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THE DETAILS THE

Engineers, Project Managers & Planners

November 3, 2022

CASTLEGLENN CONSULTANTS LTD.

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APPENDICESB

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LIST OF EXHIBITS B



1.0 SUMMARY OF DEVELOPMENT

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2.0 THE TIA PROCESS

3.0 SCREENING:

3.1 Trip Generation Triggers

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3.2 SCREENING: LOCATION TRIGGERS

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3.3 SCREENING SAFETY TRIGGERS

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3.4 EXEMPTION REQUEST

Include **Element**B **Exemption ConsiderationsB Module**B Module in TIA Design Review Component B Linere are no new streets being proposed as part of 4.1 Development Design **4.1.3** New Street Networks 져 B this development. B LX LBS/EHX (OBC) "/AAABHAKK (LBK LBK CB) X (OBCB) BK 4.2 Parking 4.2.2 Spillover Parking 雨 B THE REPORT TO VE STORE AND VEST AS **Network Impact Component** The development is not expected to generate more than 60 vehicle-trips during the peak hours of travel All Elements B 頃₿ 4.5 through 4.9 B demand. Therefore, the "Network Impact" component of the TIA is not required.

Table 3-1: Exemptions as per TIA Guidelines

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3.5 SCREENING CONCLUSIONS

3.6 STUDY AREA AND TIME PERIODS

3.6.1 Study Area

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3.6.2 Time Periods

3.6.3 Horizon Years

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4.0 SCOPING

4.1 EXISTING AND PLANNED CONDITIONS

4.1.1 Proposed Development

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Exhibit 4-1: Location of Proposed Development

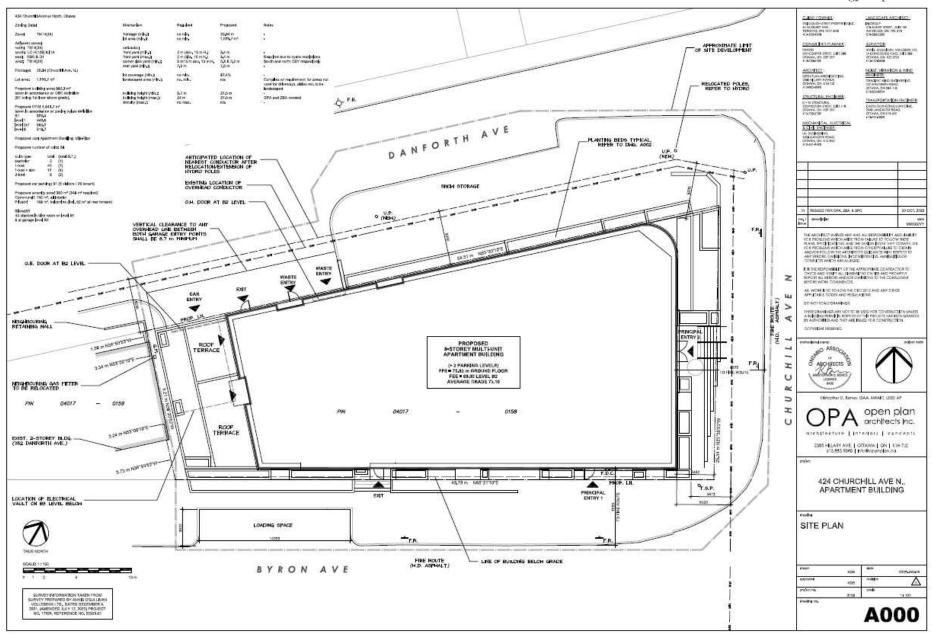


Exhibit 4-2: Site Plan (October 20, 2022)

4.1.2 Existing Conditions

4.1.2.1 Study Area Roadways

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Roosevelt Avenue

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Danforth Avenue

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4.1.2.2 Study Area Intersections

1. Richmond Road and Roosevelt Avenue

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Exhibit 4-3: Richmond Road and Roosevelt Avenue Intersection

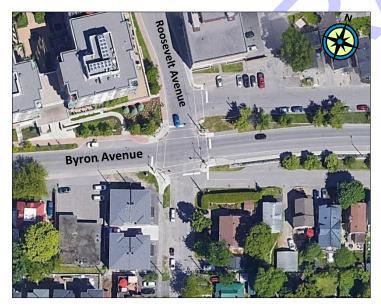


Exhibit 4-4: Roosevelt Avenue and Byron Avenue Intersection

2. Roosevelt Avenue and Byron Avenue

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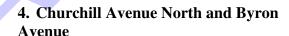
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3. Richmond Road and Churchill Avenue North

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Exhibit 4-5 Richmond Road and Churchill Avenue **North Intersection**



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Exhibit 4-6: Churchill Avenue North and Byron Avenue

5. Churchill Avenue North and Danforth Avenue

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Exhibit 4-7: Churchill Avenue North and Danforth Avenue Intersection

6. Roosevelt Avenue and Danforth Avenue



Exhibit 4-8: Roosevelt Avenue and Danforth Avenue Intersection

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7. Danforth Avenue and MEC Parking Accesses



Exhibit 4-9: Danforth Avenue and MEC Parking Accesses

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4.1.2.3 Existing Surrounding Driveways

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Exhibit 4-10: Overview of Existing Adjacent Driveways



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Churchill Avenue North Accesses:

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- 377-381 Churchill Avenue North 型網路銀河口口 《地路山口路河山湖山) 中心 〇個」 《路路山》 「山」 型 (B) 《路山》 「山」 四 (B) 四 (B

4.1.2.4 Existing Pedestrian and Cycling Facilities

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4.1.2.5 Area Traffic Management

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4.1.2.6 Existing Transit Provisions

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424 Churchill Avenue Residential Apartments Development

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Exhibit 4-11: Transit Lines in the Study Area (Not to Scale)

Table 4-1: Existing Transit Routes

Route	Description
11	This "Frequent" bus route connects Bayshore rapid transit station to downtown (Mackenzie King station) travelling via Richmond Road, Wellington Street and Somerset Street. The route runs Monday-thru-Sunday with peak hour headways of 15 minutes.
50	This is a "Local" bus route that connects the Tunney's Pasture rapid transit station to neighbourhoods along Scott Street, Churchill Avenue North and Maitland Avenue. The route then connects to Iris, the Queensway and the Lincoln Fields stations. The buses run Monday-thru-Saturday with 30-minute headways.
153	This is a "Local" bus route that travels between Lincoln Fields station, the Carlingwood Mall and Tunney's Pasture rapid bus station. Select trips only run between Lincoln Fields and Carlingwood Mall (outside of the study area). The headways are 1-2 hours.



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4.1.2.7 Existing Peak Hour Travel Demands by Mode

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Pedestrian and Cyclist Travel Demand

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Table 4-2: Pedestrian Peak Hour and 8-Hour Traffic Volumes

Period	Pedestrians Crossing	Richmond Road and Roosevelt Avenue	Roosevelt Avenue and Byron Avenue	Richmond Road and Churchill Avenue North	Churchill Avenue North and Byron Avenue
8 Hour	Constant Foot	530	148	330	206
AM Peak	Crossing East Leg	40	13	36	18
PM Peak	Leg	92	17	54	29
8 - Hour	Consider Man	714	277	553	282
AM Peak	Crossing West	65	33	45	43
PM Peak	Leg	116	46	84	40
8 Hour	C	912	72	589	107
AM Peak	Crossing North Leg	74	8	34	13
PM Peak	North Leg	135	9	86	10
8 Hour	C	1032	78	822	125
AM Peak	Crossing	69	13	68	13
PM Peak	South Leg	182	7	163	11
	Total	3,188	575	2,294	720

Table 4-3: Cyclist Peak Hour and 8-Hour Traffic Volumes

Period	Cyclists Travelling	Richmond Road and Roosevelt Avenue	Roosevelt Avenue and Byron Avenue	Richmond Road and Churchill Avenue North	Churchill Avenue North and Byron Avenue
8 Hour		26	5	26	5
AM Peak	Eastbound	7	0	11	1
PM Peak		3	2	2	1
8 - Hour		19	3	10	5
AM Peak	Westbound	4	0	1	0
PM Peak		9	1	4	1
8 Hour		12	4	24	27
AM Peak	Northbound	4	0	8	7
PM Peak		2	1	1	0
8 Hour		14	3	14	16
AM Peak	Southbound	4	0	0	0
PM Peak		5	0	3	6
_	Total	71	15	74	53

Vehicular Travel Demand

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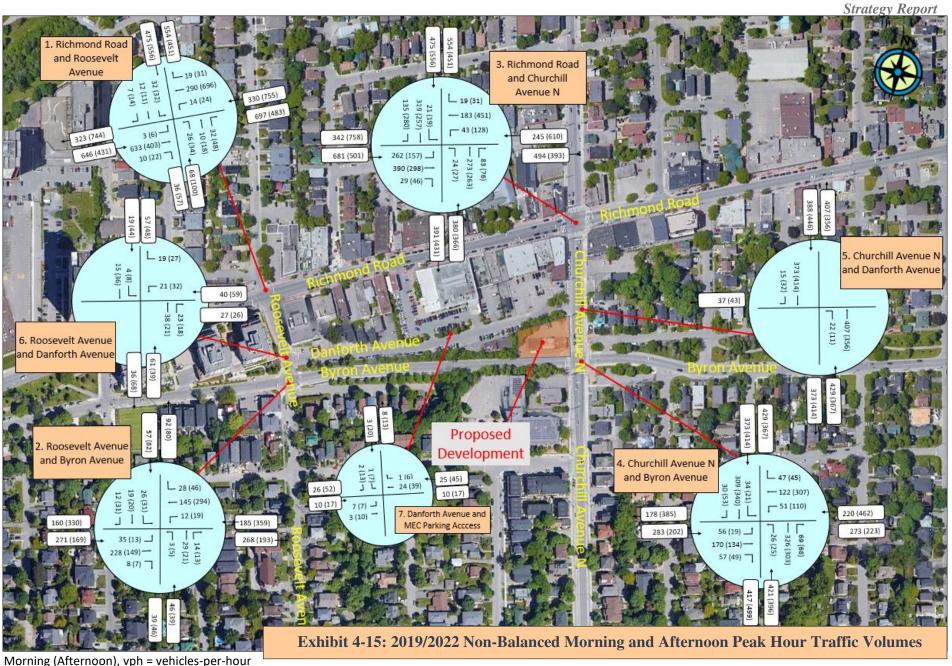
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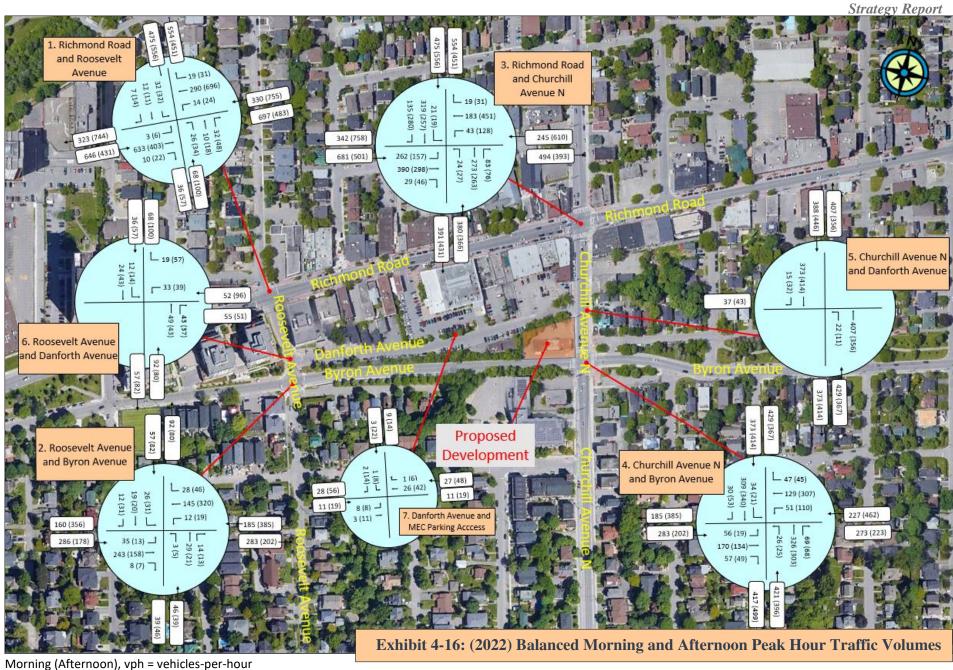
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424 Churchill Avenue Residential Apartments Development

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Existing Traffic Volumes Intersection Capacity Analysis

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Table 4-4: Existing (2022) Traffic Analysis

			Weekday Morning Peak Hour (Afternoon Peak Hour)				
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Intersection		Control Type	上。 FD 中央	・》□》□8□ H8 □□□□8□ H8 □□□□8□ H8	₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	岬帽邛	[「] ₹\$
1.	Richmond Road and Roosevelt Avenue	Traffic Signal	EB-TH	143	23.5	D	0.81
1.	(Distance to Dominion Station – 440 m)	Traffic Signal	(WB-TH)	(189)	(27.3)	(D)	(0.87)
			SB-TH	14	20.0	Α	0.16
2.	Roosevelt Avenue and Byron Avenue	Traffic Signal	(NB-TH)	(10)	(17.5)	(A)	(0.11)
۷.	(Distance to Dominion Station – 530 m)	Traffic Signal	EB-TH	26	6.3	Α	0.28
			(WB-TH)	(36)	(6.8)	(A)	(0.37)
			NB-TH	76	28.1	С	0.77
		Traffic Signal [2020 Layout]	(NB-TH)	(82)	(38.0)	(C)	(0.70)
	Richmond Road and Churchill Avenue (Distance to Churchill Alternative School – 185 m)		SB-TH	73	37.4	С	0.78
			(SB-TH)	(73)	(34.9)	(C)	(0.74)
3.		Traffic Signal [2022 Layout]	NB-TH/RT	85	18.4	В	0.61
٥.			(NB-TH/RT)	(47)	(23.3)	(C)	(0.72)
			Southbound	121	35.4	D	0.83
			Southbound	(204)	(213)	(F)	(1.38)
			Southbound	(202)	(198)	(F)	(1.35)
			-no LT in PM	(202)	(130)	(' /	(1.55)
			EB-TH	49	12.1	Α	0.35
	Churchill Avenue North and Byron		(WB-TH)	(105)	(17.8)	(A)	(0.59)
4.	Avenue	Traffic Signal	NB-TH	75	30.3	С	0.73
٦.	(Distance to Churchill Alternative School	Trume Signar	(SB-TH)	(44)	(23.9)	(C)	(0.77)
	– 50 m)		SB-LT	8	37.2	Α	0.26
			(SB-LT)	(3)	(14.6)	(A)	(0.16)
5.	Churchill Avenue and Danforth Avenue (Distance to Churchill Alternative School – 110 m)	Free Flow (Inbound only)	nd N/A				
	Roosevelt Avenue and Danforth Avenue	Minor Leg-STOP	WB	2	9.3	Α	0.06
6.	(Distance to Dominion Station – 500 m)	control	(WB)	(3)	(94)	(A)	(0.11)

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4.1.2.8 Existing Road Safety Information

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Table 4-5: Five -Year Collision History (January 1st, 2016 -to- December 31st, 2020)

Intersection / Mid-block Location		Richmond Road and Roosevelt Avenue	Roosevelt Avenue and Byron Avenue	Richmond Road and Churchill Avenue North	Churchill Avenue North and Byron Avenue	Churchill Avenue North and Danforth Avenue	Roosevelt Avenue and Danforth Avenue	Danforth Avenue between Churchill Avenue North and Roosevelt Avenue (mid-block)
Tota	l Collisions	8	3	25	5	1	1	5
	Rear End	6	-	8	1	-	-	-
	Single Vehicle	-	1	2	ı	-	ı	2
Collision	Sideswipe	1	1	5	ı	1	ı	-
Type	Turning Movement	-	1	5	3	-	-	-
Type	Angle	-	1	2	ı	-	ı	2
	Pedestrian	-	1	3	1	-	ı	
	Other	1	2	-	i	-	1	1
Collision	Property Damage	7	3	18	3	1	1	5
Severity	Non-Fatal Injury	1	-	7	2	-	-	-
Severity	Fatal	-	-	-	-	-	-	-
Inters	Intersection AADT		6,500	23,400	16,300	5,600	1,100	N/A
Collision	Rate per MEV	0.27	0.25	0.59	0.17	0.10	0.5	N/A

MEV = Millions of Vehicles Entering the Intersection or (mid-block) travelling along the corridor. AADT = Average Annual Daily Traffic

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4.1.3 Planned Conditions

4.1.3.1 Changes to the Study Area Transportation Network

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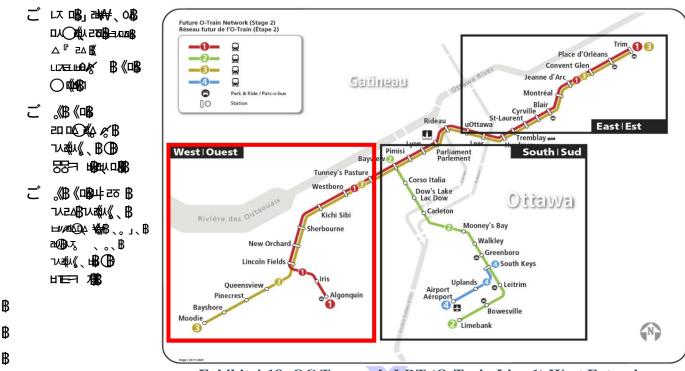


Exhibit 4-18: OC Transpo's LRT (O-Train Line 1) West Extension

4.1.3.2 Other Study Area Developments

249-255 Richmond Road & 372 Tweedsmuir Avenue:

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319-327 Richmond Road, 380 Winona Avenue & 381 Churchill Avenue

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398-406 Roosevelt Avenue

403 Richmond Road

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397-399 Richmond Road

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5.0 FORECASTING

5.1 DEVELOPMENT GENERATED TRAVEL DEMAND

5.1.1 Trip Generation and Mode Shares

5.1.1.1 Trip Generation Rate

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Table 5-1: Person Trip Generation per Peak Period (TRANS 2020)B

ITE/TRANS Land Use	Size	Morning Peak Hour Rate	Afternoon Peak Hour Rate
		0.80	0.90
221&222 Multi-Unit (High-Rise)	58 units	Trips*B	Trips*B
		46	52

^{*}The calculated value for total trips in the subsequent table is higher due to rounding by model

5.1.1.2 Existing Dry-Cleaning Establishment

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5.1.1.3 Mode Shares

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5.1.1.4 Directional Split

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Table 5-2: Mode Shares High-Rise Multifamily Housing, Ottawa West (TRANS 2020)

Peak <u>Period</u> Mode Share Split (TRANS 2020 Table 8)							
Mode	Mode Share, AM	Mode Share, PM					
Auto Driver	28%	33%					
Auto Passenger	11%	11%					
Transit	41%	26%					
Cycling	3%	7%					
Walking	16%	23%					
Peak <u>Perio</u>	d Trips by Mode (Rounde	d Up)					
Mode	Trips, AM	Trips, PM					
Auto Driver	13	17					
Auto Passenger	5	6					
Transit	18	13					
Cycling	2	4					
Walking	7	12					
Total Person Trips	45	52					
Peak Period to Peak Hour	Trip Adjustment Factor (TRANS 2020 Table 4)					
Mode	AM	PM					
Auto Driver	0.48	0.44					
Auto Passenger	0.48	0.44					
Transit	0.55	0.47					
Cycling	0.58	0.48					
Walking	0.58	0.52					
Peak <u>Hou</u>	<u>r</u> Trips by Mode (Rounded	l Up)					
Mode	Trips, AM	Trips, PM					
Auto Driver	7	8					
Auto Passenger	3	3					
Transit	11	7					
Cycling	2	2					
Walking	5	7					
Total	28	27					

Table 5-3: Vehicle Directional Splits (TRANS 2020, Table 9)

Peak Hour Vehicle Directional Split (TRANS 2020 Table 9)							
Total Vehicles AM Peak PM Peak							
Direction	In	Out	In	Out			
Directional Split	31%	69%	58%	42%			
New Vehicle Trips (peak hour)	2	5	5	3			

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5.1.1.5 Future Mode Shares

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Travel Mode	Mode Share Target	Rationale
Transit	30-45%	With the advent of LRT stage 2 and the site being within an 800-metre walking distance to Westboro station, the transit mode share is expected to remain high or increase
Walking	15-25%	Good pedestrian and cycling infrastructure is present in the area. Richmond Road houses commercial,
Cycling	5%	services and employment sectors within walking/cycling distance from the development
Auto Passenger	10-15%	Auto passenger mode share is assumed to remain between 10% and 15%. The upper limit of 15% is assuming a 1.15 vehicle occupancy rate
Auto-driver	20-35%	With the advent of LRT stage 2, as well as intensification and active transportation improvements in the area, the auto driver mode is anticipated to remain low or decrease

Table 5-4: Future Mode Share Targets

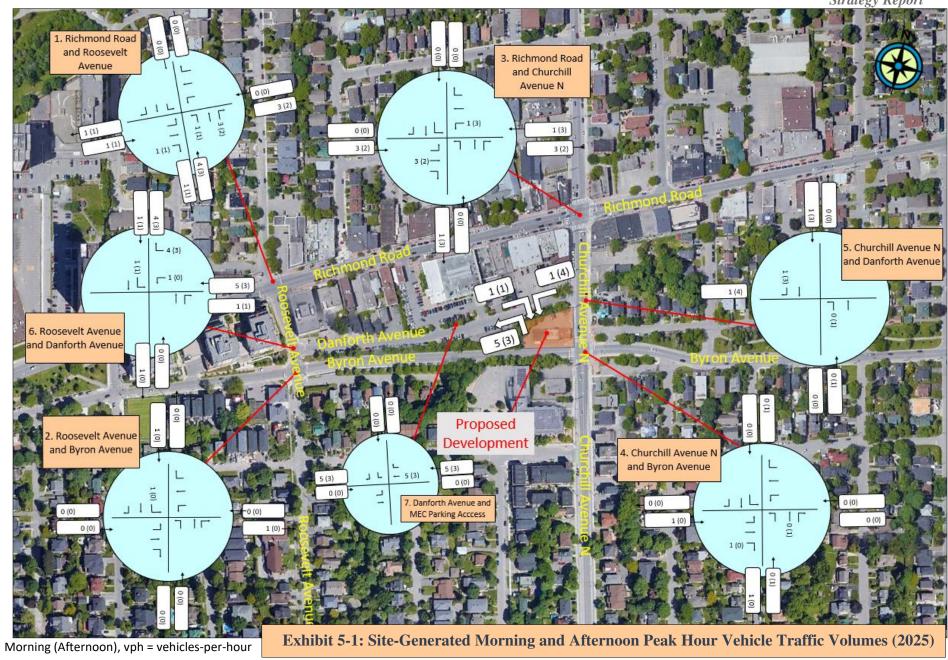
5.1.2 Trip Distribution

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5.1.3 Trip Assignment

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5.2 BACKGROUND NETWORK TRAVEL DEMAND

5.2.1 Transportation Network Plans

5.2.2 Background Growth

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5.2.3 Other Developments

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319-327 Richmond Road, 380 Winona Avenue & 381 Churchill Avenue

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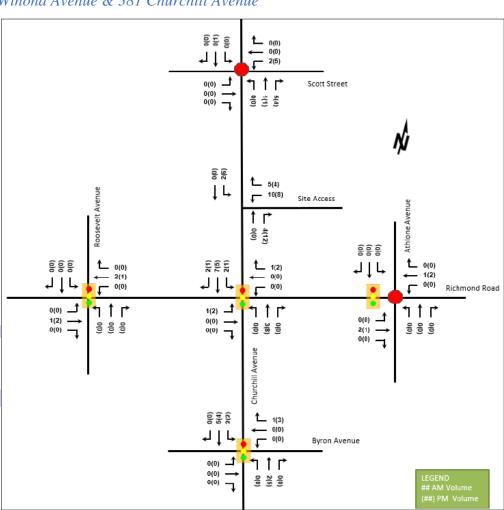


Exhibit 5-2: 319-327 Richmond Road Traffic Generation

335 Roosevelt Avenue

2050 Scott Street

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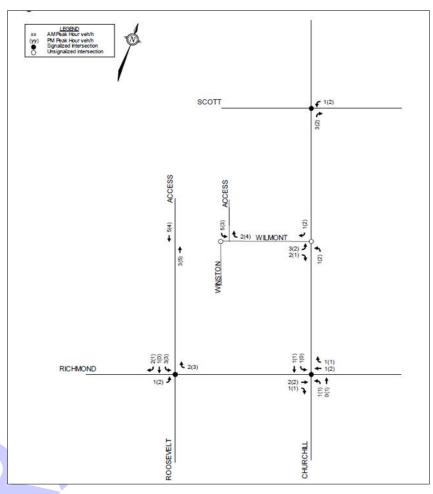


Exhibit 5-3: 335 Roosevelt Avenue Traffic Generation

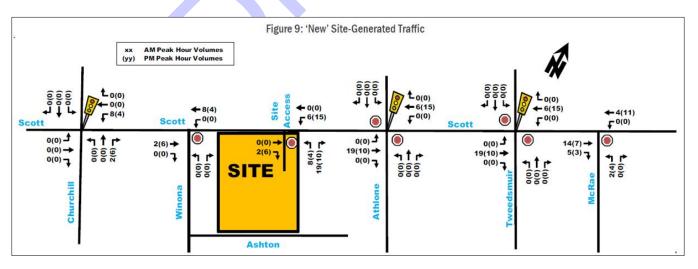


Exhibit 5-4: 2050 Scott Street Traffic Generation

398 Roosevelt Avenue

403 Richmond Road

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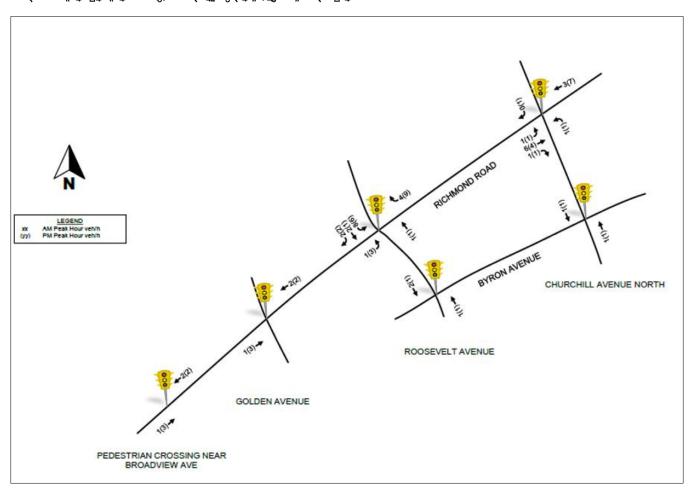


Exhibit 5-5: 403 Richmond Road Traffic Generation

397-399 Richmond Road

2070 Scott Street

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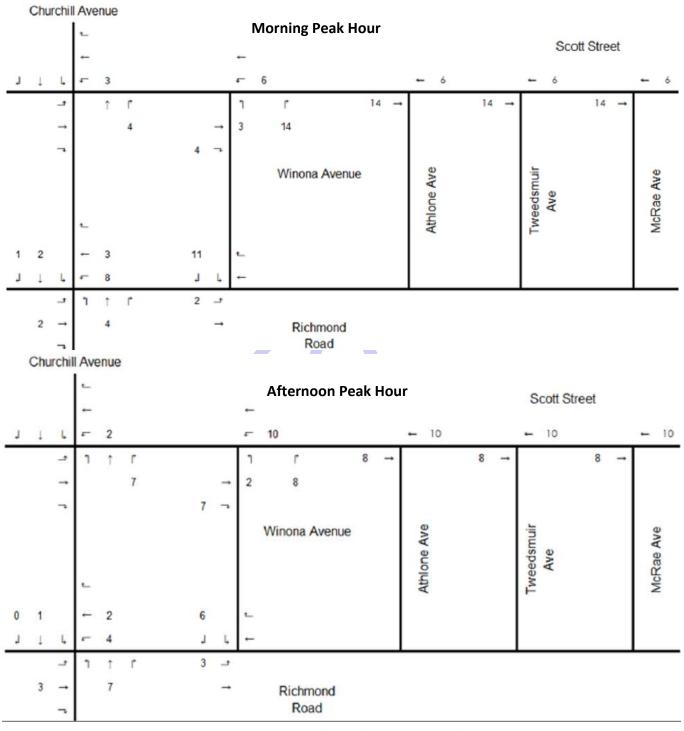


Exhibit 5-6: 2070 Scott Street Traffic Generation

5.3 DEMAND RATIONALIZATION

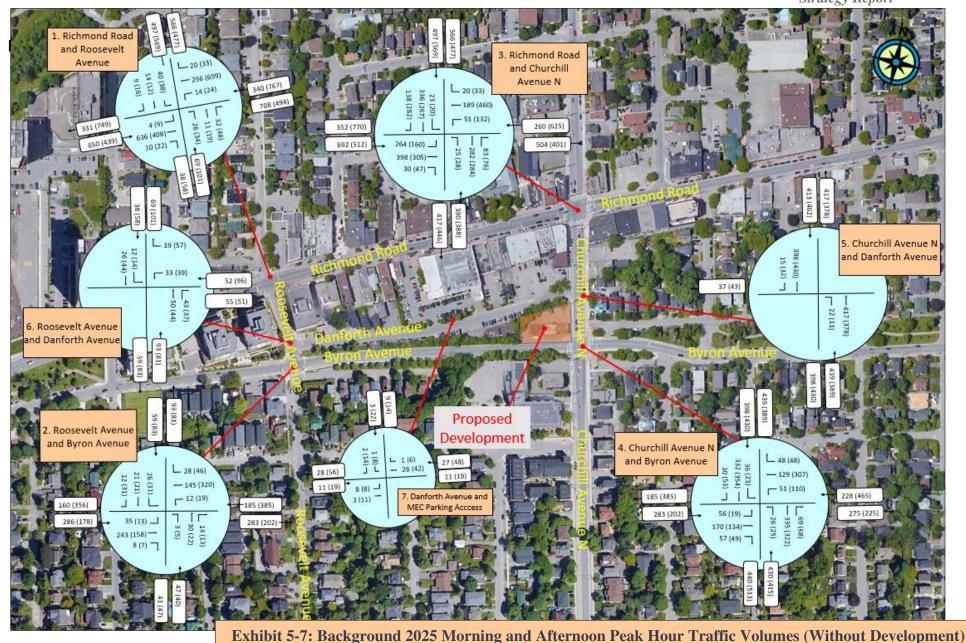
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5.3.1 Background Traffic Forecasts

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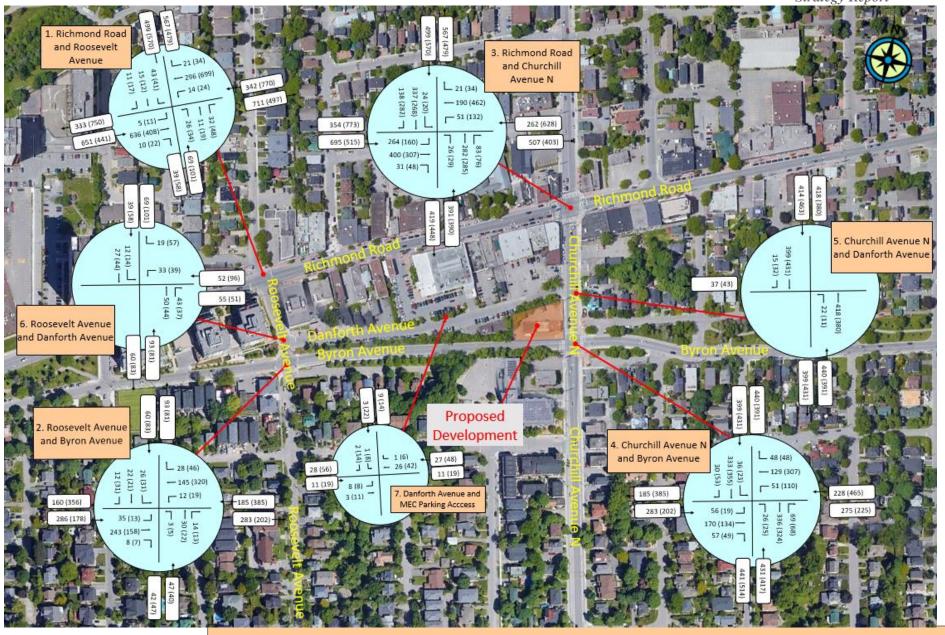
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Morning (Afternoon), vph = vehicles-per-hour

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Morning (Afternoon), vph = vehicles-per-hour

Exhibit 5-8: Background 2030 Morning and Afternoon Peak Hour Traffic Volumes (Without Development)

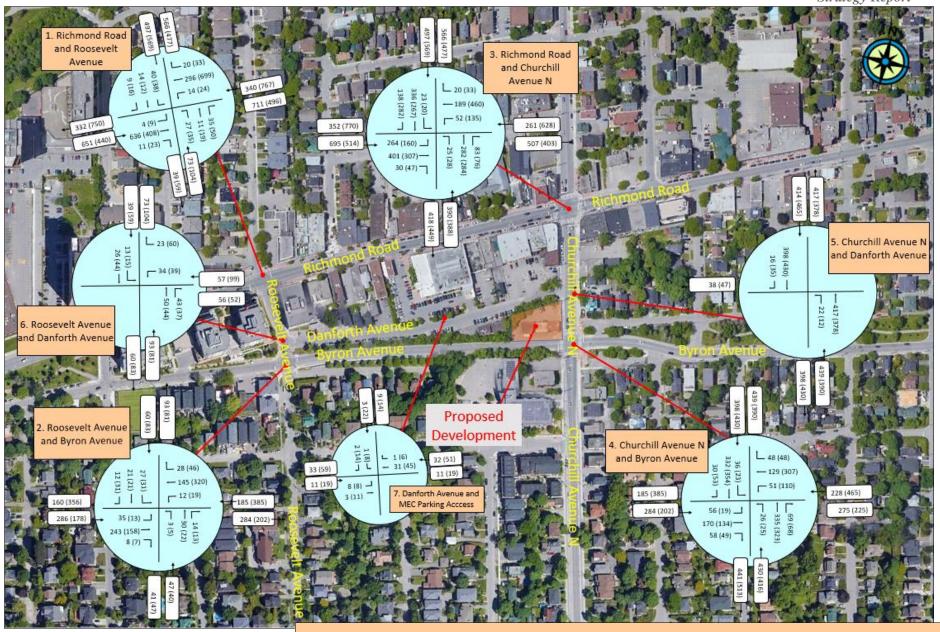
5.3.2 Total Traffic Forecasts

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Development-Generated Traffic Impacts

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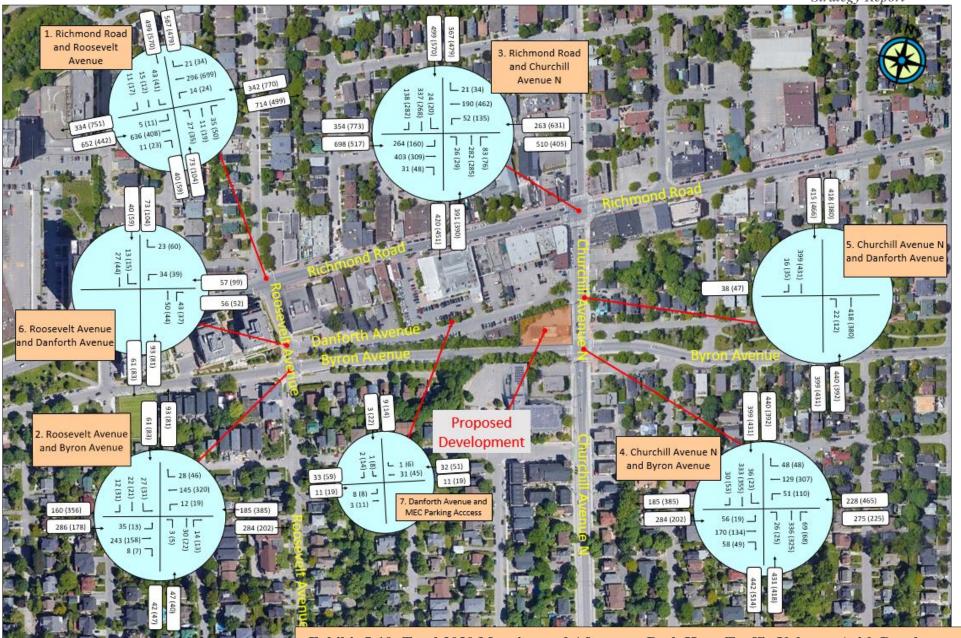
Cut-Through Traffic Impacts



Morning (Afternoon), vph = vehicles-per-hour

Exhibit 5-9: Total 2025 Morning and Afternoon Peak Hour Traffic Volumes (with Development)

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Morning (Afternoon), vph = vehicles-per-hour

Exhibit 5-10: Total 2030 Morning and Afternoon Peak Hour Traffic Volumes (with Development)

6.0 STRATEGY

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6.1 DEVELOPMENT DESIGN

6.1.1 Design for Sustainable Modes

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6.1.2 Circulation and Access

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6.2 PARKING

6.2.1 Motor Vehicle Parking

」《、BHB-118-1-12* □2%(BB-1817》)(BB-25* 、。、BT-45%(BB-1817)(《BT-62 □20》 \$4486—3148(BB-1812)(BB-182)(BB-1812)(BB-

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- Visitor Parking 电影上型 AL OB 《B·UB·U SB OB FI 《 OB PER OB PROBLEM COB SB OB TO SB
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^{15.} See section 4.1.2.6 Exhibit 4-13B

^{16.} City of Ottawa By-Law 2016-249, Section 101, Clause (3) (a)

^{17.} City of Ottawa By-Law 2016-249, Table 101, Row 12, Dwelling, Mid-High Rise Apartment, Area "X" on Schedule 1A

^{18.} City of Ottawa By-Law 2016-249, Section 102, Clause (2)

^{19.} City of Ottawa By-Law 2016-249, Table 102, Row 1, Area "X" on Schedule 1A

^{20.} Proposed Development is located in Area B (Inner Urban) on Schedule 1 of the by-law

^{21.} City of Ottawa By-Law 2016-249, Table 103, Row 1, Area B on Schedule 1 – Inner Urban

6.2.2 Bicycle Parking

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Table 6-1: Auto Parking Provisions Summary

Land Use	Development Size	Reduced Development Size	City Parking Requirement Rate	City Parking Requirement	Parking Provisions (Underground Stalls)	
Residential Dwellings, Mid- high-Rise Apartment - Residents	58 dwelling	58-12 = 46	0.5 per dwelling unit	23 stalls	25 stalls	
Residential Dwellings, Mid- high-Rise Apartment - Visitors	units	dwelling units	0.1 per dwelling unit	5 stalls	5 stalls	
	Min: 28 stalls Max: 101 stalls	30 stalls				

₿

Table 6-2: Bicycle Parking Provisions Summary

I and II a	City	Parking I	Parking Provisions			
Land Use	Requirement	Horizontal Stalls	Vertical Stalls			
Residential Apartments	29 stalls	28 stalls	22 stalls			
Total	29 stalls	50 s	talls			

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6.3 BOUNDARY STREET DESIGN

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²² City of Ottawa By-Law 2016-249, Section 111, Table 11A, (b) and (g)

²³ Multi-Modal Level of Service (MMLOS) Guidelines, IBI Group, September 2015

²⁴ Document 5: Addendum to the City's Multi-Modal Level of Service Guidelines, December 2016

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Exhibit 6-1: Boundary Street Segments for MMLOS Analysis

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Table 6-3: Segment MMLOS Analysis Results

Loc	ation		Level of Service and Targets										
Roadway Segment	Direction	Direction Policy Area/ Land Use Designation		OS T BLOS		Target BLOS	TLOS	Target TLOS	TkLOS	Target TkLOS			
Churchill Ave N b/w	NB		В	Α	D	В	E	D	С	D			
Richmond and Byron	SB		В	Α	D	В	E	D	С	D			
Churchill Ave N b/w	NB		С	Α	Α	В	D	D	С	D			
Byron and Ravenhill	SB		В	Α	Α	В	D	D	В	D			
Byron Avenue b/w	EB	Within 300	В	А	В	С	N/A	N/A	В	D			
Roosevelt and Churchill	WB	m of a school	F	А	D	С	N/A	N/A	В	D			
Byron Avenue b/w	EB		В	Α	Α	С	N/A	N/A	N/A	N/A			
Churchill and Athlone	WB		С	A	D (B)*	С	N/A	N/A	N/A	N/A			
Danforth Avenue	EB		F	Α	В	D	N/A	N/A	N/A	N/A			
Daniorth Avenue	WB		F	Α	В	D	N/A	N/A	N/A	N/A			

Note – Levels of Service highlighted in bold font fail to meet the respective target LOS

- Detailed segment MMLOS analysis calculations are provided within Appendix "K".
- TLOS analysis was not performed on segments without existing transit service
- TkLOS was not performed on Byron Avenue segment between Churchill and Athlone as it has prohibitive truck signage; and Danforth Avenue since it's classified as a local street
- * A complete streets concept has been developed for this segment, which includes a WB bike lane. BLOS "B" is expected after implementation

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Pedestrian Level of Service (PLOS):

В

Bicycle Level of Service (BLOS):

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Transit Level of Service (TLOS):

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Truck Level of Service (TkLOS):

6.3.1 Access Intersection Design

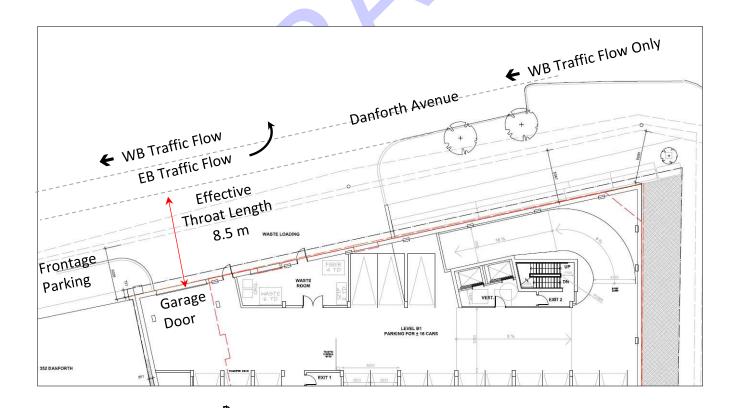
6.3.2 Access Control

424 Churchill Avenue Residential Apartments Development®

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6.3.3 Access Design

6.3.4 Location and Design Characteristics of Proposed Accesses



²⁵ Multi-Modal Level of Service (MMLOS) Guidelines, IBI Group, September 2015. Page 4

26 See section**職**

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7.0 CONCLUSION

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Yours truly,

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Mr. Arthur Gordon B.A. P.Eng

B Principal Engineer

Castleglenn Consultants Inc.

Mr. Andrey Kirillov B.Eng, EIT Transportation Planner

Castleglenn Consultants Inc.B



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APPENDIX A: CERTIFICATION FORM FOR TIA STUDY PROJECT MANAGER

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Certification Form for TIA Study PM

TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

/	the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
~	I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
v	I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
✓	I am either a licensed¹ or registered² professional in good standing, whose field of expertise is either transportation engineering or transportation planning.

^{1,2} License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

67

City Of Ottawa Infrastructure Services and Community Sustainability Planning and Growth Management 110 Laurier Avenue West, 4th fl. Ottawa, ON K1P 1J1

Tel.: 613-580-2424 Fax: 613-560-6006

Dated at	Ottawa		this 26th	day of July		, 20 22
	(0	City)				
Name :	Arthur E.	Gordon				
Profession	onal title: P	rincipal				
	(John .				
Signatur	re of individu	al certifier that s	s/he meets the a	bove criteria		



Stamp





APPENDIX B: SCREENING FORM



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City of Ottawa 2017 TIA Guidelines Screening Form

IIII. Description of Proposed Development

Municipal Address	424 Churchill Avenue
Description of Location	ਥ⊐storey residential building with 58 units
Land Use Classification	TM H(24) - Traditional Mainstreet
Development Size (units)	58 units
Development Size (m²)	N/A
Number of Accesses and Locations	1 Access off Danforth Avenue
Phase of Development	1
Buildout Year	2025

If available, please attach a sketch of the development or site plan to this form.

Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
Townhomes or apartments	90 units
Office	3,500 m ²
Industrial	5,000 m ²
Fast-food restaurant or coffee shop	100 m²
Destination retail	1,000 m ²
Gas station or convenience market	75 m²

^{*} If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, the Trip Generation Trigger is satisfied.

BB. Location Triggers

B Commence of the commence of	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	В	
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		В

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

■ Safety Triggers

5. Summary

Does the development satisfy the Safety Trigger?

В	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?	В	\mathbb{B}
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	В	B
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/suburban conditions)?	B	В
Is the proposed driveway within auxiliary lanes of an intersection?	₿	
Does the proposed driveway make use of an existing median break that serves an existing site?	В	B
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	В	B
Does the development include a drive-thru facility?	₿	B

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

B Does the development satisfy the Trip Generation Trigger? Does the development satisfy the Location Trigger? B B

If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, <u>the TIA Study must continue into the next stage</u> (Screening and Scoping).

72 Revision Date: October, 2020

^{*}DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).



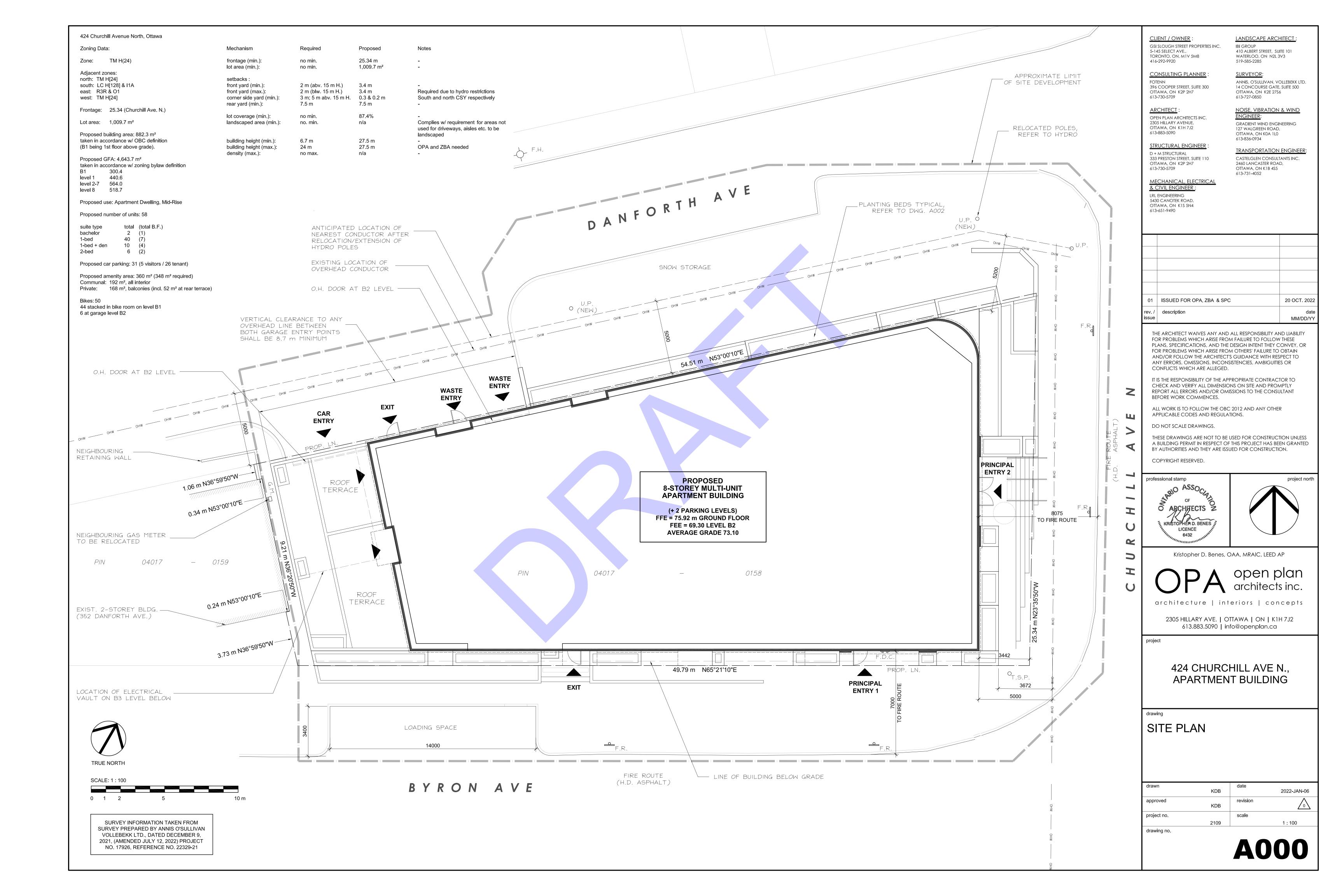
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APPENDIX C: SITE PLAN







APPENDIX D: EXISTING TRAFFIC COUNTS, SIGNAL TIMINGS AND COLLISION DATA

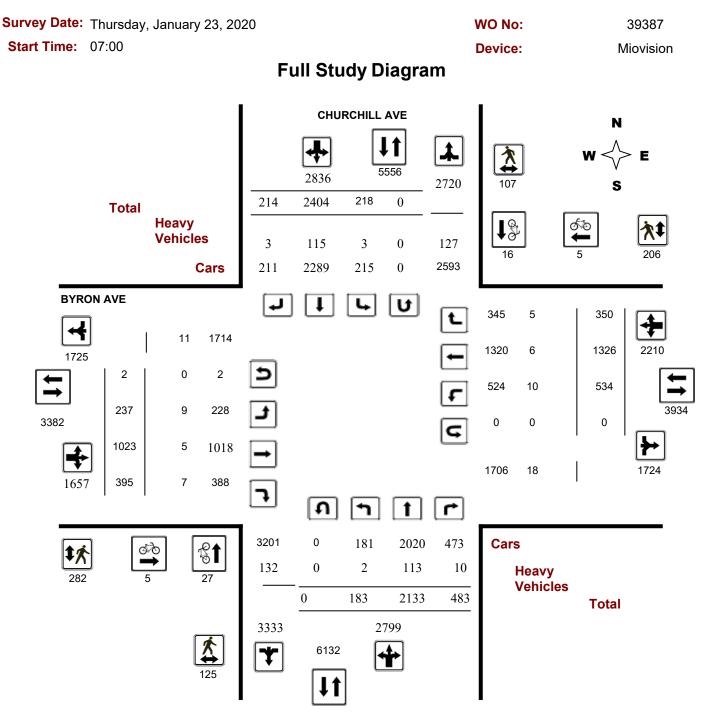
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Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE



5472205 - THU JAN 23, 2020 - 8HRS - LORETTA

May 21, 2020 Page 1 of 8



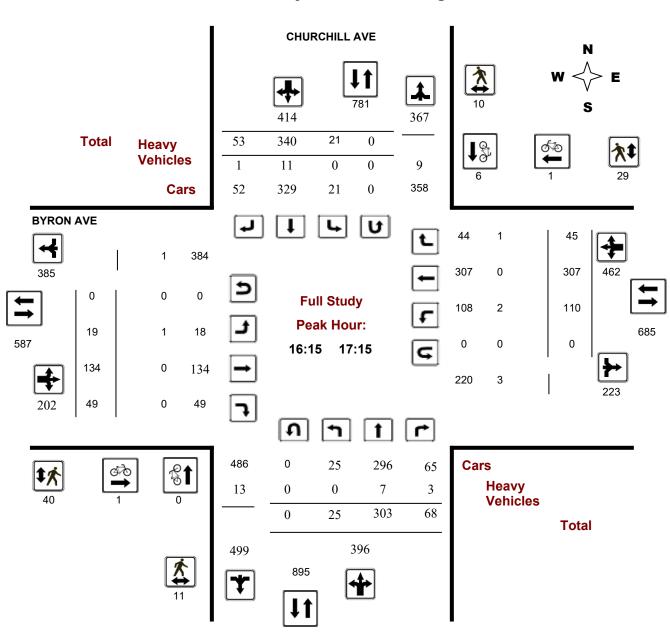
Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



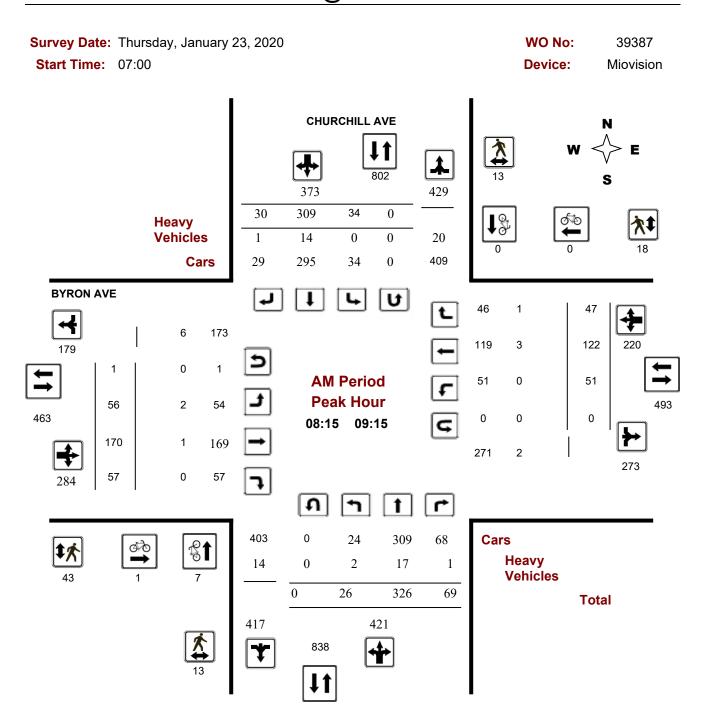
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May 21, 2020 Page 2 of 8



Turning Movement Count - Peak Hour Diagram

BYRON AVE @ CHURCHILL AVE



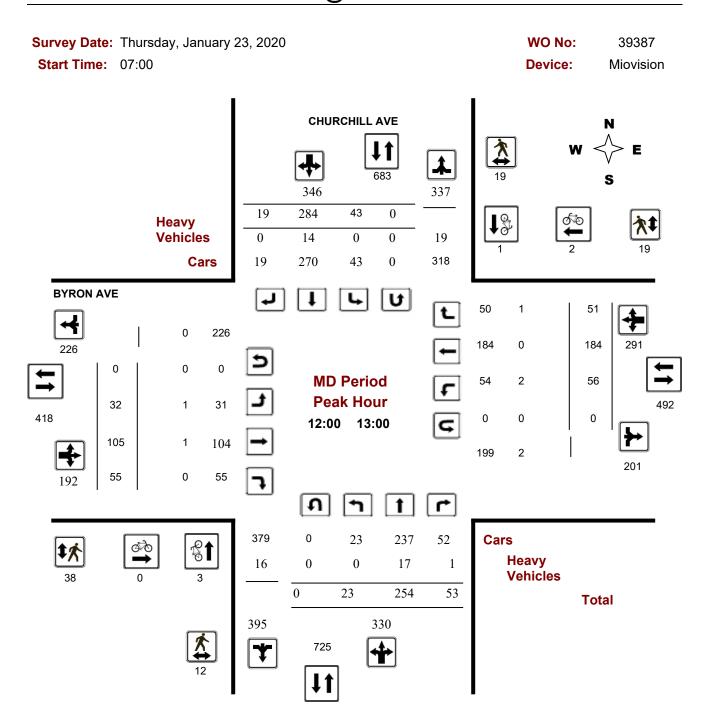
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2020-May-21 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

BYRON AVE @ CHURCHILL AVE



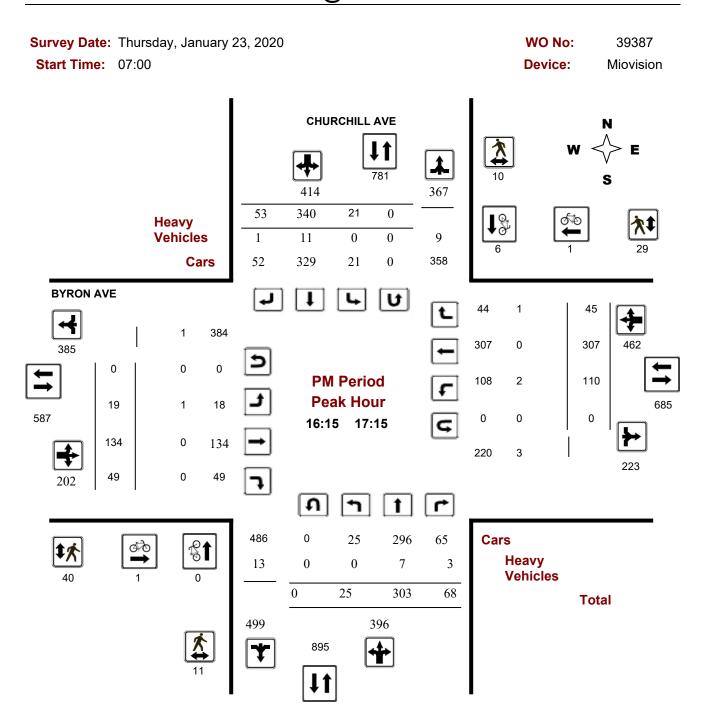
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2020-May-21 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

BYRON AVE @ CHURCHILL AVE



Comments 5472205 - THU JAN 23, 2020 - 8HRS - LORETTA

2020-May-21 Page 3 of 3



Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, January 23, 2020 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0

1.00

Eastbound: 2 Westbound: 0

	CHURCHILL AVE									BYRON AVE									
	No	rthbou	nd		So	uthbou	ınd	<u></u>		Е	astbou	ınd		V	√estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	12	215	42	269	18	255	3	276	545	20	89	34	143	42	70	22	134	277	822
08:00 09:00	25	325	62	412	32	293	25	350	762	53	165	53	271	50	108	43	201	472	1234
09:00 10:00	23	273	65	361	19	257	25	301	662	36	125	51	212	32	99	48	179	391	1053
11:30 12:30	25	240	73	338	40	275	17	332	670	24	126	46	196	58	153	59	270	466	1136
12:30 13:30	23	240	49	312	39	284	26	349	661	27	105	48	180	56	192	39	287	467	1128
15:00 16:00	23	257	52	332	25	373	30	428	760	24	150	70	244	85	213	46	344	588	1348
16:00 17:00	22	293	72	387	22	346	50	418	805	25	122	46	193	111	280	49	440	633	1438
17:00 18:00	30	290	68	388	23	321	38	382	770	28	141	47	216	100	211	44	355	571	1341
Sub Total	183	2133	483	2799	218	2404	214	2836	5635	237	1023	395	1655	534	1326	350	2210	3865	9500
U Turns				0				0	0				2				0	2	2
Total	183	2133	483	2799	218	2404	214	2836	5635	237	1023	395	1657	534	1326	350	2210	3867	9502
EQ 12Hr	254	2965	671	3891	303	3342	297	3942	7833	329	1422	549	2303	742	1843	486	3072	5375	13208
Note: These \	/alues a	re calcu	lated by	/ multiply	ying the	totals b	y the a _l	opropriate	e expans	ion fact	or.			1.39					
AVG 12Hr	240	2794	633	3667	286	3149	280	3715	7833	310	1340	517	2171	700	1737	458	2895	5375	13208
Note: These \	/olumes	are calc	culated	by multi	plying t	he Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1					
AVG 24Hr	314	3660	829	4803	374	4126	367	4867	9670	407	1756	678	2844	916	2276	601	3793	6637	16307
Note: These \	olumes/	are calc	culated	by multi	plying t	he Avera	ige Dai	ly 12 hr. 1	totals by	12 to 2	4 expans	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

May 21, 2020 Page 3 of 8



Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

CHURCHILL AVE BYRON AVE

		No	orthbou	ınd		Sc	outhbou	nd			Е	astbour	nd		We	estboun	ıd			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	2	40	10	52	4	53	0	57	4	1	15	7	23	5	3	7	15	4	147
07:15	07:30	0	32	7	39	5	56	1	62	4	4	31	10	45	6	20	1	27	4	173
07:30	07:45	6	62	11	79	3	64	2	69	10	3	17	12	32	17	15	6	38	10	218
07:45	08:00	4	81	14	99	6	82	0	88	7	12	26	5	43	14	32	8	54	7	284
08:00	08:15	7	80	9	96	5	66	6	77	7	8	38	8	54	10	12	9	31	7	258
08:15	08:30	6	89	14	109	16	83	3	102	7	13	38	15	66	10	17	15	42	7	319
08:30	08:45	5	85	22	112	8	67	11	86	8	20	45	14	79	15	31	9	55	8	332
08:45	09:00	7	71	17	95	3	77	5	85	8	12	44	16	73	15	48	10	73	8	326
09:00	09:15	8	81	16	105	7	82	11	100	12	11	43	12	66	11	26	13	50	12	321
09:15	09:30	2	62	15	79	5	69	5	79	16	14	37	13	64	8	24	12	44	16	266
09:30	09:45	4	71	14	89	4	53	7	64	12	4	18	16	38	4	19	9	32	12	223
09:45	10:00	9	59	20	88	3	53	2	58	10	7	27	10	44	9	30	14	53	10	243
11:30	11:45	6	54	27	87	6	75	3	84	19	2	37	9	48	11	24	14	49	19	268
11:45	12:00	7	71	20	98	9	56	7	72	13	6	37	9	52	17	42	14	73	13	295
12:00	12:15	6	62	15	83	14	77	2	93	12	9	26	13	48	15	43	16	74	12	298
12:15	12:30	6	53	11	70	11	67	5	83	5	7	26	15	48	15	44	15	74	5	275
12:30	12:45	5	68	11	84	11	74	4	89	10	7	28	14	49	13	34	8	55	10	277
12:45	13:00	6	71	16	93	7	66	8	81	5	9	25	13	47	13	63	12	88	5	309
13:00	13:15	7	52	11	70	6	77	10	93	9	5	28	11	44	17	45	8	70	9	277
13:15	13:30	5	49	11	65	15	67	4	86	11	6	24	10	40	13	50	11	74	11	265
15:00	15:15	5	65	11	81	7	103	4	114	9	5	45	23	73	18	47	12	77	9	345
15:15	15:30	5	64	10	79	5	99	8	112	5	8	50	17	76	21	55	14	90	5	357
15:30	15:45	7	60	18	85	9	81	7	97	3	5	25	14	44	18	50	9	77	3	303
15:45	16:00	6	68	13	87	4	90	11	105	3	6	30	16	52	28	61	11	100	3	344
16:00	16:15	10	71	25	106	4	91	5	100	5	7	30	11	48	25	53	13	91	5	345
16:15	16:30	7	82	17	106	6	73	18	97	8	7	33	16	56	30	78	9	117	8	376
16:30	16:45	3	73	14	90	6	93	13	112	7	4	23	10	37	25	77	13	115	7	354
16:45	17:00	2	67	16	85	6	89	14	109	5	7	36	9	52	31	72	14	117	5	363
17:00	17:15	13	81	21	115	3	85	8	96	2	1	42	14	57	24	80	9	113	2	381
17:15	17:30	5	76	14	95	4	86	10	100	4	10	35	9	54	28	48	7	83	4	332
17:30	17:45	7	63	16	86	8	80	10	98	5	8	36	13	57	18	52	13	83	5	324
17:45	18:00	5	70	17	92	8	70	10	88	1	9	28	11	48	30	31	15	76	1	304
Total:		183	2133	483	2799	218	2404	214	2836	246	237	1023	395	1657	534	1326	350	2210	246	9,502

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

CHURCHILL AVE BYRON AVE

	•	SHUKCHILL AV	_		DIRONAVE	•	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	1	0	1	0	0	0	1
07:15 07:30	1	1	2	0	0	0	2
07:30 07:45	1	0	1	1	1	2	3
07:45 08:00	4	0	4	0	0	0	4
08:00 08:15	6	0	6	0	0	0	6
08:15 08:30	4	0	4	0	0	0	4
08:30 08:45	1	0	1	1	0	1	2
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	2	0	2	0	0	0	2
09:15 09:30	0	0	0	1	0	1	1
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	1	0	1	0	0	0	1
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	1	0	1	0	0	0	1
12:00 12:15	2	1	3	0	0	0	3
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	1	1	1
12:45 13:00	1	0	1	0	1	1	2
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	1	1	1	0	1	2
16:00 16:15	0	1	1	0	0	0	1
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	1	1	0	0	0	1
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	5	5	1	1	2	7
17:15 17:30	0	3	3	0	0	0	3
17:30 17:45	0	3	3	0	1	1	4
17:45 18:00	2	0	2	0	0	0	2
Total	27	16	43	5	5	10	53

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Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

CHURCHILL AVE BYRON AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	0	0	2	1	3	3
07:15 07:30	6	0	6	1	6	7	13
07:30 07:45	5	3	8	7	3	10	18
07:45 08:00	11	5	16	22	5	27	43
08:00 08:15	18	6	24	18	6	24	48
08:15 08:30	2	5	7	12	2	14	21
08:30 08:45	6	0	6	11	6	17	23
08:45 09:00	2	3	5	13	4	17	22
09:00 09:15	3	5	8	7	6	13	21
09:15 09:30	2	4	6	3	7	10	16
09:30 09:45	2	2	4	3	3	6	10
09:45 10:00	6	6	12	7	7	14	26
11:30 11:45	3	2	5	6	1	7	12
11:45 12:00	4	2	6	10	7	17	23
12:00 12:15	4	9	13	8	2	10	23
12:15 12:30	3	5	8	10	4	14	22
12:30 12:45	3	1	4	13	3	16	20
12:45 13:00	2	4	6	7	10	17	23
13:00 13:15	2	1	3	6	7	13	16
13:15 13:30	3	2	5	4	6	10	15
15:00 15:15	2	2	4	8	10	18	22
15:15 15:30	2	5	7	13	6	19	26
15:30 15:45	4	10	14	13	17	30	44
15:45 16:00	4	4	8	7	7	14	22
16:00 16:15	5	3	8	10	7	17	25
16:15 16:30	2	4	6	18	10	28	34
16:30 16:45	2	1	3	6	5	11	14
16:45 17:00	4	3	7	11	11	22	29
17:00 17:15	3	2	5	5	3	8	13
17:15 17:30	8	2	10	8	13	21	31
17:30 17:45	1	2	3	9	10	19	22
17:45 18:00	1	4	5	4	11	15	20
Total	125	107	232	282	206	488	720

5472205 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

CHURCHILL AVE BYRON AVE

		No	orthbou	und		Sc	uthbou	nd			Е	astbour	nd		We	estbour	nd			
Time Peri	iod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07	7:15	0	2	1	3	0	1	0	1	4	0	0	1	1	0	0	0	0	1	5
07:15 07	7:30	0	2	0	2	0	2	0	2	4	1	0	0	1	0	1	0	1	2	6
07:30 07	7:45	0	6	0	6	0	4	0	4	10	1	0	1	2	3	0	0	3	5	15
07:45 08	3:00	0	3	0	3	0	4	0	4	7	0	0	1	1	0	0	0	0	1	8
08:00 08	3:15	0	6	0	6	0	1	0	1	7	0	0	0	0	0	0	0	0	0	7
08:15 08	3:30	1	2	0	3	0	4	0	4	7	0	0	0	0	0	1	0	1	1	8
08:30 08	3:45	0	7	0	7	0	1	0	1	8	2	0	0	2	0	0	0	0	2	10
08:45 09	9:00	1	4	1	6	0	1	1	2	8	0	0	0	0	0	0	1	1	1	9
09:00 09	9:15	0	4	0	4	0	8	0	8	12	0	1	0	1	0	2	0	2	3	15
09:15 09	9:30	0	8	0	8	0	8	0	8	16	0	0	0	0	0	0	0	0	0	16
09:30 09	9:45	0	10	0	10	0	2	0	2	12	0	0	0	0	0	0	1	1	1	13
09:45 10	0:00	0	5	0	5	0	5	0	5	10	1	0	1	2	0	0	0	0	2	12
11:30 11	1:45	0	5	2	7	0	12	0	12	19	0	0	0	0	0	0	0	0	0	19
11:45 12	2:00	0	10	0	10	0	3	0	3	13	0	0	1	1	0	1	0	1	2	15
12:00 12	2:15	0	6	0	6	0	6	0	6	12	0	0	0	0	0	0	0	0	0	12
12:15 12	2:30	0	0	1	1	0	4	0	4	5	1	1	0	2	0	0	0	0	2	7
12:30 12	2:45	0	7	0	7	0	3	0	3	10	0	0	0	0	2	0	0	2	2	12
12:45 13	3:00	0	4	0	4	0	1	0	1	5	0	0	0	0	0	0	1	1	1	6
13:00 13	3:15	0	4	0	4	1	4	0	5	9	0	0	0	0	0	0	0	0	0	9
13:15 13	3:30	0	0	0	0	1	10	0	11	11	0	1	0	1	0	0	1	1	2	13
15:00 15	5:15	0	3	0	3	1	5	0	6	9	1	1	0	2	1	0	0	1	3	12
15:15 15	5:30	0	1	0	1	0	4	0	4	5	0	1	0	1	0	0	0	0	1	6
15:30 15	5:45	0	0	0	0	0	2	1	3	3	0	0	0	0	0	0	0	0	0	3
15:45 16	3:00	0	1	1	2	0	1	0	1	3	0	0	1	1	0	0	0	0	1	4
	3:15	0	2	1	3	0	2	0	2	5	1	0	1	2	2	0	0	2	4	9
	3:30	0	3	1	4	0	3	1	4	8	1	0	0	1	1	0	0	1	2	10
16:30 16	6:45	0	1	1	2	0	5	0	5	7	0	0	0	0	1	0	0	1	1	8
16:45 17	7:00	0	1	1	2	0	3	0	3	5	0	0	0	0	0	0	1	1	1	6
17:00 17	7:15	0	2	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
17:15 17	7:30	0	2	0	2	0	2	0	2	4	0	0	0	0	0	1	0	1	1	5
17:30 17	7:45	0	2	0	2	0	3	0	3	5	0	0	0	0	0	0	0	0	0	5
17:45 18	3:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
Total: No	one	2	113	10	125	3	115	3	121	246	9	5	7	21	10	6	5	21	42	288

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Turning Movement Count - Study Results

BYRON AVE @ CHURCHILL AVE

Survey Date: Thursday, January 23, 2020 WO No: 39387

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total CHURCHILL AVE BYRON AVE

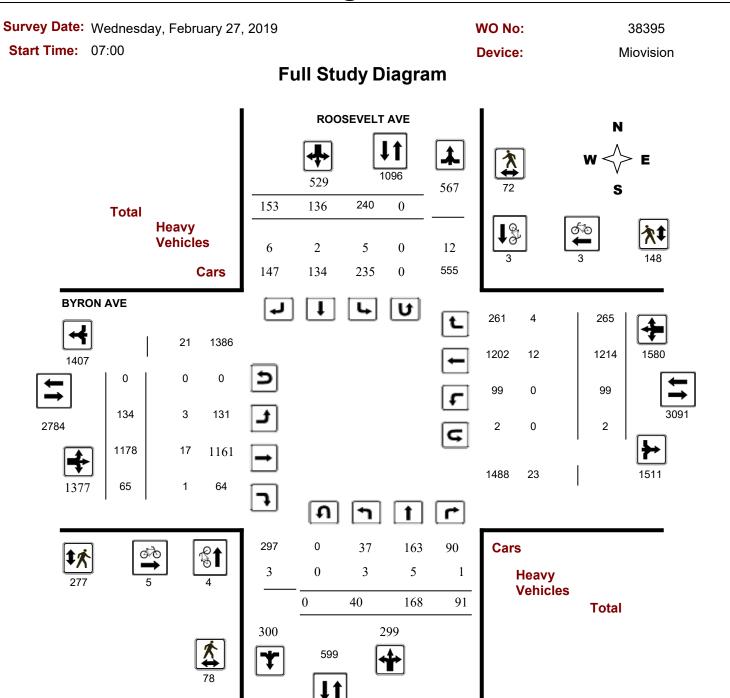
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	1	0	1
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	1	0	1
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	0	2	0	2

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE



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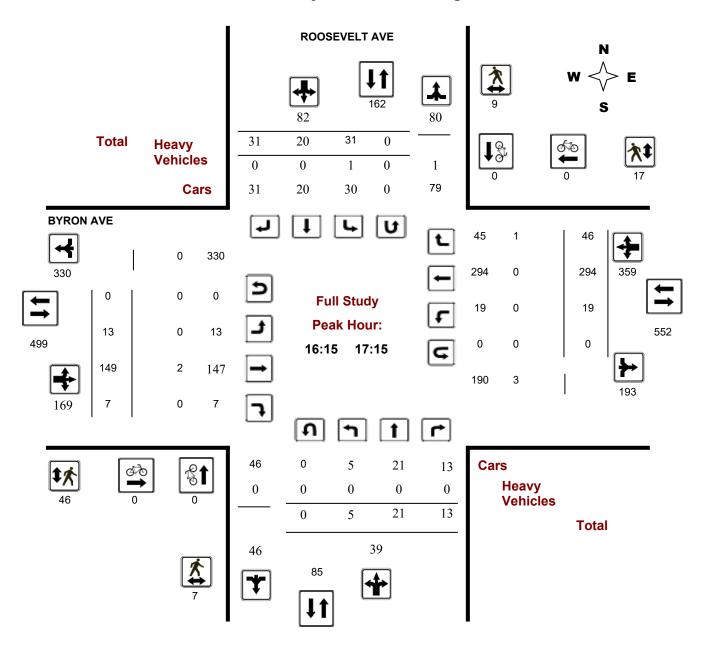
Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram

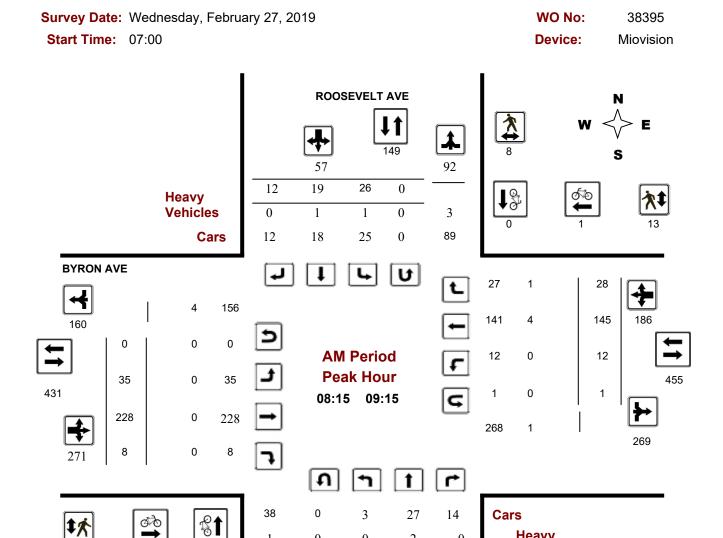


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Turning Movement Count - Peak Hour Diagram

BYRON AVE @ ROOSEVELT AVE



0

3

85

2

29

46

0

14

Heavy

Vehicles

Total



2020-Jun-22 Page 1 of 3

1

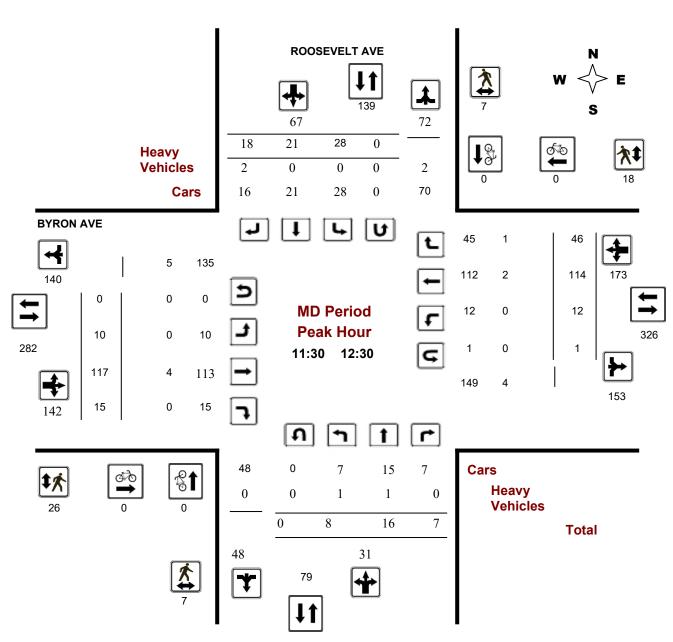
39



Turning Movement Count - Peak Hour Diagram

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395
Start Time: 07:00 Device: Miovision



Comments

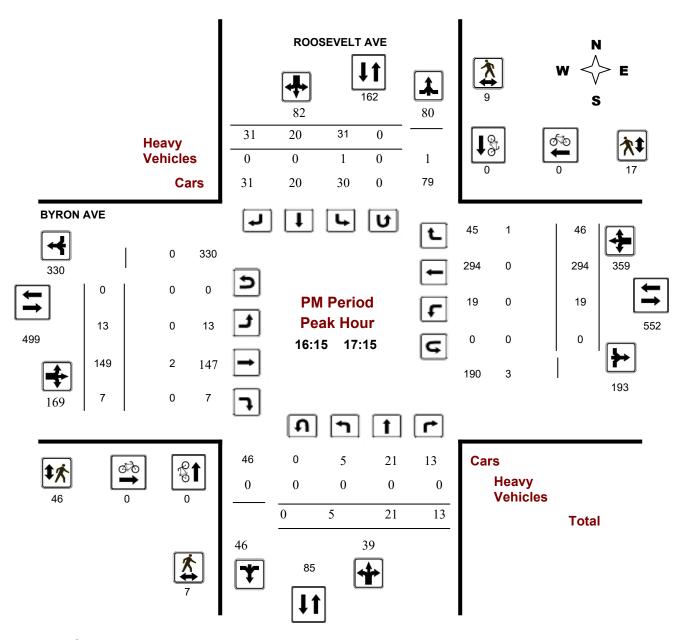
2020-Jun-22 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395
Start Time: 07:00 Device: Miovision



Comments

2020-Jun-22 Page 3 of 3



Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, February 27, Total Observed U-Turns AADT Factor

2019 Northbound: 0 Southbound: (

Eastbound: 0 Westbound: 2

		ı	ROOS	SEVEL ⁻	ΓAVE							ВУ	RON .	AVE					
	Nor	thbou	nd		So	uthbou	ınd			Е	astbou	ınd		V	/estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	3	19	17	39	14	9	7	30	69	9	161	7	177	5	54	17	76	253	322
08:00 09:00	3	32	17	52	27	15	12	54	106	28	247	2	277	10	138	28	176	453	559
09:00 10:00	0	25	13	38	21	15	12	48	86	28	152	12	192	12	104	30	146	338	424
11:30 12:30	8	16	7	31	28	21	18	67	98	10	117	15	142	12	114	46	172	314	412
12:30 13:30	11	18	6	35	37	20	24	81	116	17	101	4	122	13	111	30	154	276	392
15:00 16:00	8	14	9	31	38	19	22	79	110	16	133	5	154	12	189	31	232	386	496
16:00 17:00	6	23	9	38	30	21	29	80	118	12	137	12	161	14	253	47	314	475	593
17:00 18:00	1	21	13	35	45	16	29	90	125	14	130	8	152	21	251	36	308	460	585
Sub Total	40	168	91	299	240	136	153	529	828	134	1178	65	1377	99	1214	265	1578	2955	3783
U Turns				0				0	0				0				2	2	2
Total	40	168	91	299	240	136	153	529	828	134	1178	65	1377	99	1214	265	1580	2957	3785
EQ 12Hr	56	234	126	416	334	189	213	735	1151	186	1637	90	1914	138	1687	368	2196	4110	5261
Note: These v	alues ar	re calcu	lated by	/ multiply	ing the	totals b	y the ap	opropriate	e expans	ion fac	tor.			1.39					
AVG 12Hr	52	220	119	392	314	178	200	693	1151	176	1543	85	1804	130	1590	347	2070	4110	5261
Note: These v	olumes	are calc	culated	by multip	olying th	ne Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			1					
AVG 24Hr	69	288	156	513	412	233	263	908	1421	230	2022	112	2363	170	2083	455	2711	5074	6495
Note: These v	olumes	are calc	culated	by multip	olying th	ne Avera	ige Dail	y 12 hr. t	totals by	12 to 2	4 expans	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

ROOSEVELT AVE BYRON AVE

		No	orthbou	und		Sc	uthbou	nd			E	astbour	nd		W	estbour	nd			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	5	3	8	3	2	1	6	0	1	17	0	18	0	4	2	6	0	38
07:15	07:30	2	4	4	10	3	2	0	5	0	1	45	0	46	1	14	2	17	0	78
07:30	07:45	0	6	2	8	4	2	3	9	0	2	41	2	45	2	14	5	21	0	83
07:45	08:00	1	4	8	13	4	3	3	10	0	5	58	5	68	2	22	8	32	0	123
08:00	08:15	0	11	7	18	7	3	2	12	2	5	66	0	71	3	30	5	38	2	139
08:15	08:30	1	13	5	19	6	4	4	14	1	7	69	0	76	2	25	8	36	1	145
08:30	08:45	0	6	2	8	10	1	5	16	1	9	60	1	70	2	32	11	45	1	139
08:45	09:00	2	2	3	7	4	7	1	12	0	7	52	1	60	3	51	4	58	0	137
09:00	09:15	0	8	4	12	6	7	2	15	2	12	47	6	65	5	37	5	47	2	139
09:15	09:30	0	6	3	9	5	2	1	8	0	10	40	3	53	2	23	7	32	0	102
09:30	09:45	0	5	3	8	4	1	2	7	0	3	37	2	42	2	25	8	35	0	92
09:45	10:00	0	6	3	9	6	5	7	18	1	3	28	1	32	3	19	10	32	1	91
11:30	11:45	2	3	0	5	8	4	4	16	2	3	35	7	45	2	26	10	38	2	104
11:45	12:00	3	6	2	11	5	5	5	15	2	3	27	4	34	5	30	11	46	2	106
12:00	12:15	2	3	2	7	6	8	7	21	0	2	27	0	29	0	32	12	44	0	101
12:15	12:30	1	4	3	8	9	4	2	15	0	2	28	4	34	5	26	13	45	0	102
12:30	12:45	5	4	2	11	9	3	7	19	1	4	21	0	25	1	33	8	42	1	97
12:45	13:00	0	5	2	7	10	5	5	20	1	7	29	1	37	3	29	7	39	1	103
13:00	13:15	3	6	2	11	12	3	2	17	0	5	23	2	30	7	24	9	40	0	98
13:15	13:30	3	3	0	6	6	9	10	25	0	1	28	1	30	2	25	6	33	0	94
15:00	15:15	4	3	2	9	7	5	6	18	3	7	24	0	31	4	36	7	47	3	105
15:15	15:30	3	4	1	8	14	6	7	27	2	3	36	4	43	2	44	11	57	2	135
15:30	15:45	1	3	0	4	7	4	6	17	1	2	38	0	40	3	51	7	61	1	122
15:45	16:00	0	4	6	10	10	4	3	17	1	4	35	1	40	3	58	6	67	1	134
16:00	16:15	1	10	2	13	9	4	6	19	1	4	30	5	39	2	46	15	63	1	134
16:15	16:30	1	3	3	7	3	7	2	12	0	3	33	0	36	4	77	13	94	0	149
16:30	16:45	1	7	3	11	8	5	12	25	1	1	24	1	26	4	57	8	69	1	131
16:45	17:00	3	3	1	7	10	5	9	24	0	4	50	6	60	4	73	11	88	0	179
17:00	17:15	0	8	6	14	10	3	8	21	0	5	42	0	47	7	87	14	108	0	190
17:15	17:30	1	3	3	7	7	4	8	19	0	5	27	3	35	9	72	5	86	0	147
17:30	17:45	0	6	1	7	12	4	8	24	0	4	31	2	37	2	53	8	63	0	131
17:45	18:00	0	4	3	7	16	5	5	26	0	0	30	3	33	3	39	9	51	0	117
Total:		40	168	91	299	240	136	153	529	22	134	1178	65	1377	99	1214	265	1580	22	3,785

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 **WO No:** 38395

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

ROOSEVELT AVE BYRON AVE

		(OOOLVEEL A			DINONATE		<u></u>
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	1	0	1	1	0	1	2
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	1	0	1	0	0	0	1
08:00 08:15	1	0	1	2	0	2	3
08:15 08:30	0	0	0	1	0	1	1
08:30 08:45	1	0	1	0	0	0	1
08:45 09:00	0	0	0	1	1	2	2
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	2	2	2
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	1	1	0	0	0	1
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	1	1	0	0	0	1
17:45 18:00	0	1	1	0	0	0	1
Total	4	3	7	5	3	8	15

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

ROOSEVELT AVE BYRON AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	2	0	2	4	1	5	7
07:15 07:30	2	1	3	7	2	9	12
07:30 07:45	1	0	1	5	2	7	8
07:45 08:00	2	0	2	13	4	17	19
08:00 08:15	2	1	3	9	5	14	17
08:15 08:30	5	2	7	12	2	14	21
08:30 08:45	4	3	7	7	4	11	18
08:45 09:00	3	3	6	10	5	15	21
09:00 09:15	1	0	1	4	2	6	7
09:15 09:30	2	3	5	9	0	9	14
09:30 09:45	1	1	2	3	3	6	8
09:45 10:00	1	0	1	0	3	3	4
11:30 11:45	1	0	1	6	1	7	8
11:45 12:00	1	4	5	10	8	18	23
12:00 12:15	2	1	3	7	6	13	16
12:15 12:30	3	2	5	3	3	6	11
12:30 12:45	16	17	33	26	18	44	77
12:45 13:00	1	0	1	10	7	17	18
13:00 13:15	2	3	5	12	8	20	25
13:15 13:30	1	2	3	6	2	8	11
15:00 15:15	6	4	10	4	8	12	22
15:15 15:30	0	1	1	0	6	6	7
15:30 15:45	1	3	4	9	7	16	20
15:45 16:00	2	5	7	14	5	19	26
16:00 16:15	3	2	5	14	6	20	25
16:15 16:30	0	1	1	9	8	17	18
16:30 16:45	0	4	4	11	1	12	16
16:45 17:00	3	4	7	11	4	15	22
17:00 17:15	4	0	4	15	4	19	23
17:15 17:30	4	2	6	10	6	16	22
17:30 17:45	1	1	2	13	3	16	18
17:45 18:00	1	2	3	4	4	8	11
Total	78	72	150	277	148	425	575

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

ROOSEVELT AVE BYRON AVE

	N	orthbou	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
07:45 08:00	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
08:00 08:15	0	0	0	0	0	0	2	2	2	0	0	0	0	0	1	0	1	1	3
08:15 08:30	0	1	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
08:30 08:45	0	0	0	0	1	0	0	1	1	0	0	0	0	0	1	1	2	2	3
08:45 09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	2
09:00 09:15	0	1	0	1	0	1	0	1	2	0	0	0	0	0	1	0	1	1	3
09:15 09:30	0	0	0	0	0	0	0	0	0	1	1	0	2	0	0	0	0	2	2
09:30 09:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	1
11:30 11:45	0	1	0	1	0	0	1	1	2	0	2	0	2	0	1	0	1	3	5
11:45 12:00	1	0	0	1	0	0	1	1	2	0	2	0	2	0	0	0	0	2	4
12:00 12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
12:15 12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1
12:30 12:45	0	1	0	1	0	0	0	0	1	0	1	0	1	0	0	0	0	1	2
12:45 13:00	0	0	0	0	1	0	0	1	1	0	1	0	1	0	0	0	0	1	2
13:00 13:15	0	0	0	0	0	0	0	0	0	1	1	1	3	0	0	0	0	3	3
13:15 13:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2	2
15:00 15:15	2	0	0	2	1	0	0	1	3	1	2	0	3	0	0	1	1	4	7
15:15 15:30	0	0	0	0	0	0	2	2	2	0	0	0	0	0	2	0	2	2	4
15:30 15:45	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	0	0	0	1
15:45 16:00	0	0	1	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
16:00 16:15	0	1	0	1	0	0	0	0	1	0	1	0	1	0	1	0	1	2	3
16:15 16:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	1	1	3	3
16:30 16:45	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	0	0	0	1
16:45 17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1
17:30 17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total: None	3	5	1	9	5	2	6	13	22	3	17	1	21	0	12	4	16	37	59

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Turning Movement Count - Study Results

BYRON AVE @ ROOSEVELT AVE

Survey Date: Wednesday, February 27, 2019 WO No: 38395

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total ROOSEVELT AVE BYRON AVE

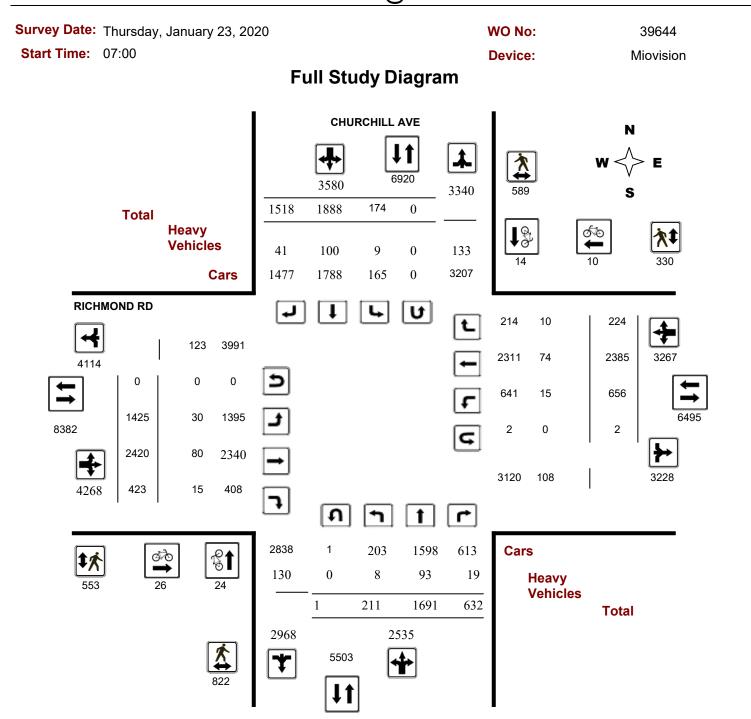
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	1	1
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	1	1
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	tal	0	0	0	2	2

June 22, 2020 Page 8 of 8



Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD



June 9, 2020 Page 1 of 8



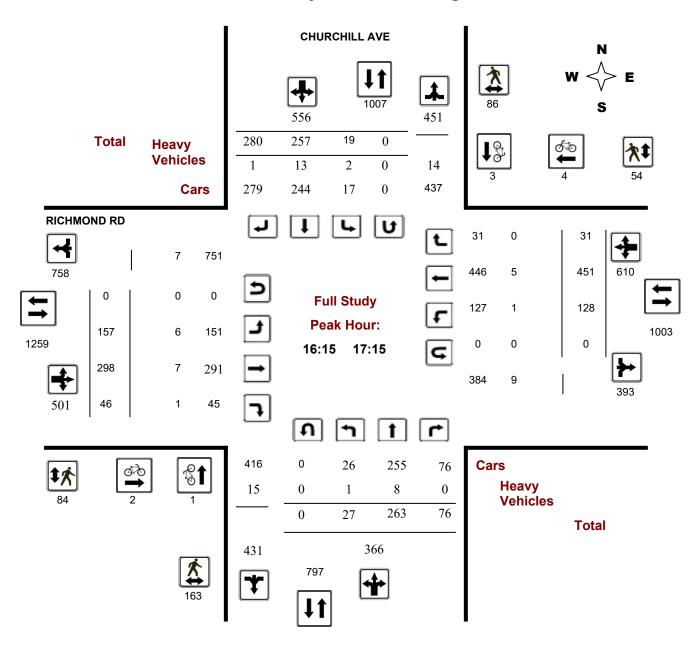
Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



June 9, 2020 Page 2 of 8



Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, January 23, 2020 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: 0

1.00

Eastbound: 0 Westbound: 2

			CHUF	RCHILL	_ AVE							RIC	HMON	D RD					
	No	rthbou	nd		So	uthbo	und			Е	astbou	ınd		V	Vestbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	14	161	59	234	16	226	90	332	566	274	347	24	645	35	126	22	183	828	1394
08:00 09:00	16	270	93	379	21	296	136	453	832	287	373	32	692	40	182	16	238	930	1762
09:00 10:00	27	205	81	313	18	219	137	374	687	162	329	35	526	64	173	33	270	796	1483
11:30 12:30	42	173	77	292	36	195	161	392	684	122	289	72	483	79	308	33	420	903	1587
12:30 13:30	31	183	83	297	30	215	187	432	729	128	254	83	465	73	340	27	440	905	1634
15:00 16:00	28	201	84	313	18	247	276	541	854	145	283	78	506	116	393	29	538	1044	1898
16:00 17:00	28	260	71	359	16	256	270	542	901	145	279	57	481	132	453	25	610	1091	1992
17:00 18:00	25	238	84	347	19	234	261	514	861	162	266	42	470	117	410	39	566	1036	1897
Sub Total	211	1691	632	2534	174	1888	1518	3580	6114	1425	2420	423	4268	656	2385	224	3265	7533	13647
U Turns				1				0	1				0				2	2	3
Total	211	1691	632	2535	174	1888	1518	3580	6115	1425	2420	423	4268	656	2385	224	3267	7535	13650
EQ 12Hr	293	2350	878	3524	242	2624	2110	4976	8500	1981	3364	588	5933	912	3315	311	4541	10474	18974
Note: These	values a	re calcu	ılated b	y multiply	ying the	totals b	y the a	opropriat	e expans	sion fac	tor.			1.39					
AVG 12Hr	276	2215	828	3321	228	2473	1989	4690	8500	1867	3170	554	5591	859	3124	293	4280	10474	18974
Note: These	volumes	are cal	culated	by multi	plying tl	he Equi	valent 1	2 hr. tota	als by the	AADT	factor.			1					
AVG 24Hr	362	2902	1085	4350	299	3240	2605	6144	10494	2445	4153	726	7324	1126	4093	384	5606	12930	23424
Note: These	volumes	are cal	culated	by multi	plying tl	he Aver	age Dai	ly 12 hr.	totals by	12 to 2	4 expans	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Survey Date: Thursday, January 23, 2020

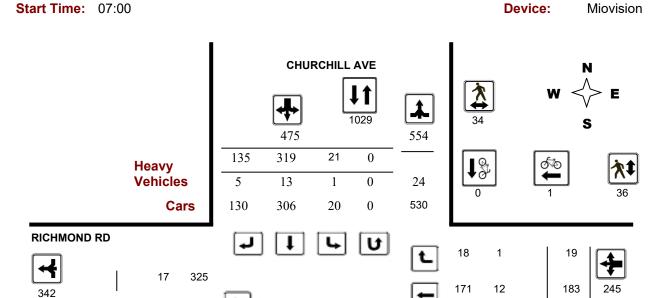
Transportation Services - Traffic Services

WO No:

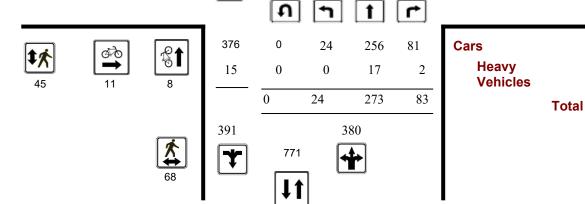
39644

Turning Movement Count - Peak Hour Diagram

CHURCHILL AVE @ RICHMOND RD







Comments

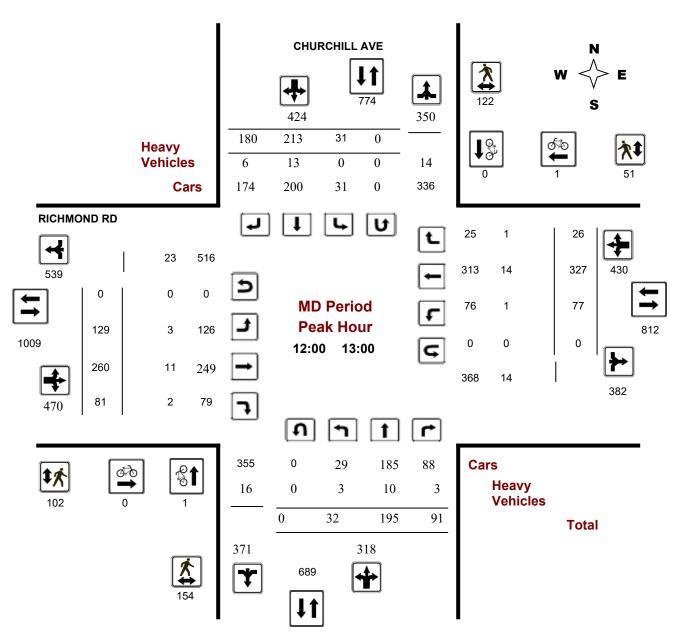
2020-Jun-09 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

CHURCHILL AVE @ RICHMOND RD

Survey Date:Thursday, January 23, 2020WO No:39644Start Time:07:00Device:Miovision



Comments

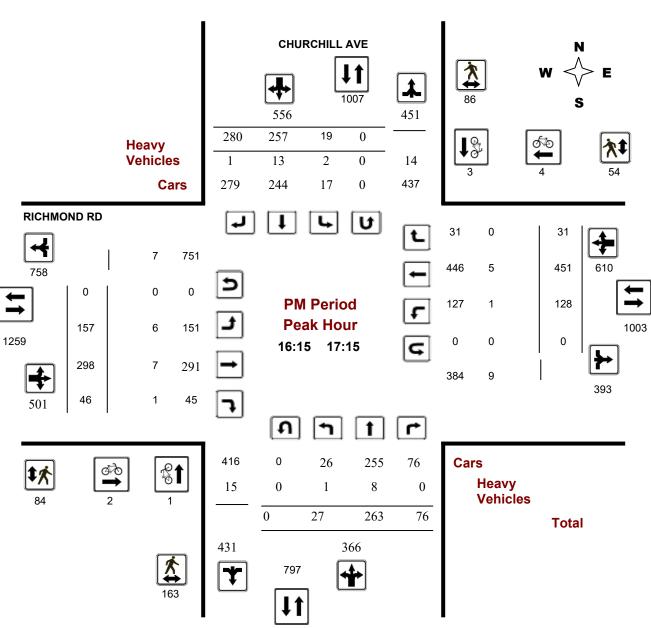
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Turning Movement Count - Peak Hour Diagram

CHURCHILL AVE @ RICHMOND RD





Comments

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CHURCHILL AVE

Transportation Services - Traffic Services

Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments RICHMOND RD

	Northbound			ınd	Southbound						Eastbound				Westbound					
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	4	25	10	39	3	46	20	69	4	56	75	5	136	7	27	6	40	4	284
07:15	07:30	4	28	8	40	2	50	19	71	6	63	91	5	159	5	35	2	42	6	312
07:30	07:45	2	37	18	57	9	59	24	92	13	80	85	10	175	11	26	4	41	13	365
07:45	08:00	4	71	23	98	2	71	27	100	8	75	96	4	175	12	38	10	60	8	433
08:00	08:15	1	69	22	93	6	60	32	98	10	72	83	11	166	11	39	3	53	10	410
08:15	08:30	6	60	30	96	7	83	29	119	7	77	95	7	179	9	43	7	59	7	453
08:30	08:45	4	66	20	90	3	74	36	113	11	76	83	8	167	8	53	4	65	11	435
08:45	09:00	5	75	21	101	5	79	39	123	8	62	112	6	180	12	47	2	61	8	465
09:00	09:15	9	72	12	93	6	83	31	120	12	47	100	8	155	14	40	6	60	12	428
09:15	09:30	4	48	27	79	5	55	39	99	13	46	79	12	137	14	37	6	58	13	373
09:30	09:45	6	46	19	71	4	43	34	81	17	38	74	8	120	19	56	10	85	17	357
09:45	10:00	8	39	23	70	3	38	33	74	12	31	76	7	114	17	40	11	68	12	326
11:30	11:45	9	32	16	57	12	46	29	87	14	28	75	13	116	22	78	11	111	14	371
11:45	12:00	15	52	19	86	8	43	39	90	14	31	73	22	126	19	63	12	94	14	396
12:00	12:15	10	48	22	80	7	59	44	110	14	31	82	20	133	17	90	6	113	14	436
12:15	12:30	8	41	20	69	9	47	49	105	5	32	59	17	108	21	77	4	102	5	384
12:30	12:45	5	46	25	76	9	51	41	101	10	31	52	27	110	16	77	6	99	10	386
12:45	13:00	9	60	24	93	6	56	46	108	6	35	67	17	119	23	83	10	116	6	436
13:00	13:15	11	42	13	66	6	50	56	112	9	30	70	24	124	17	83	4	104	9	406
13:15	13:30	6	35	21	62	9	58	44	111	12	32	65	15	112	17	97	7	121	12	406
15:00	15:15	10	48	16	74	5	61	62	128	7	32	77	34	143	28	98	11	137	7	482
15:15	15:30	10	53	24	87	6	66	71	143	8	46	74	18	138	32	84	7	123	8	491
15:30	15:45	4	49	15	68	6	57	61	124	4	35	68	9	112	30	110	8	148	4	452
15:45	16:00	4	51	29	84	1	63	82	146	3	32	64	17	113	26	101	3	131	3	474
16:00	16:15	4	53	15	72	4	57	62	123	7	28	60	18	106	36	114	5	155	7	456
16:15	16:30	8	70	12	90	4	60	68	132	6	37	76	12	125	29	113	7	149	6	496
16:30	16:45	6	64	23	93	4	71	67	142	8	43	72	13	128	35	114	7	156	8	519
16:45	17:00	10	73	21	104	4	68	73	145	7	37	71	14	122	32	112	6	150	7	521
17:00	17:15	3	56	20	79	7	58	72	137	4	40	79	7	126	32	112	11	155	4	497
17:15	17:30	10	62	16	88	5	61	68	134	4	44	52	16	112	28	116	8	152	4	486
17:30	17:45	8	59	22	89	3	68	66	137	6	40	72	5	117	28	89	7	124	6	467
17:45	18:00	4	61	26	91	4	47	55	106	1	38	63	14	115	29	93	13	135	1	447
Total:		211	1691	632	2535	174	1888	1518	3580	270	1425	2420	423	4268	656	2385	224	3267	270	13,650

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

CHURCHILL AVE RICHMOND RD

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	 Grand Total
07:00 07:15	0	0	0	1	0	1	1
07:15 07:30	2	0	2	0	0	0	2
07:30 07:45	1	0	1	0	0	0	1
07:45 08:00	3	0	3	4	0	4	7
08:00 08:15	4	1	5	0	1	1	6
08:15 08:30	5	0	5	4	0	4	9
08:30 08:45	2	0	2	2	1	3	5
08:45 09:00	0	0	0	2	0	2	2
09:00 09:15	1	0	1	3	0	3	4
09:15 09:30	2	1	3	0	1	1	4
09:30 09:45	0	1	1	1	1	2	3
09:45 10:00	1	0	1	0	0	0	1
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	1	1	1
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	1	0	1	0	0	0	1
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	1	0	1	1
15:15 15:30	0	0	0	1	0	1	1
15:30 15:45	0	0	0	2	0	2	2
15:45 16:00	0	1	1	1	0	1	2
16:00 16:15	0	2	2	0	1	1	3
16:15 16:30	0	0	0	0	1	1	1
16:30 16:45	0	1	1	0	0	0	1
16:45 17:00	0	0	0	1	0	1	1
17:00 17:15	1	2	3	1	3	4	7
17:15 17:30	0	2	2	0	0	0	2
17:30 17:45	0	1	1	2	0	2	3
17:45 18:00	1	2	3	0	0	0	3
Total	24	14	38	26	10	36	74

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Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

CHURCHILL AVE

RICHMOND RD

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	2	5	2	2	4	9
07:15 07:30	4	3	7	6	3	9	16
07:30 07:45	10	8	18	4	2	6	24
07:45 08:00	17	4	21	14	4	18	39
08:00 08:15	11	9	20	6	3	9	29
08:15 08:30	18	5	23	10	11	21	44
08:30 08:45	19	15	34	18	10	28	62
08:45 09:00	15	7	22	8	10	18	40
09:00 09:15	16	7	23	9	5	14	37
09:15 09:30	8	16	24	6	4	10	34
09:30 09:45	14	8	22	12	6	18	40
09:45 10:00	14	9	23	10	7	17	40
11:30 11:45	26	24	50	19	7	26	76
11:45 12:00	23	28	51	24	14	38	89
12:00 12:15	46	21	67	31	12	43	110
12:15 12:30	34	35	69	20	16	36	105
12:30 12:45	32	34	66	24	8	32	98
12:45 13:00	42	32	74	27	15	42	116
13:00 13:15	37	19	56	18	11	29	85
13:15 13:30	35	28	63	33	14	47	110
15:00 15:15	33	27	60	26	14	40	100
15:15 15:30	28	20	48	24	12	36	84
15:30 15:45	23	28	51	17	18	35	86
15:45 16:00	28	24	52	16	12	28	80
16:00 16:15	27	16	43	24	5	29	72
16:15 16:30	29	23	52	16	15	31	83
16:30 16:45	48	20	68	24	11	35	103
16:45 17:00	48	25	73	18	17	35	108
17:00 17:15	38	18	56	26	11	37	93
17:15 17:30	31	27	58	27	11	38	96
17:30 17:45	33	24	57	17	16	33	90
17:45 18:00	32	23	55	17	24	41	96
Total	822	589	1411	553	330	883	2294

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Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

CHURCHILL AVE

RICHMOND RD

		No	orthbou	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Pe	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 0	07:15	0	1	1	2	0	1	1	2	4	3	2	0	5	0	1	0	1	6	10
07:15 0	07:30	0	1	2	3	0	2	1	3	6	1	4	0	5	0	2	0	2	7	13
07:30 0	07:45	0	5	1	6	1	2	4	7	13	1	1	2	4	1	1	0	2	6	19
07:45 0	00:80	1	2	1	4	1	3	0	4	8	1	2	0	3	1	4	1	6	9	17
08:00 0	08:15	0	8	0	8	1	1	0	2	10	2	4	0	6	0	5	1	6	12	22
08:15 0	08:30	0	2	0	2	1	4	0	5	7	1	4	0	5	0	3	1	4	9	16
08:30 0	08:45	0	5	2	7	0	1	3	4	11	3	3	0	6	0	2	0	2	8	19
08:45 0	09:00	0	7	0	7	0	1	0	1	8	2	2	0	4	1	3	0	4	8	16
09:00 0	09:15	0	3	0	3	0	7	2	9	12	0	4	0	4	1	4	0	5	9	21
09:15 0	09:30	0	6	1	7	0	5	1	6	13	0	5	0	5	1	1	1	3	8	21
09:30 0	09:45	1	7	2	10	1	3	3	7	17	0	5	0	5	1	3	0	4	9	26
09:45 1	10:00	0	2	3	5	0	4	3	7	12	1	5	0	6	2	1	1	4	10	22
11:30 1	11:45	0	2	1	3	1	10	0	11	14	0	1	1	2	0	4	2	6	8	22
11:45 1	12:00	1	7	2	10	0	2	2	4	14	0	3	0	3	0	2	1	3	6	20
12:00 1	12:15	0	4	2	6	0	6	2	8	14	0	4	2	6	0	4	1	5	11	25
12:15 1	12:30	1	0	0	1	0	4	0	4	5	2	2	0	4	0	3	0	3	7	12
12:30 1	12:45	1	4	0	5	0	3	2	5	10	1	2	0	3	0	6	0	6	9	19
12:45 1	13:00	1	2	1	4	0	0	2	2	6	0	3	0	3	1	1	0	2	5	11
13:00 1	13:15	0	4	0	4	1	3	1	5	9	1	2	1	4	0	1	0	1	5	14
13:15 1	13:30	0	2	0	2	0	8	2	10	12	3	1	1	5	2	4	0	6	11	23
15:00 1	15:15	1	1	0	2	0	3	2	5	7	1	5	4	10	2	4	0	6	16	23
15:15 1	15:30	0	3	0	3	0	4	1	5	8	0	1	1	2	0	2	0	2	4	12
15:30 1	15:45	0	0	0	0	0	2	2	4	4	0	2	1	3	0	0	0	0	3	7
15:45 1	16:00	0	1	0	1	0	1	1	2	3	1	2	0	3	0	2	0	2	5	8
	16:15	0	2	0	2	0	1	4	5	7	0	2	0	2	1	3	1	5	7	14
16:15 1	16:30	0	2	0	2	0	4	0	4	6	1	1	1	3	0	2	0	2	5	11
	16:45	1	2	0	3	1	4	0	5	8	3	1	0	4	0	0	0	0	4	12
16:45 1	17:00	0	2	0	2	0	5	0	5	7	1	3	0	4	1	0	0	1	5	12
17:00 1	17:15	0	2	0	2	1	0	1	2	4	1	2	0	3	0	3	0	3	6	10
	17:30	0	2	0	2	0	1	1	2	4	0	1	1	2	0	1	0	1	3	7
	17:45	0	2	0	2	0	4	0	4	6	0	0	0	0	0	1	0	1	1	7
17:45 1	18:00	0	0	0	0	0	1	0	1	1	0	1	0	1	0	1	0	1	2	3
Total: N	None	8	93	19	120	9	100	41	150	270	30	80	15	125	15	74	10	99	224	494

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Turning Movement Count - Study Results

CHURCHILL AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39644

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total CHURCHILL AVE RICHMOND RD

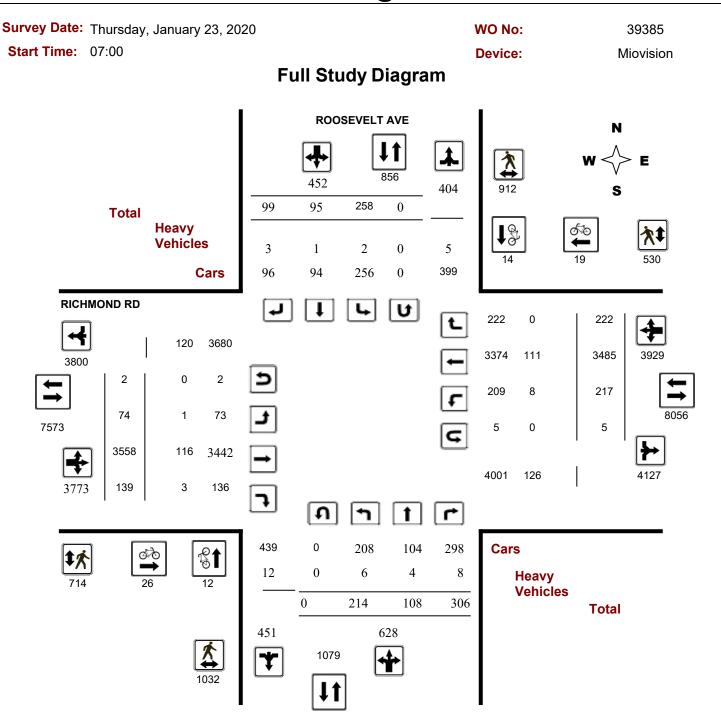
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	1	0	0	0	1
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	1	1
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	1	1
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	1	0	0	2	3

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD



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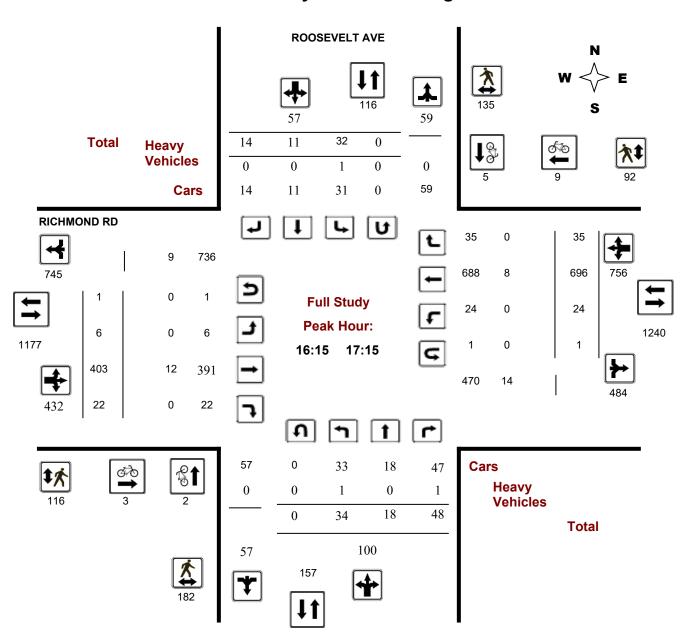
Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



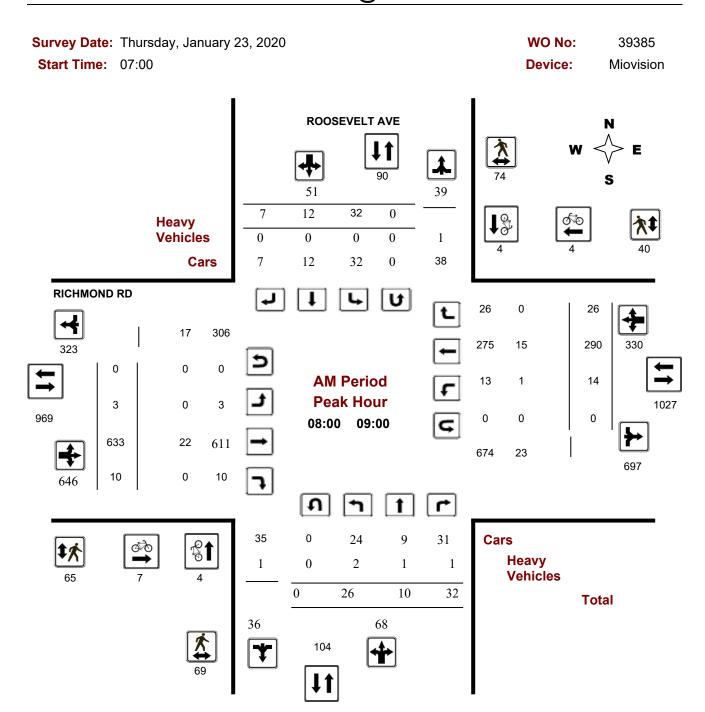
5472203 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Peak Hour Diagram

ROOSEVELT AVE @ RICHMOND RD



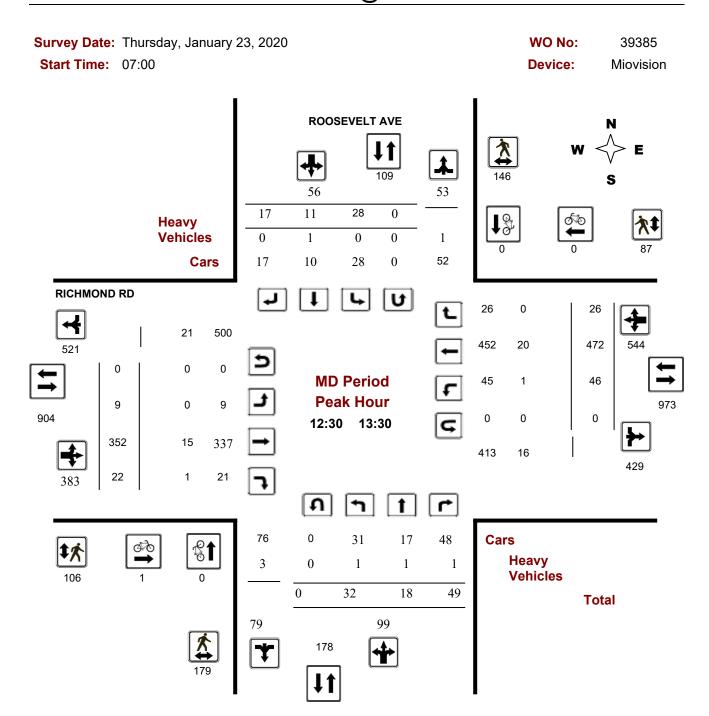
Comments 5472203 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Peak Hour Diagram

ROOSEVELT AVE @ RICHMOND RD



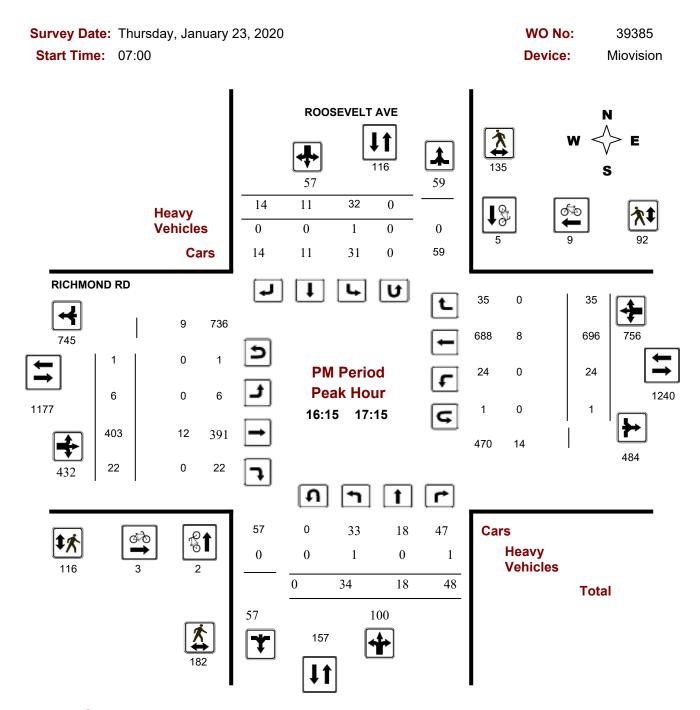
Comments 5472203 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Peak Hour Diagram

ROOSEVELT AVE @ RICHMOND RD



Comments 5472203 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, January 23, 2020 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 1.39

Eastbound: 2 Westbound: 5

		ı	ROOS	SEVEL	ΓAVE							RIC	HMON	D RD					
	Noi	thbou	nd		So	uthbou	ınd			Е	astbou	ınd		V	Vestbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	13	3	23	39	21	10	6	37	76	10	605	10	625	7	195	8	210	835	911
08:00 09:00	26	10	32	68	32	12	7	51	119	3	633	10	646	14	290	26	330	976	1095
09:00 10:00	20	18	27	65	34	15	8	57	122	4	462	12	478	25	263	21	309	787	909
11:30 12:30	30	16	62	108	39	15	16	70	178	15	359	20	394	41	375	48	464	858	1036
12:30 13:30	32	18	49	99	28	11	17	56	155	9	352	22	383	46	472	26	544	927	1082
15:00 16:00	31	14	27	72	36	10	21	67	139	16	401	27	444	32	603	27	662	1106	1245
16:00 17:00	27	18	50	95	27	13	13	53	148	8	376	22	406	17	670	37	724	1130	1278
17:00 18:00	35	11	36	82	41	9	11	61	143	9	370	16	395	35	617	29	681	1076	1219
Sub Total	214	108	306	628	258	95	99	452	1080	74	3558	139	3771	217	3485	222	3924	7695	8775
U Turns				0				0	0				2				5	7	7
Total	214	108	306	628	258	95	99	452	1080	74	3558	139	3773	217	3485	222	3929	7702	8782
EQ 12Hr	297	150	425	873	359	132	138	628	1501	103	4946	193	5244	302	4844	309	5461	10706	12207
Note: These	values ai	re calcu	lated by	y multiply	ing the	totals b	y the ap	opropriate	e expans	ion fac	tor.			1.39					
AVG 12Hr	297	150	425	873	359	132	138	628	1501	103	4946	193	5244	302	4844	309	5461	10706	12207
Note: These	volumes	are calc	culated	by multip	olying th	ne Equiv	alent 1	2 hr. tota	Is by the	AADT	factor.			1					
AVG 24Hr	390	197	557	1144	470	173	180	823	1967	135	6479	253	6870	395	6346	404	7154	14024	15991
Note: These	volumes	are calc	culated	by multip	olying th	ne Avera	age Dail	ly 12 hr. 1	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

ROOSEVELT AVE Northbound Southbound Eastbound Westbound s STR W **STR** Grand Ε **Time Period** LT ST LT ST RT LT ST RT LT ST RT TOT TOT TOT TOT TOT TOT **Total** 07:00 07:15 07:15 07:30 07:30 07:45 07:45 08:00 08:00 08:15 08:15 08:30 08:30 08:45 08:45 09:00 09:15 09:00 09:15 09:30 09:30 09:45 09:45 10:00 11:30 11:45 11:45 12:00 12:00 12:15 12:15 12:30 12:30 12:45 12:45 13:00 13:00 13:15 13:15 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00 17:00 17:15 17:30 17:15 17:30 17:45 17:45 18:00

Note: U-Turns are included in Totals.

8,782

Total:

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

ROOSEVELT AVE

RICHMOND RD

	ROOSEVELT AVE RICHIMOND RD								
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total		
07:00 07:15	0	0	0	1	0	1	1		
07:15 07:30	0	0	0	0	0	0	0		
07:30 07:45	0	0	0	3	1	4	4		
07:45 08:00	2	0	2	3	0	3	5		
08:00 08:15	0	2	2	3	1	4	6		
08:15 08:30	3	1	4	1	2	3	7		
08:30 08:45	1	1	2	2	1	3	5		
08:45 09:00	0	0	0	1	0	1	1		
09:00 09:15	0	1	1	3	0	3	4		
09:15 09:30	0	0	0	0	1	1	1		
09:30 09:45	0	0	0	1	0	1	1		
09:45 10:00	0	0	0	0	0	0	0		
11:30 11:45	0	0	0	0	0	0	0		
11:45 12:00	0	0	0	0	0	0	0		
12:00 12:15	0	0	0	0	0	0	0		
12:15 12:30	1	0	1	0	0	0	1		
12:30 12:45	0	0	0	1	0	1	1		
12:45 13:00	0	0	0	0	0	0	0		
13:00 13:15	0	0	0	0	0	0	0		
13:15 13:30	0	0	0	0	0	0	0		
15:00 15:15	0	0	0	1	0	1	1		
15:15 15:30	1	2	3	0	1	1	4		
15:30 15:45	0	0	0	0	0	0	0		
15:45 16:00	0	0	0	1	0	1	1		
16:00 16:15	0	1	1	1	0	1	2		
16:15 16:30	0	0	0	0	3	3	3		
16:30 16:45	1	2	3	2	3	5	8		
16:45 17:00	0	1	1	1	1	2	3		
17:00 17:15	1	2	3	0	2	2	5		
17:15 17:30	0	1	1	0	1	1	2		
17:30 17:45	2	0	2	1	1	2	4		
17:45 18:00	0	0	0	0	1	1	1		
Total	12	14	26	26	19	45	71		

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

ROOSEVELT AVE

RICHMOND RD

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	7	10	5	3	8	18
07:15 07:30	6	7	13	10	4	14	27
07:30 07:45	9	10	19	18	6	24	43
07:45 08:00	10	13	23	8	9	17	40
08:00 08:15	9	10	19	13	8	21	40
08:15 08:30	14	15	29	20	4	24	53
08:30 08:45	29	17	46	18	13	31	77
08:45 09:00	17	32	49	14	15	29	78
09:00 09:15	9	18	27	16	15	31	58
09:15 09:30	23	8	31	11	7	18	49
09:30 09:45	15	20	35	5	15	20	55
09:45 10:00	28	25	53	19	12	31	84
11:30 11:45	39	26	65	16	14	30	95
11:45 12:00	50	39	89	33	32	65	154
12:00 12:15	42	66	108	31	22	53	161
12:15 12:30	34	45	79	29	13	42	121
12:30 12:45	36	32	68	31	23	54	122
12:45 13:00	43	31	74	21	25	46	120
13:00 13:15	51	32	83	15	19	34	117
13:15 13:30	49	51	100	39	20	59	159
15:00 15:15	35	39	74	20	26	46	120
15:15 15:30	52	34	86	31	20	51	137
15:30 15:45	57	39	96	23	17	40	136
15:45 16:00	48	46	94	36	26	62	156
16:00 16:15	38	41	79	39	24	63	142
16:15 16:30	39	37	76	24	23	47	123
16:30 16:45	56	34	90	29	23	52	142
16:45 17:00	34	25	59	38	23	61	120
17:00 17:15	53	39	92	25	23	48	140
17:15 17:30	33	21	54	17	14	31	85
17:30 17:45	39	31	70	35	17	52	122
17:45 18:00	32	22	54	25	15	40	94
Total	1032	912	1944	714	530	1244	3188

5472203 - THU JAN 23, 2020 - 8HRS - LORETTA

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Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

ROOSEVELT AVE RICHMOND RD

		N	orthbo	und		Sc	uthbou	ınd			Е	astbour	nd		W	estbour	nd			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	0	0	0	0	0	0	0	0	0	5	0	7	0	2	0	7	14	7
07:15	07:30	0	0	0	2	0	0	0	0	2	0	4	1	8	1	3	0	8	16	9
07:30	07:45	0	0	1	1	0	0	0	0	1	0	3	0	8	0	5	0	9	17	9
07:45	08:00	0	0	0	0	0	0	0	0	0	0	2	0	5	0	3	0	5	10	5
08:00	08:15	0	0	0	0	0	0	0	0	0	0	7	0	12	0	5	0	12	24	12
08:15	08:30	1	0	1	2	0	0	0	0	2	0	4	0	9	0	4	0	9	18	10
08:30	08:45	1	1	0	3	0	0	0	1	4	0	6	0	10	1	3	0	10	20	12
08:45	09:00	0	0	0	0	0	0	0	0	0	0	5	0	8	0	3	0	8	16	8
09:00	09:15	0	0	0	0	0	0	1	2	2	1	4	0	12	0	6	0	10	22	12
09:15	09:30	0	0	0	0	0	0	1	1	1	0	5	0	8	0	2	0	7	15	8
09:30	09:45	0	0	0	0	0	0	0	0	0	0	5	0	11	0	6	0	11	22	11
09:45	10:00	0	2	1	3	0	0	0	2	5	0	6	0	10	0	4	0	11	21	13
11:30	11:45	0	0	1	2	0	0	0	0	2	0	1	0	4	1	3	0	6	10	6
11:45	12:00	0	0	0	2	0	0	0	0	2	0	4	0	7	2	3	0	9	16	9
12:00	12:15	0	0	1	2	0	0	0	0	2	0	5	0	11	1	6	0	13	24	13
12:15	12:30	1	0	0	2	0	0	0	0	2	0	5	0	9	1	3	0	9	18	10
12:30	12:45	0	0	1	2	0	0	0	0	2	0	3	1	13	0	9	0	13	26	14
12:45	13:00	0	0	0	1	0	1	0	1	2	0	4	0	8	0	4	0	8	16	9
13:00	13:15	1	0	0	2	0	0	0	0	2	0	3	0	5	1	1	0	5	10	6
13:15	13:30	0	1	0	1	0	0	0	1	2	0	5	0	11	0	6	0	11	22	12
15:00	15:15	0	0	1	1	0	0	0	0	1	0	6	0	8	0	2	0	9	17	9
15:15	15:30	0	0	0	0	1	0	0	1	1	0	1	0	6	0	5	0	7	13	7
15:30	15:45	0	0	0	0	0	0	0	0	0	0	3	0	5	0	2	0	5	10	5
15:45	16:00	0	0	0	0	0	0	1	1	1	0	3	0	7	0	3	0	6	13	7
16:00	16:15	1	0	0	2	0	0	0	0	2	0	3	1	11	0	6	0	9	20	11
16:15	16:30	0	0	0	0	0	0	0	0	0	0	2	0	5	0	3	0	5	10	5
16:30	16:45	1	0	1	2	0	0	0	0	2	0	3	0	5	0	1	0	5	10	6
16:45	17:00	0	0	0	0	0	0	0	0	0	0	4	0	4	0	0	0	4	8	4
17:00	17:15	0	0	0	0	1	0	0	1	1	0	3	0	7	0	4	0	8	15	8
17:15	17:30	0	0	0	0	0	0	0	0	0	0	1	0	2	0	1	0	2	4	2
17:30	17:45	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	4	2
17:45	18:00	0	0	0	0	0	0	0	0	0	0	1	0	2	0	1	0	2	4	2
Total:	None	6	4	8	30	2	1	3	11	41	1	116	3	240	8	111	0	245	485	263

February 18, 2020 Page 7 of 8



Turning Movement Count - Study Results

ROOSEVELT AVE @ RICHMOND RD

Survey Date: Thursday, January 23, 2020 WO No: 39385

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total ROOSEVELT AVE RICHMOND RD

Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	1	1
09:30	09:45	0	0	1	0	1
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	1	1
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	1	1
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	1	1	2
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	1	1
17:45	18:00	0	0	0	0	0
To	otal	0	0	2	5	7

February 18, 2020 Page 8 of 8

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

 Intersection:
 Main:
 Richmond
 Side:
 Churchill

 Controller:
 ATC3
 TSD:
 5229

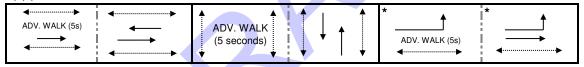
 Author:
 Matthew Anderson
 Date:
 26-May-2022

Existing Timing Plans[†]

		Plan					Ped Min	imum T	ime
		AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
-		1	2	3	4	5			
	Cycle	80	75	90	65	75			
	Offset	43	16	0	29	16			
	EB Thru	45	43	57	33	43	14	11	3.3+2.8
	WB Thru	31	31	45	33	31	14	11	3.3+2.8
	NB Thru	35	32	33	32	32	7	11	3.6+2.6
	SB Thru	35	32	33	32	32	7	11	3.6+2.6
	EB Left	14	12	12	-	12	-	-	3.3+2.8

Phasing Sequence[‡]





Plan: 4



Notes: 1) The Thru arrow is displayed during the East-West advanced walk, followed by the green ball.

Schedule

 Weekday

 Time
 Plan

 0:15
 4

 6:30
 1

 9:30
 2

 15:00
 3

 18:30
 2

 22:30
 4

Saturda	y
Time	Plan
0:15	4
6:30	2
9:00	5
18:30	2
22:30	4

Sulluay	
Time	Plan
0:15	4
6:30	2
9:00	5
18:00	2
22:30	4

Cundov

Notes

Asterisk (*) Indicates actuated phase (fp): Fully Protected Left Turn

^{†:} Time for each direction includes amber and all red intervals

^{‡:} Start of first phase should be used as reference point for offset

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection: Main: Richmond Side: Roosevelt

 Controller:
 MS 3200
 TSD: 5231

Author: Date: 26-May-2022

Existing Timing Plans[†]

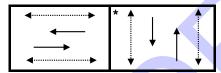
Plan

Ped Minimum Time

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	75	70	85	65	70			
Offset	27	X	78	X	Х			
EB Thru	45	40	55	35	40	18	8	3.3+2.1
WB Thru	45	40	55	35	40	18	8	3.3+2.1
NB Thru	30	30	30	30	30	14	10	3.3+2.3
SB Thru	30	30	30	30	30	14	10	3.3+2.3

Phasing Sequence[‡]

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:00	2
15:00	3
18:30	2
23:00	4

Saturday

Time	Plan
0:15	4
9:10	5
18:30	2
23:30	4

Sunday

Time	Plan
0:15	4
9:10	2
22:30	4

Notes

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

^{†:} Time for each direction includes amber and all red intervals

^{‡:} Start of first phase should be used as reference point for offset

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection:	Main:	Churchill	Si	ide:	Byron
Controller:	ATC 3		T	SD:	5634
Author:	Matthew	Anderson		ate:	26-May-2022

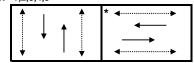
Existing Timing Plans[†]

Plan Ped Minimum Time

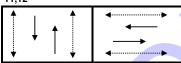
	AM Peak	Off Peak	PM Peak	Night	Weekend	AM School	PM School	Walk	DW	A+R
_	1	2	3	4	5	11	12			
Cycle	80	75	90	60	75	80	75			
Offset	74	45	40	Х	45	74	45			
NB Thru	42	40	45	32	40	42	40	10	11	3.3+2.1
SB Thru	42	40	45	32	40	42	40	10	11	3.3+2.1
EB Thru	38	35	45	28	35	38	35	10	15	3.3+2.3
WB Thru	38	35	45	28	35	38	35	10	15	3.3+2.3

Phasing Sequence[‡]

Plan: 1,2,3,4,5



Plan: 11,12



Notes: 1) In plan 4, the EW walk time is 7s

Schedule

Weekday

0:15 4 6:30 1 7:45 11 8:15 1 9:30 2 14:15 12
7:45 11 8:15 1 9:30 2 14:15 12
8:15 1 9:30 2 14:15 12
9:30 2 14:15 12
14:15 12
45.00
15:00 3
18:30 2
22:30 4

Saturday

	,
Time	Plan
0:15	4
6:30	2
9:00	5
18:30	2
22:30	1

Sunday

Curiady	
Time	Plan
0:15	4
6:30	2
9:00	5
18:00	2
22:30	4

Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase (fp): Fully Protected Left Turn

Pedestrian signal

City of Ottawa, Public Works Department

Traffic Signal Operations Unit

Intersection: Byron Side: Roosevelt

Controller: ATC 3 TSD: 6765

Author: Matthew Anderson Date: 26-May-2022

Existing Timing Plans[†]

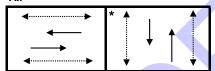
Plan

Ped Minimum Time

	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R
	1	2	3	4	5			
Cycle	70	65	70	60	65			
Offset	X	Х	Х	X	Х			
EB Thru	50	45	50	40	45	7	10	3.3+2.2
WB Thru	50	45	50	40	45	7	10	3.3+2.2
NB Thru	20	20	20	20	20	7	8	3.3+1.7
SB Thru	20	20	20	20	20	7	8	3.3+1.7

Phasing Sequence[‡]

Plan: All



Schedule

Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
23:00	4

Saturday

Time	Plan
0:15	4
9:10	5
18:30	2
23:30	4

Sunday

Time	Plan
0:15	4
9:10	2
22:30	4

Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset

Asterisk (*) Indicates actuated phase

(fp): Fully Protected Left Turn

∢.....

Pedestrian signal



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: BYRON AVE @ CHURCHILL AVE

Traffic Control: Traffic signal Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Mar-26, Sat,12:30	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jun-11, Sat,09:49	Rain	Turning movement	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
					South	Stopped	Pick-up truck	Other motor vehicle	
2017-Sep-12, Tue,14:43	Clear	Turning movement	P.D. only	Dry	West	Turning right	Unknown	Cyclist	0
					West	Going ahead	Bicycle	Other motor vehicle	
2019-Nov-05, Tue,21:32	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Cyclist	0
					West	Changing lanes	Bicycle	Other motor vehicle	
2020-Jan-30, Thu,08:58	Clear	SMV other	Non-fatal injury	Loose snow	South	Turning left	Passenger van	Pedestrian	1



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: BYRON AVE @ ROOSEVELT AVE

Traffic Control: Traffic signal Total Collisions: 3

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Feb-11, Sat,08:55	Clear	Other	P.D. only	Packed snow	East	Overtaking	Automobile, station wagon	Other motor vehicle	0
					West	Reversing	Truck - closed	Other motor vehicle	
2017-Mar-24, Fri,10:41	Snow	Other	P.D. only	Packed snow	West	Reversing	Pick-up truck	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Oct-12, Thu,15:26	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CHURCHILL AVE @ DANFORTH AVE

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Mar-21, Wed,11:46	Clear	Sideswipe	P.D. only	Dry	South	Merging	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CHURCHILL AVE @ RICHMOND RD

Traffic Control: Traffic signal Total Collisions: 25

Trainic Control. Tra	illo signai						Total Comsions	20	
ate/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Jan-04, Mon,12:08	Clear	Rear end	P.D. only	Wet	South	Changing lanes	Pick-up truck	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	
2016-Jun-02, Thu,14:22	Clear	SMV other	Non-fatal injury	Dry	South	Turning right	Automobile, station wagon	Pedestrian	1
2016-Jun-17, Fri,05:29	Clear	Angle	P.D. only	Dry	East	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Pick-up truck	Other motor vehicle	
2016-Aug-07, Sun,12:12	Clear	Rear end	P.D. only	Dry	South	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Pick-up truck	Other motor vehicle	
2016-Aug-13, Sat,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	North	Unknown	Unknown	Unattended vehicle	0
2016-Dec-09, Fri,08:40	Clear	Rear end	P.D. only	Ice	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Dec-31, Sat,12:01	Snow	SMV other	Non-fatal injury	Loose snow	East	Turning left	Automobile, station wagon	Pedestrian	1
2017-Jan-28, Sat,15:02	Snow	Rear end	Non-fatal injury	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	1
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Sep-06, Wed,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0
2017-Sep-26, Tue,19:08	Clear	Turning movement	P.D. only	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Feb-09, Fri,12:45	Clear	Sideswipe	P.D. only	Wet	East	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Mar-12, Mon,10:21	Clear	Turning movement	P.D. only	Dry	North	Stopped	Pick-up truck	Other motor vehicle	0
					North	Turning right	Truck - tractor	Other	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CHURCHILL AVE @ RICHMOND RD

Traffic Control: Traffic signal Total Collisions: 25

Trainic Control. Tra	ino oignai						rotal completions	20	
ate/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-Jun-09, Sat,10:48	Clear	Angle	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Sep-11, Tue,18:51	Clear	Turning movement	P.D. only	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Oct-27, Sat,21:39	Snow	SMV other	Non-fatal injury	Wet	West	Turning left	Passenger van	Pedestrian	2
2018-Nov-06, Tue,13:25	Rain	Rear end	Non-fatal injury	Wet	East	Going ahead	Truck - closed	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Dec-17, Mon,10:39	Clear	Sideswipe	P.D. only	Dry	South	Turning right	Automobile, station wagon	Other motor vehicle	0
					South	Turning right	Truck - open	Other motor vehicle	
2019-Jan-31, Thu,07:25	Clear	Turning movement	P.D. only	Packed snow	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Mar-20, Wed,16:55	Clear	Rear end	P.D. only	Dry	West	Going ahead	Unknown	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jul-12, Fri,21:01	Clear	Sideswipe	P.D. only	Dry	South	Overtaking	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Dec-09, Mon,14:41	Rain	Turning movement	Non-fatal injury	Wet	South	Overtaking	Automobile, station wagon	Other motor vehicle	0
					South	Turning right	Automobile, station wagon	Other motor vehicle	
2020-Jan-15, Wed,13:10	Clear	Sideswipe	P.D. only	Wet	North	Overtaking	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2020-May-17, Sun,17:18	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2020-Sep-26, Sat,17:07	Clear	Sideswipe	P.D. only	Dry	South	Changing lanes	Delivery van	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: CHURCHILL AVE @ RICHMOND RD

Traffic Control: Traffic signal Total Collisions: 25

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2020-Dec-13, Sun,07:26	Clear	Rear end	Non-fatal injury	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: DANFORTH AVE @ ROOSEVELT AVE

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Aug-12, Sat,14:08	Clear	Other	P.D. only	Dry	West	Unknown	Unknown	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: DANFORTH AVE btwn CHURCHILL AVE N & ROOSEVELT AVE

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-May-12, Thu,15:51	Clear	Angle	P.D. only	Dry	North	Reversing	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Pick-up truck	Other motor vehicle	
2018-Dec-21, Fri,10:00	Clear	Angle	P.D. only	Dry	South	Reversing	Unknown	Other motor vehicle	0
					West	Going ahead	Passenger van	Other motor vehicle	
2019-Feb-24, Sun,12:00	Rain	SMV unattended vehicle	P.D. only	Wet	West	Unknown	Unknown	Unattended vehicle	0
2019-Sep-13, Fri,11:30	Clear	Other	P.D. only	Dry	East	Reversing	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2020-Mar-03, Tue,00:00	Rain	SMV unattended vehicle	P.D. only	Wet	Unknown	Unknown	Unknown	Unattended vehicle	0



Collision Details Report - Public Version

From: January 1, 2016 **To:** December 31, 2020

Location: ROOSEVELT AVE @ RICHMOND RD

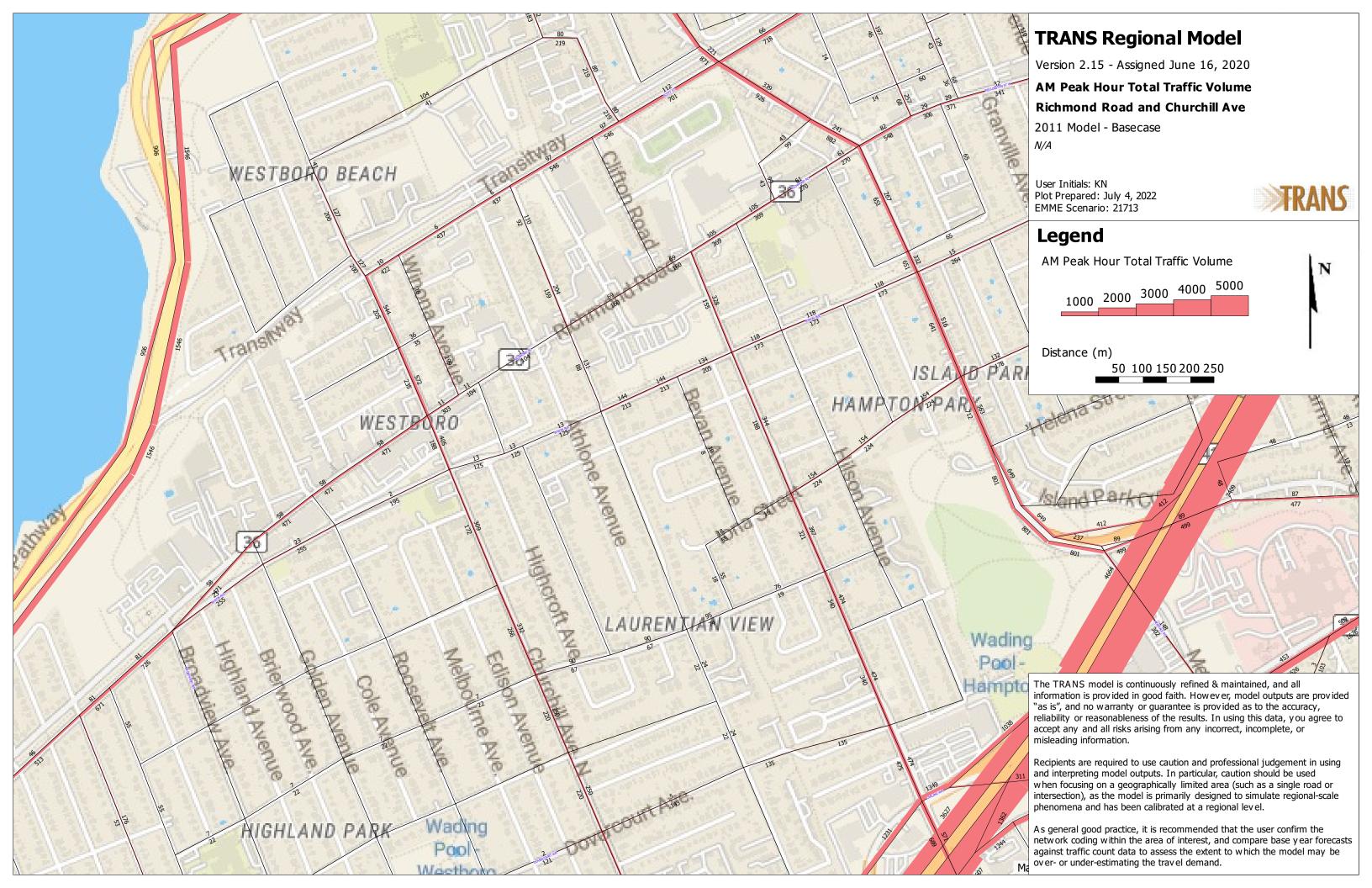
Traffic Control: Traffic signal Total Collisions: 8

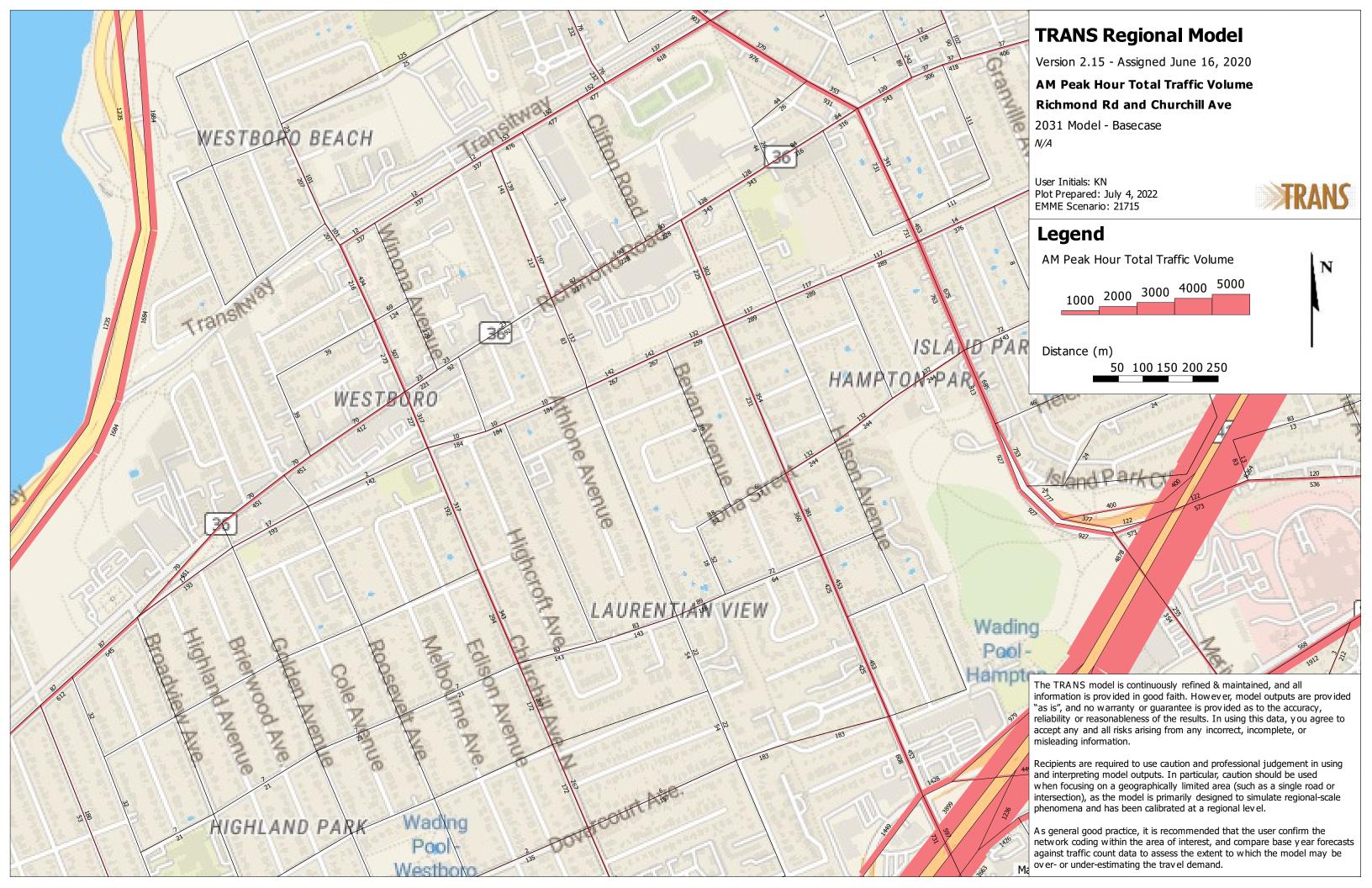
	3							•	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Apr-09, Sat,10:57	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-02, Tue,12:14	Snow	Rear end	P.D. only	Loose snow	East	Slowing or stopping	g Passenger van	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jan-03, Thu,14:19	Snow	Rear end	Non-fatal injury	Loose snow	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2019-Jan-03, Thu,15:48	Snow	Rear end	P.D. only	Slush	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2019-Jul-26, Fri,07:45	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Sep-17, Tue,13:51	Clear	Sideswipe	P.D. only	Dry	East	Pulling away from shoulder or curb	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
2020-Jan-06, Mon,10:55	Snow	Rear end	P.D. only	Slush	East	Unknown	Unknown	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	
2020-Jul-22, Wed,08:44	Clear	Other	P.D. only	Dry	East	Reversing	Unknown	Other motor vehicle	0
					West	Stopped	Pick-up truck	Other motor vehicle	



APPENDIX E: TRANS SNAPSHOTS, 2011 AND 2031 HORIZON YEARS





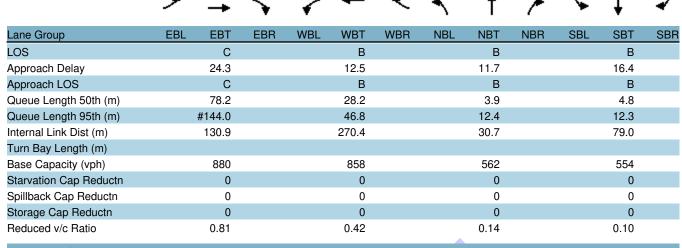




APPENDIX F: EXISTING (2022) SYNCHRO ANALYSIS



	۶	→	•	•	←	•	•	†	<i>></i>	/	+	-✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	3	633	10	14	290	19	26	10	32	32	12	7
Future Volume (vph)	3	633	10	14	290	19	26	10	32	32	12	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.7	3.7	3.7	3.7	4.0	3.7	3.7	4.5	3.7	3.7	4.5	3.7
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998			0.992			0.936			0.981	
Flt Protected					0.998			0.981			0.969	
Satd. Flow (prot)	0	1676	0	0	1687	0	0	1812	0	0	1987	0
Flt Permitted		0.999			0.963			0.896			0.824	
Satd. Flow (perm)	0	1675	0	0	1628	0	0	1655	0	0	1689	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2			6			36			8	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		154.9			294.4			54.7			103.0	
Travel Time (s)		11.2			21.2			3.9			7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	3%	0%	7%	5%	0%	8%	10%	3%	0%	0%	0%
Parking (#/hr)		0			0							
Adj. Flow (vph)	3	703	11	16	322	21	29	11	36	36	13	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	717	0	0	359	0	0	76	0	0	57	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		30.0	30.0		30.0	30.0	
Total Split (s)	45.0	45.0		45.0	45.0		30.0	30.0		30.0	30.0	
Total Split (%)	60.0%	60.0%		60.0%	60.0%		40.0%	40.0%		40.0%	40.0%	
Maximum Green (s)	39.4	39.4		39.4	39.4		24.4	24.4		24.4	24.4	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		2.3	2.3		2.3	2.3	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.6			5.6			5.6			5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		Max	Max		Max	Max	
Walk Time (s)	14.0	14.0		14.0	14.0		14.0	14.0		14.0	14.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)	<u> </u>	39.4		<u> </u>	39.4		<u> </u>	24.4		3	24.4	
Actuated g/C Ratio		0.53			0.53			0.33			0.33	
v/c Ratio		0.81			0.42			0.14			0.10	
Control Delay		24.3			12.5			11.7			16.4	
Queue Delay		0.0			0.0			0.0			0.0	
		24.3			12.5			11.7			16.4	
Total Delay		۷4.3			12.5			11./			10.4	



Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 27 (36%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 19.7

Intersection Capacity Utilization 48.9%

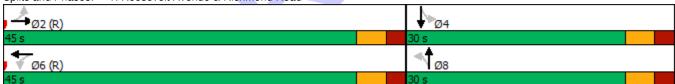
Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

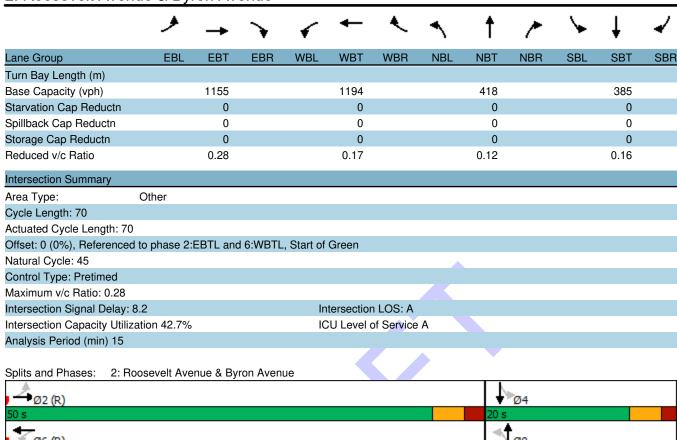
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Roosevelt Avenue & Richmond Road



	۶	→	*	•	—	•	•	†	~	/	ļ	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	35	243	8	12	145	28	3	29	14	26	19	12
Future Volume (vph)	35	243	8	12	145	28	3	29	14	26	19	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.7	3.7	3.7	3.7	4.1	3.7	3.7	4.5	3.7	3.7	4.8	3.7
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.996			0.980			0.958			0.972	
Flt Protected		0.994			0.997			0.997			0.978	
Satd. Flow (prot)	0	1902	0	0	1903	0	0	1912	0	0	1978	0
Flt Permitted		0.949			0.977			0.987			0.865	
Satd. Flow (perm)	0	1816	0	0	1865	0	0	1893	0	0	1749	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		4			25			16			13	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.7			54.4			135.0			20.2	
Travel Time (s)		8.2			3.9			9.7			1.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	0%	3%	4%	0%	7%	0%	4%	5%	0%
Adj. Flow (vph)	39	270	9	13	161	31	3	32	16	29	21	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	318	0	0	205	0	0	51	0	0	63	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Minimum Split (s)	23.5	23.5		23.5	23.5		20.0	20.0		20.0	20.0	
Total Split (s)	50.0	50.0		50.0	50.0		20.0	20.0		20.0	20.0	
Total Split (%)	71.4%	71.4%		71.4%	71.4%		28.6%	28.6%		28.6%	28.6%	
Maximum Green (s)	44.5	44.5		44.5	44.5		15.0	15.0		15.0	15.0	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2		2.2	2.2		1.7	1.7		1.7	1.7	
Lost Time Adjust (s)		0.0			0.0			0.0		•••	0.0	
Total Lost Time (s)		5.5			5.5			5.0			5.0	
Lead/Lag		0.0			0.0			0.0			0.0	
Lead-Lag Optimize?												
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0		8.0	8.0		8.0	8.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		44.5			44.5			15.0			15.0	
Actuated g/C Ratio		0.64			0.64			0.21			0.21	
v/c Ratio		0.28			0.17			0.12			0.16	
Control Delay		6.3			5.0			17.7			20.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		6.3			5.0			17.7			20.0	
LOS		Α			Α			В			C C	
Approach Delay		6.3			5.0			17.7			20.0	
Approach LOS		0.5 A			3.0 A			В			20.0 C	
Queue Length 50th (m)		15.7			8.2			3.7			5.3	
Queue Length 95th (m)		26.2			15.4			11.6			14.3	
• ,												
Internal Link Dist (m)		89.7			30.4			111.0			0.1	



	۶	→	•	•	←	•	4	†	~	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	£		7	f)			ર્ન	7		ર્ન	7
Traffic Volume (vph)	262	390	29	43	183	19	24	273	83	21	319	135
Future Volume (vph)	262	390	29	43	183	19	24	273	83	21	319	135
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	4.0	3.7	3.3	4.0	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Storage Length (m)	33.0		0.0	27.0		0.0	0.0		25.0	0.0		35.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	7.6			7.6			7.6			7.6		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.990			0.986				0.850			0.850
Flt Protected	0.950			0.950				0.996			0.997	
Satd. Flow (prot)	1711	1911	0	1662	1832	0	0	1814	1601	0	1841	1570
Flt Permitted	0.493			0.496				0.800			0.947	
Satd. Flow (perm)	888	1911	0	868	1832	0	0	1457	1601	0	1748	1570
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		6			7				180			180
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		294.4			106.1			75.8			111.4	
Travel Time (s)		21.2			7.6			5.5			8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	0%	5%	7%	5%	0%	6%	2%	5%	4%	4%
Adj. Flow (vph)	291	433	32	48	203	21	27	303	92	23	354	150
Shared Lane Traffic (%)												
Lane Group Flow (vph)	291	465	0	48	224	0	0	330	92	0	377	150
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	5	2			6			4			8	
Permitted Phases	2			6			4		4	8		8
Detector Phase	5	2		6	6		4	4	4	8	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	11.1	31.1		31.0	31.0		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	14.0	40.0		31.0	31.0		30.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	17.5%	50.0%		38.8%	38.8%		37.5%	37.5%	37.5%	37.5%	37.5%	37.5%
Maximum Green (s)	7.9	33.9		24.9	24.9		23.8	23.8	23.8	23.8	23.8	23.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None	None	None	None	None
Walk Time (s)		14.0		14.0	14.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0		11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0		0	0	0	0	0	0
Act Effct Green (s)	45.5	45.5		29.4	29.4			22.2	22.2		22.2	22.2
Actuated g/C Ratio	0.57	0.57		0.37	0.37			0.28	0.28		0.28	0.28
v/c Ratio	0.48	0.43		0.15	0.37			0.20	0.16		0.78	0.27
Control Delay	13.5	12.4		21.2	21.1			31.1	0.10		37.4	3.2
- Delay	10.0	14.4		۷۱.۷	۱.۱			91.1	0.5		37.4	٥.۷

Lane Group	Ø1	Ø3	Ø7	
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Lane Width (m)				
Storage Length (m)				
Storage Lanes				
Taper Length (m)				
Lane Util. Factor				
Frt				
Flt Protected				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (k/h)				
Link Distance (m)				
Travel Time (s)				
Peak Hour Factor				
Heavy Vehicles (%)				
Adj. Flow (vph)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	7	
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	3.0	3.0	1.0	
Minimum Split (s)	5.0	5.0	5.0	V
Total Split (s)	5.0	5.0	5.0	
Total Split (%)	6%	6%	6%	
Maximum Green (s)	3.0	3.0	3.0	
Yellow Time (s)	2.0	2.0	2.0	
All-Red Time (s)	0.0	0.0	0.0	
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	
Recall Mode	None	None	None	
Walk Time (s)				
Flash Dont Walk (s)				
Pedestrian Calls (#/hr)				
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				

0.72

0.15

0.69

0.24

3: Churchill Avenue N & Richmond Road

	•	→	`	•	—	4	4	†	/	\	Ţ	1
	ED !			*(\mathred{\pi})	WOT	WDD		· ·	,	0.01	0.0.7	000
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	13.5	12.4		21.2	21.1			31.1	0.5		37.4	3.2
LOS	В	В		С	С			С	Α		D	Α
Approach Delay		12.8			21.1			24.4			27.7	
Approach LOS		В			С			С			С	
Queue Length 50th (m)	21.5	37.2		5.2	24.9			24.5	0.0		52.0	0.0
Queue Length 95th (m)	42.7	69.3		13.3	43.9			30.5	0.0		73.0	7.7
Internal Link Dist (m)		270.4			82.1			51.8			87.4	
Turn Bay Length (m)	33.0			27.0					25.0			35.0
Base Capacity (vph)	607	1088		318	677			457	626		549	616
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0

0.33

0.15

Intersection Summary

Reduced v/c Ratio

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 52 (65%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

0.48

Natural Cycle: 75

Control Type: Actuated-Coordinated

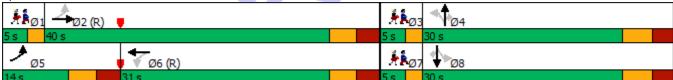
Maximum v/c Ratio: 0.81

Intersection LOS: C Intersection Signal Delay: 20.4 Intersection Capacity Utilization 76.0% ICU Level of Service D

0.43

Analysis Period (min) 15

3: Churchill Avenue N & Richmond Road Splits and Phases:



Lane Group	Ø1	Ø3	Ø7
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

	۶	-	•	•	—	•	•	†	<i>></i>	/	ţ	√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ř	f)		ř	ĵ»	
Traffic Volume (vph)	56	170	57	51	129	47	26	326	69	34	309	30
Future Volume (vph)	56	170	57	51	129	47	26	326	69	34	309	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.7	3.7	3.7	3.7	3.7	3.7	3.0	4.0	3.7	3.0	4.0	3.7
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	18.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6			7.6			7.6			7.6		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.973			0.972			0.974			0.987	
Flt Protected		0.990			0.989		0.950			0.950		
Satd. Flow (prot)	0	1825	0	0	1819	0	1560	1853	0	1685	1869	0
Flt Permitted		0.889			0.869		0.449			0.389		
Satd. Flow (perm)	0	1639	0	0	1598	0	737	1853	0	690	1869	0
Right Turn on Red	•		Yes			Yes			Yes		, , ,	Yes
Satd. Flow (RTOR)		19			20			18			8	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		222.5			63.6			184.9			45.3	
Travel Time (s)		16.0			4.6			13.3			3.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	1%	0%	0%	2%	2%	8%	5%	1%	0%	5%	3%
Adj. Flow (vph)	62	189	63	57	143	52	29	362	77	38	343	33
Shared Lane Traffic (%)	02	100	- 00	<u> </u>	1.0	02		002	• • •	00	0.10	00
Lane Group Flow (vph)	0	314	0	0	252	0	29	439	0	38	376	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	J
Protected Phases		4			8		. 0	2			6	
Permitted Phases	4			8			2	_		6		
Minimum Split (s)	30.6	30.6		30.6	30.6		26.4	26.4		26.4	26.4	
Total Split (s)	38.0	38.0		38.0	38.0		42.0	42.0		42.0	42.0	
Total Split (%)	47.5%	47.5%		47.5%	47.5%		52.5%	52.5%		52.5%	52.5%	
Maximum Green (s)	32.4	32.4		32.4	32.4		36.6	36.6		36.6	36.6	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag		0.0			0.0		.	.		.	U. 1	
Lead-Lag Optimize?												
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		32.4			32.4		36.6	36.6		36.6	36.6	
Actuated g/C Ratio		0.40			0.40		0.46	0.46		0.46	0.46	
v/c Ratio		0.47			0.38		0.09	0.51		0.12	0.44	
Control Delay		19.1			17.5		13.2	17.3		4.8	5.1	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.2	
Total Delay		19.1			17.5		13.2	17.3		4.8	5.3	
LOS		19.1 B			17.5 B		13.2 B	17.3 B		4.0 A	5.3 A	
							В			А		
Approach LOS		19.1			17.5			17.1			5.3	
Approach LOS		В			В			В			Α	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (m)		31.7			23.9		2.4	43.3		1.0	9.7	
Queue Length 95th (m)		53.4			41.7		7.0	68.2		m2.2	18.2	
Internal Link Dist (m)		198.5			39.6			160.9			21.3	
Turn Bay Length (m)							15.0			18.0		
Base Capacity (vph)		675			659		337	857		315	859	
Starvation Cap Reductn		0			0		0	0		0	97	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.47			0.38		0.09	0.51		0.12	0.49	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

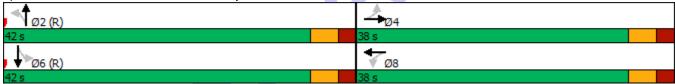
Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.51

Intersection Signal Delay: 14.2 Intersection LOS: B
Intersection Capacity Utilization 57.4% ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Churchill Avenue N & Byron Avenue



Intersection							
Int Delay, s/veh	3.2						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		f)			ની	
Traffic Vol, veh/h	33	19	49	43	12	24	
Future Vol, veh/h	33	19	49	43	12	24	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage,	# 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	0	0	5	0	0	5	
Mvmt Flow	37	21	54	48	13	27	
Major/Minor M	linor1	N	//ajor1	ı	Major2		
Conflicting Flow All	131	78	0	0	102	0	
Stage 1	78	-	-	-	-	-	
Stage 2	53	-		-	-	-	
Critical Hdwy	6.4	6.2	-	-	4.1	-	
Critical Hdwy Stg 1	5.4	-		-	-	-	
Critical Hdwy Stg 2	5.4	-	-	-	-	-	
Follow-up Hdwy	3.5	3.3	-	-	2.2	-	
Pot Cap-1 Maneuver	868	988	-	-	1503	-	
Stage 1	950	-	-	-	-	-	
Stage 2	975	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	860	988	-	-	1503	-	
Mov Cap-2 Maneuver	860		-		-	-	
Stage 1	950	-	-	-	-	-	
Stage 2	966	-	-	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	9.3		0		2.5		
HCM LOS	Α						
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT	
Capacity (veh/h)		-	-		1503	-	
HCM Lane V/C Ratio		_		0.064		_	
HCM Control Delay (s)		_	_	9.3	7.4	0	
HCM Lane LOS		_	-	Α.	Α	A	
HCM 95th %tile Q(veh)		_	_	0.2	0	- '.	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	6	403	22	24	696	31	34	18	48	32	11	14
Future Volume (vph)	6	403	22	24	696	31	34	18	48	32	11	14
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Width (m)	3.7	3.7	3.7	3.7	4.0	3.7	3.7	4.5	3.7	3.7	4.5	3.7
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.993			0.994			0.936			0.966	
Flt Protected		0.999			0.998			0.983			0.973	
Satd. Flow (prot)	0	1581	0	0	1663	0	0	1786	0	0	1830	0
Flt Permitted		0.989			0.977			0.891			0.819	
Satd. Flow (perm)	0	1565	0	0	1628	0	0	1619	0	0	1540	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		5			4			53			16	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		154.9			294.4			54.7			103.0	
Travel Time (s)		11.2			21.2			3.9			7.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	3%	0%	0%	1%	0%	3%	0%	2%	3%	0%	0%
Parking (#/hr)		0			0							
Adj. Flow (vph)	7	448	24	27	773	34	38	20	53	36	12	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	479	0	0	834	0	0	111	0	0	64	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		30.0	30.0		30.0	30.0	
Total Split (s)	55.0	55.0		55.0	55.0		30.0	30.0		30.0	30.0	
Total Split (%)	64.7%	64.7%		64.7%	64.7%		35.3%	35.3%		35.3%	35.3%	
Maximum Green (s)	49.4	49.4		49.4	49.4		24.4	24.4		24.4	24.4	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		2.3	2.3		2.3	2.3	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.6			5.6			5.6			5.6	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		Max	Max		Max	Max	
Walk Time (s)	14.0	14.0		14.0	14.0		14.0	14.0		14.0	14.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		49.4			49.4			24.4			24.4	
Actuated g/C Ratio		0.58			0.58			0.29			0.29	
v/c Ratio		0.53			0.88			0.22			0.14	
Control Delay		13.2			28.3			14.4			19.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		13.2			28.3			14.4			19.0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS		В			С			В			В	
Approach Delay		13.2			28.3			14.4			19.0	
Approach LOS		В			С			В			В	
Queue Length 50th (m)		42.7			106.3			6.9			5.7	
Queue Length 95th (m)		67.5			#190.3			18.9			14.9	
Internal Link Dist (m)		130.9			270.4			30.7			79.0	
Turn Bay Length (m)												
Base Capacity (vph)		911			947			502			453	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.53			0.88			0.22			0.14	

Intersection Summary

Area Type: Other

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 27 (32%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 22.0

The section digital Delay, 22.0

Intersection Capacity Utilization 71.2%

Intersection LOS: C

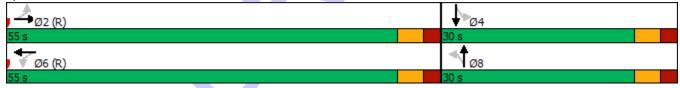
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Roosevelt Avenue & Richmond Road

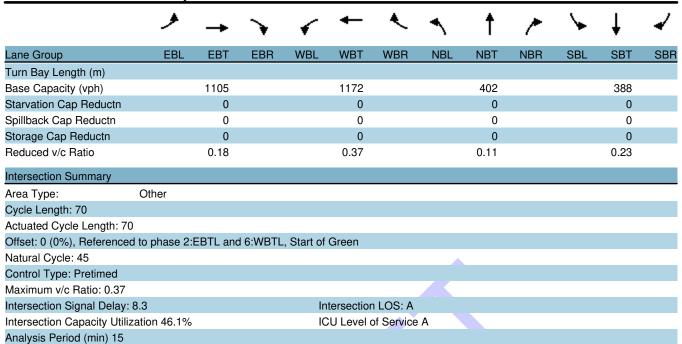


2: Roosevelt Avenue & Byron Avenue

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	13	158	7	19	320	46	5	21	13	31	20	31
Future Volume (vph)	13	158	7	19	320	46	5	21	13	31	20	31
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Width (m)	3.7	3.7	3.7	3.7	4.1	3.7	3.7	4.5	3.7	3.7	4.8	3.7
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995			0.984			0.956			0.949	
Flt Protected		0.996			0.998			0.993			0.981	
Satd. Flow (prot)	0	1788	0	0	1861	0	0	1880	0	0	1878	0
Flt Permitted		0.967			0.983			0.964			0.883	
Satd. Flow (perm)	0	1736	0	0	1833	0	0	1825	0	0	1690	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		6			19			14			34	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		113.7			54.4			135.0			20.2	
Travel Time (s)		8.2			3.9			9.7			1.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	1%	0%	0%	0%	2%	0%	0%	0%	3%	0%	0%
Adj. Flow (vph)	14	176	8	21	356	51	6	23	14	34	22	34
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	198	0	0	428	0	0	43	0	0	90	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Minimum Split (s)	23.5	23.5		23.5	23.5		20.0	20.0		20.0	20.0	
Total Split (s)	50.0	50.0		50.0	50.0		20.0	20.0		20.0	20.0	
Total Split (%)	71.4%	71.4%		71.4%	71.4%		28.6%	28.6%		28.6%	28.6%	
Maximum Green (s)	44.5	44.5		44.5	44.5		15.0	15.0		15.0	15.0	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.2	2.2		2.2	2.2		1.7	1.7		1.7	1.7	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	10.0	10.0		10.0	10.0		8.0	8.0		8.0	8.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		44.5			44.5			15.0			15.0	
Actuated g/C Ratio		0.64			0.64			0.21			0.21	
v/c Ratio		0.18			0.37			0.11			0.23	
Control Delay		5.6			6.8			17.5			17.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		5.6			6.8			17.5			17.3	
LOS		A			A			В			В	
Approach Delay		5.6			6.8			17.5			17.3	
Approach LOS		A			A			В			В	
Queue Length 50th (m)		8.9			21.7			3.1			6.0	
Queue Length 95th (m)		16.3			35.7			10.3			16.8	
Internal Link Dist (m)		89.7			30.4			111.0			0.1	
mornar Link Dist (iii)		00.1			50.4			111.0			0.1	

2: Roosevelt Avenue & Byron Avenue

07/07/2022



Splits and Phases: 2: Roosevelt Avenue & Byron Avenue



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^		ሻ	(ર્ન	7		ર્ન	7
Traffic Volume (vph)	157	298	46	128	451	31	27	263	76	19	257	280
Future Volume (vph)	157	298	46	128	451	31	27	263	76	19	257	280
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Width (m)	3.3	4.0	3.7	3.3	4.0	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Storage Length (m)	33.0		0.0	27.0		0.0	0.0		25.0	0.0		35.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	7.6			7.6			7.6			7.6		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.980			0.990				0.850			0.850
Flt Protected	0.950			0.950				0.995			0.997	
Satd. Flow (prot)	1589	1806	0	1637	1844	0	0	1757	1547	0	1721	1547
Flt Permitted	0.250			0.535				0.917			0.960	
Satd. Flow (perm)	418	1806	0	922	1844	0	0	1619	1547	0	1658	1547
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13			5				160			300
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		294.4			106.1			75.8			111.4	
Travel Time (s)		21.2			7.6			5.5			8.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	4%	2%	2%	1%	1%	0%	4%	3%	0%	11%	5%	0%
Adj. Flow (vph)	174	331	51	142	501	34	30	292	84	21	286	311
Shared Lane Traffic (%)												
Lane Group Flow (vph)	174	382	0	142	535	0	0	322	84	0	307	311
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	5	2			6			4			8	
Permitted Phases	2			6			4		4	8		8
Detector Phase	5	2		6	6		4	4	4	8	8	8
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	11.1	31.1		32.0	32.0		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	12.0	52.0		45.0	45.0		28.0	28.0	28.0	28.0	28.0	28.0
Total Split (%)	13.3%	57.8%		50.0%	50.0%		31.1%	31.1%	31.1%	31.1%	31.1%	31.1%
Maximum Green (s)	5.9	45.9		38.9	38.9		21.8	21.8	21.8	21.8	21.8	21.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None	None	None	None	None
Walk Time (s)		14.0		14.0	14.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0		11.0	11.0		11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0		0	0		0	0	0	0	0	0
Act Effct Green (s)	52.0	52.0		39.8	39.8			25.7	25.7		25.7	25.7
Actuated g/C Ratio	0.58	0.58		0.44	0.44			0.29	0.29		0.29	0.29
v/c Ratio	0.54	0.36		0.35	0.65			0.70	0.15		0.65	0.47
Control Delay	16.5	11.3		20.1	24.6			38.0	7.4		35.2	6.2
	10.5	11.0		20.1	۷-4.0			55.0	7.7		00.2	0.2

Lane Group	Ø1	Ø3	Ø7	
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Lane Width (m)				
Storage Length (m)				
Storage Lanes				
Taper Length (m)				
Lane Util. Factor				
Frt				
Flt Protected				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (k/h)				
Link Distance (m)				
Travel Time (s)				
Peak Hour Factor				
Heavy Vehicles (%)				
Adj. Flow (vph)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	7	
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	3.0	3.0	1.0	
Minimum Split (s)	5.0	5.0	5.0	
Total Split (s)	5.0	5.0	5.0	
Total Split (%)	6%	6%	6%	
Maximum Green (s)	3.0	3.0	3.0	
Yellow Time (s)	2.0	2.0	2.0	
All-Red Time (s)	0.0	0.0	0.0	
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	
Recall Mode	None	None	None	
Walk Time (s)				
Flash Dont Walk (s)				
Pedestrian Calls (#/hr)				
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Control Dolay				

	•	-	•	•	•	•	4	†	_	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Delay	0.0	0.0		0.1	0.0			0.0	0.0		0.0	0.0
Total Delay	16.5	11.3		20.3	24.6			38.0	7.4		35.2	6.2
LOS	В	В		С	С			D	Α		D	Α
Approach Delay		12.9			23.7			31.7			20.6	
Approach LOS		В			С			С			С	
Queue Length 50th (m)	13.7	32.5		16.0	71.0			54.7	2.6		45.5	1.4
Queue Length 95th (m)	23.8	50.6		30.7	106.2			m82.3	m9.9		72.4	19.3
Internal Link Dist (m)		270.4			82.1			51.8			87.4	
Turn Bay Length (m)	33.0			27.0					25.0			35.0
Base Capacity (vph)	321	1049		407	817			463	557		474	657
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	46		23	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.54	0.38		0.37	0.65			0.70	0.15		0.65	0.47

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 21.6 Intersection LOS: C
Intersection Capacity Utilization 88.3% ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 3: Churchill Avenue N & Richmond Road



m Volume for 95th percentile queue is metered by upstream signal.

Lane Group	Ø1	Ø3	Ø7
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

	۶	→	•	•	←	•	•	†	~	/	ţ	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	ĵ»		ሻ	ĵ.	
Traffic Volume (vph)	19	134	49	110	307	45	25	303	69	21	340	53
Future Volume (vph)	19	134	49	110	307	45	25	303	69	21	340	53
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Width (m)	3.7	3.7	3.7	3.7	3.7	3.7	3.0	4.0	3.7	3.0	4.0	3.7
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	18.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6			7.6			7.6			7.6		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.967			0.987			0.972			0.980	
Flt Protected		0.995			0.988		0.950			0.950		
Satd. Flow (prot)	0	1717	0	0	1740	0	1565	1792	0	1565	1806	0
Flt Permitted		0.936			0.859		0.371			0.393		
Satd. Flow (perm)	0	1615	0	0	1513	0	611	1792	0	647	1806	0
Right Turn on Red	•		Yes	-		Yes			Yes		, , ,	Yes
Satd. Flow (RTOR)		23			8			16			11	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		222.5			63.6			184.9			45.3	
Travel Time (s)		16.0			4.6			13.3			3.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	21	149	54	122	341	50	28	337	77	23	378	59
Shared Lane Traffic (%)			<u> </u>	,	011			00.	.,		0.0	
Lane Group Flow (vph)	0	224	0	0	513	0	28	414	0	23	437	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2	_		6	-	
Minimum Split (s)	30.6	30.6		30.6	30.6		26.4	26.4		26.4	26.4	
Total Split (s)	45.0	45.0		45.0	45.0		45.0	45.0		45.0	45.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Maximum Green (s)	39.4	39.4		39.4	39.4		39.6	39.6		39.6	39.6	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?		·										
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		39.4			39.4		39.6	39.6		39.6	39.6	
Actuated g/C Ratio		0.44			0.44		0.44	0.44		0.44	0.44	
v/c Ratio		0.31			0.77		0.10	0.52		0.08	0.55	
Control Delay		16.1			30.6		16.2	20.4		18.9	25.7	
Queue Delay		0.0			0.0		0.0	0.0		0.0	1.4	
Total Delay		16.1			30.6		16.2	20.4		18.9	27.1	
LOS		В			00.0 C		В	20.4 C		10.3	C C	
Approach Delay		16.1			30.6			20.2			26.7	
Approach LOS		В			00.0 C			20.2 C			20.7 C	
Queue Length 50th (m)		21.6			71.9		2.8	48.3		2.8	56.5	
Quoue Length John (III)		۷۱.0			1 1.J		۷.0	+0.0		۷.0	50.5	

4: Churchill Avenue N & Byron Avenue

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (m)		37.7			#115.6		8.0	74.6		m6.0	90.0	
Internal Link Dist (m)		198.5			39.6			160.9			21.3	
Turn Bay Length (m)							15.0			18.0		
Base Capacity (vph)		719			666		268	797		284	800	
Starvation Cap Reductn		0			0		0	0		0	189	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.31			0.77		0.10	0.52		0.08	0.72	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.77

Intersection Signal Delay: 24.7 Intersection LOS: C
Intersection Capacity Utilization 74.2% ICU Level of Service D

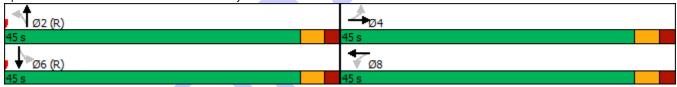
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Churchill Avenue N & Byron Avenue



Intersection							
Int Delay, s/veh	4.3						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	¥		f)			ની	
Traffic Vol, veh/h	39	57	43	37	14	43	
Future Vol, veh/h	39	57	43	37	14	43	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	e, # 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	42	62	47	40	15	47	
Major/Minor	Minor1	N	/lajor1		//ajor2		
Conflicting Flow All	144	67	0	0	87	0	
Stage 1	67	-	-	-	-	-	
Stage 2	77	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	849	997	-	-	1509	-	
Stage 1	956	-	-	-	-	-	
Stage 2	946	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	841	997	-	-	1509	-	
Mov Cap-2 Maneuver	841		-	-	-	-	
Stage 1	956	-	-	-	-	-	
Stage 2	937	-	-	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	9.4		0		1.8		
HCM LOS	Α						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT	
Capacity (veh/h)			-		1509		
HCM Lane V/C Ratio		_		0.113	0.01	_	
HCM Control Delay (s)		_	_	9.4	7.4	0	
HCM Lane LOS		_	_	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.4	0	-	



APPENDIX G: RESPONSE TO SCREENING AND SCOPING REPORT COMMENTS





The following email was received on September 12, 2022 regarding the Screening and Scoping Report submission. The responses in red font were prepared by the Consultant. All concerns were addressed in the subsequent Forecasting Report.

From: McMahon, Patrick <patrick.mcmahon@ottawa.ca>

Sent: Monday, September 12, 2022 7:59 AM **To:** Andrey Kirillov <a kirillov@castleglenn.ca>

Cc: Arthur Gordon <agordon@castleglenn.ca>; Jemmy Taing <jemmy@gsiproperties.ca>

Subject: RE: 424 Churchill Avenue North TIA Screening and Scoping Report

Hi Andrey,

Thank you for the submission, here are my comments:

Section 2.1.2.2: Include the pedestrian and cycling crossing treatments, as applicable.
 Response: Section 2.1.2.2 now includes a discussion on pedestrian and cycling treatments at each intersection, or a lack thereof.

 Section 2.1.2.6: Consider including the locations of the stops for the identified routes on Exhibit 2-13 or another figure.

Response: A new Exhibit 2-12 now includes locations of the 7 nearest bus stops and their corresponding bus routes.

 Section 2.1.3.1: Include the changes to Byron Avenue as part of the integrated road works project, see Ottawa.ca

Response: Section 2.1.3.1 now includes a discussion on changes to pedestrian and cycling infrastructure along Byron Avenue. The changes will be considered in the MMLOS segment analysis.

Thank you and please proceed to the forecasting report.

Best regards,

Patrick McMahon

Project Manager, Infrastructure Approvals | GPRJ Approbation des demandes d'infrastructure Development Review Branch | Dir Examen des projets d'aménagement

Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa

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web | Site Web : www.ottawa.ca



APPENDIX H: TDM-Supportive Design and Infrastructure Measures



TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

REQUIRED The Official Plan or Zoning By-law provides related guidance that must be followed BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	X
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	Refer to site plan
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	Churchill entrance provides best access to rapid transit
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	ř
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	X
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	□ _{N/A}
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	X
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	All spaces are secure
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	□ _{N/A}
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	□ N/A
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	□ N/A

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	X
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	□ _{N/A}
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	visitor parking at the front of garage



APPENDIX I: PARKING GARAGE ONE-WAY RAMP STRATEGY



Appendix I: Parking Garage One-Way Ramp Strategy

I-1. Parking Garage Requirements

The proposed development calls for a two-level parking garage (with the upper level referred to as "B2" and the lower level as "B3") connected by a single lane ramp. The ramp is characterized by a down grade between "B2" and "B3" which transition from 16% over a 10m distance, then 10% over a 5m distance and then a level surface with the "B3" level over the remaining 5m length of the ramp. Access up and down the ramp would be controlled by traffic signals to minimize any chance of conflict between an entering and exiting vehicle.

A review of vehicle turning movements was undertaken on both levels of the parking garage to identify maneuverability constraints which may be evident. The analysis assumed that a conventional passenger vehicle as defined by Transportation Association of Canada (TAC) standards was assumed to represent the design vehicle that would circulate through the parking garage. This vehicle is 5.6m in length, has a wheel base of 3.2m, a width of 2m and a front overhang of 1.1m. The TAC standard passenger vehicle is intended to incorporate the requirements of compacts, subcompacts, all light vehicles, and all light delivery trucks (vans and pick ups).

I-2. Turning Movement Analysis

The attached design sheets (Sheets 1) illustrate the turning movements and swept paths of vehicles circulating into, and out of, the upper B2 level of the parking garage and the proximity to adjacent parking stalls.

• Sheet 1: illustrates a TAC standard passenger vehicle leaving and entering the "B2" parking level in a single continuous movement. The maneuvers illustrate one-directional access into, and out of, the ramp connecting the B2 level to the B3 level.

The attached design sheets (Sheets 2-thru-7) illustrate the turning movements and swept paths of vehicles circulating into, and out of, the lower B3 level of the parking garage and the proximity to adjacent parking stalls.

- Sheet 2: illustrates a TAC standard passenger vehicle leaving and entering the "B3" parking level in a single continuous movement. The drawing indicates a potential conflict with two vehicle stalls parked against the southern wall of the garage in the front of the ramp. The maneuvers cannot succeed as the turning vehicle would encroach into the parked ones.
- Sheet 3: illustrates a situation where a TAC standard passenger vehicle would be required to make a 3-point turn into and out of the "B3" level to avoid conflicts with the two vehicle stalls identified on Sheet 1. This maneuver is facilitated by the relatively flat surface along the 5m of the ramp that transitions onto the "B3" level.
- Sheet 4: illustrates a situation where a TAC standard passenger vehicle leaves and enters the "B3" parking level in a single continuous movement, but when the two stalls parked against the southern wall are dedicated for smaller vehicles. For the purpose of this exercise a Honda Civic with a length of 4.67m was assumed to be parked in the two critical parking stalls. Given the presence of these shorter vehicles in the critical parking stalls, the turning movement of the TAC standard passenger vehicle was found to succeed.

- Sheet 5 and Sheet 6: illustrates a smaller 5.23m long Chevrolet vehicle and 5.04m long Acura MDX respectively leaving and entering the "B3" parking level in a single continuous movement when the same sized vehicles are parked in the two critical spaces. This maneuver was determined to succeed provided the smaller same size vehicles are parked way to the back wall.
- *Sheet 7*: illustrates the movements within the B3 level originating from, and destined to the west side of the B3 level lot and indicates that all movements can be successfully navigated.

I-3. Turning Movements Assumptions

The following assumptions were incorporated in the vehicle turning analyses:

- The vehicle dimensions (which include length, width, wheel base, overhangs and track) were obtained from the Canadian Association of the Road Safety Professional database.
 - The Honda Civic characteristics illustrated on Sheet 3 represent a 2.0L vehicle intended to represent all vehicles less than 5m in length.
 - The "Chevrolet" characteristics illustrated on Sheet 4 were derived from a 2023 Chevrolet Traverse 4DR SUV which was selected to represent all vehicles of approximately 5.2m in length.
 - The Acura MDX characteristics illustrated on Sheet 5 were derived from a 2023 ACURA MDX 4 Door vehicle which was selected to represent all vehicles of approximately 5.0 m in length.
- The steering angles and lock-to-lock time used in the assessment of vehicle turning maneuvers were assumed to be the same as the TAC Passenger car as provided in the Transoft Solutions AutoTURN software. Therefore, the simulation provided does not guarantee that the actual vehicle would maneuver as illustrated.

I-4. One-Way Ramp Strategy

The 15 parking stalls on level B3 (bottom parking level) will be accessed by way of a one-way ramp. This in turn means that vehicles entering and leaving level B3 may conflict with each other while on the ramp.

A strategy to avoid potential vehicle conflicts was developed to ensure only one direction (inbound or outbound) has the right-of-way to enter the ramp. The traffic entering the ramp from either level (outbound traffic from B3 to B2; and inbound traffic from B2 to B3) will be controlled by way of a traffic signal located at each ramp entrance.

The default phase for each traffic signal is red, thus prohibiting entrance to the ramp unless one of the following conditions is met:

• *To permit outbound movements (from level B3 up the ramp):* Motion detector 3 (level B3 exit) detects motion, while motion detectors 1 and 2 (level B2 entrance and along the ramp) detect no motion.

• To permit inbound movements (from level B2 down the ramp): Motion detector 1 (level B2 entrance) detects motion, while motion detectors 2 and 3 (along the ramp and level B3 exit) detect no motion.

The 4 (four) parking stalls on Level B2 nearest to the ramp were found to also cause a potential conflict with vehicles leaving parking Level B3. A series of auxiliary parking lights is to be placed at each stall indicating whether movement in and out of the stall is permitted. The movement is to be prohibited if motion sensor 2 or 3 (along the ramp and level B3 exit) detects any motion. The default condition for the auxiliary parking light permits movement in and out of the stalls.

To supplement the traffic signals, blind spot mirrors are recommended to provide some view of traffic on the ramp to the traffic entering and leaving the ramp. Exact location of the mirrors is to be confirmed.

Exhibit 1 provide approximate locations for the traffic signal lights, auxiliary parking lights and motion sensors / detectors. Table 1 and Table 2 summarize signal configurations and conditions for permitting inbound or outbound movements from and to level B3.

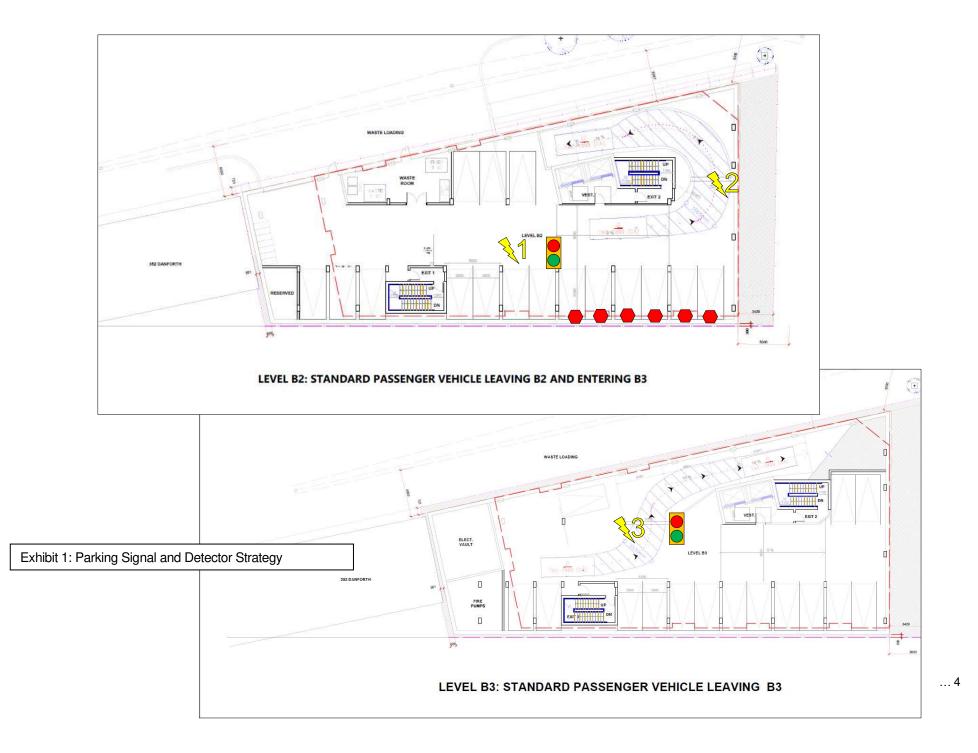


Table 1: Main Ramp Signal Phasing Configuration

Table 1: Main Kamp Signal Fhashig Comiguration									
Traffic Signal Phase		Condition	Explanation						
Level B2 (inbound)	Level B3 (outbound)								
"STOP"	"GO"	1 = no motion	Outbound movement permitting phase.						
		2 = no motion							
		3 = motion							
"GO"	"STOP"	1 = motion	Inbound movement permitting phase						
		2 = no motion							
		3 = no motion							
"STOP"	"STOP"	All other conditions	Default phase prohibits entry to the ramp unless either of the previous conditions is met						

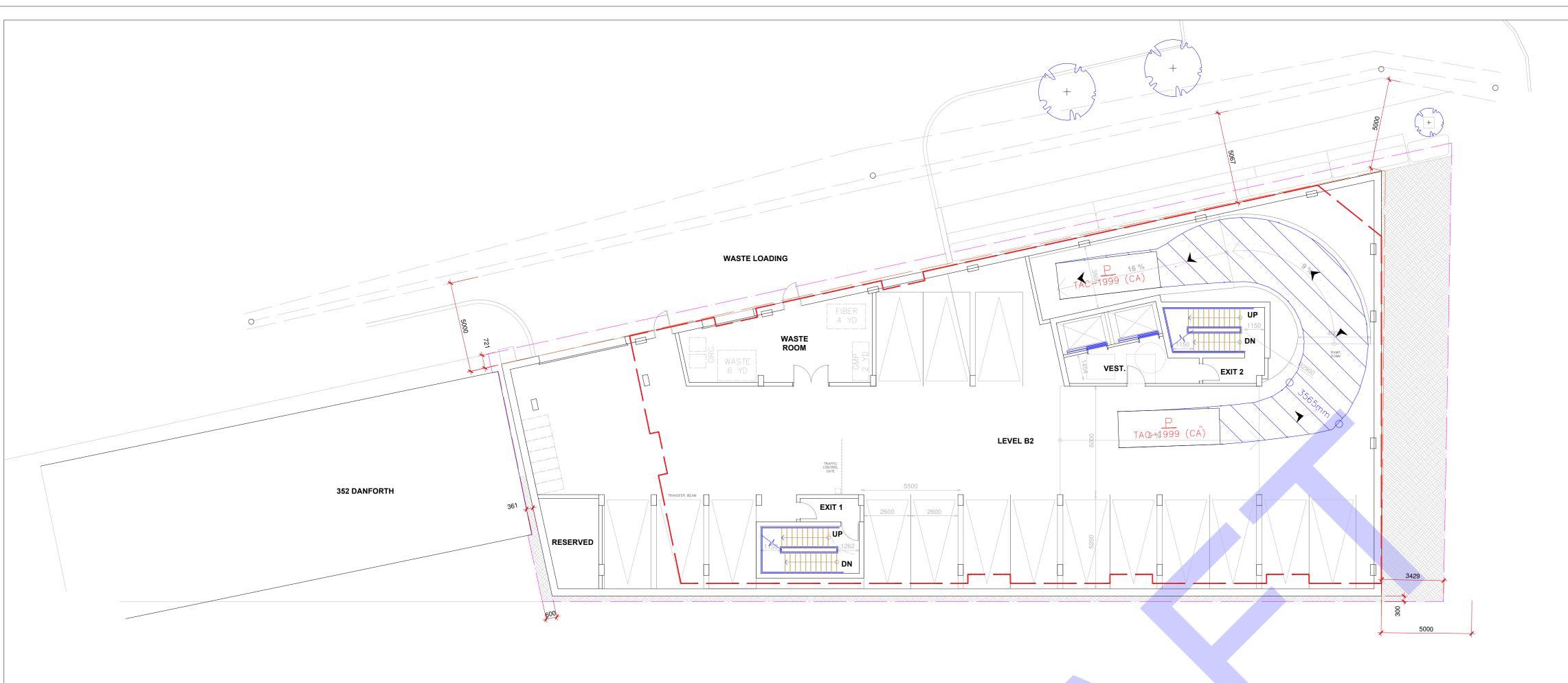
Table 2: Auxiliary Parking Signal Phasing Configuration

Auxiliary Parking Signal Phase	Condition	Explanation
Movement Prohibited	when sensor 2 or 3 detects motion	Do not permit movement out of the stall if there is motion on the bottom floor or along the ramp
Movement Permitted	All other conditions	Default condition permits movement out of the stall

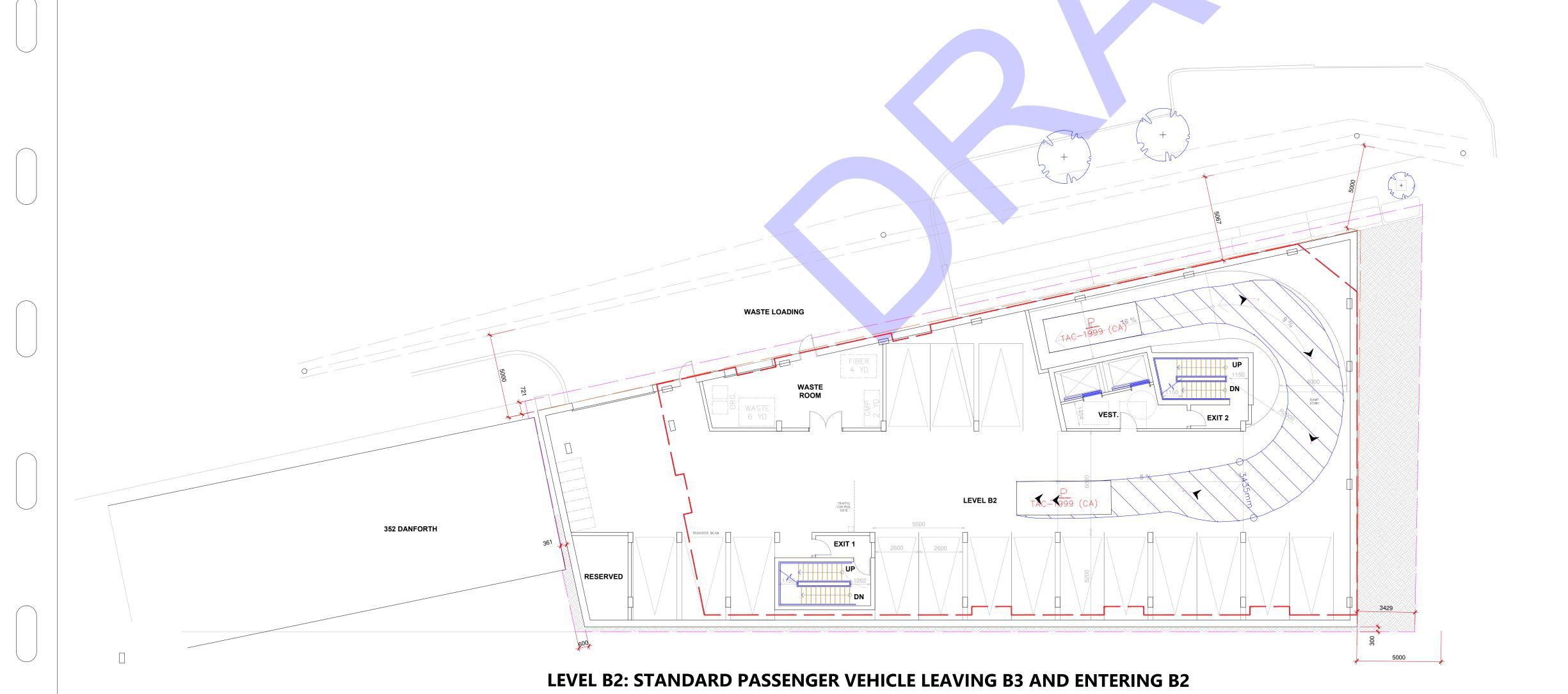
I-5. Conclusions and Recommendations

It was concluded from the above evaluation that:

- the two parking stalls nearest the "B3" ramp along the south wall of Level "B2" should be designated specifically for small vehicle parking only and not to exceed 4.7m in length;
- signage at the bottom of the Level "B3" ramp should be prominently displayed which indicates a 3 point-turn may be necessary to avoid parked vehicles and other obstacles;
- All leases, agreements with tenants should indicate that the two critical stalls are to be designated for small vehicles only not to exceed 4.7m in length;
- Given the design grade transition between the ramp segments, it is thought prudent that owners of vehicles characterized with low (less than 5") undercarriage clearances be cautioned that higher operational speeds on the ramps could well result in a "bottoming-out" effect and vehicle damage.
- To facilitate movement along a one-way ramp, a traffic signal solution activated upon detecting motion in conflicting direction should be implemented.
- Auxiliary parking lights permitting or prohibiting movement out of the 6 parking stalls on Level B2 nearest tot the ramp are recommended.
- Blind spot mirrors are recommended along the ramp. The exact location of the mirrors is to be confirmed.



LEVEL B2: STANDARD PASSENGER VEHICLE LEAVING B2 AND ENTERING B3



424 CHURCHILL AVE N., CONDOMINIUM

VEHICLE TURNING ANALYSIS
TAC VEHICLE TURNING ON LEVEL B2

Castleglenn

Engineers, Project Managers & Planners

Sheet 01

Asset Group

Contract No.

es. Chk'd.

Dwn. Chk'd.

RM AEG

Utility Circ. No. Index No.

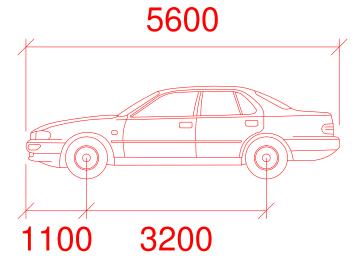
Cost. Inspector

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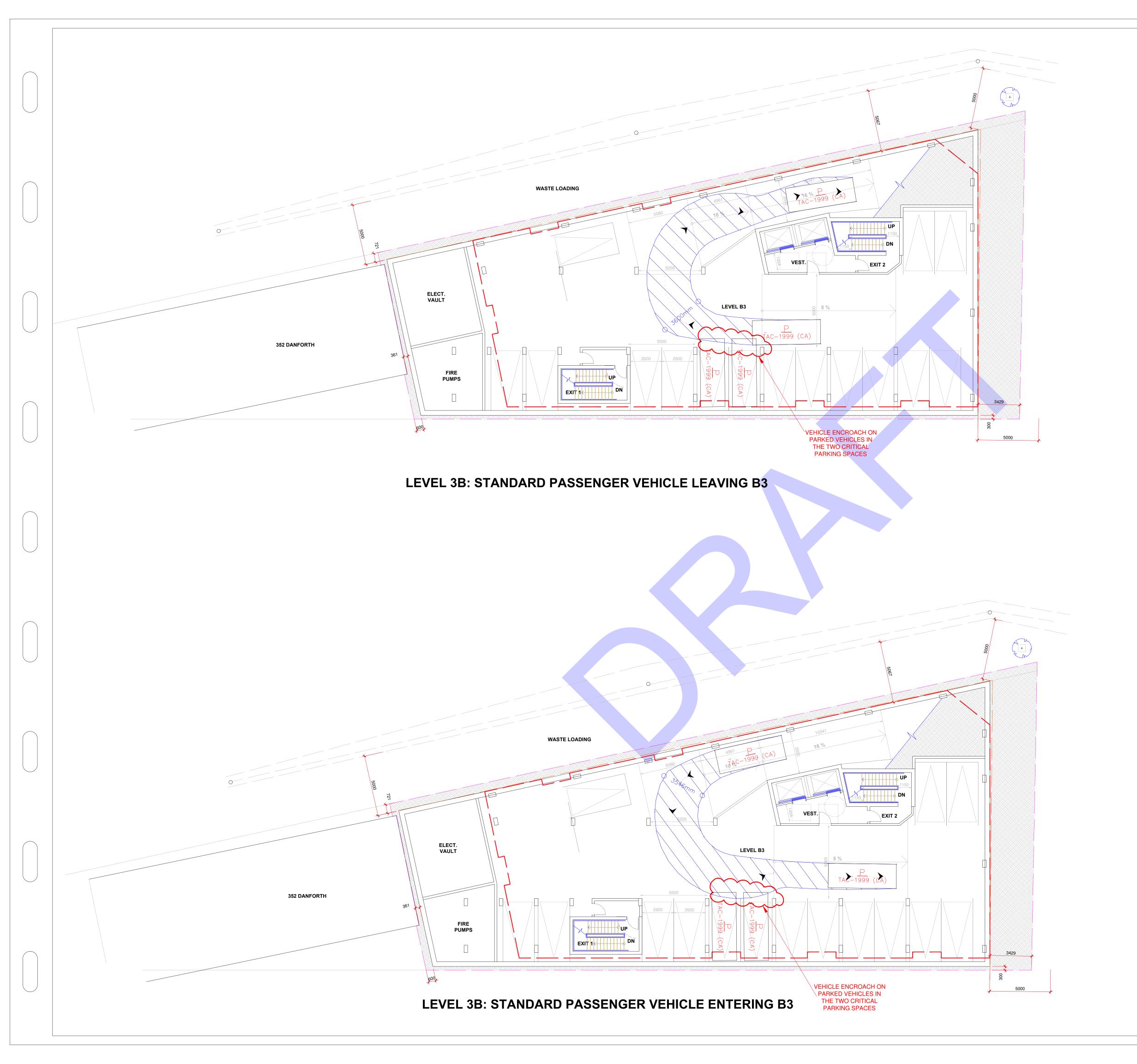
HORIZ 1:150

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

	No.	Description	Ву	Date (dd/mm/yy)
REVISIONS	1	XX		XX/XX/XX
	2			
	3			
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VEHICLE TURNING ANALYSIS
TAC PASSENGER CAR TURNING WHEN OTHER LONGER
VEHICLES ARE PARKED IN CRITICAL SPACES

Sheet 02
Asset No.

Contract No.

Asset Group

Des. Chk'd.

Castleglenn
Consultants
Engineers, Project Managers & Planners

Dwn.

Chk'd.

RM

AEG

Utility Circ. No.

Index No.

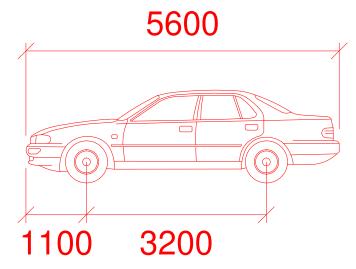
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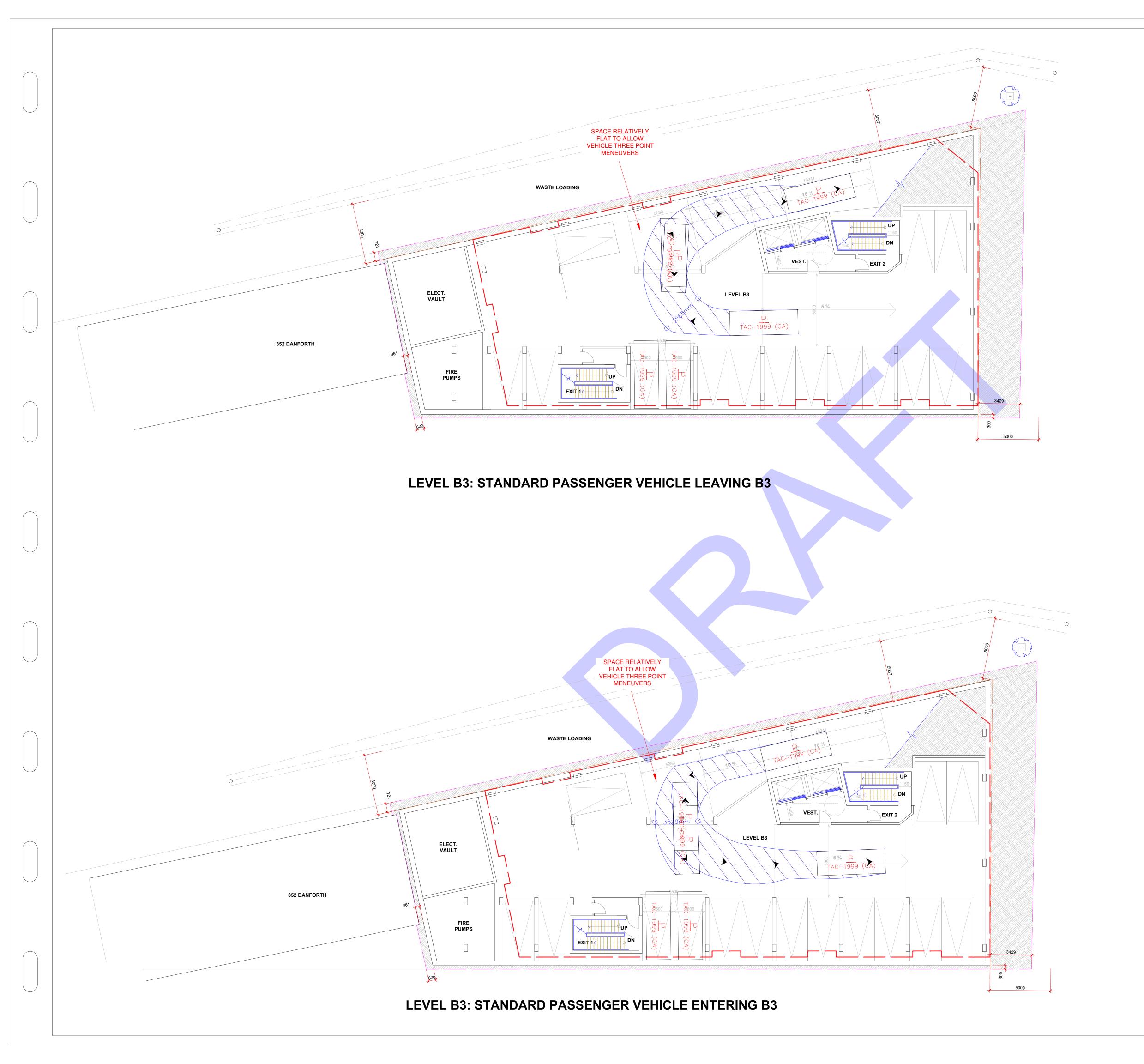
HORIZ 1:150

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REVIS	3			
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VEHICLE TURNING ANALYSIS
TAC PASSENGER CAR MAKING 3 POINT TURN

Asset No.

Asset Group

Sheet 03

Contract No.

RM
Dwn.
Castleglenn
Utility Circ. No.

Dwn.

RM
AEG

Utility Circ. No.
Index No.

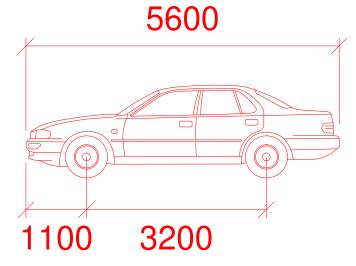
Cost. Inspector

HORIZ 1:150

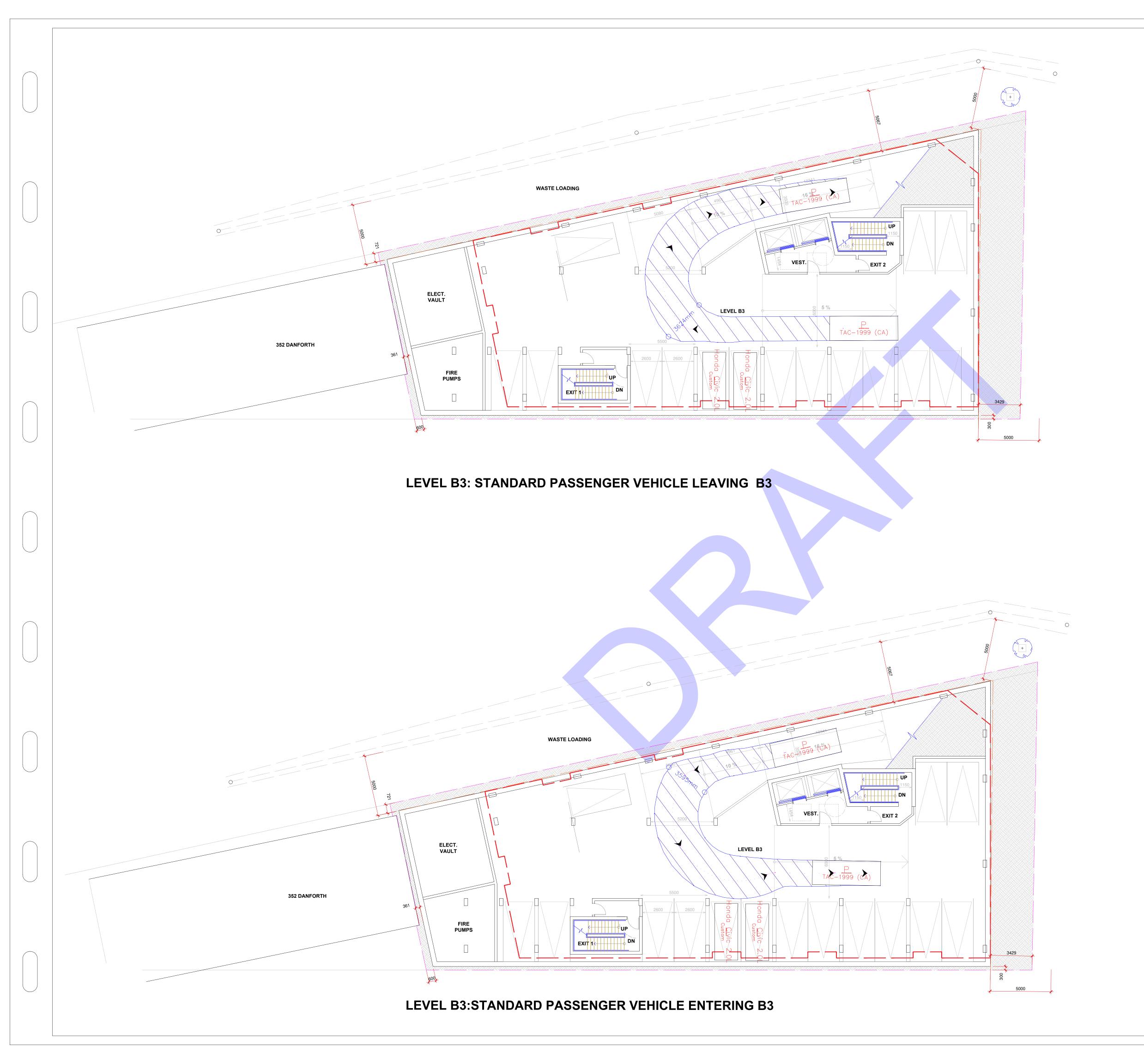
NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

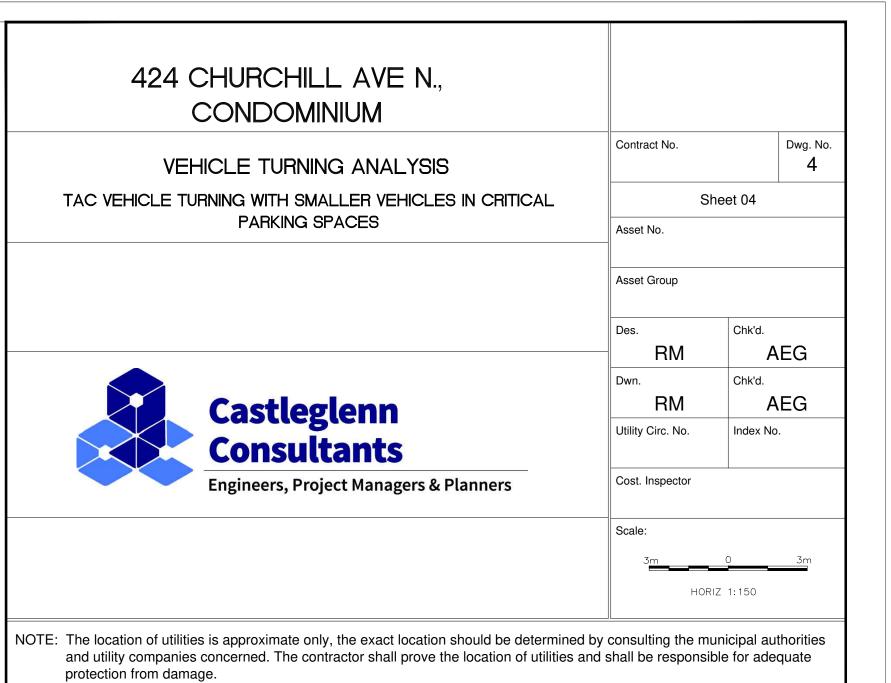
Engineers, Project Managers & Planners

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REVIS	3			
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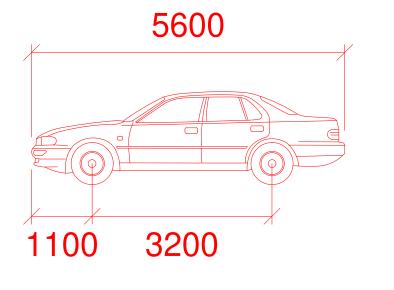




No. Description By Date (dd/mm/yy) 1 XX XXXXXXX 2

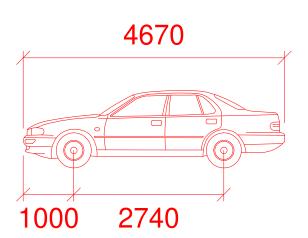
NOTES:

1. THE HONDA CIVIC DIMENSIONS (WHICH INCLUDE LENGTH, WIDTH, WHEELBASE, OVERHANGS AND TRACK) WERE OBTAINED FROM THE CANADIAN ASSOCIATION OF THE ROAD SAFETY PROFESSIONAL DATABASE. THE WIDTH DOES NOT INCLUDE SIDE MIRRORS

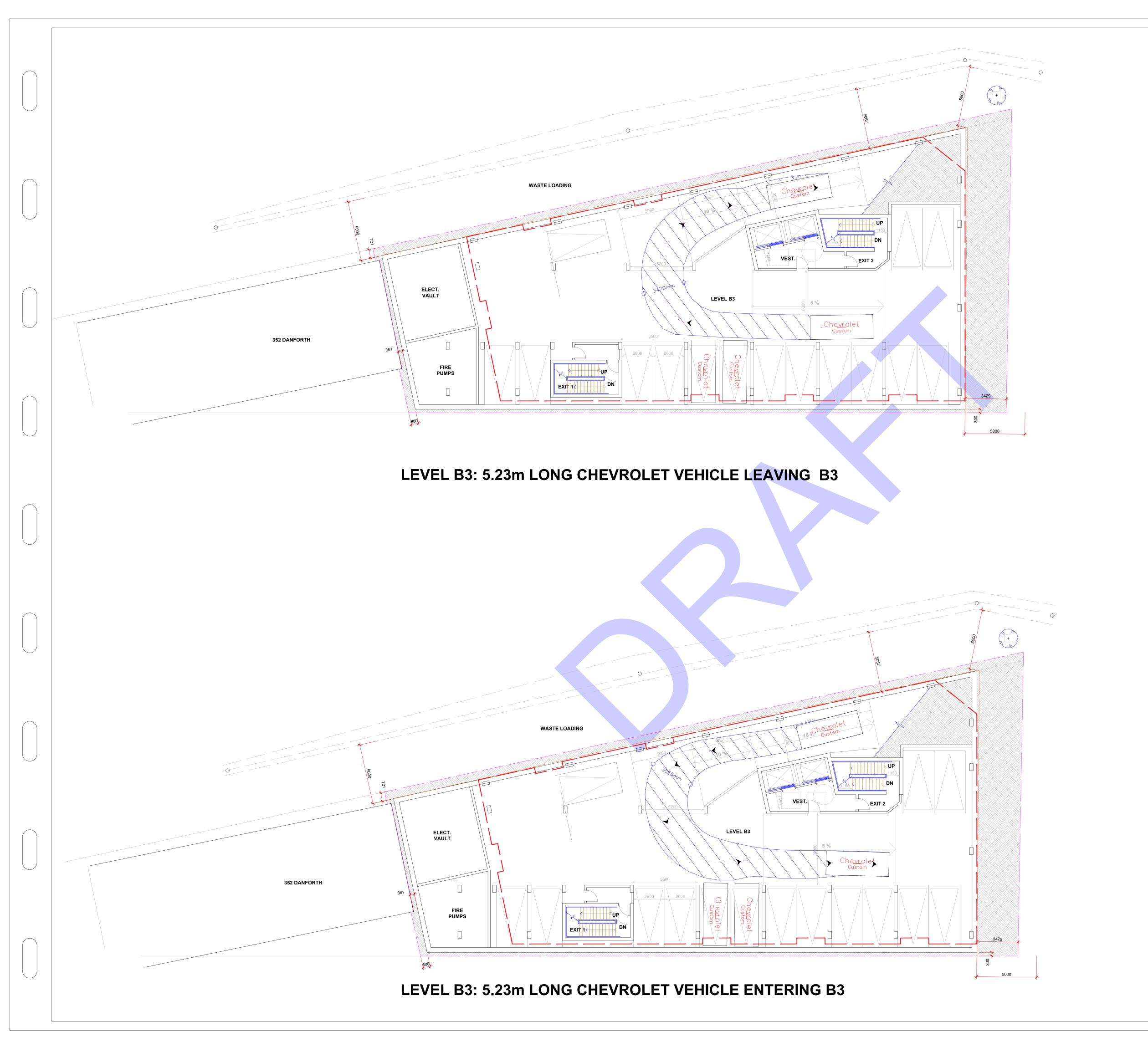


P mm

Width : 2000
Track : 2000
Lock to Lock Time : 6.0
Steering Angle : 36.2



Honda Civic 2.0L mm



VEHICLE TURNING ANALYSIS

5.23m LONG VEHICLE TURNING MENEUVERS

Asset Group

Des. Chk'd.

Sheet 05

Castleglenn
Consultants
Engineers, Project Managers & Planners

Des.

RM
AEG

Dwn.
Chk'd.
RM
AEG

Utility Circ. No.
Index No.

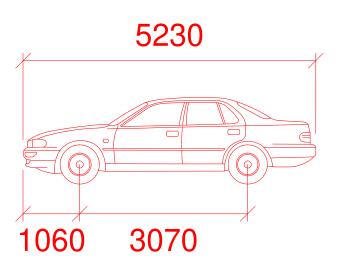
Cost. Inspector

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

	No.	Description	Ву	Date (dd/mm/yy)
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SNOIS	2			
REVIS	3			
1 "				

NOTES:

- 1. THE VEHICLE DIMENSIONS (WHICH INCLUDE LENGTH, WIDTH, WHEELBASE, OVERHANGS AND TRACK) WERE OBTAINED FROM THE CANADIAN ASSOCIATION OF THE ROAD SAFETY PROFESSIONAL DATABASE. THE WIDTH DOES NOT INCLUDE SIDE MIRRORS.
- 2. THE STEERING ANGLE AND LOCK TO LOCK TIME USED IN THE HONDA CIVIC WERE ASSUMED TO BE THE SAME AS THE TAC PASSENGER CAR AS PROVIDED IN THE TRANSOFT SOLUTIONS AUTOTURN SOFTWARE. THEREFOR THE SIMULATION DOES NOT GUARANTEE THAT THE ACTUAL VEHICLE WOULD MANEUVER AS ILLUSTRATED.

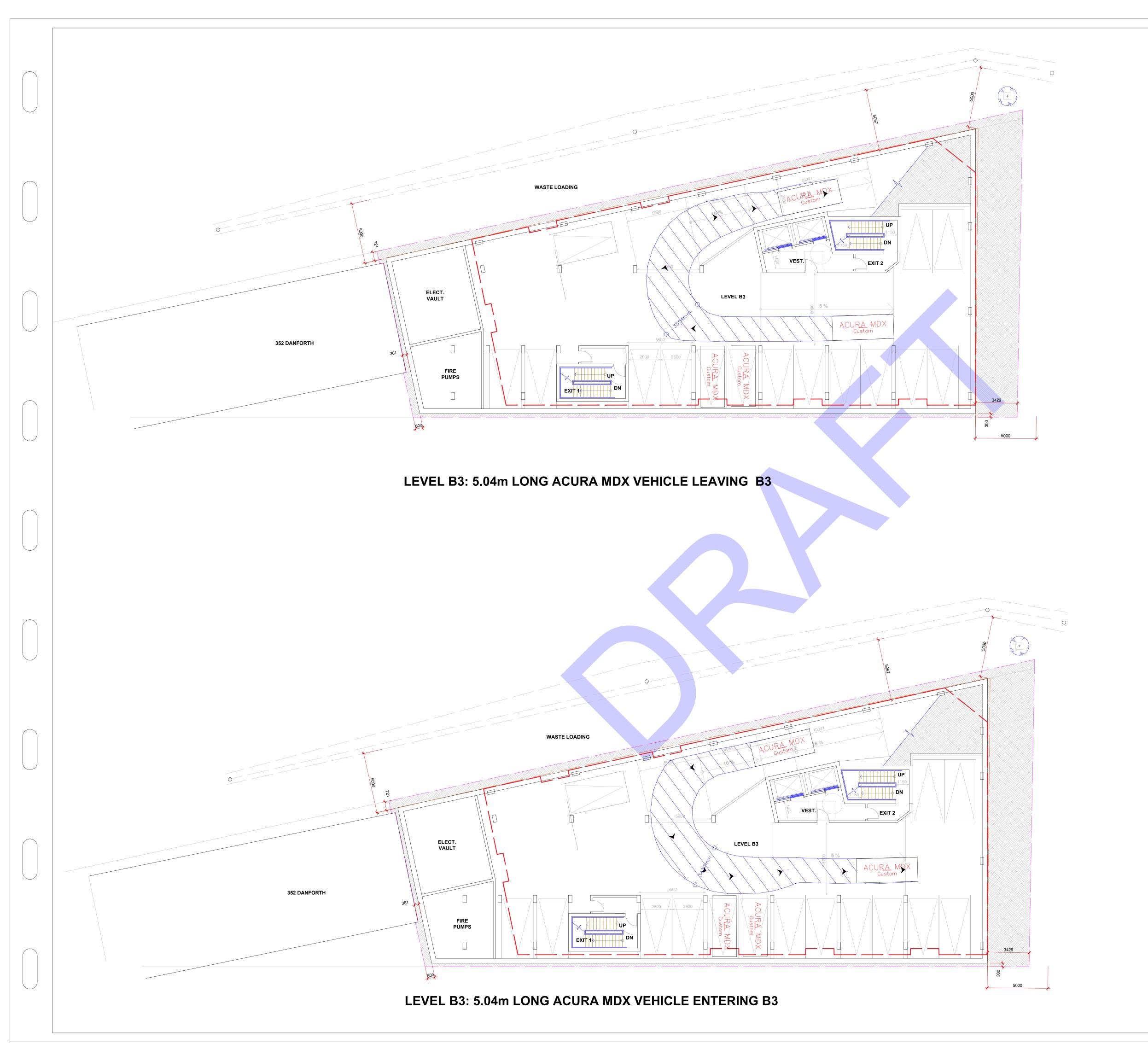


Chevrolet

Width : 2000
Track : 1700
Lock to Lock Time : 6.0

mm

Steering Angle : 36.2



VEHICLE TURNING ANALYSIS
5.04m LONG VEHICLE TURNING MENEUVERS

Castleglenn

Consultants

Engineers, Project Managers & Planners

Asset No.

Asset Group

Des.

RM

AEG

Dwn.

Chk'd.

RM

AEG

Utility Circ. No.

Index No.

Cost. Inspector

Scale:

HORIZ 1:150

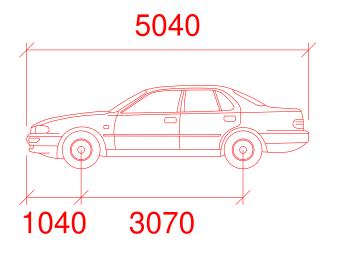
Sheet 06

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

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REVIS	3			
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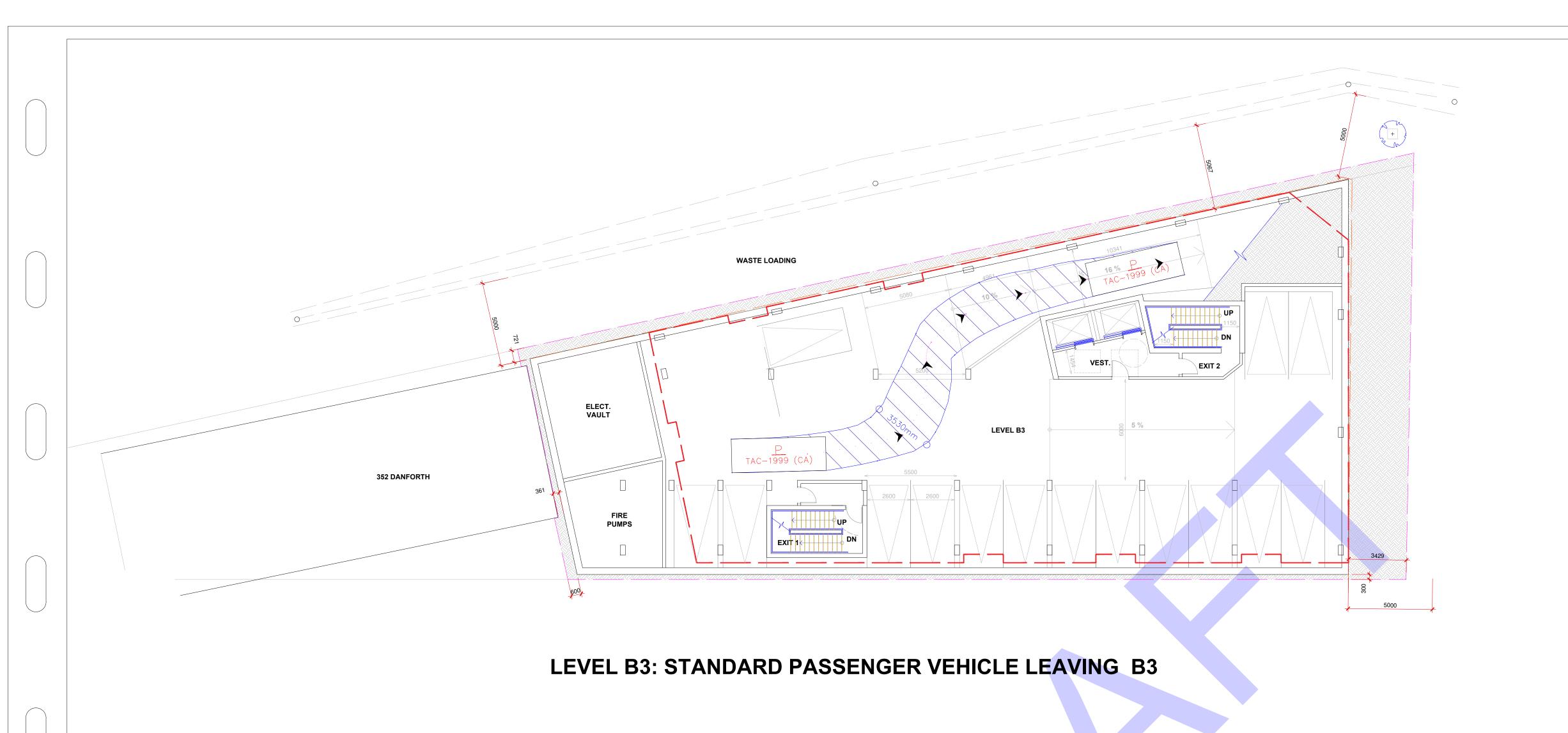
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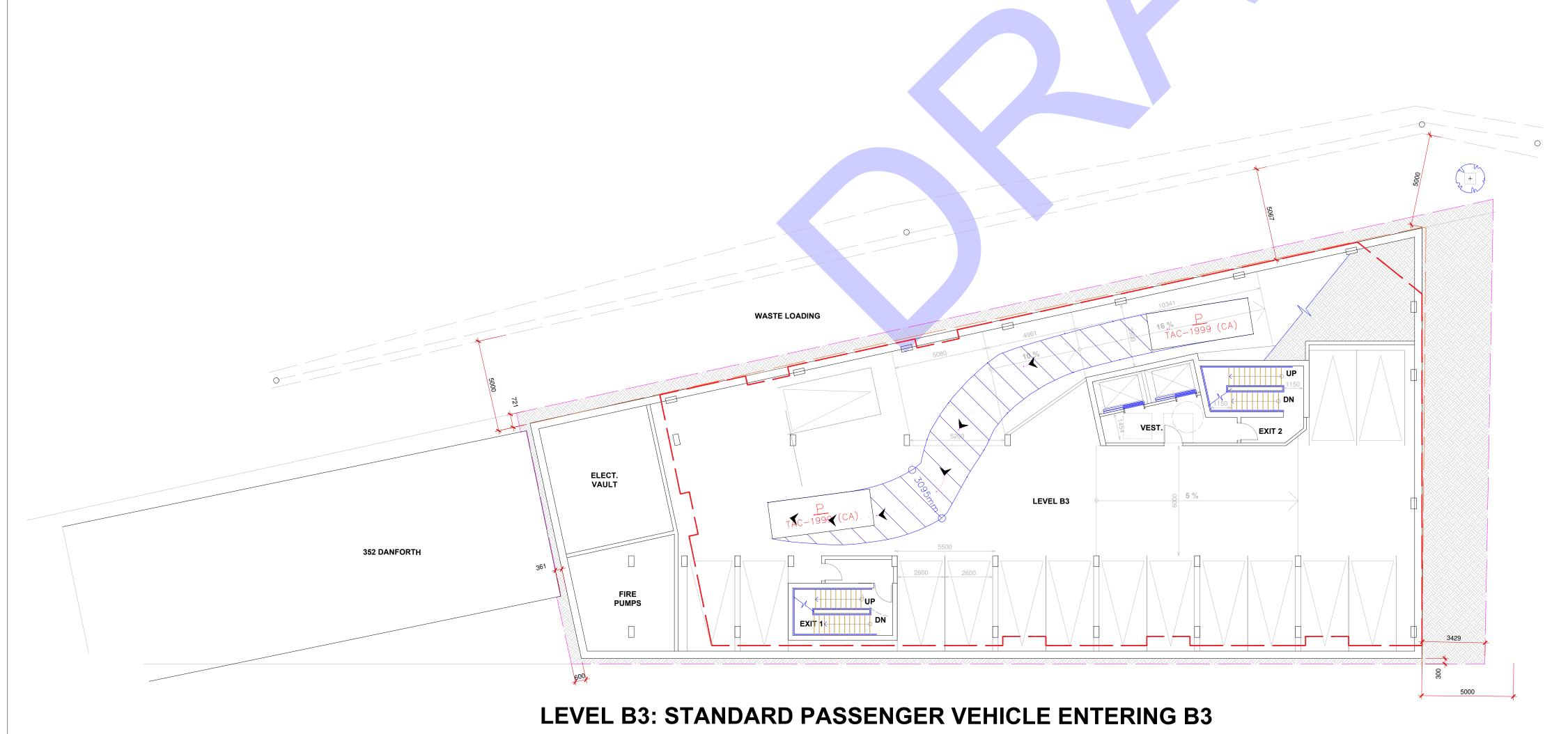
- 1. THE VEHICLE DIMENSIONS (WHICH INCLUDE LENGTH, WIDTH, WHEELBASE, OVERHANGS AND TRACK) WERE OBTAINED FROM THE CANADIAN ASSOCIATION OF THE ROAD SAFETY PROFESSIONAL DATABASE. THE WIDTH DOES NOT INCLUDE SIDE MIRRORS.
- 2. THE STEERING ANGLE AND LOCK TO LOCK TIME USED IN THE HONDA CIVIC WERE ASSUMED TO BE THE SAME AS THE TAC PASSENGER CAR AS PROVIDED IN THE TRANSOFT SOLUTIONS AUTOTURN SOFTWARE. THEREFORE THE SIMULATION DOES NOT GUARANTEE THAT THE ACTUAL VEHICLE WOULD MANEUVER AS ILLUSTRATED.



ACURA MDX

mm





VEHICLE TURNING ANALYSIS

TAC VEHICLE TURNING TO AND FROM WEST OF B3

Asset No.

Asset Group

Contract No.

Castleglenn
Consultants
Engineers, Project Managers & Planners

Des. Chk'd.

RM AEG

Dwn. Chk'd.

RM AEG

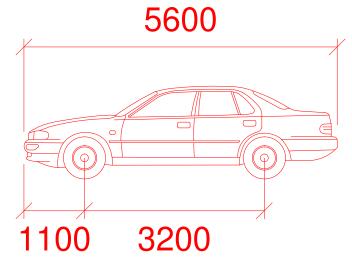
Utility Circ. No. Index No.

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HORIZ 1:150

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

REVISIONS	No.	Description	Ву	Date (dd/mm/yy)
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APPENDIX J: RESPONSE TO FORECASTING REPORT COMMENTS





The following email was received on October 25, 2022 regarding the Forecasting Report submission. Below, in red font, are the consultant responses to each issue raised which have been addressed within this Strategy Report.

From: McMahon, Patrick <patrick.mcmahon@ottawa.ca>

Sent: Tuesday, October 25, 2022 11:32 AM **To:** Andrey Kirillov <a kirillov@castleglenn.ca>

Cc: Arthur Gordon <agordon@castleglenn.ca>; Jemmy Taing <jemmy@gsiproperties.ca>

Subject: RE: 424 Churchill Avenue North TIA Forecasting Report

Hi Andrey,

Here are the comments for the forecasting submission:

Transportation Engineering Services

- Section 2.1.1.1 Proposed Development: Please include estimated date of occupancy in this section.
 Response: Added "The development is envisioned to be fully occupied by the end of 2025." (See Section 4.1.1 3
 Paragraph)
- 2. Section 2.1.2.2 [Existing] Study Area Intersections: For the Roosevelt Avenue and Byron Avenue intersection, the fifth bullet says, "The east leg of the intersection has a sidewalk along the south side of the corridor, and the north leg has a sidewalk along the west side of the corridor." It is the south leg (not the north leg) that has a sidewalk along the west side only.

Response: This has been corrected in the text. (See Section 4.1.2 2 Intersection No. 2)

3. For the Richmond Road and Churchill Avenue North intersection, the northbound and southbound approach lane arrangement has changed. Refer to the latest Google Street View imagery from July 2022. The northbound approach now includes a single shared through-right lane and an auxiliary left turn lane, while the southbound approach includes a single shared all-movement lane. The southbound left-turn movement is restricted between 3:30PM and 5:30PM, Monday to Friday.

Response: The intersection configuration has been updated in the text. (See Section 4.1.2 2 Intersection No. 3)

Synchro analysis was performed using the updated lane arrangement, southbound movement was found to fail due to reduced capacity.

- 4. Section 2.1.2.4 [Existing] Pedestrian and Cycling Facilities: Include a description of the multi-use pathway (MUP) that runs parallel to Byron Avenue and is identified as a major pathway in the ultimate cycling network.
 - Response: A paragraph addressing existing MUP (and planned future upgrades) was added. (See Last paragraph of Section 4.1.2 4)
- 5. Section 2.1.2.6 Existing Transit Provisions: Bus stop #4860 no longer exists. It has been replaced by the new eastbound bus stop #4870, located west of the Richmond Road and Churchill Avenue North intersection. Bus stop #4870 accommodates Route #11 and Route #153.
 - Response: Exhibit 2-12 has been updated to reflect the above noted conditions. (See Exhibit 4-12)
- 6. Section 2.1.2.7 Existing Peak Hour Travel Demands by Mode: At the bottom of page 20 it is stated that "all other intersections' target minimum desirable level of service is LOS 'D'". However, all intersections in the study area have a target auto LOS of 'E' because they are either within 600m of a rapid transit station or within 300m of a school (Churchill Alternative).

Response: Section 2.1.2.7 has been updated to indicate the distances to Churchill Alternative school. Text has been amended to indicate that all intersections in the study area have a target LOS "E" (See Section 4.1.2.7)

7. Section 2.1.3.2 Other Study Area Developments: Include a description of 2070 Scott Street, a 25-storey residential tower with ground floor commercial that is currently under construction.



Response: 2070 Scott Street was added to the list of adjacent development initiatives, and the traffic it generates is now accounted for in 2025 and 2030 background and design traffic exhibits (See Section 4.1.3.2)

8. Section 3.1.1 Trip Generation and Mode Shares: The first sentence of Section 3.1.1.2 is truncated at the start. Please correct.

Response: This was a missing reference to Table 3-2. It is now added to the sentence, (See Section 5.1.1)

9. Trip generation of the existing land use (laundromat / dry cleaning service) should be estimated and/or acknowledged, and these trips should be deducted from the future background network before adding the new site generated volumes to find the future total traffic volumes.

Response: The traffic generated by existing land use was assumed to be low-to-negligible during the peak hours of travel demand, and thus was not included as a part of the calculation of site's net-effect on the traffic in the area. Given an already low auto vehicle traffic generation assumptions (7 vehicles in the AM and 8 vehicles in the PM), we do not believe additional reductions to traffic should be applied) (See Section 5.1.1.2)

10. Preliminary Comments on the Next Step (TIA Strategy) and the Site Plan: For Element 4.2.1 Parking Supply, note that Section 103 of the Zoning By-Law (Maximum Limit on Number of Parking Spaces Near Rapid Transit Stations) applies to this development. Ensure Element 4.2.1 includes discussion of the number of accessible parking spaces required in the Zoning By-law and the number provided.

Response: Thank you for the heads up – this has been incorporated into the Strategy report. (See Section 6.2.1)

- 11. Transportation Engineering Services does not support the loading bay proposed on Byron Avenue:
 - Section 4.6.5 3) of the new Official Plan states that "Development shall minimize conflict between vehicles and
 pedestrians and improve the attractiveness of the public realm by internalizing all servicing, loading areas,
 mechanical equipment and utilities into the design of the building, and by accommodating space on the site for
 trees, where possible."
 - If loading activities must occur on public right-of-way, loading activities should occur on Danforth Avenue. There is an existing area of no parking (but stopping permitted) on the south side of Danforth Avenue appropriately 15m west of Churchill Avenue North that could potentially serve as loading space.

Response: This was communicated to the architect and owner on October 28th, 2022. Viability of loading from Danforth Avenue will be assessed. (See Section 6.1.2)

12. The Draft 2023 Transportation Master Plan includes a cycling feasibility study to add cycling facilities on Churchill Avenue from Byron Avenue to Scott Street. The project will likely include design of a protected intersection at Byron Avenue and Churchill Avenue North to facilitate safe crossings and turning movements for cyclists travelling on the Byron Avenue bike lanes / cycle tracks and the Churchill Avenue cycle tracks. Protected intersections require additional space for pedestrian and cyclist circulation at the corners (refer to the City's Protected Intersection Design Guide for more information and minimum dimensions). Consequently, the City of Ottawa would require land for a large corner site triangle on the northwest corner of Byron Avenue and Churchill Avenue North. A 10m-by-10m corner site triangle is preferred if possible.

Response: This was communicated to the architect on November 3rd, 2022 to confirm the requirement for a 10m-by-10m sight triangle recognizing that the west leg of Byron Avenue is unlikely to accommodate cycling facilities in the future.

13. Infrastructure such as staircases, ramps, and retaining walls must not be located on public right-of-way. For example, the site plan shows a staircase leading to the 'Principal Entry 2' encroaching on the Churchill Avenue North right-of-way. There also appears to be a staircase to an 'Exit' encroaching on the Byron Avenue right-of-way. Please remove these encroachments in future revisions.

Response: This was communicated to the architect and owner on October 28th, 2022.

Traffic Engineering

14. North-south phases must be modelled as ped recalled in Synchro

Response: This Strategy report includes revised synchro analysis with ped recalled N-S phases along Churchill Ave N and Roosevelt Ave. (See Section 4.1.2.7 – Table 4-4 and Appendix "F")



15. There is an advance walk after the eastbound left turn and before the westbound thru phases. This must be included. Additionally, there are leading thru arrows displayed during the advance walks in the east-west directions. While it would be proper to model these advance walks with a thru arrow display, we can consider the omission of this as a conservative approach to the intersection's capacity analysis.

Response: The signal phasing for Richmond Road / Churchill Ave N intersection was revised. Section 4.1.2.7 along with Appendix "F" contains updated intersection capacity analysis

Thank you and proceed to step 4.

Best regards,

Patrick McMahon

Project Manager, Infrastructure Approvals | GPRJ Approbation des demandes d'infrastructure Development Review Branch | Dir Examen des projets d'aménagement Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa

Tel |Tél.: 613-580-2424 ext. | poste 23298

web | Site Web: www.ottawa.ca



APPENDIX K: MMLOS ANALYSIS WORKSHEET



					Roadway Segments A	Adjacent to the Development								
Performance Measure	Churchill Ave N b/w Richmond and Byron		Churchill Ave N b/v	Churchill Ave N b/w Byron and Ravenhill		Byron Ave b/w Roosevelt and Churchill		Byron Ave b/w Churchill and Athlone		Danforth Avenue				
	Northbound	Southbound	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Eastbound	Westbound				
					Pedestrian LOS (PLOS)									
Sidewalk Width (m)	>2	>2	>2	>2	1.8	0	>2	>2	0	0				
Boulevard Width (m)	0	0	0	>2 (segr bike lane)	0	0	0	0	0	0				
Average Daily Curb Lane Traffic Volume	>3000	>3000	>3000	>3000	<3000	>3000	<3000	>3000	N/A	N/A				
Presence of On-Street Parking	Yes	Yes	N/A	N/A	N/A	N/A	N/A	N/A	n/a	n/a				
Operating Speed (km/h)	50	50	50	50	50	50	50	50	50	50				
Segment PLOS	В	В	С	В	В	F	В	С	F	F				
Target PLOS	A	A	A	A	A	A	A	A	A	A				
					Bicycle LOS (BLOS)									
Bikeway Type	Mixed Traffic	Mixed Traffic	Physically Separated	Physically Separated	Bike Lane not adj to Parking	Mixed Traffic	Bike Lane not adj to Parking	Mixed Traffic	Mixed Traffic	Mixed Traffic				
Number of Lanes per direction	1	1	N/A	N/A	1	-1	1	1	1	1				
Bike Lane Width (m)	N/A	N/A	N/A	N/A	1.5 m = B	N/A	1.8 m =A	N/A	N/A	N/A				
Operating Speed (km/h)	50	50	N/A	N/A	50 = A	50	50 = A	50	50	50				
Bike Lane Blockage	N/A	N/A	N/A	N/A	Rare = A	N/A	Rare = A	N/A	N/A	N/A				
Segment BLOS	D	D	A	A	В	D	A	D	В	В				
Designation	Spine Route	Spine Route	Spine Route	Spine Route	Major Pathway / Local	Major Pathway / Local	Major Pathway / Local	Major Pathway / Local	Local	Local				
Target BLOS	В	В	В	В	С	С	C	C	D	D				
					Transit LOS (TLOS)									
Facility Type	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	N/A	N/A	N/A	N/A	N/A	N/A				
Level/Exposure to Parking/Driveway Friction	Medium	Medium	Low	Low	N/A	N/A	N/A	N/A	N/A	N/A				
Segment TLOS	E	E	D	D	N/A	N/A	N/A	N/A	N/A	N/A				
Target TLOS	D	D	D	D	N/A	N/A	N/A	N/A	N/A	N/A				
					Truck LOS (TkLOS)									
Number of lanes (in each direction)	1	1	1	1	1	1	N/A	N/A	N/A	N/A				
Curb Lane Width (m)	>3.7	>3.7	3.5	>3.7	3.5	>3.7	N/A	N/A	N/A	N/A				
Segment TkLOS	C	C	C	В	В	В	N/A	N/A	N/A	N/A				
Target TkLOS	D	D	D	D	D	D	N/A	N/A	N/A	N/A				