BRIGIL

Petrie Landing II - 8466 Jeanne D'Arc Boulevard

Transportation Impact Assessment Report







Petrie's Landing II 8466 Jeanne D'Arc Blvd

Transportation Impact Assessment Report

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February 23, 2021

476705 - 02000



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$ is either transportation engineering $\sqrt{}$ or transportation planning \square .

License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

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TIA Report

Parsons has been retained by Brigil to update a TIA in support of a Site Plan Application for a residential development located at 8466 Jeanne D'Arc Boulevard. The previous submission was written prior to the new TIA Guidelines. This document follows the new TIA process, as outlined in the City Transportation Impact Assessment (TIA) Guidelines (2017). The following report represents Step 5 – Transportation Impact Assessment Report.

1. SCREENING FORM

The screening form confirmed the need for a TIA based on the Trip Generation, given that the proposed development consists of the addition of a 4-stofrey residential block with a total of 113 apartment units. The Location and Safety modules were not triggered. The screening form has been provided in Appendix A along with responses to City Comments.

2. SCOPING REPORT

2.1. EXISTING AND PLANNED CONDITIONS

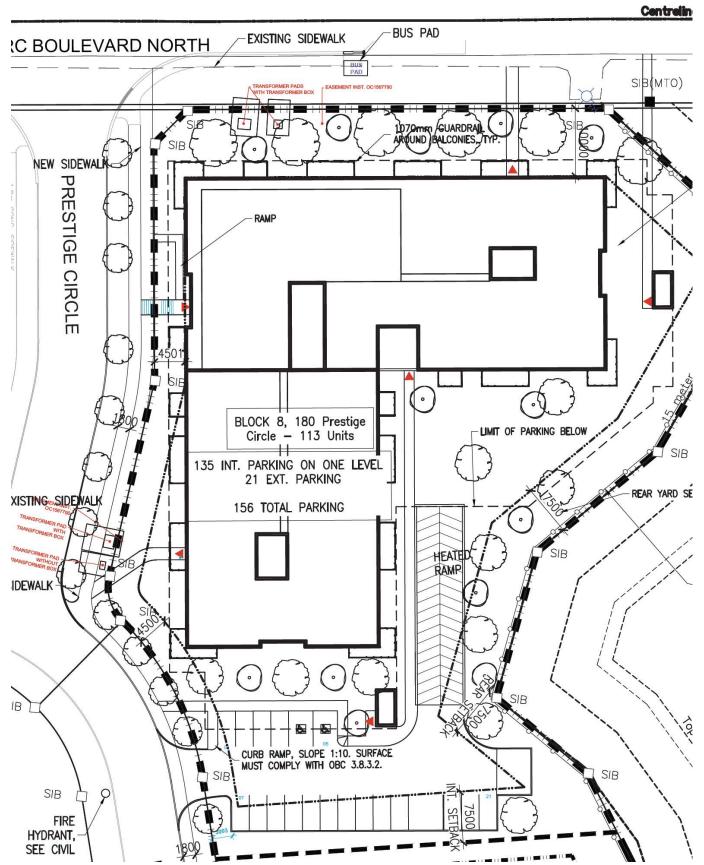
2.1.1. PROPOSED DEVELOPMENT

From the information provided, it is our understanding that Brigil Construction is proposing to proceed with the construction of block 8 within the planned Petrie's Landing II development, located at 8466 Jeanne D'Arc Boulevard in Orleans. The proposed residential block will consist of approximately 113 residential apartment units. Vehicular access will be provided via Prestige Circle and its unsignalized intersection with Jeanne D'Arc Boulevard. The local context of the site is provided as **Figure 1** and the proposed Site Plan is provided as **Figure 2**.



Figure 1: Local Context

Figure 2: Proposed Site Plan



2.1.2. EXISTING CONDITIONS

Area Road Network

OR 174 is an east-west City-owned freeway, which extends from HWY 417 in the west to Trim Road and continues east. Within the study area, OR 174 has a four-lane cross section and is grade separated at its interchange with Tenth Line Road, located approximately 1 km southwest of the site. The posted speed limit within the study area is 100 km/h.

Tenth Line Road is a north-south arterial roadway that extends from Jeanne D'Arc Boulevard in the north to Smith Road in the south. Within the study area, Tenth Line Road has a four-lane cross section and the posted speed limit is 60 km/h.

Jeanne D'Arc Boulevard North is a major collector roadway that extends from Trim Road in the east to OR 174 in the west, where is continues as Jeanne D'Arc Boulevard South. Within the study area, Jeanne D'Arc Boulevard has a two-lane cross section with auxiliary turn lanes provided at major intersections. The posted speed limit is 60 km/h.

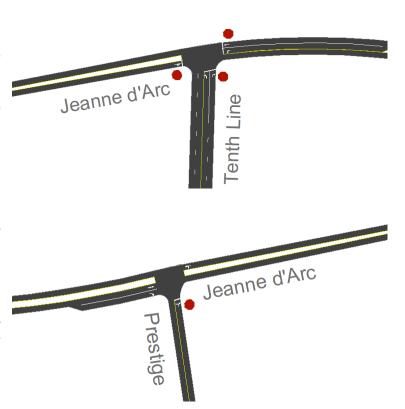
Existing Study Area Intersections

Tenth Line/Jeanne D'Arc

The Tenth Line/Jeanne D'Arc intersection is an unsignalized three-legged all-way stop intersection. The eastbound approach consists of a single through-right turn lane. The westbound approach consists of a left-turn lane and a through lane. The northbound approach consists of a left-turn lane and a right-turn lane. All movements are permitted at this location.

Prestige/Jeanne D'Arc

The Prestige/Jeanne D'Arc intersection is an unsignalized three-legged intersection with STOP control on the northbound approach only. The eastbound approach consists of a single right-turn lane and a through lane. The westbound approach consists of a single left-through lane. The northbound approach consists of a single left-right turn lane. All movements are permitted at this location.



Existing Driveways to Adjacent Developments

There are no existing driveways located on Jeanne D'Arc Boulevard between Rossignol Drive and Parkrose Private. Within Prestige Circle, there are currently 4 existing driveways which provide access to blocks 1 through 6 (block 6 is currently under construction). It is anticipated that block 7 will have its own private driveway when built, adding one more access to the Prestige Circle loop.

Pedestrian/Cycling Network

With regard to pedestrian facilities, there are currently sidewalks provided along the west side of Prestige Circle and the south side of Jeanne D'Arc Boulevard. A multi-use pathway (MUP) is provided along the north side of Jeanne D'Arc Boulevard. A connection from Jeanne D'Arc Boulevard to the MUP is provided adjacent to Prestige Circle. Sidewalks are also provided along the east and west side of Tenth Line Road.

According to the City's Cycling Plan, Jeanne D'Arc Boulevard and Tenth Line Road are classified as "Spine" cycling routes. North of the study area, Tenth Line Road is identified as a Cross-Town Bikeway that connects to St. Joseph Boulevard and Innes Road, which are both identified as Cross-Town Bikeways. Currently, paved shoulders exist along Jeanne D'Arc Boulevard and a MUP is provided along the north side of Jeanne D'Arc Boulevard.

Transit Network

Transit service within the vicinity of the site have recently been updated as of October 6th, 2019 in response to the opening of the Confederation LRT Line. Current service provided by OC Transpo near the vicinity of site includes route #38. Bus stops for route #38 are provided along Jeanne D'Arc Boulevard, adjacent to Prestige Circle, which is an approximate 25 to 150 m walk for residents of the proposed development. Route #38 provides local daily service from Trim Station to Blair Station which connects to the Confederation LRT Line. The transit network for the study area is illustrated in **Figure 3**.

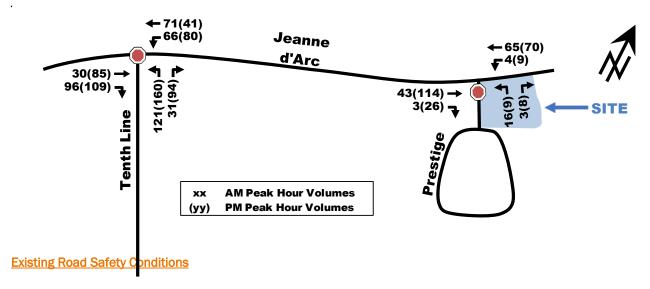


Figure 3: Area Transit Network

Peak Hour Travel Demand

The existing peak hour traffic volumes within the study area are shown in **Figure 4**. The counts were collected by Parsons in 2017. The raw count data has been included in Appendix B.

Figure 4: Existing Peak Hour Traffic Volumes



Collision history for study area intersections and roads (2013 to 2017, inclusive) was obtained from the City of Ottawa. Within the 5 years, only 3 collisions were reported, and most collisions (2/3) involved property damage only, indicating low impact speeds, and 1 of 3 involved personal injury. Of the collisions, 2 were rear end and 1 was single vehicle.

Both rear end collisions occurred at Tenth Line/Jeanne D'Arc while the single vehicle collision took place within the loop at Prestige Circle where a vehicle ran off the road.

The source collision data as provided by the City of Ottawa and related analysis is provided as Appendix C.

2.1.3. PLANNED CONDITIONS

Planned Study Area Transportation Network Changes

Transit

As part of Phase 2 of the LRT Confederation Line in Ottawa, LRT is planned to be extended to Trim Road Station in the east, which is located approximately 1.4 km walk from the subject development. Phase 2 is expected to be completed by 2024. Also identified on the City's TMP Affordable Network is transit priority (isolated measures) along Tenth Line Road, south of St. Joseph Boulevard.

OR-174 Widening

An Environmental Assessment for the potential widening of OR-174 was conducted by the Townships of Prescott-Russell/City of Ottawa. The widening of OR-174 to six-lanes from Hwy 417 to Trim Road and to four-lanes from Trim Road to the City boundary is identified as a road project in the current 2013 City of Ottawa Transportation Master Plan. However, the widening of OR-174 is not identified as part of the Affordable Network Plan within the TMP. Therefore, the road widening of OR-174 east of Trim Road is unlikely within the foreseeable future.

Jeanne D'Arc Culvert Renewal

According to Ward 1 Construction Map, culvert renewal is planned along Jeanne D'Arc Boulevard, west and east of Trim Road for the period 2018 - 2021.

Other Area Development

The following section outlines adjacent developments in the general area that were considered in the TIA.

Petrie's Landing I

Brigil is proposing the construction of the remainder 4 of 6 total residential Towers, consisting of approximately 806 additional residential units and 1,500 sq. meters of commercial. The proposed Petrie's Landing I is located on Jeanne D'Arc Boulevard, approximately 1.5 km east of the subject site, as illustrated in **Figure 5**. Currently, Tower I has been built and Tower II is nearing completion. The projected two-way vehicle trips for this proposed residential development are approximately 70 to 65 veh/h for Tower II and 210 to 180 veh/h for Towers III-VI during the AM and PM peak hours respectively.

Petrie's Landing III

Brigil is proposing the construction of a mixed-use development consisting of approximately $370,000 \, \text{ft}^2$ of office, $23,000 \, \text{ft}^2$ of retail and up to $790 \, \text{residential}$ units. The proposed Petrie's Landing III is located on Jeanne D'Arc Boulevard, approximately $500 \, \text{meters}$ east of the subject site, as illustrated in **Figure 5**. The projected two-way vehicle trips for this proposed mixed-use development is approximately $660 \, \text{and} \, 685 \, \text{veh/h}$ during the morning and afternoon peak hours, respectively.



Figure 5: Petrie's Landing I, II and III Concept Plan

Cardinal Creek Village

Tamarack Homes is currently constructing a 1,446-unit subdivision and a 430,000 ft² shopping centre, south of OR-174 and east of Cardinal Creek, as illustrated in **Figure 6**. The Transportation Impact Study (prepared by IBI Group) projected approximately 1,460 veh/h and 2,619 veh/h by horizon year 2031 (full build-out) during the morning and afternoon peak hours, respectively.

LEGEND

- EUTURE COLLECTOR ROAD
- FUTURE LOCAL ROAD
- FUTURE COLLECTOR ROAD
- FUTURE LOCAL ROAD
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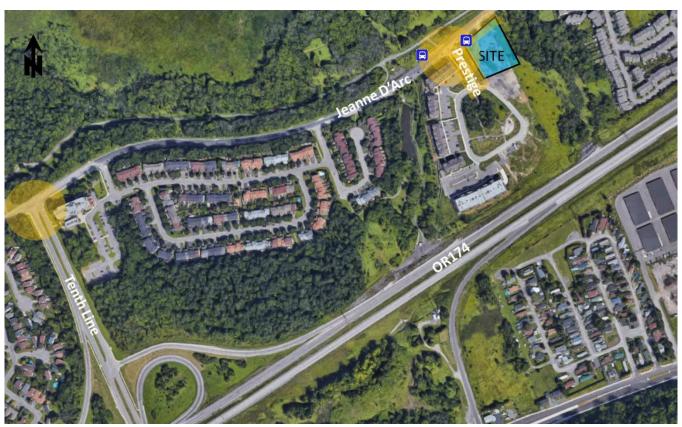
Figure 6: Cardinal Creek Village

2.2. STUDY AREA AND TIME PERIODS

As the proposed site is a residential development, the time periods assessed will be the weekday morning and afternoon peak hours. The proposed study area is outlined below and highlighted in **Figure 7**.

- Tenth Line/Jeanne D'Arc intersection;
- Prestige/Jeanne D'Arc intersection;
- Jeanne D'Arc Boulevard adjacent to the site; and
- Prestige Circle adjacent to the site.

Figure 7: Study Area



2.3. EXEMPTION REVIEW

The following modules/elements of the TIA process recommended to be exempt in the subsequent steps of the TIA process, based on the City's TIA guidelines and the subject site:

Table 1: Exemptions Review Summary

Module	Element	Exemption Consideration
4.1 Development	4.1.2 New	Not required for applications involving site plans.
Design	Streets Network	Not required for applications involving site plans.
4.2 Darking	4.2.2 Spillover	The parking is expected to most Bullow requirements
4.2 Parking	Parking	The parking is expected to meet By-Law requirements.
4.8 Review of	All elements	The site is not expected to generate 200 trips more than the established
Network Concept	All elements	zoning.

3. FORECASTING REPORT

3.1. DEVELOPMENT GENERATED TRAVEL DEMAND

3.1.1. TRIP GENERATION AND MODE SHARES

Trip generation rates for the proposed development, consisting of 113 mid-rise residential apartment units, were obtained from the City's TRANS Trip Generation Report. **Table 2** provides the appropriate trip generation rates for residential use.

Table 2: Vehicle Trip Generation Rates

Landlles	Data Cauras	Trip Rates			
Land Use	Data Source	AM Peak	PM Peak		
Mid-Rise Apartments	TRANS 223 (Table 3.18)	T = 0.29(du)	T = 0.37(du)		
Notes: T = Average Vehicle Trip Ends					

Using the TRANS Trip Generation rates, the total amount of vehicle trips generated by the proposed block 8 development consisting of 113 units was calculated. The results are summarized in **Table 3**.

Table 3: TRANS Vehicle Trip Generation

Land Has Data		Data Units		Peak (veh/l	າ)	PM Peak (veh/h)		
Land Use	Source		In	Out	Total	In	Out	Total
Block 8	TRANS	113	7	26	33	26	16	42

Using the TRANS trip projections in **Table 3** and the mode share percentages from the TRANS Trip Generation Report (Table 3.13), the total projected number of person trips by mode were calculated and are summarized in **Table 4**. The person trips were then used to calculate the vehicle trips generated based on mode shares for Orleans extracted from the OD-Survey conducted in 2011 as seen in **Table 5**.

Table 4: Site Person Trip Generation - Block 8

Trovol Mada	AM Mode	AM	AM Peak (persons/h)			PM Peak (persons/h)		
Travel Mode	Share	In	Out	Total	Share	In	Out	Total
Auto Driver	44%	7	26	33	44%	26	16	42
Auto Passenger	9%	1	5	6	14%	8	6	14
Transit	34%	6	20	26	33%	19	12	31
Non-motorized	13%	2	8	10	9%	5	3	8
Total People Trips	100%	16	59	75	100%	58	37	95

Table 5: Site Vehicle Trip Generation Using OD Mode Shares - Block 8

Troval Mada	AM Mode	AM Peak (veh/h)			PM Mode	PM Peak (veh/h)		
Travel Mode	Share	In	Out	Total	Share	ln	Out	Total
Auto Driver	50%	9	29	38	55%	32	20	52
Auto Passenger	15%	3	8	11	15%	8	6	14
Transit	25%	4	15	19	20%	11	8	19
Non-motorized	10%	1	6	7	10%	6	4	10
Total People Trips	100%	17	58	75	100%	57	38	95
Total 'New' Auto Trips for Block 8		9	29	38	-	32	20	52

As per **Table 5**, block 8 of the subject site is projected to generated approximately 40 and 50 vehicles in the AM and PM peaks respectively.

3.1.2. TRIP DISTRIBUTION AND ASSIGNMENT

Traffic distribution was based on the site's connectivity to the existing road network and our knowledge of the surrounding area. The resultant distribution is outlined as follows:

80% to/from the west; and

• 20% to/from the east.

Based on these distributions, 'new' site-generated trips to/from the proposed development are assigned to study area intersections and are illustrated as **Figure 8**.

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Figure 8: Site-Generated Traffic (Block 8)

3.2.1. TRANSPORTATION NETWORK PLANS

The transportation network changes have been discussed within Section 2.1.3., and none were anticipated to impact the transportation analysis for this development.

3.2.2. BACKGROUND GROWTH

A 2% annual background traffic growth has been added along the Jeanne D'Arc Boulevard through movements to anticipate future development growth along the corridor. Given that Jeanne D'Arc Boulevard between Tenth Line Road and Trim Road (arterials on each side of the study area) are bound by OR 174 and the Ottawa River, a 2% background growth is conservative. Known future developments were superimposed on top of the 2% annual growth and are described in section 3.2.3.

3.2.3. OTHER AREA DEVELOPMENTS

Other area developments were outlined in **Section 2.1.3**. Trips generated by these developments have been summarized in **Table 6**.

Table 6: Other Area Developments Vehicle Trip Generation

		AM Peak (perso	ns/h)	PM Peak (persons/h)			
	In	Out	Total	In	Out	Total	
Petrie's Landing I	72	210	282	144	101	245	
Petrie's Landing II – Blocks 6 & 7	11	35	46	48	30	78	
Petrie's Landing III	422	237	659	254	430	584	
Cardinal Creek (External Only)	412	940	1,352	1,246	980	2,226	
Total	917	1,422	2,339	1,692	1,541	3,233	

Petrie's Landing I - Tower II to VI

Petrie's Landing I – Tower II to VI are expected to be fully occupied by 2024. For a more conservative analysis, all Towers were superimposed to background 2022 and forward. The projected traffic volumes are illustrated in **Figure 9**.

Figure 9: Petrie's Landing I Tower II - IV Projected Traffic Volumes

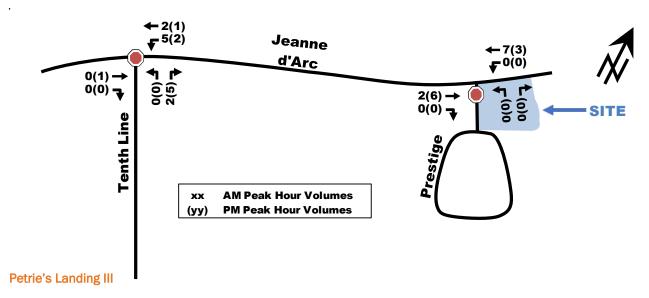
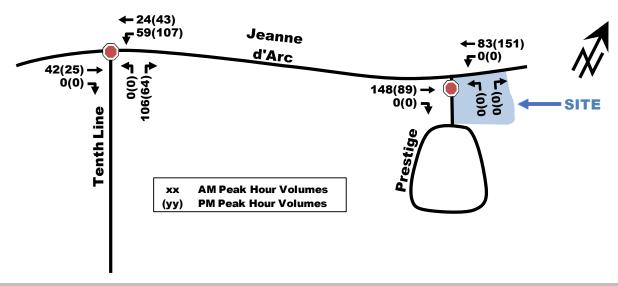


Figure 10 illustrates the projected traffic volumes for Petrie's Landing III at full build-out, obtained from the 2013 Petrie's Landing I TIS. Considering assumed time horizons, 30% of build-out volumes will be applied in year 2022, and 100% in year 2027.

Figure 10: Petrie's Landing III Projected Traffic Volumes - Full Build-Out



Cardinal Creek Village

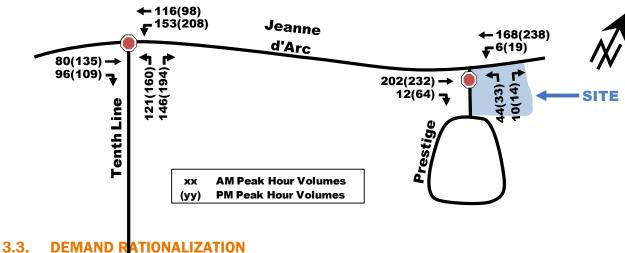
Due to the location of Cardinal Creek Village to the south of OR 174 Highway and unlikely trips to bounded Jeanne D'Arc Boulevard by the Ottawa River and Highway, it is anticipated that no vehicles will enter the study area.

3.2.4. **BACKGROUND TRAFFIC GROWTH**

The future background volumes were calculated by superimposing other area developments on to the network and adding a background growth of 2% along the east-west through lanes on Jeanne D'Arc Boulevard. Background volumes were created for the buildout year 2022 and for 5 years after buildout, year 2027. The resulting background traffic volumes for buildout and buildout plus 5 years are depicted in Figure 11 and Figure 12.

Figure 11: 2022 Background Traffic Volumes

+92(64)**_** 112(133) Jeanne \leftarrow 104(125) d'Arc (19) **عے** 48(110) 96(109) 121(160) 72(149) $93(158) \rightarrow$ 0(14) (33)12(64) 🚤 Prestige **AM Peak Hour Volumes** XX **PM Peak Hour Volumes** (yy) Figure 12: 2027 Background Traffic Volumes



EXISTING CAPACITY CONDITIONS 3.3.1.

The following Table 7 provides a summary of the existing traffic operations at the study area intersection based on the Synchro (V10) traffic analysis software. The subject intersections were assessed in terms of the volume-to-capacity (v/c) ratio and the corresponding Level of Service (LoS) for the critical movement(s). The Synchro model outputs of existing conditions are provided within Appendix D.

Table 7: Existing Intersection Performance

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	nent	Intersection					
	LoS	Delay (s)	Movement	Delay (s)	LoS	v/c			
Tenth Line/Jeanne D'Arc	B(B)	10(10)	NB(NB)	9(10)	A(B)	-			
Prestige/Jeanne D'Arc	A(A)	9(9)	NB(NB)	2(1)	A(A)	-			
Note: Analysis of signalized intersections assumes a PHF of 0.90 and a saturation flow rate of 1800 veh/h/lane.									

As shown in **Table 7**, all the intersections within the subject area are currently operating 'as a whole' at good LoS 'B' or better during the AM and PM peak hours. All 'critical movements' at study area intersections are currently operating at a good LoS 'B' or better during both peak hours.

Overall, the existing Synchro analysis indicates there are no capacity constraints in the network.

4. STRATEGY REPORT

4.1. DEVELOPMENT DESIGN

Location of Transit Facilities

Both the eastbound and westbound OC-Transpo bus stops are located less than 100 meters from block 8 on Jeanne D'Arc Boulevard.

Pedestrian Routes and Facilities

Sidewalks are located along Prestige Circle and the front of each building, connecting all the blocks, the parking areas, to Prestige Circle and Jeanne D'Arc Boulevard. Block 8 proposes sidewalks which connect to existing infrastructure. A multiuse pathway (MUP) connection at Prestige/Jeanne D'Arc currently exists which links the development site to the MUP along the Ottawa River which provides east-west connectivity.

Bicycle Parking

The proponent is providing bicycle parking spaces at a rate of 0.5 per unit which equates to 56 bicycle parking spaces, meeting the City's By-Law requirements. Bicycle parking spaces will be provided indoors in a secure, well-lit area located in the underground parking lot.

Vehicle Access

The proposed development access will be provided through a driveway connecting to Prestige Circle. A two-way ramp is proposed for underground parking in addition to the surface parking proposed.

Its anticipated that City services can be accommodated within the proposed site plan. Fire services will be accommodated from the adjacent frontage roads (Prestige Circle and St. Joseph Blvd). Likewise, curbside garbage service is proposed off Prestige Circle, similar to other blocks within the development.

4.2. PARKING SUPPLY

Based on the City of Ottawa parking bylaws, vehicle and bicycle parking requirements were calculated based on the site's location and are summarized in **Table 8** and **Table 9** respectively.

Table 8: Vehicle Parking Spaces Requirements

Building / Units		Rate	Required	Proposed #					
		(spaces/unit)	Residential	Visitor	Total	of Parking Spaces			
Block 8	113	1.2*	121	20	141	157			
*The first 12 units	*The first 12 units are exempt from the rate 1.2 spaces per unit								

According to **Table 8**, the subject development is required to provide 121 parking spaces for residents, and 20 parking spaces for visitors, for a total of 141 parking spaces. With a total of 157 proposed parking spaces (141 underground and 16 surface parking spaces), the subject development meets City requirements.

Table 9 summarizes bicycle parking requirements as per City of Ottawa Zoning By-Law-Part 4, sections 100-114.

Table 9: Bicycle Parking Requirements

Land Use	Units	# of Bicycle Spaces			
Land USE	Units	Required	Proposed		
Block 8	113	56	56		

According to **Table 9**, the subject development is required to provide 56 bicycle parking spaces. With a total of 56 proposed bicycle parking spaces located indoors in the underground parking garage, the subject meets City requirements.

4.3. BOUNDARY STREET DESIGN

The boundary streets for the development are Jeanne D'Arc Boulevard and Prestige Circle. The existing roadway geometry consists of the following features:

- Jeanne D'Arc Boulevard
 - 1 vehicle travel lane in each direction;
 - o 2m sidewalk with no boulevard on south side of the roadway only; and,
 - Less than 3,000 vehicles per day.
- Prestige Circle
 - 1 vehicle travel lane in each direction;
 - o 2m sidewalk with boulevard on west side of the roadway east side of the roadway proposed; and,
 - Less than 3,000 vehicles per day.

The multi-modal level of service analysis for the subject road segments adjacent to the site is summarized in **Table 10** with detail analysis provided in Appendix E.

Table 10: MMLOS -Road Segments Adjacent to the Site

	Level of Service							
Road Segment	Pedestria	an (PLoS)	Bicycle (BLoS)					
	PLoS	Target	BLoS	Target				
Jeanne D'Arc Boulevard South Side	С	С	С	С				
Jeanne D'Arc Boulevard North Side	F	С	С	С				
Prestige Circle	Α	С	Α	С				

Pedestrian PLoS and cyclist BLoS targets were met on the Prestige Circle road segment. The pedestrian level of service targets for Jeanne D'Arc Boulevard were only met on the south side. The north side of Jeanne D'Arc Boulevard has a PLoS of 'F' due to the lack of a sidewalk. It is noteworthy that there are no destinations to be reached on the north side of the boulevard. The cycling level of service BLoS were met on Jeanne D'Arc Boulevard. There is an isolated multi-use pathway located north of Jeanne D'Arc Boulevard for pedestrians and cyclists.

4.4. ACCESS INTERSECTION DESIGN

The proposed development will make use of a new driveway for access which connects to Prestige Circle. The driveway will access at grade parking and provide a two-way heated ramp with a width of 7 meters to underground parking spaces, meeting the minimum ramp width requirement. Internal circulation is provided by two-way roadways which meet City By-Law requirements.

The driveway access does not exceed a 2% transition grade within 6 meters from the property line. There are no clear throat minimums as the driveway connects to a local roadway. Auxiliary turn lanes and traffic signal lights were not warranted due to the low volume of vehicles generated by the site.

4.5. TRANSPORTATION DEMAND MANAGEMENT

The development generated travel demand has been estimated in Section 3.1.1 using modal shares from the 2011 TRANS O-D survey for Orléans. These modal shares reflect conditions for a wide variety of transportation services supply within Orléans. Once the envisioned LRT is completed, a rise in transit ridership is anticipated, post-occupancy TDM measures are recommended and attached as Appendix F.

4.6. NEIGHBOURHOOD TRAFFIC MANAGEMENT

The development will rely on Prestige Circle for access, which is a local road. It is anticipated that Prestige Circle will have traffic volumes of less than 1,000 vehicles per day and less than 120 vehicles during the peak hours. Since the development is located on a closed loop circuit with this development being the last available space for new developments, it is not anticipated that vehicle volumes will increase within the loop in the future. The large cross-section of approximately 10 meters wide on Prestige Circle will be able to accommodate additional vehicles from this development while continuing to operate as a local roadway.

4.7. ROUTE CAPACITY

It is anticipated that there will be sufficient transit capacity on local route #38 with service every 30 minutes or less during regular service hours. Once the LRT reaches Trim Station in 2024, there will be additional capacity from the 600 passengers per train and 12 trains per hour capacity during peak hours.

4.7.1. PEDESTRIAN CROSSING TREATMENT

Based on Figure 2 – Decision Support Tool located on Book 15 of the Ontario Traffic Manual (OTM) and pedestrian counts conducted on August 13th, 2017, the site is considered a good candidate for a pedestrian crossover. Using a pedestrain crossing count expansion factor to account for new units to be added to the area, it is projected that there will be pedestrian volumes of approximately 65 and vehicle volumes greater than 395 within a 4-hour block which warrants a pedestrian crossover. Additionally, the site is located more than 200 meters from another traffic control device on Jeanne D'Arc Boulevard and the crossing point is considered a pedestrian desire line to access westbound transit and the MUP located north of Jeanne D'Arc Boulevard. Based on the two-way vehicular traffic on Jeanne D'Arc Boulevard, the number of lanes and posted speed limit, OTM recommends a level 2 type C pedestrian crossover as seen in Appendix G.

4.8. INTERSECTION DESIGN

Multi-Modal Level of Service for Intersections

In accordance to the TIA Guidelines, 'Intersection MMLOS' must be completed for signalized intersections only. Given that the study area has no signalized intersections, no MMLOS for intersections will be completed.

4.8.1. PROJECTED BACKGROUND 2022 OPERATIONS

The 2022 background volumes from **Section 3.2.4** and **Figure 11** were inputted in Synchro to analyze the 2022 background conditions. Results are summarized in **Table 11** with detailed analyses provided in Appendix H.

Table 11: Projected 2022 Background Operations at Study Area Intersections

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	nent	Intersection					
	LoS Delay (s)		Movement	Delay (s)	LoS	v/c			
Tenth Line/Jeanne D'Arc	A(B)	9(11)	NB(EB)	9(10)	A(B)	-			
Prestige/Jeanne D'Arc	B(B)	10(10)	NB(NB)	2(1) A(A) -					
Note: Analysis of signalized interse	ections assu	mes a PHF of 1.0 and a	a saturation flow rate	of 1800 veh/h/lane.					

As shown in **Table 11**, all the intersections within the subject area are projected to operate 'as a whole' at good LoS 'B' or better during the AM and PM peak hours. All of the 'critical movements' at study area intersections are projected to operate at good LoS 'B' or better during both peak.

4.8.2. PROJECTED BACKGROUND 2027 OPERATIONS

The 2027 background volumes from Section 3.2.4 and **Figure 12** were inputted in Synchro to analyze the future ultimate background conditions. Results are summarized in **Table 12** with detailed analyses provided in Appendix H.

Table 12: Projected Background 2027 Operations at Study Area Intersections

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	Intersection						
	LoS Delay (s)		Movement	Delay (s)	LoS	v/c			
Tenth Line/Jeanne D'Arc	B(B)	10(12)	WB(EB)	10(12)	B(B)	-			
Prestige/Jeanne D'Arc	B(B)	11(12)	NB(NB)	2(1)	A(A)	-			
Note: Analysis of signalized interse	ections assu	mes a PHF of 1.0 and a	a saturation flow rate	of 1800 veh/h/lane.	•				

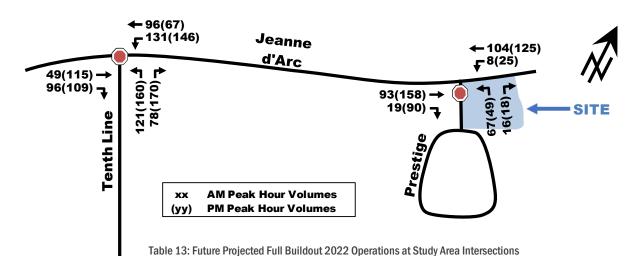
As shown in **Table 12**, all the intersections within the subject area are projected to operate 'as a whole' at good LoS 'B' or better during the AM and PM peak hours. All of the 'critical movements' at study area intersections are projected to operate at good LoS 'B' during both peak hours. No significant changes between existing conditions and 2027 background future were noted.

4.8.3. FUTURE PROJECTED FULL BUILDOUT 2022 CONDITIONS

The future full buildout projected 2022 conditions were derived by superimposing the 2022 background volumes onto the site-generated volumes and are illustrated in **Figure 13**. Synchro results for study area intersection performance are summarized in **Table 13** with detailed analyses provided in Appendix J.



Figure 13: Future Projected Full Buildout 2022 Conditions



_	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	Intersection						
	LoS	Delay (s)	Movement	Delay (s)	LoS	v/c			
Tenth Line/Jeanne D'Arc	A(B)	9(11)	WB(EB)	9(10)	A(B)	-			
Prestige/Jeanne D'Arc	B(B)	10(11)	NB(NB)	3(2)	A(A)	-			
Note: Analysis of signalized interse	Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.								

As shown in **Table 13**, all the intersections within the subject area are projected to operate 'as a whole' at good LoS 'B' or better during the AM and PM peak hours. All of the 'critical movements' at study area intersections are projected to operate at good LoS 'B' or better during both peak hours. No significant changes between 2022 background conditions and 2022 future conditions were noted.

4.8.4. FUTURE PROJECTED FULL BUILDOUT +5 YEARS CONDITIONS

The future projected ultimate buildout conditions plus 5 years were derived by superimposing the 2027 background volumes onto the site-generated volumes and are illustrated in **Figure 14**. Synchro results for study area intersection performance are summarized in **Table 14** with detailed analyses provided in Appendix I.

← 120(101) (221) 172 ــ Jeanne -168(238) d'Arc **_8(25)** 81(140) -96(109) 52(215) 121(160) 202(232) 6(18) (49) 19(90) **Tenth Line** Prestig, **AM Peak Hour Volumes** ХX **PM Peak Hour Volumes** (yy)

Figure 14: Future Projected Full Buildout +5 Years Conditions

17

Table 14: Future Projected Full Buildout + 5 Years Operations at Study Area Intersections

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	nent	Intersection					
	LoS Delay (s)		Movement	Delay (s)	LoS	v/c			
Tenth Line/Jeanne D'Arc	B(B)	10(12)	NB(EB)	10(12)	B(B)	-			
Prestige/Jeanne D'Arc	B(B)	11(12)	NB(NB)	3(2)	A(A)	-			
Note: Analysis of signalized interse	ections assu	mes a PHF of 1.0 and a	a saturation flow rate	of 1800 veh/h/lane.	•				

As shown in **Table 14**, the majority of the intersections within the subject area are projected to operate 'as a whole' at good LoS 'B' or better during the AM and PM peak hours. All of the 'critical movements' at study area intersections are projected to operate at good LoS 'B' during both peak hours.

Overall, there are no significant traffic changes within the study area with the addition of Block 8, a 2% annual growth rate and nearby other area developments when compared to existing conditions.

5. SUMMARY OF FINDINGS

Proposed Development

- The proposed development is located at 8466 Jeanne D'Arc Boulevard
- The site is currently a vacant lot on the south-east corner of Prestige/Jeanne D'Arc
- The proposed development will consist of a 4-storey, 113 residential apartment units and is expected to have occupancy by 2022

Existing Conditions

- Tenth Line/Jeanne D'Arc and Prestige/Jeanne D'Arc intersections operate overall at good LoS 'B' or better
- Both intersections operate with critical movements of LoS 'B' or better

Background Conditions

- A 2% annual growth rate was applied to Jeanne D'Arc Boulevard through movements
- Other area developments noted in the study included:
 - Petrie's Landing I (280-245 veh/h)
 - Petrie's Landing II, addition of blocks 6 & 7 (45-80 veh/h)
 - Petrie's Landing III (660-585 veh/h)
 - Cardinal Creek Community (1,350-2,225 veh/h)
- The other area developments were accounted for separately in the traffic analysis

Trip Generation and Parking

- Full buildout (2022) of the proposed development was expected to generate approximately 40 and 50 vehicle trips during the weekday morning and afternoon periods respectively
- The subject development will provide a total of 157 underground and surface vehicle parking spaces, which meets
 City parking bylaws
- The subject development will provide a total of 56 bicycle parking spaces located indoors in the underground parking garage, which meets City bicycle parking bylaws

Projected Conditions

- Projected full buildout plus 5 years (2027) traffic conditions are projected to be similar to existing traffic conditions, indicating that the new development does not play a big role in worsening network operations
- The MMLOS road segment analysis showed pedestrian and cyclist levels-of-service targets (PLoS and BLoS respectively) were met on Jeanne D'Arc Boulevard and Prestige Circle

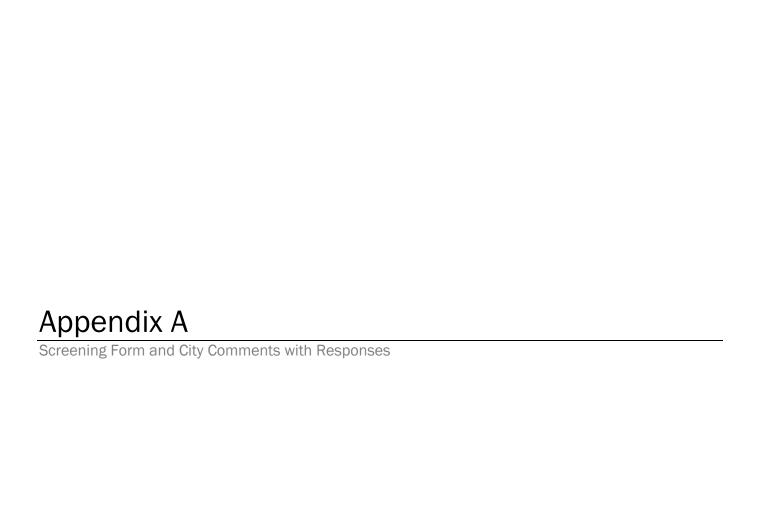
Transit

- Site-generated transit trips at full buildout were approximately 20 trips during the weekday morning and afternoon peak hours
- The estimated transit trips could be accommodated by route #38 with 30-minute intervals
- The future phase 2 of the LRT Confederation Line will add Trim Station, located approximately 1.4km away and offer grade separated public transit with 5-minute intervals during peak hours

Site Access, Circulation and Connectivity

- The proposed development will make use of a new driveway for access which connects to Prestige Circle. The driveway will access at grade parking and provide a two-way heated ramp to underground parking spaces
- The proposed site plan is considered supportive of pedestrian connectivity by providing internal surface sidewalks within the site which offers pedestrian connectivity between all the blocks, to Prestige Circle and Jeanne D'Arc Boulevard
- A level 2 type C pedestrian crossover is recommended at the intersection of Prestige/Jeanne D'Arc to provide improved pedestrian crossing safety to westbound transit services and the MUP located north of Jeanne D'Arc
- The proposed site plan is considered supportive of cycling connectivity by providing a paved MUP connection to the MUP located on the north side of Jeanne D'Arc Boulevard

Based on the foregoing, the proposed 8466 Jeanne D'Arc Boulevard Block 8 of Petrie's Landing II development is recommended from a transportation perspective.





City of Ottawa 2017 TIA Guidelines Date 5-Jun-19 **TIA Screening Form** Project Petrie's Landing 2

Project Number 476705-02000

Results of Screening	Yes/No
Development Satisfies the Trip Generation Trigger	Yes
Development Satisfies the Location Trigger	No
Development Satisfies the Safety Trigger	No

Module 1.1 - Description of Proposed Development	
Municipal Address	8466 Jeanne D'Arc Boulevard
Description of location	Currently a vacant lot, located south of Jeanne D'Arc Blvd and just east of Prestige Circle entry throat
Land Use	Proposed residential building
Development Size	214 Residential Units
Number of Accesses and Locations	A single proposed access to Prestige Circle
Development Phasing	Single-Phased
Buildout Year	2022
Sketch Plan / Site Plan	See attached

Module 1.2 - Trip Generation Trigger		
Land Use Type	Townhomes or Apartments	
Development Size	214	Units
Trip Generation Trigger Met?	Yes	

Module 1.3 - Location Triggers	
Development Proposes a pay driveyout a a boundary street	
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid	No
Transit, or Spine Bicycle Networks (See Sheet 3)	
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone. (See Sheet 3)	No
Location Trigger Met?	No

Module 1.4 - Safety Triggers	
Posted Speed Limit on any boundary road	<80
Horizontal / Vertical Curvature on a boundary street limits sight	No
lines at a proposed driveway	NO
A proposed driveway is within the area of influence of an	
adjacent traffic signal or roundabout (i.e. within 300 m of	
intersection in rural conditions, or within 150 m of intersection	No
in urban/ suburban conditions) or within auxiliary lanes of an	
intersection;	
A proposed driveway makes use of an existing median break	No
that serves an existing site	NO
There is a documented history of traffic operations or safety	
concerns on the boundary streets within 500 m of the	No
development	
The development includes a drive-thru facility	No
Safety Trigger Met?	No





23 February 2021

City of Ottawa

Development Review Services
110 Laurier Avenue West

Ottawa, ON K1P 1J1

Attention: Mike Giampa, P.Eng.

Dear Mike:

Re: Block 8 – 8466 Jeanne D'Arc Boulevard Step 4 – Comment and Response Form

The following letter has been prepared in response to City of Ottawa comments received on January 23rd, 2020. City comments are noted in black with the corresponding responses from Parsons in red.

Note, the site plan has been updated since the prior submission, based on feedback during public consultation. These changes included a reduction in the total number of residential units by nearly half. Therefore, the future road network operations improve, and conclusions and recommendations do not materially change in the updated TIA.

TRANSPORTATION ENGINEERING SERVICES

Correct Jeanne d'Arc's segment PLOS. As indicated in the appendix, the north side has a PLOS of F. Table 10 updated to include both sides of Jeanne D'Arc Boulevard.

Correct BLOS target for Jeanne d'Arc as it is a spine route. Provide any measures that could be used to mitigate the failing conditions. Noted, text corrected.

Provide turning templates for municipal services vehicles for access and emergency services on site.

Fire services will be accommodated from the adjacent streets, and curbside garbage service is proposed off Prestige Circle, similar to other blocks within the development. Therefore, no truck turning templates were necessary.

Confirm the location of the bicycle parking spaces in the final submission. Noted, text updated in Section 4.2.

Provide all access design parameters for the development. Noted, included in Section 4.4.

Include the TDM Measures checklist. TDM Measures Checklist included in Appendix G.

Section 2.3 Table 1 Exemptions Review Summary does not exempt Module 4.6 - Neighbourhood Traffic Management. Complete this review if not exempted or add to Table 1. Section 4.6 added to report.

The comment provided in February 2018 about a PXO requirement is not accepted. Review the need for a PXO at Prestige Circle and Jeanne D'Arc to provide access for pedestrians and cyclists to the MUP and transit stop on the north side. Included in Section 4.7.1.

STREET LIGHTING

Due to the proposed changes in the existing roadway geometry the City of Ottawa Street Lighting Asset Management Group is required to provide a full street light design. Please send hard copy & digital drawings so that we may proceed with the detailed street light design to City of Ottawa, Street Light unit, 185 Slidell St. Ottawa, K1Y 3B5,



attention Barrie Forrester. Be advised that the applicant will be 100% responsible for all costs associated with any relocations/modifications to the existing street light plant. Noted.



Turn Count Summary

Location: Prestige at Jeanne D'Arc, Ottawa

GPS Coordinates:

Date: 2017-09-13
Day of week: Wednesday
Weather: Sunny
Analyst: Rani Nahas

Total vehicle traffic

	1 _								_	_			
Interval starts	So	outhBou	ınd	We	estboun	ıd	Northbound			Ea	astbour	ıd	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IOlai
07:15	0	0	0	0	6	0	1	0	0	0	0	0	7
07:20	0	0	0	0	2	0	2	0	1	0	5	0	10
07:25	0	0	0	0	9	0	2	0	0	0	7	0	18
07:30	0	0	0	0	5	0	0	0	0	0	4	0	9
07:35	0	0	0	1	9	0	3	0	0	0	6	1	20
07:40	0	0	0	1	5	0	0	0	0	0	3	0	9
07:45	0	0	0	0	3	1	1	0	0	0	7	0	12
07:50	0	0	0	1	4	0	2	0	0	0	1	0	8
07:55	0	0	0	0	6	0	2	0	1	0	3	0	12
08:00	0	0	0	0	3	0	2	0	0	0	3	1	9
08:05	0	0	0	0	6	0	1	0	1	0	1	1	10
08:10	0	0	0	1	10	0	0	0	0	0	4	0	15

Car traffic

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	orthbour	nd	E	astbour	ıd	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	iotai
07:15	0	0	0	0	5	0	1	0	0	0	0	0	6
07:20	0	0	0	0	1	0	2	0	1	0	4	0	8
07:25	0	0	0	0	9	0	2	0	0	0	7	0	18
07:30	0	0	0	0	5	0	0	0	0	0	3	0	8
07:35	0	0	0	1	7	0	3	0	0	0	6	1	18
07:40	0	0	0	1	4	0	0	0	0	0	3	0	8
07:45	0	0	0	0	3	1	1	0	0	0	6	0	11
07:50	0	0	0	1	3	0	2	0	0	0	0	0	6
07:55	0	0	0	0	6	0	2	0	1	0	3	0	12
08:00	0	0	0	0	3	0	2	0	0	0	3	1	9
08:05	0	0	0	0	6	0	1	0	1	0	1	1	10
08:10	0	0	0	1	6	0	0	0	0	0	4	0	11

Truck traffic

Interval starts	Sc	uthBou	nd	We	estboun	d	No	rthbour	nd	E	astbour	d	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IUIAI
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0
07:20	0	0	0	0	1	0	0	0	0	0	1	0	2
07:25	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	1	0	1
07:35	0	0	0	0	1	0	0	0	0	0	0	0	1
07:40	0	0	0	0	1	0	0	0	0	0	0	0	1
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0
07:50	0	0	0	0	1	0	0	0	0	0	1	0	2
07:55	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:05	0	0	0	0	0	0	0	0	0	0	0	0	0
08:10	0	0	0	0	3	0	0	0	0	0	0	0	3

Bicycle traffic

08:10

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	ıd	Total
interval starts	Left	Thru	Right	IOIAI									
07:15	0	0	0	0	1	0	0	0	0	0	0	0	1
07:20	0	0	0	0	0	0	0	0	0	0	0	0	0
07:25	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:35	0	0	0	0	1	0	0	0	0	0	0	0	1
07:40	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	1	0	1
07:50	0	0	0	0	0	0	0	0	0	0	0	0	0
07:55	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:05	0	0	0	0	0	0	0	0	0	0	0	0	0
08:10	0	0	0	0	1	0	0	0	0	0	0	0	1
Dodostria	n w	مبياد	100										

Pedestria	an vo	olum	nes										
Interval starts		NE			NW			SW			SE		Total
interval starts	Left	Right	Total	Left	Right	Total	Left	Right	Total	Left	Right	Total	IOlai
07:15	0	0	0	0	0	0	0	1	1	0	0	0	1
07:20	0	0	0	0	0	0	0	0	0	0	0	0	0
07:25	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:35	0	0	0	0	0	0	1	0	1	0	0	0	1
07:40	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0
07:50	0	0	0	0	0	0	0	0	0	0	0	0	0
07:55	0	0	0	0	0	0	1	0	1	0	0	0	1
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:05	0	0	0	0	0	0	0	0	0	0	0	0	0

Intersection Peak Hour

07:15 - 08:15

	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right										
Vehicle Total	0	0	0	4	68	1	16	0	3	0	44	3	139
Factor	0.00	0.00	0.00	0.33	0.57	0.08	0.44	0.00	0.25	0.00	0.52	0.25	0.58
Approach Factor		0.00			0.55			0.53			0.56		

Peak Hour Vehicle Summary

Vehicle	S	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	d	Total
Verlicie	Left	Thru	Right										
Car	0	0	0	4	58	1	16	0	3	0	40	3	125
Truck	0	0	0	0	7	0	0	0	0	0	3	0	10
Bicycle	0	0	0	0	3	0	0	0	0	0	1	0	4

Peak Hour Pedestrians

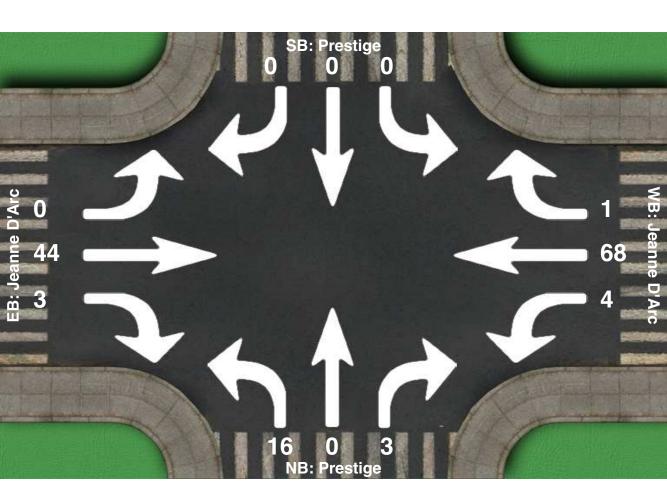
		NE			NW	_		SW			SE		Total
	Left	Right	Total	Iotai									
Pedestrians	0	0	0	0	0	0	2	2	4	0	0	0	4

Intersection Peak Hour

Location: Prestige at Jeanne D'Arc, Ottawa

GPS Coordinates:

Date: 2017-09-13
Day of week: Wednesday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

07:15 - 08:15

	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right										
Vehicle Total	0	0	0	4	68	1	16	0	3	0	44	3	139
Factor	0.00	0.00	0.00	0.33	0.57	0.08	0.44	0.00	0.25	0.00	0.52	0.25	0.58
Approach Factor		0.00			0.55			0.53			0.56		

Turn Count Summary

Prestige at Jeanne D'Arc, Ottawa Location:

GPS Coordinates:

2017-09-13 Date: Day of week: Wednesday Weather: Sunny Analyst: Rani Nahas

Total vehicle traffic

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	ıd	Total
interval starts	Left	Thru	Right	TOTAL									
16:14	0	0	0	0	3	0	0	0	0	0	2	0	5
16:15	1	0	0	0	6	0	2	0	0	0	4	1	14
16:20	0	0	0	0	9	0	1	0	0	0	9	2	21
16:25	0	0	0	0	6	0	1	0	0	0	5	0	12
16:30	0	0	0	0	10	0	0	0	0	0	6	3	19
16:35	0	0	0	0	8	0	2	0	1	0	8	4	23
16:40	0	0	0	0	8	0	1	0	0	0	9	4	22
16:45	0	0	0	0	8	0	1	0	2	0	11	1	23
16:50	0	0	0	2	5	0	0	0	1	0	10	3	21
16:55	0	0	0	1	2	0	0	0	0	0	14	2	19
17:00	0	0	0	0	7	0	1	0	0	0	14	2	24
17:05	0	0	0	2	8	0	2	0	2	0	3	0	17
17:10	0	0	0	1	4	0	3	0	1	0	8	4	21
17:15	0	0	0	1	4	0	0	0	0	0	12	1	18
17:20	0	0	0	1	4	0	0	0	1	0	12	1	19

3

Car traffic

17:25

lutam ral atauta	Sc	outhBou	ınd	We	estboun	d	No	orthbour	nd	Ea	astbour	nd	Takal
Interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
16:14	0	0	0	0	2	0	0	0	0	0	2	0	4
16:15	0	0	0	0	6	0	2	0	0	0	4	1	13
16:20	0	0	0	0	9	0	1	0	0	0	7	2	19
16:25	0	0	0	0	6	0	1	0	0	0	4	0	11
16:30	0	0	0	0	9	0	0	0	0	0	6	3	18
16:35	0	0	0	0	8	0	2	0	1	0	7	4	22
16:40	0	0	0	0	7	0	1	0	0	0	8	4	20
16:45	0	0	0	0	8	0	1	0	2	0	10	1	22
16:50	0	0	0	2	5	0	0	0	1	0	9	2	19
16:55	0	0	0	1	2	0	0	0	0	0	13	2	18
17:00	0	0	0	0	7	0	1	0	0	0	14	2	24
17:05	0	0	0	2	8	0	2	0	2	0	3	0	17
17:10	0	0	0	1	3	0	2	0	1	0	8	4	19
17:15	0	0	0	1	4	0	0	0	0	0	12	1	18
17:20	0	0	0	1	3	0	0	0	1	0	10	1	16
17:25	0	0	0	1	3	0	0	0	0	0	9	1	14

SouthBound Interval starts

Truck traffic

lt.a	So	outhBou	ınd	We	estboun	d	No	orthbour	nd	E	astbour	nd	Takal
Interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
16:14	0	0	0	0	1	0	0	0	0	0	0	0	1
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0
16:20	0	0	0	0	0	0	0	0	0	0	2	0	2
16:25	0	0	0	0	0	0	0	0	0	0	1	0	1
16:30	0	0	0	0	1	0	0	0	0	0	0	0	1
16:35	0	0	0	0	0	0	0	0	0	0	1	0	1
16:40	0	0	0	0	1	0	0	0	0	0	0	0	1
16:45	0	0	0	0	0	0	0	0	0	0	1	0	1
16:50	0	0	0	0	0	0	0	0	0	0	0	1	1
16:55	0	0	0	0	0	0	0	0	0	0	1	0	1
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:05	0	0	0	0	0	0	0	0	0	0	0	0	0
17:10	0	0	0	0	1	0	0	0	0	0	0	0	1
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0
17:20	0	0	0	0	0	0	0	0	0	0	2	0	2
17:25	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycle t	raffi	C											
Interval starts	Sc	outhBou	ınd	We	estboun	d	No	orthbour	nd	E	astbour	nd	Total
micival stalls	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IOIAI

Westbound

Northbound

Eastbound

												9	
16:14	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	1	0	0	0	0	0	0	0	0	0	0	0	1
16:20	0	0	0	0	0	0	0	0	0	0	0	0	0
16:25	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0
16:35	0	0	0	0	0	0	0	0	0	0	0	0	0
16:40	0	0	0	0	0	0	0	0	0	0	1	0	1
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:50	0	0	0	0	0	0	0	0	0	0	1	0	1
16:55	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:05	0	0	0	0	0	0	0	0	0	0	0	0	0
17:10	0	0	0	0	0	0	1	0	0	0	0	0	1
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0
17:20	0	0	0	0	1	0	0	0	0	0	0	0	1
17:25	0	0	0	0	0	0	0	0	0	0	2	0	2
Pedestria	an vo	olun	nes										
Interval starts		NE			NW			SW			SE		Total

17:25

Interval starts	NE			NW			SW			SE			Total
interval starts	Left	Right	Total										
16:14	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0
16:20	0	0	0	0	0	0	0	0	0	1	0	1	1
16:25	0	0	0	0	0	0	4	0	4	0	0	0	4
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0
16:35	0	0	0	0	0	0	0	0	0	0	0	0	0
16:40	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:50	0	0	0	0	0	0	0	0	0	0	0	0	0
16:55	0	0	0	0	0	0	0	0	0	2	0	2	2
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:05	0	0	0	0	0	0	0	0	0	0	0	0	0
17:10	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	2	2	0	0	0	2
17:20	0	0	0	0	0	0	0	0	0	1	0	1	1

Intersection Peak Hour

16:30 - 17:30

	SouthBound			Westbound			Northbound			Ea	Total		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Vehicle Total	0	0	0	9	71	0	10	0	8	0	118	26	242
Factor	0.00	0.00	0.00	0.38	0.59	0.00	0.28	0.00	0.33	0.00	0.70	0.54	0.84
Approach Factor		0.00			0.67			0.38			0.75		

Peak Hour Vehicle Summary

Vehicle	Sc	SouthBound			Westbound			Northbound			Eastbound			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total	
Car	0	0	0	9	67	0	9	0	8	0	109	25	227	
Truck	0	0	0	0	3	0	0	0	0	0	5	1	9	
Bicycle	0	0	0	0	1	0	1	0	0	0	4	0	6	

Peak Hour Pedestrians

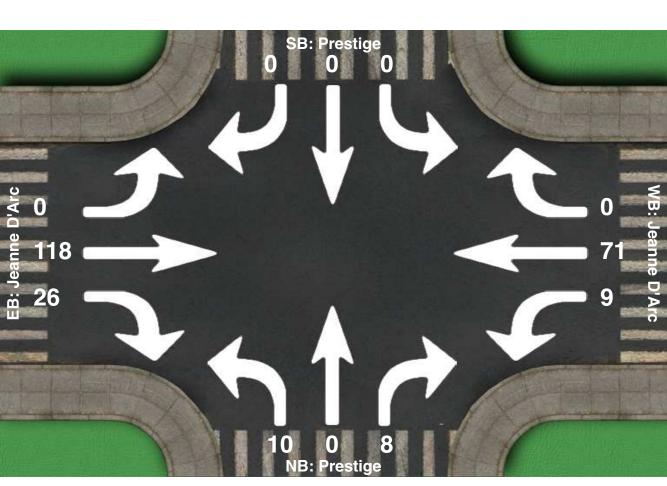
	NE			NW			SW			SE			Total
	Left	Right	Total	Iotai									
Pedestrians	0	0	0	0	0	0	0	2	2	3	0	3	5

Intersection Peak Hour

Location: Prestige at Jeanne D'Arc, Ottawa

GPS Coordinates:

Date: 2017-09-13
Day of week: Wednesday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

16:30 - 17:30

	Sc	outhBou	nd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right										
Vehicle Total	0	0	0	9	71	0	10	0	8	0	118	26	242
Factor	0.00	0.00	0.00	0.38	0.59	0.00	0.28	0.00	0.33	0.00	0.70	0.54	0.84
Approach Factor		0.00			0.67			0.38			0.75		

Turn Count Summary

Location: Tenth Line at Jeanne D'Arc , Ottawa

GPS Coordinates:

Date: 2017-09-14
Day of week: Thursday
Weather: Sunny
Analyst: Rani Nahas

Total vehicle traffic

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	d	Total
interval starts	Left	Thru	Right	iotai									
07:11	0	0	0	5	6	0	4	0	2	0	4	3	24
07:15	0	0	0	7	3	1	2	0	1	0	0	8	22
07:20	0	0	0	7	7	0	8	0	3	0	3	8	36
07:25	0	2	0	6	9	0	10	0	4	0	1	11	43
07:30	0	0	0	2	4	1	13	0	0	0	2	8	30
07:35	0	0	0	4	9	0	8	0	4	0	4	8	37
07:40	0	0	0	4	9	0	8	0	3	0	1	8	33
07:45	0	0	0	5	7	0	16	0	0	0	1	8	37
07:50	0	0	0	2	2	0	16	0	2	0	4	8	34
07:55	0	0	0	7	6	0	7	0	5	0	7	11	43
08:00	0	0	0	9	4	0	16	0	4	0	2	8	43
08:05	0	0	0	7	2	0	13	0	5	0	5	4	36
08:10	0	0	0	6	10	1	14	0	0	0	0	7	38

Car traffic

Interval atorta	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	ıd	Total
Interval starts	Left	Thru	Right	TOTAL									
07:11	0	0	0	4	4	0	4	0	2	0	4	3	21
07:15	0	0	0	7	3	1	2	0	1	0	0	8	22
07:20	0	0	0	6	6	0	6	0	3	0	2	8	31
07:25	0	0	0	6	9	0	8	0	4	0	1	11	39
07:30	0	0	0	2	4	1	11	0	0	0	2	8	28
07:35	0	0	0	4	9	0	6	0	4	0	4	8	35
07:40	0	0	0	4	7	0	7	0	2	0	1	8	29
07:45	0	0	0	4	7	0	16	0	0	0	1	8	36
07:50	0	0	0	2	2	0	16	0	2	0	3	7	32
07:55	0	0	0	7	5	0	7	0	5	0	7	11	42
08:00	0	0	0	9	4	0	15	0	4	0	2	7	41
08:05	0	0	0	7	1	0	13	0	5	0	5	4	35
08:10	0	0	0	6	6	1	12	0	0	0	0	5	30

Truck traffic

Intonial atoms	Sc	outhBou	ınd	We	estboun	d	No	orthbou	nd	E	astbour	nd	Total
Interval starts	Left	Thru	Right	Total									
07:11	0	0	0	1	2	0	0	0	0	0	0	0	3
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0
07:20	0	0	0	1	1	0	1	0	0	0	1	0	4
07:25	0	0	0	0	0	0	1	0	0	0	0	0	1
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:35	0	0	0	0	0	0	0	0	0	0	0	0	0
07:40	0	0	0	0	2	0	0	0	1	0	0	0	3
07:45	0	0	0	1	0	0	0	0	0	0	0	0	1
07:50	0	0	0	0	0	0	0	0	0	0	1	1	2
07:55	0	0	0	0	1	0	0	0	0	0	0	0	1
08:00	0	0	0	0	0	0	0	0	0	0	0	1	1
08:05	0	0	0	0	0	0	0	0	0	0	0	0	0
08:10	0	0	0	0	4	0	0	0	0	0	0	1	5

Bicycle traffic

08:10

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	orthbou	nd	E	astbour	ıd	Total
interval starts	Left	Thru	Right	Iotai									
07:11	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0
07:20	0	0	0	0	0	0	1	0	0	0	0	0	1
07:25	0	2	0	0	0	0	1	0	0	0	0	0	3
07:30	0	0	0	0	0	0	2	0	0	0	0	0	2
07:35	0	0	0	0	0	0	2	0	0	0	0	0	2
07:40	0	0	0	0	0	0	1	0	0	0	0	0	1
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0
07:50	0	0	0	0	0	0	0	0	0	0	0	0	0
07:55	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	1	0	0	0	0	0	1
08:05	0	0	0	0	1	0	0	0	0	0	0	0	1
08:10	0	0	0	0	0	0	2	0	0	0	0	1	3

Interval starts		NE			NW			SW			SE		Total
interval starts	Left	Right	Total	TOTAL									
07:11	0	0	0	0	0	0	0	0	0	1	0	1	1
07:15	0	0	0	0	0	0	0	1	1	0	0	0	1
07:20	0	0	0	0	0	0	0	0	0	0	0	0	0
07:25	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:35	0	0	0	0	0	0	0	1	1	0	0	0	1
07:40	1	0	1	0	0	0	0	0	0	0	0	0	1
07:45	1	0	1	0	0	0	0	1	1	1	0	1	3
07:50	0	0	0	0	0	0	0	0	0	0	0	0	0
07:55	0	0	0	0	0	0	0	1	1	0	1	1	2
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:05	0	0	0	0	0	0	0	0	0	0	0	0	0

Intersection Peak Hour

07:15 - 08:15

	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right										
Vehicle Total	0	2	0	66	72	3	131	0	31	0	30	97	432
Factor	0.00	0.08	0.00	0.61	0.60	0.25	0.68	0.00	0.52	0.00	0.36	0.73	0.84
Approach Factor		0.08			0.69			0.68			0.59		

Peak Hour Vehicle Summary

Vehicle	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
verlicie	Left	Thru	Right										
Car	0	0	0	64	63	3	119	0	30	0	28	93	400
Truck	0	0	0	2	8	0	2	0	1	0	2	3	18
Bicycle	0	2	0	0	1	0	10	0	0	0	0	1	14

Peak Hour Pedestrians

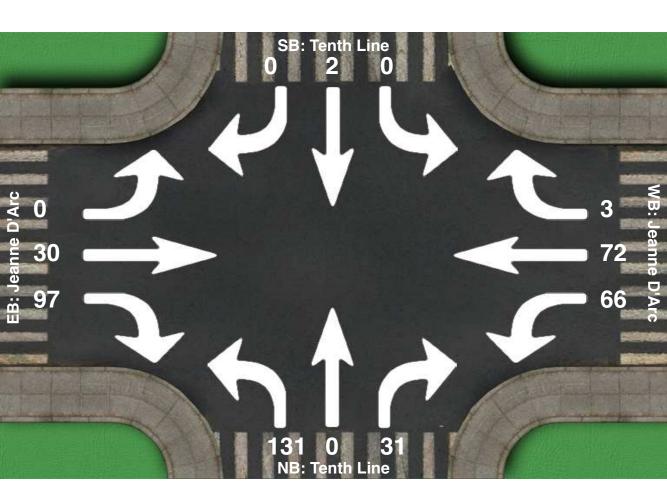
		NE	-		NW			SW			SE	-	Total
	Left	Right	Total	iotai									
Pedestrians	2	0	2	0	0	0	0	4	4	1	1	2	8

Intersection Peak Hour

Location: Tenth Line at Jeanne D'Arc , Ottawa

GPS Coordinates:

Date: 2017-09-14
Day of week: Thursday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

07:15 - 08:15

	Sc	uthBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IOIAI
Vehicle Total	0	2	0	66	72	3	131	0	31	0	30	97	432
Factor	0.00	0.08	0.00	0.61	0.60	0.25	0.68	0.00	0.52	0.00	0.36	0.73	0.84
Approach Factor		0.08			0.69			0.68			0.59		

Turn Count Summary

Location: Tenth Line at Jeanne D'Arc, Ottawa

GPS Coordinates:

2017-09-14 Date: Day of week: Thursday Weather: Sunny Rani Nahas Analyst:

Total vehicle traffic

Interval atoms	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	d	Total
Interval starts	Left	Thru	Right	iotai									
16:27	0	1	0	5	2	1	3	0	4	0	4	3	23
16:30	0	0	0	6	3	0	9	0	7	0	8	7	40
16:35	0	1	0	5	3	0	15	0	7	0	6	8	45
16:40	0	1	0	6	2	0	13	0	5	0	8	12	47
16:45	0	0	0	13	1	0	9	0	6	0	14	7	50
16:50	0	1	0	8	5	0	21	0	6	0	8	13	62
16:55	0	0	0	4	3	0	15	0	9	0	12	10	53
17:00	0	0	0	4	4	0	13	0	9	0	4	5	39
17:05	0	0	0	9	6	0	13	0	10	0	7	9	54
17:10	0	0	0	7	3	0	10	0	13	0	5	6	44
17:15	0	1	0	7	7	1	16	0	7	0	5	10	54
17:20	0	0	0	6	1	0	15	0	7	0	8	17	54
17:25	0	0	0	5	3	0	15	0	9	0	4	9	45
17:30	0	0	0	1	1	0	2	0	2	0	0	2	8

Car traffic

Interval starts	Sc	outhBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	d	Total
interval starts	Left	Thru	Right	iotai									
16:27	0	0	0	5	1	1	3	0	4	0	3	3	20
16:30	0	0	0	6	3	0	9	0	7	0	8	6	39
16:35	0	0	0	5	2	0	14	0	7	0	5	8	41
16:40	0	0	0	6	2	0	13	0	5	0	6	12	44
16:45	0	0	0	13	1	0	8	0	6	0	12	7	47
16:50	0	0	0	8	5	0	21	0	6	0	6	11	57
16:55	0	0	0	4	3	0	15	0	9	0	10	10	51
17:00	0	0	0	4	4	0	11	0	8	0	4	5	36
17:05	0	0	0	9	6	0	13	0	10	0	6	9	53
17:10	0	0	0	7	3	0	10	0	13	0	5	6	44
17:15	0	0	0	7	6	1	16	0	7	0	5	10	52
17:20	0	0	0	6	1	0	15	0	7	0	7	16	52
17:25	0	0	0	5	3	0	15	0	9	0	4	9	45
17:30	0	0	0	1	1	0	2	0	2	0	0	2	8

Truck traffic

Interval starts	Sc	outhBou	ınd	We	estboun	ıd	No	orthbour	nd	E	astbour	nd	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
16:27	0	0	0	0	0	0	0	0	0	0	1	0	1
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0
16:35	0	0	0	0	1	0	0	0	0	0	1	0	2
16:40	0	0	0	0	0	0	0	0	0	0	1	0	1
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:50	0	0	0	0	0	0	0	0	0	0	1	0	1
16:55	0	0	0	0	0	0	0	0	0	0	2	0	2
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:05	0	0	0	0	0	0	0	0	0	0	1	0	1
17:10	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	1	0	0	0	0	0	0	0	1
17:20	0	0	0	0	0	0	0	0	0	0	1	0	1
17:25	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0

SouthBound

17:30

Interval starts	Sc	uthBou	ınd	We	estboun	d	No	rthbour	nd	Ea	astbour	ıd	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IOlai
16:27	0	1	0	0	1	0	0	0	0	0	0	0	2
16:30	0	0	0	0	0	0	0	0	0	0	0	1	1
16:35	0	1	0	0	0	0	1	0	0	0	0	0	2
16:40	0	1	0	0	0	0	0	0	0	0	1	0	2
16:45	0	0	0	0	0	0	1	0	0	0	2	0	3
16:50	0	1	0	0	0	0	0	0	0	0	1	2	4
16:55	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	2	0	1	0	0	0	3
17:05	0	0	0	0	0	0	0	0	0	0	0	0	0
17:10	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	1	0	0	0	0	0	0	0	0	0	0	1
17:20	0	0	0	0	0	0	0	0	0	0	0	1	1
17:25	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestria	an vo	olun	nes										

lutam val atauta		NE			NW			SW			SE		Tatal
Interval starts	Left	Right	Total	Total									
16:27	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	1	1	1
16:35	0	0	0	0	0	0	0	1	1	0	1	1	2
16:40	0	0	0	0	0	0	0	0	0	1	1	2	2
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:50	0	0	0	0	0	0	0	0	0	0	0	0	0
16:55	1	0	1	0	0	0	0	0	0	0	0	0	1
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:05	0	0	0	0	1	1	0	0	0	0	0	0	1
17:10	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0
17:20	0	0	0	0	0	0	0	1	1	0	1	1	2
17:25	0	0	0	2	0	2	0	0	0	0	0	0	2

Intersection Peak Hour

16:30 - 17:30

	SouthBound			Westbound			Northbound			Eastbound			Total
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Vehicle Total	0	4	0	80	41	1	164	0	95	0	89	113	587
Factor	0.00	0.33	0.00	0.51	0.49	0.08	0.65	0.00	0.61	0.00	0.53	0.55	0.79
Approach Factor		0.33		0.68		0.80			0.67				

Peak Hour Vehicle Summary

Vol	Vehicle	SouthBound		Westbound		Northbound			Eastbound			Total		
Vei	licie	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
С	ar	0	0	0	80	39	1	160	0	94	0	78	109	561
Tro	uck	0	0	0	0	2	0	0	0	0	0	7	0	9
Bic	ycle	0	4	0	0	0	0	4	0	1	0	4	4	17

Peak Hour Pedestrians

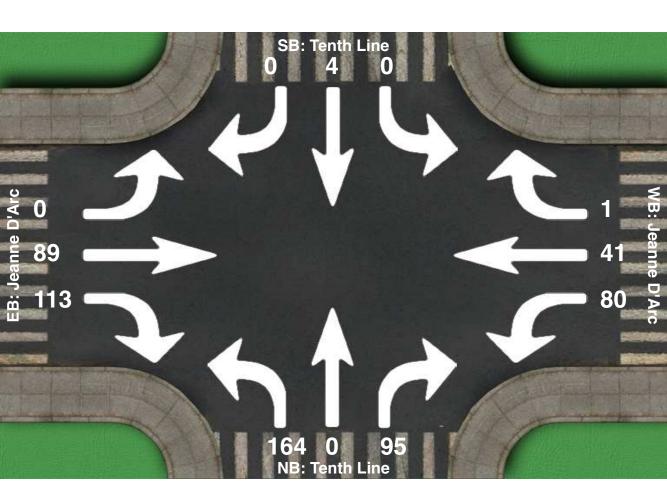
		NE			NW		SW			SE			Total
	Left	Right	Total	Iotai									
Pedestrians	1	0	1	2	1	3	0	2	2	1	4	5	11

Intersection Peak Hour

Location: Tenth Line at Jeanne D'Arc, Ottawa

GPS Coordinates:

Date: 2017-09-14
Day of week: Thursday
Weather: Sunny
Analyst: Rani Nahas



Intersection Peak Hour

16:30 - 17:30

SouthB		outhBou	ınd	Westbound		Northbound			Ea	astboun	d	Total	
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Iolai
Vehicle Total	0	4	0	80	41	1	164	0	95	0	89	113	587
Factor	0.00	0.33	0.00	0.51	0.49	0.08	0.65	0.00	0.61	0.00	0.53	0.55	0.79
Approach Factor		0.33		0.68			0.80		0.67				



Total Area

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	2	0	0	0	0	0	0	0	2
Non-fatal injury	0	0	0	0	0	1	0	0	1
Non reportable	0	0	0	0	0	0	0	0	0
Total	2	0	0	0	0	1	0	0	3
	#1 or 67%	#3 or 0%	#3 or 0%	#3 or 0%	#3 or 0%	#2 or 33%	#3 or 0%	#3 or 0%	

67% 33% 0% 100%

JEANNE D'ARC BLVD/NORTH SERVICE RD @ TENTH LIN

· - /		· (·	
Years	Total #	24 Hr AADT	Dave	Collisions/MEV
rears	Collisions	Veh Volume	Days	Collisions/MEV
5	2	4,320	1825	0.25

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	2	0	0	0	0	0	0	0	2
Non-fatal injury	0	0	0	0	0	0	0	0	0
Non reportable	0	0	0	0	0	0	0	0	0
Total	2	0	0	0	0	0	0	0	2
	1000/	00/-	00/	00/	00/	00/	00/	00/	

100% 0% 0% 100%

PRESTIGE CIR btwn JEANNE D'ARC BLVD N & PRESTIGE CIR

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
5	1	1,250	1825	0.44

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	0	0	0	0	0	0	0	0	0
Non-fatal injury	0	0	0	0	0	1	0	0	1
Non reportable	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	1	0	0	1
	0%	0%	0%	0%	0%	100%	0%	0%	

0% 100% 0% 100%



Intersection						
Intersection Delay, s/veh	9					
Intersection LOS	A					
IIILEI SEULIUII LUS	A					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		7	†	7	7
Traffic Vol, veh/h	30	96	66	71	121	31
Future Vol, veh/h	30	96	66	71	121	31
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	33	107	73	79	134	34
Number of Lanes	1	0	1	1	1	1
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB		_		WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	8.5		8.8		9.5	
HCM LOS	Α.		Α		Α.	
TCIVITUS	A					
HCIVI LOS	A		A		A	
	A	NID! 4		ED! -4		WDLO
Lane	Α	NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Lane Vol Left, %	A	100%	NBLn2	0%	WBLn1 100%	0%
Lane Vol Left, % Vol Thru, %	A	100% 0%	NBLn2 0% 0%	0% 24%	WBLn1 100% 0%	0% 100%
Lane Vol Left, % Vol Thru, % Vol Right, %	A	100% 0% 0%	NBLn2 0% 0% 100%	0% 24% 76%	WBLn1 100% 0% 0%	0% 100% 0%
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	A	100% 0% 0% Stop	NBLn2 0% 0% 100% Stop	0% 24% 76% Stop	WBLn1 100% 0% 0% Stop	0% 100% 0% Stop
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane	A	100% 0% 0% Stop 121	NBLn2 0% 0% 100% Stop 31	0% 24% 76% Stop 126	WBLn1 100% 0% 0% Stop 66	0% 100% 0% Stop 71
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol	A	100% 0% 0% Stop 121 121	NBLn2 0% 0% 100% Stop 31 0	0% 24% 76% Stop 126	WBLn1 100% 0% 0% Stop 66 66	0% 100% 0% Stop 71
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol	A	100% 0% 0% Stop 121 121 0	NBLn2 0% 0% 100% Stop 31 0 0	0% 24% 76% Stop 126 0 30	WBLn1 100% 0% 0% Stop 66 66 0	0% 100% 0% Stop 71 0
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol	A	100% 0% 0% Stop 121 121 0	NBLn2 0% 0% 100% Stop 31 0 0 31	0% 24% 76% Stop 126 0 30	WBLn1 100% 0% 0% Stop 66 66 0	0% 100% 0% Stop 71 0 71
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate	A	100% 0% 0% Stop 121 121 0 0	NBLn2 0% 0% 100% Stop 31 0 0 31 34	0% 24% 76% Stop 126 0 30 96 140	WBLn1 100% 0% 0% Stop 66 66 0 0 73	0% 100% 0% Stop 71 0 71 0 79
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp	A	100% 0% 0% Stop 121 121 0 0 134	NBLn2 0% 0% 100% Stop 31 0 31 34 7	0% 24% 76% Stop 126 0 30 96 140	WBLn1 100% 0% 0% Stop 66 66 0 0 73 7	0% 100% 0% Stop 71 0 71 0 79
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)	A	100% 0% 0% Stop 121 121 0 0 134 7	NBLn2 0% 0% 100% Stop 31 0 31 34 7 0.043	0% 24% 76% Stop 126 0 30 96 140 4 0.176	WBLn1 100% 0% 0% Stop 66 66 00 73 7	0% 100% 0% Stop 71 0 71 0 79 7
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535	WBLn1 100% 0% 0% Stop 66 60 0 73 7 0.115 5.628	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes	WBLn1 100% 0% 0% Stop 66 66 0 0 73 7 0.115 5.628 Yes	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126 Yes
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes 625	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes 788	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes 791	WBLn1 100% 0% 0% Stop 66 66 0 0 73 7 0.115 5.628 Yes 637	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126 Yes 700
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time	A	100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes 625 3.476	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes 788 2.271	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes 791 2.563	WBLn1 100% 0% 0% Stop 66 66 0 73 7 0.115 5.628 Yes 637 3.356	0% 100% 0% Stop 71 0 71 7 0 79 7 0.112 5.126 Yes 700 2.853
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes 625 3.476 0.214	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes 788 2.271 0.043	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes 791 2.563 0.177	WBLn1 100% 0% 0% Stop 66 66 0 73 7 0.115 5.628 Yes 637 3.356 0.115	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126 Yes 700 2.853 0.113
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes 625 3.476 0.214 10	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes 788 2.271 0.043 7.5	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes 791 2.563 0.177 8.5	WBLn1 100% 0% 0% Stop 66 66 0 73 7 0.115 5.628 Yes 637 3.356 0.115 9.1	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126 Yes 700 2.853 0.113 8.5
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 121 121 0 0 134 7 0.214 5.741 Yes 625 3.476 0.214	NBLn2 0% 0% 100% Stop 31 0 0 31 34 7 0.043 4.536 Yes 788 2.271 0.043	0% 24% 76% Stop 126 0 30 96 140 4 0.176 4.535 Yes 791 2.563 0.177	WBLn1 100% 0% 0% Stop 66 66 0 73 7 0.115 5.628 Yes 637 3.356 0.115	0% 100% 0% Stop 71 0 71 0 79 7 0.112 5.126 Yes 700 2.853 0.113

Synchro 10 Report Parsons

Intersection						
Intersection Delay, s/veh	9.9					
Intersection LOS	Α					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		*	<u> </u>	*	7
Traffic Vol, veh/h	85	109	80	41	160	94
Future Vol, veh/h	85	109	80	41	160	94
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	94	121	89	46	178	104
Number of Lanes	1	0	1	1	1/0	1
		J	•	<u>'</u>	•	'
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	10		9.4		10	
HCM LOS	Α		Α		Α	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
		NBLn1 100%	NBLn2	EBLn1	WBLn1 100%	WBLn2
Vol Left, %		100%				
Vol Left, % Vol Thru, %		100% 0%	0% 0%	0% 44%	100% 0%	0% 100%
Vol Left, % Vol Thru, % Vol Right, %		100% 0% 0%	0% 0% 100%	0% 44% 56%	100% 0% 0%	0% 100% 0%
Vol Left, % Vol Thru, % Vol Right, % Sign Control		100% 0% 0% Stop	0% 0% 100% Stop	0% 44% 56% Stop	100% 0% 0% Stop	0% 100% 0% Stop
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		100% 0% 0% Stop 160	0% 0% 100%	0% 44% 56% Stop 194	100% 0% 0% Stop 80	0% 100% 0%
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		100% 0% 0% Stop 160	0% 0% 100% Stop 94	0% 44% 56% Stop 194	100% 0% 0% Stop	0% 100% 0% Stop 41
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		100% 0% 0% Stop 160 160	0% 0% 100% Stop 94 0	0% 44% 56% Stop 194 0 85	100% 0% 0% Stop 80 80	0% 100% 0% Stop 41 0
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		100% 0% 0% Stop 160 160 0	0% 0% 100% Stop 94 0 0	0% 44% 56% Stop 194 0 85	100% 0% 0% Stop 80 80 0	0% 100% 0% Stop 41 0 41
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		100% 0% 0% Stop 160 160 0	0% 0% 100% Stop 94 0 0 94 104	0% 44% 56% Stop 194 0 85 109 216	100% 0% 0% Stop 80 80 0	0% 100% 0% Stop 41 0 41 0 46
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		100% 0% 0% Stop 160 160 0 0 178	0% 0% 100% Stop 94 0 0 94 104	0% 44% 56% Stop 194 0 85 109 216	100% 0% 0% Stop 80 80 0 0	0% 100% 0% Stop 41 0 41 0 46 7
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		100% 0% 0% Stop 160 160 0 0 178 7	0% 0% 100% Stop 94 0 0 94 104 7	0% 44% 56% Stop 194 0 85 109 216 4 0.294	100% 0% 0% Stop 80 80 0 0 89 7	0% 100% 0% Stop 41 0 41 0 46 7
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		100% 0% 0% Stop 160 160 0 0 178 7 0.292 5.916	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915	100% 0% 0% Stop 80 0 0 89 7 0.148 6.008	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		100% 0% 0% Stop 160 160 0 0 178 7 0.292 5.916 Yes	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes	100% 0% 0% Stop 80 0 0 89 7 0.148 6.008 Yes	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% 0% Stop 160 0 0 178 7 0.292 5.916 Yes 604	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes 756	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes 729	100% 0% 0% Stop 80 0 0 89 7 0.148 6.008 Yes 595	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes 648
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		100% 0% 0% Stop 160 0 0 178 7 0.292 5.916 Yes 604 3.676	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes 756 2.468	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes 729 2.968	100% 0% 0% Stop 80 0 0 0 89 7 0.148 6.008 Yes 595 3.77	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes 648 3.265
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 160 0 0 178 7 0.292 5.916 Yes 604 3.676 0.295	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes 756 2.468 0.138	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes 729 2.968 0.296	100% 0% 0% Stop 80 0 0 89 7 0.148 6.008 Yes 595 3.77 0.15	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes 648 3.265 0.071
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		100% 0% 0% Stop 160 160 0 178 7 0.292 5.916 Yes 604 3.676 0.295 11.1	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes 756 2.468 0.138 8.2	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes 729 2.968 0.296 10	100% 0% 0% Stop 80 0 0 0 89 7 0.148 6.008 Yes 595 3.77 0.15 9.8	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes 648 3.265 0.071 8.7
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 160 0 0 178 7 0.292 5.916 Yes 604 3.676 0.295	0% 0% 100% Stop 94 0 0 94 104 7 0.137 4.709 Yes 756 2.468 0.138	0% 44% 56% Stop 194 0 85 109 216 4 0.294 4.915 Yes 729 2.968 0.296	100% 0% 0% Stop 80 0 0 89 7 0.148 6.008 Yes 595 3.77 0.15	0% 100% 0% Stop 41 0 41 0 46 7 0.07 5.504 Yes 648 3.265 0.071

Synchro 10 Report Parsons

Intersection						
Int Delay, s/veh	1.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	T T	TTDL	₩ <u>₩</u>	₩.	וטוז
Traffic Vol, veh/h	43	3	4	65	16	3
Future Vol, veh/h	43	3	4	65	16	3
Conflicting Peds, #/hr	0	0	0	00	0	0
•		Free				
Sign Control	Free		Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length		450	-	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	48	3	4	72	18	3
Major/Minor N	/lajor1	N	Major2	N	Minor1	
						40
Conflicting Flow All	0	0	51	0	128	48
Stage 1	-	-	-	-	48	-
Stage 2	-	-	-	-	80	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1555	-	866	1021
Stage 1	-	-	-	-	974	-
Stage 2	-	-	-	-	943	-
Platoon blocked, %	-	-		_		
Mov Cap-1 Maneuver	_	_	1555	_	863	1021
Mov Cap-2 Maneuver	_	_	-	_	863	-
Stage 1		_	_	<u>-</u>	971	-
•		_			943	
Stage 2	-	-	-	-	943	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.2	
HCM LOS			0.1		A	
1 TOWN EOO					А	
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		885	-	-	1555	-
HCM Lane V/C Ratio		0.024	-	-	0.003	-
HCM Control Delay (s)		9.2	-	-	7.3	0
HCM Lane LOS		Α	_	-	Α	A
HCM 95th %tile Q(veh)		0.1	-	-	0	-
		U. 1				

Intersection						
Int Delay, s/veh	1					
	-	ED.5	14/5	14/57	NIS	NES
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	7		4	Y	
Traffic Vol, veh/h	114	26	9	70	9	8
Future Vol, veh/h	114	26	9	70	9	8
Conflicting Peds, #/hr	0	0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	127	29	10	78	10	9
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	156	0	225	127
Stage 1	-	-	-	-	127	-
Stage 2	-	-	-	-	98	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1424	-	763	923
Stage 1	-	-	-	-	899	-
Stage 2	-	-	-	-	926	-
Platoon blocked, %	-	_		_		
Mov Cap-1 Maneuver	_	_	1424	_	758	923
Mov Cap-2 Maneuver	_	_	-	_	758	-
Stage 1	_		_	_	893	_
Stage 2					926	
Staye 2			_	_	320	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.9		9.4	
HCM LOS					Α	
		unu 4			MA	14/5-
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		828	-	-	1424	-
HCM Lane V/C Ratio		0.023	-	-	0.007	-
HCM Control Delay (s)		9.4	-	-	7.5	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)		0.1	-	-	0	-



Multi-Modal Level of Service - Segments Form

Consultant	Parsons	Project	476705	
Scenario	Prestige Circle	Date	9/25/2017	
Comments	1 - Prestige, 3 - Jeanne,			
	5 - Jeanne w/Sidewalk			

SEGMENTS		Street A	Prestige 1	Section 2	Jeanne North 3	Section 4	Jeanne South 5
	Sidewalk Width Boulevard Width		≥ 2 m > 2 m		no sidewalk n/a		≥ 2 m < 0.5
	Avg Daily Curb Lane Traffic Volume		≤ 3000		≤ 3000		≤ 3000
Pedestrian	Operating Speed On-Street Parking		> 30 to 50 km/h yes		> 50 to 60 km/h no		> 50 to 60 km/h no
ist (Exposure to Traffic PLoS	-	Α	-	F	•	С
Ď	Effective Sidewalk Width						
ď	Pedestrian Volume						
	Crowding PLoS		-	-	-	-	-
	Level of Service		-	-	-	-	-
	Type of Cycling Facility		Mixed Traffic		Curbside Bike Lane		Curbside Bike Lane
	Number of Travel Lanes		≤ 2 (no centreline)		≤ 1 each direction		≤ 1 each direction
	Operating Speed		≤ 40 km/h		>50 to 70 km/h		>50 to 70 km/h
	# of Lanes & Operating Speed LoS		Α	-	С	-	С
Bicycle	Bike Lane (+ Parking Lane) Width						
λίς	Bike Lane Width LoS	С	-	-	-	-	-
ä	Bike Lane Blockages						
	Blockage LoS Median Refuge Width (no median = < 1.8 m)			-	- 10 (-	- 10 (
	No. of Lanes at Unsignalized Crossing		< 1.8 m refuge ≤ 3 lanes		< 1.8 m refuge ≤ 3 lanes		< 1.8 m refuge ≤ 3 lanes
	Sidestreet Operating Speed		≤ 3 lanes ≤ 40 km/h		>50 to 60 km/h		>50 to 60 km/h
	Unsignalized Crossing - Lowest LoS		A	-	B	-	B
	Level of Service		Α	-	С	-	С
±	Facility Type				Mixed Traffic		Mixed Traffic
Transit	Friction or Ratio Transit:Posted Speed	D			Vt/Vp ≥ 0.8		Vt/Vp ≥ 0.8
Ė	Level of Service		-	-	D	-	D
J	Truck Lane Width		> 3.7 m		≤ 3.5 m		≤ 3.5 m
- Pr	Travel Lanes per Direction	С	1		1		1
Truck	Level of Service		В	-	С	-	С



TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

Legend The Official Plan or Zoning By-law provides related guidance that must be followed The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	parking located on backside of site
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	✓ proposed entrances near Jeanne D'Arc
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	yes
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	Bus stop located on corner of Jeanne D'Arc and Prestige, adjacent to building and across the street from building
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	Sidewalks provided to connecting entrances

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	Sidewalks to City standard
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	Sidewalks to City standard
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	Planned connections to existing network
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	Recommended POX on Jeanne D'Arc
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	☑
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	_
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	✓
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	upportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	Bicycle parking located indoors in P1 parking
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	Meets City by-law
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	yes yes
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	•
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	Located indoors
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	. •
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	Meets City parking by-laws
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	Visitor and resident parking separated
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

Legend The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC ★	1.1.1	Designate an internal coordinator, or contract with an external coordinator	✓ Currently existing with other blocks
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	to be determined/provided on website
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	to be determined/provided on website
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
	3.2	Transit fare incentives	
BASIC *	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
	3.3	Enhanced public transit service	
BETTER *	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station (<i>multi-family</i>)	✓ to investigate
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized <i>(multi-family)</i>	
	4.2	Carshare vehicles & memberships	
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	to investigate
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	
	5.	PARKING	
	5.1	Priced parking	
BASIC	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC	5.1.2	Unbundle parking cost from monthly rent (multi-family)	to be determined

	T	DM	measures: Residential developments	Check if proposed & add descriptions
	6.		TDM MARKETING & COMMUNICATIONS	
	6.	1	Multimodal travel information	
BASIC	★ 6.1		Provide a multimodal travel option information package to new residents	
	6.2	2	Personalized trip planning	
BETTER	★ 6.2	2.1	Offer personalized trip planning to new residents	



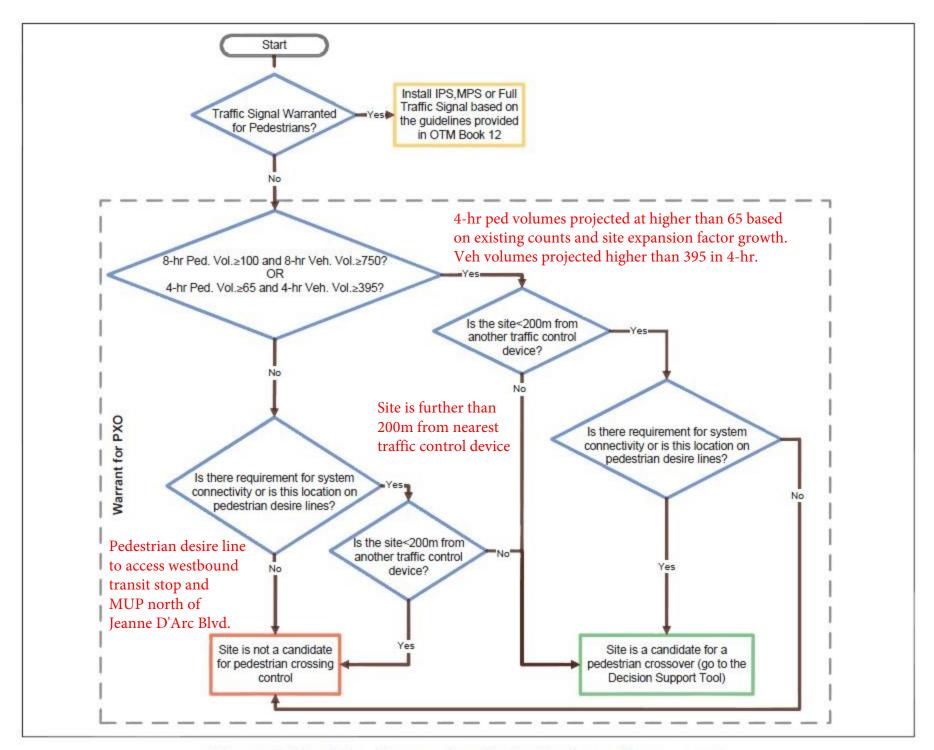


Figure 2: Decision Support Tool - Preliminary Assessment

Table 7: Pedestrian Crossover Selection Matrix

Two-w	ay Vehicular	Volume		Total I	Total Number of Lanes for the Roadway Cross Section ¹								
Time Period	Lower Bound	Upper Bound	Posted Speed Limit (km/h	1 or 2 Lanes	3 lanes	4 lanes w/raised refuge	4 lanes w/o raised refuge						
8 Hour	750	2,250	150	Level 2	Level 2	Level 2	Level 2						
4 Hour	395	1,185	_ ≤50	Type D	Type C ³	Type D ²	Type B						
8 Hour	750	2,250	60	Level 2	Level 2	Level 2	Level 2						
4 Hour	395	1,185	60	Type C	Type B	Type C ²	Type B						
8 Hour	2,250	4,500	<f0< td=""><td>Level 2</td><td>Level 2</td><td>Level 2</td><td>Level 2</td></f0<>	Level 2	Level 2	Level 2	Level 2						
4 Hour	1,185	2,370	_ ≤50	Type D	Type B	Type D ²	Type B						
8 Hour	2,250	4,500	60	Level 2	Level 2	Level 2	Level 2						
4 Hour	1,185	2,370	60	Type C	Type B	Type C ²	Type B						
8 Hour	4,500	6,000	- ≤50	Level 2	Level 2	Level 2	Level 2						
4 Hour	2,370	3,155	_ ≤50	Type C	Type B	Type C ²	Type B						
8 Hour	4,500	6,000	- 60	Level 2	Level 2	Level 2	Level 2						
4 Hour	2,370	3,155	00	Type B	Type B	Type C ²	Type B						
8 Hour	6,000	7,500	_ ≤50	Level 2	Level 2	Level 2	Level 1						
4 Hour	3,155	3,950	≥30	Type B	Type B	Type C ²	Type A						
8 Hour	6,000	7,500	- 60	Level 2	Level 2								
4 Hour	3,155	3,950	00	Type B	Type B								
8 Hour	7,500	17,500	<50	Level 2	Level 2								
4 Hour	3,950	9,215	230	Туре В	Type B								
8 Hour	7,500	17,500	- 60	Level 2									
4 Hour	3,950	9,215		Type B									

Type A Type B Type C Type D

Approaches to roundabouts should be considered a separate roadways.

The hatched cells in this table show that a PXO is not recommended for sites with these traffic and geometric conditions. Generally a traffic signal is warranted for such conditions.

¹The total number of lanes is representative of crossing distance. The width of these lanes is assumed to be between 3.0 m and 3.75 m according to MTO Geometric Design Standards for Ontario Highways (Chapter D.2). A cross sectional feature (e.g. bike lane or on-street parking) may extend the average crossing distance beyond this range of lane widths.

²Use of two sets of side mounted signs for each direction (one on the right side and one on the median)

³ Use Level 2 Type B PXO up to 3 lanes total, cross section one-way.

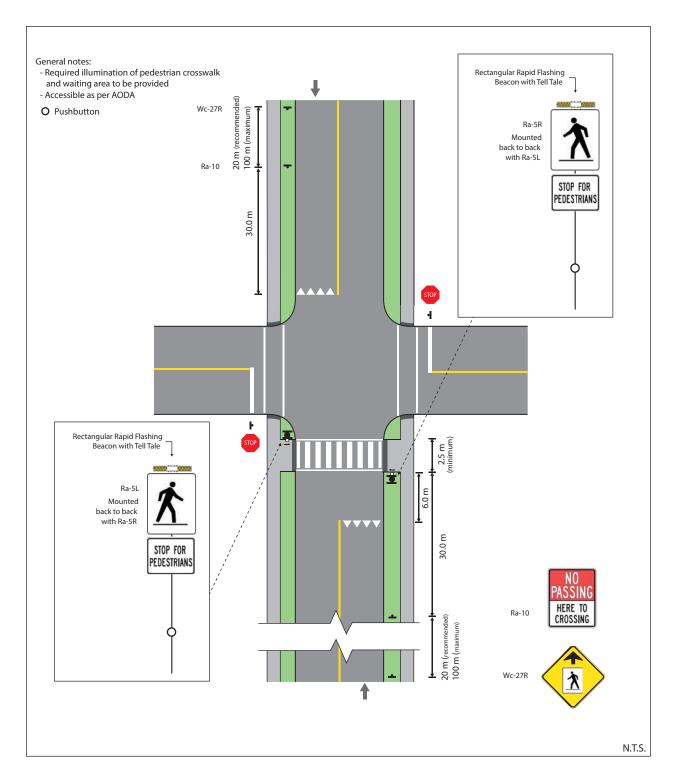


Figure 36: Pedestrian Crossover Level 2 Type C – Intersection (2-way)



Intersection						
Intersection Delay, s/veh	9.1					
Intersection LOS	Α					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		7	^	7	7
Traffic Vol, veh/h	48	96	112	92	121	72
Future Vol, veh/h	48	96	112	92	121	72
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	48	96	112	92	121	72
Number of Lanes	1	0	1	1	1	1
			•	•	•	
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	8.8		9.2		9.3	
HCM LOS	Α		Α		Α	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Thru, %		0%	0%	33%	0%	100%
Vol Right, %		0%	100%	67%	0%	0%
Sign Control		Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane		121	72	144	112	92
LT Vol		121	0	0	112	0
Through Vol		0	0	48	0	92
RT Vol		0	72	96	0	0
Lane Flow Rate		121	72	144	112	92
Geometry Grp		7	7	4	7	7
Degree of Util (X)		0.198	0.094	0.188	0.177	0.133
Departure Headway (Hd)		5.895	4.688	4.703	5.688	5.185
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes
Cap		609	762	762	631	691
Service Time		3.637	2.43	2.741	3.425	2.922
HCM Lane V/C Ratio		0.199	0.094	0.189	0.177	0.133
HCM Control Delay		10.1	7.9	8.8	9.7	8.7
HCM Lane LOS		В	A	A	A	A
HOM OF the tile O		0.7	0.2	0.7	0.6	Λ. Λ.Ε.

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0.7

HCM 95th-tile Q

0.3

0.7

0.6

HCM 95th-tile Q

Intersection						
IIIICI SCOLIOII						
Intersection Delay, s/veh	10.1					
Intersection LOS	В					
	FOT	E55	14/51	MATE	NE	Voc
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		ሻ	^	ሻ	7
Traffic Vol, veh/h	110	109	112	92	160	149
Future Vol, veh/h	110	109	112	92	160	149
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	110	109	112	92	160	149
Number of Lanes	1	0	1	1	1	1
Approach	EB		WB		NB	
					IND	
Opposing Approach	WB		EB			
Opposing Lanes	2		1 ND		0	
Conflicting Approach Left	0		NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB		^		WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	10.5		9.8		10.1	
HCM LOS	В		Α		В	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Thru, %		0%	0%	50%	0%	100%
Vol Right, %		0%	100%	50%		100/0
Sign Control				30 70	0%	0%
0		Stop	Stop			0%
Traffic Vol by Lane		Stop 160	Stop 149	Stop	Stop	0% Stop
Traffic Vol by Lane LT Vol		160	149	Stop 219	Stop 112	0% Stop 92
LT Vol		160 160	149 0	Stop 219 0	Stop 112 112	0% Stop 92 0
LT Vol Through Vol		160 160 0	149 0 0	Stop 219 0 110	Stop 112 112 0	0% Stop 92 0 92
LT Vol Through Vol RT Vol		160 160 0	149 0 0 149	Stop 219 0 110 109	Stop 112 112 0 0	0% Stop 92 0 92
LT Vol Through Vol RT Vol Lane Flow Rate		160 160 0 0 160	149 0 0 149 149	Stop 219 0 110 109 219	Stop 112 112 0 0 112	0% Stop 92 0 92 0
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		160 160 0 0 160 7	149 0 0 149 149 7	Stop 219 0 110 109 219 4	Stop 112 112 0 0 112 7	0% Stop 92 0 92 0 92 7
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		160 160 0 0 160 7 0.271	149 0 0 149 149 7 0.203	Stop 219 0 110 109 219 4 0.31	Stop 112 112 0 0 112 7 0.189	0% Stop 92 0 92 0 92 7 0.142
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		160 160 0 0 160 7 0.271 6.108	149 0 0 149 149 7 0.203 4.9	Stop 219 0 110 109 219 4 0.31 5.097	Stop 112 112 0 0 112 7 0.189 6.079	0% Stop 92 0 92 0 92 7 0.142 5.575
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		160 160 0 0 160 7 0.271 6.108 Yes	149 0 0 149 149 7 0.203 4.9 Yes	Stop 219 0 110 109 219 4 0.31 5.097 Yes	Stop 112 112 0 0 112 7 0.189 6.079 Yes	0% Stop 92 0 92 0 92 7 0.142 5.575 Yes
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		160 160 0 0 160 7 0.271 6.108 Yes 584	149 0 0 149 149 7 0.203 4.9 Yes 726	Stop 219 0 110 109 219 4 0.31 5.097 Yes 701	Stop 112 112 0 0 112 7 0.189 6.079 Yes 586	0% Stop 92 0 92 0 92 7 0.142 5.575 Yes 638
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		160 160 0 0 160 7 0.271 6.108 Yes 584 3.883	149 0 0 149 149 7 0.203 4.9 Yes 726 2.674	Stop 219 0 110 109 219 4 0.31 5.097 Yes 701 3.167	Stop 112 112 0 0 112 7 0.189 6.079 Yes 586 3.858	0% Stop 92 0 92 7 0.142 5.575 Yes 638 3.353
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		160 160 0 0 160 7 0.271 6.108 Yes 584 3.883 0.274	149 0 0 149 149 7 0.203 4.9 Yes 726 2.674 0.205	Stop 219 0 110 109 219 4 0.31 5.097 Yes 701 3.167 0.312	Stop 112 0 0 112 7 0.189 6.079 Yes 586 3.858 0.191	0% Stop 92 0 92 7 0.142 5.575 Yes 638 3.353 0.144
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		160 160 0 0 160 7 0.271 6.108 Yes 584 3.883	149 0 0 149 149 7 0.203 4.9 Yes 726 2.674	Stop 219 0 110 109 219 4 0.31 5.097 Yes 701 3.167	Stop 112 112 0 0 112 7 0.189 6.079 Yes 586 3.858	0% Stop 92 0 92 7 0.142 5.575 Yes 638 3.353

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0.5

0.7

1.3

8.0

Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>LDI</u>	T T	VVDL	₩ 4	₩.	NDIX
Traffic Vol, veh/h	T 93	12	6	104	44	10
	93	12	6		44	10
Future Vol, veh/h			6	104		
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	93	12	6	104	44	10
Major/Minor Major/Minor	ajor1	N	Major2	ı	Minor1	
Conflicting Flow All	0	0	105	0	209	93
Stage 1	-	U	-	-	93	-
Stage 2	_	-	_	_	116	_
	-	-	4.12		6.42	6.22
Critical Hdwy		-		-		
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218		3.518	
Pot Cap-1 Maneuver	-	-	1486	-	779	964
Stage 1	-	-	-	-	931	-
Stage 2	-	-	-	-	909	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1486	-	776	964
Mov Cap-2 Maneuver	-	-	-	-	776	-
Stage 1	-	-	-	-	927	-
Stage 2	-	-	-	-	909	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.8	
	U		0.4			
HCM LOS					Α	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		805	-	-	1486	-
HCM Lane V/C Ratio		0.067	_		0.004	-
HCM Control Delay (s)		9.8	-	-	7.4	0
HCM Lane LOS		Α	_	_	Α	A
HCM 95th %tile Q(veh)		0.2	-	-	0	-

Intersection						
Int Delay, s/veh	1.5					
	EDT	EDD	\\/DI	WDT	NDI	NBR
Movement	EBT	EBR	WBL	WBT	NBL	NBK
Lane Configurations	450	7	40	4	**	4.4
Traffic Vol, veh/h	158	64	19	125	33	14
Future Vol, veh/h	158	64	19	125	33	14
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	158	64	19	125	33	14
Major/Minor	laia-1		Mais =0		Mineral	
	1ajor1		Major2		Minor1	4=0
Conflicting Flow All	0	0	222	0	321	158
Stage 1	-	-	-	-	158	-
Stage 2	-	-	-	-	163	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1347	-	673	887
Stage 1	-	-	-	-	871	-
Stage 2	_	_	_	_	866	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	1347	-	663	887
Mov Cap-2 Maneuver	_	_	-	_	663	-
Stage 1	_	_	_	_	858	_
Stage 2				_	866	_
Slaye Z	_	-	<u>-</u>	<u>-</u>	000	<u>-</u>
Approach	EB		WB		NB	
HCM Control Delay, s	0		1		10.4	
HCM LOS					В	
		.D. 4			\./D.	MOT
Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		717	-		1347	-
HCM Lane V/C Ratio		0.066	-	-	0.014	-
HCM Control Delay (s)		10.4	-	-	7.7	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.2	-	-	0	-

HCM 95th-tile Q

Intersection						
Intersection Delay, s/veh	9.9					
Intersection LOS	Α					
Movement	EDT	EDD	WBL	\\/DT	NDI	NBR
Movement	EBT	EBR		WBT	NBL	
Lane Configurations	}	00	452	110	1 04	140
Traffic Vol, veh/h	80	96	153	116	121	146
Future Vol, veh/h	80	96	153	116	121	146
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	80	96	153	116	121	146
Number of Lanes	1	0	1	1	1	1
Approach	EB		WB		NB	
Opposing Approach	WB		EB			
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	9.8		10.1		9.7	
HCM LOS	9.0 A		В		9.1 A	
HOW LOO						
Lane		NDL 4	ND! 0	ED!	MDL 4	WDL C
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Thru, %		0%	0%	45%	0%	100%
Vol Right, %		0%	100%	55%	0%	0%
Sign Control		Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane		121	146	176	153	116
LT Vol		121	0	0	153	0
Through Vol		0	0	80	0	116
RT Vol		0	146	96	0	0
Lane Flow Rate		121	146	176	153	116
Geometry Grp		7	7	4	7	7
Degree of Util (X)		0.207	0.201	0.246	0.251	0.174
Departure Headway (Hd)		6.155	4.946	5.036	5.915	5.411
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes
Сар		581	720	708	604	659
Service Time		3.921	2.712	3.103	3.683	3.178
HCM Lane V/C Ratio		0.208	0.203	0.249	0.253	0.176
HCM Control Delay		10.5	9	9.8	10.7	9.3
HCM Lane LOS		В	A	Α	В	Α
HOM CELL CL		^ 0	0.7			0.0

Synchro 10 Report Parsons

0.6

0.7

Intersection						
Intersection Delay, s/veh	11.5					
Intersection LOS	В					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		7	†	7	7
Traffic Vol, veh/h	135	109	208	98	160	194
Future Vol, veh/h	135	109	208	98	160	194
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	135	109	208	98	160	194
Number of Lanes	1	0	1	1	1	1
Approach	EB		WB		NB	
Opposing Approach	WB		EB		•	
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	11.8		11.8		11	
HCM LOS	В		В		В	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Thru, %		0%	0%	55%	0%	100%
Vol Right, %		0%	100%	45%	0%	0%
Sign Control		Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane		160	194	244	208	98
LT Vol		160	0	0	208	0
Through Vol		0	0	135	0	98
RT Vol		0	194	109	0	0
Lane Flow Rate		160	194	244	208	98
				4	7	7
Geometry Grp		7		-		
Geometry Grp Degree of Util (X)		7 0.292	7 0.288		· ·	
Degree of Util (X)		0.292	0.288	0.373	0.368	0.16
Degree of Util (X) Departure Headway (Hd)		0.292 6.566	0.288 5.353	0.373 5.507	0.368 6.374	0.16 5.868
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0.292 6.566 Yes	0.288 5.353 Yes	0.373 5.507 Yes	0.368 6.374 Yes	0.16 5.868 Yes
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0.292 6.566 Yes 549	0.288 5.353 Yes 671	0.373 5.507 Yes 654	0.368 6.374 Yes 566	0.16 5.868 Yes 613
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0.292 6.566 Yes 549 4.293	0.288 5.353 Yes 671 3.08	0.373 5.507 Yes 654 3.533	0.368 6.374 Yes 566 4.1	0.16 5.868 Yes 613 3.594
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0.292 6.566 Yes 549 4.293 0.291	0.288 5.353 Yes 671 3.08 0.289	0.373 5.507 Yes 654 3.533 0.373	0.368 6.374 Yes 566 4.1 0.367	0.16 5.868 Yes 613 3.594 0.16
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0.292 6.566 Yes 549 4.293 0.291	0.288 5.353 Yes 671 3.08 0.289 10.2	0.373 5.507 Yes 654 3.533 0.373 11.8	0.368 6.374 Yes 566 4.1 0.367 12.8	0.16 5.868 Yes 613 3.594 0.16 9.7
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0.292 6.566 Yes 549 4.293 0.291	0.288 5.353 Yes 671 3.08 0.289	0.373 5.507 Yes 654 3.533 0.373	0.368 6.374 Yes 566 4.1 0.367	0.16 5.868 Yes 613 3.594 0.16

Synchro 10 Report Parsons

Interception						
Intersection Int Delay, s/veh	1.5					
			14/51	14/5-		
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†	7	_	ન	N.	
Traffic Vol, veh/h	202	12	6	168	44	10
Future Vol, veh/h	202	12	6	168	44	10
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	202	12	6	168	44	10
Major/Minor	nior1		/lois=0		Minera	
	ajor1		Major2		Minor1	000
Conflicting Flow All	0	0	214	0	382	202
Stage 1	-	-	-	-	202	-
Stage 2	-	-	-	-	180	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1356	-	620	839
Stage 1	-	-	-	-	832	-
Stage 2	-	-	-	-	851	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1356	-	617	839
Mov Cap-2 Maneuver	-	-	-	-	617	-
Stage 1	-	-	_	_	828	_
Stage 2	_	_	_	_	851	_
5 kg 5 L					30 1	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		11.1	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		649	-	LDIN	1356	-
HCM Lane V/C Ratio		0.083	-		0.004	-
		11.1		-	7.7	0
HCM Control Delay (s) HCM Lane LOS			-			
		В	-	-	A	Α
HCM 95th %tile Q(veh)		0.3	-	-	0	-

latana attan						
Intersection	1.0					
Int Delay, s/veh	1.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	7		र्स	W	
Traffic Vol, veh/h	232	64	19	238	33	14
Future Vol, veh/h	232	64	19	238	33	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	232	64	19	238	33	14
Mainu/Minnu	-!1		4-:0		M: 4	
	ajor1		Major2		Minor1	000
Conflicting Flow All	0	0	296	0	508	232
Stage 1	-	-	-	-	232	-
Stage 2	-	-	-	-	276	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1265	-	525	807
Stage 1	-	-	-	-	807	-
Stage 2	-	-	-	-	771	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1265	-	516	807
Mov Cap-2 Maneuver	-	-	-	-	516	-
Stage 1	-	-	-	-	793	-
Stage 2	_	-	-	-	771	-
0						
A			\A./D		, LID	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.6		11.8	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	<u> </u>	578	-	-	1265	-
HCM Lane V/C Ratio		0.081	_		0.015	<u>-</u>
HCM Control Delay (s)		11.8	_	_	7.9	0
HCM Lane LOS		В	_	_	7.5 A	A
HCM 95th %tile Q(veh)		0.3	_	_	0	-
HOW SOUT /OUIE Q(VEII)		0.5	-	-	U	_



Intersection						
Intersection Delay, s/veh	9.2					
Intersection LOS	9.2 A					
IIIGISECTION FOS	A					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		ሻ		ሻ	7
Traffic Vol, veh/h	49	96	131	96	121	78
Future Vol, veh/h	49	96	131	96	121	78
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	49	96	131	96	121	78
Number of Lanes	1	0	1	1	1	1
Approach	EB		WB		NB	
Opposing Approach	WB		EB		IND	
Opposing Lanes	2		1		0	
Conflicting Approach Left			NB		EB	
Conflicting Lanes Left	0		2		1	
Conflicting Approach Right	NB				WB	
Conflicting Lanes Right	2		0		2	
HCM Control Delay	8.9		9.4		9.3	
HCM LOS	0.3 A		3. 4		3.5 A	
HOW LOO						
		ND/	ND. C		14/D:	14/D: 6
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Thru, %		0%	0%	34%	0%	100%
Vol Right, %		0%	100%	66%	0%	0%
Sign Control		Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane		121	78	145	131	96
LT Vol		121	0	0	131	0
Through Vol		0	0	49	0	96
RT Vol		0	78	96	0	0
Lane Flow Rate		121	78	145	131	96
Lane Flow Rate Geometry Grp		121 7	78 7	145 4	131 7	96 7
Geometry Grp Degree of Util (X)		7	7	4	7	7
Geometry Grp Degree of Util (X) Departure Headway (Hd)		7 0.2	7 0.103	4 0.191	7 0.208	7 0.139
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		7 0.2 5.957	7 0.103 4.75	4 0.191 4.749	7 0.208 5.706	7 0.139 5.203
Geometry Grp Degree of Util (X) Departure Headway (Hd)		7 0.2 5.957 Yes 601	7 0.103 4.75 Yes 752	4 0.191 4.749 Yes 753	7 0.208 5.706 Yes 629	7 0.139 5.203 Yes
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		7 0.2 5.957 Yes	7 0.103 4.75 Yes	4 0.191 4.749 Yes	7 0.208 5.706 Yes	7 0.139 5.203 Yes 688
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		7 0.2 5.957 Yes 601 3.704	7 0.103 4.75 Yes 752 2.496	4 0.191 4.749 Yes 753 2.793	7 0.208 5.706 Yes 629 3.447	7 0.139 5.203 Yes 688 2.944
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		7 0.2 5.957 Yes 601 3.704 0.201	7 0.103 4.75 Yes 752 2.496 0.104 8	4 0.191 4.749 Yes 753 2.793 0.193	7 0.208 5.706 Yes 629 3.447 0.208	7 0.139 5.203 Yes 688 2.944 0.14
Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		7 0.2 5.957 Yes 601 3.704 0.201 10.2	7 0.103 4.75 Yes 752 2.496 0.104	4 0.191 4.749 Yes 753 2.793 0.193 8.9	7 0.208 5.706 Yes 629 3.447 0.208 9.9	7 0.139 5.203 Yes 688 2.944 0.14 8.8

Intersection						
Intersection Delay, s/veh	10.4					
Intersection LOS	В					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		ች		ች	7
Traffic Vol, veh/h	115	109	146	67	160	170
Future Vol, veh/h	115	109	146	67	160	170
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	115	109	146	67	160	170
Number of Lanes	1	0	1	1	1	1
	EB	,	WB	•	NB	·
Approach Opposing Approach	WB		EB		IND	
Opposing Approach Opposing Lanes	vvB 2		1		0	
	2		NB		EB	
Conflicting Approach Left Conflicting Lanes Left	0		2		1	
	NB				WB	
Conflicting Approach Right Conflicting Lanes Right	2		0		2	
	10.7		10.4		10.3	
HCM LOS					10.3 B	
HCM LOS	В		В		D	
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
Lane Vol Left, %		NBLn1 100%	NBLn2	EBLn1	WBLn1 100%	WBLn2
Vol Left, %		100%	0%	0%	100%	0%
Vol Left, % Vol Thru, %		100% 0%	0% 0%	0% 51%	100% 0%	0% 100%
Vol Left, % Vol Thru, % Vol Right, %		100% 0% 0%	0% 0% 100%	0% 51% 49%	100% 0% 0%	0% 100% 0%
Vol Left, % Vol Thru, % Vol Right, % Sign Control		100% 0% 0% Stop	0% 0% 100% Stop	0% 51% 49% Stop	100% 0% 0% Stop	0% 100% 0% Stop
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		100% 0% 0% Stop 160	0% 0% 100% Stop 170	0% 51% 49% Stop 224	100% 0% 0% Stop 146	0% 100% 0% Stop 67
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		100% 0% 0% Stop 160	0% 0% 100% Stop 170	0% 51% 49% Stop 224 0	100% 0% 0% Stop 146	0% 100% 0% Stop 67
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		100% 0% 0% Stop 160 160	0% 0% 100% Stop 170 0	0% 51% 49% Stop 224 0 115	100% 0% 0% Stop 146 146	0% 100% 0% Stop 67 0
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		100% 0% 0% Stop 160 160 0	0% 0% 100% Stop 170 0 0	0% 51% 49% Stop 224 0 115	100% 0% 0% Stop 146 146 0	0% 100% 0% Stop 67 0 67
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		100% 0% 0% Stop 160 160 0	0% 0% 100% Stop 170 0 0 170	0% 51% 49% Stop 224 0 115 109 224	100% 0% 0% Stop 146 146 0 0	0% 100% 0% Stop 67 0 67
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		100% 0% 0% Stop 160 160 0 160 7	0% 0% 100% Stop 170 0 0 170 170	0% 51% 49% Stop 224 0 115 109 224 4	100% 0% 0% Stop 146 146 0 0	0% 100% 0% Stop 67 0 67 0
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		100% 0% 0% Stop 160 160 0 160 7	0% 0% 100% Stop 170 0 0 170 170 7	0% 51% 49% Stop 224 0 115 109 224 4 0.322	100% 0% 0% Stop 146 146 0 0 146 7	0% 100% 0% Stop 67 0 67 7 0.105
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		100% 0% 0% Stop 160 160 0 160 7 0.274 6.159	0% 0% 100% Stop 170 0 0 170 170 7 0.234 4.95	0% 51% 49% Stop 224 0 115 109 224 4 0.322 5.169	100% 0% 0% Stop 146 146 0 0 146 7 0.249 6.139	0% 100% 0% Stop 67 0 67 7 0.105 5.634
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		100% 0% 0% Stop 160 160 0 160 7 0.274 6.159 Yes	0% 0% 100% Stop 170 0 0 170 170 7 0.234 4.95 Yes	0% 51% 49% Stop 224 0 115 109 224 4 0.322 5.169 Yes	100% 0% 0% Stop 146 146 0 0 146 7 0.249 6.139 Yes	0% 100% 0% Stop 67 0 67 7 0.105 5.634 Yes
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% 0% Stop 160 0 0 160 7 0.274 6.159 Yes 579	0% 0% 100% Stop 170 0 170 7 0.234 4.95 Yes 717	0% 51% 49% Stop 224 0 115 109 224 4 0.322 5.169 Yes 690	100% 0% 0% Stop 146 146 0 0 146 7 0.249 6.139 Yes 580	0% 100% 0% Stop 67 0 67 7 0.105 5.634 Yes 630
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		100% 0% 0% Stop 160 0 0 160 7 0.274 6.159 Yes 579 3.945	0% 0% 100% Stop 170 0 0 170 170 7 0.234 4.95 Yes 717 2.735	0% 51% 49% Stop 224 0 115 109 224 4 0.322 5.169 Yes 690 3.25	100% 0% 0% Stop 146 146 0 0 146 7 0.249 6.139 Yes 580 3.928	0% 100% 0% Stop 67 0 67 7 0.105 5.634 Yes 630 3.423
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 160 0 0 160 7 0.274 6.159 Yes 579 3.945 0.276	0% 0% 100% Stop 170 0 170 170 7 0.234 4.95 Yes 717 2.735 0.237	0% 51% 49% Stop 224 0 115 109 224 4 0.322 5.169 Yes 690 3.25 0.325	100% 0% 0% Stop 146 146 0 0 146 7 0.249 6.139 Yes 580 3.928 0.252	0% 100% 0% Stop 67 0 67 7 0.105 5.634 Yes 630 3.423 0.106

Int Delay, s/veh 2.9 Movement EBT EBR WBL WBT NBL NBR Lane Configurations ↑ ↑ ↓ ↑ ↑ ↓ ↑
Movement EBT EBR WBL WBT NBL NBR Lane Configurations ↑
Lane Configurations ↑ ↑ ↓
Traffic Vol, veh/h 93 19 8 104 67 16 Future Vol, veh/h 93 19 8 104 67 16 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length - 450 - - 0 - - O - - Vol - - 0 0 - - O 0 - - O 0 - - O 0 - - O 0 - - Peak Hour Factor 100
Future Vol, veh/h 93 19 8 104 67 16 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Stop Stop RT Channelized - None - - 0 0 - - 0 0 - - 0 0 - - 0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Stop Stop RT Channelized - None - None - None Storage Length - 450 - - 0 - Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mwmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Minor1 Conflicting Flow All 0 0 112 0 213 93
Sign Control Free Free Free Free Free Stop Stop RT Channelized - None - None - None - None - None Storage Length - 450 0 0 - Confactor 0 0 0 - Conflicting Flow All 0 0 0 - Conflicting Flow 0 0 112 0 213 93
RT Channelized - None - None - None Storage Length - 450 0 - Veh in Median Storage, # 0 0 0 - Grade, % 0 0 0 - Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 Mvmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Storage Length - 450 - - 0 - Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 93 19 8 104 67 16 Major/Minor Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Grade, % 0 - - 0 0 - Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 Mvmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Peak Hour Factor 100 100 100 100 100 100 Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 Mvmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Heavy Vehicles, % 2
Mvmt Flow 93 19 8 104 67 16 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 112 0 213 93
Conflicting Flow All 0 0 112 0 213 93
Conflicting Flow All 0 0 112 0 213 93
Conflicting Flow All 0 0 112 0 213 93
Stage 1 93 -
Stage 2 120 -
Critical Hdwy 4.12 - 6.42 6.22
Critical Hdwy Stg 1 5.42 -
Critical Hdwy Stg 2 5.42 -
Follow-up Hdwy 2.218 - 3.518 3.318
Pot Cap-1 Maneuver 1478 - 775 964
Stage 1 931 -
Stage 2 905 -
Platoon blocked, %
Mov Cap-1 Maneuver 1478 - 770 964
Mov Cap-2 Maneuver 770 -
Stage 1 931 -
Stage 2 900 -
Glago 2
Approach EB WB NB
HCM Control Delay, s 0 0.5 10
HCM LOS B
Minister (M. in M. a.) AIDLA EDT EDD MIDL MIDT
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT
Capacity (veh/h) 801 1478 -
HCM Lane V/C Ratio 0.104 0.005 -
HCM Lane V/C Ratio 0.104 0.005 - HCM Control Delay (s) 10 7.4 0
HCM Lane V/C Ratio 0.104 0.005 -

Intersection						
Int Delay, s/veh	2					
		EDD	14/51	\A/DT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	7		4	À	
Traffic Vol, veh/h	158	90	25	125	49	18
Future Vol, veh/h	158	90	25	125	49	18
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	158	90	25	125	49	18
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	248	0	333	158
Stage 1	-	-	-	-	158	-
Stage 2	-	_	-	-	175	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	_	_	1318	_	662	887
Stage 1	_	_	-	_	871	-
Stage 2	_	_	_	_	855	_
Platoon blocked, %	_	_		_	000	
Mov Cap-1 Maneuver	_	_	1318	_	649	887
Mov Cap-1 Maneuver	_		-		649	- 001
Stage 1	_	<u>-</u>	_	_	871	-
· · · · · · · · · · · · · · · · · · ·	_	-			838	-
Stage 2	-	-	-	-	000	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.3		10.7	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		699	-	-	1318	-
HCM Lane V/C Ratio		0.096	-	-	0.019	-
HCM Control Delay (s)		10.7	-	-	7.8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.3	-	-	0.1	-

HCM Lane V/C Ratio

HCM Control Delay

HCM Lane LOS

HCM 95th-tile Q

Intersection							
Intersection Delay, s/veh	10.1						
Intersection LOS	В						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	₽		ሻ		ች	7	_
Traffic Vol, veh/h	81	96	172	120	121	152	
Future Vol, veh/h	81	96	172	120	121	152	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	81	96	172	120	121	152	
Number of Lanes	1	0	1	1	1	1	
Approach	EB		WB		NB		
Opposing Approach	WB		EB				
Opposing Lanes	2		1		0		
Conflicting Approach Left			NB		EB		
Conflicting Lanes Left	0		2		1		
Conflicting Approach Right	NB				WB		
Conflicting Lanes Right	2		0		2		
HCM Control Delay	9.9		10.4		9.8		
HCM LOS	Α		В		Α		
Lane		NBLn1	NBLn2	EBLn1	WBLn1	WBLn2	
Vol Left, %		100%	0%	0%	100%	0%	
Vol Thru, %		0%	0%	46%	0%	100%	
Vol Right, %		0%	100%	54%	0%	0%	
Sign Control		Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane		121	152	177	172	120	
LT Vol		121	0	0	172	0	
Through Vol		0	0	81	0	120	
RT Vol		0	152	96	0	0	
Lane Flow Rate		121	152	177	172	120	
Geometry Grp		7	7	4	7	7	
Degree of Util (X)		0.209	0.211	0.25	0.284	0.181	
Departure Headway (Hd)		6.216	5.006	5.084	5.935	5.431	
Convergence, Y/N		Yes	Yes	Yes	Yes	Yes	
						0-0	
Cap		574	711	701	602	656	
Cap Service Time		574 3.991	711 2.78	701 3.158	3.708	3.204	

Parsons Synchro 10 Report

0.286

11.1

В

1.2

0.183

9.4

Α

0.7

0.252

9.9

1

0.214

9.1

8.0

Α

0.211

10.6

В

Intersection						
Intersection Delay, s/veh	11.9					
Intersection LOS	В					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f		*	†	ች	7
Traffic Vol, veh/h	140	109	221	101	160	215
Future Vol, veh/h	140	109	221	101	160	215
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	140	109	221	101	160	215
Number of Lanes	1	0	1	1	1	1
	EB	,	WB	•	NB	·
Approach Opposing Approach	WB		EB		IND	
Opposing Approach	vvB 2		1		0	
Opposing Lanes			NB		EB	
Conflicting Approach Left Conflicting Lanes Left	0		2		1	
	NB				WB	
Conflicting Approach Right Conflicting Lanes Right	NB 2		0		WB 2	
	12.1		12.3		11.3	
HCM Control Delay HCM LOS	12.1 B		12.3 B		11.3 B	
					D	
TOW LOO	U					
HOW LOO	Б					
Lane	, ,	NBLn1	NBLn2	EBLn1	WBLn1	WBLn2
	, ,	NBLn1 100%		EBLn1		WBLn2
Lane			NBLn2		WBLn1	
Lane Vol Left, %		100%	NBLn2	0%	WBLn1 100%	0%
Lane Vol Left, % Vol Thru, %		100% 0%	NBLn2 0% 0%	0% 56%	WBLn1 100% 0%	0% 100%
Lane Vol Left, % Vol Thru, % Vol Right, %		100% 0% 0%	NBLn2 0% 0% 100%	0% 56% 44%	WBLn1 100% 0% 0%	0% 100% 0%
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		100% 0% 0% Stop	NBLn2 0% 0% 100% Stop	0% 56% 44% Stop	WBLn1 100% 0% 0% Stop	0% 100% 0% Stop
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		100% 0% 0% Stop 160	NBLn2 0% 0% 100% Stop 215	0% 56% 44% Stop 249	WBLn1 100% 0% 0% Stop 221	0% 100% 0% Stop 101
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		100% 0% 0% Stop 160	NBLn2 0% 0% 100% Stop 215 0	0% 56% 44% Stop 249	WBLn1 100% 0% 0% Stop 221 221	0% 100% 0% Stop 101
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		100% 0% 0% Stop 160 160	NBLn2 0% 0% 100% Stop 215 0	0% 56% 44% Stop 249 0 140	WBLn1 100% 0% 0% Stop 221 221 0	0% 100% 0% Stop 101 0
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		100% 0% 0% Stop 160 160 0	NBLn2 0% 0% 100% Stop 215 0 0 215	0% 56% 44% Stop 249 0 140 109	WBLn1 100% 0% 0% Stop 221 221 0	0% 100% 0% Stop 101 0 101
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		100% 0% 0% Stop 160 160 0	NBLn2 0% 0% 100% Stop 215 0 0 215 215	0% 56% 44% Stop 249 0 140 109 249	WBLn1 100% 0% 0% Stop 221 221 0 0 221	0% 100% 0% Stop 101 0 101
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		100% 0% 0% Stop 160 160 0 160	NBLn2 0% 0% 100% Stop 215 0 215 215 7	0% 56% 44% Stop 249 0 140 109 249	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7	0% 100% 0% Stop 101 0 101 7
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		100% 0% 0% Stop 160 160 0 0 160 7	NBLn2 0% 0% 100% Stop 215 0 215 215 7 0.324	0% 56% 44% Stop 249 0 140 109 249 4 0.387	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396	0% 100% 0% Stop 101 0 101 7
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		100% 0% 0% Stop 160 160 0 160 7 0.295 6.637	NBLn2 0% 0% 100% Stop 215 0 0 215 215 7 0.324 5.423	0% 56% 44% Stop 249 0 140 109 249 4 0.387 5.597	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396 6.449	0% 100% 0% Stop 101 0 101 7 0.167 5.943
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		100% 0% 0% Stop 160 160 0 0 160 7 0.295 6.637 Yes	NBLn2 0% 0% 100% Stop 215 0 0 215 215 7 0.324 5.423 Yes	0% 56% 44% Stop 249 0 140 109 249 4 0.387 5.597 Yes	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396 6.449 Yes	0% 100% 0% Stop 101 0 101 7 0.167 5.943 Yes
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% 0% Stop 160 0 0 160 7 0.295 6.637 Yes 542	NBLn2 0% 0% 100% Stop 215 0 0 215 215 7 0.324 5.423 Yes 664	0% 56% 44% Stop 249 0 140 109 249 4 0.387 5.597 Yes 642	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396 6.449 Yes 559	0% 100% 0% Stop 101 0 101 7 0.167 5.943 Yes 604
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		100% 0% 0% Stop 160 0 0 160 7 0.295 6.637 Yes 542 4.367	NBLn2 0% 0% 100% Stop 215 0 0 215 215 7 0.324 5.423 Yes 664 3.153	0% 56% 44% Stop 249 0 140 109 249 4 0.387 5.597 Yes 642 3.628	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396 6.449 Yes 559 4.181	0% 100% 0% Stop 101 0 101 7 0.167 5.943 Yes 604 3.674
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% 0% Stop 160 0 0 160 7 0.295 6.637 Yes 542 4.367 0.295	NBLn2 0% 0% 100% Stop 215 0 05 215 7 0.324 5.423 Yes 664 3.153 0.324	0% 56% 44% Stop 249 0 140 109 249 4 0.387 5.597 Yes 642 3.628 0.388	WBLn1 100% 0% 0% Stop 221 221 0 0 221 7 0.396 6.449 Yes 559 4.181 0.395	0% 100% 0% Stop 101 0 101 7 0.167 5.943 Yes 604 3.674 0.167

Synchro 10 Report Parsons

Interception						
Intersection	2.1					
Int Delay, s/veh						
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	7		4	¥	
Traffic Vol, veh/h	202	19	8	168	67	16
Future Vol, veh/h	202	19	8	168	67	16
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	450	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	202	19	8	168	67	16
N A - ' (N A'			1.1.0		A	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	221	0	386	202
Stage 1	-	-	-	-	202	-
Stage 2	-	-	-	-	184	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1348	-	617	839
Stage 1	-	-	-	-	832	-
Stage 2	-	-	-	-	848	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1348	_	613	839
Mov Cap-2 Maneuver	-	-	-	-	613	-
Stage 1	_	_	_	_	832	_
Stage 2	_	_	_	_	842	_
					J 12	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		11.4	
HCM LOS					В	
Minor Lang/Major Mumt	N	NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Major Mvmt	ľ					
Capacity (veh/h)		647	-	-	1348	-
HCM Lane V/C Ratio		0.128	-	-	0.006	-
HCM Control Delay (s)		11.4	-	-	7.7	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.4	-	-	0	-

Interception						
Intersection	4.0					
Int Delay, s/veh	1.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7		4	¥	
Traffic Vol, veh/h	232	90	25	238	49	18
Future Vol, veh/h	232	90	25	238	49	18
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	450	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	232	90	25	238	49	18
N.A. '. (N.A'						
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	322	0	520	232
Stage 1	-	-	-	-	232	-
Stage 2	-	-	-	-	288	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1238	-	516	807
Stage 1	-	-	-	-	807	-
Stage 2	-	-	-	-	761	-
Platoon blocked, %	-	_		-		
Mov Cap-1 Maneuver	_	-	1238	-	504	807
Mov Cap-2 Maneuver	_	_	-	_	504	-
Stage 1	_	-	-	_	807	-
Stage 2	_	_	_	_	743	_
Jugo L					. 10	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		12.3	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
	- 1					
Capacity (veh/h)		561	-	-	1238	-
HCM Cartral Dalay (2)		0.119	-	-	0.02	-
HCM Control Delay (s)		12.3	-	-	8	0
HCM Lane LOS		В	-	-	A	Α
HCM 95th %tile Q(veh)		0.4	-	-	0.1	-