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Functional Serviceability Report

Official Plan Amendment and Zoning By-law Amendment
16 Storey High Rise Apartment Building
1400 Bank Street, Ottawa ON

Prepared for:

Serco Realty Group
9 Capella Court
Unit 200, Ottawa ON.
K2E 8A7

Attention: Loredena Parcari

LRL File No.: 210617

October 26th, 2021



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1 INTRODUCTION AND SITE DESCRIPTION

LRL Associates LTD. was retained by Serco Realty Group to prepare a functional serviceability report to support the Official Plan Amendment and Zoning By-law amendment application for the property located at 1400 Bank Street within the City of Ottawa.

The subject site is within the Capital Ward, located on the west side of Bank Street and has an approximate area of 0.1814 ha, south of Belanger Avenue and north of Rockingham Street. The land is currently occupied by a one storey retail building with a paved parking area, brick walkway and some grass. The subject site can be seen below in figure 1.



Figure 1: Aerial View of Subject Lands

Under the City of Ottawa Zoning by-law, the existing land is currently zoned Arterial Mainstreet, AM, Exception 1913 (AM1, [1913]). This servicing study has been prepared to support an Official Plan amendment and Zoning By-law amendment required to add a high-rise building as a permitted use on the Subject Property which is further expanded in in the planning documentation accompanying the submission.

The serviceability review summarized in this document has been completed to further investigate the potential for this property to be redeveloped to accommodate a 16-storey mixed use residential building with approximately 160 residential units and 6 commercial and office units. The concept site plan prepared by Figurr Architects, illustrating the proposed development can be found in appendix A at the back of this report.

Following the Official Plan Amendment and Zoning By-law amendment application, a detailed design will advance with intentions for full Site Plan Control application submission.



2 EXISTING SITE AND AVAILABLE SERVICES

J. D. Barnes Limited prepared a topographic survey of the subject property in January of 2020 which has been included in appendix B for reference. Based on the topography information available, the general elevation of the land is relatively flat.

Utilizing the GeoOttawa mapping portal as well as plan and profile drawings provided by the City of Ottawa indicate the following services are running along Bank Street and Belanger Avenue within the right-of-way in front of the property boundary.

Bank Street:

- 375 mm Sanitary Sewer
- 205 mm Water Main
- 305 mm Storm Sewer

Belanger Avenue:

- 300 mm Sanitary Sewer
- 305 mm Water Main
- 375 mm Storm Sewer

The existing building located central on the site is currently serviced to meet the domestic demand; however, these will be abandoned to accommodate the proposed development of a greater population.

3 WATER SUPPLY SERVICING

The site is intended to be serviced through a new 150 mm diameter water service connecting to the existing 305 mm PVC watermain within Bank Street.

Based on the location and available data from the city, the subject property lies within the 2C pressure zone of the water distribution system.

3.1 Domestic Water Demands

Proposed populations have been interpreted from the architectural drawings. Based on the number of residential units within the building combined with the City of Ottawa design guidelines for population projection, the number of units translates to approximately 309 residents. Table 1 below summarizes the proposed population count based on the residential units as interpreted using table 4.1 of the City of Ottawa design guidelines.

Table 1: Development Residential Population Estimate

Proposed Unit Type	Persons Per Unit	Number of Units	Total Population
1 Bedroom unit	1.8	91	163.8
2 Bedroom Unit	2.1	69	144.9
Total Residential Population			308.7



Table 2 below summarizes the water supply guidelines which must be employed during detailed design and sizing of the service to the building.

Table 2: Water Supply Guidelines

Design Parameter	Value
Residential Average Dailey Demand	280 L/person/Day
Maximum Dailey Peaking Factor (As per MOE Table 3-3)	7.3
Peak Hour Factor (As per MOE Table 3-3)	10.9
Minimum Depth of Cover	2.4m
Desired operating pressure range during normal operating conditions	350 kPa and 480 kPa
During normal operating conditions pressure must not drop below	275 kPa
During normal operating conditions pressure shall not exceed	552 kPa
During fire flow operating conditions pressure must not drop below	140 kPa

The required water supply requirements for the residential units in the proposed building have been calculated using the following formula:

Where:

$$Q = (q \times P \times M)$$

q = average water consumption (L/capita/day)

P = design population (capita)

M = Peak factor

Using a calculated Maximum Day Factor and Peak Hour factor of 3.8 and 5.6 respectively as per Table 3-3 in the *MOE Design Guidelines*, anticipated demands were calculated as follows:

- Average daily domestic water demand is **1.00** L/s,
- Maximum daily demand is **3.56** L/s, and
- Maximum hourly demand is **18.93** L/s.

Additionally, the proposed development comprises of commercial space equivalent to approximately 549 m² on the first floor and office space equivalent to approximately 498m² on the second floor. According to the City of Ottawa Guidelines, the average water demand for commercial use is **2,500 L/(1000 m²/d)**, and the daily and hourly peak factors are approximately **1.5** and **2.7**, respectively. The peak design commercial water demands were calculated as follows:

$$Q = q \times A \times M$$



Where:

q = Average water demand (L/m²/d)

A = Total site area (m²)

M = Peak factor

Therefore, for the commercial and office area have an average daily demand of **0.030 L/s**, maximum daily is **0.045 L/s**, and maximum hourly is **0.082 L/s**. Adding the water demands from domestic and commercial uses, the total required water supply becomes **1.031 L/s** for average daily demand, **3.606 L/s** for maximum daily demand, and **19.011 L/s** for maximum hourly demand.

Based on maximum hourly rate of 19.011 L/s a minimum of 150 mm dia. servicing is required. Refer to *Appendix C* for water demand calculations.

During the detailed design which will take place during the site plan control process, it is recommended that the City of Ottawa is contacted to obtain boundary conditions associated with the final calculated water demands. At that time, further review will take place to ensure that pressures of the water network remain within the pressure ranges outlined in Table 2 above.

3.2 Fire Protection

The estimated fire flow for the proposed buildings was calculated in accordance with *ISTB-2018-02*. The following parameters were assumed during this functional serviceability review, with detailed input from the building Architect to be requested during the Site Plan Design stage.

- Type of construction – Non-Combustible/Concrete etc.
- Occupancy type – Combustible
- Sprinkler Protection – Automatic Fully Supervised Sprinkler System.

The estimated fire flow demand was estimated to be **4 000 L/min**, see *Appendix C* for details

There are four (4) existing fire hydrants in close proximity to the proposed buildings that are available to provide fire flow demands of approximately 22 000 L/min. Refer to *Appendix C* for fire hydrant locations. Table 3 below summarizes the aggregate fire flow of the contributing hydrants in close proximity to the proposed development based on Table 18.5.4.3 of *ISTB-2018-02*.

Table 3: Fire Protection Summary Table

Building	Fire Flow Demand (L/min)	Fire Hydrants(s) within 75m	Available Combined Fire Flow (L/min)
Proposed 6 Storey Building	TBD During Detailed Design (assumed 10 000 L/min)	4	(4 x 5678) = 22 000

The total available fire flow from contributing hydrants is equal to approximately 22,000 L/min which is sufficient to provide adequate fire flow for the proposed development. A certified fire protection system specialist will need to be employed to design the building's fire suppression system and confirm the actual fire flow demand.



4 SANITARY SERVICE

There is an existing 375 mm sanitary within Bank Street running across the frontage of the subject site ultimately conveying flow in the north-west direction.

The post development total sanitary effluent was calculated to be is **4.0 L/s** as a result of proposed residential population, commercial uses and a small portion of infiltration. Refer to Appendix D for further information on the calculated sanitary flows.

Based on existing as-built information (Refer to Appendix E), the existing 375 mm dia. sanitary sewer is sloped at 1.26% and is calculated to have a maximum capacity of approximately 211 L/s. The proposed increase in total wastewater flow from the increased population represents approximately 1.9% of the existing maximum capacity in the single sanitary sewer. Given that typical design principles are to design with a capacity contingency, and that the overall increase in flow is less than 2%, it is anticipated that the existing local sewer network has sufficient capacity to accommodate the proposed development.

5 STORMWATER MANAGEMENT

The existing site has a large portion of impervious area which will be mostly mimicked in post development conditions. A slight increase is expected in post development conditions as a result of a larger building footprint minimizing a portion of the landscaped area along the south border of the property.

At the time of detailed design, stormwater quantity will be dealt with to ensure that the post-development runoff discharge from the site is controlled to not exceed the 5-year storm release rate.

Given that the development concept is to have the building take up a large portion of the site boundary, there will be little opportunity for any overland drainage or storage. In order to control the quantity of runoff from the site, storage within the building (cistern) or the rooftop is intended.

Additionally, it is noted that the proposed site plan results in only a slightly higher runoff coefficient than that in the pre-development conditions. This is summarized in table 4 below.

Table 4: Pre-Development and Post-Development Runoff Coefficients

Pre-Development Conditions			Post-Development Conditions		
Grass Area (C=0.2)	Building Area/Asphalt (C=0.9)	Combined C Value	Grass Area (C=0.2)	Building Area/Asphalt (C=0.9)	Combined C Value
120m ²	1694m ²	0.85	50m ²	1764m ²	0.88

6 CONCLUSIONS AND SERVICEABILITY CONSIDERATIONS

This evaluation is limited to assessing the technical feasibility of servicing the site described within this document to support an Official Plan Amendment and Zoning By-law Amendment.

Based on the forgoing the conclusions in relation to the serviceability of the site are as follows:



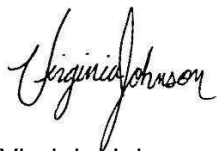
- Water:
 - Total water (domestic and commercial) demands from the proposed high rise building are expected to be in the range of **1.031 L/s** for the Average daily demand, **3.606 L/s** for the maximum daily and **19.011 L/s** for maximum hourly.
 - 4 fire hydrants within 75m are expected to provide the required fire flow.
 - During detailed design, pressures available along Bank Street are to be investigated as it is expected to be serviced from the existing 205 mm PVC watermain.
- Sanitary Sewage:
 - The post development total sanitary effluent was calculated to be is **4.0 L/s** as a result of proposed residential population, commercial uses and a small portion of infiltration.
 - The flow from this development would make up a very small percentage (Less than 2.0 %) of the flow capacity of the existing 375 mm sanitary sewer in Bank Street.
- Stormwater
 - The small increase in impervious area along with strict quantity control requirements outlined by the City of Ottawa will require the site to implement a stormwater quantity management system.
 - Storage could be achieved via a cistern or rooftop storage on site to ensure flow is regulated to not release more than the allowable release rate.

7 CLOSURE

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Prepared by:

LRL Associates Ltd.

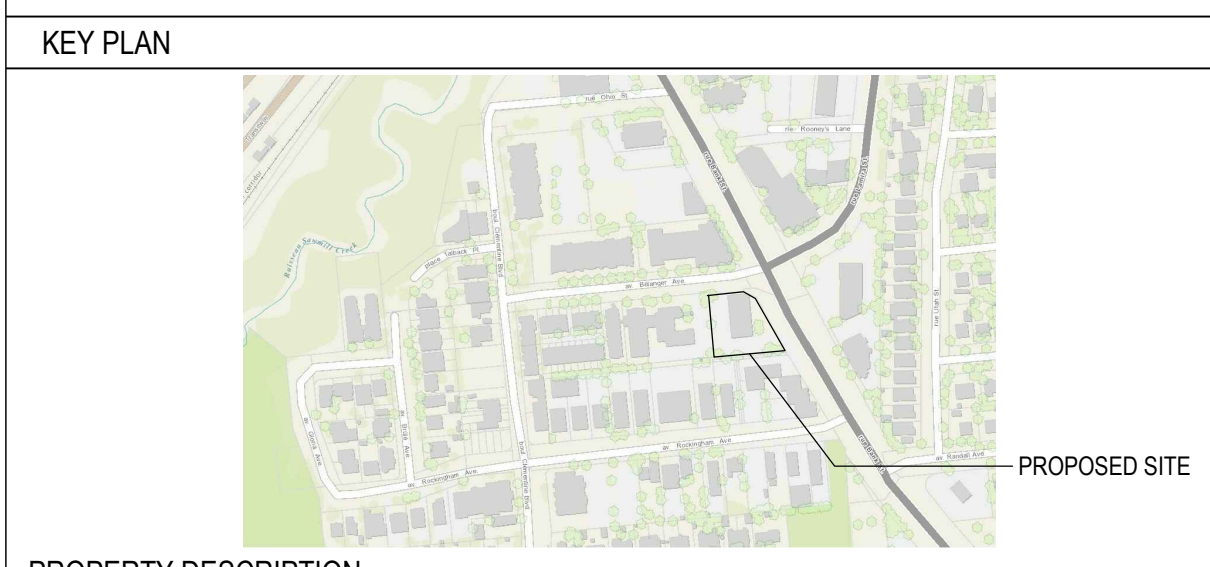


Virginia Johnson, P. Eng.



APPENDIX A
Concept Site Plan





PROPERTY DESCRIPTION

SIXTEEN STOREY MIXED-USE RESIDENTIAL BUILDING

CITY OF OTTAWA PIN NUMBER

MUNICIPAL ADDRESS: 1400 BANK STREET

SITE INFORMATION

LOT AREA: 1460m²

LOT FRONTAGE: 42m (BANK STREET), 25.6m (BELANGER AVE.)

LOT DEPTH: 25.6m (NORTH) & 51m (SOUTH)

BUILDING INFORMATION

BUILDING AREA: 1,053 m²

GROSS AREA: 17,079 m²

PROPOSED USE: MIXED-USE RESIDENTIAL

UNIT BREAKDOWN:

LEVEL 2:	6 UNITS	RETAIL/OFFICE USE:	LEVEL 1:	352 m ²
LEVEL 3-6:	12 UNITS EA.		LEVEL 2:	500 m ²
LEVEL 7-9:	12 UNITS EA.			
LEVEL 10-16:	10 UNITS EA.			

TOTAL: 160 UNITS

ZONING TABLE

CITY OF OTTAWA ZONING BY-LAW No. 2008-250	REQUIRED	PROPOSED
AM1 [1913] - ARTERIAL MAINSTREET, SUBZONE 1, EXCEPTION 1913		
MINIMUM LOT AREA	NO MINIMUM	1,459m ²
MINIMUM LOT WIDTH	NO MINIMUM	N/A
MINIMUM FRONT YARD SETBACK	0 m	VARIABLE
MINIMUM INTERIOR SIDE YARD SETBACK	7.5m (ABUTTING RESIDENTIAL ZONE)	N/A
MINIMUM REAR YARD SETBACK (south)	7.5 m (REAR LOT LINE ABUTTING RESIDENTIAL ZONE)	9.3m- GROUND FLOOR 9m- LEVEL 2-9 11.6m- LEVEL 10-16
MINIMUM CORNER SIDYARD SETBACK	0 m	0.8 m
MAXIMUM BUILDING HEIGHT	25 m	50 m
MAXIMUM FLOOR SPACE INDEX	3.5	5
MAX # of RESIDENTIAL UNITS	N/A	5
LANDSCAPED AREA	N/A	N/A
VEHICLE PARKING REQUIREMENTS	RESIDENTIAL: 80 SPACES VISITORS: 14.8 SPACES RETAIL: N/A OFFICE: 5	66 SPACES
AMENITY AREA REQUIREMENTS	960 m ² 481 m ² COMMUNAL	BALCONIES: 696 m ² ROOF AMENITY: 379 m ² EXTERIOR: 134 m ²
BICYCLE PARKING SPACES	0.5 per dwelling unit = 80 1 PER 250m ² GFA = 3.4	186 SPACES

LEGEND

	GRASS		EXISTING TREE TO REMAIN (REFER TO LANDSCAPE DRAWINGS)
	UNIT PAVERS REFER TO LANDSCAPE		NEW TREE (REFER TO LANDSCAPE DRAWINGS)
	ASPHALT PAVING		NEW SHRUBS (REFER TO LANDSCAPE DRAWINGS)
	CONCRETE		NEW EVERGREEN SHRUBS (REFER TO LANDSCAPE DRAWINGS)
	RIVER STONES REFER TO LANDSCAPE		EXISTING GROUND ELEVATION (TO DETERMINE EXISTING AVERAGE GRADE)
	FIRE ROUTE		PROPOSED GROUND ELEVATION REFER TO CIVIL
	EXISTING FENCE		
	NEW SCREEN FENCE		
	NEW SOUND FENCE		
	LOT LINE		
	SETBACK LINE		
	DESIGNATED BUILDING ENTRANCE / EXIT		
	NEW FIRE HYDRANT, REFER TO CIVIL		
	CATCH BASIN		
	MANHOLE		
	FLOOR DRAIN		
	ELECTRICAL POST		
	LIGHT STANDARD		
	DEPRESSED CURB		

NOTE: 'X' E INDICATES EXISTING TO REMAIN

No. Date Enis pour / Object

- 2021-09-29 FOR COORD.
- 2021-10-13 FOR COORD.
- 2021-10-21 FOR COORD.
- 2021-10-26 OFFICIAL PLAN AMENDMENT AND ZONING BY-LAW AMENDMENT

Planner / Planner

NOVATECH ENGINEERING CONSULTANTS LTD

Ingenieur / Engineer (Mécanisme & Électrique / Mechanical & Electrical)

Ingenieur / Engineer (Structure / Structure)

Architecte / Architect (Paysagiste / Landscape)

Ingenieur / Engineer (Civil / Civil)

Client / Client

Architecte / Architect Collectif d'architectes / Architects Collective

Fig 1 3550, Saint-Antoine O. Montreal QC H4C 1A8 T. 514 491-9122

Fig 2 190 Somerset St W #208 Ottawa ON K2P 1J4 T. 613 695-4122

www.figuri.ca

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Scale / Scale

Note: L'entrepreneur doit vérifier toutes les dimensions et informations sur le site et agir immédiatement. Fournir de toutes erreurs ou omissions.

Contractor shall verify all information and dimensions on site and immediately report any errors or omissions to the architect.

Project / Project

16-STOREY MIXED-USE RESIDENTIAL BUILDING

1400 BANK ST. OTTAWA, ON

Site Plan

Drawn by / Drawn by No. projet / Project number 2144

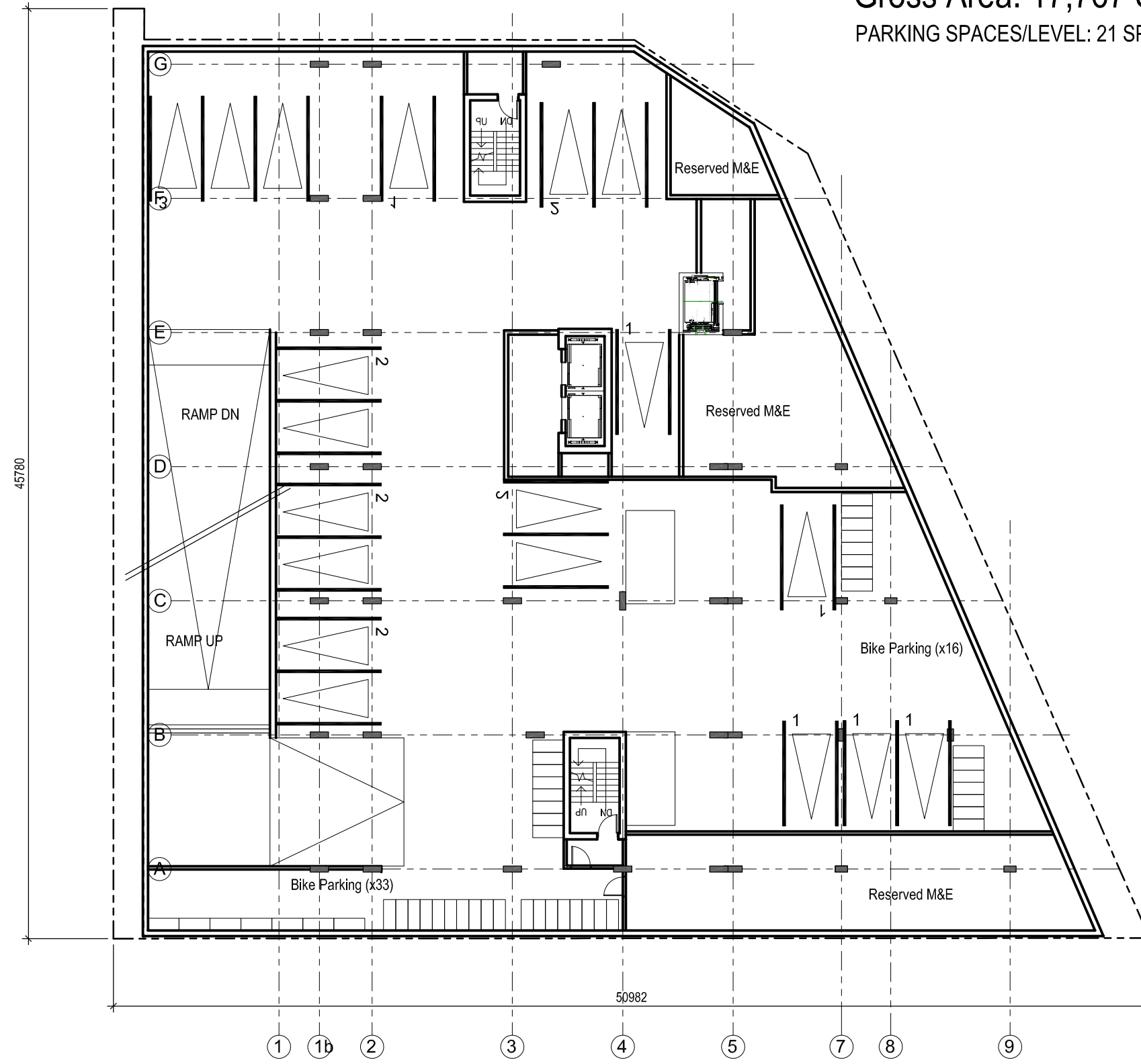
Verified by / Verified by No. dessin / Drawing number Revision / Revision RC

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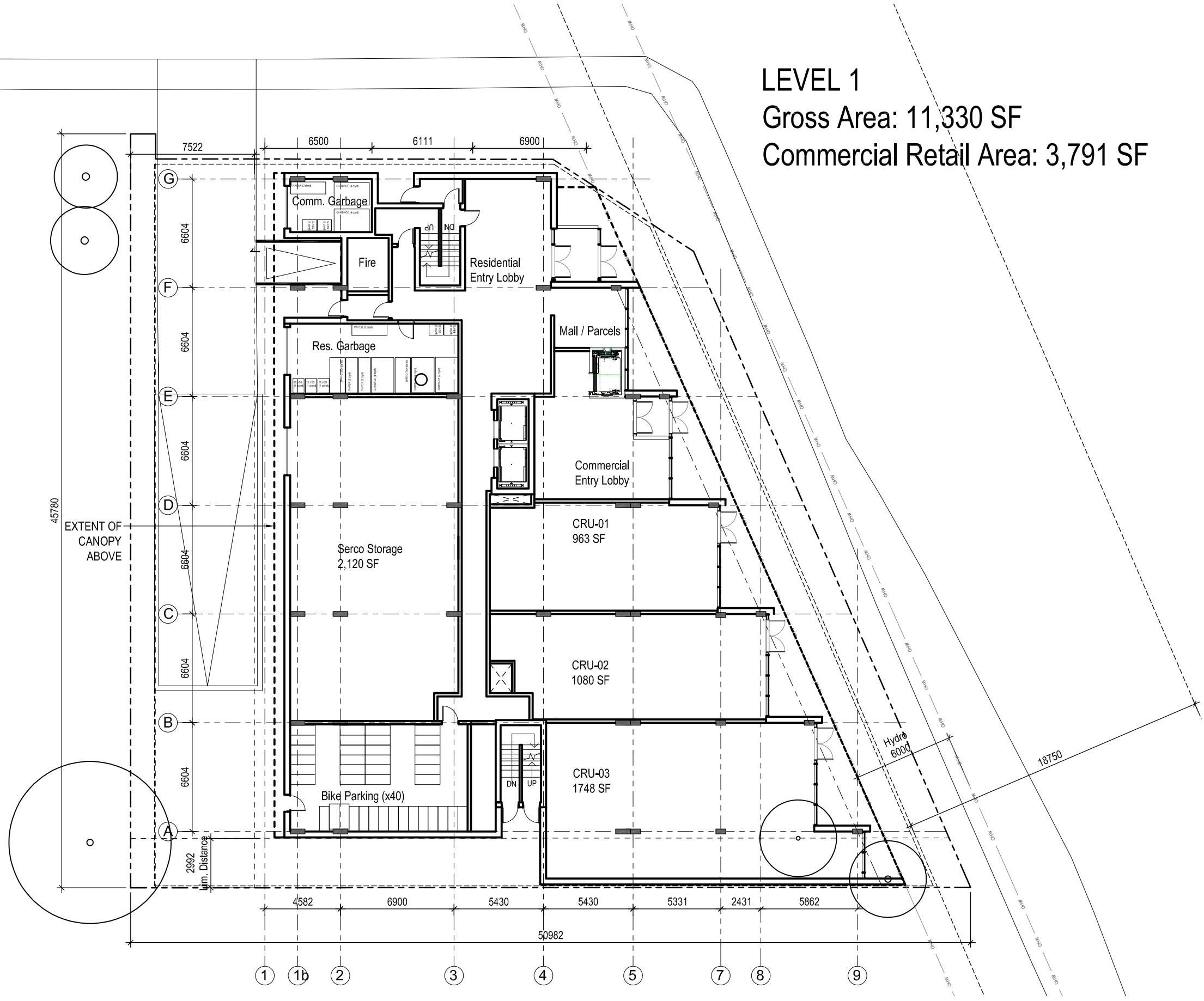
Date de création du dessin / Drawing creation date 2021-09-29

A-105

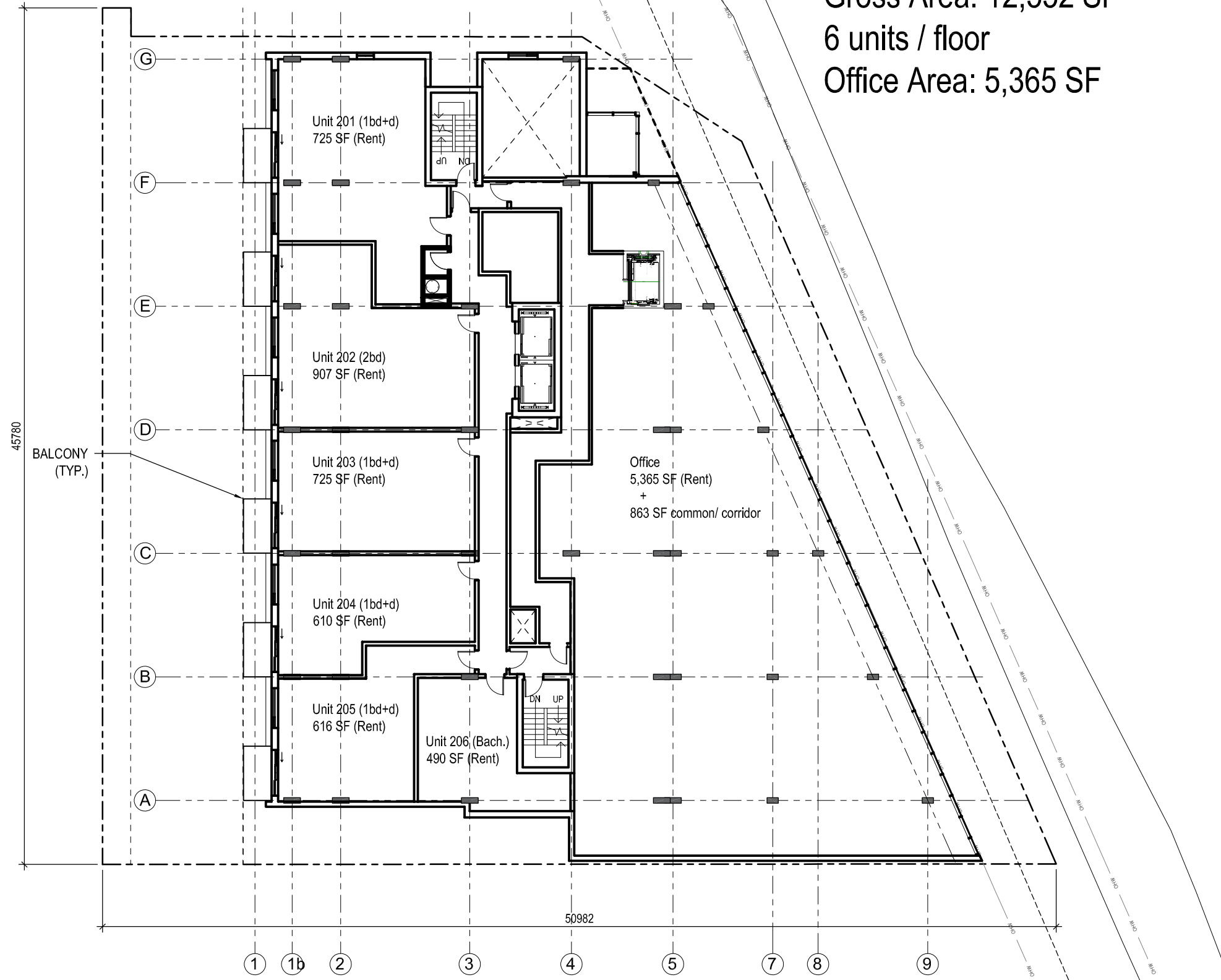
LEVEL P1/P2/P3
 Gross Area: 17,767 SF / Level
 PARKING SPACES/LEVEL: 21 SPACES



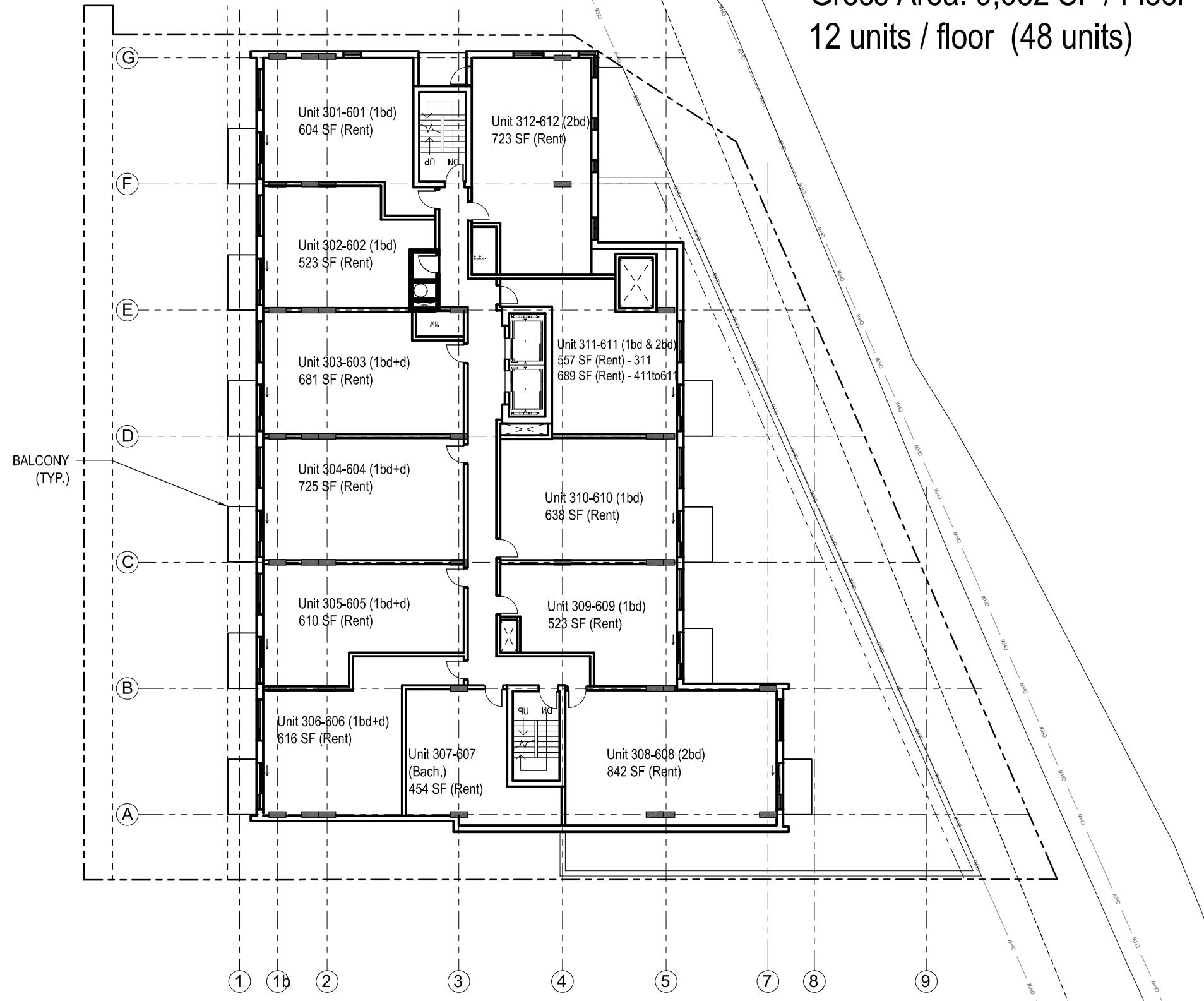
LEVEL 1
 Gross Area: 11,330 SF
 Commercial Retail Area: 3,791 SF



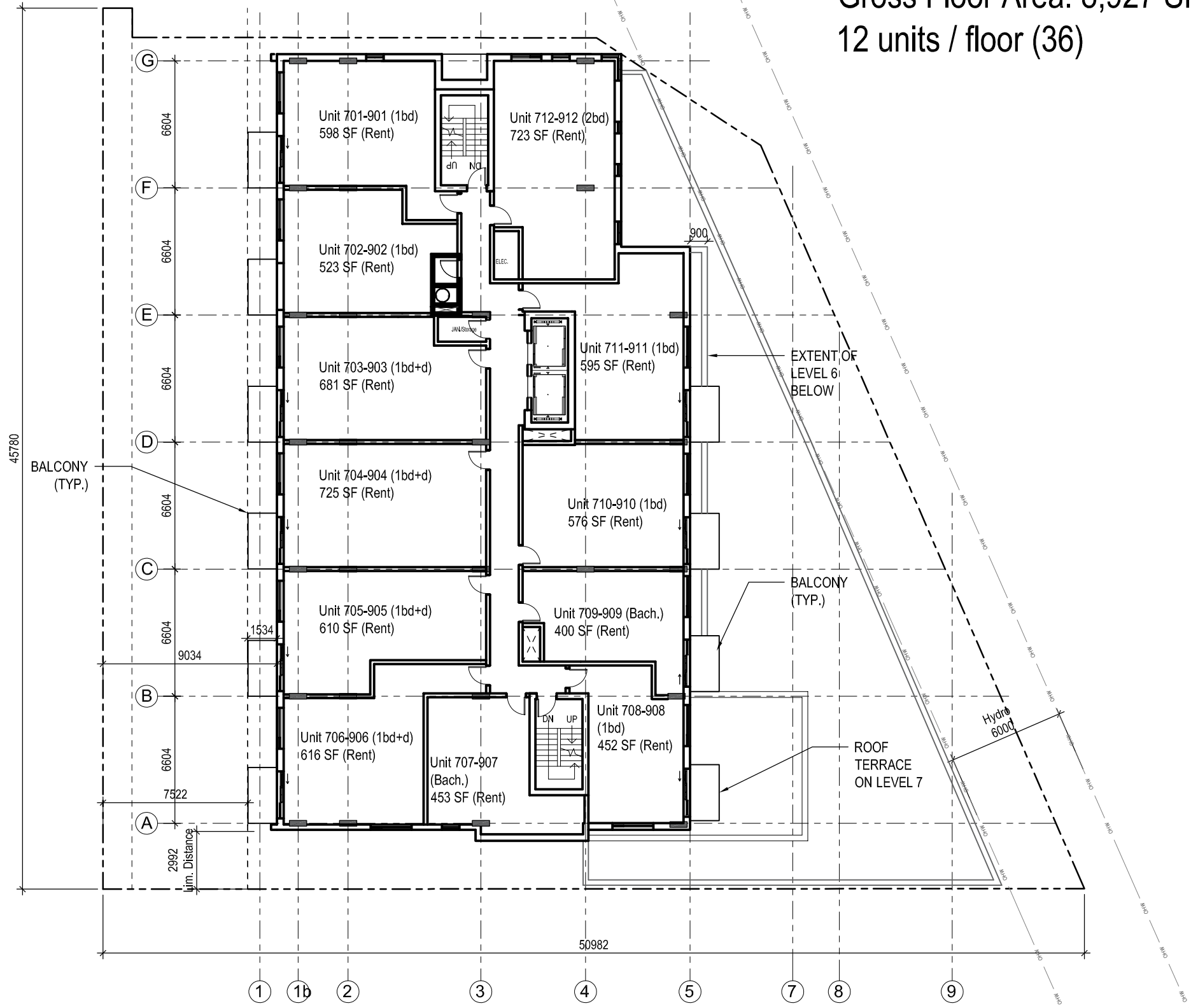
LEVEL 2
 Gross Area: 12,552 SF
 6 units / floor
 Office Area: 5,365 SF



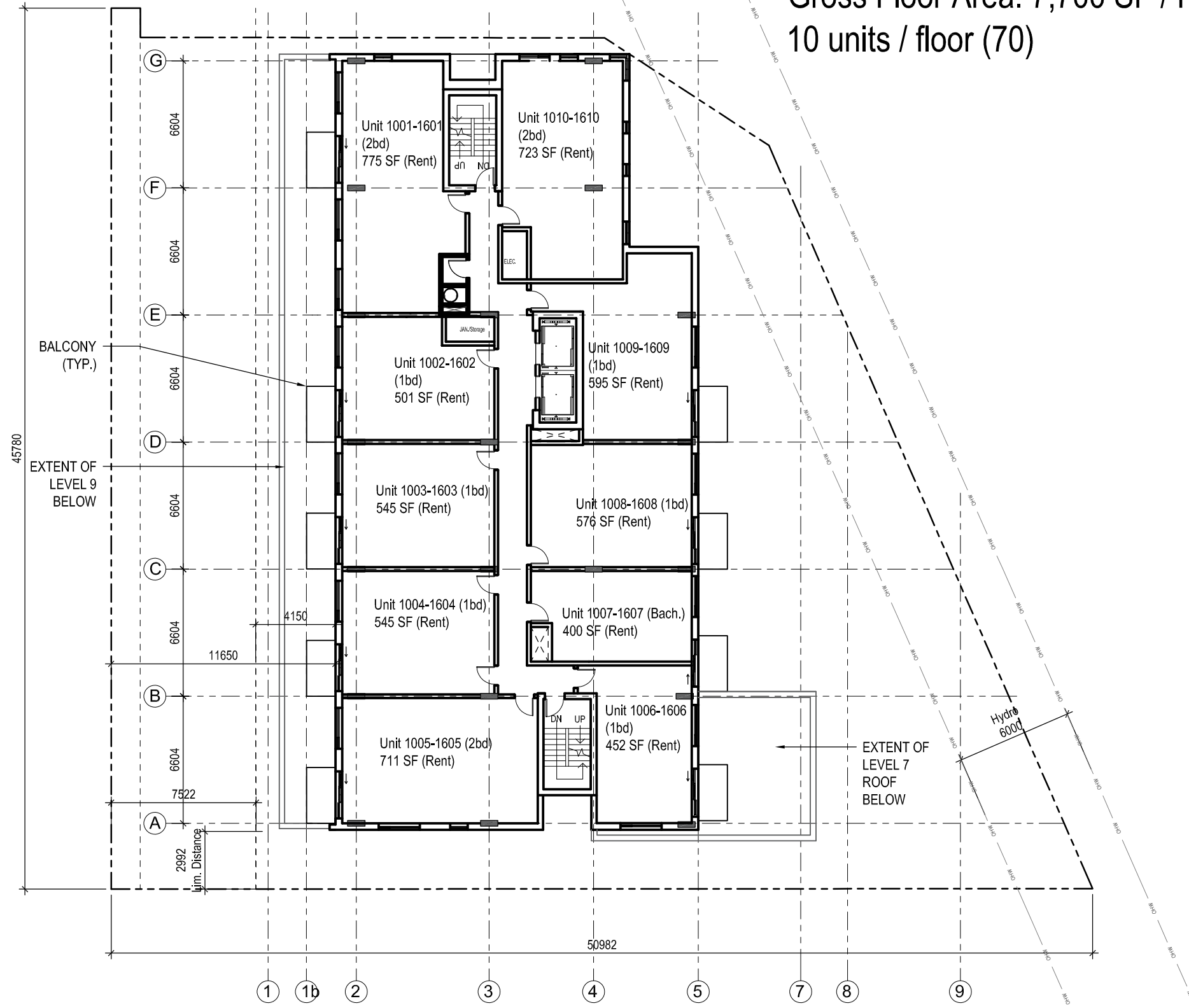
LEVEL 3-6
Gross Area: 9,682 SF / Floor
12 units / floor (48 units)

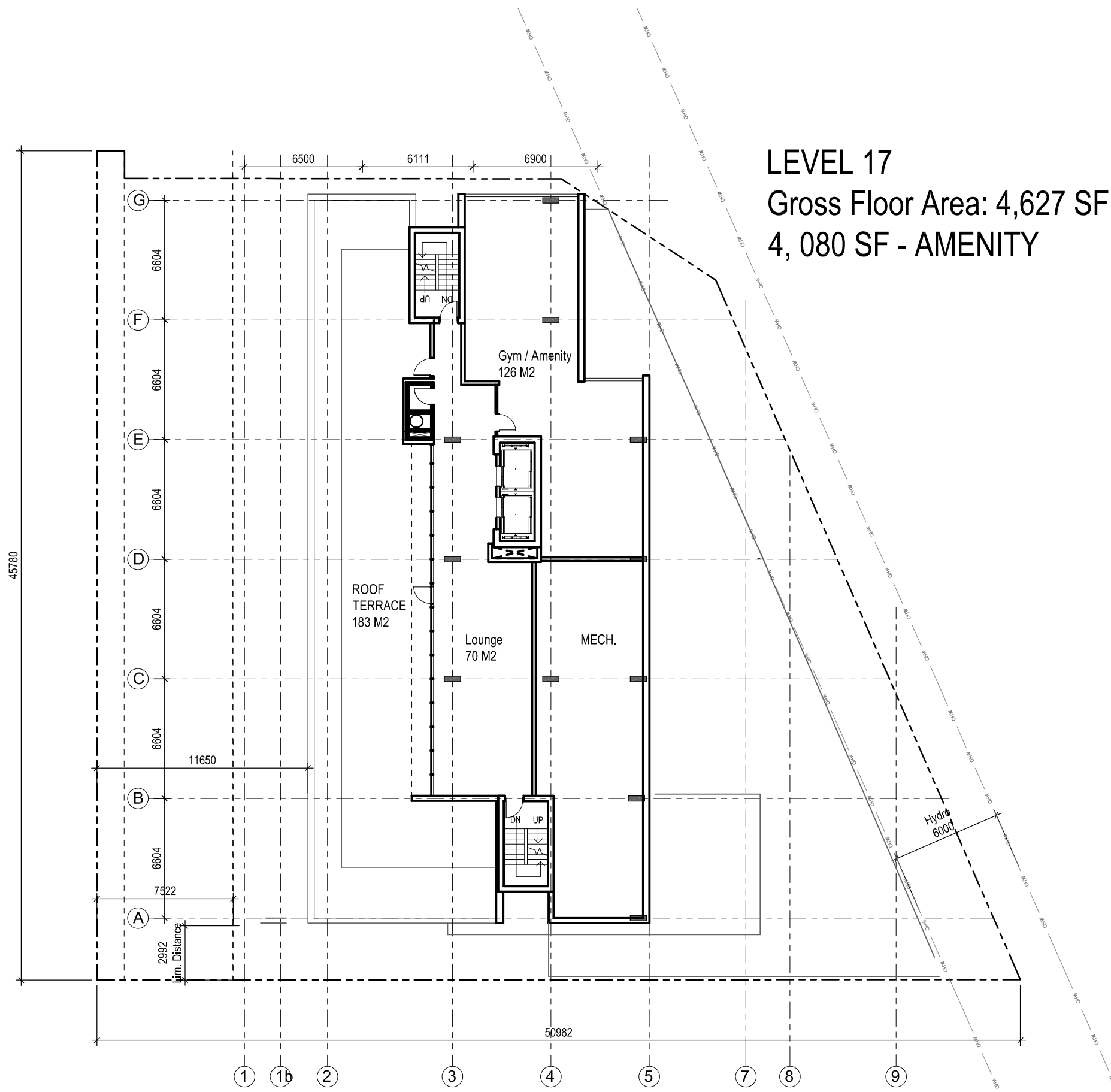


LEVEL 7-9
Gross Floor Area: 8,927 SF / Floor
12 units / floor (36)



LEVEL 10-16
Gross Floor Area: 7,700 SF / Floor
10 units / floor (70)



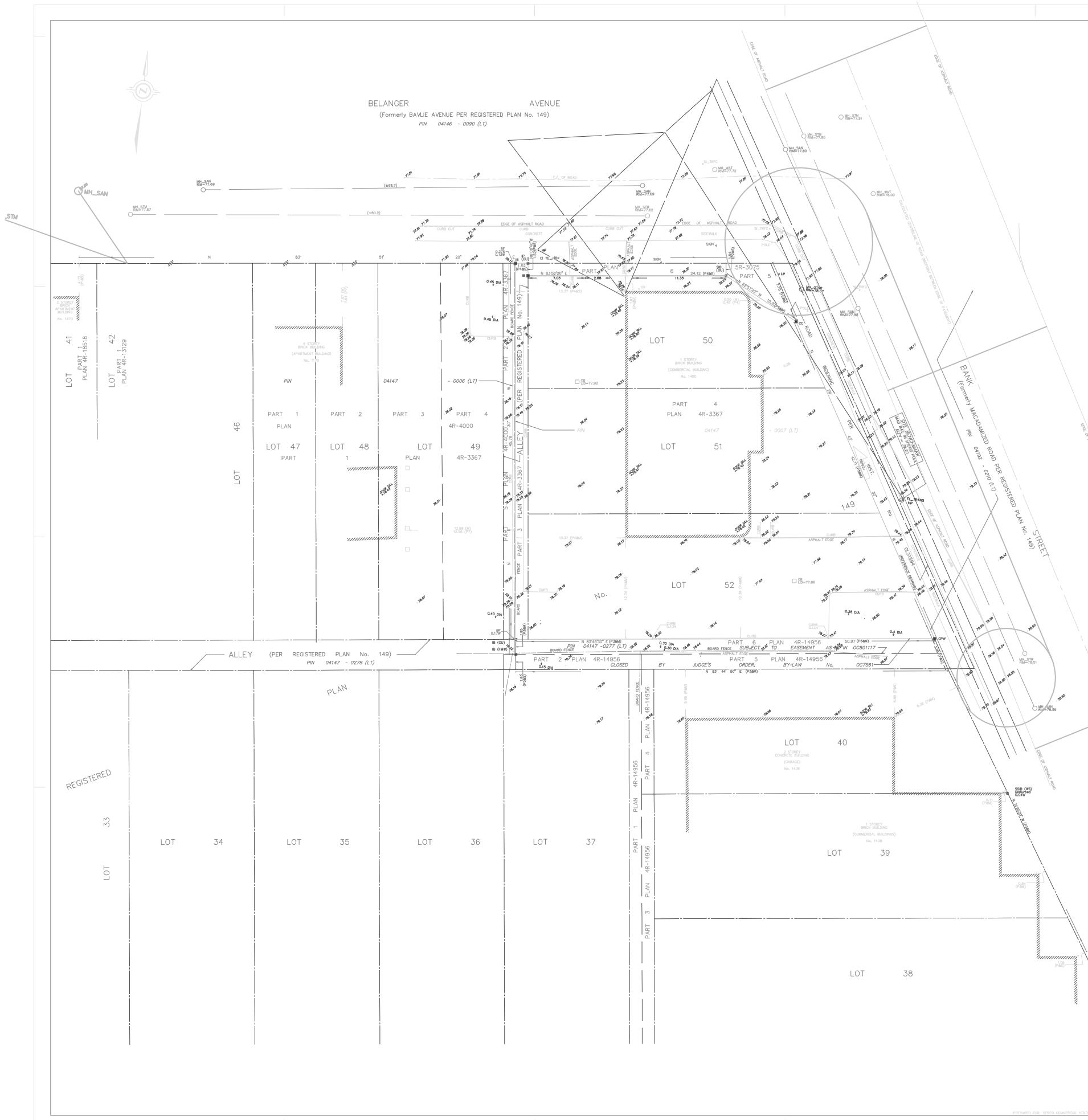


LEVEL 17
 Gross Floor Area: 4,627 SF
 4,080 SF - AMENITY

APPENDIX B

Topography Survey





SURVEYOR'S REAL PROPERTY REPORT
 WITH TOPOGRAPHIC DETAILS
 PART 1 - PLAN SHOWING
 PART OF LOTS 50, 51 AND 52
 AND PART OF THE ALLEY BETWEEN
 LOT 49 AND LOTS 50, 51 AND 52
 REGISTERED PLAN No. 149
 CITY OF OTTAWA
 J.D. BARNES LIMITED
 SCALE 1 : 150
 METRIC DISTANCES AND/OR COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048.

NOTES
 REFERENCED AND PROPOSED ARE REFERRED TO THE METRIC LIMITS OF ADJACENT LOTS AND ARE SHOWN IN BLACK ON THIS PLAN.
 ALL BUILDINGS ARE TAKEN TO CONCRETE FOUNDATION UNLESS OTHERWISE NOTED.
 COMPLIANCE WITH ONTARIO BUILDING CODE DETACH REQUIREMENTS ARE NOT SHOWN BY THIS SURVEY.
 NOTES ON TREES:
 - "T" DENOTES TREE LOCATION
 - "C" DENOTES TREE CALIBRE AT 1.37M ABOVE GROUND
 - "D" DENOTES TREE DIAMETER AT 1.37M ABOVE GROUND
 - "S" DENOTES TREE SPECIES
 - "H" DENOTES TREE HEIGHT

PART 2 - SURVEY REPORT
 - DESCRIPTION
 - REGISTERED EASEMENTS AND/OR RIGHTS-OF-WAY
 - BOUNDARY FEATURES

LEGEND
 S DENOTES SURVEY MONUMENT FOUND
 SM DENOTES SURVEY MONUMENT SET
 SB DENOTES STORM BARRICLE
 SW DENOTES WATER BARRICLE
 W DENOTES WALL (RANDOM)
 CB DENOTES CATCH BASIN
 F DENOTES FENCE POST
 LP DENOTES LAMP POST
 BF DENOTES BOARD FENCE
 C DENOTES CANTONMENT
 D DENOTES DIAMETER
 LT DENOTES LIGHT TRANSFORMER
 T DENOTES TELEPHONE TRANSFORMER BOX
 TR DENOTES TRAFFIC SIGNAL
 S DENOTES SIGN
 S DENOTES SIGN
 C DENOTES OVERHEAD HYDRO CABLE
 C DENOTES OVERHEAD TELEPHONE CABLE
 S DENOTES UNDERGROUND STORM SEWER
 S DENOTES UNDERGROUND SANITARY SEWER
 T DENOTES DECIDUOUS TREE
 T DENOTES CONIFEROUS TREE

TOPOGRAPHIC LEGEND
 SML SAN DENOTES SANITARY MANHOLE
 SML STM DENOTES STORM MANHOLE
 SML SW DENOTES WATER MANHOLE
 SML W DENOTES WALL (RANDOM)
 CB DENOTES CATCH BASIN
 F DENOTES FENCE POST
 LP DENOTES LAMP POST
 BF DENOTES BOARD FENCE
 C DENOTES CANTONMENT
 D DENOTES DIAMETER
 LT DENOTES LIGHT TRANSFORMER
 T DENOTES TELEPHONE TRANSFORMER BOX
 TR DENOTES TRAFFIC SIGNAL
 S DENOTES SIGN
 S DENOTES SIGN
 C DENOTES OVERHEAD HYDRO CABLE
 C DENOTES OVERHEAD TELEPHONE CABLE
 S DENOTES UNDERGROUND STORM SEWER
 S DENOTES UNDERGROUND SANITARY SEWER
 T DENOTES DECIDUOUS TREE
 T DENOTES CONIFEROUS TREE

ELEVATION NOTE:
 1. ELEVATIONS ARE REFERRED TO THE CANADIAN DATUM 1984 (CD) UNLESS OTHERWISE NOTED.
 2. IT IS THE RESPONSIBILITY OF THE USER OF THIS INFORMATION TO VERIFY THAT THE INFORMATION AND DATA ARE ACCURATE AND COMPLETE.

SURVEYOR'S CERTIFICATE
 I, J.D. BARNES, SURVEYOR,
 DO HEREBY CERTIFY THAT:
 1. THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEY ACT AND THE REGULATIONS MADE UNDER THERE.
 2. THE SURVEY WAS COMPLETED ON JANUARY 15, 2025.
 WITNESSED BY ME AND MY DEPUTY SURVEYOR
 DATE: _____

J.D. BARNES LIMITED
 LAW INFORMATION REGULATORS
 19-10-150-00

APPENDIX C

Water Servicing Calculations





Water Supply Calculations

LRL File No. 210617
 Date October 26, 2021
 Prepared by Virginia Johnson

Residential Demand based on the City of Ottawa Design Guidelines-Water Distribution, 2010

Unit Type	Persons Per Unit	Number of Units	Population
1 Bedroom/Bachelor	1.8	91	163.8
2 Bedroom/1 + Den	2.1	69	144.9
Total		160	308.7

Average Water Consumption Rate	280 L/c/d		
Average Day Demand	86,436 L/d		1.00 L/s
Maximum Day Factor	3.6		(MOE Table 3-3)
Maximum Daily Demand	307,630 L/d		3.56 L/s
Peak Hour Factor	5.3		(MOE Table 3-3)
Maximum Hour Demand	1,635,443 L/d		18.93 L/s

Commercial Demands

Commercial Area	352 m ²		
Office Area	695 m ²		
Average Daily Demand	2500 L/(1000m ² /d)		
Maximum Daily Peak Factor	1.5		
Maximum Hourly Peak Factor	2.7		
Average Commercial Water Demand	2,618 L/d		0.030 L/s
Maximum Daily Commercial Water Dem	3,926 L/d		0.045 L/s
Maximum Hourly Commercial Water Dei	7,067 L/d		0.082 L/s

Total Water Demand

Average Total Water Demand	89,054 L/d		1.031 L/s
Maximum Daily Total Water Demand	311,556 L/d		3.606 L/s
Maximum Hourly Total Water Demand	1,642,511 L/d		19.011 L/s

Water Service Pipe Sizing

$$Q = VA$$

Where: V = velocity, A=Area of Pipe, Q=Flow rate

Assuming a maximum velocity of 1.8m/s, the diameter of pipe is calculated as:

$$\begin{aligned} \text{Minimum pipe diameter (d)} &= (4Q/\pi V)^{1/2} \\ &= 0.116 \text{ m} \\ &= 116 \text{ mm} \\ \text{Proposed pipe diameter (d)} &= 150 \text{ mm} \end{aligned}$$

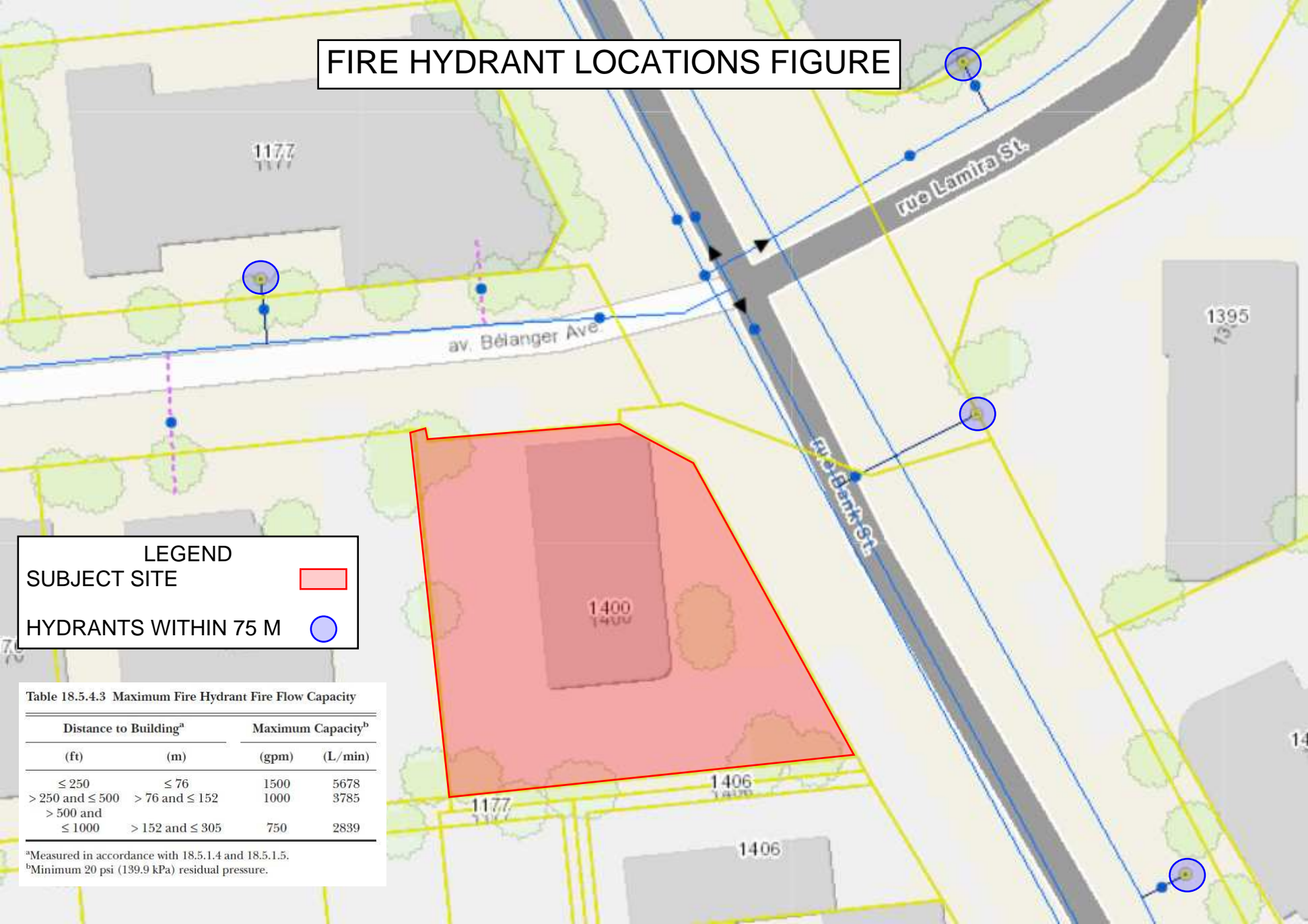


Fire Flow Calculations

LRL File No. 210617
 Date September 29, 2021
 Method Fire Underwriters Survey (FUS)
 Prepared by Virginia Johnson

Step	Task	Term	Options	Multiplier	Choose:	Value	Unit	Fire Flow
Structural Framing Material								
1	Choose frame used for building	Coefficient C related to the type of construction	Wood Frame	1.5	Non-combustible construction	0.8		
			Ordinary Construction	1.0				
			Non-combustible construction	0.8				
			Fire resistive construction <2 hrs	0.7				
			Fire resistive construction >2 hrs	0.6				
Floor Space Area (A)								
2	Total area (ASSUMED BASED ON PRELIMINARY ARCH DWGS)					1,200	m ²	
3	Obtain fire flow before reductions	Required fire flow	Fire Flow = 220 x C x A ^{0.5}				L/min	6,097
Reductions or surcharge due to factors affecting burning								
4	Choose combustibility of contents	Occupancy hazard reduction or surcharge	Non-combustible	-25%	Combustible	0%	L/min	6,097
			Limited combustible	-15%				
			Combustible	0%				
			Free burning	15%				
			Rapid burning	25%				
5	Choose reduction for sprinklers	Sprinkler reduction	Full automatic sprinklers	-30%	True	-30%	L/min	3,048
			Water supply is standard for both the system and fire department hose lines	-10%	True	-10%		
			Fully supervised system	-10%	True	-10%		
6	Choose separation	Exposure distance between units	North side	20.1 to 30m	10%	L/min	4,420	
			East side	>30m	0%			
			South side	3.1 to 10m	20%			
			West side	10.1 to 20m	15%			45%
Net required fire flow								
7	Obtain fire flow, duration, and volume	Minimum required fire flow rate (rounded to nearest 1000)					L/min	4,000
		Minimum required fire flow rate					L/s	66.7
		Required duration of fire flow					hr	2

FIRE HYDRANT LOCATIONS FIGURE



LEGEND

SUBJECT SITE

HYDRANTS WITHIN 75 M

Table 18.5.4.3 Maximum Fire Hydrant Fire Flow Capacity

Distance to Building ^a		Maximum Capacity ^b	
(ft)	(m)	(gpm)	(L/min)
≤ 250	≤ 76	1500	5678
> 250 and ≤ 500	> 76 and ≤ 152	1000	3785
> 500 and ≤ 1000	> 152 and ≤ 305	750	2839

^aMeasured in accordance with 18.5.1.4 and 18.5.1.5.
^bMinimum 20 psi (139.9 kPa) residual pressure.

APPENDIX D

Sanitary Service Calculations





LRL File No. 210617
Project: Apartment Building
Location: 1400 Bank Street, Ottawa
Date: October 26, 2021

Sanitary Design Parameters

Average Daily Flow = 280 L/p/day
 Commercial & Institutional Flow = 50000 L/ha/day
 Light Industrial Flow = 35000 L/ha/day
 Heavy Industrial Flow = 55000 L/ha/day
 Maximum Residential Peak Factor = 4.0
 Commercial & Institutional Peak Factor = 1.5

Industrial Peak Factor = as per Appendix 4-B = 7
 Extraneous Flow = 0.33L/s/gross ha

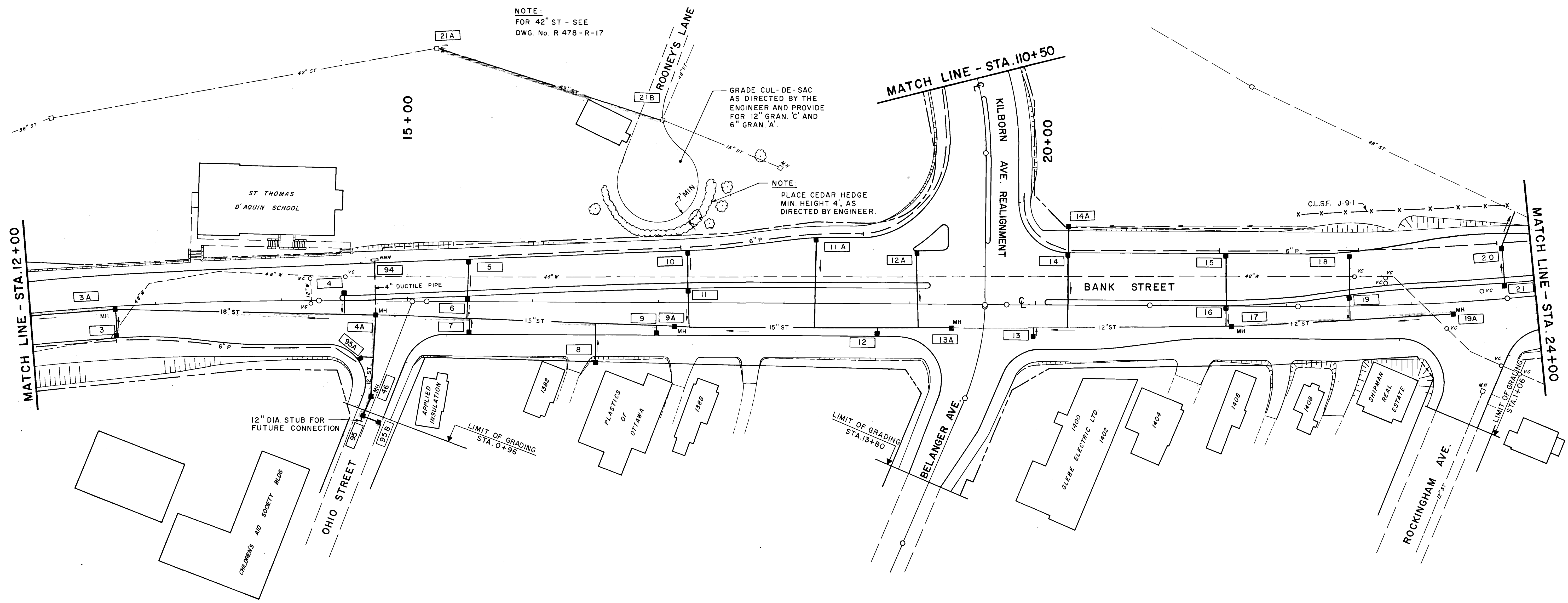
Pipe Design Parameters

Minimum Velocity = 0.60 m/s
 Manning's n = 0.013

LOCATION			RESIDENTIAL AREA AND POPULATION					COMMERCIAL		INDUSTRIAL			INSTITUTIONAL		C+I+I	INFILTRATION			TOTAL FLOW	PIPE						
STREET	FROM MH	TO MH	AREA (Ha)	POP.	CUMMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (Ha)	ACCU. AREA (Ha)	AREA (Ha)	ACCU. AREA (Ha)	PEAK FACT.	AREA (Ha)	ACCU. AREA (Ha)	PEAK FLOW (l/s)	TOTAL AREA (Ha)	ACCU. AREA (Ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	LENGTH (m)	DIA. (mm)	SLOPE (%)	MATERIAL	CAP. (FULL) (l/s)	VEL. (FULL) (m/s)
					AREA (Ha)	POP.																				
SITE	PROP. BLDG	EX. SAN	0.060	308.0	0.06	308.0	4.0	3.99	0.118	0.000	0.00	0.00	7.0	0.0	0.0	0.00	0.18	0.18	0.06	4.05	10.0	150	2.00%	PVC	21.54	1.22
NOTES	Existing inverts and slopes are estimated. They are to be confirmed on-site.													Designed: VJ		PROJECT: Apartment Building										
														Checked: VJ		LOCATION: 1400 Bank										
														Dwg. Reference: N/A		File Ref.: 210617			Date: 2021-10-26		Sheet No. 1 of 1					

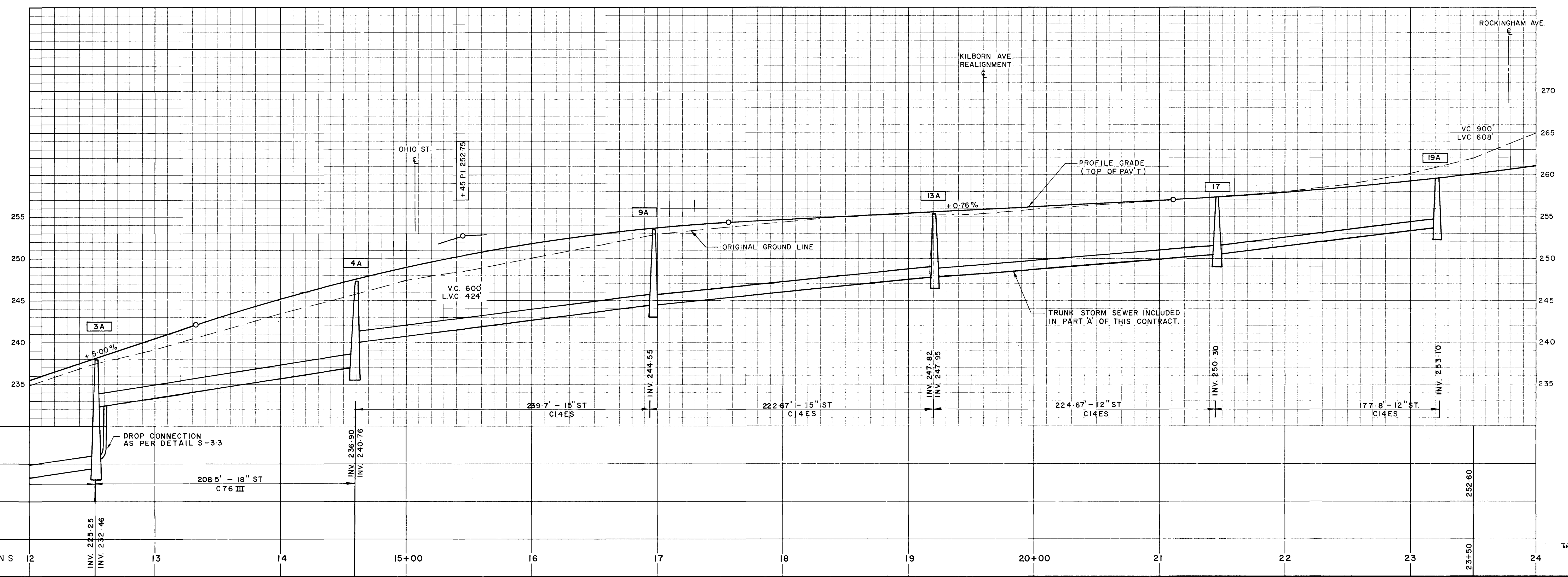
APPENDIX E
Bank Street Plan and Profile Drawing





NOTE:
1. ALL AREAS OUTSIDE ROADWAY AFFECTED BY CONSTRUCTION TO BE GRADED AS DIRECTED BY THE ENGINEER.
2. AREAS WITHIN RIGHT-OF-WAY TO BE LANDSCAPED WITH 4" OF TOPSOIL AND SODDED.
3. OUTSIDE OF RIGHT-OF-WAY TO BE LANDSCAPED WITH 4" OF TOPSOIL, SEED AND MULCH.

— TRUNK STORM SEWER INCLUDED IN PART 'A' OF THIS CONTRACT.
(CATCH BASINS, LATERALS E.T.C. INCLUDED IN PART 'B')



AS BUILT

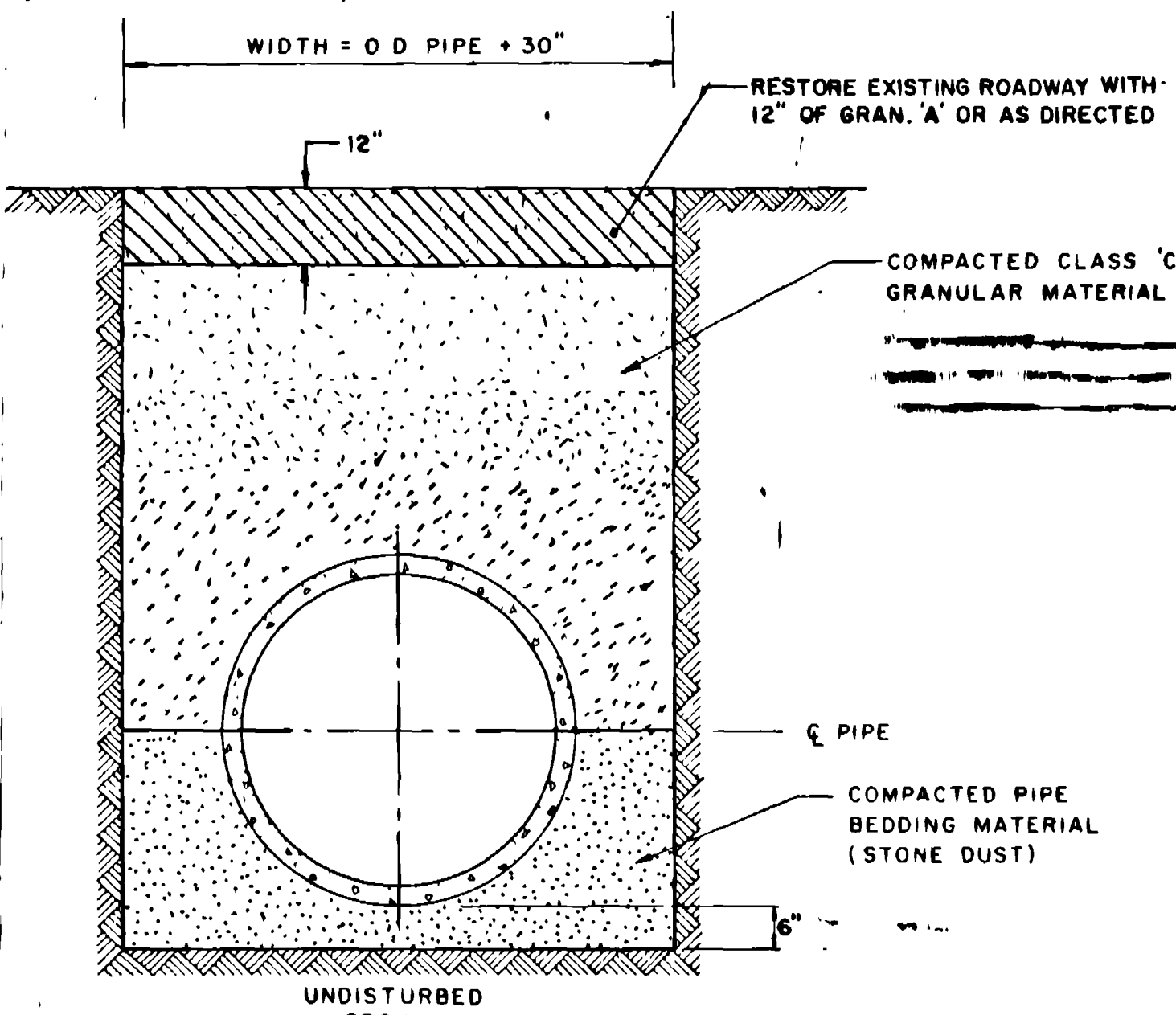
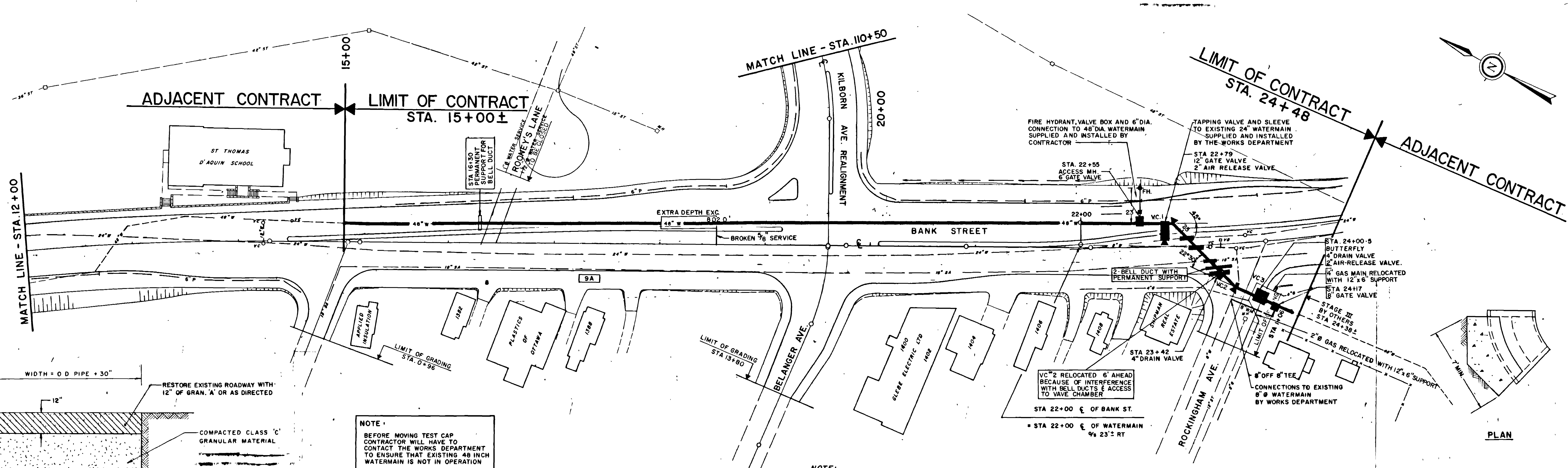


NO.	REVISION	BY	DATE
THE REGIONAL MUNICIPALITY OF OTTAWA - CARLETON Roads Department			
BANK STREET RECONSTRUCTION			
GRADING & DRAINAGE STA. 12+00 TO STA. 24+00			
Des:	O.H.S.	Chkd:	O.B.J.
Dwn:	H.R.	Chkd:	O.H.S.
Date:	MAY 1973		
Scale:	Horiz. 1" = 40'		
	Vert. 1" = 6'		
CONTRACT NO.		DWG. NO. R 478-R-8	
70-518-1		SHEET 8 OF 25	
PHASE II		TYPE "A"	

ENGINEER IN CHARGE

N.H. ORR P.ENG.
Regional Roads Engineer

Eng. I/C Design Services Branch

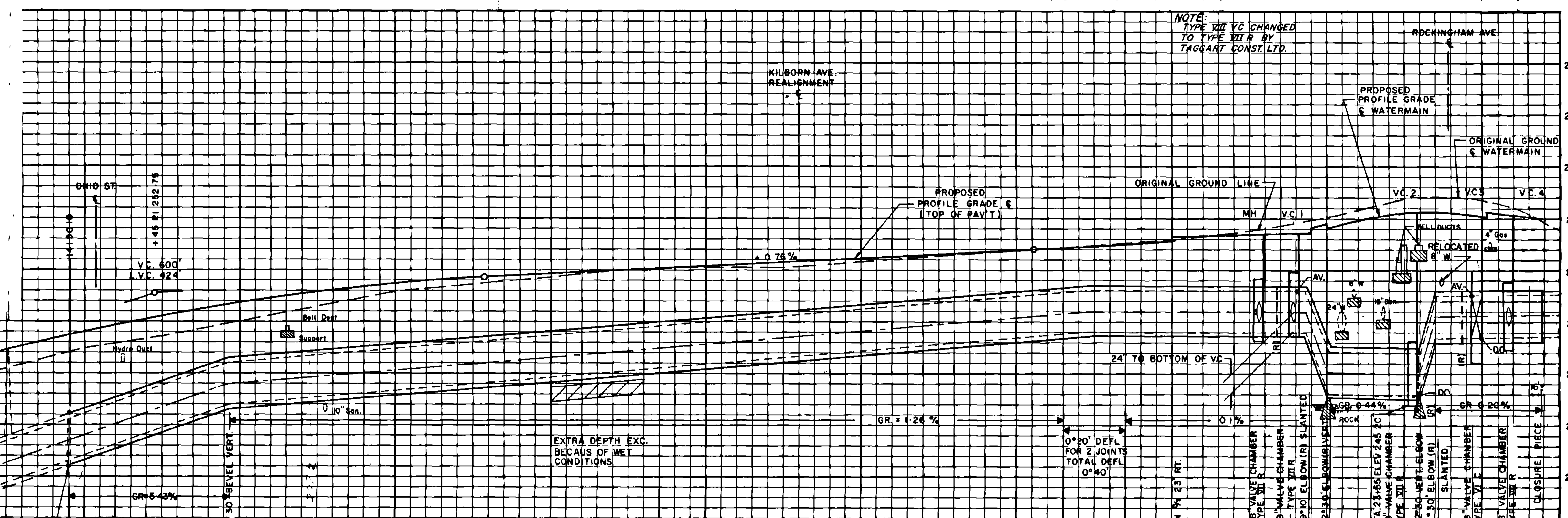


TYPICAL TRENCH SECTION
SCALE 1/2" = 1'-0"

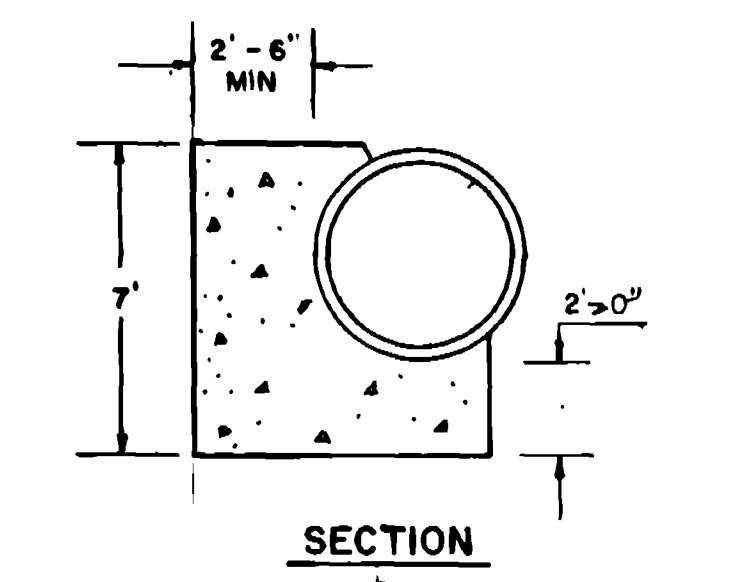
NOTE:
BEFORE MOVING TEST CAP CONTRACTOR WILL HAVE TO CONTACT THE WORKS DEPARTMENT TO ENSURE THAT EXISTING 48 INCH WATERMAIN IS NOT IN OPERATION

NOTE:
GRADING AND DRAINAGE FOR RECONSTRUCTION OF BANK STREET IS NOT PART OF WATERMAIN CONTRACT.

NOTE:
TYPE AND VC CHANGED TO TYPE III & BY TAGGART CONST. LTD.



PROPOSED 48" DIAMETER "HYPRESCON" PRE-STRESSED CONC. LINED CYLINDER PIPE A.W.W.A. C-3011	
STATIONS	12 13 14 15+00 15+53 16 17 18 19 20+00 21 21+30 21+70 22 22+00 22+55 22+79 23+00 23+40 23+80 23+87 24+00 24+17 24+38 24+50
48" W.M.	REMOVE EXIST. 48" TEST CAP. 2350' BEND INV. ELEVA. 236.50
	244.12
	239.87
	231.11
	22+00' OF BANK ST. = 22+00' OF WATERMAIN IN 43' RT.
	22+55
	22+79
	23+00
	23+40
	23+80
	23+87
	24+00
	24+17
	24+38
	24+50



SECTION
THRUST BLOCKS
STA. 22+87 TO STA. 23+73
SCALE: 1/4" = 1'-0"

NOTE:
THRUST BLOCK UNDER BEND STA 23+54 SHALL BE 2' DEEP AND 3' X 5' WIDE.
AS BUILT
AV - 2" AIR RELEASE VALVE
DO - 4" DRAIN VALVE
(R) - DENOTES BELL BOLT-TIED JOINTS
- PERMANENT UTILITY SUPPORTS

NO.	REVISION	BY	DATE
2	AS BUILT (REVISIONS)	H.B.	28/01/74
1	ELBOW STA 23+73 REVISED, RESTRAINED JOINTS AND FIRE HYDRANT ADDED.		23/07/73

THE REGIONAL MUNICIPALITY OF OTTAWA - CARLETON
Works Department
SOUTH OTTAWA FEEDER WATERMAIN
OHIO ST. TO ROCKINGHAM AVE.

PLAN & PROFILE
STA. 12+00 TO STA. 24+48

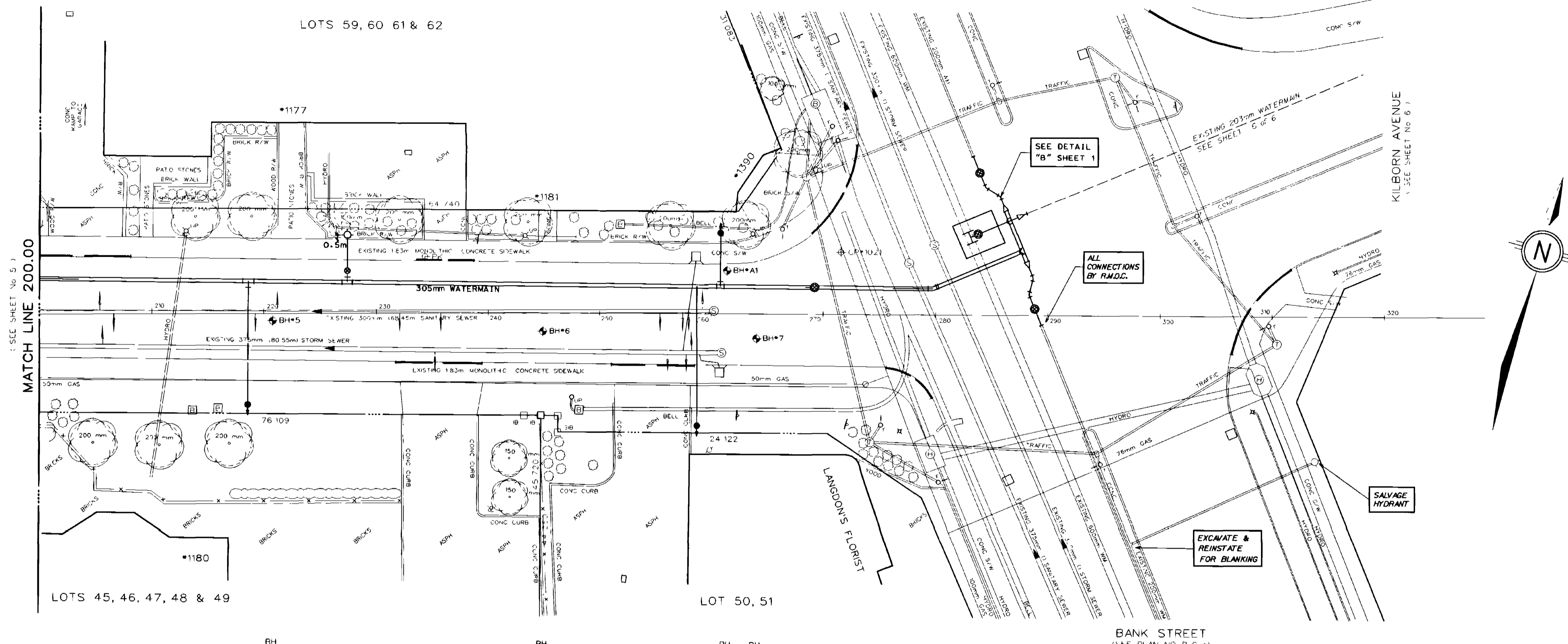
Des: H.V.M. Chd: O.B.J.
Own: E.B.M. Chd: O.H.B.
Date: MAY 1973
Scale: Horiz. 1" = 40'
Vert. 1" = 4'

CONTRACT NO. DWG. NO. 4302-5
11-73 SHEET 5 OF 8
2335-1-4

De Leuw, Cather

BELANGER AVENUE

LOTS 59, 60 61 & 62

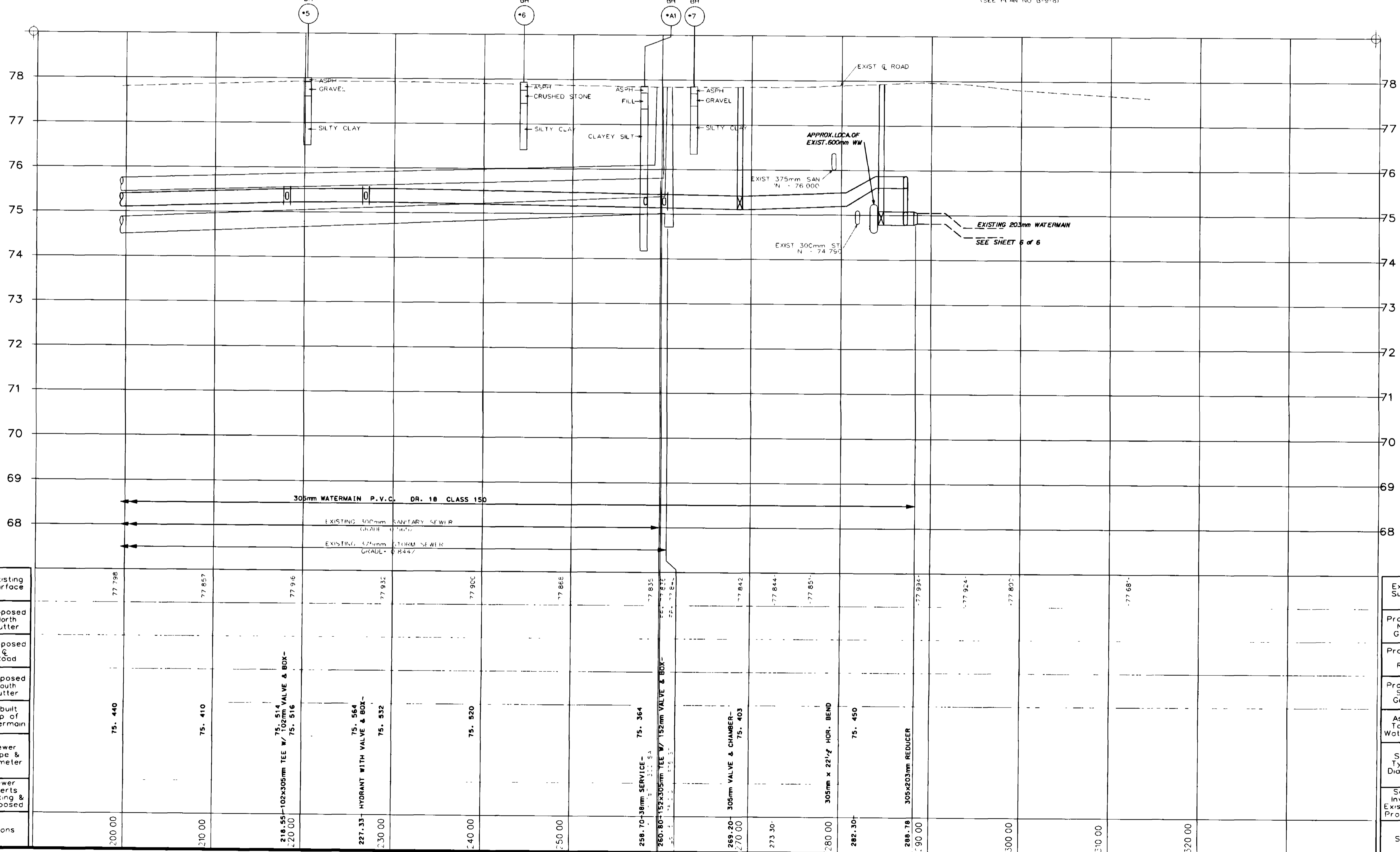


MATCH LINE 200.00

(SEE SHEET NO. 5)

KILBORN AVENUE
(SEE SHEET NO. 6)

BANK STREET
(SEE PLAN NO. B-5-a)



Existing Surface	77.798
Proposed North Gutter	77.857
Proposed Road	77.916
Proposed South Gutter	77.932
Asphalt Top of Watermain	75.410
Sewer Type & Diameter	305mm WATERMAIN P.V.C. DR. 18 CLASS 150
Sewer Inverts Existing & Proposed	200.00
Stations	200.00

Existing Surface	77.798
Proposed North Gutter	77.857
Proposed Road	77.916
Proposed South Gutter	77.932
Asphalt Top of Watermain	75.410
Sewer Type & Diameter	305mm WATERMAIN P.V.C. DR. 18 CLASS 150
Sewer Inverts Existing & Proposed	200.00
Stations	200.00

No.	Date	Description	Drawn By	Approved By
1	March 2000	Asphalt Watermain	M Wood	

Designed By	Date	Checked By	Date
Survey, Detail By	Date	Field Checked By	Date
Drawing By	Date	Checked By	Date

PROFESSIONAL ENGINEER
REG. REHBEIN
PROVINCE OF ONTARIO
ROAD & SEWER

PROFESSIONAL ENGINEER
F. MARCUCCIO
PROVINCE OF ONTARIO
WATER ONLY

PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
NARROWINGS

Chief Designer: H. V. Pascoe, P. Eng.

Construction Type	WATERMAIN, ROAD AND SIDEWALKS	Inspector	GUY QUROUETTE
Work Commenced	APRIL 26, 1999	Project Manager	REG. REHBEIN
Work Completed	AUGUST 1999	Field Book #	NL
Contractor	CLEARY'S LTD	Date	AUGUST 9, 1999
Drawn By	GUY QUROUETTE	Checked By	

- Notes:**
- While illustrations and utilities shown are taken from the best available information, they cannot be guaranteed.
 - The contractor is requested to check with utility companies before digging.
 - Soil information shown is not guaranteed and contractors are advised to collect additional soils information as deemed necessary.
 - The actual sewer line was recorded during construction of the existing sewer.
 - Soil information taken from J.D. PATERSON & ASS. LTD. *S6385-1 *G6929-1
 - Reference bench mark
 - Date of television inspection: Oct 12, 1990 & March 1, 1995
 - This plan super-sedes (in whole or in part) plan

- Watermain Notes:**
- All watermain materials and construction methods shall be in accordance with the latest edition of the R.M.O.C. Environment Section Specifications and Standard Drawings.
 - All connections of new watermains to existing watermains and of blankings of existing mains and services shall be performed by R.M.O.C. staff. The Contractor shall provide excavation, backfilling and reinstatement.
 - All copper services (19mm to 51mm) shall be installed by R.M.O.C. staff after the watermain has been successfully disinfected.
 - All new water services shall be installed at 2.4m cover.
 - All new water services that conflict with sanitary and storm sewers at crossings shall be installed under the sewers unless otherwise directed by the Regional Project Manager.
 - The proposed watermain shall be insulated at specified locations per R.M.O.C. Specification WSD-21 and 23.
 - A minimum 2.0m separation is required between all new water services and catch basins or open structures and shall be insulated per R.M.O.C. Specification WSD-23 as applicable.
 - A minimum 2.0m separation is required between all new hydrants and catch basins or open structures and shall be insulated per R.M.O.C. Specification WSD-23 as applicable.
 - The Contractor shall be responsible to determine via excavation the exact location and elevation of the existing watermains as required for all connections, relocations and blankings.

Regional Municipality of Ottawa-Gatineau
Municipal Department of Transportation

Municipal Department of Urban Services
Le Service de Transport

Approved

Date: _____
WATER WORKS

5238 - 3

RMOC Drawing No. 5238

Boundary information shown hereon has been compiled and calculated from Teranel data and not based on an actual survey. Distances shown to survey monuments are for reference purposes only; survey monuments may not define property boundaries.

THIS IS NOT A PLAN OF SURVEY

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City of Ottawa
Ville d'Ottawa

Department of Urban Planning & Public Works
Engineering Branch
Design And Construction Division

111 SUSSEX DRIVE, SUSSEX PAVILION, 7TH FLOOR, OTTAWA, ONTARIO K1N 5A1

E.M. Robinson	W.R. Cole, P.Eng.
---------------	-------------------

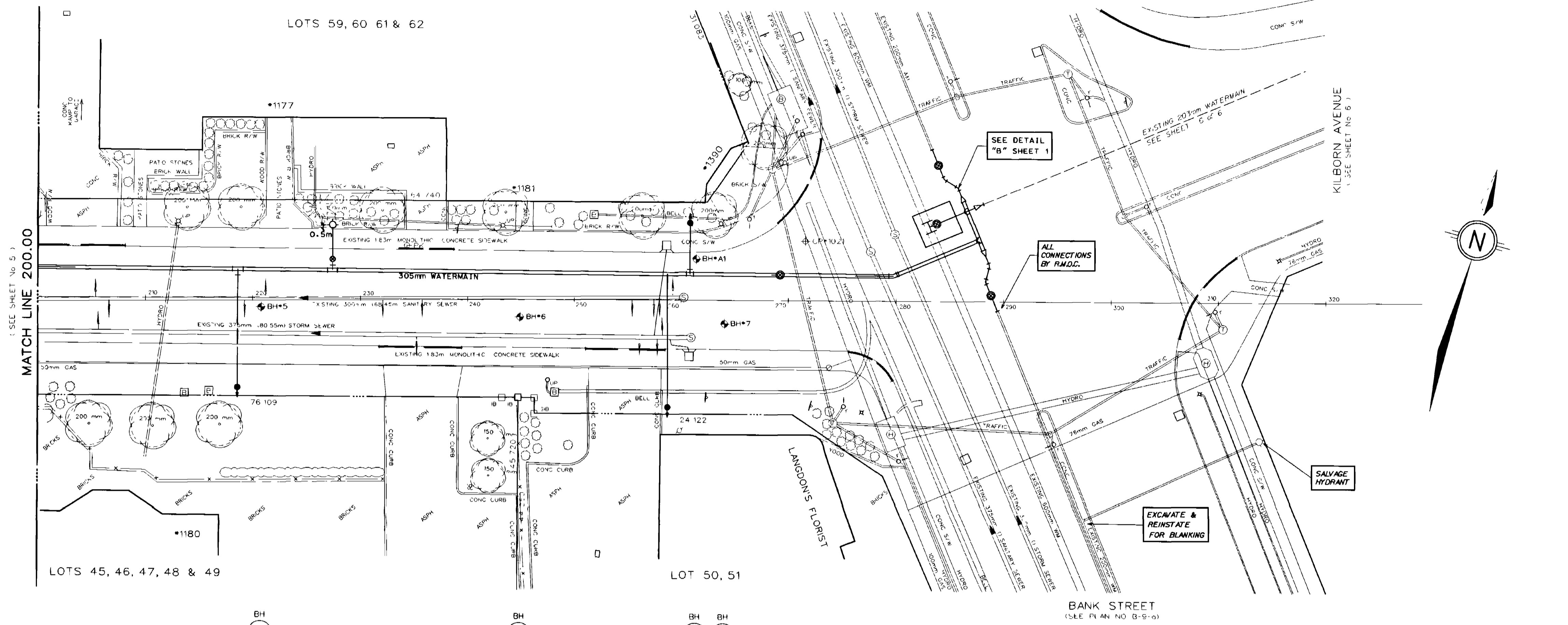
BELANGER AVENUE

FROM STATION 200.00 TO BANK STREET

99C2803	HOR. 1:250 VERT. 1:50	2803
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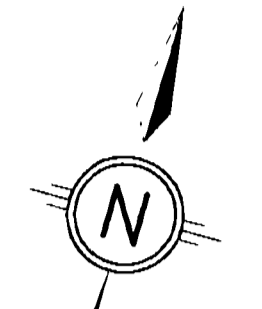
BELANGER AVENUE

LOTS 59, 60 61 & 62



MATCH LINE 200.00

SEE SHEET NO. 5



No	Date	Description	Drawn By	Approved By
1	March 2000	Asbuilt Watermain	M Wood	

Designed By	Date	Checked By	Date
Survey, Detail By	Date	Field Checked By	Date
Drawing By	Date	Checked By	Date

PROFESSIONAL ENGINEER
REG REHBEIN
PROVINCE OF ONTARIO
ROAD & SEWER

PROFESSIONAL ENGINEER
F MARCUCCIO
PROVINCE OF ONTARIO
WATER ONLY

PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
NARROWINGS

Chief Designer: **H. V. Pascoe, P. Eng.**

Construction Type	Inspector
WATERMAIN, ROAD AND SIDEWALKS	GUY QUROUETTE
Work Commenced	Project Manager
APRIL 26, 1999	REG REHBEIN
Work Completed	Field Book #
AUGUST 1, 1999	ML
Contractor	Date
CLEARY'S LTD	AUGUST 9, 1999
Starting Date	Checked By
AUGUST 9, 1999	GUY QUROUETTE

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 - Reference bench mark
 - Date of television inspection Oct 12, 1990 & March 1, 1995
 - This plan super-edges (in whole or in part) plan

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 - 8 A minimum 2.0m separation is required between all new hydrants and catch basins or open structures and shall be insulated per R.M.O.C. Specification WSD-23 as applicable.
 - 9 The Contractor shall be responsible to determine via excavation the exact location and elevation of the existing watermains as required for all connections, relocations and blankings.

Approved

Date

WATER WORKS

RMOC Drawing No. 5238

5238 - 3

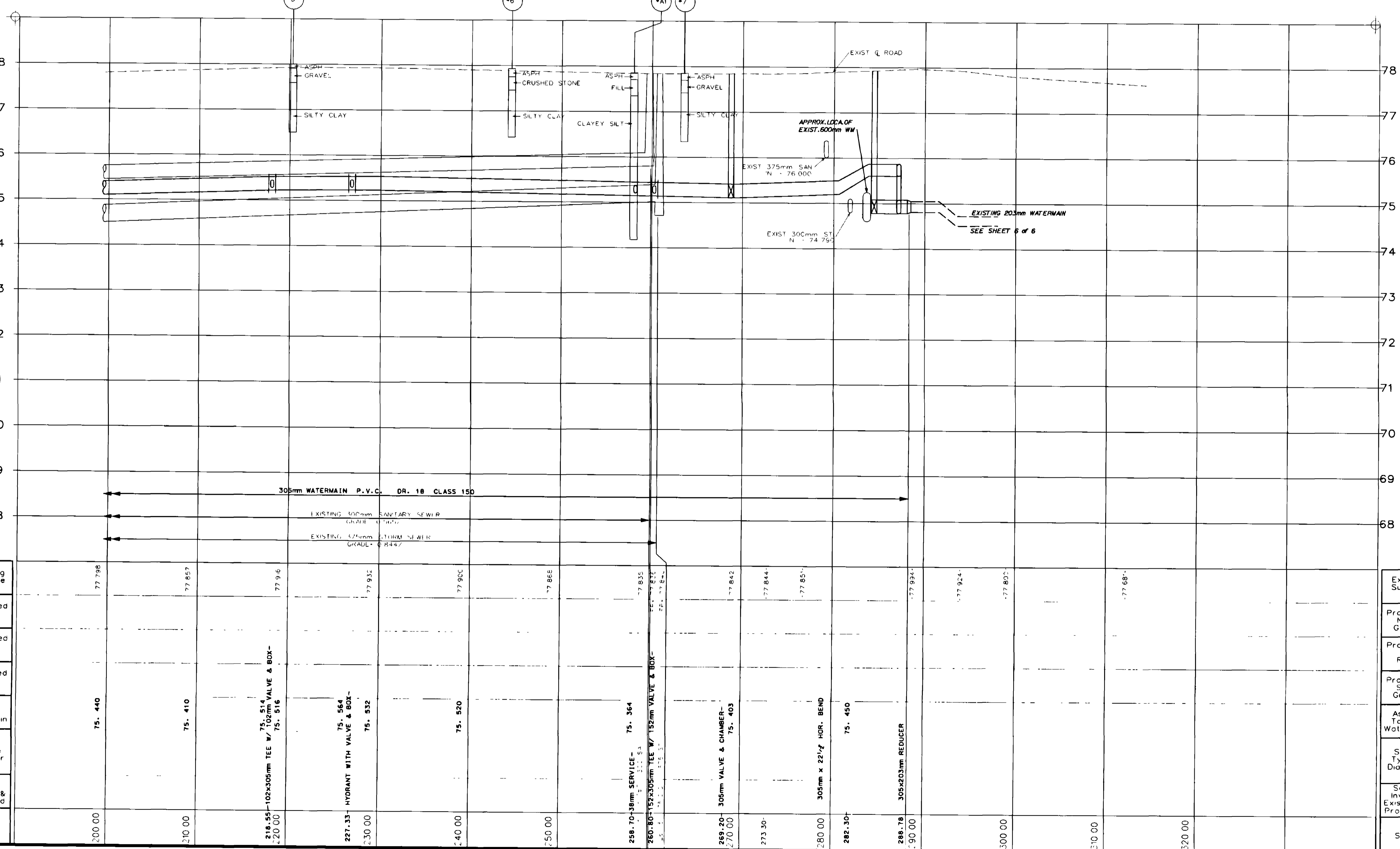
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THIS IS NOT A PLAN OF SURVEY

This plan was compiled from plans and documents recorded on the Land Registry System and has been prepared for property marking purposes only.

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Existing Surface	77.798	77.857	77.916	77.932	77.900	77.868	77.835	77.842	77.844	77.851	77.893	77.924	77.892	77.891	77.891
Proposed North Gutter															
Proposed Road															
Proposed South Gutter															
Asbuilt Top of Watermain	75.410	75.410	75.514	75.516	75.564	75.532	75.364	75.403	75.450	75.450	75.450	75.450	75.450	75.450	75.450
Sewer Type & Diameter	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150
Sewer Inverts Existing & Proposed	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00
Stations	200.00	210.00	220.00	230.00	240.00	250.00	260.00	270.00	280.00	290.00	300.00	310.00	320.00	330.00	340.00

Existing Surface	77.798	77.857	77.916	77.932	77.900	77.868	77.835	77.842	77.844	77.851	77.893	77.924	77.892	77.891	77.891
Proposed North Gutter															
Proposed Road															
Proposed South Gutter															
Asbuilt Top of Watermain	75.410	75.410	75.514	75.516	75.564	75.532	75.364	75.403	75.450	75.450	75.450	75.450	75.450	75.450	75.450
Sewer Type & Diameter	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150	305mm WATERMAIN P.V.C. DR. 18 CLASS 150
Sewer Inverts Existing & Proposed	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00	218.55-102x305mm TEE W/ 102mm VALVE & BOX - 220.00
Stations	200.00	210.00	220.00	230.00	240.00	250.00	260.00	270.00	280.00	290.00	300.00	310.00	320.00	330.00	340.00

City of Ottawa
Ville d'Ottawa

Department of Urban Planning & Public Works
Engineering Branch
Design And Construction Division

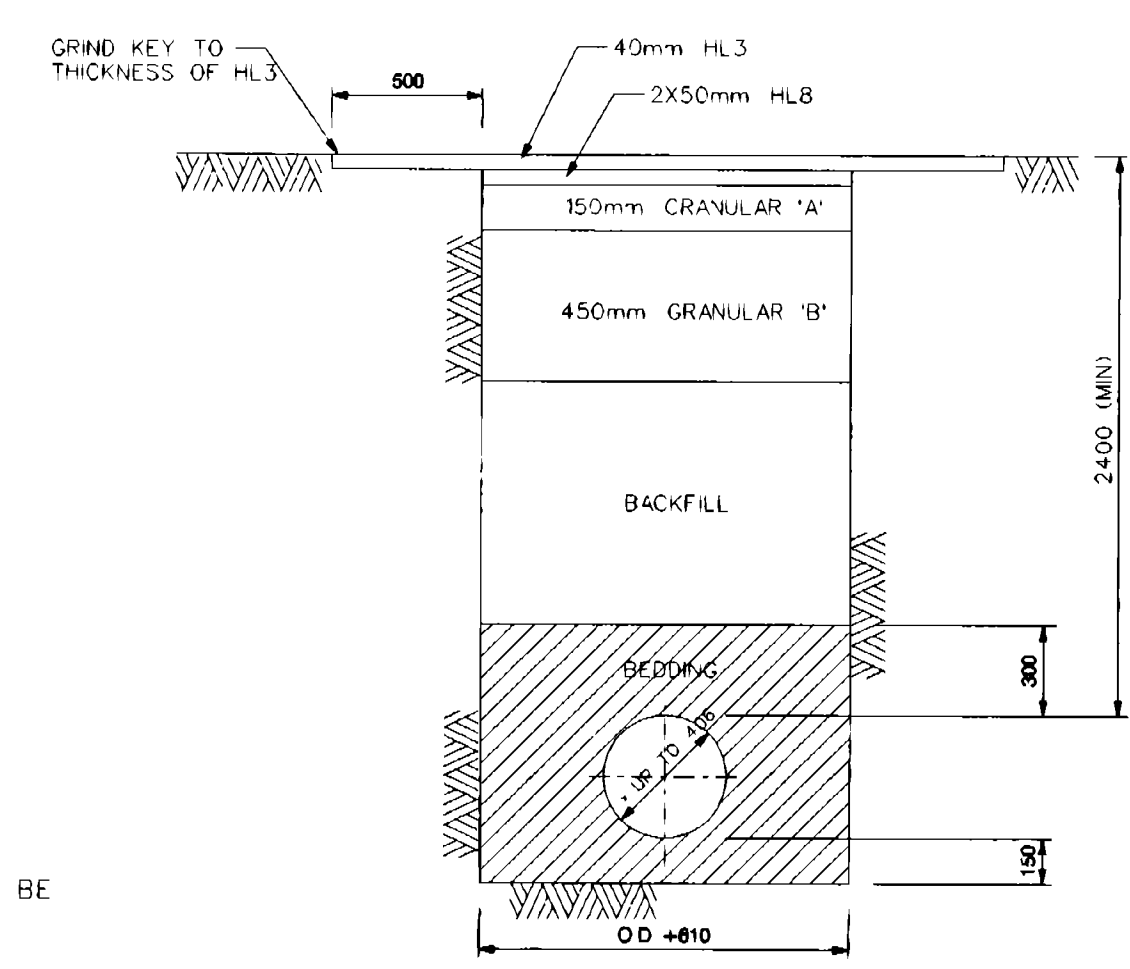
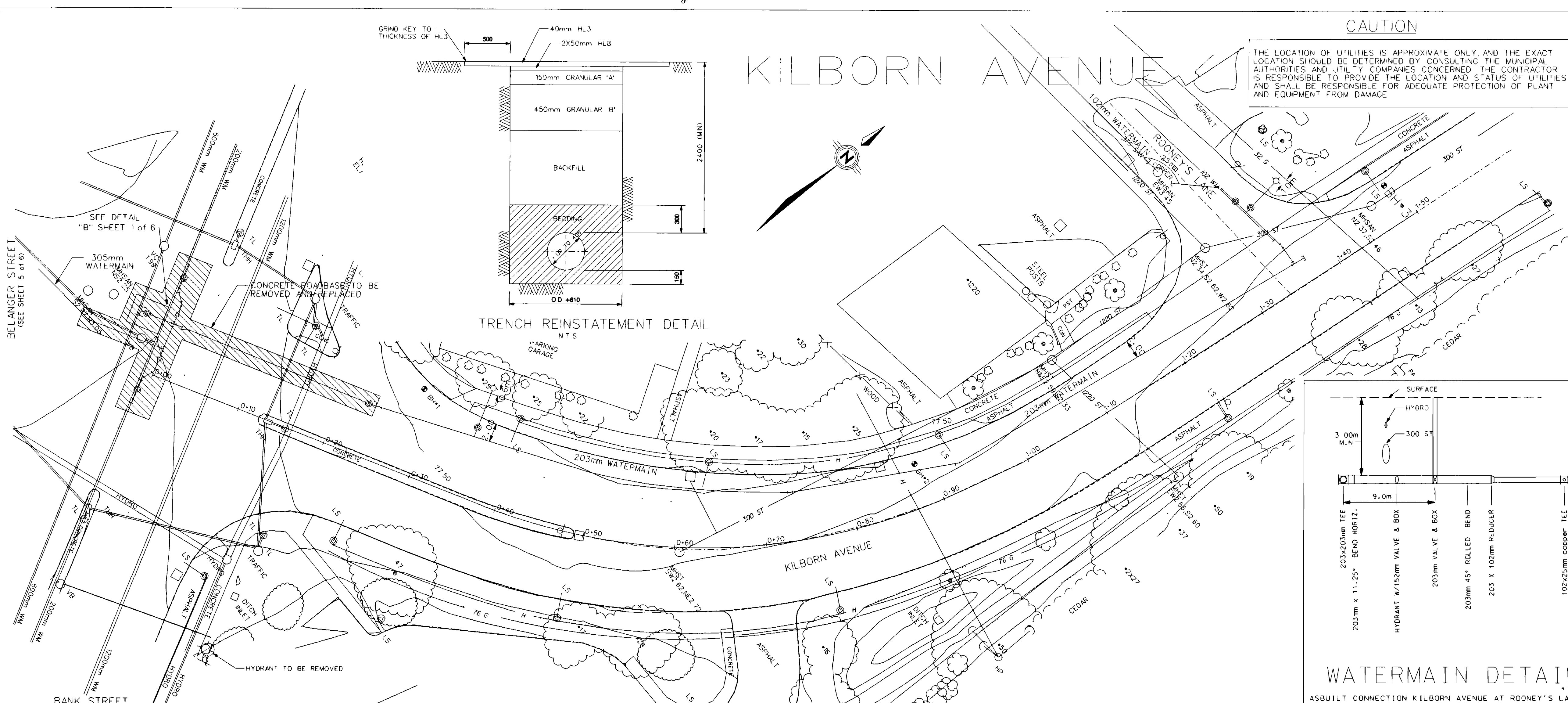
111 SUSSEX DRIVE, SUSSEX PAVILION, 7TH FLOOR, OTTAWA, ONTARIO K1N 5A1

E.M. Robinson
W.R. Cole, P.Eng.

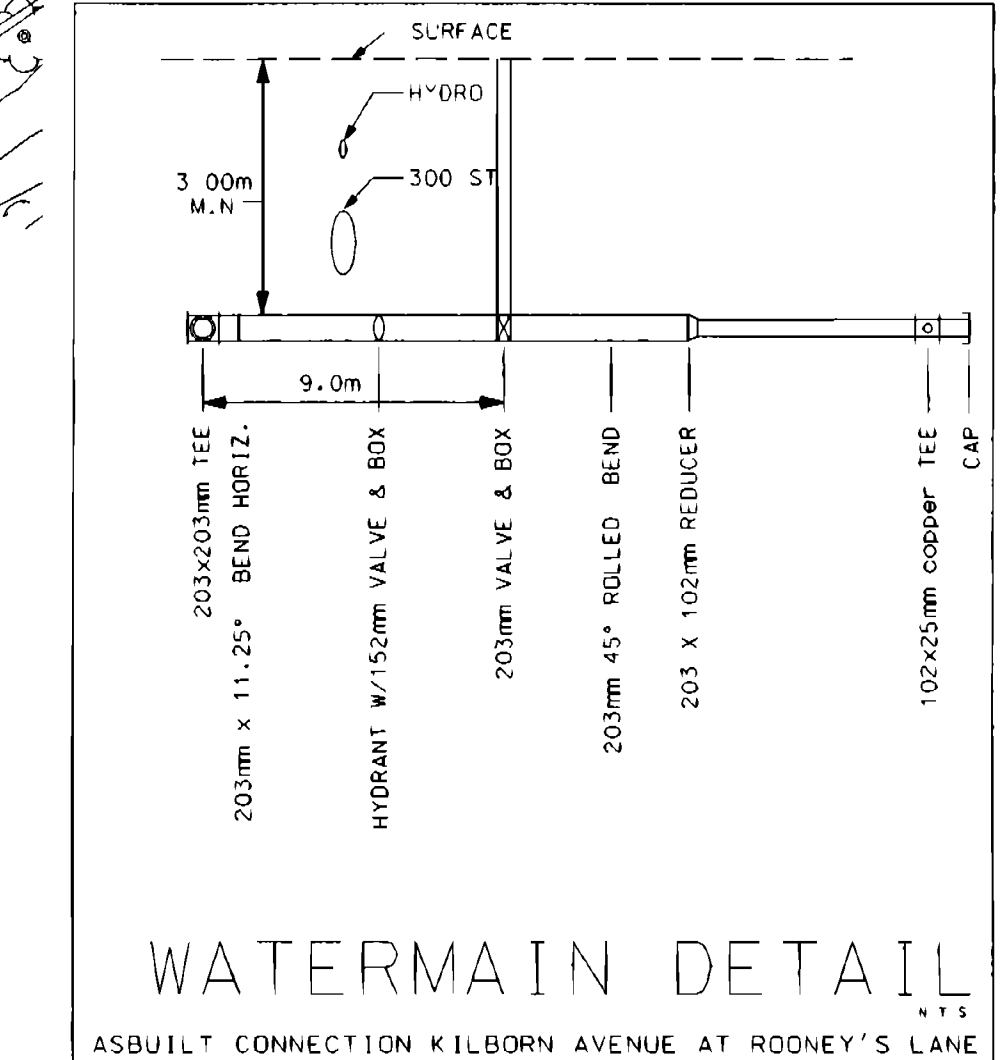
BELANGER AVENUE

FROM STATION 200.00 TO BANK STREET

99C2803
HOR. 1:250
VERT. 1:50
2803



CAUTION
 THE LOCATION OF UTILITIES IS APPROXIMATE ONLY, AND THE EXACT LOCATION SHOULD BE DETERMINED BY CONSULTING THE MUNICIPAL AUTHORITIES AND UTILITY COMPANIES CONCERNED. THE CONTRACTOR IS RESPONSIBLE TO PROVIDE THE LOCATION AND STATUS OF UTILITIES AND SHALL BE RESPONSIBLE FOR ADEQUATE PROTECTION OF PLANT AND EQUIPMENT FROM DAMAGE.



Ottawa-Carleton

ENVIRONMENT and TRANSPORTATION DEPARTMENT

M J E SHEFLIN P.Eng
 ENVIRONMENT and TRANSPORTATION COMMISSIONER

Approved by: _____ Date: _____

Engineering Services: _____

Project Officer: MICHAEL WILLMETS Date: _____

Drawn by: JAMES G CHARRON Verified by: _____ Date: _____

Survey details by: _____ Book: _____ Date: _____

REGION: _____

As Built Inspection by: _____ Date: _____

- NOTES**
- SOIL INFORMATION, IF SHOWN IS NOT GUARANTEED. CONTRACTORS ARE ADVISED TO COLLECT ADDITIONAL SOIL INFORMATION AS DEEMED NECESSARY. SEE McROSTIE, GENEST, ST. - LOUIS REPORT NO SF-4518 FROM NOV 3, '98.
 - ALL COPPER SERVICES (19mm TO 57mm) SHALL BE INSTALLED BY R.M.O.C STAFF AFTER THE W.M HAS BEEN SUCCESSFULLY DISINFECTED.
 - ALL WATERMAIN MATERIALS AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE R.M.O.C STANDARD SPECIFICATIONS AND STANDARD DRAWINGS.
 - ALL CONNECTIONS OF NEW W.M TO EXIST W.M AND ALL BRANCHINGS OF EXIST MAINS AND SERVICES SHALL BE PERFORMED BY R.M.O.C FORCES. THE CONTRACTOR SHALL PROVIDE EXCAVATION, BACKFILL AND REINSTATEMENT.
 - ALL NEW WATER SERVICES SHALL BE INSTALLED AT 2.4m COVER.
 - THE CONTRACTOR IS RESPONSIBLE TO MAINTAIN IN GOOD WORKING ORDER ALL SERVICE LATERALS ENCOUNTERED DURING CONSTRUCTION.
 - REINSTATEMENT SHALL BE IN ACCORDANCE WITH REQUIREMENTS OF THE LOCAL MUNICIPAL AUTHORITY, OR AS SPECIFIED.
 - ALL WATER SERVICES THAT CONFLICT WITH SAN AND STORM SEWERS AT CROSSINGS SHALL BE INSTALLED UNDER THE SEWERS UNLESS OTHERWISE DIRECTED BY THE R.M.O.C PROJECT MANAGER.
 - THE PROPOSED W.M SHALL BE INSULATED AT SPECIFIED LOCATIONS AS PER R.M.O.C SPEC WSD-23.
 - A MINIMUM 2m SEPARATION IS REQUIRED BETWEEN ALL NEW WATER SERVICES OR HYDRANTS AND CATCH-BASINS OR OPEN STRUCTURES AND SHALL BE INSULATED AS PER R.M.O.C SPEC WSD-23 AS APPLICABLE.

Approved: _____

DATE: _____

R.M.O.C Drawing No 5238

M/W	ASBUILT	1	03/00
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By: _____ Description: _____ Rev: _____ Date: _____

Scale: **HORIZ 1:250**
VERT 1:50

Project Title: **KILBORN STREET**
203mm WATERMAIN
BANK STREET TO ROONEY'S LANE

Drawing No: **5238** Sheet No: **4 of 1**

