REPORT 37543-5.2.2

## SCOTIABANK RIDEAU AND WILLIAM STREET STORMWATER MANAGEMENT BRIEF



Prepared for: SCOTIABANK

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October 14, 2016

Prepared for: Scotiabank

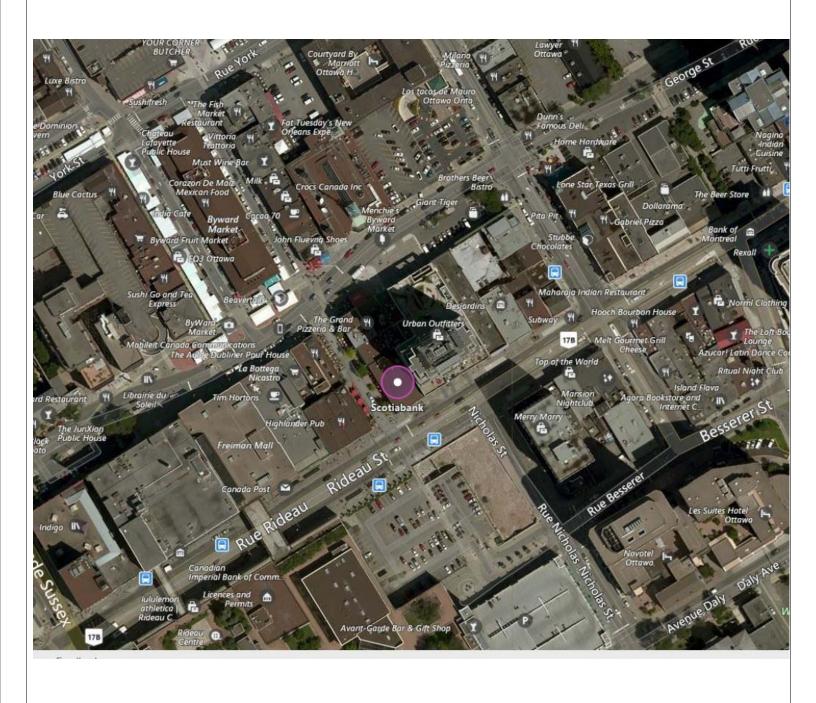
### 1 INTRODUCTION

IBI Group has been retained by Scotiabank to design their new building at Rideau Street and William Street in the City of Ottawa. These lands are known as 117 Rideau Street and are zoned mixed use MD S 80.

The development also forms part of the City of Ottawa's LRT project, at this location commuters will be able to access the LRT tunnel which is located under the building. The development also provides the LRT a mechanical connection point to surface where heating and ventilation is provided to the LRT facility through the building. The LRT project has previously received approval for the relocation of municipal infrastructure, for information on site services, see Pageau and Morel Site Servicing Brief for details.

This report deals with addressing the on-site attenuation of stormwater for this development. This site is bounded by existing commercial buildings on two sides and William Street and Rideau Street on the other, see **Figure 1**. This report provides details on stormwater management to support the Site Plan for this development. No pre-consultation meetings were requested with the Rideau Valley Conservation Authority (RVCA), and Ministry of Environment (MOE) to solicit input on the detailed design as this is an infill parcel with connections to existing sewers.

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					PROJECT TITLE SCOTIABANK - RIDEAU	SCALE	SKETCH NO.	ISSUE
					AND WILLIAM BRANCH	JOB NO. 37543	FIGURE 1	
						DRAWN BY		
				IBI	EXISTING AREA - WILLIAM AND RIDEAU	DO NOT SCALE RESPONSIBLE F AND REPORTING	THIS DRAWING. THE CONTRACTOR SHALL FOR TAKING AND VERIFYING ALL THE DIMEN GERRORS AND/OR OMISSIONS TO THE ARC	NSIONS
REVISION NO.	ISSUED WITH	STATUS	REVISION DATE	ibigroup.com	STREET	IN WRITING BEF	ORE PROCEEDING WITH THE WORK.	

Prepared for: Scotiabank

### 2 STORMWATER MANAGEMENT

The proposed development replaces a previous building which as per the proposed development, occupies 100% of the site. The existing building was connected to the municipal storm system and unattenuated roof drain discharged to the storm sewer system. Based on a roof area of 512  $m^2$ , C=1.0, 100 year I = 178.6 mm/hr at a Tc of 10 min., the existing building would have a peak flow of 25.86 l/s during a 1:100 yr. rainfall event.

City of Ottawa design guidelines for the redevelopment of sites in the City core area (separated sewer areas) identified the preferred stormwater management, and provided on-site attenuation to achieve restricting peak flow from the site to mirror a 1:5 year rainfall event with C=0.5.

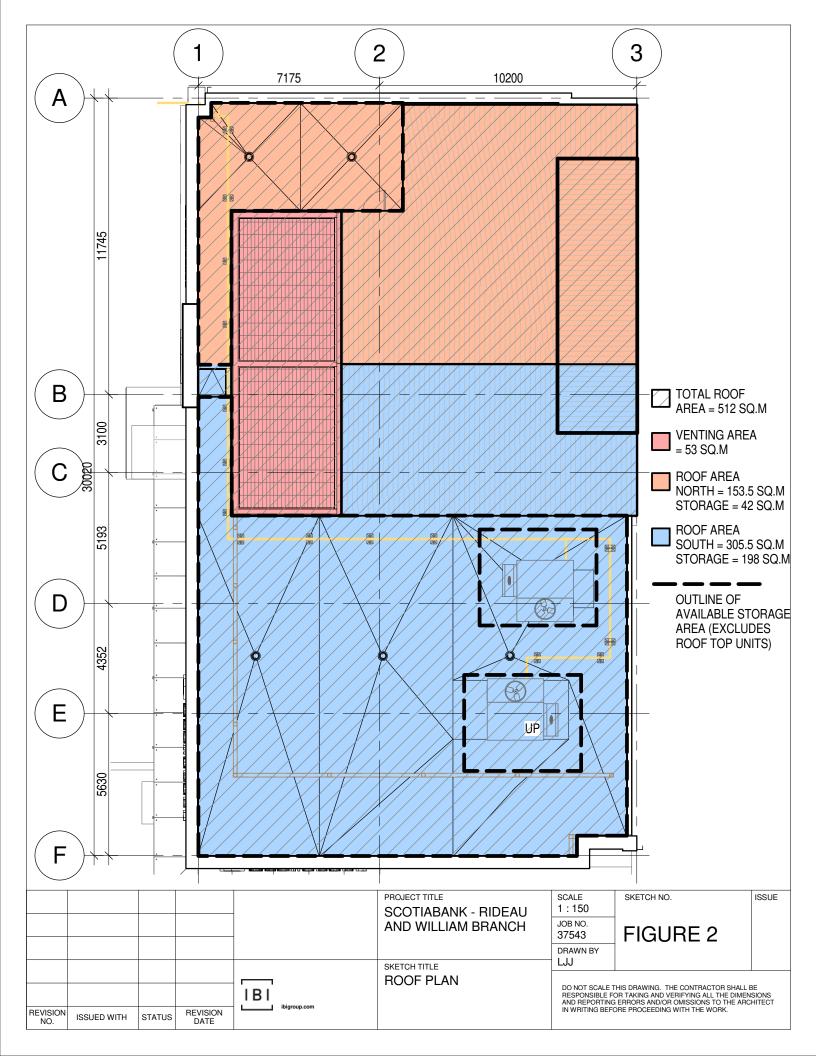
As noted previously, this building is serving dual function in that not only is it a commercial building but it provides access to grade for the LRT tunnel located below the building. This includes both LRT riders and mechanical systems. Figure 2 illustrates the proposed roof configuration for the building which includes features for the LRT's mechanical facilities. These features restrict the area available for roof top storage use. Alternative storage solutions include below grade storage, however, due to the presence of the LRT tunnel this is not possible, hence the stormwater management solution will need to work with the available roof area.

The proposed roof configuration provides for two roof drains on the north half and three on the south half. Utilizing Watts adjustable flow control roof drains (see attached spec sheet, or approved equal) at a discharge rate of 2.27 l/s (30gpm) for the north drains (2X2.27=4.54), and 1.51 l/s (20gpm) for the south drains (3X1.51=4.54), combining the roof drains, a total flow of 9.08l/s with a Tc of 10 min, for a 1:100 yr. rainfall event the effective runoff coefficient is C=0.76, which equates to a runoff coefficient of C=0.61 for a 1:5 yr. event.

The total area of the roof is  $521 \text{ m}^2$ , of which  $240 \text{ m}^2$  is available for use as stormwater roof storage, based on 0.15 m maximum depth ponding and the proposed roof configuration. The available roof top storage for the northern and southern halves of the roof is  $2.1 \text{ m}^3$  and  $7.42 \text{ m}^3$  respectively. The modified rational method notes for the roof drainage areas with C=1.0 (1:100), and discharge rate of 4.54l/s for each of the north and south section of roof,  $1.9 \text{ m}^3$  and  $6.86 \text{ m}^3$  of storage is required respectively, see attached calculation.

While the runoff coefficient does not meet the preferred C=0.5 (because of the limited storage due to the LRT requirements) it must be recognized the existing condition peak flow during a 1:100 event is 25.86 l/s and the proposed solution reduces that peak flow to 9.08 l/s, hence the post development condition is a significant improvement.

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### **RD-100**

Tag:

### Large Capacity **Roof Drain**

### Components:

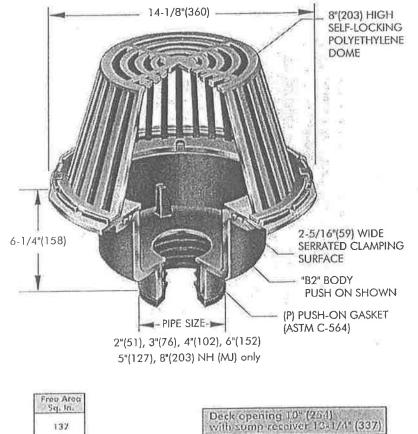








SPECIFICATION: Watts Drainage Products RD-100 epoxy coated cast iron roof drain with deep sump, wide serrated flashing flange, flashing clamp device with integral gravel stop and self-locking polyethylene (standard) dome strainer.



\*\* Side Outlet (-SO) option only available in 2"(51), 3"(76), 4"(102) pipe sizes. Underdeck Clamp (-BED and -D options) are not available when -SO is selected,

x. RI	D-102P-K	1
900	ipe Sizing (Select One)	E
offix.	Description	
2	2"(51) Pipe Size	
3	3"(76) Pipe Size	ו⊒
4	4"(102) Pipe Size	밀
5	5"(127) Pipe Size	$\exists$
6	6"(152) Pipe Size	님ㅣ
8	B"(203) Pipe Size	니
0	utlet Type (Select One)	100
ollix	Description	
NH	No Hub (MJ)	
P	Push On	
7	Threaded Outlet	
X	Inside Coulk	Ш
Opt	ions (Select One or Mo	re)
offix	Description	
·A	Accutrol weir (specify # 1-6 slat	пП
-B	Sump Receiver Flange	'n
-BED	Sump Receiver, Adj Ext.,	
	Deck Clamp	
-C	Secondary Membrana Clamp	$\exists$
-D	Underdeck Clomp	닏
-E	Adjustable Extension	닏
-GSS	Stainless Steel Ballast Guard	님
-H	Adj. to 6" IRMA Ballast Guard	$\vdash$
-K	Ductile Iron Dome	님
-K80	Aluminum Dome	
-L	Vandal Proof Dome	$\exists$
-R	2" High External Water Dam	$\exists$
-\$0	Side Outlet**	님
-V	Fixed Extension (1-1/2",2",3",4"	Ы
-W	Adj. Water Level Regulator	
-W-1	Waterproofing Flange	
-Z	Extended Integral Wide Flange	
-5	Sediment Bucket	
-12	Galvanized Dome	닏
-13	All Galvanized	Ц
-83	Mesh Covered Dame	
113M	Special Epoxy from 3M Range	
Optio	onal Body Material (NH O	ily)
Sulfix	Description'	
-60	PVC Body w/Socket Outlet	П
-61	ABS Body w/Socket Outlet	

Job Name	Contractor
Job Location	Contractor's P.O. No.
Engineer	Representative

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without increasing any obligation to make similar changes and modifications to product previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances. CANADA



CANADA: 5435 North Service Rood, Burlington, ON, L7L SH7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Websile: ywww.wullsdroinage.co



### Adjustable Accutrol Weir

Tag:

# Adjustable Flow Control for Roof Drains

### ADJUSTABLE ACCUTROL(for Large Sump Roof Drains only)

For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below. Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

#### **FXAMPLE:**

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be: [5 gpm(per inch of head) x 2 inches of head] + 2-1/2 gpm(for the third inch of head) = 12-1/2 gpm.

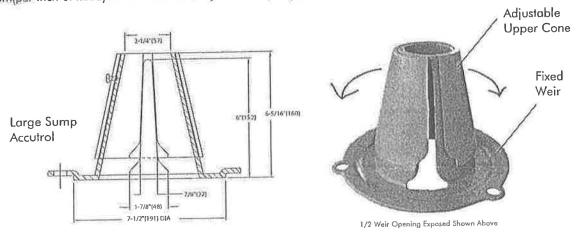


TABLE 1. Adjustable Accutrol Flow Rate Settings

			Head of Wate	r		
Weir Opening	1"	2"	3"	4"	5"	6"
Exposed		Flow	Rate (gallons p	er minute)		
Fully Exposed	5	10	15	20	25	30
3/4	5	10	13.75	17.5	21.25	25
1/2	5	10	12.5	15	17.5	20
1/4	5	10	11.25	12.5	13.75	15
Closed	5	10	10	10	10	10

Job Name	Contrador
Job Location	Contractor's P.O. No.
Engineer	Representative

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CANADA: 5435 North Service Road, Burlington, ON, L7L 5M7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Website: www.waltsdrainage,ca



IBI 333 Preston St OTTAWA, ONTARIO K1S 5N4

ONSITE SWM 100yr design
PROJECT: Scotia Bank Rideau/William

CITY OF OTTAWA

DEVELOPER : Scotiabank

PAGE: 1 OF 1 JOB #: 37543 DATE: Oct 14, 2016 DESIGN: DY Rev#0

#### 100yr design

#### MAXIMUM ALLOWABLE FLOW - Flow Restricted to 5yr C= 0.61

#### Time of concentration = 10 minutes

Area (ha) =	0.051
post C Average =	0.61
post C Average =	0.90

#### Intensity - 5 year event storm

intensity - 5 year event storm						
10 min Tc	i5yr = 998.071/(T+6.053)^0.814=	104.2 mm/hr				

#### Unrestricted Flowrate (Q5)

Qpre-devo = 2.78\*A\*Cw\*i = **9.08** l/s 10 min Tc

#### Intensity - 100 year event storm

10 min Tc i100yr = 1735.688/(T+6.014)^0.82= 178.6 mm/hr

#### Unrestricted Flowrate (Q100)

Qpost-devo = 2.78\*A\*Cw\*i = **25.42** l/s

#### STORM WATER MANAGEMENT - Post-Development Controlled

(5 year post-development with 100yr inlets)

ROOF AREA (north)	
153.5 sm	
100 -YR FLOW	
On (I/c)	

Area(ha)=	0.0154					
Cw =	1.00	STORMWATER MANAGEMENT Qm =		TER MANAGEMENT Qm = 4.54		l/s
Tc		Qp	Qm	Qp-Qm	Volume	Г
Variable	i	2.78 x Area x c x i		-		ĺ
(min)	(mm/hour)	(l/s)	(l/s)	(l/s)	(m3)	ĺ
5	242.7	10.4	4.54	5.8	1.75	j
6	226.0	9.6	4.54	5.1	1.84	j
7	211.7	9.0	4.54	4.5	1.89	j
8	199.2	8.5	4.54	4.0	1.90	ĺ
9	188.3	8.0	4.54	3.5	1.89	ĺ
10	178.6	7.6	4.54	3.1	1.85	ĺ
11	169.9	7.3	4.54	2.7	1.79	ĺ
12	162.1	6.9	4.54	2.4	1.71	ĺ
13	155.1	6.6	4.54	2.1	1.62	j
14	148.7	6.3	4.54	1.8	1.52	j
15	142.9	6.1	4.54	1.6	1.40	j
16	137.5	5.9	4.54	1.3	1.28	

Required volume for roof storage

ROOF AREA (south)	
305.5 sm	

100 -YR FLOW

Qp (l/s)						
Area(ha)=	0.0306					
Cw =	1.00	STORMWATER MANAGEMENT Q	m =		4.54	l/s
Tc		Qp	Qm	Qp-Qm	Volume	
Variable	i	2.78 x Area x c x i				l
(min)	(mm/hour)	(l/s)	(l/s)	(l/s)	(m3)	l
14	148.7	12.6	4.54	8.1	6.80	l
15	142.9	12.1	4.54	7.6	6.84	l
16	137.5	11.7	4.54	7.1	6.86	l
17	132.6	11.3	4.54	6.7	6.86	l
18	128.1	10.9	4.54	6.3	6.84	l
19	123.9	10.5	4.54	6.0	6.82	l
20	120.0	10.2	4.54	5.6	6.78	l
21	116.3	9.9	4.54	5.3	6.72	l
22	112.9	9.6	4.54	5.0	6.66	l
23	109.7	9.3	4.54	4.8	6.59	l
24	106.7	9.1	4.54	4.5	6.51	l
25	103.8	8.8	4.54	4.3	6.42	1

<=== Required volume for roof storage

Prepared for: Scotiabank

### 3 CONCLUSIONS

Stormwater sewers required to support the development are adjacent and available to service this site. The stormwater management approach noted in this report demonstrates the 1:100 yr post development flows from the site will be significantly reduced from pre development levels, and will not negatively impact the existing municipal storm sewer system.

The detail design is subject to governmental approval prior to construction, including but not limited to the following:

• Commence Work Order: City of Ottawa

Report prepared by:

Demetrius Yannoulepoulos, P. Eng. Associate Director