



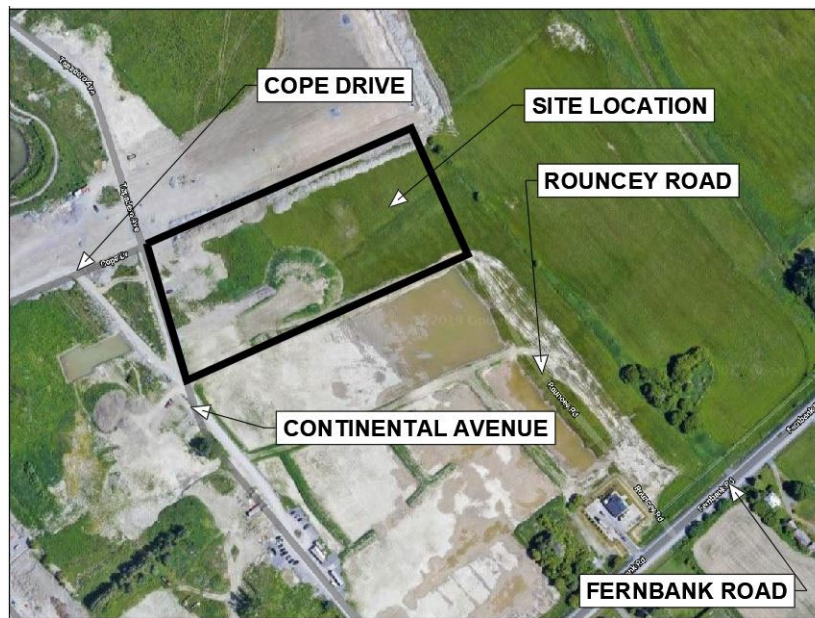
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Project No. 19-5070A

# Stormwater Management Report

## Fernbank Elementary School

480 Cope Drive, Ottawa, Ontario



Revision 1

Prepared for



City of Ottawa  
Infrastructure Services and Community Sustainability  
110 Laurier Ave. West, 4th floor, Mail Code 01-14  
Ottawa, Ontario, K1P 1J1

February 18, 2020

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## 1 Introduction

Jp2g Consultants Inc. was retained by N45 Architecture Inc. to complete a Stormwater Management Report suitable for City of Ottawa Site Plan Control Application, for the Ottawa Carleton District School Board's proposed Elementary School and Daycare Addition at Cope Drive and Rouncey Road, in the City of Ottawa. The total site area is approximately 2.84 ha and the proposed site development area includes the construction of a 360 m<sup>2</sup> one-storey day-care building, associated parking areas, play area, and landscaped areas. A pre-Consultation meeting was held with City of Ottawa staff on August 23, 2019, to determine the project constraints and requirements. The following report details the stormwater management calculations used for water quantity and quality control in accordance with the City of Ottawa's requirements.

Reference Drawings: [SD1 – Stormwater Management Sub-Drainage Areas](#), [C02 – Servicing Tables](#), [C03 - Site Servicing Plan \(November 28, 2019\)](#), and [C04 - Site Grading and Drainage, Erosion and Sediment Control Plan \(November 28, 2019\)](#).

## 2 Objective

The objective of the stormwater management plan is to control post-development peak flows to pre-determined levels, and detain onsite, stormwater up to and including the 100-year storm event without affecting adjacent lands. Stormwater quality control will be provided by the downstream pond 6, no onsite quality control is required.

## 3 Design Parameters

Stormwater management criteria for this site, in terms of quantity control, is based on the following approved reports:

- Servicing and Stormwater Management Report – Blackstone Community Phase 4-8 by Stantec, April 11, 2019.

The maximum allowable release rate for this site shall not exceed the criteria set in the approved servicing report. Flows in excess of the allowable release rate up to and including the 100-year event will be detained onsite.

The Modified Rational Method ( $Q = 2.78CiA$ ) was chosen to calculate the post-development release rates, and onsite storage requirements for this development. Detailed stormwater management calculations are included in [Appendix B](#). All proposed storm sewers were assigned a Manning's coefficient of roughness of 0.013 corresponding to smooth wall pipes. In accordance with City of Ottawa Sewer Design Guidelines (Section 5.4.5.2.1), the coefficients used for calculating the post-development release rate were  $C = 0.25$  for grassed areas and  $C = 0.90$  for hard surfaced areas including rooftops. The rainfall intensities used in this analysis are based on the IDF curves and equations, as per City of Ottawa Sewer Design Guidelines (Section 5.4.2).

## 4 Water Quantity Controls

### 4.1 Pre-Development Conditions

The existing site is an undeveloped parcel with a generally flat site topography that is sloped toward the east side of the property with an approximate elevation difference of 0.5 meter over approximately a 220-meter length. The proposed day-care facility will be constructed at the same time as the main school building. Services have been installed along Rouncey Road, as part of the development of the Blackstone Community Phase 4-8.

### 4.2 Allowable Release Rate

The stormwater management design criteria for this site is based on the subdivision servicing report as noted above. According to the Stantec study, the school site has an allowable release rate up to the 5 year-event of **575.7 L/s**. Refer to **Appendix A** – Background excerpts – Storm Water Management. Minimum storage requirement was set to **50 m<sup>3</sup> / ha** per the Subdivision study.

### 4.3 Post-Development Conditions

Proposed site grading and drainage of the overall site was designed such that stormwater runoff will be collected by the roof and a new storm water collection system which will be connected to the existing 1200mm diameter municipal storm sewer on Rouncey Road and discharged to Pond 6 of the subdivision.

The storm sewer system consists of manholes, catchbasins, storm sewers, perforated subdrain system in landscaped area of the sportsfield. Predominantly, drainage areas are piped to the storm system except for areas fronting onto Cope Drive, the bus layby and a portion of the paved basketball court area. This last area is uncontrolled to permit the unrestricted flow from the building storm sewer and prevent backup to the building.

No surface ponding will occur during the 5-year event. Flows exceeding the allowable release rate up to the 100-year event are to be detained on site.

The overall site development area is approximately **2.84 ha** and has a post-development average weighted runoff coefficient of **C=0.59**, and **C=0.64** for the maximum and 100-year events, respectively. Overall onsite storage requirement was calculated to be **190 m<sup>3</sup>** for the 100-year event, which exceeds the minimum storage requirement of 50m<sup>3</sup>/ha.

### 4.4 Onsite Stormwater Detention

Stormwater detention is proposed on the school roof, in the proposed parking area, in the paved school yard as well as along the sportsfield swale.

Flows from the roof area, parking area and grassed areas will be controlled individually in order to provide less restricted flow path. The maximum allowable ponding depth will be limited to 250mm for paved areas and 300mm in grassed areas during 100-year events. Flow control is proposed at manholes CBMH5 and CBMH7.

Flow at CBMH5 serving the parking and paved areas will be limited to 300 L/s. The maximum ponding elevation in the parking area is 100.85 which is below the building finished floor elevation of 101.25. The flows from the parking areas will be restricted at CBMH5 by installing an orifice plate with a discharge rate of **300.0 L/s** at an estimated head of **2.05 m**. The orifice plate is shown on Drawing C006. The maximum head of water is dictated by the overland overflow elevation of 103.46m. The invert of the outlet pipe is 98.64.

The orifice plate was sized using the orifice equation:

$$Q = 0.61 \times A \times (2 \times g \times H)^{0.5} \text{ , where } Q \text{ is the discharge rate in m}^3/\text{s}$$

$$\text{Orifice coefficient} = 0.61$$

$$G = \text{gravitational constant} = 9.81 \text{ m}^2/\text{s}$$

$$H = \text{head of water (m) above the centre of the orifice} = (100.85 - 98.64) - (0.5 \times \text{orifice diameter})$$

The orifice of 314mm provides the required flow

$$Q = 0.61 \times A \times (2 \times g \times H)^{0.5} = 0.61 \times (\pi \times (0.314/2)^2) \times (2 \times 9.81 \times (2.21 - (0.5 \times 0.314)))^{0.5} = 0.300 \text{ m}^3/\text{s}$$

Flow from the sportsfield and grassed areas will be restricted at CBMH7 by installing an orifice plate with a discharge rate corresponding to the 5-year event of **60 L/s** at an estimated head of **1.93 m**. The orifice plate is shown on Drawing C006. The maximum head of water is dictated by the overland overflow elevation of 100.76m. The invert of the outlet pipe is 98.90.

The orifice plate was sized using the orifice equation:

$$Q = 0.61 \times A \times (2 \times g \times H)^{0.5} \text{ , where } Q \text{ is the discharge rate in m}^3/\text{s}$$

$$\text{Orifice coefficient} = 0.61$$

$$G = \text{gravitational constant} = 9.81 \text{ m}^2/\text{s}$$

$$H = \text{head of water (m) above the centre of the orifice} = (100.76 - 98.90) - (0.5 \times \text{orifice diameter})$$

The orifice of 145mm provides the required flow

$$Q = 0.61 \times A \times (2 \times g \times H)^{0.5} = 0.61 \times (\pi \times (0.145/2)^2) \times (2 \times 9.81 \times (1.86 - (0.5 \times 0.145)))^{0.5} = 0.060 \text{ m}^3/\text{s}$$

Flow from the combined school and daycare roof will also be detained on the roof by installing parabolic weirs, (Watts Drainage Adjustable Flow Control for Roof Drains, or equivalent approved products), at the 29 proposed roof drains limiting the flow from the roof to **18.3 L/s**. The resulting required storage is **153 m<sup>3</sup>** for the 100-year event. Based on a maximum ponding depth of 150mm on the roof, the total available storage is approximately **194 m<sup>3</sup>**, which is sufficient to accommodate the 100-year event. Each flow control roof drain, complete with a single slot parabolic weir, will restrict flow at 5 GPM (0.32 L/s) per inch (25.4mm) of head to a maximum of 10 GPM (0.63 L/s). The restricted flow will outlet to the school's 300mm diameter storm sewer service at 4.0% slope downstream of any flow restriction.

The following table summarizes on-site requirements during the 100-year event:

Total Area	Controlled / Uncontrolled	Run-off Coefficient		Outlet Location	Total Storage Provided (m <sup>3</sup> )	Flow (L/s)	Required Storage (cu.m.)
		5 year	100 year			100-year event	
2.364	Controlled	0.59	0.64		343	378.3	190
0.480		0.54	0.58	Cope Drive & Rouncey Road ROW		138.6	
2.844					343	516.9	190

Note that storage within the subdrain clearstone trench was not included within the available storage volume. There is sufficient available ponding to accommodate the 100-year event.

The maximum ponding limits are indicated on **SD1**. In the event of a rainfall exceeding the 100-year event, runoff will remain on-site until ponding reaches the overspill elevation. In such event, the school yard and sport field major overland flow route will overflow towards Rouncey Drive and the parking area will overflow to Continental Avenue. The grade elevation at the overflow point is **0.30 m** below the school's finished floor elevation.

#### 4.5 Proposed Release Rates

The proposed release rate for this site during the 100-year event, including uncontrolled flows (**138.6 L/s**) and controlled flows into the minor system of **378.3 L/s**. Therefore, proposed release rates are within the allowable release rate for this site, determined to be **575.7 L/s** in Section 4.2.

### 5 Erosion and Sediment Control

In accordance with City of Ottawa requirements, best management practices are to be implemented by the Contractor to provide protection of the area drainage system and the receiving water course, during construction activities. This includes limiting the amount of exposed soil, using filter bag inserts under the grates of catch basins and manholes, installing silt fences and other effective sediment traps, and installing and maintaining mud mats for outgoing construction traffic during construction activities.

## 6 Conclusion

The proposed site development includes a new school building with an attached daycare, a bus lay-by, asphalt parking, hard surface walkways and play areas, landscaped areas, a sports field and an area for portables. Roof drainage and surface runoff will be collected by a new storm sewer system which will be connected to the existing 1200mm diameter municipal storm sewer located on Rouncey Road. Post-development peak flows will be detained on the roof, in the parking area, in the school yard and in the bus lay-by in order to limit the post-development release rate to allowable levels. There is sufficient onsite storage to accommodate the 100-year event.

### Summary of report

ID	Description	Value/result
01	Allowable release rate	$Q_{\text{allowable}} = 575.7 \text{ L/s}$
02	Proposed release rate	$Q_{100\text{-yr post}} = 516.9 \text{ L/s}$
03	Post-development runoff coefficient	$C_{5\text{-yr post}} = 0.58, C_{100\text{-yr post}} = 0.63$
04	Post-development onsite storage requirement	190 m <sup>3</sup>
05	Proposed onsite storage	Parking lot and school yard: 190 m <sup>3</sup> , Roof: 153m <sup>3</sup> , Sportsfield: 6 m <sup>3</sup>
06	Discharge outlet location	1200mm $\phi$ storm sewer on Rouncey Road to Pond 6
07	Emergency runoff overflow locations	West side – curb cut in parking area towards Continental Avenue  East side – from school yard to layby towards Rouncey Road

### END OF REPORT

Prepared by:

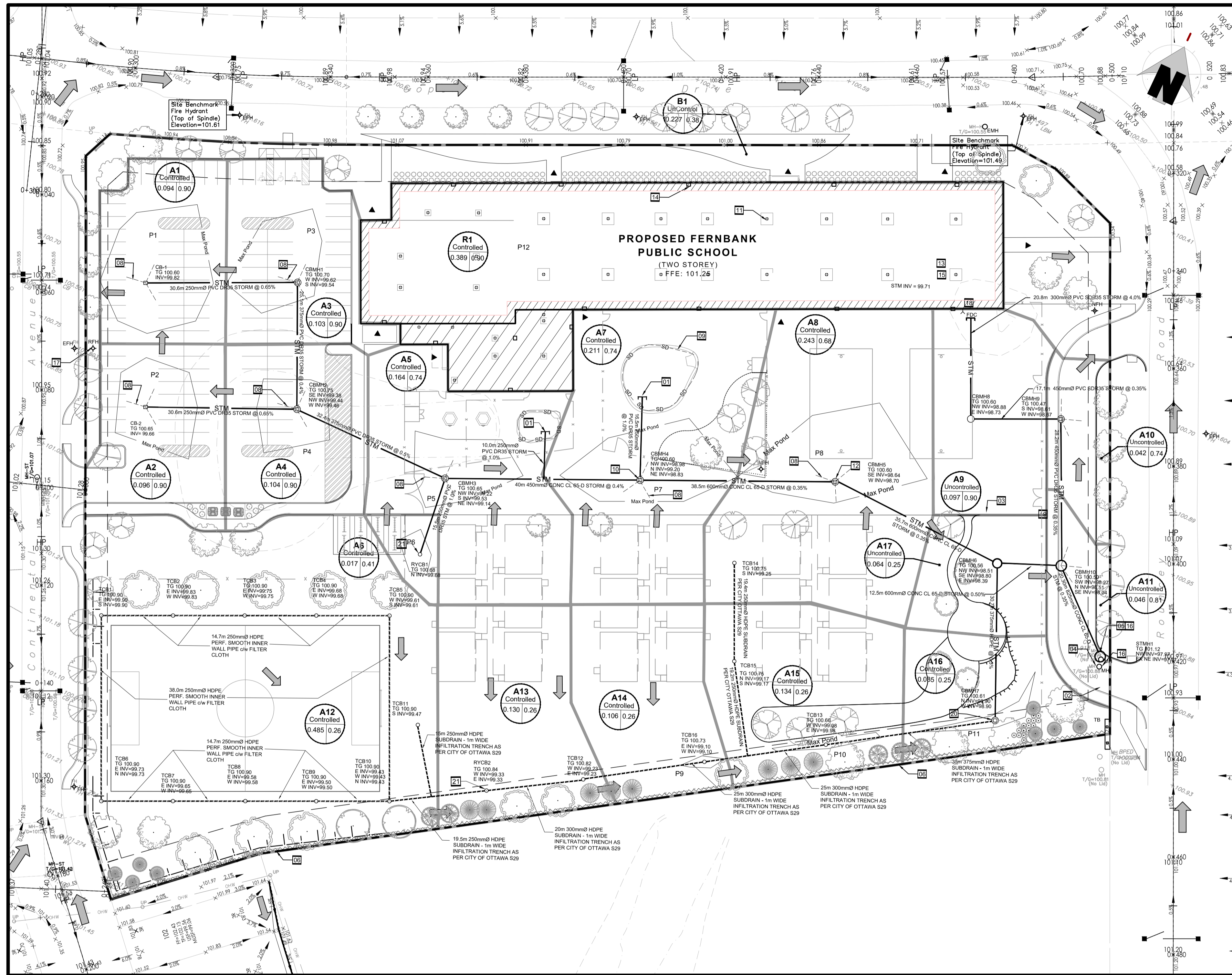


Barbra Kimmerle, P.Eng.  
 Civil Engineer

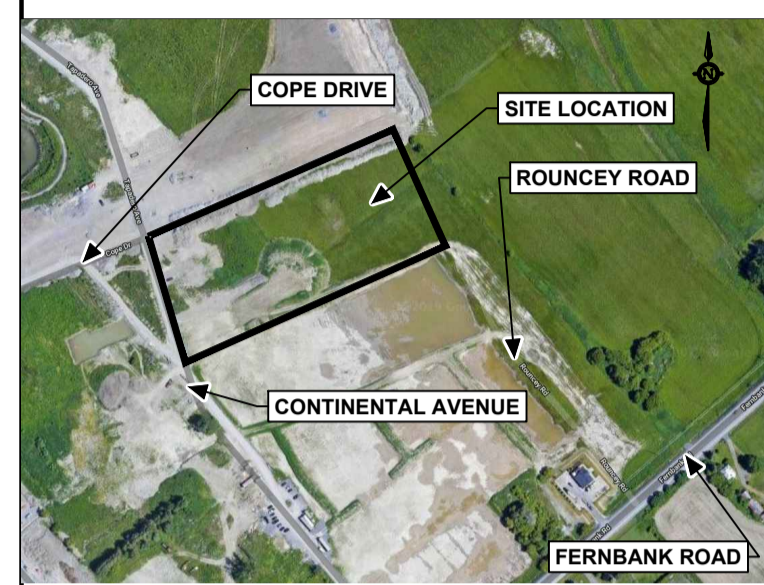


PONDING TABLE					
POND NO.	LOCATION	POND ELEV. (m)	TOP OF CB ELEV. (m)	POND DEPTH (m)	POND VOL. (m <sup>3</sup> )
P1	CB1	100.85	100.60	0.25	37.93
P2	CB2	100.85	100.67	0.18	15.42
P3	CBM1	100.85	100.70	0.15	13.81
P4	CBM2	100.85	100.72	0.13	24.81
P5	CBM3	100.85	100.65	0.20	15.98
P6	RYCB1	100.85	100.59	0.26	3.44
P7	CBM4	100.85	100.62	0.18	30.60
P8	CBM5	100.85	100.60	0.25	41.66
P9	TCB12	100.76	100.71	0.06	0.38
P10	TCB13	100.76	100.76	0.10	1.00
P11	CBM7	100.76	100.61	0.15	4.58
P12	ROOF			0.12	153.01
				Total=	342.62

NOTE: PONDING VOLUME CALCULATED BY MULTIPLYING THE PONDING AREA BY THE MAXIMUM PONDING DEPTH AND DIVIDING BY 3 FOR A CONICAL POND



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No.	DESCRIPTION	YYYY-MM-DD
3	REVISED PER CITY COMMENTS	2020-02-18
2	ISSUED FOR BUILDING PERMIT	2019-12-19
1	ISSUED FOR SITE PLAN CONTROL	2019-11-28

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project  
**FERNBANK ELEMENTARY SCHOOL**

480 COPE DRIVE, OTTAWA, ONTARIO

seal

drawing title <b>STORM DRAINAGE PLAN</b>	
scale 1:500	drawn by M.S.
date DEC 2019	checked by B.K.
project number	drawing number <b>SD1</b>
CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE WORK COMMENCES. DO NOT SCALE DRAWINGS.	
revision	



## **Appendix A - Background Excerpts Stormwater Management**











## **Appendix B - Stormwater Management Calculations**

## Fernbank Elementary School – Cope Drive, Ottawa



## B.1.1 - Allowable release rate

Using the criteria for the site from the subdivision Final Serviceability Report Development by Stantec (April 2019), the maximum allowable release rate is up to the 5 year discharge rate. Based on the weighted average the allowable release rate is:

$$Q_{\text{allowable}} = Q = 2.78 C / A$$

$$Q_{\text{allowable}} = 575.7 \quad \text{l/s} \quad \text{①}$$

## B.1.2 - Post-development release rate

ID	Description	Type	Areas (m <sup>2</sup> )		Total (m <sup>2</sup> )	C <sub>post-5-year</sub>	C <sub>post-100-yr</sub> *
			C <sub>0.90</sub>	C <sub>0.25</sub>			
B1	Front of building	uncontrolled	447	1854	2301	0.38	0.40
A1	Western parking area 1	controlled	945		945	0.90	1.00
A2	Western parking area 2	controlled	960		960	0.90	1.00
A3	Western parking area 3	controlled	1033		1033	0.90	1.00
A4	Western parking area 4	controlled	1041		1041	0.90	1.00
A5	Playground at Daycare area 1	controlled	1140	499	1639	0.70	0.77
A6	Future portables area 1	controlled	43	130	173	0.41	0.44
A7	Playground at Daycare area 2	controlled	1591	517	2108	0.74	0.82
A8	Courts area 1	controlled	1604	829	2433	0.68	0.74
A9	Courts area 2	uncontrolled	970		970	0.90	1.00
A10	Bus loop area 1	uncontrolled	317	106	423	0.74	0.81
A11	Bus loop area 2	uncontrolled	391	65	456	0.81	0.89
A12	Sports Field	controlled	63	4791	4854	0.26	0.26
A13	Future portables area 2	controlled	24	1280	1304	0.26	0.26
A14	Future portables area 3	controlled	24	1040	1064	0.26	0.27
A15	Future portables area 4	controlled	24	1319	1343	0.26	0.26
A16	Back lot area 1	controlled	0	850	850	0.25	0.25
A17	Back lot area 2	uncontrolled	0	645	645	0.25	0.25
R1	Roof	controlled	3890		3890	0.90	1.00
			<b>14507</b>	<b>13925</b>	<b>28432</b>	<b>0.58</b>	<b>0.63</b>

\*including 25% increase as per City of Ottawa Sewer Design Guidelines

Calculations for post-development runoff coefficient

$$C_{\text{post-5-year (col. D)}} = (\text{column A} * 0.9 + \text{column B} * 0.2) / \text{column C}$$

$$C_{\text{post-100-yr (col. E)}} = (\text{column A} * 1.0 + \text{column B} * 0.2 * 1.25) / \text{column C}$$

note:  $0.90 \times 1.25 = 1.125$ , use max. 1.0

Calculations for average weighted runoff coefficient

$$C_{\text{post-5-year}} = ((15170 * 0.9) + (13211 * 0.25)) / 28381 = 0.58$$

$$C_{\text{post-100-yr}} = ((15170 * 1.0) + (13211 * 0.25 * 1.25)) / 28381 = 0.63$$

Estimated time of concentration,  $t_c =$

**10.0** minutes

\*\*\*As per City of Ottawa Sewer Design Guidelines (Section 5.4.5.2)

Based on Ottawa IDF curve,  $i_{5\text{-year}} =$

$$998.071 / (t_c + 6.053)^{0.814}$$

**104.2** mm/hr

Based on Ottawa IDF curve,  $i_{100\text{-years}} =$

$$1735.688 / (t_c + 6.014)^{0.820}$$

**178.6** mm/hr

## A.1.2.1 - uncontrolled flow

Total uncontrolled area =

**0.480** ha

5-year Runoff coefficient, C =

**0.54**

100-year Runoff coefficient, C =

**0.58**

Estimated time of concentration,  $t_c =$

**10.0** minutes

$$Q_{\text{uncontrolled 5-year}} = 74.7 \quad \text{l/s} \quad \text{②}$$

$$Q_{\text{net-allowable 5-year}} = 501.0 \quad \text{l/s} \quad \text{③} = \text{①} - \text{②}$$

$$Q_{\text{uncontrolled 100-year}} = 138.6 \quad \text{l/s} \quad \text{④}$$

$$Q_{\text{net-allowable 100-year}} = 437.1 \quad \text{l/s} \quad \text{⑤} = \text{①} - \text{④}$$

**B.1.3 - Post-development onsite storage**

A.1.3.1 - Overall onsite storage requirements

Total controlled area, A1 to A8 & A12 to A16 & R1	<b>2.364</b>	ha
5-year Runoff coefficient, C	<b>0.59</b>	
100-year Runoff coefficient, C	<b>0.64</b>	
100 yr net-allowable release rate	<b>437.1</b>	l/s <span style="float: right;">⑤</span>

**Table 1.3.1a - 5-year onsite storage requirements**

	Time (minutes)	$i_{5\text{-year}}$ (mm/hr)	$Q_{\text{actual}}$ (l/s)	$Q_{\text{allowable}}$ (l/s)	$Q_{\text{stored}}$ (l/s)	$V_{\text{stored}}$ (m <sup>3</sup> )
<i>peak V<sub>stored</sub> ---&gt;</i>	10	104.2	404.3	437.1	-32.8	-19.7
	15	83.6	324.2	437.1	-112.9	-101.6
	20	70.3	272.6	437.1	-164.5	-197.4
	25	60.9	236.3	437.1	-200.8	-301.2
	30	53.9	209.2	437.1	-227.8	-410.1
	35	48.5	188.3	437.1	-248.8	-522.5
	40	44.2	171.4	437.1	-265.6	-637.5
	45	40.6	157.6	437.1	-279.4	-754.5
	50	37.7	146.1	437.1	-291.0	-872.9
	55	35.1	136.3	437.1	-300.8	-992.6
	60	32.9	127.8	437.1	-309.3	-1113.3

Therefore **-20** m<sup>3</sup> of onsite storage required during 5-year even

**Table 1.3.1b - 100-year onsite storage requirements**

	Time (min)	$i_{100\text{-year}}$ (mm/hr)	$Q_{\text{actual}}$ (l/s)	$Q_{\text{allowable}}$ (l/s)	$Q_{\text{stored}}$ (l/s)	$V_{\text{stored}}$ (m <sup>3</sup> )
<i>peak V<sub>stored</sub> ---&gt;</i>	10	178.6	754.3	437.1	317.2	190.3
	15	142.9	603.6	437.1	166.6	149.9
	20	120.0	506.7	437.1	69.6	83.6
	25	103.8	438.7	437.1	1.6	2.4
	30	91.9	388.1	437.1	-49.0	-88.2
	35	82.6	348.8	437.1	-88.2	-185.3
	40	75.1	317.4	437.1	-119.6	-287.1
	45	69.1	291.7	437.1	-145.4	-392.5
	50	64.0	270.2	437.1	-166.9	-500.7
	55	59.6	251.9	437.1	-185.2	-611.2
	60	55.9	236.1	437.1	-201.0	-723.5

Therefore **190** m<sup>3</sup> of onsite storage required during 100-year even



**B.1.3.2 - Estimated detention created by installing roof weirs**

Total roof area, A3	<b>0.389</b>	ha	
5-year Runoff coefficient, C	<b>0.90</b>		
100-year Runoff coefficient, C	<b>1.00</b>		
Install 0.6309 l/s weirs at each of the 29 roof drains	<b>18.3</b>	l/s	<i>Watts Drainage Adjustable Flow Control for Roof Drains, or approved equivalent</i>

**Table 1.3.2a - 5-year estimated detention on new roof**

Time (minutes)	$i_{5\text{-year}}$ (mm/hr)	$Q_{\text{actual}}$ (l/s)	$Q_{\text{allowable}}$ (l/s)	$Q_{\text{stored}}$ (l/s)	$V_{\text{stored}}$ (m <sup>3</sup> )
10	104.2	101.4	18.3	83.1	49.9
15	83.6	81.3	18.3	63.0	56.7
20	70.3	68.4	18.3	50.1	60.1
25	60.9	59.3	18.3	41.0	61.5
30	53.9	52.5	18.3	34.2	61.5
35	48.5	47.2	18.3	28.9	60.7
40	44.2	43.0	18.3	24.7	59.3
45	40.6	39.5	18.3	21.2	57.4
50	37.7	36.6	18.3	18.4	55.1
55	35.1	34.2	18.3	15.9	52.4
60	32.9	32.1	18.3	13.8	49.6

*peak V<sub>stored</sub> --->*

Therefore **62** m<sup>3</sup> estimated roof detention

**Table 1.3.2b - 100-year estimated detention on new roof**

Time (min)	$i_{100\text{-year}}$ (mm/hr)	$Q_{\text{actual}}$ (l/s)	$Q_{\text{allowable}}$ (l/s)	$Q_{\text{stored}}$ (l/s)	$V_{\text{stored}}$ (m <sup>3</sup> )
10	178.6	193.1	18.3	174.8	104.9
15	142.9	154.5	18.3	136.2	122.6
20	120.0	129.7	18.3	111.4	133.7
25	103.8	112.3	18.3	94.0	141.0
30	91.9	99.3	18.3	81.1	145.9
35	82.6	89.3	18.3	71.0	149.1
40	75.1	81.3	18.3	63.0	151.1
45	69.1	74.7	18.3	56.4	152.2
50	64.0	69.2	18.3	50.9	152.6
55	59.6	64.5	18.3	46.2	152.4
60	55.9	60.4	18.3	42.1	151.7

*peak V<sub>stored</sub> --->*

Therefore **153** m<sup>3</sup> estimated roof detention

**B.1.4 - Site storage**

	100-year event		
overall site storage requirements	190	m <sup>3</sup>	Table B.1.3.1
Roof storage requirements	153	m <sup>3</sup>	
estimated roof ponding volume	153	m <sup>3</sup>	Table B.1.3.2
roof ponding depth	0.118	m	maximum allowable: 0.15m
estimated parking area and school yard volume	184	m <sup>3</sup>	
maximum parking area and school yard ponding depth	0.25	m	maximum allowable: 0.25m
Sportsfield ponding volume	6	m <sup>3</sup>	Table B.1.3.4
Maximum sportsfield ponding depth	0.15	m	maximum allowable: 0.30m
Total available roof storage	153	m <sup>3</sup>	at maximum ponding depth of 0.15m
Total available parking area and school yard storage	190	m <sup>3</sup>	
Total available onsite storage > overall storage requirements	<u>OK</u>		
Total available onsite storage > estimated detention	<u>OK</u>		

**B.1.5 - Release rate for site**

<u>Release rate</u>		
Allowable release rate (5-year)	575.7	Section B.1.1
Uncontrolled release rate for (100-yr)	138.6	Section B.1.2.1
Controlled release rate at roof drain (100-yr)	18.3	Section B.1.3.2
Controlled release rate at CBMH-5 (100-yr)	300.0	Section B.1.3.3
Controlled release rate at CBMH-7 (100-yr)	60.0	
Total release rate (100-yr)	516.9	
Total release rate (100-yr) < Allowable release rate (5-year)	<u>OK</u>	

Fernbank Elementary School



Storm Sewer Pipe Design  
Without ICD Flow Control

Definitions  
Manning's Coefficient = 0.013  
Return Frequency (Yrs) = 5  
1 acre = 0.4047 hectares

Rational Method  
Q = 2.78 CIA (l/s), where  
C = Runoff Coefficient  
i = Rainfall Intensity (mm/hr)  
A = Areas in Hectares (ha)

Notes  
1) Used City of Ottawa IDF Curve  
2) Min. velocity = 0.8 m/sec  
3) Max. velocity = 6.0 m/sec

Designed BK  
Checked SM  
Dwg. Reference C1  
Jp2g project No 19-5070A

hide

LOCATION		AREA (ha)		FLOW				SEWER DATA											
From	To	C=	Individual	Cum.	tc (min.)	i <sub>5-year</sub> (mm/hr)	i <sub>100 years</sub> (mm/hr)	Flow <sub>5-year</sub> (l/s)	Flow <sub>100 years</sub> (l/s)	Dia. (mm)	actual dia	Slope (%)	Length (m)	Capacity (full) (l/s)	Velocity (full) (m/s)	Sect. Time (minutes)	Tot. Time (minutes)	Utilization (%)	
		0.90	2.78CA	2.78CA															
CB-1	CBMH1	0.094	0.24	0.24	10.0	104.2	178.6	24.5	42.0	250	250	0.65	30.6	47.9	1.0	0.5	10.5	51.1	
	CBMH2	0.103	0.26	0.49	10.5	101.5	173.9	50.0	85.7	375	381	0.40	25.1	115.7	1.0	0.4	10.9	43.3	
	CB-2	0.096	0.24	0.24	10.0	104.2	178.6	25.0	42.9	250	250	0.65	30.6	47.9	1.0	0.5	10.5	52.2	
	CBMH2	0.104	0.26	0.99	10.9	99.5	170.4	98.8	163.3	375	381	0.50	32.4	129.3	1.1	0.5	11.4	76.4	
	RYCB1	0.004	0.02	0.02	10.0	104.2	178.6	2.1	3.5	250	250	1.00	15.5	59.5	1.2	0.2	10.2	3.5	
	CBMH3	0.114	0.32	1.33	11.4	97.2	166.5	129.6	222.0	450	457	0.40	40.0	187.9	1.1	0.6	12.0	69.0	
	CBMH4	0.151	0.41	1.75	12.0	94.7	162.1	165.3	283.0	600	610	0.35	39.5	379.6	1.3	0.5	12.5	43.5	
	CBMH5	0.160	0.46	2.20	12.5	92.5	158.4	204.0	346.1	600	610	0.35	35.7	379.6	1.3	0.5	13.0	53.7	
	Roof *	0.389	0.97	0.97	10.0	104.2	178.6	101.4	173.7	300	300	4.00	20.8	193.4	2.7	0.1	10.1	52	
	CBMH8	0.087	0.24	1.22	12.5	92.5	158.4	112.5	192.6	450	457	0.35	17.1	175.7	1.1	0.3	12.8	64.0	
	TCB1	0.479	0.33	0.33	15.0	83.6	142.9	27.8	47.6	250	250	0.50	116.5	42.0	0.9	2.3	17.3	66.2	
	RYCB2	0.128	0.15	0.48	17.3	76.9	131.4	37.1	63.3	300	300	0.50	20.0	68.4	1.0	0.3	17.6	54.2	
	TCB12	0.024	0.13	0.61	17.6	76.0	129.8	46.7	79.7	300	300	0.50	25.0	68.4	1.0	0.4	18.0	68.3	
	TCB16	0.024	0.14	0.75	18.0	74.9	127.9	96.2	96.0	300	300	0.50	25.0	68.4	1.0	0.4	18.5	82.2	
	TCB13	0.110	0.14	0.69	18.5	73.8	126.1	65.5	111.8	375	375	0.30	25.0	96.0	0.9	0.5	19.0	68.2	
	CBMH7***	0.085	0.06	0.81	18.5	73.8	126.1	99.8	102.1	375	375	0.30	35.0	96.0	0.9	0.7	19.1	62.2	
	CBMH6	0.064	0.04	3.06	13.0	90.7	155.2	277.4	474.8	600	610	0.50	32.4	453.7	1.5	0.4	13.3	61.1	
	CBMH9	0.032	0.09	1.30	12.8	91.5	156.5	119.1	203.9	450	457	0.35	28.2	175.7	1.1	0.4	13.2	67.8	
	CBMH10	0.039	0.10	4.46	13.0	90.7	155.2	404.8	692.8	825	825	0.35	20.3	849.2	1.6	0.2	13.2	47.7	

Flow control to be installed at outlet

Notes:  
\* Flow from controlled roof drains is limited to 18.3 L/s  
\*\* Flow restricted to 300 L/s  
\*\*\* Flow from infiltration trench - restricted to 60L/s



**Appendix C - Stormwater Management Calculations**  
**Watts Drainage Adjustable Flow Control for Roof Drains – Data Sheet**



# RD-100

Tag: \_\_\_\_\_

## Large Capacity Roof Drain

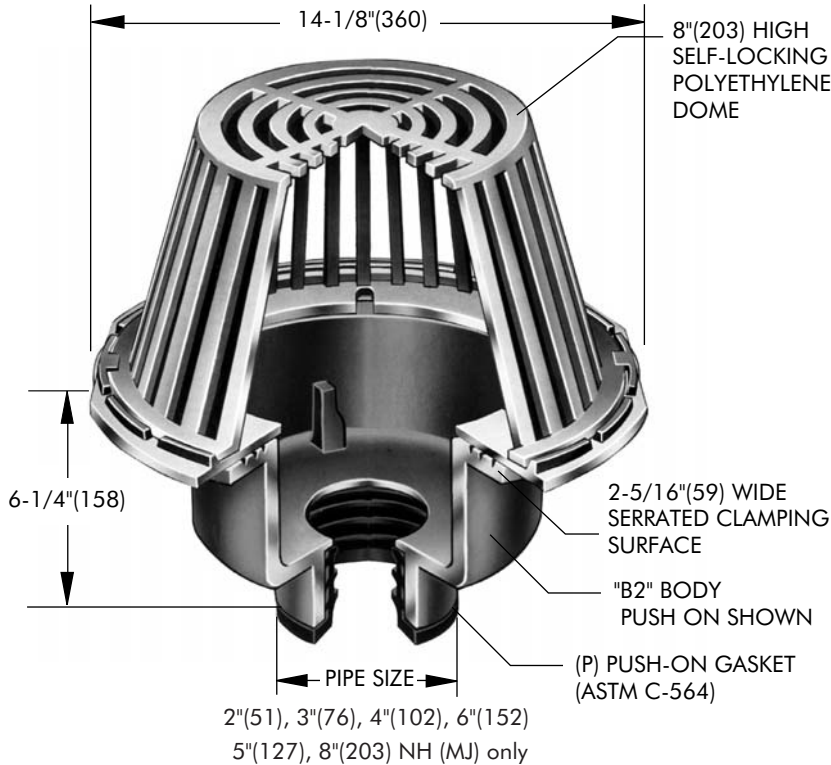
### Components:



**SPECIFICATION:** Watts Drainage Products RD-100 epoxy coated cast iron roof drain with deep sump, wide serrated flashing flange, flashing clamp device with integral gravel stop and self-locking polyethylene (standard) dome strainer.

Order Code: RD-10   -  -

Ex. RD-102P-K



Free Area Sq. In.
137

Deck opening 10" (254)  
with sump receiver 13-1/4" (337)

Pipe Sizing (Select One)		
Suffix	Description	
2	2"(51) Pipe Size	<input type="checkbox"/>
3	3"(76) Pipe Size	<input type="checkbox"/>
4	4"(102) Pipe Size	<input type="checkbox"/>
5	5"(127) Pipe Size	<input type="checkbox"/>
6	6"(152) Pipe Size	<input type="checkbox"/>
8	8"(203) Pipe Size	<input type="checkbox"/>

Outlet Type (Select One)		
Suffix	Description	
NH	No Hub (MJ)	<input type="checkbox"/>
P	Push On	<input type="checkbox"/>
T	Threaded Outlet	<input type="checkbox"/>
X	Inside Caulk	<input type="checkbox"/>

Options (Select One or More)		
Suffix	Description	
-A	Accutrol weir (specify # 1-6 slots)	<input type="checkbox"/>
-B	Sump Receiver Flange	<input type="checkbox"/>
-BED	Sump Receiver, Adj Ext., Deck Clamp	<input type="checkbox"/>
-C	Secondary Membrane Clamp	<input type="checkbox"/>
-D	Underdeck Clamp	<input type="checkbox"/>
-E	Adjustable Extension	<input type="checkbox"/>
-GSS	Stainless Steel Ballast Guard	<input type="checkbox"/>
-H	Adj. to 6" IRMA Ballast Guard	<input type="checkbox"/>
-K	Ductile Iron Dome	<input type="checkbox"/>
-K80	Aluminum Dome	<input type="checkbox"/>
-L	Vandal Proof Dome	<input type="checkbox"/>
-R	2" High External Water Dam	<input type="checkbox"/>
-SO	Side Outlet**	<input type="checkbox"/>
-V	Fixed Extension (1-1/2", 2", 3", 4")	<input type="checkbox"/>
-W	Adj. Water Level Regulator	<input type="checkbox"/>
-W-1	Waterproofing Flange	<input type="checkbox"/>
-Z	Extended Integral Wide Flange	<input type="checkbox"/>
-5	Sediment Bucket	<input type="checkbox"/>
-12	Galvanized Dome	<input type="checkbox"/>
-13	All Galvanized	<input type="checkbox"/>
-83	Mesh Covered Dome	<input type="checkbox"/>
-113M	Special Epoxy from 3M Range	<input type="checkbox"/>

Optional Body Material (NH Only)		
Suffix	Description	
-60	PVC Body w/Socket Outlet	<input type="checkbox"/>
-61	ABS Body w/Socket Outlet	<input type="checkbox"/>

\*\* Side Outlet (-SO) option only available in 2"(51), 3"(76), 4"(102) pipe sizes.  
Underdeck Clamp (-BED and -D options) are not available when -SO is selected.

Job Name \_\_\_\_\_ Contractor \_\_\_\_\_

Job Location \_\_\_\_\_ Contractor's P.O. No. \_\_\_\_\_

Engineer \_\_\_\_\_ Representative \_\_\_\_\_

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without incurring any obligation to make similar changes and modifications to products previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances.



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