



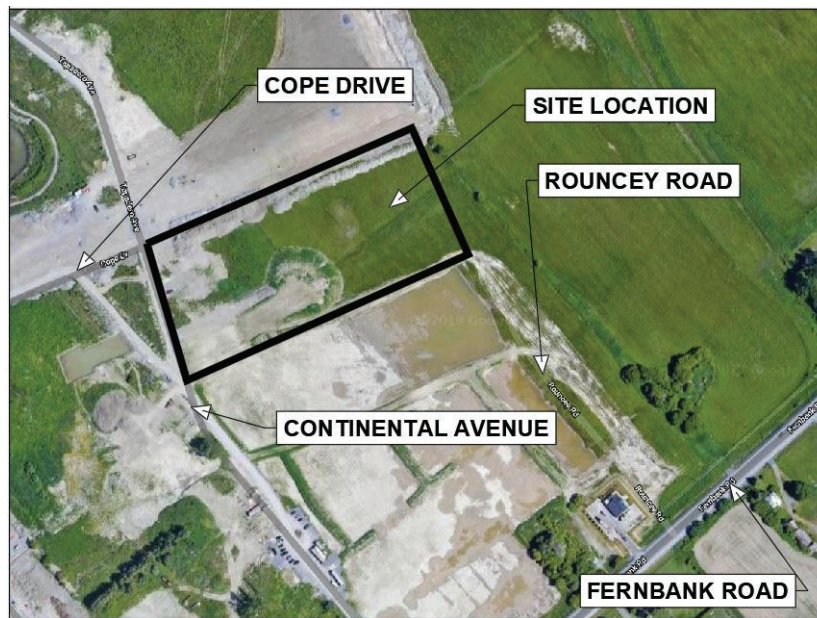
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Project No. 19-5070A

Stormwater Management Report

Fernbank Elementary School

480 Cope Drive, Ottawa, Ontario



Prepared for



City of Ottawa
Infrastructure Services and Community Sustainability
110 Laurier Ave. West, 4th floor, Mail Code 01-14
Ottawa, Ontario, K1P 1J1

November 28, 2019

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1 Introduction

Jp2g Consultants Inc. was retained by N45 Architecture Inc. to complete a Stormwater Management Report suitable for City of Ottawa Site Plan Control Application, for the Ottawa Carleton District School Board's proposed Elementary School and Daycare Addition at Cope Drive and Rouncey Road, in the City of Ottawa. The total site area is approximately 2.84 ha and the proposed site development area includes the construction of a 360 m² one-storey day-care building, associated parking areas, play area, and landscaped areas. A pre-Consultation meeting was held with City of Ottawa staff on August 23, 2019, to determine the project constraints and requirements. The following report details the stormwater management calculations used for water quantity and quality control in accordance with the City of Ottawa's requirements.

Reference Drawings: SD1 – Stormwater Management Sub-Drainage Areas, C02 – Servicing Tables, C03 - Site Servicing Plan (November 28, 2019), and C04 - Site Grading and Drainage, Erosion and Sediment Control Plan (November 28, 2019).

2 Objective

The objective of the stormwater management plan is to control post-development peak flows to pre-determined levels, and detain onsite, stormwater up to and including the 100-year storm event without affecting adjacent lands. Stormwater quality control will be provided by the downstream pond 6, no onsite quality control is required.

3 Design Parameters

Stormwater management criteria for this site, in terms of quantity control, is based on the following approved reports:

- Servicing and Stormwater Management Report – Blackstone Community Phase 4-8 by Stantec, April 11, 2019.

The maximum allowable release rate for this site shall not exceed the criteria set in the approved servicing report. Flows in excess of the allowable release rate up to and including the 100-year event will be detained onsite.

The Modified Rational Method ($Q = 2.78CiA$) was chosen to calculate the post-development release rates, and onsite storage requirements for this development. Detailed stormwater management calculations are included in **Appendix B**. All proposed storm sewers were assigned a Manning's coefficient of roughness of 0.013 corresponding to smooth wall pipes. In accordance with City of Ottawa Sewer Design Guidelines (Section 5.4.5.2.1), the coefficients used for calculating the post-development release rate were $C = 0.25$ for grassed areas and $C = 0.90$ for hard surfaced areas including rooftops. The rainfall intensities used in this analysis are based on the IDF curves and equations, as per City of Ottawa Sewer Design Guidelines (Section 5.4.2).

4 Water Quantity Controls

4.1 Pre-Development Conditions

The existing site is an undeveloped parcel with a generally flat site topography that is sloped toward the east side of the property with an approximate elevation difference of 0.5 meter over approximately a 220-meter length. The proposed day-care facility will be constructed at the same time as the main school building. Services have been installed along Rouncey Road, as part of the development of the Blackstone Community Phase 4-8.

4.2 Allowable Release Rate

The stormwater management design criteria for this site is based on the subdivision servicing report as noted above. According to the Stantec study, the school site has an allowable release rate up to the 5 year-event of **575.7 L/s**. Refer to **Appendix A** – Background excerpts – Storm Water Management.

Minimum storage requirement was set to **50 m³ / ha** per the Subdivision study.

4.3 Post-Development Conditions

Proposed site grading and drainage of the overall site was designed such that stormwater runoff will be collected by the roof and a new storm water collection system which will be connected to the existing 1200mm diameter municipal storm sewer on Rouncey Road and discharged to Pond 6 of the subdivision.

The storm sewer system consists of manholes, catchbasins, storm sewers, perforated subdrain system in landscaped area of the sportsfield. Most of the drainage areas are piped to the storm system except for the grassed area fronting onto Cope Drive.

No surface ponding will occur during the 5-year event. Flows exceeding the allowable release rate up to the 100-year event are to be detained on site.

The overall site development area is approximately **2.84 ha** and has a post-development average weighted runoff coefficient of **C=0.60**, and **C=0.65** for the maximum and 100-year events, respectively. Overall onsite storage requirement was calculated to be **254 m³** for the 100-year event, which exceeds the minimum storage requirement of 50m³/ha.

4.4 Onsite Stormwater Detention

Stormwater detention is proposed on the school roof, in the proposed parking area, in the paved school yard as well as along the sportsfield swale.

The maximum allowable ponding depth will be limited to 250mm for paved areas and 300mm in grassed areas during 100-year events. The maximum ponding elevation is 100.85 which is below the building finished floor elevation of 101.25.

Flow is to be restricted by installing an IPEX Tempest orifice plate at CBMH6 with a discharge rate of **530.0 L/s** at an estimated head **2.11 m**.

Flow from the combined school and daycare roof will also be detained on the roof by installing parabolic weirs, (Watts Drainage Adjustable Flow Control for Roof Drains, or equivalent approved products), at the 29 proposed roof drains limiting the flow from the roof to **18.3 L/s**. The resulting required storage is **153 m³** for the 100-year event. Based on a maximum ponding depth of 150mm on the roof, the total available storage is approximately **194 m³**, which is sufficient to accommodate the 100-year event. Each flow control roof drain, complete with a single slot parabolic weir, will restrict flow at 5 GPM (0.32 L/s) per inch (25.4mm) of head to a maximum of 10 GPM (0.63 L/s). The restricted flow will outlet to the school's 300mm diameter storm sewer service at 4.0% slope. The storm sewer service is connected to STMH1, upstream of the ICD; a backflow preventer valve will be installed in the mechanical room.

The following table summarizes on-site requirements during the 100-year event:

Total Area	Controlled /Uncontrolled	Run-off Coefficient		Outlet Location	Total Storage Provided	100 Year event	
		5 year	100 year			Restricted Flow	Required Storage
2.611	Controlled	0.62	0.67	STMH1	386	530	206
0.227	Uncontrolled	0.37	0.39	Cope Drive ROW		43.8	0
2.838					386	573.8	206

Note that storage within the subdrain clearstone trench was not included within the available storage volume. There is sufficient available ponding to accommodate the 100-year event.

The maximum ponding limits are indicated on **SD1** and grading plan. In the event of a rainfall exceeding the 100-year event, runoff will remain on-site until ponding reaches the overspill elevation. In such event, the school yard and sport field major overland flow route will overflow towards Rouncey Drive and the parking area will overflow to Continental Avenue. The grade elevation at the overflow point is greater than **0.30 m** below the school's finished floor elevation.

4.5 Proposed Release Rates

The proposed release rate for this site during the 100-year event, including uncontrolled flows (**43.8 l/s**) and controlled flows (**530 L/s**) into the minor system of is **573.8 L/s**. Therefore, proposed release rates are within the allowable release rate for this site, determined to be **575.7 L/s** in Section 4.2.

5 Erosion and Sediment Control

In accordance with City of Ottawa requirements, best management practices are to be implemented by the Contractor to provide protection of the area drainage system and the receiving water course, during construction activities. This includes limiting the amount of exposed soil, using filter bag inserts under the

grates of catch basins and manholes, installing silt fences and other effective sediment traps, and installing and maintaining mud mats for outgoing construction traffic during construction activities.

6 Conclusion

The proposed site development includes a new school building with an attached daycare, a bus lay-by, asphalt parking, hard surface walkways and play areas, landscaped areas, a sports field and an area for portables. Roof drainage and surface runoff will be collected by a new storm sewer system which will be connected to the existing 1200mm diameter municipal storm sewer located on Rouncey Road. Post-development peak flows will be detained on the roof, in the parking area, in the school yard and in the bus lay-by in order to limit the post-development release rate to allowable levels. There is sufficient onsite storage to accommodate the 100-year event.

Summary of report

ID	Description	Value/result
01	Allowable release rate	$Q_{\text{allowable}} = 575.7 \text{ L/s}$
02	Proposed release rate	$Q_{100\text{-yr post}} = 573.8 \text{ L/s}$
03	Post-development runoff coefficient	$C_{5\text{-yr post}} = 0.60, C_{100\text{-yr post}} = 0.65$
04	Post-development onsite storage requirement	206 m ³
05	Proposed onsite storage	Parking lot and school yard: 191 m ³ , Roof: 153m ³ , Sportsfield: 34 m ³ Lay-by: 8m ³
06	Discharge outlet location	1200mm ϕ storm sewer on Rouncey Road to Pond 6
07	Emergency runoff overflow locations	West side – curb cut in parking area towards Continental Avenue East side – from school yard to layby towards Rouncey Road

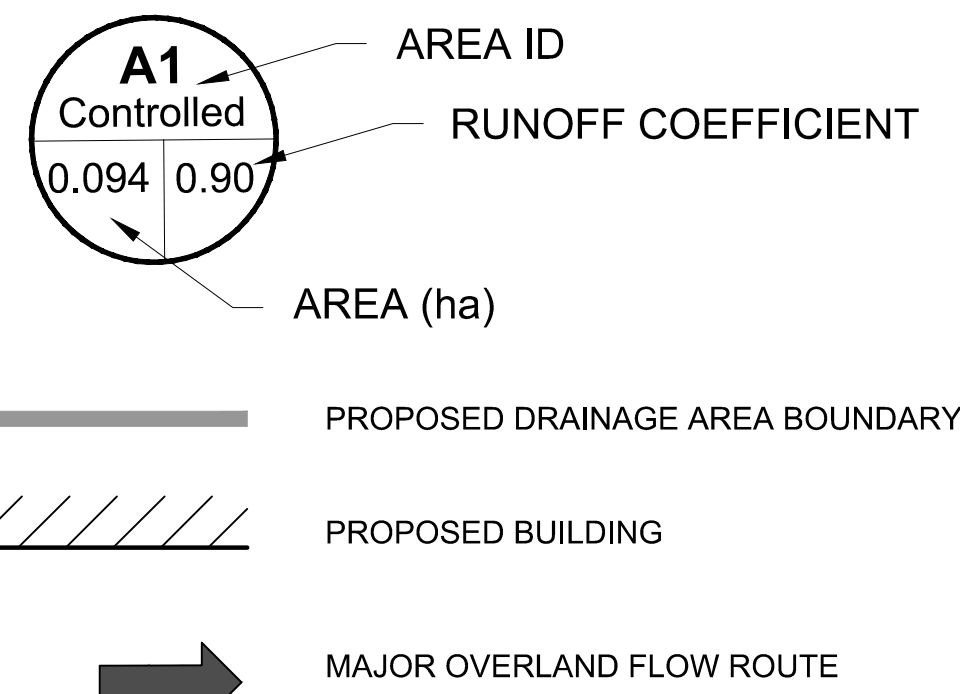
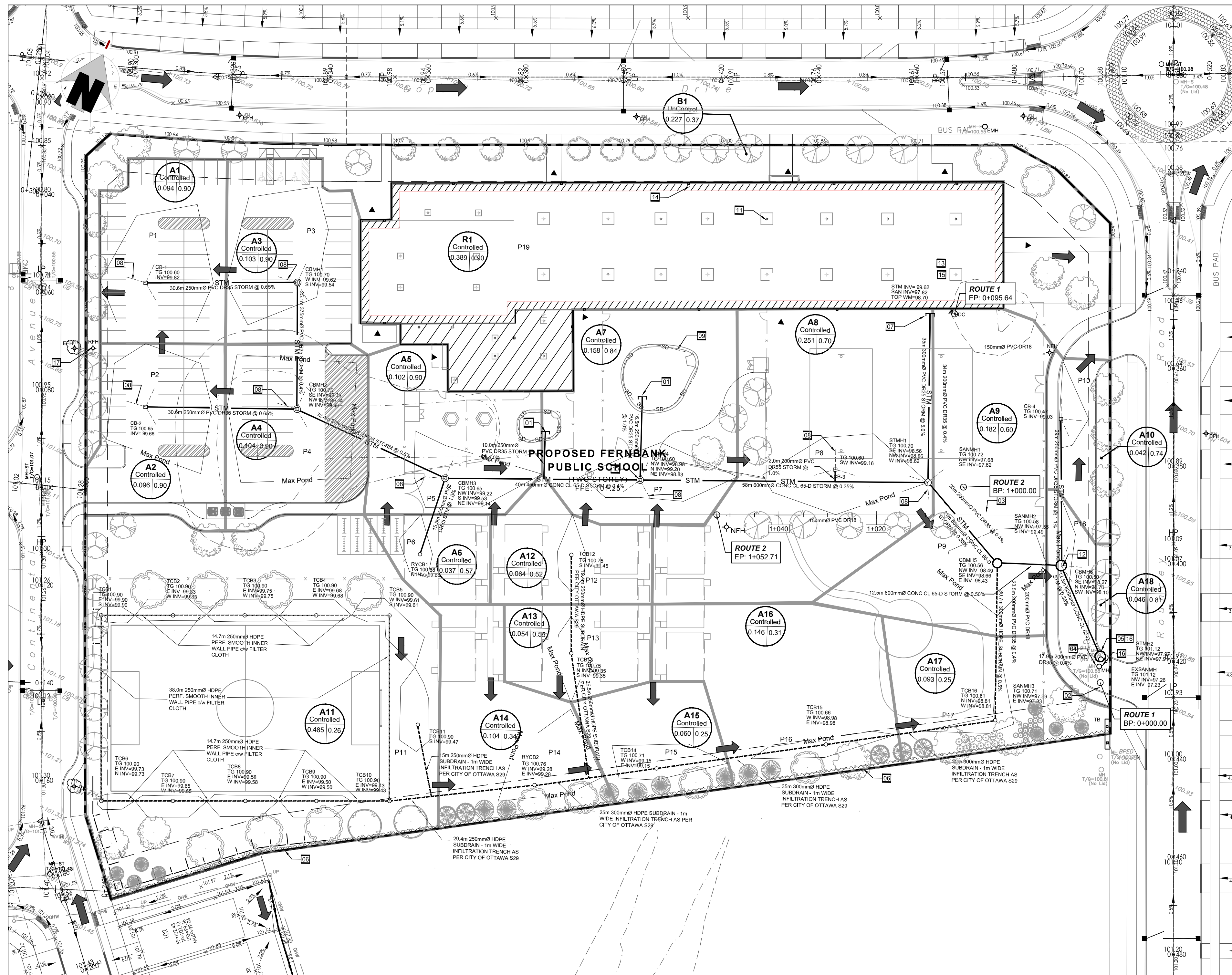
END OF REPORT



Barbra Kimmerle, P.Eng.
Civil Engineer

PONDING TABLE					
POND NO.	LOCATION	POND ELEV. (m)	TOP OF CB ELEV. (m)	POND DEPTH (m)	POND VOL. (m ³)
P1	CB1	100.85	100.60	0.25	35.21
P2	CB2	100.90	100.67	0.23	31.99
P3	CBM1	100.90	100.70	0.20	28.37
P4	CBM2	100.92	100.72	0.20	41.03
P5	CBM3	100.85	100.65	0.20	15.50
P6	RYCB1	100.90	100.68	0.22	3.89
P7	CBM4	100.75	100.60	0.15	11.20
P8	CB3	100.70	100.55	0.15	16.95
P9	CBM5	100.70	100.56	0.14	10.85
P10	CB4	100.59	100.47	0.12	2.39
P11	TCB1 - TCB11	100.96	100.90	0.06	10.36
P12	TCB12	100.90	100.75	0.15	2.96
P13	TCB13	100.90	100.75	0.15	3.61
P14	RYCB2	100.89	100.76	0.13	10.00
P15	TCB14	100.86	100.71	0.15	6.24
P16	TCB15	100.81	100.66	0.15	3.76
P17	TCB16	100.76	100.61	0.15	3.31
P18	CBM6	100.62	100.50	0.12	5.41
P19	ROOF			0.12	153.01
Total=					396.04

NOTE: PONDING VOLUME CALCULATED BY MULTIPLYING THE PONDING AREA BY THE MAXIMUM PONDING DEPTH AND DIVIDING BY 3 FOR A CONICAL POND



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1	ISSUED FOR SITE PLAN CONTROL	2019-11-28
No.	DESCRIPTION	YYYY-MM-DD

N45 ARCHITECTURE INC.
 43 Eccles Street, 2nd Floor - Ottawa, Ontario, K1R 6S3
 tel. 613.224.0095 fax 613.224.9811

project

seal

drawing title

STORM DRAINAGE PLAN

scale	1:500	drawn by	M.S.
date	NOV 2019	checked by	B.K.
project number		drawing number	SD1

CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE WORK COMMENCES. DO NOT SCALE DRAWINGS.

revision

Appendix A - Background Excerpts Stormwater Management

Appendix B - Stormwater Management Calculations

Fernbank Elementary School - 480 Cope Drive



B.1.1 - Allowable release rate

Using the criteria for the site from the subdivision Final Serviceability Report Development by Stantec (April 2019), the maximum allowable release rate is up to the 5 year discharge rate.

$$Q_{\text{allowable}} = Q = 2.78 C / A$$

$$Q_{\text{allowable}} = 575.7 \quad \text{l/s} \quad \text{①}$$

B.1.2 - Post-development release rate

ID	Description	Type	Areas (m ²)		Total (m ²)	C _{post-5-year}	C _{post-100-yr} *
			C _{0.90}	C _{0.25}			
B1	Front of building	uncontrolled	419	1854	2273	0.37	0.39
A1	Western parking area 1	controlled	945		945	0.90	1.00
A2	Western parking area 2	controlled	960		960	0.90	1.00
A3	Western parking area 3	controlled	1033		1033	0.90	1.00
A4	Western parking area 4	controlled	1041		1041	0.90	1.00
A5	Playground at Daycare area 1	controlled	1022		1022	0.90	1.00
A6	Future portables area 1	controlled	182	187	369	0.57	0.62
A7	Playground at Daycare area 2	controlled	1433	145	1578	0.84	0.93
A8	Courts area 1	controlled	1720	786	2506	0.70	0.76
A9	Courts area 2	controlled	970	845	1815	0.60	0.65
A10	Bus loop area 1	controlled	317	106	423	0.74	0.81
A11	Sports Field	controlled	63	4791	4854	0.26	0.26
A12	Future portables area 2	controlled	265	374	639	0.52	0.56
A13	Future portables area 3	controlled	246	296	542	0.55	0.59
A14	Back lot area 1	controlled	142	904	1046	0.34	0.35
A15	Back lot area 2	controlled		598	598	0.25	0.25
A16	Back lot area 3	controlled	131	1327	1458	0.31	0.32
A17	Back lot area 4	controlled		933	933	0.25	0.25
A18	Bus loop area 2	controlled	391	65	456	0.81	0.89
R1	Roof	controlled	3890		3890	0.90	1.00
			15170	13211	28381	0.60	0.65

*including 25% increase as per City of Ottawa Sewer Design Guidelines

Calculations for post-development runoff coefficient

$$C_{\text{post-5-year (col. D)}} = (\text{column A} * 0.9 + \text{column B} * 0.2) / \text{column C}$$

$$C_{\text{post-100-yr (col. E)}} = (\text{column A} * 1.0 + \text{column B} * 0.2 * 1.25) / \text{column C}$$

note: $0.90 \times 1.25 = 1.125$, use max. 1.0

Calculations for average weighted runoff coefficient

$$C_{\text{post-5-year}} = ((15170 * 0.9) + (13211 * 0.25)) / 28381 = 0.60$$

$$C_{\text{post-100-yr}} = ((15170 * 1.0) + (13211 * 0.25 * 1.25)) / 28381 = 0.65$$

Estimated time of concentration, t_c =

10.0 minutes

***As per City of Ottawa Sewer Design Guidelines (Section 5.4.5.2)

Based on Ottawa IDF curve, $i_{5\text{-year}}$ =

$$998.071 / (t_c + 6.053)^{0.814}$$

104.2 mm/hr

Based on Ottawa IDF curve, $i_{100\text{-years}}$ =

$$1735.688 / (t_c + 6.014)^{0.820}$$

178.6 mm/hr

B.1.2.1 - uncontrolled flow

Total uncontrolled area =

0.227 ha

5-year Runoff coefficient, C =

0.37

100-year Runoff coefficient, C =

0.39

Estimated time of concentration, t_c =

10.0 minutes

$$Q_{\text{uncontrolled 5-year}} = 24.3 \quad \text{l/s} \quad \text{②}$$

$$Q_{\text{net-allowable 5-year}} = 551.4 \quad \text{l/s} \quad \text{③} = \text{①} - \text{②}$$

$$Q_{\text{uncontrolled 100-year}} = 43.8 \quad \text{l/s} \quad \text{④}$$

$$Q_{\text{net-allowable 100-year}} = 531.9 \quad \text{l/s} \quad \text{⑤} = \text{①} - \text{④}$$

B.1.3 - Post-development onsite storage

A.1.3.1 - Overall onsite storage requirements

Total controlled area, A1 to A18 & R1	2.611	ha	
5-year Runoff coefficient, C	0.62		
100-year Runoff coefficient, C	0.67		
net-allowable release rate	530.0	l/s	⑤

Table 1.3.1a - 5-year onsite storage requirements

	Time (minutes)	$i_{5\text{-year}}$ (mm/hr)	Q_{actual} (l/s)	$Q_{\text{allowable}}$ (l/s)	Q_{stored} (l/s)	V_{stored} (m ³)
<i>peak V_{stored} ----></i>	10	104.2	466.8	530.0	-63.2	-37.9
	15	83.6	374.3	530.0	-155.7	-140.1
	20	70.3	314.7	530.0	-215.3	-258.3
	25	60.9	272.8	530.0	-257.2	-385.8
	30	53.9	241.6	530.0	-288.4	-519.1
	35	48.5	217.4	530.0	-312.6	-656.5
	40	44.2	197.9	530.0	-332.1	-796.9
	45	40.6	182.0	530.0	-348.0	-939.6
	50	37.7	168.7	530.0	-361.3	-1083.9
	55	35.1	157.4	530.0	-372.6	-1229.7
	60	32.9	147.6	530.0	-382.4	-1376.7

Therefore **-38** m³ of onsite storage required during 5-year event

Table 1.3.1b - 100-year onsite storage requirements

	Time (min)	$i_{100\text{-year}}$ (mm/hr)	Q_{actual} (l/s)	$Q_{\text{allowable}}$ (l/s)	Q_{stored} (l/s)	V_{stored} (m ³)
<i>peak V_{stored} ----></i>	10	178.6	873.2	530.0	343.2	205.9
	15	142.9	698.8	530.0	168.8	151.9
	20	120.0	586.6	530.0	56.6	67.9
	25	103.8	507.8	530.0	-22.2	-33.3
	30	91.9	449.2	530.0	-80.8	-145.4
	35	82.6	403.8	530.0	-126.2	-265.0
	40	75.1	367.5	530.0	-162.5	-390.1
	45	69.1	337.7	530.0	-192.3	-519.3
	50	64.0	312.7	530.0	-217.3	-651.8
	55	59.6	291.6	530.0	-238.4	-786.8
	60	55.9	273.3	530.0	-256.7	-924.0

Therefore **206** m³ of onsite storage required during 100-year event

B.1.3.2 - Estimated detention created by installing roof weirs

Total roof area, A3	0.389	ha	
5-year Runoff coefficient, C	0.90		
100-year Runoff coefficient, C	1.00		
Install 0.6309 l/s weirs at each of the 29 roof drains	18.3	l/s	<i>Watts Drainage Adjustable Flow Control for Roof Drains, or approved equivalent</i>

Table 1.3.2a - 5-year estimated detention on new roof

Time (minutes)	$i_{5\text{-year}}$ (mm/hr)	Q_{actual} (l/s)	$Q_{\text{allowable}}$ (l/s)	Q_{stored} (l/s)	V_{stored} (m ³)
10	104.2	101.4	18.3	83.1	49.9
15	83.6	81.3	18.3	63.0	56.7
20	70.3	68.4	18.3	50.1	60.1
25	60.9	59.3	18.3	41.0	61.4
30	53.9	52.5	18.3	34.2	61.5
35	48.5	47.2	18.3	28.9	60.7
40	44.2	43.0	18.3	24.7	59.3
45	40.6	39.5	18.3	21.2	57.3
50	37.7	36.6	18.3	18.3	55.0
55	35.1	34.2	18.3	15.9	52.4
60	32.9	32.1	18.3	13.8	49.5

peak V_{stored} --->

Therefore **62** m³ estimated roof detention

Table 1.3.2b - 100-year estimated detention on new roof

Time (min)	$i_{100\text{-year}}$ (mm/hr)	Q_{actual} (l/s)	$Q_{\text{allowable}}$ (l/s)	Q_{stored} (l/s)	V_{stored} (m ³)
10	178.6	193.0	18.3	174.8	104.9
15	142.9	154.5	18.3	136.2	122.6
20	120.0	129.7	18.3	111.4	133.7
25	103.8	112.3	18.3	94.0	141.0
30	91.9	99.3	18.3	81.0	145.8
35	82.6	89.3	18.3	71.0	149.1
40	75.1	81.2	18.3	62.9	151.1
45	69.1	74.7	18.3	56.4	152.2
50	64.0	69.1	18.3	50.8	152.5
55	59.6	64.5	18.3	46.2	152.3
60	55.9	60.4	18.3	42.1	151.7

peak V_{stored} --->

Therefore **153** m³ estimated roof detention

B.1.4 - Site storage

	100-year event		
overall storage requirements	206	m ³	Table B.1.3.1
estimated roof ponding volume	153	m ³	Table B.1.3.2
roof ponding depth	0.118	m	maximum allowable: 0.15m
estimated parking area and school yard volume	191	m ³	
maximum parking area and school yard ponding depth	0.25	m	maximum allowable: 0.25m
estimated bus lay-by	8	m ³	Table B.1.3.3
maximum bus lay-by	0.13	m	maximum allowable: 0.25m
Sportsfield ponding volume	34	m ³	Table B.1.3.4
Maximum sportsfield ponding depth	0.15	m	maximum allowable: 0.30m
Total available roof storage	153	m ³	at maximum ponding depth of 0.15m
Total available parking area and school yard storage	233	m ³	
Total available onsite storage > overall storage requirements	<u>OK</u>		
Total available onsite storage > estimated detention	<u>OK</u>		

B.1.5 - Release rate for siteRelease rate

Allowable release rate (5-year)	575.7	Section B.1.1
Uncontrolled release rate for (100-yr)	43.8	Section B.1.2.1
Controlled release rate at roof drain (100-yr)	18.3	Section B.1.3.2
Controlled release rate at CBMH-1 (100-yr)	530.0	Section B.1.3.3
Total release rate (100-yr)	573.8	
Total release rate (100-yr) < Allowable release rate (5-year)	<u>OK</u>	

Fernbank Elementary School - 480 Cope Drive



Storm Sewer Pipe Design
Without ICD Flow Control

Definitions

Manning's Coefficient = 0.013
Return Frequency (yrs) = 5
1 acre = 0.4047 hectares

Rational Method

$Q = 2.78 CIA (I/s)$, where
C = Runoff Coefficient
 i = Rainfall Intensity (mm/hr)
A = Areas in Hectares (ha)

Notes

1) Used City of Ottawa IDF Curve
2) Min. velocity = 0.8 m/sec
3) Max. velocity = 6.0 m/sec

Designed BK

Checked NC
Dwg. Reference C03
Jp2g project No 19-5070A

LOCATION		AREA (ha)		FLOW			SEWER DATA										
From	To	C=	C=	Individual	Cum.	$i_{5\text{-year}}$ (mm/hr)	$i_{100\text{-years}}$ (mm/hr)	Flow _{5-year} (l/s)	Flow _{100 years} (l/s)	Dia. (mm)	Slope (%)	Length (m)	Capacity (full) (l/s)	Velocity (full) (m/s)	Sect. Time (minutes)	Tot. Time (minutes)	Utilization (%)
CB-1	CBMH1	0.90	0.25	2.78CA	2.78CA	104.2	178.6	24.5	42.0	250	0.65	30.6	47.9	1.0	0.5	10.5	51.1
	CBMH2	0.094		0.24	0.24	10.5	173.9	47.7	81.8	375	0.40	25.1	115.7	1.0	0.4	10.9	41.3
	CBMH3	0.103		0.26	0.26	10.0	178.6	26.9	46.0	250	0.65	30.6	47.9	1.0	0.5	10.5	56.0
	CBMH4	0.103		0.26	0.99	10.9	170.4	98.1	168.0	375	0.50	32.4	129.3	1.1	0.5	11.4	75.8
	RYCB1	0.017	0.020	0.06	0.06	10.0	178.6	5.9	10.1	250	1.00	15.5	59.5	1.2	0.2	10.2	9.9
	CBMH3	0.102		0.26	1.30	11.4	166.5	126.2	216.1	450	0.40	40.0	187.9	1.1	0.6	12.0	67.2
	STMH1	0.329		0.82	2.12	12.0	162.1	200.7	343.7	600	0.35	58.0	379.6	1.3	0.8	12.8	52.9
Roof *	STMH1	0.389	0.000	0.97	0.97	10.0	178.6	101.4	173.7	300	4.00	19.0	193.4	2.7	0.1	10.1	52
	CBMH5			0.00	3.09	12.8	156.7	283.3	484.9	600	0.35	21.0	379.6	1.3	0.3	13.0	74.6
	TCB10			0.485	0.34	15.0	142.9	28.2	48.2	250	0.50	123.0	42.0	0.9	2.4	17.4	67.0
	RYCB2	0.065	A14+A12+A13	0.157	0.61	17.4	130.8	46.7	79.8	300	0.50	54.0	68.4	1.0	0.9	18.3	68.3
	TCB14		A15	0.060	0.04	18.3	126.7	48.3	82.5	300	0.50	35.0	68.4	1.0	0.6	18.9	70.7
	TCB15	0.137	A16	0.013	1.00	18.9	124.2	72.9	124.6	300	0.50	35.0	68.4	1.0	0.6	19.5	106.7
	TCB16**		A17	0.093	1.07	19.5	121.8	76.1	130.0	300	0.50	35.0	68.4	1.0	0.6	20.1	111.4
	CBMH5	0.158		0.40	4.56	13.0	154.9	412.4	705.8	600	0.50	32.4	453.7	1.5	0.4	13.4	90.9
	CBMH6	0.149		0.37	0.37	10.0	178.6	38.8	66.6	250	0.65	32.4	47.9	1.0	0.6	10.6	81.0
	STMH1	0.046		0.12	5.04	13.4	152.6	449.8	769.7	825	0.35	21.0	849.2	1.6	0.2	13.6	53.0

Flow control to be installed at outlet

- * Flow from controlled roof drains is limited to 18.3 L/s
- ** Flow restricted to 450 L/s
- *** Flow from subdrain and clear stone infiltration trench

Appendix C - Stormwater Management Calculations
Watts Drainage Adjustable Flow Control for Roof Drains – Data Sheet



RD-100

Tag: _____

Large Capacity Roof Drain

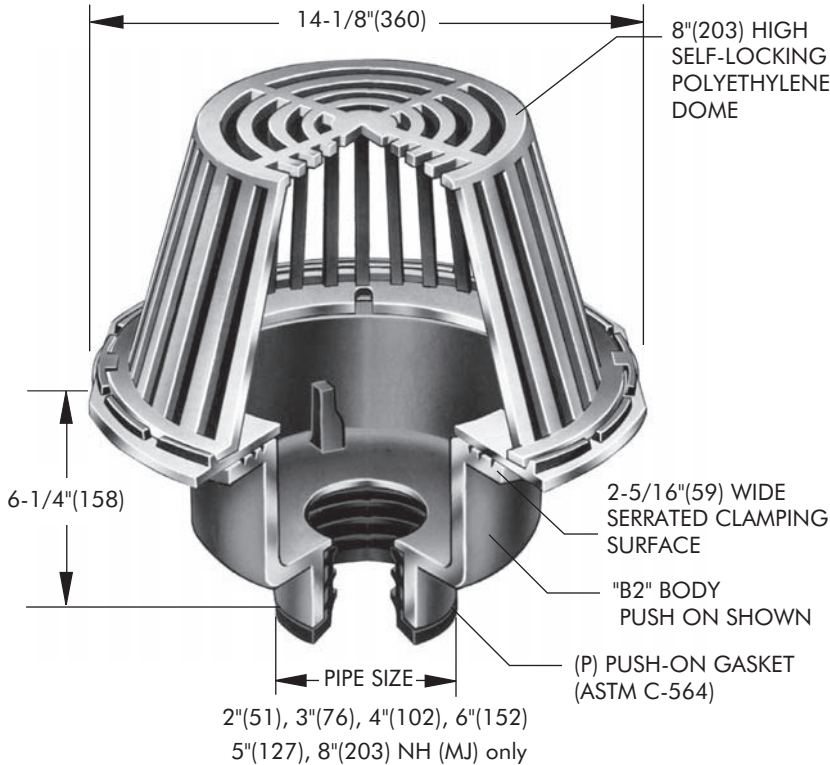
Components:



SPECIFICATION: Watts Drainage Products RD-100 epoxy coated cast iron roof drain with deep sump, wide serrated flashing flange, flashing clamp device with integral gravel stop and self-locking polyethylene (standard) dome strainer.

Order Code: RD-10 - -

Ex. RD-102P-K



Free Area Sq. In.
137

Deck opening 10" (254)
with sump receiver 13-1/4" (337)

Pipe Sizing (Select One)		
Suffix	Description	
2	2"(51) Pipe Size	<input type="checkbox"/>
3	3"(76) Pipe Size	<input type="checkbox"/>
4	4"(102) Pipe Size	<input type="checkbox"/>
5	5"(127) Pipe Size	<input type="checkbox"/>
6	6"(152) Pipe Size	<input type="checkbox"/>
8	8"(203) Pipe Size	<input type="checkbox"/>

Outlet Type (Select One)		
Suffix	Description	
NH	No Hub (MJ)	<input type="checkbox"/>
P	Push On	<input type="checkbox"/>
T	Threaded Outlet	<input type="checkbox"/>
X	Inside Caulk	<input type="checkbox"/>

Options (Select One or More)		
Suffix	Description	
-A	Accutrol weir (specify # 1-6 slots)	<input type="checkbox"/>
-B	Sump Receiver Flange	<input type="checkbox"/>
-BED	Sump Receiver, Adj Ext., Deck Clamp	<input type="checkbox"/>
-C	Secondary Membrane Clamp	<input type="checkbox"/>
-D	Underdeck Clamp	<input type="checkbox"/>
-E	Adjustable Extension	<input type="checkbox"/>
-GSS	Stainless Steel Ballast Guard	<input type="checkbox"/>
-H	Adj. to 6" IRMA Ballast Guard	<input type="checkbox"/>
-K	Ductile Iron Dome	<input type="checkbox"/>
-K80	Aluminum Dome	<input type="checkbox"/>
-L	Vandal Proof Dome	<input type="checkbox"/>
-R	2" High External Water Dam	<input type="checkbox"/>
-SO	Side Outlet**	<input type="checkbox"/>
-V	Fixed Extension (1-1/2",2",3",4")	<input type="checkbox"/>
-W	Adj. Water Level Regulator	<input type="checkbox"/>
-W-1	Waterproofing Flange	<input type="checkbox"/>
-Z	Extended Integral Wide Flange	<input type="checkbox"/>
-5	Sediment Bucket	<input type="checkbox"/>
-12	Galvanized Dome	<input type="checkbox"/>
-13	All Galvanized	<input type="checkbox"/>
-83	Mesh Covered Dome	<input type="checkbox"/>
-113M	Special Epoxy from 3M Range	<input type="checkbox"/>

Optional Body Material (NH Only)		
Suffix	Description	
-60	PVC Body w/Socket Outlet	<input type="checkbox"/>
-61	ABS Body w/Socket Outlet	<input type="checkbox"/>

** Side Outlet (-SO) option only available in 2"(51), 3"(76), 4"(102) pipe sizes.
Underdeck Clamp (-BED and -D options) are not available when -SO is selected.

Job Name _____ Contractor _____

Job Location _____ Contractor's P.O. No. _____

Engineer _____ Representative _____

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without incurring any obligation to make similar changes and modifications to products previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances.



CANADA: 5435 North Service Road, Burlington, ON, L7L 5H7 TEL: 905-332-6718 TOLL-FREE: 1-888-208-8927 Website: www.wattsdrainage.ca

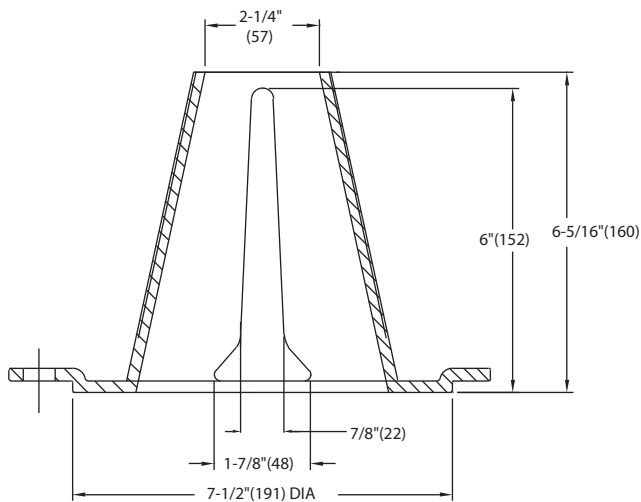


ACCUTROL WEIR FLOW CONTROL

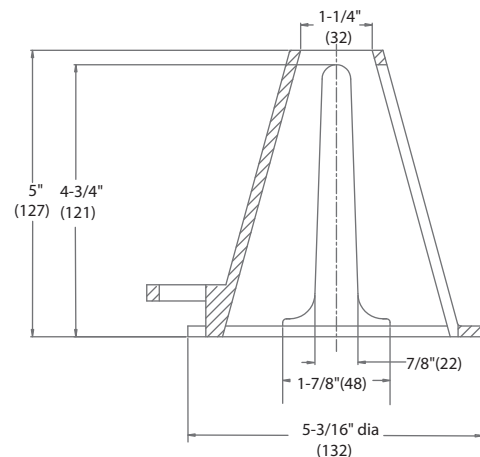
SPECIFICATION: Watts Drainage Products epoxy coated cast iron Accutrol Weir is designed with parabolic openings which limit the flow of rain water off a roof. Each weir slot controls flow to 5 gpm per inch of head to a maximum of 30 gpm at 6" head (for large sump), 25 gpm at 5" head (for small sump). The Accutrol Weir is secured to the flashing clamp of the roof drain. The Accutrol Weir is available with 1 to 4 slots for the large sump drain and up to 3 slots for the small sump drain.

For Large Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-100-A2" for two slot weir)

For Small Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-200-A1" for one slot weir)



LARGE SUMP ACCUTROL WEIR



SMALL SUMP ACCUTROL WEIR

Job Name _____ Contractor _____

Job Location _____ Contractor's P.O. No. _____

Engineer _____ Representative _____

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