# FUNCTIONAL SERVICING STUDY REPORT

For 2409 Carlsen Avenue, Ottawa

# Prepared by:

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### 1. **Project Description:**

#### 1.1. Introduction:

Property at 2409 Carlsen Avenue is located close to intersection of Carlsen Avenue and Heron Road, Ottawa, Ontario. The property is about 0.11 Hectare severed from an existing lot which contain an existing two story building.

Property at 2409 Carlsen Avenue is currently under R3A Zoning. Due to market demand for residential, the idea initiated to use the lot to build 3 three-story dwelling that contains 8 units each. This report will address the servicing (water, sanitary) requirements associated with the proposed development located at 2409 Carlsen Avenue within the City of Ottawa, Ontario. This report is prepared in response to the request from City of Ottawa Planning department.

#### 1.2. Existing Conditions:

The existing site located at 2409 Carlsen Avenue. The property measure a total area of approximately 0.11 Hectare. The site is fronting 305mm diameter UCI water main on Heron Road, 152mm diameter CI water main on Carlsen Avenue and 152mm diameter CI water main on Chasseur Ave. Also the site is fronting 300mm diameter Concrete sanitary main on Heron Road, 225mm diameter Concrete sanitary main on Carlsen Avenue and 300mm diameter Concrete sanitary main on Chasseur Avenue. In this report the development water and sanitary connection will be to mains on Carlsen Avenue and Chasseur Avenue.



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#### 1.3. Guidelines, Previous Studies, And Reports

The following studies were utilized in the preparation of this report:

- Ottawa Sewer Design Guidelines,
   City of Ottawa, SDG002, October 2012.
   (City Standards)
  - Technical Bulletin ISTB-2018-01 City of Ottawa, March 21, 2018. (ISTB-2018-01)
  - Technical Bulletin ISTB-2018-04 City of Ottawa, June 27, 2018. (ISTB-2018-04)
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010.
   (Water Supply Guidelines)
  - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
  - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 27, 2014. (ISDTB-2014-02)
  - Technical Bulletin ISTB-2018-02 City of Ottawa, March 21, 2018. (ISTB-2018-02)
- Design Guidelines for Sewage Works, Ministry of the Environment, 2008.
   (MOE Design Guidelines)
- Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (SWMP Design Manual)
- Ontario Building Code Compendium Ministry of Municipal Affairs and Housing Building Development Branch, January 1, 2012 Update. (OBC)
- ➤ Geotechnical Investigation Report

# 2. Water Supply

## **Residential Water Demand:**

The water demand is calculated based on the City of Ottawa Design Guidelines – Water Distribution as follows:

Design Parameter	Value
Residential 1 Bedroom Apartment	1.4 P/unit
Residential 2 Bedroom Apartment	2.1 P/unit
Residential Average Daily Demand	280 L/d/P
Residential Maximum Daily Demand	2.5 x Average Daily *
Residential Maximum Hourly	2.2 x Average Daily *
Commercial Retail	2.5 L/m <sup>2</sup> /d
Commercial Maximum Daily Demand	1.5 x avg. day
Commercial Maximum Hour Demand	1.8 x max. day
Minimum Watermain Size	150mm diameter
Minimum Depth of Cover	2.4m from top of watermain to finished grade
During normal operating conditions desired operating pressure is within	350kPa and 480kPa
During normal operating conditions pressure must not drop below	275kPa
During normal operating conditions pressure must not exceed	552kPa
During fire flow operating pressure must not drop below	140kPa

Residential occupancy = 1.4 persons per one bedroom apartment and 2.1 persons per 2 bedroom apartment and 3.1 persons per 3 bedroom apartment
□ 7 x 2 bedroom units x 2.1 (average) pers./unit = 14.7 persons □ 1 x 1 bedroom units x 1.4 (average) pers./unit = 1.4 persons
Total occupancy taken as $= 16.1$ persons taken as 17 persons
Residential Average Daily Demand = 280 L/c/d.  □ Average daily demand of 280 L/c/day x 17 persons =4760 Liters/day or 0.06 L/s  □ Maximum daily demand (factor of 2.5) is 0.06 L/s x 2.5 = 0.15 L/s  □ Peak hourly demand (factor of 2.2) = 0.15 L/s x 2.2 = 0.33 L/s

# Fire Fighting Requirement

## **Based on Fire Underwriter Survey Method**

Fire flow protection requirements were calculated as per the Fire Underwriter's Survey (FUS). The estimated fire flow for the proposed buildings was calculated in accordance with ISTB-2018-02. The following parameters were provided by the Architect:

• Type of construction – Ordinary Construction

F=220C\A

- Occupancy type Limited Combustibility
- Sprinkler Protection Standard Fully Supervised Sprinkler System

The fire flow demand was estimated to be 9,000 L/min,

Address:

where			File No.:					
F=	Required fir	ed fire flow in L/min						
C=	Coefficient	ent related to the type of construction						
A=	Total floor a	irea in m'						
	Coefficie	at Related to	Type of C	onstructio		3057		C-Value
	0.0000000000000000000000000000000000000	<ul> <li>Wood Frame</li> </ul>	Construction	1	4			1.5
		<ul> <li>Ordinary Con:</li> </ul>	struction		7	₩.		1.0
C		<ul> <li>Non-Combust</li> </ul>	tible Constru	iction	4			0.8
		<ul> <li>Fire-Resistive</li> </ul>	Constructio	n	4			0.6
						c	=	1.0
۸	100	Total Floor	Area (m')		•	DRI.		
		5 ft <sup>1</sup>	41	450.1	2 m	<u>'</u>		
F		red Fire Flow	(L/min)					
		= 220 CNA = <b>4668</b> I	L/min					
Оссеран	cy Reduction	s or Surcharge						2000
		• Non-Combus						-25%
		<ul> <li>Limited Comb</li> </ul>	ustible			₩.		-15%
		<ul> <li>Combustible</li> </ul>				$\vdash$		0%
		Free Burning				$\vdash$		15%
		<ul> <li>Rapid Burning</li> </ul>	1			П		25%
								-15%
						3967	L	/min
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		Adequately D		em	4	H		-30%
		<ul> <li>Water Supply</li> </ul>			31.	_		-10%
		• Fully Supervis	ed System			☑	=	-10% -10%
			3961	/ L/min				-10%
		Reduction: _	397	7 L/min	_			
		Fire Flow	3571	L/min	_			
Exposur	e Surcharge	Distance	Charge	8 of Side				
		• O to 3m	25%	a or side		25%		
		• 3.1 to 10m	20%	0.5		23%		
		• 10.1 to 20m	15%					
		• 20.1 to 30m	10%	2		20%		
		• 30.1 to 45m	5%	1		5%		
		- 50.110 45111		8.6	=	50%		
			357	1 L/min		30%		
		Surcharge:		L/min				
		Fire Flow:	5356	L/min				
REQUIR	ED FIRE FLO	<b>₩</b> ed 45,000 L/min n	or be less th	an 2 000 L/m	in			
						535	6 L	./min
				or			89 L	
				or				GPM
							_	

There are three (3) existing fire hydrants in proximity to the proposed building that are available to provide the required fire flow demand of 9,000 L/min. Fire hydrant locations are demonstrated in below sketch. Table below summarizes the aggregate fire flow of the contributing hydrants in close proximity to the proposed development based on Table 18.5.4.3 of ISTB-2018-02.



Fire Protection Summary Table

Building	Fire Flow Demand (L/min)	Fire Hydrant within 75m	Fire Hydrant within 150m	Fire Hydrant within 300m	Available Combine Fire Flow (L/min)
Proposed 2409 Carlsen Avenue	5,400	2	0	0	$(2 \times 5678) + (0 \times 3785) = 11,356$

The total available fire flow from contributing hydrants is equal to 11,356L/min which will provide adequate fire flow for the proposed development. A certified fire protection system specialist will need to be employed to design the building's fire suppression system and confirm the actual fire flow demand.

The city of Ottawa was contacted to obtain boundary conditions associated with the estimated water demand. The followings are boundary conditions, HGL, for hydraulic analysis at 2409 Carlsen Avenue assumed to be three separate buildings connected via three connections to 152mm watermain at Carlsen and 152mm watermain at Chasseur. Note that 2409 Carlsen Avenue is located in zone 2W2C water pressure.

#### All three connections:

Minimum HGL: 123.9 m Maximum HGL: 132.0 m

Max Day + Fire Flow (90 L/s): 116.0 m (Connection 1), 116.0 m (Connection 2), 115.7 m

(Connection 3)

Average ground elevation of 78.50 m

#### Analyzing results:

Demand Senario	Head (m)	Pressure (KPa)
Max. HGL	132.0 - 78.50 = 53.50	524
Min HGL	123.9 - 78.50 = 45.40	445
Max Day + Fire Flow	116.0 - 78.50 = 37.50	368

#### ❖ Ground Elevation = 78.50 m

Floor Elevation	Max Day + Fire Flow (m) = 116.0	Pressure (KPa) at Each Floor
Ground Floor EL. = 80.00 m	36.0	353

Based on City of Ottawa Design Guidelines – Water Distribution existing water service size of 25mm is adequate where the residential water pressure is over 310 kPa. As such, since the calculated pressure is approximately above the minimum requirement, the service diameter for the proposed development recommended to be 25mm.

Note that pressure test will be required at the time of construction to confirm minimum pressure is supplied for proposed development.

The proposed water supply design conforms to all relevant City Guidelines and Policies.

## 3. Sanitary Sewage

The sanitary flow is calculated based on the Ministry of Environment Guidelines as follow:

Design Parameter	Value
Residential 1 Bedroom Apartment	1.4 P/unit
Residential 2 Bedroom Apartment	2.1 P/unit
Average Daily Demand	280 L/d/per
Peaking Factor	Harmon's Peaking Factor. Max 4.0, Min 2.0 Harmon Correction Factor 0.8
Commercial Floor/Amenity Space	2.5 L/m <sup>2</sup> /d
Commercial Peaking Factor*	1.0
Infiltration and Inflow Allowance	0.05 L/s/ha (Dry) 0.28 L/s/ha (Wet) 0.33 L/s/ha (Total)
Sanitary sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}}$
Minimum Sewer Size	200 mm diameter
Minimum Manning's 'n'	0.013
Minimum Depth of Cover	2.5 m from crown of sewer to grade
Minimum Full Flowing Velocity	0.6 m/s
Maximum Full Flowing Velocity	3.0 m/s

# 3.1. Sanitary Sewage Calculation

#### **Design Flows**

Residential

 $\Box$  7 x 2 bedroom units x 2.1 (average) pers./unit = 14.7 persons

 $\Box$  1 x 1 bedroom units x 1.4 (average) pers./unit = 1.4 persons

Total occupancy taken as = 16.1 persons taken as 17 persons

Residential Average Daily Demand = 280 L/c/d.

 $\hfill \Box$  Average daily of 280 L/c/day x 17 persons =4760 Liters/day or 0.06 L/s

Peaking Factor =  $1 + 14/(4 + (13/1000)^0.5) = 4.40 *use 4 maximum$ 

Q Peak Domestic =  $0.06 \text{ L/sec} \times 4.0 = 0.24 \text{ L/sec}$ 

#### **Infiltration**

Q Infiltration =  $0.11 \text{ L/S/Gross hectare } \times 0.10 \text{ ha} = 0.01 \text{ L/sec}$ 

#### Total Peak Sanitary Flow = 0.24 + 0.01 = 0.25 L/sec

The Ontario Building Code specifies minimum pipe size and maximum hydraulic loading for sanitary sewer pipe. OBC 7.4.10.8 (2) states "Horizontal sanitary drainage pipe shall be designed to carry no more than 65% of its full capacity." A 150 mm diameter sanitary service with a minimum slope of 5.0% has a capacity of 73 Litres per second.

The maximum peak sanitary flows for the site is 0.43 L/s. Since 0.43 L/s is much less than 0.65 x 73 = 47 L/s, which means existing 150mm sanitary line has enough capacity.

The flow depth under peak flow is less than 0.3 of the pipe diameter, therefore, the actual velocity is calculated and pipe slope increased to 5% achieve minimum self-cleansing velocity of 0.6m/s as per the recommendation of section 6.1.2 of the City of Ottawa Sewer Design Guidelines. Please refer to the appendix for sanitary design calculation sheet.

Sewage discharges will be domestic in type and in compliance with the Ministry of Environment guidelines. The peak sanitary flow from the proposed development is less than 10 percent of the capacity of the existing sanitary. As such the proposed increase in sanitary flow as a result of the construction of the proposed development is negligible and there is sufficient available capacity for the proposed development.

Should you have any questions or comments, please feel free to contact undersigned.

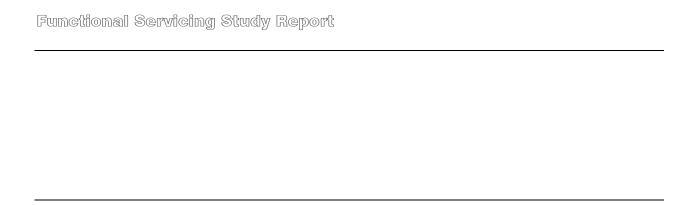
Yours truly, Wissam Elias, P. Eng

Senior Project Manager

WOE OF ON

Functional Servicing Study Report			
APPENDIX A:			
GeoOttawa Snapshot			





# **APPENDIX B:**

Correspondent &

Architectural/Engineering Drawings

