

Appendix A

Servicing Guidelines Checklist & Concept Plan

DEVELOPMENT SERVICING STUDY CHECKLIST

4.1 General Content	
<input type="checkbox"/>	Executive Summary (for larger reports only). N/A
<input type="checkbox"/>	Date and revision number of the report. Title Page
<input type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development. Figure 1
<input type="checkbox"/>	Plan showing the site and location of all existing services. Figures 2/3/4
<input type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to applicable subwatershed and watershed plans that provide context to which individual developments must adhere. Section 1.0 & Section 2.0
<input type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies. Section 1.4
<input type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria. All sections
<input type="checkbox"/>	Statement of objectives and servicing criteria. Section 1.0 & Section 3.2, Section 4.2, and Section 5.2
<input type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area. Sections 3.1, Section 4.1, and Section 5.1
<input type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available). Sections 1.1 & 1.2
<input type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths. Drawing 1
<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts. MSS
<input type="checkbox"/>	Proposed phasing of the development, if applicable. N/A. Depends on landowner preferred timing
<input type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing. Section 1.1 & Section 2.1
<input type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: -Metric scale -North arrow (including construction North) -Key plan -Name and contact information of applicant and property owner -Property limits including bearings and dimensions -Existing and proposed structures and parking areas -Easements, road widening and rights-of-way -Adjacent street names All Figures
4.2 Development Servicing Report: Water	
<input type="checkbox"/>	Confirm consistency with Master Servicing Study, if available Section 3.2
<input type="checkbox"/>	Availability of public infrastructure to service proposed development MSS & Section 3.2
<input type="checkbox"/>	Identification of system constraints MSS & Section 3.2
<input type="checkbox"/>	Identify boundary conditions Detailed hydraulic assessment N/A for FSR

DEVELOPMENT SERVICING STUDY CHECKLIST

<input type="checkbox"/>	Confirmation of adequate domestic supply and pressure	MSS. Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter’s Survey. Output should show available fire flow at locations throughout the development.	MSS. Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Address reliability requirements such as appropriate location of shut-off valves	Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification	MSS.
<input type="checkbox"/>	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	MSS. Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	MSS, Section 3.2 & Figure 5. Detailed hydraulic assessment N/A for FSR.
<input type="checkbox"/>	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	MSS.
<input type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Section 3.2, Appendix C
<input type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Detailed hydraulic assessment N/A for FSR.

4.3 Development Servicing Report: Wastewater

<input type="checkbox"/>	Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Section 4.2
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	Section 4.2
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	MSS
<input type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 4.1 & 4.2
<input type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	MSS, Section 4.2, Appendix D
<input type="checkbox"/>	Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix ‘C’) format.	Appendix D
<input type="checkbox"/>	Description of proposed sewer network including sewers, pumping stations, and forcemains.	MSS, Section 4.2, Appendix C & Figure 3

DEVELOPMENT SERVICING STUDY CHECKLIST

<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	MSS
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

<input type="checkbox"/>	Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 1.1 & Section 5.1
<input type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	MSS & Section 5.3
<input type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Figure 5, Appendix B
<input type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	MSS, Section 5.2
<input type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	MSS & Section 5.2
<input type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information	MSS, Section 5.3, & Figure 5
<input type="checkbox"/>	Set-back from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	MSS, Section 5.3
<input type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
<input type="checkbox"/>	Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	MSS, Section 5.2, Section 5.3 & Section 5.4
<input type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).	MSS, Section 5.3
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	MSS, Section 5.3
<input type="checkbox"/>	Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	MSS
<input type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	Section 5.3
<input type="checkbox"/>	Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Section 5.3, Appendix E & Figure 2

DEVELOPMENT SERVICING STUDY CHECKLIST

<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/A
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses	MSS
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input type="checkbox"/>	Descriptions of how the conveyance and storage capacity will be achieved for the development.	MSS, Section 5.5
<input type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	MSS, Section 5.3 & Drawing 1
<input type="checkbox"/>	Inclusion of hydraulic analysis including hydraulic grade line elevations.	<i>MSS & EUC Pond 1 North Main Cell and North Forebay Modifications (DSEL, August 31, 2020)</i>
<input type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 7.0
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	MSS
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	Section 1.1 & 5.6

4.5 Approval and Permit Requirements: Checklist

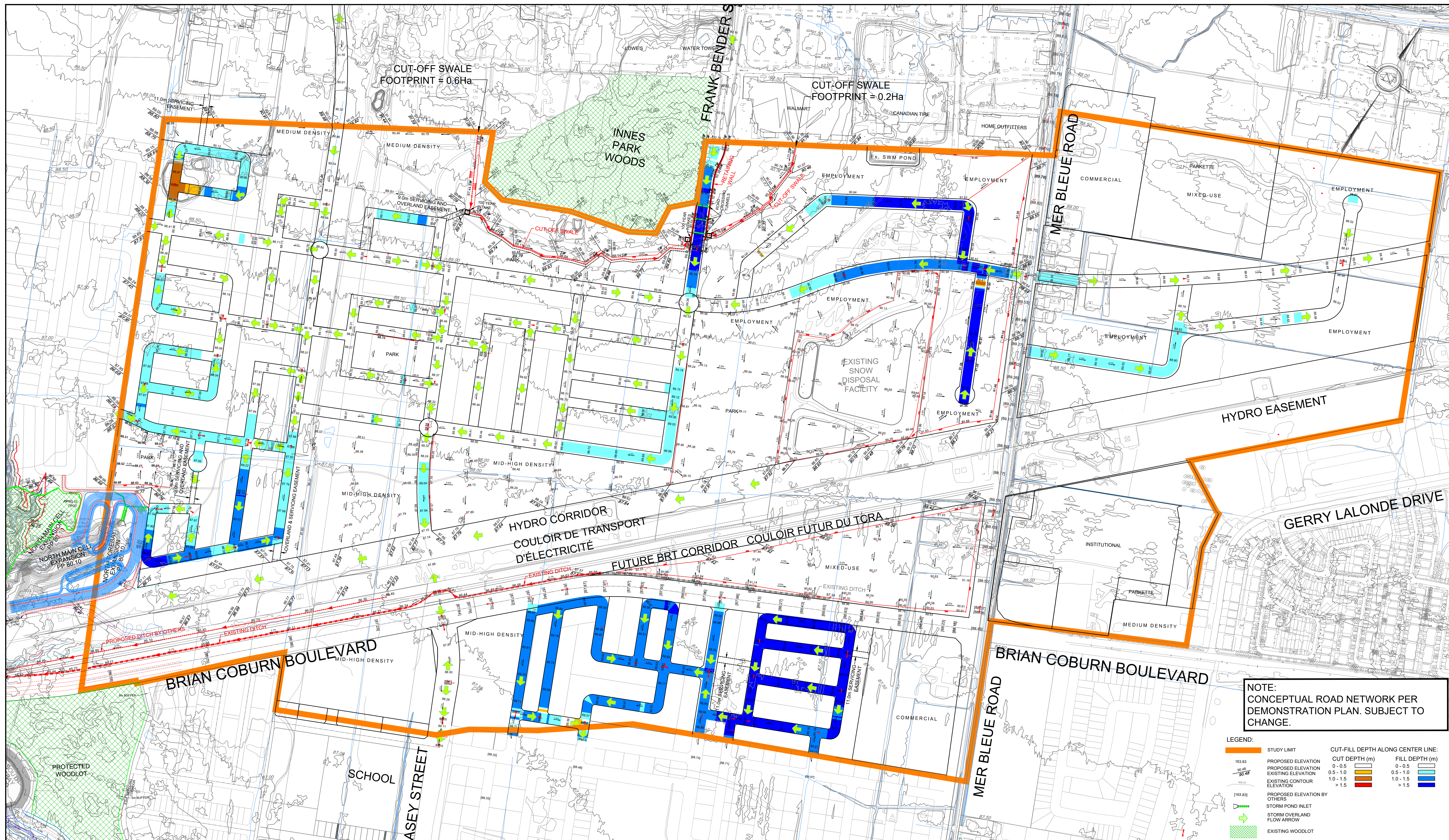
<input type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement ct. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	Section 1.2
<input type="checkbox"/>	Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	Section 1.2
<input type="checkbox"/>	Changes to Municipal Drains.	N/A
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	Section 1.2

4.6 Conclusion Checklist

<input type="checkbox"/>	Clearly stated conclusions and recommendations	Section 8.0
<input type="checkbox"/>	Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	N/A first submission
<input type="checkbox"/>	All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	Section 8.0

Appendix B

Excerpts from Supporting EUC Phase 3 Area CDP MSS (DSEL, Dec 2020)



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

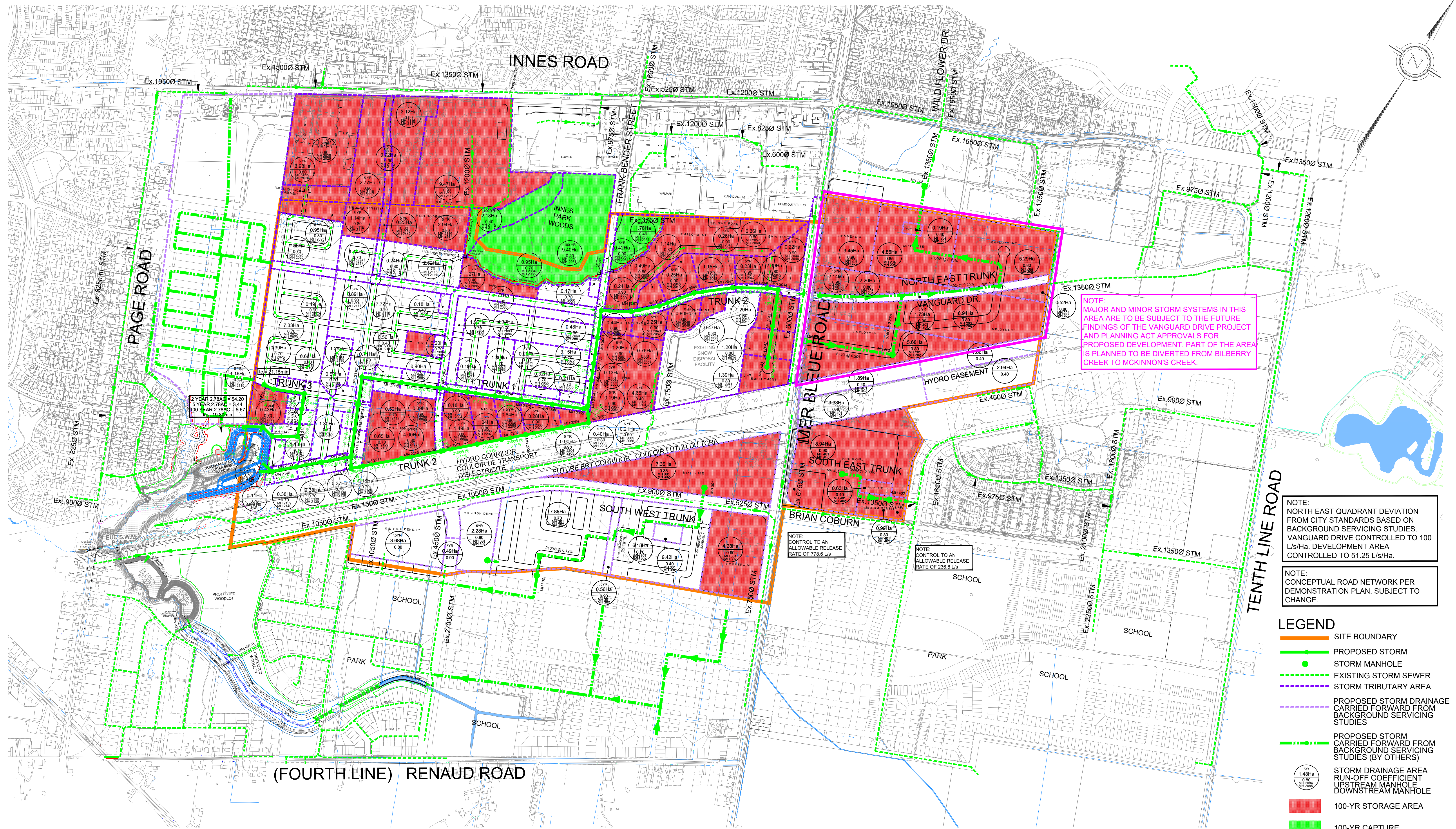
LEGEND:		CUT-FILL DEPTH ALONG CENTER LINE:	
[Orange line]	STUDY LIMIT	CUT DEPTH (m)	FILL DEPTH (m)
[Elevation symbol]	PROPOSED ELEVATION	0 - 0.5	0 - 0.5
[Elevation symbol]	PROPOSED ELEVATION	0.5 - 1.0	0.5 - 1.0
[Elevation symbol]	EXISTING ELEVATION	1.0 - 1.5	1.0 - 1.5
[Elevation symbol]	EXISTING CONTOUR ELEVATION	> 1.5	> 1.5
[Elevation symbol]	PROPOSED ELEVATION BY OTHERS		
[Green arrow]	STORM POND INLET		
[Green arrow]	STORM OVERLAND FLOW ARROW		
[Green hatched]	EXISTING WOODLOT		



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EAST URBAN COMMUNITY PHASE 3 AREA COMMUNITY DESIGN PLAN
GRADING PLAN

PROJECT No. :	14-733
SCALE	1:4000
DATE:	OCTOBER 2019
DRAWING No.	2



NOTE: MAJOR AND MINOR STORM SYSTEMS IN THIS AREA ARE TO BE SUBJECT TO THE FUTURE FINDINGS OF THE VANGUARD DRIVE PROJECT AND PLANNING ACT APPROVALS FOR PROPOSED DEVELOPMENT. PART OF THE AREA IS PLANNED TO BE DIVERTED FROM BILBERRY CREEK TO MCKINNON'S CREEK.

NOTE: NORTH EAST QUADRANT DEVIATION FROM CITY STANDARDS BASED ON BACKGROUND SERVICING STUDIES. VANGUARD DRIVE CONTROLLED TO 100 L/s/ha. DEVELOPMENT AREA CONTROLLED TO 51.25 L/s/ha.

NOTE: CONCEPTUAL ROAD NETWORK PER DEMONSTRATION PLAN. SUBJECT TO CHANGE.

- LEGEND**
- SITE BOUNDARY
 - PROPOSED STORM
 - STORM MANHOLE
 - EXISTING STORM SEWER
 - STORM TRIBUTARY AREA
 - PROPOSED STORM DRAINAGE CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
 - PROPOSED STORM DRAINAGE CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES (BY OTHERS)
 - 5 YR
1.48Ha
0.80
MH 2115 STORM DRAINAGE AREA RUN-OFF COEFFICIENT UPSTREAM MANHOLE DOWNSTREAM MANHOLE
 - 100-YR STORAGE AREA
 - 100-YR CAPTURE

2 YEAR 2.78AC = 54.20
5 YEAR 2.78AC = 3.44
100 YEAR 2.78AC = 5.67
10.88mm

NOTE: CONTROL TO AN ALLOWABLE RELEASE RATE OF 778.6 L/s

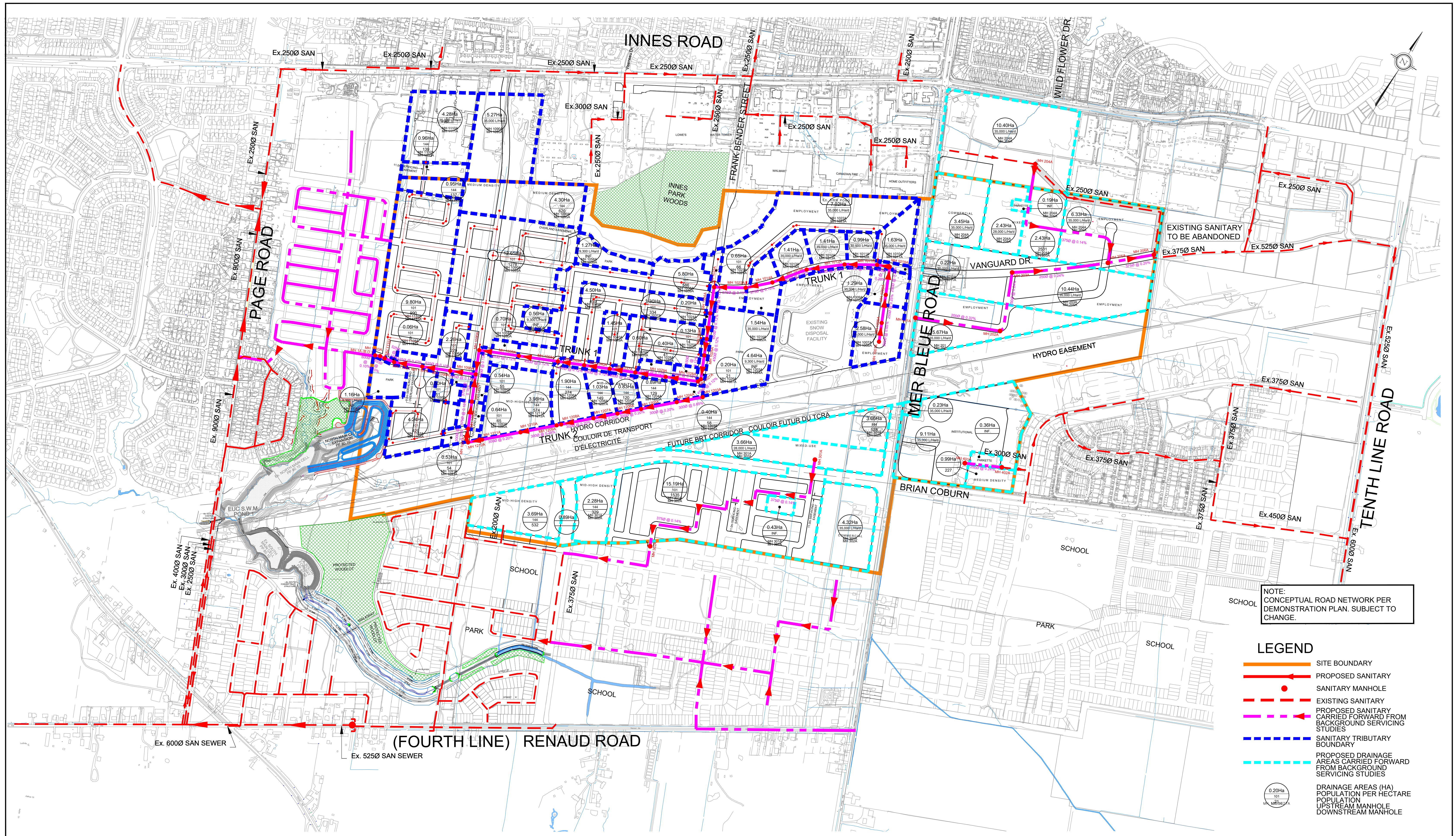
NOTE: CONTROL TO AN ALLOWABLE RELEASE RATE OF 236.8 L/s



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**EAST URBAN COMMUNITY PHASE 3 AREA COMMUNITY DESIGN PLAN
CONCEPTUAL STORM SERVICING**

PROJECT No. :	14-733
SCALE	1:5000
DATE:	OCTOBER 2019
DRAWING No.	4



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

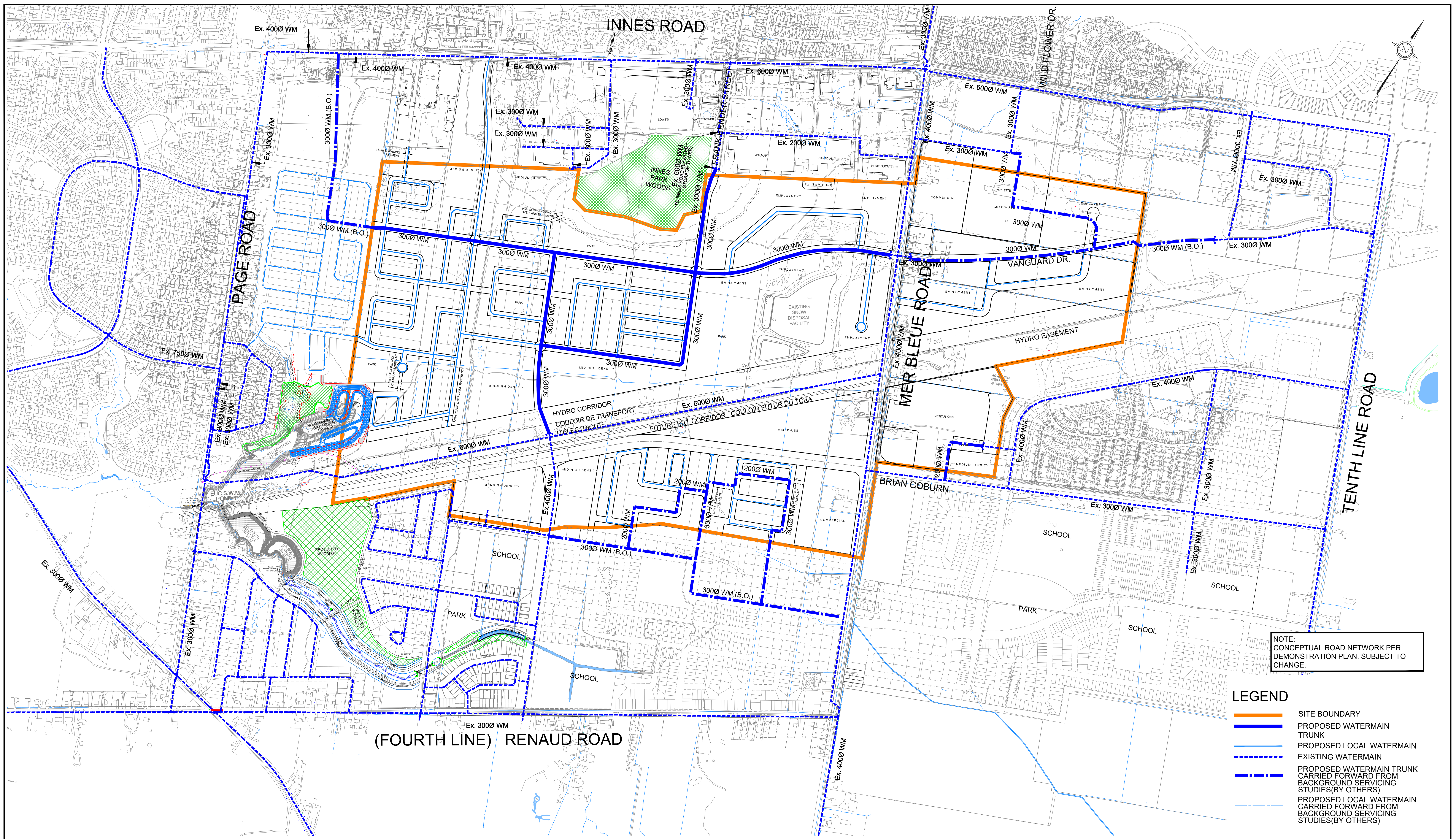
- LEGEND**
- SITE BOUNDARY
 - PROPOSED SANITARY
 - SANITARY MANHOLE
 - - - EXISTING SANITARY
 - - - PROPOSED SANITARY CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
 - - - SANITARY TRIBUTARY BOUNDARY
 - - - PROPOSED DRAINAGE AREAS CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
 - 0.20Ha
101
4400 DRAINAGE AREAS (HA)
POPULATION PER HECTARE
POPULATION
UPSTREAM MANHOLE
DOWNSTREAM MANHOLE



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**EAST URBAN COMMUNITY PHASE 3 AREA COMMUNITY DESIGN PLAN
CONCEPTUAL SANITARY SERVICING**

PROJECT No. :	14-733
SCALE	1:5000
DATE:	OCTOBER 2019
DRAWING No.	5



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

- LEGEND**
- SITE BOUNDARY
 - PROPOSED WATERMAIN TRUNK
 - - - PROPOSED LOCAL WATERMAIN
 - - - EXISTING WATERMAIN
 - - - PROPOSED WATERMAIN TRUNK CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES(BY OTHERS)
 - - - PROPOSED LOCAL WATERMAIN CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES(BY OTHERS)



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EAST URBAN COMMUNITY PHASE 3 AREA COMMUNITY DESIGN PLAN
WATERMAIN SERVICING

PROJECT No. :	14-733
SCALE	1:5000
DATE:	OCTOBER 2019
DRAWING No.	6

Appendix C

Water Demand Calculations

Water Demand Design Flows per Unit Count
City of Ottawa - Water Distribution Guidelines, July 2010



Domestic Demand

Type of Housing	Per / Unit	Units	Pop
Single Family	3.4	340	1156
Townhouse	2.7	529	1429
B2B	2.7	114	308
Apartment			0
Bachelor	1.4	-	0
1 Bedroom	1.4	-	0
2 Bedroom	2.1	-	0
3 Bedroom	3.1	-	0
Average	1.8	1,059	1907

	Pop	Avg. Daily		Max Day		Peak Hour	
		m ³ /d	L/min	m ³ /d	L/min	m ³ /d	L/min
Total Domestic Demand	4800	1344.0	933.3	2688.0	1866.7	4032.0	2800.0

Institutional / Commercial / Industrial Demand

Property Type	Unit Rate	Units	Avg. Daily		Max Day		Peak Hour	
			m ³ /d	L/min	m ³ /d	L/min	m ³ /d	L/min
Employment	35,000.0 L/ha/d	19.34	676.90	470.1	1015.4	705.1	1827.6	1269.2
Parks	9,300 L/ha/d	6.41	59.61	41.4	89.4	62.1	161.0	111.8

Total I/CI Demand	736.5	511.5	1104.8	767.2	1988.6	1381.0
Total Demand	2080.5	1444.8	3792.8	2633.9	6020.6	4181.0

Water Demand Design Flows per Unit Count
City of Ottawa - Water Distribution Guidelines, July 2010



Domestic Demand

Type of Housing	Per / Unit	Units	Pop
Single Family	3.4	319	1085
Townhouse	2.7	746	2015
B2B	2.7	-	0
Apartment			0
Bachelor	1.4	-	0
1 Bedroom	1.4	-	0
2 Bedroom	2.1	252	530
3 Bedroom	3.1	-	0
Average	1.8	688	1239

	Pop	Avg. Daily		Max Day		Peak Hour	
		m ³ /d	L/min	m ³ /d	L/min	m ³ /d	L/min
Total Domestic Demand	4869	1363.3	946.8	2726.6	1893.5	4090.0	2840.3

Institutional / Commercial / Industrial Demand

Property Type	Unit Rate	Units	Avg. Daily		Max Day		Peak Hour	
			m ³ /d	L/min	m ³ /d	L/min	m ³ /d	L/min
Employment	35,000.0 L/ha/d	19.42	679.70	472.0	1019.6	708.0	1835.2	1274.4
Parks	9,300 L/ha/d	6.26	58.22	40.4	87.3	60.6	157.2	109.2

Total I/CI Demand	737.9	512.4	1106.9	768.7	1992.4	1383.6
Total Demand	2101.2	1459.2	3833.5	2662.2	6082.3	4223.8

Appendix D

Sanitary Servicing Design

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION						COMM		INSTIT		PARK		C+H		INFILTRATION			PIPE							
STREET	FROM M.H.	TO M.H.	AREA (ha)	UNITS	POP.	CUMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL.	
						AREA (ha)	POP.																			(FULL) (m/s)	(ACT.) (m/s)
TRUNK 2																											
	1203A	1204A	0.36		52	0.36	52	3.6	0.61		0.00		0.00	4.59	4.59	0.74	4.95	4.95	1.63	2.99	81.0	300	0.65	77.96	0.04	1.10	0.52
	1204A	1205A	0.75		108	1.11	160	3.5	1.84		0.00		0.00		4.59	0.74	0.75	5.70	1.88	4.46	111.0	300	0.20	43.25	0.10	0.61	0.39
	1205A	1206A	0.77		111	1.88	271	3.5	3.05		0.00		0.00		4.59	0.74	0.77	6.47	2.14	5.93	74.0	300	0.20	43.25	0.14	0.61	0.43
	1206A	1207A	0.97		140	2.85	411	3.4	4.55		0.00		0.00		4.59	0.74	0.97	7.44	2.46	7.74	75.0	300	0.20	43.25	0.18	0.61	0.46
	1207A	1208A				2.85	411	3.4	4.55		0.00		0.00		4.59	0.74	0.00	7.44	2.46	7.74	100.5	300	0.20	43.25	0.18	0.61	0.46
	1208A	1209A	1.77		255	4.62	666	3.3	7.18		0.00		0.00		4.59	0.74	1.77	9.21	3.04	10.96	14.5	300	0.20	43.25	0.25	0.61	0.51
	1209A	1210A	1.64		237	6.26	903	3.3	9.55		0.00		0.00		4.59	0.74	1.64	10.85	3.58	13.87	112.5	300	0.20	43.25	0.32	0.61	0.54
	1210A	1211A	2.83		408	9.09	1311	3.2	13.50		0.00		0.00		4.59	0.74	2.83	13.68	4.51	18.75	120.0	300	0.20	43.25	0.43	0.61	0.59
	1211A	1212A				9.09	1311	3.2	13.50		0.00		0.00		4.59	0.74	0.00	13.68	4.51	18.75	43.5	300	0.20	43.25	0.43	0.61	0.59
	1212A	1091A				9.09	1311	3.2	13.50		0.00		0.00		4.59	0.74	0.00	13.68	4.51	18.75	10.0	300	0.20	43.25	0.43	0.61	0.59
	1091A	1093A				9.09	1311	3.2	13.50		0.00		0.00		4.59	0.74	0.00	13.68	4.51	18.75	33.5	300	0.20	43.25	0.43	0.61	0.59
	1093A	1094A	1.16		118	10.25	1429	3.2	14.61		0.00		0.00		4.59	0.74	1.16	14.84	4.90	20.25	84.0	450	0.12	98.76	0.21	0.62	0.49
	1094A	1095A	0.52		53	10.77	1482	3.1	15.11		0.00		0.00		4.59	0.74	0.52	15.36	5.07	20.92	81.0	450	0.12	98.76	0.21	0.62	0.49
To TRUNK 1, Pipe 1095A - 1096A						10.77	1482				0.00		0.00		4.59			15.36									
TRUNK 1																											
	1007A	1008A				0.00				1.87	1.87		0.00		0.00	1.14	1.87	1.87	0.62	1.75	58.0	300	0.65	77.96	0.02	1.10	0.44
	1008A	1009A				0.00	0			1.19	3.06		0.00		0.00	1.86	1.19	3.06	1.01	2.87	86.5	300	0.25	48.35	0.06	0.68	0.37
	1009A	1010A				0.00	0			0.90	3.96		0.00		0.00	2.41	0.90	3.96	1.31	3.71	86.5	300	0.25	48.35	0.08	0.68	0.40
	1010A	1011A				0.00	0			2.04	6.00		0.00		0.00	3.65	2.04	6.00	1.98	5.63	46.0	300	0.25	48.35	0.12	0.68	0.46
	1011A	1012A				0.00	0			1.02	7.02		0.00		0.00	4.27	1.02	7.02	2.32	6.58	97.5	375	0.15	67.91	0.10	0.61	0.39
	1012A	1013A				0.00	0			2.12	9.14		0.00		0.00	5.55	2.12	9.14	3.02	8.57	125.5	375	0.15	67.91	0.13	0.61	0.42
	1013A	1014A				0.00	0			2.12	11.26		0.00		0.00	6.84	2.12	11.26	3.72	10.56	88.0	375	0.15	67.91	0.16	0.61	0.45
	1014A	1022A				0.00	0			1.18	12.44		0.00		0.00	7.56	1.18	12.44	4.11	11.66	93.0	375	0.15	67.91	0.17	0.61	0.46
	1022A	1023A				0.00	0			6.81	19.25		0.00		0.00	11.70	6.81	19.25	6.35	18.05	100.5	375	0.15	67.91	0.27	0.61	0.52
	1023A	1024A	0.72		73	0.72	73	3.6	0.86		19.25		0.00		0.00	11.70	0.72	19.97	6.59	19.14	82.0	450	0.12	98.76	0.19	0.62	0.48
	1024A	1025A	0.19		20	0.91	93	3.6	1.09		19.25		0.00		0.00	11.70	0.19	20.16	6.65	19.44	79.0	450	0.12	98.76	0.20	0.62	0.48
	1025A	1026A	0.14		15	1.05	108	3.6	1.26		19.25		0.00		0.00	11.70	0.14	20.30	6.70	19.65	58.0	450	0.12	98.76	0.20	0.62	0.48
	1026A	1027A	0.24		25	1.29	133	3.6	1.54		19.25		0.00		0.00	11.70	0.24	20.54	6.78	20.01	63.5	450	0.12	98.76	0.20	0.62	0.49
	1027A	1028A				1.29	133	3.6	1.54		19.25		0.00		0.00	11.70	0.00	20.54	6.78	20.01	25.0	450	0.12	98.76	0.20	0.62	0.49
	1028A	1029A	0.52		53	1.81	186	3.5	2.13		19.25		0.00		0.00	11.70	0.52	21.06	6.95	20.77	93.0	450	0.12	98.76	0.21	0.62	0.49
	1029A	1037A	0.48		49	2.29	235	3.5	2.66		19.25		0.00		0.00	11.70	0.48	21.54	7.11	21.47	93.0	450	0.12	98.76	0.22	0.62	0.49
	1037A	1040A	3.56		360	5.85	595	3.3	6.45		19.25		0.00		0.00	11.70	3.56	25.10	8.28	26.43	79.0	450	0.12	98.76	0.27	0.62	0.52
	1040A	1049A	1.54		156	7.39	751	3.3	8.03		19.25		0.00		0.00	11.70	1.54	26.64	8.79	28.52	79.0	450	0.12	98.76	0.29	0.62	0.53
	1049A	1058A	4.52		457	11.91	1208	3.2	12.51		19.25		0.00		0.00	11.70	4.52	31.16	10.28	34.49	81.0	450	0.12	98.76	0.35	0.62	0.56
	1058A	1059A	5.68		574	17.59	1782	3.1	17.90		19.25		0.00	1.37	1.37	11.92	7.05	38.21	12.61	42.43	121.5	450	0.12	98.76	0.43	0.62	0.60
	1059A	1090A	0.46		47	18.05	1829	3.1	18.33		19.25		0.00		1.37	11.92	0.46	38.67	12.76	43.01	121.5	450	0.12	98.76	0.44	0.62	0.60
			2.41		348	20.46	2177			5.07	24.32		0.00	0.59	1.96		8.07	46.74									
Contribution From TRUNK 1, Pipe 1094A - 1095A						14.50	1465	34.96	3642	2.9	34.18		0.00		1.96	15.09	14.50	61.24	20.21	69.48	68.5	450	0.15	110.42	0.63	0.69	0.73
	1095A	1096A	0.50		51	46.23	5175	2.8	46.71		24.32		0.00		6.55	15.84	0.50	77.10	25.44	87.98	79.5	525	0.12	148.98	0.59	0.69	0.72
	1096A	1107A	1.98		200	48.21	5375	2.8	48.30		24.32		0.00		6.55	15.84	1.98	79.08	26.10	90.23	76.0	525	0.10	136.00	0.66	0.63	0.67
			1.91		276	50.12	5651				24.32		0.00		6.55		1.91	80.99									
			4.43		448	54.55	6099				24.32		0.00		6.55		4.43	85.42									
	1107A	1108A	9.77		987	64.32	7086	2.7	61.57	4.28	28.60		0.00		6.55	18.44	14.05	99.47	32.83	112.83	97.5	525	0.22	201.72	0.56	0.93	0.95
	1108A	1133A	0.31		32	64.63	7118	2.7	61.81		28.60		0.00	1.16	7.71	18.62	1.47	100.94	33.31	113.75	47.5	600	0.22	288.00	0.39	1.02	0.96
	1133A	1A (B.O.)				64.63	7118	2.7	61.81		28.60		0.00		7.71	18.62	0.00	100.94	33.31	113.75	15.5	600	0.10	194.17	0.59	0.69	0.71



DESIGN PARAMETERS										Designed: V.W.					PROJECT: Trailsedge North												
Park Flow = 9300 L/ha/da 0.10764 l/s/ha Average Daily Flow = 280 l/p/day Comm/Inst Flow = 35000 L/ha/da 0.4051 l/s/ha Industrial Flow = 35000 L/ha/da 0.40509 l/s/ha Max Res. Peak Factor = 4.00 Commercial/Inst./Park Peak Factor = 1.50 Institutional = 0.41 l/s/ha										Industrial Peak Factor = as per MOE Graph Extraneous Flow = 0.330 L/s/ha Minimum Velocity = 0.600 m/s Manning's n = (Conc) 0.013 (Pvc) 0.013 Townhouse coeff= 2.7 Single house coeff= 3.4					Checked: W.L.												

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION				COMM		INSTIT		PARK		C+I		INFILTRATION			PIPE										
STREET	FROM M.H.	TO M.H.	AREA (ha)	POP.	CUMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL					
					AREA (ha)	POP.																	(FULL) (m/s)	(ACT.) (m/s)				
North West Sanitary Trunk																												
Trunk 1	1007A	1008A			0.00	0			2.58	2.58						1.57	2.58	2.58	0.85	2.42	58.00	200.00	0.65	26.44	0.09	0.84	0.52	
COMMERCIAL	1008A	1009A			0.00	0				2.58						1.57	0.00	2.58	0.85	2.42	86.50	250.00	0.25	29.73	0.08	0.61	0.37	
COMMERCIAL	1009A	1010A			0.00	0			1.29	3.87						2.35	1.29	3.87	1.28	3.63	86.50	250.00	0.25	29.73	0.12	0.61	0.41	
COMMERCIAL					0.00	0			0.22	4.09							0.22	4.09										
COMMERCIAL	1010A	1011A			0.00	0			1.63	5.72						3.48	1.63	5.72	1.89	5.37	39.50	300.00	0.20	43.25	0.12	0.61	0.00	
COMMERCIAL	1011A	1012A			0.00	0			0.99	6.71						4.08	0.99	6.71	2.21	6.29	99.50	375.00	0.15	67.91	0.09	0.61	0.38	
COMMERCIAL	1012A	1013A			0.00	0			1.41	8.12						4.93	1.41	8.12	2.68	7.61	117.00	375.00	0.15	67.91	0.11	0.61	0.40	
COMMERCIAL	1013A	1014A			0.00	0			1.41	9.53						5.79	1.41	9.53	3.14	8.93	112.00	375.00	0.15	67.91	0.13	0.61	0.41	
COMMERCIAL	1014A	1022A			0.00	0			1.54	11.07						6.73	1.54	11.07	3.65	10.38	83.50	375.00	0.15	67.91	0.15	0.61	0.44	
COMMERCIAL	1022A	1023A			0.00	0			7.02	18.09						10.99	7.02	18.09	5.97	16.96	96.50	375.00	0.15	67.91	0.25	0.61	0.51	
	1023A	1024A	0.65	66	0.65	66	3.63	0.78		18.09						10.99	0.65	18.74	6.18	17.95	81.00	450.00	0.12	98.76	0.18	0.62	0.47	
	1024A	1025A	0.20	21	0.85	87	3.61	1.02		18.09						10.99	0.20	18.94	6.25	18.26	79.00	450.00	0.12	98.76	0.18	0.62	0.47	
	1025A	1026A	0.13	14	0.98	101	3.59	1.18		18.09						10.99	0.13	19.07	6.29	18.46	51.00	450.00	0.12	98.76	0.19	0.62	0.48	
	1026A	1027A	0.20	21	1.18	122	3.58	1.42		18.09						10.99	0.20	19.27	6.36	18.77	74.00	450.00	0.12	98.76	0.19	0.62	0.48	
	1027A	1028A			1.18	122				18.09						10.99	0.00	19.27	6.36	17.35	11.00	450.00	0.12	98.76	0.18	0.62	0.47	
	1028A	1029A	0.40	41	1.58	163	3.54	1.87		18.09						10.99	0.40	19.67	6.49	19.35	100.00	450.00	0.12	98.76	0.20	0.62	0.48	
	1029A	1037A	0.60	61	2.18	224	3.50	2.54		18.09						10.99	0.60	20.27	6.69	20.22	94.00	450.00	0.12	98.76	0.20	0.62	0.48	
	1037A	1040A	3.30	334	5.48	558	3.36	6.08		18.09						10.99	3.30	23.57	7.78	24.85	79.00	450.00	0.12	98.76	0.25	0.62	0.51	
	1040A	1049A	1.45	147	6.93	705	3.31	7.56		18.09						10.99	1.45	25.02	8.26	26.81	79.00	450.00	0.12	98.76	0.27	0.62	0.52	
	1049A	1058A	4.50	455	11.43	1160	3.21	12.07		18.09						10.99	4.50	29.52	9.74	32.80	81.50	450.00	0.12	98.76	0.33	0.62	0.56	
PARK	1058A	1059A	5.80	586	17.23	1746	3.10	17.54		18.09		1.27	1.27	11.20	7.07	36.59	12.07	40.81	120.50	450.00	0.12	98.76	0.41	0.62	0.59			
	1059A	1090A	0.70	71	17.93	1817	3.09	18.20		18.09			1.27	11.20	0.70	37.29	12.31	41.71	123.00	450.00	0.12	98.76	0.42	0.62	0.59			
PARK, EXT FUT			4.30	620	22.23	2437			5.27	23.36			0.56	1.83		47.42												
	1090A	1095A	12.65	1278	34.88	3715	2.89	34.79		23.36			1.83	14.49	12.65	60.07	19.82	69.10	75.00	450.00	0.15	110.42	0.63	0.69	0.73			
Contribution from Trunk 2, MH 1094A-1095A					10.74	1478				0.00			4.64			15.38												
	1095A	1096A	0.50	51	46.12	5244	2.78	47.24		23.36			6.47	15.24	0.50	75.95	25.06	87.54	79.00	525.00	0.12	148.98	0.59	0.69	0.72			
	1096A	1107A	2.26	229	48.38	5473	2.77	49.13		23.36			6.47	15.24	2.26	78.21	25.81	90.18	86.50	525.00	0.10	136.00	0.66	0.63	0.67			
	1107A	1108A	4.24	429	52.62	5902	2.74	52.41		23.36			6.47	15.24	4.24	82.45	27.21	94.86	87.00	525.00	0.42	278.71	0.34	1.29	1.16			
PARK	1108A	1132A	0.06	8	52.68	5910	2.74	52.48		23.36		1.16	7.63	15.43	1.22	83.67	27.61	95.52	31.50	525.00	0.10	136.00	0.70	0.63	0.68			
CONTRIBUTION FROM EXTERNAL					0.96	144	53.64	6054	2.73	53.56	4.42		27.78			7.63	5.38	89.05										
			0.95	137	54.59	6191				27.78			7.63		0.95	90.00												
	1132A	1133A	9.80	990	64.39	7181	2.68	62.37		27.78			7.63	18.11	9.80	99.80	32.93	113.41	15.50	600.00	0.10	194.17	0.58	0.69	0.72			
	1133A	1A (B.O.)			64.39	7181	2.68	62.37		27.78			7.63	18.11	0.00	99.80	32.93	113.41	15.50	600.00	0.10	194.17	0.58	0.69	0.72			
To MH 1A By Other																												
Trunk 2																												
PARK	1203A	1204A	0.40	58	0.40	58				0.00			4.64	4.64	0.75	5.04	5.04	1.66	2.41	81.00	300.00	0.65	77.96	0.03	1.10	0.48		
	1204A	1205A	0.89	129	1.29	187	3.53	2.14		0.00			4.64	0.75	0.89	5.93	1.96	4.85	111.00	300.00	0.20	43.25	0.11	0.61	0.40			
	1205A	1206A	0.83	120	2.12	307	3.46	3.44		0.00			4.64	0.75	0.83	6.76	2.23	6.42	74.00	300.00	0.20	43.25	0.15	0.61	0.44			
	1206A	1207A	1.03	149	3.15	456	3.40	5.02		0.00			4.64	0.75	1.03	7.79	2.57	8.34	75.00	300.00	0.20	43.25	0.19	0.61	0.47			
	1207A	1208A			3.15	456				0.00			4.64	0.75	0.00	7.79	2.57	3.32	100.50	300.00	0.20	43.25	0.08	0.61	0.37			



DESIGN PARAMETERS Park Flow = 9300 L/ha/da Average Daily Flow = 280 l/p/day Comm/Inst Flow = 35000 L/ha/da Industrial Flow = 35000 L/ha/da Max Res. Peak Factor = 4.00 Commercial/Inst./Park Peak Factor = 1.50 Mixed Use Institutional = 35000.00 L/ha/da Institutional = 0.405 l/s/ha										Harmon Correction Factor = 0.800 Industrial Peak Factor = as per MOE Graph Extraneous Flow = 0.330 L/s/ha Minimum Velocity = 0.600 m/s Manning's n = (Conc) 0.013 (Pvc) 0.013										Designed: R.B. Checked: K.M. Dwg. Reference:					PROJECT: Orleans EUC MUC LOCATION: City of Ottawa File Ref: 14-733 Date: October, 2019 Sheet No. 1 of 2				
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SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION				COMM		INSTIT		PARK		C+H		INFILTRATION			PIPE												
STREET	FROM M.H.	TO M.H.	AREA (ha)	POP.	CUMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL.					
					AREA (ha)	POP.																			(FULL) (m/s)	(ACT.) (m/s)				
	1208A	1209A	1.90	274	5.05	730	3.31	7.83		0.00					4.64	0.75	1.90	9.69	3.20	11.78	14.50	300.00	0.20	43.25	0.27	0.61	0.51			
	1209A	1210A			5.05	730				0.00					4.64	0.75	0.00	9.69	3.20	3.95	112.50	300.00	0.20	43.25	0.09	0.61	0.38			
	1210A	1211A			5.05	730				0.00					4.64	0.75	0.00	9.69	3.20	3.95	120.00	300.00	0.20	43.25	0.09	0.61	0.38			
	1211A	1212A	3.98	574	9.03	1304	3.18	13.44		0.00					4.64	0.75	3.98	13.67	4.51	18.70	43.50	300.00	0.20	43.25	0.43	0.61	0.59			
	1212A	1091A			9.03	1304				0.00					4.64	0.75	0.00	13.67	4.51	5.26	10.00	300.00	0.20	43.25	0.12	0.61	0.41			
	1091A	1093A	0.53	54	9.56	1358	3.17	13.95		0.00					4.64	0.75	0.53	14.20	4.69	19.39	33.00	300.00	0.20	43.25	0.45	0.61	0.59			
	1093A	1094A	0.64	65	10.20	1423	3.16	14.57		0.00					4.64	0.75	0.64	14.84	4.90	20.22	84.00	375.00	0.15	67.91	0.30	0.61	0.53			
	1094A	1095A	0.54	55	10.74	1478	3.15	15.09		0.00					4.64	0.75	0.54	15.38	5.08	20.92	84.50	375.00	0.15	67.91	0.31	0.61	0.54			
To Trunk 1, Pipe 1095A-1096A					10.74	1478				0.00					4.64			15.38												
North East Sanitary Trunk																														
External Commercial					0.00	0			10.40	10.40						10.40	10.40													
Mixed Use Block*			2.43	2531	2.43	2531	3.00	24.61	2.43	12.83					4.86	15.26														
	204A	205A			2.43	2531			3.45	16.28					3.45	18.71														
To Pipe 205A - 206A					2.43	2531			6.33	22.61			0.19	0.19	13.77	6.52	25.23	8.33	22.10	525.00	375.00	0.14	65.60	0.34	0.59	0.53				
To Pipe 205A - 206A					2.43	2531				22.61						25.23			22.10											
	201A	202A			0.00	0			5.67	5.67					3.45	5.67	5.67	1.87	5.32	266.00	200.00	0.32	18.55	0.29	0.59	0.51				
	202A	203A			0.00	0			0.00	5.67					3.45	0.00	5.67	1.87	5.32	176.00	250.00	0.24	29.13	0.18	0.59	0.44				
	203A	205A			0.00	0			10.44	16.11					9.79	10.44	16.11	5.32	15.11	292.50	250.00	0.24	29.13	0.52	0.59	0.60				
Contribution from Pipe 204A - 205A					2.43	2531				22.61			0.19			25.23														
	205A	206A			2.43	2531				38.72			0.19	23.56	0.00	41.34	13.64	37.20	150.50	375.00	0.20	78.41	0.47	0.71	0.70					
To Existing Vanguard Drive Sanitary					2.43	2531				38.72			0.19			41.34			37.20											
South West Sanitary Trunk																														
Mixed Use Block			3.66	528	3.66	528			3.66	3.66					2.22	7.32	7.32													
Mid-High Density Residential			15.19	1535	18.85	2063	3.06	20.46	4.32	7.98					4.85	19.51	26.83													
	301A	302A	2.28	329	21.13	2392	3.02	23.41		7.98			0.43	0.43	4.92	2.71	29.54	9.75	38.08	791.00	375.00	0.14	65.60	0.58	0.59	0.61				
To Sanitary By Others					21.13	2392				7.98					0.43		29.54		38.08											
Road			0.89	0	0.89	0									0.00	0.89	0.89	0.29	0.29	49.00	200.00	0.32	18.55	0.02	0.59	0.23				
To Existing Sanitary, Fern Casey Street					0.89	0				0.00					0.00		0.89		0.29											
Mid-High Density Residential			3.69	532	3.69	532	3.37	5.81		0.00					0.00	0.00	3.69	3.69	1.22	7.03	49.00	200.00	0.32	18.55	0.38	0.59	0.55			
To Existing Sanitary, Axis Way					3.69	532				0.00					0.00		3.69		7.03											
South East Sanitary Trunk																														
Existing Medium Density**			401A	402A	0.99	227	0.99	227	3.50	2.57		0.00	0.23	9.34	0.36	5.73	1.22	10.69	3.53	11.83	114.00	250.00	0.24	29.13	0.41	0.59	0.56			
To Existing Sanitary to Gerry Lalonde Drive					0.99	227				0.00				9.34	0.36		10.69		11.83											

*Note: Proposed population 2531 per background servicing study
 **Note: Existing population 227 per background servicing study

DESIGN PARAMETERS			
Park Flow =	9300	L/ha/da	0.108
Average Daily Flow =	280	l/p/day	
Comm/Inst Flow =	35000	L/ha/da	0.405
Industrial Flow =	35000	L/ha/da	0.405
Max Res. Peak Factor =	4.00		
Commercial/Inst./Park Peak Factor =	1.50	if ICI >20%	1.00 if ICI <20%
Mixed Use	35000.00	L/ha/da	
Institutional =	0.405	l/s/ha	
Harmon Correction Factor =	0.800		
Industrial Peak Factor = as per MOE Graph			
Extraneous Flow =	0.330	L/s/ha	
Minimum Velocity =	0.600	m/s	
Manning's n = (Conc)	0.013	(Pvc)	0.013

Designed: R.B.

Checked: K.M.

Dwg. Reference: 14-733

Date: October, 2019

Sheet No. 2 of 2

Orleans EUC MUC

City of Ottawa

SANITARY SEWER CALCULATION SHEET



Manning's n=0.013

LOCATION			RESIDENTIAL AREA AND POPULATION				COMM		INSTIT		PARK		C+I-I		INFILTRATION			PIPE										
STREET	FROM M.H.	TO M.H.	AREA (ha)	POP.	CUMULATIVE		PEAK FACT.	PEAK FLOW (l/s)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	AREA (ha)	ACCU. AREA (ha)	PEAK FLOW (l/s)	TOTAL AREA (ha)	ACCU. AREA (ha)	INFILT. FLOW (l/s)	TOTAL FLOW (l/s)	DIST (m)	DIA (mm)	SLOPE (%)	CAP. (FULL) (l/s)	RATIO Q act/Q cap	VEL.			
					AREA (ha)	POP.																			(FULL) (m/s)	(ACT.) (m/s)		
NW Quadrant to Nature Trail Crescent	1133A	1A (B.O.)			64.33	7168	2.68	62.26		35.83				7.63	23.00	0.00	107.79	35.57	120.83									
Per Sanitary Sewer Calculation Sheet - prepared by DSEL, October 2018					64.33	7168	2.68		35.83					7.63					120.83									
3490 Innes Rd. Future Dev. Blocks					4.33	1402	3.16	14.36	5.40	5.40				0.00	3.28	9.73	9.73	3.21	20.85									
Future Dev. Blocks taken at EUC Phase 3 CDP Mid-High Residential Density (144 pop/ha)																												
3490 Innes Road					19.75	1516	3.14	15.43	0.00	0.00			1.42	1.42	0.23	21.17	21.17	6.99	22.65									
Per Sanitary Sewer Calculation Sheet - Caivan Communities Orleans Village - prepared by DSEL, May 2018																												
Total to Existing Nature Trail Crescent sewer					88.41	10086	2.56	83.68	41.23	41.23			9.05	9.05	26.51	138.69	138.69	45.77	155.96									

DESIGN PARAMETERS										Designed:		PROJECT:					
Park Flow =	9300	L/ha/da	0.108	Harmon Correction Factor =	0.800			BK		Orleans EUC MUC							
Average Daily Flow =	280	l/p/day		Industrial Peak Factor = as per MOE Graph						LOCATION:							
Comm/Inst Flow =	35000	L/ha/da	0.405	Extraneous Flow =	0.330	L/s/ha				City of Ottawa							
Industrial Flow =	35000	L/ha/da	0.405	Minimum Velocity =	0.600	m/s				File Ref:		14-733	Date:	October, 2018	Sheet No.	1	
Max Res. Peak Factor =	4.00			Manning's n = (Conc)	0.013	(Pvc)	0.013									of	1
Commercial/Inst./Park Peak Factor =	1.50	if ICI >20%	1.00	if ICI <20%													
Mixed Use	28000.00	L/ha/da															
Institutional =	0.405	l/s/Ha															



Appendix E

Stormwater Servicing Design

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years

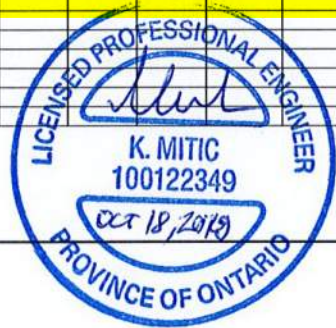


Manning 0.013

LOCATION			AREA (Ha)																FLOW					SEWER DATA										
Location	From Node	To Node	2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of Conc. (min)	Intensity 2 Year (mm/h)	Intensity 5 Year (mm/h)	Intensity 10 Year (mm/h)	Intensity 100 Year (mm/h)	Peak Flow Q (l/s)	DIA. (mm) (actual)	DIA. (mm) (nominal)	TYPE	SLOPE (%)	LENGTH (m)	CAPACITY (l/s)	VELOCITY (m/s)	TIME OF LOW (min)	RATIO Q/Q full	
			AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC																
	2136	2138	0.37	0.70	0.72	93.83			0.00	69.94			0.00	5.35			0.00	14.86	27.52	42.41	57.14	66.82	97.40	9781	2700	2700	CONC	0.15	90.5	13126	2.29	0.66	0.75	
			1.29	0.70	2.51	96.34			0.00	69.94			0.00	5.35			0.00	14.86																
			1.30	0.70	2.53	98.87			0.00	69.94			0.00	5.35			0.00	14.86																
			0.50	0.70	0.97	99.85			0.00	69.94			0.00	5.35			0.00	14.86																
	2138	2139	0.38	0.70	0.74	100.59			0.00	69.94			0.00	5.35			0.00	14.86	21.02	50.44	68.08	79.66	116.22	11988	2700	2700	CONC	0.16	77.0	13556	2.37	0.54	0.88	
	2139	2140	0.38	0.70	0.74	101.33			0.00	69.94			0.00	5.35			0.00	14.86	21.56	49.64	66.99	78.38	114.34	11834	2700	2700	CONC	0.15	73.5	13126	2.29	0.53	0.90	
	2140	HW	0.11	0.70	0.21	101.54			0.00	69.94			0.00	5.35			0.00	14.86	21.56	49.64	66.99	78.38	114.34	11844	2700	2700	CONC	0.15	47.0	13126	2.29	0.34	0.90	
TO POND 1																																		
TRUNK 3																																		
					0.00	0.00	0.98	0.80	2.18	2.18			0.00	0.00			0.00	0.00	21.15															
			2.86	0.70	5.57	5.57	5.61	0.90	14.04	16.22			0.00	0.00			0.00	0.00																
					0.00	5.57	0.95	0.80	2.11	18.33			0.00	0.00			0.00	0.00																
	2025	2026	7.33	0.70	14.26	19.83	0.49	0.90	1.23	19.55			0.00	0.00			0.00	0.00	21.15	50.25	67.82	79.36	115.77	148	1650	1650	CONC	0.14	32.0	3410	1.59	0.33	0.04	
	2026	2119	0.39	0.70	0.76	20.59	1.16	0.40	1.29	20.84			0.00	0.00			0.00	0.00	21.48	49.76	67.15	78.57	114.61	1366	1650	1650	CONC	0.16	92.5	3646	1.71	0.90	0.37	
	2119	2120	0.66	0.70	1.28	21.87			0.00	20.84			0.00	0.00			0.00	0.00	22.39	48.48	65.41	76.52	111.61	1469	1650	1650	CONC	0.10	47.0	2882	1.35	0.58	0.51	
	2120	2121	0.43	0.70	0.84	22.71			0.00	20.84			0.00	0.00			0.00	0.00	22.97	47.70	64.34	75.27	109.78	2204	1650	1650	CONC	0.10	84.5	2882	1.35	1.04	0.76	
	2121	2142	1.13	0.70	2.20	24.91			0.00	20.84			0.00	0.00			0.00	0.00	24.01	46.36	62.52	73.13	106.63	2258	1650	1650	CONC	0.10	76.0	2882	1.35	0.94	0.78	
	2142	2143	0.37	0.70	0.72	25.63			0.00	20.84			0.00	0.00			0.00	0.00	22.97	47.70	64.34	75.27	109.78	2204	1650	1650	CONC	0.10	43.0	2882	1.35	0.53	0.76	
	2143	2144			0.00	25.63			0.00	20.84			0.00	0.00			0.00	0.00	24.01	46.36	62.52	73.13	106.63	2258	1800	1800	CONC	0.10	51.1	3635	1.43	0.60	0.62	
	2144	HW			0.00	25.63			0.00	20.84			0.00	0.00			0.00	0.00	24.95	45.22	60.97	71.31	103.97	2260	1800	1800	CONC	0.10	22.5	3635	1.43	0.26	0.62	
TO POND 1																																		

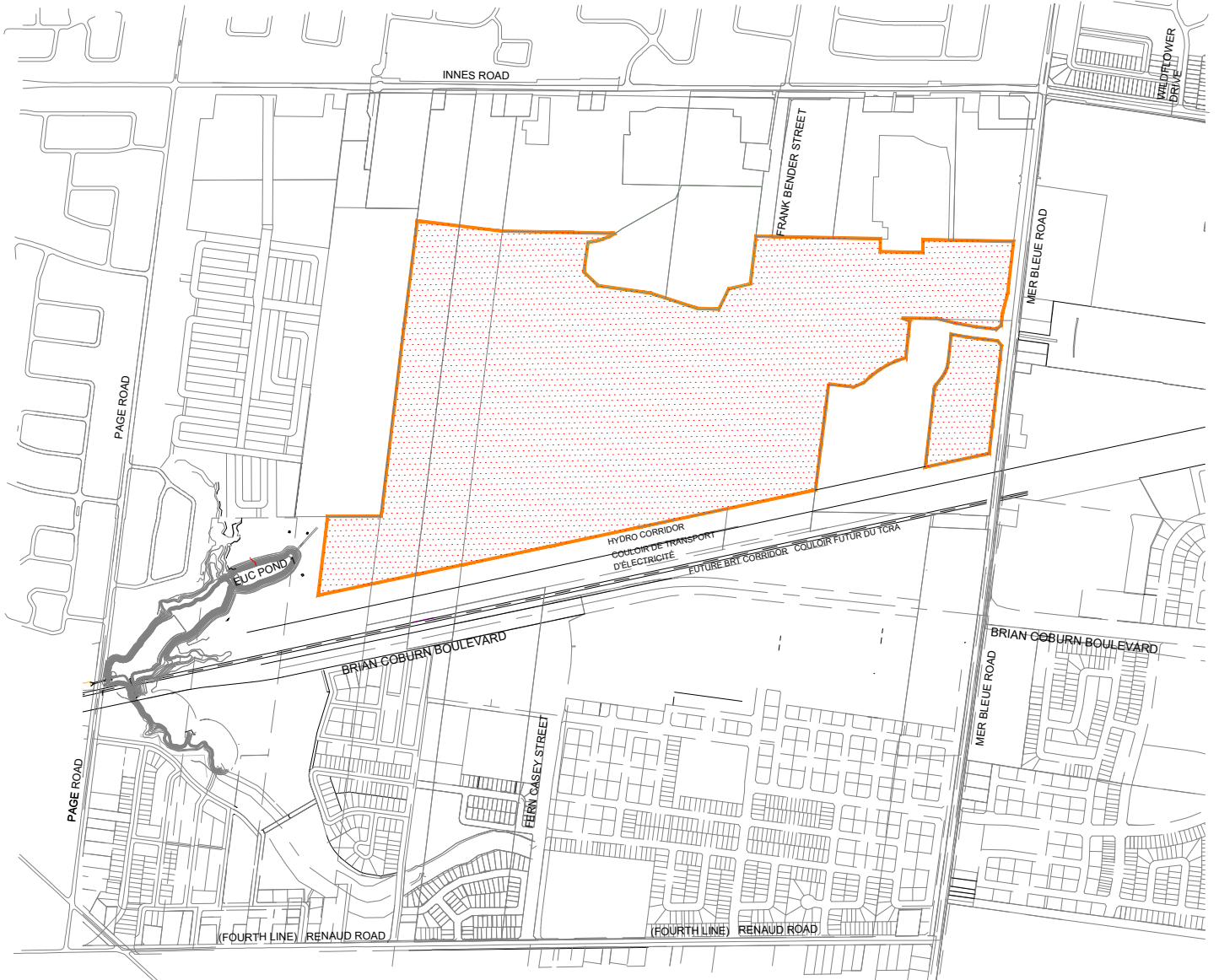
Definitions:
 Q = 2.78 AIR, where
 Q = Peak Flow in Litres per second (L/s)
 A = Areas in hectares (ha)
 I = Rainfall Intensity (mm/h)
 R = Runoff Coefficient

Notes:
 1) Ottawa Rainfall-Intensity Curve
 2) Min. Velocity = 0.80 m/s



Designed: R.B.	PROJECT: Orleans EUC MUC
Checked: K.M.	LOCATION: City of Ottawa
Dwg Reference:	File Ref: 14-733
	Date: October 2019
	Sheet No: 2

DRAWINGS & FIGURES



LEGEND

 **SITE BOUNDARY**

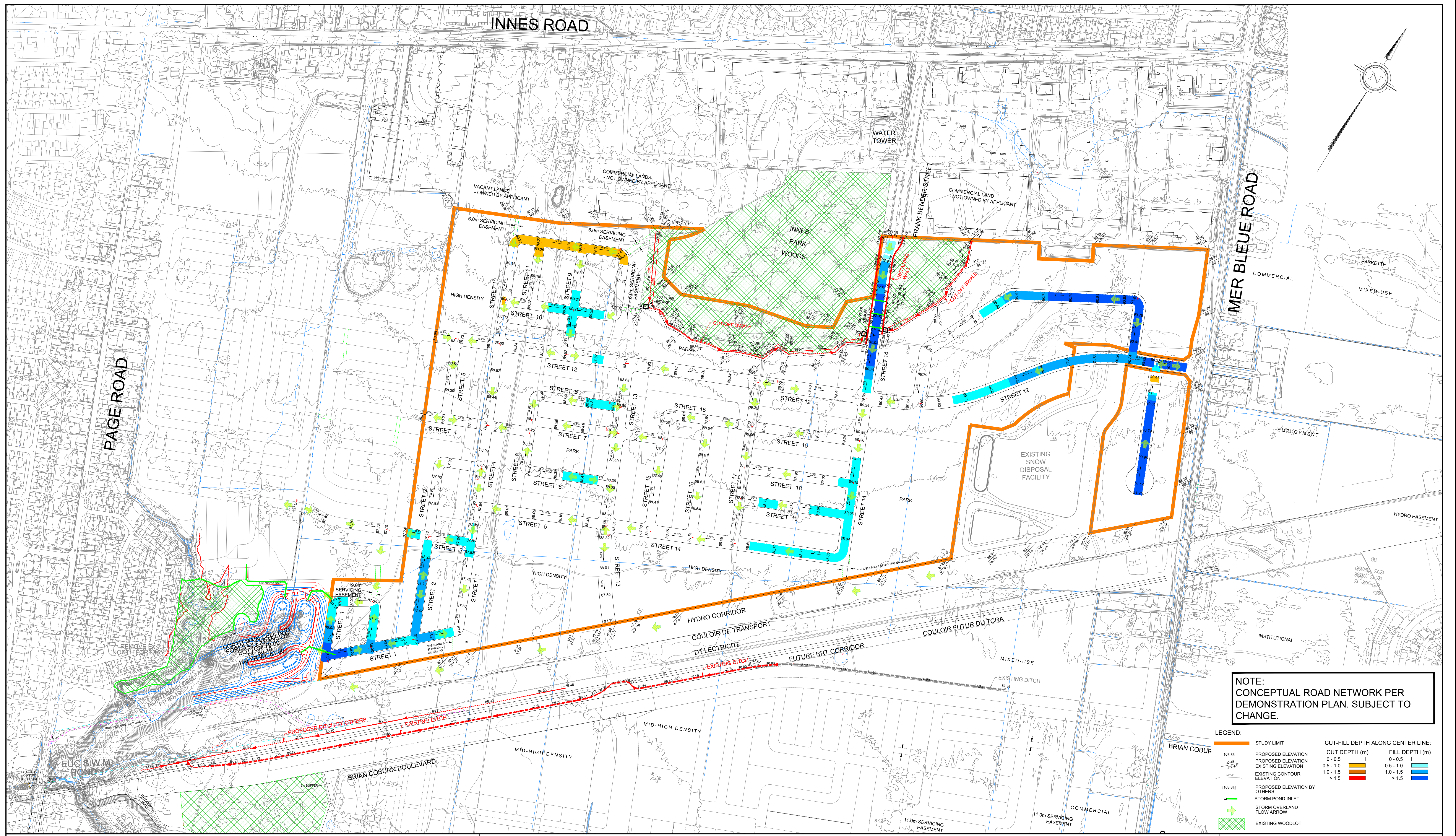
TRAILSEDGE NORTH

SITE LOCATION



120 Iber Road, Unit 203
Stittsville, ON K2S 1E9
TEL: (613) 836-0856
FAX: (613) 836-7183
www.DSEL.ca

DATE:	September 2020
SCALE:	1:15,000
PROJECT No.:	20-1195
FIGURE:	1



NOTE:
 CONCEPTUAL ROAD NETWORK PER
 DEMONSTRATION PLAN. SUBJECT TO
 CHANGE.

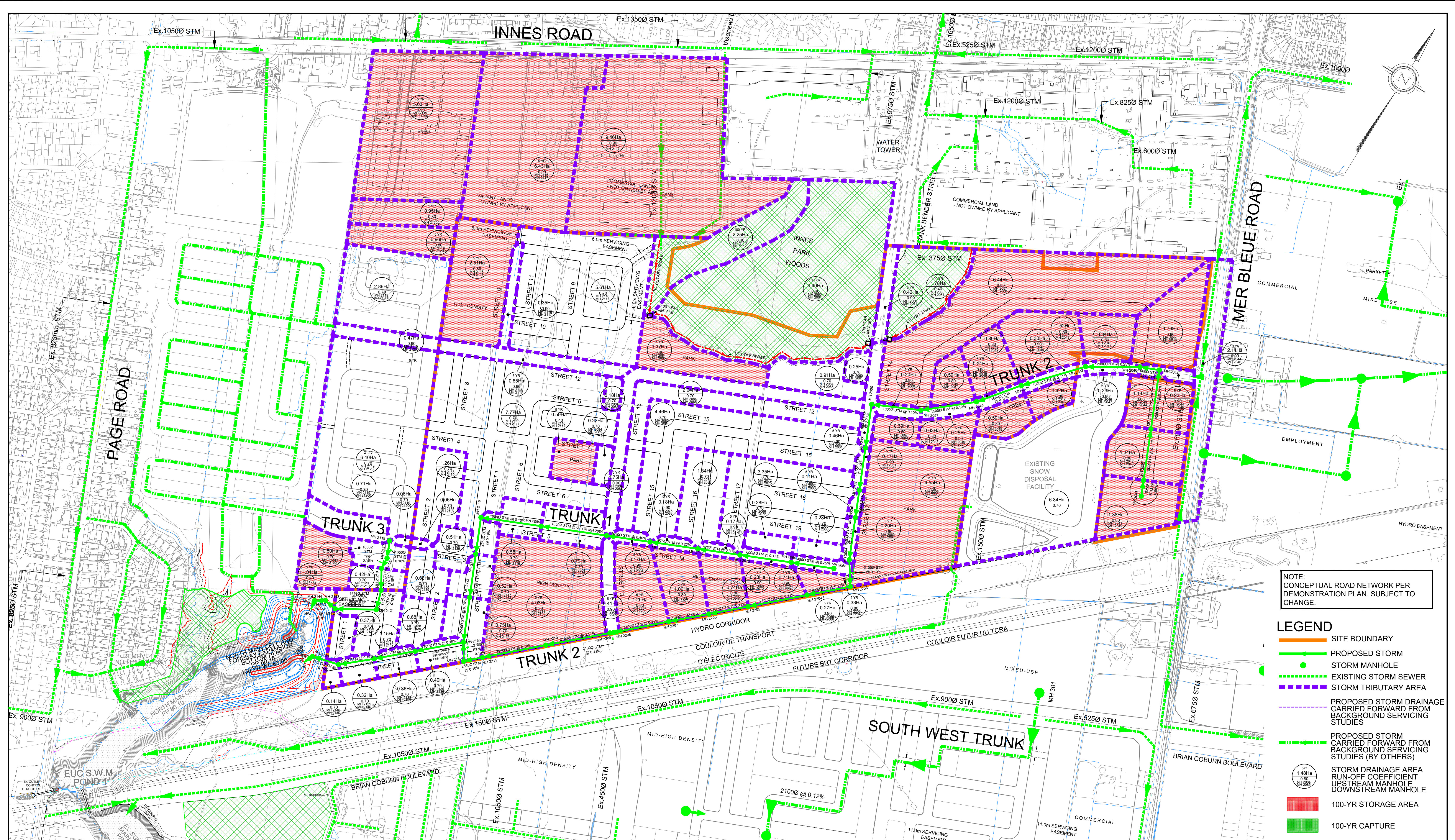
LEGEND:		CUT-FILL DEPTH ALONG CENTER LINE:	
[Orange line]	STUDY LIMIT	CUT DEPTH (m)	FILL DEPTH (m)
[Green line]	PROPOSED ELEVATION	0 - 0.5	0 - 0.5
[Red line]	EXISTING ELEVATION	0.5 - 1.0	0.5 - 1.0
[Blue line]	PROPOSED CONTOUR ELEVATION	1.0 - 1.5	1.0 - 1.5
[Green line]	PROPOSED ELEVATION BY OTHERS	> 1.5	> 1.5
[Green arrow]	STORM POND INLET		
[Green arrow]	STORM OVERLAND FLOW ARROW		
[Green hatched area]	EXISTING WOODLOT		



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**TRAILSEDGE NORTH
 GRADING PLAN**

PROJECT No. : 20-1195
 SCALE 1:3000
 DATE: SEPTEMBER 2020
 DRAWING No. 1



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

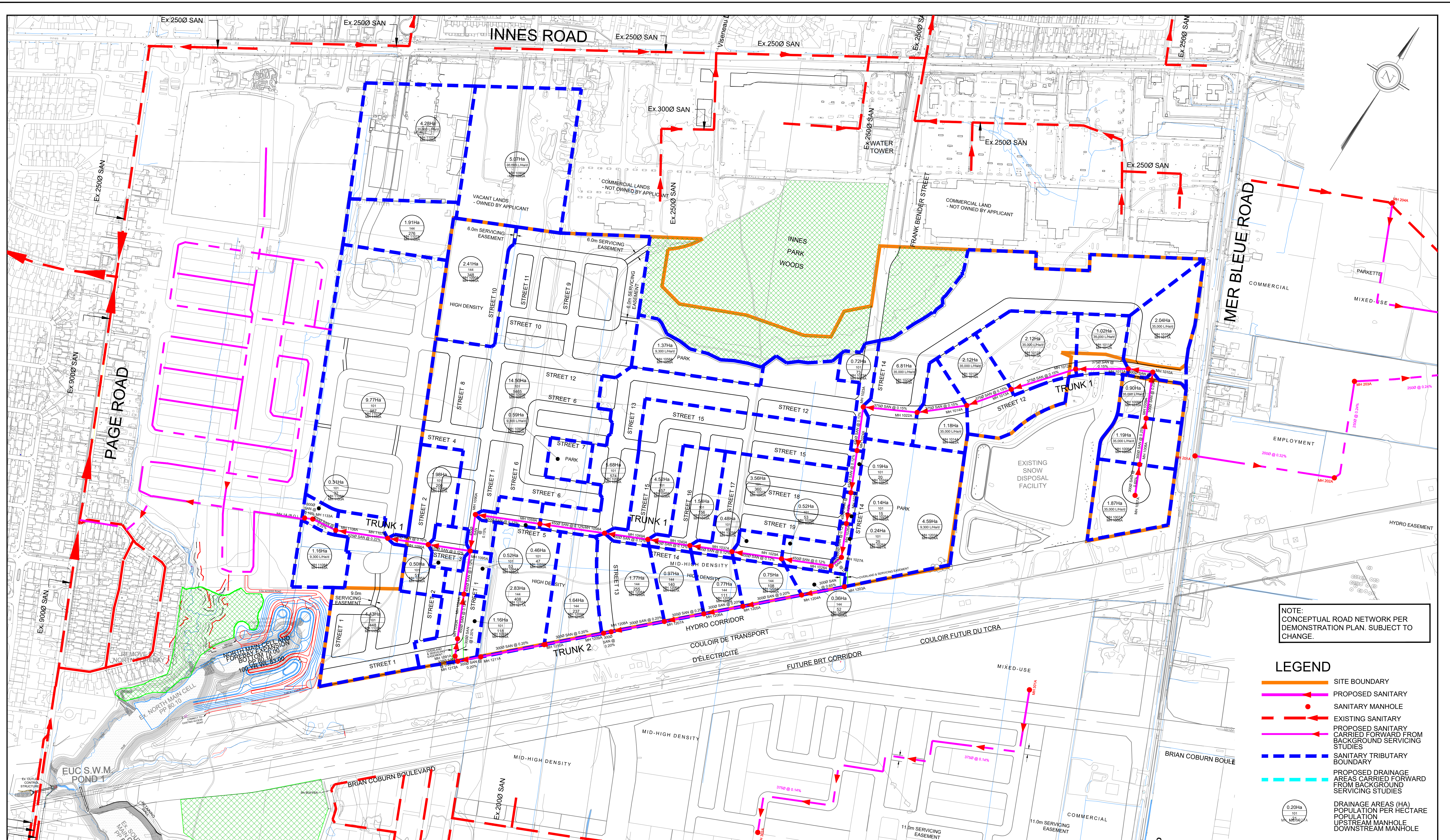
- LEGEND**
- SITE BOUNDARY
 - PROPOSED STORM
 - STORM MANHOLE
 - - - EXISTING STORM SEWER
 - - - STORM TRIBUTARY AREA
 - - - PROPOSED STORM DRAINAGE CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
 - - - PROPOSED STORM DRAINAGE CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES (BY OTHERS)
 - STORM DRAINAGE AREA RUN-OFF COEFFICIENT UPSTREAM MANHOLE DOWNSTREAM MANHOLE
 - 100-YR STORAGE AREA
 - 100-YR CAPTURE



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**TRAILSEDGE NORTH
CONCEPTUAL STORM SERVICING**

PROJECT No. : 20-1195
SCALE 1:3000
DATE: SEPTEMBER 2020
DRAWING No. 2



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

LEGEND

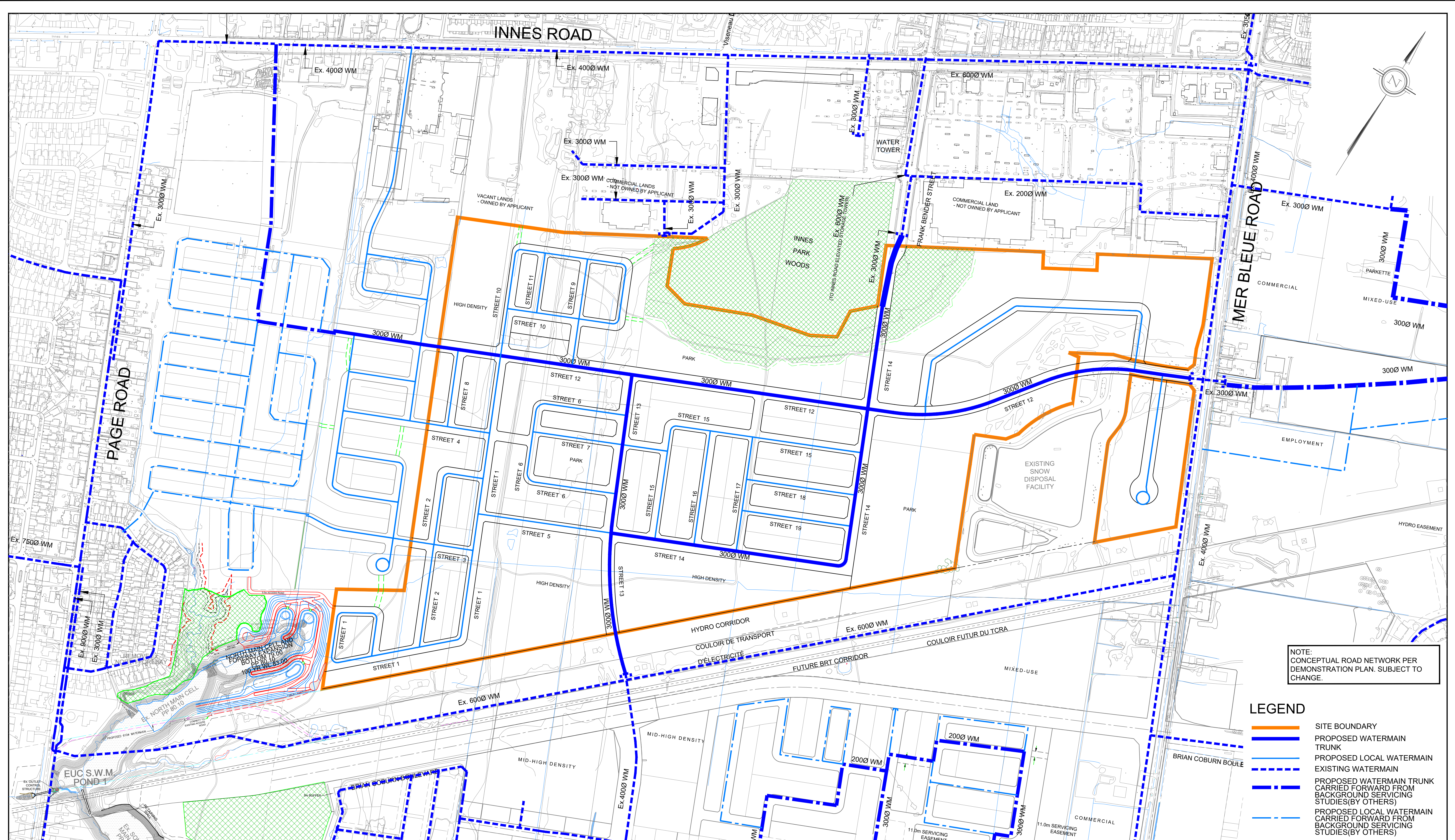
- SITE BOUNDARY
- - - PROPOSED SANITARY
- SANITARY MANHOLE
- EXISTING SANITARY
- - - PROPOSED SANITARY CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
- - - SANITARY TRIBUTARY BOUNDARY
- ▨ PROPOSED DRAINAGE AREAS CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES
- DRAINAGE AREAS (HA) POPULATION PER HECTARE UPSTREAM MANHOLE DOWNSTREAM MANHOLE



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**TRAILSEDGE NORTH
CONCEPTUAL SANITARY SERVICING**

PROJECT No. :	20-1195
SCALE	1:3000
DATE:	SEPTEMBER 2020
DRAWING No.	3



NOTE:
CONCEPTUAL ROAD NETWORK PER
DEMONSTRATION PLAN. SUBJECT TO
CHANGE.

- LEGEND**
- SITE BOUNDARY
 - PROPOSED WATERMAIN TRUNK
 - PROPOSED LOCAL WATERMAIN
 - - - EXISTING WATERMAIN
 - - - PROPOSED WATERMAIN TRUNK CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES (BY OTHERS)
 - - - PROPOSED LOCAL WATERMAIN CARRIED FORWARD FROM BACKGROUND SERVICING STUDIES (BY OTHERS)



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**TRAILSEDGE NORTH
WATERMAIN SERVICING**

PROJECT No. :	20-1195
SCALE	1:3000
DATE:	SEPTEMBER 2020
DRAWING No.	4