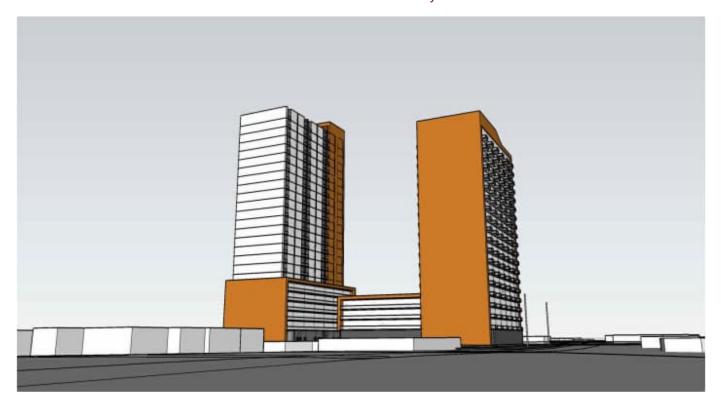
SERVICING & STORMWATER MANAGEMENT REPORT 1531 ST. LAURENT BOULEVARD, OTTAWA



Project No.: CCO-23-4499

City File No.: PC2022-0271

Prepared for:

1531 St-Laurent Ltd. 69 Rue Jean-Proulx #301 Gatineau, QC J8Z 1W2

Prepared by:

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May 31, 2023

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1.0 PROJECT DESCRIPTION

1.1 Purpose

McIntosh Perry (MP) has been retained by 1531 St-Laurent Limited Partnership. to prepare this Servicing and Stormwater Management Report in support of the Site Plan Control application for the proposed 25 and 20-storey residential buildings, located at 1531 St. Laurent Boulevard within the City of Ottawa.

The main purpose of this report is to present a servicing design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary and storm sewer servicing for the development, ensuring that existing and available services will adequately service the proposed development.

This report should be read in conjunction with the following drawings:

- COO-23-4499, C101 Ste Grading and Drainage Plan,
- COO-23-4499, C102 Site Servicing Plan,
- CCO-23-4499, PRE Pre-Development Drainage Area Plan (Appendix E), and
- CCO-23-4499, POST Post-Development Drainage Area Plan (Appendix F)

1.2 Ste Description

The property is located at 1531 St. Laurent Boulevard. It is described as Part of Lots 40, 41, 53 & 54, Registered Plan 63, Ward 18 Alta Vista, City of Ottawa. The land in question covers approximately 0.50 ha and is bounded by St. Laurent Boulevard, Belfast Road and Lagan Way. The development area for the proposed works is approximately 0.50 ha which includes 0.05 ha of parkland dedication fronting Belfast Road. The site is zoned as an Arterial Maintstreet Zone (AM). See Site Location Plan in Appendix A for more details.



Figure 1: Site Map

1.3 Proposed Development and Statistics

The proposed development consists of two high rise residential towers. Visitor parking and drive aisles will be provided through an internal courtyard. Underground parking will be provided for residents and visitors with site accesses extending from Belfast Road and Lagan Way. Further details are provided in the Ste Plan provided by Figurr Architects Collective. Refer to Appendix B.

1.4 Existing Conditions and Infrastructure

The existing site is currently developed as a restaurant complete with parking accessed from Belfast Road. It is assumed that sanitary and water servicing for the existing development is provided via services extending from the existing sewers within Belfast Road. Storm servicing for the site is provided by a series of private catch basins within the existing parking lot. It is assumed that the storm network conveys runoff to the storm sewer within Belfast Road.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal rights-of-way(s):

St. Laurent Boulevard

- o 406mm diameter cast iron watermain, and a
- o 1200mm diameter concrete storm sewer, tributary to the South Cyrville Drain with approximately 0.70 Km to the outlet.

Belfast Road

- 305mm diameter cast iron watermain,
- 250mm diameter PVC sanitary sewer, tributary to the Innes Road Collector, and a
- o 1200mm diameter concrete storm sewer, tributary to the South Cyrville Drain with approximately 0.70 Km to the outlet.

Lagan Way

- o 203mm diameter PVC watermain,
- o 250mm diameter PVC sanitary sewer, tributary to the Innes Road Collector, and a
- o 750mm diameter concrete storm sewer, tributary to the South Cyrville Drain with approximately 0.70 Km to the outlet.

1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (ECA) through the Ministry of Environment, Conservation and Parks (MEOP) is not anticipated to be required for the development since the development is contained within a single parcel of land, is not within a combined sewershed, and does not propose industrial sewage. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90..

2.0 BACKROUND STUDIES, STANDARDS, AND REFRENCES

2.1 Background Reports or Referenced Information

Background studies that have been referenced or completed for the proposed site include City of Ottawa as-built drawings and a topographical survey.

As-built drawings of existing services within the vicinity of the proposed site were reviewed in order to determine accurate servicing and stormwater management schemes for the site.

A topographic survey of the site (File No. 432-22) was completed by Farley, Smith & Denis Surveying LTD., dated August 25, 2022.

2.2 Applicable Guidelines and Standards

Oity of Ottawa:

- ♦ Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (Ottawa Sewer Guidelines)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (ISTB-2014-01)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (PIEDTB-2016-01)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, January 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (ISTB-2018-03)
 - Technical Bulletin ISTB-2019-01 City of Ottawa, January 2019. (ISTB-2019-01)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (ISTB-2019-02)
- Ottawa Design Guidelines Water Distribution Otty of Ottawa, July 2010. (Ottawa Water Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 2018. (ISTB-2018-02)

Ministry of Environment, Conservation and Parks:

- ◆ Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (MECP Stormwater Design Manual)
- ◆ Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (MECP Sewer Design Guidelines)

Other:

♦ Water Supply for Public Fire Protection, Fire Underwriters Survey, 2020. (FUS Guidelines)

3.0 PRE-CONSULTATION SUMMARY

A pre-consultation meeting was conducted on October 26, 2022 regarding the proposed site. Specific design parameters to be incorporated within this design include the following:

- Control post-development flows to the 2-year pre-development storm with a maximum combined C value of 0.50, and calculated time of concentration.
- Hows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.
- No surface ponding within parking areas during the 2-year event.
- Quality Control to be determined by the RVCA.

The notes from the City of Ottawa can be found in Appendix B.

4.0 WATERMAIN

4.1 Existing Watermain

The site is located within the 1E pressure zone, as per the Water Distribution System mapping included in Appendix C. There is an existing 305mm diameter Cl watermain within Belfast Raod. The watermain also provides servicing to the municipal fire hydrant along the south side of Belfast Road

4.2 Proposed Watermain

Two new 150mm diameter PVC water services are proposed to service the development complete with water valves located at the property line. The water services are proposed to be serviced by the existing 305mm diameter watermain within Belfast Road. The services are designed to have a minimum of 2.4m cover. Refer to drawing C102 for a detailed servicing layout.

The Fire Underwriters Survey 2020 (FUS) method was utilized to determine the required fire flow for the site. The 'C' factor (type of construction) for the FUS calculation was determined to be 0.8 (non-combustible type). The total effective floor area ('A' value) for the FUS calculation was determined to be 6,600.0 m². The results of the calculations yielded a required fire flow of 7,000 L/min. The detailed calculations for the FUS can be found in Appendix C.

The water demands for the proposed building have been calculated to adhere to the Ottawa Design Guidelines – Water Distribution manual and can be found in Appendix C. The results have been summarized in Table 1, below. In accordance with Section 4.3.1 of the guidelines, service areas with a basic day demand greater than $50 \, \text{m}^3/\text{day}$ require a dual connection to the municipal system. The basic day demand for the development is estimated to be $199 \, \text{m}^3/\text{day}$, therefore a dual connection is required.

Table 1: Water Supply Design Criteria and Water Demands

Ste Area	0.50ha
Residential	280 L/person/day
1 Bedroom Apartment	1.4 persons/unit
2 Bedroom Apartment	2.1 persons/unit
3 Bedroom Apartment	3.1 persons/unit
Bachelor Apartment	1.4 persons/unit
Maximum Daily Peaking Factor	2.5 x avg day
Maximum Hour Peaking Factor	2.2 x max day
Average Day Demand (L/s)	2.31
Maximum Daily Demand (L/s)	5.75
Peak Hourly Demand (L/s)	12.63
FUS Fire How Requirement (L/s)	116.66 (7,000 L/min)

Boundary Conditions have been requested from the City however were not available at the time of submission. Once boundary conditions are provided by the City, operating pressures at the proposed connections will be analysed to confirm they adhere to the Ottawa Water Guidelines.

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were analysed per City of Ottawa ISTB 2018-02 Appendix I Table 1. Based on City guidelines (ISTB-2018-02), the existing hydrants can provide adequate fire protection to the proposed development. The results are summarized in Table 2, below.

Table 2: Fire Protection Confirmation

Building	Fire Flow Demand (L/ min.)	Fire Hydrant(s) within 75m	Fire Hydrant (s) within 150m	Combined Fire Flow (L/ min.)	
1531 St. Laurent	7,000 (FUS)	2	4	26,600	
Boulevard					

^{*} Fire hydrants within 75 metres contribute 5,700 L/min to fire flow and fire hydrants within 150 meters contribute 3,800 L/min to fire flow, respectively, per ISTB-2018-02.

5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

There is an existing 250mm diameter PVC sanitary sewer within Belfast Road.

5.2 Proposed Sanitary Sewer

A new 200 mm diameter gravity sanitary service is proposed be connected to the existing 250mm diameter sanitary sewer within Belfast Poad. A maintenance hole (MH1A) is proposed at the property to provide monitoring per the Ottawa Sewer Design Guidelines and City of Ottawa Sewer-Use By-Law 2003-514 (14).

The peak design flows for the proposed building were calculated using criteria from the Ottawa Sewer Guidelines and are summarized in Table 3, below. Based on the unit occupancy statistics provided by the architect, the proposed site development will generate a flow of 7.82 L/s. See Appendix D of this report for more details.

Table 3: Sanitary Design Criteria

Design Parameter	Value
Ste Area	0.50 ha
Residential	280 L/ person/ day
Commercial/Amenity	2,800 L/ (m ² /day)
1 Bedroom Apartment	1.4 persons/unit
2 Bedroom Apartment	2.1 persons/unit
3 Bedroom Apartment	3.1 persons/unit
Bachelor Apartment	1.4 persons/unit
Residential Peaking Factor	3.31
Extraneous Row Allowance	0.33 L/s/ha

Table 4, below, summarizes the estimated wastewater flow from the proposed development. Refer to Appendix D for detailed calculations.

Table 4: Summary of Estimated Sanitary Flow

Design Parameter	Total How (L/s)
Total Estimated Average Dry Weather Flow	2.39
Total Estimated Peak Dry Weather Flow	7.68
Total Estimated Peak Wet Weather Flow	7.82

The proposed 200 mm diameter gravity sanitary service will be installed with a minimum full flow target velocity (cleansing velocity) of 0.6 m/s and a full flow velocity of not more than 3.0 m/s. The capacity of the service lateral is 33.74 L/s at a proposed slope of 1.00%. Due to the complexity of the downstream network, the City will need to advise of any downstream constraints. See Appendix D of this report for more details.

6.0 STORM SEWER DESIGN

6.1 Existing Storm Sewers

Water runoff from the site is currently tributary to the South Cyrville Drain within the Ottawa River East Subwatershed. The subject property is currently serviced by a series of private catch basins within the parking lot area surrounding the existing building/ There is an existing 1200mm diameter storm sewer within Belfast Road that is available to service the site.

6.2 Proposed Storm Sewers

A new 300mm PVC storm service will be extended from the existing 1200mm diameter storm sewer within Belfast Road. The sewer system will provide attenuation for the roof area and drive aisle/entrance area by an internal cistern complete with a Tempest HMF ICD or an approved equivalent. A cistern detail is to be provided by the Mechanical Engineer under separate cover.

Foundation drainage is proposed to be conveyed without flow attenuation via the 300mm diameter storm service downstream of any cistern controls.

0.05 ha of the site is proposed to be conveyed to the City as parkland. A new catchbasin is proposed to provide drainage for the parkland site via a 250mm storm lead extending to the existing 1200mm diameter storm sewer within Belfast Road.

See CCO-23-4499 - POST include in Appendix F of this report for more details. The Stormwater Management design for the subject property will be outlined in Section 7.0 of this report.

7.0 PROPOSED STORM WATER MANAGEMENT

7.1 Design Criteria and Methodology

Stormwater management for the proposed site will be maintained through the use of an internal cistern and will collect runoff from the at-grade areas within the site. The flow will be directed to the existing 1200mm diameter storm sewer within Belfast Road.

The following design criteria have been employed in developing the stormwater management design for the site as coordinated with the City and the RVCA. Please note the following methodology is based upon further coordination with the City and supersedes the requirements outlined in the pre-consultation notes.

Quality Control

 Based on the distance to the downstream storm outlet being less than 2 Km, it has been assumed that enhanced level quality control measures are required (80% TSS removal).

Quantity Control

- Pre-development time of concentration (TC) shall be calculated and be no less than 10 minutes.
- Control site post-development flows to the 2-year pre-development flows with a combined C value of no greater than 0.50.

7.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

Q = 2.78 CIA (L/s)

Where C = Runoff coefficient

= Rainfall intensity in mm/hr (City of Ottawa IDF curves)

A = Drainage area in hectares

It is recognized that the Pational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended.

The following coefficients were used to develop an average Cfor each area:

Roofs/ Concrete/ Asphalt	0.90
Gravel	0.60
Undeveloped and Grass	0.20

As per the City of Ottawa - Sewer Design Guidelines, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

As per coordination with the City of Ottawa the time of concentration (Tc) used for predevelopment shall be calculated but not less than a Tc of 10 minutes and post-development flows shall be calculated using a Tc of 10 minutes. The calculated Tc in pre-development conditions is less than 10 minutes, therefore, a Tc of 10 minutes was used in the pre-development runoff calculations. Refer to Appendix G.

7.3 Pre-Development Drainage

It has been assumed that the site contains no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 5, and 100-year events are summarized below in Table 5. See COO-23-4499 - PRE in Appendix E and Appendix G for calculations.

Table 5: Pre-Development Runoff Summary

Q Area

Drainage (L/s)Area (ha) 2-Year 5-Year 100-Year Α1 0.45*84.89 115.16 219.36 Total 0.45 84.89 115.16 219.36

7.4 Post-Development Drainage

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See COO-23-4499 - POST in Appendix Fof this report for more details. Based on the quantity control criteria discussed in Section 7.1 and a site area of 0.45 ha, post development drainage from the site is to be limited to a maximum release rate of 47.66 L/S. A summary of the Post-Development Runoff Calculations can be found below.

^{*} Total Ste Area Less Parkland Dedication

0.45

51.6

100-year Storage 100-year Storage Drainage 5-year Peak 100-year Peak Area (ha) Area How (L/s) How (L/s) Required (m³) Available (m³) 27.34 14.35 51.6 B1 0.37 118.3 B2 0.07 10.37 20.32

47.66

118.3

24.71

Table 6: Post-Development Runoff Summary

Runoff for area B1 will be collected by roof drains (uncontrolled) and surface drains and conveyed to the internal cistern. The 118.3 m³ internal cistern is anticipated to convey stormwater to the outlet at a maximum flow rate of 14.35 L/s and 27.34 L/s for the 5 and 100-year storms, respectively. Hows in excess of the 100-year storm event will need to be directed Belfast Road via a cistern overflow. The cistern details are to be provided by the Mechanical Engineer, however, it is anticipated that the cistern will equipped with Tempest HMF ICD for attenuation.

Foundation drainage is proposed to be conveyed without flow attenuation via the 300 mm storm service, downstream of cistern controls.

7.5 Quality Controls

Total

A Hydro International DD4 OGS unit (or approved equivalent) will provide quality control for the site. Runoff from area B1 will be conveyed from the cistern to the OGS unit which will provide 80% TSS removal prior to discharging to the municipal storm sewer. It is anticipated that the OGS will act as a monitoring port for storm water leaving the site. Pefer to Appendix G.

8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catchbasins and filter fabric is to be placed under the grates of all existing catchbasins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures is to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the Ste Grading, Drainage and Sediment & Erosion Control Plan for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

Rip-rap will be placed at all locations that have the potential for concentrated flow. It is crucial that the Contractor ensure that the geotextile is keyed in properly to ensure runoff does not undermine the rip rapped area. Additional rip rap is to be placed at erosion prone locations as identified by the Contractor / Contract Administrator / City or Conservation Authority.

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the

site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

9.0 SUMMARY

- Two residential towers are proposed to be constructed at 1531 St. Laurent Boulevard.
- Two 150mm diameter water services are proposed to be connected to the existing 305mm diameter within Belfast Road.
- A new 200 mm diameter sanitary service is proposed, complete with a monitoring maintenance hole at the property line, to service the development via the 250mm diameter sanitary sewer within Belfast Road tributary to the Innes Road Collector.
- A new 300mm storm service for rooftop, surface, and foundation drainage is proposed to service the development. The storm service will connect to the 1200mm diameter storm sewer within Belfast Poad, tributary to the South Cyrville Drain approximately 0.70 Km downstream.
- Storage for the 5- through 100-year storm events will be provided through internal cistern attenuation.
- Quality control will be provided for the development via an OGS unit.

10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management Report in support of the proposed development at 1531 St. Laurent Boulevard

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



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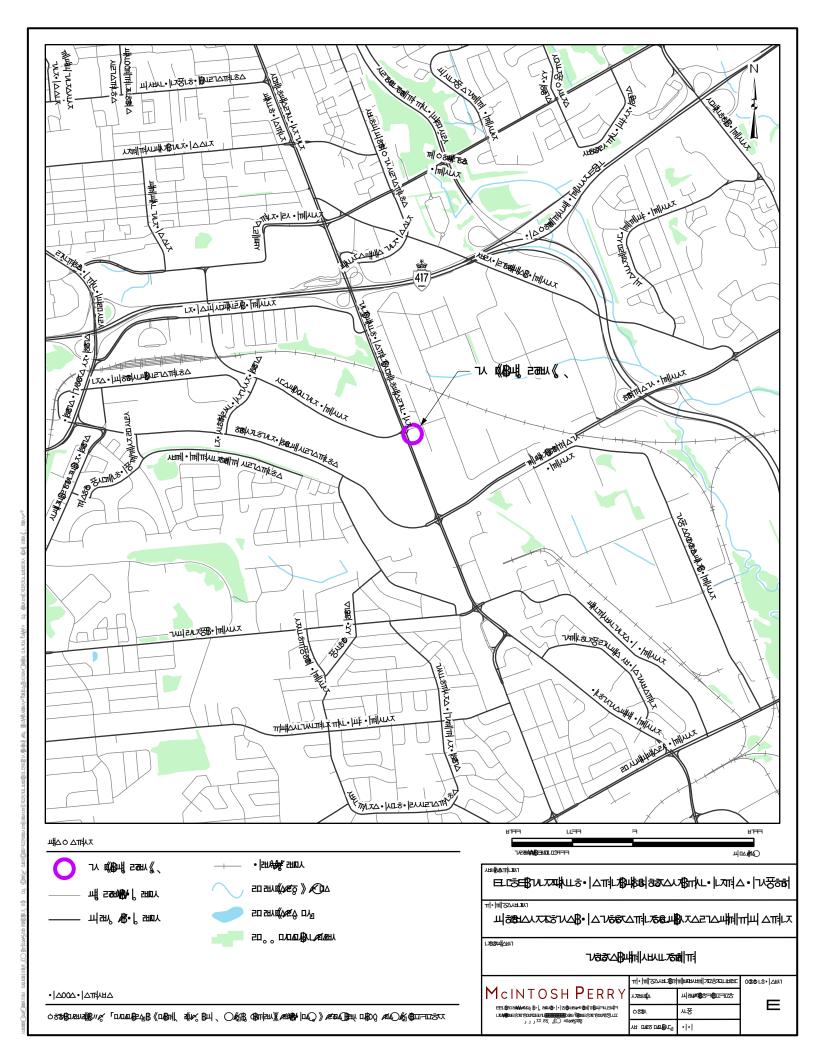
11.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of 1531 St. Laurent Limited Partnership. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Conservation and Parks, City of Ottawa and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.

Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

APPENDIX A KEY PLAN



APPENDIX B BACKGROUND DOCUMENTS

1531 St. Laurent Blvd

Meeting Summary Notes October 26, 2022. Online Teams Meeting

Attendees:

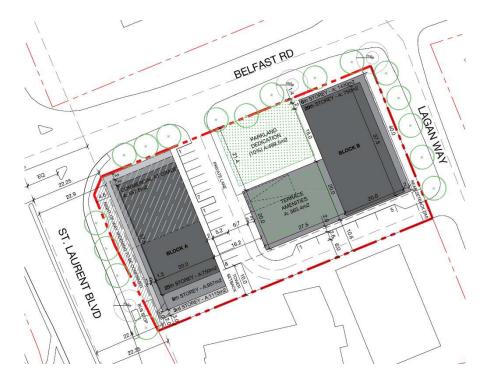
- Jillian Simpson
- Paul Black
- Chaxu Baria
- Tanya Chowieri
- Katie Morphet (File Lead, Panner, City of Ottawa)
- Bruce Bramah (Project Manager, City of Ottawa)
- Mark Richardson (Forestry, City of Ottawa)
- Hayley Murray (Forestry, City of Ottawa)
- Randolph Wang (Urban Design, City of Ottawa)
- Steve Gauthier (Parks, City of Ottawa)

Not in Attendance:

- Matthew Hayley (Environmental Planner, City of Ottawa)
- Mike Giampa (Senior Transportation Project Manager, City of Ottawa)

Issue of Discussion:

 Site Plan Control for two towers (25 storeys and 20 storeys) containing approximately 421 units with 474m2 of commercial at-grade in Building A. 561 parking spaces provided (underground + surface), amenity space, parkland dedication @10%



1. Infrastructure/Servicing - Bruce Bramah

See Attached

2. Initial Planning Comments - Katie Morphet

- Please include zoning table on site plan to identify all required zone and applicable general provisions and that they are being met.
- A Survey Plan will be required to clarify property boundaries and lot ownership.
- A Planning brief outlines how the proposed plan meets the Zoning By-law and Official Plan policies will be required.

3. Urban Design (Randolph Wang)

- 1. A Design Brief is required. The Terms of Reference of the Design Brief is attached for convenience.
 - a. Please study alternative site plan and massing, and compare the pros and cons of these options.
 - b. Please note both a wind study and a shadow study is required.
- 2. The site is within a Design Priority Area. Formal review by the City's Urban Design Review Panel is required. Please contact udrp@ottawa.ca or visit the City's UDRP website for scheduling details and submission requirements.
- 3. Please retain services of an architect and a landscape architect. Such services are crucial in the investigation of design alternatives and preparation of appropriate design materials.

- 4. With respect to the materials presented at the preconsultation meeting, urban design appreciates the images that show the potential "highest and best uses" in the vicinities but caution the limitations of these images. A more thorough and comprehensive study is required with respect to not only the potential "highest and best uses" of lands, but also the framework and elements of the new community.
- 5. The conceptual site plan shows the potential density that can be contemplated when the general rules around the high-rise development are followed. There were discussions about the location of the proposed park, which is one of the key factors that will influence the site plan. As indicated above, alternative site plan option and massing options should be explored. When exploring the alternative options, please consider the following (in addition to the location of the municipal park):
 - a. Relationship with potential future development on the property to the south of the subject property.
 - b. Function and characteristics of Lagan Way.
 - c. Cross section design of St Laurent and Belfast.
 - d. Functional requirements for viable commercial and their relationship with residential uses.
 - e. Impacts on the solar panels on the building across from St. Laurent.
 - f. How will this development stimulates community building.

4. Parks - Steve Gauthier

The proposed park location is not acceptable. The park should be located at the corner of Belfast Rd and Lagan Way, or to the south-east of the property for potential consolidation with future park development to the south.

5. Trees – Hayley Murray

Project Comments:

- All City owned trees must be retained and mitigation measures, if work is to occur in the CRZ of these trees, must be detailed in the TCR.
 - If a City owned tree is proposed for removal it must be justified and the applicant can expect to pay monetary compensation.
- Planting large canopy trees in the parkland area would be ideal.

TCR requirements:

- 1. A Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City
 - a. an approved TCR is a requirement of Site Plan approval.
 - The TCR may be combined with the LP provided all information is supplied

- 2. Any removal of privately-owned trees 10cm or larger in diameter, or city-owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 340); the permit will be based on an approved TCR and made available at or near plan approval.
- 3. The Planning Forester from Planning and Growth Management as well as foresters from Forestry Services will review the submitted TCR
 - a. If tree removal is required, both municipal and privately-owned trees will be addressed in a single permit issued through the Planning Forester
 - b. Compensation may be required for city owned trees if so, it will need to be paid prior to the release of the tree permit
- 4. The TCR must contain 2 separate plans:
 - a. Plan/Map 1 show existing conditions with tree cover information
 - b. Plan/Map 2 show proposed development with tree cover information
 - c. Please ensure retained trees are shown on the landscape plan
- 5. The TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition
 - a. please identify trees by ownership private onsite, private on adjoining site, city owned, boundary (trees on a property line)
- 6. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained
- 7. All retained trees must be shown, and all retained trees within the area impacted by the development process must be protected as per City guidelines available at Tree Protection Specification or by searching Ottawa.ca
- 8. The location of tree protection fencing must be shown on the plan
- 9. The City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.
- 10. For more information on the process or help with tree retention options, contact Hayley Murray hayley.murray@ottawa.ca or on City of Ottawa

LP tree planting requirements:

For additional information on the following please contact tracy.smith@Ottawa.ca

Minimum Setbacks

- Maintain 1.5m from sidewalk or MUP/cycle track or water service laterals.
- Maintain 2.5m from curb
- Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
- Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing, except where otherwise approved in naturalization / afforestation areas. Adhere to Ottawa Hydro's planting guidelines (species and setbacks) when planting around overhead primary conductors.

Tree specifications

- Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
- Maximize the use of large deciduous species wherever possible to maximize future canopy coverage
- Tree planting on city property shall be in accordance with the City of Ottawa's Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- Plant native trees whenever possible
- No root barriers, dead-man anchor systems, or planters are permitted.
- No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree)

Hard surface planting

- · Curb style planter is highly recommended
- No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.
- Trees are to be planted at grade

Soil Volume

• Please document on the LP that adequate soil volumes can be met:

Tree Type/Size	Single Tree Soil Volume (m3)	Multiple Tree Soil Volume (m3/tree)		
Ornamental	15	9		
Columnar	15	9		
Small	20	12		
Medium	25	15		
Large	30	18		
Conifer	25	15		

Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

• Please follow the City's 2017 Tree Planting in Sensitive Marine Clay guidelines

Tree Canopy

- The landscape plan shall show how the proposed tree planting will replace and increase canopy cover on the site over time, to support the City's 40% urban forest canopy cover target.
- At a site level, efforts shall be made to provide as much canopy cover as possible, through tree planting and tree retention, with an aim of 40% canopy cover at 40 years, as appropriate.
- Indicate on the plan the projected future canopy cover at 40 years for the site.

6. Environment – Matthew Hayley

Urban Heat Island Effect

Please add features that reduce the urban heat island effect (see OP 10.3.3) produced by the parking lot and a building footprint. For example, this impact can be reduced by adding large canopy trees, green roofs or vegetation walls, or constructing the parking lot or building differently.

Bird-safe Development

Please review and incorporate bird safe design elements. Some of the risk factors include glass and related design traps such as corner glass and fly-through conditions, ventilation grates and open pipes, landscaping, light pollution. More guidance and solutions are available in the guidelines which can be found here: https://documents.ottawa.ca/sites/documents/files/birdsafedesign_guidelines_en.pdf

7. South Nation Conservation

The downstream storm water outlet is just over 700 metres from the site without any downstream stormwater management facility. Therefore, on-site water quality of 'enhanced' (80% TSS Removal) would be required. The applicant is strongly encouraged to incorporate LIDs into the stormwater management strategy.

8. Transportation – Mike Giampa

A TIA is warranted- proceed to scoping.

The application will not be deemed complete until the submission of the draft step 2-4, including the functional draft RMA package (if applicable) and/or monitoring report (if applicable).

Although a full review of the TIA Strategy report (Step 4) is not required prior to an application, it is strongly recommended. Synchro files are required at Step 4.

ROW protection on St Laurent Boulevard is 44.5 metres.

A Noise Impact Study is required

Due to the northbound right-turn channel, relocating the Belfast access away from the intersection is recommended.

On site plan:

Show all details of the roads abutting the site up to and including the opposite curb; include such items as pavement markings, accesses and/or sidewalks.

Turning templates will be required for all accesses showing the largest vehicle to access the site; required for internal movements and at all access (entering and exiting and going in both directions).

Show all curb radii measurements; ensure that all curb radii are reduced as much as possible

9. General Information

a. Ensure that all plans and studies are prepared as per City guidelines – as available online...

https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans

PLEASE NOTE: Due to implementation of Bill 109 should the application associated with this pre-consultation meeting be filled with the City and deemed adequate on or after January 1st, 2023 a new pre-consultation process may need to be undertaken.



MEMO

Date:

To / Katie Morphet, Planner Destinataire Bruce Bramah, Project Manager, Infrastructure From / Approvals Expéditeur **Pre-Application Consultation** 1531 St Laurent. File No. PC2022-0271 Two towers containing approximately 421 units Subject / with 474 m2 of commercial at grade in building. Objet 561 underground and surface parking spaces provided.

Please note the following information regarding the engineering design submission for the above noted site:

- 1. The Servicing Study Guidelines for Development Applications are available at the following address: https://ottawa.ca/en/planning-development-and-construction/developing-property/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#servicing-study-guidelines-development-applications
- 2. Servicing and site works shall be in accordance with the following documents:
 - ⇒ Ottawa Sewer Design Guidelines (October 2012)
 - ⇒ Ottawa Design Guidelines Water Distribution (2010)
 - ⇒ Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)

 - ⇒ City of Ottawa Environmental Noise Control Guidelines (January, 2016)
 - ⇒ City of Ottawa Park and Pathway Development Manual (2012)
 - ⇒ City of Ottawa Accessibility Design Standards (2012)



- ⇒ Ottawa Standard Tender Documents (latest version)
- ⇒ Ontario Provincial Standards for Roads & Public Works (2013)
- 3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at lnformationCentre@ottawa.ca or by phone at (613) 580-2424 x.44455).
- 4. The Stormwater Management Criteria, for the subject site, is to be based on the following:
 - The 2-yr storm event using the IDF information derived from the Meteorological Services of Canada rainfall data, taken from the MacDonald Cartier Airport, collected 1966 to 1997.
 - ii. The pre-development runoff coefficient or a maximum equivalent 'C' of 0.5, whichever is less (§ 8.3.7.3).
 - iii. A calculated time of concentration (Cannot be less than 10 minutes).
 - iv. Flows to the storm sewer in excess of the 2-year storm release rate, up to and including the 100-year storm event, must be detained on site.
 - v. Quality control to be determined by the RVCA.
 - vi. Servicing preferred from Lagan Way
 - vii. No surface ponding within parking areas during the 2-year event.

Note: There may be area specific SWM Criteria that may apply. Check for any related SWM &/or Sub-watershed studies that may have been completed.

- 5. Deep Services (Storm, Sanitary & Water Supply)
 - i. Provide existing servicing information and the recommended location for the proposed connections. Services should ideally be grouped in a common trench to minimize the number of road cuts.
 - ii. Connections to trunk sewers and easement sewers are typically not permitted.



- iii. Provide information on the monitoring manhole requirements should be located in an accessible location on private property near the property line (ie. Not in a parking area).
- iv. Review provision of a high-level sewer.
- v. Provide information on the type of connection permitted

Sewer connections to be made above the springline of the sewermain as per:

- a. Std Dwg S11.1 for flexible main sewers connections made using approved tee or wye fittings.
- b. Std Dwg S11 (For rigid main sewers) *lateral must be less that 50% the diameter of the sewermain*,
- c. Std Dwg S11.2 (for rigid main sewers using bell end insert method) for larger diameter laterals where manufactured inserts are not available; lateral must be less that 50% the diameter of the sewermain,
- d. Connections to manholes permitted when the connection is to rigid main sewers where the lateral exceeds 50% the diameter of the sewermain. – Connect obvert to obvert with the outlet pipe unless pipes are a similar size.
- e. No submerged outlet connections.
- 6. Water Boundary condition requests must include the location of the service and the expected loads required by the proposed development. Please provide the following information:
 - i. Location of service
 ii. Type of development and the amount of fire flow required (as per FUS, 1999).
 iii. Average daily demand: ____ l/s.
 - v. Maximum hourly daily demand: I/s.

Maximum daily demand: I/s.

iv.



7. Phase 1 ESAs and Phase 2 ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Should you have any questions or require additional information, please contact me directly at (613) 580-2424, ext. 29686 or by email at Bruce.Bramah@ottawa.ca.

- SITE PLAN APPLICATION - Municipal servicing

Legend:

The letter **S** indicates that the study or plan <u>is</u> required with application submission.

The letter **M** indicates that the study or plan <u>may</u> be required with application submission.

For information on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENGINEERING			S/A	Number of copies	
s		1.	Site Servicing Plan	2.	Assessment of Adequacy of Public Services / Site Servicing Study / Brief	S	
S		3.	Grade Control and Drainage Plan	4.	Geotechnical Study / Slope Stability Study	S/M	
		5.	Composite Utility Plan	6.	Groundwater Impact Study		
		7.	Servicing Options Report	8.	Wellhead Protection Study		
		9.	Community Transportation Study and/or Transportation Impact Study / Brief	10.	Erosion and Sediment Control Plan / Brief	s	
s		11.	Storm water Management Report / Brief	12.	Hydro-geological and Terrain Analysis		
M		13.	Water main Analysis	14.	Noise / Vibration Study	М	
		15.	Roadway Modification Design Plan	16.	Confederation Line Proximity Study		



- ZONING BYLAW - Municipal servicing

Legend:

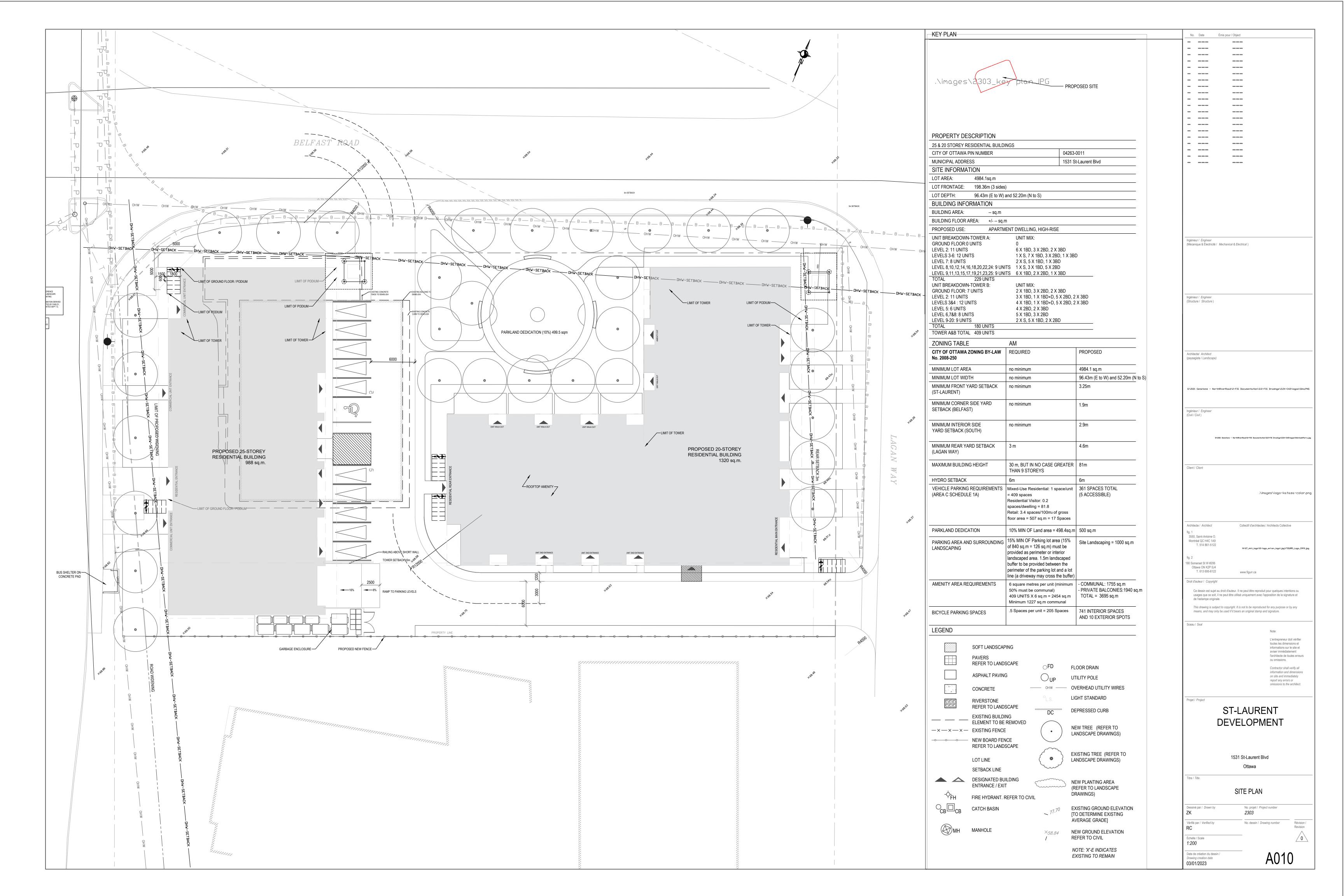
The letter **S** indicates that the study or plan <u>is</u> required with application submission.

The letter \mathbf{M} indicates that the study or plan \underline{may} be required with application submission.

For information on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENGINEERING	S/A	Number of copies
		Site Servicing Plan Site Servicing Plan Site Servicing Study / Brief	S	
		Grade Control and Drainage Plan Geotechnical Study / Slope Stability Study	S/M	
		Composite Utility Plan 6. Groundwater Impact Study		
S		7. Servicing Options Report 8. Wellhead Protection Study		
		Community Transportation Study and/or Transportation Impact Study / Brief Transportation 10. Erosion and Sediment Control Plan / Brief		
		11. Storm water Management Report / Brief 12. Hydro-geological and Terrain Analysis		
		13. Water main Analysis 14. Noise / Vibration Study		
		15. Roadway Modification Design Plan 16. Confederation Line Proximity Study		



APPENDIX C WATERWAIN CALCULATIONS

000-23-4499 - 1531 St Laurent - Water Demands

 Project:
 1531 St Laurent

 Project No.:
 COO-23-4499

 Designed By:
 RRR

 Checked By:
 RDF

Date: May 31, 2023

Ste Area: 0.50 gross ha

Residential **NUMBER OF UNITS UNIT RATE** Single Family 3.4 homes persons/unit Semi-detached 2.7 homes persons/unit Townhouse homes 2.7 persons/unit Bachelor Apartment 39 units persons/unit 1.4 208 units 1 Bedroom Apartment persons/unit 1.4 2 Bedroom Apartment 133 units 2.1 persons/unit 3 Bedroom Apartment 26 units 3.1 persons/unit 1.8 Average Apartment units persons/unit

Total Population 706 persons

 Commercial
 552 m2

 Industrial - Light
 m2

 Industrial - Heavy
 m2

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d	
Industrial - Light	35,000	L/gross ha/d	
Industrial - Heavy	55,000	L/gross ha/d	
Shopping Centres	2,500	L/ (1000m² /d	
Hospital	900	L/ (bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/(Space/d)	
Motels	150	L/ (bed-space/d)	
Hotels	225	L/ (bed-space/d)	
Tourist Commercial	28,000	L/gross ha/d	
Other Commercial	28,000	L/gross ha/d	
	Residential	2.29	L/s
AVERAGE DAILY DEMAND	Commercial/Industrial/		
	Institutional	0.02	L∕s

MAXIMUM DAILY DEMAND

DEMAND TYPE	Į.	AMOUNT	UNITS			
Residential	2.5	x avg. day	L/c/d			
Industrial	1.5	x avg. day	L/gross ha/d			
Commercial	1.5	x avg. day	L/gross ha/d			
Institutional	1.5	x avg. day	L/ gross ha/ d			
	Residential	5.72	L/s			
MAXIMUM DAILY DEMAND	Commercial/Industrial/					
	Institutional	0.03	L/s			

MAXIMUM HOUR DEMAND

DEM AND TYPE	P	UNITS			
Residential	2.2	x max. day	L/c/d		
Industrial	1.8	x max. day	L/ gross ha/ d		
Commercial	1.8	x max. day	L/ gross ha/ d		
Institutional	1.8	x max. day	L/ gross ha/ d		
	Residential	12.58	L/s		
MAXIMUM HOUR DEMAND	Commercial/Industrial/				
	Institutional	0.05	L/s		

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND		L/s
MAXIMUM DAILY DEMAND	5.75	L/s
MAXIMUM HOUR DEMAND	12.63	L/s

RRR - 1531 St Laurent - Fire Underwriters Survey - 25 Storey Building

 Project:
 1531 St Laurent

 Project No.:
 PRR

 Designed By:
 PRR

 Checked By:
 PDF

 Date:
 May 31, 2023

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Pequired Fire Flow Copyright I.SO.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Pounded to the nearest 1000 L/ min)

 $F = 220 \times C \times VA$ Where: F =Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in

the building being considered.

Construction Type Non-Combustible Construction

0

Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 4,940.0 m²

-50%

* Unprotected Vertical Openings

 Calculated Fire Flow
 12,370.2 L/min

 12,000.0 L/min
 12,000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:

Limited Combustible -15%

Fire Flow 10,200.0 L/min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Fully Supervised Sprinklered

D. INCRE	EASE FOR EXPOSURE (No Round	ding)					
	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor		
Exposure 1	Over 30 m	Ordinary - Mass Timber (Unprotected)	20	2	40.0	0%	
Exposure 2	10.1 to 20	Fire Resistive - Non Combustible (Unprotected Openings)	20	20	400.0	8%	
Exposure 3	10.1 to 20	Ordinary - Mass Timber (Unprotected)	29	1	29.0	6%	
Exposure 4	Over 30 m	Ordinary - Mass Timber (Unprotected)	20	1	20.0	0%	
					%Increase*	14%	

Increase* 1,428.0 L/mi

E Total Fire Flow (Rounded to the Nearest 1000 L/min)

 Hre How
 6,528.0 L/ min

 Fire Row Required**
 7,000.0 L/ min

 $^{^{\}star}$ In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

^{**} In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

RRR - 1531 St Laurent - Fire Underwriters Survey - 20 Storey Building

 Project:
 1531 St Laurent

 Project No.:
 PRR

 Designed By:
 PRR

 Checked By:
 PDF

 Date:
 May 31, 2023

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Pequired Fire Flow Copyright I.SO.: City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Pounded to the nearest 1000 L/ min)

 $F = 220 \times C \times VA$ Where: F = I

F = Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in

the building being considered.

Construction Type Non-Combustible Construction

)

Total Hoor Area (per the 2020 FUS Page 20 - Total Effective Area) 6,600

m² *II

%Increase*

* Unprotected Vertical Openings

 Calculated Fire Flow
 14,298.3 L/min

 14,000.0 L/min
 14,000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:

Limited Combustible -15%

Fire Flow 11,900.0 ☑ min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Fully Supervised Sprinklered

-50%

Re	eduction		-5,950.0 L/ min									
D. INCREASE FOR EXPOSURE (No Rounding)		ling)										
	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor							
Exposure 1	Over 30 m	Ordinary - Mass Timber (Unprotected)	20	2	40.0	0%						
Exposure 2	Over 30 m	Ordinary - Mass Timber (Unprotected)	20	20	400.0	0%						
Exposure 3	10.1 to 20	Ordinary - Mass Timber (Unprotected)	21	1	21.0	6%						
Exposure 4	Over 30 m	Fire Resistive - Non Combustible (Unprotected Openings)	47	25	1175.0	0%						

ncrease* 714.0 L/mi

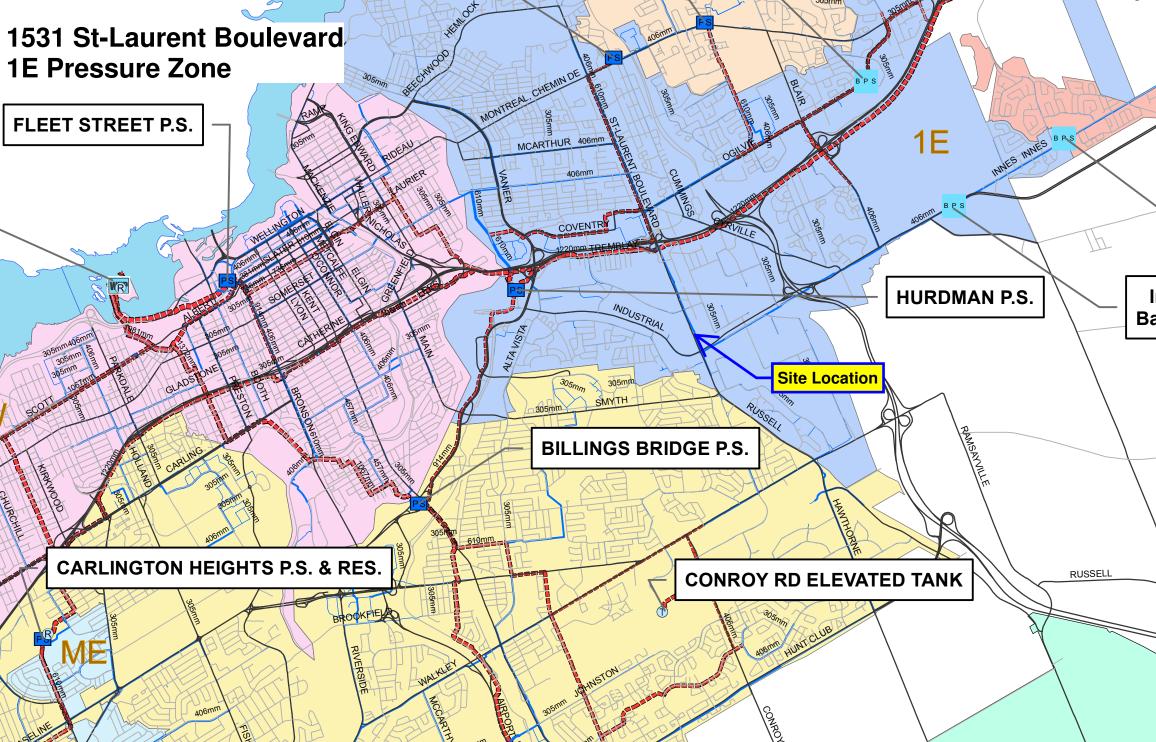
E Total Fire Flow (Rounded to the Nearest 1000 L/min)

 Hre How
 6,664.0 L/min

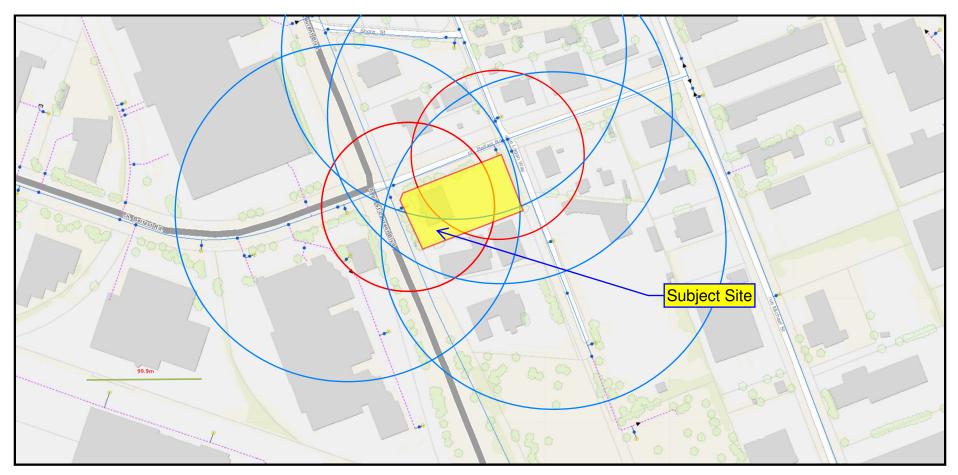
 Fire Flow Required**
 7,000.0 L/min

 $^{^{\}star}$ In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

^{**} In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min



1531 St-Laurent FUS Hydrant Coverage Figure



Hydrants Within 75m: 2

Hydrants Within 150m: 4

APPENDIX D SANITARY CALCULATIONS

000-23-4499 - 1531 St Laurent - Sanitary Demands

Project: 1531 St Laurent Project No.: 000-23-4499 Designed By: RDF Checked By: May 31, 2023 Date: 0.50 Ste Area Gross ha 39 Bachelor 1.40 Persons per unit 1 Bedroom 208 1.40 Persons per unit 2 Bedroom 133 2.10 Persons per unit 26 3 Bedroom 3.10 Persons per unit

 Total Population
 706 Persons

 Commercial Area
 552.00 m²

 Amenity Space
 1755.00 m²

DESIGN PARAMETERS

Institutional/Commercial Peaking Facto

1 *Check technical bulleting (Either use 1.0 or 1.5)

Pesidential Peaking Factor 3.31 * Using Harmon Formula = $1+(14/(4+P^{\Lambda}0.5))^{*}0.8$

where P = population in thousands, Harmon's Correction Factor = 0.8

Mannings coefficient (n) 0.013

Demand (per capita) 280 L/day Infiltration allowance 0.33 L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	How (L/s)
Dry	0.02
Wet	0.14
Total	0.16

AVERAGE DAILY DEMAND

DEM AND TYPE	AMOUNT	UNITS	POPULATION / AREA	How (L/s)
Residential	280	L/c/d	706	2.29
Industrial - Light**	35,000	L/ gross ha/ d		0
Industrial - Heavy* *	55,000	L/ gross ha/ d		0
Commercial / Amenity	2,800	L/ (1000m² /d)	2307.00	0.07
Hospital	900	L/ (bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/ (campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/ gross ha/ d		0
Other Commercial	28,000	L/ gross ha/ d		0

AVERAGE RESIDENTIAL FLOW	2.29	L/s
PEAK RESIDENTIAL FLOW	7.58	L/s
AVERAGE ICI FLOW	0.07	L/s
PEAK INSTITUTIONAL/ COMMERCIAL FLOW	0.07	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.07	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	2.39	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	7.68	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	7.82	L/s

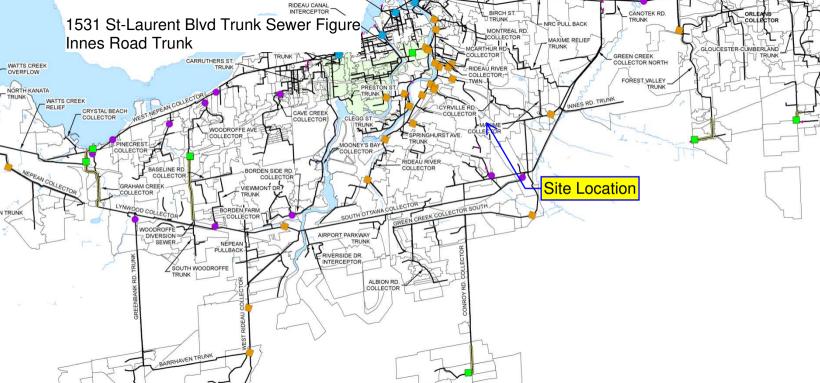
^{**} PEAK INDUSTRIAL FLOW PER CITY OF OTTAWA SEWER DESIGN GUIDELINES APPENDIX 4B

SANITARY SEWER DESIGN SHEET

PROJECT: CCO-23-4499

LOCATION: 1531 \$\frac{1}{2}\$ Laurent

	LOCATION						LOCATION RESIDENTIAL							ICI AREAS					INFILTRATION ALLOWANCE FLOW			FLOW	SEWER DATA							
1	2		3	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
					UNIT	TYPES		AREA	POPUI	LATION		PEAK			AREA	REA (ha)			PEAK	AREA	A (ha)	FLOW	DESIGN	CAPACITY	LENGTH	DIA	SLOPE	VELOCITY	AVAIL	ABLE
STREET	AREA	ID	FROM	Bac/ 1-Bed	2-Bed	3-Bed	APT	(ha)	IND	СЛМ	PEAK	FLOW	INSTITU	JTIONAL	COM M	ERCIAL	INDU	STRIAL	FLOW	IND	алм	(L/s)	FLOW	(L/s)	(m)	(mm)	(%)	(full)	CAPA	aty
			МН	Dati 1-Deti	z-beu	3-Deu	AFI	(IIa)	שאוו	COIVI	FACTOR	(L/s)	IND	CUM	IND	СИМ	IND	CUM	(L/s)	שאוו	COIVI	(11 5)	(L/s)	(12 5)	(111)	(111111)	(/0)	(m/s)	L/s	(%)
Belfast Road	BLDO	G	MH1A	247	133	26		0.50	706.0	706.0	3.31	7.58	0.00	0.00	0.23	0.23		0.00	0.07	0.50	0.50	0.16	7.82	34.22		200	1.00	1.055	26.40	77.14
	MH1.	Α	Ex. Sewer									7.58		0.00		0.23		0.00	0.07	0.00	0.50	0.16	7.82	34.22		200	1.00	1.055	26.40	77.14
Design Parameters:				Notes:							Designed:		RRR			No.					Revision							Date		
				1. Manning	s coefficien	it (n) =		0.013																						
Residential		ICI Area	S	2. Demand	(per capita)):	280	L/day																						
1-BED 1.4 p/p/u				3. Infiltration	on allowand	æ:	0.33	BL/s/Ha			Checked:		RDF																	
2-Bed 2.1 p/p/u	INST	28,000	L/Ha/day	Resident	ial Peaking	Factor:																								
3-Bed 3.1 p/p/u	COM	28,000	L/Ha/day		Harmon Fo	rmula = 1+(14/(4+P^0.5	5)* 0.8)																						
Other 60 p/p/Ha	IND	35,000	L/Ha/day		where P=p	opulation in	n thousands	3			Project No.	:	OOO-23-4499																	
																	•											Sheet No:		
																												1 of 1		



APPENDIX E PRE-DEVELOPMENT DRAINAGE PLAN



GENERAL NOTES

- 1. THE ORIGINAL TOPOGRAPHY, GROUND ELEVATION AND SURVEY DATA SHOWN ARE SUPPLIED FOR INFORMATION PURPOSES ONLY, AND IMPLY NO GUARANTEE OF ACCURACY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL INFORMATION SHOWN.
- 2. THIS PLAN IS NOT A CADASTRAL SURVEY SHOWING LEGAL PROPERTY BOUNDARIES AND EASEMENTS. THE PROPERTY BOUNDARIES SHOWN HEREON HAVE BEEN DERIVED INFORMATION SUPPLIED BY FARLEY, SMITH AND DENNIS SURVEYING LTD. (JOB NO. 432-22) AND CANNOT BE RELIED UPON TO BE ACCURATE OR COMPLETE. THE PRECISE LOCATION OF THE CURRENT PROPERTY BOUNDARIES AND EASEMENTS CAN ONLY BE DETERMINED BY AN UP-TO-DATE LAND TITLES SEARCH AND A SUBSEQUENT CADASTRAL SURVEY PERFORMED AND CERTIFIED BY AN ONTARIO LAND SURVEYOR.
- 3. THE CONTRACTOR IS TO OBTAIN AND PAY FOR ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY BEFORE COMMENCING CONSTRUCTION.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR ALL LAYOUT.
- 5. THE CONTRACTOR IS TO DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME ALL RESPONSIBILITY FOR EXISTING UTILITIES WHETHER OR NOT SHOWN ON THESE DRAWINGS. IF THERE IS ANY DISCREPANCY THE CONTRACTOR IS TO NOTIFY THE ENGINEER
- 6. RESTORE ALL TRENCHES AND SURFACES OF PUBLIC ROAD ALLOWANCES TO CONDITION EQUAL OR BETTER THAN ORIGINAL CONDITION AND TO THE SATISFACTION OF THE CITY AUTHORITIES.
- 7. EXCAVATE AND DISPOSE OF ALL EXCESS EXCAVATED MATERIAL, SUCH AS ASPHALT, CURBING AND DEBRIS, OFF SITE AS DIRECTED BY THE ENGINEER AND THE CITY.
- 8. ALL DISTURBED AREAS TO BE RESTORED TO ORIGINAL CONDITION OR BETTER UNLESS OTHERWISE SPECIFIED.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TRAFFIC CONTROL AND SAFETY MEASURES DURING THE CONSTRUCTION PERIOD, INCLUDING THE SUPPLY, INSTALLATION, AND REMOVAL OF ALL NECESSARY SIGNAGE, DELINEATORS, MARKERS AND BARRIERS.
- 10. DO NOT ALTER GRADING OF THE SITE WITHOUT PRIOR APPROVAL OF THE ENGINEER/CITY.
- 11. ALL ROADWAY, PARKING LOT, AND GRADING WORKS TO BE UNDERTAKEN IN ACCORDANCE WITH CITY STANDARDS AND SPECIFICATIONS. THE CONTRACTOR IS TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE BUILDING.
- 12. CONTACT THE CITY FOR INSPECTION OF ROUGH GRADING OF PARKING LOTS, ROADWAYS AND LANDSCAPED AREAS PRIOR TO PLACEMENT OF ASPHALT AND TOPSOIL. ALL DEFICIENCIES NOTED SHALL BE RECTIFIED TO THE CITY'S SATISFACTION PRIOR TO PLACEMENT OF ANY ASPHALT, TOPSOIL, SEED & MULCH AND/OR SOD.
- 13. ALL DIMENSIONS AND INVERTS MUST BE VERIFIED PRIOR TO CONSTRUCTION, IF THERE IS ANY DISCREPANCY THE CONTRACTOR IS TO NOTIFY THE ENGINEER PROMPTLY.
- 14. ELECTRICAL, GAS, TELEPHONE AND TELEVISION SERVICE LOCATIONS ARE SUBJECT TO THE INDIVIDUAL AGENCY: • ELECTRICAL SERVICE - HYDRO OTTAWA, GAS SERVICE - ENBRIDGE,
 TELEPHONE SERVICE - BELL CANADA,
 TELEVISION SERVICE - ROGERS.
- 16. INSTALLATION TO BE IN ACCORDANCE WITH CURRENT CODES AND STANDARDS OF APPROVAL AGENCIES HYDRO OTTAWA, BELL AND THE CITY.
- 17. CONTRACTOR TO ENSURE ALL APPLICABLE OPS SPECIFICATIONS ARE FOLLOWED DURING CONSTRUCTION
- 18. ALL PROPOSED CURB TO BE CONCRETE BARRIER CURB UNLESS
- OTHERWISE SPECIFIED. THIS PLAN MUST BE READ IN CONJUNCTION WITH THE GEOTECHNICAL INVESTIGATION

Site Location NATION AVENUE

LOCATION PLAN

LEGEND LIMIT OF CONSTRUCTION PROPERTY LINE ---- · ---- DRAINAGE SWALE

— — — — — DRAINAGE DITCH CONCRETE WALKWAY SLOPING AT 3:1
UNLESS SPECIFIED PROPOSED ASPHALT LSCB# LANDSCAPING CATCHBASIN SURFACE ELEVATION

CBMH# CATCHBASIN MANHOLE SWALE ELEVATION ×T/W95.50 B/W94.25 TOP OF WALL ELEVATION BOTTOM OF WALL ELEVATION CATCHBASIN OVERLAND FLOW ROUTE SANITARY SEWER MANHOLE SILT FENCE BARRIER STRAW BALE CHECK DAM

MUD MAT WATER METER REMOTE WATER METER

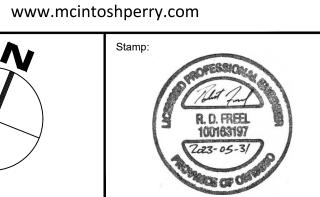
WATER VALVE

NOT FOR CONSTRUCTION

01 ISSUED FOR SITE PLAN CONTROL 05.31.2023 Date

Check and verify all dimensions before proceeding with the work SCALE 1:250

McINTOSH PERRY 115 Walgreen Road, RR3, Carp, ON KOA 1L0 Tel: 613-836-2184 Fax: 613-836-3742



Do not scale drawings

1531 ST LAURENT LIMITED PARTNERSHIP

69 RUE JEAN-PROULX #301
GATINEAU, QC J8Z 1W2

RESIDENTIAL MIXED-USE BUILDING
1531 ST LAURENT BLVD

Drawing Title:

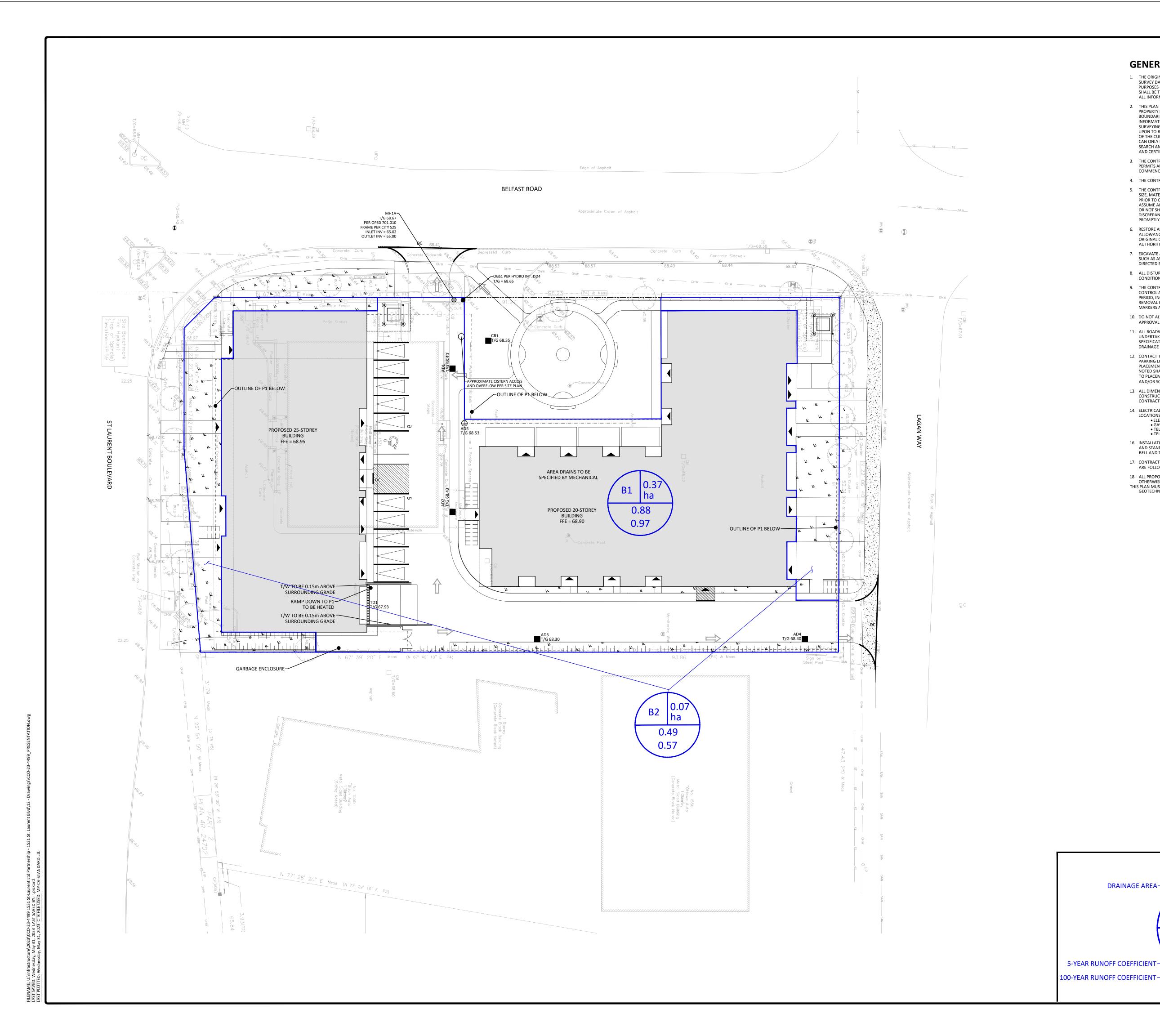
OTTAWA

PRE-DEVELOPMENT DRAINAGE PLAN

1:250 CCO-23-4499 R.D.F. Designed By: R.R.R. PRE

APPENDIX F POST-DEVELOPMENT DRAINAGE PLAN

APPENDIX G STORWWATER MANAGEMENT CALCULATIONS



GENERAL NOTES

- 1. THE ORIGINAL TOPOGRAPHY, GROUND ELEVATION AND SURVEY DATA SHOWN ARE SUPPLIED FOR INFORMATION PURPOSES ONLY, AND IMPLY NO GUARANTEE OF ACCURACY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL INFORMATION SHOWN.
- 2. THIS PLAN IS NOT A CADASTRAL SURVEY SHOWING LEGAL PROPERTY BOUNDARIES AND EASEMENTS. THE PROPERTY BOUNDARIES SHOWN HEREON HAVE BEEN DERIVED INFORMATION SUPPLIED BY FARLEY, SMITH AND DENNIS SURVEYING LTD. (JOB NO. 432-22) AND CANNOT BE RELIED UPON TO BE ACCURATE OR COMPLETE. THE PRECISE LOCATION OF THE CURRENT PROPERTY BOUNDARIES AND EASEMENTS CAN ONLY BE DETERMINED BY AN UP-TO-DATE LAND TITLES SEARCH AND A SUBSEQUENT CADASTRAL SURVEY PERFORMED AND CERTIFIED BY AN ONTARIO LAND SURVEYOR.
- THE CONTRACTOR IS TO OBTAIN AND PAY FOR ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY BEFORE COMMENCING CONSTRUCTION.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR ALL LAYOUT.
- 5. THE CONTRACTOR IS TO DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME ALL RESPONSIBILITY FOR EXISTING UTILITIES WHETHER OR NOT SHOWN ON THESE DRAWINGS. IF THERE IS ANY DISCREPANCY THE CONTRACTOR IS TO NOTIFY THE ENGINEER
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- OTHERWISE SPECIFIED.
 THIS PLAN MUST BE READ IN CONJUNCTION WITH THE GEOTECHNICAL INVESTIGATION

Site Location - - - INDUSTRIAL AVENUE NATION AVENUE

LOCATION PLAN LEGEND

LIMIT OF CONSTRUCTION PROPERTY LINE ---- · ---- DRAINAGE SWALE — — — — DRAINAGE DITCH CONCRETE WALKWAY

SLOPING AT 3:1
UNLESS SPECIFIED PROPOSED ASPHALT LSCB# LANDSCAPING CATCHBASIN SURFACE ELEVATION CBMH# CATCHBASIN MANHOLE SWALE ELEVATION TOP OF WALL ELEVATION BOTTOM OF WALL ELEVATION

CATCHBASIN OVERLAND FLOW ROUTE SANITARY SEWER MANHOLE SILT FENCE BARRIER FIRE HYDRANT STRAW BALE CHECK DAM MUD MAT WATER METER

REMOTE WATER METER

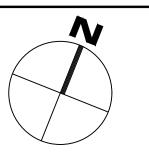
NOT FOR CONSTRUCTION

05.31.2023 01 ISSUED FOR SITE PLAN CONTROL Date

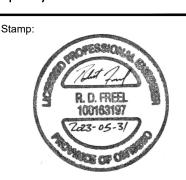
SCALE 1:250

McINTOSH PERRY

115 Walgreen Road, RR3, Carp, ON KOA 1L0 Tel: 613-836-2184 Fax: 613-836-3742 Fax: 613-836-3742 www.mcintoshperry.com



Check and verify all dimensions before proceeding with the work



Do not scale drawings

1531 ST LAURENT LIMITED PARTNERSHIP

69 RUE JEAN-PROULX #301
GATINEAU, QC J8Z 1W2

RESIDENTIAL MIXED-USE BUILDING 1531 ST LAURENT BLVD

OTTAWA

Drawing Title:

R.R.R.

POST-DEVELOPMENT DRAINAGE PLAN

1:250 CCO-23-4499 Checked By: R.D.F. POST

CO-22-4499 - 1531 St. Laurent - SWM Calculations

1 of 3

Tc (min)		Intensity (mm/hr)		
(111111)	2-Year	5-Year	100-Year	
10	76.8	104.2	178.6	PRE-DEVELOPM ENT
10	76.8	104.2	178.6	POST-DEVELOPMENT

C-Values					
Impervious	0.90				
Gravel	0.60				
Pervious	0.20				

Pre-Development Runoff Coefficient

Drainage	Impervious	Gravel	Pervious Area	Average C	Average C
Area	Area (m²)	(m²)	(m²)	(5-year)	(100-year)
A1	4,404	0	60	0.89	

Pre-Development Runoff Calculations

Drainage	Area	С	C	Tc (min)		Q (L/s)	
Area	(ha)	5-Year	100-Year		2-Year	5-Year	100-Year
A1	0.446	0.89	0.99	10	84.89	115.16	219.36
Total	0.45				84.89	115.16	219.36

Post-Development Runoff Coefficient

Drainage Area	Impervious Area (m²)	Gravel (m²)	Pervious Area (m²)	Average C (5-year)	Average C (100-year)
B1	3,610	0	130	0.88	0.97
B2	305	0	419	0.49	0.57

Post-Development Runoff Calculations

Drainage	Area	С	C Tc		Q(L/s)	
Area	(ha)	5-Year	100-Year	(min)	5-Year	100-Year	
B1	0.37	0.88	0.97	10	94.86	180.81	Restricted
B2	0.07	0.49	0.57	10	10.37	20.32	Unrestricted
Total	0.45				105.23	201.14	

Required Restricted How

Drainage	Area	С	Tc	Q (L/s)
Area	(ha)	2-Year	(min)	2-Year
A1	0.45	0.50	10	47.66

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow (L/S)		Restricted Flow (L/S)		Storage Required (m ³)		Storage Provided (m³)	
Area	5-year	100-Year	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year
B1	94.86	180.81	14.35	27.34	62.5	118.3	27.0	51.6
B2	10.37	20.32	10.37	20.32				
Total	105.23	201.14	24.71	47.66	62.5	118.3	27.0	51.6

CO-22-4499 - 1531 St. Laurent - SWM Calculations

Storage Requirements for Area B1

2 of 3

5-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
10	104.2	94.87	14.35	80.52	48.31
20	70.3	64.00	14.35	49.66	59.59
30	53.9	49.07	14.35	34.73	62.51
40	44.2	40.24	14.35	25.90	62.15
50	37.7	34.32	14.35	19.98	59.94

Maximum Storage Required 5-year = 63 m³

100-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	178.6	180.85	27.34	153.51	92.11
20	120.0	121.51	27.34	94.17	113.01
30	91.9	93.06	27.34	65.72	118.29
40	75.1	76.05	27.34	48.71	116.90
50	64.0	64.81	27.34	37.47	112.40
60	55.9	56.61	27.34	29.27	105.35
70	49.8	50.43	27.34	23.09	96.97
80	45.0	45.57	27.34	18.23	87.49
90	41.1	41.62	27.34	14.28	77.10
100	37.9	38.38	27.34	11.04	66.23

Maximum Storage Required 100-year = 118 m³

5-Year Storm Event Storage Summary

Storage Available (m³) = 118.3 * Provided by on-site cistern
Storage Required (m³) = 62.5

100-Year Storm Event Storage Summary

Storage Available (m³) = 118.3

* Provided by on-site cistern
Storage Required (m³) = 118.3

CO-22-4499 - 1531 St. Laurent - SWM Calculations

3 of 3

Time of Concentration Pre-Development

Drainage Area	Sheet Flow	Sope of	Tc (min)	Tc (min)
ID	Distance (m)	Land (%)	(5-Year)	(100-Year)
A1	112	0.71	8	4

Therefore, a Tc of 10 can be used

 $Tc = (3.26(1.1-c)L^0.5/S^0.33)$

c = Balanced Runoff Coefficient
 L = Length of drainage area
 S = Average slope of watershed

Hydro Downstream Defender®

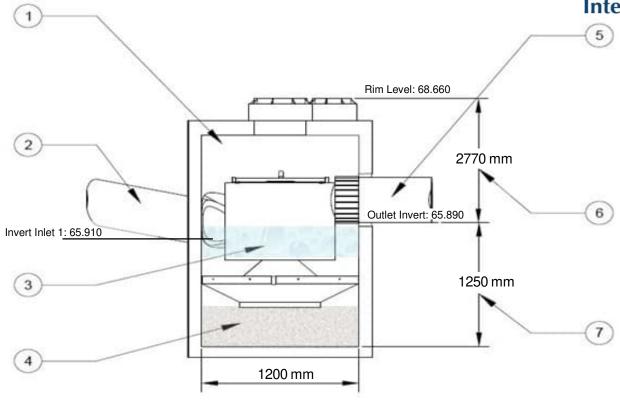


Net Annual Water Quality Worksh	eet						Interna	tional 📚
Rev. 12.5	001				Ne	et Annual Rem	oval Model: DI	D4
Project Name: Residential Building Street: 1531 St Laurent Province: Ontario	Report Date: City: Country:	Ottawa		Paste	Intensity ⁽¹⁾	Fraction of Rainfall ⁽¹⁾	DD4 Removal Efficiency ⁽²⁾	Weighted Net Annual Efficiency ⁽⁴⁾
Designer: RRR	email:	r.robinea	u@mcint	oshperry	(mm/hr)	(%)	(%)	(%)
					0.50	0.1%	100.0%	0.09%
Treatment Parameters		DECII	LTS SUMI	MARV	1.00	14.1%	100.0%	14.11%
Structure ID: OGS1		HESO	L 1 O O O IVII	WALL	1.50	14.2%	98.6%	14.00%
TSS Goal: 80 % Removal		Model	TSS	Volume	2.00	14.1%	94.1%	13.29%
TSS Particle Size: Fine		DD4	85.0%	>90%	2.50	4.2%	90.7%	3.77%
<i>Area:</i> 0.37 ha		DD6	93.0%	>90%	3.00	1.5%	88.1%	1.31%
Percent Impervious: 97%		DD8	96.0%	>90%	3.50	8.5%	85.9%	7.34%
Rational C value: 0.97 Calc Cn		DD10	98.0%	>90%	4.00	5.4%	84.1%	4.56%
Rainfall Station: Ottawa, ONT	MAP	DD12	98.0%	>90%	4.50	1.2%	82.5%	0.96%
Peak Storm Flow: 27.34 L/s					5.00	5.5%	81.0%	4.48%
Peak Storm Return: 100 yrs					6.00	4.3%	78.7%	3.41%
					7.00	4.5%	76.7%	3.46%
Model Specification					8.00	3.1%	75.1%	2.32%
					9.00	2.3%	73.6%	1.72%
Select Model DD4					10.00	2.6%	72.4%	1.85%
Diameter: 1200 mm					20.00	9.2%	64.7%	5.98%
шинининин					30.00	2.6%	60.5%	1.59%
Peak Flow Capacity: 85.00 L/s OK					40.00	1.2%	57.7%	0.67%
Sediment Storage: 0.54 m ³					50.00	0.5%	55.7%	0.29%
<i>Oil Storage:</i> 265.00 L					100.00	0.7%	0.0%	0.00%
					150.00	0.1%	0.0%	0.00%
Installation Configuration					200.00	0.0%	0.0%	0.00%
Placement: Offline					200.00	0.070	0.076	0.0076
Outlet Pipe Size: 300 mm OK					Total Net	Annual Remo	val Efficiency:	85.0%
Inlet Pipe 1 Size: 300 mm OK						ual Runoff Vo		>90%
Inlet Pipe 2 Size: mm OK							a, ONT, 6105976 & 610	
Rim Level: 68.660 m Calc Invs. Outlet Pipe Invert: 65.890 m OK	I				Based on third par	ty verified data and ap	poximating the removal	of a PSD similar to
Invert Pipe 1: 65.910 m OK Invert Pipe 2: m					4. Factored to accoun			

Designer Notes:

Hydro Downstream Defender®





All drawing elevations are metres.

DD4 Specification

1	Vortex Chamber Diameter	1200 mm
2	Inlet Pipe Diameter	300 mm
3	Oil Storage Capacity	265 L
4	Min. Provided Sediment Storage Capacity	0.54 m ³
5	Outlet Pipe Diameter	300 mm
6	Rim to Outlet Invert	2770 mm
7	Outlet Invert to Sump	1250 m
	Total Depth	4020 mm

Notes:	
The Downstream Defender is certified by Canada ETV	

STORM SEWER DESIGN SHEET

PROJECT: COO-23-4499
LOCATION: 1531 St Laurent

	LOCATION	N			CONTRIBUTING AREA (h.	a)						RATIO	ONAL DESIGN	FLOW									SEWER DATA				
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24 2	:5	26	27	28
STREET	AREA ID	FROM	TO	C-VALUE	AREA	INDIV	CUMUL	INLET	TIME	TOTAL	i (5)	i (10)	i (100)	5yr PEAK	10yr PEAK	100yr PEAK	FIXED	DESIGN	CAPACITY	LENGTH	PIPES	IÆ(mm)	SL	DPE V	VELOGITY	AVAILG	AP (5yr)
SINCE	ANDATO	МН	MH	GVALUE	ANDA	AC	AC	(min)	IN PIPE	(min)	(mm/hr)	(mm/hr)	(mm/hr)	FLOW (L/s)	(L/s)	(m)	DIA	W	Н (%)	(m/s)	(L/s)	(%)				
		BLDG	OGS1	0.88	0.37	0.33	0.33	10.00	0.00	10.00	104.19	122.14	178.56	94.86	111.75	163.37		94.86	100.88		300		1	00	1.383	6.02	5.97%
Belfast Road	B1	OGS1	EX. Sewer			0.00	0.33	10.00	0.00	10.00	104.19	122.14	178.56	94.86	111.75	163.37		94.86	100.88		300		1	00	1.383	6.02	5.97%
Definitions:				Notes:			<u> </u>	Designed:	J	J			No.					Revision							Date		
Q = 2.78QA, where:				1. Mannings coefficient (n) :	=		0.013						1.														
Q = Peak Flow in Litres p	per Second (L/s)							RRR																			
A = Area in Hectares (ha	a)							Checked:																			
i = Rainfall intensity in n	millimeters per hour ((mm/hr)																									
[i = 998.071 / (TC+6.0	053)^0.814]	5 YEAR						RDF																			
[i = 1174.184 / (TC+6.0	.014)^0.816]	10 YEAR						Project No.:																			
[i = 1735.688 / (TC+6.0	.014)^0.820]	100 YEAR															Date:								Sheet No:		
· ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` `								000-23-4499	ı																1 of 1		

APPENDIX H
CITY OF OTTAWA DESIGN CHECKLIST

Oty of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Oriteria Criteria Cri	Location (if applicable)
☐ Executive Summary (for larger reports only).	N/A
☐ Date and revision number of the report.	On Cover
Location map and plan showing municipal address, boundary, and layout of proposed development.	Appendix A
☐ Plan showing the site and location of all existing services.	Ste Servicing Plan (C102)
 Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual 	1.1 Purpose 1.2 Ste Description
developments must adhere.	6.0 Stormwater Management
Summary of pre-consultation meetings with City and other approval agencies.	Appendix B
☐ Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments,	1.1 Purpose
Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and	1.2 Ste Description
develop a defendable design criteria.	6.0 Stormwater Management
☐ Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary



☐ Identification of existing and proposed infrastructure available in the immediate area.	N/A
☐ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Ste Grading Plan (C101)
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Ste Grading Plan (C101)
☐ Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
Proposed phasing of the development, if applicable.	N/ A
Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Background Studies, Standards and References
 All preliminary and formal site plan submissions should have the following information: Metric scale North arrow (including construction North) Key plan Name and contact information of applicant and property owner Property limits including bearings and dimensions Existing and proposed structures and parking areas Easements, road widening and rights-of-way Adjacent street names 	Ste Grading Plan (C101)

4.2 Development Servicing Report: Water

Oriteria	Location (if applicable)
☐ Confirm consistency with Master Servicing Study, if available	N/A
Availability of public infrastructure to service proposed development	N/A
☐ Identification of system constraints	N/A
☐ Identify boundary conditions	Appendix C
☐ Confirmation of adequate domestic supply and pressure	N/A
 Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. 	Appendix C
 Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. 	N/A
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design	N/A
Address reliability requirements such as appropriate location of shut-off valves	N/ A
☐ Check on the necessity of a pressure zone boundary modification.	N/ A
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	Appendix C, Section 4.2

Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Ste Servicing Plan (C101)
Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix C
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N/A

4.3 Development Servicing Report: Wastewater

Oriteria	Location (if applicable)
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/ A
☐ Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/ A
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 5.2 Proposed Sanitary Sewer

☐ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Section 5.3 Proposed Sanitary Design
☐ Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	N/ A
 Description of proposed sewer network including sewers, pumping stations, and forcemains. 	Section 5.2 Proposed Sanitary Sewer
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/ A
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

Oriteria	Location (if applicable)
Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ Analysis of available capacity in existing public infrastructure.	N/A
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Pre & Post-Development Plans
Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
☐ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Set-back from private sewage disposal systems.	N/A
☐ Watercourse and hazard lands set backs.	N/A
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period).	Appendix G

☐ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Ste Grading Plan
Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 7.0 Proposed Stormwater Management Appendix G
Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/ A
☐ Identification of potential impacts to receiving watercourses	N/A
Identification of municipal drains and related approval requirements.	N/ A
Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 6.0 Stormwater Sewer Design & Section 7.0 Proposed Stormwater Management
100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Ste Grading Plan (C101)
☐ Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/ A

Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Section 8.0 Sediment & Erosion Control
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
☐ Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

O riteria	Location (if applicable)
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/ A
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A
☐ Changes to Municipal Drains.	N/A
Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A

4.6 Conclusion Checklist

Oriteria	Location (if applicable)
Gearly stated conclusions and recommendations	Section 9.0 Summary
	Section 10.0 Recommendations
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped