15-17 des Oblats Avenue Transportation Impact Assessment

Step 1 Screening Report
Step 2 Scoping Report
Step 3 Forecasting Report
Step 4 Strategy Report (Revision #1)

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1 Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for the TIA Study PM. As shown in the Screening Form, a TIA is required including the Design Review component and the Network Impact Component. This TIA is in support of a zoning by-law amendment and site plan application.

2 Existing and Planned Conditions

2.1 Proposed Development

The subject site, zoned as Residential Fourth and Fifth Densities (R4UD, R5B), contains a four-storey T-shaped building, previously the Convent of the Sisters of the Sacred Heart, two surface parking lots each accessing Springhurst Avenue, and perpendicular parking along des Oblats Avenue. The proposed development is a retrofit of the existing T-Shape structure with an addition on the north-west corner for a total of 284 rental apartment units. The existing east side surface parking lot is proposed to remain. The site will access Springhurst Avenue via a right-in-right-out access to a surface parking lot and a depressed curb accessing a loading area. Parking is proposed as 27 vehicle spaces for site visitors and 418 total bicycle spaces, and two carshare spaces are proposed to be reserved on the des Oblats frontage. The development is anticipated to be built-out in a single phase by 2025.

Figure 1: Area Context Plan

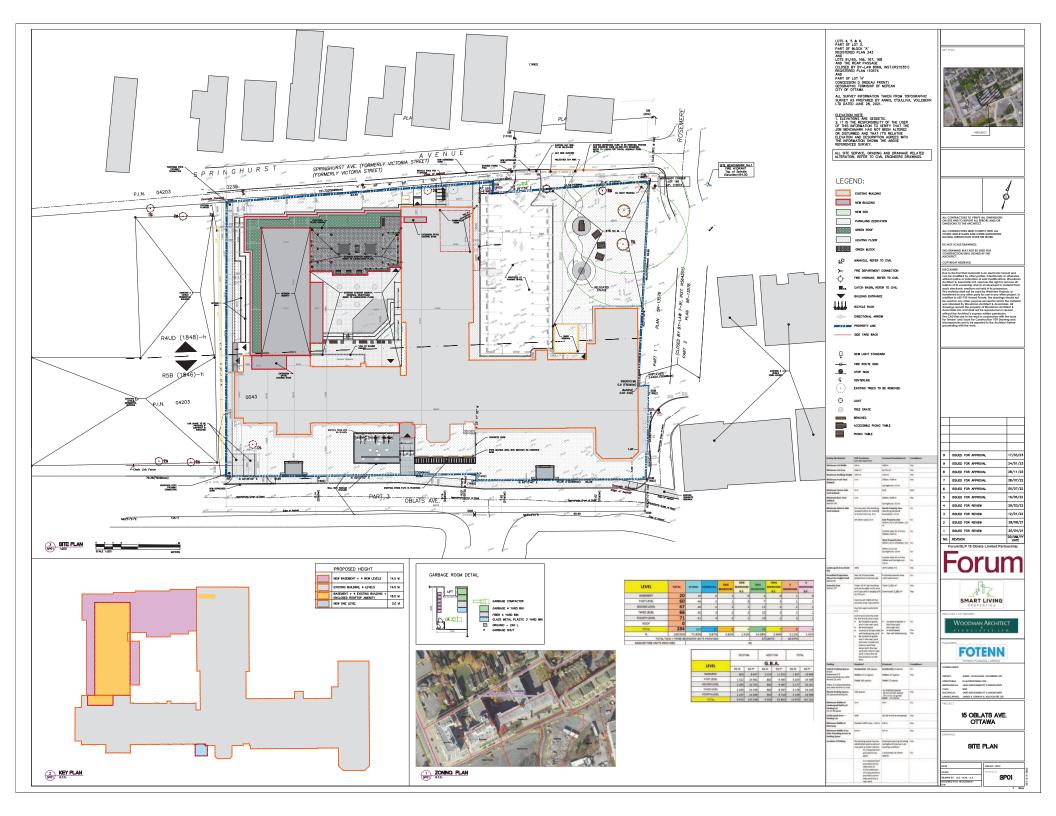
417

15-17 des Oblats Avenue

Figure 1 illustrates the study area context. Figure 2 illustrates the proposed concept plan.

Source: http://maps.ottawa.ca/geoOttawa/ Accessed: February 6, 2023





2.2 Existing Conditions

2.2.1 Area Road Network

Main Street: Main Street is a City of Ottawa arterial road. North of Hawthorne Avenue within the study area, Main Street has a three-lane urban cross-section (two northbound lanes), and between Hawthorne Avenue and Lees Avenue, it has a four-lane urban cross-section. Between Lees Avenue and Evelyn Avenue, Main Street has a three-lane urban cross-section (two southbound lanes), and south of Evelyn Avenue Main Street has a two-lane urban cross-section. Cycletracks are on both sides of the road between Harvey Street and the south end of the Highway 417 overpass, sharrow markings are on the west side of the road and a cycletrack is on the east side of the road between the south end of the Highway 417 overpass and Graham Avenue, and cycletracks are on both sides of the road south of Graham Avenue. Sidewalks are present on both sides of the road within the study area. Onstreet parking is permitted in framed parking lanes on the east side of the road south of Evelyn Avenue within the study area, and on the on the west side of the road south of Hazel Street within the study area. The posted speed limit is 40 km/h, and the Ottawa Official Plan reserves a 23.0-metre right of way within the study area. Main Street is a truck route.

Hawthorne Avenue: Hawthorne Avenue is a City of Ottawa arterial road with a four-lane urban cross-section including on-street parking permitted (no stopping weekdays 7:00AM-9:00AM, 3:30PM-5:30PM) and sidewalks on both sides of the road west of Main Street. East of Main Street, Hawthorne Avenue is a one-way eastbound local road with a one-lane urban cross-section including sidewalks on both sides of the road, a curbside bike lane on the south side of the road, and on-street parking permitted on the north side of the road. West of Main Street, the unposted speed limit is assumed to be 50 km/h and the posted speed limit is 30 km/h to the east. The Ottawa Official Plan reserves a 20.0-metre right of way to the west, and the measured right of way is 18.0 metres to the east of Main Street. Hawthorne Avenue is a truck route west of Main Street.

Lees Avenue: Lees Avenue is a City of Ottawa arterial road with a two-lane urban cross-section including sidewalks on both sides of the road, a curbside bike lane on the south side of the road, and a parking lane on the north side of the road. The posted speed limit is 30 km/h, and the Ottawa Official Plan reserves a 23.0 metre right of way within the study area. Lees Avenue is a truck route.

Graham Avenue: Graham Avenue is a City of Ottawa one-way westbound local road with a one-lane urban cross-section including sidewalks on both sides of the road, an eastbound curbside bike lane on the south side of the road, and on-street parking permitted on the north side of the road. The unposted speed limit is assumed to be 50 km/h and the measured right of way is 15.5 metres.

Evelyn Avenue: Evelyn Avenue is a City of Ottawa one-way westbound local road west of Rosemere Avenue with a one-lane urban cross-section including sidewalks on both sides of the road and on-street parking permitted in a school bus loading layby along the Lady Evelyn School frontage and on the north side of the road east of the school. East of Rosemere Avenue, Evelyn Avenue is a two-way local road with a two-lane urban cross-section with on-street parking permitted on the south side of the road to its termination at Brunswick Street, and with sidewalks on both sides of the road west of Chestnut Street. The posted gateway speed limit for the neighbourhood is 30 km/h and the measured right of way is 10.0 metres between Main Street and the school, 14.0 metres along the school building, 10.0 metres between the school building and the intersection at Rosemere Avenue, and 15.0 metres through the intersection at Rosemere Avenue and eastward.

Springhurst Avenue: Springhurst Avenue is a City of Ottawa one-way eastbound local road west of Rosemere Avenue with a one-lane urban cross-section including sidewalks on both sides of the road and with on-street parking permitted on the north side of the road. East of Rosemere Avenue, Springhurst Avenue is a two-way local



road with a two-lane urban cross-section that includes sidewalks on both sides of the road and on-street parking permitted on the north side of the road west of Chestnut Street. The posted speed limit 30 km/h and the measured right of way is 10.0 metres west of the intersection at Rosemere Avenue, and 15.0 metres through the intersection at Rosemere Avenue and eastward.

Des Oblats Avenue: des Oblats Avenue is a City of Ottawa local road with a two-lane urban cross-section including a sidewalk on the north side of the road to the east and west of 15-17 des Oblats Avenue and a sidewalk on the south side of the road. The posted speed limit is 30 km/h, consistent with the gateway speed limit on Hazel Street east of Main Street, and the measured right of way varies between 12.0 metres and 20.0 metres.

Hazel Street: Hazel Street is a City of Ottawa local road with a two-lane urban cross-section with on-street parking permitted on the north side of the road west of Main Street and with sidewalks on both sides of the road. The posted speed limit is 30 km/h. East of Main Street, the measured right of way is 18.5 metres and west of Main Street, the measured right of way varies between 15.0 metres and 19.0 metres.

2.2.2 Existing Intersections

The existing signalized area intersections within 400 metres of the site have been summarized below:

Main Street at Hawthorne Avenue

The intersection of Main Street at Hawthorne Avenue is a signalized intersection. The northbound approach consists of a shared left-turn/through lane, a shared through/right-turn lane, and a cycletrack and the southbound approach consists of a shared left-turn/through lane and an auxiliary through/right-turn lane. The eastbound approach consists of a shared left-turn/through lane with a bike box, and a right-turn lane and the east leg is inbound only. No turn restrictions were noted.

Main Street at Graham Avenue / Lees
Avenue

The intersection of Main Street at Graham Avenue/Lees Avenue is a signalized intersection. The northbound approach consists of a shared left-turn/through lane, an auxiliary through/right-turn lane, and a cycletrack and the southbound approach consists of a shared left-turn/through lane and a shared through/right-turn lane. The westbound approach consists of an auxiliary left-turn lane with a bike box and a shared through/right-turn lane, and the west leg is inbound only, though includes a contraflow bike lane. Southbound left-turns are restricted weekdays between 3:30AM-5:30PM, buses excepted.

Main Street at Evelyn Avenue

The intersection of Main Street at Evelyn Avenue is a signalized T-intersection. The northbound approach consists of a through lane and a cycletrack and the southbound approach consists of two through lanes and a cycletrack. The westbound approach consists of a shared left-turn/right-turn lane with a bike box. No turn restrictions were noted.

Main Street at Springhurst Avenue

The intersection of Main Street at Springhurst Avenue is an uncontrolled T-intersection. The northbound approach consists of a shared through/right-turn lane and a cycletrack, and the southbound approach consists of an auxiliary left-turn lane, a through lane, and a cycletrack. The east leg of the intersection is inbound only.



Northbound right turns are prohibited weekdays between 7:00AM-9:00AM, bicycles excepted.

Main Street at Immaculata HS / des Oblats Avenue The intersection of Main Street at the Immaculata High School access/des Oblats Avenue is a signalized intersection. The northbound and southbound approaches each consist of an auxiliary left-turn, a shared through/right-turn, and a cycletrack. The eastbound approach consists of a shared all-movements lane and a bike box, and the westbound approach consists of an auxiliary left-turn lane, and a shared through/right-turn lane. No turn restrictions were noted.

Main Street at Hazel Street

The intersection of Main Street at Hazel Street is a signalized intersection. The northbound and southbound approaches each consist of an auxiliary left-turn lane, a shared through/right-turn lane, and a cycletrack. The eastbound approach consists of a shared all-movements lane with a bike box and the westbound approach consists of a shared left-turn/through lane and auxiliary right-turn lane with a bike box. No turn restrictions were noted.

2.2.3 Existing Driveways

On des Oblats Avenue, two driveways connecting a rear lane for townhomes is present on the north side of the road to the east of the site and a driveway to underground parking for a mixed-use building is present on the north side and a driveway to two mixed-use buildings is present on the south side of the road to the west of the site.

On Springhurst Avenue, a driveway to a mixed-use complex is present on the south side, and driveways to a vacant site and four detached residential dwellings are present on the north side of the road west of the site. Along the site, driveways to seven detached residential dwellings are present on the north side of the road. East of the site on Springhurst Avenue, and east of Rosemere Avenue, a driveway to an institutional building and ten driveways to attached and detached residential dwellings are present on the south side, and six driveways to detached and attached residential dwellings and two public rear lanes are present on the north side of the road.

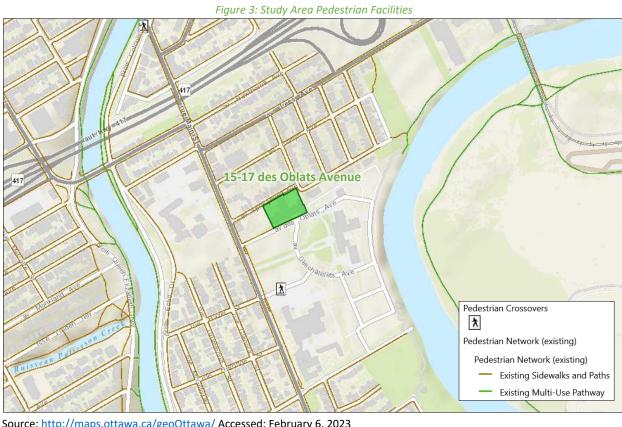
2.2.4 Cycling and Pedestrian Facilities

Figure 3 illustrates the pedestrian facilities in the study area and Figure 4 illustrates the cycling facilities.

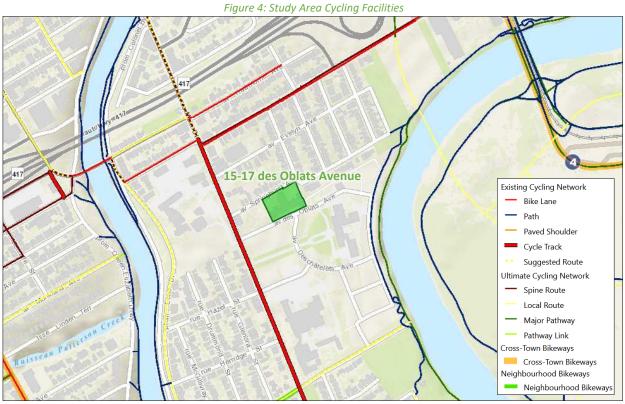
Sidewalks are provided along both sides of all study area roads, excluding des Oblats Avenue whose north sidewalk is discontinuous. A pedestrian crossover is located on Hazel Street midblock between Main Street and Deschatelets Avenue.

Cycling facilities include cycletracks on both sides of the Main Street south of Lees Avenue and on the east side of the road between Lees Avenue and Hawthorne Avenue, a curbside bike lane on both sides of Hawthorne Avenue over the Pretoria Bridge and on the south side of Hawthorne Avenue east of Main Street, and on the south sides of Lees Avenue and Graham Avenue. The Rideau Canal Eastern Pathway runs along the canal 375 metres west of the site, and the Rideau River Western Pathway runs along the river 300 metres east of the site and connects des Oblats Avenue and Springhurst Avenue to Lees Station. Clegg Street west of Main Street is a neighbourhood bikeway, Hawthorne Avenue west of Colonel By Drive, Colonel By Drive between Hawthorne Avenue and Graham Avenue, Graham Avenue, and Lees Avenue form a spine route and Main Street is a spine route. Hawthorne Avenue east of Colonel By Drive, Echo Drive, and Clegg Street are local routes.





Source: http://maps.ottawa.ca/geoOttawa/ Accessed: February 6, 2023



Source: http://maps.ottawa.ca/geoOttawa/ Accessed: February 6, 2023



Pedestrian and cyclist volumes included in study area intersection counts, presented in Section 2.2.7, have been compiled and are illustrated in Figure 5 and Figure 6 respectively.

Figure 5: Existing Pedestrian Volumes

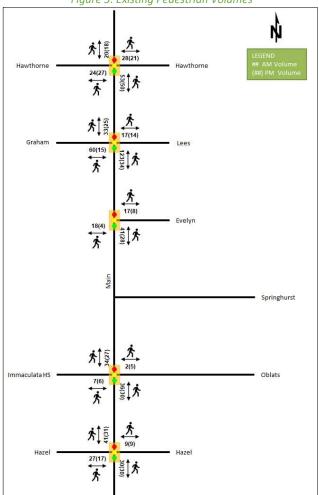
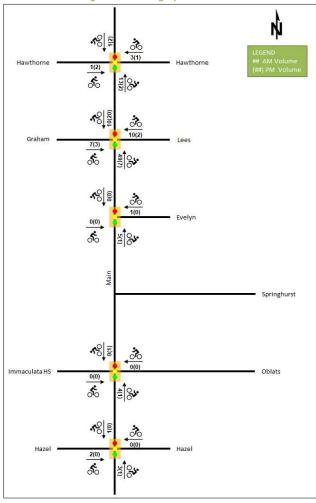


Figure 6: Existing Cyclist Volumes



2.2.5 Existing Transit

Figure 7 illustrates the transit system map in the study area and Figure 8 illustrates nearby transit stops. All transit information is from February 6, 2023 and is included for general information purposes and context to the surrounding area.

Within the study area, the routes #5, #16, #55, #56 travel along Main Street, with the route #5 continuing along Hawthorne Avenue, the route #16 continuing along Lees Avenue and the routes #55 and #56 continuing along both Hawthorne Avenue and Lees Avenue. The site is overlapped by the 800-metre radius from the Lees LRT Station on the O-Train Confederation Line and is within 950 metres' walk. The frequency of these routes within proximity of the proposed site currently are:

- Route #5 30-minute service all day
- Route #16 30-minute service all day
- Route #55 15-minute daytime service, 30-minute service after 7:00PM



• Route #56 – Operating during peak periods only, 15-minute service in peak period/direction, 30-minute service in remainder of peak period and in off-peak period/direction



Source: http://www.octranspo.com/ Accessed: February 6, 2023

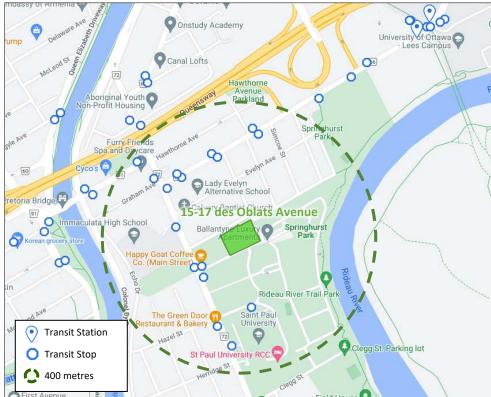


Figure 8: Existing Study Area Transit Stops

Source: http://www.octranspo.com/ Accessed: February 6, 2023

2.2.6 Existing Area Traffic Management Measures

Bulb-outs, including cycle-friendly bulb-outs, narrow curb radii, and textured crossing treatments at intersections along Main Street, speed humps on Hawthorne Avenue, and on-street parking throughout are the primary traffic calming measures within the study area.



2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa for the existing study area intersections. Table 1 summarizes the intersection count dates.

Table 1: Intersection Count Date

Intersection	Count Date		
Main Street at Hawthorne Avenue	Tuesday, March 3, 2020		
Main Street at Graham Avenue/Lees Avenue	Tuesday, November 6, 2018		
Main Street at Evelyn Avenue	Tuesday, March 7, 2017		
Main Street at Immaculata HS/des Oblats Avenue	Tuesday, March 7, 2017		
Main Street at Hazel Street	Tuesday, March 7, 2017		

Figure 9 illustrates the existing traffic counts, balanced along Main Street, and Table 2 summarizes the existing intersection operations. The level of service for signalized intersections is based on volume-to-capacity ratio (v/c) calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. Detailed turning movement count data is included in Appendix B and the Synchro worksheets are provided in Appendix C.

Figure 9: Existing Traffic Counts 114(181) 393(429) 5(8) 4 | 4 Hawthorne Hawthorne 289(290) 12(70) 220(254) 80(27) 515(630) 24(4) 163(132) 53(13) 177(174) ا إ لـ Lees 7 1 1 64(68) 622(534) 7(7) 684(794) 48(7) 43(23) Evelyn 638(590) Springhurst 11(8) 709(825) 4(5) **1** 5(7) ← 0(0) Immaculata HS Oblats ᅪ 11(4) 1(0) 17(11) ኅ ተ ተ 3(0) 658(589) 17(52) ← 0(1) 5(25) Hazel Hazel 32(18) ኅ↑୯ 1(0) 9(7) 16(11) 644(517) 9(10)



Table 2: Existing Intersection Operations

Intersection	lana		AM Pe	ak Hour			PM Peak Hour			
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)	
	EBL/T	D	0.83	55.2	#136.6	F	1.09	117.3	#197.5	
Main Street at	EBR	Α	0.33	3.9	14.6	Α	0.43	5.4	18.9	
Hawthorne Avenue	NB	В	0.68	10.0	31.0	Α	0.57	12.7	51.3	
Signalized	SB	Α	0.54	25.1	58.1	Α	0.53	21.3	69.2	
	Overall	D	0.86	21.0	-	D	0.82	34.2	-	
Main Street at	WBL	D	0.84	69.8	#74.7	В	0.70	53.6	#64.6	
Graham	WBT/R	Α	0.53	16.8	37.4	Α	0.37	8.9	17.4	
Avenue/Lees	NB	Α	0.47	14.5	54.7	Α	0.41	13.4	46.1	
Avenue	SB	Α	0.44	6.9	23.4	Α	0.41	16.3	50.5	
Signalized	Overall	Α	0.56	17.8	-	Α	0.47	18.6	-	
NA - ! C+ + - +	WBL/R	Α	0.29	20.8	22.2	Α	0.09	23.3	11.0	
Main Street at	NBT	В	0.64	15.8	112.8	В	0.63	8.3	34.6	
Evelyn Avenue Signalized	SBT	Α	0.36	11.1	m45.6	Α	0.45	12.7	53.3	
Signalizea	Overall	Α	0.52	13.5	-	Α	0.46	11.1	-	
	EB	Α	0.12	19.3	9.4	Α	0.04	0.2	0.0	
	WBL	Α	0.01	30.5	2.2	Α	0.02	30.3	2.8	
Main Street at		Α	0.01	0.0	0.0	Α	0.01	0.0	0.0	
Immaculata	NBL	Α	0.12	7.8	m3.1	Α	0.01	9.0	m0.2	
HS/des Oblats	NBT/R	С	0.75	16.2	#180.1	В	0.67	15.3	#72.5	
Avenue Signalized	SBL	Α	0.01	6.5	1.3	Α	0.02	5.6	m0.7	
Signanzea	SBT/R	E	0.91	38.1	#216.7	Е	0.95	37.2	#261.8	
	Overall	В	0.64	26.9	-	В	0.69	27.7	-	
	EB	Α	0.21	29.3	15.0	Α	0.08	0.5	0.0	
	WBL/T	Α	0.03	31.6	4.1	Α	0.14	34.1	12.0	
NA - ! C++ - !	WBR	Α	0.06	0.4	0.0	Α	0.17	1.0	0.0	
Main Street at Hazel Street	NBL	Α	0.04	7.2	2.3	Α	0.05	7.0	2.3	
	NBT/R	С	0.77	23.8	#170.5	Α	0.54	12.8	97.8	
Signalized	SBL	Α	0.23	9.9	m3.9	Α	0.06	3.2	m0.5	
	SBT/R	В	0.69	10.8	m44.3	С	0.80	7.5	m#33.9	
	Overall	В	0.68	17.1	-	С	0.74	9.5	-	

Notes:

Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres

Peak Hour Factor = 0.90

Delay = average driver delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours, the study area intersections operate well with the exception of the intersection of Main Street and Hawthorne Avenue under the existing signal timing.

Extended queueing may be observed during the AM peak hour on the northbound through/right movements at the intersections of Main Street at the Immaculata High School access/des Oblats Avenue and Main Street at Hazel Street. Extended queueing may additionally be observed during both peak hours on the westbound left movement at the intersection of Main Street at Graham Avenue/Lees Avenue, the southbound through/right movements at the intersections of Main Street at the Immaculata High School access/des Oblats Avenue, and Main Street at Hazel Street.

It is noted that during the PM peak hour, queuing on the northbound movement at the intersection of Main Street at Hawthorne Avenue and on the southbound movement at the intersection of Main Street and Graham Avenue/Lees Avenue will exceed the distance between these two intersections, which may interfere with operations beyond the modelled conditions.



The eastbound left/through movement at the intersection of Main Street at Hawthorne Avenue is modeled as being over theoretical capacity with high delays and extended queues during both peak hours and with potential high delays for the overall intersection during the PM peak hour. As the remaining movements operate with high level of service during both peak hours, split could be reallocated to the overcapacity movement to reduce all movements' v/c ratios to 1.00 or below.

2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study are road network. Table 3 summarizes the collisions types and conditions in the study area, Figure 10 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix D.

Table 3: Study Area Collision Summary, 2016-2020

		Number	%
Total C	Collisions	23	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	2	9%
	Property Damage Only	21	91%
	Angle	5	22%
	Rear end	8	35%
	Sideswipe	5	22%
Initial Impact Type	Turning Movement	1	4%
	SMV Unattended	1	4%
	SMV Other	2	9%
	Other	1	4%
	Dry	15	65%
	Wet	4	17%
Road Surface Condition	Loose Snow	2	9%
	Packed Snow	1	4%
	Ice	1	4%
Pedestrian Involved		1	4%
Cyclists Involved		0	0%



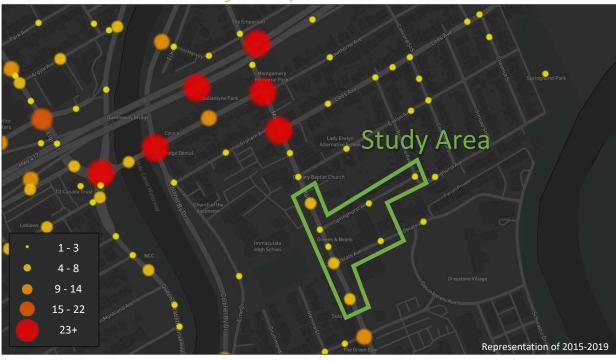


Figure 10: Study Area Collision Records

Table 4: Summary of Collision Locations, 2016-2020

	Number	%
Intersections / Segments	23	100%
Main St @ Springhurst Ave	1	4%
Main St @ des Oblats Ave	6	26%
Rosemere Ave @ Springhurst Ave	1	4%
Main St btwn Evelyn Ave & Springhurst Ave	5	22%
Main St btwn Springhurst Ave & Oblats Ave	2	9%
Main St btwn des Oblats Ave & Hazel St	6	26%
Springhurst Ave btwn Main St & Rosemere Ave	1	4%
Des Oblats Ave btwn Main St & End	1	4%

Within the study area, no locations were subject to a high incidence of collisions within the 2016-2020 time period. Twenty-one of the total 23 collisions involved property damage only and the remaining two had non-fatal injuries. The collision types are most represented by rear end with eight collisions, followed by sideswipe and angle with five collisions each, and two or fewer as SMV (other), turning movement, SMV (unattended), and other. Rear end and sideswipe collisions, comprising 57% of study area collisions, are generally associated with congestion. Three of the five angle collisions happened along Main Street and may be associated with private accesses, where the remaining two angle collisions were at the intersection with des Oblats Avenue. No further patterns were noted, and weather conditions are not considered to affect collisions within the study area.

2.3 Planned Conditions

2.3.1 Changes to the Area Transportation Network

No changes are listed for the study area transportation network in the TMP and in Ottawa's Planned Construction Projects portal, and the subject site is not within a CDP area and is not subject to any additional policy considerations.



Clegg Street West of Main Street is identified as being included in the Glebe Neighbourhood Bikeway affordable cycling phase one project (2014-2019).

The Greenfield Avenue, Main Street, Hawthorne Avenue et al. Reconstruction Project was initiated in summer 2021 and is anticipated to be completed in 2025. As part of the civil infrastructure project, complete street upgrades will be undertaken to improve the experience of all road users in the northern section of Old Ottawa East. Figure 11 illustrates the project limits.

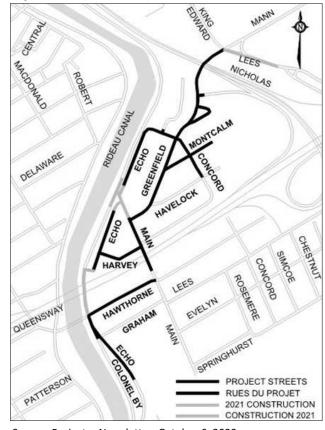


Figure 11: Greenfield Avenue, Main Street, Hawthorne Avenue et al. Reconstruction Project

Source: Project e-Newsletter, October 6, 2022

2.3.2 Other Study Area Developments

172 Main Street, 10 des Oblats Avenue

The proposed development application includes a site plan for the construction of a nine-storey, 119-unit mixed use building with 2,000ft² of ground floor commercial space and a six-storey, 125-unit mixed use building with 18,000ft² of commercial space. The development was anticipated to be built-out in 2021 and to generate 74 new AM and 101 new PM peak hour two-way auto trips. (Novatech, 2018)

175 Main Street, 225 Scholastic Drive

The proposed development application includes a site plan for the construction of an eight-storey, 146-unit retirement home. Built-out in 2020, the development was anticipated to generate five new AM and 14 new PM peak hour two-way auto trips. (Novatech, 2017)

225 Scholastic Drive

The proposed development application includes a zoning by-law amendment to permit the redevelopment of an existing heritage building to include a school supporting up to 351 students. Anticipated to see initial occupancy



in 2021, the development was anticipated to generate 57 new AM and 140 new PM peak hour two-way auto trips. (Novatech, 2020)

248 Main Street

The proposed development application includes a site plan for the construction a three-storey building including two residential units and a commercial unit at grade. No TIA is available for this development.

375 Deschatelets Avenue

The proposed development application includes a site plan for the construction of three-and-a-half-storey 18-unit stacked townhouse dwelling. The development was anticipated to be built out in 2021 and to generate seven new AM and eight new PM peak hour two-way auto trips. (Novatech, 2018)

360 Deschatelets Avenue

The proposed development application includes a site plan for the construction an 85-unit apartment building. No TIA is available for this development.

3 Study Area and Time Periods

3.1 Study Area

The study area will include the intersections of Main Street at:

- Hawthorne Avenue
- Graham Avenue/Lees Avenue
- Evelyn Avenue
- Immaculata HS/des Oblats Avenue
- Hazel Street

The boundary streets will be Springhurst Avenue and des Oblats Avenue and no screenlines are present within proximity to the site.

3.2 Time Periods

As the proposed development is composed entirely of residential units the AM and PM peak hours will be examined.

3.3 Horizon Years

The anticipated build-out year is 2025. As a result, the full build-out plus five years horizon year is 2030.

4 Exemption Review

Table 5 summarizes the exemptions for this TIA.

Table 5: Exemption Review

Module	Element	Explanation	Exempt/Required						
Design Review Component									
4.1 Development	4.1.2 Circulation and Access	Only required for site plans	Required						
Design	4.1.3 New Street Networks	Only required for plans of subdivision	Exempt						



Module	Element	Explanation	Exempt/Required
422 11	4.2.1 Parking Supply	Only required for site plans	Required
4.2 Parking	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Required
Network Impact Comp	onent		
4.5 Transportation Demand Management	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Required
4.8 Network Concept		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of equivalent volume permitted by established zoning	Exempt

5 Development-Generated Travel Demand

Walking

Total

5.1 Mode Shares

Examining the mode shares presented in the TRANS Trip Generation Manual (2020) for the district derived from the most recent National Capital Region Origin-Destination survey (OD Survey), the existing mode shares by land use for Ottawa Inner are summarized in Figure 9. While the development may achieve low-to-no personal auto ownership by the tenants, the auto driver mode has not been modified to account for deliveries and ride-hailing.

Multi-Unit (High-Rise) **Travel Mode** AM PM **Auto Driver** 26% 25% **Auto Passenger** 6% 8% 21% **Transit** 28% Cycling 5% 6%

34%

100%

39%

100%

Table 6: Mode Shares – Ottawa Inner

5.2 Trip Generation

This TIA has been prepared using the vehicle and person trip rates for the residential dwellings using the TRANS Trip Generation Manual (2020). Table 7 summarizes the person trip rates for the proposed residential land use for each peak period.

Table 7: Trip Generation Person Trip Rates

Dwelling Type	Land Use Code	Peak Period	Person Trip Rates	
Multi Unit (Uiah Bisa)	221 & 222	AM	0.80	
Multi-Unit (High-Rise)	(TRANS)	PM	0.90	

Using the above Person Trip rates, the total person trip generation for each peak period has been estimated. Table 8 below illustrates the total person trip generation by dwelling type.



Table 8: Total Person Trip Generation

Land Use	Units /	AM Peak Period			PM Peak Period			
Land Use	GFA	In	Out	Total	In	Out	Total	
Multi-Unit (High-Rise)	284	70	157	227	148	108	256	

Using the above site-specific AM and PM mode shares and the person trip rates, the person trips by mode have been projected. Trip generation by peak hour has been forecasted using the prescribed peak period conversion factors presented in the TRANS Trip Generation Manual (2020) for the residential component. Figure 10 summarizes the trip generation by mode for the residential dwellings.

Table 9: Trip Generation by Mode

		AM Peak Hour				PM Peak Hour			
Travel Mode		Mode Share	In	Out	Total	Mode Share	In	Out	Total
	Auto Driver	26%	9	20	28	25%	16	12	28
e)	Auto Passenger	6%	2	4	7	8%	5	4	9
그 뜻	Transit	28%	11	24	35	21%	15	11	25
Multi-Unit (High-Rise)	Cycling	5%	2	5	6	6%	4	3	7
ΣΞ	Walking	34%	14	31	45	39%	30	22	52
	Total	100%	35	79	114	100%	65	48	113

As shown above, 28 AM and 28 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

5.3 Trip Distribution

To understand the travel patterns of the subject development, the OD Survey has been reviewed to determine the travel for the residential dwellings, and these patterns were applied based on the build-out of Ottawa Inner. Table 10 below summarizes the distributions.

Table 10: OD Survey Distribution – Ottawa Inner

To/From	Residential % of Trips
North	15%
South	30%
East	10%
West	45%
Total	100%

5.4 Trip Assignment

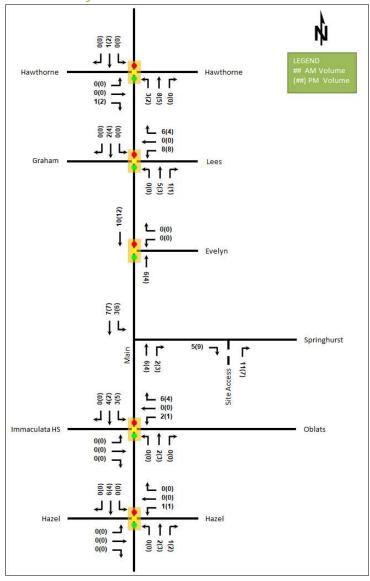
Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the study area road network. Table 11 summarizes the proportional assignment to the study area roadways, and Figure 12 illustrates the new site generated volumes. The vehicles assigned to des Oblats Avenue are accessing the site frontage.



Table 11: Trip Assignment

To/From	Inbound Via	Outbound Via
North	10% Main St (N)	15% Main St (N)
NOITH	5% Lees Ave (E)	13% Wall 3t (N)
South	30% Main St (S)	30% Main St (S)
East	10% Lees Ave (E)	10 % Lees Ave (E)
	25% Lees Ave (E)	25% Main St (N)
West	15% Hawthorne Ave (W)	15% Hawthorne Ave (W)
	5% Main St (S)	5% Main St (S)
Total	100%	100%

Figure 12: New Site Generation Auto Volumes





6 Background Network Travel Demands

6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3. No study area projects are considered to have any notable impact on the study area traffic volumes and travel patterns.

6.2 Background Growth

A review of the background projections from the City's TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways. The TRANS model plots and a summary of the results of the model interpolation are provided in Appendix E.

In general, the growth rates in the study area derived from the two TRANS model horizons are projected to be negative along the Main Street corridor, to be low in the eastbound direction and to be higher in the westbound direction on Lees Avenue during the AM peak hour. Growth rates derived from the 2011 to the 2031 model horizons rounded to the nearest 0.25% will be peak-directionally applied to the appropriate major turning movements to and from the identified roadways at the study area intersections. Table 12 summarizes the growth rates applied within the study area.

•	date zzi iii iii o negion	ui iviouei Projections – 3	, ,			
Chuach	AM Pea	ak Hour	PM Peak Hour			
Street	Eastbound	Westbound	Eastbound	Westbound		
Hawthorne Ave	0.75%	-	-	0.75%		
Lees Ave	0.25%	6.00%	6.00%	0.25%		
	Northbound	Southbound	Northbound	Southbound		
Main St	-	-	-	-		

Table 12: TRANS Regional Model Projections – Study Area Growth Rates

6.3 Other Developments

The background developments explicitly considered in the background conditions (Section 6.2) include:

- 172 Main Street, 10 des Oblats Avenue
- 175 Main Street, 225 Scholastic Drive
- 225 Scholastic Drive
- 375 Deschatelets Avenue

These developments are anticipated to be built-out before the subject site and will be included in both background horizons. The background development traffic volumes are illustrated in Figure 13.



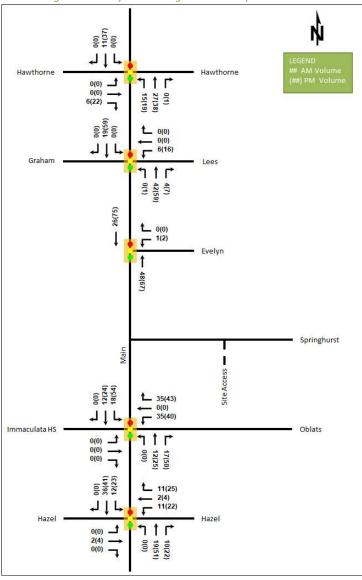


Figure 13: Study Area Background Development Volumes

7 Demand Rationalization

7.1 2025 Future Background Operations

Figure 14 illustrates the 2025 background volumes and Table 13 summarizes the 2025 background intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets for the 2025 future background horizon are provided in Appendix F.



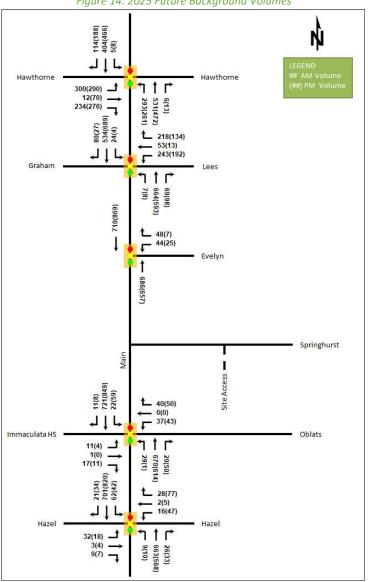


Figure 14: 2025 Future Background Volumes

Table 13: 2025 Future Background Intersection Operations

latana atian	1		AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EBL/T	С	0.77	50.6	#126.1	E	0.98	89.3	#175.9
Main Street at	EBR	Α	0.32	3.9	14.5	Α	0.42	5.4	18.7
Hawthorne Avenue Signalized	NB	В	0.63	9.5	31.3	Α	0.56	12.6	50.7
	SB	Α	0.49	24.2	52.5	Α	0.50	20.9	66.2
	Overall	D	0.81	19.6	-	С	0.78	27.8	-
Main Street at	WBL	F	1.03	109.8	#97.0	В	0.70	53.3	#64.2
Graham	WBT/R	Α	0.56	13.9	35.0	Α	0.34	8.9	16.6
Avenue/Lees Avenue	NB	Α	0.45	13.9	50.9	Α	0.43	13.6	47.9
	SB	Α	0.40	6.7	22.0	Α	0.40	15.8	49.4
Signalized	Overall	Α	0.59	23.8	-	Α	0.48	18.5	-



l			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	WBL/R	Α	0.27	20.1	20.3	Α	0.09	23.7	10.5
Main Street at Evelyn Avenue Signalized	NBT	В	0.62	15.1	106.6	В	0.64	9.2	m37.0
	SBT	Α	0.34	12.7	m42.5	Α	0.44	12.5	52.2
	Overall	Α	0.49	14.3	-	Α	0.46	11.3	-
	EB	Α	0.11	19.7	8.9	Α	0.04	0.2	0.0
	WBL	Α	0.23	35.6	14.3	Α	0.24	35.2	15.8
Main Street at	WBT/R	Α	0.07	0.2	0.0	Α	0.09	0.3	0.0
Immaculata	NBL	Α	0.09	3.8	m1.4	Α	0.00	10.0	m0.1
HS/des Oblats Avenue	NBT/R	С	0.75	12.6	#168.2	D	0.81	24.1	#161.1
Signalized	SBL	Α	0.06	6.7	3.7	Α	0.17	6.5	6.3
Signanzea	SBT/R	С	0.79	26.7	#189.3	D	0.88	27.4	#234.7
	Overall	Α	0.60	19.3	-	В	0.69	24.5	-
	EB	Α	0.20	29.2	14.5	Α	0.13	27.9	10.8
	WBL/T	Α	0.09	32.7	8.5	Α	0.26	36.6	18.3
Main Chuach at	WBR	Α	0.09	0.5	0.0	Α	0.22	1.5	0.0
Main Street at	NBL	Α	0.03	7.2	2.1	Α	0.04	7.0	2.2
Hazel Street Signalized	NBT/R	С	0.73	21.9	139.1	Α	0.59	15.6	106.1
	SBL	Α	0.23	11.4	m8.3	Α	0.13	4.0	m1.5
	SBT/R	В	0.65	14.2	94.4	С	0.76	7.4	m#42.2
	Overall	В	0.64	17.8	-	С	0.73	11.3	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00 Delay = average driver delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours at the 2025 future background horizon, the study area intersections generally operate similarly to the existing conditions.

The westbound left movement at the intersection of Main Street and Graham Avenue/Lees Avenue is forecasted to be over capacity with high delays during the AM peak hour at this horizon.

The reallocation of one second of split from the northbound/southbound phase to the overcapacity westbound phase would reduce all v/c ratios at the intersection to 1.00 or below.

7.2 2030 Future Background Operations

Figure 15 illustrates the 2030 background volumes and Table 14 summarizes the 2030 background intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets for the 2030 future background horizon are provided in Appendix G.



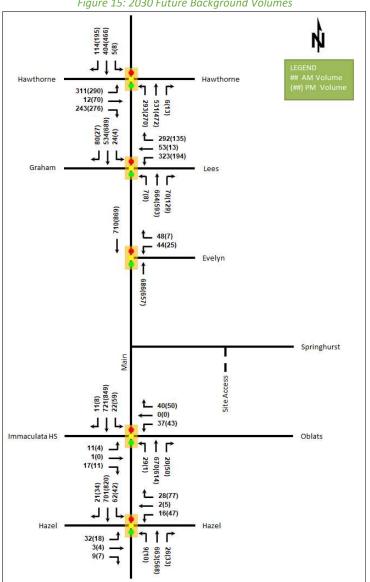


Figure 15: 2030 Future Background Volumes

Table 14: 2030 Future Background Intersection Operations

lusta una anti a un	Lana		AM Pe	ak Hour		PM Peak Hour				
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)	
Main Street at Hawthorne Avenue Signalized	EBL/T	С	0.80	52.7	#131.3	Е	0.98	89.3	#175.9	
	EBR	Α	0.33	3.9	14.6	Α	0.42	5.4	18.7	
	NB	В	0.63	9.9	35.1	Α	0.57	12.7	51.5	
	SB	Α	0.49	24.2	52.5	Α	0.51	20.9	66.7	
	Overall	D	0.82	20.3	-	С	0.79	27.8	-	
Main Street at	WBL	F	1.37	226.9	#134.7	С	0.71	53.8	#65.0	
Graham	WBT/R	С	0.71	21.5	55.8	Α	0.35	8.9	16.6	
Avenue/Lees	NB	Α	0.45	13.9	51.1	Α	0.46	14.0	50.5	
Avenue	SB	Α	0.40	6.8	22.3	Α	0.40	15.8	49.4	
Signalized	Overall	В	0.67	46.6	-	Α	0.50	18.6	-	



1			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Main Church at	WBL/R	А	0.27	20.1	20.3	Α	0.09	23.7	10.5
Main Street at Evelyn Avenue Signalized	NBT	В	0.62	15.1	106.6	В	0.64	9.2	m37.0
	SBT	Α	0.34	14.6	m40.9	Α	0.44	12.5	52.2
	Overall	Α	0.49	15.2	-	Α	0.46	11.3	-
	EB	Α	0.11	19.7	8.9	Α	0.04	0.2	0.0
Main Street at	WBL	Α	0.23	35.6	14.3	Α	0.24	35.2	15.8
	WBT/R	Α	0.07	0.2	0.0	Α	0.09	0.3	0.0
Immaculata	NBL	Α	0.09	3.8	m1.4	Α	0.00	10.0	m0.1
HS/des Oblats Avenue	NBT/R	С	0.75	12.6	#168.2	D	0.81	24.1	#161.1
Signalized	SBL	Α	0.06	6.7	3.7	Α	0.17	6.5	6.3
Signanzea	SBT/R	С	0.79	26.7	#189.3	D	0.88	27.4	#234.7
	Overall	Α	0.60	19.3	-	В	0.69	24.5	-
	EB	Α	0.20	29.2	14.5	Α	0.13	27.9	10.8
	WBL/T	Α	0.09	32.7	8.5	Α	0.26	36.6	18.3
NA-in Church of	WBR	Α	0.09	0.5	0.0	Α	0.22	1.5	0.0
Main Street at	NBL	Α	0.03	7.2	2.1	Α	0.04	7.0	2.2
Hazel Street Signalized	NBT/R	С	0.73	21.9	139.1	Α	0.59	15.6	106.1
	SBL	Α	0.23	11.4	m8.3	Α	0.13	4.0	m1.5
	SBT/R	В	0.65	14.2	94.4	С	0.76	7.4	m#42.2
	Overall	В	0.64	17.8	-	С	0.73	11.3	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00 Delay = average driver delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours at the 2030 future background horizon, the study area intersections operate similarly to the 2025 future background conditions.

The capacity, delay and queuing issues have increased at this horizon on the westbound left movement at the intersection of Main Street and Graham Avenue/Lees Avenue during the AM peak period.

The reallocation of seven seconds of split from the northbound/southbound phase to the overcapacity westbound phase would reduce all v/c ratios at the intersection to 1.00 or below.

7.3 Modal Share Sensitivity

Capacity issues have been noted at the Main Street at Hawthorne Avenue on the eastbound left/through movement during the AM peak hour. This effect is due to the intersection timing having been previously calibrated for north-south arterial priority and the balance of volumes being forecast to change with the addition of background growth. Residual capacity exists in the remaining movements and, as previously stated, shifting split from the north-south phases to the westbound phase would resolve the capacity issue. As such, no further rationalization for network demand is required.

The development proposes a reduced parking rate and site-generated auto trips are assumed to be conservative. Considering the foregoing, rationalization for site-generated demand is not required for this TIA.

8 Development Design

8.1 Design for Sustainable Modes

Hard surface connections are provided between building entrances and the surrounding pedestrian facilities on des Oblats Avenue and Springhurst Avenue. Bicycle parking is provided internal to the building and in surface racks



for visitors/short term on each the des Oblats Avenue and Springhurst Avenue frontages. Surface vehicle parking for visitors is proposed to be accessed via Springhurst Avenue.

Transit stops for all area routes listed in Section 2.2.5 are within 400 metres' walking distance of the closest building entrance, and Lees Station is a 950-metre walk from the building entrance on Springhurst Avenue.

A 1.5-metre-wide mid-block pathway connection is provided between the sidewalks on Springhurst Avenue and des Oblats Avenue on the west property line.

8.2 Circulation and Access

Vehicle access is to be provided via a 6.0-metre-wide right-in/right-out access on Springhurst Avenue. Emergency services are anticipated to access the site via the two public road frontages. Garbage collection will and loading operations will occur via a depressed curb on Springhurst Road adjacent to the parking lot access.

The existing west site access on Springhurst Avenue is to be removed as part of the development, and the east access is to be relocated, where the curbs will be reinstated to full height in the old access locations.

9 Parking

9.1 Parking Supply

The development is to provide 390 bicycle parking spaces located below ground and 28 bicycle parking spaces in surface racks. Twenty-seven vehicle parking spaces for visitors are proposed in a surface lot accessing Springhurst Avenue, and two carshare spaces are reserved on the des Oblats Avenue frontage.

The zoning by-law prescribes the inclusion of 136 vehicle parking spaces for residents, 27 vehicle parking spaces for visitors, and 142 bicycle parking spaces for the proposed development. The minimum bicycle parking requirement is satisfied, the minimum vehicle parking requirement for visitors is satisfied, however, a Parking By-Law exemption is being sought to eliminate the vehicle parking requirement for residents. The following section will outline the support and limited impacts of the exemption.

9.2 Spillover Parking

As the proposed parking provision is more than 15% below that prescribed by the by-law, spillover parking should be considered. While the required visitor parking is proposed as being met by the development, it is noted that resident parking is below the value prescribed by the zoning by-law by the full 136 spaces.

9.2.1 Tenant Factors

A number of mitigating factors are present within the proposed development, however, chief among them will be the composition of the tenancy, and the marketing of units as not to have access to parking.

The building is marketed to prospective tenants who work nearby, attend school nearby, or generally have an urban lifestyle. The housing model is one of "attainable housing" where the units come furnished. It is not anticipated that private vehicle ownership will be high amongst prospective tenants. Furthermore, making these prospective tenants aware that they will have no parking space early in the process of engagement will select for tenants who do not require regular use of a car, especially given the cycling facilities nearby, the proposed bike parking spaces, and proximity to transit. Only a minority of prospective tenants with vehicles might proceed to sign an annual lease a unit with no access to parking and the resultant overall potential for spillover parking will be low.



9.2.2 Mobility Options

The site is overlapped by the 800-metre radial distance from the Lees LRT Station and is less than a one-kilometre walking distance to the Station from the building entrances. This distance is similar to TOD applicable sites throughout Ottawa, for which policies including the elimination of parking requirements may be applicable. The walking routes to Lees Station available include sidewalk connectivity through Springhurst Avenue to Rosemere Avenue and Lees Avenue, or through Springhurst Avenue or des Oblats Avenue and the Rideau River Nature Trail along the Rideau River.

Main Street provides protected cycling corridor, an enhanced, wide pedestrian realm including bus stops allowing connections to the larger area network. Similarly, the Rideau River Nature Trail also allows for connectivity to the north and south. To enable the use of these facilities by residents, on-site bicycle parking is proposed at 1.47 spaces per unit, approximately three times the amount as required by the zoning by-law.

Two car share spaces are being provided on the site to provide car flexible options for residents without the need to own and park a car on site. This was an option discussed with the City's Area Traffic Management team as a potential mitigation for the site. The additional benefit of providing the car share is it will add a community benefit for the area to encourage the reduction of auto dependency.

9.2.3 Area Parking Restrictions

Notwithstanding the site and tenant factors reducing the likelihood of spill over parking, the adjacent neighbourhood parking restrictions were reviewed.

On-street parking is permitted within the community to the north of the site where most local streets permit parking on at least one side of the road. It is also anticipated that parking will be available within the developing area to the south of the site. Any potential spillover parking would be located as close to the building as available, and Springhurst Avenue, Rosemere Avenue, Concord Street South, and Evelyn Avenue would be the roadways most likely to be utilized.

The community may work with the City and local Councillor to adjust parking restrictions or permitted times on these roadways. It is cautioned that any changes will also affect the existing residents as much as restrict future residents of the community.

9.2.4 Community Adaptation

It is acknowledged that the adjacent community has concerns with on-street parking demands increasing. The road space is a public space for everyone to use as the community evolves. The brunt of this change is currently incumbent on the new developments to mitigate/restrict and the existing area is left to continue operating in old habits. The existing community will need to evolve with the community and adapt their mode of travel to reflect the path the City has charted forward with a decrease in auto modes and an increase on pedestrian/cycling and transit trips.

10 Boundary Street Design

Table 15 summarizes the MMLOS analysis for the boundary streets of Springhurst Avenue and Des Oblats Avenue. Where the existing and future conditions for both streets will be the same and are considered in one row. The boundary street analysis is based on the policy area of "within 300 m of a school" given these roads are in proximity to Immaculata High School. The MMLOS worksheets has been provided in Appendix H.



Table 15: Boundary Street MMLOS Analysis

Sagment	Sagment		Pedestrian LOS		Bicycle LOS		Transit LOS		k LOS
Segment		PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Des Oblets Avenue	Ex.		Α	В	D	-	-	-	-
Des Oblats Avenue Fut.		В	Α	Α					
Springhurst Avenue	Ex./Fut.	В	Α	В	D	-	-	-	-

The pedestrian LOS targets will not meet the area targets along boundary streets. To meet pedestrian LOS targets of 'A', des Oblates Avenue and Springhurst Avenue would each need boulevard width of 0.5-to-2.0 metres, which would not be consistent with the urban context.

11 Access Intersection Design

11.1 Location and Design of Access

The site will access Springhurst Avenue via a right-in-right-out access located in a similar location to the existing eastern parking lot access and approximately 26 metres from the adjacent property line. No turn lanes are proposed for the access intersections, the width is proposed as being 6.0 metres, and the throat length of the access is proposed as being approximately five metres.

A loading area is also proposed via a depressed curb to the west of the proposed parking lot access. This area is proposed as being approximately 6.4 metres wide and will also serve garbage collection operations.

11.2 Intersection Control

The site access is proposed as being stop-controlled on the minor approach of the access.

11.3 Access Intersection Design

No existing volumes are available along Springhurst Avenue; therefore, no access intersection operational analysis could be performed. The Springhurst Avenue approach operates as free-flow, and even moderate local one-way volumes interacting with a right-in/right-out access to a lot of 27 parking stalls would be anticipated to operate with good level of service on the minor approach.

11.3.1 Access Intersection MMLOS

The access intersection is unsignalized, therefore no access intersection MMLOS analysis is required.

11.3.2 Recommended Design Elements

No design elements for the site accesses on Springhurst Avenue are proposed outside of the typical private approach considerations.

12 Transportation Demand Management

12.1 Context for TDM

The mode shares used within the TIA represent the unmodified district mode shares. Overall, the modal shares are likely to be achieved and supporting TDM measures should be provided to encourage shifts towards the sustainable modes available within the area context.

The subject site is within the Old Ottawa East Secondary Plan. The total bedroom count within the development is 351 based upon 215 studio, 12 one-bedroom, 47 two-bedroom, and 10 three-bedroom units.



12.2 Need and Opportunity

The subject site has been assumed to rely most prominently on walking, followed by transit, and auto travel, and those assumptions have been carried through the analysis. Given the limited parking, the access to cycling infrastructure and the Lees LRT Station, and as the unmodified district mode shares have been applied, risks to other network users from failing to meet mode share targets are low.

12.3 TDM Program

The "suite of post occupancy TDM measures" has been summarized in the TDM checklists for the residential land uses. The checklist is provided in Appendix I. The key TDM measures recommended include:

- Display local area maps with walking and cycling routes, and transit route information and schedules at major entrances
- Provide a multimodal travel option information package to new residents
- Contract with providers to install on-site bikeshare (or other micro-mobility, e.g. scootershare) and/or carshare spaces
- Provide a permanent bike repair station

13 Neighbourhood Traffic Management

Site traffic is proposed to access the arterial network via Springhurst Avenue, Rosemere Avenue, and Des Oblats Avenue. The TIA Guidelines propose a threshold of 120 vehicles per peak hour for the classification of local roads, equivalent to 2 cars per minute total in both directions.

The overall site auto trips expected in each peak hour is 28 two-way vehicles, which would constitute 23% of the local road classification thresholds if concentrated on a single roadway. Therefore, no impact on the function or classification of the study area roadways is anticipated as a result of the proposed development.

14 Transit

14.1 Route Capacity

In Section 5.1 the trip generation by mode was estimated, including an estimate of the number of transit trips that will be generated by the proposed development. Table 16 summarizes the transit trip generation.

Table 16: Trip Generation by Transit Mode

Travel Mode	Mode Share	Al	M Peak Ho	ur	PM Peak Hour			
	wode Share	In	Out	Total	In	Out	Total	
Transit	28% (21%)	11	24	35	15	11	26	

The proposed development is anticipated to generate 35 AM peak hour transit trips and 26 PM peak hour transit trips. Trips in all directions may be serviced by Lees Station or those routes connecting to Lees Station. Trips servicing the University of Ottawa campus may additionally be more highly utilized, from a proportional standpoint.

From the trip distribution found in section 5.3, the transit trips can be further broken down. Table 17 summarizes resultant site-generated transit ridership trips by direction and the equivalent peak direction bus loads.



Table 17: Forecasted Site-Generated Transit Ridership

Direction	AM Pea	ak Hour	PM Pea	1 Peak Hour Service Type		Equivalent Service Increase						
	In	Out	In	Out								
North	2	4	2	2	Bus, LRT	Negligible						
South	3	7	5	3	Bus, BRT	Negligible						
East	1	2	2	1	Bus, LRT	Negligible						
West	5	11	7	5	Bus	One fifth standard bus load						

14.2 Transit Priority

At either future horizon, site traffic is anticipated to increase average delay on existing transit movements by no more than 2.7 seconds except for the westbound left-turn movement at the intersection of Main Street at Graham Avenue/Lees Avenue where the maximum increase of delay is 16.1 seconds. No change in transit level of service at study area intersections is resultant from the addition of site traffic to the network.

15 Network Intersection Design

15.1 Network Intersection Control

No change to the existing signalized control is recommended for the network intersections.

15.2 Network Intersection Design

15.2.1 2025 Future Total Network Intersection Operations

The 2025 future total volumes are illustrated in Figure 16 and the network intersection operations are summarized below in Table 18. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix J.



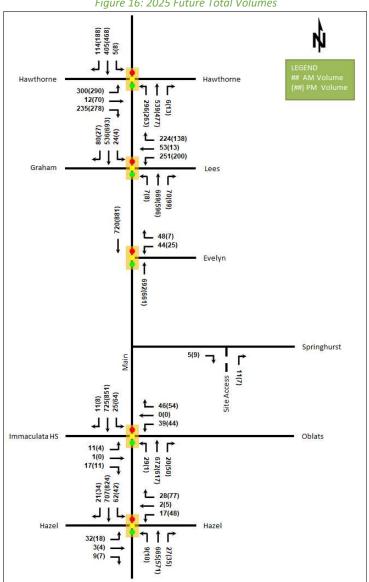


Figure 16: 2025 Future Total Volumes

Table 18: 2025 Future Total Network Intersection Operations

latere etiere	1		AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EBL/T	С	0.78	51.3	#126.6	E	0.99	91.9	#176.5
Main Street at Hawthorne Avenue Signalized	EBR	Α	0.32	3.9	14.4	Α	0.43	5.5	18.7
	NB	В	0.64	9.6	31.8	Α	0.56	12.7	51.3
	SB	Α	0.50	24.4	53.0	Α	0.52	21.3	67.1
	Overall	D	0.82	19.8	-	С	0.79	28.4	-
Main Street at	WBL	F	1.07	120.2	#101.0	С	0.73	55.9	#68.7
Graham	WBT/R	Α	0.58	14.4	36.4	Α	0.36	9.0	16.9
Avenue/Lees Avenue	NB	Α	0.46	14.0	51.8	Α	0.43	13.8	48.5
	SB	Α	0.40	6.7	22.1	Α	0.41	16.0	49.9
Signalized	Overall	Α	0.60	25.6	-	Α	0.49	19.0	-



1			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	WBL/R	Α	0.27	20.1	20.3	Α	0.09	23.7	10.5
Main Street at Evelyn Avenue Signalized	NBT	В	0.63	15.3	108.3	В	0.64	9.3	m37.1
	SBT	Α	0.34	12.9	m42.6	Α	0.45	12.7	53.1
	Overall	Α	0.50	14.5	-	Α	0.46	11.5	-
	EB	Α	0.11	19.7	8.9	Α	0.04	0.2	0.0
	WBL	Α	0.25	36.2	15.0	Α	0.25	35.5	16.2
Main Street at	WBT/R	Α	0.09	0.3	0.0	Α	0.10	0.4	0.0
Immaculata	NBL	Α	0.09	3.8	m1.3	Α	0.00	10.0	m0.1
HS/des Oblats Avenue	NBT/R	С	0.76	12.8	#169.1	D	0.81	24.6	#162.9
Signalized	SBL	Α	0.07	6.8	4.0	Α	0.19	6.6	6.6
Signanzea	SBT/R	С	0.79	27.0	#190.8	D	0.88	30.1	#235.7
	Overall	В	0.61	19.4	-	В	0.70	26.0	-
	EB	Α	0.20	29.2	14.5	Α	0.13	27.9	10.8
	WBL/T	Α	0.10	32.8	8.8	Α	0.27	37.1	18.8
NA-i- Ct	WBR	Α	0.09	0.5	0.0	Α	0.23	1.5	0.0
Main Street at	NBL	Α	0.03	7.2	2.1	Α	0.04	7.0	2.2
Hazel Street Signalized	NBT/R	С	0.73	22.1	140.7	Α	0.60	15.8	108.3
	SBL	Α	0.23	11.4	m8.3	Α	0.13	4.0	m1.5
	SBT/R	В	0.66	14.3	95.6	С	0.76	7.5	m#137.9
	Overall	В	0.65	18.0	-	С	0.74	11.5	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00 Delay = average driver delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The network intersection operations for the 2025 future total horizon operate similarly to the 2025 future background conditions. No new capacity issues are noted.

Similar to the timing adjustments proposed in the background conditions in Section 7.2, shifting two seconds of split from the north-south phased to the overcapacity westbound left-turn, would reduce the v/c of all movements to 1.00 or below at the intersection of Main Street at Graham Avenue/Lees Avenue during the AM peak hour.

15.2.2 2030 Future Total Network Intersection Operations

The 2030 future total volumes are illustrated in Figure 17 and the network intersection operations are summarized below in Table 19. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix K.



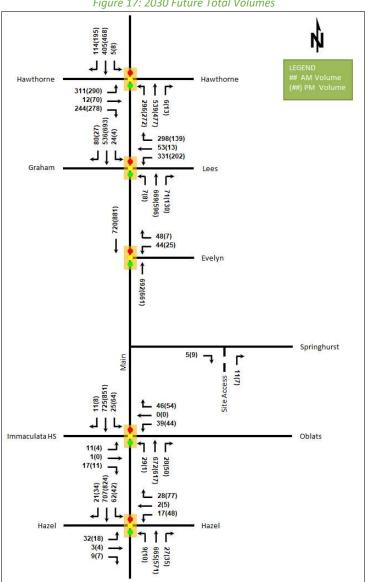


Figure 17: 2030 Future Total Volumes

Table 19: 2030 Future Total Network Intersection Operations

Intersection	Lane		AM Pe	ak Hour		PM Peak Hour				
		LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)	
Main Street at Hawthorne Avenue Signalized	EBL/T	D	0.81	53.5	#131.8	Е	0.99	91.9	#176.5	
	EBR	Α	0.33	3.9	14.6	Α	0.43	5.5	18.7	
	NB	В	0.64	10.1	35.9	Α	0.57	12.8	52.1	
	SB	Α	0.50	24.4	53.0	Α	0.53	21.3	68.0	
	Overall	D	0.83	20.5	-	С	0.79	28.3	-	
Main Street at Graham Avenue/Lees Avenue Signalized	WBL	F	1.41	243.0	#139.1	С	0.74	56.5	#69.5	
	WBT/R	С	0.75	24.6	#68.3	Α	0.36	9.0	17.1	
	NB	Α	0.46	14.0	52.0	Α	0.46	14.1	51.0	
	SB	Α	0.40	6.8	22.4	Α	0.41	16.0	49.9	
	Overall	В	0.69	50.2	-	Α	0.52	19.1	-	



Intersection	Lane		AM Pe	ak Hour			PM Pe	ak Hour	
		LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
Main Street at Evelyn Avenue Signalized	WBL/R	Α	0.27	20.1	20.3	Α	0.09	23.7	10.5
	NBT	В	0.63	15.3	108.3	В	0.64	9.3	m37.1
	SBT	Α	0.34	14.7	m41.1	Α	0.45	12.7	53.1
Signanzea	Overall	Α	0.50	15.3	-	Α	0.46	11.5	-
	EB	Α	0.11	19.7	8.9	Α	0.04	0.2	0.0
Main Street at	WBL	Α	0.25	36.2	15.0	Α	0.25	35.5	16.2
	WBT/R	Α	0.09	0.3	0.0	Α	0.10	0.4	0.0
Immaculata	NBL	Α	0.09	3.8	m1.3	Α	0.00	10.0	m0.1
HS/des Oblats Avenue Signalized	NBT/R	С	0.76	12.8	#169.1	D	0.81	24.6	#162.9
	SBL	Α	0.07	6.8	4.0	Α	0.19	6.6	6.6
	SBT/R	С	0.79	27.0	#190.8	D	0.88	30.1	#235.7
	Overall	В	0.61	19.4	-	В	0.70	26.0	-
Main Street at Hazel Street Signalized	EB	Α	0.20	29.2	14.5	Α	0.13	27.9	10.8
	WBL/T	Α	0.10	32.8	8.8	Α	0.27	37.1	18.8
	WBR	Α	0.09	0.5	0.0	Α	0.23	1.5	0.0
	NBL	Α	0.03	7.2	2.1	Α	0.04	7.0	2.2
	NBT/R	С	0.73	22.1	140.7	Α	0.60	15.8	108.3
	SBL	Α	0.23	11.4	m8.3	Α	0.13	4.0	m1.5
	SBT/R	В	0.66	14.3	95.6	С	0.76	7.5	m#137.9
	Overall	В	0.65	18.0	-	С	0.74	11.5	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00 Delay = average driver delay in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The network intersection operations for the 2030 future total horizon operate similarly to the 2030 future background conditions. The westbound through/right movement at the intersection of Main Street at Graham Avenue/Lees Avenue may exhibit extended queues during the AM peak hour at this horizon. No new capacity issues are noted.

Similar to the timing adjustments proposed in the background conditions in Section 7.2, shifting eight seconds of split from the north-south phased to the overcapacity westbound left-turn, would reduce the v/c of all movements to 1.00 or below at the intersection of Main Street at Graham Avenue/Lees Avenue during the AM peak hour.

15.2.3 Network Intersection MMLOS

Table 20 summarizes the MMLOS analysis for the network intersections within study area. The existing and future conditions for both intersections will be the same and are considered in one row. The intersection analysis is based on the policy area of "within 300 m of a school" given the proximity of Immaculata High School. The MMLOS worksheets has been provided in Appendix H.

Table 20: Study Area Intersection MMLOS Analysis

Intersection	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target
Main Street at Hawthorne Avenue	D	Α	С	В	F	D	D	D	D	E
Main Street at Graham Avenue/Lees Avenue	D	Α	E	В	F	D	С	D	В	E
Main Street at Evelyn Avenue	D	А	Α	С	С	D	-	-	Α	E



Intovocation	Pedesti	rian LOS	Bicyc	le LOS	Trans	it LOS	Truc	k LOS	Auto LOS		
Intersection	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target	
Main Street at Immaculata HS/des Oblats Avenue	D	А	А	С	D	D	-	-	В	E	
Main Street at Hazel Street	D	Α	D	С	D	D	-	-	С	Е	

The pedestrian LOS targets will not be met at the intersections throughout the study area. While pedestrian delay is typically the limiting factor, to meet LOS targets, crossing distances of no more than two lane-widths would be required at each crossing based upon Pedestrian Exposure to Traffic at Signalized Intersection (PETSI) measures.

The bicycle LOS targets will not be met at the intersections of Main Street at Hawthorne Avenue, Main Street at Graham Avenue/Lees Avenue, and Main Street at Hazel Street. To meet bicycle LOS at the intersections, two-stage left turns or left-turn boxes would be required on all approaches of the intersections of Main Street at Graham Avenue/Lees Avenue and separated facilities would be required on the southbound approach at the intersection of Main Street and Hazel Street.

Transit LOS will not be met at the intersection of Main Street at Hawthorne Avenue due to delays on the eastbound approach and of Main Street at Graham Avenue/Lees Avenue due to delays on the westbound approach, where each would need to be reduced to 30 seconds or less to meet LOS targets.

15.2.4 Recommended Design Elements

No study area intersection design elements are proposed as part of this study.

16 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

Proposed Site and Screening

- The proposed site includes 284 apartment units
- A right-in-right-out access is proposed to the parking area, and a loading area with a depressed curb are proposed on Springhurst Avenue
- The development is proposed to be completed as a single phase by 2025
- The trip generation and safety triggers were met for the TIA Screening
- This report is in support of a zoning by-law amendment and site plan application

Existing Conditions

- Main Street, Hawthorne Avenue, and Lees Avenue are arterial roads in the study area
- Sidewalks are provided along both sides of all study area roads, excluding des Oblats Avenue whose north sidewalk is discontinuous
- Cycletracks are provided on both sides of the Main Street south of Lees Avenue and on the east side of
 the road between Lees Avenue and Hawthorne Avenue, a curbside bike lane on both sides of Hawthorne
 Avenue over the Pretoria Bridge, on the south side of Hawthorne Avenue east of Main Street, and on the
 south sides of Lees Avenue and Graham Avenue
- Clegg Street west of Main Street is a neighbourhood bikeway, Hawthorne Avenue west of Colonel By
 Drive, Colonel By Drive between Hawthorne Avenue and Graham Avenue, Graham Avenue, and Lees



Avenue form a spine route and Main Street is a spine route and Hawthorne Avenue east of Colonel By Drive, Echo Drive, and Clegg Street are local routes

- Four transit routes run along Main Street and the site is within a one-kilometre walk of Lees LRT Station
- Within the study area, the majority of collisions were rear end and sideswipe indicating they are lower speed and influenced by congestion
- Generally the study area intersections operate well with the exception of the intersection of Main Street and Hawthorne Avenue during the PM peak hour which may be improved through a reallocation of split

Development Generated Travel Demand

- The proposed development is forecasted produce 114 two-way people trips during the AM peak hour and 113 two-way people trips during the PM peak hour
- Of the forecasted people trips, 28 two-way trips will be vehicle trips during each the AM peak and PM peak hour based on 26% and 25% modal share target
- Of the forecasted trips, 15% are anticipated to travel north, 30% to the south, 10% to the east, and 45 % to the west

Background Conditions

- Growth rates derived from the 2011 to the 2031 model horizons rounded to the nearest 0.25% will be peak-directionally applied to the appropriate major turning movements to and from the identified roadways at the study area intersections
- The study area intersections at future background horizons operate similarly to the existing conditions
 with increasing capacity, delay and queuing issues on the westbound left movement at the intersection
 of Main Street and Graham Avenue/Lees Avenue during the AM peak hour due to background growth,
 which may be alleviated with reallocation of split

Development Design

- Hard surface connections are provided to surrounding pedestrian facilities and bicycle parking is provided internal to the building and via surface racks
- The existing west surface parking lot will be removed, the east lot will be reconfigured for visitor parking, and the bay parking on des Oblates Avenue frontage will be removed
- A 1.5-metre-wide mid-block pathway connection is provided between the sidewalks on Springhurst Avenue and des Oblats Avenue on the west property line
- A 6.0-metre-wide right-in/right-out access with an approximately five-metre long throat is proposed for the site access on Springhurst Avenue
- A loading area accessing Springhurst Avenue through a depressed curb is proposed west of the proposed parking lot access which will permit garbage collection operations

Parking

- The site provides 390 bicycle spaces located below ground and 28 spaces in surface racks, approximately three times the amount required by the zoning by-law
- The zoning by-law prescribes 136 vehicle parking spaces for residents and 27 for visitors, where the site
 plan proposes 27 visitor spaces in the surface lot, leaving a deficit of 136 spaces from the prescribed
 amount in the zoning by-law, and two carshare spaces are reserved on the des Oblats Avenue frontage



- The tenants are unlikely to rely on private auto travel, and the units are marketed to individuals with an urban lifestyle who live or work nearby, and tenants are to be made aware that they will have no parking space early in the rental process, thereby limiting the demand for on-site parking
- Factors contributing to the elimination of tenant reliance on personal auto travel include rapid transit within a one kilometre walk of the site, protected cycling facilities, enhanced pedestrian facilities, transit facilities, and pathways provided nearby, and a high degree of bicycle parking provided on-site, and carshare on-site
- Any desired mitigation of local area parking impacts may be achieved through the public consultation
 process and may include further permitting areas, area enforcement, or modification of signed parking,
 although an evolution of the wider community in orientation towards sustainable modes is in progress

Boundary Street Design

- The pedestrian LOS targets will not meet the area targets along boundary streets, which would require two-metre-wide sidewalks with 0.5-to-2.0-metre-wide boulevards
- Large boulevard widths are not considered appropriate in the highly urban context

Access Intersections Design

- The site will access Springhurst Avenue via a 6.0-metre-wide right-in-right-out access approximately 26
 metres from the adjacent property line, and an approximately 6.4-metre-wide loading area is proposed
 via a depressed curb to the west of the parking lot access
- Concerning the site access intersection, while no background traffic volumes were available, Springhurst
 Avenue approach operates as free-flow, and even moderate local one-way volumes interacting with a
 right-in/right-out access to a lot of 27 parking stalls would be anticipated to operate with good level of
 service on the minor approach
- No design elements for the site accesses on Springhurst Avenue are proposed outside of the typical private approach considerations

TDM

- Supportive TDM measures to be included within the proposed development should include:
 - Display local area maps with walking and cycling routes, and transit route information and schedules at major entrances
 - Provide a multimodal travel option information package to new residents
 - Contract with providers to install on-site bikeshare (or other micro-mobility, e.g. scootershare)
 and carshare spaces
 - Provide a permanent bike repair station

NTM

- One parking lot is proposed as being removed on Springhurst Avenue as part of the redevelopment, and traffic is anticipated to be reduced on the roadway as a result
- The total auto trip generation of the site is anticipated to be 28 two-way vehicles per peak hour, which would comprise 23% of the local road classification thresholds
- Therefore no impacts to the classification or function of study area local roads is anticipated as a result of the proposed development



Transit

- The proposed development is anticipated to generate an additional 35 AM peak hour transit trips and 26
 PM peak hour transit trips
- Negligible service changes are anticipated as being required to accommodate site-generated transit trips
- No increase in transit LOS at study area intersections is resultant from the addition of site traffic

Network Intersection Design

- Generally, the network intersections will operate similarly to the future background conditions, where
 additional queueing may be noted on the westbound through/right movement at the intersection of
 Mains Street at Graham Avenue/Lees Avenue during the AM peak hour at the 2030 future total horizon
- A reallocation of split of two second at the 2025 horizon and eight seconds at the 2030 horizon from the
 north-south phases to the overcapacity westbound phase would reduce the v/c of all movements to 1.00
 or below during the AM peak hour at the intersection of Main Street at Graham Avenue/Lees Avenue at
 the future total horizons
- The pedestrian LOS targets will not be met at the intersections throughout the study area, and would require a maximum of two lanes at a crossing to meet a LOS A based upon PETSI measures
- The bicycle LOS targets will not be met at the intersections of Main Street at Hawthorne Avenue, Main Street at Graham Avenue/Lees Avenue, and Main Street at Hazel Street, where approaches would require two-stage left-turns or turn boxes, and separated facilities to meet targets
- Transit LOS will not be met at the intersection of Main Street at Hawthorne Avenue due to delays on the
 eastbound approach and at the intersection Main Street at Graham Avenue/Lees Avenue due to delays
 on the westbound approach

17 Conclusion

It is recommended that, from a transportation perspective, the proposed development applications proceed.

Prepared By:

John Kingsley, EIT

Transportation Engineering-Intern

Reviewed By:



Andrew Harte, P.Eng.

Senior Transportation Engineer



Appendix A

TIA Screening Form and PM Certification Form





City of Ottawa 2017 TIA Guidelines Step 1 - Screening Form Date: 26-Apr-21
Project Number: 2021-004
Project Reference: 15 Oblats

1.1 Description of Proposed Development	
Municipal Address	15 Oblats Avenue
Description of Location	Existing t-shaped building
Land Use Classification	Residential Fourth and Fifth Densit (R4UD, R5B)
Development Size	339 residential units
A	Existing penpendicular parking on Oblats Ave, existing
Accesses	parking access on Springhurst Ave
Phase of Development	Single Phase
Buildout Year	2025
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Townhomes or apartments
Development Size	339 Units
Trip Generation Trigger	Yes

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is	
designated as part of the City's Transit Priority, Rapid Transit or Spine	No
Bicycle Networks?	
Is the development in a Design Priority Area (DPA) or Transit-oriented	Ne
Development (TOD) zone?	No
Location Trigger	No

1.4. Safety Triggers	
Are posted speed limits on a boundary street 80 km/hr or greater?	No
Are there any horizontal/vertical curvatures on a boundary street limits	No
sight lines at a proposed driveway?	INO
Is the proposed driveway within the area of influence of an adjacent traffic	
signal or roundabout (i.e. within 300 m of intersection in rural conditions,	Yes
or within 150 m of intersection in urban/ suburban conditions)?	
Is the proposed driveway within auxiliary lanes of an intersection?	No
Does the proposed driveway make use of an existing median break that	No
serves an existing site?	NO
Is there is a documented history of traffic operations or safety concerns on	No
the boundary streets within 500 m of the development?	INO
Does the development include a drive-thru facility?	No
Safety Trigger	Yes



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$] is either transportation engineering $\sqrt{\text{or}}$ or transportation planning \square .
- License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at Ottawa (City)	this 20 day of September	, 2018
Name:	Andrew Harte (Places Print)	
Professional Title:	(Please Print) Professional Engineer	
Signatura	of Individed Market that she shows four criteria	
Signature	of Individual certifier that s/he meets the above four criteria	

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Appendix B

Turning Movement Counts





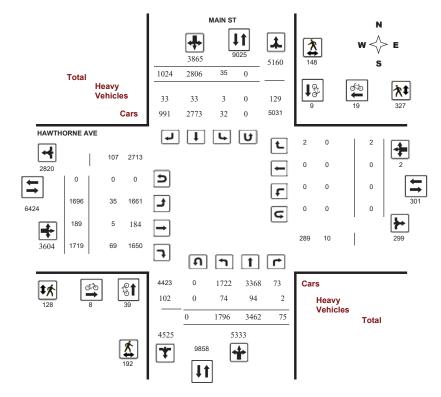
Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study Diagram



5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

Survey Date: Tuesday, March 03, 2020 Start Time: 07:00

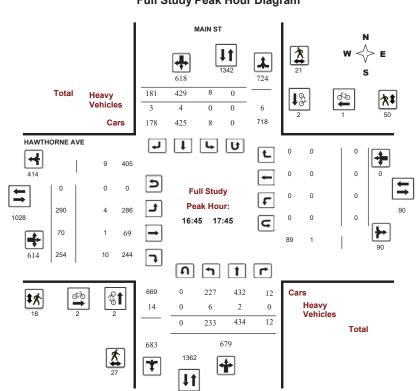
Full Study Peak Hour Diagram

WO No:

Device:

39570

Miovision



5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK

March 9, 2020 Page 1 of 8 March 9, 2020 Page 2 of 8

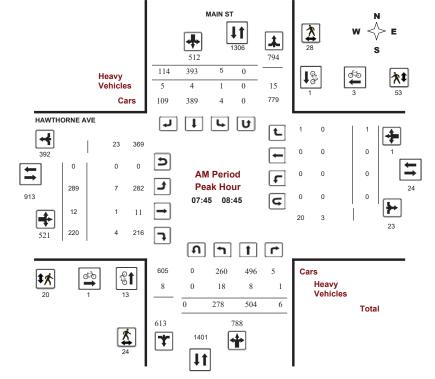


Turning Movement Count - Peak Hour Diagram

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision



Comments 5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK



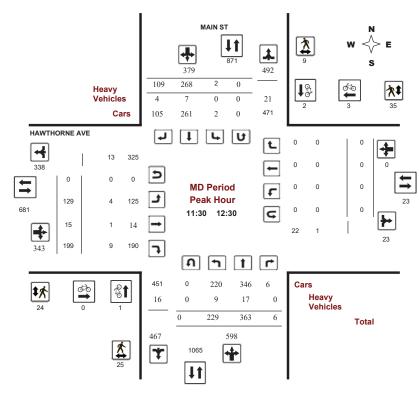
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision



Comments 5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK

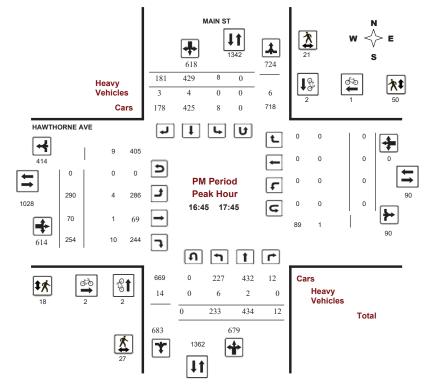


Turning Movement Count - Peak Hour Diagram

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision



Comments 5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, March 03, 2020 Total Observed U-Turns

Northbound: 0 Southbound: 0 1,000

Eastbound: () Westbound: ()

								Eastboul	na: ()		vves	tbound	0						
			N	IAIN S	Γ							HAW	THOR	NE AVE					
	No	rthbour	nd		So	uthbou	und			Е	astbo	und		W	estbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Gran Tota
07:00 08:00	217	462	5	684	3	380	64	447	1131	174	4	194	372	0	0	0	0	372	150
08:00 09:00	273	487	5	765	4	399	126	529	1294	299	13	212	524	0	0	1	1	525	181
09:00 10:00	229	442	14	685	4	285	109	398	1083	164	11	170	345	0	0	0	0	345	142
11:30 12:30	229	363	6	598	2	268	109	379	977	129	15	199	343	0	0	0	0	343	132
12:30 13:30	137	363	11	511	1	297	105	403	914	138	7	189	334	0	0	0	0	334	124
15:00 16:00	229	492	11	732	7	331	149	487	1219	205	20	267	492	0	0	0	0	492	171
16:00 17:00	237	420	13	670	6	416	189	611	1281	307	56	246	609	0	0	0	0	609	189
17:00 18:00	245	433	10	688	8	430	173	611	1299	280	63	242	585	0	0	1	1	586	188
Sub Total	1796	3462	75	5333	35	2806	1024	3865	9198	1696	189	1719	3604	0	0	2	2	3606	1280
U Turns				0				0	0				0				0	0	(
Total	1796	3462	75	5333	35	2806	1024	3865	9198	1696	189	1719	3604	0	0	2	2	3606	1280
EQ 12Hr	2496	4812	104	7413	49	3900	1423	5372	12785	2357	263	2389	5010	0	0	3	3	5012	1779
Note: These v	/alues a	re calcul	ated by	/ multiply	ring the	totals b	y the a	ppropriat	e expans	sion fact	tor.			1.39					
AVG 12Hr	2353	4535	98	6986	46	3676	1341	5063	12785	2222	248	2252	4721	0	0	3	3	5012	1779
Note: These v	/olumes	are calc	ulated	by multip	olying t	he Equiv	/alent 1	2 hr. tota	ıls by the	AADT	factor.			1					
AVG 24Hr	3082	5941	129	9152	60	4815	1757	6633	15785	2911	324	2950	6185	0	0	3	3	6188	2197
Note: These	olumes/	are calc	ulated	by multip	olying t	he Aver	age Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					
Note: U-Tur	ns prov	rided for	appro	ach tot	als. Re	efer to '	U-Turn	' Repor	t for spe	ecific br	eakdov	wn.							

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Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

MAIN ST HAWTHORNE AVE bound Southbound Eastbound West

	N	orthbou	ınd		So	outhbou	nd			E	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	41	97	0	138	0	104	11	115	521	31	2	36	69	0	0	0	0	521	322
07:15 07:30	60	107	1	168	0	86	10	96	524	29	1	38	68	0	0	0	0	524	332
07:30 07:45	54	126	1	181	2	96	13	111	629	54	0	61	115	0	0	0	0	629	407
07:45 08:00	62	132	3	197	1	94	30	125	667	60	1	59	120	0	0	0	0	667	442
08:00 08:15	66	114	2	182	2	103	30	135	673	83	5	55	143	0	0	1	1	673	461
08:15 08:30	78	134	0	212	0	112	27	139	723	75	3	51	129	0	0	0	0	723	480
08:30 08:45	72	124	1	197	2	84	27	113	644	71	3	55	129	0	0	0	0	644	439
08:45 09:00	57	115	2	174	0	100	42	142	652	70	2	51	123	0	0	0	0	652	439
09:00 09:15	59	128	3	190	0	68	32	100	572	45	2	41	88	0	0	0	0	572	378
09:15 09:30	62	106	4	172	1	66	20	87	516	42	4	43	89	0	0	0	0	516	348
09:30 09:45	53	103	2	158	2	77	26	105	529	37	2	49	88	0	0	0	0	529	351
09:45 10:00	55	105	5	165	1	74	31	106	527	40	3	37	80	0	0	0	0	527	351
11:30 11:45	61	76	1	138	1	70	24	95	449	25	5	45	75	0	0	0	0	449	308
11:45 12:00	56	82	1	139	0	63	36	99	475	45	6	47	98	0	0	0	0	475	336
12:00 12:15	54	86	3	143	1	66	23	90	475	35	2	55	92	0	0	0	0	475	325
12:15 12:30	58	119	1	178	0	69	26	95	537	24	2	52	78	0	0	0	0	537	351
12:30 12:45	42	85	3	130	0	74	22	96	464	36	1	43	80	0	0	0	0	464	306
12:45 13:00	35	91	2	128	1	61	29	91	462	39	3	52	94	0	0	0	0	462	313
13:00 13:15	26	99	2	127	0	80	31	111	504	36	2	51	89	0	0	0	0	504	327
13:15 13:30	34	88	4	126	0	82	23	105	471	27	1	43	71	0	0	0	0	471	302
15:00 15:15	40	131	2	173	2	73	20	95	558	31	4	55	90	0	0	0	0	558	358
15:15 15:30	57	130	2	189	2	97	32	131	663	44	3	72	119	0	0	0	0	663	439
15:30 15:45	66	110	2	178	1	75	49	125	619	60	7	71	138	0	0	0	0	619	441
15:45 16:00	66	121	5	192	2	86	48	136	674	70	6	69	145	0	0	0	0	674	473
16:00 16:15	52	111	1	164	1	93	42	136	642	85	8	53	146	0	0	0	0	642	446
16:15 16:30	56	103	4	163	2	114	40	156	677	72	11	69	152	0	0	0	0	677	471
16:30 16:45	65	106	4	175	2	110	41	153	674	71	19	59	149	0	0	0	0	674	477
16:45 17:00	64	100	4	168	1	99	66	166	677	79	18	65	162	0	0	0	0	677	496
17:00 17:15	47	114	1	162	5	99	32	136	653	74	17	68	159	0	0	0	0	653	457
17:15 17:30	65	109	5	179	1	95	33	129	640	76	20	52	148	0	0	0	0	640	456
17:30 17:45	57	111	2	170	1	136	50	187	734	61	15	69	145	0	0	0	0	734	502
17:45 18:00	76	99	2	177	1	100	58	159	658	69	11	53	133	0	0	1	1	658	470
Total:	1796	3462	75	5333	35	2806	1024	3865	18883	1696	189	1719	3604	0	0	2	2	18883	12,804

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study Cyclist Volume

		MAIN ST	-	ı ı	HAWTHORNE AVE							
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total					
07:00 07:15	2	0	2	0	1	1	3					
07:15 07:30	1	0	1	0	1	1	2					
07:30 07:45	3	0	3	0	1	1	4					
07:45 08:00	3	1	4	1	0	1	5					
08:00 08:15	4	0	4	0	1	1	5					
08:15 08:30	2	0	2	0	1	1	3					
08:30 08:45	4	0	4	0	1	1	5					
08:45 09:00	4	0	4	0	0	0	4					
09:00 09:15	4	0	4	0	3	3	7					
09:15 09:30	2	0	2	0	1	1	3					
09:30 09:45	0	0	0	0	0	0	0					
09:45 10:00	0	1	1	1	0	1	2					
11:30 11:45	0	0	0	0	0	0	0					
11:45 12:00	0	2	2	0	1	1	3					
12:00 12:15	0	0	0	0	0	0	0					
12:15 12:30	1	0	1	0	2	2	3					
12:30 12:45	0	0	0	0	0	0	0					
12:45 13:00	0	0	0	0	0	0	0					
13:00 13:15	3	0	3	0	3	3	6					
13:15 13:30	0	0	0	0	0	0	0					
15:00 15:15	0	1	1	0	0	0	1					
15:15 15:30	0	1	1	1	0	1	2					
15:30 15:45	3	0	3	2	1	3	6					
15:45 16:00	1	0	1	0	0	0	1					
16:00 16:15	0	1	1	0	1	1	2					
16:15 16:30	0	0	0	0	0	0	0					
16:30 16:45	0	0	0	1	0	1	1					
16:45 17:00	1	1	2	0	0	0	2					
17:00 17:15	0	0	0	0	0	0	0					
17:15 17:30	1	1	2	2	1	3	5					
17:30 17:45	0	0	0	0	0	0	0					
17:45 18:00	0	0	0	0	0	0	0					
Total	39	9	48	8	19	27	75					

March 9, 2020 Page 4 of 8 March 9, 2020 Page 5 of 8



Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

MAIN ST HAWTHORNE AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	2	1	3	2	5	7	10
07:15 07:30	3	5	8	0	5	5	13
07:30 07:45	8	9	17	5	6	11	28
07:45 08:00	5	7	12	3	8	11	23
08:00 08:15	6	10	16	7	11	18	34
08:15 08:30	4	5	9	5	27	32	41
08:30 08:45	9	6	15	5	7	12	27
08:45 09:00	3	4	7	4	3	7	14
09:00 09:15	4	4	8	4	8	12	20
09:15 09:30	4	2	6	3	7	10	16
09:30 09:45	4	5	9	4	14	18	27
09:45 10:00	4	0	4	3	2	5	9
11:30 11:45	6	2	8	6	13	19	27
11:45 12:00	7	3	10	5	7	12	22
12:00 12:15	6	2	8	5	7	12	20
12:15 12:30	6	2	8	8	8	16	24
12:30 12:45	8	6	14	1	15	16	30
12:45 13:00	5	6	11	6	7	13	24
13:00 13:15	3	4	7	3	3	6	13
13:15 13:30	5	2	7	9	10	19	26
15:00 15:15	2	10	12	4	9	13	25
15:15 15:30	17	9	26	2	31	33	59
15:30 15:45	5	3	8	1	9	10	18
15:45 16:00	2	0	2	2	19	21	23
16:00 16:15	9	5	14	2	6	8	22
16:15 16:30	10	7	17	2	5	7	24
16:30 16:45	8	5	13	5	9	14	27
16:45 17:00	4	3	7	4	10	14	21
17:00 17:15	10	2	12	6	11	17	29
17:15 17:30	5	8	13	5	11	16	29
17:30 17:45	8	8	16	3	18	21	37
17:45 18:00	10	3	13	4	16	20	33
Total	. 192	148	340	128	327	455	795

5478558 - MAR 3, 2020 - 8HRS - VANESSA BLACK



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

MAIN ST HAWTHORNE AVE

	No	orthbo	und		So	outhbou	ınd			E	astbour	nd		W	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR	Grand Total
07:00 07:15	2	3	0	6	0	0	2	5	11	0	0	1	5	0	0	0	0	5	8
07:15 07:30	5	3	0	13	0	2	0	5	18	0	0	3	8	0	0	0	0	8	13
07:30 07:45	3	9	1	15	0	1	2	13	28	1	0	1	7	0	0	0	1	8	18
07:45 08:00	5	2	0	10	0	1	1	5	15	1	0	2	9	0	0	0	0	9	12
08:00 08:15	1	1	0	3	1	1	1	6	9	2	0	0	4	0	0	0	1	5	7
08:15 08:30	6	2	0	10	0	1	2	6	16	1	0	1	10	0	0	0	0	10	13
08:30 08:45	6	3	1	12	0	1	1	8	20	3	1	1	12	0	0	0	2	14	17
08:45 09:00	2	3	0	9	0	1	2	8	17	2	0	3	9	0	0	0	0	9	13
09:00 09:15	4	2	0	16	0	3	3	8	24	0	0	7	14	0	0	0	0	14	19
09:15 09:30	3	6	0	13	0	2	1	10	23	1	0	2	7	0	0	0	0	7	15
09:30 09:45	3	7	0	12	0	0	0	10	22	3	0	2	8	0	0	0	0	8	15
09:45 10:00	2	3	0	11	0	2	1	6	17	0	0	4	7	0	0	0	0	7	12
11:30 11:45	1	6	0	10	0	1	1	10	20	2	0	2	6	0	0	0	0	6	13
11:45 12:00	1	4	0	9	0	2	0	7	16	1	1	2	5	0	0	0	1	6	11
12:00 12:15	4	3	0	8	0	0	1	4	12	0	0	1	6	0	0	0	0	6	9
12:15 12:30	3	4	0	15	0	4	2	11	26	1	0	4	10	0	0	0	0	10	18
12:30 12:45	2	3	0	6	0	0	0	4	10	1	0	1	4	0	0	0	0	4	7
12:45 13:00	1	5	0	8	0	0	0	8	16	3	0	2	6	0	0	0	0	6	11
13:00 13:15	1	5	0	7	0	1	1	7	14	0	0	0	2	0	0	0	0	2	8
13:15 13:30	1	2	0	5	0	1	1	4	9	0	0	1	3	0	0	0	0	3	6
15:00 15:15	0	6	0	9	0	1	0	8	17	1	0	2	3	0	0	0	0	3	10
15:15 15:30	2	5	0	10	1	2	1	10	20	1	1	1	6	0	0	0	2	8	14
15:30 15:45	3	1	0	10	0	1	1	4	14	1	1	5	11	0	0	0	1	12	13
15:45 16:00	3	0	0	5	1	0	1	4	9	2	0	2	8	0	0	0	1	9	9
16:00 16:15	0	2	0	5	0	1	3	6	11	0	0	2	5	0	0	0	0	5	8
16:15 16:30	2	2	0	7	0	0	1	5	12	2	0	3	8	0	0	0	0	8	10
16:30 16:45	1	0	0	3	0	0	1	1	4	0	0	2	4	0	0	0	0	4	4
16:45 17:00	3	1	0	7	0	1	1	4	11	1	0	2	7	0	0	0	0	7	9
17:00 17:15	1	1	0	6	0	2	0	4	10	1	0	2	4	0	0	0	0	4	7
17:15 17:30	1	0	0	4	0	1	2	3	7	0	0	2	5	0	0	0	0	5	6
17:30 17:45	1	0	0	5	0	0	0	2	7	2	1	4	8	0	0	0	1	9	8
17:45 18:00	1	0	0	3	0	0	0	2	5	2	0	2	5	0	0	0	0	5	5
Total: None	74	94	2	272	3	33	33	198	470	35	5	69	216	0	0	0	10	226	348

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Turning Movement Count - Study Results

HAWTHORNE AVE @ MAIN ST

 Survey Date:
 Tuesday, March 03, 2020
 WO No:
 39570

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total MAIN ST HAWTHORNE AVE

Time	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
T	otal	0	0	0	0	0



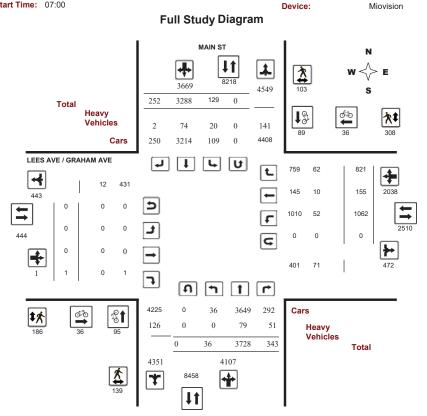
Transportation Services - Traffic Services

Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Micvision



March 9, 2020 Page 8 of 8 June 19, 2020 Page 1 of 8



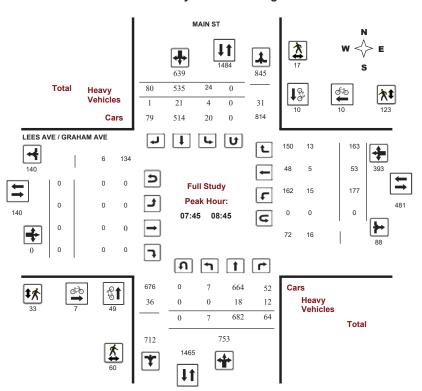
Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram





Transportation Services - Traffic Services

Work Order

38085

Turning Movement Count - Full Study Summary (No AADT) Report

LEES AVE / GRAHAM AVE @ MAIN ST

Survey Date: Tuesday, November 06, 2018

Total Observed U-Turns

Northbound: 0

Eastbound: 0

Southbound: 0
Westbound: 0

	Pe		

				MAIN	ST						LE	ES A	/E / GI	RAHAI	M AVE				
_		Vorthb	ound		5	Southb	ound		_		Eastbo	und		1	Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
15:00 16:00	8	671	63	742	22	537	26	585	1327	0	0	1	1	159	13	124	296	297	1624
16:00 17:00	5	638	73	716	16	574	42	632	1348	0	0	0	0	156	22	134	312	312	1660
17:00 18:00	6	580	50	636	28	585	41	654	1290	0	0	0	0	214	24	129	367	367	1657
Sub Total	19	1889	186	2094	66	1696	109	1871	3965	0	0	1	1	529	59	387	975	976	4941
U Turns				0				0	0				0				0	0	0
Total	19	1889	186	2094	66	1696	109	1871	3965	0	0	1	1	529	59	387	975	976	4941

Survey Date: Tuesday, November 06, 2018

Total Observed U-Turns

Northbound: 0

Southbound: ()

Eastbound: 0 Westbound: 0

AM Period

				MAIN	ST						LE	ES A	VE / GI	RAHAI	M AVE				
-	-	Vorthb	ound		5	Southb	ound		_		Eastbo	ound		1	Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	3	593	50	646	17	573	68	658	1304	0	0	0	0	175	36	138	349	349	1653
08:00 09:00	8	687	66	761	21	537	54	612	1373	0	0	0	0	188	43	161	392	392	1765
09:00 10:00	6	559	41	606	25	482	21	528	1134	0	0	0	0	170	17	135	322	322	1456
Sub Total	17	1839	157	2013	63	1592	143	1798	3811	0	0	0	0	533	96	434	1063	1063	4874
U Turns				0				0	0				0				0	0	0
Total	17	1839	157	2013	63	1592	143	1798	3811	0	0	0	0	533	96	434	1063	1063	4874

Comments

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown

 June 19, 2020
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 2020-Jun-19
 Page 1 of 1



Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

MAIN ST LEES AVE / GRAHAM AVE Northbound LT LT ST RT Time Period LT ST 483 434 352 122 351 122 409 462 42 3 32 383 129 142 426 11 36 112

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Miovision

Full Study Cyclist Volume

		MAIN ST		LEE	S AVE / GRAHA	AM AVE	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	3	1	4	1	4	5	9
07:15 07:30	5	2	7	2	6	8	15
07:30 07:45	6	3	9	0	1	1	10
07:45 08:00	9	0	9	2	3	5	14
08:00 08:15	14	4	18	3	5	8	26
08:15 08:30	14	3	17	1	1	2	19
08:30 08:45	12	3	15	1	1	2	17
08:45 09:00	4	1	5	2	0	2	7
09:00 09:15	5	2	7	0	1	1	8
09:15 09:30	3	1	4	1	0	1	5
09:30 09:45	1	0	1	0	3	3	4
09:45 10:00	0	1	1	0	0	0	1
15:00 15:15	1	2	3	2	0	2	5
15:15 15:30	2	5	7	3	2	5	12
15:30 15:45	0	3	3	0	0	0	3
15:45 16:00	5	5	10	0	0	0	10
16:00 16:15	0	7	7	0	0	0	7
16:15 16:30	1	9	10	1	1	2	12
16:30 16:45	4	5	9	2	3	5	14
16:45 17:00	1	6	7	2	1	3	10
17:00 17:15	3	4	7	3	0	3	10
17:15 17:30	0	11	11	7	1	8	19
17:30 17:45	2	3	5	3	0	3	8
17:45 18:00	0	8	8	0	3	3	11
Total	95	89	184	36	36	72	256

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Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

MAIN ST LEES AVE / GRAHAM AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	4	7	4	5	9	16
07:15 07:30	0	6	6	4	21	25	31
07:30 07:45	4	3	7	5	14	19	26
07:45 08:00	34	5	39	11	52	63	102
08:00 08:15	22	4	26	7	26	33	59
08:15 08:30	2	6	8	11	13	24	32
08:30 08:45	2	2	4	4	32	36	40
08:45 09:00	3	2	5	5	13	18	23
09:00 09:15	2	5	7	6	9	15	22
09:15 09:30	3	2	5	0	6	6	11
09:30 09:45	3	4	7	5	8	13	20
09:45 10:00	2	1	3	4	2	6	9
15:00 15:15	1	2	3	5	2	7	10
15:15 15:30	3	4	7	5	4	9	16
15:30 15:45	2	2	4	7	15	22	26
15:45 16:00	4	2	6	5	7	12	18
16:00 16:15	6	6	12	8	8	16	28
16:15 16:30	2	8	10	9	13	22	32
16:30 16:45	6	8	14	13	12	25	39
16:45 17:00	6	4	10	18	16	34	44
17:00 17:15	5	11	16	14	5	19	35
17:15 17:30	10	8	18	18	12	30	48
17:30 17:45	5	2	7	12	7	19	26
17:45 18:00	9	2	11	6	6	12	23
Total	139	103	242	186	308	494	736



Transportation Services - Traffic Services

Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

Survey Date: Tuesday, November 06, 2018 WO No: 38085
Start Time: 07:00 Device: Miovision

Start Time	: 07	7:00											Dev	ice:			Mi	oisivo	า
						F	ull S	Stud	v He	avv	Vel	nicle	es						
			M	AIN S	ST.					•			/ GR	AHAN	I AVE				
	N	orthbo	und		Sc	outhbou	ınd			F	astbour	nd		W	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	2	2	4	1	1	0	2	6	0	0	0	0	0	0	4	4	4	10
07:15 07:30	0	2	0	2	1	3	0	4	6	0	0	0	0	2	0	4	6	6	12
07:30 07:45	0	3	4	7	0	8	1	9	16	0	0	0	0	2	0	3	5	5	21
07:45 08:00	0	6	3	9	2	3	1	6	15	0	0	0	0	4	1	3	8	8	23
08:00 08:15	0	4	4	8	1	5	0	6	14	0	0	0	0	6	1	3	10	10	24
08:15 08:30	0	4	1	5	0	6	0	6	11	0	0	0	0	2	1	4	7	7	18
08:30 08:45	0	4	4	8	1	7	0	8	16	0	0	0	0	3	2	3	8	8	24
08:45 09:00	0	1	3	4	0	5	0	5	9	0	0	0	0	3	1	3	7	7	16
09:00 09:15	0	8	4	12	2	2	0	4	16	0	0	0	0	2	2	4	8	8	24
09:15 09:30	0	4	5	9	1	0	0	1	10	0	0	0	0	2	0	2	4	4	14
09:30 09:45	0	2	3	5	0	6	0	6	11	0	0	0	0	1	0	2	3	3	14
09:45 10:00	0	4	2	6	1	4	0	5	11	0	0	0	0	4	0	2	6	6	17
15:00 15:15	0	4	2	6	0	2	0	2	8	0	0	0	0	0	0	3	3	3	11
15:15 15:30	0	5	3	8	1	1	0	2	10	0	0	0	0	3	0	1	4	4	14
15:30 15:45	0	6	1	7	0	4	0	4	11	0	0	0	0	4	2	3	9	9	20
15:45 16:00	0	2	1	3	1	2	0	3	6	0	0	0	0	3	0	4	7	7	13
16:00 16:15	0	6	2	8	2	3	0	5	13	0	0	0	0	1	0	2	3	3	16
16:15 16:30	0	2	1	3	2	2	0	4	7	0	0	0	0	1	0	2	3	3	10
16:30 16:45	0	1	2	3	1	1	0	2	5	0	0	0	0	1	0	0	1	1	6
16:45 17:00	0	2	1	3	0	2	0	2	5	0	0	0	0	1	0	2	3	3	8
17:00 17:15	0	3	0	3	1	2	0	3	6	0	0	0	0	1	0	1	2	2	8
17:15 17:30	0	1	1	2	0	2	0	2	4	0	0	0	0	3	0	1	4	4	8
17:30 17:45	0	2	0	2	1	1	0	2	4	0	0	0	0	1	0	4	5	5	9
17:45 18:00	0	1	2	3	1	2	0	3	6	0	0	0	0	2	0	2	4	4	10
Total: None	0	79	51	130	20	74	2	96	226	0	0	0	0	52	10	62	124	124	350

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Turning Movement Count - Study Results

LEES AVE / GRAHAM AVE @ MAIN ST

 Survey Date:
 Tuesday, November 06, 2018
 WO No:
 38085

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total MAIN ST LEES AVE / GRAHAM AVE

Time	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
T	otal	0	0	0	0	0



Transportation Services - Traffic Services

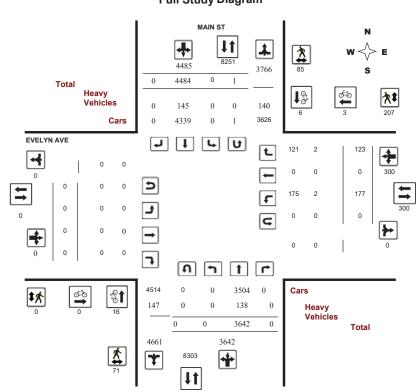
Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

Survey Date: Tuesday, March 07, 2017
Start Time: 07:00

WO No: Device: 36734 Miovision

Full Study Diagram



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May 10, 2021

Transportation Services - Traffic Services

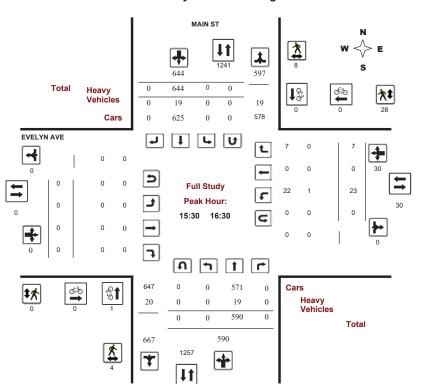
Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram





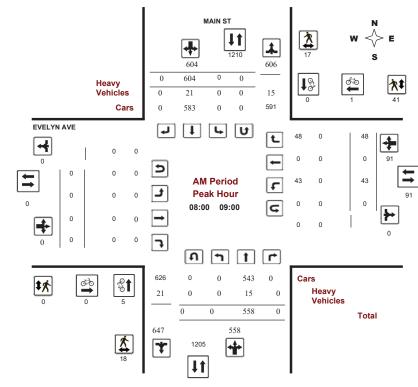
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision



Comments

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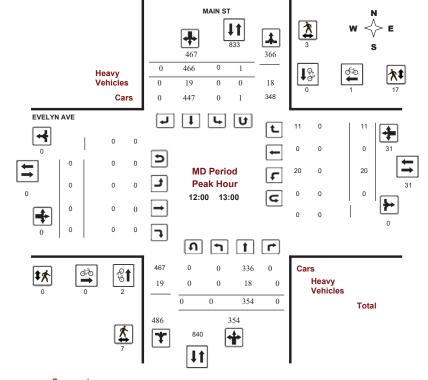


Turning Movement Count - Peak Hour Diagram

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision



Comments



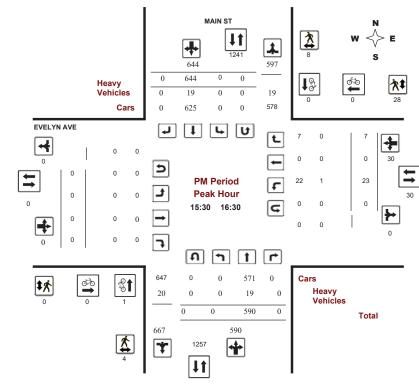
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision



Comments



Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

Survey Date: Tuesday, March 07, 2017 WO No: 36734 Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, March 07, 2017 **Total Observed U-Turns AADT Factor** Southbound: 1

> Westbound: 0 Eastbound: 0

1.00

			N	IAIN S	Γ		EVELYN AVE												
	No	rthbour	nd		So	uthbou	nd			Ea	astbou	nd		W	estbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	399	0	399	0	638	0	638	1037	0	0	0	0	23	0	15	38	38	1075
08:00 09:00	0	558	0	558	0	604	0	604	1162	0	0	0	0	43	0	48	91	91	1253
09:00 10:00	0	422	0	422	0	443	0	443	865	0	0	0	0	14	0	10	24	24	889
11:30 12:30	0	357	0	357	0	446	0	446	803	0	0	0	0	15	0	11	26	26	829
12:30 13:30	0	333	0	333	0	456	0	456	789	0	0	0	0	19	0	8	27	27	816
15:00 16:00	0	567	0	567	0	544	0	544	1111	0	0	0	0	25	0	13	38	38	1149
16:00 17:00	0	533	0	533	0	691	0	691	1224	0	0	0	0	13	0	6	19	19	1243
17:00 18:00	0	473	0	473	0	662	0	662	1135	0	0	0	0	25	0	12	37	37	1172
Sub Total	0	3642	0	3642	0	4484	0	4484	8126	0	0	0	0	177	0	123	300	300	8426
U Turns	0			0	1			1	1	0			0	0			0	0	1
Total	0	3642	0	3642	1	4484	0	4485	8127	0	0	0	0	177	0	123	300	300	8427
EQ 12Hr	0	5062	0	5062	1	6233	0	6234	11296	0	0	0	0	246	0	171	417	417	11713
Note: These v	alues a	re calcul	lated by	/ multiply	ring the	totals b	y the a	ppropria	te expans	ion facto	or.			1.39					
AVG 12Hr	. 0	5062	0	5062	1	6233	0	6234	11296	0	0	0	0	246	0	171	417	417	11713
Note: These v	olumes	are calc	ulated	by multip	olying t	he Equiv	alent 1	2 hr. tota	als by the	AADT fa	actor.			1.00					
AVG 24Hr	0	6631	0	6631	1	8165	0	8166	14797	0	0	0	0	322	0	224	546	546	15343
Note: These v	olumes	are calc	ulated	by multip	olying t	he Avera	ige Dai	ly 12 hr.	totals by	12 to 24	expans	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

Survey Date: Tuesday, March 07, 2017 WO No: 36734 Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

MAIN ST **EVELYN AVE**

	N	orthbo	und		Sc	outhbou	nd	Eastbound			We	estbour	nd						
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	61	0	61	0	156	0	156	217	0	0	0	0	6	0	0	6	6	223
07:15 07:30	0	93	0	93	0	148	0	148	241	0	0	0	0	3	0	2	5	5	246
07:30 07:45	0	104	0	104	0	191	0	191	295	0	0	0	0	6	0	2	8	8	303
07:45 08:00	0	141	0	141	0	143	0	143	284	0	0	0	0	8	0	11	19	19	303
08:00 08:15	0	128	0	128	0	144	0	144	272	0	0	0	0	10	0	18	28	28	300
08:15 08:30	0	143	0	143	0	151	0	151	294	0	0	0	0	13	0	16	29	29	323
08:30 08:45	0	155	0	155	0	155	0	155	310	0	0	0	0	8	0	5	13	13	323
08:45 09:00	0	132	0	132	0	154	0	154	286	0	0	0	0	12	0	9	21	21	307
09:00 09:15	0	125	0	125	0	103	0	103	228	0	0	0	0	6	0	3	9	9	237
09:15 09:30	0	104	0	104	0	123	0	123	227	0	0	0	0	3	0	3	6	6	233
09:30 09:45	0	99	0	99	0	115	0	115	214	0	0	0	0	1	0	4	5	5	219
09:45 10:00	0	94	0	94	0	102	0	102	196	0	0	0	0	4	0	0	4	4	200
11:30 11:45	0	97	0	97	0	107	0	107	204	0	0	0	0	5	0	2	7	7	211
11:45 12:00	0	90	0	90	0	105	0	105	195	0	0	0	0	2	0	1	3	3	198
12:00 12:15	0	89	0	89	0	109	0	109	198	0	0	0	0	3	0	2	5	5	203
12:15 12:30	0	81	0	81	1	125	0	126	207	0	0	0	0	5	0	6	11	11	218
12:30 12:45	0	82	0	82	0	121	0	121	203	0	0	0	0	6	0	1	7	7	210
12:45 13:00	0	102	0	102	0	111	0	111	213	0	0	0	0	6	0	2	8	8	221
13:00 13:15	0	68	0	68	0	110	0	110	178	0	0	0	0	4	0	4	8	8	186
13:15 13:30	0	81	0	81	0	114	0	114	195	0	0	0	0	3	0	1	4	4	199
15:00 15:15	0	127	0	127	0	109	0	109	236	0	0	0	0	6	0	4	10	10	246
15:15 15:30	0	147	0	147	0	126	0	126	273	0	0	0	0	6	0	3	9	9	282
15:30 15:45	0	130	0	130	0	170	0	170	300	0	0	0	0	6	0	5	11	11	311
15:45 16:00	0	163	0	163	0	139	0	139	302	0	0	0	0	7	0	1	8	8	310
16:00 16:15	0	146	0	146	0	166	0	166	312	0	0	0	0	5	0	1	6	6	318
16:15 16:30	0	151	0	151	0	169	0	169	320	0	0	0	0	5	0	0	5	5	325
16:30 16:45	0	121	0	121	0	184	0	184	305	0	0	0	0	1	0	4	5	5	310
16:45 17:00	0	115	0	115	0	172	0	172	287	0	0	0	0	2	0	1	3	3	290
17:00 17:15	0	146	0	146	0	178	0	178	324	0	0	0	0	5	0	3	8	8	332
17:15 17:30	0	130	0	130	0	163	0	163	293	0	0	0	0	5	0	4	9	9	302
17:30 17:45	0	101	0	101	0	192	0	192	293	0	0	0	0	8	0	1	9	9	302
17:45 18:00	0	96	0	96	0	129	0	129	225	0	0	0	0	7	0	4	11	11	236
Total:	0	3642	0	3642	1	4484	0	4485	8127	0	0	0	0	177	0	123	300	8127	8,427

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

Survey Date: Tuesday, March 07, 2017 WO No: 36734 Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

			i an otaay	Oychist v	olullic		
		MAIN ST			EVELYN AVE		
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	2	0	2	0	0	0	2
07:15 07:30	1	0	1	0	0	0	1
07:30 07:45	2	0	2	0	0	0	2
07:45 08:00	0	1	1	0	1	1	2
08:00 08:15	1	0	1	0	0	0	1
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	2	0	2	0	1	1	3
08:45 09:00	2	0	2	0	0	0	2
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	1	0	1	0	0	0	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	1	0	1	0	0	0	1
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	1	0	1	0	0	0	1
12:45 13:00	1	0	1	0	1	1	2
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	1	0	1	0	0	0	1
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	1	0	1	0	0	0	1
16:30 16:45	0	2	2	0	0	0	2
16:45 17:00	0	3	3	0	0	0	3
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0
Total	16	6	22	0	3	3	25



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

Survey Date: Tuesday, March 07, 2017 WO No: 36734 Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

MAIN ST **EVELYN AVE**

	NB Approach or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	2	2	0	1	1	3
07:15 07:30	1	0	1	0	5	5	6
07:30 07:45	1	5	6	0	5	5	11
07:45 08:00	21	21	42	0	24	24	66
08:00 08:15	11	13	24	0	9	9	33
08:15 08:30	2	4	6	0	13	13	19
08:30 08:45	5	0	5	0	10	10	15
08:45 09:00	0	0	0	0	9	9	9
09:00 09:15	1	1	2	0	9	9	11
09:15 09:30	0	1	1	0	4	4	5
09:30 09:45	0	1	1	0	0	0	1
09:45 10:00	1	4	5	0	5	5	10
11:30 11:45	4	4	8	0	4	4	12
11:45 12:00	1	1	2	0	6	6	8
12:00 12:15	3	0	3	0	8	8	11
12:15 12:30	1	1	2	0	3	3	5
12:30 12:45	1	0	1	0	3	3	4
12:45 13:00	2	2	4	0	3	3	7
13:00 13:15	1	1	2	0	3	3	5
13:15 13:30	2	3	5	0	3	3	8
15:00 15:15	0	0	0	0	3	3	3
15:15 15:30	5	0	5	0	11	11	16
15:30 15:45	1	1	2	0	5	5	7
15:45 16:00	0	0	0	0	8	8	8
16:00 16:15	2	0	2	0	10	10	12
16:15 16:30	1	7	8	0	5	5	13
16:30 16:45	0	3	3	0	9	9	12
16:45 17:00	2	2	4	0	5	5	9
17:00 17:15	1	1	2	0	2	2	4
17:15 17:30	1	3	4	0	6	6	10
17:30 17:45	0	1	1	0	10	10	11
17:45 18:00	0	3	3	0	6	6	9
Total	71	85	156	0	207	207	363

May 10, 2021 Page 5 of 8 May 10, 2021 Page 6 of 8



Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision

Otal	t i iiiie	. 0	.00											Dev	ice:			IVII	OVISIO	11
							F	ull S	Stud	y He	avy	Vel	nicle	es						
				M	AIN S	ST.				-	-		EVE	ELYN	AVE					
		N	orthbo	und		Sc	outhbou	ınd			Е	astbou	nd		W	estbou	nd			
Time	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	7	0	7	0	3	0	3	10	0	0	0	0	0	0	0	0	0	10
07:15	07:30	0	5	0	5	0	3	0	3	8	0	0	0	0	0	0	0	0	0	8
07:30	07:45	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
07:45	08:00	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	0	0	0	8
08:00	08:15	0	6	0	6	0	6	0	6	12	0	0	0	0	0	0	0	0	0	12
08:15	08:30	0	2	0	2	0	6	0	6	8	0	0	0	0	0	0	0	0	0	8
08:30	08:45	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
08:45	09:00	0	2	0	2	0	5	0	5	7	0	0	0	0	0	0	0	0	0	7
09:00	09:15	0	7	0	7	0	5	0	5	12	0	0	0	0	0	0	0	0	0	12
09:15	09:30	0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
					1 .					1		1 -		1 -		1 -	1 -	1 -	$\overline{}$	

07:45	08:00	0	4	- 0	4	0	- 4	- 0	4	8	- 0	0	0	0	0	0	- 0	0	0	8
08:00	08:15	0	6	0	6	0	6	0	6	12	0	0	0	0	0	0	0	0	0	12
08:15	08:30	0	2	0	2	0	6	0	6	8	0	0	0	0	0	0	0	0	0	8
08:30	08:45	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
08:45	09:00	0	2	0	2	0	5	0	5	7	0	0	0	0	0	0	0	0	0	7
09:00	09:15	0	7	0	7	0	5	0	5	12	0	0	0	0	0	0	0	0	0	12
09:15	09:30	0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
09:30	09:45	0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
09:45	10:00	0	6	0	6	0	5	0	5	11	0	0	0	0	0	0	0	0	0	11
11:30	11:45	0	4	0	4	0	5	0	5	9	0	0	0	0	0	0	1	1	1	10
11:45	12:00	0	6	0	6	0	8	0	8	14	0	0	0	0	0	0	0	0	0	14
12:00	12:15	0	8	0	8	0	3	0	3	11	0	0	0	0	0	0	0	0	0	11
12:15	12:30	0	2	0	2	0	7	0	7	9	0	0	0	0	0	0	0	0	0	9
12:30	12:45	0	5	0	5	0	1	0	1	6	0	0	0	0	0	0	0	0	0	6
12:45	13:00	0	3	0	3	0	8	0	8	11	0	0	0	0	0	0	0	0	0	11
13:00	13:15	0	4	0	4	0	1	0	1	5	0	0	0	0	0	0	0	0	0	5
13:15	13:30	0	4	0	4	0	3	0	3	7	0	0	0	0	0	0	0	0	0	7
15:00	15:15	0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
15:15	15:30	0	2	0	2	0	4	0	4	6	0	0	0	0	1	0	1	2	2	8
15:30	15:45	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	0	0	0	8
15:45	16:00	0	6	0	6	0	3	0	3	9	0	0	0	0	0	0	0	0	0	9
16:00	16:15	0	6	0	6	0	6	0	6	12	0	0	0	0	0	0	0	0	0	12
16:15	16:30	0	3	0	3	0	6	0	6	9	0	0	0	0	1	0	0	1	1	10
16:30	16:45	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
16:45	17:00	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	0	0	0	8
17:00	17:15	0	2	0	2	0	5	0	5	7	0	0	0	0	0	0	0	0	0	7

0 **138** 0 **145** 0 **145 283** 0 0



Transportation Services - Traffic Services

Turning Movement Count - Study Results

EVELYN AVE @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36734

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total MAIN ST EVELYN AVE

Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	1	0	0	1
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	0	1	0	0	1

May 10, 2021 Page 7 of 8 May 10, 2021 Page 8 of 8

0 0



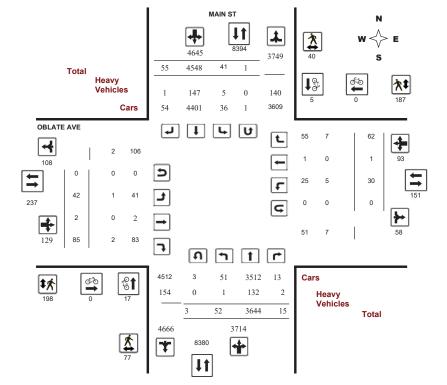
Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

Full Study Diagram





Transportation Services - Traffic Services

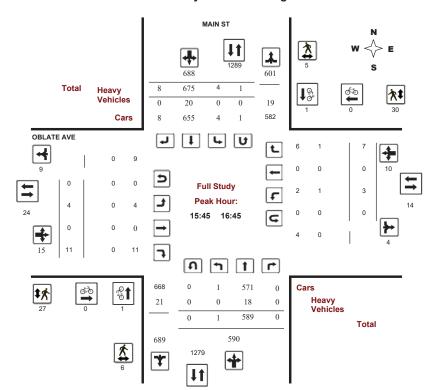
Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram



April 2, 2020 Page 1 of 8 April 2, 2020 Page 2 of 8



Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

Survey Date: Tuesday, March 07, 2017 WO No: 36739 Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, March 07, 2017 **Total Observed U-Turns AADT Factor** Southbound:

1.00

																	1.00		
								Eastbou	nd: 0		West	bound:	0						
			Λ	AAIN S	Т							OB	LATE	AVE					
	No	rthbou	nd		So	uthbou	ınd			E	astbou	ınd		W	estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	14	402	4	420	10	624	8	642	1062	5	1	10	16	5	0	5	10	26	1088
08:00 09:00	19	559	2	580	2	646	10	658	1238	8	0	14	22	1	0	4	5	27	1265
09:00 10:00	3	427	2	432	3	448	6	457	889	3	0	8	11	4	0	8	12	23	912
11:30 12:30	4	350	2	356	9	438	6	453	809	5	1	12	18	6	0	13	19	37	846
12:30 13:30	6	337	4	347	7	458	7	472	819	4	0	14	18	3	0	4	7	25	844
15:00 16:00	1	563	1	565	4	568	5	577	1142	10	0	7	17	7	1	18	26	43	1185
16:00 17:00	3	536	0	539	2	693	8	703	1242	4	0	13	17	3	0	3	6	23	1265
17:00 18:00	2	470	0	472	4	673	5	682	1154	3	0	7	10	1	0	7	8	18	1172
Sub Total	52	3644	15	3711	41	4548	55	4644	8355	42	2	85	129	30	1	62	93	222	8577
U Turns				3				1	4				0				0	0	4
Total	52	3644	15	3714	41	4548	55	4645	8359	42	2	85	129	30	1	62	93	222	8581
EQ 12Hr	. 72	5065	21	5162	57	6322	76	6457	11619	58	3	118	179	42	1	86	129	309	11928
Note: These v	alues a	are calcu	lated b	y multiply	ying the	e totals b	y the a	ppropria	te expans	ion fact	or.			1.39					
AVG 12Hr Note: These v	68	4774	20	4865	54	5958	72	6085	11619	55	3 factor	111	169	39 1	1	81	122	309	11928
Note. These v	olullies	are care	uiateu	by muni	piyirig t	ne Equiv	aleni i	2 111. 1018	als by trie	AADTI	actor.			1					
AVG 24Hr	89	6253	26	6374	70	7805	94	7971	14345	72	3	146	221	51	2	106	160	381	14726

April 2, 2020 Page 3 of 8

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

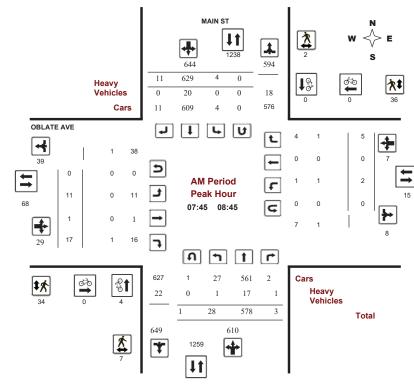


Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

MAIN ST @ OBLATE AVE

Survey Date: Tuesday, March 07, 2017 WO No: 36739 Start Time: 07:00 Device: Miovision



Comments

2020-Apr-02 Page 1 of 3

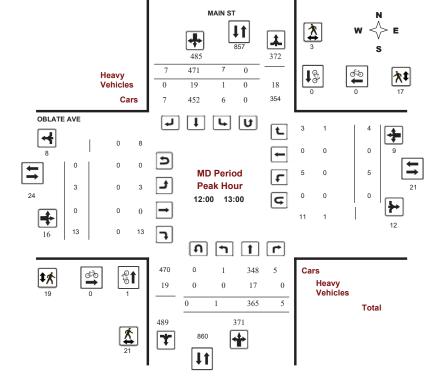


Turning Movement Count - Peak Hour Diagram

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision



Comments



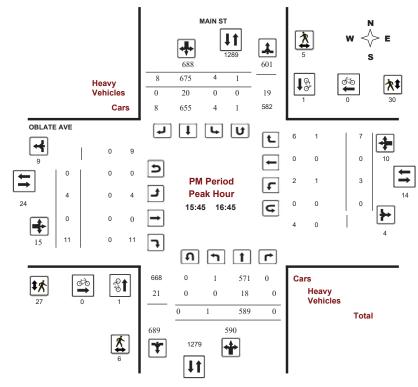
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision



Comments



Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

MAIN ST

Full Study 15 Minute Increments OBLATE AVE

Northbound LT Time Period LT ST LT ST RT LT ST 291 322 321 214 209 216 126 0 128 124 0 235 194 290 336 331 325 299 292

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

MAIN ST

Full Study Cyclist Volume

OBLATE AVE

		WAINSI			OBLATEAVI	=	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	2	0	2	0	0	0	2
07:15 07:30	1	0	1	0	0	0	1
07:30 07:45	2	0	2	0	0	0	2
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	2	0	2	0	0	0	2
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	2	0	2	0	0	0	2
08:45 09:00	2	0	2	0	0	0	2
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	1	0	1	0	0	0	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	1	0	1	0	0	0	1
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	1	0	1	0	0	0	1
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	1	0	1	0	0	0	1
15:15 15:30	1	0	1	0	0	0	1
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	1	0	1	0	0	0	1
16:30 16:45	0	1	1	0	0	0	1
16:45 17:00	0	2	2	0	0	0	2
17:00 17:15	0	1	1	0	0	0	1
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	1	1	0	0	0	1
17:45 18:00	0	0	0	0	0	0	0
Total	17	5	22	0	0	0	22

April 2, 2020 Page 4 of 8 April 2, 2020 Page 5 of 8



Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

MAIN ST OBLATE AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	2	2	4	4
07:15 07:30	0	0	0	2	5	7	7
07:30 07:45	6	0	6	1	5	6	12
07:45 08:00	0	2	2	13	5	18	20
08:00 08:15	4	0	4	7	6	13	17
08:15 08:30	1	0	1	8	17	25	26
08:30 08:45	2	0	2	6	8	14	16
08:45 09:00	4	1	5	5	8	13	18
09:00 09:15	1	0	1	6	14	20	21
09:15 09:30	3	1	4	2	4	6	10
09:30 09:45	7	4	11	4	2	6	17
09:45 10:00	4	3	7	3	2	5	12
11:30 11:45	0	1	1	6	4	10	11
11:45 12:00	2	0	2	6	4	10	12
12:00 12:15	10	0	10	7	4	11	21
12:15 12:30	8	1	9	5	2	7	16
12:30 12:45	1	0	1	2	4	6	7
12:45 13:00	2	2	4	5	7	12	16
13:00 13:15	3	0	3	7	5	12	15
13:15 13:30	0	0	0	5	3	8	8
15:00 15:15	2	2	4	6	4	10	14
15:15 15:30	3	12	15	8	6	14	29
15:30 15:45	1	1	2	3	6	9	11
15:45 16:00	2	0	2	5	6	11	13
16:00 16:15	2	0	2	5	10	15	17
16:15 16:30	0	0	0	6	4	10	10
16:30 16:45	2	5	7	11	10	21	28
16:45 17:00	3	0	3	7	6	13	16
17:00 17:15	1	0	1	14	3	17	18
17:15 17:30	3	1	4	12	9	21	25
17:30 17:45	0	0	0	5	7	12	12
17:45 18:00	0	4	4	14	5	19	23
Total	77	40	117	198	187	385	502



Transportation Services - Traffic Services

Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

MAIN ST OBLATE AVE

		No	orthbo	und		So	uthbou	nd			E	astbour	nd		We	estbour	nd			
Time Perio	od	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:	:15	0	5	0	5	1	1	0	2	7	0	0	1	1	1	0	1	2	3	10
07:15 07:	:30	0	5	1	6	0	5	0	5	11	0	0	0	0	0	0	0	0	0	11
07:30 07:	:45	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
07:45 08:	:00	0	4	1	5	0	3	0	3	8	0	0	0	0	1	0	0	1	1	9
08:00 08:	:15	0	6	0	6	0	7	0	7	13	0	0	0	0	0	0	0	0	0	13
08:15 08:	:30	1	2	0	3	0	4	0	4	7	0	0	1	1	0	0	0	0	1	8
08:30 08:	:45	0	5	0	5	0	6	0	6	11	0	0	0	0	0	0	1	1	1	12
08:45 09:	:00	0	2	0	2	1	4	0	5	7	0	0	0	0	0	0	0	0	0	7
09:00 09:	:15	0	5	0	5	0	5	0	5	10	0	0	0	0	1	0	0	1	1	11
09:15 09:	:30	0	4	0	4	0	6	0	6	10	0	0	0	0	0	0	0	0	0	10
09:30 09:	:45	0	4	0	4	0	6	1	7	11	0	0	0	0	1	0	0	1	1	12
09:45 10:	:00	0	6	0	6	0	5	0	5	11	1	0	0	1	0	0	1	1	2	13
11:30 11:	:45	0	4	0	4	0	6	0	6	10	0	0	0	0	0	0	0	0	0	10
11:45 12:	:00	0	5	0	5	0	8	0	8	13	0	0	0	0	0	0	1	1	1	14
12:00 12:	:15	0	8	0	8	0	3	0	3	11	0	0	0	0	0	0	0	0	0	11
12:15 12:	:30	0	2	0	2	1	5	0	6	8	0	0	0	0	0	0	0	0	0	8
12:30 12:	:45	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	1	1	1	9
12:45 13:	:00	0	3	0	3	0	7	0	7	10	0	0	0	0	0	0	0	0	0	10
13:00 13:	:15	0	4	0	4	1	0	0	1	5	0	0	0	0	0	0	0	0	0	5
13:15 13:	:30	0	5	0	5	1	3	0	4	9	0	0	0	0	0	0	0	0	0	9
15:00 15:	:15	0	5	0	5	0	9	0	9	14	0	0	0	0	0	0	0	0	0	14
15:15 15:	:30	0	3	0	3	0	4	0	4	7	0	0	0	0	0	0	0	0	0	7
15:30 15:	:45	0	4	0	4	0	5	0	5	9	0	0	0	0	0	0	0	0	0	9
15:45 16:	:00	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	1	1	1	10
16:00 16:	:15	0	6	0	6	0	6	0	6	12	0	0	0	0	0	0	0	0	0	12
16:15 16:	:30	0	3	0	3	0	3	0	3	6	0	0	0	0	1	0	0	1	1	7
16:30 16:	:45	0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
16:45 17:	:00	0	3	0	3	0	5	0	5	8	0	0	0	0	0	0	0	0	0	8
17:00 17:	:15	0	3	0	3	0	5	0	5	8	0	0	0	0	0	0	0	0	0	8
17:15 17:	:30	0	2	0	2	0	2	0	2	4	0	0	0	0	0	0	0	0	0	4
17:30 17:	:45	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	1	1	1	7
17:45 18:	:00	0	3	0	3	0	2	0	2	5	0	0	0	0	0	0	0	0	0	5
Total: No	ne	1	132	2	135	5	147	1	153	288	1	0	2	3	5	0	7	12	15	303

April 2, 2020 Page 6 of 8 April 2, 2020 Page 7 of 8



Turning Movement Count - Study Results

MAIN ST @ OBLATE AVE

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36739

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total MAIN ST OBLATE AVE

ime Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
00 07:	0	0	0	0	0
5 07:	0	0	0	0	0
30 07:	0	0	0	0	0
15 08:	0	0	0	0	0
00 08:	0	0	0	0	0
5 08:	0	0	0	0	0
30 08:	i 1	0	0	0	1
15 09:	0	0	0	0	0
00 09:	i 1	0	0	0	1
5 09:	0	0	0	0	0
09:	0	0	0	0	0
5 10:	0	0	0	0	0
30 11:	0	0	0	0	0
15 12:	0	0	0	0	0
00 12:	0	0	0	0	0
5 12:	0	0	0	0	0
30 12:	0	0	0	0	0
15 13:	0	0	0	0	0
00 13:	0	0	0	0	0
5 13:	0	0	0	0	0
00 15:	0	0	0	0	0
5 15:	0	0	0	0	0
30 15:	0	0	0	0	0
15 16:	0	0	0	0	0
00 16:	0	1	0	0	1
5 16:	0	0	0	0	0
30 16:	0	0	0	0	0
l5 17:	0	0	0	0	0
00 17:	0	0	0	0	0
5 17:	0	0	0	0	0
30 17:	i 1	0	0	0	1
15 18:	0	0	0	0	0
Total	3	1	0	0	



Transportation Services - Traffic Services

Turning Movement Count - Study Results

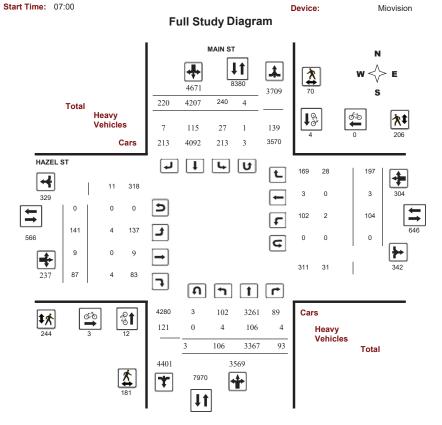
HAZEL ST @ MAIN ST

WO No:

Survey Date: Tuesday, March 07, 2017

: Tuesday, March 07, 2017

36733



April 2, 2020 Page 8 of 8 April 2, 2020 Page 1 of 8



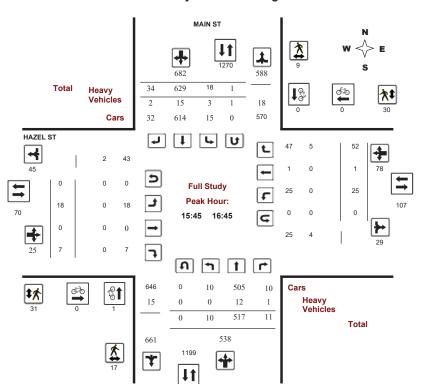
Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram





Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study Summary (8 HR Standard)

 Survey Date:
 Tuesday, March 07, 2017
 Total Observed U-Turns
 AADT Factor

 Northbound:
 3
 Southbound:
 4
 1.00

							- 1	Eastbour	nd: 0		West	bound:	0						
			N	IAIN S	Т							Н	AZEL	ST					
	No	rthbou	nd		So	uthbou	ınd			E	astbou	ınd		W	estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Gran Tot
07:00 08:00	3	400	10	413	32	579	13	624	1037	16	1	3	20	2	0	9	11	31	10
08:00 09:00	12	552	20	584	69	567	27	663	1247	23	1	9	33	10	0	17	27	60	13
09:00 10:00	12	390	12	414	39	400	26	465	879	20	3	8	31	8	0	20	28	59	9:
11:30 12:30	31	302	6	339	22	405	27	454	793	15	0	18	33	16	0	27	43	76	8
12:30 13:30	13	312	12	337	23	427	26	476	813	21	2	13	36	6	1	14	21	57	8
15:00 16:00	11	502	8	521	15	535	34	584	1105	22	0	13	35	19	1	29	49	84	11
16:00 17:00	12	473	10	495	20	659	31	710	1205	11	1	8	20	24	1	52	77	97	13
17:00 18:00	12	436	15	463	20	635	36	691	1154	13	1	15	29	19	0	29	48	77	12
Sub Total	106	3367	93	3566	240	4207	220	4667	8233	141	9	87	237	104	3	197	304	541	87
U Turns				3				4	7				0				0	0	
Total	106	3367	93	3569	240	4207	220	4671	8240	141	9	87	237	104	3	197	304	541	87
EQ 12Hr	147	4680	129	4961	334	5848	306	6493	11454	196	13	121	329	145	4	274	423	752	122
lote: These v	alues a	re calcu	lated by	y multiply	ying the	totals b	y the ap	ppropriat	e expans	ion facto	or.			1.39					
AVG 12Hr	139	4411	122	4675	314	5511	288	6119	11454	185	12	114	310	136	4	258	398	752	122
lote: These v	olumes	are calc	culated	by multi	plying t	he Equiv	alent 1	2 hr. tota	Is by the	AADT f	actor.			1					
AVG 24Hr	182	5778	160	6125	412	7220	378	8016	14141	242	15	149	407	178	5	338	522	929	150

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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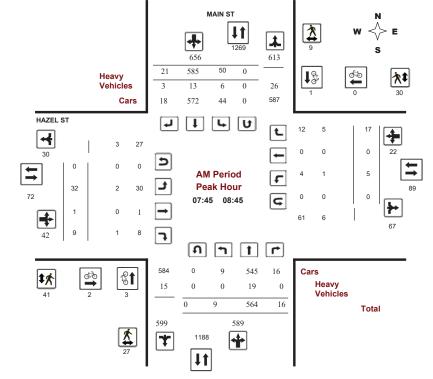


Turning Movement Count - Peak Hour Diagram

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision



Comments



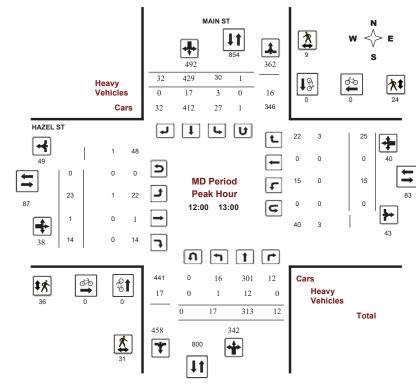
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision



Comments

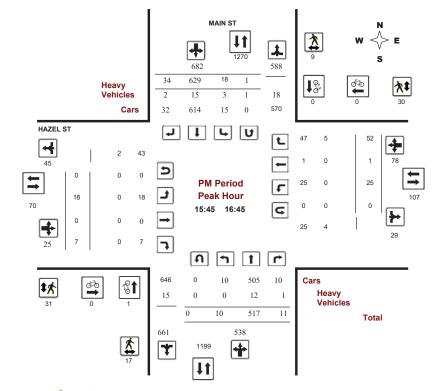


Turning Movement Count - Peak Hour Diagram

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute Increments

MAIN ST HAZEL ST

Northbound Southbound Eastbound Westbound

Prime Period Prime Period Prime Period Prime P			No	orthboi	und		So	outhbou	nd			E	astbour	nd		VVe	estbour	nd			
07:15 07:30 1 89 4 94 5 134 2 141 10 3 0 0 3 2 0 2 4 10 242 07:30 07:45 1 108 2 111 12 160 6 178 10 3 1 2 6 0 0 0 0 10 295 07:45 1 109 3 143 5 145 3 153 11 10 0 0 3 3 11 310 08:00 08:15 5 138 4 147 9 142 6 157 13 7 0 4 117 0 1 11 0 0 1 127 133 154 5 172 8 7 1 1 9 1 0 0 0 1 14 9 3 1	Time Peri	iod	LT	ST	RT		LT	ST	RT			LT	ST	RT		LT	ST	RT			
07:30 07:45 1 108 2 111 12 160 6 178 10 3 1 2 6 0 0 0 0 10 295 07:45 08:00 1 139 3 143 5 145 3 153 11 10 0 1 11 0 0 3 3 31 11 310 08:05 08:30 0 144 3 147 13 154 5 172 8 7 1 1 9 1 0 3 4 8 332 08:30 08:45 3 143 6 152 23 144 7 174 9 8 0 3 11 4 0 10 14 9 351 08:45 09:00 4 113 2 119 13 117 10 140 8 4 1	07:00 07	:15	0	64	1	65	10	140	2	152	5	0	0	0	0	0	0	4	4	5	221
07:45	07:15 07	:30	1	89	4	94	5	134	2	141	10	3	0	0	3	2	0	2	4	10	242
08:00 08:15 5 138	07:30 07	:45	1	108	2	111	12	160	6	178	10	3	1	2	6	0	0	0	0	10	295
08:15	07:45 08	:00	1	139	3	143	5	145	3	153	11	10	0	1	11	0	0	3	3	11	310
08:30	08:00 08	:15	5	138	4	147	9	142	6	157	13	7	0	4	11	0	0	1	1	13	316
08:45 09:00	08:15 08	:30	0	144	3	147	13	154	5	172	8	7	1	1	9	1	0	3	4	8	332
09:00 09:15	08:30 08	:45	3	143	6	152	23	144	7	174	9	8	0	3	11	4	0	10	14	9	351
09:15	08:45 09	:00	4	127	7	138	24	127	9	160	4	1	0	1	2	5	0	3	8	4	308
09:30 09:45 4 89 2 95 6 94 5 105 13 5 1 3 9 1 0 2 3 13 212	09:00 09	:15	4	113	2	119	14	89	7	112	9	6	1	1	8	1	0	6	7	9	246
09:45 10:00 2 86 3 91 6 100 4 110 10 5 1 2 8 2 0 7 9 10 218	09:15 09	:30	2	102	5	109	13	117	10	140	8	4	0	2	6	4	0	5	9	8	264
11:30 11:45 7 78 0 85 0 99 6 105 6 1 0 6 7 1 0 8 9 6 206 11:45 12:00 13 82 1 96 4 88 6 98 12 4 0 4 8 2 0 5 7 12 209 12:00 12:15 10 72 3 85 5 113 7 125 12 6 0 6 12 4 0 5 9 12 231 12:30 12:45 3 72 3 78 5 113 11 129 6 9 1 3 13 1 0 5 6 6 226 12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7	09:30 09	:45	4	89	2	95	6	94	5	105	13	5	1	3	9	1	0	2	3	13	212
11:45 12:00 13 82 1 96 4 88 6 98 12 4 0 4 8 2 0 5 7 12 209 12:00 12:15 10 72 3 85 5 113 7 125 12 6 0 6 12 4 0 5 9 12 231 12:30 12:45 3 72 3 78 5 113 11 129 6 9 1 3 1 0 5 6 6 226 12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7 1 0 6 7 8 231 13:00 13:15 4 61 4 72 5 109 4 118 2 3 0 3 6 0	09:45 10	:00	2	86	3	91	6	100	4	110	10	5	1	2	8	2	0	7	9	10	218
12:00 12:15 10 72 3 85 5 113 7 125 12 6 0 6 12 4 0 5 9 12 231 12:15 12:30 1 70 2 73 13 105 8 127 7 4 0 2 6 9 0 9 18 7 224 12:30 12:45 3 72 3 78 5 113 11 129 6 9 1 3 13 1 0 5 6 6 226 12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7 1 0 6 6 226 13:00 13:15 4 6i 14 72 5 109 4 118 2 3 0 3 6 <t< td=""><td>11:30 11</td><td>:45</td><td>7</td><td>78</td><td>0</td><td>85</td><td>0</td><td>99</td><td>6</td><td>105</td><td>6</td><td>1</td><td>0</td><td>6</td><td>7</td><td>1</td><td>0</td><td>8</td><td>9</td><td>6</td><td>206</td></t<>	11:30 11	:45	7	78	0	85	0	99	6	105	6	1	0	6	7	1	0	8	9	6	206
12:15 12:30 1 70 2 73 13 105 8 127 7 4 0 2 6 9 0 9 18 7 224 12:30 12:45 3 72 3 78 5 113 11 129 6 9 1 3 13 1 0 5 6 6 226 12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7 1 0 6 7 8 231 13:00 13:15 4 61 4 72 5 109 4 118 2 3 0 3 6 0 0 2 2 2 198 15:15 13:30 3 80 1 8 6 107 5 118 9 5 1 4 10 </td <td>11:45 12</td> <td>:00</td> <td>13</td> <td>82</td> <td>1</td> <td>96</td> <td>4</td> <td>88</td> <td>6</td> <td>98</td> <td>12</td> <td>4</td> <td>0</td> <td>4</td> <td>8</td> <td>2</td> <td>0</td> <td>5</td> <td>7</td> <td>12</td> <td>209</td>	11:45 12	:00	13	82	1	96	4	88	6	98	12	4	0	4	8	2	0	5	7	12	209
12:30 12:45 3 72 3 78 5 113 11 129 6 9 1 3 13 1 0 5 6 6 226 12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7 1 0 6 7 8 231 13:00 13:15 4 61 4 72 5 109 4 118 2 3 0 3 6 0 0 2 2 2 198 13:15 13:30 3 80 1 84 6 107 5 118 9 5 1 4 10 4 1 1 6 9 218 15:00 15:15 4 120 0 124 2 130 7 139 11 2 0 2 4	12:00 12	::15	10	72	3	85	5	113	7	125	12	6	0	6	12	4	0	5	9	12	231
12:45 13:00 3 99 4 106 7 98 6 111 8 4 0 3 7 1 0 6 7 8 231 13:00 13:15 4 61 4 72 5 109 4 118 2 3 0 3 6 0 0 2 2 2 188 13:15 13:30 3 80 1 84 6 107 5 118 9 5 1 4 10 4 1 1 6 9 218 15:00 15:15 4 120 0 124 2 130 7 139 11 2 0 2 4 4 1 1 8 13 11 120 3 124 3 133 5 141 7 4 0 4 8 5 0 7 12	12:15 12	:30	1	70	2	73	13	105	8	127	7	4	0	2	6	9	0	9	18	7	224
13:00 13:15 4 61 4 72 5 109 4 118 2 3 0 3 6 0 0 2 2 2 198 13:15 13:30 3 80 1 84 6 107 5 118 9 5 1 4 10 4 1 1 6 9 218 15:00 15:15 4 120 0 124 2 130 7 139 11 2 0 2 4 4 1 8 13 11 280 15:15 15:30 1 120 3 124 3 133 5 141 7 4 0 4 8 5 0 7 12 7 285 15:30 15:45 4 112 4 120 6 138 11 155 10 6 0 5	12:30 12	:45	3	72	3	78	5	113	11	129	6	9	1	3	13	1	0	5	6	6	226
13:15 13:30 3 80 1 84 6 107 5 118 9 5 1 4 10 4 1 1 6 9 218 15:00 15:15 4 120 0 124 2 130 7 139 11 2 0 2 4 4 1 8 13 11 280 15:15 15:30 1 120 3 124 3 133 5 141 7 4 0 4 8 5 0 7 12 7 285 15:30 15:45 4 112 4 120 6 138 11 155 10 6 0 5 11 4 0 8 12 10 298 15:45 16:00 2 150 1 153 4 134 11 150 8 10 0 2	12:45 13	:00	3	99	4	106	7	98	6	111	8	4	0	3	7	1	0	6	7	8	231
15:00 15:15 4 120 0 124 2 130 7 139 11 2 0 2 4 4 1 8 13 11 280 15:15 15:30 1 120 3 124 3 133 5 141 7 4 0 4 8 5 0 7 12 7 285 15:30 15:45 4 112 4 120 6 183 11 155 10 6 0 5 11 4 0 8 12 10 288 15:45 16:00 2 150 1 153 4 134 11 150 8 10 0 2 12 6 0 6 12 8 327 16:00 16:15 2 119 1 122 6 164 9 179 12 3 0 2<	13:00 13	:15	4	61	4	72	5	109	4	118	2	3	0	3	6	0	0	2	2	2	198
15:15 15:30 1 120 3 124 3 133 5 141 7 4 0 4 8 5 0 7 12 7 285 15:30 15:45 4 112 4 120 6 138 11 155 10 6 0 5 11 4 0 8 12 10 288 15:45 16:00 2 150 1 153 4 134 11 150 8 10 0 2 12 6 0 6 12 8 327 16:00 16:15 2 119 1 122 6 164 9 179 12 3 0 2 5 8 1 19 28 12 334 16:15 16:30 2 138 2 142 3 164 7 174 6 1 0 1<	13:15 13	:30	3	80	1	84	6	107	5	118	9	5	1	4	10	4	1	1	6	9	218
15:30 15:45 4 112 4 120 6 138 11 155 10 6 0 5 11 4 0 8 12 10 298 15:45 16:00 2 150 1 153 4 134 11 150 8 10 0 2 12 6 0 6 12 8 327 16:00 16:15 2 119 1 122 6 164 9 179 12 3 0 2 5 8 1 19 28 12 334 16:15 16:30 2 138 2 142 3 164 7 174 6 1 0 1 2 5 0 14 19 6 337 16:30 17:00 7 121 5 167 7 179 8 4 0 2 6 6 </td <td>15:00 15</td> <td>:15</td> <td>4</td> <td>120</td> <td>0</td> <td>124</td> <td>2</td> <td>130</td> <td>7</td> <td>139</td> <td>11</td> <td>2</td> <td>0</td> <td>2</td> <td>4</td> <td>4</td> <td>1</td> <td>8</td> <td>13</td> <td>11</td> <td>280</td>	15:00 15	:15	4	120	0	124	2	130	7	139	11	2	0	2	4	4	1	8	13	11	280
15:45 16:00 2 150 1 153 4 134 11 150 8 10 0 2 12 6 0 6 12 8 327 16:00 16:15 2 119 1 122 6 164 9 179 12 3 0 2 5 8 1 19 28 12 334 16:15 16:30 2 138 2 142 3 164 7 174 6 1 0 1 2 5 0 14 19 6 337 16:30 16:45 4 110 7 121 5 167 7 179 8 4 0 2 6 6 0 13 19 8 325 16:45 17:00 4 106 0 110 6 164 8 178 4 3 1 3 <td>15:15 15</td> <td>:30</td> <td>1</td> <td>120</td> <td>3</td> <td>124</td> <td>3</td> <td>133</td> <td>5</td> <td>141</td> <td>7</td> <td>4</td> <td>0</td> <td>4</td> <td>8</td> <td>5</td> <td>0</td> <td>7</td> <td>12</td> <td>7</td> <td>285</td>	15:15 15	:30	1	120	3	124	3	133	5	141	7	4	0	4	8	5	0	7	12	7	285
16:00 16:15 2 119 1 122 6 164 9 179 12 3 0 2 5 8 1 19 28 12 334 16:30 16:30 2 138 2 142 3 164 7 174 6 1 0 1 2 5 0 14 19 6 337 16:30 16:45 4 110 7 121 5 167 7 179 8 4 0 2 6 6 0 13 19 8 325 16:45 17:00 4 106 0 110 6 164 8 178 4 3 1 3 7 5 0 6 11 4 306 17:00 17:15 2 118 3 123 3 161 11 175 11 4 0 13 <td>15:30 15</td> <td>:45</td> <td>4</td> <td>112</td> <td>4</td> <td>120</td> <td>6</td> <td>138</td> <td>11</td> <td>155</td> <td>10</td> <td>6</td> <td>0</td> <td>5</td> <td>11</td> <td>4</td> <td>0</td> <td>8</td> <td>12</td> <td>10</td> <td>298</td>	15:30 15	:45	4	112	4	120	6	138	11	155	10	6	0	5	11	4	0	8	12	10	298
16:15 16:30 2 138 2 142 3 164 7 174 6 1 0 1 2 5 0 14 19 6 337 16:30 16:45 4 110 7 121 5 167 7 179 8 4 0 2 6 6 0 13 19 8 325 16:45 17:00 4 106 0 110 6 164 8 178 4 3 7 5 0 6 11 4 306 17:00 17:15 2 118 3 123 3 161 11 175 11 6 0 5 11 4 306 17:15 17:30 4 130 5 139 3 154 9 166 3 3 0 1 4 5 0 5 10 3	15:45 16	:00	2	150	1	153	4	134	11	150	8	10	0	2	12	6	0	6	12	8	327
16:30 16:45 4 110 7 121 5 167 7 179 8 4 0 2 6 6 0 13 19 8 325 16:45 17:00 4 106 0 110 6 164 8 178 4 3 1 3 7 5 0 6 11 4 306 17:00 17:15 2 118 3 123 3 161 11 175 11 6 0 5 11 4 0 13 17 11 326 17:15 17:30 4 130 5 139 3 154 9 166 3 3 0 1 4 5 0 5 10 3 319 17:30 17:45 3 91 3 97 7 191 6 204 6 0 1 6 7 3 0 7 10 6 318	16:00 16	:15	2	119	1	122	6	164	9	179	12	3	0	2	5	8	1	19	28	12	334
16:45 17:00 4 106 0 110 6 164 8 178 4 3 1 3 7 5 0 6 11 4 306 17:00 17:15 2 118 3 123 3 161 11 175 11 6 0 5 11 4 0 13 17 11 326 17:15 17:30 4 130 5 139 3 154 9 166 3 3 0 1 4 5 0 5 10 3 319 17:30 17:45 3 91 3 97 7 191 6 204 6 0 1 6 7 3 0 7 10 6 318	16:15 16	:30	2	138	2	142	3	164	7	174	6	1	0	1	2	5	0	14	19	6	337
17:00 17:15 2 118 3 123 3 161 11 175 11 6 0 5 11 4 0 13 17 11 326 17:15 17:30 4 130 5 139 3 154 9 166 3 3 0 1 4 5 0 5 10 3 319 17:30 17:45 3 91 3 97 7 191 6 204 6 0 1 6 7 3 0 7 10 6 318	16:30 16	:45	4	110	7	121	5	167	7	179	8	4	0	2	6	6	0	13	19	8	325
17:15 17:30 4 130 5 139 3 154 9 166 3 3 0 1 4 5 0 5 10 3 319 17:30 17:45 3 91 3 97 7 191 6 204 6 0 1 6 7 3 0 7 10 6 318	16:45 17	:00	4	106	0	110	6	164	8	178	4	3	1	3	7	5	0	6	11	4	306
17:30 17:45 3 91 3 97 7 191 6 204 6 0 1 6 7 3 0 7 10 6 318	17:00 17	:15	2	118	3	123	3	161	11	175	11	6	0	5	11	4	0	13	17	11	326
	17:15 17	:30	4	130	5	139	3	154	9	166	3	3	0	1	4	5	0	5	10	3	319
17:45 18:00 3 97 4 104 7 129 10 146 6 4 0 3 7 7 0 4 11 6 268	17:30 17	:45	3	91	3	97	7	191	6	204	6	0	1	6	7	3	0	7	10	6	318
	17:45 18	:00	3	97	4	104	7	129	10	146	6	4	0	3	7	7	0	4	11	6	268
Total: 106 3367 93 3569 240 4207 220 4671 264 141 9 87 237 104 3 197 304 264 8,781	Total:		106	3367	93	3569	240	4207	220	4671	264	141	9	87	237	104	3	197	304	264	8,781

Note: U-Turns are included in Totals.



Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study Cyclist Volume

			. un otaay	0,0	0.40		
		MAIN ST			HAZEL ST		
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	2	0	2	0	0	0	2
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	1	0	1	0	0	0	1
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	2	0	2	0	0	0	2
08:15 08:30	0	1	1	0	0	0	1
08:30 08:45	1	0	1	2	0	2	3
08:45 09:00	1	0	1	1	0	1	2
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	1	0	1	0	0	0	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	1	0	1	0	0	0	1
1:30 11:45	0	0	0	0	0	0	0
1:45 12:00	0	0	0	0	0	0	0
2:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
2:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
3:15 13:30	0	0	0	0	0	0	0
15:00 15:15	1	0	1	0	0	0	1
15:15 15:30	1	0	1	0	0	0	1
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	1	0	1	0	0	0	1
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	2	2	0	0	0	2
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	1	1	0	0	0	1

16



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

MAIN ST HAZEL ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	1	1	0	1	1	2
07:15 07:30	2	3	5	5	6	11	16
07:30 07:45	4	3	7	2	2	4	11
07:45 08:00	6	1	7	15	3	18	25
08:00 08:15	4	2	6	12	8	20	26
08:15 08:30	4	5	9	6	13	19	28
08:30 08:45	13	1	14	8	6	14	28
08:45 09:00	11	6	17	4	16	20	37
09:00 09:15	7	1	8	9	13	22	30
09:15 09:30	1	3	4	2	4	6	10
09:30 09:45	6	1	7	3	1	4	11
09:45 10:00	3	1	4	2	5	7	11
11:30 11:45	4	3	7	8	3	11	18
11:45 12:00	8	0	8	12	7	19	27
12:00 12:15	8	2	10	13	4	17	27
12:15 12:30	6	1	7	8	5	13	20
12:30 12:45	8	2	10	5	5	10	20
12:45 13:00	9	4	13	10	10	20	33
13:00 13:15	2	1	3	11	3	14	17
13:15 13:30	6	5	11	9	10	19	30
15:00 15:15	9	2	11	10	1	11	22
15:15 15:30	4	2	6	8	2	10	16
15:30 15:45	4	3	7	10	8	18	25
15:45 16:00	1	2	3	10	8	18	21
16:00 16:15	5	2	7	11	3	14	21
16:15 16:30	1	4	5	6	8	14	19
16:30 16:45	10	1	11	4	11	15	26
16:45 17:00	5	2	7	10	11	21	28
17:00 17:15	6	2	8	4	7	11	19
17:15 17:30	8	1	9	7	6	13	22
17:30 17:45	8	1	9	9	6	15	24
17:45 18:00	8	2	10	11	10	21	31
Total	181	70	251	244	206	450	701

April 2, 2020 Page 5 of 8 April 2, 2020 Page 6 of 8



Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

MAIN ST HAZEL ST

	Northbound				Southbound					Eastbound				Westbound					
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR	Grand Total
07:00 07:15	0	3	0	3	0	2	0	2	5	0	0	0	0	0	0	1	1	1	6
07:15 07:30	0	6	0	6	1	3	0	4	10	0	0	0	0	0	0	1	1	1	11
07:30 07:45	0	6	0	6	1	3	0	4	10	0	0	0	0	0	0	0	0	0	10
07:45 08:00	0	7	0	7	0	3	1	4	11	0	0	0	0	0	0	1	1	1	12
08:00 08:15	0	6	0	6	2	5	0	7	13	1	0	0	1	0	0	1	1	2	15
08:15 08:30	0	3	0	3	2	1	2	5	8	1	0	1	2	0	0	1	1	3	11
08:30 08:45	0	3	0	3	2	4	0	6	9	0	0	0	0	1	0	2	3	3	12
08:45 09:00	0	1	0	1	1	2	0	3	4	0	0	0	0	0	0	1	1	1	5
09:00 09:15	1	4	0	5	1	3	0	4	9	0	0	1	1	0	0	1	1	2	11
09:15 09:30	0	3	0	3	2	3	0	5	8	1	0	0	1	1	0	1	2	3	11
09:30 09:45	0	3	0	3	2	8	0	10	13	0	0	0	0	0	0	1	1	1	14
09:45 10:00	0	5	0	5	0	5	0	5	10	0	0	0	0	0	0	1	1	1	11
11:30 11:45	0	1	0	1	0	5	0	5	6	0	0	0	0	0	0	1	1	1	7
11:45 12:00	1	4	0	5	1	5	1	7	12	0	0	1	1	0	0	0	0	1	13
12:00 12:15	1	5	0	6	0	6	0	6	12	1	0	0	1	0	0	1	1	2	14
12:15 12:30	0	2	0	2	2	3	0	5	7	0	0	0	0	0	0	0	0	0	7
12:30 12:45	0	2	0	2	0	4	0	4	6	0	0	0	0	0	0	2	2	2	8
12:45 13:00	0	3	0	3	1	4	0	5	8	0	0	0	0	0	0	0	0	0	8
13:00 13:15	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	1	1	1	3
13:15 13:30	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
15:00 15:15	0	4	0	4	1	6	0	7	11	0	0	0	0	0	0	1	1	1	12
15:15 15:30	0	2	1	3	1	3	0	4	7	0	0	0	0	0	0	1	1	1	8
15:30 15:45	0	3	1	4	0	5	1	6	10	0	0	1	1	0	0	1	1	2	12
15:45 16:00	0	3	1	4	0	3	0	4	8	0	0	0	0	0	0	1	1	1	9
16:00 16:15	0	5	0	5	1	6	0	7	12	0	0	0	0	0	0	1	1	1	13
16:15 16:30	0	2	0	2	0	2	2	4	6	0	0	0	0	0	0	1	1	1	7
16:30 16:45	0	2	0	2	2	4	0	6	8	0	0	0	0	0	0	2	2	2	10
16:45 17:00	0	4	0	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	4
17:00 17:15	1	2	1	4	1	6	0	7	11	0	0	0	0	0	0	1	1	1	12
17:15 17:30	0	1	0	1	0	2	0	2	3	0	0	0	0	0	0	1	1	1	4
17:30 17:45	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	0	0	6
17:45 18:00	0	2	0	2	3	1	0	4	6	0	0	0	0	0	0	1	1	1	7
Total: None	4	106	4	114	27	115	7	150	264	4	0	4	8	2	0	28	30	38	302



Transportation Services - Traffic Services

Turning Movement Count - Study Results

HAZEL ST @ MAIN ST

 Survey Date:
 Tuesday, March 07, 2017
 WO No:
 36733

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total

MAIN ST HAZEL ST

		MAIN 0	•			
Time	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	2	0	0	2
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	1	0	0	1
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	3	0	0	0	3
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	1	0	0	1
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Total		3	4	0	0	7

April 2, 2020 Page 7 of 8 April 2, 2020 Page 8 of 8

Appendix C

Synchro Intersection Worksheets – Existing Conditions



Existing AM Peak Hour

15 Oblates

Lane Group

Lane Configurations

	-	7	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	4	7		414		414						
Traffic Volume (vph)	12	220	278	504	5	393						
Future Volume (vph)	12	220	278	504	5	393						
Lane Group Flow (vph)	334	244	0	876	0	570						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										-
Total Lost Time (s)	6.3	6.3										
Lead/Lag	0.0	0.0			Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0	max	max		2.0		3.0	2.0	max	3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	28				20		53	53		20	53	53
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.83	0.33		0.68		0.54						
Control Delay	55.2	3.9		9.9		25.1						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	55.2	3.9		10.0		25.1						
LOS	E	A		В		C						
Approach Delay	33.5	,,		10.0		25.1						
Approach LOS	C			В		C						
Queue Length 50th (m)	59.1	0.0		26.4		41.7						
Queue Length 95th (m)	#136.6	14.6		31.0		58.1						
Internal Link Dist (m)	198.7	11.0		59.0		262.1						
Turn Bay Length (m)	100.7			00.0		202.1						
Base Capacity (vph)	403	733		1295		1057						
Starvation Cap Reductn	0	0		30		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.83	0.33		0.69		0.54						
Intersection Summary												
Cycle Length: 100												

Offiset: 60 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 85

	\rightarrow	*	1	Ť	-	¥						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ની	7		413-		479						
Traffic Volume (vph)	12	220	278	504	5	393						
Future Volume (vph)	12	220	278	504	5	393						
Lane Group Flow (vph)	334	244	0	876	0	570						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		5 6	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	28				20		53	53		20	53	53
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.83	0.33		0.68		0.54						
Control Delay	55.2	3.9		9.9		25.1						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	55.2	3.9		10.0		25.1						
LOS	Е	Α		В		С						
Approach Delay	33.5			10.0		25.1						
Approach LOS	С			В		С						
Queue Length 50th (m)	59.1	0.0		26.4		41.7						
Queue Length 95th (m)	#136.6	14.6		31.0		58.1						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	403	733		1295		1057						
Starvation Cap Reductn	0	0		30		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.83	0.33		0.69		0.54						
Intersection Summary												
Cycle Length: 100												

Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	12%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	3
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Internaction Cummens	
Intersection Summary	

Lanes, Volumes, Timings 1: Main & Hawthorne

Existing AM Peak Hour

15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.83 Intersection Signal Delay: 21.0
Intersection Capacity Utilization 75.5%
Analysis Period (min) 15 Intersection LOS: C
ICU Level of Service D # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne



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Lanes, Volumes, Timings 2: Main & Graham/Lees

Existing AM Peak Hour 15 Oblates

	€	-	1	†	-	↓					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĵ.		र्दी		414					
Traffic Volume (vph)	177	53	7	622	24	515					
Future Volume (vph)	177	53	7	622	24	515					
Lane Group Flow (vph)	197	240	0	770	0	688					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	60		123		33		123	33	60	7	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	0.84	0.53		0.47		0.44					
Control Delay	69.8	16.8		13.7		6.6					
Queue Delay	0.0	0.0		0.8		0.3					
Total Delay	69.8	16.8		14.5		6.9					
LOS	Е	В		В		Α					
Approach Delay		40.7		14.5		6.9					
Approach LOS		D		В		Α					
Queue Length 50th (m)	37.3	13.6		32.5		15.0					
Queue Length 95th (m)	#74.7	37.4		54.7		23.4					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	235	449		1631		1576					
Starvation Cap Reductn	0	0		525		336					
Spillback Cap Reductn	0	0		0		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	0.84	0.53		0.70		0.55					

Intersection Summary

Cycle Length: 100

Offset: 39 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

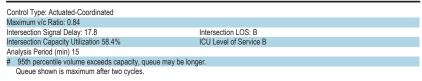
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Lanes, Volumes, Timings 2: Main & Graham/Lees

Existing AM Peak Hour
15 Oblates

Lanes, Volumes, Timings 3: Main & Evelyn

Existing AM Peak Hour
15 Oblates



	•	†	ļ	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥	†	^	
Traffic Volume (vph)	43	638	684	
Future Volume (vph)	43	638	684	
Lane Group Flow (vph)	101	709	760	
Turn Type	Perm	NA	NA	
Protected Phases	1 61111	2	6	7
Permitted Phases	8	2	U	,
Detector Phase	8	2	6	
Switch Phase	0	2	U	
	10.0	10.0	10.0	1.0
Minimum Initial (s)	25.4	27.1	15.1	
Minimum Split (s)				5.0
Total Split (s)	26.0	69.0	69.0	5.0
Total Split (%)	26.0%	69.0%	69.0%	5%
Maximum Green (s)	20.6	63.9	63.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	18	41		18
Act Effct Green (s)	20.6	63.9	63.9	
Actuated g/C Ratio	0.21	0.64	0.64	
v/c Ratio	0.29	0.64	0.36	
Control Delay	20.8	14.4	10.2	
	0.0	1.4	0.9	
Queue Delay	20.8	15.8	11.1	
Total Delay				
LOS	С	В	В	
Approach Delay	20.8	15.8	11.1	
Approach LOS	С	В	В	
Queue Length 50th (m)	8.2	76.6	33.1	
Queue Length 95th (m)	22.2	112.8	m45.6	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	348	1104	2097	
Starvation Cap Reductn	0	211	974	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.29	0.79	0.68	
Intersection Summary				
Cycle Length: 100				
Actuated Cycle Length: 100				

Intersection Summary
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 59 (59%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 65

Lanes, Volumes, Timings
3: Main & Evelyn

Control Type: Actuated-Coordinated

J Ø6 (R)

Existing AM Peak Hour 15 Oblates Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Existing AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated		
Maximum v/c Ratio: 0.64		
Intersection Signal Delay: 13.9	Intersection LOS: B	
Intersection Capacity Utilization 60.9%	ICU Level of Service B	
Analysis Period (min) 15		
m Volume for 95th percentile queue is metered by up	ostream signal.	
Splits and Phases: 3: Main & Evelyn		
†		
Ø2 (R)		_

	۶	→	•	-	4	†	-	↓				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	ሻ	î»	ሻ	î»	ሻ	ĵ»				
Traffic Volume (vph)	11	1	2	0	29	658	4	709				
Future Volume (vph)	11	1	2	0	29	658	4	709				
Lane Group Flow (vph)	0	32	2	6	32	734	4	800				
Turn Type	Perm	NA	Perm	NA	custom		custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	5.0	33.0	5.0
Total Split (s)	22.0	22.0	22.0	22.0	16.0		16.0		42.0	5.0	42.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	17.8%		17.8%		47%	6%	47%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	11.0		11.0		37.0	3.0	37.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0	3.0	20.0	3.0
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0	0.0	8.0	0.0
Pedestrian Calls (#/hr)	7	7	2	2					36	7	34	2
Act Effct Green (s)		16.7	16.7	16.7	56.2	50.8	51.5	46.1				
Actuated g/C Ratio		0.19	0.19	0.19	0.62	0.56	0.57	0.51				
v/c Ratio		0.12	0.01	0.01	0.12	0.75	0.01	0.91				
Control Delay		19.3	30.5	0.0	7.8	16.0	6.5	38.1				
Queue Delay		0.0	0.0	0.0	0.0	0.2	0.0	0.0				
Total Delay		19.3	30.5	0.0	7.8	16.2	6.5	38.1				
LOS		В	С	Α	Α	В	Α	D				
Approach Delay		19.3		7.6		15.9		37.9				
Approach LOS		В		Α		В		D				
Queue Length 50th (m)		1.9	0.3	0.0	1.3	31.6	0.3	132.7				
Queue Length 95th (m)		9.4	2.2	0.0	m3.1	#180.1	1.3	#216.7				
Internal Link Dist (m)		109.1		180.1		118.6		47.0				
Turn Bay Length (m)			15.0		15.0		30.0					
Base Capacity (vph)		275	158	545	336	974	416	882				
Starvation Cap Reductn		0	0	0.0	0	24	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.12	0.01	0.01	0.10	0.77	0.01	0.91				

Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 90

La	anes, Volumes, Timings
5:	: Main & Immaculata HS/Oblats
_	

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.91 Intersection Signal Delay: 26.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service B

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

M Volume for 95th percentile queue is metered by upstream signal.



Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	36	34
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		
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Existing AM Peak Hour

15 Oblates

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Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ની	7	7	ĵ.	ሻ	1>			
Traffic Volume (vph)	32	1	5	0	17	9	644	50	665			
Future Volume (vph)	32	1	5	0	17	9	644	50	665			
Lane Group Flow (vph)	0	47	0	6	19	10	734	56	762			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	27	27	9	9	9					30	30	
Act Effct Green (s)		15.8		15.8	15.8	47.5	50.7	55.4	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.21		0.03	0.06	0.04	0.77	0.23	0.69			
Control Delay		29.3		31.6	0.4	7.2	23.2	9.9	10.7			
Queue Delay		0.0		0.0	0.0	0.0	0.6	0.0	0.1			
Total Delay		29.3		31.6	0.4	7.2	23.8	9.9	10.8			
LOS		С		С	Α	Α	С	Α	В			
Approach Delay		29.3		7.9			23.6		10.8			
Approach LOS		С		Α			С		В			
Queue Length 50th (m)		5.5		0.9	0.0	0.6	96.4	3.2	31.4			
Queue Length 95th (m)		15.0		4.1	0.0	2.3	#170.5	m3.9	m44.3			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		221		174	322	339	959	275	1109			
Starvation Cap Reductn		0		0	0	0	0	0	30			
Spillback Cap Reductn		0		0	4	0	47	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.21		0.03	0.06	0.03	0.80	0.20	0.71			
Intersection Cummens												

In	te	rs	e	cti	01	1	S	u	m	m	ıa	ŋ	

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 75

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Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	43.0	5.0
Total Split (%)	6%	48%	6%
Maximum Green (s)	3.0	37.2	3.0
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	41	41	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

Lanes, Volumes, Timings 6: Main & Hazel

Existing AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.77 Intersection LOS: B
ICU Level of Service D Intersection Signal Delay: 17.1 Intersection Capacity Utilization 75.5% Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Main & Hazel



02-06-2023 CGH Transportation Page 13 Lanes, Volumes, Timings 1: Main & Hawthorne

Existing PM Peak Hour 15 Oblates

	→	*	1	†	-	+						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		413						
Traffic Volume (vph)	70	254	233	434	8	429						
Future Volume (vph)	70	254	233	434	8	429						
Lane Group Flow (vph)	400	282	0	754	0	687						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	15.0
Total Split (s)	24.0	20.0	20.0		54.0		5.0	54.0	5.0	5.0	5.0	15.0
Total Split (%)	20.0%	16.7%	16.7%		45.0%		4%	45%	4%	4%	4%	13%
Maximum Green (s)	17.7	13.7	13.7		47.7		3.0	47.7	3.0	3.0	3.0	8.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	None	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	6.7
Pedestrian Calls (#/hr)	27				18		50	50		18	50	50
Act Effct Green (s)	27.3	41.0		75.0		57.0						
Actuated g/C Ratio	0.23	0.34		0.62		0.48						
v/c Ratio	1.09	0.43		0.57		0.53						
Control Delay	117.3	5.4		11.9		21.3						
Queue Delay	0.0	0.0		0.8		0.0						
Total Delay	117.3	5.4		12.7		21.3						
LOS	F	A		В		C						
Approach Delay	71.1	,,		12.7		21.3						
Approach LOS	E			В		C						
Queue Length 50th (m)	~95.0	0.0		40.3		51.8						
Queue Length 95th (m)	#197.5	18.9		51.3		69.2						
Internal Link Dist (m)	198.7	10.0		59.0		262.1						
Turn Bay Length (m)	100.7			00.0		202.1						
Base Capacity (vph)	367	659		1327		1304						
Starvation Cap Reductn	0	0.09		276		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	U	U				-						
	0	0		0		0						

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 26 (22%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Lanes, Volumes, Timings 1: Main & Hawthorne

Existing PM Peak Hour 15 Oblates

Lane Group	Ø11
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	11
Detector Phases	
Switch Phase	F.0
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	10%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	1
Act Effct Green (s)	'
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

02-06-2023 JK CGH Transportation Page 2 Lanes, Volumes, Timings 1: Main & Hawthorne

Existing PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.09

Intersection Signal Delay: 34.2 Intersection Capacity Utilization 79.1% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service D

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne



Lanes, Volumes, Timings 2: Main & Graham/Lees

Existing PM Peak Hour 15 Oblates

	•	←	4	†	/	+					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	ሻ	1		475		479					
Traffic Volume (vph)	174	13	7	534	4	630					
Future Volume (vph)	174	13	7	534	4	630					
Lane Group Flow (vph)	193	161	0	677	0	734					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	15		34		25		34	25	15	3	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated q/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	0.70	0.37		0.41		0.41					
Control Delay	53.6	8.9		12.0		12.3					
Queue Delay	0.0	0.0		1.4		3.9					
Total Delay	53.6	8.9		13.4		16.3					
LOS	D	Α		В		В					
Approach Delay		33.2		13.4		16.3					
Approach LOS		С		В		В					
Queue Length 50th (m)	35.4	2.0		34.1		38.2					
Queue Length 95th (m)	#64.6	17.4		46.1		50.5					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	275	436		1667		1782					
Starvation Cap Reductn	0	0		746		942					
Spillback Cap Reductn	0	0		0		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	0.70	0.37		0.74		0.87					

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 35 (35%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

02-06-2023 **CGH Transportation** Page 4 JK

Lanes, Volumes, Timings 2: Main & Graham/Lees

Existing PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.70

Intersection LOS: B
ICU Level of Service A Intersection Signal Delay: 18.6 Intersection Capacity Utilization 42.0% Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Main & Graham/Lees



Lanes, Volumes, Timings 3: Main & Evelyn

Existing PM Peak Hour 15 Oblates

	•	†	↓	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	W	<u></u>	^	
Traffic Volume (vph)	23	590	794	
Future Volume (vph)	23	590	794	
Lane Group Flow (vph)	34	656	882	
Turn Type	Perm	NA	NA	
Protected Phases	r emi	2	6	7
Permitted Phases	8		U	- 1
Detector Phase	8	2	6	
Switch Phase	0	Z	0	
Minimum Initial (s)	10.0	10.0	10.0	1.0
	25.4		15.1	5.0
Minimum Split (s)	25.4	27.1 59.0	15.1 59.0	5.0
Total Split (s)				
Total Split (%)	28.9%	65.6%	65.6%	6%
Maximum Green (s)	20.6	53.9	53.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	8	28		8
Act Effct Green (s)	20.6	53.9	53.9	
Actuated g/C Ratio	0.23	0.60	0.60	
v/c Ratio	0.09	0.63	0.45	
Control Delay	23.3	8.3	10.8	
Queue Delay	0.0	0.0	1.9	
Total Delay	23.3	8.3	12.7	
LOS	C	Α.	В	
Approach Delay	23.3	8.3	12.7	
Approach LOS	25.5 C	Α.	В	
Queue Length 50th (m)	3.5	12.4	40.3	
Queue Length 95th (m)	11.0	34.6	53.3	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)	402.4	00.0	05.4	
Base Capacity (vph)	372	1034	1966	
	0	1034	881	
Starvation Cap Reductn			881	
Spillback Cap Reductn	0	0	-	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.09	0.64	0.81	
Intersection Summary				
Cycle Length: 90				
Actuated Cycle Langth: 00				

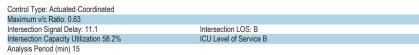
Actuated Cycle Length: 90
Offset: 54 (60%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

02-06-2023 JK CGH Transportation Page 6

Lanes, Volumes, Timings 3: Main & Evelyn

Existing PM Peak Hour 15 Oblates



Splits and Phases: 3: Main & Evelyn



	۶	-	1	←	4	†	-	↓				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		43-	ሻ	1>	76	ħ	*	1>				
Traffic Volume (vph)	4	0	3	0	1	589	5	825				
Future Volume (vph)	4	0	3	0	1	589	5	825				
Lane Group Flow (vph)	0	16	3	8	1	654	6	926				
Turn Type	Perm	NA	Perm	NA	custom	NA	custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	3.0	33.0	3.0
Total Split (s)	22.0	22.0	22.0	22.0	15.0		15.0		43.0	5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		48%	6%	48%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	10.0		10.0		38.0	3.0	38.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0		20.0	
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0		8.0	
Pedestrian Calls (#/hr)	6	6	5	5					30		27	
Act Effct Green (s)		16.7	16.7	16.7	55.9	50.8	56.1	50.9				
Actuated g/C Ratio		0.19	0.19	0.19	0.62	0.56	0.62	0.57				
v/c Ratio		0.04	0.02	0.01	0.01	0.67	0.02	0.95				
Control Delay		0.2	30.3	0.0	9.0	15.3	5.6	37.2				
Queue Delay		0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Total Delay		0.2	30.3	0.0	9.0	15.3	5.6	37.2				
LOS		Α	С	Α	Α	В	Α	D				
Approach Delay		0.2		8.3		15.3		37.0				
Approach LOS		Α		Α		В		D				
Queue Length 50th (m)		0.0	0.4	0.0	0.1	37.2	0.3	68.5				
Queue Length 95th (m)		0.0	2.8	0.0	m0.2	#72.5	m0.7	#261.8				
Internal Link Dist (m)		109.1		180.1		118.6		47.0				
Turn Bay Length (m)			15.0		15.0		30.0					
Base Capacity (vph)		377	181	560	274	975	453	975				
Starvation Cap Reductn		0	0	0	0	1	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.04	0.02	0.01	0.00	0.67	0.01	0.95				
Intersection Summary												

n	ter	se	cti	on	Su	mr	nary

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 62 (69%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 90

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases	J	13
Detector Phases		
Switch Phase		
	1.0	1.0
Minimum Initial (s)		
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	30	27
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		
intoroccion ouminary		

CGH Transportation Page 8

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Existing PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.95 Intersection LOS: C
ICU Level of Service C

Intersection Signal Delay: 27.7 Intersection Capacity Utilization 66.6% Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Main & Immaculata HS/Oblats



02-06-2023 CGH Transportation Page 10 Lanes, Volumes, Timings 6: Main & Hazel

Existing PM Peak Hour 15 Oblates

Lane Configurations	8 3.0 0 4.8 % 5%
Traffic Volume (vph) 18 0 25 1 52 10 517 19 779 Future Volume (vph) 18 0 25 1 52 10 517 19 779 Lane Group Flow (vph) 0 28 0 29 58 11 586 21 904 Turn Type Perm NA Perm NA Perm Lustom NA NA Protected Phases 4 8 8 13 12 9 56 1 Permitted Phases 4 8 8 8 2 6 6 Detector Phase 4 4 8 8 8 13 12 9 56 Switch Phase 8 8 8 13 12 9 56 Whirimum Initial (s) 10.0 10.0 10.0 10.0 5.0 5.0 5.0 1.0 10 Minimum Split (s) 21.2 <th>.0 1.0 8 3.0 0 4.8 % 5%</th>	.0 1.0 8 3.0 0 4.8 % 5%
Traffic Volume (vph) 18 0 25 1 52 10 517 19 779 Lane Group Flow (vph) 0 28 0 29 58 11 586 21 904 Turn Type Perm NA Perm NA Perm NA Perm NA NA Lane NA Lane NA Lane NA Lane NA Perm NA Perm NA Perm NA Perm NA Perm NA Lane NA NA NA Lane NA	.0 1.0 8 3.0 0 4.8 % 5%
Lane Group Flow (vph) 0 28 0 29 58 11 586 21 904 Turn Type Perm NA Perm NA Perm custom NA custom NA Protected Phases 4 8 8 13 12 9 56 1 Permitted Phases 4 4 8 8 13 12 9 56 Switch Phase 4 4 8 8 13 12 9 56 Switch Phase 4 10 10 10 10 5.0 5.0 5.0 1.0 1.0 11 Minimum Initial (s) 10.0 10.0 10.0 10.0 5.	.0 1.0 8 3.0 0 4.8 % 5%
Turn Type Perm NA Perm NA Perm custom NA custom NA Protected Phases 4 8 13 12 9 56 1 Permitted Phases 4 8 8 2 6 6 Detector Phase 4 4 8 8 13 12 9 56 Switch Phase Winimum Initial (s) 10.0 10.0 10.0 10.0 5.0 5.0 1.0 1.0 11.0	.0 1.0 8 3.0 0 4.8 % 5%
Protected Phases 4 8 13 12 9 56 1 Permitted Phases 4 8 8 2 6 6 Detector Phase 4 4 8 8 8 13 12 9 56 Switch Phase 8 8 13 12 9 56 Minimum Initial (s) 10.0 10.0 10.0 10.0 5.0 5.0 1.0 1.0 11.0	.0 1.0 8 3.0 0 4.8 % 5%
Permitted Phases 4 8 8 2 6 Detector Phase 4 4 8 8 8 13 12 9 56 Switch Phase Minimum Initial (s) 10.0 10.0 10.0 10.0 5.0 5.0 5.0 1.0 1.0 1.0 Minimum Initial (s) 21.2 21.2 21.2 21.2 21.2 10.8 10.8 5.0 3.	.0 1.0 8 3.0 0 4.8 % 5%
Detector Phase 4 4 8 8 8 13 12 9 56 Switch Phase 8 10.0 10.0 10.0 10.0 10.0 5.0 5.0 1	8 3.0 0 4.8 % 5%
Switch Phase Minimum Initial (s) 10.0 10.0 10.0 10.0 10.0 5.0 5.0 1.0 1.0 1.0 Minimum Split (s) 21.2 21.2 21.2 21.2 21.2 10.8 10.8 5.0 3.0	8 3.0 0 4.8 % 5%
Minimum Initial (s) 10.0 10.0 10.0 10.0 10.0 5.0 5.0 1.0 1.0 1.0 Minimum Split (s) 21.2 21.2 21.2 21.2 21.2 10.8 10.8 5.0 3.0	8 3.0 0 4.8 % 5%
Minimum Split (s) 21.2 21.2 21.2 21.2 21.2 10.8 10.8 5.0 34	8 3.0 0 4.8 % 5%
	.0 4.8 % 5%
	% 5%
Total Split (s) 21.2 21.2 21.2 21.2 21.2 15.0 15.0 5.0 4	
Total Split (%) 23.6% 23.6% 23.6% 23.6% 23.6% 16.7% 16.7% 6% 49	2 28
Maximum Green (s) 15.0 15.0 15.0 15.0 9.2 9.2 3.0 36	
Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0	.3 2.0
All-Red Time (s) 2.9 2.9 2.9 2.9 2.5 2.5 0.0	.5 0.0
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0	
Total Lost Time (s) 6.2 6.2 5.8 5.8	
Lead/Lag Lag Lag Lag Lag Lead Lead Lag	Lead
Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Yes	Yes
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	.0 3.0
Recall Mode Max Max Max Max None None Max C-W	x Max
Walk Time (s) 2.0 2.0 2.0 2.0 2.0 3.0 18	.0
Flash Dont Walk (s) 13.0 13.0 13.0 13.0 13.0 0.0 10	.0
Pedestrian Calls (#/hr) 17 17 9 9 9 9 30	10
Act Effct Green (s) 15.0 15.0 15.0 53.7 57.1 56.2 59.6	
Actuated g/C Ratio 0.17 0.17 0.60 0.63 0.62 0.66	
v/c Ratio 0.08 0.14 0.17 0.05 0.54 0.06 0.80	
Control Delay 0.5 34.1 1.0 7.0 12.8 3.2 7.4	
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.1	
Total Delay 0.5 34.1 1.0 7.0 12.8 3.2 7.5	
LOS A C A A B A A	
Approach Delay 0.5 12.1 12.7 7.4	
Approach LOS A B B A	
Queue Length 50th (m) 0.0 4.3 0.0 0.6 41.3 0.4 3.4	
Queue Length 95th (m) 0.0 12.0 0.0 2.3 97.8 m0.5 m#33.9	
Internal Link Dist (m) 237.6 98.5 241.0 118.6	
Turn Bay Length (m) 30.0 20.0 15.0	
Base Capacity (vph) 344 204 346 260 1092 384 1125	
Starvation Cap Reductn 0 0 0 0 0 8	
Spillback Cap Reductn 1 0 1 0 16 0 0	
Storage Cap Reductn 0 0 0 0 0 0	
Reduced v/c Ratio 0.08 0.14 0.17 0.04 0.54 0.05 0.81	

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90
Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Lanes, Volumes, Timings 6: Main & Hazel

Intersection Summary

Existing PM Peak Hour 15 Oblates

Lana Craun	OF.	αc	OZ.
Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	44.0	4.8
Total Split (%)	6%	49%	5%
Maximum Green (s)	3.0	38.2	2.8
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	31	31	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductin			
Reduced v/c Ratio			
Reduced v/c Ratio			

02-06-2023 JK CGH Transportation Page 12 Lanes, Volumes, Timings 6: Main & Hazel

Existing PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80

Intersection Signal Delay: 9.5 Intersection Capacity Utilization 68.1% Analysis Period (min) 15 Intersection LOS: A ICU Level of Service C

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Main & Hazel †_{Ø1} ⁴†_{Ø2 (R)} **↑** Ø13 Ø6 (R)

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Appendix D

Collision Data

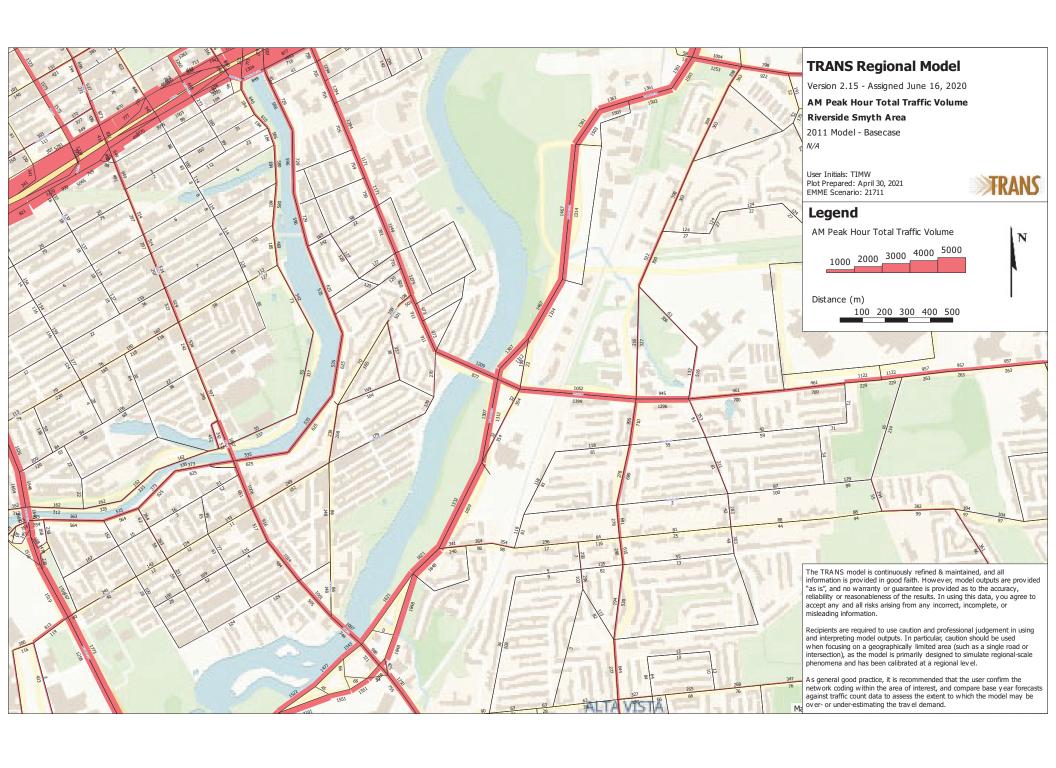


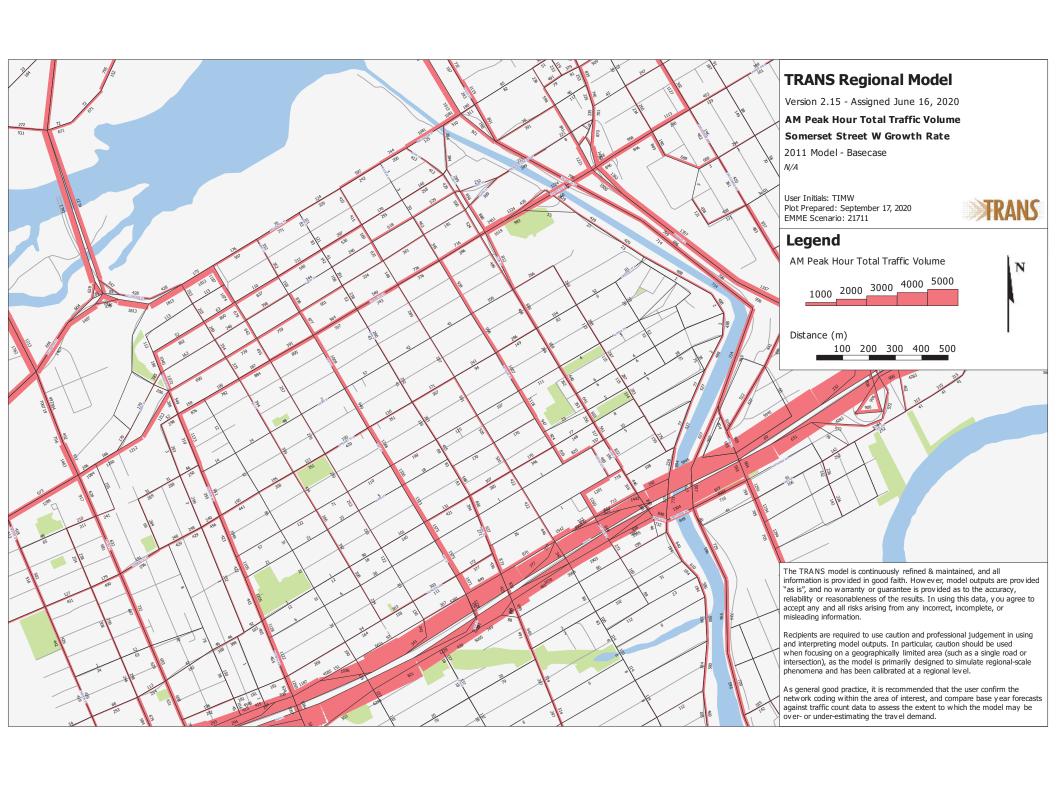
Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition	# Vehicles	# Motorcycles	# Bicycles	# Pedestrians
2019-09-28	2019	23:19	MAIN ST @ SPRINGHURST AVE (0007196)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2020-10-30	2020	14:50	MAIN ST @ SPRINGHURST AVE (0007196)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	05 - Turning movement	01 - Dry	2	0	1	0
2018-12-04	2018	11:00	MAIN ST @ OBLATE AVE (0002597)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	02 - Angle	01 - Dry	2	0	0	0
2019-02-02	2019	10:01	MAIN ST @ OBLATE AVE (0002597)	03 - Snow	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	02 - Angle	03 - Loose snow	2	0	0	0
2019-06-11	2019	13:55	MAIN ST @ OBLATE AVE (0002597)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2019-07-30	2019	13:54	ROSEMERE AVE @ SPRINGHURST AVE (0007320)	01 - Clear	01 - Daylight	02 - Stop sign	01 - Functioning	02 - Non-fatal injury	07 - SMV other	02 - Wet	1	0	0	1
2017-11-24	2017	12:35	MAIN ST btwn EVELYN AVE & SPRINGHURST AVE (3ZA3PZ)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2018-12-11	2018	16:07	MAIN ST btwn EVELYN AVE & SPRINGHURST AVE (3ZA3PZ)	03 - Snow	05 - Dusk	10 - No control	0	03 - P.D. only	04 - Sideswipe	02 - Wet	2	0	0	0
2019-11-30	2019	17:06	MAIN ST btwn EVELYN AVE & SPRINGHURST AVE (3ZA3PZ)	01 - Clear	05 - Dusk	10 - No control	0	03 - P.D. only	02 - Angle	01 - Dry	2	0	0	0
2019-03-01	2019	20:45	MAIN ST btwn EVELYN AVE & SPRINGHURST AVE (3ZA3PZ)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	99 - Other	01 - Dry	2	0	0	0
2019-07-24	2019	16:00	MAIN ST btwn EVELYN AVE & SPRINGHURST AVE (3ZA3PZ)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2019-10-01	2019	8:50	MAIN ST btwn SPRINGHURST AVE & OBLATE AVE (3ZA3B0)	02 - Rain	01 - Daylight	10 - No control	0	03 - P.D. only	04 - Sideswipe	02 - Wet	2	0	0	0
2019-03-18	2019	15:58	MAIN ST btwn SPRINGHURST AVE & OBLATE AVE (3ZA3B0)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2016-08-27	2016	2:41	MAIN ST btwn OBLATE AVE & HAZEL ST (3ZA3B8)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	07 - SMV other	01 - Dry	1	0	0	0
2018-01-06	2018	19:31	MAIN ST btwn OBLATE AVE & HAZEL ST (3ZA3B8)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	03 - Rear end	02 - Wet	2	0	0	0
2018-04-19	2018	20:49	MAIN ST btwn OBLATE AVE & HAZEL ST (3ZA3BB)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	05 - Turning movement	01 - Dry	2	0	0	0
2019-11-08	2019	12:00	MAIN ST btwn OBLATE AVE & HAZEL ST (3ZA3B8)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2019-07-06	2019	12:00	MAIN ST btwn OBLATE AVE & HAZEL ST (3ZA3B8)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	02 - Angle	01 - Dry	2	0	0	0
2018-04-27	2018	14:49	DES OBLATS AVE btwn DESCHATELETS AVE & MAIN ST (3ZA1EA)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle	01 - Dry	1	0	0	0

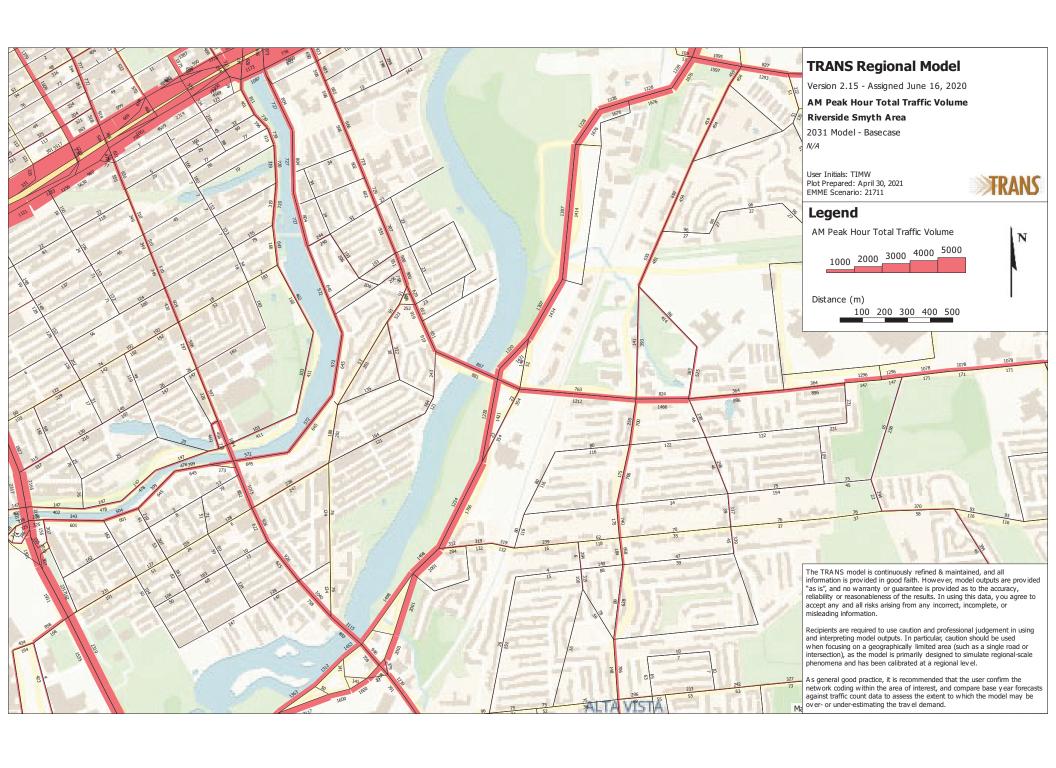
Appendix E

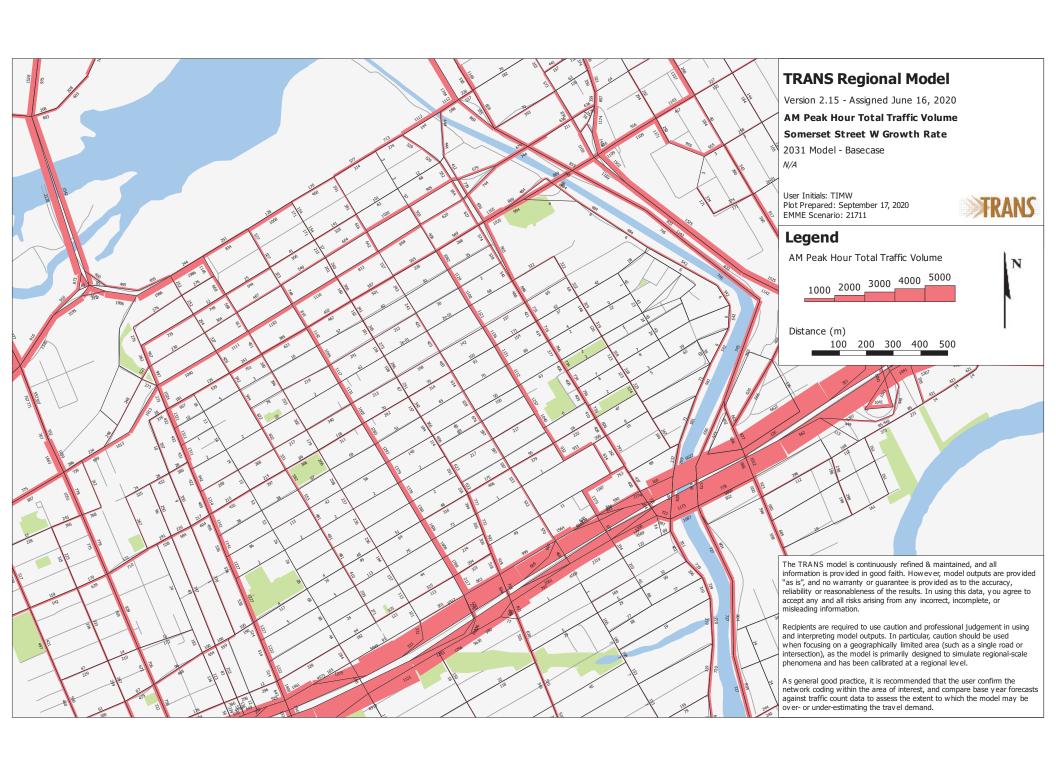
TRANS Model Plots











Appendix F

Synchro Intersection Worksheets – 2025 Future Background Conditions



	-	•	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		ની કે						
Traffic Volume (vph)	12	234	293	531	5	404						
Future Volume (vph)	12	234	293	531	5	404						
Lane Group Flow (vph)	312	234	0	830	0	523						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	28				20		53	53		20	53	53
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.77	0.32		0.63		0.49						
Control Delay	50.6	3.9		9.4		24.2						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	50.6	3.9		9.5		24.2						
LOS	D	Α		Α		С						
Approach Delay	30.6			9.5		24.2						
Approach LOS	С			Α		С						
Queue Length 50th (m)	54.2	0.0		24.6		37.2						
Queue Length 95th (m)	#126.1	14.5		31.3		52.5						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	403	728		1317		1059						
Starvation Cap Reductn	0	0		37		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.77	0.32		0.65		0.49						
Intersection Summary												

m	ter	se	cti	on	Su	mn	nary	/

Intersection Summary
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 60 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 85

	\rightarrow	7	1	†	-	¥						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		र्दी						
Traffic Volume (vph)	12	234	293	531	5	404						
Future Volume (vph)	12	234	293	531	5	404						
Lane Group Flow (vph)	312	234	0	830	0	523						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	28				20		53	53		20	53	53
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.77	0.32		0.63		0.49						
Control Delay	50.6	3.9		9.4		24.2						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	50.6	3.9		9.5		24.2						
LOS	D	Α		Α		С						
Approach Delay	30.6			9.5		24.2						
Approach LOS	С			Α		С						
Queue Length 50th (m)	54.2	0.0		24.6		37.2						
Queue Length 95th (m)	#126.1	14.5		31.3		52.5						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	403	728		1317		1059						
Starvation Cap Reductn	0	0		37		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.77	0.32		0.65		0.49						
Intersection Summary												

Lane Group	Ø11
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	12%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	3
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings 1: Main & Hawthorne

Future Background 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.77 Intersection Signal Delay: 19.6
Intersection Capacity Utilization 77.7%
Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne



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Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2025AM Peak Hour

Lane Group		€	-	1	†	-	ţ					
Traffic Volume (vph)	Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Traffic Volume (vph)	Lane Configurations	*	ĵ.		413		4134					
Future Volume (vph)		243		7		24						
Turn Type		243	53	7	664	24	534					
Protected Phases 8 78 12 56 1 5 7 13 Permitted Phases 8 2 6 Detector Phase 8 78 2 12 6 56 Switch Phase Minimum Initial (s) 10.0 10.0 10.0 1.0 1.0 1.0 5.0 Minimum Initial (s) 24.0 18.2 18.2 5.0 5.0 5.0 11.0 Total Split (s) 24.0 55.0 55.0 5.0 5.0 5.0 11.0 Total Split (s) 24.0 55.0 55.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 5.0 5.0 5.0 5.0 5.0 11.0 Total Split (w) 24.0 55.0 55.0 55.0 5.0 5.0 5.0 5.0 5.0 5.	Lane Group Flow (vph)	243	271	0	740	0	638					
Permitted Phases 8 78 2 12 6 5 5 6 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	Turn Type	custom	NA	custom	NA	custom	NA					
Detector Phase 8	Protected Phases		78		12		56	1	5	7	13	
Switch Phase Minimum Initial (s)	Permitted Phases	8		2		6						
Minimum Initial (s) 10.0 10.0 10.0 1.0 1.0 1.0 5.0 Minimum Split (s) 24.0 18.2 18.2 5.0 5.0 5.0 11.0 Total Split (s) 24.0 55.0 55.0 5.0 5.0 5.0 11.0 Total Split (%) 24.0% 55.0% 55.0% 55.0 55.0 50.0 11.0 Total Split (%) 24.0% 55.0% 55.0% 55.0% 55.0 55.0 50.0 11.0 Maximum Green (s) 18.0 48.8 48.8 3.0 3.0 3.0 5.0 Yellow Time (s) 2.7 2.9 2.9 0.0 0.0 0.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	Detector Phase	8	78	2	12	6	56					
Minimum Split (s) 24.0 18.2 18.2 5.0 5.0 5.0 11.0 Total Split (s) 24.0 55.0 55.0 55.0 5.0 5.0 5.0 11.0 Total Split (%) 24.0% 55.0% 55.0% 5% 11% Maximum Green (s) 18.0 48.8 48.8 3.0 3.0 3.0 5.0 20 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	Switch Phase											
Total Split (s)	Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Total Split (%)	Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Maximum Green (s) 18.0 48.8 48.8 3.0 3.0 3.0 5.0 Yellow Time (s) 3.3 3.3 3.3 2.0 2.0 2.0 3.3 All-Red Time (s) 2.7 2.9 2.9 0.0 0.0 0.0 2.7 Lost Time Adjust (s) 0.0 Total Lost Time (s) Lag Lag Lead Lead Lead Lag Lead	Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Yellow Time (s) 3.3 3.3 3.3 2.0 2.0 2.0 3.3 All-Red Time (s) 2.7 2.9 2.9 0.0 0.0 0.0 2.7 Lost Time Adjust (s) 0.0 Total Lost Time (s) Lead-Lag Optimize? Yes	Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
All-Red Time (s) 2.7 2.9 2.9 0.0 0.0 0.0 2.7 Total Lost Time Adjust (s) 0.0 Total Lost Time (s) 6.0 Lead/Lag Lag Lag Lead Lead Lag Lead Lead-Lag Optimize? Yes	Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Lost Time Adjust (s) 0.0 Total Lost Time (s) 6.0 Lead/Lag Lag Lag Lead Lead Lag Lead Lead/Lag Cytimize? Yes Yes Yes Yes Yes Yes Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0	Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
Total Lost Time (s) 6.0 Lead/Lag Lag Lag Lead Lead <td>All-Red Time (s)</td> <td>2.7</td> <td></td> <td>2.9</td> <td></td> <td>2.9</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>2.7</td> <td></td>	All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lead/Lag Lag Lag Lead Lead L	Lost Time Adjust (s)	0.0										
Lead-Lag Optimize? Yes	Total Lost Time (s)	6.0										
Vehicle Extension (s) 3.0	Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Recall Mode Max C-Max C-Max Max	Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Walk Time (s) 2.0 2.0 2.0 3.0 3.0 3.0 5.0 Flash Dont Walk (s) 9.0 10.0 10.0 7 Act Effet Green (s) 18.0 27.0 58.0 58.0 58.0 Act Carlot Green (s) 4.0 Carlot Green (s) 4.0 Carlot Green (s) 4.0 Carlot Green (s) 4.0 Carlot Green (s) 6.0 4.5 0.40 Carlot Green (s) 6.0 4.0 Carlot Green (s) 6.5 Carlot Green (s) 6.7 Carl	Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Flash Dont Walk (s) 9.0 10.0 10.0 0.0 0.0 0.0 0.0 0.0 Pedestrian Calls (#/hr) 60 123 33 123 33 60 7 Act Effet Green (s) 18.0 27.0 58.0 58.0	Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Pedestrian Calls (#/hr) 60 123 33 123 33 60 7 Act Effet Green (s) 18.0 27.0 58.0 58.0 Act Actuated g/C Ratio 0.18 0.27 0.58 0.59 0.59 0.59 0.09 <td>Walk Time (s)</td> <td>2.0</td> <td></td> <td>2.0</td> <td></td> <td>2.0</td> <td></td> <td>3.0</td> <td>3.0</td> <td>3.0</td> <td>5.0</td> <td></td>	Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Act Effct Green (s) 18.0 27.0 58.0 58.0 Actuated g/C Ratio 0.18 0.27 0.58 0.58 (vic Ratio 1.03 0.56 0.45 0.40 Control Delay 109.8 13.9 13.2 6.5 Queue Delay 0.0 0.0 0.7 0.2 Total Delay 109.8 13.9 13.9 6.7 LOS F B B A A Approach Delay 59.2 13.9 6.7 Approach LOS E B A A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 50th (m) 470.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reducth 0 0 0 529 360 Spillback Cap Reducth 0 0 0 0 0 0		9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Actuated g/C Ratio 0.18 0.27 0.58 0.58 v/c Ratio 1.03 0.56 0.45 0.40 Control Delay 109.8 13.9 13.2 6.5 Queue Delay 0.0 0.0 0.7 0.2 Total Delay 109.8 13.9 13.9 6.7 LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0	Pedestrian Calls (#/hr)	60		123		33		123	33	60	7	
v/c Ratio 1.03 0.56 0.45 0.40 Control Delay 109.8 13.9 13.2 6.5 Queue Delay 0.0 0.0 0.7 0.2 Total Delay 109.8 13.9 13.9 6.7 LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 8ase Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0	Act Effct Green (s)	18.0	27.0		58.0		58.0					
Control Delay 109.8 13.9 13.2 6.5 Queue Delay 0.0 0.0 0.7 0.2 Total Delay 109.8 13.9 13.9 6.7 LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 40.0 426.1 69.4 59.0 Turn Bay Length (m) 40.0 8ase Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Actuated g/C Ratio	0.18	0.27		0.58		0.58					
Queue Delay 0.0 0.0 0.7 0.2 Total Delay 109.8 13.9 13.9 6.7 LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 8 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	v/c Ratio	1.03	0.56		0.45		0.40					
Total Delay 109.8 13.9 13.9 6.7 LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 8ase Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0	Control Delay	109.8	13.9		13.2		6.5					
LOS F B B A Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0	Queue Delay	0.0	0.0		0.7		0.2					
Approach Delay 59.2 13.9 6.7 Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 888e Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0	Total Delay		13.9		13.9		6.7					
Approach LOS E B A Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Storage Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	LOS	F										
Queue Length 50th (m) ~50.8 10.2 30.7 13.8 Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 8ase Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0	Approach Delay				13.9							
Queue Length 95th (m) #97.0 35.0 50.9 22.0 Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Approach LOS				В							
Internal Link Dist (m) 426.1 69.4 59.0 Turn Bay Length (m) 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0												
Turn Bay Length (m) 40.0 Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0		#97.0										
Base Capacity (vph) 235 480 1631 1589 Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0	Internal Link Dist (m)		426.1		69.4		59.0					
Starvation Cap Reductn 0 0 529 360 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0												
Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0		235	480									
Storage Cap Reductn 0 0 0 0												
		-	-		-		-					
Reduced v/c Ratio 1.03 0.56 0.67 0.52												
	Reduced v/c Ratio	1.03	0.56		0.67		0.52					

Intersection Summary

Cycle Length: 100

Offset: 39 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

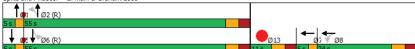
JK

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03 Intersection LOS: C ICU Level of Service B Intersection Signal Delay: 23.8 Intersection Capacity Utilization 62.6% Analysis Period (min) 15 Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Main & Graham/Lees



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Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2025AM Peak Hour 15 Oblates

	•	1	ļ	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	W	†	44	
Traffic Volume (vph)	44	686	710	
Future Volume (vph)	44	686	710	
Lane Group Flow (vph)	92	686	710	
Turn Type	Perm	NA	NA	
Protected Phases		2	6	7
Permitted Phases	8			
Detector Phase	8	2	6	
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	69.0	69.0	5.0
Total Split (%)	26.0%	69.0%	69.0%	5%
Maximum Green (s)	20.6	63.9	63.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag		***	Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0	-	3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	18	41		18
Act Effct Green (s)	20.6	63.9	63.9	
Actuated g/C Ratio	0.21	0.64	0.64	
v/c Ratio	0.27	0.62	0.34	
Control Delay	20.1	13.9	11.8	
Queue Delay	0.0	1.2	0.9	
Total Delay	20.1	15.1	12.7	
LOS	C	В	В	
Approach Delay	20.1	15.1	12.7	
Approach LOS	С	В	В	
Queue Length 50th (m)	7.0	72.4	34.4	
Queue Length 95th (m)	20.3	106.6	m42.5	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	347	1104	2097	
Starvation Cap Reductn	0	216	1035	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.27	0.77	0.67	
	V	····	0.07	
Intersection Summary				

Cycle Length: 100

Actuated Cycle Length: 100
Offset: 59 (59%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.62 Intersection LOS: B
ICU Level of Service B Intersection Signal Delay: 14.3 Intersection Capacity Utilization 63.5% Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn 1 Ø2 (R) Ø6 (R)

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Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2025AM Peak Hour

Lane Group	Ø7 7
Traffic Volume (vph)	7
Traffic Volume (vph) 11 1 37 0 29 670 22 721 Future Volume (vph) 11 1 37 0 29 670 22 721 Lane Group Flow (vph) 0 29 37 40 29 690 22 732 Turn Type Perm NA Perm NA custom NA custom NA Protected Phases 4 8 5 29 1 613 2 3 6 Permitted Phases 4 4 8 8 5 29 1 613 2 3 6 Permitted Phases 4 4 8 8 5 29 1 613 3 6 Permitted Phases 4 4 8 8 5 29 1 613 3 6 0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0	7
Lane Group Flow (vph)	7
Turn Type	7
Protected Phases	7
Permitted Phases	7
Detector Phase 4	- 1
Switch Phase Swit	
Minimum Initial (s) 10.0 33.0 5.0 33.0 5.0 33.0 5.0 33.0 5.0 33.0 5.0 30.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 42.0 47	
Minimum Split (s) 19.3 19.3 19.3 19.3 19.3 10.0 10.0 33.0 5.0 33.0 Total Split (s) 22.0 22.0 22.0 16.0 16.0 42.0 5.0 42.0 Total Split (w) 24.4% 24.4% 24.4% 17.8% 17.8% 47% 6% 47% Maximum Green (s) 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 2.0 2.0 2.0 2.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 1.7 1.7 0.0 1.7 Lead-Lag Lag La	
Total Split (s) 22.0 22.0 22.0 22.0 22.0 16.0 42.0 5.0 42.0 Total Split (%) 24.4% 24.4% 24.4% 17.8% 17.8% 47% 6% 47% Maximum Green (s) 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 3.3 All-Red Time (s) 2.0 2.0 2.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 5.3 5.3 5.3 5.0	1.0
Total Split (%)	5.0
Maximum Green (s) 16.7 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 2.0 3.3 3.3 3.3 3.3 2.0 2.0 3.3 3.3 3.3 2.0 3.3 3.3 3.3 3.3 2.0 3.3 3.3 3.3 3.3 3.3 2.0 2.0 3.7 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 0.0 1.7 0.0 1.7 1.7 1.7 1.7 0.0 0.0 1.7 0.0 1.7 0.0 0.0 0.0	5.0
Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 3.3 All-Red Time (s) 2.0 2.0 2.0 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 0.0 1.7 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	6%
All-Red Time (s) 2.0 2.0 2.0 2.0 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.3 5.3 5.3 5.3 5.0 5.0 Lead/Lag Lag Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Yes Yes Yes Optimize? Yes	3.0
Lost Time Adjust (s) 0.0	2.0
Total Lost Time (s)	0.0
Lead/Lag Lag Lag Lag Lag Lag Lead	
Lead-Lag Optimize? Yes	
Vehicle Extension (s) 3.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 3.0 8.0 9.0 8.0 9.0 3.0 8.0 9.0 3.0 3.0 3.0 3.0 3.0	Lead
Recall Mode Max Max Max Max None None C-Max Max C-Max	Yes
Walk Time (s) 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 20.0 8.0 20.0 8.0 20.0 8.0 0.0 8.0 8.0 0.0 8.0 8.0 9.0 8.0 7 34	3.0
Flash Dont Walk (s) 12.0 12.0 12.0 12.0 12.0 8.0 0.0 8.0 Pedestrian Calls (#/hr) 7 7 7 2 2 2 2 3 36 7 34 Act Effet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 0.60 0.54 0/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	Max
Pedestrian Calls (#/hr) 7 7 2 2 36 7 34 Act Effet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 Vic Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	3.0
Act Leffet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.60 0.54 0.60 0.54 V/C Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	0.0
Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 v/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	2
V/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	
Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	
O	
Queue Delay 0.0 0.0 0.0 0.0 0.3 0.0 0.0	
Total Delay 19.7 35.6 0.2 3.8 12.6 6.7 26.7	
LOS B D A A B A C	
Approach Delay 19.7 17.2 12.3 26.1	
Approach LOS B B B C	
Queue Length 50th (m) 1.7 5.5 0.0 0.6 15.0 1.2 83.2	
Queue Length 95th (m) 8.9 14.3 0.0 m1.4 #168.2 3.7 #189.3	
Internal Link Dist (m) 109.1 138.0 118.6 47.0	
Turn Bay Length (m) 15.0 15.0 30.0	
Base Capacity (vph) 271 159 556 394 917 428 926	
Starvation Cap Reductn 0 0 0 0 24 0 0	
Spillback Cap Reductn 0 0 0 0 0 0 0	
Storage Cap Reductn 0 0 0 0 0 0	
Reduced v/c Ratio 0.11 0.23 0.07 0.07 0.77 0.05 0.79	

Intersection Summary

Cycle Length: 90

Office Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 80

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Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.79	
	Intersection LOS: B
Intersection Capacity Utilization 61.1%	ICU Level of Service B
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be long	er.
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream sign	gnal.

Splits and Phases:	5: Main & Immaculata HS/Oblats	
ø ₀₁	↑ Ø2 (R)	● Ø8 → Ø4
16 s	5 s 42 s	5 s 22 s
↑ ø5	Ø13 Ø6 (R)	● ø ₇ ▼ ø ₈
16 s	5 s 42 s	5 s 22 s

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	36	34
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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 CGH Transportation

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Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ર્ન	7	, F	f)	ሻ	î»			
Traffic Volume (vph)	32	3	16	2	28	9	663	62	701			
Future Volume (vph)	32	3	16	2	28	9	663	62	701			
Lane Group Flow (vph)	0	44	0	18	28	9	689	62	722			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	27	27	9	9	9					30	30	
Act Effct Green (s)		15.8		15.8	15.8	47.3	50.5	55.5	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.20		0.09	0.09	0.03	0.73	0.23	0.65			
Control Delay		29.2		32.7	0.5	7.2	21.5	11.4	14.1			
Queue Delay		0.0		0.0	0.0	0.0	0.4	0.0	0.1			
Total Delay		29.2		32.7	0.5	7.2	21.9	11.4	14.2			
LOS		С		C	Α	Α	C	В	В			
Approach Delay		29.2		13.1			21.7		14.0			
Approach LOS		C		В	0.0	0.5	C	0.0	В			
Queue Length 50th (m)		5.2		2.7	0.0	0.5	87.4	3.0	35.0			
Queue Length 95th (m)		14.5		8.5	0.0	2.1	139.1	m8.3	94.4			
Internal Link Dist (m)		237.6		98.5	00.0	00.0	241.0	45.0	118.6			
Turn Bay Length (m)		000		400	30.0	20.0	0.40	15.0	4444			
Base Capacity (vph)		223		192	322	364	949	300	1111			
Starvation Cap Reductn		0		0	0	0	0	0	37			
Spillback Cap Reductn		0		0	4	0	44	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.20		0.09	0.09	0.02	0.76	0.21	0.67			

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 9 Offset: 46 (51%), Referen

Natural Cycle: 75

02-06-2023 JK

	•	-	•	←	*	1	†	-	ļ			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		4	7	*	ĵ,	ሻ	₽			
Traffic Volume (vph)	32	3	16	2	28	9	663	62	701			
Future Volume (vph)	32	3	16	2	28	9	663	62	701			
Lane Group Flow (vph)	0	44	0	18	28	9	689	62	722			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	27	27	9	9	9					30	30	
Act Effct Green (s)		15.8		15.8	15.8	47.3	50.5	55.5	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.20		0.09	0.09	0.03	0.73	0.23	0.65			
Control Delay		29.2		32.7	0.5	7.2	21.5	11.4	14.1			
Queue Delay		0.0		0.0	0.0	0.0	0.4	0.0	0.1			
Total Delay		29.2		32.7	0.5	7.2	21.9	11.4	14.2			
LOS		С		С	Α	Α	С	В	В			
Approach Delay		29.2		13.1			21.7		14.0			
Approach LOS		С		В			С		В			
Queue Length 50th (m)		5.2		2.7	0.0	0.5	87.4	3.0	35.0			
Queue Length 95th (m)		14.5		8.5	0.0	2.1	139.1	m8.3	94.4			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		223		192	322	364	949	300	1111			
Starvation Cap Reductn		0		0	0	0	0	0	37			
Spillback Cap Reductn		0		0	4	0	44	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.20		0.09	0.09	0.02	0.76	0.21	0.67			

h: 90 erenced to phase 2:NBTL and 6:SBTL, Start of Green		
CGH Transportation Page 12	02-06-2023 JK	CGH Transportation Page 13

Lanes, Volumes, Timings

1.0 10.0 34.8

5.0

6% 48%

2.0

0.0

Lag

Yes

3.0

3.0 18.0 10.0

41 41

Max C-Max

43.0 5.0

37.2

3.3

2.5

3.0

3.0

5.0

6%

3.0

2.0

0.0

Lead

Yes

3.0

Max

6: Main & Hazel

Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s)

Minimum Split (s)

Maximum Green (s)

Lost Time Adjust (s) Total Lost Time (s) Lead/Lag

Lead-Lag Optimize?

Recall Mode

Vehicle Extension (s)

Walk Time (s)
Flash Dont Walk (s)

Pedestrian Calls (#/hr)

Intersection Summary

Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m) Internal Link Dist (m) Turn Bay Length (m) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio

Total Split (s)

Total Split (%)

Yellow Time (s)

All-Red Time (s)

Lanes, Volumes, Timings 6: Main & Hazel

Future Background 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 17.8
Intersection Capacity Utilization 77.3%
Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Main & Hazel

Opinto una i masc	Jo. O. Main a nazor	
Ø9	↑ø1 ↑ø2 (R)	● ∅β ↑ ∅4
15 s	5 s 43 s	5 s 22 s
↑ ø13	₩ Ø6 (R)	● ø7 ◆ ø8
15 s	5 s 43 s	5 s 22 s

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Lanes, Volumes, Timings 1: Main & Hawthorne

Future Background 2025PM Peak Hour 15 Oblates

	-	•	4	†	-	ļ						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ની	7		ની કે		413						
Traffic Volume (vph)	70	276	261	472	8	466						
Future Volume (vph)	70	276	261	472	8	466						
Lane Group Flow (vph)	360	276	0	746	0	662						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	15.0
Total Split (s)	24.0	20.0	20.0		54.0		5.0	54.0	5.0	5.0	5.0	15.0
Total Split (%)	20.0%	16.7%	16.7%		45.0%		4%	45%	4%	4%	4%	13%
Maximum Green (s)	17.7	13.7	13.7		47.7		3.0	47.7	3.0	3.0	3.0	8.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	None	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	6.7
Pedestrian Calls (#/hr)	27				18		50	50		18	50	50
Act Effct Green (s)	27.3	41.0		75.0		57.0						
Actuated g/C Ratio	0.23	0.34		0.62		0.48						
v/c Ratio	0.98	0.42		0.56		0.50						
Control Delay	89.3	5.4		11.8		20.9						
Queue Delay	0.0	0.0		0.7		0.0						
Total Delay	89.3	5.4		12.6		20.9						
LOS	F	Α		В		С						
Approach Delay	52.9			12.6		20.9						
Approach LOS	D			В		С						
Queue Length 50th (m)	82.5	0.0		39.7		49.4						
Queue Length 95th (m)	#175.9	18.7		50.7		66.2						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	367	654		1336		1311						
Starvation Cap Reductn	0	0		285		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.98	0.42		0.71		0.50						

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 26 (22%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

JK

1: Main & Hawthorne
Control Type: Actuated-Coordina

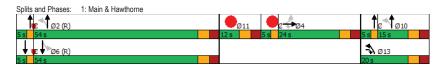
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98
Intersection Signal Delay: 27.8
Intersection Capacity Utilization 82.4%

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Lane Group	Ø11		
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	11		
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	5.0		
Minimum Split (s)	12.0		
Total Split (s)	12.0		
Total Split (%)	10%		
Maximum Green (s)	5.7		
Yellow Time (s)	3.3		
All-Red Time (s)	3.0		
Lost Time Adjust (s)	0.0		
Total Lost Time (s)			
Lead/Lag	Lead		
Lead-Lag Optimize?	Yes		
Vehicle Extension (s)	3.0		
Recall Mode	None		
Walk Time (s)	5.7		
Flash Dont Walk (s)	0.0		
Pedestrian Calls (#/hr)	1		
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

 02-06-2023
 CGH Transportation

 JK
 Page 2

Future Background 2025PM Peak Hour 15 Oblates Lanes, Volumes, Timings Future Background 2025PM Peak Hour 2: Main & Graham/Lees

> 4 689

10.0

18.2

55.0

48.8

3.3

2.9

Lag

Yes

3.0

2.0

10.0

25

0.58

0.40

12.2

3.6

15.8

15.8

37.2

49.4

59.0

1795

C-Max

55.0%

720

NA 56

56

1.0 1.0

5.0 5.0

5.0

5% 5%

3.0

2.0

0.0

Lead Lead

Yes

3.0

Max

3.0

0.0

34

5.0

3.0

2.0

0.0

Yes

3.0

Max

3.0

25

1.0 5.0

5.0 11.0

5.0 11.0

5% 11%

3.0

2.0 3.3

0.0 2.7

Lag Lead

Yes

3.0

Max

3.0 5.0

15

5.0

Yes

3.0

Max

0.0

3

NBT

41 → 593

699

NA custom

8 593

2

10.0

18.2

55.0

48.8

3.3

2.9

Lag

Yes

3.0

2.0

10.0

34

58.0 0.58

0.43

12.1

1.5

13.6

13.6

35.3

47.9

69.4

1635

В

В

C-Max

55.0%

Lane Group

Lane Configurations

Traffic Volume (vph)

Future Volume (vph)

Turn Type Protected Phases

Permitted Phases

Minimum Initial (s)

Minimum Split (s)

Maximum Green (s)

Lost Time Adjust (s)

Total Lost Time (s)

Lead-Lag Optimize?

Flash Dont Walk (s)

Act Effct Green (s)

Actuated g/C Ratio

v/c Ratio

Control Delay

Queue Delay

Total Delay

Approach Delay

Queue Length 50th (m)

Queue Length 95th (m)

Internal Link Dist (m)

Turn Bay Length (m)

Approach LOS

LOS

JK

Pedestrian Calls (#/hr)

Vehicle Extension (s)

Lead/Lag

Recall Mode

Walk Time (s)

Detector Phase

Switch Phase

Total Split (s)

Total Split (%)

Yellow Time (s)

All-Red Time (s)

Lane Group Flow (vph)

192

192

192

8

10.0

24.0

24.0

18.0

3.3

2.7

0.0

6.0

3.0

Max

2.0

9.0

15

18.0 27.0

0.18 0.27

0.70 0.34

53.3

0.0

53.3

35.3

#64.2

40.0

D

8.9

0.0

8.9

34.0

1.9

16.6

426.1

24.0%

custom

13

NA custom

15 Oblates

	•	\rightarrow	*	1	-	•	1	1	1	-	Į.	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ની	7					413			414	
Traffic Volume (vph)	290	70	276	0	0	0	261	472	13	8	466	188
Future Volume (vph)	290	70	276	0	0	0	261	472	13	8	466	188
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)		6.3	6.3					2.0			2.0	
Lane Util. Factor		1.00	1.00					0.95			0.95	
Frpb, ped/bikes		1.00	0.95					0.99			0.89	
Flpb, ped/bikes		0.96	1.00					1.00			1.00	
Frt		1.00	0.85					1.00			0.96	
Flt Protected		0.96	1.00					0.98			1.00	
Satd. Flow (prot)		1614	1384					3205			2836	
Flt Permitted		0.96	1.00					0.55			0.95	
Satd. Flow (perm)		1614	1384					1799			2686	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	290	70	276	0	0	0	261	472	13	8	466	188
RTOR Reduction (vph)	0	0	182	0	0	0	0	1	0	0	40	0
Lane Group Flow (vph)	0	360	94	0	0	0	0	745	0	0	622	0
Confl. Peds. (#/hr)	21		27	27		21	18		50	50		18
Confl. Bikes (#/hr)			2			1			2			2
Heavy Vehicles (%)	2%	2%	4%	2%	2%	2%	3%	2%	2%	2%	2%	2%
Turn Type	Perm	NA	pm+ov				custom	NA		custom	NA	
Protected Phases		4	13				13 1	1 2 9 10			56	
Permitted Phases	4		4				10 2			6		
Actuated Green, G (s)		27.3	41.0					61.4			47.7	
Effective Green, g (s)		27.3	41.0					61.4			47.7	
Actuated g/C Ratio		0.23	0.34					0.51			0.40	
Clearance Time (s)		6.3	6.3									
Vehicle Extension (s)		3.0	3.0									
Lane Grp Cap (vph)		367	545					1081			1067	
v/s Ratio Prot			0.02					c0.08				
v/s Ratio Perm		0.22	0.05					c0.27			0.23	
v/c Ratio		0.98	0.17					0.69			0.58	
Uniform Delay, d1		46.1	27.6					22.1			28.3	
Progression Factor		1.00	1.00					1.00			1.00	
Incremental Delay, d2		42.4	0.7					1.8			2.3	
Delay (s)		88.5	28.3					23.9			30.7	
Level of Service		F	С					С			С	
Approach Delay (s)		62.4			0.0			23.9			30.7	
Approach LOS		Е			Α			С			С	
Intersection Summary												
HCM 2000 Control Delay			38.1	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capacity	/ ratio		0.78									
Actuated Cycle Length (s)			120.0	S	um of lost	time (s)			31.2			
Intersection Capacity Utilization	n		82.4%	IC	U Level	of Service)		Е			
Associated Posteriol (sector)												
Analysis Period (min)			15									

base Capacity (vpri)	210	421	1035	1700	
Starvation Cap Reductn	0	0	706	950	
Spillback Cap Reductn	0	0	0	0	
Storage Cap Reductn	0	0	0	0	
Reduced v/c Ratio	0.70	0.34	0.75	0.86	
Intersection Summary					
Cycle Length: 100					
Actuated Cycle Length: 100					
Offset: 35 (35%), Reference	d to phase 2	2:NBTL and	6:SBTL, Start of Gre	en	
Natural Cycle: 65					

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Lanes, Volumes, Timings 2: Main & Graham/Lees

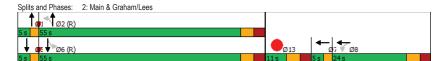
Future Background 2025PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70
Intersection Signal Delay: 18.5
Intersection LOS: B
Intersection Capacity Utilization 46.7%
ICU Level of Service A
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



 02-06-2023
 CGH Transportation

 JK
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HCM Signalized Intersection Capacity Analysis 2: Main & Graham/Lees

Future Background 2025PM Peak Hour
15 Oblates

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ሻ	1>			414			414	
Traffic Volume (vph)	0	0	0	192	13	134	8	593	98	4	689	27
Future Volume (vph)	0	0	0	192	13	134	8	593	98	4	689	27
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)				6.0	2.0			2.0			2.0	
Lane Util. Factor				1.00	1.00			0.95			0.95	
Frpb, ped/bikes				1.00	0.86			0.93			0.98	
Flpb, ped/bikes				0.96	1.00			1.00			1.00	
Frt				1.00	0.86			0.98			0.99	
Flt Protected				0.95	1.00			1.00			1.00	
Satd. Flow (prot)				1530	1220			2948			3225	
Flt Permitted				0.95	1.00			0.95			0.95	
Satd. Flow (perm)				1530	1220			2797			3074	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	0	0	0	192	13	134	8	593	98	4	689	27
RTOR Reduction (vph)	0	0	0	0	103	0	0	14	0	0	3	0
Lane Group Flow (vph)	0	0	0	192	44	0	0	685	0	0	717	0
Confl. Peds. (#/hr)	14		15	15		14	25		34	34		25
Confl. Bikes (#/hr)			3			2			7			20
Heavy Vehicles (%)	2%	2%	2%	6%	15%	8%	2%	3%	10%	100%	2%	2%
Turn Type				custom	NA		custom	NA		custom	NA	
Protected Phases					7 8			12			56	
Permitted Phases				8			2			6		
Actuated Green, G (s)				18.0	23.0			53.8			53.8	
Effective Green, g (s)				18.0	23.0			53.8			53.8	
Actuated g/C Ratio				0.18	0.23			0.54			0.54	
Clearance Time (s)				6.0								
Vehicle Extension (s)				3.0								
Lane Grp Cap (vph)				275	280			1504			1653	
v/s Ratio Prot					c0.04							
v/s Ratio Perm				c0.13				c0.24			0.23	
v/c Ratio				0.70	0.16			0.46			0.43	
Uniform Delay, d1				38.5	30.8			14.1			13.9	
Progression Factor				1.00	1.00			1.00			1.00	
Incremental Delay, d2				13.7	1.2			1.0			0.8	
Delay (s)				52.2	31.9			15.1			14.8	
Level of Service				D	С			В			В	
Approach Delay (s)		0.0			43.4			15.1			14.8	
Approach LOS		Α			D			В			В	
Intersection Summary												
HCM 2000 Control Delay			20.4	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	y ratio		0.48									
Actuated Cycle Length (s)			100.0	S	um of lost	time (s)			22.2			
Intersection Capacity Utilization	n		46.7%	IC	CU Level o	of Service	9		Α			_
Analysis Period (min)			15									
c Critical Lane Group												

Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2025PM Peak Hour 15 Oblates

	•	†	ļ	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥	†	^	
Traffic Volume (vph)	25	657	869	
Future Volume (vph)	25	657	869	
Lane Group Flow (vph)	32	657	869	
Turn Type	Perm	NA	NA	
Protected Phases	. 01111	2	6	7
Permitted Phases	8	_		
Detector Phase	8	2	6	
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	59.0	59.0	5.0
Total Split (%)	28.9%	65.6%	65.6%	6%
Maximum Green (s)	20.9 %	53.9	53.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
	0.0	0.0	0.0	0.0
Lost Time Adjust (s) Total Lost Time (s)	5.4	5.1	0.0 5.1	
		5.1	5.1	Look
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes	2.0	2.0	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	8	28		8
Act Effct Green (s)	20.6	53.9	53.9	
Actuated g/C Ratio	0.23	0.60	0.60	
v/c Ratio	0.09	0.64	0.44	
Control Delay	23.7	9.1	10.7	
Queue Delay	0.0	0.1	1.8	
Total Delay	23.7	9.2	12.5	
LOS	С	Α	В	
Approach Delay	23.7	9.2	12.5	
Approach LOS	С	Α	В	
Queue Length 50th (m)	3.4	25.8	39.5	
Queue Length 95th (m)	10.5	m37.0	52.2	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	372	1034	1966	
Starvation Cap Reductn	0.2	16	886	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.09	0.65	0.80	
	0.00	0.00	0.00	
Intersection Summary				

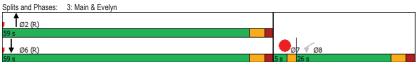
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 54 (60%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

02-06-2023 JK **CGH Transportation** Page 8 Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2025PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.64 Intersection Signal Delay: 11.3 Intersection Capacity Utilization 61.9% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service B m Volume for 95th percentile queue is metered by upstream signal.



Movement WBL WBR NBT NBR SBL SBT Lane Configurations Y ↑
Traffic Volume (vph) 25 7 657 0 0 869 Future Volume (vph) 25 7 657 0 0 869 Ideal Flow (vphpl) 1800 1800 1800 1800 1800 Total Lost time (s) 5.4 5.1 5.1 Lane Util. Factor 1.00 1.00 0.95 Frpb, ped/bikes 0.99 1.00 1.00 Flpb, ped/bikes 0.99 1.00 1.00 Firt 0.97 1.00 1.00 Satd. Flow (prot) 1605 1728 3283 Flt Permitted 0.96 1.00 1.00 Satd. Flow (perm) 1605 1728 3283 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 Adj. Flow (vph) 25 7 657 0 0 869
Traffic Volume (vph) 25 7 657 0 0 869 Future Volume (vph) 25 7 657 0 0 869 Ideal Flow (vphpl) 1800 1800 1800 1800 1800 Total Lost time (s) 5.4 5.1 5.1 5.1 Lane Util. Factor 1.00 1.00 0.95 Frpb, ped/bikes 0.99 1.00 1.00 Flpb, ped/bikes 0.99 1.00 1.00 Frt 0.97 1.00 1.00 Flt Protected 0.96 1.00 1.00 Satd. Flow (prot) 1605 1728 3283 Flt Permitted 0.96 1.00 1.00 Satd. Flow (perm) 1605 1728 3283 Peak-hour factor, PHF 1.00 1.00 1.00 Adj. Flow (vph) 25 7 657 0 869
deal Flow (vphpl) 1800 1800 1800 1800 1800 1800 Total Lost time (s) 5.4 5.1
Total Lost time (s) 5.4 5.1 5.1 ane Util. Factor 1.00 1.00 0.95 ripb, ped/bikes 0.99 1.00 1.00 lipb, ped/bikes 0.99 1.00 1.00 rit 0.97 1.00 1.00 lit Protected 0.96 1.00 1.00 satd. Flow (prot) 1605 1728 3283 lit Permitted 0.96 1.00 1.00 satd. Flow (perm) 1605 1728 3283 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 dd; Flow (vph) 25 7 657 0 0 869
ane Util. Factor 1.00 1.00 0.95 irpb, ped/bikes 0.99 1.00 1.00 irpb, ped/bikes 0.99 1.00 1.00 irt 0.97 1.00 1.00 irt 0.97 1.00 1.00 irt 0.96 1.00 1.00 satd. Flow (prot) 1605 1728 3283 itt Permitted 0.96 1.00 1.00 satd. Flow (prom) 1605 1728 3283 ive satd. Flow (prom) 1.00 1.00 1.00 1.00 1.00 1.00 ive satd. Flow (prom) 1.00 1.00 1.00 1.00 1.00 1.00 ive satd. Flow (prom) 1.00 1.00 1.00 1.00 1.00 1.00 ive satd. Flow (prom) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 ive satd. Flow (prom) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0
ripb, ped/bikes 0.99 1.00 1.00 in ped/bikes in 0.97 1.00 1.00 in ped/bikes in 0.96 1.00 1.00 in ped/bikes in ped/bik
pb, ped/bikes
pb, ped/bikes
It Protected 0.96 1.00 1.00 atd. Flow (prot) 1605 1728 3283 It Permitted 0.96 1.00 1.00 atd. Flow (perm) 1605 1728 3283 eak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 dj. Flow (vph) 25 7 657 0 869
latd. Flow (prot) 1605 1728 3283 1t Permitted 0.96 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0
It Permitted 0.96 1.00 1.00 latd. Flow (perm) 1605 1728 3283 leak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 dj. Flow (vph) 25 7 657 0 0 869
Satd: Flow (perm) 1605 1728 3283 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 vdj. Flow (vph) 25 7 657 0 0 869
Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 dj. Flow (vph) 25 7 657 0 0 869
dj. Flow (vph) 25 7 657 0 0 869
, , , , , , , , , , , , , , , , , , , ,
)TOP Deduction (ash)
TOR Reduction (vph) 5 0 0 0 0
ane Group Flow (vph) 27 0 657 0 0 869
Confl. Peds. (#/hr) 4 8 28 28
Confl. Bikes (#/hr) 1
leavy Vehicles (%) 2% 2% 3% 2% 3%
urn Type Perm NA NA
rotected Phases 2 6
ermitted Phases 8
ctuated Green, G (s) 20.6 53.9 53.9
ffective Green, g (s) 20.6 53.9 53.9
ctuated g/C Ratio 0.23 0.60 0.60
Clearance Time (s) 5.4 5.1 5.1
/ehicle Extension (s) 3.0 3.0 3.0
ane Grp Cap (vph) 367 1034 1966
/s Ratio Prot c0.38 0.26
/s Ratio Perm c0.02
/c Ratio 0.07 0.64 0.44
Iniform Delay, d1 27.2 11.7 9.8
Progression Factor 1.00 0.59 1.00
ncremental Delay, d2 0.4 1.9 0.7
Delay (s) 27.6 8.8 10.6
evel of Service C A B
Approach Delay (s) 27.6 8.8 10.6
pproach LOS C A B
ntersection Summary
HCM 2000 Control Delay 10.2 HCM 2000 Level of Service B
HCM 2000 Volume to Capacity ratio 0.46
ctuated Cycle Length (s) 90.0 Sum of lost time (s) 12.5
ntersection Capacity Utilization 61.9% ICU Level of Service B
Analysis Period (min) 15
Critical Lane Group

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Øī
Lane Configurations		4	ሻ	î»	7	ĵ»	ሻ	ĵ»				
Traffic Volume (vph)	4	0	43	0	1	614	59	849				
Future Volume (vph)	4	0	43	0	1	614	59	849				
Lane Group Flow (vph)	0	15	43	50	1	664	59	857				
Turn Type	Perm	NA	Perm	NA	custom		custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	3.0	33.0	3.0
Total Split (s)	22.0	22.0	22.0	22.0	15.0		15.0		43.0	5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		48%	6%	48%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	10.0		10.0		38.0	3.0	38.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0		20.0	
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0		8.0	
Pedestrian Calls (#/hr)	6	6	5	5					30		27	
Act Effct Green (s)		16.7	16.7	16.7	48.8	43.4	56.5	50.9				
Actuated g/C Ratio		0.19	0.19	0.19	0.54	0.48	0.63	0.57				
v/c Ratio		0.04	0.24	0.09	0.00	0.81	0.17	0.88				
Control Delay		0.2	35.2	0.3	10.0	24.0	6.5	27.4				
Queue Delay		0.0	0.0	0.0	0.0	0.1	0.0	0.0				
Total Delay		0.2	35.2	0.3	10.0	24.1	6.5	27.4				
LOS		Α	D	Α	Α	С	Α	С				
Approach Delay		0.2	_	16.5		24.1		26.0				
Approach LOS		Α		В		C		С				
Queue Length 50th (m)		0.0	6.4	0.0	0.1	41.6	2.9	53.7				
Queue Length 95th (m)		0.0	15.8	0.0	m0.1	#161.1	6.3	#234.7				
Internal Link Dist (m)		109.1	10.0	138.0		118.6	0.0	47.0				
Turn Bay Length (m)		100.1	15.0	100.0	15.0	110.0	30.0					
Base Capacity (vph)		375	181	569	320	823	392	975				
Starvation Cap Reductn		0	0	0	0_0	6	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.04	0.24	0.09	0.00	0.81	0.15	0.88				
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 62 (69%), Reference	ed to phase	2·NRTI	and 6:SB	TI Start	of Green							
Natural Cycle: 90				,								

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Natural Cycle: 90

JK

CGH Transportation Page 12

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2025PM Peak Hour 15 Oblates

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases	_	
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)		
Lead/Lag	Lag	Lag
	Yes	Yes
Lead-Lag Optimize? Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	
Pedestrian Calls (#/hr)	30	27
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

02-06-2023 JK CGH Transportation Page 13 Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2025PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.88

Intersection Signal Delay: 24.5 Intersection Capacity Utilization 72.0% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service C

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

M Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Main & Immaculata HS/Oblats



Lane Configurations 1		•	\rightarrow	*	1	+	•	1	1		-	Į.	4
Traffic Volume (vph)	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (vph) 4	Lane Configurations		4		ሻ	ĥ		7	- 1>		ሻ	1>	
Ideal Flow (vphpl)	Traffic Volume (vph)	4	0	11	43	0	50	1	614	50	59	849	8
Total Lost time (s)	Future Volume (vph)	4	0	11	43	0	50	1	614	50	59	849	8
Lane Uil. Factor 1.00	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Frpb, ped/bikes	Total Lost time (s)		5.3		5.3	5.3		5.0	5.0		5.0	5.0	
Fipb, ped/bikes	Lane Util. Factor		1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Fit Protected 0.99 0.95 1.00 0.85 1.00 0.99 1.00 1.00 Fit Protected 0.99 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 Fit Protected 0.99 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1501 1243 1292 1666 1700 1653 1725 Fit Permitted 0.95 0.75 1.00 0.17 1.00 0.23 1.00 Satd. Flow (perm) 1440 979 1292 290 1700 403 1725 Fit Permitted 0.95 0.75 1.00 0.17 1.00 1.00 1.00 1.00 1.00 1.00	Frpb, ped/bikes		0.97		1.00	0.97		1.00	0.99		1.00	1.00	
Fit Protected 0.99 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1501 1243 1292 1656 1770 1653 1725 Fit Permitted 0.95 0.75 1.00 0.17 1.00 0.23 1.00 Satd. Flow (perm) 1440 979 1292 290 1700 403 1725 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Flpb, ped/bikes		1.00		0.98	1.00		1.00	1.00		1.00	1.00	
Satd. Flow (prot)	Frt		0.90		1.00	0.85		1.00	0.99		1.00	1.00	
Fit Permitted	Flt Protected		0.99		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (perm)	Satd. Flow (prot)		1501		1243	1292		1656	1700		1653	1725	
Peak-hour factor, PHF	Flt Permitted		0.95		0.75	1.00		0.17	1.00		0.23	1.00	
Adj. Flow (vph)	Satd. Flow (perm)		1440		979	1292		290	1700		403	1725	
RTOR Reduction (vph)	Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
RTOR Reduction (vph)	Adi, Flow (vph)	4	0	11	43	0	50	1	614	50	59	849	8
Lane Group Flow (vph) 0 3 0 43 9 0 1 661 0 59 857 0 Confl. Peds. (#/hr) 2 7 7 2 34 36 36 36 36 36 36 36 Confl. Bikes (#/hr) 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		0	12	0	0	41	0	0	3	0	0	0	0
Confl. Peds. (#/hr) 2 7 7 7 2 34 36 36 36 36 Confl. Bikes (#/hr) 4 Heavy Vehicles (%) 2% 2% 2% 33% 2% 14% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 2% 3% 2% 2% 2% 2% 3% 2% 2% 2% 2% 2% 3% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2%		0	3	0	43	9	0	1	661	0	59	857	0
Confi. Bikes (#/hr) Heavy Vehicles (%) 2% 2% 2% 33% 2% 14% 2% 3% 2% 2% 3% 2% 2% 3% 29 Turn Type Perm NA Perm NA custom NA custom NA Protected Phases 4 8 5 2 9 1 6 613 Permitted Phases 4 8 2 2 6 6 Actuated Green, G (s) 16.7 16.7 16.7 43.5 47.4 52.5 51.9 Effective Green, g (s) 16.7 16.7 16.7 16.7 43.5 45.4 52.5 51.9 Actuated g/C Ratio 0.19 0.19 0.19 0.48 0.50 0.58 0.55 Clearance Time (s) 5.3 5.3 5.3 5.0 5.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 267 181 239 156 857 312 956 v/s Ratio Perm 0.00 c0.04 0.00 0.39 c0.01 c0.50 v/s Ratio Port 0.01 0.00 0.39 c0.01 c0.50 v/s Ratio Port 0.01 0.00 0.77 0.19 0.90 Uniform Delay, d1 29.9 31.2 30.1 14.7 18.1 10.7 17.8 Progression Factor 1.00 1.00 1.00 1.51 0.73 0.88 0.85 Incremental Delay, d2 0.1 3.1 0.3 0.0 5.7 0.3 11.9 Delay (s) 30.0 34.3 30.4 22.2 19.0 9.7 27.0 Level of Service C C C C C B A C C Approach Delay (s) 30.0 32.2 19.0 25.9 Approach LOS C B HCM 2000 Level of Service C C HCM 2000 Volume to Capacity ratio 72.0% ICU Level of Service C C Analysis Period (min) 15		2		7				34		36	36		34
Heavy Vehicles (%)										4			
Turn Type Perm NA Perm NA custom NA custom NA Protected Phases 4 8 5 2.9 1 613 Permitted Phases 4 8 2 6 Actuated Green, G (s) 16.7 16.7 16.7 43.5 47.4 52.5 51.9 Effective Green, g (s) 16.7 16.7 16.7 43.5 45.4 52.5 49.9 Actuated g/C Ratio 0.19 0.19 0.19 0.48 0.50 0.58 0.55 Clearance Time (s) 5.3 5.3 5.3 5.0 5.0 5.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 267 181 239 156 857 312 956 Vis Ratio Prot 0.01 0.00 0.39 0.01 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 0.50 <td></td> <td>2%</td> <td>2%</td> <td>2%</td> <td>33%</td> <td>2%</td> <td>14%</td> <td>2%</td> <td>3%</td> <td>2%</td> <td>2%</td> <td>3%</td> <td>2%</td>		2%	2%	2%	33%	2%	14%	2%	3%	2%	2%	3%	2%
Protected Phases													
Permitted Phases 4 8 2 6 6 Actuated Green, G (s) 16.7 16.7 16.7 43.5 47.4 52.5 51.9 Actuated Green, G (s) 16.7 16.7 16.7 43.5 45.4 52.5 51.9 Actuated gly C Ratio 0.19 0.19 0.19 0.48 0.50 0.58 0.55 Clearance Time (s) 5.3 5.3 5.3 5.0 5.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 4.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 5.0 5.0 Vehicle Extension (s) 5.0 0.0													
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Intersection Capacity Utilization 72.0% ICU Level of Service C Analysis Period (min) 15					S	um of lost	time (s)			19.3			
Analysis Period (min) 15		tion											
							22						

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Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ની	7	7	î,	ሻ	1>			
Traffic Volume (vph)	18	4	47	5	77	10	568	42	820			
Future Volume (vph)	18	4	47	5	77	10	568	42	820			
Lane Group Flow (vph)	0	29	0	52	77	10	601	42	854			
Turn Type	Perm	NA	Perm	NA	Perm	custom		custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	21.2	21.2	21.2	21.2	21.2	15.0		15.0		5.0	44.0	4.8
Total Split (%)	23.6%	23.6%	23.6%	23.6%	23.6%	16.7%		16.7%		6%	49%	5%
Maximum Green (s)	15.0	15.0	15.0	15.0	15.0	9.2		9.2		3.0	38.2	2.8
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	17	17	9	9	9					30	30	
Act Effct Green (s)		15.0		15.0	15.0	51.0	54.3	56.4	59.6			
Actuated g/C Ratio		0.17		0.17	0.17	0.57	0.60	0.63	0.66			
v/c Ratio		0.13		0.26	0.22	0.04	0.59	0.13	0.76			
Control Delay		27.9		36.6	1.5	7.0	15.5	4.0	7.3			
Queue Delay		0.0		0.0	0.0	0.0	0.1	0.0	0.2			
Total Delay		27.9		36.6	1.5	7.0	15.6	4.0	7.4			
LOS		С		D	Α	Α	В	Α	Α			
Approach Delay		27.9		15.6			15.4		7.3			
Approach LOS		С		В			В		Α			
Queue Length 50th (m)		3.3		7.9	0.0	0.6	66.9	1.2	10.7			
Queue Length 95th (m)		10.8		18.3	0.0	2.2	106.1		m#42.2			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		229		201	346	292	1015	357	1126			
Starvation Cap Reductn		0		0	0	0	0	0	22			
Spillback Cap Reductn		0		0	3	0	39	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.13		0.26	0.22	0.03	0.62	0.12	0.77			

Intersection Summary

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 80

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Future	Background	2025PM	Peak	Hour
			15	Oblates

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	44.0	4.8
Total Split (%)	6%	49%	5%
Maximum Green (s)	3.0	38.2	2.8
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	31	31	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			
y			

02-06-2023 JK CGH Transportation Page 17 Lanes, Volumes, Timings 6: Main & Hazel

Future Background 2025PM Peak Hour

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76

Intersection Signal Delay: 11.3 Intersection Capacity Utilization 72.5% Analysis Period (min) 15

Intersection LOS: B
ICU Level of Service C

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

M Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Main & Hazel †ø₁ ⁴†ø₂ (R) **↑** Ø13 Ø6 (R)

	۶	→	*	1	←	*	4	†	1	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			ર્ન	7	7	ĵ.		7	ĵ»	
Traffic Volume (vph)	18	4	7	47	5	77	10	568	33	42	820	34
Future Volume (vph)	18	4	7	47	5	77	10	568	33	42	820	34
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)		6.2			6.2	6.2	5.8	2.0		5.8	2.0	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.98			1.00	0.95	1.00	0.97		1.00	0.98	
Flpb, ped/bikes		0.98			0.96	1.00	1.00	1.00		1.00	1.00	
Frt		0.97			1.00	0.85	1.00	0.99		1.00	0.99	
Flt Protected		0.97			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1585			1596	1312	1658	1681		1398	1699	
Flt Permitted		0.82			0.73	1.00	0.15	1.00		0.28	1.00	
Satd. Flow (perm)		1340			1210	1312	254	1681		407	1699	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	18	4	7	47	5	77	10	568	33	42	820	34
RTOR Reduction (vph)	0	6	0	0	0	64	0	2	0	0	1	0
Lane Group Flow (vph)	0	23	0	0	52	13	10	599	0	42	853	0
Confl. Peds. (#/hr)	9		17	17		9	31		30	30		31
Confl. Bikes (#/hr)									1			
Heavy Vehicles (%)	2%	2%	2%	2%	2%	10%	2%	2%	9%	21%	2%	6%
Turn Type	Perm	NA		Perm	NA	Perm	custom	NA		custom	NA	
Protected Phases		4			8		13	12		9	56	
Permitted Phases	4			8		8	2			6		
Actuated Green, G (s)		15.0			15.0	15.0	44.4	48.2		50.4	51.2	
Effective Green, q (s)		15.0			15.0	15.0	44.4	48.2		50.4	51.2	
Actuated g/C Ratio		0.17			0.17	0.17	0.49	0.54		0.56	0.57	
Clearance Time (s)		6.2			6.2	6.2	5.8			5.8		
Vehicle Extension (s)		3.0			3.0	3.0	3.0			3.0		
Lane Grp Cap (vph)		223			201	218	144	900		274	966	
v/s Ratio Prot							0.00	0.36		c0.01	c0.50	
v/s Ratio Perm		0.02			c0.04	0.01	0.03			0.08		
v/c Ratio		0.10			0.26	0.06	0.07	0.67		0.15	0.88	
Uniform Delay, d1		31.8			32.7	31.6	15.2	15.1		10.7	16.8	
Progression Factor		1.00			1.00	1.00	1.00	1.00		0.58	0.25	
Incremental Delay, d2		0.9			3.1	0.5	0.2	3.9		0.1	6.5	
Delay (s)		32.7			35.8	32.1	15.4	19.0		6.3	10.7	
Level of Service		C			D	C	В	В		A	В	
Approach Delay (s)		32.7			33.6	ŭ		18.9			10.5	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			15.7	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	y ratio		0.73									
Actuated Cycle Length (s)			90.0	S	um of lost	time (s)			21.8			
Intersection Capacity Utilizatio	n		72.5%	IC	U Level	of Servic	е		С			
Analysis Period (min)			15									
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 CGH Transportation

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Appendix G

Synchro Intersection Worksheets – 2030 Future Background Conditions



	-	•	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		414						
Traffic Volume (vph)	12	243	293	531	5	404						
Future Volume (vph)	12	243	293	531	5	404						
Lane Group Flow (vph)	323	243	0	830	0	523						
Turn Type	NA	pm+ov			custom	NA						
Protected Phases	4	13		12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	28				20		53	53		20	53	53
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.80	0.33		0.63		0.49						
Control Delay	52.7	3.9		9.8		24.2						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	52.7	3.9		9.9		24.2						
LOS	D	Α		Α		С						
Approach Delay	31.7			9.9		24.2						
Approach LOS	С			Α		С						
Queue Length 50th (m)	56.6	0.0		28.8		37.2						
Queue Length 95th (m)	#131.3	14.6		35.1		52.5						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	403	733		1317		1059						
Starvation Cap Reductn	0	0		44		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.80	0.33		0.65		0.49						
Intersection Summary												

	Summai	

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 60 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 85

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CGH Transportation
Page 1

Lane Group	Ø11
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	12%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	3.0
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	3
Act Effct Green (s)	3
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Lanes, Volumes, Timings 1: Main & Hawthorne

Future Background 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 20.3
Intersection Capacity Utilization 78.3%
Analysis Period (min) 15 Intersection LOS: C
ICU Level of Service D # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne **♦** Ø13

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Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2030AM Peak Hour

	€	+	1	1	-	ļ					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĵ»		414		414					
Traffic Volume (vph)	323	53	7	664	24	534					
Future Volume (vph)	323	53	7	664	24	534					
Lane Group Flow (vph)	323	345	0	741	0	638					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	60		123		33		123	33	60	7	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	1.37	0.71		0.45		0.40					
Control Delay	226.9	21.5		13.2		6.5					
Queue Delay	0.0	0.0		0.7		0.3					
Total Delay	226.9	21.5		13.9		6.8					
LOS	F	С		В		Α					
Approach Delay		120.8		13.9		6.8					
Approach LOS		F		В		Α					
Queue Length 50th (m)	~83.2	21.8		30.7		14.1					
Queue Length 95th (m)	#134.7	55.8		51.1		22.3					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	235	484		1630		1589					
Starvation Cap Reductn	0	0		525		365					
Spillback Cap Reductn	0	0		8		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	1.37	0.71		0.67		0.52					

Intersection Summary

Cycle Length: 100

Offset: 39 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

JK

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.37 Intersection LOS: D
ICU Level of Service C Intersection Signal Delay: 46.6 Intersection Capacity Utilization 67.5% Analysis Period (min) 15 Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Main & Graham/Lees



02-07-2023 CGH Transportation Page 5 Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2030AM Peak Hour 15 Oblates

	•	†	↓	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	N/		44	
Traffic Volume (vph)	44	686	710	
Future Volume (vph)	44	686	710	
Lane Group Flow (vph)	92	686	710	
Turn Type	Perm	NA	NA	
Protected Phases		2	6	7
Permitted Phases	8			
Detector Phase	8	2	6	
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	69.0	69.0	5.0
Total Split (%)	26.0%	69.0%	69.0%	5%
Maximum Green (s)	20.6	63.9	63.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag	0.1	0.1	Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0	U-IVIAX	3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	18	41		18
Act Effct Green (s)	20.6	63.9	63.9	10
Actuated g/C Ratio	0.21	0.64	0.64	
v/c Ratio	0.21	0.62	0.04	
	-	13.9		
Control Delay	20.1	13.9	13.5	
Queue Delay	0.0		1.1	
Total Delay	20.1	15.1	14.6	
LOS	C	В	В	
Approach Delay	20.1	15.1	14.6	
Approach LOS	С	В	В	
Queue Length 50th (m)	7.0	72.4	37.1	
Queue Length 95th (m)	20.3	106.6	m40.9	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	347	1104	2097	
Starvation Cap Reductn	0	216	1073	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.27	0.77	0.69	
Intersection Summary				
Cycle Length: 100				

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 59 (59%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.62 Intersection LOS: B
ICU Level of Service B Intersection Signal Delay: 15.2 Intersection Capacity Utilization 63.5% Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn



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Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2030AM Peak Hour

Lane Group	Ø7 7
Traffic Volume (vph)	7
Traffic Volume (vph) 11 1 37 0 29 670 22 721 Future Volume (vph) 11 1 37 0 29 670 22 721 Lane Group Flow (vph) 0 29 37 40 29 690 22 732 Turn Type Perm NA Perm NA custom NA custom NA Protected Phases 4 8 5 29 1 613 2 3 6 Permitted Phases 4 4 8 8 5 29 1 613 2 3 6 Permitted Phases 4 4 8 8 5 29 1 613 3 6 Permitted Phases 4 4 8 8 5 29 1 613 3 6 0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0 10.0	7
Lane Group Flow (vph)	7
Turn Type	7
Protected Phases	7
Permitted Phases	7
Detector Phase 4	- 1
Switch Phase Swit	
Minimum Initial (s) 10.0 33.0 5.0 33.0 5.0 33.0 5.0 33.0 5.0 33.0 5.0 30.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 5.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 7.0 42.0 42.0 47	
Minimum Split (s) 19.3 19.3 19.3 19.3 19.3 10.0 10.0 33.0 5.0 33.0 Total Split (s) 22.0 22.0 22.0 16.0 16.0 42.0 5.0 42.0 Total Split (w) 24.4% 24.4% 24.4% 17.8% 17.8% 47% 6% 47% Maximum Green (s) 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 2.0 2.0 2.0 2.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 1.7 1.7 0.0 1.7 Lead-Lag Lag La	
Total Split (s) 22.0 22.0 22.0 22.0 22.0 16.0 42.0 5.0 42.0 Total Split (%) 24.4% 24.4% 24.4% 17.8% 17.8% 47% 6% 47% Maximum Green (s) 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 3.3 All-Red Time (s) 2.0 2.0 2.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 5.3 5.3 5.3 5.0	1.0
Total Split (%)	5.0
Maximum Green (s) 16.7 16.7 16.7 16.7 11.0 11.0 37.0 3.0 37.0 Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 2.0 3.3 3.3 3.3 3.3 2.0 2.0 3.3 3.3 3.3 2.0 3.3 3.3 3.3 3.3 2.0 3.3 3.3 3.3 3.3 3.3 2.0 2.0 3.7 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 0.0 1.7 0.0 1.7 1.7 1.7 1.7 0.0 0.0 1.7 0.0 1.7 0.0 0.0 0.0	5.0
Yellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 2.0 3.3 All-Red Time (s) 2.0 2.0 2.0 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 0.0 1.7 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	6%
All-Red Time (s) 2.0 2.0 2.0 2.0 1.7 1.7 1.7 0.0 1.7 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.3 5.3 5.3 5.3 5.0 5.0 Lead/Lag Lag Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Yes Yes Yes Optimize? Yes	3.0
Lost Time Adjust (s) 0.0	2.0
Total Lost Time (s)	0.0
Lead/Lag Lag Lag Lag Lag Lag Lead	
Lead-Lag Optimize? Yes	
Vehicle Extension (s) 3.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 3.0 8.0 9.0 8.0 9.0 3.0 8.0 9.0 3.0 3.0 3.0 3.0 3.0	Lead
Recall Mode Max Max Max Max None None C-Max Max C-Max	Yes
Walk Time (s) 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 3.0 20.0 8.0 20.0 8.0 20.0 8.0 0.0 8.0 8.0 0.0 8.0 8.0 20.0 8.0 8.0 0.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 9.0 8.0 9.0 8.0 9.0	3.0
Flash Dont Walk (s) 12.0 12.0 12.0 12.0 12.0 8.0 8.0 8.0 Pedestrian Calls (#/hr) 7 7 7 2 2 2 2 3 36 7 34 Act Effet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 0.60 0.54 0/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	Max
Pedestrian Calls (#/hr) 7 7 2 2 36 7 34 Act Effet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 Vic Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	3.0
Act Leffet Green (s) 16.7 16.7 16.7 54.1 48.4 53.9 48.3 Actuated g/C Ratio 0.19 0.19 0.60 0.54 0.60 0.54 V/C Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	0.0
Actuated g/C Ratio 0.19 0.19 0.19 0.60 0.54 0.60 0.54 v/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	2
V/c Ratio 0.11 0.23 0.07 0.09 0.75 0.06 0.79 Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	
Control Delay 19.7 35.6 0.2 3.8 12.4 6.7 26.7	
O	
Queue Delay 0.0 0.0 0.0 0.0 0.3 0.0 0.0	
Total Delay 19.7 35.6 0.2 3.8 12.6 6.7 26.7	
LOS B D A A B A C	
Approach Delay 19.7 17.2 12.3 26.1	
Approach LOS B B B C	
Queue Length 50th (m) 1.7 5.5 0.0 0.6 15.0 1.2 83.2	
Queue Length 95th (m) 8.9 14.3 0.0 m1.4 #168.2 3.7 #189.3	
Internal Link Dist (m) 109.1 138.0 118.6 47.0	
Turn Bay Length (m) 15.0 15.0 30.0	
Base Capacity (vph) 271 159 556 394 917 428 926	
Starvation Cap Reductn 0 0 0 0 24 0 0	
Spillback Cap Reductn 0 0 0 0 0 0 0	
Storage Cap Reductn 0 0 0 0 0 0	
Reduced v/c Ratio 0.11 0.23 0.07 0.07 0.77 0.05 0.79	

Intersection Summary

Cycle Length: 90

Cycle Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 80

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Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2030AM Peak Hour
15 Oblates

Lanes, Volumes,	Timings
5: Main & Immac	ulata HS/Oblats

Future Background 2030AM Peak Hour
15 Oblates

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.79

Intersection LOS: B

Intersection Capacity Utilization 61.1%

ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases	-	
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)		
Yellow Time (s)	3.0 2.0	3.0 2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)	_	
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	36	34
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		
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6: Main & Hazel											15	Oblates
	۶	→	€	←	•	1	†	-	↓			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		44		ર્ન	7		ĵ,	7	ĥ			
Traffic Volume (vph)	32	3	16	2	28	9	663	62	701			
Future Volume (vph)	32	3	16	2	28	9	663	62	701			
Lane Group Flow (vph)	0	44	0	18	28	9	689	62	722			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	27	27	9	9	9					30	30	
Act Effct Green (s)		15.8		15.8	15.8	47.3	50.5	55.5	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.20		0.09	0.09	0.03	0.73	0.23	0.65			
Control Delay		29.2		32.7	0.5	7.2	21.5	11.4	14.1			
Queue Delay		0.0		0.0	0.0	0.0	0.4	0.0	0.1			
Total Delay		29.2		32.7	0.5	7.2	21.9	11.4	14.2			
LOS		С		С	Α	Α	С	В	В			
Approach Delay		29.2		13.1			21.7		14.0			
Approach LOS		С		В			С		В			
Queue Length 50th (m)		5.2		2.7	0.0	0.5	87.4	3.0	35.0			
Queue Length 95th (m)		14.5		8.5	0.0	2.1	139.1	m8.3	94.4			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		223		192	322	364	949	300	1111			
Starvation Cap Reductn		0		0	0	0	0	0	37			
Spillback Cap Reductn		0		0	4	0	44	0	0			

Reduced v/c Ratio Intersection Summary

Storage Cap Reductn

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 46 (51%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

0.20

0.09

0.09 0.02

Natural Cycle: 75

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	43.0	5.0
Total Split (%)	6%	48%	6%
Maximum Green (s)	3.0	37.2	3.0
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	41	41	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

Lanes, Volumes, Timings 6: Main & Hazel

02-07-2023 JK CGH Transportation Page 13

0.76

0.67 0.21

Lanes, Volumes, Timings 6: Main & Hazel

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 17.8
Intersection Capacity Utilization 77.3%
Analysis Period (min) 15 Future Background 2030AM Peak Hour 15 Oblates

Intersection LOS: B
ICU Level of Service D

Splits and Phases: 6: Main & Hazel

m Volume for 95th percentile queue is metered by upstream signal.



02-07-2023 JK CGH Transportation Page 14 Lanes, Volumes, Timings 1: Main & Hawthorne

Future Background 2030PM Peak Hour 15 Oblates

	-	•	1	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ની	7		414		413						
Traffic Volume (vph)	70	276	270	472	8	466						
Future Volume (vph)	70	276	270	472	8	466						
Lane Group Flow (vph)	360	276	0	755	0	669						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	15.0
Total Split (s)	24.0	20.0	20.0		54.0		5.0	54.0	5.0	5.0	5.0	15.0
Total Split (%)	20.0%	16.7%	16.7%		45.0%		4%	45%	4%	4%	4%	13%
Maximum Green (s)	17.7	13.7	13.7		47.7		3.0	47.7	3.0	3.0	3.0	8.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	None	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	6.7
Pedestrian Calls (#/hr)	27				18		50	50		18	50	50
Act Effct Green (s)	27.3	41.0		75.0		57.0						
Actuated g/C Ratio	0.23	0.34		0.62		0.48						
v/c Ratio	0.98	0.42		0.57		0.51						
Control Delay	89.3	5.4		11.9		20.9						
Queue Delay	0.0	0.0		0.8		0.0						
Total Delay	89.3	5.4		12.7		20.9						
LOS	F	Α		В		С						
Approach Delay	52.9			12.7		20.9						
Approach LOS	D			В		С						
Queue Length 50th (m)	82.5	0.0		40.5		49.8						
Queue Length 95th (m)	#175.9	18.7		51.5		66.7						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	367	654		1329		1307						
Starvation Cap Reductn	0	0		278		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.98	0.42		0.72		0.51						

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 26 (22%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

JK

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.98 Intersection Signal Delay: 27.8 Intersection Capacity Utilization 82.9% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service E # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.



Lane Group	Ø11
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	10%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	1
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

02-07-2023 JK CGH Transportation Page 2

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2030PM Peak Hour 15 Oblates

	•	-	1	†	-	+					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĵ.		413		413					
Traffic Volume (vph)	194	13	8	593	4	689					
Future Volume (vph)	194	13	8	593	4	689					
Lane Group Flow (vph)	194	148	0	730	0	720					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	15		34		25		34	25	15	3	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	0.71	0.35		0.46		0.40					
Control Delay	53.8	8.9		12.3		12.2					
Queue Delay	0.0	0.0		1.7		3.6					
Total Delay	53.8	8.9		14.0		15.8					
LOS	D	Α		В		В					
Approach Delay		34.4		14.0		15.8					
Approach LOS		С		В		В					
Queue Length 50th (m)	35.7	1.9		37.2		37.2					
Queue Length 95th (m)	#65.0	16.6		50.5		49.4					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	275	427		1594		1785					
Starvation Cap Reductn	0	0		656		950					
Spillback Cap Reductn	0	0		0		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	0.71	0.35		0.78		0.86					

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 35 (35%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

02-07-2023 JK CGH Transportation Page 4 Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Background 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.71

Intersection Signal Delay: 18.6 Intersection Capacity Utilization 48.0% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service A

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2030PM Peak Hour 15 Oblates

	•	†	ļ	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥	<u> </u>	A A	~ '
Traffic Volume (vph)	25	657	869	
Future Volume (vph)	25	657	869	
Lane Group Flow (vph)	32	657	869	
Turn Type	Perm	NA.	NA	
Protected Phases		2	6	7
Permitted Phases	8	_	-	•
Detector Phase	8	2	6	
Switch Phase		_		
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	59.0	59.0	5.0
Total Split (%)	28.9%	65.6%	65.6%	6%
Maximum Green (s)	20.6	53.9	53.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag	0.1	0.1	Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0	O Wax	3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	8	28		8
Act Effct Green (s)	20.6	53.9	53.9	Ŭ
Actuated g/C Ratio	0.23	0.60	0.60	
v/c Ratio	0.09	0.64	0.44	
Control Delay	23.7	9.1	10.7	
Queue Delay	0.0	0.1	1.8	
Total Delay	23.7	9.2	12.5	
LOS	23.7 C	3.2 A	12.3 B	
Approach Delay	23.7	9.2	12.5	
Approach LOS	23.7 C	9.2 A	12.3 B	
Queue Length 50th (m)	3.4	25.8	39.5	
Queue Length 95th (m)	10.5	m37.0	52.2	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)	402.4	00.0	05.4	
Base Capacity (vph)	372	1034	1966	
Starvation Cap Reductn	0	1034	886	
Spillback Cap Reductn	0	0	088	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.09	0.65	0.80	
Reduced V/C Ratio	0.09	0.05	0.80	
Intersection Summary				

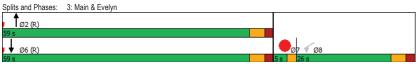
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 54 (60%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

02-07-2023 JK **CGH Transportation** Page 6 Lanes, Volumes, Timings 3: Main & Evelyn

Future Background 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.64 Intersection Signal Delay: 11.3 Intersection Capacity Utilization 61.9% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service B m Volume for 95th percentile queue is metered by upstream signal.



	•	-	1	+	1	†	-	↓				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	7	ĵ.,	76	f,	*	₽				
Traffic Volume (vph)	4	0	43	0	1	614	59	849				
Future Volume (vph)	4	0	43	0	1	614	59	849				
Lane Group Flow (vph)	0	15	43	50	1	664	59	857				
Turn Type	Perm	NA	Perm	NA	custom	NA	custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	3.0	33.0	3.0
Total Split (s)	22.0	22.0	22.0	22.0	15.0		15.0		43.0	5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		48%	6%	48%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	10.0		10.0		38.0	3.0	38.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0		20.0	
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0		8.0	
Pedestrian Calls (#/hr)	6	6	5	5					30		27	
Act Effct Green (s)		16.7	16.7	16.7	48.8	43.4	56.5	50.9				
Actuated g/C Ratio		0.19	0.19	0.19	0.54	0.48	0.63	0.57				
v/c Ratio		0.04	0.24	0.09	0.00	0.81	0.17	0.88				
Control Delay		0.2	35.2	0.3	10.0	24.0	6.5	27.4				
Queue Delay		0.0	0.0	0.0	0.0	0.1	0.0	0.0				
Total Delay		0.2	35.2	0.3	10.0	24.1	6.5	27.4				
LOS		A	D	A	A	C	A	C				
Approach Delay		0.2		16.5		24.1		26.0				
Approach LOS		A		В		C		C				
Queue Length 50th (m)		0.0	6.4	0.0	0.1	41.6	2.9	53.7				
Queue Length 95th (m)		0.0	15.8	0.0	m0.1	#161.1	6.3	#234.7				
Internal Link Dist (m)		109.1	10.0	138.0	1110.1	118.6	0.0	47.0				
Turn Bay Length (m)		100.1	15.0	100.0	15.0	110.0	30.0	11.0				
Base Capacity (vph)		375	181	569	320	823	392	975				
Starvation Cap Reductn		0	0	0	0_0	6	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.04	0.24	0.09	0.00	0.81	0.15	0.88				
Intersection Summary												

n	ters	ecti	on	Sun	nma	ary

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 62 (69%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 90

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		.0
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
	5.0 6%	6%
Total Split (%)		3.0
Maximum Green (s)	3.0	
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	30	27
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		_
outlining		

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Background 2030PM Peak Hour
15 Oblates

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 24.5

Intersection LOS: C

Intersection Capacity Utilization 72.0%

Analysis Period (min) 15

95th percentile volume exceeds capacity queue may be longer.

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Main & Immaculata HS/Oblats



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Lanes, Volumes, Timings 6: Main & Hazel

Future Background 2030PM Peak Hour
15 Oblates

	<i>></i>	→	•	-	*	4	†	-	ļ			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		4	7	7	f)	7	1>			
Traffic Volume (vph)	18	4	47	5	77	10	568	42	820			
Future Volume (vph)	18	4	47	5	77	10	568	42	820			
Lane Group Flow (vph)	0	29	0	52	77	10	601	42	854			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	21.2	21.2	21.2	21.2	21.2	15.0		15.0		5.0	44.0	4.8
Total Split (%)	23.6%	23.6%	23.6%	23.6%	23.6%	16.7%		16.7%		6%	49%	5%
Maximum Green (s)	15.0	15.0	15.0	15.0	15.0	9.2		9.2		3.0	38.2	2.8
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	17	17	9	9	9					30	30	
Act Effct Green (s)		15.0		15.0	15.0	51.0	54.3	56.4	59.6			
Actuated g/C Ratio		0.17		0.17	0.17	0.57	0.60	0.63	0.66			
v/c Ratio		0.13		0.26	0.22	0.04	0.59	0.13	0.76			
Control Delay		27.9		36.6	1.5	7.0	15.5	4.0	7.3			
Queue Delay		0.0		0.0	0.0	0.0	0.1	0.0	0.2			
Total Delay		27.9		36.6	1.5	7.0	15.6	4.0	7.4			
LOS		С		D	Α	Α	В	Α	Α			
Approach Delay		27.9		15.6			15.4		7.3			
Approach LOS		С		В			В		Α			
Queue Length 50th (m)		3.3		7.9	0.0	0.6	66.9	1.2	10.7			
Queue Length 95th (m)		10.8		18.3	0.0	2.2	106.1	m1.5	m#42.2			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		229		201	346	292	1015	357	1126			
Starvation Cap Reductn		0		0	0	0	0	0	22			
Spillback Cap Reductn		0		0	3	0	39	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.13		0.26	0.22	0.03	0.62	0.12	0.77			

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated						
Maximum v/c Ratio: 0.76						
Intersection Signal Delay: 11.3	Intersection LOS: B					
Intersection Capacity Utilization 72.5%	ICU Level of Service C					
Analysis Period (min) 15						
# 95th percentile volume exceeds capacity, queue may be longer.						
Queue shown is maximum after two cycles.						
m Volume for 95th percentile queue is metered by upstream s	signal.					

Splits and Phases:	6: Main & Hazel	
№ Ø9	↑a₁ ↑a₂(R)	● ø3 → ø4
15 s	5 s 44 s	4.8 s 21.2 s
↑ ø13	↓ Ø ↓ Ø6 (R)	● ₀₇ ★ ₀₈
15 s	5 s 44 s	4.8 s 21.2 s

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	44.0	4.8
Total Split (%)	6%	49%	5%
Maximum Green (s)	3.0	38.2	2.8
Yellow Time (s)	2.0	38.2	2.8
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	31	31	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

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Appendix H

MMLOS Analysis



Multi-Modal Level of Service - Segments Form

Consultant Scenario Comments

CGH Transportation Inc.	Project
Existing/Future	Date

15 des Oblats Avenue	
2022-11-30	

OFOMENTO			Oblats	Springhurst	Oblats
SEGMENTS			Ex	Ex./Fut.	Fut.
	Sidewalk Width		no sidewalk	≥ 2 m	≥ 2 m
	Boulevard Width		n/a	< 0.5	< 0.5
	Avg Daily Curb Lane Traffic Volume		≤ 3000	≤ 3000	≤ 3000
an	Operating Speed		> 30 to 50 km/h	> 30 to 50 km/h	> 30 to 50 km/h
Ę.	On-Street Parking		no	yes	no
Pedestrian	Exposure to Traffic PLoS	-	F	В	В
eq	Effective Sidewalk Width				
	Pedestrian Volume				
	Crowding PLoS		-	-	-
	Level of Service		-	-	-
	Type of Cycling Facility		Mixed Traffic	Mixed Traffic	Mixed Traffic
	Number of Travel Lanes		2-3 lanes total	≤ 2 (no centreline)	2-3 lanes total
	Operating Speed		≤ 40 km/h	≤ 40 km/h	≤ 40 km/h
	# of Lanes & Operating Speed LoS		В	Α	В
Bicycle	Bike Lane (+ Parking Lane) Width				
Š	Bike Lane Width LoS	В	-	-	-
Bi	Bike Lane Blockages				
	Blockage LoS		-	-	-
	Median Refuge Width (no median = < 1.8 m)		< 1.8 m refuge	< 1.8 m refuge	< 1.8 m refuge
	No. of Lanes at Unsignalized Crossing		≤ 3 lanes	≤ 3 lanes	≤ 3 lanes
	Sidestreet Operating Speed		≤ 40 km/h	≤ 40 km/h	≤ 40 km/h
	Unsignalized Crossing - Lowest LoS		Α	Α	Α
	Level of Service		В	Α	В
Ħ	Facility Type		Mixed Traffic		Mixed Traffic
Transit	Friction or Ratio Transit:Posted Speed	D	Vt/Vp ≥ 0.8		Vt/Vp ≥ 0.8
Ë	Level of Service		D	-	D
	Truck Lane Width				
\ \ \ \	Travel Lanes per Direction				
Truck	Level of Service	-	-	-	-

Multi-Modal Level of Service - Intersections Form

Consultant	CGH Transportation Inc.	Project	15 des Oblats Avenue
Scenario	Existing/Future	Date	2022-11-30
Comments			

	INTERSECTIONS		Main @ F	lawthorne			Main @ Le	ees/Graham			Main @	② Evelyn			Main @ o	des Oblats			Main @	D Hazel	
	Crossina Side	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
	Lanes	4	4	0 - 2	4	4	5	4	0 - 2	3	3	0 - 2		4	3	3	3	3	3	3	0 - 2
	Median	No Median - 2.4 m	No Median - 2.4 m			No Median - 2.4 m		No Median - 2.4 m		-	No Median - 2.4 m	No Median - 2.4 m		No Median - 2.4 m		No Median - 2.4 m		No Median - 2.4 m	No Median - 2.4 m		
					Protected/											Protected/	Protected/			Protected/	Protected/
	Conflicting Left Turns	Permissive	No left turn / Prohib.		Permissive	No left turn / Prohib.	Permissive	Permissive	Permissive	No left turn / Prohib.	Permissive	No left turn / Prohib.		Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive
	Conflicting Right Turns	No right turn	Protected/ Permissive	Permissive or yield control	Permissive or yield control	Permissive or yield control	No right turn	Permissive or yield control	Permissive or yield control	Permissive or yield control	No right turn	Permissive or yield control		Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control
	Right Turns on Red (RToR) ?	RTOR allowed	RTOR allowed	RTOR prohibited	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR prohibited	RTOR prohibited	RTOR allowed	RTOR allowed		RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed
_	Ped Signal Leading Interval?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
ri ar	Right Turn Channel	No Right Turn	No Channel	No Channel	No Channel	No Channel	No Right Turn	No Channel	No Channel	No Channel	No Right Turn	No Channel		No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel
St	Corner Radius	No Right Turn	3-5m	3-5m	3-5m	10-15m	No Right Turn	5-10m	3-5m	5-10m	No Right Turn	5-10m		5-10m	5-10m	5-10m	5-10m	5-10m	5-10m	5-10m	5-10m
용	Crosswalk Type	Textured/coloured		Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured		Textured/coloured	Textured/coloured		Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured	Textured/coloured
B		pavement	pavement	pavement	pavement	pavement	pavement	pavement	pavement	pavement	pavement	pavement		pavement	pavement	pavement	pavement	pavement	pavement	pavement	pavement
	PETSI Score	73	68	95	60	66	57	59	95	87	90	97		59	76	76	76	76	76	76	91
	Ped. Exposure to Traffic LoS	С	С	Α	С	С	D	D	Α	В	Α	Α	-	D	В	В	В	В	В	В	Α
	Cycle Length	100	100	100	100	100	100	100	100	100	100	90		90	90	90	90	90	90	90	90
	Effective Walk Time	24	12	46	24	14	14	44	44	16	16	49		10	10	35	35	7	7	32	32
	Average Pedestrian Delay	29	39	15	29	37	37	16	16	35	35	9		36	36	17	17	38	38	19	19
	Pedestrian Delay LoS	С	D	В	С	D	D	В	В	D	D	Α	-	D	D	В	В	D	D	В	В
		С	D	В	С	D	D	D	В	D	D	Α		D	D	В	В	D	D	В	В
	Level of Service			D				D				D				D					_
				ט				ט				ט				ט				J	
	Approach From	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
	Bicycle Lane Arrangement on Approach		Curb Bike Lane, Cycletrack or MUP				Curb Bike Lane, Cycletrack or MUP				Curb Bike Lane, Cycletrack or MUP	1		Curb Bike Lane, Cycletrack or MUP	Curb Bike Lane, Cycletrack or MUP			Curb Bike Lane, Cycletrack or MUP	Curb Bike Lane, Cycletrack or MUP	Mixed Traffic	
	Right Turn Lane Configuration		Not Applicable				Not Applicable				Not Applicable			Not Applicable	Not Applicable			Not Applicable	Not Applicable	≤ 50 m	
	Right Turning Speed		Not Applicable				Not Applicable				Not Applicable			Not Applicable	Not Applicable			Not Applicable	Not Applicable	≤ 25 km/h	
	Cyclist relative to RT motorists	-	Not Applicable			-	Not Applicable				Not Applicable		-	Not Applicable	Not Applicable			Not Applicable	Not Applicable	D	
-	Separated or Mixed Traffic	-	Separated		-	-	Separated	-	-	-	Separated		-	Separated	Separated	-	-	Separated	Separated	Mixed Traffic	-
<u> </u>	•		•				•				oopa. a.oa				•				<u> </u>		
Bicycl	Left Turn Approach	No lane crossed	2-stage, LT box		2-stage, LT box	One lane crossed	2-stage, LT box	2-stage, LT box	2-stage, LT box			2-stage, LT box		2-stage, LT box	2-stage, LT box	No lane crossed	2-stage, LT box	2-stage, LT box	2-stage, LT box	2-stage, LT box	2-stage, LT box
	Operating Speed	> 50 to < 60 km/h	> 50 to < 60 km/h		> 50 to < 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	≤ 40 km/h			> 50 to < 60 km/h		> 50 to < 60 km/h	> 50 to < 60 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h
	Left Turning Cyclist	С	Α		A	E	Α	A	A	-		A	<u> </u>	A	A	В	A	A	A	A	Α
	Level of Complete	С	Α	-	Α	E	Α	Α	Α	-	-	Α	-	Α	Α	Α	Α	Α	Α	D	Α
	Level of Service			С				E				Α				A			l	ס	
=	Average Signal Delay	≤ 30 sec	≤ 20 sec		> 40 sec	≤ 20 sec	≤ 20 sec	> 40 sec		≤ 20 sec	≤ 20 sec			≤ 30 sec	≤ 30 sec	≤ 10 sec		≤ 20 sec	≤ 30 sec		
ns ns		D	С	-	F	С	С	F	-	С	С	-	-	D	D	В	-	С	D	-	-
Tra	Level of Service			F				F				С				D				D	
	Effective Corner Radius	> 15 m			< 10 m		> 15 m	> 15 m													
8	Number of Receiving Lanes on Departure from Intersection	1			≥2		1	≥ 2													
Ę		С	-	-	D	-	С	Α	-	-	-		-	-	-	-	-	-		-	-
	Level of Service			 D				C				-				-				-	
9	Volume to Capacity Ratio		0.81	- 0.90			0.61	- 0.70			0.0	- 0.60			0.61	- 0.70			0.71	- 0.80	
1	Lovel of Comitee			D				В				٨				В				C	
▼	Level of Service			<u>י</u>				D				Α				D					

Appendix I

TDM Checklist



TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

	Legend
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
*	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC ★	1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	abla
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	▽
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
	3.2	Transit fare incentives	
BASIC ★	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
	3.3	Enhanced public transit service	
BETTER ★	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station (multi-family)	abla
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
	4.2	Carshare vehicles & memberships	
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	
	5.	PARKING	
	5.1	Priced parking	
BASIC *	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC ★	5.1.2	Unbundle parking cost from monthly rent (multi-family)	

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TDM	measures: Residential developments	Check if proposed & add descriptions
6.	TDM MARKETING & COMMUNICATIONS	
6.1	Multimodal travel information	
BASIC ★ 6.1.1	Provide a multimodal travel option information package to new residents	♥
6.2	Personalized trip planning	
BETTER ★ 6.2.1	Offer personalized trip planning to new residents	

TDM-Supportive Development Design and Infrastructure Checklist: Residential Developments (multi-family or condominium)

Legend				
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed			
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users			
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance			

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	□ ✓
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	□ .
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	alpha
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	ℴ

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	✓
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	✓
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	Ø
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

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	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	abla
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	♥
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	abla
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	✓
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	♥
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area	

	TDM-	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	abla
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	abla
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and visitor spaces)	

Appendix J

Synchro Intersection Worksheets – 2025 Future Total Conditions



Future Total 2025AM Peak Hour

15 Oblates

CGH Transportation Page 2

	-	*	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	4	7		414		414						
Traffic Volume (vph)	12	235	296	539	5	405						
Future Volume (vph)	12	235	296	539	5	405						
Lane Group Flow (vph)	312	235	0	841	0	524						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	33				25		58	58		25	58	58
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.78	0.32		0.64		0.50						
Control Delay	51.3	3.9		9.6		24.4						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	51.3	3.9		9.6		24.4						
LOS	D	A		A		C						
Approach Delay	30.9	,,		9.6		24.4						
Approach LOS	С			A		С						
Queue Length 50th (m)	54.3	0.0		25.5		37.5						
Queue Length 95th (m)	#126.6	14.4		31.8		53.0						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)	100.1			00.0		202.1						
Base Capacity (vph)	399	724		1317		1039						
Starvation Cap Reductn	0	0		37		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.78	0.32		0.66		0.50						
Intersection Summary												
Cycle Length: 100												

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 60 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

	-	*	1	T	-	¥						
ane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
ne Configurations	લી	7		413		419						
affic Volume (vph)	12	235	296	539	5	405						
uture Volume (vph)	12	235	296	539	5	405						
ane Group Flow (vph)	312	235	0	841	0	524						
ırn Type		pm+ov	custom	NA	custom	NA						
otected Phases	4	13		1 2 9 10		5 6	1	2	3	5	9	10
ermitted Phases		4	10.2	0 . 0	6	- 00		_				.0
etector Phase	4	13		1 2 9 10	6	56						
vitch Phase	4	10	10	12010	U	0.0						
inimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
inimum Split (s)	22.3				34.0							
otal Split (s)		22.0	22.0				5.0	34.0	4.7	5.0	5.0	17.0
otal Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
aximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
ellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
I-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
ost Time Adjust (s)	0.0	0.0										
otal Lost Time (s)	6.3	6.3										
ead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
ead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
ehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
ecall Mode	Max	Max	Max		C-Max			C-Max	Max	Max	Max	Max
alk Time (s)	7.0	. Jidan			2.0		3.0	2.0		3.0	3.0	2.0
ash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
edestrian Calls (#/hr)	33				25		58	58		25	58	58
et Effet Green (s)	25.6	41.3		57.0	20	37.0	50	50		20	50	50
ctuated g/C Ratio	0.26	0.41		0.57		0.37						
	0.26	0.41										
c Ratio				0.64		0.50						
ontrol Delay	51.3	3.9		9.6		24.4						
ueue Delay	0.0	0.0		0.1		0.0						
otal Delay	51.3	3.9		9.6		24.4						
OS	D	Α		Α		С						
proach Delay	30.9			9.6		24.4						
proach LOS	С			Α		С						
ueue Length 50th (m)	54.3	0.0		25.5		37.5						
ueue Length 95th (m)	#126.6	14.4		31.8		53.0						
ternal Link Dist (m)	198.7			59.0		262.1						
urn Bay Length (m)				00.0								
ase Capacity (vph)	399	724		1317		1039						
arvation Cap Reductn	0	0		37		0						
oillback Cap Reductin	0	0		0		0						
	•					-						
orage Cap Reductn	0	0		0		0						
educed v/c Ratio	0.78	0.32		0.66		0.50						
tersection Summary												

Lanes, Volumes, Timings 1: Main & Hawthorne

Future Total 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.78 Intersection Signal Delay: 19.8
Intersection Capacity Utilization 78.1%
Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne **♣** Ø13

02-07-2023 CGH Transportation Page 3 Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2025AM Peak Hour 15 Oblates

	•	•	4	†	-	ļ					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĥ		414		414					
Traffic Volume (vph)	251	53	7	669	24	536					
Future Volume (vph)	251	53	7	669	24	536					
Lane Group Flow (vph)	251	277	0	746	0	640					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	62		137		42		137	42	62	9	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	1.07	0.58		0.46		0.40					
Control Delay	120.2	14.4		13.3		6.5					
Queue Delay	0.0	0.0		0.7		0.2					
Total Delay	120.2	14.4		14.0		6.7					
LOS	F	В		В		Α					
Approach Delay		64.7		14.0		6.7					
Approach LOS		Е		В		Α					
Queue Length 50th (m)	~54.3	10.7		30.9		13.9					
Queue Length 95th (m)	#101.0	36.4		51.8		22.1					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	234	476		1623		1583					
Starvation Cap Reductn	0	0		519		357					
Spillback Cap Reductn	0	0		0		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	1.07	0.58		0.68		0.52					

Intersection Summary

Cycle Length: 100

Offset: 39 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

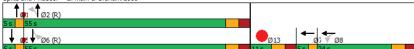
Natural Cycle: 65

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.07 Intersection Signal Delay: 25.6 Intersection Capacity Utilization 63.3% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service B Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles. Splits and Phases: 2: Main & Graham/Lees



02-07-2023 CGH Transportation Page 5 Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2025AM Peak Hour 15 Oblates

	•	†	↓	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	W		44	
Traffic Volume (vph)	44	692	720	
Future Volume (vph)	44	692	720	
Lane Group Flow (vph)	92	692	720	
Turn Type	Perm	NA	NA	
Protected Phases		2	6	7
Permitted Phases	8	=	-	
Detector Phase	8	2	6	
Switch Phase		_		
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	69.0	69.0	5.0
Total Split (%)	26.0%	69.0%	69.0%	5%
Maximum Green (s)	20.6	63.9	63.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
	2.1	1.8	1.8	0.0
All-Red Time (s)				0.0
Lost Time Adjust (s)	0.0 5.4	0.0 5.1	0.0 5.1	
Total Lost Time (s)		5.1	5.1	Local
Lead/Lag Lead-Lag Optimize?	Lag			Lead Yes
	Yes 3.0	3.0	3.0	3.0
Vehicle Extension (s)				
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	24	70	00.0	24
Act Effct Green (s)	20.6	63.9	63.9	
Actuated g/C Ratio	0.21	0.64	0.64	
v/c Ratio	0.27	0.63	0.34	
Control Delay	20.1	14.1	12.0	
Queue Delay	0.0	1.3	0.9	
Total Delay	20.1	15.3	12.9	
LOS	С	В	В	
Approach Delay	20.1	15.3	12.9	
Approach LOS	С	В	В	
Queue Length 50th (m)	7.0	73.4	35.0	
Queue Length 95th (m)	20.3	108.3	m42.6	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	343	1104	2097	
Starvation Cap Reductn	0	215	1034	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.27	0.78	0.68	
Intersection Summary				
Cycle Length: 100				
Actuated Cycle Length: 100				
,				

Offset: 59 (59%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

CGH Transportation Page 6 Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.63 Intersection Signal Delay: 14.5
Intersection Capacity Utilization 63.9%
Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service B m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn



02-07-2023 CGH Transportation Page 7 Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2025AM Peak Hour 15 Oblates

	•	\rightarrow	1	-	1	1	-	Į.				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	7	f)	٦	f)	٦	î»				
Traffic Volume (vph)	11	1	39	0	29	672	25	725				
Future Volume (vph)	11	1	39	0	29	672	25	725				
Lane Group Flow (vph)	0	29	39	46	29	692	25	736				
Turn Type	Perm	NA	Perm	NA	custom		custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	2 9	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	5.0	33.0	5.0
Total Split (s)	22.0	22.0	22.0	22.0	16.0		16.0		42.0	5.0	42.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	17.8%		17.8%		47%	6%	47%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	11.0		11.0		37.0	3.0	37.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0	3.0	20.0	3.0
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0	0.0	8.0	0.0
Pedestrian Calls (#/hr)	12	12	2	2					45	12	36	2
Act Effct Green (s)		16.7	16.7	16.7	54.0	48.4	54.0	48.3				
Actuated g/C Ratio		0.19	0.19	0.19	0.60	0.54	0.60	0.54				
v/c Ratio		0.11	0.25	0.09	0.09	0.76	0.07	0.79				
Control Delay		19.7	36.2	0.3	3.8	12.5	6.8	27.0				
Queue Delay		0.0	0.0	0.0	0.0	0.3	0.0	0.0				
Total Delay		19.7	36.2	0.3	3.8	12.8	6.8	27.0				
LOS		В	D	Α	Α	В	Α	С				
Approach Delay		19.7		16.8		12.4		26.3				
Approach LOS		В		В		В		С				
Queue Length 50th (m)		1.7	5.8	0.0	0.6	15.0	1.4	84.1				
Queue Length 95th (m)		8.9	15.0	0.0	m1.3	#169.1	4.0	#190.8				
Internal Link Dist (m)		109.1		138.0		118.6		47.0				
Turn Bay Length (m)			15.0		15.0		30.0					
Base Capacity (vph)		262	156	531	391	916	425	926				
Starvation Cap Reductn		0	0	0	0	25	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.11	0.25	0.09	0.07	0.78	0.06	0.79				

Intersection Summary

Cycle Length: 90

Cycle Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 80

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2025AM Peak Hour
15 Oblates

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	45	36
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

02-07-2023 CGH Transportation Page 10

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2025AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated

Maximum vic Ratio: 0.79

Intersection Signal Delay: 19.4

Intersection Capacity Utilization 61.3%

ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

 Splits and Phases:
 5: Main & Immaculata HS/Oblats

 Ø1
 Ø2 (R)

 I6s
 5s

 Ø5
 Ø8 Ø6 (R)

15 Oblates

Future Total 2025AM Peak Hour

	•	→	•	←	*	4	†	-	ļ			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ની	7	٦	fa fa	ሻ	f)			
Traffic Volume (vph)	32	3	17	2	28	9	665	62	707			
Future Volume (vph)	32	3	17	2	28	9	665	62	707			
Lane Group Flow (vph)	0	44	0	19	28	9	692	62	728			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	29	29	14	14	14					35	35	
Act Effct Green (s)		15.8		15.8	15.8	47.3	50.5	55.5	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.20		0.10	0.09	0.03	0.73	0.23	0.66			
Control Delay		29.2		32.8	0.5	7.2	21.7	11.4	14.2			
Queue Delay		0.0		0.0	0.0	0.0	0.4	0.0	0.1			
Total Delay		29.2		32.8	0.5	7.2	22.1	11.4	14.3			
LOS		С		С	Α	Α	С	В	В			
Approach Delay		29.3		13.6			22.0		14.1			
Approach LOS		С		В			С		В			
Queue Length 50th (m)		5.2		2.8	0.0	0.5	88.1	3.0	35.1			
Queue Length 95th (m)		14.5		8.8	0.0	2.1	140.7	m8.3	95.6			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		221		190	319	361	947	299	1111			
Starvation Cap Reductn		0		0	0	0	0	0	36			
Spillback Cap Reductn		0		0	4	0	45	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.20		0.10	0.09	0.02	0.77	0.21	0.68			

In	te	rs	e	cti	01	1	S	u	m	m	ıa	ŋ	

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 46 (51%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 75

02-07-2023

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			D1
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type Protected Phases	5	6	7
Permitted Phases	5	0	1
Detector Phase			
Switch Phase	4.0	40.0	4.0
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	43.0	5.0
Total Split (%)	6%	48%	6%
Maximum Green (s)	3.0	37.2	3.0
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	46	46	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

Lanes, Volumes, Timings 6: Main & Hazel

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 18.0
Intersection Capacity Utilization 77.5%
Analysis Period (min) 15 Future Total 2025AM Peak Hour 15 Oblates

Intersection LOS: B
ICU Level of Service D

Splits and Phases: 6: Main & Hazel

m Volume for 95th percentile queue is metered by upstream signal.



02-07-2023 CGH Transportation Page 14 Lanes, Volumes, Timings 1: Main & Hawthorne

Future Total 2025PM Peak Hour 15 Oblates

	-	•	4	†	-	ļ						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ની	7		सीक		414						
Traffic Volume (vph)	70	278	263	477	8	468						
Future Volume (vph)	70	278	263	477	8	468						
Lane Group Flow (vph)	360	278	0	753	0	664						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	15.0
Total Split (s)	24.0	20.0	20.0		54.0		5.0	54.0	5.0	5.0	5.0	15.0
Total Split (%)	20.0%	16.7%	16.7%		45.0%		4%	45%	4%	4%	4%	13%
Maximum Green (s)	17.7	13.7	13.7		47.7		3.0	47.7	3.0	3.0	3.0	8.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	None	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	6.7
Pedestrian Calls (#/hr)	29				25		55	55		25	55	55
Act Effct Green (s)	27.3	41.0		75.0		57.0						
Actuated g/C Ratio	0.23	0.34		0.62		0.48						
v/c Ratio	0.99	0.43		0.56		0.52						
Control Delay	91.9	5.5		11.9		21.3						
Queue Delay	0.0	0.0		0.8		0.0						
Total Delay	91.9	5.5		12.7		21.3						
LOS	F	Α		В		С						
Approach Delay	54.3			12.7		21.3						
Approach LOS	D			В		С						
Queue Length 50th (m)	82.7	0.0		40.3		50.1						
Queue Length 95th (m)	#176.5	18.7		51.3		67.1						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	363	652		1334		1273						
Starvation Cap Reductn	0	0		282		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.99	0.43		0.72		0.52						
Internation Cummen												

Intersection Summary

Cycle Length: 120

Actuated Cycle Length: 120
Offset: 26 (22%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

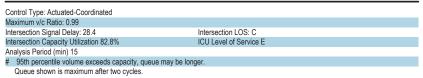
Natural Cycle: 90

15 Oblates

Future	Total 2025F	'M Peak Hour
		15 Oblates

Lane Group	Ø11	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	11	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	5.0	
Minimum Split (s)	12.0	
Total Split (s)	12.0	
Total Split (%)	10%	
Maximum Green (s)	5.7	
Yellow Time (s)	3.3	
All-Red Time (s)	3.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	
Lead-Lag Optimize?	Yes	
Vehicle Extension (s)	3.0	
Recall Mode	None	
Walk Time (s)	5.7	
Flash Dont Walk (s)	0.0	
Pedestrian Calls (#/hr)	2	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

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Lanes, Volumes, Timings

1: Main & Hawthorne

Splits and Phases: 1: Main & Hawthorne **3** ø₁₃

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2025PM Peak Hour 15 Oblates

	€	-	1	†	-	ļ					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĵ.,		414		413					
Traffic Volume (vph)	200	13	8	596	4	693					
Future Volume (vph)	200	13	8	596	4	693					
Lane Group Flow (vph)	200	151	0	703	0	724					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	17		48		35		48	35	17	5	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	0.73	0.36		0.43		0.41					
Control Delay	55.9	9.0		12.2		12.3					
Queue Delay	0.0	0.0		1.5		3.7					
Total Delay	55.9	9.0		13.8		16.0					
LOS	Е	Α		В		В					
Approach Delay		35.7		13.8		16.0					
Approach LOS		D		В		В					
Queue Length 50th (m)	37.0	1.9		35.7		37.5					
Queue Length 95th (m)	#68.7	16.9		48.5		49.9					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	273	419		1623		1778					
Starvation Cap Reductn	0	0		696		941					
Spillback Cap Reductn	0	0		0		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	0.73	0.36		0.76		0.86					

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 35 (35%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

02-07-2023 CGH Transportation Page 4 Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2025PM Peak Hour 15 Oblates

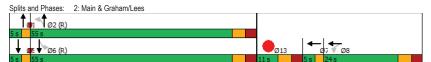
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73

Intersection LOS: B
ICU Level of Service A

Intersection Signal Delay: 19.0 Intersection Capacity Utilization 47.4% Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2025PM Peak Hour 15 Oblates

	•	1	↓	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥	A	^	
Traffic Volume (vph)	25	661	881	
Future Volume (vph)	25	661	881	
Lane Group Flow (vph)	32	661	881	
Turn Type	Perm	NA	NA	
Protected Phases	r Gill	2	6	7
Permitted Phases	8		- 0	- 1
Detector Phase	8	2	6	
Switch Phase	0	2	U	
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	59.0	59.0	5.0
	28.9%	65.6%	65.6%	6%
Total Split (%)				
Maximum Green (s)	20.6	53.9	53.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	16	57		16
Act Effct Green (s)	20.6	53.9	53.9	
Actuated g/C Ratio	0.23	0.60	0.60	
v/c Ratio	0.09	0.64	0.45	
Control Delay	23.7	9.2	10.8	
Queue Delay	0.0	0.1	1.9	
Total Delay	23.7	9.3	12.7	
LOS	С	Α	В	
Approach Delay	23.7	9.3	12.7	
Approach LOS	С	Α	В	
Queue Length 50th (m)	3.4	26.8	40.2	
Queue Length 95th (m)	10.5	m37.1	53.1	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	369	1034	1966	
Starvation Cap Reductn	0	16	882	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.09	0.65	0.81	
INGUUGEU WE INDU	0.09	0.00	0.01	
Intersection Summary				
Cycle Length: 90				
Actuated Cyala Langth: 00				

Actuated Cycle Length: 90
Offset: 54 (60%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

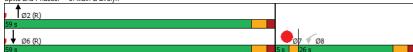
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Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2025PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.64 Intersection Signal Delay: 11.5 Intersection Capacity Utilization 62.1% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service B m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn



15 Oblates

CGH Transportation Page 10

	•	-	1	+	1	†	-	↓				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	ች	ĵ»	*	ĵ,	*	f)				
Traffic Volume (vph)	4	0	44	0	1	617	64	851				
Future Volume (vph)	4	0	44	0	1	617	64	851				
Lane Group Flow (vph)	0	15	44	54	1	667	64	859				
Turn Type	Perm	NA	Perm	NA	custom	NA	custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase	-		_		-							
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	3.0	33.0	3.0
Total Split (s)	22.0	22.0	22.0	22.0	15.0		15.0		43.0	5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		48%	6%	48%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	10.0		10.0		38.0	3.0	38.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)	2.0	0.0	0.0	0.0	0.0		0.0		1.7	0.0	1.7	0.0
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	140116		INOTIC		20.0	IVICIA	20.0	IVIGA
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0		8.0	
Pedestrian Calls (#/hr)	11	11	25	25					40		30	
Act Effct Green (s)	- 11	16.7	16.7	16.7	48.8	43.3	56.5	50.9	40		30	
Actuated g/C Ratio		0.19	0.19	0.19	0.54	0.48	0.63	0.57				
v/c Ratio		0.13	0.15	0.10	0.00	0.40	0.03	0.88				
Control Delay		0.04	35.5	0.10	10.00	24.5	6.6	27.6				
Queue Delay		0.2	0.0	0.4	0.0	0.1	0.0	2.6				
Total Delay		0.0	35.5	0.0	10.0	24.6	6.6	30.1				
LOS		0.2 A	33.3 D	0.4 A	10.0	24.0 C	0.0	30.1				
Approach Delay		0.2	U	16.1	А	24.6	A	28.5				
Approach LOS		0.2 A		В.		24.0 C		20.5 C				
Queue Length 50th (m)		0.0	6.6	0.0	0.1	41.9	3.2	54.0				
		0.0	16.2	0.0	m0.1	#162.9	6.6	#235.7				
Queue Length 95th (m) Internal Link Dist (m)		109.1	10.2	138.0	IIIU. I	118.6	0.0	47.0				
		109.1	45.0	130.0	45.0	110.0	20.0	47.0				
Turn Bay Length (m) Base Capacity (vph)		367	15.0 179	547	15.0 319	821	30.0 387	975				
Starvation Cap Reductn		0	0	0	0	7	0	0				
Spillback Cap Reductn		4	0	0	0	0	0	51				
Storage Cap Reductn		0	0	0	0	0	0 47	0				
Reduced v/c Ratio		0.04	0.25	0.10	0.00	0.82	0.17	0.93				
Intersection Summary												

n	ter	se	ctio	on	Su	mm	ary

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 62 (69%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

02-07-2023

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		10
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
	6%	6%
Total Split (%)		3.0
Maximum Green (s)	3.0	2.0
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	40	30
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2025PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.88 Intersection Signal Delay: 26.0
Intersection Capacity Utilization 76.4%
Analysis Period (min) 15 Intersection LOS: C
ICU Level of Service D

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Main & Immaculata HS/Oblats



02-07-2023 **CGH Transportation** Page 11 Lanes, Volumes, Timings 6: Main & Hazel

Future Total 2025PM Peak Hour 15 Oblates

	•	-	•	—	•	4	†	-	ļ			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ર્ન	7	Ĭ	f)	٦	î»			
Traffic Volume (vph)	18	4	48	5	77	10	571	42	824			
Future Volume (vph)	18	4	48	5	77	10	571	42	824			
Lane Group Flow (vph)	0	29	0	53	77	10	606	42	858			
Turn Type	Perm	NA	Perm	NA	Perm	custom		custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	21.2	21.2	21.2	21.2	21.2	15.0		15.0		5.0	44.0	4.8
Total Split (%)	23.6%	23.6%	23.6%	23.6%	23.6%	16.7%		16.7%		6%	49%	5%
Maximum Green (s)	15.0	15.0	15.0	15.0	15.0	9.2		9.2		3.0	38.2	2.8
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	20	20	14	14	14					35	35	
Act Effct Green (s)		15.0		15.0	15.0	51.0	54.3	56.4	59.6			
Actuated g/C Ratio		0.17		0.17	0.17	0.57	0.60	0.63	0.66			
v/c Ratio		0.13		0.27	0.23	0.04	0.60	0.13	0.76			
Control Delay		27.9		37.1	1.5	7.0	15.7	4.0	7.4			
Queue Delay		0.0		0.0	0.0	0.0	0.1	0.0	0.2			
Total Delay		27.9		37.1	1.5	7.0	15.8	4.0	7.5			
LOS		С		D	Α	Α	В	Α	Α			
Approach Delay		27.9		16.0			15.7		7.4			
Approach LOS		С		В			В		Α			
Queue Length 50th (m)		3.3		8.1	0.0	0.6	68.0	1.2	11.1			
Queue Length 95th (m)		10.8		18.8	0.0	2.2	108.3	m1.5 ı	m#137.9			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		224		194	342	290	1009	354	1124			
Starvation Cap Reductn		0		0	0	0	0	0	21			
Spillback Cap Reductn		0		0	3	0	41	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.13		0.27	0.23	0.03	0.63	0.12	0.78			

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90
Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Lanes, Volumes, Timings 6: Main & Hazel

Future Total 2025PM Peak Hour 15 Oblates

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	44.0	4.8
Total Split (%)	6%	49%	5%
Maximum Green (s)	3.0	38.2	2.8
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	36	36	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Cummens			
Intersection Summary			

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Lanes, Volumes, Timings 6: Main & Hazel

Future Total 2025PM Peak Hour 15 Oblates

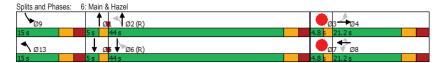
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76

Intersection Signal Delay: 11.5 Intersection Capacity Utilization 72.8% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



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Appendix K

Synchro Intersection Worksheets – 2030 Future Total Conditions



15 Oblates

Future Total 2030AM Peak Hour

Lane Group Lane Configurations

	-	•	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		4 î b						
Traffic Volume (vph)	12	244	296	539	5	405						
Future Volume (vph)	12	244	296	539	5	405						
Lane Group Flow (vph)	323	244	0	841	0	524						
Turn Type	NA	pm+ov			custom	NA						
Protected Phases	4	13		12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	17.0
Total Split (s)	22.3	22.0	22.0		34.0		5.0	34.0	4.7	5.0	5.0	17.0
Total Split (%)	22.3%	22.0%	22.0%		34.0%		5%	34%	5%	5%	5%	17%
Maximum Green (s)	16.0	15.7	15.7		27.7		3.0	27.7	2.7	3.0	3.0	10.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	Max	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	8.7
Pedestrian Calls (#/hr)	33				25		58	58		25	58	58
Act Effct Green (s)	25.6	41.3		57.0		37.0						
Actuated g/C Ratio	0.26	0.41		0.57		0.37						
v/c Ratio	0.81	0.33		0.64		0.50						
Control Delay	53.5	3.9		10.0		24.4						
Queue Delay	0.0	0.0		0.1		0.0						
Total Delay	53.5	3.9		10.1		24.4						
LOS	D	Α		В		С						
Approach Delay	32.2			10.1		24.4						
Approach LOS	С			В		С						
Queue Length 50th (m)	56.8	0.0		30.3		37.5						
Queue Length 95th (m)	#131.8	14.6		35.9		53.0						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	399	730		1317		1039						
Starvation Cap Reductn	0	0		44		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.81	0.33		0.66		0.50						
Intersection Summary												

	ection	

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 60 (60%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle: 85

Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
	F 0
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	12%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	4
Act Effct Green (s)	4
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Reduced V/C Ratio	
Intersection Summany	

Lanes, Volumes, Timings 1: Main & Hawthorne

Future Total 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.81 Intersection Signal Delay: 20.5 Intersection Capacity Utilization 78.8% Analysis Period (min) 15 Intersection LOS: C
ICU Level of Service D # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Main & Hawthorne **♣** Ø13

02-07-2023 CGH Transportation Page 3 Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2030AM Peak Hour 15 Oblates

	•	-	1	1	-	↓					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĥ		414		414					
Traffic Volume (vph)	331	53	7	669	24	536					
Future Volume (vph)	331	53	7	669	24	536					
Lane Group Flow (vph)	331	351	0	747	0	640					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		78		12		56	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	78	2	12	6	56					
Switch Phase											
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0										
Total Lost Time (s)	6.0										
Lead/Lag			Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	62		137		42		137	42	62	9	
Act Effct Green (s)	18.0	27.0		58.0		58.0					
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	1.41	0.75		0.46		0.40					
Control Delay	243.0	24.6		13.3		6.5					
Queue Delay	0.0	0.0		0.7		0.3					
Total Delay	243.0	24.6		14.0		6.8					
LOS	F	С		В		Α					
Approach Delay		130.6		14.0		6.8					
Approach LOS		F		В		Α					
Queue Length 50th (m)	~86.8	25.4		31.0		14.1					
Queue Length 95th (m)	#139.1	#68.3		52.0		22.4					
Internal Link Dist (m)		426.1		69.4		59.0					
Turn Bay Length (m)	40.0										
Base Capacity (vph)	234	470		1621		1583					
Starvation Cap Reductn	0	0		517		361					
Spillback Cap Reductn	0	0		10		0					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	1.41	0.75		0.68		0.52					

Intersection Summary

Cycle Length: 100

Offset: 39 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

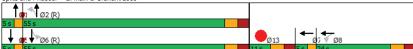
Natural Cycle: 65

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.41 Intersection Signal Delay: 50.2 Intersection LOS: D Intersection Capacity Utilization 68.2% Analysis Period (min) 15 ICU Level of Service C Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Main & Graham/Lees



02-07-2023 CGH Transportation Page 5 Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2030AM Peak Hour 15 Oblates

	€	†	ļ	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥		^	
Traffic Volume (vph)	44	692	720	
Future Volume (vph)	44	692	720	
Lane Group Flow (vph)	92	692	720	
Turn Type	Perm	NA	NA	
Protected Phases	1 61111	2	6	7
Permitted Phases	8		U	,
Detector Phase	8	2	6	
Switch Phase	0		U	
	40.0	40.0	40.0	4.0
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	69.0	69.0	5.0
Total Split (%)	26.0%	69.0%	69.0%	5%
Maximum Green (s)	20.6	63.9	63.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	24	70		24
Act Effct Green (s)	20.6	63.9	63.9	
Actuated g/C Ratio	0.21	0.64	0.64	
v/c Ratio	0.27	0.63	0.34	
Control Delay	20.1	14.1	13.6	
Queue Delay	0.0	1.3	1.1	
Total Delay	20.1	15.3	14.7	
LOS	20.1	13.3 B	14.7 B	
	20.1	15.3	14.7	
Approach Delay				
Approach LOS	C	В	В	
Queue Length 50th (m)	7.0	73.4	37.6	
Queue Length 95th (m)	20.3	108.3	m41.1	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	343	1104	2097	
Starvation Cap Reductn	0	215	1072	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.27	0.78	0.70	
Intersection Summary				
Cycle Length: 100				
ersection Summary				Start of (

Natural Cycle: 65

Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2030AM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.63 Intersection LOS: B
ICU Level of Service B Intersection Signal Delay: 15.3 Intersection Capacity Utilization 63.9% Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn 1 Ø2 (R) Ø6 (R)

02-07-2023 **CGH Transportation** Page 7 Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2030AM Peak Hour 15 Oblates

	•	\rightarrow	•	+	4	†	-	Į.				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	ሻ	f)	ሻ	f)	ሻ	1>				
Traffic Volume (vph)	11	1	39	0	29	672	25	725				
Future Volume (vph)	11	1	39	0	29	672	25	725				
Lane Group Flow (vph)	0	29	39	46	29	692	25	736				
Turn Type	Perm	NA	Perm	NA	custom	NA	custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	5.0	33.0	5.0
Total Split (s)	22.0	22.0	22.0	22.0	16.0		16.0		42.0	5.0	42.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	17.8%		17.8%		47%	6%	47%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	11.0		11.0		37.0	3.0	37.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0	3.0	20.0	3.0
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0	0.0	8.0	0.0
Pedestrian Calls (#/hr)	12	12	2	2					45	12	36	2
Act Effct Green (s)		16.7	16.7	16.7	54.0	48.4	54.0	48.3				
Actuated g/C Ratio		0.19	0.19	0.19	0.60	0.54	0.60	0.54				
v/c Ratio		0.11	0.25	0.09	0.09	0.76	0.07	0.79				
Control Delay		19.7	36.2	0.3	3.8	12.5	6.8	27.0				
Queue Delay		0.0	0.0	0.0	0.0	0.3	0.0	0.0				
Total Delay		19.7	36.2	0.3	3.8	12.8	6.8	27.0				
LOS		В	D	Α	Α	В	Α	С				
Approach Delay		19.7		16.8		12.4		26.3				
Approach LOS		В		В		В		С				
Queue Length 50th (m)		1.7	5.8	0.0	0.6	15.0	1.4	84.1				
Queue Length 95th (m)		8.9	15.0	0.0	m1.3	#169.1	4.0	#190.8				
Internal Link Dist (m)		109.1		138.0		118.6		47.0				
Turn Bay Length (m)			15.0		15.0		30.0					
Base Capacity (vph)		262	156	531	391	916	425	926				
Starvation Cap Reductn		0	0	0	0	25	0	0				
Spillback Cap Reductn		0	0	0	0	0	0	0				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.11	0.25	0.09	0.07	0.78	0.06	0.79				

Intersection Summary

Cycle Length: 90

Office Length: 90
Offset: 57 (63%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 80

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2030AM Peak Hour
15 Oblates

Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)	0.0	3.0
Total Lost Time (s)		
Lead/Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	45	36
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductin		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

02-07-2023 CGH Transportation Page 10

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2030AM Peak Hour
15 Oblates

Control Type: Actuated-Coordinated

Maximum vic Ratio: 0.79

Intersection Signal Delay: 19.4

Intersection Capacity Utilization 61.3%

ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

 Splits and Phases:
 5: Main & Immaculata HS/Oblats

 16 s
 15 s
 142 s
 22 s

 20 s
 22 s
 22 s

 20 s
 22 s
 22 s

15 Oblates

Future Total 2030AM Peak Hour

	۶	→	•	←	*	4	†	-	↓			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		4	7	٦	ĥ	ሻ	1•			
Traffic Volume (vph)	32	3	17	2	28	9	665	62	707			
Future Volume (vph)	32	3	17	2	28	9	665	62	707			
Lane Group Flow (vph)	0	44	0	19	28	9	692	62	728			
Turn Type	Perm	NA	Perm	NA	Perm	custom	NA	custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	22.0	22.0	22.0	22.0	22.0	15.0		15.0		5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		6%	48%	6%
Maximum Green (s)	15.8	15.8	15.8	15.8	15.8	9.2		9.2		3.0	37.2	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	29	29	14	14	14					35	35	
Act Effct Green (s)		15.8		15.8	15.8	47.3	50.5	55.5	58.6			
Actuated g/C Ratio		0.18		0.18	0.18	0.53	0.56	0.62	0.65			
v/c Ratio		0.20		0.10	0.09	0.03	0.73	0.23	0.66			
Control Delay		29.2		32.8	0.5	7.2	21.7	11.4	14.2			
Queue Delay		0.0		0.0	0.0	0.0	0.4	0.0	0.1			
Total Delay		29.2		32.8	0.5	7.2	22.1	11.4	14.3			
LOS		С		С	Α	Α	С	В	В			
Approach Delay		29.3		13.6			22.0		14.1			
Approach LOS		С		В			С		В			
Queue Length 50th (m)		5.2		2.8	0.0	0.5	88.1	3.0	35.1			
Queue Length 95th (m)		14.5		8.8	0.0	2.1	140.7	m8.3	95.6			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		221		190	319	361	947	299	1111			
Starvation Cap Reductn		0		0	0	0	0	0	36			
Spillback Cap Reductn		0		0	4	0	45	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.20		0.10	0.09	0.02	0.77	0.21	0.68			
latera estima Occasion												

In	te	rs	e	cti	01	1	S	u	m	m	ıa	ŋ	

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 46 (51%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 75

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases		·	
Detector Phase			
Switch Phase			
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	43.0	5.0
Total Split (%)	6%	48%	6%
Maximum Green (s)	3.0	37.2	3.0
Yellow Time (s)	2.0	3.3	2.0
	0.0	2.5	0.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	46	46	
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

Lanes, Volumes, Timings 6: Main & Hazel

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 18.0
Intersection Capacity Utilization 77.5%
Analysis Period (min) 15 Future Total 2030AM Peak Hour 15 Oblates

Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.



02-07-2023 **CGH Transportation** Page 14 Lanes, Volumes, Timings 1: Main & Hawthorne

Future Total 2030PM Peak Hour 15 Oblates

	-	•	4	†	-	↓						
Lane Group	EBT	EBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3	Ø5	Ø9	Ø10
Lane Configurations	ર્ન	7		414		413						
Traffic Volume (vph)	70	278	272	477	8	468						
Future Volume (vph)	70	278	272	477	8	468						
Lane Group Flow (vph)	360	278	0	762	0	671						
Turn Type	NA	pm+ov	custom	NA	custom	NA						
Protected Phases	4	13	13	12910		56	1	2	3	5	9	10
Permitted Phases		4	10 2		6							
Detector Phase	4	13	13	12910	6	56						
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0		10.0		1.0	10.0	1.0	1.0	1.0	5.0
Minimum Split (s)	22.3	11.3	11.3		17.3		5.0	17.3	3.0	5.0	5.0	15.0
Total Split (s)	24.0	20.0	20.0		54.0		5.0	54.0	5.0	5.0	5.0	15.0
Total Split (%)	20.0%	16.7%	16.7%		45.0%		4%	45%	4%	4%	4%	13%
Maximum Green (s)	17.7	13.7	13.7		47.7		3.0	47.7	3.0	3.0	3.0	8.7
Yellow Time (s)	3.3	3.3	3.3		3.3		2.0	3.3	2.0	2.0	2.0	3.3
All-Red Time (s)	3.0	3.0	3.0		3.0		0.0	3.0	0.0	0.0	0.0	3.0
Lost Time Adjust (s)	0.0	0.0										
Total Lost Time (s)	6.3	6.3										
Lead/Lag					Lag		Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?					Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max		C-Max		Max	C-Max	Max	Max	None	Max
Walk Time (s)	7.0				2.0		3.0	2.0		3.0	3.0	2.0
Flash Dont Walk (s)	9.0				9.0		0.0	9.0		0.0	0.0	6.7
Pedestrian Calls (#/hr)	29				25		55	55		25	55	55
Act Effct Green (s)	27.3	41.0		75.0		57.0						
Actuated g/C Ratio	0.23	0.34		0.62		0.48						
v/c Ratio	0.99	0.43		0.57		0.53						
Control Delay	91.9	5.5		12.0		21.3						
Queue Delay	0.0	0.0		0.8		0.0						
Total Delay	91.9	5.5		12.8		21.3						
LOS	F	Α		В		С						
Approach Delay	54.3			12.8		21.3						
Approach LOS	D			В		С						
Queue Length 50th (m)	82.7	0.0		40.8		50.6						
Queue Length 95th (m)	#176.5	18.7		52.1		68.0						
Internal Link Dist (m)	198.7			59.0		262.1						
Turn Bay Length (m)												
Base Capacity (vph)	363	652		1329		1267						
Starvation Cap Reductn	0	0		274		0						
Spillback Cap Reductn	0	0		0		0						
Storage Cap Reductn	0	0		0		0						
Reduced v/c Ratio	0.99	0.43		0.72		0.53						

Intersection Summary

Cycle Length: 120

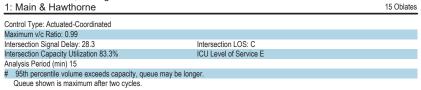
Actuated Cycle Length: 120
Offset: 26 (22%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 90

Intersection Summary

Lane Group	Ø11
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases	11
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0
Minimum Split (s)	12.0
Total Split (s)	12.0
Total Split (%)	10%
Maximum Green (s)	5.7
Yellow Time (s)	3.3
All-Red Time (s)	3.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	Lead
Lead-Lag Optimize?	Yes
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.7
Flash Dont Walk (s)	0.0
Pedestrian Calls (#/hr)	2
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (m)	
Queue Length 95th (m)	
Internal Link Dist (m)	
Turn Bay Length (m)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	

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Lanes, Volumes, Timings

 Splits and Phases:
 1: Main & Hawthorne

 1: Main & Hawthorne
 011

 02 (R)
 011

 5sl
 5sl

 2 → 04
 12 → 04

 5sl
 5sl

 12s
 5sl

 24s
 5sl

 3013

Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2030PM Peak Hour 15 Oblates

	€	—	1	1	-	ļ					
Lane Group	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø5	Ø7	Ø13	
Lane Configurations	*	ĵ.		414		413					
Traffic Volume (vph)	202	13	8	596	4	693					
Future Volume (vph)	202	13	8	596	4	693					
Lane Group Flow (vph)	202	152	0	734	0	724					
Turn Type	custom	NA	custom	NA	custom	NA					
Protected Phases		7.8		12		5 6	1	5	7	13	
Permitted Phases	8		2		6						
Detector Phase	8	7.8	2	12	6	56					
Switch Phase	_				_						
Minimum Initial (s)	10.0		10.0		10.0		1.0	1.0	1.0	5.0	
Minimum Split (s)	24.0		18.2		18.2		5.0	5.0	5.0	11.0	
Total Split (s)	24.0		55.0		55.0		5.0	5.0	5.0	11.0	
Total Split (%)	24.0%		55.0%		55.0%		5%	5%	5%	11%	
Maximum Green (s)	18.0		48.8		48.8		3.0	3.0	3.0	5.0	
Yellow Time (s)	3.3		3.3		3.3		2.0	2.0	2.0	3.3	
All-Red Time (s)	2.7		2.9		2.9		0.0	0.0	0.0	2.7	
Lost Time Adjust (s)	0.0		2.3		2.3		0.0	0.0	0.0		
Total Lost Time (s)	6.0										
Lead/Lag	0.0		Lag		Lag		Lead	Lead	Lag	Lead	
Lead-Lag Optimize?			Yes		Yes		Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0		3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max		C-Max		Max	Max	Max	Max	
Walk Time (s)	2.0		2.0		2.0		3.0	3.0	3.0	5.0	
Flash Dont Walk (s)	9.0		10.0		10.0		0.0	0.0	0.0	0.0	
Pedestrian Calls (#/hr)	17		48		35		48	35	17	5	
Act Effct Green (s)	18.0	27.0	70	58.0	00	58.0	40	55	- 17		
Actuated g/C Ratio	0.18	0.27		0.58		0.58					
v/c Ratio	0.74	0.36		0.46		0.41					
Control Delay	56.5	9.0		12.4		12.3					
Queue Delay	0.0	0.0		1.7		3.7					
Total Delay	56.5	9.0		14.1		16.0					
LOS	50.5 E	3.0 A		14.1		В					
Approach Delay		36.1		14.1		16.0					
Approach LOS		J0.1		14.1		В					
Queue Length 50th (m)	37.4	1.9		37.5		37.5					
Queue Length 95th (m)	#69.5	17.1		51.0		49.9					
Internal Link Dist (m)	που.υ	426.1		69.4		59.0					
Turn Bay Length (m)	40.0	42U. I		05.4		33.0					
Base Capacity (vph)	273	420		1581		1778					
Starvation Cap Reductn	0	420		640		941					
Spillback Cap Reductin	0	0		040		941					
Storage Cap Reductn	0	0		0		0					
Reduced v/c Ratio	0.74	0.36		0.78		0.86					
Neuroen Ale Light	0.74	0.30		0.70		0.00					
Internal of the Original of											

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 35 (35%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 65

02-07-2023 **CGH Transportation** Page 4 Lanes, Volumes, Timings 2: Main & Graham/Lees

Future Total 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.74

Intersection Signal Delay: 19.1 Intersection Capacity Utilization 48.7% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service A

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Main & Graham/Lees



02-07-2023 CGH Transportation

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Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2030PM Peak Hour 15 Oblates

	•	1	↓	
Lane Group	WBL	NBT	SBT	Ø7
Lane Configurations	¥	A	^	
Traffic Volume (vph)	25	661	881	
Future Volume (vph)	25	661	881	
Lane Group Flow (vph)	32	661	881	
Turn Type	Perm	NA	NA	
Protected Phases	r cill	2	6	7
Permitted Phases	8		- 0	- 1
Detector Phase	8	2	6	
Switch Phase	0	2	U	
Minimum Initial (s)	10.0	10.0	10.0	1.0
Minimum Split (s)	25.4	27.1	15.1	5.0
Total Split (s)	26.0	59.0	59.0	5.0
	28.9%	65.6%	65.6%	6%
Total Split (%)				
Maximum Green (s)	20.6	53.9	53.9	3.0
Yellow Time (s)	3.3	3.3	3.3	2.0
All-Red Time (s)	2.1	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.1	5.1	
Lead/Lag	Lag			Lead
Lead-Lag Optimize?	Yes			Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	C-Max	C-Max	Max
Walk Time (s)	10.0	17.0		3.0
Flash Dont Walk (s)	10.0	5.0		0.0
Pedestrian Calls (#/hr)	16	57		16
Act Effct Green (s)	20.6	53.9	53.9	
Actuated g/C Ratio	0.23	0.60	0.60	
v/c Ratio	0.09	0.64	0.45	
Control Delay	23.7	9.2	10.8	
Queue Delay	0.0	0.1	1.9	
Total Delay	23.7	9.3	12.7	
LOS	С	Α	В	
Approach Delay	23.7	9.3	12.7	
Approach LOS	С	Α	В	
Queue Length 50th (m)	3.4	26.8	40.2	
Queue Length 95th (m)	10.5	m37.1	53.1	
Internal Link Dist (m)	452.4	86.0	69.4	
Turn Bay Length (m)				
Base Capacity (vph)	369	1034	1966	
Starvation Cap Reductn	0	16	882	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.09	0.65	0.81	
INGUUGEU WE INDU	0.09	0.00	0.01	
Intersection Summary				
Cycle Length: 90				
Actuated Cyala Langth: 00				

Actuated Cycle Length: 90
Offset: 54 (60%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

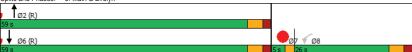
CGH Transportation Page 6 02-07-2023

Lanes, Volumes, Timings 3: Main & Evelyn

Future Total 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.64 Intersection Signal Delay: 11.5 Intersection Capacity Utilization 62.1% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service B m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Main & Evelyn



02-07-2023 CGH Transportation

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	•	\rightarrow	1	←	1	†	1	Į.				
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø3	Ø6	Ø7
Lane Configurations		4	7	1 >	7	fa fa	*	1 >				
Traffic Volume (vph)	4	0	44	0	1	617	64	851				
Future Volume (vph)	4	0	44	0	1	617	64	851				
Lane Group Flow (vph)	0	15	44	54	1	667	64	859				
Turn Type	Perm	NA	Perm	NA	custom	NA	custom	NA				
Protected Phases		4		8	5	29	1	6 13	2	3	6	7
Permitted Phases	4		8		2		6					
Detector Phase	4	4	8	8	5	29	1	6 13				
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0		5.0		10.0	1.0	10.0	1.0
Minimum Split (s)	19.3	19.3	19.3	19.3	10.0		10.0		33.0	3.0	33.0	3.0
Total Split (s)	22.0	22.0	22.0	22.0	15.0		15.0		43.0	5.0	43.0	5.0
Total Split (%)	24.4%	24.4%	24.4%	24.4%	16.7%		16.7%		48%	6%	48%	6%
Maximum Green (s)	16.7	16.7	16.7	16.7	10.0		10.0		38.0	3.0	38.0	3.0
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		3.3		3.3	2.0	3.3	2.0
All-Red Time (s)	2.0	2.0	2.0	2.0	1.7		1.7		1.7	0.0	1.7	0.0
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0					
Total Lost Time (s)		5.3	5.3	5.3	5.0		5.0					
Lead/Lag	Lag	Lag	Lag	Lag	Lead		Lead			Lead		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	None		None		C-Max	Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0					20.0		20.0	
Flash Dont Walk (s)	12.0	12.0	12.0	12.0					8.0		8.0	
Pedestrian Calls (#/hr)	11	11	25	25					40		30	
Act Effct Green (s)		16.7	16.7	16.7	48.8	43.3	56.5	50.9				
Actuated g/C Ratio		0.19	0.19	0.19	0.54	0.48	0.63	0.57				
v/c Ratio		0.04	0.25	0.10	0.00	0.81	0.19	0.88				
Control Delay		0.2	35.5	0.4	10.0	24.5	6.6	27.6				
Queue Delay		0.0	0.0	0.0	0.0	0.1	0.0	2.6				
Total Delay		0.2	35.5	0.4	10.0	24.6	6.6	30.1				
LOS		A	D	Α.	Α	C	Α.	C				
Approach Delay		0.2		16.1	71	24.6	/ (28.5				
Approach LOS		A		В		C		C				
Queue Length 50th (m)		0.0	6.6	0.0	0.1	41.9	3.2	54.0				
Queue Length 95th (m)		0.0	16.2	0.0	m0.1	#162.9	6.6	#235.7				
Internal Link Dist (m)		109.1	10.2	138.0	1110.1	118.6	0.0	47.0				
Turn Bay Length (m)		100.1	15.0	100.0	15.0	110.0	30.0	47.0				
Base Capacity (vph)		367	179	547	319	821	387	975				
Starvation Cap Reductn		0	0	0	0	7	0	0				
Spillback Cap Reductin		4	0	0	0	0	0	51				
Storage Cap Reductn		0	0	0	0	0	0	0				
Reduced v/c Ratio		0.04	0.25	0.10	0.00	0.82	0.17	0.93				
		0.04	0.20	0.10	0.00	0.02	0.17	0.33				
Intersection Summary												

In	te	rs	e	cti	or	า 8	SU	m	m	a	ŋ	

Cycle Length: 90
Actuated Cycle Length: 90
Offset: 62 (69%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 90

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Lane Group	Ø9	Ø13
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	13
Permitted Phases		.0
Detector Phase		
Switch Phase		
Minimum Initial (s)	1.0	1.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)		
Lead/Lag	Log	Lag
Lead-Lag Optimize?	Lag Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	Max
Walk Time (s)	3.0	3.0
Flash Dont Walk (s)	0.0	0.0
Pedestrian Calls (#/hr)	40	30
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Lanes, Volumes, Timings 5: Main & Immaculata HS/Oblats Future Total 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.88 Intersection Signal Delay: 26.0
Intersection Capacity Utilization 76.4%
Analysis Period (min) 15

Intersection LOS: C
ICU Level of Service D

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Main & Immaculata HS/Oblats



02-07-2023 CGH Transportation Page 11 Lanes, Volumes, Timings 6: Main & Hazel

Future Total 2030PM Peak Hour 15 Oblates

	•	→	•	←	•	4	†	-	ļ			
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø3
Lane Configurations		4		ર્ન	7	ሻ	î,	٦	î»			
Traffic Volume (vph)	18	4	48	5	77	10	571	42	824			
Future Volume (vph)	18	4	48	5	77	10	571	42	824			
Lane Group Flow (vph)	0	29	0	53	77	10	606	42	858			
Turn Type	Perm	NA	Perm	NA	Perm	custom		custom	NA			
Protected Phases		4		8		13	12	9	56	1	2	3
Permitted Phases	4		8		8	2		6				
Detector Phase	4	4	8	8	8	13	12	9	56			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	5.0		5.0		1.0	10.0	1.0
Minimum Split (s)	21.2	21.2	21.2	21.2	21.2	10.8		10.8		5.0	34.8	3.0
Total Split (s)	21.2	21.2	21.2	21.2	21.2	15.0		15.0		5.0	44.0	4.8
Total Split (%)	23.6%	23.6%	23.6%	23.6%	23.6%	16.7%		16.7%		6%	49%	5%
Maximum Green (s)	15.0	15.0	15.0	15.0	15.0	9.2		9.2		3.0	38.2	2.8
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3		3.3		2.0	3.3	2.0
All-Red Time (s)	2.9	2.9	2.9	2.9	2.9	2.5		2.5		0.0	2.5	0.0
Lost Time Adjust (s)		0.0		0.0	0.0	0.0		0.0				
Total Lost Time (s)		6.2		6.2	6.2	5.8		5.8				
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lead		Lead		Lag		Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0		3.0	3.0	3.0
Recall Mode	Max	Max	Max	Max	Max	None		None		Max	C-Max	Max
Walk Time (s)	2.0	2.0	2.0	2.0	2.0					3.0	18.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0					0.0	10.0	
Pedestrian Calls (#/hr)	20	20	14	14	14					35	35	
Act Effct Green (s)		15.0		15.0	15.0	51.0	54.3	56.4	59.6			
Actuated g/C Ratio		0.17		0.17	0.17	0.57	0.60	0.63	0.66			
v/c Ratio		0.13		0.27	0.23	0.04	0.60	0.13	0.76			
Control Delay		27.9		37.1	1.5	7.0	15.7	4.0	7.4			
Queue Delay		0.0		0.0	0.0	0.0	0.1	0.0	0.2			
Total Delay		27.9		37.1	1.5	7.0	15.8	4.0	7.5			
LOS		С		D	Α	Α	В	Α	Α			
Approach Delay		27.9		16.0			15.7		7.4			
Approach LOS		С		В			В		Α			
Queue Length 50th (m)		3.3		8.1	0.0	0.6	68.0	1.2	11.1			
Queue Length 95th (m)		10.8		18.8	0.0	2.2	108.3	m1.5 ı	m#137.9			
Internal Link Dist (m)		237.6		98.5			241.0		118.6			
Turn Bay Length (m)					30.0	20.0		15.0				
Base Capacity (vph)		224		194	342	290	1009	354	1124			
Starvation Cap Reductn		0		0	0	0	0	0	21			
Spillback Cap Reductn		0		0	3	0	41	0	0			
Storage Cap Reductn		0		0	0	0	0	0	0			
Reduced v/c Ratio		0.13		0.27	0.23	0.03	0.63	0.12	0.78			

Intersection Summary

Cycle Length: 90

Actuated Cycle Length: 90
Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Lanes, Volumes, Timings 6: Main & Hazel

Intersection Summary

Future Total 2030PM Peak Hour 15 Oblates

Lane Group	Ø5	Ø6	Ø7
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Lane Group Flow (vph)			
Turn Type			
Protected Phases	5	6	7
Permitted Phases	3	U	- 1
Detector Phase			
Switch Phase			
	4.0	40.0	4.0
Minimum Initial (s)	1.0	10.0	1.0
Minimum Split (s)	5.0	34.8	3.0
Total Split (s)	5.0	44.0	4.8
Total Split (%)	6%	49%	5%
Maximum Green (s)	3.0	38.2	2.8
Yellow Time (s)	2.0	3.3	2.0
All-Red Time (s)	0.0	2.5	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lag		Lead
Lead-Lag Optimize?	Yes		Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Max	C-Max	Max
Walk Time (s)	3.0	18.0	
Flash Dont Walk (s)	0.0	10.0	
Pedestrian Calls (#/hr)	36	36	
Act Effct Green (s)	30	30	
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			

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Lanes, Volumes, Timings 6: Main & Hazel

Future Total 2030PM Peak Hour 15 Oblates

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76

Intersection Signal Delay: 11.5 Intersection Capacity Utilization 72.8% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Main & Hazel †_{Ø1} ⁴†_{Ø2 (R)} **↑** Ø13 ₩ Ø6 (R)

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