></td>	Lost Time Adjust (s)		-1.5			-1.5			-2.3			-2.3	
Lead-Lag Optimize?         Recall Mode         Max         Max         Max         None         None         None           Act Effct Green (s)         63.0         63.0         12.6         12.6           Actuated g/C Ratio         0.79         0.79         0.16         0.16           v/c Ratio         0.40         0.72         0.30         0.15           Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         28.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (v	Total Lost Time (s)		4.0			4.0			4.0			4.0	
Recall Mode         Max         Max         Max         None         None         None           Act Effct Green (s)         63.0         63.0         12.6         12.6           Actuated g/C Ratio         0.79         0.79         0.16         0.16           v/c Ratio         0.40         0.72         0.30         0.15           Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (vph)         2685	Lead/Lag												
Act Effct Green (s)       63.0       63.0       12.6       12.6         Actuated g/C Ratio       0.79       0.79       0.16       0.16         v/c Ratio       0.40       0.72       0.30       0.15         Control Delay       3.9       7.6       21.1       23.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       3.9       7.6       21.1       23.6         LOS       A       A       C       C         Approach Delay       3.9       7.6       21.1       23.6         Approach LOS       A       A       C       C         Queue Length 50th (m)       26.1       72.8       5.7       4.0         Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)       Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0													
Actuated g/C Ratio       0.79       0.79       0.16       0.16         v/c Ratio       0.40       0.72       0.30       0.15         Control Delay       3.9       7.6       21.1       23.6         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       3.9       7.6       21.1       23.6         LOS       A       A       C       C         Approach Delay       3.9       7.6       21.1       23.6         Approach LOS       A       A       A       C       C         Queue Length 50th (m)       26.1       72.8       5.7       4.0         Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)       Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0					Max			None			None		
V/c Ratio         0.40         0.72         0.30         0.15           Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0	. ,												
Control Delay         3.9         7.6         21.1         23.6           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0	The state of the s												
Queue Delay       0.0       0.0       0.0       0.0         Total Delay       3.9       7.6       21.1       23.6         LOS       A       A       C       C         Approach Delay       3.9       7.6       21.1       23.6         Approach LOS       A       A       C       C         Queue Length 50th (m)       26.1       72.8       5.7       4.0         Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0													
Total Delay         3.9         7.6         21.1         23.6           LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)         Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0													
LOS         A         A         C         C           Approach Delay         3.9         7.6         21.1         23.6           Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)           Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0													
Approach Delay       3.9       7.6       21.1       23.6         Approach LOS       A       A       C       C         Queue Length 50th (m)       26.1       72.8       5.7       4.0         Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0													
Approach LOS         A         A         C         C           Queue Length 50th (m)         26.1         72.8         5.7         4.0           Queue Length 95th (m)         37.4         107.6         17.6         12.6           Internal Link Dist (m)         354.8         288.5         38.4         167.1           Turn Bay Length (m)           Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0													
Queue Length 50th (m)       26.1       72.8       5.7       4.0         Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0													
Queue Length 95th (m)       37.4       107.6       17.6       12.6         Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0	• •												
Internal Link Dist (m)       354.8       288.5       38.4       167.1         Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0	• ,												
Turn Bay Length (m)         Base Capacity (vph)       2685       2590       582       632         Starvation Cap Reductn       0       0       0       0         Spillback Cap Reductn       0       0       0       0         Storage Cap Reductn       0       0       0       0													
Base Capacity (vph)         2685         2590         582         632           Starvation Cap Reductn         0         0         0         0           Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0			354.8			288.5			38.4			167.1	
Starvation Cap Reductn         0         0         0           Spillback Cap Reductn         0         0         0           Storage Cap Reductn         0         0         0	, ,												
Spillback Cap Reductn         0         0         0         0           Storage Cap Reductn         0         0         0         0													
Storage Cap Reductn 0 0 0													
Reduced v/c Ratio 0.40 0.72 0.14 0.07													
	Reduced v/c Ratio		0.40			0.72			0.14			0.07	

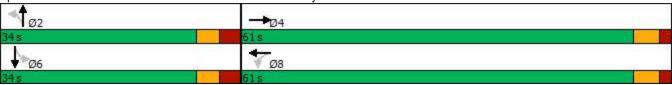
Cycle Length: 95

Actuated Cycle Length: 79.3

Natural Cycle: 90 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.72



Splits and Phases: 15: Slidell St & Sir John A. Macdonald Pkwy



	۶	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ»		ሻ	<b>†</b>	7	ሻ	<b>†</b>	7	ሻ	f)	
Traffic Volume (vph)	10	612	88	94	803	121	88	185	84	59	68	11
Future Volume (vph)	10	612	88	94	803	121	88	185	84	59	68	11
Satd. Flow (prot)	1555	1733	0	1710	1782	1530	1710	1800	1485	1710	1762	0
Flt Permitted	0.246			0.301			0.702			0.459		
Satd. Flow (perm)	403	1733	0	542	1782	1530	1264	1800	1485	826	1762	0
Satd. Flow (RTOR)		14				90			79		8	
Lane Group Flow (vph)	11	737	0	99	845	127	93	195	88	62	84	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	32.5	32.5		32.5	32.5	32.5	31.4	31.4	31.4	31.4	31.4	
Total Split (s)	68.0	68.0		68.0	68.0	68.0	32.0	32.0	32.0	32.0	32.0	
Total Split (%)	68.0%	68.0%		68.0%	68.0%	68.0%	32.0%	32.0%	32.0%	32.0%	32.0%	
Yellow Time (s)	3.3	3.3		3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	3.2	3.2		3.2	3.2	3.2	3.1	3.1	3.1	3.1	3.1	
Lost Time Adjust (s)	-2.5	-2.5		-2.5	-2.5	-2.5	-2.4	-2.4	-2.4	-2.4	-2.4	
Total Lost Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	71.6	71.6		71.6	71.6	71.6	20.4	20.4	20.4	20.4	20.4	
Actuated g/C Ratio	0.72	0.72		0.72	0.72	0.72	0.20	0.20	0.20	0.20	0.20	
v/c Ratio	0.04	0.59		0.26	0.66	0.11	0.36	0.53	0.24	0.37	0.23	
Control Delay	10.5	20.6		8.5	12.3	2.5	36.3	39.7	9.8	38.4	29.3	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.5	20.6		8.5	12.3	2.5	36.3	39.7	9.8	38.4	29.3	
LOS	В	C		Α	B	Α	D	D	Α	D	C	
Approach Delay		20.5			10.8			31.9			33.2	
Approach LOS	1.0	C		EE	B	17	16.0	C	1 5	11.0	C	
Queue Length 50th (m)	1.0	127.5		5.5	73.3	1.7	16.9	36.7	1.5	11.3	13.3	
Queue Length 95th (m)	1112.3	m176.0 635.7		17.2	155.5 497.2	8.8	28.9	53.3 83.8	13.2	22.0	24.1 171.5	
Internal Link Dist (m)	45.0	033.7		50 O	491.2	40.0	50.0	03.0	20.0	45.0	171.5	
Turn Bay Length (m) Base Capacity (vph)	288	1244		50.0 388	1275	1121	353	504	472	231	499	
		0										
Starvation Cap Reductn Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.04	0.59		0.26	0.66	0.11	0.26	0.39	0.19	0.27	0.17	
Neduced V/C Rallo	0.04	0.59		0.20	0.00	0.11	0.20	0.39	0.19	0.27	0.17	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 65 (65%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 75

### 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 18.7 Intersection LOS: B
Intersection Capacity Utilization 84.9% ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 16: Bayswater Ave/Bayview Station Rd & Scott St/Albert St



	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	<b>†</b>	7	ሻ	<b>^</b>	7	Ĭ	<b>†</b>		7	<b>^</b>	
Traffic Volume (vph)	227	530	86	56	677	63	40	546	48	58	233	210
Future Volume (vph)	227	530	86	56	677	63	40	546	48	58	233	210
Satd. Flow (prot)	1693	1765	1515	1710	1800	1530	1710	1776	0	1710	1640	0
Flt Permitted	0.111			0.455			0.277			0.116		
Satd. Flow (perm)	198	1765	1515	819	1800	1530	499	1776	0	209	1640	0
Satd. Flow (RTOR)			68			119		5			55	
Lane Group Flow (vph)	239	558	91	59	713	66	42	626	0	61	466	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	10.6	28.1	28.1	28.1	28.1	28.1	35.3	35.3		35.3	35.3	
Total Split (s)	15.0	49.0	49.0	34.0	34.0	34.0	45.0	45.0		45.0	45.0	
Total Split (%)	15.0%	49.0%	49.0%	34.0%	34.0%	34.0%	45.0%	45.0%		45.0%	45.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.0	3.0		3.0	3.0	
All-Red Time (s)	2.3	2.8	2.8	2.8	2.8	2.8	3.3	3.3		3.3	3.3	
Lost Time Adjust (s)	-1.6	-2.1	0.0	-2.1	-2.1	0.0	-2.3	-2.3		-2.3	-2.3	
Total Lost Time (s)	4.0	4.0	6.1	4.0	4.0	6.1	4.0	4.0		4.0	4.0	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	Min	Min		Min	Min	
Act Effct Green (s)	52.9	52.9	50.8	32.6	32.6	30.5	39.1	39.1		39.1	39.1	
Actuated g/C Ratio	0.53	0.53	0.51	0.33	0.33	0.30	0.39	0.39		0.39	0.39	
v/c Ratio	0.69	0.60	0.11	0.22	1.21	0.12	0.22	0.90		0.75	0.69	
Control Delay	30.9	20.1	5.5	28.8	139.4	3.2	22.8	45.6		78.7	28.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	30.9	20.1	5.5	28.8	139.4	3.2	22.8	45.6		78.7	28.0	
LOS	С	С	Α	С	F	Α	С	D		Е	С	
Approach Delay		21.5			120.9			44.2			33.9	
Approach LOS		С			F			D			С	
Queue Length 50th (m)	30.2	78.1	2.3	6.7	~191.5	0.1	5.4	112.9		10.2	66.7	
Queue Length 95th (m)	57.4	114.2	10.6	m12.7	#259.7	m1.8	13.8	#179.3		#34.6	103.3	
Internal Link Dist (m)		92.9			635.7			70.7			630.1	
Turn Bay Length (m)	55.0		45.0	65.0		35.0	55.0			50.0		
Base Capacity (vph)	348	934	803	267	587	549	204	731		85	704	
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	
Reduced v/c Ratio	0.69	0.60	0.11	0.22	1.21	0.12	0.21	0.86		0.72	0.66	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 8 (8%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 100

Lane Configurations  Traffic Volume (vph)  Future Volume (vph)  Satd. Flow (prot)  Fit Permitted  Satd. Flow (perm)  Satd. Flow (RTOR)  Lane Group Flow (vph)  Turn Type  Protected Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (%)  Yellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Tum Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio  Intersection Summary	Lane Group	Ø9		
Traffic Volume (vph) Future Volume (vph) Satd. Flow (prot) FIt Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Future Volume (vph) Satd. Flow (prot) Fit Permitted Satd. Flow (prom) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Solution Split (s) So				
Satd. Flow (prot) Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Fit Permitted Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6.0 Source Substited Split (s) 6.0 Total Split (s) 7 Total Cost Time (s) 7 Lead/Lag 8 Lead/Lag 9 Lead/Lag 9 Lead/Lag 9 Lead/Lag 9 Lead/Lag 10 Lead/Lag				
Satd. Flow (perm) Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases 9 Permitted Phases Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Ype Internal Park (m) Internal Link Dist (m) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Satd. Flow (RTOR) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%) Sellow Time (s) All-Red Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Solution Split (s				
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Minimum				
Protected Phases  Permitted Phases  Detector Phase  Switch Phase  Minimum Initial (s)  Minimum Split (s)  Total Split (s)  Total Split (%)  Sylellow Time (s)  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach LOS  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (yph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Cotal Split (s) Cotal Split (%) Sellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lost Time (s) Lead-Lag Optimize? Recall Mode Act Effct Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 55th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Detector Phase Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (s) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach LoS Approach LoS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		9		
Switch Phase Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effet Green (s) Actuated g/C Ratio V/c Ratio Control Delay Queue Delay Total Delay LOS Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Minimum Initial (s) 2.0 Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Tum Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Minimum Split (s) 6.0  Total Split (s) 6.0  Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio	Switch Phase			
Minimum Split (s) 6.0 Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Minimum Initial (s)	2.0		
Total Split (s) 6.0 Total Split (%) 6% Yellow Time (s) 2.0 All-Red Time (s) 0.0 Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio		6.0		
Total Split (%) 6%  Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
Yellow Time (s) 2.0  All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Reduced v/c Ratio				
All-Red Time (s) 0.0  Lost Time Adjust (s)  Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Recall Mode None Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Total Lost Time (s)  Lead/Lag  Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio  v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio		3.0		
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS  Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio				
Lead-Lag Optimize?  Recall Mode None  Act Effct Green (s)  Actuated g/C Ratio v/c Ratio  Control Delay  Queue Delay  Total Delay  LOS  Approach Delay  Approach LOS  Queue Length 50th (m)  Queue Length 95th (m)  Internal Link Dist (m)  Turn Bay Length (m)  Base Capacity (vph)  Starvation Cap Reductn  Spillback Cap Reductn  Storage Cap Reductn  Reduced v/c Ratio				
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Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
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Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio	Internal Link Dist (m)			
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio				
Storage Cap Reductn Reduced v/c Ratio				
Reduced v/c Ratio				
Intersection Summary				
	Intersection Summary			

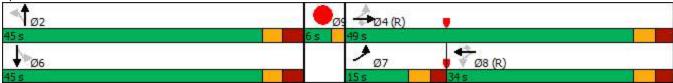
Maximum v/c Ratio: 1.21
Intersection Signal Delay: 57.5 Intersection LOS: E
Intersection Capacity Utilization 106.0% ICU Level of Service G
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 17: Parkdale Ave & Scott St



	ሻ	<b>†</b>	7	(w	<b>↓</b>	w	<b>4</b>	>	4	*	
Lane Group	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	SWL	SWR	
Lane Configurations		<b>†</b>			<b>†</b>			7			
Traffic Volume (vph)	0	534	469	0	295	0	0	0	0	0	
Future Volume (vph)	0	534	469	0	295	0	0	0	0	0	
Satd. Flow (prot)	0	1654	0	0	1765	0	0	1765	0	0	
Flt Permitted											
Satd. Flow (perm)	0	1654	0	0	1765	0	0	1765	0	0	
Lane Group Flow (vph)	0	1056	0	0	311	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilizat	ion 63.3%			IC	U Level c	of Service	В				
Analysis Period (min) 15											

	•	•	1	<b>†</b>	<b>↓</b>	4		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø9	
Lane Configurations	¥		ች	<b>^</b>	<b>†</b> 1>			_
Traffic Volume (vph)	1	8	8	1124	1093	10		
Future Volume (vph)	1	8	8	1124	1093	10		
Satd. Flow (prot)	1574	0	1710	3420	3417	0		
Flt Permitted	0.994		0.237					
Satd. Flow (perm)	1574	0	427	3420	3417	0		
Satd. Flow (RTOR)	8				1			
Lane Group Flow (vph)	9	0	8	1183	1162	0		
Turn Type	Prot		Perm	NA	NA			
Protected Phases	4			2	6		9	
Permitted Phases			2					
Detector Phase	4		2	2	6			
Switch Phase								
Minimum Initial (s)	10.0		10.0	10.0	10.0		10.0	
Minimum Split (s)	17.0		30.9	30.9	30.9		29.0	
Total Split (s)	17.0		46.0	46.0	46.0		29.0	
Total Split (%)	18.5%		50.0%	50.0%	50.0%		32%	
Yellow Time (s)	3.3		3.3	3.3	3.3		3.0	
All-Red Time (s)	2.9		2.6	2.6	2.6		4.0	
Lost Time Adjust (s)	-2.2		-1.9	-1.9	-1.9			
Total Lost Time (s)	4.0		4.0	4.0	4.0			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None		C-Max	C-Max	C-Max		None	
Act Effct Green (s)	12.2		88.0	88.0	88.0			
Actuated g/C Ratio	0.13		0.96	0.96	0.96			
v/c Ratio	0.04		0.02	0.36	0.36			
Control Delay	21.7		1.4	1.3	1.2			
Queue Delay	0.0		0.0	0.0	0.0			
Total Delay	21.7		1.4	1.3	1.2			
LOS	С		Α	Α	Α			
Approach Delay	21.7			1.3	1.2			
Approach LOS	С			Α	Α			
Queue Length 50th (m)	0.2		0.0	0.0	0.0			
Queue Length 95th (m)	4.7		1.1	38.7	37.6			
Internal Link Dist (m)	148.9			192.9	188.6			
Turn Bay Length (m)			35.0					
Base Capacity (vph)	229		408	3270	3267			
Starvation Cap Reductn	0		0	0	0			
Spillback Cap Reductn	0		0	0	0			
Storage Cap Reductn	0		0	0	0			
Reduced v/c Ratio	0.04		0.02	0.36	0.36			
Intono action Common and								

Cycle Length: 92

Actuated Cycle Length: 92

Offset: 10 (11%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 80

Intersection Signal Delay: 1.3 Intersection Capacity Utilization 47.8%	Intersection LOS: A ICU Level of Service A		
Analysis Period (min) 15	100 20101 01 00111007		
Splits and Phases: 19: Booth St & War Museum			
Ø2 (R)	<b>→</b> Ø4	<b>O</b> Ø9	
<b>▲</b>	<b>→</b> 04	Ø9	
<b>▲</b>	<b>→</b> <sub>04</sub> 17 s	<b>⊘</b> ø9 <b>29 s</b>	

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>†</b>	7	ň	<b>†</b>	7	¥	f)			4	
Traffic Volume (vph)	9	640	76	84	880	28	57	4	80	42	0	8
Future Volume (vph)	9	640	76	84	880	28	57	4	80	42	0	8
Satd. Flow (prot)	1710	1782	1530	1693	1765	1530	1644	1500	0	0	1690	0
Flt Permitted	0.211			0.404			0.760				0.630	
Satd. Flow (perm)	380	1782	1530	720	1765	1530	1315	1500	0	0	1110	0
Satd. Flow (RTOR)			73			87		84			87	
Lane Group Flow (vph)	9	674	80	88	926	29	60	88	0	0	52	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	7	4	4	8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	16.3	27.3	27.3	27.3	27.3	27.3	29.3	29.3		29.3	29.3	
Total Split (s)	17.0	84.0	84.0	67.0	67.0	67.0	36.0	36.0		36.0	36.0	
Total Split (%)	14.2%	70.0%	70.0%	55.8%	55.8%	55.8%	30.0%	30.0%		30.0%	30.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3		3.3	3.3	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-2.3	-2.3	0.0	-2.3	-2.3	0.0	-2.3	-2.3			-2.3	
Total Lost Time (s)	4.0	4.0	6.3	4.0	4.0	6.3	4.0	4.0			4.0	
Lead/Lag	Lead			Lag	Lag	Lag						
Lead-Lag Optimize?	Yes			Yes	Yes	Yes						
Recall Mode	None	C-Max	C-Max	C-Max	C-Max	C-Max	None	None		None	None	
Act Effct Green (s)	97.7	97.7	95.4	94.4	94.4	92.1	14.3	14.3			14.3	
Actuated g/C Ratio	0.81	0.81	0.80	0.79	0.79	0.77	0.12	0.12			0.12	
v/c Ratio	0.02	0.46	0.06	0.16	0.67	0.02	0.38	0.35			0.25	
Control Delay	2.7	4.8	1.0	3.6	11.1	0.0	55.5	14.7			5.5	
Queue Delay	0.0	0.1	0.0	0.0	0.3	0.0	0.0	0.0			0.0	
Total Delay	2.7	4.8	1.0	3.6	11.3	0.0	55.5	14.7			5.5	
LOS	А	A	Α	Α	В	Α	E	В			A	
Approach Delay		4.4			10.4			31.2			5.5	
Approach LOS	0.0	A	0.0	0.0	В	0.0	44.4	С			A	
Queue Length 50th (m)	0.3	36.9	0.3	0.0	132.3	0.0	14.1	0.9			0.0	
Queue Length 95th (m)	1.5	68.2	3.8	0.0	272.6	m0.0	27.3	16.1			3.9	
Internal Link Dist (m)	20.0	497.2	50.0	25.0	109.7	50.0	20.0	178.8			36.6	
Turn Bay Length (m)	30.0	4450	50.0	35.0	4200	50.0	30.0	101			250	
Base Capacity (vph)	453	1450	1231	566	1389	1195	350	461			359	
Starvation Cap Reductn	0	104	0	0	94	0	0	0			0	
Spillback Cap Reductn	0	104	0	0	0	0	0	4			5	
Storage Cap Reductn	0	0.50	0 06	0 16	0 72	0 02	0 17	0 10			0 15	
Reduced v/c Ratio	0.02	0.50	0.06	0.16	0.72	0.02	0.17	0.19			0.15	

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 80 (67%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

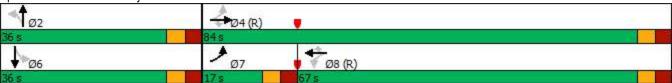
Natural Cycle: 100

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 9.5	Intersection LOS: A	
Intersection Capacity Utilization 76.9%	ICU Level of Service D	
Analysis Pariod (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 20: City Centre Ave & Albert St



	٠	<b>→</b>	<b>←</b>	•	<b>/</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	<b></b>	<b>†</b> ‡		W	
Traffic Volume (vph)	34	1139	1117	105	107	35
Future Volume (vph)	34	1139	1117	105	107	35
Satd. Flow (prot)	1676	1751	3303	0	1645	0
Flt Permitted	0.159				0.964	
Satd. Flow (perm)	281	1751	3303	0	1645	0
Satd. Flow (RTOR)			14		14	
Lane Group Flow (vph)	36	1199	1287	0	150	0
Turn Type	pm+pt	NA	NA		Prot	
Protected Phases	7	4	8		6	
Permitted Phases	4					
Detector Phase	7	4	8		6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0		10.0	
Minimum Split (s)	14.7	22.0	22.0		29.0	
Total Split (s)	15.0	80.0	65.0		30.0	
Total Split (%)	13.6%	72.7%	59.1%		27.3%	
Yellow Time (s)	3.3	3.3	3.3		3.0	
All-Red Time (s)	1.4	1.4	1.4		2.0	
Lost Time Adjust (s)	-0.7	-0.7	-0.7		-1.0	
Total Lost Time (s)	4.0	4.0	4.0		4.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	C-Max	C-Max		None	
Act Effct Green (s)	86.2	86.2	77.4		15.8	
Actuated g/C Ratio	0.78	0.78	0.70		0.14	
v/c Ratio	0.10	0.87	0.55		0.61	
Control Delay	3.9	18.5	10.7		49.9	
Queue Delay	0.0	27.7	0.6		0.0	
Total Delay	3.9	46.2	11.3		49.9	
LOS	Α	D	В		D	
Approach Delay		45.0	11.3		49.9	
Approach LOS		D	В		D	
Queue Length 50th (m)	1.5	148.3	78.4		29.2	
Queue Length 95th (m)	4.6	#344.8	115.8		48.1	
Internal Link Dist (m)		122.5	138.6		61.4	
Turn Bay Length (m)	30.0					
Base Capacity (vph)	359	1372	2328		399	
Starvation Cap Reductn	0	231	577		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.10	1.05	0.74		0.38	

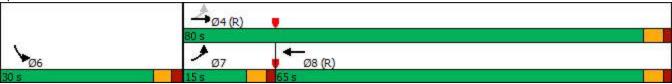
Cycle Length: 110 Actuated Cycle Length: 110

Offset: 22 (20%), Referenced to phase 4:EBTL and 8:WBT, Start of Green

Natural Cycle: 100

Maximum v/c Ratio: 0.87
Intersection Signal Delay: 29.0 Intersection LOS: C
Intersection Capacity Utilization 78.5% ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 21: Albert St & Access 1



	-	•	•	•	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> Ъ		ች	<b>^</b>	¥	
Traffic Volume (vph)	1158	35	64	1342	40	63
Future Volume (vph)	1158	35	64	1342	40	63
Satd. Flow (prot)	3370	0	1710	3386	1601	0
Flt Permitted			0.172		0.981	
Satd. Flow (perm)	3370	0	310	3386	1601	0
Satd. Flow (RTOR)	4				63	
Lane Group Flow (vph)	1256	0	67	1413	108	0
Turn Type	NA		pm+pt	NA	Prot	
Protected Phases	4		3	8	2	
Permitted Phases			8			
Detector Phase	4		3	8	2	
Switch Phase						
Minimum Initial (s)	10.0		5.0	10.0	10.0	
Minimum Split (s)	30.0		14.0	22.0	33.8	
Total Split (s)	66.0		20.0	86.0	34.0	
Total Split (%)	55.0%		16.7%	71.7%	28.3%	
Yellow Time (s)	3.7		3.7	3.7	3.3	
All-Red Time (s)	1.9		1.9	1.9	2.5	
Lost Time Adjust (s)	-1.6		-1.6	-1.6	-1.8	
Total Lost Time (s)	4.0		4.0	4.0	4.0	
Lead/Lag	Lag		Lead			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	C-Max		None	C-Max	None	
Act Effct Green (s)	86.4		96.4	96.4	15.6	
Actuated g/C Ratio	0.72		0.80	0.80	0.13	
v/c Ratio	0.52		0.19	0.52	0.41	
Control Delay	9.8		4.8	5.7	25.2	
Queue Delay	0.5		0.0	2.0	0.0	
Total Delay	10.3		4.8	7.7	25.3	
LOS	В		A	Α	C	
Approach Delay	10.3			7.5	25.3	
Approach LOS	В			A	C	
Queue Length 50th (m)	41.1		2.2	39.2	10.5	
Queue Length 95th (m)	m92.1		9.7	110.0	24.2	
Internal Link Dist (m)	117.1		<b>.</b>	245.9	41.8	
Turn Bay Length (m)			70.0			
Base Capacity (vph)	2428		435	2719	447	
Starvation Cap Reductn	670		0	0	0	
Spillback Cap Reductn	0		0	1099	19	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.71		0.15	0.87	0.25	
	<b></b>					

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 23 (19%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 80

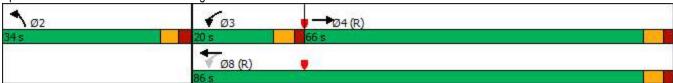
Maximum v/c Ratio: 0.52

Intersection Signal Delay: 9.4	Intersection LOS: A
Intersection Capacity Utilization 57.5%	ICU Level of Service B
–	

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 22: Lett St & Wellington St



	۶	-	$\rightarrow$	•	←	•	1	<b>†</b>	<b>/</b>	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>∱</b> ∱			414			4			4	
Traffic Volume (vph)	43	774	16	11	1090	15	9	0	14	43	0	11
Future Volume (vph)	43	774	16	11	1090	15	9	0	14	43	0	11
Satd. Flow (prot)	1710	3344	0	0	3377	0	0	1619	0	0	1683	0
Flt Permitted	0.202				0.944			0.896			0.755	
Satd. Flow (perm)	364	3344	0	0	3191	0	0	1477	0	0	1321	0
Satd. Flow (RTOR)		3			2			56			56	
Lane Group Flow (vph)	45	832	0	0	1175	0	0	24	0	0	57	0
Turn Type	pm+pt	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	9.6	22.0		22.0	22.0		30.5	30.5		30.5	30.5	
Total Split (s)	15.0	80.0		65.0	65.0		40.0	40.0		40.0	40.0	
Total Split (%)	12.5%	66.7%		54.2%	54.2%		33.3%	33.3%		33.3%	33.3%	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	1.3	1.3		1.3	1.3		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	-0.6	-0.6			-0.6			-0.3			-0.3	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag	Lead			Lag	Lag							
Lead-Lag Optimize?	Yes			Yes	Yes							
Recall Mode	None	C-Min		C-Max	C-Max		None	None		None	None	
Act Effct Green (s)	101.6	102.4			93.7			13.3			13.3	
Actuated g/C Ratio	0.85	0.85			0.78			0.11			0.11	
v/c Ratio	0.12	0.29			0.47			0.11			0.29	
Control Delay	2.0	1.6			7.6			1.7			15.4	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	2.0	1.6			7.6			1.7			15.4	
LOS	Α	Α			Α			Α			В	
Approach Delay		1.6			7.6			1.7			15.4	
Approach LOS		Α			Α			Α			В	
Queue Length 50th (m)	0.8	12.9			49.6			0.0			0.2	
Queue Length 95th (m)	m2.0	m19.2			105.3			1.0			11.7	
Internal Link Dist (m)		82.0			212.6			72.6			45.7	
Turn Bay Length (m)	70.0											
Base Capacity (vph)	431	2852			2490			482			435	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.10	0.29			0.47			0.05			0.13	

Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 75

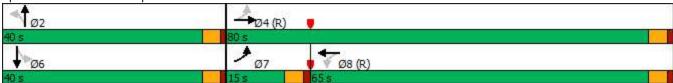
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 5.3	Intersection LOS: A	
Intersection Capacity Utilization 55.5%	ICU Level of Service B	
Analysis Period (min) 15		

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 23: Empress Ave & Albert St



	<b>≯</b>	<b>→</b>	←	•	<b>\</b>	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<b>^</b>	<b>∱</b> ∱			7
Traffic Volume (vph)	0	790	1121	14	0	11
Future Volume (vph)	0	790	1121	14	0	11
Satd. Flow (prot)	0	3353	3346	0	0	1526
Flt Permitted						
Satd. Flow (perm)	0	3353	3346	0	0	1526
Lane Group Flow (vph)	0	832	1195	0	0	12
Sign Control		Free	Free		Stop	
Intersection Summary						
Control Type: Unsignalized		_	_			_
Intersection Capacity Utilization	on 43.2%			IC	U Level c	of Service
Analysis Period (min) 15						

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			7			7		<b>↑</b> ↑			<b>∱</b> β	
Traffic Volume (vph)	0	0	17	0	0	10	0	1215	27	0	756	9
Future Volume (vph)	0	0	17	0	0	10	0	1215	27	0	756	9
Satd. Flow (prot)	0	0	1526	0	0	1526	0	3343	0	0	3346	0
Flt Permitted												
Satd. Flow (perm)	0	0	1526	0	0	1526	0	3343	0	0	3346	0
Lane Group Flow (vph)	0	0	18	0	0	11	0	1307	0	0	805	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizati	Intersection Capacity Utilization 46.4% ICU Level of Service A											
Analysis Period (min) 15												

	-	•	•	<b>—</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>ተ</b> ኈ			<b>^</b>		7
Traffic Volume (vph)	991	7	0	1664	0	18
Future Volume (vph)	991	7	0	1664	0	18
Satd. Flow (prot)	3350	0	0	3353	0	1526
Flt Permitted						
Satd. Flow (perm)	3350	0	0	3353	0	1526
Lane Group Flow (vph)	1050	0	0	1752	0	19
Sign Control	Free			Free	Stop	
Intersection Summary						
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 51.9%			IC	U Level c	of Service
Analysis Period (min) 15						

	-	¥ •	· •	- 🔨	1
Lane Group	EBT	EBR WI	BL WE	T NBL	NBR
Lane Configurations	<b>†</b> Ъ		ኻ ተ		
Traffic Volume (vph)	980	11 1	22 154		73
Future Volume (vph)	980		22 154		73
Satd. Flow (prot)	3413	0 17			0
Flt Permitted	3110	0.2		0.990	
Satd. Flow (perm)	3413		12 342		0
Satd. Flow (RTOR)	2	· ·		77	
Lane Group Flow (vph)	1044	0 1	28 162		0
Turn Type	NA	pm+		A Prot	
Protected Phases	4	Pill.	3	8 2	
Permitted Phases	Т.		8	2	
Detector Phase	4		3	8 2	
Switch Phase	Т		9	2	
Minimum Initial (s)	10.0		.0 10	.0 10.0	
Minimum Split (s)	37.0	11			
Total Split (s)	72.0	15			
Total Split (%)	60.0%	12.5			
Yellow Time (s)	3.7			70 21.5% .7 3.3	
All-Red Time (s)	2.3			.7 3.3	
Lost Time Adjust (s)	-2.0		.0 -2		
Total Lost Time (s)	4.0			.0 -2.3	
Lead/Lag	4.0 Lag	Le		.0 4.0	
Lead-Lag Optimize?	Yes		au es		
Recall Mode	C-Max	Y No		ax None	
	86.0	99			
Act Effct Green (s)	0.72	0.			
Actuated g/C Ratio					
v/c Ratio	0.43	0.			
Control Delay	6.2			.4 21.1	
Queue Delay	0.1		.0 0		
Total Delay	6.3	5	.2 10		
LOS	A		Α	B C	
Approach Delay	6.3			.8 21.1	
Approach LOS	Α			A C	
Queue Length 50th (m)	32.5		.5 106		
Queue Length 95th (m)	38.5	m12	.2 m106		
Internal Link Dist (m)	71.8		49	.0 28.0	
Turn Bay Length (m)	0445	60		7 4/2	
Base Capacity (vph)	2447	4	59 282		
Starvation Cap Reductn	240		0 76		
Spillback Cap Reductn	64		0	0 1	
Storage Cap Reductn	0		0	0 0	
Reduced v/c Ratio	0.47	0.	28 0.7	9 0.22	
_					

# Intersection Summary

Cycle Length: 120 Actuated Cycle Length: 120

Offset: 22 (18%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 85

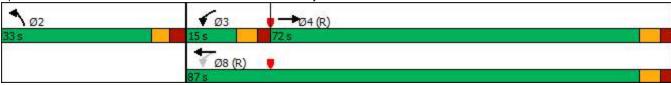
Control Type: Actuated-Coordinated

# 27: Broad St & Sir John A. Macdonald Pkwy

Maximum v/c Ratio: 0.57
Intersection Signal Delay: 8.9
Intersection Capacity Utilization 60.0%
ICU Level of Service B
Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 27: Broad St & Sir John A. Macdonald Pkwy



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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	<b>∱</b> }			<b>^</b>		7		
Traffic Volume (vph)	966	4	0	1558	0	18		
Future Volume (vph)	966	4	0	1558	0	18		
Satd. Flow (prot)	3350	0	0	3353	0	1526		
Flt Permitted								
Satd. Flow (perm)	3350	0	0	3353	0	1526		
Lane Group Flow (vph)	1021	0	0	1640	0	19		
Sign Control	Free			Free	Stop			
Intersection Summary								
Control Type: Unsignalized								
Intersection Capacity Utiliz	ation 48.8%			IC	U Level o	of Service		
Analysis Period (min) 15								

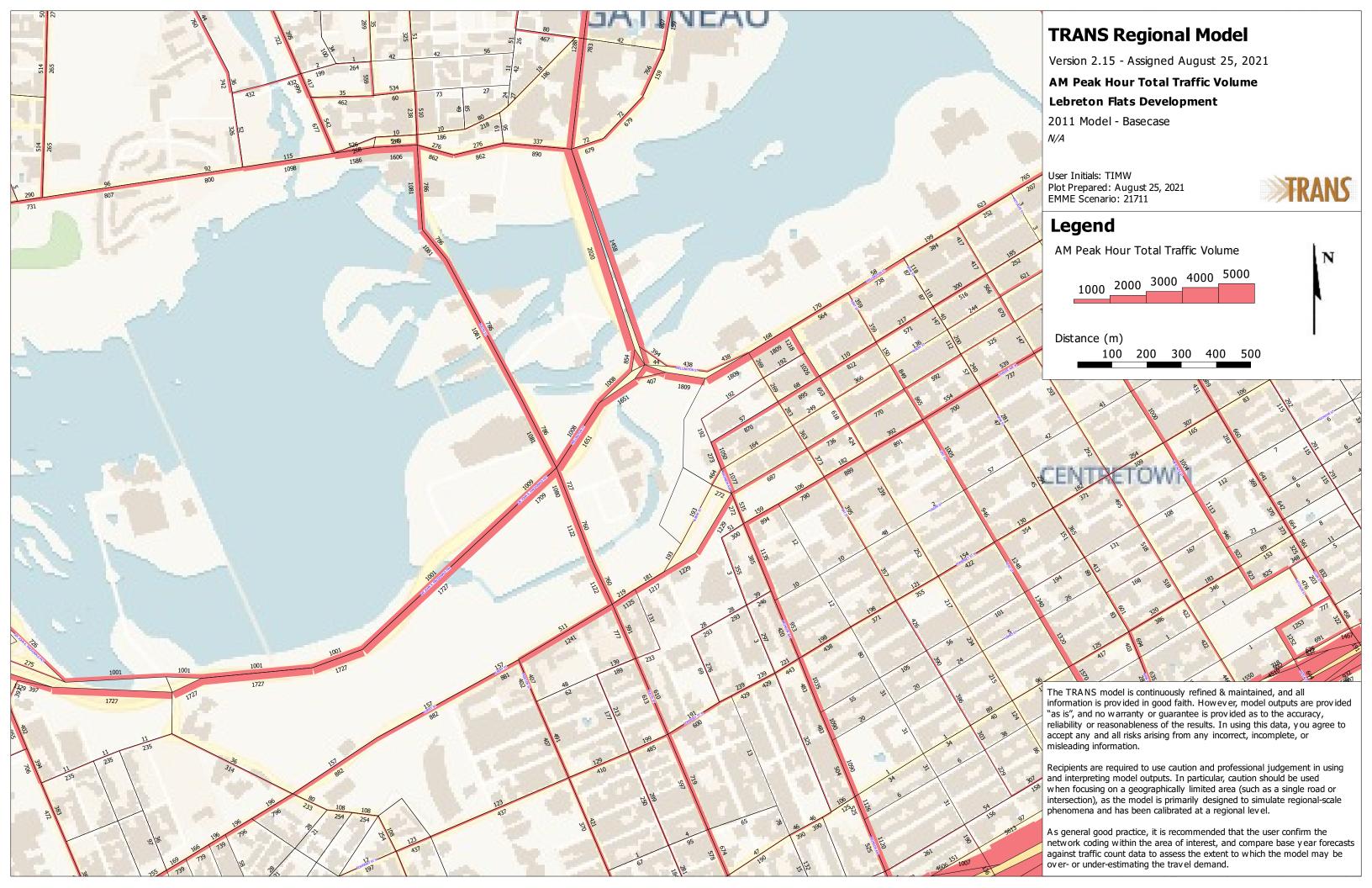
	<b>&gt;</b>	-	7	<b>*</b>	←	*_	<b>\</b>	)	ን	/	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SER	NEL	NER	
Lane Configurations		<b>^</b>			<b>^</b>					7	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Satd. Flow (prot)	0	3353	0	0	3353	0	0	0	0	1765	
Flt Permitted											
Satd. Flow (perm)	0	3353	0	0	3353	0	0	0	0	1765	
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilizatio	n 13.3%			IC	U Level o	of Service	Α				
Analysis Period (min) 15											

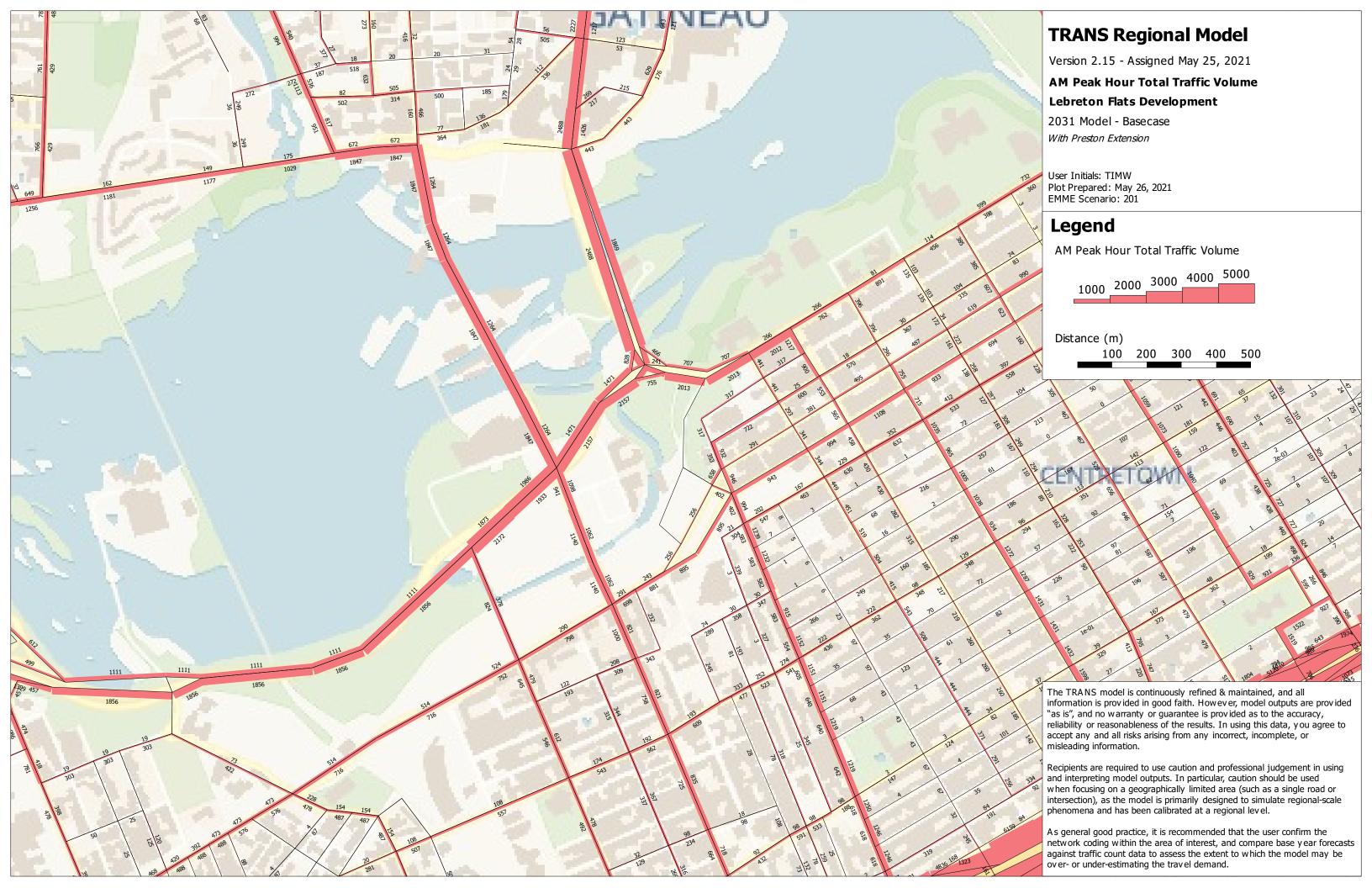
	ሻ	r*	×	7	•	×
Lane Group	NBL	NBR	SET	SER	NWL	NWT
Lane Configurations			<b>†</b>			<b>†</b>
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0
Satd. Flow (prot)	0	0	1765	0	0	1765
Flt Permitted						
Satd. Flow (perm)	0	0	1765	0	0	1765
Lane Group Flow (vph)	0	0	0	0	0	0
Sign Control	Free		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	U Level o	of Service
Analysis Period (min) 15						

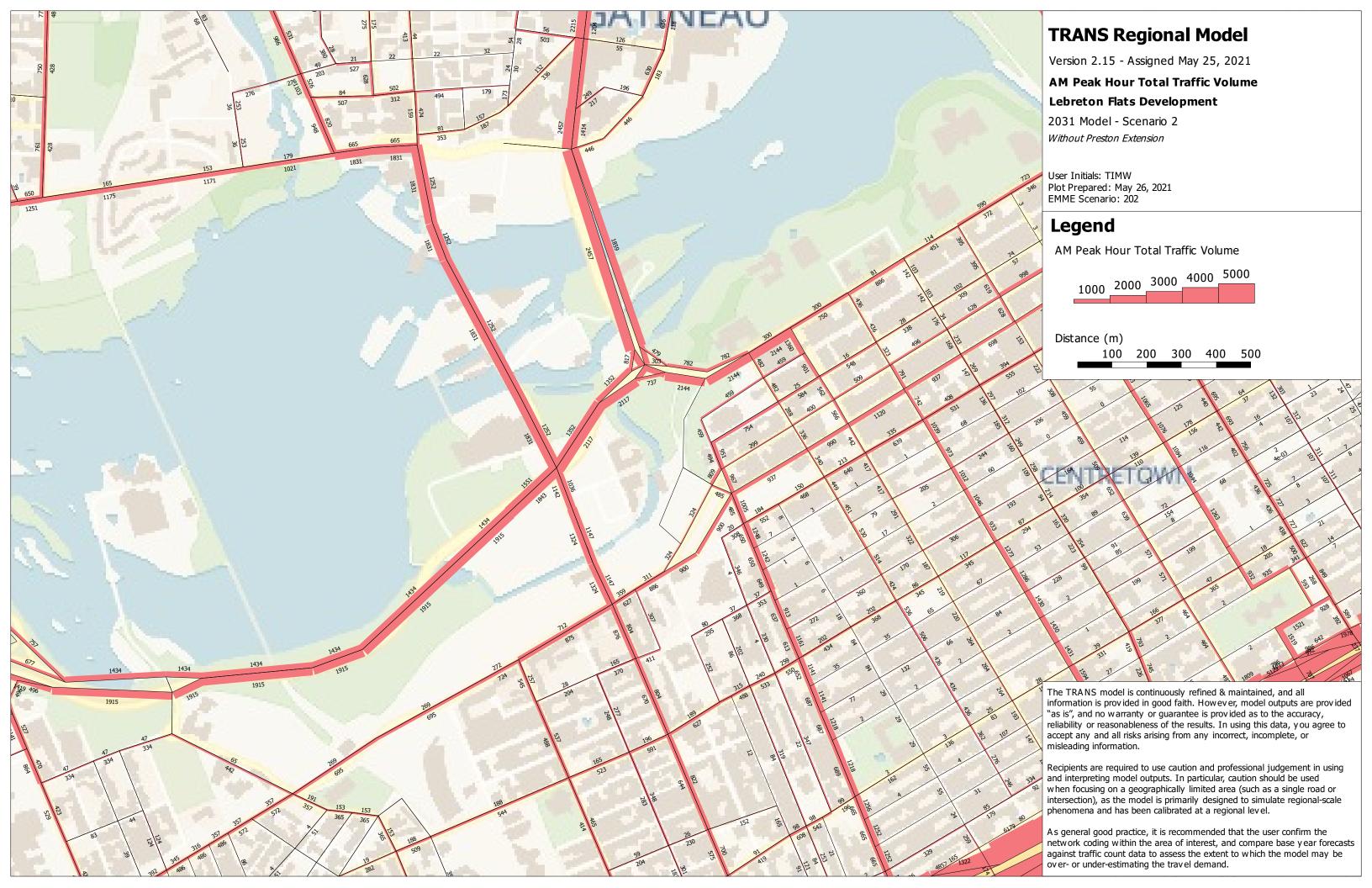
	_#	<b>→</b>	<b>-</b> *	4	←	€_	4	₹	4	1	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NWL	NWR	SWL	SWR	
Lane Configurations		<b>^</b>			44				ሻ	•	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	
Satd. Flow (prot)	0	3353	0	0	3353	0	0	0	1765	0	
FIt Permitted											
Satd. Flow (perm)	0	3353	0	0	3353	0	0	0	1765	0	
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	
Sign Control		Free			Free		Free		Free		
Intersection Summary											
Control Type: Unsignalized											
Intersection Capacity Utilization	Intersection Capacity Utilization 13.3% ICU Level of Service A										
Analysis Period (min) 15											

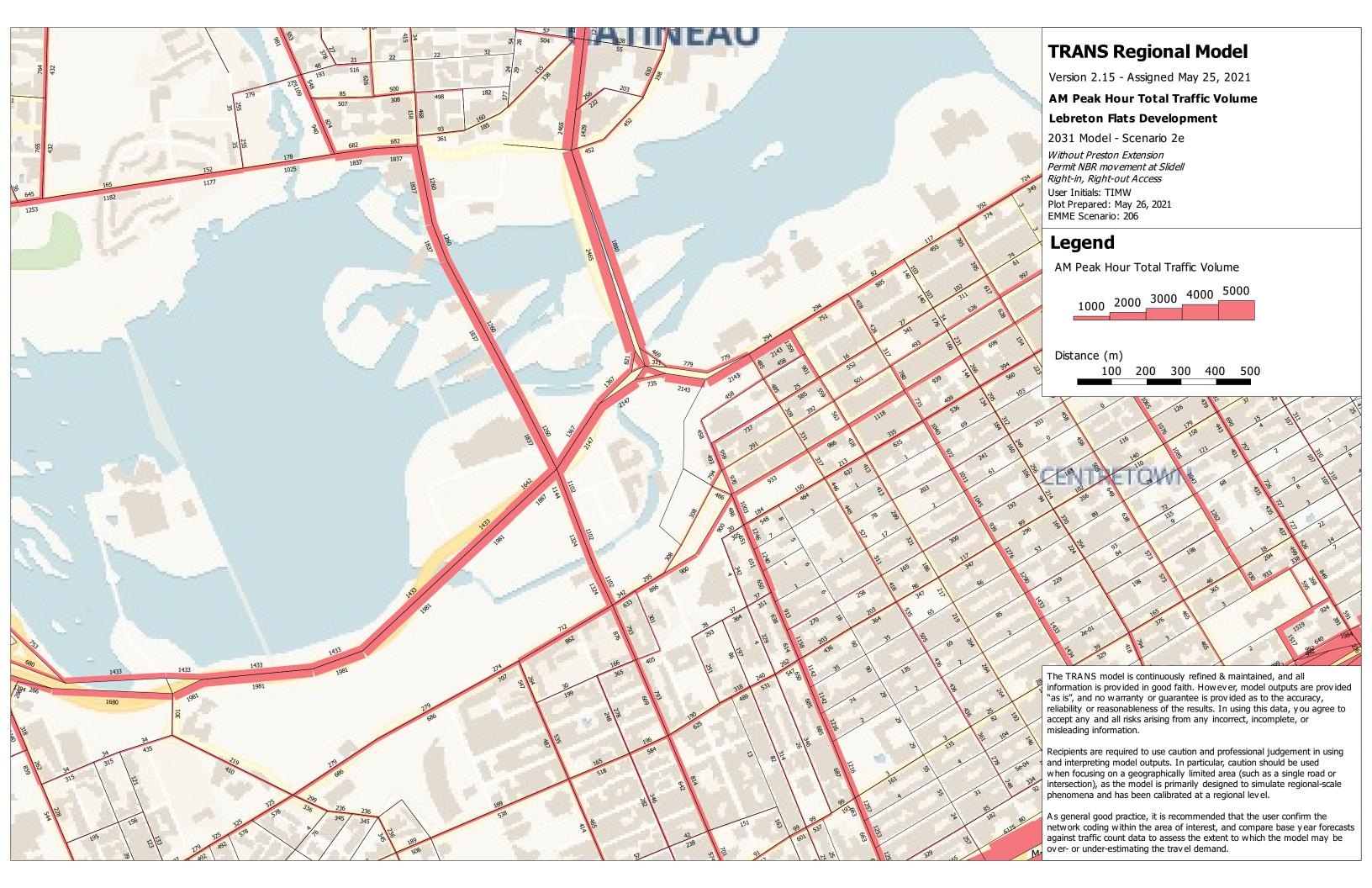
# **APPENDIX G: Preston Street Extension EMME Models**

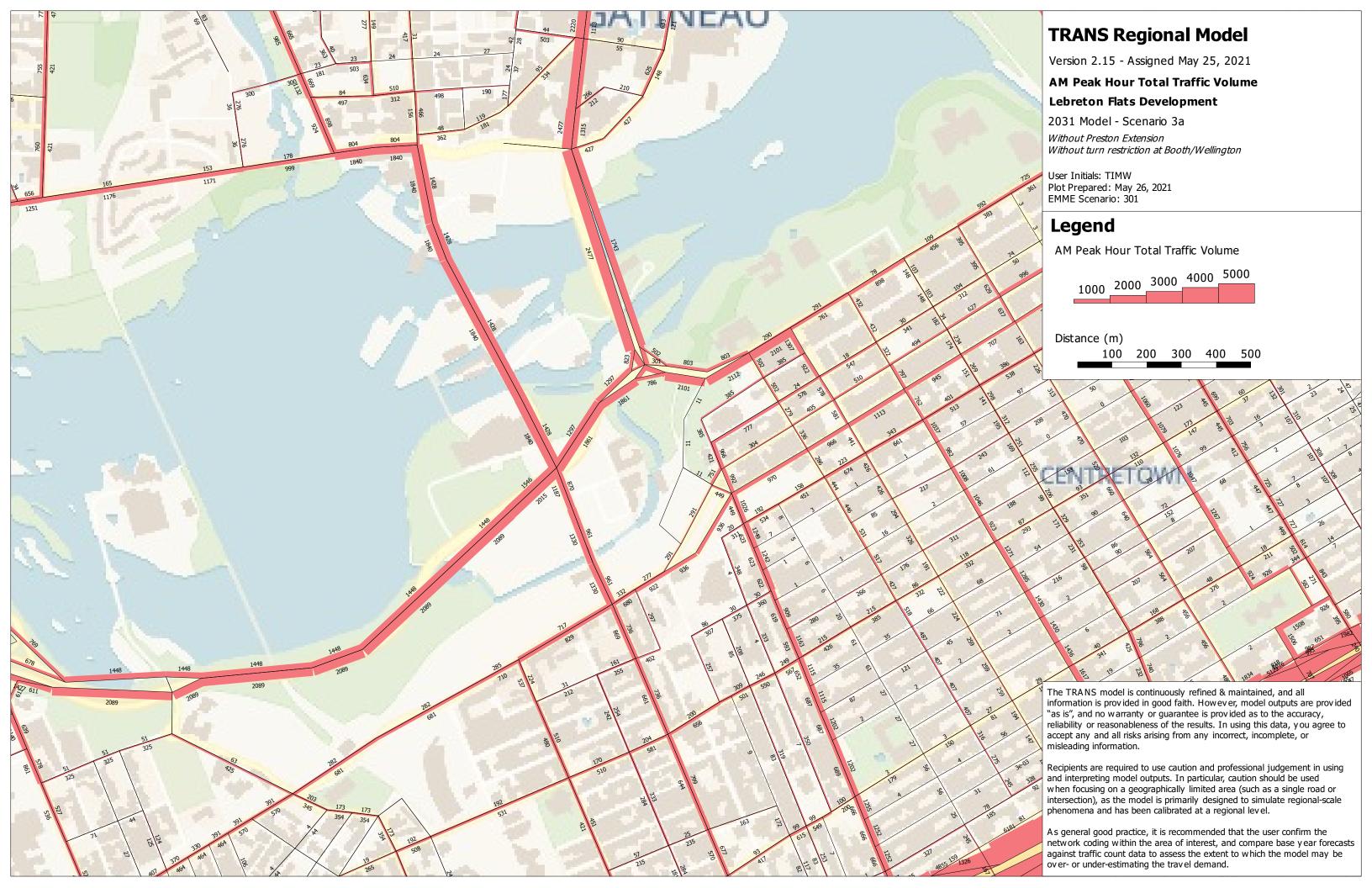


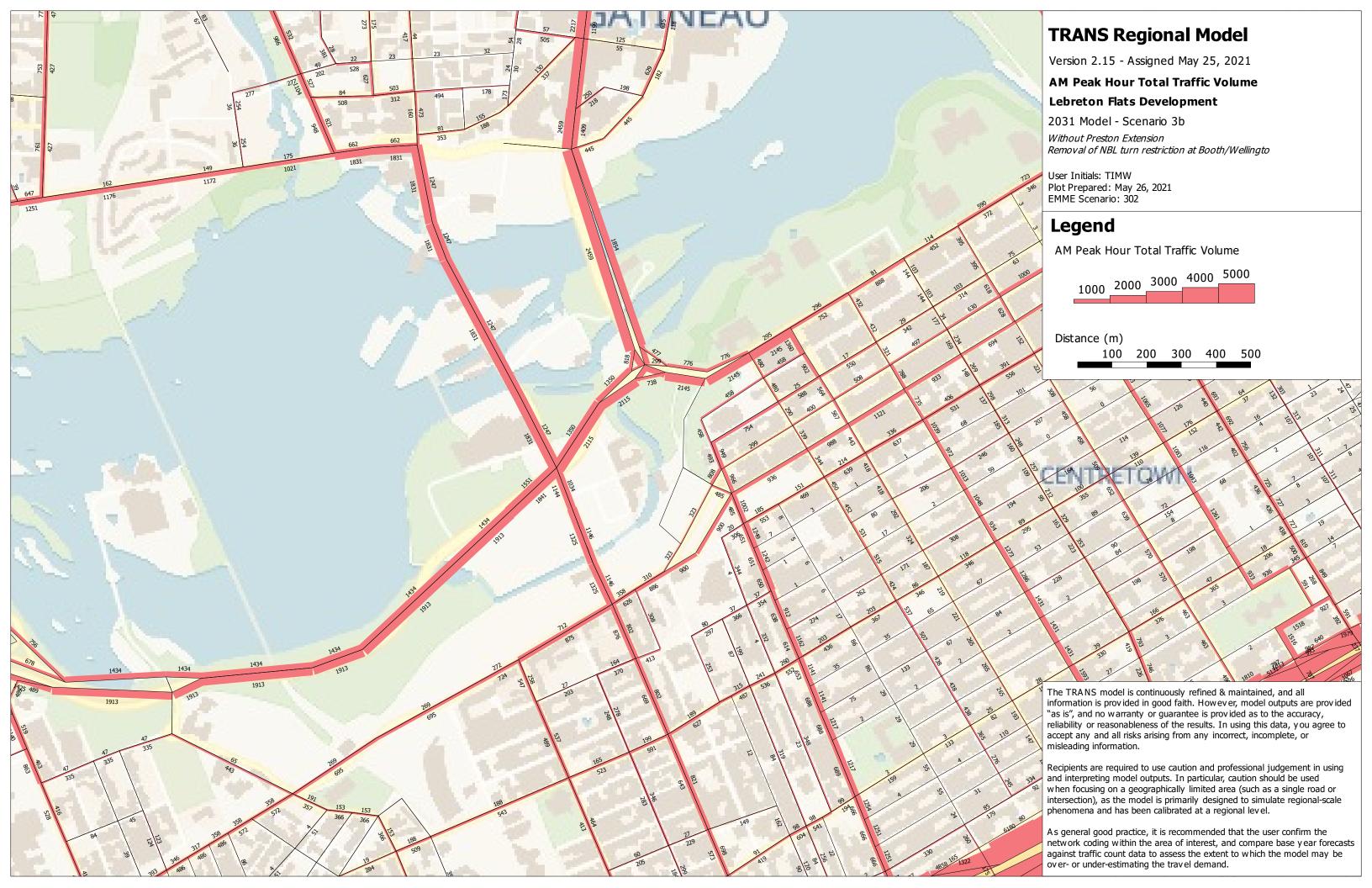












# **APPENDIX H: Transportation Demand Management Checklist**



## Introduction

The City of Ottawa's *Transportation Impact Assessment (TIA) Guidelines* (specifically Module 4.3—Transportation Demand Management) requires proponents of qualifying developments to assess the context, need and opportunity for transportation demand management (TDM) measures at their development. The guidelines require that proponents complete the City's **TDM Measures Checklist**, at a minimum, to identify any TDM measures being proposed.

The remaining sections of this document are:

- Using the Checklist
- Glossary
- TDM Measures Checklist: Non-Residential Developments
- TDM Measures Checklist: Residential developments

Readers are encouraged to contact the City of Ottawa's TDM Officer for any guidance and assistance they require to complete this checklist.

# **Using the Checklist**

The City's *TIA Guidelines* are designed so that *Module 3.1—Development-Generated Travel Demand*, *Module 4.1—Development Design*, and *Module 4.2—Parking* are complete before a proponent begins *Module 4.3—Transportation Demand Management*.

Within Module 4.3, *Element 4.3.1—Context for TDM* and *Element 4.3.2—Need and Opportunity* are intended to create an understanding of the need for any TDM measures, and of the results they are expected to achieve or support. Once those two elements are complete, proponents begin *Element 4.3.3—TDM Program* that requires proponents to identify proposed TDM measures using the **TDM Measures Checklist**, at a minimum. The *TIA Guidelines* note that the City may require additional analysis for large or complex development proposals, or those that represent a higher degree of performance risk; as well, proponents proposing TDM measures for a new development must also propose an implementation plan that addresses planning and coordination, funding and human resources, timelines for action, performance targets and monitoring requirements.

This **TDM Measures Checklist** document includes two actual checklists, one for non-residential developments (office, institutional, retail or industrial) and one for residential developments (multifamily, condominium or subdivision). Readers may download the applicable checklist in electronic format and complete it electronically, or print it out and complete it by hand. As an alternative, they may create a freestanding document that lists the TDM measures being proposed and provides additional detail on them, including an implementation plan as required by the City's *TIA Guidelines*.

Each measure in the checklist is numbered for easy reference. Each measure is also flagged as:

- BASIC —The measure is generally feasible and effective, and in most cases would benefit the development and its users.
- BETTER —The measure could maximize support for users of sustainable modes, and optimize development performance.
- —The measure is one of the most dependably effective tools to encourage the use of sustainable modes.

# **Glossary**

This glossary defines and describes the following measures that are identified in the **TDM Measures Checklist**:

## TDM program management

- Program coordinator
- Travel surveys

### **Parking**

Priced parking

## Walking & cycling

- Information on walking/cycling routes & destinations
- Bicycle skills training
- Valet bike parking

### Transit

- Transit information
- Transit fare incentives
- Enhanced public transit service
- Private transit service

### Ridesharing

- Ridematching service
- Carpool parking price incentives
- Vanpool service

## Carsharing & bikesharing

- Bikeshare stations & memberships
- Carshare vehicles & memberships

### **TDM marketing & communications**

- Multimodal travel information
- Personalized trip planning
- Promotions

### Other incentives & amenities

- Emergency ride home
- Alternative work arrangements
- Local business travel options
- Commuter incentives
- On-site amenities

For further information on selecting and implementing TDM measures (particularly as they apply to non-residential developments, with a focus on workplaces), readers may find it helpful to consult Transport Canada's *Workplace Travel Plans: Guidance for Canadian Employers*, which can be downloaded in English and French from the ACT Canada website at

www.actcanada.com/resources/act-resources.

# ► TDM program management

While some TDM measures can be implemented with a minimum of effort through routine channels (e.g. parking or human resources), more complex measures or a larger development site may warrant assigning responsibility for TDM program coordination to a designated person either inside or outside the implementing organization. Similarly, some TDM measures are more effective if they are targeted or customized for specific audiences, and would benefit from the collection of related information.

**Program coordinator**. This person is charged with day-to-day TDM program development and implementation. Only in very large employers with thousands of workers is this likely to be a full-time, dedicated position. Usually, it is added to an existing role in parking, real estate, human resources or environmental management. In practice, this role may be called TDM coordinator, commute trip reduction coordinator or employee transportation coordinator. The City of Ottawa can identify external resources (e.g. non-profit organizations or consultants) that could provide these services.

**Travel surveys.** Travel surveys are most commonly conducted at workplaces, but can be helpful in other settings. They identify how and why people travel the way they do, and what barriers and opportunities exist for different behaviours. They usually capture the following information:

- Personal data including home address or postal code, destination, job type or function, employment status (full-time, part-time and/or teleworker), gender, age and hours of work
- Commute information including distance or time for the trip between home and work, usual methods of commuting, and reasons for choosing them
- Barriers and opportunities including why other commuting methods are unattractive, willingness to consider other options, and what improvements to other options could make them more attractive

# ► Parking

**Priced parking.** Charging for parking is typically among the most effective ways of getting drivers to consider other travel options. While drivers may not support parking fees, they can be more accepting if the revenues are used to improve other travel options (e.g. new showers and change rooms, improved bicycle parking or subsidized transit passes). At workplaces or daytime destinations, parking discounts (e.g. early bird specials, daily passes that cost significantly less than the equivalent hourly charge, monthly passes that cost significantly less than the equivalent daily charge) encourage long-term parking and discourage the use of other travel options. For residential uses, unbundling parking costs from dwelling purchase, lease or rental costs provides an incentive for residents to own fewer cars, and can reduce car use and the costs of parking provision.

# ► Walking & cycling

Active transportation options like cycling and walking are particularly attractive for short trips (typically up to 5 km and 2 km, respectively). Other supportive factors include an active, health-conscious audience, and development proximity to high-quality walking and cycling networks. Common challenges to active transportation include rain, darkness, snowy or icy conditions, personal safety concerns, the potential for bicycle theft, and a lack of shower and change facilities for those making longer trips.

**Information on walking/cycling routes & destinations.** Ottawa, Gatineau and the National Capital Commission all publish maps to help people identify the most convenient and comfortable walking or cycling routes.

**Bicycle skills training.** Potential cyclists can be intimidated by the need to ride on roads shared with motor vehicles. This barrier can be reduced or eliminated by offering cycling skills training to interested cyclists (e.g. CAN-BIKE certification courses).

**Valet bike parking.** For large events, temporary "valet parking" areas can be easily set up to maximize convenience and security for cyclists. Experienced local non-profit groups can help.

### ► Transit

**Transit information.** Difficulty in finding or understanding basic information on transit fares, routes and schedules can prevent people from trying transit. Employers can help by providing online links to OC Transpo and STO websites. Transit users also appreciate visible maps and schedules of transit routes that serve the site; even better, a screen that shows real-time transit arrival information is particularly useful at sites with many transit users and an adjacent transit stop or station.

**Transit fare incentives.** Free or subsidized transit fares are an attractive incentive for non-transit riders to try transit. Many non-users are unsure of how to pay a fare, and providing tickets or a preloaded PRESTO card (or, for special events, pre-arranging with OC Transpo that transit fares are included with event tickets) overcome that barrier.

**Enhanced public transit service**. OC Transpo may adjust transit routes, stop locations, service hours or frequencies for an agreed fee under contract, or at no cost where warranted by the potential ridership increase. Information provided by a survey of people who travel to a given development can support these decisions.

**Private transit service.** At remote suburban or rural workplaces, a poor transit connection to the nearest rapid transit station can be an obstacle for potential transit users, and an employer in this situation could initiate a private shuttle service to make transit use more feasible or attractive. Other circumstances where a shuttle makes sense include large special events, or a residential development for people with limited independent mobility who still require regular access to shops and services.

# ► Ridesharing

Ridesharing's potential is greatest in situations where transit ridership is low, where parking costs are high, and/or where large numbers of car commuters (e.g. employees or full-time students) live reasonably far from the workplace.

Ridematching service. Potential carpoolers in Ottawa are served by www.OttawaRideMatch.com, an online service to help people find carpool partners. Employers can arrange for a dedicated portal where their employees can search for potential carpool partners only among their colleagues, if they desire. Some very large employers may establish internal ridematching services, to maximize employee uptake and corporate control. Ridematching service providers typically include a waiver to relieve employers of liability when their employees start carpooling through a ridematching service. Ridesharing with co-workers also tends to eliminate security concerns.

**Carpool parking price incentives.** Discounted parking fees for carpools can be an extra incentive to rideshare.

**Vanpool service.** Vanpools operate in the Toronto and Vancouver metropolitan areas, where vans that carry up to about ten occupants are driven by one of the vanpool members. Vanpools tend to operate on a cost-recovery basis, and are most practical for long-distance commutes where transit is not an option. Current legislation in Ontario does not permit third-party (i.e. private or non-profit) vanpool services, but does permit employers to operate internal vanpools.

# ► Carsharing & bikesharing

**Bikeshare station & memberships.** VeloGO Bike Share and Right Bike both operate bikesharing services in Ottawa. Developments that would benefit from having a bikeshare station installed at or near their development may negotiate directly with either service provider.

Carshare vehicles & memberships. VRTUCAR and Zipcar both operate carsharing services in Ottawa, for use by the general public or by businesses as an alternative to corporate fleets. Carsharing services offer 24-hour access, self-serve reservation systems, itemized monthly billings, and outsourcing of all financing, insurance, maintenance and administrative responsibilities.

# ► TDM marketing & communications

**Multimodal travel information.** Aside from mode-specific information discussed elsewhere in this document, multimodal information that identifies and explains the full range of travel options available to people can be very influential—especially when provided at times and locations where individuals are actively choosing among those options. Examples include: employees when their employer is relocating, or when they are joining a new employer; students when they are starting a program at a new institution; visitors or customers travelling to an unfamiliar destination, or when faced with new options (e.g. shuttle services or parking restrictions); and residents when they purchase or occupy a residence that is new to them.

**Personalized trip planning.** As an extension to the simple provision of information, this technique (also known as *individualized marketing*) is effective in helping people make more sustainable travel choices. The approach involves identifying who is most likely to change their travel choices (notably relocating employees, students or residents) giving them customized information, training and incentives to support them in making that change. It may be conducted with assistance from an external service provider with the necessary skills, and delivered in a variety of settings including workplaces and homes.

**Promotions.** Special events and incentives can raise awareness and encourage individuals to examine and try new travel options.

- Special events can help attract attention, build participation and celebrate successes. Events that have been held in Ottawa include Earth Day (in April) Bike to Work Month (in May), Environment Week (early June), International Car Free Day (September 22), and Canadian Ridesharing Week (October). At workplaces or educational institutions, similarly effective internal events could include workshops, lunch-and-learns, inter-departmental challenges, pancake breakfasts, and so on.
- Incentives can encourage trial of sustainable modes, and might include loyalty rewards for duration or consistency of activity (e.g. 1,000 km commuted by bicycle), participation prizes (e.g. for completing a survey or joining a special event), or personal recognition that highlights individual accomplishments.

### ► Other incentives & amenities

**Emergency ride home.** This measure assures non-driving commuters that they will be able to get home quickly and conveniently in case of family emergency (or in some workplaces, in case of unexpected overtime, severe weather conditions, or the early departure of a carpool driver) by offering a chit or reimbursement for taxi, carshare or rental car usage. Limits on annual usage or cost per employee may be set, although across North America the actual rates of usage are typically very low.

**Alternative work arrangements.** A number of alternatives to the standard 9-to-5, Monday-to-Friday workweek can support sustainable commuting (and work-life balance) at workplaces:

- Flexible working hours allow transit commuters to take advantage of the fastest and most convenient transit services, and allow potential carpoolers to include people who work slightly different schedules in their search for carpool partners. They also allow active commuters to travel at least one direction in daylight, either in the morning or the afternoon, during the winter.
- Compressed workweeks allow employees to work their required hours over fewer days (e.g. five days in four, or ten days in nine), eliminating the need to commute on certain days. For employees, this can promote work-life balance and gives flexibility for appointments. For employers, this can permit extended service hours as well as reduced parking demands if employees stagger their days off.
- Telework is a normal part of many workplaces. It helps reduce commuting activity, and can lead to significant cost savings through workspace sharing. Telework initiatives involve many stakeholders, and may face as much resistance as support within an organization. Consultation, education and training are helpful.

**Local business travel options.** A common obstacle for people who might prefer to not drive to work is that their employer requires them to bring a car to work so they can make business trips during the day. Giving employees convenient alternatives to private cars for local business travel during the workday makes walking, cycling, transit or carpooling in someone else's car more practical.

- Walking and cycling—Active transportation can be a convenient and enjoyable way to make short business trips. They can also reduce employer expenses, although they may require extra travel time. Providing a fleet of shared bikes, or reimbursing cyclists for the kilometres they ride, are inexpensive ways to validate their choice.
- Public transit—Transit can be convenient and inexpensive compared to driving.
   OC Transpo's PRESTO cards are transferable among employees and automatically reloadable, making them the perfect tool for enabling transit use during the day.
- *Ridesharing*—When multiple employees attend the same off-site meeting or event, they can be reminded to carpool whenever possible.
- Taxis or ride-hailing—Taxis and ride-hailing can eliminate parking costs, save time and eliminate collision liability concerns. Taxi chits eliminate cash transactions and minimize paperwork.
  - Fleet vehicles or carsharing—Fleet vehicles can be cost-effective for high travel volumes, while carsharing is a great option for less frequent trips.
  - o *Interoffice shuttles*—Employers with multiple worksites in the region could use a shuttle service to move people as well as mail or supplies.
  - Videoconferencing—New technologies mean that staying in the office to hold meetings electronically is more viable, affordable and productive than ever.

Commuter incentives. Financial incentives can help create a level playing field and support commuting by sustainable modes. A "commuting allowance" given to all employees as a taxable benefit is one such incentive; employees who choose to drive could then be charged for parking, while other employees could use the allowance for transit fares or cycling equipment, or for spending or saving. (Note that in the United States this practice is known as "parking cash-out," and is popular because commuting allowances are not taxable up to a certain limit). Alternatively, a monthly commuting allowance for non-driving employees would give drivers an incentive to choose a different commuting mode. Another practical incentive for active commuters or transit users is to offer them discounted "rainy day" parking passes for a small number of days each month.

**On-site amenities.** Developments that offer services to limit employees' need for a car during their commute (e.g. to drop off clothing at the dry cleaners) or during their workday (e.g. to buy lunch) can free employees to make the commuting decision that otherwise works best for them.

# **TDM Measures Checklist:**

Non-Residential Developments (office, institutional, retail or industrial)

# Legend The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	★ 1.1.1	Designate an internal coordinator, or contract with an external coordinator	To be determined by individual developers
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	The NCC could commission a travel survey every 5 years during the development to gauge the effectiveness of measures.
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & destin	
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances	This could be made to be a requirement of all developments in the LeBreton Flats area.
	2.2	Bicycle skills training	
		Commuter travel	
BETTER	★ 2.2.1	Offer on-site cycling courses for commuters, or subsidize off-site courses	To be determined by individual developers
	2.3	Valet bike parking	
		Visitor travel	
BETTER	2.3.1	Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	To be determined by individual developers

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances	This could be made to be a requirement of all developments in the LeBreton Flats area.
BASIC	3.1.2	Provide online links to OC Transpo and STO information	To be determined by individual developers
BETTER	3.1.3	Provide real-time arrival information display at entrances	This could be made to be a requirement of all developments in the LeBreton Flats area.
	3.2	Transit fare incentives	
		Commuter travel	
BETTER	3.2.1	Offer preloaded PRESTO cards to encourage commuters to use transit	To be determined by individual developers
BETTER ★	3.2.2	Subsidize or reimburse monthly transit pass purchases by employees	To be determined by individual developers
		Visitor travel	
BETTER	3.2.3	Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	To be determined by individual developers
	3.3	Enhanced public transit service	
		Commuter travel	Given the existing OC Transpo
BETTER	3.3.1	Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	service in the area, it is assumed this will be ongoing throughout the development of LeBreton Flats.
		Visitor travel	Given the existing OC Transpo
BETTER	3.3.2	Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	service in the area, it is assumed this will be ongoing throughout the development of LeBreton Flats.
	3.4	Private transit service	
		Commuter travel	
BETTER	3.4.1	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	To be determined by individual developers
		Visitor travel	
BETTER	3.4.2	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	To be determined by individual developers

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	4.	RIDESHARING	
	4.1	Ridematching service	
		Commuter travel	
BASIC *	4.1.1	Provide a dedicated ridematching portal at OttawaRideMatch.com	To be determined by individual developers
	4.2	Carpool parking price incentives	
		Commuter travel	
BETTER	4.2.1	Provide discounts on parking costs for registered carpools	This could be made to be a requirement of all developments in the LeBreton Flats area.
	4.3	Vanpool service	
		Commuter travel	
BETTER	4.3.1	Provide a vanpooling service for long-distance commuters	To be determined by individual developers
	5.	CARSHARING & BIKESHARING	
	5.1	Bikeshare stations & memberships	
BETTER	5.1.1	Contract with provider to install on-site bikeshare station for use by commuters and visitors	This could be made to be a requirement of all developments in the LeBreton Flats area.
		Commuter travel	
BETTER	5.1.2	Provide employees with bikeshare memberships for local business travel	To be determined by individual developers
	5.2	Carshare vehicles & memberships	
		Commuter travel	
BETTER	5.2.1	Contract with provider to install on-site carshare vehicles and promote their use by tenants	To be determined by individual developers
BETTER	5.2.2	Provide employees with carshare memberships for local business travel	To be determined by individual developers
	6.	PARKING	
	6.1	Priced parking	
		Commuter travel	
BASIC ★	6.1.1		
BASIC	6.1.2		This could be made to be a requirement of all developments in
			the LeBreton Flats area.
		Visitor travel	

	TDM	measures: Non-residential developments	С	heck if proposed & add descriptions
	7.	TDM MARKETING & COMMUNICATIONS		
	7.1	Multimodal travel information		
		Commuter travel		
BASIC *	7.1.1	Provide a multimodal travel option information package to new/relocating employees and students		To be determined by individual developers
	_	Visitor travel		
BETTER ★	7.1.2	Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)		To be determined by individual developers
	7.2	Personalized trip planning		
		Commuter travel		
BETTER ★	7.2.1	Offer personalized trip planning to new/relocating employees		To be determined by individual developers
	7.3	Promotions		
		Commuter travel		
BETTER	7.3.1	Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes		To be determined by individual developers
	8.	OTHER INCENTIVES & AMENITIES		
	8.1	Emergency ride home		
		Commuter travel		
BETTER ★	8.1.1	Commuter travel Provide emergency ride home service to non-driving commuters		To be determined by individual developers
BETTER *	8.1.1 <b>8.2</b>	Provide emergency ride home service to non-driving		
BETTER *		Provide emergency ride home service to non-driving commuters		
		Provide emergency ride home service to non-driving commuters  Alternative work arrangements		individual developers
	<b>8.2</b> 8.2.1	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel		
BASIC ★	<b>8.2</b> 8.2.1	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours		individual developers  To be determined by
BASIC ★ BETTER	<b>8.2</b> 8.2.1 8.2.2	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks		individual developers  To be determined by
BASIC ★ BETTER	8.2.1 8.2.2 8.2.3	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework		individual developers  To be determined by
BASIC ★ BETTER	8.2.1 8.2.2 8.2.3	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options		individual developers  To be determined by
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the		To be determined by individual developers  To be determined by individual developers
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3.3	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work		To be determined by individual developers  To be determined by individual developers
BASIC ★ BETTER BETTER ★	8.2.1 8.2.2 8.2.3 8.3.3	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter incentives		To be determined by individual developers  To be determined by individual developers
BASIC ★ BETTER ★ BETTER ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1 8.4	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter incentives  Commuter travel  Offer employees a taxable, mode-neutral commuting		To be determined by individual developers  To be determined by individual developers  To be determined by individual developers
BASIC ★ BETTER ★ BETTER ★	8.2.1 8.2.2 8.2.3 8.3 8.3.1 8.4 8.4.1	Provide emergency ride home service to non-driving commuters  Alternative work arrangements  Commuter travel  Encourage flexible work hours  Encourage compressed workweeks  Encourage telework  Local business travel options  Commuter travel  Provide local business travel options that minimize the need for employees to bring a personal car to work  Commuter incentives  Commuter travel  Offer employees a taxable, mode-neutral commuting allowance		To be determined by individual developers  To be determined by individual developers  To be determined by individual developers

# **TDM Measures Checklist:**

Residential Developments (multi-family, condominium or subdivision)

# The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions					
	1.	TDM PROGRAM MANAGEMENT						
	1.1	Program coordinator						
BASIC *	1.1.1	Designate an internal coordinator, or contract with an external coordinator	To be determined by individual developers					
	1.2	Travel surveys						
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	The NCC could commission a travel survey every 5 years during the development to gauge the effectiveness of measures.					
	2.	WALKING AND CYCLING						
	2.1	Information on walking/cycling routes & des	tinations					
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	This could be made to be a requirement of all developments in the LeBreton Flats area.					
	2.2	Bicycle skills training						
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	To be determined by individual developers					

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	This could be made to be a requirement of all developments in the LeBreton Flats area.
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	This could be made to be a requirement of all developments in the LeBreton Flats area.
	3.2	Transit fare incentives	
BASIC #	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	To be determined by individual developers
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	To be determined by individual developers
	3.3	Enhanced public transit service	
BETTER #	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	Given the existing OC Transpo service in the area, it is assumed this will be ongoing throughout the development of LeBreton Flats.
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	To be determined by individual developers
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station ( <i>multi-family</i> )	This could be made to be a requirement of all developments in the LeBreton Flats area.
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	To be determined by individual developers
	4.2	Carshare vehicles & memberships	
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	To be determined by individual developers
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	To be determined by individual developers
	5.	PARKING	
	5.1	Priced parking	
BASIC	5.1.1	Unbundle parking cost from purchase price (condominium)	This could be made to be a requirement of all developments in the LeBreton Flats area.
BASIC	5.1.2	Unbundle parking cost from monthly rent (multi-family)	This could be made to be a requirement of all developments in the LeBreton Flats area.

TDM	measures: Residential developments		Check if proposed & add descriptions
6.	TDM MARKETING & COMMUNICATION	S	
6.1	Multimodal travel information		
BASIC ★ 6.1.1	Provide a multimodal travel option information package to new residents		To be determined by individual developers
6.2	Personalized trip planning		
<b>BETTER</b> ★ 6.2.1	Offer personalized trip planning to new residents		To be determined by individual developers

# **APPENDIX I: Intersection MMLOS Analysis**



	INTERSECTIONS		Wellingt	on / Vimy			Booth / V	Vellington	
	Designation/Policy Area		Within 600m of a R	apid Transit Station			Within 600m of a R	apid Transit Station	
	Roadway Classification		Art	erial			Art	erial	
	Approach	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound
	Total travel lanes crossed	-	2	5	5	4	5	4	5
	Centre Median (>2.4m)		No	No	No	No	No	Yes	Yes
	Left-Turn Conflict		Perm.	None/Prohibited	Perm.	None/Prohibited	None/Prohibited	None/Prohibited	Prot./Perm.
	Right-Turn Conflict		Perm./Yield	Prot./Perm.	None	None	Perm./Yield	Perm./Yield	Perm./Yield
	RTOR		Perm.	Perm.	Perm.	Perm.	Perm.	Prohibited	Prohibited
	Leading Ped Interval		No	No	No	No	No	No	No
	Right-Turn Channel	-	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn	No Right-Turn Channel
S	Right-Turn Corner Radius	-	> 5m to 10m	> 10m to 15m	No Right-Turn	No Right-Turn	> 5m to 10m	> 5m to 10m	> 5m to 10m
riar	Crosswalk Treatment	-	Standard Transverse Markings	Standard Transverse Markings	Standard Transverse Markings		Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings
esti	PETSI Score	-	86	45	48	75	49	74	47
ede	PETSI LOS	-	В	D	D	В	D	С	D
•	Cycle Length (s)			95				95	
	Directional Split (s)	-	62	33	33	48	48	47	35
	FDW + Intergreen	-	17	25.3	25.3	25.8	25.8	21.9	21.9
	Pedestrian Delay (s)	-	13	40	40	28	28	26	35
	Delay LOS	-	В	D	D	С	С	С	D
	Resultant LOS	-	В	D	D	С	D	С	D
	Overall Intersection			D			l l	D	
	Target LOS			A				A	
	Route Classification			Route				Route	
	Facility	-	Mixed Traffic	Mixed Traffic	Mixed Traffic	Bike Lane (or better)	Bike Lane (or better)	Bike Lane (or better)	Bike Lane (or better)
	Right-Turn Type	-	Shared Lane	Shared Lane	Shared Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane
	Turning Bay Length (m)	-	> 50	<= 50	> 50	> 50	> 50	<= 50	> 50
	Veh. Turning Speed (km/h)	-	-	<= 25	-	<= 30	<= 30	<= 25	<= 30
ists	Right-Turn LOS	-	F	D	F	A	A	A	A
)cli	Left-Turn Type	-	Single Left	Single Left	-	Cycle Track	Cycle Track	Cycle Track	Cycle Track
Q.	# of Lanes Crossed	-	1	2+	-	-	-	-	-
	Veh. Approach Speed (km/h)	-	50	>= 60	-	-	-	-	-
	Left-Turn LOS	-	D	F	-	A	A	A	A
	Resultant LOS	-	F	F	F	A	A	A	A
	Overall Intersection			F				A	
	Target LOS			C .			TD leaded	G	
	Transit Facility			<u>-</u> I		> 40		ed Measures	T
ısit	Transit Delay (s)	-	-	-	-	> 40 F	<= 40	-	-
ra	Resultant LOS	-	-	-	-	F	Е	-	-
_	Overall Intersection Target LOS			<u>-                                      </u>				F D	
							Truck	_	
	Truck Facility Effective Corner Radius (m)			-		< 10	< 10		<u> </u>
~		-	-	-	-			-	-
2	# of Receiving Lanes Resultant LOS	-	-	-	-	2+ D	2+ D	-	-
F		-	<u>-</u>	-	-	D			-
	Overall Intersection Target LOS			·				D D	
	Target LOS			-				U	

INTERSECTIONS		Booth	/ Albert			Albert	Preston		Albert / City Centre				
Designation/Policy Area		Within 600m of a R	apid Transit Station				apid Transit Station		Within 600m of a Rapid Transit Station				
Roadway Classification			erial				erial			Arte			
Approach	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	
Total travel lanes crossed	3	5	6	7	3	-	6	6	3	2	6	6	
Centre Median (>2.4m)	No	No	No	No	No	-	No	No	No	No	No	No	
Left-Turn Conflict	Perm.	Prot/Perm.	Perm.	Prot./Perm.	Prot/Perm.	-	Perm.	None/Prohibited	Perm.	Prot./Perm.	Perm.	Perm.	
Right-Turn Conflict	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	-	None	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	
RTOR	Perm.	Perm.	Perm.	Prohibited	Perm.	-	Perm.	Prohibited	Perm.	Perm.	Perm.	Perm.	
Leading Ped Interval	No	No	No	No	No	-	No	No	No	No	No	No	
Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	-	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	
Right-Turn Corner Radius	> 5m to 10m	> 5m to 10m	> 15m to 25m	> 5m to 10m	> 5m to 10m	-	No Right-Turn	> 5m to 10m	> 10m to 15m	> 5m to 10m	> 10m to 15m	> 10m to 15m	
Crosswalk Treatment	Zebra Stripe Hi-Vis Markings		-	Zebra Stripe Hi-Vis Markings			Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings					
PETSI Score	74	41	21	11	74	-	34	35	73	89	23	23	
PETSILOS	С	E	F	F	С	-	E	E	С	В	F	F	
Cycle Length (s)			20				20		<u> </u>	. 12			
Directional Split (s)	55	37	65	40	65	-	30	30	84	67	36	36	
FDW + Intergreen	29.5	29.5	27.5	27.5	24.2	-	22.3	22.3	20.3	20.3	22.3	22.3	
Pedestrian Delay (s)	37	53	28	48	26	-	53	53	13	22	47	47	
Delay LOS	D	E	С	E	C	-	E	E	В	С	E	E	
Resultant LOS	D	E	F	F	С	-	E	E	С	С	F	F	
Overall Intersection			F				E						
Target LOS			Α				Α		A				
Route Classification			vn Bikeway				wn Bikeway		Cross-Town Bikeway				
Facility Right-Turn Type	Mixed Traffic	Mixed Traffic	Mixed Traffic	Bike Lane (or better)	Mixed Traffic	•	Mixed Traffic	Bike Lane (or better)	Mixed Traffic	-	Mixed Traffic	Bike Lane (or better)	
	Shared Lane	Shared Lane	Shared Lane	Cycle Track	Shared Lane	-	Shared Lane	-	Shared Lane	-	Shared Lane	-	
Turning Bay Length (m)	> 50	> 50	> 50	-	> 50	-	> 50	-	> 50	-	> 50	-	
Veh. Turning Speed (km/h)	- F	- F	- F	-	- F		- F	-		-	- F	-	
Right-Turn LOS				Α		•		-		-			
Left-Turn Type # of Lanes Crossed	Single Left 2+	Single Left 2+	Single Left	Single Left 2+	Single Left 0	•	-	Single Left 2+	Single Left	-	Single Left 2+	Single Left 2+	
Veh. Approach Speed (km/h)	50	2+ 50	50	50	<= 40	•	-	50	<= 40	-	50	50	
ven. Approach Speed (km/h) Left-Turn LOS	50	50	50	50	<= 40 B	-		50	<= 40 B	-	50	50	
Resultant LOS	-		В		F	-	-		В Е	-		F	
Overall Intersection	F	F	F	F	F		F	<u> </u>	-	-		-	
Target LOS			^				^						
Transit Facility		TP Contin	uous Lanes			TR Contin	uous Lanes			TP - Contin	wous Lance		
# Transit Delay (s)		> 40	<= 40	> 40	<= 30		<= 40	<= 40		· ·	<= 10	<= 10	
Resultant LOS		, 40 F	K= 40	F F	D D	· · · · · · · · · · · · · · · · · · ·	E E	E E	-		N= 10	B	
Overall Intersection					5		E				a S		
Target LOS			C				c						
Truck Facility		Truck	Route			Truck	Route			Truck	Poute		
Effective Corner Radius (m)	< 10	> 15	< 10	< 10	< 10		< 10		10 - 15	- ITUCK	10 - 15		
# of Receiving Lanes	2+	2+	1	2+	2+		1		2+		1		
Resultant LOS	 D	A A	F	D D	D D		F		27 В	-	F		
	5	_ ^			_ B		· · · · · · · · · · · · · · · · · · ·						
Overall Intersection Target LOS			F				F						

INTERSECTIONS		Wellingto	on / Vimy			Booth / V	Vellington		Booth / Albert				
Designation/Policy Area		Within 600m of a R	apid Transit Station			Within 600m of a R	apid Transit Station		Within 600m of a Rapid Transit Station				
Roadway Classification		Arte	erial			Art	erial			Artı	erial		
Approach	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	
Total travel lanes crossed	2	2	5	5	4	5	4	5	3	5	6	7	
Centre Median (>2.4m)	No	No	No	No	No	No	Yes	Yes	No	No	No	No	
Left-Turn Conflict	Perm.	Perm.	None/Prohibited	Perm.	None/Prohibited	None/Prohibited	None/Prohibited	Prot/Perm.	Perm.	Prot./Perm.	Perm.	Prot./Perm.	
Right-Turn Conflict	Perm./Yield	Perm./Yield	Prot/Perm.	None	None	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	
RTOR	Perm.	Perm.	Perm.	Perm.	Perm.	Perm.	Prohibited	Prohibited	Perm.	Perm.	Perm.	Prohibited	
Leading Ped Interval	No	No	No	No	No	No	No	No	No	No	No	No	
Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channe	
Right-Turn Corner Radius	> 5m to 10m	> 5m to 10m	> 5m to 10m	No Right-Turn	No Right-Turn	> 5m to 10m	> 5m to 10m	> 5m to 10m	> 5m to 10m	> 5m to 10m	> 15m to 25m	> 5m to 10m	
Crosswalk Treatment		Standard Transverse Markings					Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Marki	
PETSI Score	86	86	46	48	75	49	74	47	74	41	21	11	
PETSILOS	В	В	D	D	В	D	С	D	С	E	F	F	
Cycle Length (s)			5				5				20		
Directional Split (s)	62	62	33	33	48	48	47	35	55	37	65	40	
FDW + Intergreen	17	17	25.3	25.3	25.8	25.8	21.9	21.9	29.5	29.5	27.5	27.5	
Pedestrian Delay (s)	13	13	40	40	28	28	26	35	37	53	28	48	
Delay LOS	В	В	D	D	С	С	С	D	D	E	С	E	
Resultant LOS	В	В	D	D	С	D	С	D	D	E	F	F	
Overall Intersection		į.	)				)				F		
Target LOS			١				Α						
Route Classification		Spine				Spine			Cross-Town Bikeway				
Facility	Bike Lane (or better)	Bike Lane (or better)	Mixed Traffic	Mixed Traffic	Bike Lane (or better)	Bike Lane (or better)	Bike Lane (or better)	Bike Lane (or better)	Mixed Traffic	Mixed Traffic	Mixed Traffic	Bike Lane (or better)	
Right-Turn Type	Curbside	Curbside	Shared Lane	Shared Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Shared Lane	Shared Lane	Shared Lane	Cycle Track	
Turning Bay Length (m)	-	-	<= 50	> 50	> 50	> 50	<= 50	> 50	> 50	> 50	> 50	-	
Veh. Turning Speed (km/h)	-	-	<= 25	-	<= 30	<= 30	<= 25	<= 30	-	-	-	-	
Right-Turn LOS	A	A	D	F	A	A	A	A	F	F	F	A	
Left-Turn Type	Single Left	Single Left	Single Left	Single Left	Cycle Track	Cycle Track	Cycle Track	Cycle Track	Single Left	Single Left	Single Left	Single Left	
# of Lanes Crossed	1	1	2+	2+	-	-	-	-	2+	2+	0	2+	
Veh. Approach Speed (km/h)	<= 40	<= 40	>= 60	>= 60	-	į.	-		50	50	50	50	
Left-Turn LOS	В	В	F	F	A	A	A	A	F	F	В	F	
Resultant LOS	В	В	F	F	A	A	A	A	F	F	F	F	
Overall Intersection							Α						
Target LOS			,			TP - Isolate				70.0	uous Lanes		
Transit Facility													
Transit Delay (s)	-	-	-	-	> 40 F	<= 40	-	-	-	> 40	<= 40	> 40	
Resultant LOS	-	-	-	-	F	E	-		-	F	E	F	
Overall Intersection						_			_				
Target LOS							Route			Truck			
Truck Facility					- 10				140			- 40	
Effective Corner Radius (m)		-		-	< 10	< 10	-	-	< 10	> 15	< 10	< 10	
# of Receiving Lanes		-		-	2+	2+	-	-	2+	2+	1	2+	
Resultant LOS	-	-	-	-	D	D	-	-	D	A	F	D	
Overall Intersection			-										
Target LOS													

INTERSECTIONS		Albert /	Preston			Albert / C	ity Centre		Wellington / Broad				
Designation/Policy Area		Within 600m of a R	apid Transit Station			Within 600m of a R	apid Transit Station		Within 600m of a Rapid Transit Station				
Roadway Classification		Art	erial			Arte	erial				Arterial		
Approach	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	
Total travel lanes crossed	3	2	6	6	3	2	6	6	2		6	6	
Centre Median (>2.4m)	No	No	No	-	No	No							
Left-Turn Conflict	Prot./Perm.	Prot/Perm.	Perm.	None/Prohibited	Perm.	Prot./Perm.	Perm.	Perm.	Perm.	-	Perm.	None/Prohibited	
Right-Turn Conflict	Perm./Yield	Perm./Yield	None	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	-	None	Perm./Yield	
RTOR	Perm.	Perm.	Perm.	Prohibited	Perm.	Perm.	Perm.	Perm.	Perm.		Perm.	Perm.	
Leading Ped Interval	No	No	No		No	No							
Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Channel		No Right-Turn Channel	No Right-Turn Channe	
Right-Turn Corner Radius	> 5m to 10m	> 5m to 10m	No Right-Turn	> 5m to 10m	> 10m to 15m	> 5m to 10m	> 10m to 15m	> 10m to 15m	> 5m to 10m		> 5m to 10m	> 5m to 10m	
	Zebra Stripe Hi-Vis Markings				Standard Transverse Markings								
PETSI Score	74	89	34	35	73	89	23	23	86		26	29	
PETSILOS	С	В	E	E	С	В	F	F	В	-	F	F	
Cycle Length (s)			20				20				95		
Directional Split (s)	65	65	30	30	84	67	36	36	62	-	33	33	
FDW + Intergreen	24.2	24.2	22.3	22.3	20.3	20.3	22.3	22.3	17		25.3	25.3	
Pedestrian Delay (s)	26	26	53	53	13	22	47	47	13		40	40	
Delay LOS	С	С	E	E	В	С	E	E	В		D	D	
Resultant LOS	С	С	E.	Е	С	С	F	F	В	-	F	F	
Overall Intersection			E				F						
Target LOS			A				A		Α			/	
Route Classification		Cross-Tov	vn Bikeway				vn Bikeway		Spine Route				
Facility	Mixed Traffic	-	Mixed Traffic	Bike Lane (or better)	Mixed Traffic	Mixed Traffic	Mixed Traffic	Bike Lane (or better)	Mixed Traffic	-	Mixed Traffic	Mixed Traffic	
Right-Turn Type	Shared Lane	-	Shared Lane	•	Shared Lane	Shared Lane	Shared Lane	-	Shared Lane		Shared Lane	Shared Lane	
Turning Bay Length (m)	> 50	-	> 50	-	> 50	<= 50	> 50	-	> 50		> 50	> 50	
Veh. Turning Speed (km/h)	-	-	-	-	-	<= 25	-	-	-	•	-		
Right-Turn LOS	F	-	F	•	F	D	F	-	F	-	F	F	
Left-Turn Type	Single Left	-	-	Single Left	Single Left	Single Left	Single Left	Single Left	Single Left		-	Single Left	
# of Lanes Crossed	0	-	-	2+	1	1	2+	2+	0	-	-	2+	
Veh. Approach Speed (km/h)	<= 40	-	-	50	<= 40	50	50	50	<= 40	•	-	>= 60	
Left-Turn LOS	B	-		F	В	D	F	F	В	-	-	F	
Resultant LOS	F	-	F	F	F	D	F	F	F		F	F	
Overall Intersection											<u> </u>		
Target LOS		TO 0	uous Lanes			TP - Contin	A				t .		
Transit Facility	. 00			. 40				. 40			-		
Transit Delay (s)	<= 30 D	-	<= 40	<= 40	-	-	<= 10 B	<= 10 B	-	•	-	-	
Resultant LOS  Overall Intersection	υ	-	E	Е	-	-	B	В	-	-		-	
Target LOS							^				-		
Truck Facility		Touris	Route			Torrete	Route						
Truck Facility	< 10	- Iruck	< 10	-	10 - 15	- Iruck	10 - 15		-		Ť -		
Effective Corner Padius ()		_			10 - 15		10 - 15			-			
Effective Corner Radius (m)	2.							-				-	
# of Receiving Lanes	2+	-	1	•									
Effective Corner Radius (m) # of Receiving Lanes Resultant LOS Overall Intersection	2+ D		1 F		В В		Ē		-	-		-	

INTERSECTIONS			gton / Lett			Albert /			Albert / Access 1				
Designation/Policy Area			Rapid Transit Station				apid Transit Station		Within 600m of a Rapid Transit Station				
Roadway Classification		A	rterial			Arte	rial			Art	erial		
Approach	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	Westbound	
Total travel lanes crossed	3	-	7	7	2	2	6	6	-	2	6	6	
Centre Median (>2.4m)	No	-	No	No	No	No	No	No	-	No	No	No	
Left-Turn Conflict	Perm.	-	Perm.	None/Prohibited	Perm.	Perm.	Perm.	Perm.	-	Perm.	Perm.	Perm.	
Right-Turn Conflict	Perm./Yield	-	None	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	Perm./Yield	-	Perm./Yield	Perm./Yield	Perm./Yield	
RTOR	Perm.	-	Perm.	Perm.	Perm.	Perm.	Perm.	Perm.	-	Perm.	Perm.	Perm.	
Leading Ped Interval	No	-	Yes	Yes	No	No	No	No	-	No	No	No	
Right-Turn Channel	No Right-Turn Channel	-	No Right-Turn Channel	-	No Right-Turn Channel	No Right-Turn Channel	No Right-Turn Chann						
Right-Turn Corner Radius	> 5m to 10m	-	> 5m to 10m	No Right-Turn	> 5m to 10m	-	> 5m to 10m	> 5m to 10m	> 5m to 10m				
Crosswalk Treatment	Zebra Stripe Hi-Vis Markings	-	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Standard Transverse Markings	Standard Transverse Markings	Standard Transverse Markings	Standard Transverse Markings	-	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Markings	Zebra Stripe Hi-Vis Mark	
PETSI Score	74	-	15	23	86	-	21	21		89	24	24	
PETSILOS	C		F	F	В	-	F	F		В	F	F	
Cycle Length (s)			95		_		20				10		
Directional Split (s)	61		34	34	81	81	39	39		80	30	30	
FDW + Intergreen	26.9		14.8	14.8	9.6	9.6	22.3	22.3		9.7	21	21	
Pedestrian Delay (s)	20		30	30	10	10	44	44		7	46	46	
Delay LOS	R R		G	C	B	B	F	F		A	F	F	
Resultant LOS	Č		F	F	B	B	F	F		B	F	Ē	
Overall Intersection	ű		F	·	5	, and a				5	F		
Target LOS			À								Δ.		
Route Classification		Snir	ne Route			Cross-Tov	n Bikeway		Cross-Town Bikeway				
Facility	Mixed Traffic		Mixed Traffic	Mixed Traffic	Bike Lane (or better)	-	Mixed Traffic	Mixed Traffic	Bike Lane (or better)				
Right-Turn Type	Shared Lane	-	Shared Lane	Shared Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	Veh. Crosses Bike Lane	-	Shared Lane	Shared Lane	Veh. Crosses Bike Lan	
Turning Bay Length (m)	> 50	_	> 50	> 50	<= 50	<= 50	<= 50	<= 50		<= 50	> 50	<= 50	
Veh. Turning Speed (km/h)			-	-	<= 25	<= 25	<= 25	<= 25		<= 25	-	<= 25	
Right-Turn LOS	F		F	F	В	В	В	В		D	F	B	
Left-Turn Type	Single Left	-	-	Single Left	Crossride	Crossride	Crossride	Crossride		Single Left	Single Left		
# of Lanes Crossed	0	-	-	2+	-	-	-	-		1	2+		
Veh. Approach Speed (km/h)	<= 40	-	-	>= 60	<= 40	<= 40	50	50	-	<= 40	50		
Left-Turn LOS	В	-	-	F	-	-	-		-	В	F		
Resultant LOS	F	-	F	F	В	В	В	В	-	D	F	В	
Overall Intersection			F	•			3				F		
Target LOS			С				١				A		
Transit Facility						TP - Contin	uous Lanes			TP - Contin	uous Lanes		
Transit Delay (s)	-		-	-	<= 30	-	<= 20	<= 10		-	> 40	<= 20	
Resultant LOS	-		-	-	D	-	C	В		-	F	C	
			-	1							F		
Overall Intersection			-								С		
Target LOS						Truck	Route			Truck	Route		
Target LOS			1 .	-		-	-	-	-	-	-	-	
Target LOS Truck Facility Effective Corner Radius (m)			-	-		-					-		
Target LOS Truck Facility Effective Corner Radius (m) # of Receiving Lanes	-	-	-	-	-	-		-		-	-	-	
Target LOS Truck Facility Effective Corner Radius (m)			-			-							