



**PATERSON  
GROUP**

**Consulting Engineers**

9 Auriga Drive

Ottawa, Ontario

K2E 7T9

Tel: (613) 226-7381

Geotechnical Engineering  
Environmental Engineering  
Hydrogeology  
Materials Testing  
Building Science  
Rural Development Design  
Retaining Wall Design  
Noise and Vibration Studies

February 24, 2025

File: PH4841-LET.01

**Percy Pyper (1997) Ltd**  
1971 Old Prescott Road,  
Greely, ON  
K4P 1N6

Attention: **Brent Pyper**

Subject: **Septic System Impact Assessment  
(Terrain Analysis)  
Re-zoning and Site Plan Control Application**  
5360 Bank Street  
Ottawa (Gloucester), Ontario

[petersongroup.ca](http://petersongroup.ca)

## INTRODUCTION

Paterson Group Inc. (Paterson) was retained by Greely Sand and Gravel Inc. to carry out a Septic System Impact Assessment in support of a proposed Re-zoning application and a Site Plan Control application (hereby referred to as Site Plan application) for the aforementioned property. It is our understanding that the current property, identified as Part of Lot 29, Concession 4 and located at 5360 Bank Street, Ottawa, consists of a 6.74 hectares (ha) parcel with an existing development in the central portion of the site. The proposed Re-zoning application aims to rezone the 6.74 ha parcel from Rural Countryside (RU) to Light Industrial. Please refer to the Key Plan attached for more details.

The purpose of this study has been to carry out a septic system impact assessment using publicly available information and fieldwork performed by others to determine the site's suitability for private on-site wastewater systems. Specifically, the intent of this report is to provide a maximum sewage flow volume which the subject site can support from a nitrate attenuation standpoint and assess the theoretical septic impact of the proposed replacement sewage system. This hydrogeological study is required to assess the risk that the development's individual on-site sewage system will not cause concentrations of nitrate in groundwater to exceed 10 mg/L at the property boundary.



## BACKGROUND

### Hydrogeological Assessment

As the subject site is to be municipally serviced with a water supply, a Hydrogeological Assessment under the City of Ottawa's (City) Hydrogeological and Terrain Analysis Guidelines (HTAG) is not required to support the Re-zoning application and Site Plan application.

### Subject Site

The subject property consists of a light industrial development with associated storage facilities and is located at 5360 Bank Street in the City of Ottawa, Ontario (refer to City of Ottawa, Plan of Survey, attached). The subject site is currently occupied by a maintenance garage, a site trailer and a number of covered storage areas with associated paved and gravel access lanes. The site is currently serviced by a private water supply and private septic system, however the Site Plan application is proposed to be serviced using municipal water supply. A proposed replacement septic system design has been approved by the Ottawa Septic System Office (OSSO) in support of the Site Plan application. There are no proposed building changes as part of the Site Plan application.

The subject site is largely rectangular in shape with a total area of 6.74 ha. The John Boyce Municipal Drain flows north-west to south-east through the western half of the severed property. The site and buildings are currently considered legal non-conforming as the site is currently zoned as RU (rural). The intention of the aforementioned Re-zoning application is to rezone the subject site as light industrial.

### Regional Geology

Published surficial geology mapping (OGS MRD128) for the area in the vicinity of the subject site indicates that the majority of the site is underlain predominantly by glaciofluvial deposits consisting of river deposits and delta topset facies. The north-eastern portion of the site is mapped to consist of stone-poor, silty sand to sandy silt-textured till on Paleozoic terrain.

Published bedrock geology mapping (OGS MRD219) indicates that the subject lands are underlain by dolostone with minor shale and sandstone of the Beekmantown Group and Oxford Formation. The available bedrock mapping coincides with the well driller's description on the Ministry of the Environment, Conservation and Parks (MECP) Water Well Records (WWR) for the surrounding well supplies installed within the subject area, which generally indicate a grey limestone.



### **On-site Geological Studies**

A series of boreholes were excavated on the subject parcel to delineate the subsurface soil conditions as part of the Phase II Environmental Site Assessment (ESA) completed by Gemtec (Project:100227.101 dated October 2, 2023).

The subsurface profile generally consisted of a sand to silt with varying amounts of trace clay or gravel extending to the depth of the borehole. Topsoil was recorded to extend to a maximum depth of 1.98 m bgs (BH23-06). A varying fill layer was identified on the site in the locations of BH 23-3, BH23-4, BH23-5 and BH23-6 and extended to a maximum depth of 6.1 m bgs.

### **Karst Mapping**

Available Karst mapping (OGS GRS005) was reviewed as part of this assessment. The available mapping does not indicate the presence of any inferred or potential karstic features. Furthermore, no indication of karstic features were observed during the site visits completed by Paterson personnel.

### **Mississippi-Rideau Source Protection Plan**

The Mississippi-Rideau Source Protection Plan (MRSPP) provides guidance as to which policies apply to a given property, municipality or specific activity and if there are specific designations that apply to the area. The subject site and surrounding areas have been designated as a Highly Vulnerable Aquifer (HVA), with parts of the site being mapped as an Intake Protection Zone (IPZ) Zone 3 and a Significant Groundwater Recharge Area (SGRA).

Based upon the designation of an SGRA, IPZ Zone 3 and HVA, the MRSPP provides a list of activities that are prohibited, managed or encouraged to change dependent upon the vulnerable area type. The subject site is mapped to be in IPZ zone 3 (Source Protection Atlas), however has a IPZ score of less than 8 (MRSPP). There is no prohibition of land uses on the subject site based upon its existing usage.

Therefore, there are no related requirements for an HVA, a IPZ with a score of less than 8 or SGRA at this location.

## **TERRAIN ANALYSIS**

The fieldwork which was completed as part of the Phase II ESA by Gemtec for the site (Report 100227.101, dated October 2, 2023) was used in support of this assessment. Additional information pertaining to this investigation was gathered from available geological mapping and surrounding WWR's.



## **Surficial Geology**

A series of boreholes were put down on the subject parcel to delineate the subsurface soil conditions as part of the environmental investigation (Gemtec Report 100227.101 dated October 2, 2023). In August, 2023, seven (7) boreholes were completed on the property to delineate the subsurface profile. The location of the boreholes on the property are delineated on Gemtec's Borehole and Monitoring Well Location Plan, Figure A-4, attached.

The test hole locations were recorded and the subsurface conditions, including the soil morphology and depth to the groundwater table (if encountered), were carefully observed and recorded. The soils encountered were classified texturally in the field, and later reviewed in the laboratory.

The boreholes were advanced to a maximum depth of 6.9 m bgs. Auger refusal was recorded to occur at 4.0 m bgs in boreholes 23-01, 23-02, and 23-07, and the remainder of the boreholes extended past 4.0 m bgs to a maximum depth of 6.9 m bgs.

According to the borehole logs, the subsurface profile generally consisted of a sand to silt with varying amounts of clay or gravel extending to the depth of the borehole. Topsoil was recorded to extend to a maximum depth of 1.98 m bgs (BH23-06). A varying fill layer was identified on the site in the locations of BH 23-3, BH23-4, BH23-5 and BH23-6 and extended to a maximum depth of 6.1 m bgs.

Reference should be made to the borehole logs appended to this report for the details of the soil profiles encountered at each test hole location.

Materials encountered during Gemtec's Phase II ESA were consistent with the available surficial and bedrock geology mapping.

## **Hydrogeological Sensitivity of the Site**

The subject site currently consists of a developed area which has been used for construction and light industrial use since prior to 1958 (Milestone Aggregate Consulting Services Inc. Chronological History – 5360 Bank Street dated March 21, 2023).

The topography of the site is generally sloping gently away from the central portion. The local flow direction of the surficial aquifer is expected to be towards the western portion of the site, where the John Boyce Municipal Drain runs from northwest to southeast across the site. The regional groundwater flow is considered to be in an southeasterly direction.

According to surrounding Water Well Records (WWR), the bedrock depths surrounding the proposed site vary from 0 to 13.4 m bgs. According to the field investigation, the overburden thickness was observed to be greater than 2 m at all borehole locations. As



the proposed site does not have bedrock within 2 m of the ground surface, the site is not considered hydrogeologically sensitive.

### **Conceptual Lot Development**

The existing development is not anticipated to change as part of the Site Plan application. The Site Plan application is being completed to ensure that the existing site use conforms to current City of Ottawa policies.

### **Sewage System Design and Total Daily Design Sewage Flow**

As this Terrain Analysis is completed to support both a Re-zoning and Site Plan application, the proposed sewage system will be analysed and the maximum predicted nitrate concentration for the Site will be determined.

An approved Ottawa Septic System Office (OSSO) Sewage System Installation Permit (SSIP) will be submitted as part of the Site Plan control application submission. The approved OSSO SSIP is for a Total Daily Design Sanitary Sewage Flow (TDDSSF) of 450 L/day. Please refer to the approved OSSO SSIP for additional details.

## **PREDICTIVE NITRATE IMPACT ASSESSMENT**

Nitrate is considered to be a critical parameter of concern when assessing impacts to groundwater quality downgradient of an onsite sewage system. The City of Ottawa (City) annotated MECP Procedure D-5-4 in the City of Ottawa's (City) Hydrogeological and Terrain Analysis Guidelines (HTAG) applies for the proposed development. For the purpose of this guideline, the Ontario Drinking Water Objective of 10 mg/L of nitrate is the maximum allowable concentration detectable in the groundwater prior to the property line.

A detailed impact assessment is required due to the commercial nature of the site. In order to demonstrate that private services would adequately support the proposed Re-zoning application, a predictive nitrate impact assessment (NIA) for the subject site was completed. This calculation was completed to determine the maximum sewage flow volume which could be applied to the subject site with the current site conditions, to support the Re-zoning application. One calculation is completed using a conventional system (no nitrate reduction) (Scenario 1) and the other with an NSF 245/ BNQ tertiary treatment system with 50 % nitrate reduction (Scenario 2). A third calculation was completed using the TDDSSF of 450 L/day, as per the approved OSSO SSIP submitted as part of the Site Plan application, to demonstrate that the proposed usage will meet the City's minimum requirements (Scenario 3). The values shown in the Predictive Nitrate Impact Assessment calculation attached to this report are summarized below:



<input type="checkbox"/> Site area	6.74 ha
<input type="checkbox"/> Impervious area %	40 %
<input type="checkbox"/> Concentration of nitrate in effluent	
• Scenario 1+3: Value based on conventional effluent concentration	40 mg/L
• Scenario 2: Value based on NSF245/BNQ certified nitrate reduction system with 50% nitrate reduction	20 mg/L
<input type="checkbox"/> Surplus Water	298 mm/year
<i>(The surplus water value was estimated based on Environment Canada Climate Office values with a soil type comprised of fine sandy loam (Mature Forest) and anthropogenic sources, which can be found attached.)</i>	
<input type="checkbox"/> Combined infiltration factor based on:	0.75
• Topography infiltration factor	0.20
• Soil texture infiltration factor	0.40
• Cover infiltration factor	0.15

The topography infiltration factor of 0.20 is based upon rolling land (average slope of 2.8 to 3.8 m/km). The soil texture infiltration factor was based upon an “open sandy loam” with a value of 0.4 which is a reasonable generalization based upon the field investigation by others, available geological mapping and surrounding WWR’s. The “vegetative cover infiltration factor” was calculated as 0.15 based upon the site being approximately halfway between undeveloped land consisting of partially wooded areas and developed areas.

#### **Scenario 1 – Site Maximum TDDSSF with Conventional Sewage system:**

The Predictive Nitrate Impact Assessment was completed to determine the maximum sewage flow volume which could be applied to the subject site using the current site conditions and a conventional septic system without surpassing the maximum nitrate attenuation concentration of 10 mg/L in the groundwater prior to the property line. Based on the existing site conditions and the use of a conventional sewage system (40 mg/L nitrate concentration), the predicted maximum allowable sewage flow volume is **9.5 m<sup>3</sup>/day** to attenuate the nitrate concentration to below the 10 mg/L nitrate concentration in the groundwater prior to the property line.

#### **Scenario 2 – Site Maximum TDDSSF with Tertiary Sewage system (NSF 245/BNQ):**

The Predictive Nitrate Impact Assessment was completed to determine the maximum sewage flow volume which could be applied to the subject site using the current site conditions and a NSF 245/BNQ certified tertiary treatment septic system without surpassing the maximum nitrate attenuation concentration of 10 mg/L in the groundwater prior to the property line. This type of system would have a minimum 50 % nitrate



reduction requirement. Based on the existing site conditions and the use of a NSF 245 / BNQ tertiary treatment sewage system (20 mg/L nitrate concentration), the predicted maximum allowable sewage flow volume is **greater than 10 m<sup>3</sup>/day** to attenuate the nitrate concentration to below the 10 mg/L nitrate concentration in the groundwater prior to the property line. It should be noted that a sewage system with a TDDSSF of greater than 10,000 L/day would require a MECP Environmental Compliance approval (ECA).

### **Scenario 3 – OSSO SSIP System for Site Plan:**

As per the approved OSSO SSIP permit that has been submitted as part of the Site Plan application, the proposed TDDSSF volumes are **0.45 m<sup>3</sup>/day**. The proposed system is a conventional system (40 mg/L nitrate concentration). This TDDSSF is less than the maximum allowable sewage low volume presented for a conventional system.

Using the existing site conditions, a predictive NIA using a TDDSSF of 450 L/day results in a nitrate concentration of 0.31 mg/L at the property boundary. Please refer to the Predictive Nitrate Impact Assessment calculation attached to this report for further details.

The OSSO approved SSIP consists of a sewage system which would result in a predictive nitrate concentration of 0.31 mg/L at the property boundary. As the Ontario Drinking Water Objective of 10 mg/L of nitrate is the maximum allowable concentration detectable in the groundwater prior to the property line, it is our opinion that the property can adequately support the proposed Site Plan application without having an adverse impact on the underlying bedrock aquifer based on the results of the predictive NIA.



## CONCLUSIONS

Based on the information contained within the body of this review, the following conclusions can be drawn:

- 1.0 A Sewage System Permit and Building Permit need to be issued prior to the commencement of construction on the proposed sewage system.
- 2.0 As overburden thickness was recorded to be greater than 2 m, the site is not considered hydrogeologically sensitive.
- 3.0 Based on the existing site conditions and the use of a conventional sewage system (40 mg/L nitrate concentration), the predicted maximum allowable sewage flow volume is 9.5 m<sup>3</sup>/day to attenuate the nitrate concentration to the maximum 10 mg/L nitrate concentration in the groundwater prior to the property line.
- 4.0 The predictive NIA completed on the OSSO approved SSIP with TDDSSF of 0.45 m<sup>3</sup>/day resulted in a predictive nitrate concentration of 0.31 mg/L at the property boundary, which is below the Ontario Drinking Water Objective maximum allowable concentration of 10 mg/L of nitrate detectable in the groundwater prior to the property line.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Greely Sand and Gravel, or their agents, is not authorized without review by Paterson for the applicability of our recommendations to the alternative use of the report.

We trust that this report satisfies your present requirements. Should you have any questions regarding this report, do not hesitate to contact us.

Yours truly,

PATERSON GROUP INC.

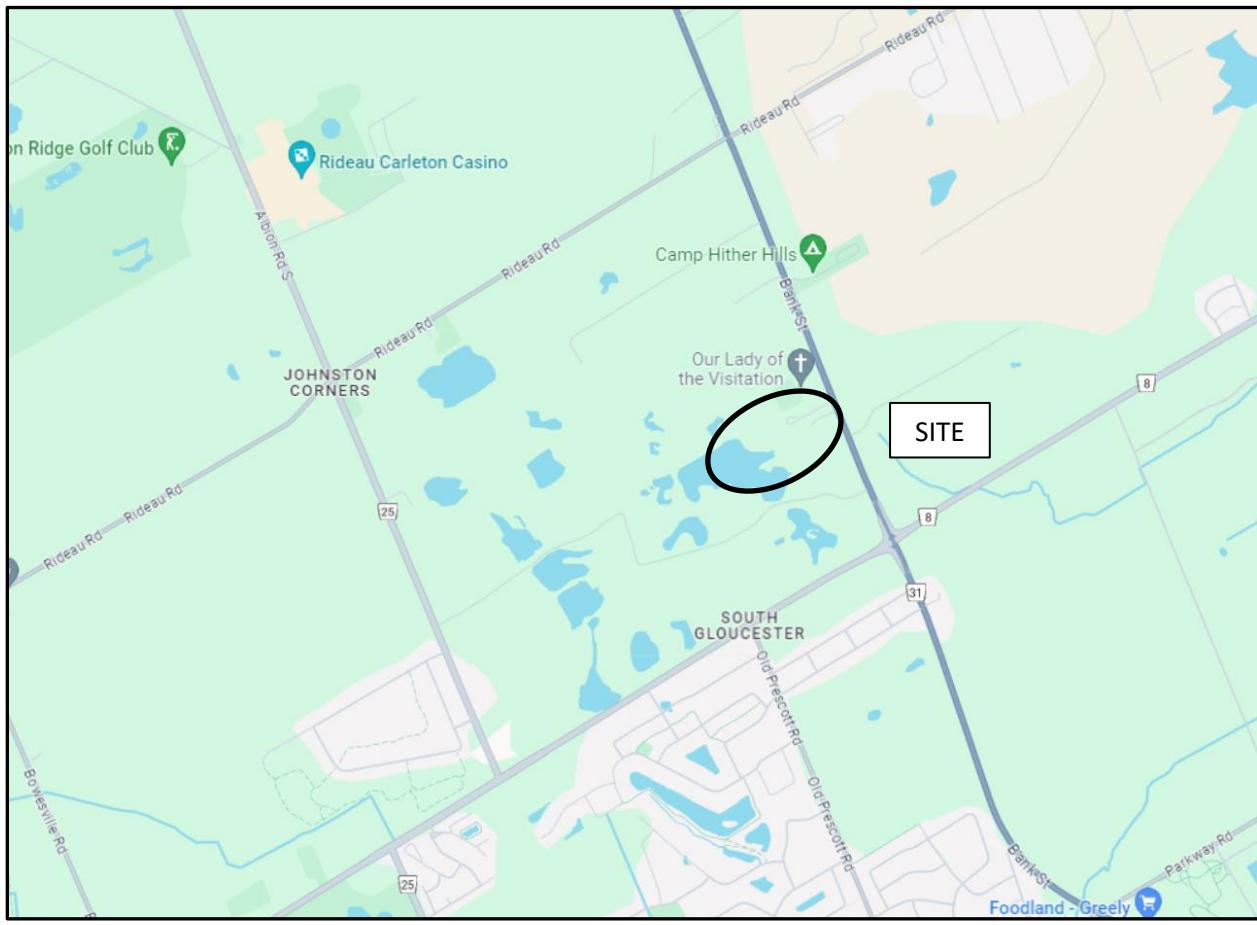
Alex Schopf  
PhD, E.I.T

Attachments:

- Key Plan
- MECP Water Well Records
- Nitrate Impact Assessment Calculation
- City of Ottawa Plan of Survey
- Gemtec Soil Profile and Test Data Sheets
- Gemtec – Figure A-4 Borehole and Monitoring Well Location Plan



Erik Ardley  
P.Geo



# FIGURE 1

## KEY PLAN



UTM 118<sup>Z</sup> 4514191210<sup>E</sup>  
5<sup>R</sup> 50114181910<sup>N</sup>

Elev. ~~2410~~ 2418.67 017  
Basin 2251 11

**The Water-well Drillers Act, 1954**  
**Department of Mines**

15 N.  
GROUND WATER BRANCH  
DEC 19 1958  
ONTARIO WATER  
RESOURCES COMMISSION

## Water-Well Record

TABLETAN Township, Village, Town or City..... *BLADEFIELD*  
Village, Town or City).....  
Address .....

Date completed ..... 2 0 07 .....  
(day) (month) (year)

### **Pipe and Casing Record**

### Pumping Test

Casing diameter(s) ..... 4  
Length(s) ..... 21  
Type of screen ..... 110  
Length of screen ..... 100

8  
Static level .....  
Pumping rate ..... 250 GPH  
Pumping level ..... 14  
Duration of test ..... 111/3

## Well Log

## Water Record

For what purpose(s) is the water to be used?

House

Is water clear or cloudy?..... clear.....

Drilling firm .....

**Address** .....

Name of Driller JOHN MEAGHER

Name of Driver John T. Tamm  
Address 877 10th

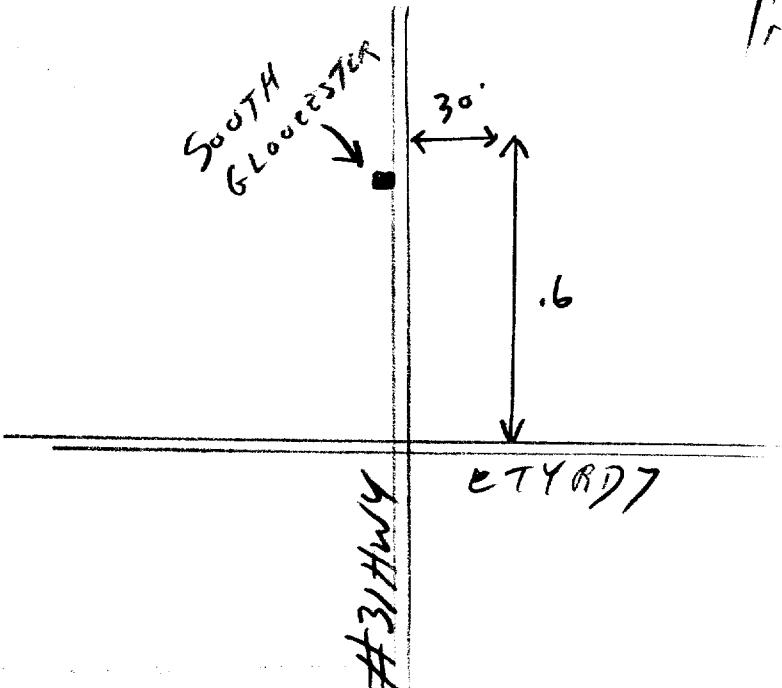
..... Licence Number.....

I certify that the foregoing  
statements of fact are true.

Date DEC 15 To Meath

**Signature of Licensee**

In diagram below show distances of well from road and lot line. Indicate north by arrow.





31G/52



UTM 118E 41514191310E

Ridgeway 51697 418110N

The Ontario Water Resources Commission Act

Elev. 294 R 0131510

Basin 28  
County or District

Con. 5 RF [REDACTED] Lot 28

GROUND WATER BRANCH

15G 1N 1961 2275

ONTARIO WATER  
RESOURCES COMMISSION

## WATER WELL RECORD

Township, Village, Town or City

Gloucester

Date completed

2

JULY

61

(day)

month

year

Address BILLINGS BRIDGE

## Casing and Screen Record

Inside diameter of casing	4"
Total length of casing	10'
Type of screen	—
Length of screen	—
Depth to top of screen	—
Diameter of finished hole	4"

## Pumping Test

Static level	8'
Test-pumping rate	4 G.P.M.
Pumping level	8'
Duration of test pumping	1 HR
Water clear or cloudy at end of test	CLEAR
Recommended pumping rate	4 G.P.M.
with pump setting of	25' feet below ground surface

## Well Log

## Overburden and Bedrock Record

From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
----------	--------	----------------------------------	---------------------------------------

GREY Limestone

0 100 100 FRESH

For what purpose(s) is the water to be used?

STOKE - HOUSE &amp; GARAGE

Is well on upland, in valley, or on hillside?

Drilling or Boring Firm

M. MEAGHER

Address OTTAWA

Licence Number

245

3PM6

Name of Driller or Borer

M. MEAGHER

Address

Date

AUG 9/61

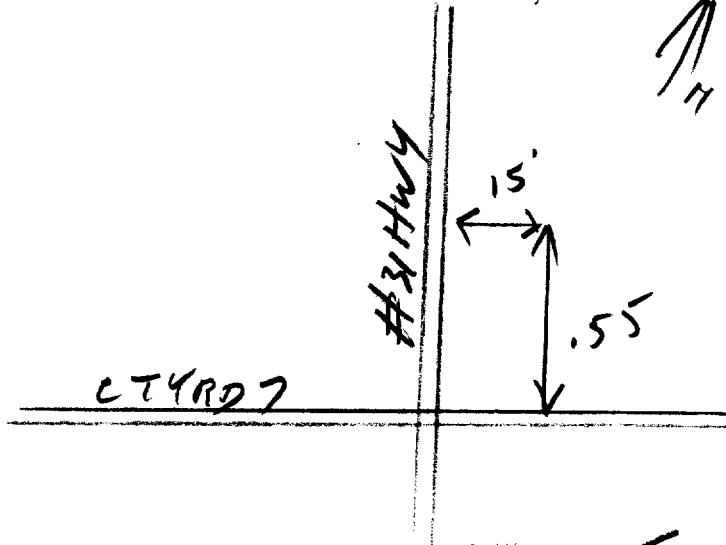
(Signature of Licensed Drilling or Boring Contractor)

Form 7 15M Sets 60-5930

OWRC COPY

## Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



31G/52



718  
UTM 118z 41514191410E  
KIDEAU FRONT  
STR 510114181510N  
CON 7 R 28 1315161

The Ontario Water Resources Commission Act

WATER RESOURCES  
DIVISION

5 JAN 19 1965

ONTARIO WATER  
RESOURCES COMMISSION

# WATER WELL RECORD

Basin 215

County or District Carleton

Township, Village, Town or City

Gloucester  
Oct 1964

Elev. 428 Con. 5 RF Lot. PT N 1/2 28

Date completed 8

(day)

Con. 5 RF Lot. PT N 1/2 28

month

year)

Owner [REDACTED]

Address RR 6 Ottawa

(print in block letters)

## Casing and Screen Record

Inside diameter of casing 2  
 Total length of casing 34  
 Type of screen 1  
 Length of screen 2  
 Depth to top of screen 9  
 Diameter of finished hole 2

## Pumping Test

Static level 22  
 Test-pumping rate 2 G.P.M.  
 Pumping level 60  
 Duration of test pumping 2 hr  
 Water clear or cloudy at end of test Cloudy  
 Recommended pumping rate 2 G.P.M.  
 with pump setting of 60 feet below ground surface

## Well Log

## Overburden and Bedrock Record

Top Soil  
Sand  
Lime Stone

		Water Record	
From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
0	4	152	Fresh
4	5		
5	152		

For what purpose(s) is the water to be used?

House  
Upland

Is well on upland, in valley, or on hillside?

House  
Upland

Drilling or Boring Firm

J. R. Cassette  
1510 Base line Rd.

Address

Ottawa

Licence Number

1472

Name of Driller or Borer

Same

Address

Oct 9-1964

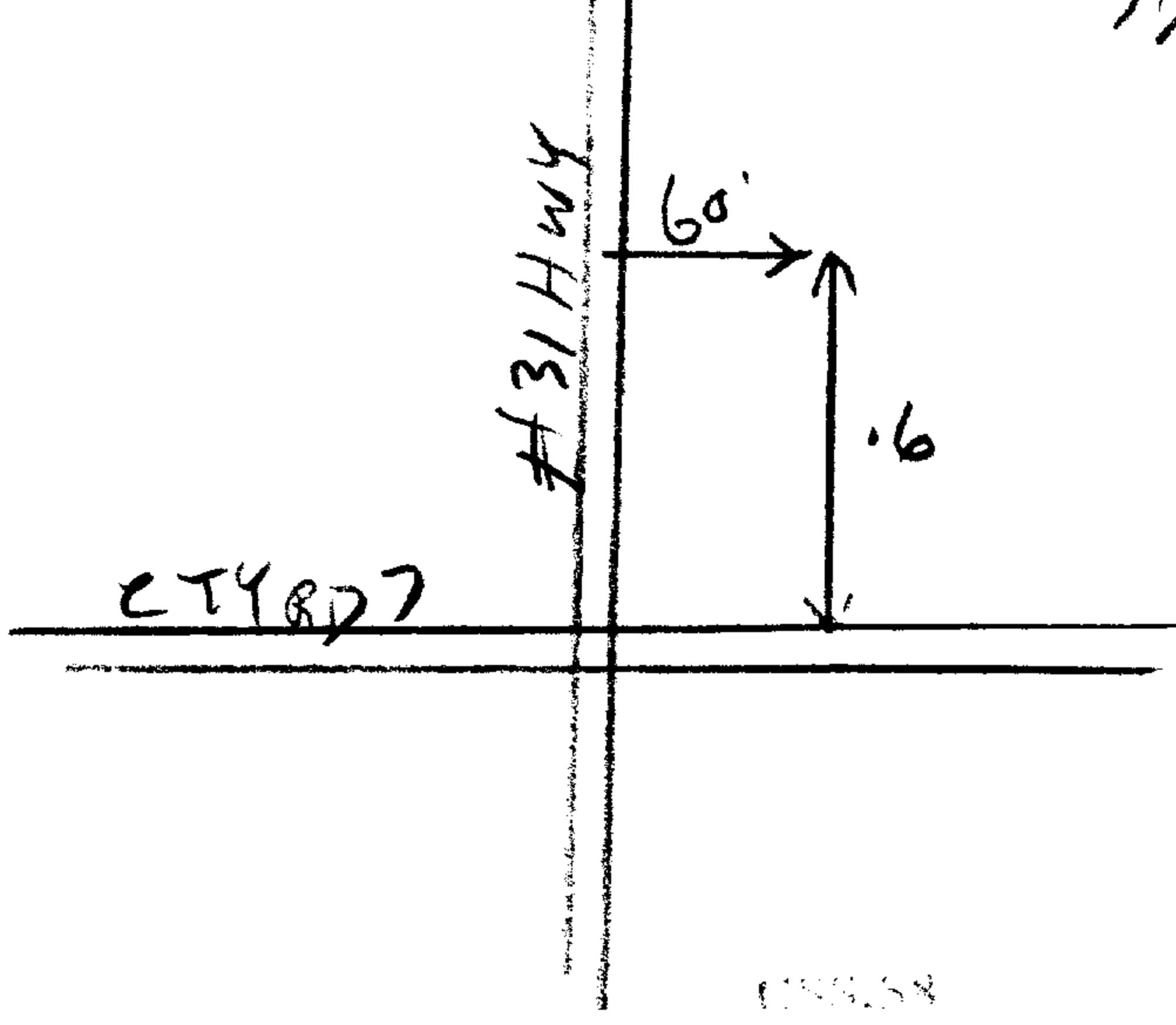
Date

J. R. Cassette

(Signature of Licensed Drilling or Boring Contractor)

## Location of Well

In diagram below show distances of well from road and lot line. Indicate north by arrow.



UTM 11a Z 4151511410 E

31G/52



ONTARIO

SK 510114131910 N

Elev. 14 R 0131315

Basin 215

## The Well Drillers Act

Department of Mines, Province of Ontario DEPARTMENT OF MINES

15 N. RECEIVED

JAN 5 1950

GEOLOGICAL BRANCH  
DEPARTMENT OF MINES

2278

## Water Well Record

Caledon Co.

Gloucester

029

Con. 1 R.F.

200

Pt. Lot

Manotick station Acres

8/65.00

Cost of well (not including pump)

## Pipe and Casing Record

## Pumping Test

Casing diameter(s) 4" . . . . .  
 Length(s) of casing(s) 15 feet . . . . .  
 Length of screen . . . . .  
 Type of screen . . . . .  
 Type of pump . . . . .  
 Capacity of pump . . . . .  
 Depth of pump setting . . . . .

Date May 10/48 . . . . .  
 Developed Capacity 400 gals a min . . . . .  
 Duration of Test 1/2 hour . . . . .  
 Pumping Rate 74 gals . . . . .  
 Drawdown 3 feet . . . . .  
 Static level of completed well 18 feet . . . . .  
 Is well a gravel-wall type? no . . . . .

## Water Record

Kind (fresh or mineral) . . . . .	Depth(s) to Water Horizon(s) . . . . .	Kind of Water . . . . .	No. of Feet Water Rises . . . . .
Kind (fresh or mineral) . . . . .	64 feet	fresh	54 feet
Quality (hard, soft, contains iron, sulphur etc.) . . . . .			42 ft
Appearance (clear, cloudy, coloured) . . . . .			
For what purpose(s) is the water to be used? . . . . .			
How far is well from possible source of contamination? . . . . .			
What is source of contamination? . . . . .			
Enclose a copy of any mineral analysis that has been made of water . . . . .			

## Well Log

## Drift and Bedrock Record

15 feet of hard pan and  
bedrock  
lime stone rock

From

To

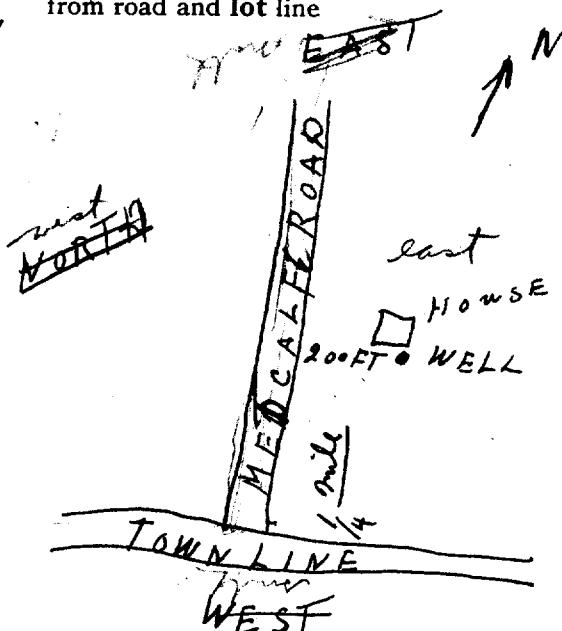
0 ft. . . . . ft.

1 . . . . . 15 ft.

15 . . . . . 22

## Location of Well

In diagram below show distances of well from road and lot line



Situation: Is well on upland, in valley, or on hillside? hill . . . . .  
 Drilling Firm G. Gordon & Son, Milligan . . . . .  
 Address Westboro T. R. #1 . . . . .  
 Recorded by G. Gordon & Son, Milligan . . . . .  
 Date May 10/48 . . . . .

Address Westboro T. R. #1 . . . . .

Licence Number . . . . .

316/52

UTM 118z 45 5 10 E

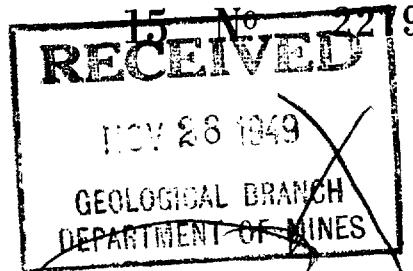
5|R 5|0|1|4|3|9|0|N

Elev. 4R 0335



**ONTARIO**

## **The Well Drillers Act**



## Water Well Record

Date Completed: 12/20/2013, Cost of Well (not including pump): \$10,000

## Pipe and Casing Record

## Pumping Test

Casing diameter(s) . . . . .	4"	Date . . . . .	July 28
Length(s) of casing(s) . . . . .	6'	Developed Capacity . . . . .	
Length of screen . . . . .		Duration of Test . . . . .	1
Type of screen . . . . .		Pumping Rate . . . . .	
Type of pump . . . . .		Drawdown . . . . .	
Capacity of pump . . . . .		Static level of completed well . . . . .	6'
Depth of pump setting . . . . .		Is well a gravel-wall type? . . . . .	

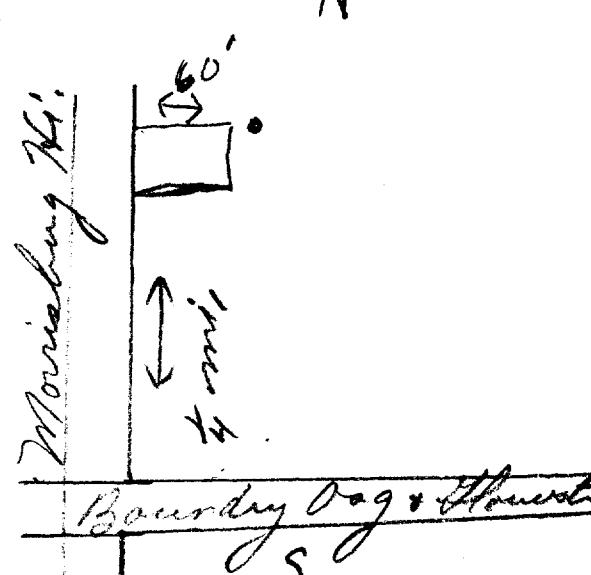
## Water Record

Kind (fresh or mineral) . . . . .	fresh	Depth(s) to Water Horizon(s)	Kind of Water	No. of Feet Water Rises
Quality (hard, soft, contains iron, sulphur etc.) . . . . .	hard	40	good	34
Appearance (clear, cloudy, coloured) . . . . .	clear			
For what purpose(s) is the water to be used? . . . . .	farm			
How far is well from possible source of contamination? . . . . .	60'			
What is source of contamination? . . . . .	outflow			
Enclose a copy of any mineral analysis that has been made of water . . . . .				

## Well Log

### Location of Well

In diagram below show distances of well from road and lot line



Situation: Is well on upland, in valley, or on hillside? uplands  
Drilling Firm M. M. eagle  
Address Bentonia Bay  
Recorded by M. M. eagle Address   
Date  Licence Number 499

















# **WATER WELL RECORD**

31G/59

## LOG OF OVERTBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
gray	clay			0	5
gray	shale rock			5	15
gray	limestone			15	130
white	sandstone			130	155

31 0005205 0015217 0130215 0155118

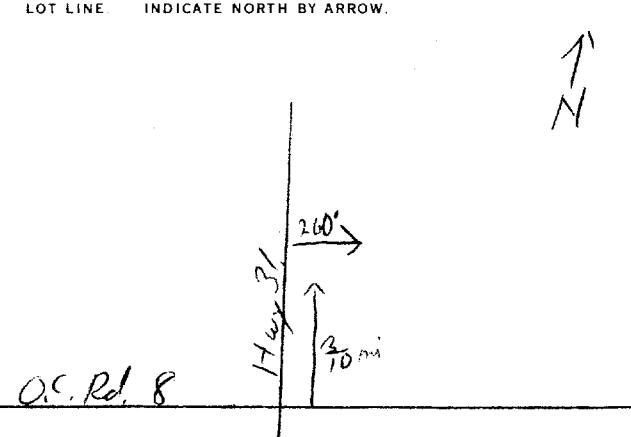
32		33		34		35		36		37		38		39	
10		14		15		19		20		21		22		23	
WATER FOUND IN FEET		KIND OF WATER		INSIDE DIAM INCHES		MATERIAL		WALL THICKNESS INCHES		DEPTH - FEET		FROM		TO	
10-13		1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 14 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL		075		10-11		1 <input checked="" type="checkbox"/> STEEL 12		13-16		0		0042	
15-18		1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 19 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL		152		17-18		1 <input type="checkbox"/> STEEL 19		20-23		10-13		14-17	
20-23		1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 24 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL		25		24-25		1 <input type="checkbox"/> STEEL 26		27-30		18-21		22-25	
25-28		1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 25 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL		30		6		2 <input type="checkbox"/> GALVANIZED		30-33		26-29		30-33 80	
30-33		1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 34 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL		30		1 <input type="checkbox"/> CONCRETE		3 <input type="checkbox"/> CONCRETE		34-38		30-33		39-40	
				3 <input type="checkbox"/> OPEN HOLE		4 <input type="checkbox"/> OPEN HOLE		4 <input type="checkbox"/> OPEN HOLE		34-38		30-33		39-40	

PUMPING TEST 71	PUMPING TEST METHOD		10	PUMPING RATE	11-14	DURATION OF PUMPING	
	<input checked="" type="checkbox"/> PUMP	<input type="checkbox"/> BAILER		0025	GPM	01	15-16 HOURS
STATIC LEVEL	WATER LEVEL END OF PUMPING	25	WATER LEVELS DURING			<input checked="" type="checkbox"/> PUMPING <input type="checkbox"/> RECOVERY	
	19-21 FEET	22-24 FEET	15 MINUTES 26-28 FEET	30 MINUTES 29-31 FEET	45 MINUTES 32-34 FEET	60 MINUTES 35-37 FEET	
IF FLOWING, GIVE RATE	38-41	PUMP INTAKE SET AT		WATER AT END OF TEST			42
	GPM			FEET	1 <input type="checkbox"/> CLEAR	2 <input checked="" type="checkbox"/> CLOUDY	
RECOMMENDED PUMP TYPE		RECOMMENDED PUMP SETTING	43-45 FEET	RECOMMENDED PUMP RATE	46-49 GPM		
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP		070		0005			
50-53 GPM./FT. SPECIFIC CAPACITY							

<b>FINAL STATUS OF WELL</b>	<b>54</b>	<input checked="" type="checkbox"/> WATER SUPPLY <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY <input type="checkbox"/> OBSERVATION WELL <input type="checkbox"/> ABANDONED, POOR QUALITY <input type="checkbox"/> TEST HOLE <input type="checkbox"/> UNFINISHED <input type="checkbox"/> RECHARGE WELL		
	<b>55-56</b>		<input checked="" type="checkbox"/> DOMESTIC <input type="checkbox"/> COMMERCIAL <input type="checkbox"/> STOCK <input type="checkbox"/> MUNICIPAL <input type="checkbox"/> IRRIGATION <input type="checkbox"/> PUBLIC SUPPLY <input type="checkbox"/> INDUSTRIAL <input type="checkbox"/> COOLING OR AIR CONDITIONING <input type="checkbox"/> OTHER <input type="checkbox"/> NOT USED	
	<b>WATER USE</b>		<b>01</b>	
	<b>METHOD OF DRILLING</b>	<b>57</b>	<input type="checkbox"/> CABLE TOOL <input type="checkbox"/> BORING <input type="checkbox"/> ROTARY (CONVENTIONAL) <input type="checkbox"/> DIAMOND <input type="checkbox"/> ROTARY (REVERSE) <input type="checkbox"/> JETTING <input type="checkbox"/> ROTARY (AIR) <input type="checkbox"/> DRIVING <input checked="" type="checkbox"/> AIR PERCUSSION	
		<b>5</b>		

**LOCATION OF WELL**

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND  
LOT LINE. INDICATE NORTH BY ARROW.



**DRILLERS REMARKS**

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	Henry Marie Well Drilling	3644
ADDRESS	Box 326, Richmond, Ont.	
NAME OF DRILLER OR BORER	LICENCE NUMBER	
SIGNATURE OF CONTRACTOR	SUBMISSION DATE	
	DAY	MO.
	29	7
	YR.	76

OFFICE USE ONLY	DATA SOURCE	58	CONTRACTOR	59-62	DATE RECEIVED	63-68	80
	1	3644		0 111 26			
	DATE OF INSPECTION	18/6/77	INSPECTOR	<i>h</i>		<i>PD</i>	
	REMARKS:	<i>0 111 26</i>					
		<i>P</i>					
		<i>W1</i>					









<b>PREDICTIVE NITRATE IMPACT ASSESSMENT - 40 mg/L</b>		
<b>Infiltration Factors</b>		
Topography	0.20	
Soil	0.40	
Cover	0.15	
<b>Total</b>	<b>0.75</b>	
<b>Site Characteristics</b>		
Area of Site :	67400	$m^2$
Roof + paved driveway areas	20723	$m^2$
Impervious Area	20723	$m^2$
Percent Impervious Area =	31	%
Infiltration Area =	46677	$m^2$
<b>Septic Effluent</b>		
Concentration of Effluent (Cs) =	40	mg/L
<b>Infiltration Calculation</b>		
Nitrate concentration in precipitation (C <sub>i</sub> ) =	0	mg/L
Surplus Water (Environment Canada)	298	mm/yr
Factored Water Surplus =	224	mm/yr
Infiltration % due to stormwater management measures	-	%
Infiltration rate from stormwater management measures =	0	mm/yr
Infiltration Flow Entering the System (Q <sub>i</sub> ) =	29	$m^3/day$
<b>Mass Balance Model (MOEE, 1995)</b>		
$C_T = (Q_b C_b + Q_e C_e + Q_i C_i) / (Q_b + Q_e + Q_i) = \text{Cumulative Nitrate Concentration}$		
Q <sub>b</sub> = flow entering the system across the upgradient area	0	$m^3/day$
C <sub>b</sub> = background nitrate concentration	0	mg/L
Q <sub>e</sub> = flow entering the system from the septic drainfield	9.5	$m^3/day$
C <sub>e</sub> = concentration of nitrates in the septic effluent	40	mg/L
Q <sub>i</sub> = flow entering the system from infiltration	29	$m^3/day$
C <sub>i</sub> = Concentration of nitrates in the infiltrate	0	mg/L
<b>C<sub>T</sub> = 9.98</b>		mg/L
<b>Maximum Allowable Sewage Flow Volume</b>		
Daily Sewage Flow (Q <sub>s</sub> ) =	<b>9.5</b>	$m^3$
<i>Notes: Site characteristic values were measured as approximate values from the available site plans and GeoOttawa.</i>		

<b>PREDICTIVE NITRATE IMPACT ASSESSMENT - 20 mg/L</b>		
<b>Infiltration Factors</b>		
Topography	0.20	
Soil	0.40	
Cover	0.15	
<b>Total</b>	<b>0.75</b>	
<b>Site Characteristics</b>		
Area of Site :	67400	$m^2$
Roof + paved driveway areas	20723	$m^2$
Impervious Area	20723	$m^2$
Percent Impervious Area =	31	%
Infiltration Area =	46677	$m^2$
<b>Septic Effluent</b>		
Concentration of Effluent (Cs) =	20	mg/L
<b>Infiltration Calculation</b>		
Nitrate concentration in precipitation (C <sub>i</sub> ) =	0	mg/L
Surplus Water (Environment Canada)	298	mm/yr
Factored Water Surplus =	224	mm/yr
Infiltration % due to stormwater management measures	-	%
Infiltration rate from stormwater management measures =	0	mm/yr
Infiltration Flow Entering the System (Q <sub>i</sub> ) =	29	$m^3/day$
<b>Mass Balance Model (MOEE, 1995)</b>		
$C_T = (Q_b C_b + Q_e C_e + Q_i C_i) / (Q_b + Q_e + Q_i) = \text{Cumulative Nitrate Concentration}$		
Q <sub>b</sub> = flow entering the system across the upgradient area	0	$m^3/day$
C <sub>b</sub> = background nitrate concentration	0	mg/L
Q <sub>e</sub> = flow entering the system from the septic drainfield	10	$m^3/day$
C <sub>e</sub> = concentration of nitrates in the septic effluent	20	mg/L
Q <sub>i</sub> = flow entering the system from infiltration	29	$m^3/day$
C <sub>i</sub> = Concentration of nitrates in the infiltrate	0	mg/L
$C_T = 5.18$		mg/L
<b>Maximum Allowable Sewage Flow Volume</b>		
Daily Sewage Flow (Q <sub>s</sub> ) =	10	$m^3$
<i>Notes: Site characteristic values were measured as approximate values from the available site plans and GeoOttawa.</i>		

## PREDICTIVE NITRATE IMPACT ASSESSMENT - 450 L/day

### Infiltration Factors

Topography	0.20
Soil	0.40
Cover	0.15
<b>Total</b>	<b>0.75</b>

### Site Characteristics

Area of Site :	67400	m <sup>2</sup>
Roof + paved driveway areas	20723	m <sup>2</sup>
Impervious Area	20723	m <sup>2</sup>
Percent Impervious Area =	31	%
Infiltration Area =	46677	m <sup>2</sup>

### Septic Effluent

Concentration of Effluent (Cs) =	20	mg/L
----------------------------------	----	------

### Infiltration Calculation

Nitrate concentration in precipitation (C <sub>i</sub> ) =	0	mg/L
Surplus Water (Environment Canada)	298	mm/yr
Factored Water Surplus =	224	mm/yr
Infiltration % due to stormwater management measures	-	%
Infiltration rate from stormwater management measures =	0	mm/yr
Infiltration Flow Entering the System (Q <sub>i</sub> ) =	29	m <sup>3</sup> /day

### Mass Balance Model (MOEE, 1995)

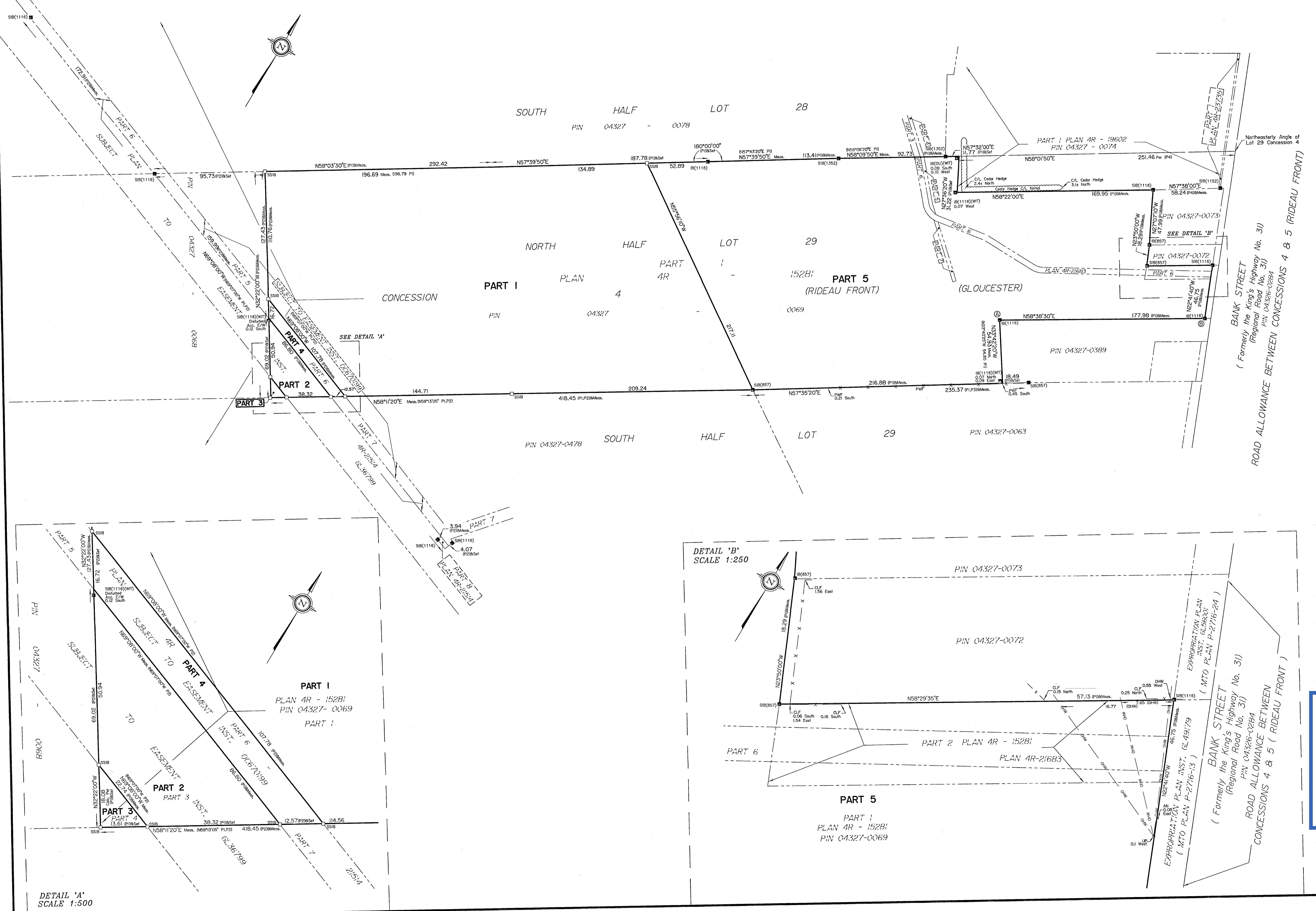
$$C_T = (Q_b C_b + Q_e C_e + Q_i C_i) / (Q_b + Q_e + Q_i) = \text{Cumulative Nitrate Concentration}$$

Q <sub>b</sub> = flow entering the system across the upgradient area	0	m <sup>3</sup> /day
C <sub>b</sub> = background nitrate concentration	0	mg/L
Q <sub>e</sub> = flow entering the system from the septic drainfield	0.45	m <sup>3</sup> /day
C <sub>e</sub> = concentration of nitrates in the septic effluent	20	mg/L
Q <sub>i</sub> = flow entering the system from infiltration	29	m <sup>3</sup> /day
C <sub>i</sub> = Concentration of nitrates in the infiltrate	0	mg/L
<b>C<sub>T</sub> =</b>	<b>0.31</b>	<b>mg/L</b>

### Sewage Flow Volume

Daily Sewage Flow (Q <sub>s</sub> ) =	0.45	m <sup>3</sup>
---------------------------------------	------	----------------

*Notes: Site characteristic values were measured as approximate values from the available site plans and GeoOttawa.*



REQUIRE THIS PLAN TO BE EPOSITED UNDER THE AND TITLES ACT.	PLAN 4R-34411
DATE: <u>Feb 25/22</u>	RECEIVED AND DEPOSITED DATE: <u>2022 02 28</u>
 STEFAN S. BAZAR ONTARIO LAND SURVEYOR	
 REPRESENTATIVE FOR LAND REGISTRAR FOR THE LAND TITLES DIVISION OF OTTAWA-CARLETON NO. 4.	

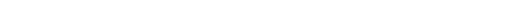
SCHEDULE			
PART	LOT	CONCESSION	PIN
1 to 5 (both inclusive)	PART OF 29	4	All of 04327-0069

Part 2: Subject to easement, Inst. GL36799  
Part 4: Subject to easement, Inst. OC670199

PLAN OF SURVEY OF  
**PART OF LOT 29  
CONCESSION 4 (RIDEAU FRONT)  
Geographic Township of Gloucester  
CITY OF OTTAWA**

Surveyed by Annis, O'Sullivan, Vollebekk Ltd.

scale 1 : 1500



A scale bar with markings at 45, 30, 15, 0, 30, and 60 Metres. The 0 and 30 markings are at the center, with 45 and 15 to the left and 30 and 60 to the right. The 60 Metre mark is at the far right end of the bar.

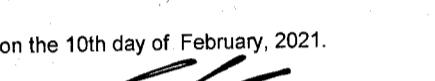
**etric**  
**DISTANCES AND COORDINATES SHOWN ON THIS PLAN  
ARE IN METRES AND CAN BE CONVERTED TO FEET BY  
DIVIDING BY 0.3048.**

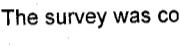
Surveyor's Certificate

CERTIFY THAT:

This survey and plan are correct and in accordance with the Surveys Act, the Surveyors Act and the Land Titles Act and the regulations made under them.

The survey was completed on the 10th day of February, 2021.

  
Stefan S. Bazar  
Ontario Land Surveyor

  
Date

□	Denotes	Survey Monument Planted
■	"	Survey Monument Found
SIB	"	Standard Iron Bar
SSIB	"	Short Standard Iron Bar
B	"	Iron Bar
Meas.	"	Measured
(WIT)	"	Witness
(AOG)	"	Annis, O'Sullivan, Vollebekk Ltd.
(P1)	"	Plan 4R-15281
(P2)	"	Plan 4R-21514
(P3)	"	Plan by (857) dated June 19, 1986
(P4)	"	Plan 4R-19602
PWF	"	Post & Wire Fence
CLF	"	Chain Link Fence
C/L	"	Centreline
— OHW —	"	Overhead Wires
• AN	"	Anchor
• UPL	"	Utility Pole

Distances shown on this plan are ground distances and can be converted to grid distances by multiplying by the combined scale factor of 0.999948.

Bearings are grid, derived from Can-Net 2016 Real Time Network GPS observations on reference points A and B, shown hereon, having a bearing of N58°38'30"E and are referenced to Specified Control Points 01919760735 and 01919871649, MTM Zone 9 ( 76°30' West Longitude )

NAD-83 (original).  
For bearing comparisons, a rotation of  $0^{\circ}38'25''$  counter-clockwise was applied to bearings on plan P1&P2 and a rotation of  $00^{\circ}02'40''$  to P4.  
Coordinates are derived from Can-Net 2016 Real Time Network GPS observations referenced to Specified Control Points 01919760735 and 01919871649, MTM Zone 9 ( $76^{\circ}30'$  West Longitude) NAD-83 (original).

Coordinate values are to urban accuracy in accordance with O. Reg. 216/10.

. 01919760735	Northing	5026903.34	Easting	376968.72
. 01919871649	Northing	5007189.87	Easting	372435.05
. Point A	Northing	5016475.64	Easting	377317.38
. Point B	Northing	5016568.25	Easting	377469.36

Caution: Coordinates cannot, in themselves, be used to re-establish corners

or boundaries shown on this plan.

# Committee of Adjustment

## Received | Reçu le

2022-12-09

# City of Ottawa | Ville d'Ottawa

## Comité de dérogation

# RECORD OF BOREHOLE 23-01

CLIENT: PERCY PYPER (1997) LTD.  
 PROJECT: 5360 Bank Street Phase Two ESA  
 JOB#: 100227.101  
 LOCATION: Salt Domes

SHEET: 1 OF 1  
 DATUM: CGVD28  
 BORING DATE: Aug 21 2023

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE			SAMPLE DATA				MONITORING WELL INSTALLATION AND NOTES			
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY (mm)	BLOW/S/0.3m	LABORATORY ANALYSES	COMBUSTIBLE VAPOUR CONCENTRATION (ppm)	ODOUR	TPH (mg/kg)
0		Ground Surface			SA1	CS			M&I, BTEX, PHCs F1-F4		None	
1		Brown gravelly sand with trace silt			SA2	CS			M&I, BTEX, PHCs F1-F4		None	
2	Direct Push				NR						None	
3					SA3	CS					None	
					SA4	CS					None	
					NR						None	
					SA5	CS					None	
					SA6	CS					None	
		End of Borehole Refusal		3.96								

# RECORD OF BOREHOLE 23-02

CLIENT: PERCY PYPER (1997) LTD.  
 PROJECT: 5360 Bank Street Phase Two ESA  
 JOB#: 100227.101  
 LOCATION: Cold Patch Pile

SHEET: 1 OF 1  
 DATUM: CGVD28  
 BORING DATE: Aug 21 2023

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE			SAMPLE DATA				MONITORING WELL INSTALLATION AND NOTES			
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY (mm)	BLOW/S/0.3m	LABORATORY ANALYSES	COMBUSTIBLE VAPOUR CONCENTRATION (ppm)	ODOUR	TPH (mg/kg)
0		Ground Surface			SA1	CS			M&I, BTEX, PHCs F1-F4		None	
1		Dark brown silty sand, trace clay (top soil)			NR							
2	Direct Push	Orangey brown silty sand (beach sand)		1.83	SA2	CS			M&I, BTEX, PHCs F1-F4		None	
3					SA3	CS					None	
		End of Borehole Refusal		3.96								

## RECORD OF BOREHOLE 23-03 (MW)

CLIENT: PERCY PYPER (1997) LTD.  
PROJECT: 5360 Bank Street Phase Two ESA  
JOB#: 100227.101  
LOCATION: Tanks Outside CACE

SHEET: 1 OF 1  
DATUM: CGVD28  
BORING DATE: Aug 21 2023



LOGGED: CZ

CHECKED: EW

## RECORD OF BOREHOLE 23-04 (MW)

CLIENT: PERCY PYPER (1997) LTD.  
PROJECT: 5360 Bank Street Phase Two ESA  
JOB#: 100227.101  
LOCATION: Side of Building

SHEET: 1 OF 1  
DATUM: CGVD28  
BORING DATE: Aug 21 2023



LOGGED: CZ

CHECKED: EW

## RECORD OF BOREHOLE 23-05 (MW)

CLIENT: PERCY PYPER (1997) LTD.  
PROJECT: 5360 Bank Street Phase Two ESA  
JOB#: 100227.101  
LOCATION: Beside Diesel Tank

SHEET: 1 OF 1  
DATUM: CGVD28  
BORING DATE: Aug 21 2023



LOGGED: CZ

CHECKED: EW

# RECORD OF BOREHOLE 23-06 (MW)

CLIENT: PERCY PYPER (1997) LTD.  
 PROJECT: 5360 Bank Street Phase Two ESA  
 JOB#: 100227.101  
 LOCATION: Beside Sea Cans

SHEET: 1 OF 1  
 DATUM: CGVD28  
 BORING DATE: Aug 22 2023

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE			SAMPLE DATA			COMBUSTIBLE VAPOUR CONCENTRATION (ppm)	ODOUR	TPH (mg/kg)	MONITORING WELL INSTALLATION AND NOTES	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY (mm)	BLOW/S 0.3m	LABORATORY ANALYSES			
0	Auger	Ground Surface Dark brown silty sand, trace clay (Topsoil)		110.29	NR				BTEX, PHCs F1-F4			
1					SA1	CS				HEX: 0, IBI: 0	None	
2		Grey brown sand, trace silt (Beach sand)		108.31 1.98	NR					HEX: 0, IBI: 0	None	
3	Direct Push				SA2	CS				HEX: 0, IBI: 2	None	
4		Wet, brown silty sand. Organic debris at 3.2 mbgs.		106.48 3.81	SA3	CS				HEX: 0, IBI: 0	None	
5					SA4	CS				HEX: 0, IBI: 0	None	
6		End of Borehole Intersection of Water Table		104.20 6.10	SA5	CS			BTEX, PHCs F1-F4	HEX: 0, IBI: 0	None	
					SA6	CS				HEX: 15, IBI: 1	None	
50 mm diameter PVC well screen												
GROUNDWATER OBSERVATIONS												
DATE		DEPTH (m)		ELEVATION (m)								
Aug. 23/23		3.92		106.22								
Aug. 25/23		3.95		106.20								

# RECORD OF BOREHOLE 23-07 (MW)

CLIENT: PERCY PYPER (1997) LTD.  
 PROJECT: 5360 Bank Street Phase Two ESA  
 JOB#: 100227.101  
 LOCATION: Asphalt Front of Building

SHEET: 1 OF 1  
 DATUM: CGVD28  
 BORING DATE: Aug 22 2023

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE			SAMPLE DATA			COMBUSTIBLE VAPOUR CONCENTRATION (ppm)	ODOUR	TPH (mg/kg)	MONITORING WELL INSTALLATION AND NOTES	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY (mm)	BLOW/S 0.3m	LABORATORY ANALYSES			
0	Auger	Ground Surface Asphalt Light brown silty sand, trace gravel and rock		110.20	NR				BTEX, PHCs F1-F4	HEX: 0, IBI: 0	None	
1					SA1	CS						
2					NR							
3	Direct Push				SA2	CS						
4					SA3	CS						
					SA4	CS			HEX: 0, IBI: 0	None		
					SA5	CS						
				105.63								
		End of Borehole Auger Refusal at 3.96 mbgs, Core Refusal at 4.57 mbgs		4.57								
38 mm diameter PVC well screen												
GROUNDWATER OBSERVATIONS												
DATE		DEPTH (m)		ELEVATION (m)								
Aug. 23/23		3.40		106.71								
Aug. 23/23		3.43		106.68								

