

3430 Carling Avenue

TIA Strategy Report

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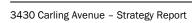




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STRATEGY REPORT

Parsons has been retained by 3430 Carling Inc. to prepare a Transportation Impact Assessment (TIA) in support of an Official Plan Amendment (OPA) and Zoning By-Law Amendment (ZBLA) applications for a residential development located at 108 Nepean Street. This document follows the TIA process, as outlined in the City Transportation Impact Assessment (TIA) Guidelines (2017). The following report represents Step 3 – Forecasting Report.

1. Screening Form

The screening form confirmed the need for a TIA Report based on the site meeting the trip generation, location, and safety triggers. The trip generation trigger is met due to the number of person trips anticipated to be generated by the development exceeding 60 person trips per hour. The safety trigger is met due to the horizontal curvature on a boundary street that limits sight lines of the site's proposed driveway. The Screening Form has been provided in Appendix A.

2. Scoping Report

2.1. Existing and Planned Conditions

2.1.1. Proposed Development

The proposed development is located on Carling Avenue, at the municipal addresses of 3430 Carling Ave. It is understood that the development proposes a 9-storey residential building that includes approximately 216 residential units with surface and underground parking, to be constructed in a single phase by 2022. 266 vehicle parking spaces are proposed, 12 spaces provided in a surface parking lot and 254 spaces provided in a two-level underground parking garage. A single driveway access to the site is proposed on Carling Avenue. The site is currently zoned as GM20[2628] H(18.5). The site is currently occupied by a single storey restaurant and surface parking lot. Local context is provided in Figure 1 and the Site Plan of the proposed development is illustrated in Figure 2.

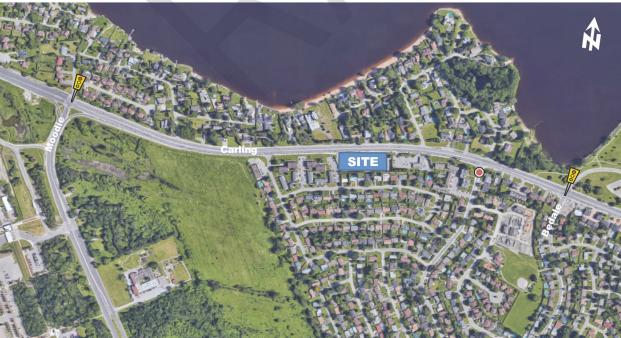
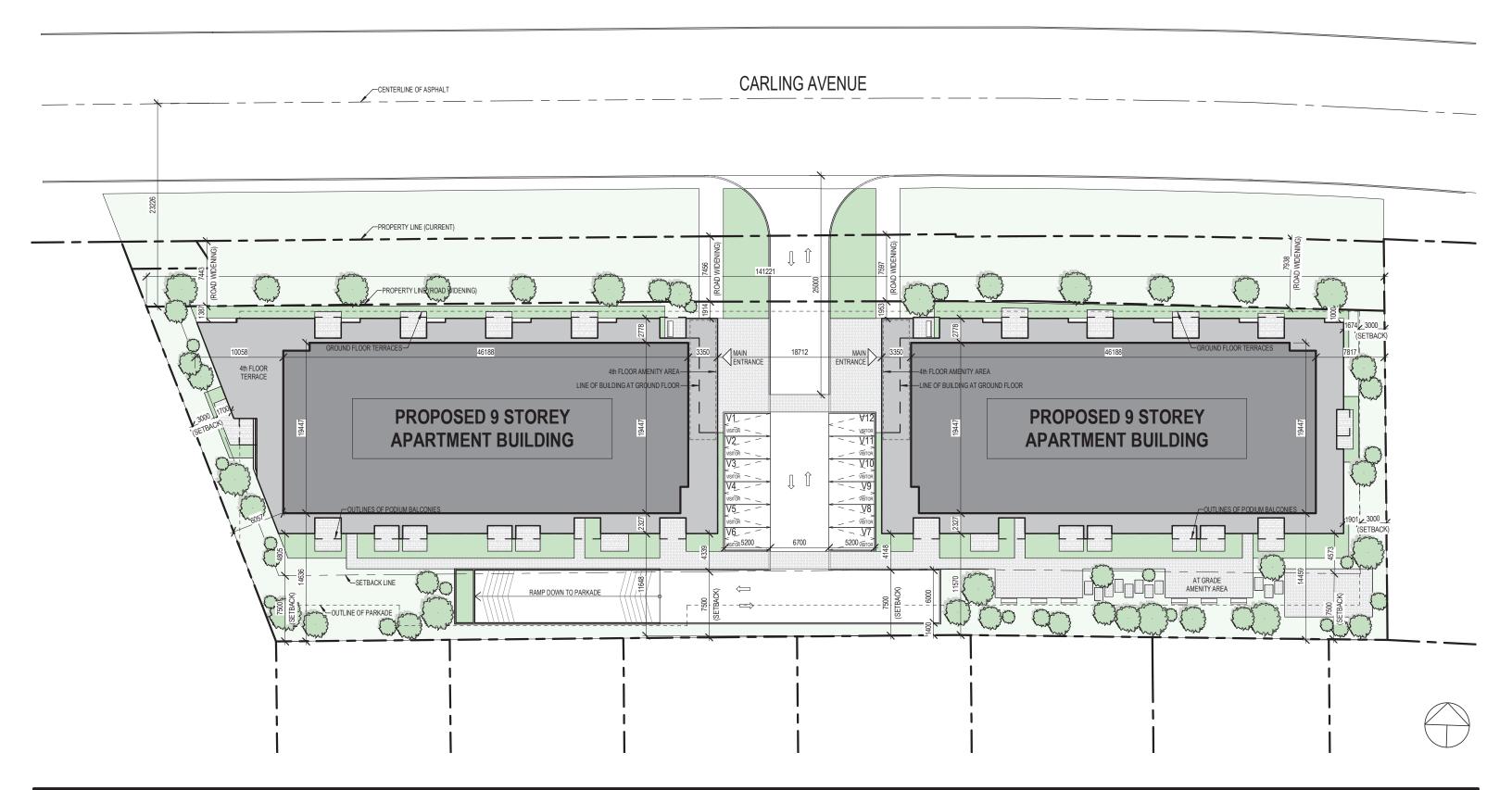


Figure 1: Local Context

Source: Google Maps





2.1.2. Existing Conditions

Area Road Network

Carling Avenue is an east-west municipal road that extends from March Road in the west to Bronson Avenue in the east. Within the study area, Carling Avenue is classified as an arterial road and operates with a four-lane undivided cross-section. Auxiliary turn lanes are provided at side streets and major intersections. The posted speed limit is 60km/h.

Moodie Drive is a north-south municipal road that extends from Carling Avenue in the north to Brophy Drive in the south. Within the study area, Metcalfe St is classified as an arterial road and operates with a four-lane undivided cross-section. The posted speed limit is 80 km/h.

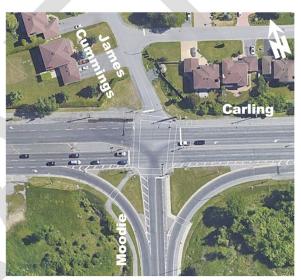
Bedale Drive is north-south municipal local road that extends from Carling Avenue in the north to Crystal Beach Drive in the south. Bedale Drive operates with a two-lane cross-section with on-street parking provided on the east side of the road. The posted speed limit is 40km/h.

Existing Study Area Intersections

The following describes the existing physical geometry of the study area intersections.

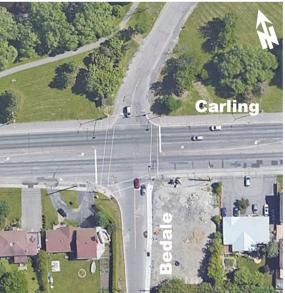
Moodie/Carling

The Moodie/Carling intersection is a signalized four-legged intersection. The eastbound approach consists of a shared left-through lane, a through lane and a channelized right-turn lane. The westbound approach consists of an auxiliary left-turn lane, a through lane and shared through-right turn lane. The northbound approach consists of a shared left-through lane and a channelized right-turn lane. The southbound approach consists of a single all-movement lane. All movements are permitted at this location. Painted crosswalks are provided on all legs of this intersection.



Bedale/Carling

The Bedale/Carling intersection is a signalized four-legged intersection. The eastbound approach consists of an auxiliary left-turn lane, two through lanes and an auxiliary right-turn lane. The westbound approach consists of an auxiliary left-turn lane, through lane, and a shared through/right-turn lane. The north and southbound approaches consist of a single all-movement lane. All movements are permitted at this location. Painted crosswalks are provided on all legs of this intersection.





Existing Driveways to Adjacent Developments

The subject site, which is currently a restaurant, has two driveways to Carling Avenue. The following provides a list of adjacent driveways located within 200m of the site along the boundary road (Carling Ave.):

South Side of Roadway, Distance from East Site Access

- 15 m 3420 Carling Ave, Restaurant Entrance
- 70 m 3420 Carling Ave, Restaurant & Gas Station Entrance
- 95 m 3420 Carling Ave, Gas Station Entrance

North Side of Roadway, Distance from East Site Access

- 0 m 3375 Carling Avenue, residential access
- 30 m 3379 Carling Avenue, residential access
- 55 m 3381 Carling Avenue, residential access
- 80 m 3383 Carling Avenue, residential access
- 155 m 3357 Carling Avenue, residential access

North Side of Roadway, Distance from West Site Access

- 10 m 3401 Carling Avenue, residential access
- 55 m 3405 Carling Avenue, residential access
- 95 m 3409 Carling Avenue, residential access
- 145 m 415 Carling Avenue, residential access



Figure 3: Adjacent Parking Lot Locations

Existing Area Traffic Management Measures

Existing area traffic management measures within the study area include:

- Sidewalks;
- Multi-use pathway (MUP); and,
- Curbs.

Pedestrian/Cycling Network

Figure 4 illustrates active transportation facilities within the study area. Sidewalks are provided on the south side Carling Avenue, and west side of Moodie Drive and Bedale Drive throughout the study area. A MUP is available along Carling Avenue and extends from Grandview Road in the west and Andrew Hayden Park in the east where it continues along the Ottawa River. Based on the City of Ottawa Transportation Master Plan (TMP),



Carling Avenue is classified as a Major Pathway, Moodie Drive is classified as a Spine Route and Bedale Drive is classified as a Local Route with regards to the overall cycling network.

Figure 4: Existing Pedestrian and Cycling Network

Source: https://maps.ottawa.ca/geoottawa/

Transit Network

The existing transit network surrounding the proposed development site is illustrated in Figure 5. Bus stops within walking distance to the site are approximately 170m to 205m in both directions. Transit stop locations are shown highlighted blue in Figure 6. Currently, Connection Route #258 and Local Route #58 service the site.

- Route #58 (Moodie <-> Tunney's Pasture): Identified by OC Transpo as a 'local' route that operates approximately every 30 minutes and includes various additional stops depending on time of day. During peak periods, Route #55 provides service between Moodie and Tunney's Pasture with connection to Confederation Line 1. During the off-peak periods, the route provides service from Lincoln Fields through to Moodie. Nearest stops are located along Carling Avenue, approximately 125 meters east and west of the proposed site.
- Route #258 (Elmvale <-> Bayshore): Identified by OC Transpo as a 'connexion' route that only operates during peak periods. It travels westbound during the morning peak period and eastbound during the afternoon peak period. When in operation, the bus runs on a schedule of approximately 30-minute interval. Route #258 provides service from Tunney's Pasture through to Grandview. Nearest stops are located along Carling Avenue, approximately 125 meters east and west of the proposed site.







Peak Hour Travel Demands

The existing peak hour traffic volumes within the study area were obtained from the City of Ottawa for the following intersections:

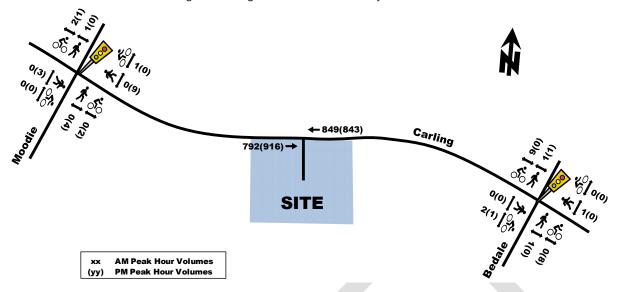
- Moodie/Carling conducted Wednesday, Mar. 30, 2016; and,
- Bedale/Carling conducted Wednesday, Feb. 28, 2018.

Figure 7 displays the existing vehicle traffic volumes while Figure 8 shows the existing pedestrian and cyclist volumes. Peak hour count data is provided in Appendix B.

Figure 7: Existing Peak Hour Vehicle Traffic Volumes



Figure 8: Existing Peak Hour Pedestrian and Cyclist Volumes



Existing Road Safety Conditions

A five-year collision history data (2015-2019, inclusive) was requested and obtained from the City of Ottawa for all intersections and road segments within the study area. Upon analyzing the collision data, the total number of collisions occurring within the study area was determined to be 48 collisions within the past five-years. Of the reported collisions, 1 (2%) resulted in fatal injury, 35 (73%) resulted in property damage and 12 (25%) resulted in non-fatal injury. The types of impact were broken down into the following: 14 (29%) rear-end, 13 (27%) turning movement, 11 (23%) single vehicle (other), 6 (13%) angle, 2 (6%) sideswipe, and 1 (2%) other. It is noted that of the total collisions, 1 collision involved a pedestrian, 1 involved a cyclist, and 5 involved wild animals. These resulted in property damage or non-fatal injuries. The fatal collision occurred at the Carling/Moodie intersection and involved an WBL turning vehicle and EBT vehicle.

To help quantify the relative safety risk at intersections within the study area, an industry standard unit of measure for assessing collisions at an intersection was used based on the number of collisions per million entering vehicles (MEV). An MEV value greater than 1.00 indicates a relatively high frequency of collisions. Furthermore, the City of Ottawa TIA Guidelines identifies more than six collisions of the same nature occurring within a five-year period to be a collision pattern. Reported collisions have historically taken place at a rate of:

- <u>Carling/Bedale:</u> 0.17 collisions/MEV, with a total of 6 collisions occurring within the five-year period. No particular collision pattern is present.
- Carling/Moodie: 0.53 collisions/MEV, with a total of 27 collisions occurring within the five-year period. No particular collision pattern is present. The collisions consisted of 6 rear ends, 12 turning movement, 2 sideswipes, 1 angled, and 6 single vehicle (other) collisions. Of the 12 turning movement collisions, 4 occurred between EBL turning vehicles and WBT vehicles and 8 occurred between WBL turning vehicles and EBT vehicles. There appears to be a collision pattern between the WBL and EBT vehicle movements. This will be further explored in Section 4.3 Boundary Road Design of the Step 4 Strategy Report. No other collision patterns are present.

With regards to road segments within the study area, the following collision data is identified:

• <u>Carling Avenue between Moodie Drive and Bedale Drive:</u> a total of 15 collisions occurred along this road segment within the past five-years. No particular collision pattern is present.

The source collision data as provided by the City of Ottawa and related analysis are provided as Appendix C.

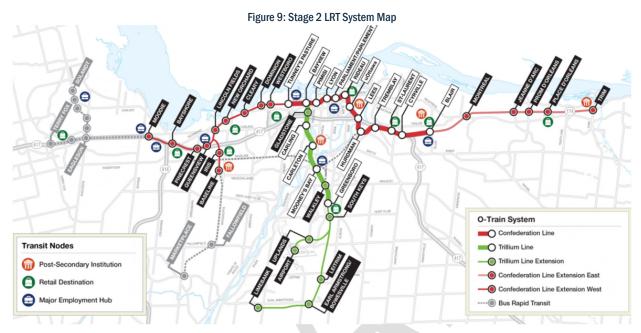


2.1.3. Planned Conditions

Planned Study Area Transportation Network Changes

LRT Stage 2

Stage 2 of the City of Ottawa LRT system is currently under construction. Stage 2, as shown in Figure 9, is a combination of three extensions – south, east, and west – totaling 44 km of new rail and 24 new LRT stations. The proposed development site is located approximately 2.4km driving distance of the LRT's Moodie Station.



Other Area Developments

The following section outlines adjacent developments in the general area that were considered in the TIA. The criteria for inclusion of other area developments is the proximity to the proposed development site and the potential impact to study area intersections. Developments that are either approved or have an active planning application in the City are included below.

1) 3368 Carling Avenue

A Transportation Brief was prepared by Stantec in January 2016, for a residential development consisting of a three-storey apartment building with 15 units and 18 underground parking spaces. The development is not anticipated to generate a significant amount of traffic and impact study area intersections and as such, will not be included in the traffic analysis.

2.2. Study Area and Time Periods

Full buildout of the proposed residential development is assumed to be 2022. As such, the horizon years being analyzed in this report are 2022 and 2027 (five years after full buildout) horizon years, using the weekday morning and afternoon peak hour time periods.

Proposed study area intersections and boundary roads are outlined below and highlighted in Figure 10.



Figure 10: Study Area Intersections



- Carling/Moodie (signalized)
- Carling/Bedale (signalized)
- Carling Avenue adjacent to the site

2.3. Exemption Review

The following modules/elements of the TIA process recommended to be exempt in the subsequent steps of the TIA process, based on the City's TIA guidelines and the subject site:

Table 1: Exemptions Review Summary

Module	Element	Exemption Consideration
4.1 – 4.4 Design Review Component	All elements	Not required for applications involving ZBLA or OPA. However, a brief description may be provided.
4.8 Network Concept	4.8 Network Concept	Only required if proposed development is anticipated to generate more than 200 person-trips over the permitted zoning

3. Forecasting Report

3.1. Development Generated Travel Demand

3.1.1. Trip Generation and mode shares

The proposed development will consist of 216 residential units within a 9-storey high-rise apartment building. The appropriate trip generation rates for a high-rise apartment land use were obtained from the 2020 TRANS Trip Generation Manual. Table 3 in the Manual provides person-trip rates during the peak AM and PM periods (7am-9:30am and 3:30PM-6PM). The trip rates are summarized in Table 2 below.

Table 2: Trip Generation Trip Rates

Land Use	Data	Trip Rates				
Land USE	Source	AM Peak Period (7-9:30am)	PM Peak Period (3:30-6pm)			
High-Rise Apartments (9 floors)	TRANS 2020	T = 0.8(du);	T = 0.9(du);			
Notes: T = Average Vehicle Trip Ends						
du = Dwelling unit						

Using the trip rates provided in Table 2, the total number of person trips generated during the morning and afternoon peak periods can be found in Table 3.

Table 3: Apartment Units Peak Period Person Trip Generation

Land Use	Dwelling Units	AM Peak Period Person Trips	PM Peak Period Person Trips
High-Rise Apartments (9 floors)	216	173	194

The proposed development is anticipated to generate 200 and 225 person trips during the morning and afternoon peak periods, respectively. The total peak period person trips in Table 3 are then divided into different travel modes, as shown in Table 4, using mode share percentages obtained from the 2020 TRANS Manual.



Table 4: Peak Period Trips Mode Shares Breakdown

Travel Mode	Mode Share	AM Peak Period Person Trip	Mode Share	PM Peak Period Person Trips
Auto Driver	45%	78	47%	91
Auto Passenger	13%	22	16%	31
Transit	31%	54	25%	49
Cycling	2%	3	1%	2
Walking	9%	16	11%	21
Total Person Trips	100%	173	100%	194

Standard traffic analysis is usually conducted using the morning and afternoon peak hour trips as they represent a worst-case scenario. In the 2020 TRANS Manual, Table 4 provides conversions rates from peak period to peak hours for different mode shares. The conversion rates are provided in Table 5 below.

Table 5: Peak Period to Peak Hour Conversion Factors (2020 TRANS Manual)

Travel Mode	Peak Period to Peak Hour Conversion Factors					
Travel Mode	AM	PM				
Auto Driver	0.48	0.44				
Passenger	0.31	0.29				
Transit	0.55	0.47				
Bike	0.58	0.48				
Walk	0.58	0.52				

Note that conversion factors for auto passenger trips are not available in the 2020 TRANS Manual. To obtain the passenger trip factor it is assumed that the total person trip peak hour conversion factor is the average of the provided adjustment factors minus the passenger trip peak hour conversion factor and has been calculated as shown in the example below:

$$0.5 = \frac{x + 0.48 + 0.55 + 0.58 + 0.58}{5}$$

$$x = 2.5 - 0.48 - 0.55 - 0.58 - 0.58$$

$$x = 0.31 \rightarrow AM \ passenger \ trip \ peak \ hour \ conversion \ factor$$

Using the conversion rates in Table 5 and the peak period person trips for different travel modes in Table 4, the peak hour trips for different travel modes can be calculated as shown in Table 6. Inbound and outbound percentages were obtained from Table 9 of the 2020 TRANS Manual.

Table 6: Peak Hour Travel Mode Trips

Travel Mode	AM I	Peak (Person Tr	ips/h)	PM Peak (Person Trips/h)			
Travel Mode	In (31%)	Out (69%)	Total	In (58%)	Out (42%)	Total	
Auto Driver	11	26	37	23	17	40	
Passenger	2	5	7	5	4	9	
Transit	9	21	30	13	10	23	
Bike	1	1	2	1	0	1	
Walk	3	6	9	6	5	11	
Total Person Trips	26	59	85	48	36	84	
Total "New" Vehicle Trips	11	26	37	23	17	40	

As shown in Table 6, the total person trips anticipated to be generated by the proposed development are approximately 85 trips during both the morning and afternoon peak hours. Vehicle trips are anticipated to be approximately 40 veh/h during both the morning and afternoon peak hours. Approximately 25 to 35 new transit trips and approximately 10 new active mode trips are anticipated.



3.1.2. Trip Distribution and 3.1.2 Trip Assignment

Based on the 2011 OD Survey (Ottawa Inner Area district) and the location of adjacent arterial roadways and neighbourhoods, the distribution of site-generated traffic volumes was estimated as follows:

- 10% to/from the west, via Carling Avenue;
- 25% to/from the east, via Carling Avenue; and,
- 65% to/from the south, via Moodie Drive and HWY-417.

The anticipated site-generated auto trips for the proposed development from Table 6 were then assigned to the road networks as shown in Figure 11.

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Figure 11: 2022 Site-Generated Traffic

3.2. Background Network Traffic

3.2.1. Transportation network plans

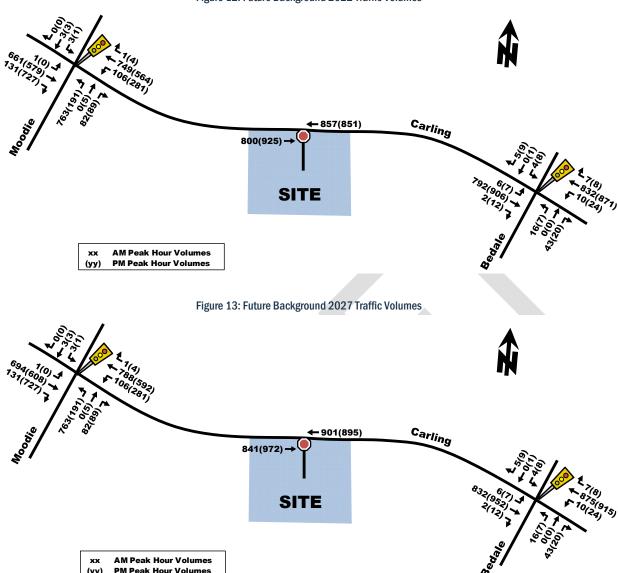
Refer to Section 2.1.3: Planned Study Area Transportation Network Changes.

3.2.2. Background Growth

Given that the proposed development will be located along the well-developed Carling Avenue and adjacent to the NCC Greenbelt, traffic along study area roadways is not anticipated to increase drastically within the future horizon years. Nonetheless, a background growth rate of 1% has been applied to the Carling Avenue to account for trips that may be generated by future other area developments that are minor or located outside the scope of the study area. Figure 12 provides the future background traffic in 2022 and Figure 13 provides the future background traffic in 2027.



Figure 12: Future Background 2022 Traffic Volumes



3.2.3. Other Developments

Description of other area developments taking place within the study area was provided in Section 2.1.3: Other Area Developments. As mentioned previously, this development is not expected to generate significant traffic and as such, will not be included in background traffic.

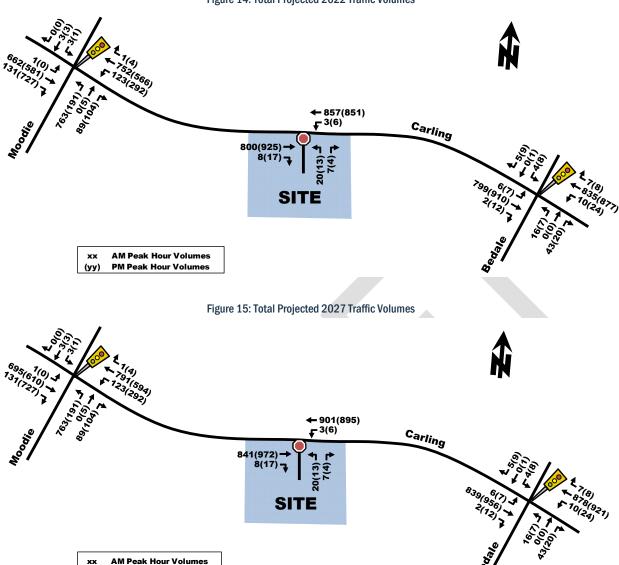
3.3. Demand Rationalization

The total projected future traffic volumes can be determined by superimposing the site-generated traffic volumes in Figure 11, onto the total future background traffic volumes in Figure 12 and Figure 13, resulting in the total projected traffic volumes 2022 and 2027 illustrated in Figure 14 and Figure 15.

Since this site is anticipated to generate few vehicle trips during the morning and afternoon peak hours (i.e., approximately 45 two-way trips per peak hour or on averages of less than 1 vehicle a minute) it is anticipated that the additioplnal trips will have little impact on the vehicle operations along the study area intersections. Further analysis will be provided as part of Step 4 Strategy Report.



Figure 14: Total Projected 2022 Traffic Volumes



4. Analysis

4.1. Development Design

<u>(</u>yy)

Exempt - see Section 2.3 Exemption Review.

PM Peak Hour Volumes

4.2. Parking

Exempt - see Section 2.3 Exemption Review.

Currently 266 vehicle parking spaces are proposed, 12 spaces provided in a surface parking lot and 254 spaces provided in a two-level underground parking garage.

By-law requirements for vehicle and bicycle parking spaces will be confirmed in the subsequent Site Plan Application.



4.3. Boundary Street Design

Exempt - see Section 2.3 Exemption Review.

The detailed Multi-Modal Level of Service (MMLOS) analysis for boundary streets and signalized intersections will be provided in the future Site Plan Application.

4.4. Access Intersection Design

Exempt - see Section 2.3 Exemption Review.

4.5. Transportation Demand Management

4.5.1. Context for TDM

Based on the type of development, it is assumed that most trips generated by the proposed site will be residents leaving the site in the AM peak to go to work and returning from work to the proposed site in the PM peak. Sections 3.1.1 and 3.1.2 describe how many trips are anticipated per travel mode and anticipates the likely locations that they will travel to and from based on the OD-Survey 2011 for Ottawa. The site is not located in a Transit-Oriented Development (TOD) zone or the "Downtown Ottawa Urban Design Strategy" Design Priority Area (DPA) according to the Official Plan.

4.5.2. Need and Opportunity

The proposed development is located in a well-developed suburb area of the City of Ottawa, where transit and active transportation facilities, such as the bike tracks and the sidewalks, are well-maintained and developed. However, the distance from the City core and employment areas naturally results in higher auto mode shares. For this reason, a program of post-occupancy TDM measures is not likely in decrease auto mode shares significantly.

4.5.3. TDM Program

The TDM Infrastructure and TDM Measures Checklists have been provided in Appendix D. The proposed measures for each respective checklist are provided below.

Proposed measures identified in the TDM Measures Checklist are:

Nothing has been checked as of the submission of this report.

Proposed measures identified in the TDM-supportive Development Design and Infrastructure Checklist are:

- Locate building close to the street, and do not locate parking areas between the street and building
- Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations
- Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort
- Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations
- Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances
 through such measures as: reducing distances between public sidewalks and major building
 entrances; providing walkways from public streets to major building entrances; within a site, providing
 walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas
 where people may congregate, such as courtyards and transit stops; and providing weather protection
 through canopies, colonnades, and other design elements wherever possible



- Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks
- Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps
- Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by
 active transportation. Provide links to the existing or planned network of public sidewalks, multi-use
 pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with
 roads, consider providing traffic control devices to give priority to cyclists and pedestrians

4.6. Neighbourhood Traffic Management

As the site uses an arterial roadway (Carling Avenue) for access, this section is exempt.

4.7. Transit

Transit facilities are anticipated to continue operating in the future as mentioned in Section 2.1.2: Transit Network. The proposed development is anticipated to generate 34 and 24 two-way transit trips during the morning and afternoon peak hours respectively. As such, the proposed development will have little impact to the surrounding transit network.

4.8. Review of Network Concept

Exempt - see Section 2.3 Exemption Review.

4.9. Intersection Design

4.9.1. Intersection control

The site accesses to the surface and underground parking garage are assumed to use Stop control for vehicles exiting.

4.9.2. Intersection design

Synchro 10 Trafficware was used to analyze intersection performance of intersections within the study area. Critical movements at each of the intersections were assessed based on either the movement with the highest volume-to-capacity ratio (for signalized intersections), or the movement experiencing the highest average delay (for unsignalized intersections). It should be noted that, as per the TIA Guidelines, the Peak Hour Factor (PHF) used for analysis was 0.95 in existing conditions and 1.0 in all future scenario conditions. All Synchro report outputs for existing and future conditions have been provided in Appendix E.

Existing Conditions

Table 7 below summarizes the intersection performance of study area intersections, based on the existing conditions traffic volumes illustrated in Figure 7.

Table 7: Existing Conditions Intersection Performance

Intersection	Weekday AM Peak (PM Peak)						
	Critical Movement			Intersection 'As a Whole'			
mersouon	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c	
Moodie/Carling (S)	F(C)	1.09(0.75)	NBT(NBT)	56.2(13.7)	E(B)	0.95(0.69)	
Bedale/Carling (S)	A(A)	0.33(0.33)	WBT(EBT)	5.5(4.7)	A(A)	0.32(0.31)	

Note: Analysis of signalized intersections assumes a PHF of 0.95 and a saturation flow rate of 1800 veh/h/lane.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



As shown in Table 7, signalized intersections 'as a whole' operate at a LOS 'B' or better during both peak hours, within City standards of LoS 'D' (v/c < 0.91). The exception is the Moodie/Carling intersection which operates at LoS 'E' during the morning peak hour. The critical movements operate at LOS 'C' or better with the exception of the NBT at the Moodie/Carling intersection where it operates at capacity with an LoS 'F'.

Total Future Background 2022

Table 8 below summarizes the Synchro traffic operations at study area intersections, based on the total future background 2022 volumes presented in Figure 12.

Table 8: Future Background 2022 Intersection Performance

	Weekday AM Peak (PM Peak)							
		Critical Move	ment	Intersection 'As a Whole'				
Intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c		
Moodie/Carling (S)	F(C)	1.03(0.75)	NBT(NBT)	49.5(13.1)	E(B)	0.91(0.67)		
Bedale/Carling (S)	A(A)	0.31(0.31)	WBT(EBT)	5.4(4.6)	A(A)	0.30(0.29)		

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As shown in Table 8, study area intersections are projected to operate similar or slightly better than existing conditions due to increasing the PHF to 1.0.

Total Future Background 2027

Table 9 below summarizes the Synchro traffic operations at study area intersections, based on future background 2027 traffic volumes in Figure 13.

Table 9: Total Future Background 2027 Intersection Performance

	Weekday AM Peak (PM Peak)							
	Critical Movement			Intersection 'As a Whole'				
Intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c		
Moodie/Carling (S)	F(C)	1.04(0.75)	NBT(NBT)	50.9(13.2)	E(B)	0.92(0.67)		
Bedale/Carling (S)	A(A)	0.33(0.33)	WBT(EBT)	5.5(4.7)	A(A)	0.32(0.31)		

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As indicated by Table 9, traffic operations are anticipated to be similar to the total future background 2022 traffic operations, with slightly higher delays and v/c ratios.

Total Projected 2022

Based on total projected 2022 traffic volumes in Figure 14, study area intersections were analyzed using Synchro, with results summarized in Table 10 below.

Table 10: Total Projected 2022 Intersection Performance

	Weekday AM Peak (PM Peak)										
		Critical Move	ment	Intersection 'As a Whole'							
Intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c					
Moodie/Carling (S)	F(C)	1.03(0.75)	NBT(NBT)	49.7(13.2)	E(B)	0.91(0.67)					
Bedale/Carling (S)	A(A)	0.31(0.32)	WBT(EBT)	5.4(4.6)	A(A)	0.30(0.30)					
Site Access/Carling (U)	D(D)	27.7(32.6)	NB(NB)	0.4(0.3)	A(A)	-					

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



As indicated by Table 10, traffic operations are anticipated to be similar to the total future background 2022 traffic operations, with slightly higher delays and v/c ratios. The unsignalized site access is projected to operate at an LoS 'A' with critical movements projected to operate at LoS 'D'.

Total Projected 2027

Based on total projected 2027 traffic volumes in Figure 15, study are intersections were analyzed using Synchro, with results summarized in Table 11 below.

Table 11: Total Projected 2027 Intersection Performance

	Weekday AM Peak (PM Peak)										
		Critical Move	ment	Intersection 'As a Whole'							
Intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c					
Moodie/Carling (S)	F(C)	1.04(0.75)	NBT(NBT)	51.1(13.3)	E(B)	0.93(0.67)					
Bedale/Carling (S)	A(A)	0.33(0.33)	WBT(EBT)	5.5(4.7)	A(A)	0.32(0.31)					
Site Access/Carling (U)	D(E)	30.4(36.5)	NB(NB)	0.5(0.4)	A(A)	-					

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As indicated by Table 11, traffic operations are anticipated to be similar to the total future background 2027 traffic operations, with slightly higher delays and v/c ratios.

5. Findings, Conclusions and Recommendations

Based on the results summarized herein, the following transportation related conclusions are offered:

Proposed Development

- The proposed development will be located at 3430 Carling Avenue.
- The development will consist of a high-rise residential building with 216 apartment units, constructed in a single phase by 2022.
- Access will be provided via a single driveway access located on Carling Avenue.
- Parking will be provided in a surface and underground parking lot. Currently 266 vehicle parking spaces
 are proposed, 12 spaces provided in a surface parking lot and 254 spaces provided in a two-level
 underground parking garage. Vehicle and bicycle parking numbers will be confirmed in the Site Plan
 Application.
- At full buildout in 2022, the development is anticipated to generate a total of 85 person trips during both
 peak hours. Vehicle trips are anticipated to be 40 veh/h during peak hours. Transit trips are anticipated
 to be 30 and 23 trips during the morning and afternoon peak hours respectively. Active transportation
 modes (bike and walk) are anticipated to generate 11 trips during the morning and afternoon peak hours.

Existing and Background Conditions

- In existing conditions, intersections 'as a whole' operate at a LOS 'B' or better during both peak hours, within City standards. The exception is the Moodie/Carling intersection which operates at LoS 'E' during the morning peak hour. Critical movement operate at LOS 'C' or better with the exception of the NBT at the Moodie/Carling intersection which operates at an LoS 'F'.
- A background growth rate of 1% per year was conservatively applied to study area intersections at horizon years 2022 and 2027.
- As required by the TIA Guidelines, the PHF in future conditions is increased to 1.0, which results in improved or similar traffic operations for total future background 2022 and 2027 compared to existing conditions.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



Projected Conditions

 Total projected 2022 and 2027 traffic operations are similar to their respective future background operations, with slightly higher delays and v/c ratios.

In summary, the proposed development will have little impact on the surrounding road network and transit facilities and is recommended to proceed from a transportation perspective.



APPENDIX A

SCREENING FORM



City of Ottawa 2017 TIA Guidelines

Date 8-Feb-21 **TIA Screening Form** Project 3430 Carling Avenue Project Number 908979 10052

Results of Screening	Yes/No
Development Satisfies the Trip Generation Trigger	Yes
Development Satisfies the Location Trigger	No
Development Satisfies the Safety Trigger	Yes

Module 1.1 - Description of Proposed Developmen	t en
Municipal Address	3430 Carling Ave.
Description of location	Property currently occupied by a single storey building (restaurant) and surface parking
Land Use	GM 20 (H 18.5)
Development Size	250 Residential units, 9-Storeys
Number of Accesses and Locations	2 driveway (assumed)
Development Phasing	1 phase (assumed)
Buildout Year	2022
Sketch Plan / Site Plan	See attached

Module 1.2 - Trip Generation Trigger		
Land Use Type	Townhomes or Apartments	
Development Size	250	Units
Trip Generation Trigger Met?	Yes	

Module 1.3 - Location Triggers		
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks (See Sheet 3)	No	
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone. (See Sheet 3)	No	
Location Trigger Met?	No	

Module 1.4 - Safety Triggers			
Posted Speed Limit on any boundary road	<80	km/h	
Horizontal / Vertical Curvature on a boundary street limits sight lines at a proposed driveway	Yes		
A proposed driveway is within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions) or within auxiliary lanes of an intersection:	No		
A proposed driveway makes use of an existing median break that serves an existing site	No		
There is a documented history of traffic operations or safety			
concerns on the boundary streets within 500 m of the	No		
development			
The development includes a drive-thru facility	No		
Safety Trigger Met?	Yes		



APPENDIX B

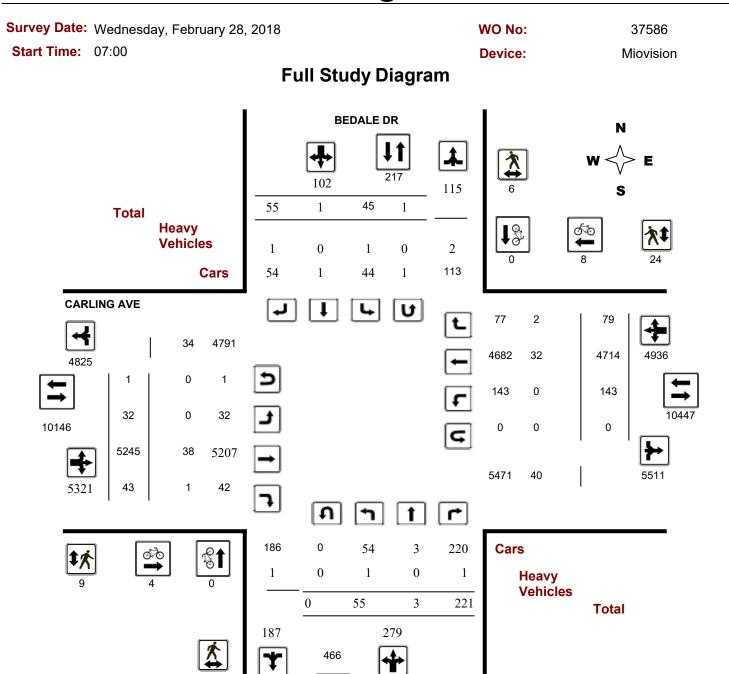
TRAFFIC COUNT DATA





Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR



April 1, 2021 Page 1 of 8



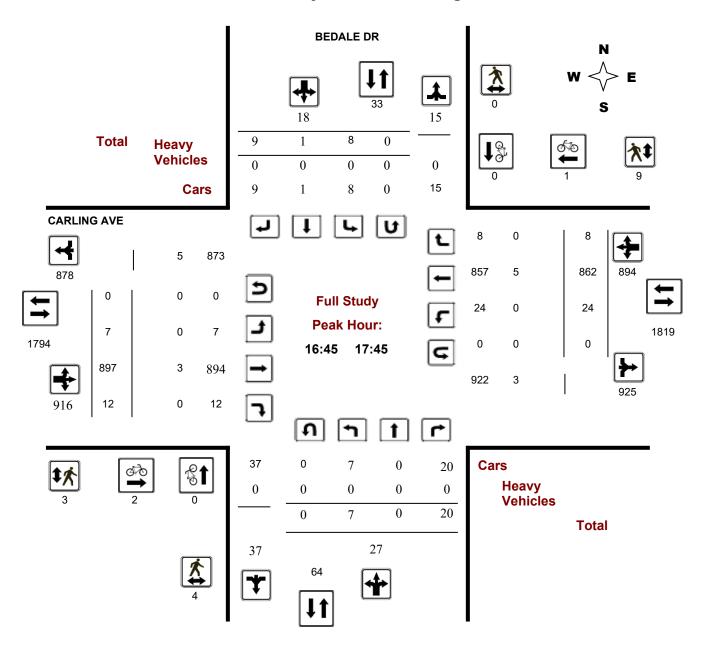
Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



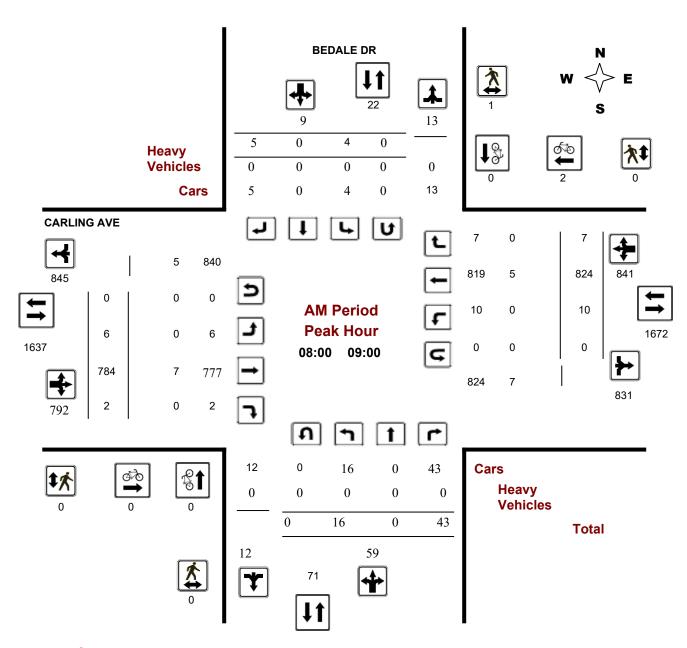
April 1, 2021 Page 2 of 8



Turning Movement Count - Peak Hour Diagram

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586
Start Time: 07:00 Device: Miovision



Comments

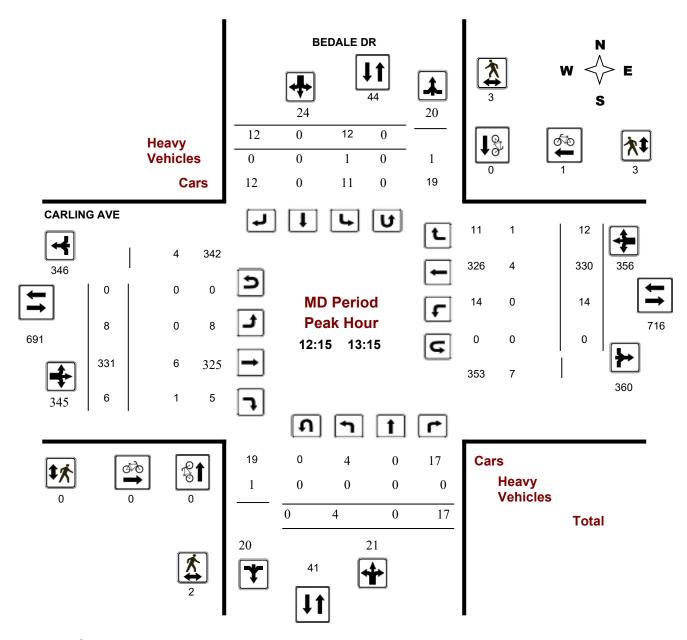
2021-Apr-01 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586
Start Time: 07:00 Device: Miovision



Comments

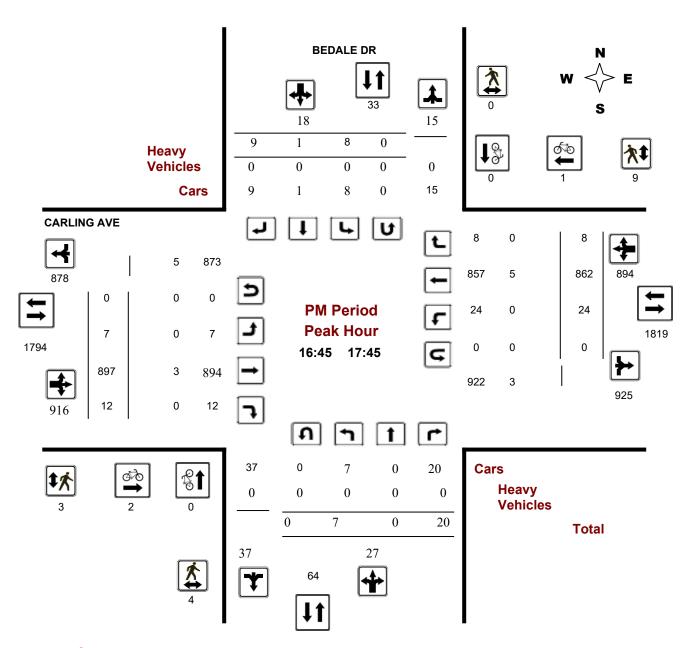
2021-Apr-01 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586
Start Time: 07:00 Device: Miovision



Comments

2021-Apr-01 Page 3 of 3



Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 **Device:** Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, February 28,

Total Observed U-Turns

AADT Factor

2018

Northbound: Eastbound: Southbound: Westbound:

1.31

1.00

			BE	DALE [OR .							CAI	RLING	AVE					
	Nor	thbou	nd		Sou	ıthbou	ınd			Е	astbou	ınd		V	/estbo	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	3	0	41	44	0	0	2	2	46	0	1102	1	1103	2	495	2	499	1602	1648
08:00 09:00	16	0	43	59	4	0	5	9	68	6	784	2	792	10	824	7	841	1633	1701
09:00 10:00	9	0	39	48	3	0	5	8	56	3	457	3	463	9	505	10	524	987	1043
11:30 12:30	4	3	22	29	6	0	3	9	38	4	333	4	341	25	281	13	319	660	698
12:30 13:30	4	0	19	23	13	0	14	27	50	6	307	8	321	13	323	16	352	673	723
15:00 16:00	9	0	21	30	9	0	8	17	47	3	588	6	597	30	684	12	726	1323	1370
16:00 17:00	3	0	13	16	4	1	7	12	28	3	841	3	847	29	801	10	840	1687	1715
17:00 18:00	7	0	23	30	6	0	11	17	47	7	833	16	856	25	801	9	835	1691	1738
Sub Total	55	3	221	279	45	1	55	101	380	32	5245	43	5320	143	4714	79	4936	10256	10636
U Turns	0			0	1			1	1	1			1	0			0	1	2
Total	55	3	221	279	46	1	55	102	381	33	5245	43	5321	143	4714	79	4936	10257	10638
EQ 12Hr	76	4	307	387	64	1	76	141	528	46	7291	60	7397	199	6552	110	6861	14258	14786
Note: These v	alues ar	e calcu	lated by	y multiply	ing the	totals b	y the ap	opropriate	e expans	ion fac	tor.			1.39					
AVG 12Hr	76	4	307	387	64	1	76	141	528	46	7291	60	7397	199	6552	110	6861	14258	14786
Note: These v	olumes :	are calc	culated	by multip	olying th	e Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1.00					
AVG 24Hr	100	5	402	507	84	1	100	185	692	60	9551	79	9690	261	8583	144	8988	18678	19370

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

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Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

BEDALE DR

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments CARLING AVE

Northbound Southbound Eastbound Westbound s STR W **STR** Grand Ε **Time Period** LT ST LT ST RT LT ST RT LT ST RT TOT TOT TOT TOT TOT TOT **Total** 07:00 07:15 07:15 07:30 07:30 07:45 07:45 08:00 08:00 08:15 08:15 08:30 08:45 08:30 08:45 09:00 09:15 09:00 09:15 09:30 09:30 09:45 09:45 10:00 11:30 11:45 11:45 12:00 12:00 12:15 12:15 12:30 12:30 12:45 12:45 13:00 13:00 13:15 13:15 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00 17:00 17:15 17:30 17:15 17:30 17:45 17:45 18:00 10,638 Total:

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

BEDALE DR CARLING AVE

07:00 07:15 0 0 0 0 0 0 0 0 0			DEDALL DI			<u> </u>		
07:15 07:30 0	Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:30 07:45 0 0 0 1 1 1 1 1 0	07:00 07:15	0	0	0	0	0	0	0
07:45 08:00 0	07:15 07:30	0	0	0	0	0	0	0
08:00 08:15 0 0 0 0 0 0 0 0 0	07:30 07:45	0	0	0	0	1	1	1
08:15 08:30 0 0 0 0 0 0 0 0 0	07:45 08:00	0	0	0	0	0	0	0
08:30 08:45 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 1 1 1 1 0	08:00 08:15	0	0	0	0	1	1	1
08:45 09:00 0 0 0 0 1	08:15 08:30	0	0	0	0	0	0	0
09:00 09:15 0 0 0 1	08:30 08:45	0	0	0	0	0	0	0
09:15 09:30 0	08:45 09:00	0	0	0	0	1	1	1
09:30 09:45 0	09:00 09:15	0	0	0	0	1	1	1
09:45 10:00 0 0 0 0 0 0 11:30 11 1	09:15 09:30	0	0	0	0	2	2	2
11:30 11:45 0 0 0 1 0 1	09:30 09:45	0	0	0	0	0	0	0
11:45 12:00 0	09:45 10:00	0	0	0	0	0	0	0
12:00 12:15 0	11:30 11:45	0	0	0	1	0	1	1
12:15 12:30 0	11:45 12:00	0	0	0	0	0	0	0
12:30 12:45 0 0 0 0 0 0 1	12:00 12:15	0	0	0	0	0	0	0
12:45 13:00 0 0 0 1	12:15 12:30	0	0	0	0	0	0	0
13:00 13:15 0	12:30 12:45	0	0	0	0	0	0	0
13:15 13:30 0 0 0 0 0 0 0 15:00 <	12:45 13:00	0	0	0	0	1	1	1
15:00 15:15 0	13:00 13:15	0	0	0	0	0	0	0
15:15 15:30 0	13:15 13:30	0	0	0	0	0	0	0
15:30 15:45 0	15:00 15:15	0	0	0	0	0	0	0
15:45 16:00 0	15:15 15:30	0	0	0	0	0	0	0
16:00 16:15 0	15:30 15:45	0	0	0	0	0	0	0
16:15 16:30 0	15:45 16:00	0	0	0	0	0	0	0
16:30 16:45 0 0 0 0 0 0 16:45 17:00 0 0 0 0 0 0 17:00 17:15 0 0 0 1 1 2 2 17:15 17:30 0 0 0 1 0 1 1 17:30 17:45 0 0 0 0 0 0 17:45 18:00 0 0 1 0 1 1	16:00 16:15	0	0	0	0	0	0	0
16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 1 1 2 2 17:15 17:30 0 0 0 1 0 1 1 1 17:30 17:45 0 0 0 0 0 0 0 17:45 18:00 0 0 1 0 1 1 1	16:15 16:30	0	0	0	0	0	0	0
17:00 17:15 0 0 0 1 1 2 2 17:15 17:30 0 0 0 1 0 1 1 17:30 17:45 0 0 0 0 0 0 17:45 18:00 0 0 1 0 1 1	16:30 16:45	0	0	0	0	0	0	0
17:15 17:30 0 0 0 1 0 1 1 17:30 17:45 0 0 0 0 0 0 17:45 18:00 0 0 0 1 0 1 1	16:45 17:00	0	0	0	0	0	0	0
17:30 17:45 0 0 0 0 0 0 0 0 0 0 0 0 0 1 1 0 1 1 1 1 1 1 0 1 1 0 1 1 0 1 1 0	17:00 17:15	0	0	0	1	1	2	2
17:45 18:00 0 0 0 1 0 1 1	17:15 17:30	0	0	0	1	0	1	1
	17:30 17:45	0	0	0	0	0	0	0
Total 0 0 0 4 8 12 12	17:45 18:00	0	0	0	1	0	1	1
	Total	0	0	0	4	8	12	12

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Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

BEDALE DR CARLING AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	0	1	1	1
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	1	0	1	1	0	1	2
08:00 08:15	0	1	1	0	0	0	1
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	1	0	1	1	0	1	2
09:15 09:30	2	0	2	0	0	0	2
09:30 09:45	2	0	2	0	0	0	2
09:45 10:00	0	0	0	1	0	1	1
11:30 11:45	1	0	1	0	0	0	1
11:45 12:00	0	0	0	1	2	3	3
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	1	1	1
12:30 12:45	1	1	2	0	0	0	2
12:45 13:00	0	2	2	0	0	0	2
13:00 13:15	1	0	1	0	2	2	3
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	4	0	4	0	1	1	5
15:15 15:30	1	1	2	0	1	1	3
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	1	0	1	0	0	0	1
16:00 16:15	2	1	3	0	3	3	6
16:15 16:30	1	0	1	1	3	4	5
16:30 16:45	1	0	1	0	1	1	2
16:45 17:00	0	0	0	1	3	4	4
17:00 17:15	1	0	1	2	3	5	6
17:15 17:30	2	0	2	0	1	1	3
17:30 17:45	1	0	1	0	2	2	3
17:45 18:00	1	0	1	1	0	1	2
Total	24	6	30	9	24	33	63

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Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

BEDALE DR CARLING AVE

	N	orthbou	und		Sc	uthbou	ınd			E	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
07:15 07:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
07:30 07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
07:45 08:00	0	0	0	0	0	0	0	0	0	0	4	0	4	0	1	0	1	5	5
08:00 08:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
08:15 08:30	0	0	0	0	0	0	0	0	0	0	1	0	1	0	4	0	4	5	5
08:30 08:45	0	0	0	0	0	0	0	0	0	0	5	0	5	0	1	0	1	6	6
08:45 09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
09:15 09:30	0	0	0	0	0	0	0	0	0	0	3	0	3	0	1	0	1	4	4
09:30 09:45	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
09:45 10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1
11:45 12:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
12:00 12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	2
12:15 12:30	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
12:30 12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
12:45 13:00	0	0	0	0	0	0	0	0	0	0	3	0	3	0	1	1	2	5	5
13:00 13:15	0	0	0	0	1	0	0	1	1	0	2	1	3	0	0	0	0	3	4
13:15 13:30	0	0	1	1	0	0	1	1	2	0	2	0	2	0	2	1	3	5	7
15:00 15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	2
15:15 15:30	1	0	0	1	0	0	0	0	1	0	3	0	3	0	0	0	0	3	4
15:30 15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
15:45 16:00	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
16:00 16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	0	2	4	4
16:30 16:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
16:45 17:00	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
17:00 17:15	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	1	2	2
17:15 17:30	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3	3
17:30 17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total: None	1	0	1	2	1	0	1	2	4	0	38	1	39	0	32	2	34	73	77

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Turning Movement Count - Study Results

CARLING AVE @ BEDALE DR

Survey Date: Wednesday, February 28, 2018 WO No: 37586

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total BEDALE DR CARLING AVE

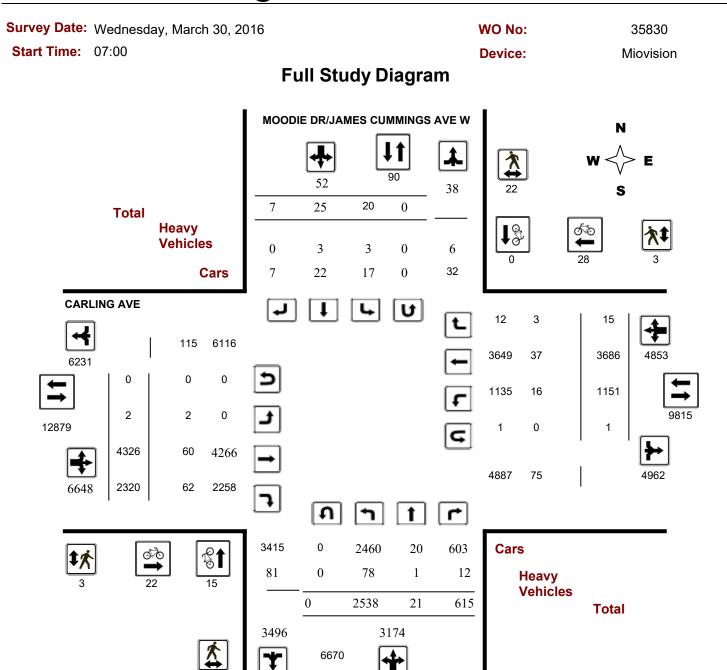
Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	1	0	1
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	1	0	0	1
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Tot	tal	0	1	1	0	2

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Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W



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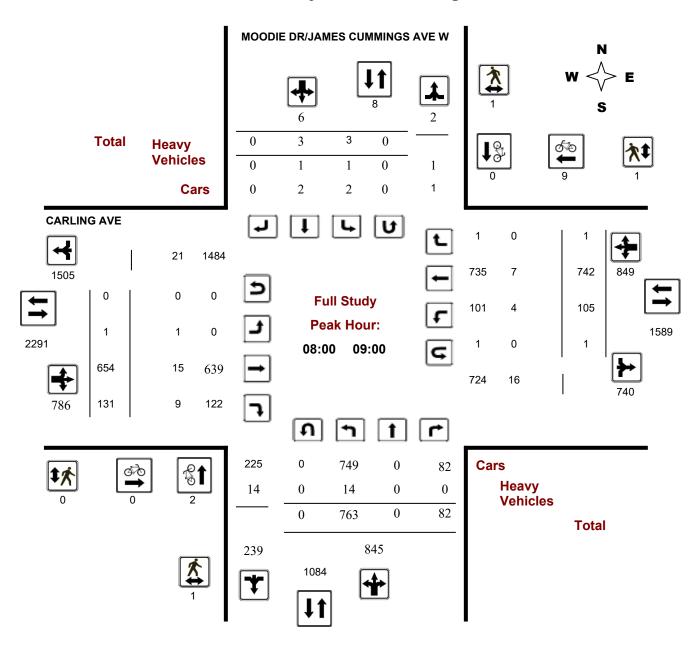


Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date:Wednesday, March 30, 2016WO No:35830Start Time:07:00Device:Miovision

Full Study Peak Hour Diagram



April 1, 2021 Page 2 of 8



Survey Date: Wednesday, March 30, 2016

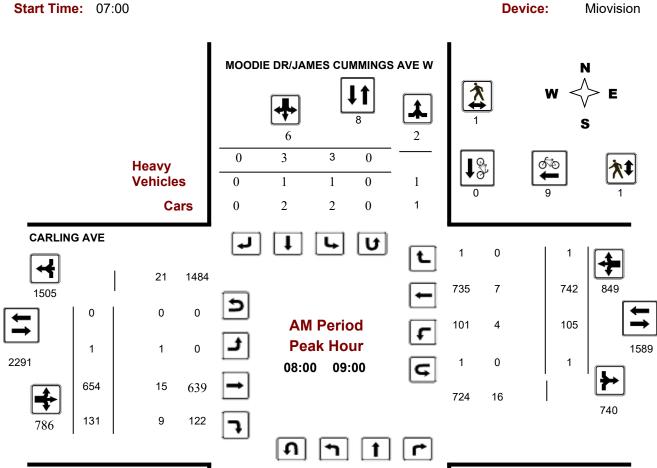
Transportation Services - Traffic Services

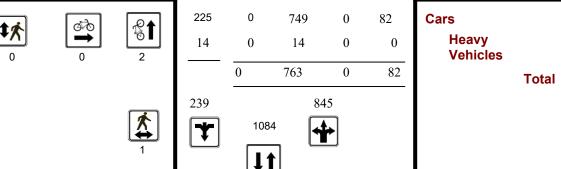
WO No:

35830

Turning Movement Count - Peak Hour Diagram

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W





Comments

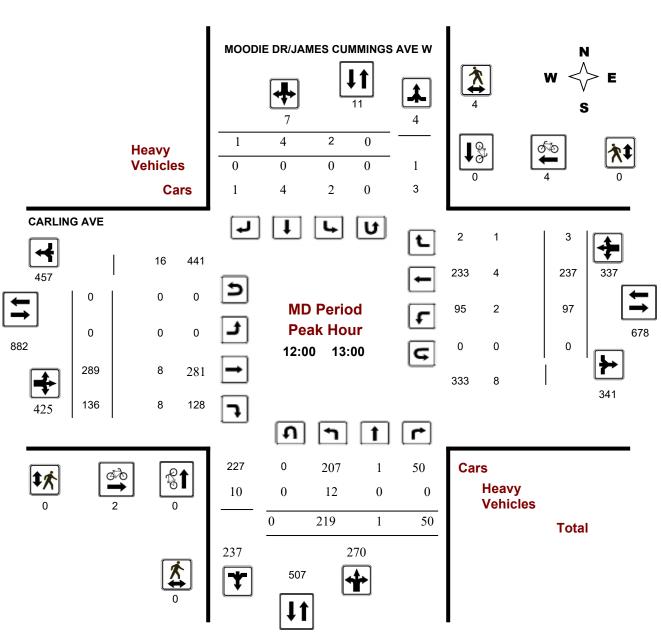
2021-Apr-01 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830
Start Time: 07:00 Device: Miovision



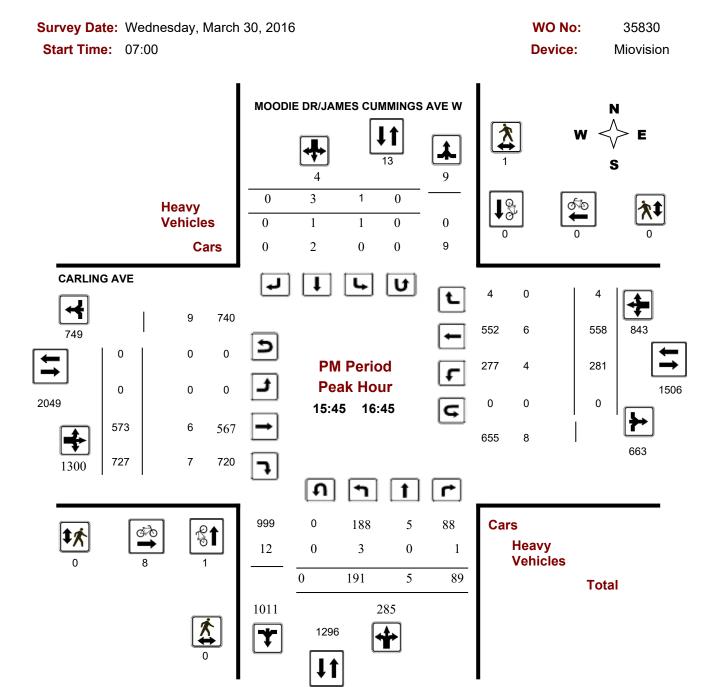
Comments

2021-Apr-01 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W



Comments

2021-Apr-01 Page 3 of 3



Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, March 30, 2016 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 1.00

Eastbound: 0 Westbound: 1

	MOOI	DIE DI	R/JAM	IES CU	MMIN	GS A	/E W					CA	RLING	AVE					
-	Nor	thbou	nd		Sou	uthbou	ınd			Е	astbo	und		٧	Vestbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	386	0	120	506	4	4	1	9	515	0	987	140	1127	69	455	0	524	1651	2166
08:00 09:00	763	0	82	845	3	3	0	6	851	1	654	131	786	105	742	1	848	1634	2485
09:00 10:00	500	4	56	560	2	4	4	10	570	1	287	150	438	94	499	1	594	1032	1602
11:30 12:30	185	4	53	242	0	4	1	5	247	0	269	143	412	101	200	4	305	717	964
12:30 13:30	217	1	51	269	2	3	0	5	274	0	225	157	382	93	239	1	333	715	989
15:00 16:00	148	3	91	242	4	3	0	7	249	0	480	608	1088	222	457	2	681	1769	2018
16:00 17:00	191	5	81	277	3	3	0	6	283	0	644	648	1292	258	579	5	842	2134	2417
17:00 18:00	148	4	81	233	2	1	1	4	237	0	780	343	1123	209	515	1	725	1848	2085
Sub Total	2538	21	615	3174	20	25	7	52	3226	2	4326	2320	6648	1151	3686	15	4852	11500	14726
U Turns	0			0	0			0	0	0			0	1			1	1	1
Total	2538	21	615	3174	20	25	7	52	3226	2	4326	2320	6648	1152	3686	15	4853	11501	14727
EQ 12Hr	3528	29	855	4412	28	35	10	73	4485	3	6013	3225	9241	1601	5124	21	6746	15987	20472
Note: These	values ar	e calcu	lated by	y multiply	ing the	totals b	y the ap	opropriat	e expans	ion fac	tor.			1.39					
AVG 12Hr	3528	29	855	4412	28	35	10	73	4485	3	6013	3225	9241	1601	5124	21	6746	15987	20472
Note: These	volumes	are cal	culated	by multip	olying th	e Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1.00					
AVG 24Hr	4622	38	1120	5780	37	46	13	96	5876	4	7877	4225	12106	2097	6712	28	8837	20943	26819
Note: These	volumes	are cal	culated	by multip	olying th	ie Avera	age Dail	y 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

MOODIE DR/JAMES CUMMINGS AVE W **CARLING AVE**

		No	orthbou	und		• Sc	uthbou	nd		Eastbound					We	estboun	nd			
Time Per	riod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07	7:15	57	0	33	90	2	1	1	4	94	0	218	33	251	10	103	0	113	364	458
07:15 07	7:30	67	0	32	99	1	1	0	2	101	0	262	46	308	20	109	0	129	437	538
07:30 07	7:45	115	0	33	148	0	1	0	1	149	0	272	27	299	24	106	0	130	429	578
07:45 08	8:00	147	0	22	169	1	1	0	2	171	0	235	34	269	15	137	0	152	421	592
08:00	8:15	131	0	31	162	1	0	0	1	163	0	233	19	252	24	153	0	177	429	592
08:15 08	8:30	199	0	17	216	1	1	0	2	218	0	160	42	202	24	183	0	207	409	627
08:30 08	8:45	237	0	16	253	0	1	0	1	254	0	162	36	198	18	203	1	222	420	674
08:45 09	9:00	196	0	18	214	1	1	0	2	216	1	99	34	134	40	203	0	243	377	593
09:00 09	9:15	197	1	15	213	0	1	3	4	217	0	76	28	104	26	188	0	214	318	535
09:15 09	9:30	135	1	14	150	0	2	1	3	153	0	98	37	135	20	133	0	153	288	441
09:30 09	9:45	92	1	9	102	1	1	0	2	104	0	59	49	108	22	103	1	126	234	338
09:45 10	0:00	76	1	18	95	1	0	0	1	96	1	54	36	91	26	75	0	101	192	288
11:30 11	1:45	47	3	12	62	0	2	0	2	64	0	58	25	83	31	44	0	75	158	222
11:45 12	2:00	35	0	11	46	0	0	0	0	46	0	60	41	101	26	48	2	76	177	223
12:00 12	2:15	43	0	18	61	0	0	0	0	61	0	86	44	130	20	45	1	66	196	257
12:15 12	2:30	60	1	12	73	0	2	1	3	76	0	65	33	98	24	63	1	88	186	262
12:30 12	2:45	56	0	10	66	0	2	0	2	68	0	79	38	117	23	65	1	89	206	274
12:45 13	3:00	60	0	10	70	2	0	0	2	72	0	59	21	80	30	64	0	94	174	246
13:00 13	3:15	55	0	14	69	0	0	0	0	69	0	45	45	90	24	59	0	83	173	242
13:15 13	3:30	46	1	17	64	0	1	0	1	65	0	42	53	95	16	51	0	67	162	227
15:00 15	5:15	37	0	26	63	2	0	0	2	65	0	132	118	250	36	74	2	112	362	427
15:15 15	5:30	33	0	23	56	2	2	0	4	60	0	117	151	268	49	115	0	164	432	492
15:30 15	5:45	28	2	18	48	0	1	0	1	49	0	113	144	257	61	140	0	201	458	507
15:45 16	6:00	50	1	24	75	0	0	0	0	75	0	118	195	313	76	128	0	204	517	592
16:00 16	6:15	51	2	25	78	1	2	0	3	81	0	164	179	343	64	130	0	194	537	618
16:15 16	6:30	47	0	18	65	0	1	0	1	66	0	134	187	321	81	159	1	241	562	628
16:30 16	6:45	43	2	22	67	0	0	0	0	67	0	157	166	323	60	141	3	204	527	594
16:45 17	7:00	50	1	16	67	2	0	0	2	69	0	189	116	305	53	149	1	203	508	577
17:00 17	7:15	42	1	21	64	0	0	0	0	64	0	228	110	338	70	153	0	223	561	625
17:15 17	7:30	40	1	21	62	0	1	1	2	64	0	210	105	315	49	130	0	179	494	558
17:30 17	7:45	38	1	19	58	1	0	0	1	59	0	191	68	259	54	135	1	190	449	508
17:45 18	8:00	28	1	20	49	1	0	0	1	50	0	151	60	211	36	97	0	133	344	394
Total:		2538	21	615	3174	20	25	7	52	3226	2	4326	2320	6648	1152	3686	15	4853	3226	14,727

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

MOODIE DR/JAMES CUMMINGS AVE W CARLING AVE

	MICODIE DK	JAIVIES COIVIIV	IINGS AVE W		CARLING AV	–	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	0	1	1	1
07:15 07:30	2	0	2	0	1	1	3
07:30 07:45	0	0	0	0	4	4	4
07:45 08:00	4	0	4	0	0	0	4
08:00 08:15	2	0	2	0	1	1	3
08:15 08:30	0	0	0	0	2	2	2
08:30 08:45	0	0	0	0	5	5	5
08:45 09:00	0	0	0	0	1	1	1
09:00 09:15	1	0	1	0	3	3	4
09:15 09:30	0	0	0	0	1	1	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	1	0	1	0	1	1	2
11:45 12:00	1	0	1	0	0	0	1
12:00 12:15	0	0	0	0	2	2	2
12:15 12:30	0	0	0	0	2	2	2
12:30 12:45	0	0	0	1	0	1	1
12:45 13:00	0	0	0	1	0	1	1
13:00 13:15	0	0	0	1	1	2	2
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	1	0	1	1	0	1	2
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	1	0	1	0	0	0	1
15:45 16:00	0	0	0	3	0	3	3
16:00 16:15	0	0	0	2	0	2	2
16:15 16:30	1	0	1	0	0	0	1
16:30 16:45	0	0	0	3	0	3	3
16:45 17:00	1	0	1	2	0	2	3
17:00 17:15	0	0	0	2	1	3	3
17:15 17:30	0	0	0	3	1	4	4
17:30 17:45	0	0	0	2	1	3	3
17:45 18:00	0	0	0	1	0	1	1
Total	15	0	15	22	28	50	65
		,		•	,	,	,

April 1, 2021 Page 5 of 8



Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

MOODIE DR/JAMES CUMMINGS AVE W **CARLING AVE**

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	3	0	3	3
07:15 07:30	0	1	1	0	0	0	1
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	1	1	2	0	1	1	3
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	1	1	0	0	0	1
11:45 12:00	0	1	1	0	0	0	1
12:00 12:15	0	4	4	0	0	0	4
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	1	1	0	0	0	1
16:45 17:00	0	1	1	0	0	0	1
17:00 17:15	0	2	2	0	0	0	2
17:15 17:30	1	5	6	0	2	2	8
17:30 17:45	0	3	3	0	0	0	3
17:45 18:00	0	2	2	0	0	0	2
Total	2	22	24	3	3	6	30

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Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

MOODIE DR/JAMES CUMMINGS AVE W

CARLING AVE

	No	orthbou	und		Sc	uthbou	nd			Е	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	6	0	2	8	0	0	0	0	8	0	3	2	5	0	0	0	0	5	13
07:15 07:30	5	0	2	7	0	0	0	0	7	0	1	3	4	0	1	0	1	5	12
07:30 07:45	1	0	3	4	0	0	0	0	4	0	2	2	4	0	2	0	2	6	10
07:45 08:00	7	0	0	7	0	0	0	0	7	0	2	2	4	0	1	0	1	5	12
08:00 08:15	3	0	0	3	0	0	0	0	3	0	3	1	4	1	2	0	3	7	10
08:15 08:30	4	0	0	4	0	0	0	0	4	0	3	2	5	2	1	0	3	8	12
08:30 08:45	2	0	0	2	0	0	0	0	2	0	5	3	8	0	1	0	1	9	11
08:45 09:00	5	0	0	5	1	1	0	2	7	1	4	3	8	1	3	0	4	12	19
09:00 09:15	2	0	1	3	0	0	0	0	3	0	4	1	5	0	0	0	0	5	8
09:15 09:30	4	0	0	4	0	0	0	0	4	0	2	1	3	0	1	0	1	4	8
09:30 09:45	1	0	0	1	0	0	0	0	1	0	0	2	2	0	1	0	1	3	4
09:45 10:00	5	0	0	5	1	0	0	1	6	1	2	2	5	0	0	0	0	5	11
11:30 11:45	4	0	0	4	0	0	0	0	4	0	2	2	4	2	2	0	4	8	12
11:45 12:00	2	0	0	2	0	0	0	0	2	0	4	0	4	2	1	0	3	7	9
12:00 12:15	2	0	0	2	0	0	0	0	2	0	1	4	5	0	3	1	4	9	11
12:15 12:30	3	0	0	3	0	0	0	0	3	0	3	1	4	0	0	0	0	4	7
12:30 12:45	4	0	0	4	0	0	0	0	4	0	3	3	6	1	0	0	1	7	11
12:45 13:00	3	0	0	3	0	0	0	0	3	0	1	0	1	1	1	0	2	3	6
13:00 13:15	2	0	1	3	0	0	0	0	3	0	1	0	1	0	1	0	1	2	5
13:15 13:30	0	0	1	1	0	0	0	0	1	0	2	2	4	0	2	0	2	6	7
15:00 15:15	3	0	1	4	0	0	0	0	4	0	2	5	7	1	1	2	4	11	15
15:15 15:30	2	0	0	2	0	0	0	0	2	0	1	5	6	0	2	0	2	8	10
15:30 15:45	1	1	0	2	0	1	0	1	3	0	1	3	4	1	0	0	1	5	8
15:45 16:00	1	0	1	2	0	0	0	0	2	0	0	2	2	0	2	0	2	4	6
16:00 16:15	2	0	0	2	1	0	0	1	3	0	2	3	5	1	2	0	3	8	11
16:15 16:30	0	0	0	0	0	1	0	1	1	0	2	0	2	3	1	0	4	6	7
16:30 16:45	0	0	0	0	0	0	0	0	0	0	2	2	4	0	1	0	1	5	5
16:45 17:00	1	0	0	1	0	0	0	0	1	0	0	3	3	0	2	0	2	5	6
17:00 17:15	0	0	0	0	0	0	0	0	0	0	1	1	2	0	0	0	0	2	2
17:15 17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
17:30 17:45	1	0	0	1	0	0	0	0	1	0	1	1	2	0	1	0	1	3	4
17:45 18:00	2	0	0	2	0	0	0	0	2	0	0	1	1	0	1	0	1	2	4
Total: None	78	1	12	91	3	3	0	6	97	2	60	62	124	16	37	3	56	180	277

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Turning Movement Count - Study Results

CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Survey Date: Wednesday, March 30, 2016 WO No: 35830

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total

	МС	OODIE DR/JAMES		CAI	RLING AVE	
Time I	Period	AVE W Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	1	1
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	0	0	0	1	1

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APPENDIX C

COLLISION DATA





Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ BEDALE DR

Traffic Control: Traffic signal Total Collisions: 6

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Mar-03, Tue,07:53	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	
2017-Feb-16, Thu,08:54	Clear	Rear end	P.D. only	Dry	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-02, Tue,12:45	Clear	Angle	Non-fatal injury	Packed snow	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Turning left	Snow plow	Other motor vehicle	
2019-Aug-14, Wed,16:14	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Bicycle	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Cyclist	
2019-Dec-02, Mon,00:10	Clear	SMV other	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Pole (utility, power)	0
2019-Dec-09, Mon,19:00	Rain	SMV other	Non-fatal injury	Wet	South	Turning left	Automobile, station wagon	Pedestrian	1

Location: CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Traffic Control: Traffic signal Total Collisions: 27

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-Jan-06, Tue,08:50	Clear	Rear end	P.D. only	Dry	North	Turning right	Pick-up truck	Other motor vehicle	0
					North	Turning right	Pick-up truck	Other motor vehicle	
2015-Feb-12, Thu,06:40	Snow	SMV other	P.D. only	Slush	East	Overtaking	Pick-up truck	Snowbank/drift	0
2015-Mar-09, Mon,18:00	Clear	Rear end	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2015-Sep-14, Mon,18:38	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Jan-20, Wed,13:21	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Traffic Control: Traffic signal Total Collisions: 27

Cond'n									
Date/Day/Time	Environment	Impact Type	Classification		Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Apr-19, Tue,17:35	Clear	Turning movement	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Nov-24, Thu,20:03	Clear	SMV other	P.D. only	Slush	East	Going ahead	Automobile, station wagon	Animal - wild	0
2017-Jan-13, Fri,17:03	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-17, Fri,16:43	Clear	Angle	P.D. only	Dry	West	Turning left	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-22, Wed,05:15	Clear	SMV other	P.D. only	Wet	West	Turning left	Pick-up truck	Animal - wild	0
2017-May-02, Tue,12:47	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Aug-24, Thu,20:30	Clear	SMV other	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Animal - wild	0
2017-Sep-09, Sat,14:15	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Nov-24, Fri,16:57	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Nov-27, Mon,09:02	Clear	Rear end	P.D. only	Dry	North	Going ahead	Construction equipment	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Dec-12, Tue,09:15	Snow	Rear end	P.D. only	Slush	West	Going ahead	Passenger van	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Feb-14, Wed,18:01	Clear	Turning movement	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-May-31, Thu,09:00	Clear	Rear end	P.D. only	Dry	West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE @ MOODIE DR/JAMES CUMMINGS AVE W

Traffic Control: Traffic signal Total Collisions: 27

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Sep-30, Sun,01:05	Clear	SMV other	P.D. only	Wet	North	Turning right	Automobile, station wagon	Pole (utility, power)	0
2018-Nov-23, Fri,12:08	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2019-Jan-18, Fri,07:23	Snow	Turning movement	Non-fatal injury	Loose snow	West	Turning left	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Feb-08, Fri,11:38	Clear	Turning movement	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-May-21, Tue,08:34	Clear	Sideswipe	Non-fatal injury	Dry	West	Pulling away from shoulder or curb	Bicycle	Other motor vehicle	0
					West	Overtaking	Automobile, station wagon	Cyclist	
2019-Jul-15, Mon,20:52	Clear	Turning movement	Fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Motorcycle	Other motor vehicle	
2019-Jul-19, Fri,18:19	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Aug-24, Sat,13:35	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Oct-18, Fri,12:00	Clear	SMV other	P.D. only	Dry	North	Turning right	Automobile, station wagon	Ran off road	0

Location: CARLING AVE @ ROCKY POINT RD

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Jan-05, Thu,07:56	Snow	Angle	P.D. only	Slush	South	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Snow plow	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE btwn HARBOUR LANDING PRIV & BEDALE DR

Traffic Control: No control Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2019-Aug-15, Thu,09:08	Clear	Angle	P.D. only	Dry	North	Turning left Automobile, station wagor		Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	

Location: CARLING AVE btwn MOODIE DR & JAMES CUMMINGS AVE

Traffic Control: No control Total Collisions: 6

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Jan-09, Fri,08:07	Clear	Rear end	P.D. only	Loose snow	East	Going ahead	Truck and trailer	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jul-30, Sat,05:29	Clear	SMV other	P.D. only	Dry	West	Going ahead	Pick-up truck	Ran off road	0
2016-Aug-15, Mon,16:55	Clear	Other	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Debris falling off vehicle	0
					East	Going ahead	Car and trailer	Other	
2016-Sep-26, Mon,08:44	Clear	Sideswipe	Non-fatal injury	Dry	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-28, Tue,16:05	Clear	SMV other	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Animal - wild	0
2017-Dec-05, Tue,17:24	Clear	SMV other	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Animal - wild	0

Location: CARLING AVE btwn ROCKY POINT RD & HARBOUR LANDING PRIV

Traffic Control: No control Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehic	cle type	First Event	No. Ped
2015-Mar-03, Tue,07:35	Clear	Rear end	P.D. only	Dry	East	Going ahead Autor	mobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping Autor	mobile, station wagon	Other motor vehicle	

Location: CARLING AVE btwn SUNNY BRAE AVE & CRYSTAL BEACH DR

Traffic Control: No control Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped
		. ,,		Cond'n		,,		

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: CARLING AVE btwn SUNNY BRAE AVE & CRYSTAL BEACH DR

Traffic Control: No control

Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2019-Jan-21, Mon,12:13	Drifting Snow	Turning movement	P.D. only	Packed snow	East	Turning right	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: CARLING AVE btwn ULLSWATER DR & SUNNY BRAE AVE

Traffic Control: No control

Total Collisions: 4

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Feb-16, Tue,07:38	Snow	Rear end	P.D. only	Slush	West	Slowing or stopping	g Pick-up truck	Other motor vehicle	0
					West	Turning left	Pick-up truck	Other motor vehicle	
2016-May-12, Thu,10:30	Clear	Angle	P.D. only	Dry	North	Turning right	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Sep-12, Mon,07:40	Clear	Rear end	Non-fatal injury	Dry	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping	g Truck - tractor	Other motor vehicle	
2017-Jun-14, Wed,13:35	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Turning left	Automobile, station wagon	Other motor vehicle	

Location: ULLSWATER DR @ CARLING AVE

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Oct-07, Fri,14:50	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Delivery van	Other motor vehicle	

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APPENDIX D

TDM Checklists



TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

Legend The Official Plan or Zoning By-law provides related guidance that must be followed The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	\square
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	\square
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	\mathbf{Z}
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	To be confirmed in SPA
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	To be confirmed in SPA
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	To be confirmed in SPA
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	To be confirmed in SPA
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	ı
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	To be confirmed in SPA
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	- <u>-</u>
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

Legend The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

		TDM	measures: Residential developments	Check if proposed & add descriptions
		3.	TRANSIT	
		3.1	Transit information	
BASIC		3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	
BETTER		3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
		3.2	Transit fare incentives	
BASIC	*	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER		3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
		3.3	Enhanced public transit service	
BETTER	*	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
		3.4	Private transit service	
BETTER		3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
		4.	CARSHARING & BIKESHARING	
		4.1	Bikeshare stations & memberships	
BETTER		4.1.1	Contract with provider to install on-site bikeshare station (<i>multi-family</i>)	
BETTER		4.1.2	Provide residents with bikeshare memberships, either free or subsidized <i>(multi-family)</i>	
		4.2	Carshare vehicles & memberships	
BETTER		4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER		4.2.2	Provide residents with carshare memberships, either free or subsidized	
		5.	PARKING	
		5.1	Priced parking	
BASIC	*	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC	*	5.1.2	Unbundle parking cost from monthly rent (multi-family)	

TDM	measures: Residential developments	Check if proposed & add descriptions
6.	TDM MARKETING & COMMUNICATION	S
6.1	Multimodal travel information	
BASIC ★ 6.1.1	Provide a multimodal travel option information package to new residents	
6.2	Personalized trip planning	
BETTER ★ 6.2.1	Offer personalized trip planning to new residents	

APPENDIX E

Synchro Analysis



	•	→	•	•	←	1	†	~	/		
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		414	7	75	Φß		*	7		43-	
Traffic Volume (vph)	1	654	131	106	742	763	0	82	3	3	
Future Volume (vph)	1	654	131	106	742	763	0	82	3	3	
Lane Group Flow (vph)	0	689	138	112	782	0	803	86	0	6	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		2		1	6		8			4	
Permitted Phases	2		2	6		8		8	4		
Detector Phase	2	2	2	1	6	8	8	8	4	4	
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	28.6	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0	
Total Split (s)	40.0	40.0	40.0	15.0	55.0	75.0	75.0	75.0	75.0	75.0	
Total Split (%)	30.8%	30.8%	30.8%	11.5%	42.3%	57.7%	57.7%	57.7%	57.7%	57.7%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)		-1.6	-1.6	-1.6	-1.6		-2.0	-2.0		-2.0	
Total Lost Time (s)		4.0	4.0	4.0	4.0		4.0	4.0		4.0	
Lead/Lag	Lag	Lag	Lag	Lead							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes							
Recall Mode	C-Min	C-Min	C-Min	None	C-Min	None	None	None	None	None	
Act Effct Green (s)		34.1	34.1	48.7	48.7		73.3	73.3		73.3	
Actuated g/C Ratio		0.26	0.26	0.37	0.37		0.56	0.56		0.56	
v/c Ratio		0.81	0.28	0.53	0.62		1.06	0.10		0.01	
Control Delay		53.4	7.1	35.8	35.2		78.8	4.5		13.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay		53.4	7.1	35.8	35.2		78.8	4.5		13.5	
LOS		D	Α	D	D		Е	Α		В	
Approach Delay		45.6			35.3		71.6			13.5	
Approach LOS		D			D		Е			В	
Queue Length 50th (m)		85.9	0.0	18.6	82.7		~231.1	1.6		0.7	
Queue Length 95th (m)		108.1	15.3	31.8	102.7		#306.4	9.1		2.7	
Internal Link Dist (m)		229.2			650.7		185.4			48.7	
Turn Bay Length (m)			125.0	75.0							
Base Capacity (vph)		895	519	216	1329		757	886		868	
Starvation Cap Reductn		0	0	0	0		0	0		0	
Spillback Cap Reductn		0	0	0	0		0	0		0	
Storage Cap Reductn		0	0	0	0		0	0		0	
Reduced v/c Ratio		0.77	0.27	0.52	0.59		1.06	0.10		0.01	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.06 Intersection Signal Delay: 50.9

Intersection Capacity Utilization 102.1%

Intersection LOS: D
ICU Level of Service G

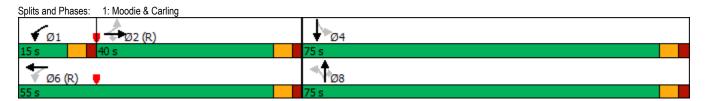
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	44	7	ሻ	4 %	HDL	43	ODL	43-
Traffic Volume (vph)	6	77	2	10	824	16	0	4	0
Future Volume (vph)	6	784	2	10	824	16	0	4	0
Lane Group Flow (vph)	6	825	2	11	874	0	62	0	9
Furn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases	i eiiii	2	I CIIII	I CIIII	6	I CIIII	8	I GIIII	4
Permitted Phases	2	2	2	6	U	8	U	4	7
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase	_			U	U	U	U	-	-
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	-1.6	-1.6	-1.6	-1.6	-1.6	2.0	-2.1	2.0	-2.1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0		4.0		4.0
Lead/Lag	7.0	7.0	7.0	7.0	7.0		7.0		7.0
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	60.3	60.3	60.3	60.3	60.3		14.7		14.7
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.80		0.20		0.20
v/c Ratio	0.01	0.30	0.00	0.02	0.32		0.19		0.03
Control Delay	5.7	4.6	0.0	5.6	4.7		11.5		0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.7	4.6	0.0	5.6	4.7		11.5		0.2
LOS	Α	Α	Α	Α	Α		В		Α
Approach Delay		4.6			4.7		11.5		0.2
Approach LOS		Α			Α		В		Α
Queue Length 50th (m)	0.2	17.1	0.0	0.3	18.3		2.1		0.0
Queue Length 95th (m)	1.9	45.0	0.0	2.7	48.3		9.1		0.1
Internal Link Dist (m)		432.6			93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	446	2727	1229	472	2725		579		565
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.01	0.30	0.00	0.02	0.32		0.11		0.02
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phas	se 2:FBTL and	6·WRTL S	tart of Gree	n					
Natural Cycle: 55	oc Z.EDTE ana	O.VVD IL, O	tuit of Oloo						
Control Type: Actuated-Coordinate	ed								
laximum v/c Ratio: 0.32									
ntersection Signal Delay: 4.9	001				ersection Lo				
Intersection Capacity Utilization 39	.3%			IC	U Level of S	Service A			
Analysis Period (min) 15									
Splits and Phases: 2: Bedale & C	Carling								
A						- I - I\[-\)			
♥ Ø2 (R)						▼ "	Ø4		
43 s						32 s			
						-4.₱			
Ø6 (R)						-	Ø8		
						22 -			

Existing PM 1: Moodie & Carling

	-	•	•	•	1	†		-	↓
Lane Group	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	413	#	*	♠ Ъ		•	7		4 \(\)
Traffic Volume (vph)	573	727	281	558	191	5	89	1	4 3
Future Volume (vph)	573	727	281	558	191	5	89	1	3
Lane Group Flow (vph)	603	765	296	591	0	206	94	0	4
Turn Type	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA
Protected Phases	2		<u> </u>	6		8			4
Permitted Phases		2	6		8		8	4	
Detector Phase	2	2	1	6	8	8	8	4	4
Switch Phase	_	_	•						
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0
Total Split (s)	40.0	40.0	25.0	65.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	40.0%	40.0%	25.0%	65.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	-1.6	-1.6	-1.6	-1.6	2.0	-2.0	-2.0	2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0		4.0	4.0		4.0
Lead/Lag	Lag	Lag	Lead	1.0		1.0	1.5		1.0
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	C-Min	C-Min	None	C-Min	None	None	None	None	None
Act Effct Green (s)	50.8	50.8	69.0	69.0	140110	23.0	23.0	140110	23.0
Actuated g/C Ratio	0.51	0.51	0.69	0.69		0.23	0.23		0.23
v/c Ratio	0.35	0.67	0.52	0.03		0.23	0.23		0.23
Control Delay	17.4	5.0	10.2	6.9		46.5	7.1		25.8
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	17.4	5.0	10.2	6.9		46.5	7.1		25.8
LOS	17. 4 B	3.0 A	10.2 B	0.9 A		40.3 D	7.1 A		23.0 C
Approach Delay	10.5	Λ	U	8.0		34.1	٨		25.8
Approach LOS	10.5 B			6.0 A		34.1 C			25.6 C
Queue Length 50th (m)	34.7	0.0	19.0	19.9		36.6	0.0		0.6
Queue Length 95th (m)	62.3	28.2	38.9	35.1		54.8	10.8		3.0
Internal Link Dist (m)	229.2	20.2	30.3	650.7		185.4	10.0		41.8
Turn Bay Length (m)	223.2	125.0	75.0	030.7		103.4			41.0
Base Capacity (vph)	1722	1146	648	2337		403	535		524
Starvation Cap Reductn	0	0	040	2337		403	0		0
Spillback Cap Reductn	0	0	0	0		0	0		0
Storage Cap Reductn	0	0	0	0		0	0		0
Reduced v/c Ratio	0.35	0.67	0.46	0.25		0.51	0.18		0.01
Neudoed V/C Rallo	0.33	0.07	0.40	0.23		0.51	0.10		0.01

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75 Control Type: Actuated-Coordinated

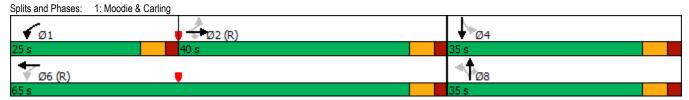
Maximum v/c Ratio: 0.69

Intersection Signal Delay: 12.4

Intersection Capacity Utilization 82.3%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15



Synchro 9 - Report Parsons

	asung P		
2:	Bedale	&	Carling

	•	→	•	•	←	4	†	-	ļ
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	Ť	^	7	*	∳ ኄ		414		43-
Traffic Volume (vph)	7	897	12	24	862	7	4	8	1
Future Volume (vph)	7	897	12	24	862	7	0	8	1
Lane Group Flow (vph)	7	944	13	25	915	0	28	0	18
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2			6		8		4
Permitted Phases	2		2	6		8		4	
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	-1.6	-1.6	-1.6	-1.6	-1.6	0	-2.1		-2.1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0		4.0		4.0
Lead/Lag	1.0	1.0	1.0	1.0	1.0		1.0		1.0
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	64.4	64.4	64.4	64.4	64.4	140110	14.7	140110	14.7
Actuated g/C Ratio	0.86	0.86	0.86	0.86	0.86		0.20		0.20
v/c Ratio	0.02	0.32	0.01	0.06	0.31		0.09		0.06
Control Delay	5.3	4.1	0.01	5.1	4.0		6.0		15.8
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.3	4.1	0.0	5.1	4.0		6.0		15.8
LOS	3.3 A	4.1 A	0.0 A	3.1 A	4.0 A		0.0 A		15.6 B
	A	4.1	A	A	4.1		6.0		15.8
Approach Delay Approach LOS		4.1 A			4.1 A		6.0 A		15.8 B
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.0		0.0		1.1
	2.0	53.5	0.0	5.0	51.2		3.9		4.8
Queue Length 95th (m) Internal Link Dist (m)	2.0	432.6	0.3	5.0	93.8		51.2		59.5
	30.0	432.0	15.0	30.0	33.0		31.2		59.5
Turn Bay Length (m)	30.0 461	2909	1308	30.0 445	2907		574		540
Base Capacity (vph) Starvation Cap Reductn	461	2909	1308	445	2907		0		0
	0	0	0				0		0
Spillback Cap Reductn				0	0				_
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.02	0.32	0.01	0.06	0.31		0.05		0.03
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase	se 2:EBTL and	6:WBTL, S	tart of Gree	n					
Natural Cycle: 55									

Natural Cycle: 55 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.32
Intersection Signal Delay: 4.2
Intersection Capacity Utilization 41.2%
Analysis Period (min) 15

Intersection LOS: A ICU Level of Service A

Splits and Phases: 2: Bedale & Carling





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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		414	*	¥	♦ 13-		*	*		43-	
Traffic Volume (vph)	1	661	131	106	749	763	0	82	3	3	
Future Volume (vph)	1	661	131	106	749	763	0	82	3	3	
Lane Group Flow (vph)	0	662	131	106	750	0	763	82	0	6	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		2		1	6		8			4	
Permitted Phases	2		2	6		8		8	4		
Detector Phase	2	2	2	1	6	8	8	8	4	4	
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	28.6	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0	
Total Split (s)	40.0	40.0	40.0	15.0	55.0	75.0	75.0	75.0	75.0	75.0	
Total Split (%)	30.8%	30.8%	30.8%	11.5%	42.3%	57.7%	57.7%	57.7%	57.7%	57.7%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)		5.6	5.6	5.6	5.6		6.0	6.0		6.0	
Lead/Lag	Lag	Lag	Lag	Lead							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes							
Recall Mode	C-Min	C-Min	C-Min	None	C-Min	None	None	None	None	None	
Act Effct Green (s)		31.8	31.8	46.4	46.4		72.0	72.0		72.0	
Actuated g/C Ratio		0.24	0.24	0.36	0.36		0.55	0.55		0.55	
v/c Ratio		0.84	0.28	0.55	0.62		1.03	0.09		0.01	
Control Delay		56.9	7.6	38.9	36.7		69.3	4.6		14.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay		56.9	7.6	38.9	36.7		69.3	4.6		14.5	
LOS		Е	Α	D	D		Е	Α		В	
Approach Delay		48.7			37.0		63.0			14.5	
Approach LOS		D			D		Е			В	
Queue Length 50th (m)		84.1	0.0	18.1	81.2		~212.5	1.4		0.7	
Queue Length 95th (m)		105.3	15.1	30.9	99.7		#290.1	9.0		2.8	
Internal Link Dist (m)		229.2			650.7		182.2			48.7	
Turn Bay Length (m)			125.0	75.0							
Base Capacity (vph)		855	497	197	1288		744	871		854	
Starvation Cap Reductn		0	0	0	0		0	0		0	
Spillback Cap Reductn		0	0	0	0		0	0		0	
Storage Cap Reductn		0	0	0	0		0	0		0	
Reduced v/c Ratio		0.77	0.26	0.54	0.58		1.03	0.09		0.01	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03

Intersection Signal Delay: 49.5

Intersection Capacity Utilization 106.8%

Intersection LOS: D
ICU Level of Service G

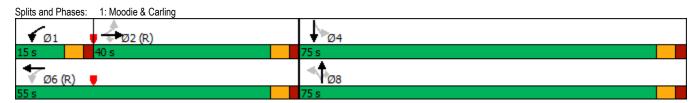
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	^	7	*	∳ ሴ		43-		43-
Traffic Volume (vph)	6	792	2	10	832	16	0	4	0
Future Volume (vph)	6	792	2	10	832	16	0	4	0
ane Group Flow (vph)	6	792	2	10	839	0	59	0	9
Furn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases	1 01111	2	1 01111	1 01111	6	1 01111	8	1 01111	4
Permitted Phases	2	_	2	6	v	8	•	4	•
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase	=	=	_	· ·			· ·	•	•
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	59.4	59.4	59.4	59.4	59.4		12.6		12.6
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79		0.17		0.17
v/c Ratio	0.01	0.30	0.00	0.02	0.31		0.21		0.03
Control Delay	6.2	5.1	0.0	6.2	5.2		12.7		0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	6.2	5.1	0.0	6.2	5.2		12.7		0.2
LOS	Α	Α	Α	Α	Α		В		Α
Approach Delay		5.1			5.2		12.7		0.2
Approach LOS		Α			Α		В		Α
Queue Length 50th (m)	0.2	18.1	0.0	0.3	19.4		2.0		0.0
Queue Length 95th (m)	2.0	45.3	0.0	2.7	48.6		9.2		0.1
Internal Link Dist (m)		432.6			93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	466	2684	1210	493	2682		531		509
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.01	0.30	0.00	0.02	0.31		0.11		0.02
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase	e 2:EBTL and	6:WBTL, S	tart of Gree	n					
Natural Cycle: 55									
Control Type: Actuated-Coordinated	d								
Maximum v/c Ratio: 0.31									
ntersection Signal Delay: 5.4				Int	ersection Lo	OS: A			
ntersection Capacity Utilization 42.	.6%			IC	U Level of S	Service A			
Analysis Period (min) 15									
Splits and Phases: 2: Bedale & C	Carling								
→ Ø2 (R)						- 1 +∞	Ø4		
43 e						32 s			
4						J2 J			
▼ Ø6 (R)						4/1	Ø8		
+ 20 (K)						'	20		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	414	#	*	♠ ₽		*	7		43-
Traffic Volume (vph)	579	727	281	564	191	5	89	1	3
Future Volume (vph)	579	727	281	564	191	5	89	1	3
Lane Group Flow (vph)	579	727	281	568	0	196	89	0	4
Turn Type	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA
Protected Phases	2		1	6	. •	8		. •	4
Permitted Phases	-	2	6		8		8	4	•
Detector Phase	2	2	1	6	8	8	8	4	4
Switch Phase		-	•	•	-	-		•	•
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0
Total Split (s)	40.0	40.0	25.0	65.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	40.0%	40.0%	25.0%	65.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6		6.0	6.0		6.0
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	C-Min	C-Min	None	C-Min	None	None	None	None	None
Act Effct Green (s)	50.2	50.2	68.2	68.2		20.2	20.2		20.2
Actuated g/C Ratio	0.50	0.50	0.68	0.68		0.20	0.20		0.20
v/c Ratio	0.34	0.65	0.50	0.25		0.75	0.24		0.01
Control Delay	17.6	4.8	10.4	7.1		53.8	7.9		27.8
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	17.6	4.8	10.4	7.1		53.8	7.9		27.8
LOS	В	Α	В	Α		D	Α		С
Approach Delay	10.5			8.2		39.5			27.8
Approach LOS	В			Α		D			С
Queue Length 50th (m)	33.6	0.0	18.4	19.6		35.9	0.0		0.6
Queue Length 95th (m)	59.8	27.0	37.6	34.4		54.4	11.0		3.1
Internal Link Dist (m)	229.2			650.7		185.4			41.8
Turn Bay Length (m)		125.0	75.0						
Base Capacity (vph)	1702	1123	637	2308		377	503		488
Starvation Cap Reductn	0	0	0	0		0	0		0
Spillback Cap Reductn	0	0	0	0		0	0		0
Storage Cap Reductn	0	0	0	0		0	0		0
Reduced v/c Ratio	0.34	0.65	0.44	0.25		0.52	0.18		0.01
Intersection Summary									
Cycle Length: 100									
Actuated Cycle Length: 100									
Offset: 0 (0%), Referenced to pha	se 2:EBTL and	6:WBTL, S	tart of Gree	n					
Natural Cycle: 75									
Control Type: Actuated-Coordinate	ed								
Maximum v/c Ratio: 0.75									
Intersection Signal Delay: 13.1					ersection Lo				
Intersection Capacity Utilization 86	6.8%			IC	U Level of S	Service E			
Analysis Period (min) 15									
Outlie and Diagram A. Mandia O	Onding								
Splits and Phases: 1: Moodie &	Carling								
	1 4								



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	
ane Configurations	*	^	#	*	Φß		43-		43-	
affic Volume (vph)	7	906	12	24	871	7	0	8	1	
ure Volume (vph)	7	906	12	24	871	7	0	8	1	
ne Group Flow (vph)	7	906	12	24	879	0	27	0	18	
rn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	
otected Phases	1 01111	2	1 01111	1 01111	6	1 01111	8	1 01111	4	
ermitted Phases	2	_	2	6	•	8		4		
etector Phase	2	2	2	6	6	8	8	4	4	
witch Phase	_	-	=	•	· ·	<u>.</u>		•	•	
inimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
inimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1	
otal Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0	
otal Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%	
ellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	
II-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8	
ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0	
otal Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1	
ead/Lag										
ead-Lag Optimize?										
ecall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None	
ct Effct Green (s)	63.7	63.7	63.7	63.7	63.7		12.6		12.6	
ctuated g/C Ratio	0.85	0.85	0.85	0.85	0.85		0.17		0.17	
c Ratio	0.01	0.31	0.01	0.05	0.31		0.10		0.07	
ontrol Delay	5.7	4.5	0.0	5.7	4.4		6.4		17.1	
ueue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0	
otal Delay	5.7	4.5	0.0	5.7	4.4		6.4		17.1	
OS	Α	Α	Α	Α	Α		Α		В	
pproach Delay		4.4			4.4		6.4		17.1	
pproach LOS		Α			Α		Α		В	
ueue Length 50th (m)	0.0	0.0	0.0	0.0	0.0		0.0		1.1	
ueue Length 95th (m)	2.1	53.7	0.1	5.0	51.5		3.9		5.0	
ternal Link Dist (m)		432.6			93.8		51.2		59.5	
urn Bay Length (m)	30.0		15.0	30.0						
ase Capacity (vph)	483	2880	1295	468	2878		527		490	
tarvation Cap Reductn	0	0	0	0	0		0		0	
pillback Cap Reductn	0	0	0	0	0		0		0	
Storage Cap Reductn	0	0	0	0	0		0		0	
educed v/c Ratio	0.01	0.31	0.01	0.05	0.31		0.05		0.04	
itersection Summary										
ycle Length: 75										
ctuated Cycle Length: 75										
ffset: 7 (9%), Referenced to phase 2:	=RTI and	16 WRTL S	tart of Gree	n						
atural Cycle: 55		7 0.11D 1 L, O	tart or Oroo							
ontrol Type: Actuated-Coordinated										
aximum v/c Ratio: 0.31										
tersection Signal Delay: 4.6				Inf	ersection Lo	OS: A				
ersection Capacity Utilization 44.5%					U Level of S					
nalysis Period (min) 15				10	C LOVOI 01 C	701 VIOU /1				
, ,										
olits and Phases: 2: Bedale & Carlin	ng					T k				
♥ Ø2 (R)						- ↓	Ø4			
43 c						32 s				
.d						J2 5				
▼ Ø6 (R)						4/	Ø8			



	•	→	*	•	←	1	†	~	/		
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		414	7	*	♠ ₽		*	7		₽.	
Traffic Volume (vph)	1	694	131	106	788	763	0	82	3	3	
Future Volume (vph)	1	694	131	106	788	763	0	82	3	3	
Lane Group Flow (vph)	0	695	131	106	789	0	763	82	0	6	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		2		<u>'</u> 1	6		8			4	
Permitted Phases	2		2	6		8		8	4		
Detector Phase	2	2	2	1	6	8	8	8	4	4	
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	28.6	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0	
Total Split (s)	40.0	40.0	40.0	15.0	55.0	75.0	75.0	75.0	75.0	75.0	
Total Split (%)	30.8%	30.8%	30.8%	11.5%	42.3%	57.7%	57.7%	57.7%	57.7%	57.7%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)		5.6	5.6	5.6	5.6		6.0	6.0		6.0	
Lead/Lag	Lag	Lag	Lag	Lead						***	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes							
Recall Mode	C-Min	C-Min	C-Min	None	C-Min	None	None	None	None	None	
Act Effct Green (s)		32.7	32.7	47.3	47.3		71.1	71.1		71.1	
Actuated g/C Ratio		0.25	0.25	0.36	0.36		0.55	0.55		0.55	
v/c Ratio		0.85	0.27	0.56	0.64		1.04	0.10		0.01	
Control Delay		57.5	7.5	39.2	36.8		73.7	4.6		14.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay		57.5	7.5	39.2	36.8		73.7	4.6		14.5	
LOS		E	Α.	D	D		E	Α		В	
Approach Delay		49.6	, ,		37.0		67.0	, ,		14.5	
Approach LOS		D			D		E			В	
Queue Length 50th (m)		88.1	0.0	17.8	85.2		~216.1	1.4		0.7	
Queue Length 95th (m)		111.3	15.1	30.9	106.1		#290.1	9.0		2.8	
Internal Link Dist (m)		229.2	10.1	00.0	650.7		182.2	0.0		48.7	
Turn Bay Length (m)		LLV.L	125.0	75.0	000.1		102.2			10.1	
Base Capacity (vph)		855	497	193	1288		735	861		842	
Starvation Cap Reductn		0	0	0	0		0	0		0	
Spillback Cap Reductn		0	0	0	0		0	0		0	
Storage Cap Reductn		0	0	0	0		0	0		0	
Reduced v/c Ratio		0.81	0.26	0.55	0.61		1.04	0.10		0.01	
reduced v/c realio		0.01	0.20	0.55	0.01		1.04	0.10		0.01	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.04

Intersection Signal Delay: 50.9

Intersection Capacity Utilization 108.9%

Intersection LOS: D
ICU Level of Service G

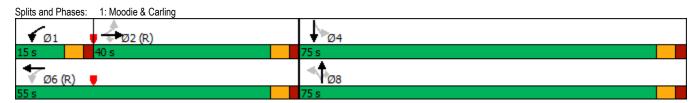
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	*	#	*	A 13		43-		43-
Traffic Volume (vph)	6	832	2	10	875	16	0	4	0
Future Volume (vph)	6	832	2	10	875	16	0	4	0
Lane Group Flow (vph)	6	832	2	10	882	0	59	0	9
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases	1 01111	2	1 01111	1 01111	6	1 01111	8	7 01111	4
Permitted Phases	2	_	2	6	•	8	Ū	4	•
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase	=	_	_	· ·	· ·	•		•	•
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	59.4	59.4	59.4	59.4	59.4		12.6		12.6
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79		0.17		0.17
v/c Ratio	0.01	0.31	0.00	0.02	0.33		0.21		0.03
Control Delay	6.2	5.2	0.0	6.2	5.3		12.7		0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	6.2	5.2	0.0	6.2	5.3		12.7		0.2
LOS	Α	Α	Α	Α	Α		В		Α
Approach Delay		5.2			5.3		12.7		0.2
Approach LOS		Α			Α		В		Α
Queue Length 50th (m)	0.2	19.2	0.0	0.3	20.7		2.0		0.0
Queue Length 95th (m)	2.0	48.2	0.0	2.7	51.8		9.2		0.1
Internal Link Dist (m)		432.6			93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	443	2684	1210	470	2682		531		509
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.01	0.31	0.00	0.02	0.33		0.11		0.02
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase 2	:EBTL and	6:WBTL. S	tart of Gree	n					
Natural Cycle: 55		, 0	2. 2.30						
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.33									
Intersection Signal Delay: 5.5				Int	ersection L	OS: A			
Intersection Capacity Utilization 43.8%	0				U Level of S				
Analysis Period (min) 15									
Splits and Phases: 2: Bedale & Carl	ling								
A	iing								
J 🖘 Ø2 (R)						₩	Ø4		
43 s						32 s			
4									
Ø6 (R)							Ø8		

	→	•	•	←	4	†	/	-	ļ
Lane Group	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	413	7	*	♦ %		*	7		
Traffic Volume (vph)	608	727	281	592	191	5	89	1	4 3
Future Volume (vph)	608	727	281	592	191	5	89	1	3
Lane Group Flow (vph)	608	727	281	596	0	196	89	0	4
Turn Type	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA.
Protected Phases	2		1	6	. •	8	. •	. •	4
Permitted Phases	_	2	6	· ·	8	•	8	4	•
Detector Phase	2	2	1	6	8	8	8	4	4
Switch Phase	_	_	•	•				•	•
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0
Total Split (s)	40.0	40.0	25.0	65.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	40.0%	40.0%	25.0%	65.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6		6.0	6.0		6.0
Lead/Lag	Lag	Lag	Lead	0.0		0.0	0.0		0.0
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	C-Min	C-Min	None	C-Min	None	None	None	None	None
Act Effct Green (s)	50.2	50.2	68.2	68.2	110110	20.2	20.2	1100	20.2
Actuated g/C Ratio	0.50	0.50	0.68	0.68		0.20	0.20		0.20
v/c Ratio	0.36	0.65	0.51	0.26		0.75	0.24		0.01
Control Delay	17.8	4.8	10.6	7.2		53.8	7.9		27.8
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	17.8	4.8	10.6	7.2		53.8	7.9		27.8
LOS	В	A	В	A		D	A		C
Approach Delay	10.7			8.3		39.5			27.8
Approach LOS	В			A		D			C
Queue Length 50th (m)	35.6	0.0	18.4	20.7		35.9	0.0		0.6
Queue Length 95th (m)	63.3	27.0	37.6	36.3		54.4	11.0		3.1
Internal Link Dist (m)	229.2		30	650.7		185.4			41.8
Turn Bay Length (m)		125.0	75.0						
Base Capacity (vph)	1702	1123	624	2308		377	503		488
Starvation Cap Reductn	0	0	0	0		0	0		0
Spillback Cap Reductn	0	0	0	0		0	0		0
Storage Cap Reductn	0	0	0	0		0	0		0
Reduced v/c Ratio	0.36	0.65	0.45	0.26		0.52	0.18		0.01
Intersection Summary									
Cycle Length: 100									
Actuated Cycle Length: 100									
Offset: 0 (0%), Referenced to phase 2:	FBTL and	16·WBTL S	tart of Gree	n					
Natural Cycle: 75	LD I L and	O.11D12, O	tart or oroon						
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.75									
Intersection Signal Delay: 13.2				Int	ersection LO	OS: B			
Intersection Capacity Utilization 87.6%					U Level of S				
Analysis Period (min) 15				10	O E0401 01 C	701 ¥100 L			
·									
Splits and Phases: 1: Moodie & Carl	ling								
_	1 4	B.					- 1 1		



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	44	7	*	∳ ኄ		43		43-
Traffic Volume (vph)	7	952	12	24	915	7	0	8	1
Future Volume (vph)	7	952	12	24	915	7	0	8	1
Lane Group Flow (vph)	7	952	12	24	923	0	27	0	18
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2			6		8		4
Permitted Phases	2	_	2	6	•	8		4	•
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	63.7	63.7	63.7	63.7	63.7		12.6		12.6
Actuated g/C Ratio	0.85	0.85	0.85	0.85	0.85		0.17		0.17
v/c Ratio	0.02	0.33	0.01	0.05	0.32		0.10		0.07
Control Delay	5.9	4.6	0.0	5.7	4.5		6.4		17.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.9	4.6	0.0	5.7	4.5		6.4		17.1
LOS	Α	Α	Α	Α	Α		Α		В
Approach Delay		4.5			4.5		6.4		17.1
Approach LOS		Α			Α		Α		В
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.0		0.0		1.1
Queue Length 95th (m)	2.1	57.3	0.1	5.1	54.9		3.9		5.0
Internal Link Dist (m)		432.6			93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	459	2880	1295	444	2878		527		490
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.02	0.33	0.01	0.05	0.32		0.05		0.04
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase	e 2·FRTL and	6 WRTL S	tart of Gree	n					
Natural Cycle: 55	C Z.LDTL and	O.VVDTL, O	tart or Orco	I.I.					
Control Type: Actuated-Coordinated	Н								
Maximum v/c Ratio: 0.33	u								
Intersection Signal Delay: 4.7				Int	ersection Lo	DS: A			
Intersection Capacity Utilization 45.	9%				U Level of S				
Analysis Period (min) 15	/0			IU	C LOVEI UI C	OI VIOG A			
Splits and Phases: 2: Bedale & C	Carling								
Ø2 (R)						1/2	Ø4		
43 s						32 s	דש		
4_									
▼ Ø6 (R)						- T	Ø8		



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		414	7	¥	♠ ₽		*	7		43-	
Traffic Volume (vph)	1	662	131	126	752	763	0	90	3	3	
Future Volume (vph)	1	662	131	126	752	763	0	90	3	3	
Lane Group Flow (vph)	0	663	131	126	753	0	763	90	0	6	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		2		1	6		8			4	
Permitted Phases	2		2	6		8		8	4		
Detector Phase	2	2	2	1	6	8	8	8	4	4	
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	28.6	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0	
Total Split (s)	40.0	40.0	40.0	15.0	55.0	75.0	75.0	75.0	75.0	75.0	
Total Split (%)	30.8%	30.8%	30.8%	11.5%	42.3%	57.7%	57.7%	57.7%	57.7%	57.7%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)		5.6	5.6	5.6	5.6		6.0	6.0		6.0	
Lead/Lag	Lag	Lag	Lag	Lead							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes							
Recall Mode	C-Min	C-Min	C-Min	None	C-Min	None	None	None	None	None	
Act Effct Green (s)		31.7	31.7	46.5	46.5		71.9	71.9		71.9	
Actuated g/C Ratio		0.24	0.24	0.36	0.36		0.55	0.55		0.55	
v/c Ratio		0.84	0.28	0.65	0.62		1.03	0.10		0.01	
Control Delay		57.2	7.6	44.7	36.8		69.6	4.5		14.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay		57.2	7.6	44.7	36.8		69.6	4.5		14.5	
LOS		Е	Α	D	D		Е	Α		В	
Approach Delay		49.0			37.9		62.8			14.5	
Approach LOS		D			D		Е			В	
Queue Length 50th (m)		84.1	0.0	21.8	81.5		~212.8	1.5		0.7	
Queue Length 95th (m)		105.3	15.1	35.9	100.3		#290.1	9.6		2.8	
Internal Link Dist (m)		229.2			716.2		182.2			48.7	
Turn Bay Length (m)			125.0	75.0							
Base Capacity (vph)		855	497	196	1288		744	873		852	
Starvation Cap Reductn		0	0	0	0		0	0		0	
Spillback Cap Reductn		0	0	0	0		0	0		0	
Storage Cap Reductn		0	0	0	0		0	0		0	
Reduced v/c Ratio		0.78	0.26	0.64	0.58		1.03	0.10		0.01	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03

Intersection Signal Delay: 49.7

Intersection Capacity Utilization 106.9%

Intersection LOS: D
ICU Level of Service G

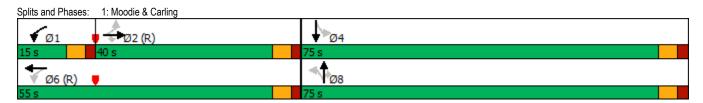
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	44	7	*	ት ጌ		43		43-
Traffic Volume (vph)	6	800	2	10	835	16	0	4	0
Future Volume (vph)	6	800	2	10	835	16	0	4	0
Lane Group Flow (vph)	6	800	2	10	842	0	59	0	9
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2			6		8		4
Permitted Phases	2		2	6		8		4	
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag									• • •
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	59.4	59.4	59.4	59.4	59.4		12.6		12.6
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79		0.17		0.17
v/c Ratio	0.01	0.30	0.00	0.02	0.31		0.21		0.03
Control Delay	6.2	5.1	0.0	6.2	5.2		12.7		0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	6.2	5.1	0.0	6.2	5.2		12.7		0.2
LOS	Α	Α	Α	Α	Α		В		Α
Approach Delay		5.1			5.2		12.7		0.2
Approach LOS		Α			Α		В		Α
Queue Length 50th (m)	0.2	18.3	0.0	0.3	19.4		2.0		0.0
Queue Length 95th (m)	2.0	45.9	0.0	2.7	48.7		9.2		0.1
Internal Link Dist (m)		488.1			93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	465	2684	1210	488	2682		531		509
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.01	0.30	0.00	0.02	0.31		0.11		0.02
ntersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase	2:FRTI and	6·WRTI S	tart of Gree	n					
Natural Cycle: 55	, 2.LD L and	U.VVDTL, U	tart or Orco	I.I.					
Control Type: Actuated-Coordinated	ı								
Maximum v/c Ratio: 0.31									
ntersection Signal Delay: 5.4				Int	ersection Lo	Λ - 2 - 2			
ntersection Capacity Utilization 42.7	7%				U Level of S				
Analysis Period (min) 15	70			10	0 20001 01 0	OCIVIOC 71			
Splits and Phases: 2: Bedale & C	arling								
A	<u></u>								
♥ Ø2 (R)							Ø4		
43 s						32 s			
₩ (ac (b)						⊸†	Ø8		
♥ Ø6 (R)							108		

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1 12	LDIK	1100	414	W	HUIT
Traffic Volume (veh/h)	T → 800	8	3	857	20	7
Future Volume (Veh/h)	800	8	3	857	20	7
Sign Control	Free	0		Free	Stop	'
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	842	8	3	902	21	7
Pedestrians	042	U	J	302	۷1	1
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	Maria			Mari		
Median type	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			850		1303	425
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			850		1303	425
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		86	99
cM capacity (veh/h)			784		152	578
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	561	289	304	601	28	
Volume Left	0	0	3	0	21	
Volume Right	0	8	0	0	7	
cSH	1700	1700	784	1700	186	
Volume to Capacity	0.33	0.17	0.00	0.35	0.15	
	0.33	0.17	0.00	0.0	3.9	
Queue Length 95th (m)						
Control Delay (s)	0.0	0.0	0.1	0.0	27.8	
Lane LOS	2.2		A		D	
Approach Delay (s)	0.0		0.0		27.8	
Approach LOS					D	
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			37.2%	ICI	J Level of S	ervice
Analysis Period (min)			15			
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Lane Group	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	414	7	*	∳ ሴ		*	#		₽.
Traffic Volume (vph)	582	727	294	566	191	5	107	1	3
Future Volume (vph)	582	727	294	566	191	5	107	1	3
Lane Group Flow (vph)	582	727	294	570	0	196	107	0	4
Turn Type	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA
Protected Phases	2		1	6		8	. •		4
Permitted Phases	=	2	6	•	8		8	4	•
Detector Phase	2	2	1	6	8	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0
Total Split (s)	40.0	40.0	25.0	65.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	40.0%	40.0%	25.0%	65.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6		6.0	6.0		6.0
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	C-Min	C-Min	None	C-Min	None	None	None	None	None
Act Effct Green (s)	49.8	49.8	68.2	68.2		20.2	20.2		20.2
Actuated g/C Ratio	0.50	0.50	0.68	0.68		0.20	0.20		0.20
v/c Ratio	0.34	0.65	0.52	0.25		0.75	0.27		0.01
Control Delay	17.9	4.9	10.7	7.1		53.8	7.6		27.8
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	17.9	4.9	10.7	7.1		53.8	7.6		27.8
LOS	В	Α	В	Α		D	Α		С
Approach Delay	10.7			8.3		37.5			27.8
Approach LOS	В			Α		D			С
Queue Length 50th (m)	34.0	0.0	19.5	19.6		35.9	0.0		0.6
Queue Length 95th (m)	61.0	27.6	39.5	34.6		54.4	11.9		3.1
Internal Link Dist (m)	229.2			691.7		185.4			41.8
Turn Bay Length (m)		125.0	75.0						
Base Capacity (vph)	1689	1120	635	2308		377	515		488
Starvation Cap Reductn	0	0	0	0		0	0		0
Spillback Cap Reductn	0	0	0	0		0	0		0
Storage Cap Reductn	0	0	0	0		0	0		0
Reduced v/c Ratio	0.34	0.65	0.46	0.25		0.52	0.21		0.01
Intersection Summary									
Cycle Length: 100									
Actuated Cycle Length: 100									
Offset: 0 (0%), Referenced to phase	2·FRTL and	I 6 WRTL S	tart of Gree	n					
Natural Cycle: 75	Z.EDTE and	0. WD 1 L, O	tuit of Oloo						
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.75									
Intersection Signal Delay: 13.2				Int	ersection Lo	OS: B			
Intersection Capacity Utilization 87.4	%				U Level of S				
Analysis Period (min) 15				.0	2 20.01010				
Splits and Phases: 1: Moodie & Ca	arling								



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	
ane Configurations	*	44	7	*	ት ጌ		43-		43-	
raffic Volume (vph)	7	911	12	24	878	7	0	8	1	
uture Volume (vph)	7	911	12	24	878	7	0	8	1	
ane Group Flow (vph)	7	911	12	24	886	0	27	0	18	
rn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	
otected Phases	1 01111	2	1 01111	1 01111	6	1 01111	8	1 01111	4	
ermitted Phases	2		2	6	0	8	U	4	-	
etector Phase	2	2	2	6	6	8	8	4	4	
witch Phase				U	0	0	U	-	-	
inimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
inimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1	
otal Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0	
otal Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%	
ellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	
II-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8	
ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	2.0	0.0	2.0	0.0	
otal Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1	
ead/Lag	3.0	3.0	3.0	5.0	5.0		0.1		0.1	
.ead-Lag Optimize?										
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None	
act Effct Green (s)	63.7	63.7	63.7	63.7	63.7	None	12.6	None	12.6	
actuated g/C Ratio	0.85	0.85	0.85	0.85	0.85		0.17		0.17	
ctuated g/C Ratio	0.03	0.83	0.63	0.05	0.83		0.17		0.17	
Control Delay	5.7	4.5	0.01	5.7	4.4		6.4		17.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.4		0.0	
otal Delay	5.7	4.5	0.0	5.7	4.4		6.4		17.1	
OS	3.7 A	4.5 A	0.0 A	3.7 A	4.4 A		0.4 A		17.1 B	
	A	4.4	A	A	4.4		6.4		17.1	
pproach Delay pproach LOS		4.4 A			4.4 A		0.4 A		17.1 B	
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.0		0.0		1.1	
Queue Length 95th (m)	2.1	54.0	0.0	5.0	52.0		3.9		5.0	
	2.1	515.4	0.1	5.0	93.8		51.2		59.5	
nternal Link Dist (m)	30.0	313.4	15.0	30.0	33.0		JI.Z		J3.J	
Turn Bay Length (m)	30.0 479	2880	1295	465	2878		527		490	
Base Capacity (vph) Starvation Cap Reductn	479	2000	1295	400	2070		0		490	
pillback Cap Reductn	0	0	0	0	0		0		0	
Storage Cap Reductn	0	0	0	0	0		0		0	
Reduced v/c Ratio	0.01	0.32	0.01	0.05	0.31		0.05		0.04	
	0.01	0.32	0.01	0.00	0.31		0.05		0.04	
ntersection Summary										
ycle Length: 75										
ctuated Cycle Length: 75										
ffset: 7 (9%), Referenced to phase 2	:EBTL and	6:WBTL, S	tart of Gree	n						
atural Cycle: 55										
ontrol Type: Actuated-Coordinated										
aximum v/c Ratio: 0.32										
tersection Signal Delay: 4.6					ersection Lo					
tersection Capacity Utilization 44.7%	, 0			IC	U Level of S	Service A				
nalysis Period (min) 15										
olits and Phases: 2: Bedale & Carl	ling									
A						- N				
❤️Ø2 (R)							Ø4			
13 s						32 s				
										
₩ Ø6 (R)						-	Ø8			

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	_	▼	▼		١,	/
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	≜ t₃			413	14	
Traffic Volume (veh/h)	925	17	6	851	13	4
Future Volume (Veh/h)	925	17	6	851	13	4
Sign Control	Free	•••		Free	Stop	•
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	974	18	6	896	14	4
Pedestrians	314	10	U	090	14	4
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			992		1443	496
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			992		1443	496
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		89	99
cM capacity (veh/h)			693		122	519
						310
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	649	343	305	597	18	
Volume Left	0	0	6	0	14	
Volume Right	0	18	0	0	4	
cSH	1700	1700	693	1700	147	
Volume to Capacity	0.38	0.20	0.01	0.35	0.12	
Queue Length 95th (m)	0.0	0.0	0.2	0.0	3.1	
Control Delay (s)	0.0	0.0	0.3	0.0	32.9	
Lane LOS			Α		D	
Approach Delay (s)	0.0		0.1		32.9	
Approach LOS	0.0		• • • • • • • • • • • • • • • • • • • •		D D	
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			39.3%	ICI	U Level of S	ervice
Analysis Period (min)			15			
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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations		414	7	*	∳ Љ		*	#		4	
Traffic Volume (vph)	1	695	131	126	791	763	0	90	3	3	
Future Volume (vph)	1	695	131	126	791	763	0	90	3	3	
Lane Group Flow (vph)	0	696	131	126	792	0	763	90	0	6	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		2		1	6		8			4	
Permitted Phases	2		2	6		8		8	4		
Detector Phase	2	2	2	1	6	8	8	8	4	4	
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	28.6	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0	
Total Split (s)	40.0	40.0	40.0	15.0	55.0	75.0	75.0	75.0	75.0	75.0	
Total Split (%)	30.8%	30.8%	30.8%	11.5%	42.3%	57.7%	57.7%	57.7%	57.7%	57.7%	
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)		5.6	5.6	5.6	5.6		6.0	6.0		6.0	
Lead/Lag	Lag	Lag	Lag	Lead							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes							
Recall Mode	C-Min	C-Min	C-Min	None	C-Min	None	None	None	None	None	
Act Effct Green (s)		32.6	32.6	47.4	47.4		71.0	71.0		71.0	
Actuated g/C Ratio		0.25	0.25	0.36	0.36		0.55	0.55		0.55	
v/c Ratio		0.86	0.27	0.66	0.64		1.04	0.10		0.01	
Control Delay		57.9	7.5	45.4	36.8		74.0	4.5		14.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay		57.9	7.5	45.4	36.8		74.0	4.5		14.5	
LOS		E	A	D	D		E	A		В	
Approach Delay		49.9			38.0		66.7			14.5	
Approach LOS		D			D		Е			В	
Queue Length 50th (m)		88.3	0.0	21.4	85.6		~216.1	1.5		0.7	
Queue Length 95th (m)		111.5	15.1	#37.0	106.6		#290.1	9.6		2.8	
Internal Link Dist (m)		229.2			687.9		182.2			48.7	
Turn Bay Length (m)			125.0	75.0							
Base Capacity (vph)		855	497	193	1288		734	863		841	
Starvation Cap Reductn		0	0	0	0		0	0		0	
Spillback Cap Reductn		0	0	0	0		0	0		0	
Storage Cap Reductn		0	0	0	0		0	0		0	
Reduced v/c Ratio		0.81	0.26	0.65	0.61		1.04	0.10		0.01	

Intersection Summary

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.04

Intersection Signal Delay: 51.1

Intersection Capacity Utilization 109.0%

Intersection LOS: D
ICU Level of Service H

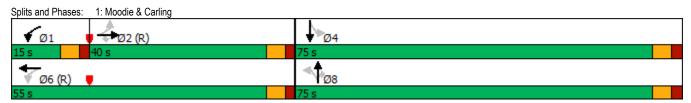
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

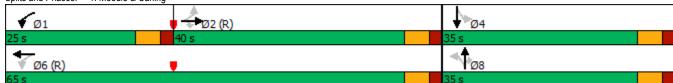
Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	44	#	*	∳ ሴ		43-		43-
Traffic Volume (vph)	6	840	2	10	878	16	0	4	0
Future Volume (vph)	6	840	2	10	878	16	0	4	0
Lane Group Flow (vph)	6	840	2	10	885	0	59	0	9
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2			6		8		4
Permitted Phases	2		2	6		8		4	
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	59.4	59.4	59.4	59.4	59.4		12.6		12.6
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79		0.17		0.17
v/c Ratio	0.01	0.31	0.00	0.02	0.33		0.21		0.03
Control Delay	6.2	5.2	0.0	6.2	5.3		12.7		0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	6.2	5.2	0.0	6.2	5.3		12.7		0.2
LOS Approach Delay	A	Α	Α	Α	A		B		A
Approach LOS		5.2 A			5.3 A		12.7 B		0.2 A
Approach LOS Queue Length 50th (m)	0.2	19.5	0.0	0.3	20.8		2.0		0.0
Queue Length 95th (m)	2.0	19.5 48.7	0.0	0.3 2.7	20.8 52.2		9.2		0.0
Internal Link Dist (m)	2.0	518.9	0.0	Z.1	93.8		9.2 51.2		59.5
Turn Bay Length (m)	30.0	310.3	15.0	30.0	33.0		J1.Z		33.3
Base Capacity (vph)	441	2684	1210	466	2682		531		509
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.01	0.31	0.00	0.02	0.33		0.11		0.02
Intersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75	0.EDTI 004	6-MDTI C	tart of Cross	n					
Offset: 7 (9%), Referenced to phase Natural Cycle: 55	e z.ebil and	U.WBIL, S	iait of Gree	II					
Control Type: Actuated-Coordinated	d								
Maximum v/c Ratio: 0.33	u								
Intersection Signal Delay: 5.5				Inf	ersection Lo	OS: A			
Intersection Signal Delay, 5.5 Intersection Capacity Utilization 43.	9%				U Level of S				
Analysis Period (min) 15	J /0			10	O FEARI OI S	OCI VICE A			
Splits and Phases: 2: Bedale & C	Carling								
						-1	70.4		
Ø2 (R)							Ø4		
43 S						32 s			
706 (D)						- ≪∱	Ø8		
▼ Ø6 (R)						22	סש		

EBT 841 841 Free 0% 0.95 885	8 8 8	WBL 3	WBT 901 901	NBL 20	NBR
841 841 Free 0% 0.95	8	3	41 ↑ 901	*/*	
841 841 Free 0% 0.95			901		
841 Free 0% 0.95				20	7
Free 0% 0.95				20	7
0% 0.95			Free	Stop	<u>'</u>
0.95			0%	0%	
	0.95	0.95	0.95	0.95	0.95
	0.93	0.93	948	21	0.95 7
003	O	J	340	۷۱	1
None			None		
		893		1369	446
		893		1369	446
		4.1		6.8	6.9
		2.2		3.5	3.3
		100		85	99
					559
ED 1	ED 2		M/D 2		
				-	
0.0	0.0		0.0		
		Α		D	
0.0		0.0		30.5	
				D	
		0.5			
			ICI	J Level of S	ervice
		EB 1 EB 2 590 303 0 0 0 8 1700 1700 0.35 0.18 0.0 0.0 0.0 0.0	893 893 4.1 2.2 100 755 EB 1 EB 2 WB 1 590 303 319 0 0 3 0 8 0 1700 1700 755 0.35 0.18 0.00 0.0 0.0 0.1 0.0 0.0 0.1 A 0.0 0.0 0.0	893 893 4.1 2.2 100 755 EB 1 EB 2 WB 1 WB 2 590 303 319 632 0 0 3 0 0 8 0 0 1700 1700 755 1700 0.35 0.18 0.00 0.37 0.0 0.0 0.1 0.0 0.0 0.0 0.1 0.0 A 0.0 0.0 0.1 0.0 A 0.0 0.0 0.1 0.0 A 0.0 0.0 0.1 0.0	893 1369 893 1369 4.1 6.8 2.2 3.5 100 85 755 137 EB1 EB2 WB1 WB2 NB1 590 303 319 632 28 0 0 3 3 0 21 0 8 0 0 7 1700 1700 755 1700 169 0.35 0.18 0.00 0.37 0.17 0.0 0.0 0.1 0.0 4.4 0.0 0.0 0.1 0.0 4.4 0.0 0.0 0.1 0.0 30.5 A D 0.0 0.0 0.1 0.0 30.5 D 0.5 38.5% ICU Level of S

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	413	7	*	∳ ሴ		*	#		€\$
Traffic Volume (vph)	611	727	294	594	191	5	107	1	3
Future Volume (vph)	611	727	294	594	191	5	107	1	3
Lane Group Flow (vph)	611	727	294	598	0	196	107	0	4
Turn Type	NA	Perm	pm+pt	NA	Perm	NA	Perm	Perm	NA
Protected Phases	2		1	6		8	. •		4
Permitted Phases	=	2	6	•	8		8	4	•
Detector Phase	2	2	1	6	8	8	8	4	4
Switch Phase									
Minimum Initial (s)	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	28.6	28.6	10.6	28.6	31.0	31.0	31.0	31.0	31.0
Total Split (s)	40.0	40.0	25.0	65.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	40.0%	40.0%	25.0%	65.0%	35.0%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	1.9	1.9	1.9	1.9	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6		6.0	6.0		6.0
Lead/Lag	Lag	Lag	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes						
Recall Mode	C-Min	C-Min	None	C-Min	None	None	None	None	None
Act Effct Green (s)	49.8	49.8	68.2	68.2		20.2	20.2		20.2
Actuated g/C Ratio	0.50	0.50	0.68	0.68		0.20	0.20		0.20
v/c Ratio	0.36	0.65	0.54	0.26		0.75	0.27		0.01
Control Delay	18.1	4.9	11.0	7.2		53.8	7.6		27.8
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	18.1	4.9	11.0	7.2		53.8	7.6		27.8
LOS	В	Α	В	Α		D	Α		С
Approach Delay	10.9			8.5		37.5			27.8
Approach LOS	В			Α		D			С
Queue Length 50th (m)	36.1	0.0	19.5	20.8		35.9	0.0		0.6
Queue Length 95th (m)	64.4	27.6	39.5	36.4		54.4	11.9		3.1
Internal Link Dist (m)	229.2			650.7		185.4			41.8
Turn Bay Length (m)		125.0	75.0						
Base Capacity (vph)	1689	1120	622	2308		377	515		488
Starvation Cap Reductn	0	0	0	0		0	0		0
Spillback Cap Reductn	0	0	0	0		0	0		0
Storage Cap Reductn	0	0	0	0		0	0		0
Reduced v/c Ratio	0.36	0.65	0.47	0.26		0.52	0.21		0.01
Intersection Summary									
Cycle Length: 100									
Actuated Cycle Length: 100									
Offset: 0 (0%), Referenced to phase	2-FBTL and	16·WRTL S	tart of Gree	n					
Natural Cycle: 75	L.LD I L GIIO	· •.··· · · · · · · · · · · · · · · · ·	tart or Groo						
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.75									
Intersection Signal Delay: 13.3				Inf	ersection Lo	OS: B			
Intersection Capacity Utilization 87.6	%				U Level of S				
Analysis Period (min) 15				.0	2 20.01010				
0.111 1.111 1.110									
Splits and Phases: 1: Moodie & Ca	ariing								



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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	*	44	1	*	∳ ሴ		43-		4
Traffic Volume (vph)	7	957	12	24	922	7	0	8	1
Future Volume (vph)	7	957	12	24	922	7	0	8	1
Lane Group Flow (vph)	7	957	12	24	930	0	27	0	18
Furn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA
Protected Phases	1 Cilli	2	1 Cilli	1 Cilli	6	1 Cilli	8	T CITII	4
Permitted Phases	2		2	6	U	8	U	4	7
Detector Phase	2	2	2	6	6	8	8	4	4
Switch Phase				U	0	J	U	-	-
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	25.6	25.6	25.6	25.6	25.6	29.1	29.1	29.1	29.1
Total Split (s)	43.0	43.0	43.0	43.0	43.0	32.0	32.0	32.0	32.0
Total Split (%)	57.3%	57.3%	57.3%	57.3%	57.3%	42.7%	42.7%	42.7%	42.7%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.9	1.9	1.9	1.9	1.9	2.8	2.8	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	2.0	0.0	2.0	0.0
Total Lost Time (s)	5.6	5.6	5.6	5.6	5.6		6.1		6.1
Lead/Lag	0.0	0.0	0.0	0.0	0.0		0.1		0.1
Lead-Lag Optimize?									
Recall Mode	C-Min	C-Min	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	63.7	63.7	63.7	63.7	63.7		12.6		12.6
Actuated g/C Ratio	0.85	0.85	0.85	0.85	0.85		0.17		0.17
v/c Ratio	0.02	0.33	0.01	0.05	0.32		0.10		0.07
Control Delay	5.9	4.6	0.0	5.8	4.5		6.4		17.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	5.9	4.6	0.0	5.8	4.5		6.4		17.1
LOS	A	A	A	Α	A		Α		В
Approach Delay		4.5			4.5		6.4		17.1
Approach LOS		Α			Α		Α		В
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.0		0.0		1.1
Queue Length 95th (m)	2.1	57.7	0.1	5.1	55.4		3.9		5.0
Internal Link Dist (m)		556.4	***	***	93.8		51.2		59.5
Turn Bay Length (m)	30.0		15.0	30.0					
Base Capacity (vph)	456	2880	1295	441	2878		527		490
Starvation Cap Reductn	0	0	0	0	0		0		0
Spillback Cap Reductn	0	0	0	0	0		0		0
Storage Cap Reductn	0	0	0	0	0		0		0
Reduced v/c Ratio	0.02	0.33	0.01	0.05	0.32		0.05		0.04
ntersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 7 (9%), Referenced to phase	2:EBTL and	6:WBTL, S	tart of Gree	n					
Natural Cycle: 55									
Control Type: Actuated-Coordinated									
Maximum v/c Ratio: 0.33									
Intersection Signal Delay: 4.7					ersection Lo				
Intersection Capacity Utilization 46.0	1%			IC	U Level of S	Service A			
Analysis Period (min) 15									
Splits and Phases: 2: Bedale & Ca	arling								
A						1/2			
√ Ø2 (R)							Ø4		
43 s						32 s			
+ ac (n)						- I⊸†	<i>a</i> 0		
∮ Ø6 (R)							Ø8		

		_		+	4	
	-	*	₹	•	7	
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	♠ ₽			413	W	
Traffic Volume (veh/h)	972	17	6	895	13	4
Future Volume (Veh/h)	972	17	6	895	13	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	1023	18	6	942	14	4
Pedestrians	1020	10		J-12	17	7
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked			1011		4545	F00
vC, conflicting volume			1041		1515	520
vC1, stage 1 conf vol						
vC2, stage 2 conf vol			4044		4545	500
vCu, unblocked vol			1041		1515	520
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		87	99
cM capacity (veh/h)			664		109	501
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	682	359	320	628	18	
Volume Left	0	0	6	0	14	
Volume Right	0	18	0	0	4	
cSH	1700	1700	664	1700	132	
Volume to Capacity	0.40	0.21	0.01	0.37	0.14	
Queue Length 95th (m)	0.0	0.0	0.2	0.0	3.5	
Control Delay (s)	0.0	0.0	0.3	0.0	36.5	
Lane LOS			A		E	
Approach Delay (s)	0.0		0.1		36.5	
Approach LOS	0.0		• • • • • • • • • • • • • • • • • • • •		E	
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			40.5%	ICI	J Level of S	nvice
			40.5%	ICC	LEVELOI O	SI VICE
Analysis Period (min)			15			