

# **SERVICING AND STORMWATER MANAGEMENT REPORT**

**FOR**

**THE PROPERTIES GROUP  
70 BEECH STREET & 75 NORMAN  
STREET**

**CITY OF OTTAWA**

**PROJECT NO.: 18-1015**

**MARCH 2019 – REV 1  
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FOR  
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**THE PROPERTIES GROUP  
MARCH 2019 – REV 1**

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- City of Ottawa Boundary Conditions dated January 29<sup>th</sup>, 2019
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- Site Plan, prepared by Alcaide Webster Architects Inc., dated February 5<sup>th</sup>, 2019
- Topographic Survey prepared by Annis, O’Sullivan, Vollebakk Ltd. Dated March 5<sup>th</sup>, 2018

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**1.0 INTRODUCTION**

David Schaeffer Engineering Limited (DSEL) has been retained by The Properties Group to prepare a Servicing and Stormwater Management report in support of the application for Site Plan Control (SPC) at 70 Beech Street and 75 Norman Street.

The subject property is located within the City of Ottawa urban boundary, in the Somerset ward. As illustrated in **Figure 1**, the subject property is bound by Beech Street to the North and Norman Street to the South. Currently comprised of two parcels of land, 70 Beech Street and 75 Norman Street, the properties are to be joined into one parcel. The subject properties measure approximately **0.12 ha**. 70 Beech Street is zoned Traditional Mainstreet (TM) and 75 Norman is zoned Residential Fourth Density (R4).



**Figure 1: Site Location**

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The proposed SPC would allow for the development of a 6-storey commercial/residential building fronting onto Beech Street, within **0.08 ha** of the subject site. It is proposed surface parking for the building is to be located within the existing parking area of 75 Norman Street with access through the existing driveway off of Norman Street. The existing house and all existing service connections to Norman Street are to remain. The proposed development would include approximately **538 m<sup>2</sup>** of commercial/amenity space and approximately residential **48** dwelling units. A copy of the Site Plan prepared by RLA Architecture is included in **Drawings/Figures**.

The objective of this report is to provide sufficient detail to demonstrate that the proposed development is supported by existing municipal services.

## 1.1 Existing Conditions

The existing 70 Beech street parcel includes an auto-mechanic garage including a paved parking area. 75 Norman Street contains a duplex and paved parking area in the rear yard. The elevations range between 61.97 m and 63.37 m with a grade change of approximately 1.25 m from the Northeast to the Southwest corner of the property.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal right-of-ways:

### **Beech Street:**

- 203 mm diameter PVC watermain; and
- 375 mm diameter concrete combined sewer, tributary to the Preston Street Trunk.

### **Norman Street:**

- 152 mm diameter PVC watermain; and
- 675 mm diameter concrete combined sewer, tributary to the Preston Street Trunk.

## 1.2 Required Permits / Approvals

The proposed development is subject to the site plan control approval process. The City of Ottawa must approve the engineering design drawings and reports prior to the issuance of site plan control.

The proposed development is will outlet to a combined sewer. As a result, the Ministry of the Environment, Conservation and Parks (MOECP) will require an Environmental Compliance Application (ECA) to be submitted under the Direct Submission process.

### **1.3 Pre-consultation**

Pre-consultation correspondence, along with the servicing guidelines checklist, is located in ***Appendix A***.

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## 2.0 GUIDELINES, PREVIOUS STUDIES, AND REPORTS

### 2.1 Existing Studies, Guidelines, and Reports

The following studies were utilized in the preparation of this report:

- **Ottawa Sewer Design Guidelines,**  
City of Ottawa, *SDG002*, October 2012.  
**(City Standards)**
  - **Technical Bulletin ISTB-2018-01**  
City of Ottawa, March 21, 2018.  
**(ISTB-2018-01)**
  - **Technical Bulletin ISTB-2018-03**  
City of Ottawa, March 21, 2018.  
**(ISTB-2018-03)**
- **Ottawa Design Guidelines – Water Distribution**  
City of Ottawa, July 2010.  
**(Water Supply Guidelines)**
  - **Technical Bulletin ISD-2010-2**  
City of Ottawa, December 15, 2010.  
**(ISD-2010-2)**
  - **Technical Bulletin ISDTB-2014-02**  
City of Ottawa, May 27, 2014.  
**(ISDTB-2014-02)**
  - **Technical Bulletin ISDTB-2018-02**  
City of Ottawa, March 21, 2018.  
**(ISDTB-2018-02)**
- **Design Guidelines for Sewage Works,**  
Ministry of the Environment, 2008.  
**(MOE Design Guidelines)**
- **Stormwater Planning and Design Manual,**  
Ministry of the Environment, March 2003.  
**(SWMP Design Manual)**
- **Ontario Building Code Compendium**  
Ministry of Municipal Affairs and Housing Building Development Branch,  
January 1, 2010 Update.  
**(OBC)**



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- **Geotechnical Investigation**  
Paterson Group, PG4430-1 Revision 1, February 8, 2019.  
***(Geotechnical Report)***

### 3.0 WATER SUPPLY SERVICING

#### 3.1 Existing Water Supply Services

The subject property lies within the City of Ottawa 1W pressure zone, as shown by the Water Distribution System map, included in **Appendix B**. A local 203 mm diameter watermain exists within the Beech Street right-of-way and a local 152 mm diameter watermain exists within the Norman Street right-of way.

#### 3.2 Water Supply Servicing Design

It is proposed that the development be serviced via a 150 mm diameter water service, to be confirmed by the mechanical engineering, connecting to the existing 203 mm diameter watermain located within the Beech Street right-of-way. Refer to drawing **SSP-1**, accompanying this report for a detailed site servicing layout.

**Table 1**, below, summarizes the **Water Supply Guidelines** employed in the preparation of the water demand estimate.

**Table 1**  
**Water Supply Design Criteria**

Design Parameter	Value
Residential 1 Bedroom Apartment	1.4 P/unit
Residential 2 Bedroom Apartment	2.1 P/unit
Residential Average Daily Demand	280 L/d/P
Residential Maximum Daily Demand	4.9 x Average Daily **
Residential Maximum Hourly	7.4 x Average Daily **
Commercial Retail	5.0 L/m <sup>2</sup> /d
Commercial Maximum Daily Demand	1.5 x avg. day
Commercial Maximum Hour Demand	1.8 x max. day
Minimum Watermain Size	150mm diameter
Minimum Depth of Cover	2.4m from top of watermain to finished grade
During normal operating conditions desired operating pressure is within	350 kPa and 480 kPa
During normal operating conditions pressure must not drop below	275 kPa
During normal operating conditions pressure must not exceed	552 kPa
During fire flow operating pressure must not drop below	140k Pa

\*Daily average based on Appendix 4-A from **Water Supply Guidelines**

\*\* Residential Max. Daily and Max. Hourly peaking factors per MOE Guidelines for Drinking-Water Systems Table 3-3 for 0 to 500 persons.

-Table updated to reflect ISD-2010-2

**Table 2**, below, summarizes the estimated water supply demand and boundary conditions for the proposed development based on the development statistics provided by RLA Architecture.

**Table 2**  
**Water Demand and Boundary Conditions**

Design Parameter	Proposed Demand <sup>1</sup> (L/min)	Boundary Condition <sup>2</sup> Connection 1 (m H <sub>2</sub> O / kPa)
Average Daily Demand	17.4	114.9 / 520.6
Max Day + Fire Flow	79.0 + 10,000 = 10,079.0	104.0 / 446.1
Peak Hour	120.2	107.3 / 413.7
1) Water demand calculation per <b>Water Supply Guidelines</b> . See <b>Appendix B</b> for detailed calculations. 2) Boundary conditions supplied by the City of Ottawa for the demands indicated in the correspondence; assumed ground elevation 61.8 m. See <b>Appendix B for detailed calculations</b> .		

The City of Ottawa was contacted to obtain boundary conditions associated with the estimated water demand, as indicated in the boundary request correspondence included in **Appendix B**.

The City provided both the anticipated minimum and maximum water pressures, as well as, the estimated water pressure during fire flow demand for the demands as indicated by the correspondence in **Appendix B**. The minimum and maximum pressures fall within the required range identified in **Table 1**.

The required fire flow was estimated in accordance with City of Ottawa Technical Bulletin **ISTB-2018-02**; the resulting highest flows for each building type were sent to the City of Ottawa for boundary conditions. The following parameters were assumed:

- Type of construction – Ordinary Construction;
- Occupancy type – Limited combustible; and
- Sprinkler Protection – Sprinklered system

The following assumptions resulted in a fire flow estimation of **10,000 L/s**. Calculation sheets per the **ISTB-2018-02** can be found in **Appendix B**.

### 3.3 Water Supply Conclusion

A 150 mm diameter connection will service the development by connecting to the existing 203 mm diameter watermain within Beech Street.

The estimated water demand was submitted to the City of Ottawa for establishing boundary conditions. The City provided both the anticipated minimum and maximum water pressures, as well as, the estimated water pressure during fire flow. The minimum and maximum pressures fall within the required range identified in **Table 1**.

DSEL employed a daily consumption rate of 280 L/person/day to align with the revised wastewater rates identified by City of Ottawa Technical Bulletin **ISTB-2018-03**. As a result, DSEL is submitting for a deviation from the **Water Supply Guidelines**.

## 4.0 WASTEWATER SERVICING

### 4.1 Existing Wastewater Services

The subject site lies within the Preston Street Trunk catchment area, as shown by the City sewer mapping included in **Appendix C**. An existing 375 mm diameter combined sanitary sewer within the Beech Street right-of-way, and a 675 mm diameter combined sewer within Norman Street right-of-way are available to service the proposed development.

### 4.2 Wastewater Design

It is proposed that the development be serviced via a 200 mm diameter sanitary service to the existing 375 mm diameter combined sanitary sewer located within the Beech Street right-of-way. Refer to drawing **SSP-1**, accompanying this report, for a detailed site servicing layout.

**Table 3**, below, summarizes the **City Standards** employed in the design of the proposed wastewater sewer system.

**Table 3**  
**Wastewater Design Criteria**

Design Parameter	Value
Residential 1 Bedroom Apartment	1.4 P/unit
Residential 2 Bedroom Apartment	2.1 P/unit
Average Daily Demand	280 L/d/per
Peaking Factor	Harmon's Peaking Factor. Max 4.0, Min 2.0 Harmon Correction Factor 0.8
Infiltration and Inflow Allowance	0.33 L/s/ha
Commercial Floor/Amenity Space	5.0 L/m <sup>2</sup> /d
Nursing/Rest Homes	450 L/Bed/d
Housekeeping Facilities	275 L/P/d
Dining Room/Kitchen	115 L/9.3m <sup>3</sup> /d
Office Space	75 L/9.3m <sup>3</sup> /d
Commercial Peaking Factor	1.0
Sanitary sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} AR^{2/3} S^{1/2}$
Minimum Sewer Size	200 mm diameter
Minimum Manning's 'n'	0.013
Minimum Depth of Cover	2.5 m from crown of sewer to grade
Minimum Full Flowing Velocity	0.6 m/s
Maximum Full Flowing Velocity	3.0 m/s

*Extracted from Sections 4 and 6 of the City of Ottawa Sewer Design Guidelines, October 2012 and Technical Bulletin ISTB-2018-03.*

**Table 4**, below, demonstrates the estimated peak flow from the proposed development based on the development statistics provided by RLA Architecture. See **Appendix C** for associated calculations.

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**Table 4**  
**Summary of Estimated Peak Wastewater Flow**

<b>Design Parameter</b>	<b>Total Flow (L/s)</b>
Estimated Average Dry Weather Flow	0.33
Estimated Peak Dry Weather Flow	1.00
Estimated Peak Wet Weather Flow	<b>1.03</b>

The peak wet weather sanitary flow based on the Site Plan dated February 5<sup>th</sup> 2019, prepared by RLA Architecture and included in **Drawings/Figures**, is **1.03 L/s**. The increase in wastewater discharge will be compensated for by a reduction in stormwater flow, as discussed in **Section 6.0** of the report.

### **4.3 Wastewater Servicing Conclusions**

The subject site lies within the Preston Street Trunk catchment area. It is proposed that the development be serviced via a 200 mm diameter sanitary service to the existing 375 mm diameter combined sewer within Beech Street, tributary to the Preston Street Trunk.

As discussed in **Section 6.0** of the report, there will be a net decrease in total stormwater and sanitary flow to the existing combined sewer.

The proposed wastewater design conforms to all relevant **City Standards**.

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## 5.0 STORMWATER MANAGEMENT

### 5.1 Existing Stormwater Services

Stormwater runoff from the subject property is tributary to the City of Ottawa combined sewer system and is located within the Ottawa Central sub-watershed. As such, approvals for proposed development within this area are under the approval authority of the City of Ottawa.

Flows that influence the watershed in which the subject property is located are further reviewed by the principal authority. The subject property is located within the Ottawa River watershed, and is therefore subject to review by the Rideau Valley Conservation Authority (RVCA). Consultation with the RVCA is located in **Appendix A**.

The development proposed to retain the existing parking area and house within the 75 Norman Street subject site. The existing one-story garage is to be removed and replaced with surface parking. Existing drainage patterns within 75 Norman Street are proposed to be maintained. Based on topography and City of Ottawa 1K mapping, stormwater is directed overland East of the subject site towards Rochester Street. Please refer to drawing **FIG-1** within **Appendix D**.

Existing drainage within the 70 Beech Street subject site indicate that stormwater is directed towards the combined sewers within the Beech Street right-of-way.

It was assumed that the existing development contained no stormwater management controls for flow attenuation. The estimated combined pre-development peak flows directed towards the Beech Street right-of-way for the 2, 5, and 100-year storm events are summarized in **Table 5**, below:

**Table 5**  
**Summary of Existing Peak Storm Flow Rates (Beech Street)**

City of Ottawa Design Storm	Estimated Peak Flow Rate (L/s)
2-year	19.1
5-year	26.1
100-year	56.1

### 5.2 Post-development Stormwater Management Target

Stormwater management requirements for the proposed development were reviewed with the City of Ottawa, generating the following requirements for the proposed development:

- Meet a total allowable combined release rate based on a Rational Method Coefficient of 0.40, employing the City of Ottawa IDF parameters for a 2-year storm with a time of concentration equal to or greater than 10 minutes;

- Attenuate all storms up to and including the City of Ottawa 100-year design event on site;
- Quality controls are not required for the proposed development due to the combined sewer outlet; correspondence with the RVCA is included in **Appendix A**.

Based on the above stated requirements, the allowable stormwater release rate for the proposed development is **7.0 L/s**. As discussed in **Section 6.0** of this report and outlined in **Table 7**, there is a net reduction in combined sanitary and stormwater flow rates to the existing combined sewer from the existing condition.

### 5.3 Proposed Stormwater Management System

It is proposed that the stormwater outlet from the development will be to the existing 375 mm diameter combined sewer within Beech Street via a 250 mm diameter storm service. Refer to drawing **SSP-1**, accompanying this report, for a detailed site servicing layout.

To meet the stormwater objectives the proposed development will attenuate flow attenuation via an internal cistern. Detailed design of the cistern will be completed by the architect and the mechanical engineer.

Stormwater runoff collected from the roof area will be directed to a **40.0 m<sup>3</sup>** internal stormwater cistern. Cistern flow will be pumped to a maximum release rate of **1.3 L/s** via the internal mechanical system and will outlet to the municipal combined sewer within Beech Street, as shown by **SSP-1**. Foundation drains are to be connected downstream of any cistern controls. Detailed calculations are located in **Appendix D**.

Based on the Geotechnical Investigation (**Geotechnical Report**), post-development groundwater flows will be less than **25,000 L/day**, with peak periods noted after rain events. This groundwater infiltration was included within the total post-development site release rate.

**Table 6**, below, summarizes post-development flow rates based on the proposed Site Plan and **Geotechnical Report**.

**Table 6  
Stormwater Flow Rate Summary**

Control Area	5-Year Release Rate	5-Year Storage	100-Year Release Rate	100-Year Storage Required
	(L/s)	(m <sup>3</sup> )	(L/s)	(m <sup>3</sup> )
Unattenuated Areas	2.5	0.0	5.5	0.0
Attenuated Areas	0.7	19.8	1.3	37.2
Ground Water Infiltration*	0.3	0.0	0.3	0.0
<b>Total</b>	<b>3.5</b>	<b>19.8</b>	<b>7.0</b>	<b>37.2</b>
<i>*Including 25,000 L/day groundwater infiltration as indicated by (Geotechnical Report)</i>				

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It is calculated that **37.2 m<sup>3</sup>** of storage will be required on site to attenuate flow to the established release rate of **7.0 L/s**. Storage will be provided via a **40.0 m<sup>3</sup>** internal cistern.

As discussed in **Section 6.0** of this report and outlined in **Table 7**, there is a net reduction in total sanitary and stormwater flow rates to the existing combined sewer proposed.

#### **5.4 Stormwater Quality Control**

Quality controls are not required for the proposed development due to the combined sewer outlet; correspondence with the RVCA is included in **Appendix A**.

#### **5.5 Stormwater Servicing Conclusions**

Post development stormwater runoff will be required to be restricted to the allowable target release rate for storm events up to and including the 100-year storm, in accordance with **City Standards**.

Based on the above stated requirements, the allowable release rate for the proposed development is **7.0 L/s**. It is estimated that **37.2 m<sup>3</sup>** of storage is required to meet this release rate and the storage will be provided via a **40.0 m<sup>3</sup>** internal cistern.

Quality controls are not required for the proposed development due to the combined sewer outlet.

The proposed stormwater design conforms to all relevant **City Standards** and Policies for approval.



## 6.0 COMBINED SEWER SYSTEM FLOW

Under existing conditions, it is assumed that the site contains no stormwater management system for flow attenuation. Therefore, the pre-development combined flow during the 2-year storm event was estimated to be approximately **19.2 L/s**.

The post-development combined flow for all storms up to and including a 100-year event will be limited to **8.0 L/s**. This value includes the peak dry weather sanitary flow and both controlled and uncontrolled flows directed towards the municipal infrastructure.

Based on the current Site Plan, the development proposes to control stormwater to an allowable release rate of **7.0 L/s**, calculated based on the design parameters outlined in **Section 5.2**.

**Table 7**, below, summarizes the allowable and proposed release rates from the development.

**Table 7**  
**Summary of Existing and Proposed Release Rates**

City of Ottawa Design Storm	Existing Peak Flow Rate (L/s)	Proposed Peak Flow Rate (L/s)
Wastewater	0.1	1.0
Stormwater*	19.1	7.0
<b>Combined</b>	19.2	8.0
<i>*Including 25,000 L/day groundwater infiltration as indicated by (Geotechnical Report)</i>		

As indicated by **Table 7**, above, based on the proposed flow rate of **8.0 L/s**, the post-development combined flow will result in a net reduction to the existing combined sewer by approximately **11.2 L/s** or **58%** during a 2-year storm event.

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## **7.0 UTILITIES**

Gas and Hydro services currently exist within the Beech Street and Norman Street right-of-ways. Utility servicing will be coordinated with the individual utility companies prior to site development.

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## 8.0 EROSION AND SEDIMENT CONTROL

Soil erosion occurs naturally and is a function of soil type, climate and topography. During construction the extent of erosion losses is exaggerated due to the removal of vegetation and the top layer of soil becoming agitated.

Prior to topsoil stripping, earthworks or underground construction, erosion and sediment controls will be implemented and will be maintained throughout construction.

Silt fence will be installed around the perimeter of the site and will be cleaned and maintained throughout construction. Silt fence will remain in place until the working areas have been stabilized and re-vegetated.

Catch basins will have SILTSACKS or an approved equivalent installed under the grate during construction to protect from silt entering the storm sewer system.

A mud mat will be installed at the construction access in order to prevent mud tracking onto adjacent roads.

Erosion and sediment controls must be in place during construction. The following recommendations to the contractor will be included in contract documents:

- Limit extent of exposed soils at any given time;
- Re-vegetate exposed areas as soon as possible;
- Minimize the area to be cleared and grubbed;
- Protect exposed slopes with plastic or synthetic mulches;
- Install silt fence to prevent sediment from entering existing ditches;
- No refueling or cleaning of equipment near existing watercourses;
- Provide sediment traps and basins during dewatering;
- Install filter cloth between catch basins and frames;
- Plan construction at proper time to avoid flooding; and
- Establish material stockpiles away from watercourses, so that barriers and filters may be installed.

The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:

- Verification that water is not flowing under silt barriers; and
- Clean and change filter cloth at catch basins.

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## 9.0 CONCLUSION AND RECOMMENDATIONS

David Schaeffer Engineering Ltd. (DSEL) has been retained by The Properties Group to prepare a Servicing and Stormwater Management report in support of the application for Site Plan Control (SPC) at 70 Beech Street & 75 Norman Street. The preceding report outlines the following:

- Based on boundary conditions provided by the City, the existing municipal water infrastructure is capable of providing the proposed development with water within the City's required pressure range;
- The FUS method for estimating fire flow indicated **10,000 L/min** is required for the mixed-use commercial/residential apartment building. As indicated by the boundary conditions provided by the City, the municipal system is capable of providing the required flow;
- The proposed development will be required to attenuate post development storm and sanitary flows to an equivalent combined release rate of **8.0 L/s**. The combined release rate of sanitary and stormwater flow from the proposed site provides an approximately **58%** reduction in flow into the combined sewer within Beech Street;
- The proposed development is estimated to have a peak wet weather flow of **1.03 L/s**.
- Based on City of Ottawa design criteria, the post development storm allowable release rate was established as **7.0 L/s**;
- It is proposed that stormwater objective be met through storm water retention via cistern storage, it is calculated that **37.2 m<sup>3</sup>** of onsite storage will be required to attenuate flow to the established release rate above, **40.0 m<sup>3</sup>** is provided;
- Quality controls are not required for the proposed development due to the combined sewer outlet, correspondence with the RVCA is included in **Appendix A**;
- The development proposes to outlet to a combined sewer, as a result, the Ministry of the Environment, Conservation and Parks (MOECP) will require an Environmental Compliance Application (ECA) to be submitted under the Direct Submission process.

---

Prepared by,  
**David Schaeffer Engineering Ltd.**

Reviewed by,  
**David Schaeffer Engineering Ltd.**

*C. Kelly*



Per: Charlotte M. Kelly, EIT.

Per: Robert D. Freel, P.Eng.



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***APPENDIX A***

***Pre-Consultation***

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# DEVELOPMENT SERVICING STUDY CHECKLIST

18-1015

04/03/2019

4.1 General Content		
<input type="checkbox"/>	Executive Summary (for larger reports only).	N/A
<input checked="" type="checkbox"/>	Date and revision number of the report.	Report Cover Sheet
<input checked="" type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development.	Drawings/Figures
<input checked="" type="checkbox"/>	Plan showing the site and location of all existing services.	Figure 1, EX-1
<input checked="" type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Section 1.0
<input checked="" type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies.	Section 1.3
<input checked="" type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Section 2.1
<input checked="" type="checkbox"/>	Statement of objectives and servicing criteria.	Section 1.0
<input checked="" type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area.	Sections 3.1, 4.1, 5.1, EX-1
<input type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	N/A
<input checked="" type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	GP-1
<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
<input type="checkbox"/>	Proposed phasing of the development, if applicable.	N/A
<input type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing.	N/A
<input checked="" type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: -Metric scale -North arrow (including construction North) -Key plan -Name and contact information of applicant and property owner -Property limits including bearings and dimensions -Existing and proposed structures and parking areas -Easements, road widening and rights-of-way -Adjacent street names	SP-1
4.2 Development Servicing Report: Water		
<input type="checkbox"/>	Confirm consistency with Master Servicing Study, if available	N/A
<input checked="" type="checkbox"/>	Availability of public infrastructure to service proposed development	Section 3.1
<input checked="" type="checkbox"/>	Identification of system constraints	Section 3.1
<input checked="" type="checkbox"/>	Identify boundary conditions	Section 3.1, 3.2
<input checked="" type="checkbox"/>	Confirmation of adequate domestic supply and pressure	Section 3.3

<input checked="" type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Section 3.2
<input type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	N/A
<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
<input type="checkbox"/>	Address reliability requirements such as appropriate location of shut-off valves	N/A
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification	N/A
<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Section 3.2, 3.3
<input type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	N/A
<input type="checkbox"/>	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Section 3.2
<input type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

#### 4.3 Development Servicing Report: Wastewater

<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Section 4.2
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 4.1
<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Section 4.2
<input checked="" type="checkbox"/>	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Section 4.2, Appendix C
<input checked="" type="checkbox"/>	Description of proposed sewer network including sewers, pumping stations, and forcemains.	Section 4.2
<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A

<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations such as contamination, corrosive environment etc.	N/A

#### 4.4 Development Servicing Report: Stormwater Checklist

<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 5.1
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	Section 5.1, 7.0, Appendix D
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Drawings/Figures
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 5.2, 5.4, 5.5
<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 5.2, 5.4, 5.5
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information	Section 5.3
<input type="checkbox"/>	Set-back from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	N/A
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Appendix A
<input type="checkbox"/>	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).	Section 5.3
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
<input checked="" type="checkbox"/>	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 5.1, 5.3
<input type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	N/A
<input type="checkbox"/>	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	N/A
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/A
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses	N/A
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A

<input checked="" type="checkbox"/>	Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 5.3
<input type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	N/A
<input type="checkbox"/>	Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A
<input checked="" type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 8.0
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

**4.5 Approval and Permit Requirements: Checklist**

<input checked="" type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement ct. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	Section 1.2
<input type="checkbox"/>	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A
<input type="checkbox"/>	Changes to Municipal Drains.	N/A
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A

**4.6 Conclusion Checklist**

<input checked="" type="checkbox"/>	Clearly stated conclusions and recommendations	Section 9.0
<input type="checkbox"/>	Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	
<input type="checkbox"/>	All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	

## Charlotte Kelly

---

**From:** Jamie Batchelor <jamie.batchelor@rvca.ca>  
**Sent:** February 22, 2019 2:06 PM  
**To:** Charlotte Kelly  
**Cc:** Alison Gosling  
**Subject:** RE: 70 Beech Street - Quality Control Requirement

Good Afternoon Charlotte,

I can confirm that if the stormwater is outletting to a combined sewer, then onsite water quality controls are not required save and except best management practices.

Jamie Batchelor, MCIP,RPP  
Planner, ext. 1191  
[jamie.batchelor@rvca.ca](mailto:jamie.batchelor@rvca.ca)



3889 Rideau Valley Drive  
PO Box 599, Manotick ON K4M 1A5  
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**From:** Charlotte Kelly <CKelly@dsel.ca>  
**Sent:** Friday, February 22, 2019 12:08 PM  
**To:** Jamie Batchelor <jamie.batchelor@rvca.ca>  
**Cc:** Alison Gosling <AGosling@dsel.ca>  
**Subject:** 70 Beech Street - Quality Control Requirement

Good afternoon Jamie,

We wanted to touch base with you regarding a development at 70 Beech Street / 75 Norman Street. The development involves the construction of a 6-storey residential and commercial building occupying the entirety of the 70 Beech parcel. The existing site fronting Beech consists of a paved surface parking lot and auto-mechanic shop. The existing property fronting Norman consist of a detached home with a detached garage and a paved rear yard parking area. The house is to remain however the garage is to be converted to more paved parking. Please refer to the figure below for the existing property conditions and the attached site plan for the proposed site layout.

The development proposes to outlet to the existing combined sewer within Beech Street.

It is anticipated that stormwater quality controls are not required as the development proposes to outlet to the combined sewer which will ultimately be treated at ROPEC. Can you please confirm?



Thanks,

Charlotte Kelly, E.I.T.  
Project Coordinator / Junior Designer

## **DSEL**

**david schaeffer engineering ltd.**

120 Iber Road, Unit 103  
Stittsville, ON K2S 1E9

**phone:** (613) 836-0856 ext.511  
**email:** [ckelly@dsel.ca](mailto:ckelly@dsel.ca)

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## **70 Beech Street (75 Norman St)**

Meeting Summary Notes  
Nov. 3, 2017, Ottawa City Hall  
Revised, November 15, 2017

### **Attendees:**

- Brian Lahey (Owner, The Properties Group)
- Rod Lahey (Architect, RLA Architecture)
- Matt McElligott (Agent, Fotenn Consultants)
- Richard Buchanan (Project Manager, City of Ottawa)
- Christopher Moise (Urban Designer, Architect, City of Ottawa)
- Tracey Scaramozzino (File Lead, Planner, City of Ottawa)
- Michael Powell, Eric Darwin (Dalhousie Community Association) – not in attendance but will receive a copy of the minutes.

### **Issue of Discussion:**

- Construction of new 6-storey mixed-use building at 70 Beech Street, with commercial on the ground floor, offices on the second floor, residential on floors 3-6.
- [Demolition of dwelling at 75 Norman Street to provide 13 surface parking spaces for the development at 70 Beech. (this part of the proposal has since been removed from the proposal)].
- Existing hydro wires will be relocated to the opposite side of the hydro poles to provide adequate 5m clearance for the devt.
- Roof-top amenity space will be provided to meet the zoning provisions.
- A minor re-zoning application will be requested to deal with several zoning deficiencies. These will be further reviewed once the site/bldg. design is further advanced.
- Garbage and bikes are located within the building

---

### **1. Official Plan**

- a. Designated “Mixed Use” in the Official Plan.
- b. Beech Street, in this area is designated “Main Street” in Preston-Carling District Secondary Plan; the lands to the rear, fronting along Norman Street are designated “Mixed-Use Neighbourhood”

*“Preston Street and Beech Street will be animated and active Mainstreets with generous pedestrian zones for walking and seasonal outdoor patios, ample bicycle parking, strategic on street parking, special furnishing, street trees and planting beds. Bus stops and shelters will be located along Preston Street.”*



Secondary Plan: Main Street in raspberry colour along Beech and Preston; mixed-use neighbourhood for sites along Norman.

- Main Street / Rues principales
- Mixed-Use Blocks / Îlots d'utilisations polyvalentes
- Mixed-Use Neighbourhood / Quartier polyvalent



Secondary Plan: Heights of 6-storeys along Beech and Preston; 4-storeys backing onto the site, fronting onto Norman.

- Emplacement possible d'un bâtiment de grande hauteur
- HEIGHT / HAUTEUR**  
 MAXIMUM BUILDING HEIGHTS / HAUTEURS MAXIMALES DES IMMEUBLES
- 4 storeys / étages
  - 6 storeys / étages
  - 9 storeys / étages



## **2. Zoning Information**

- a. TM [86]: for 70 Beech

## **3. Infrastructure/Servicing**

- a. Standard Requirements: storm, sanitary, MOE application for combined services, ECA, noise study, Phase 1 and 2 ESA, eligible for Brownfield Contamination – as they are not the same owner that caused the contamination
- b. Applicants consultant: Carlos Da Silva, Paterson Group

## **4. Initial Planning/Design Comments**

- a. Remove surface parking (applicant has agreed to do this)
  - b. We will need to further review the proposed variances once the site/building design have been revised.
  - c. The site is subject to the Urban Design Review Panel (UDRP)
  - d. Design comments will be provided as the proposal evolves
  - e. Cash-in-lieu of parkland is req'd – to a max of 10% of the land value.
  - f. The proposed variances will be further reviewed once the site/bldg. design is further advanced.
- Point of Note: Upon reflection after the meeting, City staff would like to advise the Applicants that the proposed rear yard setback of 1.5m rather than 7.5m is of concern to staff.

## **5. Transportation**

- a. No transportation issues, as there are no longer any parking spaces being provided and bicycle parking is being provided.

## **6. Waste Collection**

- a. Waste management will be controlled within an interior garbage room.
- b. Applicant to confirm if the residential units will be on City or Private garbage collection. The Commercial and Offices uses will both be on private collection.

## **7. Process/Required Applications**

- a. Site Plan Control Application, Manager Approval, Public Consultation
- b. Minor Re-zoning to deal with zoning deficiencies (instead of going to the Committee of Adjustment)

## **8. General Information**

- a. Rod Lahey will contact the Dalhousie Community Association (Michael Powell and Eric Darwin) to discuss the project. Tracey Scaramozzino is willing to attend if requested.
- b. Applicant to also speak to Councillor McKenney
- c. The Applicant will spend some time to revise the application before being submitted.



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***APPENDIX B***

***Water Supply***

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Water Demand Design Flows per Unit Count  
City of Ottawa - Water Distribution Guidelines, July 2010



**Domestic Demand**

Type of Housing	Per / Unit	Units	Pop
Single Family	3.4	-	0
Semi-detached	2.7	-	0
Townhouse	2.7	-	0
Apartment			0
Bachelor	1.4	-	0
1 Bedroom	1.4	30	42
2 Bedroom	2.1	18	38
3 Bedroom	3.1	-	0
Average	1.8	-	0

	Pop	Avg. Daily*		Max Day		Peak Hour*	
		m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min
<b>Total Domestic Demand</b>	80	22.4	15.6	109.8	76.2	165.8	115.1

**Institutional / Commercial / Industrial Demand**

Property Type	Unit Rate	Units	Avg. Daily		Max Day		Peak Hour	
			m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min	m <sup>3</sup> /d	L/min
Commercial floor space	5.0 L/m <sup>2</sup> /d	538	2.69	1.9	4.0	2.8	7.3	5.0
Office	75 L/9.3m <sup>2</sup> /d	-	0.00	0.0	0.0	0.0	0.0	0.0
<b>Total I/CI Demand</b>			2.7	1.9	4.0	2.8	7.3	5.0
<b>Total Demand</b>			<b>25.1</b>	<b>17.4</b>	<b>113.8</b>	<b>79.0</b>	<b>173.0</b>	<b>120.2</b>

Residential Maximum Daily Demand = 4.9 x Average Daily \*

Residential Maximum Hourly Demand = 7.4 x Average Daily \*

## Fire Flow Estimation per Fire Underwriters Survey

Water Supply For Public Fire Protection - 1999



### Fire Flow Required

#### 1. Base Requirement

$$F = 220C\sqrt{A}$$

L/min      Where *F* is the fire flow, *C* is the Type of construction and *A* is the Total floor area

Type of Construction:      Ordinary Construction

**C**      1      Type of Construction Coefficient per FUS Part II, Section 1  
**A**      2684.0      m<sup>2</sup>      Total floor area based on FUS Part II section 1

<b>Fire Flow</b>	11397.6 L/min
	<b>11000.0 L/min</b> rounded to the nearest 1,000 L/min

### Adjustments

#### 2. Reduction for Occupancy Type

Limited Combustible      -15%

<b>Fire Flow</b>	<b>9350.0 L/min</b>
------------------	---------------------

#### 3. Reduction for Sprinkler Protection

Sprinklered - Supervised      -50%

<b>Reduction</b>	<b>-4675 L/min</b>
------------------	--------------------

#### 4. Increase for Separation Distance

Cons. of Exposed Wall	S.D	Lw	Ha	LH	EC	
<b>N</b> Non-Combustible	>45m		0	0	0	0%
<b>S</b> Wood Frame	10.1m-20m		20	2	40	13%
<b>E</b> Wood Frame	0m-3m		28	1	28	22%
<b>W</b> Wood Frame	0m-3m		29	2	58	23%
	<b>% Increase</b>					<b>58%</b> value not to exceed 75%

<b>Increase</b>	<b>5423.0 L/min</b>
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Lw = Length of the Exposed Wall

Ha = number of storeys of the adjacent structure. Max 5 stories

LH = Length-height factor of exposed wall. Value rounded up.

EC = Exposure Charge

### Total Fire Flow

<b>Fire Flow</b>	10098.0 L/min	fire flow not to exceed 45,000 L/min nor be less than 2,000 L/min per FUS Section
	<b>10000.0 L/min</b>	rounded to the nearest 1,000 L/min

#### Notes:

-Type of construction, Occupancy Type and Sprinkler Protection information provided by \_\_\_\_\_.

-Calculations based on Fire Underwriters Survey - Part II

**Boundary Conditions Unit Conversion**

Grnd Elev            61.83

	<b>m H<sub>2</sub>O</b>	<b>PSI</b>	<b>kPa</b>			
Avg. Day	<b>114.9</b>	75.5	520.6			
Peak Hour	<b>107.3</b>	64.7	446.1			
Max Day + FF	<b>104.0</b>	60.0	413.7	Fire Flow	L/s	L/min
					167	10020





## Alison Gosling

---

**To:** Robert Freel  
**Subject:** RE: 70 Beech Street - Boundary Condition Request - 18-1015

---

**From:** [Richard.Buchanan@ottawa.ca](mailto:Richard.Buchanan@ottawa.ca)  
**Sent:** January 29, 2019 10:27 AM  
**To:** [RFreel@dsel.ca](mailto:RFreel@dsel.ca)  
**Subject:** FW: 70 Beech Street - Boundary Condition Request - 18-1015

Hi Bobby

The following are boundary conditions, HGL, for hydraulic analysis at 70 Beech (zone 1W) assumed to be connected to the 203mm on Beech St (see attached PDF for location).

Minimum HGL = 107.3m

Maximum HGL = 114.9m

MaxDay + FireFlow (167 L/s) = 104.0m

These are for current conditions and are based on computer model simulation.

*Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.*

### Richard Buchanan, CET

Project Manager, Development Approvals  
Planning, Infrastructure and Economic Development Department  
Planning & Growth Management Branch  
City of Ottawa | Ville d'Ottawa  
☎ 613.580.2424 ext./poste 27801  
[ottawa.ca/planning](http://ottawa.ca/planning) / [ottawa.ca/urbanisme](http://ottawa.ca/urbanisme)

---

**From:** Robert Freel <[RFreel@dsel.ca](mailto:RFreel@dsel.ca)>  
**Sent:** Wednesday, January 23, 2019 2:32 PM  
**To:** Buchanan, Richard <[Richard.Buchanan@ottawa.ca](mailto:Richard.Buchanan@ottawa.ca)>  
**Subject:** 70 Beech Street - Boundary Condition Request - 18-1015

Good afternoon Richard,

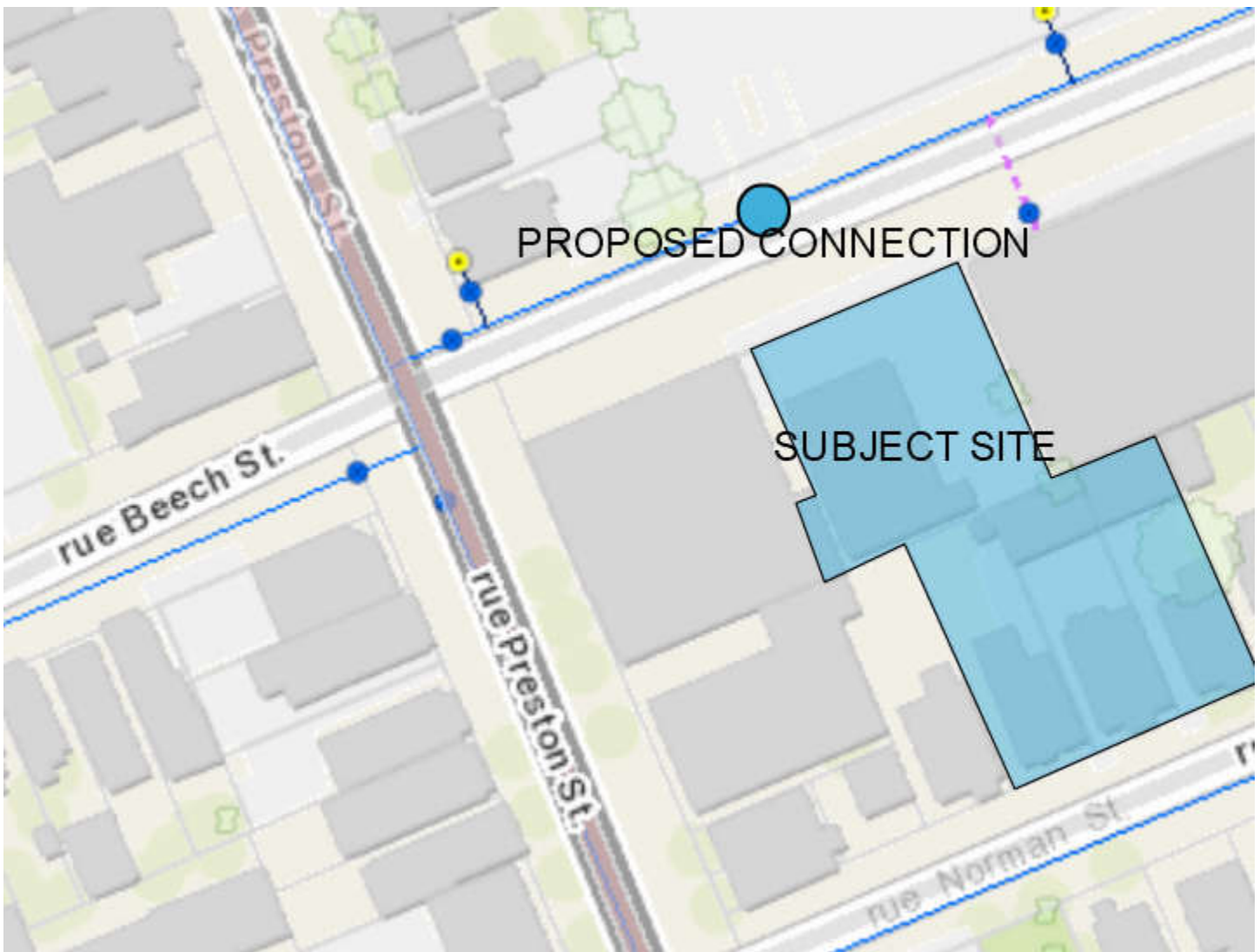
Hope you are surviving the snow.

We would like to request water boundary conditions for the proposed development at 70 Beech Street using the following proposed development demands:

1. Location of Service / Street Number: 70 Beech Street / 75 Norman Street
2. Type of development and the amount of fire flow required for the proposed development:
  - The development would include a **48 unit low rise condominium** with approximately **538 m<sup>2</sup>** of commercial/amenity space. Existing houses facing Norman Street are to remain.
  - It is anticipated that the development will have a single connection to be serviced from the existing 203mm diameter watermain within the Beech St. right-of-way, as shown by the attached map.
  - Fire demand based on Technical Bulletin ISTB-2018-02 has been used to calculate an estimate the max fire demand of **10,000 L/min**. Refer to the attached for detailed calculations.

	L/min	L/s
<b>Avg. Daily</b>	17.4	0.29
<b>Max Day</b>	79.0	1.32
<b>Peak Hour</b>	120.2	2.00

If you have any questions, please feel free to contact me.



Thank you,

Bobby Freel, P.Eng.  
Project Manager / Intermediate Designer

**DSEL**

**david schaeffer engineering ltd.**

120 Iber Road, Unit 103

Stittsville, ON K2S 1E9

**phone:** (613) 836-0856 ext.558

**cell:** (613) 314-7675

**email:** [rffreel@DSEL.ca](mailto:rffreel@DSEL.ca)

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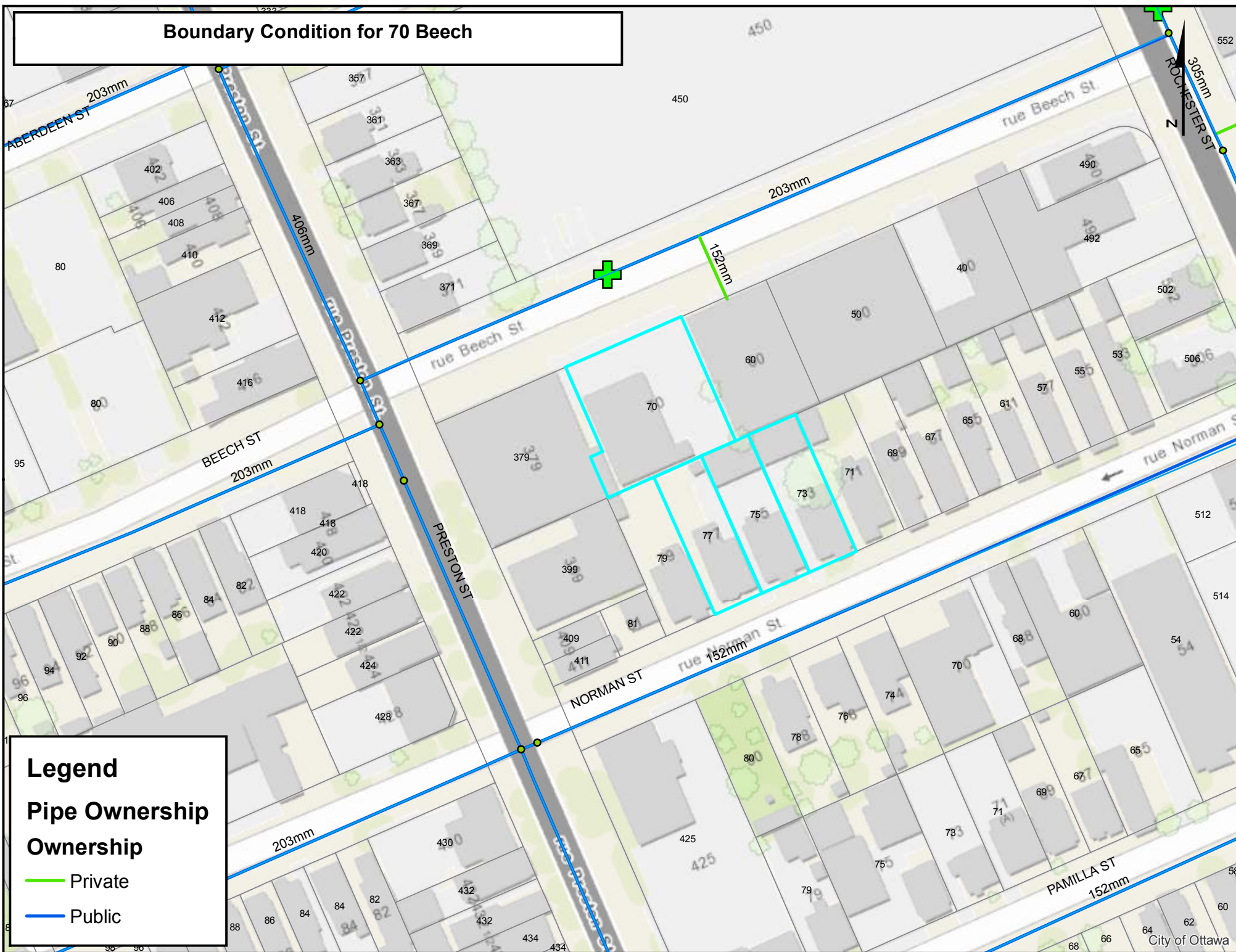
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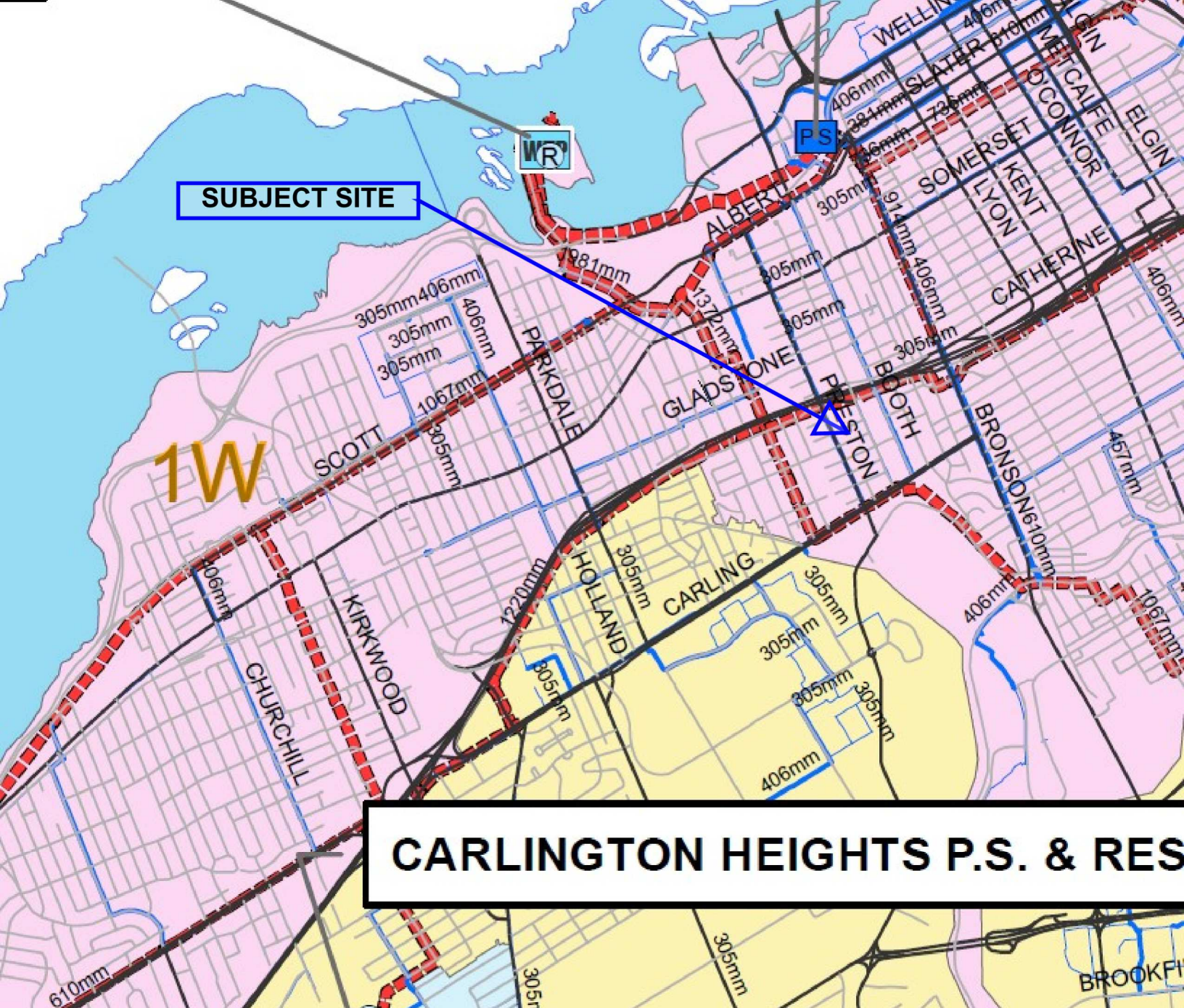


# Boundary Condition for 70 Beech



- Legend**
- Pipe Ownership**
- Private
  - Public





Water Distribution System Facilities & Feeder mains  
City of Ottawa





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***APPENDIX C***

***Wastewater Collection***

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Wastewater Design Flows per Unit Count  
City of Ottawa Sewer Design Guidelines, 2004



Site Area 0.083 ha

Extraneous Flow Allowances

Infiltration / Inflow 0.02 L/s

Domestic Contributions

Unit Type	Unit Rate	Units	Pop
Single Family	3.4		0
Semi-detached and duplex	2.7		0
Townhouse	2.7		0
Stacked Townhouse	2.3		0
Apartment			
Bachelor	1.4		0
1 Bedroom	1.4		0
2 Bedroom	2.1		0
3 Bedroom	3.1		0
Average	1.8		0

Total Pop 0

Average Domestic Flow 0.00 L/s

Peaking Factor 4.00

Peak Domestic Flow 0.00 L/s

Institutional / Commercial / Industrial Contributions

Property Type	Unit Rate	No. of Units	Avg Wastewater (L/s)
Commercial floor space*	5 L/m <sup>2</sup> /d	306	0.04
Hospitals	900 L/bed/d		0.00
School	70 L/student/d		0.00
Ex. Industrial - Light**	35,000 L/gross ha/d	-	0.00
Industrial - Light**	35,000 L/gross ha/d	-	0.00
Industrial - Heavy**	55,000 L/gross ha/d		0.00

Average I/C/I Flow 0.04

Peak Institutional / Commercial Flow 0.05

Peak Industrial Flow\*\* 0.00

Peak I/C/I Flow 0.05

\* assuming a 12 hour commercial operation

\*\* peak industrial flow per City of Ottawa Sewer Design Guidelines Appendix 4B

Total Estimated Average Dry Weather Flow Rate	0.04 L/s
Total Estimated Peak Dry Weather Flow Rate	0.05 L/s
Total Estimated Peak Wet Weather Flow Rate	0.08 L/s

Wastewater Design Flows per Unit Count  
City of Ottawa Sewer Design Guidelines, 2004



Site Area 0.083 ha

Extraneous Flow Allowances

Infiltration / Inflow (Dry)	0.00 L/s
Infiltration / Inflow (Wet)	0.02 L/s
Infiltration / Inflow (Total)	0.03 L/s

Domestic Contributions

Unit Type	Unit Rate	Units	Pop
Single Family	3.4		0
Semi-detached and duplex	2.7		0
Townhouse	2.7		0
Stacked Townhouse	2.3		0
Apartment			
Bachelor	1.4		0
1 Bedroom	1.4	30	42
2 Bedroom	2.1	18	38
3 Bedroom	3.1		0
Average	1.8		0

Total Pop 80

Average Domestic Flow 0.26 L/s

Peaking Factor 3.62

Peak Domestic Flow 0.94 L/s

Institutional / Commercial / Industrial Contributions

Property Type	Unit Rate	No. of Units	Avg Wastewater (L/s)
Commercial floor space*	5 L/m <sup>2</sup> /d	538	0.06
Hospitals	900 L/bed/d		0.00
School	70 L/student/d		0.00
Industrial - Light**	35,000 L/gross ha/d		0.00
Industrial - Heavy**	55,000 L/gross ha/d		0.00

Average I/C/I Flow 0.06

Peak Institutional / Commercial Flow 0.06

Peak Industrial Flow\*\* 0.00

Peak I/C/I Flow 0.06

\* assuming a 12 hour commercial operation

\*\* peak industrial flow per City of Ottawa Sewer Design Guidelines Appendix 4B

Total Estimated Average Dry Weather Flow Rate	0.33 L/s
Total Estimated Peak Dry Weather Flow Rate	1.00 L/s
Total Estimated Peak Wet Weather Flow Rate	1.03 L/s

# CITY OF OTTAWA-SANITARY TRUNK AND COLLECTION AREAS





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***APPENDIX D***

***Stormwater Management***

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**Estimated Peak Stormwater Flow Rate**  
City of Ottawa Sewer Design Guidelines, 2012



**Existing Drainage Characteristics From Internal Site**

<b>Area</b>	0.0825 ha
<b>C</b>	0.78 Rational Method runoff coefficient
<b>L</b>	30.34 m
<b>Up Elev</b>	62.21 m
<b>Dn Elev</b>	61.6 m
<b>Slope</b>	2.0 %
<b>Tc</b>	4.6 min

	<b>Imp.</b>	<b>GRAV</b>	<b>Total</b>
<b>Area</b>	0.033	0.049	0.082
<b>C</b>	0.9	0.7	0.78

1) Time of Concentration per Federal Aviation Administration

$$t_c = \frac{1.8(1.1 - C)L^{0.5}}{S^{0.333}}$$

tc, in minutes

C, rational method coefficient, (-)

L, length in ft

S, average watershed slope in %

**Estimated Peak Flow**

	<b>2-year</b>	<b>5-year</b>	<b>100-year</b>
<b>i</b>	107.0	146.0	251.1 mm/hr
<b>Q</b>	19.1	26.1	56.1 L/s

Stormwater - Proposed Development  
City of Ottawa Sewer Design Guidelines, 2012



Target Flow Rate

Area 0.08 ha  
C 0.40 Rational Method runoff coefficient  
 $t_c$  10.0 min

2-year

i 76.8 mm/hr  
Q 7.0 L/s

Target Long Tem Post-Development Groundwater

Q 25000 L/day  
Q 0.3 L/s

\*As per Geotechnical Report (PG4430-1) prepared by Paterson Group and dated February 8th, 2019.

Estimated Post Development Peak Flow from Unattenuated Areas

Total Area 0.012 ha  
C 0.75 Rational Method runoff coefficient

	Imp.	Perv.	Total
Area	0.009	0.003	0.012
C	0.9	0.2	0.75

$t_c$ (min)	5-year					100-year				
	i (mm/hr)	$Q_{actual}$ (L/s)	$Q_{release}$ (L/s)	$Q_{stored}$ (L/s)	$V_{stored}$ ( $m^3$ )	i (mm/hr)	$Q_{actual}$ (L/s)	$Q_{release}$ (L/s)	$Q_{stored}$ (L/s)	$V_{stored}$ ( $m^3$ )
10.0	104.2	2.5	2.5	0.0	0.0	178.6	5.5	5.5	0.0	0.0

Estimated Post Development Peak Flow from Attenuated Areas

Total Area 0.071 ha  
C 0.90 Rational Method runoff coefficient

	Imp.	Perv.	Total
Area	0.071	0.000	0.071
C	0.9	0.2	0.90

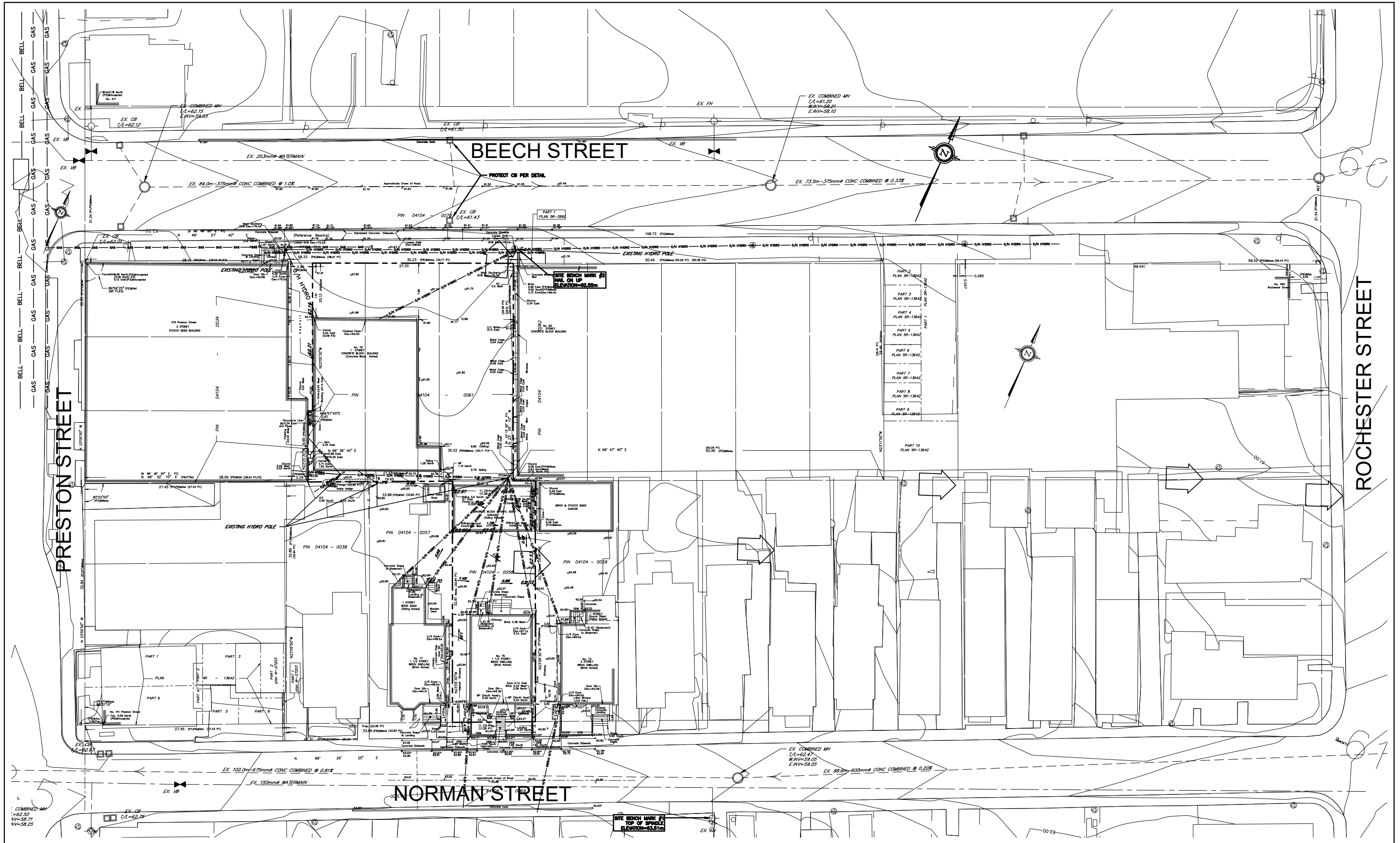
$t_c$ (min)	5-year					100-year				
	i (mm/hr)	$Q_{actual}$ (L/s)	$Q_{release}$ (L/s)	$Q_{stored}$ (L/s)	$V_{stored}$ ( $m^3$ )	i (mm/hr)	$Q_{actual}$ (L/s)	$Q_{release}$ (L/s)	$Q_{stored}$ (L/s)	$V_{stored}$ ( $m^3$ )
150	16.4	2.9	0.7	2.2	19.8	27.6	5.4	1.3	4.1	37.2
155	15.9	2.8	0.7	2.1	19.8	26.9	5.3	1.3	4.0	37.1
160	15.6	2.8	0.7	2.1	19.8	26.2	5.2	1.3	3.9	37.1
165	15.2	2.7	0.7	2.0	19.7	25.6	5.0	1.3	3.7	37.0
170	14.8	2.6	0.7	1.9	19.7	25.0	4.9	1.3	3.6	36.9
175	14.5	2.6	0.7	1.9	19.7	24.4	4.8	1.3	3.5	36.8
180	14.2	2.5	0.7	1.8	19.6	23.9	4.7	1.3	3.4	36.8
185	13.9	2.5	0.7	1.8	19.6	23.4	4.6	1.3	3.3	36.6
190	13.6	2.4	0.7	1.7	19.5	22.9	4.5	1.3	3.2	36.5
195	13.3	2.4	0.7	1.7	19.5	22.4	4.4	1.3	3.1	36.4
200	13.0	2.3	0.7	1.6	19.4	22.0	4.3	1.3	3.0	36.3
205	12.8	2.3	0.7	1.6	19.3	21.6	4.2	1.3	2.9	36.2
210	12.6	2.2	0.7	1.5	19.3	21.1	4.2	1.3	2.9	36.0
215	12.3	2.2	0.7	1.5	19.2	20.8	4.1	1.3	2.8	35.9
220	12.1	2.1	0.7	1.4	19.1	20.4	4.0	1.3	2.7	35.8
225	11.9	2.1	0.7	1.4	19.0	20.0	3.9	1.3	2.6	35.6
230	11.7	2.1	0.7	1.4	19.0	19.7	3.9	1.3	2.6	35.5
235	11.5	2.0	0.7	1.3	18.9	19.3	3.8	1.3	2.5	35.3
240	11.3	2.0	0.7	1.3	18.8	19.0	3.7	1.3	2.4	35.1
245	11.1	2.0	0.7	1.3	18.7	18.7	3.7	1.3	2.4	35.0
250	10.9	1.9	0.7	1.2	18.6	18.4	3.6	1.3	2.3	34.8

5-year  $Q_{attenuated}$  0.69 L/s  
5-year Max. Storage Required 19.8  $m^3$

100-year  $Q_{attenuated}$  1.30 L/s  
100-year Max. Storage Required 37.2  $m^3$

Summary of Release Rates and Storage Volumes

Control Area	5-Year Release Rate (L/s)	5-Year Storage ( $m^3$ )	100-Year Release Rate (L/s)	100-Year Storage ( $m^3$ )
Unattenuated Areas	2.5	0.0	5.5	0.0
Attenuated Areas	0.7	19.8	1.3	37.2
Ground Water Infiltration	0.3	0.0	0.3	0.0
<b>Total</b>	<b>3.5</b>	<b>19.8</b>	<b>7.0</b>	<b>37.2</b>



**DSEL**  
 david schaeffer engineering ltd  
 SMART SUBCONTRACTORS™

120 Iber Road Unit 103  
 Stittsville, Ontario, K2S 1E9  
 Tel. (613) 836-0856  
 Fax. (613) 836-7183  
 www.DSEL.ca

**EXISTING DRAINAGE PLAN**  
**70 Beech Street/ 75 Norman Street**

PROJ NO.:	18-1015
DRAWN BY:	CMK
DATE:	2019-02-12
SCALE:	N.T.S.
FIGURE NO.:	FIG-1



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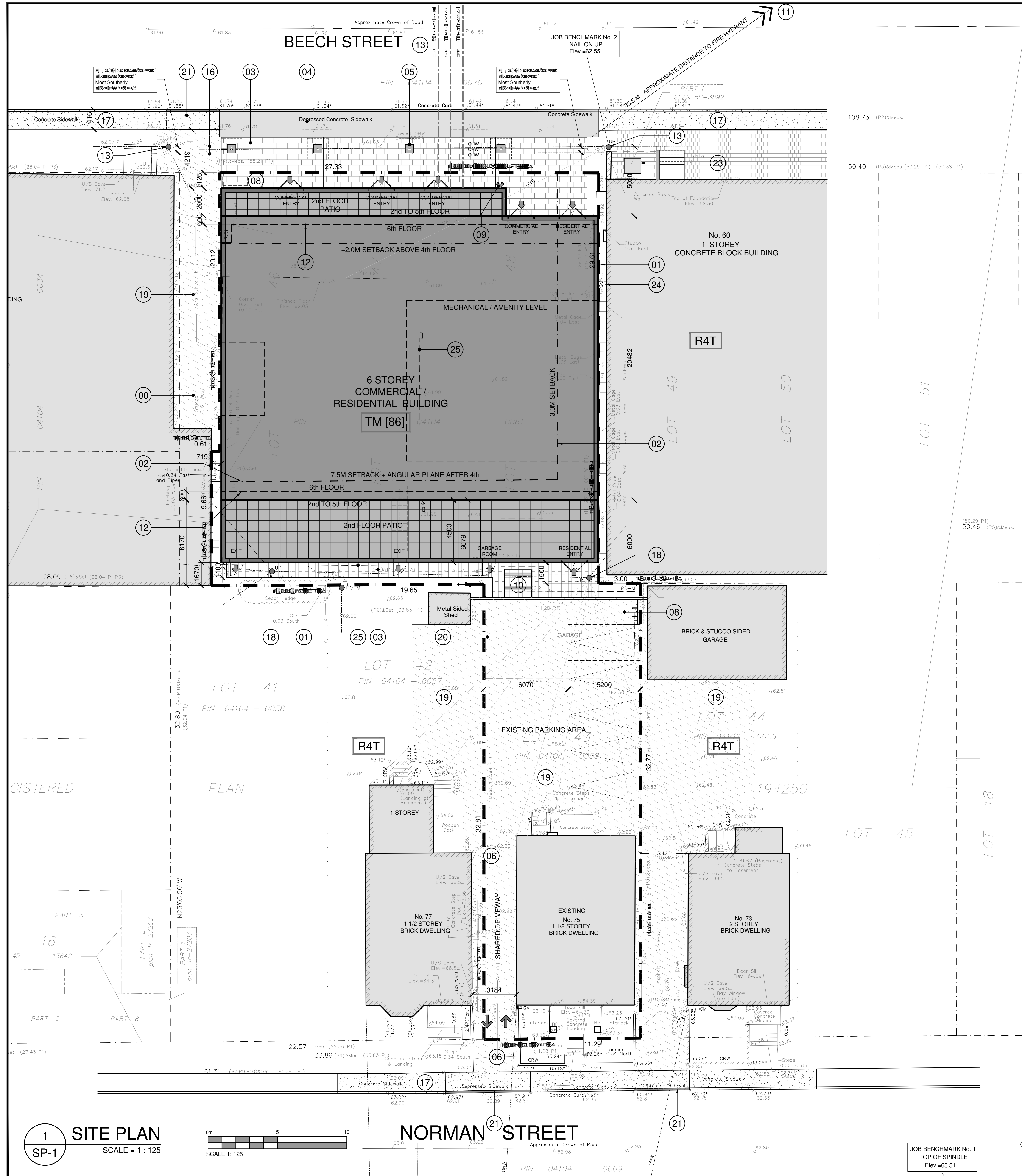
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***DRAWINGS / FIGURES***

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- ### DRAWING NOTES
- PROPERTY LINE
  - BUILDING SETBACKS
  - HARD SURFACE PAVING. SEE LANDSCAPE PLAN FOR PATTERN AND TYPE
  - EXISTING DEPRESSED CURB AND SIDEWALK TO BE REPLACED WITH BARRIER CURB AND 2.0 M WIDE SIDEWALK TO CITY STANDARD
  - TREE WITH GRATE - SEE LANDSCAPE PLAN
  - EXISTING SHARED ENTRY DRIVEWAY TO REMAIN
  - STORM WATER TANK - SEE CIVIL PLAN
  - EXTERIOR BICYCLE PARKING SPACES WITH RACK
  - SIAMSESE CONNECTION
  - HYDRO EXHAUST GRILL
  - EXISTING FIRE HYDRANT
  - OUTLINE OF BUILDING ABOVE
  - EXISTING UTILITY POLE
  - PROPOSED LOCATION OF UNDERGROUND UTILITIES
  - GAS REGULATOR / METER EQUIPMENT AREA
  - EXISTING HYDRO / UTILITY LINES TO BE ADJUSTED
  - EXISTING CONCRETE SIDEWALK WITH STREET CURB
  - EXISTING UTILITY POLE TO BE RELOCATED
  - EXISTING ASPHALT PARKING / LOADING AREA
  - EXISTING 1 STOREY CONCRETE BLOCK GARAGE TO BE REMOVED. REPAIR ASPHALT AS REQUIRED
  - EXISTING DEPRESSED CURB AND WALK
  - EXISTING PARKING SPACE
  - EXISTING UTILITY BOX
  - EXISTING GAS EQUIPMENT TO BE RELOCATED
  - REMOVE EXISTING 1 STOREY COMMERCIAL BUILDING

- ### SITE PLAN SYMBOLS
- CONCRETE UNIT PAVERS SURFACE
  - ASPHALT SURFACE
  - NEW CITY SIDEWALK
  - EXISTING CONCRETE SIDEWALK
  - BICYCLE PARKING SPACE
  - TWO WAY VEHICLE CIRCULATION
  - MAIN ENTRANCE
  - COMMERCIAL DOOR / FIRE EXIT
  - PROPERTY LINE
  - ZONING SETBACKS

### PROJECT INFORMATION

ZONING: Zoning By-Law 2017-113 TM [86] / R4T  
 SITE AREA: 1,187.3 sq. m (12,780 sq. ft.)  
 BUILDING HEIGHT (TM): 20.0 m  
 FRONT SIDE YARD SETBACK ABOVE 4th: -2.0 m  
 INTERIOR SIDE YARD SETBACK - EAST ONLY (TM): 3.0 m  
 REAR YARD SETBACK (TM): 7.5 m  
 AMENITY SPACE (6.0M X 48 UNITS): 288 sq. m  
 FRONT SIDE YARD SETBACK (R4): 3.0 m  
 INTERIOR SIDE YARD SETBACK (R4) - TOTAL 1.6 m: 0.9 m

### PROJECT STATISTICS

BUILDING HEIGHT: 24.0 M  
 AMENITY SPACE: 2nd FLOOR PRIVATE PATIOS = 194.4 sq. m  
 COMMUNAL INTERIOR ROOF TOP = 53.0 sq. m  
 COMMUNAL EXTERIOR ROOF TOP = 179.5 sq. m  
 TOTAL = 366.9 sq. m

### BUILDING STATISTICS

**GROSS BUILDING - AREAS**  
 (CITY OF OTTAWA'S DEFINITION)

BASEMENT LEVEL	0.0 sq. m
GROUND FLOOR	484.4 sq. m
TYPICAL FLOORS (2 - 5)	4 x 445.6 sq. m = 1,782.2 sq. m
6th FLOOR	417.2 sq. m
MECHANICAL / AMENITY LEVEL	0.0 sq. m
TOTAL AREA ABOVE GRADE	2,683.8 sq. m
EXISTING DWELLING - 75 NORMAN	200.0 sq. m
EXISTING DWELLING - 75 NORMAN	2,153.8 sq. m

### UNIT STATISTICS

1 BEDROOM UNIT	30
1 BEDROOM + STUDY UNIT	18
TOTAL	48
COMMERCIAL AREA	484.4 sq. m
EXISTING DWELLING - 75 NORMAN	5,214 sq. ft.

### CAR PARKING

REQUIRED	
RESIDENCE	- AREA 'Z' - NON REQUIRED 0
VISITOR	- 0.1 PER UNIT AFTER 12 UNITS 4
COMMERCIAL RETAIL	- AREA 'Z' - NON REQUIRED 0
75 NORMAN STREET	- AREA 'Z' - NON REQUIRED 0
TOTAL	4

### PROVIDED

RESIDENCE	0
VISITOR	0
COMMERCIAL RETAIL	0
75 NORMAN STREET	5
TOTAL	5

### BICYCLE PARKING

REQUIRED	
RESIDENCE	- 0.5 PER UNIT (48 UNITS) 24
COMMERCIAL	- 0.1 PER UNIT (48 UNITS) 4
TOTAL	28
PROVIDED	
BASEMENT LEVEL	50
AT GRADE	4
TOTAL	54

### LOT COVERAGE

PAVED SURFACE	= 196.5 sq. m. 16.5%
BUILDING FOOTPRINT	= 823.75 sq. m. 69.4%
LANDSCAPE OPEN SPACE	= 176.05 sq. m. 14.1%
TOTAL	= 1,187.3 sq. m. 100.0%

### CIVIL ENGINEER

**David Schaeffer Engineering Ltd.**  
 120 Iber Road, Unit 203  
 Tel: (613) 836-0856  
 Fax: (613) 836-7183  
 Email: rfree@DSEL.ca

### LEGAL DESCRIPTION

SURVEYOR'S REAL PROPERTY REPORT  
 PART 1 Plan of  
 LOTS 43, 47 AND 48  
 AND PART OF LOT 46  
 REGISTERED PLAN 194250  
 CITY OF OTTAWA  
 Surveyed by Annis, O'Sullivan, Vollebakk Ltd.

### LANDSCAPE ARCHITECT

**James B. Lennox & Associates Inc.**  
 Landscape Architects  
 3332 Carling Ave.  
 Tel: 613-722-5168  
 Fax: 1-866-343-3942  
 Email: JL@jbla.ca

### PROJECT DEVELOPER

**BEECH HOLDINGS Ltd.**  
 236 Metcalfe Street,  
 Ottawa, Canada K2P 1R3  
 Tel: 613.237.2425  
 Fax: 613.237.7300

### GEOTECHNICAL ENGINEER

**paterson group**  
 Ontario Land Surveyors  
 154 Colonnade Road South  
 Ottawa, Ontario  
 K2E 7J5  
 Tel: 613.226-7381  
 Email: DGilbert@Patersongroup.ca

### SURVEYOR

**Annis O'Sullivan Vollebakk Ltd.**  
 Ontario Land Surveyors  
 14 Concourse Gate, Suite 500,  
 Nepean, Ontario K2E 7S6  
 Tel: (613) 727-0850  
 Fax: (613) 727-1079  
 E-Mail: EdL@aovltd.com

### URBAN PLANNER

**FoTenn Consultants Inc.**  
 223 McLeod Street  
 Ottawa, ON Canada, K2P 0Z8  
 Tel.: (613) 730-5709  
 Fax: (613) 730-1136  
 E-Mail: mcelligott@fotenn.com

**THE PROPERTIES GROUP**  
 236 Metcalfe Street, Ottawa, Ontario K2P 1R3

**rla/architecture**  
 roderick lahey architect inc.  
 56 beech street, ottawa, ontario K1S 3J6  
 t. 613.724.9932 f. 613.724.1209 rlaarchitecture.ca

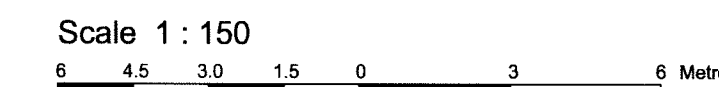
PROJECT TITLE:	70 BEECH STREET	
OTTAWA	ONTARIO	
SHEET TITLE:	SITE PLAN	
DRAWN:	RV	CHECKED: M.L.
SCALE:	1:125	SHEET No.
PROJECT No:	1724	<b>SP-1</b>





**SURVEYOR'S REAL PROPERTY REPORT**  
**PART 1** Plan of  
**LOTS 43, 47 AND 48**  
**AND PART OF LOT 46**  
**REGISTERED PLAN 194250**  
**CITY OF OTTAWA**

Surveyed by Annis, O'Sullivan, Vollebek Ltd.



Metric  
 DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND  
 CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

**Surveyor's Certificate**

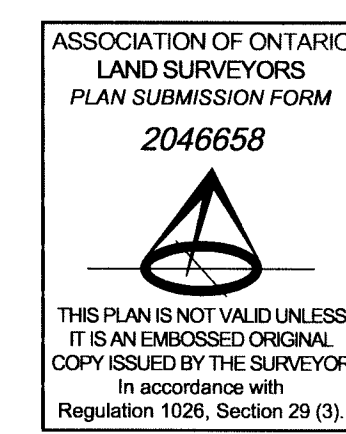
I CERTIFY THAT:  
 1. This survey and plan are correct and in accordance with the  
 Survey Act, the Surveyors Act and the Land Titles Act and the  
 regulations made under them.  
 2. The survey was completed on the 2nd day of March, 2018.  
 Date: March 5th, 2018  
 Richard R. Gauthier  
 Ontario Land Surveyor

**PART 2**  
 THIS PLAN MUST BE READ IN CONJUNCTION WITH  
 SURVEY REPORT DATED March 5, 2018

ANNIS, O'SULLIVAN, VOLLEBEK LTD. grants to  
 Beech Holdings Ltd. & Norman Beech Properties Ltd. ("The Client"), their  
 solicitors, mortgagees, and other related parties, permission to use original,  
 signed, sealed copies of the Surveyor's Real Property Report in transactions  
 involving The Client.

**Notes & Legend**

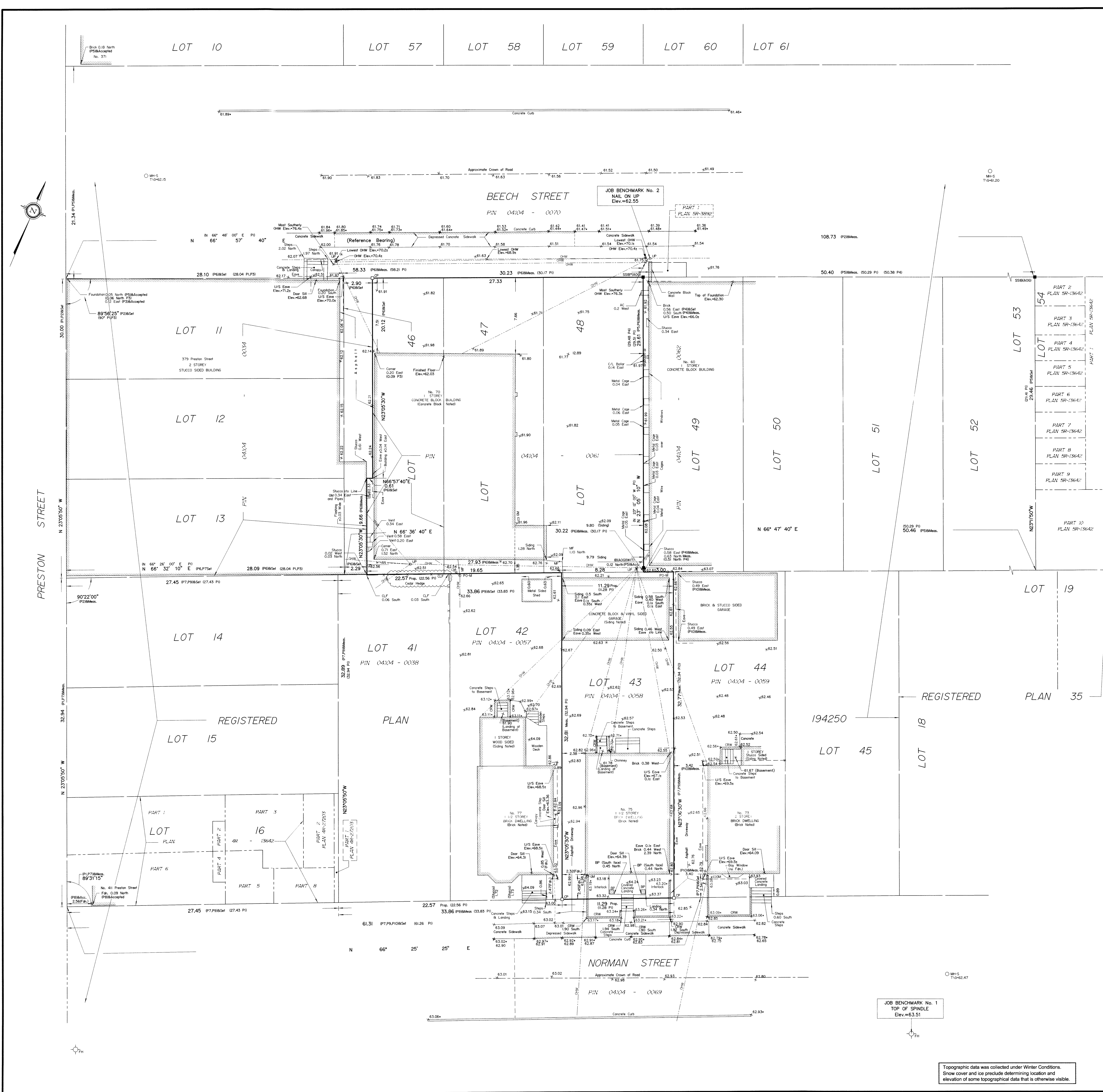
—○—	Denotes	Survey Monument Planted
—■—	•	Survey Monument Found
—SIB—	•	Standard Iron Bar
—IB—	•	Short Standard Iron Bar
—CP—	•	Iron Bar
•	•	Concrete Pin
(WIT)	•	Survey Monument 0.3 metres Long
(WIT)	•	Witness
(AOG)	•	Annis, O'Sullivan, Vollebek Ltd.
Meas.	•	Measured
Prop.	•	Proportioned
Acc.	•	Accepted
(PI)	•	Registered Plan 194250
(P2)	•	Plan 4R-28420
(P3)	•	(857) Plan February 18, 1957
(P4)	•	(857) Plan March 21, 1975
(P5)	•	Plan SR-13642
(P6)	•	(AOG) Plan November 27, 2014
(P7)	•	Plan 4R-28420
(P8)	•	(725) Plan July 9, 1985
(P9)	•	(857) Plan August 27, 1998
(P10)	•	(725) Plan January 10 1986
Acc.	•	Accepted
CLF	•	Chain Link Fence
MF	•	Metal Fence
C/L	•	Centreline
AC	•	Air Conditioner
GM	•	Gas Meter
UP	•	Utility Pole
OHW	•	Overhead Wires
AN	•	Anchor
UP	•	Utility Pole
PO-M	•	Metal Pole
OH-M	•	Maintenance Hole (Sanitary)
OH-S	•	Fire Hydrant
OH-S	•	Location of Elevations
OH-S	•	Top of Wall/ Concrete Curb Elevations
C/L	•	Centreline
CRW	•	Concrete Retaining Wall
U/S	•	Underside
Fdn.	•	Foundation
BP	•	Brick Pillar
B	•	Bollard



Bearings are astronomic, derived from the southerly limit of  
 Beech Street shown to be N66°57'40"E on Plan SR-13642.  
 For bearing comparisons, a rotation of  
 0°22'10" clockwise was applied to bearings on plan P7,  
 0°06'10" clockwise was applied to bearings on plan P9.

**ELEVATION NOTES**  
 1. Elevations shown are geoidic and are referred to the CGVD28 geoidic datum.  
 2. It is the responsibility of the user of this information to verify that the job benchmark  
 has not been altered or disturbed and that its relative elevation and description  
 agrees with the information shown on this drawing.

**UTILITY NOTES**  
 1. This drawing cannot be accepted as acknowledging all of the utilities and it will  
 be the responsibility of the user to contact the respective utility authorities for  
 confirmation.  
 2. Only visible surface utilities were located.  
 3. A field location of underground plant by the pertinent utility authority is  
 mandatory before any work involving breaking ground, probing, excavating, etc.



Topographic data was collected under Winter Conditions.  
 Snow cover and ice preclude determining location and  
 elevation of some topographical data that is otherwise visible.

