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Proposed High-Rise Residential Development 1200 Maritime Way

Serviceability & Stormwater Management Report

**Proposed High-Rise Residential Development
1200 Maritime Way**

Serviceability and Stormwater Management Report

Prepared for:

Claridge Homes

Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
K2M 1P6

January 25, 2021

Novatech File: 120144
Ref No. R-2021-012

January 25, 2021

City of Ottawa
Planning, Infrastructure and Economic Development Department
Planning and Infrastructure Approvals Branch
110 Laurier Avenue West, 4th Floor
Ottawa ON, K1P 1J1

Attention: Ms. Laurel McCreight, MCIP, RPP

Dear Ms.:

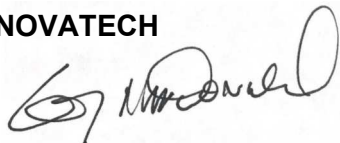
**Reference: 1200 Maritime Way - Claridge Development
Serviceability and Stormwater Management Report**

Enclosed is the Serviceability and Stormwater Management Report for the proposed 1200 Maritime Way development located along the Highway 417, Kanata Avenue and Maritime Way in the City of Ottawa. This report is submitted in support of the zoning amendment/site plan control applications and outlines how the site will be serviced with public infrastructure.

Trusting this report is adequate for your purposes. Should you have any questions, or require additional information, please contact me.

Yours truly,

NOVATECH



Greg MacDonald, P. Eng.
Director, Land Development and Public Sector Infrastructure

cc: Vincent, Denomme, Claridge Homes

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1.0 INTRODUCTION

This Serviceability Study has been prepared in support of Zoning By-Law Amendment and Site Plan Control applications for the Claridge lands located at 1200 Maritime Way, as shown in **Figure 1 – Key Plan of Subject Site**. The subject site is currently occupied by a vacant land. The proposed redevelopment will include a total of 632 dwelling units and 662 parking spaces.

The subject site has an approximate area of 1.28 hectares, and is surrounded by the following:

- Maritime Way and Townplace Suites by Marriott hotel to the north;
- Highway 417 to the south;
- Vacant land to the east; and
- A retirement residence to the west.

The most recent aerial view of the subject site is provided in **Figure 1**.

Figure 1: Key Plan of Subject Site



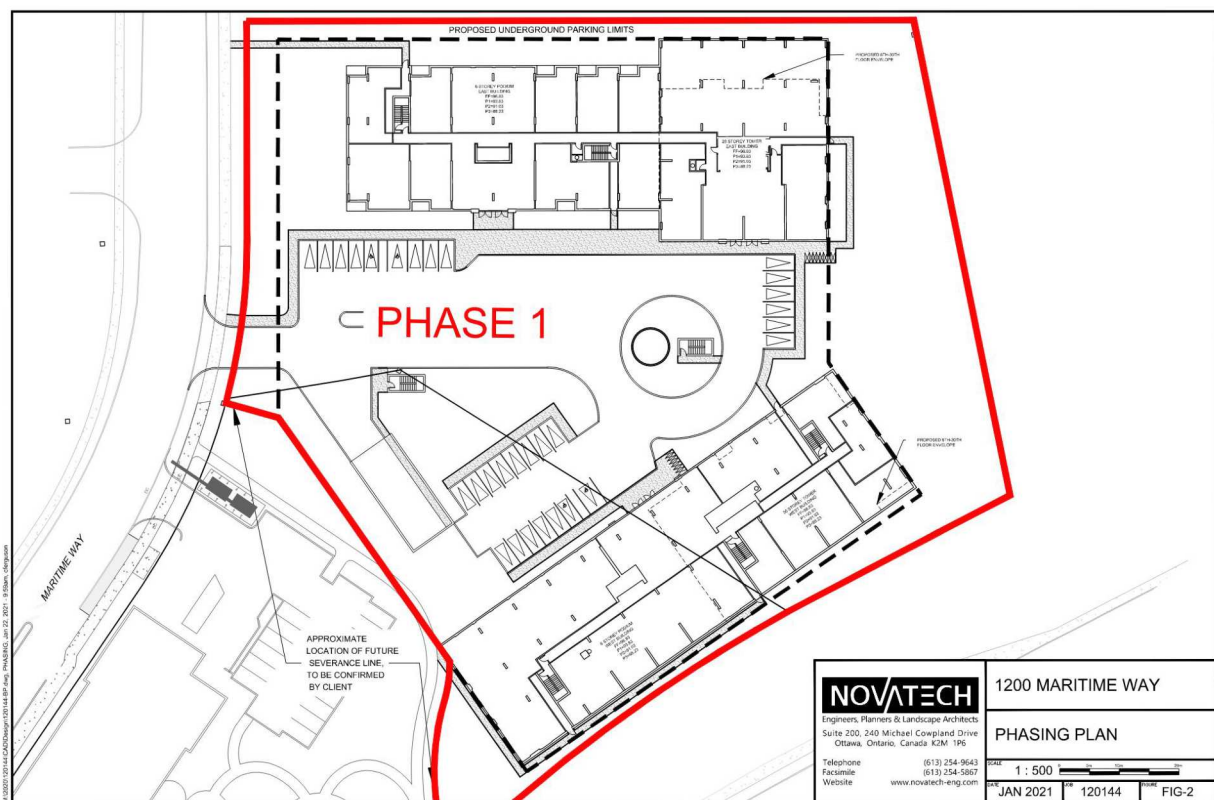
1.1 Proposed Development

The subject site is located within one of the Zone AM10 of the City of Ottawa. The implemented zoning for the property permits the proposed land uses. However, a Zoning By-Law Amendment is required to permit certain attributes of the proposed development, such as building height and FSI.

The proposed development will be constructed in one phase as shown in **Figure 2 – Phasing Plan** and as described below.

- Phase 1 (East/West Towers):
 - East Tower: 28-storey high-rise including 7-storey podium with 300 dwellings;
 - West Tower: 30-storey high-rise including 7-storey podium with 332 dwellings.

Figure 2 Phasing Plan



In total, the proposed development will consist of 632 apartment dwellings. The entire site will include 662 parking spaces for residents (632 inside) and visitors (30 outside) and will be accessed via full-movement driveway to Maritime Way. Phase 1 of the development is anticipated to be built out by 2028.

A copy of the Site Plan is included in **Appendix A – Site Plan**.

2.0 SANITARY SEWER

The development will be serviced by the existing 825mm diameter sanitary sewer on Maritime Way, as shown on the general plan of services.

The service will be a 375mm diameter sanitary sewer to Maritime Way.

The proposed development flows are based on the City of Ottawa Sewer Design Guidelines and are provided below.

2.1 Proposed Sanitary Flows from Development Site

Proposed sanitary flows are summarized in **Table 2.1 – Proposed Sanitary Flows** with detailed calculations below. Development statistics, e.g. apartment and townhouses are summarized in **Table 2.2 – Development Statistics**. A Phasing Plan is shown in **Figure 2 – Phasing Plan**.

Table 2.1 Proposed Sanitary Flows

Phase	Peak Sanitary Flow (L/sec)
East Tower	5.52
West Tower	6.02
Total	11.54

Table 2.2 Development Statistics

Building Component	Area (ha)	1 Bdr (x1.4)	2 Bdr (x2.1)	Total
East Tower				
Tower (incl. Podium)	-	177	123	300
Total	+/- 0.64	177	123	300
West Tower				
Tower (incl. Podium)	-	204	128	332
Total	+/- 0.64	204	128	332
Grand Total	1.28	381	251	632

Sanitary Flows East Tower

Area = 1.28 ha

Tower (incl. Podium): 247.8 + 258.3 = 507 people

Sanitary flows are calculated below using the City's new Sewer Design Criteria.

Population = 507 people

Peak Factor = $1 + 14/(4 + (P/1000)^{1/2}) \times 0.80 = 3.23$ (using entire population of 1,062)

Area = 1.23 ha

$$Q_{\text{Phases 1}} = \frac{(507)(280)(3.23)}{86,400} + (0.64)(0.33) = 5.52 \text{ L/sec}$$

Therefore, the total peak sanitary flow for **East Tower is 5.52 L/sec.**

Sanitary Flows West Tower

Area = 1.28 ha

Tower (incl. Podium): 285.6 + 268.8 = 555 people

Sanitary flows are calculated below using the City's new Sewer Design Criteria.

Population = 555 people

Peak Factor = $1 + 14/(4 + (P/1000)^{1/2}) \times 0.80 = 3.23$ (using entire population of 1,062)

Area = 0.52 ha

$$Q_{\text{Phases 2}} = \frac{(555)(280)(3.23)}{86,400} + (0.64)(0.33) = 6.02 \text{ L/sec}$$

Therefore, the total peak sanitary flow for **West Tower is 6.02 L/sec.**

Furthermore, the total peak sanitary flow for all **Phase 1 is 11.54 L/sec.**

The development of the adjacent site at 1250 Maritime Way (Timberwalk Retirement Home) accounted for flows of 3.86 L/s from the future development at 1200 Maritime Way. Because of an increase in flows over and above the flows allocated to the site (e.g. 2.84 L/s per the Robinson Consultants letter dated March 27, 1996) an analysis of the downstream sewer system was completed by J.L. Richards allowing for the additional flows of 4.35 L/sec (e.g. 7.19 L/s development flows for 1250 Maritime Way and future 1200 Maritime Way – 2.84 L/s allocated flows). Both the Robinson letter and the JLR analysis is included in **Appendix B – Sanitary Sewer Design Downstream Capacity.**

The calculated flow for the current proposed development at 1200 Maritime Way is 11.54 L/sec. This flow was input into the same spreadsheet analysis used for 1250 Maritime Way to assess the impact on the downstream sewer. To provide a comparison to the initial assessment completed by JLR, the same flow parameters were used, as follows:

- Per capita flow 350 L/cap/day
- Harmon Factor = 1.0
- Infiltration = 0.28 L/sec/ha
- Commercial/Retail = 50,000 L/ha/day

The resulting analysis shows an increase in peak sanitary flow in the truck sewer of 6.48 L/sec, explained as follows:

490.60 L/s	Peak Sanitary Flow in Trunk Sewer including proposed 1200 Maritime Way
<u>480.26 L/s</u>	Peak Sanitary Flow in Trunk Sewer under current conditions
10.34 L/s	Difference
<u>3.86 L/s</u>	Less portion of 1200 Maritime Way included in original analysis
6.48 L/s	Additional Sanitary Flows to be Added to System

The total additional flows of 6.48 L/sec represents 1.35 % of the total flow. Given the available capacity in the trunk sewer the impacts are negligible. Further, to be consistent with the original analysis completed for 1250 Maritime Way, the previous sanitary flow parameters were used.

Using current standards, e.g. 280 L/cap/day and Harmon 0.80 would further reduce the flows going into the sanitary sewer.

3.0 STORM SEWER AND STORMWATER MANAGEMENT

As part of this development, stormwater will be controlled on-site and discharged via a 375mm dia. service that will connect to the existing 1650mm dia. storm sewer on Maritime Way as shown on the General Plan of Services.

The site is fairly flat overall and the majority of storm runoff from the site is self-contained with some being conveyed overland towards the neighboring properties.

3.1 Storm Water Management Criteria

Stormwater management (SWM) design criteria for the proposed development were established by the City of Ottawa Sewer Design Guidelines (October 2012); Kanata Town Centre, Central Business District, Stormwater Management Report (J.L. Richards, January 1999) and Servicing Brief (Revised) – Kanata Town Centre Central Business District Subdivision, Technical Memorandum (J.L. Richards, June 13, 2012). The SWM design criteria are as follows:

- Control post-development peak flows up-to and including the 100-year storm event to the allowable release rate. Provide on-site water quantity control for all flow in excess of the allowable release rate. The allowable release rate is to be determined by applying the following parameters to the site area:
 - A runoff coefficient of 0.8
 - A time of concentration of 20 minutes
 - A 5-year intensity using the City of Ottawa Intensity-Duration-Frequency (IDF) curves
- Minimize the impact on the downstream receiving watercourses by minimizing the potential erosion and volume of sediment entering the watercourses both on a temporary basis (during construction) and on a permanent basis.
- Provide guidelines to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

3.2 Hydrologic and Hydraulic Modelling

The allowable release rate for the 1.28 ha site was determined to be 199.99 L/s based on the SWM criteria provided by the City of Ottawa.

The rational method was used to estimate post-development peak flows (quantity control targets) and determine approximate storage requirements for the site. The storage requirements for the site were determined for each tower of the development.

The post-development drainage areas were delineated based on the proposed development grading. Refer to **Drawing 120144-GR** for the proposed site grading and **Drawing 120144-SWM** for the drainage areas. The storage requirements are based on meeting the allowable release rate generated for the site.

The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Maritime Way.

Design Storms

The design storms are based on City of Ottawa design storms. Design storms were used for the 5 and 100-year return periods (i.e. storm events).

Model Parameters

Post-development catchments were modelled based on the proposed site plan and grading as shown on **Drawing 120144-SWM**. All the sub-catchments are assumed to be 100% impervious with exception to the grassed areas not over underground parking (A-1, part A-2, A-3, part A-4, part A-5 and part A-6) which are 0% impervious. The building roofs were assumed to have no depression storage.

A summary of the allowable release rate, post-development parameters and output for the 5 and 100-year storm events are provided in **Appendix C – Stormwater Management Calculations**.

3.3 Water Quantity Control

On-site stormwater management will be implemented to control post-development stormwater discharge to the allowable release rate of 199.99 L/s and will be achieved using internal stormwater tanks that will be pumped to the storm sewers on Maritime Way.

Runoff from the grassed areas (Sub-catchments A-1, A-2, A-3, A-4, A-5 and A-6) will be uncontrolled and will drain towards to Maritime Way. The total uncontrolled flows from the site in the 100-year event will be 87.81 L/s which requires the remaining areas of the site to be controlled to 112.18 L/s in order to meet the allowable release rate.

The remaining 112.18 L/s of allowable release rate was divided between the development phases using area-weighting as shown in **Table 3.1**.

Table 3.1 Controlled Release Rates

Phase	Drainage Area (ha)	Allowable Release Rate (L/s)
East Tower (incl. CB1/2)	0.54	70.44
West Tower (incl. CB3/4 & TD)	0.32	41.74
<i>Total</i>	<i>0.86</i>	<i>112.18</i>

The runoff from each tower and corresponding CBs or TD will be collected into at least one tank located within the development. The site was modeled so that the pump rate for each phase was equal to the allowable release rate for that phase. East and West towers will be pumped to the Maritime Way storm sewer. The tanks will have an emergency overflow that will connect to the ground surface. The required storage in the 100-year event for each phase is summarized in **Table 3.2**.

Table 3.2 Required Tank Storage for the 100-year Storm

Phase	Required Storage Volume (m ³)
East Tower	131.56
West Tower	77.96
<i>Total</i>	<i>209.52</i>

The storage provided allows for the proposed development to meet the allowable release rate of 199.99 L/s. The total release rates from the site during the 100-year storm event are provided in **Table 3.3**.

Table 3.3 Overall Site Release Rate for the 100-year Storm

Phase	Drainage Area (ha)	Allowable Release Rate (L/s)
East Tower (incl. CB1/2)	0.54	70.44
West Tower (incl. CB3/4 & TD)	0.32	41.74
Uncontrolled	0.42	87.81
<i>Total</i>	<i>1.28</i>	<i>199.99</i>

3.4 Water Quality Control

Runoff from the roofs, podiums, and uncontrolled grassed areas would be considered clean and will not require treatment. Additionally, the storage tanks will allow for some settling of particulates in the stored runoff from the remaining site areas. Additional water quality treatment will not be required. Erosion and sediment control measures will be implemented during all phases of construction and inspected regularly.

Cisterns from the East Tower and West Tower will discharge to the existing storm sewer on Maritime Way.

Also, there will be water quality control provided by the downstream SWM facility which has been designed to provide quantity and quality control for the proposed development (as per the Stormwater Management Study prepared by JL Richards).

The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Maritime Way.

4.0 WATERMAIN

4.1 Domestic Water Demand

The proposed development will be serviced by the 200mm dia. watermain on Maritime Way as shown on the General Plan of Services. Shutoff valves will be provided at property lines as per City of Ottawa Specifications. The water meters will be in the basement level mechanical rooms of the buildings. Similarly, remote receptacles will be located at the surface near the entrances to the buildings on the exterior.

The services will be two (2) 200mm diameter water services to Maritime Way, with a valve in between both of them.

Estimated domestic water demands for the development are provided below with a detailed breakdown per phase:

Watermain Flows East Tower

Average Day Demand = 2.05 L/sec

Maximum Day Demand (x2.5) = 5.13 L/sec

Peak Hour Demand (x2.2) = 11.28 L/sec

Watermain Flows West Tower

Average Day Demand = 2.25 L/sec

Maximum Day Demand (x2.5) = 5.63 L/sec

Peak Hour Demand (x2.2) = 12.38 L/sec

4.2 Fire Demand

An estimate of the water required to meet firefighting demands is described below.

Section 4.2.11 of the City of Ottawa Water Design Guidelines reads:

“When calculating the fire flow requirements and affected pipe sizing, designers shall use the method developed by the Fire Underwriters Survey”, and

“The requirements for levels of fire protection on private property are covered in Section 7.2.11 of the Ontario Building Code.”

The Fire Underwriters Survey is used to assess the performance of the water distribution system on a “City Block” basis rather than an individual building basis. The Ontario Building Code governs the assessment of fire demand for individual buildings.

Section 7.2.11.1 of the Ontario Building Code states that the design, construction, installation and testing of fire service mains and water service pipe combined with fire service mains shall be in conformance with NFPA 24.

NFPA 24 is the standard for the “Installation of Private Fire Service Mains and their Appurtenances”. Chapter 13 of NFPA 24 discusses sizing the private service fire mains for fire

protection systems which shall be approved by the authority having jurisdiction, considering the following factors:

- Construction and Occupancy of the Building
- Fire Flow and Pressure of the Water Required
- Adequacy of the Water Supply

It is expected that any future building on the site will be sprinklered per Section 3.2.2.45 of the OBC. Section 3.2.5.7 of the OBC requires that an adequate water supply for fire fighting be provided to each building, and references Appendix A of the OBC. Sentence 3 of Section A 3.2.5.7 of the OBC (Appendix A) states that NFPA 13 be used for determining both sprinkler and hose stream demands for a sprinklered building.

The design of the sprinkler system is completed by a Fire Protection Engineer, or typically computed by the sprinkler contractor and approved by the Fire Protection Engineer. This process involves detailed hydraulic calculations based on building layout, pipe runs, head losses, fire pump requirements, etc. At this stage in the planning and site design process, these details are not available. Therefore, this report will confirm the maximum anticipated sprinkler and hose stream demands as per NFPA 13.

Section 11.2.3 of the NFPA 13, “Water Demand Requirements – Hydraulic Calculations Methods” was used to estimate the sprinkler and hose stream demands. Figure 11.2.3.1.1 – Area/Density Curves confirms the sprinkler demand, assuming Ordinary 1 construction. Table 11.2.3.1.2 confirms the hose stream allowance and water supply demand requirements, assuming ordinary hazard construction.

For Ordinary 1 type construction, design is based on a density of 0.15 gpm (US), and a maximum area of sprinkler operation limited to 1500 ft² (139 m²). As per NFPA 13 Figure 11.2.3.1.1, the maximum anticipated sprinkler demand is 225 gpm (US). As per NFPA 13 Table 11.2.3.1.2, the maximum total combined inside and outside hose demand is 250 gpm (US) with a duration of 60-90 minutes.

Based on the calculations above, the total estimated sprinkler and hose demand for the development is 475 gpm (US). However, because the development has not been finalized to-date, it is recommended to add a 50% contingency. Therefore, a sprinkler demand of 713 gpm (US), 2700L/min, should be anticipated at this stage. Refer to **Appendix E – Fire Demand Calculations**.

Boundary conditions are requested from the City of Ottawa using a fire demand calculated using the **Fire Underwriters Insurance** procedure. This method is used by municipalities to assess their systems on a more global basis and results in a more conservative fire demand for individual sites, as compared to Building Code calculations. The estimated fire demand using FUS for each of the phases is provided in **Table 4.1 – Calculated Fire Demand**. Detailed calculations are included in **Appendix D – Fire Demand**.

Table 4.1 Calculated Fire Demand

Phase	Fire Demand (L/min)
East Tower	6000
West Tower	5000

5.0 CONCLUSIONS

Based on the foregoing, report conclusions are:

- Adequate sanitary sewer capacity is available on Maritime Way and in the downstream system to the trunk sewer.
- On site stormwater management will be implemented to control post-development flows to that value calculated using a tc of 20 minutes, run-off coefficient of 0.80 and 5-year storm. This will be implemented through construction of cisterns in the underground parking structure as summarized below. Uncontrolled flow from grass areas will drain overland to Maritime Way.

Phase	Cistern Volume (m ³)	Discharge (L/s)	Street Sewer
East Tower	131.56	70.44	Maritime Way
West Tower	77.96	41.74	Maritime Way
1	Uncontrolled	87.81	Maritime Way
Total	209.52	199.99	

- Adequate water services are available on Maritime Way for domestic demand. It is expected that adequate water supply is available for firefighting which will be confirmed once boundary conditions are received from the City. Calculated fire demand ranged from 5000 L/min to 6,000 L/min. The buildings will be equipped with fire pumps and sprinklers.

NOVATECH

Prepared by:



Jazmine Gauthier, B.A.Sc.
Project Manager | Land Development

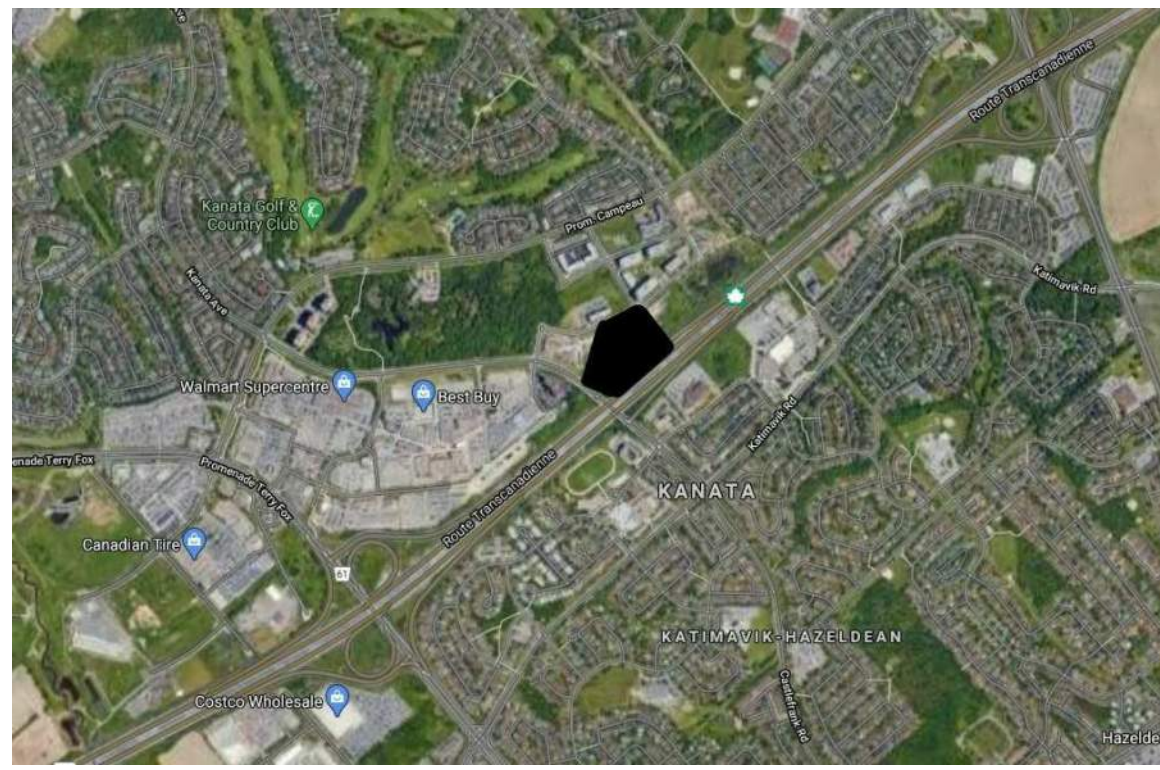
Reviewed by:



Greg MacDonald, P.Eng.
Director | Land Development and Public Sector
Infrastructure

APPENDIX A

Site Plan



KEY PLAN

ZONE AM10		
PROVISION	REQUIRED	PROVIDED
Min Lot Width	no minimum	+/- 69.65 m
Min Lot Area	no minimum	+/- 12 808 m ²
Max Building Height	67m	+/- 93.5 m
Min Front Yard Setback	no minimum	3.05 m / 3.09 m
Min Corner Side Yard Setback	no minimum	16.74 m
Min FSI	2	+/- 4.88
Min Interior Side Yard Setback	no minimum	14.70 m / 15.13 m

SITE AREA :	+/- 12 808 sq.m. (To be confirmed by surveyor)
SITE COVERAGE :	+/- 2 207 m ² (East Tower)
	+/- 1 968 m ² (West Tower)
	Total = +/- 4 175 m ² = 32.6 %
GROUND PARKING AREA :	+/- 2 298 m ² = 17.9 %
LANDSCAPED AREA (EXCLUDING PARKING) :	+/- 6 335 m ² = 49.5 %

RENTAL - EAST TOWER

PROPOSED GROSS FLOOR AREA :	+/- 21 964 m ²
BASEMENT G.F.A. :	+/- 0m ²
GROUND FLOOR G.F.A. :	+/- 635 m ²
RENTAL FLOORS G.F.A. (2nd to 30th floor) :	+/- 21 329 m ²
PRIVATE AMENITY AREA (G.F.A.) :	+/- 1 953 m ²
COMMUNAL AMENITY AREA :	+/- 1 925 m ²
NUMBER OF FLOORS AND BUILDING HEIGHT :	28 FLOORS + MECH. / +/- 87.50m
DWELLING UNITS :	300
PARKING STALLS :	315 (300 INSIDE / 15 VISITORS OUTSIDE)
PROVIDED BICYCLE STALLS :	150 (142 INSIDE / 8 OUTSIDE)

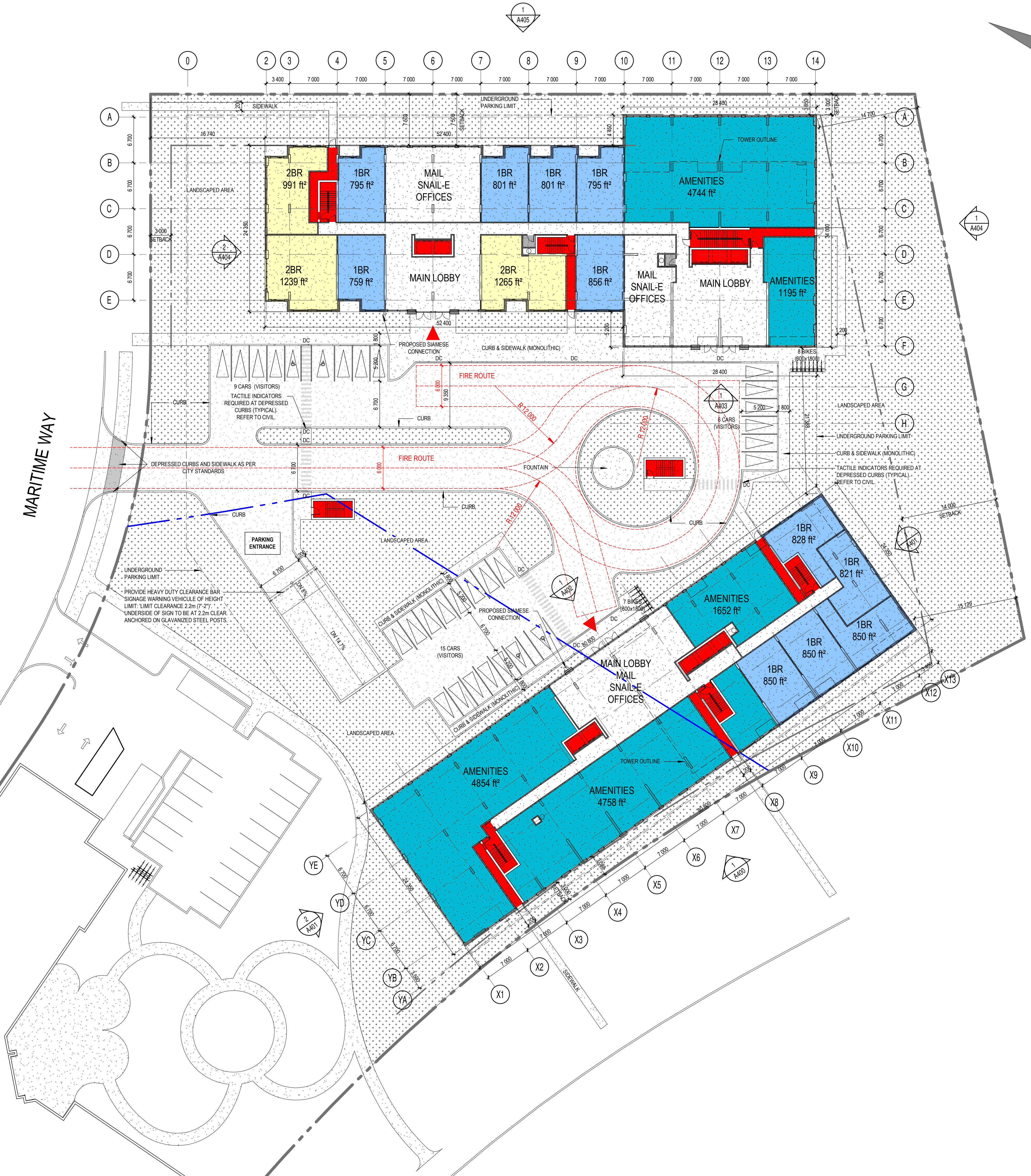
NUMBER OF SUITES REQUIRED TO BE BARRIER-FREE :
 300 UNITS = 45 UNITS HAVE TO BE BARRIER-FREE
 THEY WILL BE DISTRIBUTED BETWEEN THE 28 FLOORS

RENTAL - WEST TOWER

PROPOSED GROSS FLOOR AREA :	+/- 30 179 m ²
BASEMENT G.F.A. :	+/- 0m ²
GROUND FLOOR G.F.A. :	+/- 375 m ²
RENTAL FLOORS G.F.A. (2nd to 30th floor) :	+/- 29 804 m ²
PRIVATE AMENITY AREA (G.F.A.) :	+/- 2 247 m ²
COMMUNAL AMENITY AREA :	+/- 1 045 m ²
NUMBER OF FLOORS AND BUILDING HEIGHT :	30 FLOORS + MECH. / +/- 87.50m
DWELLING UNITS :	332
PARKING STALLS :	347 (332 INSIDE / 15 VISITORS OUTSIDE)
PROVIDED BICYCLE STALLS :	166 (159 INSIDE / 7 OUTSIDE)

NUMBER OF SUITES REQUIRED TO BE BARRIER-FREE :
 332 UNITS = 50 UNITS HAVE TO BE BARRIER-FREE
 THEY WILL BE DISTRIBUTED BETWEEN THE 30 FLOORS

- FOR EXISTING SITE CONDITIONS, SEE SURVEY PLAN BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD., SUBMITTED SEPARATELY;
- FOR NEW GRADES AND SITE SERVICES, SEE CIVIL ENGINEERING PLAN BY NOVATECH ENGINEERING CONSULTANTS, SUBMITTED SEPARATELY;
- FOR PROPOSED VEGETATION AND LANDSCAPE INFORMATION, SEE LANDSCAPE PLAN BY JAMES B. LENNOX & ASSOCIATES, SUBMITTED SEPARATELY.



NOTES GÉNÉRALES / General Notes

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- Veuillez aviser l'architecte de toute dimension en erreur ou divergences entre ces documents et ceux des autres professionnels. / The architect must be notified of all errors, omissions and discrepancies between these documents and those of the others professionals.
- Les dimensions sur ces documents doivent être lues et non mesurées. / The dimensions on these documents must be read and not measured.

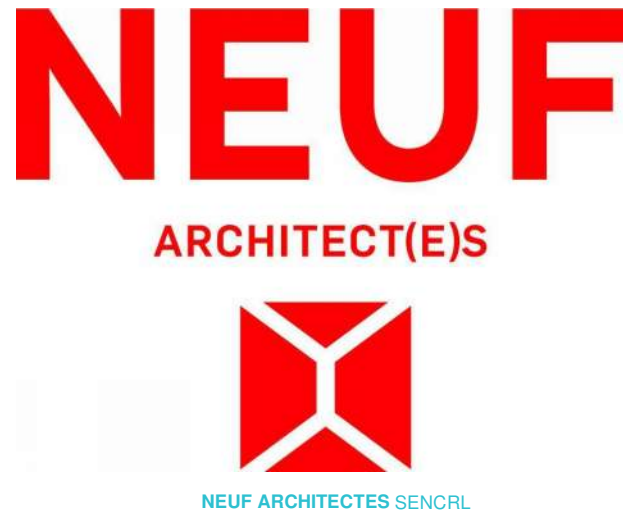
STRUCTURE / Structural
Goodeve Structural Inc.
 1877, Argus Drive, Ottawa ON K2E 7Z7
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ARCHITECTURE DE PAYSAGE / Landscape Architect
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NEUF architect(e)s SENCRL
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SCEAU / Seal



OUVRAGE / Project
1200 MARITIME WAY (KANATA RENTAL)

EMPLACEMENT / Location
 OTTAWA

NO PROJET / No.
 12371.00

NO	RÉVISION	DATE (aa-mm-jj)
A	FOR COMMENTS	2020.05.28
B	FOR COMMENTS	2020.06.05
C	FOR COMMENTS	2020.07.23
D	IN PROGRESS	2020.09.16
E	SITE PLAN COORDINATION	2020.12.08
F	SITE PLAN COORDINATION	2020.12.16

DESSINÉ PAR / Drawn by
 PV

VERIFIÉ PAR / Checked
 LH

DATE (aa.mm.jj)
 05/28/20

ÉCHELLE / Scale
 1 : 300

SITE PLAN AT GROUND FLOOR LEVEL

RÉVISION / Revision
F

NO. DESSIN / Dwg Number
A203

APPENDIX B

Sanitary Sewer Design Downstream Capacity

MEMORANDUM



**J.L. Richards
& Associates Limited**
864 Lady Ellen Place
Ottawa, ON Canada
K1Z 5M2
Tel: 613 728 3571
Fax: 613 728 6012

Page 1 of 2

To: Greg MacDonald, P.Eng.
Novatech Engineering Consultants Ltd.

Date: August 18, 2017

Job No.: 15712-015.1

CC: Lucie Dalrymple, P.Eng.
J.L. Richards & Associates Ltd.

From: Karla Ferrey, P.Eng.

Re: Kanata Town Centre Central Business District
Master Design Sheet Update - Sanitary Peak Flows
Block 4, Block 5 and Block west of Block 9 (Zone 122)

We understand that the City is requesting an update to the Master Sanitary Sewer Design Sheet for the Kanata Town Centre Central Business District (KTC-CBD) from JL Richards such to incorporate the proposed peak flow revision from Block 4, Block 5, and the parcel west of Block 9 (previously Robinson '96 - Zone 122). Refer to attached JLR Sanitary Drainage Plan and Robinson Consultants Figure 7.1 for locations of Block 4, Block 5 and Zone 122.

We understand that the City will ultimately decide (as the owner of the existing sewers within the KTC-CBD and downstream system) whether the proposed peak flow increase is acceptable and that if accepted, it will not require a reduction of the allowable peak flows for the remaining future development in the KTC-CBD.

As requested, we have incorporated the proposed sanitary peak flow increase associated with your following developments:

a) Proposed Block 4 - Residential development

The proposed development will result in a theoretical increase in peak flow from 3.88 L/s to 4.71 L/s at MH 513 where the Block 4 development outlets to Maritime Way. This represents a theoretical peak flow increase of 0.83 L/s from the anticipated 2012 land use (i.e., hotel use, based on 270 L/pers/day).

b) Proposed Block 5 - Residential development

The proposed development will result in a theoretical increase in peak flow from 3.52 L/s to 5.13 L/s at MH 511 where the Block 5 development outlets to Maritime Way. This represents a theoretical peak flow increase of 1.61 L/s from the anticipated 2012 land use (i.e., hotel use, based on 270 L/pers/day).

c) Proposed parcel west of Block 9 (previously identified in the 1996 Robinson KTC Sanitary Design as Zone 122) – Retirement Home – Claridge Homes

The proposed development will result in a theoretical increase in peak flow from 2.84 L/s to 7.19 L/s at MH 7A where Claridge Homes development outlets to Maritime Way. This represents a theoretical peak flow increase of 3.57 L/s from the anticipated 2012 land use (i.e., Commercial use based on 2787m² office space and Infiltration based 1.5ha). Theoretical flows for Zone 122 were taken from Robinson Consultants Sanitary Trunk Information from Table 4.7 and Figure 7.1, see attached copies.

At the most downstream MH at the intersection of Teron Rd and Campeau Dr (MH Ex. 2) shown on the attached Sanitary Sewer Design Sheet for the Kanata Village Green subdivision (prepared in 1998 by JLR), the proposed 3 developments would result in a theoretical increase in peak flow from 475.94 L/s to 480.24 L/s which corresponds to a 4.3 L/s (0.9%) peak flow increase.

Based on the available theoretical residual capacities noted in the attached updated Master Sanitary Sewer Design Sheet, the existing sanitary sewer system from the intersection of Rock Mountain Gate and Maritime Way to the intersection of Campeau Dr and Teron Rd has the capacity to accommodate the additional theoretical peak flows of Block 4, Block 5 and Zone 122. Downstream of the Campeau Drive intersection, JLR does not have on record design sheets for the City's existing downstream sanitary sewer system.

Should you have any questions or require anything further, please do not hesitate to call.

J.L. RICHARDS & ASSOCIATES LIMITED

Prepared by:

A handwritten signature in blue ink, appearing to read 'Karla Ferrey', with a stylized flourish at the end.

Karla Ferrey, P.Eng.



CITY OF OTTAWA

KANATA TOWN CENTRE
CENTRAL BUSINESS DISTRICT
URBAN DALE CORPORATION
JLR PROJECT NO.: 15712

Commercial Flow = 50000 L/ha/d
q residential = 350 l/cap/d
q hotel = 270 l/cap/d
q retirement homes = 450 l/cap/d
i = 0.28 l/s/ha
SING. HOUSING 3.4 pers/hse
MULT. HOUSING 2.7 pers/hse
Hotel/Appartments 1.8 pers/room
Retirement Homes 1.6 pers/room

MASTER SANITARY SEWER DESIGN SHEET
Designed: L.D.

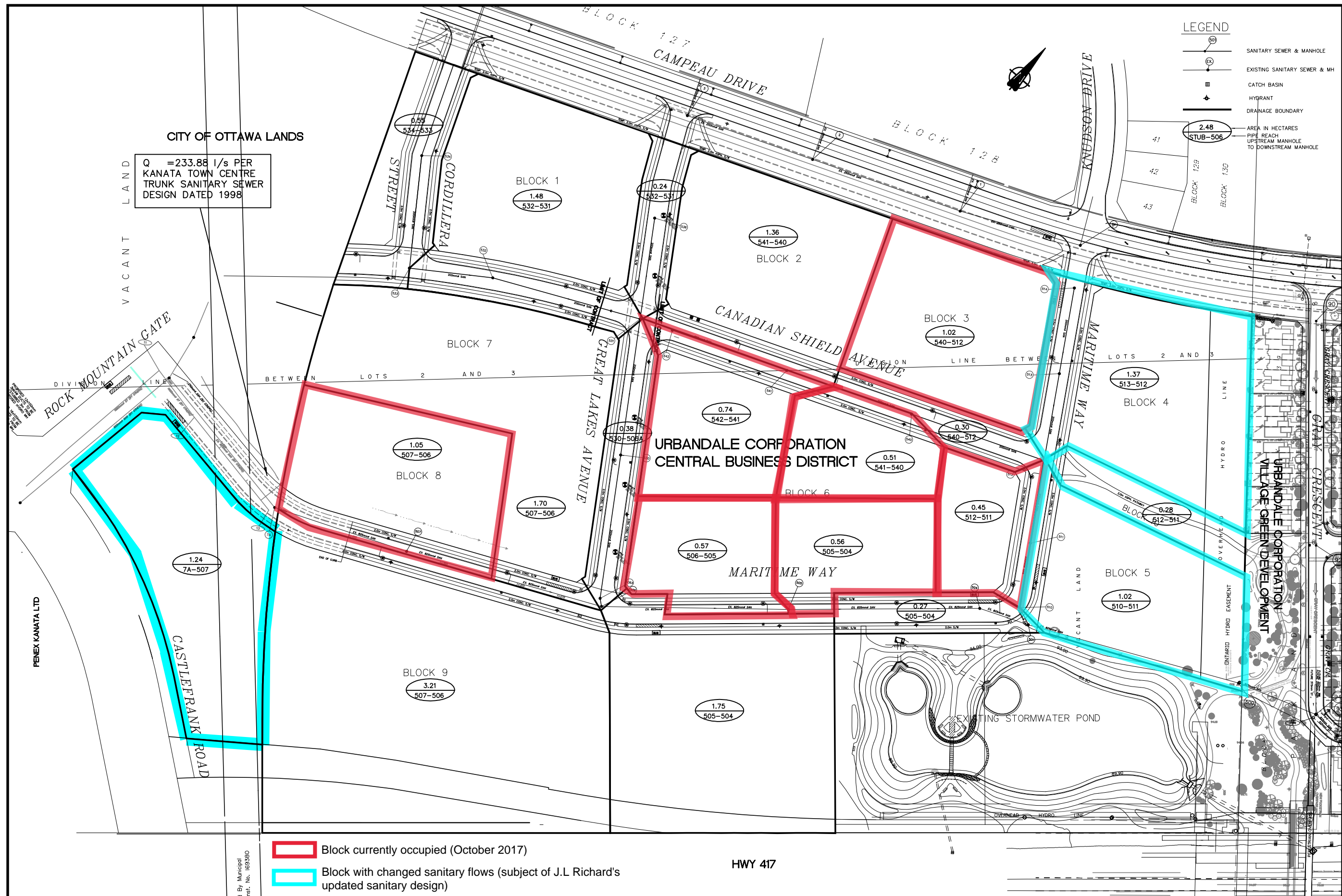
2017 Update by: KF
2017 Check by: LD

Date: August 15, 2017

Manning's Coefficient (n) = 0.013

STREET	M.H. # FROM TO		RESIDENTIAL											COMMERCIAL / INSTITUTIONAL			PLUGGED FLOW		R+C		2017 Updates to Block 4.5, West of 9 Peak Flows SEWER DATA					CAPACITY									
			SING.	Stacks	Towns	Ext. Care		Hotel/Apart.		POPUL. people	AREA ha	CUMMULATIVE POPUL. people	AREA ha	PEAKING FACTOR	POPUL. FLOW l/s	Actual AREA ha	CUMM. AREA ha	COMM. FLOW l/s	FLOW l/s	CUMM. FLOW l/s	PEAK EXTR. FLOW l/s	PEAK DES. FLOW l/s	DIA. mm	SLOPE %	CAPAC. l/s	VEL. m/s	LENGTH m	Residual (L/s)	% Full						
						No units	Act. pop	No units	Act. pop																					Equ. pop.					
COLCHESTER SQUARE	14	11		4						11	0.16	15158	259.68	2.77	170.31		30.02	26.06		201.54	81.48	479.39	900	0.11	600.38	0.94	72.6	120.99	80%						
TERON	11	10										15158	259.68	2.77	170.31		30.02	26.06		201.54	81.48	479.39	900	0.11	600.38	0.94	29.6	120.99	80%						
	10	EX.									0.25	15158	259.93	2.77	170.31		30.02	26.06		201.54	81.55	479.46	900	0.11	600.38	0.94	72.3	120.92	80%						
TERON	O.P.P.	EX.																					100	Forcemain											
TERON	EX.	EX. 2										15158	259.93	2.77	170.31		30.02	26.06		202.32	81.55	480.24	680	0.96	838.61	2.31	9.4	358.37	57%						
			(1)	As per Kanata Town Centre Sanitary Trunk Sewer Study, revised March 27, 1996, by Robinson Consultants Inc.																															
			(2)	Park or open space area.																															
			(3)	Equivalent population base on 208 rooms and 20 staff members.																															
			(4)	Allowance for an ultimate flow of 188 l/s to provide flexibility in future development as per Kanata Town Centre Sanitary Trunk Study.																															
			(5)	Additional flow associated with hotel amenities including swimming pool with bathrooms and laundry as per design calculations for Block 1 provided by WSP (October 2016)																															
			(6)	Additional flow associated with overall amenities including beauty salon, staff, dining and laundry as per design calculations for 1250 Maritime Way (Timberwalk Retirement Home) provided by Novatech (July 31, 2017)																															





CITY OF OTTAWA LANDS
 Q = 233.88 l/s PER
 KANATA TOWN CENTRE
 TRUNK SANITARY SEWER
 DESIGN DATED 1998

LEGEND

- SANITARY SEWER & MANHOLE
- EXISTING SANITARY SEWER & MH
- CATCH BASIN
- HYDRANT
- DRAINAGE BOUNDARY
- AREA IN HECTARES
- PIPE REACH

Block currently occupied (October 2017)
 Block with changed sanitary flows (subject of J.L. Richard's updated sanitary design)

DATE	REVISIONS	BY	NO.	DATE	REVISIONS	BY
12/06/12	REVISED CBD DRAINAGE	JLP	4	13/09/08	ISSUED TO CITY FOR REVIEW	LND
24/07/07	REVISED AS PER CITY COMMENTS	CSB	3	21/07/06	ISSUED TO CITY FOR REVIEW (STREET 'A' NORTH-SOUTH LEG)	LND
25/05/07	PHASE 2 ISSUED FOR CITY REVIEW	CSB	2	05/11/98	REVISED PER RMOC	LND
08/03/07	ISSUED FOR TENDER	CSB	1	08/06/98	ISSUED FOR MOE APPROVAL (SAN)	LND

SCALE
 0 10m 20m 30m 40m
 HORIZONTAL 1:1000

J.L. Richards & Associates Limited
 864 Lady Ellen Place
 Ottawa, ON Canada
 K1Z 5M2
 Tel: 613 728 3571
 Fax: 613 728 6012

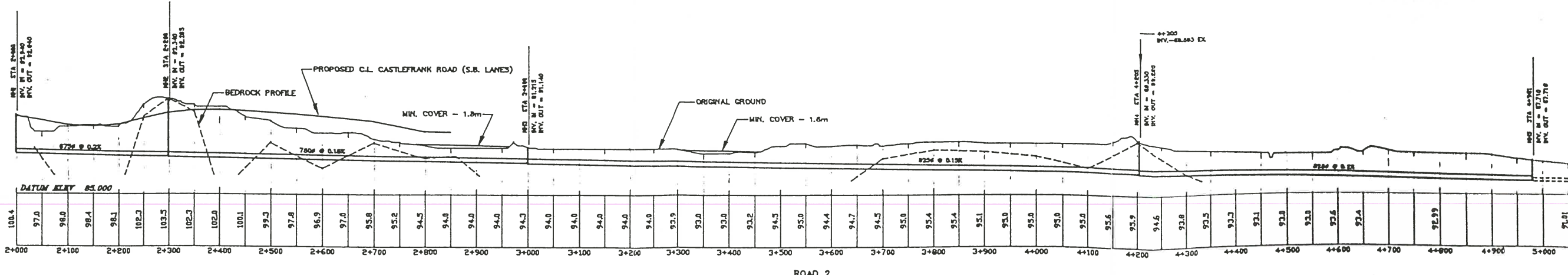
THIS DRAWING IS COPYRIGHT PROTECTED AND MAY NOT BE REPRODUCED OR USED FOR PURPOSES OTHER THAN EXECUTION OF THE DESCRIBED WORK WITHOUT THE EXPRESS WRITTEN CONSENT OF J.L. RICHARDS & ASSOCIATES LIMITED.

DESIGN	C.B.
CHECKED	L.N.D.
DRAWN	T.S.
CHECKED	
APPROVED	



KANATA TOWN CENTRE
 CENTRAL BUSINESS DISTRICT
 SANITARY DRAINAGE PLAN

PROJECT No.	15712-NAD 83
STARTED	JUNE 1998
DWG. No.	15712-SAN



NOTE: THIS PLAN WAS PRODUCED ON AN AUTOCAD® DRAFTING SYSTEM

No.	DATE	REVISION	BY	No.	DATE	REVISION	BY
1	05.16.94	REVISED PROFILE	P.A.				
2	06.28.94	GENERAL REVISIONS AND ADDITIONS	P.A.				
3	07.11.94	GENERAL REVISIONS	P.A.				
4	07.28.95	PIPE SIZE PENFIELD DR. AND LOOP	A.J.R.				



SCALES
 1: 4000
 HORIZONTAL
 1: 400
 VERTICAL

Robinson
 Consultants

CONSULTING ENGINEERS
 136, Michael Coulson Dr.
 Kanata, Ontario, K2M 2E9
 Telephone (873) 882-8080

DESIGN P.G.
 CHECKED A.J.R.
 DRAWN I.D.M.
 CHECKED
 APPROVED A.J.R.

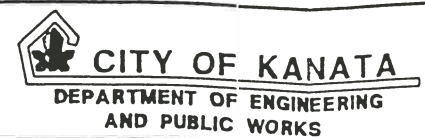


Figure 7.1
KANATA TOWN CENTRE
 Trunk Sanitary Sewer Functional Design

PROJECT No. 93055
 CONTRACT No. _____
 DATED OCT. 93
 DWG. No. 93055-C

Table 4.7 - Revised as per RMOC Letter Dated March 27, 1996

KANATA TOWN CENTRE SANITARY TRUNK SEWER STUDY

Ultimate Development Flows Worksheet

Revised March, 1996

Project 93055

SR Pump Strn. Qp= 163 l/s

q (res)= 4.1E-03 l/cap x s 0.35 cu. m/capita/day

q (ret)= 5.8E-05 l/s x m2 5000 l/1000m2 x day

q (com)= 5.8E-05 l/s x m2 5000 l/1000m2 x day

q (hot)= 2.6E-03 l/s x bed 225 l/bed x day

Peaking factor for ret & off & hot= 1.5

Flow Scenario - III

2 beds/room

3.8 persons/dwelling (low & med density)

I= 0.28 l/s/ha

2.2 persons/dwelling (high density)

Zone	Area	Residential Units			Retail		Office		Special Gen.		Peaking Factor	Qp (l/s)	Qi (l/s)	Qtot (l/s)	Cummul. Qtot (l/s)	
		Low	Med	High	GLA (m2)	Emp.	Area (m2)	Emp.	Hotel Rooms	Emp.						
112	1.6		100		2230	47	5574	200								
111	2.2															
109	2.2	200	33						200	88						
115	0.8						1394	50								
116	0.20															
114	0.10															
118	1.7			50			9755	350								
120	1.1		87													
100	7.40				16908	386					4.00	1.47	2.07	3.54	166.62	
101	1.30				4041	87					4.00	0.35	0.36	0.71	167.34	
102	0.80				1579	34					4.00	0.14	0.22	0.36	167.70	
104	1.50			168	10080	217					4.00	6.86	0.42	7.28	174.98	
110	8.20		300								3.68	16.98	2.30	19.28	193.78	
103	13.30				74459	1603					3.68	6.46	3.72	10.19	203.97	
105	2.10			90	8826	190					3.64	3.68	0.59	4.27	208.00	
106	1.50				3298	71					3.64	0.29	0.42	0.71	208.70	
117	0.04										3.64	0.00	0.01	0.01	208.72	
119	2.60			100	2230	47	34838	1250			3.60	6.42	0.73	7.15	215.59	
107	9.10								100	88	3.60	0.78	2.55	3.33	218.53	
113	2.10			300	2230	47	16722	600			3.50	10.99	0.59	11.58	229.31	
121	0.10						19509	700			3.50	1.69	0.03	1.72	231.04	
122	1.50						27870	1000			3.50	2.42	0.42	2.84	233.88	
123	1.70		72	50			1394	50			3.45	5.48	0.48	5.95	239.30	
124	0.60										3.45	0.00	0.17	0.17	239.47	
125	1.40										3.45	0.00	0.39	0.39	239.86	
126	2.80										3.45	0.00	0.78	0.78	240.64	
127	1.80		80				4181	150			3.41	4.56	0.50	5.07	245.27	
128	1.20		36				4181	150			3.39	2.24	0.34	2.58	247.65	
129	1.70		70				6968	250			3.37	4.23	0.48	4.71	251.96	
130	1.10						11148	400			3.37	0.97	0.31	1.28	253.24	
131	2.00										3.37	0.00	0.56	0.56	253.80	
132	0.60		40								3.35	2.06	0.17	2.23	255.80	
133	0.60										3.35	0.00	0.17	0.17	255.97	
134	0.70						4181	150			3.35	0.36	0.20	0.56	256.52	
135	0.60		36								3.34	1.85	0.17	2.02	258.33	
136	1.00		18								3.33	0.92	0.28	1.20	259.43	
137	0.80	10	18								3.32	1.43	0.22	1.65	260.92	
138	1.50		93								3.29	4.71	0.42	5.13	265.50	
139	0.80	18	8								3.28	1.31	0.22	1.54	266.88	
156	1.10		37								3.27	1.86	0.31	2.17	268.82	
140	0.90	8	27								3.26	1.75	0.25	2.01	270.62	
141	1.00		59								3.24	2.94	0.28	3.22	273.48	
142	0.50										3.24	0.00	0.14	0.14	273.62	
144	0.60		34								3.23	1.69	0.17	1.86	275.27	
143	1.10	10	30								3.22	1.98	0.31	2.29	277.31	
145	1.30		92								3.19	4.52	0.36	4.88	281.63	
146	1.00	16	19								3.18	1.71	0.28	1.99	283.41	
108	1.20		34								3.17	1.66	0.34	2.00	285.19	
148	1.00	8	18								3.17	1.27	0.28	1.55	286.58	
150	0.70		11								3.16	0.54	0.20	0.73	287.24	
151	0.30										3.16	0.00	0.08	0.08	287.32	
152	2.00										3.16	0.00	0.56	0.56	287.88	
154	1.20		66								3.15	3.20	0.34	3.53	291.00	
155	1.80						3177	114			3.15	0.28	0.50	0.78	291.78	
147	1.30		49								3.13	2.36	0.36	2.73	294.20	
153	0.80			100							3.12	2.78	0.22	3.00	296.84	
149	0.60				1858	39					3.12	0.16	0.17	0.33	297.17	
Totals	90.84	70	1247	808	125509	2768	134169	5414	100	176						
Total Town Centre Population					6782.2											
Average Persons per Dwelling Unit					3.19		Combined Down Stream Flow					425.64				

CITY OF KANATA

SANITARY SEWER DESIGN SHEET

q (res) = 350 l/cap/day
 q (com) = 50,000 l/ha/day
 q (inst) = 50,000 l/ha/day
 I = 0.280 l/s/ha
 Singles, Townhouses, Ter. Bungalows = 3.8 pers / unit (low & medium density)
 Stacked Townhouses / Apartments = 2.2 pers / unit (high density)
 Stacked Townhouses / Apartments = 80 units / ha (high density)

**KANATA TOWN CENTRE
(RESIDENTIAL)
URBAN DALE CORPORATION**

Designed by: L.N.D.

Checked by: M.F.S.

STREET	M.H. #		No. of UNITS		AREA ha	CUMMULATIVE		Peaking Factor	POPUL. FLOW l/s	INFIL. FLOW l/s	PEAK FLOW l/s	SEWER DATA				
	FROM	TO	Singles & Townhouses	Stacked Townhouses		POPUL. AREA	AREA					DIA mm	Slope %	CAPAC. l/s	VEL. m/s	LENGTH m
						peop.	ha									
A	90	92	37		0.80	141	0.80	4.00	2.28	0.22	2.50	250	0.60	46.06	0.94	120.0
	92	94	13		1.19	190	1.99	4.00	3.08	0.56	3.64	250	2.20	88.20	1.80	103.0
	94	95			66.80	4831	68.79	3.26	63.77	19.26	270.61	825	0.12	497.22	0.93	17.5
	95	89	10		0.52	4869	69.31	3.26	64.21	19.41	271.20	825	0.12	497.22	0.93	66.6
B	85	87	19		1.19	72	1.19	4.00	1.17	0.33	1.50	250	0.40	37.61	0.77	116.9
	87	89	26		0.82	171	2.01	4.00	2.77	0.56	3.33	250	1.41	70.70	1.44	116.7
A	89	84	12		0.35	5085	71.67	3.24	66.71	20.07	274.35	825	0.12	497.22	0.93	79.0
C	80	82	20		1.08	76	1.08	4.00	1.23	0.30	1.53	250	0.40	37.61	0.77	120.0
	82	84	28		0.83	182	1.91	4.00	2.96	0.53	3.49	250	1.20	65.18	1.33	118.5
A	84	79	14		0.54	5321	74.12	3.22	69.40	20.75	277.74	825	0.12	497.22	0.93	79.0
D	75	76	19		0.37	72	0.37	4.00	1.17	0.10	1.27	250	0.40	37.61	0.77	57.0
	76	77	20		0.29	148	0.66	4.00	2.40	0.18	2.59	250	0.40	37.61	0.77	78.4
	77	79	14		0.63	201	1.29	4.00	3.26	0.36	3.62	250	0.81	53.66	1.09	117.7
PARK EASEMENT	79	67			0.98	5522	76.39	3.20	71.69	21.39	280.66	825	0.12	497.22	0.93	55.0
	67	66	6		0.33	5545	76.72	3.20	71.95	21.48	281.01	825	0.12	497.22	0.93	70.0
BELLROCK DRIVE	70	73	26		2.56	99	2.56	4.00	1.60	0.72	2.32	250	0.40	37.61	0.77	87.2
	73	74	10		0.54	137	3.10	4.00	2.22	0.87	3.08	250	0.40	37.61	0.77	60.3
EASEMENT CAMBRAY LANE	74	62			0.31	137	3.41	4.00	2.22	0.95	3.17	250	0.40	37.61	0.77	39.9
	62	66	25		0.48	232	3.89	4.00	3.76	1.09	4.85	250	0.77	52.18	1.06	100.5
BISHOPS MILLS WAY	66	65	9		0.53	5811	81.14	3.18	74.95	22.72	285.25	825	0.12	497.22	0.93	62.0
SOUTH of HWY 417	EX.	65			191.60	7792	191.60	3.06	96.63	53.65	188.16	900	0.11	600.38	0.94	50.2
BISHOPS MILLS WAY	65	64	2			13610	272.74	2.82	155.52	76.37	457.35	900	0.11	600.38	0.94	17.0
EDENVALE DRIVE KETTLEBY STREET	59	60	8		0.50	30	0.50	4.00	0.49	0.14	0.63	200	1.40	38.80	1.24	77.0
	60	61	24		0.62	122	1.12	4.00	1.97	0.31	2.28	250	0.40	37.61	0.77	103.6
CAMBRAY LANE	58	61	8		0.41	30	0.41	4.00	0.49	0.11	0.61	200	0.70	27.44	0.67	74.5
KETTLEBY STREET	61	64	25		0.42	247	1.95	4.00	4.00	0.55	4.55	250	0.90	56.41	1.15	105.0
BISHOPS MILLS WAY	64	63	3			13869	274.69	2.81	158.01	76.91	460.38	900	0.11	600.38	0.94	13.0
	63	57	10		0.68	13907	275.37	2.81	158.38	77.10	460.94	900	0.11	600.38	0.94	64.9
TER. BUNGALOW Ph.2	51	53	48		0.94	182	0.94	4.00	2.96	0.26	3.22	200	0.70	27.44	0.87	122.3
	53	54	4			198	0.94	4.00	3.20	0.26	3.47	200	0.70	27.44	0.87	13.6
BISHOPS MILLS WAY	54	55			0.27	198	1.21	4.00	3.20	0.34	3.54	200	0.70	27.44	0.87	36.7
	55	56	11		0.81	239	2.02	4.00	3.88	0.57	4.44	250	0.40	37.61	0.77	107.1
	56	57	19		0.65	312	2.67	4.00	5.05	0.75	5.80	250	0.60	46.06	0.94	101.5
PARK	57	34	1		0.37	14222	278.41	2.80	161.40	77.95	464.82	900	0.11	600.38	0.94	53.5
	34	33	3		0.00	14234	278.41	2.80	161.51	77.95	464.93	900	0.11	600.38	0.94	50.3
HAWKSTONE	43	44	16		1.19	61	1.19	4.00	0.99	0.33	1.32	250	1.00	59.46	1.21	51.0
	44	45	8		0.09	91	1.28	4.00	1.48	0.36	1.84	250	0.50	42.05	0.85	29.0
ENDENVALE BIRKENDALE DRIVE	45	35			0.08	91	1.36	4.00	1.48	0.38	1.86	250	0.60	42.05	0.86	39.8
	35	36	7		1.18	118	2.54	4.00	1.91	0.71	2.62	250	0.37	36.18	0.74	93.2
	36	37	13		0.79	167	3.33	4.00	2.71	0.93	3.64	250	0.37	36.09	0.74	77.1
	37	33	2		0.00	175	3.33	4.00	2.83	0.93	3.76	250	0.40	37.61	0.77	17.9
BIRKENDALE DRIVE	33	32	13		0.56	14458	282.30	2.79	163.66	79.04	468.16	900	0.11	600.38	0.94	72.7
TEESWATER STREET	30	31	18		0.66	68	0.66	4.00	1.11	0.18	1.29	250	0.40	37.61	0.77	75.1
	31	32	19		0.41	141	1.07	4.00	2.28	0.30	2.58	250	0.40	37.61	0.77	77.9
BIRKENDALE STREET	32	18	4		0.37	14614	283.74	2.79	165.14	79.45	470.05	900	0.11	600.38	0.94	44.4
	18	16	6			14636	283.74	2.79	165.36	79.45	470.27	900	0.11	600.38	0.94	44.4
COMMERCIAL PLAZA COLCHESTER SQUARE	19	17			0.52	0	0.52	1.50	0.45	0.15	0.60	150	0.90	14.45	0.82	26.5
	17	16			0.10	0	0.62	4.00	0.45	0.17	0.62	250	0.40	37.61	0.77	33.2
COLCHESTER SQUARE	16	15	10		0.56	14674	284.92	2.79	166.17	79.78	471.41	900	0.11	600.38	0.94	66.0
	15	14 A	2			14682	284.92	2.79	166.25	79.78	471.48	900	0.11	600.38	0.94	25.8
ELSINORE LANE	39	28	22		0.53	84	0.53	4.00	1.35	0.15	1.50	250	1.00	59.46	1.21	56.7
	28	24	14		1.47	137	2.00	4.00	2.22	0.56	2.78	250	0.40	37.61	0.77	43.0
	24	23	12		0.14	182	2.14	4.00	2.96	0.60	3.55	250	0.40	37.61	0.77	34.0
	23	306	8		0.24	213	2.38	4.00	3.45	0.67	4.11	250	0.44	39.41	0.80	48.8
ENDENVALE DRIVE	306	14 A			0.45	213	2.83	4.00	3.45	0.79	4.24	250	0.49	41.68	0.85	46.4
	14 A	14				14895	287.75	2.78	167.82	80.57	473.85	900	0.11	600.38	0.94	14.7
COLCHESTER SQUARE TERON	Church	14			0.52	0	0.52	1.50	0.45	0.15	0.60	150	1.00	15.23	0.86	35.0
	14	11	4		0.16	14910	288.43	2.78	168.87	80.76	475.09	900	0.11	600.38	0.94	72.6
TERON	11	10				14910	288.43	2.78	168.87	80.76	475.09	900	0.11	600.38	0.94	29.6
	10	EX.			0.25	14910	288.68	2.78	168.87	80.83	475.16	900	0.11	600.38	0.94	72.3
TERON	O.P.P.	EX.									0.78	100	Forcemain			
TERON	EX.	EX.									475.94	680	0.96	838.61	2.31	9.4



Karla Ferrey

From: Lucie Dalrymple
Sent: August 1, 2017 9:43 AM
To: Karla Ferrey
Subject: FW: Kanata Town Centre - Sanitary Flows
Attachments: 1088 San Drainage.pdf; 1136 San Drainage.pdf

...here it is

Lucie Dalrymple, P.Eng.
Associate
Senior Civil Engineer

J.L. Richards & Associates Limited
864 Lady Ellen Place, Ottawa, ON K1Z 5M2
Tel: 613-728-3571 Fax: 613-728-6012



**J.L. Richards
& Associates Limited**
ENGINEERS • ARCHITECTS • PLANNERS



From: Matthew Hrehoriak [mailto:m.hrehoriak@novatech-eng.com]
Sent: July 31, 2017 10:16 AM
To: Lucie Dalrymple
Subject: RE: Kanata Town Centre - Sanitary Flows

Hi Lucie,

The sanitary info for the block 4 and 5 developments are as follows:

1088 Maritime Way (Block 4)

San service connection between SANMH 512-513
San Drainage Area = 1.121 ha
No. Units = 144
Population = 271

1136 Maritime Way (Block 5)

San service connection between SANMH 510-511
San Drainage Area = 0.915 ha
No. Units = 154
Population = 301

Regards,

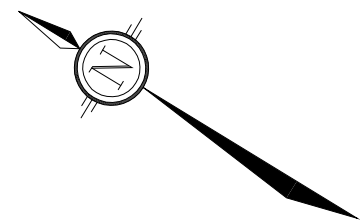
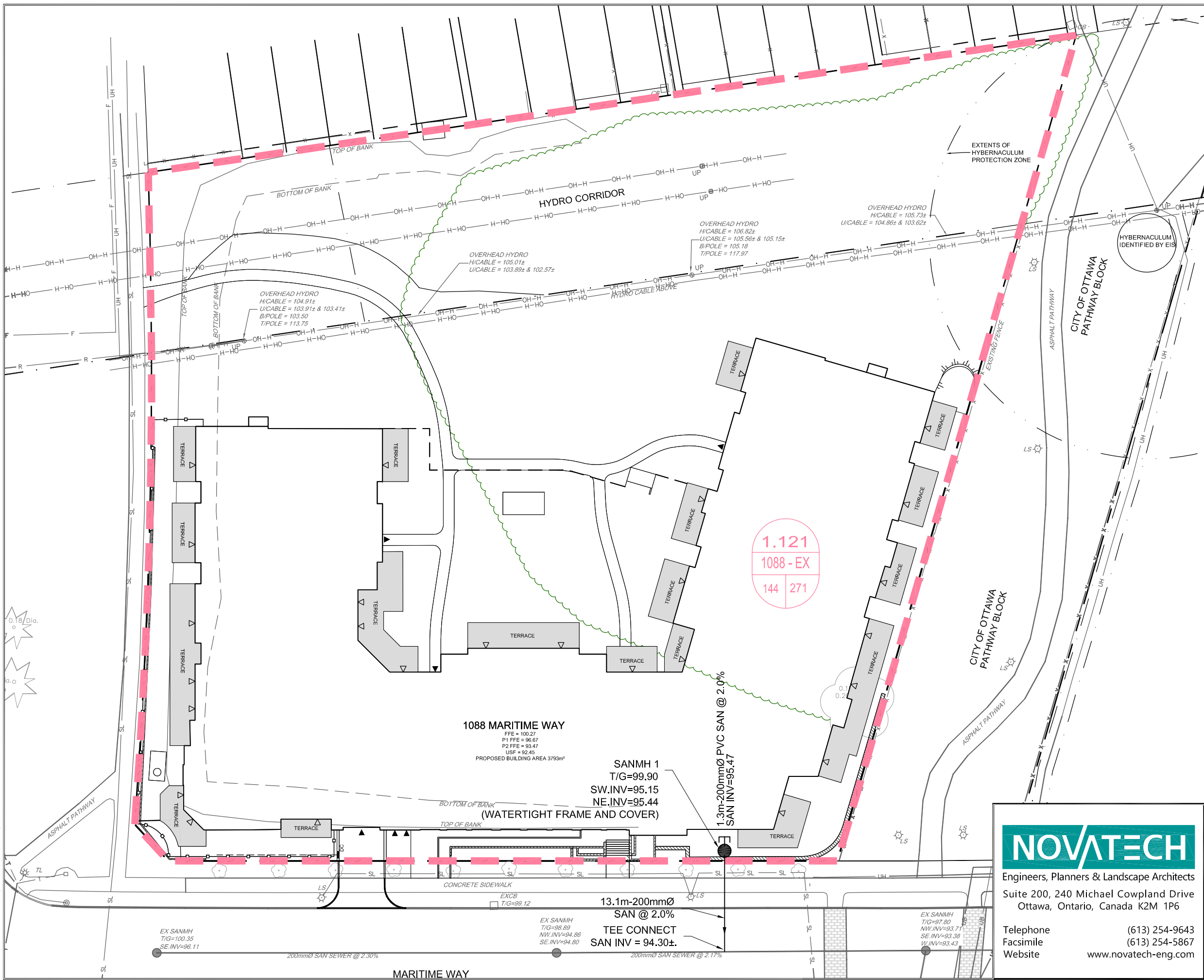
Matthew Hrehoriak, B.Eng., EIT

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 273 | Fax: 613.254.5867

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M:\2015\115198\cad\design\115198-GP.dwg, SAN, Jul 13, 2017 - 3:58pm, mhrehorlak



LEGEND

- 1.121
1088 - EX
144 | 271 SANITARY DRAINAGE AREA (ha)
- 1088 - EX LOCATION
- 144 | 271 UNITS / POPULATION
- SANMH
- SANITARY MANHOLE
- SANITARY SERVICE WITH FLOW DIRECTION

1088 MARITIME WAY
 FFE = 100.27
 P1 FFE = 96.67
 P2 FFE = 93.47
 USF = 92.45
 PROPOSED BUILDING AREA 3793m²

SANMH 1
 T/G=99.90
 SW.INV=95.15
 NE.INV=95.44
 (WATERTIGHT FRAME AND COVER)

1.3m-200mmØ PVC SAN @ 2.0%
 SAN INV=95.47

13.1m-200mmØ
 SAN @ 2.0%
 TEE CONNECT
 SAN INV = 94.30±

EX SANMH
 T/G=100.35
 SE.INV=96.11

EX SANMH
 T/G=98.89
 NW.INV=94.86
 SE.INV=94.80

EX SANMH
 T/G=97.60
 NW.INV=93.71
 SE.INV=93.38
 W.INV=93.43



Engineers, Planners & Landscape Architects
 Suite 200, 240 Michael Cowpland Drive
 Ottawa, Ontario, Canada K2M 1P6

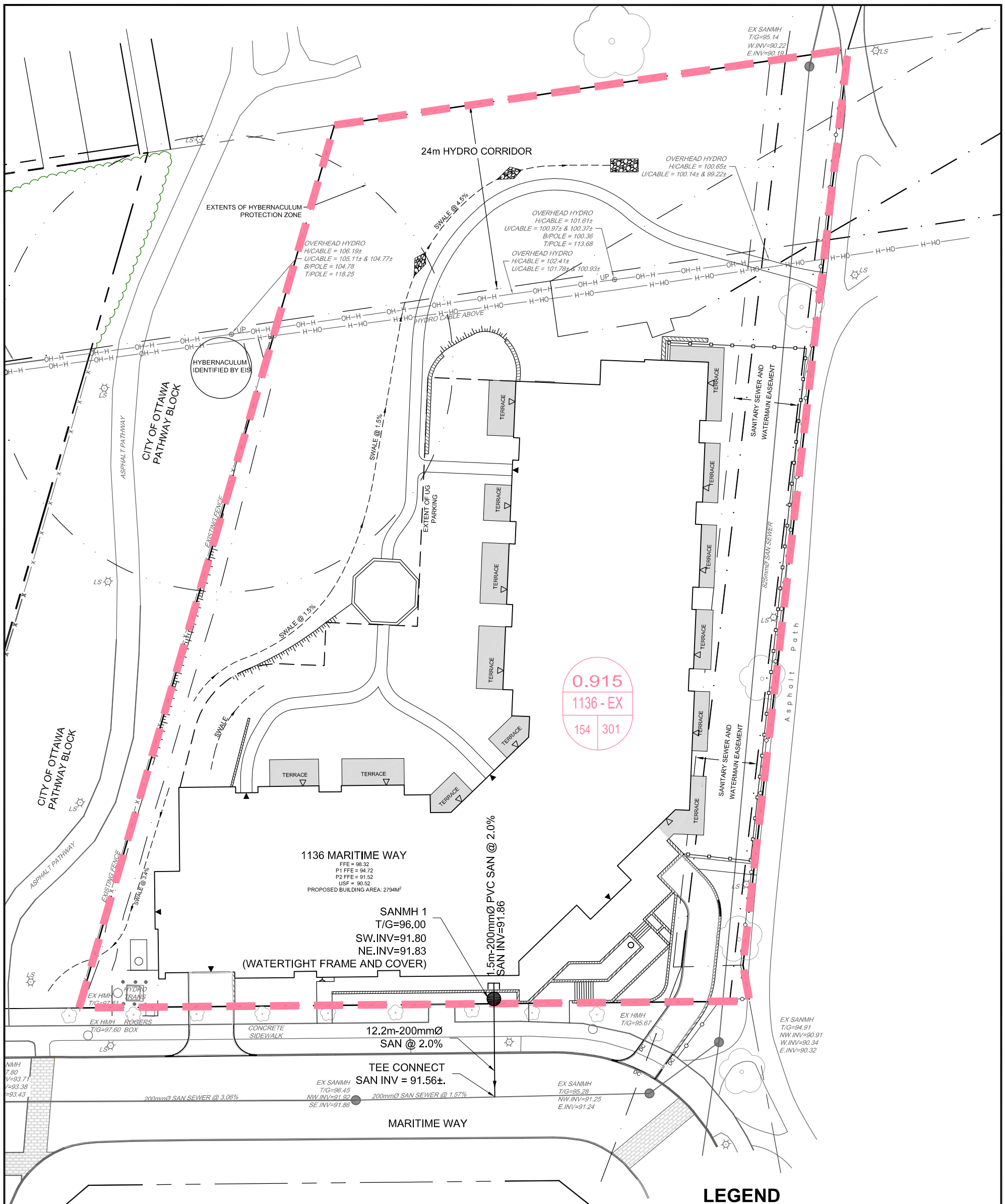
Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

1088 MARITIME WAY

SANITARY DRAINAGE AREA PLAN

SCALE 1 : 500

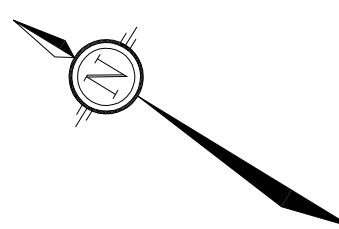
DATE JULY 2017 JOB 115198 FIGURE A3



0.915
1136-EX
154 301

LEGEND

- 0.915
1136-EX
154 301 SANITARY DRAINAGE AREA (ha)
- LOCATION
- UNITS / POPULATION
- SANMH
- SANITARY MANHOLE
- SANITARY SERVICE WITH FLOW DIRECTION



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1136 MARITIME WAY

SANITARY DRAINAGE AREA PLAN

SCALE 1 : 500 0 5 10 15 20

DATE JULY 2017 JOB 115199 FIGURE A3

SANITARY SEWER DESIGN SHEET

1250 Maritime Way
 Timberwalk Retirement Home
 Developer: Claridge Homes

Date: 31-Jul-17

Designed: CMS
 Checked: GJM

Location			RESIDENTIAL							INSTITUTIONAL				COMMECIAL			OTHER										INFILTRATION		Total Flow (L/s)	PIPE							
ID	From	To	1 Bedroom		2 Bedroom		Total (Residential)			Assisted Care				Convenience Store			Staff			Beauty Salon			Laundry			Dining				Total Area (ha)	Infil. Flow (L/s)	Size (mm)	Slope (%)	Length (m)	Capacity (l/s)	Full Flow Vel. (m/s)	Q/Q _{full} (%)
			Units	Pop.	Units	Pop.	Pop.	Peak Factor	Flow (L/s)	Units / Bed	Pop.	Peak Factor	Flow (L/s)	Area (m2)	Peak Factor	Flow (L/s)	Pop.	Peak Factor	Flow (L/s)	Stations	Peak Factor	Flow (L/s)	Machines	Peak Factor	Flow (L/s)	Seats	Peak Factor	Flow (L/s)									
Part A (current application)	BLD-1	MH101	92	129.0	8	17.0	146.0	4.0	2.37	54	60.0	1.5	0.47	50	1.5	0.004	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.48	0.13	3.33	200	2.00	2.5	48.4	1.49	6.9%
Part A (current application)	MH101	TEE-1	0	0.0	0	0.0	146.0	4.0	2.37	0	0.0	1.5	0.47	0	1.5	0.004	0	1.5	0.10	0	1.5	0.02	0	1.5	0.13	0	1.5	0.11	0.00	0.13	3.33	200	2.00	13.4	48.4	1.49	6.9%
Part B (future application)	BLD-2	MH103	0	0.0	110	231.0	231.0	4.0	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.41	0.11	3.86	200	2.00	2.5	48.4	1.49	8.0%
Part B (future application)	MH103	TEE-2	0	0.0	0	0.0	231.0	4.0	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.00	0.11	3.86	200	2.00	13.4	48.4	1.49	8.0%
TOTAL (Parts A + B)	-	-	92	129.0	118	248.0	377.0	4.0	6.11	54	60.0	1.5	0.47	50	1.5	0.004	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.89	0.25	7.18	200	2.00	2.5	48.4	1.49	14.8%

Design Parameters:

Residential	350 L/cap/day	Peaking Factor:
Institutional	450 L/bed/day	Residential Harmon Equation (max 4, min 2)
Commercial	5 L/m ² per day	Institutional 1.5
Staff	275 L/cap/day	Commercial 1.5
Beauty Salon	650 L/day per station	Other 1.5
Laundry	1200 L/day per machine	People/Unit:
Dining	115 L/seat/day	1.10 Assisted Care
Infiltration	0.28 L/s/ha	1.40 1 Bedroom
		2.10 2 Bedroom
		1.00 Studio

Notes:

1. The harmon peaking factor calculated for section 507 to 7A is 3.5 per JLR Design Sheet dated October 12th, 2016
2. Residential flows were used for senior apartments (350 L/cap/day, Harmon Peaking Factor)
3. Institutional flow used for assisted care units (450 L/bed/day, Peaking Factor = 1.5)
4. Future building assumed to be a 10 storey building comprised of 110 2 bedroom units

END OF J.L RICHARDS MEMORANDUM

Matthew Linton

From: Mike Traub <mike.traub@claridgehomes.com>
Sent: July-13-17 1:12 PM
To: Matthew Linton
Cc: Pascal Vendette; Conrad Stang
Subject: Re: FW: 1250 Maritime Way - Sanitary

Hi Matt,

There will be two stations in the hair salon and about 55 seats in the main dining room.

Let me know if you have any further questions.

Thanks,

Mike

On Wed, Jul 12, 2017 at 4:40 PM, Matthew Linton <m.linton@novatech-eng.com> wrote:

Pascal/Mike,

Could we obtain some clarifications on the following below? This is for city comments as they are stating our assumed values seem low.

Thanks,

Matthew Linton, CAD Technologist

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: [613.254.9643](tel:613.254.9643) Ext: 207 | Fax: [613.254.5867](tel:613.254.5867)

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Conrad Stang
Sent: July-12-17 3:55 PM

Matthew Linton

From: Pascal Vendette <pascal@neufarchitectes.com>
Sent: May-01-17 1:39 PM
To: Matthew Linton
Subject: RE: Unit Counts - Timberwalk (Maritime Way)

2nd floor : 28 assisted care units
3rd floor : 26 assisted care units
4th to 7th floor : 25 units
 6 suites
 17 1br
 2 br

NEUF
ARCHITECT(E)S



PASCAL VENDETTE

Technologue senior en architecture
Senior Architectural Technologist
T 514 847 1117 #269 F 514 847 2287 C 514 833 6005
630, boul. René-Lévesque O. 32^e étage, Montréal (QC) H3B 1S6
47 Clarence Street, suite 406, Ottawa (ON) K1N 9K1
NEUF ARCHITECTES SENCRL

Politiques de transmission et de confidentialité de NEUF architect(e)s
NEUF architect(e)s transmission and confidentiality policy

De : Matthew Linton [mailto:m.linton@novatech-eng.com]
Envoyé : 1 mai 2017 11:21
À : Pascal Vendette <pascal@neufarchitectes.com>
Objet : Unit Counts - Timberwalk (Maritime Way)

Pascal,

Can you please send us over either the calculated dwelling units (I see you have the dwelling units on drawing A050 however we need to know 1 bedroom, 2 bedroom, etc.) or floor plans for each of the floors for the retirement home?

Thanks,

Matthew Linton, CAD Technologist

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 207 | Fax: 613.254.5867

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Matthew Linton

From: Pascal Vendette <pascal@neufarchitectes.com>
Sent: July-13-17 9:49 AM
To: Matthew Linton
Cc: mike.traub@claridgehomes.com; Conrad Stang
Subject: RE: 1250 Maritime Way - Sanitary

Follow Up Flag: Follow up
Flag Status: Completed

Hi Matthew.
Sorry for the dealy ... it's crazy here.

Here is my response to item #1.
There are commercial washers-dryers in the basement ...

- 2 gas heater tumble dryers 75 lbs
- 1 high-performance washer extractor 65 lbs
- 1 cabinet hardmount washer extractor 20 lbs

... and residential type washer-dryer (one of each) on floors 4 to 7

Mike can you please take care of items #2 & #3.

Best regards,

NEUF
ARCHITECT(E)S



PASCAL VENDETTE

Technologue senior en architecture
Senior Architectural Technologist
T 514 847 1117 #269 F 514 847 2287 C 514 833 6005
630, boul. René-Lévesque O. 32^e étage, Montréal (QC) H3B 1S6
47 Clarence Street, suite 406, Ottawa (ON) K1N 9K1
NEUF ARCHITECTES SENCRL

Politiques de transmission et de confidentialité de NEUF architect(e)s
NEUF architect(e)s transmission and confidentiality policy

De : Matthew Linton [mailto:m.linton@novatech-eng.com]
Envoyé : 13 juillet 2017 09:38
À : Pascal Vendette <pascal@neufarchitectes.com>
Cc : mike.traub@claridgehomes.com; Conrad Stang <c.stang@novatech-eng.com>
Objet : RE: 1250 Maritime Way - Sanitary

Pascal,

Can we have some clarification on this?

SANITARY SEWER DESIGN SHEET

1250 Maritime Way
 Timberwalk Retirement Home
 Developer: Claridge Homes

Date: 30-Nov-17

Designed: CMS
 Revised: JDM
 Checked: GJM

Location			RESIDENTIAL							INSTITUTIONAL				COMMECIAL			OTHER									INFILTRATION		PIPE									
ID	From	To	1 Bedroom		2 Bedroom		Total (Residential)			Assisted Care				Convenience Store			Staff			Beauty Salon			Laundry			Dining			Total Area (ha)	Infiltr. Flow (L/s)	Total Flow (L/s)	Size (mm)	Slope (%)	Length (m)	Capacity (l/s)	Full Flow Vel. (m/s)	Q/Q _{full} (%)
			Units	Pop.	Units	Pop.	Pop.	Peak Factor	Flow (L/s)	Units / Bed	Pop.	Peak Factor	Flow (L/s)	Area (m2)	Peak Factor	Flow (L/s)	Pop.	Peak Factor	Flow (L/s)	Stations	Peak Factor	Flow (L/s)	Machines	Peak Factor	Flow (L/s)	Seats	Peak Factor	Flow (L/s)									
Part A (current application)	BLD1	MH4	92	129.0	8	17.0	146.0	4.0	2.37	54	60.0	1.5	0.47	100	1.5	0.009	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.48	0.13	3.33	200	2.66	9.6	55.8	1.72	6.0%
Part A (current application)	MH4	MH2	0	0.0	0	0.0	146.0	4.0	2.37	0	0.0	1.5	0.47	0	1.5	0.009	0	1.5	0.10	0	1.5	0.02	0	1.5	0.13	0	1.5	0.11	0.00	0.13	3.33	200	2.70	27.8	56.2	1.73	5.9%
Part B (future application)	FUT-BLD2	MH2	0	0.0	110	231.0	231.0	4.0	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.41	0.11	3.86	200	2.00	2.5	48.4	1.49	8.0%
TOTAL (Parts A + B)	MH2	EX MH	92	129.0	118	248.0	377.0	4.0	6.11	54	60.0	1.5	0.47	100	1.5	0.009	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.89	0.25	7.19	200	1.50	13.8	41.9	1.29	17.2%

Design Parameters:

Residential	350 L/cap/day
Institutional	450 L/bed/day
Commercial	5 L/m ² per day
Staff	275 L/cap/day
Beauty Salon	650 L/day per station
Laundy	1200 L/day per machine
Dining	115 L/seat/day
Infiltration	0.28 L/s/ha

Peaking Factor:

Residential	Harmon Equation (max 4, min 2)
Institutional	1.5
Commercial	1.5
Other	1.5

People/Unit:

1.10	Assisted Care
1.40	1 Bedroom
2.10	2 Bedroom
1.00	Studio

Notes:

1. The harmon peaking factor calculated for section 507 to 7A is 3.5 per JLR Design Sheet dated October 12th, 2016
2. Residential flows were used for senior apartments (350 L/cap/day, Harmon Peaking Factor)
3. Institutional flow used for assisted care units (450 L/bed/day, Peaking Factor = 1.5)
4. Future building assumed to be a 10 storey building comprised of 110 2 bedroom units



LOCATION			RESIDENTIAL												COMMERCIAL/INSTITUTIONAL			PLUGGED FLOW		R + C		PROPOSED SEWER									
STREET	FROM MH	TO MH	NUMBER OF UNITS									INDIVIDUAL		CUMULATIVE		PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW l/s	FLOW l/s	COMM FLOW l/s	PEAK EXTR. FLOW	PEAK DESIGN FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)	
			Houses			Extended Care		Hotel/Apt				POPUL. People	AREA ha	POPUL. People	AREA ha								l/s	L/S							
			Singles	Stacks	Towns	No. Units	Act Pop	No. Units	Act. Pop	Equ. Pop																					
B	85	87	19									65	1.19	65	1.19	4.000	1.05							0.340	1.39	116.9	250	0.40	39.237	0.77	4%
	87	89			24							65	0.82	130	2.01	4.000	2.11							0.573	2.68	116.7	250	1.41	73.667	1.45	4%
A	89	84			12							32	0.35	5859	43.97	3.181	75.49		28.985	25.161		163.820	20.792	285.26	79.0	825	0.12	518.749	0.94	55%	
C	80	82	19									65	1.08	65	1.08	4.000	1.05							0.308	1.36	120.0	250	0.40	39.237	0.77	3%
	82	84			25							67	0.83	132	1.91	4.000	2.14							0.544	2.68	118.5	250	1.20	67.960	1.34	4%
A	84	79			14							38	0.54	6029	46.42	3.169	77.39		28.985	25.161		163.820	21.490	287.86	79.0	825	0.12	518.749	0.94	55%	
D	75	76			17							46	0.37	46	0.37	4.000	0.75							0.105	0.85	57.0	250	0.40	39.237	0.77	2%
	76	77			20							54	0.29	100	0.66	4.000	1.62							0.188	1.81	78.4	250	0.40	39.237	0.77	5%
	77	79			13							35	0.63	135	1.29	4.000	2.19							0.368	2.56	117.7	250	0.81	55.835	1.10	5%
Park Easement	79	67											0.98	6164	48.69	3.160	78.89		28.985	25.161		163.820	22.099	289.97	55.0	825	0.12	518.749	0.94	56%	
	67	66			6							16	0.33	6180	49.02	3.159	79.07		28.985	25.161		163.820	22.192	290.25	70.0	825	0.12	518.749	0.94	56%	
BELLROCK DRIVE	70	73		12	14							70	2.56	70	2.56	4.000	1.13							0.728	1.86	87.2	250	0.40	39.237	0.77	5%
	73	74			12							32	0.54	102	3.1	4.000	1.65							0.882	2.53	80.3	250	0.40	39.237	0.77	6%
EASEMENT	74	62											0.31	102	3.41	4.000	1.65							0.970	2.62	39.9	250	0.40	39.237	0.77	7%
CAMBRAV LANE	62	66			25							68	0.48	170	3.89	4.000	2.75							1.107	3.86	100.5	250	0.40	39.237	0.77	10%
BISHOPS MILLS WAY	66	65			9							24	0.53	6374	53.44	3.146	81.22		28.985	25.161		163.820	23.450	293.65	62.0	825	0.12	518.749	0.94	57%	
SOUTH OF HWY 7	EX.	65										7792	191.6	7792	191.6	3.061	96.63					37.720	37.720	53.648	188.00	50.2	900	0.11	626.373	0.95	30%
BISHOPS MILLS WAY	65	64			2							5		14171	245.04	2.803	160.91		28.985	25.161		201.540	77.083	464.70	17.0	900	0.11	626.373	0.95	74%	
EDENVALE DRIVE	59	60			8							22	0.50	22	0.50	4.000	0.36							0.141	0.50	77.0	200	1.40	40.486	1.25	1%
KETTLEBY STREET	60	61			22							59	0.62	81	1.12	4.000	1.31							0.315	1.63	103.6	250	0.40	39.237	0.77	4%
CAMBRAV LANE	58	61			5							14	0.41	14	0.41	4.000	0.23							0.115	0.34	74.5	200	0.70	28.628	0.88	1%
KETTLEBY STREET	61	64			25							68	0.42	163	1.95	4.000	2.64							0.549	3.19	105.0	250	0.90	58.855	1.16	5%
BISHOPS MILLS WAY	64	63			3							8		14342	246.99	2.798	162.55		28.985	25.161		201.540	77.632	466.88	13.0	900	0.11	626.373	0.95	75%	
	63	57			10							27	0.68	14369	247.67	2.797	162.81		28.985	25.161		201.540	77.823	467.33	64.9	900	0.11	626.373	0.95	75%	
TER. BUNGALOW Ph. 2	51	53		48								130	0.94	130	0.94	4.000	2.11							0.264	2.37	122.3	200	0.70	28.628	0.88	8%
	53	54		4								11		141	0.94	4.000	2.28							0.264	2.55	13.6	200	0.70	28.628	0.88	9%
	54	55											0.27	141	1.21	4.000	2.28							0.340	2.63	36.7	200	0.70	28.628	0.88	9%
BISHOPS MILLS WAY	55	56	11									37	0.81	178	2.02	4.000	2.88							0.568	3.45	107.1	250	0.40	39.237	0.77	9%
	56	57	7		12							56	0.65	234	2.67	4.000	3.79							0.751	4.54	101.5	250	0.60	48.055	0.95	9%
PARK	57	34			1							3	0.37	14606	250.71	2.790	165.07		28.985	25.161		201.540	78.678	470.45	53.5	900	0.11	626.373	0.95	75%	
	34	33			3							8		14614	250.71	2.790	165.15		28.985	25.161		201.540	78.678	470.53	50.3	900	0.11	626.373	0.95	75%	

LOCATION			RESIDENTIAL												COMMERCIAL/INSTITUTIONAL			PLUGGED FLOW		R + C		PROPOSED SEWER								
STREET	FROM MH	TO MH	NUMBER OF UNITS									INDIVIDUAL		CUMULATIVE		PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW l/s	FLOW l/s	COMM FLOW l/s	PEAK EXTR. FLOW	PEAK DESIGN FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)
			Houses			Extended Care		Hotel/Apt				POPUL. People	AREA ha	POPUL. People	AREA ha															
			Singles	Stacks	Towns	No. Units	Act Pop	No. Units	Act. Pop	Equ. Pop																				
HAWSTONE	43	44		22							59	1.19	59	1.19	4.000	0.96							0.335	1.29	51.0	250	1.00	62.039	1.22	2%
	44	45		8							22	0.09	81	1.28	4.000	1.31							0.360	1.67	29.0	250	0.50	43.868	0.87	4%
EDENVALE	45	35										0.06	81	1.34	4.000	1.31							0.377	1.69	39.8	250	0.50	43.868	0.87	4%
BIRKENDALE DRIVE	35	36	7								24	1.18	105	2.52	4.000	1.70							0.709	2.41	93.2	250	0.37	37.737	0.74	6%
	36	37	13								44	0.79	149	3.31	4.000	2.41							0.931	3.35	77.1	250	0.37	37.737	0.74	9%
	37	33	2		3						15		164	3.31	4.000	2.66							0.931	3.59	17.9	250	0.40	39.237	0.77	9%
BIRKENDALE DRIVE	33	32			10						27	0.56	14805	254.58	2.784	166.96		28.985	25.161		201.540	79.767	473.43	72.7	900	0.11	626.373	0.95	76%	
TEESWATER STREET	30	31			16						43	0.66	43	0.66	4.000	0.70							0.186	0.88	75.1	250	0.40	39.237	0.77	2%
	31	32			19						51	0.41	94	1.07	4.000	1.52							0.301	1.82	77.9	250	0.40	39.237	0.77	5%
BIRKENDALE STREET	32	18			6						16	0.37	14915	256.02	2.781	168.01		28.985	25.161		201.540	80.172	474.88	44.4	900	0.11	626.373	0.95	76%	
	18	16			4						11		14926	256.02	2.780	168.11		28.985	25.161		201.540	80.172	474.99	44.4	900	0.11	626.373	0.95	76%	
COMMERCIAL PLAZA	19	17													4.000	0.00	0.520	0.520	0.451				0.146	0.60	26.5	150	0.90	15.073	0.83	4%
COLCHESTER SQUARE	17	16											0.10	0.10	4.000	0.00		0.520	0.451				0.174	0.63	33.2	250	0.40	39.237	0.77	2%
COLCHESTER SQUARE	16	15			10						27	0.56	14953	256.68	2.780	168.37		29.505	25.612		201.540	80.504	476.03	66.0	900	0.11	626.373	0.95	76%	
	15	14A			2						5		14958	256.68	2.779	168.42		29.505	25.612		201.540	80.504	476.07	25.8	900	0.11	626.373	0.95	76%	
ELSINORE LANE	39	28		32							86	0.53	86	0.53	4.000	1.39							0.149	1.54	56.7	250	1.00	62.039	1.22	2%
	28	24		18							49	1.47	135	2.00	4.000	2.19							0.563	2.75	43.0	250	0.40	39.237	0.77	7%
	24	23		12							32	0.14	167	2.14	4.000	2.71							0.602	3.31	34.0	250	0.40	39.237	0.77	8%
ELSINORE LANE	23	306		8							22	0.24	189	2.38	4.000	3.06							0.669	3.73	48.8	250	0.44	41.152	0.81	9%
ENDENVALE DRIVE	306	14-A										0.45	189	2.83	4.000	3.06							0.796	3.86	46.4	250	0.49	43.427	0.86	9%
COLCHESTER SQUARE	14-A	14											15147	259.51	2.774	170.21		29.505	25.612		201.540	81.300	478.66	14.7	900	0.11	626.373	0.95	76%	
	Church	14															0.520	0.520	0.451				0.146	0.60	35.0	150	1.00	15.888	0.87	4%
COLCHESTER SQUARE	14	11		4							11	0.16	15158	259.67	2.774	170.31		30.025	26.063		201.540	81.491	479.41	72.6	900	0.11	626.373	0.95	77%	
TERON	11	10											15158	259.67	2.774	170.31		30.025	26.063		201.540	81.491	479.41	29.6	900	0.11	626.373	0.95	77%	
	10	EX.										0.25	15158	259.92	2.774	170.31		30.025	26.063		201.540	81.562	479.48	72.3	900	0.11	626.373	0.95	77%	
TERON	O.P.P.	EX.																			0.780	0.780	0.78	100 FORCEMAIN						
TERON	EX.	EX. 2											15158	259.92	2.774	170.31		30.025	26.063		202.320	81.562	480.26	9.400	680.000	0.960	876.293	2.34	55%	

- Notes:
- 1) As per Kanata Town Centre Sanitary Trunk Sewer Study revised March 27, 1996 by Robinson Consultants Inc.
 - 2) Park or open space area.
 - 3) Equivalent population base on 208 rooms and 20 staff members.
 - 4) Allowance for an ultimate flow of 188 l/s to provide flexibility in future development as per Kanata Town Centre Sanitary Trunk Study.

LOCATION			RESIDENTIAL											COMMERCIAL/INSTITUTIONAL			PLUGGED FLOW		R + C		PROPOSED SEWER									
STREET	FROM MH	TO MH	NUMBER OF UNITS									INDIVIDUAL		CUMULATIVE		PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW l/s	FLOW l/s	COMM FLOW l/s	PEAK EXTR. FLOW	PEAK DESIGN FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)
			Houses			Extended Care		Hotel/Apt				POPUL. People	AREA ha	POPUL. People	AREA ha								l/s	L/S						
			Singles	Stacks	Towns	No. Units	Act Pop	No. Units	Act. Pop	Equ. Pop																				

5) Additional flow associated with hotel amenities including swimming pool with bathrooms and laundry as per design calculations for Block 1 provided by WSP (October 2016).
 6) Additional flow associated with overall amenities including beauty salon, staff, dining and laundry as per design calculations for 1250 Maritime Way (Timberwalk Retirement Home) provided by Novatech (July 31, 2017).

Design Parameters:

- 1) Q(e) = 0.28 L/sec/ha 3.4
- 2) Q(p) = (P x q x M / 86,400) 2.7
- 3) Q(d) = Q(p) + Q(e) 1.4 1BDR; 2.1 2 BDR.

Definitions:

P = Population 2.3
 q = Average per capita flow = 350 L/person/day
 M = Residential Peaking Factor (Harmon Formula from section 4.4.1 of the City Sewer Design Guidelines):

$$M = 1 + \frac{14}{(4 + \text{Pop}/1000)^{1/2}} - (\text{Maximum of } 4.0)$$

N = Commercial Peak Factor 1.5

Q(d) = Design Flow (L/sec)

Q(p) = Population Flow (L/sec)

Q(r) = Commercial Flow (L/sec)

Q(e) = Extraneous Flow (L/sec)

1200 Maritime Way SANITARY SEWER DESIGN SHEET					
Date	January 27, 2021				
Design	GMAC				
Job No.	wg. Referenc		Checked and Stamped:		
120144	120144-SAN				--

1200 Maritime Way
 SANITARY SEWER DESIGN SHEET OF DOWNSTREAM SEWERS
 JOB# 120144



B	85	87	19							65	1.19	65	1.19	4.000	1.05					0.340	1.39	116.9	250	0.40	39.237	0.77	4%	
	87	89		24						65	0.82	130	2.01	4.000	2.11					0.573	2.68	116.7	250	1.41	73.667	1.45	4%	
A	89	84		12						32	0.35	6921	45.25	3.111	87.23		28.985	25.161		163.820	21.157	297.37	79.0	825	0.12	518.749	0.94	57%
C	80	82	19							65	1.08	65	1.08	4.000	1.05					0.308	1.36	120.0	250	0.40	39.237	0.77	3%	
	82	84		25						67	0.83	132	1.91	4.000	2.14					0.544	2.68	118.5	250	1.20	67.960	1.34	4%	
A	84	79		14						38	0.54	7091	47.70	3.101	89.08		28.985	25.161		163.820	21.855	299.92	79.0	825	0.12	518.749	0.94	58%
D	75	76		17						46	0.37	46	0.37	4.000	0.75					0.105	0.85	57.0	250	0.40	39.237	0.77	2%	
	76	77		20						54	0.29	100	0.66	4.000	1.62					0.188	1.81	78.4	250	0.40	39.237	0.77	5%	
	77	79		13						35	0.63	135	1.29	4.000	2.19					0.368	2.56	117.7	250	0.81	55.835	1.10	5%	
Park Easement	79	67									0.98	7226	49.97	3.093	90.55		28.985	25.161		163.820	22.463	301.99	55.0	825	0.12	518.749	0.94	58%
	67	66		6						16	0.33	7242	50.30	3.092	90.72		28.985	25.161		163.820	22.557	302.26	70.0	825	0.12	518.749	0.94	58%
BELLROCK DRIVE	70	73		12	14					70	2.56	70	2.56	4.000	1.13					0.728	1.86	87.2	250	0.40	39.237	0.77	5%	
	73	74		12						32	0.54	102	3.1	4.000	1.65					0.882	2.53	80.3	250	0.40	39.237	0.77	6%	
EASEMENT	74	62									0.31	102	3.41	4.000	1.65					0.970	2.62	39.9	250	0.40	39.237	0.77	7%	
CAMBRAY LANE	62	66		25						68	0.48	170	3.89	4.000	2.75					1.107	3.86	100.5	250	0.40	39.237	0.77	10%	
BISHOPS MILLS WAY	66	65		9						24	0.53	7436	54.72	3.081	92.81		28.985	25.161		163.820	23.814	305.61	62.0	825	0.12	518.749	0.94	59%
SOUTH OF HWY 7	EX.	65								7792	191.6	7792	191.6	3.061	96.63				37.720	37.720	53.648	188.00	50.2	900	0.11	626.373	0.95	30%
BISHOPS MILLS WAY	65	64		2						5		15233	246.32	2.771	171.02		28.985	25.161		201.540	77.443	475.17	17.0	900	0.11	626.373	0.95	76%
EDENVALE DRIVE	59	60		8						22	0.50	22	0.50	4.000	0.36					0.141	0.50	77.0	200	1.40	40.486	1.25	1%	
KETTLEBY STREET	60	61		22						59	0.62	81	1.12	4.000	1.31					0.315	1.63	103.6	250	0.40	39.237	0.77	4%	
CAMBRAY LANE	58	61		5						14	0.41	14	0.41	4.000	0.23					0.115	0.34	74.5	200	0.70	28.628	0.88	1%	
KETTLEBY STREET	61	64		25						68	0.42	163	1.95	4.000	2.64					0.549	3.19	105.0	250	0.90	58.855	1.16	5%	
BISHOPS MILLS WAY	64	63		3						8		15404	248.27	2.767	172.64		28.985	25.161		201.540	77.992	477.33	13.0	900	0.11	626.373	0.95	76%
	63	57		10						27	0.68	15431	248.95	2.766	172.89		28.985	25.161		201.540	78.183	477.78	64.9	900	0.11	626.373	0.95	76%
TER. BUNGALOW Ph. 2	51	53		48						130	0.94	130	0.94	4.000	2.11					0.264	2.37	122.3	200	0.70	28.628	0.88	8%	
	53	54		4						11		141	0.94	4.000	2.28					0.264	2.55	13.6	200	0.70	28.628	0.88	9%	
	54	55									0.27	141	1.21	4.000	2.28					0.340	2.63	36.7	200	0.70	28.628	0.88	9%	
BISHOPS MILLS WAY	55	56	11							37	0.81	178	2.02	4.000	2.88					0.568	3.45	107.1	250	0.40	39.237	0.77	9%	
	56	57	7	12						56	0.65	234	2.67	4.000	3.79					0.751	4.54	101.5	250	0.60	48.055	0.95	9%	
PARK	57	34		1						3	0.37	15668	251.99	2.759	175.12		28.985	25.161		201.540	79.038	480.86	53.5	900	0.11	626.373	0.95	77%
	34	33		3						8		15676	251.99	2.759	175.20		28.985	25.161		201.540	79.038	480.94	50.3	900	0.11	626.373	0.95	77%
HAWSTONE	43	44		22						59	1.19	59	1.19	4.000	0.96					0.335	1.29	51.0	250	1.00	62.039	1.22	2%	
	44	45		8						22	0.09	81	1.28	4.000	1.31					0.360	1.67	29.0	250	0.50	43.868	0.87	4%	
EDENVALE	45	35									0.06	81	1.34	4.000	1.31					0.377	1.69	39.8	250	0.50	43.868	0.87	4%	
BIRKENDALE DRIVE	35	36	7							24	1.18	105	2.52	4.000	1.70					0.709	2.41	93.2	250	0.37	37.737	0.74	6%	
	36	37	13							44	0.79	149	3.31	4.000	2.41					0.931	3.35	77.1	250	0.37	37.737	0.74	9%	
	37	33	2	3						15		164	3.31	4.000	2.66					0.931	3.59	17.9	250	0.40	39.237	0.77	9%	
BIRKENDALE DRIVE	33	32		10						27	0.56	15867	255.86	2.754	176.99		28.985	25.161		201.540	80.127	483.82	72.7	900	0.11	626.373	0.95	77%
TEESWATER STREET	30	31		16						43	0.66	43	0.66	4.000	0.70					0.186	0.88	75.1	250	0.40	39.237	0.77	2%	

1200 Maritime Way
SANITARY SEWER DESIGN SHEET OF DOWNSTREAM SEWERS
JOB# 120144



	31	32			19					51	0.41	94	1.07	4.000	1.52					0.301	1.82	77.9	250	0.40	39.237	0.77	5%				
BIRKENDALE STREET	32	18			6					16	0.37	15977	257.30	2.751	178.03					28.985	25.161		201.540	80.532	485.26	44.4	900	0.11	626.373	0.95	77%
	18	16			4					11		15988	257.30	2.750	178.13					28.985	25.161		201.540	80.532	485.36	44.4	900	0.11	626.373	0.95	77%
COMMERCIAL PLAZA	19	17												4.000	0.00	0.520	0.520	0.451						0.146	0.60	26.5	150	0.90	15.073	0.83	4%
COLCHESTER SQUARE	17	16									0.10		0.10	4.000	0.00									0.174	0.63	33.2	250	0.40	39.237	0.77	2%
COLCHESTER SQUARE	16	15			10					27	0.56	16015	257.96	2.750	178.38					29.505	25.612		201.540	80.864	486.40	66.0	900	0.11	626.373	0.95	78%
	15	14A			2					5		16020	257.96	2.749	178.43					29.505	25.612		201.540	80.864	486.44	25.8	900	0.11	626.373	0.95	78%
ELSINORE LANE	39	28		32						86	0.53	86	0.53	4.000	1.39									0.149	1.54	56.7	250	1.00	62.039	1.22	2%
	28	24		18						49	1.47	135	2.00	4.000	2.19									0.563	2.75	43.0	250	0.40	39.237	0.77	7%
	24	23		12						32	0.14	167	2.14	4.000	2.71									0.602	3.31	34.0	250	0.40	39.237	0.77	8%
ELSINORE LANE	23	306		8						22	0.24	189	2.38	4.000	3.06									0.669	3.73	48.8	250	0.44	41.152	0.81	9%
ENDENVALE DRIVE	306	14-A									0.45	189	2.83	4.000	3.06									0.796	3.86	46.4	250	0.49	43.427	0.86	9%
COLCHESTER SQUARE	14-A	14										16209	260.79	2.744	180.20					29.505	25.612		201.540	81.660	489.01	14.7	900	0.11	626.373	0.95	78%
	Church	14																		0.520	0.520	0.451		0.146	0.60	35.0	150	1.00	15.888	0.87	4%
COLCHESTER SQUARE	14	11		4						11	0.16	16220	260.95	2.744	180.30					30.025	26.063		201.540	81.851	489.75	72.6	900	0.11	626.373	0.95	78%
TERON	11	10										16220	260.95	2.744	180.30					30.025	26.063		201.540	81.851	489.75	29.6	900	0.11	626.373	0.95	78%
	10	EX.									0.25	16220	261.20	2.744	180.30					30.025	26.063		201.540	81.922	489.82	72.3	900	0.11	626.373	0.95	78%
TERON	0.P.P.	EX.																					0.780	0.780	0.78	100 FORCEMAIN					
TERON	EX.	EX. 2										16220	261.20	2.744	180.30					30.025	26.063		202.320	81.922	490.60	9.400	680.000	0.960	876.293	2.34	56%

- Notes:**
- As per Kanata Town Centre Sanitary Trunk Sewer Study revised March 27, 1996 by Robinson Consultants Inc.
 - Park or open space area.
 - Equivalent population base on 208 rooms and 20 staff members.
 - Allowance for an ultimate flow of 188 l/s to provide flexibility in future development as per Kanata Town Centre Sanitary Trunk Study.
 - Additional flow associated with hotel amenities including swimming pool with bathrooms and laundry as per design calculations for Block 1 provided by WSP (October 2016).
 - Additional flow associated with overall amenities including beauty salon, staff, dining and laundry as per design calculations for 1250 Maritime Way (Timberwalk Retirement Home) provided by Novatech (July 31, 2017).
 - JLR Spreadsheet up-dated to include development flows from 1200 Maritime Way. Reference Appendix A of Serviceability Report for 1250 Maritime Way attached in Appendix of 1200 Maritime Way Serviceability Report (Novatech January 28, 2021)..

Design Parameters:

- Q(e) = 0.28 L/sec/ha **3.4**
- Q(p) = (P x q x M / 86,400) **2.7**
- Q(d) = Q(p) + Q(e) **1.4 1BDR; 2.1 2 BDR.**

Definitions:

P = Population **2.3**

q = Average per capita flow = 350 L/person/day

M = Residential Peaking Factor (Harmon Formula from section 4.4.1 of the City Sewer Design Guidelines):

$$M = 1 + \frac{14}{(4 + \text{Pop}/1000)}^{1/2} - 1 \text{ (Maximum of 4.0)}$$

N = Commercial Peak Factor 1.5

Q(d) = Design Flow (L/sec)

Q(p) = Population Flow (L/sec)

Q(r) = Commercial Flow (L/sec)

Q(e) = Extraneous Flow (L/sec)

1200 Maritime Way SANITARY SEWER DESIGN SHEET			
Date	January 27, 2021		
Design	GMAC		
Job No.	wg. Referenc	Checked and Stamped:	
120144	120144-SAN	--	

APPENDIX C

Stormwater Management Calculations

Runoff Coefficients

Drainage Area	Total Area (m ²)	Hard Surface Area		Grass Area		5-Year Runoff Coefficient	100-Year Runoff Coefficient
		Area (m ²)	C	Area (m ²)	C		
A-01	253.7	27.3	0.95	226.4	0.20	0.28	0.33
A-02	462.2	199.9	0.95	262.3	0.20	0.52	0.57
A-03	1578.1	32.6	0.95	1545.5	0.20	0.22	0.27
A-04	585.8	267.4	0.95	318.4	0.20	0.54	0.59
A-05	685.0	418.1	0.95	266.9	0.20	0.66	0.71
A-06	296.4	236.0	0.95	60.4	0.20	0.80	0.85
Total	3861.16	1181.3	0.95	2679.8	0.20	0.43	0.48

Controlled Flow

5 YR

Area No.	Area (ha)	C _{5yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zurn Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
A-01	0.0254	0.28	20.00	70.25	1.39	no control	-	-	-	-	-	-	-
A-02	0.0462	0.52	20.00	70.25	4.73	no control	-	-	-	-	-	-	-
A-03	0.1578	0.22	20.00	70.25	6.64	no control	-	-	-	-	-	-	-
A-04	0.0586	0.54	20.00	70.25	6.20	no control	-	-	-	-	-	-	-
A-05	0.0685	0.66	20.00	70.25	8.80	no control	-	-	-	-	-	-	-
A-06	0.0296	0.80	20.00	70.25	4.61	no control	-	-	-	-	-	-	-
CB Storage	-	-	-	-	-	-	-	-	-	-	-	-	-
Total:	0.3861				32.38								

100 YR

Area ID	Area (ha)	C _{100yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zurn Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
A-01	0.0254	0.33	10.00	178.56	4.16	no control	-	-	-	-	-	-	-
A-02	0.0462	0.57	10.00	178.56	13.18	no control	-	-	-	-	-	-	-
A-03	0.1578	0.27	10.00	178.56	20.80	no control	-	-	-	-	-	-	-
A-04	0.0586	0.59	10.00	178.56	17.22	no control	-	-	-	-	-	-	-
A-05	0.0685	0.71	10.00	178.56	24.07	no control	-	-	-	-	-	-	-
A-06	0.0296	0.85	20.00	119.95	8.37	no control	-	-	-	-	-	-	-
CB Storage	-	-	-	-	-	-	-	-	-	-	-	-	-
Total:	0.3861				87.81								

Note: In all cases, there is only one notch in the Zurn roof drain and flows through each drain is further reduced with and adjustable weir. See Zurn roof drains sheet and adjustable weir specification for more details on the reduction of flow.

55.36

Allowable release rate

Area	1.28 ha
C	0.8
tc	20 min
i ₅	70.25
Q allowable = 2.78 x C x i x A	
199.99 L/s	

tank all = 112.18 tank A = 70.44
 tank B = 41.74

REQUIRED STORAGE - 5-YEAR EVENT					
AREA	East Tower (incl. CB1/2)		: TANK		
OTTAWA IDF CURVE					
Area =	0.5400	ha	Qallow =	70.44	
C =	0.95		Vol(max) =	46.89	
Time (min)	Intensity (mm/hr)	Q Uncontrolled (L/s)	Q Controlled (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	201.34	0.00	130.90	39.27
10	104.19	148.59	0.00	78.15	46.89
15	83.56	119.16	0.00	48.72	43.85
20	70.25	100.19	0.00	29.75	35.70
25	60.90	86.85	0.00	16.41	24.61
30	53.93	76.91	0.00	6.47	11.64
35	48.52	69.19	0.00	-1.25	-2.62
40	44.18	63.01	0.00	-7.43	-17.82
45	40.63	57.94	0.00	-12.50	-33.74
50	37.65	53.70	0.00	-16.74	-50.22
55	35.12	50.09	0.00	-20.35	-67.15
60	32.94	46.98	0.00	-23.46	-84.45
65	31.04	44.27	0.00	-26.17	-102.05
70	29.37	41.89	0.00	-28.55	-119.92
75	27.89	39.77	0.00	-30.67	-138.00
80	26.56	37.88	0.00	-32.56	-156.28
85	25.37	36.18	0.00	-34.26	-174.73
90	24.29	34.64	0.00	-35.80	-193.33
95	23.31	33.24	0.00	-37.20	-212.06
100	22.41	31.96	0.00	-38.48	-230.91
105	21.58	30.78	0.00	-39.66	-249.86
110	20.82	29.70	0.00	-40.74	-268.91
115	20.12	28.69	0.00	-41.75	-288.05
120	19.47	27.76	0.00	-42.68	-307.27
125	18.86	26.90	0.00	-43.54	-326.56
130	18.29	26.09	0.00	-44.35	-345.92
135	17.76	25.34	0.00	-45.10	-365.35
140	17.27	24.63	0.00	-45.81	-384.83
145	16.80	23.96	0.00	-46.48	-404.36
150	16.36	23.33	0.00	-47.11	-423.95
155	15.95	22.74	0.00	-47.70	-443.58
160	15.56	22.18	0.00	-48.26	-463.26
165	15.18	21.65	0.00	-48.79	-482.97
170	14.83	21.15	0.00	-49.29	-502.73

East Tower

REQUIRED STORAGE - 100-YEAR EVENT					
AREA	East Tower (incl. CB1/2)		: TANK		
OTTAWA IDF CURVE					
Area =	0.5400	ha	Qallow =	70.44	
C =	1.00		Vol(max) =	131.56	
Time (min)	Intensity (mm/hr)	Q Uncontrolled (L/s)	Q Controlled (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	364.35	0.00	293.91	88.17
10	178.56	268.05	0.00	197.61	118.57
15	142.89	214.51	0.00	144.07	129.67
20	119.95	180.07	0.00	109.63	131.56
25	103.85	155.90	0.00	85.46	128.18
30	91.87	137.91	0.00	67.47	121.45
35	82.58	123.97	0.00	53.53	112.41
40	75.15	112.81	0.00	42.37	101.68
45	69.05	103.66	0.00	33.22	89.69
50	63.95	96.01	0.00	25.57	76.70
55	59.62	89.51	0.00	19.07	62.92
60	55.89	83.91	0.00	13.47	48.49
65	52.65	79.03	0.00	8.59	33.51
70	49.79	74.74	0.00	4.30	18.08
75	47.26	70.94	0.00	0.50	2.25
80	44.99	67.54	0.00	-2.90	-13.92
85	42.95	64.48	0.00	-5.96	-30.38
90	41.11	61.72	0.00	-8.72	-47.11
95	39.43	59.20	0.00	-11.24	-64.07
100	37.90	56.90	0.00	-13.54	-81.24
105	36.50	54.79	0.00	-15.65	-98.60
110	35.20	52.85	0.00	-17.59	-116.12
115	34.01	51.05	0.00	-19.39	-133.80
120	32.89	49.38	0.00	-21.06	-151.62
125	31.86	47.83	0.00	-22.61	-169.57
130	30.90	46.38	0.00	-24.06	-187.63
135	30.00	45.03	0.00	-25.41	-205.81
140	29.15	43.76	0.00	-26.68	-224.09
145	28.36	42.57	0.00	-27.87	-242.46
150	27.61	41.45	0.00	-28.99	-260.92
155	26.91	40.39	0.00	-30.05	-279.46
160	26.24	39.39	0.00	-31.05	-298.08
165	25.61	38.44	0.00	-32.00	-316.76
170	25.01	37.55	0.00	-32.89	-335.52

REQUIRED STORAGE - 5-YEAR EVENT					
AREA	West Tower (incl. CB3/4 & TD)		: TANK		
OTTAWA IDF CURVE					
Area =	0.3200	ha	Qallow =	41.74	
C =	0.95		Vol(max) =	27.79	
Time (min)	Intensity (mm/hr)	Q Uncontrolled (L/s)	Q Controlled (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	119.31	0.00	77.57	23.27
10	104.19	88.06	0.00	46.32	27.79
15	83.56	70.62	0.00	28.88	25.99
20	70.25	59.37	0.00	17.63	21.16
25	60.90	51.46	0.00	9.72	14.59
30	53.93	45.58	0.00	3.84	6.90
35	48.52	41.00	0.00	-0.74	-1.55
40	44.18	37.34	0.00	-4.40	-10.56
45	40.63	34.34	0.00	-7.40	-19.99
50	37.65	31.82	0.00	-9.92	-29.76
55	35.12	29.68	0.00	-12.06	-39.79
60	32.94	27.84	0.00	-13.90	-50.04
65	31.04	26.24	0.00	-15.50	-60.47
70	29.37	24.82	0.00	-16.92	-71.05
75	27.89	23.57	0.00	-18.17	-81.77
80	26.56	22.45	0.00	-19.29	-92.60
85	25.37	21.44	0.00	-20.30	-103.53
90	24.29	20.53	0.00	-21.21	-114.55
95	23.31	19.70	0.00	-22.04	-125.65
100	22.41	18.94	0.00	-22.80	-136.82
105	21.58	18.24	0.00	-23.50	-148.05
110	20.82	17.60	0.00	-24.14	-159.34
115	20.12	17.00	0.00	-24.74	-170.68
120	19.47	16.45	0.00	-25.29	-182.07
125	18.86	15.94	0.00	-25.80	-193.50
130	18.29	15.46	0.00	-26.28	-204.98
135	17.76	15.01	0.00	-26.73	-216.48
140	17.27	14.59	0.00	-27.15	-228.03
145	16.80	14.20	0.00	-27.54	-239.60
150	16.36	13.83	0.00	-27.91	-251.21
155	15.95	13.48	0.00	-28.26	-262.84
160	15.56	13.15	0.00	-28.59	-274.50
165	15.18	12.83	0.00	-28.91	-286.18
170	14.83	12.54	0.00	-29.20	-297.89

West Tower

REQUIRED STORAGE - 100-YEAR EVENT					
AREA	West Tower (incl. CB3/4 & TD)		: TANK		
OTTAWA IDF CURVE					
Area =	0.3200	ha	Qallow =	41.74	
C =	1.00		Vol(max) =	77.96	
Time (min)	Intensity (mm/hr)	Q Uncontrolled (L/s)	Q Controlled (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	215.91	0.00	174.17	52.25
10	178.56	158.85	0.00	117.11	70.26
15	142.89	127.12	0.00	85.38	76.84
20	119.95	106.71	0.00	64.97	77.96
25	103.85	92.38	0.00	50.64	75.96
30	91.87	81.73	0.00	39.99	71.97
35	82.58	73.46	0.00	31.72	66.62
40	75.15	66.85	0.00	25.11	60.26
45	69.05	61.43	0.00	19.69	53.16
50	63.95	56.89	0.00	15.15	45.46
55	59.62	53.04	0.00	11.30	37.29
60	55.89	49.72	0.00	7.98	28.74
65	52.65	46.83	0.00	5.09	19.87
70	49.79	44.29	0.00	2.55	10.72
75	47.26	42.04	0.00	0.30	1.34
80	44.99	40.02	0.00	-1.72	-8.24
85	42.95	38.21	0.00	-3.53	-17.99
90	41.11	36.57	0.00	-5.17	-27.91
95	39.43	35.08	0.00	-6.66	-37.96
100	37.90	33.72	0.00	-8.02	-48.13
105	36.50	32.47	0.00	-9.27	-58.41
110	35.20	31.32	0.00	-10.42	-68.80
115	34.01	30.25	0.00	-11.49	-79.27
120	32.89	29.26	0.00	-12.48	-89.83
125	31.86	28.34	0.00	-13.40	-100.47
130	30.90	27.49	0.00	-14.25	-111.17
135	30.00	26.69	0.00	-15.05	-121.94
140	29.15	25.93	0.00	-15.81	-132.77
145	28.36	25.23	0.00	-16.51	-143.66
150	27.61	24.56	0.00	-17.18	-154.60
155	26.91	23.94	0.00	-17.80	-165.58
160	26.24	23.34	0.00	-18.40	-176.62
165	25.61	22.78	0.00	-18.96	-187.69
170	25.01	22.25	0.00	-19.49	-198.80

APPENDIX D

Fire Demand Calculations

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 120144

Project Name: 1200 Maritime Way - East Tower

Date: 1/22/2021

Input By: Jazmine Gauthier

Reviewed By: Greg MacDonald

Legend

Input by User

No Information or Input Required

Building Description: 28 Storey Building with 7 Storey Podium

Fire Resistive Construction

Step		Choose		Value Used	Total Fire Flow (L/min)	
Base Fire Flow						
1	Construction Material		Multiplier			
	Coefficient related to type of construction C	Wood frame		1.5		0.6
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Modified Fire resistive construction (2 hrs)	Yes	0.6		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area					
	A	Podium Level Footprint (m ²)	2204			
		Total Floors/Storeys (Podium)	7			
		Tower Footprint (m ²)	742			
		Total Floors/Storeys (Tower)	28			
		Protected Openings (1 hr)	Yes			
		Area of structure considered (m ²)		3,306		
F	Base fire flow without reductions			8,000		
	$F = 220 C (A)^{0.5}$					
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge		Reduction/Surcharge		6,800	
	(1)	Non-combustible		-25%		-15%
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction		Reduction		-3,400	
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%		-30%
		Standard Water Supply	Yes	-10%		-10%
		Fully Supervised System	Yes	-10%		-10%
Cumulative Total			-50%			
5	Exposure Surcharge (cumulative %)		Surcharge		2,720	
	(3)	North Side	30.1- 45 m			5%
		East Side	0 - 3 m			25%
		South Side	> 45.1m			0%
		West Side	20.1 - 30 m			10%
Cumulative Total			40%			
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min		L/min	6,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	100	
				or	1,585	
7	Storage Volume	Required Duration of Fire Flow (hours)		Hours	2	
		Required Volume of Fire Flow (m ³)		m ³	720	

FUS - Fire Flow Calculations - User Guide - Fire Resistant

<p>Novatech Project #: 120144 Project Name: 1200 Maritime Way - East Date: 1/22/2021 Input By: Jazmine Gauthier Reviewed By: Greg MacDonald</p>	<ul style="list-style-type: none"> • Please use the notes below as a guide when completing the FUS Fire Flow Calculations • When in doubt, confirm construction material, firewalls, etc. with architect/owner • When in doubt, err on conservative side
--	---

Note: This form only applies for Fire Resistant

Enter a description of the building or unit being considered, i.e. use/most stringent condition/address

Summary	
Construction Type	Fire Resistant Construction
Floor Area Considered	3,306 m ²
Occupancy Reduction	-15%
Sprinkler Reduction	-50%
Exposure Surcharge	40%
Total Fire Flow	6,000 L/min

Base Fire Flow

1	<p>Construction Material Does not apply for this form Does not apply for this form Does not apply for this form Only Use if can be confirmed with client/architect (ISO Cl 5) Only Use if can be confirmed with client/architect (ISO Cl 6)</p>	<p>Project Manager Review Date: _____ Name: _____ Signature: _____</p>
----------	---	---

Floor Area

If considered gross floor area, then enter 1 floor/storey. If Fire wall, then reduce footprint accordingly.

Un-Protected = number of floors above first 2, up to max of 10 floors total

Protected = number of additional immediately adjoining floors to be considered, up to 2

Do vertical openings have minimum 1 hour rating between floors? Confirm this with the architect.

For unprotected openings scenario only, can be mix of podium and tower

Reductions or Surcharges

Occupancy hazard reduction or surcharge

3
 Residential - with no garage
 Residential - with garage
 General Commercial - Generally, no reduction
 Check usage with FUS
 Check usage with FUS

Sprinkler Reduction

4
 Only Use if can be confirmed with client/architect
 Only Use if can be confirmed with client/architect
 Only Use if can be confirmed with client/architect

Exposure Surcharge (cumulative %)

5
 For Fire walls: FUS considers a Fire wall to have a minimum 2 hour rating per NBC.

Results

6
 NOTE: Refer to City Technical Bulletin ISDTB-2014-02 for additional considerations to cap this value at 10,000L/min

If IGPM is needed, divide USGPM by 1.20095

7
 For Rural areas, or where required

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 120144

Project Name: 1200 Maritime Way - West Tower

Date: 1/22/2021

Input By: Jazmine Gauthier

Reviewed By: Greg MacDonald

Legend

Input by User

No Information or Input Required

Building Description: 30 Storey Building with 7 Storey Podium

Fire Resistive Construction

Step		Choose		Value Used	Total Fire Flow (L/min)	
Base Fire Flow						
1	Construction Material		Multiplier			
	Coefficient related to type of construction C	Wood frame		1.5		
		Ordinary construction		1		
		Non-combustible construction		0.8		
		Modified Fire resistive construction (2 hrs)	Yes	0.6		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area					
	A	Podium Level Footprint (m ²)	1947			
		Total Floors/Storeys (Podium)	7			
		Tower Footprint (m ²)	906			
		Total Floors/Storeys (Tower)	30			
		Protected Openings (1 hr)	Yes			
		Area of structure considered (m ²)		2,921		
F	Base fire flow without reductions			7,000		
	$F = 220 C (A)^{0.5}$					
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge		Reduction/Surcharge		5,950	
	(1)	Non-combustible		-25%		-15%
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction		Reduction		-2,975	
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%		-30%
		Standard Water Supply	Yes	-10%		-10%
		Fully Supervised System	Yes	-10%		-10%
	Cumulative Total		-50%			
5	Exposure Surcharge (cumulative %)		Surcharge		2,380	
	(3)	North Side	> 45.1m			0%
		East Side	20.1 - 30 m			10%
		South Side	30.1- 45 m			5%
		West Side	0 - 3 m			25%
	Cumulative Total		40%			
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min		L/min	5,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	83
				or	USGPM	1,321
7	Storage Volume	Required Duration of Fire Flow (hours)		Hours	1.75	
		Required Volume of Fire Flow (m ³)		m ³	525	

FUS - Fire Flow Calculations - User Guide - Fire Resistant															
<p>Novatech Project #: 120144 Project Name: 1200 Maritime Way - West Tower Date: 1/22/2021 Input By: Jazmine Gauthier Reviewed By: Greg MacDonald</p>	<ul style="list-style-type: none"> • Please use the notes below as a guide when completing the FUS Fire Flow Calculations • When in doubt, confirm construction material, firewalls, etc. with architect/owner • When in doubt, err on conservative side 														
<p>Note: This form only applies for Fire Resistant</p> <p>Enter a description of the building or unit being considered, i.e. use/most stringent condition/address</p>															
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2" style="text-align: center;">Summary</th> </tr> <tr> <th style="width: 70%;">Construction Type</th> <th style="width: 30%;">Fire Resistant Construction</th> </tr> </thead> <tbody> <tr> <td>Floor Area Considered</td> <td style="text-align: right;">2,921 m²</td> </tr> <tr> <td>Occupancy Reduction</td> <td style="text-align: right;">-15%</td> </tr> <tr> <td>Sprinkler Reduction</td> <td style="text-align: right;">-50%</td> </tr> <tr> <td>Exposure Surcharge</td> <td style="text-align: right;">40%</td> </tr> <tr> <td>Total Fire Flow</td> <td style="text-align: right;">5,000 L/min</td> </tr> </tbody> </table>		Summary		Construction Type	Fire Resistant Construction	Floor Area Considered	2,921 m ²	Occupancy Reduction	-15%	Sprinkler Reduction	-50%	Exposure Surcharge	40%	Total Fire Flow	5,000 L/min
Summary															
Construction Type	Fire Resistant Construction														
Floor Area Considered	2,921 m ²														
Occupancy Reduction	-15%														
Sprinkler Reduction	-50%														
Exposure Surcharge	40%														
Total Fire Flow	5,000 L/min														
<p>Base Fire Flow</p>															
<p>1 Construction Material Does not apply for this form Does not apply for this form Does not apply for this form Only Use if can be confirmed with client/architect (ISO Cl 5) Only Use if can be confirmed with client/architect (ISO Cl 6)</p>	<p>Project Manager Review Date: _____ Name: _____ Signature: _____</p>														
<p>2 Floor Area If considered gross floor area, then enter 1 floor/storey. If Fire wall, then reduce footprint accordingly. Un-Protected <input type="text" value="8"/> = number of floors above first 2, up to max of 10 floors total Protected <input type="text" value="2"/> = number of additional immediately adjoining floors to be considered, up to 2 Do vertical openings have minimum 1 hour rating between floors? Confirm this with the architect.</p> <p>For unprotected openings scenario only, can be mix of podium and tower</p>															
<p>Reductions or Surcharges</p>															
<p>3 Occupancy hazard reduction or surcharge Residential - with no garage Residential - with garage General Commercial - Generally, no reduction Check usage with FUS Check usage with FUS</p>															
<p>4 Sprinkler Reduction Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect</p>															
<p>5 Exposure Surcharge (cumulative %) For Fire walls: FUS considers a Fire wall to have a minimum 2 hour rating per NBC.</p>															
<p>Results</p>															
<p>6 NOTE: Refer to City Technical Bulletin ISDTB-2014-02 for additional considerations to cap this value at 10,000L/min If IGPM is needed, divide USGPM by 1.20095</p>															
<p>7 For Rural areas, or where required</p>															

APPENDIX E

Servicing Study Guidelines Checklist

Development Servicing Study Checklist

4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Executive Summary (for larger reports only).	NA		
Date and revision number of the report.	Y	p.1	
Location map and plan showing municipal address, boundary, and layout of proposed development.	Y	Dwgs	GP, GR, STM
Plan showing the site and location of all existing services.	Y	Dwg	GP
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Y	Intro	
Summary of Pre-consultation Meetings with City and other approval agencies.	N		
Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Y	Report	All sections
Statement of objectives and servicing criteria.	Y	Report	
Identification of existing and proposed infrastructure available in the immediate area.	Y	Dwg	GP
Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	NA		
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighboring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Y	Report	

Development Servicing Study Checklist

4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	NA		
Proposed phasing of the development, if applicable.	Y		
Reference to geotechnical studies and recommendations concerning servicing.	Y	Report	
All preliminary and formal site plan submissions should have the following information:			
Metric scale	Y		All Drawings
North arrow (including construction North)	Y		All Drawings
Key plan	Y		All Drawings
Name and contact information of applicant and property owner	Y		Drawings/Report
Property limits including bearings and dimensions	Y		Report
Existing and proposed structures and parking areas	Y		All Drawings
Easements, road widening and rights-of-way	Y		All Drawings
Adjacent street names	Y		All Drawings

Development Servicing Study Checklist

4.2 Water	Addressed (Y/N/NA)	Section	Comments
Confirm consistency with Master Servicing Study, if available.	NA		
Availability of public infrastructure to service proposed development.	Y		
Identification of system constraints.	NA		
Identify boundary conditions.	NA		
Confirmation of adequate domestic supply and pressure.	NA		
Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Y		Appendix
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	NA		
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design.	NA		
Address reliability requirements such as appropriate location of shut-off valves.	Y		Drawings
Check on the necessity of a pressure zone boundary modification.	NA		
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	NA		
Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Y	Report	
Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	NA		
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Y	Report	
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	NA		

Development Servicing Study Checklist

4.3 Wastewater	Addressed (Y/N/NA)	Section	Comments
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Y	Report	
Confirm consistency with Master Servicing Study and/or justifications for deviations.	NA		
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	NA		
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Y	Report	Drawings
Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Y	Report	Appendix
Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	NA		
Description of proposed sewer network including sewers, pumping stations, and forcemains.	Y		
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	NA		
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	NA		
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	NA		
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	NA		
Special considerations such as contamination, corrosive environment etc.	NA		

Development Servicing Study Checklist

4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Description of drainage outlets and downstream constraints including legality of outlet (i.e. municipal drain, right-of-way, watercourse, or private property).	Y	Report	
Analysis of the available capacity in existing public infrastructure.	NA		
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns and proposed drainage patterns.	Y		GR, STM
Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Y	Report	
Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Y	Report	
Description of stormwater management concept with facility locations and descriptions with references and supporting information.	Y	Report	
Set-back from private sewage disposal systems.	NA		
Watercourse and hazard lands setbacks.	Y		
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N		
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N		
Storage requirements (complete with calcs) and conveyance capacity for 5 yr and 100 yr events.	Y		Appendix
Identification of watercourse within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	NA		
Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Y		Appendix
Any proposed diversion of drainage catchment areas from one outlet to another.	NA		
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and SWM facilities.	Y	Report	And Appendix
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Y	Report	And Appendix

Development Servicing Study Checklist

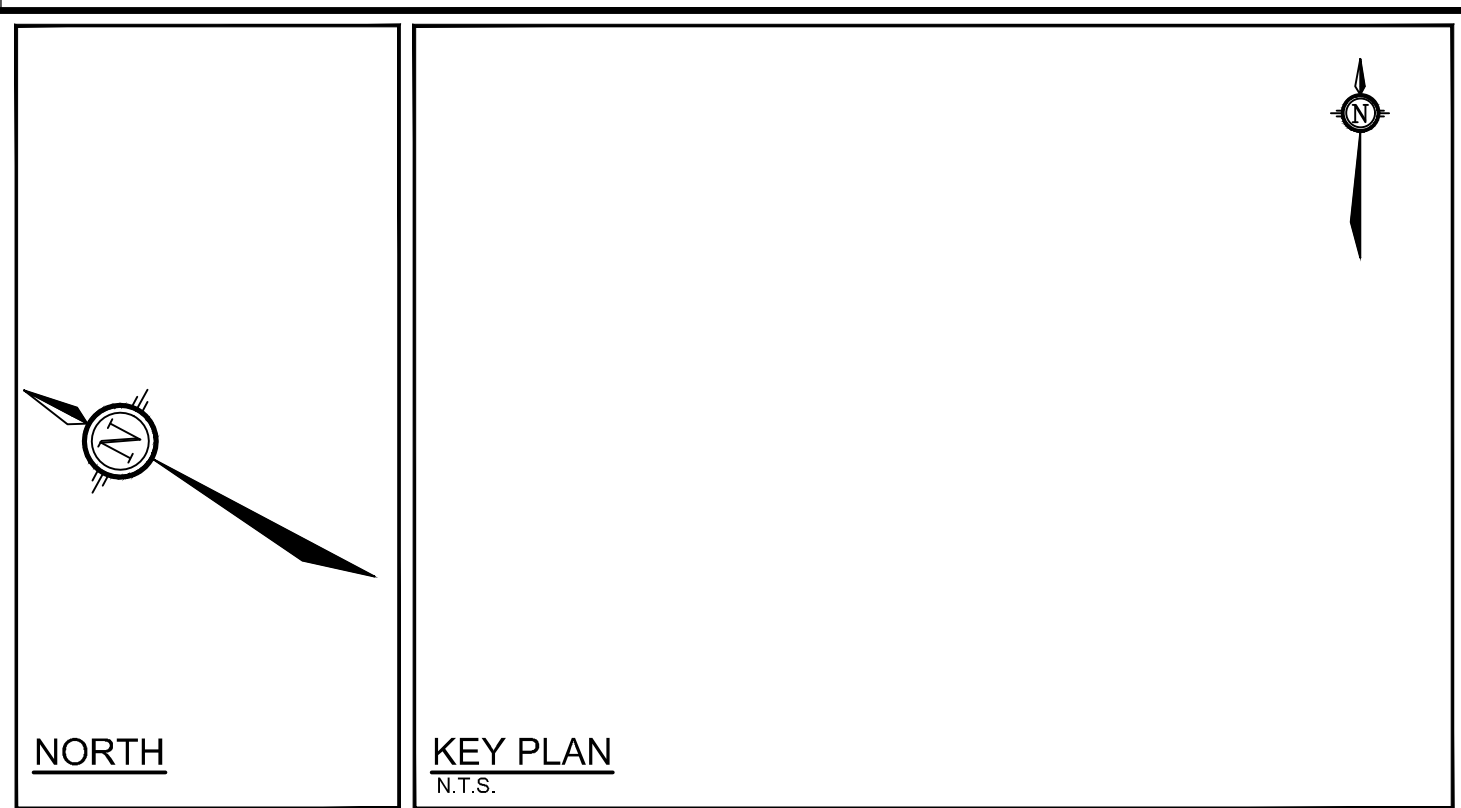
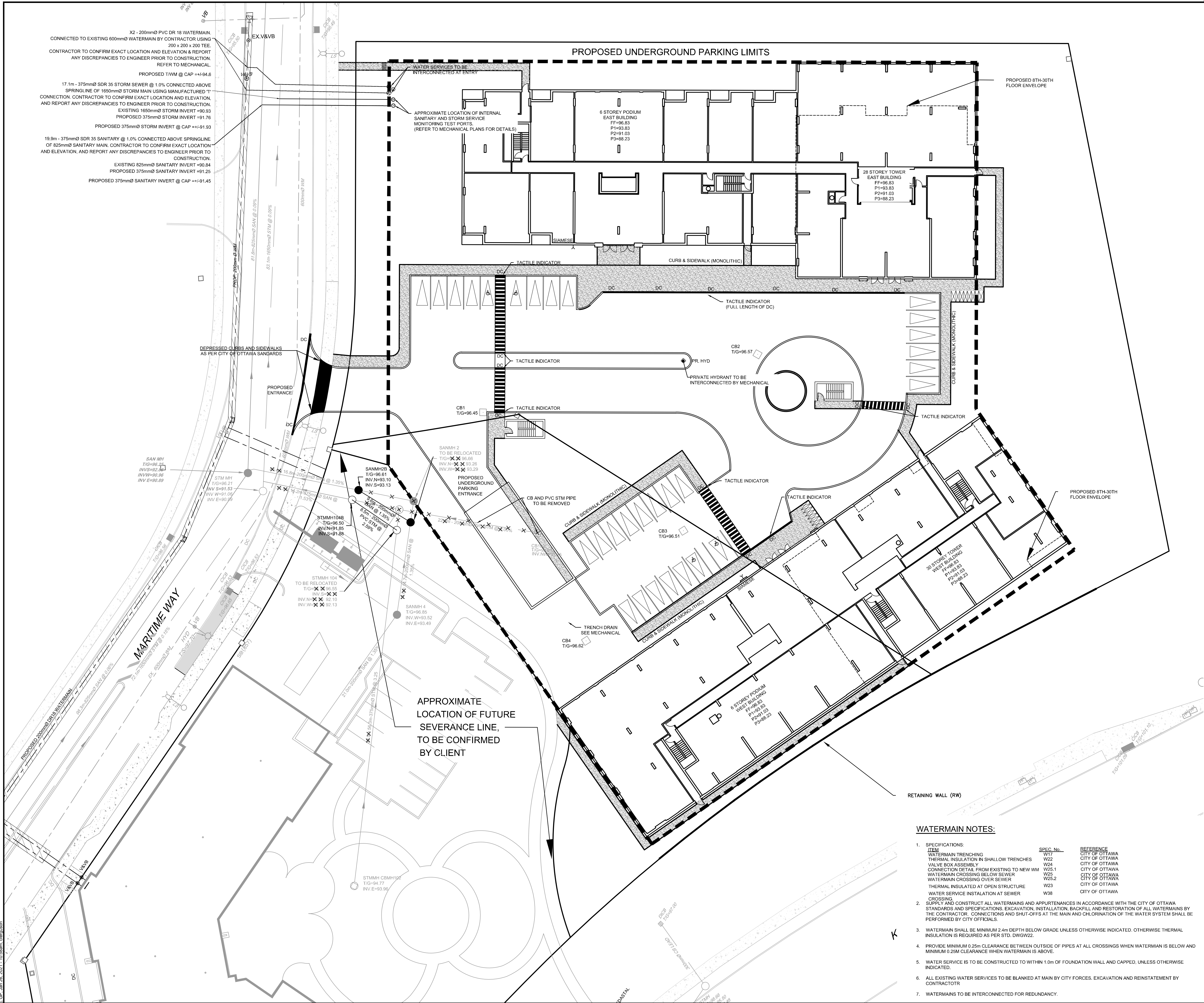
4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Identification of municipal drains and related approval requirements.	Y	Report	
Description of how the conveyance and storage capacity will be achieved for the development.	Y	Report	
100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Y		Appendix
Inclusion of hydraulic analysis including HGL elevations.	Y		Appendix
Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Y	Report	Drawings
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	NA		
Identification of fill constrains related to floodplain and geotechnical investigation.	NA		

Development Servicing Study Checklist

4.5 Approval and Permit Requirements	Addressed (Y/N/NA)	Section	Comments
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	NA		
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	NA		
Changes to Municipal Drains.	NA		
Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	NA		

4.6 Conclusion	Addressed (Y/N/NA)	Section	Comments
Clearly stated conclusions and recommendations.	Y	Report	
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	NA		
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario.	Y	Report	

DRAWINGS



PROPOSED PROPERTY LINE	VC	EXISTING WATERMAIN VALVE CHAMBER
PROPOSED WATERMAIN	VB	EXISTING WATERMAIN SHUT-OFF VALVE BOX
PROPOSED VALVE BOX	WV	EXISTING WATERMAIN
PROPOSED STORM MANHOLE	VS	EXISTING HYDRANT C/W LEAD & SHUT OFF VALVE BOX
PROPOSED SANITARY SEWER	GV	EXISTING GAS VALVE
PROPOSED FOUNDATION DRAIN	GA	EXISTING GAS MAIN
UNDERGROUND PARKING LIMITS	XB	EXISTING ABANDONED GAS MAIN
PROPOSED STORM SEWER	HRUP	EXISTING BELL CONDUIT
PROPOSED STORM TRENCH DRAIN	HRUP	EXISTING OVER HEAD WIRE
PROPOSED CAP	GT	EXISTING HYDROUTILITY POLE
PROPOSED BACKWATER VALVE	TAM	EXISTING GUY WIRE
EXISTING LEGAL ADJACENT LINE	CP	EXISTING TRAFFIC MANHOLE
EXISTING STANDARD IRON BAR / CONTROL POINT	CP	EXISTING JOINT USE STREET LIGHT
EXISTING FLOW DIRECTION OF SEWERS	CP	EXISTING STREET LIGHT
EXISTING SANITARY MANHOLE & SEWER	CP	EXISTING TRAFFIC HAND HOLE
EXISTING STORM MANHOLE & SEWER	CP	EXISTING TRAFFIC SIGN
EXISTING CATCH BASIN	CP	EXISTING DITCH INLET CATCH BASIN
EXISTING BUILDING ENVELOPE	EX HYD	EXISTING HYDRANT
	PR HYD	PROPOSED HYDRANT
	SI	PROPOSED SIAMESE CONNECTION

- GENERAL NOTES:**
- COORDINATE AND SCHEDULE ALL WORK WITH OTHER TRADES AND CONTRACTORS.
 - DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME RESPONSIBILITY FOR ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THIS DRAWING.
 - OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF OTTAWA BEFORE COMMENCING CONSTRUCTION.
 - BEFORE COMMENCING CONSTRUCTION OBTAIN AND PROVIDE PROOF OF COMPREHENSIVE ALL RISK AND OPERATIONAL LIABILITY INSURANCE FOR \$5,000,000.00. INSURANCE POLICY TO NAME OWNERS, ENGINEERS AND ARCHITECTS AS CO-INSURED.
 - RESTORE ALL DISTURBED AREAS ON-SITE AND OFF-SITE, INCLUDING TRENCHES AND SURFACES ON PUBLIC ROAD ALLOWANCES TO EXISTING CONDITIONS OR BETTER TO THE SATISFACTION OF THE CITY OF OTTAWA AND ENGINEER.
 - REMOVE FROM SITE ALL EXCESS EXCAVATED MATERIAL, ORGANIC MATERIAL AND DEBRIS UNLESS OTHERWISE INSTRUCTED BY ENGINEER. EXCAVATE AND REMOVE FROM SITE ANY CONTAMINATED MATERIAL. ALL CONTAMINATED MATERIAL SHALL BE DISPOSED OF AT A LICENSED LANDFILL FACILITY.
 - ALL ELEVATIONS ARE GEODETIC.
 - REFER TO GEOTECHNICAL REPORT (No. PB5281-1, DATED JULY 16TH, 2020), PREPARED BY PATerson, FOR SUBSURFACE CONDITIONS, CONSTRUCTION RECOMMENDATIONS, AND GEOTECHNICAL INSPECTION REQUIREMENTS, THE GEOTECHNICAL CONSULTANT IS TO REVIEW ON-SITE CONDITIONS AFTER EXCAVATION PRIOR TO PLACEMENT OF THE GRANULAR MATERIAL.
 - REFER TO ARCHITECTS AND LANDSCAPE ARCHITECT'S DRAWINGS FOR BUILDING AND HARDSURFACE AREAS AND DIMENSIONS.
 - REFER TO STORMWATER MANAGEMENT REPORT PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
 - SAW CUT AND KEY GRIND ASPHALT AT ALL ROAD CUTS AND ASPHALT TIE IN POINTS AS PER CITY OF OTTAWA STANDARDS (R10).
 - PROVIDE LINE-PARKING PAINTING.
 - CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GENERAL PLAN OF SERVICES INDICATING ALL SERVING AS-BUILT INFORMATION SHOWN ON THIS PLAN. AS-BUILT INFORMATION MUST INCLUDE: PIPE MATERIAL, SIZES, LENGTHS, SLOPES, INVERT AND TIG ELEVATIONS, STRUCTURE LOCATIONS, VALVE AND HYDRANT LOCATIONS, TWM ELEVATIONS AND ANY ALIGNMENT CHANGES, ETC.

- SEWER NOTES:**
- SPECIFICATIONS:

ITEM	SPEC. No.	REFERENCE
CATCH BASIN (600x600mm)	705.010	OPSD
STORM / SANITARY MANHOLE (1200)	701.010	OPSD
CB, FRAME & COVER	400.020	OPSD
STORM / SANITARY MH FRAME & COVER	401.010	OPSD
SEWER TRENCH - BEDDING (GRANULAR A)	S6, S7, W17	CITY OF OTTAWA / OPSD
COVER (GRANULAR A OR GRANULAR B TYPE I, WITH MAXIMUM PARTICLE SIZE=25mm)		
STORM SEWER	PVC DR 35	CITY OF OTTAWA
SANITARY SEWER	PVC DR 35	CITY OF OTTAWA
CATCH BASIN LEAD	PVC DR 35	CITY OF OTTAWA
SEWER SERVICE CONNECTION - RIGID PIPE	S11	CITY OF OTTAWA
SEWER SERVICE ABANDONMENT	S11.4	CITY OF OTTAWA
 - INSULATE ALL PIPES (SAN/STM) THAT HAVE LESS THAN 1.5m COVER WITH 50mmX1200mm H-40 INSULATION. PROVIDE 150mm CLEARANCE BETWEEN PIPE AND INSULATION.
 - SERVICES ARE TO BE CONSTRUCTED TO 1.0m FROM FACE OF BUILDING AT A MINIMUM SLOPE OF 1.0%.
 - PIPE BEDDING, COVER AND BACKFILL ARE TO BE COMPACTED TO AT LEAST 98% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY. THE USE OF CLEAR CRUSHED STONE AS A BEDDING LAYER SHALL NOT BE PERMITTED.
 - FLEXIBLE CONNECTIONS ARE REQUIRED FOR CONNECTING PIPES TO MANHOLES (FOR EXAMPLE KORAN-SEAL, PSX POSITIVE SEAL AND DURASEAL). THE CONCRETE CRADLE FOR THE PIPE CAN BE ELIMINATED.
 - THE OWNER SHALL REQUIRE THAT THE SITE SERVICING CONTRACTOR PERFORM FIELD TESTS FOR QUALITY CONTROL OF ALL SANITARY SEWERS. LEAKAGE TESTING SHALL BE COMPLETED IN ACCORDANCE WITH OPS 410.07.16, 410.07.16.04 AND 407.07.24. DYE TESTING IS TO BE COMPLETED ON ALL SANITARY SERVICES TO CONFIRM PROPER CONNECTION TO THE SANITARY SEWER MAIN. THE FIELD TESTS SHALL BE PERFORMED IN THE PRESENCE OF A CERTIFIED PROFESSIONAL ENGINEER WHO SHALL SUBMIT A CERTIFIED COPY OF THE TEST RESULTS.
 - STORM MANHOLES AND CBMHs ARE TO HAVE 300mm SUMPUS UNLESS OTHERWISE INDICATED.
 - CONTRACTOR TO TELEVIEW (CCTV) ALL PROPOSED SEWERS, 200mm OR GREATER PRIOR TO BASE COURSE ASPHALT. UPON COMPLETION OF CONTRACT, THE CONTRACTOR IS RESPONSIBLE TO FLUSH AND CLEAN ALL SEWERS & APPURTENANCES.
 - FULL PORT BACKWATER VALVES ARE REQUIRED ON THE SANITARY SERVICES, INSTALLED AS PER THE MANUFACTURER'S RECOMMENDATIONS AND A BACKWATER VALVE IS REQUIRED ON THE STORM SERVICES / FOUNDATION DRAINS FOR EACH BUILDING, INSTALLED AS PER STD. DWG514
 - REINSTATE ALL EXISTING PAVEMENT, CURB AND BOULEVARDS AS PER CITY OF OTTAWA R10.
 - ALL EXISTING SANITARY AND STORM SERVICES ARE TO BE CAPPED AT THE PROPERTY LINE TO THE SATISFACTION OF THE CITY OF OTTAWA'S SEWER OPERATION.
 - MONITORING TEST PORTS FOR BUILDING SERVICES TO BE INSTALLED IN PARKING GARAGE.

- WATERMAIN NOTES:**
- SPECIFICATIONS:

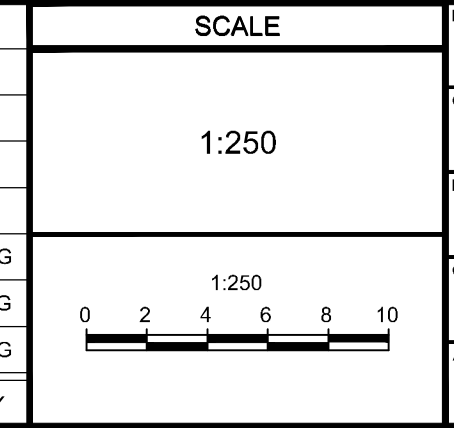
ITEM	SPEC. No.	REFERENCE
WATERMAIN TRENCHING	W17	CITY OF OTTAWA
THERMAL INSULATION IN SHALLOW TRENCHES	W22	CITY OF OTTAWA
VALVE BOX ASSEMBLY	W24	CITY OF OTTAWA
CONNECTION DETAIL FROM EXISTING TO NEW WM	W25.1	CITY OF OTTAWA
WATERMAIN CROSSING BELOW SEWER	W25.2	CITY OF OTTAWA
WATERMAIN CROSSING OVER SEWER	W25.2	CITY OF OTTAWA
THERMAL INSULATED AT OPEN STRUCTURE	W23	CITY OF OTTAWA
WATER SERVICE INSTALLATION AT SEWER	W38	CITY OF OTTAWA
 - CROSSING: SUPPLY AND CONSTRUCT ALL WATERMANS AND APPURTENANCES IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARDS AND SPECIFICATIONS. EXCAVATION, INSTALLATION, BACKFILL AND RESTORATION OF ALL WATERMANS BY THE CONTRACTOR. CONNECTIONS AND SHUT-OFFS AT THE MAIN AND CHLORINATION OF THE WATER SYSTEM SHALL BE PERFORMED BY CITY OFFICIALS.
 - WATERMAIN SHALL BE MINIMUM 2.4m DEPTH BELOW GRADE UNLESS OTHERWISE INDICATED. OTHERWISE THERMAL INSULATION IS REQUIRED AS PER STD. DWG22.
 - PROVIDE MINIMUM 0.25m CLEARANCE BETWEEN OUTSIDE OF PIPES AT ALL CROSSINGS WHEN WATERMAIN IS BELOW AND MINIMUM 0.25m CLEARANCE WHEN WATERMAIN IS ABOVE.
 - WATER SERVICE IS TO BE CONSTRUCTED TO WITHIN 1.0m OF FOUNDATION WALL AND CAPPED, UNLESS OTHERWISE INDICATED.
 - ALL EXISTING WATER SERVICES TO BE BLANKED AT MAIN BY CITY FORCES. EXCAVATION AND REINSTATEMENT BY CONTRACTOR.
 - WATERMANS TO BE INTERCONNECTED FOR REDUNDANCY.

NOTE: THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

CLARIDGE HOMES
SUITE 2001,
210 GLADSTONE AVENUE,
OTTAWA, ONTARIO
K2P 0Y6.



No.	REVISION	DATE	BY
3	SUBMITTED WITH ZONING/SITE PLAN APPLICATIONS	JAN25/21	JAG
2	RE-ISSUED FOR COORDINATION	JAN 22/21	JAG
1	ISSUED FOR COORDINATION	JAN15/21	JAG

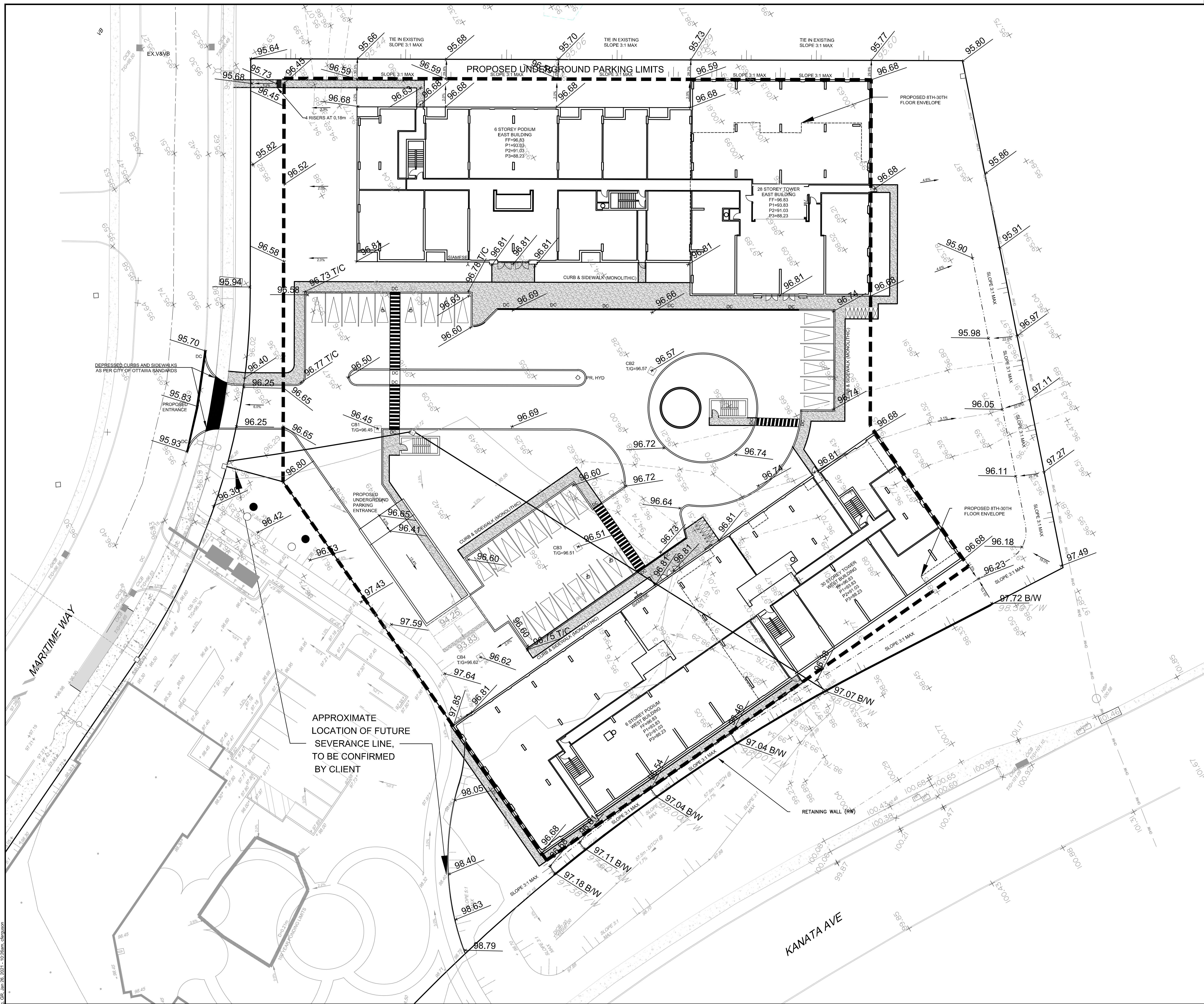


FOR REVIEW ONLY	DATE	BY
DESIGN		JAG
CHECKED		GJM
DRAWN		CJF
CHECKED		JAG
APPROVED		GJM



LOCATION CITY OF OTTAWA 1200 MARITIME WAY	PROJECT No. 120144
DRAWING NAME GENERAL PLAN OF SERVICES	REV # 3
DATE JAN 28, 2021	DRAWING No. 120144 - GP

NOVATECH 120144-GP.dwg (120144-GP.dwg) GP, Jan 26, 2021, 10:58am, c:\temp\...



KEY PLAN
N.T.S.

LEGEND

—	PROPOSED PROPERTY LINE	—	EXISTING GAS VALVE	
- - -	EXISTING LEGAL ADJACENT LINE	- - -	EXISTING GAS MAIN	
—	EXISTING ELEVATION	- - -	EXISTING ABANDONED GAS MAIN	
—	PROPOSED GRADES	- - -	EXISTING BELL CONDUIT	
—	PROPOSED SWALE GRADES	- - -	EXISTING OVER HEAD WIRE	
—	PROPOSED TOP OF RETAINING WALL	- - -	EXISTING TRAFFIC CONDUIT	
—	PROPOSED BOTTOM OF RETAINING WALL	- - -	EXISTING HYDRO UTILITY POLE	
—	PROPOSED UNDERGROUND PARKING RAMP GRADES	- - -	HR/UP	EXISTING GUY WIRE
—	PROPOSED SLOPE AND FLOW DIRECTION	- - -	TMV	EXISTING TRAFFIC MANHOLE
—	PROPOSED FINISHED FLOOR ELEVATION	- - -	—	EXISTING JOINT USE STREET LIGHT
—	PROPOSED DEPRESSED CURB	- - -	—	EXISTING STREET LIGHT
—	TRENCH DRAIN	- - -	—	EXISTING TRAFFIC HAND HOLE
—	PROPOSED TERRACING SLOPE 3:1 MAX	- - -	—	EXISTING TRAFFIC SIGN
—	PROPOSED PONDING AREA	- - -	—	EXISTING STANDARD IRON BAR
—	DIRECTION OF MAJOR OVERLAND FLOW	- - -	—	EXISTING CATCHBASIN
—	PROPOSED SWALE	- - -	—	EXISTING DITCH INLET CATCHBASIN
		- - -	—	EXISTING BUILDING ENVELOPE
		- - -	—	PROPOSED FILTER CLOTH IN EXISTING CATCHBASIN
		- - -	—	PROPOSED SILT FENCE

- GENERAL NOTES:**
- COORDINATE AND SCHEDULE ALL WORK WITH OTHER TRADES AND CONTRACTORS.
 - DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME RESPONSIBILITY FOR ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THIS DRAWING.
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 - BEFORE COMMENCING CONSTRUCTION OBTAIN AND PROVIDE PROOF OF COMPREHENSIVE, ALL RISK AND OPERATIONAL LIABILITY INSURANCE FOR \$5,000,000.00. INSURANCE POLICY TO NAME OWNERS, ENGINEERS AND ARCHITECTS AS CO-INSURED.
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 - ALL ELEVATIONS ARE GEODETIC.
 - REFER TO GEOTECHNICAL REPORT (No. PB5281-1, DATED JUL 16TH, 2020), PREPARED BY PATERSON FOR SUBSURFACE CONDITIONS, CONSTRUCTION RECOMMENDATIONS, AND GEOTECHNICAL INSPECTION REQUIREMENTS. THE GEOTECHNICAL CONSULTANT IS TO REVIEW ON-SITE CONDITIONS AFTER EXCAVATION PRIOR TO PLACEMENT OF THE GRANULAR MATERIAL.
 - REFER TO ARCHITECT'S AND LANDSCAPE ARCHITECT'S DRAWINGS FOR BUILDING AND HARDSURFACE AREAS AND DIMENSIONS.
 - REFER TO SERVICEABILITY AND STORMWATER MANAGEMENT REPORT PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
 - SAW CUT AND KEY GRIND ASPHALT AT ALL ROAD CUTS AND ASPHALT TIE IN POINTS AS PER CITY OF OTTAWA STANDARDS (R10).
 - PROVIDE LINE/PARKING PAINTING.
 - ALL MATERIALS AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARDS AND SPECIFICATIONS AND ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS. ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS WILL APPLY WHERE NO CITY STANDARDS ARE AVAILABLE.
 - ALL PRIVATE APPROACHES MUST BE CONSTRUCTED AS PER CITY SPECIFICATION SC13.
 - ALL EXISTING INFRASTRUCTURE (STORM AND SANITARY NETWORK & MANHOLES) ON-SITE TO BE REMOVED.

- GRADING NOTES:**
- ALL TOPSOIL, ORGANIC OR DELETERIOUS MATERIAL MUST BE ENTIRELY REMOVED FROM BENEATH THE PROPOSED PAVED AREAS AS DIRECTED BY THE SITE ENGINEER OR GEOTECHNICAL ENGINEER.
 - EXPOSED SUBGRADES IN PROPOSED PAVED AREAS SHOULD BE PROOF ROLLED WITH A LARGE STEEL DRUM ROLLER AND INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF GRANULARS.
 - ANY SOFT AREAS EVIDENT FROM THE PROOF ROLLING SHOULD BE SUB-EXCAVATED AND REPLACED WITH SUITABLE MATERIAL THAT IS FROST COMPATIBLE WITH THE EXISTING SOILS AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
 - THE GRANULAR BASE SHOULD BE COMPACTED TO AT LEAST 100% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE. ANY ADDITIONAL GRANULAR FILL USED BELOW THE PROPOSED PAVED AREA SHOULD BE COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE.
 - MINIMUM OF 2% GRADE FOR ALL GRASS AREAS UNLESS OTHERWISE NOTED.
 - MAXIMUM TERRACING GRADE TO BE 3:1 UNLESS OTHERWISE NOTED.
 - ALL GRADES BY CURBS ARE EDGE OF PAVEMENT GRADES UNLESS OTHERWISE INDICATED.
 - ALL CURBS SHALL BE BARRIER CURB (150mm) UNLESS OTHERWISE NOTED AND CONSTRUCTED AS PER CITY OF OTTAWA STANDARDS (SC1.1).
 - REFER TO LANDSCAPE PLAN FOR PLANTING AND OTHER LANDSCAPE FEATURE DETAILS.
 - CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GRADING PLAN INDICATING AS-BUILT ELEVATIONS OF ALL DESIGN GRADES SHOWN ON THIS PLAN.

- PAVEMENT STRUCTURE:**
- | | |
|---|------------------------|
| □ | LIGHT DUTY |
| □ | 50mm H.L.3 |
| □ | 150mm GRAN "A" |
| □ | 300mm GRAN "B" TYPE II |
| □ | HEAVY DUTY |
| □ | 40mm H.L.3 |
| □ | 50mm H.L.2 |
| □ | 150mm GRAN "A" |
| □ | 450mm GRAN "B" TYPE II |

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

CLARIDGE HOMES
CLARIDGE HOMES SUITE 2001,
210 GLADSTONE AVENUE,
OTTAWA, ONTARIO
K2P 0Y6.



No.	REVISION	DATE	BY
3.	SUBMITTED WITH ZONING/SITE PLAN APPLICATIONS	JAN25/21	JAG
2.	RE-ISSUED FOR COORDINATION	JAN22/21	JAG
1.	SITE PLAN COORDINATION	JAN15/21	JAG

SCALE

1:250

0 2 4 6 8 10

DESIGN	FOR REVIEW ONLY
CHECKED: C.J.F.	
DRAWN: J.A.G.	
CHECKED: C.J.F.	
DRAWN: J.A.G.	
APPROVED: J.A.G.	
APPROVED: G.J.M.	

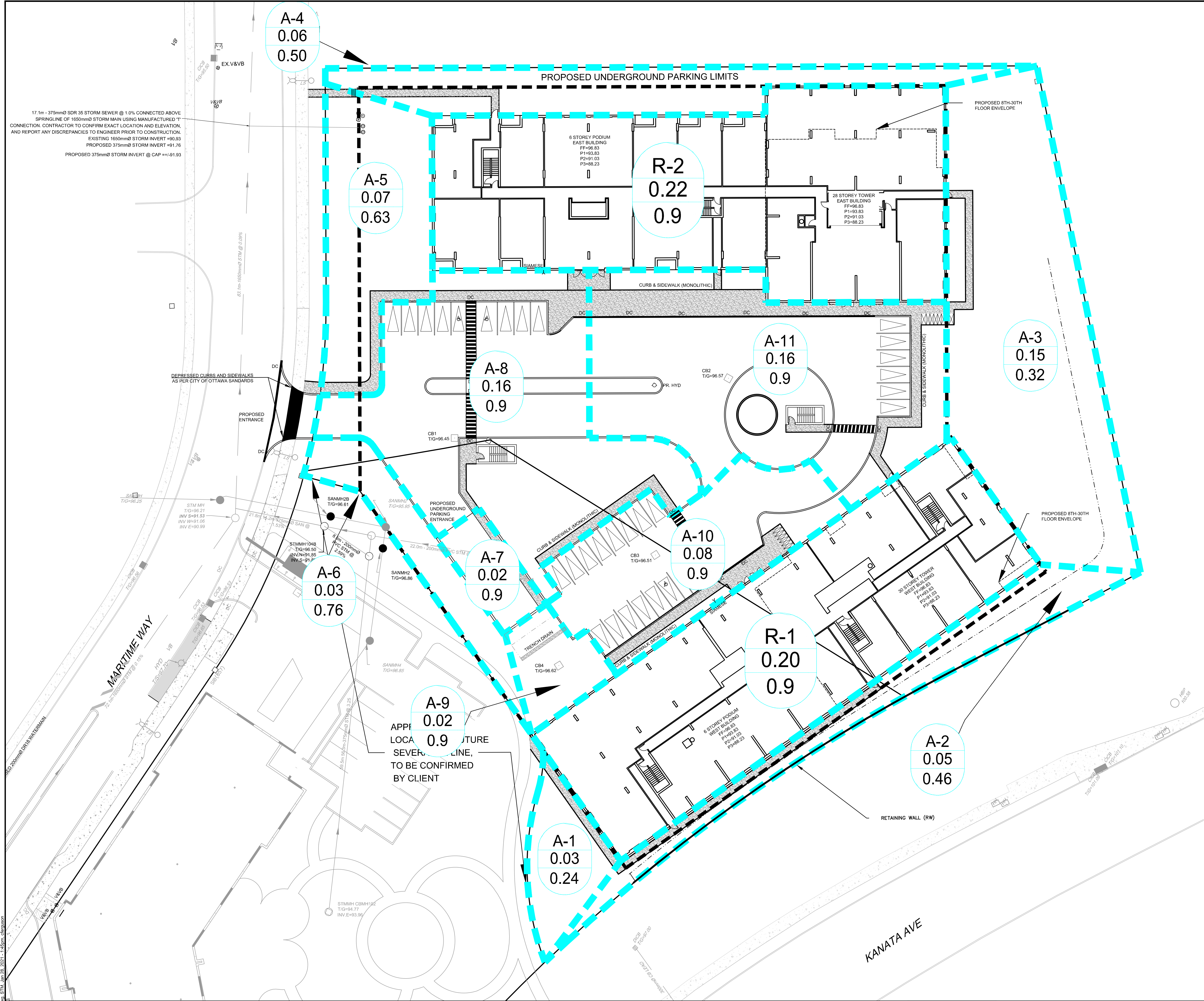
NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

PROFESSIONAL ENGINEER
G.J. MacDONALD
Jan 28, 2021
PROVINCE OF ONTARIO

LOCATION
CITY OF OTTAWA
1200 MARITIME WAY

DRAWING NAME
GRADING AND EROSION SEDIMENT CONTROL PLAN

PROJECT No. 120144
REV # 3
DRAWING No. 120144-GR



17.1m - 375mm Ø SDR 35 STORM SEWER @ 1.0% CONNECTED ABOVE SPRINGLINE OF 150mm Ø STORM MAIN USING MANUFACTURED "T" CONNECTION. CONTRACTOR TO CONFIRM EXACT LOCATION AND ELEVATION, AND REPORT ANY DISCREPANCIES TO ENGINEER PRIOR TO CONSTRUCTION.
 EXISTING 150mm Ø STORM INVERT =+90.83
 PROPOSED 375mm Ø STORM INVERT =+91.76
 PROPOSED 375mm Ø STORM INVERT @ CAP =+91.93

DEPRESSED CURBS AND SIDEWALKS AS PER CITY OF OTTAWA STANDARDS

APPLICABLE SEWER LINE, TO BE CONFIRMED BY CLIENT

KEY PLAN
N.T.S.

NORTH

---	DRAINAGE AREA LIMITS
A-X	DRAINAGE AREA ID
0.039	AREA (ha)
0.46	RUNOFF COEFFICIENT (5-YEAR)

STORMWATER MANAGEMENT NOTES:

- REFER TO 'SERVICEABILITY AND STORMWATER MANAGEMENT REPORT' PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
- NO ROOF TOP CONTROL ALLOWED.
- TOTAL AREA: 1.28 ha

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CLARIDGE HOMES SUITE 2001, 210 GLADSTONE AVENUE, OTTAWA, ONTARIO K2P 0Y6.



No.	REVISION	DATE	BY
1.	SUBMITTED WITH ZONING/SITE PLAN APPLICATIONS	JAN25/21	JAG

SCALE	1:250
1:250	0 2 4 6 8 10

FOR REVIEW ONLY	DESIGN: JAG
	CHECKED: GJM
	DRAWN: C.J.F.
	CHECKED: JAG
	APPROVED: GJM



LOCATION: CITY OF OTTAWA, MARITIME WAY - KANATA RENTAL
 DRAWING NAME: STORM WATER MANAGEMENT PLAN

PROJECT NO.	120144
REV	REV # 1
DRAWING NO.	120144-SWM