Servicing and Stormwater Management Brief – Wellings of Stittsville Phase 2, 20 Cedarow Court

Project # 160401511

Stantec

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Sign-off Sheet

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1.0 INTRODUCTION

Stantec Consulting Ltd. has been commissioned by Nautical Land Group. to prepare the following servicing study in support of the development at 20 Cedarow Court located within the City of Ottawa. The subject property is located northwest of the intersection of Huntmar Road and Hazeldean Road. The property location is indicated in **Figure 1**. The proposed mixed use residential and commercial development comprises approximately 2.29ha of land and proposes construction of a 284 unit, six storey residential building (Phase 2 and 3), a second 200 unit (Phase 4), six storey residential building, as well as commercial buildings, all of which are proposed to be connected via one level of underground parking. The site will be constructed in two phases, beginning with building phases 2 and 3 located adjacent to Hazeldean Road, and ultimately constructing phase 4. The intent of this report is to provide a servicing scenario for the site that is free of conflicts, provides on-site servicing in accordance with City of Ottawa design guidelines, and utilizes the existing local infrastructure in accordance with the guidelines outlined in background documents, and as per consultation with City of Ottawa.

Figure 1 Location Plan

2.0 BACKGROUND

Documents referenced in preparing the site design for the 20 Cedarow Court Development include:

- Kanata West Master Servicing Study, Stantec Consulting Ltd., Cumming Cockburn Limited / IBI, October 1, 2014.
- Carp River PCSWMM Model Documentation Draft Report, City of Ottawa, March 2016.
- Geotechnical Investigation, Proposed Mixed Use Development Wellings of Stittsville – Phase 2 20 Cedarow Court, Ottawa, Ontario, Paterson Group, March 7, 2019.
- Geotechnical Plan Review, Proposed Mixed Use Development Wellings of Stittsville – Phase 2 20 Cedarow Court, Ottawa, Ontario, Paterson Group, August 12, 2021.
- Servicing and Stormwater Management Brief-5731 Hazeldean Road, Stantec Consulting Ltd., March 22, 2017
- Tree Conservation Report – 5731 Hazeldean Road, IFS Associates, March 11, 2016.
- City of Ottawa Sewer Design Guidelines, City of Ottawa, October 2012.f
- City of Ottawa Design Guidelines – Water Distribution, City of Ottawa, July 2010.

3.0 WATER SUPPLY SERVICING

3.1 BACKGROUND

The proposed development comprises two residential apartment buildings with commercial space fronting Hazeldean Road, and complete with associated infrastructure and underground parking. The site is located west of Huntmar Drive, north of Hazeldean Road, and south of Poole Creek, and lies within the City's 3W pressure zone. The site will be serviced at two connection points via a proposed 200mm diameter connection to the existing stub within the Fringewood Avenue ROW at the eastern quadrant of the site, and a 300mm diameter connection to the existing 300mm diameter watermain within Cedarow Court along the western boundary of the site. The stub on Fringewood Avenue connects directly to the existing 762mm feedermain within Hazeldean Road immediately south of the site. The plumbing through the buildings will be looped.

3.2 WATER DEMANDS

Water demands for the development were estimated using the Ministry of Environment's Design Guidelines for Drinking Water Systems (2008) and the Ottawa Design Guidelines – Water Distribution (2010). A daily rate of 28,000 L/gross ha/day has been applied for commercial building space, whereas the residential facility demand was estimated at 350L/person/day with an estimated population of 1.4 persons/unit for bachelor or one bedroom apartments and 2.1 persons/unit for two bedroom apartments. See **Appendix A.1** for detailed domestic water demand estimates.

The average day demand (AVDY) for the entire site was determined to be 3.2 L/s. The maximum daily demand (MXDY) is 1.5 times the AVDY for commercial property demand and 2.5 times the AVDY for residential demand, which equates to 7.86 L/s. The peak hour demand (PKHR) is 1.8 times the MXDY for commercial property and 2.2 times the MXDY for residential properties, totaling 17.19 L/s.

Non-combustible construction with 2-hour fire separation between each floor was considered in the assessment of the fire flow requirements for the site according to the FUS Guidelines. The FUS Guidelines indicate that low hazard occupancies include apartments, dwellings, dormitories, hotels, and schools, and as such, a low hazard occupancy / limited combustible building contents credit was applied. A sprinkler system conforming to NFPA 13 was considered, and a credit applied per FUS Guidelines. Based on calculations per the FUS Guidelines (**Appendix A.2**), the maximum required fire flows for this development is 167 L/s (10,000 L/min) occurring at the proposed six-storey apartment building fronting Hazeldean Road ROW. Two hydrants located in proximity to both buildings siamese connections are proposed on the subject site. The existing hydrants along the northeastern boundary and the two proposed on site hydrants will provide amply fire flow.

3.3 PROPOSED SERVICING

Per boundary conditions provided by the City of Ottawa and an approximate elevation on-site of 104.7m, adequate domestic water supply is available for the subject site with pressures ranging from 44.9m (75.4psi) to 56.4m (80.3psi). These values are within the normal operating pressure range as defined by the MECP and City of Ottawa design guidelines (desired 50-80 psi and not less than 40 psi). A pressure check once construction is completed is required to determine if pressure reducing valves are needed.

The boundary conditions for the proposed development under maximum day demands were initially provided under an assumed fire flow demand of 267L/s. As such, it can be confirmed that the system will maintain a residual pressure which is in excess of the required 140 kPa (20 psi) under the required fire flow demand of 167L/s. The above demonstrates that the existing watermain within Fringewood Avenue and Cedarow Court can provide adequate fire and domestic flows in excess of flow requirements for the subject site. An existing hydrant is located approximately 18m northeast of the subject site and at least one proposed hydrant is to be located within 45m of the building fire department connection (siamese) per OBC requirements.

3.4 SUMMARY OF FINDINGS

The proposed development is located in an area of the City's water distribution system that has sufficient capacity to provide both the required domestic and emergency fire flows. Based on the boundary conditions as provided by the City of Ottawa staff, fire flows are available for this development based on FUS guidelines and as per the City of Ottawa water distribution guidelines.

4.0 WASTEWATER SERVICING

4.1 BACKGROUND

The site will be serviced via an existing 675mm dia. sanitary sewer located within the Hazeldean Road ROW south of the site and west of the intersection of Hazeldean Road and Huntmar Drive, which will ultimately outlet to the Kanata West Pump Station (see **Drawing SSP-1**).

4.2 DESIGN CRITERIA

As outlined in the City of Ottawa Sewer Design Guidelines and the MECP's Design Guidelines for Sewage Works, the following criteria were used to calculate estimated wastewater flow rates and to size the sanitary sewers:

- Minimum Velocity – 0.6 m/s (0.8 m/s for upstream sections)
- Maximum Velocity – 3.0 m/s
- Manning roughness coefficient for all smooth wall pipes – 0.013
- Minimum size – 250mm dia. for commercial areas
- Average Wastewater Generation (Commercial) – 28,000L/gross ha/day of building space
- Average Wastewater Generation (Residential) – 280L/cap/day
- Peak Factor (Commercial) – 1.5 (Max Day Demand per MOE Design Guidelines for Drinking Water Systems)
- Peak Factor (Residential) – 4.0 (Harmon's)
- Extraneous Flow Allowance – 0.33 l/s/ha (conservative value)
- Manhole Spacing – 120 m
- Minimum Cover – 2.5m
- Population density for single-bedroom and bachelor apartments – 1.4 pers./apartment
- Population density for two-bedroom apartments – 2.1 pers./apartment

4.3 PROPOSED SERVICING

The proposed site will be serviced by a gravity sewer which will direct the wastewater flows (approx. 10.4 L/s with allowance for infiltration) to the existing 675mm dia. Hazeldean Road sanitary sewer. A backflow preventer will be required for the on-site building in the event of surcharge of the sanitary sewer and will be coordinated with building mechanical engineers. A proposed excavation cross section of the Hazeldean Road connection to the existing 675mm diameter sanitary sewer has been included on **Drawing SSP-1.** Extra precaution should be taken to ensure no damages are made to the existing 762 backbone watermain located 2.9m south of the proposed sanitary connection. Additional construction details will be included by the contractor prior to construction.

The proposed drainage pattern is in accordance with the Kanata West Master Servicing Study (KWMSS) for Hazeldean Road and is detailed on **Drawing SAN-1.** Sanitary flows will ultimately be discharging to the downstream Kanata West Pump Station. A Sanitary sewer design sheet is included in **Appendix B.1**. Excerpts of the overall sanitary system discharging to the Kanata West Pump Station based on the KWMSS are included in **Appendix B.2.** It is noted that peak ultimate sanitary discharge to the KWPS is likely to be far lower than that indicated within the KWMSS design sheet, as current operational parameters estimating peak flow from residential uses have decreased from 350L/person/day to 280L/person/day, and commercial lands contributions have decreased from 50,000L/ha/day to 28,000L/ha/day. As a result, it is assumed that there is ample capacity within the downstream conveyance network and KWPS to receive any additional flows from that originally assumed for the area (50,000L/ha/day x 2.29ha x 1.5 P.F. = approximately 2.0L/s).

5.0 STORMWATER MANAGEMENT

5.1 OBJECTIVES

The objective of this stormwater management plan is to determine the measures necessary to control the quantity of stormwater released from the proposed development to established criteria, and to provide sufficient detail for approval and construction. The proposed development will discharge treated and controlled stormwater runoff to Poole Creek.

5.2 SWM CRITERIA AND CONSTRAINTS

Criteria were established by combining current design practices outlined by the City of Ottawa Design Guidelines (2012), Ministry of Environment Conservation and Parks (MECP) and Mississippi Valley Conservation Authority (MVCA). The following summarizes the criteria, with the source of each criterion indicated in italics:

General

- Use of the dual drainage principle (City of Ottawa)
- Wherever feasible and practical, site-level measures should be used to reduce and control the volume and rate of runoff (City of Ottawa)
- Assess impact of 100-year event outlined in the City of Ottawa Sewer Design Guidelines, and climate change scenarios with a 20% increase of rainfall intensity, on major & minor drainage system (City of Ottawa)
- Quality control to be provided for 80% TSS removal (MVCA, MECP)
- Site discharge to be controlled to pre-development rates (City of Ottawa)

Storm Sewer & Inlet Controls

- Size storm sewers to convey the 2-year storm event under free-flow conditions using City of Ottawa I-D-F parameters (City of Ottawa)
- Minimum sewer inlet capture rates to be set such that no ponding occurs at the end of the - 2-year event (City of Ottawa)
- Request made by the client to not allow ponding to occur in the 100-year event
- Hydraulic Grade Line (HGL) analysis to be conducted using the 100 year 12 hour SCS storm distribution (City of Ottawa)
- 100-year Storm HGL to be a minimum of 0.30 m below building foundation footing otherwise foundation drains will be pumped (City of Ottawa)

Surface Storage & Overland Flow

- Building openings to be a minimum of 0.30m above the 100-year water level (City of Ottawa)
- Maximum depth of flow under either static or dynamic conditions shall be less than 0.30m (City of Ottawa)
- Subdrains required in swales where longitudinal gradient is less than 1.5% (City of Ottawa)
- Provide adequate emergency overflow conveyance off-site (City of Ottawa)

5.2.1 Pre-Development Conditions

A background report for 20 Cedarow Court Commercial Development was completed on April 6, 2009 by Novatech Engineering for the proposed property. Currently, a large portion of the site is pervious, and sheet drains north west towards Poole Creek. Based on topography, existing drainage is directed through the site for properties on Cedarow Court adjacent to the subject lands. The additional runoff will be returned to the Cedarow Court storm sewer and was not included in the overall area contributing to the pre-development rate. The sewers on Cedarow Court were analyzed based on 2K mapping data corroborated by field investigation, and the additional flows were determined not to impact the downstream 525mm diameter storm sewer. The design sheet and area map for the Cedarrow Court sewer can be found in **Appendix C.6.**

The site discharge will be conveyed to the approved outlet located at the northwestern boundary of the subject site. The outlet was constructed as part of Wellings of Stittsville Inc. and Extendicare Inc. Phase 1 and was sized to convey flows from both sites. Excerpts from the Wellings of Stittsville Phase 1 servicing and stormwater management brief can be found in **Appendix C.7**.

A lumped catchment PCSWMM model was created for the subject site based on a site area of 2.3ha, and utilizing an existing SCS curve number of 82 per background documents (Carp River Full Restoration PCSWMM Model). Additional subcatchment parameters were defined based upon recent topographical survey of the property:

Area (ha)	Width (m)	Slope (%)	Imperv. (%)	Subarea Routing
2.29	143	1.0	0.0	Outlet

Based on the above, 2 through 100-year 12hr SCS event (MTO Distribution curves) peak pre-development outflow rates from the subject site were identified per the tables below:

Storm Event	Peak Outflow Rate (L/s)
2-Year	17.9
5-Year	43.4
10-Year	69.8
25-Year	111.6
50-Year	142.4
100-Year	182.1

PCSWMM model input and output files for the predevelopment scenario are included within **Appendix C.**

5.3 STORMWATER MANAGEMENT DESIGN

5.3.1 Design Methodology

The intent of the stormwater management plan presented herein is to mitigate negative impacts that the proposed development might have on the receiving watercourse (Poole Creek), while providing adequate capacity to service the proposed buildings, underground parking and access areas. The proposed stormwater management plan is designed to detain runoff on available flat rooftops, and in a subsurface storage unit to ensure that peak flows after construction will not exceed the target discharge rates.

Runoff from the site is captured via catchbasins and roof drains and conveyed to a hydrodynamic separator for water quality treatment before entering an underground storage unit for quantity control. The storage unit is restricted by an ICD at the downstream end while the roof runoff is controlled via roof drains discharging through the internal building plumbing. Eight interconnected tanks are proposed to act as subsurface storage for the development. Each tank is capable of storing up to 79m^3 (20,000 gallons) of runoff for a total allowable storage of 633 m^3. The underground storage unit is sized assuming that the entirety of the roof area is available to capture and store water up to150mm in depth during the 100-year storm event.

In case of subsurface storage tank failure, overflows are managed first via installed weir wall within STM 101 to address orifice blockage, followed by surface vents/openings at each tank in series to the surface to ensure failure of an individual tank does not cause failure of the system at large. Flows are then recaptured by the remaining tank cells in operation. Building internal pumping to the building storm outlet upstream of the hydrodynamic separator has been air-gapped with provision for overflow per OBC requirements. As such, the system is not at risk of blockage under the proposed configuration.

As the proposed invert of the hydrodynamic separator lies above anticipated downstream 100-year HGL of the subsurface storage tanks, no tailwater concerns are noted for design of the separator. The proposed hydrodynamic separator maintains an internal overflow weir for large storm events for protection of building internal plumbing, and will not impede inflow to the downstream storage tanks.

The site discharge will be conveyed to the previously approved outlet location at the western boundary of the site which ultimately directs flow into Poole Creek. The existing outlet is designed to convey flows from the proposed site as well as the existing adjacent site to the northeast, Wellings of Stittsville Inc. and Extendicare Inc Phase 1.

The site will be constructed in two phases, including build out of the underground parking structure. As the first phase is built, the entirety of the storm water storage tanks will be constructed.

5.3.2 Modeling Rationale

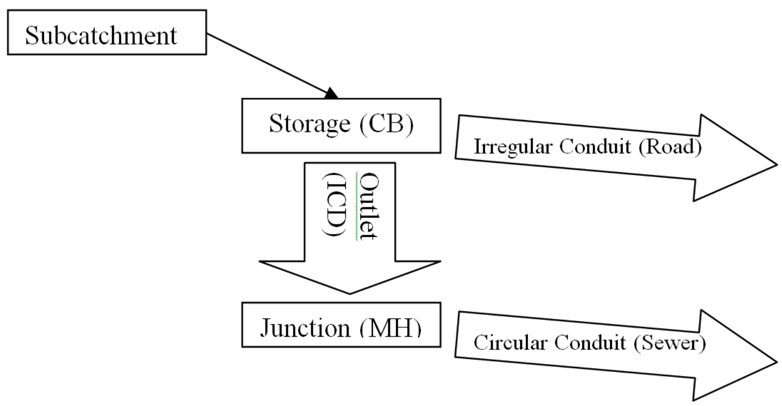
A comprehensive hydrologic modeling exercise was completed with PCSWMM, accounting for the estimated major and minor systems to evaluate the storm sewer infrastructure. The use of PCSWMM for modeling of the site hydrology and hydraulics allowed for an analysis of the systems response during various storm events. Surface storage estimates were based on the final grading plan design (see **Drawing GP-1**). The following assumptions were applied to the detailed model:

- Hydrologic parameters as per Ottawa Sewer Design Guidelines, including Horton infiltration, Manning's 'n', and depression storage values
- 12-hour SCS Storm distribution for the 100-year analysis to model 'worst-case' scenario in regards to on-site storage volume.
- 12hr SCS distributions (2 and 100-year events) with free flowing boundary condition to model 'worst-case' scenario in regards to site discharge rates to meet target rate. It is of note that the 100-Year floodplain elevation of the Creek at the site discharge point will not affect upstream HGLs or storage volumes provided.
- To 'stress test' the system a 'climate change' scenario was created by adding 20% of the individual intensity values of the 100-year SCS storm event at their specified time step.
- Percent imperviousness calculated based on actual soft and hard surfaces on each subcatchment, converted to equivalent Runoff Coefficient using the relationship C = (Imp. x 0.7) + 0.2
- Subcatchment areas are defined from high-point to high-point where sags occur. Subcatchment width (average length of overland sheet flow) determined by dividing subcatchment area by subcatchment length (length of overland flow path measured from high-point to high-point).
- Number of catchbasins based on servicing plan (**Drawing SP-1**)

5.3.2.1 SWMM Dual Drainage Methodology

The proposed site is modeled in one modeling program as a dual conduit system (see **Figure 2**), with: 1) circular conduits representing the sewers & junction nodes representing manholes; 2) irregular conduits using street-shaped cross-sections to represent the sawtoothed overland road network from high-point to low-point and storage nodes representing catchbasins. The dual drainage systems are connected via outlet link objects (or orifices) from storage node (i.e. CB) to junction (i.e. MH), and represent inlet control devices (ICDs). Subcatchments are linked to the storage node on the surface so that generated hydrographs are directed there firstly.

Figure 2: Schematic Representing Model Object Roles



Storage nodes are used in the model to represent catchbasins as well as major system junctions. For storage nodes representing catchbasins (CBs), the invert of the storage node represents the invert of the CB and the rim of the storage node is the top of the CB plus the maximum above ground storage depth (all catch basins on top of the underground structure will not have any surface storage). An additional 0.3m has been added to rim elevations to allow routing from one surface storage to the next and is unused where no spillage occurs between ponding areas.

Inlet control devices, as represented by orifice links, use a user-specified discharge coefficient to approximate manufacturer's specifications for the chosen ICD model. Discharge rates from the rooftops are based on the quantity of roof drains provided in the site plan per roof level. The roof drains are modelled using outlets with rating curves which specifies the outflows per roof level.

Subcatchment imperviousness was calculated via impervious area measured from **Drawing SSP-1**.

5.3.2.2 Boundary Conditions

The detailed PCSWMM hydrology and the proposed storm sewers were used to assess the peak inflows and hydraulic grade line (HGL) for the site. The elevation of the outlet sewer at STM 100 immediately upstream of Poole Creek has been set conservatively to be above the 100-Year water elevation of the Creek per MVCA Flood Risk Mapping at an invert elevation of 99.8m to enable free-flowing model condition for the site outlet. The elevation of the water level within Cedarow Court was conservatively set to an obvert of the receiving sewer at 102.17m.

5.3.3 Input Parameters

Drawing SD-1 summarizes the discretized subcatchments used in the analysis of the proposed site, and outlines the major overland flow paths. The grading plans are also enclosed for review.

Appendices C2 and C3 summarize the modeling input parameters and results for the subject area; an example input and output file are provided for the 100-year 12hr SCS storm. For all other input files and results of storm scenarios, please examine the electronic model files located on the CD provided with this report. This analysis was performed using PCSWMM, which is a front-end GUI to the EPA-SWMM engine. Model files can be examined in any program which can read EPA-SWMM files version 5.1.014.

5.3.3.1 Hydrologic Parameters

Table 1 presents the general subcatchment parameters used:

Table 1: General Subcatchment Parameters

Parameter	Value
Infiltration Method	Horton
Max. Infil. Rate (mm/hr)	76.2
Min. Infil. Rate (mm/hr)	13.2
Decay Constant (1/hr)	4.14
N Impervious	0.013
N Pervious	0.2
Dstore Imperv. (mm)	1.57
Dstore perv. (mm)	4.67
Zero Imperv. (%)	0

Table 2 presents the individual parameters that vary for each of the proposed subcatchments.

Table 2: Subcatchment Parameters

Name	Outlet	Area (ha)	Width (m)	Slope (%)	Imperv. (%)
EXT-1	CB509-S	0.069	95	1.5	38.6
ROOF_10	ROOF-10-S	0.281	136	1.5	100.0

ROOF_11	ROOF-11-S	0.011	21	1.5	100.0
ROOF_12	ROOF-12-S	0.013	15.6	1.5	100.0
ROOF_3	ROOF-3-S	0.110	130	1.5	100.0
ROOF_4	ROOF-4-S	0.035	46	1.5	100.0
ROOF_5	ROOF-5-S	0.110	130	1.5	100.0
ROOF_8	ROOF-8-S	0.010	21	1.5	100.0
ROOF_9	ROOF-9-S	0.005	15	1.5	100.0
ROOF1_2	ROOF-1-2-S	0.095	95	1.5	100.0
ROOF6_7	ROOF-6-7-S	0.096	95	1.5	100.0
UGPK_1	TANKS	0.144	115	2.0	77.1
UGPK_2	TANKS	0.152	122	2.0	80.0
UGPK_3	TANKS	0.060	60	2.0	58.6
UGPK_4	TANKS	0.120	95	2.0	70.0
UGPK_5	TANKS	0.110	85	2.0	70.0
UGPK_6	TANKS	0.022	60	15.0	100.0
UGPK_7	TANKS	0.112	78	2.0	78.6
UGPK_8	TANKS	0.062	42	2.0	75.7
UGPK_9	TANKS	0.032	42	2.0	100.0
UNC-1	OF1	0.078	78	2.0	41.4
UNC-2	OF2	0.515	25	1.0	8.6
UNC-3	OF3	0.069	122	2.0	61.4
UNC-4	CB509-S	0.052	90	2.0	37.1

Table 3 summarizes the storage node parameters used in the model. Storage curves for each node have been created based on available volumes within each roof top or subsurface storage as applicable. Rim elevations for each node correspond to the rim elevation of the associated area's roof top drain or catch basin plus maximum depth of storage. Catch basins located above underground parking areas flow uncontrolled to the underground storage tank and provide no quantity storage for events up to the 100-year design event. No quantity storage has been assumed for model conservatism for the water balance BMP described in **Section 5.3.6**

Storage volumes and release rates for the underground storage tank were obtained through PCSWMM hydrologic/hydraulic modeling:

Table 3: Storage Node Parameters

Name	Invert El. (m)	Rim Elev. (m)	Depth (m)	Coefficient	Exponent	Constant (m²)	Curve Name	Storage Curve
CB509-S	102.56	104.79	2.23	0	0	0	*	FUNCTIONAL
ROOF-10-S	114	114.15	0.15	0	0	0	ROOF10	TABULAR
ROOF-11-S	114	114.15	0.15	0	0	0	ROOF11	TABULAR
ROOF-12-S	114	114.15	0.15	0	0	0	ROOF12	TABULAR
ROOF-1-2-S	114	114.15	0.15	0	0	0	ROOF1and2	TABULAR
ROOF-3-S	114	114.15	0.15	0	0	0	ROOF3	TABULAR
ROOF-4-S	114	114.15	0.15	0	0	0	ROOF4	TABULAR
ROOF-5-S	114	114.15	0.15	0	0	0	ROOF5	TABULAR
ROOF-6-7-S	114	114.15	0.15	0	0	0	ROOF6and7	TABULAR
ROOF-8-S	114	114.15	0.15	0	0	0	ROOF8	TABULAR
ROOF-9-S	114	114.15	0.15	0	0	0	ROOF9	TABULAR
TANKS	99.7	103.31	3.61	0	0	222	*	FUNCTIONAL

5.3.3.2 Hydraulic Parameters

As per the Ottawa Sewer Design Guidelines (OSDG 2012), Manning's roughness values of 0.013 were used for sewer modeling.

Storm sewers were modeled to confirm flow capacities and hydraulic grade lines (HGLs) in the proposed condition. The detailed storm sewer design sheet is included in **Appendix C**.

PCSWMM output hydrographs from Phase 1 for each storm event were used at manhole structure 100 in the current PCSWMM model to accurately represent to total outflow from both properties at the headwall.

Table 4 below presents the parameters for the orifice and outlet link objects in the model, which represent ICDs and restricted roof release drains respectively. The underground storage orifice was assigned a discharge coefficient of 0.61. The tank is designed with a 75mm ICD to restrict flows during the 2-year event, as well as a weir to allow additional flows to be directed towards the outlet during larger storm events. The weir is placed in manhole structure 1000 and designed with a width of 0.5m (see **Table 4** for invert elevation).

The roof release discharge curves assume the use of standard Watts Model R1100 Accutrol controlled release roof drains as noted in the calculation sheets in **Appendix C**. The number of roof notches for each roof level was confirmed with the building mechanical engineer. Details for the IPEX ICDs and Watts drains are included as part of **Appendix C**.

Table 4: Outlet/Orifice Parameters

Name	Inlet	Outlet	Inlet Elev.	Type	Diameter (m)
CISTERN-O	TANKS	100	99.95	CIRCULAR	0.075
			102.25	WEIR	0.50
ROOF1-2-O	ROOF-1-2-S	TANKS	114	ROOF-1-2-O	-
ROOF3-O	ROOF-3-S	TANKS	114	ROOF-3-O	-
ROOF4-O	ROOF-4-S	TANKS	114	ROOF-4-O	-
ROOF5-O	ROOF-5-S	TANKS	114	ROOF-5-O	-
ROOF6-7-O	ROOF-6-7-S	TANKS	114	ROOF-6-7-O	-
ROOF8-O	ROOF-8-S	TANKS	114	ROOF-8-O	-
ROOF9-O	ROOF-9-S	TANKS	114	ROOF-9-O	-
ROOF10-O	ROOF-10-S	TANKS	114	ROOF-10-O	-
ROOF11-O	ROOF-11-S	TANKS	114	ROOF-11-O	-
ROOF12-O	ROOF-12-S	TANKS	114	ROOF-12-O	-

5.3.4 Model Results

The following section summarizes the key hydrologic and hydraulic model results. For detailed model results or inputs, please refer to the example input file in **Appendix C.2 and C.3** and the electronic model files provided.

5.3.4.1 Hydrologic Results

The following tables demonstrate the peak outflow from each modeled outfall during the design storm (12hr SCS 2-100yr) events. A free-flowing outfall condition has been modeled for these events to be conservative with respect to site peak release rates. Outfalls OF1 to OF4 denote uncontrolled flows from the perimeter of the site that, due to grading restrictions, are captured by the existing ROW on Fringewood Avenue at the eastern boundary, Poole Creek at the north boundaries of the site, Hazeldean Road to the south and Cedarow Court Row to the west. The adjacent site on the eastern boundary (2500 Wellings Private) has sufficient capacity to capture minor uncontrolled flows from subcatchment UNC1. Flows from area UNC3-OF will have a minimal contribution to the infrastructure within Hazeldean Road. Based on existing external and proposed grading, subcatchments EXT-1 and UNC-4 are proposed to drain to a swale and runoff is to be captured in the subdrain. Connection to the existing 300mm diameter storm sewer on Cedarow Court is proposed to direct the flows captured from the subdrain. The storm sewer along Cedarow Court ultimately discharges to Poole Creek upstream of the proposed site. Appendix C.6 provides calculations based of general

Results of the PCSWMM model run have been provided in **Appendix C**. Peaks from the uncontrolled flows with the exception of UNC-2 are non-coincident with peaks from the subsurface storage tank/weir, and as such, flows from the conduit downstream of the subsurface storage tank (conduit C2) and UNC-2 have been considered in meeting the site pre-development release rate target. The required subsurface storage tank volume was determined through iteration of each event and sized to mirror the site release rate target.

Table 5: Site Peak Discharge Rates

Event	Location	Discharge Rate (L/s)	Target (L/s)
2-Year 12 Hour SCS	Outlet Headwall	16.3	17.9
5-Year 12 Hour SCS	Outlet Headwall	25.6	43.4
10-Year 12 Hour SCS	Outlet Headwall	57.2	69.8
25-Year 12 Hour SCS	Outlet Headwall	90.1	111.6
50-Year 12 Hour SCS	Outlet Headwall	113.8	142.4
100-Year 12 Hour SCS	Outlet Headwall	150.3	182.1
100-Year 12 Hour SCS +20%	Outlet Headwall	308.8	-

*Post-development flows are a sum of the hydrographs from conduit C2 and outfall OF2

Table 6: Schedule of Roof Release Rates

Roof Area ID	Storage Depth (mm)	Discharge (L/s)	Required Volume (m3)	Available Volume (m3)
1 + 2	136	8.2	29.1	38.2
3	142	7.2	37.9	43.9
4	135	3.2	10.5	14.1
5	142	7.2	37.9	43.9

6 + 7	136	8.2	29.1	38.2
8	117	1.7	1.9	3.9
9	100	1.6	0.6	2.1
10	141	19.3	94.7	112.4
11	120	1.7	2.2	4.2
12	118	1.7	2.0	4.0

5.3.4.2 Hydraulic Results

The City of Ottawa requires that during major storm events, the maximum hydraulic grade line be kept at least 0.30 m below the underside-of-footing (USF) of any adjacent units connected to the storm sewer during design storm events. The USFs elevations have been considered at 0.5m below the lowest top of basement slab elevation of the proposed buildings. As the proposed building perimeter foundation drain will be disconnected from the storm sewer and pumped to the surface, the proposed building footings will not be hydraulically connected to the underground storage tank. The ramp drain is to be pumped to the storage tanks. The maximum hydraulic grade line (HGL) of the underground storage tank reaches 102.48m and 102.68m during the 100 year and 100year +20% event. The HGL elevations in both scenarios remain at least 0.30 m below the proposed surface elevations as the lowest elevation of the connected catch basins within the aboveground parking structure are at 104.18m.

Table 7 presents the maximum total surface water depths (static ponding depth + dynamic flow) above the top-of-grate of the catch basin discharging to the Cedarow Drive sewer for the 100-year design storm and climate change storm. Based on the model results, no surface ponding is anticipated within the swale/subdrain within area UNC-4.

Table 7: Maximum Surface Water Depths

Storage node ID	Structure ID	Rim Elevation (m)	100 year, 12hr SCS		100 year, 12hr SCS +20%	
			Max HGL (m)	Total Surface Water Depth (m)	Max HGL (m)	Total Surface Water Depth (m)
CB507-S	CB 507	104.44	102.74	0.00	102.78	0.00

5.3.5 Water Quality Control

On-site water quality control is required to provide 80% TSS removal prior to discharging to Poole Creek. A Stormceptor unit STC300 is proposed upstream of the underground storage tank. Runoff from roof top areas are considered clean and were assumed as pervious when calculating the total imperviousness of the contributing catchment area to the stormceptor. Design calculations for the Stormceptor indicate that the selected model will provide greater than 80% TSS removal on an annual basis. The Stormceptor unit will be privately maintained. The location and general arrangement of the Stormceptor unit is indicated on **Drawing SD-1**. Detailed sizing calculations for the Stormceptor unit are included in **Appendix C.4.**

5.3.6 Water Balance

The KWMSS and Carp River Watershed Study report identify that the site is located within a low groundwater recharge area. The Watershed Study in particular recommends a minimum of 73mm per year of infiltration (or 1171m^3/yr for the 2.29ha site) for water balance purposes and to support Poole Creek baseflow. As such, it is proposed that roof runoff from Phase 4 buildings (areas Roof-8 through Roof-12) be directed to an infiltration trench BMP composed of clear stone be located within the Poole Creek regulation limit corridor (but outside of any limit of hazard lands and top of stable slope line as determined by Paterson/MVCA) to provide baseflow to the creek during the inter-event period. The BMP is to tie in behind the orifice control for the subsurface storage tanks to allow overflow via perforated pipe for larger storm events to be controlled prior to release to the creek. Inverts of the BMP have been set to avoid high groundwater elevations and provide a minimum offset of 1.0m from anticipated bedrock elevations. Sizing of the BMP has been provided within **Appendix C.8**, and demonstrates that sufficient storage exists below the perforated pipe drain to sequester runoff from up to the 25mm storm event for connected areas, and provide up to 2675m^3 of annual infiltration.

6.0 GRADING AND DRAINAGE

The proposed development site measures approximately 2.29 ha in area. The topography across the site decreases from south to north, with a change in elevation of approximately 1.5 m to the top of bank of the existing Poole Creek. A detailed grading plan (see **Drawing GP-1**) has been provided to satisfy the stormwater management requirements, adhere to permissible grade raise restrictions (see **Section 10.0**) for the site, and provide for minimum cover requirements for storm and sanitary sewers where possible. Site grading has been established to provide emergency overland flow routes required for stormwater management in accordance with City of Ottawa requirements.

The subject site in its majority maintains emergency overland flow routes for flows deriving from storm events in excess of the maximum design event to Poole Creek as depicted in **Drawings GP-1, SD-1**.

7.0 UTILITIES

Utility infrastructure exists within the Hazeldean Road ROW at the south property boundary of the proposed site. Overhead utility poles are located along the south side of Hazeldean Road. It is anticipated that existing infrastructure will be sufficient to provide a means of distribution for the proposed site. Exact size, location and routing of utilities will be finalized after design circulation.

8.0 APPROVALS

As the site will be discharging to an existing storm sewer outlet, will remain under singular ownership, and will not drain industrial lands or industrial land uses, exemption from the Ontario Ministry of Environment, Conservation and Parks (MECP) Environmental Compliance Approval (ECA) process is expected for works within the subject site.

The outlet headwall has been previously approved under the neighboring property. The ECA application number is NUMBER 7185-ARZMHZ.

The Mississippi Valley Conservation Authority (MVCA) will need to be consulted in order to obtain municipal approval for site development, and permits acquired for any proposed fill within the Poole Creek regulatory limit.

Requirement for a MECP Permit to Take Water (PTTW) for sewer construction is unlikely for the site as the proposed works are above the groundwater elevations shown in the geotechnical report. Building excavation areas, however, will likely be within the groundwater table and may require a PTTW. The geotechnical consultant shall confirm at the time of application that a PTTW is not required.

9.0 EROSION CONTROL DURING CONSTRUCTION

Erosion and sediment controls must be in place during construction. The following recommendations to the contractor will be included in contract documents.

1. Implement best management practices to provide appropriate protection of the existing and proposed drainage system and the receiving water course(s).
2. Limit extent of exposed soils at any given time.
3. Re-vegetate exposed areas as soon as possible.
4. Minimize the area to be cleared and grubbed.
5. Protect exposed slopes with plastic or synthetic mulches.
6. Provide sediment traps and basins during dewatering.
7. Install sediment traps (such as SiltSack® by Terrafix) between catch basins and frames.
8. Plan construction at proper time to avoid flooding.
9. Installation of a mud matt to prevent mud and debris from being transported off site.
10. Installation of a silt fence to prevent sediment runoff.

The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:

11. Verification that water is not flowing under silt barriers.
12. Clean and change silt traps at catch basins.

Refer to **Drawing EC/DS-1** for the proposed location of silt fences, straw bales, and other erosion control structures.

10.0 GEOTECHNICAL INVESTIGATION

A geotechnical investigation was completed by Paterson Group Ltd. in March of 2019. The report summarizes the existing soil conditions within the subject area and construction recommendations. For details which are not summarized below, please see the original Paterson report.

Subsurface soil conditions within the subject area were determined from 29 boreholes distributed across the proposed site. In general soil stratigraphy consisted of topsoil underlain by a hard to very stiff silty clay, followed by very stiff to stiff silty clay layer over a glacial till layer.

Groundwater Levels were measured on January 29, 2019 and vary in elevation from 1.7 to 3.2m below the original ground surface. It is expected that construction occur below the existing groundwater table and therefore a permit to take water may be required as well as requirements for damp proofing or foundation waterproofing may be required.

A permissible grade raise restriction of 2.0 m has been recommended within the Paterson Group report. The grade raise restrictions were accounted for in the grading design of the property.

The required pavement structure for the at-grade parking areas and access lanes are outlined in **Table 8** and **Table 9** below:

Table 8: Recommended Pavement Structure – At-Grade Parking Areas

Thickness (mm)	Material Description
50	Wear Course – HL-3 or Superpave 12.5 Asphaltic Concrete
150	Base – OPSS Granular A Crushed Stone
300	Subbase - OPSS Granular B Type II
-	Subgrade – In situ soil, or OPSS Granular B Type I or II material placed over in situ soil

Geotechnical Investigation
July 14, 2022

Table 9: Recommended Pavement Structure – Access Lanes and Heavy Truck Parking Areas

Thickness (mm)	Material Description
40	Wear Course – HL-3 or Superpave 12.5 Asphaltic Concrete
50	Binder Course – HL-8 or Superpave 19.0 Asphaltic Concrete
150	Base – OPSS Granular A Crushed Stone
450	Subbase - OPSS Granular B Type II
-	Subgrade – In situ soil, or OPSS Granular B Type I or II material placed over in situ soil.

11.0 CONCLUSIONS

11.1 WATER SERVICING

Based on the supplied boundary conditions for existing watermains and estimated domestic and fire flow demands for the subject site, it is anticipated that the proposed servicing in this development will provide sufficient capacity to sustain both the required domestic demands and emergency fire flow demands of the proposed site. Fire flows greater than those required per the FUS Guidelines are available for this development.

11.2 SANITARY SERVICING

The proposed sanitary sewer network is sufficiently sized to provide gravity drainage of the site. The proposed site will be serviced by a gravity sewer which will direct the wastewater flows (approx. 10.4 L/s) to the existing 675mm dia. Hazeldean Road sanitary sewer. The proposed drainage pattern is in accordance with the Kanata West Master Servicing Report for the Hazeldean Road sewer.

11.3 STORMWATER SERVICING

The proposed stormwater management plan is in compliance with the criteria established for the site. Rooftop and subsurface storage have been designed to limit outflows from the subject site to calculated predevelopment levels. Poole Creek is located downstream of the site and has sufficient capacity to receive runoff volumes from the site based on anticipated peak flows and detention times for the subsurface storage tank servicing the development.

11.4 GRADING

Grading for the site has been designed to provide an emergency overland flow route as per City requirements and reflects the grade raise restrictions recommended in the Supplemental Geotechnical Investigation prepared by Paterson Group (March, 2019) . Erosion and sediment control measures will be implemented during construction to reduce the impact on existing facilities.

11.5 UTILITIES

Utility infrastructure exists within the Hazeldean Road ROW at the south property boundary of the proposed site. Overhead poles are located along the south side of Hazeldean Road. It is anticipated that existing infrastructure will be sufficient to provide a means of distribution for the proposed site. Exact size, location and routing of utilities will be finalized after design circulation.

11.6 APPROVALS/PERMITS

MECP Environmental Compliance Approval is not expected to be required for the proposed site works. A Permit to Take Water is not anticipated to be required for pumping requirements for sewer installation, however, will likely be a requirement for building excavation. The Mississippi Valley Conservation Authority will need to be consulted in order to obtain municipal approval for site development. No other approval requirements from other regulatory agencies are anticipated.

Appendix A WATER SUPPLY SERVICING

A.1 DOMESTIC WATER DEMAND ESTIMATE

Wellings of Stittsville Phase 2 - 20 Cedarow Court - Domestic Water Demand Estimates

- Based on Wellings of Stittsville Site Phase 2 (160401511)

Building ID	Area (m^2)	Population	Daily Rate of Demand [1]	Avg Day Demand (L/min)	Avg Day Demand (L/s)	Max Day Demand [2,3] (L/min)	Max Day Demand [2,3] (L/s)	Peak Hour Demand [2,3] (L/min)	Peak Hour Demand [2,3] (L/s)
Phase 2 and Phase 3									
Residential	-	441	350	107.2	1.79	268.0	4.47	589.5	9.83
Commercial and communal Amenity Areas	4726	-	28,000	9.2	0.15	13.8	0.23	24.8	0.41
Phase 4									
Residential	-	312	350	75.9	1.26	189.7	3.16	417.4	6.96
Total Site :				**192.3**	**3.20**	**471.5**	**7.86**	**1031.7**	**17.19**

1. 28,000 L/gross ha/day is used to calculate water demand for retail, restaurants and office space.
2. The City of Ottawa water demand criteria used to estimate peak demand rates for commercial space are as follows:
   maximum day demand rate = 1.5 x average day demand rate
   maximum hour demand rate = 1.8 x maximum day demand rate
3. The City of Ottaw water demand criteria used to estimate peak demand rates for residential areas are as follows:
   maximum day demand rate = 2.5 x average day demand rate
   maximum hour demand rate = 2.2 x maximum day demand rate

A.2 FIRE FLOW REQUIREMENTS PER FUS

FUS Fire Flow Calculation Sheet

Stantec

Stantec Project #: 160401317
Project Name: 20 Cedarow Court
Date: 9/1/2021
Fire Flow Calculation #: 1
Description: Phase 2 and 3

Notes: 6 storey building with 2hr horizontal firewalls between each floor

Step	Task	Notes						Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction	Non-Combustible Construction						0.8	-
2	Determine Ground Floor Area of One Unit	-						4456	-
	Determine Number of Adjoining Units	-						1	-
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space						1	-
4	Determine Required Fire Flow	($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min						-	12000
5	Determine Occupancy Charge	Limited Combustible						-15%	10200
6	Determine Sprinkler Reduction	Conforms to NFPA 13						-30%	-4080
		Standard Water Supply						-10%	
		Not Fully Supervised or N/A						0%	
		% Coverage of Sprinkler System						100%	
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	10.1 to 20	30	6	> 120	Wood Frame or Non-Combustible	15%	3978
		East	20.1 to 30	82	5	> 120	Wood Frame or Non-Combustible	10%	
		South	> 45	123	1	> 120	Wood Frame or Non-Combustible	0%	
		West	10.1 to 20	82	1	61-90	Wood Frame or Non-Combustible	14%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							10000
		Total Required Fire Flow in L/s							166.7
		Required Duration of Fire Flow (hrs)							2.00
		Required Volume of Fire Flow (m^3)							1200

FUS Fire Flow Calculation Sheet

Stantec Project #: 160401317
Project Name: 20 Cedarow Court
Date: 9/1/2021
Fire Flow Calculation #: 1
Description: Phase 4

Notes: 6 storey building with2hr horizontal firewalls between each floor

<table>
<tr><th>Step</th><th>Task</th><th colspan="6">Notes</th><th>Value Used</th><th>Req'd Fire Flow (L/min)</th></tr>
<tr><td>1</td><td>Determine Type of Construction</td><td colspan="6">Non-Combustible Construction</td><td>0.8</td><td>-</td></tr>
<tr><td rowspan="2">2</td><td>Determine Ground Floor Area of One Unit</td><td colspan="6">-</td><td>3192</td><td>-</td></tr>
<tr><td>Determine Number of Adjoining Units</td><td colspan="6">-</td><td>1</td><td>-</td></tr>
<tr><td>3</td><td>Determine Height in Storeys</td><td colspan="6">Does not include floors >50% below grade or open attic space</td><td>1</td><td>-</td></tr>
<tr><td>4</td><td>Determine Required Fire Flow</td><td colspan="6">($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min</td><td>-</td><td>10000</td></tr>
<tr><td>5</td><td>Determine Occupancy Charge</td><td colspan="6">Limited Combustible</td><td>-15%</td><td>8500</td></tr>
<tr><td rowspan="4">6</td><td rowspan="4">Determine Sprinkler Reduction</td><td colspan="6">Conforms to NFPA 13</td><td>-30%</td><td rowspan="4">-3400</td></tr>
<tr><td colspan="6">Standard Water Supply</td><td>-10%</td></tr>
<tr><td colspan="6">Not Fully Supervised or N/A</td><td>0%</td></tr>
<tr><td colspan="6">% Coverage of Sprinkler System</td><td>100%</td></tr>
<tr><td rowspan="5">7</td><td rowspan="5">Determine Increase for Exposures (Max. 75%)</td><td>Direction</td><td>Exposure Distance (m)</td><td>Exposed Length (m)</td><td>Exposed Height (Stories)</td><td>Length-Height Factor (m x stories)</td><td>Construction of Adjacent Wall</td><td>-</td><td>-</td></tr>
<tr><td>North</td><td>> 45</td><td>122</td><td>1</td><td>> 120</td><td>Wood Frame or Non-Combustible</td><td>0%</td><td rowspan="4">2125</td></tr>
<tr><td>East</td><td>30.1 to 45</td><td>54</td><td>5</td><td>> 120</td><td>Wood Frame or Non-Combustible</td><td>5%</td></tr>
<tr><td>South</td><td>10.1 to 20</td><td>122</td><td>6</td><td>> 120</td><td>Wood Frame or Non-Combustible</td><td>15%</td></tr>
<tr><td>West</td><td>30.1 to 45</td><td>28</td><td>1</td><td>0-30</td><td>Wood Frame or Non-Combustible</td><td>5%</td></tr>
<tr><td rowspan="4">8</td><td rowspan="4">Determine Final Required Fire Flow</td><td colspan="7">Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min</td><td>7000</td></tr>
<tr><td colspan="7">Total Required Fire Flow in L/s</td><td>116.7</td></tr>
<tr><td colspan="7">Required Duration of Fire Flow (hrs)</td><td>2.00</td></tr>
<tr><td colspan="7">Required Volume of Fire Flow (m^3)</td><td>840</td></tr>
</table>

A.3 BOUNDRY CONDITIONS

Boundary Conditions - 20 Cedarow Court

Date Provided October-19

Scenario	Demand	
	L/min	L/s
Average Daily Demand	156	2.60
Maximum Daily Demand	388	6.46
Peak Hour	850	14.17
Fire Flow Demand #1	16,020	267

# of connections 2

Location:

Results:

Connection 1 - Cedarow Crescent

Demand Scenario	Head (m)	Pressure[1] (psi)
Maximum HGL	161.1	80.3
Peak Hour	157.7	75.5
Max Day plus Fire 1	150.2	64.8

[1] Ground Elevation = 104.6m

Connection 2 - Wellings Pvt

Demand Scenario	Head (m)	Pressure[1] (psi)
Maximum HGL	161.1	80.3
Peak Hour	157.7	75.4
Max Day plus Fire 1	149.6	63.9

[1] Ground Elevation = 104.7m

Notes:

1. Pressure reducing valve is required since the maximum pressure exceeds 80 psi.
2. Looping of the watermain is required to decrease vulnerability of the water system in case of breaks.
3. Confirm the ownership of the watermain on Wellings Private.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Appendix B WASTEWATER SERVICING

B.1 SANITARY SEWER DESIGN

SUBDIVISION:

Nautical Lands Group - Wellings of Stittsville Senior's Living and Extendicare L.T.C.

DATE:	8/31/2021
REVISION:	2
DESIGNED BY:	TR
CHECKED BY:	DT

SANITARY SEWER DESIGN SHEET

(City of Ottawa)

FILE NUMBER: 1604-01511

DESIGN PARAMETERS

MAX PEAK FACTOR (RES.)=	4.0	AVG. DAILY FLOW / PERSON	280 L/p/day	MINIMUM VELOCITY	0.60 m/s
MIN PEAK FACTOR (RES.)=	2.0	COMMERCIAL	28,000 L/ha/day	MAXIMUM VELOCITY	3.00 m/s
PEAKING FACTOR (INDUSTRIAL):	2.4	INDUSTRIAL (HEAVY)	55,000 L/ha/day	MANNINGS n	0.013
PEAKING FACTOR (COMM., INST.):	1.5	INDUSTRIAL (LIGHT)	35,000 L/ha/day	BEDDING CLASS	B
STUDIO APARTMENT	1.4	INSTITUTIONAL	28,000 L/ha/day	MINIMUM COVER	2.50 m
1 BEDROOM	1.4	INFILTRATION	0.33 L/s/ha		
2 BEDROOM	2.1				

LOCATION			RESIDENTIAL AREA AND POPULATION									
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (ha)	Studio	Single 1 Bedroom Units	2 Bedroom	POP.	CUMULATIVE AREA (ha)	CUMULATIVE POP.	PEAK FACT.	PEAK FLOW (L/s)
Wellings of Stittsville Ph2											
ENTIRE SITE	STUB	MAIN	1.82	0	376	108	753	1.82	753	3.88	9.5

AREA ID NUMBER	COMMERCIAL AREA (ha)	COMMERCIAL ACCU. AREA (ha)	INDUSTRIAL (L) AREA (ha)	INDUSTRIAL (L) ACCU. AREA (ha)	INDUSTRIAL (H) AREA (ha)	INDUSTRIAL (H) ACCU. AREA (ha)	INSTITUTIONAL AREA (ha)	INSTITUTIONAL ACCU. AREA (ha)	GREEN / UNUSED AREA (ha)	GREEN / UNUSED ACCU. AREA (ha)	C+I+I PEAK FLOW (L/s)
ENTIRE SITE	0.47	0.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.2

AREA ID NUMBER	INFILTRATION TOTAL AREA (ha)	INFILTRATION ACCU. AREA (ha)	INFILT. FLOW (L/s)	TOTAL FLOW (L/s)	PIPE LENGTH (m)	DIA (mm)	MATERIAL	CLASS	SLOPE (%)	CAP. (FULL) (l/s)	CAP. V PEAK FLOW (%)	VEL. (FULL) (m/s)	VEL. (ACT.) (m/s)
ENTIRE SITE	2.29	2.29	0.8	10.4	23.1	300 675	PVC	SDR 35	0.40	60.7	**17.20%**	0.86	0.53

From:	Tousignant, Eric
To:	Thiffault, Dustin
Cc:	Moroz, Peter
Subject:	RE: Confirmation of Sanitary Capacity - 20 Cedarow Court
Date:	Tuesday, July 12, 2022 10:03:53 AM

Hi Dustin

I don't see a problem with this additional flow, there is capacity in the sanitary sewer system.

Regards
Eric

Eric Tousignant, P.Eng.
Senior Water Resources Engineer/ Ingénieur principal en resources hydriques
Infrastructure and Water Services / *services d'infrastructure et d'eau*
613-580-2424 ext 25129

Vacation Notice : Note that I will be away on vacation from July 25th to August 12, but will be checking emails periodically to forward them to appropriate staff.

From: Thiffault, Dustin <Dustin.Thiffault@stantec.com>
Sent: July 05, 2022 4:44 PM
To: Tousignant, Eric <Eric.Tousignant@ottawa.ca>
Cc: Moroz, Peter <peter.moroz@stantec.com>
Subject: Confirmation of Sanitary Capacity - 20 Cedarow Court

Hi Eric,

We are working on a site plan control application for a residential development on 20 Cedarow Court set to have sanitary discharge to the 675mm trunk sewer within Hazeldean Road just west of Huntmar. The KWMSS had previously assumed the site to be entirely commercial, and so development review is asking for confirmation that the trunk can accept the increase in expected flows.

The KWMSS had estimated peak flows from the development at approx. 2.0L/s (under the older discharge parameters), whereas we are anticipating approximately 10.4L/s including allowance for infiltration. Would you be able to confirm that the 10.4L/s rate can be sufficiently accommodated within the Hazeldean sewer? I've attached a servicing drawing for your reference.

Thanks very much for your help!

Dustin Thiffault P.Eng.
Project Engineer

Mobile: 343-996-2211
dustin.thiffault@stantec.com

Stantec
300-1331 Clyde Avenue
Ottawa ON K2C 3G4

B.2 SANITARY EXCERPTS FROM THE KWMSS

4.0 SANITARY SEWER SERVICING

4.1 Introduction

This section outlines the evaluation criteria for wastewater servicing options, describes the alternative wastewater servicing alignments, summarizes the evaluation process, and compares the recommended alternatives to select the preferred option.

4.2 Evaluation Criteria and Weightings

The evaluation of alternatives is based, in part, on criteria previously developed for the Regional Master Plan for Water, Wastewater and Transportation, which can be found in Volume 2 of the "Planning and Environmental Assessment Summary Report" prepared by the former Region of Ottawa-Carleton.

The criteria are divided into four categories. The first three categories consider environmental, social, and economic impacts of the project on the Study area. The fourth category (Constructability/Functionality) considers project-specific criteria assessing the technical aspects and impacts of the project on the Study area. A list of each criteria and its respective category, as well as an explanation of their indicators, is provided in **Table 4.1-1**.

TABLE 4.1-1
Evaluation Criteria

Category	Criteria	Indicator
Constructability/Functionality		
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.
CO1.2	Infrastructure Requirements	Extent of works required.
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.
CO1.4	Construction Scheduling	Impact of construction on development timing/phasing.
CO1.5	Property Acquisition	Ease of property acquisition. Depends on status of required and adjacent lands (i.e. vacant, leased or owner occupied).
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow.
CO1.7	System Flexibility	Ease of accommodating potential changes in servicing plans.
Economy		
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.
E2	Efficiency of use of existing infrastructure	Use of existing capacity.
E3	Energy consumption	Pumping requirements.
E5	Impact on Agriculture	Agricultural area likely to be affected by infrastructure.

E9	Construction Cost	Estimated construction cost.
Caring and Healthy Community		
C3	Displacement of Residents, Community/Recreation Features and Institutions	Affects on residential areas, institutions or businesses.
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses.
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Transportation Master Plan Report and Storm Sewer and Watermain Needs).
Natural Environment		
N1	Impact on Significant Natural Features	Loss of natural areas due to installation of works.
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pumping station failure.
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.
N6	Effects on Urban Green Space, Open Space and Vegetation	Disruption to green space and trees.

4.2.1 Description of Evaluation Categories

Presented below is a description of the categories used to assess each of the three servicing alternatives. The four categories were selected to ensure that the various servicing alternatives were evaluated in a consistent and comprehensive manner. Further details on the criteria and weightings for each category are provided in Appendix 2.1.

Constructability/Functionality (C/F) – 36%

Wastewater infrastructure is required to facilitate the development of Kanata West. The infrastructure needs to provide a flexible servicing solution to accommodate the orderly development of the entire area in a series of phases. It is important that the construction of the wastewater servicing be coordinated with other major infrastructure projects such as storm sewers, waterwain and transportation, to ensure all services are available when required. Various alignment alternatives, construction techniques and phasing options will be assessed.

Economy (E) – 25 %

The Kanata West area is recognized within the City of Ottawa Official Plan as a future growth area comprised of a mixture of residential, business, and commercial lands. The accelerated rate of development and design concerns within the Study area requires a cost-effective solution to providing municipal wastewater services.

Caring and Healthy Communities (CHC) – 25 %

Impact to the surrounding community is an important factor when determining the preferred servicing alternative. The selected alignment and construction techniques are evaluated to minimize disruption to surrounding communities. It is anticipated that impacts will be limited to the time of construction for the off-site servicing.

Natural Environment (NE) – 14%

The majority of the required wastewater infrastructure is aligned within existing or proposed public roads to limit the impacts to the natural environment. Servicing alignments selected outside of roadways were chosen to minimize impacts where possible. Construction of the wastewater services will be performed in conjunction with other servicing projects required as part of the overall development. Further information on the environmental impacts of the proposed road allowances are documented in the Kanata West Transportation Master Plan Report.

In the rare event that the pump station overflows, impacts to surface water quality are anticipated to be minimal. All discharges from the overflow will be directed to the stormwater management pond where they will be collected. Increases in CO_2 emissions from the emergency diesel generators during power failures or maintenance procedures will be negligible.

4.2.2 Outlet Alternatives

4.2.2.1 Description of Outlet Alternatives

To provide an adequate outlet for the KWCP wastewater system three servicing options were evaluated. Each of these options will ultimately discharge to the Tri Township Collector Sewer. The first servicing option utilizes a gravity sewer (tunnel), the remaining two options make use of a pumping station located at the intersection of Maple Grove Road and Silver Seven Road, with alternate forcemain alignments. **Figures 4.1-1, 4.1-2 and 4.1-3** illustrate the alternative outlet alignments, which are further described below:

- Alternative I (Gravity Outlet) – A gravity sewer (tunnel) along the Highway 417 corridor to the Tri Township Collector. The tunnel would be constructed within the existing road allowance, adjacent to the travel lanes. The alignment crosses Highway 417 east of Eagleson Road and parallels the Glen Cairn Collector. Refer to **Figure 4.1-1**.
- Alternative II (Forcemain Alignment 1) - A forcemain along the Highway 417 corridor from a proposed pumping station on Maple Grove Road, extending to the Glen Cairn Collector Sewer east of Eagleson Road. Refer to **Figure 4.1-2**.
- Alternative III (Forcemain Alignment 2) - A forcemain along Katimavik Road and Palladium Drive from a proposed Pumping Station on Maple Grove Road to the Glen Cairn Collector Sewer east of Eagleson Road. Refer to **Figure 4.1-3**.

4.2.2.2 Evaluation of Outlet Alternatives

Evaluation of the criteria was completed using the "standard pair-wise comparison" methodology. The weightings assigned to each of the criteria were selected based on the weightings applied for past similar projects, knowledge of environmental constraints, community

concerns and professional judgment. The scores for each category and criterion were summed to determine the overall category weighting. Evaluation results are summarized in **Table 4.1-2**. An explanation of the category rankings and weightings are provided below.

Constructability/Functionality (C/F) (36%)

A review of the three proposed servicing options indicates that the forcemain alternatives present fewer issues with respect to the geotechnical constraints when compared to the gravity sewer alternative. The forcemain alternatives would require a relatively shallow excavation, reducing the conflict with the shallow bedrock formations that exist along each forcemain alignment. The shallow depth of the forcemains would also minimize the technical difficulties arising from earth to rock transitions along the trench. The effort required to install either of the forcemain alternatives would be much less than the gravity outlet alternative because the need to tunnel would be eliminated.

When comparing the two forcemain alternatives, an obvious benefit of Alternative II is its location along Katimavik Road as compared to the location of Alternative III, along Highway 417. Katimavik Road has a lower classification than Highway 417, reducing traffic management issues during construction and routine maintenance operations. The central location of the Alternative II forcemain alignment in relation to the area to be serviced also improves the flexibility for developing internal servicing options. The various alignments available for Alternative II, west of Terry Fox Drive (see **Figure 4.1-3**), are all located within existing road allowances and are considered equal when evaluated with the prescribed criteria. The Alternative II alignment along Silver Seven Road also allows the opportunity for the services to be installed in an easement located immediately adjacent to the east side of the right-of-way. Construction in this easement would eliminate the need to reconstruct this portion of the road. The use of easements for construction of the necessary services was not factored into the evaluation criteria and therefore the ranking was not affected.

Economy (E) (25%)

The costs of both forcemain alternatives are similar and much less expensive than the gravity sewer alternative. The increased costs of the gravity sewer are attributed to the need to tunnel through the existing bedrock. The forcemain alternatives allow for a relatively shallow excavation over the entire length of the alignment. The level of effort required to construct the gravity sewer would also be significantly greater than the effort required to install either of the forcemain alternatives.

Caring and Healthy Community (CHC) (25%)

Both the gravity outlet and the Alternative I forcemain would have minimal impact on the community given that the majority of the work would occur within the Highway 417 road allowance. The Alignment II forcemain alignment along Katimavik Road would require open cut excavation and would have a temporary impact on the residents during construction.

GRAVITY SANITARY OUTLET LOCATION

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FIG. 4.1-1

SANITARY FORCEMAIN OUTLET ALIGNMENT 1 HWY 417

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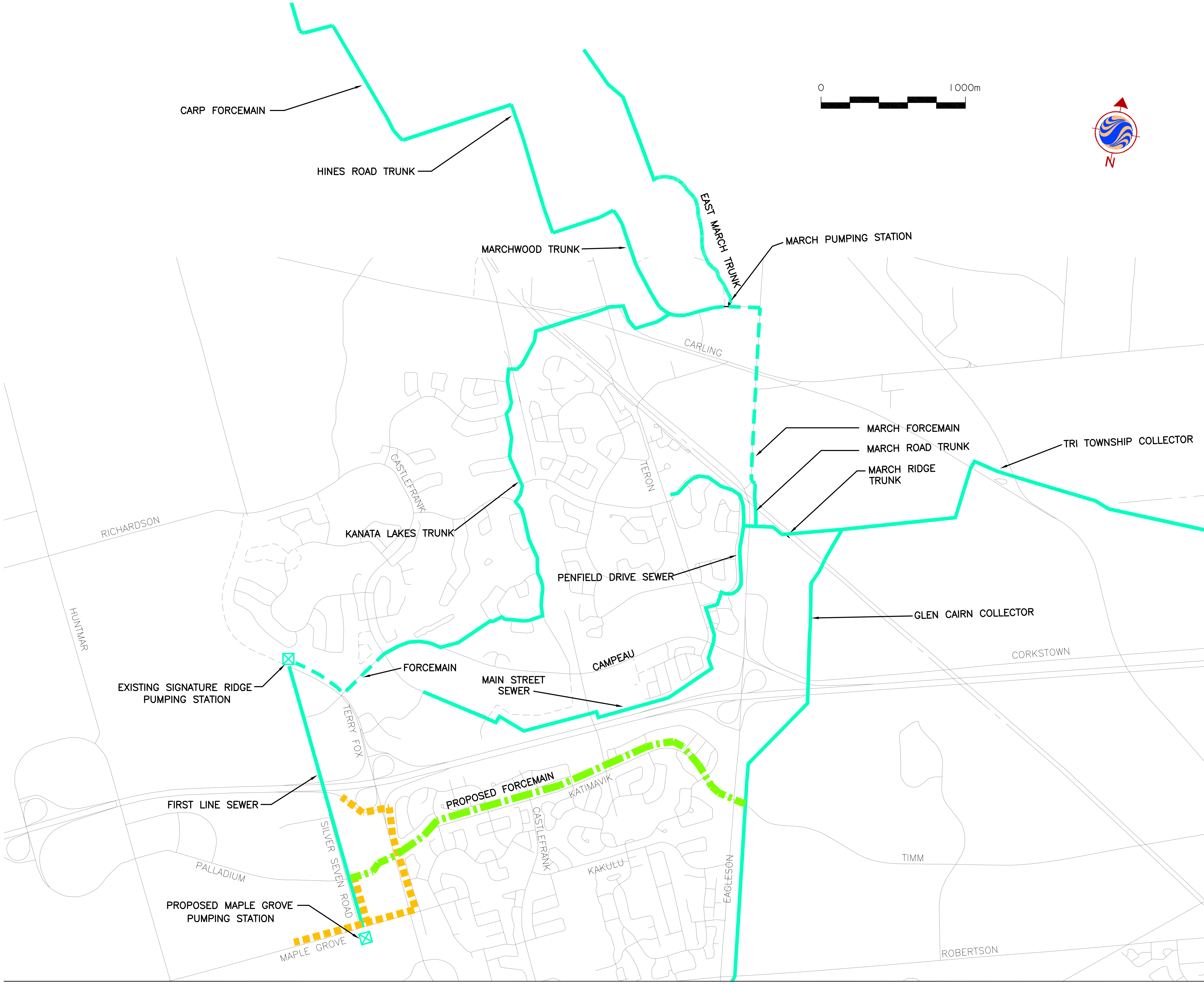
FIG. 4.1-2



SANITARY FORCEMAIN OUTLET ALIGNMENT 2 KATIMAVIK RD.

(PREFERRED OPTION)

Legend:

- Forcemain
- Optional F.M. locations to accomodate internal servicing
- Existing Trunk Sewer

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FIG. 4.1-3

The construction of both forcemain alternatives is compatible with existing City design standards and construction practices. However, only Alignment II can easily be integrated into other servicing or roadway improvements. The time required for the construction of the gravity outlet would be significantly longer than that of the forcemain alternatives.

Natural Environment (NE) (14%)

There are no significant differences on the impacts to the natural environment between the gravity outlet and forcemain Alternative II. The gravity outlet will be tunneled below ground for the majority of the alignment resulting in minimal impact to surface conditions. Forcemain Alternative II is located within the Katimavik Road allowance, which is already developed and has minimal environmental impact. Forcemain Alternative I has a greater impact on the natural areas located along the Highway 417 corridor then the gravity sewer.

4.2.2.3 Selection of Preferred Outlet Alternative

Based on the above evaluation Alternative II, the Katimavik Road alignment, is selected as the preferred outlet alternative. This alignment offers the greatest amount of flexibility for internal servicing design, uses a lower road classification corridor, which simplifies routine maintenance operations, and provides maximum separation from the sensitive natural areas located in the 417 corridor east of Terry Fox Drive.

While Forcemain Alignment II has a marginal cost increase over Alignment I, the benefits of improved internal servicing and phasing options more than offset this discrepancy.

4.2.3 Internal Servicing Alternatives

4.2.3.1 Description of Internal Servicing Alternatives

The preliminary servicing report prepared in support of the approved Community Design Plan identified the need for two pumping stations for the wastewater discharge from KWCP. The two stations identified are required to satisfy phasing needs for construction of the overall development area and to produce a cost effective initial phasing scheme. The new sanitary pumping station(s) south of Highway 417 will be required to provide internal wastewater service to that portion of KWCP south of Highway 417.

Three potential servicing alternatives were considered for the configuration and location for the pumping station(s) required to service these lands south of Hwy. 417. Internal servicing alternatives were chosen based on their proximity to the preferred outlet described in Section 4.2.3.3. above, and accessibility to the servicing areas as illustrated in **Figures 4.1-4, 4.1-5, 4.1-6 and 4.1-6A**. A brief description of the alternative pumping station locations are as follows:

- Alternative I - Two pumping stations connected with a combination of gravity sewer and forcemain. One pumping station will be located on Silver Seven Road at Highway 417. The second station will be located on Maple Grove Road at the Carp River. Refer to **Figure 4.1-4**.
- Alternative II - Two pumping stations connected with a gravity sewer. One station will be located on Maple Grove Road near the Carp River and will discharge to the main station located near the Carp River south of Highway 417. A diversion sewer will also be required to intercept the existing Silver Seven Road sanitary sewer. Refer to **Figure 4.1-5**.

TABLE 4.1-2

Kanata West Wastewater - Outlet Alternatives

Criteria		Indicators	Weighting	Rationale for Relative Weights	Alternatives		
					Gravity Sewer Outlet	PS FM Alignment I	PS FM Alignment II
CONSTRUCTABILITY/FUNCTIONALITY			**36%**		16	20	22
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.	7%	Alt. I has potential for poor soils conditions due to depth and tunnelling in and out of rock.	2	3	3
CO1.2	Infrastructure Requirements	Extent of works required.	7%	Alt. I with tunnelling is a very large scale operation.	1	3	3
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.	6%	Alt. II and III require more extensive maintenance due to pumping.	3	2	2
CO1.4	Construction Scheduling	Impact of construction on development timing.	4%	Alt. I with tunnelling is an extended construction schedule.	1	3	3
CO1.5	Property Acquisition	Ease of property acquisition. (Depends on status of lands and adjacent lands, i.e. vacant, leased or owner occupied.)	2%		4	4	4
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow	6%		3	3	3
CO1.7	Servicing Flexibility	Ease of accommodating potential changes in servicing plans.	5%	Alt. I and II have fixed alignments along the north limit of the servicing area. Alt. II has some flexibility to be realigned within the development area, but Alt. III due to its more central location has maximum flexibility within Kanata West.	2	2	4
ECONOMY			**25%**		9	13	15
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.	6%	Alt. I with the requirement for tunnelling does not offer any potential to use combined corridors.	1	2	3
E2	Efficiency of Use of Existing Infrastructure	Use of exisitng capacity	5%	Alt. I requires reconstruction beyond the closest connection point to the Glen Cairn Collector sewer.	1	3	4
E3	Energy Consumption	Pumping requirements	4%	Alt. II & III require pumping.	3	2	2
E5	Impact on Agriculture	Agriculture area likely to be affected by infrastructure.	2%		3	3	3
E9	Capital Cost	Estimated cost of construction.	8%	Alt. I is very expensive due to the tunnelling requirement.	1	3	3
CARING AND HEALTHY COMMUNITIES			**25%**		7	9	9
C3	Displacement of Residents, Community/Recreation Features and Institutions.	Affects areas of residence, institutions or businesses.	6%	Length of Construction for Alt. I will result in increasedconstruction traffic, etc.	3	3	3
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses and visibility of additional infrastructure.	11%		3	3	3
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Roadwork Environmental Study Report and Storm Sewer and Watermain Needs).	8%	Alt. I would provide service for larger area than the existing urban boundary due to size of pipe required to tunnel. Alt. II provides greater flexibility for internal servicing.	1	3	3
NATURAL ENVIRONMENT			**14%**		16	12	14
N1	Impact on Significant Natural Features	Loss of natural area due to installation of works.	3%	Alt. I mostly tunnel therefore minimal impact. Alt. II in vicinity of ANSI in 417 corridor at Terry Fox.	4	1	3
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.	3%		3	3	3
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pump station failure.	3%		3	3	3
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.	1%	Alt. II and III require pumping in long term. Alt. I does not.	3	2	2
N6	Effects on Urban Greenspace, Open Space and Vegetation (i.e.trees,shrubs,etc.)	Disruption to greenspace and trees.	5%		3	3	3
Total Score			**100%**		2.17	2.75	3.01
Ranking					3	2	1
Estimated Capital Cost (in $million)					30	8.8	9

Description of Alternatives
Gravity Sewer Outlet
Pump Station - Forcemain Alignment I - HWY 417
Pump Station - Forcemain Alignment II - Katimavik Rd.

Evaluation Ranking
1 -2 High or Negative Impact
3 Moderate or No Impact
4-5 Low or Positive Impact

- Alternatives III and IIIA – Alternative III is a single pumping station with a gravity sewer intercepting the existing Silver Seven Road sanitary sewer. The gravity sewer alignment will be adjacent to the Carp River and connect to the pump station located at Maple Grove Road west of the Carp River. Alternative IIIA is a variation of this alternative utilizing a single pumping station and gravity sewer intercepting the existing Silver Seven

  Road sewer. The variation from Alternative III is that the gravity sewer will be located within a proposed road right-of-way, or an easement, north of Palladium Drive. Refer to **Figures 4.1-6 and 4.1-6A.**

4.2.3.2 Evaluation of Internal Servicing Alternatives

The alternative internal servicing alignments were evaluated as discussed in Section 4.2. The results of the evaluation are summarized in **Table 4.1-3**. An explanation of the category rankings and weightings are provided below.

Constructability/Functionality (C/F) (36%)

All proposed alternatives use pumping stations to provide internal wastewater servicing. The use of pumps allows the sewer system to be constructed at a relatively shallow depth. This reduces the potential for contact with poor subsurface conditions during construction. Deep Excavations will be confined to a limited area in the vicinity of the pumping station.

A benefit of Alternatives III and IIIA is that a single pumping station is required to provide the internal servicing. This is advantageous from a constructability and operational point of view when compared to Alternatives I and II which require two pumping stations to service the same area. A further benefit of Alternatives III and IIIA is that either servicing scenario will eliminate the need for the existing Palladium siphon under the Carp River. Removing the siphon will improve the overall reliability to the system.

A benefit of Alternative IIIA over Alternative III is that work in the Carp River corridor is reduced to a single crossing at Palladium Drive. Both Alternatives are close to a stormwater management pond which can be used as an emergency overflow in the rare event of flooding. (see **Figures 4.1-6 and 4.1-6A**)

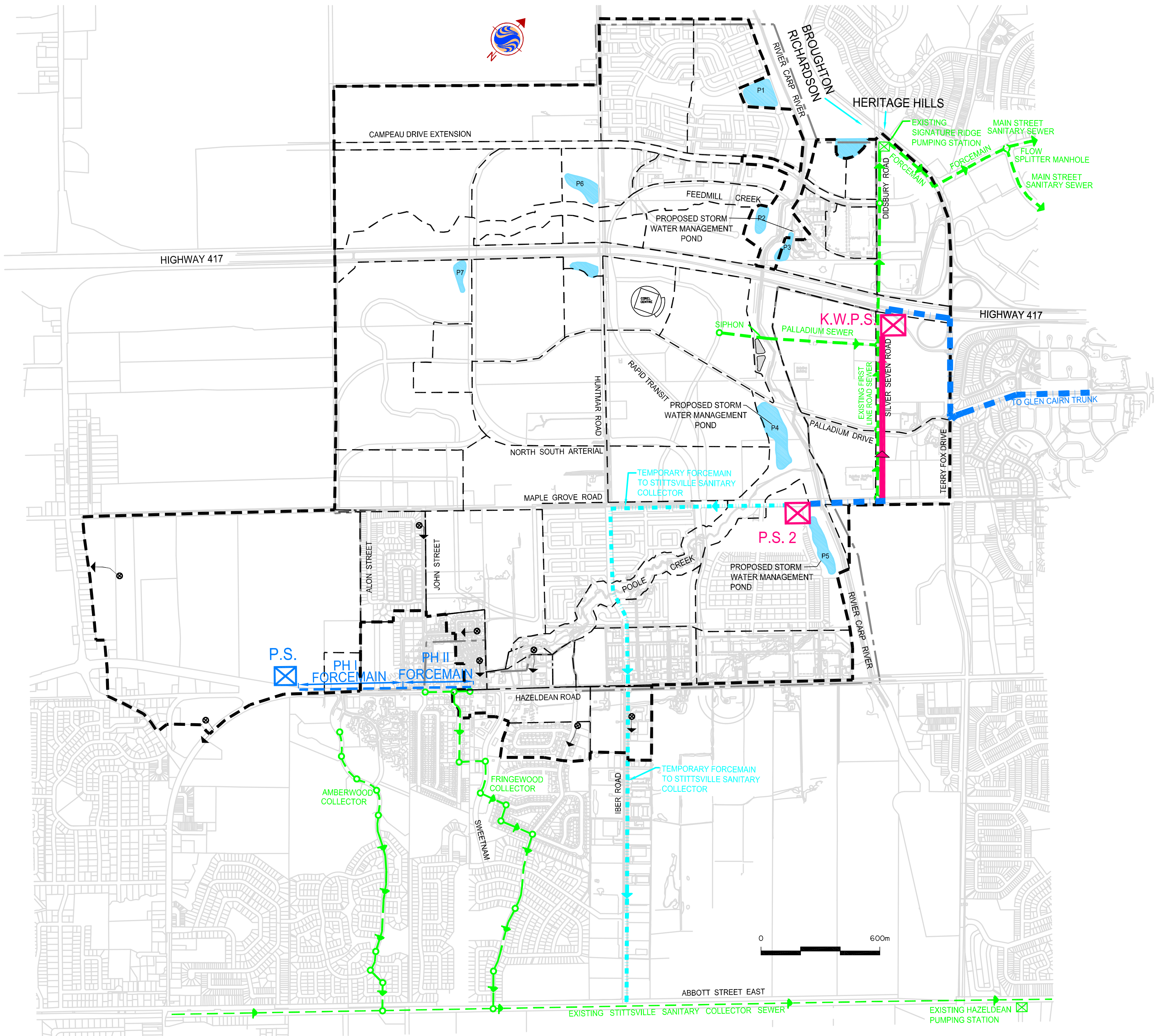
All alternatives are capable of satisfying a phased development process.

Economy (E) (25%)

Alternatives I and II use two pumping stations and are significantly more expensive than Alternatives III and IIIA which use a single pump station. The additional pump stations in Alternatives I and II also increase the energy demand over the remaining options. Alternatives III and IIIA are able to service the entire KWCP with a single pump station resulting in equal or fewer economic impacts.

Caring and Healthy Community (CHC) (25%)

In terms of impact on the community, there are no differences between the alternatives. All options require construction in the vicinity of existing businesses. Impacts are anticipated to be relatively short in duration (less than two years).



INTERNAL SANITARY SERVICING ALTERNATIVE I

Ultimate Major Drainage Limit

Proposed Trunk Sewer

Forcemain

Temporary Frocemain

Existing Trunk Sewers

Existing Pumping Station and Forcemain (To be Decommissioned)

Pump Station

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FIG. 4.1-4

INTERNAL SANITARY SERVICING ALTERNATIVE II

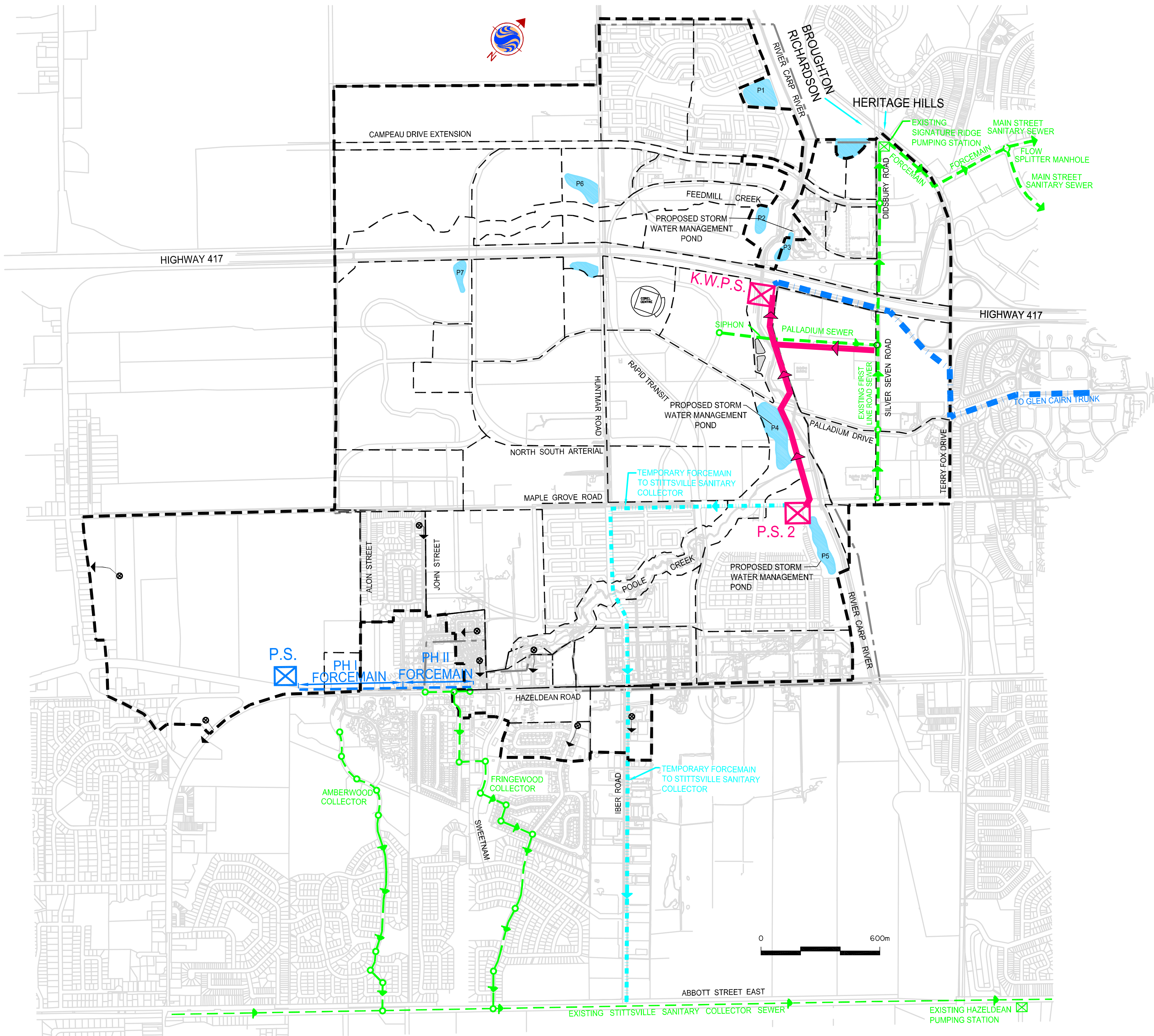

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FIG. 4.1-5

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INTERNAL SANITARY SERVICING ALTERNATIVE III

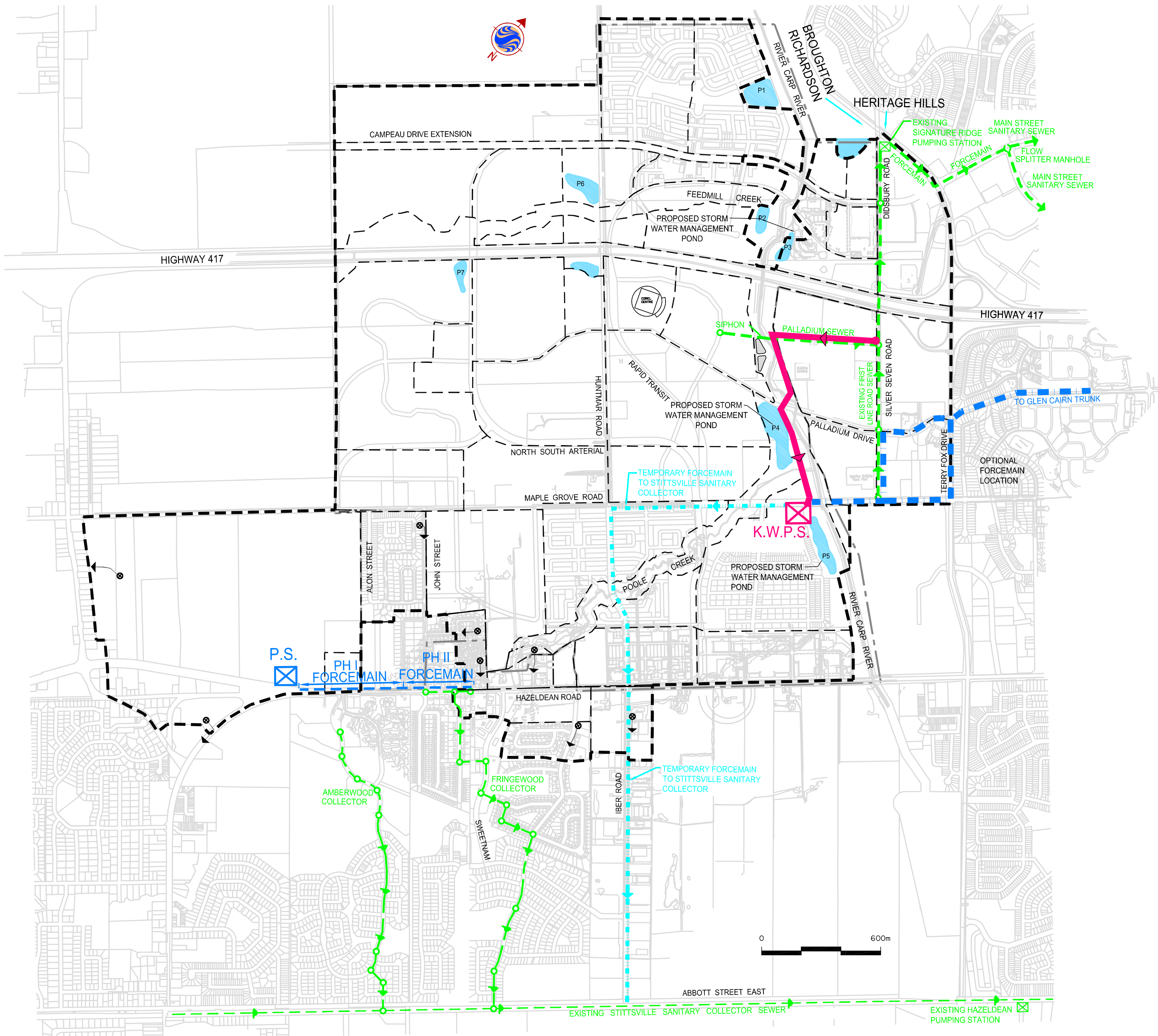

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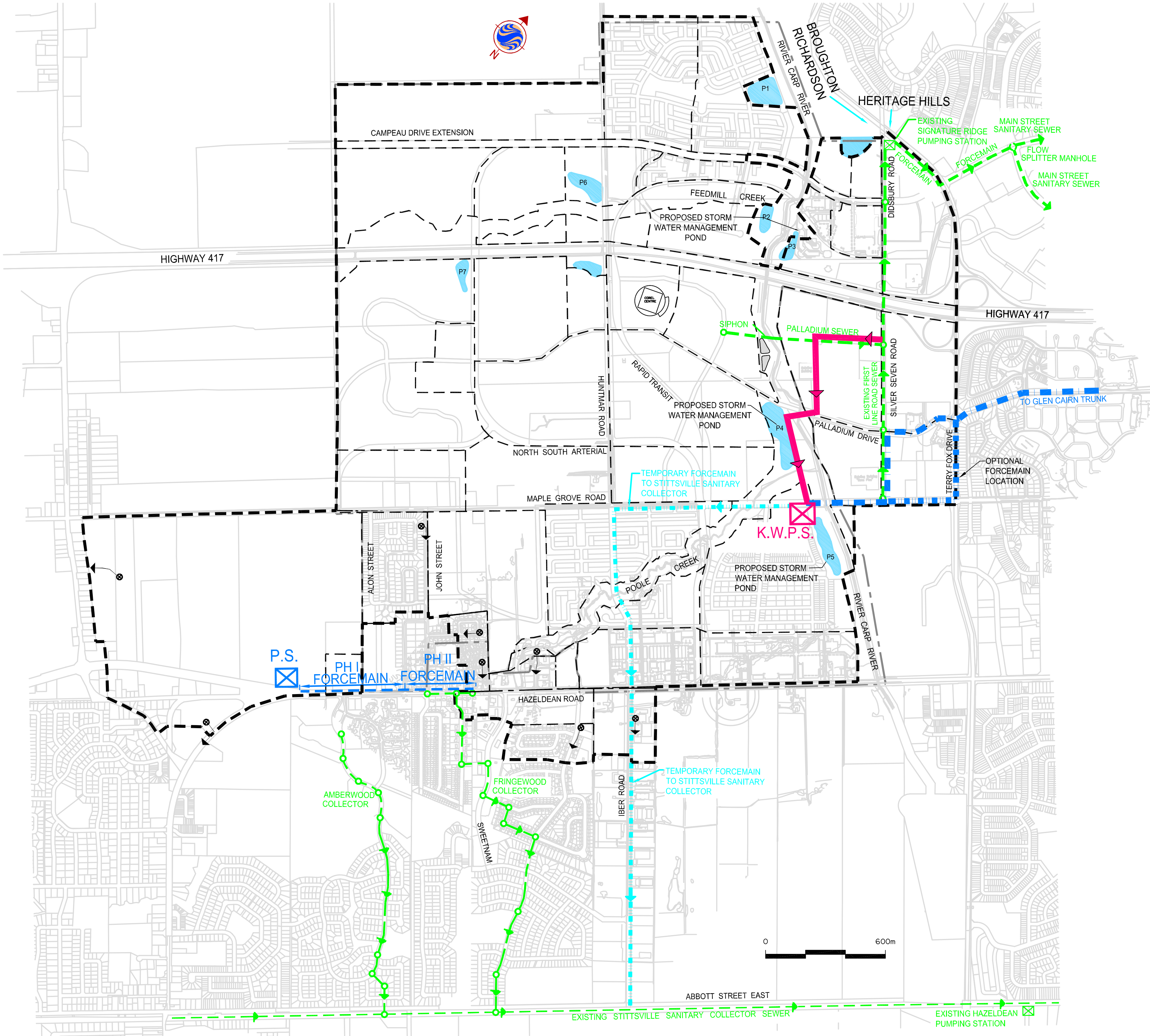
FIG. 4.1-6



INTERNAL SANITARY SERVICING

ALTERNATIVE IIIA

(PREFERRED OPTION)

Ultimate Major Drainage Limit

Proposed Trunk Sewer

Forcemain

Temporary Frocemain

Existing Trunk Sewers

Existing Pumping Station and Forcemain (To be Decommissioned)

Pump Station

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FIG. 4.1-6A

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Natural Environment (NE) (14%)

All four servicing options have a similar level of impact on the natural environment. Alternatives III and IIIA use a gravity sewer and a single pump station, thereby using less energy to discharge the sanitary flow from the KWCP. Alternatives II & III require the greatest amount of construction within the Carp River corridor.

Alternatives I and II both require two pumping stations. This increases the potential for impacts over the remaining options from the use of the emergency diesel generators and construction and construction.

4.2.3.3 Selection of Preferred Internal Servicing Alternative

Based on the above evaluation, Alternatives III and IIIA are considered to be the most viable options for the internal wastewater servicing for the KWCP. When comparing the two options, all of the evaluation criteria are similar. However, Alternative III requires the construction of the trunk sanitary sewer within the Carp River corridor. Alternative IIIA utilizes the proposed road allowances for the construction of a portion of the trunk sewer alignment, minimizing the potential for impacts to the Carp River. Based on the evaluation results, Alternative IIIA is selected as the preferred servicing alternative.

4.2.4 Temporary Forcemain Alternatives

4.2.4.1 Description of Temporary Forcemain Alternatives

A temporary forcemain will be required to service the initial phases of development within the KWCP. Three potential alignments were selected based on available corridors through the existing community. Each alignment begins at the preferred location of the Kanata West Pump Station, located on Maple Grove Road and west of the Carp River. All three servicing scenarios ultimately discharge to a temporary outlet, the Stittsville Collector Sewer. As illustrated on **Figure 4.1-7** the alternative forcemain alignments are:

- Alternative I – West along Maple Grove Road to Huntmar Road. South along Huntmar Road and Iber Road to the Stittsville Collector Sewer situated along Abbott Street East.

- Alternative II – South, parallel to the west side of the Carp River and through the proposed development lands to the Glen Cairn stormwater pond. East to Terry Fox Drive, then south along Terry Fox Drive to the Stittsville Collector Sewer.

- Alternative III – East on Maple Grove Road to Terry Fox Drive. South on Terry Fox Drive to the Stittsville Collector Sewer.

4.2.4.2 Evaluation of Temporary Forcemain Alternatives

The temporary forcemain alternatives were evaluated and ranked using the criteria discussed in Section 4.2 The results of the evaluation are summarized in **Table 4.1-4**. An explanation of the category rankings and weightings are provided below.

TABLE 4.1-3

Kanata West Wastewater - Internal Servicing Alternatives

Criteria		Indicators	Weighting	Rationale for Relative Weights	Alternatives: Internal Servicing I	II	III	IIIA
CONSTRUCTABILITY/FUNCTIONALITY			**36%**		14	14	23	22
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.	7%		3	3	3	3
CO1.2	Infrastructure Requirements	Extent of works required.	7%	Alt. I and II require two pumping stations. Alt. III and IIIA require one pumping station. All Alts. require the same amount of piping.	1	1	3	3
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.	6%	Alt. I and II (with two pumping stations) have more maintenace intensive infratructure.	1	1	2	2
CO1.4	Construction Scheduling	Impact of construction on development timing.	4%		3	3	3	3
CO1.5	Property Acquisition	Ease of property acquisition. (Depends on status of lands and adjacent lands, i.e. vacant, leased or owner occupied.)	2%	Alt. III requires the least amount of property acquisition with only one pumping station located on active developers lands and using the Carp River corridor for sewer alignment.	2	2	4	3
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow	6%	Alt. I and II have pumping stations remotely located relative to proposed storm ponds.	2	2	4	4
CO1.7	Servicing Flexibility	Ease of accommodating potential changes in servicing plans.	5%	The more central location of the main pumping station to the tributary area makes Alt. III and IIIA more flexible to change.	2	2	4	4
ECONOMY			**25%**		11	11	18	18
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.	6%	Alt. III and IIIA service the entire area south of Hwy 417 with one pumping station.	2	2	4	4
E2	Efficiency of Use of Existing Infrastructure	Use of exisitng capacity	5%		4	4	4	4
E3	Energy Consumption	Pumping requirements	4%	Alt. I and II requires double pumping of a significant portion of the service area.	1	1	3	3
E5	Impact on Agriculture	Agriculture area likely to be affected by infrastructure.	2%		3	3	3	3
E9	Capital Cost	Estimated cost of construction.	8%	Alt. I and II are significantly more expensive primarily due to the cost of two pumping stations.	1	1	4	4
CARING AND HEALTHY COMMUNITIES			**25%**		10	10	10	10
C3	Displacement of Residents, Community/Recreation Features and Institutions.	Affects areas of residence, institutions or businesses.	6%		4	4	4	4
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses and visibility of additional infrastructure.	11%		3	3	3	3
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Roadwork Environmental Study Report and Storm Sewer and Watermain Needs).	8%		3	3	3	3
NATURAL ENVIRONMENT			**14%**		13	9	11	14
N1	Impact on Significant Natural Features	Loss of natural area due to installation of works.	3%	Alts. II & III require a significant amount of work inside the Carp River corridor.	4	2	2	3
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.	3%	Alts. II & III require a significant amount of work inside the Carp River corridor.	3	2	2	3
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pump station failure.	3%	Alts. I & II require two pumping stations for each alternative. Alts. III & IIIA require only one station each.	2	2	3	3
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.	1%	Alt. I and II require double pumping where Alt. III and IIIA only require single pumping.	1	1	2	2
N6	Effects on Urban Greenspace, Open Space and Vegetation (i.e.trees,shrubs,etc.)	Disruption to greenspace and trees.	5%	Alt. III requires work within the Carp River Corridor.	3	2	2	3
Total Score			**100%**		2.39	2.26	3.21	3.29
Ranking					**3**	**4**	**2**	**1**
Estimated Capital Cost (in $million)					**8.5**	**8.5**	**5.5**	**5.5**

Description of Alternatives
Internal Servicing Alternative I - Silver Seven Road at HWY 417
Internal Servicing Alternative II - HWY 417 East of Carp River
Internal Servicing Alternative III - Maple Grove Road West of the Carp River
Internal Servicing Alternative IIIA - Maple Grove Road West of the Carp River with an Alternative Sewer Alignment

Evaluation Ranking
1 -2 High or Negative Impact
3 Moderate or No Impact
4-5 Low or Positive Impact

Constructability/Functionality (C/F) –36%

All three alternatives require the construction of a shallow forcemain so geotechnical issues are not considered to be a concern along the selected alignments. However, an assessment of the subsurface conditions indicates that unlike Alternative III, Alternatives I and II will not require rock excavation.

Alternatives I and III are located entirely within existing or proposed road allowances eliminating the need for additional land or easements. A benefit of Alternative II is that the length of the require forcemain is moderately less than Alternatives I and III.

Alternative I is advantageous for routine maintenance operations as the alignment is located within a lower classification of roadway when compared to Alternative III.

Economy (E) – 25%

Approximately 50% of the Alternative I forcemain will be installed in conjunction with other development works minimizing the amount of reinstatement required. This reduces the overall cost of Alternative I relative to the other remaining options. A large portion of Alternative II would be constructed in open fields requiring fewer costs for reinstatement when compared to Alternative III.

Caring and Healthy Community (CHC) – 25%

All three alternatives present similar impacts to the community. These impacts are limited to the duration of construction and are therefore considered minimal. Alternative I creates the least amount of impact when compared to Alternatives II and III. This is due to the fact that approximately half of the construction of the temporary forcemain will be done with other development works. Alternative II requires construction along major arterials within existing communities east of the KWCP, resulting in the highest level of impact.

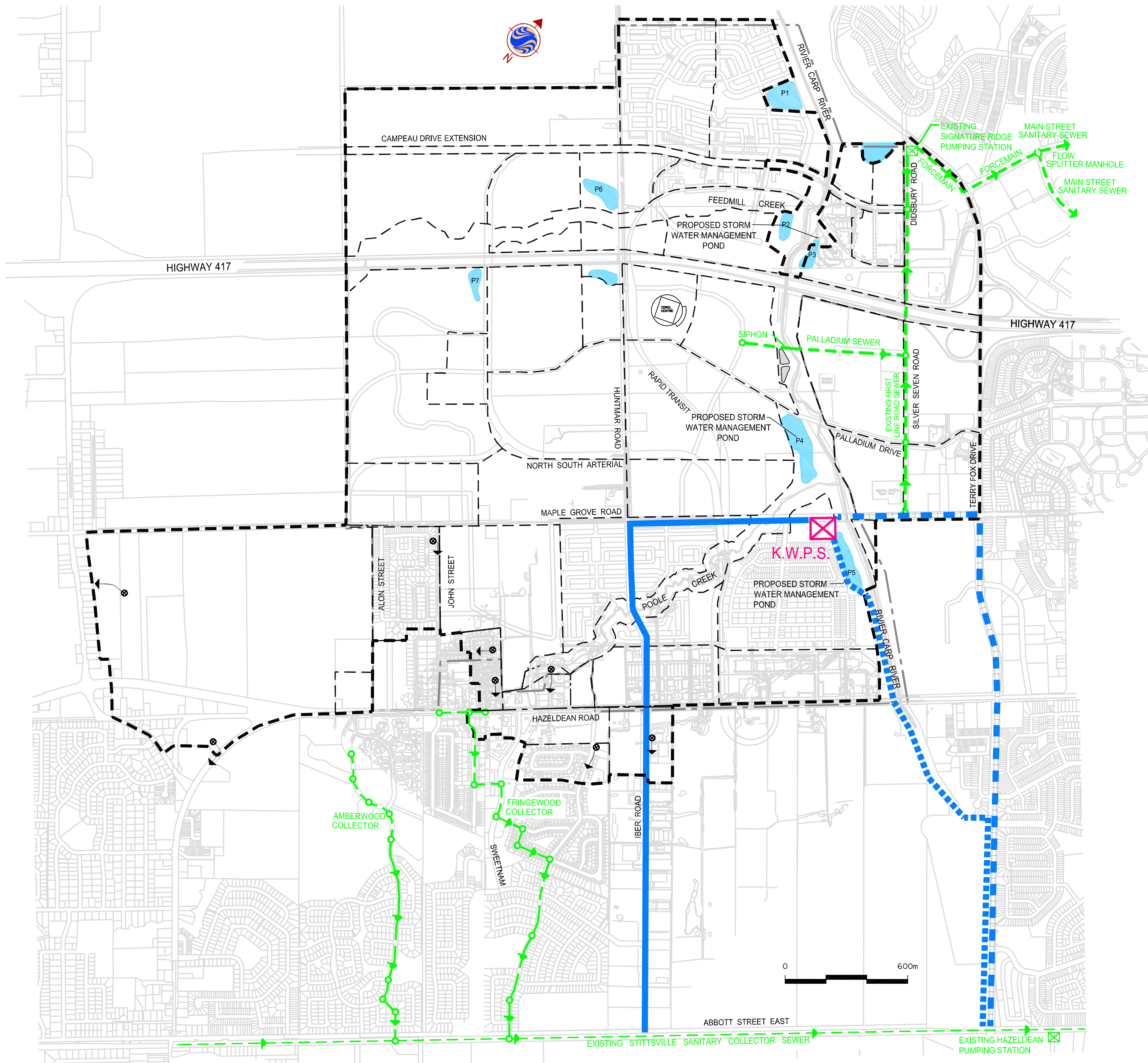
Natural Environment (NE) – 14%

Alternatives I and III are entirely contained within existing or proposed road allowances. However, Alternative III would require a crossing at the Carp River. Construction monitoring to detect any required mitigation measures for potential impacts to water quality would be required. A large portion of the Alternative II alignment is within the Carp River corridor and will have the highest impact on existing natural features.

4.2.4.3 Selection of Preferred Temporary Forcemain Alternative

Based on the above evaluation, temporary forcemain Alternative I, the Huntmar Road/Iber Road alignment, is selected as the preferred alternative. This alignment facilitates routine maintenance operations, as it is located within a roadway of lower classification when compared to the other alternatives (Terry Fox Drive). This alignment also results in the least amount of impact on the existing natural features. The Alternative I alignment is similar to Alternative II as the most economical options. Over half of the alignment will be constructed in conjunction with other works, unlike Alternative II.

TEMPORARY SANITARY FORCEMAIN ALTERNATIVES



Ultimate Major Drainage Limit

Alternate I (Preferred Option)

Alternate II

Alternate III

Existing Trunk Sewers

Existing Pumping Station and Forcemain (To be Decommissioned)

Pump Station

MAY 2006

CCL/IBI

Cumming Cockburn Limited

Stantec

FIG. 4.1-7

TABLE 4.1-4

Kanata West Wastewater - Temporary Forcemain Alternatives

Criteria		Indicators	Weighting	Rationale for Relative Weights	Temporary Forcemain Alternatives		
					I	II	III
CONSTRUCTABILITY/FUNCTIONALITY			**36%**		21	18	20
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.	7%	Alt. III requires acrossing of the Carp River through deep clay deposits.	3	3	2
CO1.2	Infrastructure Requirements	Extent of works required.	7%		2	2	2
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.	6%		3	3	3
CO1.4	Construction Scheduling	Impact of construction on development timing.	4%		3	3	3
CO1.5	Property Acquisition	Ease of property acquisition. (Depends on status of lands and adjacent lands, i.e. vacant, leased or owner occupied.)	2%	Alt. II requires property acquisition from private owners.	4	1	4
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow	6%		3	3	3
CO1.7	Servicing Flexibility	Ease of accommodating potential changes in servicing plans.	5%		3	3	3
ECONOMY			**25%**		16	14	13
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.	6%	Alts. I uses a common corridor with other new works for half of length. Alt. II requires a new single use corridor for 1/3 of its length.	4	1	2
E2	Efficiency of Use of Existing Infrastructure	Use of exisitng capacity	5%		3	3	3
E3	Energy Consumption	Pumping requirements	4%		3	3	3
E5	Impact on Agriculture	Agriculture area likely to be affected by infrastructure.	2%		3	3	3
E9	Capital Cost	Estimated cost of construction.	8%	Alt II is the least expensive and Alt. III is the most expensive to install.	3	4	2
CARING AND HEALTHY COMMUNITIES			**25%**		8	5	7
C3	Displacement of Residents, Community/Recreation Features and Institutions.	Affects areas of residence, institutions or businesses.	6%	Alt. II is adjacent to Carp River corridor.	3	2	3
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses and visibility of additional infrastructure.	11%	Alt. II and III are along major arterials in existing communities.	3	1	2
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Roadwork Environmental Study Report and Storm Sewer and Watermain Needs).	8%		2	2	2
NATURAL ENVIRONMENT			**14%**		15	9	14
N1	Impact on Significant Natural Features	Loss of natural area due to installation of works.	3%	Alt. II is adjacent to Carp River corridor.	3	1	2
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.	3%	Alt. II is adjacent to the Carp River corridor which presents a potential for impacts to aquatic systems..	3	2	3
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pump station failure.	3%	Alt. II requires construction along a significant portion of the Carp River corridor which is currently vegetated.	3	2	3
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.	1%		3	3	3
N6	Effects on Urban Greenspace, Open Space and Vegetation (i.e.trees,shrubs,etc.)	Disruption to greenspace and trees.	5%	Alt. II is adjacent to Carp River corridor which presents a potential for impacts to aquatic systems..	3	1	3
Total Score			**100%**		2.93	2.29	2.52
Ranking					**1**	**3**	**2**
Estimated Capital Cost (in $million)					**1.5**	**1.5**	**2**

Description of Alternatives
Temporary Forcemain Alternative I - Maple Grove/Huntmar/Iber Road to the Sittsville Collector
Temporary Forcemain Alternative II- Carp River/Terry Fox to the Stittsville Collector
Temporary Forcemain Alternative III- Maple Grove/Terry Fox to the Stittsville Collector

Evaluation Ranking
1 -2 High or Negative Impact
3 Moderate or No Impact
4-5 Low or Positive Impact

4.2.5 Trunk Sewer Alignment Alternatives

4.2.5.1 Description of Trunk Sewer Alignment Alternatives

Three potential alignments were considered for the gravity sewer that will service the un-serviced lands on Hazeldean Road. This sewer will also permit the decommissioning of several small existing pumping stations located along the north limit of the Village of Stittsville. As illustrated in **Figure 4.1-8** the alternative alignments considered for this sewer are:

- Alternative I - Maple Grove Road from the proposed pumping station to Huntmar Road, south on Huntmar Road to Hazeldean Road at Iber Road.
- Alternative II - Maple Grove Road to south of Poole Creek, southerly along Poole Creek to the transit corridor, southerly along the transitway to Hazeldean Road at Iber Road.
- Alternative III - South from the Maple Grove Road Pumping Station through the proposed development lands adjacent to the Carp River to Hazeldean Road, west on Hazeldean Road to Iber Road.

4.2.5.2 Evaluation of the Trunk Sewer Alignment Alternatives

The alternative sewer alignments were evaluated and ranked using the criteria discussed in Section 4.2. The results of the evaluation are summarized in **Table 4.1-5.** . An explanation of the category rankings and weightings are provided below.

Constructability/Functionality (C/F) – 36%

All three alternatives require approximately the same depth of excavation and present similar geotechnical issues. A benefit of Alternative I is that at least half of the works will be installed in conjunction with other infrastructure. In addition, the Alternative I alignment will be installed in a corridor that will be part of Phase One of construction providing flexibility in phasing works outside the KWCP area.

Alternatives I and II require the least amount of infrastructure to reach Iber Road.

Economy (E) – 25%

Alternatives I and II offer the opportunity to use combined service corridors along Maple Grove Road and Huntmar Road (Alternative I) and Hazeldean Road and the transitway (Alternative II). Alternative I would be part of Phase 1 of construction and will ensure that the timing of installation will coincide with other joint use utilities. This ensures that the economies of the combined corridor servicing will materialize for Alternative I.

Alternatives I and II are the least costly to install as they require the least amount of infrastructure.

Caring and Healthy Community (CHC) – 25%

There are no significant differences between the three alternatives in terms of the impact on the community. The alignment of all three alternatives is primarily confined to within the development area. Impacts will be confined to the period of construction in all cases.

Natural Environment (NE) – 14%

All three sewer alignment alternatives have a similar impact on the environment. Each alignment is confined to existing right-of-ways or in new right-of-ways proposed within the development area. Alternative I requires crossing Poole Creek that may impact water quality.

4.2.5.3 Selection of Preferred Huntmar Road Sewer Alignment Alternative

Based on the above evaluation, Huntmar Road sewer Alternative I is selected as the preferred alignment for the gravity sewer. This sewer will service Hazeldean Road and the southern portion of the KWCP. The alignment is preferred because it maximizes the flexibility for development within the KWCP without compromising the surrounding communities or natural environment.

4.2.6 Signature Ridge Pumping Station Alternatives

4.2.6.1 Description of Signature Ridge Pumping Station Alternatives

The Signature Ridge Pumping Station is a critical element for providing sanitary service to the KWCP. The present condition of the station is insufficient to provide the necessary level of service required to service the proposed area. To the capacity, two alternatives were considered for the Station. The station can be upgraded (Alternative II) or it can be completely rebuilt (Alternative I), including the construction of a new wet well, pumps and auxiliary power facility. Upgrade recommendations have been described in the “Signature Ridge Pumping Station Feasibility Study” by R.V. Anderson Assoc. Ltd., dated Sept. 2003.

These alternatives were considered because of the significant amount of infrastructure that is currently dependent on the Signature Ridge Pumping Station for an outlet. The station is also located in close proximity to the northeast portion of the KWCP. **Figure 4.1-9** illustrates the location of the Signature Ridge Pumping Station.

4.2.6.2 Evaluation of Signature Ridge Pumping Station Alternatives

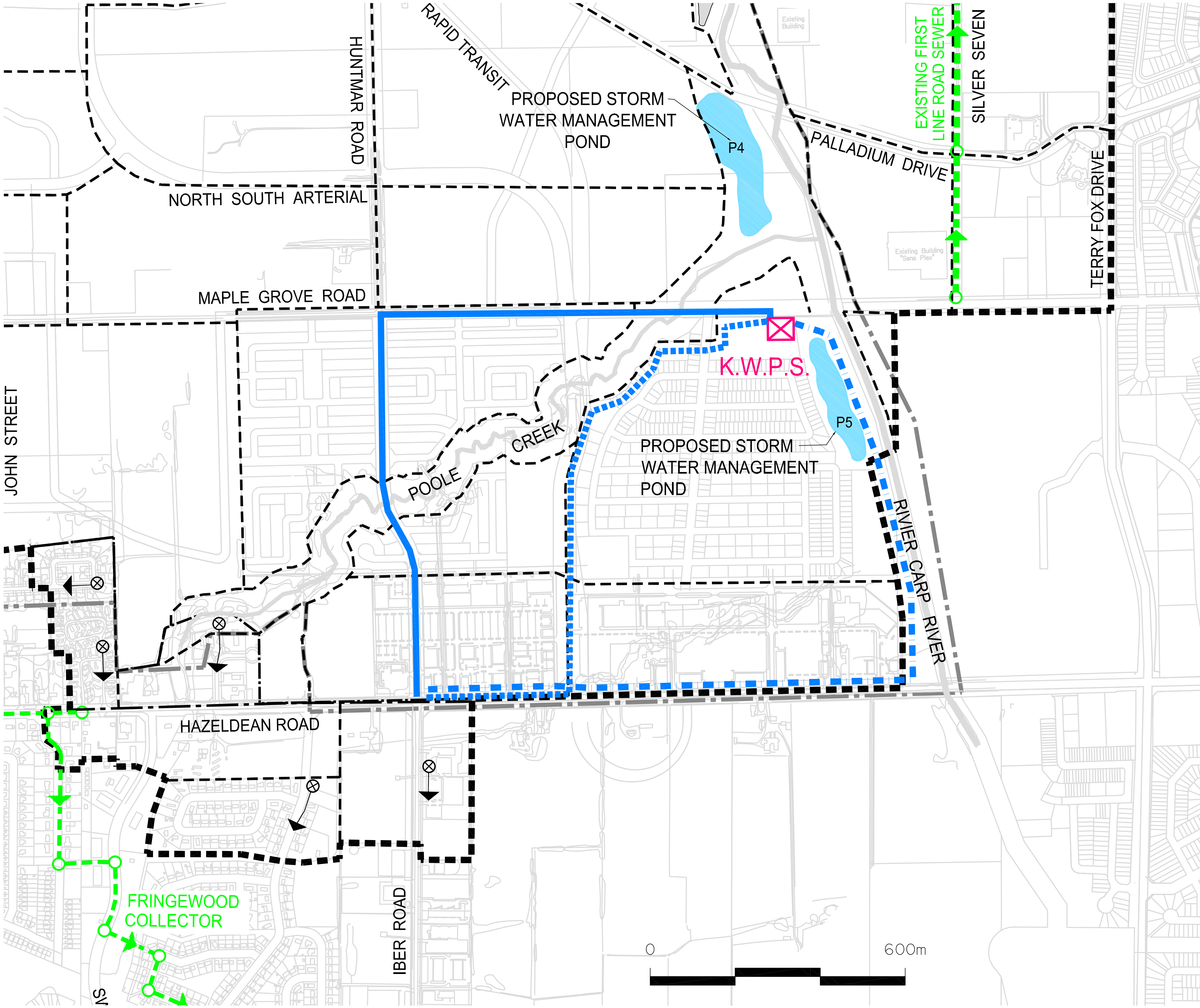
The alternative pumping station alternatives were evaluated and ranked using the criteria discussed in Section 4.2. The results of the evaluation are summarized in **Table 4.1-6**. An explanation of the category rankings and weightings are provided below.

Constructability/Functionality (C/F) 36%

The Signature Ridge Pumping Station requires only mechanical upgrades to provide the necessary level of service, which can be accomplished through Alternative I (Station up-grade). This eliminates the need to perform deep excavations in soft clays for reconstruction of the wet well. A benefit of constructing a new pumping station would be the ability to increase the pumping capacity to more than that required for the KWCP, increasing the flexibility of the overall wastewater system.

Upgrading the existing station will not require any property acquisition and can be completed in stages to meet the needs of future development over time.

TRUNK SEWER ALIGNMENT ALTERNATIVES



MAY 2006

FIG. 4.1-8

TABLE 4.1-5

Kanata West Wastewater - Trunk Sewer Alternatives

Criteria		Indicators	Weighting	Rationale for Relative Weights	Trunk Sewer Alternatives		
					I	II	III
CONSTRUCTABILITY/FUNCTIONALITY			**36%**		27	17	19
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.	7%		3	3	3
CO1.2	Infrastructure Requirements	Extent of works required.	7%	Alt. III requires the most sewer .	3	3	2
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.	6%		3	3	3
CO1.4	Construction Scheduling	Impact of construction on development timing.	4%	Alt. I ensures the trunk sewer is constructed as part of Phase I due to the requirement to install Huntmar Road as part of Phase I.	5	2	2
CO1.5	Property Acquisition	Ease of property acquisition. (Depends on status of lands and adjacent lands, i.e. vacant, leased or owner occupied.)	2%	Alts. I and III are entirely within existing road right-of-ways or in new roads.	5	2	5
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow	6%		3	3	3
CO1.7	Servicing Flexibility	Ease of accommodating potential changes in servicing plans.	5%	The central location of Alt. I to the service area maximizes flexibility.	5	1	1
ECONOMY			**25%**		17	15	12
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.	6%	Alt. I is entirely within a joint use corridor where Alt. II and III require extensive specific corridors.	5	3	2
E2	Efficiency of Use of Existing Infrastructure	Use of exisitng capacity	5%		3	3	3
E3	Energy Consumption	Pumping requirements	4%		3	3	3
E5	Impact on Agriculture	Agriculture area likely to be affected by infrastructure.	2%		3	3	3
E9	Capital Cost	Estimated cost of construction.	8%	Alt. III is significantly more expensive than Alt. I and II due to overall length and singular service construction.	3	3	1
CARING AND HEALTHY COMMUNITIES			**25%**		9	9	9
C3	Displacement of Residents, Community/Recreation Features and Institutions.	Affects areas of residence, institutions or businesses.	6%		3	3	3
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses and visibility of additional infrastructure.	11%		3	3	3
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Roadwork Environmental Study Report and Storm Sewer and Watermain Needs).	8%		3	3	3
NATURAL ENVIRONMENT			**14%**		13	15	15
N1	Impact on Significant Natural Features	Loss of natural area due to installation of works.	3%	Alt. I crosses Poole Creek requring construction within the river corridor.	2	3	3
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.	3%	Alt. I crosses Poole Creek increasing the potential to impact fish habitat.	2	3	3
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pump station failure.	3%		3	3	3
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.	1%		3	3	3
N6	Effects on Urban Greenspace, Open Space and Vegetation (i.e.trees,shrubs,etc.)	Disruption to greenspace and trees.	5%		3	3	3
Total Score			**100%**		3.29	2.84	2.61
Ranking					**1**	**2**	**3**
Estimated Capital Cost (in $million)					**1.5**	**1.5**	**2.5**

Description of Alternatives
Trunk Sewer Alternative I - Maple Grove/Huntmar/Hazeldean Road
Trunk Sewer Alternative II- Maple Grove/Poole Creek/Transitway/Hazeldean Road
Trunk Sewer Alternative III - Maple Grove/Hazeldean Road

Evaluation Ranking
1 -2 High or Negative Impact
3 Moderate or No Impact
4-5 Low or Positive Impact

Economy (E) 25%

The reconstruction of the Signature Ridge Pumping Station is significantly more than the costs to upgrade the existing station.

Caring and Healthy Community (CHC) 25%

In terms of the impact on the Community, there are no significant differences between the two alternatives.

Natural Environment (NE) 14%

There are no significant differences between the two options with respect to impacts to the natural environment. Both alternatives require the construction of an emergency overflow to the Carp River. Impacts to surface water quality as a result of potential station overflows during an emergency situation are not expected to occur. Should an overflow occur for either alternative, the impacts would be mitigated by a SWM pond. Increases in CO_2 emissions as a result of the use of diesel generators during power failures or maintenance procedures will be negligible and are similar in both alternatives.

4.2.6.3 Selection of Preferred Signature Ridge Pumping Station Alternative

Based on the above evaluation, the Signature Ridge Pumping Station Alternative I, station upgrade, is selected as the preferred alternative. This alternative maximizes the use of existing infrastructure and offers the most flexibility in phasing of the works with the least amount of capital expenditure or impacts.

4.2.6.4 Summary

The preferred alternatives selected for the wastewater outlet, the internal servicing system, the temporary forcemain, the trunk sewer alignment, and the Signature Ridge Pumping Station have been used to develop a comprehensive wastewater servicing plan for the KWCP. This servicing plan is discussed in future detail in the following section of this report.

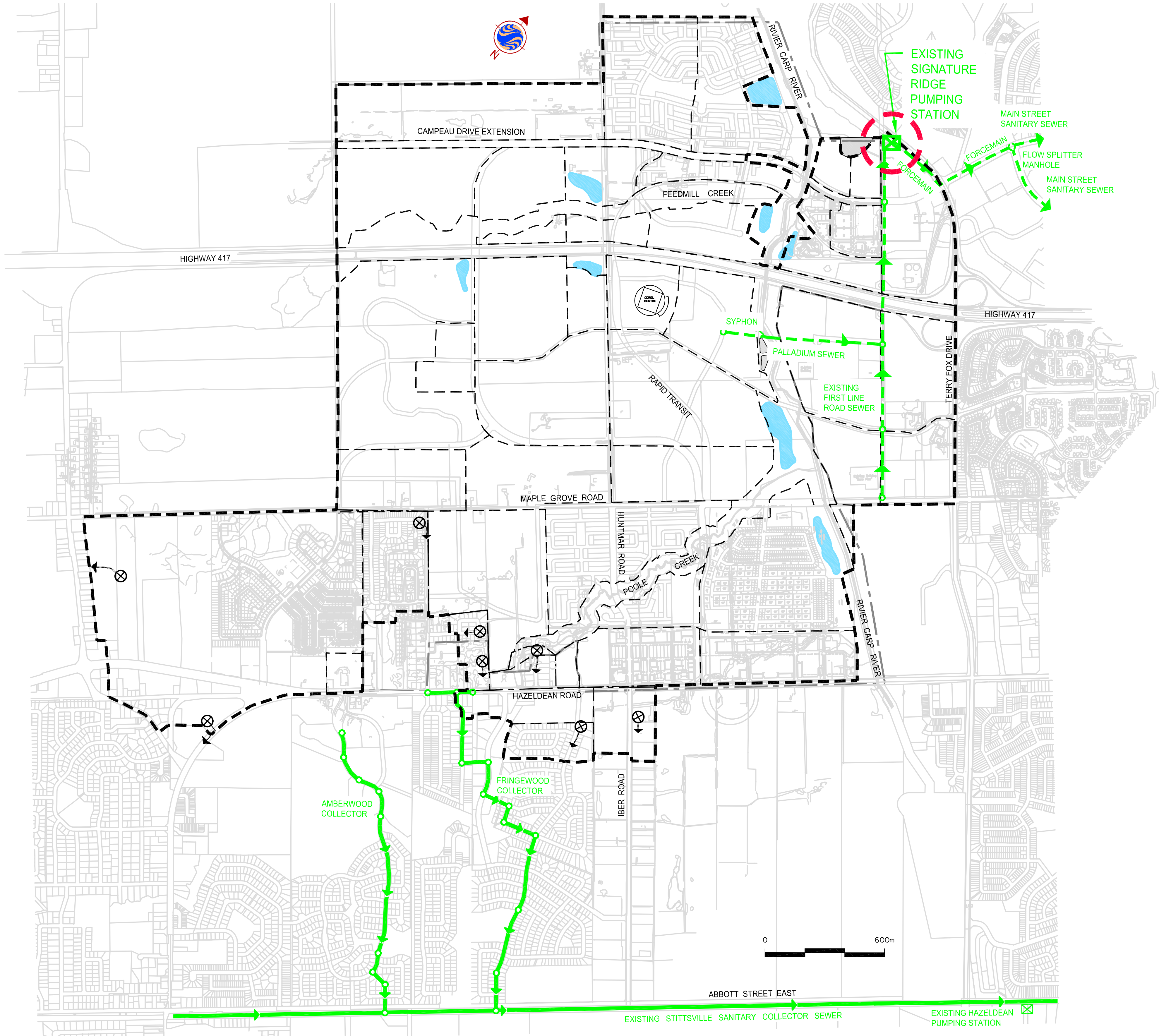
4.3 Preferred Sanitary Sewer Servicing Plan

Section 4.2 has detailed the selection of preferred alternatives for the major infrastructure required to provide sanitary sewer service to the KWCP. These preferred alternatives have been used to develop a Master Sanitary Servicing Plan for the area. This plan is illustrated on **Drawing S-1** (appended to this report). The major features of this plan are:

(i.) An upgraded Signature Ridge Pumping Station (SRPS) to service all the KWCP lands north of the Queensway, the existing urban area north of the Queensway currently proposed to drain to the SRPS, and the Broughton/Richardson Interstitial lands. A spreadsheet detailing the exact areas and flows tributary to the SRPS is included in **Figure 4.2-1**.

The 400 l/sec peak flow capacity identified in **Figure 4.2-1** for the upgraded SRPS, is consistent with the findings of the R.V. Anderson Report titled “Signature Ridge Pumping Station Upgrades Feasibility Study”.

SIGNATURE RIDGE PUMPING STATION LOCATION



Legend:

- Ultimate Drainage Limit
- Existing Stittsville Sewer
- Existing Trunk Sewer
- Existing Pumping Station and Forcemain (To be Decommissioned)

Stantec

CCL/IBI
Cumming Cockburn Limited

MAY 2006

FIG. 4.1-9

TABLE 4.1-6

Kanata West Wastewater - Temporary Forcemain/Trunk Sewer/Signature Ridge Alternatives

Criteria		Indicators	Weighting	Rationale for Relative Weights	Signature Ridge PS Alternative Upgrade	Signature Ridge PS Alternative Rebuild
CONSTRUCTABILITY/FUNCTIONALITY			**36%**		24	16
CO1.1	Geotechnical Issues and Construction Risks	Potential for encountering poor soils and/or elevated groundwater conditions.	7%	Alt. II requires reconstruction of the pumping station in very soft clays where Alt. I does not require reconstruction of the wet well.	3	1
CO1.2	Infrastructure Requirements	Extent of works required.	7%	Alt. I only requires upgrading of hardware within the existing pumping station.	4	1
CO1.3	Operational Impacts	Amount of maintenance intensive infrastructure required.	6%		3	3
CO1.4	Construction Scheduling	Impact of construction on development timing.	4%	Alt. I can be phased to suit development timing where Alt. II requires a lengthy total reconstruction program.	4	2
CO1.5	Property Acquisition	Ease of property acquisition. (Depends on status of lands and adjacent lands, i.e. vacant, leased or owner occupied.)	2%	Alt. II requires property acquisition for a new station because existing station will have to remain in service during construction.	5	2
CO1.6	System Reliability	Proximity of a storm sewer, SWM or other surface water for emergency overflow	6%		3	3
CO1.7	Servicing Flexibility	Ease of accommodating potential changes in servicing plans.	5%	Alt. II can be built to accommodate changes where Alt. I is designed to the maximum.	2	4
ECONOMY			**25%**		19	12
E1	Potential to Use Combined Service Corridor	Length and area of combined service corridor.	6%		3	3
E2	Efficiency of Use of Existing Infrastructure	Use of exisitng capacity	5%	Alt. I maximizes the use of the existing station.	5	2
E3	Energy Consumption	Pumping requirements	4%		3	3
E5	Impact on Agriculture	Agriculture area likely to be affected by infrastructure.	2%		3	3
E9	Capital Cost	Estimated cost of construction.	8%	Alt. II is significantly more expensive to construct.	5	1
CARING AND HEALTHY COMMUNITIES			**25%**		12	9
C3	Displacement of Residents, Community/Recreation Features and Institutions.	Affects areas of residence, institutions or businesses.	6%		4	4
C4	Disruption to Existing Community	Extent of works affecting existing residences and businesses and visibility of additional infrastructure.	11%	Alt. 1 requires only internal up-grades and will have minimal construction traffic or related impacts.	4	3
C9	Consistency with Planned Land Use and Infrastructure	Compatibility with City land use, design guidelines and infrastructure servicing corridor planning (Kanata West Roadwork Environmental Study Report and Storm Sewer and Watermain Needs).	8%	Alt. I maximizes use of currently planned infrastructure by upgrading existing station to its maximum potential.	4	2
NATURAL ENVIRONMENT			**14%**		14	14
N1	Impact on Significant Natural Features	Loss of natural area due to installation of works.	3%		3	3
N3	Impact on Aquatic Systems	Potential impact on fish habitat due to installation of works.	3%		3	3
N4	Impact on Quality and Quantity of Surface Water and Groundwater	Potential impact on water quality in the Carp River resulting from rare emergency overflows to the SWM pond due to pump station failure.	3%		3	3
N5	Impact on Global Warming	Difference in carbon dioxide emissions resulting from occasional use of diesel generator.	1%		2	2
N6	Effects on Urban Greenspace, Open Space and Vegetation (i.e.trees,shrubs,etc.)	Disruption to greenspace and trees.	5%		3	3
Total Score			**100%**		3.60	2.48
Ranking					**1**	**2**
Estimated Capital Cost (in $million)					**1**	**4**

Evaluation Ranking
1 -2 High or Negative Impact
3 Moderate or No Impact
4-5 Low or Positive Impact

Description of Alternatives
Signature Ridge PS Alternative 1 - Rebuild
Signature Ridge PS Alternative II - Upgrade

The Signature Ridge Pumping Station is currently not equipped with catastrophic failure protection in the form of a gravity overflow. A hydraulic analysis of the proposed sewer system was therefore completed to evaluate the potential for providing a gravity overflow. This analysis demonstrates that catastrophic protection can be provided by gravity. The analysis is included in **Appendix 4.2** and demonstrates that overflows to the existing stormwater management pond on First Line Road and to Pond I can provide the necessary level of protection.

(ii.) A single new pumping station and forcemain located south of Maple Grove Road and west of the Carp River.

This new pumping station ultimately services all the KWCP south of Highway 417, the lands south of the 417 originally tributary to the SRPS, and the lands in the Village of Stittsville, along Hazeldean Road which are currently unserviceable by gravity to the Stittsville Sanitary Sewer System. This new pumping station has also been designed to accommodate the decommissioning of up to eight small public and private pumping stations along Hazeldean Road without deepening the Kanata West system. **Figure 4.2-1** details the exact areas and flows from Stittsville which will ultimately be tributary to the new pumping station. The areas are also illustrated on **Drawing S-1**.

Figures 4.2-3 and 4.2-4 illustrate a conceptual layout and cross-section for the new pumping station and **Appendix 4.3** details the conceptual design of the pumping station.

The new pumping station will temporarily outlet to the Stittsville Collector Sewer via a temporary forcemain in Huntmar Road and Iber Road. This temporary forcemain is designed to accommodate a flow of 190 l/sec (approximately 3,000 units). The temporary outlet will be located entirely within a public right-of-way. The single 405 mm diameter forcemain used for the initial outlet can be kept in service for long-term use as an emergency back up outlet. Rationale on the availability of capacity in the Stittsville Collector Sewer is attached as **Appendix 4.1**.

The permanent outlet for the new pumping station consists of a forcemain leading from the pumping station to the Glen Cairn Collector Sewer east of Eagleson Road. The preferred route for this forcemain in along Maple Grove Road to Silver Seven Road; along the east side of Silver Seven Road, in an easement, in the undeveloped lands between Maple Grove Road and Palladium Drive; easterly along Palladium Drive to Katimavik Road; and easterly along the north side of Katimavik Road, in the corridor for the unbuilt westbound lanes of Katimavik Road, to Eagleson Road and the Glen Cairn Collector Sewer. The location of the new pumping station is in close proximity to Stormwater Management Ponds 4 and 5. This provides catastrophic failure protection to the new pumping station in the form of a gravity overflow. The hydraulic analysis of this overflow system is attached as **Appendix 4.2**.

The preferred sanitary sewer system also includes a gravity sewer, which collects flow from several minor internal sanitary sewers and directs this flow to the new pumping station location. As illustrated on **Drawing S-1** this minor collector sewer runs parallel to the west side of the Carp River corridor between Maple Grove Road and Palladium Drive, crossing under the Carp River by boring beneath the river. The sewer extends northerly to intercept flows from Silver Seven Road and diverts them from the Signature Ridge Pumping Station. The inclusion of this north south sewer is a key element in eliminating the need for double pumping within Kanata

SANITARY SEWER DESIGN SHEET
PROJECT : Kanata West Servicibility Study
LOCATION : CITY OF OTTAWA

PROJECT: 3598-LD-03
DATE: April 2005
DESIGN: JIM
FILE: 3598LD.sewers.XLS

MODEL 1 ULTIMATE (population based criteria..ICI simultaneous peaking)

LOCATION: STREET	FROM MH	TO MH		TOTAL AREA (Ha)	RESIDENTIAL: APPLIC AREA (Ha)	UNIT/Ha	TOTAL UNITS	POPULATION INDIV	POPULATION ACCUM	PEAK FACTOR	PEAK FLOW (l/s)	EMPLOYMENT/RETAIL/BUSINESS PARK/OPEN SPACES: APPLIC AREA (Ha)	ACCUM AREA (Ha)	TOTAL AREA (Ha)	FLOW RATE (l/Ha/d)	PEAK FLOW INDIV (l/s)	PEAK FLOW ACCUM (l/s)	PEAK FLOW TOTAL (l/s)	INFILTRATION: AREA (Ha) INDIV	AREA (Ha) CUMUL	TOTAL CUMUL	PEAK FLOW (l/s)	TOTAL FLOW (l/s)	PROPOSED SEWER: CAPACITY l/s	VELOCITY (full) m/s	LGTH. (m)	PIPE (mm)	GRADE %	AVAIL. CAP. (%)	HARMON PF	ACTUAL q/Q	va/Vf	ACTUAL VELOCITY (m/s)
Campeau Drive Trunk Sewer	1	2	Area 1 (PBP)	38.11								38.11	38.11		35000	23.16	23.16		38.11	38.11													
			Area 2 (PBP)	27.29								27.29	65.40		35000	16.58	39.74		27.29	65.40													
			Area 3 Ext Employment	14.05								14.05	79.45		50000	12.20	51.94		14.05	79.45													
			Area 4 HP Employment	10.93								10.93	90.38	90.38	50000	9.49	61.42	61.42	10.93	90.38	90.38	25.31	86.73	283.79	1.27	525.0	525	0.40	69.44%		0.306	0.730	0.927
	2	3	Area 5 Residential	29.19	29.19	19	555	1664	1664	3.65	24.58			90.38				61.42	29.19	29.19										3.65			
			Area 9 Ext Employment	8.45							24.58	8.45	8.45	98.83	50000	7.34	7.34	68.76	8.45	128.02	128.02	35.85	129.18	286.61	0.98	700.0	600	0.20	54.93%		0.451	0.830	0.815
	14	3	Area 6/8 Ext Employment	16.65								16.65	16.65		50000	14.45	14.45		16.65	16.65													
			Area 7 HP Employment	5.48								5.48	22.13	22.13	50000	4.76	19.21	19.21	5.48	22.13	22.13	6.20	25.41	148.74	0.91	910.0	450	0.25	82.92%		0.171	0.630	0.571
	3	4							1664	3.65	24.58	0.00	0.00	120.96		0.00	0.00	87.97	0.00	0.00	150.15	42.04	154.59	392.29	1.06	300.0	675	0.20	60.59%	3.65	0.394	0.790	0.839
	4A	4	Area 10 Residential	27.86	27.86	19	529	1588	1588	3.66	23.55								27.86	27.86	27.86	7.80	31.36	148.74	0.91	750.0	450	0.25	78.92%	3.66	0.211	0.660	0.598
	4	5	14 Mixed Use	4.13	1.76	50	88	263	3515	3.38	48.17	2.37	2.37	123.33	35000	1.44	1.44	89.41	4.13	4.13	182.14	51.00	188.58	392.29	1.06	450.0	675	0.20	51.93%	3.38	0.481	0.840	0.892
	Queensway	5	Area 13 Community Retail	6.35								6.35	6.35	6.35	35000	3.86	3.86	3.86	6.35														
			Area 11/12 Mixed Use	11.80	5.02	50	251	752	752	3.88	11.81	6.79	13.14	13.14	35000	4.12	7.98	7.98	11.80	18.15	18.15	5.08	24.88	43.88	0.87	420.0	250	0.50	43.31%	3.88	0.567	0.880	0.762
	5	5A	Area 15 Community Retail	3.88				0	4267	3.31	57.19	3.88			35000	2.36			3.88											3.31			
First Line Road Sewer			Area 44	25.54							57.19	25.54	29.42	165.89	35000	15.52	17.88	115.27	25.54	29.42	229.71	64.32	236.77	519.43	1.14	300.0	750	*0.20*	54.42%		0.456	0.830	0.945
				229.71							57.19							115.27				64.32	236.77										
Signature Ridge		5A	Area 100 Residential	90.20	90.20	19	1714	5141	5141	3.23	67.35	0.00																		3.23			
Signature Ridge		5A	Area 100 Non-Residential	4.88							67.35	4.88	4.88	4.88	50000	4.24	4.24	4.24	95.08	95.08	95.08	26.62	98.21										
Intersticial Lands & Broughton/Richardson		5A																					65.00										
Total To SRPS	**5A**	**SRPS**		**324.79**	**154.02**		**3136**		**9409**		**124.54**	**170.77**						**119.51**			**324.79**	**90.94**	**399.98**	**580.53**	**1.27**	**30.0**	**750**	**0.25**	**31.10%**	2.98	0.689	0.940	1.197
Palladium Drive Trunk Sewer	6	7	Area 32 (PBP)	57.03								57.03	57.03		50000	49.51	49.51		57.03	57.03													
			Area 32A Park	8.34								8.34	65.37		0	0.00	49.51		8.34	65.37													
			Area 33/34 Ext Employment	54.85								54.85	120.22	120.22	50000	47.61	97.12	97.12	54.85	120.22	120.22												
	7	8	Area 37 Mixed Use	36.70	15.60	50	780	2340	2340	3.53	33.47	21.10	21.10	141.32	50000	18.32	18.32	115.44	36.70	36.70	156.92	43.94	192.85	455.83	1.23	925.0	675	0.27	57.69%	3.53	0.423	0.810	1.000
				156.92	15.60		780		2340		33.47	141.32						115.44	156.92		156.92	43.94	192.85							3.53			
Corel Centre Etc. (Existing Sewer)		16	Area 35 HP Employment	6.05								6.05	6.05		30000	3.15	3.15		6.05														
		16	Area 36 (Corel Centre)																			30.00											
		16	Area 38 Exten Employment	20.15								20.15	26.20	26.20	14400	5.04	8.19	8.19	20.15	26.20	26.20	7.34	45.52				*Existing*						
First Line Road Sewer	15	16	Area 40 Employment	14.59								14.59	14.59		35000	8.87	8.87		14.59	14.59													
			Area 41 Employment	11.97								11.97	26.56		35000	7.27	16.14		11.97	26.56													
			Area 42 Employment	20.66								20.66	47.22		35000	12.55	28.69		20.66	47.22													
			Area 43 Employment	28.89								28.89	76.11	76.11	35000	17.55	46.25	46.25	28.89	76.11	76.11	21.31	67.56	224.35	1.00	525.0	525	0.25	69.89%		0.301	0.730	0.733
Carp River Trunk	16	8	Nothing To Add	102.31	15.60		780		2340	3.53	33.47	102.31	102.31	102.31	0	0.00	54.44	54.44	0.00	102.31	102.31	28.65	113.08	286.61	0.98	400.0	600	0.20	60.54%	3.53	0.395	0.790	0.776
Carp River Trunk	8	10A	Nothing To Add	259.23	15.60		780		2340		33.47	0.00	0.00	243.63		0.00	0.00	169.87	0.00	139.01	259.23	109.92	305.93	579.95	1.05	550.0	825	0.15	47.25%	3.53	0.528	0.860	0.904
Marle Grove Road Trunk Sewer	9	10	Area 18/19 Exist. Residential	23.34	23.34	19	443	1330	1330										23.34	23.34										3.72			
			Area 22/26/27 Residential	79.32	79.32	30	2380	7139	8469	3.03	103.82								79.32	102.66	102.66	28.74	132.56	405.11	1.39	775.0	600	0.40	67.28%	3.03	0.327	0.740	1.027
Hazeldean/Huntmar Trunk Sewer	11	12	Area 16/20 Residential	99.01	99.01	19	1881	5644	5644	3.20	73.06								99.01	99.01										3.20			
			Area 16/20 Commercial	33.50								33.50	33.50	33.50	50000	29.08	29.08	29.08	33.50	132.51													
			Area 16/20 Open Space	14.13								14.13							14.13	146.64													
			Area 17 Ex. Commercial	3.44							73.06	3.44	36.94	36.94	35000	2.09	31.17	31.17	3.44	150.08	150.08	42.02	146.26	554.82	1.50	775.0	675	0.40	73.64%		0.264	0.700	1.051
	12	10	Area 21 Exist. Employment	10.89								10.89	10.89	10.89	50000	9.45	9.45		10.89	10.89													
			Area 19A Exist Residential	6.63	6.63	19	126	378									9.45		6.63	17.52													
			Area 23/24 Community Retail	17.61								17.61	28.50	28.50	35000	10.70	20.15	51.32	17.61	35.13													
			Area 28/30 Residential	27.10	27.10	30	813	2439	8460	3.03	103.72	0.00	0.00	65.44				51.32	27.10	62.23	212.31	59.45	214.49	519.43	1.14	950.0	750	0.20	58.71%	3.03	0.413	0.800	0.911
Marle Grove Road Trunk Sewer	10	10A	Area 39 Mixed Use	21.13	8.98	50	449	1347				12.15	12.15	77.59	35000	7.38	7.38	58.71	21.13														
			Area 29 Residential	15.00	15.00	30	450	1350	19627	2.66	211.54							58.71	15.00	36.13	351.10	98.31	368.56	669.89	1.21	1000.0	825	0.20	44.98%	2.66	0.550	0.870	1.056
Carp River Trunk Sewer	13	10A	Area 25 Community Retail	20.24								20.24	20.24	20.24	35000	12.30	12.30	12.30	20.24														
			Area 31 residential	38.72	38.72	30	1162	3485	3485	3.39	47.80							12.30	38.72	58.96	58.96	16.51	76.61	320.17	1.10	1000.0	600	0.25	76.07%	3.39	0.239	0.680	0.746
		10A	Area 31A (PBP)	0.75								0.75	0.75	0.75	50000	0.65	0.65	0.65	0.75	0.75	0.75	0.21	0.86	36.69	0.72	100.0	250	0.35	97.65%		0.023	0.340	0.246
Pumping Station 2 to KWPS	**10A**	**KWPS**		**670.04**	**313.70**		**8484**		**25451**		**292.82**	**356.34**						**241.53**			**670.04**	**224.95**	**759.29**	**1273.71**	**1.43**	**30.0**	**1050**	**0.20**	**40.39%**	2.55	0.596	0.900	1.283
STUDY TOTALS				994.83	467.72		11620		34860			527.11																		2.41			

Average Daily Per capita Flow Rate =	350 l/cap/d
Infiltration Allowance Flow Rate =	0.28 l/sec/Ha

Residential Peaking Factor = 1+(14/(4+(P^0.5))), P=Pop. in 1000's, Max of 4

Population density per unit =	3.00
P. F. For Employment/Retail/Business Park =	1.50

Mixed Uses Assumes: 15% Community Retail, 42.5% Business Park and 42.5% Residential

Revision No. 1:	April 01, 2005	Revision No. 6:	Oct. 14, 2005
Revision No. 2:	April 11, 2005	Revision No. 7:	Nov. 10, 2005
Revision No. 3:	April 21, 2005	Revision No. 8:	Nov. 11, 2005
Revision No. 4:	June 07, 2005	Revision No. 9:	Apr. 19, 2006
Revision No. 5:	August 10, 2005		

Stantec CCL Cumming Cockburn Limited /IBI

FIG. 4.2-1

SANITARY SEWER DESIGN SHEET
PROJECT : Kanata West Servicibility Stury
LOCATION : CITY OF OTTAWA

PROJECT: 3598-LD-03
DATE: Apr 2005
DESIGN: JIM
FILE: 3598LD.sewers.XLS

PHASE 1 SIGNATURE RIDGE (population based criteria..ICI simultaneous peaking)

LOCATION				TOTAL	RESIDENTIAL							EMPLOYMENT/RETAIL/BUSINESS PARK/OPEN SPACES							INFILTRATION				TOTAL	PROPOSED SEWER					
				AREA	APPLIC	UNIT/Ha	TOTAL	POPULATION		PEAK	PEAK	APPLIC	ACCUM	TOTAL	FLOW	PEAK FLOW			AREA (Ha)			PEAK	FLOW	CAPACITY	VELOCITY	LGTH.	PIPE	GRADE	AVAIL.
STREET	FROM	TO			AREA		UNITS	INDIV	ACCUM	FACTOR	FLOW	AREA	AREA	AREA	RATE	INDIV	ACCUM	TOTAL	INDIV	CUMUL	TOTAL	FLOW			(full)				CAP.
	MH	MH		(Ha)	(Ha)						(l/s)	(Ha)	(Ha)	(Ha)	(l/Ha/d)	(l/s)	(l/s)	(l/s)			CUMUL	(l/s)	(l/s)	l/s	m/s	(m)	(mm)	%	(%)
Campeau Drive Trunk Sewer	1	2	Area 1 (PBP)	0.00								0.00	0.00		35000	0.00	0.00		0.00	0.00									
			Area 2 (PBP)	0.00								0.00	0.00		35000	0.00	0.00		0.00	0.00									
			Area 3 Ext Employment	0.00								0.00	0.00		50000	0.00	0.00		0.00	0.00									
			Area 4 HP Employment	0.00								0.00	0.00	0.00	50000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	283.79	1.27	500.0	525	0.40	100.00%
	2	3	Area 5 Residential	29.19	29.19	19	555	1664	1664	3.65	24.58			0.00				0.00	29.19	29.19									
			Area 9 Ext Employment	0.00							24.58	0.00	0.00		50000	0.00	0.00	0.00	0.00	0.00	29.19	8.17	32.75	286.61	0.98	700.0	600	0.20	88.57%
	14	3	Area 6/8 Ext Employment	0.00								0.00	0.00	0.00	50000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
			Area 7 HP Employment	0.00								0.00	0.00	0.00	50000	0.00	0.00	0.00	0.00	0.00				148.74	0.91	920.0	450	0.25	100.00%
	3	4							1664	3.65	24.58	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	29.19	8.17	32.75	200.67	0.90	150.0	675	0.20	83.68%
	4A	4	Area 10 Residential	27.86	27.86	19	529	1588	1588	3.66	23.55								27.86	27.86	27.86	7.80	31.36	34.00	0.67	750.0	450	0.25	7.76%
	4	5	14 Mixed Use	4.13	1.76	50	88	263	3515	3.38	48.17	2.37	2.37	123.33	35000	1.44	1.44	1.44	4.13	4.13	61.18	17.13	66.74	200.67	0.90	600.0	750	0.20	66.74%
Corel Centre Etc. (Existing Sewer)		15	Area 35 HP Employment	6.05								6.05	6.05		30000	3.15	3.15		6.05										
			Area 36 (Corel Centre)																			30.00							
			Area 38 Exten Employment	20.15								20.15	26.20	26.20	14400	5.04	8.19	8.19	20.15	26.20	26.20	7.34	45.52				*Existing*		
First Line Road Sewer		15	Area 40 Employment	14.59								14.59	14.59		35000	8.87	8.87		14.59	14.59									
			Area 41 Employment	11.97								11.97	26.56		35000	7.27	16.14		11.97	26.56									
			Area 42 Employment	20.66								20.66	47.22		35000	12.55	28.69		20.66	47.22									
			Area 43 Employment	28.89								28.89	76.11	76.11	35000	17.55	46.25	46.25	28.89	76.11	76.11	21.31	67.56	*100.21*	*0.88*	*694.0*	*375*	*0.30*	*32.59%*
Totals South Of Queensway To SRPS	**15**	**5A**		**102.31**	**0.00**		**0**		**0**		**0.00**	**102.31**						**54.44**	**102.31**		**102.31**	**58.65**	**113.08**	**203.90**	**1.24**	**230.0**	**450**	**0.47**	**44.54%**
	Queensway	5	Area 13 Community Retail	6.35								6.35	108.66		35000	3.86	58.29		6.35	6.35									
			Area 11/12 Mixed Use	11.80	5.02	50	251	752	752	3.88	11.81	6.79	115.45	115.45	35000	4.12	62.42	62.42	11.80	18.15	120.46	63.73	137.96	*203.90*	*1.24*	*420.0*	*450*	*0.47*	*32.34%*
	5	5A	Area 15 Community Retail	3.88								3.88	119.33		35000	2.36	64.77		3.88	124.34									
			Area 44	25.54							59.98	25.54	144.87	268.20	35000	15.52	81.73	81.73	25.54	149.88	211.06	89.10	230.81	*519.43*	*1.14*	*300.0*	*750*	*0.20*	*55.56%*
				149.88																		63.73	63.73						
Heritage Hills		5A	Area 100 Residential	90.20	90.20	19	1714	5141	5141	3.23	67.35	0.00							90.20										
Heritage Hills		5A	Area 100 Non-Residential	4.88							67.35	4.88	4.88	4.88	50000	4.24	4.24	4.24	4.88	95.08	95.08	26.62	98.21						
Broughton-Richardson / Interstitial		5A																					65.00						
Total To SRPS	**5A**	**SRPS**		**306.14**	**154.03**		**3136**		**9409**		**127.33**	**152.12**						**85.97**			**306.14**	**115.72**	**394.02**	**625.68**	**1.37**	**30.0**	**750**	**0.29**	**37.03%**

Revision No. 1: April 11, 2005
Revision No. 2: April 20, 2005
Revision No. 3: June 07, 2005
Revision No. 4: Oct. 14, 2005
Revision No. 5: Feb. 15, 2006

Average Daily Per capita Flow Rate = 350 l/cap/d
Infiltration Allowance Flow Rate = 0.28 l/sec/Ha
Residential Peaking Factor = 1+(14/(4+(P^0.5))), P=Pop. in 1000's, Max of 4
Population density per unit = 3.00
P. F. For Employment/Retail/Business Park = 1.50
Mixed Uses Assumes: 15% Community Retail, 42.5% Business Park and 42.5% Residential

Note: Sewer from node 5 to SRPS is existing and is to be replaced.

Stantec CCL/IBI

FIG. 4.2-2

Appendix C STORMWATER MANAGEMENT

C.1 STORM SEWER DESIGN SHEET AND ROOF STORAGE CALCULATIONS

Stantec	Wellings of Stittsville Ph 2- 20 Cedarow Court		STORM SEWER DESIGN SHEET (City of Ottawa)
	DATE:	2021-09-01	
	REVISION:	1	
	DESIGNED BY:	TR	FILE NUMBER: 160401511
	CHECKED BY:	-	

DESIGN PARAMETERS

$I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)

	1:2 yr	1:5 yr	1:10 yr	1:100 yr					
a =	732.951	998.071	1174.184	1735.688	MANNING'S n =	0.013		BEDDING CLASS =	B
b =	6.199	6.053	6.014	6.014	MINIMUM COVER:	2.00	m		
c =	0.810	0.814	0.816	0.820	TIME OF ENTRY	10	min		

LOCATION / DRAINAGE AREA

AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (2-YEAR) (ha)	AREA (5-YEAR) (ha)	AREA (10-YEAR) (ha)	AREA (100-YEAR) (ha)	AREA (ROOF) (ha)	C (2-YEAR) (-)	C (5-YEAR) (-)	C (10-YEAR) (-)	C (100-YEAR) (-)	A x C (2-YEAR) (ha)	ACCUM AxC (2YR) (ha)	A x C (5-YEAR) (ha)	ACCUM. AxC (5YR) (ha)	A x C (10-YEAR) (ha)	ACCUM. AxC (10YR) (ha)	A x C (100-YEAR) (ha)	ACCUM. AxC (100YR) (ha)
ROOF 1-ROOF 12, UGPK 1 TO UGPK - 7	BLDG	STM STC 300	0.81	0.00	0.00	0.00	0.76	0.90	0.00	0.00	0.00	0.729	0.729	0.000	0.000	0.000	0.000	0.000	0.000
	STM STC 3(	STM 100	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.729	0.000	0.000	0.000	0.000	0.000	0.000
	STM 100	TANK	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.729	0.000	0.000	0.000	0.000	0.000	0.000
	TANK	STM 101	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.729	0.000	0.000	0.000	0.000	0.000	0.000
	STM 101	EX STM	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.729	0.000	0.000	0.000	0.000	0.000	0.000

DRAINAGE AREA (continued) / PIPE SELECTION

FROM M.H.	TO M.H.	T of C (min)	$I_{2\text{-}YEAR}$ (mm/h)	$I_{5\text{-}YEAR}$ (mm/h)	$I_{10\text{-}YEAR}$ (mm/h)	$I_{100\text{-}YEAR}$ (mm/h)	$Q_{CONTROL}$ (L/s)	ACCUM. $Q_{CONTROL}$ (L/s)	Q_{ACT} (CIA/360) (L/s)	LENGTH (m)	PIPE WIDTH OR DIAMETE (mm)	PIPE HEIGHT (mm)	PIPE SHAPE (-)	MATERIAL (-)	CLASS (-)	SLOPE %	Q_{CAP} (FULL) (L/s)	% FULL (-)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)
BLDG	STM STC 300	10.00	76.81	104.19	122.14	178.56	61.0	61.0	216.5	2.4	450	450	CIRCULAR	CONCRETE	-	1.00	297.4	**72.80%**	1.81	1.73	0.02
STM STC 3(	STM 100	10.02	76.72	104.07	122.00	178.35	0.0	61.0	216.4	2.8	450	450	CIRCULAR	CONCRETE	-	1.00	297.4	**72.74%**	1.81	1.73	0.03
STM 100	TANK	10.05	76.61	103.93	121.83	178.10	0.0	61.0	216.1	8.7	450	450	CIRCULAR	CONCRETE	-	1.00	297.4	**72.67%**	1.81	1.73	0.08
		10.13																			
TANK	STM 101	10.13	76.29	103.49	121.32	177.34	0.0	0.0	125.0	1.5	525	525	CIRCULAR	CONCRETE	-	0.20	200.6	**62.31%**	0.90	0.82	0.03
STM 101	EX STM	10.16	76.18	103.33	121.13	177.07	0.0	0.0	154.3	20.8	525	525	CIRCULAR	CONCRETE	-	0.20	200.6	**76.88%**	0.90	0.87	0.40
		10.56									675	675									

Note:

ICD and weir are proposed to be constructed in STM 101 prior to flows discharging to approved outlet, therefore a 450mm diameter pipe is sufficient as flows will be restricted.

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF1 and 2
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0022	0	0.025	21	0	0	0.025
0.050	0.0006	0.0044	1	0.050	85	1	1	0.050
0.075	0.0008	0.0055	5	0.075	191	3	5	0.075
0.100	0.0009	0.0066	11	0.100	339	7	11	0.100
0.125	0.0011	0.0077	22	0.125	530	11	22	0.125
0.150	0.0013	0.0088	38	0.150	763	16	38	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
1.2	280.0	1.2	0.07778
4.6	608.0	3.4	0.24669
11.1	986.7	6.5	0.52078
21.9	1394.4	10.8	0.90812
38.0	1820.1	16.1	1.41371

Rooftop Storage Summary

Total Building Area (sq.m)		954
Assume Available Roof Area (sq.	80%	763.2
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		7
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		38
Estimated 100 Year Drawdown Time (h)		1.1

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	Open	75%	**50%**	25%	Closed
0.025	0.3155	0.3155	**0.3155**	0.3155	0.3155
0.050	0.6309	0.6309	**0.6309**	0.6309	0.6309
0.075	0.9464	0.8675	**0.7886**	0.7098	0.6309
0.100	1.2618	1.1041	**0.9464**	0.7886	0.6309
0.125	1.5773	1.3407	**1.1041**	0.8675	0.6309
0.150	1.8927	1.5773	**1.2618**	0.9464	0.6309

Calculation Results

	5yr	**100yr**	**Available**
Qresult (cu.m/s)	0.006	0.008	-
Depth (m)	0.077	0.136	0.150
Volume (cu.m)	5.3	29.1	38.2
Draintime (hrs)	0.3	1.1	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF3
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				Water Depth
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	(m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.00126	0	0.025	24	0	0	0.025
0.050	0.0006	0.00252	2	0.050	98	1	2	0.050
0.075	0.0009	0.00379	5	0.075	220	4	5	0.075
0.100	0.0013	0.00505	13	0.100	390	8	13	0.100
0.125	0.0016	0.00631	25	0.125	610	12	25	0.125
0.150	0.0019	0.00757	44	0.150	878	19	44	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
1.4	564.0	1.4	0.15667
5.3	1020.6	3.9	0.44016
12.8	1490.6	7.5	0.85422
25.2	1966.0	12.4	1.40032
43.7	2444.0	18.5	2.07922

Rooftop Storage Summary

Total Building Area (sq.m)		1098
Assume Available Roof Area (sq.	80%	878.4
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		4
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		44
Estimated 100 Year Drawdown Time (h)		1.9

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	**Open**	75%	50%	25%	Closed
0.025	**0.3155**	0.3155	0.3155	0.3155	0.3155
0.050	**0.6309**	0.6309	0.6309	0.6309	0.6309
0.075	**0.9464**	0.8675	0.7886	0.7098	0.6309
0.100	**1.2618**	1.1041	0.9464	0.7886	0.6309
0.125	**1.5773**	1.3407	1.1041	0.8675	0.6309
0.150	**1.8927**	1.5773	1.2618	0.9464	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.004	0.007	-
Depth (m)	0.080	0.142	0.150
Volume (cu.m)	7.0	37.9	43.9
Draintime (hrs)	0.5	1.9	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF4
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0016	0	0.025	8	0	0	0.025
0.050	0.0006	0.0032	1	0.050	31	0	1	0.050
0.075	0.0006	0.0032	2	0.075	70	1	2	0.075
0.100	0.0006	0.0032	4	0.100	125	2	4	0.100
0.125	0.0006	0.0032	8	0.125	196	4	8	0.125
0.150	0.0006	0.0032	14	0.150	282	6	14	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.5	144.6	0.5	0.04018
1.7	392.6	1.2	0.14924
4.1	764.6	2.4	0.36162
8.1	1260.5	4.0	0.71176
14.0	1880.4	5.9	1.23411

Rooftop Storage Summary

Total Building Area (sq.m)		352
Assume Available Roof Area (sq.	80%	281.6
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		5
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		14
Estimated 100 Year Drawdown Time (h)		0.9

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

	Open	75%	50%	25%	**Closed**
0.025	0.3155	0.3155	0.3155	0.3155	**0.3155**
0.050	0.6309	0.6309	0.6309	0.6309	**0.6309**
0.075	0.9464	0.8675	0.7886	0.7098	**0.6309**
0.100	1.2618	1.1041	0.9464	0.7886	**0.6309**
0.125	1.5773	1.3407	1.1041	0.8675	**0.6309**
0.150	1.8927	1.5773	1.2618	0.9464	**0.6309**

Calculation Results

	5yr	**100yr**	**Available**
Qresult (cu.m/s)	0.002	0.003	-
Depth (m)	0.029	0.135	0.150
Volume (cu.m)	0.1	10.5	14.1
Draintime (hrs)	0.0	0.9	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF5
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.00000	0	0.000	0	0	0	0.000
0.025	0.0003	0.00126	0	0.025	24	0	0	0.025
0.050	0.0006	0.00252	2	0.050	98	1	2	0.050
0.075	0.0009	0.00379	5	0.075	220	4	5	0.075
0.100	0.0013	0.00505	13	0.100	390	8	13	0.100
0.125	0.0016	0.00631	25	0.125	610	12	25	0.125
0.150	0.0019	0.00757	44	0.150	878	19	44	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
1.4	564.0	1.4	0.15667
5.3	1020.6	3.9	0.44016
12.8	1490.6	7.5	0.85422
25.2	1966.0	12.4	1.40032
43.7	2444.0	18.5	2.07922

Rooftop Storage Summary

Total Building Area (sq.m)		1098
Assume Available Roof Area (sq.	80%	878.4
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		4
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		44
Estimated 100 Year Drawdown Time (h)		1.9

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

	Open	75%	50%	25%	Closed
0.025	**0.3155**	0.3155	0.3155	0.3155	0.3155
0.050	**0.6309**	0.6309	0.6309	0.6309	0.6309
0.075	**0.9464**	0.8675	0.7886	0.7098	0.6309
0.100	**1.2618**	1.1041	0.9464	0.7886	0.6309
0.125	**1.5773**	1.3407	1.1041	0.8675	0.6309
0.150	**1.8927**	1.5773	1.2618	0.9464	0.6309

Calculation Results	5yr	100yr	Available
Qresult (cu.m/s)	0.004	0.007	-
Depth (m)	0.080	0.142	0.150
Volume (cu.m)	7.0	37.9	43.9
Draintime (hrs)	0.5	1.9	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF6 and 7
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0022	0	0.025	21	0	0	0.025
0.050	0.0006	0.0044	1	0.050	85	1	1	0.050
0.075	0.0008	0.0055	5	0.075	191	3	5	0.075
0.100	0.0009	0.0066	11	0.100	340	7	11	0.100
0.125	0.0011	0.0077	22	0.125	531	11	22	0.125
0.150	0.0013	0.0088	38	0.150	764	16	38	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
1.2	280.3	1.2	0.07787
4.6	608.7	3.4	0.24694
11.1	987.8	6.5	0.52133
21.9	1395.9	10.8	0.90907
38.0	1822.1	16.1	1.41519

Rooftop Storage Summary

Total Building Area (sq.m)		955
Assume Available Roof Area (sq.	80%	764
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		7
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		38
Estimated 100 Year Drawdown Time (h)		1.1

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	Open	75%	50%	25%	Closed
0.025	0.3155	0.3155	**0.3155**	0.3155	0.3155
0.050	0.6309	0.6309	**0.6309**	0.6309	0.6309
0.075	0.9464	0.8675	**0.7886**	0.7098	0.6309
0.100	1.2618	1.1041	**0.9464**	0.7886	0.6309
0.125	1.5773	1.3407	**1.1041**	0.8675	0.6309
0.150	1.8927	1.5773	**1.2618**	0.9464	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.006	0.008	-
Depth (m)	0.077	0.136	0.150
Volume (cu.m)	5.3	29.1	38.2
Draintime (hrs)	0.3	1.1	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF8
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	2	0	0	0.025
0.050	0.0006	0.0013	0	0.050	9	0	0	0.050
0.075	0.0007	0.0014	0	0.075	19	0	0	0.075
0.100	0.0008	0.0016	1	0.100	34	1	1	0.100
0.125	0.0009	0.0017	2	0.125	54	1	2	0.125
0.150	0.0009	0.0019	4	0.150	78	2	4	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.1	99.7	0.1	0.02768
0.5	240.4	0.3	0.09447
1.1	421.4	0.7	0.21152
2.2	631.6	1.1	0.38695
3.9	863.6	1.6	0.62685

Rooftop Storage Summary

Total Building Area (sq.m)		97
Assume Available Roof Area (sq.	80%	77.6
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		4
Estimated 100 Year Drawdown Time (h)		0.3

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

	Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.6309
0.075	0.9464	0.8675	0.7886	**0.7098**	0.6309
0.100	1.2618	1.1041	0.9464	**0.7886**	0.6309
0.125	1.5773	1.3407	1.1041	**0.8675**	0.6309
0.150	1.8927	1.5773	1.2618	**0.9464**	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.000	0.002	-
Depth (m)	0.000	0.117	0.150
Volume (cu.m)	0.0	1.9	3.9
Draintime (hrs)	0.0	0.3	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF9
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	1	0	0	0.025
0.050	0.0006	0.0013	0	0.050	5	0	0	0.050
0.075	0.0007	0.0014	0	0.075	11	0	0	0.075
0.100	0.0008	0.0016	1	0.100	19	0	1	0.100
0.125	0.0009	0.0017	1	0.125	29	1	1	0.125
0.150	0.0009	0.0019	2	0.150	42	1	2	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.1	54.4	0.1	0.01512
0.3	131.4	0.2	0.05162
0.6	230.2	0.4	0.11557
1.2	345.1	0.6	0.21143
2.1	471.9	0.9	0.34251

Rooftop Storage Summary

Total Building Area (sq.m)		53
Assume Available Roof Area (sq.	80%	42.4
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		2
Estimated 100 Year Drawdown Time (h)		0.1

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.6309
0.075	0.9464	0.8675	0.7886	**0.7098**	0.6309
0.100	1.2618	1.1041	0.9464	**0.7886**	0.6309
0.125	1.5773	1.3407	1.1041	**0.8675**	0.6309
0.150	1.8927	1.5773	1.2618	**0.9464**	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.000	0.002	-
Depth (m)	0.000	0.100	0.150
Volume (cu.m)	0.0	0.6	2.1
Draintime (hrs)	0.0	0.1	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF10
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0066	1	0.025	62	1	1	0.025
0.050	0.0006	0.0132	4	0.050	250	4	4	0.050
0.075	0.0007	0.0149	14	0.075	562	10	14	0.075
0.100	0.0008	0.0166	33	0.100	999	19	33	0.100
0.125	0.0009	0.0182	65	0.125	1561	32	65	0.125
0.150	0.0009	0.0199	112	0.150	2248	47	112	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
3.6	274.9	3.6	0.07637
13.5	663.3	9.9	0.26063
32.8	1162.6	19.3	0.58357
64.5	1742.4	31.7	1.06758
111.9	2382.8	47.4	1.72946

Rooftop Storage Summary

Total Building Area (sq.m)		2810
Assume Available Roof Area (sq.	80%	2248
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		21
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		112
Estimated 100 Year Drawdown Time (h)		1.5

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.6309
0.075	0.9464	0.8675	0.7886	**0.7098**	0.6309
0.100	1.2618	1.1041	0.9464	**0.7886**	0.6309
0.125	1.5773	1.3407	1.1041	**0.8675**	0.6309
0.150	1.8927	1.5773	1.2618	**0.9464**	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.017	0.019	-
Depth (m)	0.100	0.141	0.150
Volume (cu.m)	33.1	94.7	112.4
Draintime (hrs)	0.6	1.5	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF11
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	2	0	0	0.025
0.050	0.0006	0.0013	0	0.050	9	0	0	0.050
0.075	0.0007	0.0014	1	0.075	21	0	1	0.075
0.100	0.0008	0.0016	1	0.100	38	1	1	0.100
0.125	0.0009	0.0017	2	0.125	59	1	2	0.125
0.150	0.0009	0.0019	4	0.150	85	2	4	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.1	108.9	0.1	0.03025
0.5	262.7	0.4	0.10323
1.2	460.5	0.7	0.23114
2.4	690.2	1.2	0.42285
4.2	943.8	1.8	0.68501

Rooftop Storage Summary

Total Building Area (sq.m)		106
Assume Available Roof Area (sq.	80%	84.8
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		4
Estimated 100 Year Drawdown Time (h)		0.4

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

Head (m)	Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.6309
0.075	0.9464	0.8675	0.7886	**0.7098**	0.6309
0.100	1.2618	1.1041	0.9464	**0.7886**	0.6309
0.125	1.5773	1.3407	1.1041	**0.8675**	0.6309
0.150	1.8927	1.5773	1.2618	**0.9464**	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.000	0.002	-
Depth (m)	0.000	0.120	0.150
Volume (cu.m)	0.0	2.2	4.2
Draintime (hrs)	0.0	0.4	

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Roof Drain Design Sheet, Area ROOF12
Standard Watts Model R1100 Accutrol Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0006	0	0.025	2	0	0	0.025
0.050	0.0006	0.0013	0	0.050	9	0	0	0.050
0.075	0.0007	0.0014	1	0.075	20	0	1	0.075
0.100	0.0008	0.0016	1	0.100	36	1	1	0.100
0.125	0.0009	0.0017	2	0.125	56	1	2	0.125
0.150	0.0009	0.0019	4	0.150	80	2	4	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.1	102.7	0.1	0.02854
0.5	247.9	0.4	0.09739
1.2	434.4	0.7	0.21806
2.3	651.1	1.1	0.39892
4.0	890.4	1.7	0.64624

Rooftop Storage Summary

Total Building Area (sq.m)		100
Assume Available Roof Area (sq.	80%	80
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		4
Estimated 100 Year Drawdown Time (h)		0.3

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Watts Drain Catalogue

Head (m) L/s

	Open	75%	50%	**25%**	Closed
0.025	0.3155	0.3155	0.3155	**0.3155**	0.3155
0.050	0.6309	0.6309	0.6309	**0.6309**	0.6309
0.075	0.9464	0.8675	0.7886	**0.7098**	0.6309
0.100	1.2618	1.1041	0.9464	**0.7886**	0.6309
0.125	1.5773	1.3407	1.1041	**0.8675**	0.6309
0.150	1.8927	1.5773	1.2618	**0.9464**	0.6309

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.001	0.002	-
Depth (m)	0.073	0.118	0.150
Volume (cu.m)	0.5	2.0	4.0
Draintime (hrs)	0.1	0.3	

Stormwater Management Calculations

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Modified Rational Method Calculatons for Storage

100 yr Intensity
City of Ottawa

$I = a/(t + b)^c$

a =	1735.688
b =	6.014
c =	0.820

t (min)	I (mm/hr)
10	178.56
20	119.95
30	91.87
40	75.15
50	63.95
60	55.89
70	49.79
80	44.99
90	41.11
100	37.90
110	35.20
120	32.89

100 YEAR Modified Rational Method for Entire Site

Subdrainage Area: ROOF12 — Roof
Area (ha): 0.01 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	4.96	1.69	3.28	1.97	117.3	0.00
20	119.95	3.33	1.69	1.65	**1.98**	117.5	0.00
30	91.87	2.55	1.64	0.91	1.64	110.1	0.00
40	75.15	2.09	1.58	0.51	1.22	100.7	0.00
50	63.95	1.78	1.50	0.28	0.84	87.4	0.00
60	55.89	1.55	1.42	0.14	0.49	74.6	0.00
70	49.79	1.38	1.32	0.07	0.28	59.0	0.00
80	44.99	1.25	1.22	0.03	0.14	48.4	0.00
90	41.11	1.14	1.12	0.02	0.12	44.4	0.00
100	37.90	1.05	1.04	0.02	0.10	41.1	0.00
110	35.20	0.98	0.97	0.01	0.09	38.3	0.00
120	32.89	0.91	0.90	0.01	0.07	35.8	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	117.51	0.12	1.69	1.98	4.00	0.00

Subdrainage Area: ROOF11 — Roof
Area (ha): 0.01 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	5.26	1.69	3.57	2.14	118.5	0.00
20	119.95	3.53	1.70	1.83	**2.20**	119.7	0.00
30	91.87	2.71	1.66	1.05	1.88	113.1	0.00
40	75.15	2.21	1.60	0.61	1.46	104.3	0.00
50	63.95	1.88	1.53	0.35	1.05	93.0	0.00
60	55.89	1.65	1.45	0.19	0.69	80.6	0.00
70	49.79	1.47	1.37	0.10	0.41	67.0	0.00
80	44.99	1.33	1.28	0.04	0.21	53.3	0.00
90	41.11	1.21	1.19	0.03	0.14	47.0	0.00
100	37.90	1.12	1.10	0.02	0.12	43.5	0.00
110	35.20	1.04	1.02	0.02	0.10	40.5	0.00
120	32.89	0.97	0.96	0.01	0.09	37.9	0.00

Stormwater Management Calculations

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Modified Rational Method Calculatons for Storage

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	119.70	0.12	1.70	2.20	4.24	0.00

Subdrainage Area: ROOF10 Roof
Area (ha): 0.28 Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	139.49	18.48	121.01	72.60	129.0	0.00
20	119.95	93.70	19.07	74.63	89.55	137.9	0.00
30	91.87	71.77	19.25	52.52	94.53	140.6	0.00
40	75.15	58.70	19.25	39.45	**94.68**	140.6	0.00
50	63.95	49.96	19.17	30.79	92.36	139.4	0.00
60	55.89	43.66	19.04	24.62	88.64	137.5	0.00
70	49.79	38.89	18.88	20.01	84.05	135.0	0.00
80	44.99	35.15	18.70	16.44	78.93	132.3	0.00
90	41.11	32.12	18.51	13.60	73.46	129.4	0.00
100	37.90	29.61	18.31	11.30	67.78	126.4	0.00
110	35.20	27.50	18.07	9.43	62.23	122.8	0.00
120	32.89	25.70	17.79	7.90	56.91	118.6	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	140.64	0.14	19.25	94.68	112.40	0.00

Subdrainage Area: ROOF9 Roof
Area (ha): 0.01 Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	2.63	1.58	1.05	**0.63**	100.1	0.00
20	119.95	1.77	1.46	0.30	0.37	81.9	0.00
30	91.87	1.35	1.29	0.06	0.11	54.6	0.00
40	75.15	1.11	1.08	0.02	0.06	42.9	0.00
50	63.95	0.94	0.93	0.01	0.04	36.8	0.00
60	55.89	0.82	0.82	0.01	0.03	32.3	0.00
70	49.79	0.73	0.73	0.00	0.02	28.9	0.00
80	44.99	0.66	0.66	0.00	0.01	26.2	0.00
90	41.11	0.61	0.60	0.00	0.01	23.9	0.00
100	37.90	0.56	0.56	0.00	0.01	22.1	0.00
110	35.20	0.52	0.52	0.00	0.01	20.5	0.00
120	32.89	0.48	0.48	0.00	0.01	19.2	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	100.15	0.10	1.58	0.63	2.12	0.00

Subdrainage Area: ROOF8 Roof
Area (ha): 0.01 Maximum Storage Depth: 150 mm
C: 1.00

tc	I (100 yr)	Qactual	Qrelease	Qstored	Vstored	Depth

Stormwater Management Calculations

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Modified Rational Method Calculatons for Storage

(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m^3)	(mm)	
10	178.56	4.82	1.68	3.13	**1.88**	116.7	0.00
20	119.95	3.23	1.68	1.55	1.87	116.3	0.00
30	91.87	2.48	1.63	0.85	1.52	108.5	0.00
40	75.15	2.03	1.57	0.46	1.10	98.3	0.00
50	63.95	1.72	1.48	0.25	0.74	84.5	0.00
60	55.89	1.51	1.39	0.12	0.42	70.3	0.00
70	49.79	1.34	1.29	0.05	0.21	54.9	0.00
80	44.99	1.21	1.19	0.03	0.13	47.0	0.00
90	41.11	1.11	1.09	0.02	0.11	43.1	0.00
100	37.90	1.02	1.01	0.02	0.09	39.9	0.00
110	35.20	0.95	0.94	0.01	0.08	37.1	0.00
120	32.89	0.89	0.88	0.01	0.07	34.8	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	116.66	0.12	1.68	1.88	3.88	0.00

Subdrainage Area: ROOF6 and 7 — Roof
Area (ha): 0.10 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	47.41	7.84	39.57	23.74	127.5	0.00
20	119.95	31.85	8.16	23.69	28.42	134.8	0.00
30	91.87	24.39	8.21	16.18	**29.12**	135.9	0.00
40	75.15	19.95	8.15	11.80	28.32	134.7	0.00
50	63.95	16.98	8.05	8.93	26.79	132.3	0.00
60	55.89	14.84	7.92	6.92	24.91	129.4	0.00
70	49.79	13.22	7.78	5.44	22.84	126.1	0.00
80	44.99	11.94	7.60	4.34	20.85	122.1	0.00
90	41.11	10.91	7.40	3.51	18.96	117.7	0.00
100	37.90	10.06	7.21	2.85	17.09	113.4	0.00
110	35.20	9.35	7.03	2.32	15.28	109.2	0.00
120	32.89	8.73	6.85	1.88	13.56	105.2	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	135.90	0.14	8.21	29.12	38.20	0.00

Subdrainage Area: ROOF5 — Roof
Area (ha): 0.11 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	54.50	6.54	47.96	28.78	129.5	0.00
20	119.95	36.61	7.00	29.61	35.54	138.7	0.00
30	91.87	28.04	7.14	20.90	37.62	141.5	0.00
40	75.15	22.94	7.16	15.78	**37.87**	141.8	0.00
50	63.95	19.52	7.11	12.41	37.23	141.0	0.00
60	55.89	17.06	7.04	10.02	36.08	139.4	0.00
70	49.79	15.20	6.94	8.26	34.68	137.5	0.00
80	44.99	13.73	6.83	6.90	33.12	135.4	0.00
90	41.11	12.55	6.72	5.83	31.48	133.2	0.00
100	37.90	11.57	6.61	4.96	29.76	130.9	0.00
110	35.20	10.75	6.49	4.26	28.09	128.6	0.00

Stormwater Management Calculations

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Modified Rational Method Calculatons for Storage

120	32.89	10.04	6.37	3.67	26.40	126.3	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	141.83	0.14	7.16	37.87	43.92	0.00

Subdrainage Area: ROOF4 Roof
Area (ha): 0.04 Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	17.47	3.15	14.32	8.59	126.9	0.00
20	119.95	11.74	3.15	8.59	10.31	134.1	0.00
30	91.87	8.99	3.15	5.84	**10.51**	135.0	0.00
40	75.15	7.35	3.15	4.20	10.09	133.2	0.00
50	63.95	6.26	3.15	3.11	9.32	130.0	0.00
60	55.89	5.47	3.15	2.32	8.35	125.9	0.00
70	49.79	4.87	3.15	1.72	7.23	119.2	0.00
80	44.99	4.40	3.15	1.25	6.01	111.6	0.00
90	41.11	4.02	3.15	0.87	4.71	103.4	0.00
100	37.90	3.71	3.15	0.56	3.35	91.5	0.00
110	35.20	3.44	3.15	0.29	1.95	76.9	0.00
120	32.89	3.22	3.15	0.07	0.50	48.6	-0.08

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	134.96	0.13	3.15	10.51	14.08	0.00

Subdrainage Area: ROOF3 Roof
Area (ha): 0.11 Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	54.50	6.54	47.96	28.78	129.5	0.00
20	119.95	36.61	7.00	29.61	35.54	138.7	0.00
30	91.87	28.04	7.14	20.90	37.62	141.5	0.00
40	75.15	22.94	7.16	15.78	**37.87**	141.8	0.00
50	63.95	19.52	7.11	12.41	37.23	141.0	0.00
60	55.89	17.06	7.04	10.02	36.08	139.4	0.00
70	49.79	15.20	6.94	8.26	34.68	137.5	0.00
80	44.99	13.73	6.83	6.90	33.12	135.4	0.00
90	41.11	12.55	6.72	5.83	31.48	133.2	0.00
100	37.90	11.57	6.61	4.96	29.76	130.9	0.00
110	35.20	10.75	6.49	4.26	28.09	128.6	0.00
120	32.89	10.04	6.37	3.67	26.40	126.3	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	141.83	0.14	7.16	37.87	43.92	0.00

Subdrainage Area: ROOF1 and 2 Roof
Area (ha): 0.10 Maximum Storage Depth: 150 mm
C: 1.00

Stormwater Management Calculations

Project #160401511, Wellings of Stittsville Phase 2, 20 Cedarow Court
Modified Rational Method Calculatons for Storage

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	47.36	7.84	39.52	23.71	127.5	0.00
20	119.95	31.81	8.16	23.65	28.38	134.8	0.00
30	91.87	24.36	8.21	16.16	**29.08**	135.9	0.00
40	75.15	19.93	8.15	11.78	28.27	134.6	0.00
50	63.95	16.96	8.05	8.91	26.73	132.2	0.00
60	55.89	14.82	7.92	6.90	24.85	129.3	0.00
70	49.79	13.20	7.78	5.42	22.78	126.1	0.00
80	44.99	11.93	7.60	4.33	20.79	122.0	0.00
90	41.11	10.90	7.40	3.50	18.92	117.7	0.00
100	37.90	10.05	7.21	2.84	17.05	113.3	0.00
110	35.20	9.34	7.03	2.31	15.22	109.1	0.00
120	32.89	8.72	6.85	1.87	13.49	105.1	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	135.88	0.14	8.21	29.08	38.16	0.00

WATTS DRAINAGE	Adjustable Accutrol Weir Tag: ____________	Adjustable Flow Control for Roof Drains

ADJUSTABLE ACCUTROL(for Large Sump Roof Drains only)

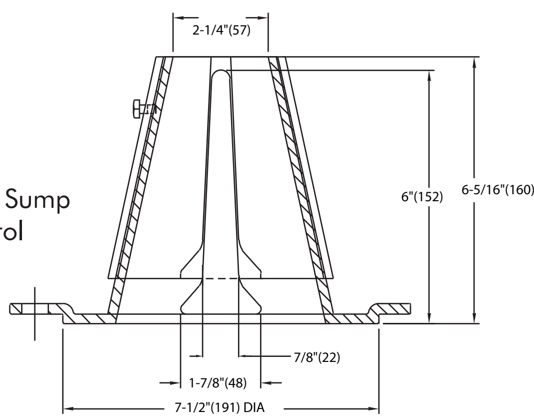
For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below.
Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

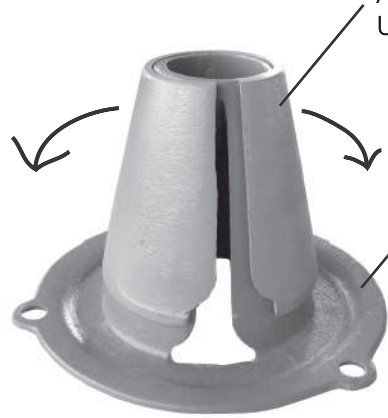
EXAMPLE:

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be:
[5 gpm(per inch of head) x 2 inches of head] + 2-1/2 gpm(for the third inch of head) = 12-1/2 gpm.

Large Sump
Accutrol





1/2 Weir Opening Exposed Shown Above

TABLE 1. Adjustable Accutrol Flow Rate Settings

Weir Opening Exposed	Head of Water 1"	2"	3"	4"	5"	6"
	Flow Rate (gallons per minute)					
Fully Exposed	5	10	15	20	25	30
3/4	5	10	13.75	17.5	21.25	25
1/2	5	10	12.5	15	17.5	20
1/4	5	10	11.25	12.5	13.75	15
Closed	5	10	10	10	10	10

Job Name ____________ Contractor ____________

Job Location ____________ Contractor's P.O. No. ____________

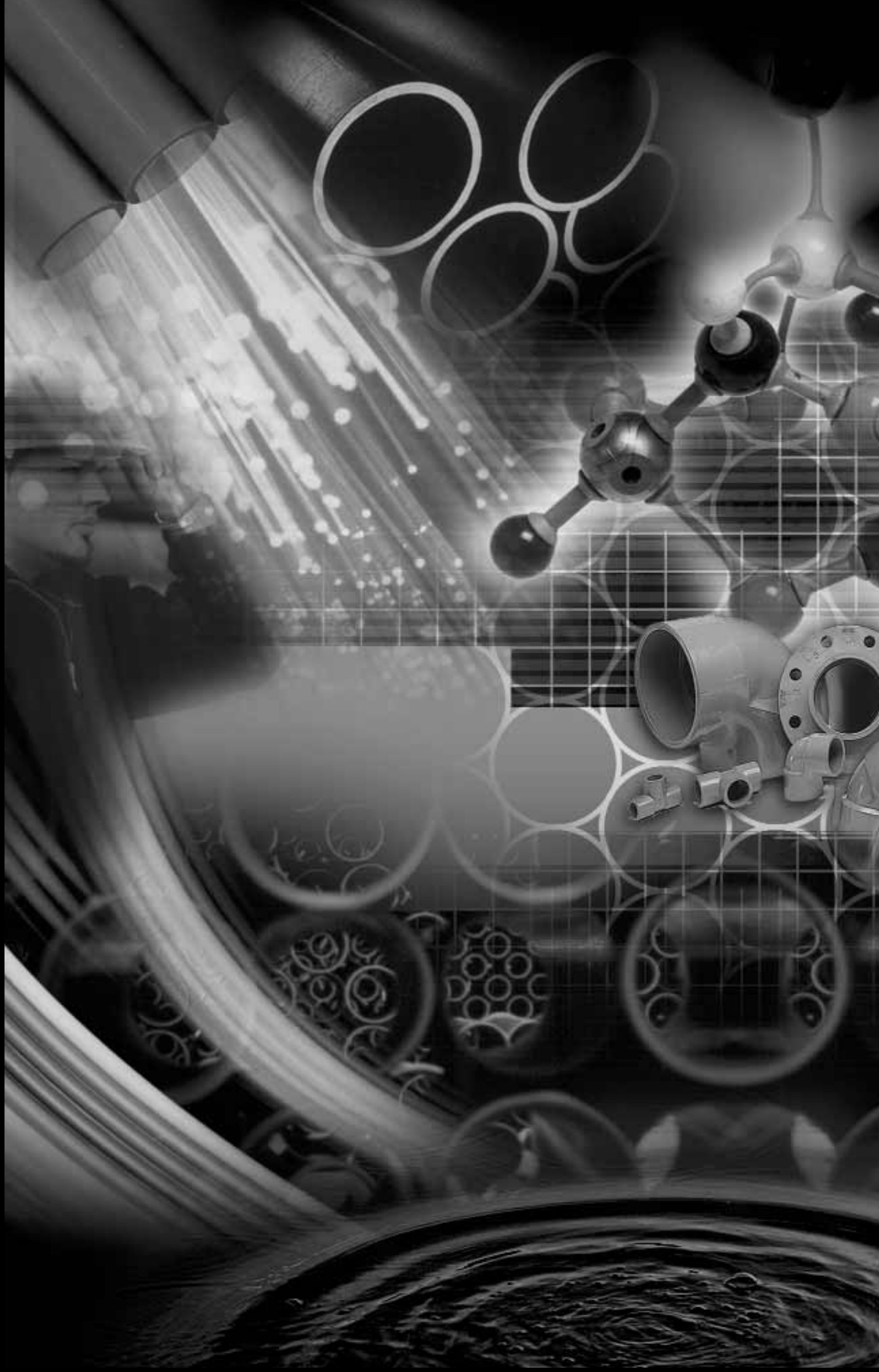
Engineer ____________ Representative ____________

CANADA

WATTS DRAINAGE

CANADA: 5435 North Service Road, Burlington, ON, L7L 5H7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Website: www.wattsdrainage.ca

Volume III: TEMPEST™ INLET CONTROL DEVICES



Municipal Technical Manual Series

FIRST EDITION

LMF (Low to Medium Flow) ICD

HF (High Flow) ICD

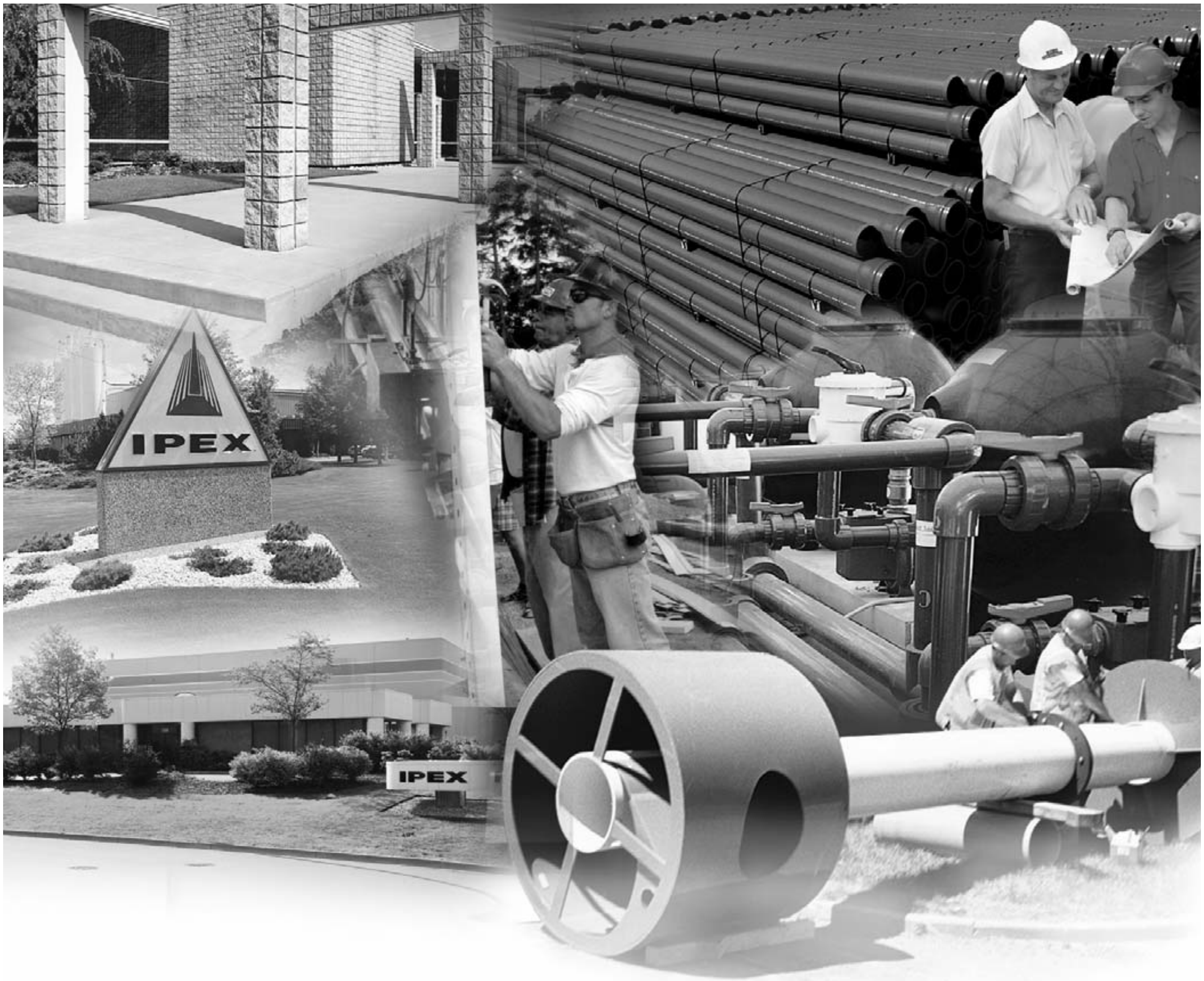
MHF (Medium to High Flow) ICD

IPEX

IPEX Tempest™ Inlet Control Devices

Municipal Technical Manual Series

Vol. I, 1st Edition



About IPEX

At IPEX, we have been manufacturing non-metallic pipe and fittings since 1951. We formulate our own compounds and maintain strict quality control during production. Our products are made available for customers thanks to a network of regional stocking locations throughout North America. We offer a wide variety of systems including complete lines of piping, fittings, valves and custom-fabricated items.

More importantly, we are committed to meeting our customers' needs. As a leader in the plastic piping industry, IPEX continually develops new products, modernizes manufacturing facilities and acquires innovative process technology. In addition, our staff take pride in their work, making available to customers their extensive thermoplastic knowledge and field experience. IPEX personnel are committed to improving the safety, reliability and performance of thermoplastic materials. We are involved in several standards committees and are members of and/or comply with the organizations listed on this page.

For specific details about any IPEX product, contact our customer service department.

CONTENTS

PRODUCT INFORMATION: TEMPEST LOW, MEDIUM FLOW (LMF) ICD

Purpose

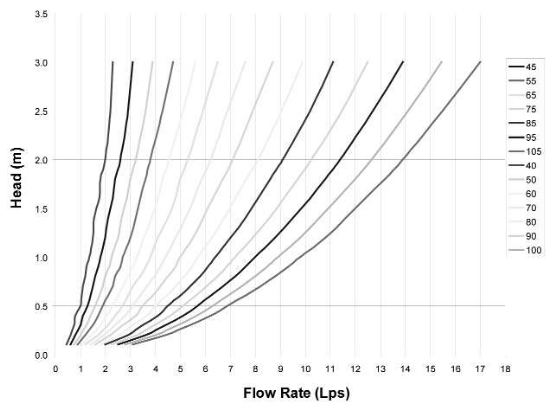
To control the amount of storm water runoff entering a sewer system by allowing a specified flow volume out of a catch basin or manhole at a specified head. This approach conserves pipe capacity so that catch basins downstream do not become uncontrollably surcharged, which can lead to basement floods, flash floods and combined sewer overflows.

Product Description

Our LMF ICD is designed to accommodate catch basins or manholes with sewer outlet pipes 6" in diameter and larger. Any storm sewer larger than 12" may require custom modification. However, IPEX can custom build a TEMPEST device to accommodate virtually any storm sewer size.

Available in 14 preset flow curves, the LMF ICD has the ability to provide flow rates: 2lps – 17lps (31gpm – 270gpm)

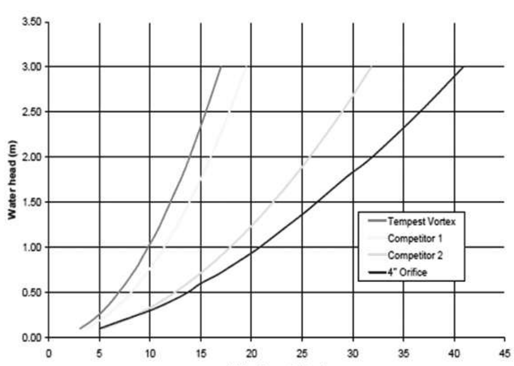
LMF 14 Preset Flow Curves



Product Function

The LMF ICD vortex flow action allows the LMF ICD to provide a narrower flow curve using a larger orifice than a conventional orifice plate ICD, making it less likely to clog. When comparing flows at the same head level, the LMF ICD has the ability to restrict more flow than a conventional ICD during a rain event, preserving greater sewer capacity.

LMF Flow vs. ICD Alternatives



Product Construction

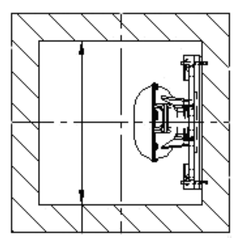
Constructed from durable PVC, the LMF ICD is light weight 8.95 Kg (19.7 lbs).

Product Applications

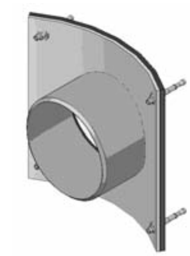
Will accommodate both square and round applications:

Square Application

Universal Mounting Plate



Round Application

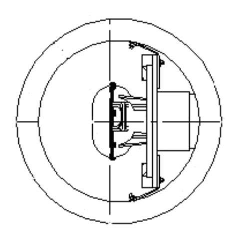


Spigot CB Wall Plate

+

Universal Mounting Plate Hub Adapter

=



PRODUCT INSTALLATION

Instructions to assemble a TEMPEST LMF ICD into a Square Catch Basin:

STEPS:

1. Materials and tooling verification:
   - Tooling: impact drill, 3/8" concrete bit, torque wrench for 9/16" nut, hand hammer, level, and marker.
   - Material: (4) concrete anchor 3/8 x 3-1/2, (4) washers, (4) nuts, universal mounting plate, ICD device.
2. Use the mounting wall plate to locate and mark the hole (4) pattern on the catch basin wall. You should use a level to ensure that the plate is at the horizontal.
3. Use an impact drill with a 3/8" concrete bit to make the four holes at a minimum of 1-1/2" depth up to 2-1/2". Clean the concrete dust from the holes.
4. Install the anchors (4) in the holes by using a hammer. Thread the nuts on the top of the anchors to protect the threads when you will hit the anchors with the hammer. Remove the nuts the ends of the anchors
5. Install the universal mounting plate on the anchors and screw the 4 nuts in place with a maximum torque of 40 N.m (30 lbf-ft). There should be no gap between the wall mounting plate and the catch basin wall.
6. From the ground above using a reach bar, lower the ICD device by hooking the end of the reach bar to the handle of the ICD device. Align the triangular plate portion into the mounting wall plate. Push down the device to be sure it has centered in to the universal mounting plate and has created a seal.

WARNING

- Verify that the outlet pipe doesn’t protrude into the catch basin. If it does, cut down the pipe flush to the catch basin wall.
- Call your IPEX representative for more information or if you have any questions about our products.

Instructions to assemble a TEMPEST LMF ICD into a Round Catch Basin:

STEPS:

1. Materials and tooling verification.
   - Tooling: impact drill, 3/8" concrete bit, torque wrench for 9/16" nut, hand hammer, level and marker.
   - Material: (4) concrete anchor 3/8 x 3-1/2, (4) washers and (4) nuts, spigot CB wall plate, universal mounting plate hub adapter, ICD device.
2. Use the spigot catch basin wall plate to locate and mark the hole (4) pattern on the catch basin wall. You should use a level to sure that the plate is at the horizontal.
3. Use an impact drill with a 3/8" concrete bit to make the four holes at a depth between 1-1/2" to 2-1/2". Clean the concrete dust from the holes.
4. Install the anchors (4) in the holes by using a hammer. Thread the nuts on the top of the anchors to protect the threads when you will hit the anchors with the hammer. Remove the nuts from the ends of the anchors
5. Install the CB spigot wall plate on the anchors and screw the 4 nuts in place with a maximum torque of 40 N.m (30 lbf-ft). There should be no gap between CB the spigot wall plate and the catch basin wall.
6. Apply solvent cement on the hub of universal mounting plate, hub adapter and the spigot of spigot CB wall plate slide the hub over the spigot. Make sure the universal mounting plate is at the horizontal and its hub is completely inserted onto the spigot. Normally, the corners of the universal mounting plate hub adapter should touch the catch basin wall.
7. From ground above using a reach bar, lower the ICD device by hooking the end of the reach bar to the handle of the ICD device. Align the triangular plate portion into the mounting wall plate. Push down the device to be sure it has centered in to the mounting plate and has created a seal.

WARNING

- Verify that the outlet pipe doesn’t protrude into the catch basin. If it does, cut back the pipe flush to the catch basin wall.
- The solvent cement which is used in this installation is to be approved for PVC.
- The solvent cement should not be used below 0°C (32°F) or in a high humidity environment. Refer to the IPEX solvent cement guide to confirm the required curing time or visit the IPEX Online Solvent Cement Training Course available at www.ipexinc.com.
- Call your IPEX representative for more information or if you have any questions about our products.

PRODUCT TECHNICAL SPECIFICATION

General

Inlet control devices (ICD's) are designed to provide flow control at a specified rate for a given water head level and also provide odour and floatable control. All ICD's will be IPEX Tempest or approved equal.

All devices shall be removable from a universal mounting plate. An operator from street level using only a T-bar with a hook will be able to retrieve the device while leaving the universal mounting plate secured to the catch basin wall face. The removal of the TEMPEST devices listed above must not require any unbolting or special manipulation or any special tools.

High Flow (HF) Sump devices will consist of a removable threaded cap which can be accessible from street level with out entry into the catchbasin (CB). The removal of the threaded cap shall not require any special tools other than the operator's hand.

ICD's must have no moving parts.

Materials

ICD's are to be manufactured from Polyvinyl Chloride (PVC) or Polyurethane material, designed to be durable enough to withstand multiple freeze-thaw cycles and exposure to harsh elements.

The inner ring seal will be manufactured using a Buna or Nitrile material with hardness between Duro 50 and Duro 70.

The wall seal is to be comprised of a 3/8" thick Neoprene Closed Cell Sponge gasket which is attached to the back of the wall plate.

All hardware will be made from 304 stainless steel.

Dimensioning

The Low Medium Flow (LMF), High Flow (HF) and the High Flow (HF) Sump shall allow for a minimum outlet pipe diameter of 200mm with a 600mm deep Catch Basin sump.

Installation

Contractor shall be responsible for securing, supporting and connecting the ICD's to the existing influent pipe and catchbasin/manhole structure as specified and designed by the Engineer.

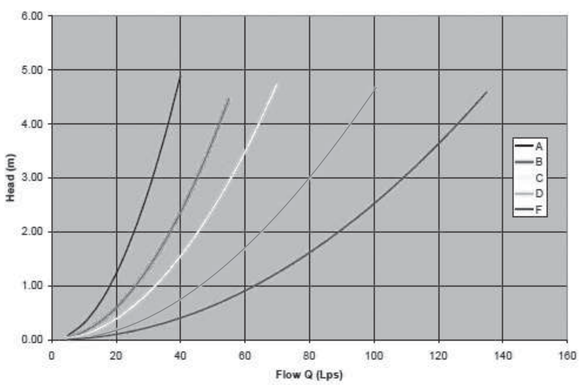
PRODUCT INFORMATION: TEMPEST HF & MHF ICD

Product Description

Our HF, HF Sump and MHF ICD is designed to accommodate catch basins or manholes with sewer outlet pipes 6" in diameter or larger. Any storm sewer larger than 12" may require custom modification. However, IPEX can custom build a TEMPEST device virtually to accommodate any storm sewer size.

Available in 5 preset flow curves, these ICDs have the ability to provide constant flow rates: 9lps (143 gpm) and greater

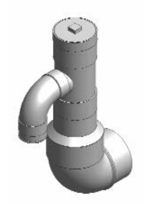
HF & MHF Preset Flow Curves



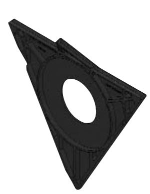
Product Function

TEMPEST HF (High Flow): designed to manage moderate to higher flows between 15 L/s (240 gpm) or greater and prevents the propagation of odour and floatables. With this device, the cross-sectional area of the device is larger than the orifice diameter and has been designed to limit head losses. The HF ICD can also be ordered without flow control when only odour and floatable control is required.

TEMPEST HF (High Flow) Sump: The height of a sewer outlet pipe in a catch basin is not always conveniently located. At times it may be located very close to the catch basin floor, not providing enough sump for one of the other TEMPEST ICDs with universal back plate to be installed. In these applications, a HF Sump is offered. The HF Sump offers the same features and benefits as the HF ICD; however, is designed to raise the outlet in a square or round catch basin structure. When installed, the HF sump is fixed in place and not easily removed. Any required service to the device is performed through a clean-out located in the top of the device which can be often accessed from ground level.



TEMPEST MHF (Medium to High Flow): The MHF plate or plug is designed to control flow rates 9 L/s (143 gpm) or greater. It is not designed to prevent the propagation of odour and floatables.

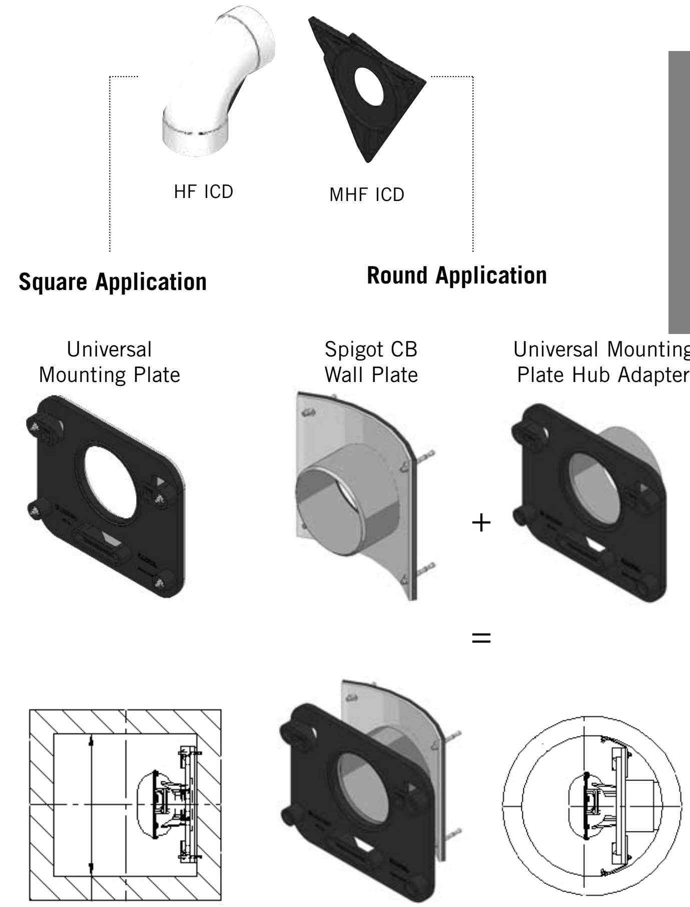


Product Construction

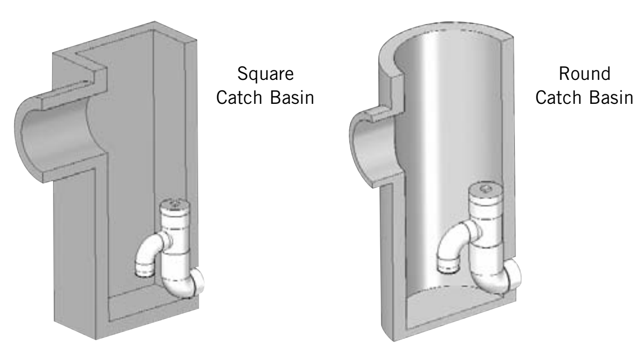
The HF, HF Sump and MHF ICDs are built to be light weight at a maximum weight of 6.82 Kg (14.6 lbs).

Product Applications

The HF and MHF ICD are available to accommodate both square and round applications:



The HF Sump is available to accommodate low to no sump applications in both square and round catch basins:



NOTE: Do not use or test the products in this manual with compressed air or other gases including air-over-water-boosters

PRODUCT INSTALLATION

Instructions to assemble a TEMPEST HF or MHF ICD into a Square Catch Basin:

1. Materials and tooling verification:
   - Tooling: impact drill, 3/8" concrete bit, torque wrench for 9/16" nut, hand hammer, level, and marker.
   - Material: (4) concrete anchor 3/8 x 3-1/2, (4) washers, (4) nuts, universal mounting plate, ICD device
2. Use the mounting wall plate to locate and mark the hole (4) pattern on the catch basin wall. You should use a level to ensure that the plate is at the horizontal.
3. Use an impact drill with a 3/8" concrete bit to make the four holes at a minimum of 1-1/2" depth up to 2-1/2". Clean the concrete dust from the holes.
4. Install the anchors (4) in the holes by using a hammer. Thread the nuts on the top of the anchors to protect the threads when you will hit the anchors with the hammer. Remove the nuts the ends of the anchors
5. Install the universal from wall mounting plate on the anchors and screw the 4 nuts in place with a maximum torque of 40 N.m (30 lbf-ft). There should be no gap between the wall mounting plate and the catch basin wall.
6. From the ground above using a reach bar, lower the device by hooking the end of the reach bar to the handle of the LMF device. Align the triangular plate portion into the mounting wall plate. Push down the device to be sure it has centered in to the universal wall mounting plate and has created a seal.

WARNING

- Verify that the outlet pipe doesn’t protrude into the catch basin. If it does, cut down the pipe flush to the catch basin wall.
- Call your IPEX representative for more information or if you have any questions about our products.

Instructions to assemble a TEMPEST HF or MHF ICD into a Round Catch Basin:

STEPS:

1. Materials and tooling verification.
   - Tooling: impact drill, 3/8" concrete bit, torque wrench for 9/16" nut, hand hammer, level and marker.
   - Material: (4) concrete anchor 3/8 x 3-1/2, (4) washers and (4) nuts, spigot CB wall plate, universal mounting plate hub adapter, ICD device.
2. Use the round catch basin spigot adaptor to locate and mark the hole (4) pattern on the catch basin wall. You should use a level to sure that the plate is at the horizontal.
3. Use an impact drill with a 3/8" concrete bit to make the four holes at a depth between 1-1/2" to 2-1/2". Clean the concrete dust from the holes.
4. Install the anchors (4) in the holes by using a hammer. Thread the nuts on the top of the anchors to protect the threads when you will hit the anchors with the hammer. Remove the nuts from the ends of the anchors
5. Install the spigot CB wall plate on the anchors and screw the 4 nuts in place with a maximum torque of 40 N.m (30 lbf-ft). There should be no gap between the spigot CB wall plate and the catch basin wall.
6. Put solvent cement on the hub of the universal mounting plate, hub adapter and the spigot of spigot CB wall plate and slide the hub over the spigot. Make sure the universal mounting plate is at the horizontal and its hub is completely inserted onto the spigot. Normally, the corners of the hub adapter should touch the catch basin wall.
7. From ground above using a reach bar, lower the ICD device by hooking the end of the reach bar to the handle of the ICD device. Align the triangular plate portion into the mounting wall plate. Push down the device to be sure it has centered in to the wall mounting plate and has created a seal.

WARNING

- Verify that the outlet pipe doesn’t protrude into the catch basin. If it does, cut down the pipe flush to the catch basin wall.
- The solvent cement which is used in this installation is to be approved for PVC.
- The solvent cement should not be used below 0°C (32°F) or in a high humidity environment. Refer to the IPEX solvent cement guide to confirm the required curing time or visit the IPEX Online Solvent Cement Training Course available at www.ipexinc.com.
- Call your IPEX representative for more information or if you have any questions about our products.

Instructions to assemble a TEMPEST HF Sump into a Square or Round Catch Basin:

STEPS:

1. Materials and tooling verification:
   - Tooling: impact drill, 3/8" concrete bit, torque wrench for 9/16" nut, hand hammer, level, mastic tape and metal strapping
   - Material: (2) concrete anchor 3/8 x 3-1/2, (2) washers, (2) nuts, HF Sump pieces (2).
2. Apply solvent cement to the spigot end of the top half of the sump. Apply solvent cement to the hub of the bottom half of the sump. Insert the spigot of the top half of the sump into the hub of the bottom half of the sump.
3. Install the 8" spigot of the device into the outlet pipe. Use the mastic tape to seal the device spigot into the outlet pipe. You should use a level to be sure that the fitting is standing at the vertical.
4. Use an impact drill with a 3/8" concrete bit to make a series of 2 holes along each side of the body throat. The depth of the hole should be between 1-1/2" to 2-1/2". Clean the concrete dust from the 2 holes.
5. Install the anchors (2) in the holes by using a hammer. Put the nuts on the top of the anchors to protect the threads when you will hit the anchors. Remove the nuts on the anchors at the end.
6. Cut the metal strapping to length and connect each end of the strapping to the anchors. Screw the nuts in place with a maximum torque of 40 N.m (30 lbf-ft). The device should be completely flush with the catch basin wall.

WARNING

- Verify that the outlet pipe doesn't protrude into the catch basin. If it does, cut down the pipe flush to the catch basin wall.
- The solvent cement which is used in this installation is to be approved for PVC.
- The solvent cement should not be used below 0°C (32°F) or in a high humidity environment. Refer to the IPEX solvent cement guide to confirm the required curing time or visit the IPEX Online Solvent Cement Training Course available at www.ipexinc.com.
- Call your IPEX representative for more information or if you have any questions about our products.

PRODUCT TECHNICAL SPECIFICATION

General

Inlet control devices (ICD's) are designed to provide flow control at a specified rate for a given water head level and also provide odour and floatable control where specified. All ICD's will be IPEX Tempest or approved equal.

All devices shall be removable from a universal mounting plate. An operator from street level using only a T-bar with a hook will be able to retrieve the device while leaving the universal mounting plate secured to the catch basin wall face. The removal of the TEMPEST devices listed above must not require any unbolting or special manipulation or any special tools.

High Flow (HF) Sump devices will consist of a removable threaded cap which can be accessible from street level with out entry into the catchbasin (CB). The removal of the threaded cap shall not require any special tools other than the operator's hand.

ICD's must have no moving parts.

Materials

ICD's are to be manufactured from Polyvinyl Chloride (PVC) or Polyurethane material, designed to be durable enough to withstand multiple freeze-thaw cycles and exposure to harsh elements.

The inner ring seal will be manufactured using a Buna or Nitrile material with hardness between Duro 50 and Duro 70.

The wall seal is to be comprised of a 3/8" thick Neoprene Closed Cell Sponge gasket which is attached to the back of the wall plate.

All hardware will be made from 304 stainless steel.

Dimensioning

The Low Medium Flow (LMF), High Flow (HF) and the High Flow (HF) Sump shall allow for a minimum outlet pipe diameter of 200mm with a 600mm deep Catch Basin sump.

Installation

Contractor shall be responsible for securing, supporting and connecting the ICD's to the existing influent pipe and catchbasin/manhole structure as specified and designed by the Engineer.

SALES AND CUSTOMER SERVICE

Canadian Customers call IPEX Inc.

Toll free: (866) 473-9462

www.ipexinc.com

U.S. Customers call IPEX USA LLC

Toll free: (800) 463-9572

www.ipexamerica.com

About the IPEX Group of Companies

As leading suppliers of thermoplastic piping systems, the IPEX Group of Companies provides our customers with some of the largest and most comprehensive product lines. All IPEX products are backed by more than 50 years of experience. With state-of-the-art manufacturing facilities and distribution centers across North America, we have established a reputation for product innovation, quality, end-user focus and performance.

Markets served by IPEX group products are:

- Electrical systems
- Telecommunications and utility piping systems
- PVC, CPVC, PP, ABS, PEX, FR-PVDF and PE pipe and fittings (1/4" to 48")
- Industrial process piping systems
- Municipal pressure and gravity piping systems
- Plumbing and mechanical piping systems
- PE Electrofusion systems for gas and water
- Industrial, plumbing and electrical cements
- Irrigation systems

Products manufactured by IPEX Inc. and distributed in the United States by IPEX USA LLC.

Tempest™ is a trademark of IPEX Branding Inc.

MNMNTPIP110817

IPEX

C.2 SAMPLE PCSWMM MODEL INPUT (12HR 100YR SCS)

```
[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option             Value
FLOW_UNITS           LPS
INFILTRATION         HORTON
FLOW_ROUTING         DYNWAVE
LINK_OFFSETS         ELEVATION
MIN_SLOPE            0
ALLOW_PONDING        YES
SKIP_STEADY_STATE    NO

START_DATE           07/23/2009
START_TIME           00:00:00
REPORT_START_DATE    07/23/2009
REPORT_START_TIME    00:00:00
END_DATE             07/24/2009
END_TIME             00:00:00
SWEEP_START          01/01
SWEEP_END            12/31
DRY_DAYS             0
REPORT_STEP          00:05:00
WET_STEP             00:05:00
DRY_STEP             00:05:00
ROUTING_STEP         1
RULE_STEP            00:00:00

INERTIAL_DAMPING     PARTIAL
NORMAL_FLOW_LIMITED  BOTH
FORCE_MAIN_EQUATION  H-W
VARIABLE_STEP        0
LENGTHENING_STEP     0
MIN_SURFAREA         0
MAX_TRIALS           8
HEAD_TOLERANCE       0.0015
SYS_FLOW_TOL         5
LAT_FLOW_TOL         5
MINIMUM_STEP         0.5
```

```
THREADS              4

[EVAPORATION]
;;Data Source    Parameters
;;-------------- ----------------
CONSTANT         0.0
DRY_ONLY         NO

[RAINGAGES]
;;Name           Format    Interval SCF      Source
;;-------------- --------- -------- -------- ----------
RG1              INTENSITY 0:15     1.0      TIMESERIES 100SCS

[SUBCATCHMENTS]
;;Name           Rain Gage        Outlet           Area     %Imperv  Width    %Slope   CurbLen  SnowPack

;;-------------- ---------------- ---------------- -------- -------- -------- -------- --------
----------------
EXT-1            RG1              CB507-S          0.068859 38.571   95       1.5      0

ROOF_10          RG1              ROOF-10-S        0.281012 100      136      1.5      0

ROOF_11          RG1              ROOF-11-S        0.010607 100      21       1.5      0

ROOF_12          RG1              ROOF-12-S        0.01253  100      15.6     1.5      0

ROOF_3           RG1              ROOF-3-S         0.109818 100      130      1.5      0

ROOF_4           RG1              ROOF-4-S         0.035229 100      46       1.5      0

ROOF_5           RG1              ROOF-5-S         0.109819 100      130      1.5      0

ROOF_8           RG1              ROOF-8-S         0.009743 100      21       1.5      0

ROOF_9           RG1              ROOF-9-S         0.005311 100      15       1.5      0

ROOF1_2          RG1              ROOF-1-2-S       0.0954   100      95       1.5      0

ROOF6_7          RG1              ROOF-6-7-S       0.0955   100      95       1.5      0
```

```
UGPK_1           RG1              TANKS            0.144022 77.143   115      2        0

UGPK_2           RG1              TANKS            0.152475 80       122      2        0

UGPK_3           RG1              TANKS            0.059673 58.571   60       2        0

UGPK_4           RG1              TANKS            0.119964 70       95       2        0

UGPK_5           RG1              TANKS            0.110163 70       85       2        0

UGPK_6           RG1              TANKS            0.021989 100      60       15       0

UGPK_7           RG1              TANKS            0.112091 78.571   78       2        0

UGPK_8           RG1              TANKS            0.061679 75.714   42       2        0

UGPK_9           RG1              TANKS            0.032467 100      42       2        0

UNC-1            RG1              OF1              0.078091 41.429   78       2        0

UNC-2            RG1              OF2              0.515043 8.571    25       1        0

UNC-3            RG1              OF3              0.069306 61.429   122      2        0

UNC-4            RG1              CB507-S          0.051524 37.143   90       2        0

[SUBAREAS]
;;Subcatchment   N-Imperv   N-Perv     S-Imperv   S-Perv     PctZero    RouteTo    PctRouted
;;-------------- ---------- ---------- ---------- ---------- ---------- ---------- ----------
EXT-1            0.013      0.2        1.57       4.67       0          PERVIOUS   100
ROOF_10          0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_11          0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_12          0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_3           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_4           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_5           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_8           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF_9           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF1_2          0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ROOF6_7          0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_1           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_2           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_3           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_4           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_5           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_6           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_7           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_8           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UGPK_9           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
UNC-1            0.013      0.2        1.57       4.67       0          PERVIOUS   100
UNC-2            0.013      0.2        1.57       4.67       0          PERVIOUS   100
UNC-3            0.013      0.2        1.57       4.67       0          PERVIOUS   100
UNC-4            0.013      0.2        1.57       4.67       0          PERVIOUS   100

[INFILTRATION]
;;Subcatchment   Param1     Param2     Param3     Param4     Param5
;;-------------- ---------- ---------- ---------- ---------- ----------
EXT-1            76.2       13.2       4.14       7          0
ROOF_10          76.2       13.2       4.14       7          0
ROOF_11          76.2       13.2       4.14       7          0
ROOF_12          76.2       13.2       4.14       7          0
ROOF_3           76.2       13.2       4.14       7          0
ROOF_4           76.2       13.2       4.14       7          0
ROOF_5           76.2       13.2       4.14       7          0
ROOF_8           76.2       13.2       4.14       7          0
ROOF_9           76.2       13.2       4.14       7          0
ROOF1_2          76.2       13.2       4.14       7          0
ROOF6_7          76.2       13.2       4.14       7          0
UGPK_1           76.2       13.2       4.14       7          0
UGPK_2           76.2       13.2       4.14       7          0
UGPK_3           76.2       13.2       4.14       7          0
UGPK_4           76.2       13.2       4.14       7          0
UGPK_5           76.2       13.2       4.14       7          0
UGPK_6           76.2       13.2       4.14       7          0
UGPK_7           76.2       13.2       4.14       7          0
UGPK_8           76.2       13.2       4.14       7          0
```

```
UGPK_9          76.2       13.2       4.14       7          0
UNC-1           76.2       13.2       4.14       7          0
UNC-2           76.2       13.2       4.14       7          0
UNC-3           76.2       13.2       4.14       7          0
UNC-4           76.2       13.2       4.14       7          0

[JUNCTIONS]
;;Name          Elevation  MaxDepth   InitDepth  SurDepth   Aponded
;;-------------- ---------- ---------- ---------- ---------- ----------
100             99.4       2.735      0          0          0

[OUTFALLS]
;;Name          Elevation  Type       Stage Data       Gated    Route To
;;-------------- ---------- ---------- ---------------- -------- ----------------
HEADWALL        98.7       FREE                        NO
OF1             0          FREE                        NO
OF2             0          FREE                        NO
OF3             0          FREE                        NO
OF4             101.87     FIXED      102.17           NO

[STORAGE]
;;Name          Elev.    MaxDepth   InitDepth  Shape      Curve Name/Params            N/A      Fevap
Psi      Ksat     IMD
;;-------------- -------- ---------- ---------- ---------- ---------------------------- -------- --------
       -------- --------
1000            99.92    4.18       0          FUNCTIONAL 1.13      0         0        0        0
CB507-S         102.56   2.23       0          FUNCTIONAL 0         0         0        0        0
ROOF-10-S       114      0.15       0          TABULAR    ROOF10                     0        0
ROOF-11-S       114      0.15       0          TABULAR    ROOF11                     0        0
ROOF-12-S       114      0.15       0          TABULAR    ROOF12                     0        0
ROOF-1-2-S      114      0.15       0          TABULAR    ROOF1and2                  0        0
ROOF-3-S        114      0.15       0          TABULAR    ROOF3                      0        0
ROOF-4-S        114      0.15       0          TABULAR    ROOF4                      0        0
ROOF-5-S        114      0.15       0          TABULAR    ROOF5                      0        0
ROOF-6-7-S      114      0.15       0          TABULAR    ROOF6and7                  0        0
ROOF-8-S        114      0.15       0          TABULAR    ROOF8                      0        0
ROOF-9-S        114      0.15       0          TABULAR    ROOF9                      0        0
TANKS           99.7     3.61       0          FUNCTIONAL 0         0         222      0        0
```

```
[CONDUITS]
;;Name          From Node        To Node          Length     Roughness  InOffset   OutOffset  InitFlow
MaxFlow
;;-------------- ---------------- ---------------- ---------- ---------- ---------- ---------- ----------
----------
C1              CB507-S          OF4              21.3       0.013      102.56     102.45     0          0

C2              1000             100              20.8       0.013      99.87      99.83      0          0

Pipe_13         100              HEADWALL         11.135     0.013      99.548     99.52      0          0


[ORIFICES]
;;Name          From Node        To Node          Type         Offset     Qcoeff     Gated    CloseTime
;;-------------- ---------------- ---------------- ------------ ---------- ---------- -------- ----------
CISTERN-O       TANKS            1000             SIDE         99.95      0.61       NO       0

[WEIRS]
;;Name          From Node        To Node          Type         CrestHt    Qcoeff     Gated    EndCon
EndCoeff   Surcharge  RoadWidth  RoadSurf   Coeff. Curve
;;-------------- ---------------- ---------------- ------------ ---------- ---------- -------- --------
---------- ---------- ---------- ---------- ----------------
W1              TANKS            1000             TRANSVERSE   102.25     1.67       NO       0        0
       YES

[OUTLETS]
;;Name          From Node        To Node          Offset     Type             QTable/Qcoeff    Qexpon
Gated
;;-------------- ---------------- ---------------- ---------- ---------------- ---------------- ----------
--------
ROOF10-O        ROOF-10-S        TANKS            114        TABULAR/HEAD     ROOF-10-O
NO
ROOF11-O        ROOF-11-S        TANKS            114        TABULAR/HEAD     ROOF-11-O
NO
ROOF12-O        ROOF-12-S        TANKS            114        TABULAR/HEAD     ROOF-12-O
NO
ROOF1-2-O       ROOF-1-2-S       TANKS            114        TABULAR/HEAD     ROOF-1-2-O
NO
ROOF3-O         ROOF-3-S         TANKS            114        TABULAR/HEAD     ROOF-3-O
```

```
NO
ROOF4-O          ROOF-4-S         TANKS            114        TABULAR/HEAD     ROOF-4-O
NO
ROOF5-O          ROOF-5-S         TANKS            114        TABULAR/HEAD     ROOF-5-O
NO
ROOF6-7-O        ROOF-6-7-S       TANKS            114        TABULAR/HEAD     ROOF-6-7-O
NO
ROOF8-O          ROOF-8-S         TANKS            114        TABULAR/HEAD     ROOF-8-O
NO
ROOF9-O          ROOF-9-S         TANKS            114        TABULAR/HEAD     ROOF-9-O
NO

[XSECTIONS]
;;Link           Shape        Geom1            Geom2      Geom3      Geom4      Barrels    Culvert
;;-------------- ------------ ---------------- ---------- ---------- ---------- ---------- ----------
C1               CIRCULAR     0.25             0          0          0          1
C2               CIRCULAR     0.45             0          0          0          1
Pipe_13          CIRCULAR     0.9              0          0          0          1
CISTERN-O        CIRCULAR     0.075            0          0          0
W1               RECT_OPEN    1                0.5        0          0

[TRANSECTS]
;;Transect Data in HEC-2 format
;
NC 0.013    0.013    0.013
X1 Overland          5        0.15     6.85     0.0      0.0      0.0      0.0      0.0
GR 0.15     0        0        0.15     0        6.85     0.15     7        0.15     7
;
;[LE: 0][RE: 7]
NC 0.013    0.013    0.013
X1 Overland(orig)    4        0.15     6.85     0.0      0.0      0.0      0.0      0.0
GR 0.15     0        0        0.15     0        6.85     0.15     7

[LOSSES]
;;Link           Kentry     Kexit      Kavg       Flap Gate  Seepage
;;-------------- ---------- ---------- ---------- ---------- ----------
C2               0          0.14       0          NO         0

[INFLOWS]
;;Node           Constituent      Time Series      Type     Mfactor  Sfactor  Baseline Pattern
;;-------------- ---------------- ---------------- -------- -------- -------- -------- --------
100              FLOW             100yrHydrograph  FLOW     1.0      1        0

[CURVES]
;;Name           Type       X-Value    Y-Value
;;-------------- ---------- ---------- ----------
BIOSWALE_BASEFLOW Rating     0          0
BIOSWALE_BASEFLOW            0.01       0.3
BIOSWALE_BASEFLOW            10         0.3

ROOF-10-O        Rating     0          0
ROOF-10-O                   0.025      6.624
ROOF-10-O                   0.05       13.249
ROOF-10-O                   0.075      14.905
ROOF-10-O                   0.1        16.561
ROOF-10-O                   0.125      18.217
ROOF-10-O                   0.15       19.873

ROOF-11-O        Rating     0          0
ROOF-11-O                   0.025      0.631
ROOF-11-O                   0.05       1.262
ROOF-11-O                   0.075      1.42
ROOF-11-O                   0.1        1.577
ROOF-11-O                   0.125      1.735
ROOF-11-O                   0.15       1.893

ROOF-12-O        Rating     0          0
ROOF-12-O                   0.025      0.631
ROOF-12-O                   0.05       1.262
ROOF-12-O                   0.075      1.42
ROOF-12-O                   0.1        1.577
ROOF-12-O                   0.125      1.735
ROOF-12-O                   0.15       1.893

ROOF-1-2-O       Rating     0          0
ROOF-1-2-O                  0.025      2.21
ROOF-1-2-O                  0.05       4.42
ROOF-1-2-O                  0.075      5.52
```

ROOF-1-2-O		0.1	6.62
ROOF-1-2-O		0.125	7.73
ROOF-1-2-O		0.15	8.83
ROOF-3-O	Rating	0	0
ROOF-3-O		0.025	1.26
ROOF-3-O		0.05	2.52
ROOF-3-O		0.075	3.79
ROOF-3-O		0.1	5.05
ROOF-3-O		0.125	6.31
ROOF-3-O		0.15	7.57
ROOF-4-O	Rating	0	0
ROOF-4-O		0.025	1.58
ROOF-4-O		0.05	3.15
ROOF-4-O		0.075	3.15
ROOF-4-O		0.1	3.15
ROOF-4-O		0.125	3.15
ROOF-4-O		0.15	3.15
ROOF-5-O	Rating	0	0
ROOF-5-O		0.025	1.26
ROOF-5-O		0.05	2.52
ROOF-5-O		0.075	3.79
ROOF-5-O		0.1	5.05
ROOF-5-O		0.125	6.31
ROOF-5-O		0.15	7.57
ROOF-6-7-O	Rating	0	0
ROOF-6-7-O		0.025	2.21
ROOF-6-7-O		0.05	4.42
ROOF-6-7-O		0.075	5.52
ROOF-6-7-O		0.1	6.62
ROOF-6-7-O		0.125	7.73
ROOF-6-7-O		0.15	8.83
ROOF-8-O	Rating	0	0
ROOF-8-O		0.025	0.63
ROOF-8-O		0.05	1.26
ROOF-8-O		0.075	1.42
ROOF-8-O		0.1	1.58
ROOF-8-O		0.125	1.73
ROOF-8-O		0.15	1.89
ROOF-9-O	Rating	0	0
ROOF-9-O		0.025	0.631
ROOF-9-O		0.05	1.262
ROOF-9-O		0.075	1.42
ROOF-9-O		0.1	1.577
ROOF-9-O		0.125	1.735
ROOF-9-O		0.15	1.893
ROOF10	Storage	0	0
ROOF10		0.025	62.44
ROOF10		0.05	249.78
ROOF10		0.075	562
ROOF10		0.1	999.11
ROOF10		0.125	1561.11
ROOF10		0.15	2248
ROOF11	Storage	0	0
ROOF11		0.025	2.36
ROOF11		0.05	9.42
ROOF11		0.075	21.2
ROOF11		0.1	37.69
ROOF11		0.125	58.89
ROOF11		0.15	84.8
ROOF12	Storage	0	0
ROOF12		0.025	2.22
ROOF12		0.05	8.89
ROOF12		0.075	20
ROOF12		0.1	35.56
ROOF12		0.125	55.56
ROOF12		0.15	80
ROOF1and2	Storage	0	0
ROOF1and2		0.025	21.2

ROOF1and2		0.05	84.8
ROOF1and2		0.075	190.8
ROOF1and2		0.1	339.2
ROOF1and2		0.125	530
ROOF1and2		0.15	763.2
ROOF3	Storage	0	0
ROOF3		0.025	24.4
ROOF3		0.05	97.6
ROOF3		0.075	219.6
ROOF3		0.1	390.4
ROOF3		0.125	610
ROOF3		0.15	878.4
ROOF4	Storage	0	0
ROOF4		0.025	7.822222222
ROOF4		0.05	31.28888889
ROOF4		0.075	70.4
ROOF4		0.1	125.1555556
ROOF4		0.125	195.5555556
ROOF4		0.15	281.6
ROOF5	Storage	0	0
ROOF5		0.025	24.4
ROOF5		0.05	97.6
ROOF5		0.075	219.6
ROOF5		0.1	390.4
ROOF5		0.125	610
ROOF5		0.15	878.4
ROOF6and7	Storage	0	0
ROOF6and7		0.025	21.22
ROOF6and7		0.05	84.89
ROOF6and7		0.075	191
ROOF6and7		0.1	339.56
ROOF6and7		0.125	530.56
ROOF6and7		0.15	764
ROOF8	Storage	0	0

ROOF8		0.025	2.16
ROOF8		0.05	8.62
ROOF8		0.075	19.4
ROOF8		0.1	34.49
ROOF8		0.125	53.89
ROOF8		0.15	77.6
ROOF9	Storage	0	0
ROOF9		0.025	1.18
ROOF9		0.05	4.71
ROOF9		0.075	10.6
ROOF9		0.1	18.84
ROOF9		0.125	29.44
ROOF9		0.15	42.4
TANK	Storage	0	560.7
TANK		0.026	560.7
TANK		0.051	560.7
TANK		0.077	560.7
TANK		0.102	559.44
TANK		0.127	559.44
TANK		0.153	558.18
TANK		0.178	556.92
TANK		0.204	555.66
TANK		0.229	554.4
TANK		0.254	551.88
TANK		0.28	549.36
TANK		0.305	546.84
TANK		0.331	543.06
TANK		0.356	539.28
TANK		0.381	534.24
TANK		0.407	527.94
TANK		0.432	521.64
TANK		0.458	514.08
TANK		0.483	505.26
TANK		0.508	495.18
TANK		0.534	483.84
TANK		0.559	478.8
TANK		0.585	464.94

```
TANK                        0.61       449.82
TANK                        0.635      434.7
TANK                        0.661      419.58
TANK                        0.686      403.2
TANK                        0.712      383.04
TANK                        0.737      360.36
TANK                        0.762      347.76
TANK                        0.796      335.16
TANK                        0.813      320.04
TANK                        0.839      304.92
TANK                        0.864      289.8
TANK                        0.889      272.16
TANK                        0.915      258.3
TANK                        0.94       244.44
TANK                        0.965      233.1
TANK                        0.991      221.76
TANK                        1.016      211.68
TANK                        1.041      201.6
TANK                        1.067      192.78
TANK                        1.092      185.22
TANK                        1.118      180.18
TANK                        1.143      176.4
TANK                        1.168      172.62
TANK                        1.194      170.1
TANK                        1.219      167.58
TANK                        1.245      165.06
TANK                        1.27       163.8
TANK                        1.295      162.54
TANK                        1.321      162.54
TANK                        1.346      162.54
TANK                        1.372      161.28
TANK                        1.397      161.28
TANK                        1.422      161.28
TANK                        1.448      161.28
TANK                        1.473      161.28
TANK                        1.499      161.28
TANK                        1.524      161.28
TANK                        1.549      161.28
TANK                        1.575      161.28
```

```
TANK                        1.6        161.28
TANK                        1.626      161.28
TANK                        1.651      161.28
TANK                        1.676      161.28
TANK                        1.702      161.28
TANK                        1.727      161.28
TANK                        1.753      161.28
TANK                        1.778      161.28
TANK                        1.803      161.28
TANK                        1.829      161.28
TANK                        1.83       0
TANK                        5          0

[TIMESERIES]
;;Name           Date       Time       Value
;;-------------- ---------- ---------- ----------
;MTO Distribution, 15min intervals
002SCS                      0:00       0
002SCS                      0:15       1.08
002SCS                      0:30       1.08
002SCS                      0:45       1.08
002SCS                      1:00       1.08
002SCS                      1:15       1.08
002SCS                      1:30       1.08
002SCS                      1:45       1.08
002SCS                      2:00       1.296
002SCS                      2:15       1.296
002SCS                      2:30       1.296
002SCS                      2:45       1.296
002SCS                      3:00       1.728
002SCS                      3:15       1.728
002SCS                      3:30       1.728
002SCS                      3:45       1.728
002SCS                      4:00       2.592
002SCS                      4:15       2.592
002SCS                      4:30       3.456
002SCS                      4:45       3.456
002SCS                      5:00       5.184
002SCS                      5:15       5.184
```

002SCS	5:30	20.736
002SCS	5:45	57.024
002SCS	6:00	7.776
002SCS	6:15	7.776
002SCS	6:30	3.456
002SCS	6:45	3.456
002SCS	7:00	2.592
002SCS	7:15	2.592
002SCS	7:30	2.592
002SCS	7:45	2.592
002SCS	8:00	1.512
002SCS	8:15	1.512
002SCS	8:30	1.512
002SCS	8:45	1.512
002SCS	9:00	1.512
002SCS	9:15	1.512
002SCS	9:30	1.512
002SCS	9:45	1.512
002SCS	10:00	0.864
002SCS	10:15	0.864
002SCS	10:30	0.864
002SCS	10:45	0.864
002SCS	11:00	0.864
002SCS	11:15	0.864
002SCS	11:30	0.864
002SCS	11:45	0.864
002SCS	12:00	0

005SCS	0:00:00	0
005SCS	0:15:00	1.44
005SCS	0:30:00	1.44
005SCS	0:45:00	1.44
005SCS	1:00:00	1.44
005SCS	1:15:00	1.44
005SCS	1:30:00	1.44
005SCS	1:45:00	1.44
005SCS	2:00:00	1.728
005SCS	2:15:00	1.728
005SCS	2:30:00	1.728
005SCS	2:45:00	1.728
005SCS	3:00:00	2.304
005SCS	3:15:00	2.304
005SCS	3:30:00	2.304
005SCS	3:45:00	2.304
005SCS	4:00:00	3.456
005SCS	4:15:00	3.456
005SCS	4:30:00	4.608
005SCS	4:45:00	4.608
005SCS	5:00:00	6.912
005SCS	5:15:00	6.912
005SCS	5:30:00	27.648
005SCS	5:45:00	76.032
005SCS	6:00:00	10.368
005SCS	6:15:00	10.368
005SCS	6:30:00	4.608
005SCS	6:45:00	4.608
005SCS	7:00:00	3.456
005SCS	7:15:00	3.456
005SCS	7:30:00	3.456
005SCS	7:45:00	3.456
005SCS	8:00:00	2.016
005SCS	8:15:00	2.016
005SCS	8:30:00	2.016
005SCS	8:45:00	2.016
005SCS	9:00:00	2.016
005SCS	9:15:00	2.016
005SCS	9:30:00	2.016
005SCS	9:45:00	2.016
005SCS	10:00:00	1.152
005SCS	10:15:00	1.152
005SCS	10:30:00	1.152
005SCS	10:45:00	1.152
005SCS	11:00:00	1.152
005SCS	11:15:00	1.152
005SCS	11:30:00	1.152
005SCS	11:45:00	1.152
005SCS	12:00:00	0

010SCS	0:00:00	0
010SCS	0:15:00	1.68
010SCS	0:30:00	1.68
010SCS	0:45:00	1.68
010SCS	1:00:00	1.68
010SCS	1:15:00	1.68
010SCS	1:30:00	1.68
010SCS	1:45:00	1.68
010SCS	2:00:00	2.02
010SCS	2:15:00	2.02
010SCS	2:30:00	2.02
010SCS	2:45:00	2.02
010SCS	3:00:00	2.69
010SCS	3:15:00	2.69
010SCS	3:30:00	2.69
010SCS	3:45:00	2.69
010SCS	4:00:00	4.03
010SCS	4:15:00	4.03
010SCS	4:30:00	5.38
010SCS	4:45:00	5.38
010SCS	5:00:00	8.06
010SCS	5:15:00	8.06
010SCS	5:30:00	32.26
010SCS	5:45:00	88.7
010SCS	6:00:00	12.1
010SCS	6:15:00	12.1
010SCS	6:30:00	5.38
010SCS	6:45:00	5.38
010SCS	7:00:00	4.03
010SCS	7:15:00	4.03
010SCS	7:30:00	4.03
010SCS	7:45:00	4.03
010SCS	8:00:00	2.35
010SCS	8:15:00	2.35
010SCS	8:30:00	2.35
010SCS	8:45:00	2.35
010SCS	9:00:00	2.35
010SCS	9:15:00	2.35
010SCS	9:30:00	2.35

010SCS	9:45:00	2.35
010SCS	10:00:00	1.34
010SCS	10:15:00	1.34
010SCS	10:30:00	1.34
010SCS	10:45:00	1.34
010SCS	11:00:00	1.34
010SCS	11:15:00	1.34
010SCS	11:30:00	1.34
010SCS	11:45:00	1.34
010SCS	12:00:00	0
025SCS	0:00:00	0
025SCS	0:15:00	1.98
025SCS	0:30:00	1.98
025SCS	0:45:00	1.98
025SCS	1:00:00	1.98
025SCS	1:15:00	1.98
025SCS	1:30:00	1.98
025SCS	1:45:00	1.98
025SCS	2:00:00	2.376
025SCS	2:15:00	2.376
025SCS	2:30:00	2.376
025SCS	2:45:00	2.376
025SCS	3:00:00	3.168
025SCS	3:15:00	3.168
025SCS	3:30:00	3.168
025SCS	3:45:00	3.168
025SCS	4:00:00	4.752
025SCS	4:15:00	4.752
025SCS	4:30:00	6.336
025SCS	4:45:00	6.336
025SCS	5:00:00	9.504
025SCS	5:15:00	9.504
025SCS	5:30:00	38.016
025SCS	5:45:00	104.544
025SCS	6:00:00	14.256
025SCS	6:15:00	14.256
025SCS	6:30:00	6.336
025SCS	6:45:00	6.336

```
025SCS                    7:00:00    4.752
025SCS                    7:15:00    4.752
025SCS                    7:30:00    4.752
025SCS                    7:45:00    4.752
025SCS                    8:00:00    2.772
025SCS                    8:15:00    2.772
025SCS                    8:30:00    2.772
025SCS                    8:45:00    2.772
025SCS                    9:00:00    2.772
025SCS                    9:15:00    2.772
025SCS                    9:30:00    2.772
025SCS                    9:45:00    2.772
025SCS                    10:00:00   1.584
025SCS                    10:15:00   1.584
025SCS                    10:30:00   1.584
025SCS                    10:45:00   1.584
025SCS                    11:00:00   1.584
025SCS                    11:15:00   1.584
025SCS                    11:30:00   1.584
025SCS                    11:45:00   1.584
025SCS                    12:00:00   0

050SCS                    0:00:00    0
050SCS                    0:15:00    2.19
050SCS                    0:30:00    2.19
050SCS                    0:45:00    2.19
050SCS                    1:00:00    2.19
050SCS                    1:15:00    2.19
050SCS                    1:30:00    2.19
050SCS                    1:45:00    2.19
050SCS                    2:00:00    2.628
050SCS                    2:15:00    2.628
050SCS                    2:30:00    2.628
050SCS                    2:45:00    2.628
050SCS                    3:00:00    3.504
050SCS                    3:15:00    3.504
050SCS                    3:30:00    3.504
050SCS                    3:45:00    3.504
050SCS                    4:00:00    5.256
```

```
050SCS                    4:15:00    5.256
050SCS                    4:30:00    7.008
050SCS                    4:45:00    7.008
050SCS                    5:00:00    10.512
050SCS                    5:15:00    10.512
050SCS                    5:30:00    42.048
050SCS                    5:45:00    115.632
050SCS                    6:00:00    15.768
050SCS                    6:15:00    15.768
050SCS                    6:30:00    7.008
050SCS                    6:45:00    7.008
050SCS                    7:00:00    5.256
050SCS                    7:15:00    5.256
050SCS                    7:30:00    5.256
050SCS                    7:45:00    5.256
050SCS                    8:00:00    3.066
050SCS                    8:15:00    3.066
050SCS                    8:30:00    3.066
050SCS                    8:45:00    3.066
050SCS                    9:00:00    3.066
050SCS                    9:15:00    3.066
050SCS                    9:30:00    3.066
050SCS                    9:45:00    3.066
050SCS                    10:00:00   1.752
050SCS                    10:15:00   1.752
050SCS                    10:30:00   1.752
050SCS                    10:45:00   1.752
050SCS                    11:00:00   1.752
050SCS                    11:15:00   1.752
050SCS                    11:30:00   1.752
050SCS                    11:45:00   1.752
050SCS                    12:00:00   0

;MTO Distribution, 15min intervals
100SCS                    0:00       0
100SCS                    0:15       2.4
100SCS                    0:30       2.4
100SCS                    0:45       2.4
100SCS                    1:00       2.4
```

100SCS	1:15	2.4
100SCS	1:30	2.4
100SCS	1:45	2.4
100SCS	2:00	2.4
100SCS	2:15	2.88
100SCS	2:30	2.88
100SCS	2:45	2.88
100SCS	3:00	2.88
100SCS	3:15	3.84
100SCS	3:30	3.84
100SCS	3:45	3.84
100SCS	4:00	3.84
100SCS	4:15	5.76
100SCS	4:30	5.76
100SCS	4:45	7.68
100SCS	5:00	7.68
100SCS	5:15	11.52
100SCS	5:30	11.52
100SCS	5:45	46.08
100SCS	6:00	126.72
100SCS	6:15	17.28
100SCS	6:30	17.28
100SCS	6:45	7.68
100SCS	7:00	7.68
100SCS	7:15	5.76
100SCS	7:30	5.76
100SCS	7:45	5.76
100SCS	8:00	5.76
100SCS	8:15	3.36
100SCS	8:30	3.36
100SCS	8:45	3.36
100SCS	9:00	3.36
100SCS	9:15	3.36
100SCS	9:30	3.36
100SCS	9:45	3.36
100SCS	10:00	3.36
100SCS	10:15	1.92
100SCS	10:30	1.92
100SCS	10:45	1.92

100SCS	11:00	1.92
100SCS	11:15	1.92
100SCS	11:30	1.92
100SCS	11:45	1.92
100SCS	12:00	0

100yrHydrograph	0:05	0
100yrHydrograph	0:10	0
100yrHydrograph	0:15	0
100yrHydrograph	0:20	0
100yrHydrograph	0:25	0
100yrHydrograph	0:30	0
100yrHydrograph	0:35	0
100yrHydrograph	0:40	0
100yrHydrograph	0:45	0
100yrHydrograph	0:50	0
100yrHydrograph	0:55	0
100yrHydrograph	1:00	0
100yrHydrograph	1:05	0
100yrHydrograph	1:10	0
100yrHydrograph	1:15	0
100yrHydrograph	1:20	0
100yrHydrograph	1:25	0
100yrHydrograph	1:30	0
100yrHydrograph	1:35	0
100yrHydrograph	1:40	0
100yrHydrograph	1:45	0
100yrHydrograph	1:50	0
100yrHydrograph	1:55	0
100yrHydrograph	2:00	0
100yrHydrograph	2:05	0.03368589
100yrHydrograph	2:10	0.400265
100yrHydrograph	2:15	0.6780789
100yrHydrograph	2:20	0.8096212
100yrHydrograph	2:25	0.9188437
100yrHydrograph	2:30	1.041047
100yrHydrograph	2:35	1.160273
100yrHydrograph	2:40	1.279933
100yrHydrograph	2:45	1.400491

100yrHydrograph	2:50	1.521803
100yrHydrograph	2:55	1.6431
100yrHydrograph	3:00	1.770213
100yrHydrograph	3:05	1.89238
100yrHydrograph	3:10	2.011746
100yrHydrograph	3:15	2.129715
100yrHydrograph	3:20	2.24681
100yrHydrograph	3:25	2.372673
100yrHydrograph	3:30	2.52147
100yrHydrograph	3:35	2.690327
100yrHydrograph	3:40	2.870473
100yrHydrograph	3:45	3.049115
100yrHydrograph	3:50	3.225464
100yrHydrograph	3:55	3.398892
100yrHydrograph	4:00	3.569112
100yrHydrograph	4:05	3.736014
100yrHydrograph	4:10	3.899172
100yrHydrograph	4:15	4.062318
100yrHydrograph	4:20	4.221156
100yrHydrograph	4:25	4.414243
100yrHydrograph	4:30	4.674353
100yrHydrograph	4:35	4.971942
100yrHydrograph	4:40	5.279111
100yrHydrograph	4:45	6.154143
100yrHydrograph	4:50	6.507444
100yrHydrograph	4:55	6.698976
100yrHydrograph	5:00	6.914319
100yrHydrograph	5:05	7.139013
100yrHydrograph	5:10	7.358335
100yrHydrograph	5:15	7.568453
100yrHydrograph	5:20	7.776987
100yrHydrograph	5:25	8.043731
100yrHydrograph	5:30	8.375088
100yrHydrograph	5:35	8.720076
100yrHydrograph	5:40	9.065854
100yrHydrograph	5:45	9.411835
100yrHydrograph	5:50	9.817601
100yrHydrograph	5:55	10.7888
100yrHydrograph	6:00	12.18063

100yrHydrograph	6:05	13.55768
100yrHydrograph	6:10	15.79406
100yrHydrograph	6:15	18.70004
100yrHydrograph	6:20	42.87194
100yrHydrograph	6:25	89.41938
100yrHydrograph	6:30	110.0801
100yrHydrograph	6:35	120.6727
100yrHydrograph	6:40	126.8955
100yrHydrograph	6:45	131.1839
100yrHydrograph	6:50	135.1019
100yrHydrograph	6:55	136.1944
100yrHydrograph	7:00	134.3278
100yrHydrograph	7:05	133.2315
100yrHydrograph	7:10	131.9975
100yrHydrograph	7:15	130.5499
100yrHydrograph	7:20	128.9494
100yrHydrograph	7:25	127.0917
100yrHydrograph	7:30	124.968
100yrHydrograph	7:35	122.8077
100yrHydrograph	7:40	120.8129
100yrHydrograph	7:45	118.6745
100yrHydrograph	7:50	116.2747
100yrHydrograph	7:55	113.7336
100yrHydrograph	8:00	111.03
100yrHydrograph	8:05	107.4665
100yrHydrograph	8:10	103.368
100yrHydrograph	8:15	99.26289
100yrHydrograph	8:20	95.15655
100yrHydrograph	8:25	90.6512
100yrHydrograph	8:30	85.96979
100yrHydrograph	8:35	81.40397
100yrHydrograph	8:40	77.20897
100yrHydrograph	8:45	73.056
100yrHydrograph	8:50	68.54881
100yrHydrograph	8:55	63.82666
100yrHydrograph	9:00	59.70413
100yrHydrograph	9:05	56.16789
100yrHydrograph	9:10	53.06845
100yrHydrograph	9:15	50.40911

100yrHydrograph	9:20	48.21593
100yrHydrograph	9:25	46.66951
100yrHydrograph	9:30	45.38166
100yrHydrograph	9:35	44.14623
100yrHydrograph	9:40	42.96396
100yrHydrograph	9:45	41.84916
100yrHydrograph	9:50	40.78232
100yrHydrograph	9:55	39.74205
100yrHydrograph	10:00	38.7661
100yrHydrograph	10:05	37.83093
100yrHydrograph	10:10	36.92952
100yrHydrograph	10:15	36.06311
100yrHydrograph	10:20	35.16625
100yrHydrograph	10:25	33.88804
100yrHydrograph	10:30	32.4535
100yrHydrograph	10:35	31.05881
100yrHydrograph	10:40	29.78304
100yrHydrograph	10:45	28.63077
100yrHydrograph	10:50	27.58944
100yrHydrograph	10:55	26.653
100yrHydrograph	11:00	25.66802
100yrHydrograph	11:05	24.91771
100yrHydrograph	11:10	24.2445
100yrHydrograph	11:15	23.63535
100yrHydrograph	11:20	23.08379
100yrHydrograph	11:25	22.58303
100yrHydrograph	11:30	22.12786
100yrHydrograph	11:35	21.71402
100yrHydrograph	11:40	21.33782
100yrHydrograph	11:45	20.99621
100yrHydrograph	11:50	20.68643
100yrHydrograph	11:55	20.40586
100yrHydrograph	12:00	20.1529
100yrHydrograph	12:05	19.91762
100yrHydrograph	12:10	19.63007
100yrHydrograph	12:15	19.3239
100yrHydrograph	12:20	19.12781
100yrHydrograph	12:25	18.97093
100yrHydrograph	12:30	18.80998

100yrHydrograph	12:35	18.64461
100yrHydrograph	12:40	18.47622
100yrHydrograph	12:45	18.30533
100yrHydrograph	12:50	18.13219
100yrHydrograph	12:55	17.95722
100yrHydrograph	13:00	17.74718
100yrHydrograph	13:05	17.56915
100yrHydrograph	13:10	17.39227
100yrHydrograph	13:15	17.21504
100yrHydrograph	13:20	17.03772
100yrHydrograph	13:25	16.86038
100yrHydrograph	13:30	16.68295
100yrHydrograph	13:35	16.50546
100yrHydrograph	13:40	16.32839
100yrHydrograph	13:45	16.15076
100yrHydrograph	13:50	15.97309
100yrHydrograph	13:55	15.79539
100yrHydrograph	14:00	15.61763
100yrHydrograph	14:05	15.44018
100yrHydrograph	14:10	15.26246
100yrHydrograph	14:15	15.08446
100yrHydrograph	14:20	14.90619
100yrHydrograph	14:25	14.72721
100yrHydrograph	14:30	14.54815
100yrHydrograph	14:35	14.36913
100yrHydrograph	14:40	14.19005
100yrHydrograph	14:45	14.011
100yrHydrograph	14:50	13.83076
100yrHydrograph	14:55	13.65002
100yrHydrograph	15:00	13.46919
100yrHydrograph	15:05	13.27224
100yrHydrograph	15:10	13.09266
100yrHydrograph	15:15	12.91431
100yrHydrograph	15:20	12.73593
100yrHydrograph	15:25	12.55792
100yrHydrograph	15:30	12.37996
100yrHydrograph	15:35	12.2021
100yrHydrograph	15:40	12.02427
100yrHydrograph	15:45	11.84651

100yrHydrograph	15:50	11.66898
100yrHydrograph	15:55	11.49166
100yrHydrograph	16:00	11.3145
100yrHydrograph	16:05	11.13787
100yrHydrograph	16:10	10.96224
100yrHydrograph	16:15	10.78573
100yrHydrograph	16:20	10.60909
100yrHydrograph	16:25	10.43269
100yrHydrograph	16:30	10.25677
100yrHydrograph	16:35	10.08101
100yrHydrograph	16:40	9.904943
100yrHydrograph	16:45	9.728643
100yrHydrograph	16:50	9.552533
100yrHydrograph	16:55	9.376752
100yrHydrograph	17:00	9.201365
100yrHydrograph	17:05	9.026419
100yrHydrograph	17:10	8.854236
100yrHydrograph	17:15	8.68074
100yrHydrograph	17:20	8.507633
100yrHydrograph	17:25	8.335072
100yrHydrograph	17:30	8.163301
100yrHydrograph	17:35	7.992058
100yrHydrograph	17:40	7.821409
100yrHydrograph	17:45	7.651455
100yrHydrograph	17:50	7.482112
100yrHydrograph	17:55	7.313401
100yrHydrograph	18:00	7.145336
100yrHydrograph	18:05	6.979621
100yrHydrograph	18:10	6.813243
100yrHydrograph	18:15	6.647324
100yrHydrograph	18:20	6.482216
100yrHydrograph	18:25	5.824821
100yrHydrograph	18:30	5.295127
100yrHydrograph	18:35	5.060137
100yrHydrograph	18:40	4.84829
100yrHydrograph	18:45	4.645963
100yrHydrograph	18:50	4.453671
100yrHydrograph	18:55	4.270811
100yrHydrograph	19:00	4.09688

100yrHydrograph	19:05	3.932506
100yrHydrograph	19:10	3.778864
100yrHydrograph	19:15	3.629502
100yrHydrograph	19:20	3.486742
100yrHydrograph	19:25	3.350802
100yrHydrograph	19:30	3.221082
100yrHydrograph	19:35	3.097033
100yrHydrograph	19:40	2.978455
100yrHydrograph	19:45	2.864869
100yrHydrograph	19:50	2.756282
100yrHydrograph	19:55	2.654007
100yrHydrograph	20:00	2.556878
100yrHydrograph	20:05	2.461823
100yrHydrograph	20:10	2.370567
100yrHydrograph	20:15	2.283155
100yrHydrograph	20:20	2.199545
100yrHydrograph	20:25	2.119371
100yrHydrograph	20:30	2.0424
100yrHydrograph	20:35	1.968455
100yrHydrograph	20:40	1.897445
100yrHydrograph	20:45	1.82913
100yrHydrograph	20:50	1.763424
100yrHydrograph	20:55	1.701357
100yrHydrograph	21:00	1.642923
100yrHydrograph	21:05	1.586518
100yrHydrograph	21:10	1.530593
100yrHydrograph	21:15	1.476459
100yrHydrograph	21:20	1.424285
100yrHydrograph	21:25	1.373979
100yrHydrograph	21:30	1.325569
100yrHydrograph	21:35	1.278883
100yrHydrograph	21:40	1.233917
100yrHydrograph	21:45	1.190506
100yrHydrograph	21:50	1.148588
100yrHydrograph	21:55	1.108119
100yrHydrograph	22:00	1.069048
100yrHydrograph	22:05	1.031321
100yrHydrograph	22:10	0.9948828
100yrHydrograph	22:15	0.9598325

```
100yrHydrograph            22:20      0.9265975
100yrHydrograph            22:25      0.895036
100yrHydrograph            22:30      0.8651206
100yrHydrograph            22:35      0.8349182
100yrHydrograph            22:40      0.8052853
100yrHydrograph            22:45      0.7765169
100yrHydrograph            22:50      0.7486013
100yrHydrograph            22:55      0.721274
100yrHydrograph            23:00      0.6946917
100yrHydrograph            23:05      0.6689637
100yrHydrograph            23:10      0.6440647
100yrHydrograph            23:15      0.619979
100yrHydrograph            23:20      0.5967006
100yrHydrograph            23:25      0.5741587
100yrHydrograph            23:30      0.5523183
100yrHydrograph            23:35      0.5311568
100yrHydrograph            23:40      0.5106529
100yrHydrograph            23:45      0.4907854
100yrHydrograph            23:50      0.4715335
100yrHydrograph            23:55      0.4528767

120SCS                     0:00       0
120SCS                     0:15       2.88
120SCS                     0:30       2.88
120SCS                     0:45       2.88
120SCS                     1:00       2.88
120SCS                     1:15       2.88
120SCS                     1:30       2.88
120SCS                     1:45       2.88
120SCS                     2:00       2.88
120SCS                     2:15       3.456
120SCS                     2:30       3.456
120SCS                     2:45       3.456
120SCS                     3:00       3.456
120SCS                     3:15       4.608
120SCS                     3:30       4.608
120SCS                     3:45       4.608
120SCS                     4:00       4.608
120SCS                     4:15       6.912
```

```
120SCS                     4:30       6.912
120SCS                     4:45       9.216
120SCS                     5:00       9.216
120SCS                     5:15       13.824
120SCS                     5:30       13.824
120SCS                     5:45       55.296
120SCS                     6:00       152.064
120SCS                     6:15       20.736
120SCS                     6:30       20.736
120SCS                     6:45       9.216
120SCS                     7:00       9.216
120SCS                     7:15       6.912
120SCS                     7:30       6.912
120SCS                     7:45       6.912
120SCS                     8:00       6.912
120SCS                     8:15       4.032
120SCS                     8:30       4.032
120SCS                     8:45       4.032
120SCS                     9:00       4.032
120SCS                     9:15       4.032
120SCS                     9:30       4.032
120SCS                     9:45       4.032
120SCS                     10:00      4.032
120SCS                     10:15      2.304
120SCS                     10:30      2.304
120SCS                     10:45      2.304
120SCS                     11:00      2.304
120SCS                     11:15      2.304
120SCS                     11:30      2.304
120SCS                     11:45      2.304
120SCS                     12:00      0

[REPORT]
;;Reporting Options
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

```
[TAGS]
Node       1000             MN

[MAP]
DIMENSIONS       350504.706880014 5015809.99134237 350738.979598863 5016090.18291699
UNITS            Meters

[COORDINATES]
;;Node           X-Coord            Y-Coord
;;-------------- ------------------ ------------------
100              350580.8           5016032
HEADWALL         350565.848         5016038.828
OF1              350702.274         5015973.798
OF2              350547.792         5015987.469
OF3              350691.907         5015873.685
OF4              350536.617         5015877.154
1000             350599.591         5016022.489
CB507-S          350565.233         5015895.797
ROOF-10-S        350597.664         5015958.821
ROOF-11-S        350644.317         5015987.934
ROOF-12-S        350612.609         5016011.36
ROOF-1-2-S       350676.499         5015951.256
ROOF-3-S         350694.246         5015914.342
ROOF-4-S         350662.511         5015888.115
ROOF-5-S         350638.638         5015863.467
ROOF-6-7-S       350600.067         5015880.741
ROOF-8-S         350571.435         5015920.731
ROOF-9-S         350589.892         5015966.401
TANKS            350611.749         5016033.953

[VERTICES]
;;Link           X-Coord            Y-Coord
;;-------------- ------------------ ------------------
W1               350597.646         5016038.74

[POLYGONS]
;;Subcatchment   X-Coord            Y-Coord
;;-------------- ------------------ ------------------
```

```
[SYMBOLS]
;;Gage           X-Coord            Y-Coord
;;-------------- ------------------ ------------------
```

C.3 SAMPLE PCSWMM MODEL OUTPUT (12HR 100YR SCS)

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

WARNING 03: negative offset ignored for Link C2

****
Element Count
****

Number of rain gages 1
Number of subcatchments ... 24
Number of nodes 19
Number of links 15
Number of pollutants 0
Number of land uses 0

****
Raingage Summary
****

Name	Data Source	Data Type	Recording Interval
RG1	100SCS	INTENSITY	15 min.

****
Subcatchment Summary
****

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT-1	0.07	95.00	38.57	1.5000	RG1	CB507-S
ROOF_10	0.28	136.00	100.00	1.5000	RG1	ROOF-10-S
ROOF_11	0.01	21.00	100.00	1.5000	RG1	ROOF-11-S
ROOF_12	0.01	15.60	100.00	1.5000	RG1	ROOF-12-S
ROOF_3	0.11	130.00	100.00	1.5000	RG1	ROOF-3-S
ROOF_4	0.04	46.00	100.00	1.5000	RG1	ROOF-4-S
ROOF_5	0.11	130.00	100.00	1.5000	RG1	ROOF-5-S
ROOF_8	0.01	21.00	100.00	1.5000	RG1	ROOF-8-S
ROOF_9	0.01	15.00	100.00	1.5000	RG1	ROOF-9-S
ROOF1_2	0.10	95.00	100.00	1.5000	RG1	ROOF-1-2-S
ROOF6_7	0.10	95.00	100.00	1.5000	RG1	ROOF-6-7-S
UGPK_1	0.14	115.00	77.14	2.0000	RG1	TANKS
UGPK_2	0.15	122.00	80.00	2.0000	RG1	TANKS
UGPK_3	0.06	60.00	58.57	2.0000	RG1	TANKS
UGPK_4	0.12	95.00	70.00	2.0000	RG1	TANKS
UGPK_5	0.11	85.00	70.00	2.0000	RG1	TANKS
UGPK_6	0.02	60.00	100.00	15.0000	RG1	TANKS
UGPK_7	0.11	78.00	78.57	2.0000	RG1	TANKS
UGPK_8	0.06	42.00	75.71	2.0000	RG1	TANKS
UGPK_9	0.03	42.00	100.00	2.0000	RG1	TANKS
UNC-1	0.08	78.00	41.43	2.0000	RG1	OF1
UNC-2	0.52	25.00	8.57	1.0000	RG1	OF2
UNC-3	0.07	122.00	61.43	2.0000	RG1	OF3
UNC-4	0.05	90.00	37.14	2.0000	RG1	CB507-S

****
Node Summary
****

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
100	JUNCTION	99.40	2.73	0.0	Yes
HEADWALL	OUTFALL	98.70	1.72	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
OF3	OUTFALL	0.00	0.00	0.0	
OF4	OUTFALL	101.87	0.83	0.0	
1000	STORAGE	99.92	4.18	0.0	
CB507-S	STORAGE	102.56	2.23	0.0	
ROOF-10-S	STORAGE	114.00	0.15	0.0	
ROOF-11-S	STORAGE	114.00	0.15	0.0	
ROOF-12-S	STORAGE	114.00	0.15	0.0	
ROOF-1-2-S	STORAGE	114.00	0.15	0.0	
ROOF-3-S	STORAGE	114.00	0.15	0.0	
ROOF-4-S	STORAGE	114.00	0.15	0.0	

ROOF-5-S	STORAGE	114.00	0.15	0.0
ROOF-6-7-S	STORAGE	114.00	0.15	0.0
ROOF-8-S	STORAGE	114.00	0.15	0.0
ROOF-9-S	STORAGE	114.00	0.15	0.0
TANKS	STORAGE	99.70	3.61	0.0

****
Link Summary
****

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	CB507-S	OF4	CONDUIT	21.3	0.5164	0.0130
C2	1000	100	CONDUIT	20.8	0.4327	0.0130
Pipe_13	100	HEADWALL	CONDUIT	11.1	0.2515	0.0130
CISTERN-O	TANKS	1000	ORIFICE			
W1	TANKS	1000	WEIR			
ROOF10-O	ROOF-10-S	TANKS	OUTLET			
ROOF11-O	ROOF-11-S	TANKS	OUTLET			
ROOF12-O	ROOF-12-S	TANKS	OUTLET			
ROOF1-2-O	ROOF-1-2-S	TANKS	OUTLET			
ROOF3-O	ROOF-3-S	TANKS	OUTLET			
ROOF4-O	ROOF-4-S	TANKS	OUTLET			
ROOF5-O	ROOF-5-S	TANKS	OUTLET			
ROOF6-7-O	ROOF-6-7-S	TANKS	OUTLET			
ROOF8-O	ROOF-8-S	TANKS	OUTLET			
ROOF9-O	ROOF-9-S	TANKS	OUTLET			

****
Cross Section Summary
****

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.25	0.05	0.06	0.25	1	42.74
C2	CIRCULAR	0.45	0.16	0.11	0.45	1	187.55
Pipe_13	CIRCULAR	0.90	0.64	0.23	0.90	1	907.85

****
Transect Summary
****

Transect Overland

Area:

0.0196	0.0392	0.0588	0.0784	0.0980
0.1177	0.1374	0.1571	0.1768	0.1965
0.2162	0.2360	0.2558	0.2756	0.2954
0.3152	0.3351	0.3550	0.3748	0.3947
0.4147	0.4346	0.4546	0.4745	0.4945
0.5145	0.5346	0.5546	0.5747	0.5947
0.6148	0.6350	0.6551	0.6752	0.6954
0.7156	0.7358	0.7560	0.7762	0.7965
0.8168	0.8371	0.8574	0.8777	0.8980
0.9184	0.9388	0.9592	0.9796	1.0000

Hrad:

0.0208	0.0415	0.0622	0.0829	0.1036
0.1242	0.1448	0.1653	0.1858	0.2063
0.2268	0.2472	0.2676	0.2879	0.3083
0.3285	0.3488	0.3690	0.3892	0.4094
0.4295	0.4496	0.4697	0.4897	0.5097
0.5297	0.5496	0.5695	0.5894	0.6093
0.6291	0.6489	0.6686	0.6884	0.7081
0.7277	0.7474	0.7670	0.7865	0.8061
0.8256	0.8451	0.8646	0.8840	0.9034
0.9228	0.9421	0.9614	0.9807	1.0000

Width:

0.9580	0.9589	0.9597	0.9606	0.9614
0.9623	0.9631	0.9640	0.9649	0.9657
0.9666	0.9674	0.9683	0.9691	0.9700
0.9709	0.9717	0.9726	0.9734	0.9743
0.9751	0.9760	0.9769	0.9777	0.9786
0.9794	0.9803	0.9811	0.9820	0.9829
0.9837	0.9846	0.9854	0.9863	0.9871
0.9880	0.9889	0.9897	0.9906	0.9914
0.9923	0.9931	0.9940	0.9949	0.9957

```
           0.9966     0.9974     0.9983     0.9991     1.0000

Transect Overland(orig)
Area:
           0.0196     0.0392     0.0588     0.0784     0.0980
           0.1177     0.1374     0.1571     0.1768     0.1965
           0.2162     0.2360     0.2558     0.2756     0.2954
           0.3152     0.3351     0.3550     0.3748     0.3947
           0.4147     0.4346     0.4546     0.4745     0.4945
           0.5145     0.5346     0.5546     0.5747     0.5947
           0.6148     0.6350     0.6551     0.6752     0.6954
           0.7156     0.7358     0.7560     0.7762     0.7965
           0.8168     0.8371     0.8574     0.8777     0.8980
           0.9184     0.9388     0.9592     0.9796     1.0000
Hrad:
           0.0208     0.0415     0.0622     0.0829     0.1036
           0.1242     0.1448     0.1653     0.1858     0.2063
           0.2268     0.2472     0.2676     0.2879     0.3083
           0.3285     0.3488     0.3690     0.3892     0.4094
           0.4295     0.4496     0.4697     0.4897     0.5097
           0.5297     0.5496     0.5695     0.5894     0.6093
           0.6291     0.6489     0.6686     0.6884     0.7081
           0.7277     0.7474     0.7670     0.7865     0.8061
           0.8256     0.8451     0.8646     0.8840     0.9034
           0.9228     0.9421     0.9614     0.9807     1.0000
Width:
           0.9580     0.9589     0.9597     0.9606     0.9614
           0.9623     0.9631     0.9640     0.9649     0.9657
           0.9666     0.9674     0.9683     0.9691     0.9700
           0.9709     0.9717     0.9726     0.9734     0.9743
           0.9751     0.9760     0.9769     0.9777     0.9786
           0.9794     0.9803     0.9811     0.9820     0.9829
           0.9837     0.9846     0.9854     0.9863     0.9871
           0.9880     0.9889     0.9897     0.9906     0.9914
           0.9923     0.9931     0.9940     0.9949     0.9957
           0.9966     0.9974     0.9983     0.9991     1.0000


*********************************************************
```

```
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
*********************************************************

****************
Analysis Options
****************
Flow Units ............... LPS
Process Models:
  Rainfall/Runoff ........ YES
  RDII ................... NO
  Snowmelt ............... NO
  Groundwater ............ NO
  Flow Routing ........... YES
  Ponding Allowed ........ YES
  Water Quality .......... NO
Infiltration Method ...... HORTON
Flow Routing Method ...... DYNWAVE
Surcharge Method ......... EXTRAN
Starting Date ............ 07/23/2009 00:00:00
Ending Date .............. 07/24/2009 00:00:00
Antecedent Dry Days ...... 0.0
Report Time Step ......... 00:05:00
Wet Time Step ............ 00:05:00
Dry Time Step ............ 00:05:00
Routing Time Step ........ 1.00 sec
Variable Time Step ....... NO
Maximum Trials ........... 8
Number of Threads ........ 1
Head Tolerance ........... 0.001500 m


**************************        Volume         Depth
Runoff Quantity Continuity     hectare-m            mm
**************************     ---------       -------
Total Precipitation ......         0.226        95.520
Evaporation Loss .........         0.000         0.000
Infiltration Loss ........         0.063        26.653
```

```
  Surface Runoff ...........         0.161        68.040
  Final Storage ............         0.002         1.029
  Continuity Error (%) .....        -0.212


  **************************        Volume        Volume
  Flow Routing Continuity        hectare-m      10^6 ltr
  **************************     ---------     ---------
  Dry Weather Inflow .......         0.000         0.000
  Wet Weather Inflow .......         0.161         1.607
  Groundwater Inflow .......         0.000         0.000
  RDII Inflow ..............         0.000         0.000
  External Inflow ..........         0.183         1.828
  External Outflow .........         0.336         3.358
  Flooding Loss ............         0.000         0.000
  Evaporation Loss .........         0.000         0.000
  Exfiltration Loss ........         0.000         0.000
  Initial Stored Volume ....         0.000         0.000
  Final Stored Volume ......         0.008         0.077
  Continuity Error (%) .....         0.005


  ********************************
  Highest Flow Instability Indexes
  ********************************
  All links are stable.


  *************************
  Routing Time Step Summary
  *************************
  Minimum Time Step           :     1.00 sec
  Average Time Step           :     1.00 sec
  Maximum Time Step           :     1.00 sec
  Percent in Steady State     :     0.00
  Average Iterations per Step :     2.00
  Percent Not Converging      :     0.00
```

```
  ***************************
  Subcatchment Runoff Summary
  ***************************
```

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imperv Runoff mm	Perv Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT-1	95.52	0.00	0.00	52.83	36.25	42.60	42.60	0.03	22.66	0.446
ROOF_10	95.52	0.00	0.00	0.00	94.22	0.00	94.22	0.26	98.91	0.986
ROOF_11	95.52	0.00	0.00	0.00	93.99	0.00	93.99	0.01	3.73	0.984
ROOF_12	95.52	0.00	0.00	0.00	94.03	0.00	94.03	0.01	4.41	0.984
ROOF_3	95.52	0.00	0.00	0.00	94.04	0.00	94.04	0.10	38.65	0.985
ROOF_4	95.52	0.00	0.00	0.00	94.03	0.00	94.03	0.03	12.40	0.984
ROOF_5	95.52	0.00	0.00	0.00	94.04	0.00	94.04	0.10	38.66	0.985
ROOF_8	95.52	0.00	0.00	0.00	93.99	0.00	93.99	0.01	3.43	0.984
ROOF_9	95.52	0.00	0.00	0.00	93.98	0.00	93.98	0.00	1.87	0.984
ROOF1_2	95.52	0.00	0.00	0.00	94.07	0.00	94.07	0.09	33.58	0.985
ROOF6_7	95.52	0.00	0.00	0.00	94.07	0.00	94.07	0.09	33.62	0.985
UGPK_1	95.52	0.00	0.00	14.36	80.16	7.64	80.16			

								0.12	49.41	0.839
UGPK_2	95.52	0.00	0.00	12.55	81.90	6.69	81.90	0.12	52.49	0.857
UGPK_3	95.52	0.00	0.00	26.08	68.85	13.80	68.85	0.04	20.01	0.721
UGPK_4	95.52	0.00	0.00	18.87	75.81	10.00	75.81	0.09	40.79	0.794
UGPK_5	95.52	0.00	0.00	18.87	75.81	10.00	75.81	0.08	37.46	0.794
UGPK_6	95.52	0.00	0.00	0.00	93.99	0.00	93.99	0.02	7.74	0.984
UGPK_7	95.52	0.00	0.00	13.46	81.04	7.16	81.04	0.09	38.52	0.848
UGPK_8	95.52	0.00	0.00	15.27	79.30	8.10	79.30	0.05	21.12	0.830
UGPK_9	95.52	0.00	0.00	0.00	94.01	0.00	94.01	0.03	11.43	0.984
UNC-1	95.52	0.00	0.00	51.92	38.94	43.47	43.47	0.03	25.77	0.455
UNC-2	95.52	0.00	0.00	72.25	8.08	23.25	23.25	0.12	39.00	0.243
UNC-3	95.52	0.00	0.00	43.07	57.73	51.73	51.73	0.04	23.41	0.542
UNC-4	95.52	0.00	0.00	53.22	34.91	42.20	42.20	0.02	16.94	0.442

****
Node Depth Summary
****

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days	hr:min	Reported Max Depth Meters
100	JUNCTION	0.23	0.43	99.83	0	06:46	0.43
HEADWALL	OUTFALL	0.00	0.00	98.70	0	00:00	0.00
OF1	OUTFALL	0.00	0.00	0.00	0	00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0	00:00	0.00
OF3	OUTFALL	0.00	0.00	0.00	0	00:00	0.00
OF4	OUTFALL	0.30	0.30	102.17	0	00:00	0.30
1000	STORAGE	0.07	0.25	100.17	0	06:23	0.25
CB507-S	STORAGE	0.00	0.18	102.74	0	06:15	0.18
ROOF-10-S	STORAGE	0.02	0.14	114.14	0	06:19	0.14
ROOF-11-S	STORAGE	0.01	0.12	114.12	0	06:18	0.12
ROOF-12-S	STORAGE	0.01	0.13	114.13	0	06:18	0.13
ROOF-1-2-S	STORAGE	0.01	0.14	114.14	0	06:19	0.14
ROOF-3-S	STORAGE	0.02	0.15	114.15	0	06:19	0.15
ROOF-4-S	STORAGE	0.01	0.13	114.13	0	06:19	0.13
ROOF-5-S	STORAGE	0.02	0.15	114.15	0	06:19	0.15
ROOF-6-7-S	STORAGE	0.01	0.14	114.14	0	06:19	0.14
ROOF-8-S	STORAGE	0.01	0.11	114.11	0	06:18	0.11
ROOF-9-S	STORAGE	0.00	0.09	114.09	0	06:16	0.08
TANKS	STORAGE	1.26	2.78	102.48	0	06:23	2.78

****
Node Inflow Summary
****

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days	hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
100	JUNCTION	136.19	222.37	0	06:46	1.83	3.12	0.008
HEADWALL	OUTFALL	0.00	222.38	0	06:46	0	3.12	0.000
OF1	OUTFALL	25.77	25.77	0	06:15	0.0339	0.0339	0.000
OF2	OUTFALL	39.00	39.00	0	06:15	0.12	0.12	0.000
OF3	OUTFALL	23.41	23.41	0	06:15	0.0359	0.0359	0.000
OF4	OUTFALL	0.00	39.58	0	06:15	0	0.0511	0.000
1000	STORAGE	0.00	111.32	0	06:23	0	1.29	0.010
CB507-S	STORAGE	39.61	39.61	0	06:15	0.0511	0.0511	-0.001
ROOF-10-S	STORAGE	98.91	98.91	0	06:10	0.265	0.265	-0.001
ROOF-11-S	STORAGE	3.73	3.73	0	06:05	0.00997	0.00997	-0.001
ROOF-12-S	STORAGE	4.41	4.41	0	06:10	0.0118	0.0118	-0.001

ROOF-1-2-S	STORAGE	33.58	33.58	0	06:10	0.0897	0.0897	-0.001
ROOF-3-S	STORAGE	38.65	38.65	0	06:10	0.103	0.103	-0.001
ROOF-4-S	STORAGE	12.40	12.40	0	06:15	0.0331	0.0331	-0.001
ROOF-5-S	STORAGE	38.66	38.66	0	06:10	0.103	0.103	-0.001
ROOF-6-7-S	STORAGE	33.62	33.62	0	06:10	0.0898	0.0898	-0.001
ROOF-8-S	STORAGE	3.43	3.43	0	06:05	0.00916	0.00916	-0.001
ROOF-9-S	STORAGE	1.87	1.87	0	06:10	0.00499	0.00499	-0.001
TANKS	STORAGE	278.96	337.67	0	06:15	0.647	1.37	0.001

****
Node Surcharge Summary
****

No nodes were surcharged.

****
Node Flooding Summary
****

No nodes were flooded.

****
Storage Volume Summary
****

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days	hr:min	Maximum Outflow LPS
1000	0.000	2	0	0	0.000	6	0	06:23	111.34
CB507-S	0.000	0	0	0	0.000	0	0	00:00	39.58
ROOF-10-S	0.006	5	0	0	0.093	82	0	06:19	19.23
ROOF-11-S	0.000	1	0	0	0.002	48	0	06:18	1.68
ROOF-12-S	0.000	2	0	0	0.003	67	0	06:18	1.77
ROOF-1-2-S	0.002	4	0	0	0.030	78	0	06:19	8.29
ROOF-3-S	0.003	7	0	0	0.041	91	0	06:19	7.34
ROOF-4-S	0.000	3	0	0	0.010	67	0	06:19	3.15
ROOF-5-S	0.003	7	0	0	0.041	91	0	06:19	7.34
ROOF-6-7-S	0.002	4	0	0	0.030	78	0	06:19	8.29
ROOF-8-S	0.000	1	0	0	0.002	44	0	06:18	1.66
ROOF-9-S	0.000	0	0	0	0.000	19	0	06:16	1.48
TANKS	0.281	35	0	0	0.618	77	0	06:23	111.32

****
Outfall Loading Summary
****

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
HEADWALL	90.72	39.77	222.38	3.117
OF1	6.58	5.97	25.77	0.034
OF2	11.58	11.97	39.00	0.120
OF3	12.63	3.29	23.41	0.036
OF4	6.36	9.30	39.58	0.051
System	25.57	70.29	253.17	3.358

****
Link Flow Summary
****

Link	Type	Maximum |Flow| LPS	Time of Max Occurrence days	hr:min	Maximum |Veloc| m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	39.58	0	06:15	1.09	0.93	0.69
C2	CONDUIT	111.34	0	06:24	1.28	0.59	0.54
Pipe_13	CONDUIT	222.38	0	06:46	1.35	0.24	0.31

CISTERN-O	ORIFICE	18.15	0	06:23	1.00
W1	WEIR	93.17	0	06:23	0.23
ROOF10-O	DUMMY	19.23	0	06:19	
ROOF11-O	DUMMY	1.68	0	06:18	
ROOF12-O	DUMMY	1.77	0	06:18	
ROOF1-2-O	DUMMY	8.29	0	06:19	
ROOF3-O	DUMMY	7.34	0	06:19	
ROOF4-O	DUMMY	3.15	0	05:52	
ROOF5-O	DUMMY	7.34	0	06:19	
ROOF6-7-O	DUMMY	8.29	0	06:19	
ROOF8-O	DUMMY	1.66	0	06:18	
ROOF9-O	DUMMY	1.48	0	06:16	

****
Flow Classification Summary
****

	Adjusted	---------- Fraction of Time in Flow Class ----------								
	/Actual		Up	Down	Sub	Sup	Up	Down	Norm	Inlet
Conduit	Length	Dry	Dry	Dry	Crit	Crit	Crit	Crit	Ltd	Ctrl
C1	1.00	0.24	0.00	0.00	0.00	0.00	0.00	0.76	0.00	0.00
C2	1.00	0.11	0.00	0.00	0.00	0.00	0.00	0.89	0.00	0.00
Pipe_13	1.00	0.09	0.00	0.00	0.00	0.00	0.00	0.91	0.00	0.00

****
Conduit Surcharge Summary
****

No conduits were surcharged.

Analysis begun on: Tue Mar 29 13:35:49 2022
Analysis ended on: Tue Mar 29 13:35:50 2022
Total elapsed time: 00:00:01

C.4 OIL/GRIT SEPARATOR SIZING CALCULATIONS

Detailed Stormceptor Sizing Report – WOS PH2 20 Cedarow Crt

Project Information & Location			
Project Name	WOS PH2	**Project Number**	20349
City	Ottawa	**State/ Province**	Ontario
Country	Canada	**Date**	11/4/2019
Designer Information		**EOR Information (optional)**	
Name	thakshika rathnasooriya	**Name**	
Company	stantec	**Company**	
Phone #	613-724-4081	**Phone #**	
Email	thakshika.rathnasooriya@stantec.com	**Email**	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WOS PH2 20 Cedarow Crt
Recommended Stormceptor Model	STC 300
Target TSS Removal (%)	80.0
TSS Removal (%) Provided	80
PSD	Fine Distribution
Rainfall Station	OTTAWA MACDONALD-CARTIER INT'L A

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	**% TSS Removal Provided**
STC 300	80
STC 750	85
STC 1000	85
STC 1500	85
STC 2000	86
STC 3000	87
STC 4000	88
STC 5000	89
STC 6000	90
STC 9000	92
STC 10000	92
STC 14000	94
StormceptorMAX	Custom

Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor's patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur.
Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM's precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor's unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour).
Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

Rainfall Station

State/Province	Ontario	**Total Number of Rainfall Events**	4093
Rainfall Station Name	OTTAWA MACDONALD-CARTIER INT'L A	**Total Rainfall (mm)**	20978.1
Station ID #	6000	**Average Annual Rainfall (mm)**	567.0
Coordinates	45°19'N, 75°40'W	**Total Evaporation (mm)**	982.0
Elevation (ft)	370	**Total Infiltration (mm)**	10341.2
Years of Rainfall Data	37	**Total Rainfall that is Runoff (mm)**	9654.9

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (ha)	1.60
Imperviousness %	50.60

Up Stream Storage	
Storage (ha-m)	Discharge (cms)
0.000	0.000
0.030	0.007
0.060	0.015
0.090	0.022

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (L)	
Peak Conveyed Flow Rate (L/s)	126.00
Water Quality Flow Rate (L/s)	

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cms)	

Design Details	
Stormceptor Inlet Invert Elev (m)	
Stormceptor Outlet Invert Elev (m)	
Stormceptor Rim Elev (m)	
Normal Water Level Elevation (m)	
Pipe Diameter (mm)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

Particle Size Distribution (PSD)

Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.

Fine Distribution		
Particle Diameter (microns)	Distribution %	Specific Gravity
20.0	20.0	1.30
60.0	20.0	1.80
150.0	20.0	2.20
400.0	20.0	2.65
2000.0	20.0	2.65

Site Name	WOS PH2 20 Cedarow Crt

Site Details

Drainage Area

Drainage Area	
Total Area (ha)	1.60
Imperviousness %	50.60

Surface Characteristics

Surface Characteristics	
Width (m)	253.00
Slope %	2
Impervious Depression Storage (mm)	0.508
Pervious Depression Storage (mm)	5.08
Impervious Manning's n	0.015
Pervious Manning's n	0.25

Maintenance Frequency

Maintenance Frequency	
Maintenance Frequency (months) >	12

Infiltration Parameters

Horton's equation is used to estimate infiltration

Infiltration Parameters	
Max. Infiltration Rate (mm/hr)	61.98
Min. Infiltration Rate (mm/hr)	10.16
Decay Rate (1/sec)	0.00055
Regeneration Rate (1/sec)	0.01

Evaporation

Evaporation	
Daily Evaporation Rate (mm/day)	2.54

Dry Weather Flow

Dry Weather Flow	
Dry Weather Flow (lps)	0

Winter Months

Winter Months	
Winter Infiltration	0

TSS Loading Parameters

TSS Loading Parameters	
TSS Loading Function	

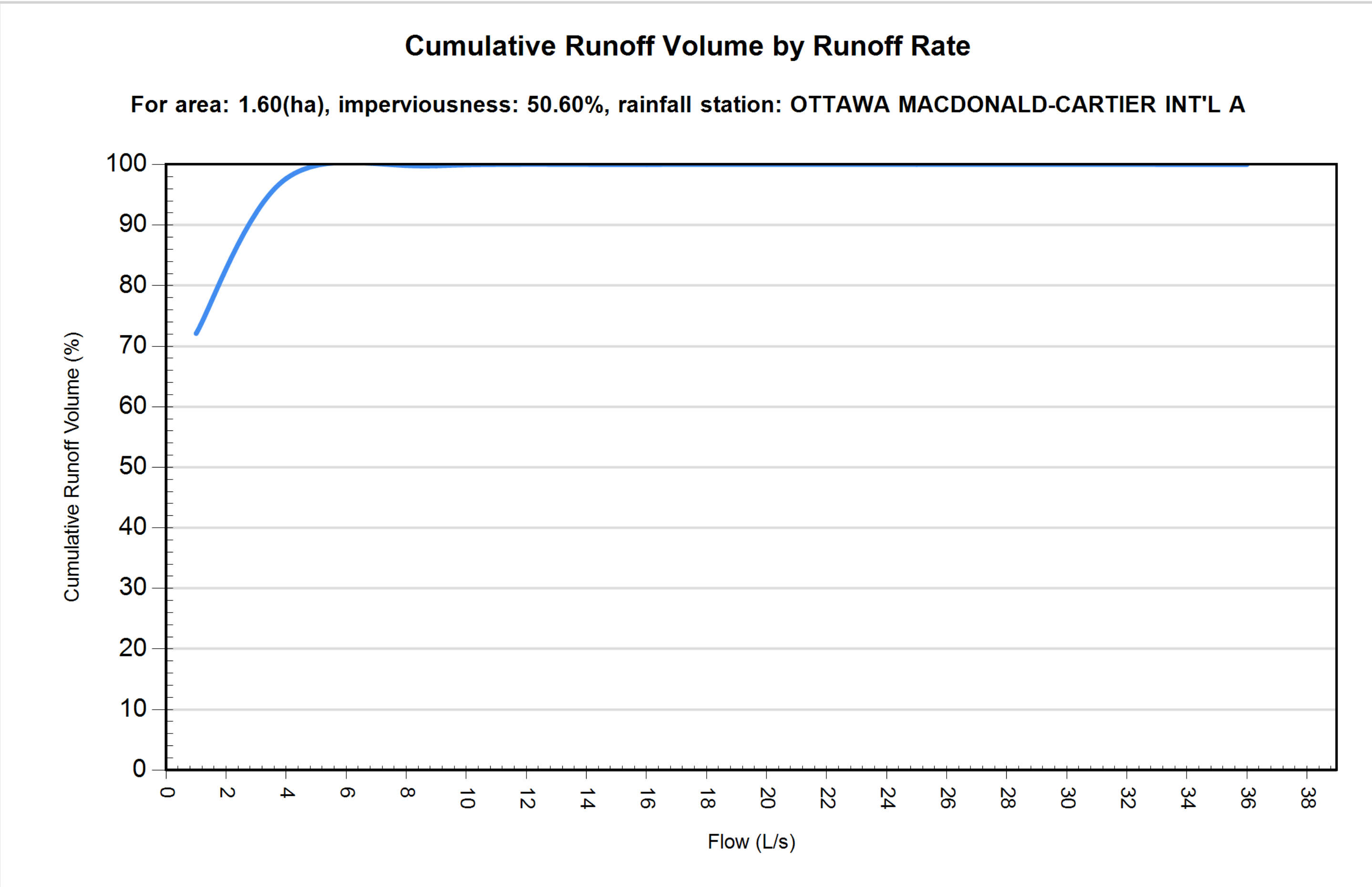
Buildup/Wash-off Parameters

Buildup/Wash-off Parameters	
Target Event Mean Conc. (EMC) mg/L	
Exponential Buildup Power	
Exponential Washoff Exponent	

TSS Availability Parameters

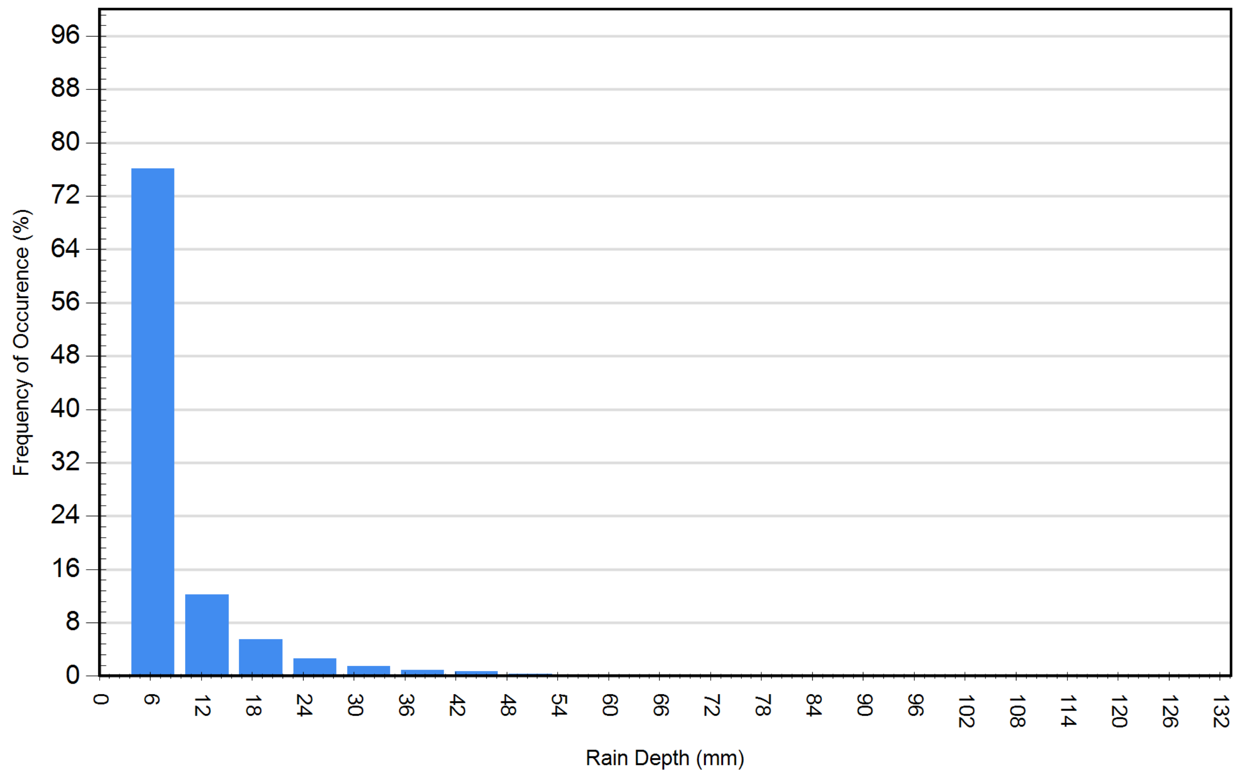
TSS Availability Parameters	
Availability Constant A	
Availability Factor B	
Availability Exponent C	
Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (L/s)	Runoff Volume (m³)	Volume Over (m³)	Cumulative Runoff Volume (%)
1	111983	43648	72.1
4	151637	3654	97.7
9	154907	370	99.8
16	155201	73	100.0
25	155273	0	100.0
36	155273	0	100.0



Rainfall Event Analysis				
Rainfall Depth (mm)	No. of Events	Percentage of Total Events (%)	Total Volume (mm)	Percentage of Annual Volume (%)
6.35	3113	76.1	5230	24.9
12.70	501	12.2	4497	21.4
19.05	225	5.5	3469	16.5
25.40	105	2.6	2317	11.0
31.75	62	1.5	1765	8.4
38.10	35	0.9	1206	5.8
44.45	28	0.7	1163	5.5
50.80	12	0.3	557	2.7
57.15	7	0.2	378	1.8
63.50	1	0.0	63	0.3
69.85	1	0.0	64	0.3
76.20	1	0.0	76	0.4
82.55	0	0.0	0	0.0
88.90	1	0.0	84	0.4
95.25	0	0.0	0	0.0
101.60	0	0.0	0	0.0
107.95	0	0.0	0	0.0
114.30	1	0.0	109	0.5
120.65	0	0.0	0	0.0
127.00	0	0.0	0	0.0

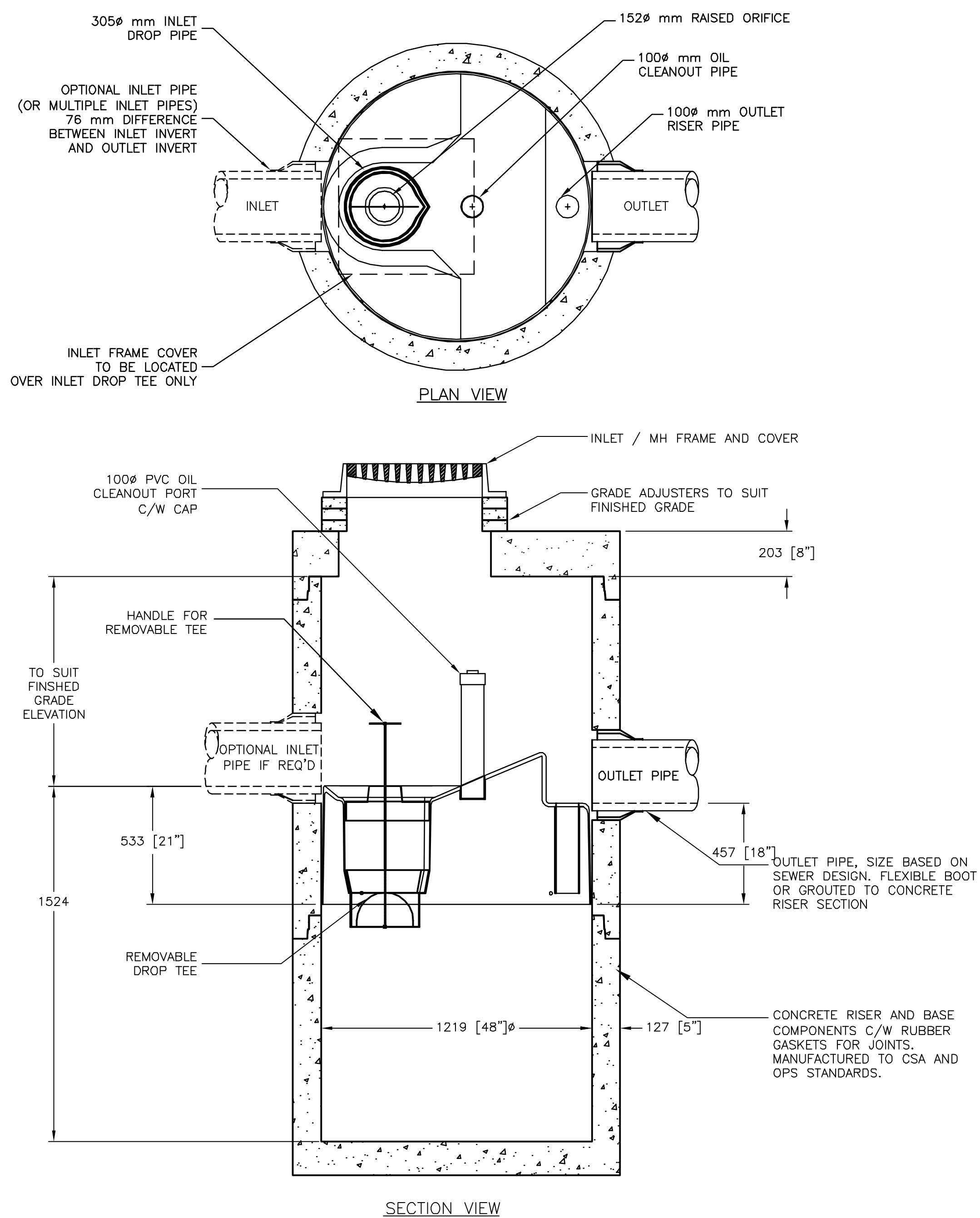
Frequency of Occurence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:
http://www.imbriumsystems.com/technical-specifications

DRAWING NOT TO BE USED FOR CONSTRUCTION

THE STORMCEPTOR SYSTEM IS PROTECTED BY ONE OR MORE OF THE FOLLOWING PATENTS:
United States Patent No. 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 693,164 • 707,133 • 729,096 • 779,401 • 289,647 • 2008,279,378 • 2008,288,900 | Canadian Patent No. 2,009,280 • 2,137,942 • 2,175,277 • 2,180,305 • 2,180,383 • 2,206,338 • 2,327,768 | Indonesian Patent No. 007058 | Japan Patent No. 3581233 • 9-11476 | Korea Patent No. 10-1451593 • 0519212 | Malaysia Patent No. 118987 | New Zealand Patent No. 314,646 • 583,583 • 583,008 | South African Patent No. 2010/00683 • 2010/01796 |



Stormceptor®

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TF 800-565-4801 CA 416-960-9900 INTL +1-416-960-9900

STC 300i
STANDARD MODEL

####

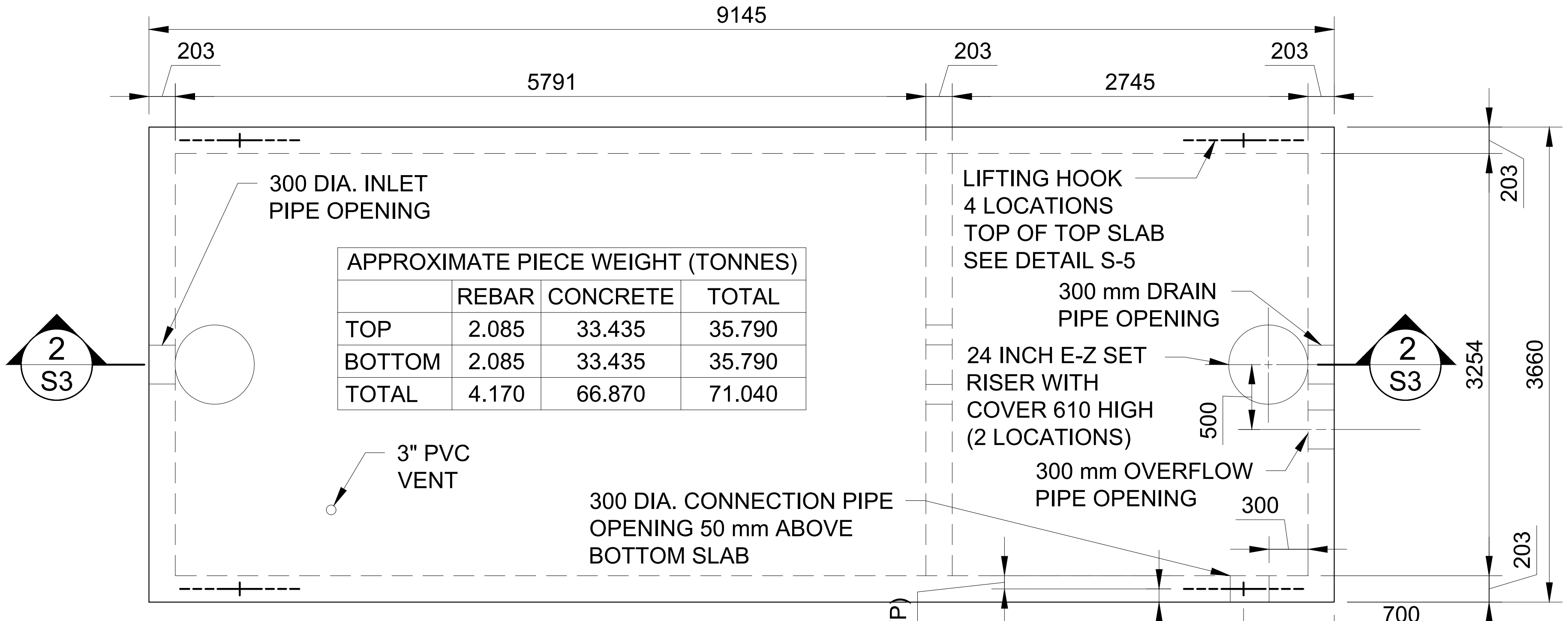
REV #	DATE	REVISION DESCRIPTION	BY

SHEET NUMBER 1 OF 1

DATE:######## | SCALE: 30 | PROJECT No.: ##### | DRAWN: ### | CHECKED: ###

I:\IMBRIUM\PRODUCTS\STORMCEPTOR\STORMCEPTOR STC40 DRAWINGS & DETAILS\STANDARD DETAILS\METRIC\STC 300I.DWG 4/12/2019 1:21 PM

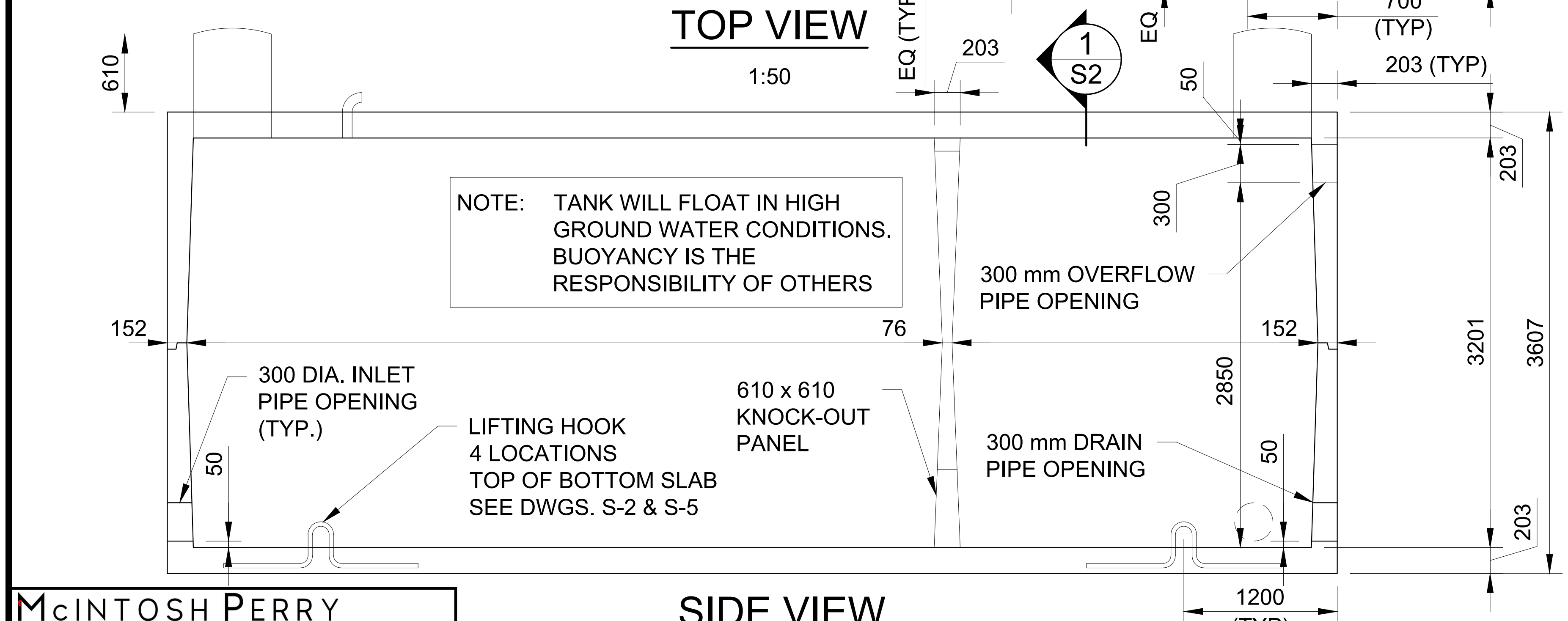
C.5 TANK SPECIFICATIONS



APPROXIMATE PIECE WEIGHT (TONNES)			
	REBAR	CONCRETE	TOTAL
TOP	2.085	33.435	35.790
BOTTOM	2.085	33.435	35.790
TOTAL	4.170	66.870	71.040

TOP VIEW

1:50



SIDE VIEW

1:50

GENERAL NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETERS UNLESS NOTED OTHERWISE.
2. PRECAST CONCRETE TANK DESIGNED TO 2012 ONTARIO BUILDING CODE CSA B66-16. BEDDING, WATERPROOFING, BACKFILL, AND ALL OTHER SITE WORK BY OTHERS.
3. CONCRETE WORK TO BE IN ACCORDANCE WITH CAN/CSA A23.1 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION, AND CAN/CSA A23.4 PRECAST CONCRETE MATERIALS AND CONSTRUCTION.
4. BOX DESIGNED FOR A MAXIMUM FILL HEIGHT OF 760 mm WITH 15 kPa LIVE LOAD. DESIGN FILL COVER ON THIS TANK IS 610 mm. 2
5. CONCRETE TO BE 45 MPa SCC WITH 600 mm ±70 mm SLUMP.
6. CONCRETE COVER AS NOTED, WITH ±10 mm TOLERANCE.
7. REINFORCING STEEL TO BE GRADE 400W BLACK DEFORMED BARS CONFORMING TO CAN/CSA G30.18.
8. DO NOT LIFT UNITS UNTIL CONCRETE HAS REACHED A MINIMUM STRENGTH OF 25 MPa, AS DETERMINED BY COMPRESSIVE TESTING OF CONCRETE CYLINDERS CURED IN SIMILAR CONDITIONS.
9. GROUT TO BE NON-SHRINK CEMENTITIOUS GROUT WITH MINIMUM COMPRESSIVE STRENGTH OF 45 MPa.
10. JOINT SEAL TO BE CONSEAL CS-102 BUTYL RUBBER SEALANT (CONFORMS TO AASHTO M-198B AND ASTM C-990-91). STORE, HANDLE AND APPLY JOINT SEALS IN STRICT ACCORDANCE WITH MANUFACTURER PRODUCT DATA SHEETS.
11. IT IS STRUCTURALLY IMPORTANT THAT THE SEALS IN THE HORIZONTAL JOINTS BE INSTALLED CORRECTLY. TOP PIECES MUST BE INSTALLED WITHOUT SLIDING THE PIECES ON THE SEALS.
12. DESIGN BASED ON GRANULAR BEDDING AND BACKFILL COMPACTED TO 95% SPDD.
13. LIFTING INSERTS TO BE GROUTED ON SITE BY OTHERS.
14. DELIVERY IS MADE BY CRANE-EQUIPPED TRUCKS.
15. EXCAVATION MUST BE READY, SAFE AND ACCESSIBLE FOR UNLOADING FROM THE REAR OF THE TRUCK.
16. MINIMUM OVERHEAD CLEARANCE OF 18 FT. IS REQUIRED.
17. ALL UNITS MUST BE HANDLED WITH PROPER LIFTING EQUIPMENT (i.e. SPREADER BARS).

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MORRISON HERSHFIELD

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5598 POWER ROAD
OTTAWA, ONTARIO
TEL: 613-822-1488

Version	Description	YYYY-MM-DD
2	REVISED FOR 15KPA LIVE LOAD	2022-07-08
1	ISSUED FOR REVIEW	2018-08-31

Stamp

LICENSED PROFESSIONAL ENGINEER
2022-07-08
A. SAIDOU SANDA
100232005
PROVINCE OF ONTARIO

McIntosh Perry

WELLINGS OF STITTSVILLE

PPS JOB NO.: 2018-0803
MH JOB NO.: 2170737.34

TANK DIMENSIONS

Scale 1:50

DWG No.	Version	Date
S-1	1	2018-08-28

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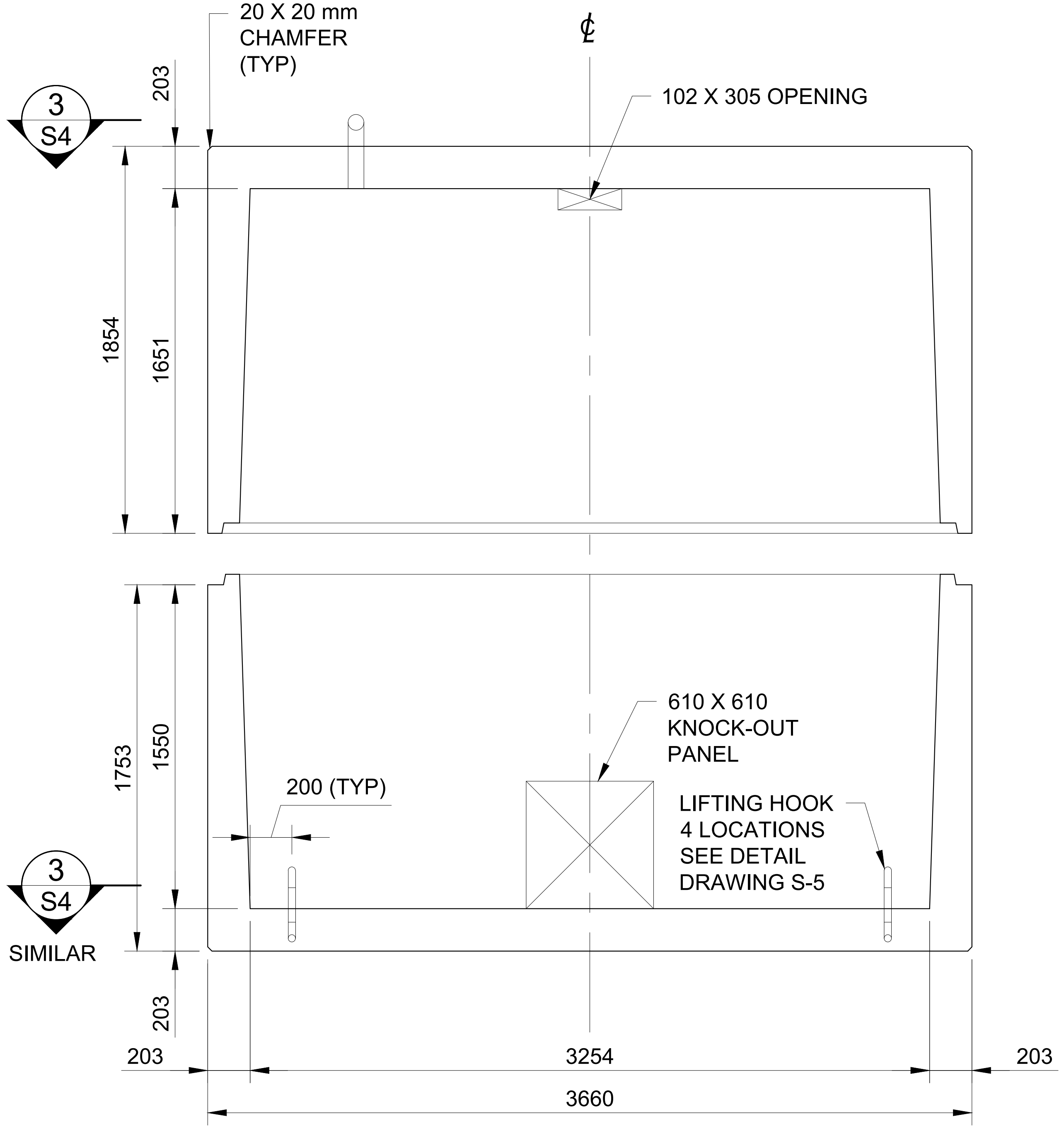
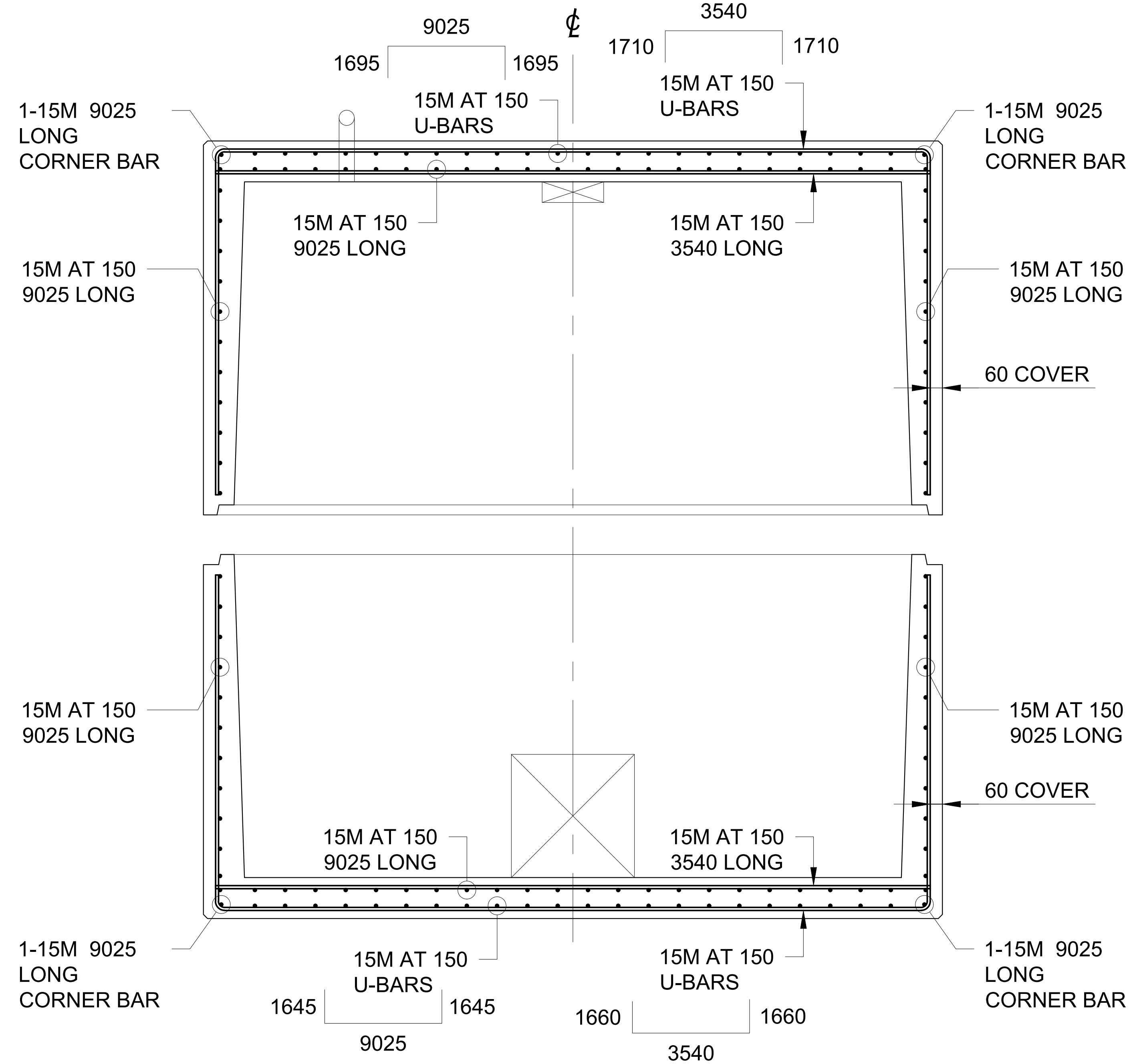


END VIEW

1:30



SECTION 1 S-1

1:30

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SECTIONS

Scale
AS NOTED

DWG No.	Version	Date
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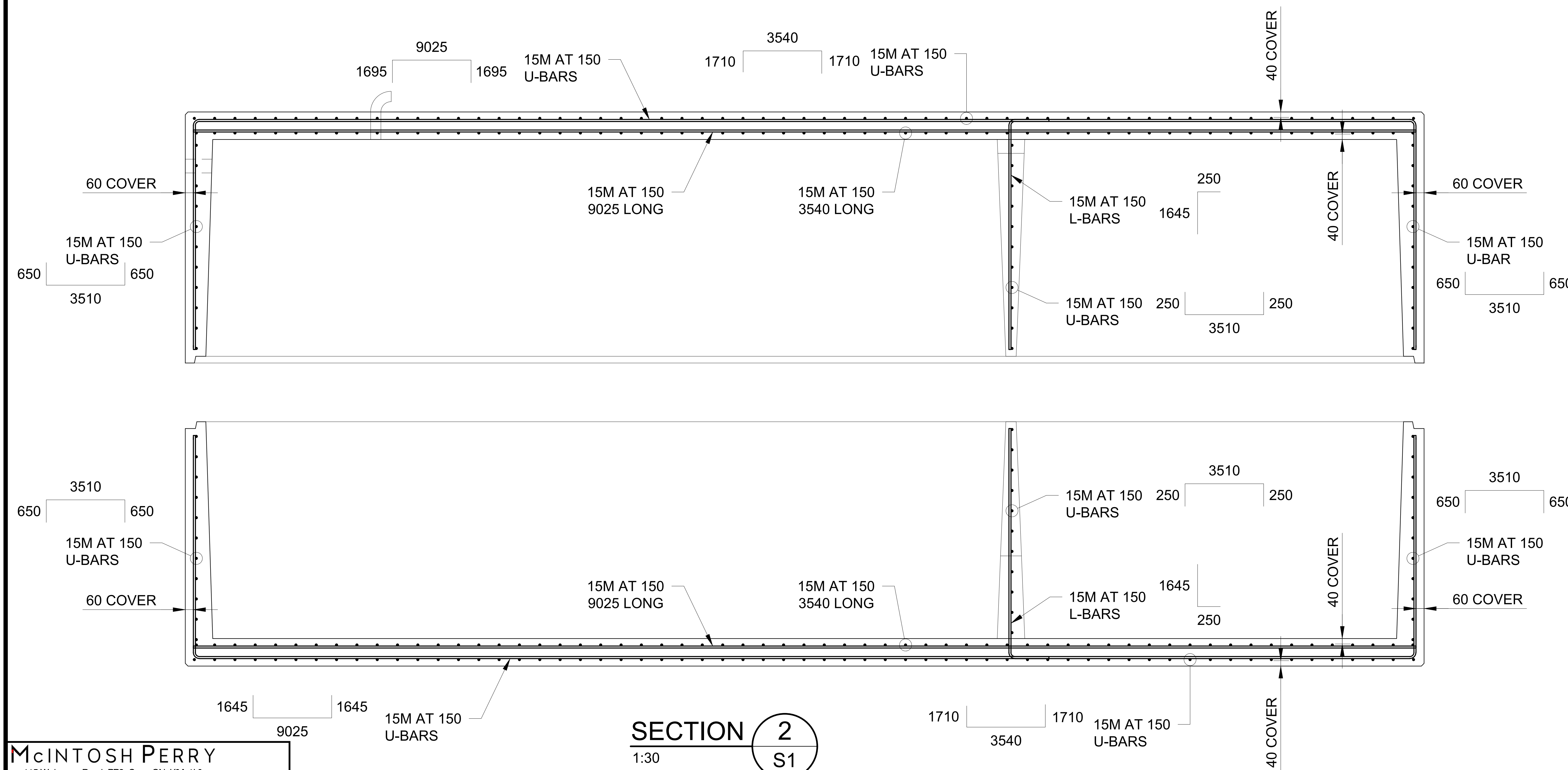

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Scale
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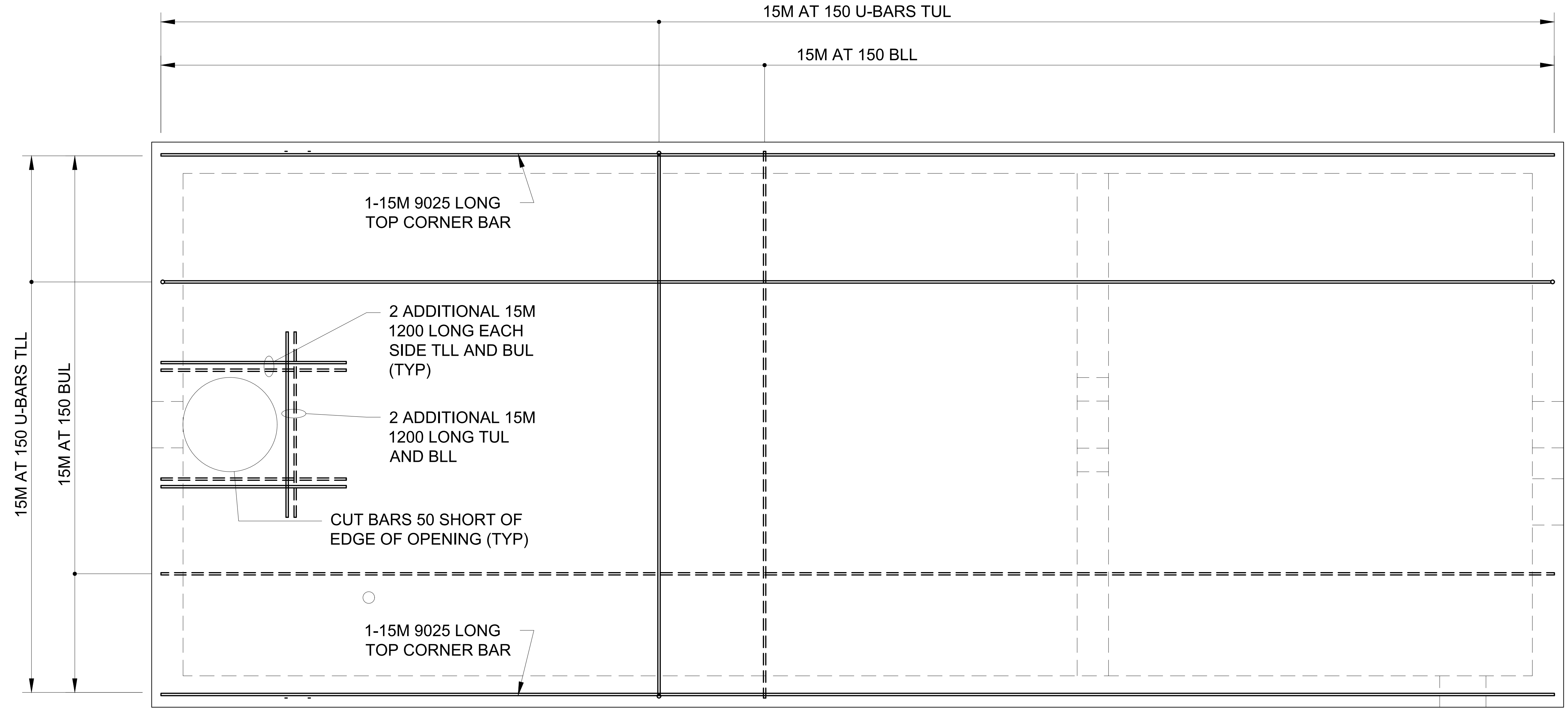

SECTION 3 / S2
1:30

LEGEND

———	DENOTES TOP MAT BARS	TUL	TOP UPPER LAYER
		TLL	TOP LOWER LAYER
- - - -	DENOTES BOTTOM MAT BARS	BUL	BOTTOM UPPER LAYER
		BLL	BOTTOM LOWER LAYER

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DWG No.	Version	Date
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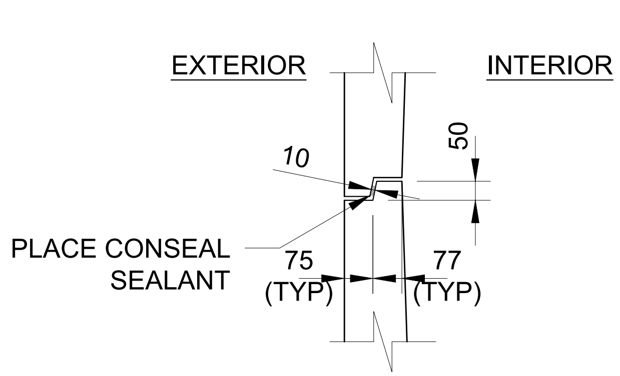
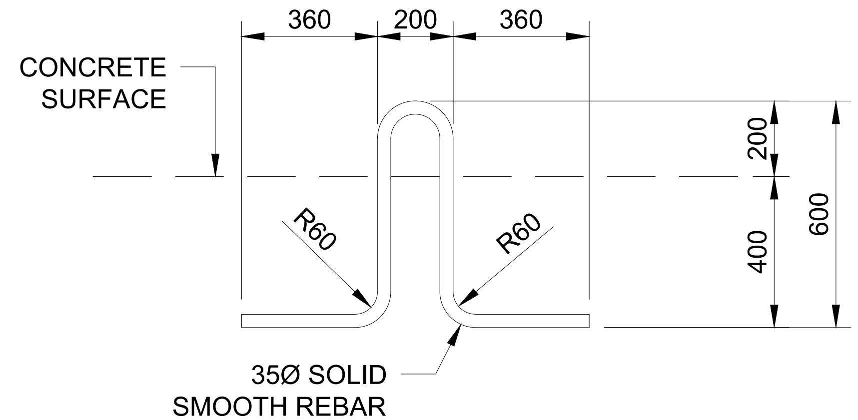
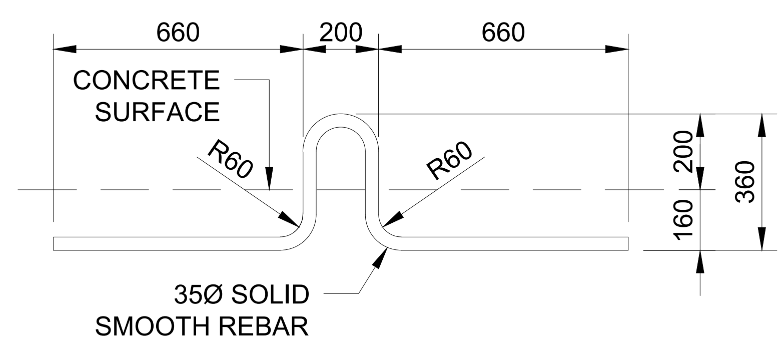


TYPICAL WALL JOINT DETAIL
1:20



TOP LIFTING HOOK DETAIL
1:20



BOTTOM LIFTING HOOK DETAIL
1:20

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DETAILS

Scale
AS NOTED

DWG No.	Version	Date
S-5	1	2018-08-28

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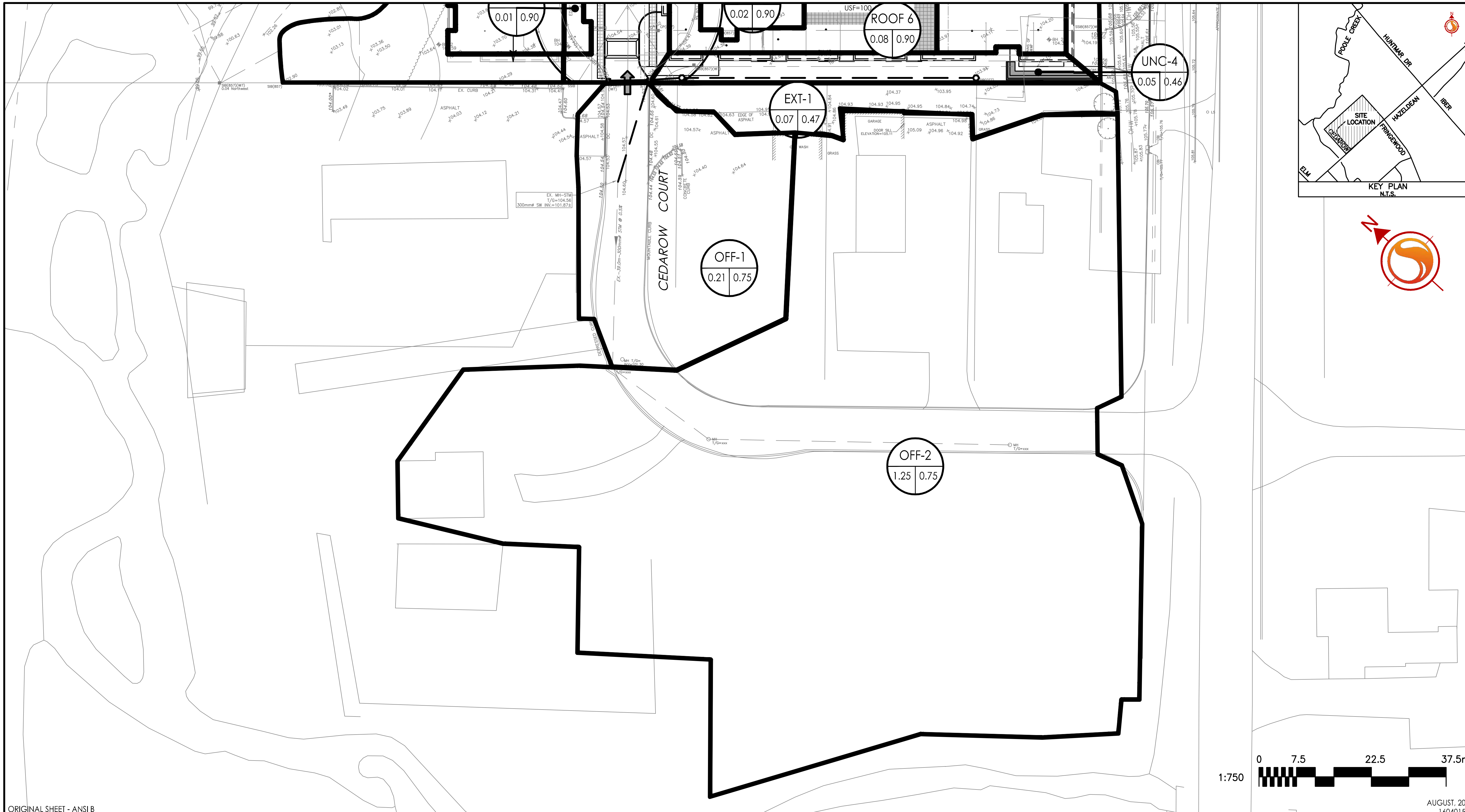
C.6 CEDAROW COURT STORM SEWER CAPACITY



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2021/08/31 7:53 AM By: Sharp, Mike

ORIGINAL SHEET - ANSI B

AUGUST, 2021
160401511

Stantec
Stantec Consulting Ltd.
400 - 1331 Clyde Avenue
Ottawa ON
Tel. 613.722.4420
www.stantec.com

Legend

OFF-1	AREA ID
0.75	RUNOFF COEFFICIENT
1.00	STORM DRAINAGE AREA ha.
(thick line)	STORM DRAINAGE BOUNDARY
(arrow)	DIRECTION OF OVERLAND FLOW

Notes

Client/Project
NAUTICAL LANDS GROUP
WELLINGS OF STITTSVILLE PH 2
20 CEDAROW COURT

Figure No.
1.0

Title
CEDAROW COURT
EXISTING STORM DRAINAGE PLAN

Stantec

Cedarow Court

DATE:	2021-09-01
REVISION:	1
DESIGNED BY:	TR
CHECKED BY:	-

STORM SEWER DESIGN SHEET
(City of Ottawa)

FILE NUMBER: **160401511**

DESIGN PARAMETERS

$I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)

	1:2 yr	1:5 yr	1:10 yr	1:100 yr
a =	732.951	998.071	1174.184	1735.688
b =	6.199	6.053	6.014	6.014
c =	0.810	0.814	0.816	0.820

MANNING'S n = 0.013 BEDDING CLASS = B

MINIMUM COVER: 2.00 m

TIME OF ENTRY 10 min

LOCATION			DRAINAGE AREA															
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (2-YEAR) (ha)	AREA (5-YEAR) (ha)	AREA (10-YEAR) (ha)	AREA (100-YEAR) (ha)	AREA (ROOF) (ha)	C (2-YEAR) (-)	C (5-YEAR) (-)	C (10-YEAR) (-)	C (100-YEAR) (-)	A x C (2-YEAR) (ha)	ACCUM AxC (2YR) (ha)	A x C (5-YEAR) (ha)	ACCUM. AxC (5YR) (ha)	A x C (10-YEAR) (ha)	ACCUM. AxC (10YR) (ha)	A x C (100-YEAR) (ha)
UNC-4 + EXT-1	CB507	EX1	0.00	0.12	0.00	0.00	0.00	0.00	0.47	0.00	0.00	0.000	0.000	0.056	0.056	0.000	0.000	0.000
OFF-1	EX1	EX2	0.00	0.21	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.000	0.000	0.154	0.210	0.000	0.000	0.000
OFF-2	EX2	POOLE CREEK	0.00	1.25	0.00	0.00	0.00	0.00	0.75	0.00	0.00	0.000	0.000	0.937	1.147	0.000	0.000	0.000

AREA ID NUMBER	ACCUM. AxC (100YR) (ha)	T of C (min)	$I_{2\text{-YEAR}}$ (mm/h)	$I_{5\text{-YEAR}}$ (mm/h)	$I_{10\text{-YEAR}}$ (mm/h)	$I_{100\text{-YEAR}}$ (mm/h)	$Q_{CONTROL}$ (L/s)	ACCUM. $Q_{CONTROL}$ (L/s)	Q_{ACT} (CIA/360) (L/s)
UNC-4 + EXT-1	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	16.2
OFF-1	0.000	10.52	74.85	101.50	118.97	173.90	0.0	0.0	59.2
OFF-2	0.000	11.19	72.52	98.30	115.20	168.37	0.0	0.0	313.1

	PIPE SELECTION											
AREA ID NUMBER	LENGTH (m)	PIPE WIDTH OR DIAMETE (mm)	PIPE HEIGHT (mm)	PIPE SHAPE (-)	MATERIAL (-)	CLASS (-)	SLOPE %	Q_{CAP} (FULL) (L/s)	% FULL (-)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)
UNC-4 + EXT-1	21.3	250	250	CIRCULAR	CONCRETE	-	0.50	42.7	**37.91%**	0.86	0.68	0.52
OFF-1	39.0	300	300	CIRCULAR	CONCRETE	-	0.50	68.0	**87.02%**	0.97	0.98	0.67
OFF-2	86.8	525	525	CIRCULAR	CONCRETE	-	0.62	353.3	**88.62%**	1.58	1.61	0.90

C.7 EXCERPTS FROM WOS PHASE 1

5.0 STORMWATER MANAGEMENT

5.1 OBJECTIVES

The objective of this stormwater management plan is to determine the measures necessary to control the quantity of stormwater released from the proposed development to established criteria, and to provide sufficient detail for approval and construction. The proposed development will discharge treated and controlled stormwater runoff to Poole Creek.

5.2 SWM CRITERIA AND CONSTRAINTS

Criteria were established by combining current design practices outlined by the City of Ottawa Design Guidelines (2012), Ministry of Environment and Climate Change (MOECC) and Mississippi Valley Conservation Authority (MVCA). The following summarizes the criteria, with the source of each criterion indicated in italics:

General

- Use of the dual drainage principle (City of Ottawa)
- Wherever feasible and practical, site-level measures should be used to reduce and control the volume and rate of runoff (City of Ottawa)
- Site-level infiltration measures to be implemented to meet infiltration criteria of minimum 50 mm/yr (MVCA)
- Assess impact of 100 year event outlined in the City of Ottawa Sewer Design Guidelines, and climate change scenarios with a 20% increase of rainfall intensity, on major & minor drainage system (City of Ottawa)
- Quality control to be provided for 80% TSS removal (MVCA, MOECC)
- Site discharge to be controlled to pre-development rates (MVCA, City of Ottawa)
- Site design to mitigate erosion impacts on Poole Creek (City of Ottawa)

Storm Sewer & Inlet Controls

- Size storm sewers to convey the 5 year storm event under free-flow conditions using City of Ottawa I-D-F parameters (City of Ottawa) with the exception of the outlet sewer from the proposed underground storage facility.
- Minimum sewer inlet capture rates to be set such that no ponding occurs at the end of the 5-year event (City of Ottawa)
- Hydraulic Grade Line (HGL) analysis to be conducted using the 100 year 12 hour SCS storm distribution (City of Ottawa).
- 100-year Storm HGL to be a minimum of 0.30 m below building foundation footing otherwise foundation drains will be pumped (City of Ottawa)

Surface Storage & Overland Flow

- Building openings to be a minimum of 0.30m above the 100-year water level (City of Ottawa)
- Maximum depth of flow under either static or dynamic conditions shall be less than 0.30m (City of Ottawa)
- Subdrains required in swales where longitudinal gradient is less than 1.5% (City of Ottawa)
- Provide adequate emergency overflow conveyance off-site (City of Ottawa)

5.2.1 Pre-Development Conditions

A lumped catchment PCSWMM model was created for the subject site based on a site area of 2.9ha, and utilizing an existing SCS curve number of 80 per background documents. Additional subcatchment parameters were defined based upon recent topographical survey of the property:

Area (ha)	Width (m)	Slope (%)	Imperv. (%)	Subarea Routing
2.90	161.1	1.0	0.0	Outlet

Based on the above and during the 2 through 100-year 12hr SCS events (MTO Distribution curves), peak pre-development outflow rates from the subject site were identified per the tables below:

Storm Event	2-Year	5-Year	10-Year
Peak Outflow Rate	17.7L/s	43.9L/s	66.2L/s

Storm Event	25-Year	50-Year	100-Year
Peak Outflow Rate	103.7L/s	136.5L/s	176.3L/s

PCSWMM model input and output files for the predevelopment scenario are included within **Appendix C.**

5.3 STORMWATER MANAGEMENT DESIGN

5.3.1 Rationale for Design and Servicing Deviations

5.3.1.1 Deviation from Kanata West MSS

Per the findings of the Kanata West MSS, stormwater outflows from the proposed site were intended to be directed to the storm sewer within Huntmar Drive, and in turn directed to the downstream Fairwinds temporary pond 5. The MSS had assumed that the entire area of land west of Huntmar Drive and bound by Poole creek to the north and Hazeldean Road to the south was to be directed to the Huntmar Drive sewer, however, the proposed site forms only part of the tributary area, within lands owned by others blocking direct access to the storm sewer within Huntmar Drive. Rather than encumbering the adjacent property, and to avoid considerable connection fees associated with the outlet from the Kanata West Owners Group (KWOG), a separate outlet for the site to Poole Creek has been considered. As the downstream Pond 5 discharges to Poole Creek as well, by restricting flows to predevelopment levels, and assessing the erosive potential of such flows for the Poole Creek reach between the site outlet and that of the downstream Pond 5, no deleterious effects to the downstream watercourse are expected. Additionally, this option provides additional potential to supplement baseflows to Poole Creek in accordance with recommendations from the MVCA.

5.3.1.2 Deviation from Standard SWM Design

The proposed SWM design includes three LID measures to encourage on-site infiltration and water re-use for irrigation. It is recognized that these measures are not currently standard SWM controls and when they are used for water balance purposes are not traditionally included in SWM calculations due to concerns over longterm reliability. The proposed SWM design has included some of the storage and infiltration/reuse rates from these measures in the supporting analysis as discussed in the following sections. However the analysis has also included simulations assuming that these measures fail in order to assess the potential associated impacts. The benefit of including some of the storage an infiltration losses associated with the LID measures was that the end-of-pipe underground storage component of the infiltration gallery was able to be reduced by 30% as compared to previous design requirements when no credit was assigned for the LID measures. As discussed later in this report, a monitoring plan will be developed and implemented to ensure that constructed LID measures are performing as designed.

5.3.2 Design Methodology

The intent of the stormwater management plan presented herein is to mitigate negative impacts that the proposed development might have on the receiving watercourse (Poole Creek), while providing adequate capacity to service the proposed buildings, underground parking and access areas. The proposed stormwater management plan is designed to detain runoff on the rooftop, surface and in the subsurface (StormTech chamber) to ensure that peak flows after construction will not exceed the target discharge rates and erosion mitigation requirements.

Runoff from the site is captured via catchbasins, landscaping drains and roof drains and conveyed to a hydrodynamic separator for water quality treatment followed by an underground storage unit for quantity control. The storage unit is restricted by an ICD at the downstream end and is an open bottom unit designed to also promote infiltration. Roof runoff is controlled via roof drains discharging through the internal building plumbing to rainwater harvesting tanks. Two rainwater harvesting tanks are proposed for each building. Each rainwater tank is capable of storing up to 91m³ of runoff (approximately 32mm of rainfall) beyond which it will overflow into the storm sewer and be conveyed to the storage unit. The underground storage unit is sized assuming that the rainwater harvesting tanks are available at the start of the rainfall event.

Additional infiltration will be achieved on-site through the implementation of a bioswale along the east side of the site. The granular subbase of the swale is sized to store runoff from its tributary area. An overflow drain is also provided to convey excess water to the underground storage unit.

The site discharge will be conveyed to the approved outlet location for the adjacent CMHC lands to the west of the subject site. The outlet will be sized to convey flows from both sites. Utilizing this location addresses concerns regarding an additional outlet to Poole Creek and prevents disturbance of the natural area to the north of the site.

5.3.3 Modeling Rationale

A comprehensive hydrologic modeling exercise was completed with PCSWMM, accounting for the estimated major and minor systems to evaluate the storm sewer infrastructure. The use of PCSWMM for modeling of the site hydrology and hydraulics allowed for an analysis of the systems response during various storm events. Surface storage estimates were based on the final grading plan design (see **Drawing GP-1**). The following assumptions were applied to the detailed model:

- Hydrologic parameters as per Ottawa Sewer Design Guidelines, including Horton infiltration, Manning's 'n', and depression storage values
- 12-hour SCS Storm distribution for the 100-year analysis to model 'worst-case' scenario in regards to on-site HGLs.
- 12hr SCS distributions (2 and 100-year events) with free flowing boundary condition to model 'worst-case' scenario in regards to site discharge rates to meet target rate.
- To 'stress test' the system a 'climate change' scenario was created by adding 20% of the individual intensity values of the 100-year SCS storm event at their specified time step.
- All LID measures were designed outside of PCSWMM (as documented in the report and calculations included in **Appendix E**) in order to allow routing of LID overflows to the next downstream LID which cannot be done in PCSWMM where an LID is defined as part of a given subcatchment. Total design storage and calculated infiltration losses were then input into PCSWMM as storage nodes with separate outlets for infiltration losses.
- Percent imperviousness calculated based on actual soft and hard surfaces on each subcatchment, converted to equivalent Runoff Coefficient using the relationship C = (Imp. x 0.7) + 0.2
- Subcatchment areas are defined from high-point to high-point where sags occur. Subcatchment width (average length of overland sheet flow) determined by dividing

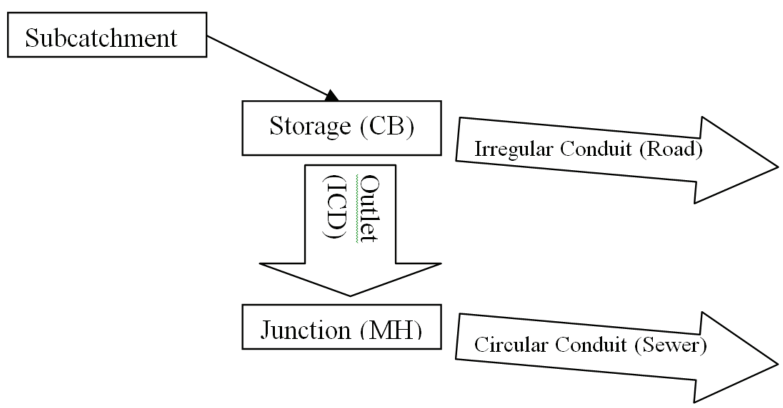
subcatchment area by subcatchment length (length of overland flow path measured from high-point to high-point).

- Number of catchbasins based on servicing plan (**Drawing SP-1**)
- Catchbasin inflow restricted with inlet-control devices (ICDs) as necessary to maintain inflow target rate and maximize use of surface storage where possible.
- Surface ponding in sag storage calculated based on grading plans (**Drawing GP-1**).

5.3.3.1 SWMM Dual Drainage Methodology

The proposed site is modeled in one modeling program as a dual conduit system (see **Figure 3**), with: 1) circular conduits representing the sewers & junction nodes representing manholes; 2) irregular conduits using street-shaped cross-sections to represent the sawtoothed overland road network from high-point to low-point and storage nodes representing catchbasins. The dual drainage systems are connected via outlet link objects (or orifices) from storage node (i.e. CB) to junction (i.e. MH), and represent inlet control devices (ICDs). Subcatchments are linked to the storage node on the surface so that generated hydrographs are directed there firstly.

Figure 3: Schematic Representing Model Object Roles



Storage nodes are used in the model to represent catchbasins as well as major system junctions. For storage nodes representing catchbasins (CBs), the invert of the storage node represents the invert of the CB and the rim of the storage node is the top of the CB plus the maximum above ground storage depth. An additional 0.3m has been added to rim elevations to allow routing from one surface storage to the next, and is unused where no spillage occurs between ponding areas. Ponding at low points is represented via storage area-depth curves for each individual storage node to match ponding volumes demonstrated on the grading plan **Drawing GP-1**. Storage volumes exceeding the sag storage available in the node will route through the connected irregular conduit to the next storage node and continue routing through the system until, ultimately, flows either re-enter the minor system or reach the outfall of the major system.

Inlet control devices, as represented by orifice links, use a user-specified discharge coefficient to approximate manufacturer's specifications for the chosen ICD model.

Subcatchment imperviousness was calculated via impervious area measured from **Drawing SSP-1**.

5.3.3.2 Boundary Conditions

The detailed PCSWMM hydrology and the proposed storm sewers were used to assess the peak inflows and hydraulic grade line (HGL) for the site. The elevation of the outlet sewer at MH100 immediately upstream of Poole Creek has been set conservatively to be above the 100-Year water elevation of the Creek per MVCA Flood Risk Mapping at an invert elevation of 99.7m to enable free-flowing model condition for the site outlet.

5.3.4 Input Parameters

Drawing SD-1 summarizes the discretized subcatchments used in the analysis of the proposed site, and outlines the major overland flow paths. The grading plans are also enclosed for review.

Appendices A1 to A3 summarize the modeling input parameters and results for the subject area; an example input and output file are provided for the 100-year 12hr SCS storm. For all other input files and results of storm scenarios, please examine the electronic model files located on the CD provided with this report. This analysis was performed using PCSWMM, which is a front-end GUI to the EPA-SWMM engine. Model files can be examined in any program which can read EPA-SWMM files version 5.1.010.

5.3.4.1 Hydrologic Parameters

Table 4 presents the general subcatchment parameters used:

Table 4: General Subcatchment Parameters

Parameter	Value
Infiltration Method	Curve Number
Drying Time (days)	7
Curve Number	80
N Impervious	0.013
N Pervious	0.2
Dstore Imperv. (mm)	1.57
Dstore perv. (mm)	4.67
Zero Imperv. (%)	0

Table 5 presents the individual parameters that vary for each of the proposed subcatchments.

Table 5: Subcatchment Parameters

Name	Outlet	Area (ha)	Width (m)	Slope (%)	Imperv. (%)
EXT1	EXT1-OF	0.07	15.1	33.3	0
EXT2	EXT2-OF	0.06	14.4	2	72.857
ST104A	ST104A-S	0.15	69	2	84.286
ST107A	ST107A-S	0.37	225.0	1.5	64.286
ST108A	ST108A-S	0.40	90.9	1.5	100
ST108B	ST108B-S	0.36	82.0	1.5	100
ST108C	ST108C-S	0.05	12.1	1.5	100
ST108D	ST108D-S	0.05	10.9	1.5	100
ST108E	ST108E-S	0.03	25.0	1.5	100
ST108F	108	0.38	86.0	1.2	44.286
ST109A	109	0.01	18.2	10	100
ST109C	ST109C-S	0.06	25.8	1	100
ST109B	ST109B-S	0.05	24.8	1	100
ST110A	110	0.07	16.8	0.8	7.143
ST110B	110	0.03	24.5	10	100
ST110C	110	0.03	26.6	10	100
ST110D	110	0.07	16.7	0.8	7.143
ST111A	ST111A-S	0.24	107.5	0.8	72.857
ST111B	ST111B-S	0.04	88.0	0.8	100
ST111C	ST111C-S	0.04	36.8	1.5	85.714
ST507A	ST507A-S	0.05	33.5	1.5	72.857
ST508A	508	0.34	189.2	1	7.143

Table 6 summarizes the storage node parameters used in the model. Storage curves for each node have been created based on volumes presented for each individual ponding area within **Drawing GP-1**. Rim elevations for each node correspond to the rim elevation of the associated area's catchbasin plus maximum depth of storage plus 0.30m to allow for demonstration of overland flow in the climate change event scenario. The 0.30m buffer is unused during other modeled events.

Storage volumes and release rates for the rainwater harvesting tank, bioswale/rain garden, and infiltration basin were obtained through iterations between design sizing calculations (final sizing attached in **Appendix E**) and PCSWMM hydrologic/hydraulic modeling.

Table 6: Storage Node Parameters

Name	Invert El. (m)	Rim Elev. (m)	Depth (m)	Coefficient	Exponent	Constant (m^2)	Curve Name	Storage Curve
108	99.27	104.37	5.10	0	0	0	RWHtank	TABULAR
508	101.06	102.85	1.79	0	0	0	ST508A-S	TABULAR
ST104A-S	101.52	103.62	2.1	1000	0	0	ST104A-S	TABULAR
ST107A-S	101.13	103.23	2.1	1000	0	0	ST107A-S	TABULAR
ST108A-S	118.6	118.75	0.15	1000	0	0	ST108A	TABULAR
ST108B-S	115.75	115.9	0.15	1000	0	0	ST108B	TABULAR
ST108C-S	110.4	110.55	0.15	1000	0	0	ST108C	TABULAR
ST108D-S	110.1	110.25	0.15	1000	0	0	ST108D	TABULAR
ST108E-S	107.2	107.35	0.15	1000	0	0	ST108E	TABULAR
ST109B-S	102.81	104.31	1.5	0	0	0	*	FUNCTIONAL
ST109C-S	102.81	104.31	1.5	0	0	0	*	FUNCTIONAL
ST111A-S	101.86	104.26	2.4	1000	0	0	ST111A-S	TABULAR
ST111C-S	101.95	104.05	2.1	0	0	0	*	FUNCTIONAL
ST507A-S	101.57	103.67	2.1	1000	0	0	ST504A-S	TABULAR
TANK	100.10	103.37	3.27	1000	0	0	TANK	TABULAR

5.3.4.2 Hydraulic Parameters

As per the Ottawa Sewer Design Guidelines (OSDG 2012), Manning's roughness values of 0.013 were used for sewer modeling and overland flow corridors representing roadways.

Storm sewers were modeled to confirm flow capacities and hydraulic grade lines (HGLs) in the proposed condition. The detailed storm sewer design sheet is included in **Appendix C**.

Table 7 below presents the parameters for the orifice and outlet link objects in the model, which represent ICDs and restricted roof release drains respectively. CB leads modeled as orifices were assigned a discharge coefficient of 0.65. The roof release discharge curves assume the use of standard Zurn model Z-105-5 controlled release roof drains as noted in the calculation sheet in **Appendix C**. The number of roof notches for each building area is to be confirmed with the building mechanical engineer. Details for the IPEX ICDs and Zurn drains are included as part of **Appendix G**.

Table 7: Outlet/Orifice Parameters

Name	Inlet	Outlet	Inlet Elev.	Type	Diameter
OR1	TANK	102	100.10	CIRCULAR	0.11
OR2	TANK	102	100.70	CIRCULAR	0.15
OR3	TANK	102	101.00	CIRCULAR	0.15
ST104A-O	ST104A-S	104	101.52	IPEX HF	0.140
ST107A-O	ST107A-S	107	101.13	CIRCULAR	0.2
ST109B-O	ST109B-S	109	102.81	CIRCULAR	0.2
ST109C-O	ST109C-S	109	102.81	CIRCULAR	0.2
ST111A-O	ST111A-S	111	101.86	IPEX HF	0.076
ST111C-O	ST111C-S	111	101.95	CIRCULAR	0.2
ST111C-O1	ST111C-S	111	101.95	CIRCULAR	0.2
OL-1	TANK	P_OF1	100.10	0.66L/s	-
OL-2	508	P_OF2	101.06	0.3L/s	-
ST507A-O	ST507A-S	TANK	101.57	IPEX LMF 95	-
ST108A-O	ST108A-S	108	118.60	ROOF	-
ST108B-O	ST108B-S	108	115.75	ROOF	-
ST108C-O	ST108C-S	108	110.40	ROOF	-
ST108D-O	ST108D-S	108	110.10	ROOF	-
ST108E-O	ST108E-S	108	107.20	ROOF	-

5.3.5 Model Results

The following section summarizes the key hydrologic and hydraulic model results. For detailed model results or inputs please refer to the example input file in **Appendix C.2 and C.3** and the electronic model files on the enclosed CD.

5.3.5.1 Hydrologic Results

The following tables demonstrate the peak outflow from each modeled outfall during the design storm (12hr SCS 2-100yr) events. A free-flowing outfall condition has been modeled for these events to be conservative with respect to site peak release rates. Outfalls EXT1-OF and EXT2-OF denote uncontrolled flows from the perimeter of the site that, due to grading restrictions, are captured by the existing right-of-way/Poole Creek at the south and north boundaries of the site.

Flows from area EXT2 will have a minimal contribution to the infrastructure within Hazeldean Road. Peaks from these uncontrolled flows are non-coincident with peaks from the subsurface storage tank/weir, and as such, flows from the outlet headwall are the only values considered in meeting the release rate target. The required subsurface storage tank volume was determined through iteration of each event, and sized to mirror the site release rate target.

Table 8: Site Peak Discharge Rates

Event	Location	Discharge Rate (L/s)	Target (L/s)
2-Year 12 Hour SCS	Outlet Headwall	15.2	17.7
5-Year 12 Hour SCS	Outlet Headwall	38.6	43.9
10-Year 12 Hour SCS	Outlet Headwall	64.5	66.2
25-Year 12 Hour SCS	Outlet Headwall	98.7	103.7
50-Year 12 Hour SCS	Outlet Headwall	116.2	136.5
100-Year 12 Hour SCS	Outlet Headwall	136.3	176.3
100-Year 12 Hour SCS +20%	Outlet Headwall	317.0	-

5.3.5.2 Hydraulic Results

Table 9 summarizes the HGL results within the site for the 100 year storm events and the 'climate change' scenario storm required by the City of Ottawa Sewer Design Guidelines (2012), where intensities are increased by 20%. The City of Ottawa requires that during major storm events, the maximum hydraulic grade line be kept at least 0.30 m below the underside-of-footing (USF) of any adjacent units connected to the storm sewer during design storm events. As the proposed building perimeter drain and ramp drains will be disconnected from the storm sewer and pumped to the surface, USFs are considered at 0.3m below the lowest finished floor elevation of the building.

Table 9: Modeled Hydraulic Grade Line Results

STM MH	Proposed Ground Elev. (m)	100-year 12hr SCS		100-year 12hr SCS + 20%	
		HGL (m)	USF-HGL Clearance (m)	HGL (m)	USF-HGL Clearance (m)
103	104.20	101.97	2.23	102.78	1.42
104	104.20	101.98	2.22	102.80	1.40
105	104.20	101.99	2.21	102.86	1.34
106	104.20	101.99	2.21	102.87	1.33
107	104.20	102.00	2.20	102.89	1.31

STM MH	Proposed Ground Elev. (m)	100-year 12hr SCS		100-year 12hr SCS + 20%	
		HGL (m)	USF-HGL Clearance (m)	HGL (m)	USF-HGL Clearance (m)
108	104.20	102.04	2.16	103.04	1.16

As is demonstrated in the table above, the worst-case scenario results in HGL elevations remain at least 0.30 m below the proposed surface elevations, and HGL elevations remain below the proposed surface elevations during the 20% increased intensity 'climate change' scenario.

Table 10 presents the maximum total surface water depths (static ponding depth + dynamic flow) above the top-of-grate of catchbasins for the 100-year design storm and climate change storm. Based on the model results, the total ponding depth (static + dynamic) does not exceed the required 0.30m maximum during the 100-year event. Total ponding depths during the climate change scenario are below adjacent building openings and should not impact the proposed building.

Table 10: Maximum Surface Water Depths

Storage node ID	Structure ID	Rim Elevation (m)	100 year, 12hr SCS		100 year, 12hr SCS +20%	
			Max HGL (m)	Total Surface Water Depth (m)	Max HGL (m)	Total Surface Water Depth (m)
ST104A-S	CB 506	103.32	103.23	0.00	103.47	0.15
ST107A-S	CB 505	102.93	102.85	0.00	103.09	0.16
ST111A-S	CB 501	103.96	104.18	0.22	104.22	0.26
ST111C-S	CB 504	103.75	102.03	0.00	102.91	0.00
ST507A-S	CB 507	103.37	103.44	0.07	103.47	0.10
508	CB T 508	102.60	102.01	0.00	102.83	0.23

5.3.6 Water Quality Control

On-site water quality control is required to provide 80% TSS removal prior to discharging to Poole Creek. A Stormceptor unit STC300 is proposed upstream of the underground storage/infiltration basin. The Stormceptor will provide greater than 80% TSS removal in the 25mm event and will act as pre-treatment for the storage/ infiltration basin thereby reducing maintenance requirements of the facility and improving long-term performance. The Stormceptor unit will be privately maintained. The location and general arrangement of the Stormceptor unit is indicated on **Drawing SD-1**. Detailed sizing calculations for the Stormceptor unit are included in **Appendix C.5**

5.3.7 Infiltration Targets

The MVCA requires that BMP measures be implemented on-site to meet the minimum infiltration target rate of 50 mm/yr (as identified in the Kanata West Master Servicing Study, Stantec, 2006). For a site area of 2.9ha with an average imperviousness of 56% the total annual infiltration requirement is therefore 812m^3/yr. The KWMSS also requires a 25% augmentation to site infiltration requirements to account for off-site road areas for which no infiltration measures were required. Therefore, the total site infiltration target is 1,015 m^3/yr. Past correspondence with the MVCA indicated that the target infiltration rates were in fact "target hydrograph volume reduction rates".

The LID bioswale and infiltration gallery proposed for the site will provide significant opportunity for stormwater infiltration. Infiltration calculations completed for the design and sizing of these LID measures were used to approximate an expected annual infiltration rate. Water balance calculations for a continuous rainfall scenario from August 2, 2009 to March 1, 2012 (see **Appendix E**) were used to determine an average daily infiltration rate over a one year period. The average rate was estimated to be 44m^3/day. Note that this rate is averaged over 365 days per year and would underestimate summer months and overestimate winter. Nevertheless, the average annual infiltration that could be provided through the LID measures would be approximately 16,262 m^3/yr. Therefore, only about 10% of the total possible infiltration is required to meet the infiltration target for the site.

The infiltration contribution from the bioswale and infiltration gallery is included in **Table 11** below. Note that this summary does not include infiltration resulting from the rainwater harvesting reuse for irrigation. The results in **Table 11** suggest that the infiltration target could be met with the bioswale infiltration only.

Table 11: Summary of Infiltration from LID Features

LID Feature	Estimated Total Annual Infiltration (m3/yr)
Bioswale	2,568
Infiltration Gallery	13,694
Total Infiltration	**16,262**

5.3.7.1 Potential Groundwater Mounding

Groundwater levels at the site were measured by Paterson Group during two separate site visits and are summarized in the attached Paterson memos in **Appendix F**. Based on the results of the groundwater monitoring Paterson Group prepared a memo discussing the variation in groundwater level measurements and anticipated seasonally high and normal groundwater levels. The results for the boreholes near the LID features are summarized in **Table 12** below. The complete memo dated January 25, 2017 is included in **Appendix F**.

Table 12: Expected Seasonal Variation of Groundwater Levels

Borehole Number	Ground Elevation (m)	Long-term Groundwater Levels		Seasonally High Groundwater Level	
		Depth (m)	Elevation (m)	Depth (m)	Elevation (m)
BH 1	102.93	3.7	99.23	3.2	99.73
BH 2	103.02	3.7	99.32	3.2	99.82
BH 3	103.07	3.7	99.37	3.2	99.87
BH 4	103.15	3.7	99.45	3.2	99.95
BH 5	103.22	3.7	99.52	3.2	100.02
BH 6	103.25	3.7	99.55	3.2	100.05
BH 7	102.91	4.5	98.41	4.0	98.91

Since the clearance from the bottom of the infiltration tank to the groundwater table is less than 1.0m the potential for groundwater mounding was considered. Groundwater mounding calculations were completed for both the seasonally high groundwater condition and the normal groundwater condition. However, per the Paterson memo, the seasonally high level is expected to occur during March-April, as such historical rainfall data was used to establish the average rainfall event volume for March-April. The analysis indicated approximately a 10mm event. The duration of infiltration for the infiltration gallery was obtained from the PCSWMM hydraulic model based on the modeled time for the infiltration gallery to empty. No PCSWMM model was run for the 10mm event so the 2-year event was used as a conservative estimate. These durations were input into the groundwater mounding calculation spreadsheet in **Appendix E**. It is noted that the calculations are based on the Hantush (1967) equation for groundwater mounding and use the hydraulic conductivity (measured by Paterson and summarized in the attached memo from September 2016) as the recharge rate and typical specific yield for silty clay. It is also noted that spreadsheet inputs and results are in imperial units. **Table 13** below summarizes the results of the groundwater mounding calculations.

Table 13: Estimated Maximum Groundwater Mounding below Infiltration Gallery

Groundwater Conditon	Mounding Height (m)	Mounding Elevation (m)	Distance to Bottom of Infiltration Gallery (m)
Long-term (99.23)	0.31	99.54	0.56
Seasonally High (99.73)	0.26	99.99	0.11

It is noted the above mounding depths are still below the bottom of the infiltration gallery. Should a larger rainfall even occur during the seasonally high groundwater condition there could be potential for the groundwater mound to extend into the infiltration gallery. However, there is a storm sewer outlet proposed at the bottom of the infiltration gallery (Invert =100.10m per attached **Drawing SSP-1**) which will limit the maximum groundwater height to the bottom of the infiltration gallery. Once the mounding reaches the bottom, the stored stormwater would discharge only through the controlled outlets and would not infiltrate. Since the groundwater mounding is caused only by infiltrating stormwater and not by external sources, there should be no loss of storage volume due to groundwater mounding.

5.3.8 Thermal Controls

The MVCA and MOECC confirmed that Poole Creek is designated as a "cool-water fish habitat". As the proposed development will increase the amount of impervious area on the site and roof top detention will increase water temperatures, thermal mitigation measures are required for the site.

As the majority of heat transfer from paved surfaces occurs during the first flush (considered as the initial 10mm of each design event), storage of the 10mm event has been given priority. With exception of the rooftop areas, the site is designed with minimal surface storage. All runoff will be captured and detained in the underground storage unit which will allow for heat dumping into the surrounding ground and granular material. Similarly, runoff conveyed through the granular subbase of the bioswale will experience cooling. Roof discharge will be the most thermally impacted water as it will be retained on the rooftops for several hours. This water will be discharged to the underground rainwater harvesting tank and will inlet at the bottom of the tank such that if the tank is full, the cooler water will be discharged first through the overflow. With 167m^3 of storage available in each of the rainwater harvesting tanks, the only occurrence where roof discharge would not experience any temperature mitigation via mixing or detention would be when total rainfall exceeds approximately a 2-year event. The reverse temperature mitigation effect (warming water during cold weather) would also occur with these measures as ground temperatures would warm the runoff.

5.3.9 Monitoring Plan

In addition to monitoring requirements to be identified by the MOECC in the Environmental Compliance Approval (ECA), the site will require regular monitoring of the LID measures installed on the site. A detailed monitoring program will be developed through consultation with the City of Ottawa and MVCA. In general, the monitoring plan will required pre-construction, during construction and post-construction monitoring and include the following:

- Installation of water level loggers in both the rainwater harvesting tank, infiltration gallery, and bioswale (monitoring "well" to be installed) to assess frequency of overflow and drawdown rates and compare with design values;
- Installation of temperature logger in the outlet manhole from the site to monitor temperature of the storm discharge. The temperature logger cannot be installed at the outlet to Poole Creek as this outlet will include discharge from the CMHC lands as well and would not be representative of the subject site;
- Collection of water quality samples upstream and downstream of the proposed OGS unit;
- Visual inspection of all LID systems at least once per month and following all large rainfall events. Including observations for:
  - Debris accumulation on the surface
  - Measurement of/inspection for sediment accumulation in rainwater harvesting tank and infiltration gallery
  - Presence of ponded water on the surface of the bioswale beyond design duration
  - Outlet/inlet blockages of tanks and OGS

A detailed monitoring plan is included in **Appendix H**.

5.3.10 Contingency Plan

It is recognized that the proposed stormwater management plan is considered a "pilot project" by the City of Ottawa and has allowed for credit from the LID measures toward the stormwater management design. As such the monitoring plan for the site will be critical in assessing the performance of the system. Should either the pre-construction monitoring result in findings that will impact the function of the system, then additional assessment of the design will be required to assess system performance and determine whether additional storage is required. Additional storage would be provided by expanding the size of the proposed infiltration gallery. This assessment would be required prior to constructing the facility. A memo will be issued to the City of Ottawa outlining the monitoring results and confirming whether there is any need for expansion of the infiltration system.

Similarly, post-construction monitoring will assess the performance of the system. Data analysis and reporting will be completed and review whether any retrofits to the system are required. The greatest benefit to the SWM design is the storage available in the rainwater harvesting system. It is estimated that the greatest impact to the system storage requirements would be if this system does not operate as designed and this entire volume cannot be relied upon for the SWM system. This would result in the need for an additional 335m^3 to be added to the infiltration gallery. While this extreme is assumed to be unlikely, it is recommended that the site MOECC ECA include this contingency volume of 335m^3 to allow for the expansion if needed without requiring an amendment to the ECA before proceeding.

Post-construction monitoring will include groundwater level monitoring and water level monitoring within the infiltration gallery. Results will be monitored to ensure no storage volume is lost as a result of groundwater influences and storage volume would be adjusted as necessary. However, it is anticipated that since the infiltration gallery design includes and outlet at the bottom of the storage area, there should be no significant loss of volume caused by seasonal groundwater fluctuations or mounding.

5.4 SUMMARY OF FINDINGS

Based on the preceding, the following conclusions can be drawn:

- The proposed stormwater management plan is in compliance with the criteria established for the site and the 2012 City of Ottawa Sewer Guidelines.
- Inlet control devices are proposed to limit inflow from the site area into the minor system to maximize the use of surface storage.
- Subsurface storage has been provided to further limit site outflows to the peak site discharge rate determined via PCSWMM model (See **Table 14** below).
- The storm sewer hydraulic grade line is maintained at least 0.30 m below finished ground elevations during design storm events.
- All dynamic surface water depths are less than 0.30 m during all design storm events.
- Quality control is provided by a Stormceptor model STC3000 upstream of the underground storage facility to maintain water quality objectives outlined in the background reports.

Table 14: Site Peak Discharge Rates/Targets

Storm Event	Site Peak Discharge Rate (L/s)	Target Discharge Rate (L/s)
2-Year 12 Hour SCS	15.2	17.7
5-Year 12 Hour SCS	38.6	43.9
10-Year 12 Hour SCS	64.5	66.2
25-Year 12 Hour SCS	98.7	103.7
50-Year 12 Hour SCS	116.2	136.5
100-Year 12 Hour SCS	136.3	176.3
100-Year 12 Hour SCS +20%	317.0	-

Appendix C STORMWATER MANAGEMENT

C.1 STORM SEWER DESIGN SHEET AND ROOF STORAGE CALCULATIONS

MAP No.
CARTE No.
2

FLOOD RISK MAP
POOLE CREEK
CARTE DU RISQUE D'INONDATION

Date Modified / Date de modification: 29/01/2015

LEGEND / LÉGENDE

- Regulatory Floodplain / La Crue Régulatrice
- Regulatory Limit / Limite Réglementaire
- Contours / Courbes
- Stream / Ruisseau
- Cross Sections / La coupe traversale

Cross Section Number — 1 — Nombre de la coupe traversale
Regulatory Flood Elevation (m) — 102.11 — Niveau de la crue regulatrice (m)

INDEX CONTOUR INTERVAL 2 METRES
WITH 0.5 METRE INTERMEDIATE CONTOUR
NORTH AMERICAN DATUM 1983

COURBES DE NIVEAU PRINCIPALES DE 2.0 MÈTRE
AVEC COURBES DE NIVEAU INTERMÉDIAIRES DE 0.5 MÈTRES
SYSTÈME DE RÉFÉRENCE GÉODÉSIQUE NORD-AMÉRIQUE 1983

GENERAL INFORMATION

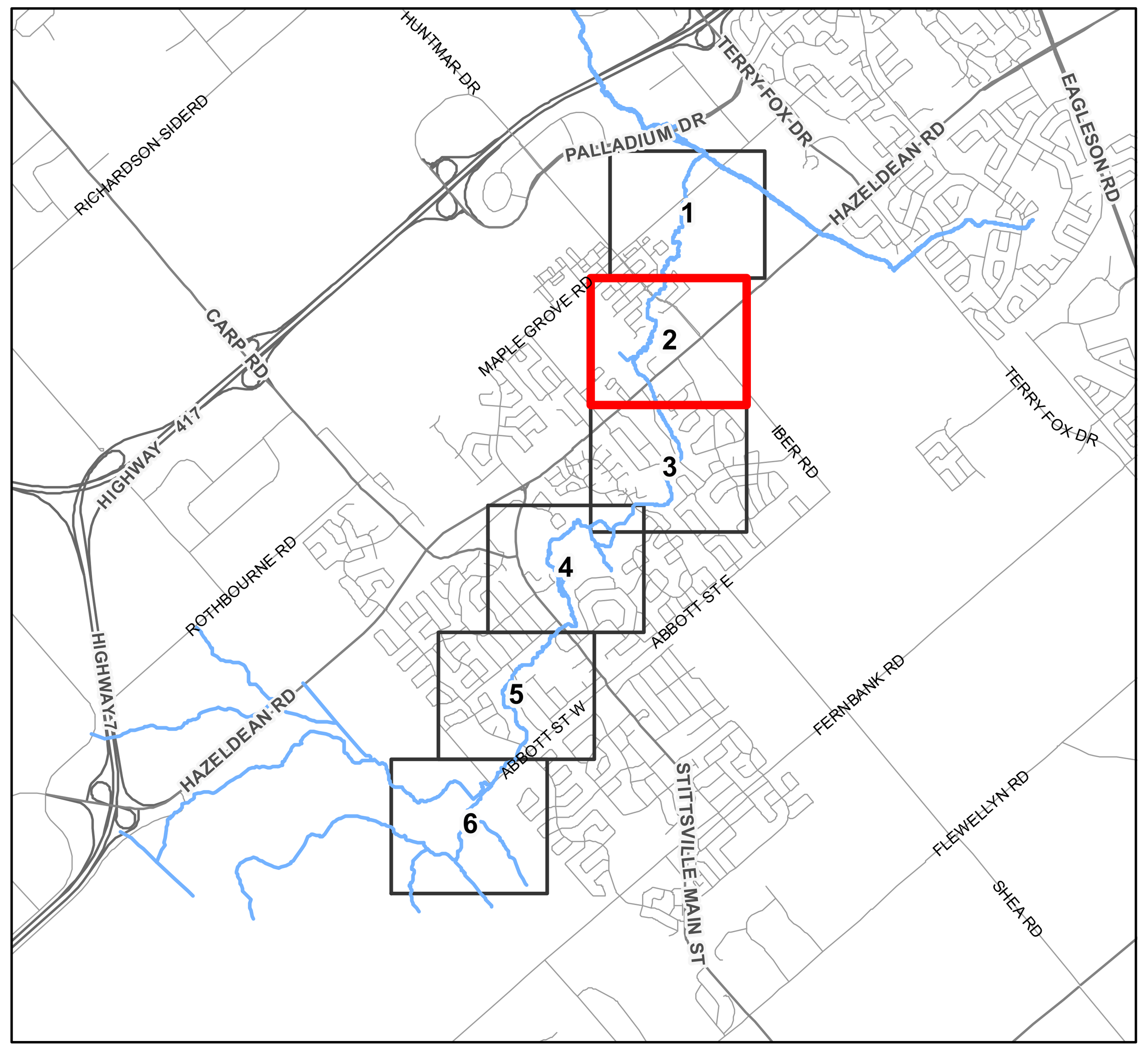
Vertical Datum: Mean sea level
Horizontal Datum: North American 1983
Map Projection: Ottawa Transverse Mercator Projection

RENSEIGNMENTS GÉNÉRAUX

Niveau de référence vertical: Niveau moyen de la mer
Niveau de référence horizontal: Nord-americain 1983
Projection cartographique: Projection Mercator Transverse d'Ottawa

Mississippi Valley Conservation Authority
Ottawa

SHEET INDEX / TABLEAU D'ASSEMBLAGE



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SCALE 1:2000 ÉCHELLE
0 25 50 100 150 200
Meters / Mètres

This map and the associated information displayed are to be used for general illustrative purposes only. Although best efforts have been made to create accuracy; due to the complex and extensive nature of the data, all representations and/or information provided herein are approximate and to be verified by user. User hereby acknowledges that this map is not intended for true and accurate navigational purposes and hereby accepts and assumes all inherent risks associated with the use of this map.

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Revision #	Issue
1 - Nov. 14 2013	Public review
2 - Dec. 4, 2013	Board approval
3 - Jan. 21,2015	Final

LICENSED PROFESSIONAL ENGINEER
J. S. A. PRICE
Jan. 20/15
PROVINCE OF ONTARIO

	5731 Hazeldean Road	STORM SEWER DESIGN SHEET (City of Ottawa)	DESIGN PARAMETERS
Stantec	DATE: 23-Mar-2017		$I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)
	REVISION: 3		
	DESIGNED BY: DT	FILE NUMBER: 1604-01195	
	CHECKED BY: AL		

	1:5 yr	1:100 yr				
a =	998.071	1735.688	MANNING'S n =	0.013	BEDDING CLASS =	B
b =	6.053	6.014	MINIMUM COVER:	2.00	m	
c =	0.814	0.820	TIME OF ENTRY	10	min	

LOCATION			DRAINAGE AREA																PIPE SELECTION											
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (5-YEAR) (ha)	AREA (100-YEAR) (ha)	AREA (ROOF) (ha)	C (-)	ACCUM. AREA (5YR) (ha)	A x C (5-YEAR) (ha)	ACCUM. AxC (5YR) (ha)	ACCUM. AREA (100YR) (ha)	A x C (100-YEAR) (ha)	ACCUM. AxC (100YR) (ha)	T of C (min)	I_{5-YEAR} (mm/h)	$I_{10-YEAR}$ (mm/h)	$Q_{CONTROL}$ (NOTE 1) (L/s)	ACCUM. $Q_{CONTROL}$ (L/s)	Q_{ACT} (CIA/360) (L/s)	LENGTH (m)	PIPE WIDTH OR DIAMETER (mm)	PIPE HEIGHT (mm)	PIPE SHAPE (-)	MATERIAL (-)	CLASS (-)	SLOPE %	Q_{CAP} (FULL) (L/s)	% FULL (-)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)
	TANKOUT	102	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	13.91	87.26	149.29	44.9	44.9	44.9	2.8	450	450	CIRCULAR	CONCRETE	-	0.20	133.0	**33.76%**	0.81	0.62	0.08
													13.98																	
	103	102	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	3.0	450	450	CIRCULAR	CONCRETE	-	1.00	298.1	**0.00%**	1.82	0.00	0.00
													10.00																	
	102	101	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	13.98	86.99	148.82	0.0	44.9	44.9	70.7	450	450	CIRCULAR	CONCRETE	-	0.20	133.0	**33.76%**	0.81	0.62	1.92
	101	100	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	15.90	80.76	138.07	0.0	44.9	44.9	101.6	450	450	CIRCULAR	CONCRETE	-	0.20	133.0	**33.76%**	0.81	0.62	2.75
	100	HEADWALL	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	18.65	73.36	125.30	62.4	107.3	107.3	11.1	675	675	CIRCULAR	CONCRETE	-	1.40	1037.6	**10.34%**	2.81	1.50	0.12
													18.77							675	675									
ST109A-C	109	104	0.13	0.00	0.00	0.90	0.13	0.114	0.114	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	32.9	38.2	375	375	CIRCULAR	PVC	-	1.00	164.8	**19.94%**	1.56	1.01	0.63
													10.63																	
ST110A-D	110	106	0.21	0.00	0.00	0.44	0.21	0.093	0.093	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	26.8	12.5	375	375	CIRCULAR	PVC	-	1.00	164.8	**16.24%**	1.56	0.95	0.22
													10.22																	
	500	501	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	26.1	200	200	CIRCULAR	PVC	-	1.00	33.3	**0.00%**	1.05	0.00	0.00
	501	111	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	4.0	200	200	CIRCULAR	PVC	-	1.00	33.3	**0.00%**	1.05	0.00	0.00
													10.00																	
	502	111	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	2.8	200	200	CIRCULAR	PVC	-	1.00	33.3	**0.00%**	1.05	0.00	0.00
													10.00																	
ST111A-C	111	107	0.32	0.00	0.00	0.68	0.32	0.221	0.221	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	64.0	110.4	375	375	CIRCULAR	PVC	-	0.70	137.9	**46.41%**	1.31	1.10	1.68
ST107A	107	106	0.37	0.00	0.00	0.65	0.70	0.243	0.464	0.00	0.000	0.000	11.68	96.10	164.56	0.0	0.0	123.8	63.3	450	450	CIRCULAR	CONCRETE	-	0.50	210.3	**58.87%**	1.28	1.15	0.91
													12.59																	
	106	105	0.00	0.00	0.00	0.00	0.91	0.000	0.556	0.00	0.000	0.000	12.59	92.25	157.90	0.0	0.0	142.6	17.6	525	525	CIRCULAR	CONCRETE	-	0.20	200.6	**71.05%**	0.90	0.85	0.34
													12.93																	
ST108C-F	108A	108	0.38	0.00	0.13	0.51	0.38	0.195	0.195	0.00	0.000	0.000	10.00	104.19	178.56	8.0	8.0	64.4	85.0	375	375	CIRCULAR	PVC	-	0.50	116.6	**55.25%**	1.11	0.97	1.46
ST108A, B	108	105	0.00	0.00	0.77	0.25	0.38	0.000	0.195	0.00	0.000	0.000	11.46	97.07	166.24	43.7	51.7	104.2	36.3	450	450	CIRCULAR	CONCRETE	-	0.30	162.9	**63.98%**	0.99	0.92	0.66
													12.12																	
	105	104	0.00	0.00	0.00	0.00	1.29	0.000	0.751	0.00	0.000	0.000	12.93	90.89	155.55	0.0	51.7	241.3	39.1	600	600	CIRCULAR	CONCRETE	-	0.20	286.5	**84.25%**	0.98	0.98	0.66
													13.60																	
ST104A	104	103	0.15	0.00	0.00	0.79	1.57	0.118	0.983	0.00	0.000	0.000	13.60	88.38	151.22	0.0	51.7	293.1	16.3	675	675	CIRCULAR	CONCRETE	-	0.20	392.2	**74.73%**	1.06	1.03	0.26
	103	TANKIN2	0.00	0.00	0.00	0.00	1.57	0.000	0.983	0.00	0.000	0.000	13.86	87.43	149.57	0.0	51.7	290.5	2.8	675	675	CIRCULAR	CONCRETE	-	0.20	392.1	**74.08%**	1.06	1.02	0.05
													13.91							675	675									
ST507A	507	TANKIN3	0.05	0.00	0.00	0.90	0.05	0.049	0.049	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	14.2	0.9	200	200	CIRCULAR	PVC	-	1.00	33.3	**42.56%**	1.05	0.85	0.02
													10.02							200	200									
	511	510	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	24.0	250	250	CIRCULAR	CONCRETE	-	0.25	30.2	**0.00%**	0.61	0.00	0.00
	510	509	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	41.2	250	250	CIRCULAR	CONCRETE	-	0.25	30.0	**0.00%**	0.61	0.00	0.00
	509	508	0.00	0.00	0.00	0.00	0.00	0.000	0.000	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	0.0	40.4	250	250	CIRCULAR	CONCRETE	-	0.25	30.2	**0.00%**	0.61	0.00	0.00
ST508A	508	TANKIN1	0.34	0.00	0.00	0.25	0.34	0.084	0.084	0.00	0.000	0.000	10.00	104.19	178.56	0.0	0.0	24.3	8.7	250	250	CIRCULAR	CONCRETE	-	0.25	30.4	**79.77%**	0.61	0.60	0.24
													10.24							250	250									

Stormwater Management Calculations

File No: **160401195**
Project: **5731 Hazeldean**
Date: **30-Jan-17**

SWM Approach:
Post-development to Pre-development flows

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

Runoff Coefficient Table								
Catchment Type	Sub-catchment Area ID / Description		Area (ha) "A"		Runoff Coefficient "C"		"A x C"	Overall Runoff Coefficient
Roof	ST108D	Hard	0.05		0.9	0.047		
		Soft	0.00		0.2	0.000		
		Subtotal		0.052			0.047	0.900
Roof	ST108E	Hard	0.03		0.9	0.024		
		Soft	0.00		0.2	0.000		
		Subtotal		0.027			0.024	0.900
Roof	ST108B	Hard	0.36		0.9	0.328		
		Soft	0.00		0.2	0.000		
		Subtotal		0.364			0.328	0.900
Roof	ST108A	Hard	0.40		0.9	0.363		
		Soft	0.00		0.2	0.000		
		Subtotal		0.404			0.363	0.900
Roof	ST108C	Hard	0.05		0.9	0.042		
		Soft	0.00		0.2	0.000		
		Subtotal		0.047			0.042	0.900
Total				**0.894**			**0.805**	
Overall Runoff Coefficient= C:								**0.90**

Total Roof Areas	**0.894 ha**
Total Tributary Surface Areas (Controlled and Uncontrolled)	**0.000 ha**
Total Tributary Area to Outlet	**0.894 ha**
Total Uncontrolled Areas (Non-Tributary)	**0.000 ha**
Total Site	**0.894 ha**

Stormwater Management Calculations

Project #160401195, 5731 Hazeldean
Modified Rational Method Calculatons for Storage

5 yr Intensity
City of Ottawa

I = a/(t + b)^c

a =	998.071
b =	6.053
c =	0.814

t (min)	I (mm/hr)
5	141.18
10	104.19
15	83.56
20	70.25
25	60.90
30	53.93
35	48.52
40	44.18
45	40.63
50	37.65
55	35.12
60	32.94

5 YEAR Modified Rational Method for Entire Site

Subdrainage Area: ST108D — Roof
Area (ha): 0.05 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
5	141.18	18.26	2.75	15.51	4.65	89.6	0.00
10	104.19	13.48	3.09	10.39	6.24	100.5	0.00
15	83.56	10.81	3.17	7.64	6.88	103.2	0.00
20	70.25	9.09	3.20	5.89	**7.07**	104.0	0.00
25	60.90	7.88	3.19	4.69	7.03	103.9	0.00
30	53.93	6.98	3.17	3.81	6.86	103.1	0.00
35	48.52	6.28	3.13	3.14	6.60	102.0	0.00
40	44.18	5.72	3.09	2.62	6.29	100.7	0.00
45	40.63	5.26	3.04	2.22	5.98	99.0	0.00
50	37.65	4.87	2.98	1.90	5.69	96.9	0.00
55	35.12	4.54	2.91	1.63	5.39	94.8	0.00
60	32.94	4.26	2.85	1.41	5.09	92.7	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
5-year Water Level	104.03	0.10	3.20	7.07	20.68	0.00

Subdrainage Area: ST108E — Roof
Area (ha): 0.03 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
5	141.18	9.61	1.38	8.23	2.47	89.9	0.00
10	104.19	7.09	1.55	5.54	3.32	100.8	0.00
15	83.56	5.68	1.59	4.09	3.68	103.7	0.00
20	70.25	4.78	1.61	3.17	**3.81**	104.7	0.00
25	60.90	4.14	1.61	2.53	3.80	104.7	0.00
30	53.93	3.67	1.60	2.07	3.73	104.1	0.00
35	48.52	3.30	1.58	1.72	3.61	103.1	0.00
40	44.18	3.01	1.57	1.44	3.46	101.9	0.00
45	40.63	2.76	1.54	1.22	3.29	100.6	0.00
50	37.65	2.56	1.52	1.04	3.13	98.8	0.00
55	35.12	2.39	1.49	0.90	2.98	96.8	0.00
60	32.94	2.24	1.46	0.79	2.83	94.7	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
5-year Water Level	104.74	0.10	1.61	3.81	10.88	0.00

Subdrainage Area: ST108B — Roof
Area (ha): 0.36 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
5	141.18	128.71	17.98	110.73	33.22	90.0	0.00
10	104.19	94.99	20.17	74.81	44.89	101.0	0.00
15	83.56	76.18	20.78	55.40	49.86	104.1	0.00
20	70.25	64.04	21.00	43.05	51.66	105.1	0.00
25	60.90	55.52	21.01	34.51	**51.76**	105.2	0.00
30	53.93	49.16	20.90	28.26	50.87	104.7	0.00
35	48.52	44.23	20.72	23.51	49.38	103.8	0.00
40	44.18	40.28	20.49	19.79	47.50	102.6	0.00
45	40.63	37.04	20.23	16.81	45.38	101.3	0.00
50	37.65	34.33	19.95	14.37	43.12	99.9	0.00
55	35.12	32.02	19.56	12.46	41.13	97.9	0.00
60	32.94	30.03	19.16	10.87	39.14	96.0	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
5-year Water Level	105.21	0.11	21.01	51.76	145.75	0.00

Subdrainage Area: ST108A — Roof
Area (ha): 0.40 — Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
5	141.18	142.64	19.39	123.25	36.97	90.2	0.00
10	104.19	105.27	21.77	83.50	50.10	101.2	0.00
15	83.56	84.42	22.44	61.98	55.78	104.3	0.00
20	70.25	70.98	22.69	48.28	57.94	105.5	0.00
25	60.90	61.53	22.72	38.80	**58.20**	105.7	0.00
30	53.93	54.48	22.62	31.86	57.35	105.2	0.00
35	48.52	49.02	22.44	26.58	55.81	104.4	0.00
40	44.18	44.64	22.21	22.43	53.84	103.3	0.00
45	40.63	41.05	21.94	19.11	51.58	102.0	0.00

Project #160401195, 5731 Hazeldean
Modified Rational Method Calculatons for Storage

100 yr Intensity
City of Ottawa

I = a/(t + b)^c

a =	1735.688
b =	6.014
c =	0.820

t (min)	I (mm/hr)
5	242.70
10	178.56
15	142.89
20	119.95
25	103.85
30	91.87
35	82.58
40	75.15
45	69.05
50	63.95
55	59.62
60	55.89

100 YEAR Modified Rational Method for Entire Site

Subdrainage Area: ST108D — Roof
Area (ha): 0.05 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	25.67	3.93	21.73	13.04	128.1	0.00
20	119.95	17.24	4.17	13.07	15.69	135.7	0.00
30	91.87	13.21	4.21	8.99	**16.19**	137.1	0.00
40	75.15	10.80	4.18	6.62	15.88	136.2	0.00
50	63.95	9.19	4.13	5.07	15.20	134.3	0.00
60	55.89	8.03	4.05	3.98	14.35	131.8	0.00
70	49.79	7.16	3.97	3.19	13.40	129.1	0.00
80	44.99	6.47	3.88	2.59	12.42	126.3	0.00
90	41.11	5.91	3.78	2.13	11.50	123.0	0.00
100	37.90	5.45	3.67	1.78	10.67	119.5	0.00
110	35.20	5.06	3.56	1.50	9.87	116.0	0.00
120	32.89	4.73	3.46	1.26	9.11	112.7	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	137.11	0.14	4.21	16.19	20.68	0.00

Subdrainage Area: ST108E — Roof
Area (ha): 0.03 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	13.50	1.97	11.53	6.92	128.4	0.00
20	119.95	9.07	2.09	6.97	8.37	136.3	0.00
30	91.87	6.94	2.12	4.82	**8.68**	138.0	0.00
40	75.15	5.68	2.11	3.57	8.57	137.4	0.00
50	63.95	4.83	2.08	2.75	8.25	135.7	0.00
60	55.89	4.23	2.05	2.18	7.83	133.4	0.00
70	49.79	3.76	2.01	1.75	7.37	130.9	0.00
80	44.99	3.40	1.97	1.43	6.88	128.2	0.00
90	41.11	3.11	1.93	1.18	6.38	125.4	0.00
100	37.90	2.87	1.88	0.99	5.94	122.1	0.00
110	35.20	2.66	1.82	0.84	5.53	118.8	0.00
120	32.89	2.49	1.77	0.71	5.13	115.5	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	138.04	0.14	2.12	8.68	10.88	0.00

Subdrainage Area: ST108B — Roof
Area (ha): 0.36 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	180.87	25.67	155.20	93.12	128.6	0.00
20	119.95	121.50	27.29	94.21	113.05	136.7	0.00
30	91.87	93.06	27.67	65.39	**117.70**	138.6	0.00
40	75.15	76.12	27.57	48.54	116.51	138.1	0.00
50	63.95	64.78	27.26	37.53	112.58	136.5	0.00
60	55.89	56.62	26.82	29.80	107.26	134.3	0.00
70	49.79	50.43	26.33	24.10	101.23	131.9	0.00
80	44.99	45.57	25.81	19.76	94.85	129.3	0.00
90	41.11	41.64	25.28	16.36	88.34	126.6	0.00
100	37.90	38.39	24.70	13.70	82.18	123.7	0.00
110	35.20	35.66	24.03	11.62	76.72	120.4	0.00
120	32.89	33.32	23.40	9.92	71.45	117.2	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	138.58	0.14	27.67	117.70	145.75	0.00

Subdrainage Area: ST108A — Roof
Area (ha): 0.40 — Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	200.45	27.68	172.76	103.66	128.7	0.00
20	119.95	134.66	29.47	105.19	126.23	137.0	0.00
30	91.87	103.13	29.91	73.22	**131.80**	139.1	0.00
40	75.15	84.36	29.83	54.52	130.86	138.7	0.00
50	63.95	71.79	29.52	42.28	126.84	137.3	0.00
60	55.89	62.75	29.07	33.67	121.23	135.2	0.00
70	49.79	55.89	28.56	27.33	114.78	132.8	0.00
80	44.99	50.51	28.02	22.48	107.93	130.3	0.00
90	41.11	46.15	27.47	18.68	100.90	127.7	0.00

Stormwater Management Calculations

Project #160401195, 5731 Hazeldean
Modified Rational Method Calculatons for Storage

50	37.65	38.04	21.66	16.39	49.16	100.7	0.00
55	35.12	35.49	21.30	14.19	46.81	99.1	0.00
60	32.94	33.28	20.88	12.40	44.65	97.1	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
5-year Water Level	105.67	0.11	22.72	58.20	161.52	0.00

Subdrainage Area: ST108C Roof
Area (ha): 0.05 Maximum Storage Depth: 150 mm
C: 0.90

tc (min)	I (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
5	141.18	16.63	2.74	13.90	4.17	89.1	0.00
10	104.19	12.27	3.06	9.21	5.53	99.6	0.00
15	83.56	9.84	3.14	6.71	6.03	102.1	0.00
20	70.25	8.28	3.15	5.12	**6.15**	102.7	0.00
25	60.90	7.17	3.14	4.03	6.05	102.2	0.00
30	53.93	6.35	3.11	3.24	5.84	101.2	0.00
35	48.52	5.72	3.07	2.65	5.56	99.9	0.00
40	44.18	5.21	3.00	2.20	5.29	97.7	0.00
45	40.63	4.79	2.93	1.85	5.00	95.5	0.00
50	37.65	4.44	2.87	1.57	4.71	93.3	0.00
55	35.12	4.14	2.80	1.34	4.42	91.0	0.00
60	32.94	3.88	2.73	1.15	4.14	88.9	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
5-year Water Level	102.66	0.10	3.15	6.15	18.83	0.00

Project #160401195, 5731 Hazeldean
Modified Rational Method Calculatons for Storage

100	37.90	42.55	26.91	15.64	93.84	125.1	0.00
110	35.20	39.52	26.21	13.31	87.81	121.9	0.00
120	32.89	36.93	25.53	11.39	82.04	118.7	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	139.08	0.14	29.91	131.80	161.52	0.00

Subdrainage Area: ST108C Roof
Area (ha): 0.05 Maximum Storage Depth: 150 mm
C: 1.00

tc (min)	I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
10	178.56	23.37	3.92	19.46	11.67	127.4	0.00
20	119.95	15.70	4.13	11.57	13.89	134.4	0.00
30	91.87	12.02	4.16	7.87	**14.16**	135.3	0.00
40	75.15	9.84	4.11	5.72	13.73	133.9	0.00
50	63.95	8.37	4.04	4.33	12.99	131.6	0.00
60	55.89	7.32	3.96	3.36	12.10	128.8	0.00
70	49.79	6.52	3.86	2.65	11.14	125.8	0.00
80	44.99	5.89	3.75	2.14	10.27	122.0	0.00
90	41.11	5.38	3.63	1.75	9.45	118.2	0.00
100	37.90	4.96	3.52	1.44	8.66	114.5	0.00
110	35.20	4.61	3.41	1.20	7.91	111.0	0.00
120	32.89	4.31	3.31	1.00	7.20	107.6	0.00

Storage: Roof Storage

	Depth (mm)	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Discharge Check
100-year Water Level	135.29	0.14	4.16	14.16	18.83	0.00

Project #160401195, 5731 Hazeldean
Roof Drain Design Sheet, Area ST108A
Standard Zurn Model Z-105-5 Control-Flo Single Notch Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0004	0.0054	1	0.025	90	1	1	0.025
0.050	0.0008	0.0108	6	0.050	359	5	6	0.050
0.075	0.0012	0.0161	20	0.075	808	14	20	0.075
0.100	0.0015	0.0215	48	0.100	1436	28	48	0.100
0.125	0.0019	0.0269	93	0.125	2243	46	93	0.125
0.150	0.0023	0.0323	162	0.150	3230	68	162	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
5.2	486.8	5.2	0.13523
19.4	881.0	14.2	0.37995
47.1	1286.7	27.7	0.73735
92.7	1697.0	45.6	1.20874
160.8	2109.7	68.0	1.79476

Rooftop Storage Summary

Total Building Area (sq.m)		4038
Assume Available Roof Area (sq.	80%	3230
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		14
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		162
Estimated 100 Year Drawdown Time (h)		1.5

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Zurn Drain Catalogue

Head (m)	L/min	L/s	Notch Rating
0.051	45.5	0.00076	232

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.023	0.030	-
Depth (m)	0.106	0.139	0.150
Volume (cu.m)	58.2	131.8	161.5
Draintime (hrs)	0.9	1.5	

Project #160401195, 5731 Hazeldean
Roof Drain Design Sheet, Area ST108B
Standard Zurn Model Z-105-5 Control-Flo Single Notch Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0004	0.0050	1	0.025	81	1	1	0.025
0.050	0.0008	0.0100	5	0.050	324	5	5	0.050
0.075	0.0012	0.0150	18	0.075	729	13	18	0.075
0.100	0.0015	0.0200	43	0.100	1296	25	43	0.100
0.125	0.0019	0.0250	84	0.125	2024	41	84	0.125
0.150	0.0023	0.0300	146	0.150	2915	61	146	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
4.7	473.1	4.7	0.13141
17.5	856.1	12.8	0.36921
42.5	1250.3	25.0	0.71652
83.7	1649.1	41.2	1.17459
145.1	2050.0	61.4	1.74404

Rooftop Storage Summary

Total Building Area (sq.m)		3644
Assume Available Roof Area (sq.	80%	2915
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		13
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		146
Estimated 100 Year Drawdown Time (h)		1.5

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Zurn Drain Catalogue

Head (m)	L/min	L/s	Notch Rating
0.051	45.5	0.00076	232

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.021	0.028	-
Depth (m)	0.105	0.139	0.150
Volume (cu.m)	51.8	117.7	145.7
Draintime (hrs)	0.8	1.5	

Project #160401195, 5731 Hazeldean
Roof Drain Design Sheet, Area ST108C
Standard Zurn Model Z-105-5 Control-Flo Single Notch Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0004	0.0008	0	0.025	10	0	0	0.025
0.050	0.0008	0.0015	1	0.050	42	1	1	0.050
0.075	0.0012	0.0023	2	0.075	94	2	2	0.075
0.100	0.0015	0.0031	6	0.100	167	3	6	0.100
0.125	0.0019	0.0038	11	0.125	262	5	11	0.125
0.150	0.0023	0.0046	19	0.150	377	8	19	0.150

Drawdown Estimate

Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.6	397.4	0.6	0.11038
2.3	719.0	1.7	0.3101
5.5	1050.1	3.2	0.60181
10.8	1385.1	5.3	0.98655
18.7	1721.9	7.9	1.46485

Rooftop Storage Summary

Total Building Area (sq.m)		471
Assume Available Roof Area (sq.	80%	377
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		19
Estimated 100 Year Drawdown Time (h)		1.2

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Zurn Drain Catalogue

Head (m)	L/min	L/s	Notch Rating
0.051	45.5	0.00076	232

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.003	0.004	-
Depth (m)	0.103	0.135	0.150
Volume (cu.m)	6.1	14.2	18.8
Draintime (hrs)	0.6	1.2	

Project #160401195, 5731 Hazeldean
Roof Drain Design Sheet, Area ST108D
Standard Zurn Model Z-105-5 Control-Flo Single Notch Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0004	0.0008	0	0.025	11	0	0	0.025
0.050	0.0008	0.0015	1	0.050	46	1	1	0.050
0.075	0.0012	0.0023	3	0.075	103	2	3	0.075
0.100	0.0015	0.0031	6	0.100	184	4	6	0.100
0.125	0.0019	0.0038	12	0.125	287	6	12	0.125
0.150	0.0023	0.0046	21	0.150	414	9	21	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.7	436.4	0.7	0.12121
2.5	789.6	1.8	0.34055
6.0	1153.3	3.5	0.6609
11.9	1521.1	5.8	1.08342
20.6	1890.9	8.7	1.60868

Rooftop Storage Summary

Total Building Area (sq.m)		517
Assume Available Roof Area (sq.	80%	414
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		2
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		21
Estimated 100 Year Drawdown Time (h)		1.3

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Zurn Drain Catalogue

Head (m)	L/min	L/s	Notch Rating
0.051	45.5	0.00076	232

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.003	0.004	-
Depth (m)	0.104	0.137	0.150
Volume (cu.m)	7.1	16.2	20.7
Draintime (hrs)	0.7	1.3	

Project #160401195, 5731 Hazeldean
Roof Drain Design Sheet, Area ST108E
Standard Zurn Model Z-105-5 Control-Flo Single Notch Roof Drain

Rating Curve				Volume Estimation				
Elevation (m)	Discharge Rate (cu.m/s)	Outlet Discharge (cu.m/s)	Storage (cu. m)	Elevation (m)	Area (sq. m)	Volume (cu. m) Increment	Volume (cu. m) Accumulated	Water Depth (m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0004	0.0004	0	0.025	6	0	0	0.025
0.050	0.0008	0.0008	0	0.050	24	0	0	0.050
0.075	0.0012	0.0012	1	0.075	54	1	1	0.075
0.100	0.0015	0.0015	3	0.100	97	2	3	0.100
0.125	0.0019	0.0019	6	0.125	151	3	6	0.125
0.150	0.0023	0.0023	11	0.150	218	5	11	0.150

Drawdown Estimate			
Total Volume (cu.m)	Total Time (sec)	Vol (cu.m)	Detention Time (hr)
0.0	0.0	0.0	0
0.4	459.0	0.4	0.1275
1.3	830.5	1.0	0.3582
3.2	1213.0	1.9	0.69516
6.2	1599.9	3.1	1.13957
10.8	1988.9	4.6	1.69206

Rooftop Storage Summary

Total Building Area (sq.m)		272
Assume Available Roof Area (sq.	80%	218
Roof Imperviousness		0.99
Roof Drain Requirement (sq.m/Notch)		232
Number of Roof Notches*		1
Max. Allowable Depth of Roof Ponding (m)		0.15
Max. Allowable Storage (cu.m)		11
Estimated 100 Year Drawdown Time (h)		1.4

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

* Note: Number of drains can be reduced if multiple-notch drain used.

From Zurn Drain Catalogue

Head (m)	L/min	L/s	Notch Rating
0.051	45.5	0.00076	232

Calculation Results

	5yr	100yr	Available
Qresult (cu.m/s)	0.002	0.002	-
Depth (m)	0.105	0.138	0.150
Volume (cu.m)	3.8	8.7	10.9
Draintime (hrs)	0.8	1.4	

Outlet Rip-Rap Sizing

US Army Corps of Enginners
1991 Procedure
EM1601
Common values

V	1.62	m/s	
y	0.37125	m	
Z	3.00	H:1V	
phi	42	degrees	
r	300	m	
W	1	m	
Ss	2.5	rock specific gravity	
g	9.806	m/s^2	
theta	18.4	degrees	bank angle with horizontal

SF	1.1
Cs	0.3
Kl	1
Cv	0.79
Ct	1

D_{50} =	0.048	m
M_{50} =	0.147	kg

Selected D50	0.060	m
Min. thickness	0.120	m

C.2 SAMPLE PCSWMM MODEL INPUT (12HR 100YR SCS)

```
[TITLE]


[OPTIONS]
;;Options            Value
;;------------------ ------------
FLOW_UNITS           LPS
INFILTRATION         CURVE_NUMBER
FLOW_ROUTING         DYNWAVE
START_DATE           07/23/2009
START_TIME           00:00:00
REPORT_START_DATE    07/23/2009
REPORT_START_TIME    00:00:00
END_DATE             07/24/2009
END_TIME             00:00:00
SWEEP_START          01/01
SWEEP_END            12/31
DRY_DAYS             0
REPORT_STEP          00:05:00
WET_STEP             00:05:00
DRY_STEP             00:05:00
ROUTING_STEP         1
ALLOW_PONDING        YES
INERTIAL_DAMPING     PARTIAL
VARIABLE_STEP        0
LENGTHENING_STEP     0
MIN_SURFAREA         0
NORMAL_FLOW_LIMITED  BOTH
SKIP_STEADY_STATE    NO
FORCE_MAIN_EQUATION  H-W
LINK_OFFSETS         ELEVATION
MIN_SLOPE            0
MAX_TRIALS           8
HEAD_TOLERANCE       0.0015
SYS_FLOW_TOL         5
LAT_FLOW_TOL         5
MINIMUM_STEP         0.5
THREADS              4

[EVAPORATION]
;;Type          Parameters
;;------------- ----------
CONSTANT     0.0
DRY_ONLY     NO

[RAINGAGES]
;;              Rain      Time   Snow   Data
;;Name          Type      Intrvl Catch  Source
```



```
;;-------------- --------- ------ ------ ----------
RG1              INTENSITY 0:15   1.0    TIMESERIES 100SCS

[SUBCATCHMENTS]
;;                                                Total    Pcnt.             Pcnt.    Curb     Snow
;;Name           Raingage         Outlet           Area     Imperv   Width    Slope    Length   Pack
;;-------------- ---------------- ---------------- -------- -------- -------- -------- -------- --------
EXT1             RG1              EXT1-OF          0.067219 0        15.124   33.3     0

EXT2             RG1              EXT2-OF          0.063791 72.857   14.353   2        0

ST104A           RG1              ST104A-S         0.149935 84.286   69       2        0

ST107A           RG1              ST107A-S         0.373344 64.286   225      1.5      0

ST108A           RG1              ST108A-S         0.403809 100      90.857   1.5      0

ST108B           RG1              ST108B-S         0.36437  100      81.983   1.5      0

ST108C           RG1              ST108C-S         0.047083 100      12.1     1.5      0

ST108D           RG1              ST108D-S         0.051706 100      10.9     1.5      0

ST108E           RG1              ST108E-S         0.027193 100      25       1.5      0

ST108F           RG1              108              0.382042 44.286   85.96    1.2      0

ST109A           RG1              109              0.013537 100      18.2     10       0

ST109B           RG1              ST109B-S         0.054305 100      24.8     1        0

ST109C           RG1              ST109C-S         0.058618 100      25.8     1        0

ST110A           RG1              110              0.074661 7.143    16.799   0.8      0

ST110B           RG1              110              0.031906 100      24.5     10       0

ST110C           RG1              110              0.029561 100      26.6     10       0

ST110D           RG1              110              0.074098 7.143    16.672   0.8      0

ST111A           RG1              ST111A-S         0.242699 72.857   107.5    0.8      0

ST111B           RG1              111              0.037296 100      88       0.8      0

ST111C           RG1              ST111C-S         0.043507 85.714   36.8     1.5      0

ST507A           RG1              ST507A-S         0.054432 72.857   33.5     1.5      0
```

```
ST508A           RG1              508              0.3356   7.143    189.2    1        0


[SUBAREAS]
;;Subcatchment   N-Imperv   N-Perv     S-Imperv   S-Perv     PctZero    RouteTo    PctRouted
;;-------------- ---------- ---------- ---------- ---------- ---------- ---------- ----------
EXT1             0.013      0.2        1.57       4.67       0          PERVIOUS   100
EXT2             0.013      0.2        1.57       4.67       0          PERVIOUS   100
ST104A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST107A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108B           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108C           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108D           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108E           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST108F           0.013      0.2        1.57       4.67       0          PERVIOUS   100
ST109A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST109B           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST109C           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST110A           0.013      0.2        1.57       4.67       0          PERVIOUS   100
ST110B           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST110C           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST110D           0.013      0.2        1.57       4.67       0          PERVIOUS   100
ST111A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST111B           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST111C           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST507A           0.013      0.2        1.57       4.67       0          IMPERVIOUS 100
ST508A           0.013      0.2        1.57       4.67       0          PERVIOUS   100

[INFILTRATION]
;;Subcatchment   CurveNum   HydCon     DryTime
;;-------------- ---------- ---------- ----------
EXT1             80         0          7
EXT2             80         0          7
ST104A           80         0          7
ST107A           80         0          7
ST108A           80         0          7
ST108B           80         0          7
ST108C           80         0          7
ST108D           80         0          7
ST108E           80         0          7
ST108F           80         0          7
ST109A           80         0          7
ST109B           80         0          7
ST109C           80         0          7
ST110A           80         0          7
ST110B           80         0          7
```



```
ST110C           80         0          7
ST110D           80         0          7
ST111A           80         0          7
ST111B           80         0.5        7
ST111C           80         0          7
ST507A           80         0          7
ST508A           80         0          7

[JUNCTIONS]
;;               Invert     Max.       Init.      Surcharge  Ponded
;;Name           Elev.      Depth      Depth      Depth      Area
;;-------------- ---------- ---------- ---------- ---------- ----------
100              99.4       2.735      0          0          0
101              99.60313   3.691      0          0          0
102              99.7793    3.752      0          0          0
103              100.035    3.526      0          0          0
104              100.0785   3.492      0          0          0
105              100.2317   3.406      0          0          0
106              100.3418   2.976      0          0          0
107              100.8033   2.417      0          0          0
109              100.83     3.67       0          0          0
110              100.686    3.814      0          0          0
111              101.4225   2.65       0          0          0

[OUTFALLS]
;;               Invert     Outfall      Stage/Table      Tide
;;Name           Elev.      Type         Time Series      Gate Route To
;;-------------- ---------- ------------ ---------------- ---- ----------------
EXT1-OF          102.88     FREE                          NO
EXT2-OF          104.2      FREE                          NO
HEADWALL         98.7       FREE                          NO
POOLE_OF1        100.1      FREE                          NO
POOLE_OF2        101.059    FREE                          NO
ST104A-OF        0          FREE                          NO
ST107A-OF        0          FREE                          NO

[STORAGE]
;;               Invert   Max.     Init.    Storage    Curve                        Ponded   Evap.
;;Name           Elev.    Depth    Depth    Curve      Params                       Area     Frac.
Infiltration parameters
;;-------------- -------- -------- -------- ---------- -------- -------- -------- -------- --------
-----------------------
108              97.239   7.127    0        TABULAR    RWHtank                      0        0
508              101.06   1.79     0        TABULAR    ST508A-S                     0        0
ST104A-S         101.52   2.1      0        TABULAR    ST104A-S                     0.01     0
ST107A-S         101.13   2.1      0        TABULAR    ST107A-S                     0.01     0
ST108A-S         118.6    0.15     0        TABULAR    ST108A                       0.01     0
ST108B-S         115.75   0.15     0        TABULAR    ST108B                       0.01     0
```

```
ST108C-S         110.4    0.15     0        TABULAR    ST108C                         0.01     0
ST108D-S         110.1    0.15     0        TABULAR    ST108D                         0.01     0
ST108E-S         107.2    0.15     0        TABULAR    ST108E                         0.01     0
ST109B-S         102.81   1.5      0        FUNCTIONAL 0          0         0         0.01     0
ST109C-S         102.81   1.5      0        FUNCTIONAL 0          0         0         0.01     0
ST111A-S         101.86   2.4      0        TABULAR    ST111A-S                       0.01     0
ST111C-S         101.95   2.1      0        FUNCTIONAL 0          0         0         0        0
ST507A-S         101.57   2.1      0        TABULAR    ST504A-S                       0.01     0
TANK             100.1    3.27     0        TABULAR    TANK                           0.01     0

[CONDUITS]
;;               Inlet            Outlet                      Manning    Inlet      Outlet     Init.
Max.
;;Name           Node             Node             Length     N          Offset     Offset     Flow
Flow
;;-------------- ---------------- ---------------- ---------- ---------- ---------- ---------- ----------
----------
Pipe_13          100              HEADWALL         11.135     0.013      99.548     99.52      0          0

Pipe_14          106              105              17.55995   0.013      100.411    100.376    0          0

Pipe_14_(1)      105              104              39.10268   0.013      100.301    100.222    0          0

Pipe_15          109              104              38.24086   0.013      100.83     100.447    0          0

Pipe_16          110              106              12.50838   0.013      100.686    100.561    0          0

Pipe_17          111              107              110.3626   0.013      101.65     100.877    0          0

Pipe_21          104              103              16.284     0.013      100.143    100.11     0          0

Pipe_23          508              TANK             8.730414   0.013      101.6588   101.637    0          0

Pipe_26          101              100              101.5684   0.013      99.936     99.733     0          0

Pipe_27          108              105              36.34      0.013      100.441    100.332    0          0

Pipe_29          107              106              63.29649   0.013      100.802    100.486    0          0

Pipe_31          102              101              70.701     0.013      100.083    99.942     0          0

Pipe_34          103              TANK             2.81       0.013      100.106    100.1      0          0

ST104A-T         ST104A-S         ST104A-OF        2.5        0.025      103.47     102.88     0          0

ST107A-T         ST107A-S         ST107A-OF        2.5        0.025      103.08     103.04     0          0

ST111A-T         ST111A-S         ST111C-S         40.9       0.013      104.26     103.75     0          0
```



```
ST111B-T         ST111C-S         ST107A-S         60         0.013      103.75     103.08     0          0

ST507A-T         ST507A-S         ST104A-S         14.9       0.013      103.5      103.47     0          0

W1               103              102              3          0.013      102        101.97     0          0

[ORIFICES]
;;               Inlet            Outlet           Orifice      Crest      Disch.     Flap Open/Close
;;Name           Node             Node             Type         Height     Coeff.     Gate Time
;;-------------- ---------------- ---------------- ------------ ---------- ---------- ---- ----------
OR1              TANK             102              SIDE         100.1      0.65       NO   0
OR2              TANK             102              SIDE         100.7      0.65       NO   0
OR3              TANK             102              SIDE         101        0.65       NO   0
ST104A-O         ST104A-S         104              SIDE         101.52     0.572      NO   0
ST107A-O         ST107A-S         107              SIDE         101.13     0.65       NO   0
ST109B-O         ST109B-S         109              SIDE         102.81     0.65       NO   0
ST109C-O         ST109C-S         109              SIDE         102.81     0.65       NO   0
ST111A-O         ST111A-S         111              SIDE         101.86     0.572      NO   0
ST111C-O         ST111C-S         111              SIDE         101.95     0.65       NO   0
ST111C-O1        ST111C-S         111              SIDE         101.95     0.65       NO   0

[OUTLETS]
;;               Inlet            Outlet           Outflow    Outlet           Qcoeff/
Flap
;;Name           Node             Node             Height     Type             QTable           Qexpon
Gate
;;-------------- ---------------- ---------------- ---------- ---------------- ---------------- ----------
----
OL1              TANK             POOLE_OF1        100.1      TABULAR/HEAD     TANK_BASEFLOW
NO
OL2              508              POOLE_OF2        101.06     TABULAR/HEAD     BIOSWALE_BASEFLOW
NO
ST108A-O         ST108A-S         108              118.6      TABULAR/HEAD     ST108A-O
NO
ST108B-O         ST108B-S         108              115.75     TABULAR/HEAD     ST108B-O
NO
ST108C-O         ST108C-S         108              110.4      TABULAR/HEAD     ST108C-O
NO
ST108D-O         ST108D-S         108              110.1      TABULAR/HEAD     ST108D-O
NO
ST108E-O         ST108E-S         108              107.2      TABULAR/HEAD     ST108E-O
NO
ST507A-O         ST507A-S         TANK             101.57     FUNCTIONAL/HEAD  7.996            0.499
NO

[XSECTIONS]
```

```
;;Link           Shape        Geom1            Geom2      Geom3      Geom4      Barrels
;;-------------- ------------ ---------------- ---------- ---------- ---------- ----------
Pipe_13          CIRCULAR     0.675            0          0          0          1
Pipe_14          CIRCULAR     0.525            0          0          0          1
Pipe_14_(1)      CIRCULAR     0.6              0          0          0          1
Pipe_15          CIRCULAR     0.375            0          0          0          1
Pipe_16          CIRCULAR     0.375            0          0          0          1
Pipe_17          CIRCULAR     0.375            0          0          0          1
Pipe_21          CIRCULAR     0.675            0          0          0          1
Pipe_23          CIRCULAR     0.25             0          0          0          1
Pipe_26          CIRCULAR     0.45             0          0          0          1
Pipe_27          CIRCULAR     0.45             0          0          0          1
Pipe_29          CIRCULAR     0.45             0          0          0          1
Pipe_31          CIRCULAR     0.45             0          0          0          1
Pipe_34          CIRCULAR     0.675            0          0          0          1
ST104A-T         IRREGULAR    Overland         0          0          0          1
ST107A-T         IRREGULAR    Overland         0          0          0          1
ST111A-T         IRREGULAR    Overland         0          0          0          1
ST111B-T         IRREGULAR    Overland         0          0          0          1
ST507A-T         IRREGULAR    Overland         0          0          0          1
W1               CIRCULAR     0.45             0          0          0          1
OR1              CIRCULAR     0.11             0          0          0
OR2              CIRCULAR     0.15             0          0          0
OR3              CIRCULAR     0.15             0          0          0
ST104A-O         CIRCULAR     0.14             0          0          0
ST107A-O         CIRCULAR     0.2              0          0          0
ST109B-O         CIRCULAR     0.2              0          0          0
ST109C-O         CIRCULAR     0.2              0          0          0
ST111A-O         CIRCULAR     0.076            0          0          0
ST111C-O         CIRCULAR     0.2              0          0          0
ST111C-O1        CIRCULAR     0.2              0          0          0

[TRANSECTS]

NC 0.013    0.013    0.013
X1 Overland         5        0.15     6.85     0.0       0.0       0.0      0.0      0.0
GR 0.15     0        0        0.15     0        6.85     0.15     7        0.15     7

;[LE: 0][RE: 7]
NC 0.013    0.013    0.013
X1 Overland(orig)   4        0.15     6.85     0.0       0.0       0.0      0.0      0.0
GR 0.15     0        0        0.15     0        6.85     0.15     7

[LOSSES]
;;Link           Inlet      Outlet     Average    Flap Gate  SeepageRate
;;-------------- ---------- ---------- ---------- ---------- ----------
Pipe_14          0          0.053      0          NO         0
Pipe_14_(1)      0          0.022      0          NO         0
```



```
Pipe_15          0          1.344      0          NO         0
Pipe_16          0          1.344      0          NO         0
Pipe_17          0          1.344      0          NO         0
Pipe_21          0          0.022      0          NO         0
Pipe_26          0          0.423      0          NO         0
Pipe_27          0          1.344      0          NO         0
Pipe_29          0          0.053      0          NO         0
Pipe_31          0          1.344      0          NO         0
W1               0          1.344      0          NO         0

[INFLOWS]
;;                                                 Param    Units    Scale    Baseline Baseline
;;Node           Parameter        Time Series      Type     Factor   Factor   Value    Pattern
;;-------------- ---------------- ---------------- -------- -------- -------- -------- --------
100              FLOW             ""               FLOW     1.0      1        175

[CURVES]
;;Name           Type       X-Value    Y-Value
;;-------------- ---------- ---------- ----------
BIOSWALE_BASEFLOW Rating     0.00       0
BIOSWALE_BASEFLOW            0.01       0.3
BIOSWALE_BASEFLOW            10.00      0.3

ST108A-O         Rating     0          0
ST108A-O                    0.025      5.4
ST108A-O                    0.050      10.8
ST108A-O                    0.075      16.1
ST108A-O                    0.100      21.5
ST108A-O                    0.125      26.9
ST108A-O                    0.150      32.3

ST108B-O         Rating     0          0
ST108B-O                    0.025      5.0
ST108B-O                    0.050      10.0
ST108B-O                    0.075      15.0
ST108B-O                    0.100      20.0
ST108B-O                    0.125      25.0
ST108B-O                    0.150      30.0

ST108C-O         Rating     0          0
ST108C-O                    0.025      0.8
ST108C-O                    0.050      1.5
ST108C-O                    0.075      2.3
ST108C-O                    0.100      3.1
ST108C-O                    0.125      3.8
ST108C-O                    0.150      4.6

ST108D-O         Rating     0          0
```

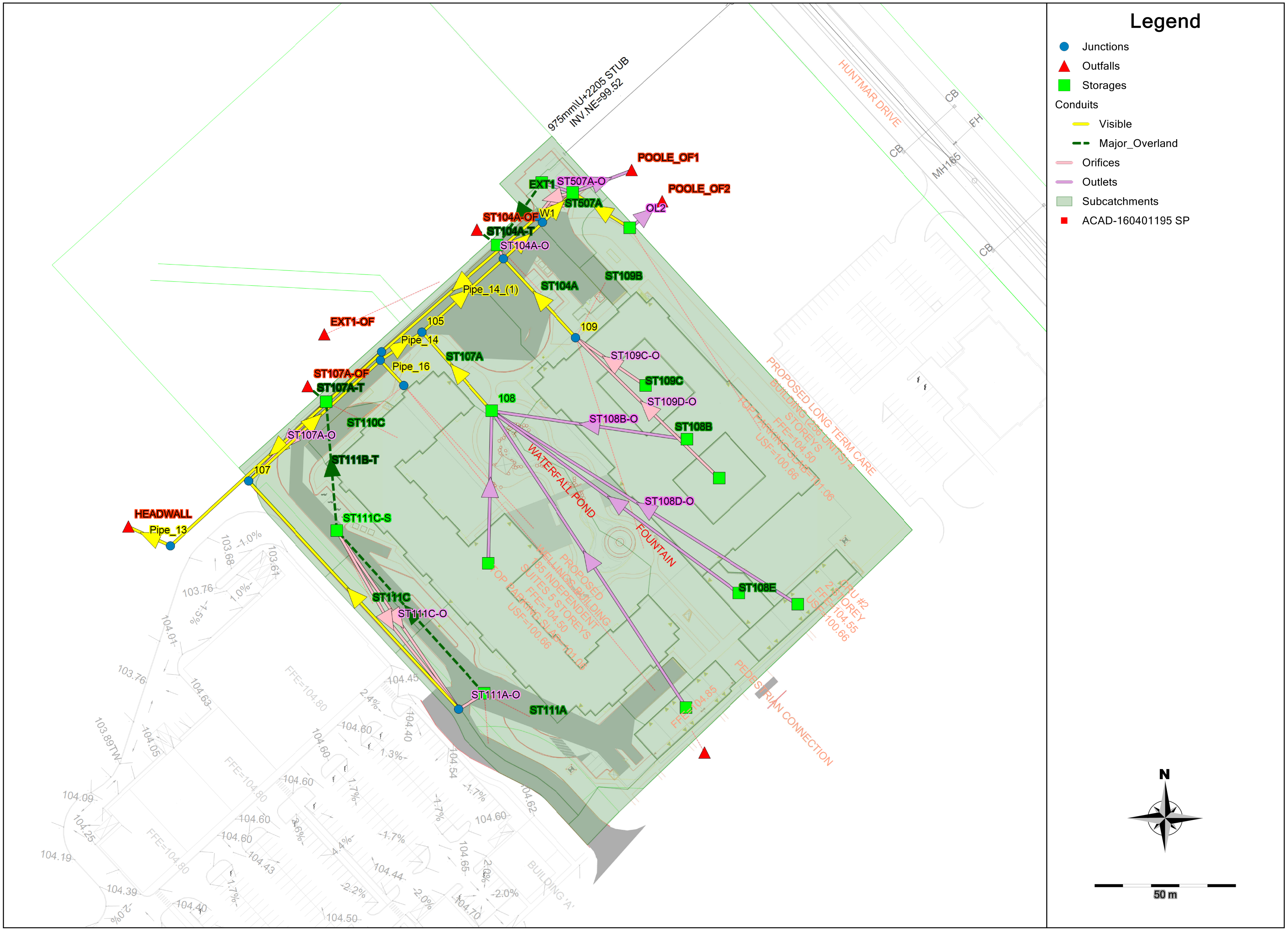
ST108D-O		0.025	0.8
ST108D-O		0.050	1.5
ST108D-O		0.075	2.3
ST108D-O		0.100	3.1
ST108D-O		0.125	3.8
ST108D-O		0.150	4.6
ST108E-O	Rating	0	0
ST108E-O		0.025	0.4
ST108E-O		0.050	0.8
ST108E-O		0.075	1.2
ST108E-O		0.100	1.5
ST108E-O		0.125	1.9
ST108E-O		0.150	2.3
TANK_BASEFLOW	Rating	0.00	0
TANK_BASEFLOW		0.01	0.66
TANK_BASEFLOW		10.00	0.66
RWHtank	Storage	0	113.747
RWHtank		3.202	113.747
RWHtank		3.203	0
RWHtank		5	0
ST104A-S	Storage	0	0
ST104A-S		1.8	0
ST104A-S		1.95	32
ST107A-S	Storage	0	0
ST107A-S		1.8	0
ST107A-S		1.95	30.67
ST108A	Storage	0	0
ST108A		0.025	90
ST108A		0.050	359
ST108A		0.075	808
ST108A		0.100	1436
ST108A		0.125	2243
ST108A		0.150	3230
ST108B	Storage	0	0
ST108B		0.025	81
ST108B		0.050	324
ST108B		0.075	729
ST108B		0.100	1296
ST108B		0.125	2024
ST108B		0.150	2915

ST108C	Storage	0	0
ST108C		0.025	10
ST108C		0.050	42
ST108C		0.075	94
ST108C		0.100	167
ST108C		0.125	262
ST108C		0.150	377
ST108D	Storage	0	0
ST108D		0.025	11
ST108D		0.050	46
ST108D		0.075	103
ST108D		0.100	184
ST108D		0.125	287
ST108D		0.150	414
ST108E	Storage	0	0
ST108E		0.025	6
ST108E		0.050	24
ST108E		0.075	54
ST108E		0.100	97
ST108E		0.125	151
ST108E		0.150	218
ST111A-S	Storage	0	0
ST111A-S		2.10	0
ST111A-S		2.40	724
ST504A-S	Storage	0	0
ST504A-S		1.8	0
ST504A-S		1.93	181.54
ST508A-S	Storage	0	0
ST508A-S		0.7	152
ST508A-S		0.701	0
ST508A-S		1.741	0
ST508A-S		1.991	114.4
TANK	Storage	0	560.7
TANK		0.026	560.7
TANK		0.051	560.7
TANK		0.077	560.7
TANK		0.102	559.44
TANK		0.127	559.44
TANK		0.153	558.18
TANK		0.178	556.92
TANK		0.204	555.66
TANK		0.229	554.4

TANK	0.254	551.88
TANK	0.28	549.36
TANK	0.305	546.84
TANK	0.331	543.06
TANK	0.356	539.28
TANK	0.381	534.24
TANK	0.407	527.94
TANK	0.432	521.64
TANK	0.458	514.08
TANK	0.483	505.26
TANK	0.508	495.18
TANK	0.534	483.84
TANK	0.559	478.8
TANK	0.585	464.94
TANK	0.61	449.82
TANK	0.635	434.7
TANK	0.661	419.58
TANK	0.686	403.2
TANK	0.712	383.04
TANK	0.737	360.36
TANK	0.762	347.76
TANK	0.796	335.16
TANK	0.813	320.04
TANK	0.839	304.92
TANK	0.864	289.8
TANK	0.889	272.16
TANK	0.915	258.3
TANK	0.94	244.44
TANK	0.965	233.1
TANK	0.991	221.76
TANK	1.016	211.68
TANK	1.041	201.6
TANK	1.067	192.78
TANK	1.092	185.22
TANK	1.118	180.18
TANK	1.143	176.4
TANK	1.168	172.62
TANK	1.194	170.1
TANK	1.219	167.58
TANK	1.245	165.06
TANK	1.27	163.8
TANK	1.295	162.54
TANK	1.321	162.54
TANK	1.346	162.54
TANK	1.372	161.28
TANK	1.397	161.28
TANK	1.422	161.28
TANK	1.448	161.28

TANK	1.473	161.28
TANK	1.499	161.28
TANK	1.524	161.28
TANK	1.549	161.28
TANK	1.575	161.28
TANK	1.6	161.28
TANK	1.626	161.28
TANK	1.651	161.28
TANK	1.676	161.28
TANK	1.702	161.28
TANK	1.727	161.28
TANK	1.753	161.28
TANK	1.778	161.28
TANK	1.803	161.28
TANK	1.829	161.28
TANK	1.83	0
TANK	5	0

C.3 SAMPLE PCSWMM MODEL OUTPUT (12HR 100YR SCS)



Legend
Junctions
Outfalls
Storages
Conduits
Visible
Major_Overland
Orifices
Outlets
Subcatchments
ACAD-160401195 SP
975mmU+2205 STUB
INV.NE=99.52
HUNTMAR DRIVE
POOLE_OF1
POOLE_OF2
ST507A-O
ST507A
EXT1
W1
OL2
ST104A-OF
ST104A-T
ST104A-O
ST104A
ST109B
Pipe_14_(1)
EXT1-OF
105
109
Pipe_14
ST107A
ST109C-O
Pipe_16
ST107A-OF
ST107A-T
ST109C
ST109D-O
108
ST108B-O
ST110C
ST108B
ST107A-O
ST111B-T
107
WATERFALL POND
ST108D-O
HEADWALL
ST111C-S
Pipe_13
FOUNTAIN
ST108E
ST111C
ST111C-O
ST111A-O
ST111A
PROPOSED LONG TERM CARE BUILDING (256 UNITS) 4 STOREYS
PEDESTRIAN CONNECTION
N
50 m

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.011)

WARNING 03: negative offset ignored for Link Pipe_29

****
Element Count
****

Number of rain gages 1
Number of subcatchments ... 22
Number of nodes 33
Number of links 37
Number of pollutants 0
Number of land uses 0

****
Raingage Summary
****

Name	Data Source	Data Type	Recording Interval
RG1	100SCS	INTENSITY	15 min.

****
Subcatchment Summary
****

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	0.07	15.12	0.00	33.3000	RG1	EXT1-OF
EXT2	0.06	14.35	72.86	2.0000	RG1	EXT2-OF
ST104A	0.15	69.00	84.29	2.0000	RG1	ST104A-S
ST107A	0.37	225.00	64.29	1.5000	RG1	ST107A-S
ST108A	0.40	90.86	100.00	1.5000	RG1	ST108A-S
ST108B	0.36	81.98	100.00	1.5000	RG1	ST108B-S
ST108C	0.05	12.10	100.00	1.5000	RG1	ST108C-S
ST108D	0.05	10.90	100.00	1.5000	RG1	ST108D-S
ST108E	0.03	25.00	100.00	1.5000	RG1	ST108E-S
ST108F	0.38	85.96	44.29	1.2000	RG1	108
ST109A	0.01	18.20	100.00	10.0000	RG1	109
ST109B	0.05	24.80	100.00	1.0000	RG1	ST109B-S
ST109C	0.06	25.80	100.00	1.0000	RG1	ST109C-S
ST110A	0.07	16.80	7.14	0.8000	RG1	110
ST110B	0.03	24.50	100.00	10.0000	RG1	110
ST110C	0.03	26.60	100.00	10.0000	RG1	110
ST110D	0.07	16.67	7.14	0.8000	RG1	110
ST111A	0.24	107.50	72.86	0.8000	RG1	ST111A-S
ST111B	0.04	88.00	100.00	0.8000	RG1	111
ST111C	0.04	36.80	85.71	1.5000	RG1	ST111C-S
ST507A	0.05	33.50	72.86	1.5000	RG1	ST507A-S
ST508A	0.34	189.20	7.14	1.0000	RG1	508

****
Node Summary
****

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
100	JUNCTION	99.40	2.73	0.0	Yes
101	JUNCTION	99.60	3.69	0.0	
102	JUNCTION	99.78	3.75	0.0	
103	JUNCTION	100.04	3.53	0.0	
104	JUNCTION	100.08	3.49	0.0	
105	JUNCTION	100.23	3.41	0.0	
106	JUNCTION	100.34	2.98	0.0	
107	JUNCTION	100.80	2.42	0.0	
109	JUNCTION	100.83	3.67	0.0	
110	JUNCTION	100.69	3.81	0.0	
111	JUNCTION	101.42	2.65	0.0	
EXT1-OF	OUTFALL	102.88	0.00	0.0	
EXT2-OF	OUTFALL	104.20	0.00	0.0	
HEADWALL	OUTFALL	98.70	1.50	0.0	
POOLE_OF1	OUTFALL	100.10	0.00	0.0	
POOLE_OF2	OUTFALL	101.06	0.00	0.0	
ST104A-OF	OUTFALL	0.00	103.03	0.0	
ST107A-OF	OUTFALL	0.00	103.19	0.0	
108	STORAGE	97.24	7.13	0.0	
508	STORAGE	101.06	1.79	0.0	
ST104A-S	STORAGE	101.52	2.10	0.0	
ST107A-S	STORAGE	101.13	2.10	0.0	
ST108A-S	STORAGE	118.60	0.15	0.0	
ST108B-S	STORAGE	115.75	0.15	0.0	
ST108C-S	STORAGE	110.40	0.15	0.0	
ST108D-S	STORAGE	110.10	0.15	0.0	
ST108E-S	STORAGE	107.20	0.15	0.0	
ST109B-S	STORAGE	102.81	1.50	0.0	
ST109C-S	STORAGE	102.81	1.50	0.0	
ST111A-S	STORAGE	101.86	2.40	0.0	
ST111C-S	STORAGE	101.95	2.10	0.0	
ST507A-S	STORAGE	101.57	2.10	0.0	
TANK	STORAGE	100.10	3.27	0.0	

****
Link Summary
****

Name	From Node	To Node	Type	Length	%Slope	Roughness
Pipe_13	100	HEADWALL	CONDUIT	11.1	0.2515	0.0130
Pipe_14	106	105	CONDUIT	17.6	0.1993	0.0130
Pipe_14_(1)	105	104	CONDUIT	39.1	0.2020	0.0130
Pipe_15	109	104	CONDUIT	38.2	1.0016	0.0130
Pipe_16	110	106	CONDUIT	12.5	0.9994	0.0130
Pipe_17	111	107	CONDUIT	110.4	0.7004	0.0130
Pipe_21	104	103	CONDUIT	16.3	0.2027	0.0130
Pipe_23	508	TANK	CONDUIT	8.7	0.2497	0.0130
Pipe_26	101	100	CONDUIT	101.6	0.1999	0.0130
Pipe_27	108	105	CONDUIT	36.3	0.2999	0.0130
Pipe_29	107	106	CONDUIT	63.3	0.5013	0.0130
Pipe_31	102	101	CONDUIT	70.7	0.1994	0.0130
Pipe_34	103	TANK	CONDUIT	2.8	0.2135	0.0130
ST104A-T	ST104A-S	ST104A-OF	CONDUIT	2.5	24.2860	0.0130
ST107A-T	ST107A-S	ST107A-OF	CONDUIT	2.5	1.6002	0.0130
ST111A-T	ST111A-S	ST111C-S	CONDUIT	40.9	1.2470	0.0130
ST111B-T	ST111C-S	ST107A-S	CONDUIT	60.0	1.1167	0.0130
ST507A-T	ST507A-S	ST104A-S	CONDUIT	14.9	0.2013	0.0130
W1	103	102	CONDUIT	3.0	1.0001	0.0130
OR1	TANK	102	ORIFICE			
OR2	TANK	102	ORIFICE			
OR3	TANK	102	ORIFICE			
ST104A-O	ST104A-S	104	ORIFICE			
ST107A-O	ST107A-S	107	ORIFICE			
ST109B-O	ST109B-S	109	ORIFICE			
ST109C-O	ST109C-S	109	ORIFICE			
ST111A-O	ST111A-S	111	ORIFICE			
ST111C-O	ST111C-S	111	ORIFICE			
ST111C-O1	ST111C-S	111	ORIFICE			
OL1	TANK	POOLE_OF1	OUTLET			
OL2	508	POOLE_OF2	OUTLET			
ST108A-O	ST108A-S	108	OUTLET			
ST108B-O	ST108B-S	108	OUTLET			
ST108C-O	ST108C-S	108	OUTLET			
ST108D-O	ST108D-S	108	OUTLET			
ST108E-O	ST108E-S	108	OUTLET			
ST507A-O	ST507A-S	TANK	OUTLET			

****
Cross Section Summary
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Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
Pipe_13	CIRCULAR	0.68	0.36	0.17	0.68	1	421.55
Pipe_14	CIRCULAR	0.53	0.22	0.13	0.53	1	192.01
Pipe_14_(1)	CIRCULAR	0.60	0.28	0.15	0.60	1	276.00
Pipe_15	CIRCULAR	0.38	0.11	0.09	0.38	1	175.48
Pipe_16	CIRCULAR	0.38	0.11	0.09	0.38	1	175.29
Pipe_17	CIRCULAR	0.38	0.11	0.09	0.38	1	146.75
Pipe_21	CIRCULAR	0.68	0.36	0.17	0.68	1	378.43
Pipe_23	CIRCULAR	0.25	0.05	0.06	0.25	1	29.72
Pipe_26	CIRCULAR	0.45	0.16	0.11	0.45	1	127.47
Pipe_27	CIRCULAR	0.45	0.16	0.11	0.45	1	156.15
Pipe_29	CIRCULAR	0.45	0.16	0.11	0.45	1	201.87
Pipe_31	CIRCULAR	0.45	0.16	0.11	0.45	1	127.33
Pipe_34	CIRCULAR	0.68	0.36	0.17	0.68	1	388.45
ST104A-T	Overland	0.15	1.03	0.14	7.00	1	10684.06
ST107A-T	Overland	0.15	1.03	0.14	7.00	1	2742.50
ST111A-T	Overland	0.15	1.03	0.14	7.00	1	2421.02
ST111B-T	Overland	0.15	1.03	0.14	7.00	1	2291.05
ST507A-T	Overland	0.15	1.03	0.14	7.00	1	972.81
W1	CIRCULAR	0.45	0.16	0.11	0.45	1	285.13

****
Transect Summary
****

Transect Overland

Area:

0.0196	0.0392	0.0588	0.0784	0.0980
0.1177	0.1374	0.1571	0.1768	0.1965
0.2162	0.2360	0.2558	0.2756	0.2954
0.3152	0.3351	0.3550	0.3748	0.3947
0.4147	0.4346	0.4546	0.4745	0.4945
0.5145	0.5346	0.5546	0.5747	0.5947
0.6148	0.6350	0.6551	0.6752	0.6954
0.7156	0.7358	0.7560	0.7762	0.7965
0.8168	0.8371	0.8574	0.8777	0.8980
0.9184	0.9388	0.9592	0.9796	1.0000

Hrad:

0.0208	0.0415	0.0622	0.0829	0.1036
0.1242	0.1448	0.1653	0.1858	0.2063
0.2268	0.2472	0.2676	0.2879	0.3083
0.3285	0.3488	0.3690	0.3892	0.4094
0.4295	0.4496	0.4697	0.4897	0.5097
0.5297	0.5496	0.5695	0.5894	0.6093

```
             0.6291     0.6489     0.6686     0.6884     0.7081
             0.7277     0.7474     0.7670     0.7865     0.8061
             0.8256     0.8451     0.8646     0.8840     0.9034
             0.9228     0.9421     0.9614     0.9807     1.0000
Width:
             0.9580     0.9589     0.9597     0.9606     0.9614
             0.9623     0.9631     0.9640     0.9649     0.9657
             0.9666     0.9674     0.9683     0.9691     0.9700
             0.9709     0.9717     0.9726     0.9734     0.9743
             0.9751     0.9760     0.9769     0.9777     0.9786
             0.9794     0.9803     0.9811     0.9820     0.9829
             0.9837     0.9846     0.9854     0.9863     0.9871
             0.9880     0.9889     0.9897     0.9906     0.9914
             0.9923     0.9931     0.9940     0.9949     0.9957
             0.9966     0.9974     0.9983     0.9991     1.0000

Transect Overland(orig)
Area:
             0.0196     0.0392     0.0588     0.0784     0.0980
             0.1177     0.1374     0.1571     0.1768     0.1965
             0.2162     0.2360     0.2558     0.2756     0.2954
             0.3152     0.3351     0.3550     0.3748     0.3947
             0.4147     0.4346     0.4546     0.4745     0.4945
             0.5145     0.5346     0.5546     0.5747     0.5947
             0.6148     0.6350     0.6551     0.6752     0.6954
             0.7156     0.7358     0.7560     0.7762     0.7965
             0.8168     0.8371     0.8574     0.8777     0.8980
             0.9184     0.9388     0.9592     0.9796     1.0000
Hrad:
             0.0208     0.0415     0.0622     0.0829     0.1036
             0.1242     0.1448     0.1653     0.1858     0.2063
             0.2268     0.2472     0.2676     0.2879     0.3083
             0.3285     0.3488     0.3690     0.3892     0.4094
             0.4295     0.4496     0.4697     0.4897     0.5097
             0.5297     0.5496     0.5695     0.5894     0.6093
             0.6291     0.6489     0.6686     0.6884     0.7081
             0.7277     0.7474     0.7670     0.7865     0.8061
             0.8256     0.8451     0.8646     0.8840     0.9034
             0.9228     0.9421     0.9614     0.9807     1.0000
Width:
             0.9580     0.9589     0.9597     0.9606     0.9614
             0.9623     0.9631     0.9640     0.9649     0.9657
             0.9666     0.9674     0.9683     0.9691     0.9700
             0.9709     0.9717     0.9726     0.9734     0.9743
             0.9751     0.9760     0.9769     0.9777     0.9786
             0.9794     0.9803     0.9811     0.9820     0.9829
             0.9837     0.9846     0.9854     0.9863     0.9871
             0.9880     0.9889     0.9897     0.9906     0.9914
```



```
             0.9923     0.9931     0.9940     0.9949     0.9957
             0.9966     0.9974     0.9983     0.9991     1.0000


*********************************************************
NOTE: The summary statistics displayed in this report are
based on results found at every computational time step,
not just on results from each reporting time step.
*********************************************************

****************
Analysis Options
****************
Flow Units ............... LPS
Process Models:
  Rainfall/Runoff ........ YES
  RDII ................... NO
  Snowmelt ............... NO
  Groundwater ............ NO
  Flow Routing ........... YES
  Ponding Allowed ........ YES
  Water Quality .......... NO
Infiltration Method ...... CURVE_NUMBER
Flow Routing Method ...... DYNWAVE
Starting Date ............ 07/23/2009 00:00:00
Ending Date .............. 07/24/2009 00:00:00
Antecedent Dry Days ...... 0.0
Report Time Step ......... 00:05:00
Wet Time Step ............ 00:05:00
Dry Time Step ............ 00:05:00
Routing Time Step ........ 1.00 sec
Variable Time Step ....... NO
Maximum Trials ........... 8
Number of Threads ........ 4
Head Tolerance ........... 0.001500 m


**************************        Volume         Depth
Runoff Quantity Continuity     hectare-m            mm
**************************     ---------       -------
Total Precipitation ......         0.285        95.520
Evaporation Loss .........         0.000         0.000
Infiltration Loss ........         0.041        13.902
Surface Runoff ...........         0.239        80.308
Final Storage ............         0.005         1.589
Continuity Error (%) .....        -0.292
```

```
**************************        Volume         Volume
Flow Routing Continuity         hectare-m       10^6 ltr
**************************      ---------      ---------
Dry Weather Inflow .......          0.000          0.000
Wet Weather Inflow .......          0.239          2.394
Groundwater Inflow .......          0.000          0.000
RDII Inflow ..............          0.000          0.000
External Inflow ..........          1.512         15.120
External Outflow .........          1.711         17.108
Flooding Loss ............          0.000          0.000
Evaporation Loss .........          0.000          0.000
Exfiltration Loss ........          0.000          0.000
Initial Stored Volume ....          0.000          0.000
Final Stored Volume ......          0.041          0.409
Continuity Error (%) .....         -0.017


********************************
Highest Flow Instability Indexes
********************************
Link Pipe_34 (3)


*************************
Routing Time Step Summary
*************************
Minimum Time Step           :     1.00 sec
Average Time Step           :     1.00 sec
Maximum Time Step           :     1.00 sec
Percent in Steady State     :     0.00
Average Iterations per Step :     2.00
Percent Not Converging      :     0.05


***************************
Subcatchment Runoff Summary
***************************
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Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	95.52	0.00	0.00	41.82	52.63	0.04	16.91	0.551
EXT2	95.52	0.00	0.00	11.35	83.12	0.05	20.73	0.870
ST104A	95.52	0.00	0.00	6.56	87.63	0.13	50.18	0.917
ST107A	95.52	0.00	0.00	14.94	79.28	0.30	115.66	0.830
ST108A	95.52	0.00	0.00	0.00	94.37	0.38	142.13	0.988

ST108B	95.52	0.00	0.00	0.00	94.37	0.34	128.25	0.988
ST108C	95.52	0.00	0.00	0.00	94.35	0.04	16.57	0.988
ST108D	95.52	0.00	0.00	0.00	94.37	0.05	18.20	0.988
ST108E	95.52	0.00	0.00	0.00	94.08	0.03	9.57	0.985
ST108F	95.52	0.00	0.00	23.30	70.82	0.27	102.04	0.741
ST109A	95.52	0.00	0.00	0.00	93.98	0.01	4.76	0.984
ST109B	95.52	0.00	0.00	0.00	94.28	0.05	19.11	0.987
ST109C	95.52	0.00	0.00	0.00	94.29	0.06	20.63	0.987
ST110A	95.52	0.00	0.00	38.83	54.74	0.04	12.92	0.573
ST110B	95.52	0.00	0.00	0.00	93.99	0.03	11.23	0.984
ST110C	95.52	0.00	0.00	0.00	93.99	0.03	10.41	0.984
ST110D	95.52	0.00	0.00	38.83	54.74	0.04	12.82	0.573
ST111A	95.52	0.00	0.00	11.35	82.89	0.20	77.09	0.868
ST111B	95.52	0.00	0.00	0.00	94.00	0.04	13.13	0.984
ST111C	95.52	0.00	0.00	5.96	88.16	0.04	14.64	0.923
ST507A	95.52	0.00	0.00	11.34	82.85	0.05	17.48	0.867
ST508A	95.52	0.00	0.00	38.83	55.31	0.19	79.58	0.579

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******************
Node Depth Summary
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Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
100	JUNCTION	0.44	0.52	99.92	0 06:53	0.52
101	JUNCTION	0.41	0.74	100.34	0 06:53	0.74
102	JUNCTION	0.39	0.76	100.54	0 06:52	0.76
103	JUNCTION	0.45	1.94	101.97	0 06:52	1.93
104	JUNCTION	0.41	1.90	101.98	0 06:52	1.89
105	JUNCTION	0.34	1.76	101.99	0 06:52	1.75
106	JUNCTION	0.29	1.65	101.99	0 06:52	1.64
107	JUNCTION	0.12	1.20	102.00	0 06:52	1.19
109	JUNCTION	0.10	1.15	101.98	0 06:52	1.14
110	JUNCTION	0.13	1.31	101.99	0 06:52	1.30
111	JUNCTION	0.25	0.59	102.02	0 06:51	0.59
EXT1-OF	OUTFALL	0.00	0.00	102.88	0 00:00	0.00
EXT2-OF	OUTFALL	0.00	0.00	104.20	0 00:00	0.00
HEADWALL	OUTFALL	0.00	0.00	98.70	0 00:00	0.00
POOLE_OF1	OUTFALL	0.00	0.00	100.10	0 00:00	0.00
POOLE_OF2	OUTFALL	0.00	0.00	101.06	0 00:00	0.00
ST104A-OF	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
ST107A-OF	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
108	STORAGE	2.74	4.80	102.04	0 06:48	4.79
508	STORAGE	0.46	0.95	102.01	0 06:14	0.95

ST104A-S	STORAGE	0.05	1.71	103.23	0	06:15	1.71
ST107A-S	STORAGE	0.09	1.72	102.85	0	06:15	1.71
ST108A-S	STORAGE	0.02	0.14	118.74	0	06:20	0.14
ST108B-S	STORAGE	0.02	0.14	115.89	0	06:20	0.14
ST108C-S	STORAGE	0.02	0.14	110.54	0	06:19	0.14
ST108D-S	STORAGE	0.02	0.14	110.24	0	06:19	0.14
ST108E-S	STORAGE	0.02	0.14	107.34	0	06:19	0.14
ST109B-S	STORAGE	0.01	0.15	102.96	0	06:10	0.15
ST109C-S	STORAGE	0.01	0.16	102.97	0	06:15	0.16
ST111A-S	STORAGE	0.23	2.32	104.18	0	06:24	2.32
ST111C-S	STORAGE	0.01	0.08	102.03	0	06:15	0.08
ST507A-S	STORAGE	0.06	1.87	103.44	0	06:17	1.87
TANK	STORAGE	0.38	1.87	101.97	0	06:52	1.86

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Node Inflow Summary
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Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
100	JUNCTION	175.00	311.33	0 06:53	15.1	16.9	0.017
101	JUNCTION	0.00	136.63	0 06:52	0	1.83	-0.075
102	JUNCTION	0.00	136.71	0 06:52	0	1.83	0.036
103	JUNCTION	0.00	457.04	0 06:14	0	1.71	-0.003
104	JUNCTION	0.00	457.02	0 06:14	0	1.71	-0.145
105	JUNCTION	0.00	368.34	0 06:14	0	1.49	0.141
106	JUNCTION	0.00	206.19	0 06:14	0	0.707	-0.315
107	JUNCTION	0.00	159.99	0 06:15	0	0.569	0.194
109	JUNCTION	4.76	44.51	0 06:10	0.0127	0.119	0.473
110	JUNCTION	47.37	74.61	0 06:14	0.139	0.139	0.082
111	JUNCTION	13.13	45.04	0 06:15	0.0351	0.275	0.493
EXT1-OF	OUTFALL	16.91	16.91	0 06:15	0.0354	0.0354	0.000
EXT2-OF	OUTFALL	20.73	20.73	0 06:15	0.053	0.053	0.000
HEADWALL	OUTFALL	0.00	311.33	0 06:54	0	16.9	0.000
POOLE_OF1	OUTFALL	0.00	0.66	0 01:27	0	0.054	0.000
POOLE_OF2	OUTFALL	0.00	0.30	0 05:14	0	0.0203	0.000
ST104A-OF	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
ST107A-OF	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
108	STORAGE	102.04	240.16	0 06:13	0.271	1.14	-0.083
508	STORAGE	79.58	79.58	0 06:15	0.186	0.186	0.039
ST104A-S	STORAGE	50.18	50.18	0 06:15	0.131	0.131	0.004
ST107A-S	STORAGE	115.66	115.66	0 06:15	0.296	0.296	0.014
ST108A-S	STORAGE	142.13	142.13	0 06:15	0.381	0.381	-0.001
ST108B-S	STORAGE	128.25	128.25	0 06:15	0.344	0.344	-0.001
ST108C-S	STORAGE	16.57	16.57	0 06:15	0.0444	0.0444	-0.001
ST108D-S	STORAGE	18.20	18.20	0 06:15	0.0488	0.0488	-0.001
ST108E-S	STORAGE	9.57	9.57	0 06:10	0.0256	0.0256	-0.000
ST109B-S	STORAGE	19.11	19.11	0 06:10	0.0512	0.0512	-0.000
ST109C-S	STORAGE	20.63	20.63	0 06:15	0.0553	0.0553	-0.000
ST111A-S	STORAGE	77.09	77.09	0 06:15	0.201	0.201	-0.001
ST111C-S	STORAGE	14.64	14.64	0 06:15	0.0384	0.0384	0.078
ST507A-S	STORAGE	17.48	17.48	0 06:15	0.0451	0.0451	-0.002
TANK	STORAGE	0.00	548.41	0 06:15	0	1.89	-0.001

****
Node Surcharge Summary
****

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
104	JUNCTION	4.28	1.156	1.592
105	JUNCTION	3.62	1.092	1.645
106	JUNCTION	3.40	1.058	1.324
107	JUNCTION	2.19	0.747	1.220
109	JUNCTION	2.28	0.773	2.522
110	JUNCTION	2.81	0.933	2.506

****
Node Flooding Summary
****

No nodes were flooded.

****
Storage Volume Summary
****

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
108	0.288	79	0	0	0.364	100	0 06:13	210.40
508	0.030	55	0	0	0.053	99	0 06:13	79.31

ST104A-S	0.000	0	0	0	0.000	0	0	00:00	49.96
ST107A-S	0.000	0	0	0	0.000	0	0	00:00	114.99
ST108A-S	0.010	6	0	0	0.141	86	0	06:20	30.68
ST108B-S	0.008	6	0	0	0.126	85	0	06:20	28.40
ST108C-S	0.001	4	0	0	0.015	80	0	06:19	4.26
ST108D-S	0.001	5	0	0	0.017	83	0	06:19	4.30
ST108E-S	0.001	5	0	0	0.009	85	0	06:19	2.17
ST109B-S	0.000	0	0	0	0.000	0	0	00:00	19.11
ST109C-S	0.000	0	0	0	0.000	0	0	00:00	20.63
ST111A-S	0.003	3	0	0	0.058	53	0	06:24	17.36
ST111C-S	0.000	0	0	0	0.000	0	0	00:00	14.64
ST507A-S	0.000	0	0	0	0.004	6	0	06:17	10.93
TANK	0.175	30	0	0	0.592	100	0	06:47	137.37

****
Outfall Loading Summary
****

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
EXT1-OF	30.18	1.36	16.91	0.035
EXT2-OF	45.84	1.34	20.73	0.053
HEADWALL	100.00	196.13	311.33	16.945
POOLE_OF1	95.39	0.65	0.66	0.054
POOLE_OF2	78.72	0.30	0.30	0.020
ST104A-OF	0.00	0.00	0.00	0.000
ST107A-OF	0.00	0.00	0.00	0.000
System	50.02	199.78	316.10	17.108

****
Link Flow Summary
****

Link	Type	Maximum |Flow| LPS	Time of Max Occurrence days hr:min	Maximum |Veloc| m/sec	Max/ Full Flow	Max/ Full Depth
Pipe_13	CONDUIT	311.33	0 06:54	1.60	0.74	0.54
Pipe_14	CONDUIT	210.46	0 06:14	1.40	1.10	1.00
Pipe_14_(1)	CONDUIT	362.62	0 06:14	1.29	1.31	1.00
Pipe_15	CONDUIT	45.52	0 06:11	1.24	0.26	1.00

Link	Type	Maximum |Flow| LPS	Time of Max Occurrence days hr:min	Maximum |Veloc| m/sec	Max/ Full Flow	Max/ Full Depth
Pipe_16	CONDUIT	64.13	0 06:14	0.94	0.37	1.00
Pipe_17	CONDUIT	45.00	0 06:15	1.00	0.31	0.99
Pipe_21	CONDUIT	457.04	0 06:14	1.28	1.21	1.00
Pipe_23	CONDUIT	79.01	0 06:15	1.65	2.66	1.00
Pipe_26	CONDUIT	136.33	0 06:53	1.08	1.07	0.74
Pipe_27	CONDUIT	210.40	0 06:14	1.32	1.35	1.00
Pipe_29	CONDUIT	158.73	0 06:14	1.45	0.79	1.00
Pipe_31	CONDUIT	136.63	0 06:52	0.88	1.07	0.94
Pipe_34	CONDUIT	458.73	0 06:15	1.35	1.18	1.00
ST104A-T	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
ST107A-T	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
ST111A-T	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
ST111B-T	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
ST507A-T	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
W1	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
OR1	ORIFICE	32.79	0 06:48			1.00
OR2	ORIFICE	55.72	0 06:52			1.00
OR3	ORIFICE	48.25	0 06:52			1.00
ST104A-O	ORIFICE	49.96	0 06:15			1.00
ST107A-O	ORIFICE	114.99	0 06:15			1.00
ST109B-O	ORIFICE	19.11	0 06:10			0.76
ST109C-O	ORIFICE	20.63	0 06:15			0.80
ST111A-O	ORIFICE	17.36	0 06:24			1.00
ST111C-O	ORIFICE	7.32	0 06:15			0.40
ST111C-O1	ORIFICE	7.32	0 06:15			0.40
OL1	DUMMY	0.66	0 01:27			
OL2	DUMMY	0.30	0 05:14			
ST108A-O	DUMMY	30.68	0 06:20			
ST108B-O	DUMMY	28.40	0 06:20			
ST108C-O	DUMMY	4.26	0 06:19			
ST108D-O	DUMMY	4.30	0 06:19			
ST108E-O	DUMMY	2.17	0 06:19			
ST507A-O	DUMMY	10.93	0 06:17			

****
Flow Classification Summary
****

Conduit	Adjusted /Actual Length	---------- Fraction of Time in Flow Class ----------								
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
Pipe_13	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
Pipe_14	1.00	0.04	0.00	0.00	0.39	0.00	0.00	0.56	0.02	0.00
Pipe_14_(1)	1.00	0.04	0.00	0.00	0.51	0.00	0.00	0.45	0.06	0.00
Pipe_15	1.00	0.04	0.00	0.00	0.36	0.00	0.00	0.61	0.19	0.00

Pipe_16	1.00	0.04	0.00	0.00	0.31	0.00	0.00	0.65	0.05	0.00
Pipe_17	1.00	0.04	0.00	0.00	0.15	0.00	0.00	0.80	0.11	0.00
Pipe_21	1.00	0.04	0.00	0.00	0.94	0.00	0.00	0.02	0.14	0.00
Pipe_23	1.00	0.62	0.00	0.00	0.05	0.00	0.00	0.33	0.00	0.00
Pipe_26	1.00	0.00	0.09	0.00	0.72	0.00	0.00	0.19	0.54	0.00
Pipe_27	1.00	0.05	0.20	0.00	0.42	0.00	0.02	0.32	0.06	0.00
Pipe_29	1.00	0.04	0.00	0.00	0.34	0.00	0.00	0.63	0.17	0.00
Pipe_31	1.00	0.07	0.00	0.00	0.90	0.00	0.00	0.02	0.00	0.00
Pipe_34	1.00	0.04	0.00	0.00	0.89	0.07	0.00	0.00	0.01	0.00
ST104A-T	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST107A-T	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST111A-T	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST111B-T	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ST507A-T	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
W1	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

****
Conduit Surcharge Summary
****

Conduit	--------- Hours Full -------- Both Ends	Upstream	Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
Pipe_14	3.40	3.40	3.62	0.05	0.01
Pipe_14_(1)	3.61	3.62	4.28	0.07	0.05
Pipe_15	2.28	2.28	4.28	0.01	0.01
Pipe_16	2.81	2.81	3.40	0.01	0.01
Pipe_17	0.01	0.01	2.19	0.01	0.01
Pipe_21	4.31	4.31	4.63	0.05	0.03
Pipe_23	0.34	0.42	0.44	0.19	0.01
Pipe_26	0.01	0.01	0.01	0.72	0.01
Pipe_27	3.71	3.71	4.69	0.06	0.16
Pipe_29	2.19	2.19	3.40	0.01	0.01
Pipe_31	0.01	0.16	0.01	0.73	0.01
Pipe_34	4.68	4.68	4.75	0.04	0.02

Analysis begun on: Thu Mar 23 14:03:56 2017
Analysis ended on: Thu Mar 23 14:03:58 2017
Total elapsed time: 00:00:02

C.4 OIL/GRIT SEPARATOR SIZING CALCULATIONS

Stormceptor Design Summary

PCSWMM for Stormceptor

Project Information

Date	11/4/2016
Project Name	5731 Hazeldean
Project Number	160401195
Location	Ottawa, ON

Designer Information

Company	Stantec Consulting Ltd.
Contact	N/A

Notes

N/A

Drainage Area

Total Area (ha)	2.72
Imperviousness (%)	70

The Stormceptor System model STC 3000 achieves the water quality objective removing 80% TSS for a CLOCA (clay, silt and sand) particle size distribution.

Rainfall

Name	OTTAWA MACDONALD-CARTIER INT'L A
State	ON
ID	6000
Years of Records	1967 to 2003
Latitude	45°19'N
Longitude	75°40'W

Water Quality Objective

TSS Removal (%)	80

Upstream Storage

Storage (ha-m)	Discharge (L/s)
0	0

Stormceptor Sizing Summary

Stormceptor Model	TSS Removal %
STC 300	60
STC 750	73
STC 1000	73
STC 1500	74
STC 2000	79
STC 3000	**80**
STC 4000	84
STC 5000	84
STC 6000	87
STC 9000	90
STC 10000	90
STC 14000	92

Stormceptor®

Particle Size Distribution

Removing silt particles from runoff ensures that the majority of the pollutants, such as hydrocarbons and heavy metals that adhere to fine particles, are not discharged into our natural water courses. The table below lists the particle size distribution used to define the annual TSS removal.

CLOCA (clay, silt and sand)								
Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s		Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s
850	3.3	2.65	0.1465		50	3.9	2.65	0.0022
425	23.4	2.65	0.0698		36	2.6	2.65	0.0012
300	17.5	2.65	0.0439		22	1.3	2.65	0.0004
250	6.5	2.65	0.0335		12	1.9	2.65	0.0004
212	6.5	2.65	0.0259		9	0	2.65	0.0004
150	11.7	2.65	0.0145		6.5	1.3	2.65	0.0004
125	5.2	2.65	0.0105		3	1.3	2.65	0.0004
100	3.9	2.65	0.0070		1.5	1.3	2.65	0.0004
75	3.9	2.65	0.0040		1	4.5	2.65	0.0004

Stormceptor Design Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor version 1.0
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal.
- Only the STC 300 is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 750 to STC 6000 may accommodate multiple inlet pipes.
- Inlet and outlet invert elevation differences are as follows:

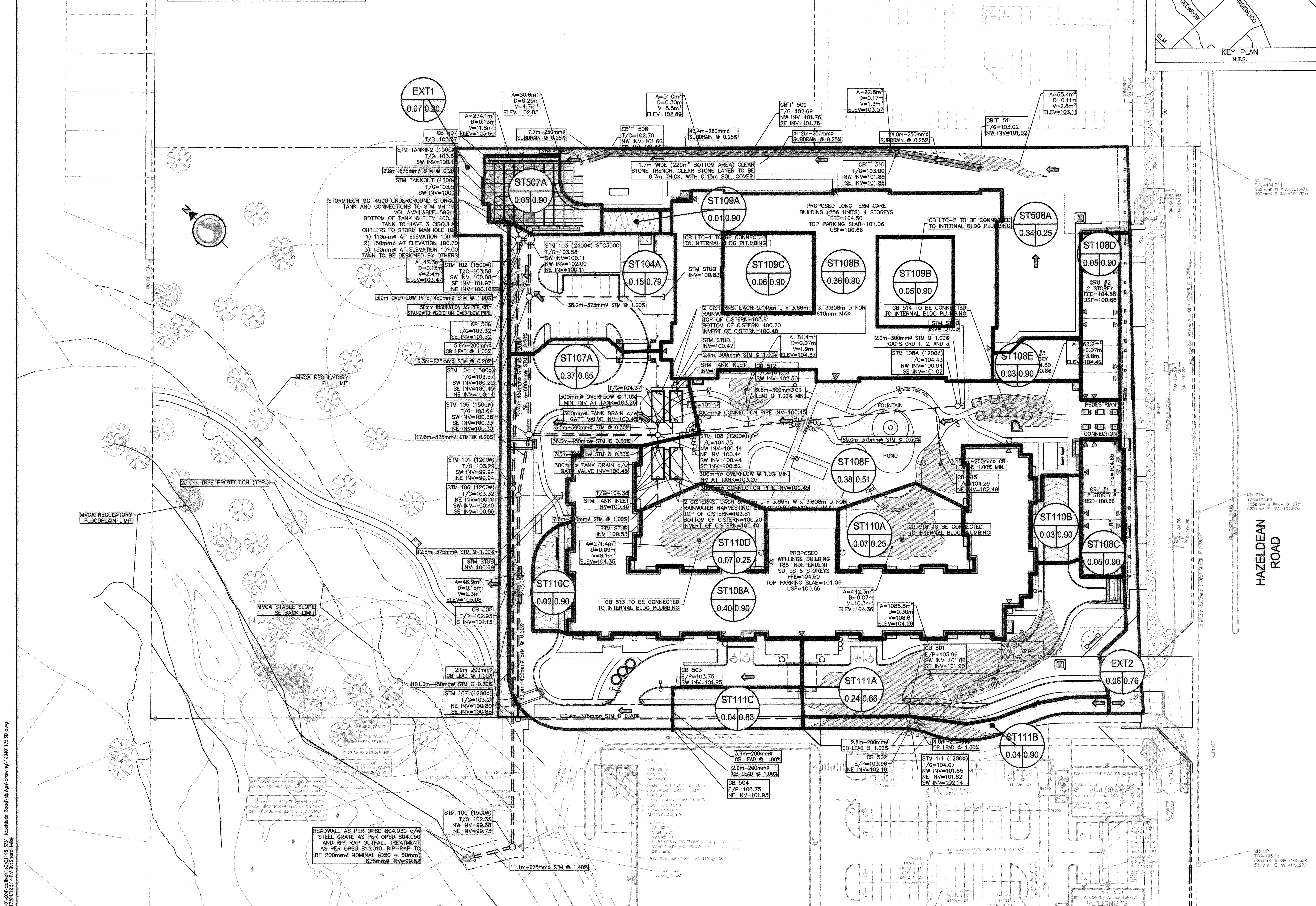
Inlet and Outlet Pipe Invert Elevations Differences

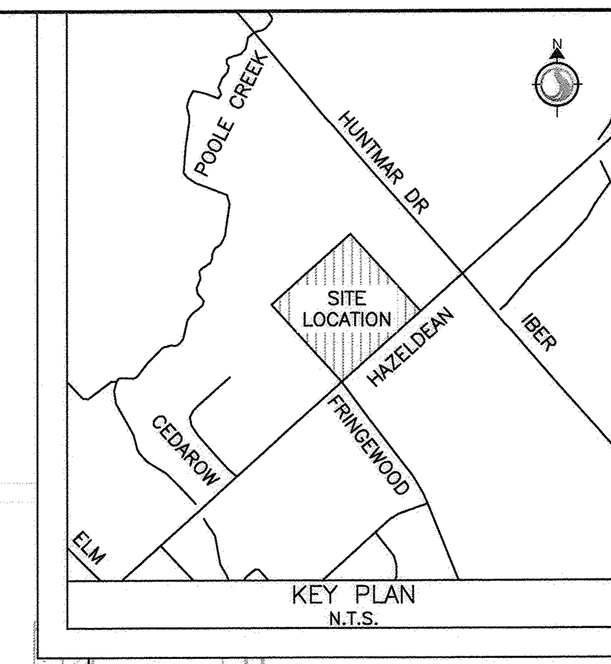
Inlet Pipe Configuration	STC 300	STC 750 to STC 6000	STC 9000 to STC 14000
Single inlet pipe	75 mm	25 mm	75 mm
Multiple inlet pipes	75 mm	75 mm	Only one inlet pipe.

- Design estimates are based on stable site conditions only, after construction is completed.
- Design estimates assume that the storm drain is not submerged during zero flows. For submerged applications, please contact your local Stormceptor representative.
- Design estimates may be modified for specific spills controls. Please contact your local Stormceptor representative for further assistance.
- For pricing inquiries or assistance, please contact Imbrium Systems Inc., 1-800-565-4801.

SCHEDULE OF INLET CONTROL DEVICES					
STRUCTURE ID	DRAINAGE AREA ID	ORIFICE DIA. (mm)	ICD INVERT (m)	100 YEAR HEAD (m)	100 YEAR FLOW (L/s)
CB 501	ST111A	IPEX HF (3.0")	101.86	2.32	17.4
CB 503/504	ST111C	200mm PIPE (x2)	101.95	0.08	7.3 (x2)
CB 505	ST107A	200mm PIPE	101.13	1.72	115.0
CB 506	ST104A	IPEX HF (5.5")	101.52	1.71	50.0
CB 507	ST507A	IPEX LMF 95	101.57	1.87	10.9
SWM TANK	–	150	101.00	0.97	48.3
SWM TANK	–	150	100.70	1.27	55.7
SWM TANK	–	110	100.10	1.87	32.8

SCHEDULE OF ROOF RELEASE RATES			
BUILDING	Head (m)	ICD TYPE	100YR RELEASE RATE (L/s)
WELLINGS BUILDING	0.14	ZURN Z-105-5	29.9
LONG TERM CARE BUILDING	0.14	ZURN Z-105-5	27.7
CRU 1	0.14	ZURN Z-105-5	4.2
CRU 2	0.14	ZURN Z-105-5	4.2
CRU 3	0.14	ZURN Z-105-5	2.1





Stantec

Stantec Consulting Ltd.
400 - 1331 Clyde Avenue
Ottawa ON
Tel. 613.722.4420
www.stantec.com

Legend

- AREA ID
- RUNOFF COEFFICIENT
- STORM DRAINAGE AREA ha.
- STORM DRAINAGE BOUNDARY
- 100YEAR PONDING LIMITS
- DIRECTION OF OVERLAND FLOW
- PROPOSED STORM SEWER
- PROPOSED CATCHBASIN MANHOLE
- PROPOSED CATCHBASIN
- PROPOSED BUILDING CATCHBASIN
- PROPOSED SUB DRAIN CATCH BASIN AS PER CITY OF OTTAWA STANDARD DETAIL DRAWINGS L10 AND L11.
- PROPOSED PERFORATED SUBDRAIN
- EXISTING STORM SEWER
- EXISTING CATCHBASIN

APPROVED ☑ REFUSED ☐

THIS 17 DAY OF May, 2017

Derrick Moodie, Manager
Development Review West
Planning, Infrastructure and Economic
Development Department, City of Ottawa

		By	Appd.	YY.MM.DD
6	ISSUED FOR MYLARS	MJS	PM	17.04.12
5	REVISED AS PER CITY COMMENTS	MJS	PM	17.03.24
4	ISSUED FOR TENDER	MJS	GR	17.03.22
3	REVISED AS PER CITY COMMENTS	DT	PM	17.01.27
2	REVISED AS PER CITY COMMENTS	MJS	PM	16.09.09
1	ISSUED TO CITY FOR SPA	MJS	PM	16.04.11

Revision

File Name: 160401195-DB.dwg	MJS	PM	MJS	16.02.08
	Dwn.	Chkd.	Dsgn.	YY.MM.DD

Permit-Seal

LICENSED PROFESSIONAL ENGINEER
D. C. THIFFAULT
100186107
13/04/2017
PROVINCE OF ONTARIO

Client/Project

WELLINGS OF STITTSVILLE INC. AND EXTENDICARE (CANADA) INC.

WELLINGS OF STITTSVILLE SENIOR'S LIVING AND EXTENDICARE L.T.C. STITTSVILLE
5731 HAZELDEAN ROAD

OTTAWA, ON

Title

STORM DRAINAGE PLAN

Project No.	Scale
160401195	1:500 (0 5 15 25m)

Drawing No.	Sheet	Revision
SD-1	6 of 7	6

Plan # 17484

D07-12-16-0046

ORIGINAL SHEET - ARCH D

C.8 WATER BALANCE CALCULATIONS

Project #160401511 - 20 Cedarrow Drive

Infiltration calculations

Required Infiltration Rate (KWMSS)	73	mm/yr
Site Area	2.29	ha
Pre-Development Imperviousness	0	%
Pre-Development Infiltration	1674.5	m^3/yr
Post Development Imperviousness	66.3	%
Post Development Pervious Area	0.69	ha
Post Development Infiltration in Pervious Areas	503.4	m^3/yr
Post Development Infiltration Volume Req.	**1171.1**	**m^3/yr**

Determine Volume of Water to be Sequestered in Infiltration Trench (Assume storage up to 25mm event)

Area Tributary to Infiltration Trench	3517	m^2		
Impervious Area to Infiltration Trench	3517	m^2	100.0	% *Impervious*
Total Depth of Annual Runoff to Infiltration Trench	760.5	mm/yr	(910.5mm/yr annual precipitation less urban ET of 150mm/yr)	
Volume of Runoff from Impervious Area to Infiltration Trench	**2674.7**	**m^3/yr**	for events with rainfall <25mm	
In order to store up to 25mm from catchment area:				
Max. Capacity Required (25mm)=	88	m^3 volume of runoff		
Trench Length (m)	40.00	m	40%	Trench Porosity
Trench Width (m)	5.50	m		
Trench Height (m)	1.00	m		
Volume Provided	**88**	**m^3**		

Appendix D GEOTECHNICAL INVESTIGATION

Geotechnical Engineering

Environmental Engineering

Hydrogeology

Geological Engineering

Materials Testing

Building Science

Archaeological Services

patersongroup

Geotechnical Investigation

Proposed Mixed-Use Development
Wellings of Stittsville - Phase 2
20 Cedarow Court
Ottawa, Ontario

Prepared For

Nautical Lands Group

March 7, 2019

Report PG4772-1

Paterson Group Inc.
Consulting Engineers
154 Colonnade Road South
Ottawa (Nepean), Ontario
Canada K2E 7J5

Tel: (613) 226-7381
Fax: (613) 226-6344
www.patersongroup.ca

Table of Contents

Appendices

Appendix 1	Soil Profile and Test Data Sheets Symbols and Terms Analytical Testing Results
Appendix 2	Figure 1 - Key Plan Figures 2 to 4 - Slope Stability Analysis Sections Drawing PG4772-1 - Test Hole Location Plan

1.0 Introduction

Paterson Group (Paterson) was commissioned by Nautical Lands Group to conduct a geotechnical investigation for the proposed mixed-use development to be located at 20 Cedarow Court in the City of Ottawa, Ontario (refer to Figure 1 - Key Plan in Appendix 2).

The objectives of the current investigation were to:

- ❑ Determine the subsurface conditions by means of boreholes.

- ❑ Provide geotechnical recommendations for the design of the proposed development including construction considerations which may affect the design.

The following report has been prepared specifically and solely for the aforementioned project. This report contains geotechnical findings and includes recommendations pertaining to the design and construction of the proposed development as understood at the time of writing this report.

2.0 Proposed Development

Based on the available drawings, it is our understanding that the proposed development will consist of four, five (5) storey mixed-use buildings with a shared underground parking level occupying the majority of the footprint of the subject site. The buildings are understood to include retail, office space and residential units. A one (1) storey restaurant building is also proposed within the centre of the site. At-grade parking areas, access lanes and landscaped areas are also anticipated a part of the development. It is anticipated that the proposed development will be municipally serviced.

3.0 Method of Investigation

3.1 Field Investigation

Field Program

The field program for the current investigation was carried out from January 14, 2019 to January 18, 2019. At that time, 29 boreholes were drilled to a maximum depth of 4 m below existing grade. The borehole locations were distributed in a manner to provide general coverage of the proposed development. The locations of the boreholes are shown on Drawing PG4772-1 - Test Hole Location Plan included in Appendix 2.

The boreholes were drilled using a track-mounted auger drill rig operated by a two-person crew. All fieldwork was conducted under the full-time supervision of Paterson personnel with the direction of a senior engineer. The drilling procedure consisted of augering to the required depths at the selected locations, sampling and testing the overburden.

Sampling and In Situ Testing

Soil samples were recovered from a 50 mm diameter split-spoon or the auger flights. The split-spoon and auger samples were classified on site and placed in sealed plastic bags. All samples were transported to our laboratory. The depths at which the split-spoon and auger samples were recovered from the boreholes are presented as SS and AU, respectively, on the Soil Profile and Test Data sheets.

Standard Penetration Tests (SPT) were conducted and recorded as “N” values on the Soil Profile and Test Data sheets. The “N” value is the number of blows required to drive the split-spoon sample 300 mm into the soil after the initial penetration of 150 mm using a 63.5 kg hammer falling from a height of 760 mm.

Undrained shear strength tests were conducted in cohesive soils with a field vane apparatus.

The subsurface conditions observed in the boreholes were recorded in detail in the field. The soil profiles are presented on the Soil Profile and Test Data sheets in Appendix 1.

Groundwater

Flexible polyethylene standpipes were installed in the majority of the boreholes to permit groundwater results subsequent to the sampling program completion. Monitoring wells were installed in BH 4, BH 9, BH 15, BH 22, and BH 27 to provide general site coverage as part of our hydrogeological study. The groundwater observations are discussed in Subsection 4.3 and presented in the Soil Profile and Test Data Sheets in Appendix 1.

Sample Storage

All samples will be stored in the laboratory for a period of one month after issuance of this report at which time the samples will be discarded unless otherwise directed.

3.2 Field Survey

The borehole locations were selected by Paterson taking in consideration site features. The ground surface at the test pit locations was located and surveyed by Annis, O'Sullivan, Vollebekk LTD. It is understood that the ground surface elevations at the borehole locations were referenced to a geodetic datum. The locations and ground surface elevation at the boreholes are presented on Drawing PG4772-1 - Test Hole Location Plan in Appendix 2.

3.3 Laboratory Testing

Soil samples recovered from the subject site were visually examined in our laboratory to review the field logs. All samples will be stored in the laboratory for a period of one month after the issuance of this report. They will then be discarded unless we are otherwise directed.

3.4 Analytical Testing

One (1) soil sample was submitted for analytical testing to assess the potential for exposed ferrous metals and the sulphate potential against subsurface concrete structures. The results are discussed further in Subsection 6.7.

4.0 Observations

4.1 Surface Conditions

The subject site is currently undeveloped and grass covered with a tree-line located along the west boundary line of Cedarow Court. The ground surface across the site is relatively flat and approximately 1 m lower than adjacent properties and Hazeldean Road. Poole Creek ravine runs along the western border of the subject site approximately 3 m below the subject site.

The subject site is bordered by an active construction site for Phase 1 of the Wellings of Stittsville development along the north, Hazeldean Road along the east, and commercial buildings at the edge of Cedarow Court along the south.

4.2 Subsurface Profile

Overburden

The subsurface profile at the borehole locations consists of topsoil overlying a hard to very stiff silty clay crust followed by a grey, very stiff to stiff silty clay layer. Glacial till was encountered below the silty clay layer consisting of compact silty sand to sandy silt with clay, gravel, cobbles and boulders. A deposit of very stiff to hard clayey silt was encountered below the topsoil in BH 17, BH 18, BH 24, BH 25, BH 26, and BH 27. Practical refusal to augering on inferred bedrock was encountered in all boreholes at depths ranging between 1.6 to 4.0 m. Specific details of the soil profile at each test hole location are presented on the Soil Profile and Test Data sheets provided in Appendix 1.

Bedrock

Based on available geological mapping, the subject site consists of interbedded dolostone and limestone of the Gull River formation and an approximate drift thickness of 2 to 15 m.

4.3 Groundwater

The measured groundwater levels at the borehole locations are presented in Table 1. Groundwater readings recorded in flexible piezometers could be influenced by surface water infiltrating the backfilled boreholes. The long-term groundwater level can also be estimated based on observations of the recovered soil samples, such as the moisture level, soil consistency and colouring. Based on these observations, the long-term groundwater level is anticipated at a depth ranging between 2.5 to 3.5 m below existing grade. Groundwater levels are subject to seasonal fluctuations and could vary at the time of construction.

Table 1 - Groundwater Readings Summary

Test Hole Number	Ground Elevation (m)	Groundwater Levels (m)		Recording Date
		Depth	Elevation	
BH 1	104.37	DRY	n/a	January 29, 2019
BH 2	103.59	3.05	100.54	January 29, 2019
BH 3	103.55	1.81	101.74	January 29, 2019
BH 4	103.28	3.05	100.23	January 29, 2019
BH 5	103.45	3.05	100.40	January 29, 2019
BH 6	103.49	3.04	100.45	January 29, 2019
BH 7	103.41	DRY	n/a	January 29, 2019
BH 8	103.46	DRY	n/a	January 29, 2019
BH 9	103.42	3.17	100.25	January 29, 2019
BH 10	103.31	2.18	101.13	January 29, 2019
BH 11	103.44	DRY	n/a	January 29, 2019
BH 12	103.58	DRY	n/a	January 29, 2019
BH 13	103.55	DRY	n/a	January 29, 2019
BH 14	104.18	DRY	n/a	January 29, 2019
BH 15	103.65	2.92	100.73	January 29, 2019
BH 16	103.66	DRY	n/a	January 29, 2019
BH 17	104.19	DRY	n/a	January 29, 2019
BH 18	104.15	DRY	n/a	January 29, 2019
BH 19	103.78	DRY	n/a	January 29, 2019
BH 20	103.59	DRY	n/a	January 29, 2019
BH 21	103.58	DRY	n/a	January 29, 2019
BH 22	103.65	DRY	n/a	January 29, 2019
BH 23	103.87	2.62	101.25	January 29, 2019
BH 24	104.04	2.55	101.49	January 29, 2019
BH 25	104.07	1.68	102.39	January 29, 2019
BH 26	104.30	DRY	n/a	January 29, 2019
BH 27	103.97	DRY	n/a	January 29, 2019
BH 28	103.78	DRY	n/a	January 29, 2019
BH 29	103.71	DRY	n/a	January 29, 2019

Note: The ground surface elevation at the borehole locations was provided by Annis, O'Sullivan, Vollebekk Ltd.

5.0 Discussion

5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is suitable for the proposed development. The proposed structures will be founded on conventional shallow foundations placed on an undisturbed, hard to very stiff silty clay, compact to dense glacial till and/or clean, surface sounded bedrock bearing surface. Alternatively, conventional shallow footings can be placed over a near vertical, zero entry, concrete in-filled trenches extending to a clean, surface sounded bedrock bearing surface.

Permissible grade raise restriction areas are also required due to the silty clay deposit. A permissible grade raise restriction of **2 m** is recommended for areas where settlement sensitive structures are founded over the silty clay deposit.

Depending on the extent of the underground parking garage and potential grade raise, the bedrock may be encountered during excavation and construction. All contractors should be prepared for bedrock removal within the subject site.

Prior to considering blasting operations, if required, the blasting effects on the existing services, buildings and other structures should be addressed. A pre-blast or pre-construction survey of the existing structures located in proximity of the blasting operations should be carried out prior to commencing site activities. The extent of the survey should be determined by the blasting consultant and should be sufficient to respond to any inquiries/claims related to the blasting operations.

The blasting operations should be planned and conducted under the supervision of a licensed professional engineer who is also an experienced blasting consultant.

The above and other considerations are discussed in the following sections.

5.2 Site Grading and Preparation

Stripping Depth

Topsoil and deleterious fill, such as those containing organic materials, should be stripped from under any buildings, paved areas, pipe bedding, and other settlement sensitive structures.

Bedrock Removal

Bedrock removal can be accomplished by hoe ramming where only small quantity of the bedrock needs to be removed. Sound bedrock may be removed by line drilling and controlled blasting and/or hoe ramming.

Prior to considering blasting operations, the blasting effects on the existing services, buildings and other structures should be addressed. A pre-blast or pre-construction survey of the existing structures located in proximity of the blasting operations should be completed prior to commencing site activities. The extent of the survey should be determined by the blasting consultant and should be sufficient to respond to any inquiries/claims related to the blasting operations.

As a general guideline, peak particle velocities (measured at the structures) should not exceed 25 mm/s during the blasting program to reduce the risks of damage to the existing structures.

The blasting operations should be planned and conducted under the supervision of a licensed professional engineer who is also an experienced blasting consultant.

Excavation side slopes in sound bedrock can be excavated almost vertical side walls. A minimum 1 m horizontal ledge, should remain between the overburden excavation and the bedrock surface. The ledge will provide an area to allow for potential sloughing or a stable base for the overburden shoring system.

Vibration Considerations

Construction operations are the cause of vibrations, and possibly, sources of nuisance to the community. Therefore, means to reduce the vibration levels as much as possible should be incorporated in the construction operations to maintain, as much as possible, a cooperative environment with the residents.

The following construction equipments could be the source of vibrations: hoe ram, compactor, dozer, crane, truck traffic, etc. Vibrations, whether caused by blasting operations or by construction operations, could be the source of detrimental vibrations on the nearby buildings and structures. Therefore, all vibrations are recommended to be limited.

Two parameters are used to determine the permissible vibrations, namely, the maximum peak particle velocity and the frequency. For low frequency vibrations, the maximum allowable peak particle velocity is less than that for high frequency vibrations. As a guideline, the peak particle velocity should be less than 15 mm/s between frequencies of 4 to 12 Hz, and 50 mm/s above a frequency of 40 Hz (interpolate between 12 and 40 Hz). The guidelines are for current construction standards. Considering that these guidelines are above perceptible human level and, in some cases, could be very disturbing to some people, a pre-construction survey is recommended be completed to minimize the risks of claims during or following the construction of the proposed buildings.

Fill Placement

Fill placed for grading beneath the structure(s) or other settlement sensitive areas should consist, unless otherwise specified, of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. This material should be tested and approved prior to delivery to the site. The engineered fill should be placed in maximum 300 mm thick lifts and compacted to 98% of the material's standard Proctor maximum dry density (SPMDD).

Non-specified existing fill along with site-excavated soil can be placed as general landscaping fill where surface settlement is a minor concern. The backfill materials should be spread in thin lifts and at a minimum compacted by the tracks of the spreading equipment to minimize voids. If the non-specified backfill is to be placed to increase the subgrade level for areas to be paved, the fill should be compacted in maximum 300 mm lifts and compacted to 95% of the material's SPMDD. Non-specified existing fill and site-excavated soils are not suitable for placement as backfill against foundation walls unless a composite drainage blanket connected to a perimeter drainage system is provided.

5.3 Foundation Design

Bearing Resistance Values (Shallow Foundation)

Footings for the proposed buildings can be designed with the following bearing resistance values presented in Table 2.

Table 2 - Bearing Resistance Values

Bearing Surface	Bearing Resistance Value at SLS (kPa)	Factored Bearing Resistance Value at ULS (kPa)
Very stiff to hard silty clay	150	250
Compact to dense glacial till	200	300
Lean Concrete In-filled Trenches	**-**	**1,500**
Clean, Surface Sounded Limestone Bedrock	-	1,500

Note: Strip footings, up to 3 m wide, and pad footings, up to 8 m wide, placed over an undisturbed, silty clay bearing surface can be designed using the abovenoted bearing resistance values.
- A geotechnical resistance factor of 0.5 was applied to the above noted bearing resistance value at ULS.

The above-noted bearing resistance values at SLS for soil bearing surfaces will be subjected to potential post-construction total and differential settlements of 25 and 20 mm, respectively. Footings bearing on an acceptable bedrock bearing surface and designed for the bearing resistance values provided herein will be subjected to negligible potential post-construction total and differential settlements.

The bearing resistance values are provided on the assumption that the footings are placed on undisturbed soil bearing surfaces. An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

A clean, surface-sounded bedrock bearing surface should be free of loose materials, and have no near surface seams, voids, fissures or open joints which can be detected from surface sounding with a rock hammer.

Lean Concrete Filled Trenches

Where bedrock is encountered below the design underside of footing elevation, consideration should be given to excavating vertical trenches to expose the underlying bedrock surface and backfilling with lean concrete (**15 MPa** 28-day compressive strength). Typically, the excavation sidewalls will be used as the form to support the concrete. The additional width of the concrete poured against an undisturbed trench sidewall will suffice in providing a direct transfer of the footing load to the underlying bedrock.

The effectiveness of this operation will depend on the ability of maintaining vertical trenches until the lean concrete can be poured. It is suggested that once the bottom of the excavation is exposed, an assessment should be completed to determine the water infiltration and stability of the excavation sidewalls extending to the bedrock surface.

The trench excavation should be at least 300 mm wider than all sides of the footing at the base of the excavation. The excavation bottom should be relatively clean using the hydraulic shovel only (workers will not be permitted in the excavation below a 1.5 m depth). Once approved by the geotechnical engineer, lean concrete can be poured up to the proposed founding elevation.

Bedrock/Soil Transition

Where a building is founded partly on bedrock and partly on soil, it is recommended to decrease the soil bearing resistance value by 25% for the footings placed on soil bearing media to reduce the potential long term total and differential settlements. Also, at the soil/bedrock and bedrock/soil transitions, it is recommended that the upper 0.5 m of the bedrock be removed for a minimum length of 2 m (on the bedrock side) and replaced with nominally compacted OPSS Granular A or Granular B Type II material. The width of the sub-excavation should be at least the proposed footing width plus 0.5 m. Steel reinforcement, extending at least 3 m on both sides of the 2 m long transition, should be placed in the top part of the footings and foundation walls.

Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to an engineered fill, stiff silty clay or glacial till above the groundwater table when a plane extending horizontally and vertically from the underside of the footing at a minimum of 1.5H:1V passing through in situ soil of the same or higher bearing capacity as the bearing medium soil.

Permissible Grade Raise Restriction

Based on the current borehole information, a **permissible grade raise restriction of 2 m** is recommended for the proposed buildings and settlement sensitive structures where founded over a silty clay deposit. A post-development groundwater lowering of 0.5 m was assumed for our calculations.

5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class C** for the foundations considered at this site. However, a higher site class, such as Class A or B can be provided if a site specific shear wave velocity test is completed to confirm the seismic site classification. The soils underlying the subject site are not susceptible to liquefaction. Refer to the latest revision of the Ontario Building Code for a full discussion of the earthquake design requirements.

5.5 Basement Slab

The basement area for the proposed project will be mostly parking and the recommended pavement structure noted in Subsection 5.7 will be applicable. However, if storage or other uses of the lower level where a concrete floor slab will be constructed, the upper 200 mm of sub-slab fill is recommended to consist of 19 mm clear crushed stone. The upper 200 mm of sub-slab fill is recommended to consist of OPSS Granular A crushed stone for slab on grade construction. All backfill material within the footprint of the proposed building(s) should be placed in maximum 300 mm thick loose layers and compacted to a minimum of 98% of the SPMDD.

Any soft areas should be removed and backfilled with appropriate backfill material prior to placing any fill. OPSS Granular A or Granular B Type II, with a maximum particle size of 50 mm, are recommended for backfilling below the floor slab. All backfill material within the footprint of the proposed building(s) should be placed in maximum 300 mm thick loose layers and compacted to a minimum of 98% of the SPMDD.

A subfloor drainage system, consisting of lines of perforated drainage pipe subdrains connected to a positive outlet, should be provided in the clear stone under the lower basement floor (discussed in Subsection 6.1).

5.6 Basement Wall

There are several combinations of backfill materials and retained soils that could be applicable for the proposed structure's basement walls. However, the conditions can be well-represented by assuming the retained soil consists of a material with an angle of internal friction of 30 degrees and a dry unit weight of 20 kN/m^3.

The foundation wall is anticipated to be provided with a perimeter drainage system; therefore, the retained soils should be considered drained. For the undrained conditions, the applicable effective unit weight of the retained soil can be designed with13 kN/m^3. A hydrostatic pressure should be added to the total static earth pressure when calculating the effective unit weight. The total earth pressure (P_{AE}) includes both the static earth pressure component (P_o) and the seismic component (ΔP_{AE}).

Two distinct conditions, static and seismic, should be reviewed for design calculations. The parameters for design calculations for the two conditions are presented below.

Static Conditions

The static horizontal earth pressure (p_o) could be calculated with a triangular earth pressure distribution equal to $K_o \cdot \gamma \cdot H$ where:

K_o = at-rest earth pressure coefficient of the applicable retained soil, 0.5
γ = unit weight of fill of the applicable retained soil (kN/m^3)
H = height of the wall (m)

An additional pressure with a magnitude equal to $K_o \cdot q$ and acting on the entire height of the wall should be added to the above formula for any surcharge loading, q (kPa), that may be placed at ground surface adjacent to the wall. The surcharge pressure should only be applicable for static analyses and not be calculated in conjunction with the seismic loading case. Actual earth pressures could be higher than the "at-rest" case if care is not exercised during the compaction of the backfill materials to maintain a minimum separation of 0.3 m from the walls with the compaction equipment.

Seismic Conditions

The total seismic force (P_{AE}) includes both the earth force component (P_o) and the seismic component (ΔP_{AE}).

The seismic earth force (ΔP_{AE}) could be calculated using $0.375 \cdot a_c \cdot \gamma \cdot H^2/g$ where:

a_c = $(1.45 - a_{max}/g)a_{max}$
γ = unit weight of fill of the applicable retained soil (kN/m^3)
H = height of the wall (m)
g = gravity, 9.81 m/s^2

The peak ground acceleration, (a_{max}), for the Ottawa area is 0.32g according to OBC 2012. The vertical seismic coefficient is assumed to be zero. The earth force component (P_o) under seismic conditions could be calculated using $P_o = 0.5\ K_o\ \gamma\ H^2$, where $K_o = 0.5$ for the soil conditions presented above.

The total earth force (P_{AE}) is considered to act at a height, h (m), from the base of the wall, where:

$$h = \{P_o \cdot (H/3) + \Delta P_{AE} \cdot (0.6 \cdot H)\}/P_{AE}$$

The earth forces calculated are unfactored. For the ULS case, the earth loads should be factored as live loads, as per OBC 2012.

5.7 Pavement Structure

For design purposes, the pavement structure presented in the following tables could be used for the design of car only parking areas and access lanes, if required.

Table 3 - Recommended Flexible Pavement Structure - At-Grade Parking Areas

Thickness (mm)	Material Description
50	**Wear Course -** HL-3 or Superpave 12.5 Asphaltic Concrete
150	**BASE** - OPSS Granular A Crushed Stone
300	**SUBBASE** - OPSS Granular B Type II
	SUBGRADE - In situ soil, or OPSS Granular B Type I or II material placed over in situ soil

Table 4 - Recommended Flexible Pavement Structure - Access Lanes and Heavy Truck Parking Areas

Thickness (mm)	Material Description
40	**Wear Course -** HL-3 or Superpave 12.5 Asphaltic Concrete
50	**Binder Course -** HL-8 or Superpave 19.0 Asphaltic Concrete
150	**BASE** - OPSS Granular A Crushed Stone
450	**SUBBASE** - OPSS Granular B Type II
	SUBGRADE - In situ soil, or OPSS Granular B Type I or II material placed over in situ soil

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be sub-excavated and replaced with OPSS Granular B Type II material.

The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 98% of the SPMDD.

6.0 Design and Construction Precautions

6.1 Foundation Drainage and Backfill

Foundation Drainage

A perimeter foundation drainage system is recommended to be provided for the proposed structures. The composite drainage system (such as Miradrain G100N, Delta Drain 6000 or an approved equivalent) is recommended to extend to the footing level. Sleeves, 150 mm diameter, at 3 m centres are recommended to be placed in the footing or at the foundation wall/footing interface for blind sided pours to allow the infiltration of water to flow to the interior perimeter drainage pipe. The perimeter drainage pipe and underfloor drainage system should direct water to sump pit(s) within the lower basement area.

Underfloor Drainage

Underfloor drainage is recommend to control water infiltration for the proposed structures. For design purposes, Paterson recommends 150 mm diameter PVC, corrugated, perforated pipes be placed at 3 to 6 m centres. The spacing of the underfloor drainage system should be confirmed at the time of completing the excavation when water infiltration can be better assessed.

Adverse Effects of Dewatering on Adjacent Properties

Due to the low permeability of the subsoils profile, any minor dewatering will be considered relatively minor due to the proposed building. Therefore, adverse effects to the surrounding buildings or properties are not expected with respect to any groundwater lowering.

Foundation Backfill

Backfill against the exterior sides of the foundation walls should consist of free-draining non frost susceptible granular materials. The greater part of the site excavated materials will be frost susceptible and, as such, are not recommended for re-use as backfill against the foundation walls where frost heave sensitive structures, such as a concrete sidewalk, will be placed. Imported granular materials, such as clean sand or OPSS Granular B Type I granular material may be used for this purpose. A composite drainage system, such as Delta Drain 6000, Miradrain G100 or an approved equivalent, should be placed against the foundation wall to promote drainage toward the perimeter drainage pipe.

6.2 Protection of Footings Against Frost Action

Perimeter footings of heated structures are recommended to be protected against the deleterious effects of frost action. A minimum of 1.5 m of soil cover alone, or a combination of soil cover and foundation insulation should be provided.

Exterior unheated footings, such as isolated exterior piers, are more prone to deleterious movement associated with frost action than the exterior walls of the structure proper and require additional protection, such as soil cover of 2.1 m or a combination of soil cover and foundation insulation.

The parking garage should not require protection against frost action due to the founding depth. Unheated structures, such as the access ramp wall footings, may be required to be insulated against the deleterious effect of frost action. A minimum of 2.1 m of soil cover alone, or a minimum of 0.6 m of soil cover, in conjunction with foundation insulation, should be provided.

6.3 Excavation Side Slopes

Temporary Side Slopes

The temporary excavation side slopes should either be excavated to acceptable slopes or retained by shoring systems from the beginning of the excavation until the structure is backfilled.

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be excavated at 1H:1V or shallower. The shallower slope is required for excavation below groundwater level. The subsurface soil is considered to be mainly Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should maintain safe working distance from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

A trench box is recommended to be installed at all times to protect personnel working in trenches with steep or vertical sides. Services are expected to be installed by “cut and cover” methods and excavations should not be remain exposed for extended periods of time.

Temporary Shoring

Temporary shoring may be required for the overburden soil to complete the required excavations where insufficient room is available for open cut methods. The shoring requirements designed by a structural engineer specializing in those works will depend on the excavation depths, the proximity of the adjacent structures and the elevation of the adjacent building foundations and underground services. The design and implementation of these temporary systems will be the responsibility of the excavation contractor and their design team. Inspections and approval of the temporary system will also be the responsibility of the designer. Geotechnical information provided below is to assist the designer in completing a suitable and safe shoring system. The designer should take into account the impact of a significant precipitation event and designate design measures to ensure that a precipitation will not negatively impact the shoring system or soils supported by the system. Any changes to the approved shoring design system should be reported immediately to the owner's structural designer prior to implementation.

The temporary system could consist of soldier pile and lagging system or interlocking steel sheet piling. Any additional loading due to street traffic, construction equipment, adjacent structures and facilities, etc., should be included to the earth pressures described below. These systems could be cantilevered, anchored or braced. Generally, it is expected that the shoring systems will be provided with tie-back rock anchors to ensure their stability. The shoring system is recommended to be adequately supported to resist toe failure and inspected to ensure that the sheet piles extend well below the excavation base. It should be noted if consideration is being given to utilizing a raker style support for the shoring system that lateral movements can occur and the structural engineer should ensure that the design selected minimizes these movements to tolerable levels.

The earth pressures acting on the shoring system may be calculated with the following parameters.

Table 6 - Soil Parameters

Parameters	Values
Active Earth Pressure Coefficient (K_a)	0.33
Passive Earth Pressure Coefficient (K_p)	3
At-Rest Earth Pressure Coefficient (K_o)	0.5
Dry Unit Weight (γ), kN/m^3	20
Effective Unit Weight (γ), kN/m^3	13

The active earth pressure should be calculated where wall movements are permissible while the at-rest pressure should be calculated if no movement is permissible. The dry unit weight should be calculated above the groundwater level while the effective unit weight should be calculated below the groundwater level.

The hydrostatic groundwater pressure should be included to the earth pressure distribution wherever the effective unit weight are calculated for earth pressures. If the groundwater level is lowered, the dry unit weight for the soil/bedrock should be calculated full weight, with no hydrostatic groundwater pressure component.

For design purposes, the minimum factor of safety of 1.5 should be calculated.

6.4 Pipe Bedding and Backfill

Bedding and backfill materials should be in accordance with the most recent Material Specifications and Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Services Branch of the City of Ottawa.

A minimum of a 150 mm layer of OPSS Granular A crushed stone should be placed for pipe bedding for sewer and water pipes for a soil subgrade. The bedding thickness should be increased to 300 mm for areas where the subgrade consists of bedrock. The bedding should extend to the spring line of the pipe. Cover material, from the spring line to at least 300 mm above the obvert of the pipe should consist of OPSS Granular A. The bedding and cover materials should be placed in maximum 300 mm thick lifts compacted to a minimum of 95% of the SPMDD.

The site excavated material may be placed above cover material if the excavation operations are completed in dry weather conditions and the site excavated material is approved by the geotechnical consultant. All cobbles greater than 200 mm in the longest dimension should be removed prior to the site materials being reused.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to reduce differential frost heaving. The trench backfill should be placed in maximum 225 mm thick loose lifts and compacted to a minimum of 95% of the SPMDD. Within the frost zone (1.8 m below finished grade), non frost susceptible materials should be used when backfilling trenches below the original bedrock level.

Clay seals are recommended for the subject site. The seals should be a minimum of 1.5 m long (in the trench direction) and should extend from trench wall to trench wall. Generally, the seals should extend from the frost line and fully penetrate the bedding, subbedding and cover material. The barriers should consist of relatively dry and compactable brown silty clay placed in maximum 225 mm thick loose layers and compacted to a minimum of 95% of the SPMDD. The clay seals should be placed at the site boundaries, roadway intersections and at a maximum distance of every 50 m in the service trenches.

6.5 Groundwater Control

Groundwater Control for Building Construction

It is anticipated that groundwater infiltration into the excavations should be low and controllable using open sumps. Pumping from open sumps should be sufficient to control the groundwater influx through the sides of shallow excavations. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

A temporary Ministry of the Environment, Conservation and Parks (MECP) permit to take water (PTTW) may be required for this project if more than 400,000 L/day of ground and/or surface water is to be pumped during the construction phase. A minimum 4 to 5 months should be allowed for completion of the PTTW application package and issuance of the permit by the MECP.

For typical ground or surface water volumes, being pumped during the construction phase, between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16. If a project qualifies for a PTTW based upon anticipated conditions, an EASR will not be allowed as a temporary dewatering measure while awaiting the MECP review of the PTTW application.

6.6 Winter Construction

Precautions must be provided if winter construction is considered for this project. Where excavations are completed in proximity of existing structures which may be adversely affected due to the freezing conditions. In particular, where a shoring system is constructed, the soil behind the shoring system will be subjected to freezing conditions and could result in heaving of the structure(s) placed within or above frozen soil. Provisions in the contract documents should be provided to protect the excavation walls from freezing, if applicable.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the installation of straw, propane heaters and tarpaulins or other suitable means. The excavation base should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be considered if such activities are to be completed during freezing conditions. Additional information could be provided, if required.

6.7 Corrosion Potential and Sulphate

The results on analytical testing show that the sulphate content is less than 0.1%. The results are indicative that Type 10 Portland Cement (Type GU) would be appropriate for this site. The chloride content and the pH of the sample indicate that they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity in indicative of a low to moderate corrosive environment.

6.8 Limit of Hazard Lands

Field Observations

Paterson conducted a site visit on January 13, 2019 to review the slope located along the west boundary of the subject site, assess the current slope conditions and confirm the grades provided in the existing topographic mapping. A section of Poole Creek is located within the west portion of the site and shown in Drawing PG4772-1 - Test Hole Location Plan.

Three (3) slope cross-sections were reviewed in the field as the worst case scenarios. The cross section locations are presented on Drawing PG4772-1 - Test Hole Location Plan in Appendix 2. Generally, the riverbanks along both sides of Poole Creek are currently well vegetated and were observed in an acceptable condition. Poole Creek was observed within a 20 to 40 m wide flood plain. The slope along the east side of Poole Creek ranged in height between 3 and 5 m with an inclination ranging between 2.3H:1V and 3.3H:1V. The upper slope was observed to be well vegetated with little to no signs of active surficial erosion.

Slope Stability Analysis

Limit of Hazard Lands

The slope condition was reviewed based on available topographic mapping along the east side slopes of Poole Creek within the west portion of the subject development. A total of 3 slope cross-sections were assessed as the worst case scenarios. The cross section locations are presented on Drawing PG4772-1 - Test Hole Location Plan in Appendix 2.

A slope stability assessment was carried out to determine the required stable slope allowance setback from the top of slope based on a factor of safety of 1.5. A toe erosion and 6 m erosion access allowances were also included in the determination of limits of hazard lands and are discussed below. The proposed limit of hazard lands (as shown on Drawing PG4772-1 - Test Hole Location Plan) includes:

- ❑ a geotechnical slope stability allowance with a factor of safety of 1.5
- ❑ a toe erosion allowance
- ❑ a 6 m erosion access allowance and top of slope

Slope Stability Analysis

The analysis of the stability of the slope sections was carried out using SLIDE, a computer program which permits a two-dimensional slope stability analysis using several methods including the Bishop's method, which is a widely used and accepted analysis method. The program calculates a factor of safety, which represents the ratio of the forces resisting failure to those favoring failure. Theoretically, a factor of safety of 1.0 represents a condition where the slope is stable. However, due to intrinsic limitations of the calculation methods and the variability of the subsoil and groundwater conditions, a factor of safety greater than one is usually required to ascertain than the risks of failure are acceptable. A minimum factor of safety of 1.5 is generally recommended for conditions where the failure of the slope would endanger permanent structures.

An analysis considering seismic loading was also completed. A horizontal acceleration of 0.16G was considered for the sections for the seismic loading condition. A factor of safety of 1.1 is considered to be satisfactory for stability analyses including seismic loading.

The cross-sections were analysed taking into account a groundwater level at ground surface, which represents a worse-case scenario that can be reasonably expected to occur in cohesive soils. The stability analysis assumes full saturation of the soil with groundwater flow parallel to the slope face. Subsoil conditions at the cross-sections were inferred based on the findings at borehole locations along the top of slope and general knowledge of the area's geology.

Stable Slope Allowance

The results of the stability analysis for static conditions at Sections A through C are presented in Figures 2A to 4A in Appendix 2. All the reviewed slope sections along the subject creek were noted to be shaped to at least a 2.3H:1V. Based on the soil conditions observed and the results of the slope stability analysis, the slope stability factor of safety was calculated to be 1.5 or greater for all the slope sections which indicates that a stable slope allowance is not required for the subject slope.

The results of the analyses including seismic loading are shown in Figures 2B to 4B for the slope sections. The results indicate that the factor of safety for the sections are greater than 1.1.

It should be noted that the existing vegetation on the slope face should not be removed as it contributes to the stability of the slope and reduces erosion. If the existing vegetation needs to be removed, it is recommended that a 100 to 150 mm of topsoil mixed with a hardy seed and/or topped with an erosion control blanket be which can be placed across the exposed slope face.

Toe Erosion and Erosion Access Allowance

The toe erosion allowance for the valley corridor wall slope was based on the cohesive nature of the top layers of the subsoils, the observed current erosional activities and the width and location of the current watercourse. It should be noted that if the flood plain is measured to be greater than 20 m, no toe erosion will be required. Therefore, based on the above factors, no toe erosion allowance is considered for the subject slope.

An erosion access allowance of 6 m is required from the top of slope to ensure access is provided should future maintenance to the slope face is required. The limit of hazard lands, which includes these allowances, is indicated on Drawing PG4772-1 - Test Hole Location Plan in Appendix 2.

6.9 Landscaping Considerations

Tree Planting Restrictions

According to the City of Ottawa Guidelines for tree planting, where a sensitive silty clay deposit is present within the vicinity of the site, tree planting restrictions should be determined. However, for this site, based on the founding medium of the underground parking level which will occupy the majority of the site, tree planting restrictions are not required from a geotechnical perspective.

7.0 Recommendations

A materials testing and observation services program is a requirement for the provided foundation design data to be applicable. The following aspects of the program should be performed by the geotechnical consultant:

- ❑ Review detailed grading plan(s) from a geotechnical perspective.
- ❑ Review groundwater conditions at the time of construction to determine if waterproofing is required.
- ❑ Observation of all bearing surfaces prior to the placement of concrete.
- ❑ Sampling and testing of the concrete and fill materials used.
- ❑ Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- ❑ Observation of all subgrades prior to backfilling.
- ❑ Field density tests to determine the level of compaction achieved.
- ❑ Sampling and testing of the bituminous concrete including mix design reviews.

A report confirming that the construction work has been conducted in general accordance with the above recommendations could be issued, upon request, following the completion of a satisfactory materials testing and observation program by the geotechnical consultant.

8.0 Statement of Limitations

The recommendations provided in the report are in accordance with Paterson's present understanding of the project. Paterson request permission to review the recommendations when the drawings and specifications are completed.

A geotechnical investigation is a limited sampling of a site. Should any conditions encountered during construction differ from the borehole locations, Paterson requests immediate notification to permit reassessment of the recommendations provided herein.

The recommendations provided should only be used by the design professionals associated with this project. The recommendations are not intended for contractors bidding on or constructing the project. The latter should evaluate the factual information provided in the report. The contractor should also determine the suitability and completeness for the intended construction schedule and methods. Additional testing may be required for the contractors purpose.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Nautical Lands Group or their agent(s) is not authorized without review by Paterson for the applicability of our recommendations to the altered use of the report.

Paterson Group Inc.

Faisal I. Abou-Seido, P.Eng.

David J. Gilbert, P.Eng.

Report Distribution:

- ❏ Nautical Lands Group (3 copies)
- ❏ Paterson Group (1 copy)

APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

SYMBOLS AND TERMS

ANALYTICAL TESTING RESULTS

patersongroup Consulting Engineers

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

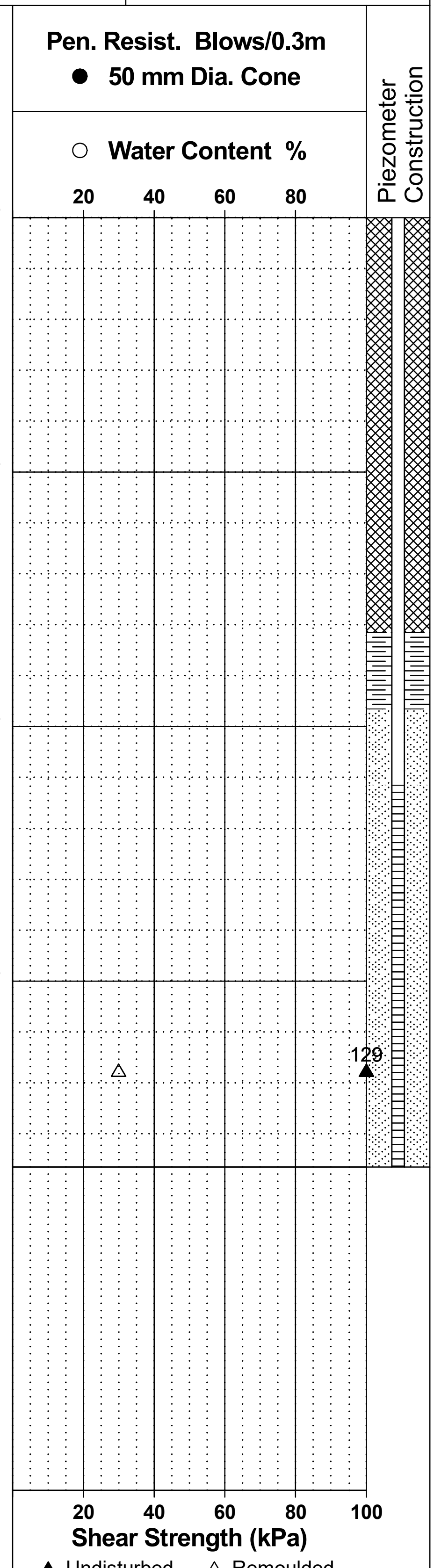
FILE NO. PG4772

REMARKS

HOLE NO. BH 1

BORINGS BY CME 55 Power Auger **DATE** 2019 January 14

SOIL DESCRIPTION	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	104.37
FILL: Compact brown silty sand, some gravel	AU	1				
	SS	2	38	15	1	103.37
1.52						
Very stiff, brown **SILTY CLAY,** trace gravel	SS	3	42	7	2	102.37
	SS	4	58	4		
					3	101.37
3.73						
End of Borehole Practical refusal to augering at 3.73m depth (BH dry - Jan 29/19)						



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Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

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FILE NO. **PG4772**

REMARKS

HOLE NO. **BH 2**

BORINGS BY CME 55 Power Auger

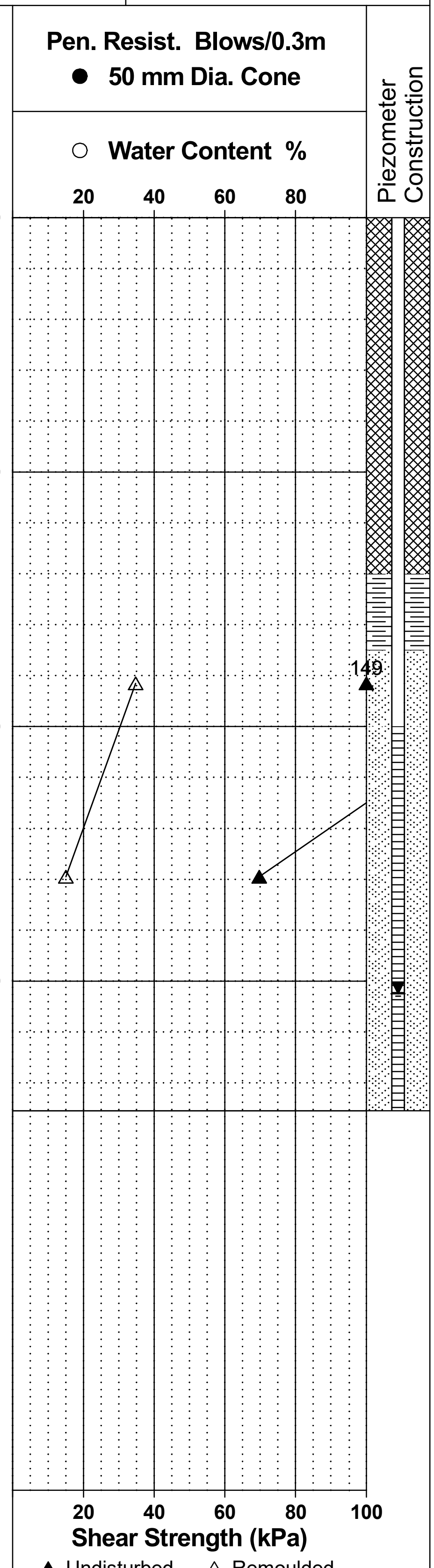
DATE 2019 January 14

SOIL DESCRIPTION	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.59
FILL: Brown silty sand, some gravel	AU	1				
0.66						
Very stiff to stiff, brown **SILTY CLAY**	SS	2	33	4	1	102.59
					2	101.59
- grey and trace gravel by 3.0m depth					3	100.59
	SS	3		50+		
3.51						

End of Borehole

Practical refusal to augering at 3.51m depth

(GWL @ 3.05m depth - Jan 29/19)



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Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
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FILE NO. PG4772

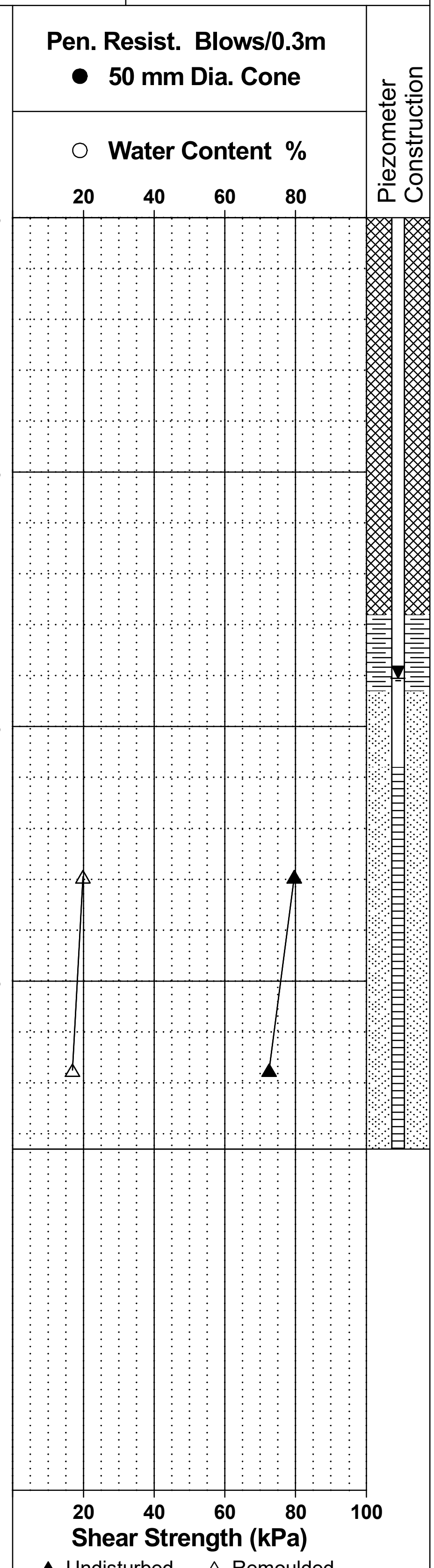
REMARKS

BORINGS BY CME 55 Power Auger

DATE 2019 January 14

HOLE NO. BH 3

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.55
TOPSOIL		AU	1				
0.33							
		SS	2	21	7	1	102.55
Very stiff to stiff, brown **SILTY CLAY**		SS	3	62	7		
						2	101.55
- grey by 2.3m depth							
						3	100.55
3.66							
End of Borehole							
Practical refusal to augering at 3.66m depth							
(GWL @ 1.81m depth - Jan 29/19)							



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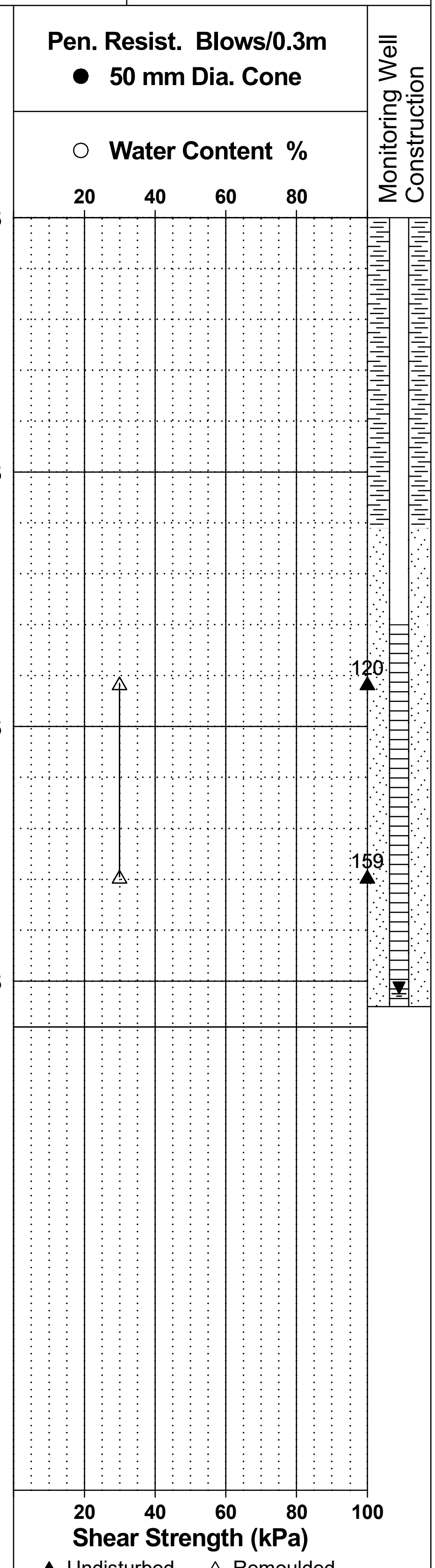
FILE NO. **PG4772**

REMARKS

HOLE NO. **BH 4**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 14

SOIL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.28
TOPSOIL (to 0.30)	AU	1				
Very stiff, brown **SILTY CLAY**	SS	2	25	6	1	102.28
- grey by 2.4m depth					2	101.28
- trace sand and gravel by 3.0m depth					3	100.28
(to 3.18)	SS	3	100	50+		
End of Borehole						
Practical refusal to augering at 3.18m depth						
(GWL @ 3.05m depth - Jan 29/19)						



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REMARKS

BORINGS BY CME 55 Power Auger **DATE** 2019 January 14

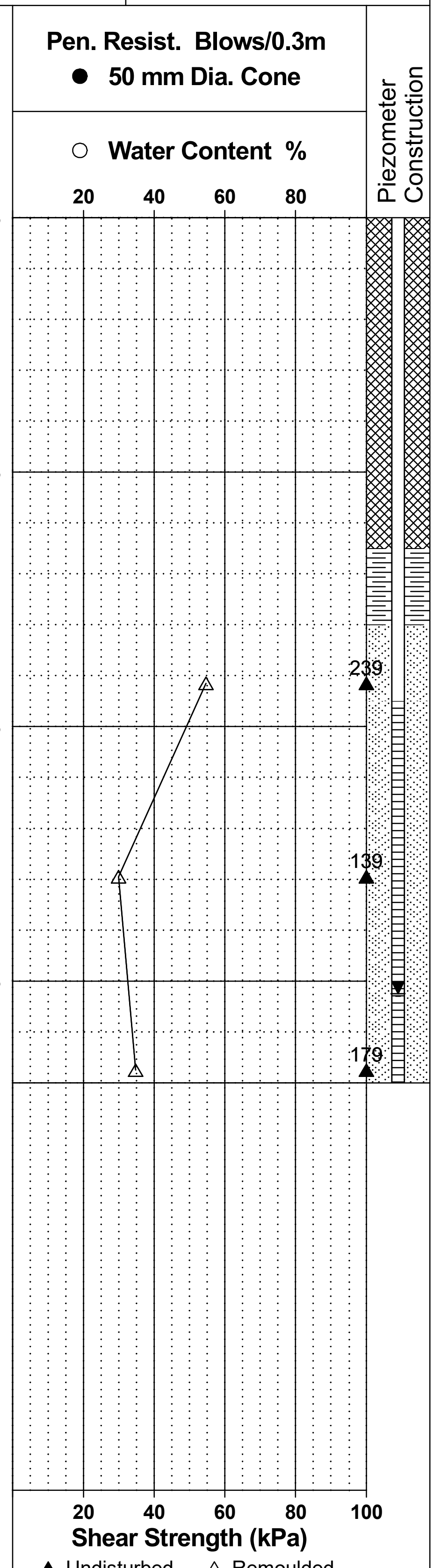
FILE NO. PG4772

HOLE NO. BH 5

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.45
TOPSOIL (to 0.36)		AU	1				
Hard to very stiff, brown **SILTY CLAY**		SS	2	38	6	1	102.45
- grey by 2.1m depth (to 3.40)						2	101.45
						3	100.45
End of Borehole							

Practical refusal to augering at 3.40m depth

(GWL @ 3.05m depth - Jan 29/19)



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Ottawa, Ontario

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FILE NO. PG4772

REMARKS

HOLE NO. BH 6

BORINGS BY CME 55 Power Auger

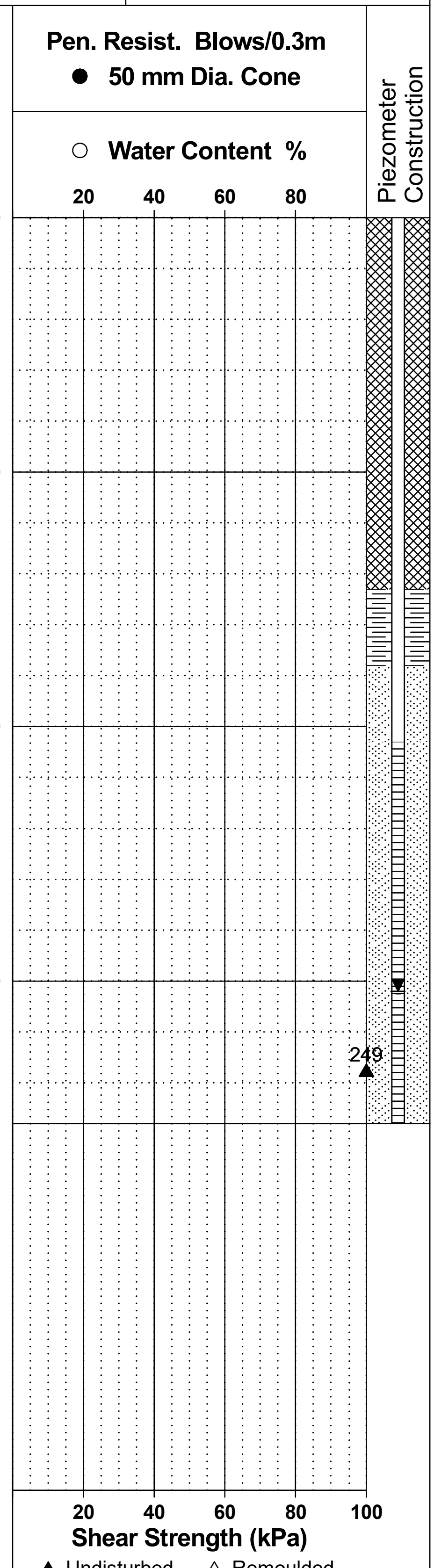
DATE 2019 January 14

SOIL DESCRIPTION	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.49
TOPSOIL (to 0.38)	AU	1				
Very stiff, brown **SILTY CLAY**	SS	2	58	8	1	102.49
	SS	3	71	9	2	101.49
- grey by 2.0m depth	SS	4	100	5	3	100.49
(to 3.56)						

End of Borehole

Practical refusal to augering at 3.56m depth

(GWL @ 3.04m depth - Jan 29/19)



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REMARKS

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DATE 2019 January 14

FILE NO. PG4772

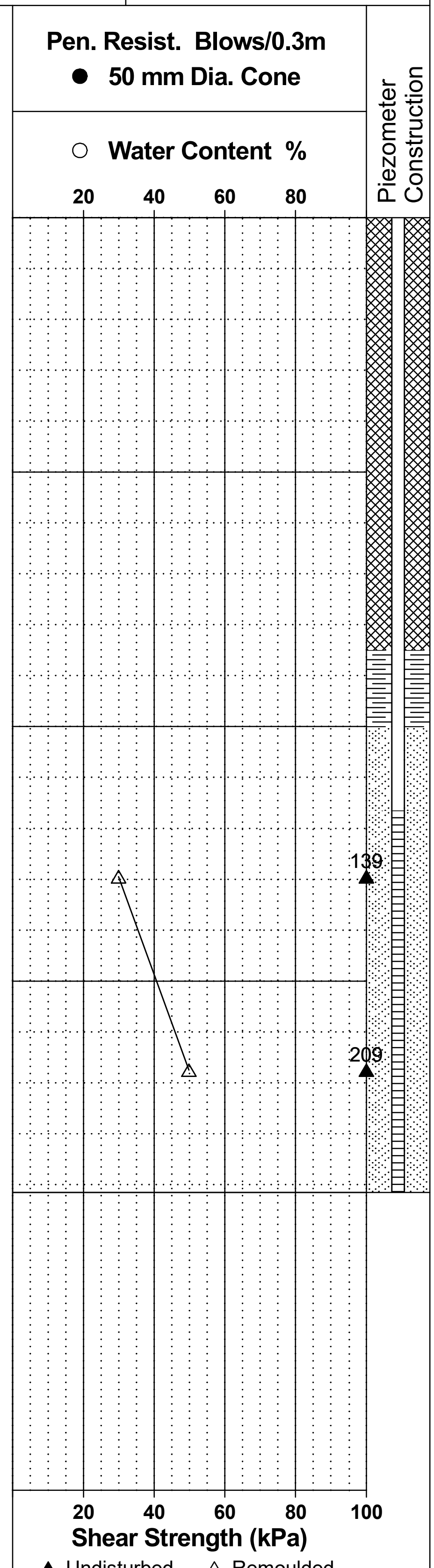
HOLE NO. BH 7

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.41
TOPSOIL (0.43)		AU	1				
Very stiff to hard, brown **SILTY CLAY**		SS	2	58	7	1	102.41
- grey by 1.8m depth		SS	3	92	6	2	101.41
(3.83)						3	100.41

End of Borehole

Practical refusal to augering at 3.83m depth

(BH dry - Jan 29/19)



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REMARKS

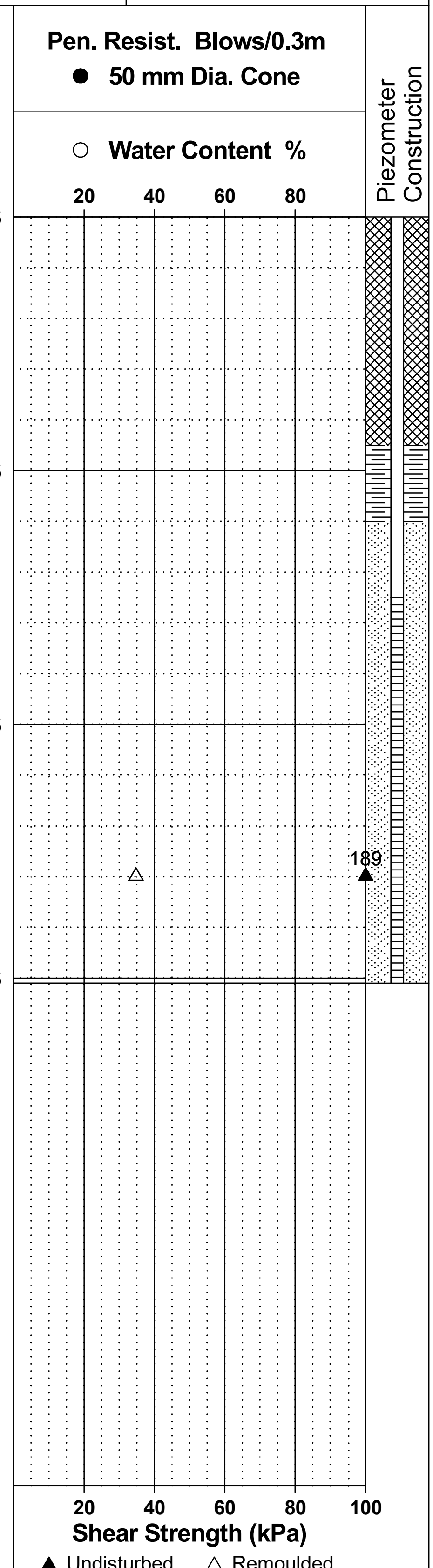
BORINGS BY CME 55 Power Auger

DATE 2019 January 14

FILE NO. PG4772

HOLE NO. BH 8

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.46
TOPSOIL 0.41		AU	1				
Very stiff, brown **SILTY CLAY**		SS	2	67	7	1	102.46
- grey by 2.0m depth		SS	3	92	6	2	101.46
3.02						3	100.46
End of Borehole Practical refusal to augering at 3.02m depth (BH Dry - Jan 29/19)							



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FILE NO. PG4772

REMARKS

HOLE NO. BH 9

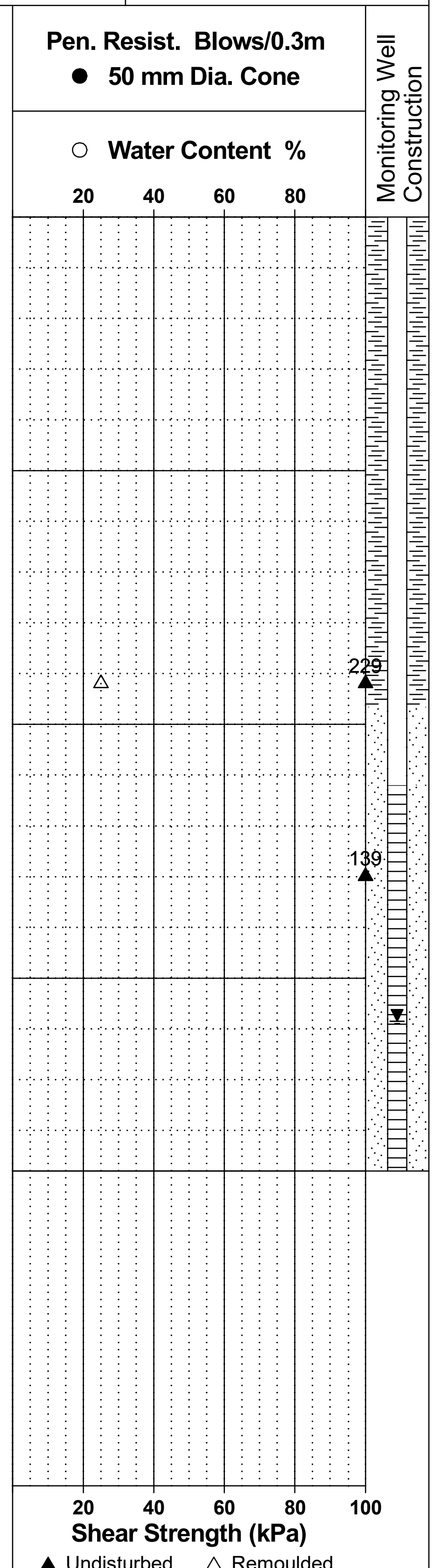
BORINGS BY CME 55 Power Auger **DATE** 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.42
TOPSOIL 0.38		AU	1				
		SS	2	71	4	1	102.42
Hard to very stiff, brown **SILTY CLAY**						2	101.42
						3	100.42
3.76		SS	3	71	14		

End of Borehole

Practical refusal to augering at 3.76m depth

(GWL @ 3.17 m depth - Jan 29/19)



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Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH10**

BORINGS BY CME 55 Power Auger

DATE 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.31
TOPSOIL (to 0.41)		AU	1				
Very stiff, brown **SILTY CLAY**		SS	2	67	9	1	102.31
		SS	3	75	6	2	101.31
- grey by 2.1m depth (to 3.05)						3	100.31
GLACIAL TILL: Compact, brown sandy silt, trace clay and gravel, occasional cobbles and boulders (to 3.66)		SS	4	83	19		

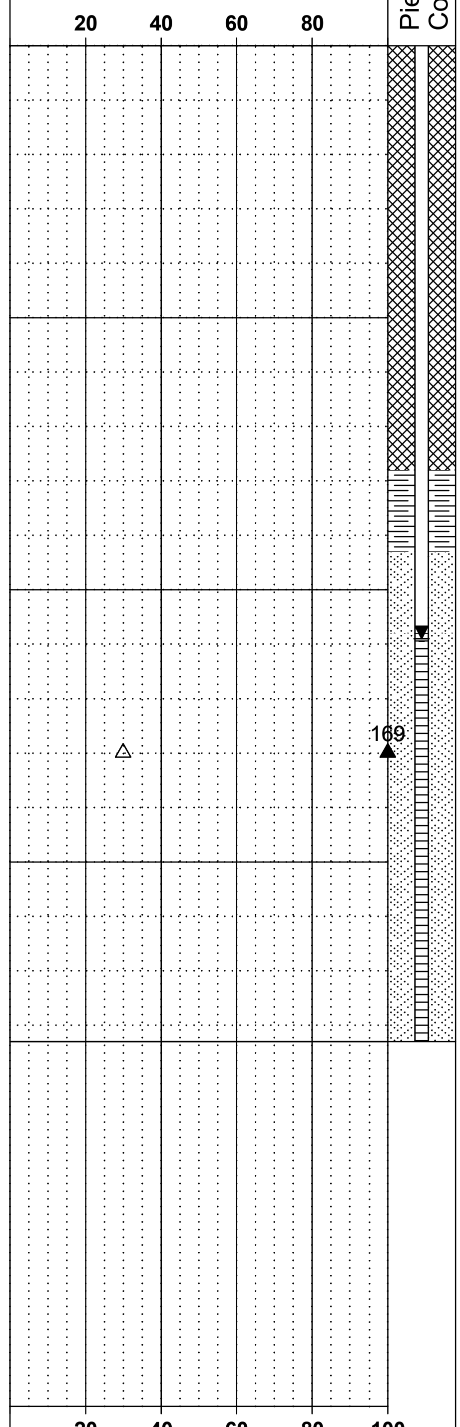
End of Borehole

Practical refusal to augering at 3.66m depth

(GWL @ 2.18m depth - Jan 29/19)

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone

○ Water Content %



Shear Strength (kPa)
▲ Undisturbed △ Remoulded

Piezometer Construction

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SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

HOLE NO. BH11

BORINGS BY CME 55 Power Auger

DATE 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.44
TOPSOIL (to 0.38)		AU	1				
Very stiff, brown **SILTY CLAY** (0.38 to 3.05)		SS	2	71	4	1	102.44
						2	101.44
GLACIAL TILL: Very dense brown to grey sandy silt, trace clay and gravel, occasional cobbles and boulders (3.05 to 3.35)		SS	3	100	50+	3	100.44
End of Borehole							
Practical refusal to augering at 3.35m depth							
(BH Dry - Jan 29/19)							

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone

○ Water Content %

20 40 60 80

Piezometer Construction

Shear strength (undisturbed): 179, 249

20 40 60 80 100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

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DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

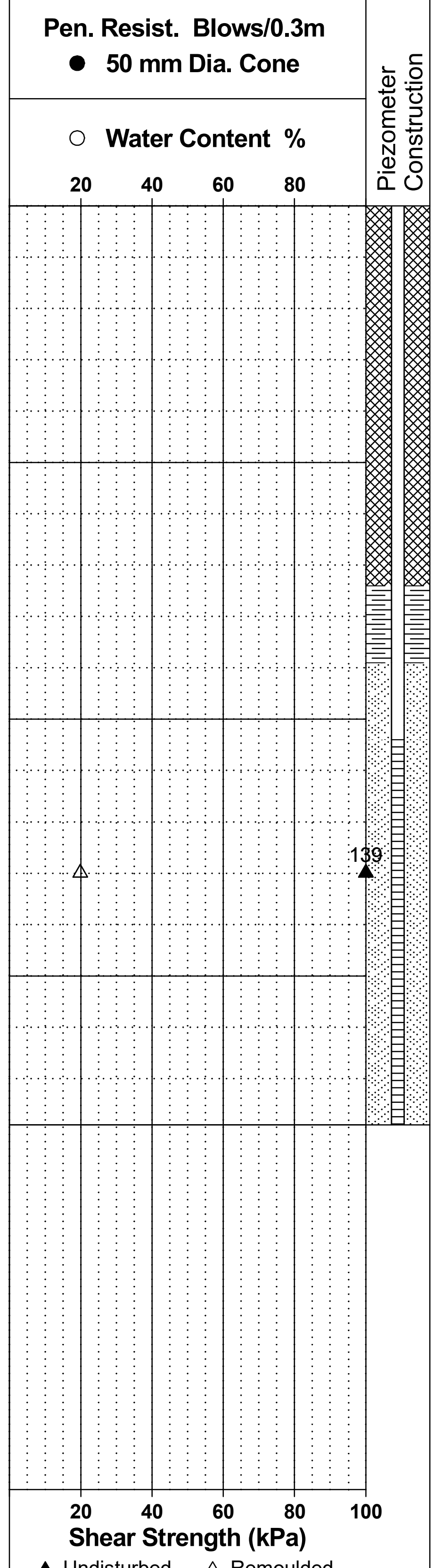
HOLE NO. BH12

BORINGS BY CME 55 Power Auger **DATE** 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.58
TOPSOIL (to 0.38)		AU	1				
Very stiff, brown **SILTY CLAY** (0.38 to 3.05)		SS	2	88	6	1	102.58
		SS	3	96	5	2	101.58
						3	100.58
GLACIAL TILL: Compact, brown to grey clayey silt, some sand, trace gravel, occasional cobbles and boulders (3.05 to 3.58)		SS	4	90	11		
End of Borehole							

Practical refusal to augering at 3.58m depth

(BH Dry - Jan 29/19)



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FILE NO. PG4772

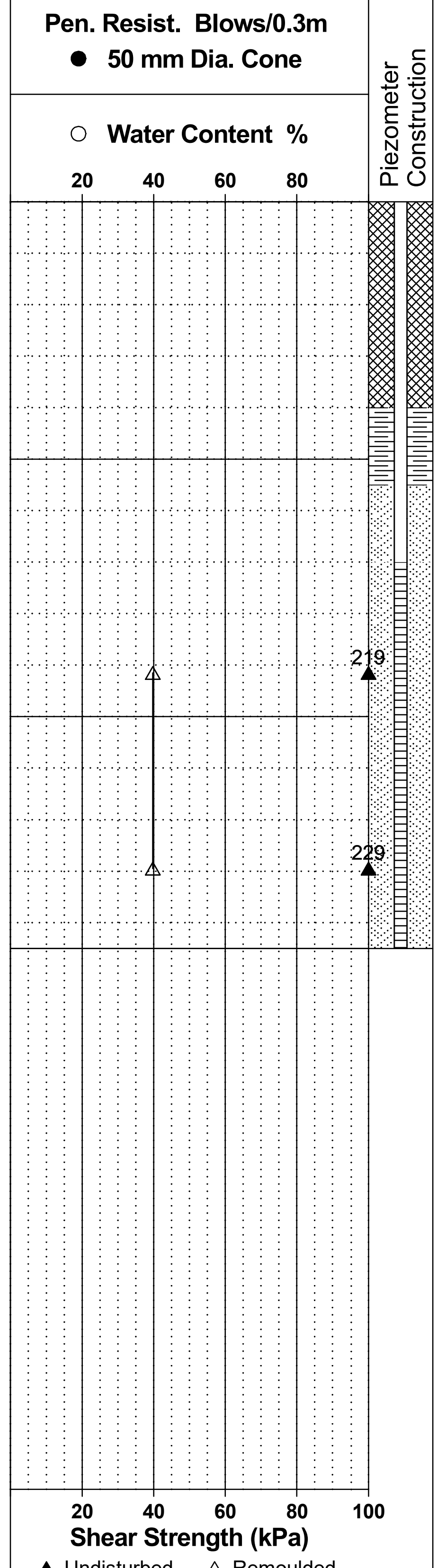
REMARKS

HOLE NO. BH13

BORINGS BY CME 55 Power Auger

DATE 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.55
TOPSOIL 0.36		AU	1				
Hard, brown **SILTY CLAY**		SS	2	88	4	1	102.55
						2	101.55
2.90							
End of Borehole Practical refusal to augering at 2.90m depth (BH Dry - Jan 29/19)							



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FILE NO. PG4772

REMARKS

HOLE NO. BH14

BORINGS BY CME 55 Power Auger

DATE 2019 January 15

SOIL DESCRIPTION	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	104.18
TOPSOIL (to 0.41)	AU	1				
Very stiff, brown **SILTY CLAY**	SS	2	67	7	1	103.18
- grey by 2.0m depth (to 2.29)	SS	3	96	6	2	102.18
GLACIAL TILL: Grey silty clay, trace sand and gravel, occasional cobbles and boulders (to 3.00)					3	101.18

End of Borehole

Practical refusal to augering at 3.00m depth

(BH Dry - Jan 29/19)

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone
○ Water Content %
20 40 60 80

Piezometer Construction

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

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REMARKS

BORINGS BY CME 55 Power Auger

DATE 2019 January 15

FILE NO. PG4772

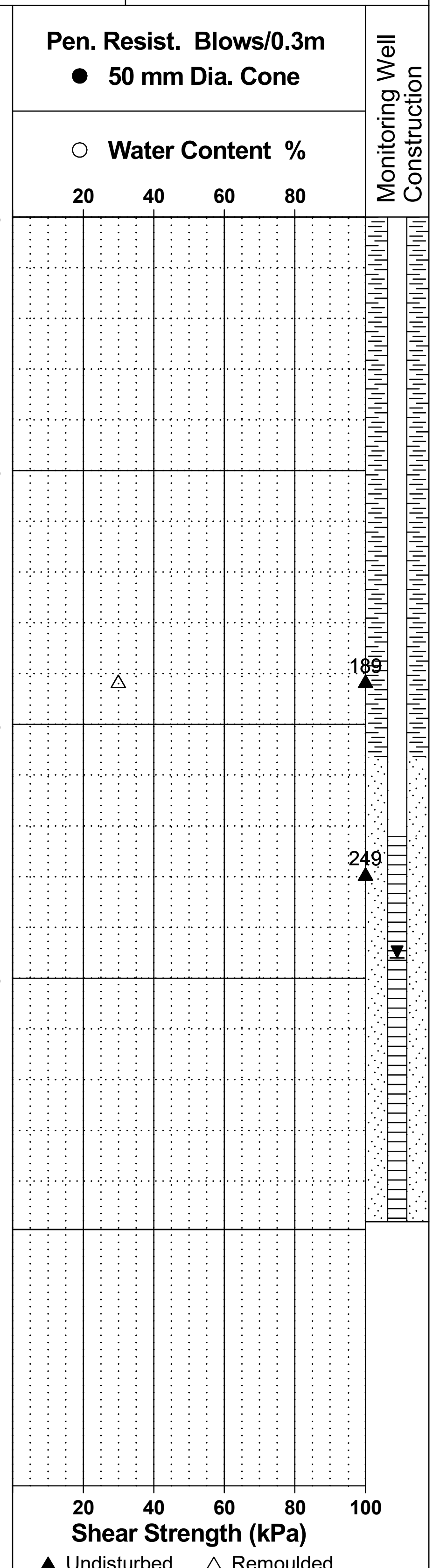
HOLE NO. BH15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.65
TOPSOIL (to 0.36)		AU	1				
Very stiff, brown **SILTY CLAY**		SS	2	71	6	1	102.65
(to 2.29)						2	101.65
Hard, brown **CLAYEY SILT** (to 3.05)						3	100.65
GLACIAL TILL: Compact to very dense, grey clayey silt, some sand, trace gravel, occasional cobbles and boulders		SS	3	79	24		
(to 3.99)		SS	4	100	50+		

End of Borehole

Practical refusal to augering at 3.99m depth

(GWL @ 2.92m depth - Jan 29/19)



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FILE NO. **PG4772**

REMARKS

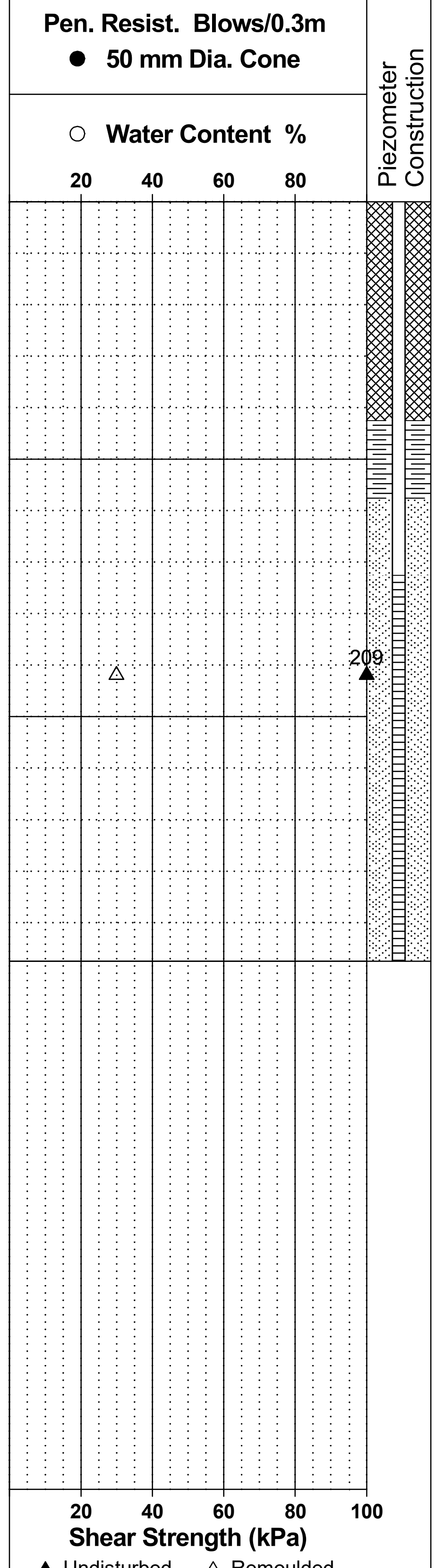
HOLE NO. **BH16**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 15

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.66
TOPSOIL (to 0.33)		AU	1				
Hard, brown **SILTY CLAY** (0.33 to 2.29)		SS	2	75	4	1	102.66
						2	101.66
GLACIAL TILL: Dense, brown to grey clayey silt, some sand, gravel, cobbles and boulders (2.29 to 2.95)		SS	3	46	31		
End of Borehole (2.95)							

Practical refusal to augering at 2.95m depth

(BH Dry - Jan 29/19)



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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

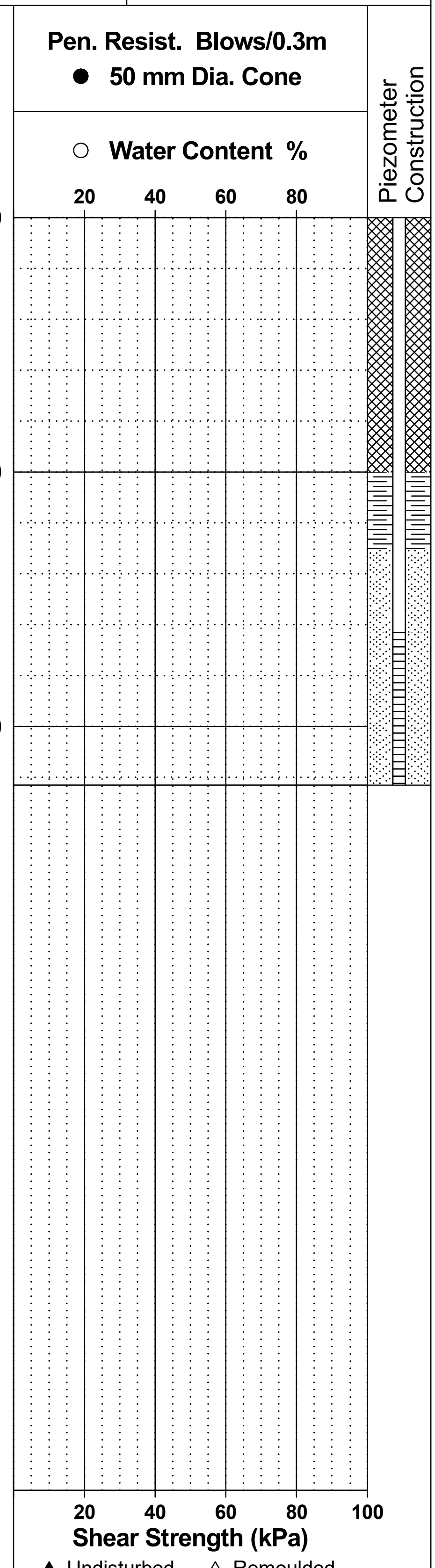
FILE NO. PG4772

REMARKS

HOLE NO. BH17

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	104.19
TOPSOIL 0.38		AU	1				
Very stiff to hard, brown **CLAYEY SILT**		SS	2	79	7	1	103.19
- grey by 1.8m depth 2.23		SS	3	100	55	2	102.19
End of Borehole Practical refusal to augering at 2.23m depth (BH Dry - Jan 29/19)							



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SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH18**

BORINGS BY CME 55 Power Auger

DATE 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	104.15
TOPSOIL 0.33		AU	1				
Hard, brown **CLAYEY SILT**		SS	2	88	11	1	103.15
- grey by 1.8m depth 1.96		SS	3	88	50+		
End of Borehole							

Practical refusal to augering at 1.96m depth

(BH Dry - Jan 29/19)

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone
○ Water Content %
20 40 60 80

Piezometer Construction

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

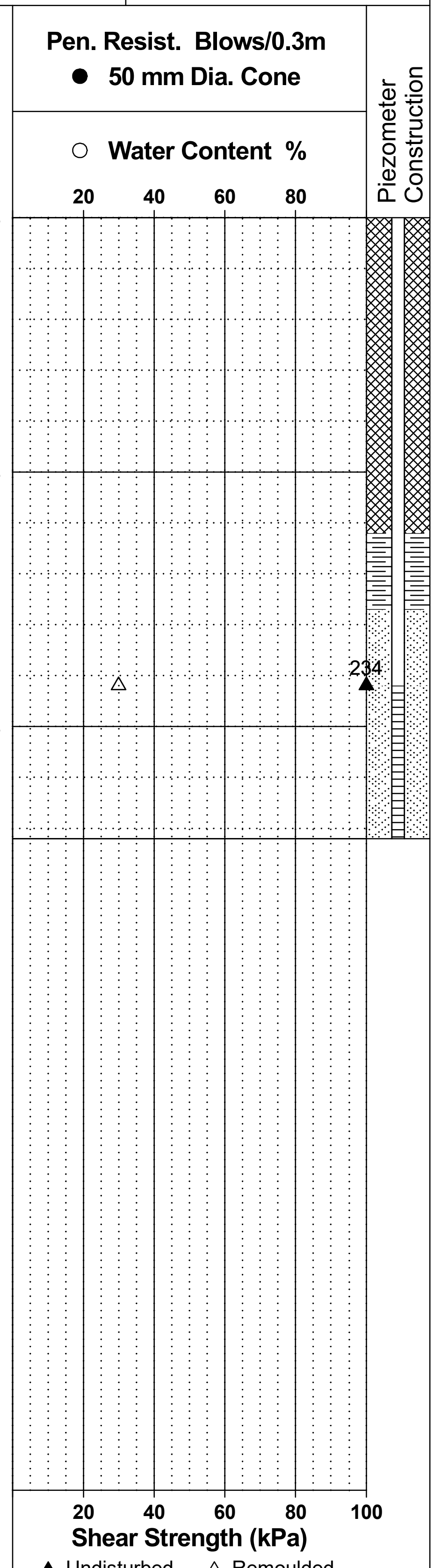
FILE NO. **PG4772**

REMARKS

HOLE NO. **BH19**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.78
TOPSOIL (to 0.36)		AU	1				
Hard, brown to grey **SILTY CLAY**		SS	2	88	3	1	102.78
						2	101.78
(to 2.44)		SS	3	100	50+		
End of Borehole Practical refusal to augering at 2.44m depth (BH Dry - Jan 29/19)							



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SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

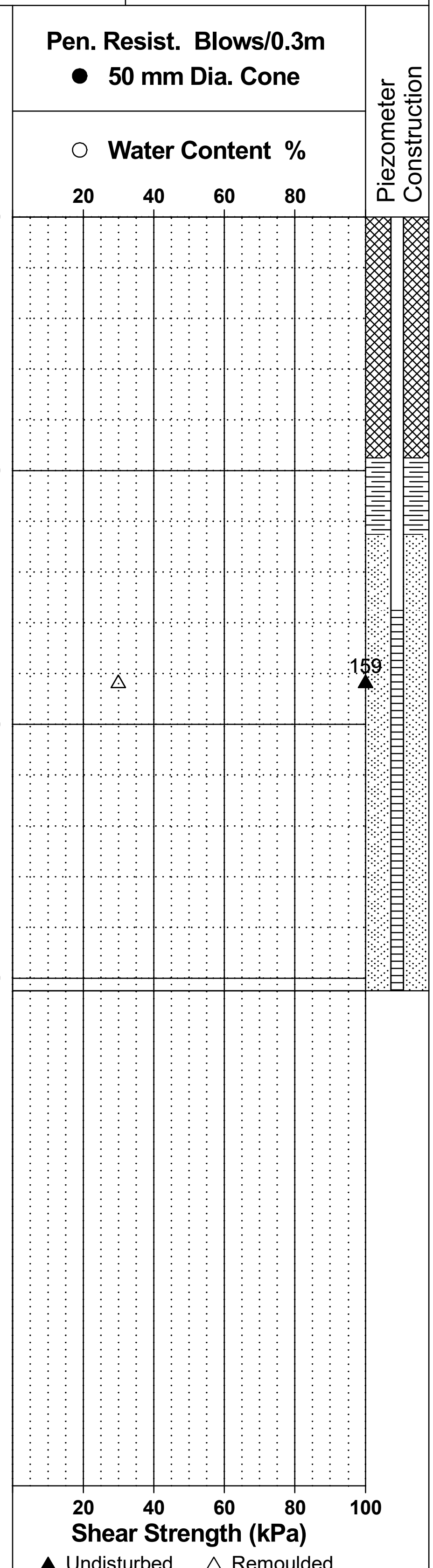
FILE NO. PG4772

REMARKS

HOLE NO. BH20

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.59
TOPSOIL 0.33		AU	1				
Very stiff, brown **SILTY CLAY**		SS	2	83	4	1	102.59
- grey by 1.8m depth 2.30						2	101.59
Loose, grey **CLAYEY SILT,** trace sand and gravel 3.05		SS	3	83	9	3	100.59
End of Borehole							
Practical refusal to augering at 3.05m depth							
(BH Dry - Jan 29/19)							



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SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

HOLE NO. BH21

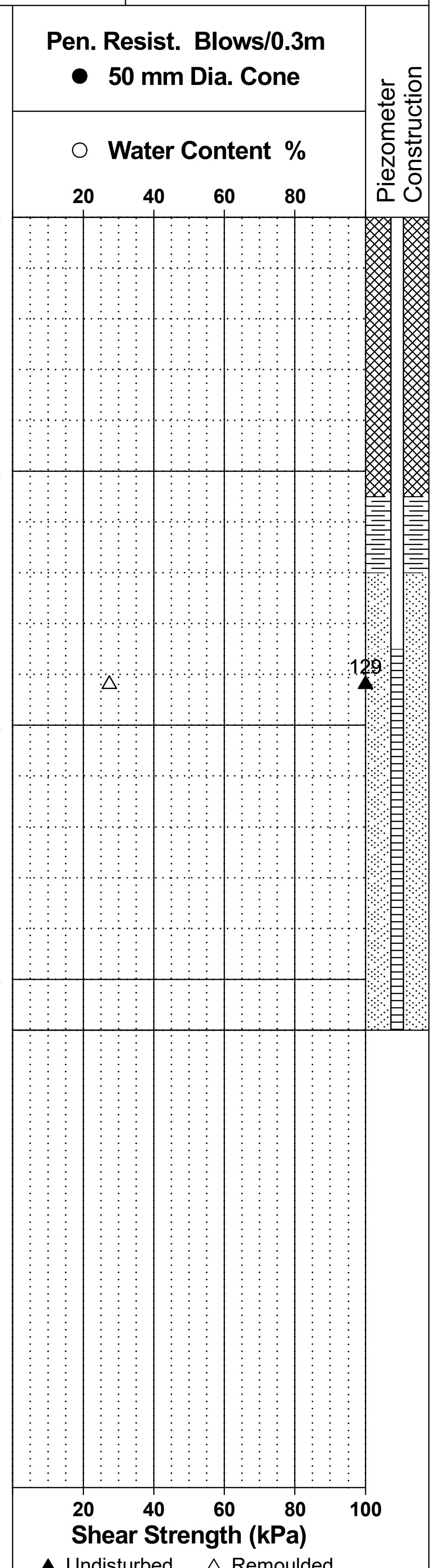
BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	Depth to base (m)	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD
GROUND SURFACE					
TOPSOIL	0.33	AU	1		
Very stiff, brown **SILTY CLAY**		SS	2	79	5
- grey by 1.8m depth	2.29				
GLACIAL TILL: Compact to very dense, brown to grey sandy silt, some clay, gravel, cobbles and boulders		SS	3	71	13
	3.20	SS	4	100	50+
End of Borehole					

Practical refusal to augering at 3.20m depth

(BH Dry - Jan 29/19)

DEPTH (m)	ELEV. (m)
0	103.58
1	102.58
2	101.58
3	100.58



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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH22**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE						0	103.65
TOPSOIL 0.25		AU	1				
Very stiff, brown **SILTY CLAY**		SS	2	71	5	1	102.65
- grey by 2.0m depth 2.29						2	101.65
End of Borehole							
Practical refusal to augering at 2.29m depth							
(BH Dry - Jan 29/19)							

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone

○ Water Content %

20 40 60 80

159

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

Monitoring Well Construction

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

HOLE NO. BH23

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.87
TOPSOIL (to 0.30)	AU	1				
Very stiff, brown **SILTY CLAY**, some sand (0.30 – 1.52)	SS	2	0	6	1	102.87
GLACIAL TILL: Dense to very dense, grey silty sand with clay, gravel, cobbles and boulders (1.52 – 3.36)	SS	3	83	11	2	101.87
	SS	4	75	36		
	SS	5	31	50+	3	100.87
End of Borehole (3.36)						

End of Borehole

Practical refusal to augering at 3.36m depth

(GWL @ 2.62m depth - Jan 29/19)

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone
○ Water Content %
20 40 60 80

Piezometer Construction

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

REMARKS

BORINGS BY CME 55 Power Auger

DATE 2019 January 16

FILE NO. PG4772

HOLE NO. BH24

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone / ○ Water Content %	Piezometer Construction
GROUND SURFACE						0	104.04		
TOPSOIL 0.36		AU	1						
Very stiff, brown to grey **CLAYEY SILT**		SS	2	67	10	1	103.04		
		SS	3	79	29	2	102.04		
2.29									
GLACIAL TILL: Compact to very dense, brown clayey silt, some sand, gravel, cobbles and boulders		SS	4	58	23				
3.15		SS	5	100	50+	3	101.04		

End of Borehole

Practical refusal to augering at 3.15m depth

(GWL @ 2.55m depth - Jan 29/19)

Water Content % / Pen. Resist. scale: 20 40 60 80

Shear Strength (kPa): 20 40 60 80 100

▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH25**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 16

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone / ○ Water Content %	Piezometer Construction
GROUND SURFACE						0	104.07		
TOPSOIL 0.38		AU	1						
Very stiff, brown **CLAYEY SILT** 1.52		SS	2	75	11	1	103.07		
GLACIAL TILL: Very dense, grey clayey silt with sand, gravel, cobbles, boulders 1.62		SS	3	75	50+				▼
End of Borehole									
Practical refusal to augering at 1.62m depth									
(GWL @ 1.68m depth - Jan 29/19)									

Water Content % axis: 20 40 60 80

Shear Strength (kPa) axis: 20 40 60 80 100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

HOLE NO. BH26

BORINGS BY CME 55 Power Auger **DATE** 2019 January 17

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone / ○ Water Content %	Piezometer Construction
GROUND SURFACE						0	104.30		
TOPSOIL 0.38		AU	1						
Very stiff, brown **CLAYEY SILT**		SS	2	75	9	1	103.30		
1.83		SS	3	50	19				
GLACIAL TILL: Compact to dense, grey silty clay with gravel, cobbles and boulders						2	102.30		
2.87		SS	4	100	46				
End of Borehole Practical refusal to augering at 2.87m depth (BH Dry - Jan 29/19)									

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH27**

BORINGS BY CME 55 Power Auger **DATE** 2019 January 17

SOIL DESCRIPTION	STRATA PLOT	SAMPLE TYPE	SAMPLE NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone / ○ Water Content %	Monitoring Well Construction
GROUND SURFACE						0	103.97		
TOPSOIL 0.33		AU	1						
Very stiff, brown **CLAYEY SILT**		SS	2	71	8	1	102.97		
- grey by 1.7m depth 1.93		SS	3	88	50+				
End of Borehole Practical refusal to augering at 1.93m depth (BH Dry - Jan 29/19)									

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. PG4772

REMARKS

HOLE NO. BH28

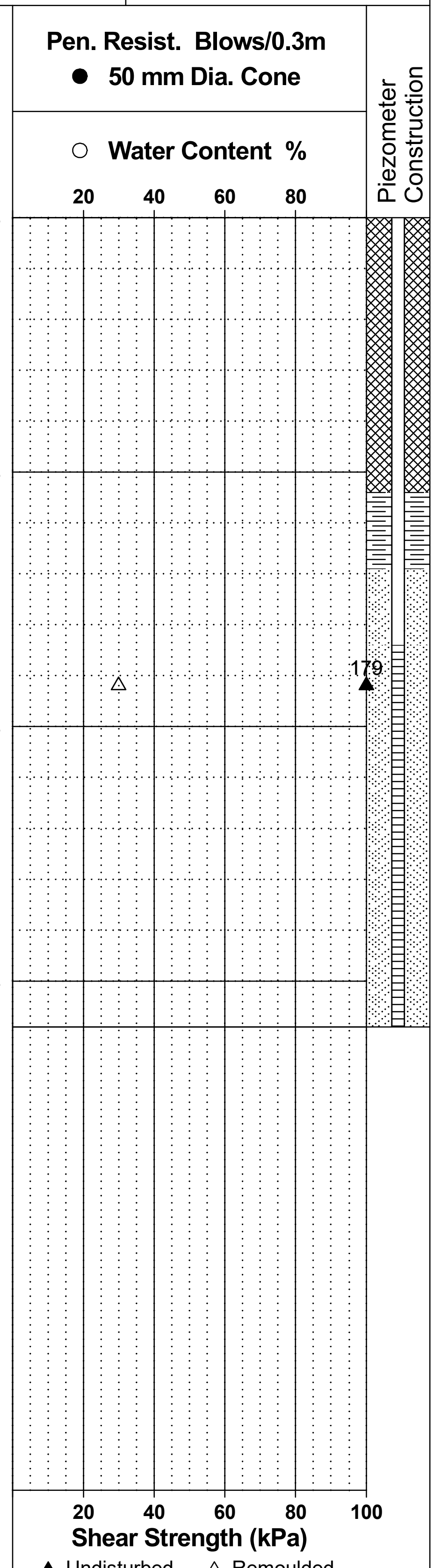
BORINGS BY CME 55 Power Auger **DATE** 2019 January 17

SOIL DESCRIPTION	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.78
TOPSOIL (to 0.36)	AU	1				
Very stiff, brown **SILTY CLAY** (to 2.29)	SS	2	38	6	1	102.78
					2	101.78
GLACIAL TILL: Loose to very dense, grey silty clay with sand, gravel, cobbles and boulders (to 3.18)	SS	3	8	2		
	SS	4	0	50+	3	100.78

End of Borehole

Practical refusal to augering at 3.18m depth

(BH Dry - Jan 29/19)



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154 Colonnade Road South, Ottawa, Ontario K2E 7J5

SOIL PROFILE AND TEST DATA

Geotechnical Investigation
Proposed Mixed-Use Development - 20 Cedarow Ct.
Ottawa, Ontario

DATUM Ground surface elevations provided by Annis, O'Sullivan, Vollebekk Ltd.

FILE NO. **PG4772**

REMARKS

HOLE NO. **BH29**

BORINGS BY CME 55 Power Auger

DATE 2019 January 17

SOIL DESCRIPTION	SAMPLE TYPE	NUMBER	% RECOVERY	N VALUE or RQD	DEPTH (m)	ELEV. (m)
GROUND SURFACE					0	103.71
TOPSOIL (to 0.38)	AU	1				
Very stiff, brown **SILTY CLAY** (0.38 to 2.29)	SS	2	50	7	1	102.71
	SS	3	71	4	2	101.71
GLACIAL TILL: Loose, grey silty clay with sand, gravel, cobbles and boulders (2.29 to 2.95)	SS	4	17	7		

End of Borehole

Practical refusal to augering at 2.95m depth

(BH Dry - Jan 29/19)

Pen. Resist. Blows/0.3m
● 50 mm Dia. Cone

○ Water Content %

20 40 60 80

Piezometer Construction

20 40 60 80 100

Shear Strength (kPa)

▲ Undisturbed △ Remoulded

SYMBOLS AND TERMS

SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

SYMBOLS AND TERMS (continued)

SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.

SYMBOLS AND TERMS (continued)

GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic limit, % (water content above which soil behaves plastically)
PI	-	Plasticity index, % (difference between LL and PL)
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D10	-	Grain size at which 10% of the soil is finer (effective grain size)
D60	-	Grain size at which 60% of the soil is finer
Cc	-	Concavity coefficient = $(D30)^2$ / (D10 x D60)
Cu	-	Uniformity coefficient = D60 / D10

Cc and Cu are used to assess the grading of sands and gravels:
Well-graded gravels have: 1 < Cc < 3 and Cu > 4
Well-graded sands have: 1 < Cc < 3 and Cu > 6
Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.
Cc and Cu are not applicable for the description of soils with more than 10% silt and clay (more than 10% finer than 0.075 mm or the #200 sieve)

CONSOLIDATION TEST

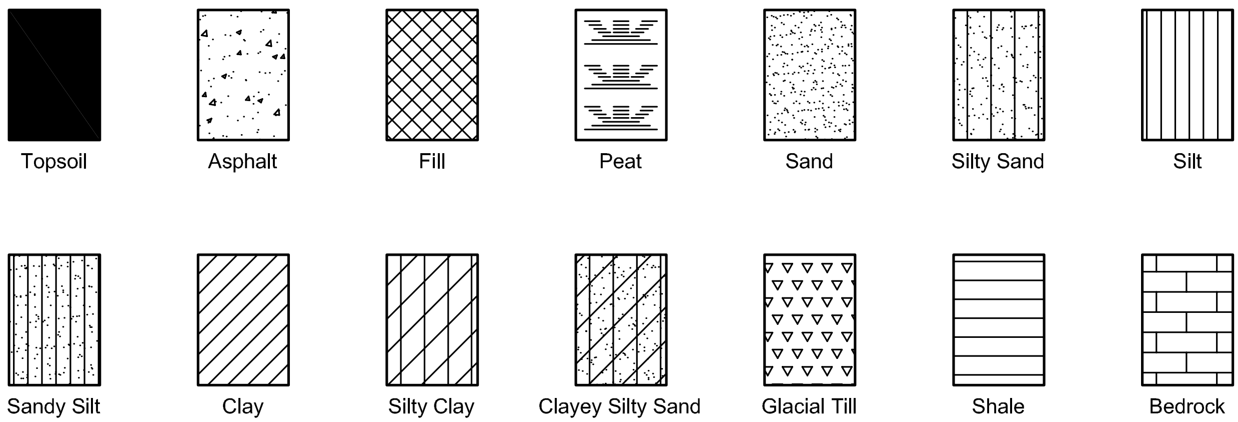
p'_o	-	Present effective overburden pressure at sample depth
p'_c	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below p'_c)
Cc	-	Compression index (in effect at pressures above p'_c)
OC Ratio		Overconsolidaton ratio = p'_c / p'_o
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

PERMEABILITY TEST

k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.

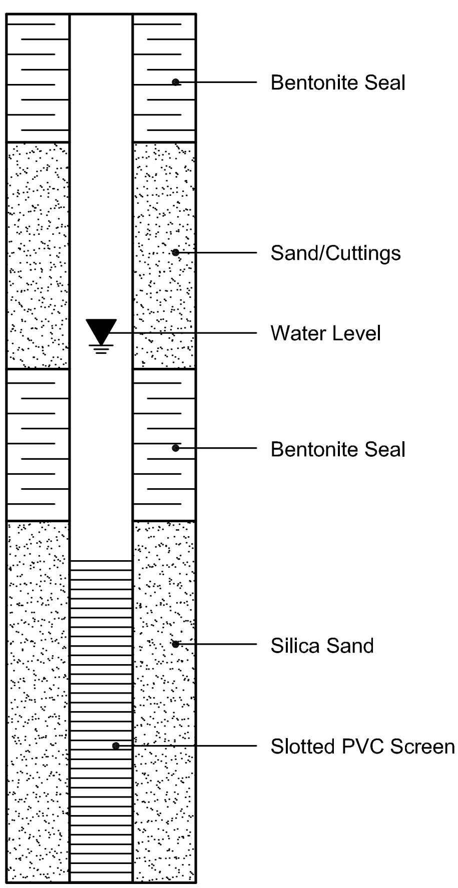
SYMBOLS AND TERMS (continued)

STRATA PLOT

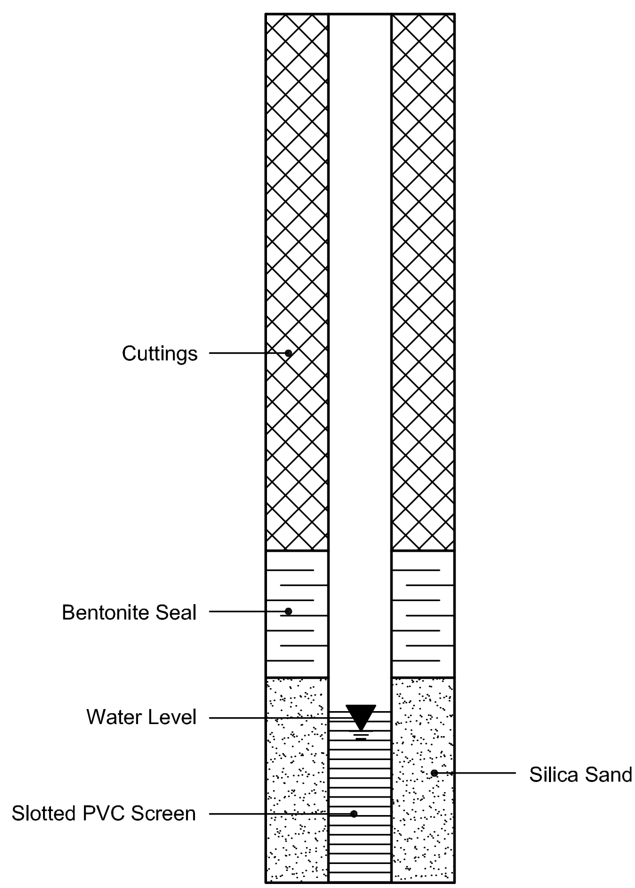


MONITORING WELL AND PIEZOMETER CONSTRUCTION

MONITORING WELL CONSTRUCTION



PIEZOMETER CONSTRUCTION



Certificate of Analysis

Client: Paterson Group Consulting Engineers

Client PO: 25648

Report Date: 22-Jan-2019

Order Date: 16-Jan-2019

Project Description: PG4772

Client ID:		BH#16-19 SS#3	-	-	-
Sample Date:		01/15/2019 09:00	-	-	-
Sample ID:		1903309-01	-	-	-
	MDL/Units	Soil	-	-	-
Physical Characteristics					
% Solids	0.1 % by Wt.	85.8	-	-	-
General Inorganics					
pH	0.05 pH Units	7.80	-	-	-
Resistivity	0.10 Ohm.m	76.2	-	-	-
Anions					
Chloride	5 ug/g dry	6	-	-	-
Sulphate	5 ug/g dry	6	-	-	-

APPENDIX 2

FIGURE 1 - KEY PLAN

FIGURES 2 TO 4 - SLOPE STABILITY ANALYSIS SECTIONS

DRAWING PG4772-1 - TEST HOLE LOCATION PLAN

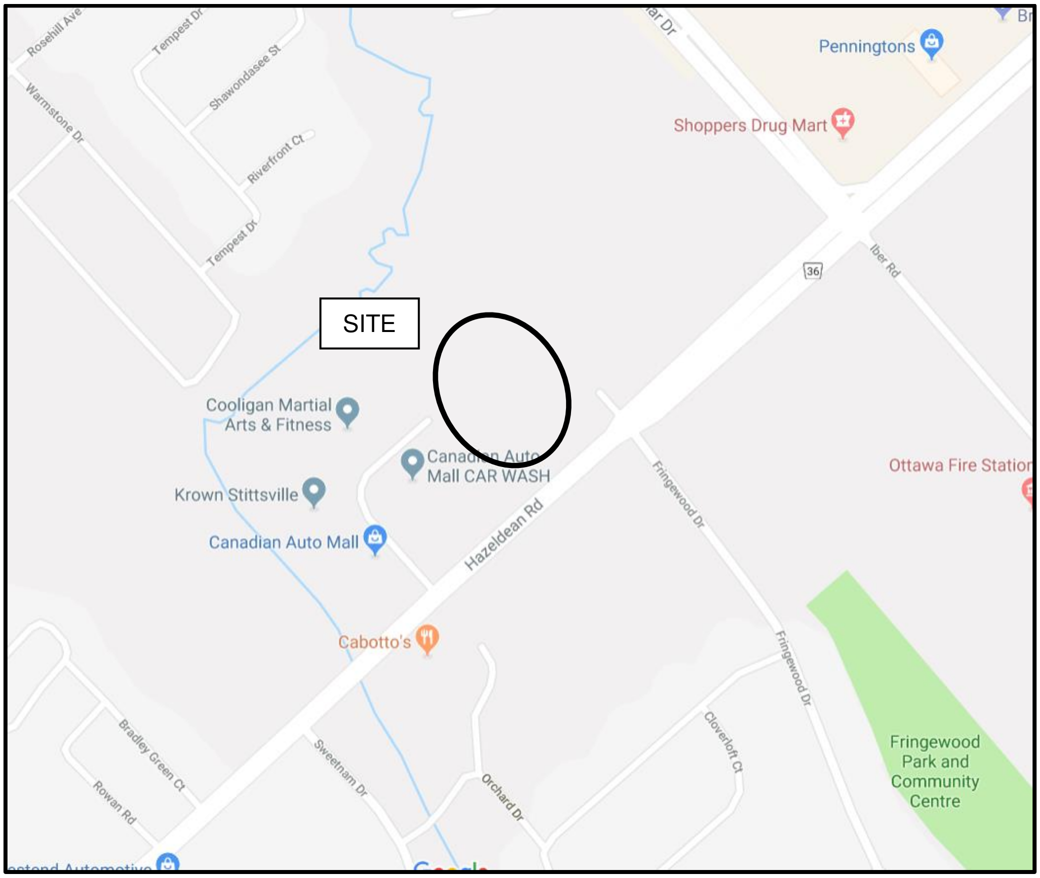


FIGURE 1

KEY PLAN

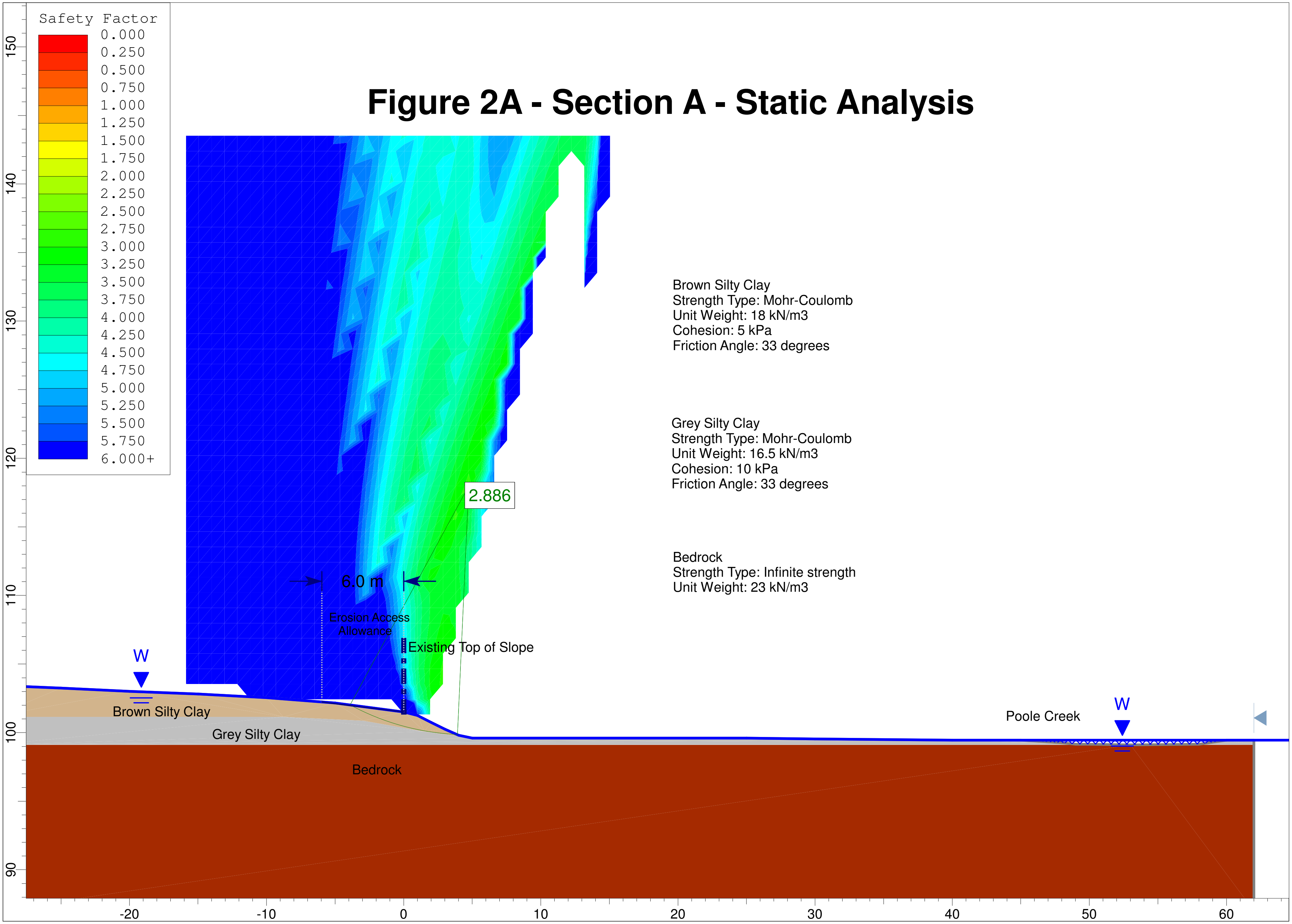


Figure 2A - Section A - Static Analysis
Safety Factor
0.000
0.250
0.500
0.750
1.000
1.250
1.500
1.750
2.000
2.250
2.500
2.750
3.000
3.250
3.500
3.750
4.000
4.250
4.500
4.750
5.000
5.250
5.500
5.750
6.000+
Brown Silty Clay
Strength Type: Mohr-Coulomb
Unit Weight: 18 kN/m3
Cohesion: 5 kPa
Friction Angle: 33 degrees
Grey Silty Clay
Strength Type: Mohr-Coulomb
Unit Weight: 16.5 kN/m3
Cohesion: 10 kPa
Friction Angle: 33 degrees
Bedrock
Strength Type: Infinite strength
Unit Weight: 23 kN/m3
2.886
6.0 m
Erosion Access Allowance
Existing Top of Slope
W
Brown Silty Clay
Grey Silty Clay
Bedrock
Poole Creek
W
150
140
130
120
110
100
90
-20
-10
0
10
20
30
40
50
60

Figure 2B - Section A - Seismic Loading

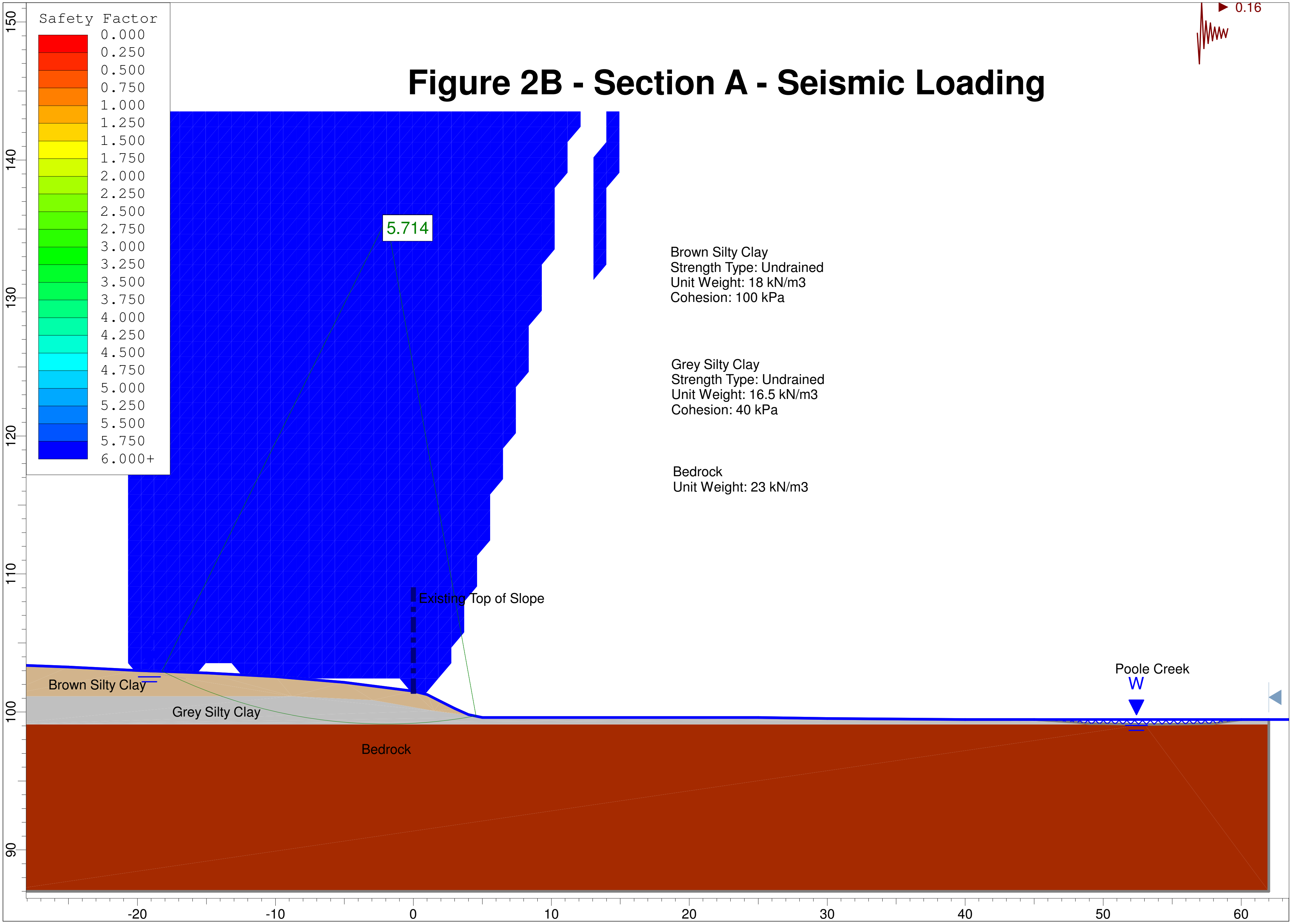


Figure 3A - Section B - Static Analysis

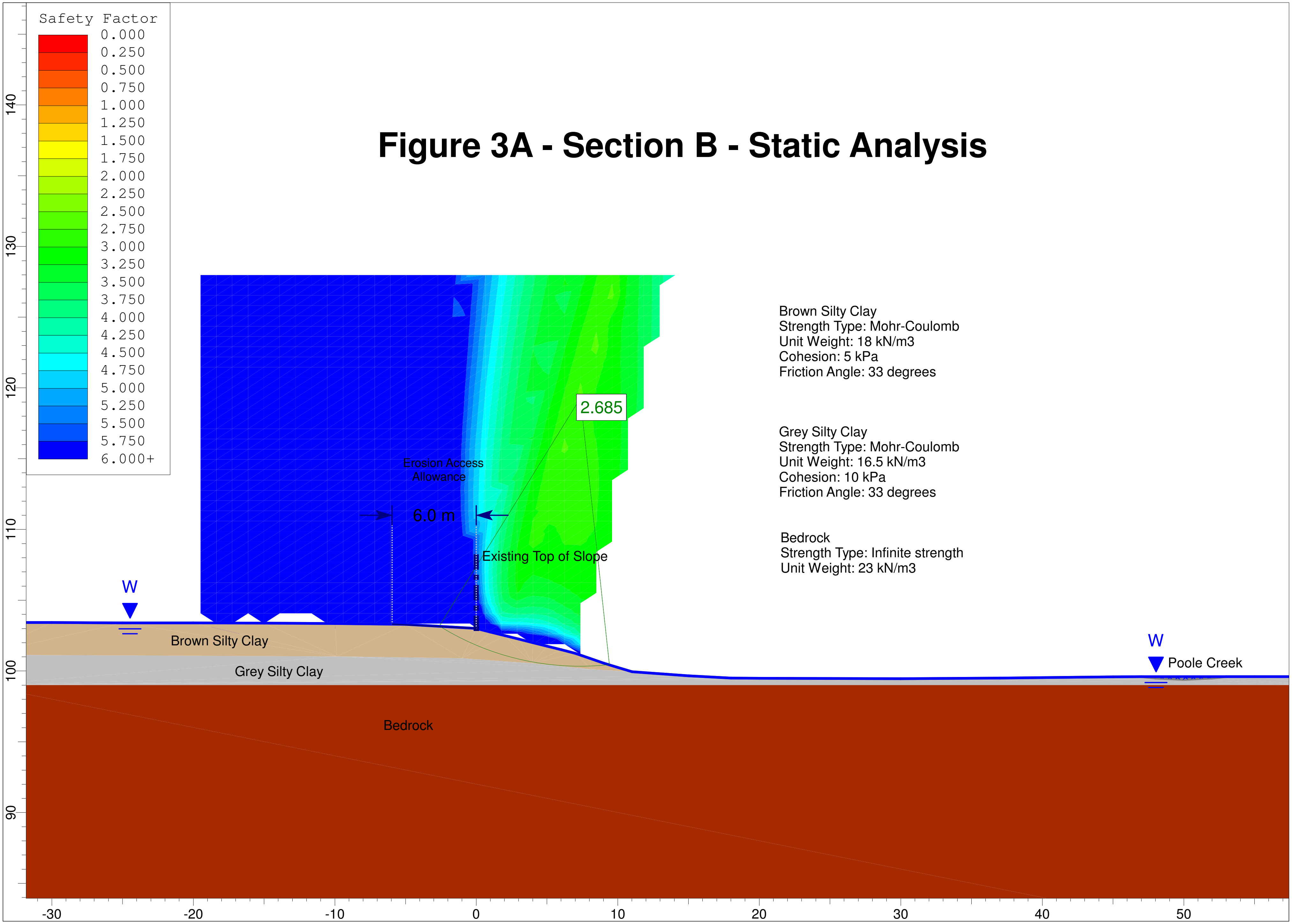
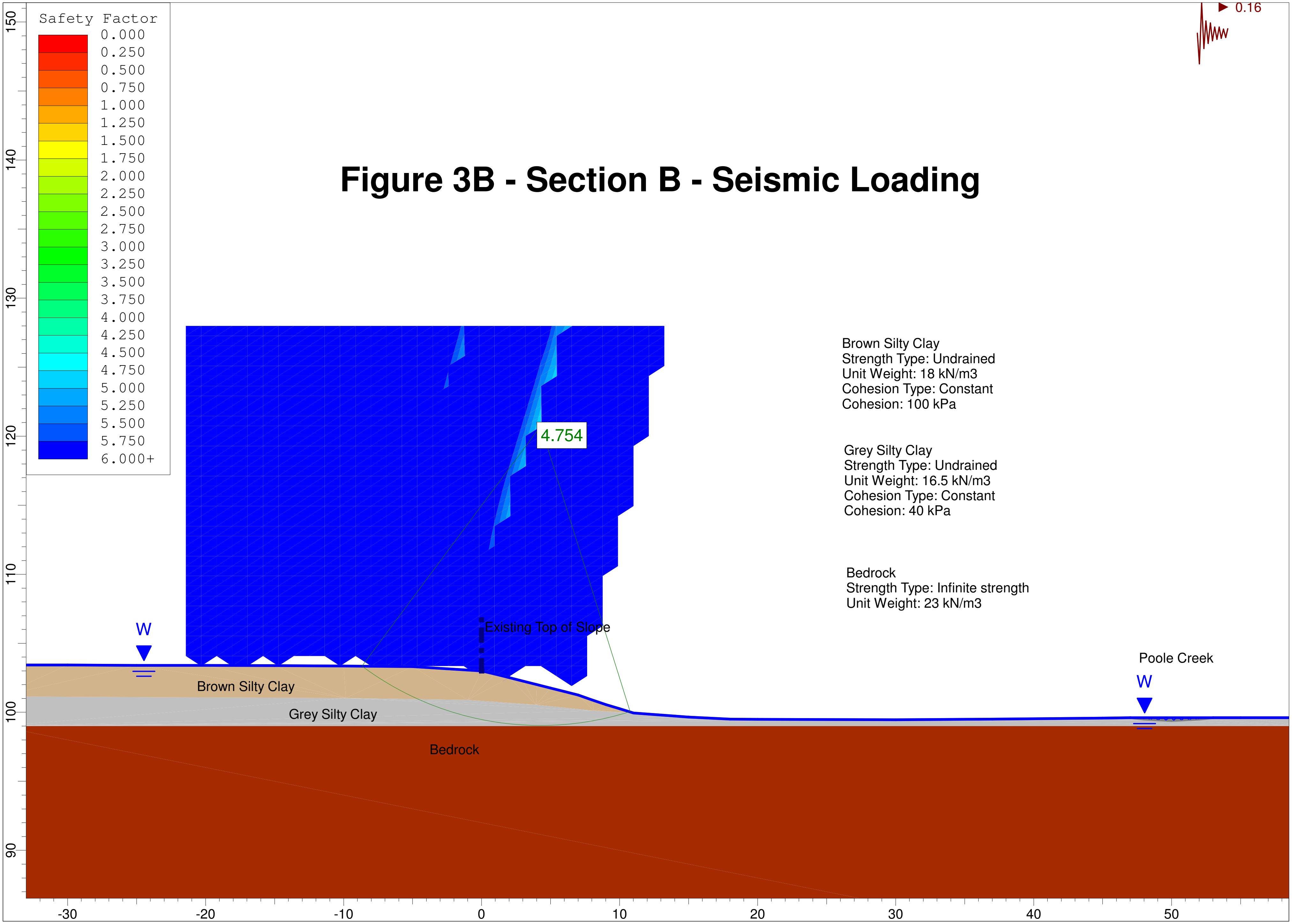


Figure 3B - Section B - Seismic Loading



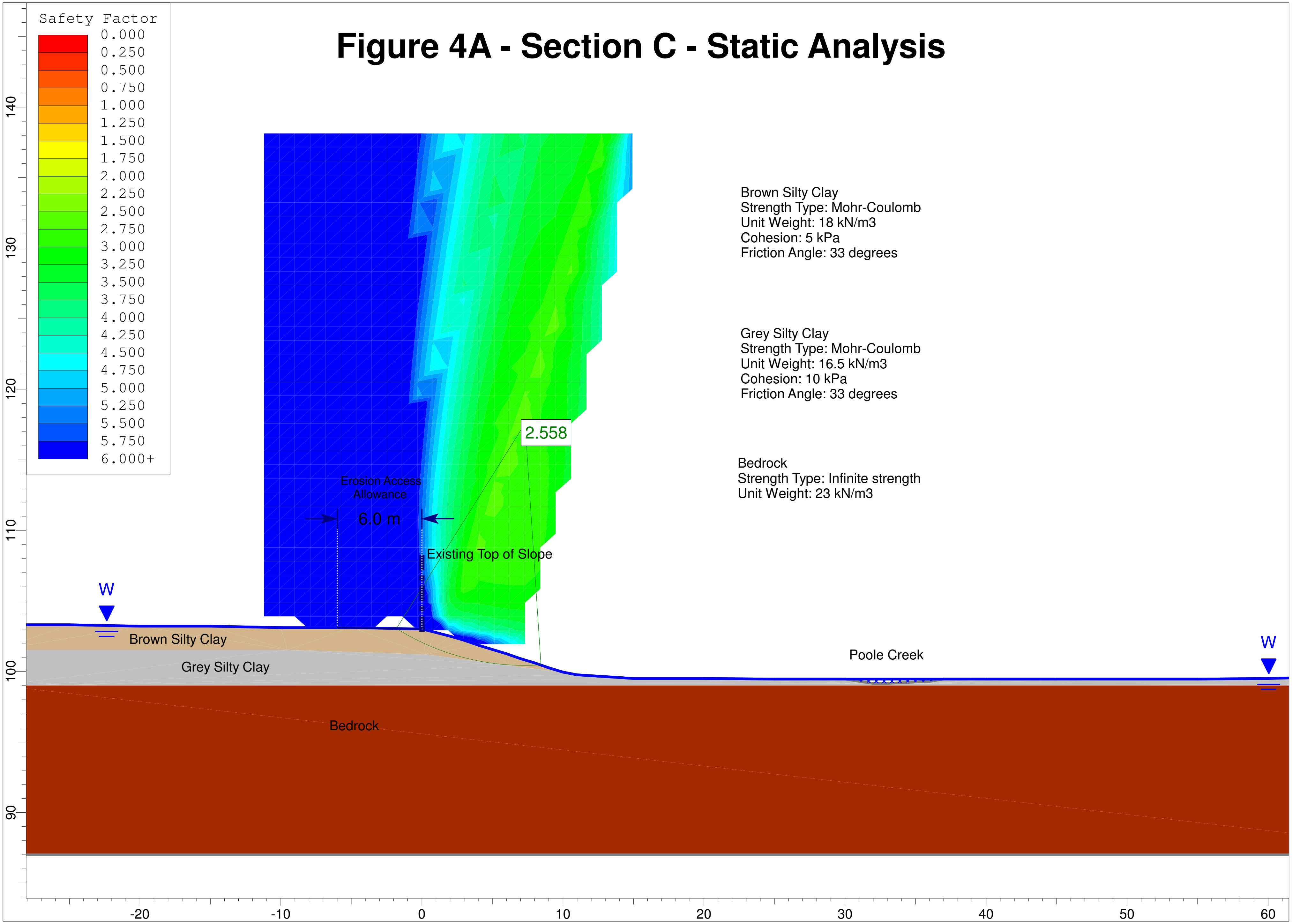
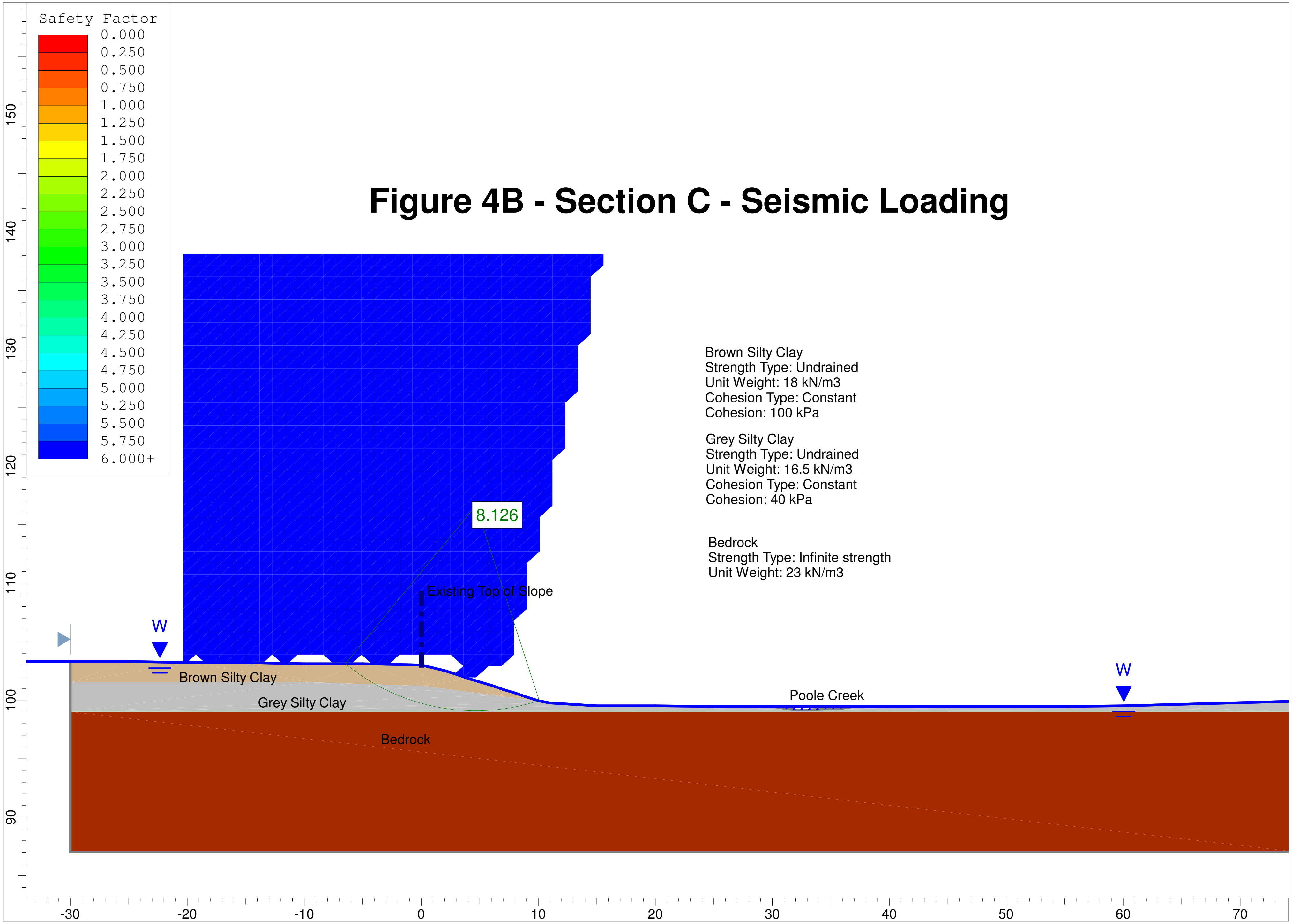
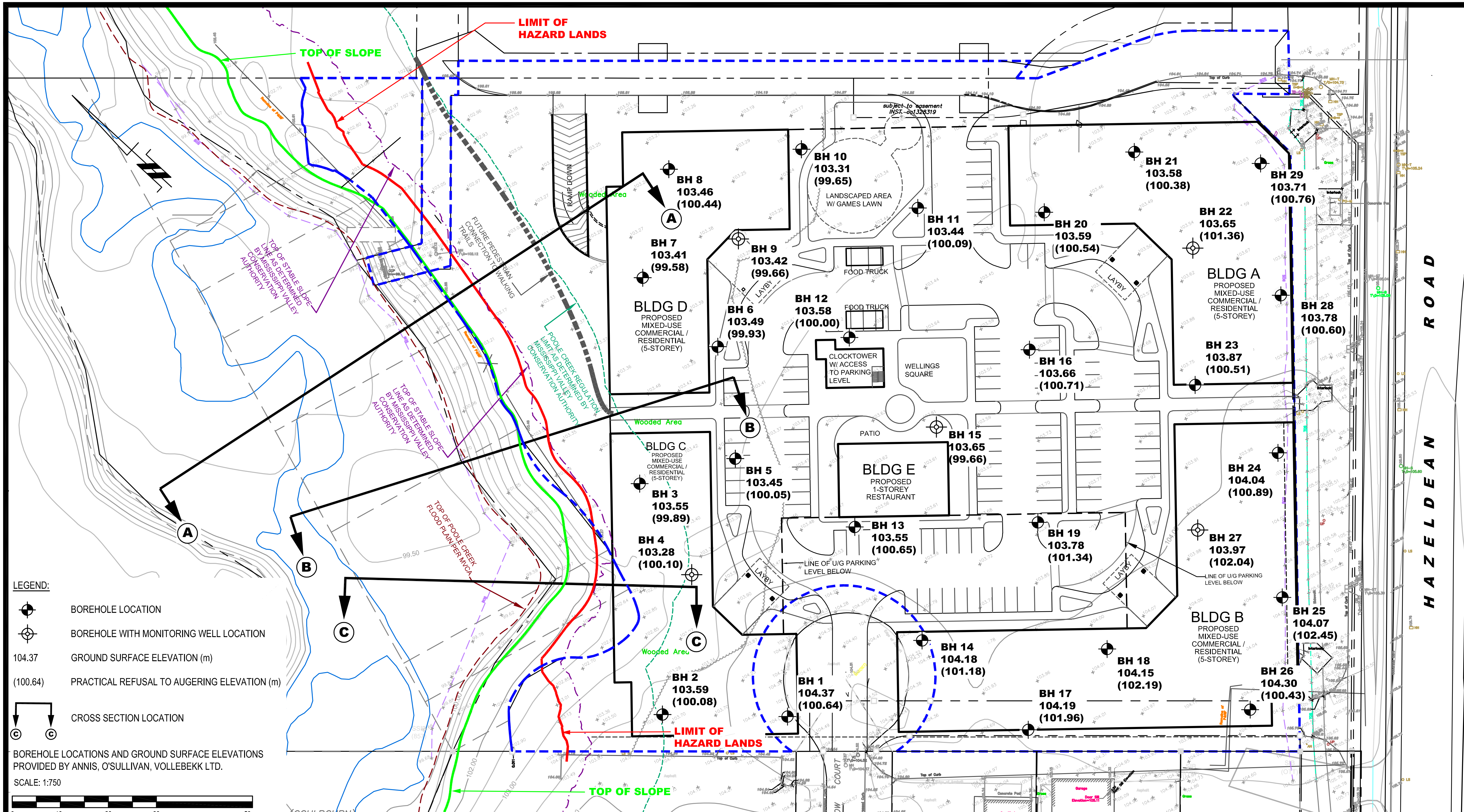


Figure 4A - Section C - Static Analysis
Safety Factor
0.000
0.250
0.500
0.750
1.000
1.250
1.500
1.750
2.000
2.250
2.500
2.750
3.000
3.250
3.500
3.750
4.000
4.250
4.500
4.750
5.000
5.250
5.500
5.750
6.000+
Brown Silty Clay
Strength Type: Mohr-Coulomb
Unit Weight: 18 kN/m3
Cohesion: 5 kPa
Friction Angle: 33 degrees
Grey Silty Clay
Strength Type: Mohr-Coulomb
Unit Weight: 16.5 kN/m3
Cohesion: 10 kPa
Friction Angle: 33 degrees
Bedrock
Strength Type: Infinite strength
Unit Weight: 23 kN/m3
2.558
Erosion Access
Allowance
6.0 m
Existing Top of Slope
W
Brown Silty Clay
Grey Silty Clay
Poole Creek
W
Bedrock
140
130
120
110
100
90
-20
-10
0
10
20
30
40
50
60

Figure 4B - Section C - Seismic Loading





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NO.	REVISIONS	DATE	INITIAL
1	LIMIT OF HAZARD LANDS REVISED	28/02/2019	FA

NAUTICAL LANDS GROUP

GEOTECHNICAL INVESTIGATION

PROP. MIXED-USE DEVELOPMENT - WELLINGS OF STITTSVILLE PHASE 2 - 20 CEDAROW CT.

OTTAWA, ONTARIO

Title:

TEST HOLE LOCATION PLAN

Scale: 1:750	Date: 01/2019
Drawn by: MPG	Report No.: PG4772-1
Checked by: FA	Dwg. No.: PG4772-1
Approved by: DJG	Revision No.: 1

p:\autocad drawings\geotechnical\pg47xx\pg4772\pg4772-1 thlp.dwg

patersongroup **memorandum**
consulting engineers

re: **Grading Plan Review**
Proposed Mixed-Use Development – Wellings of Stittsville Phase 2
20 Cedarow Court - Ottawa

to: Nautical Lands Group – **Mr. Mark Williams** – mwilliams@nauticallandsgroup.com

cc: Stantec – **Mr. Mike Sharp** – Mike.Sharp@stantec.com

date: August 12, 2021

file: PG4772-MEMO.03

Following your request and authorization, Paterson Group (Paterson) prepared the current memorandum to complete a grading plan review from a geotechnical perspective for Phase 2 of the mixed-use development to be constructed at the aforementioned site. The following memorandum should be read in conjunction with Paterson Group Report PG4772-1 Revision 1, dated September 29, 2020.

Grading Plan Review

Paterson reviewed the following grading plan prepared by Stantec regarding the aforementioned development:

- Grading Plan - Wellings of Stittsville Phase 2 - Project No. 160401511 - Drawing No. GP-1 - Sheet No. 4 of 7 - Revision 2 - dated August 3, 2021.

Based on our review of the above noted grading plan, the proposed grades within Phase 2 of the aforementioned development are within the permissible grade raise restriction of 2 m provided throughout the subject site in the aforementioned geotechnical investigation report. Therefore, the proposed grading is considered acceptable from a geotechnical perspective. No exceedances of the grade raise restriction were noted, therefore lightweight fill or other considerations to accommodate the proposed grades are not required at this time.

We trust that this information satisfies your immediate requirements.

Best Regards,

Paterson Group Inc.

LICENSED PROFESSIONAL ENGINEER
Aug. 12, 2021
F. I. ABOU-SEIDO
100156744
PROVINCE OF ONTARIO

Maha Saleh, Provisional P. Eng.

Faisal Abou-Seido, P. Eng.

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Appendix E DRAWINGS