



JFSA Canada Inc.
52 Springbrook Drive,
Ottawa, ON K2S 1B9
T 613-836-3884 F 613-836-0332

jfsa.com

November 08, 2024

Project Number: 959(03)

David Schaeffer Engineering Ltd
120 Iber Road, Unit 103
Ottawa, Ontario
K2S 1E9

Attention: Braden Kaminski, P.Eng

Subject: Cardinal Creek Village South –
Preliminary Stormwater Management Plan and Stormwater
Management Facility Design

Introduction

As requested by your office, JFSA Canada Inc. (JFSA) has evaluated, based on the provided information as described below; (i) the adequacy of the proposed minor system with respect to hydraulic grade line (HGL) analysis; and (ii) the storage required in the SWM facilities to meet quality and quantity control requirements for the proposed development at Cardinal Creek Village South. Note that this is an update of the December 21, 2021, version of this memo to reflect changes to DSEL's preliminary servicing and grading design. This includes changes to the draft plan of the subdivision, drainage areas, pipe data, and the pond stage-storage curve. Furthermore, the percent increase of the Rational Method flows used in the preliminary HGL analysis was applied and model simulations were completed reflecting the increase in runoff captured into the minor system during the 100-year storm to address a City of Ottawa review comment.

The proposed Cardinal Creek Village South site has a development area of approximately **45.82 ha**. **11.35 ha** of the proposed development as well as **0.68 ha** of external drainage area are tributary to the existing SWM Pond 1 and the stormwater management system for Cardinal Creek Village Phase 4. **32.54 ha** of the site will discharge to SWM Pond 2, which will provide quality control, erosion control and quantity control up to the 100-year level of service, before discharging to the South Tributary of Cardinal Creek. **1.93 ha** consisting primarily of rear yards will drain uncontrolled to the South Tributary of Cardinal Creek. Refer to **Figure 10** for the proposed drainage areas of the subject site.

Stormwater Management Facility (Pond 2)

As noted above, SWM Pond 2 will provide quality control for **32.54 ha** of the site with an average imperviousness of **66%**. Pond 2 also requires erosion control, provided based on the detention of the 25 mm storm runoff for a drawdown time of approximately **96 hours**. The effectiveness of this erosion control was confirmed by a continuous erosion analysis, as documented in the JFSA's June 21, 2013 "**Cardinal Creek Village / Continuous Erosion Analysis**" memo. It is important to note that the erosion thresholds identified in the June 2013 memo have been updated during the preparation of this report based on field work conducted by Geo Morphix Ltd. The continuous SWMHYMO erosion model was re-run based on the drainage area changes to Pond 2 and the updated erosion thresholds provided by Geo Morphix. Also, due to coordination that occurred during updates to the FSR design, the total proposed drainage area to Pond 2 used in the continuous erosion model updates (**33.20 ha**) was larger than the total proposed drainage area that will actually drain to Pond 2 (**32.54 ha**) as per DSEL's latest design, which is conservative considering that the pond size has not changed based on the drainage area reduction and the pond release rates to the South Tributary are now slightly less than the ones assumed in the updated continuous erosion model. The hydrographs generated by the updated continuous erosion model were provided to Geo Morphix and they subsequently prepared a preliminary erosion analysis. Based on Geo Morphix's preliminary erosion analysis results, it is anticipated that the proposed post-development scenario is acceptable from an erosion perspective. A detailed erosion analysis will be prepared in the detailed design stage of Cardinal Creek Village South.

Pond 2, discharging to the South Tributary of Cardinal Creek, also requires 2- to 100-year post-to pre-development quantity control. Target release rates for Pond 2, as per the approved July 2013 "**Master Servicing Study for Cardinal Creek Village**" (MSS report) prepared by DSEL, were calculated based on existing flows simulated with AECOM's 2013 Cardinal Creek XPSWMM model for the 24-hour SCS Type II design storms, pro-rated by the existing drainage area from the subject site to the South Tributary of Cardinal Creek. This source is appropriate as it supersedes the AECOM August 2009 "**Greater Cardinal Creek Subwatershed Study - Existing Conditions**" study. These existing flows are specifically detailed in the JFSA's June 21, 2013 "**Cardinal Creek Village/Preliminary Stormwater Management Plan and Stormwater Management Facility Design**" included as **Appendix K** of the MSS report and reproduced in **Table A-2 of Attachment A**.

The proposed drainage area to Pond 2 was simulated using SWMHYMO modelling software to assess its performance and ensure the design requirements were met. The SWMHYMO model and associated files are included in **Attachment A**.

A summary of the proposed SWM facility operating conditions is presented in **Tables A-1 to A-5 of Attachment A**, including a comparison of the existing and proposed conditions flows from the subject site to the South Tributary of Cardinal Creek. All quantity control requirements were met by the proposed outlet controls, while still providing a 0.3 m freeboard between the maximum water level in the pond and the top of bank elevation, and a maximum 100-year active storage depth of 2.0 m.

Pond 2 is equipped with one sediment forebay connected to the main cell of the pond by a standard forebay berm. Refer to **Attachment B** for preliminary calculations for the required sediment forebay dimensions for this SWM facility. Pond 2 will also be equipped with a bottom-draw outlet pipe to reduce the temperatures of the outflow to the South Tributary of Cardinal Creek.

Preliminary HGL Analysis

A preliminary hydraulic grade line analysis for the proposed Cardinal Creek Village South development was completed using PCSWMM modelling software. Pipe data, storm sewer layout and Rational Method flows in the storm sewer are as provided by DSEL. The Rational Method flows were calculated based on the 2-, 5- or 10-year level of service requirements, and the minor system flows used in the hydraulic grade line calculations were estimated as **35%** greater than the Rational Method flows, to account for the additional flows captured by catchbasin grates, lead pipes and/or inlet control devices under the higher surface water depths during the 100-year storm. The proposed storm sewer infrastructure data was provided by DSEL and incorporated into a PCSWMM model, and flows derived by DSEL's Rational Method calculations were then applied to each Maintenance Hole (MH) in the model as steady flows (using the baseline inflow option). Exit losses were applied to all storm sewer pipes in the system based on the angle of the downstream connection.

The maximum HGL obtained at each MH has been extracted and provided in **Table C1** in **Attachment C**. In absence of USF elevations for the site at this stage, the maximum HGL was compared to elevations 1.90 m below the road elevation as an assumed USF elevation. This will be updated in the detailed design stage once USF elevations are available.

An average freeboard of **2.68 m** from the top of MH was observed throughout the proposed development for the 100-year return period. With a minimum freeboard of **2.02m** at **MH-61**. As such it can be concluded that the proposed storm sewer infrastructure is sufficiently sized, to safely convey minor system flows from the development under various extreme conditions. A detailed HGL analysis will be prepared in the detailed design stage. The PCSWMM model and associated modelling files are provided electronically.

Drainage Area to Cardinal Creek Village Phase 4

As noted above, a total of **12.03 ha** (**11.35 ha** area from the northwest portion of Cardinal Creek Village South, as well as **0.68 ha** of external drainage area) is tributary to the north and is to be captured by the Phase 4 storm sewer network and drain to Pond 1. This area has an average imperviousness of **64%** according to **Figure 10**. As per the JFSA January 2020 SWM report for these lands, it was previously assumed that **15.59 ha** with an average imperviousness of **26%** would drain to the existing Cardinal Creek Village development/Pond 1.

While the proposed drainage area is less than the previously assumed drainage area, the proposed average imperviousness is more than the previously assumed average imperviousness. A preliminary analysis of the receiving storm sewer and Pond 1 within the existing Cardinal Creek Village development has been undertaken to verify the impacts on the storm sewer network and Pond 1 operation. Based on this preliminary analysis, it was found that under ultimate conditions, the receiving storm sewer network has sufficient capacity to accommodate the proposed drainage area of **12.03 ha** with an average imperviousness of **64%** with minimal impacts on the 100-yr HGL across the existing development. Additionally, based on the available design pond information, the proposed Pond 1 permanent pool, quality control and extended detention storage volumes are sufficient to provide quality treatment for the existing and proposed developments under ultimate conditions. Although Pond 1 outflows increase when compared to the previous outflows, this preliminary analysis found that the capacity of the existing culvert under Highway 174 would not be exceeded during the 100-year event.

Note that the analysis of Pond 1 under 100% blockage of the outlet controls, as well as the two sensitivity tests shown in the JFSA's December 2018 "**Design Brief for the Interim Stormwater Management Pond 1 for Phases 1 to 5 in Cardinal Creek Village**" are being re-evaluated based on the changes in drainage area and imperviousness to SWM Pond 1 as detailed above. These evaluations are expected to be supplemented with as-built information of Pond 1 when it becomes available. A detailed analysis of the HGL within the existing Cardinal Creek development, Pond 1 operation and peak flows to the existing culvert under Highway 174 will be prepared at the detailed design stage of the Cardinal Creek Village South development, to confirm if the existing storm sewer network, Pond 1 and culvert are sufficiently sized.

Uncontrolled Drainage Area to Cardinal Creek South Tributary

As noted above, **1.93 ha** of rear yard drainage areas with an average imperviousness of **29%** from Cardinal Creek Village South will drain uncontrolled to the southern tributary of Cardinal Creek. This area is to provide the southern tributary with clean runoff to mimic pre-development conditions. This area has been included in the SWMHYMO model and as seen in **Table A-2 of Attachment A**, the total outflow from Cardinal Creek Village South development including this uncontrolled drainage area does not exceed the target release rates/existing outflows. A full analysis of the peak flows to the tributary will be assessed at the detailed design stage.

Cox County Road Culvert

A **74.30 ha** area has been identified as the drainage area to a **900mm** concrete culvert underneath Cox Country Rd. A SWMHYMO model of the drainage area was built to simulate peak flows at the culvert in question to assess if the existing culvert's size is sufficient. A peak flow of **1.324 m³/s** was established by the model for the 25-year design event, the required level of service for this road.

A HY-8 model was assembled to assess the operating characteristics under the 25-year design event. Based on existing conditions, the 25-year water level was calculated to be **88.46 m**, which provides **0.81 m** of freeboard for this event. Based on the results of this analysis, this crossing has sufficient capacity to convey **2.28 m³/s** before overtopping; the 100-year flow for this location is **1.904 m³/s** and, as such, this culvert has greater than a 100-year level of service. See **Attachment D** for the full analysis of this crossing.

Conclusion

The memorandum confirms the following design conditions:

- Pond 2 is sufficiently sized to meet the existing release rates and erosion control requirements.
- The preliminary HGL analysis confirms the proposed storm sewer network connected to Pond 2 is sufficiently sized.
- A total of **12.03 ha** of drainage area with an average imperviousness of **64%** within the northwest portion of the proposed development will be treated by Pond 1.
- The **1.93 ha** of uncontrolled rear yard drainage areas with an average imperviousness of **29%** will discharge directly to the South Tributary of Cardinal Creek.
- The existing culvert at Cox County Rd is sufficiently sized.
- Pond 2's bottom-draw outlet pipe will reduce outflow temperatures to Cardinal Creek.

Yours truly,
JFSA Canada Inc.



Jonathon Burnett, B.Eng, P.Eng
Senior Water Resources Engineer



Paulo Pickart, B.Eng, P.Eng
Water Resources Project Engineer
(November 08, 2024 updates only)



cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects

Figures

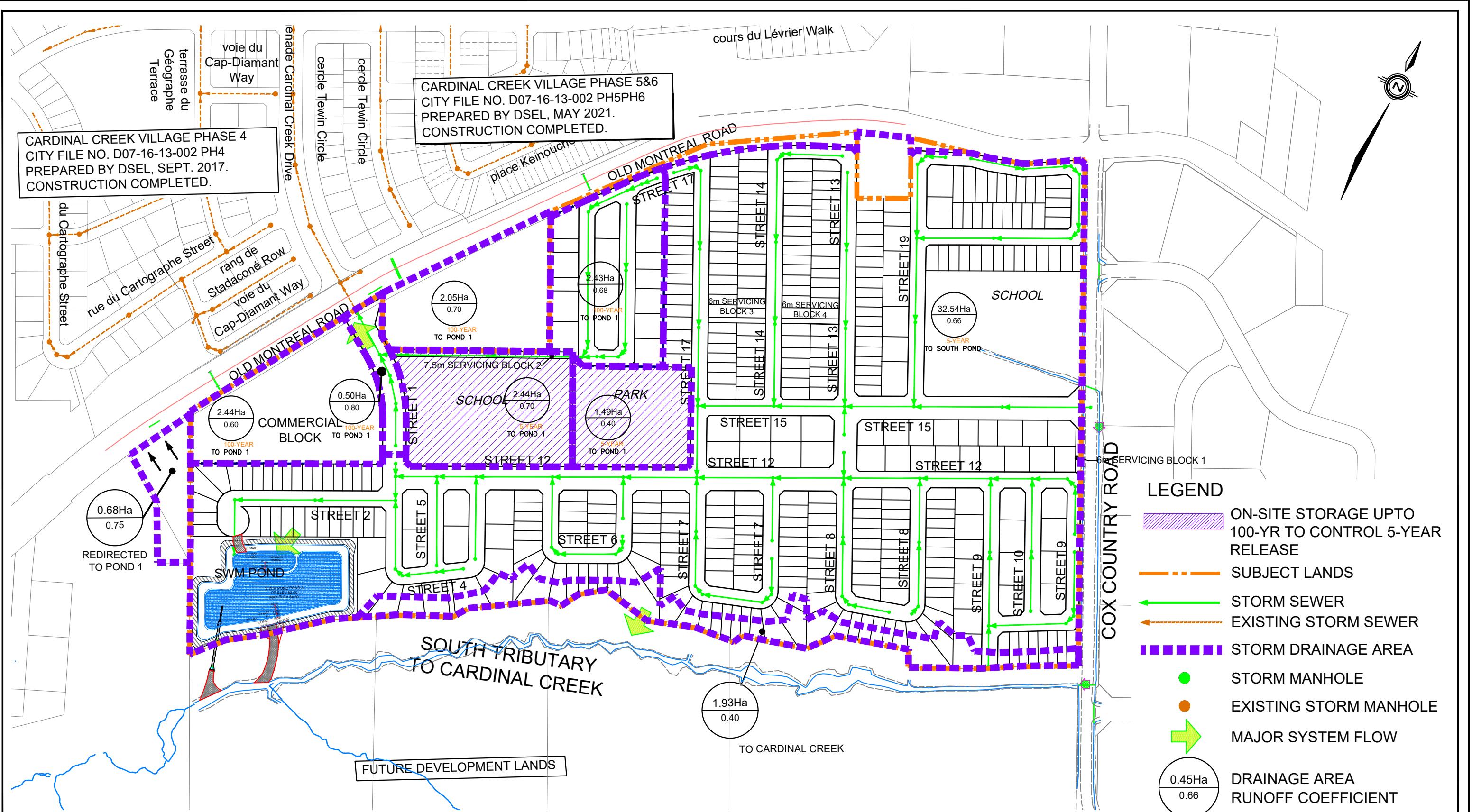
Figure 12: Post-Development Drainage Area Plan (DSEL)

Attachments

- Attachment A: Pond 2 Preliminary Summary Tables and Modelling Files
- Attachment B: Pond 2 Preliminary Forebay Calculations
- Attachment C: Storm Design Sheets (DSEL) & Preliminary HGL Analysis Results
- Attachment D: Cox Country Road Culvert Analysis

Modelling Files

PCSWM: CCVS_v02.2 (Provided Electronically)



120 Iber Road, Unit 103
Stittsville, ON K2S 1E9
TEL: (613) 836-0856
FAX: (613) 836-7183
www.DSEI.ca

POND DRAINAGE PLAN

CARDINAL CREEK VILLAGE SOUTH

PROJECT No.:	19-1153
SCALE:	1:4000
DATE:	NOVEMBER 2024
FIGURE:	10



JFSA Canada Inc.
52 Springbrook Drive,
Ottawa, ON K2S 1B9
T 613-836-3884 F 613-836-0332

jfsa.com

Attachment A

Pond 2 Preliminary Summary Tables and Modelling Files

Pond 2 - Preliminary Stage-Storage

Elevation	Volume (m3)	Volume above PP @ 95% (m3)	Area (m2)	Area @ 95% (m2)	Demarcation
80.00	0.000		2916.60	2770.77	Pond Bottom
80.05	147.502		2983.50	2834.33	
80.10	298.360		3050.79	2898.25	
80.15	452.594		3118.57	2962.64	
80.20	610.226		3186.72	3027.38	
80.25	771.277		3255.30	3092.54	
80.30	935.763		3324.16	3157.95	
80.35	1103.702		3393.40	3223.73	
80.40	1275.114		3463.07	3289.92	
80.45	1450.018		3533.10	3356.45	
80.50	1628.432		3603.45	3423.28	
80.55	1810.374		3674.21	3490.50	
80.60	1995.863		3745.37	3558.10	
80.65	2184.919		3816.88	3626.04	
80.70	2377.563		3888.86	3694.42	
80.75	2573.813		3961.16	3763.10	
80.80	2773.686		4033.73	3832.04	
80.85	2977.198		4106.79	3901.45	
80.90	3184.374		4180.23	3971.22	
80.95	3395.230		4254.02	4041.32	
81.00	3609.844		4330.52	4113.99	
81.05	3841.193		4675.12	4441.36	
81.10	4077.369		4771.90	4533.31	
81.15	4318.405		4869.55	4626.07	
81.20	4564.339		4967.82	4719.43	
81.25	4815.206		5066.85	4813.51	
81.30	5071.043		5166.62	4908.29	
81.35	5331.886		5267.11	5003.75	
81.40	5597.770		5368.27	5099.86	
81.45	5868.732		5470.21	5196.70	
81.50	6144.809		5572.87	5294.23	
81.55	6426.037		5676.23	5392.42	
81.60	6712.452		5780.37	5491.35	
81.65	7004.090		5885.18	5590.92	
81.70	7300.985		5990.62	5691.09	
81.75	7603.173		6096.90	5792.06	
81.80	7910.692		6203.86	5893.67	
81.85	8223.576		6311.52	5995.94	
81.90	8541.864		6419.99	6098.99	
81.95	8865.595		6529.24	6202.78	
82.00	9194.801		6639.00	6307.05	
82.05	9529.517		6749.64	6412.16	
82.10	9869.781		6860.90	6517.86	
82.15	10215.627		6972.96	6624.31	
82.20	10567.210		7090.33	6735.81	

Pond 2 - Preliminary Stage-Storage

Elevation	Volume (m3)	Volume above PP @ 95% (m3)	Area (m2)	Area @ 95% (m2)	Demarcation
82.25	10928.793		7373.01	7004.36	
82.30	11304.672		7662.14	7279.03	
82.35	11695.130		7956.19	7558.38	
82.40	12100.421		8255.44	7842.67	
82.45	12520.799		8559.66	8131.68	
82.50	12956.631	12308.799	8873.63	8429.95	Permanent Pool
82.55	13403.325	424.359	8994.13	8544.42	
82.60	13856.140	854.534	9118.45	8662.53	
82.65	14315.186	1290.627	9243.42	8781.25	
82.70	14780.507	1732.682	9369.41	8900.94	
82.75	15252.143	2180.736	9496.04	9021.24	
82.80	15730.258	2634.946	9628.56	9147.13	
82.85	16219.262	3099.499	9931.60	9435.02	
82.90	16718.184	3573.475	10025.25	9523.99	
82.95	17221.797	4051.908	10119.27	9613.31	
83.00	17730.121	4534.816	10213.70	9703.02	
83.05	18243.177	5022.219	10308.54	9793.11	
83.10	18760.985	5514.136	10403.77	9883.58	
83.15	19283.564	6010.586	10499.40	9974.43	
83.20	19810.937	6511.591	10595.52	10065.74	
83.25	20343.123	7017.167	10691.91	10157.31	
83.30	20880.140	7527.334	10788.78	10249.34	
83.35	21422.010	8042.110	10886.01	10341.71	
83.40	21968.752	8561.515	10983.67	10434.49	
83.45	22520.387	9085.568	11081.72	10527.63	
83.50	23076.933	9614.287	11180.16	10621.15	
83.55	23638.414	10147.694	11279.06	10715.11	
83.60	24204.845	10685.803	11378.20	10809.29	
83.65	24776.249	11228.637	11477.93	10904.03	
83.70	25352.648	11776.216	11578.03	10999.13	
83.75	25934.061	12328.559	11678.50	11094.58	
83.80	26520.508	12885.683	11779.37	11190.40	
83.85	27112.009	13447.609	11880.66	11286.63	
83.90	27708.583	14014.354	11982.30	11383.19	
83.95	28310.250	14585.938	12084.38	11480.16	
84.00	28917.031	15162.380	12186.88	11577.54	
84.05	29528.949	15743.702	12289.83	11675.34	
84.10	30146.021	16329.921	12393.05	11773.40	
84.15	30768.268	16921.055	12496.82	11871.98	
84.20	31395.709	17517.124	12600.83	11970.79	
84.25	32028.364	18118.146	12705.40	12070.13	
84.30	32666.257	18724.145	12810.30	12169.79	
84.35	33309.404	19335.134	12915.56	12269.78	
84.40	33957.825	19951.134	13021.28	12370.22	
84.45	34611.542	20572.165	13127.42	12471.05	

Pond 2 - Preliminary Stage-Storage

Elevation	Volume (m3)	Volume above PP @ 95% (m3)	Area (m2)	Area @ 95% (m2)	Demarcation
84.50	35270.636	21198.305	13236.33	12574.51	
84.55	35935.050	21829.498	13340.20	12673.19	
84.60	36604.704	22465.669	13445.96	12773.66	
84.65	37279.657	23106.875	13552.17	12874.56	
84.70	37959.936	23753.140	13658.97	12976.02	
84.75	38645.563	24404.485	13766.13	13077.82	
84.80	39336.622	25060.991	13876.21	13182.40	
84.85	40064.725	25752.689	15247.92	14485.52	
84.90	40863.266	26511.303	16693.72	15859.03	Top of Pond

Table A-1: Summary of Total Proposed Drainage Area

To SWM Facility	Area (ha)	Imperv. (%)	Area x Imp.	Required Storage ⁽¹⁾ (m ³)		
				Perm. Pool	Qual. Control	Eros. Control
Pond 2	32.54	66	2147.6	5727	1302	4696

⁽¹⁾ Quality control and permanent pool requirements based on MOE guidelines for enhanced quality control for wet ponds.

Erosion control based on 25 mm storm runoff volume for Pond 2, confirmed by 2013 continuous erosion analysis.

Table A-2: Simulated Release Rates and Volumes for Proposed SWM Facility 2 to South Tributary of Cardinal Creek ⁽¹⁾

Pond Component	Existing Outflow (m ³ /s)	SWM Facility 2 (32.54 ha)			CCVS Total Outflow ⁽⁴⁾ (m ³ /s)
		Pond Outflow (m ³ /s)	Prelim. Pond Level ⁽³⁾	Pond Storage (m ³)	
Permanent Pool ⁽²⁾	N/A	N/A	82.50	12309	N/A
Extended Detention ⁽²⁾	N/A	0.036	83.20	6512	N/A
2yr/24hr SCS	0.253	0.062	83.35	7977	0.151
5yr/24hr SCS	0.432	0.106	83.65	10790	0.260
10yr/24hr SCS	0.565	0.124	83.80	12800	0.335
25yr/24hr SCS	0.741	0.143	84.05	15330	0.443
50yr/24hr SCS	0.883	0.155	84.20	17260	0.523
100yr/24hr SCS	1.043	0.167	84.35	19330	0.596
July 1st, 1979	N/A	0.177	84.50	21100	N/A
August 4th, 1988	N/A	0.165	84.35	18890	N/A
August 8, 1996	N/A	0.157	84.20	17460	N/A

⁽¹⁾ Existing conditions flows as generated on subcatchments to south tributary as per Greater Cardinal Creek Subwatershed Study Existing Conditions XPSWMM hydrology model provided by AECOM on December 21, 2012, and pro-rated by drainage area (228.87 ha total, 31.20 ha through subject site). Post- to pre-development quantity control required for the 2- to 100-year design storms.

⁽²⁾ Extended detention based on 25 mm storm runoff volume with a drawdown time of 96 hours. Volumes are active storage only for all components except the permanent pool.

⁽³⁾ Preliminary elevations reported have been rounded up to the nearest 5cm.

⁽⁴⁾ Total Cardinal Creek Village South development outflow to South Tributary of Cardinal Creek, including 1.93 ha of uncontrolled rear yard drainage area.

Table A-3: Extended Detention Parameters for SWM Facility 2

Permanent Pool Parameters		Quality Orifice Parameters	
Area (C3)	8429.95 m ²	Diameter	0.145 m
Volume	12308.80 m ³		
PP Elev	82.500 m	Area	0.017 m ²
QC Elev	82.700 m	Invert	82.500 m
h (m)	0.200 m	C _o	0.62

- Notes:
- C3 is the intercept from the area-depth linear regression.
 - PP Elev indicates the elevation of the permanent pool.
 - QC Elev indicates the elevation of the storage volume required by MOE for quality control.
 - h is the maximum water elevation above the orifice (m).

Table A-4: Extended Detention Drawdown Time for SWM Facility 2

Elev. (m)	Active Storage			C2 (m ² /m)	Drawdown Time (h)	Drawdown Time (days)	Flow (m ³ /s)	Demarcation Point
	V (m ³)	A (m ²)	depth (m)					
82.50	0.00	8429.95	0.00				0.000	PP Elev
82.55	424.36	8544.42	0.05	2290	23.16	0.97	0.004	
82.60	854.53	8662.53	0.10	2326	32.91	1.37	0.008	
82.65	1290.63	8781.25	0.15	2342	40.49	1.69	0.013	
82.70	1732.68	8900.94	0.20	2355	46.97	1.96	0.016	QC Elev
82.75	2180.74	9021.24	0.25	2365	52.76	2.20	0.019	
82.80	2634.95	9147.13	0.30	2391	58.07	2.42	0.022	
82.85	3099.50	9435.02	0.35	2872	63.41	2.64	0.024	
82.90	3573.48	9523.99	0.40	2735	68.02	2.83	0.026	
82.95	4051.91	9613.31	0.45	2630	72.39	3.02	0.028	
83.00	4534.82	9703.02	0.50	2546	76.56	3.19	0.030	
83.05	5022.22	9793.11	0.55	2478	80.57	3.36	0.031	
83.10	5514.14	9883.58	0.60	2423	84.43	3.52	0.033	
83.15	6010.59	9974.43	0.65	2376	88.17	3.67	0.034	
83.20	6511.59	10065.74	0.70	2337	91.81	3.83	0.036	Ext. Det.
83.25	7017.17	10157.31	0.75	2303	95.41	3.98	0.042	
83.30	7527.33	10249.34	0.80	2274	98.43	4.10	0.052	
83.35	8042.11	10341.71	0.85	2249	100.92	4.20	0.063	
83.40	8561.51	10434.49	0.90	2227	102.97	4.29	0.078	
83.45	9085.57	10527.63	0.95	2208	104.75	4.36	0.086	
83.50	9614.29	10621.15	1.00	2191	106.40	4.43	0.093	
83.55	10147.69	10715.11	1.05	2176	107.95	4.50	0.099	
83.60	10685.80	10809.29	1.10	2163	109.42	4.56	0.105	
83.65	11228.64	10904.03	1.15	2151	110.82	4.62	0.110	
83.70	11776.22	10999.13	1.20	2141	112.17	4.67	0.115	
83.75	12328.56	11094.58	1.25	2132	113.48	4.73	0.120	
83.80	12885.68	11190.40	1.30	2123	114.74	4.78	0.125	
83.85	13447.61	11286.63	1.35	2116	115.97	4.83	0.129	
83.90	14014.35	11383.19	1.40	2109	117.17	4.88	0.134	
83.95	14585.94	11480.16	1.45	2104	118.33	4.93	0.138	
84.00	15162.38	11577.54	1.50	2098	119.48	4.98	0.142	
84.05	15743.70	11675.34	1.55	2094	120.60	5.03	0.146	
84.10	16329.92	11773.40	1.60	2090	121.71	5.07	0.150	
84.15	16921.06	11871.98	1.65	2086	122.79	5.12	0.153	
84.20	17517.12	11970.79	1.70	2083	123.86	5.16	0.157	
84.25	18118.15	12070.13	1.75	2080	124.91	5.20	0.160	
84.30	18724.14	12169.79	1.80	2078	125.95	5.25	0.164	
84.35	19335.13	12269.78	1.85	2076	126.97	5.29	0.167	100-year
84.40	19951.13	12370.22	1.90	2074	127.99	5.33	0.171	
84.45	20572.17	12471.05	1.95	2072	128.99	5.37	0.174	

Table A-4: Extended Detention Drawdown Time for SWM Facility 2

Elev. (m)	Active Storage			C2 (m ² /m)	Drawdown Time (h)	Drawdown Time (days)	Flow (m ³ /s)	Demarcation Point
	V (m ³)	A (m ²)	depth (m)					
84.50	21198.30	12574.51	2.00	2072	129.98	5.42	0.177	
84.55	21829.50	12673.19	2.05	2070	130.96	5.46	0.180	
84.60	22465.67	12773.66	2.10	2068	131.94	5.50	0.183	
84.65	23106.87	12874.56	2.15	2067	132.59	5.52	0.363	
84.70	23753.14	12976.02	2.20	2066	132.93	5.54	0.688	
84.75	24404.49	13077.82	2.25	2066	133.13	5.55	1.107	
84.80	25060.99	13182.40	2.30	2066	133.27	5.55	1.602	
84.85	25752.69	14485.52	2.35	2577	133.37	5.56	2.163	
84.90	26511.30	15859.03	2.40	3095	112.47	4.69	2.781	

Notes:

- C2 is the slope coefficient from the area-depth linear regression.
- PP Elev indicates the elevation of the permanent pool.
- QC Elev indicates the elevation of the storage volume required by MOE for quality control.
- Ext. Det. indicates the elevation of extended detention provided based on the detention of the 25 mm storm for a 96 hour drawdown time.
- Drawdown time is calculated based on Equation 4.11 of the MOE Guidelines up to the extended detention WSE. Above the extended detention WSE, the drawdown time is calculated based on the difference in incremental volumes divided by the average pond outflow, with the resulting time added to the previous drawdown time.

Table A-5: Stage-Storage-Outflow Curve for SWM Facility 2

			Quality Control 1		Quantity Control 1		Emergency Spillway			
			Vertical Orifice	Vertical Rect. Orifice	Broad Crested Weir					
			Dia (m)	0.145	Width (m)	0.250	L (m)	10.000		
			Area (m ²)	0.017	Height (m)	0.150				
			Invert (m)	82.50	Invert (m)	83.20	C _w	1.580		
			C _o	0.62	C _o	0.62	Invert (m)	84.60		
			Q @ D	0.012	C _w	1.800	n contr.	2		
Elevation	Active Sto.	Demarkation	Head	Outflow	Depth	Outflow	Head	Outflow	Outflow	Storage
(m)	(m ³)	Points	(m)	(m ³ /s)	(m)	(m ³ /s)	(m)	(m ³ /s)	(m ³ /s)	(ha·m)
82.50	0	PP Elev	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
82.55	424		0.050	0.004	0.000	0.000	0.000	0.000	0.004	0.042
82.60	855		0.100	0.008	0.000	0.000	0.000	0.000	0.008	0.085
82.65	1291		0.150	0.013	0.000	0.000	0.000	0.000	0.013	0.129
82.70	1733	QC Elev	0.200	0.016	0.000	0.000	0.000	0.000	0.016	0.173
82.75	2181		0.250	0.019	0.000	0.000	0.000	0.000	0.019	0.218
82.80	2635		0.300	0.022	0.000	0.000	0.000	0.000	0.022	0.263
82.85	3099		0.350	0.024	0.000	0.000	0.000	0.000	0.024	0.310
82.90	3573		0.400	0.026	0.000	0.000	0.000	0.000	0.026	0.357
82.95	4052		0.450	0.028	0.000	0.000	0.000	0.000	0.028	0.405
83.00	4535		0.500	0.030	0.000	0.000	0.000	0.000	0.030	0.453
83.05	5022		0.550	0.031	0.000	0.000	0.000	0.000	0.031	0.502
83.10	5514		0.600	0.033	0.000	0.000	0.000	0.000	0.033	0.551
83.15	6011		0.650	0.034	0.000	0.000	0.000	0.000	0.034	0.601
83.20	6512	Ext. Det.	0.700	0.036	0.000	0.000	0.000	0.000	0.036	0.651
83.25	7017		0.750	0.037	0.050	0.005	0.000	0.000	0.042	0.702
83.30	7527		0.800	0.039	0.100	0.013	0.000	0.000	0.052	0.753
83.35	8042		0.850	0.040	0.150	0.023	0.000	0.000	0.063	0.804
83.40	8562		0.900	0.041	0.200	0.036	0.000	0.000	0.078	0.856
83.45	9086		0.950	0.042	0.250	0.043	0.000	0.000	0.086	0.909
83.50	9614		1.000	0.044	0.300	0.049	0.000	0.000	0.093	0.961
83.55	10148		1.050	0.045	0.350	0.054	0.000	0.000	0.099	1.015
83.60	10686		1.100	0.046	0.400	0.059	0.000	0.000	0.105	1.069
83.65	11229		1.150	0.047	0.450	0.063	0.000	0.000	0.110	1.123
83.70	11776		1.200	0.048	0.500	0.067	0.000	0.000	0.115	1.178
83.75	12329		1.250	0.049	0.550	0.071	0.000	0.000	0.120	1.233
83.80	12886		1.300	0.050	0.600	0.075	0.000	0.000	0.125	1.289
83.85	13448		1.350	0.051	0.650	0.078	0.000	0.000	0.129	1.345
83.90	14014		1.400	0.052	0.700	0.081	0.000	0.000	0.134	1.401
83.95	14586		1.450	0.053	0.750	0.085	0.000	0.000	0.138	1.459
84.00	15162		1.500	0.054	0.800	0.088	0.000	0.000	0.142	1.516
84.05	15744		1.550	0.055	0.850	0.091	0.000	0.000	0.146	1.574
84.10	16330		1.600	0.056	0.900	0.094	0.000	0.000	0.150	1.633
84.15	16921		1.650	0.057	0.950	0.096	0.000	0.000	0.153	1.692
84.20	17517		1.700	0.058	1.000	0.099	0.000	0.000	0.157	1.752

Table A-5: Stage-Storage-Outflow Curve for SWM Facility 2

		Quality Control 1		Quantity Control 1		Emergency Spillway				
		Vertical Orifice	Vertical Rect. Orifice	Broad Crested Weir						
Elevation	Active Sto.	Demarkation Points	Head	Outflow	Depth	Outflow	Head	Outflow	Outflow	Storage
(m)	(m ³)		(m)	(m ³ /s)	(m)	(m ³ /s)	(m)	(m ³ /s)	(m ³ /s)	(ha·m)
84.25	18118	100-year	1.750	0.059	1.050	0.102	0.000	0.000	0.160	1.812
84.30	18724		1.800	0.060	1.100	0.104	0.000	0.000	0.164	1.872
84.35	19335		1.850	0.060	1.150	0.107	0.000	0.000	0.167	1.934
84.40	19951		1.900	0.061	1.200	0.109	0.000	0.000	0.171	1.995
84.45	20572		1.950	0.062	1.250	0.112	0.000	0.000	0.174	2.057
84.50	21198		2.000	0.063	1.300	0.114	0.000	0.000	0.177	2.120
84.55	21829		2.050	0.064	1.350	0.116	0.000	0.000	0.180	2.183
84.60	22466		2.100	0.065	1.400	0.119	0.000	0.000	0.183	2.247
84.65	23107		2.150	0.065	1.450	0.121	0.050	0.176	0.363	2.311
84.70	23753		2.200	0.066	1.500	0.123	0.100	0.499	0.688	2.375
84.75	24404		2.250	0.067	1.550	0.125	0.150	0.915	1.107	2.440
84.80	25061		2.300	0.068	1.600	0.127	0.200	1.408	1.602	2.506
84.85	25753		2.350	0.068	1.650	0.129	0.250	1.965	2.163	2.575
84.90	26511		2.400	0.069	1.700	0.131	0.300	2.581	2.781	2.651

Notes :

- PP Elev indicates the elevation of the permanent pool.
- QC Elev indicates the elevation of the storage volume required by MOE for quality control.
- Ext. Det. indicates the elevation of extended detention provided based on the detention of the 25 mm storm.
- Ovf Elev indicates the elevation of the emergency overflow provided above the 100-year water level.

```

00001> 20 Metric units / ID Numbers OFF
00002> *-----+
00003> *-----+ SWMHYMO Ver5.4/Prec 2015 / PREC DATA FILE
00004> *-----+
00005> * Project Name [Cardinal Creek Village South]
00006> * Project Number [ ]
00007> * Date [2024/10/29]
00008> * Modeler [FPI]
00009> * Company [ ]
00010> * License # [2349237]
00011> *-----+
00012> * 25 mm Storm based on 2-Year, 1-Hour Chicago Storm
00013> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
00014> *-----+
00015> *-----+ ["*25MMCH.stm"] <-storm filename, one per line for NSTORM time
00016> READ STORM STORM_FILENAME="*storm_001"
00017> *-----+
00018> DEFAULT VALUES ICAREdfr=[1], read and print values
00019> *-----+ ["*ICAREdfr.stm"] <-storm filename, one per line for NSTORM time
00020> *-----+
00021> *-----+ ["*STORM.stm"] <-storm filename, one per line for NSTORM time
00022> *-----+
00023> *-----+ Lumped drainage to Cardinal Creek Village South Pond 2
00024> CALIB STANDBY [POND=1, AREA=1.93, XIMP=0.56, XIMP2=0.56, TIME=0.66, DNPF=0(mms),
00025> LOSS=0.1, HGT=0.05, AREA=54 (haa), XIMP=0.56, TIME=0.66, DNPF=0(mms),
00026> Previous areas: Taper=[16.2] (mm/hr), Fc=[13.2] (mm/min), DCAY=[4.14] (hr), F=[0.00] (mm),
00027> Previous areas: Taper=[4.67] (mm), SLPF=[2.0] (%), LGP=[40] (m), MNF=[0.25], SCP=[0] (min),
00028> Previous areas: Taper=[1.57] (mm), SLPF=[0.9] (%), LGI=[466] (m), MNF=[0.013], SCI=[0] (min),
00029> RAINFALL=[ , , -1] (mm/hr)
00030> *-----+
00031> *-----+ ["*STORM.stm"] <-storm filename, one per line for NSTORM time
00032> *-----+ Estimated Pond Volumes for SWM Facility
00033> ROUTE RESERVOIR NHYDout="*Pout" NHYBdn="*CCVS*", RDT=[1] (min),
00034> TABLE OF ( CUTFLOW-STORAGE ) values
00035> *-----+
00036> (mms) - (ha-m)
00037> [ 0.000 , 0.000 ]
00038> [ 0.004 , 0.042 ]
00039> [ 0.008 , 0.085 ]
00040> [ 0.013 , 0.129 ]
00041> [ 0.018 , 0.173 ]
00042> [ 0.019 , 0.218 ]
00043> [ 0.022 , 0.263 ]
00044> [ 0.024 , 0.308 ]
00045> [ 0.026 , 0.357 ]
00046> [ 0.028 , 0.405 ]
00047> [ 0.031 , 0.453 ]
00048> [ 0.031 , 0.502 ]
00049> [ 0.033 , 0.551 ]
00050> [ 0.035 , 0.599 ]
00051> [ 0.036 , 0.651 ]
00052> [ 0.042 , 0.702 ]
00053> [ 0.044 , 0.753 ]
00054> [ 0.063 , 0.804 ]
00055> [ 0.078 , 0.856 ]
00056> [ 0.082 , 0.899 ]
00057> [ 0.091 , 0.945 ]
00058> [ 0.098 , 1.015 ]
00059> [ 0.105 , 1.089 ]
00060> [ 0.114 , 1.163 ]
00061> [ 0.115 , 1.178 ]
00062> [ 0.115 , 1.233 ]
00063> [ 0.116 , 1.289 ]
00064> [ 0.129 , 1.345 ]
00065> [ 0.134 , 1.401 ]
00066> [ 0.142 , 1.459 ]
00067> [ 0.142 , 1.516 ]
00068> [ 0.146 , 1.574 ]
00069> [ 0.148 , 1.639 ]
00070> [ 0.153 , 1.692 ]
00071> [ 0.157 , 1.752 ]
00072> [ 0.161 , 1.812 ]
00073> [ 0.164 , 1.872 ]
00074> [ 0.167 , 1.934 ]
00075> [ 0.170 , 1.995 ]
00076> [ 0.174 , 2.057 ]
00077> [ 0.177 , 2.12 ]
00078> [ 0.181 , 2.181 ]
00079> [ 0.183 , 2.247 ]
00080> *-----+ Uncontrolled rear yard drainage area to South Tributary
00081> CALIB STANDBY NHYDout="*CCVS*", DTI=[1] (min), AREA=[1.93] (haa), XIMP=[0.18], TIME=[0.29], DNPF=0(mms),
00082> Previous areas: Taper=[13.2] (mm/hr), Fc=[14.14] (hr), F=[0.00] (mm),
00083> Previous areas: Taper=[4.67] (mm), SLPF=[2.0] (%), LGP=[40] (m), MNF=[0.25], SCP=[0] (min),
00084> Impervious areas: ILimp=[1.57] (mm), SLPF=[0.9] (%), LGI=[113] (m), MNF=[0.013], SCI=[0] (min),
00085> RAINFALL=[ , , -1] (mm/hr)
00086> *-----+
00087> NHYDout="*Povr", RDT=[1] (min)
00088> *-----+ (max twenty pts)
00089> *-----+ Uncontrolled rear yard drainage area to South Tributary
00090> CALIB STANDBY NHYDout="*CCVS*", DTI=[1] (min), AREA=[1.93] (haa), XIMP=[0.18], TIME=[0.29], DNPF=0(mms),
00091> Previous areas: Taper=[13.2] (mm/hr), Fc=[14.14] (hr), F=[0.00] (mm),
00092> Previous areas: Taper=[4.67] (mm), SLPF=[2.0] (%), LGP=[40] (m), MNF=[0.25], SCP=[0] (min),
00093> Impervious areas: ILimp=[1.57] (mm), SLPF=[0.9] (%), LGI=[113] (m), MNF=[0.013], SCI=[0] (min),
00094> RAINFALL=[ , , -1] (mm/hr)
00095> *-----+
00096> * Total Pond 2 Outflow to South Tributary
00097> ADD YRD NHYDout="*Pout-T", NHYDs to add="*Pout" + "*povr"
00098> *-----+
00099> * Total CCV South Outflow to South Tributary (Controlled + Uncontrolled)
00100> ADD YRD NHYDout="*CCVS-T", NHYDs to add="*Pout" + "*povr" + "*CCVS"
00101> *-----+
00102> *-----+
00103> *-----+
00104> *-----+
00105> * 25 mm Storm based on 2-Year, 3-Hour Chicago Storm
00106> *-----+
00107> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
00108> *-----+
00109> * 2-Year, 3-Hour Chicago Storm
00110> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[002]
00111> *-----+
00112> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[003]
00113> *-----+
00114> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[005]
00115> *-----+
00116> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[007]
00117> *-----+
00118> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[010]
00119> *-----+
00120> *-----+
00121> * 25-Year, 3-Hour Chicago Storm
00122> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
00123> *-----+
00124> *-----+
00125> * 50-Year, 3-Hour Chicago Storm
00126> *-----+
00127> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[050]
00128> *-----+
00129> * 100-Year, 3-Hour Chicago Storm
00130> *-----+
00131> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00132> *-----+
00133> * 2-Year, 24-Hour SCS Storm
00134> *-----+
00135> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
00136> *-----+
00137> * 5-Year, 24-Hour SCS Storm
00138> *-----+
00139> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[105]
00140> *-----+
00141> * 10-Year, 24-Hour SCS Storm
00142> *-----+
00143> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
00144> *-----+
00145> * 25-Year, 24-Hour SCS Storm
00146> *-----+
00147> * 50-Year, 24-Hour SCS Storm
00148> *-----+
00149> * 50-Year, 24-Hour SCS Storm
00150> *-----+
00151> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[150]
00152> *-----+
00153> * 100-Year, 24-Hour SCS Storm
00154> *-----+
00155> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[199]
00156> *-----+
00157> *-----+ July 1st, 1979 - Ottawa International Airport
00158> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[199]
00159> *-----+
00160> *-----+
00161> * August 4th, 1988 Storm - Ottawa International Airport
00162> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[988]
00163> *-----+
00164> *-----+ *19880804.stm" <-storm filename, one per line for NSTORM time
00165> *-----+
00166> *-----+ August 8th, 1996 Storm - Ottawa International Airport
00167> START TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[996]
00168> *-----+
00169> *-----+ *19960808.stm" <-storm filename, one per line for NSTORM time
00170> *-----+
00171> *-----+ *100-Year, 3-Hour Chicago SCS + 200
00172> *-----+
00173> *-----+ *100YCH.stm" <-storm filename, one per line for NSTORM time
00174> *-----+
00175> FINISH

```

```

00001+ =====
00002+ =====
00003+ SSSSS N N M H H Y Y M M OOO 222 000 11 555 =====
00004+ SSSSS N N MM MM H H Y Y MM M O 2 0 0 11 5 .000 .000
00005+ SSSSS W W M M HHHHH Y M M O 2 0 0 11 5 Ver 5.500
00006+ SSSSS W W M M H H Y M M O 2 0 0 11 5 .000 .000
00007+ SSSSS W W M M H H Y M M OOO 2 0 0 11 5 FEB 2015
00008+ SSSSS W W M M H H Y M M OOO 2 0 0 11 5 # 2549237
00009+ StormWater Management Hydrologic Model 222 000 11 555 =====
00010+ =====
00011+ ***** SWMM Version 5.500 *****
00012+ ***** A single event and continuous hydrologic simulation model *****
00013+ ***** based on the principles of HDM and its successors *****
00014+ ***** OTTHMRC-83 and CTHMRC-89. *****
00015+ ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00016+ ***** Ottawa, Ontario: (613) 836-3884 *****
00017+ ***** E-mail: swmhymo@fjsa.com *****
00018+ ***** E-mail: swmhymo@fjsa.com *****
00019+ =====
00020+ =====
00021+ =====
00022+ =====
00023+ =====
00024+ ===== Licensed user: JFSAINC. *****
00025+ ***** Serial number: SERIAL#=2549237 *****
00026+ =====
00027+ =====
00028+ ***** PROGRAM ARRAY DIMENSIONS : *****
00029+ ***** Maximum value for flow numbers : 11 *****
00030+ ***** Max. number of rainfall events: 1050 *****
00031+ ***** Max. number of flow points : 105408 *****
00032+ ***** Max. number of nodes : 105408 *****
00033+ ***** Max. number of segments : 105408 *****
00034+ ***** Max. number of areas : 11 *****
00035+ ***** S U M M A R Y O U T P U T *****
00036+ =====
00037+ * RUN DATE: 2024-10-29 TIME: 17:07:59 RUN COUNTER: 012065 *
00038+ =====
00039+ Input file: C:\Temp\SMWHYMO\Fond 2\CCVS_v02.dat
00040+ Output file: C:\Temp\SMWHYMO\Fond 2\CCVS_v02.out
00041+ Summary file: C:\Temp\SMWHYMO\Fond 2\CCVS_v02.sum
00042+ User comments:
00043+ *
00044+ *
00045+ * 2:
00046+ *
00047+ *
00048+ *
00049+ *
00050+ * SWMMHYMO Ver:5.5/Feb 2015 / INPUT DATA FILE
00051+ =====
00052+ # Project Name : [Cardinal Creek Village South]
00053+ # Project Number: [559103]
00054+ # Date : [2024/10/29]
00055+ # Modeler : [FJS]
00056+ # INSTRUME: [1]
00057+ # ICAEEDW: [0]
00058+ # License #: [2549237]
00059+ # RINA:COMMAND#
00060+ R0001:COMMAND#
00061+ R0001:ID=00001
00062+ START
00063+ [*TZERO = 0.00 hrs on 01]
00064+ [*METOUT = 2 (Imperial, 2=metric output)]
00065+ [*INSTRUME = 1]
00066+ [*ICAEEWD = 0]
00067+ [*RINA = 0]
00068+ READ STORM
00069+ File: storm.001
00070+ Comment = 25 MM BASED ON CHICAGO STORM 2 Year, 3 Hours
00071+ [*STZ=10.00:SDURB 3.00:PTOT= 25.00]
00072+ =====
00073+ DEFAULT VALUES
00074+ Filename = C:\Temp\SMWHYMO\Fond 2\Ottawa.vsl
00075+ ICAEEDW = 1 (read and print data)
00076+ Filetitle: File comment: [Parameters for City of Ottawa Projects]
00077+ ***** THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDBY COM
00078+ Horton's infiltration equation parameters:
00079+ [*FCH= 76.20] [*DCAY= 4.14] [*LGP= 4.67] [*RND= .00 mm]
00080+ Parameters for PERVIOUS surfaces in STANDBY:
00081+ [*Taper = 4.67 mm] [*LGP=40.00 m] [*RND=.250]
00082+ Parameters for IMPERVIOUS surfaces in STANDBY:
00083+ [*IAimp = 1.57 mm] [*CLIP = 1.50] [*RND=.013]
00084+ Parameters used in NASHY:
00085+ [*IAimp = 1.57 mm] [*CLIP = 1.50]
00086+ Average monthly Pan Evaporation data in (mm)
00087+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00088+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00089+ Average monthly Potential Evapotranspiration in (mm)
00090+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00091+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00092+ =====
00093+ # PROPOSED CONDITIONS
00094+ # Lumped drainage to Cardinal Creek Village South Pond 2
00095+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00096+ # Horton parameters: Fc= 76.20;Fc= 13.20;DCAY=4.14; F= .00
00097+ # [Permeous area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00098+ # (Metcodeid=446400 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00099+ # Uncontrolled rear yard drainage area to South Tributary
00100+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00101+ # RINA:COMMAND#
00102+ # Estimated Pond Volumes for SWM Facility
00103+ R0001:ID=00005-----Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00104+ # ROUTE RESERVOIR > 1.0 O1CCVS
00105+ out < 1.0 O1Pout 32.54 .03 No date 3:14 14.43 n/a .000
00106+ overlaid < 1.0 O1Povf .06 .000 No date 0:00 .00 n/a .000
00107+ (*Metcodeid=446400 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00108+ # Uncontrolled rear yard drainage area to South Tributary
00109+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00110+ # RINA:COMMAND#
00111+ # RINA:COMMAND#
00112+ # [TZERO = 0.00 hrs on 01]
00113+ # [*METOUT = 2 (Imperial, 2=metric output)]
00114+ # [*INSTRUME = 1]
00115+ # [*RINA = 0]
00116+ # SWMMHYMO Ver:5.5/Feb 2015 / INPUT DATA FILE
00117+ # Project Name : [Cardinal Creek Village South]
00118+ # Project Number: [559103]
00119+ # Date : [2024/10/29]
00120+ # Modeler : [FJS]
00121+ # INSTRUME: [1]
00122+ # ICAEEDW: [0]
00123+ # License #: [2549237]
00124+ # RINA:COMMAND#
00125+ READ STORM
00126+ File: storm.001
00127+ Comment = 25 MM BASED ON CHICAGO STORM 2 Year, 3 Hours
00128+ [*STZ=10.00:SDURB 3.00:PTOT= 31.86]
00129+ R0002:COMMAND#
00130+ # READ STORM
00131+ # RINA:COMMAND#
00132+ # RINA:COMMAND#
00133+ # RINA:COMMAND#
00134+ # RINA:COMMAND#
00135+ # RINA:COMMAND#
00136+ # RINA:COMMAND#
00137+ RINA:COMMAND#
00138+ R0001:COMMAND#
00139+ START
00140+ [*TZERO = 0.00 hrs on 01]
00141+ [*METOUT = 2 (Imperial, 2=metric output)]
00142+ [*INSTRUME = 1]
00143+ [*RINA = 0]
00144+ # SWMMHYMO Ver:5.5/Feb 2015 / INPUT DATA FILE
00145+ # Project Name : [Cardinal Creek Village South]
00146+ # Project Number: [559103]
00147+ # Date : [2024/10/29]
00148+ # Modeler : [FJS]
00149+ # INSTRUME: [1]
00150+ # ICAEEDW: [0]
00151+ # License #: [2549237]
00152+ # RINA:COMMAND#
00153+ READ STORM
00154+ File: storm.001
00155+ ICAEEDW = 1 (read and print data)
00156+ Filetitle: File comment: [Parameters for City of Ottawa Projects]
00157+ ***** THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDBY COM
00158+ Horton's infiltration equation parameters:
00159+ [*FCH= 76.20 mm/hr] [*DCAY= 4.14 / hr] [*F= .00 mm]
00160+ Parameters for PERVIOUS surfaces in STANDBY:
00161+ [*Taper = 4.67 mm] [*LGP=40.00 m] [*RND=.250]
00162+ Parameters for IMPERVIOUS surfaces in STANDBY:
00163+ [*IAimp = 1.57 mm] [*CLIP = 1.50] [*RND=.013]
00164+ Parameters used in NASHY:
00165+ [*IAimp = 4.67 mm] [*CLIP = 1.50]
00166+ Average monthly Pan Evaporation data in (mm)
00167+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00168+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00169+ Average monthly Potential Evapotranspiration in (mm)
00170+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00171+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00172+ # PROPOSED CONDITIONS
00173+ # ***** THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDBY COM
00174+ # Horton's infiltration equation parameters:
00175+ # [*FCH= 76.20 mm hr] [*DCAY= 4.14 / hr] [*F= .00 mm]
00176+ # Parameters for PERVIOUS surfaces in STANDBY:
00177+ # [*Taper = 4.67 mm] [*LGP=40.00 m] [*RND=.250]
00178+ # Parameters for IMPERVIOUS surfaces in STANDBY:
00179+ # [*IAimp = 1.57 mm] [*CLIP = 1.50] [*RND=.013]
00180+ # Parameters used in NASHY:
00181+ # [*IAimp = 4.67 mm] [*CLIP = 1.50]
00182+ # Lump drainage to Cardinal Creek Village South Pond 2
00183+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00184+ # CALIB STANDBY 1.0 O1CCVS
00185+ (*XIMP=.56:TIMP=.66)
00186+ # Uncontrolled rear yard drainage area to South Tributary
00187+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00188+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00189+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00190+ # Ecaid:COMMAND#
00191+ # Ecaid:COMMAND#
00192+ # Lump drainage to Cardinal Creek Village South Pond 2
00193+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00194+ # CALIB STANDBY 1.0 O1CCVS
00195+ (*XIMP=.56:TIMP=.66)
00196+ # Uncontrolled rear yard drainage area to South Tributary
00197+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00198+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00199+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00200+ # Ecaid:COMMAND#
00201+ # Ecaid:COMMAND#
00202+ # Lump drainage to Cardinal Creek Village South Pond 2
00203+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00204+ # ADD HYD
00205+ out < 1.0 O1Pout 32.54 .03 No date 3:13 19.77 n/a .000
00206+ overlaid < 1.0 O1Povf .06 .000 No date 0:00 .00 n/a .000
00207+ # Total CCV South Outflow to South Tributary (Controlled + Uncontrolled)
00208+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00209+ # ADD HYD
00210+ out < 1.0 O1Pout 32.54 .03 No date 3:13 19.77 n/a .000
00211+ overlaid < 1.0 O1Povf .06 .000 No date 0:00 .00 n/a .000
00212+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00213+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00214+ # Ecaid:COMMAND#
00215+ # Ecaid:COMMAND#
00216+ ** END OF RUN : 4
00217+ =====
00218+ =====
00219+ =====
00220+ =====
00221+ =====
00222+ =====
00223+ =====
00224+ =====
00225+ R0005:COMMAND#
00226+ START
00227+ [*TZERO = .00 hrs on 01]
00228+ [*METOUT = 2 (Imperial, 2=metric output)]
00229+ [*INSTRUME = 1]
00230+ [*RINA = 0]
00231+ # SWMMHYMO Ver:5.5/Feb 2015 / INPUT DATA FILE
00232+ =====
00233+ # Project Name : [Cardinal Creek Village South]
00234+ # Project Number: [559103]
00235+ # Date : [2024/10/29]
00236+ # Modeler : [FJS]
00237+ # INSTRUME: [1]
00238+ # Company : J.F. Sabourin and Associates
00239+ # License #: 2549237
00240+ # RINA:COMMAND#
00241+ R0005:COMMAND#
00242+ READ STORM
00243+ File: storm.001
00244+ Comment = CHICAGO STORM 5 Year, 3 Hours
00245+ [*STZ=10.00:SDURB 3.00:PTOT= 42.51]
00246+ R0006:COMMAND#
00247+ DEFAULT VALUES
00248+ Filename = C:\Temp\SMWHYMO\Fond 2\Ottawa.vsl
00249+ ICAEEDW = 1 (read and print data)
00250+ Filetitle: File comment: [Parameters for City of Ottawa Projects]
00251+ THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDBY COM
00252+ Horton's infiltration equation parameters:
00253+ [*Fch= 76.20 mm/hr] [*DCay= 4.14 / hr] [*F= .00 mm]
00254+ Parameters for PERVIOUS surfaces in STANDBY:
00255+ [*Iapers = 4.67 mm] [*Lgp=40.00 m] [*rnd=.250]
00256+ Parameters for IMPERVIOUS surfaces in STANDBY:
00257+ [*IAimp = 1.57 mm] (*clip = 1.50) (*rnd=.013)
00258+ Parameters used in NASHY:
00259+ [*IAimp = 4.67 mm] (*clip = 1.50)
00260+ Average monthly Pan Evaporation data in (mm)
00261+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00262+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00263+ Average monthly Potential Evapotranspiration in (mm)
00264+ JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00265+ .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00266+ # PROPOSED CONDITIONS
00267+ # ***** THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDBY COM
00268+ # Lump drainage to Cardinal Creek Village South Pond 2
00269+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00270+ # CALIB STANDBY 1.0 O1CCVS
00271+ (*XIMP=.56:TIMP=.66)
00272+ # Uncontrolled rear yard drainage area to South Tributary
00273+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00274+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00275+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00276+ # Uncontrolled rear yard drainage area to South Tributary
00277+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00278+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00279+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00280+ # Uncontrolled rear yard drainage area to South Tributary
00281+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00282+ # Uncontrolled rear yard drainage area to South Tributary
00283+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00284+ # CALIB STANDBY 1.0 O1CCVS
00285+ (*XIMP=.56:TIMP=.66)
00286+ # Uncontrolled rear yard drainage area to South Tributary
00287+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00288+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00289+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00290+ # Uncontrolled rear yard drainage area to South Tributary
00291+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00292+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00293+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00294+ # Uncontrolled rear yard drainage area to South Tributary
00295+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00296+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00297+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00298+ # Uncontrolled rear yard drainage area to South Tributary
00299+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00300+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00301+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00302+ # Uncontrolled rear yard drainage area to South Tributary
00303+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00304+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00305+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00306+ # Uncontrolled rear yard drainage area to South Tributary
00307+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00308+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00309+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00310+ # Uncontrolled rear yard drainage area to South Tributary
00311+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00312+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00313+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00314+ # Uncontrolled rear yard drainage area to South Tributary
00315+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00316+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00317+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00318+ # Uncontrolled rear yard drainage area to South Tributary
00319+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00320+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00321+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00322+ # Uncontrolled rear yard drainage area to South Tributary
00323+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00324+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00325+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00326+ # Uncontrolled rear yard drainage area to South Tributary
00327+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00328+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00329+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00330+ # Uncontrolled rear yard drainage area to South Tributary
00331+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00332+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00333+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00334+ # Uncontrolled rear yard drainage area to South Tributary
00335+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00336+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00337+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00338+ # Uncontrolled rear yard drainage area to South Tributary
00339+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00340+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00341+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00342+ # Uncontrolled rear yard drainage area to South Tributary
00343+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00344+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00345+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00346+ # Uncontrolled rear yard drainage area to South Tributary
00347+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00348+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00349+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00350+ # Uncontrolled rear yard drainage area to South Tributary
00351+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00352+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00353+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00354+ # Uncontrolled rear yard drainage area to South Tributary
00355+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00356+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00357+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00358+ # Uncontrolled rear yard drainage area to South Tributary
00359+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00360+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00361+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00362+ # Uncontrolled rear yard drainage area to South Tributary
00363+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00364+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00365+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00366+ # Uncontrolled rear yard drainage area to South Tributary
00367+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00368+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00369+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00370+ # Uncontrolled rear yard drainage area to South Tributary
00371+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00372+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00373+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00374+ # Uncontrolled rear yard drainage area to South Tributary
00375+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00376+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00377+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00378+ # Uncontrolled rear yard drainage area to South Tributary
00379+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00380+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00381+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00382+ # Uncontrolled rear yard drainage area to South Tributary
00383+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00384+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00385+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00386+ # Uncontrolled rear yard drainage area to South Tributary
00387+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00388+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00389+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00390+ # Uncontrolled rear yard drainage area to South Tributary
00391+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00392+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00393+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00394+ # Uncontrolled rear yard drainage area to South Tributary
00395+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00396+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00397+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00398+ # Uncontrolled rear yard drainage area to South Tributary
00399+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00400+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00401+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00402+ # Uncontrolled rear yard drainage area to South Tributary
00403+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00404+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00405+ # (Metcodeid=616100 m3, TotVol=0.000E+00 m3, NOrfl= 0, TotDvrf= 0.hrs)
00406+ # Uncontrolled rear yard drainage area to South Tributary
00407+ # Dmtn-ID=NNYY----ARAHa-QPEAKms-Tpeakdate_hh:mm---RVMn-R.C.---DFmcns
00408+ # Imperious area: IApers = 4.67;SLP=2.00:LGP= 40.00:NRF= .250:SCP= .0]
00409+ # (Metcodeid=616100
```

```

00361> [Previous area: Iapres 4.67:SLPP=2.00:LGF= 40.:MNFP=.250:SCF= .0]
00362> [Impervious area: IAlmpg 1.57:SLPP= .90:LGF= 466.:MMNI=.013:SCI= .0]
00363> # Estimated Pond Volume to South Tributary
00364> RO010:CO0005-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00365> ROUTE RESERVOIR -> 1.0 02:CVS 32.54 .919 No_date 1:03 34.94 n/a .000
00366> + 1.0 02:CVS 32.54 .000 No_date 1:03 34.94 n/a .000
00367> + overflow <= 1.0 03:Povf .00 .000 No_date 0:00 .00 n/a .000
00368> # (MStColeds:1066&:01 m3, TotovVol=.0008&:00 m3, N-Ovrs= 0, Totburvfc= 0.hrs)
00369> # Uncontrolled rear yard drainage area to South Tributary
00370> RO010:CO0006-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00371> CALIB STANDYD 1.0 01:CCVScn 1.93 .268 No_date 1:02 22.45 .000
00372> [XMPN=19:TIMEP=29]
00373> [Horton parameters: Fov: 76.20:Fcs 13.20:DCAY=4.14: Fc .00]
00374> [Fervious area: Iapres 4.67:SLPP=2.00:LGF= 40.:MNFP=.250:SCF= .0]
00375> [Impervious area: IAlmpg 1.57:SLPP= .90:LGF= 466.:MMNI=.013:SCI= .0]
00376> # Total CVV 2 Outflow to South Tributary
00377> RO010:CO0007-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00378> ADD HYD 1.0 02:Pout 32.54 .105 No_date 3:06 34.93 n/a .000
00379> + 1.0 02:Pout 32.54 .000 No_date 0:00 .00 n/a .000
00380> SUM= 1.0 01:CCVSp-T 32.54 .105 No_date 3:06 34.93 n/a .000
00381> # Total CVV South Outflow to South Tributary (Controlled + Uncontrolled)
00382> RO010:CO0008-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00383> ADD HYD 1.0 02:Pout 32.54 .105 No_date 3:06 34.93 n/a .000
00384> + 1.0 02:Pout 32.54 .000 No_date 0:00 .00 n/a .000
00385> SUM= 1.0 01:CCVSp-T 32.54 .105 No_date 3:06 34.93 n/a .000
00386> SUM= 1.0 01:CCVSp-T 34.47 .292 No_date 1:02 34.21 n/a .000
00387> #####
00388> # STORM
00389> #####
00390> *** END OF RUN : 24
00391>
00392> #####
00393> 00394>
00395> 00396>
00397> 00398> RUN#>COMMAND#
00399> RO025:CO0001-----START
00400> [TZERO = 2.00 hrs on 0]
00402> [METOUT= 2 (Imperial, 2=metric output)]
00403> [INSTRM= 1]
00404> [HNRUN = 00999]
00405> # SWHMIMO Ver:1.5/Feb 2015 / INPUT DATA FILE
00406> # Project Name : [Cardinal Creek Village South]
00407> # Project Number: [559(03)]
00408> # Date : [2024/10/29]
00409> # Modeler : [PP]
00410> # Company : J.F. Sabourin and Associates
00411> # License # : 2549237
00412> # 
00413> # 
00414> # 
00415> RO025:CO0002-----STORM
00416> # File name: storm.001
00417> Comment = CHICAGO STORM 25 years, 3 Hours
00418> [S2T=10:00:SDUR= 3.00:PFOT= .38.23]
00419> RO025:CO0003-----STORM
00420> # DEFAULT VALUES
00421> # Filename = C:\Temp\SMWHMIMO\Fond 2\Ottawa.val
00422> # INSTRM = 1 (read and print data)
00423> # FileTitle= File comment: [Parameters for City of Ottawa Projects]
00424> # THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDYD COM
00425> # Parameters used in NASHYD:
00426> # Horton's infiltration equation parameters:
00427> # [Fov: 76.20 mm/hr] [Fcs=13.20 mm/hr] [DCAY=4.14 / hr] [Fc .00 mm]
00428> # Parameters for PERVIOUS surfaces in STANDYD:
00429> # [IAlmpg 1.57 mm] [CLIE 1.50] [MMNI .013]
00430> # Parameters for IMPERVIOUS surfaces in STANDYD:
00431> # [IAlmpg 4.67 mm] [NM 3.00]
00432> # Average monthly Fan Evaporation data in (mm)
00433> # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00434> # .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00435> # Average monthly Potential Evapotranspiration in (mm)
00436> # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00437> # .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00438> # Parameters used in NASHYD:
00439> # [IAlmpg 1.57 mm] [CLIE 1.50] [MMNI .013]
00440> # Estimated Pond Volumes for EBN Facility
00441> RO025:CO0004-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00442> ROUTE RESERVOIR -> 1.0 02:CVS 32.54 .126 No_date 1:03 42.74 n/a .000
00443> out <= 1.0 01:Povf .00 .000 No_date 3:06 42.74 n/a .000
00444> + 1.0 02:Povf .00 .000 No_date 0:00 .00 n/a .000
00445> SUM= 1.0 01:CCVSp-T 32.54 .126 No_date 1:03 42.74 n/a .000
00446> # (MStColeds:1030&:01 m3, TotovVol=.0008&:00 m3, N-Ovrs= 0, Totburvfc= 0.hrs)
00447> # Uncontrolled rear yard drainage area to South Tributary
00448> RO025:CO0005-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00449> CALIB STANDYD 1.0 01:CCVScn 1.93 .362 No_date 1:01 29.01 .498 .000
00450> [XMPN=19:TIMEP=29]
00451> [Horton parameters: Fov: 76.20:Fcs 13.20:DCAY=4.14: Fc .00]
00452> [Fervious area: Iapres 4.67:SLPP=2.00:LGF= 40.:MNFP=.250:SCF= .0]
00453> [Impervious area: IAlmpg 1.57:SLPP= .90:LGF= 466.:MMNI=.013:SCI= .0]
00454> # Total CVV 2 Outflow to South Tributary
00455> RO050:CO0007-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00456> # Uncontrolled rear yard drainage area to South Tributary
00457> RO050:CO0008-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00458> ADD HYD 1.0 02:Pout 32.54 .140 No_date 3:05 48.84 n/a .000
00459> + 1.0 02:Pout 32.54 .000 No_date 0:00 .00 n/a .000
00460> SUM= 1.0 01:CCVSp-T 34.47 .478 No_date 1:01 48.02 n/a .000
00461> #####
00462> # STORM
00463> #####
00464> ** END OF RUN : 98
00465> #####
00466> 00467>
00468> 00469>
00470> 00471>
00472> RUN#>COMMAND#
00473> RO099:CO0001-----START
00474> [TZERO = 0.00 hrs on 0]
00475> [TZERO = 1.00 hrs on 0]
00476> [METOUT= 2 (Imperial, 2=metric output)]
00477> [INSTRM= 1]
00478> [HNRUN = 00999]
00479> # SWHMIMO Ver:1.5/Feb 2015 / INPUT DATA FILE
00480> # Project Name : [Cardinal Creek Village South]
00481> # Project Number: [559(03)]
00482> # Date : [2024/10/29]
00483> # Modeler : [PP]
00484> # Company : J.F. Sabourin and Associates
00485> # License # : 2549237
00486> # 
00487> # 
00488> # 
00489> # 
00490> # 
00491> # 
00492> # 
00493> # 
00494> # 
00495> # 
00496> # 
00497> # 
00498> # 
00499> # 
00500> # 
00501> # 
00502> RO050:CO0002-----STORM
00503> # File name: storm.001
00504> Comment = CHICAGO STORM 50 Year, 3 Hours
00505> [S2T=10:00:SDUR= 3.00:PFOT= 64.81]
00506> RO050:CO0003-----STORM
00507> # DEFAULT VALUES
00508> # Filename = C:\Temp\SMWHMIMO\Fond 2\Ottawa.val
00509> # ICASEd= 1 (read and print data)
00510> # FileTitle= File comment: [Parameters for City of Ottawa Projects]
00511> # THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDYD COM
00512> # Horton's infiltration equation parameters:
00513> # [Fov: 76.20 mm hr] [Fcs=13.20:DCAY=4.14 / hr] [Fc .00 mm]
00514> # Parameters for PERVIOUS surfaces in STANDYD:
00515> # [IAlmpg 4.67 mm] [LG40=40.00 mm] [MNFP=.250]
00516> # Parameters for IMPERVIOUS surfaces in STANDYD:
00517> # [IAlmpg 1.57 mm] [CLIE 1.50] [MMNI .013]
00518> # Parameters used in NASHYD:
00519> # [IAlmpg 4.67 mm] [NM 3.00]
00520> # Average monthly Fan Evaporation data in (mm)
00521> # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00522> # .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00523> # Average monthly Potential Evapotranspiration in (mm)
00524> # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00525> # .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00526> # Parameters used in NASHYD:
00527> # [IAlmpg 4.67 mm] [NM 3.00]
00528> # PROPOSED CONDITIONS
00529> # 
00530> # 
00531> # 
00532> # 
00533> # 
00534> # 
00535> # 
00536> # 
00537> # 
00538> # 
00539> # 
00540> # 
00541> # 
00542> # 
00543> # 
00544> # 
00545> # 
00546> # 
00547> # 
00548> # 
00549> # 
00550> # 
00551> # 
00552> # 
00553> # 
00554> # 
00555> # 
00556> # 
00557> # 
00558> # 
00559> # 
00560> # 
00561> #####
00562> # STORM
00563> #####
00564> ** END OF RUN : 98
00565> #####
00566> 00567>
00568> 00569>
00570> 00571>
00572> RUN#>COMMAND#
00573> RO099:CO0001-----START
00574> [TZERO = 0.00 hrs on 0]
00575> [TZERO = 1.00 hrs on 0]
00576> [METOUT= 2 (Imperial, 2=metric output)]
00577> [INSTRM= 1]
00578> [HNRUN = 00999]
00579> # SWHMIMO Ver:1.5/Feb 2015 / INPUT DATA FILE
00580> # Project Name : [Cardinal Creek Village South]
00581> # Project Number: [559(03)]
00582> # Date : [2024/10/29]
00583> # Modeler : [PP]
00584> # Company : J.F. Sabourin and Associates
00585> # License # : 2549237
00586> # 
00587> # 
00588> # 
00589> # 
00590> # 
00591> # 
00592> # 
00593> # 
00594> # 
00595> # 
00596> # 
00597> # 
00598> # 
00599> # 
00600> # 
00601> # 
00602> # 
00603> # 
00604> # 
00605> # 
00606> # 
00607> # 
00608> # 
00609> # 
00610> # 
00611> # 
00612> # 
00613> # 
00614> # 
00615> # 
00616> # 
00617> # Lumped drainage to Cardinal Creek Village South Pond 2
00618> RO099:CO0002-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00619> ADD HYD 1.0 01:CCVScn 32.54 .948 No_date 1:02 55.14 .769 .000
00620> [XMPN=.56:TIMEP=.66]
00621> [Horton parameters: Fov: 76.20 mm/hr] [Fcs=13.20:DCAY=4.14 / hr] [Fc .00 mm]
00622> Parameters for PERVIOUS surfaces in STANDYD:
00623> [IAlmpg 4.67 mm] [LG40=40.00 mm] [MNFP=.250]
00624> Parameters for IMPERVIOUS surfaces in STANDYD:
00625> [IAlmpg 1.57 mm] [CLIE 1.50] [MMNI .013]
00626> [IAlmpg 4.67 mm] [NM 3.00]
00627> # Estimated Pond Volumes for EBN Facility
00628> ROUTE RESERVOIR -> 1.0 02:CVS 32.54 .153 No_date 1:02 55.14 n/a .000
00629> out <= 1.0 01:Povf 32.54 .153 No_date 3:05 55.14 n/a .000
00630> + 1.0 02:Povf .00 .000 No_date 0:00 .00 n/a .000
00631> SUM= 1.0 01:CCVSp-T 32.54 .153 No_date 1:02 55.14 n/a .000
00632> # Upcontrolled rear yard drainage area to South Tributary
00633> RO099:CO0003-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00634> ADD HYD 1.0 01:CCVScn 32.54 .948 No_date 1:02 55.14 .769 .000
00635> [XMPN=.56:TIMEP=.66]
00636> [Horton parameters: Iapres 4.67:SLPP=2.00:LGF= 40.:MNFP=.250:SCF= .0]
00637> [Impervious area: IAlmpg 1.57:SLPP= .90:LGF= 466.:MMNI=.013:SCI= .0]
00638> # Estimated Pond Volumes for EBN Facility
00639> RO099:CO0004-----Dtn-ID:INHYD-----ARAhA-QPEAKms-TpeakDate hh:mm-->RVm=R.C.-->DMFcms
00640> ADD HYD 1.0 02:Pout 32.54 .153 No_date 3:05 55.14 n/a .000
00641> + 1.0 02:Pout 32.54 .000 No_date 0:00 .00 n/a .000
00642> SUM= 1.0 01:CCVSp-T 32.54 .153 No_date 1:02 55.14 n/a .000
00643> #####
00644> # STORM
00645> #####
00646> ** END OF RUN : 101
00647> #####
00648> 00649>
00650> # 
00651> # 
00652> # 
00653> # 
00654> # 
00655> # 
00656> # 
00657> # 
00658> # 
00659> # 
00660> # 
00661> # 
00662> # 
00663> # 
00664> # 
00665> # 
00666> # 
00667> # SWHMIMO Ver:1.5/Feb 2015 / INPUT DATA FILE
00668> # Project Name : [Cardinal Creek Village South]
00669> # Project Number: [559(03)]
00670> # Date : [2024/10/29]
00671> # Modeler : [PP]
00672> # Company : J.F. Sabourin and Associates
00673> # License # : 2549237
00674> # 
00675> # 
00676> # 
00677> # 
00678> # 
00679> # 
00680> # 
00681> # 
00682> # 
00683> # 
00684> # 
00685> # 
00686> # 
00687> # 
00688> # 
00689> # 
00690> # 
00691> # 
00692> # 
00693> # 
00694> # 
00695> # 
00696> # 
00697> # 
00698> # 
00699> # 
00700> # 
00701> # 
00702> # 
00703> # 
00704> # 
00705> # 
00706> # 
00707> # 
00708> # 
00709> # 
00710> # 
00711> # 
00712> # 
00713> # 
00714> # 
00715> # 
00716> # 
00717> # 
00718> # 
00719> # 
00720> # 
00721> # 
00722> # 
00723> # 
00724> # 
00725> # 
00726> # 
00727> # 
00728> # 
00729> # 
00730> # 
00731> # 
00732> # 
00733> # 
00734> # 
00735> # 
00736> # 
00737> # 
00738> # 
00739> # 
00740> # 
00741> # 
00742> # 
00743> # 
00744> # 
00745> # 
00746> # 
00747> # 
00748> # 
00749> # 
00750> # 
00751> # 
00752> # 
00753> # 
00754> # 
00755> # 
00756> # 
00757> # 
00758> # 
00759> # 
00760> # 
00761> # 
00762> # 
00763> # 
00764> # 
00765> # 
00766> # 
00767> # 
00768> # 
00769> # 
00770> # 
00771> # 
00772> # 
00773> # 
00774> # 
00775> # 
00776> # 
00777> # 
00778> # 
00779> # 
00780> # 
00781> # 
00782> # 
00783> # 
00784> # 
00785> # 
00786> # 
00787> # 
00788> # 
00789> # 
00790> # 
00791> # 
00792> # 
00793> # 
00794> # 
00795> # 
00796> # 
00797> # 
00798> # 
00799> # 
00800> # 
00801> # 
00802> # 
00803> # 
00804> # 
00805> # 
00806> # 
00807> # 
00808> # 
00809> # 
00810> # 
00811> # 
00812> # 
00813> # 
00814> # 
00815> # 
00816> # 
00817> # 
00818> # 
00819> # 
00820> # 
00821> # 
00822> # 
00823> # 
00824> # 
00825> # 
00826> # 
00827> # 
00828> # 
00829> # 
00830> # 
00831> # 
00832> # 
00833> # 
00834> # 
00835> # 
00836> # 
00837> # 
00838> # 
00839> # 
00840> # 
00841> # 
00842> # 
00843> # 
00844> # 
00845> # 
00846> # 
00847> # 
00848> # 
00849> # 
00850> # 
00851> # 
00852> # 
00853> # 
00854> # 
00855> # 
00856> # 
00857> # 
00858> # 
00859> # 
00860> # 
00861> # 
00862> # 
00863> # 
00864> # 
00865> # 
00866> # 
00867> # 
00868> # 
00869> # 
00870> # 
00871> # 
00872> # 
00873> # 
00874> # 
00875> # 
00876> # 
00877> # 
00878> # 
00879> # 
00880> # 
00881> # 
00882> # 
00883> # 
00884> # 
00885> # 
00886> # 
00887> # 
00888> # 
00889> # 
00890> # 
00891> # 
00892> # 
00893> # 
00894> # 
00895> # 
00896> # 
00897> # 
00898> # 
00899> # 
00900> # 
00901> # 
00902> # 
00903> # 
00904> # 
00905> # 
00906> # 
00907> # 
00908> # 
00909> # 
00910> # 
00911> # 
00912> # 
00913> # 
00914> # 
00915> # 
00916> # 
00917> # 
00918> # 
00919> # 
00920> # 
00921> # 
00922> # 
00923> # 
00924> # 
00925> # 
00926> # 
00927> # 
00928> # 
00929> # 
00930> # 
00931> # 
00932> # 
00933> # 
00934> # 
00935> # 
00936> # 
00937> # 
00938> # 
00939> # 
00940> # 
00941> # 
00942> # 
00943> # 
00944> # 
00945> # 
00946> # 
00947> # 
00948> # 
00949> # 
00950> # 
00951> # 
00952> # 
00953> # 
00954> # 
00955> # 
00956> # 
00957> # 
00958> # 
00959> # 
00960> # 
00961> # 
00962> # 
00963> # 
00964> # 
00965> # 
00966> # 
00967> # 
00968> # 
00969> # 
00970> # 
00971> # 
00972> # 
00973> # 
00974> # 
00975> # 
00976> # 
00977> # 
00978> # 
00979> # 
00980> # 
00981> # 
00982> # 
00983> # 
00984> # 
00985> # 
00986> # 
00987> # 
00988> # 
00989> # 
00990> # 
00991> # 
00992> # 
00993> # 
00994> # 
00995> # 
00996> # 
00997> # 
00998> # 
00999> # 
01000> # 
01001> # 
01002> # 
01003> # 
01004> # 
01005> # 
01006> # 
01007> # 
01008> # 
01009> # 
01010> # 
01011> # 
01012> # 
01013> # 
01014> # 
01015> # 
01016> # 
01017> # 
01018> # 
01019> # 
01020> # 
01021> # 
01022> # 
01023> # 
01024> # 
01025> # 
01026> # 
01027> # 
01028> # 
01029> # 
01030> # 
01031> # 
01032> # 
01033> # 
01034> # 
01035> # 
01036> # 
01037> # 
01038> # 
01039> # 
01040> # 
01041> # 
01042> # 
01043> # 
01044> # 
01045> # 
01046> # 
01047> # 
01048> # 
01049> # 
01050> # 
01051> # 
01052> # 
01053> # 
01054> # 
01055> # 
01056> # 
01057> # 
01058> # 
01059> # 
01060> # 
01061> # 
01062> # 
01063> # 
01064> # 
01065> # 
01066> # 
01067> # 
01068> # 
01069> # 
01070> # 
01071> # 
01072> # 
01073> # 
01074> # 
01075> # 
01076> # 
01077> # 
01078> # 
01079> # 
01080> # 
01081> # 
01082> # 
01083> # 
01084> # 
01085> # 
01086> # 
01087> # 
01088> # 
01089> # 
01090> # 
01091> # 
01092> # 
01093> # 
01094> # 
01095> # 
01096> # 
01097> # 
01098> # 
01099> # 
01100> # 
01101> # 
01102> # 
01103> # 
01104> # 
01105> # 
01106> # 
01107> # 
01108> # 
01109> # 
01110> # 
01111> # 
01112> # 
01113> # 
01114> # 
01115> # 
01116> # 
01117> # 
01118> # 
01119> # 
01120> # 
01121> # 
01122> # 
01123> # 
01124> # 
01125> # 
01126> # 
01127> # 
01128> # 
01129> # 
01130> # 
01131> # 
01132> # 
01133> # 
01134> # 
01135> # 
01136> # 
01137> # 
01138> # 
01139> # 
01140> # 
01141> # 
01142> # 
01143> # 
01144> # 
01145> # 
01146> # 
01147> # 
01148> # 
01149> # 
01150> # 
01151> # 
01152> # 
01153> # 
01154> # 
01155> # 
01156> # 
01157> # 
01158> # 
01159> # 
01160> # 
01161> # 
01162> # 
01163> # 
01164> # 
01165> # 
01166> # 
01167> # 
01168> # 
01169> # 
01170> # 
01171> # 
01172> # 
01173> # 
01174> # 
01175> # 
01176> # 
01177> # 
01178> # 
01179> # 
01180> # 
01181> # 
01182> # 
01183> # 
01184> # 
01185> # 
01186> # 
01187> # 
01188> # 
01189> # 
01190> # 
01191> # 
01192> # 
01193> #
```



```

01081>      + 1.0 02:CCVunc 1.93 .431 No date 12:01 43.67 n/a .000
01082>  SUM= 1.0 01:CCVS-T 34.47 .523 No date 12:01 66.47 n/a .000
01083> #####
01084> # STORMS
01085> #####
01086> ** END OF RUN : 198
01087> #####
01088> ****
01089> START
01090>   [TZERO = 1.00 hrs on 01]
01091>   [METCOUT= 2 ] (Imperial, 2=metric output)
01092>   [HNRUN = 0198 ]
01093>
01094> RUN#:COMMAND#
01095> R0979:00001-----
01096>   ****
01097>   START
01098>   [TZERO = 1.00 hrs on 01]
01099>   [METCOUT= 2 ] (Imperial, 2=metric output)
01100>   [HNRUN = 0198 ]
01101>   ****
01102>   FileTitle: File comment: [INPUT DATA FILE]
01103>   FileTitle: File comment: [Parameters for City of Ottawa Projects]
01104>   Project Name : [Cardinal Creek Village South]
01105>   Project Number : [559(03)]
01106>   Date : [2024/10/29]
01107>   Modeler : [FP]
01108>   Company : [J.F. Sabourin and Associates]
01109>   License # : 2549237
01110>   ****
01111> R0979:00002-----
01112>   READ STORM
01113>   Filename = storm.001
01114>   Comment = [Aug 1st, 1979 Storm Step Type 2 Storm 24 Hours step 10 min, City of Ottawa
01115>   [STZT=10.00:BSR] [GND=24.00:PTDT=166.73]
01116> R0979:00003-----
01117>   READ STORM
01118>   DEFAULT VALUES
01119>   ICASEDw = 1 (read and print data)
01120>   Parameters File comment: [Parameters for City of Ottawa Projects]
01121>   The FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDHYD COM
01122>   Horton's infiltration equation parameters:
01123>   [For ~76.20 mm/hr] [FC=1.20 mm/hr] [DCAV= 4.14 hr] [F= .00 mm]
01124>   [TImp= 1.57 mm] [LGP=40.00 mm] [MNFS=.250]
01125>   [TImp= 4.67 mm] [LGP=40.00 mm] [MNFS=.250]
01126>   Parameters for IMPERVIOUS surfaces in STANDHYD:
01127>   [TImp= 1.57 mm] [LGP=40.00 mm] [MNFS=.013]
01128>   Parameters used in NASHYD:
01129>   [Iav= 4.67 mm] [MN= 3.00]
01130>   Average monthly Evaporation data in (mm)
01131>   JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
01132>   .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
01133>   .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
01134>   JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
01135>   .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
01136>   #####
01137>   FileTitle: File comment: [INPUT DATA FILE]
01138>   ##### Lumped drainage to Cardinal Creek Village South
01139>   R0979:00004-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01140>   CALIB STANDHYD 1.0 01:CCVS 32.54 8.427 No date 12:02 75.61 .708 .000
01141>   [XIMP=.56:TIMEP=.66]
01142>   [ROUTE RESEVOIR-> 1.0 02:CCVS]
01143>   [Infiltration area: Fc= 76.20;Fc= 13.20;DCAY=4.14; F= .00]
01144>   [Pervious areas: Timpers 4.67;SLPF2=0.00:LGP=40.00:MNFS=.250:SCF= .0]
01145>   [Impervious areas: TAimp 1.57;SLPF= .90:LGP= 466. MNFS=.013:SCF= .0]
01146>   # Extended rear yard drainage area to South Tributary
01147>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01148>   ROUTE RESEVOIR-> 1.0 02:CCVS 32.54 8.427 No date 12:02 75.61 .708 .000
01149>   ADD HWD 32.54 8.427 No date 12:02 75.61 .708 .000
01150>   overflow <= 1.0 03:Forv .0000 No date 0:00 .00/n/a .000
01151>   # Uncontrolled rear yard drainage area to South Tributary
01152>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01153>   CALIB STANDHYD 1.0 01:CCVS 1.93 .489 No date 12:01 49.56 .464 .000
01154>   [ROUTE RESEVOIR-> 1.0 02:CCVS]
01155>   ADD HWD 32.54 .167 No date 14:18 75.61 n/a .000
01156>   overflow <= 1.0 03:Forv .0000 No date 0:00 .00/n/a .000
01157>   # Uncontrolled rear yard drainage area to South Tributary
01158>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01159>   Total Flow 2 Outflow to South Tributary
01160>   # Uncontrolled rear yard drainage area to South Tributary
01161>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01162>   ROUTE RESEVOIR-> 1.0 02:CCVS 32.54 .167 No date 12:02 75.61 n/a .000
01163>   ADD HWD 32.54 .167 No date 14:18 75.61 n/a .000
01164>   overflow <= 1.0 03:Forv .0000 No date 0:00 .00/n/a .000
01165>   # Total CCV South Outflow to South Tributary (Controlled + Uncontrolled)
01166>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01167>   ADD HWD 32.54 .167 No date 14:18 75.61 n/a .000
01168>   overflow <= 1.0 03:Forv .0000 No date 0:00 .00/n/a .000
01169>   # Uncontrolled rear yard drainage area to South Tributary
01170>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01171>   ** END OF RUN : 978
01172> #####
01173> ** END OF RUN : 978
01174> #####
01175> ****
01176> ****
01177> START
01178>   [TZERO = 1.00 hrs on 01]
01179>   [METCOUT= 2 ] (Imperial, 2=metric output)
01180>   [HNRUN = 978 ]
01181>   ****
01182>   RUN#:COMMAND#
01183> R0979:00001-----
01184>   ****
01185>   START
01186>   [TZERO = 1.00 hrs on 01]
01187>   [METCOUT= 2 ] (Imperial, 2=metric output)
01188>   [HNRUN = 978 ]
01189>   ****
01190>   FileTitle: File comment: [INPUT DATA FILE]
01191>   The FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDHYD COM
01192>   Project Name : [Cardinal Creek Village South]
01193>   Project Number : [559(03)]
01194>   Date : [2024/10/29]
01195>   Modeler : [FP]
01196>   Company : [J.F. Sabourin and Associates]
01197>   License # : 2549237
01198>   ****
01199> R0979:00002-----
01200>   FileTitle: File comment: [INPUT DATA FILE]
01201>   Comment = [July 1st, 1979 Storm (38) - Ottawa International Airport step 5 min
01202>   [STZT=10.00:BSR] [GND=3.00:PTDT=83.99]
01203> R0979:00003-----
01204>   DEFAULT VALUES
01205>   FileTitle: File comment: [Parameters for City of Ottawa Projects]
01206>   The FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDHYD COM
01207>   Project Name : [Cardinal Creek Village South]
01208>   Project Number : [559(03)]
01209>   Date : [2024/10/29]
01210>   Modeler : [FP]
01211>   Company : [J.F. Sabourin and Associates]
01212>   License # : 2549237
01213>   ****
01214>   FileTitle: File comment: [INPUT DATA FILE]
01215>   The FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDHYD COM
01216>   [For ~76.20 mm/hr] [FC=13.20 mm hr] [DCAV= 4.14 hr] [F= .00 mm]
01217>   Parameters for PERVIOUS surfaces in STANDHYD:
01218>   [TImp= 1.57 mm] [CLIC= 1.50] [MNFS=.250]
01219>   Parameters for IMPERVIOUS surfaces in STANDHYD:
01220>   [TAimp= 1.57 mm] [CLIC= 1.50] [MNFS=.013]
01221>   Parameters used in NASHYD:
01222>   [Iav= 4.67 mm] [MN= 3.00]
01223>   Average monthly Fm Evaporation data in (mm)
01224>   JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
01225>   .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
01226>   .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
01227>   #####
01228>   FileTitle: File comment: [INPUT DATA FILE]
01229>   ##### Lumped drainage to Cardinal Creek Village South 2
01230>   R0979:00004-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01231>   CALIB STANDHYD 1.0 01:CCVS 32.54 7.062 No date 1:34 68.14 .811 .000
01232>   [XIMP=.56:TIMEP=.66]
01233>   ROUTE RESEVOIR-> 1.0 02:CCVS ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01234>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01235>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01236>   # Uncontrolled rear yard drainage area to South Tributary
01237>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01238>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01239>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01240>   # Uncontrolled rear yard drainage area to South Tributary
01241>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01242>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01243>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01244>   # Uncontrolled rear yard drainage area to South Tributary
01245>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01246>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01247>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01248>   # Uncontrolled rear yard drainage area to South Tributary
01249>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01250>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01251>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01252>   # Total CCV South Outflow to South Tributary (Controlled + Uncontrolled)
01253>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01254>   ADD HWD 32.54 .177 No date 3:04 68.14 n/a .000
01255>   overflow <= 1.0 01:Forv .0000 No date 0:00 .00/n/a .000
01256>   # Uncontrolled rear yard drainage area to South Tributary
01257>   DTM-ID:INNYD-----ARArha-QPEAKms-TpeakData.hhmm---Rvmm-R.C.--DWFcms
01258>   # STORMS
01259> #####
01260> ** END OF RUN : 987

```




JFSA Canada Inc.
52 Springbrook Drive,
Ottawa, ON K2S 1B9
T 613-836-3884 F 613-836-0332

jfsa.com

Attachment B

Pond 2 Preliminary Forebay Calculations

CALCULATION SHEET B-1: FOREBAY SIZING FOR SWM FACILITY

CARDINAL CREEK VILLAGE SOUTH SWM Pond 2 City of Ottawa Calculation of Forebay Size

© DSEL

Settling Criteria

From the SWMP Manual, the required length for settling is as follows:

$$L_{\min} = \left(\frac{r Q_p}{V_s} \right)^{0.5}$$

where: r = length to width ratio, at the invert of the inlet pipe.
 Q_p = peak outflow during design quality storm
 V_s = settling velocity

Input: $r = 3.35$ (67 m / 20 m)
 $Q_p = 0.036 \text{ m}^3/\text{s}$ (at elevation 83.2 m)
 $V_s = 0.0003 \text{ m/s}$

$$L_{\min} = 20.03 \text{ m}$$

The peak flow rate from the pond during the quality storm is taken as the flow that would occur just below the quantity controls (Refer to Attachment A)

Dispersion Criteria

From the SWMP Manual, the required length for dispersion is as follows:

$$L_{\min} = \frac{8Q}{d V_f}$$

where: Q = Inlet flowrate (10-Year, 24-Hour SCS Storm)
 d = depth of permanent pool (forebay)
 V_f = desired final velocity

Input: $Q = 5.324 \text{ m}^3/\text{s}$
 $d = 1.50 \text{ m}$
 $V_f = 0.5 \text{ m/s}$

$$L_{\min} = 56.79 \text{ m}$$

The minimum forebay length is determined by the larger of the settling or dispersion criteria.

Minimum Length of Forebay Required 56.79 m

Length of Forebay Provided 67.00 m (at elevation 82.5 m)

Average Forebay Velocity

From the SWMP Manual, the maximum allowable average velocity is 0.15 m/s:

$$V_{\text{avg}} = \frac{Q}{d W_{\text{avg}}}$$

where: Q = Inlet flowrate (10-Year, 24-Hour SCS Storm)
 d = depth of pond during peak 10-year inflow (12h:02min)
 W_{avg} = average width of forebay

Input: $Q = 5.324 \text{ m}^3/\text{s}$
 $d = 2.80 \text{ m}$
 $W_{\text{avg}} = 13 \text{ m}$ (5 m bottom, 20 m permanent pool)

$$V = 0.15 \text{ m/s} \leq 0.15 \text{ m/s}$$



JFSA Canada Inc.
52 Springbrook Drive,
Ottawa, ON K2S 1B9
T 613-836-3884 F 613-836-0332

jfsa.com

Attachment C

Storm Design Sheets (DSEL)
Preliminary HGL Analysis Results

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years

Manning 0.013



Location	From Node	To Node	AREA (Ha)												FLOW							SEWER DATA											
			2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	Type	Slope	Length	Capacity	Velocity	Time of	Ratio
			Area (Ha)	R	Indiv.	Accum.	Area (Ha)	R	Indiv.	Accum.	Area (Ha)	R	Indiv.	Accum.	Area (Ha)	R	Indiv.	Accum.	Conc.	2 Year	5 Year	10 Year	100 Year	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	Low (min)	Q/Q full	
SERVICING BLOCK 2																																	
Contribution From STREET 17, Pipe 105 - 106			-2.44						0.00										4.59	13.99													
106	113		0.00	2.44			0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	13.23	66.29	89.75	105.13	153.58	162	525	525	CONC	2.20	16.5	637.8854	2.9467	0.0933	0.253	
Contribution From STREET 17, Pipe 110 - 111			2.31						0.00										0.00	12.00													
111	113	0.07	0.68	0.13	2.44		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	14.06	64.09	86.74	101.59	148.39	825	900	900	CONC	1.10	105.0	1898.6704	2.9845	0.5864	0.435	
			0.00	0.00	1.49	0.40	1.66	1.66		0.00	0.00		0.00	0.00		0.00	0.00	15.18	61.34	82.98	97.17	141.89	1183	975	975	CONC	0.45	24.0	1503.3442	2.0135	0.1987	0.787	
To STREET 1, Pipe 119 - 121			0.00						6.41									4.59	15.38														
STREET 5																																	
75	76	0.41	0.68	0.78	0.78		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	60	450	450	CONC	0.20	99.5	127.5033	0.8017	2.0685	0.467	
To STREET 12, Pipe 76 - 78			0.78				0.00			0.00			0.00			0.00	12.07																
STREET 6																																	
65	66	0.33	0.68	0.62	0.62		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	48	450	450	CONC	0.80	46.0	255.0067	1.6034	0.4782	0.188	
To STREET 12, Pipe 66 - 72			0.62				0.00			0.00			0.00			0.00	10.48																
67	68	0.25	0.68	0.47	0.47		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	36	450	450	CONC	0.75	13.0	246.9092	1.5525	0.1396	0.147	
68	69	0.13	0.68	0.25	0.72		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.14	76.27	103.46	121.28	177.29	55	450	450	CONC	0.50	10.5	201.6005	1.2676	0.1381	0.272	
69	70	0.26	0.68	0.49	1.21		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.28	75.76	102.75	120.44	176.06	92	450	450	CONC	0.20	64.5	127.5033	0.8017	1.3409	0.719	
70	71	0.11	0.68	0.21	1.42		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	11.62	71.10	96.36	112.91	165.00	101	450	450	CONC	0.20	10.5	127.5033	0.8017	0.2183	0.791	
To STREET 12, Pipe 72 - 74			2.51				0.00			0.00			0.00			0.00	13.39																
STREET 13																																	
		0.43	0.68	0.81	0.81		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00																
16	17	0.64	0.68	1.21	2.02		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	155	450	450	CONC	2.70	87.0	468.4772	2.9456	0.4923	0.332	
17	18	0.71	0.68	1.34	3.36		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.49	74.97	101.66	119.16	174.18	252	450	450	CONC	2.80	87.0	477.0738	2.9997	0.4834	0.529	
To STREET 15, Pipe 20 - 26			4.61				0.00			0.00			0.00			0.00	12.10																
21	22	0.13	0.68	0.25	0.25		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	19	450	450	CONC	1.30	66.0	325.0710	2.0439	0.5382	0.058	
22	23	0.07	0.68	0.13	0.38		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.54	74.80	101.43	118.89	173.78	28	450	450	CONC	1.60	9.5	360.6339	2.2675	0.0698	0.078	
23	24	0.77	0.68	1.46	1.83		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.61	74.55	101.09	118.48	173.19	137	450	450	CONC	2.20	92.0	422.8807	2.6589	0.5767	0.323	
24	25	0.73	0.68	1.38	3.21		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	11.18	72.54	98.33	115.23	168.41	233	450	450	CONC	2.30	91.5	432.3849	2.7187	0.5609	0.539	
To STREET 15, Pipe 26 - 32			4.54				0.00			0.00			0.00			0.00	11.75	70.69	95.80	112.25	164.03	321	600	600	CONC	0.45	91.5	411.8915	1.4568	1.0468	0.779		
STREET 19																																	
8	9	0.05	0.68	0.09	0.09		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	7	450	450	CONC	0.60	22.0	220.8423	1.3886	0.2641	0.033	
9	10	0.03	0.68	0.06	0.15		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.26	75.81	102.82	120.52	176.18	11	450	450	CONC	1.35	13.0	331.2634	2.0829	0.1040	0.035	
10	11	0.44	0.68	0.83	0.98		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.37	75.42	102.29	119.90	175.26	74	450	450	CONC	2.10	82.5	413.1580	2.5978	0.5293	0.179	
1	2	0.40	0.68	0.76	0.76		0.00	0.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	58	450	450	CONC	0.20	59.5	127.5033	0.8017	1.2370	0.455	
2	3	0.18	0.																														

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
 Collector Roads Return Frequency = 5 years
 Arterial Roads Return Frequency = 10 years

LOCATION		AREA (Ha)												FLOW							SEWER DATA											
		2 YEAR		5 YEAR		10 YEAR		100 YEAR		Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	Type	Slope	Length	Capacity	Velocity	Time of	Ratio								
Location	From Node	To Node	Area (Ha)	R	Indiv.	Accum.	Area (Ha)	R	Indiv.	Accum.	Area (Ha)	R	Indiv.	Accum.	Conc.	2 Year	5 Year	10 Year	100 Year	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	Low (min)	Q/Q full				
STREET 15																																
	13	14	0.51	0.68	0.96	0.96		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	74	450	450	CONC	0.20	83.0	127.5033	0.8017	1.7255	0.581		
					0.00	0.96	2.46	0.70	4.79	4.79			0.00	0.00		0.00	0.00	12.00														
	14	15	0.54	0.68	1.02	1.98		0.00	4.79		0.00	0.00		0.00	0.00	12.00	69.89	94.70	110.96	162.13	592	900	900	CONC	0.20	98.5	809.5958	1.2726	1.2900	0.731		
Contribution From STREET 19, Pipe 12 - 15			6.35			0.00										0.00	18.12															
	15	20	0.34	0.68	0.64	8.98		0.00	4.79		0.00	0.00		0.00	0.00	18.12	55.26	74.66	87.40	127.56	854	900	900	CONC	0.80	77.5	1619.1915	2.5452	0.5075	0.527		
Contribution From STREET 13, Pipe 18 - 20			4.61			0.00										0.00	12.10															
Contribution From STREET 13, Pipe 19 - 20			0.17			0.00										0.00	10.25															
	20	26	0.34	0.68	0.64	14.40		0.00	4.79		0.00	0.00		0.00	0.00	18.63	54.35	73.41	85.92	125.40	1134	1200	1200	CONC	0.15	79.0	1509.9717	1.3351	0.9862	0.751		
Contribution From STREET 13, Pipe 25 - 26			4.54			0.00										0.00	12.79															
	26	32	0.35	0.68	0.66	19.60		0.00	4.79		0.00	0.00		0.00	0.00	19.61	52.66	71.11	83.22	121.43	1373	1200	1200	CONC	0.20	85.0	1743.5652	1.5417	0.9189	0.787		
To STREET 17, Pipe 32 - 64			19.60			4.79					0.00			0.00		20.53																
STREET 17																																
	107	108	0.55	0.68	1.04	1.04		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	80	450	450	CONC	0.50	74.0	201.6005	1.2676	0.9730	0.396		
	108	109	0.53	0.68	1.00	2.04		0.00	0.00		0.00	0.00		0.00	0.00	10.97	73.26	99.32	116.40	170.13	150	525	525	CONC	0.20	89.5	192.3297	0.8885	1.6789	0.778		
	109	110	0.14	0.68	0.26	2.31		0.00	0.00		0.00	0.00		0.00	0.00	12.65	67.93	92.00	107.79	157.47	157	525	525	CONC	0.25	10.5	215.0311	0.9933	0.1762	0.729		
	110	111			0.00	2.31		0.00	0.00		0.00	0.00		0.00	0.00	12.83	67.42	91.30	106.96	156.27	155	525	525	CONC	0.25	24.0	215.0311	0.9933	0.4027	0.723		
To SERVICING BLOCK 2, Pipe 111 - 113			2.31			0.00					0.00			0.00		13.23																
	100	101	0.04	0.68	0.08	0.08		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	6	450	450	CONC	2.80	18.5	477.0738	2.9997	0.1028	0.012		
	101	102	0.03	0.68	0.06	0.13		0.00	0.00		0.00	0.00		0.00	0.00	10.10	76.41	103.65	121.51	177.62	10	450	450	CONC	2.05	18.0	408.2099	2.5667	0.1169	0.025		
	102	103	0.06	0.68	0.11	0.25		0.00	0.00		0.00	0.00		0.00	0.00	10.22	75.97	103.05	120.79	176.58	19	450	450	CONC	0.20	43.0	127.5033	0.8017	0.8939	0.146		
	103	104	0.03	0.68	0.06	0.30		0.00	0.00		0.00	0.00		0.00	0.00	11.11	72.78	98.66	115.62	168.98	22	450	450	CONC	0.20	7.0	127.5033	0.8017	0.1455	0.173		
	104	105	0.46	0.68	0.87	1.17		0.00	0.00		0.00	0.00		0.00	0.00	11.26	72.29	97.98	114.83	167.81	85	450	450	CONC	0.20	76.0	127.5033	0.8017	1.5800	0.664		
			-2.43	0.68	-4.59	-3.42		0.00	0.00		0.00	0.00		0.00	0.00																	
			0.52	0.68	0.98	2.44		0.00	0.00		0.00	0.00		0.00	0.00																	
	105	106			0.00	-2.44		0.00	0.00		0.00	0.00		2.43	0.68	4.59	4.59	12.84	67.39	91.26	106.91	156.19	553	900	900	CONC	0.15	76.0	701.1305	1.1021	1.1493	0.789
To SERVICING BLOCK 2, Pipe 106 - 113			-2.44			0.00					0.00			4.59		13.99																
	27	28	0.05	0.68	0.09	0.09		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	7	450	450	CONC	0.20	25.0	127.5033	0.8017	0.5197	0.057		
	28	29	0.08	0.68	0.15	0.25		0.00	0.00		0.00	0.00		0.00	0.00	10.52	74.87	101.53	119.00	173.94	18	450	450	CONC	0.20	7.5	127.5033	0.8017	0.1559	0.144		
	29	30	0.71	0.68	1.34	1.59		0.00	0.00		0.00	0.00		0.00	0.00	10.68	74.30	100.75	118.09	172.61	118	450	450	CONC	0.50	87.5	201.6005	1.2676	1.1505	0.585		
	30	31	0.72	0.68	1.36	2.95		0.00	0.00		0.00	0.00		0.00	0.00	11.83	70.44	95.44	111.84	163.43	208	450	450	CONC	0.90	87.5	270.4754	1.7006	0.8575	0.768		
	31	32	0.56	0.68	1.06	4.01		0.00	0.00		0.00	0.00		0.00	0.00	12.68	67.84	91.87	107.64	157.26	272	675	675	CONC	0.20	87.5	375.9224	1.0505	1.3882	0.723		
Contribution From STREET 15, Pipe 26 - 32			19.60			4.79					0.00			0.00		20.53																
	32	64	0.13	0.68	0.25	23.86		0.00	4.79		0.00	0.00		0.00	0.00	20.53	51.19	69.10	80.86	117.97	1552	1200	1200	CONC	0.75	80.0	3376.3995	2.9854	0.4466	0.460		
To STREET 12, Pipe 64 - 66			23.86			4.79					0.00			0.00		20.98																
STREET 7																																
	56	57	0.53	0.68	1.00	1.00		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56	77	450	450	CONC	0.85	70.5	262.8549	1.6527	0.7109	0.293		
To STREET 12, Pipe 57 - 64			1.00			0.00					0.00			0.00		10.71																
	58	59	0.44	0.68	0.83	0.83		0.00	0.00		0.00	0.00		0.00	0.00	10.00	76.81	104.19	122.14	178.56</												

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years
Collector Roads Return Frequency = 5 years
Arterial Roads Return Frequency = 10 years

Manning

LOCATION		AREA (Ha)												FLOW								SEWER DATA											
Location	From Node	To Node	2 YEAR				5 YEAR				10 YEAR				100 YEAR				Time of	Intensity	Intensity	Intensity	Intensity	Peak Flow	DIA. (mm)	DIA. (mm)	TYPE	SLOPE	LENGTH	CAPACITY	VELOCIT	TIME OF	RATIO
			AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	AREA (Ha)	R	Indiv. 2.78 AC	Accum. 2.78 AC	Conc. (min)	2 Year (mm/h)	5 Year (mm/h)	10 Year (mm/h)	100 Year (mm/h)	Q (l/s)	(actual)	(nominal)	(%)	(m)	(l/s)	(m/s)	LOW (min)	Q/Q full	
STREET 8																																	
	46	47	0.18	0.68	0.34	0.34			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	26	450	450	CONC	0.20	11.0	127.5033	0.8017	0.2287	0.205
	47	48	0.58	0.68	1.10	1.44			0.00	0.00			0.00	0.00			0.00	0.00	10.23	75.94	103.00	120.74	176.49	109	450	450	CONC	0.25	71.5	142.5531	0.8963	1.3295	0.765
	48	49	0.52	0.68	0.98	2.42			0.00	0.00			0.00	0.00			0.00	0.00	11.56	71.30	96.62	113.23	165.47	173	600	600	CONC	0.15	71.5	237.8056	0.8411	1.4169	0.725
To STREET 12, Pipe 49 - 55						2.42			0.00				0.00				0.00		12.98														
	50	51	0.19	0.68	0.36	0.36			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	28	450	450	CONC	0.20	45.5	127.5033	0.8017	0.9459	0.216
	51	52	0.06	0.68	0.11	0.47			0.00	0.00			0.00	0.00			0.00	0.00	10.95	73.35	99.45	116.55	170.35	35	450	450	CONC	0.20	12.0	127.5033	0.8017	0.2495	0.272
	52	53	0.58	0.68	1.10	1.57			0.00	0.00			0.00	0.00			0.00	0.00	11.20	72.50	98.28	115.17	168.32	114	450	450	CONC	0.25	66.0	142.5531	0.8963	1.2272	0.798
To STREET 12, Pipe 55 - 57						2.44			0.00				0.00				0.00		13.53														
STREET 9																																	
	36	37	0.08	0.68	0.15	0.15			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	12	450	450	CONC	0.20	45.0	127.5033	0.8017	0.9355	0.091
Contribution From STREET 12, Pipe 35 - 37						1.15			0.00				0.00				0.00		13.53														
	37	38	0.51	0.68	0.96	2.27			0.00	0.00			0.00	0.00			0.00	0.00	13.53	65.46	88.61	103.80	151.62	148	525	525	CONC	0.20	82.0	192.3297	0.8885	1.5382	0.772
	38	45	0.50	0.68	0.95	3.21			0.00	0.00			0.00	0.00			0.00	0.00	15.07	61.60	83.32	97.58	142.49	198	600	600	CONC	0.20	82.0	274.5943	0.9712	1.4072	0.721
To STREET 12, Pipe 45 - 49						3.21			0.00				0.00				0.00		16.48														
STREET 10																																	
	39	40	0.32	0.68	0.60	0.60			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	46	450	450	CONC	0.20	66.5	127.5033	0.8017	1.3825	0.364
	40	44	0.34	0.68	0.64	1.25			0.00	0.00			0.00	0.00			0.00	0.00	11.38	71.88	97.42	114.16	166.84	90	450	450	CONC	0.20	81.5	127.5033	0.8017	1.6943	0.703
To STREET 12, Pipe 44 - 45						1.25			0.00				0.00				0.00		13.08														
STREET 12																																	
	33	34	0.22	0.68	0.42	0.42			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	32	450	450	CONC	0.20	71.5	127.5033	0.8017	1.4864	0.251
	34	35	0.06	0.68	0.11	0.53			0.00	0.00			0.00	0.00			0.00	0.00	11.49	71.53	96.95	113.61	166.02	38	450	450	CONC	0.20	12.0	127.5033	0.8017	0.2495	0.297
	35	37	0.33	0.68	0.62	1.15			0.00	0.00			0.00	0.00			0.00	0.00	11.74	70.73	95.84	112.30	164.11	82	450	450	CONC	0.20	86.5	127.5033	0.8017	1.7983	0.640
To STREET 9, Pipe 37 - 38						1.15			0.00				0.00				0.00		13.53														
	41	42	0.19	0.68	0.36	0.36			0.00	0.00			0.00	0.00			0.00	0.00	10.00	76.81	104.19	122.14	178.56	28	450	450	CONC	0.20	53.0	127.5033	0.8017	1.1018	0.216
	42	43	0.07	0.68	0.13	0.49			0.00	0.00			0.00	0.00			0.00	0.00	11.10	72.82	98.71	115.69	169.08	36	450	450	CONC	0.20	12.5	127.5033	0.8017	0.2599	0.281
	43	44	0.19	0.68	0.36	0.85			0.00	0.00			0.00	0.00			0.00	0.00	11.36	71.94	97.51	114.27	167.00	61	450	450	CONC	0.60	39.0	220.8423	1.3886	0.4681	0.277
Contribution From STREET 10, Pipe 40 - 44						1.25			0.00				0.00				0.00		13.08														
	44	45	0.24	0.68	0.45	2.55			0.00	0.00			0.00	0.00			0.00	0.00	13.08	66.71	90.33	105.82	154.59	170	525	525	CONC	0.25	46.5	215.0311	0.9933	0.7802	0.792
Contribution From STREET 9, Pipe 38 - 45						3.21			0.00				0.00				0.00		16.48														
	45	49	0.36	0.68	0.68	6.45			0.00	0.00			0.00	0.00			0.00	0.00	16.48	58.48	79.06	92.57	135.14	377	750	750	CONC	0.20	79.0	497.8726	1.1270	1.1683	0.757
Contribution From STREET 8, Pipe 48 - 49						2.42			0.00				0.00				0.00		12.98														
	49	55	0.31	0.68	0.59	9.45			0.00	0.00			0.00	0.00			0.00	0.00	17.65	56.15	75.88	88.82	129.64	531	900	900	CONC	0.15	85.0	701.1305	1.1021	1.2854	0.757
Contribution From STREET 8, Pipe 53 - 55						2.44			0.00				0.00				0.00		13.53														
Contribution From STREET 13, Pipe 54 - 55						0.17			0.00				0.00				0.00		10.17														
	55	57				0.00	12.06	0.39	0.68	0.74	0.74		0.00	0.00			0.00	0.00	18.93	53.81	72.68	85.07	124.14	703	900	900	CONC	0.25	79.0	905.1556	1.4228	0.9254	0.776
Contribution From STREET 7, Pipe 56 - 57						1.00			0.00				0.00				0.00		10.71														
	57	64				0.00	13.06	0.40	0.68	0.76	1.49		0.00	0.00			0.00	0.00	19.86	52.26	70.56	82.58	120.49	788	900	900	CONC	0.55	85.0	1342.5627	2.1104	0.6713	0.587
Contribution From STREET 17, Pipe 32 - 64						23.86			4.79				0.00				0.00		20.98														
Contribution From STREET 7, Pipe 63 - 64						3.31			0.00				0.00				0.00		13.76														
	64	66				0.00	40.23	0.19	0.68	0.36	6.64		0.00	0.00			0.00	0.00	20.98	50.51	68.17	79.77	116.37	2484	1500	1500	CONC	0.20	82.0	3161.2940	1.7889	0.7640	0.786
Contribution From STREET 6, Pipe 65 - 66						0.62			0.00				0.00				0.00		10.48														
	66	72				0.00	40.85	0.19	0.68	0.36	7.00		0.00	0.00			0.00	0.00	21.74	49.38	66.64	77.97	113.73	2484	1500	1500	CONC	0.20	79.0	3161.2940	1.7889	0.7360	0.786
Contribution From STREET 6, Pipe 71 - 72						2.51			0.00				0.00				0.00		13.39														
	72	74				0.00	43.37	0.20	0.68	0.38	7.38		0.00	0.00			0.00	0.00															

Definitions:

Definitions:

$Q \equiv$ Peak Flow in Litres per second (l/s)

Q = Peak Flow in Litres per second (L/s)
 A = Areas in hectares (ha)

J = Rainfall Intensity (mm/h)

R = Runoff Coefficient

R = Runoff Coefficient

Net

Notes:

1) Ottawa Rainfall-Intensity
2) Min. Velocity = 0.80 m/s

named:

ghed:

Cardinal Creek Village South ESR

Cardinal Creek Village South FSR

City of Ottawa

City of Ottawa

Date:

STORM SEWER CALCULATION SHEET (RATIONAL METHOD)

Local Roads Return Frequency = 2 years

Collector Roads Return Frequency = 5 years

Arterial Roads Return Frequency = 10 years

Manning

Definitions:

$\Omega \equiv 2.78 \text{ AIR}$ where

$\Omega \equiv$ Peak Flow in Litres per second (l/s)

Δ = Areas in hectare

A = Areas in hectares (ha)
I = Rainfall Intensity (mm/h)

I = Rainfall Intensity (mm/h)

No

Notes:

3) Min. Velocity = 0.80 m/s

urned:

,med.

ked:

ked.

References:

Cardinal Creek Village South ESB

Carroll Creek Village South Unit

City of Ottawa

Table C1: Cardinal Creek Village South
Preliminary 100-year HGL Analysis

SWM Pond	MH-ID	Invert Elevation (m)	Top of MH (m)	Max HGL (m)	Top of MH Freeboard (m)
Pond 1	MH-100	88.00	91.15	88.04	3.11
	MH-101	87.44	90.43	87.50	2.93
	MH-102	86.78	90.07	86.96	3.11
	MH-103	86.66	89.65	86.94	2.71
	MH-104	86.62	89.64	86.95	2.69
	MH-105	86.02	89.53	86.70	2.83
	MH-106	85.64	89.40	86.15	3.25
	MH-107	87.09	90.05	87.32	2.73
	MH-108	86.60	89.67	87.05	2.62
	MH-109	86.39	89.49	86.79	2.70
	MH-110	86.34	89.46	86.70	2.76
	MH-111	86.25	89.40	86.46	2.94
	MH-113	83.27	88.91	83.78	5.13
	MH-114	81.17	85.51	81.86	3.65
	MH-115	80.03	83.63	81.09	2.54
	MH-116	84.31	88.08	84.44	3.64
	MH-117	82.32	85.95	82.48	3.47
	MH-118	80.32	84.37	80.87	3.50
	MH-119	77.60	83.08	78.61	4.47
	MH-2000	77.20	80.90	78.40	2.50
Pond 2	MH-1	91.99	94.94	92.25	2.69
	MH-2	91.84	94.85	92.13	2.72
	MH-3	91.62	94.64	91.87	2.77
	MH-4	91.47	94.49	91.65	2.84
	MH-5	90.03	93.16	90.43	2.73
	MH-6	89.84	93.14	90.36	2.78
	MH-7	89.64	93.02	90.22	2.80
	MH-8	92.02	94.97	92.09	2.88
	MH-9	91.83	94.84	91.90	2.94
	MH-10	91.59	94.60	91.74	2.86
	MH-11	89.11	92.83	89.42	3.41
	MH-12	87.53	90.81	88.54	2.27
	MH-13	87.99	90.94	88.56	2.38
	MH-14	87.35	90.82	88.45	2.37
	MH-15	87.14	90.66	88.27	2.40
	MH-16	92.50	95.46	92.71	2.75
	MH-17	89.95	93.10	90.23	2.87
	MH-18	86.96	90.59	88.23	2.36
	MH-19	87.37	90.33	87.86	2.47
	MH-20	86.19	89.94	87.86	2.08
	MH-21	92.53	95.48	92.62	2.86
	MH-22	91.61	94.62	91.71	2.91
	MH-23	91.43	94.41	91.64	2.77
	MH-24	89.39	92.36	89.70	2.66

Table C1: Cardinal Creek Village South
Preliminary 100-year HGL Analysis

SWM Pond	MH-ID	Invert Elevation (m)	Top of MH (m)	Max HGL (m)	Top of MH Freeboard (m)
Pond 2	MH-25	87.04	90.33	88.31	2.02
	MH-26	85.93	89.79	87.69	2.10
	MH-27	88.45	91.40	88.57	2.83
	MH-28	88.34	91.36	88.57	2.79
	MH-29	88.26	91.35	88.56	2.79
	MH-30	87.80	90.79	88.19	2.60
	MH-31	86.63	89.98	87.33	2.65
	MH-32	85.49	89.66	87.17	2.49
	MH-33	87.33	90.28	87.92	2.36
	MH-34	87.15	90.17	87.89	2.28
	MH-35	87.10	90.16	87.89	2.27
	MH-36	87.15	90.10	87.73	2.37
	MH-37	86.85	90.03	87.73	2.30
	MH-38	86.61	89.91	87.55	2.36
	MH-39	87.13	90.08	87.69	2.39
	MH-40	86.98	89.98	87.66	2.32
	MH-41	87.33	90.28	87.53	2.75
	MH-42	87.19	90.20	87.50	2.70
	MH-43	87.13	90.12	87.49	2.63
	MH-44	86.74	89.86	87.47	2.39
	MH-45	86.30	89.79	87.34	2.45
	MH-46	86.94	89.89	87.51	2.38
	MH-47	86.89	89.88	87.51	2.37
	MH-48	86.56	89.77	87.32	2.45
	MH-49	85.99	89.67	87.17	2.50
	MH-50	86.95	89.90	87.49	2.41
	MH-51	86.83	89.83	87.48	2.35
	MH-52	86.77	89.82	87.47	2.35
	MH-53	86.53	89.72	87.28	2.44
	MH-54	87.28	90.24	87.34	2.90
	MH-55	85.84	89.54	87.02	2.52
	MH-56	87.05	90.00	87.25	2.76
	MH-57	85.62	89.42	86.78	2.64
	MH-58	87.04	89.99	87.23	2.76
	MH-59	86.68	89.70	86.97	2.73
	MH-60	86.58	89.61	86.89	2.72
	MH-61	85.57	88.77	86.75	2.02
	MH-62	85.39	88.75	86.69	2.06
	MH-63	85.23	88.66	86.60	2.06
	MH-64	84.31	88.57	86.47	2.10
	MH-65	85.86	88.81	86.24	2.57
	MH-66	84.13	88.45	86.21	2.24
	MH-67	85.84	88.80	86.30	2.50
	MH-68	85.71	88.69	86.30	2.39

**Table C1: Cardinal Creek Village South
Preliminary 100-year HGL Analysis**

SWM Pond	MH-ID	Invert Elevation (m)	Top of MH (m)	Max HGL (m)	Top of MH Freeboard (m)
Pond 2	MH-69	85.61	88.61	86.29	2.32
	MH-70	85.45	88.46	86.15	2.31
	MH-71	85.28	88.44	86.11	2.33
	MH-72	83.80	88.33	85.94	2.39
	MH-73	85.52	88.47	85.94	2.53
	MH-74	83.57	88.20	85.75	2.45
	MH-75	85.35	88.31	85.73	2.59
	MH-76	83.44	88.14	85.65	2.49
	MH-77	85.42	88.37	85.51	2.86
	MH-78	83.11	88.11	85.23	2.88
	MH-79	85.50	88.45	85.62	2.83
	MH-80	85.36	88.34	85.60	2.74
	MH-81	85.22	88.27	85.56	2.71
	MH-82	85.13	88.26	85.54	2.72
	MH-83	82.99	88.00	84.98	3.02
	MH-84	82.74	87.87	84.83	3.04
	MH-85	82.46	87.72	84.46	3.26
	HW1	82.31	-	84.35	-
				Min	2.02
				Max	5.13
				Average	2.68

Notes:

- (1) Analysis assumes the use of ICDs throughout the development, therefore the Rational Method flows as per DSEL's storm design sheets were increased by 35% to account for additional flows captured into the minor system during the 100-year event.
- (2) Analysis assumes a preliminary 100-year water level of 84.35m in Pond 2.
- (3) Free outlet condition assumed at MH-2000 outfall, as the preliminary 100-yr HGL in this MH is below the invert of the inlet pipe (100-yr HGL of 75.652m based on the Nov. 2024 preliminary Pond 1 modelling update).
- (4) Model Name: CCVS_v02.2.inp.



JFSA Canada Inc.
52 Springbrook Drive,
Ottawa, ON K2S 1B9
T 613-836-3884 F 613-836-0332

jfsa.com

Attachment D

Cox Country Road Culvert Analysis

Area ID	Area (ha)	Soil Description	Soil Group	Land Use Description	CN	CN*	Tp (h)
eCCR	74.3	F1, G4, R3	B / BC / D	50% B = 70% Woods, 30% Farm; 40% BC = 15% Imp, 25% Woods, 60% Urban Lawn; 10% D = 15% Imp, 15% Woods, 70% Urban Lawn	71.525	61	1.29

As per Ontario Soil Map 58 and the MTO Manual:

Short ID	Soil Description	Soil Group
F1	Farmington, fine sandy loam or sandy loam or loam, good drainage	B
G4	Grenville, sandy loam or loam or silt loam, mix of good and imperfect drainage	BC
R1	Rideau, silty clay or clay, imperfect drainage	D
R3	Rideau, silty clay or clay, poor drainage	D
X1	Escarpment, marine clay or heavy clay	D
X3	Escarpment, limestone or dolomite or sandstone scarps	D

Calculation of Time to Peak (Tp)

EXISTING CONDITIONS		
UNITS	eCCR metric	
Area	(ha)	74.3
Hydrologic Soil Group ¹		B / BC / D
CN ²		72
C (as per Rational Method) ³		0.25
Length of Channel ⁴	(m)	1997
Elevation of Channel Outlet	(m)	87.31
Elevation of Channel Headwater	(m)	111.5
Average Slope of Channel	(m/m)	0.0121
Time to Peak (=2/3 Tc)		
Kirpich	(min)	25
FAA	(min)	77
SCS	(min)	111
Brainby Williams	(min)	48
		1.29

NOTES:

- 1- As per Ontario Soil Map
- 2- See CN C spreadsheet for detail
- 3- See CN C spreadsheet for detail
- 4- As measured on topographic map provided by DSEL



Tc Equations applicability

Kirpich	Best for rural watersheds with slopes ranging from 3% to 10%
FAA	Best for flat drainage areas (was developed for air field drainage) but used frequently for urban watersheds
SCS	Best for Agricultural SW in general and urban SW < 2000 acres
BW	One of the best method for predicting Tc. Especially good for small culvert design

Result
in input L as

Tc Equations and inputs (imperial unless otherwise noted)

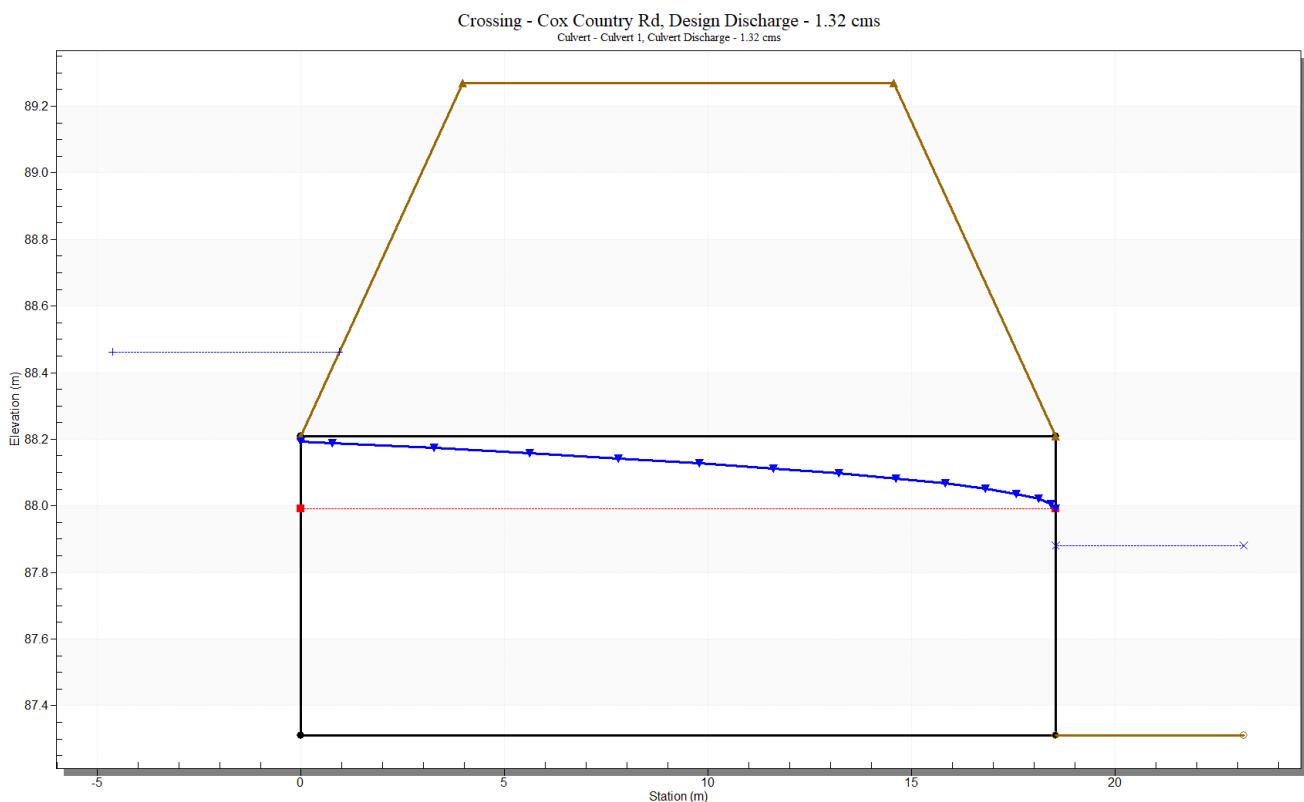
Kirpich	$Tc = 0.0078 L^{0.77} S^{-0.385}$	(min)	(ft)
FAA	$Tc = (1.8(1.1-C)L^{0.50}) / (S^{0.333})$	(min)	(ft)
SCS Lag	$Tc = (100L^{0.8}((1000/CN)-9)^{0.7} / (1900 S^{0.5}))$	(min)	(ft)
BW (metr)	$Tc = (0.605L) / (S^{0.2} A^{0.1})$	(hrs)	(km)

HY-8 Analysis Results

Crossing Summary Table

Culvert Crossing: Cox Country Rd

Headwater Elevation (m)	Total Discharge (cms)	Culvert 1 Discharge (cms)	Roadway Discharge (cms)	Iterations
87.31	0.00	0.00	0.00	1
87.63	0.13	0.13	0.00	1
87.76	0.26	0.26	0.00	1
87.87	0.40	0.40	0.00	1
87.96	0.53	0.53	0.00	1
88.05	0.66	0.66	0.00	1
88.13	0.79	0.79	0.00	1
88.21	0.93	0.93	0.00	1
88.29	1.06	1.06	0.00	1
88.37	1.19	1.19	0.00	1
88.46	1.32	1.32	0.00	1
89.27	2.28	2.28	0.00	Overtopping



```

00001> 20 Metric units / ID Numbers OFF
00002> *-----*
00003> *-----* Per 5.0 Jan 1998 Version 1 INPUT DATA FILE
00004> *-----*
00005> * Project Name : [Cardinal Creek Village]
00006> * Author : [J.P. Laboucane]
00007> * Date : [2021/07/07]
00008> * Modeler : [Laura Pipkins, P.Eng.]
00009> * Company : [J.P. Laboucane and Associates]
00010> * Address : [1000 5th St. S.E., Suite 200]
00011> *-----* 25-Year, 3-Hour Chicago Storm
00012> *-----* ZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
00013> START [**25YRCH3.stm*] <-storm filename, one per line for NSTORM time
00014> *-----*
00015> READ STORM [STORM_FILENAME=[*strom.001*]
00016> *-----*
00017> *-----* DEFAULT VALUES
00018> *-----* [CAEADef=[1], read and print values
00019> DEFVALDef=[1], DEFVALFILENAME=[*Octave_val*]
00020> *-----*
00021> *-----* CM -> CN - based on Ontario Soil Map 56, Nov 1985 MTO Manual Chart H2-64,
00022> *-----* Lidar data, May 2000 SWMHYMO USER's Manual, air photos, assume good condition
00023> *-----*
00024> * Time to Peak = 2/3 of FAA Tc
00025> *-----* Time to Peak = 2/3 of FAA Tc
00026> *-----* EXISTING CONDITIONS - Drainage to South Tributary East of Cox County Road
00027> *-----* 100-Year, 3-Hour Chicago Storm
00028> *-----* Existing Drainage from Subject Site to Ottawa River
00029> DESIGN_NASHYC NHYD=[*eCCK*], DT=[1[min], ARA=[74.3][ha],
00030> RAINFALL=[0.0], RAINFALLT=[0.0], RAINFALLH=[0.0], END=1
00031> *-----*
00032> *-----*
00033> *-----*
00034> *-----*
00035> *-----* STORMS
00036> *-----*
00037> *-----* 25 mm Storm based on 2-Year, 3-Hour Chicago Storm
00038> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
00039> *-----* [*25YRCH3.stm*] <-storm filename, one per line for NSTORM time
00040> *-----*
00041> *-----* 2-Year, 3-Hour Chicago Storm
00042> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[002]
00043> *-----* [*002YRCH3.stm*] <-storm filename, one per line for NSTORM time
00044> *-----* 5-Year, 3-Hour Chicago Storm
00045> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[005]
00046> *-----* [*005YRCH3.stm*] <-storm filename, one per line for NSTORM time
00047> *-----* 10-Year, 3-Hour Chicago Storm
00048> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[010]
00049> *-----* [*10YRCH3.stm*] <-storm filename, one per line for NSTORM time
00050> *-----* 100-Year, 3-Hour Chicago Storm
00051> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00052> *-----* 50-Year, 3-Hour Chicago Storm
00053> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
00054> *-----* 100-Year, 24-Hour SCS Storm
00055> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00056> *-----* 50-Year, 3-Hour Chicago Storm
00057> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[050]
00058> *-----* 100-Year, 3-Hour Chicago Storm
00059> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00060> *-----* 100-Year, 24-Hour SCS Storm
00061> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00062> *-----* 100-Year, 3-Hour Chicago Storm
00063> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
00064> *-----* 100-Year, 24-Hour SCS Storm
00065> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
00066> *-----* 100-Year, 24-Hour SCS Storm
00067> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
00068> *-----* 100-Year, 24-Hour SCS Storm
00069> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
00070> *-----* 100-Year, 24-Hour SCS Storm
00071> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
00072> *-----* 100-Year, 24-Hour SCS Storm
00073> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
00074> *-----* 100-Year, 24-Hour SCS Storm
00075> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
00076> *-----* 100-Year, 24-Hour SCS Storm
00077> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
00078> *-----* 100-Year, 24-Hour SCS Storm
00079> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[125]
00080> *-----* 100-Year, 24-Hour SCS Storm
00081> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[130]
00082> *-----* 100-Year, 24-Hour SCS Storm
00083> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[130]
00084> *-----* 100-Year, 24-Hour SCS Storm
00085> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[130]
00086> *-----* 100-Year, 24-Hour SCS Storm
00087> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[130]
00088> *-----* 100-Year, 24-Hour SCS Storm
00089> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[130]
00090> *-----* July 1st, 1979 Storm - Ottawa International Airport
00091> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[979]
00092> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[979]
00093> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[979]
00094> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[988]
00095> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[988]
00096> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[988]
00097> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[986]
00098> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[986]
00099> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00100> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00101> *-----* 100-Year, 3-Hour Chicago Storm + 20+
00102> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00103> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00104> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00105> *-----* *START [TZERO=0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
00106> FINISH

```

```

00001> ****
00002> SSSSS W W M M H H Y Y M M OOO 222 000 11 555 ****
00003> SSSSS WWW MM MM H H Y Y M M M OOO 2 0 0 11 555 ****
00004> SSSSS WWW MM MM H HHHHH Y M M M OOO 2 0 0 11 55 Ver 5.500
00005> SSSSS W W M M H H Y M M M OOO 222 0 0 11 555 FEB 2015
00006> SSSSS W W M M H H Y M M M OOO 2 0 0 11 555 ****
00007> SSSSS W W M M H H Y M M M OOO 2 0 0 11 555 ****
00008> SSSSS W W M M H H Y M M M OOO 2 0 0 11 5 # 2549237
00009> StormWater Management Hydrologic Model 222 000 11 555 ****
00010> ****
00011> ***** SWMHYMO Ver 5.500 ****
00012> ***** A Single event and continuous hydrologic simulation model ****
00013> ***** based on the principles of HMO and its successors ****
00014> ***** OTNHEM and its successors ****
00015> ***** Distributed by: J.F. Sabourin and Associates Inc. ****
00016> ***** Ottawa, Ontario, Canada K2B 3T4 ****
00017> ***** 1000 Lakeshore, Quebec, QC G1V 4S8 ****
00018> ***** E-Mail: swmhymo@fsm.ca ****
00019> ****
00020> **** Licensed user: ****
00021> **** Ottawa SERIAL#:#2549237 ****
00022> ****
00023> ****
00024> ****
00025> ****
00026> **** PROG ARRAY DIMENSIONS ****
00027> Max. number of rainfall points : 105408
00028> Max. number of rainfall points : 105408
00029> Max. number of flow points : 105408
00030> Max. number of flow points : 105408
00031> ****
00032> ****
00033> ****
00034> ****
00035> **** S U M M A R Y O U T P U T ****
00036> ****
00037> **** RUN DATE: 2021-07-19 TIME: 10:38:00 ****
00038> **** RUN NUMBER: 004037 ****
00039> ****
00040> * Input file: T:\PROJ\959\02\11\202001 Subm1\Design\SWMHYMO\202107 Pre-Dev\CCV_v1.dat
00041> * Output file: T:\PROJ\959\02\11\202001 Subm1\Design\SWMHYMO\202107 Pre-Dev\CCV_v1.out
00042> * History file: T:\PROJ\959\02\11\202001 Subm1\Design\SWMHYMO\202107 Pre-Dev\CCV_v1.sum
00043> * User comments:
00044> 1:
00045> 2:
00046> 3:
00047> ****
00048> ****
00049> ****
00050> **** SWMHYMO Ver5.02/Jan 2001 <BETA> / INPUT DATA FILE ****
00051> ****
00052> ****
00053> # Project Name : [Cardinal Creek Village]
00054> # Project Number : [959-11]
00055> # Date : 2021/07/07
00056> # Modeler : Laura Pipkins, P.Eng.
00057> # Company : J.F. Sabourin and Associates
00058> # License # : 2582634
00059> # FileID : RO099:CO0001
00060> ** END OF RUN : 24
00061> ****
00062> ****
00063> ****
00064> ****
00065> ****
00066> ****
00067> ****
00068> **** RUN#:COMMAND#
00069> RO0251:CO0001
00070> ****
00071> START
00072> [TZERO= 2.00 hrs on 0] {Imperial, 2-metric output}
00073> [INSTORM= 1]
00074> [INRSH= 0]
00075> ****
00076> # SWMHYMO Ver5.02/Jan 2001 <BETA> / INPUT DATA FILE
00077> ****
00078> # Project Name : [Cardinal Creek Village]
00079> # Project Number : [959-11]
00080> # Date : 2021/07/07
00081> # Modeler : Laura Pipkins, P.Eng.
00082> # Company : J.F. Sabourin and Associates
00083> # License # : 2582634
00084> # FileID : RO0251:CO0001
00085> ****
00086> **** READ STORM
00087> # Filename = storm.001
00088> # Comment = CHICAGO STORM 25 Year, 3 Hours
00089> # [IN=13.20 mm/hr] [DCAV= 4.14 /hr] [Fw= .00 mm]
00090> RO0251:CO0003:
00091> **** DEFAULT VALUE****
00092> # Existing Drainage from Subject Site to Ottawa River
00093> ICASEdW = 1 (read and print data)
00094> # File comment: [Parameters for City of Ottawa Projects]
00095> # The following parameters are used in the design STANDHY COM
00096> # Horton's infiltration equation parameters:
00097> [Fw= 76.20 mm/hr] [Fc=13.20 mm/hr] [DCAV= 4.14 /hr] [Fw= .00 mm]
00098> Parameters for IMPERVIOUS surfaces in STANDHY:
00099> [IApere= 4.67 mm] [LGP=40.00 m] [MNPF=.250]
00100> Parameters for IMPERVIOUS surfaces in STANDHY:
00101> [IApere= 4.67 mm] [LGP=40.00 m] [MNPF=.250]
00102> Parameters used in NASHYD:
00103> [IA= 4.67 mm] [MN= 0.01]
00104> Average monthly Fan Evaporation data in (mm)
00105> JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00106> .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00107> Average monthly Potential Evapotranspiration in (mm)
00108> JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00109> .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00110> # CN -> CN based on Ontario Soil Map 58, Nov 1985 MTO Manual Chart H2-6A,
00111> # Lidar data, May 2000 SWMHYMO USER's Manual, air photos, assume good condition
00112> # Time to Peak = 2 /3 of FAA Tc
00113> ****
00114> **** Existing Drainage to South Tributary East of Cox County Road
00115> ****
00116> ****
00117> # Existing Drainage from Subject Site to Ottawa River
00118> # File comment: [Parameters for City of Ottawa Projects]
00119> # The following parameters are used in the design STANDHY COM
00120> # DESIGN NASHYD 1.0 01eCCR .947 No_date 2:41 13.28 .228 .000
00121> # [CN= 61.0: N= 3.00: Tp= 1.29]
00122> # STORMS
00123> # INRSH= 0
00124> ** END OF RUN : 49
00125> ****
00126> ****
00127> ****
00128> ****
00129> ****
00130> ****
00131> **** RUN#:COMMAND#
00132> RO0252:CO0001
00133> ****
00134> START
00135> [TZERO= .00 hrs on 0]
00136> [INOUT= 2.00 hrs on 0] {Imperial, 2-metric output}
00137> [INSTORM= 1]
00138> [INRSH= 0]
00139> ****
00140> # SWMHYMO Ver5.02/Jan 2001 <BETA> / INPUT DATA FILE
00141> # Project Name : [Cardinal Creek Village]
00142> # Project Number : [959-11]
00143> # Date : 2021/07/07
00144> # Modeler : Laura Pipkins, P.Eng.
00145> # Company : J.F. Sabourin and Associates
00146> # License # : 2582634
00147> # FileID : RO0252:CO0001
00148> # File comment: [Parameters for City of Ottawa Projects]
00149> # The following parameters are used in the design STANDHY COM
00150> # Horton's infiltration equation parameters:
00151> # Filename = storm.001
00152> # Comment = CHICAGO STORM 50 Year, 3 Hours
00153> # [IN=13.20 mm hr] [DCAV= 4.14 /hr] [Fw= .00 mm]
00154> RO0252:CO0003:
00155> **** DEFAULT VALUE****
00156> # Existing Drainage from Subject Site to Ottawa River
00157> ICASEdW = 1 (read and print data)
00158> # File comment: [Parameters for City of Ottawa Projects]
00159> # The following parameters are used in the design STANDHY COM
00160> # Horton's infiltration equation parameters:
00161> [Fw= 76.20 mm/hr] [Fc=13.20 mm/hr] [DCAV= 4.14 /hr] [Fw= .00 mm]
00162> Parameters for IMPERVIOUS surfaces in STANDHY:
00163> [IApere= 4.67 mm] [LGP=40.00 m] [MNPF=.250]
00164> Parameters for IMPERVIOUS surfaces in STANDHY:
00165> [IApere= 4.67 mm] [LGP=40.00 m] [MNPF=.250]
00166> Parameters used in NASHYD:
00167> [IA= 4.67 mm] [MN= 3.00]
00168> Average monthly Fan Evaporation data in (mm)
00169> JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00170> .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00171> Average monthly Potential Evapotranspiration in (mm)
00172> JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00173> .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00174> # CN -> CN based on Ontario Soil Map 58, Nov 1985 MTO Manual Chart H2-6A,
00175> # Lidar data, May 2000 SWMHYMO USER's Manual, air photos, assume good condition
00176> # Time to Peak = 2 /3 of FAA Tc
00177> ****
00178> **** EXISTING CONDITIONS Drainage to South Tributary East of Cox County Road
00179> ****
00180> # Existing Drainage from Subject Site to Ottawa River
00181> # File comment: [Parameters for City of Ottawa Projects]
00182> # The following parameters are used in the design STANDHY COM
00183> # DESIGN NASHYD 1.0 01eCCR .947 No_date 2:40 16.25 .231 .000
00184> # [CN= 61.0: N= 3.00: Tp= 1.29]
00185> # STORMS
00186> ** END OF RUN : 98
00187> ****

```

```

00379: ****END OF RUN : 198
00380: ** END OF RUN : 198
00381: ****
00382: ****
00383: ****
00384: ****
00385: ****
00386: ****
00387: ****
00388: RUN#:COMMAND#
00389: R0199:00001-----
00390: ****STORM
00391: [TIZERO = 0.00 hrs on 01
00392: [METOUT= 2 (Imperial, 2-metric output)
00393: [INRND= 1]
00394: [INRNU = 0199]
00395: ****
00396: SWMHYMO\2021\Jan 2021\SMHYS INPUT DATA FILE
00397: ****
00398: # Project Name : [Cardinal Creek Village]
00399: # Project Number : [ ]
00400: # Date : 2021/07/07
00401: # Modeler : Laura Pipkins, P.Eng.
00402: # Company : [Laurie Pipkins and Associates
00403: # License # : 2582634
00404: ****
00405: R0199:00002-----
00406: READ STORM
00407: Filename = storm.001
00408: Case = Storm 24 hours SCS Type 2 Storm 24 Hours step 10 min, City of Ottawa
00409: [SDT=10.00:SOUR= 24.00:PTOT= 106.73]
00410: R0199:00003-----
00411: ****
00412: Parameters used in NSHYD:
00413: ICAEDev = 1 (read and print data)
00414: FileTitle= [Design Parameters for City of Ottawa Projects]
00415: THE FOLLOWING PARAMETERS ARE USED IN THE DESIGN STANDRDY COM
00416: Headings, infiltration equation parameters:
00417: [Pc= 76.20 mm] [Pd= 10.00 mm] [Pf= 4.14 / hr] [Pw= .00 mm]
00418: Parameters for PERVIOUS surfaces in STANDRDY:
00419: [TAper= 4.67 mm] [LGf=40.00 m] [RMF= .250]
00420: Parameters used in NSHYD:
00421: [AIimp= 1.57 mm] [CLv= 1.50] [BMT= .013]
00422: Parameters used in NSHYD:
00423: [L= 4.90 m]
00424: Average monthly Pan Evaporation data in (mm)
00425: JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00426: .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00427: Average monthly Potential Evapotranspiration in (mm)
00428: JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
00429: .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
00430: # CN -> CN based on Ontario Soil Map 58, Nov 1985 MTO Manual Chart H-6A,
00431: Lidar data, May 2008 SWMHYMO USER's Manual, air photos, assume good condition
00432: #
00433: # Time to Peak = 2/3 of FAA To
00434: ****
00435: # EXISTING CONDITIONS Drainage to South Tributary East of Cox County Road
00436: ****
00437: # Existing Conditions drainage from Dmin-ID:NSHYD--- to Ottawa River
00438: R0199:00004-----ARBAa-QPEAKmas-TpeakDate_h:mm:--RVM=R.C.---DWFMs
00439: DESIGN NSHYD
1.0 01eCCR 74.30 1.904 No_date 13:21 39.39 .369 .000
00440: ****
00441: ****
00442: # STORMS
00443: ****
00444: R0199:00002-----
00445: FINISH
00446: ****
00447: ****
00448: # WARNINGS / ERRORS / NOTES
00449: ****
00450: Simulation ended on 2021-07-19 at 10:39:03
00451: ****
00452: ****

```