

GENERAL NOTES

- ANY DEVIATION FROM THE CONDITIONS SHOWN ON THESE DRAWINGS MUST BE REPORTED TO THE ENGINEER.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE 2024 O.B.C. ONTARIO REGULATION 202/24
- STANDARDS**
 - CSA STANDARD A23.3-19 DESIGN OF CONCRETE STRUCTURES
 - CSA STANDARD A23.1-19 CONCRETE MATERIALS & METHODS OF CONCRETE CONSTRUCTION
 - CSA-S16-19 LIMIT STATES DESIGNS OF STEEL STRUCTURES
 - CSA STANDARD S304-14 DESIGN OF MASONRY STRUCTURES
 - CSA-S95-19 ENGINEERING DESIGN IN WOOD
- ANY MODIFICATIONS TO EXISTING STRUCTURES ARE TO BE LIMITED TO WORK NOTED ON THESE DRAWINGS. ANY ADDITIONAL OR PROPOSED MODIFICATIONS TO EXISTING STRUCTURES MUST BE APPROVED BY THE ENGINEER

- FOUNDATIONS**
 - ALL FOOTINGS ARE TO BEAR ON ENGINEERED GRANULAR PAD OVER CONCRETE MODULUS COLUMN GROUND IMPROVEMENT
 - CONFIRM WITH GEOTECHNICAL REPORT/ENGINEER ANY REQUIREMENTS FOR MUD SLAB TO PROTECT INSTILL SOILS.
 - BEARING CAPACITY USED IN THE FOOTING DESIGN IS ASSUMED TO BE $SLS = 200 \text{ kPa}$ / $ULS = 350 \text{ kPa}$
 - BEARING SURFACE IS TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
 - FOR FURTHER INFORMATION SEE GEOTECHNICAL REPORT No. 104638.001 PREPARED BY GENTEC
 - STEP FOOTINGS WHERE INDICATED ON PLAN AT THE RATE OF 1 HORIZONTAL TO 1 VERTICAL.

- MATERIALS**
 - CONCRETE STRENGTH AT 28 DAYS TO BE AS NOTED ON THESE DRAWINGS AND SPECIFICATIONS.
 - REINFORCING STEEL TO BE DEFORMED GRADE 40W WITH $F_y = 400 \text{ MPa}$
 - HOLLOW STRUCTURAL STEEL SECTIONS TO BE ASTM A500 GRADE C OR Q421 350W CLASS C. ALL 'W', 'C', 'L' & 'WWF' SHAPE STEEL SECTIONS TO BE GRADE Q421 350W WITH $F_y = 350 \text{ MPa}$. ALL OTHER STRUCTURAL STEEL TO BE GRADE Q421 300W WITH $F_y = 300 \text{ MPa}$ UNLESS NOTED OTHERWISE.
 - ALL STRUCTURAL STEEL TO RECEIVE 1 SHOP APPLIED COAT OF PRIMER UNLESS NOTED.
 - ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR IS TO BE HOT DIP GALVANIZED UNLESS NOTED. ANCHOR BOLTS TO BE A307.
 - ALL OTHER BOLTS TO BE A325.
 - A325 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED UN.
 - A307 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED UN.
 - ALL WOOD STUDS TO BE SPF NO.2 OR BETTER.
 - ALL PLYWOOD TO BE D, FIR PLYWOOD TO CSA Q121 OR CANADIAN SOFTWOOD PLYWOOD TO Q151.
 - ALL OSB TO MEET CSA Q225.
 - ALL WOOD TO BE DRY SEASONED, WITH A MOISTURE CONTENT LESS THAN 15%.

- CONCRETE COVER**
 - FOOTINGS 75 mm BOTTOM 50 mm SIDES
 - WALLS 50 mm UNLESS NOTED OTHERWISE

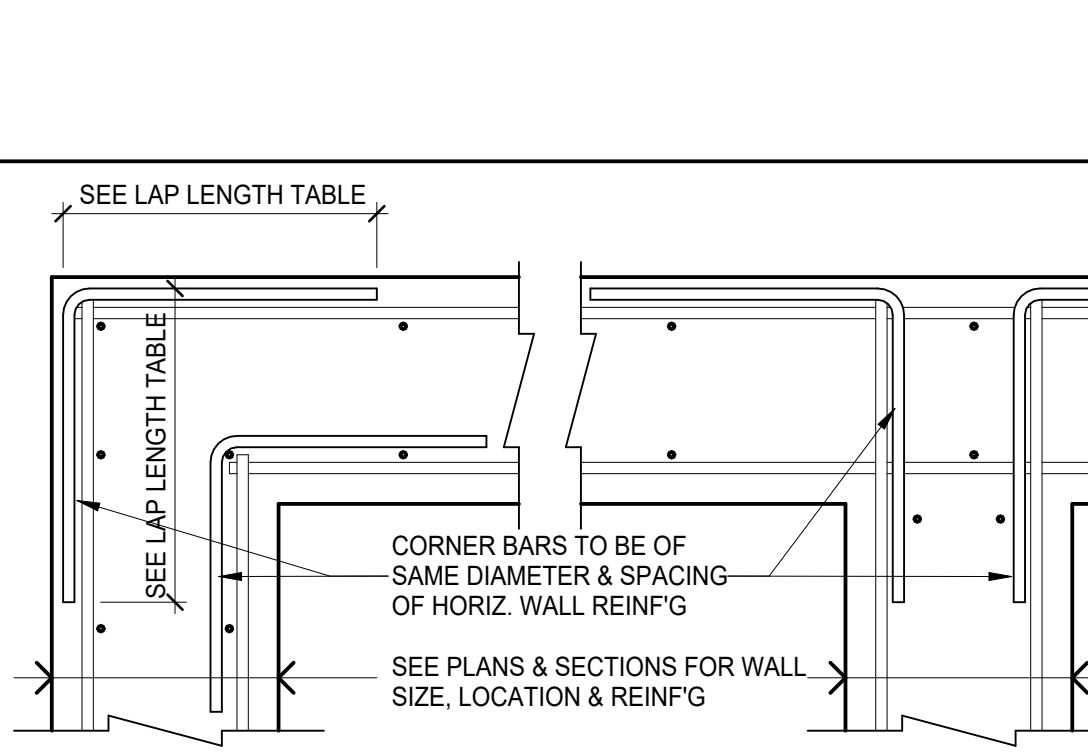
- DOWELS**
 - DOWELS TO FOOTINGS TO BE OF SAME DIAMETER AS THE LOWEST LIFT OF VERTICAL REINFORCING IN COLUMNS, PIERS OR WALLS.

- REINFORCING STEEL SPLICES**
 - REINFORCING STEEL SPLICES TO BE AS NOTED IN REINFORCING BAR LAP LENGTH TABLE UN.

- OPENINGS**
 - AT OPENINGS IN WALLS PROVIDE 2-20M T & B OF OPENING EXTENDING 600 mm MIN. BEYOND CORNERS OF OPENINGS.
 - REPORT ANY OPENINGS LARGER THAN 300 x 300 NOT SHOWN ON THESE DRAWINGS TO THE ENGINEER.

- LOADS**
 - ALL LOADS & FORCES INDICATED ON THESE DRAWINGS ARE UNFACTORED WORKING LOADS UNLESS NOTED.

- LEGEND**
 - B = BOTTOM
 - B1 = BOTTOM LOWER LAYER
 - B2 = BOTTOM UPPER LAYER
 - BL = BOTTOM LOWER LAYER
 - BBP1 = BEAM (OR DWS) BEARING PLATE NUMBER
 - BP1 = BASE PLATE NUMBER
 - BUL = BOTTOM UPPER LAYER
 - CONT = CONTINUOUS
 - DP = DEPTH
 - DWL = DOWELS
 - EE = EACH END
 - EF = EACH FACE
 - EL = ELEVATION
 - ES = EACH SIDE
 - EW = EACH WAY
 - F1 = PAD FOOTING NUMBER
 - H = HORIZONTAL
 - (H) = HOOKED BAR
 - O/C = ON CENTER
 - T = TOP
 - T1 = TOP UPPER LAYER
 - T2 = TOP LOWER LAYER
 - TLL = TOP LOWER LAYER
 - TUL = TOP UPPER LAYER
 - UN = UNLESS NOTED OTHERWISE
 - V = VERTICAL



TYPICAL WALL INTERSECTION REINFORCEMENT
CONCRETE WALLS WITH 2 SHEETS OF REINFORCING (WALL THICKNESS GREATER THAN 215 mm)
NOT APPLICABLE TO SHEARWALLS. SEE SHEARWALL ELEVATION DETAILS.

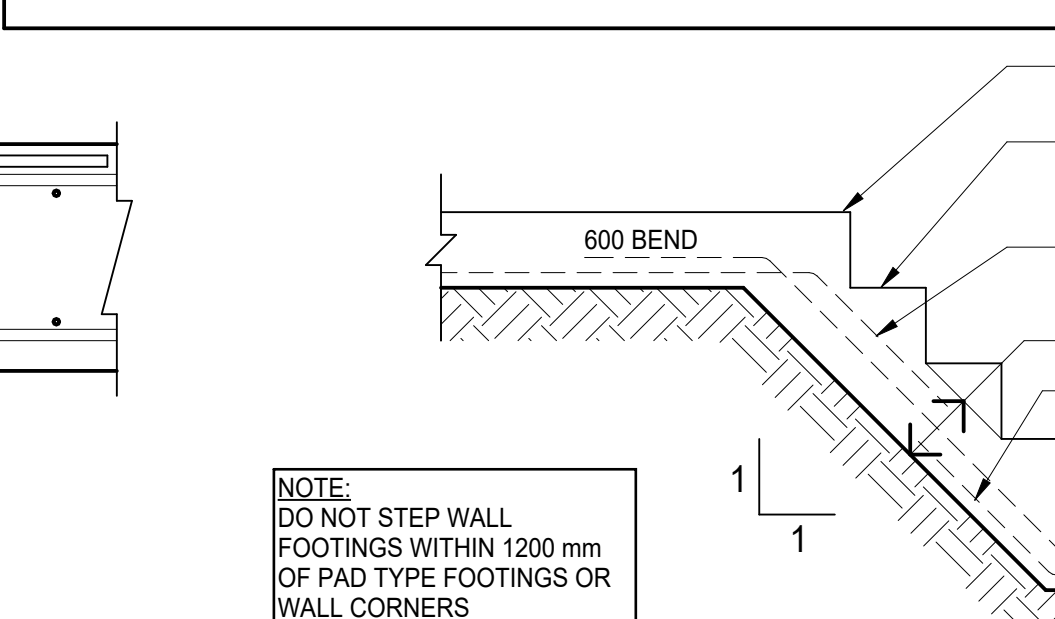
DESIGN & DETAILING CRITERIA FOR SUPPLIERS

- MISCELLANEOUS METALS & STEEL STAIRS**
 - MISC METALS & STEEL STAIRS ARE TO BE DESIGNED AND DETAILED BY MISC METALS & STEEL STAIRS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL MISC METAL & STEEL STAIR WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE MISC METALS & STEEL STAIRS DESIGN ENGINEER.

- GUARDS & HANDRAILS**
 - GUARDS & HANDRAILS ARE TO BE DESIGNED AND DETAILED BY STEEL SUPPLIER IN ACCORDANCE WITH THE CURRENT BUILDING CODE REQUIREMENTS. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL GUARDS & HANDRAIL WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE GUARD & HANDRAIL DESIGN ENGINEER. ALL GLASS IN GUARDS IS TO BE IN COMPLIANCE WITH S8-13 OF THE ONTARIO BUILDING CODE. GLASS GUARD DESIGN IS TO MEET THE REQUIREMENTS OF PART 4.1.5.14 OF THE ONTARIO BUILDING CODE. GLASS GUARDS ARE TO COMPLY WITH CANCGSS - 1220 - M89. PROVIDE A CONTINUOUS TOP RAIL ON ALL FREESTANDING GLASS GUARDS.

- TEMPORARY BRACING**
 - EACH TRADE SHALL SUBMIT TEMPORARY BRACING PLANS AND SAFE ERECTION PROCEDURES NECESSARY FOR THE COMPLETION OF THEIR WORK. THESE PLANS ARE TO BE COORDINATED WITH OTHER TRADES ON SITE TO ENSURE NO OVERLAP OR INTERFERENCE, AND SITE WORK IS NOT INTERRUPTED. DRAWINGS AND PROCEDURES ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, AND PROVIDED TO THE DESIGN TEAM FOR REVIEW. ALL TEMPORARY BRACING IS TO BE REVIEWED ON SITE BY THE BRACING DESIGN ENGINEER, AND REPORTS PROVIDED TO THE DESIGN TEAM. ALL BRACING REMOVAL IS ONLY TO OCCUR AFTER RECEIVING WRITTEN PERMISSION BY THE BRACING ENGINEER CONFIRMING THE FINAL DESIGN CONDITION HAS BEEN MET AND IS STABLE. SIGNOFF LETTERS ARE TO BE PROVIDED TO THE DESIGN TEAM. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING COORDINATION OF BRACING AND TEMPORARY WORKS MEASURES BY SUB TRADES.

- NOTE:**
 - INSPECTION REPORTS CREATED AS A RESULT OF THE ABOVE NOTED WORK MUST BE SUBMITTED TO THE CONSTRUCTION MANAGER. CONSTRUCTION MANAGER IS TO PROVIDE COPIES TO THE CONSULTANTS.



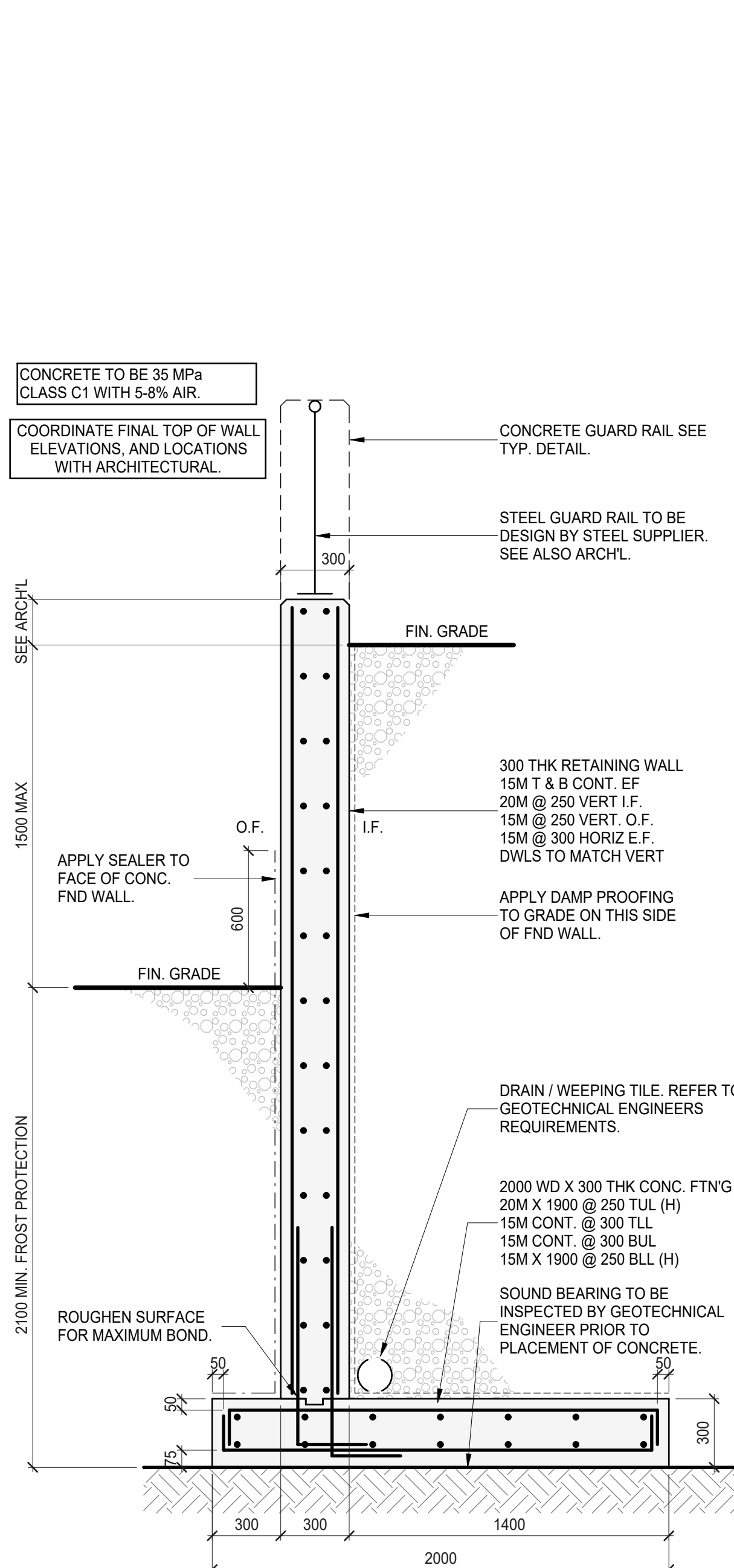
TYPICAL STEPPED WALL FOOTING DETAIL

REINFORCING BAR LAP LENGTH TABLE						
CONCRETE STRENGTH (MPa)	REINFORCING BAR LAP LENGTH (mm)					
	10M	15M	20M	25M	30M	35M
25	425	600	750	1200	1400	1675
30	400	550	675	1100	1275	1525
35	375	525	625	1000	1200	1425
40	350	475	600	950	1125	1325
45	325	450	550	900	1050	1250
50	300	425	525	850	1000	1200

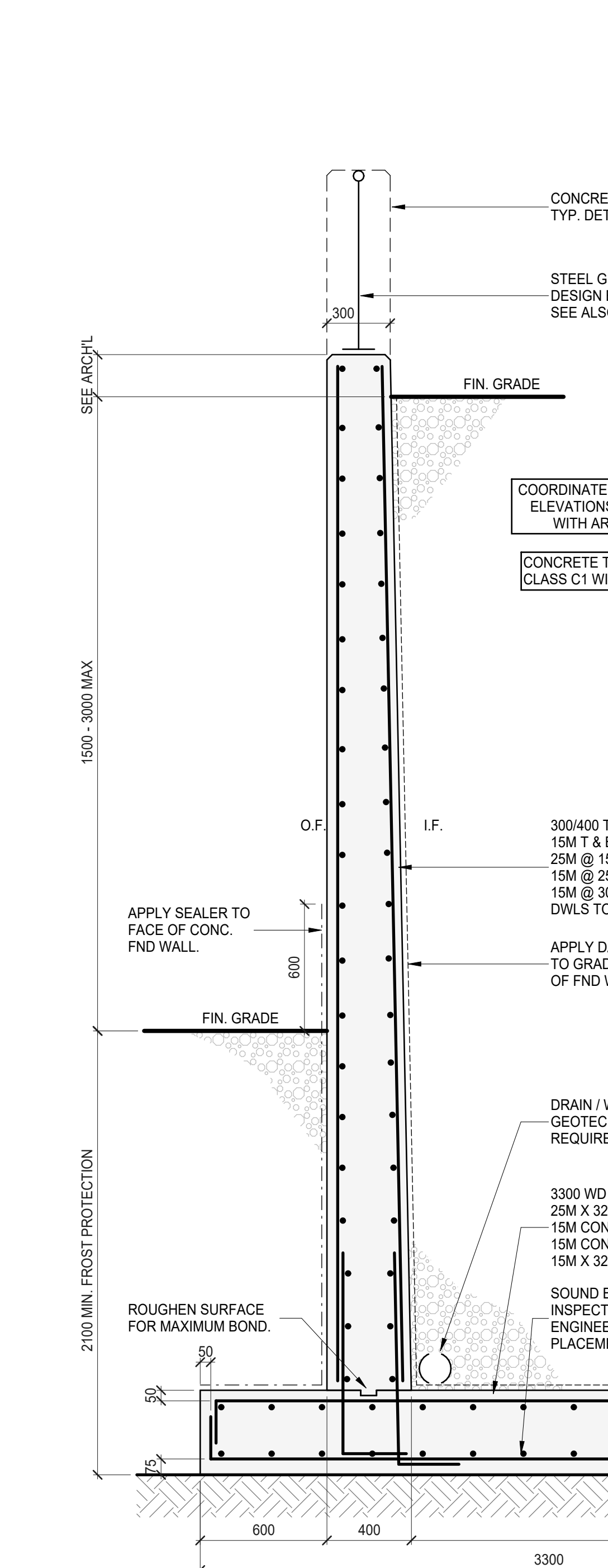
- Concrete Compressive Strength:**
 - 35 MPa With 5-8% AIR ENTRAINMENT. (CLASS C1 EXPOSURE)

- NOTE:**
 - USE NEW FORM PLY ON ALL EXPOSED ARCH'L FINISHES. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHES AND REVEALS.

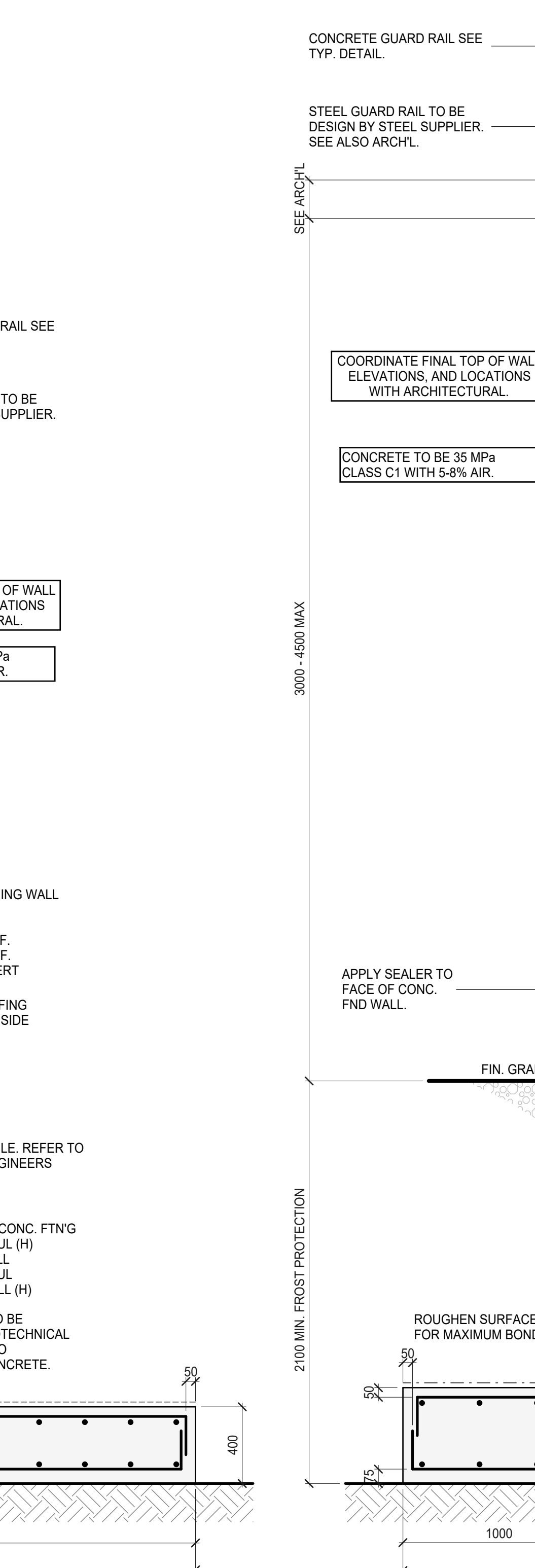
- REFER TO GEOTECHNICAL REPORT FOR ANY ADDITIONAL INSULATION REQUIREMENTS WHERE FROST PROTECTION IS LESS THAN 2100mm**



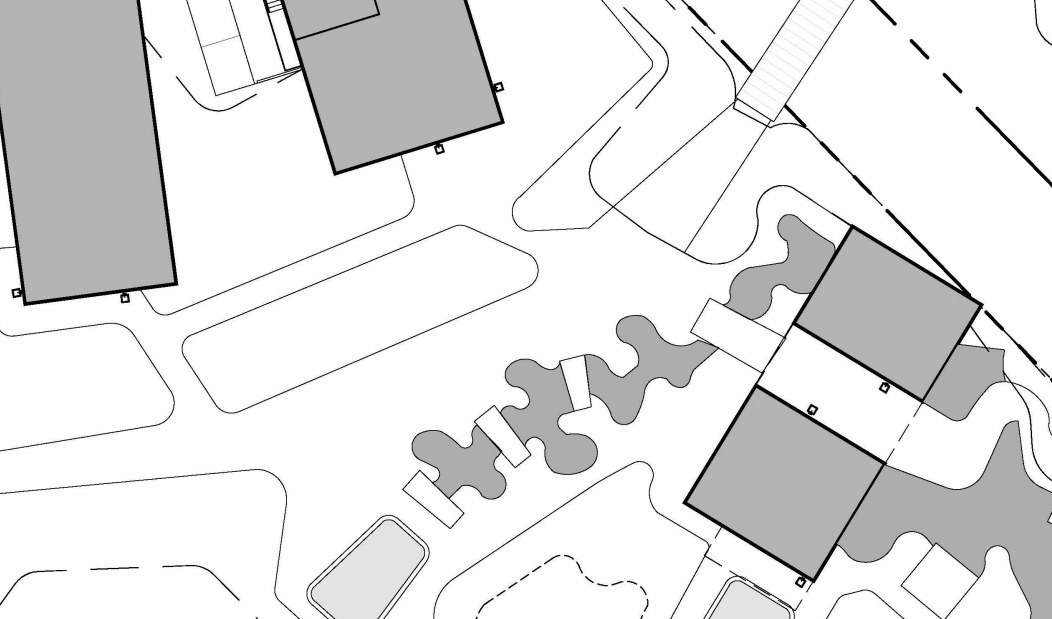
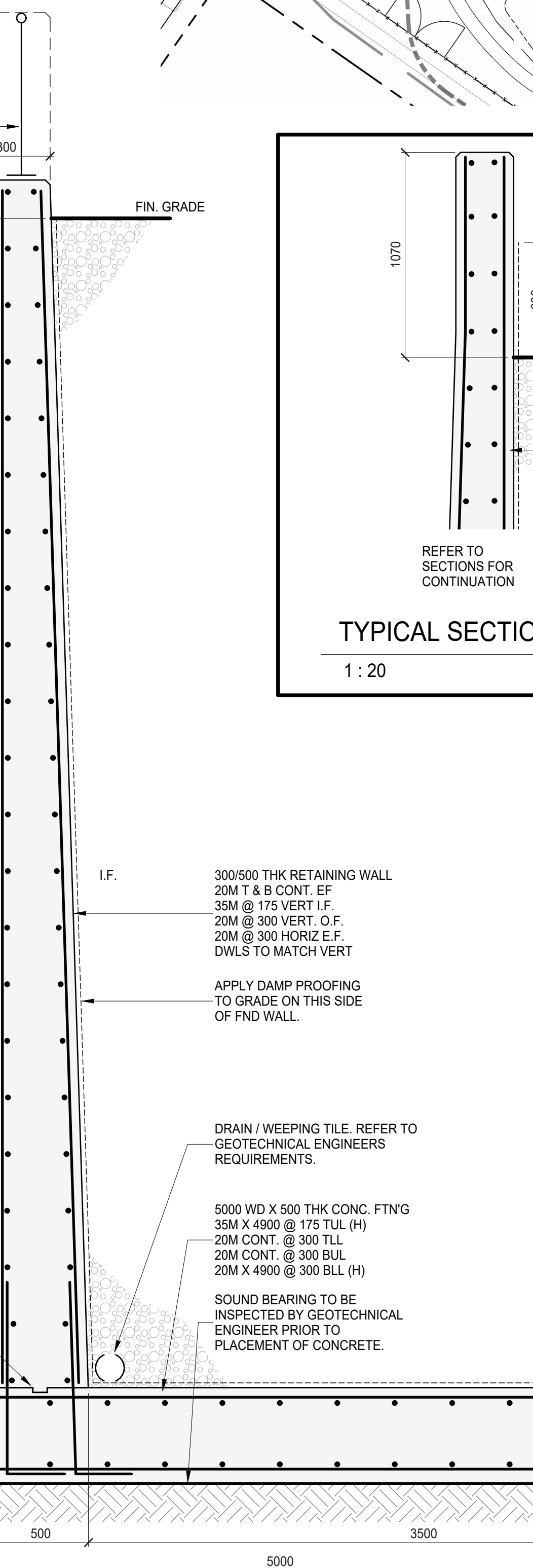
TYPICAL WALL CONSTRUCTION JOINT DETAIL
MAXIMUM SPACING OF CONSTRUCTION JOINTS TO BE 20 meters



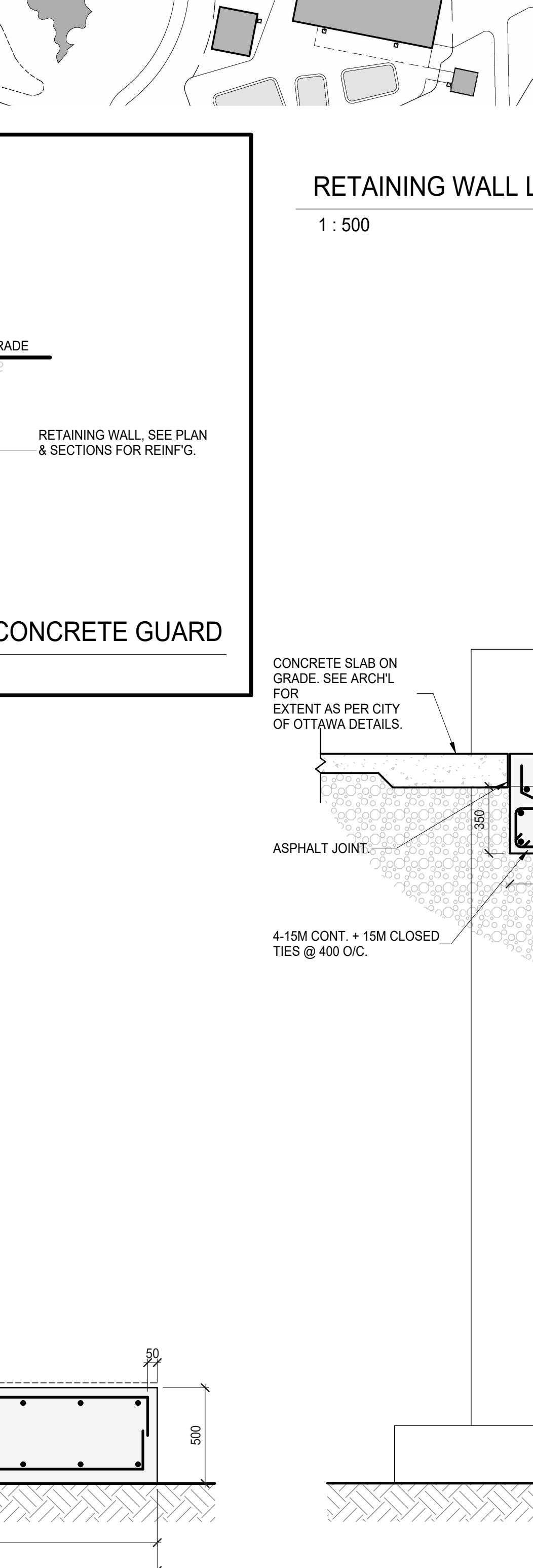
TYPICAL CONCRETE RETAINING WALL DETAILS



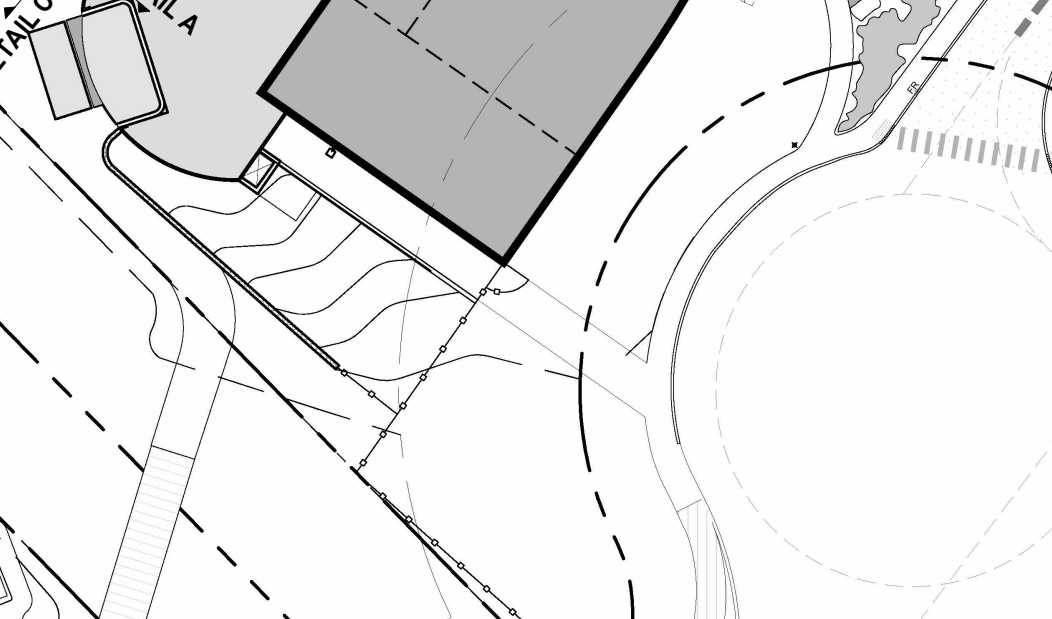
TYPICAL STEPPED WALL FOOTING DETAIL



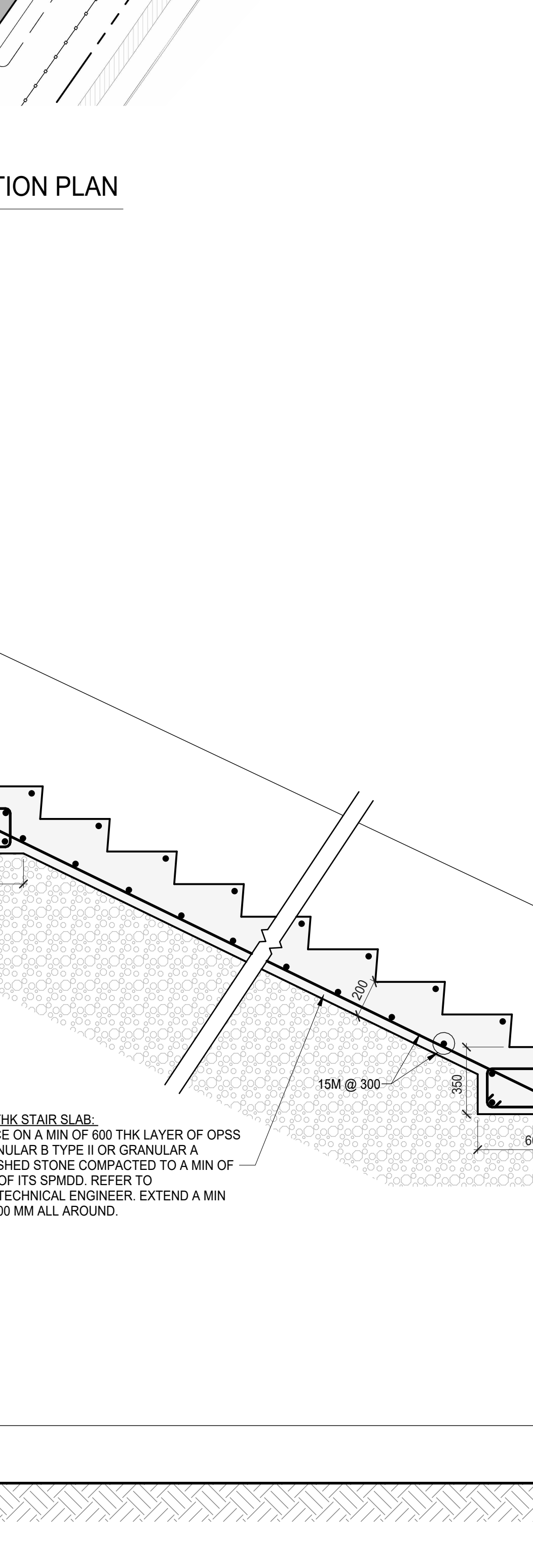
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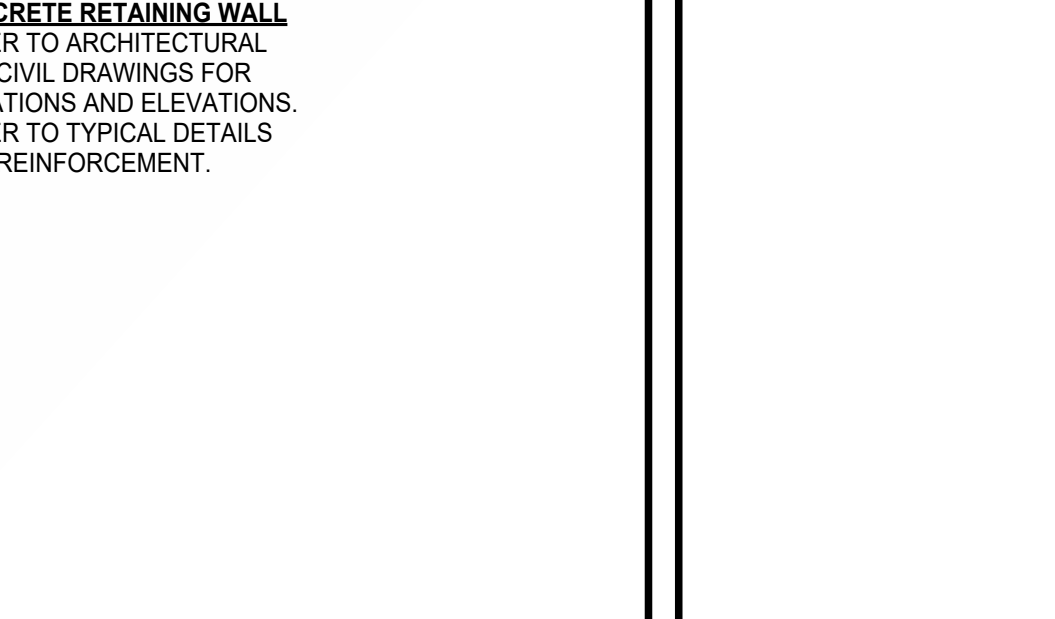
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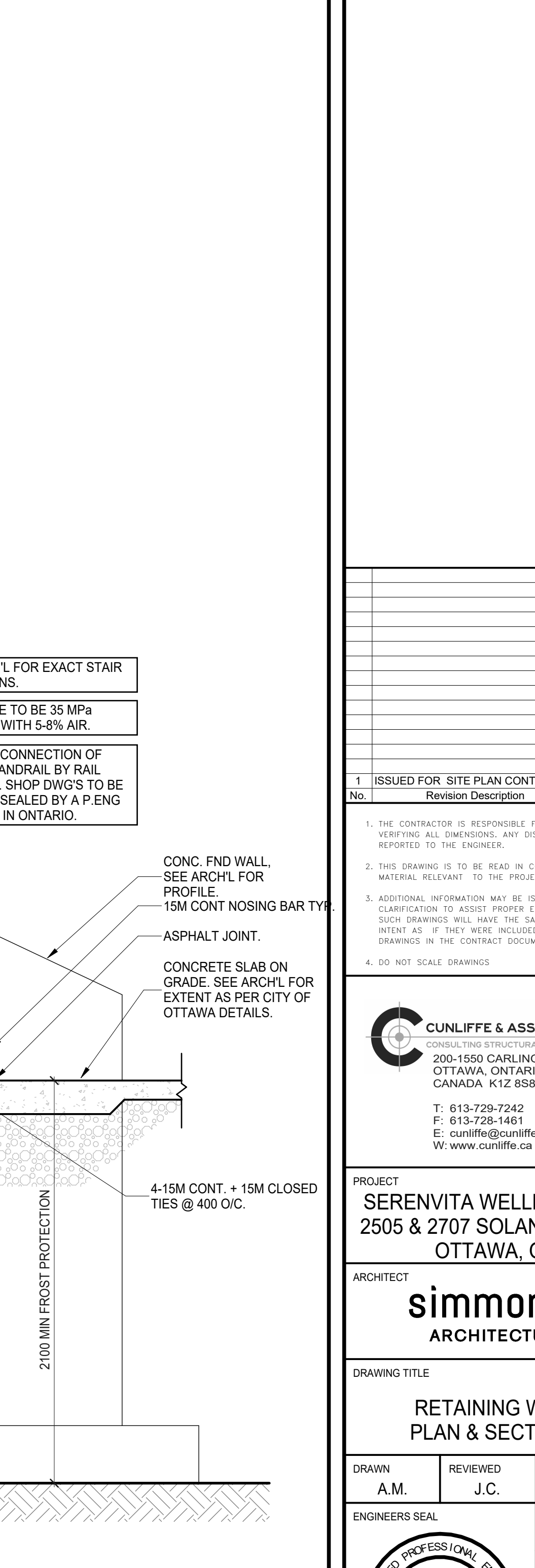
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TYPICAL CONCRETE RETAINING WALL DETAILS



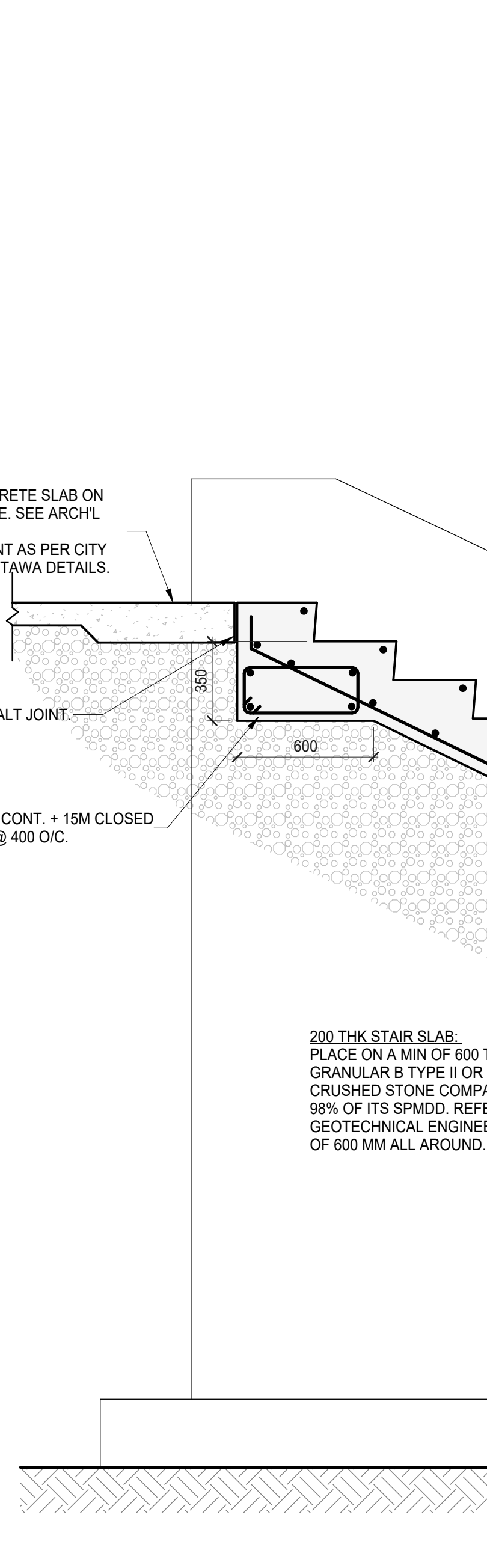
TYPICAL STEPPED WALL FOOTING DETAIL



TYPICAL CONCRETE RETAINING WALL DETAILS

RETAINING WALL LOCATION PLAN

1 : 500



DETAIL A

1	ISSUED FOR SITE PLAN CONTROL	2026-01-14
2	REVISION DESCRIPTION	DATE
3	THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AND VERIFYING ALL DIMENSIONS AND DISCREPANCY SHALL BE REPORTED TO THE ENGINEER.	
4	ADDITIONAL INFORMATION MAY BE REQUESTED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SHOP DRAWINGS WILL HAVE THE SAME MEANING AND INTENT AS IF THEY WERE INCLUDED WITH THE DRAWINGS IN THE CONTRACT DOCUMENT.	
5	THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL MATERIAL RELEVANT TO THE PROJECT.	
6	DO NOT SCALE DRAWINGS	
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PROJECT SERENVITA WELLNESS SPA 2505 & 2707 SOLANDT ROAD, OTTAWA, ON		
ARCHITECT simmonds ARCHITECTURE		
DRAWING TITLE RETAINING WALL PLAN & SECTIONS		
DRAWN A.M.	REVIEWED J.C.	SCALE As indicated
ENGINEERS SEAL 2026-01-14 J.C. CUNIFFE 100187411		PROJECT No. 25-151
SHEET No. SP.S1		REVISION No. 1

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