SERVICING & STORMWATER MANAGEMENT REPORT 68 SWEETLAND AVENUE, OTTAWA



Building Perspective Prepared by Woodman Architect & Associates Ltd.

Project No.: CCO-22-5087

City File No.: D07-12-22-0106

Prepared for:

Smart Living Properties 226 Argyle Avenue Ottawa, Ontario K2P 1B9

Prepared by:

McIntosh Perry Consulting Engineers Ltd. 115 Walgreen Road Carp, ON K0A 1L0

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1.0 PROJECT DESCRIPTION

1.1 Purpose

McIntosh Perry (MP) has been retained by Smart Living Properties to prepare this Servicing and Stormwater Management Report in support of the Ste Plan Control for the proposed development located at 68 Sweetland Avenue within the City of Ottawa.

The main purpose of this report is to present a servicing and stormwater management design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary and storm sewer servicing for the development, ensuring that existing and available services will adequately service the proposed development.

This report should be read in conjunction with the following drawings:

- CCO-22-5087, C101 Grading, Drainage, Erosion & Sediment Control Plan
- COO-22-5087, C102 Ste Servicing Plan
- COO-22-5087, PRE Pre-Development Drainage Area Plan (Appendix E)
- CCO-22-5087, POST Post-Development Drainage Area Plan (Appendix F)

1.2 Site Description



Figure 1: Ste Map

The subject property, herein referred to as the Ste, is located at 68 Sweetland Avenue within the Pideau-Vanier Ward. The site covers approximately 0.22 ha and is located along Sweetland Avenue

between Osgoode Street and Somerset Street East. The site is zoned for Residential Fourth Density (R4UD [480]). See Site Location Plan in Appendix A for reference.

1.3 Proposed Development and Statistics

The proposed development consists of a 3 ½-storey residential building addition to the existing 2 ½-storey residential building. The building addition will add an additional 33 units to the existing 9 units. The gravel drive aisle is proposed to be replaced with pavers as part of the development. Development is proposed within 0.041 ha of the site. Refer to Ste Plan prepared by Woodman Architect & Associates Ltd. in Appendix B for further details.

1.4 Existing Conditions and Infrastructures

The site currently contains a 21/2-storey detached residential building containing 9 units and a gravel drive aisle along the northern property line. The site currently slopes from the west property line towards Sweetland Avenue.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal rights-of-way(s):

Sweetland Avenue

- o 203 mm diameter ductile iron watermain,
- 250 mm diameter concrete sanitary sewer tributary to the Somerset Street Combined Trunk Sewer, and
- A 450 mm diameter concrete storm sewer tributary to the Somerset Street Combined Trunk Sewer approximately 0.16 km downstream.

1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (ECA) through the Ministry of Environment, Conservation and Parks (MECP) is not required for the development since the development is located on one parcel of land, does not propose industrial usage, and is not located within a combined sewershed. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90.

2.0 BACKROUND STUDIES, STANDARDS, AND REFERENCES

2.1 Background Reports / Reference Information

As-built drawings of existing services, provided by the City of Ottawa Information centre, within the vicinity of the proposed site were reviewed in order to identify infrastructure available to service the proposed development.

A topographic survey (2104105) of the site was completed by Annis, O'Sullivan, Vollebekk Ltd.

The Site Plan (SP01) was prepared by Woodman Architect & Associates Ltd. and revised October 12th, 2022 (Site Plan).

2.2 Applicable Guidelines and Standards

Oty of Ottawa:

- ♦ Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (Ottawa Sewer Guidelines)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (ISTB-2014-01)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (PIEDTB-2016-01)
 - Technical Bulletin ISTB-2018-01 Oty of Ottawa, January 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (ISTB-2018-03)
 - Technical Bulletin ISTB-2019-01 Oty of Ottawa, January 2019. (ISTB-2019-01)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (ISTB-2019-02)
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010. (Ottawa Water Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 2018. (ISTB-2018-02)

Ministry of Environment, Conservation and Parks:

- ◆ Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (MECP Stormwater Design Manual)
- ◆ Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (MECP Sewer Design Guidelines)

Other:

Water Supply for Public Fire Protection, Fire Underwriters Survey, 2020. (FUS Guidelines)

3.0 PRE-CONSULTATION SUMMARY

An informal pre-consultation meeting was held with City staff in October 2021 regarding the proposed site servicing. Specific design parameters to be incorporated within this design include the following:

- Pre-development and post-development flows shall be calculated using a time of concentration (Tc) no less than 10 minutes.
- Control 5 through 100-year post-development flows to the 2-year pre-development flow.

4.0 WATERMAIN

4.1 Existing Watermain

There is an existing 203 mm diameter DI located watermain within Sweetland Avenue. The site is located within the 1W pressure zone, as per the Water Distribution System mapping included in Appendix C. There are three municipal fire hydrants along Sweetland Avenue that are available to service the development.

4.2 Proposed Watermain

It is proposed to service the proposed building addition through the existing building. A mechanical consultant will need to review and confirm whether upgrades to the existing building are required to accommodate the addition.

Table 1, below, summarizes the water supply design criteria obtained from the Ottawa Water Guidelines and utilized for the water analysis.

Ste Area0.06 haPesidential280 L/day/personPesidential Apartment – 1 Bedroom1.4 person/unitMax Day Peaking Factor - Pesidential9.5 x avg. dayPeak Hour Peaking Factor - Pesidential14.3 x avg. day

Table 1: Water Supply Design Criteria

The water analysis results have been summarized in Table 2, below. The fire flow demand accounted for both the existing above-ground floor area and the proposed area.

Table 2: Summary of Estimated Water Demand

Design Parameter	Total Flow (L/s) Proposed
Average Daily Demand	0.19
Max Day Demand	1.82
Max Day Demand + FUS(133.33 L/s)	135.15
Max Day Demand + OBC (45L/s)	46.82
Peak Hour Demand	2.73

The Fire Underwriters Survey 2020 (FUS) method was utilized to estimate the required fire flow for the site. The following parameters were coordinated with the architect:

- Type of construction Wood Frame Construction
- Occupancy Type Limited Combustible
- Sprinkler Protection Standard Water Supply Sprinklered

The results of the calculations yielded a required fire flow of 8,000 L/min (133.33 L/s) using the FUS method and 2,700 L/min (45 L/s) using the OBC method. The detailed calculations for the FUS and OBC can be found in Appendix C.

The City provided the estimated water pressures for the average day scenario, peak hour scenario and the max day plus fire flow scenario for the demands indicated by the correspondence in Appendix C. The resulting pressures for the boundary conditions results are shown in Table 3, below.

Table 3: Boundary	Conditions	Results

Scenario	Proposed Demands (L/s)	Connection 1 HGL(m H₂O)*/kPa			
Average Day Demand	0.19	50.4 / 494.7			
Maximum Daily + Fire How Demand	46.82	43.8 / 430.0			
Peak Hourly Demand	41.0 / 402.5				
* Adjusted for an estimated ground elevation of 67.3m above the connection point.					

The normal operating pressure range is anticipated to be 402.5 kPa to 494.7 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi). The proposed watermains will meet the minimum required 20 psi (140 kPa) from the Ottawa Water Guidelines at the ground level under maximum day demand and fire flow conditions.

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were analysed per City of Ottawa ISTB 2018-02 Appendix I Table 1. Based on City guidelines (ISTB-2018-02), the existing hydrants can provide adequate fire protection to the proposed development. The results are summarized below.

Table 4: Fire Protection Confirmation

Building	Fire Flow Demand (L/ min.)	Fire Hydrant(s) within 75m	Fire Hydrant(s) within 150m	Combined Fire Flow (L/min.)
68 Sweetland Ave	8,000 (FUS) 2,700 (OBC)	2 public	1 public	15,200

5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

The existing building is serviced by a 150 mm diameter sanitary lateral connected to the existing 250 mm diameter sanitary sewer within Sweetland Avenue, fronting the subject site. The subject site currently contributes wastewater to the Sweetland Avenue sewer system, tributary to the Somerset Street Combined Trunk Sewer.

5.2 Proposed Sanitary Sewer

It is proposed to service the proposed building addition through the existing building. A mechanical consultant will need to review and confirm whether upgrades to the existing building are required to accommodate the addition.

Table 5, below, summarizes the wastewater design criteria identified by the Ottawa Sewer Guidelines.

Table 5: Sanitary Design Criteria

Design Parameter	Value
Ste Area	0.06 ha
Residential	280 L/ person/ day
1 Bedroom Apartment	1.4 persons/unit
Residential Peaking Factor	3.64
Extraneous Row Allowance	0.33 L/s/ha

Table 6, below, summarizes the estimated wastewater flow from the proposed development. Refer to Appendix D for detailed calculations.

Table 6: Summary of Estimated Sanitary Flow

Design Parameter	Total How (L/s)
Total Estimated Average Dry Weather Flow	0.19
Total Estimated Peak Dry Weather Flow	0.70
Total Estimated Peak Wet Weather Flow	0.72

The full flowing capacity of a 150 mm diameter service at 1.0% slope is estimated to be 15.89 L/s. Per Table 5, a peak wet weather flow of 0.72 L/s will be conveyed within the 150 mm diameter service, therefore the existing system is sufficiently sized for the development. Due to the complexity of the downstream network the City will need to advise of any downstream constraints.

6.0 STORM SEWER DESIGN

6.1 Existing Storm Sewers

Stormwater runoff from the site is currently tributary to the Somerset Street Combined Trunk Sewer. There is an existing 450 mm diameter storm sewer within Sweetland Avenue available to service the site. The existing sewer is tributary to the Somerset Street Combined Trunk Sewer approximately 0.16km downstream.

6.2 Proposed Storm Sewers

Foundation drainage for the proposed building is proposed to be conveyed via a 150 mm diameter connection to the proposed 300 mm diameter storm service.

Runoff from the rear yard will be directed towards landscaping catchbasins and one standard catchbasin. Runoff will then be conveyed through the proposed 250-300 mm subdrain and 300 mm diameter PVC storm service to the existing 450 mm diameter storm sewer within Sweetland Avenue. It is proposed to connect to the existing 450 mm diameter storm sewer with a 1200 mm diameter maintenance hole.

Runoff collected on the roof of the proposed building addition will be stored and controlled internally using two roof drains. The roof drains will be used to limit the flow from the roof to the specified allowable release rate. For calculation purposes a Watts Accutrol roof drain in the closed position was used estimate a reasonable roof flow. Other products maybe specified at detailed building design so long as release rates and storage volumes are respected. Funoff from the roof will be directed to the proposed 300 mm diameter storm service.

See CCO-22-5087 - POST include in Appendix F of this report for more details. The Stormwater Management design for the subject property will be outlined in Section 7.0 of this report.

7.0 PROPOSED STORM WATER MANAGEMENT

7.1 Design Criteria and Methodology

The following design criteria have been employed in developing the stormwater management design for the site as directed by the City and the RVCA:

Quality Control

 Quality controls are not required for this site because the site is tributary to a combined sewershed.

Quantity Control

- Any storm events greater than 2-year, up to 100-year, and including 100-year storm event must be detained on site.
- Post-development to be restricted to the 2-year storm event, based on a calculated time
 of concentration greater than 10 minutes. Refer to Section 7.2 for further details.

7.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

Q = 2.78CIA (L/s)

Where: C = Runoff coefficient

= Rainfall intensity in mm/hr (City of Ottawa IDF curves)

A = Drainage area in hectares

It is recognized that the Pational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended. The following coefficients were used to develop an average Cfor each area:

Roofs/ Concrete/ Asphalt	0.90
Undeveloped and Grass	0.20

As per the City of Ottawa - Sewer Design Guidelines, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

7.3 Pre-Development Drainage

It has been assumed that the site contains no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 2, 5, and 100-year events are summarized below in Table 7. See CCO-22-5087 - PRE in Appendix E and Appendix G for calculations.

Drainaga	Area (ha)	Q (L/s)			
Drainage Area		2-Year	5-Year	100-Year	
A1	0.041	5.27	7.15	15.30	
Total (Dev Area)	0.041	5.27	7.15	15.30	
A2	0.127	21.55	29.23	57.27	
A3	0.027	4.57	6.20	12.16	
A4	0.020	2.84	3.86	7.58	
Total (Ex. Drainage)	0.175	28.97	39.30	77.00	

Table 7: Pre-Development Runoff Summary

Development Area

Ex. Sweetland Drainage

Ex. Osgoode Drainage

Ex. Nelson Drainage

Area A1 encompasses the proposed development area. Areas A2, A3, and A4 include existing drainage towards the Sweetland Avenue, Osgoode Street, and Nelson Street ROW's, respectively.

7.4 Post-Development Drainage

To meet the stormwater objectives the development will contain rooftop control.

Based on the criteria listed in Section 7.2, the development will be required to restrict flow to the 2-year storm event. It is estimated that the target release rate during the 100-year event will be 5.27 L/s.

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See COO-22-5087 - POST in Appendix F of this report for more details. A summary of the post-development runoff calculations can be found below.

Drainage Area	Area (ha)	5-year Peak How (L/s)	100-year Peak How (L/s)	100-year Storage Required (m³)	100-year Storage Available (m³)
B1	0.023	2.52	5.03		
B2	0.003	0.27	0.56		
B3	0.015	0.63	0.63	5.90	6.09
Total (Dev Area)	0.041	3.42	6.22	5.90	6.09
B4	0.072	15.91	30.48		
B5	0.056	13.58	25.93		
B6	0.027	4.59	8.90		
B7	0.020	4.80	9.17		
Total (Ex. Drainage)	0.175	38.88	74.48		

Table 8: Post-Development Runoff Summary

7.4.1 Development Area Drainage (B1-B3)

Areas B1 to B3 include collected runoff, uncollected runoff, and roof runoff within the development area, respectively.

Runoff for area B1 will be collected by a proposed rear yard catch basin system and directed to the proposed 300 mm diameter storm service. Runoff for area B2 will surface drain towards the Sweetland Avenue ROW. Runoff for area B3 will be stored on the roof of the proposed building addition and restricted using two Watts Accutrol roof drain (or equivalent product) to a maximum release rate of 0.63 L/s and will provide up to 6.09 m³ of storage. Roof flow will be directed to the proposed 300 mm diameter storm service.

The proposed 300 mm diameter storm service will discharge flow from areas B1 and B3 to the existing 450 mm diameter storm sewer within Sweetland Avenue.

Foundation drainage is proposed to be connected to the proposed 300 mm diameter storm service without flow attenuation. The internal servicing layout is to be reviewed by the mechanical engineer.

As seen in Table 8, above, post-development runoff will exceed the allowable release rate by 0.95 L/s during the 100-year event. Surface storage options were explored in order to meet SWM requirements by reducing the flow from area B1. Given the small flow rates from this area, it was determined that any ICD sized for these flows would not meet the minimum ICD dimensions provided in the Ottawa Sewer Design Guidelines.

7.4.2 Existing Drainage (B4-B7)

Runoff from areas B4 to B7 consists of existing drainage to the Sweetland Avenue ROW (B4-B5), drainage to the Osgoode Street ROW (B6), and drainage to the Nelson Street ROW (B7).

Runoff from Area B4 primarily consists of existing runoff from the sloped roofs. Area B4 will be collected by the proposed catchbasin system and directed to the proposed 300 mm diameter storm service. Runoff from Area B5 primarily consists of existing runoff from the sloped roofs, downstream of the catchbasin system. Area B5 will surface drain towards the Sweetland Avenue ROW. Runoff from areas B6 and B7 will surface drain towards the Osgoode Street and Nelson Street ROW's, respectively.

7.4.3 Sweetland Avenue ROW - How Breakdown

Table 9, below, summarizes post-development 100-year flows to the Sweetland Avenue ROW.

100-year Peak Area ID Area (ha) C(100-Year) How (L/s) B1 0.023 0.44 5.03 **B**3 0.015 1.00 0.63 В4 0.072 0.86 30.48 Total (Collected) 0.109 36.14 0.34 B2 0.003 0.56

Table 9: Post-Development Sweetland Avenue Hows

B5	0.056	0.94	25.93
Total (Surface Flow)	0.059	-	26.48

As seen in Table 9, above, flow within the proposed 300mm storm service will be $36.14 \, \text{L/s}$ during the 100-year event, while surface drainage to the Sweetland Avenue ROW will be $26.48 \, \text{L/s}$. Refer to Appendix Gfor detailed calculations.

8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all-natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Sit fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catch basins and filter fabric is to be placed under the grates of all existing catch basins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures are to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the Ste Grading, Drainage and Sediment & Erosion Control Plan for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

9.0 SUMMARY

- A 3 ½-storey residential rear addition is proposed to be constructed at 68 Sweetland Avenue. The Ste Plan proposes an additional 33 units to the existing 9 units, with street access from Nelson Street.
- The FUS method estimated fire flow indicated that 8,000 L/min is required for the proposed development.
- The OBC method estimated fire flow indicated that 2,700 L/min is required for the proposed development.
- The development is estimated to have a combined peak wet weather flow of 0.72 L/s.
- Based on City of Ottawa guidelines, the development will be required to attenuate post-development flows to the 2-year release rate of 5.27 L/s.
- To meet the stormwater objectives the development will contain 6 m³ of rooftop storage for flow attenuation; and
- Quality controls are not required as the site is tributary to a combined sewershed.

10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management report in support of the proposed development at 68 Sweetland Avenue.

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



Charissa Hampel, P.Eng.
Project Engineer, Land Development
T: 613.714.4625
E: c.hampel@mcintoshperry.com

Francis Valent

Francis J. Valenti, ET.
Engineering Intern, Land Development
T: 613.808.2123
E: f.valenti@mcintoshperry.com

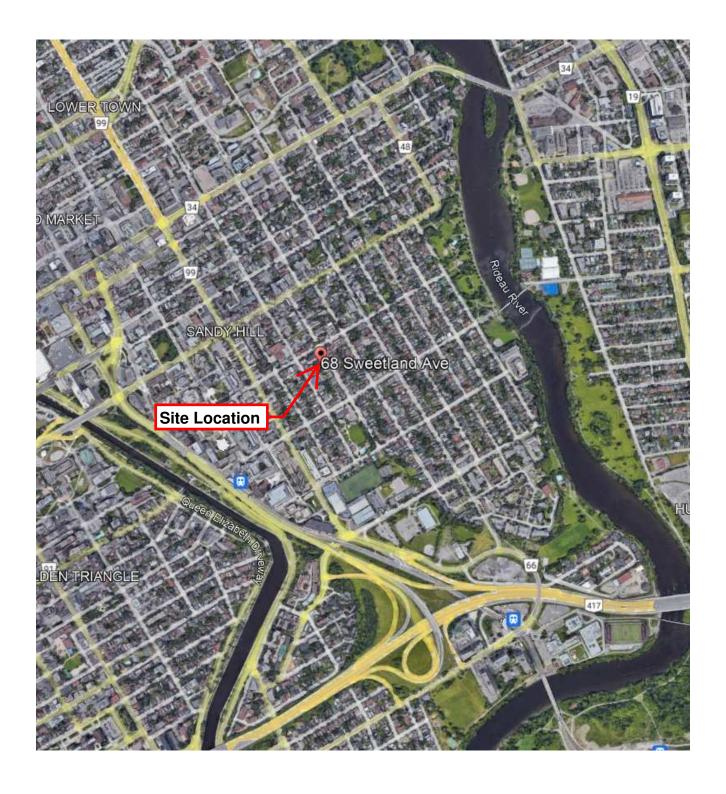
11.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of <u>Smart Living Properties</u>. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Parks and Climate Change, City of Ottawa and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.

Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

APPENDIX A KEY PLAN



CLIENT: Smart Living Properties						
PROJECT: 68 Sweetland Avenue						
TITLE: SITE LO	TITLE: SITE LOCATION					
MCINTOSH PERRY 115 Walgreen Road, RR3, Carp, ON KOA 1LO	PROJECT: CCO-22-5087	FIGURE:				
Tel: 613-836-2184 Fax: 613-836-3742 www.mcintoshperry.com	DATE: MAY. 26, 2022	SCALE: N.T.S				

APPENDIX B BACKGROUND DOCUMENTS

 From:
 Nadeau, Jeff

 To:
 Haris Khan

 Cc:
 Lisa Dalla Rosa

Subject: Informal Pre-con Follow-up - 68 Sweetland and 146 Osgoode

Date: March 31, 2022 7:17:04 PM

Attachments: 146 Osgoode & 68 Sweetland Study and Plan Identification List.pdf

CAUTION: This email is from an external sender. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Hello Haris,

Please refer to the below notes regarding review and informal discussion of the development proposal for 68 Sweetland Avenue and 146 Osgoode Avenue, as initially presented in October 2021 and as submitted in revised form in January and February 2022, to allow for the expansion of the rooming house at 68 Sweetland and for the adjustment of the lot line shared by 68 Sweetland and 146 Osgoode.

I have also attached the required Plans & Study List for application submission.

Below are staff's preliminary comments based on the information available:

Planning

- Current Official Plan has this site in the General Urban Area (3.6.1). New Official Plan has this site in a Neighbourhood (6.3) within the Downtown Core transect area (5.1).
 - Higher-density, urban development forms with minimal on-site parking are encouraged.
- The site is zoned R4UD[480].
 - It is understood that the two non-rooming units in the building were permitted through a minor variance in 1991.
 - The lot line adjustment will result in 68 Sweetland having a depth of 51.7m and a lot area of approx. 416.5m². Per s.144(3) a rear yard setback of 30% of lot depth (15.5m) and a rear yard area of 25% of lot area (104m²) is required; these are being met.
 - The landscaping requirements introduced via the R4 revisions, namely s.161(15), do not apply to rooming houses. The amenity requirements under s.137 do apply to rooming houses. The division of units and amenity spaces between 146 Osgoode and 68 Sweetland is not clear, but it is evident that the lands as a whole will fall well short of the s.137(1) requirement of 489m² amenity space for 151 units.
 - Further to the above there is a 100m² green strip indicated over the eastern half of the laneway. Is this to be counted as amenity area? It will be incumbent on the applicant to demonstrate how a space this narrow can realistically serve as an amenity space.
 - 36 bike-parking stalls are noted. This is slightly short of the 38-stall

- requirement under s.111 (rooming house: 0.25 per unit). Staff encourage maximizing the quantity and quality of bike storage on-site.
- Meeting waste management provisions in s.134 will be a high priority.
- Ocuncillor Fleury's preference for structurally-integrated waste areas is well-established. Earth Bins were considered as an option for retaining open space that would otherwise be consumed by a waste shed. The assumption had been that these would have a minimal elevation above grade, however product details have made clear that Earth Bins have a notable above-grade presence. Having a large cluster of these in the rear yard cancels out any "open-space-preserving" advantage of this approach. Given that distributing them across the site(s) was noted to be logistically difficult and intrusive on the ROW, these may not work as a solution in this case.
- The project stats noted on the February 17 plans indicate 118 existing rooming units, increasing to 151 through this proposal. It's assumed this count is inclusive of both 68 Sweetland and 146 Osgoode. Please break down unit counts per building in future submissions.
- O It is understood that the proposed applications will address the lands at 146 Osgoode and 68 Sweetland as a whole. "One Lot for Zoning Purposes" (s.93) does not formally apply to residential lots like this; as such treating these lands as one lot may be a component of your ZBLA request. This may bring us back into PUD territory. Staff expect further discussion of this approach will be required to iron out the details as your plans evolve.
- Recommend submitting for ZBLA and SPC in advance of lot line adjustment to work out initial high-level site design issues prior to making any consent application to the Committee of Adjustment.
- Recognizing that this concept is still in its early stages, these plans would benefit
 from more detailed information as the project matures. Please ensure that plans
 for any formal submissions are prepared in accordance with standards and
 guidelines available here: https://ottawa.ca/en/planning-development-and-construction/developing-property/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans.

Engineering

PLANS:

- Existing Conditions and Removals Plan
- Site Servicing Plan
- Grade Control and Drainage Plan
- Road Reinstatement Plan
- Erosion and Sediment Control Plan
- Roof Drainage Plan
- Topographical survey

REPORTS:

- Site Servicing and Stormwater Management Report
- Geotechnical Study/Investigation

Noise Control Study

- Phase I ESA
- Phase II ESA (Depending on recommendations of Phase I ESA)
- Please submit boundary condition request early in the process so we can confirm if the required fire flow is available
- Please control post-development runoff from the subject site, up to and including the 100-year storm event, to a 2-year pre-development level.

Feel free to contact the Infrastructure Project Manager, Nishant Jhamb, at nishant.ihamb@ottawa.ca, for follow-up questions.

City Surveyor

- The determination of property boundaries, minimum setbacks and other regulatory constraints are a critical component of development. An Ontario Land Surveyor (O.L.S.) needs to be consulted at the outset of a project to ensure properties are properly defined and can be used as the geospatial framework for the development.
- Topographic details may also be required for a project and should be either carried out by the O.L.S. that has provided the Legal Survey or done in consultation with the O.L.S. to ensure that the project is integrated to the appropriate control network.

Questions regarding the above requirements can be directed to the City's Surveyor, Bill Harper, at bill.harper@ottawa.ca.

Other

- Plans are to be standard A1 size (594 mm x 841 mm) sheets, utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400 or 1:500).
- o All PDF submitted documents are to be unlocked and flattened.

Please refer to the links to <u>Guide to preparing studies and plans</u> and <u>fees</u> for further information. Additional information is available related to <u>building permits</u>, <u>development charges</u>, <u>and the Accessibility Design Standards</u>. Be aware that other fees and permits may be required, outside of the development review process. You may obtain background drawings by contacting <u>informationcentre@ottawa.ca</u>.

These pre-con comments are valid for one year. If you submit a development application(s) after this time, you may be required to meet for another pre-consultation meeting and/or the submission requirements may change. You are as well encouraged to contact us for a follow-up meeting if the plan/concept will be further refined.

Please do not hesitate to contact me if you have any questions.

Regards,

Jeff Nadeau

Planning, Real Estate and Economic Development Department | Services de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West. Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 16802

ottawa.ca/planning / ottawa.ca/urbanisme

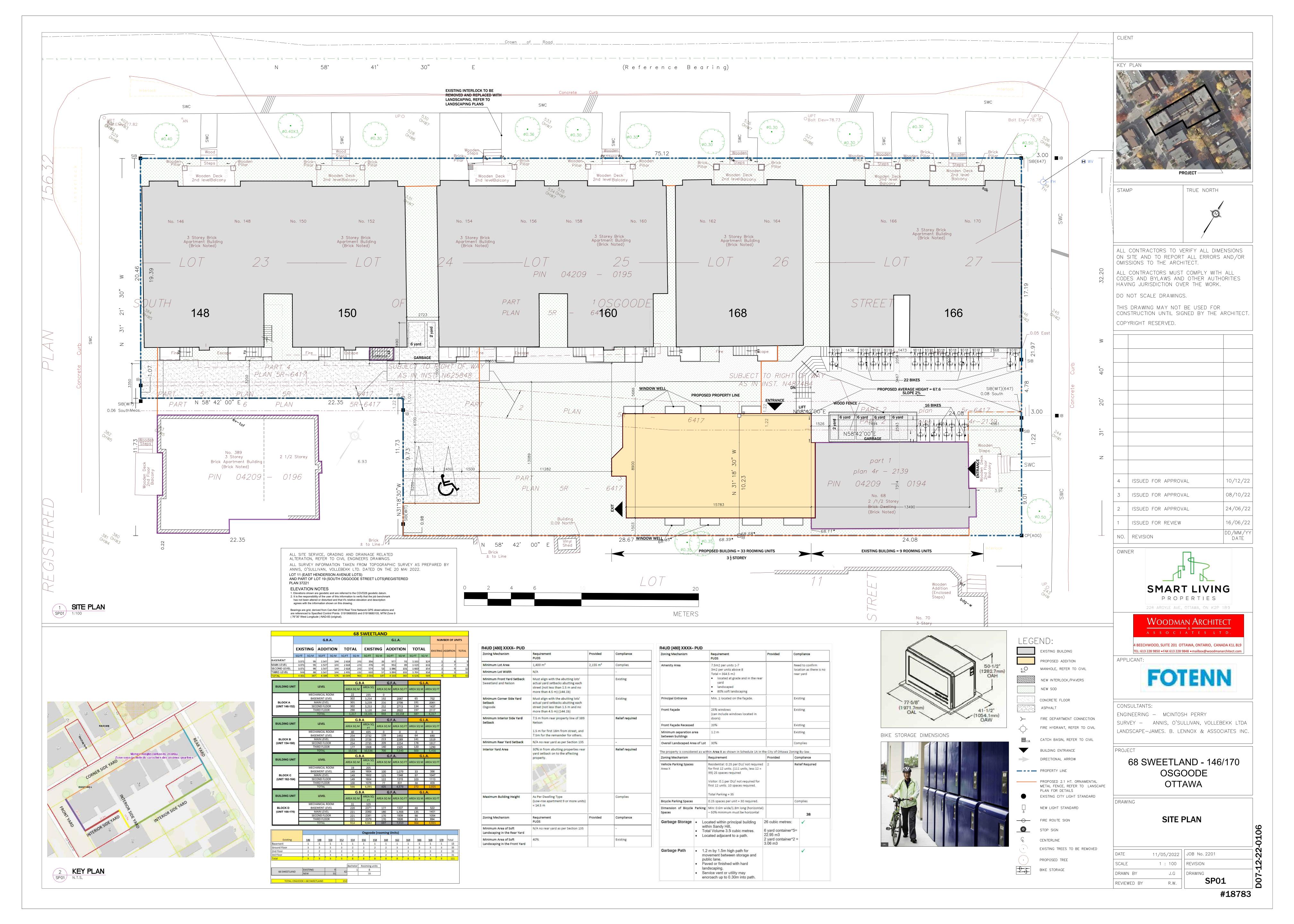
Please note that, while my work hours may be affected by the current situation, I have regular access to email and check telephone messages periodically. Email is currently the best way to contact me

•

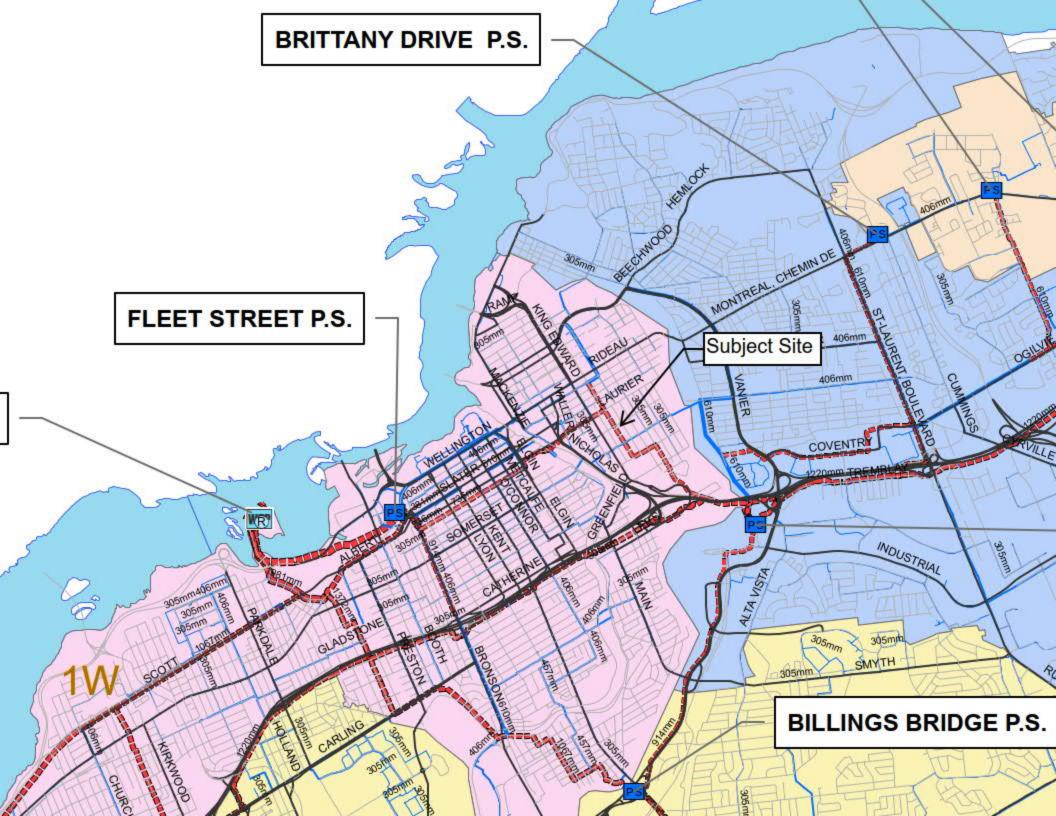
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APPENDIX C WATERWAIN CALCULATIONS



000-22-5087 - 68 Sweetland Avenue - Existing Water Demands

Project: 68 Sweetland Avenue

Project No.: COO-22-5087

Designed By: FV
Checked By: AG

Date: June 10, 2022

Ste Area: 0.02 gross ha

Residential NUMBER OF UNITS UNIT PATE

1 Bedroom Apartment 9 units 1.4 persons/unit
Total Population 13 persons

AVERAGE DAILY DEMAND

DEM AND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d]
Industrial - Light	35,000	L/ gross ha/d]
Industrial - Heavy	55,000	L/ gross ha/d	
Shopping Centres	2,500	L/(1000m²/d	
Hospital	900	L/ (bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/(Space/d)	
Motels	150	L/ (bed-space/d)	
Hotels	225	L/ (bed-space/d)	
Tourist Commercial	28,000	L/ gross ha/d	
Other Commercial	28,000	L/ gross ha/d	
	Residential	0.04	L/s
AVERAGE DAILY DEM AND	Commerical/Industrial/		
	Institutional	0.00	L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	P	MOUNT	UNITS	
Residential	9.5	x avg. day	L/c/d	
Industrial	1.5	x avg. day	L/gross ha/d	
Commercial	1.5	x avg. day	L/gross ha/d	
Institutional	Institutional 1.5 x avg. day		L/gross ha/d	
	Residential	0.40	L/s	
MAXIMUM DAILY DEMAND	Commerical/Industrial/			
	Institutional	0.00	L/s	

MAXIMUM HOUR DEMAND

DBMAND TYPE	A	MOUNT	UNITS	
Residential	14.3	x avg. day	L/c/d	
Industrial	1.8	x max. day	L/gross ha/d	
Commercial	1.8	x max. day	L/gross ha/d	
Institutional	1.8	x max. day	L/gross ha/d	
	Residential	0.60	L/s	
MAXIMUM HOUR DEMAND	Commerical/Industrial/			
	Institutional	0.00	L/s	

WATER DEMAND DESIGN FLOWS PER UNIT COUNT CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.04	L/s
MAXIMUM DAILY DEMAND	0.40	L/s
MAXIMUM HOUR DEMAND	0.60	L/s

000-22-5087 - 68 Sweetland Avenue - Proposed Water Demands

 Project:
 68 Sweetland Avenue

 Project No.:
 ∞0-22-5087

Designed By: FV
Checked By: AG

Date: June 10, 2022

Ste Area: 0.06 gross ha

<u>Pesidential</u> NUMBER OF UNITS UNIT RATE

 1 Bedroom Apartment (Existing)
 9 units
 1.4
 persons/unit

 1 Bedroom Apartment (Proposed)
 33 units
 1.4
 persons/unit

Total Population 59 persons

AVERAGE DAILY DEM AND

DBMAND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d	
Industrial - Light	35,000	L/gross ha/d	
Industrial - Heavy	55,000	L/gross ha/d	
Shopping Centres	2,500	L/ (1000m² /d	
Hospital	900	L/ (bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/ (Space/d)	
Motels	150	L/ (bed-space/d)	
Hotels	225	L/ (bed-space/d)	
Tourist Commercial	28,000	L/ gross ha/ d	
Other Commercial	28,000	L/ gross ha/ d	
	Residential	0.19	L/s
AVERAGE DAILY DEMAND	Commerical/Industrial		
	/Institutional	0.00	L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	P	UNITS	
Residential	9.5	x avg. day	L/c/d
Industrial	1.5	x avg. day	L/ gross ha/ d
Commercial	1.5	x avg. day	L/ gross ha/ d
Institutional	1.5	x avg. day	L/ gross ha/ d
	Residential	1.82	L/s
MAXIMUM DAILY DEMAND	Commerical/Industrial		
	/Institutional	0.00	L/s

MAXIMUM HOUR DEMAND

DBMAND TYPE	A	MOUNT	UNITS	
Residential	14.3	x avg. day	L/c/d	
Industrial	1.8	x max. day	L/ gross ha/ d	
Commercial	1.8	x max. day	L/gross ha/d	
Institutional	1.8	x max. day	L/ gross ha/ d	
	Residential	2.73	L/s	
MAXIMUM HOUR DEMAND	Commerical/Industrial			
	/Institutional	0.00	L/s	

WATER DEMAND DESIGN FLOWS PER UNIT COUNT

CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.19	L/s
MAXIMUM DAILY DEMAND	1.82	L/s
MAXIMUM HOUR DEMAND	2.73	L/s

000-22-5087 - 68 Sweetland Avenue - OBC Fire Calculations

Project: 68 Sweetland Avenue CCC-22-5087 Project No.: FV Designed By: Checked By: AG June 10, 2022 Date:

Ontario 2006 Building Code Compendium (Div. B - Part 3)

Water Supply for Fire-Fighting - Residential Building

Building is classified as Group: C-Residential Occupancies (from table 3.2.2.55) Building is of combustible construction. Floor assemblies are fire separations but with no fire-resistance ratings. Poof assemblies, mezzanies, loadbearing walls, columns and arches do not have a fire-resistance rating.

From Div. B A-3.2.5.7. of the Ontario Building Code - 3. Building On-Ste Water Supply:

(a) $Q = K \times V \times Stot$

where:

Q = minimum supply of water in litres

K = water supply coefficient from Table 1

V = total building volume in cubic metres

Stot = total of spatial coefficient values from the property line exposures on all sides as obtained from the formula:

Stot = 1.0 + [Sside1 + Sside2 + Sside3 + ... etc.]

K	23	(from Table 1 pg A-31) (Worst case occupancy {E/F2} 'K' value use	ed)			F	rom Figure
V	2,101	(Total building volume in m³.)					1 (A-32)
Stot	2.0	(From figure 1 pg A-32)	→ s	north	1.3	m	0.5
Q =	96,668.59	L	S	east :	3.88	m	0.5
			S	South	0.6	m	0.5
From Table 2: Required Minimum V	later Supply Flow	Rate (L/s)	S	west	16.62	m	0.0

* approximate distances

if Q < 108,000 L 2700 L/min

713 gpm

000-22-5087 - 68 Sweetland Avenue - Existing Conditions - Fire Underwriters Survey

Project: 68 Sweetland Avenue - Existing Conditions

 Project No.:
 COC-22-5087

 Designed By:
 FV

 Checked By:
 AG

Date: June 10, 2022

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Pequired Fire Flow Copyright I.SO.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Pounded to the nearest 1000 L/ min)

 $F = 220 \times C \times VA$ Where: F =Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in

the building being considered.

Construction Type Wood Frame

C 1.5 A 387.0 m²

Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 387.0 m² *Unprotected Vertical Openings

%Increase*

36%

Calculated Fire Flow 6,491.9 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:

Limited Combustible -15%

Fire Flow 5,100.0 1/min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered -40%

R	Reduction			-2,040.0 L/ min				
D. INCRE	EASE FOR EXPOSURE (No Rounding)							
	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor			
Exposure 1	3.1 to 10	Wood frame	16.4	3	49.2	17%		
Exposure 2	20.1 to 30	Wood frame	9.2	3	27.6	2%		
Exposure 3	3.1 to 10	Wood frame	15.2	3	45.6	17%		
Exposure 4	Over 30 m	Wood frame	N/A	N/A	N/A	0%		

Increase* 1,836.0 L/mir

E Total Fire How (Rounded to the Nearest 1000 L/ min)

Fire Flow 4,896.0 L/min
Fire Flow Required** 5,000.0 L/min

 $^{^{\}star}$ In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

 $^{^{**}}$ In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

000-22-5087 - 68 Sweetland Avenue - Proposed Development - Fire Underwriters Survey

Project: 68 Sweetland Avenue - Proposed Development

 Project No.:
 COO-22-5087

 Designed By:
 FV

 Checked By:
 AG

 Date:
 June 10, 2022

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

 $F = 220 \times C \times VA$ Where: F =Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade)

in the building being considered.

Construction Type Wood Frame

C 1.5 A 961.0 m^2

Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 961.0 m² *Unprotected Vertical Openings

 Calculated Fire Flow
 10,230.0 L/ min

 10,000.0 L/ min
 10,000.0 L/ min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:
Limited Combustible

ited Combustible -15%

Fire Flow 8,500.0 L/ min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered

Red	duction			-3,400.0 L	√min		
D. INORE	ASE FOR EXPOSURE (No Pounding)						
	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height L (Stories) F	Length-Height Factor		
Exposure 1	3.1 to 10	Wood frame	16.4	3	49.2	17%	

-40%

Wood frame Exposure 2 20.1 to 30 9.2 27.6 2% 3.1 to 10 Wood frame 15.2 3 45.6 17% Exposure 3 Exposure 4 20.1 to 30 Wood frame 8.9 26.7 2%

%Increase* 38%

Increase* 3,230.0 L/min

Hre How 8,330.0 U min

E Total Fire Flow (Rounded to the Nearest 1000 L/min)

^{*} In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

^{**} In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

000-22-5087 - 68 Sweetland Avenue - Boundary Condition Unit Conversion

Project: 68 Sweetland Avenue

Project No.: 000-22-5087

Designed By: FV
Checked By: AG

Date: June 10, 2022

Boundary Conditions Unit Conversion

Sweetland Avenue

Scenario	Height (m)	Elevation (m)	m H₂O	PSI	kPa
Avg. DD	115.4	65.0	50.4	71.8	494.7
Max Day + Fire Flow (45 L/s)	108.8	65.0	43.8	62.4	430.0
Peak Hour	106.0	65.0	41.0	58.4	402.5

Francis Valenti

From: Jhamb, Nishant < nishant.jhamb@ottawa.ca>

Sent: June 9, 2022 1:07 PM
To: Francis Valenti

Subject: RE: 22-5087 - Boundary Condition Request - 68 Sweetland Avenue

Attachments: 68 Sweetland Avenue June 2022.pdf

Hello Francis

The following are boundary conditions, HGL, for hydraulic analysis at 68 Sweetland Avenue (zone 1W) assumed to be connected to the 203 mm watermain on Sweetland Avenue (see attached PDF for location).

Minimum HGL: 106.0 m Maximum HGL: 115.4 m

Max Day + Fire How (45 L/s): 108.8 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Thanks

Nishant Jhamb, P.Eng
Project Manager | Gestionnaire de projet
Planning, Real Estate and Economic Development Department
Development Review - Central Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P1J1
613.580.2424 ext./poste 23112, nishant.jhamb@ottawa.ca

From: Francis Valenti <F.Valenti@McIntoshPerry.com>

Sent: May 27, 2022 9:45 AM

To: Jhamb, Nishant <nishant.jhamb@ottawa.ca>

Subject: RE: 22-5087 - Boundary Condition Request - 68 Sweetland Avenue

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Hello Nishant,

Revise water calcs are attached including OBC fire flow. The demands are summarized below:

- The estimated fire flow is 8,000 L/min based on the FUS
- The estimated fire flow is 2,700 L/min based on the OBC
- Average Daily Demand: 0.19 L/s
- Maximum Daily Demand: 1.82 L/s
- Maximum hourly daily demand: 2.73 L/s

I've also attached the pre-consultation notes that were provided to us.

Regards,

Francis Valenti, EIT

Engineering Intern, Land Development

T. 613.714.6895 | C. 613.808.2123

F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality

From: Jhamb, Nishant <nishant.jhamb@ottawa.ca>

Sent: May 26, 2022 2:42 PM

To: Francis Valenti < F. Valenti@McIntoshPerry.com>

Subject: RE: 22-5087 - Boundary Condition Request - 68 Sweetland Avenue

Hello Francis,

What is the Fire Demand with OBC method? Are you getting more than 9000L/min?

Was a pre application consultation done on this application?

Thanks

Nishant Jhamb, P.Eng

Project Manager | Gestionnaire de projet

Planning, Real Estate and Economic Development Department

Development Review - Central Branch

Oty of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P1J1

613.580.2424 ext./poste 23112, nishant.jhamb@ottawa.ca

From: Francis Valenti < F. Valenti@McIntoshPerry.com >

Sent: May 26, 2022 1:16 PM

To: Jhamb, Nishant < nishant.jhamb@ottawa.ca>

Subject: 22-5087 - Boundary Condition Request - 68 Sweetland Avenue

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Good Afternoon,

We would like to request boundary conditions for the proposed development at 68 Sweetland Avenue The proposed development consists of a 3-storey residential rear addition. The existing building includes 9 units and the proposed addition includes 33 units. The proposed connection (single) will be to the existing 203 mm dia. watermain located within Sweetland Avenue.

- The estimated fire flow is 8,000 L/min based on the FUS
- Average Daily Demand: 0.19 L/s
- Maximum Daily Demand: 1.82 L/s
- Maximum hourly daily demand: 2.73 L/s

Please find attached a map showing the proposed connection location and calculations prepared for the demands listed above.

Regards,

Francis Valenti, EIT

Engineering Intern, Land Development
T. 613.714.6895 | C. 613.808.2123
F.Valenti@McIntoshPerry.com | www.mcintoshperry.com

McINTOSH PERRY

Turning Possibilities Into Reality

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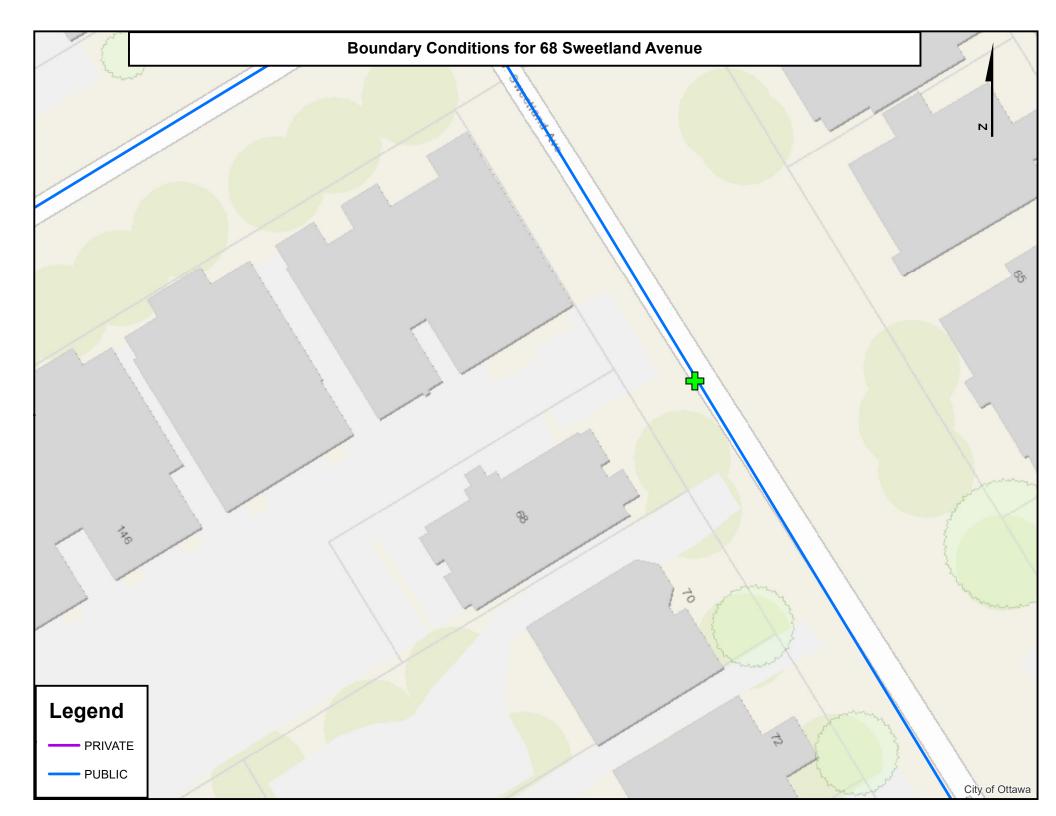
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Francis Valenti

From: Juan Gomez < gomez@woodmanarchitect.com> Sent: September 23, 2022 12:36 PM To: Francis Valenti Cc: Alison Gosling Subject: Re: 22-5087 - FUS Parameters - 68 Sweetland Ave Attachments: image759506.png Type C Le ven. 23 sept. 2022 11 h 12, Francis Valenti < F. Valenti@mcintoshperry.com > a écrit : Hi Juan, Can you please confirm the occupancy type as well? Definitions can be found on pages 24-25 of the attached PDF. Thanks, Francis Valenti, EIT **Engineering Intern, Land Development T.** 613.714.6895 | **C.** 613.808.2123 F.Valenti@McIntoshPerry.com | www.mcintoshperry.com Turning Possibilities Into Reality Confidentiality Notice - If this email wasn't intended for you, please return or delete it. Click here to read all of the legal language around this concept.

From: Juan Gomez < gomez@woodmanarchitect.com >

Sent: September 23, 2022 12:05 PM

To: Francis Valenti < F. Valenti@McIntoshPerry.com > Cc: Alison Gosling < a.gosling@mcintoshperry.com >

Subject: Re: 22-5087 - FUS Parameters - 68 Sweetland Ave

Hi Francis,
Yes to your questions, construction type, wood frame. Sprinkler Type is standard.
Thank you
Juan Gomez
To help protect your privacy. Moreauth Office prevented automatic deventional of this protons from the Internet.
Associate
4 Beechwood Ave. Suite 201, Ottawa, ON K1L 8L9
Tel: 613.228.9850 ext. 225
Fax: 613.228.9848
Email: gomez@woodmanarchitect.com
On Thu, Sep 22, 2022 at 10:01 AM Francis Valenti < F. Valenti@mcintoshperry.com > wrote:
Good morning Juan,
In order to respond to City comments we require additional confirmation of the building parameters used in fire flow calculations for 68 Sweetland. Can you please confirm once more that the below parameters are correct?
Construction Type: Wood Frame Sprinkler Type: Standard
We also require confirmation of the occupancy type. Based on the 2020 FUS guidelines (see attached), residential occupancies are classified as "Limited Combustible". Can you please confirm the occupancy type is "Limited Combustible" for the proposed building?

Thanks,

Francis Valenti, EIT

Engineering Intern, Land Development
T. 613.714.6895 | C. 613.808.2123
F.Valenti@McIntoshPerry.com | www.mcintoshperry.com



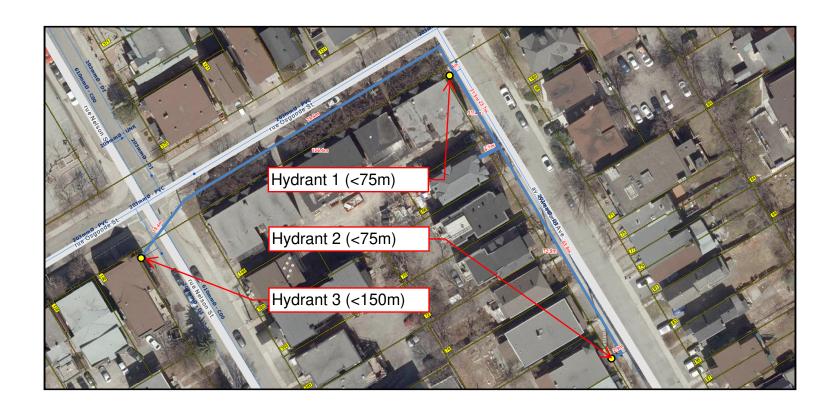
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68 Sweetland Avenue Hydrant Coverage Figure



APPENDIX D SANITARY CALCULATIONS

000-22-5087 - 68 Sweetland Avenue - Sanitary Demands

Project:	68 Sweetland Avenue
Project No.:	000-22-5087
Designed By:	FV
Checked By:	AG
Date:	May-22
Ste Area	0.06 Gross ha
1 Bedroom (Existing)	9 1.40 Persons per unit
1 Bedroom (Proposed)	33 1.40 Persons per unit
Total Population	59 Persons

DESIGN PARAMETERS

Institutional/Commercial Peaking Facto 1.5

Residential Peaking Factor 3.64 * Using Harmon Formula = $1+(14/(4+P^{n}0.5))*0.8$

where P = population in thousands, Harmon's Correction Factor = 0.8

Mannings coefficient (n) 0.013

Demand (per capita) 280 L/day Infiltration allowance 0.33 L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	How (L/s)
Dry	0.00
Wet	0.02
Total	0.02

AVERAGE DAILY DEMAND

DEM AND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	59	0.19
Industrial - Light**	35,000	L/ gross ha/d		0
Industrial - Heavy* *	55,000	L/ gross ha/d		0
Commercial / Amenity	2,800	L/ (1000m² /d)		0
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/ gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.19	L/s
PEAK RESIDENTIAL FLOW	0.70	L/s
AVERAGE ICI FLOW	0.00	L/s
PEAK INSTITUTIONAL/ COMMERCIAL FLOW	0.00	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAKIO FLOW	0.00	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.19	L∕s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.70	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.72	L/s

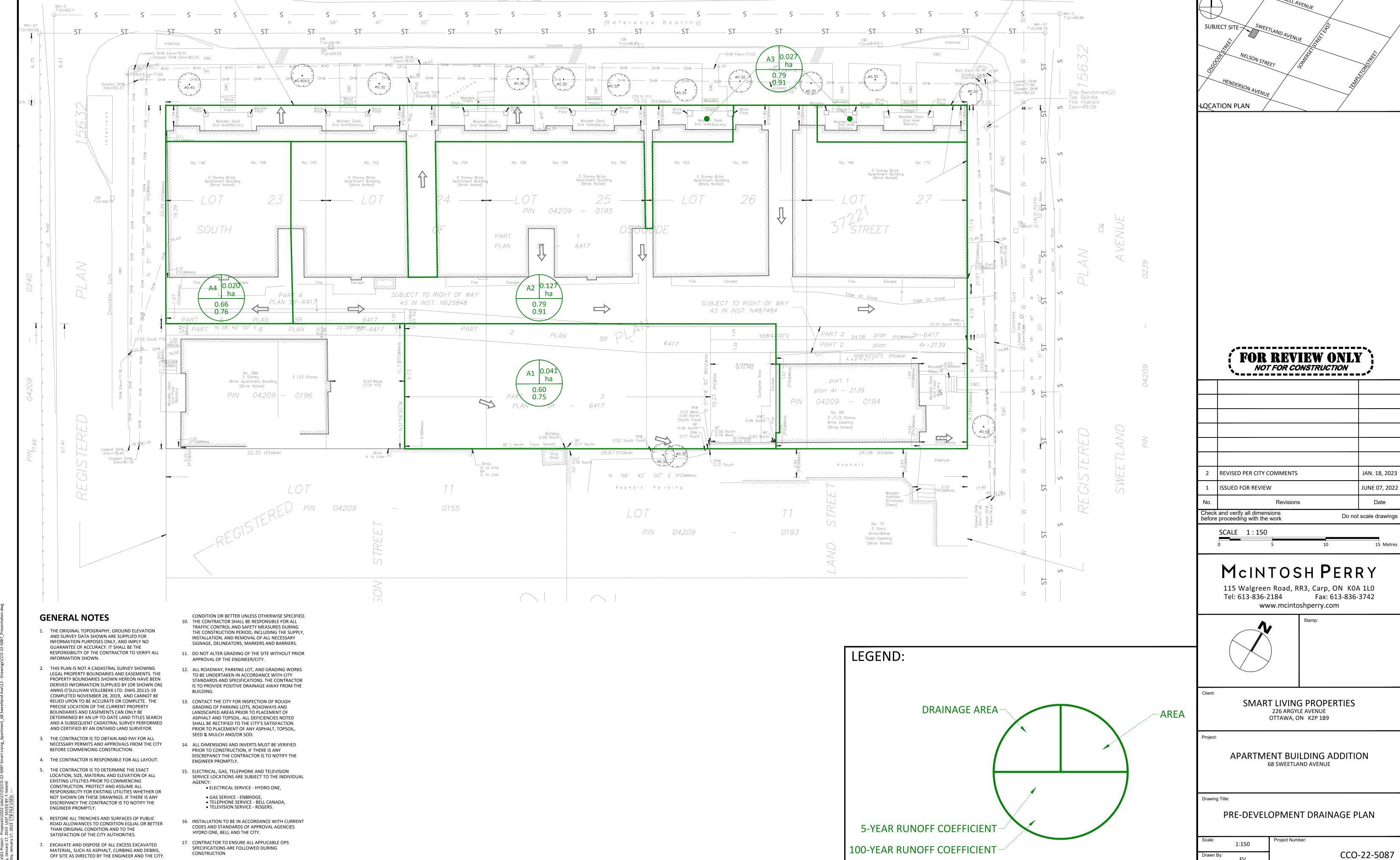
SANITARY SEWER DESIGN SHEET

PROJECT: CCC-22-5087

LOCATION: 68 Sweetland Avenue
CJENT: Smart Living Properties

	LOC	ATION			RESIDENTIAL							INFILTE	RATION ALL	OWANCE	FLOW			(SEWER DAT	Ä			
1	2	3	4	5	6	7	8	9	10	11	12	13	21	22	23	24	25	26	27	28	29	30	31
					UNIT	TYPES		AREA	POPU	LATION		PEAK	ARE	A (ha)	FLOW	DESIGN	CAPACITY	LENGTH	DIA	SLOPE	VELOCITY	AVAII	LABLE
STREET	AREA I	ID FROM MH	TO MH	SF	SD	TH	APT	(ha)	IND	CUM	PEAK FACTOR	FLOW (L/s)	IND	CUM	(L/s)	FLOW (L/s)	(L/s)	(m)	(mm)	(%)	(full) (m/s)	CAPA L/s	ACTY (%)
Sweetland Ave		EX. BLDG	MAIN				42	0.06	59.0	59.0	3.64	0.70	0.06	0.06	0.02	0.72	15.89	13.60	150	1.00	0.871	15.17	95.49
Design Parameters:				Notes:							Designed:				Rev	ision					Date		
Residential		ICI Areas		Manning Demand	gs coefficien I (per capita			0.013) L/day			FV				Revis	sion 1					2022.11.21		
SF 3.4 p/p/u			Peak Factor	3. Infiltration	on allowand	e:		B L/s/Ha			Checked:												
TH/SD 2.7 p/p/u	INST	28,000 L/Ha/day	1.5	4. Resident							CJM												
1-BR 1.4 p/p/u	COM	28,000 L/Ha/day	1.5				(14/(4+P^0.5									· · · · · · · · · · · · · · · · · · ·							
Other 60 p/p/Ha	IND	35,000 L/ Ha/ day	MOE Chart		where P=p	oopulation i	in thousands	6			Project No.										Sheet No: 1 of 1		

APPENDIX E PRE-DEVELOPMENT DRAINAGE PLAN



CONSTRUCTION

DATED JUNE 20, 2022.

UNLESS OTHERWISE SPECIFIED.

18. ALL PROPOSED CURB TO BE CONCRETE BARRIER CURB

19. THIS PLAN MUST BE READ IN CONJUNCTION WITH THE GEOTECHNICAL INVESTIGATION COMPLETED BY EXP,

OFF SITE AS DIRECTED BY THE ENGINEER AND THE CITY.

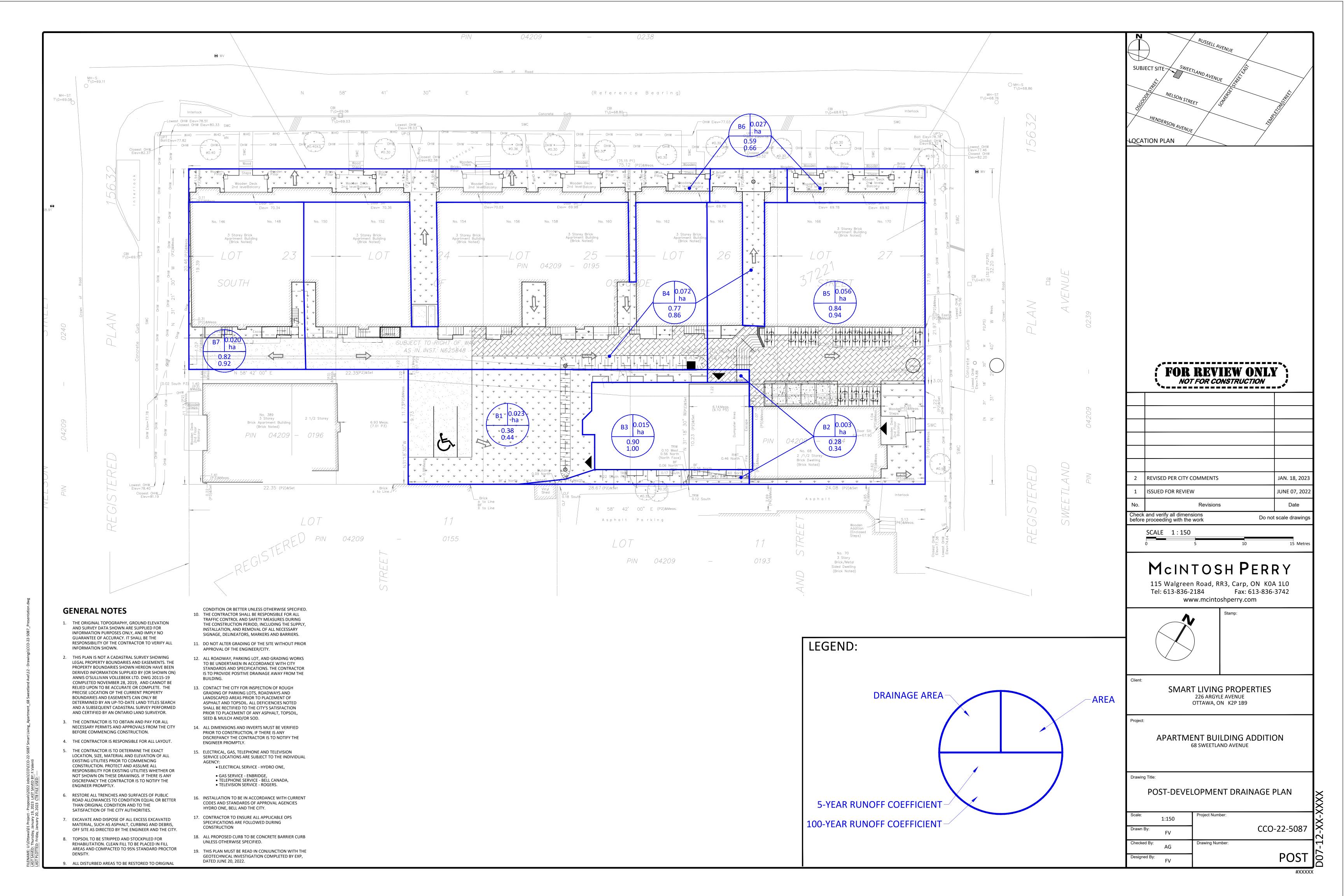
REHABILITATION, CLEAN FILL TO BE PLACED IN FILL AREAS AND COMPACTED TO 95% STANDARD PROCTOR

9. ALL DISTURBED AREAS TO BE RESTORED TO ORIGINAL

8. TOPSOIL TO BE STRIPPED AND STOCKPILED FOR

Drawing Number:

APPENDIX F POST-DEVELOPMENT DRAINAGE PLAN



APPENDIX G STORWWATER MANAGEMENT CALCULATIONS

CCO-22-5087 - 68 Sweetland Avenue

1 of 4

Tc (min)	Intensity (mm/hr)						
(11111)	2-Year	5-Year	100-Year				
10	76.8	104.2	178.6				

C-Values							
Impervious	0.90						
Gravel	0.60						
Pervious	0.20						

Pre-Development Runoff Coefficient

Drainage Area	Impervious Area (m²)	Gravel (m²)	Pervious Area (m²)	Average C (2/5-year)	Average C (100-year)	
A1	3	407	0	0.60	0.75	Development Area Drainage
A2	863	376	36	0.79	0.91	Existing Sweetland Drainage
A3	149	52	70	0.66	0.76	Existing Osgoode Drainage
A4	141	57	3	0.81	0.92	Existing Nelson Drainage

Pre-Development Runoff Calculations

Drainage	Drainage Area C C Tc			Q (L/s)			
Area	(ha)	2/5-Year	100-Year	(min)	2-Year	5-Year	100-Year
A1	0.041	0.60	0.75	10	5.27	7.15	15.30
Total (Dev Area)	0.041				5.27	7.15	15.30
A2	0.127	0.79	0.91	10	21.55	29.23	57.27
A3	0.027	0.79	0.91	10	4.57	6.20	12.16
A4	0.020	0.66	0.76	10	2.84	3.86	7.58
Total (Ex. Drainage)	0.175				28.97	39.30	77.00

Post-Development Runoff Coefficient

Drainage Area	Impervious Area (m²)	Gravel (m²)	Pervious Area (m²)	Average C (2/5-year)	Average C (100-year)	
B1	58	0	172	0.38	0.44	Collected
B2	4	0	30	0.28	0.34	Uncollected
B3	146	0	0	0.90	1.00	Roof
B4	580	0	137	0.77	0.86	Ex. Sweetland Collected
B5	510	0	48	0.84	0.94	Ex. Sweetland Uncollected
B6	149	0	122	0.59	0.66	Ex. Osgoode Drainage
B7	179	0	22	0.82	0.92	Ex. Nelson Drainage

Post-Development Runoff Calculations

Drainage	Area	С	С	Tc		Q (L/s)		
Area	(ha)	2/5-Year	100-Year	(min)	2-Year	5-Year	100-Year	
B1	0.023	0.38	0.44	10	1.85	2.52	5.03	Collected
B2	0.003	0.28	0.34	10	0.20	0.27	0.56	Uncollected
B3	0.015	0.90	1.00	10	2.81	3.81	7.25	Roof
Total (Dev Area)	0.041				4.86	6.60	12.84	
B4	0.072	0.77	0.86	10	11.73	15.91	30.48	Ex. Sweetland Collected
B5	0.056	0.84	0.94	10	10.01	13.58	25.93	Ex. Sweetland Uncollected
B6	0.027	0.59	0.66	10	3.38	4.59	8.90	Ex. Osgoode Drainage
B7	0.020	0.82	0.92	10	3.54	4.80	9.17	Ex. Nelson Drainage
Total(Ex. Drainage)	0.175				28.66	38.88	74.48	1

Required Restricted Flow (Development Area)

Drainage	Area	С	Tc	Q (L/s)
Area	(ha)	2/5-Year	(min)	2-Year
A1	0.041	0.60	10	5.27

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow (L/S)				Restricted Flow (L/S)		
Area	2-year	5-year	100-Year	2-Year	5-Year	100-Year	
B1	1.85	2.52	5.03	1.85	2.52	5.03	Collected
B2	0.20	0.27	0.56	0.20	0.27	0.56	Uncollected
B3	2.81	3.81	7.25	0.63	0.63	0.63	Roof
Total (Dev Area)	4.86	6.60	12.84	2.69	3.42	6.22	
B4	11.73	15.91	30.48	11.73	15.91	30.48	Ex. Sweetland Collected
B5	10.01	13.58	25.93	10.01	13.58	25.93	Ex. Sweetland Uncollected
B6	3.38	4.59	8.90	3.38	4.59	8.90	Ex. Osgoode
B7	3.54	4.80	9.17	3.54	4.80	9.17	Ex. Nelson
Total(Ex. Drainage)	28.66	38.88	74.48	28.66	38.88	74.48	7

Post-Development Storage Summary

Drainage Area	Storage Re	quired (m³)		Sorage Provided (m³)		
	2-Year	5-Year	100-Year	2-Year	5-Year	100-Year
B3	1.53	2.41	5.90	1.64	2.56	6.09
Total	1.53	2.41	5.90	1.64	2.56	6.09

CCO-22-5087 - 68 Sweetland Avenue - Roof Storage

2 of 4

2-Year Storm Event

Tc (min)	l (mm/hr)	B3 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	76.8	2.81	0.63	2.18	1.31
20	52.0	1.90	0.63	1.27	1.53
30	40.0	1.46	0.63	0.83	1.50
40	32.9	1.20	0.63	0.57	1.37
50	28.0	1.02	0.63	0.39	1.18
60	24.6	0.90	0.63	0.27	0.96
70	21.9	0.80	0.63	0.17	0.71
80	19.8	0.72	0.63	0.09	0.45

Maximum Storage Required 5-Year $(m^3) = 1.53$

5-Year Storm Event

Tc (min)	l (mm/hr)	B3 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	104.2	3.81	0.63	3.18	1.91
20	70.3	2.57	0.63	1.94	2.33
30	53.9	1.97	0.63	1.34	2.41
40	44.2	1.62	0.63	0.98	2.36
50	37.7	1.38	0.63	0.75	2.24
60	32.9	1.20	0.63	0.57	2.06
70	29.4	1.07	0.63	0.44	1.86
80	26.6	0.97	0.63	0.34	1.64

Maximum Storage Required 5-Year (m³) = 2.41

100-Year Storm Event

Tc (min)	l (mm/hr)	B3 Runoff (L/s)	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	178.6	7.25	0.63	6.62	3.97
20	120.0	4.87	0.63	4.24	5.09
30	91.9	3.73	0.63	3.10	5.58
40	75.1	3.05	0.63	2.42	5.81
50	64.0	2.60	0.63	1.97	5.90
60	55.9	2.27	0.63	1.64	5.90
70	49.8	2.02	0.63	1.39	5.84
80	45.0	1.83	0.63	1.20	5.74

Maximum Storage Required 100-Year (m³) = 5.9

Storage Parameters				
Roof Area (m ²)	146.09			
Usable Roof Area (%)	75%			
Usable Roof Area (m ²)	109.57			

5-Year Storage Summary				
Max. Storage Available (m ³)	2.56			
5-Year Storage Required (m ³)	2.41			
Max. Ponding Depth (m)	0.07			

2-Year Storage Summary	
Max. Storage Available (m³)	1.64
2-Year Storage Required (m ³)	1.53
Max. Ponding Depth (m)	0.045

100-Year Storage Summary				
Max. Storage Available (m³)	6.09			
100-Year Storage Required (m3)	5.90			
Max. Ponding Depth (m)	0.125			

115 Walgreen Road, R.R.3. Carp, ON K0A 1L0 | T. 613-836-2184 | F. 613-836-3742

CCO-22-5087 - 68 Sweetland Avenue - Roof Storage

Roof Drain Flow (B3)			3 of 4
Тур	Watts Drainage - Accutrol Weir		
Nu	mber of Roof Drains	2	
F	oof Drain Position	Closed	
	2-Year	5-Year	100-Year
Rooftop Storage Available (m³)	1.64	2.56	6.09
Rooftop Storage Required (m³)	1.53	2.41	5.90
Storage Depth (m)	0.045	0.070	0.125
How (Per Roof Drain) (L/s)	0.32	0.32	0.32
Total How (L/s)	0.63	0.63	0.63

Row Rate Vs. Build-Up (Individual Drain)					
	,				
Depth (mm)	Flow (L/s)				
0	0.00				
0 5	0.06				
10	0.13				
15	0.13				
20	0.19				
25	0.32				
30	0.32				
35	0.32				
40	0.32				
45	0.32				
50	0.32				
55	0.32				
60	0.32				
65	0.32				
70	0.32				
75	0.32				
80	0.32				
85	0.32				
90	0.32				
95	0.32				
100	0.32				
105	0.32				
110	0.32				
115	0.32				
120	0.32				
125	0.32				
130	0.32				
135	0.32				
140	0.32				
145	0.32				
150	0.32				

Ī	_									
	Roof Drain How									
	Individual How (I/s)	Storage Depth	Cumulative How							
		(mm)	(I/s)							
	0.00	0	0.00							
	0.06	5	0.13							
	0.13	10	0.25							
	0.19	15	0.38							
	0.25	20	0.50							
	0.32	25	0.63							
	0.32	30	0.63							
	0.32	35	0.63							
	0.32	40	0.63							
2-Year	0.32	45	0.63							
	0.32	50	0.63							
	0.32	55	0.63							
	0.32	60	0.63							
	0.32	65	0.63							
5-Year	0.32	70	0.63							
	0.32	75	0.63							
	0.32	80	0.63							
	0.32	85	0.63							
	0.32	90	0.63							
	0.32	95	0.63							
	0.32	100	0.63							
	0.32	105	0.63							
	0.32	110	0.63							
	0.32	115	0.63							
	0.32	120	0.63							
100-Year	0.32	125	0.63							
	0.32	130	0.63							
	0.32	135	0.63							
	0.32	140	0.63							
	0.32	145	0.63							
	0.32	150	0.63							

Note: The flow leaving through a restricted roof drain is based on flow vs. head information

^{*} Roof Drain model to be Accutrol Weirs, See attached sheets

^{*} Roof Drain How information taken from Watts Drainage website

CCO-22-5087 - 68 Sweetland Avenue

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Time of Concentration Pre-Development

Drainage Area	Sheet Flow	Sope of	Tc (min)	Tc (min)
ID	Distance (m)	Land (%)	(5-Year)	(100-Year)
A1	53	3.60	8	5

Therefore, a Tc of 10 can be used

 $Tc = (3.26(1.1-c)L^0.5/S^0.33)$

c = Balanced Runoff Coefficient
 L = Length of drainage area
 S = Average slope of watershed

STORM SEWER DESIGN SHEET

PROJECT: CCC-22-5087

LOCATION: 68 Sweetland Avenue
CLIENT: Smart Living Properties

McINTOSH PE

	LOCATION				CONTRIBUTING AREA (ha)			PATIONAL DESIGN FLOW											SEWER DATA	١		
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	19	20	21	22	25	26	27	28
STREET	AREA ID	FROM MH	TO MH	C-VALUE	AREA	INDIV AC	CUMUL AC	INLET (min)	TIME IN PIPE	TOTAL (min)	i (5) (mm/hr)	i (10) (mm/hr)	i (100) (mm/hr)	5yr PEAK FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	PIPESIZE(mm DIA	SLOPE (%)	VELOCITY (m/s)	AVAIL ((L/s)	CAP (5yr) (%)
		-						(,		()	(**************************************	(******)	(******)	1 2 11 (2 5)	1 = 0 11 (= 0)	(= 5)	()	1	(79	(**************************************	(= -)	(19
Sweetland Ave	B1	LSOB5	LSOB4	0.38	0.02	0.01	0.01	10.00	0.20	10.20	104.19	122.14	178.56	2.54	2.54	43.87	10.60	250	0.50	0.866	41.33	94.22%
	B1 + B4	LSOB4	OB3	0.77	0.07	0.06	0.06	10.20	0.11	10.32	103.13	120.89	176.71	18.32	18.32	142.67	13.25	300	2.00	1.955	124.34	87.16%
	B1 + B3 + B4	OB3	MH2	0.90	0.01	0.01	0.08	10.32	0.15	10.47	102.55	120.20	175.71	21.97	21.97	179.90	22.64	300	3.18	2.466	157.93	87.79%
	B1 + B3 + B4	MH2	MH1				0.08	10.47	0.10	10.57	101.77	119.29	174.37	21.80	21.80	100.88	8.54	300	1.00	1.383	79.08	78.39%
																						-
Definitions:				Notes:				Designed:					No.			Revision	<u> </u>			Da	to.	
Q = 2.78QA, where:				Mannings coefficient (n	N _		0.013	FV					1		ICC	UED FOR REVI	I 🗆 🗸 /				01.20	$\overline{}$
Q = Peak Flow in Litres	per Second (I/s)			1. Wallings Coefficient (I	ı) –		0.010	, v					- '.		100	OLD I OITHLY	ILVV			2020.	01.20	
A = Area in Hectares (h								Checked:														
	millimeters per hour (n	nm/hr)						CH														
[i = 998.071 / (TC+6.		5 YEAR																				
[i = 1174.184 / (TC+6		10 YEAR						Project No.:	-													
[i = 1735.688 / (TC+6	5.014)^0.820]	100 YEAR						OOO-22-5087	7						Da					Shee		
															2023.	.01.20				1 0	f 1	

APPENDIX H
CITY OF OTTAWA DESIGN CHECKLIST

Oty of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Oriteria	Location (if applicable)
☐ Executive Summary (for larger reports only).	N/ A
Date and revision number of the report.	On Cover
Location map and plan showing municipal address, boundary, and layout of proposed development.	Appendix A
Plan showing the site and location of all existing services.	Ste Servicing Plan (C102)
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual	1.1 Purpose 1.2 Ste Description
developments must adhere.	6.0 Stormwater Management
Summary of pre-consultation meetings with City and other approval agencies.	Appendix B
Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments,	1.1 Purpose
Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and	1.2 Site Description
develop a defendable design criteria.	6.0 Stormwater Management
Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary



☐ Identification of existing and proposed infrastructure available in the immediate area.	N/A
☐ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Ste Grading Plan (C101)
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Ste Grading Plan (C101)
☐ Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
Proposed phasing of the development, if applicable.	N/ A
Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Background Studies, Standards and References
 All preliminary and formal site plan submissions should have the following information: Metric scale North arrow (including construction North) Key plan Name and contact information of applicant and property owner Property limits including bearings and dimensions Existing and proposed structures and parking areas Easements, road widening and rights-of-way Adjacent street names 	Ste Grading Plan (C101)

4.2 Development Servicing Report: Water

Oriteria	Location (if applicable)
☐ Confirm consistency with Master Servicing Study, if available	N/A
Availability of public infrastructure to service proposed development	N/A
☐ Identification of system constraints	N/A
☐ Identify boundary conditions	Appendix C
☐ Confirmation of adequate domestic supply and pressure	N/A
 Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. 	Appendix C
 Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. 	N/A
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
Address reliability requirements such as appropriate location of shut-off valves	N/ A
☐ Check on the necessity of a pressure zone boundary modification.	N/ A
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Appendix C, Section 4.2

Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Ste Servicing Plan (C101)
Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix C
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

4.3 Development Servicing Report: Wastewater

Oriteria	Location (if applicable)
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/A
Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 5.2 Proposed Sanitary Sewer

☐ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Peference can be made to previously completed Master Servicing Study if applicable)	Section 5.3 Proposed Sanitary Design
☐ Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	N/A
 Description of proposed sewer network including sewers, pumping stations, and forcemains. 	Section 5.2 Proposed Sanitary Sewer
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/ A
 Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development. 	N/A
☐ Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
 Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding. 	N/A
Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

Oriteria	Location (if applicable)
Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Analysis of available capacity in existing public infrastructure.	N/A
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Pre & Post-Development Plans
Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Set-back from private sewage disposal systems.	N/A
☐ Watercourse and hazard lands set backs.	N/A
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period).	Appendix G

☐ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Ste Grading Plan
Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 7.0 Proposed Stormwater Management Appendix G
Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/ A
☐ Identification of potential impacts to receiving watercourses	N/A
Identification of municipal drains and related approval requirements.	N/A
Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Ste Grading Plan (C101)
☐ Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A

Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 8.0 Sediment & Erosion Control
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
☐ Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

Oriteria Criteria Cri	Location (if applicable)
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/ A
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A
☐ Changes to Municipal Drains.	N/A
Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A

4.6 Conclusion Checklist

Oriteria Criteria Cri	Location (if applicable)
☐ Clearly stated conclusions and recommendations	Section 9.0 Summary
	Section 10.0 Recommendations
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped