

December 21st, 2022

Attention: Jennifer McGahan

Reference: 1835 Stittsville Main Street Redevelopment
Zoning By-law Amendment
Servicing Brief
City File No. D02-02-22-0016
Our Project No. 22008

Dear Ms. McGahan:

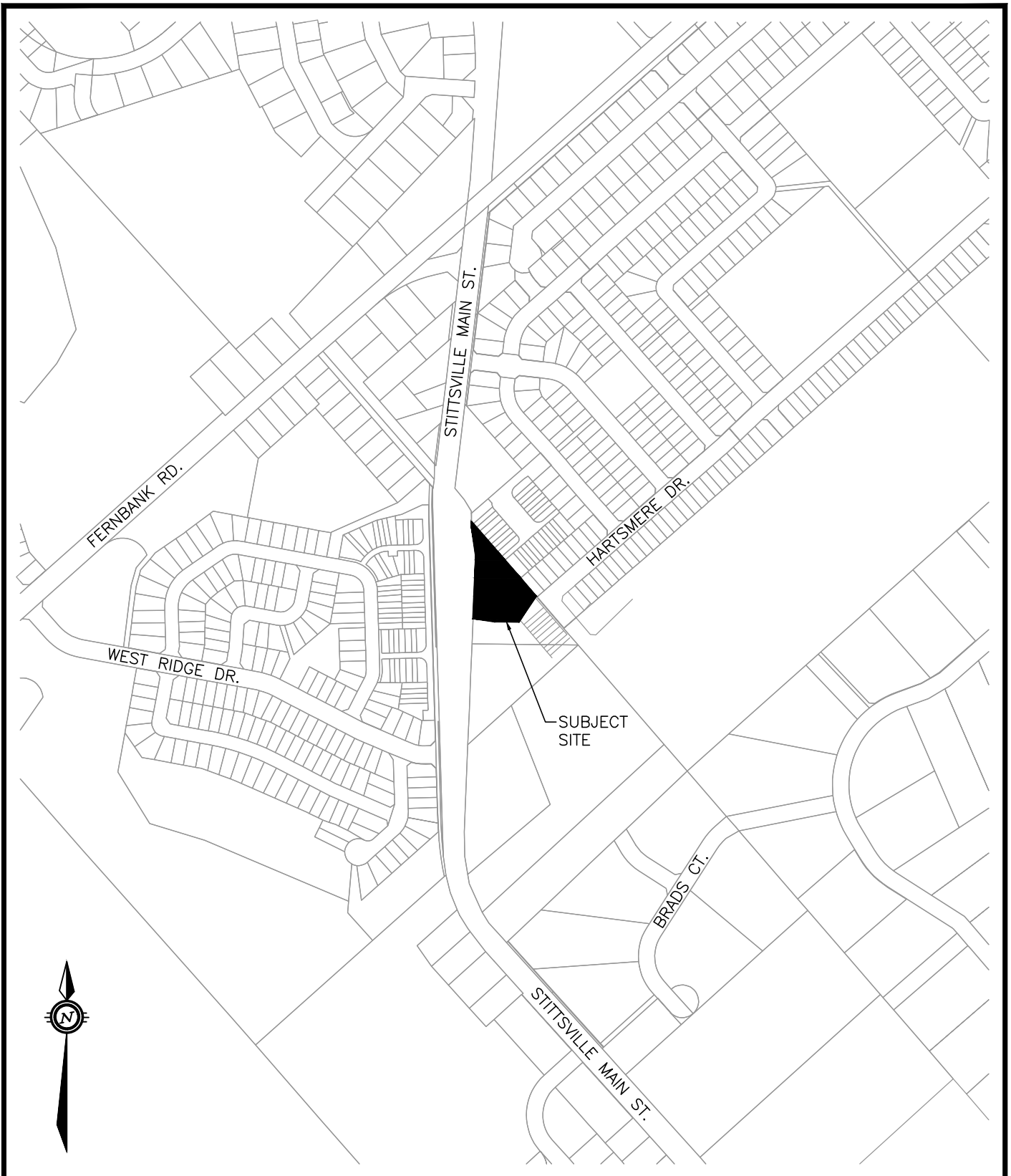
This Servicing Brief has been prepared to summarize the servicing and grading designs required in support of the zoning by-law amendment for the redevelopment of the property (currently zoned Rural Countryside Zone, RU) located at 1835 Stittsville Main Street in the community of Stittsville. The property is bounded by Stittsville Main Street to the west and existing residential homes to the north, east and south (refer to Fig. 1.0 – Key Plan following page 1). The redevelopment work is to include a severance of the 0.53 hectare property to create two lots and one retained parcel. The redevelopment will require a rezoning from the current Rural Countryside Zone (RU) designation to Residential Third Density (R3). The developer is proposing to construct two new single-family homes in addition to the existing dwelling to remain, however, the rezoning designation would allow for a higher density development.

1.0 Servicing Design

The Stittsville South – Area 6 subdivision abuts the south-east property boundary of the subject site. The subdivision design was detailed in the report *Detailed Servicing & Stormwater Management Report*, prepared by Novatech, dated July 18, 2016 (herein referred to as the Novatech Report). As part of the subdivision design (previously approved by the City of Ottawa), service stubs were provided for the future Bell lands (i.e. subject site) within a 6.0 metre service easement off of Hartsmere Drive (refer to Novatech design drawings under **Attachment A**). At the time, the subject site was anticipated to be developed into a 100-unit senior's residence. The existing services provided within the 6.0 metre easement include:

- A 150 mm diameter watermain
- A 200 mm diameter sanitary sewer
- A 375 mm diameter storm sewer

The existing service stubs contained within the 6.0 metre easement were originally proposed to be extended into the subject site to service the two new single-family homes. The extension of the existing infrastructure (water, sanitary and storm) was to be contained within a proposed 6.0 metre easement located along the north-east property boundary. However, through multiple consultations with the City of Ottawa, it was ultimately concluded that the City would not accept water supply for fire protection via an extension of the existing watermain stub off Hartsmere Drive. The City's primary concerns with



Robinson
Land Development

scale	N.T.S.	1835 STITTSVILLE MAIN STREET	project no.	22008
date	11/02/22		KEY PLAN	FIG 1.0
drawn by	BLM			

the original proposal were related to the requirement for easements on private property and the lack of accessibility between the proposed hydrant location and the new dwellings.

In keeping the recommendations provided by the City, the new dwellings are proposed to be serviced with municipal infrastructure via extensions of the existing sanitary sewer system and existing watermain system located within the Stittsville Main Street right-of-way. The existing municipal systems will be extended from Stittsville Main Street along the existing asphalt laneway which is contained within the City owned right-of-way. The existing dwelling is currently serviced via an individual drilled well and septic system. As part of the redevelopment work, the existing well and septic system are to be abandoned and new services will be provided from the existing service stubs off of Hartsmere Drive. Refer to correspondence with the City under **Attachment A**.

1.1 Water Servicing

1.1.1 Hydraulic Model

As discussed under **Section 1.0**, the City will not accept water supply for fire protection via an extension of the existing watermain stub off Hartsmere Drive. As requested by the City, water supply for domestic use and fire protection will be provided by an extension of the 203 mm diameter watermain located within the Stittsville Main Street right-of-way. A proposed 254 mm diameter watermain extension will be provided along the existing asphalt laneway which is contained within the City owned right-of-way. Two new municipal hydrants will also be provided adjacent to the existing laneway.

The new dwellings on proposed Lots 2 and 3 will be serviced by new 25 mm diameter water service connections to the proposed 254 mm diameter watermain extension located along the existing laneway. The existing dwelling on Lot 1 will be serviced by a new 25 mm diameter water service connection to the existing 152 mm watermain stub off of Hartsmere Drive. The existing on-site well will need to be decommissioned in accordance with O. Reg. 903.

A water distribution hydraulic model was created using H2OMap Water software for the proposed development. The hydraulic model incorporated the proposed watermain layout, proposed hydrant locations, boundary conditions provided by the City of Ottawa, and typical "C" factors in accordance with the current Ottawa Water Distribution Design Guidelines. The boundary conditions provided by the City are based on previous revision water demands and fire flows, however, the outputs are not expected to significantly change. Refer to the Hydraulic Water Model figure and boundary conditions provided under **Attachment B**.

1.1.2 Domestic Demands

Water demands for the proposed development on the existing municipal system have been calculated in accordance with the current Ottawa Water Distribution Design Guidelines. Since the population is below 500 persons, maximum day and maximum hour peaking factors shall be in accordance with Table 3-3 of the MOE Design Guidelines For Drinking Water Systems. Water demands for the proposed development have been calculated as follows:

Stittsville Main Watermain Connection

2 Single-Family Homes x (3.4 persons/unit) = **6.8 persons**

Average Daily Demand = (6.8 persons) x (280 L/person/day) / 86400 s/day = **0.022 L/s**

Maximum Daily Demand = (9.5) x (0.022 L/s) = **0.209 L/s**

Maximum Hourly Demand = (14.3) x (0.209 L/s) = **2.994 L/s**

Hartsmere Drive Watermain Connection

1 Single-Family Homes x (3.4 persons/unit) = **3.4 persons**

Average Daily Demand = (3.4 persons) x (280 L/person/day) / 86400 s/day = **0.011 L/s**

Maximum Daily Demand = (9.5) x (0.011 L/s) = **0.105 L/s**

Maximum Hourly Demand = (14.3) x (0.105 L/s) = **1.497 L/s**

Since the rezoning of the property would permit a higher density development in the future, water demands have also been assessed assuming that the two single-family homes are replaced with a 5-unit townhouse (largest footprint achievable based on zoning setbacks). Water demands for the demonstration development have been calculated as follows:

5-Unit Townhouse x (2.7 persons/unit) = **13.5 persons**

Average Daily Demand = (13.5 persons) x (280 L/person/day) / 86400 s/day = **0.044 L/s**

Maximum Daily Demand = (9.5) x (0.044 L/s) = **0.416 L/s**

Maximum Hourly Demand = (14.3) x (0.416 L/s) = **5.943 L/s**

As calculated above, the demonstration development (i.e. 5-unit townhouse) will create water demands approximately 2 times greater than the proposed development (i.e. 2 single-family homes). Refer to the watermain design sheets provided under **Attachment B** for more details.

1.1.3 Domestic Model Results

As noted under *Section 7.3* of the *Novatech Report*, the Bell Lands (i.e. subject site) was anticipated to have low pressures during peak hour conditions that does not meet the minimum City of Ottawa design criteria of 40 psi. This was, however, based on the assumed configuration of a 100-unit senior’s residence. A hydraulic simulation was completed for the proposed redevelopment using the water demands calculated in **Section 1.1.2** above. The system was analyzed at the proposed service connections to each new dwelling. The results of the hydraulic simulation have been summarized in **Table 1** below:

Table 1: Hydraulic Simulation Domestic Demands

Condition	Lot 1 (psi)	Lot 2 (psi)	Lot 3 (psi)
Peak Hour Pressure	44.89	45.06	45.06
Maximum Pressure	54.25	52.17	52.17

Notes:

1. Lot 1 is denoted as junction J10 on the Hydraulic Model in **Attachment B**.
2. Lot 2 is denoted as junction J5 on the Hydraulic Model in **Attachment B**.
3. Lot 3 is denoted as junction J2 on the Hydraulic Model in **Attachment B**.

As demonstrated in **Table 1** above, the peak hour pressure at each new dwelling is expected to be above the minimum allowable pressure of 40 psi. Further, the maximum pressure at each dwelling is expected to be below the maximum allowable pressure of 80 psi. A hydraulic simulation was also completed using demands for the townhouse demonstration scenario, however, the changes to outputs were very marginal. Therefore, the hydraulic simulation has demonstrated that the proposed water servicing has been designed in accordance with the current Ottawa Water Distribution Design

Guidelines. Water model outputs for the proposed single-family scenario, demonstration townhouse scenario and single connection to the Hartsmere Drive stub have been provided in **Attachment B**.

1.1.4 Fire Flow

The total required fire flow for the three single-family dwellings and the demonstration 5-unit townhouse have been calculated in accordance with the Water Supply for Public Fire Protection, Fire Underwriters Survey, v.2020 (herein referred to as the FUS guidelines). The total required fire flows have been determined using the full calculation method from the FUS guidelines and summarized below:

Total Required Fire Flow (Lot #1; Existing Single-Family)	7,000 L/min
Total Required Fire Flow (Lot #2, Proposed Single-Family)	7,000 L/min
Total Required Fire Flow (Lot #3, Proposed Single-Family)	7,000 L/min
Total Required Fire Flow (Lot #2, Proposed Townhouse)	7,000 L/min

Refer to supporting FUS calculations under **Attachment B** for more details.

In accordance with the FUS guidelines, for one and two-family dwellings not exceeding two storeys in height and having a total effective area of not more than 450 m², the total required fire flow may be determined using the values provided in Table 7 of the guidelines. For dwellings exceeding 450 m² or for row housing, the total required fire flow may be determined from Table 8. The total required fire flows have been determined using the simple calculation method from the FUS guidelines and summarized below:

Total Required Fire Flow (Lot #1; Existing Single-Family)	6,000 L/min
Total Required Fire Flow (Lot #2, Proposed Single-Family)	4,000 L/min
Total Required Fire Flow (Lot #3, Proposed Single-Family)	4,000 L/min
Total Required Fire Flow (Lot #2, Proposed Townhouse)	6,000 L/min

Since the proposed development is not considered to be large or complex, the simple method would be deemed appropriate in determining the total required fire flows.

A max. day plus fire flow simulation was completed for the proposed hydrant locations. The simulation determined that there is an available fire flow of 9,549.10 L/min (at a reference pressure of 20 psi) from Hydrant 1 and an available fire flow of 9,362.05 L/min (at a reference pressure of 20 psi) from Hydrant 2. Under the townhouse demonstration scenario, the available fire flow at hydrant 1 is marginally lower at a value of 9,534.45 L/min and therefore the increased demands can be considered negligible. Since the available fire flow from the proposed hydrants are higher than the required fire flows calculated above, the hydrant locations are adequate to service the proposed development. Refer to the fire flow reports under **Attachment B**. It should be noted that under current conditions, the existing dwelling does not have adequate fire protection and therefore any improvements for this dwelling should be considered beneficial.

1.1.5 Hydrant Analysis

As discussed under **Section 1.0**, the City will not accept water supply for fire protection via an extension of the existing watermain stub off Hartsmere Drive due to the requirement for easements on private property and the lack of accessibility between the proposed hydrant location and the new dwellings. Therefore, two new hydrants with connections to the proposed 254 mm diameter watermain extension, are proposed to be located along the existing asphalt laneway, contained within the City owned right-of-way. The proposed hydrant locations will be easily accessible and will not require the creation of any easements, alleviating the City's previous concerns. One of the proposed hydrants will also be located towards the end of the proposed watermain extension, in keeping with the City's standard drawing for residential dead-end streets. The existing asphalt laneway will be required to operate as the designated fire route, the suitability of which should be reviewed by the local fire department.

In accordance with City of Ottawa Technical Bulletin ISTB-2018-02, the aggregate fire flow capacity of all contributing fire hydrants within 150 metres of a building (measured in accordance with Table 1 – Maximum flow to be considered from a given hydrant), shall not be less than the required fire flow. As demonstrated on the hydrant coverage plans (provided under **Attachment B**), the contributing fire flow (from hydrants within 150 metres) is greater than the required fire flow for each dwelling. Therefore, it has been further demonstrated that there is sufficient fire flow available to support the proposed development.

1.1.6 Water Age Analysis

As requested by the City, a water age analysis has been completed for the proposed watermain extension given that the system terminates at a dead-end. The estimated water age for the proposed development scenario (i.e. 2 single-family homes) has been calculated as follows:

$$\text{Watermain Cross-Sectional Area} = \pi(r^2) = \pi(0.254/2)^2 = 0.0507 \text{ m}^2$$

$$\text{Watermain Length} = 121 \text{ m}$$

$$\text{Watermain Volume} = (0.0507 \text{ m}^2) \times (121 \text{ m}) = 6.13 \text{ m}^3$$

$$\text{Average Day Demand} = (6.8 \text{ persons}) \times (280 \text{ L/person/day}) = 1904 \text{ L/day} = 1.904 \text{ m}^3/\text{day}$$

$$\text{Water Age} = (6.13 \text{ m}^3) / (1.904 \text{ m}^3/\text{day}) = \mathbf{3.22 \text{ days}}$$

For comparison the water age has also been calculated for the demonstration scenario (i.e. 5-unit townhouse) as follows:

$$\text{Average Day Demand} = (13.5 \text{ persons}) \times (280 \text{ L/person/day}) = 3780 \text{ L/day} = 3.78 \text{ m}^3/\text{day}$$

$$\text{Water Age} = (6.13 \text{ m}^3) / (3.78 \text{ m}^3/\text{day}) = \mathbf{1.62 \text{ days}}$$

As demonstrated above, the estimated water age under the lowest demand scenario is 3.22 days which is reasonable to maintain water quality.

1.2 Sanitary Servicing

As requested by the City, an extension of the existing sanitary sewer system within the Stittsville Main Street right-of-way will be required to service the proposed development. A proposed 200 mm diameter sanitary sewer extension will be provided along the existing asphalt laneway which is contained within the City owned right-of-way. The proposed sanitary sewer system will outlet to the existing sanitary maintenance hole (MH43A) located on the westside of Stittsville Main Street. The existing sewer system was designed by IBI Group in support of the Harris Lands development. Refer to the design drawings provided under **Attachment A**.

The new dwellings on proposed Lots 2 and 3 will be serviced by new 135 mm diameter sanitary service connections to the proposed 200 mm diameter sanitary sewer extension located along the existing laneway. The existing dwelling on Lot 1 will be serviced by a new 135 mm diameter sanitary service connection to the existing 200 mm sanitary sewer stub off of Hartsmere Drive. A new sanitary maintenance hole will be required due to the proposed change in pipe diameter. The existing on-site septic system will need to be abandoned.

Using current City of Ottawa design guidelines, the peak sanitary design flow for the proposed development has been calculated as follows:

To Stittsville Main Sewer (2 New SFH)

Population = 2 single-family homes x (3.4 persons/unit) = **6.8 persons**

Peak Factor = **3.74** (Harmon Equation)

Peak Population Flow = (3.74) x (6.8 persons) x (280 L/person/day) / (86400 s/day)
Peak Population Flow = **0.08 L/s**

Extraneous Flow = (0.47 ha) x (0.33 L/s/ha) = **0.16 L/s**

Peak Design Flow = (0.08 L/s) + (0.16 L/s) = **0.24 L/s**

To Hartsmere Drive Sewer (1 Existing SFH)

Population = 1 single-family homes x (3.4 persons/unit) = **3.4 persons**

Peak Factor = **3.76** (Harmon Equation)

Peak Population Flow = (3.76) x (3.4 persons) x (280 L/person/day) / (86400 s/day)
Peak Population Flow = **0.04 L/s**

Extraneous Flow = (0.35 ha) x (0.33 L/s/ha) = **0.12 L/s**

Peak Design Flow = (0.04 L/s) + (0.12 L/s) = **0.16 L/s**

Since the rezoning of the property would permit a higher density development in the future, sanitary flows have also been assessed assuming that the two single-family homes are replaced with a 5-unit townhouse (largest footprint achievable based on zoning setbacks). The peak sanitary design flow for the demonstration development has been calculated as follows:

To Stittsville Main Sewer (1 New 5-Unit TH)

Population = 5-Unit Townhouse x (2.7 persons/unit) = **13.5 persons**

Peak Factor = **3.72** (Harmon Equation)

Peak Population Flow = (3.72) x (13.5 persons) x (280 L/person/day) / (86400 s/day)

Peak Population Flow = **0.16 L/s**

Extraneous Flow = (0.47 ha) x (0.33 L/s/ha) = **0.16 L/s**

Peak Design Flow = (0.16 L/s) + (0.16 L/s) = **0.32 L/s**

As calculated above, the proposed development (i.e. 2 single-family homes) is expected to generate a peak sanitary design flow of 0.24 L/s. The demonstration development (i.e. 5-unit townhouse) is expected to generate a peak sanitary design flow of 0.32 L/s. Given that the capacities of the proposed sanitary sewers are above 26 L/s, there is adequate capacity to convey peak flows from the proposed development or the higher density development scenario. As calculated above, the existing single-family home is expected to generate a peak sanitary design flow of 0.16 L/s. Given that the existing sanitary sewer system on Hartsmere Drive was designed to accept flows from a 100-unit senior's residence using outdated design parameters (i.e. 350 L/person/day instead of current 280 L/person/day) there will be adequate capacity within the existing system to accommodate flows from one single-family home. The Asset Management Branch will advise of any capacity concerns within the existing sanitary sewer system (as noted in correspondence with City under **Attachment A**). The proposed sanitary sewers have also been designed to meet the acceptable full flow velocity range of 0.60 m/s to 3.0 m/s in accordance with the current City of Ottawa Sewer Design Guidelines. Refer to the sanitary sewer design sheet and Sanitary Drainage Area Plan provided under **Attachment C**.

1.3 Storm Servicing

Given the constraints of the proposed development, the City is willing to accept the implementation of sump pumps for the dwelling foundation drainage systems. Therefore, no storm services will be required for the new dwellings and sump pump outlets to the proposed rear yard swale system will be provided (refer correspondence with City under **Attachment A**). The rear yard swale system has been designed to outlet to the existing catch basin located along the property line adjacent to Hartsmere Drive. Flows captured by the existing catch basin will be conveyed to the existing storm sewer system on Hartsmere Drive via the existing 375 mm diameter storm sewer stub contained within the existing easement.

As noted in the Novatech Report, storm drainage from the Bell Lands (i.e. subject site) was allocated within the storm sewer system designed for the Stittsville South subdivision. The subject site was assigned an area of 0.532 hectares and a runoff coefficient value of 0.75. The design parameters allocated for the subject site assumed that the property would be developed into a 100-unit senior's residence. However, the proposed redevelopment of the property is much less impactful to the downstream storm sewer system than what was allocated in the Novatech Report. The weighted runoff coefficient for the redevelopment will be approximately 0.35 compared to the 0.75 value which was previously assumed in the Novatech design. The 5-year peak design flow for the total site area has been calculated to be 43.25 L/s which is approximately 23% of the 375 mm diameter storm sewer stub capacity. As indicated in the storm sewer design sheets prepared by Novatech for the Stittsville South subdivision (refer to **Attachment D**), the downstream storm sewers (from Hartsmere Drive to the

existing SWM facility on Parade Drive) will have sufficient capacity to accommodate the 5-year peak flow from the subject site.

2.0 Grading Design

The proposed grading has been designed to tie into existing elevations along the property boundaries and to minimize cut/fill where possible. The proposed grading has been designed in accordance with the following City of Ottawa design guidelines:

- Maximum slope in grassed areas between 2% and 7%.
- Grades above 7% require terracing.
- Maximum terracing of 3H:1V.
- Driveway grades between 2% and 6%.
- Rear terrace grades to be minimum 0.30 metres above swale spillover elevation.
- Swales shall have minimum depth of 150 mm and maximum depth of 600 mm.

During detailed design, the need for a perforated rear yard subdrain system will be assessed. Refer to the conceptual Grading Plans (DWG. 22008-GR1, GR2) under **Attachment A**.

3.0 Conclusion

It has been demonstrated that the redevelopment of the property located at 1835 Stittsville Main Street can be accomplished to include two new single-family lots (or a higher density townhouse block) and one retained parcel. The redevelopment of the property can be adequately serviced for water, sanitary and storm by incorporating the following key design features:

- A 254 mm diameter watermain extension of the existing watermain system located within the Stittsville Main Street right-of-way for domestic water supply.
- Two new hydrants located adjacent to the existing laneway contained within the City owned right-of-way for fire protection.
- A 200 mm diameter sanitary sewer extension from the existing sanitary sewer system located within the Stittsville Main Street right-of-way.
- The existing dwelling will be provided with new water and sanitary service connections to the existing service stubs off of Hartsmere Drive.
- The implementation of sump pumps for the new dwelling foundation drainage systems with outlets to the proposed rear yard swale system.
- A grading design which ties into existing elevations along the property boundaries.

If you require additional information or clarification, please contact the undersigned.

Yours truly,

ROBINSON LAND DEVELOPMENT

Brandon MacKechnie, P. Eng.
Project Engineer



Angela Jonkman, P. Eng.
Manager – Land Development
& Drainage Services



Attachment A

Novatech Design Drawings –
Stittsville South – Area 6

IBI Design Drawings – Harris Lands

Correspondence with City

Conceptual Servicing Plans
(DWG. 22008-S1,S2)

Conceptual Grading Plans
(DWG. 22008-GR1, GR2)

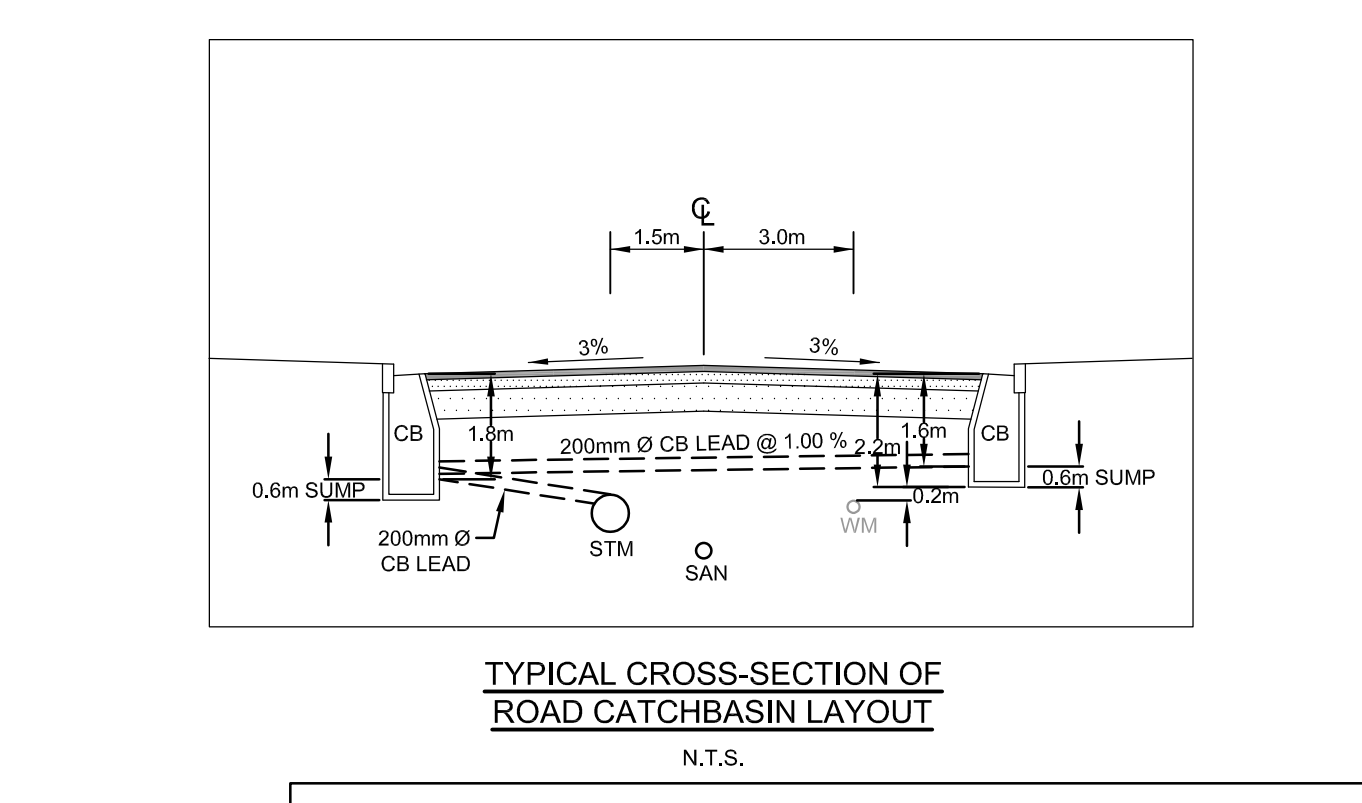
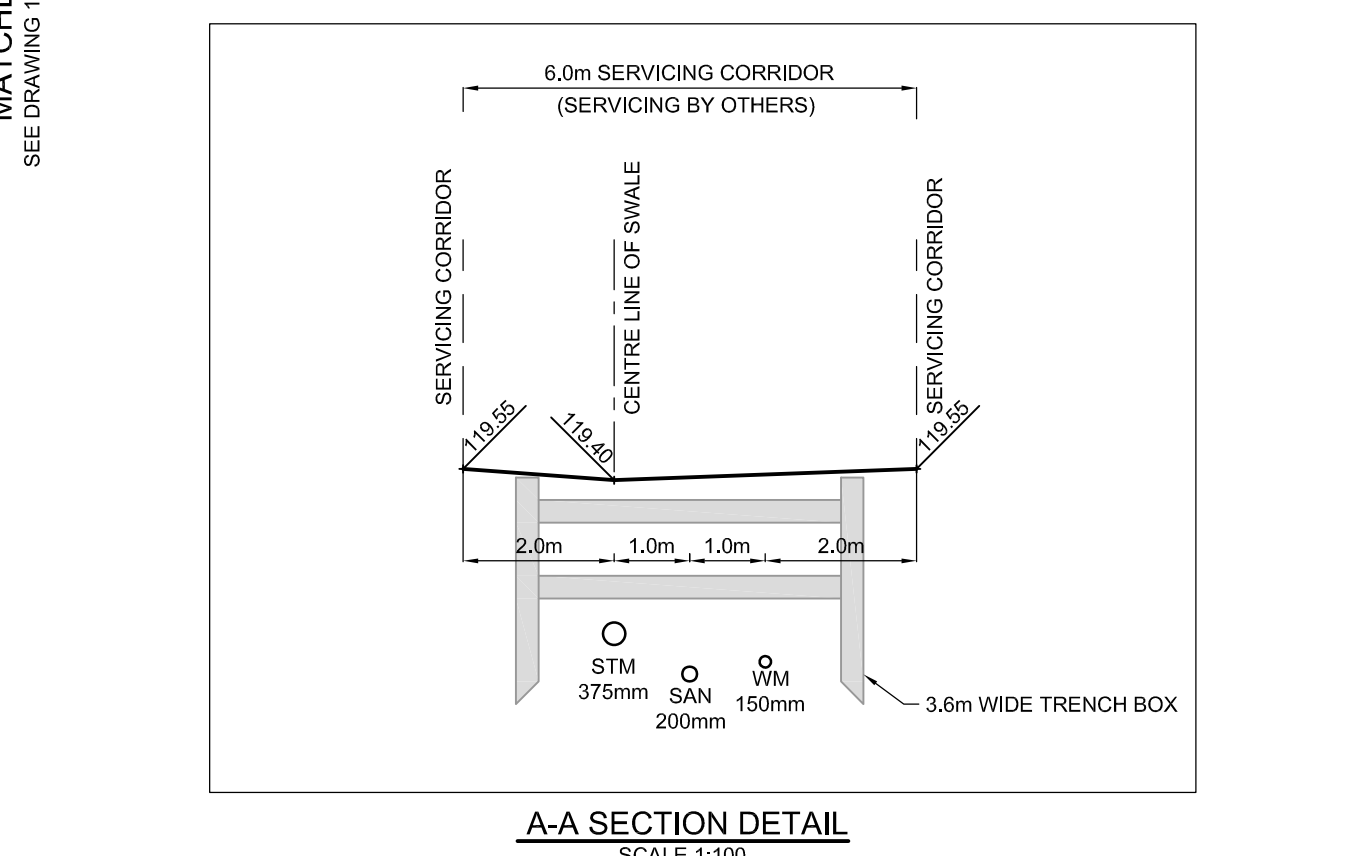
CB No.	STATION	T/O ELEVATION	INVERT	K/D DIA (MM)	CB No.	STATION	T/O ELEVATION	INVERT	K/D DIA (MM)	CB No.	STATION	T/O ELEVATION	INVERT	K/D DIA (MM)	CB No.	STATION	T/O ELEVATION	INVERT	K/D DIA (MM)	CB No.	STATION	T/O ELEVATION	INVERT	K/D DIA (MM)															
444	40+115.27	119.88	117.88	152	461	50+115.53	121.89	120.29	152	214	10+242.11	120.72	119.12	100.00	701.010	215	10+243.80	120.78	119.18	100.00	701.010	627	118.09	117.00			712	119.48	117.18	127	628	118.99	117.00			713	121.06	118.36	127



NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY	No.	REVISION	DATE	BY	No.	REVISION	DATE	BY	NO.	REVISION	DATE	BY
16.	ISSUED FOR EARLY SERVICES	OCT 26/16	BHB	8.	REVISED FOR TENDER ADDENDUM #1	FEB 04/16	BHB	SCALE	1:500			12.	ISSUED FOR TENDER	JAN 2016	BHB
15.	REVISED RYCB STRUCTURES & ISSUED FOR LAYOUT	OCT 17/16	BHB	7.	ISSUED FOR TENDER	JAN 2016	BHB	FOR REVIEW ONLY				11.	ISSUED FOR TENDER	DEC 16/15	BHB
14.	ISSUED FOR MOECC-ECA APPROVAL	SEPT 21/16	BHB	6.	REVISED AS PER CITY COMMENTS	DEC 16/15	BHB	CS/CS/TGP				10.	ISSUED FOR CITY REVIEW	SEPT 14/15	BHB
13.	REVISED AS PER CITY COMMENTS	SEPT 09/16	BHB	5.	ISSUED FOR CITY REVIEW	SEPT 14/15	BHB	BHB				9.	ISSUED TO BUILDER FOR INFORMATION	JUL 21/15	BHB
12.	ISSUED FOR MANHOLE SHOP DRAWINGS	AUG 29/16	BHB	4.	ISSUED TO BUILDER FOR INFORMATION	JUL 21/15	BHB	CS/TGP				8.	ISSUED TO BUILDER FOR INFORMATION	APR 24/15	BHB
11.	REVISED AS PER CITY COMMENTS	JULY 16/16	BHB	3.	ISSUED TO BUILDER FOR INFORMATION	APR 24/15	BHB	CHKD				7.	ISSUED TO BUILDER FOR INFORMATION	NOV 26/14	BHB
10.	REVISED FOR COMMENCE WORK NOTIFICATION STAGE 2A	OCT 18/17	BHB	2.	ISSUED TO BUILDER FOR INFORMATION	NOV 26/14	BHB	APPD				6.	ISSUED TO BUILDER FOR INFORMATION	AUG 15/14	BHB
9.	REVISED FOR FINAL APPROVAL	MAY 24/17	BHB	1.	ISSUED TO BUILDER FOR INFORMATION	AUG 15/14	BHB	DATE				5.	ISSUED FOR INFORMATION		

- ### LEGEND
- -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -
 -



SEE DRAWING 113004-ND FOR NOTES AND DETAILS

REVIEWED BY DEVELOPMENT REVIEW BRANCH

Signed: _____ Date: _____ 2016

Plan Number: 17724 D07-16-13-0033

NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario, Canada K2M 1P6

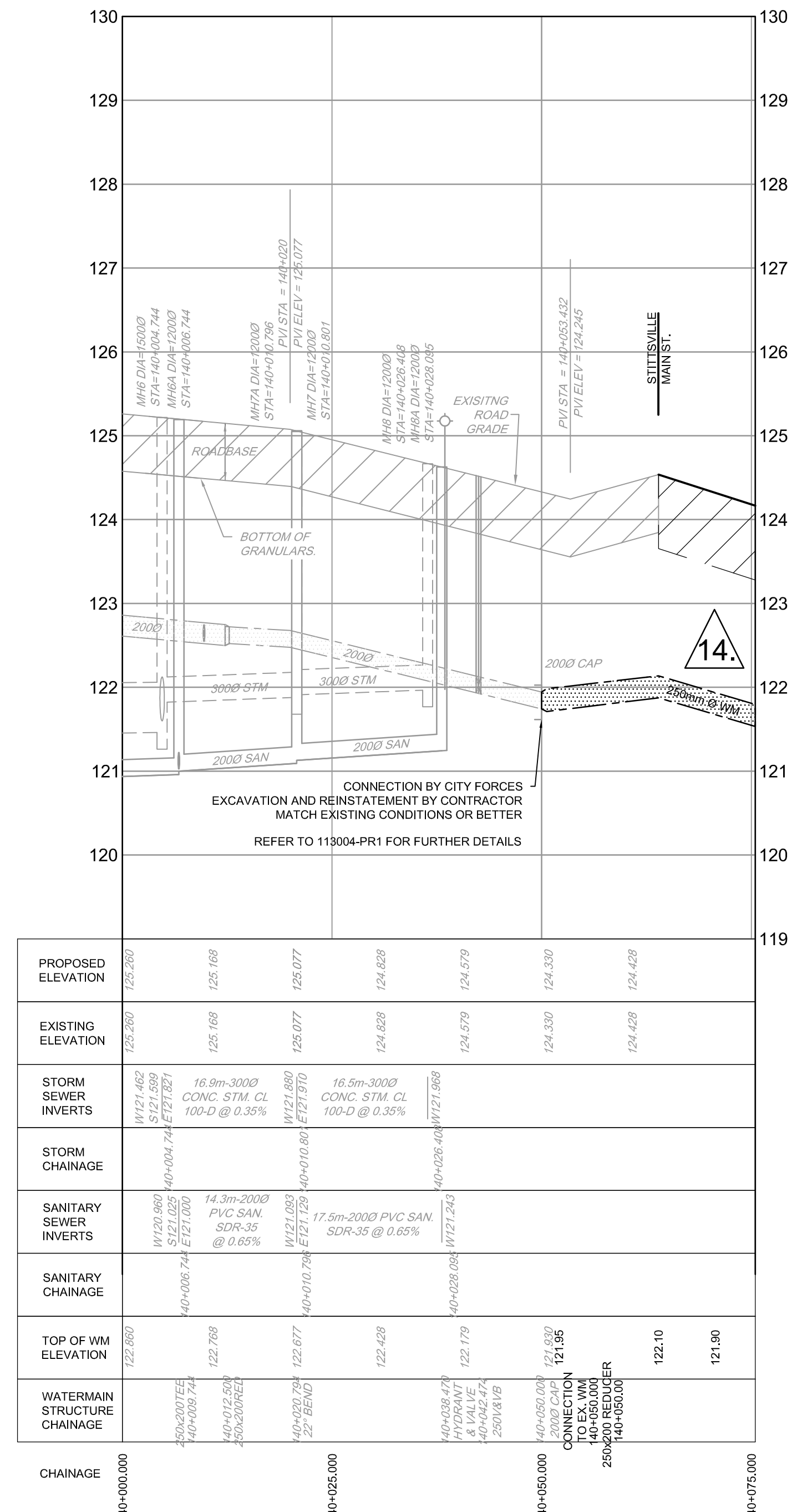
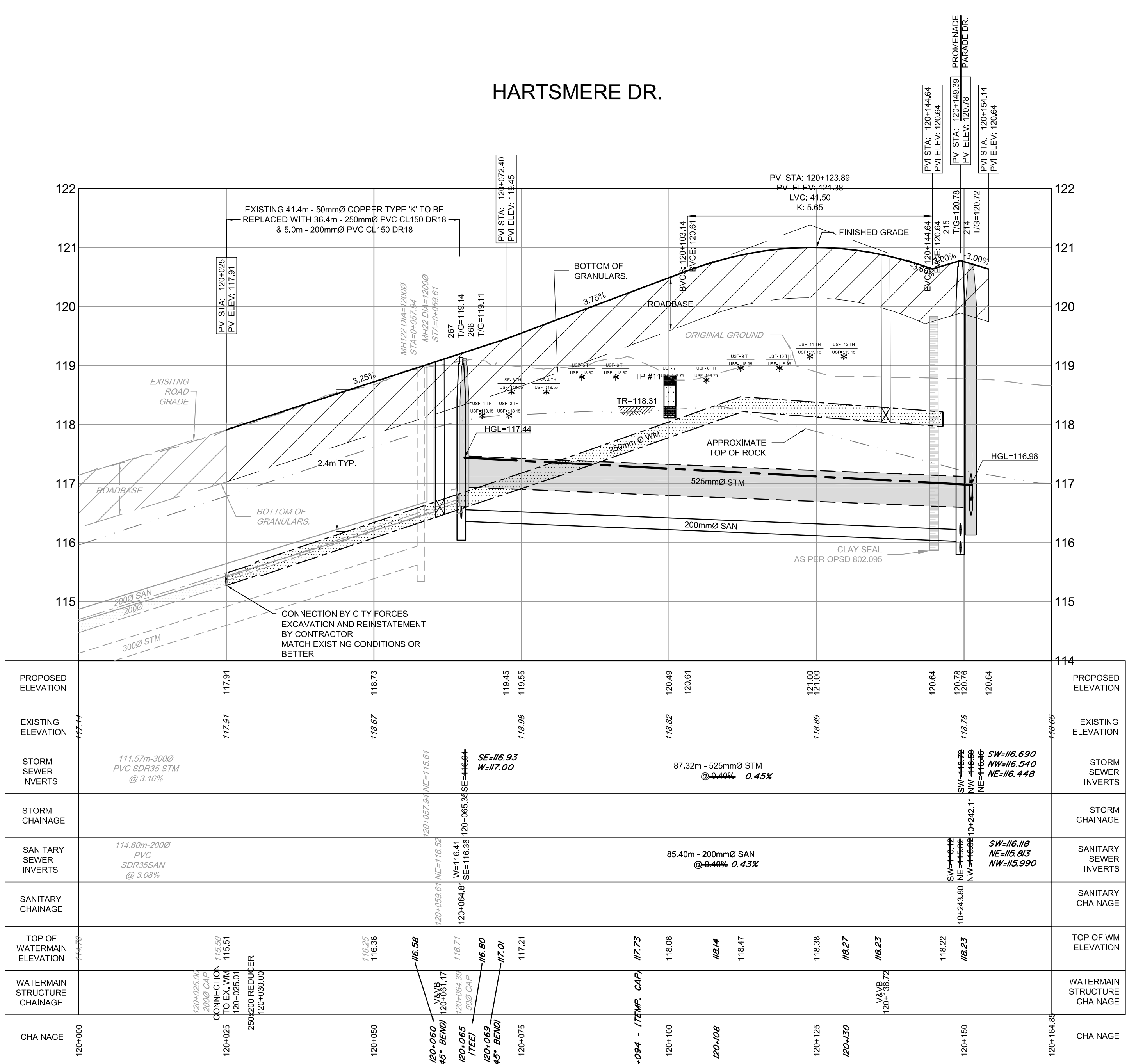
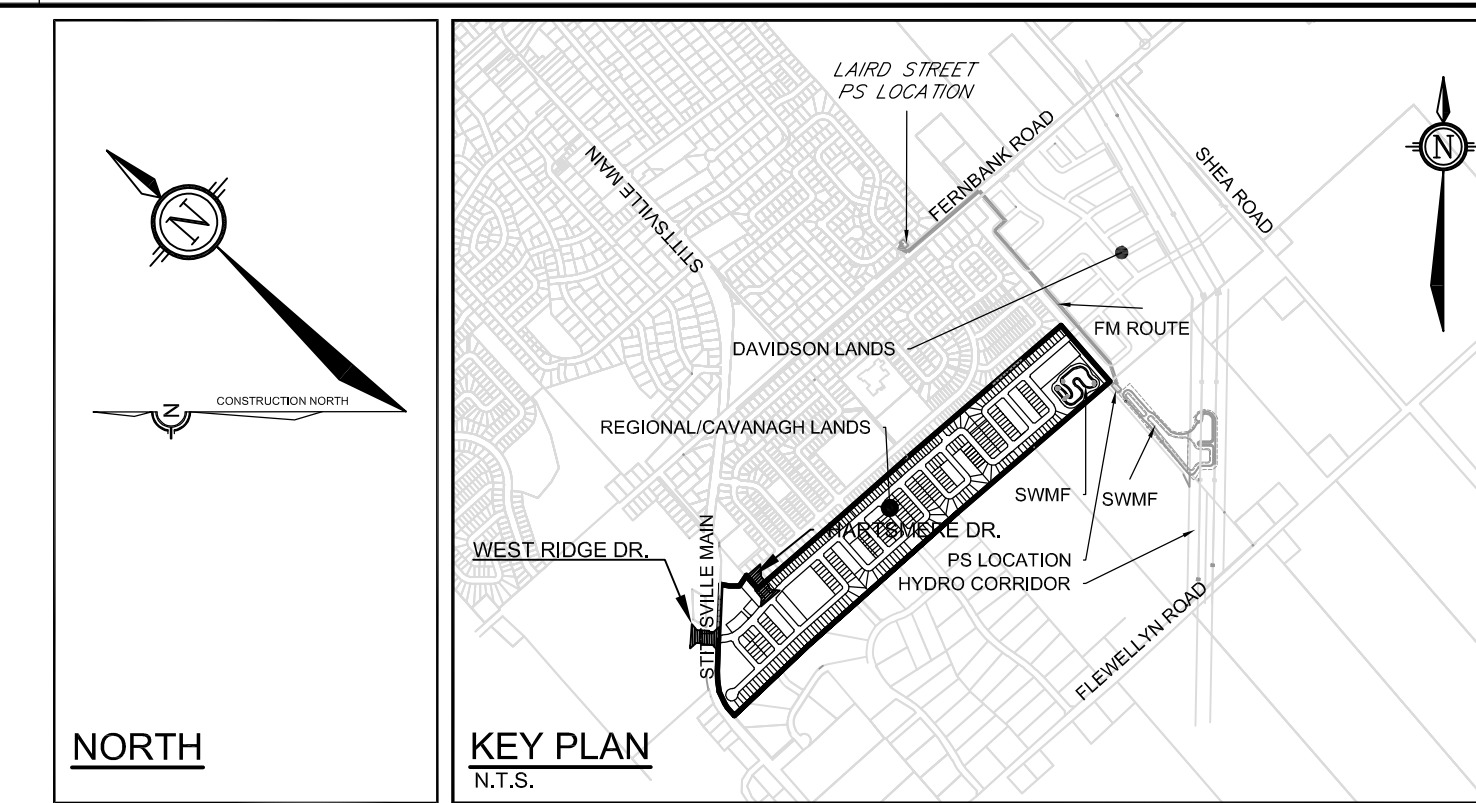
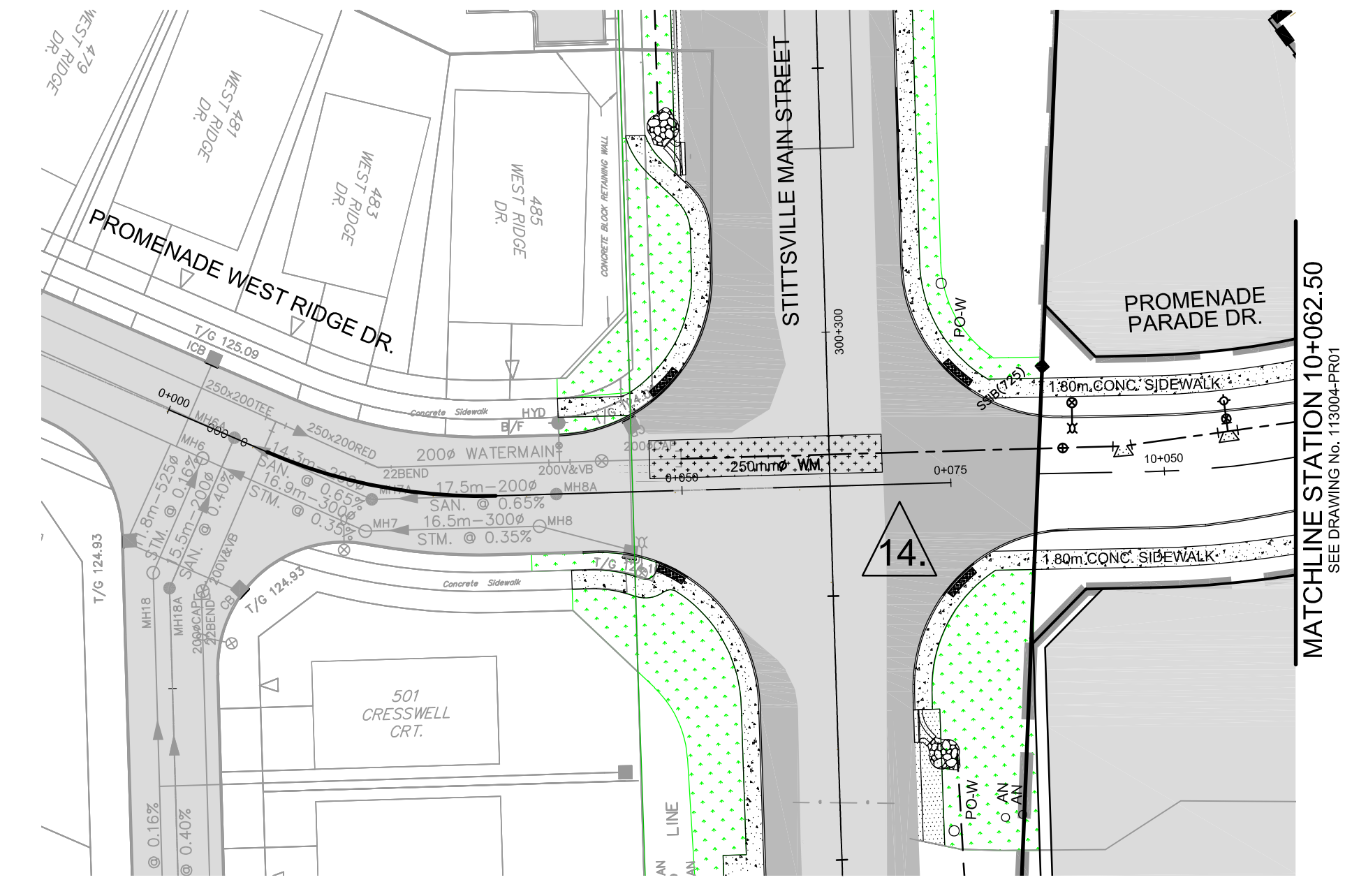
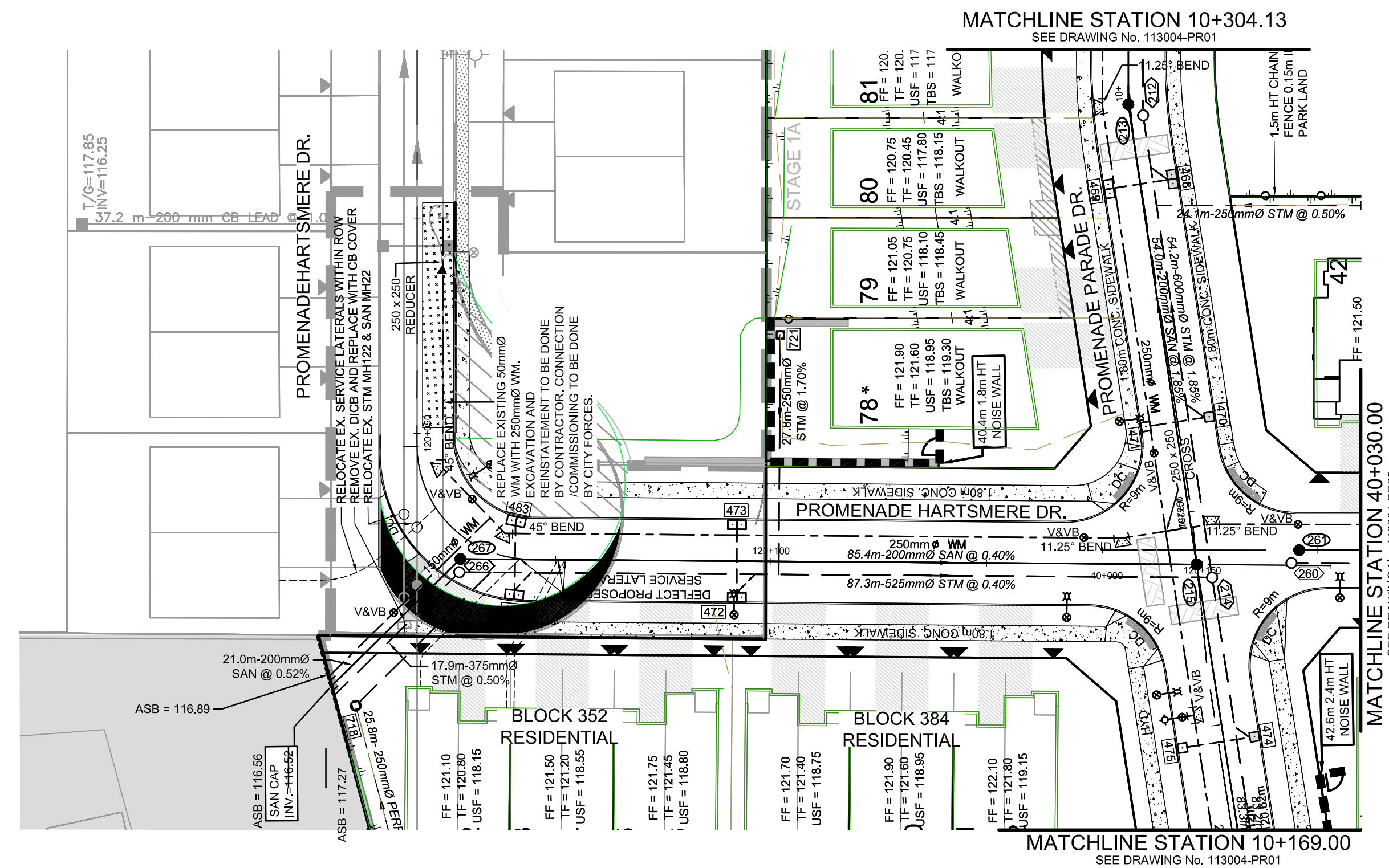
CITY OF OTTAWA
STITTSVILLE SOUTH - AREA 6

GENERAL PLAN AND SERVICES

PROJECT No.: 113004-00

REV #1

DRAWING No.: 113004-GP1



No.	REVISION	DATE	BY	NO.	REVISION	DATE	BY
16.	BLOCK 383 AS-BUILT ADDED	JAN 25/22	BHB	8.	REVISED AS PER CITY COMMENTS	APR 05/16	BHB
15.	AS-BUILT NOTES	APR 12/18	BHB	7.	ISSUED FOR TENDER	JAN 20/16	BHB
14.	REVISED WM FROM STA 140+050 TO STA 140+075	MAY 24/17	BHB	6.	MINOR REVISIONS	DEC 23/15	BHB
13.	ISSUED FOR EARLY SERVICING	OCT 26/16	BHB	5.	REVISED AS PER CITY COMMENTS	DEC 16/15	BHB
12.	ISSUED FOR MOECC-ECA APPROVAL	SEPT 21/16	BHB	4.	ISSUED FOR CITY REVIEW	SEPT 14/15	BHB
11.	REVISED AS PER CITY COMMENTS	SEPT 09/16	BHB	3.	ISSUED TO BUILDER FOR INFORMATION	APR 17/15	BHB
10.	ISSUED FOR MANHOLE SHOP DRAWINGS	AUG 30/16	BHB	2.	ISSUED TO BUILDER FOR INFORMATION	NOV 26/14	BHB
9.	REVISED AS PER CITY COMMENTS	JULY 18/16	BHB	1.	ISSUED FOR INFORMATION	AUG 15/14	BHB

SCALE	FOR REVIEW ONLY
1:500	DESIGN: BCS/TGP
0 5 10 15 20	DRAWN: BHB
	CHECKED: TGPI/EST
	APPROVED: BHB
	APPROVED: GJM

REVIEWED BY DEVELOPMENT REVIEW BRANCH

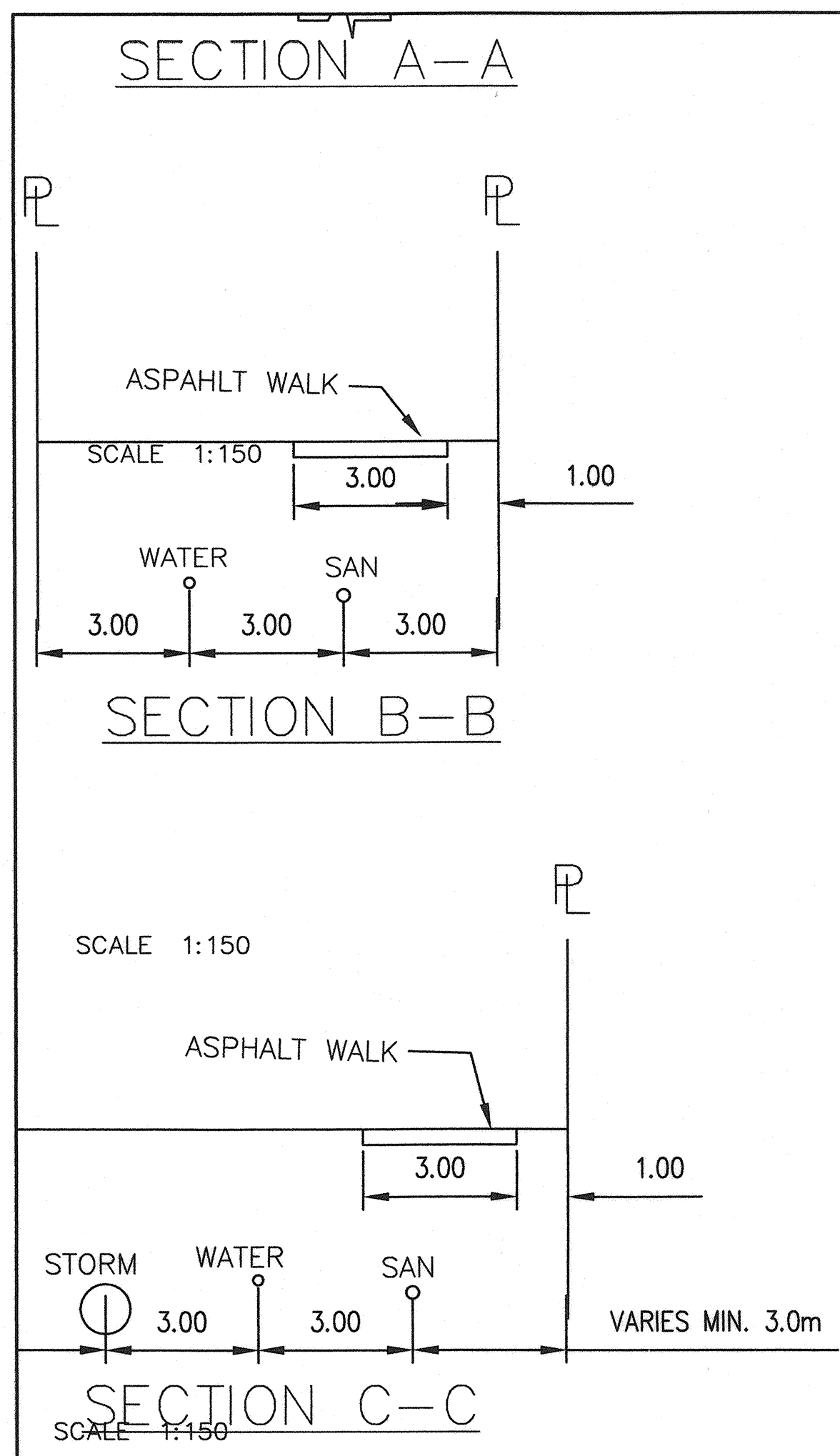
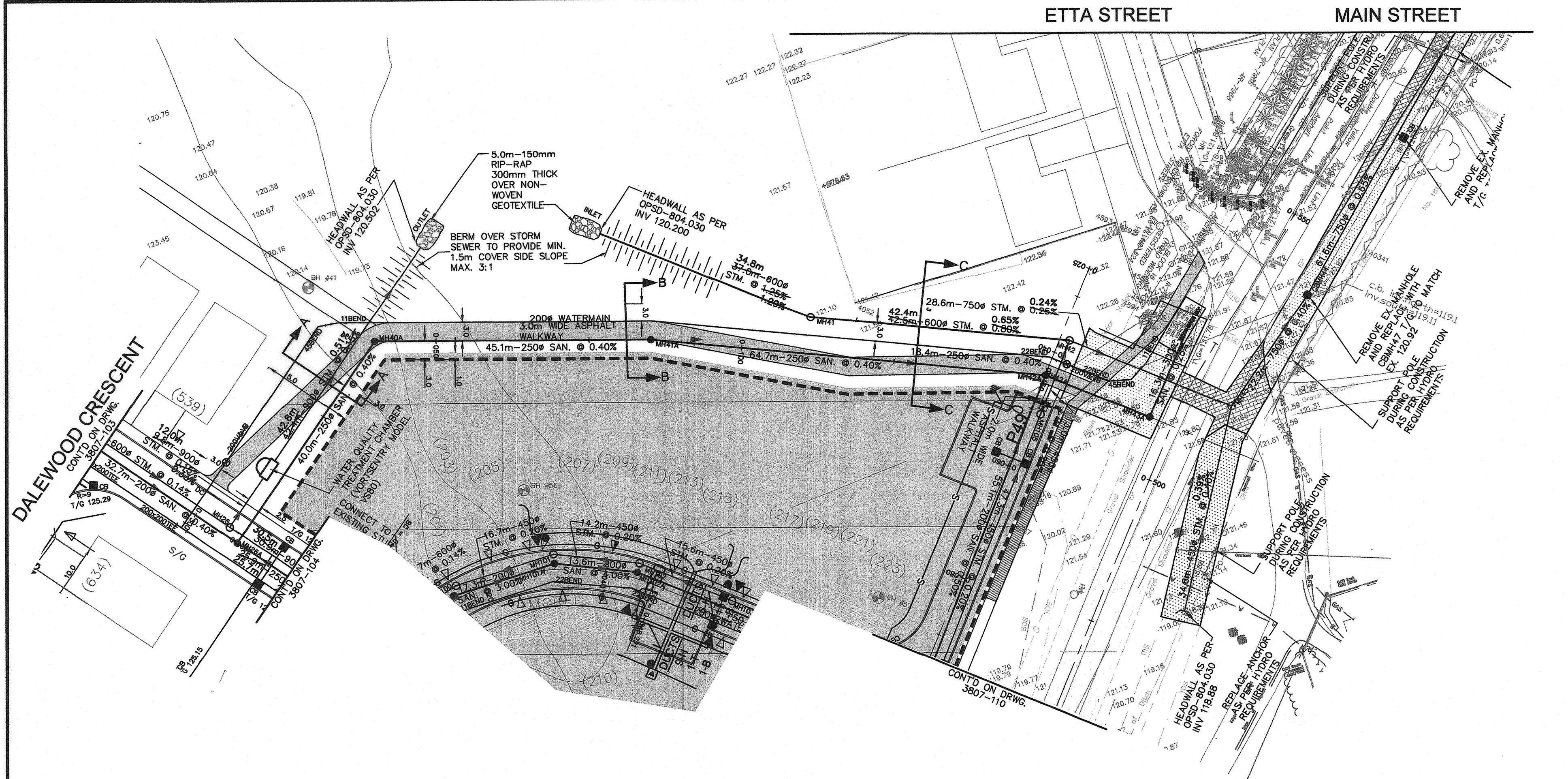
Signed: _____ Date: _____ 2016 Plan Number: 17274 D07-16-13-0033

NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowland Drive
Ottawa, Ontario, Canada K2M 1P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

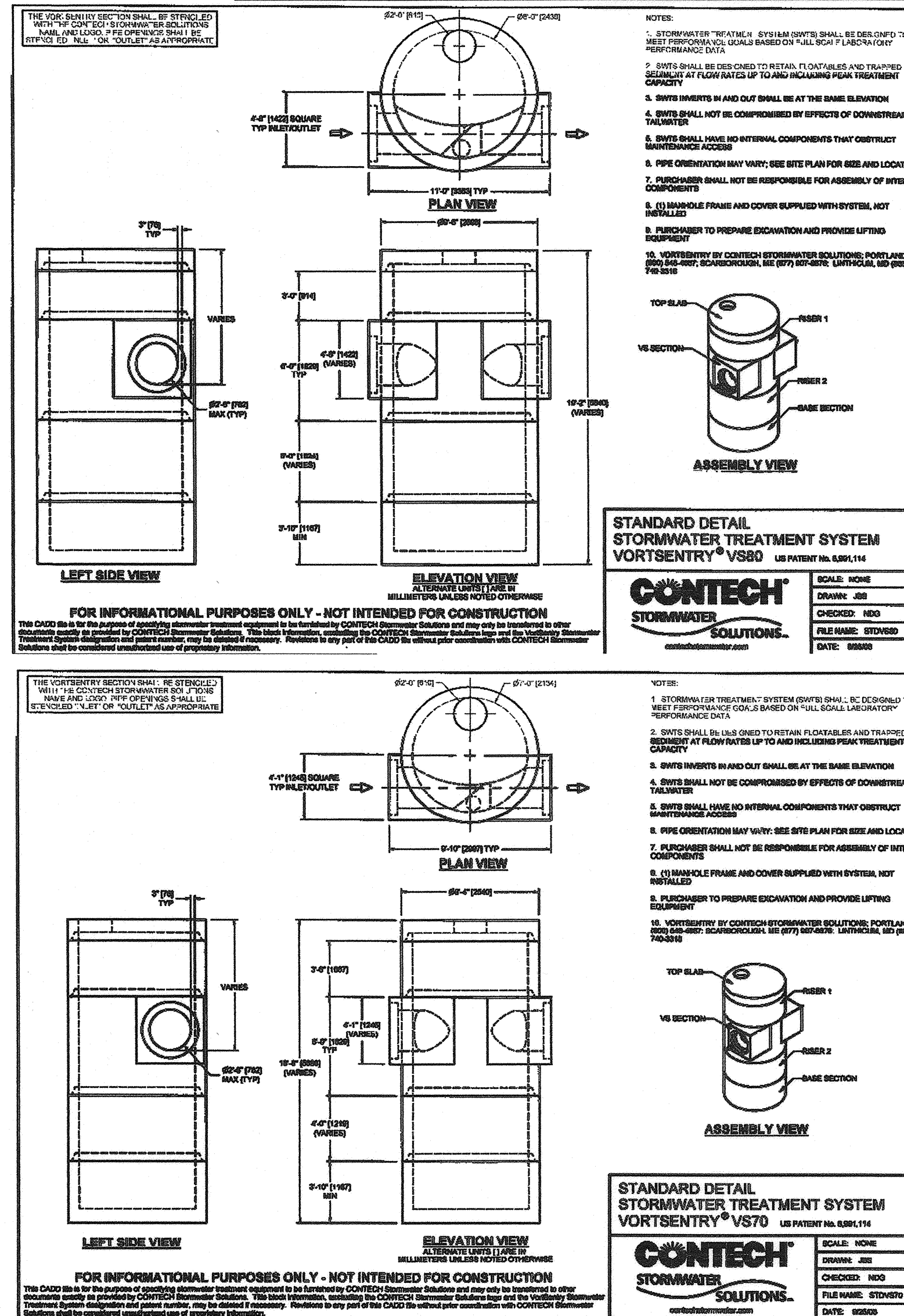
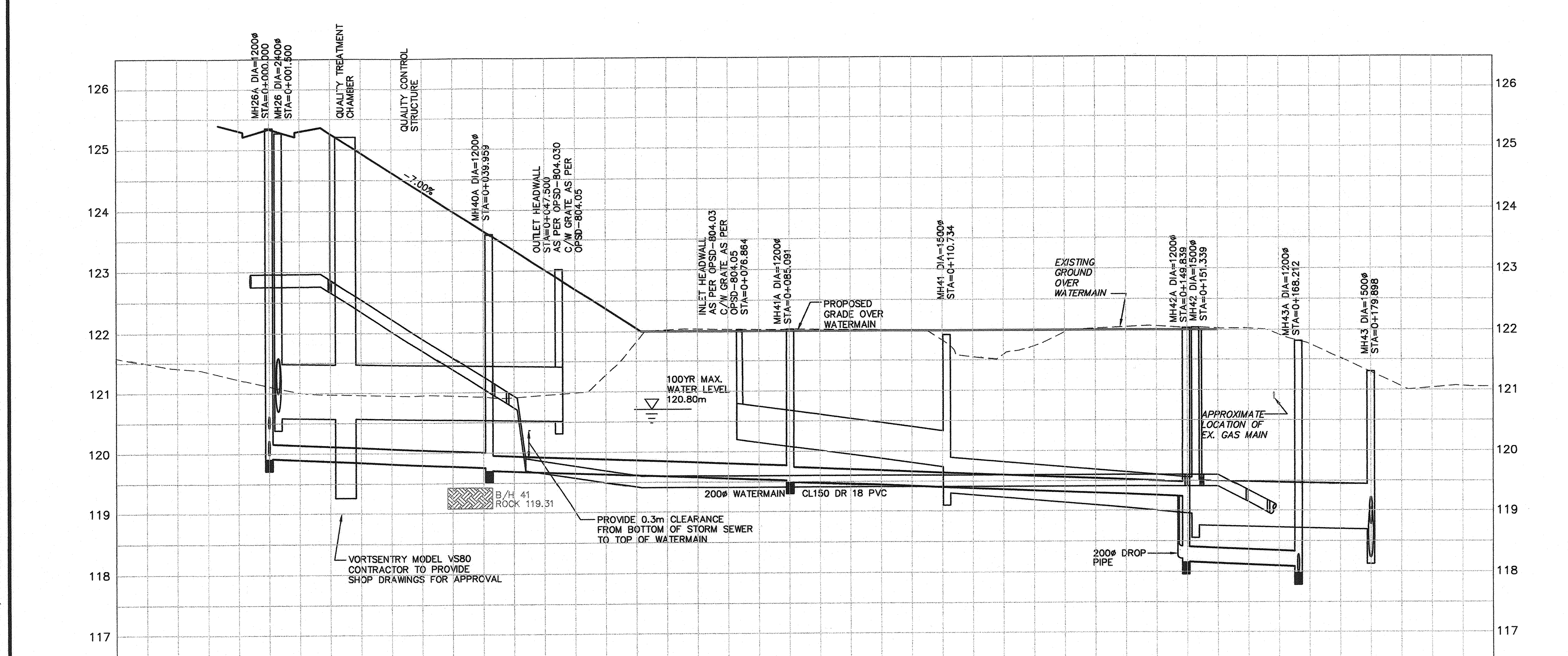
CITY OF OTTAWA
STITTSVILLE SOUTH - AREA 6
PLAN AND PROFILE
HARTSMERE DR.
STA 120+000.00 TO 120+164.85
WEST RIDGE DR.
STA 140+000.00 TO 140+075.00

PROJECT NO.: 113004-00
REV # 16
DRAWING NO.: 113004-PRO9

C:\03\113004\CAD\Drawings\113004-PRO9-09-ASB.dwg, 2016, Jan 22, 12:41:11pm, d26f6y



- NOTES:**
1. ALL WATERMAIN CONSTRUCTION IN ACCORDANCE WITH CURRENT CITY OF OTTAWA DRAWINGS & SPECIFICATIONS.
 2. ALL SEWER AND ROADWAY CONSTRUCTION IN ACCORDANCE WITH CURRENT CITY OF OTTAWA DRAWINGS & SPECIFICATIONS.
 3. ALL CONNECTIONS TO EXISTINGS WATERMAIN BY CITY OF OTTAWA FORCES. CONTRACTOR TO EXCAVATE, BACKFILL, COMPACT AND REINSTATE.
 4. CATHODIC PROTECTION AS PER CITY OF OTTAWA STANDARDS.
 5. INSULATION AT ALL STRUCTURES PER CITY OF OTTAWA DETAIL W23.
 6. ALL STREET AND REAR YARD CB'S TO HAVE INLET RESTRICTURES SEE DRAWING 3807-800.
 7. FOR LEGAL BOUNDARY INFORMATION REFER TO REGISTERED PLAN 4M BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD.



ROAD GRADE	TOP OF WATERMAIN	STM SEWER INVERT	SAN SEWER INVERT	STATION	ROAD GRADE	TOP OF WATERMAIN	STM SEWER INVERT	SAN SEWER INVERT	STATION
125.334	122.850	122.850	122.850	0+003	122.000	118.540	118.540	118.540	0+150
125.207	122.870	122.870	122.870	0+010	122.000	118.550	118.550	118.550	0+175
125.354	122.870	122.870	122.870	0+025	122.000	118.550	118.550	118.550	0+200
123.400	122.870	122.870	122.870	0+050	122.000	118.550	118.550	118.550	0+200
122.400	122.870	122.870	122.870	0+075	122.000	118.550	118.550	118.550	0+200
122.000	122.870	122.870	122.870	0+100	122.000	118.550	118.550	118.550	0+200
122.000	122.870	122.870	122.870	0+125	122.000	118.550	118.550	118.550	0+200
122.000	122.870	122.870	122.870	0+150	122.000	118.550	118.550	118.550	0+200
122.000	122.870	122.870	122.870	0+175	122.000	118.550	118.550	118.550	0+200
122.000	122.870	122.870	122.870	0+200	122.000	118.550	118.550	118.550	0+200

No.	REVISIONS	By	Date
14			
13			
12			
11			
10			
9	AS-BUILTS		10/03/11
8	REVISED AS PER NEW LEGAL		08/05/29
7	REVISE QTC AND MH'S	DGY	07/11/14
6	REVISE QCC AND MH'S	DGY	07/10/03
5	REVISED AS PER CITY COMMENTS	DGY	07/09/04
4	REVISED AS PER CITY COMMENTS	DGY	07/08/17
3	GENERAL REVISIONS	DGY	07/05/28
2	ISSUED FOR CITY REVIEW	DGY	07/05/10
1	ISSUED FOR OWNER REVIEW	DGY	07/01/15

IBI GROUP

333 Preston Street
Suite 400
Ottawa, Ontario
Canada K1S 5N4
Tel (613)225-1311
FAX (613)225-9888

Project Title

HARRIS LANDS
THOMAS CAVANAGH CONSTRUCTION LTD.

Professional Engineer
D. G. Yannopoulos
PROVINCE OF ONTARIO

Drawing Title

STORM OUTLET, SANITARY AND WATER CONNECTION
FROM DALEWOOD CRESCENT TO MAIN STREET

Scale

HOR. 1:500
VER. 1:50

Design

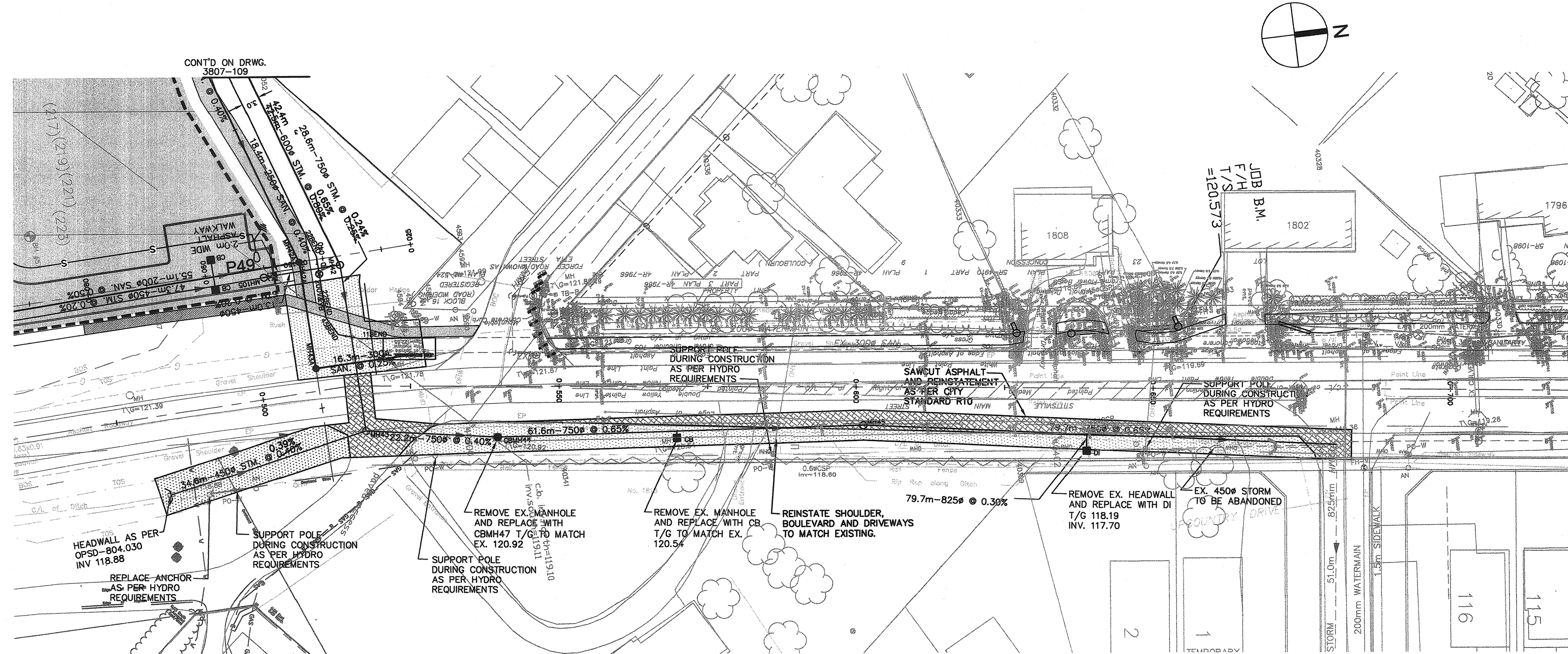
D.G.Y. Date JAN. 2007

Drawn

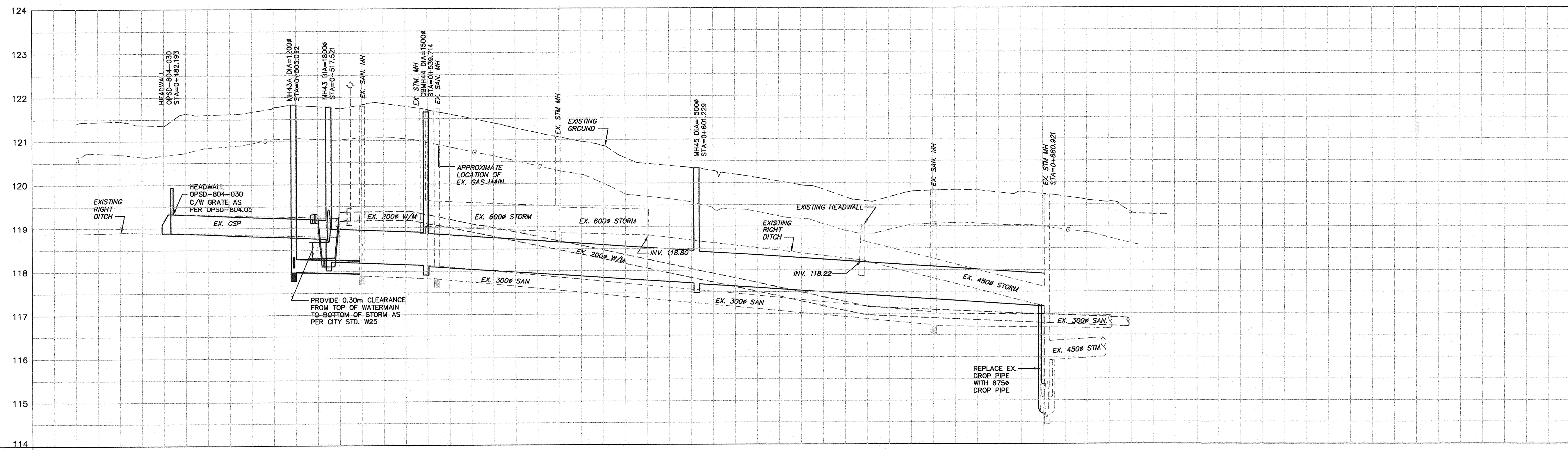
D.P.S. Checked D.G.Y.

Project No.

3807 Drawing No. 109



MAIN STREET



ROAD GRADE	ROAD GRADE	MAIN STREET	
TOP OF WATERMAIN	TOP OF WATERMAIN	FROM STA. 0+460 TO UPCOUNTRY DRIVE	
STM SEWER INVERT	STM SEWER INVERT	Scale HOR. 1:500 VER. 1:50	
SAN SEWER INVERT	SAN SEWER INVERT	Design	D.G.Y. Date JAN. 2007
STATION	STATION	Drawn	D.P.S. Checked D.G.Y.
		Project No.	3807
		Drawing No.	110

- NOTES :**
1. ALL WATERMAIN CONSTRUCTION IN ACCORDANCE WITH CURRENT CITY OF OTTAWA DRAWINGS & SPECIFICATIONS.
 2. ALL SEWER AND ROADWAY CONSTRUCTION IN ACCORDANCE WITH CURRENT CITY OF OTTAWA DRAWINGS & SPECIFICATIONS.
 3. ALL CONNECTIONS TO EXISTINGS WATERMAIN BY CITY OF OTTAWA FORCES. CONTRACTOR TO EXCAVATE, BACKFILL, COMPACT AND REINSTATE.
 4. CATHODIC PROTECTION AS PER CITY OF OTTAWA STANDARDS.
 5. INSULATION AT ALL STRUCTURES PER CITY OF OTTAWA DETAIL W23.
 6. ALL STREET AND REAR YARD CB'S TO HAVE INLET RESTRICTURES SEE DRAWING 3807-800.
 7. FOR LEGAL BOUNDARY INFORMATION REFER TO REGISTERED PLAN 4M BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD.

14			
13			
12			
11			
10			
9			
8			
7	AS-BUILTS	10/03/11	
6	REVISED AS PER NEW LEGAL	08/05/29	
5	REVISED AS PER CITY COMMENTS	DGY 07/09/04	
4	REVISED AS PER CITY COMMENTS	DGY 07/08/17	
3	GENERAL REVISIONS	DGY 07/05/28	
2	ISSUED FOR CITY REVIEW	DGY 07/05/10	
1	ISSUED FOR OWNER REVIEW	DGY 07/01/15	
No.	REVISIONS	By	Date

333 Preston Street
Suite 400
Ottawa, Ontario
Canada K1S 5N4
Tel (613)225-1311
FAX (613)225-9868

Project Title
HARRIS LANDS
THOMAS CAVANAGH CONSTRUCTION LTD.

A:\1907_HarrisLands\3.8 Drawings\Drawings\Main\3807-800-01-01.dwg: 110 MAIN ST SEWER AND STORMWATER INFRASTRUCTURE LAYOUT: 11/11/2007 10:43 AM: Printed At: 07/10/2010 8:43 AM: Plotted At: 07/10/2010 8:43 AM: Plotted By: MANSOURI, LAST: Saved By: MANSOURI, LAST: Saved At: 06/16/2010 10:43 AM

Brandon Mackechnie

From: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Sent: September 21, 2022 5:11 PM
To: Brandon Mackechnie
Cc: Angela Jonkman; Gorni, Colette; Jennifer McGahan
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

"CAUTION: External Sender"

Hi Brandon,

Thank you for your patience.

I have circulated your latest submission to the City's Asset Management Branch and met with them to discuss the proposed servicing. I have received the following notes:

- As per technical bulletin 2018-02, hydrants are to be measured along fire access roads to the building. Therefore, similar to the fire hydrant initially proposed on Hartsmere Drive, the existing fire hydrants on Parade Drive and Hartsmere Drive cannot be considered for fire protection. As such, two new fire hydrants are required for this site instead of one
- Private fire hydrants are not allowed for single residential properties. For safety matter, the proposed fire hydrants must be maintained by the City. Additionally, Fire Services wouldn't be aware that the private hydrants serve all three properties. Therefore, the proposed hydrants must be publicly owned and on City property.
- Easements in general are not desirable due to the challenge of obtaining unhindered access with equipment.
- City's easements must not include any other services.

The City originally required for this proposed development that watermain and sanitary sewer be extended from Stittsville Main to independently service the lots from municipal infrastructure. At that time, the consultant (Novatech) indicated that 170m of watermain and sewer extensions wouldn't be financially feasible and proposed instead 70m of private mains within the rear yard.

We're now at a stage where approximately 170m of watermain is required to properly service the proposed development along with a public easement through the site to maintain the fire hydrants lead, in addition to the private mains and easement along the back yard.

It is the City's opinion that the option to service the proposed development by extending the watermain and sanitary sewer from Stittsville Main along the laneway should be reconsidered for the following reasons:

Current Proposal	Watermain and Sanitary Sewer extensions from Stittsville Main along the laneway
<ul style="list-style-type: none">• 68.4m private sanitary main• 60.4m private storm main• Approximately 170m of watermain (considering the second fire hydrant required)• Two fire hydrants for fire protection	<ul style="list-style-type: none">• Less than 150m watermain and sanitary sewer extension• Storm sewer extension would not be required. The City would allow sump pumps for foundations drains discharging to the storm sewer within Hartsmere Drive through rear yard swale (or to the front yard if there's adequate outlet within the laneway)• Two fire hydrants for fire protection

<ul style="list-style-type: none"> • A third fire hydrant required at the rear yard if a flushing device is not provided (see W37.2) 	<ul style="list-style-type: none"> • Lot 1 can be serviced from the existing stubs within 205 Hartsmere Drive, if the applicant chooses to do so. This can reduce the required length of watermain and sanitary sewer extensions. Note that the owner would be responsible for the maintenance of the services crossing 205 Hartsmere Drive
<ul style="list-style-type: none"> • Each dwelling will be serviced from the back through shared private mains 	<ul style="list-style-type: none"> • Each dwelling will be serviced through the front from a public watermain as per current Guidelines
<ul style="list-style-type: none"> • Public easement required through the site and 205 Hartsmere Drive to maintain the fire hydrant leads. The easement would be minimum 6.0m free of any other services, trees, shrubs, fences, curbs and walls. • A separate private easement required for the private sewers and portion of the proposed watermain servicing dwellings 2 and 3. 	<ul style="list-style-type: none"> • No public or private easement required. The new lots would be independently serviced through public services. This would meet the City's standard requirement for severance that all parcels be independently serviced and directly connected to municipal services. There won't be any restrictions on the installations of fences, accessory structures or landscaping except within the swale area. Each owner would be responsible for the maintenance of the swale within their backyard and ensure drainage is not blocked for adjacent properties
<ul style="list-style-type: none"> • Owners would be responsible for the maintenance, repair and replacement of the private sewer mains and portion of watermain servicing dwellings 2 and 3. A Joint Use and Maintenance Agreement would be required 	<ul style="list-style-type: none"> • The City is responsible for the maintenance, repair, and replacement of public services

The next steps would be to confirm adequacy of services from Stittsville Main by:

- Conducting a hydraulic analysis to confirm adequate pressures and water age. New boundary conditions have been requested for connection to the watermain on Stittsville Main
- Demonstrating that gravity connection to the existing sanitary sewer can be accommodated. Asset Management Branch will advise of any capacity concerns

I am available to meet and discuss further should you have any questions or concerns

Regards,
Abi

From: Dieme, Abi
Sent: September 12, 2022 10:14 AM
To: Brandon Mackechnie <bmackechnie@rcii.com>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

Hi Brandon,

This proposal is not common and requires coordination with different engineers in the City's Asset Management team. I've received comments last week and our next step is an internal meeting to discuss the servicing configuration, specially the easement. We will likely meet this week. The City does not allow fire hydrants within residential single dwellings as they're required for fire protection and would be better maintained by the City.

I thank you for your patience and will reach out as soon as possible for an update.

Regards,
Abi

From: Brandon Mackechnie <bmackechnie@rcii.com>
Sent: September 12, 2022 10:07 AM
To: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi Abi,

I'm just following up on my previous email below to see if the City has any additional comments or concerns. Our Client is eager to resubmit but we would like to confirm that the City is generally accepting of our proposed changes before doing so.

Thanks,

Brandon MacKechnie, P.Eng. | Project Engineer

Robinson 350 Palladium Drive, Suite 210, Ottawa ON, K2V 1A8
Consultants T.(613) 592-6060 ext. 130 | rcii.com

This e-mail is intended solely for the individual or company to whom it is addressed. The information contained herein is confidential. Any dissemination, distribution or copying of this e-mail, other than by its intended recipient, is strictly prohibited. If you have received this e-mail in error, please notify the sender immediately, and delete this e-mail from your records. Thank you.

From: Brandon Mackechnie
Sent: August 31, 2022 3:44 PM
To: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: FW: 1835 Stittsville Main - Revised Hydrant Location

Hi Abi,

Please refer to our responses in "red" below:

1. Proposed hydrant 1 must be a public hydrant as it will be located within the Right-of-Way (ROW). This includes the proposed hydrant lead crossing the property up to the connection to the watermain on Hartsmere Drive; therefore the easement would be in favor of the City. City's watermain easements are required to be free of any

private utilities and/or sewers. The Infrastructure and Water Services Department (IWSD) is being consulted on possibility to allow the proposed configuration on an exceptional basis provided that the easement is widened to facilitate access and maintenance of the future public watermain. The required easement width would be confirmed by IWSD. **Response: The hydrant location can be pulled back to be fully contained with the property of Lot 1 and therefore will remain as a private hydrant. The hydrant will still be accessible from the existing driveway within the City ROW. Will this resolve the City's concerns regarding the hydrant being public and required easements?**

2. Access easement will be required through 205 Hartsmere Drive. **Response: The 205 Hartsmere property should already have a 6.0m easement which contains the existing storm, sanitary, and watermain stubs previously approved and installed to service this property.**
3. Please provide the results of the water age analysis **Response: Outputs from the water age analysis are attached. The analysis was simulated for a period of 14 days using the average day demand of 0.033 L/s.**
4. The size of the watermain must be either 203mm or 252mm, there is no 214mm per City Guidelines. **Response: To clarify, the nominal watermain diameter will be 200mm in keeping with City guidelines. The inside diameter of a 200mm diameter PVC pipe, used in the modelling analysis, is 214mm.**
5. A fire hydrant (or any other flushing device) is required within the rear yard dead-end **Response: The proposed watermain layout terminates with the service connection to the furthest dwelling to improve water quality in keeping with the City detail for end-end streets (W37.2). Further, the water age analysis provided has indicated that worst case (Lot 3) water age is only 9.8 hours and therefore a flushing device is not warranted.**
6. The horizontal distance between the watermain and sewers must be revised to 2.5m between the edges. For alternatives, please consult F-6-1 – Procedures to govern separation of sewers and watermains. **Response: In accordance with F-6-1, for watermains and sewers with less than 2.5m of horizontal separation, the sewer shall be constructed of materials and with joints equivalent to watermain standards. Sewers which meet these requirements will be specified.**
7. Unrelated to water servicing, the storm sewer configuration within 205 Hartsmere Drive doesn't seem accurate based on information available in the records. **Response: The existing servicing shown is in keeping with the as-built drawings, prepared by Novatech for the Stittsville South – Area 6 development. The drawings are attached for your reference.**

If you have any questions, please don't hesitate to contact me.

Regards,

Brandon MacKechnie, P.Eng. | Project Engineer

Robinson Consultants 350 Palladium Drive, Suite 210, Ottawa ON, K2V 1A8
T.(613) 592-6060 ext. 130 | rcii.com

This e-mail is intended solely for the individual or company to whom it is addressed. The information contained herein is confidential. Any dissemination, distribution or copying of this e-mail, other than by its intended recipient, is strictly prohibited. If you have received this e-mail in error, please notify the sender immediately, and delete this e-mail from your records. Thank you.

From: Dieme, Abi <Abibatou.Dieme@ottawa.ca>

Sent: August 29, 2022 12:02 PM

To: Brandon MacKechnie <bmackechnie@rcii.com>

Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>

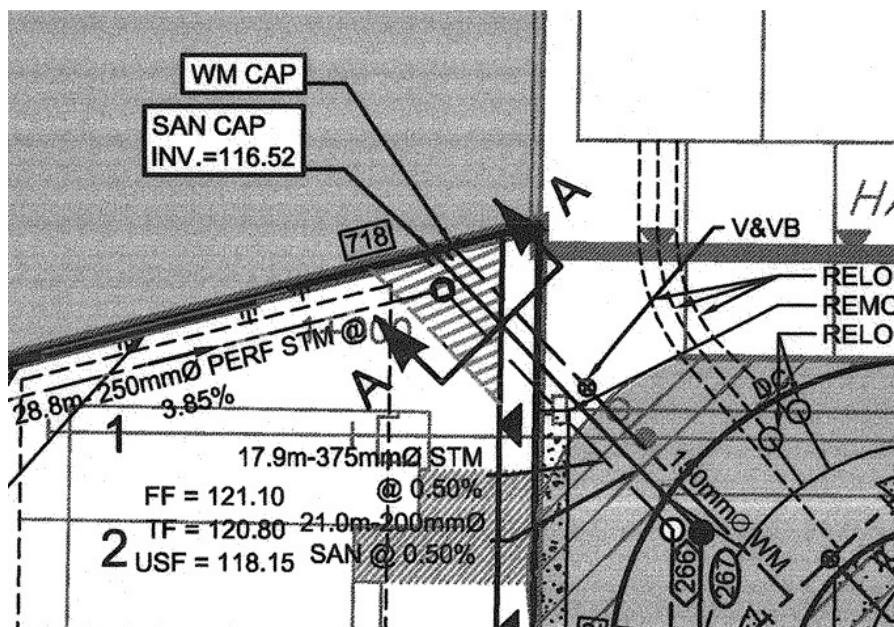
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

"CAUTION: External Sender"

Hello,

I am still waiting for feedback from the Infrastructure and Water Services Department (IWSD) . For now I can provide the following notes. I'll update them once I hear back from IWSD:

1. Proposed hydrant 1 must be a public hydrant as it will be located within the Right-of-Way (ROW). This includes the proposed hydrant lead crossing the property up to the connection to the watermain on Hartsmere Drive; therefore the easement would be in favor of the City. City's watermain easements are required to be free of any private utilities and/or sewers. The Infrastructure and Water Services Department (IWSD) is being consulted on possibility to allow the proposed configuration on an exceptional basis provided that the easement is widened to facilitate access and maintenance of the future public watermain. The required easement width would be confirmed by IWSD.
2. Access easement will be required through 205 Hartsmere Drive.
3. Please provide the results of the water age analysis
4. The size of the watermain must be either 203mm or 252mm, there is no 214mm per City Guidelines.
5. A fire hydrant (or any other flushing device) is required within the rear yard dead-end
6. The horizontal distance between the watermain and sewers must be revised to 2.5m between the edges. For alternatives, please consult *F-6-1 – Procedures to govern separation of sewers and watermains*.
7. Unrelated to water servicing, the storm sewer configuration within 205 Hartsmere Drive doesn't seem accurate based on information available in the records.



Regards,
Abi

From: Brandon Mackechnie <bmackechnie@rcii.com>
Sent: August 29, 2022 8:09 AM
To: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi Abi,

I'm just following up for a status update on the review of our revised hydrant location for 1835 Stittsville Main Street previously sent for comments on August 5th.

Thanks,

Brandon MacKechnie, P.Eng. | Project Engineer

Robinson 350 Palladium Drive, Suite 210, Ottawa ON, K2V 1A8
Consultants T.(613) 592-6060 ext. 130 | rcii.com

This e-mail is intended solely for the individual or company to whom it is addressed. The information contained herein is confidential. Any dissemination, distribution or copying of this e-mail, other than by its intended recipient, is strictly prohibited. If you have received this e-mail in error, please notify the sender immediately, and delete this e-mail from your records. Thank you.

From: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Sent: August 15, 2022 4:05 PM
To: Brandon MacKechnie <bmackechnie@rcii.com>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

"CAUTION: External Sender"

Hello,

I've circulated the Water Department. I'll send you comments as soon as they get back to me.

Regards,
Abi

From: Brandon MacKechnie <bmackechnie@rcii.com>
Sent: August 15, 2022 3:49 PM
To: Dieme, Abi <Abibatou.Dieme@ottawa.ca>
Cc: Angela Jonkman <ajonkman@rcii.com>; Gorni, Colette <colette.gorni@ottawa.ca>
Subject: RE: 1835 Stittsville Main - Revised Hydrant Location

CAUTION: This email originated from an External Sender. Please do not click links or open attachments unless you recognize the source.

ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

I'm just following up on my previous email below to see if you have had a chance to review our revised hydrant location. Our client is eager to resubmit, but we would like to ensure that the City is generally satisfied with our revised approach before we waste anymore time with a resubmission.

Thanks,

Brandon MacKechnie, P.Eng. | Project Engineer

Robinson Consultants 350 Palladium Drive, Suite 210, Ottawa ON, K2V 1A8
T.(613) 592-6060 ext. 130 | rcii.com

This e-mail is intended solely for the individual or company to whom it is addressed. The information contained herein is confidential. Any dissemination, distribution or copying of this e-mail, other than by its intended recipient, is strictly prohibited. If you have received this e-mail in error, please notify the sender immediately, and delete this e-mail from your records. Thank you.

From: Brandon Mackechnie
Sent: August 5, 2022 12:02 PM
To: Abibatou.Dieme@ottawa.ca
Cc: Angela Jonkman <ajonkman@rcii.com>; colette.gorni@ottawa.ca
Subject: 1835 Stittsville Main - Revised Hydrant Location

Hi Abi,

Refer to the attached plan for our revised approach to address the City's concerns regarding fire protection access. The key changes are as follows:

1. The existing 150mm dia. watermain stub will be blanked to the satisfaction of the City.
2. A new 214mm dia. PVC-O watermain connection will be made to the existing 250mm diameter watermain on Hartsmere Drive.
3. The 214mm watermain will be extended to the City ROW along Stittsville Main Street to provide a new hydrant adjacent to the existing driveway (owned by the City).
4. Increasing the pipe size from 150mm to 214mm will provide increased fire flow to meet the minimum requirement for the development.
5. We have located the watermain on the south side of the shared lot line between Lot 1 (existing) and Lot 2 since Lot 1 has more available area to install a watermain.
6. We have tentatively shown a 6.0m easement where the watermain passes through Lot 1. The requirements for this easement will need to be discussed with the City.
7. City of Ottawa fire services will need to review the revised hydrant location and comment if the existing driveway (owned by the City) is satisfactory for use as a fire route. The existing driveway does not meet the minimum 6.0m width requirement, but hopefully they will be willing to make an exception given the constraints present with this parcel of land.
8. The main concern of not having an accessible route between the hydrant and the main entrances of the dwellings has been addressed.

Can you please review the attached plan and provide any additional comments or concerns the City may have before we finalize our resubmission package. Note that I will be on holidays for the week of August 8-12th so please copy Angela on any responses to this email.

Thanks,

Brandon MacKechnie, P.Eng. | Project Engineer

Robinson Consultants 350 Palladium Drive, Suite 210, Ottawa ON, K2V 1A8
T.(613) 592-6060 ext. 130 | rcii.com

This e-mail is intended solely for the individual or company to whom it is addressed. The information contained herein is confidential. Any dissemination, distribution or copying of this e-mail, other than by its intended recipient, is strictly prohibited. If you have received this e-mail in error, please notify the sender immediately, and delete this e-mail from your records. Thank you.

'
This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

'
This email originated from an External Sender. Please do not click links, open attachments or reply unless you recognize the source.

'
This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

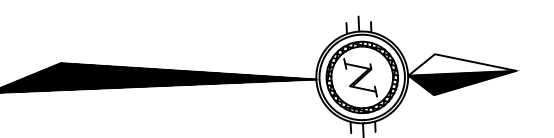
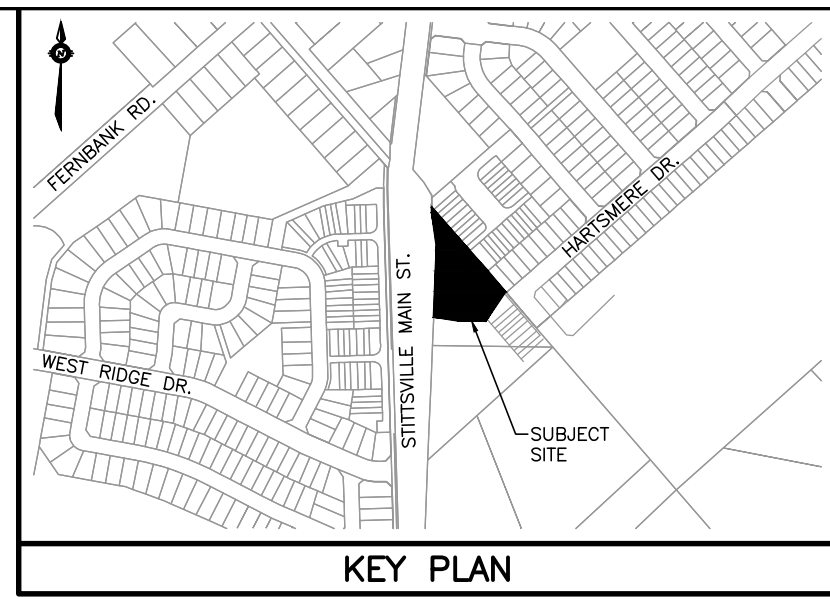
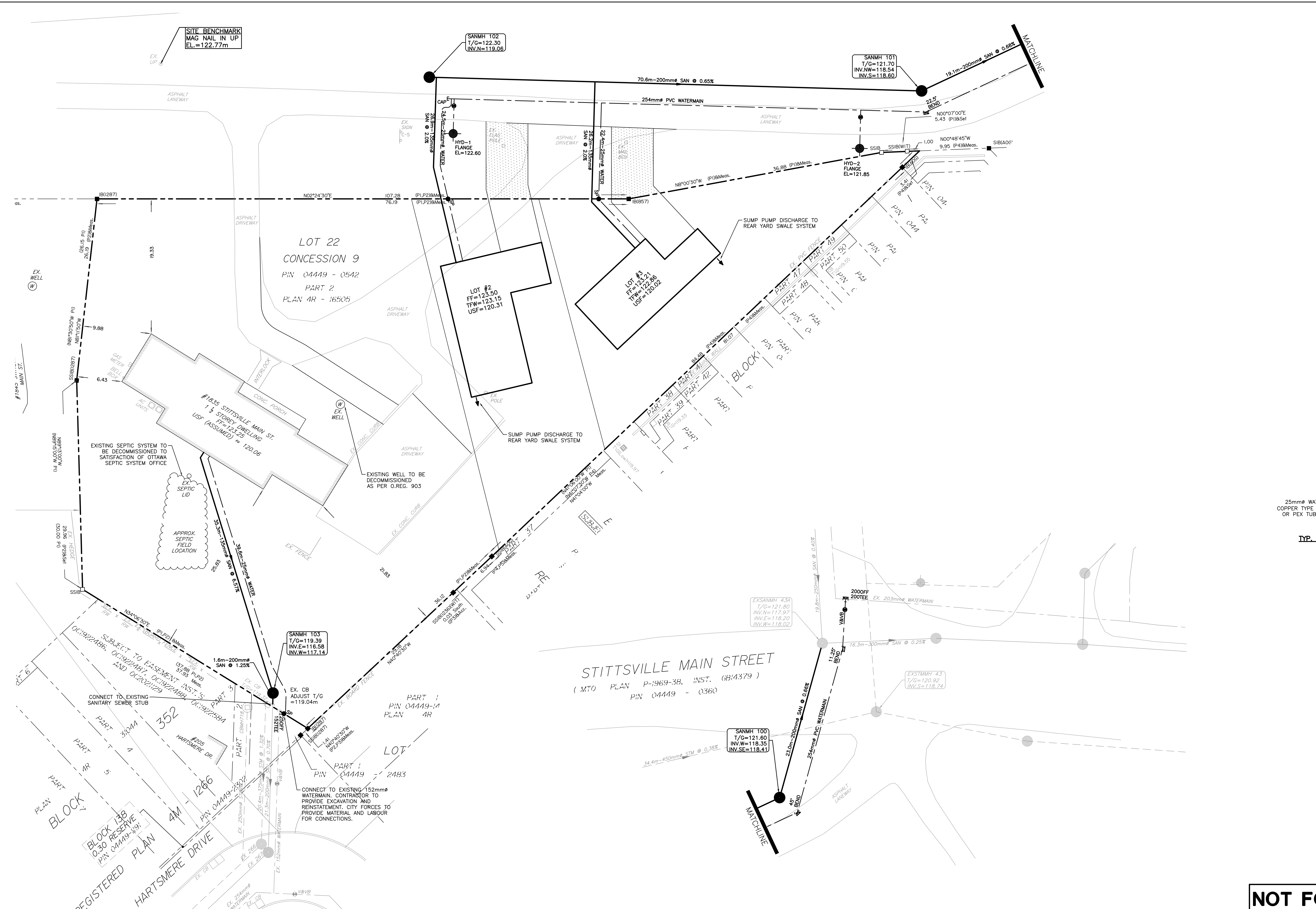
Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

'
This email originated from an External Sender. Please do not click links, open attachments or reply unless you recognize the source.

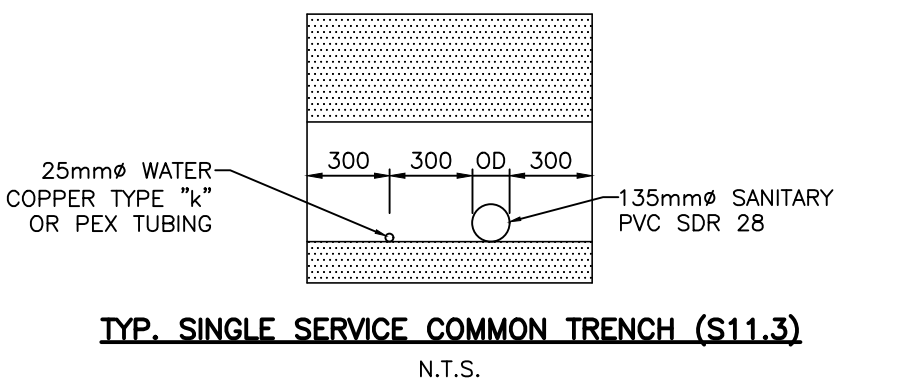
'
This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

Le présent courriel a été expédié par le système de courriels de la Ville d'Ottawa. Toute distribution, utilisation ou reproduction du courriel ou des renseignements qui s'y trouvent par une personne autre que son destinataire prévu est interdite. Je vous remercie de votre collaboration.

'
This email originated from an External Sender. Please do not click links, open attachments or reply unless you recognize the source.



- LEGEND**
- PROPERTY BOUNDARY
 - EXISTING HYDRANT
 - ⊕ EXISTING VALVE & VALVE BOX
 - EXISTING WATERMAIN
 - EXISTING SANITARY SEWER & MANHOLE
 - EXISTING STORM SEWER & MANHOLE
 - EXISTING CATCH BASIN
 - HYDRANT
 - SP CURB STOP & SERVICE POST
 - WATERMAIN
 - SANITARY SEWER & MANHOLE

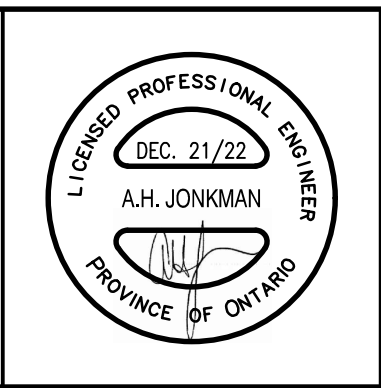
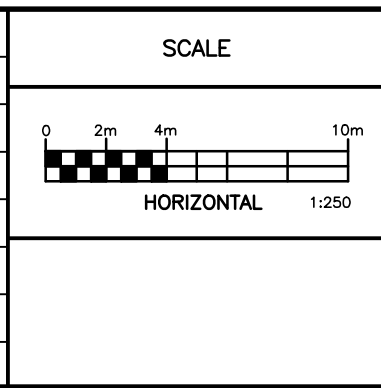


NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
4	REVISED PER COMMENTS	21/12/22	AHJ
3	REVISED PER COMMENTS	01/11/22	AHJ
2	REVISED PER COMMENTS	07/06/22	AHJ
1	ISSUED FOR ZONING BY-LAW AMENDMENT	10/02/22	AHJ



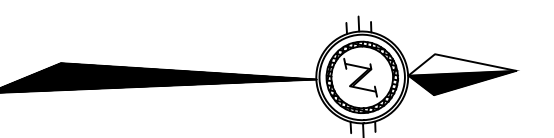
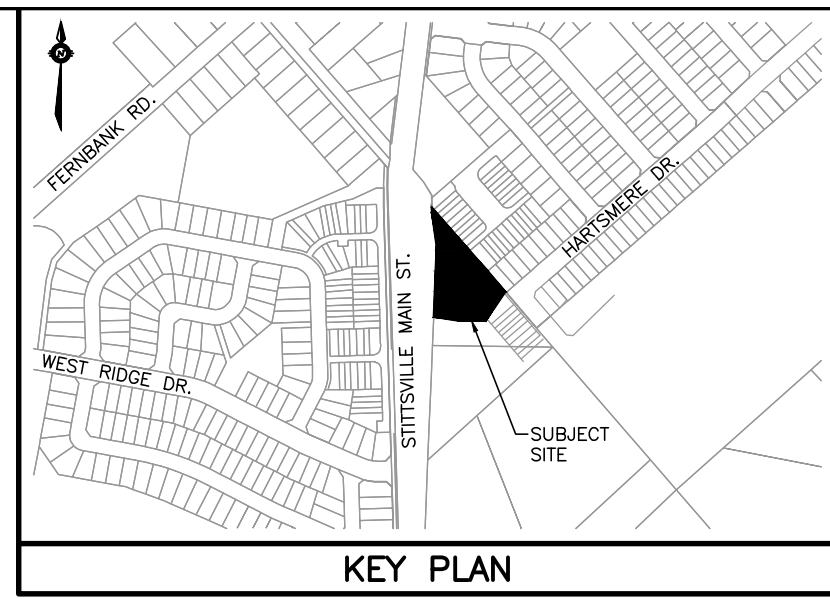
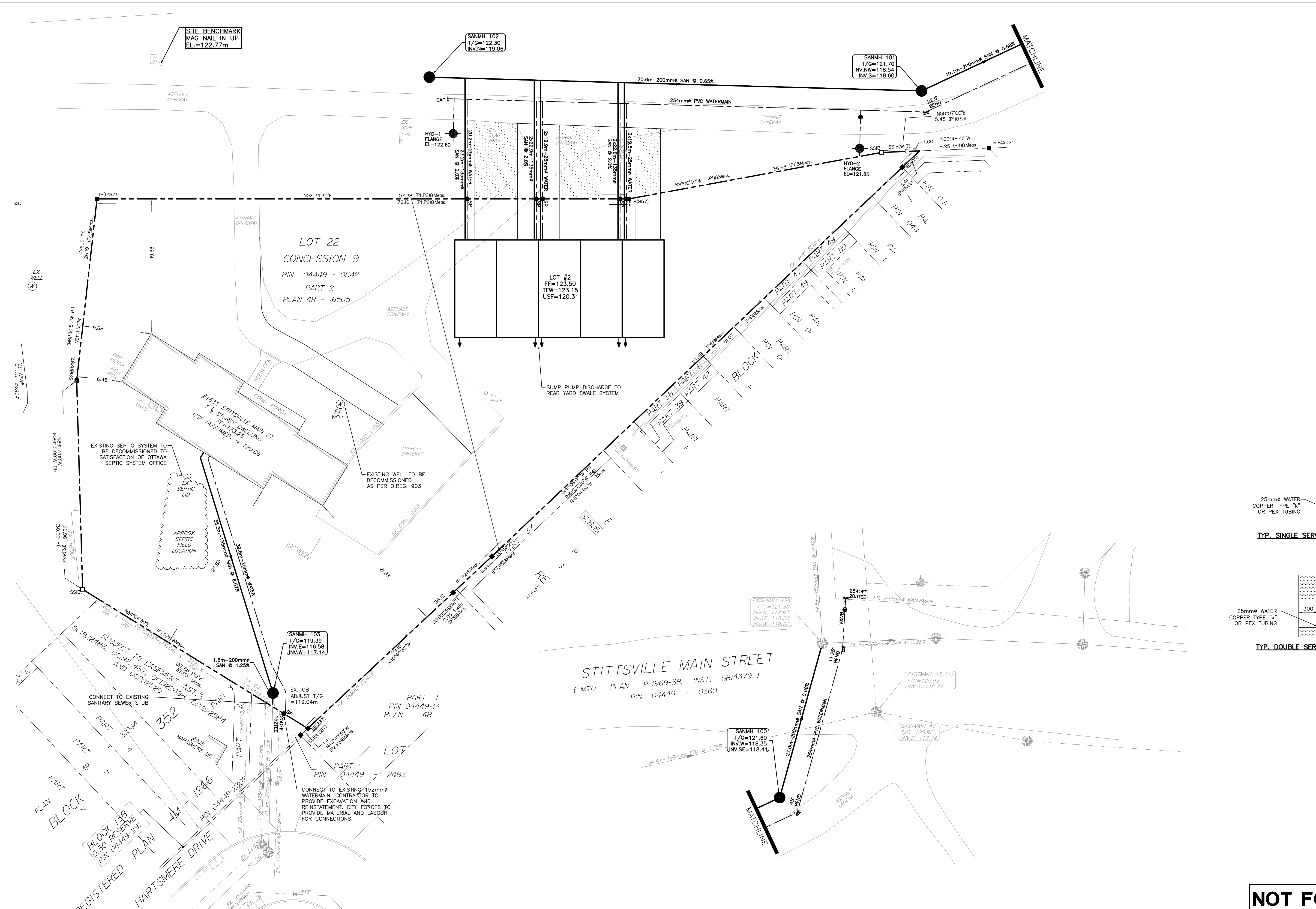
Robinson Land Development
 350 Palladium Drive
 Ottawa, ON K2V 1A8
 (613) 592-6060 roii.com

DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

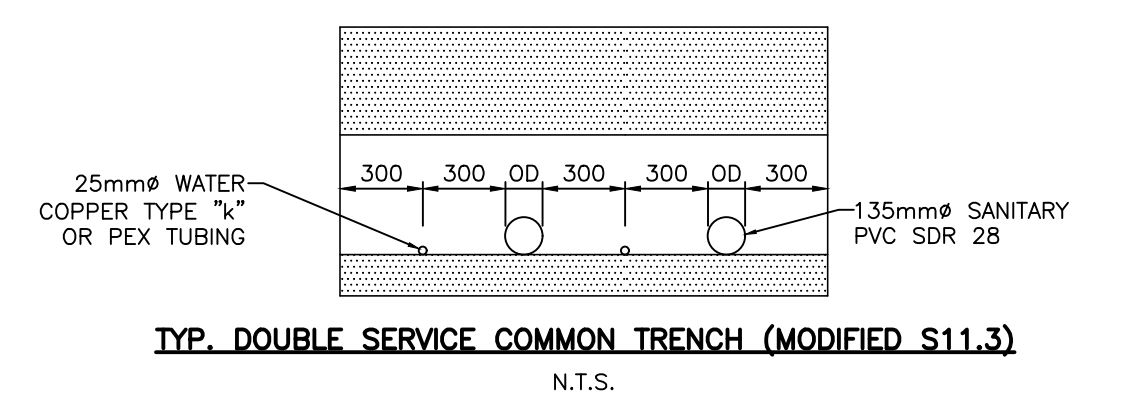
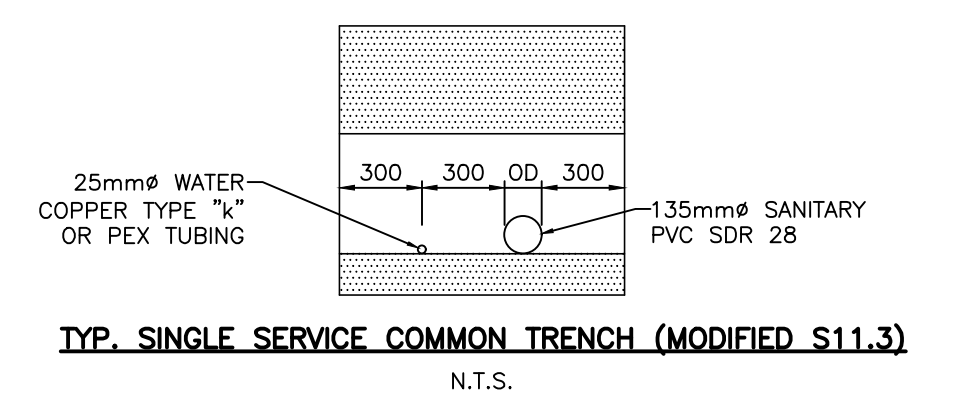
JENNIFER McGAHAN WIRE INVESTMENT CORP.
 1835 STITTSVILLE MAIN STREET
 STITTSVILLE, ON

PROPOSED DEVELOPMENT CONCEPTUAL SERVICING PLAN

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-S1



- LEGEND**
- PROPERTY BOUNDARY
 - EXISTING HYDRANT
 - ⊕ EXISTING VALVE & VALVE BOX
 - EXISTING WATERMAIN
 - EXISTING SANITARY SEWER & MANHOLE
 - EXISTING STORM SEWER & MANHOLE
 - EXISTING CATCH BASIN
 - HYDRANT
 - SP CURB STOP & SERVICE POST
 - WATERMAIN
 - SANITARY SEWER & MANHOLE



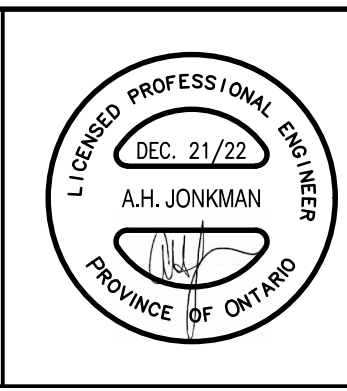
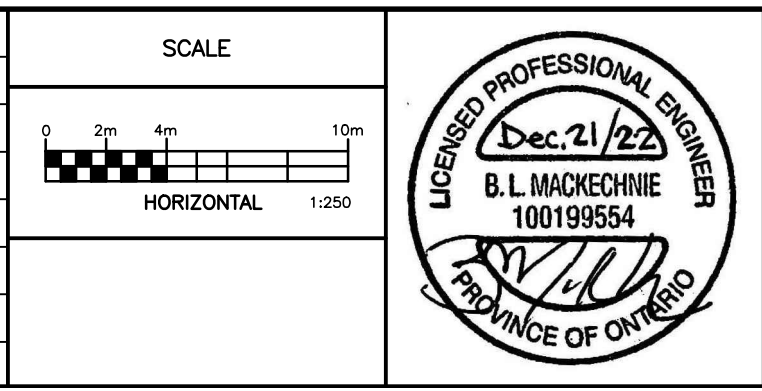
NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
2	REVISED PER COMMENTS	21/12/22	AHJ
1	REVISED PER COMMENTS	01/11/22	AHJ

SCALE	
HORIZONTAL 1:250	



Robinson Land Development

350 Palladium Drive
Ottawa, ON K2V 1A8
(613) 592-6060 roii.com

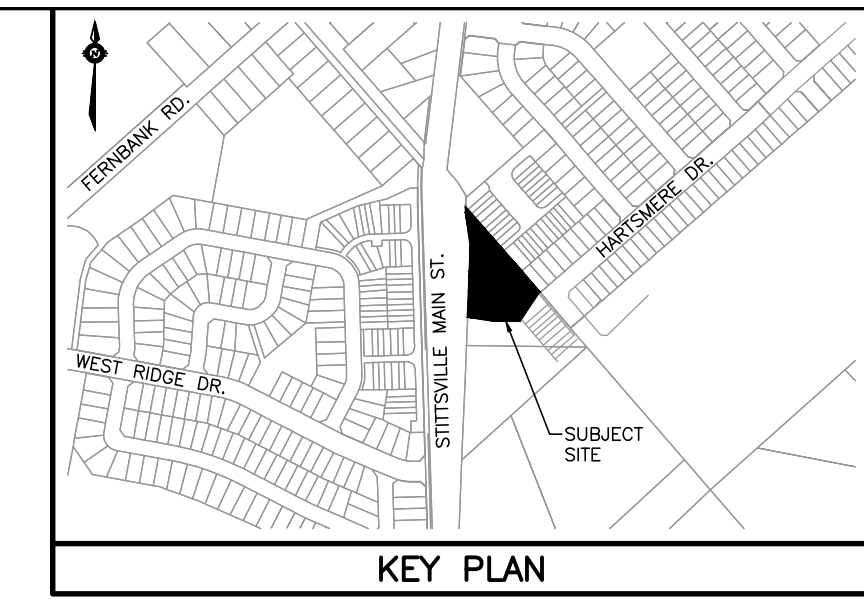
DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

**JENNIFER McGAHAN
WIRE INVESTMENT CORP.**

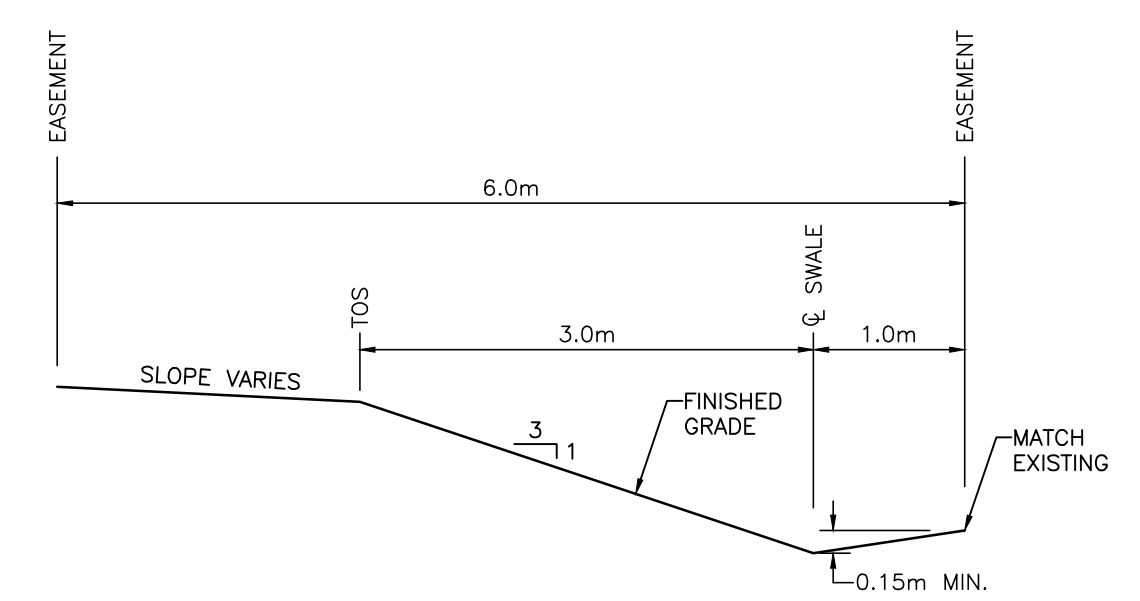
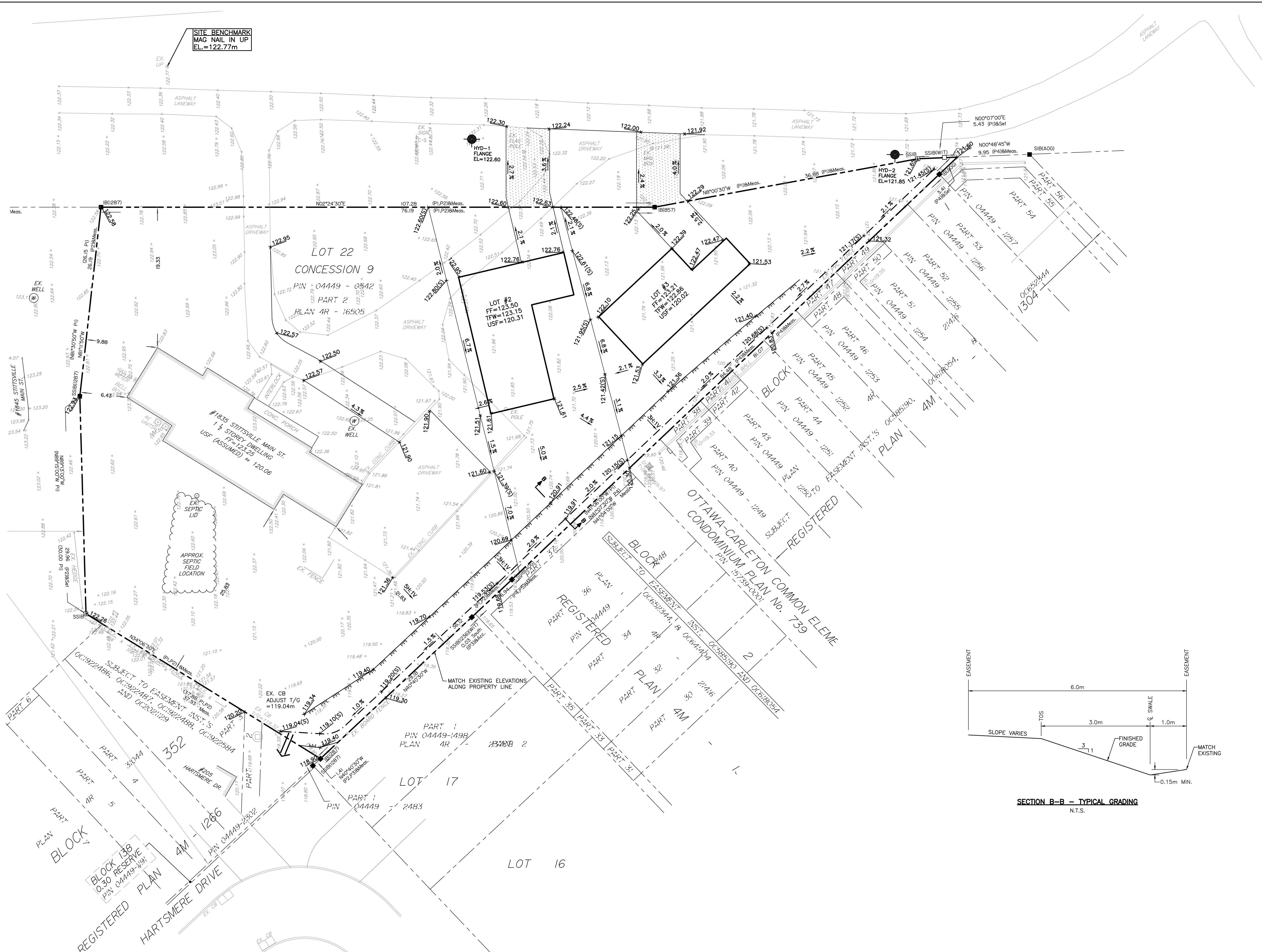
1835 STITTSVILLE MAIN STREET
STITTSVILLE, ON

**DEMONSTRATION DEVELOPMENT
CONCEPTUAL SERVICING PLAN**

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-S2



- LEGEND**
- PROPERTY BOUNDARY
 - +121.00 EXISTING ELEVATION
 - 121.00 PROPOSED GRADE
 - 121.00(S) PROPOSED SWALE GRADE
 - 2.0% DRAINAGE DIRECTION AND SLOPE
 - SWALE
 - TERRACING (3H:1V MAX.)
 - HYDRANT
 - MAJOR OVERLAND FLOW ROUTE
- FF: FINISHED FLOOR
TFW: TOP OF FOUNDATION
USF: UNDERSIDE OF FOOTING



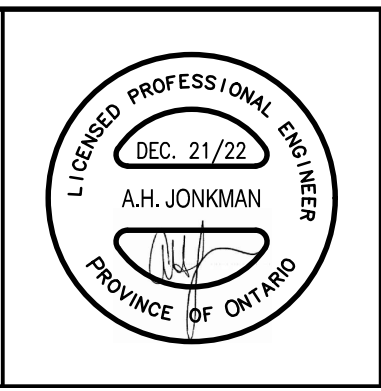
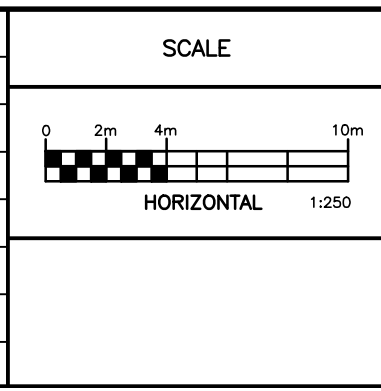
SECTION B-B - TYPICAL GRADING
N.T.S.

NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
4	REVISED PER COMMENTS	21/12/22	AHJ
3	REVISED PER COMMENTS	01/11/22	AHJ
2	REVISED PER COMMENTS	07/06/22	AHJ
1	ISSUED FOR ZONING BY-LAW AMENDMENT	10/02/22	AHJ



Robinson Land Development

350 Palladium Drive
Ottawa, ON K2V 1A8
(613) 592-6060 roii.com

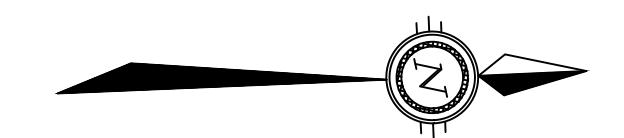
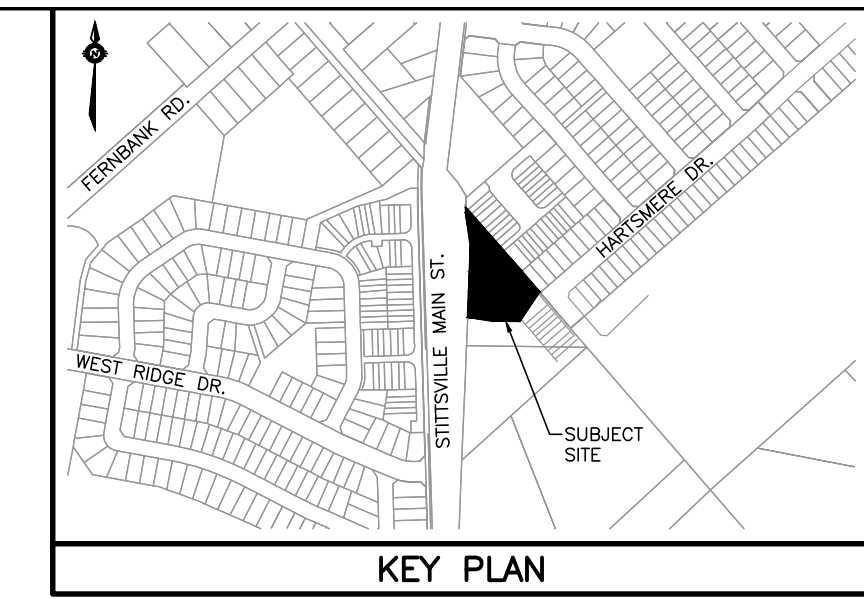
DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

**JENNIFER McGAHAN
WIRE INVESTMENT CORP.**

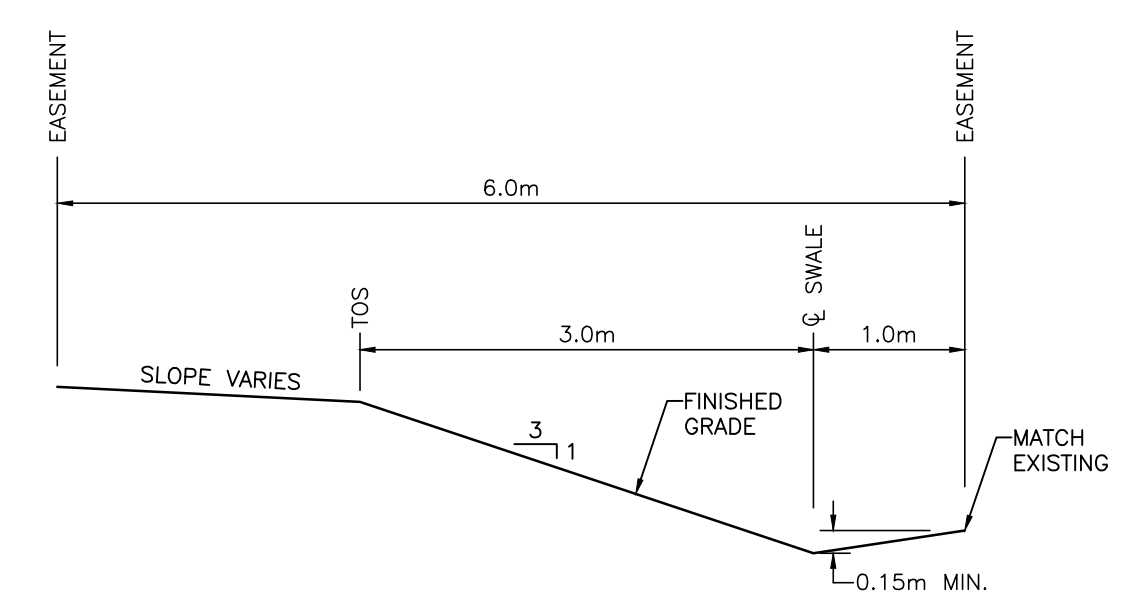
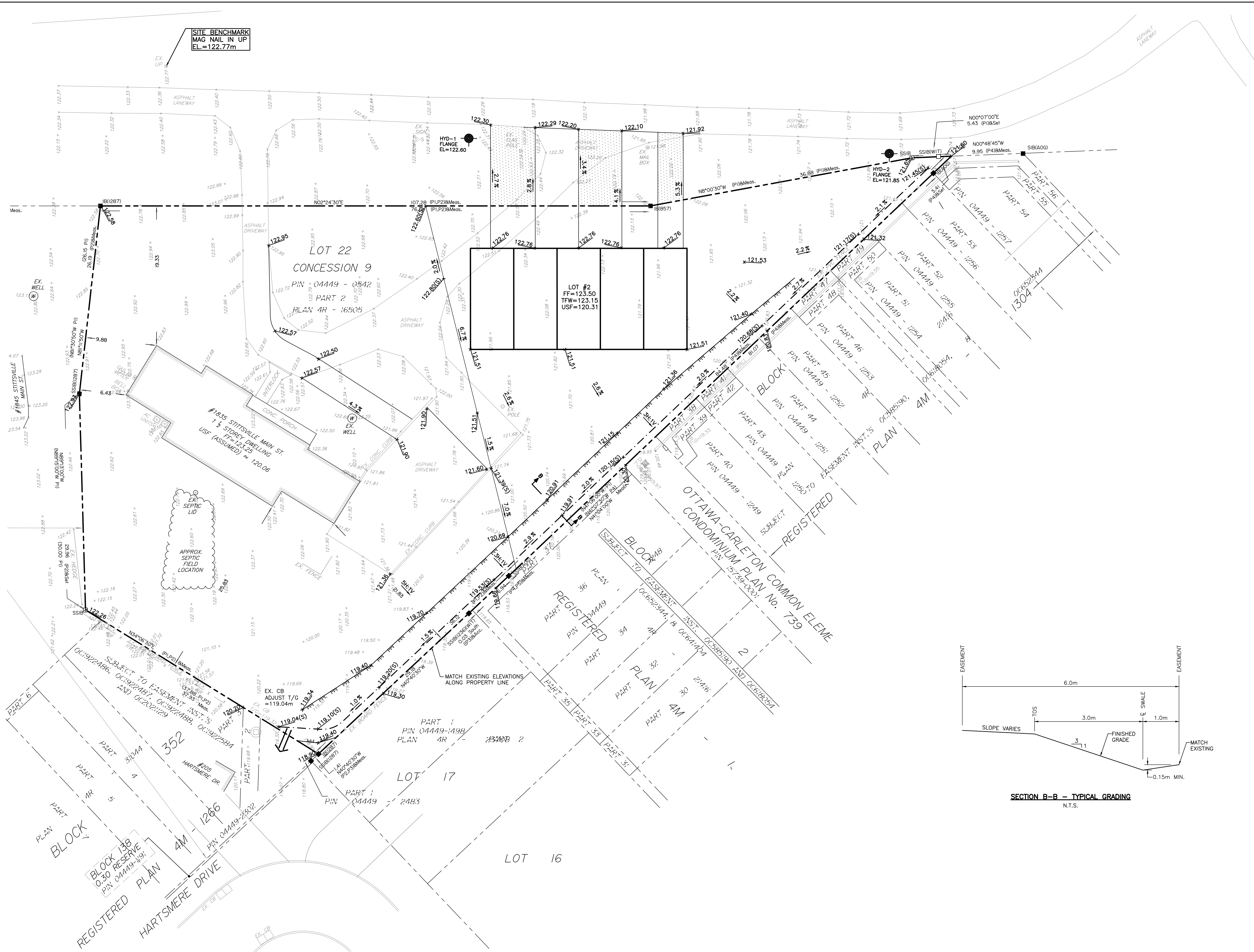
1835 STITTSVILLE MAIN STREET
STITTSVILLE, ON

**PROPOSED DEVELOPMENT
CONCEPTUAL GRADING PLAN**

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-GR1



- LEGEND**
- PROPERTY BOUNDARY
 - +121.00 EXISTING ELEVATION
 - 121.00(S) PROPOSED GRADE
 - 121.00(S) PROPOSED SWALE GRADE
 - 2.0% DRAINAGE DIRECTION AND SLOPE
 - SWALE
 - TERRACING (3H:1V MAX.)
 - HYDRANT
 - MAJOR OVERLAND FLOW ROUTE
- FF: FINISHED FLOOR
TFW: TOP OF FOUNDATION
USF: UNDERSIDE OF FOOTING



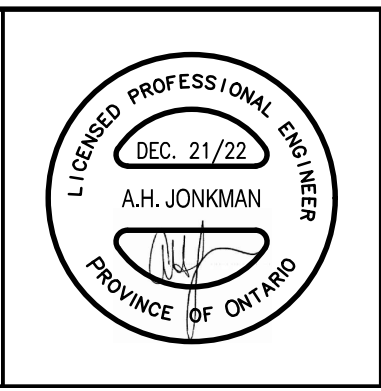
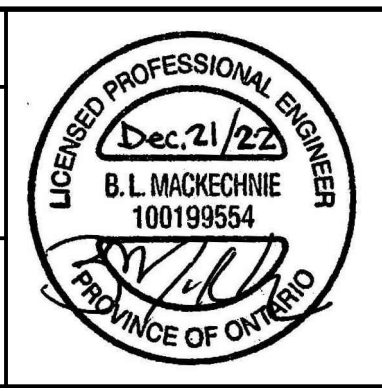
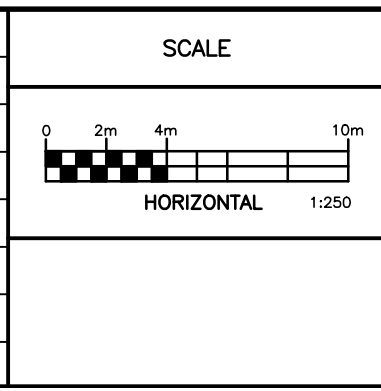
SECTION B-B - TYPICAL GRADING
N.T.S.

NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
2	REVISED PER COMMENTS	21/12/22	AHJ
1	REVISED PER COMMENTS	01/11/22	AHJ



Robinson Land Development

350 Palladium Drive
Ottawa, ON K2V 1A8
(613) 592-6060 roii.com

DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

JENNIFER McGAHAN
WIRE INVESTMENT CORP.
1835 STITTSVILLE MAIN STREET
STITTSVILLE, ON

DEMONSTRATION DEVELOPMENT
CONCEPTUAL GRADING PLAN

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-GR2

Attachment B

Hydraulic Water Model Figure

Boundary Conditions

Watermain Design Sheets

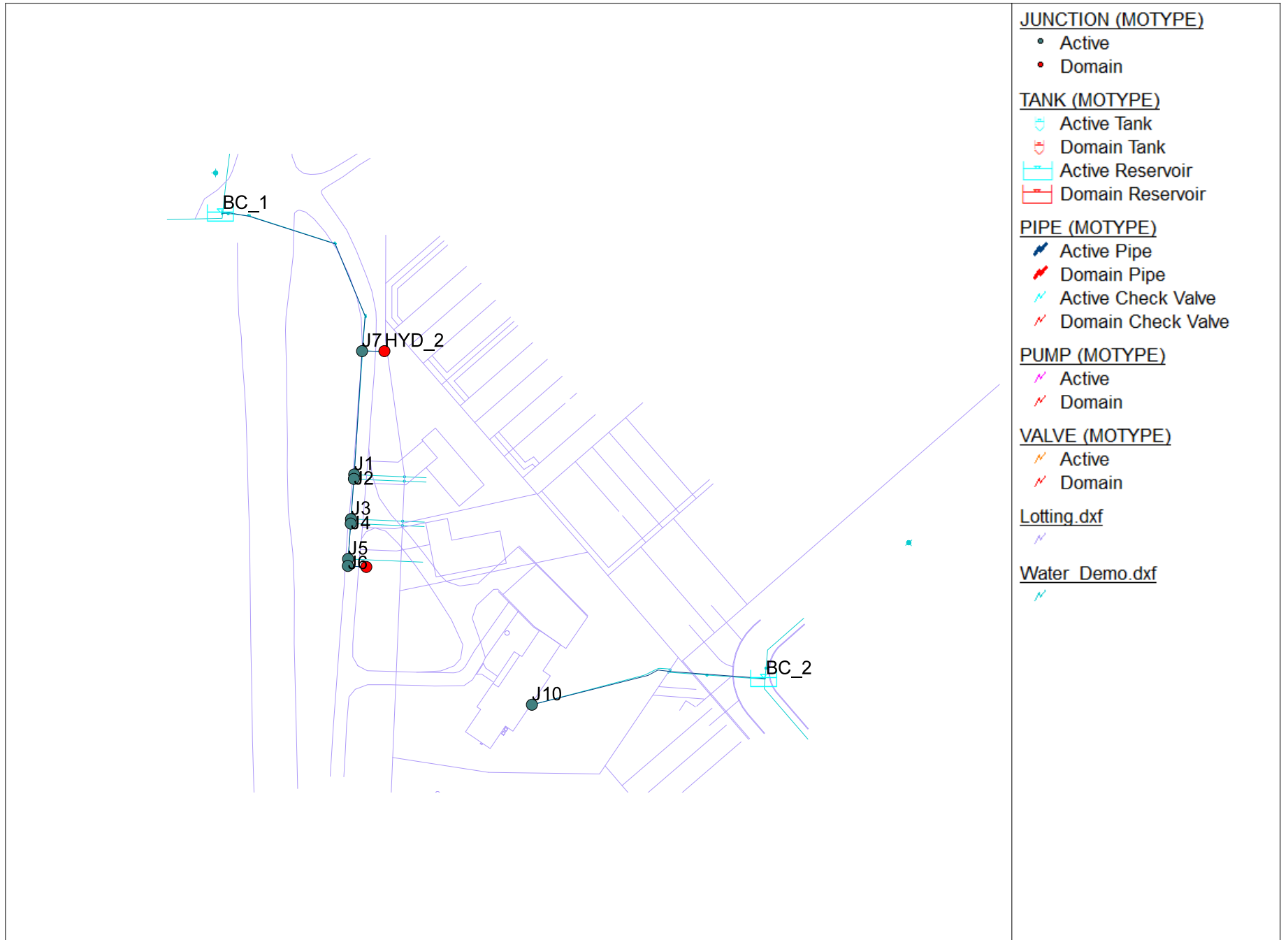
Water Model Outputs

FUS Calculations

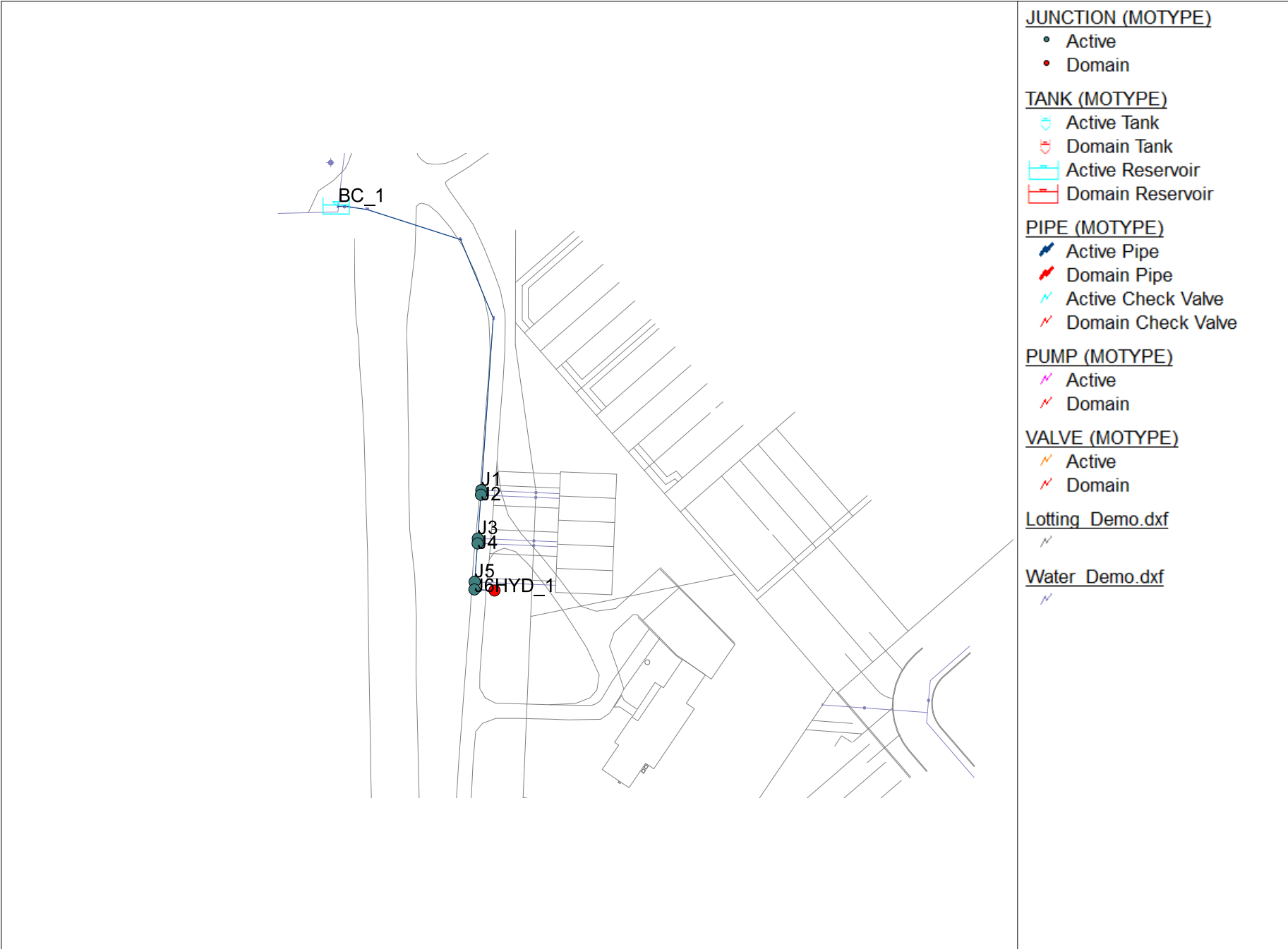
Fire Flow Reports

Hydrant Coverage Plans

22008 Water Model



1835 Stittsville Main Street

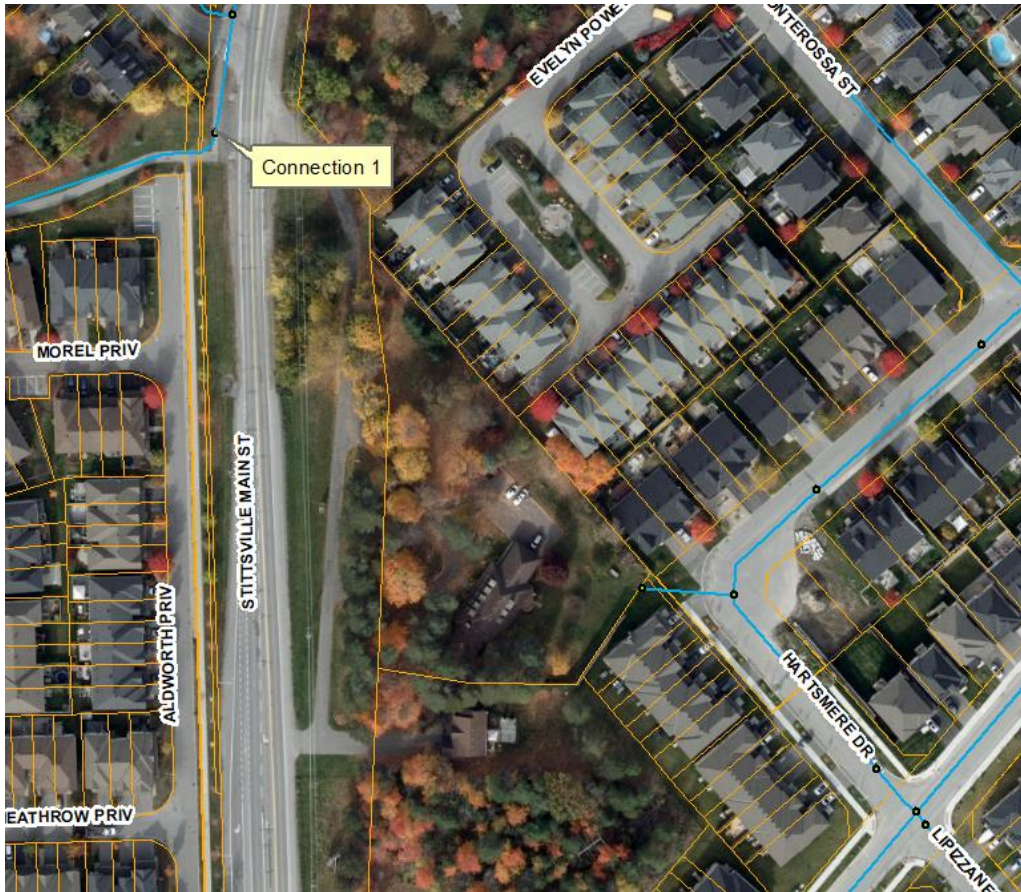


Boundary Conditions 1835 Stittsville Main Street

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	2	0.03
Maximum Daily Demand	19	0.31
Peak Hour	269	4.49
Fire Flow Demand #1	9,000	150.00

Location



Results

Connection 1 – Stittsville Main Street

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	160.2	54.5
Peak Hour	155.2	47.4
Max Day plus Fire 1	144.3	32.0

Ground Elevation = 121.9 m

Disclaimer

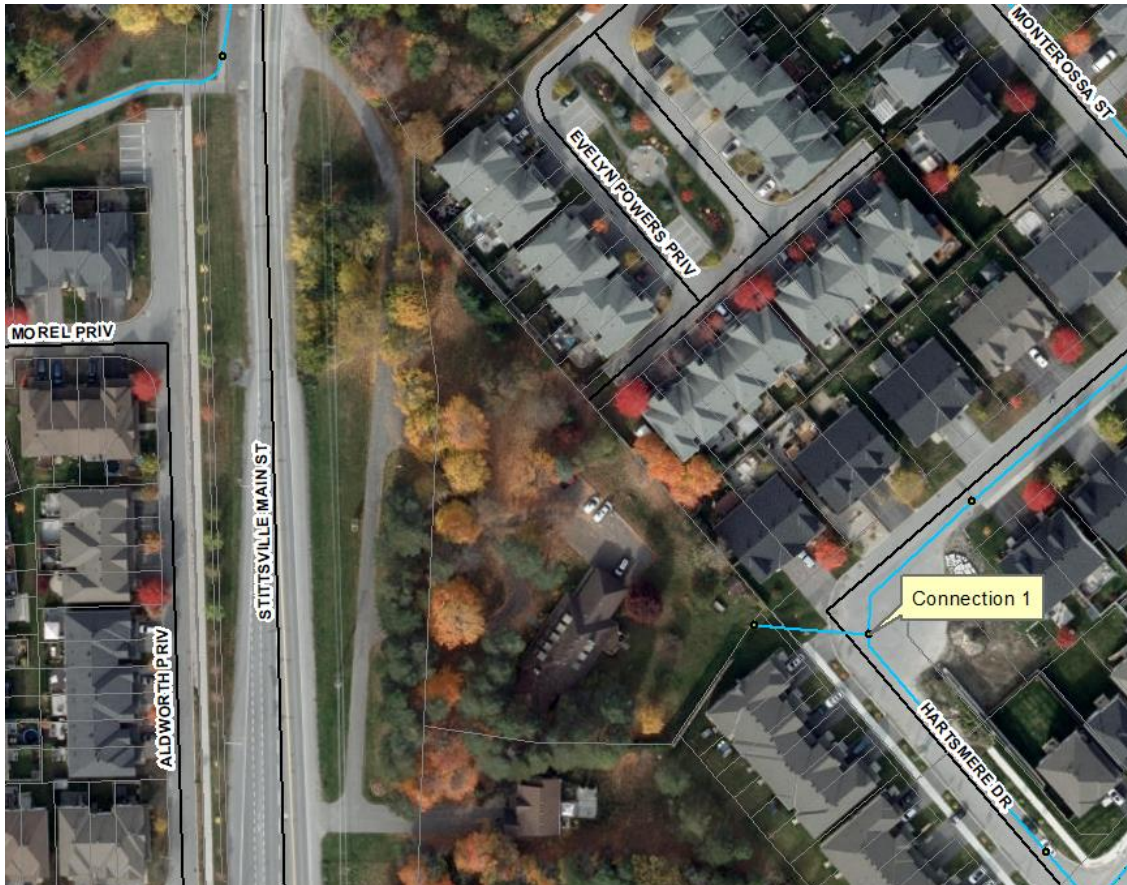
The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Boundary Conditions 1835 Stittsville Main Street

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	2	0.03
Maximum Daily Demand	19	0.31
Peak Hour	269	4.49
Fire Flow Demand #1	9,000	150.00

Location



Results

Connection 1 – Hartsmere Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	160.2	58.5
Peak Hour	155.2	51.3
Max Day plus Fire 1	147.7	40.7

Ground Elevation = 119.1 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

WATERMAIN DESIGN SHEET

1835 Stittsville Main Street, Ottawa
Project No. 22008

JUNCTION NODE	RESIDENTIAL POPULATION				COMMERCIAL AREA (ha)	INSTITUTIONAL AREA (ha)	AVG. DAY DEMAND (L/s)				MAX. DAILY DEMAND (L/s)				MAX. HOURLY DEMAND (L/s)			
	UNIT COUNT			TOTAL POPULATION			RES.	COMM.	INST.	TOTAL	RES.	COMM.	INST.	TOTAL	RES.	COMM.	INST.	TOTAL
	SINGLE FAMILY	TOWNHOUSE	APARTMENTS															
Stittsville Main	2			6.8			0.022			0.022	0.209			0.209	2.994			2.994
Hartsmere	1			3.4			0.011			0.011	0.105			0.105	1.497			1.497
Total	3			10.2			0.033			0.033	0.314			0.314	4.491			4.491

Notes:

1. Residential peaking factors as per Table 3-3 of the MOE Design Guidelines for Drinking Water Systems (2008).

Population Density

Single Family = 3.4 cap/unit
Townhouses = 2.7 cap/unit
Apartments = 1.8 cap/unit

Avg. Day Demand:

Residential 280 L/cap/day
Commercial 28000 L/ha/day
Institutional 28000 L/ha/day

Max. Daily Demand:

Residential 9.5 x Avg. Day
Commercial 1.5 x Avg. Day
Institutional 1.5 x Avg. Day

Max. Hourly Demand:

Residential 14.3 x Max. Day
Commercial 1.8 x Max. Day
Institutional 1.8 x Max. Day

Table 3-3: Peaking Factors for Drinking-Water Systems Serving Fewer than 500 People

DWELLING UNITS SERVICED	EQUIVALENT POPULATION	NIGHT MINIMUM HOUR FACTOR	MAXIMUM DAY FACTOR	PEAK HOUR FACTOR
10	30	0.1	9.5	14.3
50	150	0.1	4.9	7.4
100	300	0.2	3.6	5.4
150	450	0.3	3.0	4.5
167	500	0.4	2.9	4.3

WATERMAIN DESIGN SHEET

1835 Stittsville Main Street, Ottawa
Project No. 22008

JUNCTION NODE	RESIDENTIAL POPULATION				COMMERCIAL AREA (ha)	INSTITUTIONAL AREA (ha)	AVG. DAY DEMAND (L/s)				MAX. DAILY DEMAND (L/s)				MAX. HOURLY DEMAND (L/s)			
	UNIT COUNT			TOTAL POPULATION			RES.	COMM.	INST.	TOTAL	RES.	COMM.	INST.	TOTAL	RES.	COMM.	INST.	TOTAL
	SINGLE FAMILY	TOWNHOUSE	APARTMENTS															
Stittsville Main		5		13.5			0.044			0.044	0.416			0.416	5.943			5.943
Hartsmere	1			3.4			0.011			0.011	0.105			0.105	1.497			1.497
Total	1	5		16.9			0.055			0.055	0.520			0.520	7.440			7.440

Notes:

1. Residential peaking factors as per Table 3-3 of the MOE Design Guidelines for Drinking Water Systems (2008).

Population Density

Single Family = 3.4 cap/unit
Townhouses = 2.7 cap/unit
Apartments = 1.8 cap/unit

Avg. Day Demand:

Residential 280 L/cap/day
Commercial 28000 L/ha/day
Institutional 28000 L/ha/day

Max. Daily Demand:

Residential 9.5 x Avg. Day
Commercial 1.5 x Avg. Day
Institutional 1.5 x Avg. Day

Max. Hourly Demand:

Residential 14.3 x Max. Day
Commercial 1.8 x Max. Day
Institutional 1.8 x Max. Day

Table 3-3: Peaking Factors for Drinking Water Systems Serving Fewer than 500 People

DWELLING UNITS SERVICED	EQUIVALENT POPULATION	NIGHT MINIMUM HOUR FACTOR	MAXIMUM DAY FACTOR	PEAK HOUR FACTOR
10	30	0.1	9.5	14.3
50	150	0.1	4.9	7.4
100	300	0.2	3.6	5.4
150	450	0.3	3.0	4.5
167	500	0.4	2.9	4.3

1835 Stittsville Main Street - Maximum Pressure - TH

	ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
1	HYD_1	0.00	122.60	160.20	53.45
2	J1	0.01	123.50	160.20	52.17
3	J2	0.01	123.50	160.20	52.17
4	J3	0.01	123.50	160.20	52.17
5	J4	0.01	123.50	160.20	52.17
6	J5	0.01	123.50	160.20	52.17
7	J6	0.01	123.50	160.20	52.17

1835 Stittsville Main Street - Maximum Pressure - SF

	ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
1	HYD_1	0.00	122.60	160.20	53.45
2	J1	0.00	123.50	160.20	52.17
3	J2	0.01	123.50	160.20	52.17
4	J3	0.00	123.50	160.20	52.17
5	J4	0.01	123.50	160.20	52.17
6	J5	0.00	123.50	160.20	52.17
7	J6	0.00	123.50	160.20	52.17

1835 Stittsville Main Street - Hartsmere Connection - Max Pressure

		ID	Demand (Lpm)	Elevation (m)	Head (m)	Pressure (psi)
1	<input type="checkbox"/>	HYD_1	0.00	122.60	160.20	53.45
2	<input type="checkbox"/>	HYD_2	0.00	123.50	160.20	52.17
3	<input type="checkbox"/>	J1	0.00	123.50	160.20	52.17
4	<input type="checkbox"/>	J10	0.60	122.04	160.20	54.25
5	<input type="checkbox"/>	J2	0.60	123.50	160.20	52.17
6	<input type="checkbox"/>	J3	0.00	123.50	160.20	52.17
7	<input type="checkbox"/>	J4	0.60	123.50	160.20	52.17
8	<input type="checkbox"/>	J5	0.00	123.50	160.20	52.17
9	<input type="checkbox"/>	J6	0.00	123.50	160.20	52.17
10	<input type="checkbox"/>	J7	0.00	123.50	160.20	52.17

	ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
1	HYD_1	0.00	122.60	155.20	46.34
2	J1	0.00	123.50	155.20	45.06
3	J2	1.50	123.50	155.20	45.06
4	J3	0.00	123.50	155.20	45.06
5	J4	1.50	123.50	155.20	45.06
6	J5	0.00	123.50	155.20	45.06
7	J6	0.00	123.50	155.20	45.06

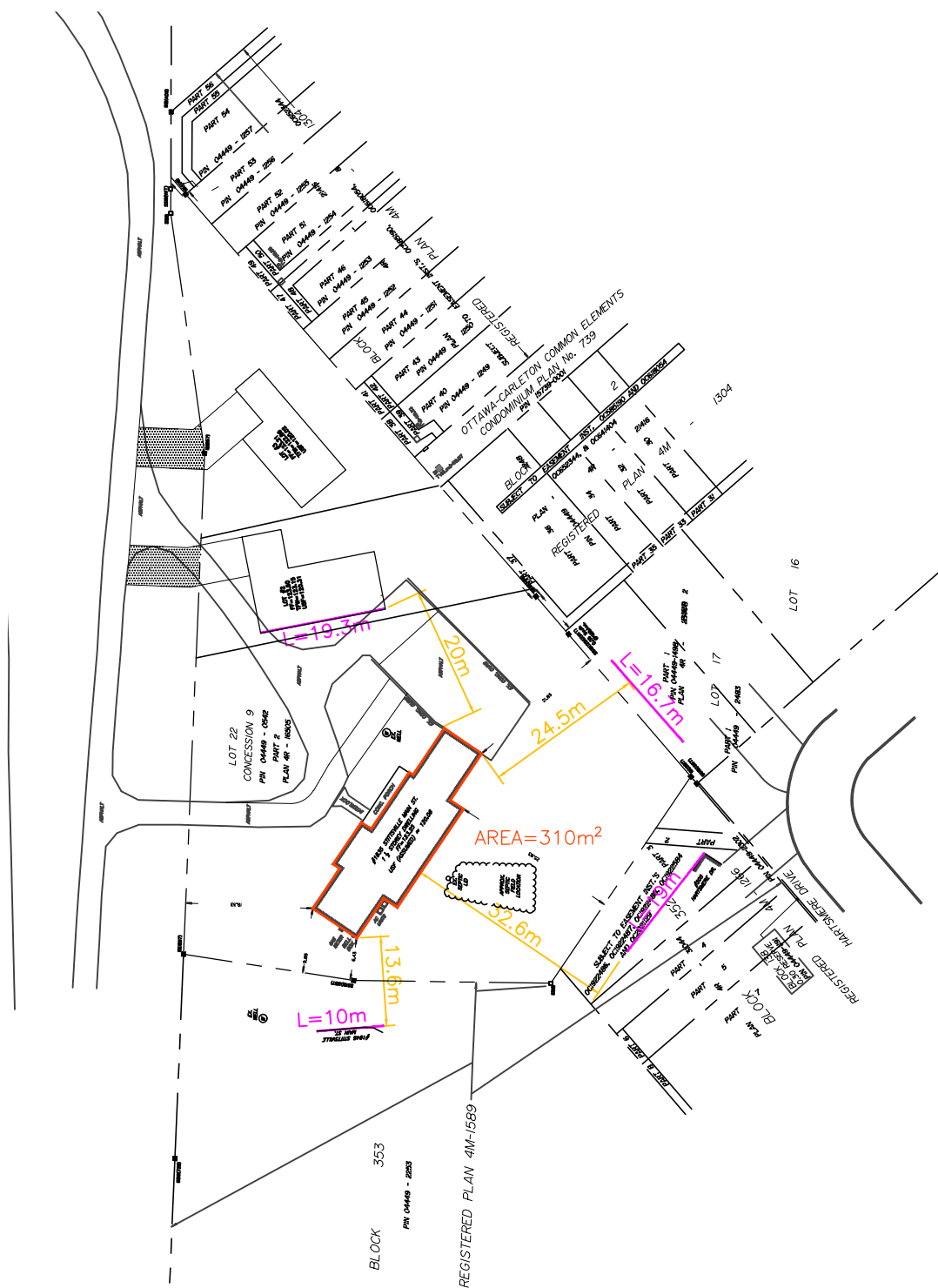
	ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
1	HYD_1	0.00	122.60	155.19	46.33
2	J1	1.19	123.50	155.19	45.05
3	J2	1.19	123.50	155.19	45.05
4	J3	1.19	123.50	155.19	45.05
5	J4	1.19	123.50	155.19	45.05
6	J5	1.19	123.50	155.19	45.05
7	J6	0.00	123.50	155.19	45.05

1835 Stittsville Main Street - Hartsmere Connection - Peak Hour

		ID	Demand (Lpm)	Elevation (m)	Head (m)	Pressure (psi)
1	<input type="checkbox"/>	HYD_1	0.00	122.60	155.20	46.34
2	<input type="checkbox"/>	HYD_2	0.00	123.50	155.20	45.06
3	<input type="checkbox"/>	J1	0.00	123.50	155.20	45.06
4	<input type="checkbox"/>	J10	90.00	122.04	153.62	44.89
5	<input type="checkbox"/>	J2	90.00	123.50	155.20	45.06
6	<input type="checkbox"/>	J3	0.00	123.50	155.20	45.06
7	<input type="checkbox"/>	J4	90.00	123.50	155.20	45.06
8	<input type="checkbox"/>	J5	0.00	123.50	155.20	45.06
9	<input type="checkbox"/>	J6	0.00	123.50	155.20	45.06
10	<input type="checkbox"/>	J7	0.00	123.50	155.20	45.06

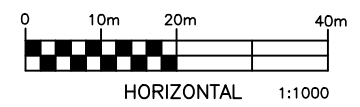
1835 Stittsville Main Street - Pipe Report

	PIPE: ID (Char)	PIPEHYD: DIAMETER (Num)	PIPEHYD: ROUGHNESS (Num)	LINK: FROM (Char)	LINK: TO (Char)
1	P1	254.00	110.00	BC_1	J1
2	P2	254.00	110.00	J1	J2
3	P3	254.00	110.00	J2	J3
4	P4	254.00	110.00	J3	J4
5	P5	254.00	110.00	J4	J5
6	P6	254.00	110.00	J5	J6
7	P7	163.00	110.00	J6	HYD_1



LEGEND

- PROPERTY BOUNDARY
- AREA BOUNDARY USED IN STEP B
- EXPOSURE LENGTH USED IN STEP F



Robinson

Land Development

<small>scale</small>	1:1000	<small>project no.</small>	22008
<small>date</small>	01/11/22	1835 STITTSVILLE MAIN STREET	
<small>drawn by</small>	BLM	FUS SKETCH – LOT 1	
			FUS1

Project Name: 1835 Stittsville Main Street
Project Location: 1835 Stittsville Main Street
Project No: 22008
Date: Nov. 01-22

Building Type: Single-Family
Building Being Considered: Lot 1 (Existing Dwelling)



Calculations for Total Required Fire Flow

Step	Parameter			Value	
A	Type of Construction	Options	C	Wood Frame (Type V) 1.5	
		Wood Frame (Type V)	1.5		
		Ordinary Construction (Type III)	1.0		
		Non-Combustible Construction (Type II)	0.8		
		Fire Resistive Construction (Type I)	0.6		
B	Ground Floor Area			310 m ²	
	Second Floor Area			155 m ²	
	Total Effective Floor Area			465.0 m²	
C	Fire Flow			7,000 L/min	
D	Occupancy Class	Options	Charge	Limited Combustible -0.15	
		Non-combustible	-0.25		
		Limited Combustible	-0.15		
		Combustible	0.00		
		Free burning	0.15		
		Rapid Burning	0.25		
	Occupancy Adjustment			-1050 L/min	
Fire Flow			5,950 L/min		
E	Sprinkler Protection	Options	Charge	None 0.00	
		Automatic Sprinkler Protection	-0.30		
		None	0.00		
		Water Supply is Standard for System and Hose Lines	-0.10		
		Full Supervision of the Sprinkler System	-0.10		
Sprinkler Reduction			0 L/min		
F	Exposures				
	West Side				
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
	Exposed Wall Length			10 m	
	Exposed Wall No. of Storeys			2	
	Length-Height Factor of Exposed Wall			20 m.storeys	
	Construction Type of Exposed Wall	Options	Wood Frame		
		Wood Frame			
		Ordinary with Unprotected Openings			
		Ordinary without Unprotected Openings			
		Noncombustible or Fire Resistive with Unprotected Openings			
		Noncombustible or Fire Resistive without Unprotected Openings			
	Separation Distance			**+3m See Note 3** 16.6 m	
	West Side Exposure Charge				0.10
	North Side				
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
	Exposed Wall Length			19.3 m	
	Exposed Wall No. of Storeys			2	
	Length-Height Factor of Exposed Wall			38.6 m.storeys	
	Construction Type of Exposed Wall	Options	Wood Frame		
		Wood Frame			
		Ordinary with Unprotected Openings			
		Ordinary without Unprotected Openings			
		Noncombustible or Fire Resistive with Unprotected Openings			
		Noncombustible or Fire Resistive without Unprotected Openings			
	Separation Distance			**+3m See Note 3** 23 m	
	North Side Exposure Charge				0.02
	East Side				
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No		
Exposed Building Fully Protected with Automatic Sprinkler Systems			No		
Exposed Wall Length			16.7 m		
Exposed Wall No. of Storeys			2		
Length-Height Factor of Exposed Wall			33.4 m.storeys		
Construction Type of Exposed Wall	Options	Wood Frame			
	Wood Frame				
	Ordinary with Unprotected Openings				
	Ordinary without Unprotected Openings				
	Noncombustible or Fire Resistive with Unprotected Openings				

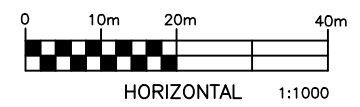
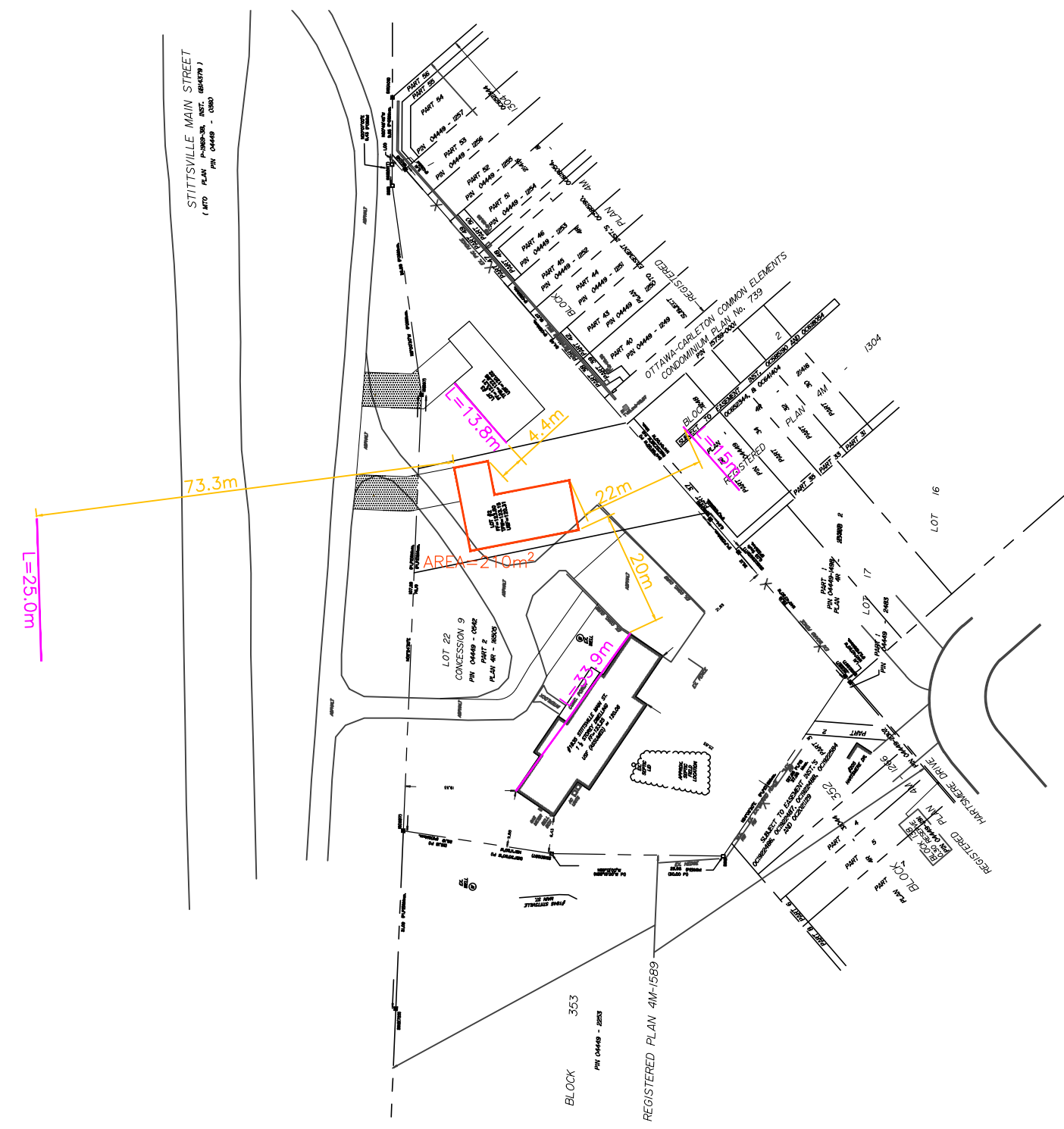
	Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance	**+3m See Note 3**	27.5 m
	East Side Exposure Charge		0.02
	South Side		
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems		No
	Exposed Building Fully Protected with Automatic Sprinkler Systems		No
	Exposed Wall Length		19 m
	Exposed Wall No. of Storeys		2
	Length-Height Factor of Exposed Wall		38 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame
		Wood Frame	
		Ordinary with Unprotected Openings	
		Ordinary without Unprotected Openings	
		Noncombustible or Fire Resistive with Unprotected Openings	
	Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance	**Separation >30m; No exposure**	35.6 m
	South Side Exposure Charge		0.00
	Total Exposure Charge		0.14 < 0.75
	Increase for Exposures		833 L/min
G	Total Required Fire Flow		7,000 L/min

Notes:

1. Fire flow calculations have been prepared in accordance with Fire Underwriters Survey (v. 2020)
2. Floor areas used in Step B are conservative values as they include the exterior footprint and garages.
3. Where buildings are at a diagonal to each other, the shortest separation distance is increased by 3 metres and used as the exposure distance (Ref. FUS v.2020 pg.30).
4. Step B, second storey floor area assumed to be half of building footprint area (noted as 1.5 storey dwelling on topographic survey)



- LEGEND**
- PROPERTY BOUNDARY
 - AREA BOUNDARY USED IN STEP B
 - EXPOSURE LENGTH USED IN STEP F



<h1>Robinson</h1> <h2>Land Development</h2>				
scale	1:1000	1835 STITTSVILLE MAIN STREET	project no.	22008
date	01/11/22		FUS SKETCH – LOT 2	
drawn by	BLM			

Project Name: 1835 Stittsville Main Street Project Location: 1835 Stittsville Main Street Project No: 22008 Date: Nov. 01-22 Building Type: Single-Family Building Being Considered: Lot 2 (Proposed Dwelling)	<h1 style="margin: 0;">Robinson</h1> <h2 style="margin: 0;">Land Development</h2>
---	---

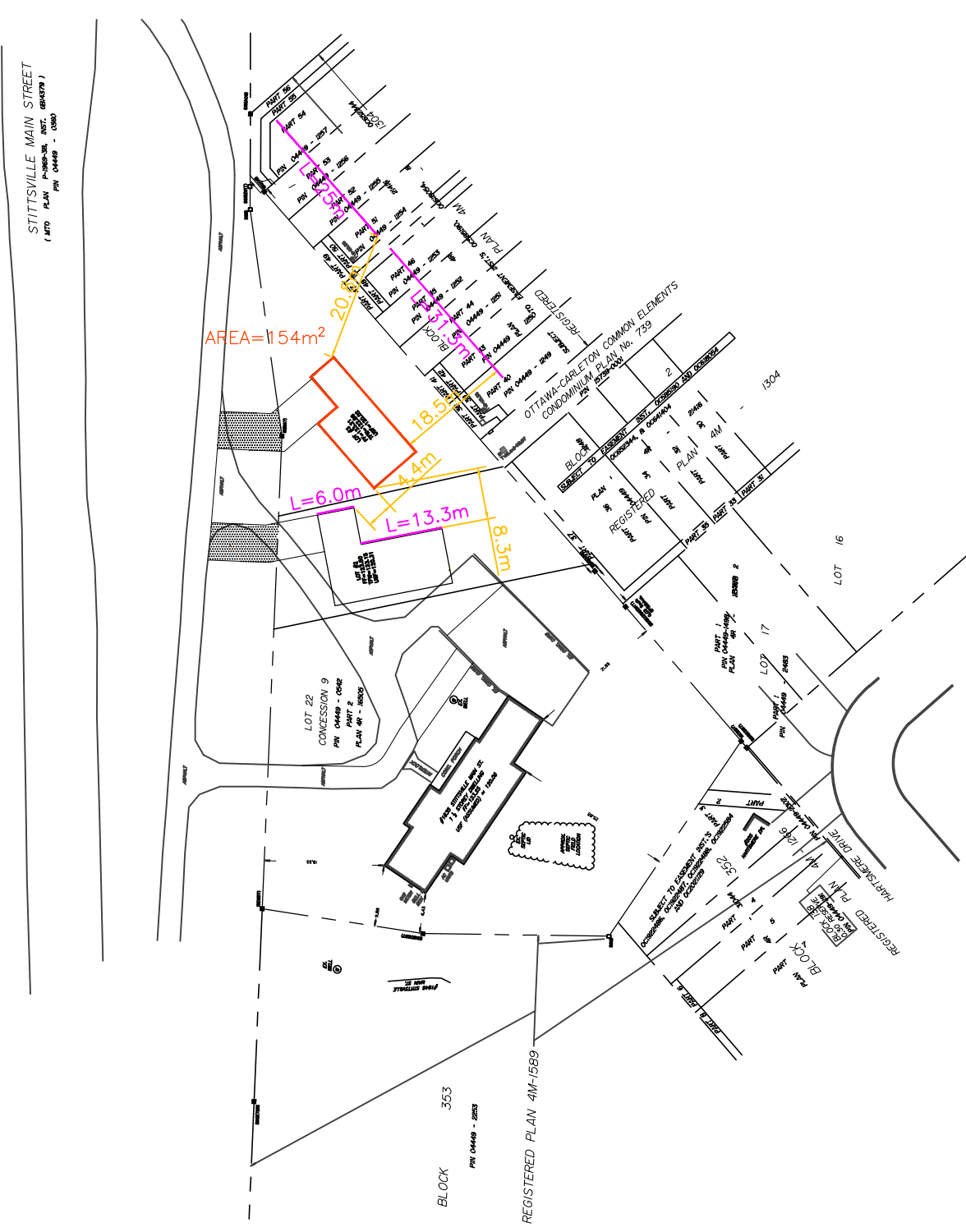
Calculations for Total Required Fire Flow

Step	Parameter			Value
A	Type of Construction	Options	C	Wood Frame (Type V) 1.5
		Wood Frame (Type V)	1.5	
		Ordinary Construction (Type III)	1.0	
		Non-Combustible Construction (Type II)	0.8	
		Fire Resistive Construction (Type I)	0.6	
B	Ground Floor Area			210 m ²
	Second Floor Area			210 m ²
	Total Effective Floor Area			420.0 m²
C	Fire Flow			7,000 L/min
D	Occupancy Class	Options	Charge	Limited Combustible -0.15
		Non-combustible	-0.25	
		Limited Combustible	-0.15	
		Combustible	0.00	
		Free burning	0.15	
		Rapid Burning	0.25	
	Occupancy Adjustment			-1050 L/min
Fire Flow			5,950 L/min	
E	Sprinkler Protection	Options	Charge	None 0.00 No 0.00 No 0.00
		Automatic Sprinkler Protection	-0.30	
		None	0.00	
		Water Supply is Standard for System and Hose Lines	-0.10	
		Full Supervision of the Sprinkler System	-0.10	
Sprinkler Reduction			0 L/min	
F	Exposures			
	West Side			
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Wall Length			25 m
	Exposed Wall No. of Storeys			2
	Length-Height Factor of Exposed Wall			50 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame	73.3 m
		Wood Frame		
		Ordinary with Unprotected Openings		
		Ordinary without Unprotected Openings		
		Noncombustible or Fire Resistive with Unprotected Openings		
		Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance			**Separation >30m; No exposure**
	West Side Exposure Charge			0.00
	North Side			
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Wall Length			13.8 m
	Exposed Wall No. of Storeys			2
	Length-Height Factor of Exposed Wall			27.6 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame	7.4 m
		Wood Frame		
		Ordinary with Unprotected Openings		
		Ordinary without Unprotected Openings		
		Noncombustible or Fire Resistive with Unprotected Openings		
		Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance			**+3m See Note 3**
	North Side Exposure Charge			0.16
	East Side			
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
Exposed Wall Length			15 m	
Exposed Wall No. of Storeys			2	
Length-Height Factor of Exposed Wall			30 m.storeys	
Construction Type of Exposed Wall	Options	Wood Frame		
	Wood Frame			
	Ordinary with Unprotected Openings			
	Ordinary without Unprotected Openings			
	Noncombustible or Fire Resistive with Unprotected Openings			

	Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance	**+3m See Note 3**	25 m
East Side Exposure Charge			0.02
South Side			
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
Exposed Building Fully Protected with Automatic Sprinkler Systems			No
Exposed Wall Length			33.9 m
Exposed Wall No. of Storeys			2
Length-Height Factor of Exposed Wall			67.8 m.storeys
Construction Type of Exposed Wall	Options		Wood Frame
	Wood Frame		
	Ordinary with Unprotected Openings		
	Ordinary without Unprotected Openings		
	Noncombustible or Fire Resistive with Unprotected Openings		
Noncombustible or Fire Resistive without Unprotected Openings			
	Separation Distance	**+3m See Note 3**	23 m
South Side Exposure Charge			0.06
Total Exposure Charge			0.24 < 0.75
Increase for Exposures			1428 L/min
G	Total Required Fire Flow		7,000 L/min

Notes:

1. Fire flow calculations have been prepared in accordance with Fire Underwriters Survey (v. 2020)
2. Floor areas used in Step B are conservative values as they include the exterior footprint and garages.
3. Where buildings are at a diagonal to each other, the shortest separation distance is increased by 3 metres and used as the exposure distance (Ref. FUS v.2020 pg.30).

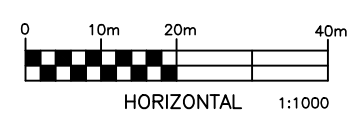


LEGEND

— — — — — PROPERTY BOUNDARY

— — — — — AREA BOUNDARY USED IN STEP B

— — — — — EXPOSURE LENGTH USED IN STEP F



Robinson Land Development		
scale 1:1000	1835 STITTSVILLE MAIN STREET	project no. 22008
date 01/11/22		FUS SKETCH – LOT 3
drawn by BLM		FUS3

Project Name: 1835 Stittsville Main Street
Project Location: 1835 Stittsville Main Street
Project No: 22008
Date: Nov. 01-22

Building Type: Single-Family
Building Being Considered: Lot 3 (Proposed Dwelling)



Calculations for Total Required Fire Flow

Step	Parameter			Value
A	Type of Construction	Options	C	Wood Frame (Type V) 1.5
		Wood Frame (Type V)	1.5	
		Ordinary Construction (Type III)	1.0	
		Non-Combustible Construction (Type II)	0.8	
		Fire Resistive Construction (Type I)	0.6	
B	Ground Floor Area			154 m ²
	Second Floor Area			154 m ²
	Total Effective Floor Area			308.0 m²
C	Fire Flow			6,000 L/min
D	Occupancy Class	Options	Charge	Limited Combustible -0.15
		Non-combustible	-0.25	
		Limited Combustible	-0.15	
		Combustible	0.00	
		Free burning	0.15	
		Rapid Burning	0.25	
	Occupancy Adjustment			-900 L/min
Fire Flow			5,100 L/min	
E	Sprinkler Protection	Options	Charge	None 0.00
		Automatic Sprinkler Protection	-0.30	
		None	0.00	
		Water Supply is Standard for System and Hose Lines	-0.10	
		Full Supervision of the Sprinkler System	-0.10	
Sprinkler Reduction			0 L/min	
F	Exposures			
	West Side			
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Wall Length			6 m
	Exposed Wall No. of Storeys			2
	Length-Height Factor of Exposed Wall			12 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame	
		Wood Frame		
		Ordinary with Unprotected Openings		
		Ordinary without Unprotected Openings		
		Noncombustible or Fire Resistive with Unprotected Openings		
		Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance			**+3m See Note 3** 7.4 m
	West Side Exposure Charge			0.15
	North Side			
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Building Fully Protected with Automatic Sprinkler Systems			No
	Exposed Wall Length			25 m
	Exposed Wall No. of Storeys			2
	Length-Height Factor of Exposed Wall			50 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame	
		Wood Frame		
		Ordinary with Unprotected Openings		
		Ordinary without Unprotected Openings		
		Noncombustible or Fire Resistive with Unprotected Openings		
		Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance			**+3m See Note 3** 23.8 m
	North Side Exposure Charge			0.04
	East Side			
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
Exposed Building Fully Protected with Automatic Sprinkler Systems			No	
Exposed Wall Length			31.3 m	
Exposed Wall No. of Storeys			2	
Length-Height Factor of Exposed Wall			62.6 m.storeys	
Construction Type of Exposed Wall	Options	Wood Frame		
	Wood Frame			
	Ordinary with Unprotected Openings			
	Ordinary without Unprotected Openings			
	Noncombustible or Fire Resistive with Unprotected Openings			




	Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance	18.5	m
	East Side Exposure Charge	0.13	
	South Side		
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems	No	
	Exposed Building Fully Protected with Automatic Sprinkler Systems	No	
	Exposed Wall Length	13.3	m
	Exposed Wall No. of Storeys	2	
	Length-Height Factor of Exposed Wall	26.6	m.storeys
	Construction Type of Exposed Wall	Options	
		Wood Frame	
		Ordinary with Unprotected Openings	
		Ordinary without Unprotected Openings	
		Noncombustible or Fire Resistive with Unprotected Openings	
		Wood Frame	
	Separation Distance	**+3m See Note 3**	11.3 m
	South Side Exposure Charge	0.11	
	Total Exposure Charge	0.43	< 0.75
	Increase for Exposures	2193	L/min
G	Total Required Fire Flow	7,000	L/min

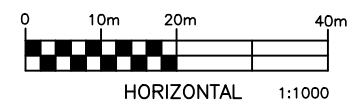
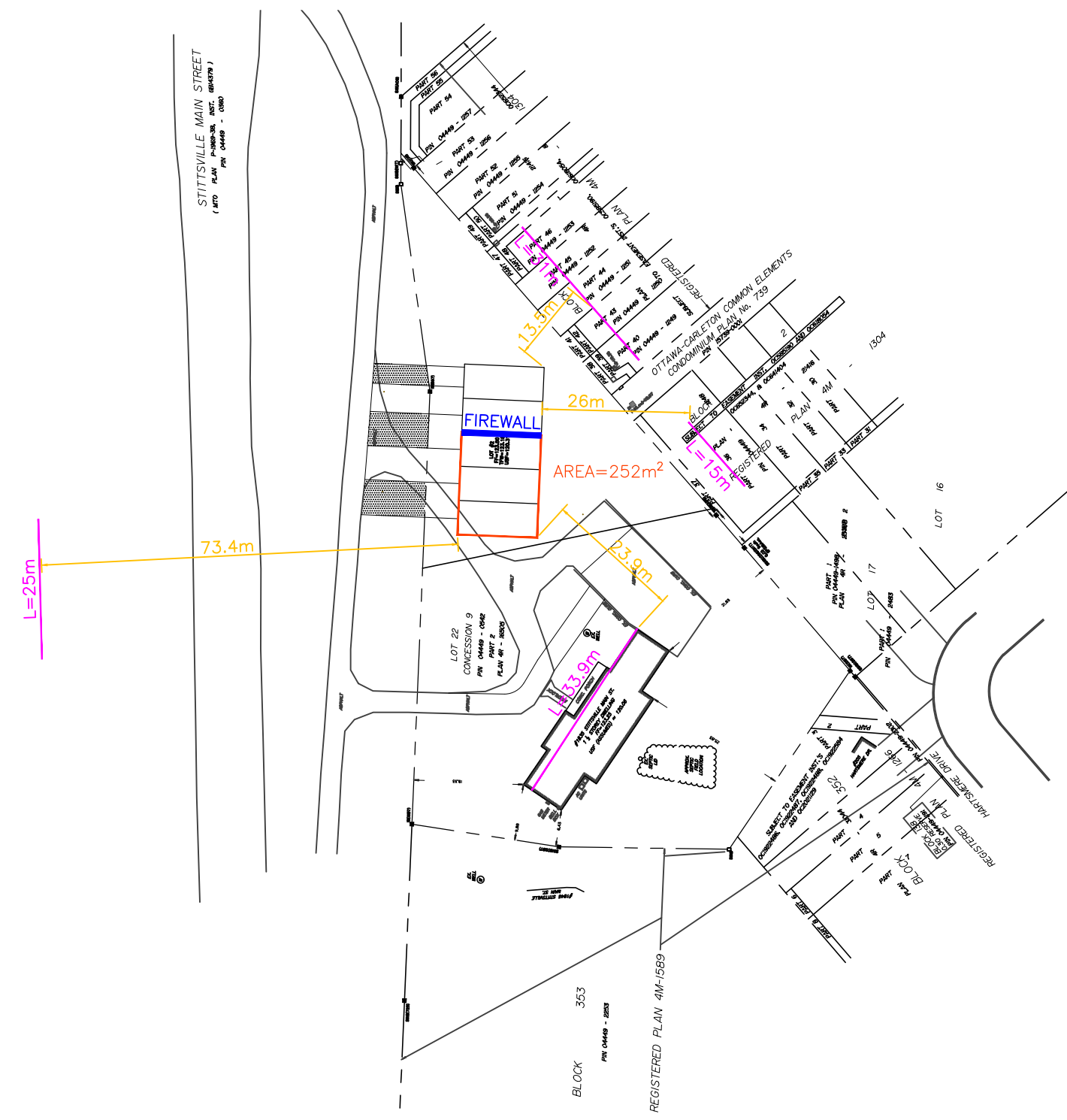
Notes:

1. Fire flow calculations have been prepared in accordance with Fire Underwriters Survey (v. 2020)
2. Floor areas used in Step B are conservative values as they include the exterior footprint and garages.
3. Where buildings are at a diagonal to each other, the shortest separation distance is increased by 3 metres and used as the exposure distance (Ref. FUS v.2020 pg.30).



LEGEND

-  PROPERTY BOUNDARY
-  AREA BOUNDARY USED IN STEP B
-  EXPOSURE LENGTH USED IN STEP F



Robinson

Land Development

scale	1:1000	1835 STITTSVILLE MAIN STREET	project no.	22008
date	01/11/22		FUS SKETCH – TOWNHOUSE	
drawn by	BLM			

Project Name: 1835 Stittsville Main Street Project Location: 1835 Stittsville Main Street Project No: 22008 Date: Nov. 01-22 Building Type: 5-Unit Townhouse (with firewall) Building Being Considered: Lot 2 (Proposed Dwelling)	<h1 style="margin: 0;">Robinson</h1> <h2 style="margin: 0;">Land Development</h2>
--	---

Calculations for Total Required Fire Flow

Step	Parameter				Value
A	Type of Construction	Options	C	Wood Frame (Type V)	1.5
		Wood Frame (Type V)	1.5		
		Ordinary Construction (Type III)	1.0		
		Non-Combustible Construction (Type II)	0.8		
		Fire Resistive Construction (Type I)	0.6		
B	Ground Floor Area				252 m ²
	Second Floor Area				252 m ²
	Total Effective Floor Area				504.0 m²
C	Fire Flow				7,000 L/min
D	Occupancy Class	Options	Charge	Limited Combustible	-0.15
		Non-combustible	-0.25		
		Limited Combustible	-0.15		
		Combustible	0.00		
		Free burning	0.15		
		Rapid Burning	0.25		
	Occupancy Adjustment				-1050 L/min
Fire Flow				5,950 L/min	
E	Sprinkler Protection	Options	Charge	None	0.00
		Automatic Sprinkler Protection	-0.30		
		None	0.00		
		Water Supply is Standard for System and Hose Lines	-0.10		
		Full Supervision of the Sprinkler System	-0.10		
Sprinkler Reduction				0 L/min	
F	Exposures				
	West Side				
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems				No
	Exposed Building Fully Protected with Automatic Sprinkler Systems				No
	Exposed Wall Length				25 m
	Exposed Wall No. of Storeys				2
	Length-Height Factor of Exposed Wall				50 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame		
		Wood Frame			
		Ordinary with Unprotected Openings			
		Ordinary without Unprotected Openings			
		Noncombustible or Fire Resistive with Unprotected Openings			
		Noncombustible or Fire Resistive without Unprotected Openings			
	Separation Distance				73.4 m
	West Side Exposure Charge				0.00
	North Side				
	Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems				No
	Exposed Building Fully Protected with Automatic Sprinkler Systems				No
	Exposed Wall Length				31 m
	Exposed Wall No. of Storeys				2
	Length-Height Factor of Exposed Wall				62 m.storeys
	Construction Type of Exposed Wall	Options	Wood Frame		
		Wood Frame			
		Ordinary with Unprotected Openings			
		Ordinary without Unprotected Openings			
		Noncombustible or Fire Resistive with Unprotected Openings			
		Noncombustible or Fire Resistive without Unprotected Openings			
	Separation Distance				16.5 m
	North Side Exposure Charge				0.13
	East Side				
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems				No	
Exposed Building Fully Protected with Automatic Sprinkler Systems				No	
Exposed Wall Length				15 m	
Exposed Wall No. of Storeys				2	
Length-Height Factor of Exposed Wall				30 m.storeys	
Construction Type of Exposed Wall	Options	Wood Frame			
	Wood Frame				
	Ordinary with Unprotected Openings				
	Ordinary without Unprotected Openings				
	Noncombustible or Fire Resistive with Unprotected Openings				

	Noncombustible or Fire Resistive without Unprotected Openings		
	Separation Distance	**+3m See Note 3**	29 m
East Side Exposure Charge			0.02
South Side			
Subject Building and Exposed Building Fully Protected with Automatic Sprinkler Systems			No
Exposed Building Fully Protected with Automatic Sprinkler Systems			No
Exposed Wall Length			33.9 m
Exposed Wall No. of Storeys			2
Length-Height Factor of Exposed Wall			67.8 m.storeys
Construction Type of Exposed Wall	Options		Wood Frame
	Wood Frame		
	Ordinary with Unprotected Openings		
	Ordinary without Unprotected Openings		
	Noncombustible or Fire Resistive with Unprotected Openings		
Noncombustible or Fire Resistive without Unprotected Openings			
Separation Distance		**+3m See Note 3**	26.9 m
South Side Exposure Charge			0.06
Total Exposure Charge			0.21 < 0.75
Increase for Exposures			1249.5 L/min
G	Total Required Fire Flow		7,000 L/min

Notes:

1. Fire flow calculations have been prepared in accordance with Fire Underwriters Survey (v. 2020)
2. Floor areas used in Step B are conservative values as they include the exterior footprint and garages.
3. Where buildings are at a diagonal to each other, the shortest separation distance is increased by 3 metres and used as the exposure distance (Ref. FUS v.2020 pg.30).
4. Step B, floor area assumes implementation of firewall to reduce effective floor area to 3-units.

1835 Stittsville Main Street - Fireflow Report

	ID	Total Demand (Lpm)	Critical Node ID	Critical Node Pressure (psi)	Critical Node Head (m)	Available Flow Pressure (psi)	Available Flow at Hydrant (Lpm)
1	<input type="checkbox"/> HYD_1	7,001.95	J10	-173.51	-0.01	20.00	9,549.10
2	<input type="checkbox"/> HYD_2	7,001.95	J10	-173.51	-0.01	20.00	9,362.05

1835 Stittsville Main Street - Max Day + FF - TH

	ID	Static Demand (Lpm)	Static Pressure (psi)	Static Head (m)	Fire-Flow Demand (Lpm)	Residual Pressure (psi)	Available Flow at Hydrant (Lpm)
1	<input type="checkbox"/> HYD_1	0.00	30.85	144.30	8,999.94	21.10	9,534.45

1835 Stittsville Main Street - Max Day + FF - TH

	ID	Available Flow Pressure (psi)
1 <input type="checkbox"/>	HYD_1	20.00



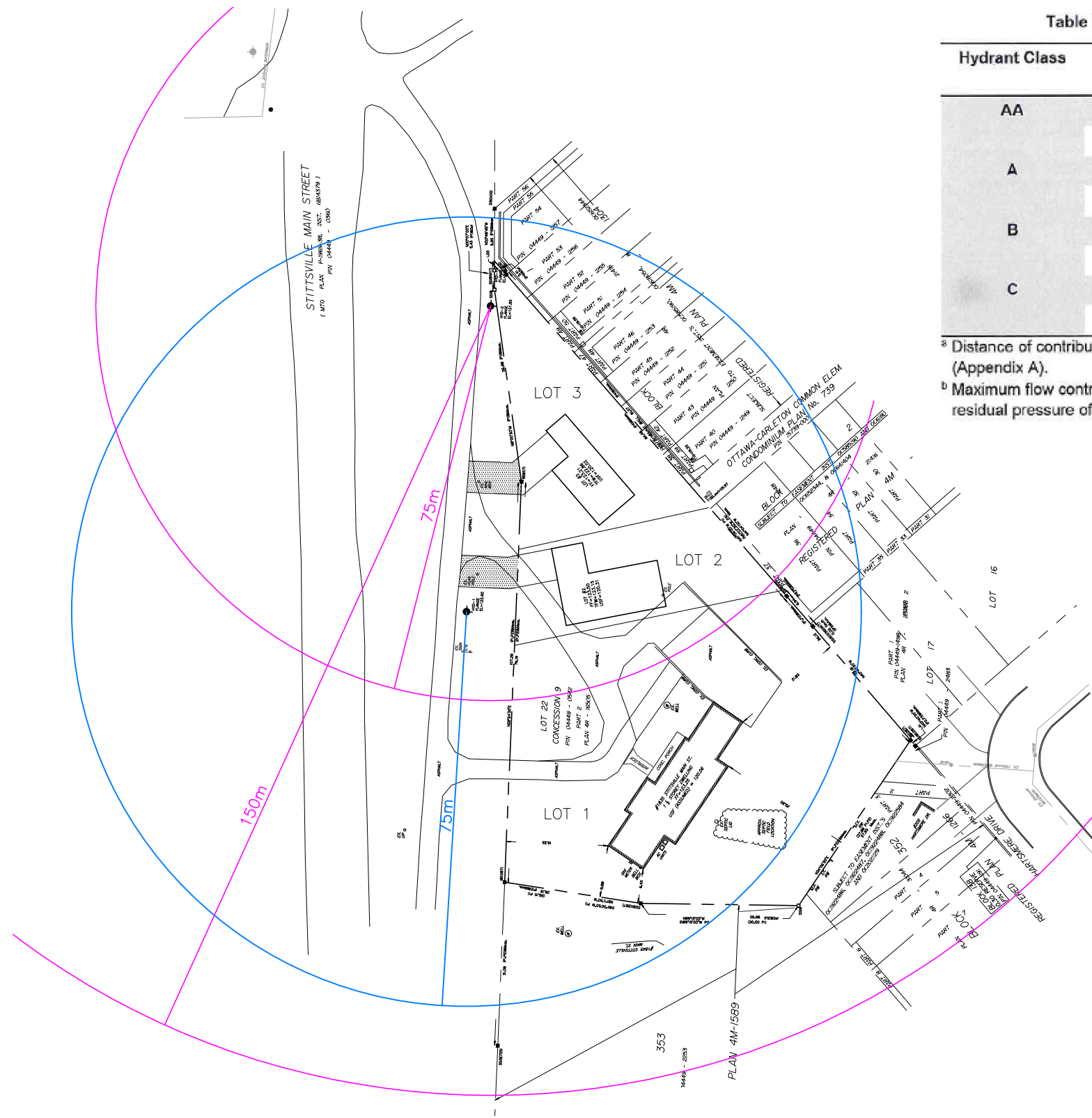
Table 1. Maximum flow to be considered from a given hydrant

Hydrant Class	Distance to asset/structure/building (m) ^a	Contribution to required fire flow (L/min) ^b
AA	≤ 75	5,700
	> 75 and ≤ 150	3,800
A	≤ 75	3,800
	> 75 and ≤ 150	2,850
B	≤ 75	1,900
	> 75 and ≤ 150	1,500
C	≤ 75	800
	> 75 and ≤ 150	800

^a Distance of contributing hydrant from the structure, measured in accordance with NFPA 1 (Appendix A).
^b Maximum flow contribution to be considered for a given asset/structure/building, at a residual pressure of 20 psi, measured at the location of the main, at ground level.

LEGEND

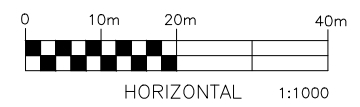
- PROPERTY BOUNDARY
- HYD-1 COVERAGE
- HYD-2 COVERAGE
- ⊙ EXISTING HYDRANT
- ⊙ PROPOSED HYDRANT



HYDRANT COVERAGE TABLE

HYDRANT ID	LOT 1		LOT 2		LOT 3	
	DISTANCE TO BUILDING (m)	CONTRIBUTION TO REQUIRED FIRE FLOW (L/min)	DISTANCE TO BUILDING (m)	CONTRIBUTION TO REQUIRED FIRE FLOW (L/min)	DISTANCE TO BUILDING (m)	CONTRIBUTION TO REQUIRED FIRE FLOW (L/min)
HYD-1	<75	5,700	<75	5,700	<75	5,700
HYD-2	>75 & <150	3,800	<75	5,700	<75	5,700
PROVIDED		9,500		11,400		11,400
REQUIRED		6,000		4,000		4,000

- NOTES:
- CONTRIBUTION TO REQUIRED FIRE FLOW DETERMINED USING TABLE 1 FROM CITY OF OTTAWA TECHNICAL BULLETIN ISTB-2018-02 APPENDIX I.
 - ASSUMED HYDRANT CLASS: AA.
 - REQUIRED FIRE FLOW AS PER FUS GUIDELINES.



Robinson

Land Development

scale	1:1000	project no.	22008
date	20/12/22	1835 STITTSVILLE MAIN STREET HYDRANT COVERAGE PLAN	
drawn by	BLM		

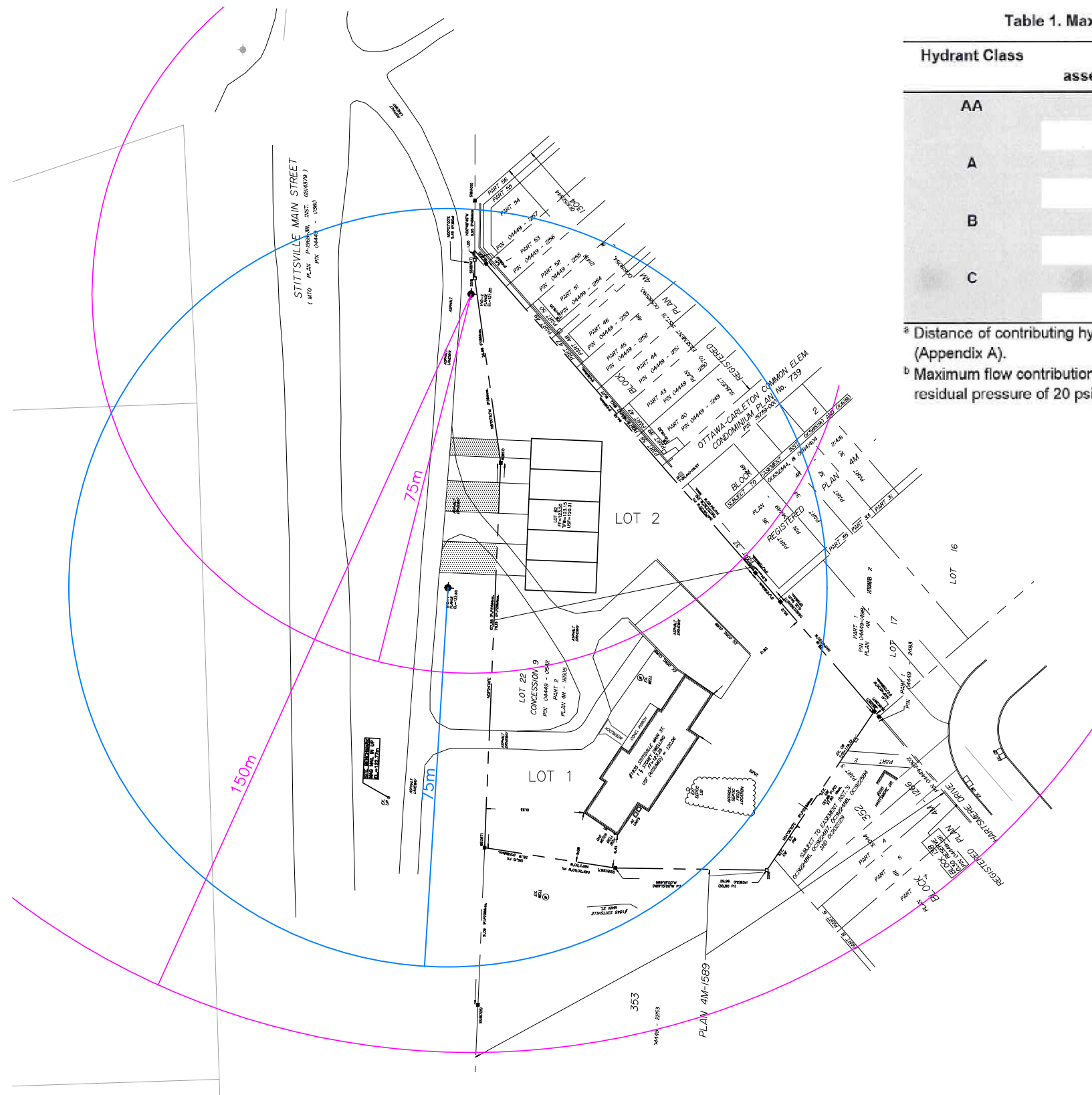


Table 1. Maximum flow to be considered from a given hydrant

Hydrant Class	Distance to asset/structure/building (m) ^a	Contribution to required fire flow (L/min) ^b
AA	≤ 75	5,700
	> 75 and ≤ 150	3,800
A	≤ 75	3,800
	> 75 and ≤ 150	2,850
B	≤ 75	1,900
	> 75 and ≤ 150	1,500
C	≤ 75	800
	> 75 and ≤ 150	800

^a Distance of contributing hydrant from the structure, measured in accordance with NFPA 1 (Appendix A).
^b Maximum flow contribution to be considered for a given asset/structure/building, at a residual pressure of 20 psi, measured at the location of the main, at ground level.

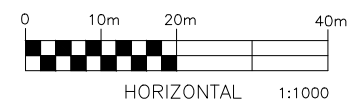


LEGEND

- PROPERTY BOUNDARY
- HYD-1 COVERAGE
- HYD-2 COVERAGE
- ⊙ EXISTING HYDRANT
- ⊙ PROPOSED HYDRANT

HYDRANT ID	LOT 1		LOT 2	
	DISTANCE TO BUILDING (m)	CONTRIBUTION TO REQUIRED FIRE FLOW (L/min)	DISTANCE TO BUILDING (m)	CONTRIBUTION TO REQUIRED FIRE FLOW (L/min)
HYD-1	<75	5,700	<75	5,700
HYD-2	>75 & <150	3,800	<75	5,700
PROVIDED		9,500		11,400
REQUIRED		6,000		6,000

- NOTES:
- CONTRIBUTION TO REQUIRED FIRE FLOW DETERMINED USING TABLE 1 FROM CITY OF OTTAWA TECHNICAL BULLETIN ISTB-2018-02 APPENDIX I.
 - ASSUMED HYDRANT CLASS: AA.
 - REQUIRED FIRE FLOW AS PER FUS GUIDELINES.



Robinson
Land Development

scale 1:1000	1835 STITTSVILLE MAIN STREET	project no. 22008
date 20/12/22		DEMO – HYDRANT COVERAGE PLAN
drawn by BLM		

Attachment C

*Novatech Sanitary Sewer Design
Sheet*

*Novatech Sanitary Drainage Area
Plan*

Sanitary Sewer Design Sheet

Sanitary Drainage Area Plans
(DWG. 22008-SAN1, SAN2)

STITTSVILLE SOUTH - AREA 6
SANITARY SEWER DESIGN SHEET



JOB# 113004

LOCATION			FLOW													PROPOSED SEWER											
FROM MH	TO MH	STREET	RESIDENTIAL UNITS				PARK PARK AREA (ha.)	COMMERCIAL COMMERCIAL AREA (ha.)	INDIVIDUAL		CUMULATIVE				PEAK FACTOR (M)	POPUL. FLOW Q(p) L/s	PEAK PARK FLOW Q(pk) L/s	PEAK COMMERCIAL FLOW Q(c) L/s	PEAK EXTRAN. FLOW Q(e) L/s	PEAK DESIGN FLOW Q(d) L/s	LENGTH (m)	PIPE SIZE (mm)	TYPE	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)
			SINGLES	SEMIS/ TOWNS	STACKS	APT.			POPUL. (1000's)	AREA (ha.)	POPUL. (1000's)	PARK AREA (ha)	COMMERCIAL AREA (ha)	RESIDENTIAL AREA (ha.)													
221	219	PARADE			70			0.161	1.023	0.161	0.00	0.00	1.023	4.000	2.609	0.00	0.00	0.287	2.895	35.3	200	PVC	1.15	36.693	1.13	8%	
219	217	PARADE	4	9				0.038	0.596	0.199	0.00	0.00	1.620	4.000	3.223	0.00	0.00	0.454	3.676	75.7	200	PVC	1.85	46.540	1.44	8%	
217	215	PARADE		5				0.014	0.293	0.212	0.00	0.00	1.913	4.000	3.442	0.00	0.00	0.536	3.977	83.3	200	PVC	2.20	50.751	1.56	8%	
267	215	HARSTMERE		12		100		0.242	1.027	0.242	0.00	0.00	1.027	4.000	3.928	0.00	0.00	0.288	4.215	84.3	200	PVC	0.40	21.640	0.67	19%	
215	213	PARADE	2					0.007	0.190	0.462	0.00	0.00	3.131	3.992	7.464	0.00	0.00	0.877	8.341	54.0	200	PVC	1.85	46.540	1.44	18%	
213	211	PARADE	7					0.024	0.412	0.485	0.00	0.00	3.543	3.981	7.828	0.00	0.00	0.992	8.820	69.0	200	PVC	1.85	46.540	1.44	19%	
211	209	PARADE	6			1.33		0.020	1.694	0.506	1.33	0.00	5.238	3.972	8.138	0.06	0.00	1.467	9.665	75.0	200	PVC	1.55	42.599	1.31	23%	
257	255	CAPMOLINA	9					0.031	0.893	0.031	0.00	0.00	0.893	4.000	0.496	0.00	0.00	0.250	0.746	120.0	200	PVC	1.50	41.907	1.29	2%	
265	255	FALABELLA	5		82			0.206	1.531	0.206	0.00	0.00	1.531	4.000	3.331	0.00	0.00	0.429	3.760	77.4	200	PVC	0.50	24.195	0.75	16%	
255	253	CAPMOLINA	7					0.024	0.557	0.260	0.00	0.00	2.982	4.000	4.213	0.00	0.00	0.835	5.048	84.0	200	PVC	0.55	25.376	0.78	20%	
263	253	QUARTER HORSE	13					0.044	0.761	0.044	0.00	0.00	0.761	4.000	0.716	0.00	0.00	0.213	0.929	119.4	200	PVC	0.40	21.640	0.67	4%	
253	251	CAPMOLINA	5					0.017	0.425	0.321	0.00	0.00	4.169	4.000	5.205	0.00	0.00	1.167	6.372	81.9	200	PVC	1.60	43.281	1.33	15%	
261	251	LIPIZZANER		31				0.084	0.940	0.084	0.00	0.00	0.940	4.000	1.356	0.00	0.00	0.263	1.620	117.2	200	PVC	0.60	26.504	0.82	6%	
251	249	CAPMOLINA	7					0.024	0.573	0.429	0.00	0.00	5.683	4.000	6.947	0.00	0.00	1.591	8.538	90.3	200	PVC	1.35	39.756	1.23	21%	
249	247	CAPMOLINA	7					0.024	0.616	0.453	0.00	0.00	6.299	3.996	7.325	0.00	0.00	1.764	9.089	98.3	200	PVC	1.35	39.756	1.23	23%	
247	245	CAPMOLINA	1					0.003	0.148	0.456	0.00	0.00	6.448	3.995	7.377	0.00	0.00	1.805	9.182	10.9	200	PVC	1.35	39.756	1.23	23%	
245	243	CAPMOLINA	11					0.037	0.632	0.493	0.00	0.00	7.080	3.977	7.948	0.00	0.00	1.982	9.930	71.4	200	PVC	0.60	26.504	0.82	37%	
243	209	CAPMOLINA	8					0.027	0.432	0.521	0.00	0.00	7.512	3.965	8.361	0.00	0.00	2.103	10.464	55.9	200	PVC	0.60	26.504	0.82	39%	
209	207	PARADE	7					0.024	0.411	1.050	1.33	0.00	13.162	3.786	16.106	0.06	0.00	3.685	19.850	82.0	250	PVC	0.85	57.197	1.13	35%	
207	205	PARADE	7	9				0.048	0.622	1.098	1.33	0.00	13.784	3.773	16.787	0.06	0.00	3.860	20.704	82.0	250	PVC	0.85	57.197	1.13	36%	
241	205	PEDIGREE	14					0.048	0.776	0.048	0.00	0.00	0.776	4.000	0.771	0.00	0.00	0.217	0.989	119.0	200	PVC	0.35	20.243	0.62	5%	
205	203	PARADE	7	9				0.048	0.609	1.194	1.33	0.00	15.170	3.749	18.132	0.06	0.00	4.248	22.437	82.0	250	PVC	0.60	48.055	0.95	47%	
239A	239B	MANEGE	16					0.054	0.865	0.054	0.00	0.00	0.865	4.000	0.881	0.00	0.00	0.242	1.124	107.7	200	PVC	0.40	21.640	0.67	5%	
239B	203	MANEGE						0.000	0.000	0.054	0.00	0.00	0.865	4.000	0.881	0.00	0.00	0.242	1.124	11.1	200	PVC	0.40	21.640	0.67	5%	
203	201	PARADE	7					0.024	0.417	1.272	1.33	0.00	16.453	3.730	19.222	0.06	0.00	4.607	23.886	82.0	250	PVC	0.60	48.055	0.95	50%	
237	235	STALLION	1	28				0.079	0.893	0.079	0.00	0.00	0.893	4.000	1.280	0.00	0.00	0.250	1.530	112.8	200	PVC	0.50	24.195	0.75	6%	
235	233	STALLION	2					0.007	0.256	0.086	0.00	0.00	1.150	4.000	1.390	0.00	0.00	0.322	1.712	11.0	200	PVC	0.50	24.195	0.75	7%	
233	231	STALLION	5					0.017	0.431	0.103	0.00	0.00	1.581	4.000	1.666	0.00	0.00	0.443	2.108	74.2	200	PVC	0.50	24.195	0.75	9%	
231	229	STALLION	4					0.014	0.499	0.116	0.00	0.00	2.081	4.000	1.886	0.00	0.00	0.583	2.469	82.0	200	PVC	0.50	24.195	0.75	10%	
229	227	STALLION	4					0.014	0.483	0.130	0.00	0.00	2.564	4.000	2.106	0.00	0.00	0.718	2.824	74.7	200	PVC	0.50	24.195	0.75	12%	
227	225	STALLION	2					0.007	0.230	0.137	0.00	0.00	2.794	4.000	2.217	0.00	0.00	0.782	2.999	10.9	200	PVC	0.50	24.195	0.75	12%	
225	223	STALLION	11					0.037	0.541	0.174	0.00	0.00	3.336	4.000	2.823	0.00	0.00	0.934	3.757	113.2	200	PVC	0.50	24.195	0.75	16%	
223	201	STALLION	8					0.027	0.418	0.201	0.00	0.00	3.754	4.000	3.263	0.00	0.00	1.051	4.315	11.1	200	PVC	0.50	24.195	0.75	18%	
201	159	PARADE	6					0.020	0.410	1.494	1.33	0.00	20.62	3.681	22.275	0.06	0.00	5.773	28.105	82.0	300	PVC	0.50	71.334	0.98	39%	
157	155	BECKETT	11					0.037	0.530	0.037	0.00	0.00	0.530	4.000	0.606	0.00	0.00	0.148	0.754	112.7	200	PVC	0.40	21.640	0.67	3%	
155	159	BECKETT	6					0.020	0.330	0.058	0.00	0.00	0.860	4.000	0.937	0.00	0.00	0.241	1.177	12.0	200	PVC	0.70	28.628	0.88	4%	
159	145	PARADE	13					0.044	0.631	1.596	1.33	0.00	22.108	3.660	23.661	0.06	0.00	6.190	29.908	82.0	300	PVC	0.50	71.334	0.98	42%	
157	153	BECKETT	2					0.007	0.244	0.007	0.00	0.00	0.244	4.000	0.110	0.00	0.00	0.068	0.179	10.9	200	PVC	0.30	18.741	0.58	1%	
153	151	BECKETT	6					0.020	0.561	0.027	0.00	0.00	0.805	4.000	0.441	0.00	0.00	0.226	0.666	66.8	200	PVC	0.30	18.741	0.58	4%	
151	149	BECKETT	1					0.003	0.114	0.031	0.00	0.00	0.920	4.000	0.496	0.00	0.00	0.258	0.753	11.1	200	PVC	0.50	24.195	0.75	3%	
149	147	BECKETT		14				0.038	0.445	0.068	0.00	0.00	1.365	4.000	1.108	0.00	0.00	0.382	1.491	112.3	200	PVC	0.50	24.195	0.75	6%	
147	145	BECKETT		9				0.024	0.393	0.093	0.00	0.00	1.759	4.000	1.502	0.00	0.00	0.492	1.994	11.9	200	PVC	0.85	31.546	0.97	6%	
145	143	PARADE	9					0.031	0.589	1.719	1.33	0.00	24.456	3.636	25.322	0.06	0.00	6.848	32.226	74.3	300	PVC	0.50	71.334	0.98	45%	
143	141	PARADE	3					0.010	0.262	1.729	1.33	0.00	24.719	3.634	25.459	0.06	0.00	6.921	32.436	13.9	300	PVC	0.50	71.334	0.98	45%	
141	139	PARADE	6					0.020	0.359	1.750	1.33	0.00	25.078	3.630	25.732	0.06	0.00	7.022	32.810	61.2	300	PVC	0.50	71.334	0.98	46%	
139	137	PARADE	12					0.041	0.569	1.791	1.33	0.00	25.647	3.623	26.277	0.06	0.00	7.181	33.514	60.8	300	PVC	0.50	71.334	0.98	47%	
137	135	PARADE	2					0.007	0.222	1.797	1.33	0.00	25.870	3.621	26.368	0.06	0.00	7.244	33.667	12.3	300	PVC	0.50	71.334	0.98	47%	
135	133	PARADE	5					0.017	0.404	1.814	1.33	0.00	26.274	3.618	26.594	0.06	0.00	7.357	34.007	74.3	300	PVC	0.50	71.334	0.98	48%	

STITTSVILLE SOUTH - AREA 6
SANITARY SEWER DESIGN SHEET



JOB# 113004

123	121	HICKSTEAD	3					0.010	0.262	0.010	0.00	0.00	0.262	4.000	0.165	0.00	0.00	0.073	0.239	12.9	200	PVC	0.50	24.195	0.75	1%
121	119	HICKSTEAD	10					0.034	0.512	0.044	0.00	0.00	0.775	4.000	0.716	0.00	0.00	0.217	0.933	60.7	200	PVC	0.60	26.504	0.82	4%
119	133	HICKSTEAD	10					0.034	0.502	0.078	0.00	0.00	1.277	4.000	1.267	0.00	0.00	0.358	1.625	71.4	200	PVC	0.80	30.604	0.94	5%
133	131	PARADE	5					0.017	0.403	1.910	1.33	0.00	27.955	3.601	27.859	0.06	0.00	7.827	35.741	82.0	375	PVC	0.30	100.184	0.88	36%
115	117	CAVALLO		16				0.043	0.496	0.043	0.00	0.00	0.496	4.000	0.700	0.00	0.00	0.139	0.839	70.9	200	PVC	1.90	47.164	1.45	2%
117	131	CAVALLO		18				0.049	0.541	0.092	0.00	0.00	1.038	4.000	1.488	0.00	0.00	0.291	1.778	71.0	200	PVC	1.90	47.164	1.45	4%
131	129	PARADE	6					0.020	0.402	2.022	1.33	0.00	29.395	3.582	29.338	0.06	0.00	8.231	37.624	74.3	375	PVC	0.30	100.184	0.88	38%
129	127	PARADE	1					0.003	0.083	2.025	1.33	0.00	29.478	3.582	29.383	0.06	0.00	8.254	37.692	12.4	375	PVC	0.30	100.184	0.88	38%
127	125	PARADE	6					0.020	0.374	2.046	1.33	0.00	29.852	3.578	29.651	0.06	0.00	8.359	38.064	69.0	375	PVC	0.30	100.184	0.88	38%
125	113	PARADE	4			0.85		0.014	1.126	2.059	2.18	0.00	30.979	3.576	29.829	0.09	0.00	8.674	38.593	63.9	375	PVC	0.15	70.841	0.62	54%
123	115	HICKSTEAD	6					0.020	0.401	0.020	0.00	0.00	0.401	4.000	0.331	0.00	0.00	0.112	0.443	73.4	200	PVC	1.35	39.756	1.23	1%
115	113	HICKSTEAD	6					0.020	0.686	0.041	0.00	0.00	1.088	4.000	0.661	0.00	0.00	0.305	0.966	83.0	200	PVC	1.35	39.756	1.23	2%
113	111	HICKSTEAD	7					0.024	0.532	2.124	2.18	0.00	32.599	3.565	30.674	0.09	0.00	9.128	39.892	111.0	375	PVC	0.15	70.841	0.62	56%
111	109	HICKSTEAD	5					0.017	0.401	2.141	2.18	0.00	33.000	3.563	30.896	0.09	0.00	9.240	40.226	115.7	375	PVC	0.60	141.682	1.24	28%
Friendly Cres.			70					0.238	4.860																	
Davidson			329	230	0	172	2.14	2.93	2.101	32.710																
109	107									4.480	4.32	2.93	70.571	3.289	59.682	0.16	2.54	20.580	82.970	71.7	375	PVC	0.25	91.455	0.80	91%
107	105									4.480	4.32	2.93	70.571	3.289	59.682	0.16	2.54	20.580	82.970	62.1	375	PVC	0.25	91.455	0.80	91%
105	101									4.480	4.32	2.93	70.571	3.289	59.682	0.16	2.54	20.580	82.970	11.0	375	PVC	2.00	258.675	2.27	32%
101	99									4.480	4.32	2.93	70.571	3.289	59.682	0.16	2.54	20.580	82.970	73.3	450	CONC	2.00	420.634	2.56	20%
99	PS									4.480	4.32	2.93	70.571	3.289	59.682	0.16	2.54	20.580	82.970	6.1	450	CONC	2.00	420.634	2.56	20%

Design Parameters:

- 1) Q(e) = 0.28 L/sec/ha Singles 3.4 persons/unit
- 2) Q(p) = (PqxM/86,400) Semis/Towns 2.7 persons/unit
- 3) Q(pk) = 1000 L/d/ha x M Stacked 2.3 persons/unit
- 4) Q(c) = 50000 L/d/ha x N Apartements 2.1 persons/unit

5) Q(d) = Q(p) + Q(pk) + Q(c) + Q(e)

Definitions:

- P = Population
- q = Average per capita flow = 350 L/person/day
- M = Residential Peaking Factor (Harmon Formula from section 4.4.1 of the City Sewer Design Guidelines):

N = Commercial / Park Peaking Factor (1.5) from City Design Guidelines



- Q(d) = Design Flow (L/sec)
- Q(p) = Population Flow (L/sec)
- Q(pk) = Park Flow (L/sec)
- Q(c) = Commercial Flow (L/sec)
- Q(e) = Extraneous Flow (L/sec)

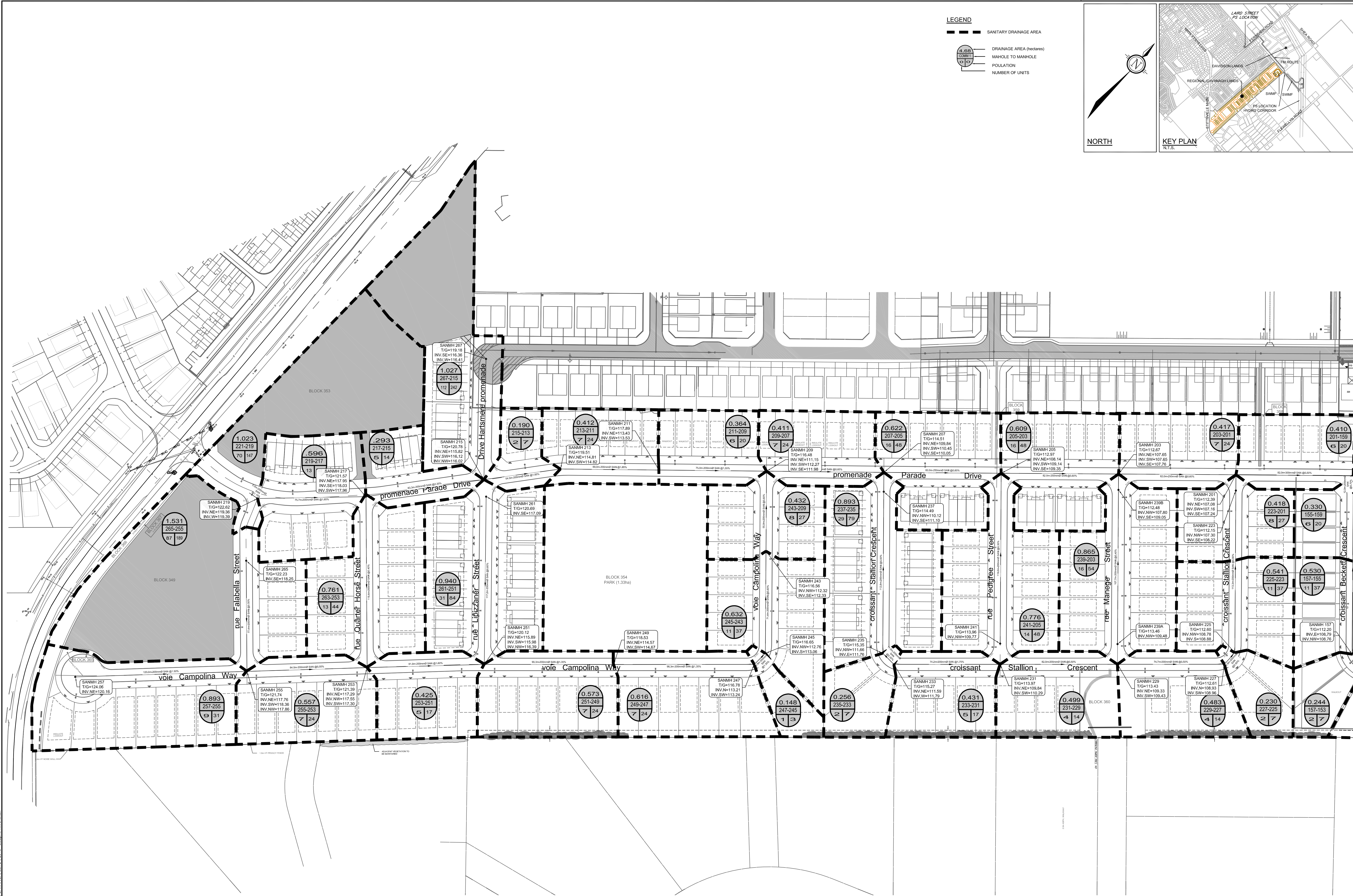
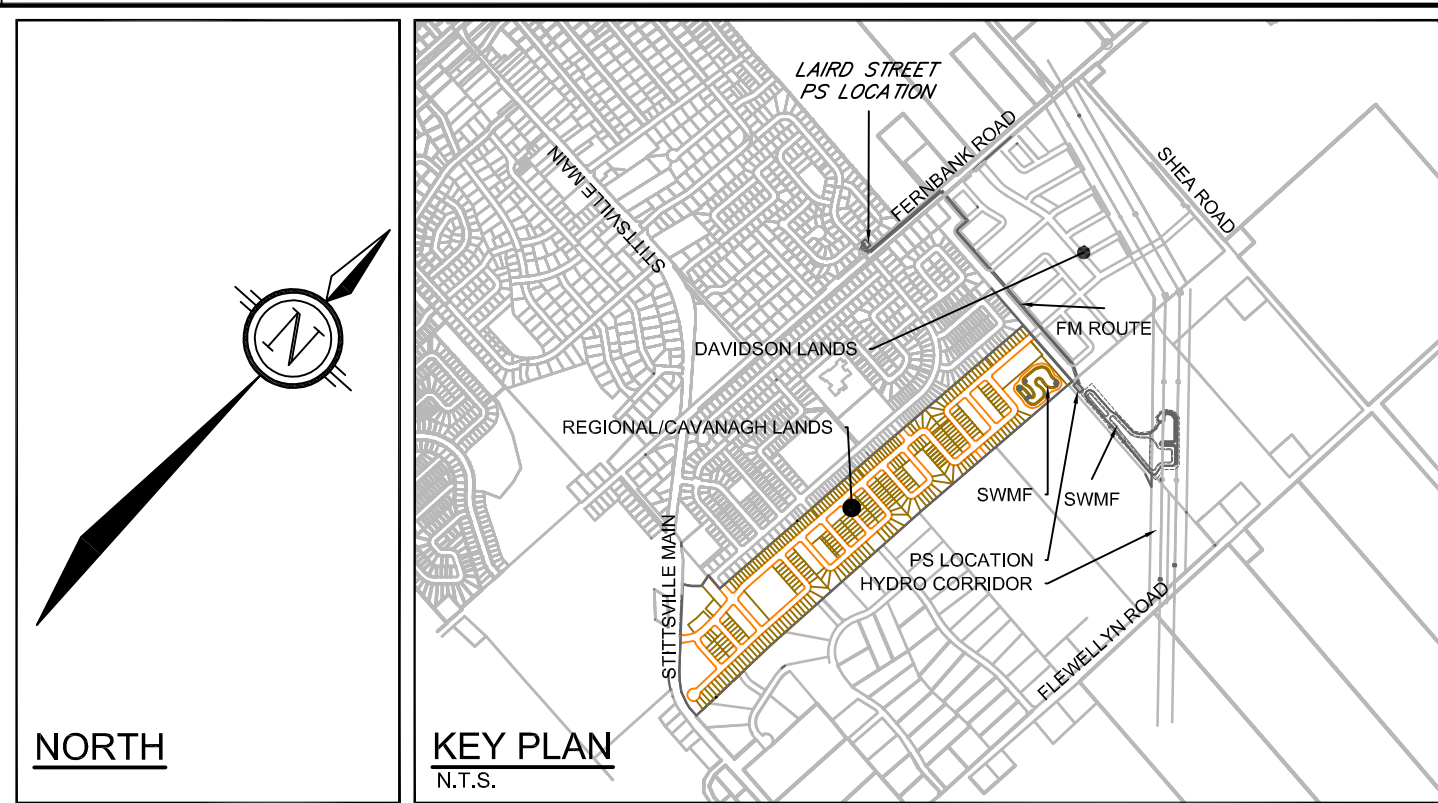
STITTSVILLE SOUTH - AREA 6
SANITARY SEWER DESIGN SHEET

Date	April 5, 2016		
Design	BHB		
Job No.	Dwg. Reference:	Checked and Stamped:	
113004	113004-SAN	--	



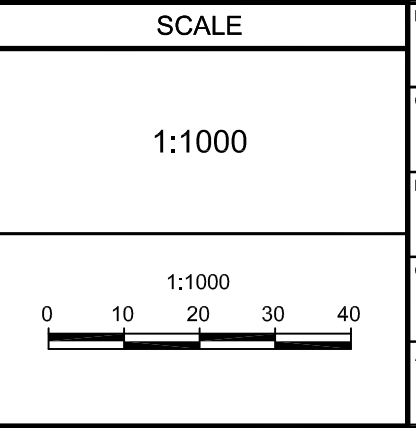
LEGEND

-  SANITARY DRAINAGE AREA
-  DRAINAGE AREA (hectares)
MAHOLE TO MANHOLE
POULATION
NUMBER OF UNITS



NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION	DATE	BY
3.	ISSUED FOR TENDER	JAN 2016	BHB
2.	REVISED AS PER CITY COMMENTS	DEC 16/15	BHB
1.	ISSUED FOR CITY REVIEW	SEPT 14/15	BHB



FOR REVIEW ONLY	
DESIGN	BCS/TGP
CHECKED	BHB
DRAWN	BCS/TGP
CHECKED	BHB
APPROVED	GJM



CITY OF OTTAWA
STITTSVILLE SOUTH - AREA 6

SANITARY DRAINAGE AREA PLAN

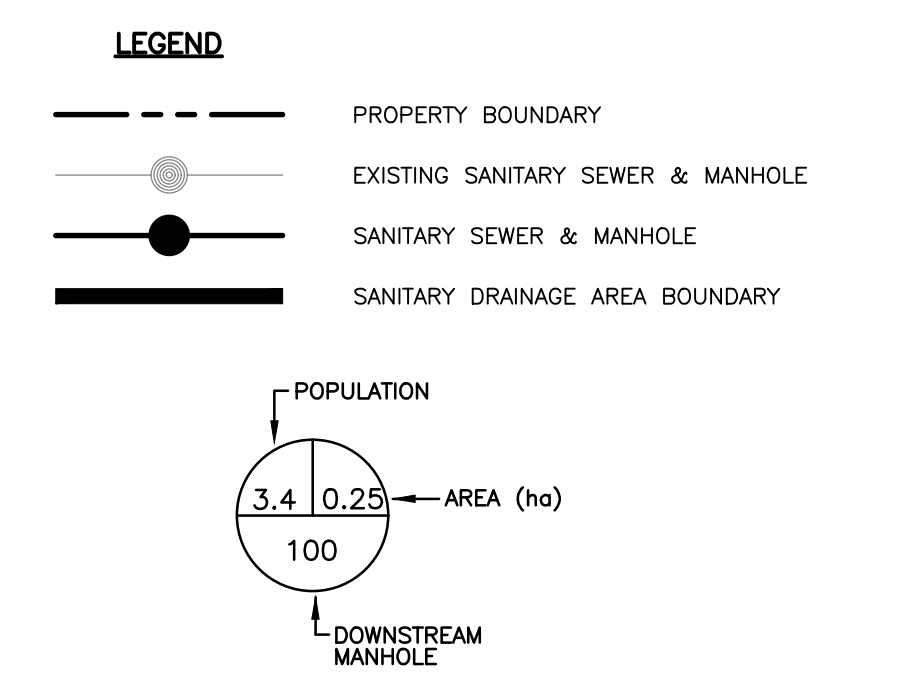
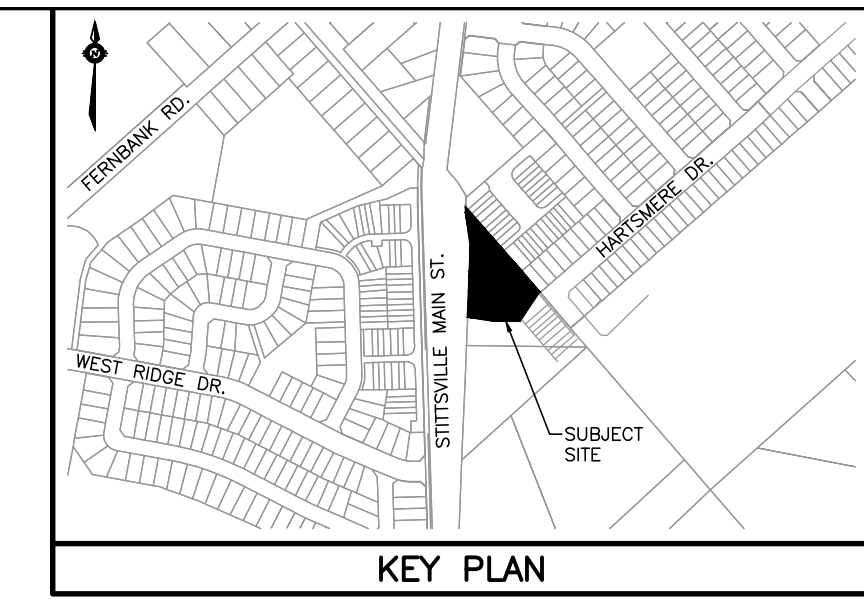
PROJECT NO.	113004-00
REV	REV # 3
DRAWING NO.	113004-SAN1

C:\03\113004\CAD\Drawings\113004-SAN1.dwg, C:\03\113004\CAD\Drawings\113004-SAN1.dwg, C:\03\113004\CAD\Drawings\113004-SAN1.dwg, C:\03\113004\CAD\Drawings\113004-SAN1.dwg

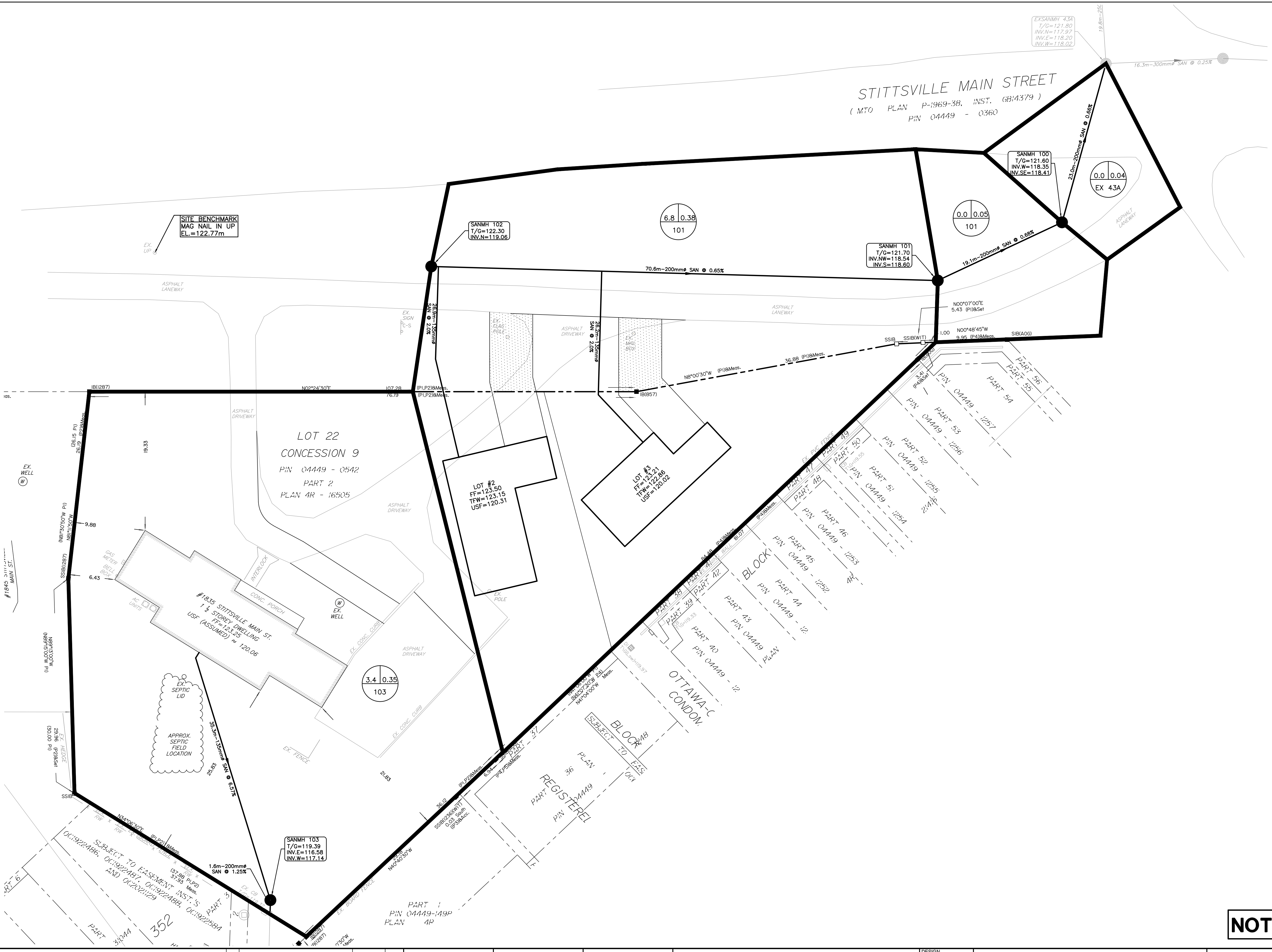
SANITARY SEWER DESIGN SHEET
1835 STITTSVILLE MAIN STREET, STITTSVILLE

LOCATION			RESIDENTIAL AREA AND POPULATION						RESIDENTIAL FLOW				PIPE						
			UNIT COUNT		INDIVIDUAL		CUMULATIVE		PEAK FACTOR	PEAK POP. FLOW (L/s)	EXTRAN. FLOW (L/s)	PEAK DESIGN FLOW (L/s)	LENGTH (m)	DIAMETER (mm)	SLOPE (%)	CAPACITY (L/s)	VELOCITY (m/s)	EXCESS CAPACITY (L/s)	PERCENT FULL
STREET	FROM MH	TO MH	SINGLE-FAMILY	TOWNHOUSE	POP.	AREA (ha)	POP.	AREA (ha)											
TO STITTSVILLE MAIN STREET SANITARY SEWER (PROPOSED DEVELOPMENT)																			
LANEWAY	102	101	2	0	6.8	0.38	6.8	0.38	3.74	0.08	0.13	0.21	70.6	201.16	0.65	26.88	0.85	26.67	0.77
LANEWAY	101	100	0	0	0.0	0.05	6.8	0.43	3.74	0.08	0.14	0.22	19.1	201.16	0.68	27.49	0.87	27.27	0.82
LANEWAY	100	EX43A	0	0	0.0	0.04	6.8	0.47	3.74	0.08	0.16	0.24	23.0	201.16	0.66	27.09	0.85	26.85	0.88
TO STITTSVILLE MAIN STREET SANITARY SEWER (DEMONSTRATION DEVELOPMENT)																			
LANEWAY	102	101	0	5	13.5	0.38	13.5	0.38	3.72	0.16	0.13	0.29	70.6	201.16	0.65	26.88	0.85	26.59	1.07
LANEWAY	101	100	0	0	0.0	0.05	13.5	0.43	3.72	0.16	0.14	0.30	19.1	201.16	0.68	27.49	0.87	27.19	1.11
LANEWAY	100	EX43A	0	0	0.0	0.04	13.5	0.47	3.72	0.16	0.16	0.32	23.0	201.16	0.66	27.09	0.85	26.77	1.17
TO HARTSMERE DRIVE SANITARY SEWER																			
LOT 1	EX BLDG	103	1	0	3.4	0.35	3.4	0.35	3.76	0.04	0.12	0.16	35.3	135.00	6.57	29.50	2.06	29.35	0.53
HARTSMERE EASEMENT	103	EX 267	0	0	0.0	0.00	3.4	0.35	3.76	0.04	0.12	0.16	21.3	201.16	0.70	27.90	0.88	27.74	0.56
DESIGN PARAMETERS																			
Average Daily Flow = 280 L/person/day Comm./Inst. Flow = 28000 L/ha/day Industrial Flow = Maximum Residential Peak Factor = 4.0 Harmon - Correction Factor (K) = 0.8 Comm./Inst. Peak Factor = 1.5 Extraneous Flow = 0.33 L/s/ha Minimum Velocity = 0.6 m/s Maximum Velocity = 3.0 m/s									Per Unit Populations: Single Family 3.4 persons/unit Semi-detached 2.7 persons/unit Duplex 2.3 persons/unit Townhouse 2.7 persons/unit Apartments: Bachelor 1.4 persons/unit 1 Bedroom 1.4 persons/unit 2 Bedroom 2.1 persons/unit 3 Bedroom 3.1 persons/unit Average Apt. 1.8 persons/unit										





STITTSVILLE MAIN STREET
(MTO PLAN P-1969-38, INST. GB14379)
PIN 04449 - 0360



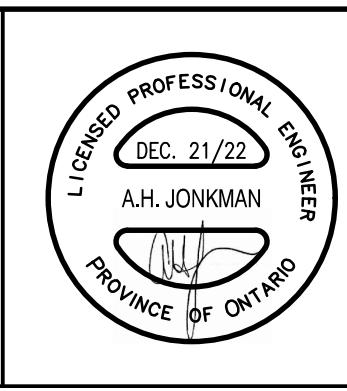
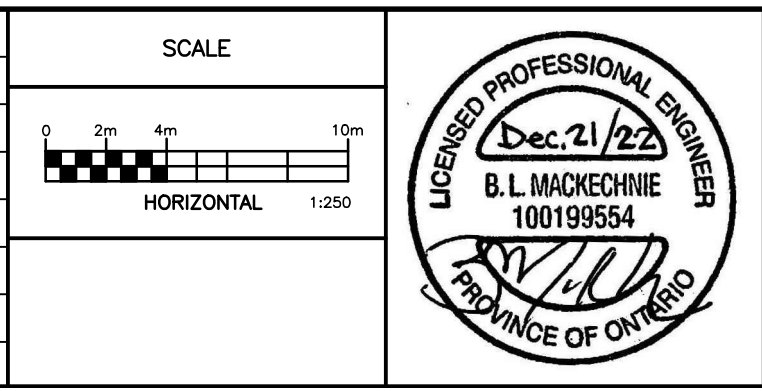
NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
2	REVISED PER COMMENTS	21/12/22	AHJ
1	REVISED PER COMMENTS	01/11/22	AHJ

SCALE
0 2m 4m 10m
HORIZONTAL 1:250



Robinson
Land Development

350 Palladium Drive
Ottawa, ON K2V 1A8
(613) 592-6060 roii.com

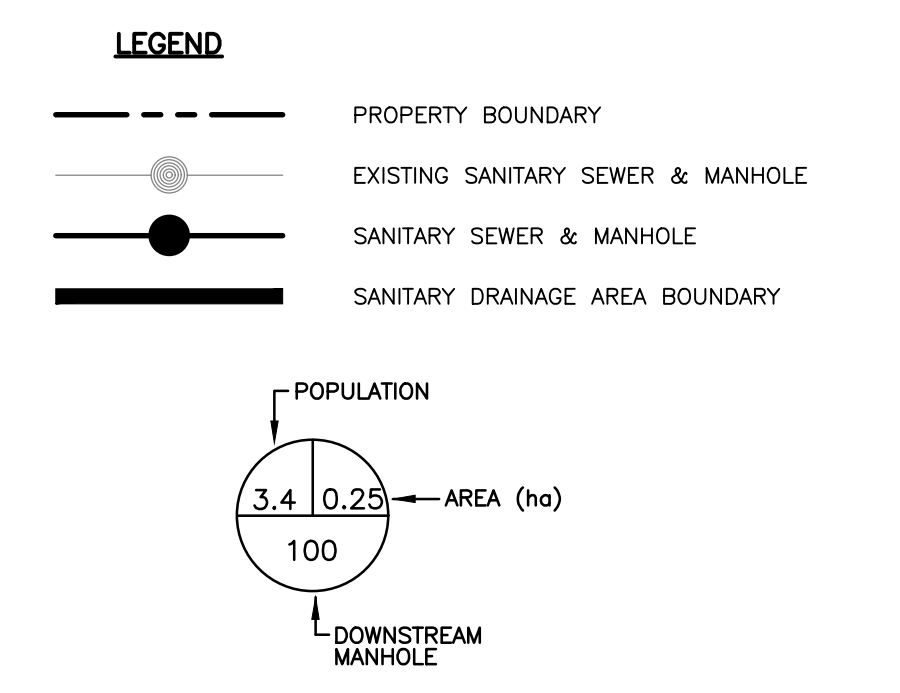
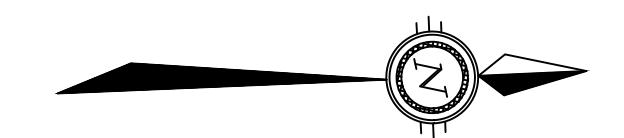
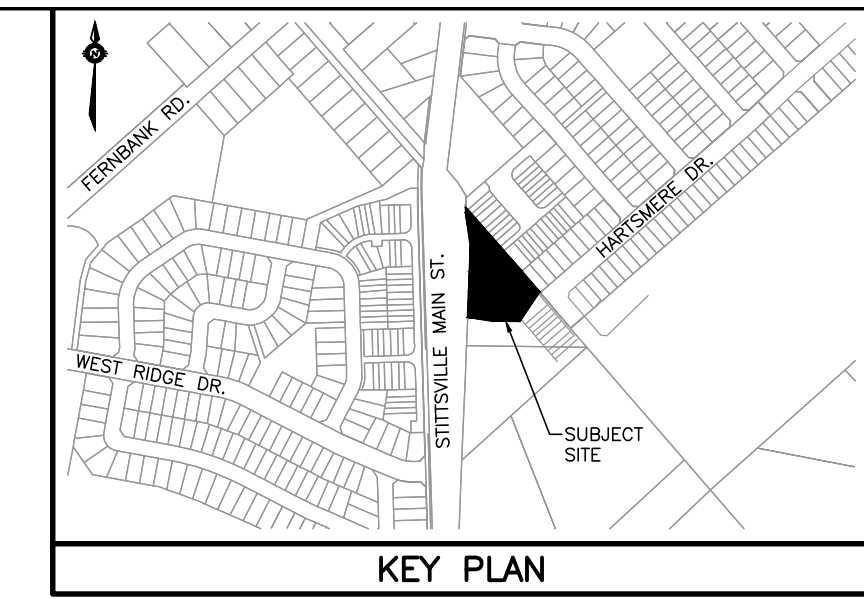
DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

JENNIFER McGAHAN
WIRE INVESTMENT CORP.

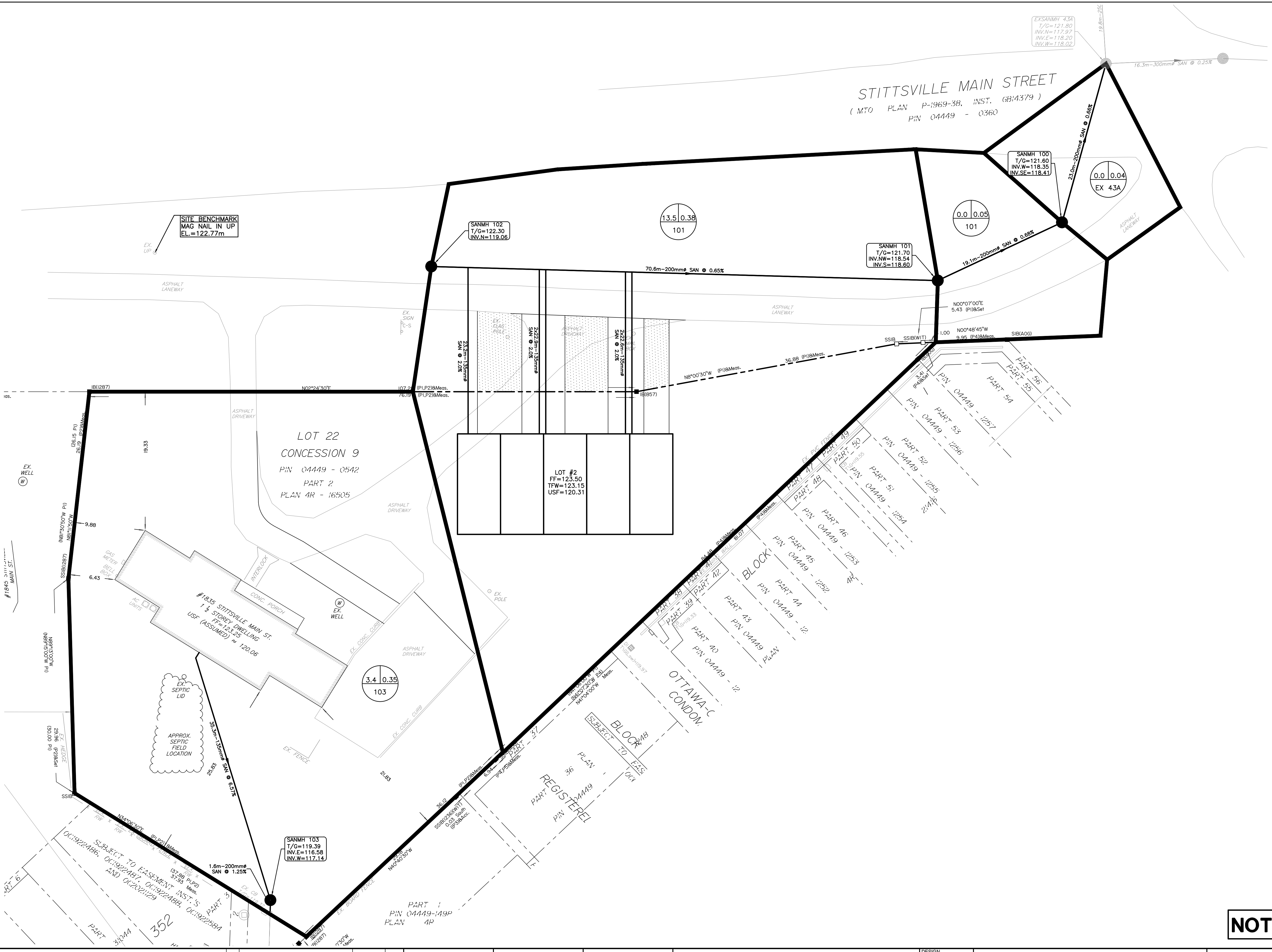
1835 STITTSVILLE MAIN STREET
STITTSVILLE, ON

PROPOSED DEVELOPMENT
SANITARY DRAINAGE AREA PLAN

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-SAN1



STITTSVILLE MAIN STREET
(MTO PLAN P-1969-38, INST. GB14379)
PIN 04449 - 0360



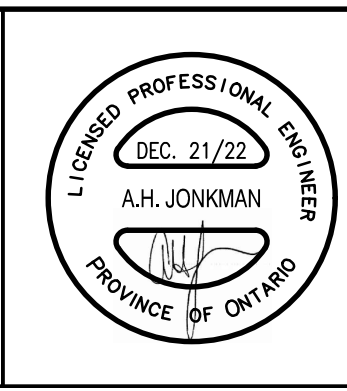
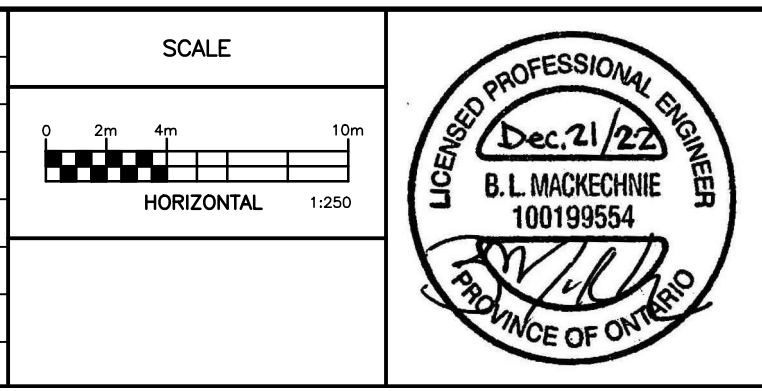
NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

NO.	REVISION DESCRIPTION	DATE	BY
2	REVISED PER COMMENTS	21/12/22	AHJ
1	REVISED PER COMMENTS	01/11/22	AHJ

SCALE	
HORIZONTAL 1:250	



Robinson
Land Development

350 Palladium Drive
Ottawa, ON K2V 1A8
(613) 592-6060 roii.com

DESIGN	BLM
CHECKED	AHJ
DRAWN	BLM
CHECKED	AHJ
APPROVED	AHJ

JENNIFER McGAHAN
WIRE INVESTMENT CORP.

1835 STITTSVILLE MAIN STREET
STITTSVILLE, ON

DEMONSTRATION DEVELOPMENT
SANITARY DRAINAGE AREA PLAN

PROJECT No.	22008
SURVEY	AOV
DATED	DECEMBER 2022
DWG. No.	22008-SAN2

Attachment D

*Novatech Storm Sewer Design
Sheet*

Novatech Storm Drainage Area Plan

Storm Sewer Design Sheet

LOCATION				AREA (ha)									FLOW						PROPOSED SEWER								
Area	Street	FROM MH	TO MH	R=	R=	R=	R=	R=	R=	R=	R=	INDIV. 2.78 AC	ACCUML. 2.78 AC	TIME OF CONC.	DESIGN STORM	RAINFALL INTENSITY	Peak Flow (L/sec)		DIA. ACTUAL (mm)	DIA. (mm)	TYPE	SLOPE (%)	LENGTH (m)	CAPACITY (L/s)	VELOCITY (m/s)	FLOW TIME (min)	Ratio (Q/Qfull)
				0.20	0.40	0.45	0.50	0.55	0.60	0.65	0.70						0.75	Q 5YR									
A-01	PARADE	220	218									0.14	0.14	10.00	5	104.19	15.0	15.0	0.457	450	CONC	1.15	21.1	318.7	1.94	0.18	5%
A-03	PARADE	218	216									0.17	0.17	10.18	5	103.25	74.7	74.7	0.457	450	CONC	1.85	73.5	404.3	2.46	0.50	18%
A-02	PARADE	218	216									0.41	0.72														
A-04	PARADE	216	214									0.87	1.82	15.00	5	83.56	151.7	151.7	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
A-06	PARADE	216	214				0.34					0.47	0.47	15.00	5	83.56	334.0	334.0	0.457	450	CONC	2.20	83.2	440.8	2.69	0.52	76%
A-05, A07	PARADE	216	214									0.99	4.00	15.52													
A-10	HEARTSMERE	266	214									0.37	0.37	10.00	5	104.19	96.7	96.7	0.533	525	CONC	0.40	85.6	283.6	1.27	1.12	34%
A-09	HEARTSMERE	266	214									0.55	0.93	11.12													
A-12	PARADE	214	212									0.27	0.27	15.52	5	81.93	450.7	450.7	0.610	600	CONC	1.85	54.2	870.7	2.98	0.30	52%
A-11, A-13	PARADE	214	212									0.30	5.50	15.82	5	81.00	471.4	471.4	0.610	600	CONC	1.85	71.7	870.7	2.98	0.40	54%
A-14	PARADE	212	210									0.32	5.82	16.22	5	79.81	575.6	575.6	0.686	675	CONC	1.55	75.0	1,091.1	2.95	0.42	53%
A-15	PARADE	210	208	1.01								0.56	0.56	16.64													
A-16	PARADE	210	208			0.21						0.26	0.26	10.00	5	104.19	98.5	98.5	0.381	375	PVC	1.6	120.0	231.2	2.03	0.99	43%
A-17	PARADE	210	208									0.57	7.21	10.99													
A-18, A-19	CAMPOLINA	256	254									0.95	0.95	15.00	5	83.56	197.0	197.0	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
A-20	FALABELLA	264	254									2.36	2.36	15.00	5	83.56	246.1	246.1	0.610	600	CONC	0.50	77.3	452.7	1.55	0.83	54%
A-21	FALABELLA	264	254									0.59	2.95	15.83													
A-22	CAMPOLINA	254	252			0.46						0.51	0.51	15.83	5	80.97	443.3	443.3	0.686	675	CONC	0.55	81.0	650.0	2.75	0.49	68%
A-23	CAMPOLINA	254	252			0.33						0.36	0.36	16.32													
A-24	CAMPOLINA	254	252									0.71	5.47	10.00	5	104.19	91.2	91.2	0.457	450	CONC	0.40	119.4	188.0	2.75	0.72	49%
A-25, A-26, A-27	QUARTER HORSE	216	252									0.88	0.88	10.72													
A-28	CAMPOLINA	252	250			0.34						0.38	0.38	16.32	5	79.52	541.2	541.2	0.686	675	CONC	1.60	81.9	1,108.6	3.00	0.45	49%
A-29	CAMPOLINA	252	250									0.46	6.81	16.78													
A-30,31	LIPIZZANER	260	250									1.02	1.02	10.00	5	104.19	105.8	105.8	0.457	450	CONC	0.60	119.7	230.2	2.75	0.73	46%
A-32	CAMPOLINA	250	248									0.47	8.29	16.78	5	78.23	648.2	648.2	0.762	750	CONC	1.35	93.3	1,348.7	2.75	0.57	48%
A-34	CAMPOLINA	248	246			0.30						0.33	0.33	17.34	5	76.68	758.6	758.6	0.762	750	CONC	1.35	97.4	1,348.7	2.96	0.55	56%
A-35	CAMPOLINA	248	246			0.38						0.42	0.75	17.34	5	76.68	758.6	758.6	0.762	750	CONC	1.35	97.4	1,348.7	2.96	0.55	56%
A-33, A-36	CAMPOLINA	248	246									0.52	9.89	17.89	5	75.25	744.5	744.5	0.762	750	CONC	1.35	12.1	1,348.7	2.75	0.07	55%
A-37	CAMPOLINA	246	244									0.00	9.89	17.96	5	75.06	804.5	804.5	0.991	975	CONC	0.60	70.5	1,810.1	2.75	0.43	44%
A-37	CAMPOLINA	244	242									0.83	10.72	18.39	5	73.99	824.0	824.0	0.991	975	CONC	0.60	55.9	1,810.1	2.75	0.34	46%
A-38	CAMPOLINA	242	208									0.42	11.14	18.73													
A-39	PARADE	208	206				0.37					0.52	0.52	18.73	5	73.17	1,458.2	1,458.2	1.067	1050	CONC	0.85	79.0	2,625.3	2.94	0.45	56%
A-40	PARADE	208	206									1.06	19.93	19.18													

LOCATION				AREA (ha)									FLOW						PROPOSED SEWER								
Area	Street	FROM MH	TO MH	R=	R=	R=	R=	R=	R=	R=	R=	INDIV. 2.78 AC	ACCUML. 2.78 AC	TIME OF CONC.	DESIGN STORM	RAINFALL INTENSITY	Peak Flow (L/sec)		DIA. ACTUAL (mm)	DIA. (mm)	TYPE	SLOPE (%)	LENGTH (m)	CAPACITY (L/s)	VELOCITY (m/s)	FLOW TIME (min)	Ratio (Q/Qfull)
				0.20	0.40	0.45	0.50	0.55	0.60	0.65	0.70						0.75	Q 5YR									
A-56	STALLION	232	234		0.38							0.42	0.42	10.00	5	104.19	43.7	43.7	0.381	375	PVC	0.30	12.3	100.1	0.88	0.23	44%
A-41	STALLION	234	236									0.48	0.90	10.23	5	102.98	92.5	92.5	0.533	525	CONC	0.50	111.9	317.0	1.42	1.31	29%
A-42	STALLION	236	206									0.57	1.47	11.55	5	96.67	141.9	141.9	0.533	525	CONC	0.50	13.5	317.0	1.42	0.16	45%
														11.71													
A-44	PARADE	206	204			0.58						0.72	0.72	19.18	5	72.11	1,646.3	1,646.3	1.067	1050	CONC	0.85	84.9	2,625.3	2.94	0.48	63%
A-43	PARADE	206	204									0.71	22.83														
														19.66													
A-46	PEDIGREE	240	204				0.36					0.50	0.50	10.00	5	104.19	148.5	148.5	0.533	525	CONC	0.35	118.4	265.3	1.19	1.66	56%
A-45, A-47, A-48	PEDIGREE	240	204									0.92	1.43														
														11.19													
A-49	PARADE	204	202									0.94	25.19	19.66	5	71.01	1,788.8	1,788.8	1.372	1350	CONC	0.60	79.0	4,311.6	2.75	0.48	41%
														20.14													
A-52	MANEGE	238	202			0.37						0.46	0.46	10.00	5	104.19	149.5	149.5	0.457	450	CONC	0.40	119.0	188.0	1.14	1.73	80%
A-50, A-51, A-53	MANEGE	238	202									0.98	1.43														
														11.73													
A-54	PARADE	202	200			0.36						0.45	0.45	20.14	5	69.95	1,936.2	1,936.2	1.372	1350	CONC	0.60	85.0	4,311.6	2.75	0.52	45%
A-55	PARADE	202	200									0.61	27.68														
														20.65													
A-57	STALLION	232	230									0.53	0.53	10.00	5	104.19	55.6	55.6	0.457	450	CONC	1.75	73.3	393.2	2.75	0.44	14%
A-58	STALLION	230	228		0.33							0.37	0.37	10.44	5	101.90	154.1	154.1	0.610	600	CONC	0.50	82.0	452.7	1.55	0.88	34%
A-59	STALLION	230	228									0.61	1.51														
A-60	STALLION	228	226		0.07							0.07	0.07	11.33	5	97.68	208.8	208.8	0.686	675	CONC	0.50	76.9	619.7	1.68	0.76	34%
A-61	STALLION	228	226									0.55	2.14														
A-62	STALLION	224	222									0.66	2.79	12.09	5	94.32	263.5	263.5	0.762	750	CONC	0.50	63.6	820.8	1.80	0.59	32%
A-63	STALLION	222	200									0.45	3.25	12.68	5	91.89	298.2	298.2	0.762	750	CONC	0.50	59.7	820.8	1.80	0.55	36%
														13.23													
A-64	PARADE	200	152			0.30						0.38	0.38														
A-65	PARADE	200	152			0.28						0.34	0.34	20.65	5	68.85	2,220.2	2,220.2	1.372	1350	CONC	0.50	79.0	3,935.9	2.66	0.49	56%
A-66	PARADE	200	152									0.60	32.25														
														21.15													
A-67, A-68	BECKETT	150	148									1.05	1.05	10.00	5	104.19	109.9	109.9	0.457	450	CONC	0.40	111.8	188.0	1.14	1.63	58%
A-67, A-68	BECKETT	148	152									0.00	1.05	11.63	5	96.32	101.6	101.6	0.457	450	CONC	0.40	12.0	188.0	2.75	0.07	54%
														11.70													
A-69	PARADE	152	138									0.54	33.84	21.15	5	67.83	2,295.4	2,295.4	1.372	1350	CONC	0.50	85.0	3,935.9	2.75	0.52	58%
														21.66													
A-70	BECKETT	150	146									0.00	0.00	10.00	5	104.19	0.0	0.0	0.381	375	PVC	0.30	12.1	100.1	0.88	0.23	0%
A-70	BECKETT	146	144									0.57	0.57	10.23	5	103.00	59.1	59.1	0.457	450	CONC	0.30	68.0	162.8	2.75	0.41	36%
A-70	BECKETT	144	142									0.00	0.57	10.64	5	100.92	57.9	57.9	0.533	525	CONC	0.50	12.3	317.0	1.42	0.14	18%
A-71	BECKETT	142	140			0.37						0.46	0.46	10.79	5	100.22	203.8	203.8	0.610	600	CONC	0.50	111.4	452.7	2.75	0.68	45%
A-72, A-73	BECKETT	142	140									1.00	2.03														
A-72, A-73	BECKETT	140	138									0.00	2.03	11.46	5	97.06	197.4	197.4	0.610	600	CONC	0.50	11.9	452.7	1.55	0.13	44%
														11.59													

LOCATION				AREA (ha)									FLOW					PROPOSED SEWER											
Area	Street	FROM MH	TO MH	R=	R=	R=	R=	R=	R=	R=	R=	R=	INDIV. 2.78 AC	ACCUMUL. 2.78 AC	TIME OF CONC.	DESIGN STORM	RAINFALL INTENSITY	Peak Flow (L/sec)		DIA. ACTUAL (mm)	DIA. (mm)	TYPE	SLOPE (%)	LENGTH (m)	CAPACITY (L/s)	VELOCITY (m/s)	FLOW TIME (min)	Ratio (Q/Qfull)	
				0.20	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75						Q 5YR	Q total										
A-74	PARADE	138	136			0.27							0.34	0.34															
A-76	PARADE	138	136			0.03							0.04	0.04	21.66	5	66.80	2,489.1	2,489.1	1.372	1350	CONC	0.50	72.2	3,935.9	2.66	0.45	63%	
A-75, A-77	PARADE	138	136										1.02	37.26															
A-75, A-77	PARADE	136	134										0.00	37.26	22.12	5	65.93	2,456.6	2,456.6	1.372	1350	CONC	0.50	12.6	3,935.9	2.75	0.08	62%	
A-78	PARADE	134	132			0.39							0.49	0.49	22.19	5	65.78	2,523.7	2,523.7	1.372	1350	CONC	0.50	59.2	3,935.9	2.66	0.37	64%	
A-79	PARADE	134	132										0.62	38.37															
A-80	PARADE	132	130										0.68	39.05	22.56	5	65.09	2,541.7	2,541.7	1.524	1500	CONC	0.50	62.7	5,212.9	2.75	0.38	49%	
A-81	PARADE	130	128		1.06								1.18	40.23	22.94	5	64.39	2,590.7	2,590.7	1.524	1500	CONC	0.50	13.6	5,212.9	2.75	0.08	50%	
A-82	PARADE	128	126			0.33							0.41	0.41															
A-83	PARADE	128	126			0.17							0.22	0.22	23.02	5	64.24	2,663.1	2,663.1	1.524	1500	CONC	0.50	72.2	5,212.9	2.86	0.42	51%	
A-84	PARADE	128	126										0.305	41.45															
															23.45														
A-85	HICKSTEAD	116	114			0.16							0.20	0.20	10.00	5	104.19	82.4	82.4	0.381	375	PVC	0.60	61.7	141.6	1.24	0.83	58%	
A-86	HICKSTEAD	116	114										0.59	0.79															
A-87	HICKSTEAD	114	126										0.68	1.47	10.83	5	100.01	147.4	147.4	0.457	450	CONC	0.80	71.4	265.8	1.62	0.73	55%	
															11.56														
A-88	PARADE	126	124				0.40						0.55	0.55															
A-89	PARADE	126	124										0.36	43.84	23.45	5	63.50	2,783.8	2,783.8	1.524	1500	CONC	0.30	82.0	4,037.9	2.21	0.62	69%	
															24.06														
A-90	CAVALLO	110	112										0.48	0.48	10.00	5	104.19	50.1	50.1	0.381	375	PVC	1.90	70.9	251.9	2.21	0.53	20%	
A-91	CAVALLO	112	124										0.65	1.13	10.53	5	101.45	114.7	114.7	0.533	525	CONC	0.45	71.0	300.8	2.75	0.43	38%	
															10.97														
A-92	PARADE	124	122				0.40						0.55	0.55	24.06	5	62.43	2,893.8	2,893.8	1.524	1500	CONC	0.30	73.4	4,037.9	2.21	0.55	72%	
A-93	PARADE	124	122										0.82	46.35															
															24.62														
A-94	HICKSTEAD	118	110			0.27							0.33	0.33	10.00	5	104.19	80.4	80.4	0.381	375	PVC	1.35	74.2	212.4	1.86	0.66	38%	
A-95	HICKSTEAD	118	110										0.44	0.77															
A-96	HICKSTEAD	110	104										0.55	1.32	10.66	5	100.81	132.9	132.9	0.381	375	PVC	1.35	83.0	212.4	1.86	0.74	63%	
															11.41														
A-97	HICKSTEAD	108	106			0.09							0.11	0.11	10.00	5	104.19	63.2	63.2	0.457	450	PVC	0.25	78.2	148.6	0.91	1.44	43%	
A-98	HICKSTEAD	108	106										0.50	0.61															
A-99	HICKSTEAD	106	104			0.16							0.20	0.20															
A-100	HICKSTEAD	106	104			0.32							0.40	0.40	11.44	5	97.16	202.0	202.0	0.762	750	CONC	0.20	90.2	519.1	1.14	1.32	39%	
A-101, A-102	HICKSTEAD	106	104										0.45	2.08															
															12.76														
A-103	PARADE	104	102										0.90	4.30	12.76	5	91.57	393.5	393.5	0.762	750	CONC	0.35	120.0	686.7	1.51	1.33	57%	
A-103	PARADE	102	100										0.00	4.30	14.09	5	86.62	372.3	372.3	0.762	750	CONC	0.35	14.9	686.7	1.51	0.16	54%	
A-103	PARADE	122	100										0.00	50.65	24.62	5	61.52	3,115.6	3,115.6	1.524	1500	CONC	0.30	23.1	4,037.9	2.21	0.17	77%	
															24.79														

DESIGN PARAMETERS

Definitions:
 Q = 2.78 AIR, where
 Q= Peak Flow in Litres per Second (l/s)
 A= Area in hectares (ha)
 I= Rainfall Intensity (mm/lr)
 R= Runoff Coefficient

Notes:
 1) Ottawa Rainfall-Intensity Curve
 2) Min Pipe Velocity = 0.80 m/s
 3) Tc =10 min (subdivision)

**Stittsville Area 6 - Regional and Cavanagh Lands
 STORM SEWER DESIGN SHEET**

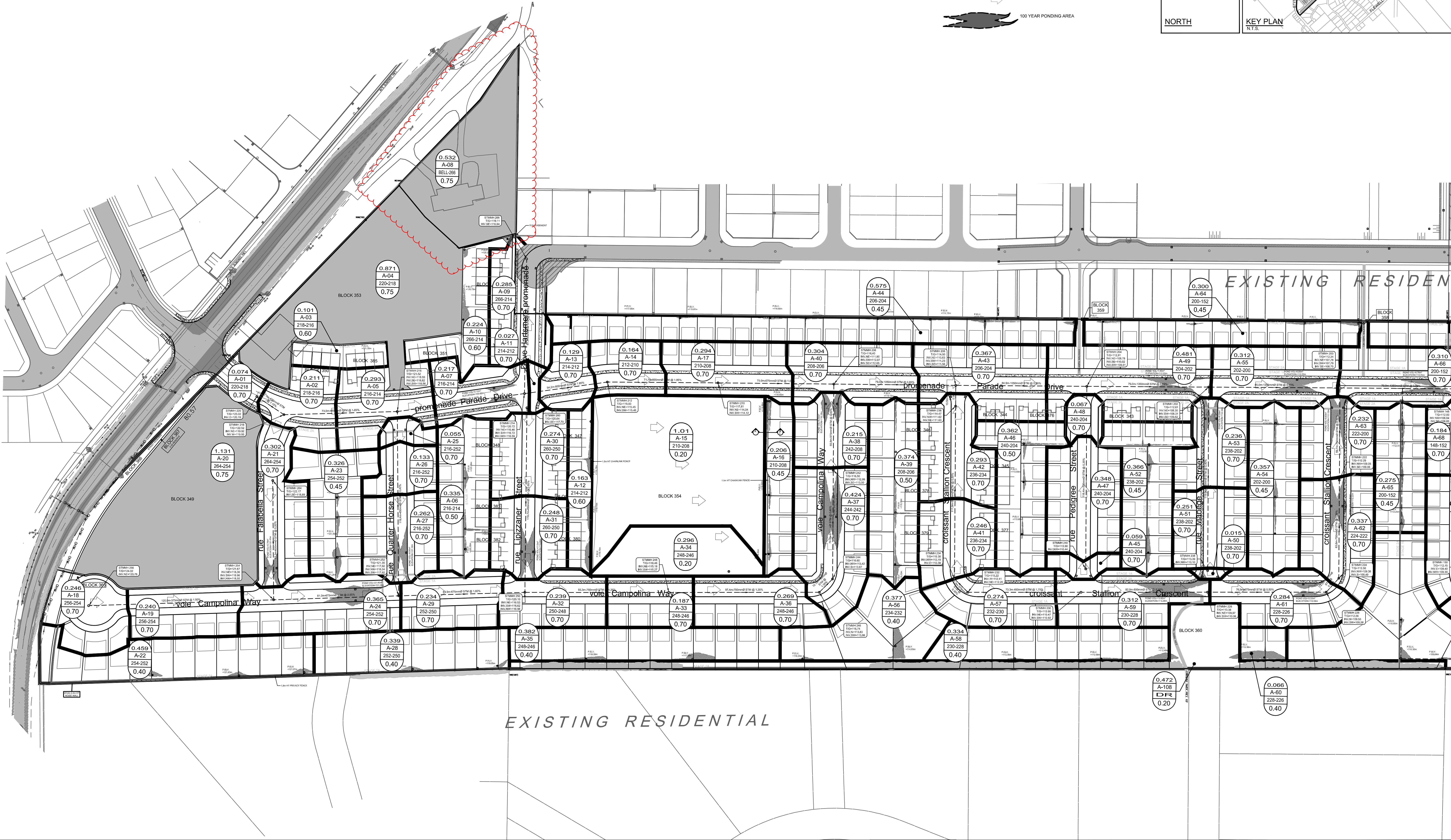
Date	April 5, 2016		
Design	I.P.	B.C.S	
Job No.	113004		Checked and Stamped:
	113004		BHB
Dwg. Reference:	113004-STM		



LEGEND

- 1.02 DRAINAGE AREA (hectares)
- A-35 DRAINAGE AREA NUMBER
- Creek MANHOLE TO MANHOLE
- 0.20 RUN-OFF COEFFICIENT
- STORM DRAINAGE AREA
- MAJOR OVERLAND FLOW
- WEST WIND SUBDIVISION OVERLAND FLOW
- 100 YEAR PONDING AREA

KEY PLAN
N.T.S.



NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

REVIEWED BY DEVELOPMENT REVIEW BRANCH
Signed: _____ 2016
Date: _____
Plan Number: D07-16-13-0033

No.	REVISION	DATE	BY
5.	REVISED AS PER CITY COMMENTS	JULY 18/16	BHB
4.	REVISED AS PER CITY COMMENTS	APR 05/16	BHB
3.	ISSUED FOR TENDER	JAN 20/16	BHB
2.	REVISED AS PER CITY COMMENTS	DEC 16/15	BHB
1.	ISSUED FOR CITY REVIEW	SEPT 14/15	BHB

SCALE

1:1000

0 10 20 30 40

FOR REVIEW ONLY

PERSON: BCS/TGP
CHECKED: BHB
DRAWN: BCS/TGP
CHECKED: BHB
APPROVED: GJM

NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowland Drive
Ottawa, Ontario, Canada K2M 3P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

CITY OF OTTAWA
STITTSVILLE SOUTH - AREA 6

STORM DRAINAGE AREA PLAN

PROJECT No.: 113004-00
REV # 5
DRAWING No.: 113004-STM1

D:\02\113004-STM1\113004-STM1.dwg C:\Users\CAVANGH\JUL 19 2016 10:10:00am

**STORM SEWER DESIGN SHEET
1835 STITTSVILLE MAIN STREET, STITTSVILLE**

LOCATION				AREA (ha)		INDIV. 2.78AR	ACCUM. 2.78AR	TIME OF CONC. (min)	5 YR RAINFALL INTENSITY (mm/hr)	5 YR PEAK FLOW (L/s)	PROPOSED SEWER						
DRAINAGE AREA	STREET NAME	FROM MH	TO MH	TOTAL AREA	C						PIPE DIA. (mm)	GRADE (%)	LENGTH (m)	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	TIME OF FLOW (min)	5 YEAR PERCENT FULL
TO HARTSMERE DR. STORM SEWER																	
PROPOSED	EX. EASEMENT	EX CB	EX 266	0.53	0.35	0.52	0.52	15.00	83.56	43.25	366.42	1.29	17.8	187.41	1.78	0.17	23%
DEMO	EX. EASEMENT	EX CB	EX 266	0.53	0.32	0.47	0.47	15.00	83.56	39.55	366.42	1.29	17.8	187.41	1.78	0.17	21%

Design Parameters

Notes:

- Rainfall intensity calculated using City of Ottawa IDF curve equations.
- Peak flows calculated using the Rational Method.
 $Q = 2.78CIA$, where:
 Q = Peak Flow (L/s)
 A = Drainage Area (ha)
 I = Rainfall Intensity (mm/hr)
 C = Runoff Coefficient
- Manning's roughness coefficient = 0.013
- Full flow velocity: MIN 0.8 m/s; MAX 3.0 m/s (City of Ottawa Sewer Design Guidelines, v.2012)

IDF curve equations (Intensity in mm/hr)

100 year Intensity = $1735.688 / (\text{Time in min} + 6.014)^{0.820}$
 50 year Intensity = $1569.580 / (\text{Time in min} + 6.014)^{0.820}$
 25 year Intensity = $1402.884 / (\text{Time in min} + 6.018)^{0.819}$
 10 year Intensity = $1174.184 / (\text{Time in min} + 6.014)^{0.816}$
 5 year Intensity = $998.071 / (\text{Time in min} + 6.053)^{0.814}$
 2 year Intensity = $732.951 / (\text{Time in min} + 6.199)^{0.810}$

Robinson
Land Development