QUEENSWOOD UNITED CHURCH

360 KENNEDY LANE EAST
STORMWATER MANAGEMENT REPORT

SEPTEMBER 20, 2022

360 KENNEDY LANE EAST

STORMWATER MANAGEMENT REPORT

QUEENSWOOD UNITED CHURCH

2ND SUBMISSION

PROJECT NO.: 211-12127-00
CLIENT REF:
DATE: SEPTEMBER 20, 2022

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REVISION HISTORY

FIRST ISSUE

November 30th, 2021	Draft SWM Report			
Prepared by	Reviewed by	Approved By		
MO	AJ	AJ		
SECOND ISSUE				
September 20th, 2022	SWM Report			
Prepared by	Reviewed by	Approved By		
KK	AJ	AJ		

SIGNATURES

PREPARED BY

September 20th, 2022

Kathryn Kerker, M.A.Sc.
Designer, Water Resources

Date

APPROVED[1] BY

September 20th, 2022

Ayham Jadallah, M.Eng., P.Eng.
Project Engineer, Water Resources

Date

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CONTRIBUTORS

CLIENT

Queenswood United Church.

WSP

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Water Resources, Project Engineer	Ayham Jadallah

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1 INTRODUCTION

1.1 SCOPE

WSP Canada Inc. was retained by Queenswood United Church to prepare a Stormwater Management (SWM) report for the proposed development at 360 Kennedy Lane in Ottawa, Ontario. This SWM report examines the potential water quality and quantity impacts of the proposed residential development and summarizes how each will be addressed in accordance with applicable guidelines.

1.2 SITE LOCATION

The site of the proposed development is located at 360 Kennedy Lane East, Ottawa, Ontario. The subject site is bounded by Queenwood United Church to the north, Queenwood Ridge Park to the west and south, and residential homes along Mountainside Crescent to the east. The site is accessed via Kennedy Lane East on the north-west end of the property. The site location is shown in Figure 1.

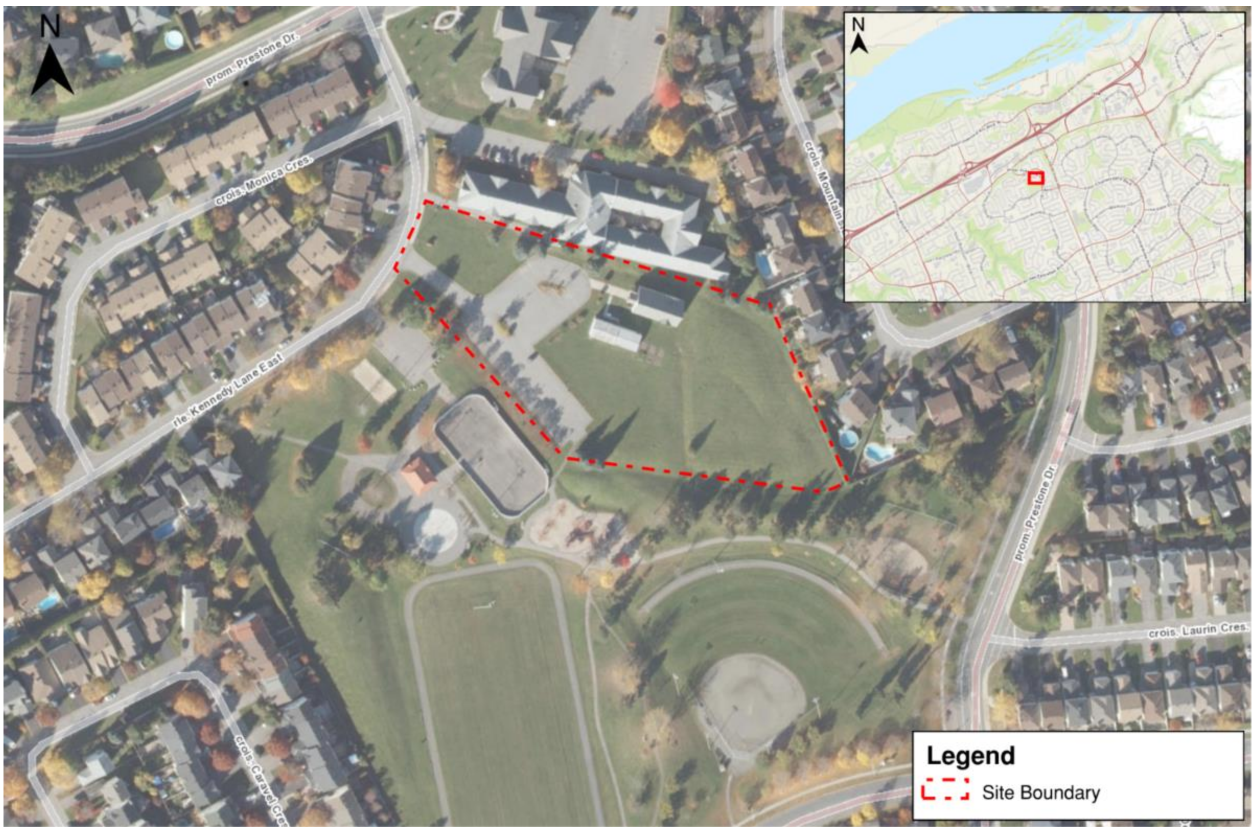


Figure 1: Site Location

1.3 STORMWATER MANAGEMENT PLAN OBJECTIVES

The objectives of the stormwater management plan are as follows:

→ Collect and review background information

→ Determine the site-specific stormwater management requirements to ensure that the proposals are in conformance with the applicable Provincial, Municipal and Conservation Authority stormwater management and development guidelines.

→ Evaluate various stormwater management practices that meet the applicable SWM and development requirements and recommend a preferred strategy.

→ Prepare a stormwater management report documenting the strategy along with the technical information necessary for the justification and sizing of the proposed stormwater management facilities.

1.4 DESIGN CRITERIA

Design criteria were obtained through the Site Plan Pre-Application Consultation Notes provided by the City of Ottawa on May 19th, 2021 (pre consultation notes in **Appendix A**). Criteria for 360 Kennedy Lane East are as follows:

→ **Stormwater Quantity-** control the 100-year post-development flows to the pre-development levels for the 5-year storm events. Allowable runoff coefficient (C) shall be the lesser of the pre-development conditions to a maximum of 0.5.

→ **Storm Quality-** enhanced level of protection per the Rideau Valley Conservation Authority (RVCA) is required (80% TSS Removal).

2 PRE-DEVELOPMENT CONDITIONS

2.1 GENERAL

The subject site is a 1.22 ha parcel of land comprised of primarily landscaped grass area, with an impervious paved parking area and two small building structures. Vehicular access to the site is via an entrance off of Kennedy Lane East. Existing drainage patterns for the site were determined using topographic survey information and arial imagery. Under pre-development conditions the western developed part of the site discharges to the 900 mm concrete storm sewer on Kennedy Lane East, and the eastern undeveloped part drains to the adjacent parkland. The pre-development imperviousness and runoff coefficient was determined using the PCSWMM area weighting tool. The existing conditions drainage area and runoff coefficient is summarized in Table 1. The existing conditions drainage mosaic is shown in Exhibit 1 found in **Appendix B**.

Table 1: Existing Drainage Areas

AREA ID	AREA (HA)	IMPERVIOUS AREA (HA)	IMPERVIOUSNESS (%)	RUNOFF COEFFICENT
EX-01	0.71	0.31	44	0.49
EX-02	0.51	0.03	6	0.21

2.2 RAINFALL INFORMATION

The rainfall intensity is calculated in accordance with Section 5.4.2 of the Ottawa Sewer Design Guidelines (October, 2012):

$$i = \left[\frac{A}{(Td + C)^B}\right]$$

Where;

- A, B, C = regression constants for each return period (defined in section 5.4.2)
- i = rainfall intensity (mm/hour)
- Td = storm duration (minutes)

The IDF parameters/regression constants are per the Ottawa Sewer Design Guidelines (October, 2012).

2.3 ALLOWABLE FLOW RATES

As noted in section 1.4, relevant policies from the OSDG for a re-development and the Site Plan Pre-Application Consultation notes require the 100-year post-development discharge rate from the site be controlled to the pre-development levels for the 5-year storm event, where pre-development conditions are analyzed using the lesser of the actual runoff coefficient and a runoff coefficient of 0.5. As previously discussed, under existing conditions the subject site has a runoff coefficient on 0.37 and therefore the actual runoff coefficient was used for existing conditions analysis.

As discussed in email correspondence on November 8th, 2021, target release rates are to be determined assuming the entire site drains to Kennedy Lane E under existing conditions. Correspondence is included in **Appendix A**. Table 2 shows the pre-development peak flow rates from the entire 1.22 ha site.

PCSWMM was used to evaluate pre-development peak flow rates. Detailed model output can be found in **Appendix C**.

Table 2: Pre-Development Peak Flow Rate

AREA ID	PEAK FLOW RATE (m^3/s)					
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
EX-001	0.08	0.12	0.16	0.22	0.26	0.31

3 POST-DEVELOPMENT CONDITIONS

3.1 GENERAL

The proposed Kennedy Lane E project is a residential development in Ottawa. Post development conditions drainage areas and runoff coefficients are shown in on Exhibit 2 in **Appendix B** and summarized in Table 3.

The proposed development includes the construction of 81 stacked residential units on the approximately 1.22 ha parcel of land. Vehicular access to the site will be via the one existing entrance off of Kennedy Lane E. All stormwater runoff will ultimately discharge via one outlet to the 900 mm concrete sewer on Kennedy Lane E, except for a strip along the boundary which continues to drain to the parkland area.

An estimated area breakdown for the new layout is provided in Table 3.

Table 3: Area Breakdown

Catchment ID	Area (ha)	% Coverage of Project Area	Pervious Area (ha)	Impervious Area (ha)	% Imperviousness	Runoff Coefficient
Controlled Drainage Areas						
S-001	0.089	7.31%	0.017	0.072	82%	0.77
S-002	0.050	4.13%	0.007	0.043	87%	0.80
S-003	0.060	4.87%	0.011	0.049	82%	0.77
S-004	0.024	1.96%	0.006	0.018	74%	0.71
S-005	0.022	1.76%	0.008	0.013	62%	0.62
S-006	0.061	4.99%	0.010	0.051	84%	0.78
S-007	0.028	2.28%	0.000	0.028	100%	0.90
S-008	0.030	2.45%	0.000	0.030	100%	0.90
S-009	0.096	7.84%	0.070	0.026	31%	0.39
S-010	0.062	5.11%	0.014	0.048	79%	0.75
S-011	0.076	6.20%	0.010	0.066	87%	0.81
S-012	0.024	1.94%	0.006	0.017	75%	0.72
S-013	0.028	2.30%	0.011	0.017	63%	0.63
S-014	0.084	6.85%	0.010	0.074	89%	0.82
S-015	0.039	3.17%	0.004	0.035	90%	0.83
S-016	0.105	8.57%	0.021	0.084	81%	0.76
S-017	0.051	4.20%	0.005	0.046	91%	0.83
S-018	0.212	17.34%	0.001	0.090	47%	0.51
Un-Controlled Drainage Areas						
S-019	0.020	1.61%	0.005	0.015	76%	0.73
S-020	0.056	4.62%	0.057	0.012	22%	0.32
S-021	0.006	0.51%	0.0062	0.000	6%	0.21

Catchment ID	Area (ha)	% Coverage of Project Area	Pervious Area (ha)	Impervious Area (ha)	% Imperviousness	Runoff Coefficient
Total Project Area	**1.22**	**100%**	**0.280**	**0.833**	**71%**	**0.68**

3.2 WATER QUANTITY

As noted previously, it is required that the 100-year post-development discharge rate from the site not exceed the 5-year pre-development level. As shown in Table 2, this means the 100-year post development flow must be controlled to 0.12 m^3/s or less.

Proposed features to achieve these targets include;

→ Surface storage with inlet control devices (ICDs) (HYDROVEX VHV or equivalent)

→ Stormtech (or equivalent) subsurface storage chambers with ICDs on outlets (HYDROVEX VHV or equivalent).

PCSWMM software was used to model the behaviour of the proposed SWM system. Storage areas were defined using storage nodes with the appropriate stage-storage relationships. Outflow controls from each storage node were defined using the appropriate Hydrovex VHV head-discharge curve. Specified Hydrovex models are shown in Table 4.

Table 4: Catchbasin Outflow Control

LOCATION	ICD
CB01	125-VHV-2
RYCB01	200-VHV-2
Tank A	100-VHV-1
Tank B	100-VHV-1
Tank C	100-VHV-1
Tank D	100-VHV-1
Tank E	125-VHV-2
Tank F	125-VHV-2

A summary of modeling results is provided in Table 5 and detailed modelling output is included in **Appendix C**.

Table 5: Summary of PCSWMM Modelling Results

	Return Period					
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Peak Discharge Rate (m^3/s)	0.053	0.075	0.086	0.102	0.113	0.121
Storage Utilized in Tank A (m^3)	9.0	13.0	15.8	19.4	22.2	25.2
Storage Utilized in Tank B (m^3)	10.5	15.2	18.4	23.0	26.6	30.4
Storage Utilized in Tank C (m^3)	22.5	32.7	40.1	50.5	58.8	67.7
Storage Utilized in Tank D (m^3)	11.6	16.7	20.2	25.0	28.7	32.6
Storage Utilized in Tank E (m^3)	12.5	17.9	21.8	26.9	30.8	35.0
Storage Utilized in Tank F (m^3)	55.5	79.3	98.2	123.3	142.5	162.8
Depth of Ponding on Surface at CB01 (m)	0	0	0	0	0.053	0.085
Depth of Ponding at RYCB01 (m)	0	0	0	0	0.047	0.101

To avoid risk of flooding to the proposed homes, surface ponding has only been proposed where sufficient freeboard is provided between the 100-year ponding elevation and the finish floor elevation of surrounding homes (i.e. CB01). All other storage will be provided via underground storage as summarized in Table 5. To determine peak surface ponding depths, reference has been made to model output at each respective storage node where surface storage is utilized. Ponding depths have been simulated in the model by routing runoff from the contributing sub-catchment area to a storage node defined with a stage-storage relationship describing the ponding volume available on the surface (based on proposed grading), and with outflow controlled by a stage-discharge rating curve based on a standard 600 mm square CB grate (per City of Ottawa standards) with a Hydrovex VHV ICD on the CB lead.

Table 6 shows the parameters governing flow control through each of the proposed ICD devices, including maximum elevation, head, flow, and selected ICD type.

Table 6: Summary of Flow Control Parameters

Location	Invert (m)	100-Year Elev. (m)	Head (m)	Q_{100} (L/s)	Max Volume (m^3)	Hydrovex Unit
Tank A	84.06	85.63	1.57	0.013	25.2	100-VHV-1

Location	Invert (m)	100-Year Elev. (m)	Head (m)	Q_{100} (L/s)	Max Volume (m^3)	Hydrovex Unit
Tank B	84.37	85.89	1.52	0.013	30.4	100-VHV-1
Tank C	84.31	85.85	1.54	0.013	67.7	100-VHV-1
Tank D	84.81	86.37	1.56	0.013	32.6	100-VHV-1
Tank E	84.21	85.74	1.53	0.013	35.0	125-VHV-2
Tank F	84.18	85.69	1.51	0.023	162.8	125-VHV-2
CB01	85.41	87.74	2.33	0.019	2.3	125-VHV-2
RYCB01	84.85	87.17	2.32	0.056	2.4	200-VHV-2

3.3 WATER QUALITY

As outlined in Section 1.4, it is required that post development runoff be treated to achieve 80% TSS removal.

Proposed features to achieve these targets include:

- → Suitably sized oil and grit separator (OGS) unit (FDC-3HC or equivalent)
- → Stormtech Isolator Row Plus
- → Grass swales

As noted previously, a single outlet location into the Kennedy Lane East sewer is proposed. A suitably sized OGS unit is proposed to achieve a minimum 80% TSS removal. Hydro First Defense (FDC-3HC, or equivalent) is proposed to meet the requirements, and details on the proposed unit can be found in **Appendix D**.

The majority of roadway and parking lot runoff will be routed to one of six proposed underground Stormtech (or equivalent) storage units. The units are proposed to include a Stormtech Isolator Row Plus filtration devices to further improve the water quality through a treatment train approach. ETV Canada testing on Stormtech Isolator Row Plus units verified the filtration device is capable of achieving an average 82% TSS removal.

It is assumed that the runoff from pervious rear yard areas will be free of typical sediment-generating activities and therefore runoff will leave them effectively unchanged and can be considered clean for the purposes of water quality assessment. Additionally, it should be noted that runoff from the rear yards along the property line of the site will be captured and conveyed towards the outlet (and OGS) via grass swales. Grass swales are vegetated open channels that convey, treat and attenuate stormwater runoff.

4 CONCLUSIONS

A stormwater management report has been prepared to support the proposed development at 360 Kennedy Lane East in the City of Ottawa. The key points are summarized below.

WATER QUALITY

An OGS unit (Hydro First Defense FD-3HC, or equivalent) is proposed at the outlet to the Kennedy Lane East Sewer along with Stormtech Isolator Row Plus filtration devices at each storage unit to meet MOE Enhanced treatment standards (80% TSS removal) through a treatment train approach. In addition, the enhanced grass swales will provide additional quality control.

WATER QUANTITY

Runoff will be controlled primary in underground storage chambers with outflow controlled using ICDs, in addition to surface storage where grading allows.

APPENDIX

A

PRE-CONSULTATION MEETING MINUTES AND TECHNICAL COMMENTS

Site Plan Pre- Application Consultation Notes

Date: Wednesday, May 19, 2021
Site Location: 360 Kennedy Lane E
Type of Development: ☒ Residential (☒ townhomes, ☒ stacked, ☐ singles, ☐ apartments), ☐ Office Space, ☐ Commercial, ☐ Retail, ☐ Institutional, ☐ Industrial, Other: N/A

Infrastructure

Water

Existing public services:

- Kennedy Lane E – 203mm DI

Watermain Frontage Fees to be paid ($190.00 per metre) on Woodroffe Avenue ☐ **Yes** ☒ **No**

Boundary conditions:
Civil consultant must request boundary conditions from the City's assigned Project Manager prior to first submission.

- Water boundary condition requests must include the location of the service(s) and the expected loads required by the proposed developments. Please provide all the following information:
    - Location of service(s)
    - Type of development and the amount of fire flow required (as per FUS, 1999)
    - Average daily demand: ___ L/s
    - Maximum daily demand: ___ L/s
    - Maximum hourly daily demand: ___ L/s
- Fire protection (Fire demand, Hydrant Locations)
- Please submit sanitary demands with the water boundary conditions

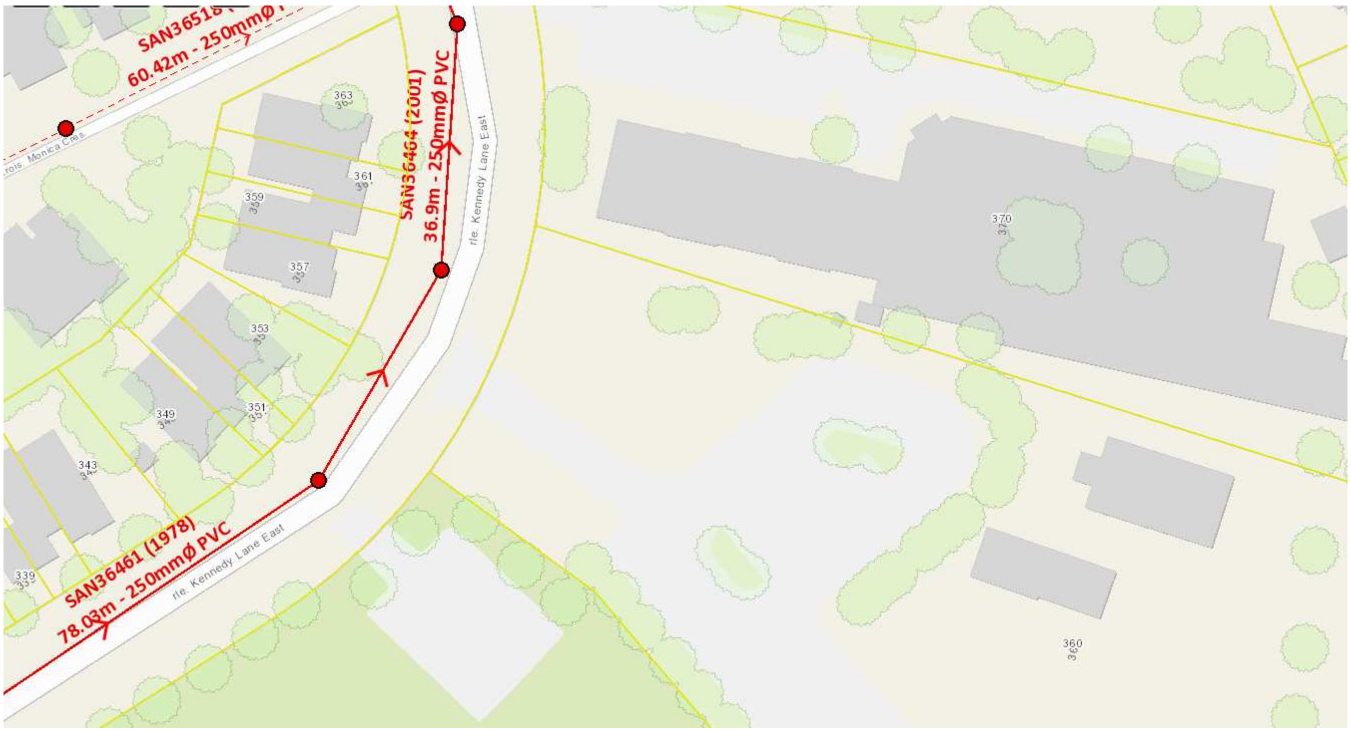
General comments

- Service areas with a basic demand greater than 50 m^3/day shall be connected with a minimum of two water services, separated by an isolation valve, to avoid creation of vulnerable service area.
- A District Metering Area Chamber (DMA) is required for new services 150mm or greater in diameter.

Sanitary Sewer

Existing public services:

- Kennedy Lane E – 250mm PVC



Is a monitoring manhole required on private property? ☒ **Yes** ☐ **No**

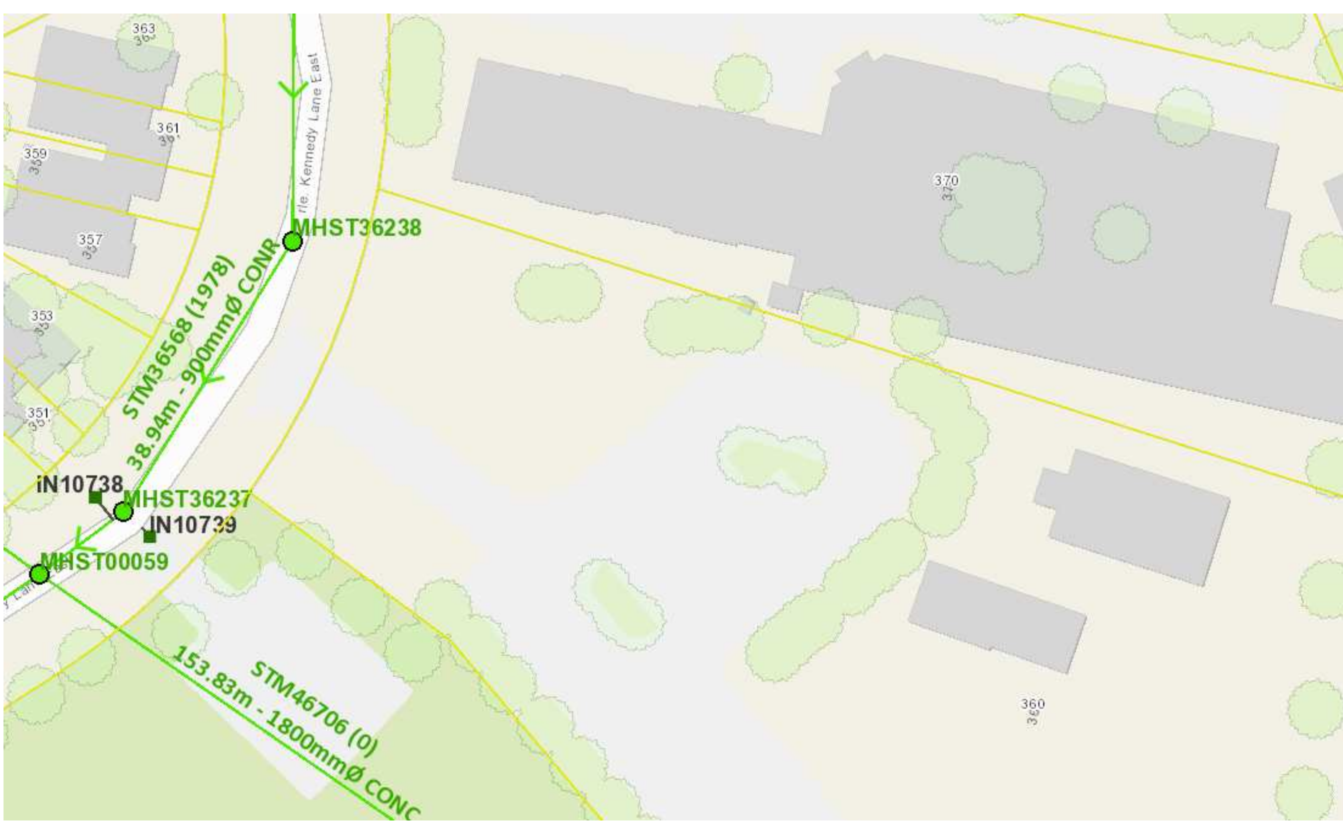
General comments

- Please submit sanitary demands with the water boundary conditions
- For infill developments within older neighbourhoods there is not an allotment for the sanitary capacity. As part of the rezoning application the consultant is required to demonstrate that there is sufficient capacity in the pipe network and system for the proposed sanitary demands.

Storm Sewer

Existing public services:

- Kennedy Lane E – 900mm Conc R



Stormwater Management

Quality Control:

- Rideau Valley Conservation Authority to confirm quality control requirements.

Quantity Control:

- LID features are strongly encouraged as the development is going from mostly pervious to impervious.
- Time of concentration (Tc): Tc = pre-development; maximum Tc = 10 min
- Allowable run-off coefficient: 0.5
- Allowable flowrate: Allowable flowrate: Control the 100-year storm events to the 5-year storm event.

Ministry of Environment, Conservation and Parks (MECEP)

All development applications should be considered for an Environmental Compliance Approval, under MECP regulations.

a. Consultants are required to determines if an approval for sewage works under Section 53 of OWRA is required.
b. ECA applications are required to be submitted online through the MECP portal. A business account required to submit ECA application. For more information visit https://www.ontario.ca/page/environmental-compliance-approval
c. If the consultants determines the site does not meet the definition of industrial site the consultant may request the MECP to exempt the works. The following information must be provided to the City Project Manager:
   (i) is designed to service one lot or parcel of land;
   (ii) discharges into a storm sewer that is not a combined sewer;
   (iii) does not service industrial land or a structure located on industrial land; and
   (iv) is not located on industrial land.

NOTE: Site Plan Approval, or Draft Approval, is required before any Ministry of the Environment and Climate Change (MOECC) application is sent

General Service Design Comments

- Existing sewers or watermains that are not reused must be decommissioned as per City Standards.
- The City of Ottawa Standard Detail Drawings should be referenced where possible for all work within the Public Right-of-Way.

Other

Capital Works Projects within proximity to application? ☐ **Yes** ☒ **No**

References and Resources

- As per section 53 of the Professional Engineers Act, O. Reg 941/40, R.S.O. 1990, all documents prepared by engineers must be signed and dated on the seal.
- All required plans & reports are to be provided in *.pdf format (at application submission and for any, and all, re-submissions)
- Please find relevant City of Ottawa Links to Preparing Studies and Plans below: https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#standards-policies-and-guidelines
- To request City of Ottawa plan(s) or report information please contact the City of Ottawa Information Centre:
  InformationCentre@ottawa.ca<mailto:InformationCentre@ottawa.ca>
  (613) 580-2424 ext. 44455
- geoOttawa
  http://maps.ottawa.ca/geoOttawa/

SITE PLAN APPLICATION – Municipal servicing

For information on preparing required studies and plans refer to:
http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENGINEERING		S/A	Number of copies
S		1. Site Servicing Plan	2. Site Servicing Report	S	
S		3. Grade Control and Drainage Plan	4. Geotechnical Study Alternatively, existing report with memo providing recommendations for works based on current geotechnical guidelines.	S	
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
		9. Community Transportation Study and/or Transportation Impact Study / Brief	10. Erosion and Sediment Control Plan / Brief	S	
S		11. Storm water Management Report	12. Hydro-geological and Terrain Analysis		
		13. Water main Analysis	14. Noise / Vibration Study	S	
		15. Roadway Modification Design Plan	16. Confederation Line Proximity Study		

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, City Planning will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the City.

Notes:

4. Geotechnical Study / Slope Stability Study – required as per Official Plan section 4.8.3. All site plan applications need to demonstrate the soils are suitable for development. A Slope Stability Study may be required with unique circumstances (Schedule K or topography may define slope stability concerns).

10. Erosion and Sediment Control Plan – required with all site plan applications as per Official Plan section 4.7.3.

11. Stormwater Management Report/Brief - required with all site plan applications as per Official Plan section 4.7.6.

REZONING APPLICATION – Municipal servicing

For information on preparing required studies and plans refer to:
http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

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		7. Servicing Options Report	8. Wellhead Protection Study		
		9. Community Transportation Study and/or Transportation Impact Study / Brief	10. Erosion and Sediment Control Plan / Brief	S	
S		11. Storm water Management Report	12. Hydro-geological and Terrain Analysis		
		13. Water main Analysis	14. Noise / Vibration Study	S	
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Kerker, Kathryn

From:	Yang, Winston
Sent:	September 6, 2022 11:18 AM
To:	Polyak, Alex
Cc:	Kerker, Kathryn
Subject:	FW: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

FYI

Ding Bang (Winston) Yang, P.Eng.
Senior Engineer
Municipal Engineering – Ottawa

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M+ 1 647-628-8108

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Ottawa, Ontario,
K2B 8K2 Canada

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From: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Sent: November 8, 2021 3:19 PM
To: Yang, Winston <Winston.Yang@wsp.com>
Subject: RE: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

Hi Winston,

As you stated the flow rates are very small. Option 1 should be used even though it results in a smaller storage volume.

Rubina

--
Rubina Rasool, E.I.T.
Project Manager
Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique
Development Review – East Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

---

From: Yang, Winston <Winston.Yang@wsp.com>
Sent: November 08, 2021 11:16 AM

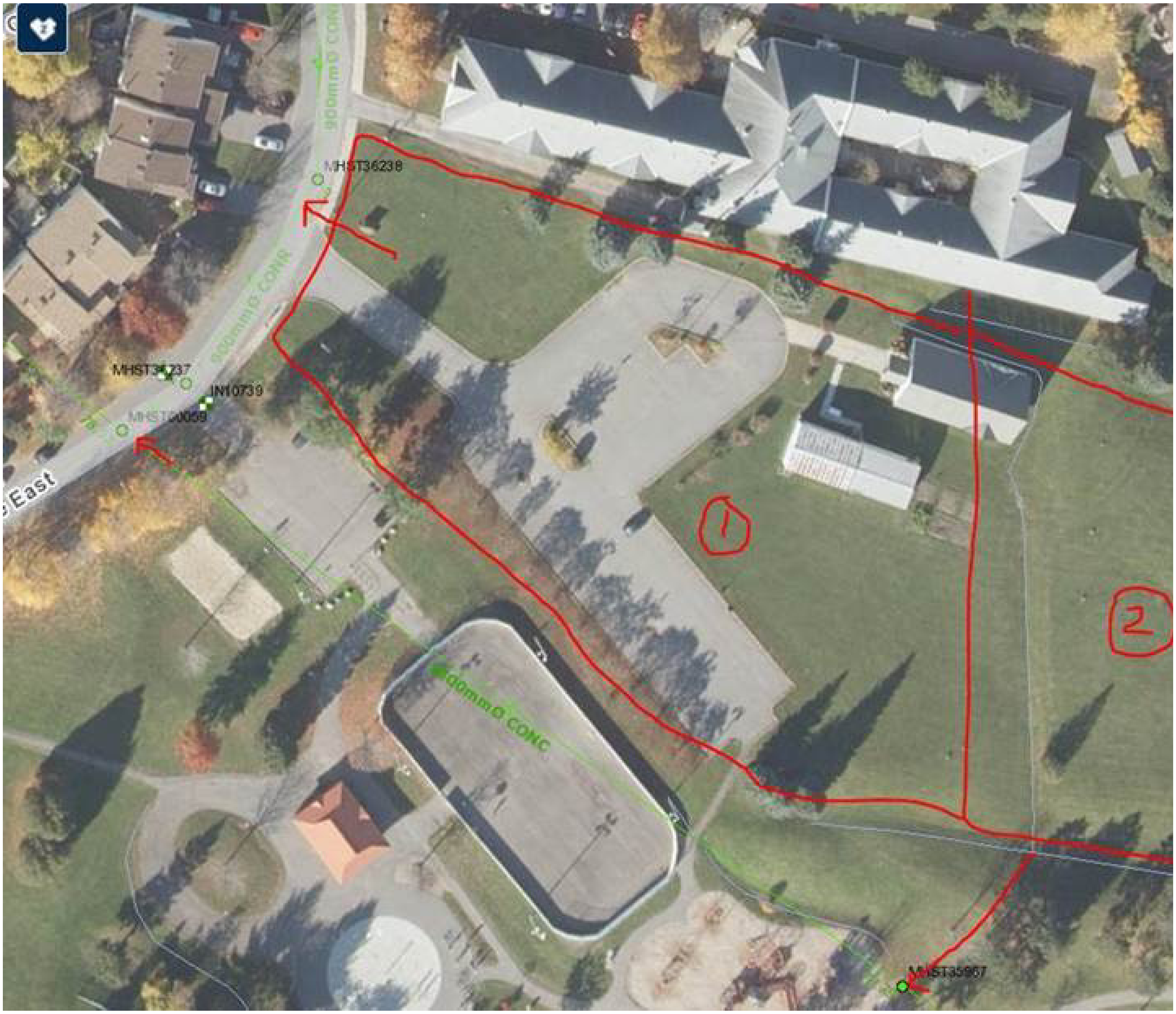
To: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Subject: RE: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

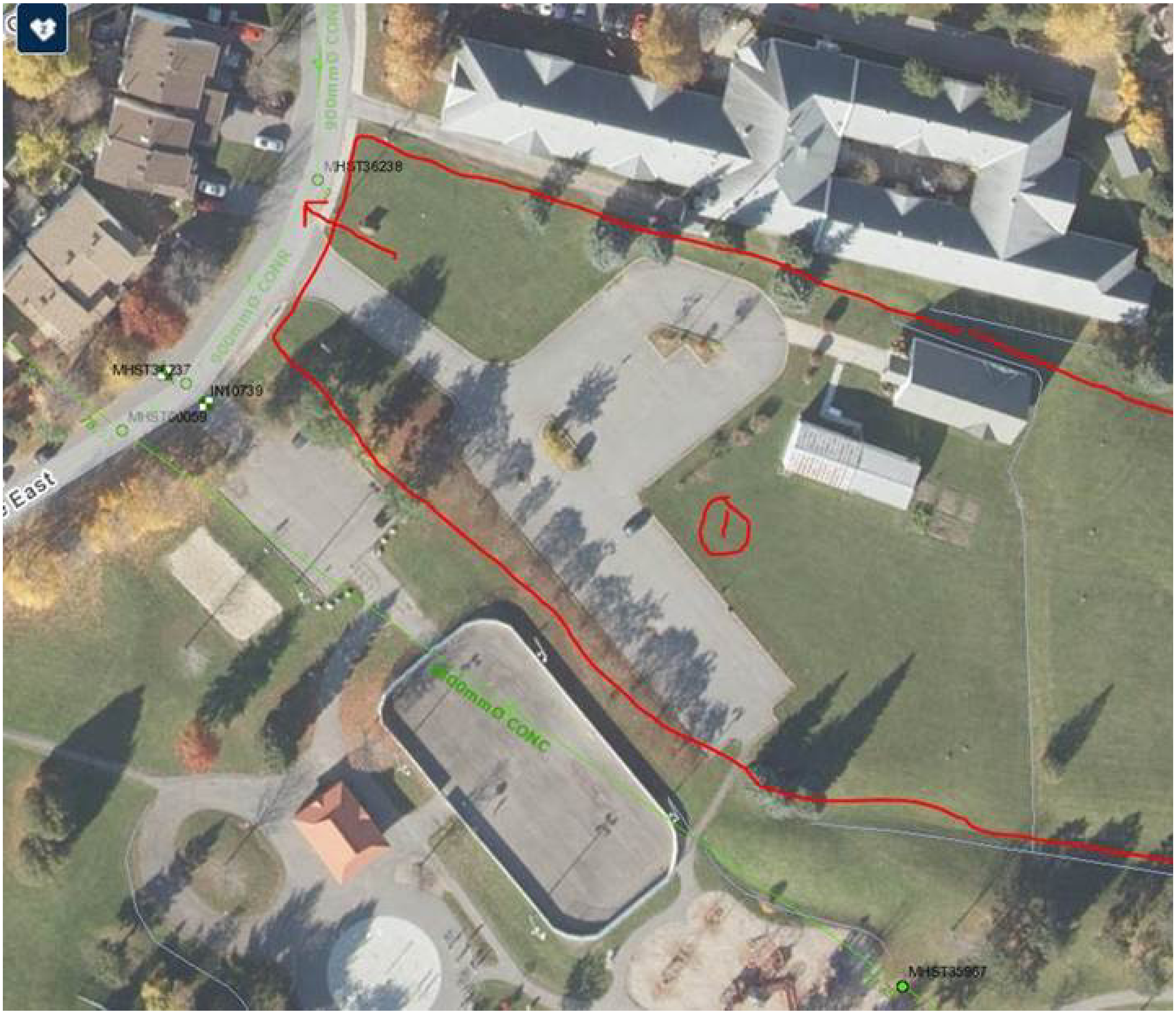
ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi Rubina,

For existing conditions, the west portion of the site (1) discharges to the sewer on Kennedy Lane E, while the east portion of the site (2) discharges to the sewer through the park (which ultimately connects to the sewer on Kennedy Lane) as shown below:



In proposed conditions we have the entire site discharging to the sewer on Kennedy Lane.



My questions is whether we need to control the post development discharge to the 5-year pre-development analyzing the site as a whole (1 and 2 combined) or control to the 5-year pre-development for just subcatchment 1, impacts the overall storage requirement. However, as subcatchment 2 is primarily grass area it does not make a significant difference. In summary:

Pre-development 5-year flow (1 and 2) **= 0.12m3/s**
Pre-development 5 year (1) = **0.099m3/s**

Storage requirement to control 100-year post development to 0.12m3/s ~ **310m3/s**
Storage requirement to control 100-year post development to 0.099m3/s ~ **350m3/s**

Can you please confirm which scenarios we should use for the SWM calculation?

Thanks,

Ding Bang (Winston) Yang, P.Eng.
Project Engineer
Municipal Engineering - Ottawa

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M+ 1 647-628-8108

WSP Canada Inc.
2611 Queensview Drive, Suite 300
Ottawa, Ontario,
K2B 8K2 Canada

wsp.com

From: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Sent: November 8, 2021 11:12 AM
To: Yang, Winston <Winston.Yang@wsp.com>
Subject: RE: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

As part of the development application the site would be required to connect to Kennedy Lane E and the overland flows would also need to be directed towards the street.

Rubina
--
Rubina Rasool, E.I.T.
Project Manager
Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique
Development Review – East Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

From: Yang, Winston <Winston.Yang@wsp.com>
Sent: November 08, 2021 10:46 AM
To: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Subject: RE: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

Hi Rubina,

Please see attached pdfs for the FUS. I have also attached architectural site plan for your reference.

And I have a question for Stormwater Management. Currently the grass area of the site is draining toward the existing ditch and picked up by the existing CB located at the park south of the site.

Can I use the entire site to calculate the pre-development allowable release rate to Kennedy Lane east or only use half of the site for our consideration since half of the site is draining toward Kennedy Lane East and half of the site is draining toward the park?

Ding Bang (Winston) Yang, P.Eng.
Project Engineer
Municipal Engineering - Ottawa

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Ottawa, Ontario,
K2B 8K2 Canada

wsp.com

From: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Sent: November 8, 2021 9:24 AM
To: Yang, Winston <Winston.Yang@wsp.com>
Subject: RE: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East

Hi Winston,

I will circulate the water boundary conditions; however, I will have to take a closer look at the FUS calculations. The development is similar to a subdivision and Technical Bulletin 2018-02 (attached) allows for 10,000 L/min if minimum separation distances are provided.

Rubina
--
Rubina Rasool, E.I.T.
Project Manager
Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique
Development Review – East Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

---

From: Yang, Winston <Winston.Yang@wsp.com>
Sent: November 04, 2021 10:37 AM
To: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Subject: Boundary Condition Request - Queenswood United Church PAR - 360 Kennedy Lane East
Importance: High

Hi Rubina,

As per the pre-consultation meeting direction, here is the water supply boundary condition request for the proposed residential development by Queenswood United Church at 360 Kennedy Lane East at Orlean.
The proposed development will be serviced from the existing 203mm diameter watermain from Kennedy Lane that as per pre-consult meeting minute where the water service from the development will be connected to the existing 203mm diameter watermain along Kennedy Lane East.

The proposed residential development consists of 21 two storey and 60 three storey Townhouse units . There are two existing public fire hydrants at Kennedy Lane East next to the subjected site. Multiple private fire hydrants will be proposed on site.

The domestic water demands were calculated using the City of Ottawa's Water Design Guidelines and fire demand were calculated using FUS 1999.
The results are summarized as follow:

Proposed development	Average Daily Demand (l/s)	Maximum Daily Demand (l/s)	Maximum Hourly Demand (l/s)	Fire Demand (l/min)
Queenswood UC PAR	0.65	1.62	3.56	16000

I have also attached the FUS calculation spreadsheet for the most fire flow required for your review. The proposed onsite water service is to be designed to connect to the existing 203mm water service pipe on the Kennedy Lane East as shown on the attached sketch for your reference. Two connections to the existing 203 W/M are required as the basic demand exceed 50 m^3/day

The sanitary total flow from the site is 2.68 L/s. The spreadsheet is attached for your reference.

If you have the report and drawings please send them to me.

Thank you,

Ding Bang (Winston) Yang, P.Eng.
Project Engineer
Municipal Engineering - Ottawa

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'

Kerker, Kathryn

From: Yang, Winston
Sent: September 6, 2022 11:21 AM
To: Polyak, Alex
Cc: Kerker, Kathryn
Subject: FW: Water Quality Requirements - Site Development- 360 Kennedy Lane E

FYI

Ding Bang (Winston) Yang, P.Eng.
Senior Engineer
Municipal Engineering – Ottawa

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K2B 8K2 Canada

wsp.com

From: Jadallah, Ayham <Ayham.Jadallah@wsp.com>
Sent: November 17, 2021 8:02 AM
To: O'Neill, Meaghan <Meaghan.ONeill@wsp.com>; Yang, Winston <Winston.Yang@wsp.com>
Cc: Hughes, Michelle <Michelle.Hughes@wsp.com>
Subject: FW: Water Quality Requirements - Site Development- 360 Kennedy Lane E

Hi,

Please find below the response from CA and note that CLI approach might be applicable.

Thanks,
Ayham

From: Jamie Batchelor <jamie.batchelor@rvca.ca>
Sent: Tuesday, November 16, 2021 9:07 PM
To: Jadallah, Ayham <Ayham.Jadallah@wsp.com>
Cc: Emma Bennett <emma.bennett@rvca.ca>
Subject: Water Quality Requirements - Site Development- 360 Kennedy Lane E

Good Evening Ayham,

Based on the distance to the downstream outlet to the Ottawa River, the water quality target would be 80% TSS removal. Any stormwater management plan must conform to the 2003 MOE Stormwater Management Planning and Design Manual and any other relevant guiding documents that may be in place at the time of the official submission. The opportunity for LID measures should be explored for any proposed stormwater management plan. Specific attention will need to be placed on water budget/balance and the items mentioned above. It should be noted that these requirements are already within the existing 2003 MOE Design Manual.

The new consolidated linear infrastructure ECA approach from the Ministry of Environment, Conservation and Parks has an implementation scheduled for summer 2021. Therefore, based on the projected timeframe for this project, it may form part of the City's ECA for which the following criteria is noted:

• Water balance or runoff volume control to the 90th percentile
• OGS units will only address 50% treatment
• Other items identified in the new consolidated linear infrastructure ECA

Therefore, the applicant is strongly encouraged to design accordingly within their stormwater management approach.

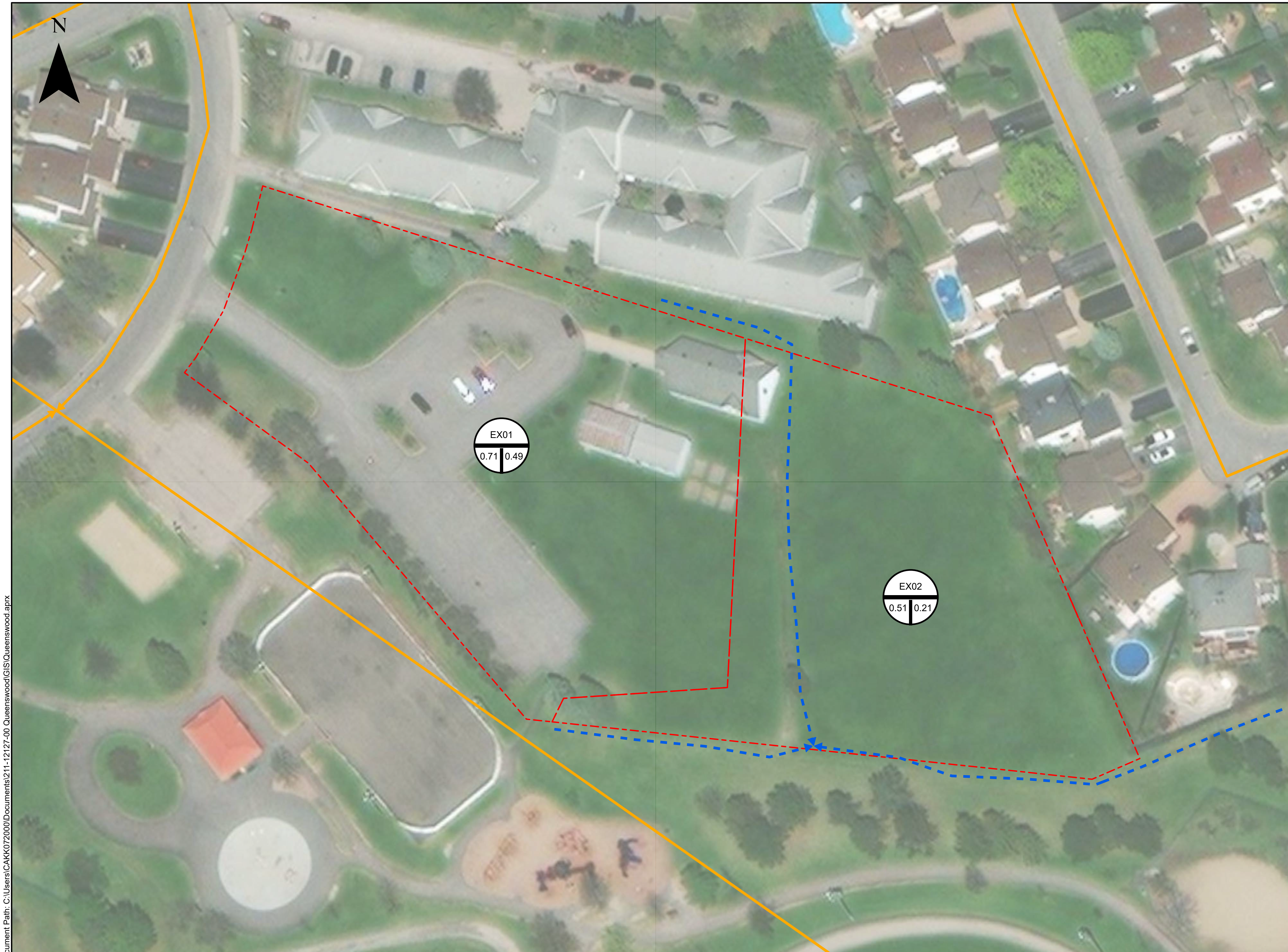
Jamie Batchelor, MCIP, RPP
Planner, ext. 1191
Jamie.batchelor@rvca.ca

Rideau Valley Conservation Authority

3889 Rideau Valley Drive
PO Box 599, Manotick ON K4M 1A5
T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

APPENDIX

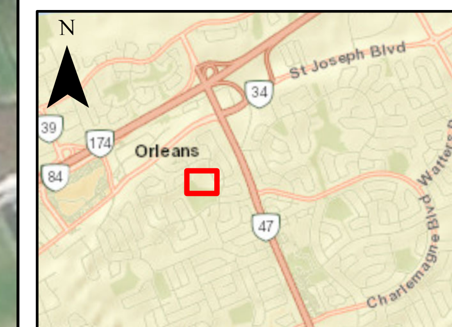
B EXHIBITS



CLIENT
QUEENSWOOD UNITED CHURCH

PROJECT
360 KENNEDY LANE EAST DEVELOPMENT

TITLE
EXHIBIT 1
EXISTING CONDITIONS
DRAINAGE MOSAIC



LEGEND

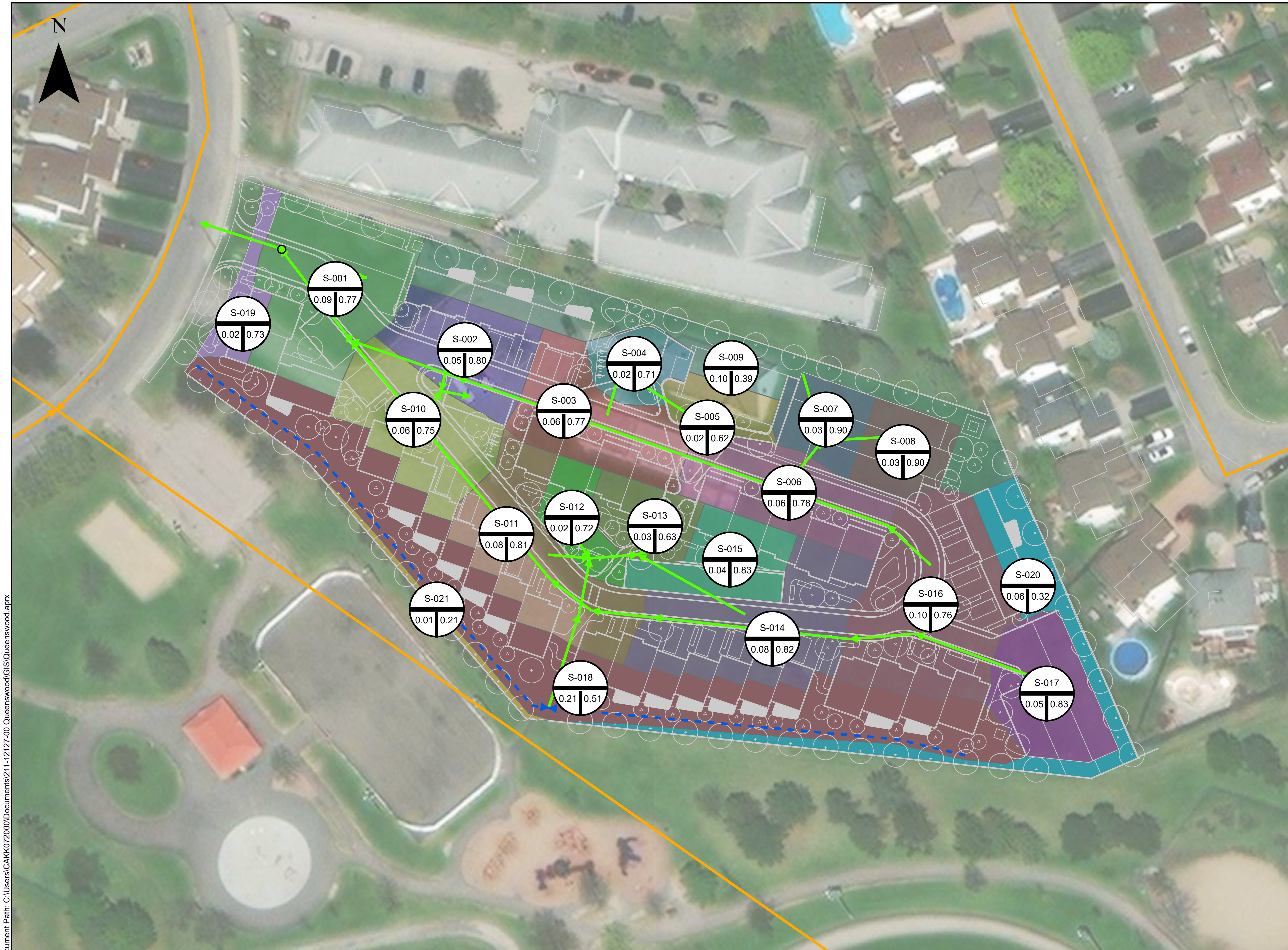
- Existing Swale
- Existing Storm
- Drainage Area

Subcatchment ID
Area (ha) | Runoff C

0 0.01 0.01 km

DRAWN BY KK
CHECKED BY AJ
September 2022
NAD 1983 MTM 9
1:750

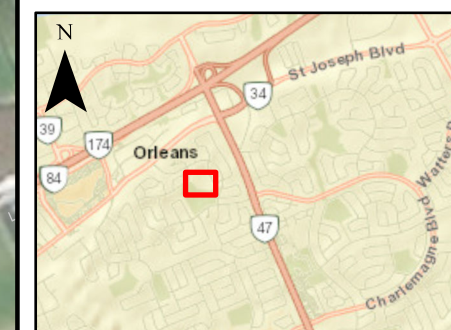
Document Path: C:\Users\CAKK072000\Documents\211-12127-00 Queenswood\GIS\Queenswood.aprx



CLIENT
QUEENSWOOD UNITED CHURCH

PROJECT
360 KENNEDY LANE EAST DEVELOPMENT

TITLE
EXHIBIT 2
PROPOSED CONDITIONS
DRAINAGE MOSAIC



LEGEND

OGS
Proposed Storm
Proposed Swale
Existing Storm

Drainage Areas
S-001
S-002
S-003
S-004
S-005
S-006
S-007
S-008
S-009
S-010
S-011
S-012
S-013
S-014
S-015
S-016
S-017
S-018
S-019
S-020
S-021

Subcatchment ID
Area (ha) | Runoff C

0 0.01 0.01 km

DRAWN BY KK
CHECKED BY AJ
September 2022
NAD 1983 MTM 9
1:750
wsp

Document Path: C:\Users\CAKK072000\Documents\211-12127-00 Queenswood\GIS\Queenswood.aprx

APPENDIX

C

CALCULATIONS & PCSWMM OUTPUT

5-Year Pre-Development

```
  EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)
  --------------------------------------------------------------


  *************
  Element Count
  *************
  Number of rain gages ...... 16
  Number of subcatchments ... 2
  Number of nodes ........... 1
  Number of links ........... 0
  Number of pollutants ...... 0
  Number of land uses ....... 0


  ****************
  Raingage Summary
  ****************
                                                     Data       Recording
  Name                 Data Source                   Type       Interval 
  -----------------------------------------------------------------------
  100yr_3hr_Chicago    100yr_3hr_Chicago             INTENSITY   10 min.
  100yr_3hr_Chicago_Climate_Change 100yr_3hr_Chicago_Increase_20percent INTENSITY   10 min.
  100yr_6hr_Chicago    100yr_6hr_Chicago             INTENSITY   10 min.
  100yr_6hr_Chicago_Climate_Change 100yr_6hr_Chicago_Increase_20percent INTENSITY   10 min.
  10yr_3hr_Chicago     10yr_3hr_Chicago              INTENSITY   10 min.
  10yr_6hr_Chicago     10yr_6hr_Chicago              INTENSITY   10 min.
  25mm_3hr_Chicago     25mm_3hr_Chicago              INTENSITY   10 min.
  25mm_4hr_Chicago     25mm_4hr_Chicago              INTENSITY   10 min.
  25yr_3hr_Chicago     25yr_3hr_Chicago              INTENSITY   10 min.
  25yr_6hr_Chicago     25yr_6hr_Chicago              INTENSITY   10 min.
  2yr_3hr_Chicago      2yr_3hr_Chicago               INTENSITY   10 min.
  2yr_6hr_Chicago      2yr_6hr_Chicago               INTENSITY   10 min.
  50yr_3hr_Chicago     50yr_3hr_Chicago              INTENSITY   10 min.
  50yr_6hr_Chicago     50yr_6hr_Chicago              INTENSITY   10 min.
  5yr_3hr_Chicago      5yr_3hr_Chicago               INTENSITY   10 min.
  5yr_6hr_Chicago      5yr_6hr_Chicago               INTENSITY   10 min.


  ********************
  Subcatchment Summary
  ********************
  Name                       Area     Width   %Imperv    %Slope Rain Gage            Outlet              
  -----------------------------------------------------------------------------------------------------------
  S1_1                       0.51     68.63      6.56    7.7320 5yr_3hr_Chicago      OF1                 
  S1_4                       0.71     58.98     44.27    3.4980 5yr_3hr_Chicago      OF1                 


  ************
  Node Summary
  ************
                                           Invert      Max.    Ponded    External
  Name                 Type                 Elev.     Depth      Area    Inflow  
  -------------------------------------------------------------------------------
  OF1                  OUTFALL              83.72      0.00       0.0


  *********************************************************
  NOTE: The summary statistics displayed in this report are
  based on results found at every computational time step,  
  not just on results from each reporting time step.
  *********************************************************

  ****************
  Analysis Options
  ****************
  Flow Units ............... CMS
  Process Models:
    Rainfall/Runoff ........ YES
    RDII ................... NO
    Snowmelt ............... NO
    Groundwater ............ NO
    Flow Routing ........... NO
    Water Quality .......... NO
  Infiltration Method ...... HORTON
  Starting Date ............ 11/10/2013 00:00:00
  Ending Date .............. 11/10/2013 06:00:00
  Antecedent Dry Days ...... 0.0
  Report Time Step ......... 00:05:00
  Wet Time Step ............ 00:05:00
  Dry Time Step ............ 00:05:00


  **************************        Volume         Depth
  Runoff Quantity Continuity     hectare-m            mm
  **************************     ---------       -------
  Total Precipitation ......         0.052        42.514
  Evaporation Loss .........         0.000         0.000
  Infiltration Loss ........         0.034        28.035
  Surface Runoff ...........         0.017        14.190
  Final Storage ............         0.001         0.449
  Continuity Error (%) .....        -0.374


  **************************        Volume        Volume
  Flow Routing Continuity        hectare-m      10^6 ltr
  **************************     ---------     ---------
  Dry Weather Inflow .......         0.000         0.000
  Wet Weather Inflow .......         0.017         0.173
  Groundwater Inflow .......         0.000         0.000
  RDII Inflow ..............         0.000         0.000
  External Inflow ..........         0.000         0.000
  External Outflow .........         0.017         0.173
  Flooding Loss ............         0.000         0.000
  Evaporation Loss .........         0.000         0.000
  Exfiltration Loss ........         0.000         0.000
  Initial Stored Volume ....         0.000         0.000
  Final Stored Volume ......         0.000         0.000
  Continuity Error (%) .....         0.000


  ***************************
  Subcatchment Runoff Summary
  ***************************

  ------------------------------------------------------------------------------------------------------------
                              Total      Total      Total      Total      Total       Total     Peak  Runoff
                             Precip      Runon       Evap      Infil     Runoff      Runoff   Runoff   Coeff
  Subcatchment                   mm         mm         mm         mm         mm    10^6 ltr      CMS
  ------------------------------------------------------------------------------------------------------------
  S1_1                        42.51       0.00       0.00      36.36       6.17        0.03     0.02   0.145
  S1_4                        42.51       0.00       0.00      21.98      20.02        0.14     0.10   0.471


  Analysis begun on:  Thu Sep 08 13:04:14 2022
  Analysis ended on:  Thu Sep 08 13:04:14 2022
  Total elapsed time: < 1 sec
```

100-Year Post-Development

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 16
Number of subcatchments ... 22
Number of nodes 40
Number of links 40
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
100yr_3hr_Chicago	100yr_3hr_Chicago	INTENSITY	10 min.
100yr_3hr_Chicago_Climate_Change	100yr_3hr_Chicago_Increase_20percent	INTENSITY	10 min.
100yr_6hr_Chicago	100yr_6hr_Chicago	INTENSITY	10 min.
100yr_6hr_Chicago_Climate_Change	100yr_6hr_Chicago_Increase_20percent	INTENSITY	10 min.
10yr_3hr_Chicago	10yr_3hr_Chicago	INTENSITY	10 min.
10yr_6hr_Chicago	10yr_6hr_Chicago	INTENSITY	10 min.
25mm_3hr_Chicago	25mm_3hr_Chicago	INTENSITY	10 min.
25mm_4hr_Chicago	25mm_4hr_Chicago	INTENSITY	10 min.
25yr_3hr_Chicago	25yr_3hr_Chicago	INTENSITY	10 min.
25yr_6hr_Chicago	25yr_6hr_Chicago	INTENSITY	10 min.
2yr_3hr_Chicago	2yr_3hr_Chicago	INTENSITY	10 min.
2yr_6hr_Chicago	2yr_6hr_Chicago	INTENSITY	10 min.
50yr_3hr_Chicago	50yr_3hr_Chicago	INTENSITY	10 min.
50yr_6hr_Chicago	50yr_6hr_Chicago	INTENSITY	10 min.
5yr_3hr_Chicago	5yr_3hr_Chicago	INTENSITY	10 min.
5yr_6hr_Chicago	5yr_6hr_Chicago	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S-001	0.09	43.67	82.26	2.9480	100yr_3hr_Chicago	DCB15
S-002	0.05	26.46	87.00	2.6160	100yr_3hr_Chicago	CB014
S-003	0.06	29.50	82.05	2.6610	100yr_3hr_Chicago	CB010
S-004	0.02	17.87	74.45	4.8050	100yr_3hr_Chicago	CB12
S-005	0.02	9.51	62.46	2.6780	100yr_3hr_Chicago	CB11
S-006	0.06	24.34	84.15	2.3870	100yr_3hr_Chicago	CB08
S-007	0.03	11.29	100.00	2.6880	100yr_3hr_Chicago	CBMH13
S-008	0.03	21.33	100.00	3.6100	100yr_3hr_Chicago	CB09
S-009	0.10	8.68	30.73	6.3810	100yr_3hr_Chicago	CBMH13
S-010	0.06	34.38	79.08	2.8970	100yr_3hr_Chicago	CB13
S-011	0.08	28.81	87.27	2.7540	100yr_3hr_Chicago	CB006
S-012	0.02	17.93	75.22	2.7540	100yr_3hr_Chicago	CB005
S-013	0.03	15.90	62.68	2.5890	100yr_3hr_Chicago	CB004
S-014	0.08	31.00	88.69	2.2900	100yr_3hr_Chicago	CB002
S-015	0.04	16.11	90.26	2.3210	100yr_3hr_Chicago	TankF
S-016	0.10	40.41	81.00	2.6420	100yr_3hr_Chicago	TankD
S-017	0.05	32.24	90.69	4.3700	100yr_3hr_Chicago	CB01
S-018	0.11	11.63	46.71	5.6740	100yr_3hr_Chicago	RYCB01
S-018-2	0.10	12.56	46.71	6.4410	100yr_3hr_Chicago	RYCB01
S-019	0.02	9.54	76.25	3.4660	100yr_3hr_Chicago	OF1
S-020	0.06	58.55	21.52	1.5000	100yr_3hr_Chicago	OF1
S-021	0.01	59.16	5.84	2.0000	100yr_3hr_Chicago	OF1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CB002	JUNCTION	84.35	3.17	0.0	
CB004	JUNCTION	85.45	2.20	0.0	
CB005	JUNCTION	85.30	2.20	0.0	
CB006	JUNCTION	85.15	2.50	0.0	
CB010	JUNCTION	84.44	3.09	0.0	
CB014	JUNCTION	85.29	2.20	0.0	
CB08	JUNCTION	84.40	3.16	0.0	
CB09	JUNCTION	85.12	2.20	0.0	
CB11	JUNCTION	84.75	2.20	0.0	
CB12	JUNCTION	85.10	2.20	0.0	
CB13	JUNCTION	84.35	3.14	0.0	
CBMH05	JUNCTION	84.25	3.37	0.0	
CBMH13	JUNCTION	84.76	2.41	0.0	
CBMH18	JUNCTION	84.24	3.32	0.0	
DCB15	JUNCTION	84.20	3.20	0.0	
J3	JUNCTION	83.51	4.08	0.0	
J4	JUNCTION	83.63	4.08	0.0	
J5	JUNCTION	83.51	4.16	0.0	
J6	JUNCTION	83.43	4.21	0.0	
J8	JUNCTION	84.85	3.20	0.0	
J9	JUNCTION	83.94	3.73	0.0	
STMH01	JUNCTION	84.00	3.66	0.0	
STMH04	JUNCTION	84.45	3.31	0.0	
STMH07	JUNCTION	83.49	4.20	0.0	
STMH08	JUNCTION	83.43	4.23	0.0	
STMH101	JUNCTION	82.87	4.68	0.0	
STMH102	JUNCTION	83.18	4.43	0.0	
STMH105	JUNCTION	83.68	4.09	0.0	
STMH106	JUNCTION	83.86	3.81	0.0	
STMH108	JUNCTION	83.58	4.17	0.0	
STMH109	JUNCTION	83.79	3.81	0.0	
OF1	OUTFALL	82.20	0.38	0.0	
CB01	STORAGE	85.41	2.50	0.0	
RYCB01	STORAGE	84.85	2.34	0.0	
TankA	STORAGE	84.06	3.49	0.0	
TankB	STORAGE	84.37	3.26	0.0	
TankC	STORAGE	84.31	3.21	0.0	
TankD	STORAGE	84.81	2.90	0.0	
TankE	STORAGE	84.21	3.37	0.0	
TankF	STORAGE	84.18	3.49	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	CB12	TankB	CONDUIT	4.7	1.0001	0.0130
C1_1	STMH102	J3	CONDUIT	1.8	-18.4482	0.0130
C1_2	J3	STMH101	CONDUIT	23.0	2.2266	0.0130
C10	STMH109	STMH108	CONDUIT	11.7	0.6845	0.0130
C11	CBMH05	TankF	CONDUIT	8.4	0.4497	0.0130
C11_1	STMH108	J4	CONDUIT	25.6	0.3127	0.0130
C11_2	J5	J6	CONDUIT	26.2	0.3059	0.0130
C11_3	J4	J5	CONDUIT	38.5	0.3120	0.0130
C11_5	J6	STMH102	CONDUIT	22.5	0.8461	0.0130
C12	CB004	TankF	CONDUIT	4.0	1.0127	0.0130
C13	CBMH18	TankE	CONDUIT	3.5	0.4507	0.0130
C2	STMH01	STMH106	CONDUIT	25.8	0.4259	0.0130
C3	STMH106	STMH105	CONDUIT	13.1	0.4577	0.0130
C4	STMH101	OF1	CONDUIT	16.8	3.9941	0.0100
C4_1	STMH105	J9	CONDUIT	39.2	-0.6758	0.0130
C4_3	J9	STMH07	CONDUIT	12.3	3.4478	0.0130
C5	STMH07	STMH08	CONDUIT	10.1	0.2977	0.0130
C6	CB11	TankB	CONDUIT	9.8	1.0205	0.0130
C6_4	STMH08	STMH102	CONDUIT	61.9	0.3072	0.0130
C7	J8	STMH04	CONDUIT	19.0	1.0506	0.0130
C8	STMH04	CBMH05	CONDUIT	11.3	0.3536	0.0100
C9	CB005	CBMH05	CONDUIT	7.6	1.0532	0.0130
CB011	CB014	CBMH18	CONDUIT	8.5	0.7081	0.0130
CB012	DCB15	TankA	CONDUIT	8.3	1.0844	0.0130
CB014	CB09	TankC	CONDUIT	7.1	0.9860	0.0130
CB02	CB13	CBMH18	CONDUIT	8.8	0.5683	0.0130
CB08	CBMH13	TankC	CONDUIT	9.7	0.4145	0.0130
CB09	CB08	TankC	CONDUIT	7.6	1.0527	0.0130
ICD_010	CB010	TankB	CONDUIT	3.3	0.9091	0.0130
ICD_03	CB006	CBMH05	CONDUIT	8.3	1.8064	0.0130
ICD_05	CB002	TankF	CONDUIT	4.4	1.0228	0.0130
W1	J8	RYCB01	WEIR			
ICD_06	CB01	STMH01	OUTLET			
ICD_A	TankA	J3	OUTLET			
ICD_B	TankB	J5	OUTLET			
ICD_C	TankC	J4	OUTLET			
ICD_D	TankD	STMH109	OUTLET			
ICD_F	TankF	J9	OUTLET			
ICD_G	TankE	J6	OUTLET			
OR1	RYCB01	J8	OUTLET			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
C1_1	CIRCULAR	0.38	0.11	0.09	0.38	1	0.75
C1_2	CIRCULAR	0.38	0.11	0.09	0.38	1	0.26
C10	CIRCULAR	0.25	0.05	0.06	0.25	1	0.05
C11	CIRCULAR	0.25	0.05	0.06	0.25	1	0.04
C11_1	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C11_2	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C11_3	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C11_5	CIRCULAR	0.38	0.11	0.09	0.38	1	0.16
C12	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
C13	CIRCULAR	0.25	0.05	0.06	0.25	1	0.04
C2	CIRCULAR	0.25	0.05	0.06	0.25	1	0.04
C3	CIRCULAR	0.25	0.05	0.06	0.25	1	0.04
C4	CIRCULAR	0.38	0.11	0.09	0.38	1	0.46
C4_1	CIRCULAR	0.38	0.11	0.09	0.38	1	0.14
C4_3	CIRCULAR	0.38	0.11	0.09	0.38	1	0.33
C5	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C6	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
C6_4	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C7	CIRCULAR	0.25	0.05	0.06	0.25	1	0.06
C8	CIRCULAR	0.25	0.05	0.06	0.25	1	0.05
C9	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
CB011	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
CB012	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
CB014	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
CB02	CIRCULAR	0.20	0.03	0.05	0.20	1	0.02
CB08	CIRCULAR	0.25	0.05	0.06	0.25	1	0.04
CB09	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
ICD_010	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03
ICD_03	CIRCULAR	0.20	0.03	0.05	0.20	1	0.04
ICD_05	CIRCULAR	0.20	0.03	0.05	0.20	1	0.03

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

```
****************
Analysis Options
****************
Flow Units ............... CMS
Process Models:
  Rainfall/Runoff ........ YES
  RDII ................... NO
  Snowmelt ............... NO
  Groundwater ............ NO
  Flow Routing ........... YES
  Ponding Allowed ........ YES
  Water Quality .......... NO
Infiltration Method ...... HORTON
Flow Routing Method ...... DYNWAVE
Surcharge Method ......... EXTRAN
Starting Date ............ 11/10/2013 00:00:00
Ending Date .............. 11/10/2013 06:00:00
Antecedent Dry Days ...... 0.0
Report Time Step ......... 00:05:00
Wet Time Step ............ 00:05:00
Dry Time Step ............ 00:05:00
Routing Time Step ........ 1.00 sec
Variable Time Step ....... YES
Maximum Trials ........... 20
Number of Threads ........ 2
Head Tolerance ........... 0.001500 m


**************************        Volume         Depth
Runoff Quantity Continuity     hectare-m            mm
**************************     ---------       -------
Total Precipitation ......         0.088        71.677
Evaporation Loss .........         0.000         0.000
Infiltration Loss ........         0.017        13.663
Surface Runoff ...........         0.070        57.571
Final Storage ............         0.001         1.099
Continuity Error (%) .....        -0.916


**************************        Volume        Volume
Flow Routing Continuity        hectare-m      10^6 ltr
**************************     ---------     ---------
Dry Weather Inflow .......         0.000         0.000
Wet Weather Inflow .......         0.070         0.703
Groundwater Inflow .......         0.000         0.000
RDII Inflow ..............         0.000         0.000
External Inflow ..........         0.000         0.000
External Outflow .........         0.065         0.647
Flooding Loss ............         0.000         0.000
Evaporation Loss .........         0.000         0.000
Exfiltration Loss ........         0.000         0.000
Initial Stored Volume ....         0.000         0.000
Final Stored Volume ......         0.006         0.056
Continuity Error (%) .....         0.054


*************************
Highest Continuity Errors
*************************
Node STMH105 (5.34%)
Node RYCB01 (2.86%)
Node J8 (-2.63%)


***************************
Time-Step Critical Elements
***************************
Link C1_1 (76.56%)


********************************
Highest Flow Instability Indexes
********************************
Link OR1 (28)
Link C8 (4)


*************************
Routing Time Step Summary
*************************
Minimum Time Step           :     0.02 sec
Average Time Step           :     0.75 sec
Maximum Time Step           :     1.00 sec
Percent in Steady State     :     0.00
Average Iterations per Step :     2.29
Percent Not Converging      :     0.01
Time Step Frequencies       :
    1.000 -  0.871 sec      :    33.71 %
    0.871 -  0.758 sec      :    10.11 %
    0.758 -  0.660 sec      :    14.44 %
    0.660 -  0.574 sec      :    12.58 %
    0.574 -  0.500 sec      :    29.17 %


***************************
Subcatchment Runoff Summary
***************************
```

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imperv Runoff mm	Perv Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
S-001	71.68	0.00	0.00	7.80	57.78	5.49	63.28	0.06	0.04	0.883
S-002	71.68	0.00	0.00	5.71	61.12	4.10	65.22	0.03	0.02	0.910
S-003	71.68	0.00	0.00	7.90	57.64	5.54	63.18	0.04	0.03	0.881
S-004	71.68	0.00	0.00	11.22	52.25	8.06	60.31	0.01	0.01	0.841
S-005	71.68	0.00	0.00	16.68	43.86	10.95	54.82	0.01	0.01	0.765
S-006	71.68	0.00	0.00	6.98	59.16	4.85	64.01	0.04	0.03	0.893
S-007	71.68	0.00	0.00	0.00	70.33	0.00	70.33	0.02	0.01	0.981
S-008	71.68	0.00	0.00	0.00	70.21	0.00	70.21	0.02	0.01	0.980
S-009	71.68	0.00	0.00	32.82	21.60	17.18	38.79	0.04	0.02	0.541
S-010	71.68	0.00	0.00	9.21	55.53	6.46	61.99	0.04	0.03	0.865
S-011	71.68	0.00	0.00	5.60	61.36	3.95	65.31	0.05	0.04	0.911
S-012	71.68	0.00	0.00	10.90	52.80	7.71	60.51	0.01	0.01	0.844
S-013	71.68	0.00	0.00	16.52	44.00	11.06	55.07	0.02	0.01	0.768
S-014	71.68	0.00	0.00	4.97	62.39	3.51	65.90	0.06	0.04	0.919
S-015	71.68	0.00	0.00	4.28	63.47	3.07	66.54	0.03	0.02	0.928
S-016	71.68	0.00	0.00	8.39	56.94	5.75	62.69	0.07	0.05	0.875
S-017	71.68	0.00	0.00	4.08	63.67	3.03	66.69	0.03	0.03	0.930
S-018	71.68	0.00	0.00	24.77	32.88	13.78	46.66	0.05	0.04	0.651
S-018-2	71.68	0.00	0.00	24.45	32.85	14.19	47.04	0.05	0.04	0.656
S-019	71.68	0.00	0.00	10.47	53.55	7.25	60.80	0.01	0.01	0.848
S-020	71.68	0.00	0.00	34.97	15.11	22.62	37.73	0.02	0.02	0.526
S-021	71.68	0.00	0.00	41.22	4.13	30.62	34.75	0.00	0.00	0.485

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******************
Node Depth Summary
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Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days	Time of Max Occurrence hr:min	Reported Max Depth Meters
CB002	JUNCTION	0.62	1.34	85.69	0	01:35	1.34
CB004	JUNCTION	0.05	0.24	85.69	0	01:34	0.24
CB005	JUNCTION	0.09	0.40	85.70	0	01:33	0.39
CB006	JUNCTION	0.15	0.55	85.70	0	01:33	0.54
CB010	JUNCTION	0.40	1.45	85.89	0	01:21	1.45
CB014	JUNCTION	0.06	0.46	85.75	0	01:21	0.45
CB08	JUNCTION	0.61	1.45	85.85	0	01:33	1.45
CB09	JUNCTION	0.19	0.73	85.85	0	01:33	0.73
CB11	JUNCTION	0.21	1.14	85.89	0	01:21	1.14
CB12	JUNCTION	0.12	0.79	85.89	0	01:21	0.79
CB13	JUNCTION	0.38	1.40	85.75	0	01:21	1.39
CBMH05	JUNCTION	0.72	1.44	85.69	0	01:33	1.44
CBMH13	JUNCTION	0.36	1.09	85.85	0	01:33	1.09
CBMH18	JUNCTION	0.47	1.50	85.74	0	01:22	1.50
DCB15	JUNCTION	0.31	1.45	85.65	0	01:15	1.44
J3	JUNCTION	0.08	0.16	83.67	0	01:17	0.16
J4	JUNCTION	0.07	0.13	83.76	0	01:19	0.13
J5	JUNCTION	0.10	0.21	83.72	0	01:18	0.21
J6	JUNCTION	0.16	0.27	83.70	0	01:17	0.27
J8	JUNCTION	0.31	0.93	85.78	0	01:16	0.90
J9	JUNCTION	0.05	0.09	84.03	0	01:16	0.09
STMH01	JUNCTION	0.03	0.12	84.12	0	01:12	0.12
STMH04	JUNCTION	0.55	1.25	85.70	0	01:33	1.25
STMH07	JUNCTION	0.12	0.23	83.72	0	01:16	0.23
STMH08	JUNCTION	0.16	0.28	83.71	0	01:16	0.28
STMH101	JUNCTION	0.06	0.12	82.99	0	01:17	0.12
STMH102	JUNCTION	0.38	0.50	83.68	0	01:17	0.50
STMH105	JUNCTION	0.29	0.36	84.04	0	01:15	0.36
STMH106	JUNCTION	0.13	0.19	84.05	0	01:15	0.19
STMH108	JUNCTION	0.15	0.22	83.80	0	01:21	0.22
STMH109	JUNCTION	0.04	0.09	83.88	0	01:21	0.09
OF1	OUTFALL	0.06	0.12	82.32	0	01:17	0.12
CB01	STORAGE	0.30	2.33	87.74	0	01:11	2.31
RYCB01	STORAGE	0.45	2.32	87.17	0	01:12	2.30
TankA	STORAGE	0.44	1.57	85.63	0	01:18	1.57
TankB	STORAGE	0.46	1.52	85.89	0	01:21	1.52
TankC	STORAGE	0.69	1.54	85.85	0	01:33	1.54
TankD	STORAGE	0.48	1.56	86.37	0	01:21	1.55
TankE	STORAGE	0.50	1.53	85.74	0	01:22	1.52
TankF	STORAGE	0.77	1.51	85.69	0	01:34	1.51

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*******************
Node Inflow Summary
*******************
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Node	Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	Time of Max Occurrence days	Time of Max Occurrence hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CB002	JUNCTION	0.041	0.041	0	01:10	0.0551	0.0551	0.108
CB004	JUNCTION	0.013	0.013	0	01:10	0.0155	0.0155	0.016
CB005	JUNCTION	0.011	0.011	0	01:10	0.0143	0.0143	0.106
CB006	JUNCTION	0.037	0.037	0	01:10	0.0495	0.0495	0.077
CB010	JUNCTION	0.029	0.029	0	01:10	0.0376	0.0376	0.358
CB014	JUNCTION	0.025	0.025	0	01:10	0.0329	0.0329	0.157
CB08	JUNCTION	0.029	0.029	0	01:10	0.039	0.0391	0.320
CB09	JUNCTION	0.015	0.015	0	01:10	0.021	0.021	0.036
CB11	JUNCTION	0.010	0.010	0	01:10	0.0118	0.0118	0.034
CB12	JUNCTION	0.011	0.011	0	01:10	0.0145	0.0145	0.048
CB13	JUNCTION	0.030	0.030	0	01:10	0.0387	0.0387	0.130
CBMH05	JUNCTION	0.000	0.103	0	01:09	0	0.163	0.020
CBMH13	JUNCTION	0.039	0.039	0	01:10	0.0567	0.0567	0.225
CBMH18	JUNCTION	0.000	0.054	0	01:09	0	0.0714	0.235
DCB15	JUNCTION	0.043	0.043	0	01:10	0.0565	0.0565	0.063
J3	JUNCTION	0.000	0.101	0	01:17	0	0.612	0.010
J4	JUNCTION	0.000	0.025	0	01:24	0	0.17	0.031
J5	JUNCTION	0.000	0.038	0	01:24	0	0.23	0.032
J6	JUNCTION	0.000	0.051	0	01:24	0	0.297	0.220
J8	JUNCTION	0.000	0.056	0	01:12	0	0.0972	-2.567
J9	JUNCTION	0.000	0.040	0	01:16	0	0.266	0.044

STMH01	JUNCTION	0.000	0.019	0	01:11	0	0.0342	0.107
STMH04	JUNCTION	0.000	0.055	0	01:13	0	0.0989	-0.135
STMH07	JUNCTION	0.000	0.040	0	01:16	0	0.266	0.046
STMH08	JUNCTION	0.000	0.040	0	01:16	0	0.266	0.331
STMH101	JUNCTION	0.000	0.101	0	01:17	0	0.611	0.004
STMH102	JUNCTION	0.000	0.089	0	01:17	0	0.562	0.706
STMH105	JUNCTION	0.000	0.019	0	01:12	0	0.0339	5.645
STMH106	JUNCTION	0.000	0.019	0	01:12	0	0.0342	0.758
STMH108	JUNCTION	0.000	0.013	0	01:21	0	0.0621	0.170
STMH109	JUNCTION	0.000	0.013	0	01:21	0	0.0621	0.032
OF1	OUTFALL	0.034	0.121	0	01:10	0.0354	0.647	0.000
CB01	STORAGE	0.025	0.025	0	01:10	0.0342	0.0342	0.035
RYCB01	STORAGE	0.073	0.073	0	01:10	0.0992	0.1	2.946
TankA	STORAGE	0.000	0.043	0	01:10	0	0.0565	0.096
TankB	STORAGE	0.000	0.050	0	01:10	0	0.0637	-0.008
TankC	STORAGE	0.000	0.083	0	01:10	0	0.116	-0.078
TankD	STORAGE	0.050	0.050	0	01:10	0.0656	0.0656	-0.000
TankE	STORAGE	0.000	0.052	0	01:10	0	0.0713	0.071
TankF	STORAGE	0.019	0.172	0	01:09	0.0257	0.259	0.052

****
Node Surcharge Summary
****

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
CB002	JUNCTION	3.51	1.138	1.832
CB004	JUNCTION	0.40	0.037	1.963
CB005	JUNCTION	1.00	0.195	1.805
CB006	JUNCTION	1.41	0.346	1.954
CB010	JUNCTION	2.05	1.253	1.637
CB014	JUNCTION	0.51	0.256	1.744
CB08	JUNCTION	3.14	1.250	1.710
CB09	JUNCTION	1.44	0.530	1.470
CB11	JUNCTION	1.14	0.942	1.058
CB12	JUNCTION	0.77	0.592	1.408
CB13	JUNCTION	1.81	1.197	1.743
CBMH05	JUNCTION	1.21	0.275	1.925
CBMH13	JUNCTION	2.04	0.841	1.319
CBMH18	JUNCTION	0.57	0.313	1.817
DCB15	JUNCTION	1.38	1.246	1.754
STMH04	JUNCTION	2.45	0.798	2.062
STMH102	JUNCTION	1.66	0.065	3.930

****
Node Flooding Summary
****

No nodes were flooded.

****
Storage Volume Summary
****

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow CMS
CB01	0.000	1	0	0	0.003	10	0 01:11	0.019
RYCB01	0.000	2	0	0	0.003	75	0 01:12	0.056
TankA	0.007	25	0	0	0.025	89	0 01:18	0.013
TankB	0.009	27	0	0	0.030	88	0 01:21	0.013
TankC	0.031	41	0	0	0.068	91	0 01:33	0.013
TankD	0.010	28	0	0	0.033	91	0 01:21	0.013
TankE	0.011	29	0	0	0.035	89	0 01:22	0.013
TankF	0.084	46	0	0	0.163	90	0 01:34	0.023

****
Outfall Loading Summary
****

Outfall Node	Flow Freq Pcnt	Avg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
OF1	95.46	0.040	0.121	0.647
System	95.46	0.040	0.121	0.647

****
Link Flow Summary
****

Link	Type	Maximum |Flow| CMS	Time of Max Occurrence days hr:min	Maximum |Veloc| m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	0.012	0 01:07	0.94	0.35	1.00
C1_1	CONDUIT	0.089	0 01:17	1.05	0.12	0.72
C1_2	CONDUIT	0.101	0 01:17	2.22	0.39	0.43
C10	CONDUIT	0.013	0 01:21	0.81	0.26	0.36
C11	CONDUIT	0.100	0 01:09	2.03	2.50	1.00
C11_1	CONDUIT	0.013	0 01:21	0.47	0.13	0.30
C11_2	CONDUIT	0.038	0 01:24	0.53	0.39	0.63
C11_3	CONDUIT	0.025	0 01:25	0.54	0.26	0.45
C11_5	CONDUIT	0.051	0 01:24	0.51	0.31	0.86
C12	CONDUIT	0.013	0 01:10	0.99	0.39	1.00
C13	CONDUIT	0.052	0 01:10	1.06	1.31	1.00
C2	CONDUIT	0.019	0 01:12	0.65	0.48	0.57
C3	CONDUIT	0.019	0 01:12	0.42	0.47	0.87
C4	CONDUIT	0.101	0 01:17	3.32	0.22	0.32
C4_1	CONDUIT	0.019	0 01:12	0.28	0.13	0.60
C4_3	CONDUIT	0.040	0 01:16	1.23	0.12	0.38
C5	CONDUIT	0.040	0 01:16	0.58	0.41	0.64
C6	CONDUIT	0.010	0 01:10	0.54	0.29	1.00
C6_4	CONDUIT	0.039	0 01:17	0.39	0.41	0.87
C7	CONDUIT	0.055	0 01:13	1.32	0.91	1.00
C8	CONDUIT	0.055	0 01:13	1.13	1.21	1.00
C9	CONDUIT	0.011	0 01:09	0.96	0.33	1.00
CB011	CONDUIT	0.024	0 01:09	1.05	0.89	1.00
CB012	CONDUIT	0.043	0 01:10	1.37	1.26	1.00
CB014	CONDUIT	0.015	0 01:10	1.01	0.47	1.00
CB02	CONDUIT	0.030	0 01:10	0.95	1.21	1.00
CB08	CONDUIT	0.039	0 01:10	0.99	1.01	1.00
CB09	CONDUIT	0.029	0 01:10	0.94	0.87	1.00
ICD_010	CONDUIT	0.029	0 01:10	0.91	0.91	1.00
ICD_03	CONDUIT	0.037	0 01:09	1.57	0.85	1.00
ICD_05	CONDUIT	0.041	0 01:10	1.30	1.23	1.00
W1	WEIR	0.000	0 00:00			0.00
ICD_06	DUMMY	0.019	0 01:11			
ICD_A	DUMMY	0.013	0 01:18			
ICD_B	DUMMY	0.013	0 01:21			
ICD_C	DUMMY	0.013	0 01:33			
ICD_D	DUMMY	0.013	0 01:21			
ICD_F	DUMMY	0.023	0 01:34			
ICD_G	DUMMY	0.013	0 01:22			
OR1	DUMMY	0.056	0 01:12			

****
Flow Classification Summary
****

Conduit	Adjusted /Actual Length	Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
		---------- Fraction of Time in Flow Class ----------								
C1	1.00	0.04	0.00	0.00	0.24	0.00	0.00	0.72	0.01	0.00
C1_1	1.00	0.06	0.00	0.00	0.93	0.01	0.00	0.00	0.08	0.00
C1_2	1.00	0.06	0.00	0.00	0.00	0.00	0.00	0.94	0.00	0.00
C10	1.00	0.05	0.00	0.00	0.61	0.28	0.00	0.06	0.61	0.00
C11	1.00	0.05	0.00	0.00	0.90	0.00	0.00	0.06	0.00	0.00
C11_1	1.00	0.06	0.06	0.00	0.88	0.00	0.00	0.00	0.84	0.00
C11_2	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.05	0.00
C11_3	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.87	0.00
C11_5	1.00	0.06	0.00	0.00	0.90	0.00	0.00	0.04	0.03	0.00
C12	1.00	0.04	0.00	0.00	0.26	0.00	0.00	0.70	0.02	0.00
C13	1.00	0.05	0.00	0.00	0.92	0.00	0.00	0.03	0.00	0.00
C2	1.00	0.05	0.00	0.00	0.90	0.00	0.00	0.06	0.83	0.00
C3	1.00	0.06	0.00	0.00	0.91	0.00	0.00	0.03	0.01	0.00
C4	1.00	0.06	0.00	0.00	0.01	0.93	0.00	0.00	0.32	0.00
C4_1	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.05	0.00
C4_3	1.00	0.06	0.00	0.00	0.17	0.67	0.00	0.10	0.79	0.00
C5	1.00	0.07	0.00	0.00	0.87	0.00	0.00	0.06	0.00	0.00
C6	1.00	0.04	0.00	0.00	0.41	0.00	0.00	0.55	0.07	0.00
C6_4	1.00	0.07	0.00	0.00	0.90	0.00	0.00	0.02	0.03	0.00
C7	1.00	0.05	0.00	0.00	0.62	0.00	0.00	0.33	0.09	0.00
C8	1.00	0.06	0.00	0.00	0.87	0.00	0.00	0.07	0.12	0.00
C9	1.00	0.04	0.00	0.00	0.37	0.00	0.00	0.58	0.03	0.00
CB011	1.00	0.04	0.00	0.00	0.18	0.00	0.00	0.78	0.01	0.00
CB012	1.00	0.04	0.00	0.00	0.91	0.00	0.00	0.04	0.02	0.00
CB014	1.00	0.04	0.00	0.00	0.42	0.00	0.00	0.54	0.02	0.00
CB02	1.00	0.04	0.00	0.00	0.90	0.00	0.00	0.06	0.01	0.00
CB08	1.00	0.04	0.00	0.00	0.55	0.00	0.00	0.41	0.02	0.00
CB09	1.00	0.04	0.00	0.00	0.92	0.00	0.00	0.04	0.03	0.00
ICD_010	1.00	0.04	0.00	0.00	0.92	0.00	0.00	0.04	0.00	0.00
ICD_03	1.00	0.04	0.00	0.00	0.47	0.00	0.00	0.48	0.06	0.00
ICD_05	1.00	0.04	0.00	0.00	0.88	0.00	0.00	0.07	0.00	0.00

****
Conduit Surcharge Summary
****

Conduit	--------- Hours Full -------- Both Ends	Upstream	Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
C1	0.77	0.77	0.82	0.01	0.01
C1_1	0.01	0.01	3.61	0.01	0.01
C11	3.85	3.85	4.19	0.32	0.32
C11_5	0.01	0.01	1.66	0.01	0.01
C12	0.40	0.40	0.58	0.01	0.01
C13	2.23	2.23	2.34	0.13	0.13
C6	1.14	1.14	1.28	0.01	0.01
C6_4	0.01	0.01	1.66	0.01	0.01
C7	2.03	2.03	2.45	0.01	0.01
C8	2.94	2.94	3.06	0.19	0.19
C9	1.00	1.00	1.21	0.01	0.01
CB011	0.51	0.51	0.57	0.01	0.01
CB012	1.38	1.38	2.13	0.13	0.13
CB014	1.44	1.44	1.56	0.01	0.01
CB02	1.81	1.81	2.13	0.12	0.12
CB08	2.04	2.04	2.12	0.01	0.01
CB09	3.14	3.14	4.25	0.01	0.01
ICD_010	2.05	2.05	2.39	0.01	0.01

ICD_03	1.41	1.41	1.76	0.01	0.01
ICD_05	3.51	3.51	3.80	0.12	0.12

```
Analysis begun on:  Thu Sep  8 11:54:51 2022
Analysis ended on:  Thu Sep  8 11:54:52 2022
Total elapsed time: 00:00:01
```

APPENDIX

D SUPPORTING DOCUMENTS

Hydro First Defense® - HC

Net Annual Water Quality Worksheet

Rev. 9.1

Hydro International

Project Name:	**360 Kennedy Lane**	*Report Date:*	**2022-09-09**
Street:		*City:*	**Ottawa**
Province:	**Ontario**	*Country:*	**Canada**
Designer:	**WSP**	*email:*	

Paste

Teatment Parameters:

Structure ID:	OGS	
TSS Goal:	80	% Removal
TSS Particle Size:	Fine	
Area:	1.22	ha
Percent Impervious:	71%	
Rational C value:	0.68	Calc. Cn
Rainfall Station:	Ottawa, ONT	MAP
Peak Storm Flow:		L/s

RESULTS SUMMARY

Model	TSS	Volume
FD-3HC	**79.6%**	**99.3%**
FD-4HC	85.1%	99.9%
FD-5HC	89.2%	99.9%
FD-6HC	91.8%	99.9%
FD-8HC	94.9%	99.9%

Model Specification:

Model:	**FD-3HC**	
Diameter:	900	mm
No Bypass Flow:	8.00	L/s
Peak Flow Capacity:	425.00	L/s
Sediment Storage:	0.31	m^3
Oil Storage:	473.00	L

Installation Configuration:

Placement:	Online		
Outlet Pipe Size:	375	mm	*OK*
Inlet Pipe 1 Size:	375	mm	*OK*
Inlet Pipe 2 Size:		mm	*OK*
Inlet Pipe 3 Size:		mm	*OK*
Rim Level:	87.550	m	Calc Invs.
Outlet Pipe Invert:	82.870	m	*OK*
Invert Pipe 1:	83.000	m	*OK*
Invert Pipe 2:		m	
Invert Pipe 3:		m	

Net Annual Removal Model: FD-3HC

Intensity[(1)] (mm/hr)	Fraction of Rainfall[(1)] (%)	FD-3HC Removal Efficiency[(2)] (%)	Weighted Net Annual Efficiency (%)
0.50	0.1%	97.3%	0.1%
1.00	14.1%	91.2%	12.9%
1.50	14.2%	87.8%	12.5%
2.00	14.1%	85.5%	12.1%
2.50	4.2%	83.8%	3.5%
3.00	1.5%	82.3%	1.2%
3.50	8.5%	81.2%	6.9%
4.00	5.4%	80.2%	4.4%
4.50	1.2%	79.3%	0.9%
5.00	5.5%	78.5%	4.3%
6.00	4.3%	77.2%	3.3%
7.00	4.5%	76.1%	3.4%
8.00	3.1%	75.2%	2.3%
9.00	2.3%	74.3%	1.7%
10.00	2.6%	73.6%	1.9%
20.00	9.2%	69.0%	6.4%
30.00	2.6%	66.5%	1.7%
40.00	1.2%	0.0%	0.0%
50.00	0.5%	0.0%	0.0%
100.00	0.7%	0.0%	0.0%
150.00	0.1%	0.0%	0.0%
200.00	0.0%	0.0%	0.0%
Total Net Annual Removal Efficiency:			**79.6%**
Total Annual Runoff Volume Treated:			**99.3%**

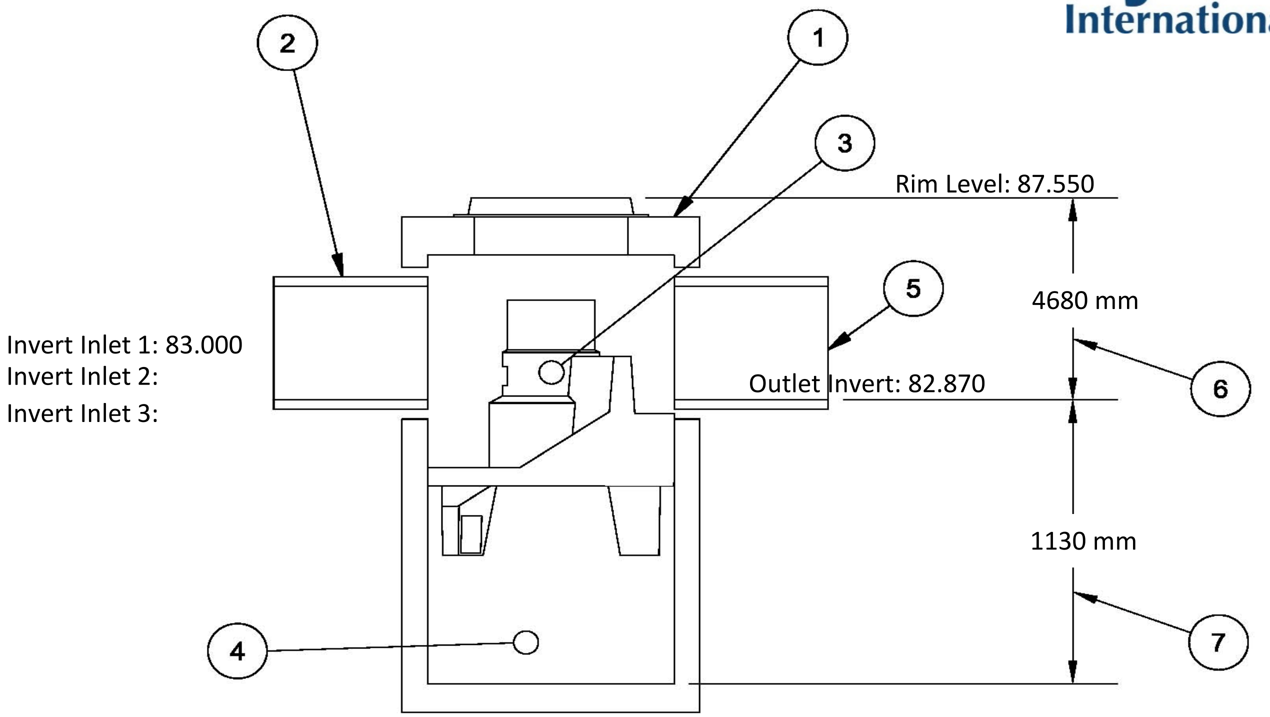
1. Rainfall Data: 1960:2007, HLY03, Ottawa, ONT, 6105976 & 6105978.

2. Based on third party verified data and appoximating the removal of a PSD similar to the STC Fine distribution

3. Rainfall adjusted to 5 min peak intensity based on hourly average.

Designer Notes:

Hydro First Defense® - HC



All drawing elevations are metres.

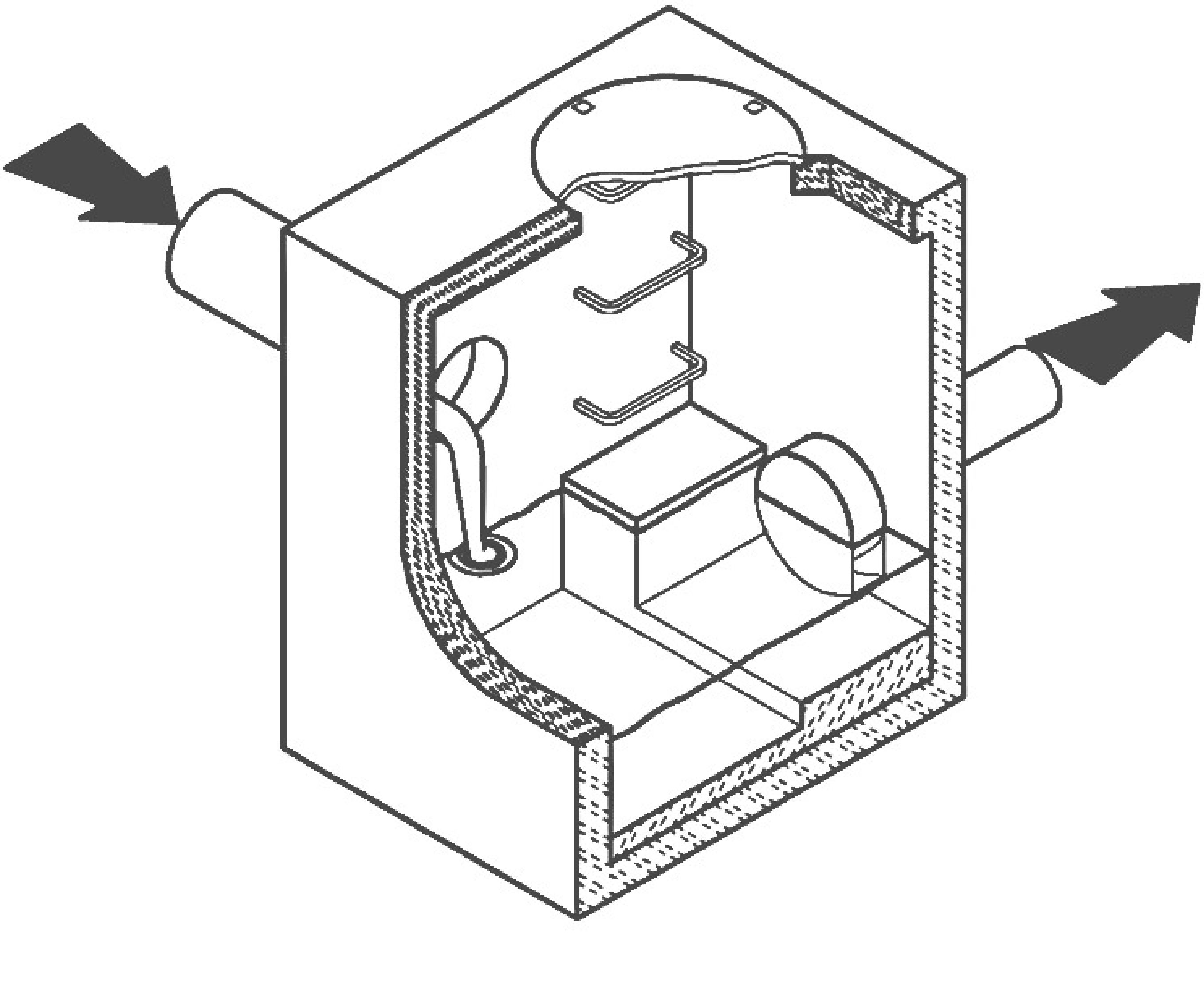
FD-3HC Specification

1	Vortex Chamber Diameter	900	mm
2	Inlet Pipe Diameter	375	mm
3	Oil Storage Capacity	473.00	L
4	Min. Provided Sediment Storage Capacity	0.31	m^3
5	Outlet Pipe Diameter	375	mm
6	Height(Final Grade to Outlet Invert)	4680	mm
7	Sump Depth(Outlet Invert to Sump)	1130	mm
	Total Depth	**5810**	**mm**

Notes:

CSO/STORMWATER MANAGEMENT

® HYDROVEX® VHV / SVHV
Vertical Vortex Flow Regulator



JOHN MEUNIER

HYDROVEX® VHV / SVHV VERTICAL VORTEX FLOW REGULATOR

APPLICATIONS

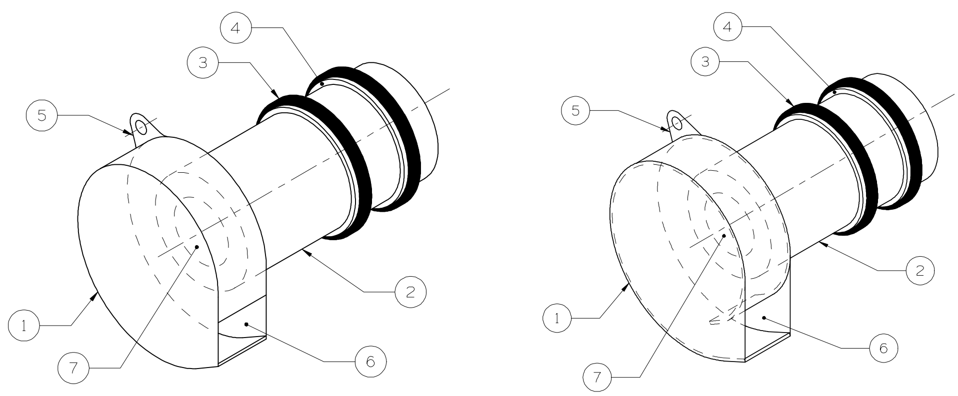
One of the major problems of urban wet weather flow management is the runoff generated after a heavy rainfall. During a storm, uncontrolled flows may overload the drainage system and cause flooding. Due to increased velocities, sewer pipe wear is increased dramatically and results in network deterioration. In a combined sewer system, the wastewater treatment plant may also experience significant increases in flows during storms, thereby losing its treatment efficiency.

A simple means of controlling excessive water runoff is by controlling excessive flows at their origin (manholes). **John Meunier Inc.** manufactures the **HYDROVEX® VHV / SVHV** line of vortex flow regulators to control stormwater flows in sewer networks, as well as manholes.

The vortex flow regulator design is based on the fluid mechanics principle of the forced vortex. This grants flow regulation without any moving parts, thus reducing maintenance. The operation of the regulator, depending on the upstream head and discharge, switches between orifice flow (gravity flow) and vortex flow. Although the concept is quite simple, over 12 years of research have been carried out in order to get a high performance.

The **HYDROVEX® VHV / SVHV** Vertical Vortex Flow Regulators **(refer to Figure 1)** are manufactured entirely of stainless steel, and consist of a hollow body (**1**) (in which flow control takes place) and an outlet orifice (**7**). Two rubber "O" rings (**3**) seal and retain the unit inside the outlet pipe. Two stainless steel retaining rings (**4**) are welded on the outlet sleeve to ensure that there is no shifting of the "O" rings during installation and use.

1. **BODY**
2. **SLEEVE**
3. **O-RING**
4. **RETAINING RINGS (SQUARE BAR)**
5. **ANCHOR PLATE**
6. **INLET**
7. **OUTLET ORIFICE**



VHV ***SVHV***

FIGURE 1: HYDROVEX® VHV-SVHV VERTICAL VORTREX FLOW REGULATORS

ADVANTAGES

- The **HYDROVEX® VHV / SVHV** line of flow regulators are manufactured entirely of stainless steel, making them durable and corrosion resistant.
- Having no moving parts, they require minimal maintenance.
- The geometry of the **HYDROVEX® VHV / SVHV** flow regulators allows a control equal to an orifice plate, having a cross section area 4 to 6 times smaller. This decreases the chance of blockage of the regulator, due to sediments and debris found in stormwater flows. **Figure 2** illustrates the comparison between a regulator model 100 SVHV-2 and an equivalent orifice plate. One can see that for the same height of water, the regulator controls a flow approximately four times smaller than an equivalent orifice plate.
- Installation of the **HYDROVEX® VHV / SVHV** flow regulators is quick and straightforward and is performed after all civil works are completed.
- Installation requires no special tools or equipment and may be carried out by any contractor.
- Installation may be carried out in existing structures.

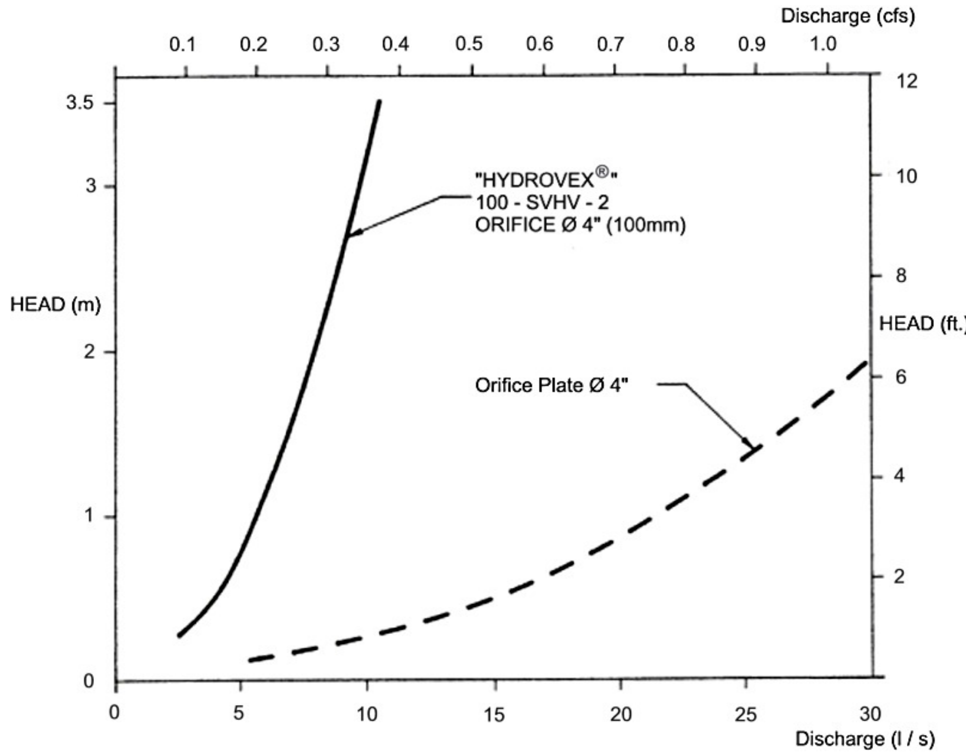


FIGURE 2: DISCHARGE CURVE SHOWING A HYDROVEX® FLOW REGULATOR VS AN ORIFICE PLATE

SELECTION

Selection of a **VHV or SVHV** regulator can be easily made using the selection charts found at the back of this brochure (see **Figure 3**). These charts are a graphical representation of the maximum upstream water pressure (head) and the maximum discharge at the manhole outlet. The maximum design head is the difference between the maximum upstream water level and the invert of the outlet pipe. All selections should be verified by John Meunier Inc. personnel prior to fabrication.

Example:

✓ Maximum design head	2m (6.56 ft.)
✓ Maximum discharge	6 L/s (0.2 cfs)
✓ Using **Figure 3** - VHV	model required is a **75 VHV-1**

INSTALLATION REQUIREMENTS

All **HYDROVEX® VHV / SVHV** flow regulators can be installed in circular or square manholes. **Figure 4** gives the various minimum dimensions required for a given regulator. ***It is imperative to respect the minimum clearances shown to ensure easy installation and proper functioning of the regulator.***

SPECIFICATIONS

In order to specify a **HYDROVEX®** regulator, the following parameters must be defined:

- The model number (ex: 75-VHV-1)
- The diameter and type of outlet pipe (ex: 6" diam. SDR 35)
- The desired discharge (ex: 6 l/s or 0.21 CFS)
- The upstream head (ex: 2 m or 6.56 ft.) *
- The manhole diameter (ex: 36" diam.)
- The minimum clearance "H" (ex: 10 inches)
- The material type (ex: 304 s/s, 11 Ga. standard)

****   Upstream head is defined as the difference in elevation between the maximum upstream water level and the invert of the outlet pipe where the HYDROVEX® flow regulator is to be installed.***

PLEASE NOTE THAT WHEN REQUESTING A PROPOSAL, WE SIMPLY REQUIRE THAT YOU PROVIDE US WITH THE FOLLOWING:

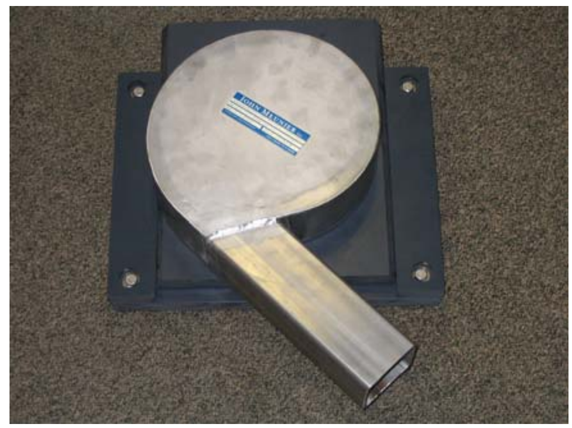
- ***project design flow rate***
- ***pressure head***
- ***chamber's outlet pipe diameter and type***

Typical VHV model in factory

OPTIONS

FV – SVHV (mounted on sliding plate)

VHV-1-O (standard model with odour control inlet)



FV – VHV-O (mounted on sliding plate with odour control inlet)

VHV with Gooseneck assembly in existing chamber without minimum release at the bottom

VHV with air vent for minimal slopes

VHV Vertical Vortex Flow Regulator

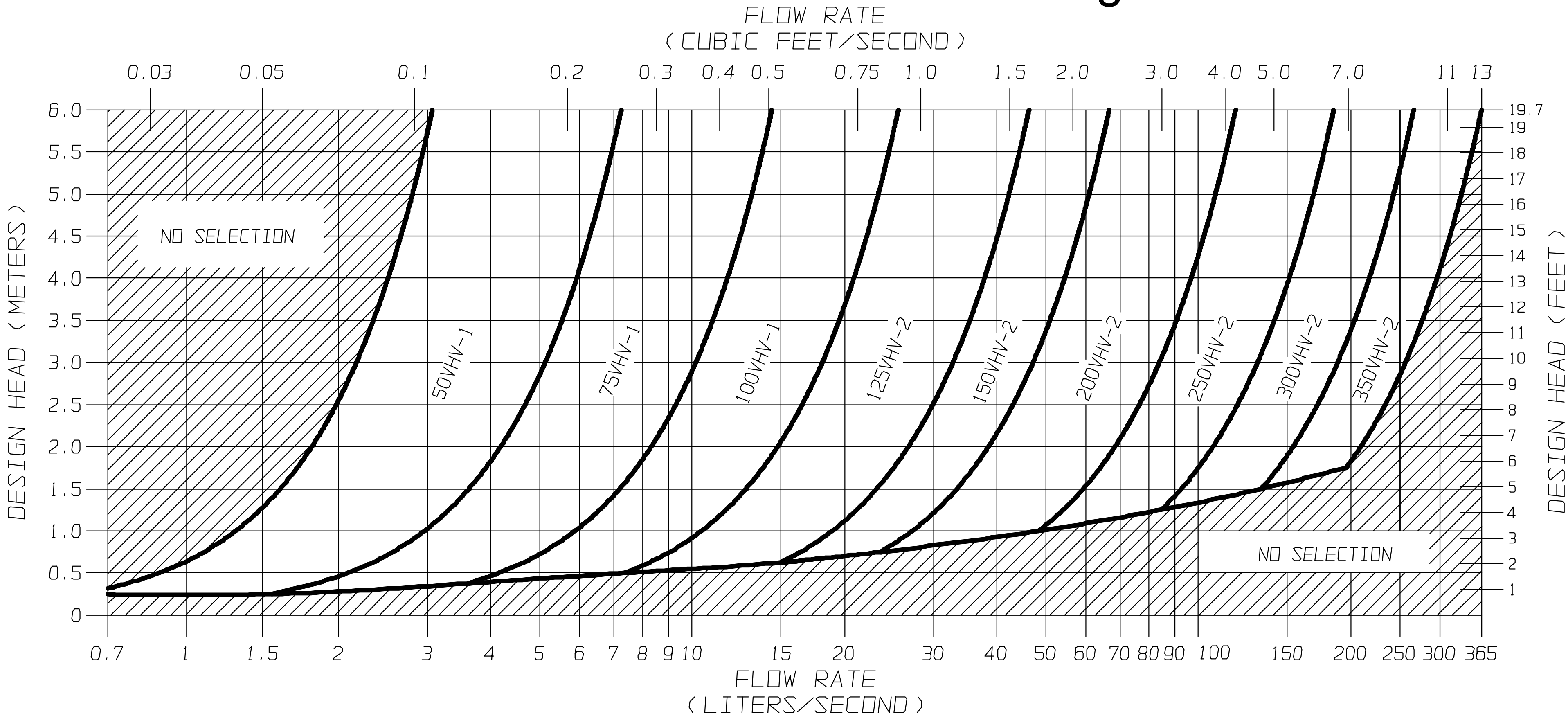


FIGURE 3 - VHV

SVHV Vertical Vortex Flow Regulator

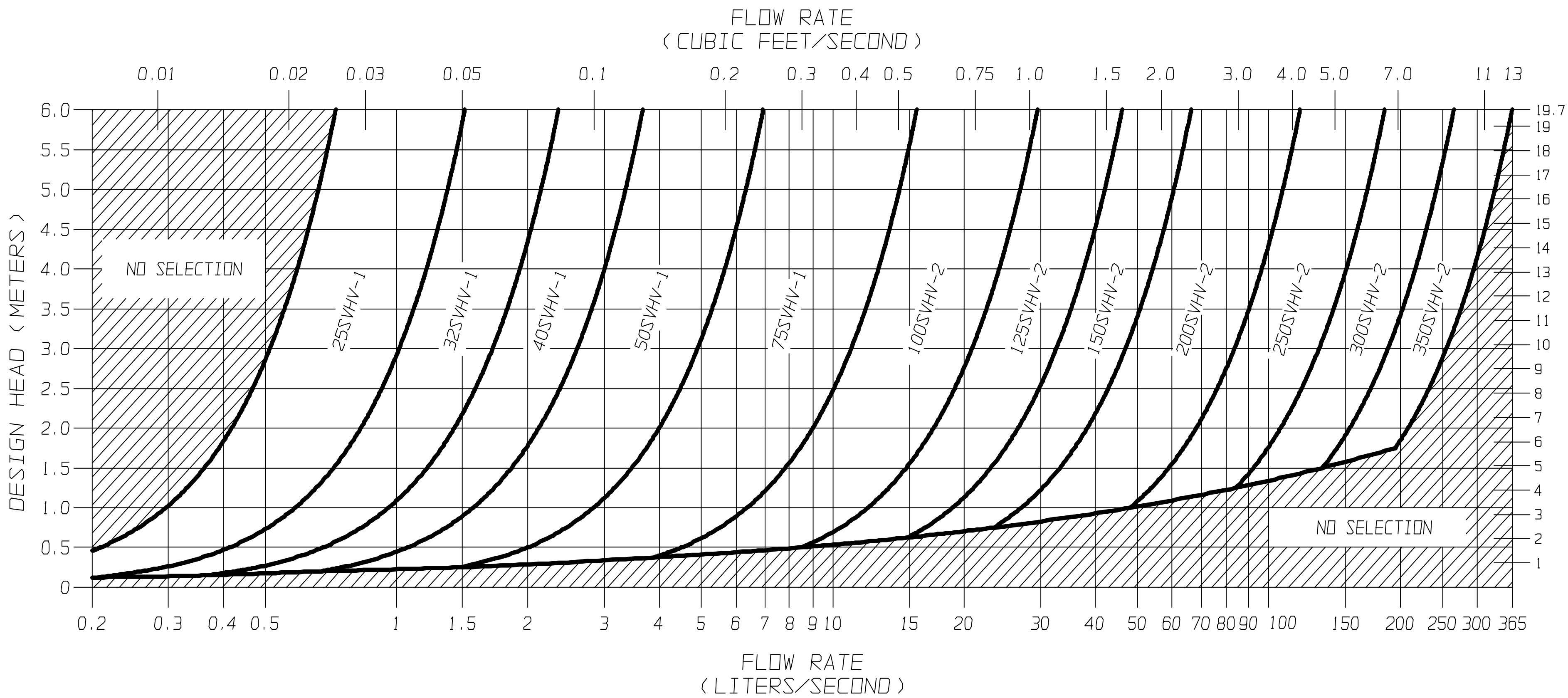
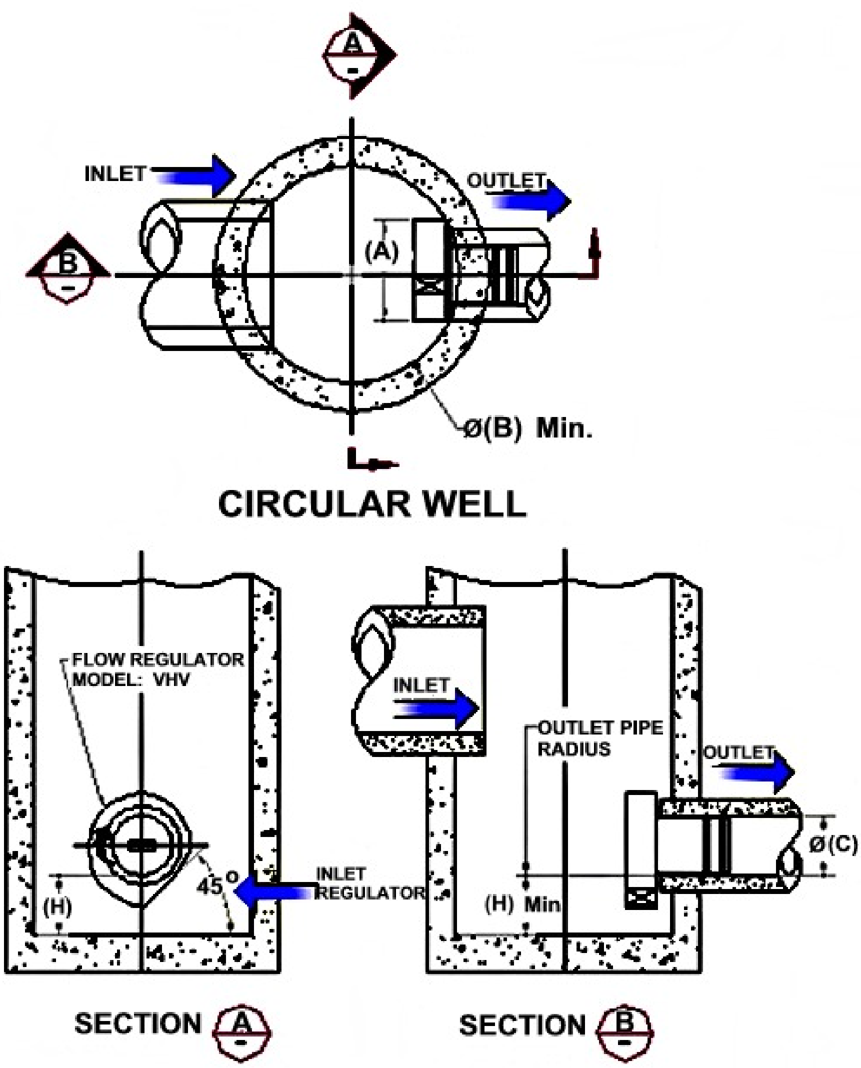


FIGURE 3 - SVHV

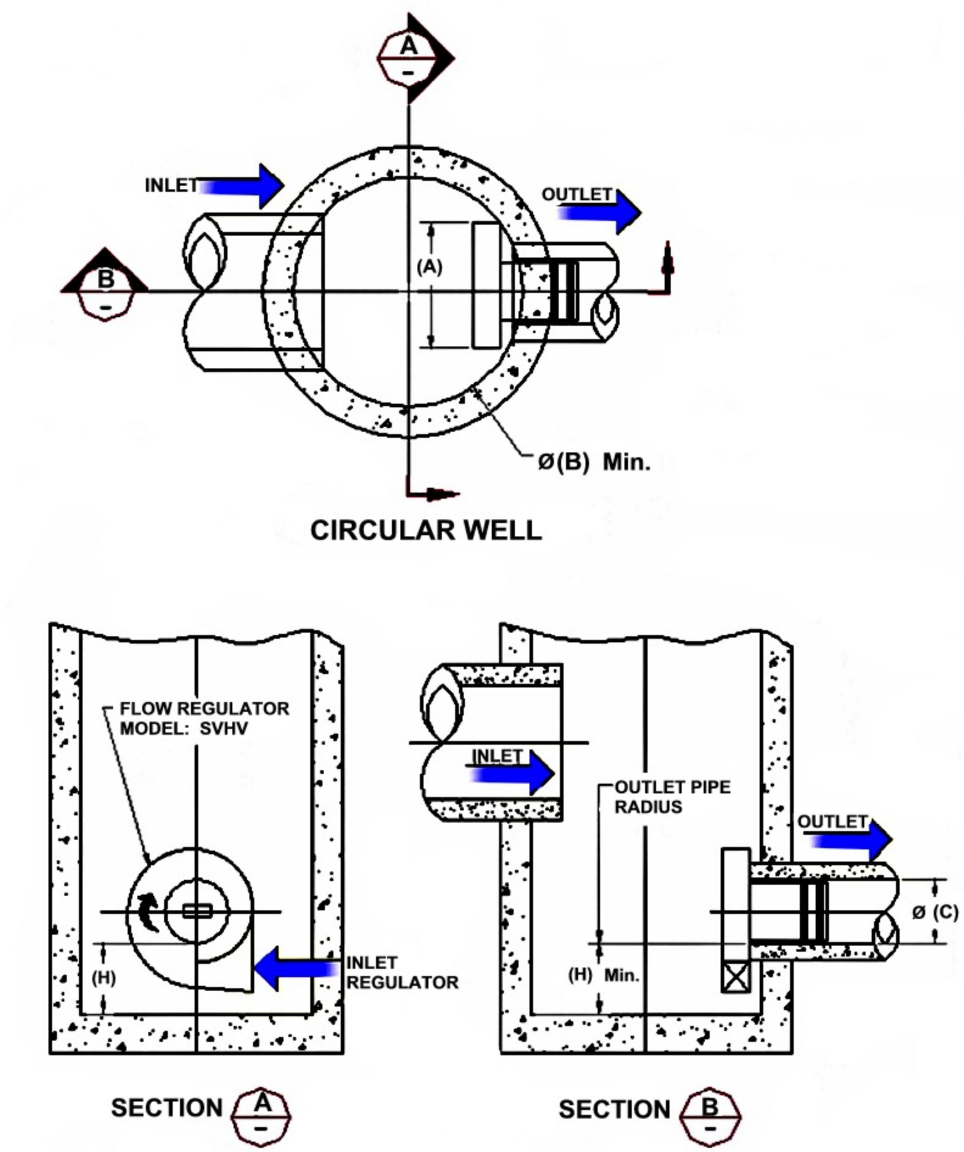
FLOW REGULATOR TYPICAL INSTALLATION IN CIRCULAR MANHOLE
FIGURE 4 (MODEL VHV)

Model Number	Regulator Diameter		Minimum Manhole Diameter		Minimum Outlet Pipe Diameter		Minimum Clearance	
	A (mm)	**A** (in.)	**B** (mm)	**B** (in.)	**C** (mm)	**C** (in.)	**H** (mm)	**H** (in.)
50VHV-1	150	6	600	24	150	6	150	6
75VHV-1	250	10	600	24	150	6	150	6
100VHV-1	325	13	900	36	150	6	200	8
125VHV-2	275	11	900	36	150	6	200	8
150VHV-2	350	14	900	36	150	6	225	9
200VHV-2	450	18	1200	48	200	8	300	12
250VHV-2	575	23	1200	48	250	10	350	14
300VHV-2	675	27	1600	64	250	10	400	16
350VHV-2	800	32	1800	72	300	12	500	20



FLOW REGULATOR TYPICAL INSTALLATION IN CIRCULAR MANHOLE
FIGURE 4 (MODEL SVHV)

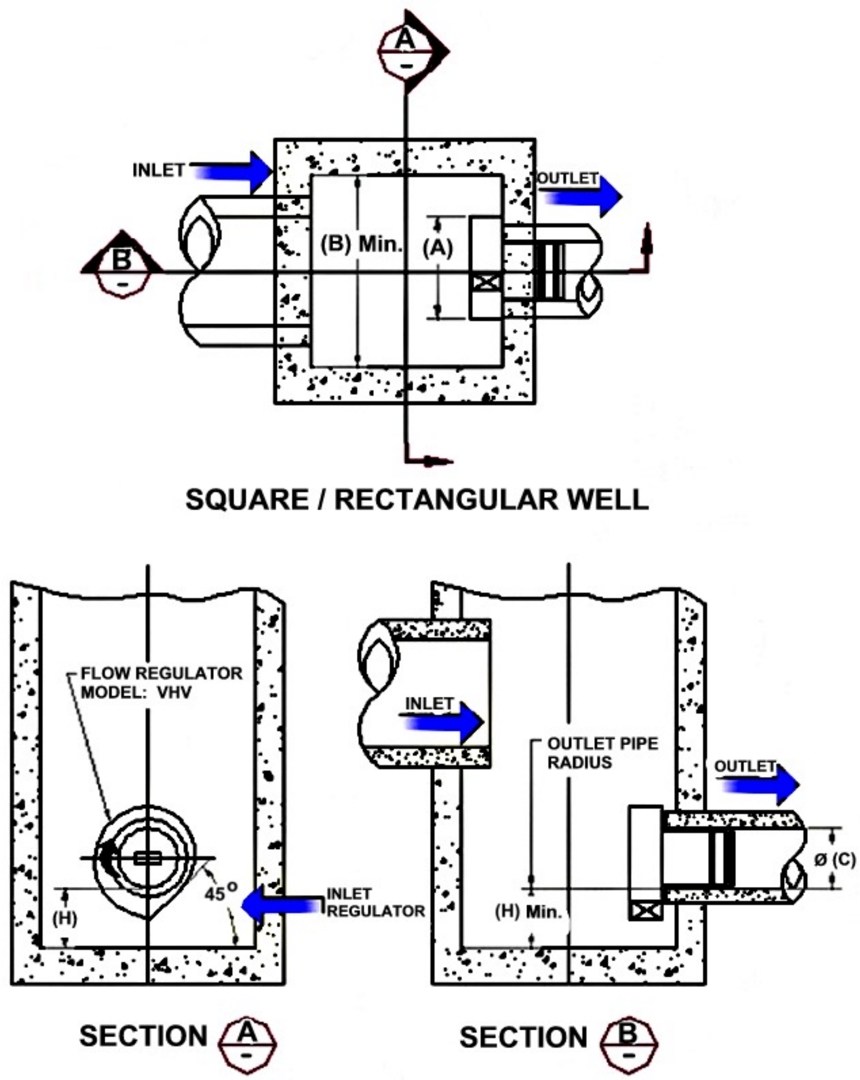
Model Number	Regulator Diameter		Minimum Manhole Diameter		Minimum Outlet Pipe Diameter		Minimum Clearance	
	A (mm)	**A** (in.)	**B** (mm)	**B** (in.)	**C** (mm)	**C** (in.)	**H** (mm)	**H** (in.)
25 SVHV-1	125	5	600	24	150	6	150	6
32 SVHV-1	150	6	600	24	150	6	150	6
40 SVHV-1	200	8	600	24	150	6	150	6
50 SVHV-1	250	10	600	24	150	6	150	6
75 SVHV-1	375	15	900	36	150	6	275	11
100 SVHV-2	275	11	900	36	150	6	250	10
125 SVHV-2	350	14	900	36	150	6	300	12
150 SVHV-2	425	17	1200	48	150	6	350	14
200 SVHV-2	575	23	1600	64	200	8	450	18
250 SVHV-2	700	28	1800	72	250	10	550	22
300 SVHV-2	850	34	2400	96	250	10	650	26
350 SVHV-2	1000	40	2400	96	250	10	700	28



FLOW REGULATOR TYPICAL INSTALLATION IN SQUARE MANHOLE
FIGURE 4 (MODEL VHV)

Model Number	Regulator Diameter		Minimum Chamber Width		Minimum Outlet Pipe Diameter		Minimum Clearance	
	A (mm)	**A** (in.)	**B** (mm)	**B** (in.)	**C** (mm)	**C** (in.)	**H** (mm)	**H** (in.)
50VHV-1	150	6	600	24	150	6	150	6
75VHV-1	250	10	600	24	150	6	150	6
100VHV-1	325	13	600	24	150	6	200	8
125VHV-2	275	11	600	24	150	6	200	8
150VHV-2	350	14	600	24	150	6	225	9
200VHV-2	450	18	900	36	200	8	300	12
250VHV-2	575	23	900	36	250	10	350	14
300VHV-2	675	27	1200	48	250	10	400	16
350VHV-2	800	32	1200	48	300	12	500	20

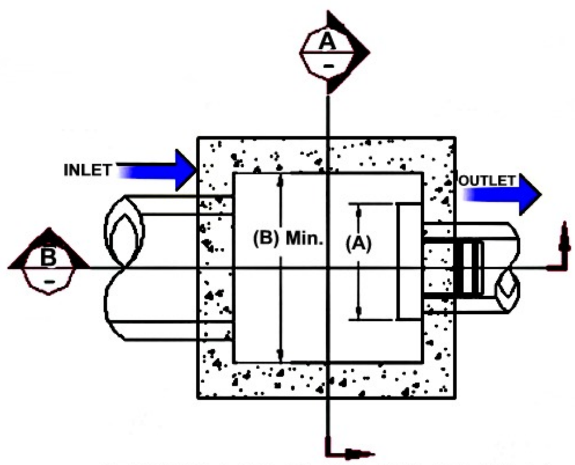
NOTE: *In the case of a square manhole, the outlet flow pipe must be centered on the wall to ensure enough clearance for the unit.*



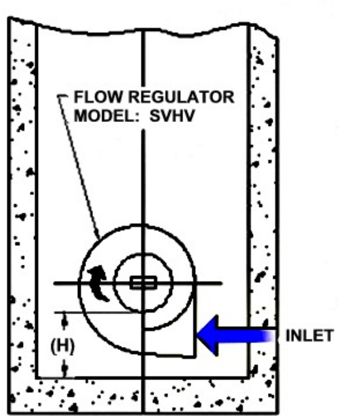
FLOW REGULATOR TYPICAL INSTALLATION IN SQUARE MANHOLE
FIGURE 4 (MODEL SVHV)

Model Number	Regulator Diameter		Minimum Chamber Width		Minimum Outlet Pipe Diameter		Minimum Clearance	
	A (mm)	**A** (in.)	**B** (mm)	**B** (in.)	**C** (mm)	**C** (in.)	**H** (mm)	**H** (in.)
25 SVHV-1	125	5	600	24	150	6	150	6
32 SVHV-1	150	6	600	24	150	6	150	6
40 SVHV-1	200	8	600	24	150	6	150	6
50 SVHV-1	250	10	600	24	150	6	150	6
75 SVHV-1	375	15	600	24	150	6	275	11
100 SVHV-2	275	11	600	24	150	6	250	10
125 SVHV-2	350	14	600	24	150	6	300	12
150 SVHV-2	425	17	600	24	150	6	350	14
200 SVHV-2	575	23	900	36	200	8	450	18
250 SVHV-2	700	28	900	36	250	10	550	22
300 SVHV-2	850	34	1200	48	250	10	650	26
350 SVHV-2	1000	40	1200	48	250	10	700	28

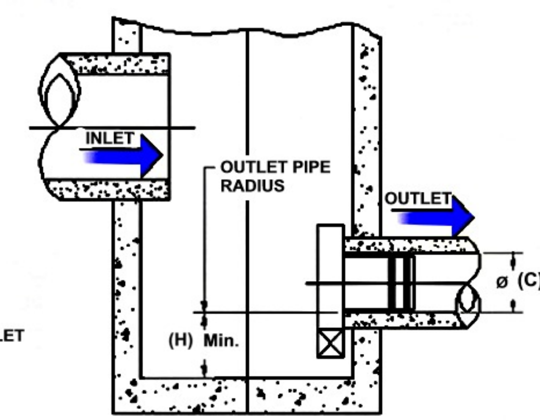
NOTE: *In the case of a square manhole, the outlet flow pipe must be centered on the wall to ensure enough clearance for the unit.*



SQUARE / RECTANGULAR WELL



SECTION A



SECTION B

INSTALLATION

The installation of a **HYDROVEX®** regulator may be undertaken once the manhole and piping is in place. Installation consists of simply fitting the regulator into the outlet pipe of the manhole. **John Meunier Inc.** recommends the use of a lubricant on the outlet pipe, in order to facilitate the insertion and orientation of the flow controller.

MAINTENANCE

HYDROVEX® regulators are manufactured in such a way as to be maintenance free; however, a periodic inspection (every 3-6 months) is suggested in order to ensure that neither the inlet nor the outlet has become blocked with debris. The manhole should undergo periodically, particularly after major storms, inspection and cleaning as established by the municipality

GUARANTY

The **HYDROVEX®** line of **VHV / SVHV** regulators are guaranteed against both design and manufacturing defects for a period of 5 years. Should a unit be defective, **John Meunier Inc.** is solely responsible for either modification or replacement of the unit.

John Meunier Inc.
ISO 9001 : 2008

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VEOLIA WATER Solutions & Technologies

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