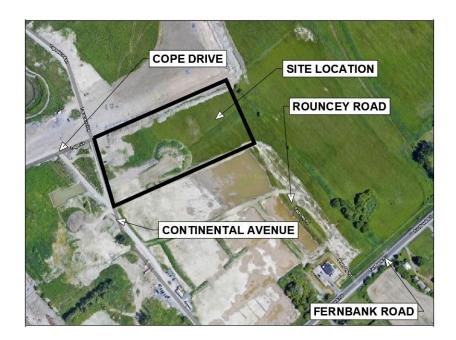


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# Stormwater Management Report Fernbank Elementary School

480 Cope Drive, Ottawa, Ontario



### Revision 2

Prepared for



City of Ottawa Infrastructure Services and Community Sustainability 110 Laurier Ave. West, 4th floor, Mail Code 01-14 Ottawa, Ontario, K1P 1J1

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### 1 Introduction

Jp2g Consultants Inc. was retained by N45 Architecture Inc. to complete a Stormwater Management Report suitable for City of Ottawa Site Plan Control Application, for the Ottawa Carleton District School Board's proposed Elementary School and Daycare Addition at Cope Drive and Rouncey Road, in the City of Ottawa. The total site area is approximately 2.84 ha and the proposed site development area includes the construction of a 360 m² one-storey day-care building, associated parking areas, play area, and landscaped areas. A pre-Consultation meeting was held with City of Ottawa staff on August 23, 2019, to determine the project constraints and requirements. The following report details the stormwater management calculations used for water quantity and quality control in accordance with the City of Ottawa's requirements.

Reference Drawings: SD1 – Stormwater Management Sub-Drainage Areas, C02 – Servicing Tables, C03 - Site Servicing Plan (November 28, 2019), and C04 - Site Grading and Drainage, Erosion and Sediment Control Plan (November 28, 2019).

### 2 Objective

The objective of the stormwater management plan is to control post-development peak flows to predetermined levels, and detain onsite, stormwater up to and including the 100-year storm event without affecting adjacent lands. Stormwater quality control will be provided by the downstream pond 6, no onsite quality control is required.

## 3 Design Parameters

Stormwater management criteria for this site, in terms of quantity control, is based on the following approved reports:

Servicing and Stormwater Management Report – Blackstone Community Phase 4-8 by Stantec, April 11, 2019.

The maximum allowable release rate for this site shall not exceed the criteria set in the approved servicing report. Flows in excess of the allowable release rate up to and including the 100-year event will be detained onsite.

The Modified Rational Method (Q = 2.78CiA) was chosen to calculate the post-development release rates, and onsite storage requirements for this development. Detailed stormwater management calculations are included in **Appendix B**. All proposed storm sewers were assigned a Manning's coefficient of roughness of 0.013 corresponding to smooth wall pipes. In accordance with City of Ottawa Sewer Design Guidelines (Section 5.4.5.2.1), the coefficients used for calculating the post-development release rate were C = 0.25 for grassed areas and C = 0.90 for hard surfaced areas including rooftops. The rainfall intensities used in this analysis are based on the IDF curves and equations, as per City of Ottawa Sewer Design Guidelines (Section 5.4.2).

### 4 Water Quantity Controls

### 4.1 Pre-Development Conditions

The existing site is an undeveloped parcel with a generally flat site topography that is sloped toward the east side of the property with an approximate elevation difference of 0.5 meter over approximately a 220-meter length. The proposed day-care facility will be constructed at the same time as the main school building. Services have been installed along Rouncey Road, as part of the development of the Blackstone Community Phase 4-8.

### 4.2 Allowable Release Rate

The stormwater management design criteria for this site is based on the subdivision servicing report as noted above. According to the Stantec study, the school site has an allowable release rate up to the 5 year-event of 575.7 L/s. Refer to Appendix A – Background excerpts – Storm Water Management. Minimum storage requirement was set to 50 m3 / ha per the Subdivision study.

### 4.3 Post-Development Conditions

Proposed site grading and drainage of the overall site was designed such that stormwater runoff will be collected by the roof and a new storm water collection system which will be connected to the existing 1200mm diameter municipal storm sewer on Rouncey Road and discharged to Pond 6 of the subdivision.

The storm sewer system consists of manholes, catchbasins, storm sewers, perforated subdrain system in landscaped area of the sportsfield. Predominantly, drainage areas are piped to the storm system except for areas fronting onto Cope Drive, the bus layby and a portion of the paved basketball court area. This last area is uncontrolled to permit the unrestricted flow from the building storm sewer and prevent backup to the building.

No surface ponding will occur during the 5-year event. Flows exceeding the allowable release rate up to the 100-year event are to be detained on site.

The overall site development area is approximately **2.87 ha** and has a post-development average weighted runoff coefficient of **C=0.59**, and **C=0.64** for the maximum and 100-year events, respectively. Overall onsite storage requirement was calculated to be **228 m³** for the 100-year event, which exceeds the minimum storage requirement of 50m³/ha.

### 4.4 Onsite Stormwater Detention

Stormwater detention is proposed on the school roof, in the proposed parking area, in the paved school yard as well as along the sportsfield swale.

Flows from the roof area, parking area and grassed areas will be controlled individually in order to provide less restricted flow path. The maximum allowable ponding depth will be limited to 250mm for paved areas and 300mm in grassed areas during 100-year events. Flow control is proposed at manholes CBMH5 and CBMH7.

Flow at CBMH5 serving the parking and paved areas will be limited to 300 L/s. The maximum ponding elevation in the parking area is 100.85 which is below the building finished floor elevation of 101.25. The flows from the parking areas will be restricted at CBMH5 by installing an orifice plate with a discharge rate of **300.0** L/s at an estimated head of **2.21 m**. The orifice plate is shown on Drawing C006. The maximum head of water is dictated by the overland overflow elevation of 100.85m. The invert of the outlet pipe is 98.64.

The orifice plate was sized using the orifice equation:

```
Q = 0.61 \times A \times (2 \times g \times H)^{\Lambda0.5}, where Q is the discharge rate in m³/s Orifice coefficient = 0.61 G= gravitational constant = 9.81 \text{ m}^2/\text{s} H = head of water (m) above the centre of the orifice = (100.85 - 98.64) - (0.5 \times g) = (0.5 \times g)
```

The orifice of 314mm provides the required flow

```
Q= 0.61 x A x (2 \times g \times H)^{0.5} = 0.61 x (\pi \times (0.314/2)^2) x (2 \times 9.81 \times (2.21 - (0.5 \times 0.314))^{0.5} = 0.300 \text{ m}^3/\text{s}
```

Flow from the sportsfield and grassed areas will be restricted at CBMH7 by installing an orifice plate with a discharge rate corresponding to the 5-year event of **60 L/s by** at an estimated head of **1.93 m**. The orifice plate is shown on Drawing C006. The maximum head of water is dictated by the overland overflow elevation of 100.76m. The invert of the outlet pipe is 98.90.

The orifice plate was sized using the orifice equation:

```
Q = 0.61 \times A \times (2 \times g \times H)^{\Lambda0.5}, where Q is the discharge rate in m³/s Orifice coefficient = 0.61 G= gravitational constant = 9.81 \text{ m}^2/\text{s} H = head of water (m) above the centre of the orifice = (100.76 - 98.90) - (0.5 \times G) = 0.000
```

The orifice of 145mm provides the required flow

Q= 0.61 x A x 
$$(2 \times g \times H)^{0.5} = 0.61 \times (\pi \times (0.145/2)^2) \times (2 \times 9.81 \times (1.86 - (0.5 \times 0.145))^{0.5} = 0.060 \text{m}^3/\text{s}$$

Flow from the combined school and daycare roof will also be detained on the roof by installing parabolic weirs, (Watts Drainage Adjustable Flow Control for Roof Drains, or equivalent approved products), at the 29 proposed roof drains limiting the flow from the roof to **18.3 L/s**. The resulting required storage is **153 m³** for the 100-year event. Based on a maximum ponding depth of 150mm on the roof, the total available storage is approximately **194 m³**, which is sufficient to accommodate the 100-year event. Each flow control roof drain, complete with a single slot parabolic weir, will restrict flow at 5 GPM (0.32 L/s) per inch (25.4mm) of head to a maximum of 10 GPM (0.63 L/s). The restricted flow will outlet to the school's 300mm diameter storm sewer service at 4.0% slope downstream of any flow restriction.

The following table summarizes on-site requirements during the 100-year event:

Total	Controlled /	Run-off		Outlet	Total	Flow (L/s)	Required
Area	Uncontrolled	Coefficient		Location	Storage		Storage
					Provided		(cu.m.)
					(m <sup>3</sup> )		
		5 year	100 year			100-year eve	ent
2.39	Controlled	0.59	0.64		333	378.3	231
0.48		0.54	0.58	Cope Drive		138.6	
				& Rouncey			
				Road ROW			
2.87					333	516.9	231

Note that storage within the subdrain clearstone trench was not included within the available storage volume. There is sufficient available ponding to accommodate the 100-year event.

The maximum ponding limits are indicated on **SD1**. In the event of a rainfall exceeding the 100-year event, runoff will remain on-site until ponding reaches the overspill elevation. In such event, the school yard and sport field major overland flow route will overflow towards Rouncey Drive and the parking area will overflow to Continental Avenue. The grade elevation at the overflow point is **0.30 m** below the school's finished floor elevation.

### 4.5 Proposed Release Rates

The proposed release rate for this site during the 100-year event, including uncontrolled flows (138.6 L/s) and controlled flows (378.3 L/s) into the minor system of 516.9 L/s. Therefore, proposed release rates are within the allowable release rate for this site, determined to be 575.7 L/s in Section 4.2.

### 5 Erosion and Sediment Control

In accordance with City of Ottawa requirements, best management practices are to be implemented by the Contractor to provide protection of the area drainage system and the receiving water course, during construction activities. This includes limiting the amount of exposed soil, using filter bag inserts under the grates of catch basins and manholes, installing silt fences and other effective sediment traps, and installing and maintaining mud mats for outgoing construction traffic during construction activities.

### 6 Conclusion

The proposed site development includes a new school building with an attached daycare, a bus lay-by, asphalt parking, hard surface walkways and play areas, landscaped areas, a sports field and an area for portables. Roof drainage and surface runoff will be collected by a new storm sewer system which will be connected to the existing 1200mm diameter municipal storm sewer located on Rouncey Road. Post-development peak flows will be detained on the roof, in the parking area, in the school yard and in the bus lay-by in order to limit the post-development release rate to allowable levels. There is sufficient onsite storage to accommodate the 100-year event.

**Summary of report** 

ID	Description	Value/result
01	Allowable release rate	Q <sub>allowable</sub> = 575.7 L/s
02	Proposed release rate	Q <sub>100-yr post</sub> = 516.9 L/s
03	Post-development runoff coefficient	C <sub>5-yr post</sub> = 0.58, C <sub>100-yr post</sub> = 0.63
04	Post-development onsite storage requirement	228 m³
05	Proposed onsite storage	Parking lot and school yard: 138.6 m³, Roof: 153m³,
06	Discharge outlet location	1200mmφ storm sewer on Rouncey Road to Pond 6
07	Emergency runoff overflow locations	West side – curb cut in parking area towards Continental Avenue  East side – from school yard to layby towards Rouncey Road

### **END OF REPORT**

### Prepared by:



Barbra Kimmerle, P.Eng. Civil Engineer

## Appendix A - Background Excerpts Stormwater Management

Stantec	DATE: REVISIO DESIGN CHECKE	ED BY:	2019	4-8 9-04-15 4 DT SG	FILE NU		DESIG	I SEWE N SHEE If Ottawa	Т		DESIGN I = a / (t-a a = b = c =	1:2 yr	1:5 yr 998.071 6.053	1:10 yr 1174.18 6.014	1:100 yr	MANNIN MINIMUN	A COVER:	0.013 2.00	m min	BEDDING	CLASS =	В																	
LOCATION AREA ID NUMBER	FROM M.H.	ТО М.Н.	AREA (2-YEAR) (ha)	AREA (5-YEAR) (ha)	AREA (10-YEAR (ha)	AREA ) (100-YEAR	AREA R) (ROOF) (ha)	C (2-YEAR) (-)	C (5-YEAR) (-)	C (10-YEAR) (-)	C (100-YEAR (-)		ACCUM AxC (2YR) (ha)	AxC		AxC		A x C ) (100-YEAR (ha)			I <sub>2-YEAR</sub>	I <sub>S-YEAR</sub>	i <sub>10-YEAR</sub>	i <sub>100-YEAR</sub>	Q <sub>CONTROL</sub>	ACCUM.  Q <sub>CONTROL</sub> (L/s)	Q <sub>ACT</sub> (CIA/360) (L/s)		PIPE WIDTH R DIAMETE (mm)		PIPE SHAPE (-)	MATERIAL	CLASS		Q <sub>CAP</sub> (FULL) (L/s)	% FULL	VEL. (FULL) (m/s)		TIME OF FLOW (min)
Fernbank Crossing P1-4 LM97A, CM97A	301 M97	M97 M98	0.00 0.67	0.00 0.41	0.00	0.00 0.00	0.00 0.00	0.00 0.45	0.00 0.70	0.00 0.00	0.00	0.000 0.303	0.000 0.303	0.000 0.284				0.000		10.00 10.50 11.08				178.56 174.09				78.3 78.0	1950 2100	1950 2100	CIRCULAR CIRCULAR			0.30 0.20		79.35% 81.52%		2.60 2.24	0.50 0.58
F2006A	2048	2005A	0.00	0.00	0.00	1.35	0.00	0.00	0.00	0.00	0.40	0.000	0.000	0.000	0.000	0.000	0.000	0.541	0.541	10.00 10.15	76.81	104.19	122.14	178.56	0.0	0.0	268.3	12.9	525	525	CIRCULAR	CONCRETE	•	0.50	317.2	84.57%	1.42	1.42	0.15
C2009A, C2009B C2008A L2007A	2009 2008 2007	2008 2007 2006	0.00 0.00 0.56	0.34 0.21 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.39	0.70 0.70 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.219	0.000 0.000 0.219	0.238 0.150 0.000	0.388	0.000 0.000 0.000	0.000 0.000 0.000	0.000		10.00 11.54 12.95	71.37	104.19 96.72 90.84	122.14 113.34 106,42	165.63	0.0 0.0 0.0	0.0 0.0 0.0	68.8 104.3 138.8	64.5 64.5 49.4	450 600 750	450 600 750	CIRCULAR CIRCULAR CIRCULAR	CONCRETE	-	0.20 0.20 0.15	286.5	51.75% 36.41% 30.85%	0.98	0.70 0.76 0.73	1.54 1.41 1.13
C2006A, C2006B	2006	2005A 2005	0.00	0.55	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.000			0.774					14.07 15.71		86.67		148.27 139.05	0.0	0.0	225.2	70.3	900	900	CIRCULAR			0.10		37.71%		0.72	
C2049A	2005A 2049		0.00		0.00	0.00	0.00		0.70	0.00	0.00	0.000	0.000	1.989		0.000	0.000			16.15 10.00				178.56	0.0	0.0		13.0	825	825	CIRCULAR	CONCRETE		0.10		70.37%			0.16
C2005A C2004A, C2004B	2005	2004 2003	0.00	0,38 0,63	0.00	0.00	0.00	0.00	0.70 0.70	0.00	0.00	0.000	0.219 0.219	0.265 0.444	3.028 3.471	0.000	0.000	0.000	0.541 0.541	10.16 16.15 18.07	59,17 55,37	80.01 74.80	93.68 87.56	136.77 127.80	0.0	0.0	914.4 947.0	120.0 119.0	1200 1350	1200 1350	CIRCULAR			0.10 0.10	1286.2 1760.8	71.09% 53.78%		1.05 1.04	1.91 1.91
C2002A	2003 2002		0.00	0.00 0.30	0.00	0.00	0.00	0.00	0.00 0.70	0.00	0.00	0.000	0.219 0.219		3,471	0.000		0.000	0.541	19.98 20.89 21.81	52.07		82.27 79.98			0.0	889.9 904.8	55.9 56.7	1350 1350 1350	1350 1350 1350	CIRCULAR	CONCRETE			1760.8	50,54% 51.39%	1,19		0.92 0.92
L2047A L2046A L2044B, L2044C, L2044A	2047 2046 2045 2044	2046 2045 2044 2001	0.31 0.49 0.00 1.20	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.71 0.71 0.00 0.49	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.219 0.347 0.000 0.587	0.219 0.566 0.566 1.153	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000	14.07		104.19 94.86 90.58 86.70	111.15	162.42 155.02	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	46.7 110.1 105.2 205.1	73.3 44.9 44.6 44.5	450 525 525 675	450 525 525 675	CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CONCRETE	-	0.20 0.15 0.15 0.15	133.0 173.8 173.8 339.6	63.34% 60.52%	0.78 0.78	0.62 0.71 0.71 0.84	1.96 1.05 1.05 0.89
L2027B, L2027A L2026A L2017B, L2017A	2027 2026 2017	2026 2017 2015	0.79 0.34 0.50	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.59 0.70 0.70	0.00 0.00 0.00	0.00 0.00 0.00	0,00 0.00 0,00	0.465 0.236 0.350	0.465 0.701 1.051	0.000	0.000	0.000	0.000			14.95 10.00 12.53 14.63	68.30	104.19 92.50 84.77	108.38	158.34	0.0 0.0 0.0	0.0 0.0 0.0	99.2 132.9 182.9	128.6 105.3 89.5	450 525 600	450 525 600	CIRCULAR CIRCULAR CIRCULAR	CONCRETE	-	0.25 0.20			0.90		2.53 2.10
L2016A, L2016B		2015	0.18	0.00	0.00	0.00	0.00	0.50	0.00	0.00	0.00	0.088	0.088	0.000				0.000		<b>16.28</b> 10.00		104.19			0.0	0.0	18.9	38.3	300	300	CIRCULAR			0.65		24.33%		0.76	0.84
L2015A	2015 2014	2014 2013	0.27	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	0.192 0.000	1,332 1,332	0.000	0.000	0.000	0.000	0.000	0.000	10.84 16.28 17.55	58.91 56.34	79.65 76.13	93.26 89.12	136.15 130.08	0.0	0.0	217.9 208.4	91.3 15.8	675 675	675 675	CIRCULAR	CONCRETE		0.40	554.6 554.6	39,29% 37,57%	1.50 1.50	1.19 1.18	1.27 0.22
L2013A L2011A	2013 2012 2011	2012 2011 2010	0,51 0.00 0.55	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.70 0.00 0.70	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.357 0.000 0.382	1.689 1.689 2.071	0,000 0,000 0,000	0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	17.77 19.04 19.59 20.37	55.91 53.63 52.69	75.55 72.43 71.15	88.44 84.77 83.27	129.08 123.70 121.51	0.0 0.0 0.0	0.0 0.0 0.0	262.3 251.6	95.7 41.4 42.1	675 675	675 675 825	CIRCULAR CIRCULAR CIRCULAR	CONCRETE CONCRETE CONCRETE	-	0.40 0.40 0.15	554.6	47.30% 45.37% 52.26%	1.50	1.25	1.27 0.55 0.77
L2042A L2041A	2042 2041	2041 2040	0.32 0.20	0.00	0.00	0.00	0.00	0.74 0.74	0.00	0.00	0.00	0.235 0.150	0.235 0.386	0.000	0.000	0.000	0.000		0.000	10.00 11.16		104.19 98.43			0.0	0.0	50.2 77.8	70.8 83.1	300 375	300 375	CIRCULAR CIRCULAR	PVC PVC	-	0.65 0.65	77.5 132.9	64.78% 58.51%			1.16 1.23
L2043B, L2043A	2043	2040	0.34	0.00	0.00	0.00	0.00	0.74	0.00	0.00	0.00	0.251	0.251	0.000	0.000	0.000	0,000	0.000	0.000	12.39 10.00 11.72	76.81	104.19	122.14	178.56	0.0	0.0	53.6	107.2	300	300	CIRCULAR	PVC		0.65	77.5	69.08%	1.10	1.04	1.72
L2040A	2040	2036	0.08	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.00	0.045	0.682	0.000	0.000	0,000	0.000	0.000	0,000	12,39 13,46	68.70	93,06	109,03	159,30	0.0	0.0	130.2	48,3	525	525	CIRCULAR	CONCRETE	AUI-NE	0,15	173,8	74.90%	0,78	0.75	1.07
L2039B, L2039A	2039	2036	0.49	0.00	0.00	0.00	0.00	0.74	0.00	0.00	0.00	0.366	0.366	0.000	0.000	0.000	0.000	0.000	0.000	10.00 <b>11.84</b>	76,81	104.19	122.14	178.56	0.0	0.0	78.0				CIRCULAR		-			74.80%			
L2038A L2037A	2038 2037	2037 2036	0.35 0.36	0.00	0.00	0.00	0.00	0.74 0.74	0.00	0.00		0.257 0.265						0.000						178.56 168.98		0.0	54.9 105.6	69,9 83.4		300 450	CIRCULAR	PVC PVC				70.85% 41.01%			
L2036A		2032		0.00											MURRINA					14.39								44.0				CONCRETE	• 10						
L2033A, L2033B		2034 2033 2032		0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.74	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.539	0.000 0.000 0.539	0.000 0.000 0.000			0.000	0.000 0.000 0.000	0.000	10.00	76.81		122.14	178.56 178.56 178.56		0.0 0.0 0.0	0.0 0.0 115.0		300 300 450	300 300 450	CIRCULAR CIRCULAR CIRCULAR	PVC PVC CONCRETE		0.40 0.40 0.25	60.8	0.00% 0.00% 77.30%	0.86		0.00
L2032A, L2032B	2032	2029	0.73	0.00	0.00	0.00	0.00	0.51	0.00	0.00	0.00	0.371	2.518	0.000	0.000	0,000	0.000	0.000	0.000		63,24	85,57	100.22	146,37	0,0	0,0	442,3	82.0	900	900	CIRCULAR	CONCRETE		0,15	731.4	60,47%	1.11	1.01	1.35
L2031A, L2031B L2030B, L2030A		2030 2029		0.00	0.00		0.00	0.70 0.61	0.00 0.00	0.00														178.56 162.20		0.0		82.0 102.4		450 825		CONCRETE				48.10% 21.06%			
L2029A, L2029B	2029	2028	1.30	0.00	0.00	0.00	0.00	0.52	0.00	0.00	0.00	0.674	3,705	0.000	0.000	0.000	0.000	0.000	0.000	15.74 17.74	60.06	81.23	95.11	138.87	0.0	0.0	618.2	111.7	1200	1200	CIRCULAR	CONCRETE		0.10	1286.2	48.07%	1.10	0.93	1.99
L2021A, L2021B	2021 2020	2020 2019	0.81 0.00	0.00	0.00	0.00	0.00	0.52 0.00	0.00	0.00	0.00							0.000						178.56 177.32		0.0	89.3 88.7		375 450	375 450	CIRCULAR	PVC CONCRETE				67.23% 66.71%			
L2026B L2024A, L2024B L2023A	2026 2025 2024 2023 2022	2025 2024 2023 2022 2019	0.00 0.57 0.35	0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00	0.70 0.00 0.70 0.70 0.00	0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00	0.000 0.400 0.245		0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000 0.000 0.000	0.000 0.000 0.000		0.000 0.000 0.000	11.69 11.91 13.88	70.87 70.17 64.55	95.08 87.37	112.54 111.41 102.34	164.46 162.80	0.0 0.0 0.0 0.0 0.0	0,0 0,0 0,0 0,0 0,0	23.5 21.7 99.4 135.5 126.7	9.6 89.0	675	300 300 600 675 675	CIRCULAR	PVC PVC CONCRETE CONCRETE CONCRETE		0.15	68.0 286.5 339.6	34.62% 31.95% 34.71% 39.88% 37.30%	0.97 0.98 0.92	0.73	0.22 1.97 1.73
L2019A, L2019B		2028	0.64	0.00	0.00	0.00	0.00	0.56	0.00	0.00	0.00							0.000		<b>15.90</b> 15.90				138.08			254.6		825	825		CONCRETE				53.76%			
L2018A, L2018B	2028 2018		0.00 0.71	0.00	0.00	0.00	0.00	0.00 0.49	0.00	0.00 0.00						0.000	0.000		0.000	17.92				129.24 128.43			814.7 863.2	11.0 71.4		1200 1350		CONCRETE				63.34% 49.02%			
L2010A, L2010B	2010	2001	0.48	0,00	0.00	0.00	0.00	0.49	0.00	0.00	0,00	0.234	7.890	0.000	0.000	0.000	0.000	0.000	0.000	19.10	51.45	69.45	81.27	118,58	0.0	0.0	1127.6	88.2	1350	1350	CIRCULAR	CONCRETE	<u>-</u>	0.10	1760.8	64.04%	1.19	1.10	1.34

<b>Stantec</b>	DATE: REVISION: DESIGNED BY:	ne Phases 4-8 2019-04-15 4 DT	FILE NUMB	DESI (City	RM SEWE GN SHEE of Ottawa	T	DESIG I = a / ( a = b =	<u> </u>	1:5 yr 998.071 1	As per City of 1:10 yr   1:10 174.184   1735 6.014   6.0	00 yr	ING'S n=	0.013		BEDDING C	LASS =	В														
LOCATION AREA IU NUMBER	CHECKED BY:  FROM TO M.H. M.H.	SG  AREA ARE (2-YEAR) (5-YEAR) (ha) (ha)		AREA ARE 00-YEAR) (ROC (ha) (ha		C (5-YEAR) (10 (-)	C = C D-YEAR) (100-YEA (-) (-)		ACCUM		GE AREA JUM. A X C (5YR) (10-YEA	ACCUM.	10 r . A x C . R) (100-YEAR) Ax (ha)	ACCUM.	T of C (min) 21.71	l <sub>2-YEAR</sub> (mm/h)	I <sub>S-YEAR</sub> (mm/h)			Q <sub>cc</sub>	CUM. U <sub>act</sub> datrol (CIA/360) L/s) (L/s)		PIPE WIDTH OR DIAMETE (mm)		PIPE SHAPE (-)		PIPE SELECT CLASS (-)	SLOPE	(FULL)	6 FULL VE (FU (-) (m)	LL) (ACT) FLOW
C2001A C2000A	2001 2000 2000 M98 M98 M99		5 0.00	0.00 0.0 0.00 0.0 0.00 0.0	0.00	0.71	0.00 0.00 0.00 0.00 0.00 0.00	0.000	9.262				0.000	0.541	21.81 22.53 23.56 23.56				111,16	0.0	0.0 2172.9 0.0 2200.2 51.8 8679.2	79.7	1800 1800 2400	1800 1800 2400	CIRCULAR CIRCULAR	CONCRETE CONCRETE	•	0,10 :	3792,1 51	7.30% 1.4 8,02% 1.4 3.07% 2.0	4 1.29 1.03
L1054A C1052A, C1052B L1053B, L1053A	1054 1052 1052 1050 1053 1050	0.00 0.8	7 0,00	0.00 0.0 0.00 0.0	0.00	0.70	0.00 0,00 0.00 0,00 0.00 0,00	0.000	0.631	0.000 0.0 0.609 0.6 0.000 0.0	0.000	0.000	0.000	0.000	12.28	75,79	104.19 102.80 104.19	120,50	176,15	0.0	0.0 134.7 0.0 306.8 0.0 181.8	111.4	2400 525 750 600	525 750 600	CIRCULAR CIRCULAR	CONCRETE CONCRETE	-	0.15	449.8 68	7.13% 0.9 8.20% 0.9 3.26% 0.8	9 0,92 2.01
L1051B, C1051A	1051 1050 1050 1049			0,00 0,00			0.00 0.00			0.127 0.1 0,000 0.7			0.000	0,000	10.84		104.19	122.14 109.58			).0 85.8 ),0 535,1	58.5 70.0	375 900 900	375 900 900	CIRCULAR	PVC CONCRETE	-			4.60% 1.2 3.16% 1.1	6 1.16 0.84 1 1.07 1.09
C1009A C1008A L1007A	1009 1008 1008 1007 1007 1006	0.00 0.64	0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.00	0.72	0.00 0.00 0.00 0.00 0.00 0.00	0.000	0.000	0.361 0.3 0.462 0.8 0.000 0.8	23 0.000	0.000	0,000	0.000 0.000 0.000	10.00 10.62	74.49	101.01	122.14 118.39 112.14	173.05	0.0 0	0,0 104,5 0,0 231,0 0,0 249,9	53.6 110.1 43.1	375 525 600	375 525 600	CIRCULAR CIRCULAR CIRCULAR	PVC CONCRETE CONCRETE		0.80	401.3 57	3,38% 1.5 7,56% 1.8 5,94% 1.3	
L1015A L1020C, L1020B L1022A	1015 1020 1020 1014 1022 1014	0.26 0.00	0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.57	0.00	0.00 0.00 0.00 0.00 0.00 0.00		0.214	0.00 000.0		0,000		0.000	11.27 12.99	72.24	104,19 97,91 104.19	114.74		0.0 0.0	0.0 13.9 0.0 42.8 0.0 468.8	52.8 97.2	300 375 675	300 375 675	CIRCULAR CIRCULAR	PVC PVC CONCRETE		0,65	132,9 32	7,91% 1.1 2,24% 1.2 5,60% 1.6	6 0,94 1,71
L1012A F1021A	1014 1013 1013 1012 1012 1011 1021 1011	0.00 0.00	0.00	0,00 0,00 0,00 0,00 0,00 0,00 0,92 0,00	0,00 0.57	0,00	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.40	0,000 0,087	2.411 2.498	0.00 000.0	00 0.000 00 0.000	0.000	0.000 0.000	0.000 0.000	13.65 13.75 14.63	65.15 64.88		103.31 1 102.87	150.90 150.26	0.0 0 0.0 0	.0 448.4 .0 436.3 .0 450.2	69.3 8.9 75.8	675 750 825	675 750 825 525	CIRCULAR CIRCULAR CIRCULAR	CONCRETE CONCRETE CONCRETE		0.40 0.40	707.0 63 734.5 59 947.1 47 224.3 81	).40% 1.6	1 1.45 0.10 2 1.44 0.88
L1011B, L1011A  L1020A L1019B, L1019C, L1019A	1011 1010 1020 1019 1019 1018	0.27 0.00	0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.70	0.00	00.00 00.00 00.00 00.00 00.00 00.00	0,200 0,189 0,532	0.189	0.000 0.00 0.000 0.00 0.000 0.00	00 0,000	0,000	0.000	0.370 0.000	15.18 10.00	76,81	104.19	122.14 1	78,56	0.0 0	.0 618,5 .0 40,4 .0 148,1	47.2 44.2 108.0	900 300 525	900 300 525	CIRCULAR CIRCULAR CIRCULAR	CONCRETE  PVC CONCRETE		0,65		0.79% 1.5 9.13% 1.1 9.21% 1.3	0 0,95 0.78
L1017A L1016A L1010A, L1010B	1018 1017 1017 1016 1016 1010 1010 1006	0.35 0.00	0.00 0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.70 0.70	0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.245 0.315	0.966 1.281	0.000 0.00 0.000 0.00 0.000 0.00	0,000 0,000 0,000 0,000	0.000 0.000 0.000	0.000	0.000 0.000 0.000 0.000	12.35 12.48 13.94 14.63	68.81 68.42 64.40	93.21	109.21 1 108.58 1 102.09 1	59.57  58.64  49.11	0.0 0 0.0 0 0.0 0	.0 137,8 .0 183,6 .0 229,1	8.8 104.3 39.7	525 600 675	525 600 675	CIRCULAR CIRCULAR CIRCULAR	CONCRETE CONCRETE CONCRETE	÷ ;	0.45 0.45 0.20	301.0 45 429.7 42 392.2 58	.79% 1.3	5 1.12 0.13 7 1.20 1.45 6 0.95 0.70
C1006A	1006 1005 1005 1004 1004 1003 1003 1002	0.00 0,00	00,0	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	0.00 0.00	0.00 ( 0.00 (	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	0,000 0,000	4.473 ( 4.473 (	0.000 1.09	94 0.000 94 0.000	0.000	0.000	0.370 0.370 0.370 0.370	17.01 17.67	57.39	77.58 75,81	90.82 1	32.57 29,54	0.0 0. 0.0 0. 0.0 0. 0.0 0.	.0 1085.1 .0 1060,6	42.5	1500 1650	1350 1500 1650 1650	CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CONCRETE CONCRETE CONCRETE CONCRETE		0.10 2 0.10 3	2332.0 46	.27% 1.3	8 1.07 0.66 6 1.05 0.68
C1023A	1023 1002 1002 1001 1001 1000	0.00 0.00	0.00	0.00 0.00	0,00	0.00 (	0.00 0.00	0.000	4.473	0.000 6,50	0.000	0,000	0.000	0,370 0,3 <b>70</b>	<b>10.23</b> 19.03	53.64	72.44	84.79 1	23.73	0.0 0.	0 2102.5	94.7		1650	CIRCULAR CIRCULAR CIRCULAR	CONCRETE	-	0.10 3	1006.9 69	.92% 1.3	5 1.55 0.23 3 1.29 1.22 5 1.28 0.08
L1000A	1000 P3 P3 P2 P2 P1	0.00 0.00	0.00 0.00	0.00 0.00 0.00 0.00 0.00 0.00	0.00	0,00 0 0,00 0	0.00 0.00 0.00 0.00 0.00 0.00	0,000 0,000	11.179 ( 11.179 (	0,000 7,24 0.000 7,24	0,000 0,000	0.000		0,370 0,370 2	20.85 21.01 21.09	50.70 50.46	68.44 68.11	80.09 1 79.69 1	16,83 16.26	0.0 0. 0.0 0.	0 3119.1 0 3071.0 0 3056.0	43.9 14.0 6.5	1800 1800	1800	CIRCULAR CIRCULAR CIRCULAR	CONCRETE CONCRETE CONCRETE	• • •	0.10 3 0.10 3	792.1 80 792.1 80	.25% 1.4 .98% 1.4 .59% 1.4	4 1.43 0,16
															21.13							2.7	2100						19		

Fernbank Elementary School, 480 Cope Drive
Stormwater Management Report

Appendix B - Stormwater Management Calculations

### Fernbank Elementary School

## Jp2g Consultants Inc. ENGINEERS · PLANNERS · PROJECT MANAGERS

#### Storm Sewer Pipe Design Without ICD Flow Control

1 acre = 0.4047 hectares

<u>Definitions</u> Manning's Coefficient =

0.013

Rational Method

Notes

Manning's Coefficient = 0.01 Return Frequency (yrs) = 5 Q = 2.78 CIA (I/s), where C= Runoff Coefficient Used City of Ottawa IDF Curve
 Min. velocity = 0.8 m/sec

i = Rainfall Intensity (mm/hr)

3) Max. velocity = 6.0 m/sec

A = Areas in Hectares (ha)

) III/Sec

hide

Designed BK
Checked SM
Dwg. Reference C1
Jp2g project No 19-5070A

					-	LOW									SEWER	חות				/
			C=	C=	Individual	Cum.	tc	i <sub>5-year</sub>	i <sub>100 years</sub>	Flow <sub>5-year</sub>	Flow 100 years	Dia.	actual	Slope	Length	Capacity	Velocity	Sect.Time	Tot. Time	Utilization
From	То		0.90	0.25	2.78CA	2.78CA	(min.)	(mm/hr)	(mm/hr)	(I/s)	(I/s)	(mm)	dia	(%)	(m)	(full) (l/s)	(full) (m/s)	(minutes)	(minutes)	(%)
CB-1	CBMH1	A1	0.094		0.24	0.24	10.0	104.2	178.6	24.5	42.0	250	250	0.65	30.6	47.9	1.0	0.5	10.5	51.1
CBMH1 (	CBMH2	A3	0.103		0.26	0.49	10.5	101.5	173.9	50.0	85.7	375	381	0.40	25.1	115.7	1.0	0.4	10.9	43.3
CB-2	CBMH2	A2	0.096		0.24	0.24	10.0	104.2	178.6	25.0	42.9	250	250	0.65	30.6	47.9	1.0	0.5	10.5	52.2
CBMH2	CBMH3	A4	0.104		0.26	0.99	10.9	99.5	170.4	98.8	169.3	375	381	0.50	32.4	129.3	1.1	0.5	11.4	76.4
CBMH3 (	CBMH4	A5	0.114	0.050	0.32	1.31	11.4	97.2	166.5	127.7	218.7	450	457	0.40	40.0	187.9	1.1	0.6	12.0	68.0
	CBMH5	A7	0.151	0.051	0.41	1.73	12.0	94.7	162.1	163.4	279.8	600	610	0.35	39.5	379.6	1.3	0.5	12.5	43.1
CBMH5 **	CBMH6	A8	0.160	0.083	0.46	2.18	12.5	92.5	158.4	202.1	346.0	600	610	0.35	35.7	379.6	1.3	0.5	13.0	53.2
Roof *	CBMH8	R1	0.389	0.000	0.97	0.97	10.0	104.2	178.6	101.4	173.7	300	300	4.00	20.8	193.4	2.7	0.1	10.1	52
	СВМН9	A9	0.097		0.24	1.22	12.5	92.5	158.4	112.5	192.6	450	457	0.35	17.1	175.7	1.1	0.3	12.8	64.0
TCB1	RYCB2	A12		0.501	0.35	0.35	15.0	83.6	142.9	29.1	49.8	250	250	0.50	116.5	42.0	0.9	2.3	17.3	69.2
RYCB2	TCB12	A13	0.024	0.124	0.15	0.49	17.3	76.9	131.4	38.0	65.0	300	300	0.50	20.0	68.4	1.0	0.3	17.6	55.6
TCB12	TCB16	A14	0.024	0.138	0.16	0.65	17.6	76.0	129.8	49.4	84.4	300	300	0.50	25.0	68.4	1.0	0.4	18.0	72.3
TCB16	TCB13	A15	0.024	0.128	0.15	0.80	18.0	74.9	127.9	59.8	102.2	300	300	0.50	25.0	68.4	1.0	0.4	18.5	87.5
CBMH7***	CBMH6	A16		0.081	0.06	0.86	18.5	73.8	126.1	63.1	107.9	375	375	0.30	35.0	96.0	0.9	0.7	19.1	65.8
CBMH6 (	CBMH10	A17		0.064	0.04	3.08	13.0	90.7	155.2	279.8	478.8	600	610	0.50	32.4	453.7	1.5	0.4	13.3	61.7
-															-					
CBMH9 (	CBMH10	A10	0.032	0.011	0.09	1.30	12.8	91.5	156.5	119.1	203.9	450	457	0.35	28.2	175.7	1.1	0.4	13.2	67.8
0014140	OTMU	***	0.000	0.007	0.40	4.40	40.0	00.7	455.0	407.0	200.0	205	005	0.05	00.0	040.0	4.0	0.0	40.0	47.0
CBMH10 S	STMH1	A11	0.039	0.007	0.10	4.49	13.0	90.7	155.2	407.2	696.8	825	825	0.35	20.3	849.2	1.6	0.2	13.2	47.9

Flow control to be installed at outlet

Notes:

Flow from controlled roof drains is limited to 18.3 L/s

\*\* Flow restricted to 300 L/s

\*\*\* Flow from infiltration trench - restricted to 60L/s

### Fernbank Elementary School - Cope Drive, Ottawa



### **B.1.1 - Allowable release rate**

Using the criteria for the site from the subdivision Final Serviceability Report Development by Stantec (April 2019), the maximum allowable release rate is up to the 5 year discharge rate. Based on the weighted average the allowable release rate is:

 $Q_{allowable} = Q = 2.78 C I A$   $Q_{allowable} = 575.7$  |/s

### **B.1.2 - Post-development release rate**

			Are	eas (m²)			
ID	Description	Type	C <sub>0.90</sub>	C <sub>0.25</sub>	Total (m <sup>2</sup> )	C <sub>post-5-year</sub>	C <sub>post-100-yr</sub> *
B1	Front of building	uncontrolled	447	1854	2301	0.38	0.40
A1	Western parking area 1	controlled	945		945	0.90	1.00
A2	Western parking area 2	controlled	960		960	0.90	1.00
A3	Western parking area 3	controlled	1033		1033	0.90	1.00
A4	Western parking area 4	controlled	1041		1041	0.90	1.00
A5	Playground at Daycare area 1	controlled	1140	499	1639	0.70	0.77
A6	Future portables area 1	controlled	43	130	173	0.41	0.44
Α7	Playground at Daycare area 2	controlled	1591	517	2108	0.74	0.82
A8	Courts area 1	controlled	1604	829	2433	0.68	0.74
A9	Courts area 2	uncontrolled	970		970	0.90	1.00
A10	Bus loop area 1	uncontrolled	317	106	423	0.74	0.81
A11	Bus loop area 2	uncontrolled	391	65	456	0.81	0.89
A12	Sports Field	controlled	63	4947	5010	0.26	0.26
A13	Future portables area 2	controlled	24	1216	1240	0.26	0.26
A14	Future portables area 3	controlled	24	1356	1380	0.26	0.26
A15	Future portables area 4	controlled	24	1256	1280	0.26	0.26
A16	Back lot area 1	controlled	0	810	810	0.25	0.25
A17	Back lot area 2	uncontrolled	0	645	645	0.25	0.25
R1	Roof	controlled	3890		3890	0.90	1.00
			14507	14230	28737	0.58	0.63

\*including 25% increase as per City of Ottawa Sewer Design Guidelines

Total uncontrolled area =

5-year Runoff coefficient, C =

Calculations for post-development runoff coefficient	C <sub>post-5-year (col. D)</sub>	=(column A * 0.9 + column B * 0.2) / column C
	C <sub>post-100-yr (col. E)</sub>	=(column A * 1.0 + column B * 0.2*1.25) / column C
		note: 0.90 x 1.25 = 1.125, use max. 1.0

Calculations for average weighted runoff coefficient	C <sub>post-5-year</sub>	=((15170*0.9)+(13211*0.25))/28381	0.58
	C <sub>post-100-yr</sub>	=((15170*1.0)+(13211*0.25*1.25))/28381	0.63

ha

**⑤** = ①-④

Estimated time of concentration, t <sub>c</sub> =	10.0	minutes
Based on Ottawa IDF curve, i <sub>5-year</sub> =	998.071/ (t	c+6.053) <sup>0.814</sup>
	104.2	mm/hr
Based on Ottawa IDF curve, i <sub>100-years</sub> =	1735.688/	(t <sub>c</sub> +6.014) <sup>0.820</sup>
	178.6	mm/hr
A.1.2.1 - uncontrolled flow		

\*\*\*As per City of Ottawa Sewer Design Guidelines (Section 5.4.5.2)

100-year Runoff coefficient, C =	0.58			
Estimated time of concentration, $t_c$ =	10.0	minutes		
$\mathbf{Q}_{uncontrol}$	led 5-year = 74.7	I/s	2	
	<sub>ble 5-year</sub> = 501.0	l/s	3	= ①-(
Quncontroller	1 100-year = 138.6	l/s	4	

Q<sub>net-allowable 100-year</sub> = 437.1

0.480

0.54

### **B.1.3 - Post-development onsite storage**

### A.1.3.1 - Overall onsite storage requirements

Total controlled area, A1 to A8 & A12 to A16 & R1 2.394 ha

5-year Runoff coefficient, C **0.59** 100-year Runoff coefficient, C **0.64** 

100 yr net-allowable release rate 378.3 l/s ⑤

Table 1.3.1a - 5-year onsite storage requirements

		- ,				
	Time (minutes)	i <sub>5-year</sub> (mm/hr)	Q <sub>actual</sub> (I/s)	Q <sub>allowable</sub> (I/s)	Q <sub>stored</sub> (I/s)	V <sub>stored</sub> (m <sup>3</sup> )
peak Vstored>	10	104.2	406.5	378.3	28.2	16.9
	15	83.6	326.0	378.3	-52.3	-47.1
	20	70.3	274.1	378.3	-104.2	-125.1
	25	60.9	237.6	378.3	-140.7	-211.1
	30	53.9	210.4	378.3	-167.9	-302.2
	35	48.5	189.3	378.3	-189.0	-396.9
	40	44.2	172.4	378.3	-205.9	-494.2
	45	40.6	158.5	378.3	-219.8	-593.4
	50	37.7	146.9	378.3	-231.4	-694.2
	55	35.1	137.0	378.3	-241.3	-796.2
	60	32.9	128.5	378.3	-249.8	-899.2

Therefore 17 m³ of onsite storage required during 5-year even

Table 1.3.1b - 100-year onsite storage requirements

	Time	i <sub>100-year</sub>	$Q_{actual}$	Q <sub>allowable</sub>	$Q_{\text{stored}}$	$V_{\text{stored}}$
	(min)	(mm/hr)	(l/s)	(l/s)	(l/s)	(m³)
	10	178.6	758.1	378.3	379.8	227.9
	15	142.9	606.7	378.3	228.4	205.5
	20	120.0	509.3	378.3	131.0	157.2
peak V stored>	25	103.8	440.9	378.3	62.6	93.9
	30	91.9	390.0	378.3	11.7	21.1
	35	82.6	350.6	378.3	-27.7	-58.2
	40	75.1	319.0	378.3	-59.3	-142.2
	45	69.1	293.2	378.3	-85.1	-229.9
	50	64.0	271.5	378.3	-106.8	-320.3
	55	59.6	253.1	378.3	-125.2	-413.0
	60	55.9	237.3	378.3	-141.0	-507.6
	Therefore	228	m <sup>3</sup> of onsite sto	rage required du	ring 100-year eve	en'

### B.1.3.2 - Estimated detention created by installing roof weirs

Total roof area, A3 0.389

5-year Runoff coefficient, C **0.90** 

100-year Runoff coefficient, C 1.00

Install 0.6309 l/s weirs at each of the 29 roof drains 18.3 l/s

Watts Drainage Adjustable Flow Control for Roof Drains, or approved equivalent

Table 1.3.2a - 5-year estimated detention on new roof

_	Time (minutes)	i <sub>5-year</sub> (mm/hr)	Q <sub>actual</sub> (I/s)	Q <sub>allowable</sub> (I/s)	Q <sub>stored</sub> (I/s)	V <sub>stored</sub> (m³)
_	10	104.2	101.4	18.3	83.1	49.9
	15	83.6	81.3	18.3	63.0	56.7
peak V stored>	20	70.3	68.4	18.3	50.1	60.1
	25	60.9	59.3	18.3	41.0	61.5
	30	53.9	52.5	18.3	34.2	61.5
	35	48.5	47.2	18.3	28.9	60.7
	40	44.2	43.0	18.3	24.7	59.3
	45	40.6	39.5	18.3	21.2	57.4
	50	37.7	36.6	18.3	18.4	55.1
	55	35.1	34.2	18.3	15.9	52.4
	60	32.9	32.1	18.3	13.8	49.6

Therefore 62 m<sup>3</sup> estimated roof detention

Table 1.3.2b - 100-year estimated detention on new roof

_	Time (min)	i <sub>100-year</sub> (mm/hr)	Q <sub>actual</sub> (I/s)	Q <sub>allowable</sub> (I/s)	Q <sub>stored</sub> (I/s)	V <sub>stored</sub> (m³)
_	10	178.6	193.1	18.3	174.8	104.9
	15	142.9	154.5	18.3	136.2	122.6
	20	120.0	129.7	18.3	111.4	133.7
	25	103.8	112.3	18.3	94.0	141.0
	30	91.9	99.3	18.3	81.1	145.9
peak V stored>	35	82.6	89.3	18.3	71.0	149.1
	40	75.1	81.3	18.3	63.0	151.1
	45	69.1	74.7	18.3	56.4	152.2
	50	64.0	69.2	18.3	50.9	152.6
	55	59.6	64.5	18.3	46.2	152.4
	60	55.9	60.4	18.3	42.1	151.7
<u></u>	herefore	153	m <sup>3</sup> estimated ro	oof detention		

### B.1.4 - Site storage

D.1.4 -	Site Storage		100-year event		
	overall site storage requirements		228	$m^3$	Table B.1.3.1
	Roof storage requirements		153	m <sup>3</sup>	
	estimated roof ponding volume		153	$m^3$	Table B.1.3.2
	roof ponding depth		0.118	m	maximum allowable: 0.15m
	estimated parking area and school yard volume		180	$m^3$	
	maximum parking area and school yard ponding depth		0.25	m	maximum allowable: 0.25m
	Total available roof storage		153	$m^3$	at maximum ponding depth of 0.15m
	Total available parking area and school yard storage		180	m <sup>3</sup>	
	Total available onsite storage > overall storage requirements		<u>OK</u>		
,	Total available onsite storage > estimated detention		<u>OK</u>		
B.1.5 -	Release rate for site				
	Release rate				
	Allowable release rate (5-year)	575.7			Section B.1.1
	Uncontrolled release rate for (100-yr)	138.6			Section B.1.2.1
	Controlled release rate at roof drain (100-yr)	18.3			Section B.1.3.2
	Controlled release rate at CBMH-5 (100-yr)	300.0			Section B.1.3.3
	Controlled release rate at CBMH-7 (100-yr)	60.0			
	Total release rate (100-yr)	516.9			
,	Total release rate (100-yr) < Allowable release rate (5-year)	<u>OK</u>			

Fernbank Elementary School, 480 Cope Drive
Stormwater Management Report

Appendix C - Stormwater Management Calculations
Watts Drainage Adjustable Flow Control for Roof Drains - Data Sheet



## **RD-100**

Tag:

## **Large Capacity Roof Drain**

### Components:



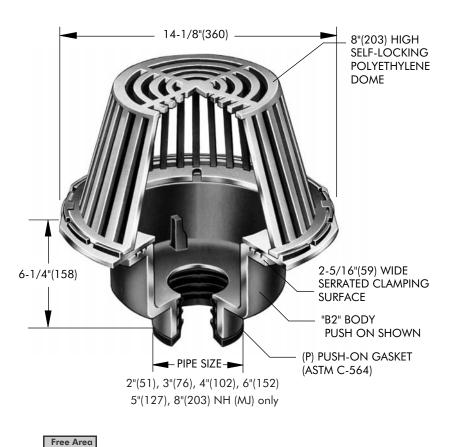






Order Code: RD-10

SPECIFICATION: Watts Drainage Products RD-100 epoxy coated cast iron roof drain with deep sump, wide serrated flashing flange, flashing clamp device with integral gravel stop and self-locking polyethylene (standard) dome strainer.



Sq. In. 137

**Deck opening 10" (254)** with sump receiver 13-1/4" (337)

\*\* Side Outlet (-SO) option only available in 2"(51), 3"(76), 4"(102) pipe sizes. Underdeck Clamp (-BED and -D options) are not available when -SO is selected.

Ex. RD-102P-K				
Pipe Sizing (Select One)				
Suffix	Description			
2	2"(51) Pipe Size			
3	3"(76) Pipe Size			
4	4"(102) Pipe Size			
5	5"(127) Pipe Size			
6	6"(152) Pipe Size			
8	8"(203) Pipe Size			
Outlet Type (Select One)				

Outlet Type (Select One)				
Suffix	Description			
NH	No Hub (MJ)			
Р	Push On			
T	Threaded Outlet			
X	Inside Caulk			

**Options (Select One or More)** 

Suffix	Description	
-A	Accutrol weir (specify # 1-6 slots	
-B	Sump Receiver Flange	
-BED	Sump Receiver, Adj Ext., Deck Clamp	
-C	Secondary Membrane Clamp	
-D	Underdeck Clamp	
-E	Adjustable Extension	
-GSS	Stainless Steel Ballast Guard	
-H	Adj. to 6" IRMA Ballast Guard	
-K	Ductile Iron Dome	
-K80	Aluminum Dome	
-L	Vandal Proof Dome	
-R	2" High External Water Dam	
-SO	Side Outlet**	
-V	Fixed Extension (1-1/2",2",3",4")	
-W	Adj. Water Level Regulator	
-W-1	Waterproofing Flange	
-Z	Extended Integral Wide Flange	$\bar{\Box}$

Optio	nai boay material (INFL)	niy
Suffix	Description	
-60	PVC Body w/Socket Outlet	
-61	ABS Body w/Socket Outlet	

Sediment Bucket

Galvanized Dome

Mesh Covered Dome -113M Special Epoxy from 3M Range

All Galvanized

-12

-13

-83

Job Name	Contractor
Job Location	Contractor's P.O. No.
Engineer	Representative

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without incurring any obligation to make similar changes and modifications to products previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances. CANADA



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