

**JLR No.: 26299-006.1**

**Revision: 0**

Prepared for:

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**July 19, 2022**

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# Site Servicing Report

## Arcadia Stage 6 450 Huntmar Drive



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ENGINEERS · ARCHITECTS · PLANNERS

# Site Servicing Report

## Arcadia Stage 6

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### 1.0 INTRODUCTION

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#### 1.1 General

In 2022, J.L. Richards & Associates Limited (JLR) was retained by Minto Communities Inc. (Minto) to prepare the detailed design of municipal infrastructure for Site Plan Approval (SPA) of Arcadia Stage 6. This Site Servicing Report (SSR) presents the servicing constraints and strategies for water, wastewater, stormwater servicing, and stormwater management in accordance with the City of Ottawa Design Guidelines, the associated technical bulletins and relevant design excerpts. This SSR also includes strategies for implementing erosion and sedimentation control measures throughout the construction phase of the project.

#### 1.2 Site Description

Minto's Arcadia Stage 6 is located within the City of Ottawa's Official Plan boundary and consists of a ±5.58 ha parcel bounded by Campeau Drive and Arcadia Stage 3 to the north, Campeau Drive SWMF and Donum Lane to the east, Country Glen Way to the west and by the Light Rail Transit (LRT) / Feedmill Creek to the south (refer to Figure 1-1: Location Plan). The legal description of the subject property is Part of Block 2, Registered Plan 4M-1563 and Part of Lot 3, Concession 1 (Geographic Township of March), City of Ottawa (refer to Appendix A1 for the Legal Plan)

A topographical survey was completed by Stantec Inc. in May 2022 (Appendix A1). The survey indicates that the existing ground surface contains fill piles and generally slopes downwards in a northeasterly direction towards Donum Lane.

#### 1.3 Proposed Development

The proposed development will consist of 368 residential units, one public parkette (0.56 ha) and two (2) amenity blocks (0.087 ha and 0.113 ha). Overall, the site will feature 11 Executive Towns, 80 Avenue Towns, 13 Urban Towns and 264 Metro Towns. The Concept Plan for Arcadia Stage 6 is attached to Appendix A1.

#### 1.4 Proposed Connections to Existing Infrastructure

A review of existing services was completed along both frontages of the subject property to identify existing sewers and watermains to service the development. The proposed connections to the existing infrastructure consists of the following (refer to Appendix A3 for a copy of the background drawings):

##### Watermain

- East: Connection to existing 305 mm diameter PVC watermain along Donum Lane.
- West: Connection to existing 305 mm diameter PVC watermain along Country Glen Way.

##### Sanitary

- East: Removal and relocation of existing maintenance hole on Donum Lane and connection using 375 mm pipe.

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### **Storm**

- East: Removal and relocation of existing maintenance hole on Donum Lane and connection using 1500 mm pipe.
- West: Connection to existing maintenance hole on Country Glen Way using 900 mm pipe. The existing 600 mm pipe initially intended to service the proposed development will be removed.

The existing watermain, storm and sanitary stubs on Donum Lane will be removed.

### **1.5 Consultation and Permits**

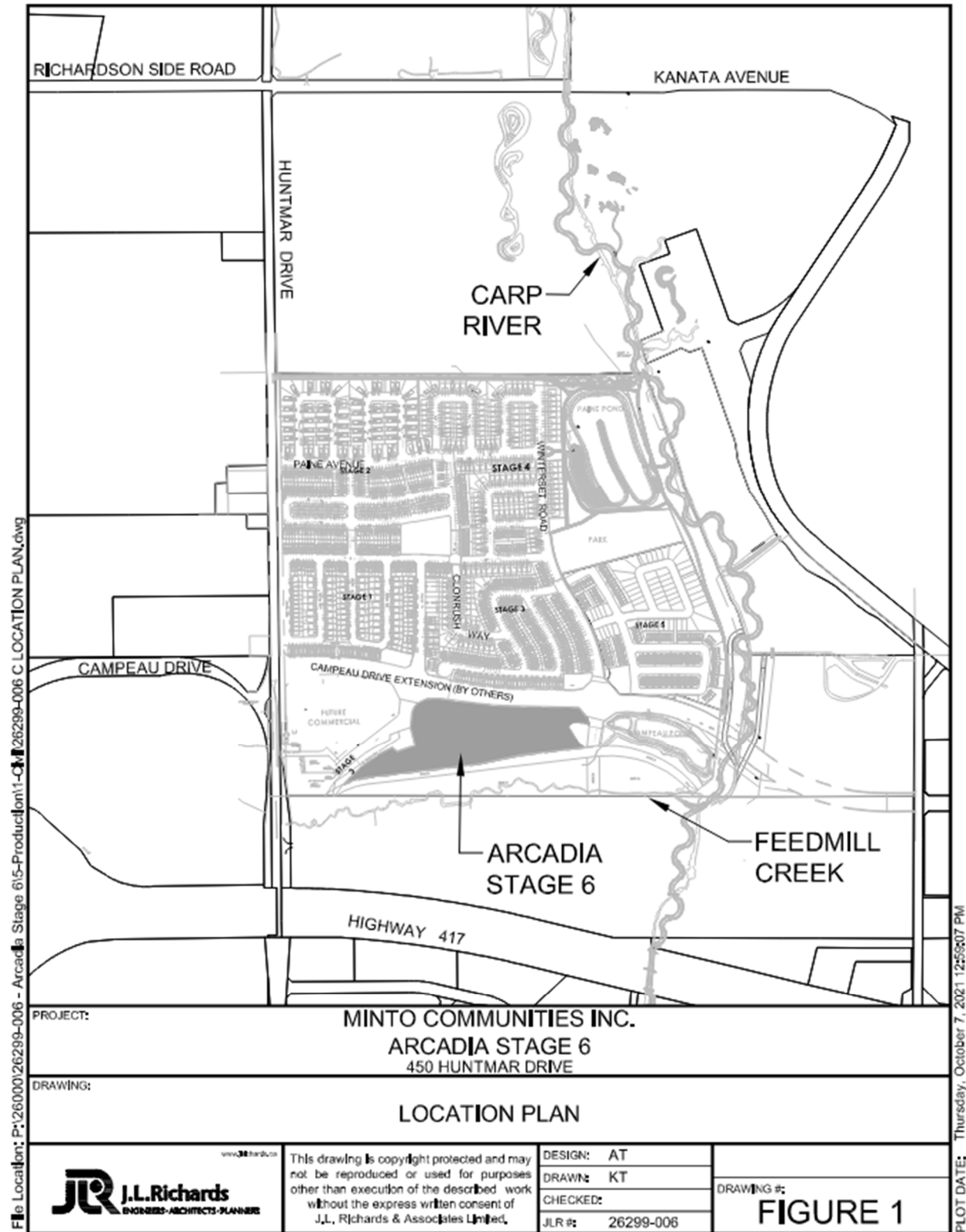
A pre-consultation meeting was held on September 2, 2021, to discuss the planning process, design criteria, and servicing constraints. A copy of the pre-consultation meeting notes has been provided in Appendix A2.

As stated during the pre-consultation meeting, Stage 6 will be a Site Plan Control Application to the City of Ottawa. Existing structures are in place at the East and West side of the property line. These structures will be removed and/or relocated. As noted in the Servicing Drawings (S1 and S2), Stage 6 will have two storm, one sanitary and two watermain connections. An Environmental Compliance Approval (ECA) will be necessary to meet the Ministry of Environment, Conservation and Parks (MECP) requirements.

In addition, a Servicing Study Checklist has been included in Appendix A4 of this report. The checklist provides all the details associated with this development as well as the approval and permit requirements.

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Figure 1-1: Location Plan



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## 2.0 WATER SERVICING

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### 2.1 Water Supply Design Criteria

A Hydraulic Network Analysis (HNA) was carried out to confirm the site's watermain sizing and to demonstrate its compliance to the Ottawa Design Guidelines for Water Distribution (July 2010) and Technical Bulletins ISDTB-2014-02, ISTB-2018-02 and ISTB-2021-03. These documents are herein referred to as the Design Guidelines and TB-2014-02, TB-2018-02, and TB-2021-03, respectively.

Section 4.2.2 of the Design Guidelines states the following criteria for development additions to the public water distribution system:

- Under maximum hourly demand conditions (peak hour), the residual pressures shall not be less than 276 kPa (40 psi);
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 140 kPa (20 psi);
- In accordance with the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi);
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi); and
- Feeder mains, which have been provided primarily for the purpose of redundancy, shall meet, at a minimum, the basic day plus fire flow demand.

### 2.2 Domestic Water Demands

The estimated domestic water demands presented in this section are based on the site layout and unit count proposed in the Concept Plan (Appendix A1). Since receiving the boundary conditions from the City (Appendix B2), the number of units has been reduced from 409 to 368. The proposed development now consists of 104 townhouses (11 Executive Towns, 80 Avenue Towns and 13 Rear Lane Towns) and 264 duplexes (Metro Towns).

The residential consumption rate for average day demand was set in accordance with the City's TB-2021-03. Table 2-1 summarizes the water consumption rates and total estimated water demands used in the HNA. Calculated in accordance with Section 4.2.8 of the Design Guidelines, the detailed water demand distribution is presented in Appendix B1.

**Table 2-1: Water Demands**

<b>Demand Scenario</b>	<b>Water Consumption or Peaking Factor</b>	<b>Residential Demand (L/s)</b>
Average Day Demand	280 L/c/d	2.88
Maximum Day Demand	2.5 x Avg Day	7.20
Peak Hour Demand	2.2 x Max Day	15.83

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### 2.3 Fire Flow Requirements

The City has specified that the Fire Underwriters Survey (FUS) method shall be used for any public or private site where new fire hydrants are being designed. Specifically, the required fire flow (RFF) for each structure was calculated in accordance with TB-2018-02. Several firewalls were specified throughout the development to limit the maximum RFF to 15,000 L/min (250 L/s) in accordance with the boundary conditions received from the City of Ottawa. Critical fire areas for Arcadia Stage 6 are presented in Table 2-2.

**Table 2-2: Fire Flow Requirements**

Location	Block Number	Calculated Fire Flow L/min (L/s)
Critical Fire Area 1	Block 14	15,000 (250)
Critical Fire Area 2	Block 15	15,000 (250)
Critical Fire Area 3	Block TE-05	15,000 (250)
Critical Fire Area 4	Block TE-11	15,000 (250)

Refer to Appendix B1 for the detailed RFF calculations for the critical fire areas.

### 2.4 Proposed Water Servicing, Boundary Conditions and Water Model

#### 2.4.1 Proposed Water Servicing

The proposed water servicing for Arcadia Stage 6 includes a private 203 mm watermain loop connected to the following existing watermains:

- Connection-1: the existing 305 mm watermain south of the intersection of Donum Lane and Campeau Drive; and
- Connection-2: the existing 305 mm watermain stub south of the intersection of Country Glen Way and Campeau Drive.

The water demands will be supplied by local 203 mm PVC watermains and 50 mm PEX water services. All units will be provided with an individual water service from the front except for the duplex blocks with underground parking, which will be provided with a shared water service into the mechanical room for each complex. A 50 mm PEX water service will be extended from the mainline to provide domestic water service to units which do not have direct access to the 203 mm watermains from the front. Refer to Drawing S1 and S2 for the water servicing layout.

#### 2.4.2 Boundary Conditions

Hydraulic boundary conditions were provided by the City at the two proposed connection locations (Connection-1 and Connection-2) listed in Section 2.4.1 above. Tables 2-3 summarizes the hydraulic boundary conditions received (refer to Appendix B2 for a copy of the City correspondence).



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**Table 2-3: Hydraulic Boundary Conditions**

Demand Scenarios	Connection-1 Head (m)	Connection-2 Head (m)
Maximum HGL	161.3	161.3
Peak Hour	156.3	156.3
Max Day plus Fire 1 (167 L/s)	153.8	151.5
Max Day plus Fire 2 (250 L/s)	150.5	145.5

### 2.4.3 Water Model

A hydraulic water model within the WaterCAD® software platform was used to carry out the HNA (refer to the overall schematics presented in Appendix B3). The water demands from Table 2-1 and the boundary conditions from Table 2-3 were input into the model for each demand scenario. Table 2-4 summarizes the watermain diameters and roughness coefficients used in the model, based on Sections 4.2.12 and 4.3.5 of the Design Guidelines.

**Table 2-4: Watermain Internal Diameters and C-Factors**

Nominal Diameter	Inside Diameter	C-Factor
50 mm	50 mm	100
150 mm	155 mm	100
200 mm	204 mm	110
300 mm	297 mm	120

## 2.5 Simulation Results

The HNA was carried out under steady-state peak hour, maximum day plus fire flow, and maximum pressure conditions to confirm that the proposed water servicing can meet the design criteria outlined in Section 2.1.

### 2.5.1 Peak Hour

The simulation results found the minimum pressure at the site during the peak hour condition to be 528 kPa (76.6 psi) (refer to Appendix B4), which exceeds the minimum pressure criterion of 276 kPa (40 psi) per the Design Guidelines.

### 2.5.2 Maximum Day Plus Fire Flow

Fire water supply will be provided by hydrants located along the 203 mm watermains. Hydrant spacing was carried out in accordance with the Design Guidelines.

To ensure adequate fire protection, the maximum day demand shown in Table 2-1 was analyzed simultaneously with the fire flow requirements. The fire flow simulation was

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carried out by allowing WaterCAD® to calculate the maximum fire flow that can be drawn from each hydrant without allowing any part of the system to experience pressures less than 140 kPa (20 psi). Except for hydrant H-6, it is expected that the targeted fire flow of 15,000 L/min (250 L/s) can be provided throughout the site (refer to Appendix B5). Contributing fire hydrants were assessed for each structure within the site to confirm that adequate water supply is available per Appendix I of TB-2018-02.

As hydrant H-6 was only able to provide an available fire flow of 247 L/s, fire flow demands of 63 L/s were manually applied to the nearby hydrants (H-4, H-5, H-6 and H-7) to confirm if the hydrants could provide the RFF (250 L/s) while achieving the minimum pressure requirement of 140 kPa (20 psi). The results indicated that the hydrants were able to provide the RFF for Block TE-05 while maintaining the minimum pressure requirement throughout the site (refer to Appendix B5).

#### 2.5.3 Maximum Pressure

Based on a zero (0 L/s) demand condition, the simulation results found the pressures at the site during the maximum pressure condition to range between 604 kPa (87.6 psi) and 648 kPa (94.0 psi) (refer to Appendix B6). Since these values exceed the maximum pressure constraint of 552 kPa (80 psi) per the Design Guidelines, all units within Arcadia Stage 6 will require pressure reducing valves (PRVs).

## 2.6 Summary and Conclusions

Based on the water simulation results, the proposed development can be serviced by a 203 mm watermain loop, 203 mm local watermains and 50 mm water service extensions as shown on Drawing S1 and S2. Simulation results under peak hour demand and maximum pressure conditions showed that the design criteria can be achieved with the installation of PRVs for all of the units within the site. Furthermore, adequate fire water supply can be achieved with the proposed servicing.

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### 3.0 WASTEWATER SERVICING

#### 3.1 Background

In accordance with the Kanata West Master Servicing Study (KWMSS), wastewater servicing in Arcadia Stage 6 is designed to outlet to the existing 675 mm diameter gravity sanitary sewer on Campeau Drive. Sanitary sewage will then be conveyed by gravity to the Signature Ridge Pump Station (SRPS) which, in turn, will eventually outlet to the Robert O. Pickard Environmental Centre where wastewater is processed and treated prior to discharge into the Ottawa River.

#### 3.2 Design Criteria

The sanitary sewer system within Arcadia Stage 6 is designed in accordance with the Ottawa Sewer Design Guidelines and subsequent technical bulletins. The design parameters are applied under two scenarios as per ISTB Technical Bulletin 2018-01. In addition to the typical design values, annual values are used for the simulation of the system with failure of the pump station and operation of the overflows. The simulation of the pump station failure should show that the HGL of the sanitary system remains below the underside of footings due to the operation of the overflows. The key design parameters have been summarized in Table 3-1.

**Table 3-1: Wastewater Key Design Parameters**

Design Parameter	Design Value	Annual Value
Duplex <sup>1</sup> Population Density	2.3 ppu	Same as design
Row Townhouse <sup>2</sup> Population Density	2.7 ppu	Same as design
Residential Average Flow	280 L/Cap/Day	200 L/cap/day
Residential Peaking Factor	Harmon's Formula	Same as design
Harmon's Correction Factor (K)	0.8	0.6
Infiltration Allowance	0.33 L/s/ha	0.3
Manning's Roughness Coefficient (n)	0.013	0.013
Allowable Slopes	Varies (Refer to Section 6.1.2.2 of ODSG)	-
Allowable Velocities	0.6 m/s – 3.0 m/s	-
Allowable Freeboard	-	> 0 m

(1) The product "Metro Towns" are duplex units.

(2) The products "Rear Lane (or Urban) Towns", "Executive Towns", and "Avenue Towns" are row townhouse units.

#### 3.3 Proposed Sanitary Servicing and Design Flows

Wastewater generated from Arcadia Stage 6 will be conveyed via a local 200 mm diameter sanitary sewer system that will discharge into the existing 375 mm sewer on Donum Lane as shown in the Servicing Drawings (S1-S2).

Wastewater flows from the proposed development are presented in the Arcadia Stage 6 Sanitary Design Sheet (refer to Appendix C1). Based on the design criteria (Table 3-1) and the site constraints, a total design peak flow of 11.24 L/s is calculated for the development. Table 3-2 summarizes the results from the sanitary design sheet.

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**Table 3-2: Sanitary Design Flow Summary**

Unit Type	Site Area	Unit Count	Unit Density	Average Flow	Res. Peak Flow	Infilt. Flow	Total Flow
Duplex	5.58 ha	264	2.3 ppu	280 L/cap/day	9.40 L/s	1.84 L/s	11.24 L/s
Row Townhouse		104	2.7 ppu				

The peak wastewater flow calculated for the proposed Arcadia Stage 6 development is 11.24 L/s as shown in Table 3-2. This is based on a total population of 888 people. The sanitary design spreadsheet prepared by IBI (Appendix C2) shows that a flow of 4.80 L/s was allocated for the western portion of Stage 6 and a flow of 20.06 L/s was allocated for a 24.3 ha land parcel which included the eastern portion of Stage 6. This amounts to a pro-rated flow allocation of 1.75 L/s for the eastern portion. Thus, the total flow allocation for the entire Stage 6 development is 6.55 L/s (4.80 L/s + 1.75 L/s).

It is noted that previously, the sanitary flows from this site were to be split between the western portion discharging to Country Glen Way and the eastern portion discharging to Donum Lane. Though two outlets were identified for this site, the flows were anticipated to quickly converge along Campeau Drive into the same sewer located northeast of the site at the Donum Lane/Winterset Road/Campeau Drive intersection (i.e., at ex. MH 307A). Although the calculated peak flow for Arcadia Stage 6 is 11.24 L/s which is greater than the original combined allocated flow of 6.55 L/s the following points discuss the downstream capacities:

- 1) The Donum Lane detailed design sheets completed as part of Arcadia Stage 3 & 4, show that there is sufficient residual downstream capacity to accept the 11.24 L/s from Arcadia Stage 6 on Donum Lane.
- 2) The design sheets from the KWMSS for Campeau Drive (Appendix C2) indicate that there is sufficient residual capacity in the downstream system up to the signature ridge pumping station to accommodate the increase in flow.

Given there is sufficient residual capacity in the sewer system, it is proposed to adopt the sanitary servicing strategy described in this section.

### 3.4 Overflow and Sanitary Hydraulic Grade Line Analysis

Protection against basement flooding within the existing Arcadia Stages 1 to 4 is currently provided by an existing overflow that outlets to the Paine Pond stormwater management facility along with other overflows in the wastewater sewer network. No new overflows are proposed for additional basement protection for Stage 6.

The hydraulic grade line (HGL) analysis carried out for the detailed design of Stage 4 (latest analysis dated February 2020) demonstrated that the design criterion for freeboard was met within the system for Arcadia Stages 1 to 4, including a flow allowance from Stage 6. An updated HGL analysis has been carried out to confirm the HGL within the Stage 6 development.

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The HGL analysis was completed using the PCSWMM software platform. The HGL analysis was based on the Signature Ridge HGL analysis completed by IBI in September 2014 with the following revisions:

- Peak wastewater flows were calculated in accordance with the parameters prescribed in Technical Bulletin ISTB-2018-01 and based on residential unit counts and land uses per the current proposed Arcadia Development (Appendix 'A1').
- The local sanitary sewers in Stage 6 were added to the model.
- Land use and residential unit counts were updated using design documents from Arcadia Stages 1, 2 and the Arcadia Retail Development. Appendix C references all applicable documents. Allocations are shown on Drawing OSAN.
- Populations and land uses on the eastern side of the Carp River were maintained as per the 2014 HGL analysis; however, wastewater flows were recalculated based on the parameters in the Technical Bulletin ISTB-2018-01.
- Existing overflows in the model on the East side of the Carp River were maintained in the model although all sanitary sewers were updated as per the GeoOttawa information.
- As per the recent constructed works implemented in Stage 4, an emergency sanitary sewer overflow was included at the Paine SWMF at the 1:25 year design elevation of 93.37 m.

The revisions to the sewershed areas are shown in the marked-up Figures from the September 2014 report (Appendix C8) along with the revised Sanitary Design Sheets for the Dry Weather Flow and Wet Weather Flow with the Annual Parameters. Table 3-4 summarize the wet and dry weather flows for Arcadia Stage 6 under the annual event.

The resulting flows at the overflows are listed in Table 3-3 below using the Annual Parameters for the Dry Weather Flow condition and the Wet Weather Flow Condition. The values can be summed to give the equivalent flow to the SRPS when the pump is operating.

**Table 3-3: Sanitary Annual Flows at Overflow Locations**

<b>Overflow Location</b>	<b>Overflow Elevation (m)</b>	<b>Dry Weather Flow (l/s)</b>	<b>Wet Weather Flow (l/s)</b>
Paine SWMF	93.37	29	42
SRPS Emergency Overflow	93.70	97	244
Richardson Ridge Overflow	94.10	6	34
<b>Total Flow</b>	-	132	319

The simulated HGL elevations were then compared to underside of footing (USF) elevations of Stage 4 and Stage 3. It should be noted that the USF used in this HGL analysis are generally lower than those assumed as part of the 2013 IBI HGL analysis due to changes in the development criteria of the site during the design phases.

Table 5 6 displays the freeboard under dry and wet weather flows. Results in Table 5 6 indicate that a minimum freeboard greater 0.01m will be achieved throughout Stage 6. HGL levels for conceptual future stages can be found in Appendix 'C8'.

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**Table 3-4: Freeboard from Sanitary HGL under Pumping Station Failure**

Manhole ID	Underside of Footing (m)	DWF Max HGL (m)	DWF Freeboard (m)	WWF Max HGL (m)	WWF Freeboard (m)
100A	94.24	93.92	0.32	94.20	0.04
101	94.24	93.92	0.32	94.20	0.04
101A	94.24	93.92	0.32	94.21	0.03
101B	94.24	93.92	0.32	94.21	0.03
102	94.24	93.92	0.32	94.21	0.03
102A	94.44	93.93	0.51	94.21	0.23
102B	94.62	93.93	0.69	94.22	0.40
103	94.24	93.92	0.32	94.21	0.03
104	94.24	93.92	0.32	94.21	0.03
105	94.24	93.92	0.32	94.21	0.03
106	94.69	93.93	0.76	94.22	0.47
107	94.80	93.93	0.87	94.22	0.58
109	94.80	93.93	0.87	94.23	0.57
110	94.80	93.93	0.87	94.23	0.57
110A	94.80	93.93	0.87	94.23	0.57
110B	94.80	93.93	0.87	94.23	0.57
111	95.10	93.93	1.17	94.23	0.87
112	95.10	93.93	1.17	94.23	0.87
113	95.10	93.93	1.17	94.23	0.87
114	95.15	93.93	1.22	94.23	0.92
114A	95.30	93.93	1.37	94.23	1.07
115	95.43	95.06	0.37	95.07	0.36
116	95.75	93.94	1.81	94.23	1.52
117	95.85	93.94	1.91	94.24	1.61
118	95.50	93.94	1.56	94.24	1.26
119	96.14	94.10	2.04	94.24	1.90
120	96.14	94.17	1.97	94.17	1.97
121	96.14	94.16	1.98	94.24	1.90
122	96.14	95.39	0.75	95.40	0.74
123	96.14	95.05	1.09	95.06	1.08
124	96.14	95.20	0.94	95.20	0.94

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Manhole ID	Underside of Footing (m)	DWF Max HGL (m)	DWF Freeboard (m)	WWF Max HGL (m)	WWF Freeboard (m)
124A	96.14	95.47	0.67	95.47	0.67

The table presented below displays the freeboard values for existing Arcadia Stages along with existing downstream infrastructure up to the Signature Ridge Pumping Station. The information summarized below provides a comparison of the updated results from the current JLR model to those presented in the SRPS report.

**Table 3-5: 2014 Signature Ridge Pumping Station (IBI Group)**

IBI 2014 Signature Ridge Pumping Station Model Parameters					JLR Model Results			
NODE ID	Ground Elevation (approx USF elev.) (m)	USF Elevation	Ultimate Buildout Scenario HGL(m)	Min. FB to ground (FB to approx. USF elev.) (m)	JLR Model Node	JLR HGL (m)	JLR Revised Minimum Freeboard to USF or Ground Elevation (m)	Difference from IBI
<b>Campeau Drive</b>								
1	103.5		99.53	3.97	MHSA66062	96.99	6.51	-2.54
2		96.68	95.47	1.21	MHSA66066	94.44	2.24	-1.03
2B		95.14	94.76	0.38	MHSA65328	94.27	0.87	-0.49
14	99.5		94.64	4.86	MHSA65349	94.19	5.31	-0.45
14A	96		94.52	1.48	MHSA65349	94.19	1.81	-0.33
3		94.8	94.5	0.3	MHSA65349	94.19	0.61	-0.31
4	94.86		94.44	0.42	MHSA65352	94.15	0.71	-0.29
5		94.2	94.23	-0.03	MHSA65123	94.12	0.08	-0.11
12465	95.7		94.27	1.43	MHSA12465	94.12	1.58	-0.15
<b>South of Highway 417</b>								
15	97.6		94.77	2.83	MHSA43765	94.26	3.34	-0.51
<b>Didsbury Road</b>								
20011	97.55		94.29	3.26	MHSA20011	94.13	3.42	-0.16
12461	96.05		94.26	1.79	MHSA12461	94.12	1.93	-0.14
Prop 5E	94.45		94.17	0.28	MHSA69564	94.11	0.34	-0.06
Prop5D	94.38		94.11	0.27	MHSA69565	94.1	0.28	-0.01
Prop 5C	94.33		94.06	0.27	MHSA69566	94.08	0.25	0.02
5A	95.15		94.03	1.12	MHSA65995	94.07	1.08	0.04

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Arcadia Stage 1 and 2								
MH9			94.79		MHSA69409	94.29		-0.5
MH41		95.12	94.77	0.35	MHSA69400	94.29	0.83	-0.48
MH32			94.75		MHSA69392	94.29		-0.46
MH31		95.11	94.81	0.3	MHSA69391	94.55	0.56	-0.26
MH30		95.17	94.87	0.3	MHSA69390	94.76	0.41	-0.11
MH29		95.46	95.05	0.41	MHSA69389	94.99	0.47	-0.06
MH28		95.46	95.17	0.29	MHSA69388	95.1	0.36	-0.07
MH27		95.8	95.4	0.4	MHSA69387	95.33	0.47	-0.07
MH26		96.5	95.73	0.77	MHSA69386	95.66	0.84	-0.07
MH25 [S1]		97.77	96.02	1.75	MHSA69385	95.97	1.8	-0.05
MH40		95.11	94.77	0.34	MHSA69399	94.44	0.67	-0.33
MH39		95.1	94.77	0.33	MHSA69398	94.68	0.42	-0.09
MH38		95.17	94.53	0.64	MHSA69397	94.88	0.29	0.35
MH36		95.59	94.76	0.83	MHSA69396	95.1	0.49	0.34
MH37		96.07	95.06	1.01	MHSA69410	95.38	0.69	0.32
MH35		96.04	95.02	1.02	MHSA69395	95.35	0.69	0.33
MH34		96.5	95.46	1.04	MHSA69394	95.78	0.72	0.32
MH33			96.37		MHSA69393	96.67		0.3
MH8			94.8		MHSA69408	94.29		-0.51
MH7			94.8		MHSA69407	94.29		-0.51
MH6		95.27	94.82	0.45	MHSA69406	94.29	0.98	-0.53
MH24		95.2	94.82	0.38	MHSA69556	94.48	0.72	-0.34
MH23		95.37	94.82	0.55	MHSA69555	94.73	0.64	-0.09
MH22		95.16	94.82	0.34	MHSA69554	94.79	0.37	-0.03
MH21		95.5	94.7	0.8	MHSA69553	95.02	0.48	0.32
MH5		95.16	94.86	0.3	MHSA69405	94.38	0.78	-0.48
MH4		95.47	94.9	0.57	MHSA69404	94.54	0.93	-0.36
MH20		95.52	94.9	0.62	MHSA69551	94.83	0.69	-0.07
MH19		95.32	94.66	0.66	MHSA69552	95.05	0.27	0.39
MH3		96.17	95.02	1.15	MHSA69403	94.94	1.23	-0.08
MH18		96.24	95.03	1.21	MHSA69421	95.36	0.88	0.33
MH17		96.59	95.3	1.29	MHSA69420	95.63	0.96	0.33
MH16		96.59	95.41	1.18	MHSA69419	95.73	0.86	0.32
MH15		96.59	95.83	0.76	MHSA69417	96.26	0.33	0.43
MH13		97.25	95.94	1.31	MHSA69417	96.26	0.99	0.32
MH2		97.27	95.73	1.54	MHSA69402	95.68	1.59	-0.05
MH14		97.09	95.7	1.39	MHSA69416	96.02	1.07	0.32
MH13		97.25	95.94	1.31	MHSA69417	96.26	0.99	0.32
MH1 [S2]		98.12	96.31	1.81	MHSA69401	96.25	1.87	-0.06
S6		95.19	94.77	0.42	MHSA65325	94.28	0.91	-0.49
MH104A			94.725		MHSA65342	94.28		-0.445



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Heritage Hills								
20116	107.9		103.86	4.04	MHSA20116	103.84	4.06	-0.02
20164	107.3		101.66	5.64	MHSA20164	101.64	5.66	-0.02
12735	102.01		96.37	5.64	MHSA12735	96.35	5.66	-0.02
12732	99.4		95.19	4.21	MHSA12732	94.84	4.56	-0.35
20098	106.8		104.32	2.48	MHSA20098	104.32	2.48	0
20179	106.6		103.67	2.93	MHSA20179	103.63	2.97	-0.04
20123	106.1		103.39	2.71	MHSA20123	103.33	2.77	-0.06
20127	105.4		101.7	3.7	MHSA20127	101.67	3.73	-0.03
20130	102.9		99.29	3.61	MHSA20130	99.27	3.63	-0.02
13058	98		95.06	2.94	MHSA13058	94.98	3.02	-0.08
20161	98		94.84	3.16	MHSA20161	94.61	3.39	-0.23
Terry Fox Drive (Richardson Ridge)								
Baylis	97.15		94.65	2.5	MHSA64878	95.03	2.12	0.38
Rchrdsn N.	96		94.6	1.4	MHSA64876	94.89	1.11	0.29
L. Rchrdsn		94.8	94.5	0.3	MHSA64834	94.45	0.35	-0.05
MH329A		94.81	94.49	0.32	MHSA64833	94.46	0.35	-0.03
MH328A		94.84	94.51	0.33	MHSA64832	94.46	0.38	-0.05
L. Rchrdsn Ea.		94.97	94.46	0.51	MHSA63532	94.45	0.52	-0.01
N62		94.7	94.44	0.26	MHSA63527	94.42	0.28	-0.02
Terry Fox Drive (Broughton to SRPS)								
Broughton		94.7	94.42	0.28	MHSA58579	94.39	0.31	-0.03
MH205	96.99		94.8	2.19	MHSA58578	94.46	2.53	-0.34
MH207		97.26	94.95	2.31	MHSA58577	94.52	2.74	-0.43
TBD	95.42		94.34	1.08	MHSA58582	94.29	1.13	-0.05
SRPS	95.35		94.03	1.32	SPRS	94.07	1.28	0.04

### 3.5 Summary and Conclusions

Wastewater servicing for Arcadia Stage 6 will be designed in accordance with the City of Ottawa Sewer Design Guidelines, the associated technical bulletins, and various background documents as highlighted throughout this section. The proposed collection and conveyance of wastewater will consist of a local 200 mm diameter sewer which will outlet into Donum Lane as shown on Drawings S1 and S2. It is recommended that this wastewater servicing plan be implemented in order to provide adequate sanitary servicing for the proposed development.

## **4.0 STORM SERVICING AND STORMWATER MANAGEMENT**

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### **4.1 Background**

Similar to Arcadia Stages 1 to 4 stormwater management requirements for this development were originally set by the 2006 Kanata West Master Servicing Study (KWMSS), which identified a single stormwater management facility (referred as a Pond One) to service the area of the then proposed Arcadia developments. In 2018, after development of Stages 1 and 2, JLR evaluated various storm servicing strategies for the remainder of the Arcadia Development as part of the document entitled “Stormwater Management Strategy Report - Arcadia Residential Stages 3, 4 and Commercial Stage 2, JLR, May 2018”. The 2018 report identified that the preferred solution for the remainder of the Arcadia development was to incorporate a second stormwater management facility (SWM facility) on the south side of Campeau Drive which would improve HGL issues, allow for the immediate servicing of Campeau Drive extension and Light Rail Transit (LRT) and reduce submergence along the minor system when compared to a single pond servicing strategy. The two-pond concept was accepted by the City of Ottawa. The second SWM facility referred to as the Campeau Drive SWMF is the dedicated storm outlet for Stage 3, which included part of the Stage 6 lands.

Storm flows from the west side of Arcadia Stage 6 discharge to existing stubs on Country Glen Road that were included as part of the design for the commercial development on the west side of Country Glen Road. Discharges to the minor system on Country Glen Road flow via the existing minor system through Stages 1, 2 and 4 of Arcadia development and outlet to the Paine Pond, which provides water quantity and quality control for discharges to Carp River. Release rates to Country Glen Road were set in the Arcadia Commercial 370 Huntmar Drive Design Brief by IBI Group, October 2014.

Storm flows from the east side of Arcadia Stage 6 discharge to an existing storm sewer on Donum Lane, which discharges into Campeau Drive SWMF. This facility provides water quantity and quality controls prior to releasing controlled flows into the Carp River. Allowable release rates from the east side of Arcadia Stage 6 were set out in the JLR 2018 report and confirmed in the design of Arcadia Stage 3.

### **4.2 Design Criteria**

Storm and stormwater management servicing for the Arcadia Stage 6 was developed in accordance with the City of Ottawa 2012 Sewer Design Guidelines (OSDG) and the more recent Technical Bulletin PIEDTB-2016-01 (September 6, 2016). These two documents are herein referred to as the Design Guidelines in this section. A summary of the key storm and stormwater management criteria follows:

- Control minor system flows to the allowable release rates at existing stubs at Country Glen Road and Donum Lane;
- Storm sewers are designed to capture the 1:5 year storm event as a minimum using the Rational Method and using the regressions derived from Intensity-Duration-Frequency (IDF) equations as per the Design Guidelines;
- Provide a freeboard in the sewer network to the underside of footing (USF) of 300 mm during the 1:100-year storm where weeping tile connections are present;
- The runoff coefficients (C-factors) for the residential development were based on the maximum lot coverage permitted by the proposed zoning, as per the Design Guidelines.

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- C-factors for non-residential land uses to be calculated based on the ratio of pervious and impervious surfaces depicted on proposed site plans;
- Minimum roadway profile grades at 0.5%;
  - Roadway cross-fall of 3% was used for all streets;
  - Minimum roadway slope of 0.1% from crest-to-crest for overland flow route;
  - Minimum rear yard slope in the absence of perforated pipe system of 1.5% along with swale side slopes of 3 horizontal to 1 vertical;
  - Maximum street ponding depth of 350 mm (static and dynamic) as per the Design Guidelines and maximum depth of rear yard flow to be 300 mm;
  - Minimum vertical clearance of 0.15 m between the spill elevation on the street and the finished grade (garage elevation);
  - Minimum vertical clearance of 0.30 m between the rear yard spill elevation and the ground elevation at the building in the rear yards;
  - During the Climate Change event, the street ponding is not to reach the lowest building opening while the storm HGL must remain at or below the USF;
  - The product of the velocity and depth of major system flows on streets during the 1:100-year design storm event is not to exceed 0.60 m<sup>2</sup>/s; and,
  - Major system flows up to and including the 1:100-year design storm event are contained within the site and internally are self-contained within the park and amenity blocks.

### 4.3 Proposed Stormwater Management Approach

It is proposed to utilize both the west and east connection points to the existing minor storm sewers on Country Glen Road and Donum Lane respectively. The stormwater management approach will require that the discharges to these locations are controlled to the allowable release rates identified in the design of Phase 1 and Stage 3 of the Arcadia site. These allowable release rates are identified in Table XX.

Outlet	Allowable Release Rate (m <sup>3</sup> /s)	Set By
West – Country Glen Road	0.667	Arcadia Commercial 370 Huntmar Drive Design Brief by IBI Group, October 2014
East – Donum Lane	0.567	Stormwater Management Strategy report- Arcadia Residential Stages 3, 4 and Commercial Stage 2, JLR, May 2018

It should be noted that the outlet to Country Glen Road is downstream of an additional outlet previously provided for lands covering the Stage 6 development. Since the proposed outlet is the downstream outlet the allowable release rates for both outlets were combined as the pipe network on Country Glen Road will have sufficient capacity for the combined flow at the proposed connection point.

In order to achieve the allowable release rates, the stormwater management of the site will include online detention of the stormwater runoff in underground oversized sewers and allowing increased headwater depths in sewer sections with no weeping tile connections. Where weeping tile connections are proposed the 1:5-year free flow capacity of the pipe network will be maintained and the 1:100-year HGL will remain below the underside of footing of connected units.

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### 4.4 Proposed Minor System Servicing

#### 4.4.1 Runoff Coefficient

Runoff coefficients (C-Factors) were calculated for Arcadia Stage 6 based on the weighted product between the percentage of the pervious area at a C-Factor of 0.2 and the percentage of the impervious area at a C-Factor of 0.9. GIS Mapping of the impervious and pervious surfaces was used in the PCSWMM software spatial weighting tool to develop an overall weighted C-Factor for the site, excluding the park and amenity site. Due to consistency and density of the site, this approach provides a conservative C-Factor to be used across the site. C-Factors are provided in Table 4-1.

**Table 4-1: C-Factors**

<b>Area</b>	<b>C-Factor</b>
Site Development Area	0.78
Parkland	0.4
Amenity Space	0.4

#### 4.4.2 Minor System Servicing

The proposed storm sewers of Arcadia Stage 6 were sized using the Rational Method based on the C-Factors presented in Table 4-1. Appropriate rainfall intensities were used in the Rational Method based on the rainfall regression equations presented in Section 5.4.2 of the OSDG along with an inlet time of ten (10) minutes at the upstream end of the system. The Rational Method Storm Sewer Design Sheet is included in Appendix 'D1', while the Storm Drainage Plans included in the drawing set provide details associated with the storm drainage areas.

The storm drainage design sheet includes sewer sections which are used to control upstream flows through a restricted size. The restricted sewer sections operate under pressure in the minor system design event to control flows downstream and so show as operating beyond capacity in the design sheet, however these sewers are accounted for in the modelling HGL analysis.

#### 4.4.3 Inlet Control Devices

Storm servicing for Arcadia Stage 6 was developed to limit all flows transmitted to the storm sewers and meet the 0.35 m criterion as the maximum street ponding depth requirement. To achieve this criterion, servicing was developed using ICDs at inlets to the minor system.

The response under the 1:5-year rational method calculation was used to determine the minimum ICD targeted flow for Arcadia Stage 6. The ICDs were selected based on the dynamic model head differential between the maximum HGL at the grate and the higher of the geodetic elevation of the centroid of the ICD or the downstream HGL, in each catch basin lead. Therefore, each ICD was sized to transmit the targeted peak flow based on the calculated water level depth at the top of grate. When water rises above the top of grate in the roadway sag, flows transmitted to the storm sewers will marginally increase due to the increase in the

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hydraulic head. Based on the range of flows and hydraulic heads at each catch basin, the following types of ICDs are proposed in Stage 6:

- IPEX Tempest Type A;
- IPEX Tempest Type B;
- IPEX Tempest Type C;
- IPEX Tempest Type D; and,
- IPEX Tempest Type E.

Comprehensive ICD Tables referred to as the Catch Basin Table were prepared and are included in Appendix 'E2'. The Catch Basin Tables show specific information including top of grate elevation, pipe size and invert, the restricted capture rate and ICD type. The information shown on the Catch Basin Tables was extracted and shown on Drawing D1.

#### 4.4.4 Water Quality

The storm discharge criterion for the subdivision is based on the enhanced protection level (80% TSS), which was set to a level greater than the required normal protection level (70% TSS).

The downstream stormwater management ponds, either Campeau Drive SWMF for eastern flows or the Paine Pond for western flows, were designed to provide water quality treatment for the Arcadia Stage 6 development. Excess permanent pool volume was available in both ponds for water quality treatment to 80% TSS removal.

### 4.5 Stormwater Management Modelling Approach

#### 4.5.1 Dual Drainage Model

The analysis of both major and minor drainage systems was carried out to demonstrate their compliance with respect to the design criteria described in Section 4.2. The performance of the major overland system and minor storm sewer system was analyzed with PCSWMM. This software is a dynamic model which allows both hydrologic and hydraulic components to be simulated in the same platform and also allows the simulation of the interaction between the major and minor systems. The PCSWMM software platform was used to:

- Generate the surface runoff hydrograph for each sub-area under various recurrences.
- Subdivide each inflow hydrograph into its minor and major system components based on the proposed inlet capture rates and roadway sag storage.
- Assess cascading, if any, and carry out dynamic routing of storm flows to determine flow depths along the roadways. As previously stated, the maximum major overland flow depths along the subdivision's roadways are to be limited to 350 mm or less, as per Technical Bulletin PIEDTB-2016-01.
- Demonstrate that the HGL along the storm sewers during the 1:100-year event without sedimentation is 300 mm below the basement's USFs.

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PCSWMM was set-up to evaluate the proposed servicing as detailed on Drawings G1 (Grading), Drawings SWM1 (Ponding Plan), Drawings DST, ODST (Drainage) and Drawings S1-S2 & OS (Servicing). As per Drawings DST, the Arcadia Stage 6 lands were discretized into more refined sub-catchments for the immediate drainage area of Stage 6 and lumped sub-catchments for the entire modelled extents. To demonstrate the model schematic, Figure E1-1 and Figure E1-2 (Appendix E1) were prepared and depict the major and minor system elements of the model along with the subcatchments.

### 4.5.2 Integration with the Carp River Modelling and Boundary Conditions

In order to evaluate the design of the Paine and Campeau Stormwater Management Facilities and the impact on the Carp River, a sub-model of the City of Ottawa PCSWMM Carp River model was extracted to act as the downstream receiver for the stormwater management facilities. The inflow hydrograph at the upstream end of the sub-section of the Carp and the downstream stage hydrograph were both extracted from the overall Carp River model and used as inputs for the respective storm events in the Arcadia Detailed Design model.

Details of the detailed design model and the control of flows released to the Carp River were provided in the Paine Stormwater Management Facility Design Brief, JLR February 2020 for Arcadia Stage 4 and the Stormwater Management Facility Design Brief, Campeau Drive SWMF, JLR May 2019 for Arcadia Stage 3.

Both reports demonstrated that the use of the sub-model was a representation of the flows in the Carp River from the full model and that the stormwater management ponds provided sufficient controls to achieve no impact to the peak flows or maximum water levels in the Carp River.

Since the Arcadia Stage 6 development is internal to the detailed model used in these two reports and discharges to the two ponds, the Arcadia Stage 6 model will use the detailed sub-model to demonstrate that flows to the ponds can be maintained under the proposed Stage 6 stormwater management strategy and therefore there will be no impact on the Carp River.

### 4.5.3 Simulation of Street Segments

Flow directed to a street segment is split at the major system node; flows are broken down into minor and major system components using an outlet rating curve representing the ICD capture and assigning the minor system flow directly into the minor conduit while maintaining the major system flows on the surface conduit. Flow through the outlet link is calculated based on the HGL above the elevation of the ICD and its rated capacity under various water surface elevations. The ICD rating curves are those provided by the manufacturer.

An additional outlet link with rating curve is placed between the street surface low point and the rear yard connection manhole in the ROW. The outlet link is set at the top of grate elevation of the rear yard connection manhole and, if the street sag storage extends into the rear yard connection manhole lid, the outlet link represents any flow through the holes in the manhole lid which will enter from the ponded runoff.

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The storage in roadway sag is included in the model as being inherent within the major system conduits. The dynamic capability of PCSWMM means that the static and any dynamic flow is calculated in the model to provide one depth value at each sag location. The low points and high points in the street conduits are taken from the Civil 3D surface.

The subdivision's grading was developed with roadway static storage depths to maximize detention and attenuation of major overland flows while those of lesser volume sags were designed to maximize the conveyance capability of the dynamic section of the cross-section during events where cascading occurs.

#### 4.5.4 Adjoining Existing Areas

The western half of Campeau Drive, from Huntmar Drive to 160 metres east of the roundabout with Country Glen Road, was included in the SWMHYMO modelling for Arcadia Phase I, which included flows from Campeau Drive, residential facing Campeau Drive, Country Glen Road and minor flows from the commercial areas. The minor system flows were extracted from the approved SWMHYMO modelling and imported into the PCSWMM modelling. The IBI design allowed for some major overland flow to continue down Campeau Drive to the low point at the junction with Donum Lane and this hydrograph was also extracted from the SWMHYMO model and imported into the PCSWMM model.

It should be noted that the previous modelling for Arcadia Phase I included Stage 6 as commercial with all major overland flows retained on site, however, due to the change to residential and some of the properties facing Campeau Drive, the grading means that there is some runoff contributing directly to Campeau Drive flows. In order to capture the change in major system flow and small changes in drainage areas as a result of Stage 4, the hydrographs along Campeau Drive from the Phase I model were removed and replaced with subcatchments, inlet links and weirs representing the runoff and conveyance included in the SWMHYMO model.

The hydrologic parameters used are consistent with the Phase I modelling and the flows in the model are consistent with those in the Phase I modelling. A comparison of the flows is provided in Table 4-2.

**Table 4-2: Major Overland Flow Comparison along Campeau Drive**

<b>Section of Major Overland Flow along Campeau Drive</b>	<b>SWMHYMO 1:100 year 3-hour Chicago flow (m<sup>3</sup>/s)</b>	<b>PCSWMM 1:100 year 3-hour Chicago flow (m<sup>3</sup>/s)</b>
MH300 to MH301 at commercial access on Campeau Drive	0.221	0.221
MH301 to MH302	0.267	0.263
MH302 to Country Glen Road roundabout	0.319	0.306
Country Glen Road roundabout to MH304	0.357	0.354
MH304 to Donum Lane roundabout	0.430	0.430

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In addition, the total inflow to the low points on Campeau Drive at the Donum Lane roundabout were simulated in the Stage 4 model. The inlet capture defined in the Stage 4 model was set at the 1:10-year rate at each of the low point inlets. Additional flows to the low points at the Donum Lane roundabout included the immediate catchments in the model. In a change to the Stage 4 model, the actual impervious surface coverage was used to refine the imperviousness values used in the hydrology for the Campeau Drive catchment. The imperviousness reduced from 85.7% to 72.0% reflecting the front yards facing the north side of Campeau Drive covered by the catchment area. A comparison of the incoming flow for the 1:10-year and 1:100-year 3-hour Chicago storms at the north and south low points is provided in Table 4-3.

Table 4-3: Incoming Flow to the North and South Low Points – 3-hour Chicago Storm

Low Point	Stage 4 Model 1:10-year incoming flow (m <sup>3</sup> /s)	Stage 4 Model 1:100-year incoming flow (m <sup>3</sup> /s)	Stage 6 Model 1:10-year incoming flow (m <sup>3</sup> /s)	Stage 6 Model 1:100-year incoming flow (m <sup>3</sup> /s)	Stage 4 Modelled Minor System Inlet Capture Rate (m <sup>3</sup> /s)
North	0.198	0.517	0.174	0.480	0.2
South	0.156	0.452	0.160	0.459	0.16

The comparison with the flows in the SWMHYMO model with those in the Stage 4 modelling shows that the changes to the drainage areas have negligible impact on the hydraulics of Campeau Drive. The flows from upstream are consistent with the previous modelling and the inclusion of the runoff from Stage 6 front yards, along with refinement of the imperviousness values for Campeau Drive, means that the 1:10-year flow rate remains below, or at, the modelled inlet capture rate and the 1:100-year event flows are no greater, or at, previously simulated flow rates.

## 4.6 Modelling Parameters

### 4.6.1 Hydrological Parameters

The following parameters were used in the hydrologic component of PCSWMM:

- Areas and Imperviousness:** Catchment ID and drainage areas used by PCSWMM match those shown on either Drawing DST or Figure E-1 (Appendix E1). In regard to the imperviousness of subcatchments, C-Factors reported in Section 4.4.1 were used to estimate PCSWMM's imperviousness. A total imperviousness (TIMP) of 67.14% was calculated and simulated for all front yard areas, while a TIMP of 54.3% was used for the rear yard areas. Subarea routing (percent of impervious surface routed to pervious surface) was calculated for typical lots.
- Flow Path Length / Catchment Width:** The flow path length is based on the measured length of the flow from the rear of the property to the street section. The catchment width is, therefore, approximately twice the length of the street segment through the subcatchment, in accordance with the OSDG.



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- **Manning's Roughness Coefficient:** Manning's Roughness Coefficients of 0.013 and 0.24 were used for the impervious and pervious surfaces, respectively, which are consistent with the OSDG.
- **CN Infiltration parameters:** For consistency with the City of Ottawa Carp River Model the CN infiltration approach was used. The CN value of 75 for pervious land cover was maintained from the Carp River Model.

Since PCSWMM is based on the Nonlinear Reservoir Routing Method (SWMM 5 engine) to generate runoff from subcatchments, the infiltration and depression storage are accounted for separately. The formulation of the SCS Loss Method incorporated into SWMM does not include the Initial Abstraction term. CN is used in SWMM to compute infiltration losses only, not total hydrologic losses as in the original SCS methodology. Therefore, the CN value is used and not a modified CN (CN\*) as this alters for term to account for the difference in Initial Abstraction.

- **Initial Abstraction:** Initial abstraction of 4.67 mm and 1.57 mm was used for the pervious and impervious surfaces respectively, consistent with the OSDG and Carp River Model.

Note that for catchments that were previously modelled in SWMHYMO for Arcadia Phase I maintained the catchment parameters as per the SWMHYMO model to maintain consistency with previous work.

#### 4.6.2 Simulation of Storm Distributions

The City of Ottawa requires that the performance of the minor and major systems be investigated under the 3-hour Chicago design storm. As such, 1:2-year, 1:5-year, 1:10-year, 1:25-year, 1:50-year, and 1:100-year 3-hour Chicago storms were evaluated. In addition, the critical storm distribution for the Carp River is the 12-hour SCS storm and so this distribution was also assessed by the model for the same durations, in addition to the standard 24-hour SCS storm distribution for the same durations. The 12-hour SCS storm was found to be critical for the minor system HGL while the Chicago 3-hour storm for the major overland flow system.

The Climate Change stress test event was run for all three storm distributions used. As per the requirements of the Ottawa Sewer Design Guidelines, historical storms were also assessed, including the July 1, 1979 storm, the August 4, 1988 storm and the August 8, 1996 storm.

#### 4.6.3 Simulation of Park and Amenity Blocks

For the park and amenity blocks the model includes a storage node with an outlet link to restrict flow to the minor system to the 1:5-year runoff rate for the blocks. In events greater than the 1:5-year return period then the storage node detains runoff over and above the release rate for the block. The 1:5-year release rates for the blocks are shown in Table 4-4.

# Site Servicing Report

## Arcadia Stage 6

Table 4-4: Release Rates for the Park and Amenity Area

Block	Imperviousness (%)	Runoff Coefficient	1:5-year Release Rate (m <sup>3</sup> /s)
Park Block	28.57	0.40	0.058
Amenity Block	54.43	0.58	0.061

### 4.6.4 Simulation of Garage Access Ramps

Two of the units within the Stage 6 development have depressed garage access ramps. One of these will be able to drain via gravity to the storm sewer system while the other to the east of the site will require to be pumped to the minor system. A maximum pump rate of 9 L/s, approximately equivalent to a 1:10 year event. Where the gravity connection is provided the drain is above the underside of footing for the unit and 300mm freeboard should be maintained below the grate elevation.

## 4.7 Simulation Results

This section of the Report presents the results of the simulation of Stage 6 as part of the detailed model for the Arcadia site as a whole, incorporating Stages 1 to 4 and the Paine Pond and Campeau Drive stormwater management facilities. The modelling includes the future Stage 5 at a conceptual level with the same parameters as used in the Stage 3 and 4 models.

The objective of this section is to assess the performance of the following systems under the build-out condition:

- The major overland system under extreme storm events (i.e., 1:100 year and climate change events) as per the OSDG; and
- The major overland system during the 1:2-year storm event and determine whether surface ponding is to occur.
- Appendices E4 and E5 provide Storm HGL analyses and Street Ponding Analyses respectively for a range of historical storms and interim conditions.

### 4.7.1 Low Point Ponding Analysis

The results at each of the low points, as generated by a 3-hour Chicago storm distribution, are set out in Table 4-5. Low points correspond to Area IDs from the ponding plan Drawing SWM1.

Table 4-5: Low Point Major System Ponding (3-hour Chicago Event)

Low Point ID	Top of Grate (m)	Maximum Static Depth	1:2 year Depth (mm)	1:5 year Depth (mm)	1:100 year Depth (mm)	Climate Change Depth (mm)
1	97.1	300	-	-	200	230
2	96.9	300	-	-	90	190
3	96.9	100	-	-	-	-

## Site Servicing Report Arcadia Stage 6

Low Point ID	Top of Grate (m)	Maximum Static Depth	1:2 year Depth (mm)	1:5 year Depth (mm)	1:100 year Depth (mm)	Climate Change Depth (mm)
4	96.8	100	-	-	-	-
5	96.7	190	-	-	-	-
6	96.2	200	-	-	50	100
7	96	150	-	-	100	140
8	95.6	220	-	-	70	110
9	96.87	230	-	-	160	200
10	97.04	230	-	-	100	150
11	96.97	300	-	-	-	-
12	96.88	290	-	-	-	60
13	96.87	200	-	-	-	-
14	96.67	300	-	-	50	100
15	96.57	130	-	-	110	150
16	96.77	160	-	-	150	200
17	96.92	250	-	-	160	210
18	96.67	170	-	-	160	200
19	96.83	240	-	-	160	210
20	96.1	250	-	-	110	150
21	96.2	200	-	-	70	120
22	95.9	250	-	-	100	140
23	95.7	200	-	-	240	260
24	95.6	220	-	-	170	250

The simulation results compiled in Table 4-5 shows that:

- No ponding nor dynamic flow will occur in the 1:2-year or 1:5 year events within the site; and,
- Maximum ponding depth of 200 mm during the 1:100-year event;
- In the climate change event, the peak ponding depth is below 350mm.

#### 4.7.2 Major System Flow

No cascading flow occurs in the site during any events and therefore the depth x velocity requirement is achieved.

# Site Servicing Report

## Arcadia Stage 6

### 4.7.3 Storm Sewer HGL Analysis

The storm sewer HGL under the ultimate servicing scenario is shown at each of the manhole nodes in Table 4-6. Where there is no Underside of Footing (USF) associated with the manhole a dash is shown in the table.

**Table 4-6: HGL Analysis (12-hour SCS Storm)**

MH ID	USF Elevation (m)	1:100 year Event Max HGL (m)	1:100 year Freeboard (m)-	Climate Change Max HGL (m)	Climate Change Freeboard (m)-
217	-	95.74	-	95.74	-
214A	-	95.74	-	95.74	-
214	95.37	94.97	0.40	94.97	0.4
216	-	95.72	-	95.72	-
213A	-	95.72	-	95.72	-
213	95.32	94.97	0.35	94.97	0.35
210	-	95.16	-	95.16	-
211	-	95.16	-	95.16	-
212	95.32	94.98	0.34	94.98	0.34
J_CBMH1	96.14	95.59	0.55	95.6	0.54
CBMH2	-	95.67	-	95.67	-
224	96.14	95.33	0.81	95.33	0.81
226	96.14	95.2	0.94	95.2	0.94
220	-	95.34	-	95.34	-
218	-	95.2	-	95.2	-
227	95.43	95.04	0.39	95.04	0.39
215	95.3	94.95	0.35	94.95	0.35
MHST77649	-	94.87	-	94.87	-
208	94.69	94.1	0.59	94.1	0.59
207	94.8	93.97	0.83	93.97	0.83
206	94.69	93.9	0.79	93.9	0.79
205	94.24	93.84	0.40	93.84	0.4
201	-	93.71	-	93.71	-
200A	-	93.65	-	93.65	-
200	-	93.65	-	93.65	-
ST6_CB102	-	96.24	-	96.24	-
J_CB103	-	94.43	-	94.44	-
204A	94.69	94.02	0.67	94.02	0.67
204	-	94	-	94	-
203	-	95.49	-	95.49	-
202	-	95.48	-	95.48	-
201A	-	95.45	-	95.45	-
CBMH3	-	95.56	-	95.56	-
202A	-	95.51	-	95.51	-
201A	-	95.45	-	95.45	-

## Site Servicing Report Arcadia Stage 6

MH ID	USF Elevation (m)	1:100 year Event Max HGL (m)	1:100 year Freeboard (m)-	Climate Change Max HGL (m)	Climate Change Freeboard (m)-
J_CB108	-	94.98	-	95.05	-
J_CBMH4	94.24	93.65	0.59	93.89	0.35

The simulation results compiled in Table 4-6 shows that:

- All nodes achieve HGLs with 300 mm freeboard to the underside of footing in the 1:100-year event with the smallest freeboard being 300 mm; and,
- All nodes maintain a clearance to the underside of footing in the climate change stress test event.

#### 4.7.4 Storm Sewer HGL Analysis for Existing Arcadia Stages

The storm sewer HGL under the ultimate servicing scenario is shown at each of the existing Arcadia Stages 1 and 2 manhole nodes in Table 4-7. Where there is no Underside of Footing (USF) associated with the manhole, a dash is shown in the table.

Table 4-7: Storm HGL Analysis Arcadia Stage 1 and 2 (12-hour SCS)

MH ID	USF (m)	Stage 1 or 2 Interim Buildout HGL (m)	Ultimate Buildout HGL (m)	Difference between Stage 1 Interim & Stage 4 Ultimate HGL (m) <sup>(1)</sup>	Freeboard
MH200	95.15	94.75	94.39	-	0.76
MH201	95.31	94.96	94.59	-	0.72
MH202	95.37	95.07	94.73	-	0.64
MH203	95.43	95.13	94.81	-	0.62
MH204	95.69	95.20	94.93	-	0.76
MH205	97.59	95.87	95.92	0.05	1.67
MH206	97.59	96.01	96.05	0.04	1.54
MH207	97.74	96.50	96.54	0.04	1.20
MH208	97.64	97.13	97.17	0.04	0.47
MH209	96.71	95.74	95.66	-	1.05
MH210	95.94	95.27	95.07	-	0.87
MH211	95.64	95.31	95.16	-	0.48
MH212	96.14	95.54	95.50	-	0.64
MH213	95.89	95.31	95.20	-	0.69
MH214	95.89	95.27	95.15	-	0.74
MH215	95.49	95.15	94.86	-	0.63
MH216	95.39	95.09	94.70	-	0.69

## Site Servicing Report Arcadia Stage 6

MH ID	USF (m)	Stage 1 or 2 Interim Buildout HGL (m)	Ultimate Buildout HGL (m)	Difference between Stage 1 Interim & Stage 4 Ultimate HGL (m) <sup>(1)</sup>	Freeboard
MH217	95.64	95.21	94.78	-	0.86
MH501	97.82	95.72	95.54	-	2.28
MH502	96.32	95.37	94.99	-	1.33
MH503	95.57	94.87	94.24	-	1.33
MH504	95.23	94.53	93.97	-	1.26
MH505	95.16	94.48	93.93	-	1.23
MH506	95.12	94.42	93.88	-	1.24
MH513	97.25	96.48	96.11	-	1.14
MH514	96.87	96.01	95.67	-	1.20
MH515	96.59	96.14	95.74	-	0.85
MH516	96.59	95.63	95.27	-	1.32
MH517	96.52	95.51	95.11	-	1.41
MH518	96.17	95.26	94.83	-	1.34
MH519	95.32	94.85	94.34	-	0.98
MH520	95.27	94.81	94.27	-	1.00
MH521	95.14	94.81	94.05	-	1.09
MH522	95.14	94.73	94.05	-	1.09
MH523	95.31	94.72	94.04	-	1.27
MH524	95.16	94.63	93.99	-	1.17
MH525	97.77	95.25	95.25	-	2.52
MH526	96.42	95.25	95.16	-	1.26
MH527	95.77	95.05	94.71	-	1.06
MH528	95.47	94.88	94.59	-	0.88
MH529	95.46	94.81	94.53	-	0.93
MH530	95.23	94.68	94.42	-	0.81
MH531	95.11	94.54	94.31	-	0.80
MH532	0	94.3	94.10	-	-94.10
MH533	97.68	95.58	95.58	-	2.10
MH534	96.42	95.78	95.38	-	1.04
MH535	96.04	95.31	94.73	-	1.31
MH536	95.59	95.05	94.58	-	1.01

# Site Servicing Report

## Arcadia Stage 6

MH ID	USF (m)	Stage 1 or 2 Interim Buildout HGL (m)	Ultimate Buildout HGL (m)	Difference between Stage 1 Interim & Stage 4 Ultimate HGL (m) <sup>(1)</sup>	Freeboard
MH537	96.07	95.16	94.81	-	1.26
MH538	95.31	94.81	94.41	-	0.90
MH539	95.11	94.59	94.25	-	0.86
MH540	95.11	94.46	94.15	-	0.96

(1) If the HGL in ultimate buildout condition is greater than the HGL of the Stage 1 Interim buildout, then the difference in HGL levels is provided.

The simulation results compiled in Table 4-7 shows that:

- The HGL levels for the ultimate buildout condition are lower than the HGL levels in Arcadia Stages 1 and 2 interim buildout conditions, except in Stage 1 at MH205, MH206, MH207, and MH208.
- A minimum freeboard of 0.3m is provided at all manhole nodes in the existing Arcadia Stages.

#### 4.7.5 System Release Rates

The allowable release rates for the system to Donum Lane and Country Glen Road were identified in Section 4.3. The results of the modelling for the 1:100-year event for the three storm distributions are shown in Table 4-8

**Table 4-8: System Release Rates Comparison**

Event	Stage 6 to Donum Lane (m <sup>3</sup> /s)	Allowable to Donum Lane (m <sup>3</sup> /s)	Stage 6 to Country Glen Road (m <sup>3</sup> /s)	Allowable Release Rate to Country Glen Road (m <sup>3</sup> /s)
3-hour Chicago	0.559	0.567	0.635	0.667
12-hour SCS	0.518	0.567	0.635	0.667
24-hour SCS	0.516	0.567	0.619	0.667

Table 4-8 shows that the system meets the allowable release rate in each of the storm distributions for events up to the 1:100-year event. Since the allowable release rates are achieved the operations of the Paine Pond and Campeau Drive SWMF will be maintained as per the Design Briefs for the facilities and there will be no impact on the downstream Carp River.

## 4.8 Summary and Conclusions

The stormwater servicing and management concept is proposed to provide stormwater servicing for Arcadia Stage 6, as shown on the Servicing Plan (Drawing S1 and S2).

## **5.0 Erosion and Sedimentation Control**

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Erosion and sediment control measures, as outlined in the Ontario Ministry of Natural Resources (MNR) Guidelines on Erosion and Sediment Control for Urban Construction Sites, will be implemented to trap sediment on site. The following erosion and sediment control measures can be implemented during construction as shown on the Erosion and Sediment Control Plan (Drawing ESC):

- Supply and installation of a silt fence barrier, as per OPSD 219.110.
- Supply and installation of siltsack or sentinel CB inserts between the frame and cover of catch basins and maintenance holes adjacent to the project area during construction, to prevent sediment from entering the sewer system.
- Stockpiling of material during construction is to be located along flat areas away from drainage paths. For material placed on sloped areas, stockpiles are to be enclosed with a silt fence to protect watercourses.
- All catch basins are to be equipped with sumps, inspected frequently, and cleaned as required.
- Temporary ICDs are to be placed blocking part of the sewer pipe in the connecting storm maintenance holes to eliminate construction debris from entering the existing storm sewer system. The ICDs are to be removed after the proposed storm sewers have been fully cleaned.
- A mud mat is to be built at each of the site entranceways to prevent the transport of sediment onto paved surfaces. The mud mat shall be:
  - Minimum of 20 m in length for the full width of the entrance way (10 m wide minimum).
  - Minimum of 400 mm thick underlain with a geotextile (or graded aggregate filter); and
  - Constructed with 50 mm diameter clear stone for the first 10 m (extending from the paved street) and the remainder of the length with 150 mm diameter clear stone.

The proposed removal and reinstatement measures as well as the erosion control measures shall conform to the following documents:

- “Guidelines on Erosion and Sediment Control for Urban Construction Sites” published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- “MTO Drainage Manual”, Chapter F: “Erosion of Materials and Sediment Control”, Ministry of Transportation & Communications, 1985.
- “Erosion and Sediment Control” Training Manual by Ministry of Environment, Spring 1998.
- Applicable Regulations and Guidelines of the Ministry of Natural Resources.



# Site Servicing Report

## Arcadia Stage 6

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### 6.0 CONCLUSIONS

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Servicing of Minto's Arcadia Stage 6 development, as depicted on the detailed design drawings, has been accounted for in previous studies completed for the subject area. In General, the lands will be serviced as follows:

- Water servicing will be provided by connections to existing watermains on Country Glen Way and Donum Lane, and to the existing feedermain located on Campeau Drive.
- Wastewater servicing will be provided by a local sanitary sewer system that will outlet to existing sanitary sewers on Campeau Drive.
- Storm servicing is to be provided on-site by means of local sewers that outlet to the existing storm sewers on Donum Land and Country Glen way.
- Flows in excess of the prescribed allowable peak flow are to be detained by means of on-site storage methods; either above ground or underground or a combination of both.

# Site Servicing Report

## Arcadia Stage 6

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This Report has been prepared for the exclusive use of Minto, for the stated purpose, for the named facility. Its discussions and conclusions are summary in nature and cannot be properly used, interpreted or extended to other purposes without a detailed understanding and discussions with the client as to its mandated purpose, scope and limitations. This report was prepared for the sole benefit and use of Minto and may not be used or relied on by any other party without the express written consent of JLR.

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J.L. RICHARDS & ASSOCIATES LIMITED



Annie Williams, P.Eng.



Ivan Dzeperoski, P.Eng.

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## **Appendix A1**

Concept Plan, Draft Plan of  
Subdivision and Topographical  
Survey



**Title:** Concept Plan 32

**Project:** Arcadia - Stage 6

**Legend**

- Public Parkette
- Amenity Area
- Open Space
- Executive Towns
- Avenue (B2B) Towns
- Urban (Rear Lane) Towns
- Infusion (Stacked) Towns
- Stage Limits
- 100 Year Floodplain

**Site Statistics (For Stacked Towns Only)**

Unit Count (Stacked Towns only)	264		
	Required	Provided	
Total Amenity Space (6 sq m./unit)	1,584 Sq m.	4,910 Sq m.	
Communal Amenity Space (50% of total amenity per unit = min. 3 Sq m./Unit)	792 Sq m.	2,006 Sq m.	
Bike Rack Count (0.5/Unit - 8 spaces per rack)	16.5	16	
Parking (Section 101)	Above Ground Parking (Units x 1.1)	162	158
	Under Ground Parking (Units x 1.1)	128	148
	Total Parking Spaces	290	306

**NOTES:**

- There is a 10m No Build Setback from the northern LRT property line.
- Each stacked town has a lower unit with a patio (16 sq. m.) and an upper unit with a balcony (6 sq.m.) which are included as private amenity area.
- All pathways are 1.5m unless otherwise noted.
- Parking requirement for stacked towns is 0.90 per unit for the residents + 0.1 per unit for visitor = total 1.0 per unit.
- Assume UG garage for TE-4 to 7 has 80 parking spaces and TE-11 to 14 has 68 parking spaces.

4	Updated curbs on Street No. 2 & 5. Update underground garage boundary for TE-3 to 7.	2022-06-09	K.G.
3	Add lapsed site plan for west property	2022-06-03	K.G.
2	Update Avenue Town & TE-4 to 7	2022-05-11	K.G.
1	Update Avenue Town models (2022)	2022-05-11	K.G.
0	Issued for Review	2022-05-03	K.G.

**Revisions**

No.	Description	Date	By
4	Updated curbs on Street No. 2 & 5. Update underground garage boundary for TE-3 to 7.	2022-06-09	K.G.
3	Add lapsed site plan for west property	2022-06-03	K.G.
2	Update Avenue Town & TE-4 to 7	2022-05-11	K.G.
1	Update Avenue Town models (2022)	2022-05-11	K.G.
0	Issued for Review	2022-05-03	K.G.

Unit Type	Stage 6	
Executive Towns	11	2.99 %
Avenue Towns	80	21.7 %
Rear Lane Towns	13	3.5 %
Infusion Terraces	264	71.7 %
<b>Total</b>	<b>368</b>	<b>100.0 %</b>
Required Parkland (1ha / 300units)	1.23 ha	
Surplus Parkland from Stage 1-4	0.67 ha	
Min. Parkette Size (Stage 6)	0.56 ha	

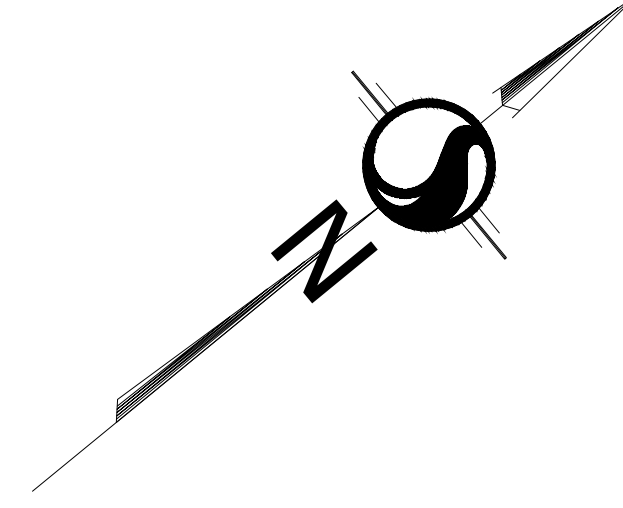
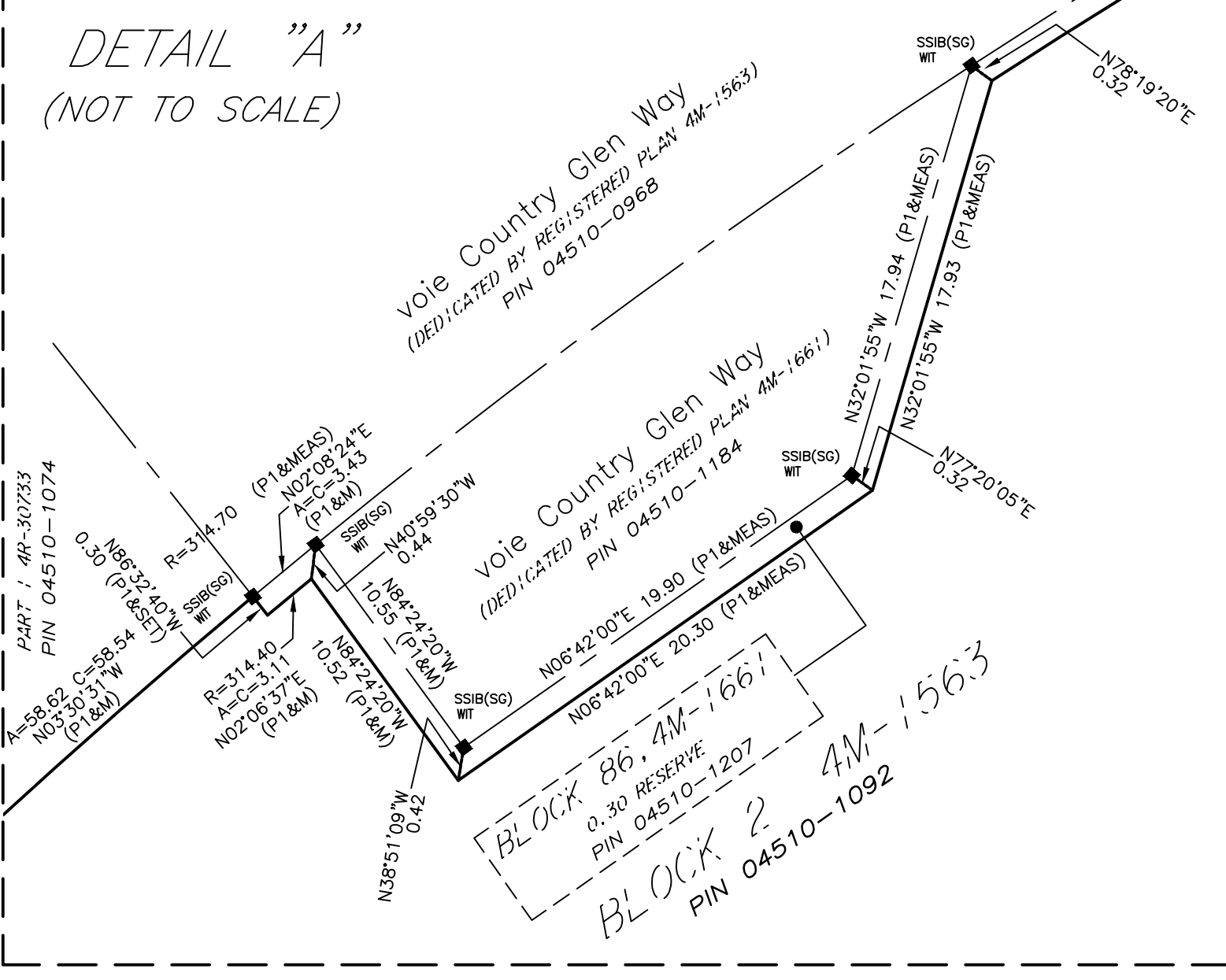
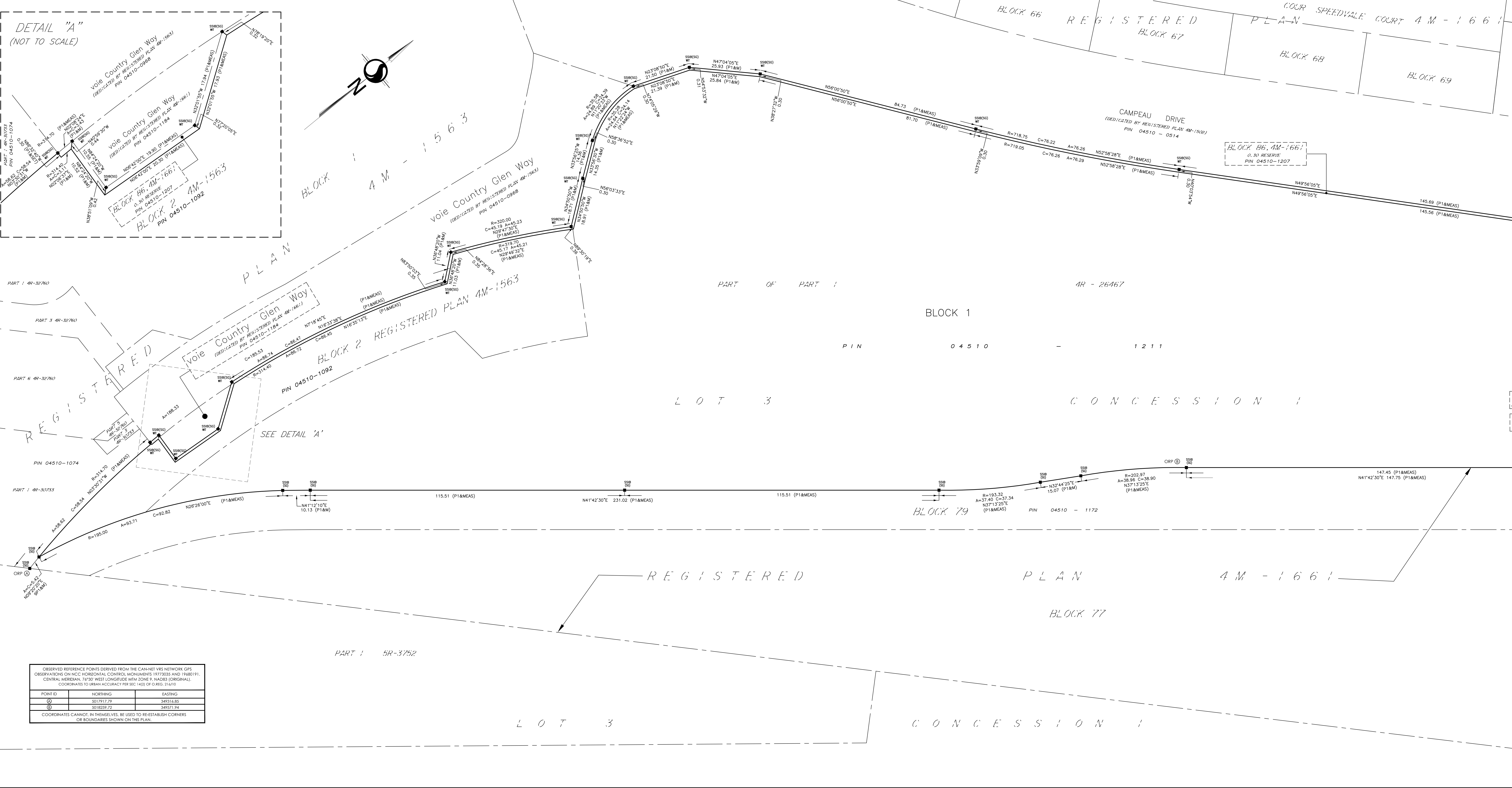
**Revisions**

**Drawn By:** K.G.  
**Checked By:** C.S.

**Minto Communities Inc**  
180 Kent Street,  
Ottawa, ON  
K1P 0B6

**North**

**Scale:** NTS



**PLAN 4M-**

I HEREBY CERTIFY THAT THIS PLAN 4M-\_\_\_\_\_ IS REGISTERED IN THE LAND REGISTRY OFFICE FOR THE LAND TITLES DIVISION OF OTTAWA-CARLETON (NO. 4) AT \_\_\_\_\_ O'CLOCK ON THE \_\_\_\_\_ DAY OF \_\_\_\_\_ 2022 AND ENTERED IN THE REGISTER FOR P.I.N. \_\_\_\_\_ AND THE REQUIRED CONSENTS ARE REGISTERED AS PLAN DOCUMENT NUMBER OC-\_\_\_\_\_

REPRESENTATIVE FOR LAND REGISTRAR

THIS PLAN COMPRISES ALL OF PINS 04510-1211, 04510-1092, 04510-1209 AND 04510-1210. PART OF BLOCK 1 IS SUBJECT TO EASEMENT AS IN INSTRUMENT OC2248967.

PLAN OF SUBDIVISION OF  
**PART OF BLOCK 2,  
 REGISTERED PLAN 4M-1563 AND  
 PART OF LOT 3  
 CONCESSION 1**  
 (GEOGRAPHIC TOWNSHIP OF MARCH)  
 CITY OF OTTAWA

Scale 1:500

**METRIC CONVERSION**  
 DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

**GRID SCALE CONVERSION**  
 DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99914.

**BEARING NOTE**  
 BEARINGS ARE GRID, DERIVED FROM THE CAN-NET VRS NETWORK OBSERVATIONS ON NCC HORIZONTAL CONTROL MONUMENTS 1972035 AND 19680191, CENTRAL MERIDIAN, 76°30' WEST LONGITUDE MTM ZONE 9, NAD83 (ORIGINAL).

19772035 N:5006040.42 E:324888.04  
 19680191 N:5003564.26 E:38804.94

**LEGEND**

□	DENOTES	FOUND MONUMENTS (STANTEC)
■	IB	SET MONUMENTS (IB)
■	IB	UNLESS OTHERWISE STATED
■	IB	IRON BAR
■	SIB	ROUND IRON BAR
■	SIB	STANDARD IRON BAR
■	CC	SHORT STANDARD IRON BAR
■	CC	CUT CROSS
■	CP	CONCRETE PIN
■	WIT	WITNESS
■	PIN	PROPERTY IDENTIFICATION NUMBER
■	M/MEAS	MEASURED
■	PROP	PROPORTIONED
■	OU	ORIGIN UNKNOWN
■	STANTEC	STANTEC GEOMATICS LTD.
■	P1	REGISTERED PLAN 4M-1661

**OWNER'S CERTIFICATE**  
 THIS IS TO CERTIFY THAT:  
 1. BLOCK 1 HAS BEEN LAID OUT IN ACCORDANCE WITH OUR INSTRUCTIONS.

DATE \_\_\_\_\_  
 \_\_\_\_\_  
 VICE-PRESIDENT, LAND DEVELOPMENT  
 MINTO COMMUNITIES INC.  
 I HAVE THE AUTHORITY TO BIND THE CORPORATION

DATE \_\_\_\_\_  
 \_\_\_\_\_  
 MINTO COMMUNITIES INC.  
 I HAVE THE AUTHORITY TO BIND THE CORPORATION

**SURVEYOR'S CERTIFICATE**  
 I CERTIFY THAT:  
 1. THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEYS ACT, THE SURVEYORS ACT AND THE LAND TITLES ACT AND THE REGULATIONS MADE UNDER THEM.  
 2. THE SURVEY WAS COMPLETED ON THE DAY OF \_\_\_\_\_, 2022.

DATE \_\_\_\_\_  
 FRANCIS LAU  
 ONTARIO LAND SURVEYOR

**Stantec Geomatics Ltd.**  
 CANADA LANDS SURVEYORS  
 ONTARIO LAND SURVEYORS  
 1331 CYDIE AVENUE, SUITE 300  
 OTTAWA, ONTARIO, K1G 3G4  
 TEL: 613.722.4400  
 stantec.com

OBSERVED REFERENCE POINTS DERIVED FROM THE CAN-NET VRS NETWORK GPS OBSERVATIONS ON NCC HORIZONTAL CONTROL MONUMENTS 19772035 AND 19680191, CENTRAL MERIDIAN, 76°30' WEST LONGITUDE MTM ZONE 9, NAD83 (ORIGINAL). COORDINATES TO URBAN ACCURACY PER SEC 14(2) OF O. REG. 216/10

POINT ID	NORTHING	EASTING
①	5017917.79	349316.85
②	5018259.72	349571.94

COORDINATES CANNOT, IN THEMSELVES, BE USED TO RE-ESTABLISH CORNERS OR BOUNDARIES SHOWN ON THIS PLAN.

TOPOGRAPHIC SKETCH OF  
**REGISTERED PLAN 4M-1661**  
**AND**  
**PART OF BLOCK 2**  
**REGISTERED PLAN 4M-1563**  
**AND**  
**PART OF LOT 3**  
**CONCESSION 1**  
 (GEOGRAPHIC TOWNSHIP OF MARCH)

CITY OF OTTAWA  
 Scale 1:100

Stantec Geomatics Ltd.  
 ONTARIO LAND SURVEYORS

**BOUNDARY NOTE**  
 BOUNDARY NETWORK AND INFORMATION IS COMPILED FROM REGISTERED PLAN 1563 AND IS NOT BASED ON ACTUAL SURVEY.

**METRIC CONVERSION**  
 DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

**ELEVATION NOTE**  
 ELEVATIONS SHOWN HEREIN ARE GEODESIC (CGVD 1928-1978) AND ARE DERIVED FROM THE CANMET VRS NETWORK (MONUMENT OTTAWA ELEVATION=95.28).

**LEGEND**

SYMBOL	NOTES	FOUND MONUMENTS
IB	IRON BAR	SET MONUMENTS
RB	ROUND IRON BAR	IRON BAR
SB	STANDARD IRON BAR	ROUND IRON BAR
SSB	SHORT STANDARD IRON BAR	STANDARD IRON BAR
CC	CUT CROSS	SHORT STANDARD IRON BAR
CP	CONCRETE PIN	CUT CROSS
WT	WITNESS	CONCRETE PIN
PN	PROPERTY IDENTIFICATION NUMBER	WITNESS
M/MEAS	MEASURED	PROPERTY IDENTIFICATION NUMBER
PROF	PROFICED	MEASURED
ORIG	ORIGIN UNKNOWN	PROFICED
STANTEC	STANTEC GEOMATICS LTD.	ORIGIN UNKNOWN
P1	REGISTERED PLAN 4M-1563	STANTEC GEOMATICS LTD.
P2	REGISTERED PLAN 4M-1661	REGISTERED PLAN 4M-1563
CP	CATCH BASIN	REGISTERED PLAN 4M-1661
SCBP	SIDE INLET CB	P2
UV	UTILITY ACCESS	CP
LS	LIGHT STANDARD	SCBP
MCSAN	MAINTENANCE HOLE SANITARY	UV
MCSW	MAINTENANCE HOLE SEWAGE	LS
MSTRAFF	MAINTENANCE HOLE TRAFFIC	MCSAN
SN	SIGN	MCSW
TRP	TRAFFIC CONTROL BOX	MSTRAFF
UP	UTILITY POLE	SN
		TRP
		UP
		OVERHEAD WIRE

**SURVEYOR'S CERTIFICATE**  
 I CERTIFY THAT:  
 1. THE SURVEY WAS COMPLETED ON THE 21st DAY OF MARCH, 2022.

DATE: FRANCIS LAU  
 ONTARIO LAND SURVEYOR



DRAWN: TMT CHECKED: FL P.M. FL FIELD: CA PROJECT NO.: 16161463-111

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## **Appendix A2**

Pre-consultation Meeting Notes

## **Pre-application Consultation Meeting Notes**

Site Address: 370 Huntmar Drive and 450 Huntmar Drive

Location: Virtual - Microsoft Teams

Meeting Date: August 12, 2021

**Attendees:** Colette Gorni – Planner, City of Ottawa  
Wendy Tse – Planner, City of Ottawa  
Justin Armstrong – Project Manager (Infrastructure), City of Ottawa  
Mark Young – Planner (Urban Design), City of Ottawa  
Jeff Goettling – Planner (Parks), City of Ottawa  
Mike Russett – Planner (Parks), City of Ottawa  
Jeffrey Ren – Co-op Student, City of Ottawa  
Matt Craig – MVCA  
Erica Ogden – MVCA  
Curtiss Scarlett – Minto Communities Inc.  
Bronwyn Anderson – Minto Communities Inc.  
Kiara Gonzales – Minto Communities Inc.  
Danielle Forget – Minto Communities Inc.  
Alexandre Tourigny – J.L Richards  
Eric Forhan – J.L Richards  
Lucie Dalrymple – J.L Richards  
Andrew Harte – CGH Transportation

**Regrets:** Mark Richardson – Planning Forester, City of Ottawa  
Mike Giampa – Project Manager (Transportation), City of Ottawa  
Matthew Hayley – Planner (Environmental), City of Ottawa

### **APPLICANT COMMENTS:**

1. Two separate developments are proposed:
  - a. Stage 5 will be a Plan of Subdivision application and a Zoning By-law Amendment application:
    - Stage 5 is located east of Arcadia Stages 1-4; it is the last piece north of Campeau Drive;
    - Existing infrastructure extends to the boundaries of the site and two accesses off of Winterset Road are proposed;
    - A total of 225 low-rise units are proposed in the form of singles, townhouses and back-to-back townhouses; and,



- The site is designated as General Urban Area in the Official Plan; the Carp River Restoration Policy Area Overlay applies; and the site is zoned Development Reserve (DR).
- b. Stage 6 will be a Site Plan Control application and a Zoning By-law Amendment application:
- Stage 6 lands are located south of Campeau Drive, the parcel closest to the intersection of Huntmar Drive and Campeau Drive was sold by Minto to a hotel developer;
  - The site is designated as Mixed-Use Centre in the Official Plan, the Kanata West Concept Plan identifies this area as a community core, and the site is zoned Development Reserve;
  - Land dedications have been made for the proposed future Campeau Station LRT Station;
  - Two accesses, one off of Campeau Drive is proposed and one off of Country Glen Way, are proposed;
  - Infrastructure connections are proposed to be made from Country Glen Way and Donum Lane;
  - A total of 480 units are proposed in the form of stacked townhouses (please note that the submitted plans reference a higher unit count); and,
  - 2 communal amenity spaces are proposed.
2. The separate applications for both developments are expected to be submitted in September 2021 (Stage 5) and October 2021 (Stage 6).

**STAFF COMMENTS:**

Planning

**Stage 5**

1. A Major Zoning By-law Amendment application will be required for the Stage 5 lands to permit the proposed development. Urban Exception 1932 can be removed through this application as the Holding Symbol has now been lifted.
2. A new Plan of Subdivision application will be required to permit the proposed development, as Stage 5 was not included in the previous draft approval.
3. Please note that there is a 30cm reserve along Winterset Road that will need to be lifted.
4. Staff are generally satisfied with the current layout.

5. Please consider adding another pedestrian connection between Street 1 and 6; please consider providing a pedestrian plan with the application submissions.
6. Please submit a streetscape plan with your application. The location of trees and sidewalks should be considered early in the design process.

### **Stage 6**

7. A Major Zoning By-law Amendment application will be required to permit the proposed development on the Stage 6 lands.
8. The Phase 6 lands previously received draft approval through a previous plan of subdivision application (File No. D07-16-16-0025).
9. It is understood that the applicant currently intends to allow the draft approval to lapse and pursue Site Plan Control and Plan of Condominium applications to permit the proposed development.
  - a. The proposed development requires a Complex (Manager Approval, Public Consultation) Site Plan Control Application.
10. Please note that there are 30cm reserves along Country Glen Way, Campeau, and Donum Lane.
11. It is understood that the applicant is interested in straightening the jagged section of the Country Glen Way right-of-way. Staff have reach out to the City's Corporate Real Estate Office (CREO) to discuss the possibility of a land swap, and will provide more information once a response is received.
12. Please ensure that adequate bicycle parking is provided both in and outside the proposed storage building.
13. Please review the City's Urban Design Guidelines for Transit Oriented Development to ensure that proposed development conforms to the guidelines

### **General**

14. Fees and forms for the above mentioned applications can be found [here](#); please note that each planning application fee will be reduced by 10 per cent if two or more applications are submitted at the same time and for the same lands.
15. Please ensure that each submission considers the Official Plan policies that are applicable at the time of the submission of the application
  - a. If a complete application is received by no later than the day before the new Official Plan is adopted (October 2021), it will be processed on the basis of existing Official Plan policy provided it is consistent with the 2020 Provincial Policy Statement.

- b. Applications received after the day before the new Official Plan is adopted (October 2021), will be reviewed and evaluated on the basis of the policies of the new Official Plan, which is consistent with the 2020 Provincial Policy Statement.

Please contact the Planner, Colette Gorni, at [Colette.Gorni@ottawa.ca](mailto:Colette.Gorni@ottawa.ca) if you have any questions or require additional information relating to the comments above.

### Urban Design

#### **Stage 5**

1. A design brief is required. A terms of reference is attached.
2. Please ensure the pathway connections to the Carp River Open Space Lands are accessible. This may require co-locating the two blocks in the vicinity of Lot 25 and Block 8 to provide additional length for these blocks.
3. Please consider locations for sidewalks and trees at the time of submission, as it relates to utilities and clay soils.
4. Orientation of units to minimize the need for noise walls on Winterset Road should be considered.
5. A pathway connection should be provided within Block 43 – Dry Pond to provide access to Winterset Road.

#### **Stage 6**

6. A design brief is required. A terms of reference is attached.
7. The subject lands are located within a design priority area. A high-quality site and building design are expected, suitable for a mixed use centre.
8. Consideration should be given to providing more than one product/dwelling type for this site.
9. Options to eliminate the need for a single loaded private street abutting Campeau Drive should be explored.
10. The current access on Campeau should be considered for a more urban treatment vs. a pork chop island.
11. The units abutting Campeau Drive in the Western Block, should be oriented to be in alignment with Campeau Drive vs. being offset.
12. The treatment of built form abutting Country Glen Way should be reviewed. Opportunities to regularize this property line should also be explored if possible.
13. Connectivity to the MUP along Feedmill Creek should be strengthened at key locations.

14. Consideration should be given to allowing for live/work arrangements at grade.
15. Consideration should be given to allowing for a mix of uses on-site.

Please contact Urban Design Planner Mark Young at [Mark.Young@ottawa.ca](mailto:Mark.Young@ottawa.ca) if you have any questions or require additional information relating to the comments above.

### Engineering

1. The Servicing Study Guidelines for Development Applications are available at the following address: <http://ottawa.ca/en/development-application-review-process-0/servicing-study-guidelines-development-applications>
2. Servicing and site works shall be in accordance with the following documents:
  - ⇒ Ottawa Sewer Design Guidelines (October 2012)
  - ⇒ Ottawa Design Guidelines – Water Distribution (2010)
  - ⇒ Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
  - ⇒ City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
  - ⇒ City of Ottawa Environmental Noise Control Guidelines (January, 2016)
  - ⇒ City of Ottawa Park and Pathway Development Manual (2012)
  - ⇒ City of Ottawa Accessibility Design Standards (2012)
  - ⇒ Ottawa Standard Tender Documents (latest version)
  - ⇒ Ontario Provincial Standards for Roads & Public Works (2013)
3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at [InformationCentre@ottawa.ca](mailto:InformationCentre@ottawa.ca) or by phone at (613) 580-2424 x.44455).
4. The Stormwater Management Criteria, for the subject site, is to be based on the criteria outlined in the KWMSS. Understanding that deviations have been made to the KWMSS in previous phases and that Phases 5 & 6 are the final two phases in the Arcadia subdivision area, and that infrastructure and stormwater management facilities surrounding the sites have been constructed as part of previous phases of development, it will be important to demonstrate that the surrounding infrastructure has been designed with enough capacity to support the proposed developments.

Deviations from previous design assumption shall be justified in the plans and reports.

5. As was mentioned in the pre-consultation meeting, Phases 5 & 6 are the final two phases in the Arcadia subdivision area, and the surrounding infrastructure intended to provide servicing for Phases 5 & 6 has been constructed as part of previous Phases. The plans and reports that are to be submitted in support of Phase 5 & 6 will need to demonstrate that the surrounding/downstream infrastructure has been designed with enough capacity to support the proposed developments and that any works required by the KWMSS to support the proposed developments have been completed. Any deviations within Phase 5 & 6 from previous design assumptions will need to be clearly justified.
6. Preference for servicing of Phase 5 would be entirely internal to Phase 5 (i.e., individual building service connections to Winterset to be avoided) in order to eliminate any potential service disruptions to existing residents.
7. Water Boundary condition requests must include the location of the service and the expected loads required by the proposed development. Please provide the following information:
  - i. Location of service
  - ii. Type of development and the amount of fire flow required (as per FUS, 1999).
  - iii. Average daily demand: \_\_\_ l/s.
  - iv. Maximum daily demand: \_\_\_ l/s.
  - v. Maximum hourly daily demand: \_\_\_ l/s.
8. As mentioned in the pre-consultation meeting, soil and geotechnical conditions are of potential concern for these sites. Sufficient justification should be provided to support the feasibility of Phase 5 and 6 proposals from a geotechnical perspective. For these proposals, where sensitive marine clays exist, the following information must be provided to the City:
  - A map that shows:
    - i. Location and depth of sensitive soils
    - ii. Location of utilities

iii. Location of proposed landscaping

9. MOECC ECA Requirements

It is anticipated that an MECP Environmental Compliance Approval(s) (ECA) will be needed (or existing will need to be amended) for sewers as well as for any deviation from previous ECA approvals.

10. Phase 1 ESAs and Phase 2 ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Please contact Infrastructure Project Manager Justin Armstrong at [Justin.Armstrong@ottawa.ca](mailto:Justin.Armstrong@ottawa.ca) if you have any questions or require additional information relating to the comments above.

Transportation

1. A TIA is warranted- proceed to scoping.
2. The application will not be deemed complete until the submission of the draft step 1-4, including the functional draft RMA package (if applicable) and/or monitoring report (if applicable). Although a full review of the TIA Strategy report (Step 4) is not required prior to an application, it is strongly recommended.
3. Synchro files are required with Step 4.
4. ROW protection on Campeau is 37.5m.
5. Geometric Road Design (GRD) drawings will be required with the first submission of underground infrastructure and grading drawings.
6. These drawings should include such items as, but is not limited to:
  - a. Road Signage and Pavement Marking for the subdivision;
  - b. Intersection control measure at new internal intersections; and
  - c. Location of depressed curbs and TWSIs;
  - d. Include traffic calming measures on roads within the limits of their subdivision to limit vehicular speed to 30 kph and improve pedestrian safety. These measures may include either vertical or horizontal features.
7. Site triangles at the following locations on the final plan will be required:
  - a. Local Road to Local Road: 3 metre x 3 metres
  - b. Local Road to Collector Road: 5 metre x 5 metres
  - c. Collector Road to Collector Road: 5 metre x 5 metres

- d. Collector Road to Arterial Road: 5 metre x 5 metres
8. A Road Noise Impact Study is required.
  9. Please note that all new applications (pre-consultation meetings dated after March 3, 2021) must use the NEW TRANS Trip Generation Manual when forecasting site generated trips using this manual. The TRANS committee (a joint transportation planning committee serving the National Capital region) finalized a new manual early in March 2021. The document will be available in French and English on the TRANS website <http://www.ncr-trans-rcn.ca/surveys/2009-trip-generation>. The new manual has simplified the conversion from vehicle trips to person trips and then trips by modal share.
  10. Any Development Charge road work may be front ended by the applicant, so long as the work is listed in the affordable network. Repayment will be based on warrants, as determined solely by the Transportation Services Department. A Front Ending application is required.

Please contact Transportation Project Manager Mike Giampa at [Mike.Giampa@ottawa.ca](mailto:Mike.Giampa@ottawa.ca) if you have any questions or require additional information relating to the comments above.

### Parks

1. Staff understand that the applicant intends to provide cash-in-lieu rather than dedicate land for parkland for both Stages 5 and 6.
2. Please confirm lands that are to be dedicated to the City (e.g., corner park blocks, dry ponds, open space blocks, etc.) in each application..
3. Please provide more information on pedestrian pathways to the adjacent park and open space blocks for Stage 5.
4. Please reach out to Councillor Sudds to discuss the cash-in-lieu of parkland proposal.

Please contact Parks Planner Mike Russett at [Mike.Russett@ottawa.ca](mailto:Mike.Russett@ottawa.ca) if you have any questions or require additional information relating to the comments above.

### Environment

1. Up-dated EIS, should focus on the transition from the developed lands to the natural area/watercourse blocks.
2. Implementing all recommendations from older EIS if still applicable.

Please contact Environmental Planner Matthew Hayley at [Matthew.Hayley@ottawa.ca](mailto:Matthew.Hayley@ottawa.ca) if you have any questions or require additional information relating to the comments above.

## Forestry

### **TCR requirements:**

1. A Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City
  - a. An approved TCR is a requirement of Site Plan approval.
  - b. The TCR may be combined with the LP provided all information is supplied
2. As of January 1 2021, any removal of privately-owned trees 10cm or larger in diameter, or publicly (City) owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 – 340); the permit will be based on an approved TCR and made available at or near plan approval.
3. The Planning Forester from Planning and Growth Management as well as foresters from Forestry Services will review the submitted TCR
  - a. If tree removal is required, both municipal and privately-owned trees will be addressed in a single permit issued through the Planning Forester
  - b. Compensation may be required for city owned trees – if so, it will need to be paid prior to the release of the tree permit
4. The TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition
5. Please identify trees by ownership – private onsite, private on adjoining site, city owned, co-owned (trees on a property line)
6. the TCR must list all trees on adjacent sites if they have a critical root zone that extends onto the development site
7. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained
8. All retained trees must be shown and all retained trees within the area impacted by the development process must be protected as per City guidelines available at [Tree Protection Specification](#) or by searching Ottawa.ca
  - a. The location of tree protection fencing must be shown on a plan;
  - b. Show the critical root zone of the retained trees; and,
  - c. If excavation will occur within the critical root zone, please show the limits of excavation .



9. The City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.

For more information on the process or help with tree retention options, contact Mark Richardson [mark.richardson@ottawa.ca](mailto:mark.richardson@ottawa.ca) or on [City of Ottawa](#)

### **Landscape Plan Tree Planting requirements:**

#### 10. Minimum Setbacks

- a. Maintain 1.5m from sidewalk or MUP/cycle track.
- b. Maintain 2.5m from curb
- c. Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
- d. Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing.
- e. Adhere to Ottawa Hydro's planting guidelines (species and setbacks) when planting around overhead primary conductors.

#### 11. Tree specifications

- a. Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
- b. Maximize the use of large deciduous species wherever possible to maximize future canopy coverage
- c. Tree planting on city property shall be in accordance with the City of Ottawa's Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- d. Plant native trees whenever possible
- e. No root barriers, dead-man anchor systems, or planters are permitted.
- f. No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree)

#### 12. Hard surface planting

- a. Curb style planter is highly recommended
- b. No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.

- c. Trees are to be planted at grade

### 13. Soil Volume

- a. Please ensure adequate soil volumes are met:

Tree Type/Size	Single Tree Soil Volume (m3)	Multiple Tree Soil Volume (m3/tree)
Ornamental	15	9
Columnar	15	9
Small	20	12
Medium	25	15
Large	30	18
Conifer	25	15

\*\*Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

### 14. Sensitive Marine Clay

- a. Please follow the City's 2017 Tree Planting in Sensitive Marine Clay guidelines.

Please contact Adam Palmer at [adam.palmer@Ottawa.ca](mailto:adam.palmer@Ottawa.ca) if you have any questions or require additional information relating to the landscape plan tree planting requirements.

### MVCA

1. MVCA staff have recently reviewed permit applications and related documents required as part of the Lifting of a Holding Symbol application for the Stage 5 lands.
2. Please ensure that the Campeau pond setbacks and LRT alignment are considered as the site designs for both applications are further refined.
3. Please note that the MVCA will be conducting a floodplain mapping update by the end of the year.
4. Please refer to MVCA comments provided for previous stages of the Arcadia subdivision.

Please contact the MVCA's Planning Manager, Matt Craig, at [MCraig@mvc.on.ca](mailto:MCraig@mvc.on.ca) if you have any questions or require additional information relating to the comments above.

**NEXT STEPS:**

Please refer to the links to [Guide to preparing studies and plans](#) and [fees](#) for further information. Additional information is available related to [building permits](#), [development charges](#), and the [Accessibility Design Standards](#). Be aware that other fees and permits may be required, outside of the development review process. You may obtain background drawings by contacting [informationcentre@ottawa.ca](mailto:informationcentre@ottawa.ca).

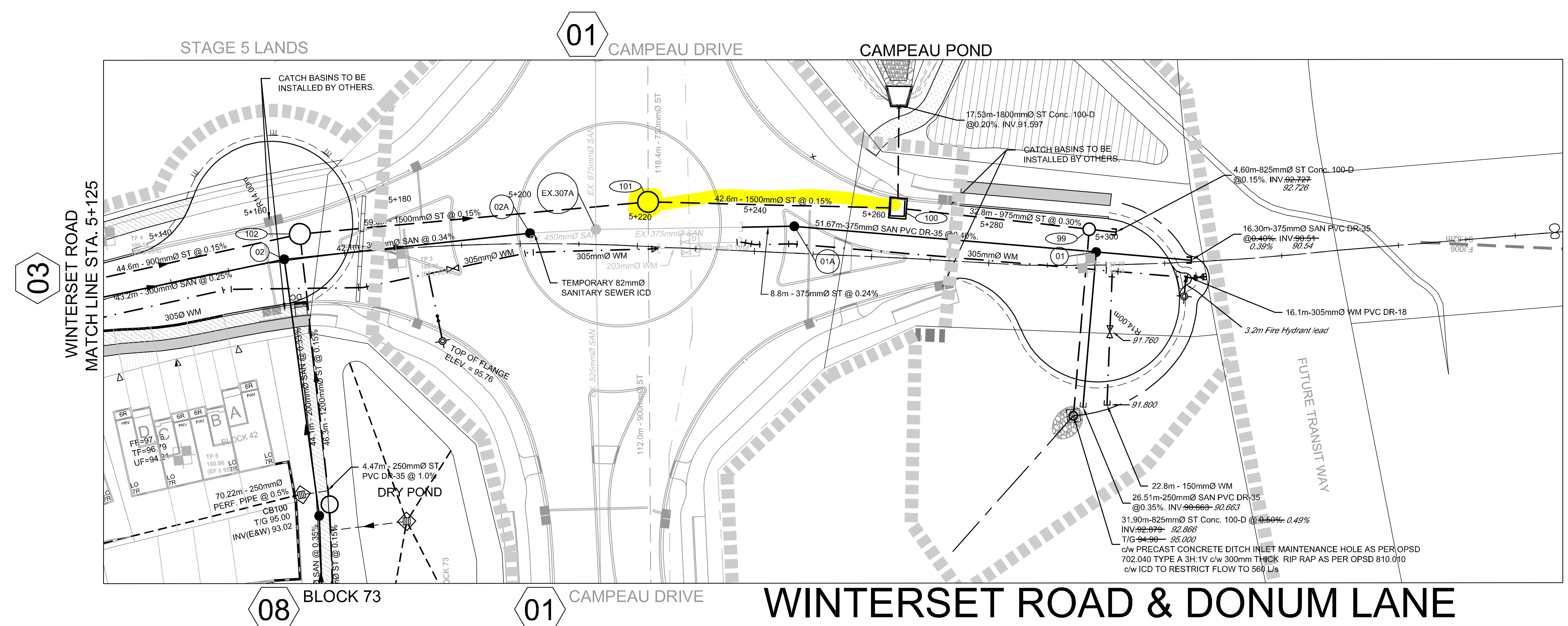
These pre-con comments are valid for one year. If you submit a development application(s) after this time, you may be required to meet for another pre-consultation meeting and/or the submission requirements may change. You are as well encouraged to contact us for a follow-up meeting if the plan/concept will be further refined.

Please do not hesitate to Colette Gorni, at [Colette.Gorni@ottawa.ca](mailto:Colette.Gorni@ottawa.ca), if you have any questions.

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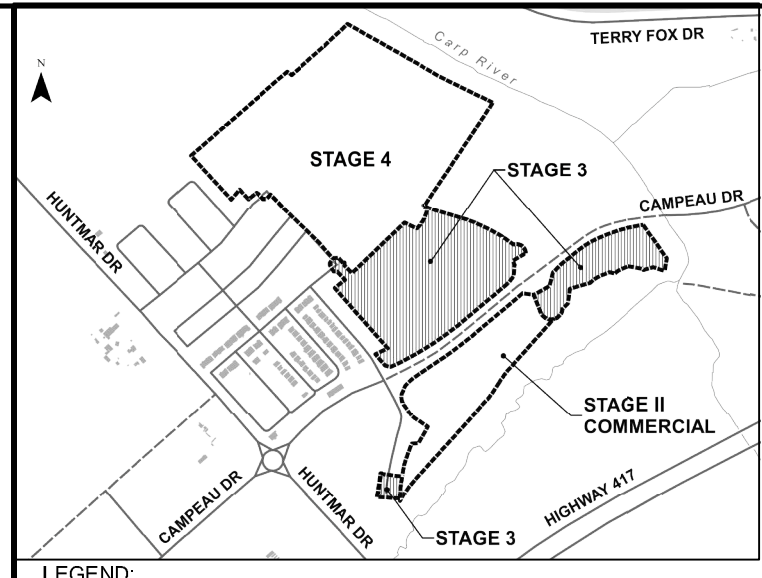
**Appendix A3**

As-Built Information



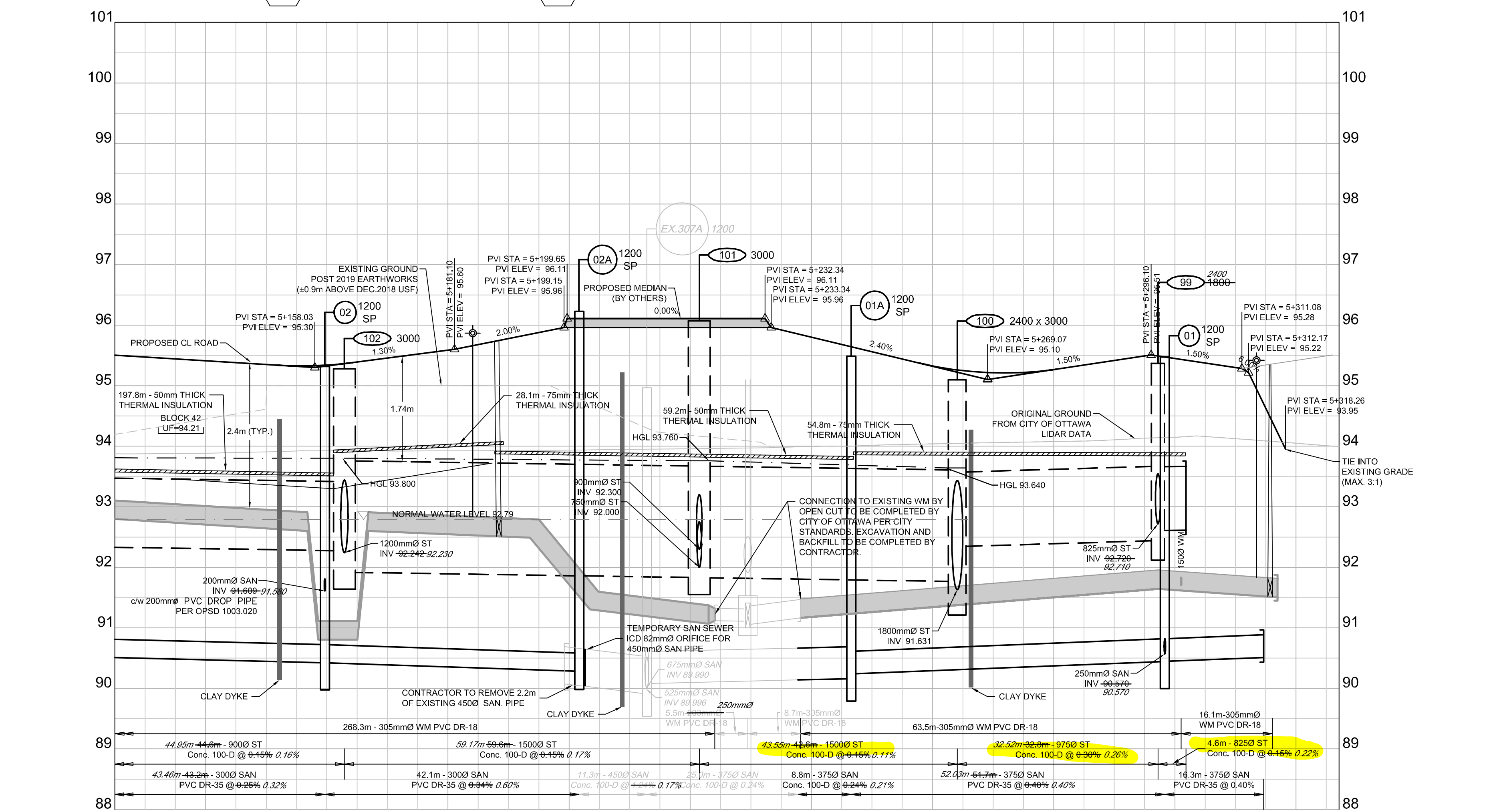
**AS CONSTRUCTED INFORMATION**  
 PRODUCED FROM INFORMATION PROVIDED BY FIELD INSPECTOR  
 Date: DECEMBER 17, 2019  
 J.L. RICHARDS & ASSOCIATES LIMITED

NOTE: ALL SANITARY SERVICES FOR BLOCKS 38, 39, 40, 41 AND 42 ARE REQUIRED TO HAVE CONTROLLED SETTLEMENT JOINTS INSTALLED.



**LEGEND:**

- EXISTING CATCH BASIN
- PROPOSED CATCH BASIN c/w ICD
- PROPOSED MAINTENANCE HOLE c/w ICD
- FUTURE CATCH BASIN BY OTHERS
- PROPOSED CATCH BASIN LEAD
- PROPOSED ELBOW CATCH BASIN
- PROPOSED TEE CATCH BASIN
- PROPOSED S-WAY CATCH BASIN
- 200mm PERFORATED PIPE AS PER CITY OF OTTAWA STANDARD DRAWING S29
- PROPOSED WATERMAIN, VALVE & HYDRANT
- PROPOSED WATERMAIN REDUCER
- EXISTING WATERMAIN, VALVE & HYDRANT
- EXISTING SANITARY SEWER & MANHOLE
- EXISTING STORM SEWER & MANHOLE
- PROPOSED SANITARY SEWER & MANHOLE
- PROPOSED STORM SEWER & MANHOLE
- MANHOLE NUMBER AND SIZE (mm) c/w SAFETY PLATFORM PER OPSD 840-0
- SINGLE SERVICE CONNECTION (STORM, SANITARY AND WATER)
- DOUBLE SERVICE CONNECTION (STORM, SANITARY AND WATER)
- SERVICE CONNECTION REQUIRING BENDS
- PHASING LIMIT
- ACOUSTIC WALL
- CONCRETE SIDEWALK
- ASPHALT SIDEWALK
- GRAVEL ACCESS ROAD



DESIGN PROFILE ELEVATIONS	W.M. TOP ELEVATIONS	STORM SEWER INV. ELEVATION	SANITARY SEWER INV. ELEVATION	C.L. ROADWAY STATION
95.505	92.461	90.531	91.250	5+25.00
95.411	92.461	90.531	91.250	5+40.00
95.354	92.461	90.531	91.250	5+150.19
95.307	92.461	90.531	91.250	5+152.20
95.307	92.461	90.531	91.250	5+158.79
95.307	92.461	90.531	91.250	5+158.85
95.307	92.461	90.531	91.250	5+160.00
95.307	92.461	90.531	91.250	5+162.90
95.307	92.461	90.531	91.250	5+165.05
95.307	92.461	90.531	91.250	5+166.85
95.307	92.461	90.531	91.250	5+172.47
95.307	92.461	90.531	91.250	5+180.00
95.307	92.461	90.531	91.250	5+184.08
95.307	92.461	90.531	91.250	5+189.87
95.307	92.461	90.531	91.250	5+200.00
95.307	92.461	90.531	91.250	5+201.05
95.307	92.461	90.531	91.250	5+212.84
95.307	92.461	90.531	91.250	5+212.84
95.307	92.461	90.531	91.250	5+220.00
95.307	92.461	90.531	91.250	5+224.05
95.307	92.461	90.531	91.250	5+229.50
95.307	92.461	90.531	91.250	5+238.24
95.307	92.461	90.531	91.250	5+240.00
95.307	92.461	90.531	91.250	5+246.53
95.307	92.461	90.531	91.250	5+246.53
95.307	92.461	90.531	91.250	5+260.00
95.307	92.461	90.531	91.250	5+264.08
95.307	92.461	90.531	91.250	5+272.07
95.307	92.461	90.531	91.250	5+280.00
95.307	92.461	90.531	91.250	5+281.03
95.307	92.461	90.531	91.250	5+313.50
95.307	92.461	90.531	91.250	5+314.46
95.307	92.461	90.531	91.250	5+315.73
95.307	92.461	90.531	91.250	5+318.89
95.307	92.461	90.531	91.250	5+320.00
95.307	92.461	90.531	91.250	5+327.12

No.	ISSUE / REVISION	DDMMYY
09	AS-CONSTRUCTED INFORMATION ADDED	17/12/2019
08	ISSUED FOR SH-006 - GEOMETRY UPDATES	13/09/2019
07	ISSUED FOR SH-003 - SERVICE LATERAL UPDATES FOR 2-CAR GARAGE UNITS	13/08/2019
06	ISSUED FOR CONSTRUCTION	28/06/2019
05	ISSUED FOR TENDER	11/06/2019
04	ISSUED TO MECP REVISED PER CITY COMMENTS	23/05/2019

This drawing is copyright protected and may not be reproduced or used for purposes other than execution of the described work without the express written consent of J.L. Richards & Associates Limited.  
 VERIFY SHEET SIZE AND SCALES. BAR TO THE RIGHT IS 25mm IF THIS IS A FULL SIZE DRAWING.  
 SCALE: 1:500H, 1:50V  
 NOTE: SEE LEGEND & GENERAL NOTES FOR SERVICE BEND REQUIREMENTS.

CLIENT:  
**minto Communities**

CONSULTANT:  
**JLR J.L. Richards ENGINEERS - ARCHITECTS - PLANNERS**

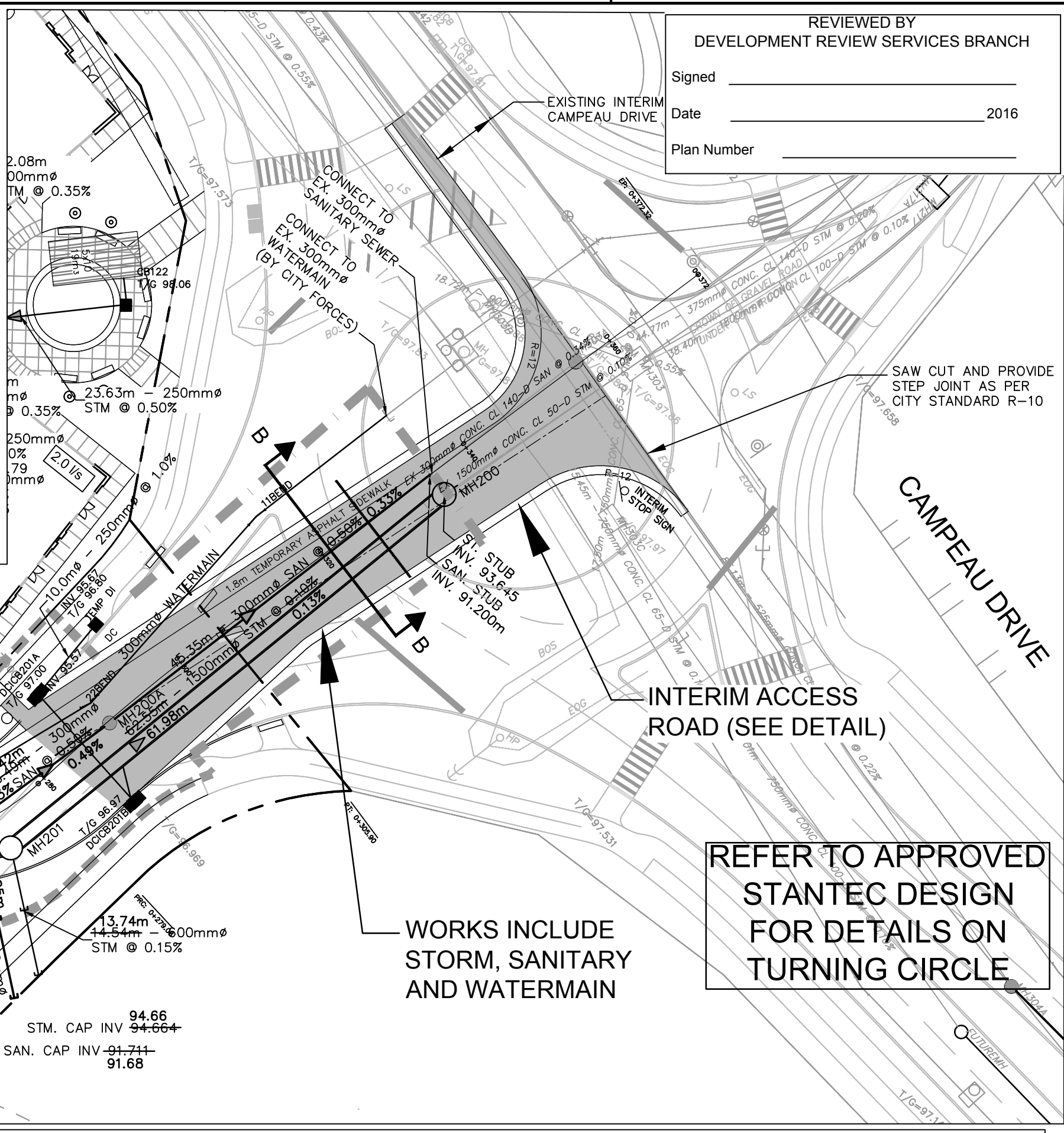
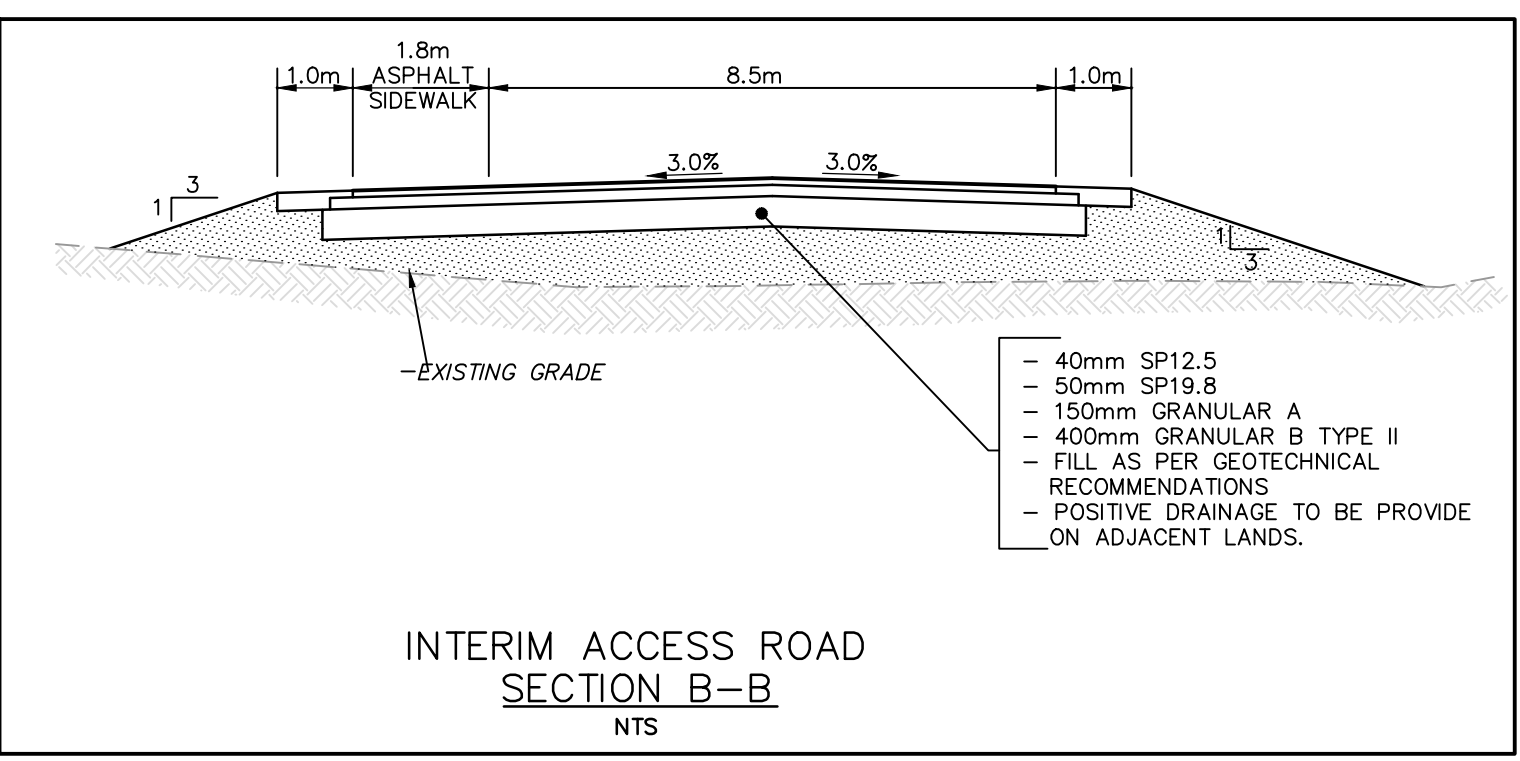
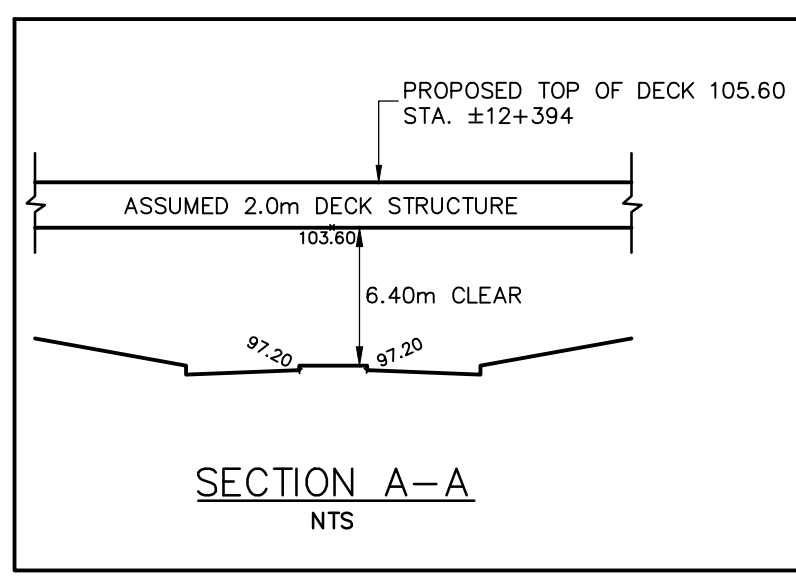
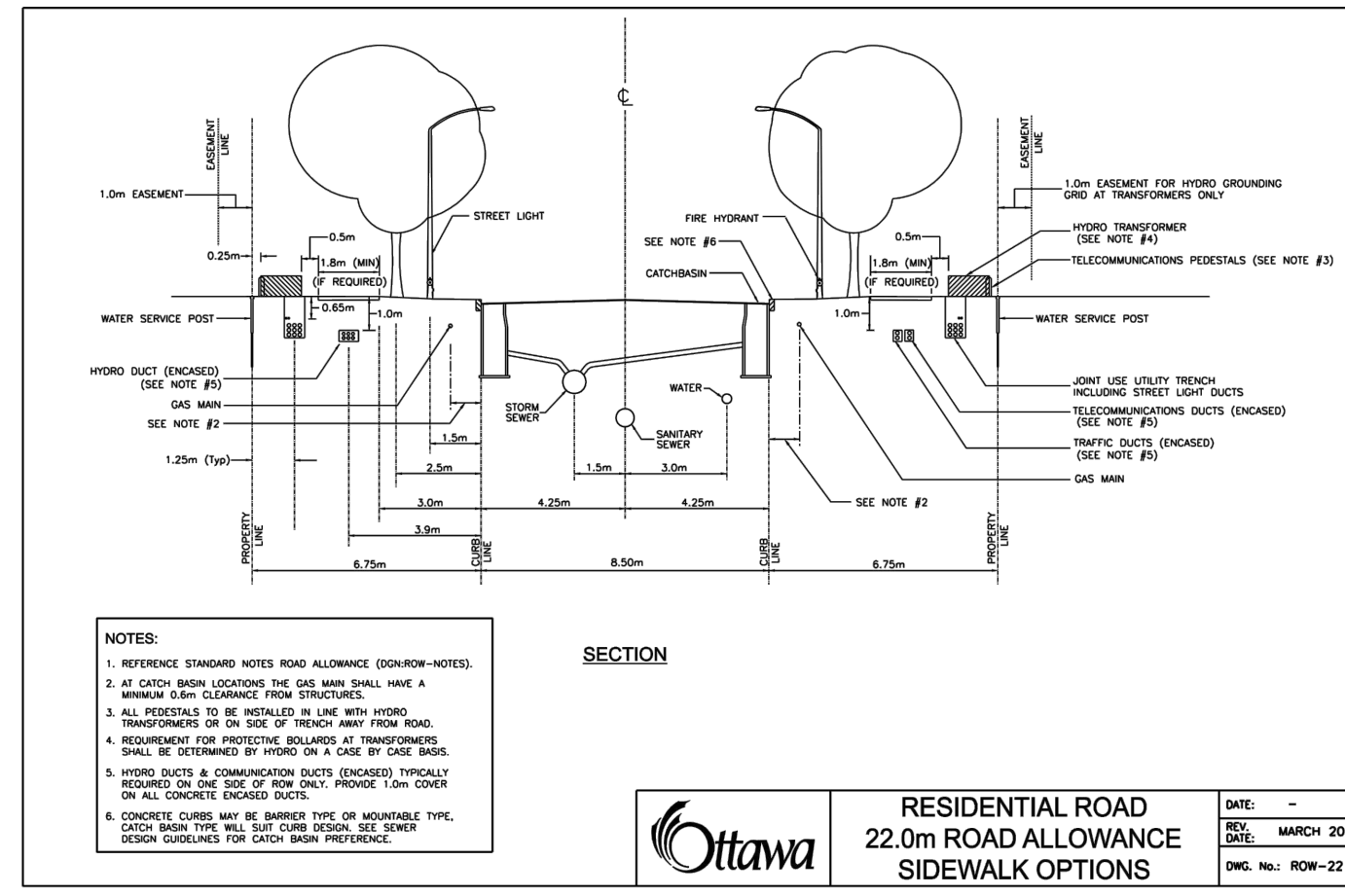
PROFESSIONAL STAMP  
 PROJECT NORTH

PROJECT:  
**MINTO COMMUNITIES INC. ARCADIA STAGE 3**  
 450 HUNTMAR DRIVE

DRAWING:  
**PLAN & PROFILE WINTERSET ROAD & DONUM LANE STA 5+125 TO 5+327.12**

DESIGN: AT  
 DRAWN: CJM  
 CHECKED: LD  
 JLR #: 26299-03

DRAWING #:  
**02**



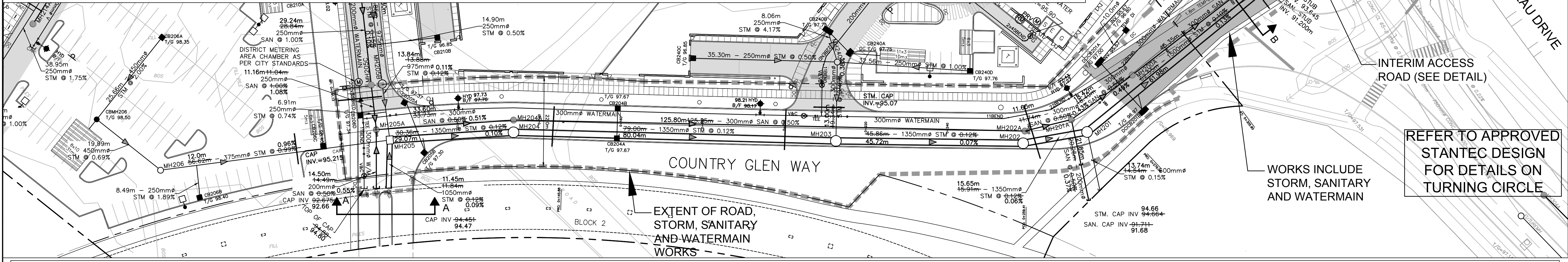
REVIEWED BY  
DEVELOPMENT REVIEW SERVICES BRANCH

Signed \_\_\_\_\_  
Date \_\_\_\_\_ 2016  
Plan Number \_\_\_\_\_

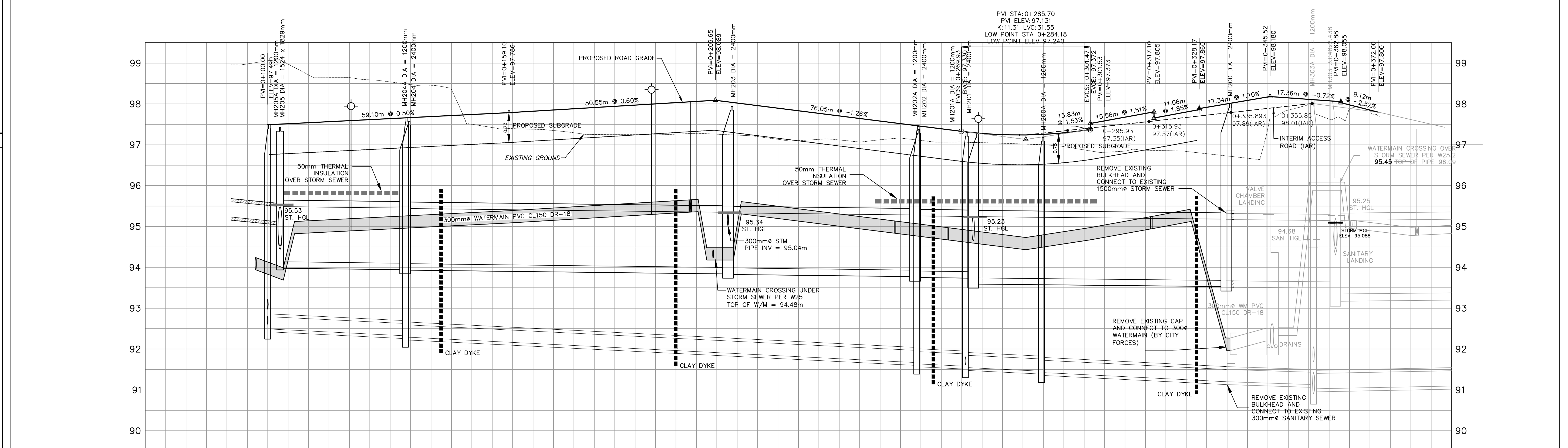
APPROVED  REFUSED

THIS DAY OF \_\_\_\_\_, 20\_\_\_\_

DERRICK MOODIE, ACTING MANAGER  
DEVELOPMENT REVIEW, SUBURBAN SERVICES



No.	Description	Date	Checked
10	As-built	17:01:17	DPS
9	Revised as per City Comments	16:08:26	DGY
8	Revised as per new Campeau & asbuilt w/m	16:08:03	DGY
7	Issued for Construction	16:07:13	DGY
6	Add Interim Access Rd	16:05:16	DGY
5	Issued for Tender	16:04:05	DGY
4	Revised as per City Comments	14:10:02	DGY
3	Revised as per City Comments	14:08:22	DGY
2	Issued for SPA Resubmission	14:06:27	DGY
1	Issued for SPA	13.11.18	D.G.Y.



PROP OF ROAD	TOP OF WATERMAIN	STORM SEWER	SANITARY SEWER	STATION
97.90	94.24	95.105	92.58E	0+080
97.90	94.13	94.14N	94.45N	0+100
97.90	94.12	94.12E	94.12E	0+120
97.90	95.30	95.07M	92.21S	0+140
97.90	95.32	95.07M	92.23N	0+160
97.90	95.43	80.04m	92.23N	0+180
97.91	95.55	79.99m - 1350mm CONC. CL 100-D STM @ 0.12%	92.23N	0+200
97.91	95.64	45.72m - 1350mm CONC. CL 100-D STM @ 0.12%	92.23N	0+220
97.91	95.60	15.65m - 1350mm CONC. CL 100-D STM @ 0.12%	92.23N	0+240
97.91	95.62	45.72m - 1350mm CONC. CL 100-D STM @ 0.12%	92.23N	0+260
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+280
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+300
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+320
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+340
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+360
97.91	95.62	11.65m - 300mm CONC. CL 140-D SAN @ 0.50%	92.23N	0+380

All measurements and conditions must be checked on the work by the contractor. This drawing not to be used for construction until signed.

Date \_\_\_\_\_

plotted \_\_\_\_\_

drawn by DPS  
checked by DGY  
printed \_\_\_\_\_

scale HOR 1:500  
VERT. 1:50  
date NOV. 2013  
file 35355

Arcadia Retail Development  
Kanata, Ontario

Minto Properties  
200 Kent Street • Suite 180 • Ottawa, Ontario • K1P 0B6  
Telephone: (613)782-3137 Fax: (613)782-5777

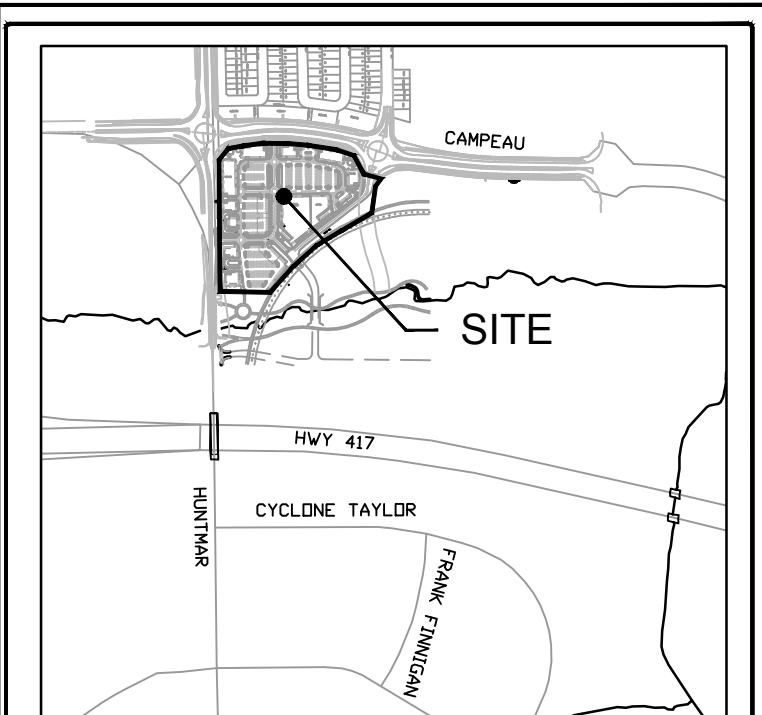
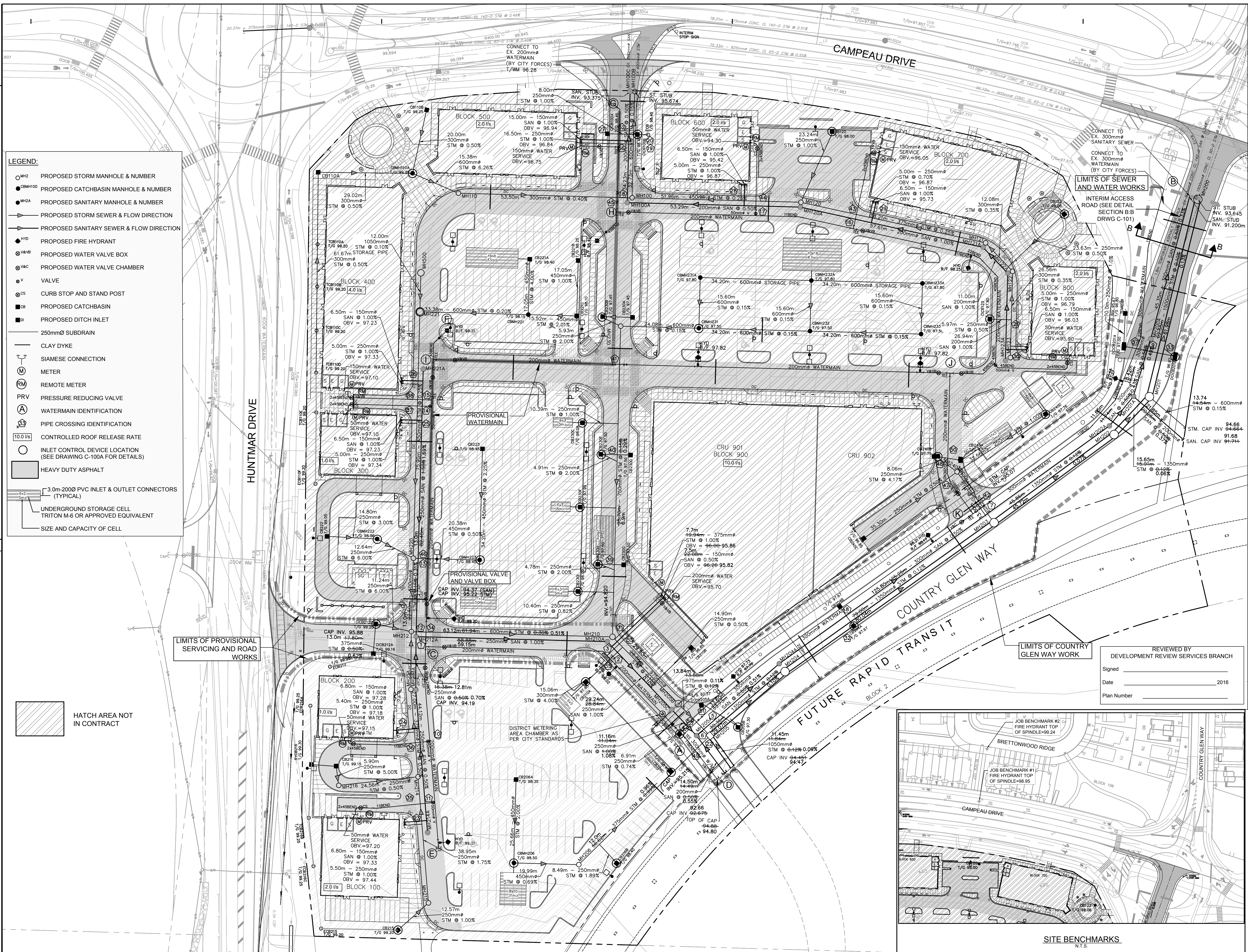
drawing title:  
PLAN AND PROFILE  
COUNTRY GLEN WAY  
370 HUNTER DRIVE  
OTTAWA, ON.

drawing no.  
C-101

IBI GROUP

333 Preston Street Tower 1, Suite 400  
Ottawa, Ontario Canada K1S 5N4 Tel (613)225-1311 FAX (613)225-9868

D07-12-14-0014



APPROVED  REFUSED

THIS DAY OF \_\_\_\_\_, 20\_\_

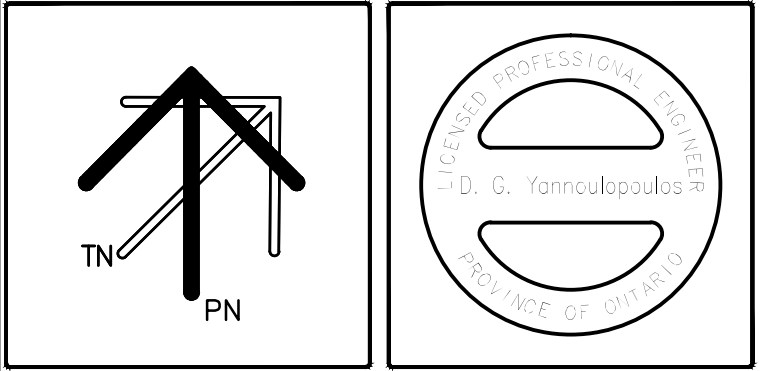
DERRICK MOODIE, ACTING MANAGER  
DEVELOPMENT REVIEW, SUBURBAN SERVICES

No.	Description	Date	Checked
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9	Revised as per City Comments	16:08:26	DGY
8	Revised Campeau Dr per Stantec Plans	16:08:03	DGY
7	Issued for Construction	16:07:13	DGY
6	Add Interim Access Rd.	16:05:16	DGY
5	Issued for Tender	16:04:05	DGY
4	Revised as per City Comments	14:10:02	DGY
3	Revised as per City Comments	14:08:22	DGY
2	Issued for SPA Resubmission	14:06:27	DGY
1	Issued for SPA	13:11:18	D.G.Y.

All measurements and conditions must be checked on the work by the contractor. This drawing not to be used for construction until signed.

Date \_\_\_\_\_

plotted by: 35355-ArcadiaComm\5.9 Drawings\5904\Projects\C-100.dwg  
Royal Names Servicing Plan Plot Size: AIA STANDARD-TITLE.B7B  
Plot Scale: 1:1 Plotted At: 3/16/2017 11:06 AM Last Saved By: dgm\m Last Saved At: Mar-16-17



drawn by	DPS	scale	1:500
checked by	DGY	date	NOV. 2013
printed		file	35355

REVIEWED BY  
DEVELOPMENT REVIEW SERVICES BRANCH

Signed \_\_\_\_\_  
Date \_\_\_\_\_ 2016  
Plan Number \_\_\_\_\_

**Arcadia Retail Development**  
Kanata, Ontario

Minto Properties  
200 Kent Street • Suite 180 • Ottawa, Ontario • K1P 0B6  
Telephone: (613)782-3137 Fax: (613)782-5777

drawing title:  
**SITE SERVICING PLAN**  
370 HUNTMAR DRIVE  
OTTAWA, ON.

drawing no.  
**C-100**

333 Preston Street Tower 1, Suite 400  
Ottawa, Ontario Canada K1S 5N4 Tel  
(613)225-1311 FAX (613)225-9868

D07-12-14-0014

WATERMAIN SCHEDULE table with columns: STATION, DESCRIPTION, FINISHED, TOP OF, AS-BUILT. Rows include various pipe sizes, materials, and fittings like 3000 x 3000 TEE, 22 5/8 VERTICAL BEND, etc.

CATCH BASIN DATA TABLE with columns: STRUCTURE ID, AREA ID, TOP OF GRATE, ELEVATION (INLET, OUTLET), DIAMETER (mm), TYPE, 100 yr HEAD, FLOW (l/s), CUSTOM IPEX TEMPEST ICD. Rows include structures like CB123, CB120, CB110A, etc.

CROSSING SCHEDULE table with columns: Location, Below Pipe (Size, Invert, Difference), Above Pipe (Size, Invert, Difference). Rows list crossing points for various pipe sizes and materials.

DRAWING NOTES

- 1.0 GENERAL
1.1 CONTRACTOR TO VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION.
1.2 DO NOT SCALE DRAWINGS.
1.3 CONTRACTOR TO REPORT ALL DISCOVERIES OF ERRORS, OMISSIONS OR DISCREPANCIES TO THE ARCHITECT OR DESIGN ENGINEER AS APPLICABLE.
... (Notes 1.4 to 1.19)

2.0 SANITARY

- 2.1 ALL SANITARY SEWER MAINS TO BE CSA CERTIFIED, BELL AND SPIGOT TYPE. ONLY FACTORY FITTINGS TO BE USED.
2.2 ALL SANITARY MAINTENANCE HOLES TO BE 1.2m DIAMETER AS PER CITY OF OTTAWA STANDARDS.
... (Notes 2.3 to 2.6)

Commercial - Storm STRUCTURE TABLE

Table with columns: NAME, RIM ELEV., INVERT IN AS-BUILT, INVERT OUT, INVERT OUT AS-BUILT, DESCRIPTION. Lists storm structures like MH100, MH100B, MH110, etc.

3.0 STORM

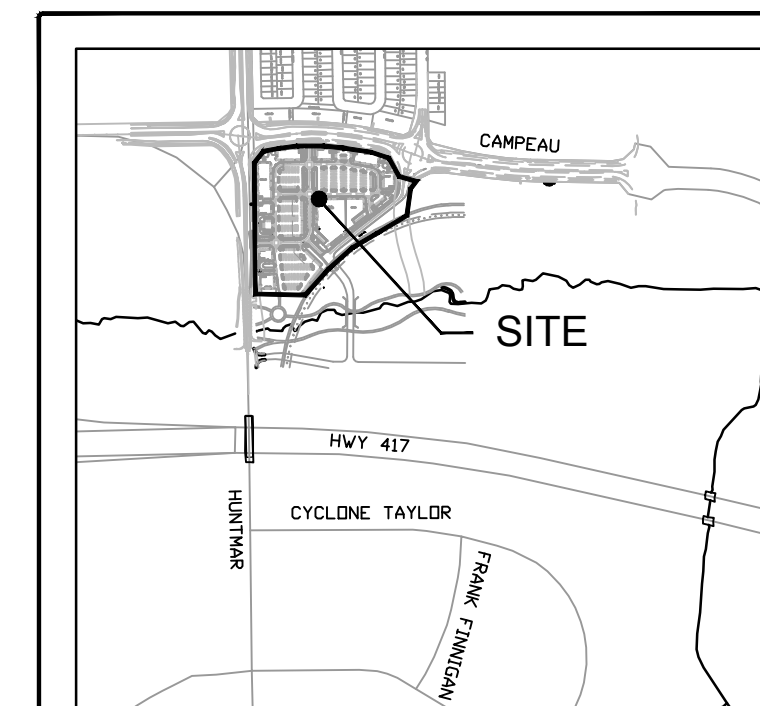
- 3.1 ALL STORM SEWERS TO BE CSA CERTIFIED, BELL AND SPIGOT TYPE. ALL STORM SEWERS TO BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS.
3.2 ALL STORM MAINTENANCE HOLES TO BE SIZED IN ACCORDANCE WITH THE PLANS AND AS PER CITY OF OTTAWA STANDARDS.
... (Notes 3.3 to 3.11)

4.0 WATER

- 4.1 ALL WATERMANS TO BE PVC DR 18, WITH MINIMUM COVER OF 2.4m AND INSTALLED PER CITY OF OTTAWA STANDARDS.
4.2 THRUST BLOCKS TO BE INSTALLED AT ALL BENDS, TEES, AND CAPS.
... (Notes 4.3 to 4.10)

5.0 PARKING LOT AND WORK IN PUBLIC RIGHTS OF WAY

- 5.1 CONTRACTOR TO REINSTATE ROAD CUTS PER CITY OF OTTAWA STANDARD R-10.
5.2 THE CONTRACTOR SHALL PREPARE A TRAFFIC MANAGEMENT PLAN FOR REVIEW AND APPROVAL BY THE CITY OF OTTAWA.
... (Notes 5.3 to 5.14)

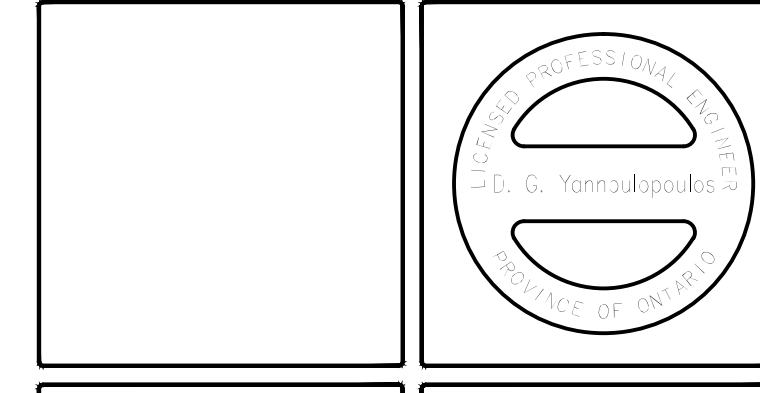


APPROVED [ ] REFUSED [ ]
THIS \_\_\_ DAY OF \_\_\_, 20\_\_
DERRICK MOODIE, ACTING MANAGER
DEVELOPMENT REVIEW, SUBURBAN SERVICES

Revision table with columns: No., Description, Date, Checked. Lists revisions 1-9 including 'As-built', 'Revised as per City Comments', 'Issued for Construction', etc.

All measurements and conditions must be checked on the work by the contractor. This drawing not to be used for construction until signed.

plotted: J:\3535-Arcadia\Comm\5.9 - Drawings\5901\5901.ctb - 100%
Plot Size: 11 Plot Size: 4in STANDARD-FULL.CT
Plot Scale: 1:1 Plot Date: 3/16/2017 11:08 AM Last Saved By: dmlam Last Saved At: Mar 16, 17



drawn by: DPS scale: 1:500
checked by: DGY date: NOV. 2013
printed: file: 35355

Arcadia Retail Development



drawing title: DETAILS AND SCHEDULES 370 HUNTMAR DRIVE OTTAWA, ON.

IBI GROUP logo and contact information: 333 Preston Street Tower 1, Suite 400 Ottawa, Ontario Canada K1S 5N4 Tel (613)225-1311 FAX (613)225-9868

PAVEMENT STRUCTURE LIGHT DUTY table with columns: NAME, RIM ELEV., INVERT IN AS-BUILT, INVERT OUT, INVERT OUT AS-BUILT, DESCRIPTION. Lists pavement structures like BLK100A, BLK200A, etc.

COUNTRY GLEN WAY INTERIM ACCESS ROAD table with columns: NAME, RIM ELEV., INVERT IN AS-BUILT, INVERT OUT, INVERT OUT AS-BUILT, DESCRIPTION. Lists pavement structures like MH100, MH100B, etc.

007-12-14-0014



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## **Appendix A4**

Servicing Study Checklist

**MINTO COMMUNITIES INC. ARCADIA STAGE 6  
DEVELOPMENT SERVICING STUDY CHECKLIST**

<b>REFERENCED STUDIES AND REPORTS</b>	<b>REFERENCE</b>
Site Servicing Report for Minto Communities Inc., Arcadia Stage 6 (J.L. Richards & Associates Limited, Revision 0 dated July 2022)	SSR
Geotechnical Investigation, Proposed Residential Development – Arcadia Stage 6, Campeau Drive - Ottawa, Ontario” Report Number PG5648-1, Revision 4 dated June 30, 2022	GR

<b>4.1</b>	<b>GENERAL CONTENT</b>	<b>REFERENCE</b>
<input type="checkbox"/>	Executive Summary (for larger reports only).	N/A
<input checked="" type="checkbox"/>	Date and revision number of the report.	SSR (Title Page)
<input checked="" type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development.	SSR (Figure 1-1, Appendix 'A1')
<input checked="" type="checkbox"/>	Plan showing the site and location of all existing services.	Site Servicing Plan (S1)
<input checked="" type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	SSR (Sect. 1.0)
<input checked="" type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies.	SSR (Appendix 'A2')
<input checked="" type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	SSR (Sect. 1.0)
<input checked="" type="checkbox"/>	Statement of objectives and servicing criteria.	SSR (Sect. 2.1, 3.1, 3.2, 4.1, 4.2)
<input checked="" type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area.	SSR (Sect. 1.4) Site Servicing Plan (S1)
<input checked="" type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	SSR (Sect. 3.1, 4.1)
<input checked="" type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	GR Grading Plans (G1, G2) Ponding Plans (SWM1, SWM2)

<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
<input type="checkbox"/>	Proposed phasing of the development, if applicable.	N/A
<input checked="" type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing.	GR
<input checked="" type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> <li>▪ Metric scale</li> <li>▪ North arrow (including construction North)</li> <li>▪ Key plan</li> <li>▪ Name and contact information of applicant and property owner</li> <li>▪ Property limits, including bearings and dimensions</li> <li>▪ Existing and proposed structures and parking areas</li> <li>▪ Easements, road widening and rights-of-way</li> <li>▪ Adjacent street names</li> </ul>	All Drawings

4.2	DEVELOPMENT SERVICING REPORT: WATER	REFERENCE
<input checked="" type="checkbox"/>	Confirm consistency with Master Servicing Study, if available.	SSR (Sect. 2)
<input checked="" type="checkbox"/>	Availability of public infrastructure to service proposed development.	SSR (Sect. 1.4, 2.4) Site Servicing Plan (S1)
<input checked="" type="checkbox"/>	Identification of system constraints.	SSR (Sect. 2.0)
<input checked="" type="checkbox"/>	Identify boundary conditions.	SSR (Sect. 2.4)
<input checked="" type="checkbox"/>	Confirmation of adequate domestic supply and pressure.	SSR (Sect. 2.5, Appendix B)
<input checked="" type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	SSR (Sect. 2.3, 2.5, Appendix B)
<input checked="" type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	SSR (Sect. 2.5)
<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modelling is required to confirm servicing for all defined phases of the project, including the ultimate design.	N/A
<input checked="" type="checkbox"/>	Address reliability requirements, such as appropriate location of shutoff valves.	Site Servicing Plan (S1)
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification.	N/A

<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	SSR (Sect. 2.0)
<input checked="" type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants), including special metering provisions.	SSR (Sect. 2.0) Site Servicing Plan (S1)
<input type="checkbox"/>	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	SSR (Sect. 2.2)
<input checked="" type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	SSR (Appendix B)

<b>4.3</b>	<b>DEVELOPMENT SERVICING REPORT: WASTEWATER</b>	<b>REFERENCE</b>
<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	SSR (Sect. 3.2)
<input checked="" type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	SSR (Sect. 3)
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the Guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	SSR (Sect. 1.4, 3.1, Appendix C)
<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable.)	SSR (Sect. 3.3)
<input checked="" type="checkbox"/>	Calculations related to dry weather and wet weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	SSR (Appendix C)
<input checked="" type="checkbox"/>	Description of proposed sewer network, including sewers, pumping stations and forcemains.	SSR (Sect. 3.4, Appendix C)

<input checked="" type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	GR
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input checked="" type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	SSR (Section 3.4)
<input type="checkbox"/>	Special considerations, such as contamination, corrosive environment, etc.	N/A

<b>4.4</b>	<b>DEVELOPMENT SERVICING REPORT: STORMWATER</b>	<b>REFERENCE</b>
<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints, including legality of outlets (i.e., municipal drain, right-of-way, watercourse, or private property).	SSR (Sect. 1.6, 6.1, 6.2)
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	SSR (Sect. 1.3)
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Drawing DST, ODST
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected sub watersheds, taking into account long-term cumulative effects.	SSR (Sect. 4.2, 4.3)
<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	SSR (Sect. 4.3)
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	SSR (Sect. 4, Drawing S1, DST, ODST)
<input type="checkbox"/>	Setback from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	N/A
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Appendix A2

<input checked="" type="checkbox"/>	Confirm consistency with subwatershed and Master Servicing Study, if applicable study exists.	SSR (Sect. 4)
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:2 year return period) and major events (1:100 year return period).	SSR (Sect. 4, Appendix E)
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
<input checked="" type="checkbox"/>	Calculate pre- and post-development peak flow rates, including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	SSR (Sect. 4, Appendix E)
<input type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	N/A
<input checked="" type="checkbox"/>	Proposed minor and major systems, including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Site Servicing Plan (S1) Plan and Profile Drawings Ponding Plans Appendix E
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	N/A
<input checked="" type="checkbox"/>	Identification of potential impacts to receiving watercourses.	SSR (Sect 4.3)
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input checked="" type="checkbox"/>	Description of how the conveyance and storage capacity will be achieved for the development.	SSR (Sect. 4.3)
<input checked="" type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	SSR (Sect. 4.3) Site Servicing Plan (S1) Ponding Plans (SWM1 & SWM2) Appendix E
<input checked="" type="checkbox"/>	Inclusion of hydraulic analysis, including hydraulic grade line elevations.	SSR (Sect. 4.3, Appendix E)
<input checked="" type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	SSR (Sect. 5.0) Erosion & Sediment Control Plan (ESC)
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input checked="" type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	GR

4.5	APPROVAL AND PERMIT REQUIREMENTS	REFERENCE
The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development, as well as the relevant issues affecting such approval. The approval and permitting shall include but not be limited to the following:		
<input type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams, as defined in the Act.	N/A
<input type="checkbox"/>	Application for Environmental Compliance Approval (ECA) under the Ontario Water Resources Act.	N/A
<input type="checkbox"/>	Changes to Municipal Drains.	N/A
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation, etc.).	N/A

4.6	CONCLUSION CHECKLIST	REFERENCE
<input checked="" type="checkbox"/>	Clearly stated conclusions and recommendations.	SSR (Sect. 2, 3 & 4)
<input type="checkbox"/>	Comments received from review agencies, including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	N/A (first submission)
<input checked="" type="checkbox"/>	All draft and final reports shall be signed and stamped by a Professional Engineer registered in Ontario.	SSR, Drawings

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## **Appendix B1**

Water Demands and FUS  
Calculations



**WATERMAIN DEMAND CALCULATION SHEET**

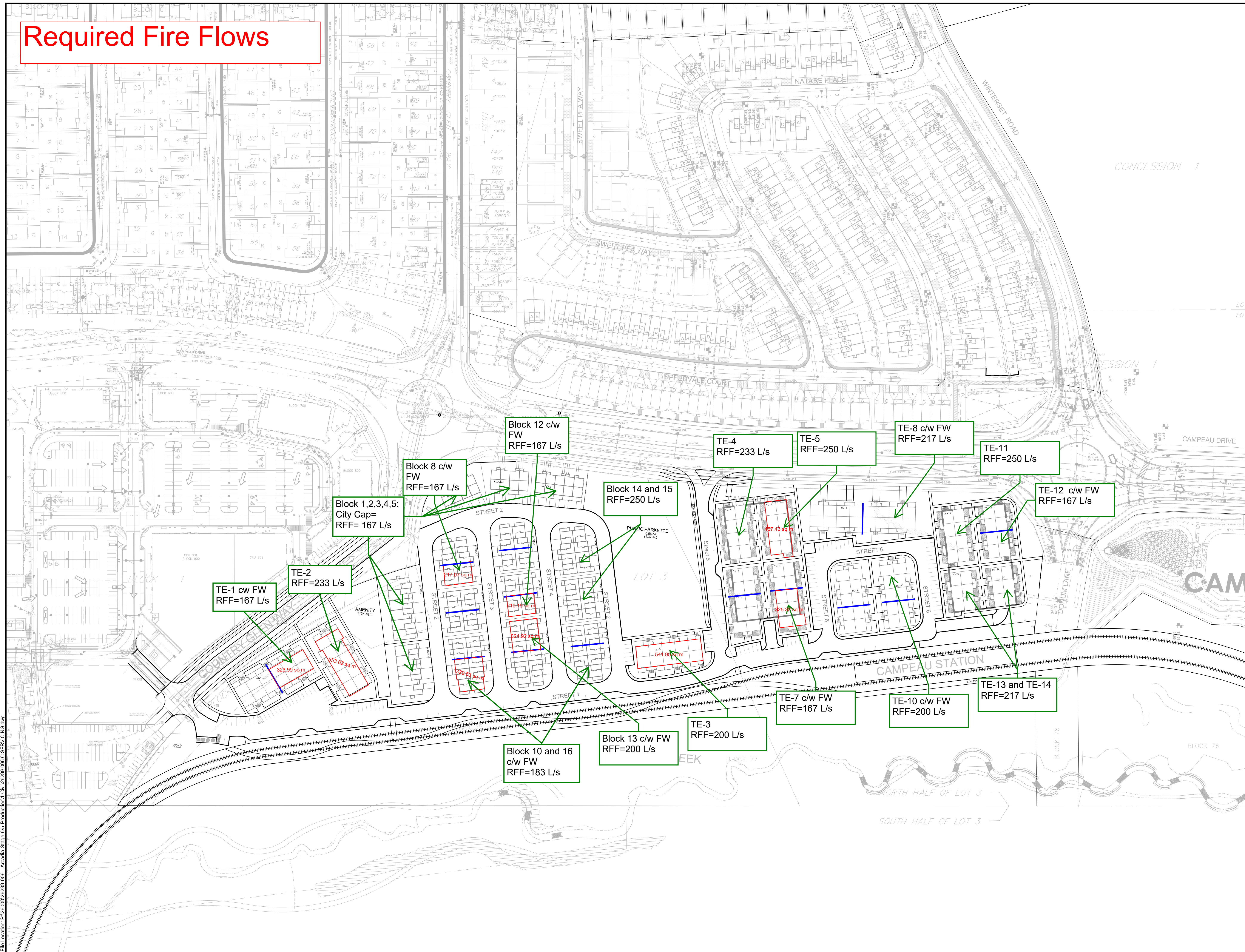
**PROJECT :** ARCADIA STAGE 6  
**LOCATION :** CITY OF OTTAWA  
**DEVELOPER :** MINTO COMMUNITIES INC.

NODE	RESIDENTIAL						NON-RESIDENTIAL			AVERAGE DAILY DEMAND (l/s)			MAXIMUM DAILY DEMAND (l/s)			PEAK HOUR DEMAND (l/s)		
	DUPLEX	EXECUTIVE TOWNHOUSE	REAR LANE TOWNHOUSE	AVENUE (B2B) TOWNHOUSE	UNIT COUNT	POP'N	COMM (ha.)	INST. (ha.)	PARK (ha.)	Res.	Non-res.	Total	Res.	Non-res.	Total	Res.	Non-res.	Total
<b>ARCADIA STAGE 6</b>																		
J-1	0	0	0	0	0	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
J-2	0	0	0	0	0	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
J-3	0	5	0	5	10	27	0.00	0.00	0.00	0.09	0.00	0.09	0.22	0.00	0.22	0.48	0.00	0.48
J-4	0	0	0	11	11	30	0.00	0.00	0.00	0.10	0.00	0.10	0.24	0.00	0.24	0.53	0.00	0.53
J-5	6	0	0	1	7	17	0.00	0.00	0.00	0.05	0.00	0.05	0.13	0.00	0.13	0.29	0.00	0.29
J-6	0	6	0	8	14	38	0.00	0.00	0.00	0.12	0.00	0.12	0.31	0.00	0.31	0.67	0.00	0.67
J-7	0	0	8	16	24	65	0.00	0.00	0.00	0.21	0.00	0.21	0.53	0.00	0.53	1.16	0.00	1.16
J-8	0	0	2	4	6	16	0.00	0.00	0.00	0.05	0.00	0.05	0.13	0.00	0.13	0.29	0.00	0.29
J-9	4	0	0	0	4	9	0.00	0.00	0.00	0.03	0.00	0.03	0.07	0.00	0.07	0.17	0.00	0.17
J-10	4	0	0	0	4	9	0.00	0.00	0.00	0.03	0.00	0.03	0.07	0.00	0.07	0.16	0.00	0.16
J-11	16	0	0	0	16	37	0.00	0.00	0.00	0.12	0.00	0.12	0.30	0.00	0.30	0.66	0.00	0.66
J-12	16	0	0	0	16	37	0.00	0.00	0.00	0.12	0.00	0.12	0.30	0.00	0.30	0.66	0.00	0.66
J-13	4	0	0	0	4	9	0.00	0.00	0.00	0.03	0.00	0.03	0.07	0.00	0.07	0.16	0.00	0.16
J-14	0	0	0	0	0	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
J-15	12	0	0	0	12	28	0.00	0.00	0.00	0.09	0.00	0.09	0.22	0.00	0.22	0.49	0.00	0.49
J-17	0	0	3	16	19	51	0.00	0.00	0.00	0.17	0.00	0.17	0.42	0.00	0.42	0.91	0.00	0.91
J-18	0	0	0	11	11	30	0.00	0.00	0.00	0.10	0.00	0.10	0.24	0.00	0.24	0.53	0.00	0.53
J-20	8	0	0	0	8	18	0.00	0.00	0.00	0.06	0.00	0.06	0.15	0.00	0.15	0.33	0.00	0.33
J-21	12	0	0	0	12	28	0.00	0.00	0.00	0.09	0.00	0.09	0.22	0.00	0.22	0.49	0.00	0.49
J-22	4	0	0	0	4	9	0.00	0.00	0.00	0.03	0.00	0.03	0.07	0.00	0.07	0.16	0.00	0.16
J-23	8	0	0	0	8	18	0.00	0.00	0.00	0.06	0.00	0.06	0.15	0.00	0.15	0.33	0.00	0.33
J-24	8	0	0	0	8	18	0.00	0.00	0.00	0.06	0.00	0.06	0.15	0.00	0.15	0.33	0.00	0.33
J-25	8	0	0	0	8	18	0.00	0.00	0.00	0.06	0.00	0.06	0.15	0.00	0.15	0.33	0.00	0.33
J-26	0	0	0	0	0	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
J-27	16	0	0	0	16	37	0.00	0.00	0.00	0.12	0.00	0.12	0.30	0.00	0.30	0.66	0.00	0.66
J-28	6	0	0	8	14	35	0.00	0.00	0.00	0.11	0.00	0.11	0.29	0.00	0.29	0.63	0.00	0.63
J-29	4	0	0	0	4	9	0.00	0.00	0.00	0.03	0.00	0.03	0.07	0.00	0.07	0.16	0.00	0.16
J-32	72	0	0	0	72	166	0.00	0.00	0.00	0.54	0.00	0.54	1.34	0.00	1.34	2.95	0.00	2.95
J-33	0	0	0	0	0	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
J-34	56	0	0	0	56	129	0.00	0.00	0.00	0.42	0.00	0.42	1.04	0.00	1.04	2.30	0.00	2.30
<b>TOTALS</b>	<b>264</b>	<b>11</b>	<b>13</b>	<b>80</b>	<b>368</b>	<b>888</b>	<b>0.00</b>	<b>0.00</b>	<b>0.00</b>	<b>2.88</b>	<b>0.00</b>	<b>2.88</b>	<b>7.20</b>	<b>0.00</b>	<b>7.20</b>	<b>15.83</b>	<b>0.00</b>	<b>15.83</b>

**ASSUMPTIONS**

<b>RESIDENTIAL DENSITIES</b>		<b>AVG. DAILY DEMAND</b>		<b>MAX. HOURLY DEMAND</b>	
- DUPLEX (INFUSION TERRACES)	2.3 p / p / u	- Residential	280 l / cap / day	- Residential	1,540 l / cap / day
- TOWNHOUSE UNITS (AVENUE, EXECUTIVE, ROW)	2.7 p / p / u	- Institutional	28,000 l / ha / day	- Institutional	75,600 l / ha / day
		- Commercial	28,000 l / ha / day	- Commercial	75,600 l / ha / day
		<b>MAX. DAILY DEMAND</b>			
		- Residential	700 l / cap / day		
		- Institutional	42,000 l / ha / day		
		- Commercial	42,000 l / ha / day		

# Required Fire Flows



01	ISSUED TO CITY FOR REVIEW FIRST ENGINEERING SUBMISSION	18/07/22
No.	ISSUE / REVISION	DDMMYY

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VERIFY SHEET SIZE AND SCALES. BAR TO THE RIGHT IS 25mm IF THIS IS A FULL SIZE DRAWING.

SCALE: 0 25mm

CLIENT:

CONSULTANT:

ENGINEERS - ARCHITECTS - PLANNERS

CONSULTANT:

PROFESSIONAL STAMP

PROJECT NORTH

PROJECT:

**ARCADIA STAGE 6**

450 HUNTMAR DRIVE

DRAWING:

DESIGN:	MM	DRAWING #:
DRAWN:	KC	
CHECKED:	KF	
JLR #:	26299-006	

File Location: P:\2020\026299-006 - Arcadia Stage 6\5-Production\1-Chal\26299-006 5-SERV\CHNG.dwg

PLOT DATE: July 14, 2022 4:05:10 PM

### FUS Fire Flow Calculations

ARCADIA STAGE 6 - Block TE - 5 - Duplex Townhouse  
(JLR 26299-06)

Step	Parameter	Value	Note
<b>A</b>	Type of Construction	Wood Frame	Block TE-5
	Coefficient (C)	1.5	
<b>B</b>	Ground Floor Area	470 m <sup>2</sup>	
<b>C</b>	Height in storeys	3 storeys	Basements are excluded.
	Total Floor Area	1410 m <sup>2</sup>	
<b>D</b>	Fire Flow Formula	F=220C√A	
	Fire Flow	12391 L/min	
	Rounded Fire Flow	12000 L/min	Flow rounded to nearest 1000 L/min.
<b>E</b>	Occupancy Class	Limited Combustible	Residential units have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-1800	
	Fire Flow	10200 L/min	No rounding applied.
<b>F</b>	Sprinkler Protection	None	No sprinkler.
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
<b>G</b>	<i>North Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-5
	Exposed Wall:	Wood Frame	No structures within 50m
	Length of Exposed Wall:	24.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	72.0 m-storeys	
	Separation Distance	50 m	
	North Side Exposure Charge	0%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-5
	Exposed Wall:	Wood Frame	TE-8
	Length of Exposed Wall:	14.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	42.0 m-storeys	
	Separation Distance	11 m	
	East Side Exposure Charge	13%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-5
	Exposed Wall:	Non-combustible	Firewall
	Length of Exposed Wall:	14.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	42.0 m-storeys	
	Separation Distance	3.5 m	
South Side Exposure Charge	18%		
<i>West Side Exposure</i>			
Exposing Wall:	Wood Frame	TE-5	
Exposed Wall:	Wood Frame	TE-4	
Length of Exposed Wall:	32.0 m		
Height of Exposed Wall:	3 storeys		
Length-Height Factor	96.0 m-storeys		
Separation Distance	11 m		
West Side Exposure Charge	15%		
Total Exposure Charge	46%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	4692 L/min		
<b>H</b>	Fire Flow	14892 L/min	
	Rounded Fire Flow	15000 L/min	Flow rounded to nearest 1000 L/min.
<b>City Cap</b>	<b>Required Fire Flow (RFF)</b>	<b>15000 L/min</b>	City Cap Does Not Apply
		<b>250 L/s</b>	

Fire Underwriters Survey (FUS) Fire Flow Calculations  
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

### FUS Fire Flow Calculations

**ARCADIA STAGE 6 - Block TE - 11 - Duplex Townhouse**  
(JLR 26299-06)

Step	Parameter	Value	Note
<b>A</b>	Type of Construction	Wood Frame	Block TE-11
	Coefficient (C)	1.5	
<b>B</b>	Ground Floor Area	470 m <sup>2</sup>	8 units (From total of 10 units)
<b>C</b>	Height in storeys	3 storeys	Basements are excluded.
	Total Floor Area	1410 m <sup>2</sup>	
<b>D</b>	Fire Flow Formula	F=220C√A	
	Fire Flow	12391 L/min	
	Rounded Fire Flow	12000 L/min	Flow rounded to nearest 1000 L/min.
<b>E</b>	Occupancy Class	Limited Combustible	Residential units have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-1800	
	Fire Flow	10200 L/min	No rounding applied.
<b>F</b>	Sprinkler Protection	None	No sprinkler.
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
<b>G</b>	<i>North Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-11
	Exposed Wall:	Wood Frame	No structure within 50m
	Length of Exposed Wall:	16.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	48.0 m-storeys	
	Separation Distance	50 m	
	North Side Exposure Charge	0%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-11
	Exposed Wall:	Wood Frame	TE-12
	Length of Exposed Wall:	32.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	96.0 m-storeys	
	Separation Distance	12 m	
	East Side Exposure Charge	15%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-11
	Exposed Wall:	Wood Frame	TE-13
	Length of Exposed Wall:	14.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	42.0 m-storeys	
	Separation Distance	3.5 m	
	South Side Exposure Charge	18%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Wood Frame	TE-11
	Exposed Wall:	Wood Frame	TE-8 and TE-10
	Length of Exposed Wall:	17.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	51.0 m-storeys	
	Separation Distance	15 m	
West Side Exposure Charge	13%		
Total Exposure Charge	46%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	4692 L/min		
<b>H</b>	Fire Flow	14892 L/min	
	Rounded Fire Flow	15000 L/min	Flow rounded to nearest 1000 L/min.
<b>City Cap</b>	<b>Required Fire Flow (RFF)</b>	<b>15000 L/min</b>	City Cap Does Not Apply
		<b>250 L/s</b>	

Fire Underwriters Survey (FUS) Fire Flow Calculations  
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

### FUS Fire Flow Calculations

**ARCADIA STAGE 6 - Block 14 - Back-to-Back Townhouse**  
(JLR 26299-06)

Step	Parameter	Value	Note
<b>A</b>	Type of Construction	Wood Frame	Block 14
	Coefficient (C)	1.5	
<b>B</b>	Ground Floor Area	443 m <sup>2</sup>	8 units
<b>C</b>	Height in storeys	3 storeys	Basements are excluded.
	Total Floor Area	1329 m <sup>2</sup>	
<b>D</b>	Fire Flow Formula	F=220C√A	
	Fire Flow	12030 L/min	
	Rounded Fire Flow	12000 L/min	Flow rounded to nearest 1000 L/min.
<b>E</b>	Occupancy Class	Limited Combustible	Residential units have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-1800	
	Fire Flow	10200 L/min	No rounding applied.
<b>F</b>	Sprinkler Protection	None	No sprinkler.
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
<b>G</b>	<i>North Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 14
	Exposed Wall:	Wood Frame	Block 5
	Length of Exposed Wall:	16.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	48.0 m-storeys	
	Separation Distance	19 m	
	North Side Exposure Charge	13%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 14
	Exposed Wall:	Wood Frame	No exposure within 50 m
	Length of Exposed Wall:	19.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	57.0 m-storeys	
	Separation Distance	50 m	
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 14
	Exposed Wall:	Wood Frame	Block 15
	Length of Exposed Wall:	18.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	54.0 m-storeys	
	Separation Distance	3.1 m	
	South Side Exposure Charge	18%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 14
	Exposed Wall:	Wood Frame	Block 11 and 12
	Length of Exposed Wall:	27.0 m	
	Height of Exposed Wall:	3 storeys	
Length-Height Factor	81.0 m-storeys		
Separation Distance	19 m		
West Side Exposure Charge	14%		
Total Exposure Charge	45%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	4590 L/min		
<b>H</b>	Fire Flow	14790 L/min	
	Rounded Fire Flow	15000 L/min	Flow rounded to nearest 1000 L/min.
<b>City Cap</b>	<b>Required Fire Flow (RFF)</b>	<b>15000 L/min</b>	City Cap Does Not Apply
		<b>250 L/s</b>	

Fire Underwriters Survey (FUS) Fire Flow Calculations  
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

## FUS Fire Flow Calculations

ARCADIA STAGE 6 - Block 15 - Back-to-Back Townhouse  
(JLR 26299-06)

Step	Parameter	Value	Note
A	Type of Construction	Wood Frame	Block 15
	Coefficient (C)	1.5	
B	Ground Floor Area	435 m <sup>2</sup>	8 units
C	Height in storeys	3 storeys	Basements are excluded.
	Total Floor Area	1305 m <sup>2</sup>	
D	Fire Flow Formula	F=220C√A	
	Fire Flow	11921 L/min	
	Rounded Fire Flow	12000 L/min	Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Limited Combustible	Residential units have a limited combustible occupancy.
	Occupancy Charge	-15%	
	Occupancy Increase or Decrease	-1800	
	Fire Flow	10200 L/min	No rounding applied.
F	Sprinkler Protection	None	No sprinkler.
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
G	<i>North Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 15
	Exposed Wall:	Wood Frame	Block 14
	Length of Exposed Wall:	17.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	51.0 m-storeys	
	Separation Distance	3.1 m	
	North Side Exposure Charge	18%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 15
	Exposed Wall:	Wood Frame	No exposure within 50 m
	Length of Exposed Wall:	19.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	57.0 m-storeys	
	Separation Distance	50 m	
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 15
	Exposed Wall:	Wood Frame	Block 16
	Length of Exposed Wall:	17.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	51.0 m-storeys	
	Separation Distance	3.1 m	
	South Side Exposure Charge	18%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Wood Frame	Block 15
	Exposed Wall:	Wood Frame	Block 12 and 13
	Length of Exposed Wall:	26.0 m	
	Height of Exposed Wall:	3 storeys	
	Length-Height Factor	78.0 m-storeys	
Separation Distance	19 m		
West Side Exposure Charge	14%		
Total Exposure Charge	50%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	5100 L/min		
H	Fire Flow	15300 L/min	
	Rounded Fire Flow	15000 L/min	Flow rounded to nearest 1000 L/min.
City Cap	Required Fire Flow (RFF)	15000 L/min	City Cap Does Not Apply
		250 L/s	

Fire Underwriters Survey (FUS) Fire Flow Calculations

In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

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## **Appendix B2**

City Correspondence –  
Boundary Conditions

## Boundary Conditions Arcadia Stage 6

### Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	179	2.98
Maximum Daily Demand	446	7.44
Peak Hour	982	16.36
Fire Flow Demand #1	10,000	166.67
Fire Flow Demand #2	15,000	250.00

### Location



### Results

#### Connection 1 – Donum Lane

Demand Scenario	Head (m)	Pressure <sup>1</sup> (psi)
Maximum HGL	161.3	95.4
Peak Hour	156.3	88.3
Max Day plus Fire 1	153.8	84.8
Max Day plus Fire 2	150.5	80.0

Ground Elevation = 94.2 m



## Connection 2 – Country Glen Way

Demand Scenario	Head (m)	Pressure <sup>1</sup> (psi)
Maximum HGL	161.3	88.8
Peak Hour	156.3	81.7
Max Day plus Fire 1	151.5	74.9
Max Day plus Fire 2	145.5	66.4

Ground Elevation = 98.8 m

### **Notes**

1. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
  - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
  - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

### **Disclaimer**

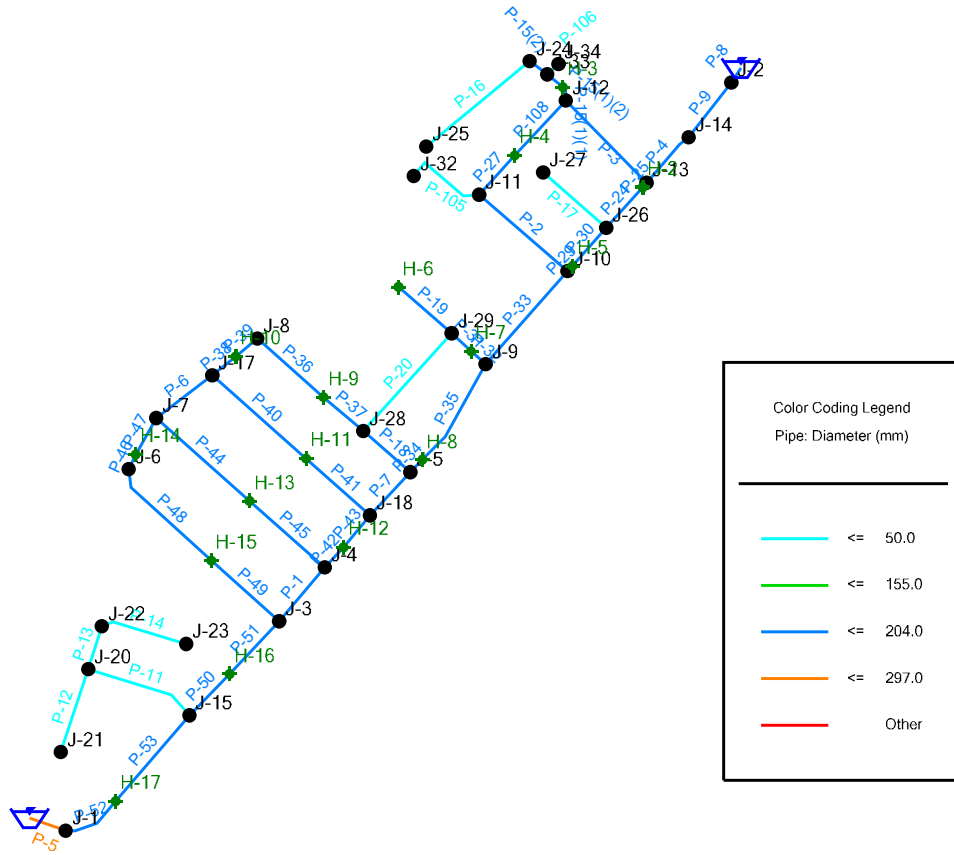
*The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.*

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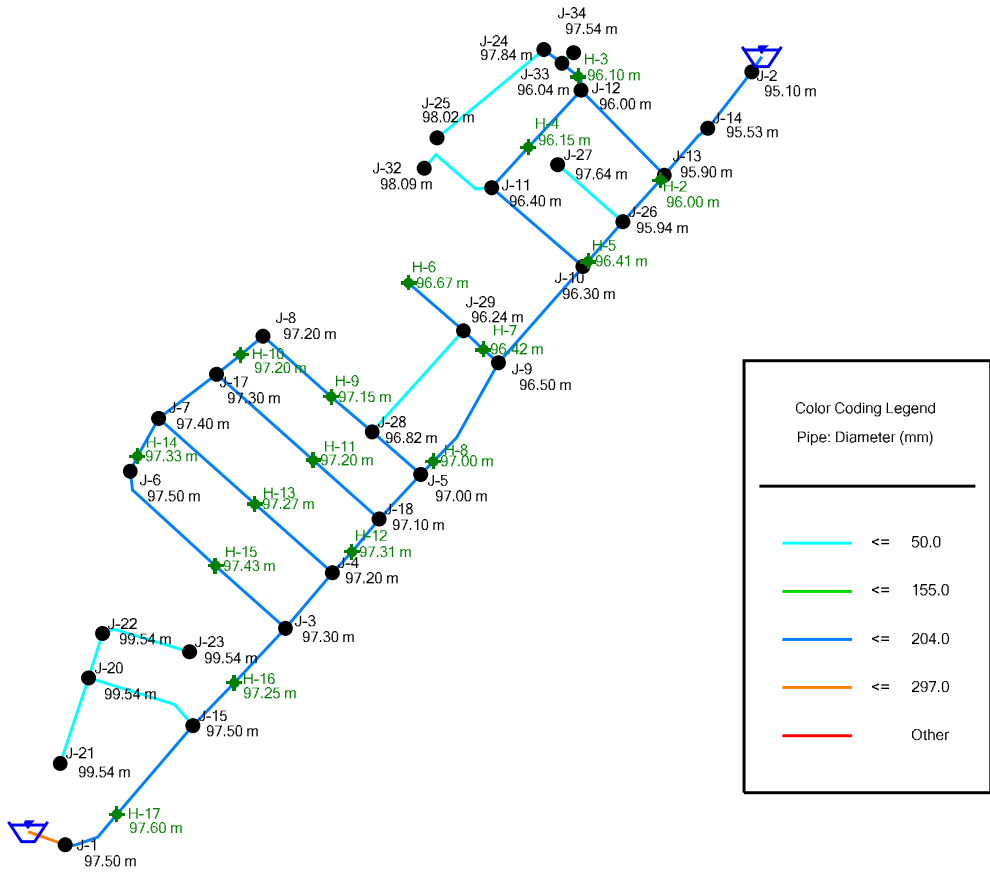
# **Appendix B3**

WaterCAD Schematics

# Arcadia Stage 6 Model Schematic



# Arcadia Stage 6 Model Schematic Elevation Model

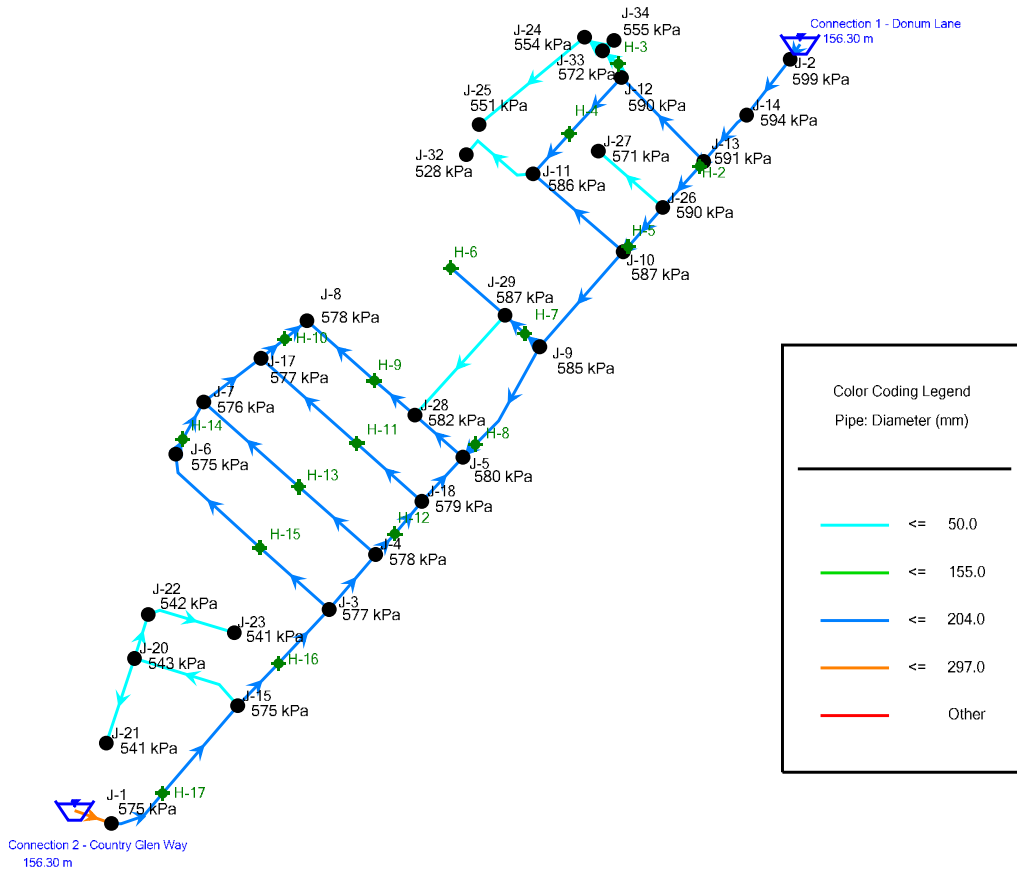


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## **Appendix B4**

Simulation Results – Peak Hour  
Demand

# Arcadia Stage 6 Peak Hour Demand Existing Condition



**Arcadia Stage 6  
Peak Hour Demand  
Existing Condition  
Junction Table**

Label	ID	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-32	436	98.09	2.95	152.07	528
J-23	345	99.54	0.33	154.85	541
J-21	341	99.54	0.49	154.85	541
J-22	343	99.54	0.16	154.92	542
J-20	337	99.54	0.33	155.00	543
J-25	349	98.02	0.33	154.32	551
J-24	347	97.84	0.33	154.44	554
J-34	442	97.54	2.30	154.21	555
J-27	354	97.64	0.66	155.98	571
J-33	439	96.04	0.00	154.50	572
J-6	53	97.50	0.67	156.24	575
J-15	95	97.50	0.49	156.26	575
J-1	33	97.50	0.00	156.30	575
J-7	54	97.40	1.16	156.24	576
J-17	167	97.30	0.91	156.24	577
J-3	44	97.30	0.48	156.25	577
J-8	55	97.20	0.29	156.24	578
J-4	47	97.20	0.53	156.24	578
J-18	171	97.10	0.53	156.24	579
J-5	50	97.00	0.29	156.24	580
J-28	360	96.82	0.63	156.24	582
J-9	62	96.50	0.17	156.24	585
J-11	70	96.40	0.66	156.24	586
J-10	67	96.30	0.16	156.24	587
J-29	363	96.24	0.16	156.24	587
J-12	72	96.00	0.66	156.24	590
J-26	351	95.94	0.00	156.24	590
J-13	74	95.90	0.16	156.25	591
J-14	78	95.53	0.00	156.27	594
J-2	34	95.10	0.00	156.29	599

**Arcadia Stage 6  
Peak Hour Demand  
Existing Condition  
Pipe Table**

Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Hydraulic Grade (Start) (m)	Hydraulic Grade (Stop) (m)	Flow (L/s)	Velocity (m/s)
P-106	8	50.0	PEX	130.0	154.50	154.21	2.30	1.17
P-16	70	50.0	PEX	100.0	154.44	154.32	0.33	0.17
P-15 (2)	12	50.0	PEX	100.0	154.50	154.44	0.66	0.34
P-15 (1)(2)	11	50.0	PEX	100.0	155.51	154.50	2.96	1.51
P-14	46	50.0	PEX	100.0	154.92	154.85	0.33	0.17
P-12	46	50.0	PEX	100.0	155.00	154.85	0.49	0.25
P-13	24	50.0	PEX	100.0	155.00	154.92	0.49	0.25
P-11	60	50.0	PEX	100.0	156.26	155.00	1.31	0.67
P-15 (1)(1)	8	50.0	PEX	100.0	156.24	155.51	2.96	1.51
P-17	44	50.0	PEX	100.0	156.24	155.98	0.66	0.34
P-2	61	204.0	PVC	110.0	156.24	156.24	2.61	0.08
P-105	44	50.0	PEX	100.0	152.07	156.24	-2.95	1.50
P-27	28	204.0	PVC	110.0	156.24	156.24	-1.00	0.03
P-108	39	204.0	PVC	110.0	156.24	156.24	-1.00	0.03
P-20	69	50.0	PEX	100.0	156.24	156.24	0.01	0.01
P-36	46	204.0	PVC	110.0	156.24	156.24	-0.02	0.00
P-37	27	204.0	PVC	110.0	156.24	156.24	-0.02	0.00
P-39	15	204.0	PVC	110.0	156.24	156.24	0.27	0.01
P-38	16	204.0	PVC	110.0	156.24	156.24	0.27	0.01
P-6	37	204.0	PVC	110.0	156.24	156.24	0.75	0.02
P-7	31	204.0	PVC	110.0	156.24	156.24	0.60	0.02
P-18	33	204.0	PVC	110.0	156.24	156.24	-0.63	0.02
P-34	9	204.0	PVC	110.0	156.24	156.24	-0.32	0.01
P-40	66	204.0	PVC	110.0	156.24	156.24	-0.43	0.01
P-19	37	204.0	PVC	110.0	156.24	156.24	0.00	0.00
P-31	14	204.0	PVC	110.0	156.24	156.24	-0.17	0.01
P-32	10	204.0	PVC	110.0	156.24	156.24	-0.17	0.01
P-33	65	204.0	PVC	110.0	156.24	156.24	0.66	0.02
P-35	61	204.0	PVC	110.0	156.24	156.24	-0.32	0.01
P-47	22	204.0	PVC	110.0	156.24	156.24	1.08	0.03
P-41	45	204.0	PVC	110.0	156.24	156.24	-0.43	0.01
P-43	22	204.0	PVC	110.0	156.24	156.24	1.57	0.05
P-46	9	204.0	PVC	110.0	156.24	156.24	1.08	0.03
P-44	65	204.0	PVC	110.0	156.24	156.24	-0.84	0.03
P-42	14	204.0	PVC	110.0	156.24	156.24	1.57	0.05
P-29	4	204.0	PVC	110.0	156.24	156.24	-3.44	0.11
P-1	37	204.0	PVC	110.0	156.25	156.24	2.93	0.09
P-45	53	204.0	PVC	110.0	156.24	156.24	-0.84	0.03
P-48	67	204.0	PVC	110.0	156.24	156.24	-1.75	0.05
P-30	27	204.0	PVC	110.0	156.24	156.24	-3.44	0.11
P-49	48	204.0	PVC	110.0	156.24	156.25	-1.75	0.05



**Arcadia Stage 6  
Peak Hour Demand  
Existing Condition  
Pipe Table**

Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Hydraulic Grade (Start) (m)	Hydraulic Grade (Stop) (m)	Flow (L/s)	Velocity (m/s)
P-51	38	204.0	PVC	110.0	156.25	156.25	5.16	0.16
P-24	29	204.0	PVC	110.0	156.24	156.25	-4.10	0.13
P-3	60	204.0	PVC	110.0	156.24	156.25	-4.62	0.14
P-25	3	204.0	PVC	110.0	156.25	156.25	-4.10	0.13
P-50	30	204.0	PVC	110.0	156.26	156.25	5.16	0.16
P-53	59	204.0	PVC	110.0	156.29	156.26	6.96	0.21
P-4	33	204.0	PVC	110.0	156.25	156.27	-8.87	0.27
P-52	32	204.0	PVC	110.0	156.30	156.29	6.96	0.21
P-9	36	204.0	PVC	110.0	156.27	156.29	-8.87	0.27
P-5	20	297.0	PVC	120.0	156.30	156.30	-6.96	0.10
P-8	9	204.0	PVC	110.0	156.29	156.30	-8.87	0.27

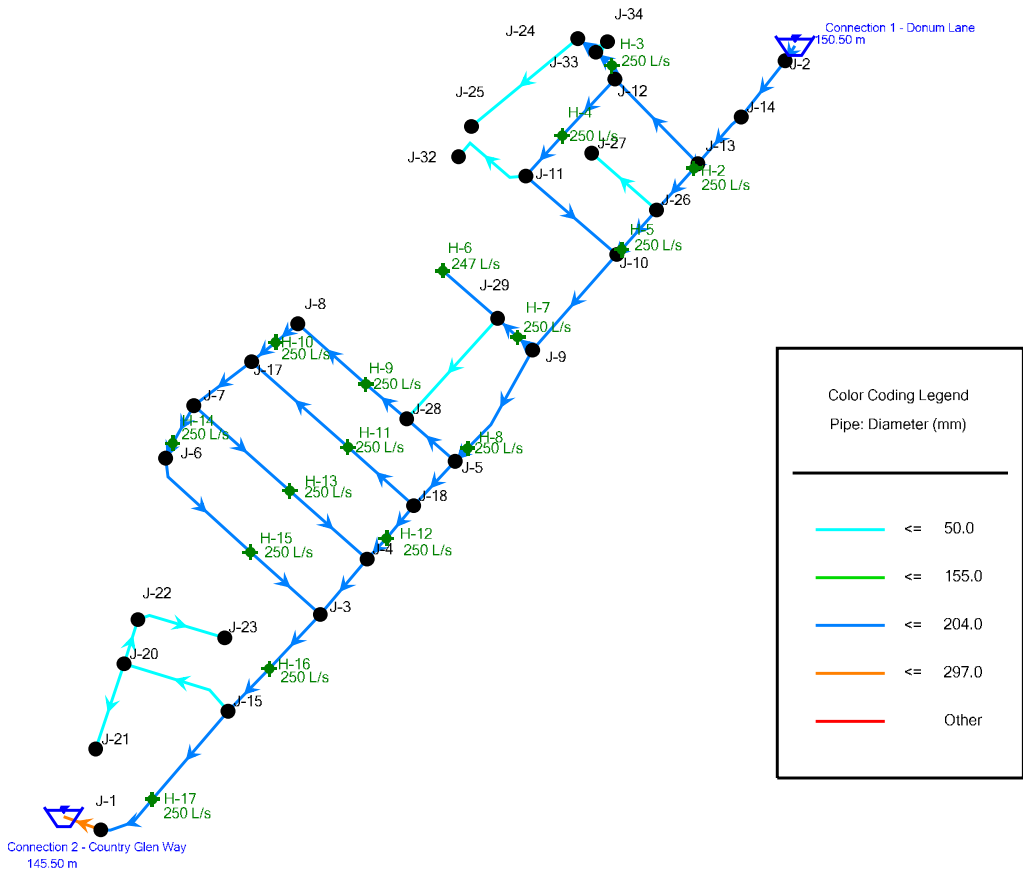
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## **Appendix B5**

Simulation Results – Maximum  
Day + Required Fire Flow

# Arcadia Stage 6

## Maximum Day Fire Flow (RFF=250L/s)



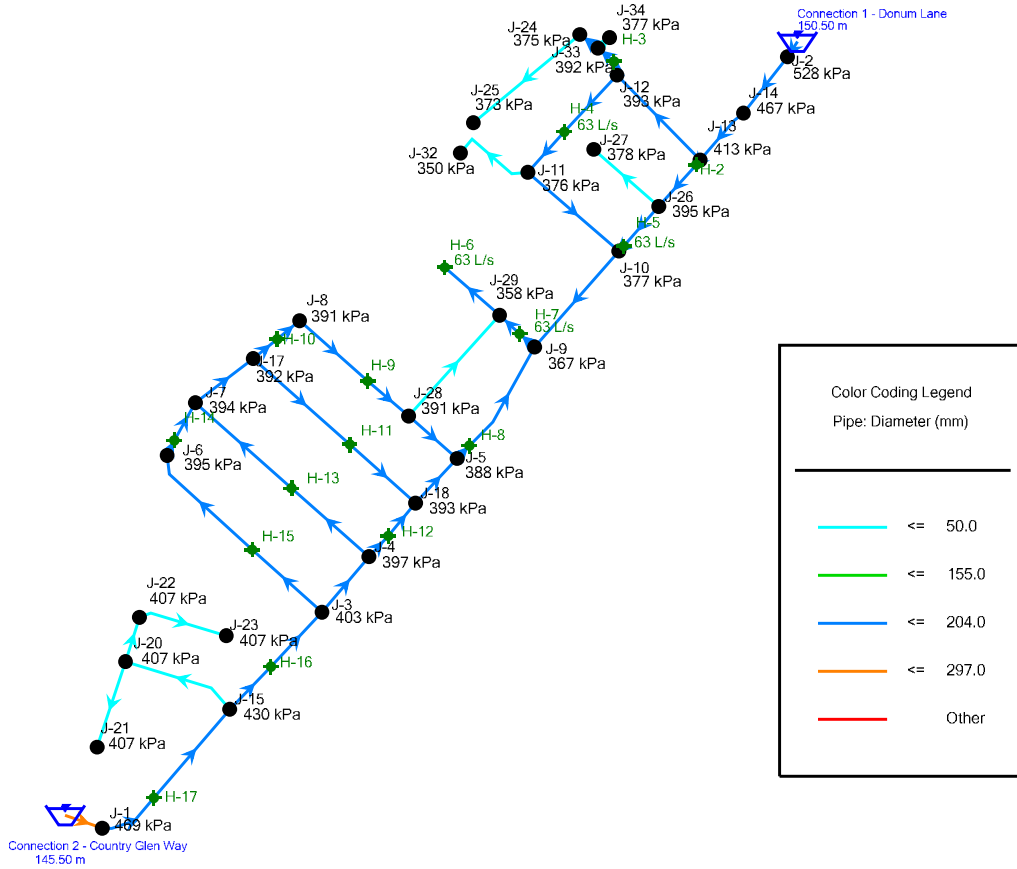
**Arcadia Stage 6**  
**Maximum Day Fire Flow (RFF=250L/s)**

Label	Fire Flow (Available) (L/s)	Flow (Total Available) (L/s)	Satisfies Fire Flow Constraints?	Pressure (Residual Lower Limit) (kPa)	Pressure (Calculated System Lower Limit) (kPa)	Pressure (Calculated Residual) (kPa)	Junction w/ Minimum Pressure (System)
H-6	247	247	True	140	256	140	J-29
H-2	250	250	True	140	363	385	J-32
H-4	250	250	True	140	302	305	J-32
H-5	250	250	True	140	348	360	J-32
H-7	250	250	True	140	287	290	H-6
H-8	250	250	True	140	311	303	J-5
H-9	250	250	True	140	280	250	J-8
H-10	250	250	True	140	268	260	J-8
H-11	250	250	True	140	298	256	J-17
H-12	250	250	True	140	304	290	J-4
H-13	250	250	True	140	297	252	H-14
H-14	250	250	True	140	260	252	J-6
H-15	250	250	True	140	289	253	J-6
H-16	250	250	True	140	346	322	J-3
H-17	250	250	True	140	420	406	J-23
H-3	250	250	True	140	280	292	J-34

# Arcadia Stage 6

## Maximum Day Fire Flow (RFF=250L/s)

### TE-05 Hydraulics Test



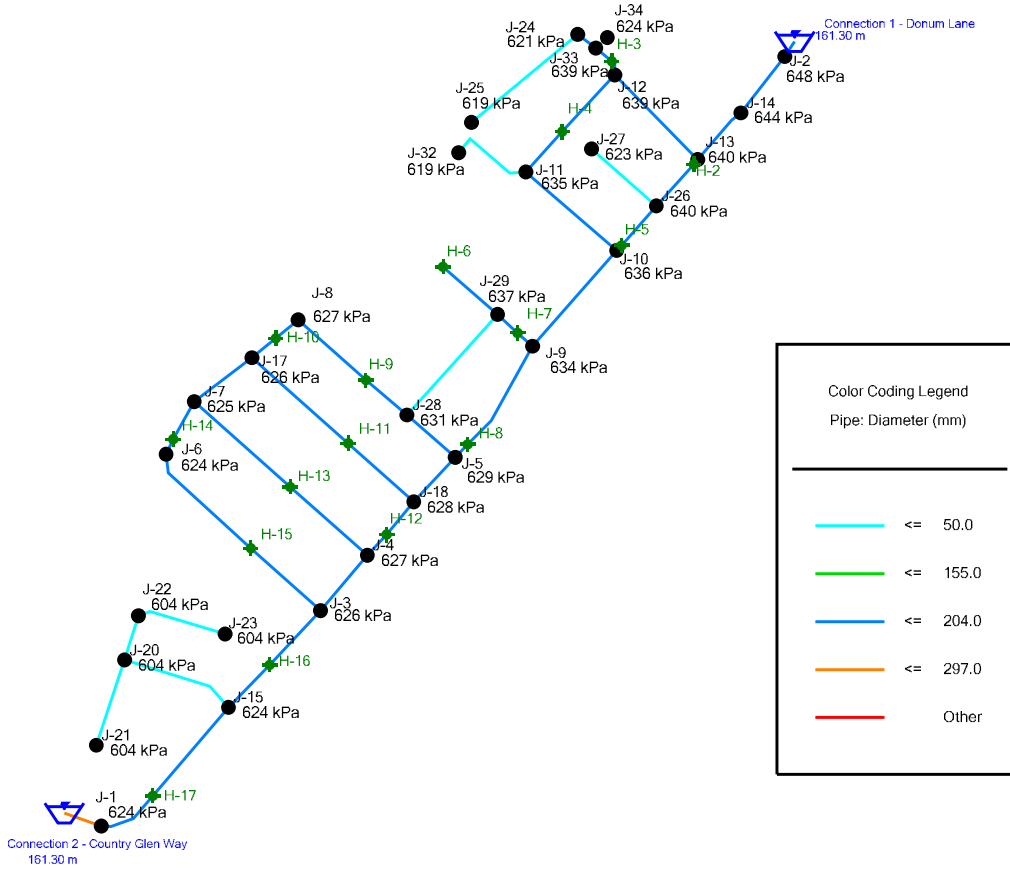
Color Coding Legend	
Pipe: Diameter (mm)	
<span style="color: cyan;">—</span>	<= 50.0
<span style="color: green;">—</span>	<= 155.0
<span style="color: blue;">—</span>	<= 204.0
<span style="color: orange;">—</span>	<= 297.0
<span style="color: red;">—</span>	Other

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## **Appendix B6**

Simulation Results – Maximum  
HGL

# Arcadia Stage 6 Maximum Day Analysis Existing Condition



Color Coding Legend	
Pipe: Diameter (mm)	
<span style="color: cyan;">—</span>	<= 50.0
<span style="color: green;">—</span>	<= 155.0
<span style="color: blue;">—</span>	<= 204.0
<span style="color: orange;">—</span>	<= 297.0
<span style="color: red;">—</span>	Other

**Arcadia Stage 6**  
**Maximum Day Analysis**  
**Existing Condition**  
**Junction Table**

Label	ID	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-20	337	99.54	0.00	161.30	604
J-21	341	99.54	0.00	161.30	604
J-22	343	99.54	0.00	161.30	604
J-23	345	99.54	0.00	161.30	604
J-32	436	98.09	0.00	161.30	619
J-25	349	98.02	0.00	161.30	619
J-24	347	97.84	0.00	161.30	621
J-27	354	97.64	0.00	161.30	623
J-34	442	97.54	0.00	161.30	624
J-1	33	97.50	0.00	161.30	624
J-6	53	97.50	0.00	161.30	624
J-15	95	97.50	0.00	161.30	624
J-7	54	97.40	0.00	161.30	625
J-3	44	97.30	0.00	161.30	626
J-17	167	97.30	0.00	161.30	626
J-4	47	97.20	0.00	161.30	627
J-8	55	97.20	0.00	161.30	627
J-18	171	97.10	0.00	161.30	628
J-5	50	97.00	0.00	161.30	629
J-28	360	96.82	0.00	161.30	631
J-9	62	96.50	0.00	161.30	634
J-11	70	96.40	0.00	161.30	635
J-10	67	96.30	0.00	161.30	636
J-29	363	96.24	0.00	161.30	637
J-33	439	96.04	0.00	161.30	639
J-12	72	96.00	0.00	161.30	639
J-26	351	95.94	0.00	161.30	640
J-13	74	95.90	0.00	161.30	640
J-14	78	95.53	0.00	161.30	644
J-2	34	95.10	0.00	161.30	648



**Arcadia Stage 6**  
**Maximum Day Analysis**  
**Existing Condition**  
**Pipe Table**

Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Hydraulic Grade (Start) (m)	Hydraulic Grade (Stop) (m)	Flow (L/s)	Velocity (m/s)
P-1	37	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-2	61	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-3	60	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-4	33	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-5	20	297.0	PVC	120.0	161.30	161.30	0.00	0.00
P-6	37	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-7	31	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-8	9	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-9	36	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-11	60	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-12	46	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-13	24	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-14	46	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-16	70	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-17	44	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-18	33	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-19	37	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-20	69	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-24	29	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-25	3	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-27	28	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-29	4	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-30	27	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-31	14	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-32	10	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-33	65	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-34	9	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-35	61	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-36	46	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-37	27	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-38	16	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-39	15	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-40	66	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-41	45	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-42	14	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-43	22	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-44	65	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-45	53	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-46	9	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-47	22	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-48	67	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-49	48	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-50	30	204.0	PVC	110.0	161.30	161.30	0.00	0.00

**Arcadia Stage 6**  
**Maximum Day Analysis**  
**Existing Condition**  
**Pipe Table**

Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Hydraulic Grade (Start) (m)	Hydraulic Grade (Stop) (m)	Flow (L/s)	Velocity (m/s)
P-51	38	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-52	32	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-53	59	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-105	44	50.0	PEX	100.0	161.30	161.30	0.00	0.00
P-15 (2)	12	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-106	8	50.0	PEX	130.0	161.30	161.30	0.00	0.00
P-15 (1)(1)	8	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-15 (1)(2)	11	204.0	PVC	110.0	161.30	161.30	0.00	0.00
P-108	39	204.0	PVC	110.0	161.30	161.30	0.00	0.00

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## **Appendix C1**

Sanitary Sewer Design Sheet



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## **Appendix C2**

Background Sanitary  
Documents



**ARCADIA STAGES 3 & 4  
CITY OF OTTAWA  
MINTO COMMUNITIES INC.  
JLR NO. 26299-03**

Single Family	3.4	pers/unit
Semi-Detached/Townhouse (row)	2.7	pers/unit
*Future Stage 5 Unit Density	44.0	units/ha
Manning's Coeff. N =	0.013	

q =	280	L/cap/day
l =	0.330	L/s/ha
Inst. =	28000	L/ha/day

Denotes Existing Sanitary Sewers from Arcadia Stage 2 Development as per JLR's design sheet dated July 2015  
 Sanitary Drainage Areas/population for future Arcadia Stages 4 and 5  
 Sanitary Drainage Areas/population for Arcadia Stage 3

STREET	M.H. #		RESIDENTIAL								COMMERCIAL			PARK/ROAD		INFILTRATION			PEAK DESIGN FLOW L/s	
			NUMBER OF UNITS		AREA ha	POPUL. peop.	CUMULATIVE		PEAKING FACTOR	POPUL. FLOW l/s	AREA ha	CUMM. AREA ha	PEAKING FACTOR	INST. FLOW l/s	AREA ha	CUMM. AREA ha	AREA ha	CUMM. AREA ha		PEAK EXTR. FLOW l/s
			SING.	MULT.			POPUL. peop.	AREA ha												
Eramosa Crescent (Stage 3B)	90	91	14.0		0.65	48	48	0.65	3.65	0.57		0.0	1.5	0.0		0.00	0.65	0.65	0.21	0.78
Eramosa Crescent (Stage 3B)	91	92	2.0		0.18	7	55	0.83	3.64	0.65		0.0	1.5	0.0		0.00	0.18	0.83	0.27	0.92
Eramosa Crescent (Stage 3B)	92	93	4.0		0.28	14	69	1.11	3.63	0.81		0.0	1.5	0.0		0.00	0.28	1.11	0.37	1.18
Eramosa Crescent (Stage 3B)	93	94	2.0		0.17	7	76	1.28	3.62	0.89		0.0	1.5	0.0		0.00	0.17	1.28	0.42	1.31
Eramosa Crescent (Stage 3B)	94	81	14.0		0.65	48	124	1.93	3.57	1.44		0.0	1.5	0.0		0.00	0.65	1.93	0.64	2.07
Paine Avenue (Stage 3B)	EX.9	81	6.0		0.28	20	20	0.28	3.70	0.24		0.0	1.5	0.0		0.00	0.28	0.28	0.09	0.33
Paine Avenue (Stage 3B)	81	80	12.0		0.52	41	185	2.73	3.53	2.12		0.0	1.5	0.0		0.00	0.52	2.73	0.90	3.02
Coco (Stage 3B)	27 E	23	5.0		0.30	17	17	0.30	3.71	0.20		0.0	1.5	0.0		0.00	0.30	0.30	0.10	0.30
Winterset Road (Stage 3B)	24	23	2.0		0.15	7	7	0.15	3.74	0.08		0.0	1.5	0.0		0.00	0.15	0.15	0.05	0.13
Winterset Road (Stage 3B)	23	22	17.0		0.83	58	82	1.28	3.61	0.96		0.0	1.5	0.0		0.00	0.83	1.28	0.42	1.38
Coco (Stage 3B)	27	28	2.0		0.18	7	7	0.18	3.74	0.08		0.0	1.5	0.0		0.00	0.18	0.18	0.06	0.14
Coco (Stage 3B)	28	80	15.0		0.69	51	58	0.87	3.64	0.68		0.0	1.5	0.0		0.00	0.69	0.87	0.29	0.97
Paine Avenue (Stage 3B)	80	22	5.0		0.33	17	260	3.93	3.48	2.94		0.0	1.5	0.0		0.00	0.33	3.93	1.30	4.23
Winterset Road (Stage 3B)	22	22B	1.0		0.16	3	345	5.37	3.44	3.85		0.0	1.5	0.0		0.00	0.16	5.37	1.77	5.62
Winterset Road (Stage 3B)	22B	21	5.0		0.32	17	362	5.69	3.43	4.03		0.0	1.5	0.0		0.00	0.32	5.69	1.88	5.91
Parabolica	72	71	2.0		0.17	7	7	0.17	3.74	0.08		0.0	1.5	0.0		0.00	0.17	0.17	0.06	0.14
Parabolica	71	21	14.0		0.67	48	55	0.84	3.64	0.65		0.0	1.5	0.0		0.00	0.67	0.84	0.28	0.93
Riverchase Drive (Stage 4)	21	20	11		0.56	37	454	7.09	3.40	5.00		0.0	1.5	0.0		0.00	0.56	7.09	2.34	7.34
Parabolica	73 S	61	5		0.30	17	17	0.30	3.71	0.20		0.0	1.5	0.0		0.00	0.30	0.30	0.10	0.30
Parabolica	72 S	73	7		0.33	24	24	0.33	3.70	0.29		0.0	1.5	0.0		0.00	0.33	0.33	0.11	0.40
Basalt	73	20	6	9	0.60	45	69	0.93	3.63	0.81		0.0	1.5	0.0		0.00	0.60	0.93	0.31	1.12
Winterset Road (Stage 4)	20	19		4	0.17	11	534	8.19	3.37	5.83		0.0	1.5	0.0		0.00	0.17	8.19	2.70	8.53
Winterset Road (Stage 4)	19	18		3	0.15	8	542	8.34	3.36	5.91		0.0	1.5	0.0		0.00	0.15	8.34	2.75	8.66
Calvington Avenue (Stage 3B)	62	61	6		0.31	20	20	0.31	3.70	0.24		0.0	1.5	0.0		0.00	0.31	0.31	0.10	0.34
Calvington Avenue (Stage 3B)	61	60	5	7	0.44	36	73	1.05	3.62	0.86		0.0	1.5	0.0		0.00	0.44	1.05	0.35	1.20
Calvington Avenue (Stage 3B)	60	18	2	2	0.22	12	85	1.27	3.61	0.99		0.0	1.5	0.0		0.00	0.22	1.27	0.42	1.41
Park (Stage 3B)	Stub	18				0	0	0.00				0.0	1.5	0.0	2.46	2.46	2.46	2.46	0.81	0.81
Winterset Road (Stage 3B)	18	16	5		0.30	17	644	9.91	3.33	6.95		0.0	1.5	0.0		2.46	0.30	12.37	4.08	11.04
Stage 5*	Stub	16		180	4.10	486	486	4.10	3.38	5.33		0.0	1.5	0.0		0.00	4.10	4.10	1.35	6.68



SANITARY SEWER DESIGN SHEET

ARCADIA STAGES 3 & 4
CITY OF OTTAWA
MINTO COMMUNITIES INC.
JLR NO. 26299-03

Designed by: NG

Checked by: TC

Date : April 2019

Table with density and Manning's coefficient values: Single Family 3.4 pers/unit, Semi-Detached/Townhouse (row) 2.7 pers/unit, \*Future Stage 5 Unit Density 44.0 units/ha, Manning's Coeff. N = 0.013

Denotes Existing Sanitary Sewers from Arcadia Stage 2 Development as per JLR's design sheet dated July 2015
Sanitary Drainage Areas/population for future Arcadia Stages 4 and 5
Sanitary Drainage Areas/population for Arcadia Stage 3

Main sewer design table with columns for STREET, M.H. #, Actual DIA, SEWER DATA (DIA, SLOPE, CAPAC., VEL., LENGTH), RESIDUAL CAP., UPSTREAM (Center Line, Obvert, Invert, Cover), DOWNSTREAM (Center Line, Obvert, Invert, Cover), and SELF-CLEANSING VELOCITIES (Angle, Depth, Area, Wetted Perimeter, Flow, Velocity, Q/Qmax).



**ARCADIA STAGES 3 & 4  
CITY OF OTTAWA  
MINTO COMMUNITIES INC.  
JLR NO. 26299-03**

Single Family	3.4	pers/unit
Semi-Detached/Townhouse (row)	2.7	pers/unit
*Future Stage 5 Unit Density	44.0	units/ha
Manning's Coeff. N =	0.013	

q =	280	L/cap/day
l =	0.330	L/s/ha
Inst. =	28000	L/ha/day

Denotes Existing Sanitary Sewers from Arcadia Stage 2 Development as per JLR's design sheet dated July 2015  
Sanitary Drainage Areas/population for future Arcadia Stages 4 and 5  
Sanitary Drainage Areas/population for Arcadia Stage 3

STREET	M.H. #		RESIDENTIAL								COMMERCIAL			PARK/ROAD		INFILTRATION			PEAK DESIGN FLOW L/s	
			NUMBER OF UNITS		AREA ha	POPUL. peop.	CUMULATIVE		PEAKING FACTOR	POPUL. FLOW l/s	AREA ha	CUMM. AREA ha	PEAKING FACTOR	INST. FLOW l/s	AREA ha	CUMM. AREA ha	AREA ha	CUMM. AREA ha		PEAK EXTR. FLOW l/s
			SING.	MULT.			POPUL. peop.	AREA ha												
Winterset Road	16	14	14		0.60	38	1168	14.61	3.20	12.13		0.0	1.5	0.0		2.46	0.60	17.07	5.63	17.76
Winterset Road	14	13	8		0.29	22	1189	14.90	3.20	12.33		0.0	1.5	0.0		2.46	0.29	17.36	5.73	18.06
Winterset Road	13	2	4		0.18	11	1200	15.08	3.20	12.44		0.0	1.5	0.0		2.46	0.18	17.54	5.79	18.23
Natare	45 E	46	17		0.48	46	46	0.48	3.66	0.54		0.0	1.5	0.0		0.00	0.48	0.48	0.16	0.70
Natare	47	46	3		0.13	8	8	0.13	3.74	0.10		0.0	1.5	0.0		0.00	0.13	0.13	0.04	0.14
Speedvale	46	39				0	54	0.61	3.65	0.64		0.0	1.5	0.0	0.03	0.03	0.03	0.64	0.21	0.85
Speedvale	39	38	16		0.48	43	97	1.09	3.60	1.13		0.0	1.5	0.0		0.03	0.48	1.12	0.37	1.50
Speedvale	38	37	12		0.31	32	130	1.40	3.57	1.50		0.0	1.5	0.0		0.03	0.31	1.43	0.47	1.97
Speedvale	37	36	9		0.24	24	154	1.64	3.55	1.77		0.0	1.5	0.0		0.03	0.24	1.67	0.55	2.32
Speedvale	36	30	7		0.24	19	173	1.88	3.54	1.98		0.0	1.5	0.0		0.03	0.24	1.91	0.63	2.61
Natare	45	44	4		0.19	11	11	0.19	3.73	0.13		0.0	1.5	0.0		0.00	0.19	0.19	0.06	0.19
Natare	44	43	6		0.18	16	27	0.37	3.69	0.32		0.0	1.5	0.0		0.00	0.18	0.37	0.12	0.44
Natare	43	42	10		0.31	27	54	0.68	3.65	0.64		0.0	1.5	0.0		0.00	0.31	0.68	0.22	0.86
Sweet Pea	54	53	17		0.80	58	58	0.80	3.64	0.68		0.0	1.5	0.0		0.00	0.80	0.80	0.26	0.95
Sweet Pea	53	52	2		0.15	7	65	0.95	3.63	0.77		0.0	1.5	0.0		0.00	0.15	0.95	0.31	1.08
Sweet Pea	52	51	10		0.44	34	99	1.39	3.60	1.15		0.0	1.5	0.0		0.00	0.44	1.39	0.46	1.61
Sweet Pea	51	50	1		0.10	3	102	1.49	3.59	1.19		0.0	1.5	0.0		0.00	0.10	1.49	0.49	1.68
Sweet Pea	50	42			0.01	0	102	1.50	3.59	1.19		0.0	1.5	0.0	0.01	0.01	0.02	1.51	0.50	1.69
Natare	42	40	16		0.48	43	199	2.66	3.52	2.27		0.0	1.5	0.0		0.01	0.48	2.67	0.88	3.15
Natare	40	31	5		0.18	14	213	2.84	3.51	2.42		0.0	1.5	0.0		0.01	0.18	2.85	0.94	3.36
Speedvale	34	33	6		0.27	16	16	0.27	3.71	0.19		0.0	1.5	0.0		0.00	0.27	0.27	0.09	0.28
Speedvale	33	32	29		0.68	78	95	0.95	3.60	1.10		0.0	1.5	0.0		0.00	0.68	0.95	0.31	1.42
Speedvale	32	31	12		0.30	32	127	1.25	3.57	1.47		0.0	1.5	0.0		0.00	0.30	1.25	0.41	1.88
Speedvale	31	30	14		0.32	38	377	4.41	3.43	4.19		0.0	1.5	0.0		0.01	0.32	4.42	1.46	5.65
Speedvale	30	30A				0	550	6.29	3.36	5.99		0.0	1.5	0.0		0.04		6.33	2.09	8.08
Speedvale	30A	2			0.20	0	550	6.49	3.36	5.99		0.0	1.5	0.0	0.20	0.24	0.40	6.73	2.22	8.22
Stage 5*	Stub	2	180		4.10	486	486	4.10	3.38	5.33		0.0	1.5	0.0		0.00	4.10	4.10	1.35	6.68
Winterset Road	2	2A			0.18	0	2236	25.85	3.04	22.02		0.0	1.5	0.0	0.18	2.88	0.36	28.73	9.48	31.50
Winterset Road	2A	ex. 307A				0	2236	25.85	3.04	22.02		0.0	1.5	0.0		2.88	0.00	28.73	9.48	31.50
Campeau Drive	ex.306A	ex. 307A			23.00	1700	1700	23.00	3.11	17.14	95.58	95.6	1.5	46.5	5.10	5.10	123.68	123.68	40.81	104.42
Donum Lane	South Stub	1A				0	0.00				24.28	24.3	1.5	11.8	0.37	0.37	24.65	24.65	8.13	19.94
Donum Lane	1A	ex. 307A				0	0.00					24.3	1.5	11.8	0.37	0.37	25.02	25.02	8.26	20.06
Campeau Drive	ex. 307A	ex. 308A					3936	48.85	2.87	36.63		119.9	1.5	58.3		8.35	0.00	177.43	58.55	153.45

**SANITARY FLOW ALLOCATIONS**  
24.65 Ha, 20.06 L/s  
Stage 6 Lands 2.15ha, 1.75 L/s







SANITARY SEWER DESIGN SHEET  
 PROJECT : Kanata West Servicibility Stury  
 LOCATION : CITY OF OTTAWA

PAGE 1 OF 1  
 PROJECT: 3598-LD-03  
 DATE: Apr 2005  
 DESIGN: JIM  
 FILE: 3598LD.sewers.XLS

PHASE 1 SIGNATURE RIDGE (population based criteria..ICI simultaneous peaking)

STREET	LOCATION		TOTAL AREA (Ha)	RESIDENTIAL						EMPLOYMENT/RETAIL/BUSINESS PARK/OPEN SPACES						INFILTRATION			TOTAL FLOW (l/s)	PROPOSED SEWER																			
	FROM MH	TO MH		APPLIC AREA (Ha)	UNIT/Ha	TOTAL UNITS	POPULATION INDIV	ACCUM	PEAK FACTOR	PEAK FLOW (l/s)	APPLIC AREA (Ha)	ACCUM AREA (Ha)	TOTAL AREA (Ha)	FLOW RATE (l/Ha/d)	PEAK FLOW			AREA (Ha)		PEAK FLOW (l/s)	CAPACITY l/s	VELOCITY (ful) m/s	LGTH. (m)	PIPE (mm)	GRADE %	AVAIL. CAP. (%)													
															INDIV	ACCUM	TOTAL	INDIV									CUMUL	TOTAL CUMUL											
Campeau Drive Trunk Sewer	1	2																																					
	2	3																																					
	14	3																																					
	3	4																																					
	4A	4																																					
	4	5																																					
Corel Centre Etc. (Existing Sewer)		15																																					
First Line Road Sewer		15																																					
<b>Totals South Of Queensway To SRPS</b>	<b>15</b>	<b>5A</b>			<b>0</b>	<b>0</b>		<b>0</b>			<b>102.31</b>		<b>0.00</b>		<b>0.00</b>		<b>0.00</b>		<b>0.00</b>								<b>100.21</b>	<b>0.88</b>	<b>694.0</b>	<b>375</b>	<b>0.30</b>		<b>32.59%</b>						
	Queensway	5																																					
	5	5A																																					
Heritage Hills		5A																																					
Heritage Hills		5A																																					
Broughton-Richardson / Interstitial		5A																																					
<b>Total To SRPS</b>	<b>5A</b>	<b>SRPS</b>		<b>306.14</b>	<b>154.03</b>	<b>3136</b>	<b>9409</b>	<b>127.33</b>	<b>152.12</b>					<b>85.97</b>			<b>306.14</b>	<b>115.72</b>	<b>394.02</b>	<b>625.68</b>	<b>1.37</b>	<b>30.0</b>	<b>750</b>	<b>0.29</b>	<b>37.03%</b>														

Average Daily Per capita Flow Rate = 350 l/cap/d  
 Infiltration Allowance Flow Rate = 0.28 l/sec/Ha  
 Residential Peaking Factor =  $1 + (14 + (P^{0.5}))$ , P=Pop. in 1000's, Max of 4  
 Population density per unit = 3.00  
 P. F. For Employment/Retail/Business Park = 1.50

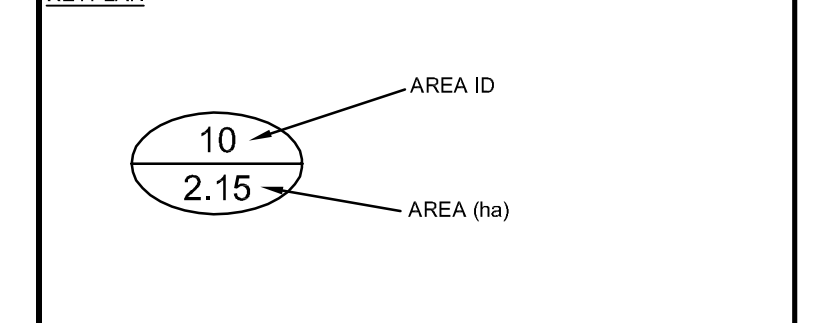
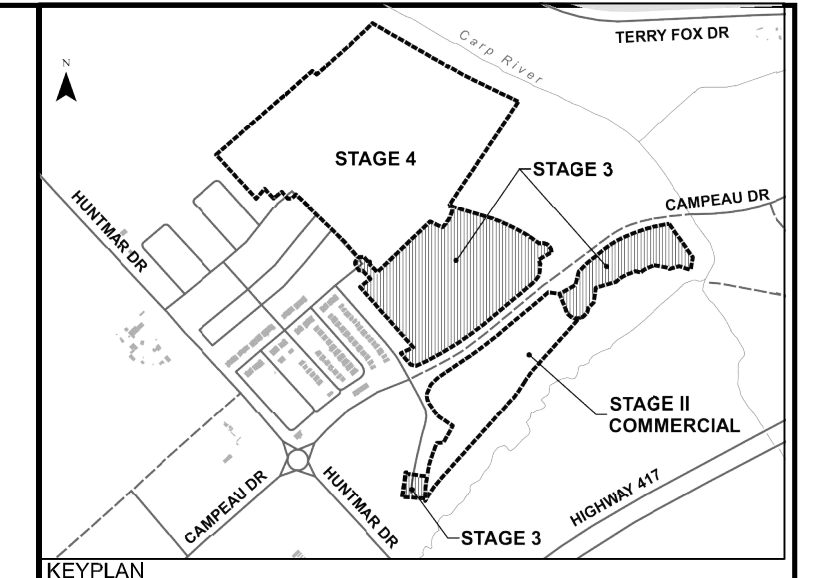
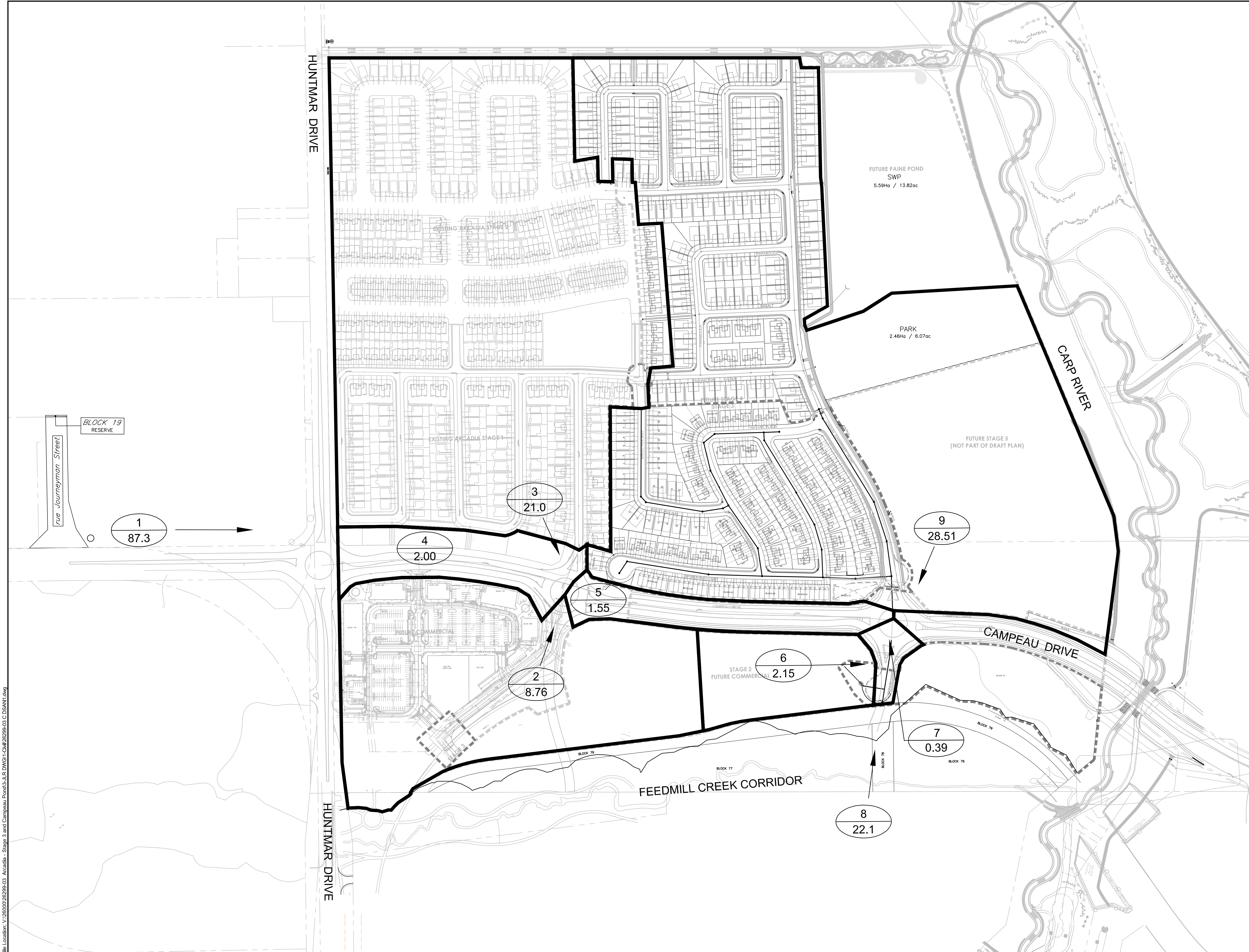
Note: Sewer from node 5 to SRPS is existing and is to be replaced.

Revision No. 1: April 11, 2005  
 Revision No. 2: April 20, 2005  
 Revision No. 3: June 07, 2005  
 Revision No. 4: Oct. 14, 2005  
 Revision No. 5: Feb. 15, 2006



FIG. 4.2-2

File Location: V:\262000\26299-03 Arcadia - Stage 3 and Campeau Pond\3-JLR.DWG\1-civil\26299-03 C DSANT.dwg



- AREA DESCRIPTIONS**
- 87.3 ha COMMERCIAL / EMPLOYMENT AS PER SIGNATURE RIDGE PUMPING STATION HYDRAULIC GRADE ANALYSIS - IBI GROUP, 2014 (REV 2)
  - 5.2 COMMERCIAL & 3.56 ha INFILTRATION AS PER ARCADIA RETAIL DEVELOPMENT DESIGN (IBI GROUP, 2014)
  - 21 ha RESIDENTIAL DEVELOPMENT AS PER ARCADIA STAGE 1 DETAILED DESIGN (IBI GROUP 2012) AND ARCADIA STAGE 2 DETAILED DESIGN (JLR 2014)
  - 2 ha RESIDENTIAL & R.O.W. AS PER ARCADIA STAGE 1 DETAILED DESIGN (IBI GROUP 2012)
  - 1.54 ha R.O.W. AS PER CURRENT DESIGN (JLR)
  - 2.15 ha FUTURE COMMERCIAL AS PER CURRENT DESIGN (JLR)
  - 0.37 ha R.O.W. AS PER CURRENT DESIGN (JLR)
  - 22.13 ha COMMERCIAL AS PER SIGNATURE RIDGE PUMPING STATION HYDRAULIC GRADE ANALYSIS - IBI GROUP, 2014 (REV 2)

No.	ISSUE / REVISION	DD/MM/YY
03	ISSUED TO CITY FOR REVIEW - SUBMISSION 3	28/04/2019
02	ISSUED TO CITY FOR REVIEW - SUBMISSION 2	22/02/2019
01	ISSUED TO CITY FOR REVIEW - SUBMISSION 1	27/11/2018

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VERIFY SHEET SIZE AND SCALES. BAR TO THE RIGHT IS 25mm IF THIS IS A FULL SIZE DRAWING.

SCALE: 1:2000

CLIENT:

CONSULTANT:

www.jrichards.ca

CONSULTANT:

PROJECT:

MINTO COMMUNITIES INC.  
ARCADIA STAGE 3

450 HUNTMAR DRIVE

DRAWING:

OVERALL SANITARY DRAINAGE PLAN

DESIGN: AT	DRAWING #:
DRAWN: CJM	OSAN
CHECKED: LD	
JLR #: 26299-03	

City File No: DO7-16-16-0025 Plan No: 17816

---

**Appendix D1**

Storm Sewer Design Sheet



---

## **Appendix E1**

Modelling Schematics



**Legend**

**Junctions**

- Carp River Nodes
- Low Points
- High Points
- Mid Points
- ▲ Outfalls

**Storages**

- On-Site Storage
- Pond

**Conduits**

- Carp River
- Streets
- Overland Flow Path (Weir)

**Subcatchments**

- IBI Model
- Rear\_Yard
- Front\_Yard
- Pond
- Road
- Commercial
- Park



100 m

PROJECT:

**Minto Communities Arcadia Stage 6  
450 Huntmar Drive**

DRAWING:

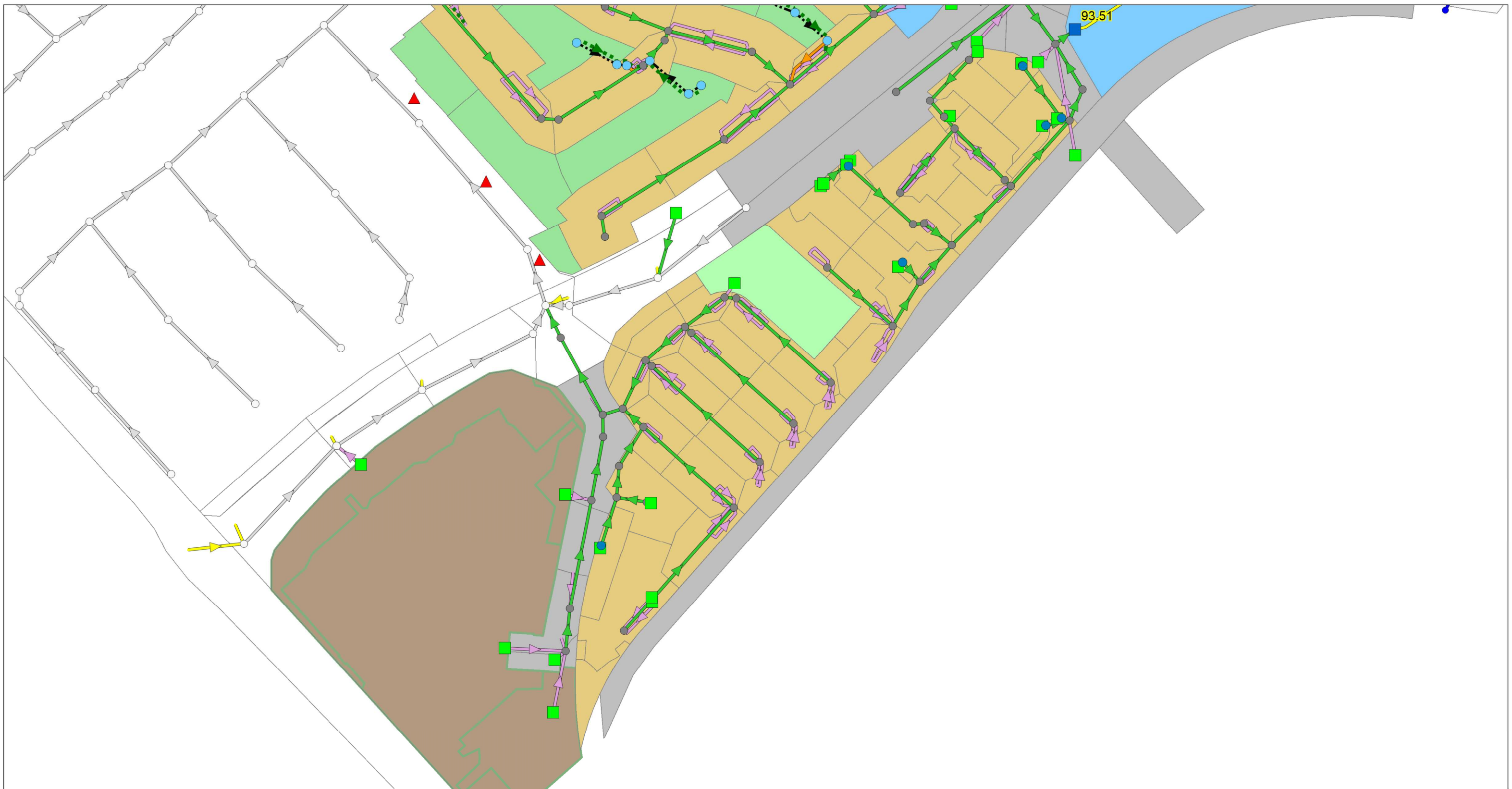
**Major System Model Schematic**



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DESIGN:	ID	JLR NO.:	26299-006
DRAWN:	ID	DRAWING NO.:	FIGURE E1-1
CHECKED:	BP		





**Legend**

Storages	Conduits	Outlets	Subcatchments
<span style="color: green;">■</span> On-Site Storage	<span style="color: yellow;">—</span> Campeau Drive Inlets	<span style="color: purple;">—</span> Street Inlet	<span style="border: 1px solid black; display: inline-block; width: 10px; height: 10px;"></span> IBIModel
<span style="color: grey;">●</span> Manhole	<span style="color: grey;">—</span> Existing Sewers	<span style="color: orange;">—</span> Rear Yard Inlet	<span style="color: lightgreen;">■</span> Rear_Yard
<span style="color: grey;">○</span> Existing Manhole	<span style="color: green;">—</span> Storm Sewers	<span style="color: grey;">—</span> Culvert	<span style="color: tan;">■</span> Front_Yard
<span style="color: blue;">●</span> Rear Yard Catchbasin	<span style="color: black;">—</span> Rear Yard Storm		<span style="color: lightblue;">■</span> Pond
			<span style="color: grey;">■</span> Road
			<span style="color: brown;">■</span> Commercial
			<span style="color: lightgreen;">■</span> Park



100 m

PROJECT:

**Minto Communities Arcadia Stage 6  
450 Huntmar Drive**

DRAWING:

**Minor System Model Schematic**



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DESIGN:	ID	JLR NO.:	26299-006
DRAWN:	ID	DRAWING NO.:	FIGURE E1-2
CHECKED:	BP		

---

# **Appendix E2**

CB Tables

## STREET CATCHBASINS

Street Name	CB ID Number	T/G	Inlet		Outlet		Return Period (years)	Rational Method Capture Rate (1:5 yr) (L/s)	Max Depth (100 yr) (m)	1:100 Yr Restricted Capture Rate (L/s)	ICD TYPE
			Pipe Dia. (mm)	Invert	Pipe Dia. (mm)	Invert					
Street 1	CB1	97.1	-	-	200	95.30	1:5 year	10	1.65	47	MHF IPEX TYPE C
	CB2	97.1	-	-	200	95.30	1:5 year	76	1.65	47	MHF IPEX TYPE C
	CB3	96.9	-	-	200	95.10	1:5 year	34	1.89	25	MHF IPEX TYPE A
	CB4	96.9	-	-	200	95.10	1:5 year	34	1.89	25	MHF IPEX TYPE A
	CB5	96.9	200	95.15	250	95.10	1:5 year	9	0.68	9	MHF IPEX TYPE A
	CB6	96.9	-	-	200	95.21	1:5 year	9	0.68	9	MHF IPEX TYPE A
	CB7	96.8	200	95.05	250	95.00	1:5 year	9	0.76	9	MHF IPEX TYPE A
	CB8	96.8	-	-	200	95.11	1:5 year	9	0.76	9	MHF IPEX TYPE A
	CB9	96.7	200	94.95	250	94.90	1:5 year	9	0.33	9	MHF IPEX TYPE A
	CB10	96.7	-	-	200	95.01	1:5 year	9	0.33	9	MHF IPEX TYPE A
	CB11	96.2	-	-	200	94.40	1:5 year	28	1.85	25	MHF IPEX TYPE A
	CB12	96.2	-	-	200	94.40	1:5 year	28	1.85	25	MHF IPEX TYPE A
	CB13	96	-	-	200	94.20	1:5 year	54	1.9	35	MHF IPEX TYPE B
	CB14	96	-	-	200	94.20	1:5 year	54	1.9	35	MHF IPEX TYPE B
	CB15	95.6	-	-	200	93.80	1:5 year	33	1.87	25	MHF IPEX TYPE A
	CB16	95.6	-	-	200	93.80	1:5 year	33	1.87	25	MHF IPEX TYPE A
Street 2	CB17	96.87	-	-	200	95.07	1:5 year	43	1.96	25	MHF IPEX TYPE A
	CB18	96.87	-	-	200	95.07	1:5 year	43	1.96	25	MHF IPEX TYPE A
	CB19	97.04	-	-	200	95.24	1:5 year	34	1.9	25	MHF IPEX TYPE A
	CB20	97.04	-	-	200	95.24	1:5 year	34	1.9	25	MHF IPEX TYPE A
	CB21	96.97	-	-	200	95.17	1:5 year	19	1.06	19	MHF IPEX TYPE A
	CB22	96.97	-	-	200	95.17	1:5 year	19	1.06	19	MHF IPEX TYPE A
	CB23	96.88	-	-	200	95.08	1:5 year	23	1.54	22	MHF IPEX TYPE A
	CB24	96.88	-	-	200	95.08	1:5 year	23	1.54	22	MHF IPEX TYPE A
	CB25	96.87	-	-	200	95.07	1:5 year	16	0.82	16	MHF IPEX TYPE A
	CB26	96.87	-	-	200	95.07	1:5 year	16	0.82	16	MHF IPEX TYPE A
	CB27	96.67	-	-	200	94.87	1:5 year	27	1.85	24	MHF IPEX TYPE A
	CB28	96.67	-	-	200	94.87	1:5 year	27	1.85	24	MHF IPEX TYPE A
	CB29	96.57	-	-	200	94.77	1:5 year	33	1.91	24	MHF IPEX TYPE A
	CB30	96.57	-	-	200	94.77	1:5 year	33	1.91	24	MHF IPEX TYPE A
Street 3	CB31	96.8	-	-	200	95.00	1:5 year	42	1.95	25	MHF IPEX TYPE A
	CB32	96.8	-	-	200	95.00	1:5 year	42	1.95	25	MHF IPEX TYPE A
	CB33	96.92	-	-	200	95.12	1:5 year	44	1.96	25	MHF IPEX TYPE A
	CB34	96.92	-	-	200	95.12	1:5 year	44	1.96	25	MHF IPEX TYPE A
Street 4	CB35	96.67	-	-	200	94.87	1:5 year	42	1.96	25	MHF IPEX TYPE A
	CB36	96.67	-	-	200	94.87	1:5 year	42	1.96	25	MHF IPEX TYPE A
	CB37	96.83	-	-	200	95.03	1:5 year	43	1.96	25	MHF IPEX TYPE A
	CB38	96.83	-	-	200	95.03	1:5 year	43	1.96	25	MHF IPEX TYPE A
Street 5	CB39	96.1	-	-	200	94.30	1:5 year	36	1.91	25	MHF IPEX TYPE A
	CB40	96.1	-	-	200	94.30	1:5 year	36	1.91	25	MHF IPEX TYPE A
	CB41	96.2	-	-	200	94.40	1:5 year	30	1.87	25	MHF IPEX TYPE A
	CB42	96.2	-	-	200	94.40	1:5 year	30	1.87	25	MHF IPEX TYPE A
Street 6	CB43	95.9	-	-	200	94.10	1:5 year	36	1.9	25	MHF IPEX TYPE A
	CB44	95.9	-	-	200	94.10	1:5 year	36	1.9	25	MHF IPEX TYPE A
	CB45	95.7	-	-	200	93.90	1:5 year	61	2.04	40	MHF IPEX TYPE C
	CB46	95.7	-	-	200	93.90	1:5 year	61	2.04	40	MHF IPEX TYPE C
	CB47	95.6	-	-	200	93.80	1:5 year	43	1.97	29	MHF IPEX TYPE B
	CB48	95.6	-	-	200	93.80	1:5 year	43	1.97	29	MHF IPEX TYPE B

### REAR YARD ICD TABLE

STREET	CB ID Number	T/G	INLET		OUTLET			1:100 Yr Restricted Flow (L/s)	ICD TYPE
			Pipe Dia. (mm)	Invert	Pipe Dia. (mm)	Pipe Length	Invert		
BLOCK TE-12	CB108	95.40	-	-	525	46.62	93.447	22	MHF_IPEX_TYPE_A
	CBMH4	94.8	525	92.981	525	6.48	92.981	61	MHF_IPEX_TYPE_D
BLOCK TE-5	CB103	96.30	250	94.338	375	63.38	94.275	65	MHF_IPEX_TYPE_D
BLOCK TE-2	CBMH1	97.70	-	-	375	37.00	95.460	57	MHF_IPEX_TYPE_D

---

# **Appendix E3**

HGL Analysis









**ARCADIA ALL STAGES HGL ANALYSIS**

MH ID	USF Elevation (m)	1:100 Year Event (3hr Chicago)		Climate Change Event (12 hr SCS)		Historical Storm July 1979		Historical Storm August 1988		Historical Storm August 1996	
		Max HGL (m)	Freeboard (m)	Max HGL (m)	Freeboard (m)	Max HGL (m)	Freeboard (m)	Max HGL (m)	Freeboard (m)	Max HGL (m)	Freeboard (m)
142_(Ex-	94.48	93.67	0.81	93.89	0.59	93.78	0.70	93.78	0.70	93.83	0.65
143_(Ex-	94.43	93.96	0.47	93.97	0.46	93.96	0.47	93.96	0.47	93.96	0.47
144_(Ex-	94.51	93.74	0.77	93.74	0.77	93.74	0.77	93.74	0.77	93.74	0.77
150_(Ex-	94.43	93.72	0.71	93.89	0.54	93.80	0.63	93.79	0.64	93.71	0.72
151_(Ex-	94.50	93.89	0.61	93.93	0.57	93.89	0.61	93.88	0.62	93.88	0.62
152_(Ex-	94.75	93.96	0.79	93.98	0.77	93.96	0.79	93.95	0.80	93.95	0.80
153_(Ex-	94.89	94.10	0.79	94.10	0.79	94.09	0.80	94.09	0.80	94.09	0.80
154_(Ex-	95.06	94.19	0.87	94.19	0.87	94.19	0.87	94.19	0.87	94.19	0.87
CAMPEAU_	-	93.51	-	93.88	-	93.61	-	93.58	-	93.51	-
TEXT	Stage 6										
TEXT	Stage 4										
TEXT	Stage3										
TEXT	Stages 1 &										
MH305	-	94.93	-	94.99	-	94.91	-	94.91	-	94.88	-
MH304	-	94.92	-	94.99	-	94.91	-	94.90	-	94.88	-
MH303C	-	94.79	-	94.94	-	94.76	-	94.77	-	94.69	-
MH303B	-	94.87	-	95.02	-	94.85	-	94.86	-	94.76	-
MH303	-	94.76	-	94.94	-	94.74	-	94.75	-	94.57	-
MH302	-	95.42	-	95.48	-	95.41	-	95.41	-	95.35	-
MH301	-	95.95	-	96.01	-	95.95	-	95.95	-	95.87	-
MH300	-	96.51	-	96.51	-	96.47	-	96.51	-	96.42	-
MH217	95.44	94.72	0.72	94.90	0.54	94.70	0.74	94.72	0.72	94.53	0.91
MH216	95.20	94.64	0.56	94.83	0.37	94.62	0.58	94.65	0.55	94.44	0.76
MH215	95.40	94.85	0.55	94.93	0.47	94.82	0.58	94.84	0.56	94.78	0.62
MH214	95.51	95.15	0.36	95.17	0.34	95.14	0.37	95.15	0.36	95.12	0.39
MH213	96.19	95.21	0.98	95.22	0.97	95.20	0.98	95.20	0.98	95.19	1.00
MH212	95.89	95.50	0.39	95.50	0.39	95.50	0.39	95.50	0.39	95.50	0.39
MH211	95.53	95.16	0.37	95.17	0.36	95.15	0.38	95.16	0.37	95.14	0.39
MH210	95.79	95.07	0.72	95.11	0.68	95.04	0.75	95.06	0.73	95.01	0.78
MH209	96.10	95.66	0.44	95.68	0.42	95.63	0.47	95.66	0.44	95.61	0.49
MH208	97.64	97.17	0.47	97.17	0.47	97.17	0.47	97.17	0.47	97.17	0.47
MH207	97.64	96.55	1.09	96.55	1.09	96.54	1.10	96.54	1.10	96.54	1.10
MH206	97.64	96.05	1.59	96.05	1.59	96.05	1.59	96.05	1.59	96.05	1.59
MH205	96.85	95.92	0.93	95.92	0.93	95.92	0.93	95.92	0.93	95.92	0.93
MH204	95.62	94.91	0.71	94.99	0.63	94.88	0.74	94.90	0.72	94.84	0.78
MH203	95.43	94.77	0.66	94.90	0.53	94.73	0.70	94.77	0.66	94.65	0.78
MH202	95.35	94.67	0.68	94.83	0.52	94.63	0.72	94.68	0.67	94.50	0.85
MH201	95.11	94.51	0.60	94.72	0.39	94.51	0.60	94.54	0.57	94.31	0.80
MH200	95.11	94.27	0.84	94.53	0.58	94.31	0.80	94.35	0.76	94.05	1.06
MH541	95.10	93.83	1.27	94.20	0.90	93.97	1.13	94.02	1.08	93.59	1.51
MH532	95.10	93.90	1.20	94.27	0.83	94.03	1.07	94.08	1.02	93.62	1.48
MH104	95.32	93.93	1.39	94.30	1.02	94.05	1.27	94.11	1.21	93.66	1.66
MH513	97.18	96.12	1.06	96.12	1.06	96.08	1.10	96.09	1.09	96.07	1.11
MH519	95.33	94.23	1.10	94.52	0.81	94.26	1.07	94.32	1.01	94.22	1.11
MH540	95.25	93.95	1.30	94.34	0.91	94.07	1.18	94.14	1.11	93.78	1.47
MH531	95.68	94.12	1.56	94.50	1.18	94.21	1.47	94.26	1.42	93.82	1.86
MH520	95.33	94.08	1.25	94.44	0.89	94.17	1.16	94.25	1.08	94.04	1.29
MH501	97.83	95.54	2.29	95.54	2.29	95.50	2.33	95.50	2.33	95.46	2.37
MH518	96.37	94.83	1.54	94.86	1.51	94.80	1.57	94.82	1.55	94.78	1.59
MH529	95.47	94.37	1.10	94.74	0.73	94.43	1.04	94.45	1.02	94.08	1.39
MH517	96.57	95.11	1.46	95.12	1.45	95.10	1.47	95.10	1.47	95.09	1.48
MH527	96.23	94.67	1.56	94.95	1.28	94.66	1.57	94.66	1.57	94.66	1.57
MH528	95.78	94.43	1.35	94.80	0.98	94.49	1.29	94.50	1.28	94.16	1.62
MH526	96.51	95.16	1.35	95.16	1.35	95.15	1.36	95.15	1.36	95.15	1.36
MH525	97.77	95.25	2.52	95.25	2.52	95.25	2.52	95.25	2.52	95.25	2.52
MH524	95.17	93.80	1.37	94.17	1.00	93.95	1.22	94.00	1.17	93.58	1.59
MH523	95.23	93.93	1.30	94.23	1.00	94.01	1.22	94.05	1.18	93.93	1.30
MH522	95.15	93.75	1.40	94.23	0.92	93.77	1.38	94.05	1.10	93.75	1.40
MH521	95.28	94.05	1.23	94.37	0.91	94.05	1.23	94.05	1.23	94.05	1.23
MH516	96.60	95.28	1.32	95.28	1.32	95.26	1.34	95.27	1.33	95.26	1.34
MH505	95.16	93.73	1.43	94.08	1.08	93.89	1.27	93.94	1.22	93.56	1.60
Ex_509	94.54	93.60	0.94	93.93	0.61	93.77	0.77	93.81	0.73	93.50	1.04
MH515	96.73	95.76	0.97	95.77	0.96	95.72	1.01	95.72	1.01	95.71	1.02
MH504	95.28	93.76	1.52	94.13	1.15	93.92	1.36	93.98	1.30	93.58	1.70
MH514	97.10	95.68	1.42	95.71	1.39	95.64	1.46	95.66	1.44	95.61	1.49
MH502	96.38	94.99	1.39	94.99	1.39	94.94	1.44	94.94	1.44	94.90	1.48
MH503	95.98	94.16	1.82	94.45	1.53	94.17	1.81	94.21	1.77	94.07	1.91
MH530	95.47	94.25	1.22	94.62	0.85	94.32	1.15	94.36	1.11	93.93	1.54
MH506	95.13	93.69	1.44	94.03	1.10	93.85	1.28	93.90	1.23	93.54	1.59
TEXT	Stage 6										
TEXT	Stage 4										
TEXT	Stage3										
TEXT	Stages 1 &										

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**Appendix E4**

Street Ponding Areas

### STREET PONDING TABLE

Ponding Area ID	Top of Grate (m)	Maximum Static Depth (m)	1:2 year	1:5 year Depth	1:10 year		1:25 year		1:50 year		1:100 year		Climate Change		Lowest Opening Grade (m)
			Total Ponding Depth (m)	Total Ponding Depth (m)	Total Ponding Depth (m)	Max. HGL (m)	Total Ponding Depth (m)	Max. HGL (m)	Total Ponding Depth (m)	Max. HGL (m)	Total Ponding Depth (m)	Max. HGL (m)	Total Ponding Depth (m)	Max. HGL (m)	
1	97.1	0.30	0	0	0.02	97.12	0.08	97.18	0.14	97.24	0.2	97.30	0.23	97.33	98.10
2	96.9	0.30	0	0	0	-	0.04	96.94	0.07	96.97	0.09	96.99	0.19	97.09	97.40
4	96.9	0.10	0	0	0	-	0	-	0.00	-	0	-	0	-	97.08
6	96.8	0.10	0	0	0	-	0	-	0.00	-	0	-	0	-	96.99
8	96.7	0.19	0	0	0	-	0	-	0.00	-	0	-	0	-	96.86
9	96.2	0.20	0	0	0	-	0	-	0.00	-	0.05	96.25	0.1	96.30	96.70
11	96	0.15	0	0	0	-	0.06	96.06	0.08	96.08	0.1	96.10	0.14	96.14	96.65
12	95.6	0.22	0	0	0	-	0.02	95.62	0.05	95.65	0.07	95.67	0.11	95.71	96.20
3	96.87	0.23	0	0	0.06	96.93	0.1	96.97	0.13	97.00	0.16	97.03	0.2	97.07	97.26
30	97.04	0.23	0	0	0	-	0.04	97.08	0.07	97.11	0.1	97.14	0.15	97.19	97.48
28	96.97	0.30	0	0	0	-	0	-	0.00	-	0	-	0	-	97.32
27	96.88	0.29	0	0	0	-	0	-	0.00	-	0	-	0.06	96.94	97.24
26	96.87	0.20	0	0	0	-	0	-	0.00	-	0	-	0	-	97.12
24	96.67	0.30	0	0	0	-	0	-	0.00	-	0.05	96.72	0.1	96.77	97.12
23	96.57	0.13	0	0	0	-	0.04	96.61	0.08	96.65	0.11	96.68	0.15	96.72	96.86
5	96.77	0.16	0	0	0.05	96.82	0.09	96.86	0.12	96.89	0.15	96.92	0.2	96.97	97.08
29	96.92	0.25	0	0	0.06	96.98	0.11	97.03	0.14	97.06	0.16	97.08	0.21	97.13	97.24
7	96.67	0.17	0	0	0.05	96.72	0.1	96.77	0.13	96.80	0.16	96.83	0.2	96.87	96.99
25	96.83	0.24	0	0	0.05	96.88	0.1	96.93	0.13	96.96	0.16	96.99	0.21	97.04	97.12
22	95.9	0.25	0	0	0	-	0.05	95.95	0.07	95.97	0.1	96.00	0.14	96.04	96.2
16	95.7	0.20	0	0	0.07	95.77	0.15	95.85	0.22	95.92	0.24	95.94	0.26	95.96	96.02
15	95.6	0.22	0	0	0.04	95.64	0.09	95.69	0.12	95.72	0.17	95.77	0.25	95.85	96.2



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