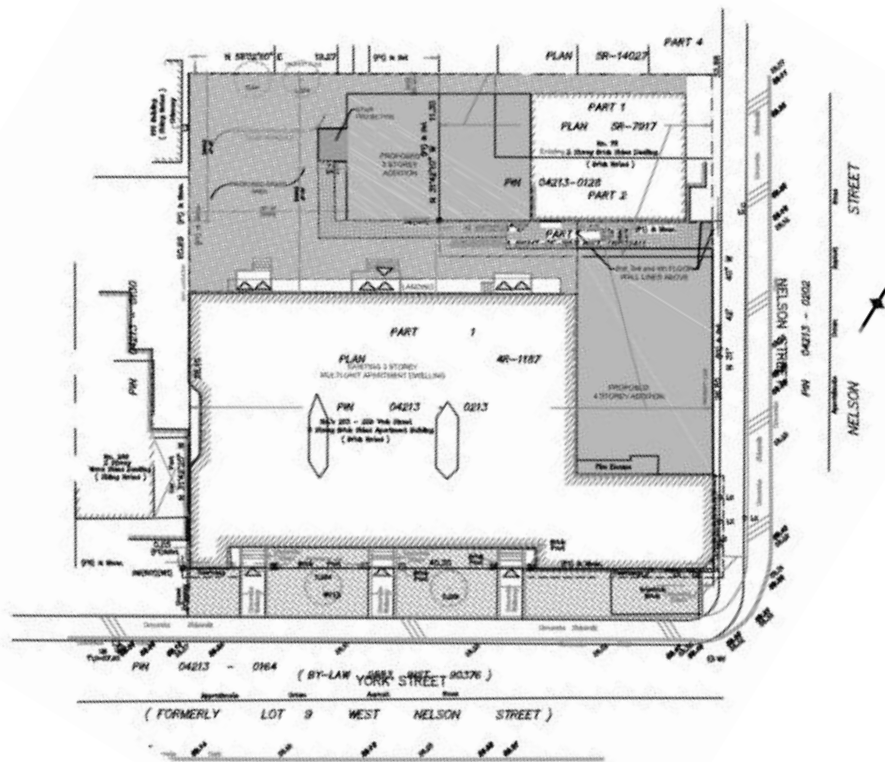


SERVICING & STORMWATER MANAGEMENT REPORT

253 YORK STREET AND 78 & 80 NELSON STREET



Project No.: CCO-22-0938

City File No.: D07-12-21-0135

Prepared for:

Smart Living Properties
226 Argyle Avenue
Ottawa, ON K2P 1B9

Prepared by:

McIntosh Perry Consulting Engineers Ltd.
115 Walgreen Road
Carp, ON K0A 1L0

2022-03-10

TABLE OF CONTENTS

1.0	PROJECT OVERVIEW	1
1.1	<i>Purpose.....</i>	1
1.2	<i>Site Description.....</i>	1
1.3	<i>Proposed Development and Statistics</i>	2
1.4	<i>Existing Conditions and Infrastructure</i>	2
1.5	<i>Approvals</i>	3
2.0	BACKGROUND STUDIES, STANDARDS AND REFERENCES	4
2.1	<i>Background Reports / Reference Information.....</i>	4
2.2	<i>Applicable Guidelines and Standards</i>	4
3.0	PRE-CONSULTATION SUMMARY	5
4.0	WATERMAIN	6
4.1	<i>Existing Watermain.....</i>	6
4.1.1	<i>York Street.....</i>	6
4.1.2	<i>Nelson Street.....</i>	6
4.2	<i>Proposed Watermain</i>	6
5.0	SANITARY DESIGN	10
5.1	<i>Existing Sanitary Sewer</i>	10
5.2	<i>Proposed Sanitary Sewer.....</i>	10
6.0	STORM SEWER & STORMWATER MANAGEMENT DESIGN.....	13
6.1	<i>Existing Storm Sewers</i>	13
6.2	<i>Proposed Storm Sewers.....</i>	13
7.0	STORMWATER MANAGEMENT	14
7.1	<i>Design Criteria and Methodology.....</i>	14
7.2	<i>Quality Control</i>	14
7.2.1	<i>Runoff Calculations.....</i>	14
7.3	<i>Pre-Development Drainage.....</i>	15
7.4	<i>Post-Development Drainage</i>	15
8.0	EROSION AND SEDIMENT CONTROL	17
8.1	<i>Temporary Measures</i>	17
8.2	<i>Permanent Measures</i>	17
9.0	SUMMARY.....	18
10.0	RECOMMENDATION	19
11.0	STATEMENT OF LIMITATIONS	20

LIST OF TABLES

Table 1: Water Supply Design Criteria	6
Table 2: Summary of Estimated Water Demand – York Street.....	7
Table 3: Summary of Estimated Water Demand – Nelson Street.....	7
Table 4: Boundary Conditions Results	8
Table 5: Fire Protection Confirmation	9
Table 6: Sanitary Design Criteria	10
Table 7: Summary of Estimated Sanitary Flow – York Street	11
Table 8: Summary of Estimated Sanitary Flow – Nelson Street.....	11
Table 9: Pre-Development Runoff Summary	15
Table 10: Post-Development Runoff Summary	16

APPENDICES

Appendix A: Site Location Plan

Appendix B: City of Ottawa Pre-Consultation Notes

Appendix C: Watermain Calculations

Appendix D: Sanitary Calculations

Appendix E: Pre-Development Drainage Area Plan

Appendix F: Post-Development Drainage Area Plan

Appendix G: Stormwater Management Calculations

1.0 PROJECT OVERVIEW

1.1 Purpose

McIntosh Perry (MP) has been retained by Smart Living Properties to prepare this Servicing and Stormwater Management Report in support of the Site Plan Control (SPC) application process for the proposed development at 253 York Street, 78 Nelson Street, and 80 Nelson Street, within the City of Ottawa.

The main purpose of this report is to present a servicing and stormwater management design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary, and storm sewer servicing for the development, ensuring that existing infrastructure available will adequately service the proposed development.

This report should be read in conjunction with the following drawing:

- CCO-22-0938, C101 – Grading, Drainage & Sediment and Erosion Control Plan
- CCO-22-0938, C102 – Site Servicing Plan
- CCO-22-0938, C103 – Existing Conditions Plan
- CCO-22-0938, PRE – Pre Development Drainage Area Plan (**Appendix 'E'**)
- CCO-22-0938, POST – Post Development Drainage Area Plan (**Appendix 'F'**)

1.2 Site Description



Figure 1: Site Map

The subject property, herein referred to as the site, is located at 253 York Street, 78 Nelson Street, 80 Nelson Street within the Rideau-Vanier Ward in the City of Ottawa. The site covers approximately **0.15 ha** and is located northwest of the York Street and Nelson Street intersection, as shown by **Figure 1** above. The site is zoned for Residential Use (R4UD). Additional details are included on the Site Location Plan included in **Appendix 'A'**.

1.3 Proposed Development and Statistics

The proposed development incorporates a building addition to the existing building within 253 York Street (**B2**) and a building addition to the existing building within 78 & 80 Nelson Street (**B1**). The **Site Plan** proposes an additional 26 bachelor units to the existing 12 bachelor units within **B1**, with street access from Nelson Street. 20 additional residential bachelor units to the existing 31 units are proposed within **B2**, with street access from Nelson Street and York Street. The development is proposed within **0.071 ha** of the site.

1.4 Existing Conditions and Infrastructure

The property is located within the City of Ottawa's Central Sub-Watershed, tributary to the Ottawa River. Two residential buildings exist within the site and are proposed to be retained. The existing building within 253 York Street is serviced via the City's infrastructure within York Street, as designed by Kollaard Associates (Project No. 190502) in January 2020. The existing building within 78 & 80 Nelson Street is serviced via the City's infrastructure within Nelson Street, as designed by W. Elias & Associates (Project No. 2020-42) in September 2020.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal right-of-ways:

◆ York Street

- 152 mm diameter ductile iron watermain;
- 600 mm diameter concrete sanitary sewer, tributary to the King Edward Avenue Trunk; and a
- 600 mm diameter concrete storm sewer, tributary to the Ottawa River approximately 1.5 km downstream.

◆ Nelson Street

- 203 mm diameter ductile iron watermain (within 312 Murray Street);
- 300 mm diameter concrete sanitary sewer, tributary to the King Edward Avenue Trunk; and a
- 300 mm diameter concrete storm Sewer, tributary to the Ottawa River approximately 1.5 km downstream.

1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control approval process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (**ECA**) through the Ministry of Environment, Conservation and Parks (**MECP**) is not anticipated to be required for the development since the three parcels of land will be amalgamated into a single parcel. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90.

2.0 BACKGROUND STUDIES, STANDARDS AND REFERENCES

2.1 Background Reports / Reference Information

As-built drawings of existing services, provided by the City of Ottawa Information centre, within the vicinity of the site were reviewed in order to identify infrastructure available to service the proposed development.

A topographic survey (20112-19) of the site was completed by Annis, O’Sullivan, Vollebakk Ltd. dated October 21, 2019.

The Site Plan, SPD-1 was prepared by Ottawa Carleton Construction dated January 2022 (Site Plan).

2.2 Applicable Guidelines and Standards

City of Ottawa:

- ◆ Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (***Ottawa Sewer Guidelines***)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (***ISTB-2014-01***)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (***PIEDTB-2016-01***)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, January 2018. (***ISTB-2018-01***)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (***ISTB-2018-03***)
 - Technical Bulletin ISTB-2019-01 City of Ottawa, January 2019. (***ISTB-2019-01***)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (***ISTB-2019-02***)
- ◆ Ottawa Design Guidelines – Water Distribution City of Ottawa, July 2010. (***Ottawa Water Guidelines***)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (***ISD-2010-2***)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (***ISDTB-2014-02***)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (***ISTB-2018-03***)

Ministry of Environment, Conservation and Parks:

- ◆ Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (***MECP Stormwater Design Manual***)
- ◆ Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (***MECP Sewer Design Guidelines***)

3.0 PRE-CONSULTATION SUMMARY

A pre-consultation meeting was conducted on July 31st, 2021, regarding the proposed development at 253 York St, 78 Nelson St, and 80 Nelson St. Specific design parameters to be incorporated within this design are noted by the City of Ottawa pre-consultation found in ***Appendix 'B'***.

4.0 WATERMAIN

4.1 Existing Watermain

The subject site is located within the 1W pressure zone, as shown by the Water Distribution figure located in **Appendix 'C'**. The following subsections outline the water infrastructure that exists within York Street and Nelson Street.

4.1.1 York Street

There is an existing 152 mm diameter watermain, that runs the entire length of the property along York Street. There is one public hydrant located approximately 25 m west of the site. Based on the Site Servicing Plan for 253 York Street, prepared by Kollaard Associates and dated January 2020, the existing building is currently serviced by a 102 mm diameter water service connected to the infrastructure within the right-of-way.

4.1.2 Nelson Street

There is an existing 203 mm diameter watermain that runs the entire length of the property, within 312 Murray Street. There is one public hydrant located at the north east corner of York Street and Nelson Street, approximately 36 m east of the site. Based on the Site Servicing Plan for 78 & 80 Nelson Street, prepared by W. Elias & Associates and dated September 2020, the existing building is currently serviced by a 25 mm diameter water service connected to the infrastructure within the right-of-way.

4.2 Proposed Watermain

B1 is proposed to be serviced through the existing building at 78 & 80 Nelson Street. **B2** is proposed to be serviced through the existing building at 253 York Street. The building additions will be serviced via a 25 mm and 100 mm diameter services, respectively. Refer to drawing **C102** for a detailed servicing layout.

Table 1, below, summarizes the water supply design criteria obtained from the **Ottawa Water Guidelines** and utilized for the water analysis.

Table 1: Water Supply Design Criteria

Site Area	0.154 ha
Residential	280 L/day/person
Residential Bachelor/1 Bedroom	1.4 person/unit
Max Day Peaking Factor	4.9 x avg. day (B1) 9.5 x avg. day (B2)
Peak Hour Peaking Factor	7.4 x avg. day (B1) 14.3 x avg. day (B2)

The water analysis results for **B2** have been summarized in **Table 2**, below. The fire flow demand accounted for both the existing above-ground floor area and the proposed area within **B2**.

Table 2: Summary of Estimated Water Demand – York Street

Design Parameter	Total Flow (L/s)	Total Flow (L/s)	Total Flow (L/s)
	Existing	Proposed	Total
Average Daily Demand	0.4*	0.09	0.49
Max Day Demand	1.0*	0.86	1.86
Max Day + Fire Flow Demand	-	367.53 (FUS) 175.86 (OBC)	-
Peak Hour Demand	2.2*	1.30	2.50

Per Kollaard Associates Engineers (File No. 190502) Servicing Brief for 253, 255, 257, 259 York Street and dated January 31, 2020 (Rev 1). Analysis based on a peaking factor of 2.5 x avg day and 2.2 x max day for the max day and peak hour demands. Refer to **Appendix 'C' for relevant excerpt.*

The water analysis results for **B1** have been summarized in **Table 3**, below. The fire flow demand accounted for both the existing above-ground floor area and the proposed area within **B1**.

Table 3: Summary of Estimated Water Demand – Nelson Street

Design Parameter	Total Flow (L/s)	Total Flow (L/s)	Total Flow (L/s)
	Existing	Proposed	Total
Average Daily Demand	0.05	0.12	0.17
Max Day Demand	0.27	0.58	0.85
Max Day + Fire Flow Demand	-	183.91 (FUS) 45.58 (OBC)	-
Peak Hour Demand	0.40	0.87	1.27

The Fire Underwriters Survey 1999 (FUS) method was utilized to estimate the required fire flow for the site. Fire flow requirements were calculated per City of Ottawa Technical Bulletin **ISTB-2018-03**.

The following parameters were coordinated with the architect:

- ◆ Type of construction – Wood Frame Construction
- ◆ Occupancy type – Combustible
- ◆ Sprinkler Protection – Standard Sprinkler System

The results of the calculations yielded a required fire flow of **366 L/s** (22,000 L/min) for **B2** and **183 L/s** (11,000 L/min) for **B1**. OBC calculations yield a required fire flow of **150 L/s** (9,000 L/min) for **B2** and **45 L/s** (2,700 L/min) for **B1**. The detailed calculations for the FUS and OBC can be found in **Appendix 'C'**.

The City provided the estimated water pressures at both for the average day scenario, peak hour scenario and the max day plus fire flow scenario for the demands indicated by the correspondence in **Appendix C**. The resulting pressures for the boundary conditions results are shown in **Table 4**, below.

Table 4: Boundary Conditions Results

Scenario	Connection 1 - York HGL (m H ₂ O)* / kPa	Connection 1 - Nelson HGL (m H ₂ O)* / kPa
Average Day Demand	57.3 / 562.0	57.3 / 561.9
Maximum Daily + Fire Flow Demand	48.8 / 478.6 (OBC) N/A FUS	50.1 / 491.3 (OBC) 46.1 / 452.0 (FUS)
Peak Hourly Demand	48.1 / 471.8	48.1 / 471.7
*Adjusted for an estimated ground elevation of 58.1m above the connection point for connection.		

The normal operating pressure range is anticipated to be 472 kPa to 562 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi). According to the boundary condition results, the infrastructure within York Street can support the estimated OBC fire flow and the infrastructure within Nelson Street can support both the OBC and FUS fire flows. The proposed watermains will meet the minimum required 20 psi (140 kPa) from the **Ottawa Water Guidelines** at the ground level under maximum day demand and fire flow conditions. A pressure reducing valve (PRV) is anticipated to be required for the site since the pressure is anticipated to exceed 552 kPa (80 psi) in the average day scenario. A pressure test is required during construction to confirm PRV requirements.

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were accounted for per *ISTB 2018-03 Appendix I*. As demonstrated by **Table 5**, below.

Table 5: Fire Protection Confirmation

Fire Flow Demand (L/min.)	Fire Hydrant(s) within 75m	Fire Hydrant(s) within 150m	Combined Fire Flow (L/min)
11,000 L/min – B1 22,000 L/min – B2	3 public	2 public	24,300 (405 L/s)

Based on City guidelines the existing hydrants located in the vicinity can provide adequate fire protection to **B1** and **B2**.

5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

The subject site lies within the King Edward Collector sewer collection area. There is an existing 600 mm diameter sanitary sewer located within York Street tributary to the King Edward Collector sewer approximately 150 m downstream. In addition, there is an existing 300 mm diameter sanitary sewer within Nelson Street tributary to the King Edward Collector sewer approximately 200 m downstream.

5.2 Proposed Sanitary Sewer

The new building additions are proposed to be serviced through the existing buildings. **B1** is proposed to be serviced by the existing 200 mm diameter sanitary lateral, tributary to the 300 mm diameter sanitary sewer within Nelson Street. **B2** is proposed to be serviced by the existing 150 mm diameter sanitary lateral, tributary to the 600 mm diameter sanitary sewer within York Street. Refer to drawing **C102** for the proposed servicing layout.

Table 6, below, summarizes the wastewater design criteria identified by the *Ottawa Sewer Guidelines*.

Table 6: Sanitary Design Criteria

Design Parameter	Value
Residential 1 Bedroom / Studio Apartment	1.4 persons/unit
Average Daily Demand	280 L/day/person
Extraneous Flow Allowance	0.33 L/s/ha

Table 7 and **Table 8** below, summarizes the estimated wastewater flow from the proposed development. Refer to **Appendix 'D'** for detailed calculations.

Table 7: Summary of Estimated Sanitary Flow – York Street

Design Parameter	Total Flow (L/s)	Total Flow (L/s)	Total Flow (L/s)
	Existing	Proposed	Total
Total Estimated Average Dry Weather Flow	0.28*	0.10	0.38
Total Estimated Peak Dry Weather Flow	1.01*	0.35	1.36
Total Estimated Peak Wet Weather Flow	1.05*	0.38	1.43

Per Kollaard Associates Engineers (File No. 190502) Servicing Brief for 253, 255, 257, 259 York Street and dated January 31, 2020 (Rev 1). Based on a peaking factor of 3.61. Refer to **Appendix 'D' for relevant excerpt.*

Capacity of the Nelson Street and York Street sanitary sewers were reviewed to demonstrate that the receiving system could accommodate development. Per the wastewater analysis included in **Appendix 'D'**, the constraining leg between the subject site and the King Edward Collector sewer (MH1 to MH2) is estimated to be at 1% capacity and can accommodate an additional **495.46 L/s** of wastewater drainage. Therefore, the proposed **0.38 L/s** can be collected by the local sewer within York Street.

As noted above, **B2** is proposed to be serviced by the existing 150 mm diameter sanitary lateral servicing the building at 253 York Street. The full flowing capacity of a 150 mm diameter service at a 1% slope is estimated to be **15.2 L/s**. Per **Table 4**, a combined flow of **1.43 L/s** is proposed to be directed towards the York Street sewer and therefore is sufficient sized. The internal servicing layout is to be reviewed by the mechanical engineer.

Table 8: Summary of Estimated Sanitary Flow – Nelson Street

Design Parameter	Total Flow (L/s)	Total Flow (L/s)	Total Flow (L/s)
	Existing	Proposed	Total
Total Estimated Average Dry Weather Flow	0.06	0.12	0.18
Total Estimated Peak Dry Weather Flow	0.21	0.44	0.65
Total Estimated Peak Wet Weather Flow	0.21	0.46	0.67

Capacity of the Nelson Street and Clarence Street sanitary sewers were reviewed to demonstrate that the receiving system could accommodate development. Per the wastewater analysis included in **Appendix 'D'**, the constraining leg between the subject site and the King Edward Collector sewer (MH6 to MH7) is estimated to be at 6.76% capacity and can accommodate an additional **28.22 L/s** of wastewater drainage. Therefore, the proposed **0.46 L/s** can be collected by the local sewer within Nelson Street.

As noted above, **B1** is proposed to be serviced by the existing 200 mm diameter sanitary lateral servicing the building at 78 & 80 Nelson Street. The full flowing capacity of a 200 mm diameter service at a 28% slope is estimated to be **173.6 L/s**. Per **Table 5**, a combined flow of **0.67 L/s** is proposed to be directed towards the Nelson Street sewer and therefore is sufficient sized. The internal servicing layout is to be reviewed by the mechanical engineer.

6.0 STORM SEWER & STORMWATER MANAGEMENT DESIGN

6.1 Existing Storm Sewers

Stormwater runoff from the site is currently tributary to the Ottawa River within the Ottawa Central sub-watershed. There is an existing 600 mm diameter storm sewer within York Street and an existing 300 mm diameter storm sewer within Nelson Street, both tributary to the Ottawa River approximately 1.58 km downstream.

The existing building at 78 & 80 Nelson Street are currently serviced by the Nelson Street storm sewer via a 150 mm diameter service lateral. Refer to W. Elias & Associates Services Plan included in **Appendix 'B'** for further details.

The existing building at 253 York Street are currently serviced by the York Street storm sewer via a 200 mm diameter service lateral. Refer to Kollaard Associates Proposed Servicing Plan included in **Appendix 'B'** for further details.

6.2 Proposed Storm Sewers

A new 250 mm diameter storm service will be extended from the existing 300 mm diameter storm sewer within Nelson Street to collect drainage from building and at-grade common areas within the site.

Foundation drainage is proposed to be connected to the existing building drainage systems, recently designed by W. Elias Associates and Kollaard Associates. The internal servicing layout is to be reviewed by the mechanical engineer.

Runoff collected on the roof of the proposed buildings **B1** and **B2** will be stored and controlled internally using two and three roof drains, respectively. Roof drains will be used to limit the flow from the roof to the specified allowable release rate. For calculation purposes a Watts Accutrol roof drain was used estimate a reasonable roof flow. Other products maybe specified at detailed building design so long as release rates and storage volumes are respected.

Runoff from the at-grade common areas within the site will be directed to two outlets. The first, along the North edge of the property, will convey stormwater overland to Nelson Street via a swale. The second, central in the site, will convey stormwater via a subdrain system to the 300 mm diameter storm sewer within Nelson Street. Storm flows from this will be unattenuated.

See CCO-22-0938 - *POST* include in **Appendix 'F'** of this report for more details. The Stormwater Management design for the subject property will be outlined in *Section 7.0* of this report.

7.0 STORMWATER MANAGEMENT

7.1 Design Criteria and Methodology

Stormwater management for the proposed site will be maintained through two methods. The first will store and control runoff collected on the roof of the proposed buildings. It is estimated that five Watts Accutrol Weirs will be used to control the release rate of the stormwater. The second will control stormwater via a subdrain system and will collect runoff from the at-grade common areas within the site. The flow will be directed to the existing 300 mm storm sewer located within Nelson Street.

The following design criteria will need to be employed to develop the stormwater management design for the site, as directed by the City:

Quality Control

- Due to the distance to the outlet and since no surface parking is proposed, quality controls are not required. Correspondence with the RVCA is included in **Appendix B**.

Quantity Control

- Any storm events greater than 5 year, up to 100 year, and including 100-year storm event must be detained on site.
- Post-development to be restricted to the 5-year storm event, based on a calculated time of concentration greater than 10 minutes and a rational method coefficient of 0.50. Refer to *Section 7.2* for further details.

7.2 Quality Control

7.2.1 *Runoff Calculations*

Runoff calculations presented in this report are derived using the Rational Method, given as:

$$Q = 2.78CIA \text{ (L/s)}$$

Where	C	= Runoff coefficient
	I	= Rainfall intensity in mm/hr (City of Ottawa IDF curves)
	A	= Drainage area in hectares

It is recognized that the Rational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended.

The following coefficients were used to develop an average C for each area:

Roofs/Concrete/Asphalt	0.90
Undeveloped and Grass	0.20

As per the **Ottawa Sewer Guidelines**, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

The time of concentration (Tc) used for pre-development shall be calculated and no less than 10 minutes and post-development flows shall be calculated and no less than 10 minutes.

Based on the criteria listed in *Section 7.2.1*, the development will be required to restrict flow to the 5-year storm event. It is estimated that the target release rate during the 100-year event will be **10.31 L/s** based on the construction limit of **0.071 ha**.

7.3 Pre-Development Drainage

It has been assumed that the existing development contained no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 2, 5, and 100-year events are summarized below in **Table 9**. See CCO-22-0938 - *PRE* in **Appendix 'E'** and **Appendix 'G'** for calculations.

Table 9: Pre-Development Runoff Summary

Drainage Area	Area (ha)	Q (L/s)	
		5-Year	100-Year
A1	0.071	17.16	32.76

7.4 Post-Development Drainage

To meet the stormwater objectives the development will contain a combination of flow attenuation with rooftop controls.

Based on the criteria listed in *Section 7.2.1*, the development will be required to restrict flow to the 5-year storm event. It is estimated that the target release rate during the 100-year event will be **10.31 L/s** based on the construction limit of **0.071 ha**. See **Appendix 'G'** for calculations.

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See CCO-22-0938 - *POST* in **Appendix 'F'** of this report for more details. A summary of the Post-Development Runoff Calculations can be found below.

Table 10: Post-Development Runoff Summary

Drainage Area	Area (ha)	5-year Peak Flow (L/s)	100-year Peak Flow (L/s)	100-year Storage Required (m³)	100-year Storage Available (m³)
B1	0.014	0.64	1.14	4.60	4.89
B2	0.020	1.14	1.89	7.51	7.66
B3	0.017	2.75	5.36	-	-
B4	0.019	3.12	6.08	-	-
Total	0.071	7.66	14.46	12.12	12.55

Runoff for area B1 will be stored on the roof of the proposed building addition (**B1**) and restricted using two Watts Accutrol roof drains (or equivalent product) to a maximum release rate of **1.14 L/s** and will provide up to **4.89 m³** of storage.

Runoff for area B2 will be stored on the roof of the proposed building addition (**B2**) and restricted using three Watts Accutrol roof drains (or equivalent product) to a maximum release rate of **1.89 L/s** and will provide up to **7.66 m³** of storage.

Based on direction from City staff, runoff for area B3 will be unrestricted before discharging to the existing storm system within Nelson Street. The unrestricted flow results in a total release rate of **14.46 L/s**, **4.15 L/s** over the target release rate. As per **Table 9**, above, the pre-development flow rate was estimated as 32.76 L/s, therefore, there is a 56% reduction from existing conditions.

The flow from Area B4 directed to the City's right of ways without restriction and will be compensated or in areas with attenuation.

8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all-natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catch basins and filter fabric is to be placed under the grates of all existing catch basins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures is to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the *Site Grading, Drainage and Sediment & Erosion Control Plan* for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to

the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

9.0 SUMMARY

- The proposed development incorporates a building addition to the existing building within 253 York Street (**B2**) and a building addition to the existing building within 78 & 80 Nelson Street (**B1**). The **Site Plan** proposes an additional 26 bachelor units to the existing 12 units within **B1**, with street access from Nelson Street. 20 additional bachelor units to the existing 31 units are proposed within **B2**, with street access from Nelson Street and York Street. The development is proposed within **0.071 ha** of the site.
- The FUS method estimated fire flow indicated that **11,000 L/min** for **B1** and **22,000 L/min** for **B2** is required for the proposed development;
- The development is estimated to have a peak wet weather flow of **0.46 L/s** for **B1** and **0.38 L/s** for **B2**. Based on the sanitary analysis, the receiving sewer system can accommodate the increased wastewater flow from the site;
- Based on City of Ottawa guidelines, the development will be required to attenuate post-development 5 and 100-year flows to the 5-year release rate of **10.31 L/s**. This flow rate is based on the limit of work area of **0.071 ha**. The unrestricted flow results in a total release rate of **14.46 L/s**, **4.15 L/s** over the target release rate. As per **Table 9**, above, the pre-development flow rate was estimated as 32.76 L/s, therefore, there is a 56% reduction from existing conditions;
- To meet the stormwater objectives the development will contain a combination of flow attenuation with rooftop controls. **12.55 m³** of onsite storage will be required to attenuate flow to the established release rate; and
- Quality controls are not required for the site based on coordination with the RVCA.

10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management report in support of the proposed development at 253 York Street, 78 Nelson Street, and 80 Nelson Street.

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



Alison J. Gosling, P.Eng.
Project Engineer – Land Development
T: 613.714.4629
E: a.gosling@mcintoshperry.com

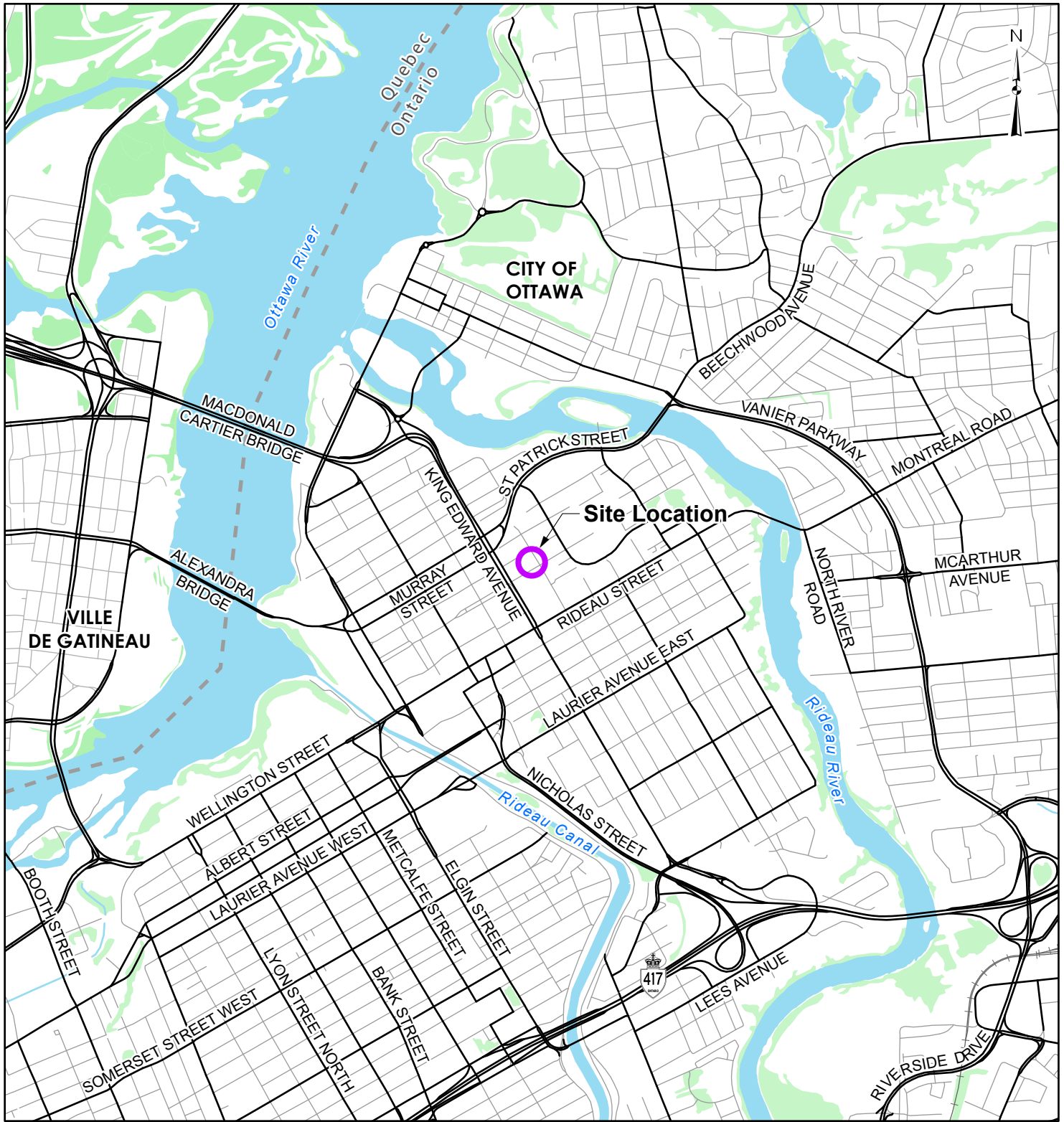
11.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of Smart Living Properties. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Parks and Climate Change, City of Ottawa and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.








Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

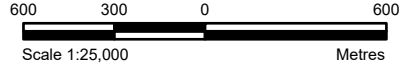
The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

**APPENDIX A
KEY PLAN**



LEGEND

-  Site Location
-  Local Road
-  Major Road
-  Railroad
-  Watercourse
-  Waterbody
-  Wooded Area



REFERENCE

GIS data provided by the Ontario Ministry of Natural Resources and Forestry, 2021.

CLIENT:		SMART LIVING PROPERTIES	
PROJECT:		253 YORK STREET, 78 NELSON STREET, 80 NELSON STREET SERVICING AND STORMWATER MANAGEMENT REPORT	
TITLE:		SITE LOCATION PLAN	
PROJECT NO: CCO-22-0938		FIGURE:	
Date	Aug., 20 2021	1	
GIS	EU		
Checked By	AG		
<p>McINTOSH PERRY 115 Walgreen Road, RR3, Carp, ON K0A1L0 Tel: 613-836-2184 Fax: 613-836-3742 www.mcintoshperry.com</p>			

C:\Users\uncum\McIntosh Perry\GIS - Documents\Projects\2022\CCO\CCO-22-0938 Smart Living - Civil - 253 York Street\MapKey Maps\Land Development\CCO-22-0938_01_SiteLocationPlan.aprx

**APPENDIX B
BACKGROUND DOCUMENTS**

Alison Gosling

From: Fraser, Mark <Mark.Fraser@ottawa.ca>
Sent: July 31, 2020 9:08 AM
To: Deiacco, Simon
Subject: PC2020-0162_253 York Street
Attachments: 253-255-257 York site plan.pdf; ServicingGuidelines_final_Dec2009.pdf; Servicing Report Template Final Version.pdf

Hi Simon,

Please forward the below engineering notes to the applicant regarding a development proposal at **253 York Street**. Note that the information is considered preliminary and the assigned DR Project Manager may modify and/or add additional requirements/conditions upon review of an application or if additional information becomes available.

Comments:

- A deep excavation and dewatering operations have the potential to cause damages to the neighboring adjacent buildings/City infrastructure. Document that construction activities (excavation, dewatering, vibrations associated with construction, etc.) will not have an impact on any adjacent buildings and infrastructure.
- A **Record of Site Condition (RSC)** in accordance with O.Reg. 153/04 will be required to be **filed and acknowledged by the Ministry prior to issuance of a building permit** due to a change to a more sensitive property use. O.Reg 153/04 permits the issuance of a building permit for excavation and shoring of excavation but not a construction permit until RSC is acknowledged by the MECP.

Changes of use, s. 168.3.1 (1) (b) of the Act

14. A person shall not change the use of property for the purposes of clause 168.3.1 (1) (b) of the Act in any of the following manners:

6. If the property is used for a commercial use as well as any other type or types of property use, a change in the use of part or all of the property used for commercial use to any or all of the following types of property uses:

- *Agricultural or other use.*
- *Institutional use.*
- *Parkland use.*
- *Residential use.*
- A singular (1) storm and (1) sanitary service connection to the sewer networks and two (2) water services are permitted for this development if the site is intended to operate under one ownership and remain as single parcel post-development. The additions shall not be serviced independent from the existing building as the City does not want to maintain multiple connections for a single property.
- The sidewalk on Nelson St. fronting the property has a depressed sidewalk for an existing parking lot. This depressed sidewalk shall be removed as part of the site works.
- The condition of the existing services must be verified for reuse to ensure absence of any structural deficiencies and ensure minimum size and materiality requirements are being met. **A CCTV scan and accompanying report must be submitted to the City for review to determine if they are acceptable.** A comment concerning the CCTV scan and lateral condition must also be included in the servicing report, stating that the existing service laterals for the subject property are absent of any structural defects and are of size and material that adheres to City of Ottawa Guidelines. Located existing services are to be placed on site servicing plan.
- Incorporate permeable pavers into the SWM design to further reduce stormwater runoff however no credit in terms of stormwater management (to be considered and impervious for design purposes).

General:

- It is the consultant /designer’s responsibility to verify all the information related to the infrastructures by using as built drawings or field visit and inspection as required.
- It is the sole responsibility of the consultant to investigate the location of existing underground utilities in the proposed servicing area to avoid any conflict with utilities. The location of existing utilities and services shall be documented on an **Existing Conditions Plan**. Verify all the information related to the infrastructures by using as built drawings or field visit and inspection as required. **Include a note on the Servicing and Grading Plan stating the existing utilities in the proposed servicing area have been investigated.**
- All underground and above ground building footprints and permanent walls need to be shown on the plans to confirm that any permanent structure does not extend either above or below into the existing property lines and sight triangles and/or future road widening protection limits.
- Please note that the proposed servicing design and site works shall be in accordance with the following documents:
 - Ottawa Sewer Design Guidelines (October 2012)
 - Technical Bulletin PIEDTB-2016-01
 - Technical Bulletins ISTB-2018-01, ISTB-2018-02 and ISTB-2018-03.
 - Ottawa Design Guidelines - Water Distribution (2010)
 - Design Guidelines for Sewage Works, MECP, 2008
 - Stormwater Planning and Design Manual, MECP, March 2003
 - Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
 - City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
 - City of Ottawa Environmental Noise Control Guidelines (January 2016)
 - City of Ottawa Accessibility Design Standards (November 2015) (City recommends development be in accordance with these standards on private property)
 - Ottawa Standard Tender Documents (latest version)
 - Ontario Provincial Standards for Roads & Public Works (2013)
 - Record drawings and utility plans are also available for purchase from the City (Contact the City’s Information Centre by email at InformationCentre@ottawa.ca or by phone at (613) 580-424 x.44455. **Include copies in the Appendix of the report as supporting documentation.**
- Any proposed work within utility/infrastructure easements requires written consent of the easement owner.



Disclaimer:

The City of Ottawa does not guarantee the accuracy or completeness of the data and information contained on the above image(s) and does not assume any responsibility or liability with respect to any damage or loss arising from the use or interpretation of the image(s) provided. This image is for schematic purposes only.

Stormwater Management Criteria (Quantity and Quality Control) and Servicing Information:

SWM Criteria: Discharging into a Separated Sewer System

- **Water Quantity Control:** Control post-development runoff (for the entire site), up to and including the 100-year storm event, to a **5-year pre-development level**. The pre-development runoff coefficient will need to be determined **using the smaller of a runoff coefficient of C=0.5 or the actual existing site runoff coefficient**. The time of concentration used to determine the pre-development condition will be the larger of 10min. or the calculated time of concentration. [*T_c should not be less than 10 min. since IDF curves become unrealistic at less than 10 min; T_c of 10 minutes shall be used for all post-development calculations*].
 - Total allowable release rate will be **5-year pre-development rate**.
 - C Coefficient of runoff will need to be determined **as per existing conditions but in no case more than 0.5**
 - TC =20 minutes or can be calculated,
 - TC should not be less than 10 minutes, since the IDF curves become unrealistic less than 10min.
 - Any uncontrolled runoff (100-year) is required to be subtracted from the allowable release rate.
- The impact from the receiving system HGL will need to be assessed and considered if proposing underground storage The SWM solution will need to be designed accordingly. The storm connection will need to be above the receiving sewer HGL.
- Any storm events greater than the calculated **5-year allowable release rate**, up to and including the 100-year storm event, shall be detained on-site by appropriate SWM measures to avoid impact on the downstream sewer system.
- **Water Quality:** Please consult with the local conservation authority (RVCA) regarding water quality criteria and onsite requirements prior to submission of an application as the site is located less than 2km from the outlet. It is consultant’s responsibility to check with the RVCA regarding water quality control and include this information and discussion within the SWM report. Contact the RVCA for further information regarding requirements for water quality for this project.

- Compare pre-development flows to post-developments flows in the SWM report to document improvement.
- If **rooftop control and storage** is considered as part of the SWM solution for this project sufficient details (Cl. 8.3.8.4) shall be discussed and documented in the report and on the engineering plans. A **roof drainage plan and detailed roof drain summary table** with supporting drain manufacturer information will be required. Any note indicating to be designed by others will not be sufficient for approval. The roof drainage plan will need to document roof drain type, flow rates, emergency scupper locations and spill over elevations and ponding areas. Roof drains are to be connected downstream of any incorporated ICDs within the SWM system and not to the foundation drain system. Provide a **Roof Drain Plan** as part of the engineering submission package.
- Emergency rooftop scuppers and rooftop flow control drains must be shown on the plan if providing roof top storage for stormwater management.
- Provide a memorandum if Rooftop Flow Control drains are used confirming the new roof will be designed to meet the Stormwater Management objectives with flow control drains and roof spill scuppers in accordance with the requirements of clause 7.4.10.4 of the 2012 Ontario Building code.
- **Underground Storage: Underground storage volumes are to be based on 50% peak flow rates or use dynamic computer model.** The Modified Rational Method for storage computation in the Sewer Design Guidelines was originally intended to be used for above ground storage (i.e. parking lot) where the change in head over the orifice varied from 1.5 m to 1.2 m (assuming a 1.2 m deep CB and a max ponding depth of 0.3 m). This change in head was small and hence the release rate fluctuated little, therefore there was no need to use an average release rate.
 When underground storage is used, the release rate fluctuates from a maximum peak flow based on maximum head down to a release rate of zero. This difference is large and has a significant impact on storage requirements. **We therefore require that an average release rate equal to 50% of the peak allowable rate shall be applied to estimate the required volume. Alternatively, the consultant may choose to use a submersible pump in the design to ensure a constant release rate.**
 In the event that there is a disagreement from the designer regarding the required storage, The City will require that the designer demonstrate their rationale utilizing dynamic modelling, that will then be reviewed by City modellers in the Water Resources Group.
 Note that the above will added to upcoming revised Sewer Design Guidelines to account for underground storage, which is now widely used.
- If an underground storage tank considered as part of the SWM solution for this project sufficient details and system information will need to be provided in the report and on the plans. A detailed cross-section of such SWM system with sufficient details and information (HWLs, release rate, volume, location, size (dimensions), control device, emergency flow outlet and backflow protection, etc.) will need to be provided. An appropriate emergency overflow location will need to be determined and documented. Backup power supply necessary if pump controlled. Details regarding the proposed on-site stormwater management system are to be provided for review.
- The minimum orifice dia. for a plug style ICD is 83mm and the minimum flow rate from a vortex ICD is 6 L/s in order to reduce the likelihood of plugging otherwise choose to pump the SWM system.
- Please include a Pre-Development Drainage Area Plan to define the pre-development drainage areas/patterns. Runoff shall not be directed toward any adjacent properties. Post-development site grading shall match existing property line grades in order to minimize disruption to the adjacent properties.
- Provide a stormwater management summary table within the body of the SWM report to summarize the SWM design for this site.
- Foundation drains are to be independently connected to sewermain (separated or combined) unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.

Implementation Considerations:

- Accounting for external overland drainage.
- Use of standard ICDs
- Requirement for ICD/roof drain information on the plans
- Requirement for plans showing 100-year ponding limits
- Provide a foundation drain backwater valve installed as per Std Dwg S14.

Storm Sewer:

- Available Storm Sewers: A 600mm dia. Conc. storm sewer is available within York Street and a 300mm dia. Conc. storm sewer is available within Nelson Street. Stormwater drains to the King Edward collector sewer system and discharged to the Ottawa River.
For concrete sewer pipe, maintenance manholes shall be installed when the service is greater than 50% of the diameter of the mainline concrete pipe.
- Only one storm connection to the sewer is permitted for this development. Investigate servicing the additions internally through the existing building utilizing the existing sewer lateral if determined to be suitable for reuse. Any new connection is to be discussed with the City. The additions shall not be serviced independent from the existing building.
- A storm sewer monitoring maintenance hole is required to be installed at the property line (on the private side of the property) as per City of Ottawa Sewer-Use By-Law 2003-514 (14) *Monitoring Devices*.
- As-built drawings of the existing services within the vicinity of the site are available and to be reviewed in order to determine proper servicing and SWM plan for the subject site(s).
- Foundation drainage system details are to be discussed in the report and document how the system will be integrated into the servicing design. Please note that foundation drain is to be independently connected to sewermain unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.
- A backwater valve is required on all service laterals for protection.
- Identify if there is an existing storm lateral that drains the foundation weeping tile, if currently installed. If weeping tile is connected to the sanitary lateral, it must be disconnected and drained via separate lateral to the storm sewer system.

Sanitary Sewer:

- Available Sanitary Sewers: A 600mm dia. Conc. sanitary sewer is located within York Street and 300mm dia. Conc. sanitary sewer within Nelson Street. Wastewater flows to the King Edward Trunk Collector sewer system. For concrete sewer pipe, maintenance manholes shall be installed when the service is greater than 50% of the diameter of the mainline concrete pipe.
- Only one sanitary connection to the sewer is permitted for this development. Investigate servicing the additions internally through the existing building utilizing the existing sewer lateral if determined to be suitable for reuse. Any new connection is to be discussed with the City. The additions shall not be serviced independent from the existing building.
- An analysis and demonstration that there is sufficient/adequate residual capacity to accommodate any increase in wastewater flows in the receiving and downstream wastewater system is required to be provided. It is suggested to **calculate the total peak wastewater demand for the proposed development and send it to the City as soon as possible in advance of a submission of an application, as an initial step to determine whether or not there is enough capacity in the city system to accommodate the proposed wastewater flow**. Please note that it takes approx. 10 business days to get a response back from the internal circulation.
- Existing and proposed increases in sanitary flow are to be calculated using the methods established in City of Ottawa Sewer Design Guidelines (SDG) 2012, as amended. Refer to Section 4, Appendix 4-A, and **Technical Bulletin ISTB-2018-01** for guidance.
- A sanitary sewer monitoring maintenance hole is required to be installed at the property line (on the private side of the property) as per City of Ottawa Sewer-Use By-Law 2003-514 (14) *Monitoring Devices*.
- A backwater valve is required on the sanitary service for protection (mandatory now anyways).
- If the site requires to discharge of remediated groundwater, it will require discharge into the sanitary sewer not into city storm sewer per the Sewer Use By-law and subject to a sewer use agreement. Review the ESA reports.

Watermain:

- Available Watermains: A 152mm dia. DI watermain is located within York Street and a 203mm dia. DI watermain is located within Nelson Street. Any connection to the watermain in Nelson Street will be subject to consent from the Owners of 312 Murray St. as the watermain is situated on private property.

- **Analyze and demonstrate there is adequate fire protection** for the buildings per *Technical Bulletin ISTB-2018-02*. Multiple municipal hydrants will be required for fire protection. Adequacy of fire flow must be demonstrated.
- Demonstrate that the pressure requirements, per Section 4.2.2.1, 4.2.2.2, and 4.2.2.3 of the WDG, are met for this building.
- **Water Supply Redundancy:** Residential buildings with a basic day demand greater than 50m³/day (0.57 L/s) are required to be connected to a minimum of two water services separated by an isolation valve to avoid a vulnerable service area as per the *Ottawa Design Guidelines - Water Distribution, WDG001, July 2010 Clause 4.3.1 Configuration*. This proposed development **will require two (2) separate water service connections** if the basic day demand for this site exceeds 50m³/day. There shall be a primary water service and a secondary connection. This is a corner lot so we will not support the installation of a new isolation valve on the City watermain to satisfy this requirement.
- Include a **hydrant coverage figure** and **demonstrate there is adequate fire protection** for the building per *Technical Bulletin ISTB-2018-02*. Multiple municipal hydrants will be required for fire protection.
- **Boundary conditions**, HGL, shall be requested and a hydraulic analysis completed to confirm that the required fire flows can be achieved as well as availability of domestic water pressure. **Use Table 3-3 of the MOE Design Guidelines for Drinking-Water System to determine Maximum Day and Maximum Hour peaking factors for 0 to 500 persons and Table 4.2 of the ODG-Water Distribution for 501 to 3,000 persons.** Provide the following information to the City of Ottawa via email to request water distribution network boundary conditions for the subject site. Please note that once this information has been provided to the City of Ottawa it takes approximately 5-10 business days to receive boundary conditions.
 - Type of Development and Units
 - Site Address (Street Number and Name)
 - Location of service(s).
 - A plan showing the proposed water service connection locations.
 - **Average Daily Demand (L/s)**
 - **Maximum Daily Demand (L/s)**
 - **Peak Hour Demand (L/s)**
 - **Required Fire Flow (L/min) FUS calculations are to be provided with request for boundary conditions.**
 [Fire flow demand requirements shall be based on Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection 1999 and Technical Bulletin ISTB-2018-02]
 Exposure separation distances shall be defined on a figure to support the FUS calculation and required fire flow (RFF).
Fire flow demands will be inputted as point loads at each connection separately unless otherwise noted. A multi-hydrant analysis can be requested if necessary.
 - If fire protection is provided by existing municipal hydrants, hydrant capacity shall be assessed to demonstrate the RFF can be achieved. Identify which hydrants are being considered to meet the RFF on a **fire hydrant coverage figure** as part of the boundary conditions request.
- Include a figure showing the location of applicable fire hydrants supplying adequate fire flow. If fire protection is provided by existing municipal hydrants, hydrant capacity shall be assessed to demonstrate the RFF can be achieved. Identify which hydrants are being considered to meet the RFF on a **fire hydrant coverage figure** as part of the boundary conditions request.
- Hydrant capacity shall be assessed if relying on any public hydrants to provide fire protection particularly if high design fire flows are being proposed to demonstrate the RFF can be achieved. **Refer to Table 1: Maximum flow to be considered from a given hydrant in Appendix I of Technical Bulletin ISTB-2018-02.** Appropriate fire protection mitigation measures shall be investigated/proposed to lower the RFF for the site to an appropriate level.
- The subject site is located within the 1W Pressure Zone.

Snow Storage:

- Any portion of the subject property which is intended to be used for permanent or temporary snow storage shall be as shown on the approved site plan and grading plan. Snow storage shall not interfere with approved grading and drainage patterns or servicing. Snow storage areas shall be setback from the property lines, foundations, fencing or landscaping a minimum of 1.5m. Snow storage areas shall not occupy driveways, aisles, required parking spaces or any portion of a road allowance. If snow is to be removed from the site please indicate this on the plan(s).

Permits and Approvals:

- The consultant shall determine if this project will be subject to an Environmental Compliance Approval (ECA). It shall be determined if the exemptions set out in Ontario Regulation 525/98: *Approval Exemptions* are satisfied. All regulatory approvals shall be documented and discussed in the report. This project will be subject to an ECA application if stormwater works are servicing more than one parcel of land (i.e. 78/80 Nelson Street), regardless of ownership.

Capital Works:

- No capital works are currently planned in this area.

Sight Triangle and Any Road widening Requirement (By Transportation Project Manager Wally Dubyk)

- The Official Plan does not identify York Street or Nelson Street as a protected road corridor. To be confirmed by the Transportation Project Manager assigned to this project.

Required Engineering Plans and Studies in Support of SPC application:

PLANS:

Design drawings are to be provided at a specified **scale (1:200, 1:250, 1:300, 1:400, or 1:500)**

- Existing Conditions and Removals Plan
- Site Servicing Plan
- Grade Control and Drainage Plan
- Erosion and Sediment Control Plan
- Pre-Development Drainage Area Plan
- Post-Development Drainage Area Plan
- Roof Drainage Plan w/ Roof Drain Summary Table (if rooftop SWM storage is being considered)
- Stormwater Storage System Detail (Internal Cistern Details from the Mechanical Engineer if being considered)
- Foundation Drainage System Details
- Legal Survey Plan
- Site Lighting Plan, Photometric Plan and Site Lighting Certification Letter

REPORTS:

- Site Servicing and Stormwater Management Report
- Geotechnical Study
- Phase 2 Detailed Noise Study (Transportation Noise Assessment and Stationary Noise Assessment) [due to proximity (within 100m) of King Edward Ave. (Arterial Road Classification)] Stationary noise to be discussed in report as per City NCG and NPC 300 Guidelines]
- Phase I ESA (in accordance with Ontario Regulation 153/04)
- Phase II ESA (depending on results and recommendation of the Phase I-ESA)
- Record of Site Condition (RSC) is anticipated to be required for this property due to change to a more sensitive property use.

Servicing Report Template and Guidelines:

Please find attached the Servicing Report Template & Study Guidelines” and prepare the servicing study accordingly. For capacity issue, please see section 3.2.1 page 3-3 and follow this section. A completed checklist with corresponding references from the servicing study is mandatory for the completeness of the study. Please add a completed checklist in the report. Please ensure you are using current guidelines, by-laws and standards.

- Include development statistics (unit type breakdown), building construction type, site area, building GFA from the Architect,
- Include description of lot topography, surface drainage patterns, and ground cover (grass, hard landscaping, asphalt, etc.)
- Include description of the existing sewer laterals servicing the building, including size and material type, and street location of connection to the City main.
- Reports must be prepared and stamped by a Professional Engineer licensed in the Province of Ontario.

Please refer to the **City of Ottawa Guide to Preparing Studies and Plans [Engineering]:**

<https://ottawa.ca/en/planning-development-and-construction/developing-property/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans>

Phase One Environmental Site Assessment (Official Plan Section 4.8.4):

- A **Phase I ESA** is required to be completed in accordance with Ontario Regulation 153/04 (not per CSA standards) in support of this development proposal to determine the potential for site contamination. A **Phase II ESA** may be required depending on the results and recommendations of the Phase I-ESA. In accordance with O.Reg. 153/04 TOC requirements.
- The Phase I ESA shall include an RSC statement discussing any requirement to file a RSC with the Ministry. A **Record of Site Condition (RSC)** in accordance with O.Reg. 153/04 is anticipated to be filed and acknowledged by the Ministry prior to issuance of a building permit due to the change in property use. As per the Official Plan (4.8.4) we do not consider an RSC acknowledged by the Ministry until either it's has been confirmed that it will not be audited or it has passed the Ministry audit.
- Please also note that in the event soil and/or groundwater contamination is identified on this site and the proposal is for a more sensitive land use, the MECP will require approximately 1-1.5 years to review the RSC. PIED will apply appropriate conditions, based on Environmental Protection Act (Section 168.3.1 (1)) and O.Reg. 153/04 (Parts IV and V) regarding requirements for RSC prior to building permit issuance. Dependent on the levels/types of contamination, timelines for building permit issuance may be longer than expected and we recommend applicant speak to Building Code Services, at the earliest convenience, so as to discuss these timelines in more detail, if deemed applicable.
- **Environmental Risk Information Services (ERIS) report** is required to be included as part of the Phase I ESA to support the findings and recommendations.
<https://www.ontario.ca/page/guide-completing-phase-one-environmental-site-assessments-under-ontario-regulation-15304>
<https://www.ontario.ca/laws/regulation/040153#BK43>

Geotechnical Investigation (Official Plan Section 4.8.3):

- A **Geotechnical Study/Investigation** shall be prepared in support of this development proposal.
- Investigate the effect of short-term and long-term lowering of the groundwater level and the impact on the adjacent lands and existing neighboring structures. The impact of groundwater lowering on adjacent properties needs to be discussed and investigated to ensure there will be no short term and long term damages associated with lowering the groundwater in this area.
- A deep excavation and dewatering operations have the potential to cause damages to the neighboring adjacent buildings. Document that construction activities (deep excavation, dewatering, vibrations associated with construction, etc.) will not have a negative impact on any adjacent buildings and infrastructure in proximity to the site.
- Dewatering during construction may be subject to volume restrictions, therefore coordinate with the City of Ottawa Sewer Use Program to discuss discharge details.
- Geotechnical Study shall be consistent with the **Geotechnical Investigation and Reporting Guidelines for Development Applications**.
<https://documents.ottawa.ca/sites/default/files/documents/cap137602.pdf>

Detailed Noise Study:

- A **Transportation Noise Assessment** is required as the subject development is located in proximity (within 100m) of King Edward Ave. (Arterial Road Classification)]
- A **Stationary Noise Assessment** is required in order to assess the noise impact of the proposed sources of stationary noise (mechanical HVAC system/equipment) of the development onto the surrounding residential area to ensure the noise levels do not exceed allowable limits specified in the City Environmental Noise Control Guidelines.
- Noise Study shall be consistent with the City's **Environmental Noise Control Guidelines**.
https://documents.ottawa.ca/sites/default/files/documents/enviro_noise_guide_en.pdf

Exterior Site Lighting:

- Any proposed light fixtures (both pole-mounted and wall mounted) must be part of the approved Site Plan. All external light fixtures must meet the criteria for Full Cut-off Classification as recognized by the Illuminating Engineering Society of North America (IESNA or IES), and must result in minimal light spillage onto adjacent properties (as a guideline, 0.5 fc is normally the maximum allowable spillage). In order to satisfy these criteria, the please provide the City with a **Site Lighting Plan, Photometric Plan and Certification (Statement) Letter** from an acceptable professional engineer stating that the design is compliant.

Please note that these comments are considered preliminary based on the information available to date and therefore maybe amended as additional details become available and presented to the City. The above pre-consultation comments are to be considered valid for one year. It is the responsibility of the applicant and their representatives/consultants to verify information provided by the City. The applicant may contact me for any follow-up questions related to engineering/infrastructure prior to submission of an application if necessary.

If you have any questions or require any clarification please let me know.

Regards,

Mark Fraser, P. Eng.

Project Manager, Planning Services
Development Review Central Branch
City of Ottawa | Ville d'Ottawa
Planning, Infrastructure and Economic Development Department
110 Laurier Avenue West, 4th Floor, Ottawa ON, K1P 1J1
[Tel:613.580.2424](tel:613.580.2424) ext. 27791
Fax: 613-580-2576
Mail: Code 01-14
Email: Mark.Fraser@ottawa.ca

*Please consider your environmental responsibility before printing this e-mail

This message, including any document or file attached, is intended only for the addressee and may contain privileged and /or confidential information. Any person is strictly prohibited from reading, using, disclosing or copying this message. If you received this message in error, please notify the sender and delete the message. Thank you.

253-255-257 YORK ST. & 78-80 NELSON ST. COMBINED (YORK STREET IS A FRONTAGE)

SITE PLAN OF SURVEY, PART 1 PLAN AND PART 2 PLAN OF PART OF LOTS 11 AND 12 (South Clarence Street) REGISTERED PLAN 43586, CITY OF OTTAWA

P.I.N.: 04213 - 0213 & P.I.N.: 04213 - 0128 PREPARED BY FARLEY, SMITH & DENIS SURVEYING LTD.

RAUD RESIDENTIAL FOURTH DENSITY ZONE (SEC. 161-162) CITY OF OTTAWA:
DWELLING TYPE: 4 STOREY & 3 STOREY RESIDENTIAL ADDITIONS TO EXISTING THREE STOREY LOW RISE APARTMENT BUILDING (RENTALS)

ZONING MECHANISMS	REQUIREMENT	PROVIDED	NOTES
A) MINIMUM LOT AREA	450 m ²	1535 m ²	
B) MINIMUM LOT WIDTH	15 m	38.10 m	
C) MINIMUM LOT DEPTH	NA	40.30 m	
D) MINIMUM FRONT YARD SET BACK	AVERAGE (0m+4.5 m) / 2 = 2.25	0 m	
E) MINIMUM INTERIOR SIDE YARD SETBACK	1.2 m	1.5 m	
F) MINIMUM CORNER YARD SET BACK	4.5 m	0 m	
G) * MINIMUM REAR YARD AREA (SEE BY-LAW PROVISION AMENDMENTS OUTLINED BELOW)	SEE ZONE PROVISIONS ACCORDING TO OMB ORDER FILE NO. PL150797	30% of 40.30 m x 30% of 38.10 m = 138.19 m ²	238 m ²
I) MINIMUM REAR YARD SET BACK	1.2 m	0 m	
J) MAXIMUM BUILDING HEIGHT	14.5 m	12.34 m	
K) AMENITY AREA	0	194 m ² @ REAR	
L) VEHICLE PARKING	0.5 PER UNITS IN EXCESS OF 12	39 x 0.5 = 19.5	0 MINOR VARIANCE
M) BICYCLE PARKING	0.25 PER ROOMING UNIT	20 x 0.5 = 10	TOTAL: 17.75 24 INDOOR & 28 OUTDOOR
N) WINDOW TO WALL RATIO ALONG THE NELSON STREET ADDITION			33.36%

- PROPOSED TREE
- EX. TREE TO BE REMOVED
- EX. TREE TO REMAIN
- DENOTES SOFT LANDSCAPING (SOD)
- DENOTES ASPHALT
- DENOTES HARD LANDSCAPING
- EXISTING BUILDING TO REMAIN
- PROPOSED ADDITION
- PROPOSED SHRUB (SEE LANDSCAPE PLAN)
- PROPOSED RETAINING WALL
- TREE PROTECTION FENCING
- PROPOSED/EXISTING ENTRY/EXIT
- EX. UTILITY POLE
- EX. CHAINED LINK/BOARD FENCE
- PROPERTY LINE

PROPOSED SITE DEVELOPMENT INFO.		BUILDING AREAS			
LOT COVERAGE:	72.5%	FLOOR NAME	EXISTING	PROPOSED ADDITION	TOTAL
SOFT LANDSCAPING COVERAGE:	282 m ²	BASEMENT			
HARD LANDSCAPING	54.5 m ²	BUILDING AREA	789.5 m ²	310.43 m ²	1099.93 m ²
ASPHALT	0 m ²	GROUND FLOOR			
REAR YARD SOFTSCAPING PERCENTAGE:	78.8%	BUILDING AREA	790 m ²	310.43 m ²	1100.43 m ²
		SECOND FLOOR			
		BUILDING AREA	790 m ²	333.61 m ²	1123.61 m ²
		THIRD FLOOR			
		BUILDING AREA	672 m ²	333.61 m ²	1005.61 m ²
		FOURTH FLOOR			
		BUILDING AREA	0	188.13 m ²	188.13 m ²
		TOTAL BUILDING AREA	3041.5 m ²	1476.21 m ²	4517.71 m ²

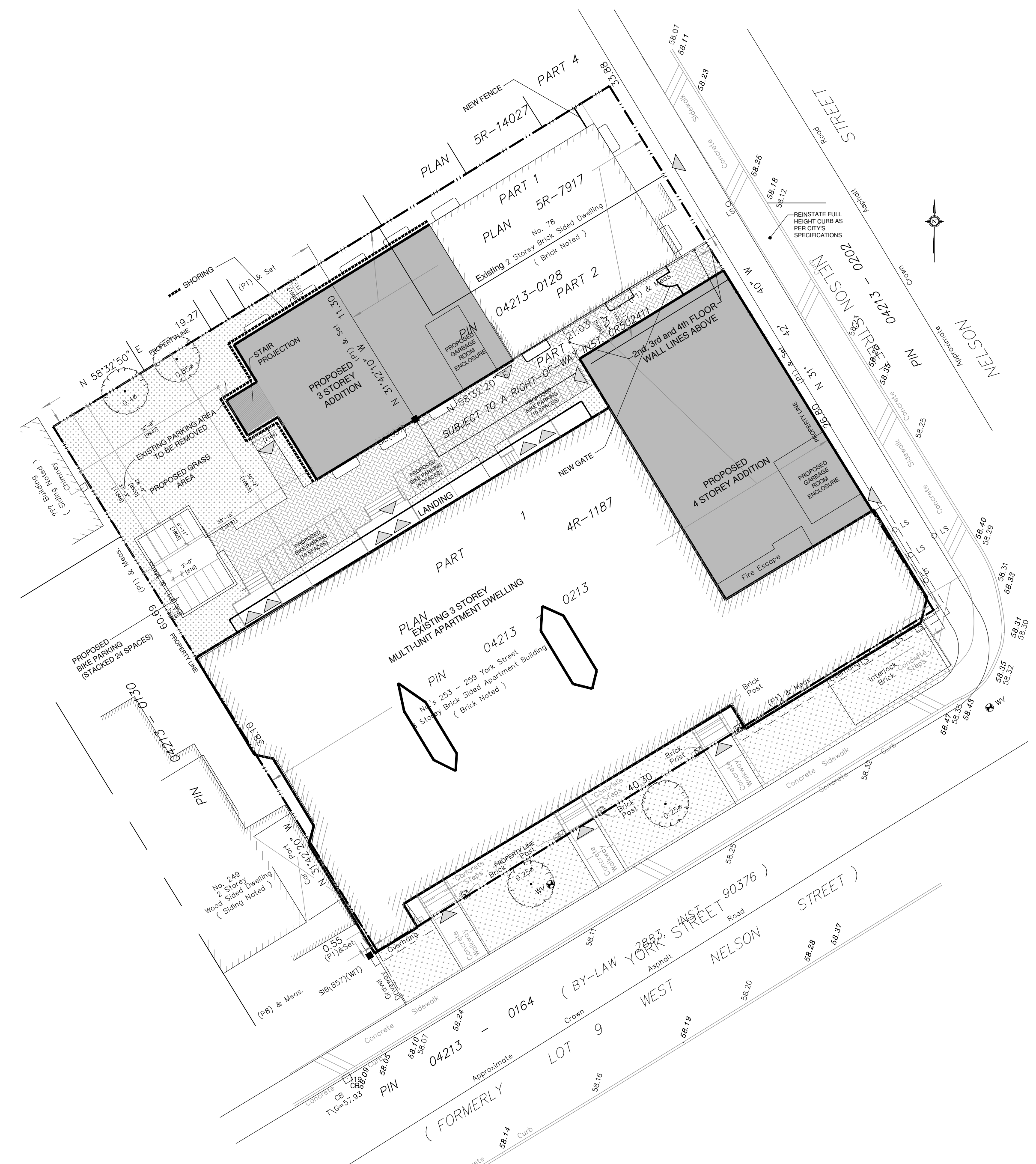
NUMBER OF UNITS	EXISTING	NEW & EXISTING TOTAL
253-255-257 YORK ST.	37 RESIDENTIAL 1 COMMERCIAL	54 RESIDENTIAL 1 COMMERCIAL
78 & 80 NELSON ST.	12 ROOMING UNITS	39 ROOMING UNITS

GARBAGE REQUIREMENT

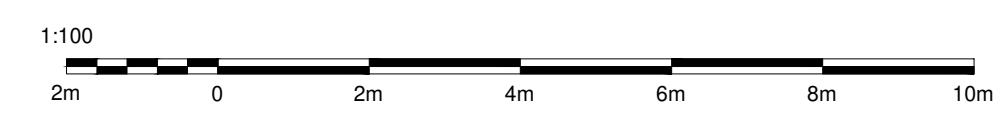
REQUIREMENT	PROVIDED
GARBAGE: 0.231 CUBIC YARDS PER UNIT: 94x0.231=21.714	2-3 CUBIC YARD & 1-2 CUBIC YARD GARBAGE CONTAINERS
FOR EVERY 6 UNITS, PROVIDE ONE 360L BLUE CART AND ONE 360L BLACK CART	1-2 CUBIC YARD FIBRE 2- 360L BLUE CART & 2- 360L BLACK CART
ORGANICS: "KITCHEN CATCHER" CONTAINER FOR EACH DWELLING UNIT AND ONE COMMUNAL 240L GREEN CONTAINER PER 50 UNITS	"KITCHEN CATCHER" CONTAINER FOR EACH DWELLING UNIT AND 3 COMMUNAL 240L GREEN CONTAINER

SNOW REMOVAL REQUIREMENT
PROPERTY OWNER WILL BE RESPONSIBLE FOR THE REMOVAL AND STORAGE OF SNOW FOR ALL WALKWAYS THROUGHOUT THE WINTER

* Alternative Setbacks for Urban Areas (OMB Order File No: PL150797, issued July 25, 2016 - By-law 2015-228)
(12.1) Despite the minimum rear yard and interior side yard setback provisions in columns IX and X of Table 162A, the minimum required rear yard and interior side yard setbacks on a corner lot where the minimum front yard setback is greater than 4.5 metres in Area A on Schedule 342 are as follows:
(a) Except for a lot containing a Planned Unit Development, the minimum setback from any rear lot line or interior side lot line is 1.2 metres; however, a further yard abutting both the interior lot line and the rear lot line must be provided, whichever case applies, as follows:
(iii) for any lots with a lot depth greater than 25 metres: an area equal to 30 per cent of the lot depth by 30 per cent of the lot width, at a minimum. (Subject to By-law 2020-289)



1 SITE PLAN SCALE: 1:200



OTTAWA CARLETON CONSTRUCTION GROUP LTD. - BCN#: 112782
337 SUNNYSIDE AVE., SUITE 101, OTTAWA, ON K1S 0P9
FERNANDO MATOS - BCN#: 22431 613-884-4425

QUALIFICATION INFO
SMALL BUILDINGS
The undersigned has reviewed and takes responsibility for this design, and has the qualifications and meets the requirements set out in the Ontario Building Code to be a designer.

RESPONSIBILITIES:
DO NOT SCALE DRAWINGS
ALL DESIGN AND CONSTRUCTION TO BE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2009
ALL CONTRACTORS MUST WORK IN ACCORDANCE WITH ALL LAWS, REGULATIONS AND BY-LAWS HAVING JURISDICTION
IT IS THE RESPONSIBILITY OF THE APPROPRIATE CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS OF THE AS-BUILT WITH ALL DRAWINGS AND OMISSIONS TO THE ARCHITECT/DESIGNER. COPYRIGHT RESERVED

GENERAL NOTES:

253-255-257 YORK ST. & 78-80 NELSON ST. COMBINED
PROPOSED ADDITION AND RENOVATION

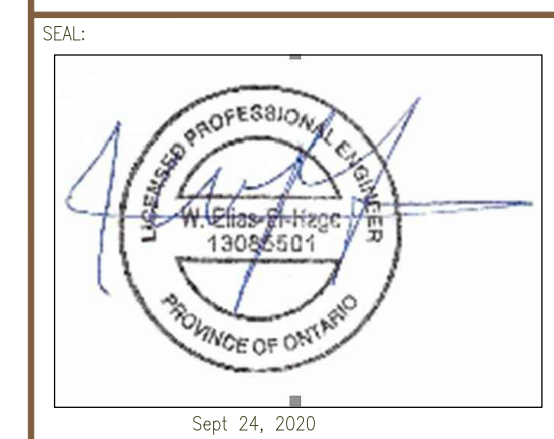
CONSULTANTS: SOMA PRO DESIGNS
STRUCTURAL -
MECHANICAL -
ELECTRICAL -
GRADING -
MDY

NO.	REVISION/ISSUE	DATE
9		
8		
7		
6		
5		
4		
3		
2		
1		

PROJECT:
253-255-257 YORK ST. & 78-80 NELSON ST.
OTTAWA, ONTARIO
K1N 8T8

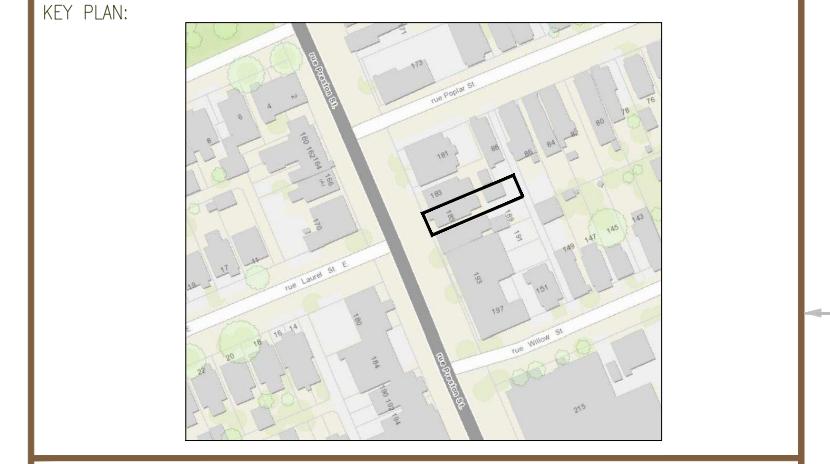
DRAWING NAME:
SITE PLAN AND NOTES

DRAWN BY: L.T. SHEET:
DATE: MAY, 2021
SCALE: AS NOTED
SPD-1



PROPOSED REDEVELOPMENT
78-80 NELSON
OTTAWA, ON K1R 7P8

KEY PLAN:



DISCLAIMER: THIS DRAWING AND DESIGN IS COPYRIGHT PROTECTED WHICH SHALL NOT BE USED, REPRODUCED OR REVISED WITHOUT WRITTEN PERMISSION BY WELIAS ENGINEERING. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND UTILITY LOCATIONS AND REPORT ALL ERRORS AND OMISSIONS PRIOR TO COMMENCING WORK. THIS DRAWING IS NOT TO BE SCALED.

ISSUED FOR - REVISION:

IS	RE	DATE	DESCRIPTION
4		2020-09-23	FOR REVIEW AS PER STAFF COMMENTS
3		2020-09-22	FOR REVIEW AS PER STAFF COMMENTS
2		2020-07-16	FOR REVIEW AS PER STAFF COMMENTS
1		2020-06-22	ISSUED FOR COORDINATION

PROJECT NO:	DATE:
2020-42	2020-03-20

ORIGINAL SCALE: 1:100
DESIGNED BY: R.E.
DRAWN BY: R.E.
CHECKED BY: W.E.
DISCIPLINE:

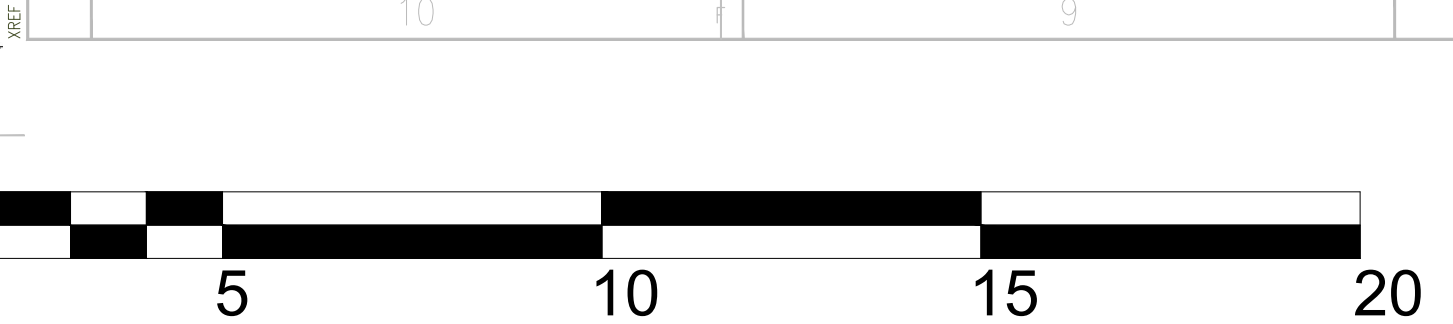
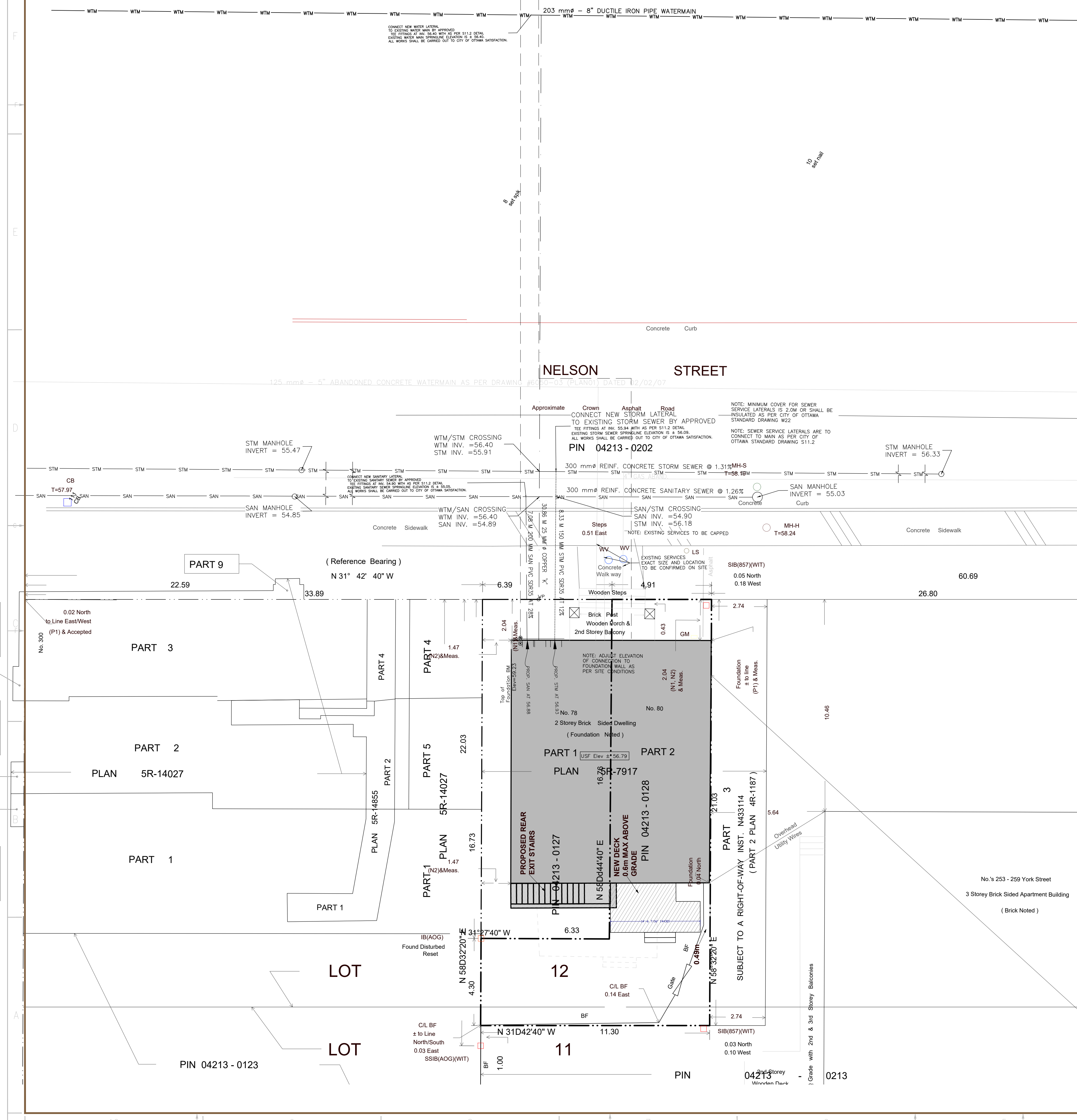
TITLE:
SERVICES PLAN

SHEET NUMBER:
S-1

ISSUED FOR REVIEW
DATE: 2020-06-20

- GENERAL NOTES FOR SERVING**
1. ALL SERVICES, MATERIALS, CONSTRUCTION METHODS AND INSTALLATIONS SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND REGULATIONS OF THE CITY OF OTTAWA STANDARD SPECIFICATIONS AND DRAWINGS, ONTARIO PROVINCIAL SPECIFICATION STANDARD SPECIFICATION (OPSS) AND ONTARIO PROVINCIAL STANDARD DRAWINGS (OPSD), UNLESS OTHERWISE SPECIFIED, TO THE SATISFACTION OF THE CITY AND THE CONSULTANT.
 2. THE POSITION OF EXISTING POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND ABOVEGROUND UTILITIES, STRUCTURES AND APPURTENANCES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWING, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM DURING THE COURSE OF CONSTRUCTION, AND RELOCATION OF UTILITIES REQUIRED BY THE DEVELOPMENT OF SUBJECT LANDS IS TO BE UNDERTAKEN AT CONTRACTOR'S EXPENSE.
 3. THE CONTRACTOR MUST NOTIFY ALL EXISTING UTILITY OWNERS (OPTIONAL FIVE (5) BUSINESS DAYS PRIOR TO START OF CONSTRUCTION) AND MAKE ALL EXISTING UTILITIES AND SERVICES UNDERGROUND AND ABOVEGROUND UTILITIES, STRUCTURES AND APPURTENANCES AS NOTED, BUT NOT LIMITED TO HYDRO, BELL, CABLE TV, AND CONSUMERS GAS LINES.
 4. ALL TRENCHING AND EXCAVATION TO BE IN ACCORDANCE WITH THE LATEST REVISIONS OF THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
 5. REFER TO ARCHITECT'S PLANS FOR BUILDING DIMENSIONS LAYOUT AND REMOVALS. REFER TO LANDSCAPE PLAN FOR LANDSCAPING AND OTHER RELEVANT INFORMATION. ALL INFORMATION SHALL BE CONFIRMED THROUGH COMMUNITY CONSULTATION.
 6. THE LOCATION OF UNDERGROUND SERVICES ARE BASED ON THE SURVEY PROVIDED WITH THE INFORMATION FROM THE CITY OF OTTAWA DRAWING PART "NELSON STREET - REDEVELOPMENT TO YORK ST." HOWEVER, CONTRACTOR MUST ENSURE THAT INFORMATION IS OBTAINED PRIOR TO CONSTRUCTION AND NOTIFY ENGINEER IMMEDIATELY OF ANY DISCREPANCIES.
 7. ALL ELEVATIONS ARE GEODETIC AND UTILIZE METRIC UNITS.
 8. JOB BENCH MARK TO BE USED FROM TO OF FOUNDATION AS INDICATED ON THE DRAWINGS. THIS ELEVATION IS BASED OFF THE DRAWING BY ANAS, O'SULLIVAN, VOLLEKOV LTD. DATED OCTOBER 21, 2018.
 9. ALL GROUND SURFACES SHALL BE EVENLY GRADED WITHOUT PONDING AREAS AND WITHOUT LOW POINTS EXCEPT WHERE APPROVED SWALE OR CATCH BASIN OUTLETS ARE PROVIDED.
 10. ALL EDGES OF DISTURBED PAVEMENT SHALL BE SAW CUT TO FORM A NEAT AND STRAIGHT LINE PRIOR TO PLACING NEW PAVEMENT. PAVEMENT REINSTATEMENT SHALL BE WITH STEP JOINTS OF 500mm WIDTH MINIMUM.
 11. ALL DISTURBED AREAS OUTSIDE PROPOSED GRADING LIMITS TO BE RESTORED TO ORIGINAL ELEVATIONS AND CONDITIONS UNLESS OTHERWISE SPECIFIED. ALL RESTORATION SHALL BE COMPLETED WITH THE GEOTECHNICAL REQUIREMENTS FOR EROSION AND COMPACTION.
 12. ALL MATERIAL SUPPLIED AND PLACED FOR PAVING LOT AND ACCESS ROAD CONSTRUCTION SHALL BE TO OPSS STANDARDS AND SPECIFICATIONS UNLESS OTHERWISE NOTED. CONSTRUCTION TO OPSS 206, 310 & 314. MATERIALS TO OPSS 1001, 1003 & 1010.
 13. CONTRACTOR SHALL OBTAIN AND PAY FOR ALL NECESSARY PERMITS AND APPROVALS FROM THE MUNICIPAL AUTHORITIES PRIOR TO COMMENCING CONSTRUCTION.
 14. MINIMIZE DISTURBANCE TO EXISTING VEGETATION DURING THE EXECUTION OF ALL WORKS.
 15. REMOVE FROM SITE ALL EXCESS EXCAVATED MATERIAL UNLESS OTHERWISE SPECIFIED FROM THE ENGINEER. EXCAVATE AND REMOVE ALL ORGANIC MATERIAL AND DEBRIS LOCATED WITHIN THE PROPOSED BUILDING FOOTPRINT AND CONSTRUCTION.
 16. AT PROPOSED UTILITIES CONNECTION POINTS AND CROSSINGS (E.G. STORM SEWER, SANITARY SEWER, WATER, ETC.) THE CONTRACTOR SHALL DETERMINE THE PRECISE LOCATION AND DEPTH OF EXISTING UTILITIES AND REPORT ANY DISCREPANCIES OR CONFLICTS TO THE ENGINEER BEFORE COMMENCING WORK.
 17. SERVICE TRENCHES ON MUNICIPAL RIGHT OF WAY TO BE REINSTATED AS PER CITY OF OTTAWA DETAIL B01.
 18. PRIOR TO CONSTRUCTION A GEOTECHNICAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO TO INSPECT ALL SUB-SURFACES FOR FOOTINGS, SERVICES AND PAVEMENT STRUCTURES.

- LEGEND**
- 300mm SAN EXISTING SANITARY SEWER
 - 600mm STM EXISTING STORM SEWER
 - 150mm SAN EXISTING WATERMAIN
 - EX SANMH EXISTING SANITARY MANHOLE
 - EX STMHM EXISTING STORM MANHOLE
 - EX CB EXISTING CATCHBASIN
 - V&V EXISTING VALVE AND VALVE CHAMBER
 - V&VB EXISTING VALVE AND VALVE BOX
 - EX FH EXISTING FIRE HYDRANT
 - EX OHB EXISTING OVERHEAD BELL
 - EX OHH EXISTING OVERHEAD HYDRO
 - EX UGH EXISTING UNDERGROUND HYDRO
 - EX GM EXISTING GAS MAIN
 - 135mm SAN PROPOSED SANITARY SEWER
 - 100mm STM PROPOSED STORM SEWER
 - 25mm WM PROPOSED WATERMAIN
 - SANMH PROPOSED SANITARY MANHOLE
 - CBMH PROPOSED CATCHBASIN MANHOLE
 - LMH LANDSCAPE MANHOLE
 - CB1 PROPOSED CATCHBASIN
 - V&VB PROPOSED VALVE AND VALVE BOX
 - FH PROPOSED FIRE HYDRANT
 - WRM PROPOSED WATER REMOTE METER
 - WM PROPOSED WATER METER
 - CS PROPOSED CURB STOP
 - STREET ASPHALT OVERLAY
 - WP WATER POST



LEGEND	
---	EXISTING WATERMAIN
---	EXISTING STORM SEWER
---	EXISTING SANITARY SEWER
---	EXISTING UNASSIGNED CONDUIT SEWER
---	PROPOSED WATERMAIN
---	PROPOSED STORM SEWER
---	PROPOSED SANITARY SEWER
---	EDGELINE OF ROAD
---	EDGE OF ROAD
---	PROPERTY LINE
---	UTILITY MARK
---	SET FENCE
---	EXISTING FIRE DEPARTMENT CONNECTION
---	LIGHT TOWER
---	SAMPLE CONNECTION
---	SANITARY CONNECTION
---	BILL-MOUNTED LIGHT
---	EXISTING FIRE HYDRANT
---	EXISTING WATER VALVE
---	WATER METER
---	REMOVE WATER METER
---	PROPOSED WATER VALVE
---	EXISTING STORM MANHOLE
---	EXISTING SANITARY MANHOLE
---	EXISTING VALVE CHAMBER
---	EXISTING CATCH BASIN
---	PROPOSED CATCH BASIN/MANHOLE
---	PROPOSED CATCH BASIN
---	PROPOSED STORM MANHOLE
---	PROPOSED SANITARY MANHOLE
---	TEMPORARY REWORKMAN

No work shall take place within the Critical Root Zone (10x tree diameter) of a protected tree (By-laws 2006-279 and 2009-200) without approval by Forestry Services. Contact 311 to request a review of the proposed work prior to commencement.

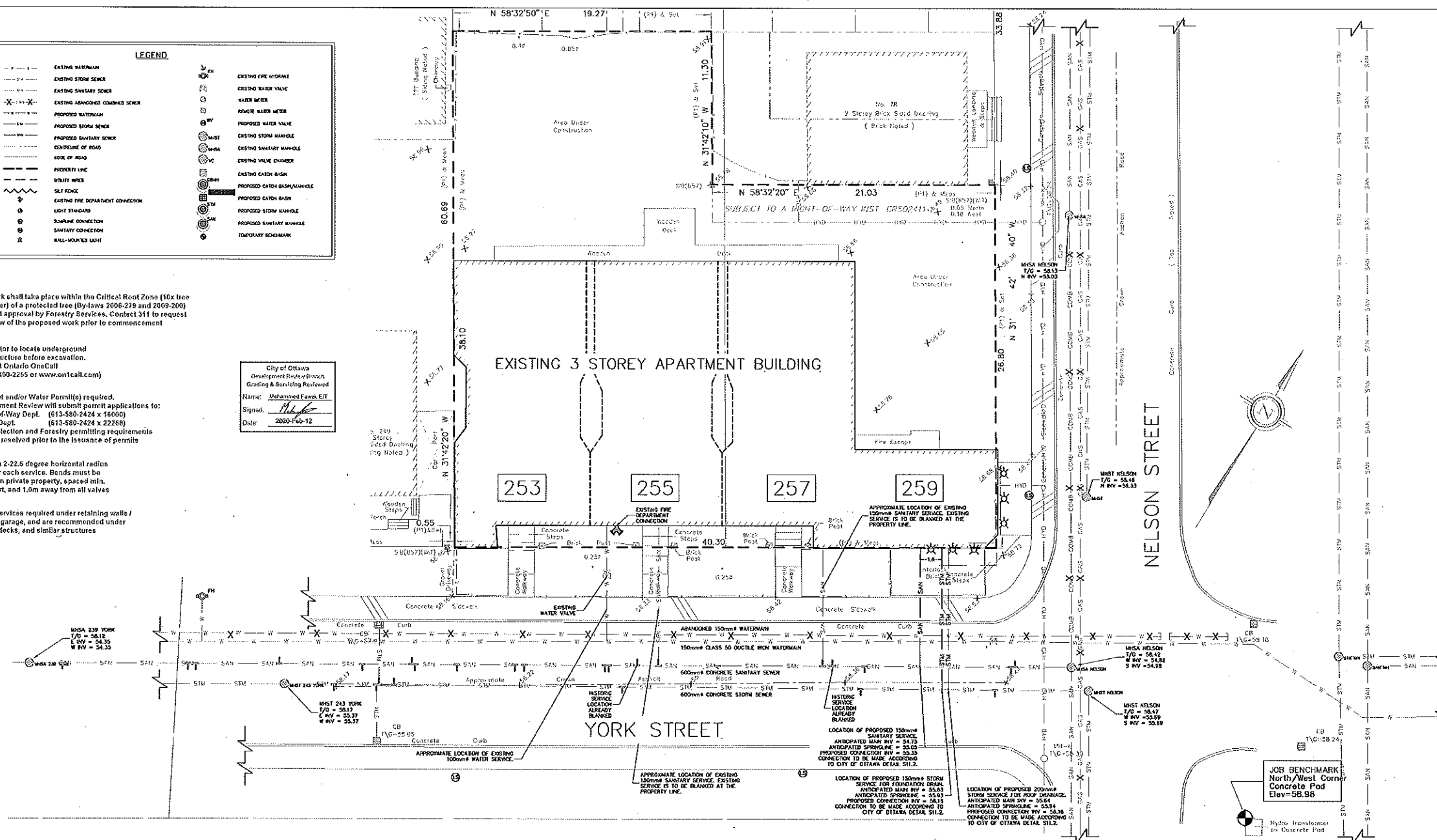
Excavator to locate underground infrastructure before excavation. Contact Ontario OneCall (1-800-400-2265 or www.onecall.com)

Road Cut and/or Water Permit(s) required. Development Review will submit permit applications to:
 • Right-of-Way Dept. (613-580-2424 x 16000)
 • Water Dept. (613-580-2424 x 22268)
 Tree protection and Forestry permitting requirements must be resolved prior to the issuance of permits.

Maximum 2-22.5 degree horizontal radius bends for each service. Bends must be located on private property, spaced min. 1.0m apart, and 1.0m away from all valves.

Sleeved services required under retaining walls / footings / garage, and are recommended under porches, decks, and similar structures.

City of Ottawa
 Development Review Branch
 Grading & Servicing Reviewed
 Name: *Muhammad Fawzi, EIT*
 Signed: *[Signature]*
 Date: 2020-FEB-12



PROPOSED SERVICING PLAN
 SCALE = 1:150

WATERMAIN NOTES:

- CITY TO SUPPLY, INSTALL & COMPLETE THE WATER SERVICE. CONTRACTOR TO EXCAVATE, BACKFILL AND REPAIR THE ROADWAY AS PER STD 010 R18.
- SPECIFICATIONS:

ITEM	REQ. NO.	CITY STD. REF. NO.	REFERENCE
WATERMAIN BEDDING AND BACKFILL (STD)	802.08/802.031	W17 (French detail)	
CATHODIC PROTECTION (STD)	1108.010	W10	
PRESSURE TESTING AREA	C-800-5		
CALIBRATION	C-431-10		
WATERMAIN MATERIAL	PVC D118	(CLASS 150)	
- WATERMAIN SHALL BE MINIMUM 2.4m DEPTH BELOW GRADE UNLESS OTHERWISE INDICATED. WHERE LESS THAN 2.4m COVER, THERMAL INSULATION IS TO BE PROVIDED AS PER CITY STD 010 R22 (in shallow trenches), R23 (at open excavations).
- A MINIMUM OF 0.5m VERTICAL CLEARANCE IS REQUIRED BETWEEN THE WATERMANS AND ALL UTILITIES AND SERVICES. IN LOCATIONS WHERE THIS IS NOT ACHIEVABLE, MUST FOLLOW PROCEDURE F-3-1 SEC. 2.2 OF THE ONTARIO DRINKING WATER RESOURCES ACT.
- METALLIC WARNING TAPE SHALL BE USED OVER ALL WATERMANS.
- INSTALL AND TEST TRACER WIRE FOR ALL PROPOSED WATERMAIN IN ACCORDANCE WITH THE CITY OF OTTAWA DESIGN STANDARDS AS SPECIFIED IN SECTION 6.26.
- EXISTING WATERMAIN INFORMATION SHOWN ON YORK STREET IS BASED ON BEST CURRENT INFORMATION. CONTRACTOR TO VERIFY EXACT LOCATION OF WATERMAIN AND REPORT ANY DISCREPANCIES TO KOLLAARD ASSOCIATES INC.
- WATER SERVICE VALVE AND VALVE BOX TO BE WITHIN THE ROAD ALLOWANCE AND LOCATED A MINIMUM OF 1.0 METERS FROM THE BUILDING FOOTING. TYPICAL PRIVATE SERVICE AS PER STD. 010, R50 (with the exception that the MAINS are to be located 1.0 m minimum from the foundation wall). VALVE BOX ASSEMBLY AS PER STD. 010, R24.
- CONNECTIONS AT ELBOWS AND TEES IN WATER MAINS SHOULD BE MADE WITH THE USE OF JOINT RESTRAINTS DESIGNED FOR WATERMAIN APPLICATION. JOINT AND PIPE RESTRAINTS SHOULD MEET THE REQUIREMENTS OF ARMA 000, 000 AND 001 AND ASTM F1814-11. JOINT RESTRAINTS SHOULD BE INSTALLED AS PER MANUFACTURERS RECOMMENDATIONS.
- ALL CONNECTIONS, HOSE AND VALVE BOLTS SHALL BE STAINLESS STEEL.
- VALVES ARE TO BE OPERATED BY CITY OF OTTAWA ONLY.
- NO CONNECTION TO EXISTING WATER NETWORK SHALL BE COMPLETED UNTIL A WATER POINT IS OBTAINED FROM THE CITY OF OTTAWA AND CITY OF OTTAWA FORCES ARE ON HAND TO MAKE THE CONNECTION.

SEWER NOTES:

- ONTARIO ONE CALL AT 1-800-400-2265.
- ALL NOTES ARE AS PER CURRENT CITY/PROVINCIAL STANDARDS, DIMENSIONS, ET-LANG AND DETAIL DRAWINGS.
- SUPPLY AND CONTRACT ALL SERVICES AND APPURTENANCES IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARDS AND SPECIFICATIONS AND ONTARIO PROVINCIAL STANDARDS FOR ROADS AND PUBLIC WORKS.
- SPECIFICATIONS:

ITEM	CITY OF OTTAWA STD. REF. NO.
CATCH BASIN	SI-541, SI9-235 & SI9-531
TRUNK/PRIMARY MANHOLE	SI-541, SI9-235 & SI9-531
MANHOLE COVER	SI-541, SI9-235 & SI9-531
WATER SERVICE CONNECTION	SI-541, SI9-235 & SI9-531
WATER SERVICE VALVE/INSPECTION CHAMBER	SI-541, SI9-235 & SI9-531
STORM MANHOLE FRAME & COVER	SI-541, SI9-235 & SI9-531
STORM MANHOLE FRAME & COVER	SI-541, SI9-235 & SI9-531
SEWER TRENCH	SI-541, SI9-235 & SI9-531
- SEWER LATERALS:

SANITARY	PVC 50R 28 @ 1.02 (DN)
STORM	PVC 50R 38 @ 1.02 (DN) (NOSE ROOF DRAINS)
WATER	PVC 50R 38 @ 1.02 (DN) (NOSE ROOF DRAINS)

 (IF BONDING REQUIRED, PER STD 010 MANHOLE WATER SERVICE TUBING (SUCH AS BUREAU) OR APPROVED EQUIVALENT)
 TRENCH BACKFILL AS PER GEOTECHNICAL ENGINEER RECOMMENDATIONS REVISOR'S REPORT PROVIDED BY KOLLAARD ASSOCIATES DATED JANUARY 26, 2018.
- ISOLATE ALL STORM PIPES THAT HAVE LESS THAN 2m COVER AND ALL SANITARY PIPES THAT HAVE LESS THAN 2m COVER WITH THERMAL INSULATION. PROVIDE 150mm CLEARANCE BETWEEN PIPE AND INSULATION.
- PIPE BEDDING COVER AND BACKFILL ARE TO BE COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY.
- SERVICE LATERALS ARE TO BE RECEIVED THROUGH THE FOUNDATION WALL.
- FLEXIBLE CONNECTIONS ARE REQUIRED FOR CONNECTION PIPES TO MANHOLES (FOR EXAMPLE: KOR-R-SEAL, PSH, POSITE SEAL AND RUBASEAL). SANITARY FLOODED CASSETT TYPE JOINTS SHALL CONFORM TO CSA (B-182.2.1.1).
- THE OWNER SHALL REQUIRE THAT THE SITE SERVICING CONTRACTOR PERFORM FIELD TESTS FOR QUALITY CONTROL OF ALL SANITARY SERVICES. LEAKAGE TESTING SHALL BE COMPLETED IN ACCORDANCE WITH OPS 410.011, 410.014 AND 410.015. THE TESTING IS TO BE COMPLETED ON ALL SANITARY SERVICES TO COVER PROPER CONNECTION TO THE SANITARY SEWER MAIN. THE FIELD TESTS SHALL BE PERFORMED IN THE PRESENCE OF A CERTIFIED PROFESSIONAL ENGINEER WHO SHALL SUBMIT A CERTIFIED COPY OF THE TEST RESULTS.
- STORM MANHOLES AND CHAMBERS ARE TO HAVE 300mm RAMPS (AS PER SUMP DETAIL ON OPD 701.015), UNLESS OTHERWISE INDICATED.
- BUILDING CONTRACTOR TO PROVIDE TEMPORARY ADDITIONAL GRANULAR BACKFILL ABOVE SHALLOW CHAMBERS AND STORM SEWERS TO SUPPORT HEAVY CONSTRUCTION EQUIPMENT.
- CONTRACTOR TO REMOVE (OCT) ALL PROPOSED SERVICES, TRENCH OR CREATION PRIOR TO BASE COURSE ASPHALT. UPON COMPLETION OF CONTRACT, THE CONTRACTOR IS RESPONSIBLE TO FLUSH AND CLEAN ALL SEWERS & APPURTENANCES TO CITY CONTRACTOR.
- WHERE THE SANITARY SEWER CROSSES ABOVE THE WATERMAIN, THE CONTRACTOR IS TO PROVIDE A MINIMUM OF 0.5m VERTICAL SEPARATION, ADEQUATE STRUCTURAL SUPPORT OF THE SEWER TO PREVENT SETTLING AND EXISTING JOINT COLLECTION AND ENSURE THAT THE LENGTH OF THE WATER PIPE BE COVERED AT THE POINT OF CROSSING SO THAT THE JOINTS ARE EQUIDISTANT AND AS FAR AS POSSIBLE FROM THE SEWER.

SERVICING NOTES:

- A 0.3-METRE CLEARANCE BETWEEN WATERMAIN AND THE SANITARY AND STORM LATERALS IS TO BE PROVIDED.
- ALL SERVICES ARE TO BE INSTALLED AT CURB/STREET CITY OF OTTAWA STANDARDS.
- EXISTING LOCATION OF WATERMAIN, SANITARY SEWER AND STORM SEWER ARE SUPPLEMENTED, ELONGATED AND TRENCH OF EXISTING LATERALS ARE LOCATIONS. IF RE-USE OF EXISTING LATERALS IS DESIRED, IT IS RECOMMENDED THAT THEY BE EXCAVATED AND INSPECTED PRIOR TO REBUILDING/REPAIRING. IF NOT DONE RE-USE, EXISTING ARE TO BE BLANKED AS PER CITY OF OTTAWA STANDARDS.
- SERVICE LATERALS ARE TO HAVE A MINIMUM CLEARANCE OF 3 METERS FROM EXISTING UTILITIES POLES.
- ALL SEWER IN THE SERVICE LATERALS ARE TO BE A MINIMUM OF 2 @ 27 INCH AND BELOW THE PROPERTY.
- EXISTING SANITARY SERVICE TO BE CAPPED AT PROPERTY LINE. WATER SERVICE TO BE BLANKED AT MAIN TO THE SATISFACTION OF CITY FORCES.
- SOME SERVICE CONNECTIONS WITH LESS THAN 2.0m COVER TO BE BLANKED PER OPD 170.010.

DRAWING NUMBER: 190502-SER

KEY PLAN: N/E
 CLASSIERE STREET, NELSON STREET, YORK STREET, NELSON ST.

SCALE: 0 1 2 3 4 5 10 METRES
 SCALE 1:150

GENERAL NOTES:

- All dimensions are in metres; all elevations are in metres and are geodetic. IBM = North/West corner of hydro transformer concrete pad. Elevation: 28.83
- This is not a legal survey. Boundary information was derived from draft Surveyor's Property Report by Anis O'Sullivan, Yorkville Ltd. on April 11th, 2019. Registered plan 43365
- Contractor is responsible for location and protection of utilities.
- All dimensions to be verified on site by contractor prior to construction.
- Any changes made to this plan must be verified and approved by Kollaard Associates Inc.
- Client is responsible for acquiring all necessary permits. This drawing is not for construction until a building permit has been granted.
- The owner agrees to prepare and implement an erosion and sediment control plan to the satisfaction of the City of Ottawa, appropriate to the site conditions, prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and during all phases of site preparation and construction in accordance with the current Best Management Practices for Erosion and Sediment Control such as, but not limited to installing filter cloth across monolith/catchbasin lids to prevent sediments from entering structures and install and maintain a light duty silt fence barrier as required.
- All materials and construction to be in accordance with City of Ottawa standards and Ontario Provincial Standards and Specifications; sewer and watermain material types; installation, provide minimum 2.4 metres of cover for water services, cathodic protection, City of Ottawa installation specifications for watermain, pipe bedding, reinstatement of disturbed areas and leakage testing.
- Reference to Kollaard File No. 190502 for Servicing brief.

No.	REVISION	DATE	BY
1	RESPONSE TO REVIEW COMMENTS	01/21/2020	AVB
2	SERVICING BRIEF SUBMISSION	10/07/2019	AVB
3	REVISION PER / DESCRIPTION	REV. DATE	INT.

Kollaard Associates Engineers
 (613) 860-0923
 info@kollaard.co

P.O. BOX 189, 210 SCOTT ST.
 KEMPTVILLE, ONTARIO
 K0G 1J0 FAX (613) 258-0475
 http://www.kollaard.co

CLIENT: SMART LIVING RESIDENCES

PROJECT: PROPOSED RESIDENTIAL RENOVATION

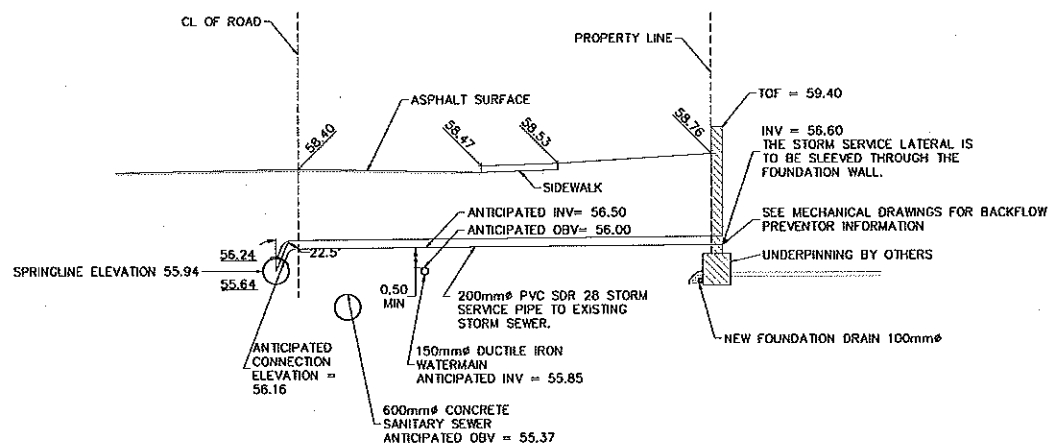
LOCATION: 253, 255, 257 YORK STREET, OTTAWA, ONTARIO

DESIGNED BY:	CHECKED BY:
AVB	WK
DRAWN BY:	APPROVED BY:
AVB	WK

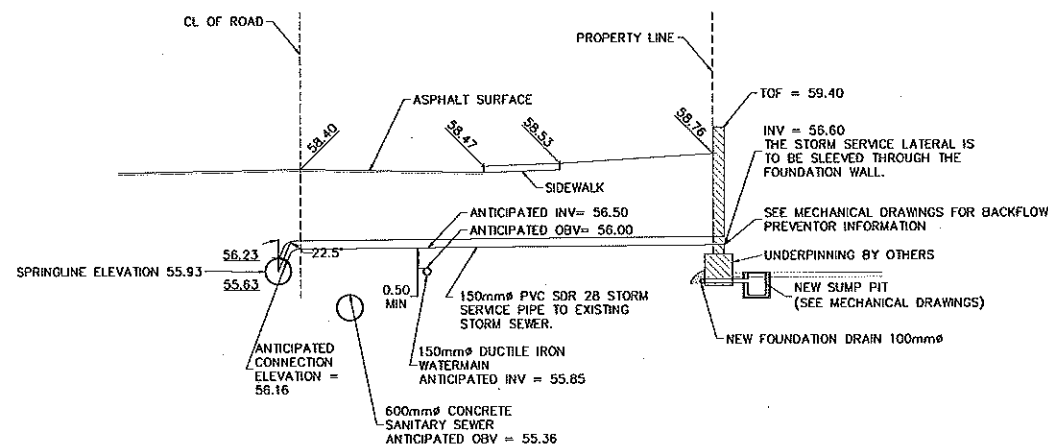
DATE: JULY 10, 2019
 ALLIANCE FILE NUMBER: 190502

DRAWING NUMBER: 1 OF 2
DRAWING NAME: PROPOSED SERVICING PLAN

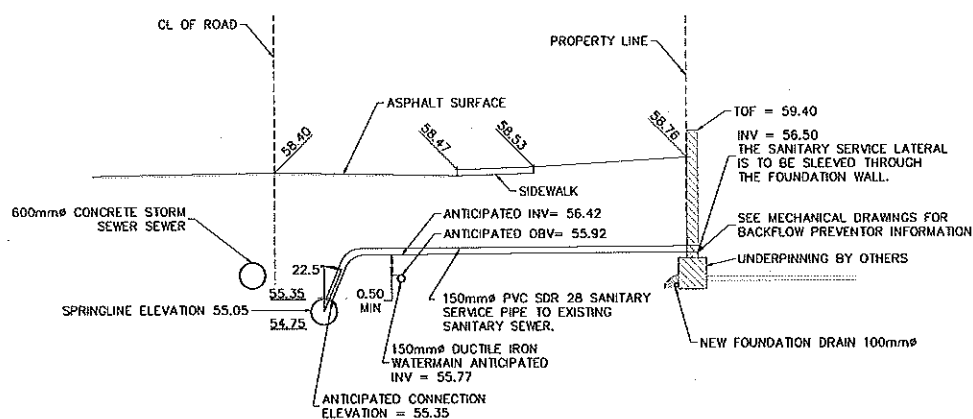
© COPYRIGHT 2019 KOLLAARD ASSOCIATES INCORPORATED



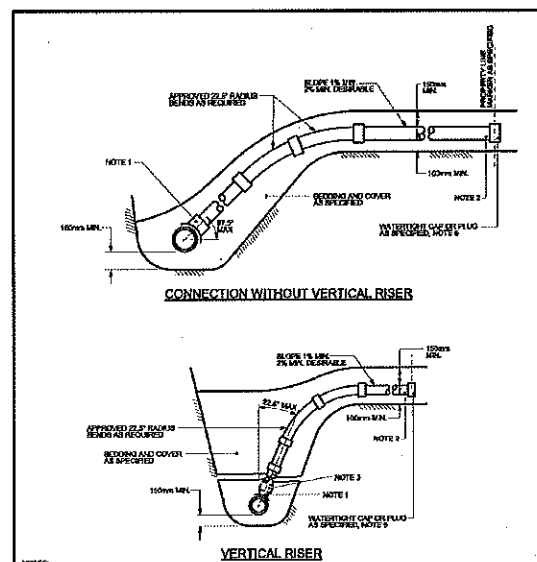
STORM CONNECTION DETAIL (ROOF DRAINAGE)
(NOT TO SCALE)



STORM CONNECTION DETAIL (FOUNDATION DRAIN)
(NOT TO SCALE)



SANITARY CONNECTION DETAIL
(NOT TO SCALE)



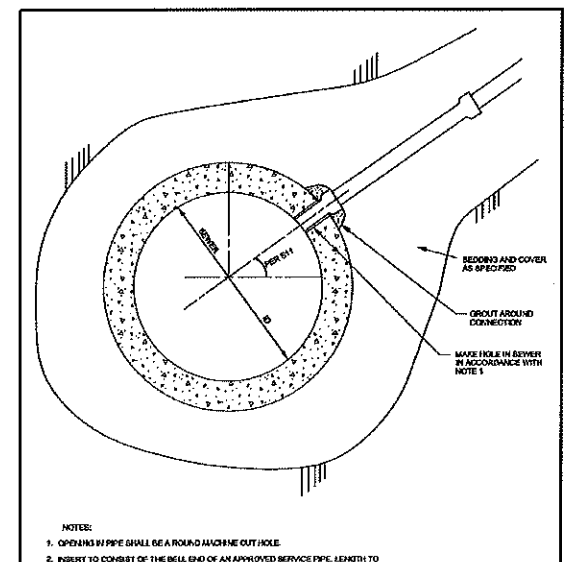
SEWER SERVICE CONNECTIONS FOR RIGID MAIN SEWER PIPE (MODIFIED OPSD-1008.010)

DATE: MARCH 2011
 REVISED: MARCH 2011
 DWG NO.: 811

NOTES:

1. ALL DIMENSIONS OF SERVICE CONNECTIONS SHALL BE IN METRES UNLESS OTHERWISE SPECIFIED.
2. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
3. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
4. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
5. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
6. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
7. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.
8. APPROVED OUT-OF-TOOL MUST BE USED FOR FIELD MADE CONNECTIONS.

N.T.S.



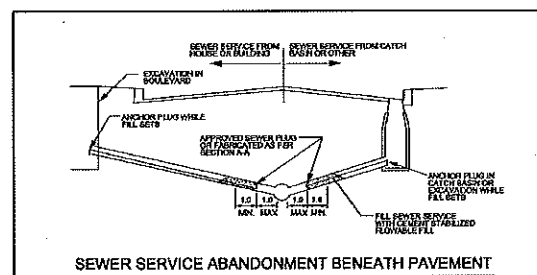
SEWER SERVICE CONNECTIONS FOR RIGID MAIN SEWER PIPE USING BELL END INSERT METHOD

DATE: MARCH 2011
 REVISED: MARCH 2011
 DWG NO.: 812

NOTES:

1. OPENINGS IN PIPE SHALL BE A ROUND MACHINE CUT HOLE.
2. INSERT TO CONSIST OF THE BELL END OF AN APPROVED SERVICE PIPE. LENGTH TO BE LIMITED SUCH THAT NO PART OF PREVIOUS SERVICE PIPE IS WITHIN THE OPENING OF THE PIPE.
3. APPROVED WATER TIGHT SEALANT SHALL BE USED TO SEAL THE JOINT BETWEEN THE BELL END PIPE TO THE SERVICE LINE. SEALANT SHALL BE APPLIED TO INSURE A TIGHT WATER TIGHT CONNECTION THAT WILL HOLD THE BELL IN POSITION.
4. GROUT AROUND THE BELL END TO PROTECT THE SEALANT AND SUPPORT THE BELL.
5. ALL OTHER NOTES OF 811 APPLY.
6. SEE 812.1 FOR APPROVED SEALANT PRODUCTS.

N.T.S.



SEWER SERVICE ABANDONMENT BENEATH PAVEMENT

DATE: MARCH 2011
 REVISED: MARCH 2011
 DWG NO.: 815A

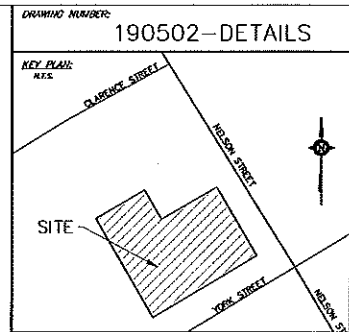
SECTION A-A FABRICATED SEWER PLUG

FRONT VIEW

NOTES:

1. ALL DIMENSIONS IN MILLIMETERS UNLESS OTHERWISE SHOWN.

N.T.S.



GENERAL NOTES:

1. All dimensions are in metres; all elevations are in metres and are geodetic. BM = North/West corner of hydro transformer concrete pad. Elevation = 58.98
2. This is not a legal survey. Boundary information was derived from draft Surveyor's Property Report by Anis O'Sullivan Volzsek Ltd. on April 11th, 2010. Registered plan #3585
3. Contractor is responsible for location and protection of utilities.
4. All dimensions to be verified on site by contractor prior to construction.
5. Any changes made to this plan must be verified and approved by Kollaard Associates Inc.
6. Client is responsible for obtaining all necessary permits. This drawing is not for construction until a building permit has been granted.
7. The owner agrees to prepare and implement an erosion and sediment control plan to the satisfaction of the City of Ottawa, appropriate to the site conditions, prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and during all phases of site preparation and construction in accordance with the current Best Management Practices for Erosion and Sediment Control such as, but not limited to installing filter cloths across roads/ditches, etc. to prevent sediments from entering structures and install and maintain a tight duty silt fence barrier as required.
8. All materials and construction to be in accordance with City of Ottawa standards and Ontario Provincial Standards and Specifications: sewer and watermain material types; installation, provide minimum 2.4 metres of cover for water services, cathodic protection, City of Ottawa insulation specifications for watermain, pipe bedding, reinstatement of disturbed areas and leakage testing.
9. Reference to Kollaard File No. 190502 for Servicing brief.

No.	REVISION	DATE	BY
1	RESPONSE TO REVIEW COMMENTS	01/31/2010	AVB
2	SERVICING BRIEF SUBMISSION	10/07/2010	AVB
3	REVISION ITEM / DESCRIPTION	REV. DATE	INT.

Kollaard Associates Engineers
 (613) 860-0923
 P.O. BOX 189, 210 PRESCOTT ST.
 KEMPVILLE, ONTARIO
 K0G 1J0 FAX (613) 258-0475
 http://www.kollaard.ca

SMART LIVING RESIDENCES

PROJECT:
PROPOSED RESIDENTIAL RENOVATION

LOCATION:
**253, 255, 257 YORK STREET
 OTTAWA, ONTARIO**

DESIGNED BY AVB	CHECKED BY WK
DRAWN BY AVB	APPROVED BY WK
DATE: JULY 10, 2019	
KOLLAARD FILE NUMBER: 190502	

DRAWING NUMBER:
2 OF 2

DRAWING NAME:
PROPOSED DETAILS PLAN

253-255-257 YORK ST. & 78-80 NELSON ST.
COMBINED
 PROPOSED ADDITION AND RENOVATION

CONSULTANTS: SOMA PRO DESIGNS
 STRUCTURAL -
 MECHANICAL -
 ELECTRICAL -
 GRADING -
 MDV

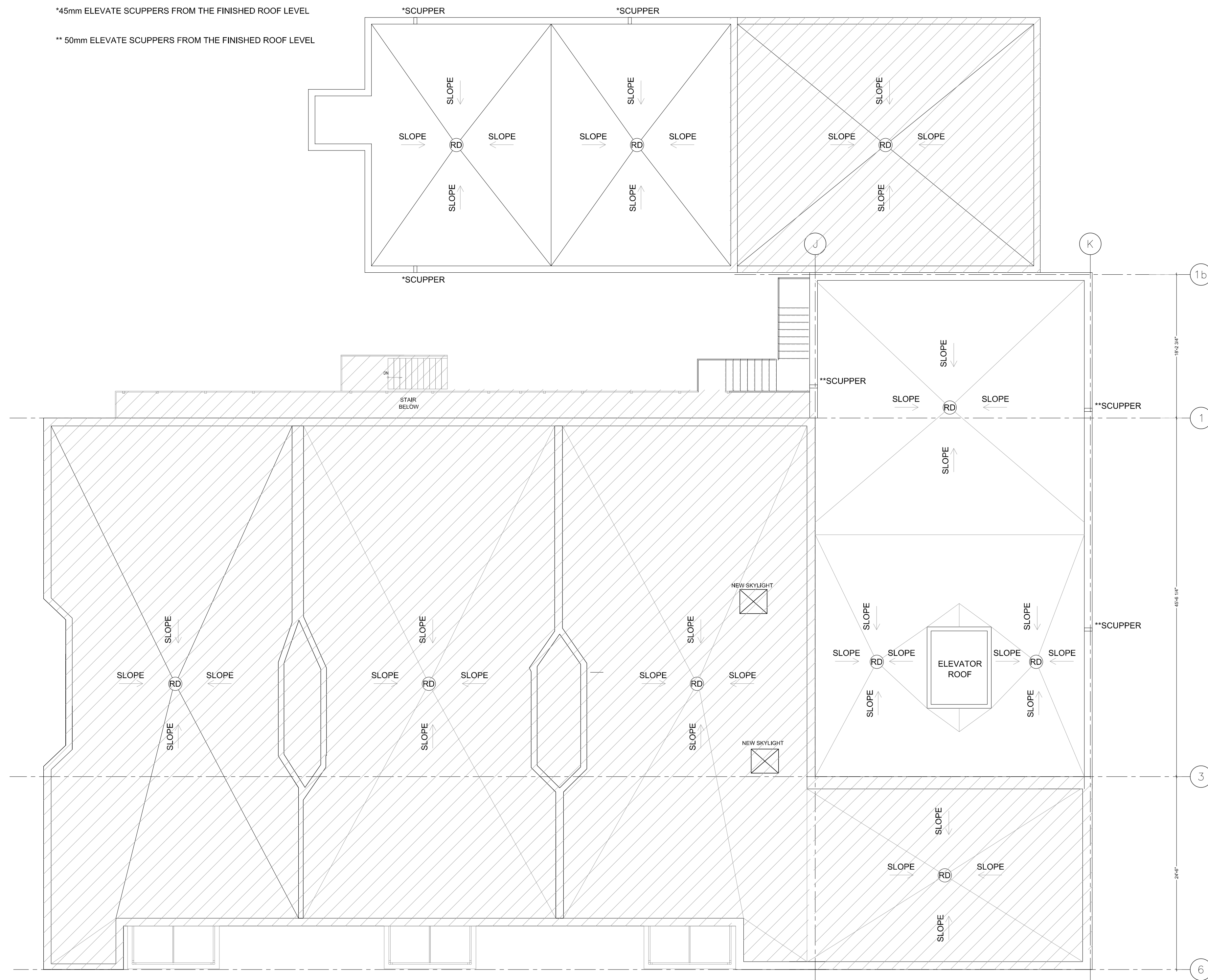
9		
8		
7		
6		
5		
4		
3		
2		
1		
NO.	REVISION/ISSUE	DATE

PROJECT:
 253-255-257 YORK ST. & 78-80 NELSON ST.
 OTTAWA, ONTARIO
 K1N 6T9

DRAWING NAME:
 EXISTING & PROPOSED ROOF PLAN

DRAWN BY: L.T. SHEET:
 DATE: MAY, 2021
 SCALE: AS NOTED **A9**

*45mm ELEVATE SCUPPERS FROM THE FINISHED ROOF LEVEL
 **50mm ELEVATE SCUPPERS FROM THE FINISHED ROOF LEVEL



1
A9 EXISTING AND PROPOSED ROOF PLAN
 SCALE: 1/8"=1'-0"

Project: COO-22-0938
Address: 253 York Street

Re: List of Referenced City Standard Detail Drawings

R10	Standard Trench Reinstatement in Paved Surface
S14	Foundation Drain Backwater Valve Installation
S24.1	Standard Circular Maintenance Hole Cover
S25	Standard Circular Frame for Maintenance Holes
S30	Catch Basin – 'T' For Rear Yard, Ditched Pipe and Landscaping Applications
S31	Catch Basin – Elbow For Rear Yard, Ditched Pipe and Landscaping Applications
SC2	Monolithic Concrete Curb and Sidewalk
SC27	Permeable Paving

Alison Gosling

From: Eric Lalande <eric.lalande@rvca.ca>
Sent: March 10, 2022 2:14 PM
To: Francis Valenti
Cc: Alison Gosling
Subject: RE: 22-0938 - 78-80 Nelson & 253-259 York Street - Quality Control Requirement

Follow Up Flag: Follow up
Flag Status: Completed

Hi Francis,

Thanks for the site plan, based on the site plan provided the RVCA will not require any additional water quality control requirements, however encourage best management practices, such as outlined in your email.

Thank you,

Eric Lalande, MCIP, RPP
Planner, RVCA
613-692-3571 x1137

From: Francis Valenti <F.Valenti@McIntoshPerry.com>
Sent: Thursday, March 10, 2022 2:02 PM
To: Eric Lalande <eric.lalande@rvca.ca>
Cc: Alison Gosling <a.gosling@mcintoshperry.com>
Subject: RE: 22-0938 - 78-80 Nelson & 253-259 York Street - Quality Control Requirement

Hi Eric,

Please find attached the site plan for your review. No surface parking is proposed. Permeable paving walkways and landscaping is proposed around the building additions. If you have any other questions please let me know.

Regards,

Francis Valenti, EIT
Engineering Intern, Land Development
T. 613.714.6895 | C. 613.808.2123
F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality

From: Eric Lalande <eric.lalande@rvca.ca>
Sent: March 10, 2022 1:50 PM
To: Francis Valenti <F.Valenti@McIntoshPerry.com>
Cc: Alison Gosling <a.gosling@mcintoshperry.com>
Subject: RE: 22-0938 - 78-80 Nelson & 253-259 York Street - Quality Control Requirement

Hi Francis,

Do you have a site plan I can look at?

I'm interested in how much surface parking will be proposed?

Thanks,

Eric Lalande, MCIP, RPP

Planner, RVCA

613-692-3571 x1137

From: Francis Valenti <F.Valenti@McIntoshPerry.com>

Sent: Thursday, March 10, 2022 1:41 PM

To: Eric Lalande <eric.lalande@rvca.ca>

Cc: Alison Gosling <a.gosling@mcintoshperry.com>

Subject: 22-0938 - 78-80 Nelson & 253-259 York Street - Quality Control Requirement

Good afternoon,

We wanted to touch base with you regarding the development at 78-80 Nelson Street and 253-259 York Street.

The development will consist of a three story addition to the existing residential building at 78-80 Nelson Street, and a 4-storey addition to the existing residential building at 253-259 York Street. Water from the existing buildings is currently collected and conveyed to the 300mm diameter concrete storm sewer with Nelson Street, and the 600mm diameter concrete storm sewer within York Street, respectively. Stormwater flow from both buildings then enters the 1800mm diameter storm sewer within King Edward, where it travels approximately 1.5km before discharging into the Ottawa River at Outlet#04452. Water from the proposed buildings will maintain existing drainage paths through the existing buildings. Surface water will be collected on site and conveyed to the 300mm diameter concrete storm sewer within Nelson Street, where it will also travel approximately 1.5km before discharging into the Ottawa River at Outlet#04452. A drainage figure is attached showing the storm drainage path for the proposed development.

Due to the distance to the outlet, it is not anticipated that quality controls will be required for the proposed development. Can you please review and confirm?

Thanks,

Francis Valenti, EIT

Engineering Intern, Land Development

T. 613.714.6895 | C. 613.808.2123

F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality

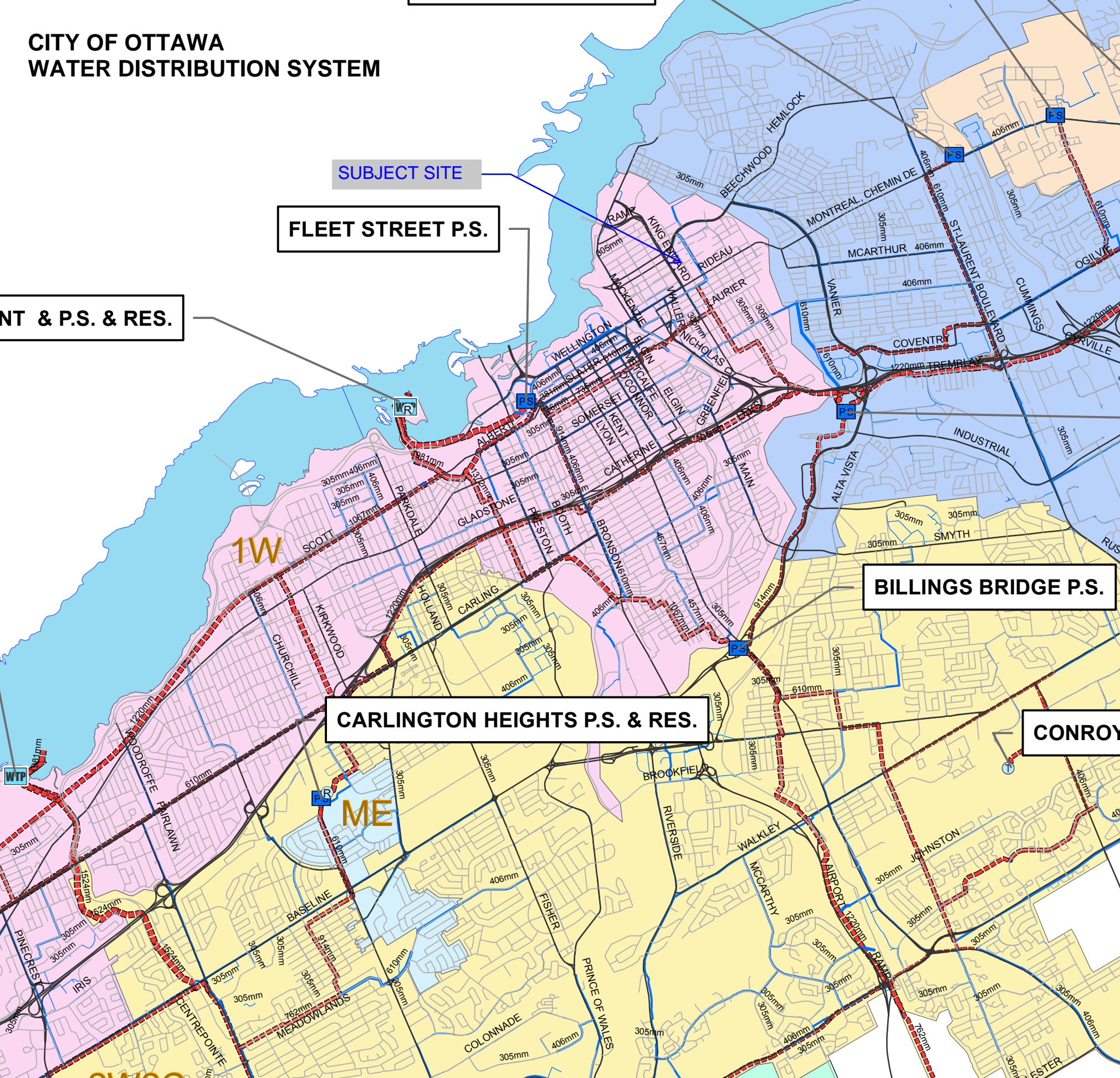


Platinum member

Confidentiality Notice – If this email wasn't intended for you, please return or delete it. Click [here](#) to read all of the legal language around this concept.

APPENDIX C
WATERMAIN CALCULATIONS

CITY OF OTTAWA WATER DISTRIBUTION SYSTEM



SUBJECT SITE

FLEET STREET P.S.

NT & P.S. & RES.

BILLINGS BRIDGE P.S.

CARLINGTON HEIGHTS P.S. & RES.

CONROY

McINTOSH PERRY

CO-22-0938 - 78 & 80 Nelson Street Addition (B1) - Water Demands

Project:	78 & 80 Nelson Street Addition (B1)	
Project No.:	CO-22-0938	
Designed By:	AJG	
Checked By:	RDF	
Date:	August 23, 2021	
Site Area:	0.154 gross ha	
Residential	26 Bachelor	1.4 Persons per unit
Population	36	

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	280	L/c/d	0.12
Industrial - Light	35,000	L/gross ha/d	0
Industrial - Heavy	55,000	L/gross ha/d	0
Shopping Centres	2,500	L/(1000m ² /d)	0
Motels	150	L/(bed-space/d)	0
Hotels	225	L/(bed-space/d)	0
Tourist Commercial	28,000	L/gross ha/d	0
Other Commercial	28,000	L/gross ha/d	0
AVERAGE DAILY DEMAND	0.12	L/s	
	7.08	L/min	

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	4.9 x avg. day	L/c/d	0.58
Industrial	1.5 x avg. day	L/gross ha/d	
Commercial	1.5 x avg. day	L/gross ha/d	
Institutional	1.5 x avg. day	L/gross ha/d	
MAXIMUM DAILY DEMAND	0.58	L/s	
	34.68	L/min	

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	7.4 x avg. day	L/c/d	0.87
Industrial	1.8 x max. day	L/gross ha/d	
Commercial	1.8 x max. day	L/gross ha/d	
Institutional	1.8 x max. day	L/gross ha/d	
MAXIMUM HOUR DEMAND	0.87	L/s	
	52.38	L/min	

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

McINTOSH PERRY

CO-22-0938 - 78 & 80 Nelson Street Addition (B1) - Water Demands

Project:	78 & 80 Nelson Street Addition (B1)	
Project No.:	CO-22-0938	
Designed By:	AJG	
Checked By:	RDF	
Date:	August 23, 2021	
Site Area:	0.154 gross ha	
Residential	12 Bachelor	1.4 Persons per unit
Population	17 *53 persons total	

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	280	L/c/d	0.05
Industrial - Light	35,000	L/gross ha/d	0
Industrial - Heavy	55,000	L/gross ha/d	0
Shopping Centres	2,500	L/(1000m ² /d)	0
Motels	150	L/(bed-space/d)	0
Hotels	225	L/(bed-space/d)	0
Tourist Commercial	28,000	L/gross ha/d	0
Other Commercial	28,000	L/gross ha/d	0
AVERAGE DAILY DEMAND	0.05	L/s	
	3.27	L/min	

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential (Peaking Factor based on total population)	4.9 x avg. day	L/c/d	0.27
Industrial	1.5 x avg. day	L/gross ha/d	
Commercial	1.5 x avg. day	L/gross ha/d	
Institutional	1.5 x avg. day	L/gross ha/d	
MAXIMUM DAILY DEMAND	0.27	L/s	
	16.01	L/min	

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential (Peaking Factor based on total population)	7.4 x avg. day	L/c/d	0.40
Industrial	1.8 x max. day	L/gross ha/d	
Commercial	1.8 x max. day	L/gross ha/d	
Institutional	1.8 x max. day	L/gross ha/d	
MAXIMUM HOUR DEMAND	0.40	L/s	
	24.17	L/min	

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

McINTOSH PERRY

CO-22-0938 - 253 York Street Addition (B2) - Water Demands

Project:	253 York Street Addition (B2)	
Project No.:	CO-22-0938	
Designed By:	AJG	
Checked By:	RDF	
Date:	August 23, 2021	
Site Area:	0.154 gross ha	
Residential	20 Bachelor/1 Bedroom	1.4 Persons per unit
Population	28	

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	280	L/c/d	0.09
Industrial - Light	35,000	L/gross ha/d	0
Industrial - Heavy	55,000	L/gross ha/d	0
Shopping Centres	2,500	L/(1000m ² /d)	0
Motels	150	L/(bed-space/d)	0
Hotels	225	L/(bed-space/d)	0
Tourist Commercial	28,000	L/gross ha/d	0
Other Commercial	28,000	L/gross ha/d	0
AVERAGE DAILY DEMAND	0.09	L/s	
	5.44	L/min	

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	9.5 x avg. day	L/c/d	0.86
Industrial	1.5 x avg. day	L/gross ha/d	
Commercial	1.5 x avg. day	L/gross ha/d	
Institutional	1.5 x avg. day	L/gross ha/d	
MAXIMUM DAILY DEMAND	0.86	L/s	
	51.72	L/min	

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS	DEMANDS
Residential	14.3 x avg. day	L/c/d	1.30
Industrial	1.8 x max. day	L/gross ha/d	
Commercial	1.8 x max. day	L/gross ha/d	
Institutional	1.8 x max. day	L/gross ha/d	
MAXIMUM HOUR DEMAND	1.30	L/s	
	77.86	L/min	

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

McINTOSH PERRY

CCO-22-0938 - 78 & 80 Nelson Addition - Fire Underwriters Survey (FUS) Fire Calculations

Project:	78 & 80 Nelson (B1)
Project No.:	CO-22-0938
Designed By:	AJG
Checked By:	RDF
Date:	August 23, 2021

From the Fire Underwriters Survey (1999)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:
Updated per City of Ottawa Technical Bulletin ISTB-2018-02

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

F = 220 x C x √A Where:

F = Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type **Wood Frame**

C 1.5

A 672.9 m²

*Note: Area is the proposed addition + the existing building.

Calculated Fire Flow	8560.3 L/min
	9000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From note 2, Page 18 of the Fire Underwriter Survey:

Combustible 0%

Fire Flow	9000.0 L/min
-----------	--------------

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered -40%

Reduction	-3600 L/min
-----------	-------------

D. INCREASE FOR EXPOSURE (No Rounding)

	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor	
Exposure 1	3.1 to 10	Ordinary (Unprotected)	5	2	10	18%
Exposure 2	<45	Ordinary (Unprotected)	30	2	60	18%
Exposure 3	0 to 3	Wood frame	10	4	40	10%
Exposure 4	10.1 to 20	Wood frame	8	2	16	14%
					% Increase*	60%

Increase*	5400.0 L/min
-----------	--------------

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	10800.0 L/min
Fire Flow Required**	11000.0 L/min

*In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

**In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

CCO-22-0938 - 253 York Street Addition (B2) - Fire Underwriters Survey (FUS) Fire Calculations

Project:	253 York Street Addition (B2)
Project No.:	CO-22-0938
Designed By:	AJG
Checked By:	RDF
Date:	August 23, 2021

From the Fire Underwriters Survey (1999)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:
Updated per City of Ottawa Technical Bulletin ISTB-2018-02

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

F = 220 x C x √A Where:

F = Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type Wood Frame

C 1.5 A 2,824.7 m²

*Note: Area is the proposed addition + the existing building.

Calculated Fire Flow	17538.8 L/min
	18000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From note 2, Page 18 of the Fire Underwriter Survey:
Combustible 0%

Fire Flow	18000.0 L/min
-----------	---------------

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered -40%

Reduction	-7200 L/min
-----------	-------------

D. INCREASE FOR EXPOSURE (No Rounding)

	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor	
Exposure 1	0 to 3	Wood frame	28	3	85	18%
Exposure 2	30.1 to 45	Wood frame	12	2	24	18%
Exposure 3	10.1 to 20	Ordinary (Unprotected)	17	3	51	10%
Exposure 4	0 to 3	Wood frame	8	2	16	14%
% Increase*						60%

Increase*	10800.0 L/min
-----------	---------------

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	21600.0 L/min
Fire Flow Required**	22000.0 L/min

*In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

**In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

McINTOSH PERRY

CO-22-0938 - 78 & 80 Nelson Street - OBC Fire Calculations

Project:	78 80 Nelson Street
Project No.:	CCO-22-0938
Designed By:	AJG
Checked By:	AJG
Date:	August 26, 2021

Ontario 2006 Building Code Compendium (Div. B - Part 3)

Water Supply for Fire-Fighting - Residential

Building is classified as Group : C (from table 3.2.2.55)

Building is of combustible construction. Floor assemblies are fire separations but with no fire-resistance ratings. Roof assemblies, mezzanies, loadbearing walls, columns and arches do not have a fire-resistance rating.

From Div. B A-3.2.5.7. of the Ontario Building Code - 3. Building On-Site Water Supply:

(a) $Q = K \times V \times Stot$

where:

Q = minimum supply of water in litres

K = water supply coefficient from Table 1

V = total building volume in cubic metres

Stot = total of spatial coefficient values from the property line exposures on all sides as obtained from the formula:

$Stot = 1.0 + [S_{side1} + S_{side2} + S_{side3} + \dots \text{etc.}]$

K	23	(from Table 1 pg A-31) (Worst case occupancy {C} 'K' value used)
V	1,641	(Total building volume in m ³ .)
Stot	2.0	(From figure 1 pg A-32)
Q =	75,476.77 L	

From
Figure 1
(A-32)

Snorth	1.50	m	0.5
Seast	2.00	m	0.5
Ssouth	26.80	m	0.0
Swest	10.00	m	0.0

*approximate distances

From Table 2: Required Minimum Water Supply Flow Rate (L/s)

2700 L/min if Q < 108,000 L
713 gpm

McINTOSH PERRY

CO-22-0938 - 253 York Street - OBC Fire Calculations

Project:	253 York Street
Project No.:	CO-22-0938
Designed By:	AJG
Checked By:	AJG
Date:	August 23, 2021

Ontario 2006 Building Code Compendium (Div. B - Part 3)

Water Supply for Fire-Fighting - Residential

Building is classified as Group : C (from table 3.2.2.55)

Building is of combustible construction. Floor assemblies are fire separations but with no fire-resistance ratings. Roof assemblies, mezzanies, loadbearing walls, columns and arches do not have a fire-resistance rating.

From Div. B A-3.2.5.7. of the Ontario Building Code - 3. Building On-Site Water Supply:

(a) $Q = K \times V \times Stot$

where:

Q = minimum supply of water in litres

K = water supply coefficient from Table 1

V = total building volume in cubic metres

Stot = total of spatial coefficient values from the property line exposures on all sides as obtained from the formula:

$Stot = 1.0 + [S_{side1} + S_{side2} + S_{side3} + \dots \text{etc.}]$

K	23	(from Table 1 pg A-31) (Worst case occupancy {C} 'K' value used)
V	6,888	(Total building volume in m ³ .)
Stot	2.0	(From figure 1 pg A-32)
Q =	316,836.43 L	

From
Figure 1
(A-32)

Snorth	13.40	m	0.0
Seast	0.00	m	0.5
Ssouth	0.00	m	0.5
Swest	0.00	m	0.5

*approximate distances

From Table 2: Required Minimum Water Supply Flow Rate (L/s)

9000 L/min if Q > 270,000 L
2378 gpm

McINTOSH PERRY

CO-22-0938 - 253 York Street Addition, 78 & 80 Nelson Street Addition - Boundary Condition Unit Conv

Project: 253 York Street Addition, 78 & 80 Nelson Street Addition

Project No.: CO-22-0938

Designed By: AJG

Checked By: RDF

Date: March 10, 2022

Boundary Conditions Unit Conversion

YORK STREET

Scenario	Height (m)	Elevation (m)	m H ₂ O	PSI	kPa
Avg. DD	115.4	58.1	57.3	81.5	562.0
Fire Flow (60 L/s OBC)	106.9	58.1	48.8	69.4	478.6
Peak Hour	106.2	58.1	48.1	68.4	471.8



4 WATERMAIN DESIGN

When underpinning was completed on the building, part of the existing water lateral was uncovered. The construction manager on site commented that the pipe was of ductile iron material and was in sufficient condition. Correspondence is included in the appendix. The existing 100mm diameter water lateral is to remain.

Fire flow protection requirements were calculated as per the Fire Underwriter's Survey (FUS) Calculations of the fire flow required for the building are provided in Appendix B.

From Appendix B, the maximum fire flow requirement is 14875 L/min or 248 L/sec.

4.1 Water Demand

The water demand for the proposed development was calculated based on the City of Ottawa Water Distribution Design Guidelines (as amended) as follows:

Residential

Total domestic pop:

One Bedroom units	(14) x 1.4 ppu:	19.6
Two Bedroom units	(14) x 2.1 ppu:	29.4
Three Bedroom	(1) x 3.1 ppu	3.1
Four Bedroom	(8) x 4.2 ppu	33.6
Total:		85.7

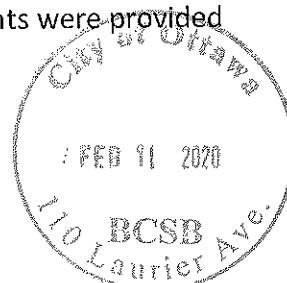
$$Q_{\text{Domestic}} = 86 \times 350 \text{ L/person/day} \times (1/86,400 \text{ sec/day}) = 0.35 \text{ L/sec}$$

- Average daily demand 0.35 L/s
- Maximum daily demand (factor of 2.5) is 0.35 L/s x 2.5 = 0.87 L/s
- Peak hourly demand (factor of 2.2) = 0.87 L/s x 2.2= 1.92 L/s

4.2 Boundary Conditions

The water demand due to occupancy together with the fire flow requirements were provided to the City of Ottawa in 2019 as follows:

- Average daily water demand 0.4 L/s
- Maximum daily water demand 1.0 L/s
- Peak hourly water demand 2.2 L/s





- Fire Flow required 273 L/s

It is assumed that the water services will be connected to the 150 mm diameter ductile iron (DI) water main along York Street.

The following are the boundary conditions, HGL, for hydraulic analysis that were provided in 2019 for the above indicated peak hourly demand and fire flow demand.

Minimum HGL = 106.5m

Maximum HGL = 115.0m

The actual water demand and sanitary was modified from what was submitted for boundary conditions, as the unit count and floor areas were verified. The average daily water demand decreased by 0.06 L/s. It is expected that this minor decrease will have negligible impact on the minimum HGL.

The City of Ottawa Design Guidelines – Water Distribution as amended by technical bulletin ISDTB-2014-02 indicates that if possible water distribution systems are to be designed to provide residual pressures of 345 to 552 kPa in all occupied areas outside of the public right-of-way.

In accordance with MOE Guidelines, the distribution system shall be sized so that under maximum hourly demand conditions the pressures are no less than 276 kPa (40 psi.)

The pressure loss to the third floor of the proposed building was calculated using Bernoulli's Equation in Combination with the Darcy – Weisbach Equation and the Colebrook Equation. The equations are shown below.

$$H_P + Z_1 - Z_2 + \frac{P_1 - P_2}{\rho g} + \frac{V_1^2 - V_2^2}{2g} = h_f + h_m \quad \text{where:}$$

$$h_m = K_m \frac{V^2}{2g} \quad Re = \frac{VD}{\nu} \quad Q = VA \quad A = \frac{\pi}{4} D^2$$

$$\text{Darcy - Weisbach Equation: } h_f = f \frac{L}{D} \frac{V^2}{2g} \quad \text{where:}$$

$$\text{If laminar flow } \left(Re < 4000 \text{ and any } \frac{e}{D} \right), \quad f = \frac{64}{Re}$$

$$\text{If turbulent flow } \left(4000 \leq Re \leq 10^8 \text{ and } 0 \leq \frac{e}{D} < 0.05 \right), \text{ then}$$

$$\text{Colebrook Equation: } \frac{1}{\sqrt{f}} = -2.0 \log \left(\frac{e/D}{37} + \frac{251}{Re \sqrt{f}} \right)$$



Alison Gosling

From: Bakhit, Reza <reza.bakhit@ottawa.ca>
Sent: September 1, 2021 10:36 AM
To: Alison Gosling
Subject: RE: 22-0938 253 York, 78 Nelson, 80 Nelson - Boundary Condition Request
Attachments: 253 York Street August 2021.pdf

Follow Up Flag: Follow up
Flag Status: Flagged

Good morning Alison,

The following are boundary conditions, HGL, for hydraulic analysis at 253 York, 78 Nelson and 80 Nelson Street (zone 1W) assumed connected to the 153 mm watermain on York Street and the 203 mm watermain on Nelson Street (see attached PDF for location).

Both Connections

Minimum HGL: 106.2 m

Maximum HGL: 115.4 m

York Street

Max Day + FF (60 L/s): 106.9 m

Max Day + FF (366.7 L/s): N/A

Nelson Street

Max Day + FF (45 L/s): 108.2 m

Max Day + FF (183.3 L/s): 104.2 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Kind regards,

Reza Bakhit, P.Eng, C.E.T

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - Central Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2400 ext./poste 19346, reza.bakhit@ottawa.ca

Please note: Given the current pandemic, I will be working from home until further notice; reaching me by email is the easiest. I will be checking my voicemail, just not as frequently as I normally would be.

From: Alison Gosling <a.gosling@mcintoshperry.com>
Sent: Thursday, August 26, 2021 8:56 AM
To: Bakhit, Reza <reza.bakhit@ottawa.ca>
Subject: RE: 22-0938 253 York, 78 Nelson, 80 Nelson - Boundary Condition Request

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good morning Reza,

OBC calculations for the building additions are attached and summarized below.

Nelson St Addition – 2,700 L/min (45 L/s)
York St Addition – 3,600 L/min (60 L/s)

Thank you,

Alison Gosling, P.Eng.

Project Engineer, Land Development
115 Walgreen Road, Carp, ON, K0A 1L0
T. 613.714.4629
a.gosling@mcintoshperry.com | www.mcintoshperry.com

McINTOSH PERRY

From: Bakhit, Reza <reza.bakhit@ottawa.ca>
Sent: August 25, 2021 10:32 AM
To: Alison Gosling <a.gosling@mcintoshperry.com>
Subject: RE: 22-0938 253 York, 78 Nelson, 80 Nelson - Boundary Condition Request

Good morning Alison,

Thanks for the info and the documents.
Could you please include the fire flow based on the OBC as well?

Appreciated,

Reza Bakhit, P.Eng, C.E.T

Project Manager
Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique
Development Review - Central Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1
613.580.2400 ext./poste 19346, reza.bakhit@ottawa.ca

Please note: Given the current pandemic, I will be working from home until further notice; reaching me by email is the easiest. I will be checking my voicemail, just not as frequently as I normally would be.

From: Alison Gosling <a.gosling@mcintoshperry.com>
Sent: Tuesday, August 24, 2021 4:47 PM
To: Bakhit, Reza <reza.bakhit@ottawa.ca>
Subject: 22-0938 253 York, 78 Nelson, 80 Nelson - Boundary Condition Request

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good afternoon Reza,

We would like to request Boundary Conditions for the proposed site located at 253 York St, 78 Nelson St, and 80 Nelson St. The proposed development consists of a 3-storey building addition with 26 units and a 4-storey building addition with 20 units. The proposed connections will be to the existing 152mm dia. watermain within York Street and to the 203mm dia. watermain within Nelson St.

York Street:

- The estimated fire flow is 22,000 L/min based on the FUS
- Average daily demand: 0.09 L/s OR 0.49 L/s with the existing building
- Maximum daily demand: 0.86 L/s OR 1.86 L/s with the existing building
- Maximum hourly daily demand: 1.30 L/s OR 2.50 L/s with the existing building

Nelson Street:

- The estimated fire flow is 11,000 L/min based on the FUS
- Average daily demand: 0.12 L/s
- Maximum daily demand: 0.58 L/s
- Maximum hourly daily demand: 0.93 L/s

Attached is a map showing the proposed connection location along with the calculations prepared for the demands listed above.

Please let me know if you have any questions.

Thank you,

Alison Gosling, P.Eng.

Project Engineer, Land Development
115 Walgreen Road, Carp, ON, K0A 1L0
T. 613.714.4629

a.gosling@mcintoshperry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality



Platinum member

Confidentiality Notice – If this email wasn't intended for you, please return or delete it. Click [here](#) to read all of the legal language around this concept.

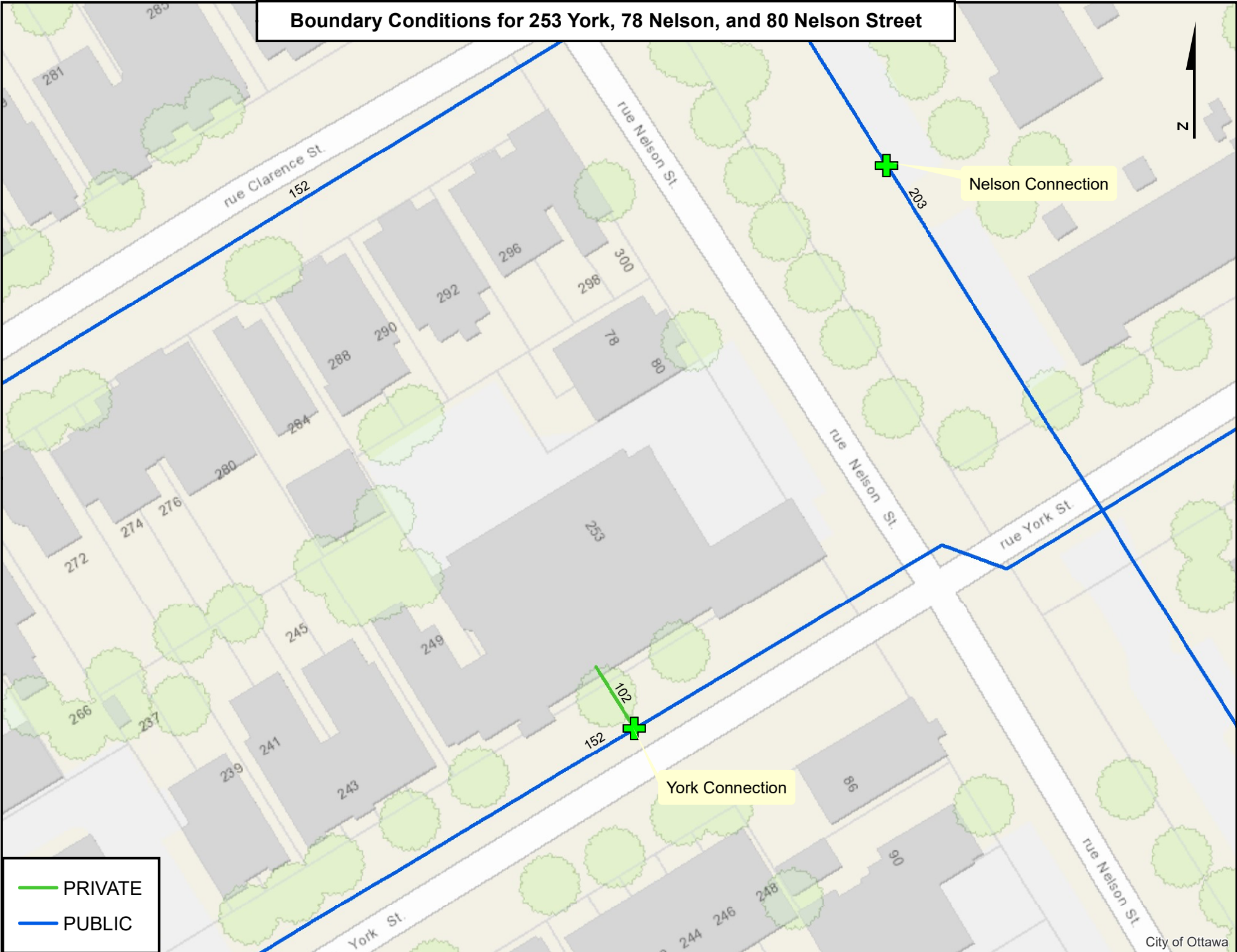
This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

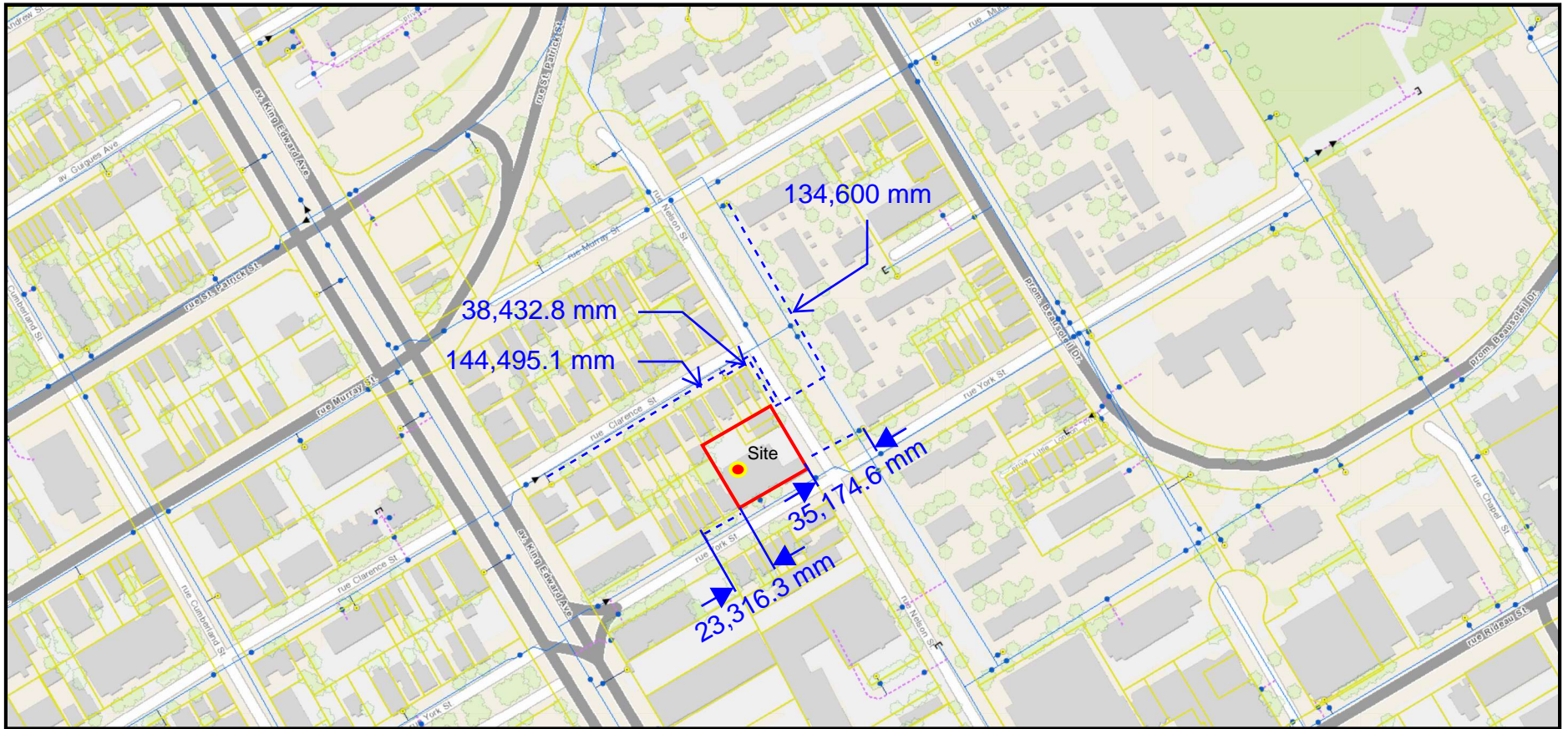
This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

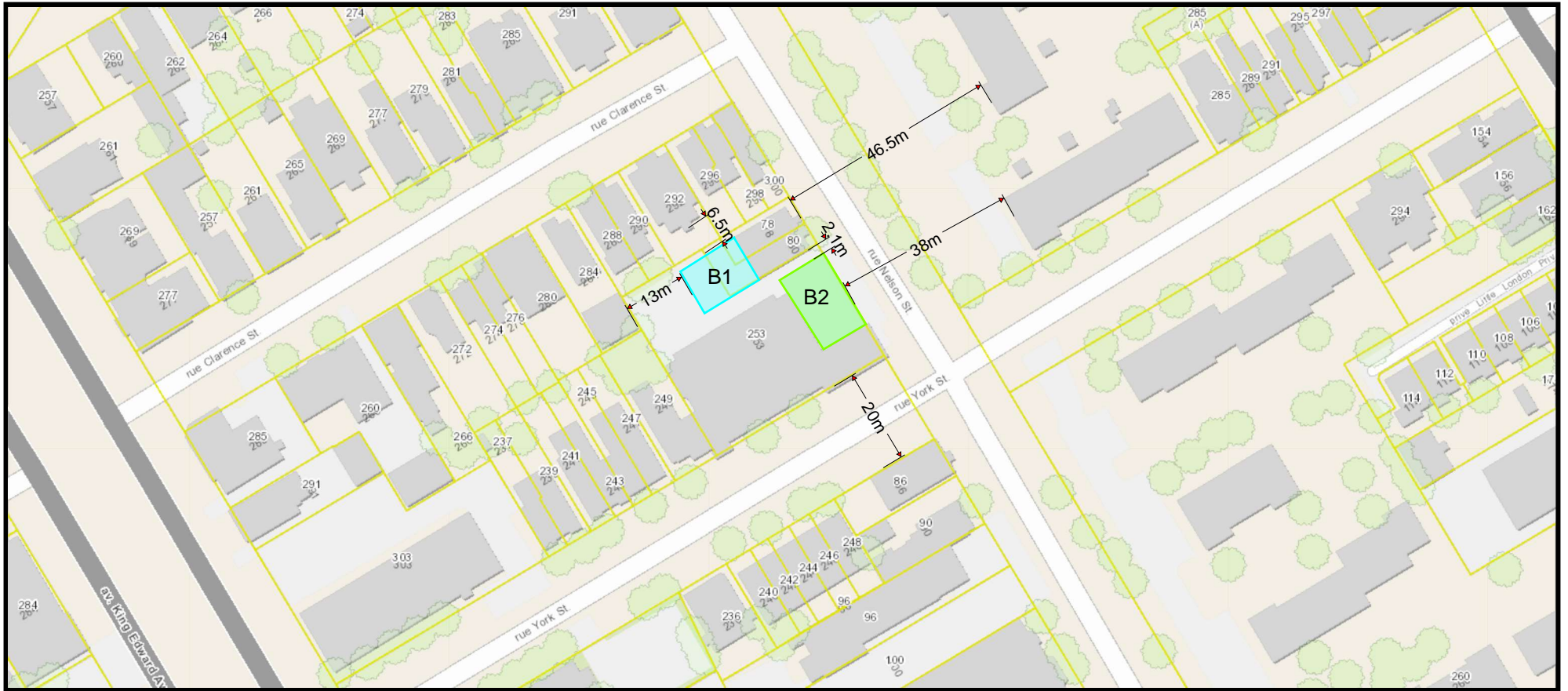
Boundary Conditions for 253 York, 78 Nelson, and 80 Nelson Street



Hydrant Coverage Figure - Existing Conditions
253 York St, 78 Nelson St, 80 Nelson St



Exposure Distance Figure



APPENDIX D
SANITARY CALCULATIONS

McINTOSH PERRY

CO-22-0938 - Ex. Building at 78 & 80 Nelson St - Sanitary Demands

Project:	Ex. Building at 78 & 80 Nelson St		
Project No.:	CO-22-0938		
Designed By:	A.J.G.		
Checked By:	R.D.F.		
Date:	11/12/2021		
Site Area	0.024	Gross ha	
Bachelor	12		1.40 Persons per unit
Total Population	17	Persons	

DESIGN PARAMETERS

Residential Peaking Factor	3.71	<i>* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{*0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8</i>	
Mannings coefficient (n)	0.013		
Demand (per capita)	280	L/day	
Infiltration allowance	0.33	L/s/Ha	

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.001
Wet	0.007
Total	0.008

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	17	0.06
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)		0
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.06	L/s
PEAK RESIDENTIAL FLOW	0.20	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.06	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.21	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.21	L/s

McINTOSH PERRY

CO-22-0938 - 78 & 80 Nelson Addition - Sanitary Demands

Project:	78 & 80 Nelson Addition		
Project No.:	CO-22-0938		
Designed By:	A.J.G.		
Checked By:	R.D.F.		
Date:	11/12/2021		
Site Area	0.064	Gross ha	
Bachelor	26	1.40	Persons per unit
Total Population	37	Persons	

DESIGN PARAMETERS

Residential Peaking Factor	3.67	* Using Harmon Formula = $1 + (14 / (4 + P^{0.5})) * 0.8$ where P = population in thousands, Harmon's Correction Factor = 0.8	
Mannings coefficient (n)	0.013		
Demand (per capita)	280	L/day	
Infiltration allowance	0.33	L/s/ha	

EXTRANEIOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.00
Wet	0.02
Total	0.02

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	37	0.12
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)		0
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.12	L/s
PEAK RESIDENTIAL FLOW	0.44	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.12	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.44	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.46	L/s

McINTOSH PERRY

CO-22-0938 - 253 York St Addition - Sanitary Demands

Project:	253 York St Addition
Project No.:	CO-22-0938
Designed By:	A.J.G.
Checked By:	R.D.F.
Date:	11/12/2021

Site Area	0.089	Gross ha		
Bachelor	17		1.40	Persons per unit
1 Bedroom	3		1.40	Persons per unit
Total Population	29	Persons		

DESIGN PARAMETERS

Residential Peaking Factor	3.69	* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{*0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.00
Wet	0.03
Total	0.03

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	29	0.09
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)		0
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.09	L/s
PEAK RESIDENTIAL FLOW	0.35	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.10	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.35	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.38	L/s

SANITARY SEWER DESIGN SHEET

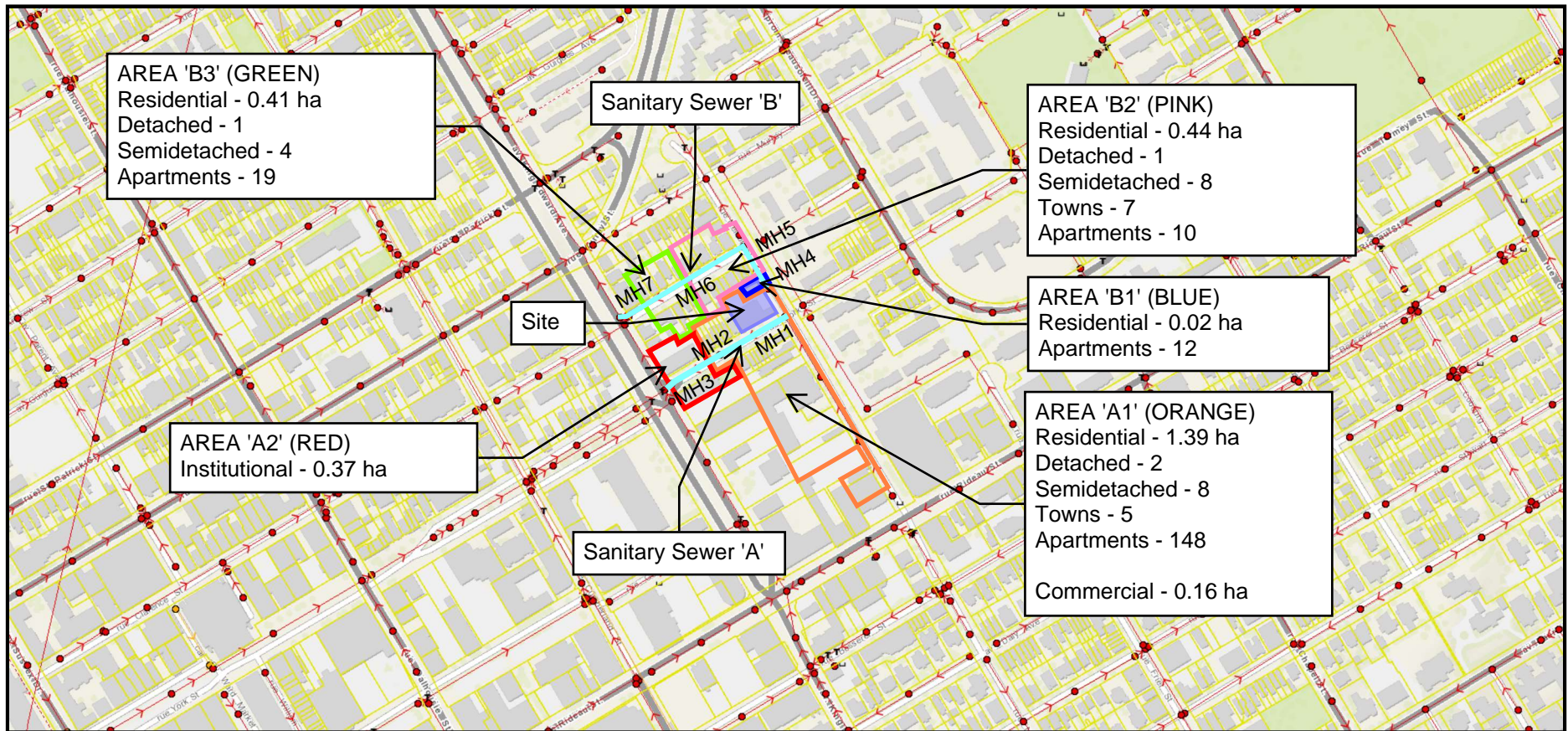
PROJECT: 253 York, 80 Nelson, 78 Nelson
LOCATION:
CLIENT: Smart Living Properties



LOCATION				RESIDENTIAL									ICI AREAS						INFILTRATION ALLOWANCE			FLOW		SEWER DATA							
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	
STREET	AREA ID	FROM MH	TO MH	UNIT TYPES				AREA (ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	AREA (ha)						PEAK FLOW (L/s)	AREA (ha)		FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY		
				SF	SD	TH	APT		IND	CUM			INSTITUTIONAL		COMMERCIAL		INDUSTRIAL			IND	CUM								IND	CUM	L/s
				IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM
Nelson Avenue	A-1	MH1	MH2	2	8	5	148	1.39	382.3	382.3	3.43	4.24			0.00	0.16	0.16		0.00	0.08	1.55	1.55	0.51	4.84	500.29	78.00	600	0.61	1.714	495.46	99.03
York Street	A-2	MH2	MH3					0.00	0.0	382.3	3.43	4.24	0.37	0.37		0.16			0.00	0.26	0.37	1.93	0.64	5.14	597.47	60.00	600	0.87	2.047	592.33	99.14
Nelson Avenue	B-1	MH4	MH5				12	0.02	27.6	27.6	3.69	0.33			0.00		0.00		0.00	0.00	0.02	0.02	0.01	0.34	113.24	50.00	300	1.26	1.552	112.90	99.70
Clarence Street	B-2	MH5	MH6	1	8	7	10	0.44	66.9	94.5	3.60	1.10			0.00	0.00	0.00		0.00	0.44	0.47	0.15	1.26	78.79	66.00	300	0.61	1.080	77.53	98.40	
Clarence Street	B-3	MH6	MH7	1	4		19	0.41	57.9	152.4	3.55	1.75			0.00	0.00	0.00		0.00	0.41	0.88	0.29	2.04	30.26	70.00	300	0.09	0.415	28.22	93.24	

Design Parameters:				Notes:				Designed: AJG				No.		Revision				Date			
Residential		ICI Areas		1. Mannings coefficient (n) = 0.013				2. Demand (per capita): 280 L/day				1.		City of Ottawa Submission 1				2021-08-23			
SF	3.4	p/p/u		3. Infiltration allowance: 0.33 L/s/Ha				4. Residential Peaking Factor:				Checked: RDF									
TH/SD	2.7	p/p/u	INST	28,000	L/Ha/day	1.5	Harmon Formula = $1 + (14 / (4 + P^{0.5})) * 0.8$				Project No.: CO-22-0938										
APT	2.3	p/p/u	COM	28,000	L/Ha/day	1.5	where P = population in thousands														
Other	60	p/p/Ha	IND	35,000	L/Ha/day	MOE Chart															
																Sheet No:					
																1 of 1					

Sanitary Drainage Figure - Existing Conditions





2 SANITARY SEWER DESIGN

The sanitary service laterals from the proposed development will be connected to the existing 600 mm diameter sanitary sewer along York Street.

Sewage discharges will be domestic in type and in compliance with the City of Ottawa Sewer Use By-law. The anticipated peak sanitary flow will be a total of approximately 1.05 L/s.

The sanitary sewage flow for the building was calculated based on the City of Ottawa Sewer Design Guidelines (Section 4.4.1.2).

2.1 Design Flows

The existing building is for residential use with domestic flows. The design flows are calculated based on the population density shown in the Ottawa Water Distribution Guidelines Section 4.2.8.

The peak sanitary design flows are calculated using the Ottawa Sewer Guidelines – Technical Bulletin ISTB-2018-01.

Residential

Total domestic pop:

One Bedroom units	(14) x 1.4 ppu:	19.6
Two Bedroom units	(14) x 2.1 ppu:	29.4
Three Bedroom	(1) x 3.1 ppu	3.1
Four Bedroom	(8) x 4.2 ppu	33.6
Total:		85.7

$$Q_{\text{Domestic}} = 86 \times 280 \text{ L/person/day} \times (1/86,400 \text{ sec/day}) = 0.28 \text{ L/sec}$$

$$\text{Peaking Factor} = 1 + \left(\frac{14}{4 + \left(\frac{P}{1000} \right)} \right) * 0.8 = 3.61 \text{ (4 maximum)}$$

$$Q_{\text{Peak Domestic}} = 0.28 \text{ L/sec} \times 3.61 = 1.01 \text{ L/sec}$$

Infiltration

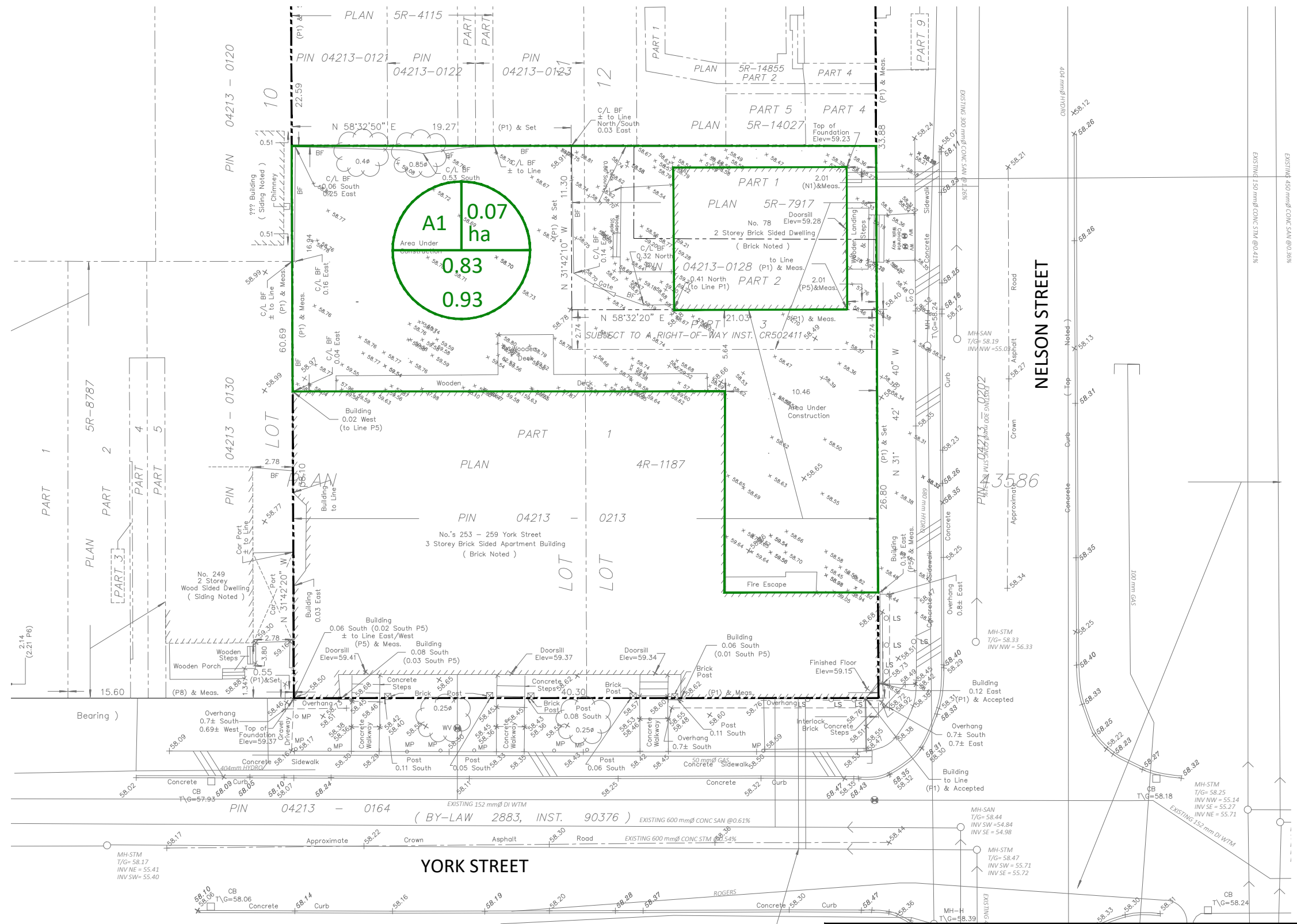
$$Q_{\text{Infiltration}} = 0.33 \text{ L/ha/sec} \times 0.13 \text{ ha} = 0.04 \text{ L/sec}$$

$$\text{Total Peak Sanitary Flow} = 1.01 + 0.04 = 1.05 \text{ L/sec}$$



**APPENDIX E
PRE-DEVELOPMENT DRAINAGE PLAN**

FILENAME: U:\Ottawa\01 - Project - Proposals\2022\01\01\CCO\22-0938 Smart Living Properties - BLDG Additions_253 York Street\12 - Drawings\CCO-22-0938 - Presentation.dwg
 LAST PLOTTED: Thursday, March 30, 2023, 8:18 AM
 LAST MODIFIED: Thursday, March 30, 2023, 8:18 AM

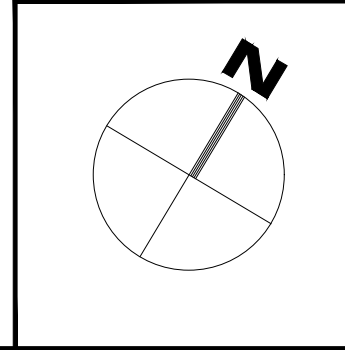
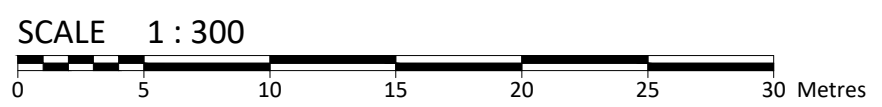


Legend:

Client: SMART LIVING PROPERTIES
 226 ARGYLE AVE OTTAWA, ON K2P 1B9

Project: BUILDING ADDITIONS
 253-257 YORK ST & 78-80 NELSON ST

Drawing Title: PRE-DEVELOPMENT DRAINAGE AREA PLAN



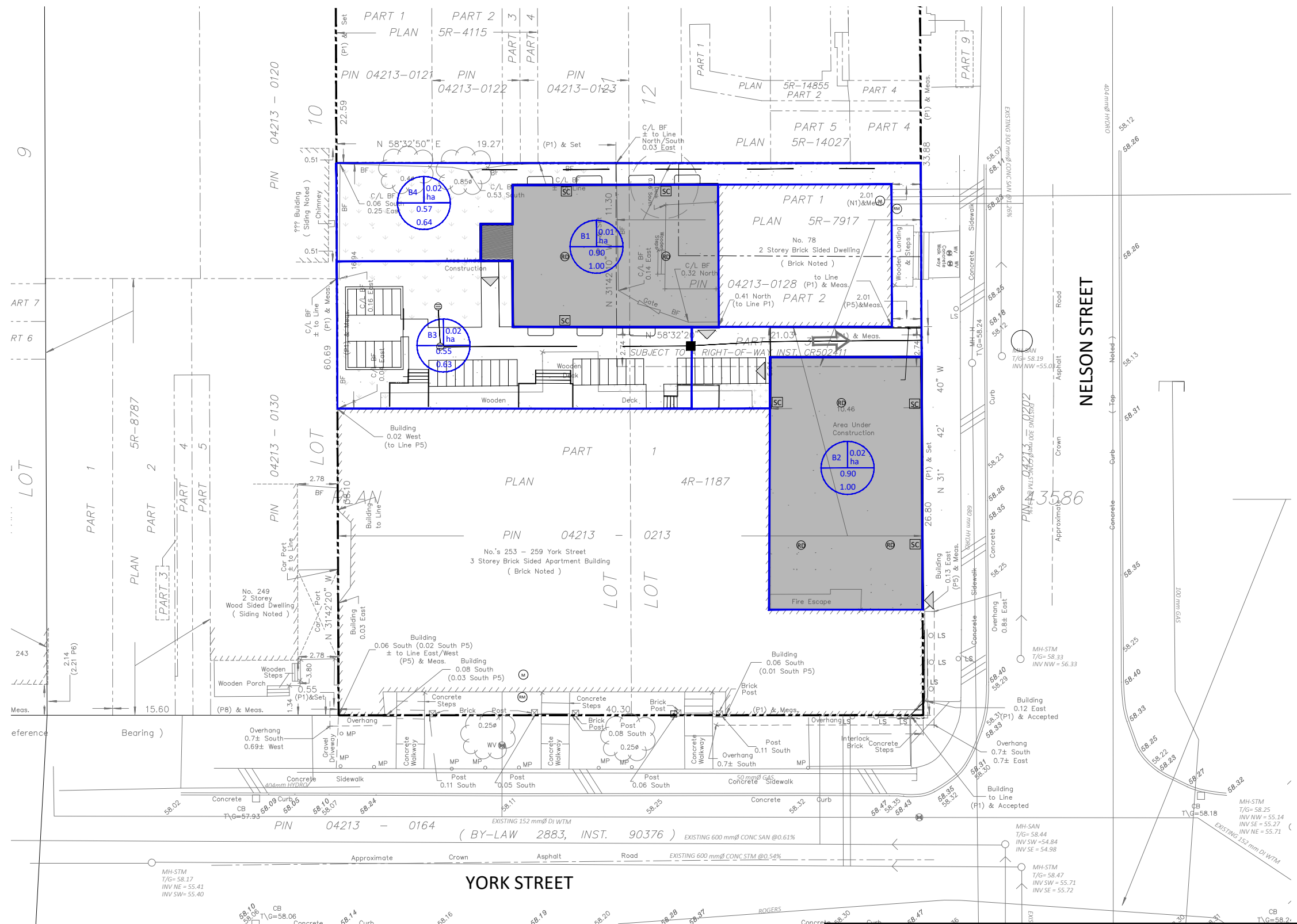
McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A 1L0
 Tel: 613-836-2184 Fax: 613-836-3742
 www.mcintoshperry.com

Client:	SMART LIVING PROPERTIES 226 ARGYLE AVE OTTAWA, ON K2P 1B9	
Project:	BUILDING ADDITIONS 253-257 YORK ST & 78-80 NELSON ST	
Drawing Title:	PRE-DEVELOPMENT DRAINAGE AREA PLAN	
Drawn by:	R.R.R.	Checked By: A.J.G.
Scale:	1:300	Project Number: CCO-22-0938
No.	Revisions	Date
2	ISSUED FOR REVIEW	MAR 10, 2022
1	ISSUED FOR REVIEW	AUG 24, 2021

PRE

APPENDIX F
POST-DEVELOPMENT DRAINAGE PLAN

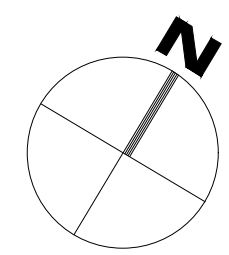
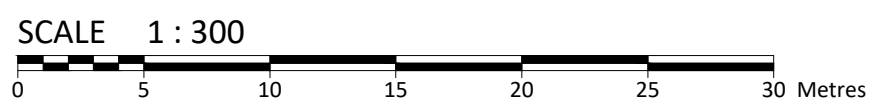
FILENAME: U:\Ottawa\01 - Project - Proposals\2022 Jobs\CCO\CCO-22-0938 Smart Living Properties - BLDG Additions_253 York Street\12 - Drawings\CCO-22-0938 - Presentation.dwg
 LAST PLOTTED: Thursday, March 30, 2023, 8:18 AM
 LAST MODIFIED: Thursday, March 30, 2023, 8:18 AM



Legend:

The legend defines the symbols used in the drainage area plan:

- DRAINAGE AREA**: Represented by a circle containing the area label (e.g., B3) and the area in hectares (e.g., 0.07 ha).
- 5-YEAR RUNOFF COEFFICIENT**: Represented by the numerical value 0.79.
- 100-YEAR RUNOFF COEFFICIENT**: Represented by the numerical value 0.88.



McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A 1L0
 Tel: 613-836-2184 Fax: 613-836-3742
 www.mcintoshperry.com

Client:		SMART LIVING PROPERTIES 226 ARGYLE AVE OTTAWA, ON K2P 1B9	
Project:		BUILDING ADDITIONS 253-257 YORK ST & 78-80 NELSON ST	
Drawing Title:		POST-DEVELOPMENT DRAINAGE AREA PLAN	
Drawn by:	Checked By:	Drawing Number:	
R.R.R.	A.J.G.	2	ISSUED FOR REVIEW
Scale:	Project Number:	1	ISSUED FOR REVIEW
1:300	CCO-22-0938		MAR 10, 2022
			AUG 24, 2021
No.	Revisions	Date	

POST

APPENDIX G
STORMWATER MANAGEMENT CALCULATIONS

McINTOSH PERRY

Pre-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m ²)	C	Gravel Area (m ²)	C	Pervious Area (m ²)	C	C _{AVG} 5-Year	C _{AVG} 100-Year
A1	0.071	642.73	0.90	0.00	0.60	69.00	0.20	0.83	0.93

Pre-Development Runoff Calculations

Drainage Area	Area (ha)	C 5-Year	C 100-Year	Tc (min)	I (mm/hr)		Q (L/s)	
					5-Year	100-Year	5-Year	100-Year
A1	0.071	0.83	0.93	10	104.2	178.6	17.16	32.76
Total	0.071						17.16	32.76

Post-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m ²)	C	Gravel Area (m ²)	C	Pervious Area (m ²)	C	C _{AVG} 5-Year	C _{AVG} 100-Year
B1	0.014	144.80	0.90	0.00	0.60	0.00	0.20	0.90	1.00
B2	0.020	204.40	0.90	0.00	0.60	0.00	0.20	0.90	1.00
B3	0.017	86.58	0.90	0.00	0.60	85.21	0.20	0.55	0.63
B4	0.019	99.60	0.90	0.00	0.60	91.15	0.20	0.57	0.64

Restricted 3 Storey Roof
 Restricted 4 Storey Roof
 Unrestricted Area
 Unrestricted Area

Post-Development Runoff Calculations

Drainage Area	Area (ha)	C 5-Year	C 100-Year	Tc (min)	I (mm/hr)		Q (L/s)	
					5-Year	100-Year	5-Year	100-Year
B1	0.014	0.90	1.00	10	104.2	178.6	3.77	7.19
B2	0.020	0.90	1.00	10	104.2	178.6	5.33	10.15
B3	0.017	0.55	0.63	10	104.2	178.6	2.75	5.36
B4	0.019	0.57	0.64	10	104.2	178.6	3.12	6.08
Total	0.071						14.98	28.76

Restricted 3 Storey Roof
 Restricted 4 Storey Roof
 Unrestricted Area
 Unrestricted Area

Required Restricted Flow Per Pre Consultation

Drainage Area	Area (ha)	C 5-Year	Tc (min)	I (mm/hr)	Q (L/s)
				5-Year	5-Year
A1	0.071	0.50	10	104.2	10.31

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow (L/s)		Restricted Flow (L/s)		Storage Required (m ³)		Storage Provided (m ³)	
	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year
B1	3.77	7.19	0.64	1.14	2.36	4.60	2.72	4.89
B2	5.33	10.15	1.14	1.89	3.89	7.51	4.60	7.66
B3	2.75	5.36	2.75	5.36				
B4	3.12	6.08	3.12	6.08				
Total	11.85	22.69	7.66	14.46	6.25	12.12	7.31	12.55

Restricted
 Restricted
 Unrestricted
 Unrestricted

McINTOSH PERRY

Storage Requirements for Area B1

5-Year Storm Event

Tc (min)	I (mm/hr)	B1 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	104.2	3.77	0.64	3.13	1.88
20	70.3	2.55	0.64	1.91	2.29
30	53.9	1.95	0.64	1.31	2.36
40	44.2	1.60	0.64	0.96	2.31
50	37.7	1.36	0.64	0.72	2.17
60	32.9	1.19	0.64	0.55	1.99
70	29.4	1.06	0.64	0.42	1.78
80	26.6	0.96	0.64	0.32	1.55
90	24.3	0.88	0.64	0.24	1.30
100	22.4	0.81	0.64	0.17	1.03

Maximum Storage Required 5-Year (m³) = 2.36

100-Year Storm Event

Tc (min)	I (mm/hr)	B1 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	178.6	7.19	1.14	6.05	3.63
20	120.0	4.83	1.14	3.69	4.43
30	91.9	3.70	1.14	2.56	4.60
40	75.1	3.02	1.14	1.88	4.52
50	64.0	2.57	1.14	1.43	4.30
60	55.9	2.25	1.14	1.11	4.00
70	49.8	2.00	1.14	0.86	3.63
80	45.0	1.81	1.14	0.67	3.22

Maximum Storage Required 100-Year (m³) = 4.60

Storage Occupied In Area B1

5-Year Storm Event

Roof Storage			
Location	Area*	Depth	Volume (m ³)
Roof	108.60	0.025	2.72
Total			2.72

Storage Available (m³) = 2.72
Storage Required (m³) = 2.36

100-Year Storm Event

Roof Storage			
Location	Area*	Depth	Volume (m ³)
Roof	108.60	0.045	4.89
Total			4.89

Storage Available (m³) = 4.89
Storage Required (m³) = 4.60

* Storage area is 75% of the total roof area

McINTOSH PERRY

CCO-22-0938 - 253 York Street - Runoff Calculations

Nelson Street Building

3 of 6

Roof Drain Flow (B1)

Roof Drains Summary		
Type of Control Device	Watts Drainage - Accutrol Weir	
Number of Roof Drains	2	
	5-Year	100-Year
Rooftop Storage (m ³)	2.72	4.89
Storage Depth (m)	0.025	0.045
Flow (Per Roof Drain) (L/s)	0.32	0.57
Total Flow (L/s)	0.64	1.14

Flow Rate Vs. Build-Up (One Weir)	
Depth (mm)	Flow (L/s)
15	0.19
20	0.25
25	0.32
30	0.38
35	0.44
40	0.50
45	0.57
50	0.63
55	0.69

Roof Drain Flow			
	Flow (l/s)	Storage Depth (mm)	Drains Flow (l/s)
5-Year	0.19	15	0.38
	0.25	20	0.50
	0.32	25	0.64
	0.38	30	0.76
	0.44	35	0.88
100-Year	0.50	40	1.00
	0.57	45	1.14
	0.63	50	1.26
	0.69	55	1.38
	0.76	60	1.52
	0.82	65	1.64
	0.88	70	1.76
	0.95	75	1.90
	1.01	80	2.02
	1.07	85	2.14
	1.13	90	2.26
	1.20	95	2.40
	1.26	100	2.52
	1.32	105	2.64
	1.39	110	2.78
1.45	115	2.90	
1.51	120	3.02	
1.58	125	3.16	
1.64	130	3.28	
1.70	135	3.40	
1.76	140	3.52	
1.83	145	3.66	
1.89	150	3.78	

* Roof Drain model to be Accutrol Weirs, See attached sheets

* Roof Drain Flow information taken from Watts Drainage website

CALCULATING ROOF FLOW EXAMPLES

2 roof drains during a 5 year storm
 elevation of water = 30mm
 Flow leaving 2 roof drains = (2 x 0.36 L/s) = 0.72 L/s

2 roof drains during a 100 year storm
 elevation of water = 45mm
 Flow leaving 2 roof drains = (2 x 0.54 L/s) = 1.08 L/s

Note: The flow leaving through a restricted roof drain is based on flow vs. head information

McINTOSH PERRY

Storage Requirements for Area B2

5-Year Storm Event

Tc (min)	I (mm/hr)	B1 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	104.2	5.33	0.64	4.69	2.81
20	70.3	3.59	0.64	2.95	3.54
30	53.9	2.76	0.64	2.12	3.81
40	44.2	2.26	0.64	1.62	3.89
50	37.7	1.93	0.64	1.29	3.86
60	32.9	1.68	0.64	1.04	3.76
70	29.4	1.50	0.64	0.86	3.62
80	26.6	1.36	0.64	0.72	3.45
90	24.3	1.24	0.64	0.60	3.25
100	22.4	1.15	0.64	0.51	3.04

Maximum Storage Required 5-Year (m³) = 3.89

100-Year Storm Event

Tc (min)	I (mm/hr)	B1 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	178.6	10.15	1.14	9.01	5.40
20	120.0	6.82	1.14	5.68	6.81
30	91.9	5.22	1.14	4.08	7.34
40	75.1	4.27	1.14	3.13	7.51
50	64.0	3.63	1.14	2.49	7.48
60	55.9	3.18	1.14	2.04	7.33
70	49.8	2.83	1.14	1.69	7.09
80	45.0	2.56	1.14	1.42	6.80

Maximum Storage Required 100-Year (m³) = 7.51

Storage Occupied In Area B2

5-Year Storm Event

Roof Storage			
Location	Area*	Depth	Volume (m ³)
Roof	153.30	0.030	4.60
Total			4.60

Storage Available (m³) = 4.60
Storage Required (m³) = 3.89

100-Year Storm Event

Roof Storage			
Location	Area*	Depth	Volume (m ³)
Roof	153.30	0.050	7.66
Total			7.66

Storage Available (m³) = 7.66
Storage Required (m³) = 7.51

* Storage area is 75% of the total roof area

McINTOSH PERRY

CCO-22-0938 - 253 York Street - Runoff Calculations

York Street Building

5 of 6

Roof Drain Flow (B2)

Roof Drains Summary		
Type of Control Device	Watts Drainage - Accutrol Weir	
Number of Roof Drains	3	
	5-Year	100-Year
Rooftop Storage (m ³)	4.60	7.66
Storage Depth (m)	0.030	0.050
Flow (Per Roof Drain) (L/s)	0.38	0.63
Total Flow (L/s)	1.14	1.89

Flow Rate Vs. Build-Up (One Weir)	
Depth (mm)	Flow (L/s)
15	0.19
20	0.25
25	0.32
30	0.38
35	0.44
40	0.50
45	0.57
50	0.63
55	0.69

* Roof Drain model to be Accutrol Weirs, See attached sheets

* Roof Drain Flow information taken from Watts Drainage website

CALCULATING ROOF FLOW EXAMPLES

3 roof drains during a 5 year storm
 elevation of water = 25mm
 Flow leaving 3 roof drains = (3 x 0.32 L/s) = 0.96L/s

3 roof drains during a 100 year storm
 elevation of water = 50mm
 Flow leaving 3 roof drains = (3 x 0.54 L/s) = 1.89L/s

Roof Drain Flow			
	Flow (l/s)	Storage Depth (mm)	Drains Flow (l/s)
	0.19	15	0.57
	0.25	20	0.75
	0.32	25	0.96
5-Year	0.38	30	1.14
	0.44	35	1.32
	0.50	40	1.50
100-Year	0.57	45	1.71
	0.63	50	1.89
	0.69	55	2.07
	0.76	60	2.28
	0.82	65	2.46
	0.88	70	2.64
	0.95	75	2.85
	1.01	80	3.03
	1.07	85	3.21
	1.13	90	3.39
	1.20	95	3.60
	1.26	100	3.78
	1.32	105	3.96
	1.39	110	4.17
	1.45	115	4.35
	1.51	120	4.53
	1.58	125	4.74
	1.64	130	4.92
	1.70	135	5.10
	1.76	140	5.28
	1.83	145	5.49
	1.89	150	5.67

Note: The flow leaving through a restricted roof drain is based on flow vs. head information

McINTOSH PERRY

CCO-22-0938 - 253 York Street - Runoff Calculations

6 of 6

Time of Concentration Pre-Development

Drainage Area ID	Sheet Flow Distance (m)	Slope of Land (%)	Tc (min) (5-Year)	Tc (min) (100-Year)
A1	39	0.76	6	4

* Therefore, a Tc of 10 can be used

$$T_c = (3.26(1.1-c)L^{0.5}/S^{0.33})$$

c= Balanced Runoff Coefficient

L= Length of Drainage Area

S= Average Slope of Watershed

STORM SEWER DESIGN SHEET

PROJECT: Building Additions
 LOCATION: 253 York Street
 CLIENT: Smart Living Properties



LOCATION				CONTRIBUTING AREA (ha)				RATIONAL DESIGN FLOW										SEWER DATA												
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22			23	24	25	26	27		28
STREET	AREA ID	FROM MH	TO MH	C-VALUE	AREA	INDIV AC	CUMUL AC	INLET (min)	TIME IN PIPE	TOTAL (min)	i (5) (mm/hr)	i (10) (mm/hr)	i (100) (mm/hr)	5yr PEAK FLOW (L/s)	10yr PEAK FLOW (L/s)	100yr PEAK FLOW (L/s)	FIXED FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	PIPE SIZE (mm)			SLOPE (%)	VELOCITY (m/s)	AVAIL CAP (5yr)				
																					DIA	W	H			(L/s)	(%)			
Nelson Street	B3	CB4	TEE	0.55	0.02	0.01	0.01	10.00	0.02	10.02	104.19	122.14	178.56	2.71					2.71	62.04	1.30	250			1.00	1.224	59.33	95.63%		
	B1	BLDG	TEE	0.90	0.01	0.01	0.01	10.00	0.02	10.02	104.19	122.14	178.56	2.61					2.61	62.04	1.23	250			1.00	1.224	59.43	95.80%		
		TEE	MH1				0.00	0.02	10.02	0.29	10.31	104.10	122.04	178.41	5.31					5.31	62.04	21.23	250			1.00	1.224	56.73	91.44%	

Definitions:
 Q = 2.78QA, where:
 Q = Peak Flow in Litres per Second (L/s)
 A = Area in Hectares (ha)
 i = Rainfall intensity in millimeters per hour (mm/hr)
 [i = 998.071 / (TC+6.053)^0.814] 5 YEAR
 [i = 1174.184 / (TC+6.014)^0.816] 10 YEAR
 [i = 1735.688 / (TC+6.014)^0.820] 100 YEAR

Notes:
 1. Mannings coefficient (n) = 0.013

Designed: FFR
 Checked: AJG
 Project No.: COO-22-0938

No.	Revision	Date
1.	ISSUED FOR REVIEW	2022-03-10

Date: 2022-03-10

Sheet No: 1 of 1

Pre Development Rational Method Coefficient Figure

Site Area = 1,536 m²
Development Area = 710.9 m²
Landscaped Area = 69 m²
Impervious Area = 642.3 m²
RM Coefficient = 0.83

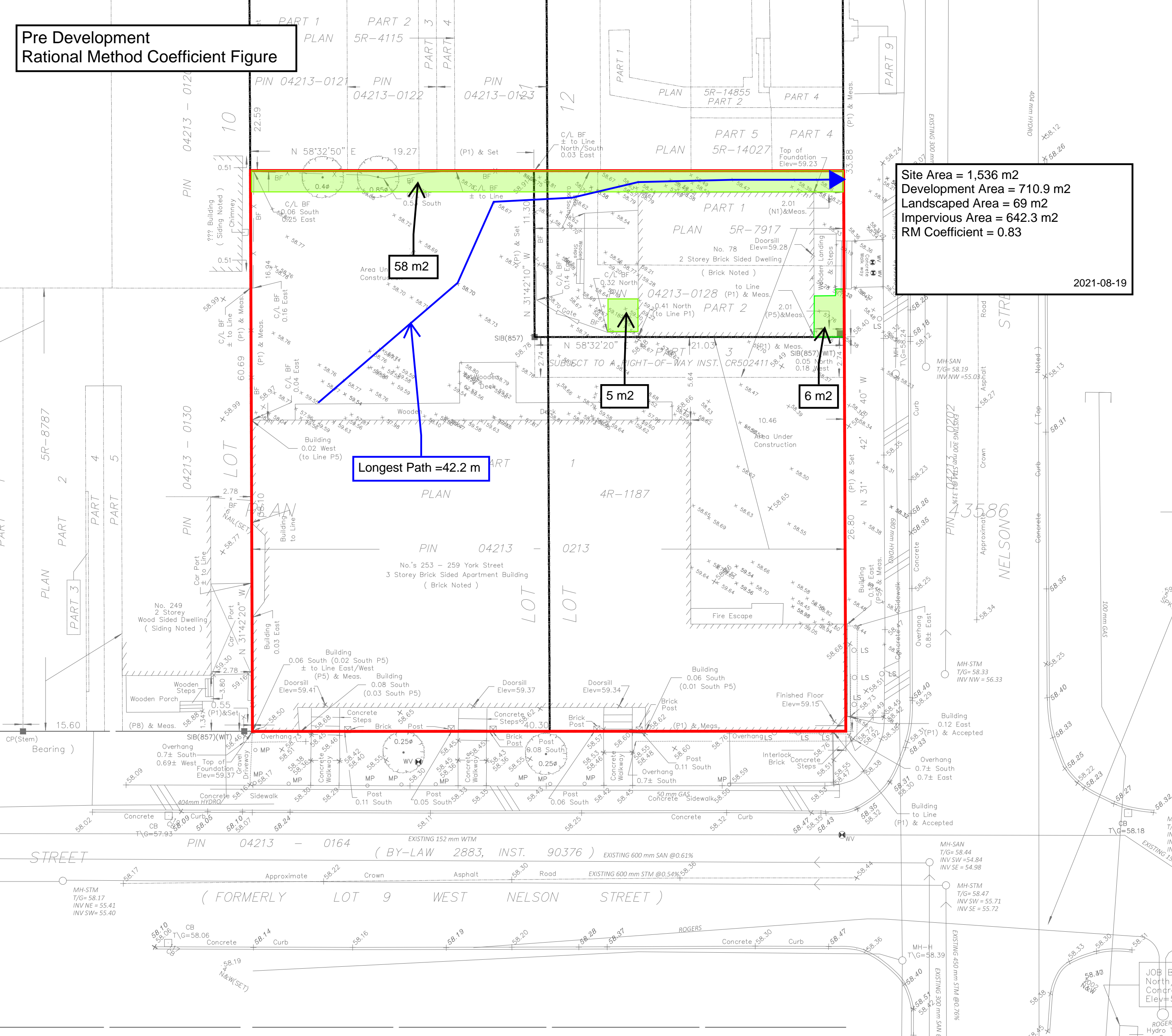
2021-08-19

58 m²

5 m²

6 m²

Longest Path = 42.2 m



**Post Development
Rational Method Coefficient Figure**

Site Area = 1,536 m²
 Development Area = 710.9 m²
 Proposed Landscaped Area = 250.2 m²
 Proposed Impervious Area = 460.7 m²
 RM Coefficient = 0.65

2021-08-19

163.5 m²

Excluded Area

3.8 m²

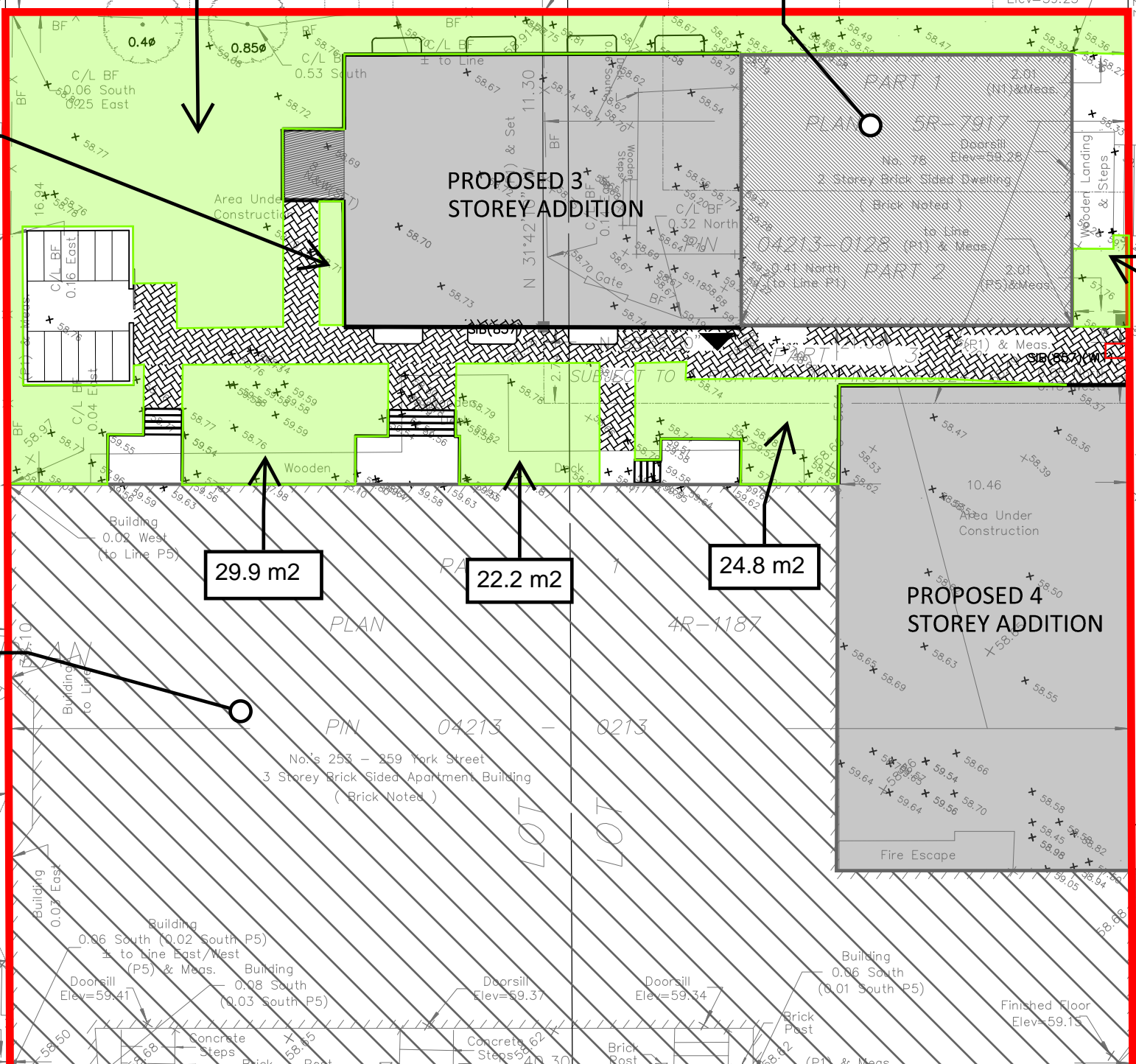
6 m²

29.9 m²

22.2 m²

24.8 m²

Excluded Area



**APPENDIX H
CITY OF OTTAWA DESIGN CHECKLIST**

City of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Criteria	Location (if applicable)
<input type="checkbox"/> Executive Summary (for larger reports only).	N/A
<input type="checkbox"/> Date and revision number of the report.	On Cover
<input type="checkbox"/> Location map and plan showing municipal address, boundary, and layout of proposed development.	Appendix A
<input type="checkbox"/> Plan showing the site and location of all existing services.	Site Servicing Plan (C101)
<input type="checkbox"/> Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	1.1 Purpose 1.2 Site Description 6.0 Stormwater Management
<input type="checkbox"/> Summary of pre-consultation meetings with City and other approval agencies.	Appendix B
<input type="checkbox"/> Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	1.1 Purpose 1.2 Site Description 6.0 Stormwater Management
<input type="checkbox"/> Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary

<input type="checkbox"/> Identification of existing and proposed infrastructure available in the immediate area.	N/A
<input type="checkbox"/> Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
<input type="checkbox"/> Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
<input type="checkbox"/> Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
<input type="checkbox"/> Proposed phasing of the development, if applicable.	N/A
<input type="checkbox"/> Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Background Studies, Standards and References
<input type="checkbox"/> All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> ○ Metric scale ○ North arrow (including construction North) ○ Key plan ○ Name and contact information of applicant and property owner ○ Property limits including bearings and dimensions ○ Existing and proposed structures and parking areas ○ Easements, road widening and rights-of-way ○ Adjacent street names 	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)

4.2 Development Servicing Report: Water

Criteria	Location (if applicable)
<input type="checkbox"/> Confirm consistency with Master Servicing Study, if available	N/A
<input type="checkbox"/> Availability of public infrastructure to service proposed development	N/A
<input type="checkbox"/> Identification of system constraints	N/A
<input type="checkbox"/> Identify boundary conditions	Unavailable at time of submission
<input type="checkbox"/> Confirmation of adequate domestic supply and pressure	N/A
<input type="checkbox"/> Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter’s Survey. Output should show available fire flow at locations throughout the development.	Appendix C
<input type="checkbox"/> Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	N/A
<input type="checkbox"/> Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
<input type="checkbox"/> Address reliability requirements such as appropriate location of shut-off valves	N/A
<input type="checkbox"/> Check on the necessity of a pressure zone boundary modification.	N/A
<input type="checkbox"/> Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Appendix C

<input type="checkbox"/> Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Site Servicing Plan (C101)
<input type="checkbox"/> Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input type="checkbox"/> Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix C
<input type="checkbox"/> Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

4.3 Development Servicing Report: Wastewater

Criteria	Location (if applicable)
<input type="checkbox"/> Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/A
<input type="checkbox"/> Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
<input type="checkbox"/> Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input type="checkbox"/> Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 5.1

<input type="checkbox"/> Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Section 5.2 Proposed Sanitary Design
<input type="checkbox"/> Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Appendix C
<input type="checkbox"/> Description of proposed sewer network including sewers, pumping stations, and forcemains.	Section 5.2 Proposed Sanitary Sewer
<input type="checkbox"/> Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
<input type="checkbox"/> Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/> Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/> Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/> Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

Criteria	Location (if applicable)
<input type="checkbox"/> Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> Analysis of available capacity in existing public infrastructure.	N/A
<input type="checkbox"/> A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Pre & Post-Development Plans
<input type="checkbox"/> Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> Set-back from private sewage disposal systems.	N/A
<input type="checkbox"/> Watercourse and hazard lands setbacks.	N/A
<input type="checkbox"/> Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
<input type="checkbox"/> Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
<input type="checkbox"/> Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period).	Appendix G

<input type="checkbox"/> Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Site Grading, Drainage, Sediment & Erosion Control Plan
<input type="checkbox"/> Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 7.0 Proposed Stormwater Management Appendix G
<input type="checkbox"/> Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/A
<input type="checkbox"/> Identification of potential impacts to receiving watercourses	N/A
<input type="checkbox"/> Identification of municipal drains and related approval requirements.	N/A
<input type="checkbox"/> Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
<input type="checkbox"/> 100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
<input type="checkbox"/> Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A

<input type="checkbox"/> Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 8.0 Sediment & Erosion Control
<input type="checkbox"/> Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/> Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 Conclusion Checklist

Criteria	Location (if applicable)
<input type="checkbox"/> Clearly stated conclusions and recommendations	Section 9.0 Summary Section 10.0 Recommendations
<input type="checkbox"/> Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
<input type="checkbox"/> All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped