

Report
Project: 135883-6.04.01

SERVICING AND STORMWATER MANAGEMENT REPORT 255 RICHMOND ROAD



Prepared for Y Street Capital C/O Vince Colizza Architects
by IBI Group
July 18, 2025

Table of Contents

1	INTRODUCTION	1
1.1	Scope	1
1.2	Subject Site	1
1.3	Pre-consultation	1
2	WATER DISTRIBUTION	2
2.1	Existing Conditions	2
2.2	Design Criteria	2
2.2.1	Water Demands	2
2.2.2	System Pressures	3
2.2.3	Fire Flow Rate	3
2.3	Proposed Water Plan	3
3	WASTEWATER SYSTEM	5
3.1	Existing Conditions	5
3.2	Design Criteria	5
3.3	Recommended Wastewater Plan	5
4	STORMWATER MANAGEMENT	7
4.1	Existing Conditions	7
4.2	Design Criteria	7
4.3	Proposed Minor System	8
4.4	Stormwater Management	8
5	SOURCE CONTROLS	11
5.1	General	11
6	SEDIMENT AND EROSION CONTROL PLAN	11
6.1	General	11
6.2	Trench Dewatering	11
6.3	Surface Structure Filters	11
7	SOILS and GRADING	12
8	Conclusions	13

Table of Contents (continued)

APPENDICIES:

Appendix A:	Figure 1 – Site Location Figure 2 – Site Plan Pre-Consult Meeting Notes City of Ottawa Servicing Checklist
Appendix B:	Existing conditions survey of site Water demand Sheet Water Boundary Conditions FUS Calculation Figure 2.2 – Hydrant Coverage
Appendix C:	Sanitary Sewer Design Sheets C-400 Sanitary Drainage Area Plan
Appendix D:	Storm Sewer Design Sheets Modified Rational Method Design Sheets C-500 Storm Drainage Area Plan C-501 Pre-Developments Storm Drainage Area Plan
Appendix E:	C-001 Servicing Plan C-200 Grading Plan C-900 Sedimentation and Erosion Control Plan

1 INTRODUCTION

1.1 Scope

The purpose of this report is to outline the required municipal services, including water supply, stormwater management and wastewater disposal, needed to support the redevelopment of the subject property. The property is approximately 0.21 hectares in area and is an assembly of parcels of land comprised of 249-255 Richmond Road and 372 Tweedsmuir Ave.

The subject site is bounded by Tweedsmuir Ave to the east, and Richmond Road to the south, as well as existing commercial lands to the west and residential lands to the north. Refer to Figure 1 in **Appendix A** for site location.

This Site Servicing Study, which also includes the Stormwater Management Plan, Watermain Analysis and Erosion and Sedimentation Control Plans, is being completed in support of the Site Plan Application.

1.2 Subject Site

The proposed development consists of one mixed use multi-storey building consisting of 127 residential units, retail on the first floor fronting Richmond Road, and underground parking to support the proposed building. A copy of the proposed Site Plan, prepared by Vince Colizza Architects is included in **Appendix A**. The plan illustrates the building occupying most of the parcel and vehicular access to the site is provided from Tweedsmuir Ave.

The site currently consists of several commercial buildings (Ottawa Medispa, Capital Printers, Rikochet Resale, and Whispers Pub) and a residential building. All existing structures within the subject property will be demolished to facilitate the proposed development.

1.3 Pre-consultation

A pre-consultation meeting with the City of Ottawa was held regarding the proposed development. Notes from this meeting may be found in **Appendix A**. No significant constraints relating to the site servicing or stormwater management for the subject lands were identified during the consultation.

2 WATER DISTRIBUTION

2.1 Existing Conditions

The proposed development is located within the City of Ottawa pressure zone 1W. There is a 300 mm diameter watermain along Richmond Road and a 150mm diameter watermain along Tweedsmuir Ave, both mains service the existing buildings within the subject parcel. Existing services within the project site will be disconnected and abandoned per City of Ottawa Standards. A survey of the subject parcel was completed by Farley, Smith, Denis Surveying Ltd. and is included in **Appendix B** the survey illustrates the location of the existing water plant adjacent to the site.

2.2 Design Criteria

2.2.1 Water Demands

The proposed development plan includes 127 residential units, as well as some commercial space on the first floor. Water demands have been calculated for the full development. Per unit population density and consumption rates are taken from Tables 4.1 and 4.2 at the Ottawa Design Guidelines – Water Distribution and are summarized as follows:

- ICI Average Day Demand 2500 l/m²/day
- ICI Peak Daily Demand 3750 l/m²/day
- ICI Peak Hour Demand 6750 l/m²/day
- Residential Average Day Demand 280 l/cap/day
- Residential Peak Daily Demand 700 l/cap/day
- Residential Peak Hour Demand 1540 l/cap/day

A watermain demand calculation sheet is included in **Appendix B** and the total water demands are summarized as follows:

- Average Day 0.68 l/s
- Maximum Day 1.67 l/s
- Peak Hour 3.67 l/s

The watermain demand calculation was forwarded to the city to determine the boundary conditions at the site, copy of the boundary conditions is included in **Appendix B** and summarized below. Note that unit count has increased slightly since boundary conditions have been received, however the difference is small.

	Richmond Connection	Tweedsmuir Connection
Minimum HGL	108.7	108.7
Maximum HGL	114.9	114.9
Max Day + FireFlow (133.3 L/s)	110.0	104.4

2.2.2 System Pressures

The 2010 City of Ottawa Water Distribution Guidelines states that the preferred practice for design of a new distribution system is to have normal operating pressures range between 345 kPa (50 psi) and 552 kPa (80 psi) under maximum daily flow conditions. Other pressure criteria identified in the guidelines are as follows:

Minimum Pressure	Minimum system pressure under peak hour demand conditions shall not be less than 276 kPa (40 psi).
Fire Flow	During the period of maximum day demand, the system pressure shall not be less than 150 kPa (21 psi) during a fire flow event.
Maximum Pressure	Maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi). In accordance with the Ontario Building/Plumbing Code the maximum pressure should not exceed 552 kPa (80 psi) in occupied areas. Pressure reduction controls may be required for buildings where it is not possible/feasible to maintain the system pressure below 552 kPa.

2.2.3 Fire Flow Rate

A calculation using the Fire Underwriting Survey (FUS) method was conducted to determine the fire flow requirement for the site. The building is considered non-combustible construction and is fully sprinklered. Results of the analysis provides a maximum fire flow rate of 8,000 l/min or 133.3 l/s is required which is used in the hydraulic analysis. A copy of the FUS calculation is included in **Appendix B**.

2.3 Proposed Water Plan

Two proposed 150mm diameter water services will connect the building to the municipal system. It is proposed to provide a connection to both Tweedsmuir and Richmond mains for redundancy purposes, see site servicing plan 135883-C-001 in **Appendix E**. An existing valve chamber separates the two connections. Two existing fire hydrants are expected to provide fire flow coverage for the site, as can be seen in **Figure 2.2** in **Appendix B**. For the purposes of this report, assuming a minimal loss within the service connection the pressures within the site can be estimated as follows:

Minimum Pressure (Peak Hour) – The minimum peak hour pressure on the site can be estimated as HGL 108.7m – meter elevation (assumed to be 1m above level P1) 63.06m = 45.64m or 447.46 kPa which exceeds the minimum requirement of 276 kPa. The pressure on the top floor can be estimated as 108.7m – 92.58m = 16.12m or 158.1 KPa which is below the minimum of 276 kPa and will require a water pump.

Fire Flow – On Richmond Rd, the max day plus fire flow can be estimated as HGL 110.0 – ground floor 67.47 = 42.53m or 417.2 KPa which exceeds the minimum of 140kPa. On Tweedsmuir Ave, the max day plus fire flow can be estimated as HGL 104.4 – 67.47 = 36.93m or 362.3 KPa which also exceeds the minimum of 150kPa.

Max HGL (High Pressure Check) – The high-pressure check can be estimated as HGL 114.9 – lowest level) 55.66 = 59.24m or 580.8 KPa which exceeds the maximum of 552 kPa, therefore a pressure reducing valve is required.

The above results indicate the municipal infrastructure can support the proposed development.

All existing water services will be located and abandoned per City of Ottawa specifications.

Two hydrants are available to service the subject property as seen in **Figure 2**. Both hydrants are within 75m of the building's Siamese connection. With 2 AA hydrants within 75m of the building the minimum number of hydrants needed to deliver the required fire flow to the structure is being provided in accordance with Technical Bulletin ISTB-2018-02 dated March 21, 2018.

BUILDING ID	FIRE FLOW DEMAND (L/MIN)	FIRE HYDRANT(S) WITHIN 75M (5,700 L/MIN)	FIRE HYDRANT(S) WITHIN 150M (3,800 L/MIN)	COMBINED FIRE FLOW (L/MIN)
255 Richmond	8,000	2	0	11,400

3 WASTEWATER SYSTEM

3.1 Existing Conditions

Municipal sanitary sewers abut the property along both Richmond Road and Tweedsmuir Ave, which provide servicing to the existing properties. A survey of the subject parcel was completed by Farley, Smith, Denis Surveying Ltd. and is included in **Appendix B** the survey illustrates the location of the existing sanitary sewers adjacent to the site.

3.2 Design Criteria

The sanitary flows for the development are based on the City of Ottawa design criteria which includes, but it not limited to the following criteria:

- Average Residential Flow 280 l/p/d
- Average Population density 1.8 PPU for apartments
- Residential Peaking Factor Harmon Formula [max = 4.0, min. = 2.0]
- Retail Flow 5 l/m²/d
- Restaurant Flow 125 l/seat/d
- ICI Peaking factor 1.5 if ICI in contributing area >20%
1.0 if ICI in contributing area <20%
- Infiltration allowance 0.33 l/s/ha
- Velocities 0.60 m/s min. to 3.0 m/s max.

3.3 Recommended Wastewater Plan

It is proposed to abandon the existing services for 372 Tweedsmuir and 249-255 Richmond, in accordance with City of Ottawa specifications. The sanitary sewer design sheet in **Appendix C** confirms the proposed service lateral to service the proposed building has sufficient capacity to accommodate the development.

The proposed development is a mixed-use development designed to provide a higher density meet this City objective of more intensification to maximize use of existing infrastructure. The following reviews the impact of increased density on the volume of wastewater to be generated from the proposed development. The existing municipal sanitary sewer system that services these parcels would have been designed based on commercial sewage loading of 50,000 l/Ha/d and infiltration allowance of 0.28 l/s/Ha, for the 0.21 Ha site would result in an average flow of 0.187 l/s.

Avg commercial flow: 50,000 l/Ha/d X 0.218 Ha = 10,900 l/d = 0.126 l/s

Infiltration allowance: 0.218 Ha X 0.28 l/s/Ha = 0.061 l/s,

Original avg. design flow, 0.126 + 0.061 = 0.187 l/s

The proposed mixed-use development includes residential and commercial uses. Based on the previously noted flow rates of 280 l/p/d for residential, 5 l/m²/d for retail, and 125 l/seat/d for the

restaurant portion, including an ICI peaking factor of 1.5, the average wastewater flow plus infiltration allowance calculates to 0.81 l/s, as noted below:

Avg pop flow: 201 (91 units @ 1.4ppu, 36 units @ 2.1ppu & 0 units @ 3.1ppu) X 280 l/p/d = 56,840 l/d = 0.66 l/s

Plus retail flow of 5 l/m²/d X 394m² X 1.5 = 2,955 l/d = 0.034 l/s,

and restaurant flow of 125 l/seat/d X 60 seats X 1.5 = 11,250 l/d = 0.130 l/s

avg flow = 0.66+0.034+0.130 = 0.824 l/s

Infiltration allowance: 0.218Ha X 0.33l/s/Ha = 0.072 l/s,

Rezoned avg flow, 0.824 + 0.072 = 0.896 l/s

The proposed redevelopment results in a theoretical increase in average flow to the downstream system of 0.896-0.187= 0.709 l/s. The sanitary service connection is to an existing 375mm dia sanitary sewer which discharges into the 1500 mm dia West Nepean Collector at the end of Tweedsmuir Ave, given the size of these sewers and that City Staff have not advised of any downstream issues, that there is ample available capacity to accommodate the proposed redevelopment. The proposed design meets City of Ottawa and MOE requirements. Existing services within the project site will be disconnected and abandoned per City of Ottawa Standards.

4 STORMWATER MANAGEMENT

4.1 Existing Conditions

During the Pre-consult with the City no infrastructure concerns were noted, and follow-up memo was provided by the City along with the preconsult meeting notes, the stormwater infrastructure comments are summarized below:

Available Infrastructure:

Tweedsmuir Avenue, Storm: 1200mm Conc (Install 2002)

Richmond Road, Storm: 375mm PVC (Install 2004)

Stormwater Management:

Coefficient (C) of runoff determined as per existing conditions but in no case more than 0.5

TC = To be calculated, minimum 10 minutes

Any storm events greater than 2 year, up to 100 year, and including 100-year storm event must be detained on site.

Foundation drains are to be independently connected to sewer main unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.

Roof drains are to be connected downstream of any incorporated ICD within the SWM system.

Stormwater management criteria (Quality Control):

Include a section in the SWM report concerning quality control requirements. It is the consultant's responsibility to check with the relevant Conservation Authority for quality control issues and include this information in the SWM report.

As noted above there is a 1200 mm diameter storm sewer along Tweedsmuir Ave and a 375mm diameter storm sewer along Richmond Road, both sewers service the existing buildings within the subject parcel. Existing services within the project site will be disconnected and abandoned per City of Ottawa Standards. A survey of the subject parcel was completed by Farley, Smith, Denis Surveying Ltd. and is included in **Appendix B** the survey illustrates the location of the existing storm sewers adjacent to the site.

4.2 Design Criteria

As noted in the preconsult memo the City of Ottawa requires the site to follow the following design criteria;

- Storm sewers designed to a 2 year level of service
- Site to be designed to limit the 100 year post development flow to a maximum of the 2 year rate with C=0.5.

The stormwater system was designed following the principles of dual drainage, making accommodations for both major and minor flow.

Some of the key criteria include the following:

- Design Storm 1:2 year return (Ottawa)
- Rational Method Sewer Sizing
- Initial Time of Concentration 10 minutes
- Runoff Coefficients
 - Landscaped Areas C = 0.25
 - Asphalt/Concrete C = 0.90
 - Roof C = 0.90
- Pipe Velocities 0.80 m/s to 6.0 m/s
- Minimum Pipe Size 250 mm diameter

4.3 Proposed Minor System

Using the above-noted criteria, the proposed storm service lateral was sized accordingly. The storm sewer design sheet and the associated storm sewer drainage area plan are included in **Appendix D**. The site servicing drawing in **Appendix E** illustrates the proposed building above grade outline, and where the underground structure extends beyond the above grade building. All decks and roof drains will be routed inside the building via the mechanical plumbing systems and directed to the building cistern located adjacent to the northeast corner of the building. Two landscaped areas will be serviced by rear yard catchbasins, these catchbasins will discharge into the building plumbing system and will be directed to the building cistern. The cistern will be equipped with duplex storm pumps to control the flow rate of the storm water runoff from the site directed to the municipal storm sewer system. The pumps will discharge to a storm sewer lateral which will also service as an outlet for the building foundation drain.

4.4 Stormwater Management

The subject site will be limited to a release rate established using the criteria described in section 4.2. This will be achieved through a duplex storm pump system set to discharge at the identified release rate. When rainfall events generate flows that are more than the site's allowable release rate excess volume will be stored within the cistern.

At certain locations within the site, the opportunity to capture runoff is limited due to grading constraints and building geometry. These locations are generally located at the perimeter of the site where it is necessary to tie into public boulevards and adjacent properties, and it is not always feasible to capture stormwater runoff. These "uncontrolled" areas total 0.016 hectares. The runoff from the remaining site will be collected and discharged into the cistern, sized to accommodate inflow during the 1:100-year event with no overflow leaving the site.

The restricted release rate for the 0.218 Ha site as noted previously is limited to the 2yr flow with C=0.5

$$Q_{\text{restricted}} = 2.78 \times C \times i_{2\text{yr}} \times A \quad \text{where:}$$

$$C = 0.5$$

$$i_{2yr} = \text{Intensity of 2-year storm event (mm/hr)}$$

$$= 732.951 \times (T_c + 6.119)^{0.810} = 78.61 \text{ mm/hr; where } T_c = 10 \text{ minutes}$$

$$Q_{\text{restricted}} = \mathbf{23.28 \text{ l/s}}$$

As noted above, a portion of the site will be left to discharge to the surrounding boulevards and roadways uncontrolled.

Based on a 1:100 year event, the flow from the three uncontrolled areas can be determined as:

$$Q_{\text{uncontrolled}} = \mathbf{2.78 \times C \times i_{100yr} \times A} \quad \text{where:}$$

$$C_{100} = \text{Average runoff coefficient of uncontrolled area} = 1.00$$

$$i_{100yr} = \text{Intensity of 100-year storm event (mm/hr)}$$

$$= 1735.688 \times (T_c + 6.014)^{0.820} = 178.56 \text{ mm/hr; where } T_c = 10 \text{ minutes}$$

$$A_{1+2+3} = \text{Uncontrolled Area} = 0.009 \text{ Ha} + 0.001 \text{ Ha} + 0.006 \text{ Ha} = 0.016 \text{ Ha}$$

Therefore, the uncontrolled release rate can be determined as:

$$Q_{\text{uncontrolled}} = \mathbf{7.94 \text{ L/s}}$$

The maximum allowable release rate from the remainder of the site can then be determined as:

$$Q_{\text{max allowable}} = Q_{\text{restricted}} - Q_{\text{uncontrolled}}$$

$$= 23.28 \text{ L/s} - 7.94 \text{ L/s} = \mathbf{15.34 \text{ L/s}}$$

As noted in the preconsult notes, any excess storm water runoff up to the 100-year event is to be stored on-site, in order to not surcharge the downstream municipal storm sewer system. For this site a building cistern will be used, no roof top or surface storage will be employed. A duplex storm pump will be designed to limit discharge from the tank to 15.00 L/s to meet the maximum allowable release rate to the storm sewer system. The Modified Rational Method was used to identify the required storage, the MRM spreadsheet in **Appendix D** identifies the required storage to accommodate the 1:100yr and 1:2yr events. The following table summarizes the on-site storage requirements during both the events.

AREA	TRIBUTARY AREA	AVAILABLE STORAGE (M ³)	100-YEAR STORM		2-YEAR STORM	
			RESTRICTED FLOW (L/S)	REQUIRED STORAGE (M ³)	RESTRICTED FLOW (L/S)	REQUIRED STORAGE (M ³)
Roof & Deck	0.201	70	15.00	65.49	15.00	14.55
Unrestricted	0.016		7.94		3.15	
TOTAL	0.218	70	22.94	65.49	18.15	14.55

In all instances the required storage is met with the building cistern.

As demonstrated above, the proposed site controls will restrict the 100-year storm event runoff from the site into the existing storm sewer system to 23.28 l/s. Restricted stormwater will be

contained onsite by the building cistern. Should a more extreme event occur, or should a roof inlet become blocked, scuppers will provide for overflow to the street. In the unlikely event the duplex pump system fails, or the storm service lateral is blocked, an emergency overflow from the building cistern to the street is provided.

5 SOURCE CONTROLS

5.1 General

The existing municipal storm sewer system collects and conveys storm runoff to the Ottawa river without any end of pipe quality treatment for captured stormwater. On site level or source control management of runoff will be provided. The proposed building configuration consists of a podium covering most of the site and no onsite surface parking or exposed drive lanes are proposed. Surface runoff will be collected and controlled by a cistern and duplex pump system. It is proposed to include a sump within the cistern. The sump will trap pollutants such as sand, grit and debris which can be mechanically removed prior to being flushed into the minor pipe system. The underground garage extends beyond the above grade building at the cistern location allowing for a maintenance hole exterior to the building to facilitate the use of a vacuum truck to clean out any debris/sediment from the tank as needed.

6 SEDIMENT AND EROSION CONTROL PLAN

6.1 General

During construction, existing conveyance systems can be exposed to significant sediment loadings. Although construction is only a temporary situation, it is proposed to introduce a number of mitigative construction techniques to reduce unnecessary construction sediment loadings. These include:

- Installation of filter cloths on open surface structures such as maintenance holes and catchbasins during building construction.
- Installation of silt fence on the site perimeter, where practical.

The Erosion and Sedimentation Control measures are detailed in drawing 135883-C-900 in **Appendix E**.

6.2 Trench Dewatering

Although little groundwater is expected during construction of municipal services, any trench dewatering using pumps will be discharged into a filter trap made up of geotextile filters and straw bales similar in design to the OPSD 219.240 Dewatering Trap. These will be constructed in a bowl shape with the fabric forming the bottom and the straw bales forming the sides. Any pumped groundwater will be filtered prior to release to the existing surface runoff. The contractor will inspect and maintain the filters as needed including sediment removal and disposal and material replacement as needed.

6.3 Surface Structure Filters

All catchbasins, and to a lesser degree manholes, convey surface water to sewers. However, until the surrounding surface has been completed these structures should be covered in some fashion to prevent sediment from entering the minor storm sewer system. Until landscaped areas are sodded or until drive isles and parking lots are asphalted and curbed, catchbasins and manholes will be constructed with geotextile filter bags or a geotextile filter fabric located between the structure frame and cover respectively. These will stay in place and be maintained during construction and build until it is appropriate to remove.

7 SOILS and GRADING

Paterson Group was retained to prepare a geotechnical investigation for the proposed development. The objectives of the investigation were to prepare a report to:

- Determine the subsoil and groundwater conditions at the site by means of test holes.
- To provide geotechnical recommendations pertaining to design of the proposed development including construction considerations.

The geotechnical report PG5946-1 rev 2, "Geotechnical Investigation 249, 255 Richmond Road and 372 Tweedsmuir Ave." dated September 24, 2021. A copy of the report has been included with the SPA application. The report contains recommendations for building construction and site services, which include but are not limited to the following for site servicing:

- At least 300 mm of OPSS Granular A should be used for pipe bedding for sewer and water pipes. The bedding should extend to the spring line of the pipe. Cover material, from the spring line to at least 300 mm above the obvert of the pipe, should consist of OPSS Granular A or Granular B Type II with a maximum size of 25 mm. The bedding and cover materials should be placed in maximum 225 mm thick lifts compacted to 99% of the material's standard Proctor maximum dry density.
- The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 99% of the material's SPMDD
- Long term groundwater level is expected to range between 5 to 7m below grade
- No Grade raise restrictions were noted.

MATERIAL	Layer Thickness
• Access Lanes	
• Asphalt Wearing Course (Superpave 12.5)	• 40 mm
• Asphalt Binder Course (Superpave 19.0)	• 50 mm
• Well Graded Granular Base Course (Granular 'A')	• 150 mm
• Well Graded Granular Sub-Base Course (Granular 'B' Type II)	• 300 mm

A topographic survey of the subject parcel was completed by Farley, Smith, Denis Surveying Ltd. and is included in **Appendix B** the survey illustrates spot elevations within and along the perimeter of the site. The topo survey reveals the high point on the site is where the property lines along Richmond Road and Tweedsmuir Ave. intersect. Along the Richmond Road frontage, the elevation drops 0.28m across the 61m frontage for an average slope of 0.46%, along Tweedsmuir Ave frontage, the elevation drops 2.05m across 57m frontage for an average slope of 3.6%. To take advantage of the drop in elevation along Tweedsmuir the ramp to the underground parking was located along that frontage. To deal with the elevation drop across Tweedsmuir Ave. and provide barrier free access at the various entrances the ground floor Finished Floor elevation drops from 67.47 for the retail entrances along Richmond Road, to 66.47 for the tower lobby, and 66.73 with steps for the north entrance. Detailed grading plan 135883-C-200 is provided in **Appendix E**.

8 Conclusions

Municipal water, wastewater and stormwater systems required to accommodate the proposed development are available to service the proposed development. Prior to construction, existing sewers are to be CCTV inspected to assess sewer condition.

This report has demonstrated sanitary and storm flows from and water supply to the subject site can be accommodated by the existing infrastructure. Also, the proposed servicing has been designed in accordance with MECP and City of Ottawa current level of service requirements.

The use of lot level controls, conveyance controls and end of pipe controls outlined in the report will result in effective treatment of surface stormwater runoff from the site. Adherence to the sediment and erosion control plan during construction will minimize harmful impacts on surface water.

Based on the information provided herein, the development can be serviced to meet City of Ottawa requirements.

Report prepared by:



Samantha Labadie, P.Eng.
Project Engineer

APPENDIX A

J:\135863_255_Richmond\7.0_Production\7.03_Design\04_Civil\Sheets\FIGURES\FIGURE-1-SITELLOCATION.dwg Layout Name: Layout1



Project Title
255 RICHMOND ROAD

Drawing Title
SITE LOCATION

Sheet No.
FIGURE 1

- | | | |
|----|--------------------------------|------------|
| 16 | Updated Site Plan | 2025-07-30 |
| 14 | Issued to Consultants | 2025-06-06 |
| 13 | Full Arch. Set | 2025-04-17 |
| 12 | Schematic Design | 2025-04-09 |
| 11 | Issued for Zoning Amendment | 2025-03-31 |
| 10 | Setback & Height Review | 2025-03-18 |
| 8 | Issued for Draft Review | 2025-02-12 |
| 7 | Issued for SPA | 2024-03-05 |
| 5 | Revised as per City Comments | 2023-08-28 |
| 3 | Re-issued for Zoning Amendment | 2023-04-14 |
| 2 | Re-issued for Zoning Amendment | 2023-04-04 |
| 1 | Issued for Zoning Amendment | 2023-04-03 |

REVISIONS

CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ANY OMISSIONS OR DISCREPANCIES TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.

	THIS DRAWING SHALL NOT BE USED FOR CONSTRUCTION PURPOSES UNTIL SIGNED BY THE ARCHITECT.
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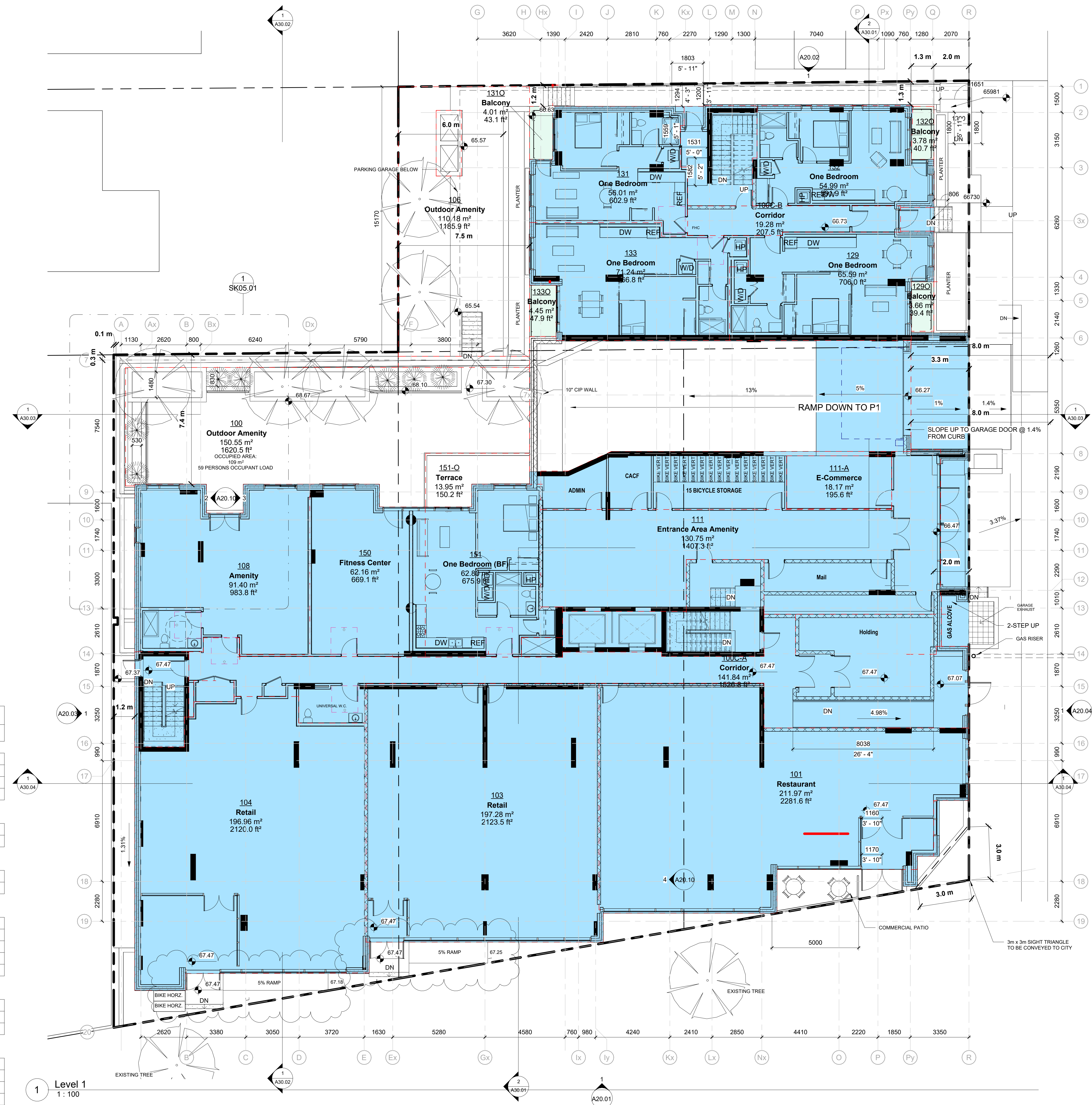
VINCENT P.
COLIZZA
ARCHITECT
INCORPORATED

249, 255 Richmond Road, 372
Tweedsmuir Avenue
Ottawa, ON

DWG. TITLE

Level 01 Floor Plan

SCALE	As indicated	DWG. NO.
PROJ. NO.	2219	A10.03



1 Level 1
1 : 100

Figure 1 consists of three diagrams labeled I, II, and III, each showing a door with a width and a height of 5.2 m. Diagram I shows a standard door with a width of 2.6 m and a triangle symbol inside. Diagram II shows a door with a width of 3.4 m and a wheelchair symbol inside. Diagram III shows a door with a width of 2.4 m and a wheelchair symbol inside, with a hatched pattern on the right side.

***NOTE: TYPICAL CONFORMING STALLS HIGHLIGHTED IN YELLOW ON PLANS. REMAINING STALLS ARE NON-CONFORMING.**

Level 1 Area Summary					
Level	Name	Number	Area Designation	Area (Sqm)	Area (Sqft)
Level 1	Amenity	108	Amenity	91.40 m²	983.78 ft²
Level 1	E-Commerce	111-A	Amenity	18.17 m²	195.63 ft²
Level 1	Entrance Area Amenity	111	Amenity	130.75 m²	1407.34 ft²
Level 1	Fitness Center	150	Amenity	62.16 m²	669.07 ft²
Amenity: 4				302.48 m²	3255.83 ft²
Level 1	Corridor	100C-A	Corridors	141.84 m²	1526.76 ft²
Level 1	Corridor	100C-B	Corridors	19.28 m²	207.55 ft²
Corridors: 2				161.12 m²	1734.31 ft²
Level 1	Outdoor Amenity	100	Outdoor Amenity	150.55 m²	1620.48 ft²
Level 1	Outdoor Amenity	106	Outdoor Amenity	110.18 m²	1185.94 ft²
Outdoor Amenity: 2				260.72 m²	2806.42 ft²
Level 1	Balcony	129O	Outdoor Private Amenity	3.66 m²	39.37 ft²
Level 1	Balcony	131O	Outdoor Private Amenity	4.01 m²	43.13 ft²
Level 1	Balcony	132O	Outdoor Private Amenity	3.78 m²	40.73 ft²
Level 1	Balcony	133O	Outdoor Private Amenity	4.45 m²	47.93 ft²
Level 1	Terrace	151-O	Outdoor Private Amenity	13.95 m²	150.20 ft²
Outdoor Private Amenity: 5				29.86 m²	321.36 ft²
Level 1	Restaurant	101	Retail	211.97 m²	2281.58 ft²
Level 1	Retail	103	Retail	197.28 m²	2123.48 ft²
Level 1	Retail	104	Retail	196.96 m²	2120.04 ft²
Retail: 3				606.20 m²	6525.10 ft²
Level 1	One Bedroom	129	Unit	65.59 m²	706.00 ft²
Level 1	One Bedroom	131	Unit	56.01 m²	602.90 ft²
Level 1	One Bedroom	132	Unit	54.99 m²	591.87 ft²
Level 1	One Bedroom	133	Unit	71.24 m²	766.80 ft²
Level 1	One Bedroom (BF)	151	Unit	62.80 m²	675.94 ft²
Unit: 5				310.62 m²	3343.50 ft²

255 Richmond Road – Infrastructure Notes

Available Infrastructure:

Tweedsmuir Avenue:

Sanitary: 375mm PVC (Install 2002)

Storm: 1200mm Conc (Install 2002)

Water: 150mm PVC (Install 2001)

Richmond Road

Storm: 375mm PVC (Install 2004)

Water: 300mm PVC (Install 2004)

Water Boundary Conditions:

Will be provided at request of consultant. Requests must include the location of the service and the expected loads required by the proposed development. Please provide the following and submit Fire Flow Calculation Sheet per FUS method with the request:

- Location of service
- Type of development and amount of required fire flow (per FUS method – include FUS calculation sheet with request)
- Average Daily Demand (l/s)
- Maximum Hourly Demand (l/s)
- Maximum Daily Demand (l/s)
- Water Supply Redundancy – Fire Flow:
Applicant to ensure that a second service with an inline valve chamber be provided where the average daily demand exceeds 50 m³ / day (0.5787 l/s per day)

Water services larger than 19 mm require a Water Data Card. Please complete card and submit.

Stormwater Management:

- Coefficient (C) of runoff determined **as per existing conditions** but in no case more than 0.5
- TC = To be calculated, minimum 10 minutes
- Any storm events greater than 2 year, up to 100 year, and including 100-year storm event must be detained on site.
- Foundation drains are to be independently connected to sewer main unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.
- Roof drains are to be connected downstream of any incorporated ICD within the SWM system.

Stormwater management criteria (Quality Control)

Include a section in the SWM report concerning quality control requirements. It is the consultant's responsibility to check with the relevant Conservation Authority for quality control issues and include this information in the SWM report.

Noise Study:

- Noise study required – property fronts an Arterial Road (Richmond Road).

Phase I and Phase II ESA:

- Phase I ESA is required; Phase II ESA may be required depending on the results of the Phase I ESA. Phase I ESA must include an EcoLog ERIS Report.
- Phase I ESA and Phase II ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Required Studies

- Servicing and Stormwater Management Report
- Geotechnical Study
- Phase I ESA
- Phase II ESA (depends on outcome of Phase I)
- Noise Study

Required Plans

- Site Servicing Plan
- Grade Control and Drainage Plan
- Erosion and Sediment Control Plan (Can be combined with grading plan)

Relevant information

1. The Servicing Study Guidelines for Development Applications are available at the following address: <https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#servicing-study-guidelines-development-applications>
2. Servicing and site works shall be in accordance with the following documents:
 - ⇒ Ottawa Sewer Design Guidelines (October 2012)
 - ⇒ Ottawa Design Guidelines – Water Distribution (2010)
 - ⇒ Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
 - ⇒ City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
 - ⇒ City of Ottawa Environmental Noise Control Guidelines (January, 2016)
 - ⇒ City of Ottawa Park and Pathway Development Manual (2012)
 - ⇒ City of Ottawa Accessibility Design Standards (2012)
 - ⇒ Ottawa Standard Tender Documents (latest version)
 - ⇒ Ontario Provincial Standards for Roads & Public Works (2013)

3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at InformationCentre@ottawa.ca or by phone at (613) 580-2424 x.44455).
4. Any proposed work in utility easements requires written consent of easement owner.

APPLICANT'S STUDY AND PLAN IDENTIFICATION LIST

Legend: **S** indicates that the study or plan is required with application submission.
A indicates that the study or plan may be required to satisfy a condition of approval/draft approval.

For information and guidance on preparing required studies and plans refer to:

<http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans>

S/A	Number of copies	ENGINEERING		S/A	Number of copies
S		1. Site Servicing Plan	2. Site Servicing Study / Brief	S	
S		3. Grade Control and Drainage Plan	4. Geotechnical Study / Slope Stability Study	S	
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
S		9. Transportation Impact Study / Brief	10.Erosion and Sediment Control Plan / Brief	S	
S		11.Storm water Management Report / Brief	12.Hydro geological and Terrain Analysis		
		13.Hydraulic Water main Analysis	14.Noise / Vibration Study (If on-site stationary noise source)	S	
		15.Roadway Modification Design Plan	16.Confederation Line Proximity Study	S	

S/A	Number of copies	PLANNING / DESIGN / SURVEY		S/A	Number of copies
		17.Draft Plan of Subdivision	18.Plan Showing Layout of Parking Garage		
		19.Draft Plan of Condominium	20.Planning Rationale	S	
S		21.Site Plan	22.Minimum Distance Separation (MDS)		
		23.Concept Plan Showing Proposed Land Uses and Landscaping	24.Agrology and Soil Capability Study		
		25.Concept Plan Showing Ultimate Use of Land	26.Cultural Heritage Impact Statement		
S		27.Landscape Plan (showing existing trees)	28.Archaeological Resource Assessment Requirements: S (site plan) A (subdivision, condo)		
S		29.Survey Plan	30.Shadow Analysis	S	
S		31.Architectural Building Elevation Drawings (dimensioned)	32.Design Brief (includes the Design Review Panel Submission Requirements)	S	
S		33.Wind Analysis			

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
S	2	34.Phase 1 Environmental Site Assessment	35.Impact Assessment of Adjacent Waste Disposal/Former Landfill Site		
		36.Phase 2 Environmental Site Assessment (depends on the outcome of Phase 1)	37.Assessment of Landform Features		
		38.Record of Site Condition	39.Mineral Resource Impact Assessment		
		40.Tree Conservation Report	41.Environmental Impact Statement / Impact Assessment of Endangered Species		
		42.Mine Hazard Study / Abandoned Pit or Quarry Study			

S/A	Number of copies	ADDITIONAL REQUIREMENTS		S/A	Number of copies
		43.	44.		

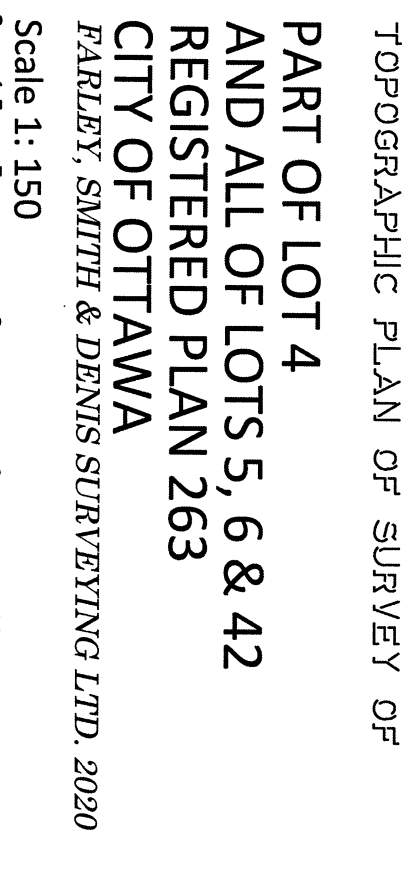
Meeting Date: June 14, 2021
 File Lead (Assigned Planner): Steve Gauthier
 Site Address (Municipal Address): 255 Richmond Road,
 249 Richmond Road & 372 Tweedsmuir Avenue

Application Type: Zoning By-law Amendment and Site Plan Control
 Infrastructure Approvals Project Manager: Mohammed Fawzi
 *Preliminary Assessment: 1 ☐ 2 ☐ 3 ☐ 4 ☐ 5 ☐

*One (1) indicates that considerable major revisions are required before a planning application is submitted, while five (5) suggests that proposal appears to meet the City's key land use policies and guidelines. **This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.**

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, the Planning and Growth Management Department will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the Planning and Growth Management Department.

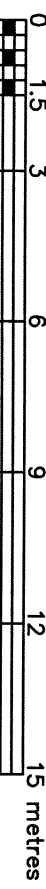
APPENDIX B



**PART OF LOT 4
AND ALL OF LOTS 5, 6 & 42
REGISTERED PLAN 263
CITY OF OTTAWA**

FARLEY, SMITH & DENIS SURVEYING LTD. 2020

Scale 1: 150



Metric Note

Distances and coordinates on this plan are in metres and can be converted to feet by dividing by 0.3048.

Distance Not

Distances shown on this plan are ground distances and can be converted to grid distances by multiplying by the combined scale factor of 0.99996.

Bearing Note

bearings are given, are referred to the eastern limit of Athlone Avenue having a bearing of N 22° 48' 00" W as shown on Plan 4R-29565 and are referred to the Central Meridian of MTM Zone 9 (76°30' West Longitude) Nad-83 (Original).

Elevation Notes

1. Elevations shown are geodetic and are referred to Geodetic Datum CGVD-19:1978. (FSD File 246-16)
2. It is the responsibility of the user of this information to verify that the job benchmark has not been altered or disturbed and that it's relative elevation description agrees with the information shown on this drawing.

Utility Notes

1. This drawing cannot be accepted as acknowledgment of the utilities and it is not the responsibility of the user to contact the respective utility authorities for confirmation.
2. Only visible surface utilities were located.
3. Underground utility data derived from City of Ottawa utility sheet reference: E-04-15, 3378&K&O2, 1383559&S&P12.
4. Sanitary and storm sewer grades and inverts were compiled from: Field measurement and City of Ottawa plans.
5. A field location of underground plant by the pertinent utility authority is mandatory before any work involving breaking ground, probing, excavating or

Notes & Legend

Den

- | | |
|--------|---|
| ■ | Survey Monument Found |
| ■ SB | Standard Iron Bar |
| ■ SSB | Short Standard Iron Bar |
| ■ IB | Iron Bar |
| ■ CB | Cut Cross |
| ■ W | Wormhole |
| ■ W/H | Wormhole |
| ■ Meas | Measured |
| (P1) | Registered Plan 263 |
| (P2) | Plan SP-102/74 |
| (P3) | Plan 4P-15886 |
| (P4) | Plan 4P-25965 |
| (P5) | Plan by 1857 dated December 10, 1991 (Ref. No. 21-2625) |
| (P6) | Plan by 16502 dated January 3, 2019 (File No. 272-18) |
| (P7) | Plan by 16502 dated August 27, 2014 (File No. 160-14) |
| (P8) | Maintenance Hole (Sutary) |
| ○ M+ST | Maintenance Hole (Sutary) |
| ○ M+H | Maintenance Hole (Bell) |
| ○ M+V | Maintenance Hole (Vegetation) |
| ○ M+D | Maintenance Hole (Ditch) |
| ○ M+L | Maintenance Hole (Landscape) |
| ○ M+T | Maintenance Hole (Tree) |
| ○ M+R | Maintenance Hole (Road) |
| ○ M+O | Maintenance Hole (Other) |
| ○ M+U | Maintenance Hole (Unknown) |



Deciduous Tree

Coniferous Tree

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ASSOCIATION OF ONTARIO
LAND SURVEYORS
ONLINE SURVEILLANCE

1. This survey and plan are correct and in accordance with the Surveys Act, the Surveyors Act and the

2. The survey was completed on the 3rd day

Date _____ Daniel Robinson _____

In accordance with Regulation 1026, Section 2

HARLEY, SMITH & DENIS SURVEYING LTD

ONTARIO LAND SURVEYORS
CANADA LAND SURVEYORS

190 COLONNADE ROAD, OTTAWA, ONTARIO K2E 7J5

505 Richmond Rd 373 Tuolumne Ave 146 4 E and 13 0033 T C 400

Samantha Labadie

From: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>
Sent: Thursday, October 7, 2021 1:41 PM
To: Samantha Labadie
Subject: RE: 255 Richmond - Boundary Conditions Request
Attachments: 255 Richmond Road October 2021.pdf

Hi Samantha,

The following are boundary conditions, HGL, for hydraulic analysis at 255 Richmond Road (zone 1W) assumed to be connected to the 305 mm watermain on Richmond Road and the 152 mm on Tweedsmuir Street (see attached PDF for location).

Both Connections

Minimum HGL: 108.7 m

Maximum HGL: 114.9 m

Max Day + FF (133.3 L/s): 110.0 m (Richmond) 104.4 m (Tweedsmuir)

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Best Regards,

Mohammed Fawzi, E.I.T.

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - Central Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

****Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me****

From: Fawzi, Mohammed

Sent: October 04, 2021 11:25 AM

To: Samantha Labadie <samantha.labadie@ibigroup.com>

Subject: RE: 255 Richmond - Boundary Conditions Request

Hi Samantha,

This is to confirm your request has been received. I will get back to you as early as possible.

Thank you.

Best Regards,

Mohammed Fawzi, E.I.T.

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - Central Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

****Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me****

From: Samantha Labadie <samantha.labadie@ibigroup.com>

Sent: October 04, 2021 11:12 AM

To: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>

Subject: 255 Richmond - Boundary Conditions Request

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi Mohammed,

We are requesting for boundary conditions for the above noted site. It is a 9-Storey Residential building with some commercial space. The proposed building will have two connections, one on Tweedsmuir Ave and one on Richmond Road, as shown in the attached site plan. Demand and fire flows can be found on the attached and are summarized below,

Fire Flow per FUS = 8000 L/min

Average Daily Demand = 0.74 L/s

Maximum Daily Demands = 1.64 L/s

Maximum Hourly Demand = 3.48 L/s

Thank you,

Sam Labadie P.ENG

Civil Engineer

IBI GROUP

Suite 400, 333 Preston Street

Ottawa ON K1S 5N4 Canada

tel +1 613 225 1311 ext 64059 cell +1 613 899 5717



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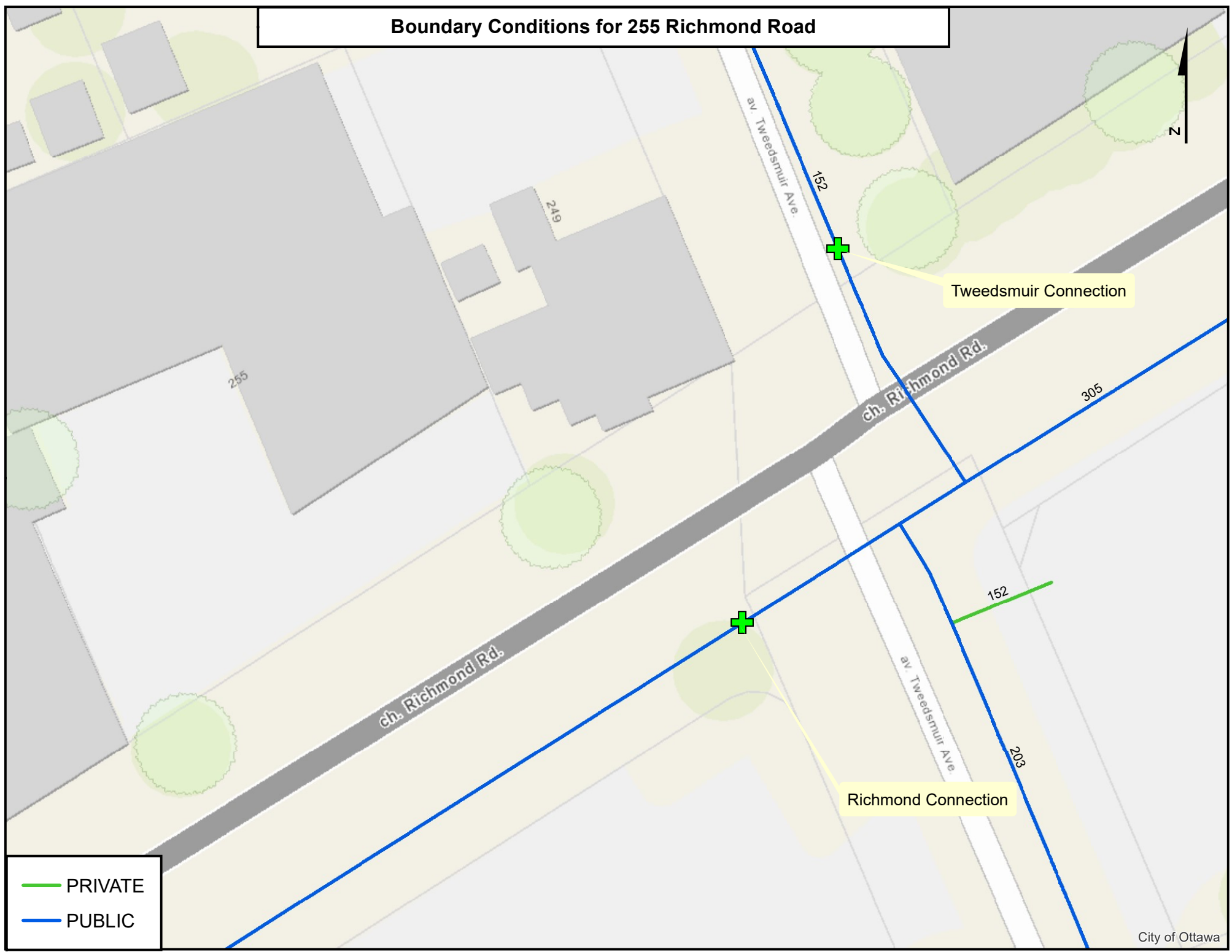
,

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,

Boundary Conditions for 255 Richmond Road



- PRIVATE
- PUBLIC

Fire Flow Requirement from Fire Underwriters Survey

Building - 9 Storey Residential

Building Floor Area (for Type II Noncombustible Construction)
(2 largest adjoining floors plus 50% of floors above up to eight)

Floors 1-2	2,980 m ²
50% Floors 3-8	<u>4,005</u>
Total	6,985 m ²

Fire Flow

$$F = 220C\sqrt{A}$$

C	0.8	C =	1.5 wood frame
A	6,985 m ²		1.0 ordinary
			0.8 non-combustile
F	14,709 l/min		0.6 fire-resistive
Use	15,000 l/min		

Occupancy Adjustment

		-25% non-combustile
		-15% limited combustile
Use	0%	0% combustile
		+15% free burning
Adjustment	0 l/min	+25% rapid burning
Fire flow	15,000 l/min	

Sprinkler Adjustment

		-30% system conforming to NFPA 13
		-50% complete automatic system
Use	-50%	
Adjustment	-7500 l/min	

Exposure Adjustment

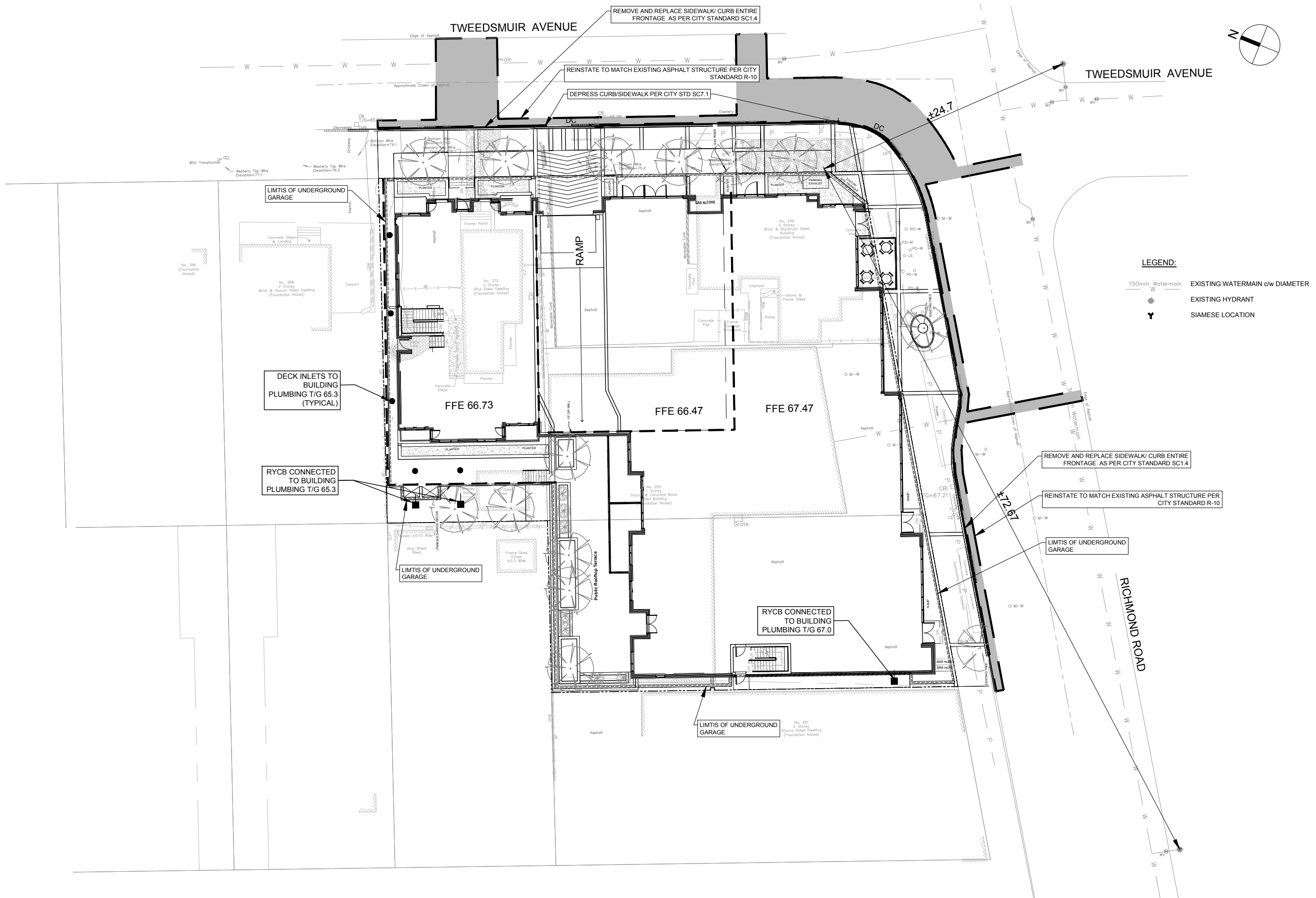
Building Face	Separation (m)	Adjacent Exposed Wall			Exposure Charge *
		Length	Stories	L*H Factor	
north	5.5	9	2	18	0%
east	19	21	1	21	4%
south	31	11	1	11	0%
west	1.3	30	1	30	1%

Total 5%

Adjustment 750 l/min

Required Fire Flow

Total adjustments	<u>(6,750) l/min</u>
Fire flow	8,250 l/min
Use	8,000 l/min
	133.3 l/s



Scale

Project Title

Drawing Title

Sheet No.



N.T.S.

255 RICHMOND ROAD

HYDRANT COVERAGE

Figure 2.2

Samantha Labadie

From: Mark Sarasin <marks@gwal.com>
Sent: Friday, March 31, 2023 11:57 AM
To: Samantha Labadie
Cc: Victor Menasce; Matthew Maxsom; Lisa Dalla Rosa; Vincent P. Colizza; Demetrius Yannoulopoulos
Subject: RE: 255 Richmond (YStreet) - Update to Plans/Studies

*** Exercise caution. This is an EXTERNAL email. DO NOT open attachments or click links from unknown senders or unexpected email. ***

Yes it is, to NFPA 13 and fully supervised

Mark Sarasin P.Eng | Senior Associate, Mechanical Engineer
GOODKEY, WEEDMARK & ASSOCIATES LTD.

Email: marks@gwal.com

Office: (613) 727-5111 ext. 308 Mobile: (613) 816-0844

Address: 1688 Woodward Drive | Ottawa, Ontario | K2C 3R8

Website: www.gwal.com



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From: Samantha Labadie <samantha.labadie@ibigroup.com>
Sent: Friday, March 31, 2023 11:53 AM
To: Mark Sarasin <marks@gwal.com>
Cc: Victor Menasce <victorm@ystreetcapital.com>; Matthew Maxsom <mattm@ystreetcapital.com>; Lisa Dalla Rosa <dallarosa@fotenn.com>; Vincent P. Colizza <vcolizza@colizzaarchitects.com>; Demetrius Yannoulopoulos <dyannoulopoulos@IBIGroup.com>
Subject: RE: 255 Richmond (YStreet) - Update to Plans/Studies

Thanks Mark, and can you indicate on page 4? Is the water supply standard for both system and fire dept hose lines, and is it a fully supervised system?

Sam

From: Mark Sarasin <marks@gwal.com>
Sent: Friday, March 31, 2023 11:48 AM
To: Samantha Labadie <samantha.labadie@ibigroup.com>
Cc: Victor Menasce <victorm@ystreetcapital.com>; Matthew Maxsom <mattm@ystreetcapital.com>; Lisa Dalla Rosa <dallarosa@fotenn.com>; Vincent P. Colizza <vcolizza@colizzaarchitects.com>; Demetrius Yannoulopoulos

<dyannouloupoulos@IBIGroup.com>

Subject: RE: 255 Richmond (YStreet) - Update to Plans/Studies

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All systems will be protected

C = 0.8 ☐	Type II Noncombustible Construction A building is considered to be of Noncombustible construction (Type II) when all structural elements, walls, arches, floors, and roofs are constructed with a minimum 1-hour fire resistance rating and are constructed with noncombustible materials. Total Effective Area (A) = ☐ if any vertical openings in the building (ex. interconnected floor spaces, atria, elevators, escalators, etc.) are unprotected**, consider the two largest adjoining floor areas plus 50% of all floors immediately above them up to a maximum of eight; or ☐ if all vertical openings and exterior vertical communications are properly protected* in accordance with the National Building Code, consider only the single largest Floor Area plus 25% of each of the two immediately adjoining floors.
----------------------------------	---

Mark Sarasin P.Eng | Senior Associate, Mechanical Engineer

GOODKEY, WEEDMARK & ASSOCIATES LTD.

Email: marks@gwal.com

Office: (613) 727-5111 ext. 308 Mobile: (613) 816-0844

Address: 1688 Woodward Drive | Ottawa, Ontario | K2C 3R8

Website: www.gwal.com



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From: Samantha Labadie <samantha.labadie@ibigroup.com>

Sent: Friday, March 31, 2023 11:27 AM

To: Mark Sarasin <marks@gwal.com>

Cc: Victor Menasce <victorm@ystreetcapital.com>; Matthew Maxsom <mattm@ystreetcapital.com>; Lisa Dalla Rosa <dallarosa@fotenn.com>; Vincent P. Colizza <vcolizza@colizzaarchitects.com>; Demetrius Yannouloupoulos <dyannouloupoulos@IBIGroup.com>

Subject: RE: 255 Richmond (YStreet) - Update to Plans/Studies

Hi Mark,

Are you able to mark which checkboxes the sprinkler system fulfills on the attached?

APPENDIX C

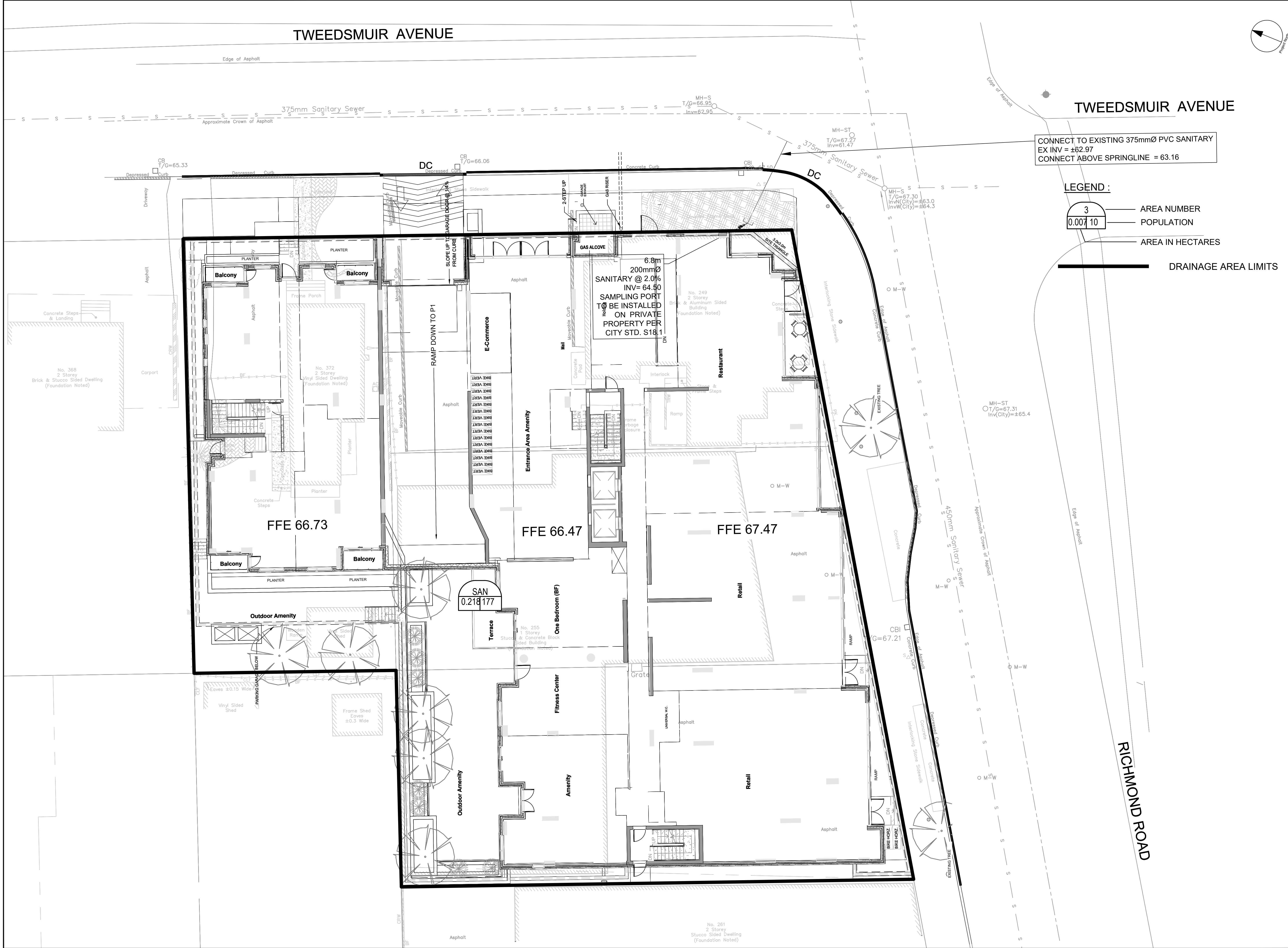


IBI GROUP
400-333 Preston Street
Ottawa, Ontario K1S 5N4 Canada
tel 613 225 1311 fax 613 225 9868
ibigroup.com

SANITARY SEWER DESIGN SHEET

255 Richmond
CITY OF OTTAWA
Y Street Capital

LOCATION				RESIDENTIAL								ICI AREAS								INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN							
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES			AREA w/o Units (Ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		RETAIL (m2)		REST. (\$)		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND		CUM	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					1BD	2BD	3BD		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	L/s
	Proposed Sanitary			0.218	91	36	0		203	203	4.00	2.63	0.00	0.00	394	394	60	60	1.50	0.16	0.218	0.218	0.07	0.00	0.00	2.87	87.74	6.80	250	2.00	1.731	84.87	96.73%
Design Parameters:				Notes: 1. Mannings coefficient (n) = 0.013 2. Demand (per capita): 280 L/day 3. Infiltration allowance: 0.33 L/s/Ha 4. Residential Peaking Factor: Harmon Formula = 1+(14/(4+(P/1000)^0.5))0.8 where K = 0.8 Correction Factor 5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0								Designed:		S.E.L.		No.		Revision										Date					
Residential												ICI Areas				Checked:		D.G.Y.				1.		Adequacy of Public Services Report									
1BD 1.4 p/p/u																																	
2BD 2.1 p/p/u				INST 28,000 L/Ha/day																													
3BD 3.1 p/p/u				RETAIL 5 L/m2/day																													
Other 60 p/p/Ha				REST. 125 L/seat/day				MOE Chart																									
												Dwg. Reference:		C-001																			



CLIENT

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ISSUES

No.	DESCRIPTION	DATE
1	ISSUED FOR SPA	2022-10-04
2	REVISED AS PER CITY COMMENTS	2023-04-03
3	REVISED AS PER CITY COMMENTS	2023-06-16
4	REVISED PER NEW SITE PLAN	2023-09-13
5	REVISED FOR SPA	2025-04-22
6	REVISED PER NEW SITE PLAN	2025-07-16
7		
8		

KEY PLAN

CONSULTANTS

SEAL

PROJECT

255 RICHMOND

PROJECT NO:

135883

DRAWN BY:

SPG

CHECKED BY:

S.E.L.

PROJECT MGR:

D.G.Y.

APPROVED BY:

D.G.Y.

SHEET TITLE

SANITARY DRAINAGE AREA PLAN

SHEET NUMBER

C-400

ISSUE

6

APPENDIX D



255 Richmond
CITY OF OTTAWA
Y Street Capital

https://arcadiso365.sharepoint.com/sites/Projects1/135883/Internal Documents/6.0_Technical/6.04_Civil/04_Design-Analysis/Design Brief Submission 3_2025-03/CCS_storm_2025-03 2025-03-14 4:32 PM



IBI GROUP
400-333 Preston Street
Ottawa, Ontario K1S 5N4 Canada
tel 613 225 1311 fax 613 225 9868
ibigroup.com

PROJECT: 255 Richmond
DATE: 2025-07-16
FILE: 135883 6.04.04
REV #: 4
DESIGNED BY: SEL
CHECKED BY: D.Y.

STORMWATER MANAGEMENT

Formulas and Descriptions

i_{2yr} = 1:2 year Intensity = $732.951 / (T_c+6.199)^{0.810}$
 i_{100yr} = 1:100 year Intensity = $1735.688 / (T_c+6.014)^{0.820}$
 T_c = Time of Concentration (min)
 C = Average Runoff Coefficient
 A = Area (Ha)
 Q = Flow = $2.78CiA$ (L/s)

Maximum Allowable Release Rate

Restricted Flowrate (based on $C=0.50$ $T_c=10min$)

C = 0.5
 T_c = 10 min
 i_{2yr} = 76.81 mm/hr
 A_{site} = 0.218 Ha

$Q_{restricted}$ = 23.28 L/s

Uncontrolled Release ($Q_{uncontrolled} = 2.78 \times C \times i_{100yr} \times A_{uncontrolled}$)

C = 1.00
 T_c = 10 min
 i_{100yr} = 178.56 mm/hr
 $A_{uncontrolled}$ = 0.016 Ha

Q_{uncon} = 7.94 L/s

Maximum Allowable Release Rate ($Q_{max\ allowable} = Q_{restricted} - Q_{uncontrolled}$)

$Q_{max\ allowable}$ = 15.34 L/s

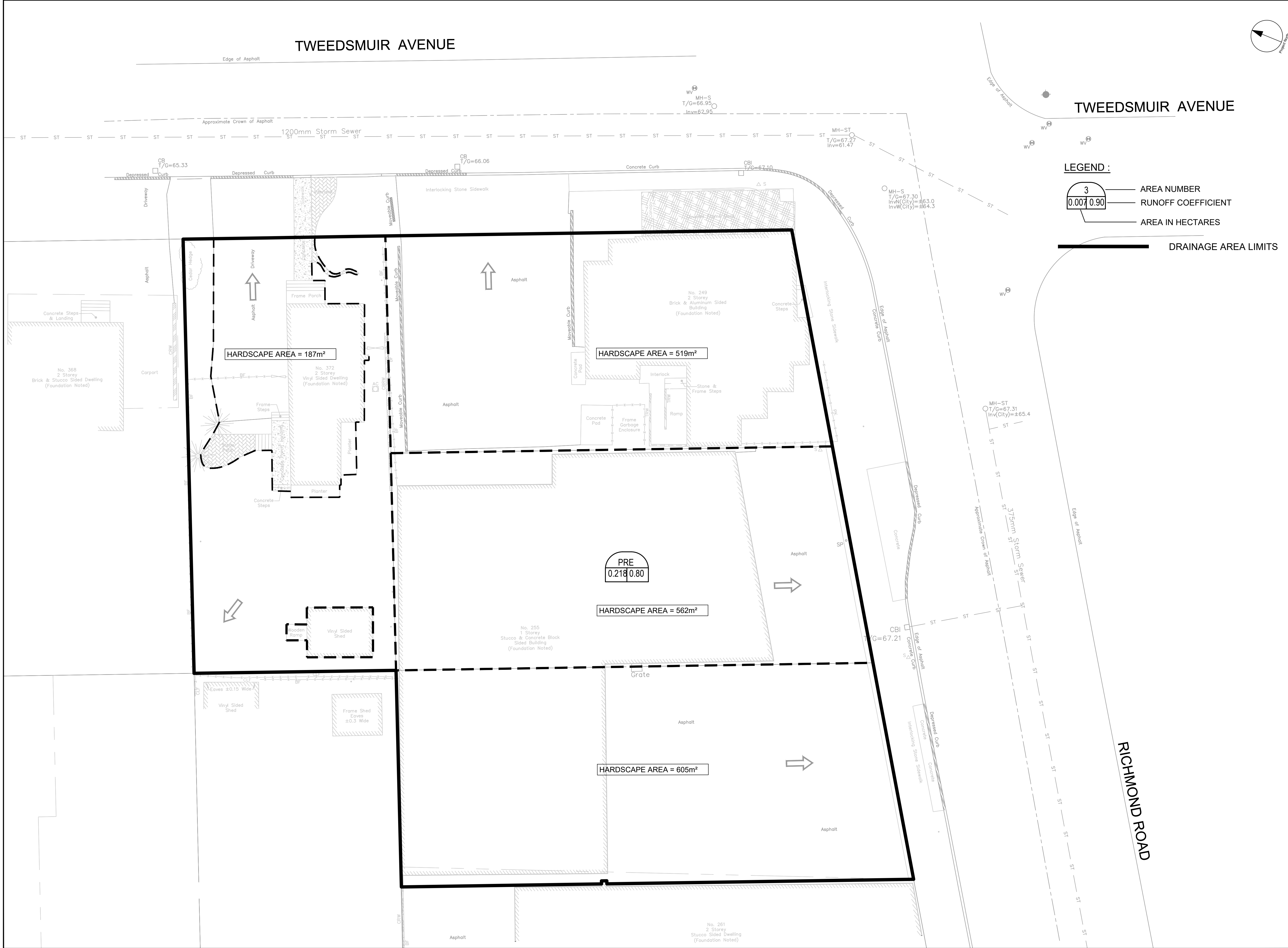
MODIFIED RATIONAL METHOD (100-Year, & 2-Year Ponding)

Drainage Area		Roof & Decks			
Area (Ha)	0.201				
C =	1.00	Restricted Flow Q _r (L/s)= 15.00			
100-Year Ponding					
T _c Variable (min)	i _{100yr} (mm/hour)	Peak Flow Q _p = 2.78xCi _{100yr} A (L/s)	Q _r (L/s)	Q _p - Q _r (L/s)	Volume 100yr (m ³)
29	94.01	52.53	15.00	37.53	65.308
31	89.83	50.19	15.00	35.19	65.461
32	87.89	49.11	15.00	34.11	65.489
33	86.03	48.07	15.00	33.07	65.487
35	82.58	46.14	15.00	31.14	65.401

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	65.49		70.00	0.00

Drainage Area		Roof & Decks			
Area (Ha)		0.201			
C =		0.90		Restricted Flow Q_r (L/s)=	15.00
2-Year Ponding					
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78 C i_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)
10	76.81	38.63	15.00	23.63	14.18
12	69.89	35.15	15.00	20.15	14.51
13	66.93	33.66	15.00	18.66	14.55
14	64.23	32.30	15.00	17.30	14.53
16	59.50	29.92	15.00	14.92	14.33

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	14.55		70.00	0.00



CLIENT

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ISSUES

No.	DESCRIPTION	DATE
1	ISSUED FOR SPA	2022-10-04
2	REVISED AS PER CITY COMMENTS	2023-04-03
3	REVISED AS PER CITY COMMENTS	2023-06-16
4	REVISED PER NEW SITE PLAN	2023-09-13
5	REVISED FOR SPA	2025-03-14
6		
7		
8		

KEY PLAN

CONSULTANTS

PROJECT

255 RICHMOND

PROJECT NO:

135883

DRAWN BY:

SPG

CHECKED BY:

S.E.L

PROJECT MGR:

D.G.Y

APPROVED BY:

D.G.Y

SHEET TITLE

PRE-DEVELOPMENT STORM DRAINAGE AREA PLAN

SHEET NUMBER

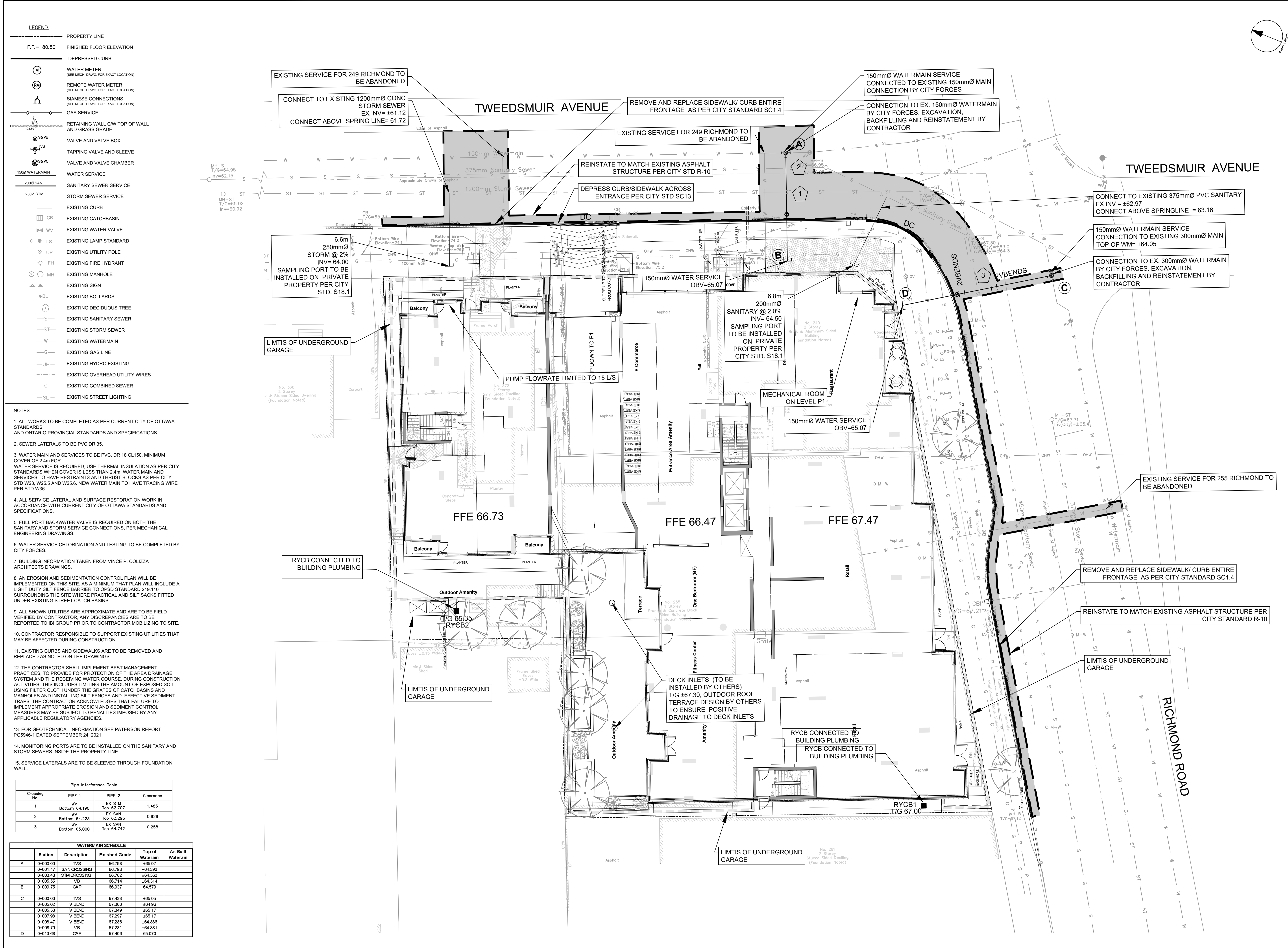
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ISSUE

5

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Last Saved: March 13, 2025, by sumad3150
Plotted: March 13, 2025, 3:56:34 PM by Summa, Don

APPENDIX E



CLIENT

STREET CAPITAL INC.

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ISSUES

No.	DESCRIPTION	DATE
1	ISSUED FOR SPA	2022-10-04
2	REVISED AS PER CITY COMMENTS	2023-04-03
3	REVISED AS PER CITY COMMENTS	2023-06-16
4	REVISED PER NEW SITE PLAN	2023-09-13
5	REVISED FOR SPA	2025-04-22
6	REVISED PER NEW SITE PLAN	2025-07-16
7		
8		

KEY PLAN

CONSULTANTS

SCALE

1:150

0 1.5 3 4.5 7.5m

SEAL

LICENCED PROFESSIONAL ENGINEER
S. E. LABADIE
100214983
2025/07/16
PROVINCE OF ONTARIO

PROJECT

255 RICHMOND

PROJECT NO: 135883

DRAWN BY: SPG

CHECKED BY: S.E.L

PROJECT MGR: D.G.Y

APPROVED BY: D.G.Y

SHEET TITLE

GENERAL PLAN OF SERVICES

SHEET NUMBER

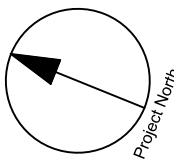
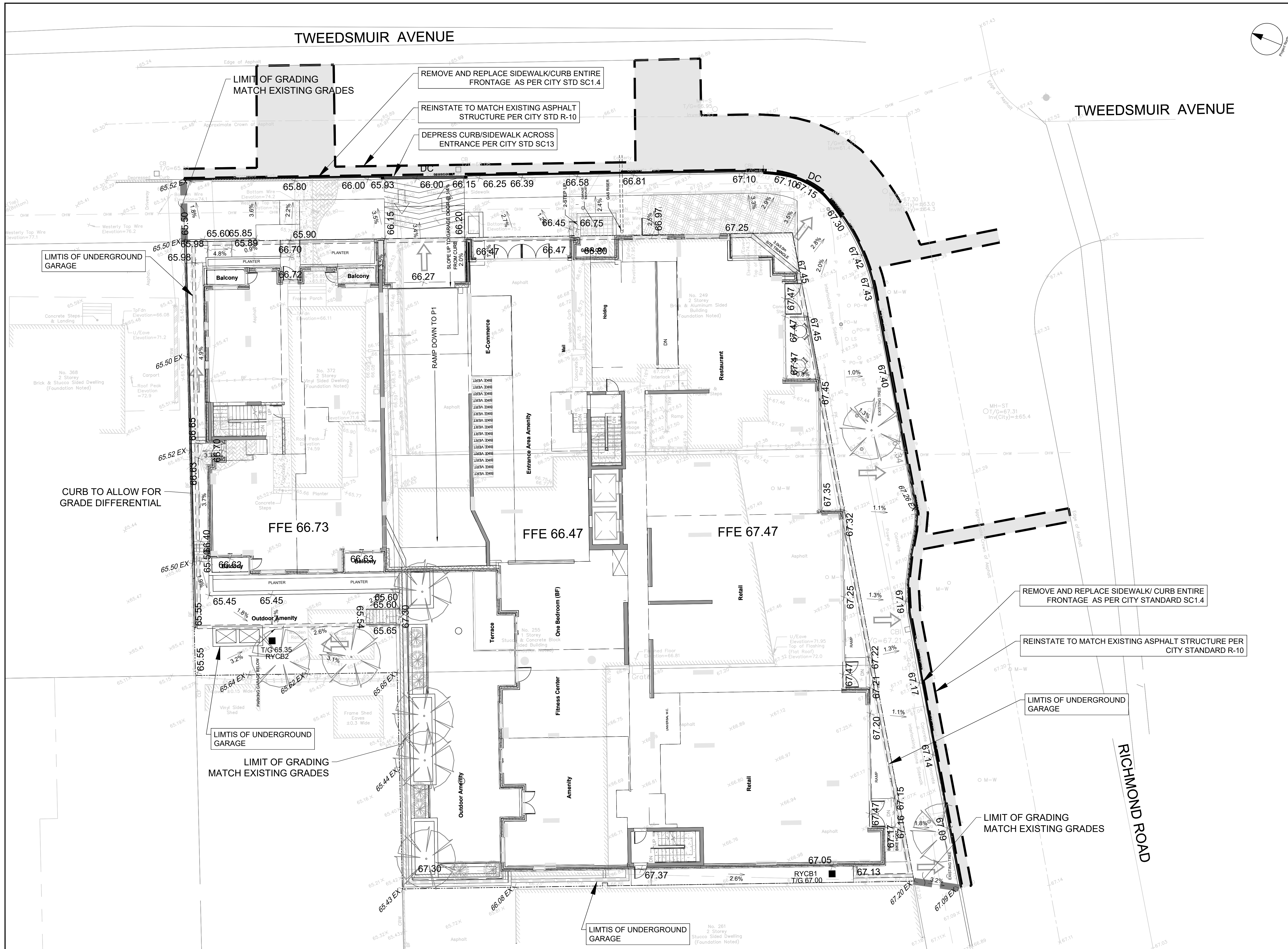
C-001

ISSUE

6

CITY FILE No. D07-12-21-0179

CITY PLAN No. 18619



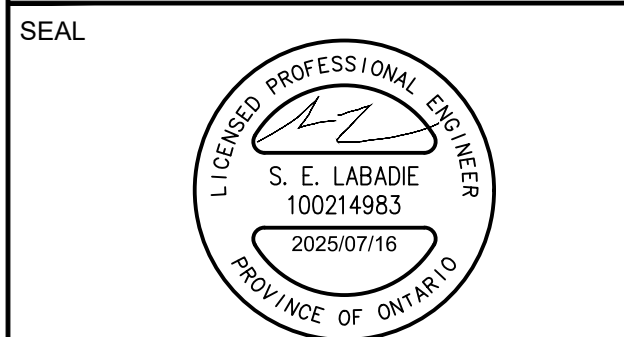
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ISSUES		
No.	DESCRIPTION	DATE
1	ISSUED FOR SPA	2022-10-04
2	REVISED AS PER CITY COMMENTS	2023-04-03
3	REVISED AS PER CITY COMMENTS	2023-06-16
4	REVISED PER NEW SITE PLAN	2023-09-13
5	REVISED FOR SPA	2025-04-22
6	REVISED PER NEW SITE PLAN	2025-07-16
7		
8		



CONSULTANTS



IBI GROUP
Suite 500 - 333 Preston Street
Ottawa ON K1S 5N4 Canada
Tel 613 225 1311 / 613 241 3300 Fax 613 225 9868
ibigroup.com

PROJECT
255 RICHMOND

PROJECT NO:
135883
DRAWN BY:
SPG
PROJECT MGR:
D.G.Y

CHECKED BY:
S.E.L.
APPROVED BY:
D.G.Y

SHEET TITLE
GRADING PLAN

SHEET NUMBER
C-200
ISSUE
6

CITY PLAN No. 18619

CITY FILE No. D07-12-21-0179
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Last Saved: June 26, 2025, by sumad3150
Plotted: July 16, 2025 1:48:07 PM by Suma, Don

NOTES:

THE CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES, TO PROVIDE FOR PROTECTION OF THE AREA DRAINAGE SYSTEM AND THE RECEIVING WATERCOURSE. DURING CONSTRUCTION ACTIVITIES, THE CONTRACTOR ACKNOWLEDGES THAT FAILURE TO IMPLEMENT APPROPRIATE EROSION AND SEDIMENT CONTROL MEASURES MAY BE SUBJECT TO PENALTIES IMPOSED BY ANY APPLICABLE REGULATORY AGENCY.

1. SILT FENCE TO BE ERRECTED PRIOR TO EARTH WORKS BEING COMMENCED. SILT FENCE TO BE MAINTAINED UNTIL VEGETATION IS ESTABLISHED OR UNTIL START OF SUBSEQUENT PHASE.
2. SILT SACK TO BE PLACED AND MAINTAINED UNDER COVER OF ALL CATCHBASINS. GEOTEXTILE SILT SACK IN STREET CBs TO REMAIN UNTIL ALL CURBS ARE CONSTRUCTED. GEOTEXTILE FABRIC IN RYCBs TO REMAIN UNTIL VEGETATION IS ESTABLISHED. ALL CATCHBASINS TO BE REGULARLY INSPECTED AND CLEANED, AS NECESSARY, UNTIL SOD AND CURBS ARE CONSTRUCTED.
3. WORKS NOTED ABOVE ARE TO BE INSTALLED, INSPECTED, MAINTAINED AND ULTIMATELY REMOVED BY SERVICING CONTRACTOR.
4. THIS IS A "LIVING DOCUMENT" AND MAY BE MODIFIED IN THE EVENT THE PROPOSED CONTROL MEASURES ARE INSUFFICIENT.
5. SEE DRAWING C-010 FOR ADDITIONAL DETAILS AND NOTES.

LEGEND :

- LIGHT DUTY SILT FENCE AS PER OPSD-219.110
- SNOW FENCE
- STRAW BALE CHECK DAM AS PER OPSD-219.180
- ◆ ROCK CHECK DAM AS PER OPSD-219.210
- SILT SACK PLACED UNDER EXISTING CB COVER
- TEMPORARY MUD MAT 0.15m THICK 50mm CLEAR STONE ON NON WOVEN FILTER CLOTH

