

PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA

SERVICEABILITY REPORT
REPORT No. R-824-83A (REV. #1)
MARCH 2025

T.L. MAK ENGINEERING CONSULTANTS LTD.
AUGUST 2024
REFERENCE FILE NUMBER 824-83

Introduction

The developer of this property is proposing to redevelop the existing residential lot described as Lot 36 Registered Plan 114 City of Ottawa by constructing a six (6) storey residential apartment building plus a basement consisting of thirty (30)-units, including thirteen (2)-bedroom units, eleven (1)-bedroom units and six (6) studio units.

The municipal address of this property is referenced as 342 Roosevelt Avenue and it is located in the City Ward (15 - Kitchissippi). The site is situated on the west side of Roosevelt Avenue, south of Workmen Avenue and north of Richmond Road, see site plan and legal survey plan in Appendix A for details.

The area of this property is ± 0.0647 hectares. In addition to the six (6) storey residential building, the other development features will comprise of an interlock paver access to the front entrance plus an interlock paver access to the waste storage and bike racks at the front (southeast) side of the building including access to the south building entrances along the south side yard and an amenity area is located at the north side yard including landscaped areas throughout the site, etc., to meet the City of Ottawa's site plan requirements.

A site geotechnical report was prepared by the owner's soils engineer Paterson Group entitled "Geotechnical Investigation – Proposed Residential Building" 342 Roosevelt Avenue (Project No. PG4210-LET.01) dated August 29, 2017 for this proposed development property.

This serviceability report will provide the City of Ottawa with our serviceability brief to address the proposed servicing scheme for this site.

Existing Site Conditions and Servicing

This property is presently occupied by a two (2) storey brick residential building. The existing house is located near the front centre on this property with an one (1) storey vinyl building addition to the south side of the existing house and asphalt parking area located along the front northeast corner of the property limit which currently provides vehicle access and parking for this lot. For additional details of the site's pre-development conditions, refer to the coloured Google Image (2020) and aerial photography from (GeoOttawa 2022) in Appendix B.

Approximately two thirds (2/3) of this site is currently permeable surface covered consisting of grass/landscaped areas with the remaining areas being roof area, asphalt laneway, asphalt walkway, covered frame steps and deck. Currently, most of the landscape areas are concentrated at the rear of lot and along the north side yard.

The topography of the land is found to be gently sloping and graded primarily to drain from rear to the front of the lot (south to north). The existing gradient of the property is sloping approximately 0.5% from back to front.

The existing house water and sanitary service lateral currently servicing the existing dwelling on 342 Roosevelt Avenue will be removed. The existing water services shall be blanked at the main and the existing house laterals shall be capped at the front property line for re-development of this lot.

As for the availability of underground municipal services, there are existing municipal services along Roosevelt Avenue in front of this property consisting of a 450mm diameter storm sewer, a 375mm diameter sanitary sewer, and a 150mm diameter watermain for development of this property. Refer to the City of Ottawa Roosevelt Avenue As-Built plan and profile drawings included in Appendix C for details.

Because the site will be connecting to and outletting into the separated Athlone Avenue storm sewer located within the Roosevelt Avenue road right of way in the City of Ottawa, therefore, the approval exemption under Ontario Regulations 525/98 would apply since storm water discharges from this site will outlet flow into a downstream storm sewer. Thus, an Environmental Compliance Approval (ECA) application will not be required to be submitted to the Ministry.

Proposed Residential Apartment Building Site

There are no requirements for vehicle access or parking for this site. Interlock pavers are proposed at the front and at the south side of the new building for pedestrian access to the waste disposal and bicycle parking located adjacent to the southeast quadrant of the new building.

A. Water Supply

The proposed building located within Pressure Zone 1W at 342 Roosevelt Avenue is a 6-storey multi-unit residential building, with a basement. The building comprises 30 units in total ranging from studio (bachelor) to 2-bedroom units. The breakdown of the units is listed in **Table 1**.

Table 1: Proposed Unit Counts

Unit	Unit Number
Studio	6
One Bedroom	11
Two Bedroom	13
Total	30

On average each floor covers an area of around 3,334 ft² (310 m²) for a total gross floor area of 23,339 ft² (2,169 m²). The building is to be serviced by the 150 mm diameter watermain along Roosevelt Avenue. The ground elevation along Roosevelt Avenue is approximately 66.9 m.

Demand Projections

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines, where the residential consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Persons per unit (PPU) for each unit were estimated based on the City of Ottawa's Water Design Guidelines. Total resident count for the proposed units was estimated at 53.

Following discussions with the City, peaking factors are to be estimated from Table 3-3 of the MECP Design Guidelines for Drinking-Water Systems, given that the proposed development population is less than 500 people. Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 8.6. Peak hour (PKHR) demands were calculated by multiplying AVDY by a factor of 13.0. Table 2 shows the estimated domestic demands of the proposed building.

Table 2: Estimated Domestic Demand

Unit Type	Unit Counts	PPU	Population	Consumption	AVDY		MXDY		PKHR	
					L/d	L/s	L/d	L/s	L/d	L/s
Apartment, Bachelor	6	1.4	9	280	2,352	0.03	20,270	0.23	30,523	0.35
Apartment, 1-Bedroom	11	1.4	16		4,312	0.05	37,162	0.43	55,959	0.65
Apartment, 2-Bedroom	13	2.1	28		7,644	0.09	65,879	0.76	99,200	1.15
Total	30	-	53	-	14,308	0.17	123,311	1.43	185,682	2.15

As per the City of Ottawa's Water Design Guidelines (and Technical Bulletin IWSTB-2024-05), the Fire Underwriter Survey (FUS) method is to be used for fire flow requirements affecting municipal watermain sizing; with regards to fire protection on private property and not requiring new watermains, these are covered by the Ontario Building Code (OBC), using the OBC's Office of the Fire Marshal (OFM) method. If the required flow using the OBC/OFM method yields 9,000 L/min for the property, Technical Bulletin IWSTB-2024-05 specifies the FUS method to be used instead.

It is understood that the building will be of ordinary construction (min. 1-hour fire rated exterior walls). As a conservative approach, Type IV construction material classifications was assumed for the OBC calculations. The building will also be equipped with sprinklers. Based on the provided draft plans, the basement is more than 50% above ground level, hence the basement level was included in the proceeding fire flow calculations.

Based on the OBC/OFM fire flow calculations, a total of 9,000 L/min for a duration of minimum of 40 mins is required. As per Technical Bulletin IWSTRB-2024-05, the FUS method should be used to calculate the required fire flow for the proposed property. The FUS calculation resulted in a total required fire flow of 8,000 L/min (133.5 L/s) for a duration of 2 hours. The fire flow requirement calculations are provided in the attached worksheet in Appendix D.

In summary, the estimated water demands for the proposed building are as follows:

- AVDY = 14,308 L/d (0.17 L/s)
- MXDY = 123,311 L/d (1.43 L/s);
- PKHR = 185,682 L/d (2.15 L/s); and,
- Fire Flow (FUS) = 8,000 L/min (133 L/s).

Details are provided in the attached under **FUS Fire Flow Calculations** (See Appendix D). **Figure 1** found in Appendix D provides separation distances from adjacent buildings. The proposed **Site Plan** attached in Appendix D was used to determine distances from the proposed building to the property lines.

Boundary Conditions

The hydraulic gradeline (HGL) boundary conditions for 342 Roosevelt Avenue, as presented in **Table 3**, were provided by the City on June 24, 2024 (see attached **Water Boundary Conditions Email** in Appendix D).

Table 3: Boundary Conditions

Demand Scenario	Head (m)	Flow (L/s)
Minimum HGL (Peak Hour)	108.7	
Maximum HGL (Average Day)	115.3	
Available Fire Flow @ Residual 20 psi		133.5

Hydraulic Analysis

Peak Hour & Average Day

During peak hour demands, the resulting minimum hydraulic gradeline of 108.7 m corresponds to a peak hour pressure of 410 kPa (59.5 psi). This value is above the minimum pressure objective of 276 kPa (40 psi) for residential buildings up to two storeys. Adding 5 psi¹ per floor above two stories, to account for headloss due to elevation and pipe losses, a minimum pressure of 413 kPa (60 psi) would be required for the sixth floor. The peak hour pressure at ground level is slightly below this objective but is still within reasonable range for peak hour conditions. As such, it is deemed acceptable.

During average day demands, the resulting maximum hydraulic gradeline of 115.3 m corresponds to a maximum pressure of 474 kPa (69 psi). This value is below the maximum pressure objective of 552 kPa (80 psi).

Supporting hydraulic calculations are attached in Appendix D.

¹ It is noted that the design of the building's plumbing system could have an impact on the anticipated pressures on the higher floors. While an allowance of 5 psi per floor is used to assess the need for additional pumping at the planning stage, the need for additional pumping within the building will need to be confirmed by the mechanical/plumbing designer of the building.

Maximum Day + Fire Flow

The reported available fire flow at a residual pressure of 20 psi is 133.5 L/s (8,010 L/min). This is greater than the RFF of 133 L/s or 8,000 L/min, as per FUS, and therefore considered acceptable. Hydrant coverage and classes in the vicinity of the proposed building are illustrated in **Figure 2** attached in Appendix D.

Based on Table 1 of Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 and discussion with the City, as well as a desktop review (i.e., Google Street View) to confirm hydrant class, two (2) Class AA hydrants are located in the vicinity of the proposed building along Roosevelt Avenue. One (1) hydrant is located within 75 m, with a capacity contribution of up to 5,700 L/min. The other hydrant is located between 75 m and 150 m with a capacity contribution of up to 3,800 L/min. The combined hydrant flow coverage for 342 Roosevelt Avenue is therefore calculated at 9,500 L/min, which is above the FUS required fire flow of 8,000 L/min. A breakdown of the hydrant coverage is summarized in **Table 4** below.

Table 4: Fire Hydrant Coverage

Building	Fire Flow Demand (L/min)	Fire Hydrants					Combined Hydrant Flow Coverage (L/min)
		Hydrant Class	Within 75 m		Between 75 m and 150 m		
			Quantity	Max Contrib. to RFF	Quantity	Max Contrib. to RFF	
342 Roosevelt Avenue	8,000 L/min (FUS)	AA	1	5,700	1	3,800	9,500
		A					
		B					
		C					

In conclusion, based on boundary conditions provided, the local watermain network in the vicinity of the proposed building at 342 Roosevelt Avenue provides adequate fire flow capacity as per the Fire Underwriters Survey (FUS) method. Anticipated demand flows meet pressure objectives during average demand conditions, as per City of Ottawa's Drinking Water Design Guidelines. During peak hour conditions, anticipated minimum pressure at ground level meets pressure objectives for residential buildings up to two storeys. However, minimum peak hour

pressure is slightly below the objective when considering the 6th floor (accounting for additional headloss due to elevation and pipe losses using a rule of thumb value of 5 psi per floor). Nonetheless, minimum pressure is still within reasonable range at the 6th floor for peak hour conditions and is deemed acceptable. It is noted that the building's mechanical/plumbing designer will have to confirm at the design phase that additional pumping is not needed for higher-level floors based on plumbing design of the building.

B. Sanitary Flow

The peak sanitary flow for the 30 units, which comprise of thirteen (2)-bedroom, eleven (1)-bedroom and six studio apartment units, is estimated at $Q = 0.59$ L/s with an infiltration rate of 0.02 L/s. Refer to Appendix E sheet 1 of 1 regarding sanitary flow calculations. This flow will enter the existing 375mm diameter sanitary sewer on Roosevelt Avenue via the proposed 150mm diameter PVC sanitary service lateral from the six (6)-storey residential apartment building.

The existing peak sanitary flow of the site for single detached dwelling unit is $Q = 0.06$ L/s with an infiltration rate of 0.02 L/s. The net increase in flow from this proposed development is 0.53 L/s which is not expected to negatively impact the existing 375mm dia. sanitary sewer.

Waste water from the Roosevelt Avenue 375mm dia. sanitary sewer then in turn outlets north into the existing downstream 1500mm \times 1500mm dia. West Nepean sanitary collector sewer located along the transitway corridor which further direct sewage flow eastward into the existing 1650mm dia. sanitary collector sewer located east of Island Park Drive.

C. Storm Flow

The storm-water outlet for the proposed development property will be the existing 450mm diameter concrete storm sewer located on Roosevelt Avenue. Stormwater attenuation on site will be accomplished by means of rooftop storage with controlled roof drains that regulate flow off site.

The building foundation weeping-tile drainage system shall have its own separate pipe for gravity flow where weeping-tile water is outletted via a 150mm diameter storm pipe to the existing 450mm diameter storm sewer. The storm-water outlet for the rooftop water from roof drains will be a separately designated proposed 150mm diameter PVC pipe that will also be outletted directly into the existing 450mm diameter storm sewer. The 150mm dia. roof water drain pipe will "we" into the 150mm dia. weeping tile storm lateral on private property and outlet to the existing Roosevelt Avenue storm sewer.

Two (2) roof drains are proposed for this apartment building to restrict flow at a maximum flow rate of 0.95 L/s each or $2 \times 0.95 \text{ L/s} = 1.90 \text{ L/s}$ into the Roosevelt Avenue storm sewer. The calculated net allowable controlled release rate from this site is estimated at 7.87 L/s.

Based on the residential site plan from the owner's architect, the average post-development runoff coefficient is estimated at $C = 0.61$ and $A = 0.0647$ hectares.

An estimation of the pre-development flow condition was carried out using the criteria accepted by the City of Ottawa. If post-development C value exceeds the lesser of the $C_{pre} = 0.42$ or $C_{allow} = 0.5$ (max) then SWM is required. So from our calculations, the $C_{pre} = 0.42$ value will be used at $t_c = 10$ minutes for pre-development allowable flow calculation off-site.

The pre-development calculated flow rate into the 450mm dia. storm sewer for this residential area is the lesser of either the five (5)-year storm event where $C_{allow} = 0.5$ (max.) runoff value or the average C_{pre} value which is 0.42 using $t_c = 10$ minutes. Because this site $C_{post} = 0.61$ and $C_{pre} = 0.42$ then SWM measures are required.

Therefore, based on our calculation, on-site retention is required for this proposed development site, because the site post-development C value of 0.61 is greater than the $C_{pre} = 0.42$.

The storage volume for the five (5)-year and up to the 100-year storm event will be stored by means of flat rooftop at the top of the 6-storey apartment building. Also refer to the site storm drainage report (Report No. R-824-83) for further details.

Conclusion

At this proposed residential site and to develop this lot to house a (6)-storey 30 unit apartment building on a 0.0647 ha. parcel of land, the estimated allowable flow off-site is calculated at 7.87 L/s based on City of Ottawa Drainage and Stormwater Management (SWM) criteria of 5-year pre-development flow at $C_{pre} = 0.42$. For on-site SWM attenuation, the flat roof top of the proposed apartment building will be utilized and (2) controlled roof drains are incorporated each with a controlled maximum release rate of 0.95 L/s (15.0 U.S. gal/min.). The controlled flow from this site totals to 1.74 L/s for the 5-Year post development condition and 1.9 L/s for the 100-Year post development condition. The uncontrolled 5-year post development flow from the remainder of the site is estimated at 3.65 L/s and 7.30 L/s for the 100-year event respectively.

During the five (5)-year storm event for the flat rooftop storage, the ponding depth of rooftop area 1 and 2 is estimated at 120 mm at the drain and 0mm at the roof perimeter, assuming a 1.5% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 3.49 m^3

and the rooftop storage available at Roof Area 2 is 3.81 m^3 , for a total of 7.30 m^3 , which is greater than the required volume of 4.19 m^3 .

During the 100-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1 and 2 is estimated at 150 mm at the drain and 0mm at the roof perimeter, assuming a 1.5% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 7.12 m^3 and the rooftop storage available at Roof Area 2 is 7.42 m^3 , for a total of 14.54 m^3 , which is greater than the required volume of 10.34 m^3 .

Therefore, by means of flat building rooftop storage and grading the site to the proposed grades as shown on the Proposed Grading and Servicing Plan and Proposed Rooftop Stormwater Management Plan Dwg. 824-83 G-1 and 824-83 SWM-1 respectively, the desirable five (5)-year storm and 100-year storm event detention volume of 7.30 m^3 and 14.54 m^3 respectively will be available on site.

Thus for this development site, the 5-year maximum post development flow draining off-site is the controlled roof top flow plus the uncontrolled flow from the remainder of the site totals to 5.39 L/s ($1.74 \text{ L/s} + 3.65 \text{ L/s}$) which is less than the allowable 7.87 L/s . For event up to and including 100 year, the estimated maximum post development flow draining off-site is 9.20 L/s ($1.90 \text{ L/s} + 7.30 \text{ L/s}$) which exceeds the site allowable of 7.87 L/s by 1.33 L/s for this site.

In comparing the pre-development flow of the current site conditions to the post development flow, the SWM regulated flow plus uncontrolled flow from the proposed site under the post development conditions at the 5-year event = 5.39 L/s and the 100 year event = 9.20 L/s where both of the post development flow events are less than current pre-development flow estimate for the site at 5-Year $_{pre} = 7.87 \text{ L/s}$ and 100-Year $_{pre} = 15.73 \text{ L/s}$. Therefore with this proposed development, stormwater flow is improved from that of the existing condition.

The building weeping tile drainage will outlet via its separate 150mm diameter PVC storm lateral. The roof drains will be outletted also via a separate 150mm PVC storm lateral from the apartment building which "we" into the proposed 150mm dia. weeping tile storm lateral, whereupon both laterals are outletting to the existing Roosevelt Avenue 450mm diameter storm sewer with only one (1) connection. The City of Ottawa recommends that pressurized drain pipe material be used in the building for the roof drain leader pipe in the event of surcharging on the City storm sewer system. Refer to the proposed site grading and servicing plan Dwg. 824-83 G-1 for details.

Erosion and Sediment Control

The contractor shall implement Best Management Practices to provide for protection of the receiving storm sewer during construction activities. These practices are required to ensure no sediment and/or associated pollutants are released to the receiving watercourse. These practices include installation of a "silt sack" catch basin sediment control device or equal in catch basins as recommended by manufacturer on-site and off-site within the Roosevelt Avenue road right of way adjacent to this property. Silt sack shall be inspected every 2 to 3 weeks and after major storm. The deposits will be disposed of as per the requirements of the contract. See Dwg. #824-83 ESC-1 for details.

Refer to Appendix F for the summary of the Development Servicing Study Checklist that is applicable to this development.

PREPARED BY T.L. MAK ENGINEERING CONSULTANTS LTD.


TONY L. MAK, P.ENG



**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX A
SITE PLAN AND LEGAL SURVEY PLAN**

**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX B
SITE PRE-DEVELOPMENT CONDITION
GOOGLE IMAGE (2020)
AND
AERIAL PHOTOGRAPHY 2022 (GEOOTTAWA)**







**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX C
ROOSEVELT AVENUE
CITY OF OTTAWA
PLAN AND PROFILE
DRAWINGS**

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OLIVER MANGIONE McCALLA
 & ASSOCIATES, LIMITED
 founding partners
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City Of Ottawa
Department Of Engineering & Works
Engineering And Surveys Branch

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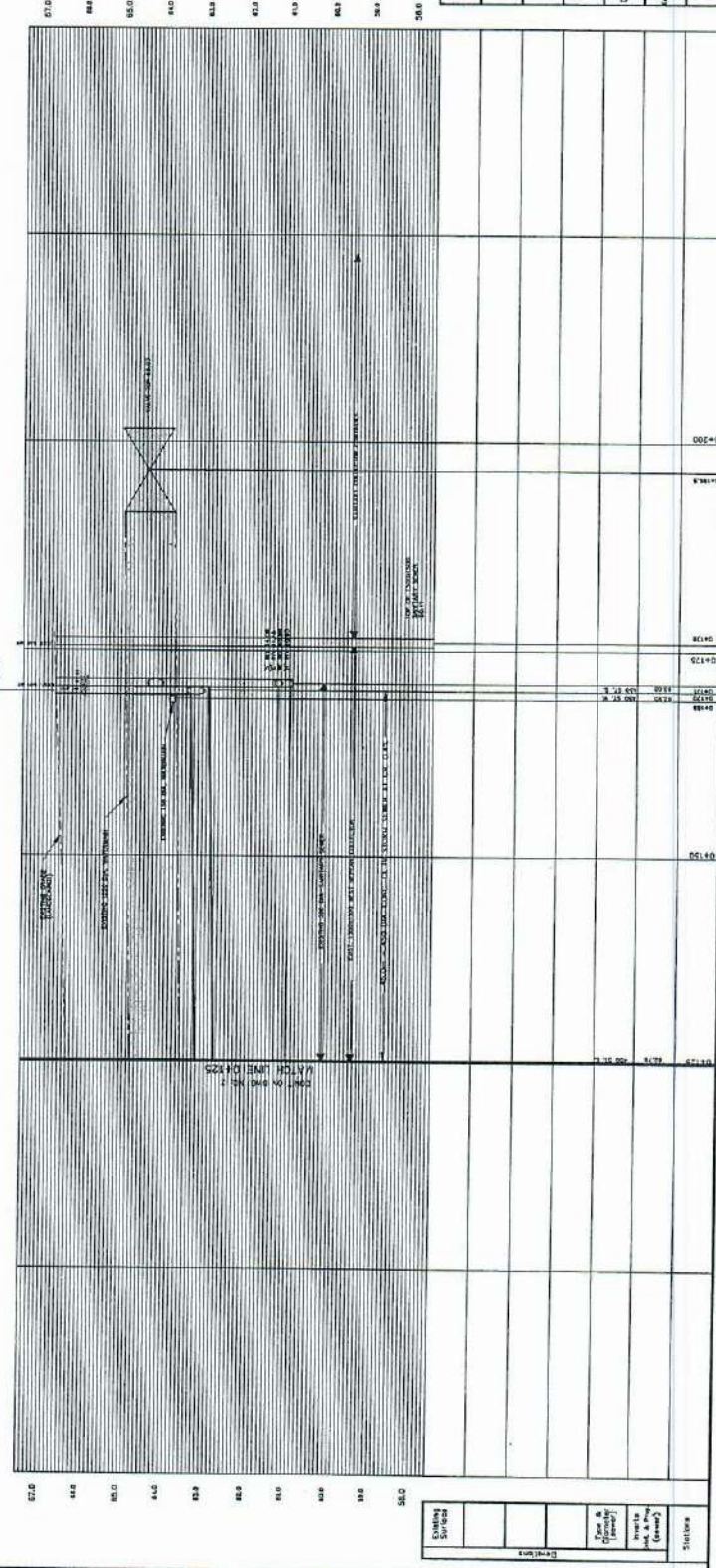
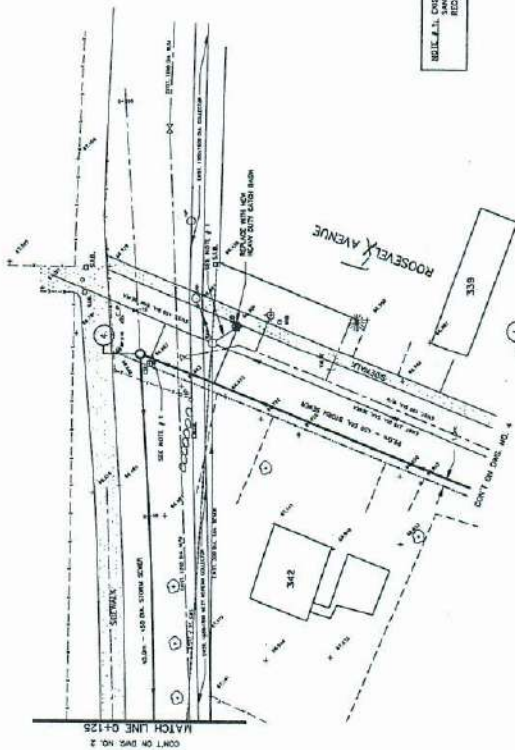
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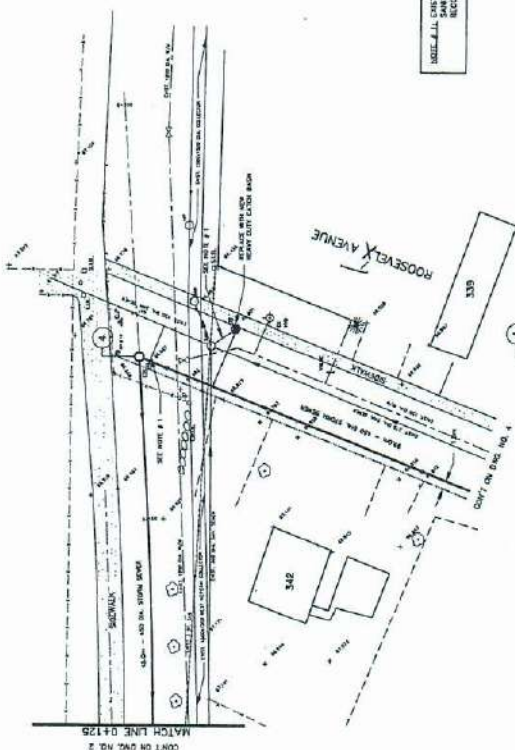


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Department Of Engineering & Works
Engineering And Surveying Branch

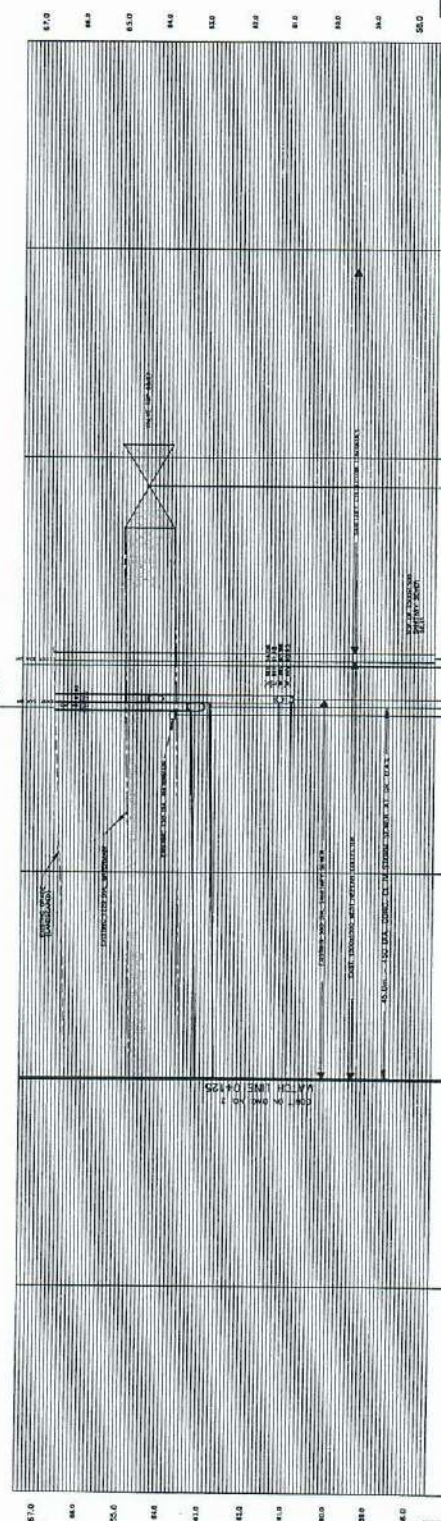
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NOTE: ALL EXISTING CATCH BASIN LEADS TO BE DISCONNECTED FROM SANITARY LINE, PLUGGED WITH CONCRETE AND RECONNECTED TO STORM SEWER

[illegible]

**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX D
CITY OF OTTAWA**

- **SITE PLAN AND ARCHITECTURAL DRAWINGS**
- **WATER BOUNDARY CONDITIONS E-MAIL**
- **FUS FIRE FLOW CALCULATIONS**
- **FUS EXPOSURE DISTANCES – FIGURE 1**
- **SUPPORTING HYDRAULIC CALCULATIONS**
- **HYDRANT SPACING – FIGURE 2**

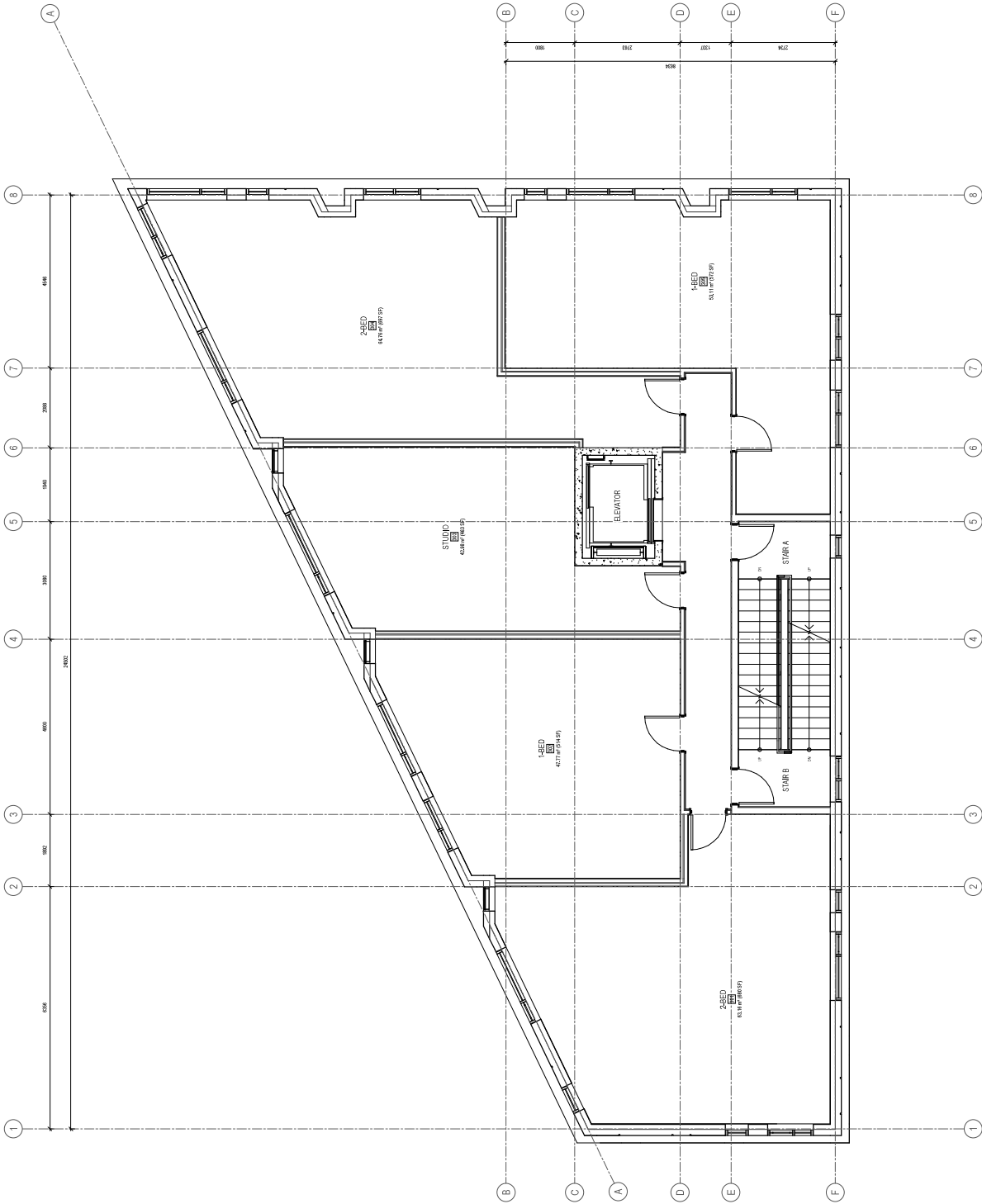
ATTACHMENT 1 : SITE PLAN AND ARCHITECTURAL DRAWINGS

GENERAL ARCHITECTURAL NOTES

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3. Dimensions shall be taken from the finished floor unless otherwise indicated.
4. The Architect reserves the right to make changes to this drawing at any time without notice.
5. This drawing is not to be used for construction of any part of the building without the written consent of Project Studio Inc.
6. This drawing is not to be used for construction of any part of the building without the written consent of Project Studio Inc.
7. This drawing is not to be used for construction of any part of the building without the written consent of Project Studio Inc.
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9. This drawing is not to be used for construction of any part of the building without the written consent of Project Studio Inc.
10. This drawing is not to be used for construction of any part of the building without the written consent of Project Studio Inc.

FLOORROOF PLAN NOTES

P1 ROOF ACCESS WAYIN



1. ISSUE FOR COORDINATION
2024/04/22
ISSUE RECORD



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342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ. 2405 SCALE NOTED DRAWN NL REVIEWED RMK

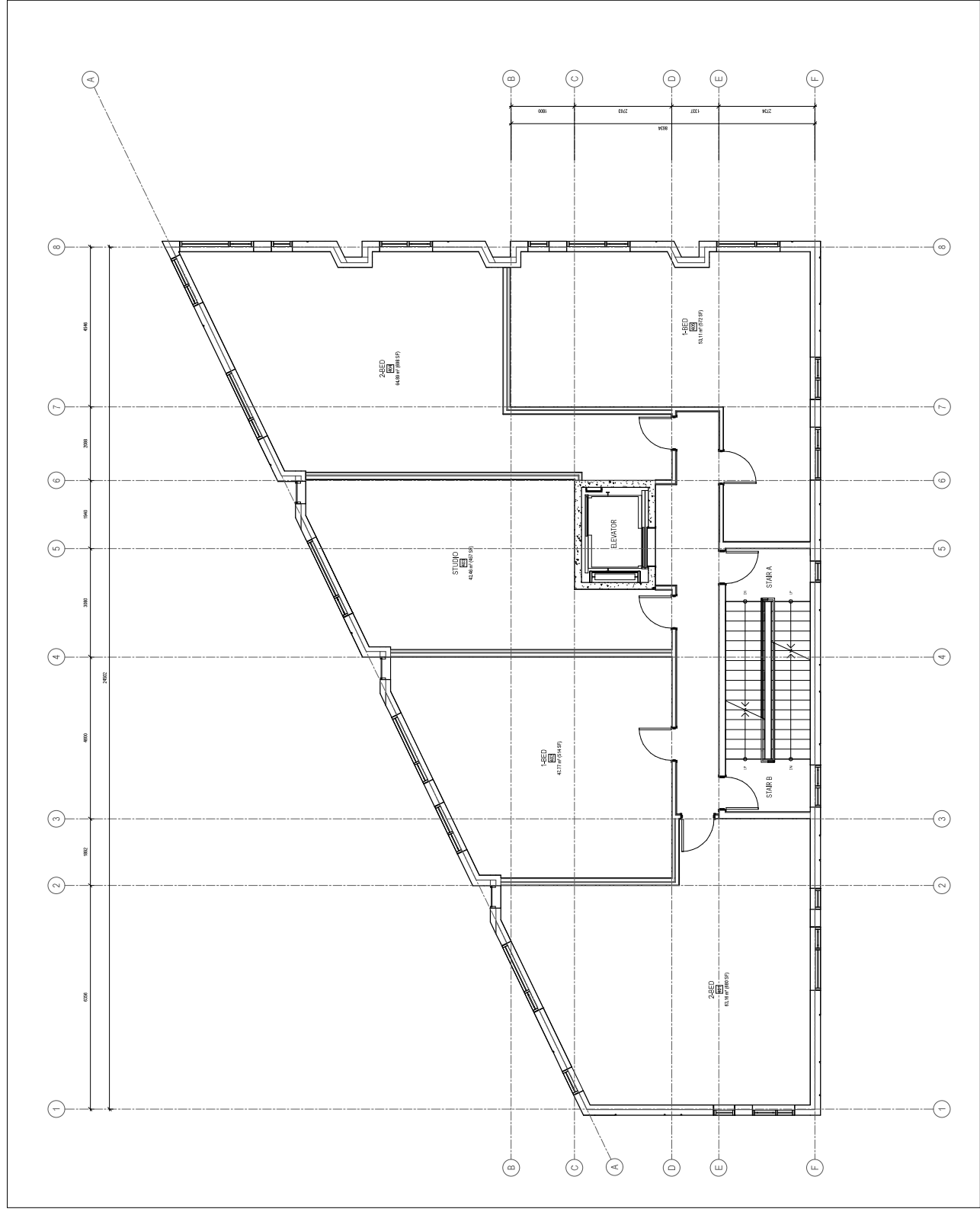
FLOOR PLAN LEVEL 03

A103

FLOORROOF PLAN NOTES

P1 ROOF ACCESS ONLY

- GENERAL AND SPECIAL NOTES
- This drawing is the property of Project Studio Inc. and shall not be reproduced or used for any other project without the written consent of Project Studio Inc.
 - Dimensions are given in feet and inches. Dimensions in parentheses are given in meters.
 - Room names and numbers are given in the schedule. Room names are given in the schedule. Room numbers are given in the schedule.
 - The building is a three-story building. The ground floor is the main floor. The second floor is the mezzanine. The third floor is the roof.
 - The building is a three-story building. The ground floor is the main floor. The second floor is the mezzanine. The third floor is the roof.
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1 FLOOR PLAN LEVEL 04
SCALE: 1" = 30'

1. ISSUED FOR COORDINATION
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342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ. 2405 SCALE NOTED DRAWN NL REVIEWED RMK

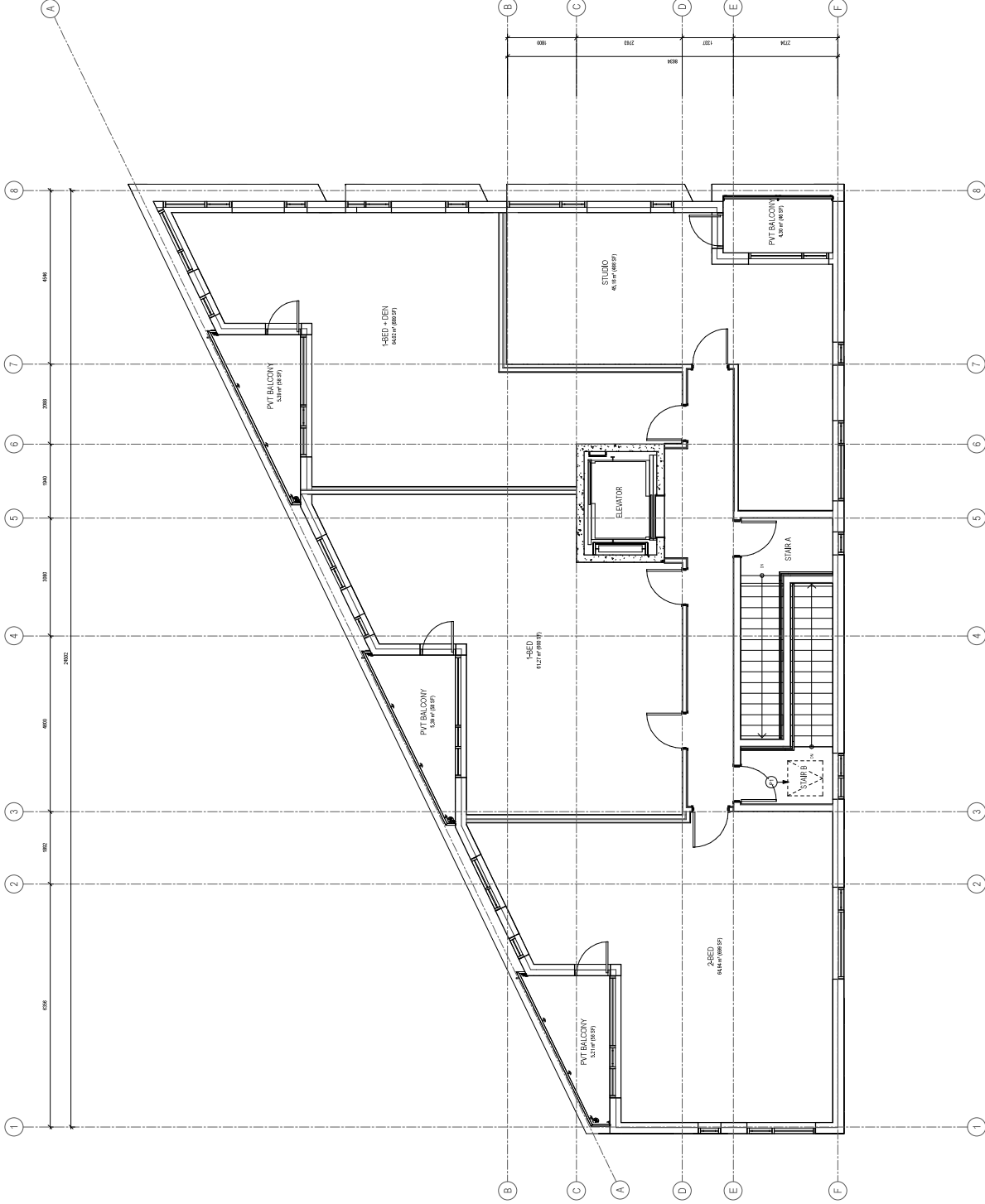
FLOOR PLAN LEVEL 04

A104

- GENERAL AND SPECIAL NOTES
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FLOORROOF PLAN NOTES

P1 ROOF ACCESS WAYIN



1 FLOOR PLAN LEVEL 06
SCALE: 1/8" = 1'-0"

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342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ. 2405 SCALE NOTED DRAWN NL REVIEWED RMK

FLOOR PLAN LEVEL 06

A106

FLOORROOF PLAN NOTES
P1 ROOF ACCESS WAYIN

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2. The drawings are prepared for the client and are not to be used for any other purpose without the written consent of Project Studio Inc.
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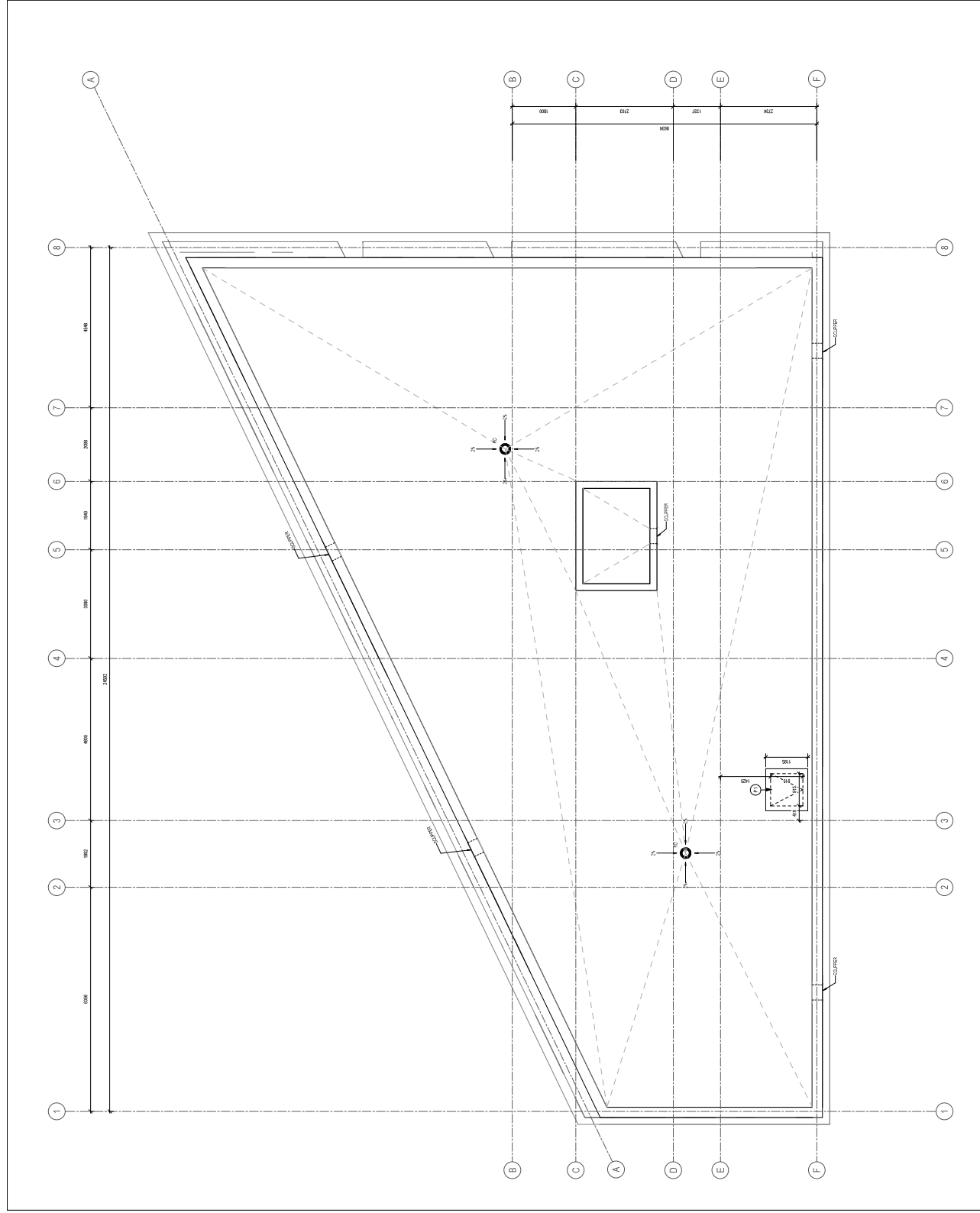
342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ SCALE NOTED NL
2405 DRAWN NL
2405 REVIEWED RMK

ROOF PLAN

A107

1 ROOF PLAN
SCALE: 1:50



1. ISSUE FOR COORDINATION
2024/04/22
ISSUE RECORD



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studio
Project Studio Incorporated
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342 ROOSEVELT
342 Roosevelt Avenue
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PROJ SCALE NOTED NL
2405 DRAWN NL
2405 REVIEWED RMK

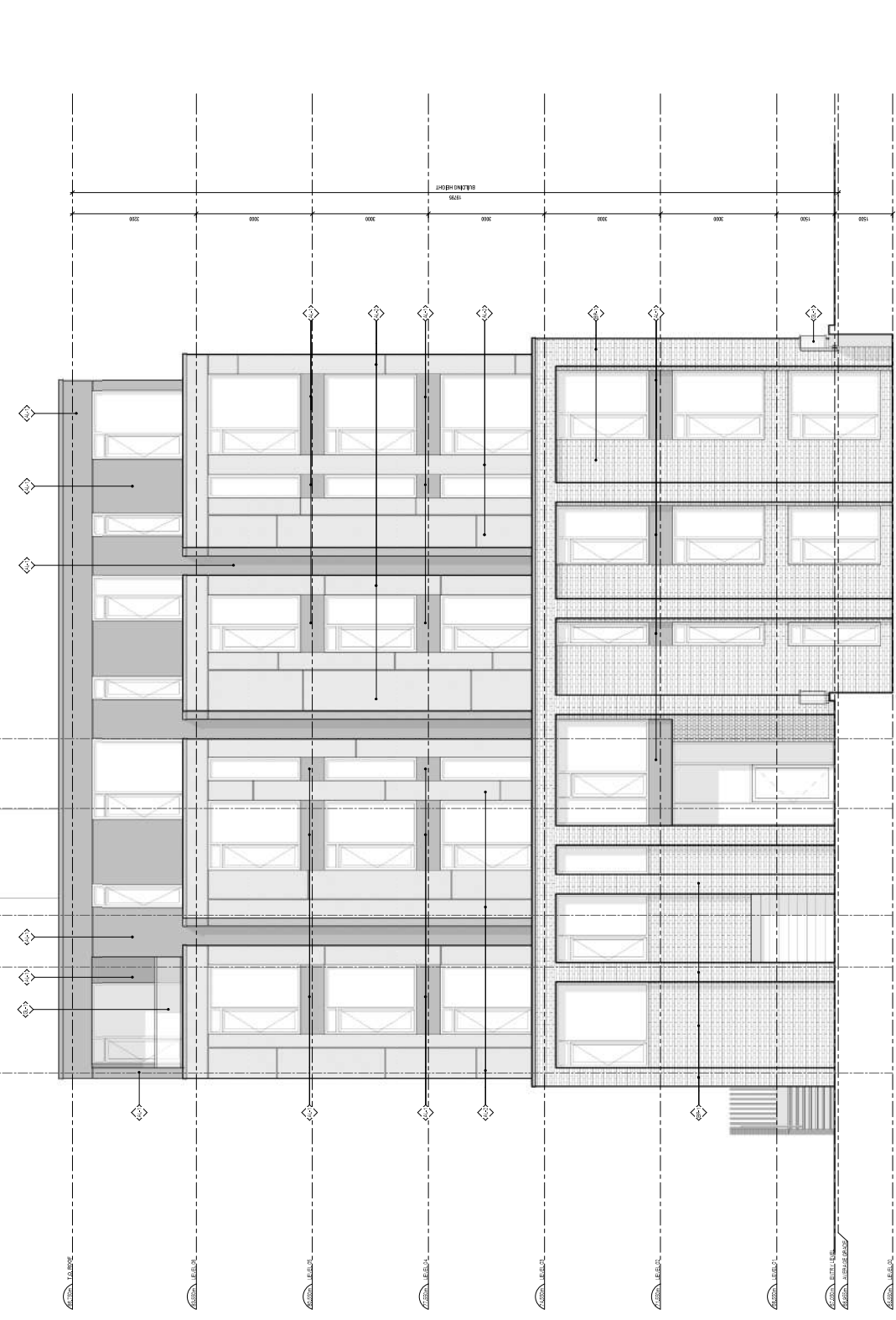
ROOF PLAN

- GENERAL CLADDING NOTES:
- The drawings show the proposed cladding system and the manufacturer's recommended details for the system. The contractor is responsible for ensuring that the cladding system is installed in accordance with the manufacturer's instructions and the applicable building codes.
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CLADDING LEGEND:

- ALUMINUM PANEL
- BRICK
- ALUMINUM PANEL
- CLAY TILE
- BRICK
- ALUMINUM PANEL
- CLAY TILE
- BRICK
- ALUMINUM PANEL
- CLAY TILE
- BRICK

CLADDING LEGEND
SCALE: 1:100



1 EAST ELEVATION
SCALE: 1:50

1. ISSUED FOR COORDINATION
2024-04-29
ISSUE RECORD

project studio
Project Studio Incorporated
[architectural representation]

342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ: 2405
SCALE: NOTED
NOTED: NL
DRAWN: RMK
REVIEWED: RMK
EAST ELEVATION

A201

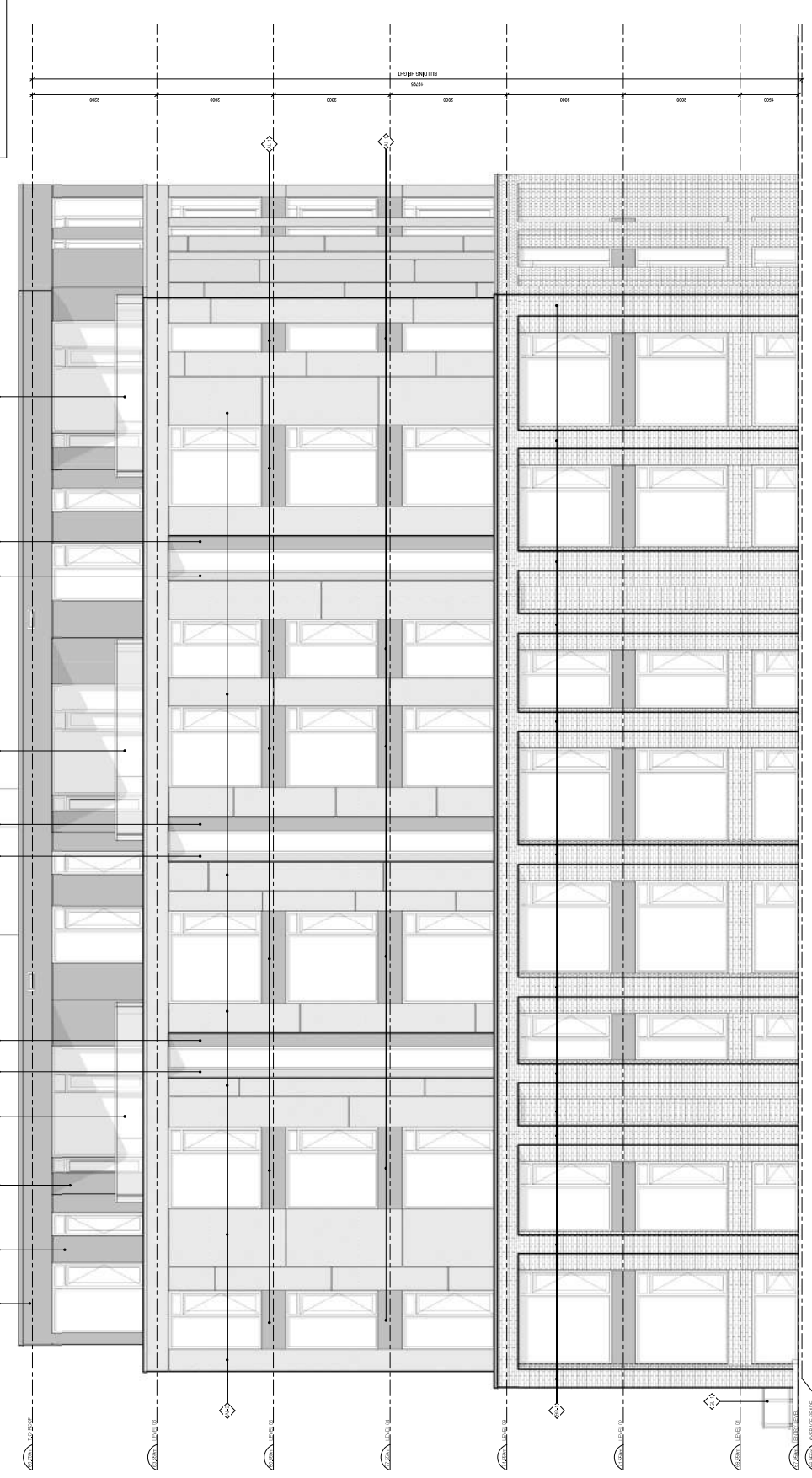
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2. Drawing is for informational purposes only. It is not to be used for construction or any other purpose without the written consent of Project Studio.
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10. The drawing is not to be used for any other purpose without the written consent of Project Studio.

CLADDING LEGEND:

- ALUMINUM PANEL
- BRICK
- ALUMINUM PANEL
- CLAY BRICK
- BRICK
- ALUMINUM PANEL
- CLAY BRICK
- BRICK

CLADDING LEGEND
SCALE: 1:100



1 NORTH ELEVATION
SCALE: 1:50

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[Project Studio Incorporated]

342 ROOSEVELT
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ: 2405 SCALE: NOTED DRAWN: NL REVISIONS: RMK

NORTH ELEVATION

A202

1. ISSUE FOR COORDINATION
2024-04-22
ISSUE RECORD

- GENERAL CLADDING NOTES:
- The drawings show the proposed cladding system and the manufacturer's standard details for the system. The contractor is responsible for ensuring that the cladding system is installed in accordance with the manufacturer's instructions and the applicable building codes.
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CLADDING LEGEND:

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- BRICK

CLADDING LEGEND

SCALE: 1/100



1. ISSUED FOR COORDINATION
2024-04-29

ISSUE RECORD

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Project Studio Incorporated
[architectural representation]

342 ROOSEVELT

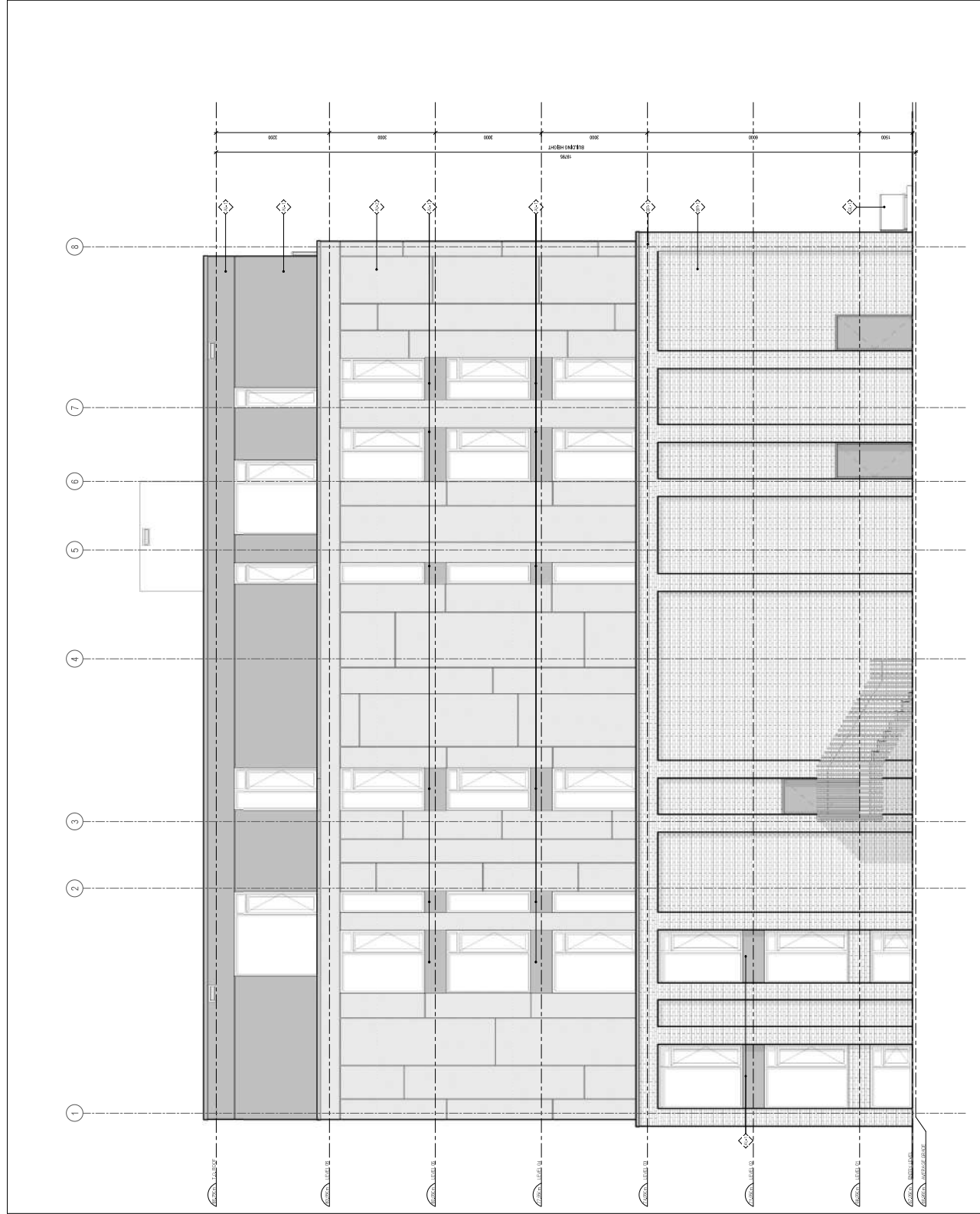
342 Roosevelt Avenue
Ottawa, ON K2A 1Z5

PROJ: 2405
SCALE: NOTED
NOTED: NL
DRAWN: RMK
REVISIONS:

SOUTH ELEVATION

A204

1 SOUTH ELEVATION
SCALE: 1/50



ATTACHMENT 2 : WATER BOUNDARY CONDITIONS E-MAIL

Mineault-Guitard, Alexandre

From: TL MaK <tlmakecl@bellnet.ca>
Sent: Monday, June 24, 2024 2:14 PM
To: Mineault-Guitard, Alexandre
Cc: Alemany, Kevin
Subject: RE: 342 Roosevelt Avenue - Water Boundary Conditions Request
Attachments: FUS_Calcs_342RooseveltAve_20240606.pdf; FUS_Exposure_Dist_342RooseveltAve_20240606.pdf; 342 Roosevelt Avenue June 2024.pdf

Hi Alex,

Attached please find the Water Boundary Conditions received from the City today for your calculation use.

Thank you,

Tony Mak

T.L. Mak Engineering Consultants Ltd.
1455 Youville Drive, Suite 218
Ottawa, ON. K1C 6Z7
Tel. 613-837-5516 | Fax: 613-837-5277
E-mail: tlmaked@bellnet.ca

From: Wessel, Shawn [mailto:shawn.wessel@ottawa.ca]
Sent: June 24, 2024 12:30 PM
To: tlmaked@bellnet.ca
Cc: Duquette, Vincent
Subject: 342 Roosevelt Avenue - Water Boundary Conditions Request

Good afternoon Tony.

Please find BC for this site, attached and below:

The following are boundary conditions, HGL, for hydraulic analysis at 342 Roosevelt Avenue (zone 1W) assumed to be connected to the 152mm watermain on Roosevelt Avenue (see attached PDF for location).

Minimum HGL: 108.7 m

Maximum HGL: 115.3 m

Available Fire Flow at 20 (psi): 133.5 L/s, assuming ground elevation of 66.9 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the

time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

If you require additional information or clarification, please do not hesitate to contact me anytime.

Thank you

Regards,

Shawn Wessel, A.Sc.T.,rcji

Pronouns: he/him | Pronom: il

Project Manager - Infrastructure Approvals

Gestionnaire de projet – Approbation des demandes d'infrastructures

Development Review Central Branch | Direction de l'examen des projets d'aménagement, Centrale
Planning, Development & Building Services Department (PDBS) | Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB)

City of Ottawa | Ville d'Ottawa

110 Laurier Ave. W. | 110, avenue Laurier Ouest, Ottawa ON K1P 1J1

(613) 580 2424 Ext. | Poste 33017

Int. Mail Code | Code de Courrier Interne 01-14

shawn.wessel@ottawa.ca



Please consider the environment before printing this email

*****Please also note that, while my work hours may be affected by the current situation and am working from home, I still have access to email, video conferencing and telephone. Feel free to schedule video conferences and/or telephone calls, as necessary.*****

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Caution: This email originated from outside of Stantec. Please take extra precaution.

Attention: Ce courriel provient de l'extérieur de Stantec. Veuillez prendre des précautions supplémentaires.

Boundary Conditions for 342 Roosevelt Avenue



City of Ottawa

av. Roosevelt Ave.

152mm

305mm

1220mm

342

346

Legend

Private

Public

ATTACHMENT 3 : FUS FIRE FLOW CALCULATIONS



FUS Fire Flow Calculation - Long Method

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 2020

Stantec Project #: 163401084

Project Name: 342 Roosevelt Avenue

Date: March 12, 2025

Data inputted by: Hamidreza Mohabbat, MASc.

Data reviewed by: Alexandre Mineault-Guitard Ing., P.Eng

Fire Flow Calculation #: 1

Building Type/Description/Name: Residential

8-storey multiunit building, ordinary construction, basement is more than 50% above ground;

Notes: Total gross floor area of 2,163 sqm. Equipped with a sprinkler system.

Building setbacks per site plan, Separation distances to adjacent building per GeoOttawa.

Fire Underwriters Survey Determination of Required Fire Flow - Long Method								
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material					
			Type V - Wood Frame	1.5	Type III - Ordinary construction	1	m	
			Type IV-A - Mass Timber	0.8				
			Type IV-B - Mass Timber	0.9				
			Type IV-C - Mass Timber	1				
			Type IV-D - Mass Timber	1.5				
			Type III - Ordinary construction	1				
			Type II - Non-combustible construction	0.8				
Type I - Fire resistive construction	0.6							
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area					
			Single Family	0	Other (Comm, Ind, Apt etc.)	30	Units	
			Townhouse - indicate # of units	0				
			Other (Comm, Ind, Apt etc.)	30				
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):		7	7	Storeys		
3	Enter Ground Floor Area of One Unit	Average Floor Area (A) based on total floor area of all floors for one unit (non-fire resistive construction):		310	310	Area in Square Metres (m ²)		
Square Metres (m ²)								
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x # of Units (if single family or townhouse) x Average Floor Area):		2,168	2168			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) ($F = 220 \cdot C \cdot \sqrt{A}$) Round to nearest 1,000 L/min						10,000
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning						
5.1	Choose Combustibility of Building Contents	Occupancy Content Hazard Reduction or Surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	8,500
			Limited combustible	-0.15				
			Combustible	0				
			Free burning	0.15				
			Rapid burning	0.25				
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler Reduction	Adequate Sprinkler conforms to NFPA13	-0.3	Adequate Sprinkler conforms to NFPA13	-0.3	N/A	-2,550
		None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is standard for sprinkler and fire dept. hose line	-0.1	N/A	-850
		Water supply is not standard or N/A	0					
		Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler system is fully supervised	-0.1	N/A	-850
Sprinkler not fully supervised or N/A	0							
5.3	Choose Presence of Sprinklers for Exposures within 30m	Sprinkler Conforms to NFPA13	Adequate sprinkler for exposures conforms to NFPA13		None for exposures	0	N/A	0
			None for exposures					
		Water Supply	Water supply is standard for sprinkler and fire dept. hose line of exposures		Water supply is not standard or N/A for exposures			
			Water supply is not standard or N/A for exposures					
		Sprinkler Supervision	Sprinkler system of exposures is fully supervised		Sprinkler not fully supervised or N/A for exposures			
Sprinkler not fully supervised or N/A for exposures								
5.4	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	20.1 to 30.1m	0.1	0.4	m	3,400
			Right Side	10.1 to 20.0m	0.15			
			Rear Yard	10.1 to 20.0m	0.15			
			Left Side	30.1m or greater	0			
6	Obtain Required Fire Flow, Duration & Volume	Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:						8,000
		Total Required Fire Flow (above) in L/s:						133
		Required Duration of Fire Flow (hrs)						2.00
		Required Volume of Fire Flow (m ³)						960

Fire Flow Calculations as per the Ontario Building Code (OBC)



OFM Fire Flow Calculation

Calculations based on *Fire Protection Water Supply Guideline for Part 3 in the Ontario Building Code* by the Office of the Fire Marshal (OFM 1999)

Stantec Project #: 163401084

Project Name: 342 Roosevelt Avenue

Date: March 12, 2025

Data inputted by: Alexandre Mineault-Guitard Ing., P.Eng

Data reviewed by: Alexandre Mineault-Guitard Ing., P.Eng

Fire Flow Calculation #: 2

Building Type/Description/Name: Residential

Office of the Fire Marshal Determination of Required Fire Protection Water Supply							
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit
1	General Building Details						
1.1	Enter Number of Storeys		Number of Floors/Storeys in the Unit (incl. basement):		7	7	Storeys
1.2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	<div>Single Family</div> <div>Townhouse - indicate # of units</div> <div>Other (Comm, Ind, Apt etc.)</div>	<div>0</div> <div>0</div> <div>30</div>	Other (Comm, Ind, Apt etc.)	30	Units
1.3	Choose Presence of Sprinklers		Sprinklers?		Yes	Yes	N/A
1.4	Choose Presence of Firewalls		Firewall separations?		None	None	N/A
1.5	Choose Presence of Stand-Pipe System		Stand-pipe system?		None	None	N/A
2	Determining Water Supply Coefficient K						
2.1	Choose Type of Construction	Type of Construction	<div>Non-combustible construction + fire separations + fire resistance ratings in accordance with Section 3.2.2 of OBC</div> <div>Non-combustible construction + fire separations + no fire-resistance rating</div> <div>Combustible construction + fire separations + fire-resistance ratings in accordance with Section 3.2.2 of OBC</div> <div>Combustible construction + fire separations + no fire-resistance rating</div>	<div>Type I</div> <div>Type II</div> <div>Type III</div> <div>Type IV</div>	Type IV	N/A	N/A
2.2	Choose Classification	Occupancy Classification (OBC)	<div>A-2, B-1, B-2, B-3, C, D</div> <div>A-4, F-3</div> <div>A-1, A-3</div> <div>E, F-2</div> <div>F-1</div>	<div>23</div> <div>28</div> <div>32</div> <div>39</div> <div>53</div>	C	A-2, B-1, B-2, B-3, C, D	N/A
2.3	Water Supply Coefficient (K)		Water Supply Coefficient K			23	N/A
3	Determining Building Volume V						
3.1	Enter Ground Floor Area of One Unit		Floor Space Area				
			Average Floor Area (A) :		310	310	Area in Square Meters (m ²)
3.2	Building Height (h)		Building Height				
			Bottom Elevation :		0.0		
					Meters (m)	21.2	Height in Meters (m)
			Top Elevation :		21.2		
					Meters (m)		
3.3	Building Volume (V)		Building Volume V = A * h			6,570	Volume in Meters Cube (m ³)
4	Determining Spatial Coefficient S						
4.1	Choose Exposure Distances from Building to Property Line	Exposure Distance from Building to Property Line in Meters (m)	<div>North Side</div> <div>Property Line to Street Centreline (Street Facing)</div> <div>Total Exposure Distance</div> <div>East Side</div> <div>Property Line to Street Centreline (Street Facing)</div> <div>Total Exposure Distance</div> <div>South Side</div> <div>Property Line to Street Centreline (Street Facing)</div> <div>Total Exposure Distance</div> <div>West Side</div> <div>Property Line to Street Centreline (Street Facing)</div> <div>Total Exposure Distance</div>	<div>5.1</div> <div>0</div> <div>5.1</div> <div>3.6</div> <div>10.9</div> <div>14.5</div> <div>1.5</div> <div>0</div> <div>1.5</div> <div>1.2</div> <div>0</div> <div>1.2</div>	<div>0.49</div> <div></div> <div></div> <div>0.00</div> <div></div> <div></div> <div>0.50</div> <div></div> <div></div> <div>0.50</div> <div></div> <div></div>	1.49	Distance in Meters (m)
4.2	Total Spatial Coefficient		Total Spatial Coefficient $S_{tot} = 1 + \sum S_x$			2.00	N/A
5	Determining Required Minimum Supply of Water Q and Fire Flow						
5.1	Obtain Required Fire Volume, Flow & Duration		Minimum Supply of Water, rounded to nearest 1,000 L; $Q = K \cdot V \cdot S_{tot}$			302,000 L	
			Required Minimum Water Supply Flow Rate (L/min)			9,000 L/min	
			Required Minimum Water Supply Flow Rate (L/s)			150 L/s	
			Required Minimum Duration of Fire Flow (min)			40 min	

ATTACHMENT 4 : FIGURE 1 – FUS EXPOSURE DISTANCES



Figure 1: OBC and FUS Exposure Distances (Property Line to Adjacent Buildings)

Source: geoOttawa 2024; Contains information licensed under the Open Government License – City of Ottawa.

ATTACHMENT 5 : SUPPORTING HYDRAULIC CALCULATIONS



Supporting Hydraulic Calculations

Stantec Project #: 163401084

Project Name: 342 Roosevelt Avenue

Date: March 12, 2025

Data inputted by: Hamidreza Mohabbat MASc.

Data reviewed by: Alexandre Mineault-Guitard ing., P.Eng

Boundary Conditions provided by the City:

Scenario 1: Peak Hour (Min HGL): 108.7 m;

Scenario 2: Average Day (Max HGL): 115.3 m; and

Scenario 3: Maximum Day plus Fire Flow: 81.2 m.

Sample Calculations

$$HGL(m) = hp + hz \quad (1)$$

where: hp = Pressure Head (m); and hz = Elevation Head (m), estimated from topography.

For Scenario 1, we have:

$$HGL(m) = 108.7 \text{ and } hz(m) = 66.9.$$

Rearranging Equation 1, we can calculate the Pressure Head (hp) as follow:

$$hp(m) = HGL - hz$$

$$\therefore hp = 108.7 - 66.9 \text{ m} = 41.8 \text{ m}.$$

To convert from Pressure Head (m) to a pressure value (kPa), the following equation can be used:

$$P(kPa) = (\rho * g * hp) / 1000 \quad (2)$$

where: ρ = density of water = 1000 kg/m³; and g = gravitational acceleration = 9.81 m/s².

Using Equation 2, we can calculate the Pressure Head (hp) as follow:

$$P(kPa) = (1000 * 9.81 * 41.8) / 1000$$

$$\therefore P = 410 \text{ kPa}.$$

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

$$P = 59 \text{ psi}.$$

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows:

Scenario 2: $P = 69$ psi; and Scenario 3: $P = 20$ psi.

To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 410 kPa (59 psi)
Scenario 2: Maximum Pressure under Average Day Demand: 474 kPa (69 psi)
Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 140 kPa (20 psi)

ATTACHMENT 6 : FIGURE 2 – HYDRANT SPACING

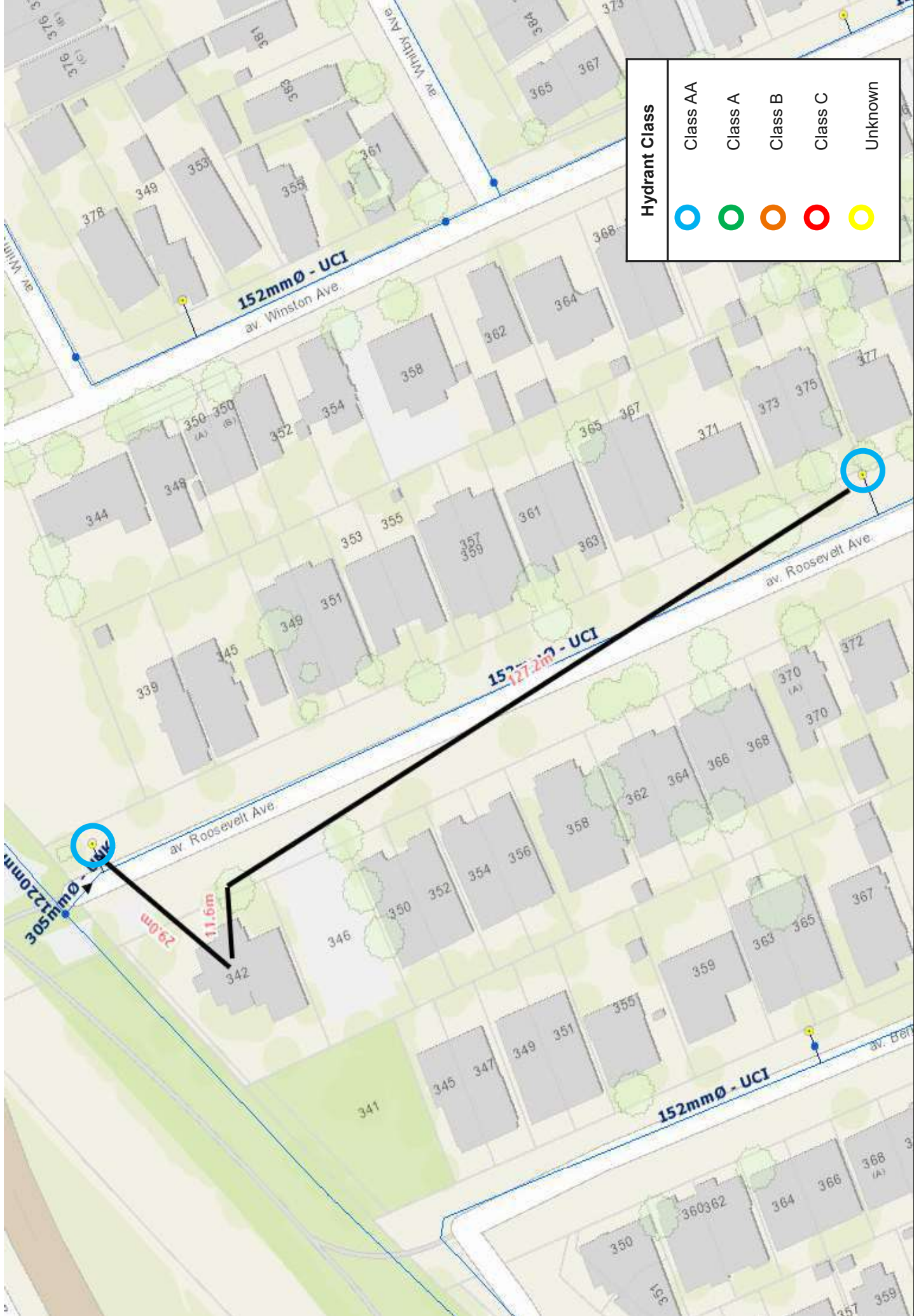


Figure 2: FUS Exposure Distances & Hydrant Spacing

Source: geoOttawa 2021; Contains information licensed under the Open Government Licence – City of Ottawa.

**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX E
CITY OF OTTAWA
SANITARY SEWER DESIGN SHEET
SHEET No. 1 OF 1**

- q = average daily per capita flow (200 l/cap. d)
- l = unit of peak extraneous flow (0.25 L/ha. s)
- M = peaking factor (4 MAX)
- Q (p) = peak population flow (L/s)
- Q (f) = peak extraneous flow (L/s)
- Q (d) = peak design flow

$M = 1 + \frac{14}{4} \sqrt{P}^K$ where P = population in 1000's
 $K = 0.8$
 $Q(P) = \frac{P \cdot M}{86.4} \text{ (L/s)}$
 $Q(I) = IA \text{ (L/s)}$ where A = area in hectares
 $Q(d) = Q(p) + Q(I) \text{ (L/s)}$

- DENSITY = 2.1 ppm
- 1 BEDROOM = 1.4 ppm
- STUDIO = 1.4 ppm

LOCATION		INDIVIDUAL		CUMULATIVE		PROPOSED SEWER											
STREET	FROM	TO	Pop.	Area A (hectares)	Pop.	Area A (hectares)	Peaking factor M	Pop. flow Q(p) (l./s)	Peak extraneous flow Q(i) (l./s)	Peak design flow Q(d) (l./s)	Length (m)	Pipe size (mm)	Type of pipe	Grade %	Capacity (l./s) n = 0.013	Full flow velocity (m/s)	Actual velocity at Q(d)
342 ROOSEVELT AVENUE	SITE	EX-3750 SANITARY SEWER	51.1	0.07	51.1	0.07	3.45	0.57	0.02	0.59	15.0	150	PVC	1.0% (min)	19.8	1.12	

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal contains the text "LICENSED PROFESSIONAL ENGINEER" around the top inner edge and "PROVINCE OF ONTARIO" around the bottom inner edge. In the center, it says "TOM I. M. MAK" and "40130271". There is a signature over the text.

(FILE# 824-83)

DESIGN TLM
CHECKED TLM
DATE AUGUST 2

PROJECT 342 ROOSEVELT AVENUE
PROPOSED SIX (6) STOREY APARTMENT
BUILDING SITE - CITY OF OTTAWA

SHEET No.

101



**PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 36
R-PLAN 114
342 ROOSEVELT AVENUE
CITY OF OTTAWA**

**APPENDIX F
DEVELOPMENT SERVICING STUDY CHECKLIST SUMMARY**

Servicing study guidelines for development applications

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

- ☐ Executive Summary (for larger reports only).
- ☒ Date and revision number of the report.
- ☒ Location map and plan showing municipal address, boundary, and layout of proposed development.
- ☒ Plan showing the site and location of all existing services.
- ☐ Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
- ☐ Summary of Pre-consultation Meetings with City and other approval agencies.
- ☐ Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.
- ☒ Statement of objectives and servicing criteria.
- ☒ Identification of existing and proposed infrastructure available in the immediate area.
- ☐ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).
- ☐ Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
- ☐ Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
- ☐ Proposed phasing of the development, if applicable.

- ☒ Reference to geotechnical studies and recommendations concerning servicing.
- ☒ All preliminary and formal site plan submissions should have the following information:
 - Metric scale
 - North arrow (including construction North)
 - Key plan
 - Name and contact information of applicant and property owner
 - Property limits including bearings and dimensions
 - Existing and proposed structures and parking areas
 - Easements, road widening and rights-of-way
 - Adjacent street names

4.2 Development Servicing Report: Water

- ☐ Confirm consistency with Master Servicing Study, if available
- ☒ Availability of public infrastructure to service proposed development
- ☒ Identification of system constraints
- ☒ Identify boundary conditions
- ☒ Confirmation of adequate domestic supply and pressure
- ☒ Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
- ☒ Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
- ☐ Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
- ☐ Address reliability requirements such as appropriate location of shut-off valves
- ☒ Check on the necessity of a pressure zone boundary modification.
- ☒ Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range

- ☒ Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
- ☐ Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
- ☒ Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
- ☒ Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.

4.3 Development Servicing Report: Wastewater

- ☒ Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
- ☐ Confirm consistency with Master Servicing Study and/or justifications for deviations.
- ☐ Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
- ☒ Description of existing sanitary sewer available for discharge of wastewater from proposed development.
- ☐ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
- ☒ Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
- ☐ Description of proposed sewer network including sewers, pumping stations, and forcemains.
- ☐ Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).
- ☐ Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
- ☐ Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
- ☐ Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
- ☐ Special considerations such as contamination, corrosive environment etc.

4.4 Development Servicing Report: Stormwater Checklist

- ☒ Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)
- ☐ Analysis of available capacity in existing public infrastructure.
- ☒ A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
- ☒ Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
- ☐ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
- ☒ Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
- ☐ Set-back from private sewage disposal systems.
- ☐ Watercourse and hazard lands setbacks.
- ☐ Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
- ☐ Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.
- ☒ Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
- ☐ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
- ☒ Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
- ☐ Any proposed diversion of drainage catchment areas from one outlet to another.
- ☒ Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
- ☐ If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.
- ☐ Identification of potential impacts to receiving watercourses
- ☐ Identification of municipal drains and related approval requirements.
- ☒ Descriptions of how the conveyance and storage capacity will be achieved for the development.
- ☒ 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.

- ☐ Inclusion of hydraulic analysis including hydraulic grade line elevations.
- ☒ Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
- ☐ Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
- ☐ Identification of fill constraints related to floodplain and geotechnical investigation.

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

- ☐ Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.
- ☐ Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
- ☐ Changes to Municipal Drains.
- ☐ Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)

4.6 Conclusion Checklist

- ☒ Clearly stated conclusions and recommendations
- ☐ Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
- ☒ All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario