

**ASSESSMENT OF ADEQUACY OF
PUBLIC SERVICES**

FOR

**100 NEW ORCHARD AVENUE NORTH
NEW ORCHARD INVESTMENT INC.**

CITY OF OTTAWA

PROJECT NO.: 18-1031

JUNE 2018 – REV 1
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FOR
100 NEW ORCHARD AVENUE NORTH
NEW ORCHARD INVESTMENT INC.**

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1.0 INTRODUCTION

David Schaeffer Engineering Limited (DSEL) has been retained by New Orchard Investment Inc. to prepare an Assessment of Adequacy of Public Services report in support of the application for a Zoning By-law Amendment (ZBLA) at 100 New Orchard Avenue North.

The subject property is located within the City of Ottawa urban boundary, in the Bay ward. As illustrated in **Figure 1**, the subject property is located North of the Richmond Road and New Orchard Avenue intersection. Comprised of a single parcel of land, the subject property measures approximately **0.07 ha** and is zoned Residential Fifth Density Zone (R5C).



Figure 1: Site Location

The proposed ZBLA would allow for the development of a 14-storey residential building fronting New Orchard Avenue North. The contemplated development would include approximately **358 m²** of amenity space, as well as underground parking with access from New Orchard Avenue North. The residential component is comprised of **84 single units**. A copy of the conceptual site plan is included in **Drawings/Figures**.

The objective of this report is to provide sufficient detail to demonstrate that the proposed re-zoning and contemplated development is supported by existing municipal services.

1.1 Existing Conditions

The existing site includes a residential property consisting of a detached home with vegetated areas. The elevations range between 62.7m and 61.5m with a grade change of approximately 1.2m from the Northeast to the Southwest corner of the property.

To the north of the site there is a 1220mm dia. Feedermain, as well as, the 1350mm dia. sanitary West Nepean Collector Trunk.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal right-of-ways:

New Orchard Avenue:

- 152mm diameter cast iron watermain
- 300mm diameter concrete sanitary sewer tributary to the West Nepean Collector
- 675mm diameter concrete storm sewer tributary to the Ottawa River which is located approximately 160m downstream

North of 100 New Orchard Avenue:

- 1220 mm diameter concrete feedermain watermain
- 1350 mm diameter concrete sanitary West Nepean Collector sewer

1.2 Required Permits / Approvals

The proposed development is subject to the site plan control approval process. The City of Ottawa must approve the engineering design drawings and reports prior to the issuance of site plan control.

1.3 Pre-consultation

Pre-consultation correspondence, along with the servicing guidelines checklist, is located in **Appendix A**.

2.0 GUIDELINES, PREVIOUS STUDIES, AND REPORTS

2.1 Existing Studies, Guidelines, and Reports

The following studies were utilized in the preparation of this report:

- **Ottawa Sewer Design Guidelines,**
City of Ottawa, *SDG002*, October 2012.
(City Standards)
- **Ottawa Design Guidelines – Water Distribution**
City of Ottawa, July 2010.
(Water Supply Guidelines)
 - **Technical Bulletin ISD-2010-2**
City of Ottawa, December 15, 2010.
(ISD-2010-2)
 - **Technical Bulletin ISDTB-2014-02**
City of Ottawa, May 27, 2014.
(ISDTB-2014-02)
- **Design Guidelines for Sewage Works,**
Ministry of the Environment, 2008.
(MOE Design Guidelines)
- **Stormwater Planning and Design Manual,**
Ministry of the Environment, March 2003.
(SWMP Design Manual)
- **Ontario Building Code Compendium**
Ministry of Municipal Affairs and Housing Building Development Branch,
January 1, 2010 Update.
(OBC)
- **Water Supply for Public Fire Protection**
Fire Underwriters Survey, 1999.
(FUS)

3.0 WATER SUPPLY SERVICING

3.1 Existing Water Supply Services

The subject property lies within the City of Ottawa 1W pressure zone, as shown by the Pressure Zone map in **Appendix B**. A local 152mm diameter watermain exists within the New Orchard Avenue right-of-way. In addition to the local service, a 1220mm diameter feedermain exists within the City of Ottawa right-of-way north of the site.

3.2 Water Supply Servicing Design

It is anticipated that the contemplated development would be serviced from the existing 152mm watermain within the New Orchard Avenue right-of-way.

Table 1 summarizes the **Water Supply Guidelines** employed in the preparation of the preliminary water demand estimate.

Table 1
Water Supply Design Criteria

| Design Parameter | Value |
|--|--|
| Residential 1 Bedroom Apartment | 1.4 P/unit |
| Residential Average Daily Demand | 280 L/d/P |
| Residential Maximum Daily Demand | 3.6 x Average Daily * |
| Residential Maximum Hourly | 5.4 x Average Daily * |
| Amenity Space | 2.5 L/m ² /d |
| Commercial Maximum Daily Demand | 1.5 x avg. day |
| Commercial Maximum Hour Demand | 1.8 x max. day |
| Minimum Watermain Size | 150mm diameter |
| Minimum Depth of Cover | 2.4m from top of watermain to finished grade |
| During normal operating conditions desired operating pressure is within | 350kPa and 480kPa |
| During normal operating conditions pressure must not drop below | 275kPa |
| During normal operating conditions pressure must not exceed | 552kPa |
| During fire flow operating pressure must not drop below | 140kPa |
| <small>*Daily average based on Appendix 4-A from Water Supply Guidelines ** Residential Max. Daily and Max. Hourly peaking factors per MOE Guidelines for Drinking-Water Systems Table 3-3 for 0 to 500 persons. -Table updated to reflect ISD-2010-2</small> | |

Table 2 summarizes the anticipated water supply demand for the contemplated development based on the **Water Supply Guidelines**.

Table 2
Summary of Anticipated Water Demand

| Design Parameter | Anticipated Demand ¹ (L/min) |
|----------------------|--|
| Average Daily Demand | 23.6 |
| Max Day + Fire Flow | 113.4 + 13,000 = 13,114.2 |
| Peak Hour | 171.5 |

1) Water demand calculation per *Water Supply Guidelines*. See *Appendix B* for detailed calculations.

Fire flow requirements are to be determined in accordance with Local Guidelines (*FUS*), City of Ottawa *Water Supply Guidelines* and the Ontario Building Code.

Using the *FUS* method a conservative estimation of fire flow had been established. The following assumptions were assumed:

- Type of construction - Ordinary Construction;
- Occupancy type – Limited Combustibility;
- Sprinkler Protection – Supervised Sprinkler System.

The above assumptions result in an estimated fire flow of approximately **13,000 L/min**, noting that actual building materials selected will affect the estimated flow. A certified fire protection system specialist would need to be employed to design the building fire suppression system and confirm the actual fire flow demand.

The City of Ottawa was contacted to obtain boundary conditions associated with the estimated water demand as indicated in *Table 2*. No response was received at the time of publication. Correspondence with the City has been included in *Appendix A*.

3.3 Water Supply Conclusion

The anticipated water demand under proposed conditions was submitted to the City of Ottawa for establishing boundary conditions. No response was received at the time of publication.

The proposed water supply design conforms to all relevant City Guidelines and Policies.

4.0 WASTEWATER SERVICING

4.1 Existing Wastewater Services

The subject site lies within the West Nepean Collector Sewer catchment area, as shown by the City sewer mapping included in **Appendix C**. An existing 300mm diameter sanitary sewer within New Orchard Avenue North is available to service the contemplated development.

The existing site consists of a single detached residential home which is contributing wastewater to the local New Orchard sewer system. The New Orchard Avenue sanitary sewer is tributary to the West Nepean Trunk Collector sewer, which is located approximately 10m downstream of the site. **Table 3** below summarizes the estimated existing wastewater flow from the site.

Table 3
Summary of Existing Peak Wastewater Flow

| Design Parameter | Total Flow (L/s) |
|------------------------------------|------------------|
| Estimated Average Dry Weather Flow | 0.01 |
| Estimated Peak Dry Weather Flow | 0.05 |
| Estimated Peak Wet Weather Flow | 0.07 |

4.2 Wastewater Design

It is anticipated that the contemplated development will connect to the existing 300mm sanitary sewer within New Orchard Avenue North.

Table 4 summarizes the **City Standards** employed in the design of the proposed wastewater sewer system.

Table 4
Wastewater Design Criteria

| Design Parameter | Value |
|---|---|
| Residential Average Apartment | 1.8 P/unit |
| Average Daily Demand | 280 L/d/per |
| Peaking Factor | Harmon's Peaking Factor. Max 4.0, Min 2.0 |
| Amenity Space | 5 L/m ² /d |
| Infiltration and Inflow Allowance | 0.28L/s/ha |
| Sanitary sewers are to be sized employing the Manning's Equation | $Q = \frac{1}{n} AR^{2/3} S^{1/2}$ |
| Minimum Sewer Size | 135mm diameter |
| Minimum Manning's 'n' | 0.013 |
| Minimum Depth of Cover | 2.5m from crown of sewer to grade |
| Minimum Full Flowing Velocity | 0.6m/s |
| Maximum Full Flowing Velocity | 3.0m/s |
| <i>Extracted from Sections 4 and 6 of the City of Ottawa Sewer Design Guidelines, October 2012.</i> | |

Table 5 demonstrates the anticipated peak flow from the contemplated development. See **Appendix C** for associated calculations.

Table 5
Summary of Estimated Peak Wastewater Flow

| Design Parameter | Total Flow (L/s) |
|------------------------------------|------------------|
| Estimated Average Dry Weather Flow | 0.42 |
| Estimated Peak Dry Weather Flow | 1.59 |
| Estimated Peak Wet Weather Flow | 1.61 |

The estimated sanitary flow, based on the concept plan provide in **Drawings/Figures**, is **1.67 L/s**. As demonstrated by **Table 3** and **Table 5**, the development results in an increase in peak wet weather sanitary flow of approximately **1.54 L/s**. As discussed with City staff, due to the distance to the West Nepean Trunk Collector sewer and the complexity of the drainage area, the anticipated flow from the site was provided to confirm capacity and resulting HGL due to the proposed development.

4.3 Wastewater Servicing Conclusions

The site is tributary to the West Nepean Trunk Collector sewer. Based on the distance to the Collector sewer and complexity of the drainage area, the anticipated flow of **1.61 L/s** peak wet weather flow from the site was provided to confirm capacity and resulting HGL due to the proposed development.

The proposed wastewater design conforms to all relevant **City Standards**.

5.0 STORMWATER MANAGEMENT

5.1 Existing Stormwater Services

Stormwater runoff from the subject property is tributary to the City of Ottawa sewer system and is located within the Ottawa Central sub-watershed. As such, approvals for proposed development within this area are under the approval authority of the City of Ottawa.

Flows that influence the watershed in which the subject property is located are further reviewed by the principal authority. The subject property is located within the Ottawa River watershed, and is therefore subject to review by the Rideau Valley Conservation Authority (RVCA). Consultation with the RVCA is located in **Appendix A**.

It was determined that the existing development contained no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 2, 5, and 100-year events are summarized in **Table 6**:

Table 6
Summary of Existing Peak Storm Flow Rates

| City of Ottawa Design Storm | Estimated Peak Flow Rate (L/s) |
|-----------------------------|--------------------------------|
| 2-year | 11.0 |
| 5-year | 14.9 |
| 100-year | 32.1 |

5.2 Post-development Stormwater Management Target

Stormwater management requirements for the proposed development were reviewed with the City of Ottawa, where the proposed development is required to:

- Meet an allowable release rate based on the lesser of either the existing calculated Rational Method Coefficient or 0.50, employing the City of Ottawa IDF parameters for a 2-year storm with a time of concentration equal to or greater than 10 minutes;
- Attenuate all storms up to and including the City of Ottawa 100-year design event on site;
- Quality controls are anticipated to be required for the proposed development due to the site's distance from the outlet; correspondence with the RVCA is included in **Appendix A**, however no response was received at the time of publication.

Based on the above the allowable release rate for the proposed development is **7.7 L/s**.

5.3 Proposed Stormwater Management System

It is contemplated that the stormwater outlet from the proposed development will be to the existing 675mm diameter storm sewer within New Orchard Avenue North.

To meet the stormwater objectives the proposed development will likely require an internal cistern.

Table 7 summarizes post-development flow rates. The following storage requirement estimate assumes that approximately 10% of the development area will be directed to the outlet without flow attenuation. These areas will be compensated for in areas with flow attenuation controls.

**Table 7
 Stormwater Flow Rate Summary**

| Control Area | 5-Year Release Rate (L/s) | 5-Year Storage (m ³) | 100-Year Release Rate (L/s) | 100-Year Storage (m ³) |
|--------------------|---------------------------------|--|-----------------------------------|--|
| Unattenuated Areas | 1.8 | 0.0 | 3.7 | 0.0 |
| Attenuated Areas | 2.0 | 12.1 | 3.9 | 24.2 |
| Total | 3.8 | 12.1 | 7.7 | 24.2 |

It is anticipated that approximately **24.2 m³** of storage provided via an internal cistern will be required on site to attenuate flow to the established release rate of **7.7 L/s**; storage calculations are contained within **Appendix D**.

Actual storage volumes will need to be confirmed at the detailed design stage based on a number of factors including, but not limited to grading constraints.

5.4 Stormwater Servicing Conclusions

In accordance with City of Ottawa **City Standards**, post development stormwater runoff will be required to be restricted to the allowable target release rate for storm events up to and including the 100-year storm. The post-development allowable release rate was calculated as **7.7 L/s**; it is estimated that **24.2 m³** of storage provided via an internal cistern will be required to meet the established release rate.

Quality controls are anticipated to be required for the proposed development due to the site's distance from the outlet; correspondence with the RVCA is included in **Appendix A**, however no response was received at the time of publication.

The proposed stormwater design conforms to all relevant **City Standards** and Policies for approval

6.0 UTILITIES

Utility servicing will be coordinated with the individual utility companies prior to site development.

7.0 CONCLUSION AND RECOMMENDATIONS

David Schaeffer Engineering Ltd. (DSEL) has been retained by New Orchard Investment Inc. to prepare an Assessment of Adequacy of Public Services report in support of the application for a Zoning By-law Amendment (ZBLA) at 100 New Orchard Avenue North. The preceding report outlines the following:

- The watermain boundary conditions have been requested from the City of Ottawa, however they were unavailable at the time of this publication;
- The FUS method for estimating fire flow indicated **13,000 L/min** of flow is required for the contemplated development,
- The contemplated development is anticipated to have a peak wet weather flow of **1.61 L/s**. Based on the distance to the Collector sewer and complexity of the drainage area, the anticipated flow from the site was provided to confirm capacity and resulting HGL due to the proposed development.
- Based on consultation with the City of Ottawa, the contemplated development will be required to attenuate post development flows to an equivalent release rate of **7.7 L/s** for all storms up to and including the 100-year storm event;
- It is contemplated that stormwater objectives may be met through storm water retention via an internal cistern, it is estimated that **24.2 m³** of onsite storage will be required to attenuate flow to the established release rate;
- Based on the distance from the Ottawa River Outlet stormwater quality controls are anticipated;

Prepared by,
David Schaeffer Engineering Ltd.



Per: Alison J. Gosling, EIT.

Reviewed by,
David Schaeffer Engineering Ltd.



Per: Robert D. Freel, P.Eng.

APPENDIX A

Pre-Consultation

DEVELOPMENT SERVICING STUDY CHECKLIST

18-1031

15/06/2018

| 4.1 General Content | | |
|---|---|------------------------|
| <input type="checkbox"/> | Executive Summary (for larger reports only). | N/A |
| <input checked="" type="checkbox"/> | Date and revision number of the report. | Report Cover Sheet |
| <input checked="" type="checkbox"/> | Location map and plan showing municipal address, boundary, and layout of proposed development. | Drawings/Figures |
| <input checked="" type="checkbox"/> | Plan showing the site and location of all existing services. | Figure 1 |
| <input checked="" type="checkbox"/> | Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to applicable subwatershed and watershed plans that provide context to which individual developments must adhere. | Section 1.0 |
| <input checked="" type="checkbox"/> | Summary of Pre-consultation Meetings with City and other approval agencies. | Section 1.3 |
| <input checked="" type="checkbox"/> | Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria. | Section 2.1 |
| <input checked="" type="checkbox"/> | Statement of objectives and servicing criteria. | Section 1.0 |
| <input checked="" type="checkbox"/> | Identification of existing and proposed infrastructure available in the immediate area. | Sections 3.1, 4.1, 5.1 |
| <input type="checkbox"/> | Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available). | N/A |
| <input type="checkbox"/> | Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths. | N/A |
| <input type="checkbox"/> | Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts. | N/A |
| <input type="checkbox"/> | Proposed phasing of the development, if applicable. | N/A |
| <input type="checkbox"/> | Reference to geotechnical studies and recommendations concerning servicing. | N/A |
| <input type="checkbox"/> | All preliminary and formal site plan submissions should have the following information: -Metric scale -North arrow (including construction North) -Key plan -Name and contact information of applicant and property owner -Property limits including bearings and dimensions -Existing and proposed structures and parking areas -Easements, road widening and rights-of-way -Adjacent street names | N/A |
| 4.2 Development Servicing Report: Water | | |
| <input type="checkbox"/> | Confirm consistency with Master Servicing Study, if available | N/A |
| <input checked="" type="checkbox"/> | Availability of public infrastructure to service proposed development | Section 3.1 |
| <input checked="" type="checkbox"/> | Identification of system constraints | Section 3.1 |
| <input checked="" type="checkbox"/> | Identify boundary conditions | Section 3.1, 3.2 |
| <input checked="" type="checkbox"/> | Confirmation of adequate domestic supply and pressure | Section 3.3 |

| | | |
|-------------------------------------|--|------------------|
| <input checked="" type="checkbox"/> | Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. | Section 3.2 |
| <input type="checkbox"/> | Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. | N/A |
| <input type="checkbox"/> | Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design | N/A |
| <input type="checkbox"/> | Address reliability requirements such as appropriate location of shut-off valves | N/A |
| <input type="checkbox"/> | Check on the necessity of a pressure zone boundary modification | N/A |
| <input checked="" type="checkbox"/> | Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range | Section 3.2, 3.3 |
| <input type="checkbox"/> | Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions. | N/A |
| <input type="checkbox"/> | Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation. | N/A |
| <input checked="" type="checkbox"/> | Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines. | Section 3.2 |
| <input type="checkbox"/> | Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference. | N/A |

4.3 Development Servicing Report: Wastewater

| | | |
|-------------------------------------|--|-------------------------|
| <input checked="" type="checkbox"/> | Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure). | Section 4.2 |
| <input type="checkbox"/> | Confirm consistency with Master Servicing Study and/or justifications for deviations. | N/A |
| <input type="checkbox"/> | Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers. | N/A |
| <input checked="" type="checkbox"/> | Description of existing sanitary sewer available for discharge of wastewater from proposed development. | Section 4.1 |
| <input checked="" type="checkbox"/> | Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable) | Section 4.2 |
| <input checked="" type="checkbox"/> | Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format. | Section 4.2, Appendix C |
| <input checked="" type="checkbox"/> | Description of proposed sewer network including sewers, pumping stations, and forcemains. | Section 4.2 |
| <input type="checkbox"/> | Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality). | N/A |

| | | |
|--------------------------|--|-----|
| <input type="checkbox"/> | Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development. | N/A |
| <input type="checkbox"/> | Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity. | N/A |
| <input type="checkbox"/> | Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding. | N/A |
| <input type="checkbox"/> | Special considerations such as contamination, corrosive environment etc. | N/A |

4.4 Development Servicing Report: Stormwater Checklist

| | | |
|-------------------------------------|--|-------------------------|
| <input checked="" type="checkbox"/> | Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property) | Section 5.1 |
| <input checked="" type="checkbox"/> | Analysis of available capacity in existing public infrastructure. | Section 5.1, Appendix D |
| <input checked="" type="checkbox"/> | A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern. | Drawings/Figures |
| <input checked="" type="checkbox"/> | Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects. | Section 5.2 |
| <input checked="" type="checkbox"/> | Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements. | Section 5.2 |
| <input checked="" type="checkbox"/> | Description of the stormwater management concept with facility locations and descriptions with references and supporting information | Section 5.3 |
| <input type="checkbox"/> | Set-back from private sewage disposal systems. | N/A |
| <input type="checkbox"/> | Watercourse and hazard lands setbacks. | N/A |
| <input checked="" type="checkbox"/> | Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed. | Appendix A |
| <input type="checkbox"/> | Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists. | N/A |
| <input checked="" type="checkbox"/> | Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period). | Section 5.3 |
| <input type="checkbox"/> | Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals. | N/A |
| <input checked="" type="checkbox"/> | Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions. | Section 5.1, 5.3 |
| <input type="checkbox"/> | Any proposed diversion of drainage catchment areas from one outlet to another. | N/A |
| <input type="checkbox"/> | Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities. | N/A |
| <input type="checkbox"/> | If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event. | N/A |
| <input type="checkbox"/> | Identification of potential impacts to receiving watercourses | N/A |
| <input type="checkbox"/> | Identification of municipal drains and related approval requirements. | N/A |

| | | |
|-------------------------------------|---|-------------|
| <input checked="" type="checkbox"/> | Descriptions of how the conveyance and storage capacity will be achieved for the development. | Section 5.3 |
| <input type="checkbox"/> | 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading. | N/A |
| <input type="checkbox"/> | Inclusion of hydraulic analysis including hydraulic grade line elevations. | N/A |
| <input checked="" type="checkbox"/> | Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors. | Section 6.0 |
| <input type="checkbox"/> | Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions. | N/A |
| <input type="checkbox"/> | Identification of fill constraints related to floodplain and geotechnical investigation. | N/A |

4.5 Approval and Permit Requirements: Checklist

| | | |
|-------------------------------------|---|-------------|
| <input checked="" type="checkbox"/> | Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement ct. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act. | Section 1.2 |
| <input type="checkbox"/> | Application for Certificate of Approval (CofA) under the Ontario Water Resources Act. | N/A |
| <input type="checkbox"/> | Changes to Municipal Drains. | N/A |
| <input type="checkbox"/> | Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) | N/A |

4.6 Conclusion Checklist

| | | |
|-------------------------------------|---|-------------|
| <input checked="" type="checkbox"/> | Clearly stated conclusions and recommendations | Section 8.0 |
| <input type="checkbox"/> | Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency. | |
| <input type="checkbox"/> | All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario | |

Alison Gosling

From: Alison Gosling
Sent: Wednesday, June 13, 2018 1:47 PM
To: 'Schaeffer, Gabrielle'
Cc: Robert Freel
Subject: 18-1031 100 New Orchard Avenue

Good afternoon Gabrielle,

We would like to confirm Stormwater Management criteria and connection locations to municipal infrastructure for the site based on the pre-consultation meeting with the Bobby on May 31st, 2018.

Based on the information we were able to obtain, the surrounding municipal infrastructure exists:

Water:

- There is an existing 152mm diameter watermain within the New Orchard Avenue right-of-way and a 1220mm diameter watermain located North of the subject site.
- It is contemplated that the development will connect to the existing 152mm diameter watermain within New Orchard Avenue.

Sanitary:

- There is an existing 300mm diameter sanitary sewer within the New Orchard Avenue right-of-way and a 1350mm diameter sanitary sewer North of the subject site.
- It is contemplated that the development will connect to the existing 300mm diameter sanitary sewer within New Orchard Avenue.
- Based on preliminary calculations, there is a net increase in peak wet weather sanitary flow of approximately 1.9 L/s. Due to the site's proximity to the West Nepean Trunk Collector, the increase is not anticipated to create a capacity issue to the existing sanitary sewer. Can you please confirm capacity and HGL with the wastewater modelling group?

Storm:

- There is an existing 675mm diameter storm sewer within the New Orchard Avenue right-of-way, which is contemplated to service the development.
- It is contemplated that the site would need to meet an allowable release rate based on either a Rational Method Coefficient of 0.50 or the calculated existing Rational Method Coefficient (the lesser), employing the City of Ottawa IDF parameters for a 2-year storm with a calculated time of concentration.

Can you please confirm the background information and assumptions above?

Thank you,

Alison Gosling, E.I.T.
Project Coordinator / Junior Designer

DSEL

david schaeffer engineering ltd.

120 Iber Road, Unit 103
Stittsville, ON K2S 1E9

phone: (613) 836-0856 ext.542

fax: (613) 836-7183
email: agosling@dsel.ca

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Alison Gosling

From: Alison Gosling
Sent: Wednesday, June 13, 2018 1:56 PM
To: 'Eric Lalande'
Cc: Charlotte Kelly
Subject: 18-1031 100 New Orchard Avenue - Quality Requirement
Attachments: N_ORCH Preliminary Plans 2018_06_09.pdf

Good afternoon Eric,

We wanted to touch base with you regarding a development we are working on located at 100 New Orchard Avenue.

The stormwater collected from the site travels approximately 200m to a direct outlet into the Ottawa River.

The development proposes to construct a 13-storey residential building with associated underground parking. The development will discharge stormwater to the existing 675 mm diameter storm sewer within New Orchard Avenue. As shown by the attached, runoff will be mainly rooftop and landscaped areas.

Can you provide a comment regarding quality controls that maybe required for the site?



Please feel free to call if you have any questions or you would like to discuss.

Thank you,

Alison Gosling, E.I.T.
Project Coordinator / Junior Designer

DSEL

david schaeffer engineering ltd.

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Stittsville, ON K2S 1E9

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Charlotte Kelly

From: Alison Gosling
Sent: June 13, 2018 2:47 PM
To: Charlotte Kelly
Subject: FW: Job:18-1031 - 100 New Orchard Ave.
Attachments: wtr-2018-05-31_1031_FUS.PDF

From: Alison Gosling <AGosling@dsel.ca>
Sent: Thursday, May 31, 2018 5:28 PM
To: Schaeffer, Gabrielle <gabrielle.schaeffer@Ottawa.ca>
Cc: Robert Freel <RFreel@dsel.ca>
Subject: Job:18-1031 - 100 New Orchard Ave.

Good afternoon Gabrielle,

We would like to request water boundary conditions for New Orchard Avenue using the following proposed development demands:

1. Location of Service / Street Number: 100 New Orchard Avenue
2. Type of development and the amount of fire flow required for the proposed development:
 - The proposed development is a condominium building consisting of **75** residential units and **243 m²** of amenity space.
 - It is anticipated that the development will have a single connection to be serviced from the existing 152 mm diameter watermain within New Orchard Avenue, as shown by the attached map.
 - It is anticipated that a fire flow of 13,000 L/min will be required for the development.
- 3.

| | L/min | L/s |
|-------------------|-------|------|
| Avg. Daily | 26.7 | 0.44 |
| Max Day | 129.3 | 2.15 |
| Peak Hour | 195.4 | 3.26 |

If you have any questions please feel free to contact me.



Thank you,

Alison Gosling, E.I.T.
Project Coordinator / Junior Designer

DSEL

david schaeffer engineering ltd.

120 Iber Road, Unit 103
Stittsville, ON K2S 1E9

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APPENDIX B

Water Supply

Water Demand Design Flows per Unit Count
City of Ottawa - Water Distribution Guidelines, July 2010



Domestic Demand

| Type of Housing | Per / Unit | Units | Pop |
|-----------------|------------|-------|-----|
| Single Family | 3.4 | | 0 |
| Semi-detached | 2.7 | | 0 |
| Townhouse | 2.7 | | 0 |
| Apartment | | | 0 |
| Bachelor | 1.4 | | 0 |
| 1 Bedroom | 1.4 | 84 | 118 |
| 2 Bedroom | 2.1 | | 0 |
| 3 Bedroom | 3.1 | | 0 |
| Average | 1.8 | | 0 |

| | Pop | Avg. Daily | | Max Day | | Peak Hour | |
|------------------------------|-----|-------------------|-------|-------------------|-------|-------------------|-------|
| | | m ³ /d | L/min | m ³ /d | L/min | m ³ /d | L/min |
| Total Domestic Demand | 118 | 33.0 | 22.9 | 161.9 | 112.4 | 244.5 | 169.8 |

Institutional / Commercial / Industrial Demand

| Property Type | Unit Rate | Units | Avg. Daily | | Max Day | | Peak Hour | |
|--------------------------|---------------------------|-------|-------------------|-------------|-------------------|--------------|-------------------|--------------|
| | | | m ³ /d | L/min | m ³ /d | L/min | m ³ /d | L/min |
| Amenity space | 2.5 L/m ² /d | 358 | 0.90 | 0.6 | 1.3 | 0.9 | 2.4 | 1.7 |
| Office | 75 L/9.3m ² /d | | 0.00 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Restaurant* | 125 L/seat/d | | 0.00 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Industrial - Light | 35,000 L/gross ha/d | | 0.00 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Industrial - Heavy | 55,000 L/gross ha/d | | 0.00 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total I/CI Demand | | | 0.9 | 0.6 | 1.3 | 0.9 | 2.4 | 1.7 |
| Total Demand | | | 33.9 | 23.6 | 163.2 | 113.4 | 246.9 | 171.5 |

Fire Flow Estimation per Fire Underwriters Survey

Water Supply For Public Fire Protection - 1999



Fire Flow Required

1. Base Requirement

$$F = 220C\sqrt{A}$$

L/min

Where *F* is the fire flow, *C* is the Type of construction and *A* is the Total floor area

Type of Construction:

Ordinary Construction

C 1 Type of Construction Coefficient per FUS Part II, Section 1
A 7865.0 m² Total floor area based on FUS Part II section 1

Fire Flow 19510.7 L/min
20000.0 L/min rounded to the nearest 1,000 L/min

Adjustments

2. Reduction for Occupancy Type

Limited Combustible -15%

Fire Flow 17000.0 L/min

3. Reduction for Sprinkler Protection

Sprinklered - Supervised -50%

Reduction -8500 L/min

4. Increase for Separation Distance

| Cons. of Exposed Wall | S.D | Lw | Ha | LH | EC | |
|--|-------------------|------|----|----|-----|------------------------------------|
| N Ordinary - Unprotected Openings | >45m | | | 0 | 0 | 0% |
| S Ordinary - Unprotected Openings | 3.1m-10m | 30.2 | | 13 | 393 | 19% |
| E Ordinary - Unprotected Openings | 20.1m-30m | 19.4 | | 1 | 20 | 6% |
| W Ordinary - Unprotected Openings | >45m | | | 0 | 0 | 0% |
| | % Increase | | | | | 25% value not to exceed 75% |

Increase 4250.0 L/min

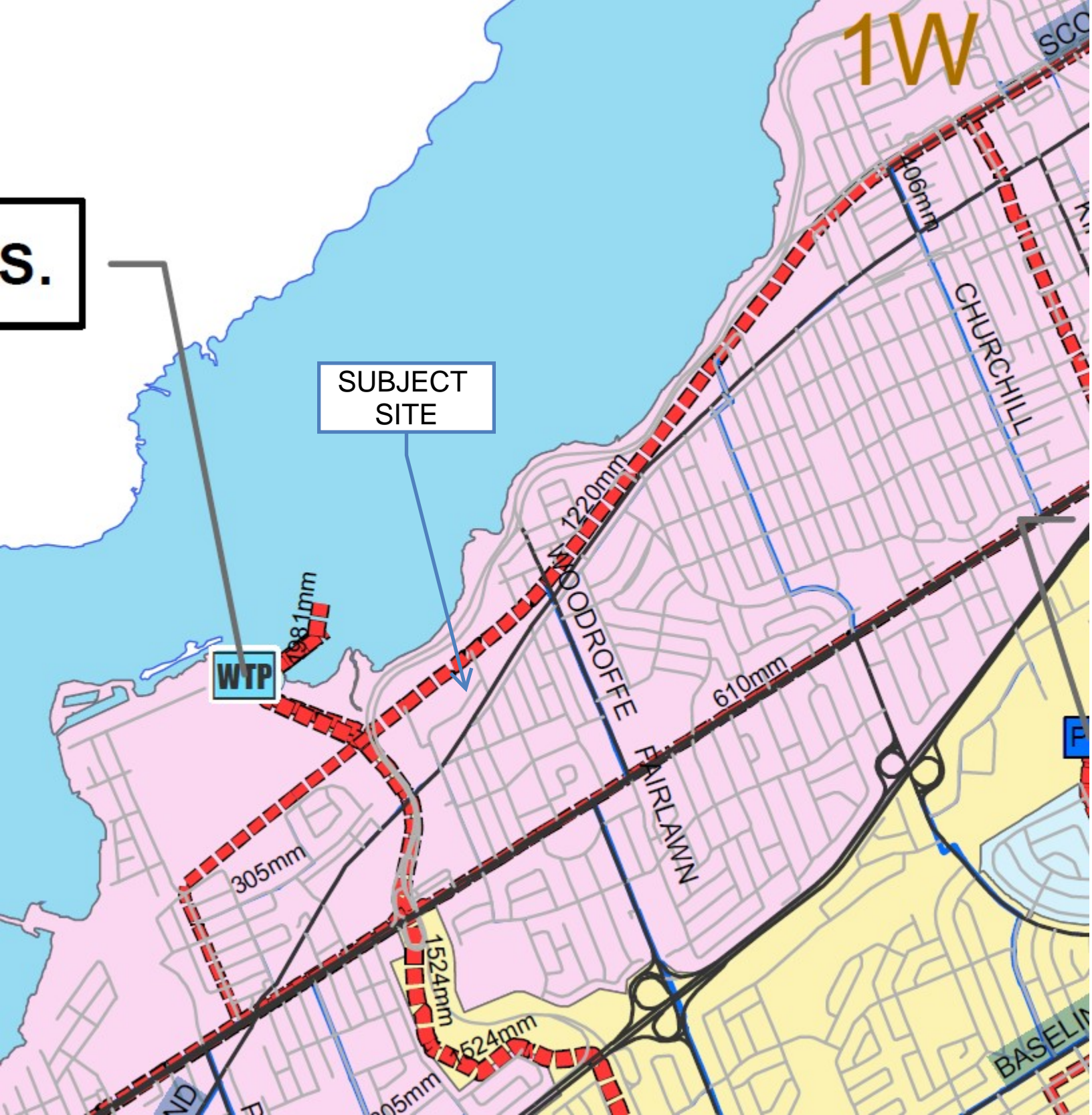
Lw = Length of the Exposed Wall
 Ha = number of storeys of the adjacent structure
 LH = Length-height factor of exposed wall. Value rounded up.
 EC = Exposure Charge

Total Fire Flow

Fire Flow 12750.0 L/min fire flow not to exceed 45,000 L/min nor be less than 2,000 L/min per FUS Section
13000.0 L/min rounded to the nearest 1,000 L/min

Notes:

- Type of construction, Occupancy Type and Sprinkler Protection information provided by _____.
- Calculations based on Fire Underwriters Survey - Part II



CITY OF OTTAWA- WATER DISTRIBUTION SYSTEM

APPENDIX C

Wastewater Collection

Wastewater Design Flows per Unit Count
City of Ottawa Sewer Design Guidelines, 2004



Site Area 0.074 ha

Extraneous Flow Allowances

Infiltration / Inflow 0.02 L/s

Domestic Contributions

| Unit Type | Unit Rate | Units | Pop |
|--------------------------|-----------|-------|-----|
| Single Family | 3.4 | 1 | 4 |
| Semi-detached and duplex | 2.7 | | 0 |
| Townhouse | 2.7 | | 0 |
| Stacked Townhouse | 2.3 | | 0 |
| Apartment | | | |
| Bachelor | 1.4 | | 0 |
| 1 Bedroom | 1.4 | | 0 |
| 2 Bedroom | 2.1 | | 0 |
| 3 Bedroom | 3.1 | | 0 |
| Average | 1.8 | | 0 |

Total Pop 4

Average Domestic Flow 0.01 L/s

Peaking Factor 4.00

Peak Domestic Flow 0.05 L/s

Institutional / Commercial / Industrial Contributions

| Property Type | Unit Rate | No. of Units | Avg Wastewater (L/s) |
|--------------------------|-----------------------|--------------|----------------------|
| Commercial floor space* | 5 L/m ² /d | | 0.00 |
| Ex. Industrial - Light** | 35,000 L/gross ha/d | | 0.00 |
| Industrial - Light** | 35,000 L/gross ha/d | | 0.00 |
| Industrial - Heavy** | 55,000 L/gross ha/d | | 0.00 |

Average I/C/I Flow 0.00

Peak Institutional / Commercial Flow 0.00

Peak Industrial Flow** 0.00

Peak I/C/I Flow 0.00

** peak industrial flow per City of Ottawa Sewer Design Guidelines Appendix 4B

| | |
|---|----------|
| Total Estimated Average Dry Weather Flow Rate | 0.01 L/s |
| Total Estimated Peak Dry Weather Flow Rate | 0.05 L/s |
| Total Estimated Peak Wet Weather Flow Rate | 0.07 L/s |

Wastewater Design Flows per Unit Count
City of Ottawa Sewer Design Guidelines, 2004



Site Area 0.074 ha

Extraneous Flow Allowances

Infiltration / Inflow 0.02 L/s

Domestic Contributions

| Unit Type | Unit Rate | Units | Pop |
|--------------------------|-----------|-------|-----|
| Single Family | 3.4 | | 0 |
| Semi-detached and duplex | 2.7 | | 0 |
| Townhouse | 2.7 | | 0 |
| Stacked Townhouse | 2.3 | | 0 |
| Apartment | | | |
| Bachelor | 1.4 | | 0 |
| 1 Bedroom | 1.4 | 84 | 118 |
| 2 Bedroom | 2.1 | | 0 |
| 3 Bedroom | 3.1 | | 0 |
| Average | 1.8 | | 0 |

Total Pop 118

Average Domestic Flow 0.38 L/s

Peaking Factor 4.00

Peak Domestic Flow 1.53 L/s

Institutional / Commercial / Industrial Contributions

| Property Type | Unit Rate | No. of Units | Avg Wastewater (L/s) |
|--------------------------|-----------------------|--------------|----------------------|
| Amenity Space | 5 L/m ² /d | 358 | 0.04 |
| Ex. Industrial - Light** | 35,000 L/gross ha/d | | 0.00 |
| Industrial - Light** | 35,000 L/gross ha/d | | 0.00 |
| Industrial - Heavy** | 55,000 L/gross ha/d | | 0.00 |

Average I/C/I Flow 0.04

Peak Institutional / Commercial Flow 0.06

Peak Industrial Flow** 0.00

Peak I/C/I Flow 0.06

** peak industrial flow per City of Ottawa Sewer Design Guidelines Appendix 4B

| | |
|---|----------|
| Total Estimated Average Dry Weather Flow Rate | 0.42 L/s |
| Total Estimated Peak Dry Weather Flow Rate | 1.59 L/s |
| Total Estimated Peak Wet Weather Flow Rate | 1.61 L/s |

**CITY OF OTTAWA-
SANITARY TRUNK AND
COLLECTION AREAS**

WEST NEPEAN COLLECTOR

**SUBJECT
SITE**

**BRITANNIA
PURIFICATION PLANT**

Cassels St.

POOLER

87 Pooler Ave.

MCEWEN

31 McEwen Ave.

**WOODROFFE AVE.
COLLECTOR**

LINCOLN HGTS.

5 Lincoln Hgts. Rd.

BRITANNIA RD.

@Howe

**CAVE CREEK
COLLECTOR**

**PINECREST
COLLECTOR**

WOODROFFE DIVERSION

1085 Woodroffe Ave.



APPENDIX D

Stormwater Management

Estimated Peak Stormwater Flow Rate
 City of Ottawa Sewer Design Guidelines, 2012



Existing Drainage Characteristics From Internal Site

| | |
|----------------|---|
| Area | 0.074 ha |
| C | 0.48 Rational Method runoff coefficient |
| L | 25.13 m |
| Up Elev | 62.75 m |
| Dn Elev | 59.25 m |
| Slope | 13.9 % |
| Tc | 4.2 min |

| | Imp. | Perv. | Total |
|-------------|-------------|--------------|--------------|
| Area | 0.030 | 0.044 | 0.074 |
| C | 0.9 | 0.2 | 0.48 |

1) Time of Concentration per Federal Aviation Administration

$$t_c = \frac{1.8(1.1 - C)L^{0.5}}{S^{0.333}}$$

- tc, in minutes
- C, rational method coefficient, (-)
- L, length in ft
- S, average watershed slope in %

Estimated Peak Flow

| | 2-year | 5-year | 100-year |
|----------|---------------|---------------|-----------------|
| i | 110.0 | 150.1 | 258.3 mm/hr |
| Q | 11.0 | 14.9 | 32.1 L/s |

Note:
 C value for the 100-year storm is increased by 25%, to a maximum of 1.0 per Ottawa Sewer Design Guidelines (5.4.5.2.1)

Stormwater - Proposed Development
City of Ottawa Sewer Design Guidelines, 2004



Target Flow Rate

Area 0.075 ha
C 0.48 Rational Method runoff coefficient
t_c 10.0 min

2-year
i 76.8 mm/hr
Q 7.7 L/s

Estimated Post Development Peak Flow from Unattenuated Areas

Total Area 0.007 ha
C 0.85 Rational Method runoff coefficient

| t _c (min) | 5-year | | | | | 100-year | | | | |
|-------------------------|--------------|------------------------------|-------------------------------|------------------------------|--|--------------|------------------------------|-------------------------------|------------------------------|--|
| | i (mm/hr) | Q _{actual} (L/s) | Q _{release} (L/s) | Q _{stored} (L/s) | V _{stored} (m ³) | i (mm/hr) | Q _{actual} (L/s) | Q _{release} (L/s) | Q _{stored} (L/s) | V _{stored} (m ³) |
| 10.0 | 104.2 | 1.8 | 1.8 | 0.0 | 0.0 | 178.6 | 3.7 | 3.7 | 0.0 | 0.0 |

Note:
C value for the 100-year storm is increased by 25%, to a maximum of 1.0 per Ottawa Sewer Design Guidelines (5.4.5.2.1)

Estimated Post Development Peak Flow from Attenuated Areas

Total Area 0.067 ha
C 0.85 Rational Method runoff coefficient

| t _c (min) | 5-year | | | | | 100-year | | | | |
|-------------------------|--------------|------------------------------|-------------------------------|------------------------------|--|--------------|------------------------------|-------------------------------|------------------------------|--|
| | i (mm/hr) | Q _{actual} (L/s) | Q _{release} (L/s) | Q _{stored} (L/s) | V _{stored} (m ³) | i (mm/hr) | Q _{actual} (L/s) | Q _{release} (L/s) | Q _{stored} (L/s) | V _{stored} (m ³) |
| 10 | 104.2 | 16.6 | 2.0 | 14.6 | 8.8 | 178.6 | 33.4 | 3.9 | 29.4 | 17.7 |
| 15 | 83.6 | 13.3 | 2.0 | 11.3 | 10.2 | 142.9 | 26.7 | 3.9 | 22.8 | 20.5 |
| 20 | 70.3 | 11.2 | 2.0 | 9.2 | 11.0 | 120.0 | 22.4 | 3.9 | 18.5 | 22.2 |
| 25 | 60.9 | 9.7 | 2.0 | 7.7 | 11.6 | 103.8 | 19.4 | 3.9 | 15.5 | 23.2 |
| 30 | 53.9 | 8.6 | 2.0 | 6.6 | 11.9 | 91.9 | 17.2 | 3.9 | 13.2 | 23.8 |
| 35 | 48.5 | 7.7 | 2.0 | 5.7 | 12.1 | 82.6 | 15.4 | 3.9 | 11.5 | 24.1 |
| 40 | 44.2 | 7.0 | 2.0 | 5.0 | 12.1 | 75.1 | 14.1 | 3.9 | 10.1 | 24.2 |
| 45 | 40.6 | 6.5 | 2.0 | 4.5 | 12.1 | 69.1 | 12.9 | 3.9 | 9.0 | 24.2 |
| 50 | 37.7 | 6.0 | 2.0 | 4.0 | 12.0 | 64.0 | 12.0 | 3.9 | 8.0 | 24.0 |
| 55 | 35.1 | 5.6 | 2.0 | 3.6 | 11.9 | 59.6 | 11.1 | 3.9 | 7.2 | 23.8 |
| 60 | 32.9 | 5.2 | 2.0 | 3.3 | 11.7 | 55.9 | 10.5 | 3.9 | 6.5 | 23.4 |
| 65 | 31.0 | 4.9 | 2.0 | 3.0 | 11.5 | 52.6 | 9.8 | 3.9 | 5.9 | 23.0 |
| 70 | 29.4 | 4.7 | 2.0 | 2.7 | 11.3 | 49.8 | 9.3 | 3.9 | 5.4 | 22.5 |
| 75 | 27.9 | 4.4 | 2.0 | 2.5 | 11.0 | 47.3 | 8.8 | 3.9 | 4.9 | 22.0 |
| 80 | 26.6 | 4.2 | 2.0 | 2.2 | 10.8 | 45.0 | 8.4 | 3.9 | 4.5 | 21.4 |
| 85 | 25.4 | 4.0 | 2.0 | 2.0 | 10.5 | 43.0 | 8.0 | 3.9 | 4.1 | 20.8 |
| 90 | 24.3 | 3.9 | 2.0 | 1.9 | 10.1 | 41.1 | 7.7 | 3.9 | 3.7 | 20.2 |
| 95 | 23.3 | 3.7 | 2.0 | 1.7 | 9.8 | 39.4 | 7.4 | 3.9 | 3.4 | 19.5 |
| 100 | 22.4 | 3.6 | 2.0 | 1.6 | 9.5 | 37.9 | 7.1 | 3.9 | 3.1 | 18.8 |
| 105 | 21.6 | 3.4 | 2.0 | 1.4 | 9.1 | 36.5 | 6.8 | 3.9 | 2.9 | 18.1 |
| 110 | 20.8 | 3.3 | 2.0 | 1.3 | 8.7 | 35.2 | 6.6 | 3.9 | 2.6 | 17.4 |

Note:
C value for the 100-year storm is increased by 25%, to a maximum of 1.0 per Ottawa Sewer Design Guidelines (5.4.5.2.1)

5-year Q_{attenuated} 1.97 L/s 100-year Q_{attenuated} 3.95 L/s
5-year Max. Storage Required 12.1 m³ 100-year Max. Storage Required 24.2 m³

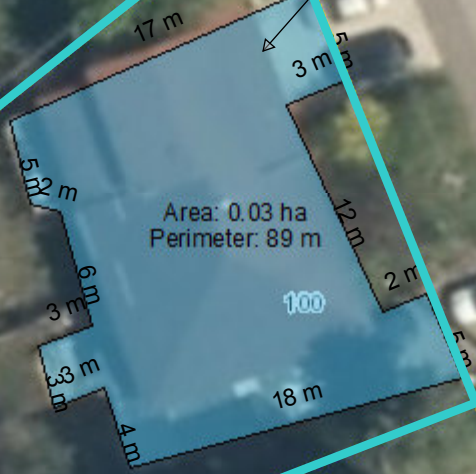
Summary of Release Rates and Storage Volumes

| Control Area | 5-Year Release Rate (L/s) | 5-Year Storage (m ³) | 100-Year Release Rate (L/s) | 100-Year Storage (m ³) |
|--------------------|---------------------------|----------------------------------|-----------------------------|------------------------------------|
| Unattenuated Areas | 1.8 | 0.0 | 3.7 | 0.0 |
| Attenuated Areas | 2.0 | 12.1 | 3.9 | 24.2 |
| Total | 3.8 | 12.1 | 7.7 | 24.2 |

IMPERVIOUS AREA ESTIMATION

SUBJECT SITE

IMPERVIOUS AREA

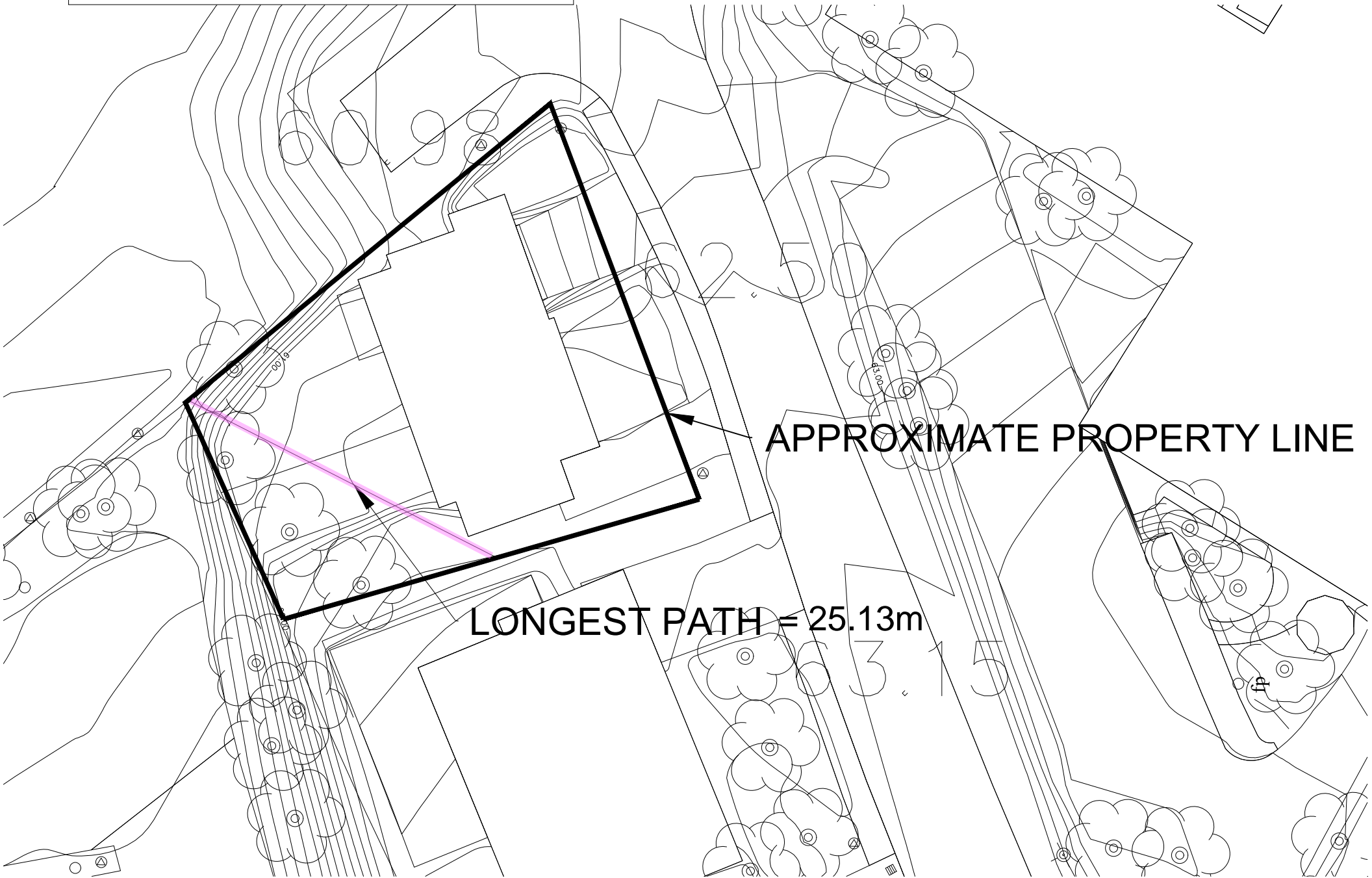


ave. New Orchard Ave. N.

99

108

ESTIMATION OF T_c



DRAWINGS / FIGURES

PROJECT INFORMATION

SITE AREA = 742.3m2
(7990sq.ft.)
 BUILDING AREA
 (max. footprint) = 605m2
(6510 sq.ft.)
 LANDSCAPED AREA =88m2 (11.9%)
 No. of STORIES = 14
 No. of Dwelling Units = 84 (includes 9 third floor units designated as affordable housing)

PARKING REQUIRED

SCHEDULE 1A AREA X
 1st 12 UNITS = 0
 63 UNITS @ 0.5 SPACES/ UNIT = 31.5
 LESS 10% U/G = 3.15
 TOTAL SPACES REQUIRED = 28
 VISITOR PARKING REQUIRED
 1st 12 UNITS = 0
 63 UNITS @ 0.1 = 6

TOTAL PARKING REQUIRED = 34 SPACES

PARKING PROVIDED

U/G LEVEL = 16 SPACES (INCL. 1 BARRIER FREE SPACE)
 GROUND = 8 SPACES
 SECOND = 11 SPACES

PARKING PROVIDED = 35 SPACES

BICYCLE PARKING REQUIRED

75 D.U. X 0.5 = 38 SPACES REQUIRED

BICYCLE PARKING PROVIDED

25 HORIZONTAL SPACES
 14 VERTICAL SPACES
 TOTAL PROVIDED = 39 SPACES

APARTMENT UNITS

FLOOR 3 @ 9 D.U./ FLR = 9 UNITS (AFFORDABLE HOUSING)
 FLOORS 3-7 @ 8 D.U./FLR = 40 UNITS
 FLOORS 8-12 @ 7 D.U./FLR = 35 UNITS > 75 UNITS
 14TH FLOOR - AMENITY AREA 358m2 (3855 sq.ft.)

TOTAL DWELLING UNITS = 84

GFA per ZONING DEFINITION = 4325m2
 PROPOSED FSI = 5.83

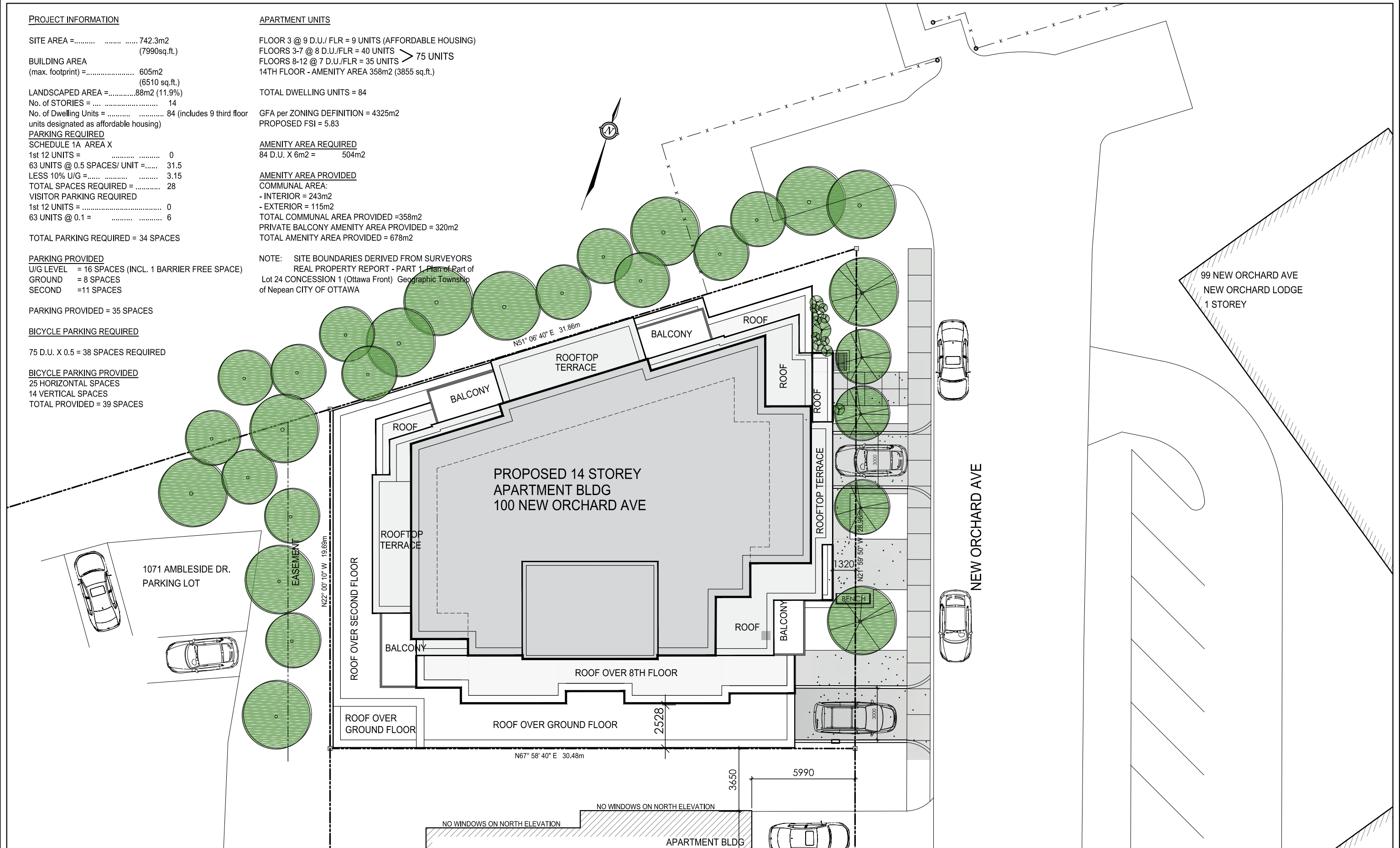
AMENITY AREA REQUIRED

84 D.U. X 6m2 = 504m2

AMENITY AREA PROVIDED

COMMUNAL AREA:
 - INTERIOR = 243m2
 - EXTERIOR = 115m2
 TOTAL COMMUNAL AREA PROVIDED = 358m2
 PRIVATE BALCONY AMENITY AREA PROVIDED = 320m2
 TOTAL AMENITY AREA PROVIDED = 678m2

NOTE: SITE BOUNDARIES DERIVED FROM SURVEYORS
 REAL PROPERTY REPORT - PART 1 Plan of Part of
 Lot 24 CONCESSION 1 (Ottawa Front) Geographic Township
 of Nepean CITY OF OTTAWA



99 NEW ORCHARD AVE
 NEW ORCHARD LODGE
 1 STOREY

NEW ORCHARD AVE

1071 AMBLESIDE DR.
 PARKING LOT

PROPOSED 14 STOREY
 APARTMENT BLDG
 100 NEW ORCHARD AVE

197m to outlet (Ottawa River)

100 New Orchard Avenue Constraints

Subject Site

