

**GWL Realty Advisors**

**320 McRae Ave**



**Transportation  
Impact  
Assessment**



# 320 McRae

## Transportation Impact Assessment

Step 1 Screening Report

Step 2 Scoping Report

Step 3 Forecasting Report

Step 4 Strategy Report

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January 2020

PN: 2019-29

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# 1 Screening

This study has been prepared according to the City of Ottawa’s 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for TIA Study PM. As shown in the Screening Form, a TIA is required including the Design Review component and the Network Impact Component. This study has been prepared to support a site plan application for 320 McRae Avenue.

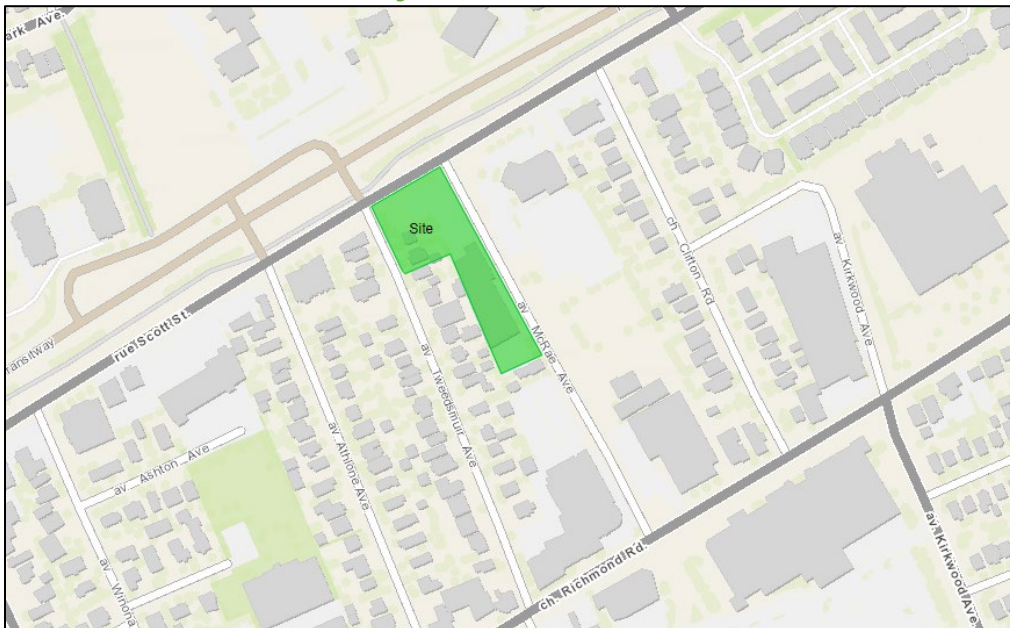
# 2 Existing and Planned Conditions

## 2.1 Proposed Development

The proposed development located at 320 McRae Avenue is currently a mix of residential and commercial buildings. The site is in an area that is zoned as part Traditional Mainstreet (TM 2489 S382-h), part Parks and Open Space (O 1) and part General Mixed Zone (GM2490 H (15) h). The proposed development is within 400 metres of the future Westboro LRT Station to be built by 2025 and therefore TOD principles apply to the applicable future horizons.

The proposed development is made up of a four-storey commercial / residential tower, and a commercial / residential tower with both a 26-storey and a six-storey component. The development is expected to have 882 square metres (9,494 square feet) of commercial space, 307 apartment units, 11 townhouse units, 185 underground automobile parking spaces and 163 bicycle parking spaces. Of the 163 bicycle spaces, 123 will be underground and due to space restrictions, 15 bicycle parking spaces will be slightly off the property and 25 will be in the loading area. The site is proposed to have two full-movement accesses, one approximately 40 metres, curb to curb, south of Scott Street on Tweedsmuir Avenue (Site Access #1) and the second approximately 120 metres, curb to curb, south of Scott Street on McRae Avenue (Site Access #2). Site Access #2 is a loading access and is intended for truck use only. A drop-off area is located on McRae Avenue, approximately 23 metres, curb to curb, south of Scott Street. The anticipated full build-out and occupancy horizon is 2022. Figure 1 illustrates the Study Area Context. Figure 2 illustrates the proposed concept plan.

Figure 1: Area Context Plan





FLOOR PLAN - GROUND LEVEL  
1:200

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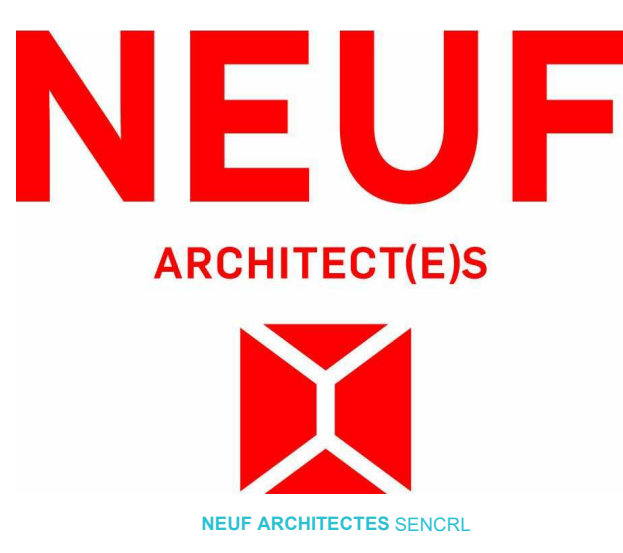
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OUVRAGE / Project  
**320 McRAE AVENUE**

EMPLACEMENT / Location  
1976 SCOTT ST & 320  
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OTTAWA, ON

NO PROJET No.  
12087

NO	REVISION	DATE

DESSINÉ PAR / Drawn by  
Author

VERIFIÉ PAR / Checked  
Checker

DATE  
29/01/20

ECHELLE / Scale  
1 : 200

TITRE DU DESSIN / Drawing Title

**FLOOR PLAN - GROUND LEVEL**

REVISION / Revision  
NO. DESSIN / Dwg Number

## 2.2 Existing Conditions

### 2.2.1 Area Road Network

**Scott Street:** Scott Street is a City of Ottawa arterial road with a two-lane cross-section and has a posted speed limit of 50 km/h. Scott Street primarily has curbs and gutters on the south side of the road and has a gravel shoulder on the north side. Intermittent parking lanes exist on the south side of the street. Bicycle lanes are present on both sides of the road. The south side has a sidewalk and the north side has a pedestrian asphalt pathway. The existing right-of-way is 26.0 metres.

**McRae Avenue:** McRae Avenue is a local road with a two-lane cross-section. There is an unposted speed limit of 50 km/h. McRae Avenue has curbs and gutters, as well as sidewalks on both sides of the road. The measured right-of-way is 17.0 metres to the south of Scott Street and gradually narrows to 16.0 metres just north of Richmond Road.

**Richmond Road:** Richmond Road is a City of Ottawa arterial road with a four-lane cross-section and on-street parking allowed on both sides. Within the Study Area, Richmond Road has curbs and gutters as well as sidewalks on both sides of the street. The posted speed limit is 50 km/h. The measured existing right-of-way within the Study Area varies between 15.0 metres and 21.0 metres. Richmond Road is designated as a trucking route.

**Tweedsmuir Avenue:** Tweedsmuir Avenue is a local road with a two-lane cross-section. There is an unposted speed limit of 50 km/h. Tweedsmuir Avenue has curbs and gutters, as well as a sidewalk on the east side of the road. There is on-street parking allowed on both the east and west side. The measured right-of-way is 18.0 metres.

### 2.2.2 Existing Intersections

#### *Scott Street / Tweedsmuir Avenue*

The intersection at Scott Street / Tweedsmuir Avenue is a two-way stop-controlled intersection with stop control on the north and south legs. Directly to the east of this intersection, on Scott Street, is a signalized pedestrian crossing that will be considered part of the intersection. The northern leg does not allow vehicle entry as it is a driveway for buses entering and exiting Westboro Station. The intersection is therefore subject to the appropriate passenger vehicle movement restrictions; no eastbound left-turn, westbound right-turn or northbound through movements. The westbound approach consists of a shared left-turn/through/right-turn lane, the eastbound approach consists of a shared left-turn/through/right-turn lane and the northbound approach consists of a shared left-turn/through/right-turn lane. Trucks are not permitted south of the intersection along Tweedsmuir Avenue.





*Scott Street / McRae Avenue*

The intersection at Scott Street / McRae Avenue is an unsignalized T-intersection. The northbound movement on McRae Avenue is stop-controlled and is a shared left-turn/right-turn lane. The eastbound movement is a shared through/right-turn lane and the westbound movement is a shared through/left-turn lane. No turn restrictions were noted.



*Richmond Road / Tweedsmuir Avenue*

The intersection at Richmond Road / Tweedsmuir Avenue is an unsignalized intersection. Both the northbound and southbound movements are stop-controlled, shared left-turn/through/right-turn lanes. The eastbound movement consists of a shared left-turn/through/right-turn lane and the westbound movement consists of a shared left-turn/through lane and a right-turn lane. No turn restrictions were noted.



*Richmond Road / McRae Avenue*

The intersection at Richmond Road / McRae Avenue is a signalized intersection. The northbound and westbound movements both have auxiliary left-turn lanes and a shared through/right-turn lane. The eastbound movement has an auxiliary left-turn lane and a shared through/right-turn lane. The southbound movement is a shared left-turn/through/right-turn movement. No turn restrictions were noted.



2.2.3 Existing Driveways

Within 200 metres of the proposed site access there are multiple existing driveways along Tweedsmuir Avenue, Scott Street, and McRae Avenue. Tweedsmuir Avenue has multiple residential driveways on both sides of the road. McRae Avenue has multiple office, residential and retail driveways on both sides of the road. The primary driveway along Scott Street within 200 metres of the proposed site is the bus entrance and exit to the Westboro Station which is just north of the proposed site. None of these driveways provide access to significant traffic generators and would therefore have no impact on this TIA.

2.2.4 Cycling and Pedestrian Facilities

Sidewalks are provided along one side of McRae Avenue, Scott Street and Tweedsmuir Avenue in the Study Area. Additionally, a multi-use pathway is provided on the other side of Scott Street. Sidewalks are provided along both sides of Richmond Road. The cycling network consists of bike lanes on Scott Street, a pathway just north of Scott Street and a suggested spine route along Richmond Road and Scott Street. Figure 3 illustrates the pedestrian facilities in the Study Area and Figure 4 illustrates the cycling facilities.

Figure 3: Study Area Pedestrian Facilities

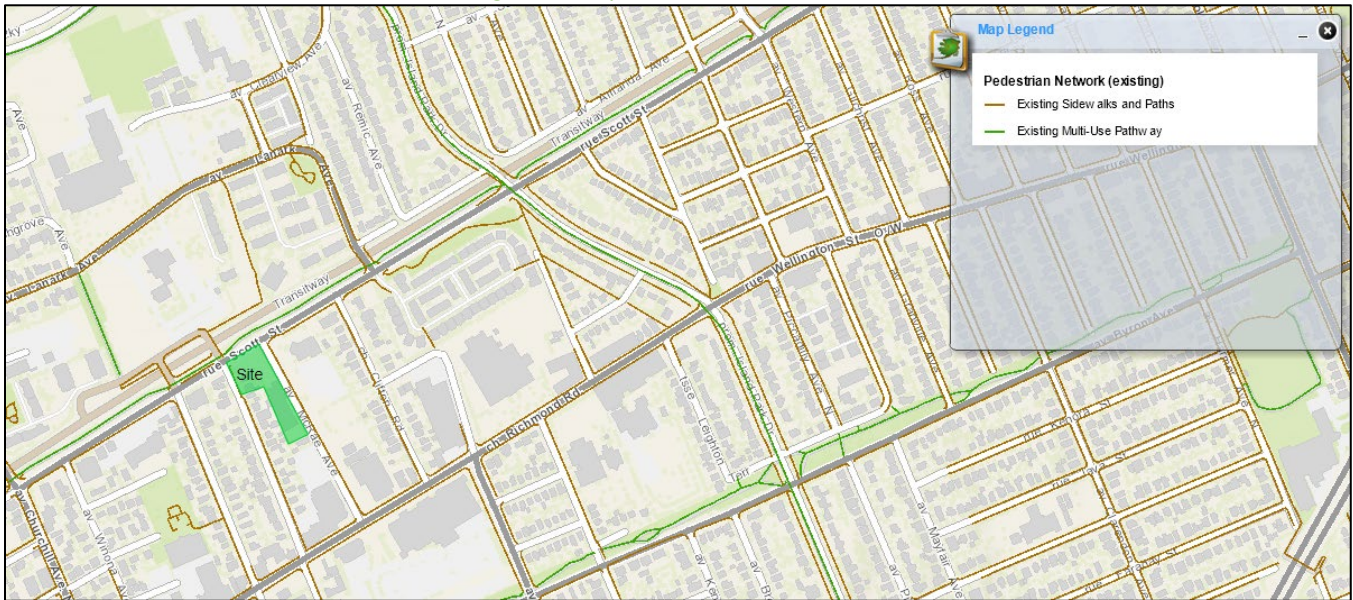
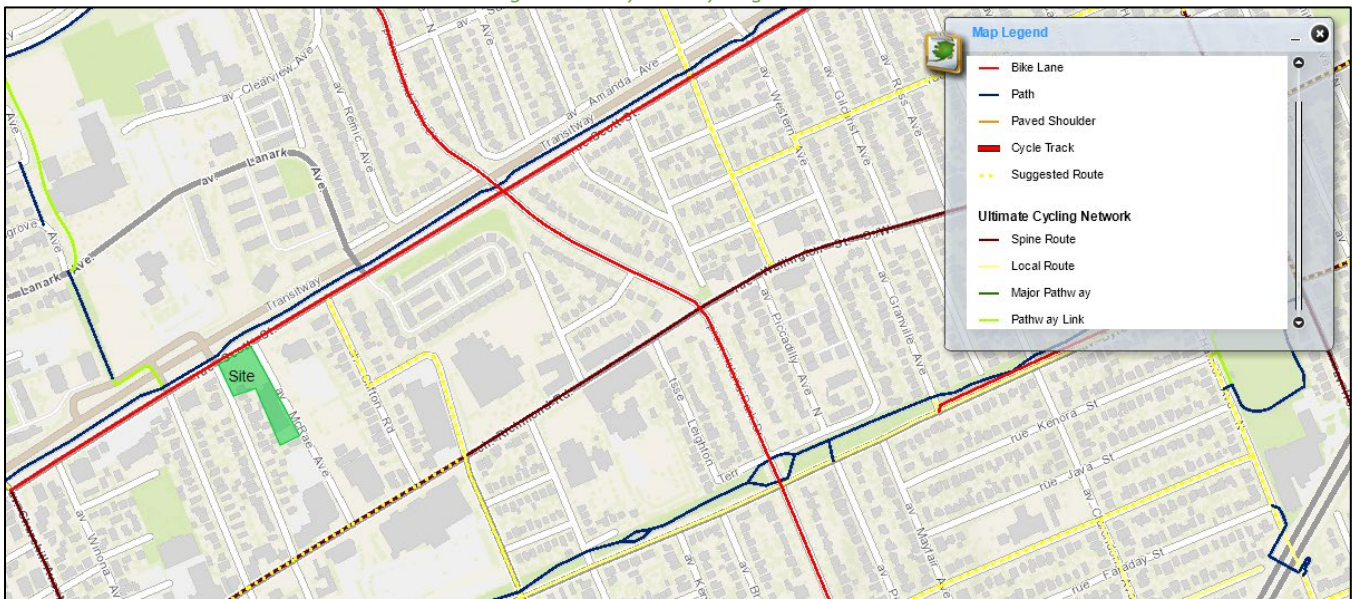


Figure 4: Study Area Cycling Facilities



### 2.2.5 Existing Transit

Within the Study Area, Route #11 has two stops at the intersection of McRae Avenue and Richmond Road. The stop on the northeast corner is also shared by Routes #81 and 153. Along McRae Avenue three stops are shared by Routes #81 and 153.

The frequencies of these routes within the proximity of the proposed site currently are:

- Route #11— every 15 minutes from AM to PM weekday peak hours and mid-day weekend peak hours, and 30 minutes in the off-peak times
- Route #50— every 15 minutes in the peak direction, and 30 minutes in the off-peak direction, off-peak times and Saturdays with no operation on Sundays

- Route #81— every 15-20 minutes in the peak direction, and 30 minutes in the off-peak direction, off-peak times and Saturdays with no operation on Sundays
- Route #153— every two hours from approximately 11AM to 6PM

Additionally, the Westboro Rapid Route station is located approximately 50 metres northwest of the development. This station is part of the Transitway.

Figure 5 illustrates the transit system map and summarizes the route information for Westboro Station. Figure 6 illustrates the transit stops in the Study Area.

Figure 5: Existing Study Area Transit Service

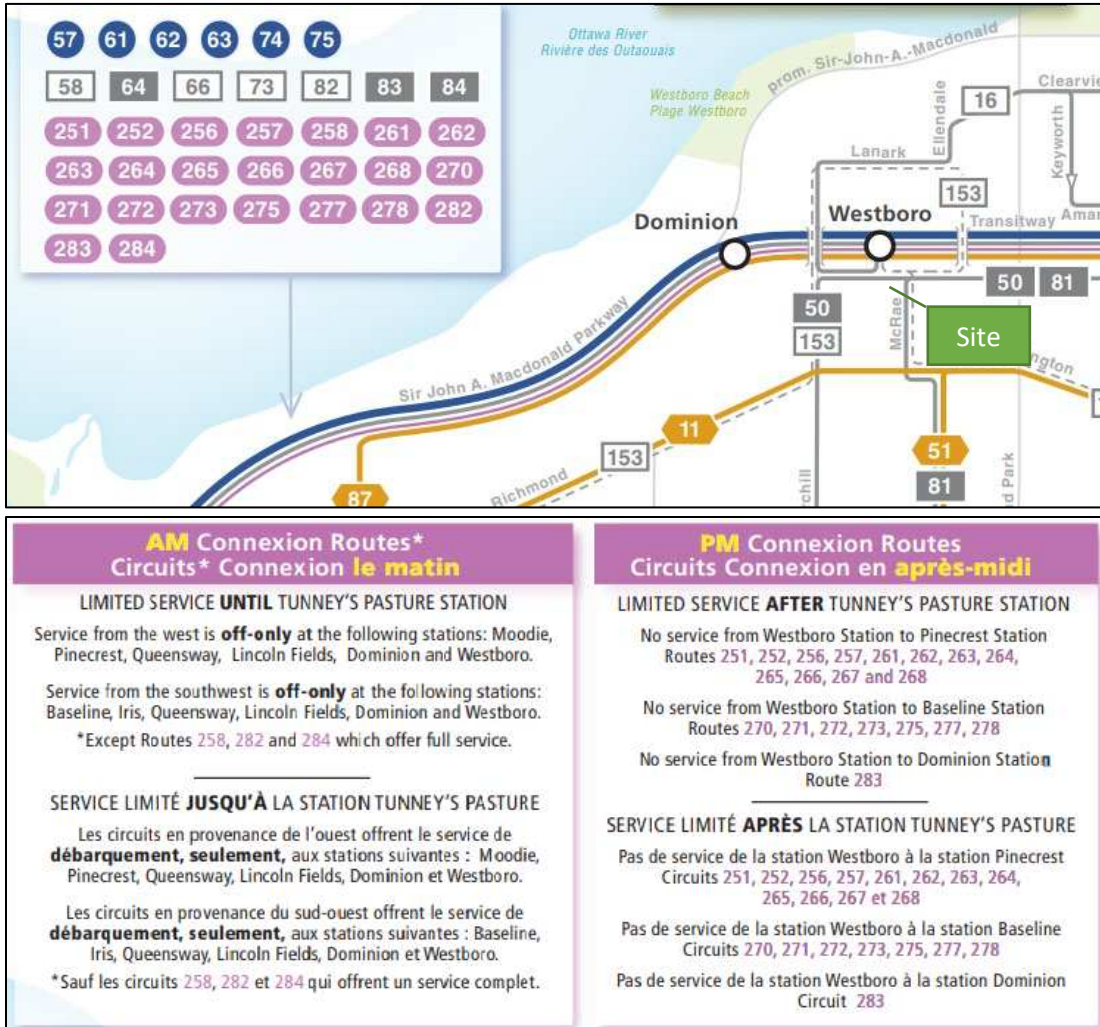
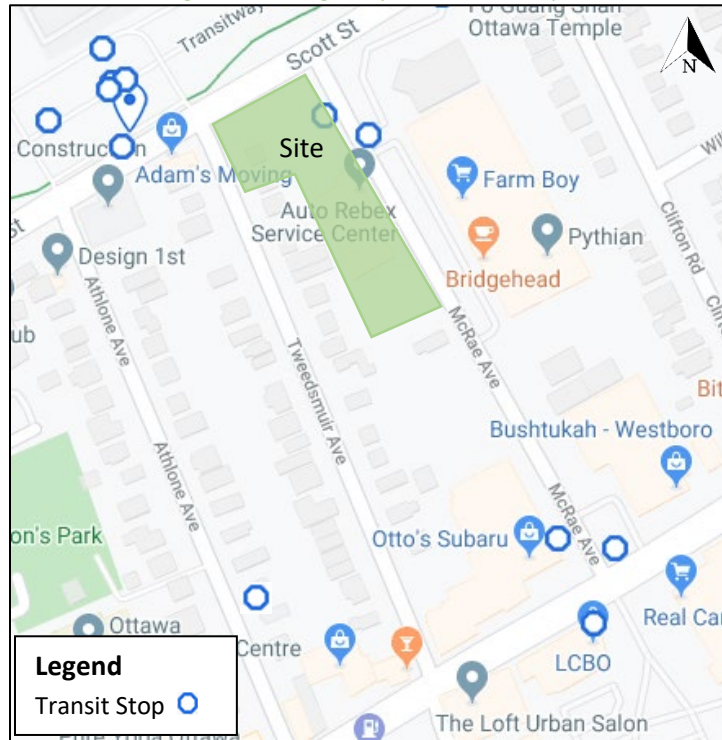


Figure 6: Existing Study Area Transit Stops



2.2.6 Existing Area Traffic Management Measures

Existing traffic management measures within the Study Area take the form of:

- Truck turning restrictions at Scott Street and Tweedsmuir Avenue – no EBR or WBL turns
- Turning restrictions (buses exempt) at Scott Street and Tweedsmuir – no EBL, NBT or WBR turns
- Truck turning restrictions at Richmond Road and Tweedsmuir Avenue – no EBL or WBR turns

2.2.7 Existing Peak Hour Travel Demand

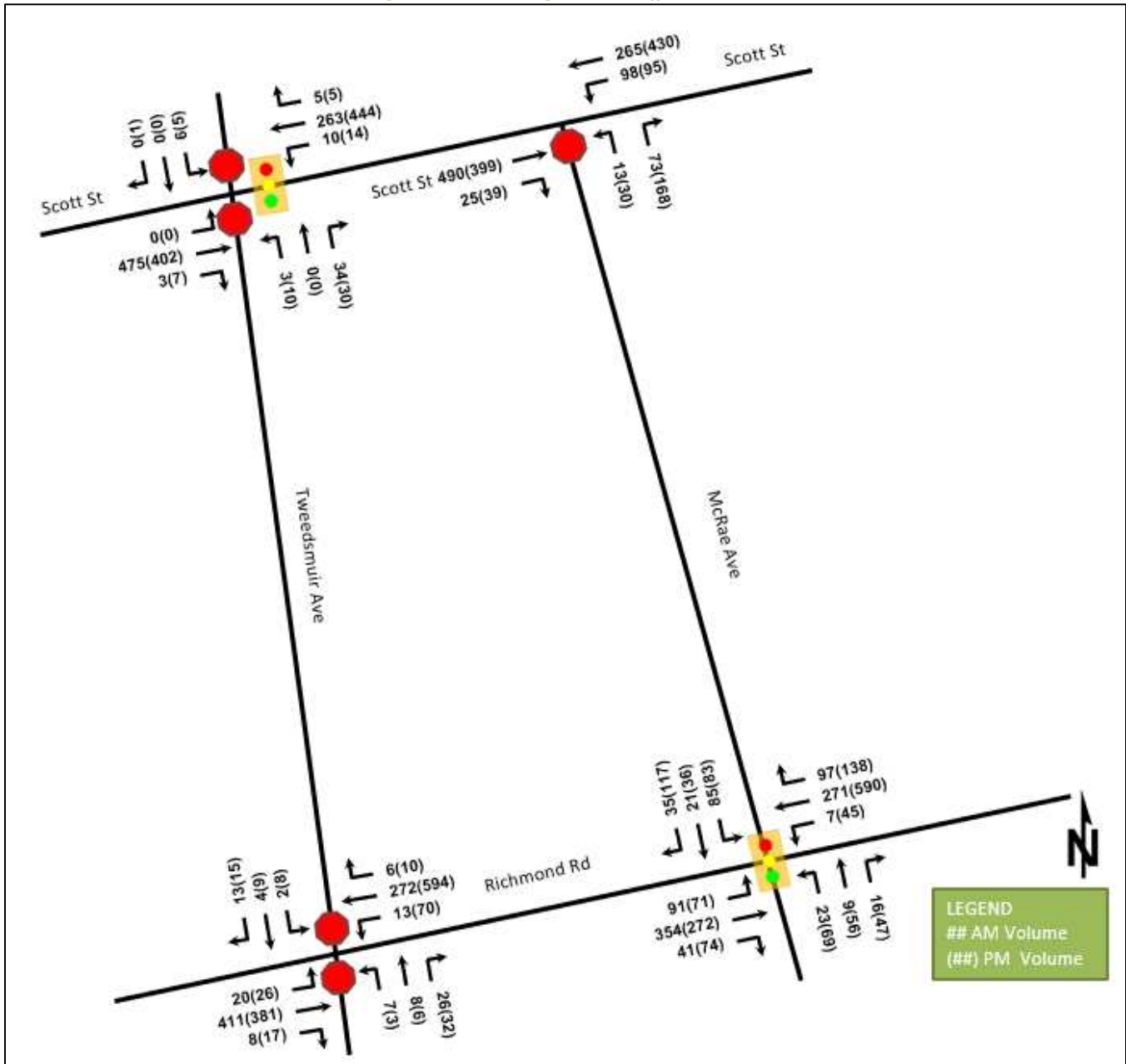
Existing turning movement counts were acquired from the City of Ottawa and Traffic Specialists for the existing Study Area intersections. Table 1 summarizes the intersection count dates and data sources.

Table 1: Intersection Count Date and Data Sources

Intersection	Count Date	Data Source
<b>Scott Street @ Tweedsmuir Avenue</b>	Thursday July 18, 2019	Traffic Specialists
<b>Scott Street @ McRae Avenue</b>	Thursday July 18, 2019	Traffic Specialists
<b>Richmond Road @ Tweedsmuir Avenue</b>	Thursday July 18, 2019	Traffic Specialists
<b>Richmond Road @ McRae Avenue</b>	Thursday April 20, 2017	City of Ottawa

Figure 7 illustrates the 2019 existing horizon traffic volumes. As shown above, the turning movement count data has been collected over different years. An adjacent area transportation study has used a 2% traffic growth within the Study Area of this report. As such, an annual background growth of 2% will be used in order to remain uniform with that study and produce a consistent horizon year. Detailed turning movement count data and signal timing plans are included in Appendix B.

Figure 7: 2019 Existing Horizon Traffic Volumes



### 2.2.8 Collision Analysis

Collision data has been acquired from the City of Ottawa for five years prior to the commencement of this TIA at each of the Study Area intersections. Specific attention is directed to the four primary intersections within the Study Area. Figure 8 illustrates the intersections and segments analyzed, and Table 2 summarizes the total collisions for the intersections of interest. Collision data is included in Appendix C.

Figure 8: Study Area Representation of Collision Locations



Table 2: Summary of Collision Locations

	Number	%
<b>Intersections / Segments</b>	<b>27</b>	<b>100%</b>
<b>Scott St @ Tweedsmuir Ave</b>	4	15%
<b>Scott St @ McRae Ave</b>	7	26%
<b>Richmond Rd @ Tweedsmuir Ave</b>	11	41%
<b>Richmond Rd @ McRae Ave</b>	5	18%

Overall, no fatal collisions were documented in the Study Area and a total of 4 collisions were noted involving pedestrians or cyclists. Three of these collisions involved pedestrians, all of which occurred at the intersection of McRae Avenue and Richmond Road. One collision involved a cyclist and it occurred at the intersection of Tweedsmuir Avenue and Richmond Road.

Table 3, Table 4, Table 5, and Table 6 summarize the collision types and conditions of the four intersections of interest within the Study Area on an individual basis.

Scott Street and Tweedsmuir Avenue experienced four collisions between 2013-2017. Three of those collisions resulted in property damage only, while one resulted in a non-fatal injury. Three collisions fall under the Rear End impact type and one falls under the Angle impact type. Weather/road conditions are considered a contributing factor for 25.00% of collisions at this intersection.

Table 3: Scott Street at Tweedsmuir Avenue Collision Summary

<b>Total Collisions</b>		Number	%
		<b>4</b>	<b>100.00%</b>
<b>Classification</b>	Fatality	0	0.00%
	Non-Fatal Injury	1	25.00%
	Property Damage Only	3	75.00%
<b>Initial Impact Type</b>	Angle	1	25.00%

	Rear end	3	75.00%
<b>Road Surface Condition</b>	Dry	3	75.00%
	Wet	1	25.00%
<b>Pedestrian Involved</b>		<b>0</b>	<b>0.00%</b>
<b>Cyclist Involved</b>		<b>0</b>	<b>0.00%</b>

Scott Street and McRae Avenue experienced seven collisions between 2013-2017. Six of those collisions resulted in property damage only, while one resulted in a non-fatal injury. Three collisions fall under the Rear End impact type and four fall under the Angle impact type. Weather/road conditions are considered a contributing factor for 14.29% of collisions at this intersection.

Table 4: Scott Street and McRae Collision Summary

<b>Total Collisions</b>		<b>Number</b>	<b>%</b>
		<b>7</b>	<b>100.00%</b>
<b>Classification</b>	Fatality	0	0.00%
	Non-Fatal Injury	1	14.29%
	Property Damage Only	6	85.71%
<b>Initial Impact Type</b>	Angle	4	42.86%
	Rear end	3	42.86%
<b>Road Surface Condition</b>	Dry	6	85.71%
	Wet	1	14.29%
<b>Pedestrian Involved</b>		<b>0</b>	<b>0.00%</b>
<b>Cyclist Involved</b>		<b>0</b>	<b>0.00%</b>

Richmond Road and McRae Avenue experienced 11 collisions between 2013-2017. Eight of those collisions resulted in property damage only, while three resulted in non-fatal injuries. The collision impact types vary between Angle, Rear End, Turning Movement, and SMV Other at 9.09%, 54.55%, 9.09%, and 27.27% of the 11 collisions respectively. Weather/road conditions are considered a contributing factor for 45.45% of collisions at this intersection.

Table 5: McRae Avenue and Richmond Road Collision Summary

<b>Total Collisions</b>		<b>Number</b>	<b>%</b>
		<b>11</b>	<b>100.00%</b>
<b>Classification</b>	Fatality	0	0.00%
	Non-Fatal Injury	3	27.27%
	Property Damage Only	8	72.73%
<b>Initial Impact Type</b>	Angle	1	9.09%
	Rear end	6	54.55%
	Turning Movement	1	9.09%
	SMV Other	3	27.27%
<b>Road Surface Condition</b>	Dry	6	54.55%
	Wet	4	36.36%
	Loose Snow	1	9.09%
<b>Pedestrian Involved</b>		<b>3</b>	<b>27.27%</b>
<b>Cyclist Involved</b>		<b>0</b>	<b>0.00%</b>

Richmond Road and Tweedsmuir Avenue experienced five collisions between 2013-2017. Three of those collisions resulted in property damage only, while two resulted in non-fatal injuries. The collision impact types vary between Angle, Turning Movement, SMV Unattended Vehicle, and SMV Other at 40.00%, 20.00%, 20.00%, and 20.00% of



the five collisions respectively. Weather/road conditions are considered a contributing factor for 60.00% of collisions at this intersection.

*Table 6: Tweedsmuir Avenue at Richmond Road Collision Summary*

Total Collisions		Number	%
		<b>5</b>	<b>100.00%</b>
Classification	Fatality	0	0.00%
	Non-Fatal Injury	2	40.00%
	Property Damage Only	3	60.00%
Initial Impact Type	Angle	2	40.00%
	Turning Movement	1	20.00%
	SMV unattended vehicle	1	20.00%
	SMV Other	1	20.00%
Road Surface Condition	Dry	2	40.00%
	Wet	2	40.00%
	Packed Snow	1	20.00%
Pedestrian Involved		<b>0</b>	<b>0.00%</b>
Cyclist Involved		<b>1</b>	<b>20.00%</b>

### 2.3 Planned Conditions

#### 2.3.1 Changes to the Area Transportation Network

The proposed development is subject to the Richmond Road / Westboro Secondary Plan as well as TOD principles, both of which promote a shift towards more sustainable modes of transportation in the area. These plans are expressed as elements of the Ottawa Official Plan, Ottawa Transportation Master Plan, Ottawa Pedestrian Plan and the Ottawa Cycling Plan. Measures to be implemented include:

- A pedestrian / cycling path along McRae Avenue within the Study Area (unspecified date)
- Pedestrian specific infrastructure improvements (unspecified date)
- Implementation of pedestrian specific safety programming and promotion (unspecified date)
- A cycling spine route along Richmond Road and Scott Street within the Study Area as part of the City of Ottawa Ultimate Cycling Plan

Additionally, as stated by the City of Ottawa, the Westboro LRT Station is expected to be completed by 2025.

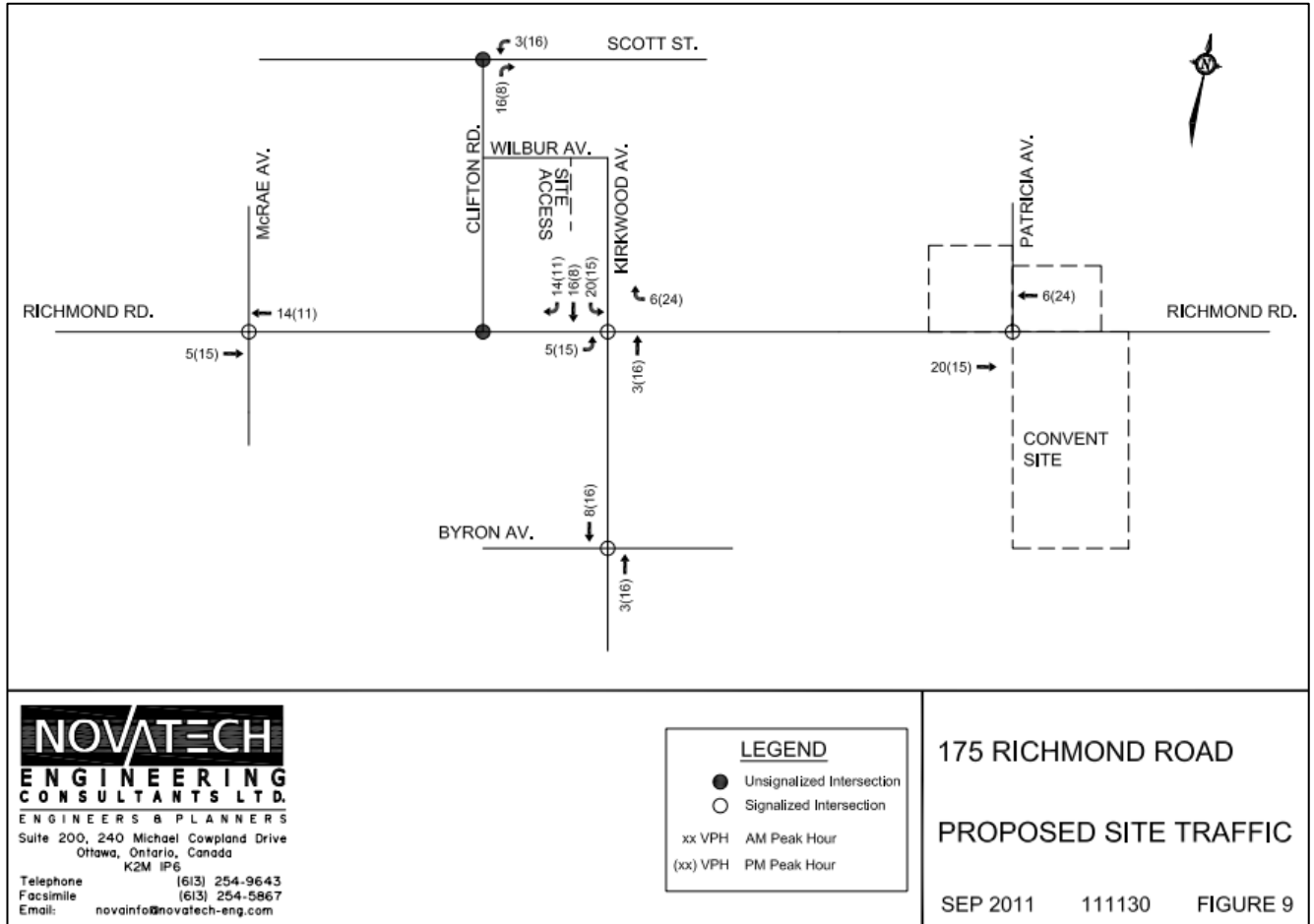
#### 2.3.2 Other Study Area Developments

At the time of this report, a few development applications were available for the adjacent properties as listed on the City’s Development Application Search tool:

- 403 Tweedsmuir Avenue – The City of Ottawa has received a Zoning By-law Amendment application to allow a six-storey mixed-use building for residential and hotel uses. A proposed underground parking garage will include 25 vehicle parking spaces. At this time, it is unclear as to the impact the trip generation from this development will have on the surrounding area
- 236 Richmond Road – The existing building will be demolished and replaced with a nine-storey mixed-use building with commercial use on the ground floor and the other floors housing approximately 70 units. At this time, it is unclear as to the impact the trip generation from this development will have on the surrounding areas
- 175 Richmond Road – The City of Ottawa has received a Zoning By-law Amendment and Site Plan Control application to allow a stepped nine-storey, six-storey, and four-storey mixed-use building. A total of 241

residential units and 675 square metres of retail commercial area are proposed. The anticipated trip generation can be seen in Figure 9 and is an excerpt from the 175 Richmond Road Transportation Brief prepared by Novatech Engineering Consultants Ltd.

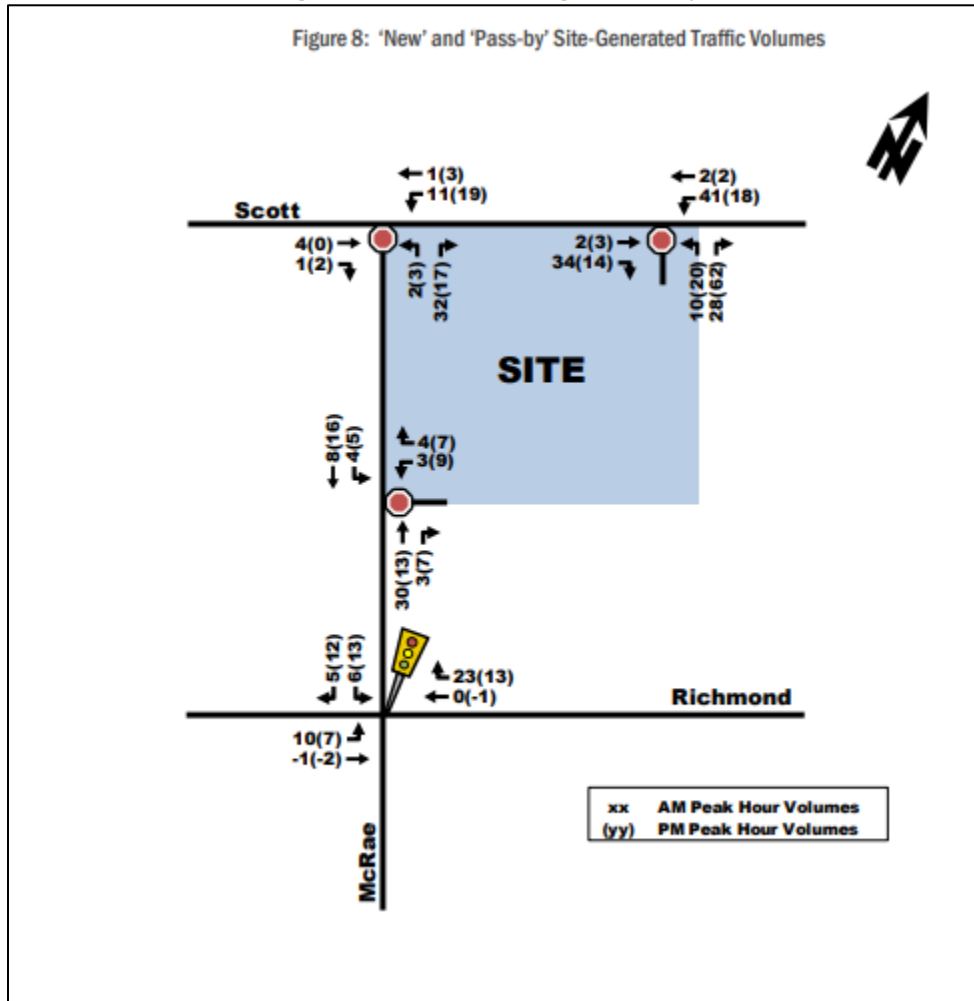
Figure 9: 175 Richmond Road Site Generated Traffic Volumes



Source: Residential Development 175 Richmond Road Transportation Brief-September 2011

- 341/343 Tweedsmuir Avenue – The City of Ottawa has received Zoning Bylaw Amendment and Site Plan Control applications to facilitate the establishment of a fourth unit in the basement of these three-unit dwellings. Trip generation is expected to have negligible impacts on the surrounding area.
- 1946 Scott Street – 12-storey residential building with 60 units and 13 above ground parking spaces. As the trip generation trigger is not met, no site generated trips are provided and so the impact of the trips generated from this development on the surrounding area is unknown
- 320 Bloomfield Avenue – The planned redevelopment of the City Works Yard includes changes to both the building and parking facilities. At this time, limited information is provided and so the impact of trips generated from this development on the surrounding area is unknown.
- 1960 Scott Street – 22-storey mixed-use development of 120 residential units, 6889 square metres of office space and 1341 square metres of retail space. An estimated 159 parking spaces and 100 bicycle spaces have been proposed. The anticipated trip generation can be seen in Figure 10 and is an excerpt from the 1960 Scott Street Transportation Brief prepared by Parsons

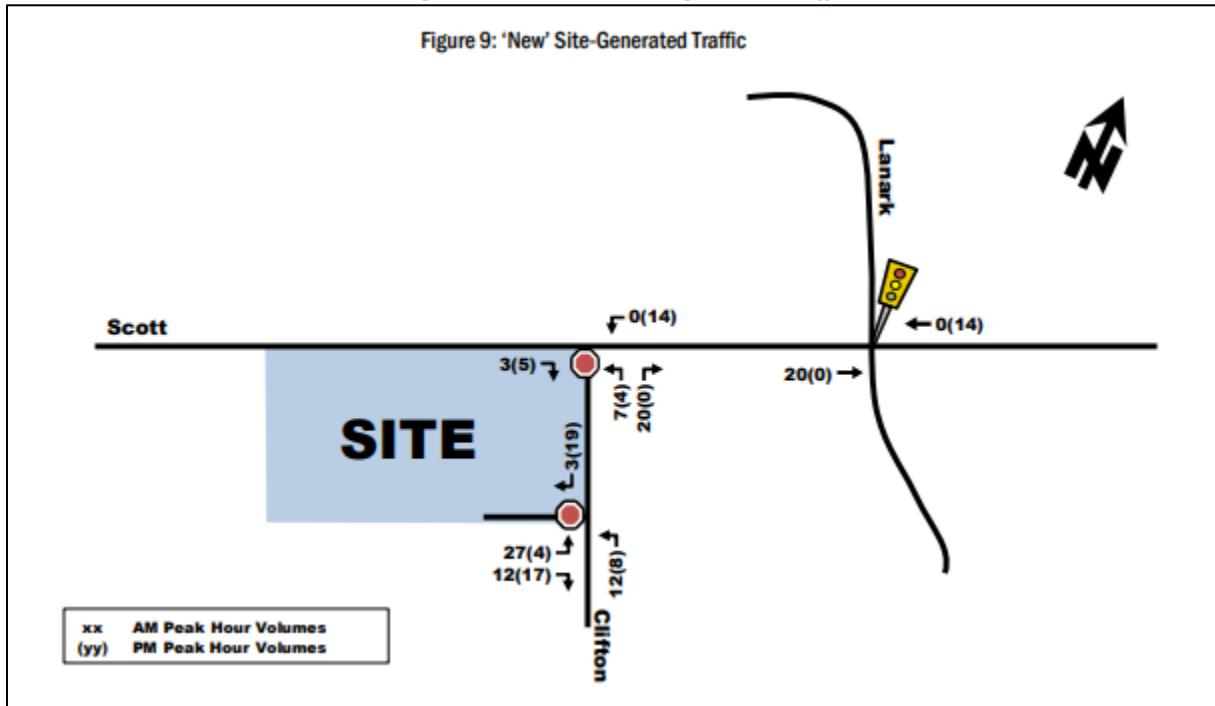
Figure 10: 1960 Scott St Site-generated Trips



Source: 1960 Scott Street-Transportation Brief-July 31, 2017

- 1950 Scott St – The City of Ottawa has received a Zoning By-law Amendment application to permit a 20-storey residential building with 141 units, 162 parking spaces and 10 visitor parking spaces. The anticipated trip generation can be seen in Figure 11 and is an excerpt from the 1950 Scott Street TIA Strategy Report prepared by Parsons.

Figure 11: 1950 Scott St Site-generated Traffic



Source: 1950 Scott Street-Transportation Impact Assessment Strategy Report-July 2018

### 3 Study Area and Time Periods

#### 3.1 Study Area

The study area will include the intersections of Scott Street and Tweedsmuir Avenue, Scott Street and McRae Avenue, Richmond Road and Tweedsmuir Avenue, and Richmond Road and McRae Avenue. Scott Street, Tweedsmuir Avenue, and McRae Avenue are noted as the boundary roads for the site.

As part of the review process, comments requesting additional intersections be considered as part of the Study Area were received from the City of Ottawa Transportation Project Manager. These intersections were Richmond Road at Churchill Avenue, Scott Street at Island Park Drive, and Richmond Road at Kirkwood Avenue. In response to this, a preliminary trip generation for the site using TOD mode shares was conducted at the time of these comments. While these findings are subject to refinement as the site plan is being finalized, it was found that the subject site would generate approximately 24 single direction trips at its peak for the ultimate future horizon within this study. By the time these trips distribute to the road network, the amount of traffic impacting the requested additional intersections would be negligible. As can be seen in the following sections, it is still estimated that the subject site will generate approximately 24 single direction trips, supporting the above conclusions. Therefore, the Study Area as defined above, is adequate to capture the auto impacts of the proposed development and no additional intersections are required to be analyzed.

#### 3.2 Time Periods

The AM and PM peak hours will be examined for the proposed development.

#### 3.3 Horizon Years

The anticipated build-out year is 2022. As a result, the full build-out plus five years horizon year is 2027.

## 4 Exemption Review

Table 7 summarizes the exemptions for this TIA.

*Table 7: Exemption Review*

Module	Element	Explanation	Exempt/Required
<b>Design Review Component</b>			
<b>4.1 Development Design</b>	4.1.2 Circulation and Access	Only required for site plans	Required
	4.2.3 New Street Networks	Only required for plans of subdivision	Exempt
<b>4.2 Parking</b>	4.2.1 Parking Supply	Only required for site plans	Required
	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
<b>Network Impact Component</b>			
<b>4.5 Transportation Demand Management</b>	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required (for residential portion only)
<b>4.6 Neighbourhood Traffic Management</b>	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Required
<b>4.8 Network Concept</b>		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of equivalent volume permitted by established zoning	Exempt

## 5 Development-Generated Travel Demand

### 5.1 Trip Generation and Mode Shares

This TIA has been prepared using the vehicle and person trip rates for the residential components using the TRANS Trip Generation Study Report (2009) and the vehicle trip rates for the retail components, a factor of 1.28 has been applied to the ITE rates. Table 8 summarizes the person trip rates for the proposed land uses.

*Table 8: Trip Generation Person Trip Rates*

Dwelling Type	Land Use Code	Peak Hour	Vehicle Trip Rate	Person Trip Rates
<b>Townhouses</b>	224 (TRANS)	AM	0.51	1.13
		PM	0.51	0.96
<b>Mid-rise Apartments</b>	223 (TRANS)	AM	0.24	0.65
		PM	0.28	0.70
<b>High-rise Apartments</b>	222 (TRANS)	AM	0.24	0.65
		PM	0.27	0.68
<b>Shopping Centre</b>	820	AM	0.94	1.20
		PM	3.81	4.88

Using the above Person Trip rates, the total person trip generation has been estimates. Table 9 below illustrates the total person trip generation by dwelling type.

Table 9: Total Person Trip Generation

Land Use	Units / GFA	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
Townhouses	11 units	4	8	12	6	5	11
Mid-Rise Apartments	46 units	7	23	30	20	12	32
High-Rise Apartments	261 units	41	129	170	110	67	177
Shopping Centre	9,494 sq.ft	6	3	9	20	17	37
<b>Total Person Trips</b>		<b>58</b>	<b>163</b>	<b>221</b>	<b>156</b>	<b>101</b>	<b>257</b>

Using the most recent National Capital Region Origin-Destination (OD Survey), the existing mode shares for Ottawa West as well as TOD mode shares, have been summarized in Table 10. The mode shares for Ottawa West will be used to develop interim trip generation for the 2022 future horizon as per the request of the City of Ottawa. This mode share is considered to be very conservative in nature and will produce a “worst case scenario” given the proposed development is within 400 metres of the existing Westboro Transitway Station. The new Westboro LRT Station is projected to be constructed and operational by 2025. As such, a TOD mode share will be used for the 2027 future horizon.

Table 10: Mode Share

Travel Mode	Ottawa West	TOD Mode Share
Auto Driver	50%	15%
Auto Passenger	15%	5%
Transit	20%	65%
Cycling	5%	5%
Walking	10%	10%
<b>Total</b>	<b>100%</b>	<b>100%</b>

Internal capture rates from the ITE Trip Generation Handbook 3<sup>rd</sup> Edition have been assigned to the development for the retail components for mixed-use developments. The retail portion of this development is the smaller of the two land uses. Therefore, the residential land use is treated as the anchor for this development and is not reduced based on the multi-use capture rate. The smaller portion of the development, the retail portion, has been reduced to reflect residents of the site utilizing the on-site retail instead of leaving the site and/or as a pass-by trip on the way to an ultimate destination (ie. work). The rates summarized in Table 11 represent the percentage of trips to/from the retail uses based on the residential component.

Table 11: Internal Capture Rates

Land Use	AM		PM	
	In	Out	In	Out
<b>Residential to/from Shopping Centre</b>	17%	14%	10%	26%

Using the above mode shares and person trip rates, the person trips by mode have been projected for the 2022 future horizon using the Ottawa West mode shares and for the 2027 future horizon using the TOD mode shares. Table 12 summarizes the trip generation by mode for the 2022 horizon and Table 13 summarizes the trip generation by mode for the 2027 horizon.

Table 12: 2022 Trip Generation by Mode

Travel Mode	Mode Share	In	Out	Total	In	Out	Total
Auto Driver	50%	29	83	112	77	50	127
Auto Passenger	25%	9	23	32	24	16	40
Transit	20%	11	34	45	31	19	50
Cycling	5%	3	8	11	8	5	13
Walking	10%	6	15	21	16	11	27
Internal Capture	(varies)	-1	-1	-2	-2	-6	-8
<b>Total</b>	<b>100%</b>	<b>58</b>	<b>163</b>	<b>221</b>	<b>156</b>	<b>101</b>	<b>257</b>

As shown above, 112 AM and 127 PM new peak hour two-way vehicle trips are projected as a result of the proposed development in the 2022 future horizon.

Table 13: 2027 Trip Generation by Mode

Travel Mode	Mode Share	In	Out	Total	In	Out	Total
Auto Driver	15%	9	24	33	24	16	40
Auto Passenger	5%	2	7	9	8	5	13
Transit	65%	38	108	146	100	64	164
Cycling	5%	3	8	11	8	5	13
Walking	10%	6	16	22	16	11	27
Internal Capture	(varies)	-1	-1	-2	-2	-6	-8
<b>Total</b>	<b>100%</b>	<b>58</b>	<b>163</b>	<b>221</b>	<b>156</b>	<b>101</b>	<b>257</b>

As shown above, 33 AM and 40 PM new peak hour two-way vehicle trips are projected as a result of the proposed development in the 2027 future horizon.

### 5.2 Trip Distribution

To understand the travel patterns of the subject development, the OD survey has been reviewed to determine the existing travel patterns that will be applied to the new vehicle trips. Table 14 below summarizes the distribution for Ottawa West.

Table 14: OD Survey Existing Mode Share - Ottawa West

To/From	% of Trips
North	15%
South	35%
East	35%
West	15%
<b>Total</b>	<b>100%</b>

### 5.3 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the Study Area road network. No trips have been assigned to the loading access on Tweedsmuir Avenue as this access will be used only during off-peak hours. Figure 12 illustrates the new site traffic assignment by percentage. Figure 13 and Figure 14 illustrate the new site generated volumes for the 2022 future horizon and the 2027 future horizons respectively.

Figure 12: New Site Generation Assignment (%)

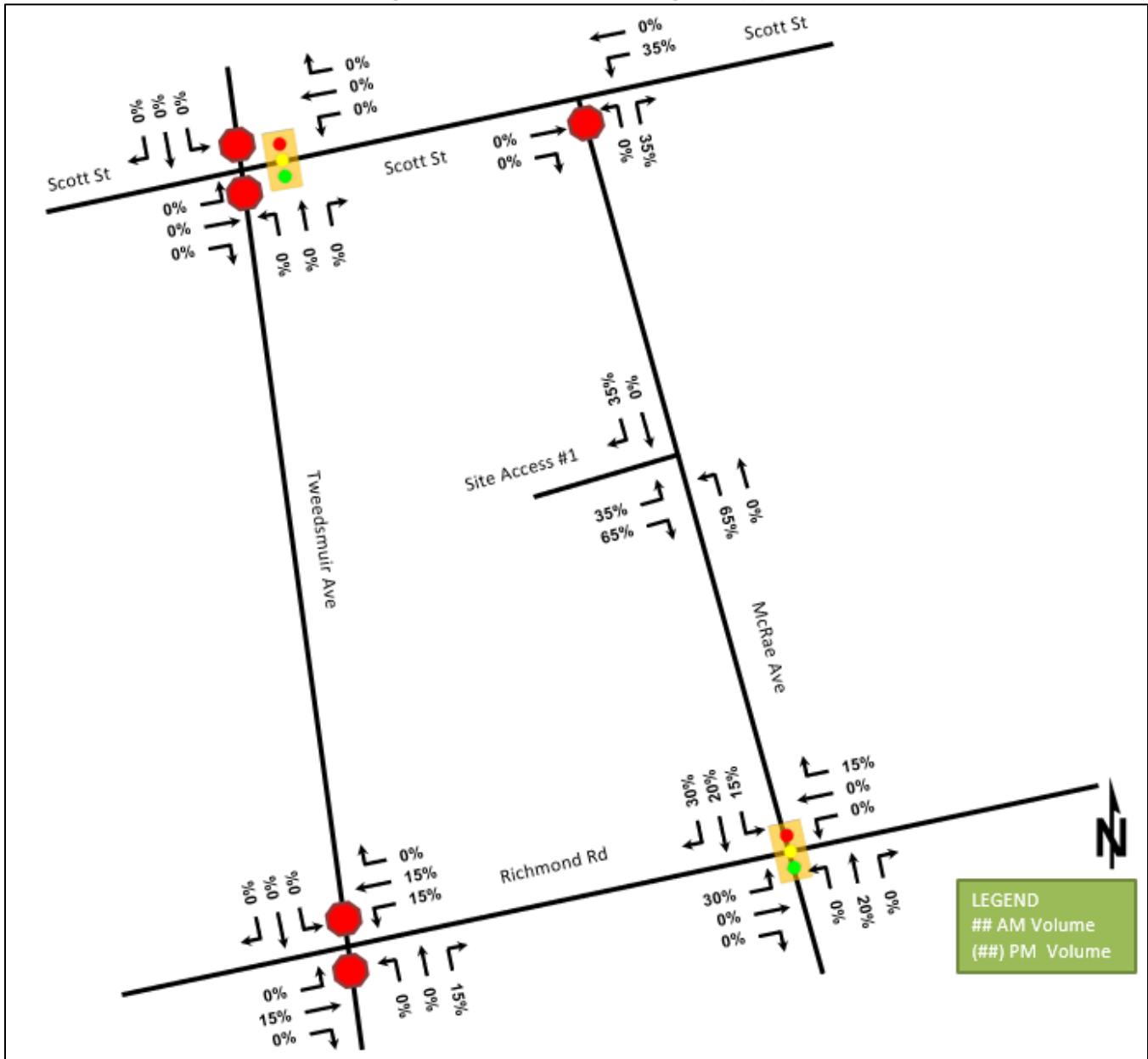




Figure 13: New 2022 Site Generation Auto Volumes

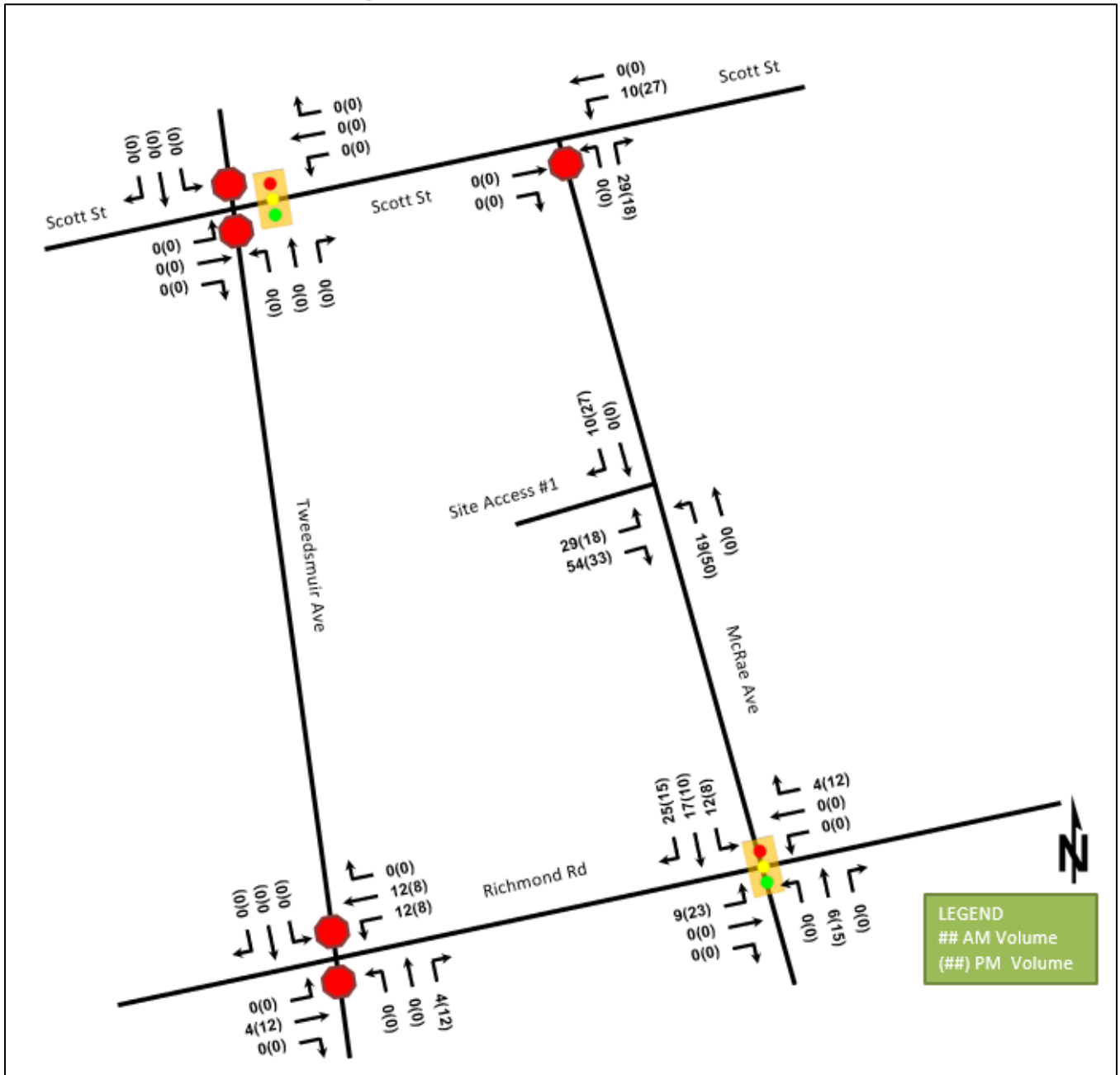
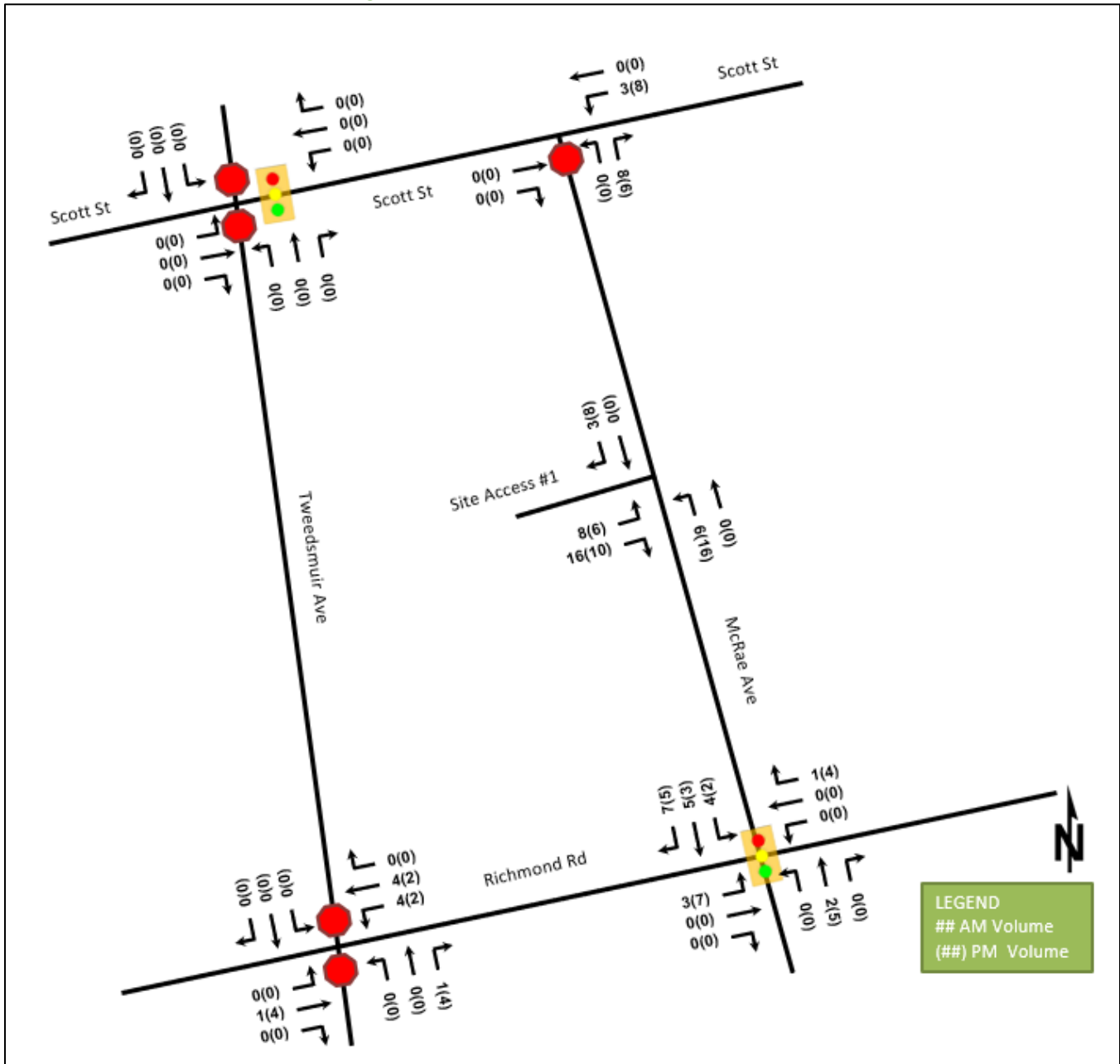


Figure 14: New 2027 Site Generation Auto Volumes



## 6 Background Network Travel Demands

### 6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3.1. The opening of the Westboro LRT station and Dominion LRT station, isolated measure transit priority along Richmond Road and TOD policies have been accounted for within the modal share assumptions. No road improvements within the study horizons are noted for this area.

The additional connectivity provided by future bicycle spine routes along Scott Street and Richmond Road as part of the City of Ottawa ultimate cycling plan will improve the active mode network.

### 6.2 Background Growth and Other Developments

As stated in Section 2.2.7, an adjacent area transportation study has used a 2% traffic growth within the Study Area of this report. As such, an annual background growth of 2% has been applied to remain consistent with that study and produce a conservative estimate of growth.

The background developments explicitly considered in the background conditions include 175 Richmond, 1960 Scott Street, and 1950 Scott Street. These developments are discussed in Section 2.3.2.

Figure 15 illustrates the 2022 background volumes and Figure 16 illustrates the 2027 background volumes.

Figure 15: Future Background 2022 Volumes

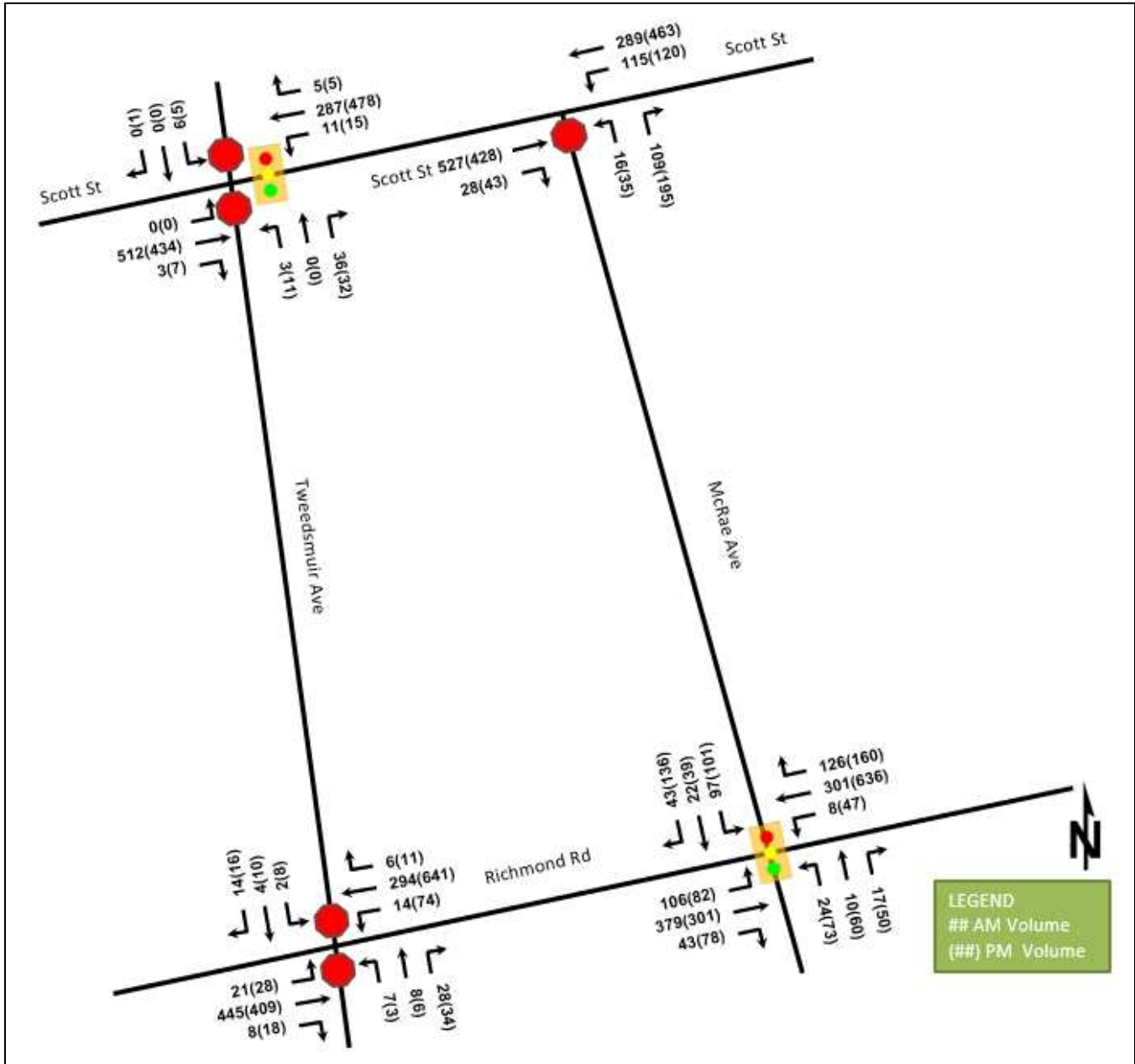
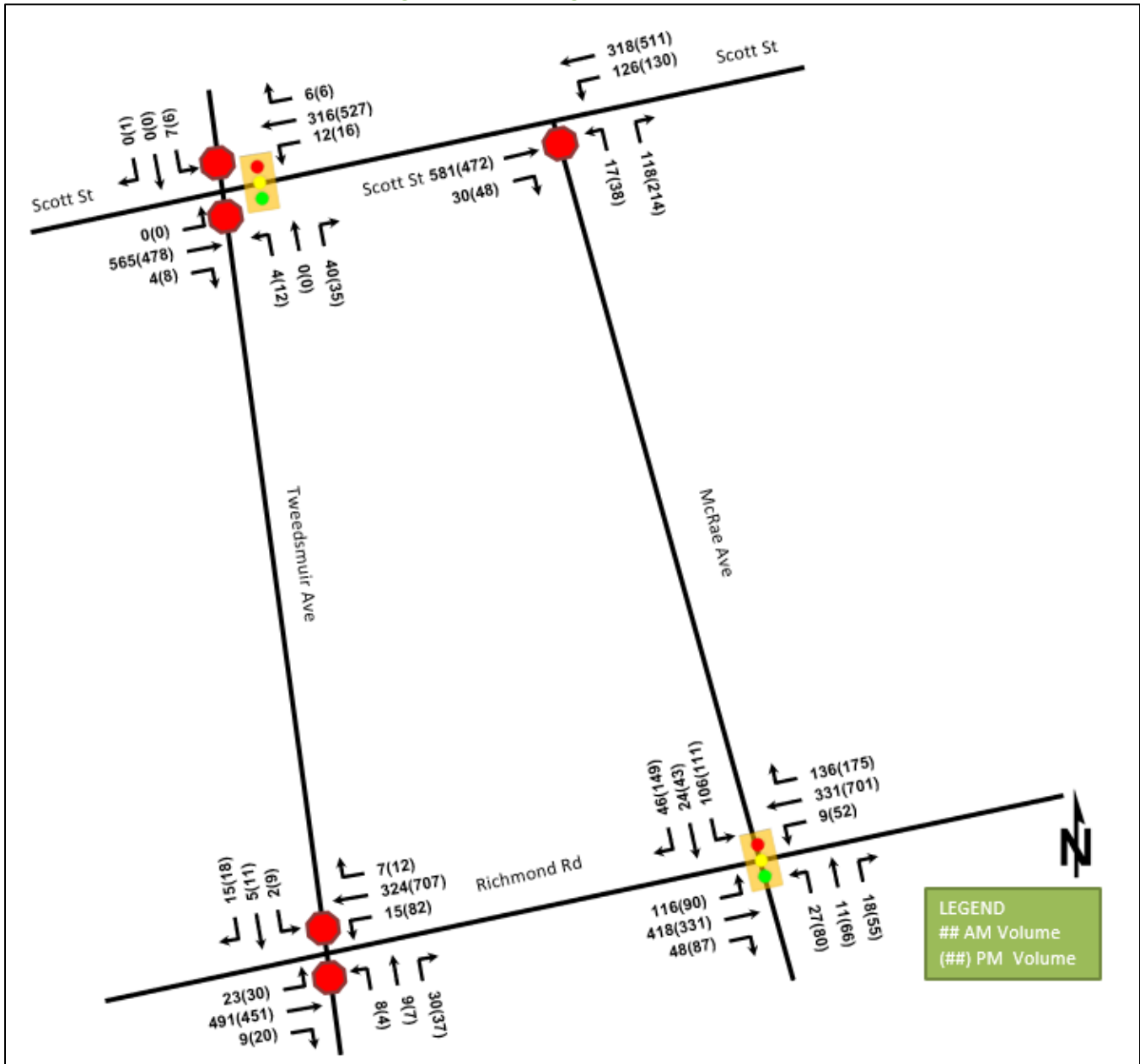


Figure 16: Future Background 2027 Volumes



## 7 Demand Rationalization

Changes in traffic volumes between existing and future conditions will come from the applied 2% background growth rate, 175 Richmond, 1960 Scott Street, 1950 Scott Street and the proposed development within this report. Additionally, the trip generation of this development will change due to a shift from the Ottawa West mode share used in 2022 to the TOD modal shares in 2027, as can be seen in Section 5, to conservatively account for the Westboro LRT. The future total 2022 and 2027 volumes are illustrated in Figure 17 and Figure 18 respectively.

Figure 17: Future Total 2022 Volumes

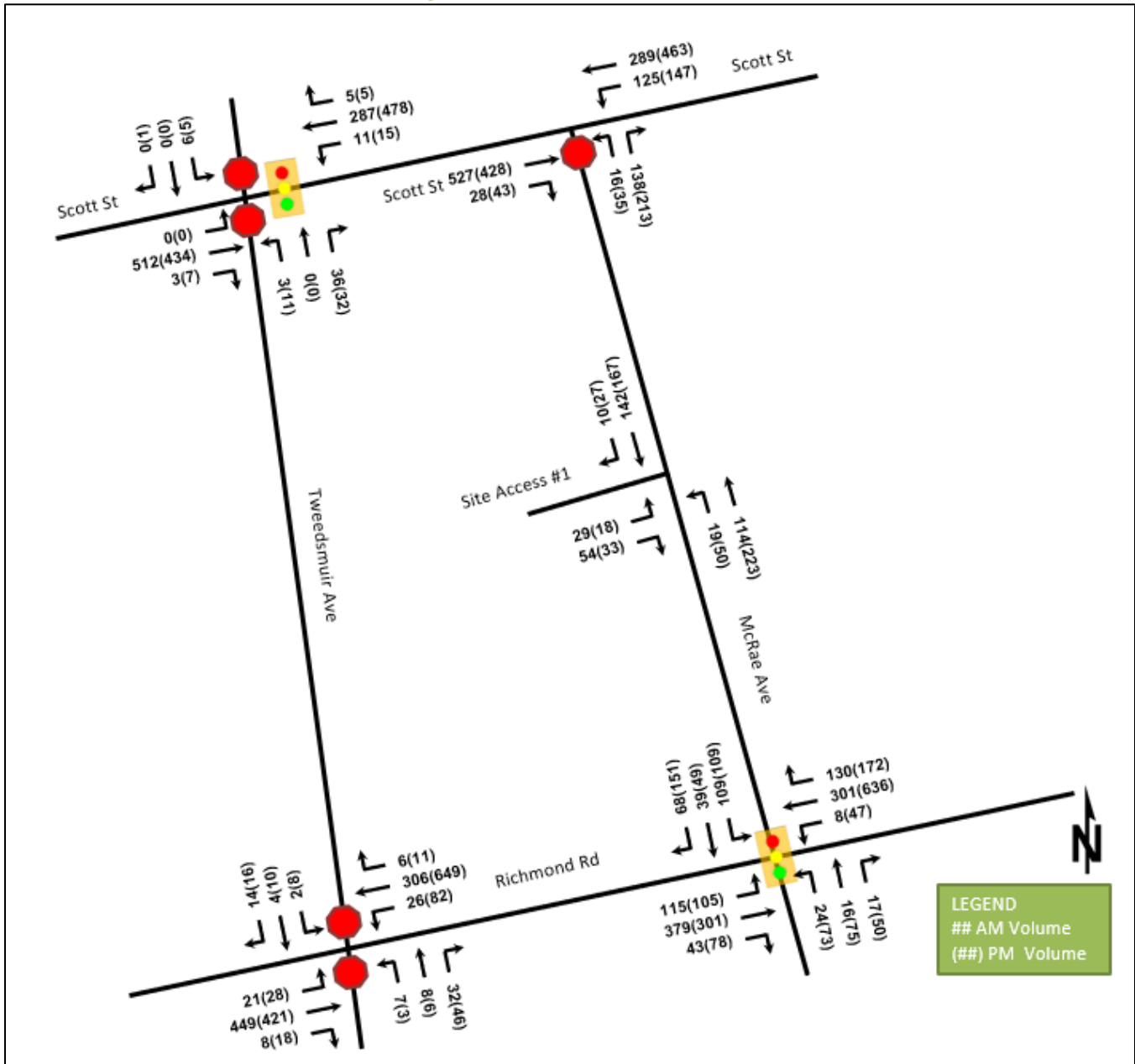
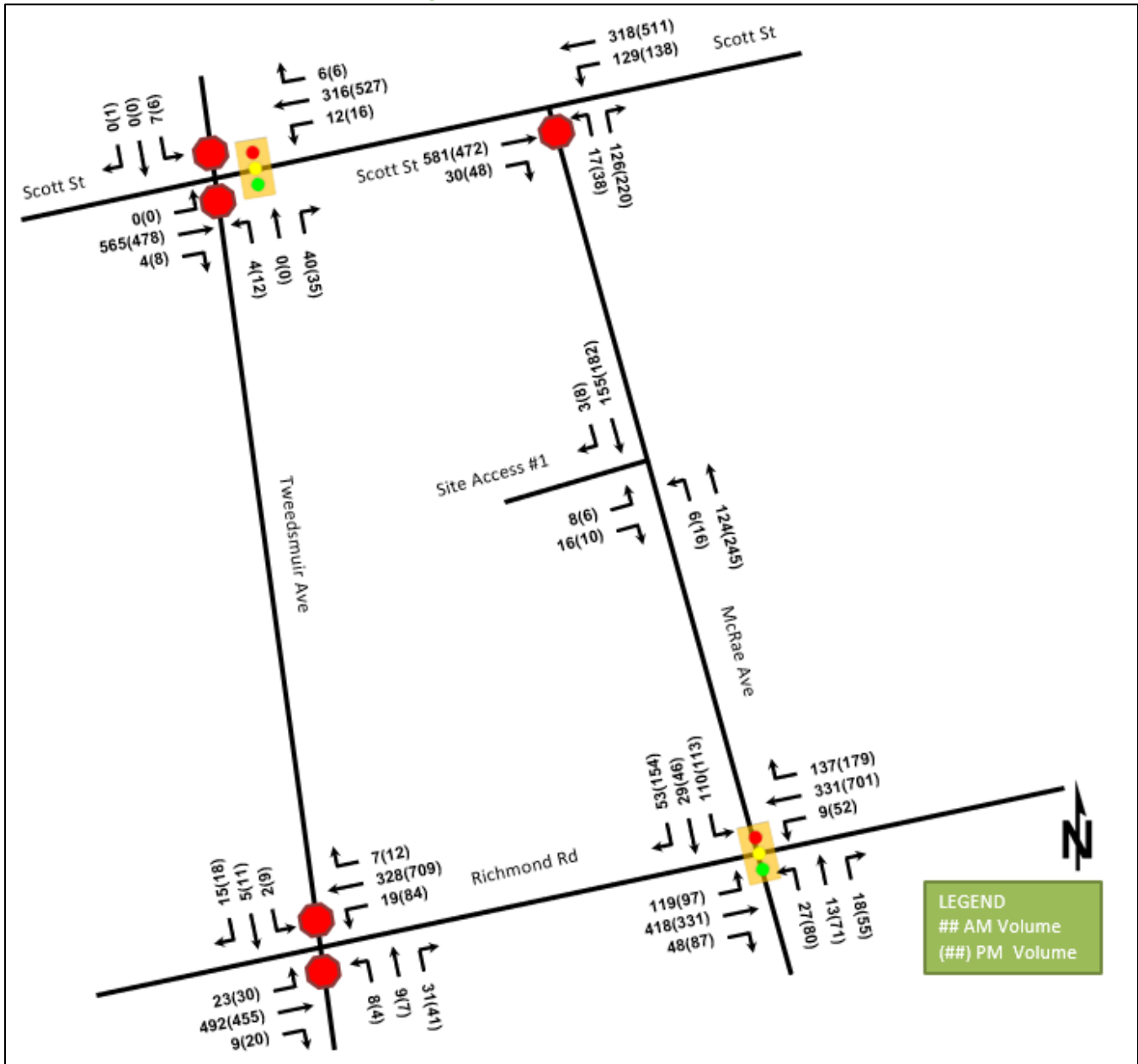


Figure 18: Future Total 2027 Volumes



Given the Study Area is subject to TOD policies, an expected shift in mode share is anticipated following the implementation of future TOD policies as well as the building of both the Dominion and Westboro LRT Stations. This shift is anticipated to result in an auto driver mode reduction and an increase in transit and non-auto mode shares.

## 8 Development Design

### 8.1 Design for Sustainable Modes

The proposed development is a commercial/residential development divided into two buildings with underground automobile and bicycle parking.

The proposed development is bordered by existing pedestrian and cyclist facilities along Scott Street, existing pedestrian facilities along Tweedsmuir Avenue and McRae Avenue, and planned future cyclist facilities along McRae Avenue. The existing Westboro station is within 400 metres from the development and can be accessed by these facilities. As such, the future Westboro LRT Station will be within 400 metres of the proposed development and will be able to be accessed by these facilities as well.

Additionally, facilities that are supportive of sustainable modes in the City of Ottawa's TDM-supportive Development Design and Infrastructure Checklist, which are required for zoning and standard site design, are recommended. The following additional measures are also recommended:

- Locate building close to the street, and do not locate parking areas between the street and building entrances
- Locate building entrances in order to minimize walking distances to sidewalks and transit facilities
- Locate building doors and entrances to ensure visibility of pedestrians from the building
- Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails
- Provide a permanent bike repair station
- Provide a designated area for carpool drivers to drop off or pick up passengers without using fire lanes or other no-stopping zones
- Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking
- Provide separate area for short-term and long-term parking to permit access controls and simplify enforcement

TDM Checklists can be found in Appendix D.

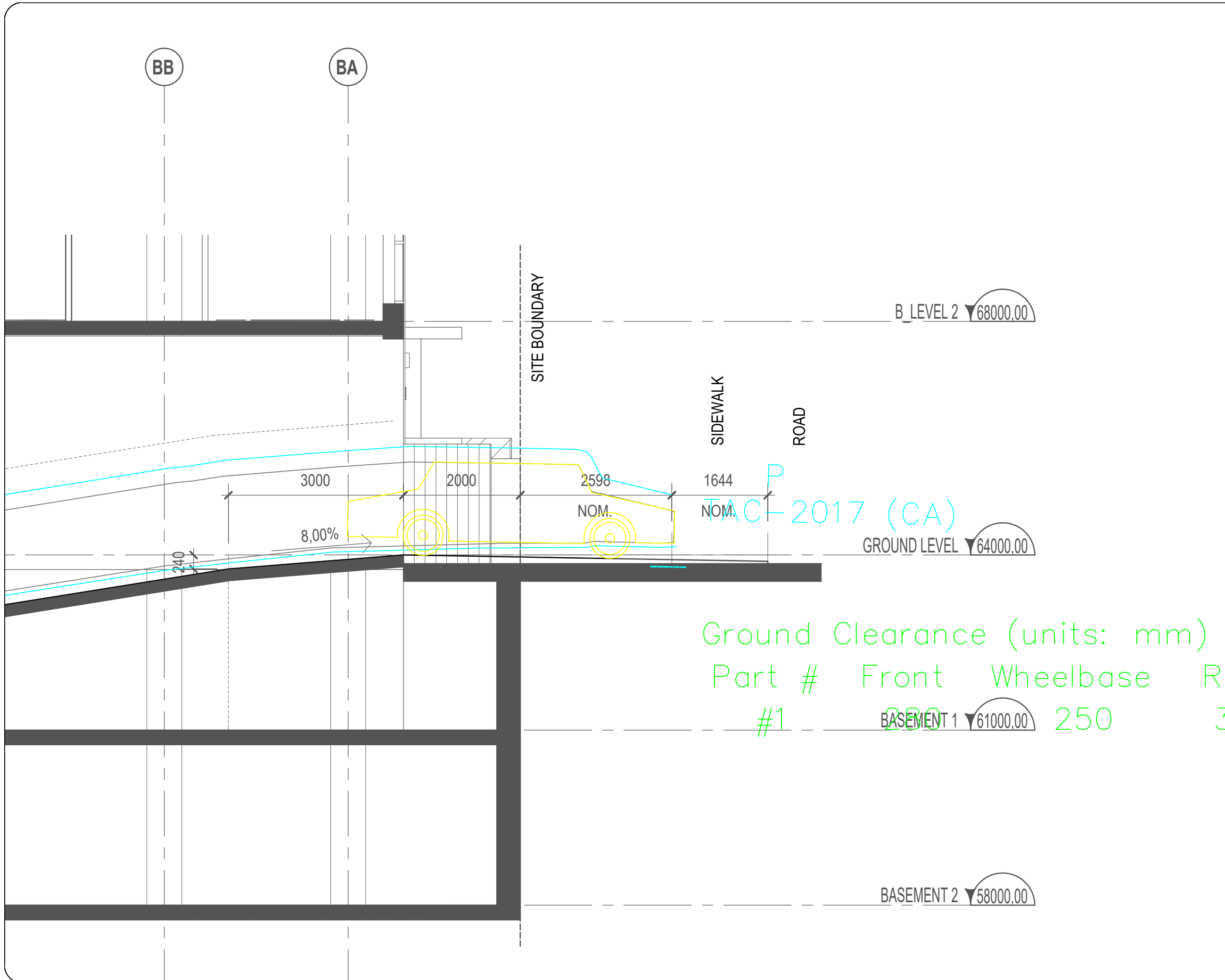
## 8.2 Circulation and Access

The primary Access #1 on McRae Avenue will accommodate passenger vehicles accessing the underground automobile parking. Access #2 is considered the primary entrance/exit to the loading area and will accommodate minimal truck volumes primarily during off-peak hours. Access #2 is also expected to be used by garbage trucks to access the development.

While the horizontal geometry of each access point is relatively straightforward, and meets the private approach by-law, the vertical geometry of the underground parking lot does not. Therefore, a review of the vertical geometry has been examined. As shown in Figure 19, a vehicle outbound on the proposed ramp would be able to exit the building and see the sidewalk and the road, without blocking the sidewalk or the road.

Additionally, turning templates have been developed for passenger cars on both parking levels as well as garbage trucks at Access #2. Garbage trucks will pull into the loading area where members of staff will bring out garbage and act as spotters to help the garbage truck back-up onto Tweedsmuir Avenue. Turning templates can be found in Appendix E.

Figure 19: Underground Parking Vertical Geometry



Notes:

A	description	by	xx/xx/xx
REV:	DESCRIPTION:	BY:	DATE:
STATUS:	status		

**CGH Transportation**  
 13 Markham Ave  
 Ottawa, ON  
 K2G 3Z1  
 (343) 999-9117

**CLIENT:** GWL Realty Advisors  
 33 Yonge Street, Suite 1000  
 Toronto Ontario  
 M5E 1G4

**ARCHITECT:** NEUF Architects  
 47 Clarence Street, Suite 406  
 Ottawa, ON  
 K1N 9K1

**SITE:** 320 McRae

**TITLE:** Access Review  
 Vertical Geometry

<b>SCALE AT A3:</b> NTS	<b>DATE:</b> 2020-01-28	<b>DRAWN:</b> MC	<b>CHECKED:</b>
<b>PROJECT NO:</b> 2019-29	<b>DRAWING NO:</b> 001	<b>REVISION:</b>	



8.3 New Street Networks

This TIA is exempt from this Module (See Table 7).

9 Parking

9.1 Parking Supply

The parking requirements and provisions for the proposed development are summarized in Table 15.

Table 15: Parking Provisions

Land Use	Parking Rate	Parking Required	Parking Provided
Dwelling units in same building as non-residential	N/A	0	185
Retail Store	N/A	0	
Dwelling units in same building as non-residential (visitor)	0.1 spaces/dwelling unit	30	163
Dwelling units in same building as non-residential (bicycle)	0.5 spaces/dwelling unit	159	
Retail Store (bicycle)	1 space/250m <sup>2</sup> GFA	4	

The parking supply has been evaluated for the ultimate horizon conditions which assumes that the Westboro LRT Station has been completed and the surrounding area is operating as a TOD area. As such, no minimum automobile parking space requirements, with the exception of residential visitor parking, are considered for the proposed development. It is noted that of the 163 bicycle parking spaces, 123 will be underground, and due to space restrictions, 25 will be in the loading area and 15 bicycle parking spaces will be slightly off the property along McRae Avenue and Scott Street. The required parking space provisions for both the automobile visitor parking and bicycle parking have been met.

9.2 Spillover Parking

This TIA is exempt from this Module (See Table 7).

10 Boundary Street Design

For the purposes of this TIA, Scott Street, Tweedsmuir Avenue and McRae Avenue are considered boundary streets for the existing, future 2022 and future 2027 horizons. All three boundary streets are not currently Complete Streets and no plans exist to upgrade them to Complete Streets. Segment MMLOS is broken down into the Pedestrian Level of Service (PLOS), Bicycle Level of Service (BLOS), Transit Level of Service (TLOS) and Truck Level of Service (TkLOS). The segment MMLOS worksheets have been provided in Appendix F.

Road Segment	Horizon	MMLOS							
		PLOS		BLOS		TLOS		TkLOS	
		Actual	Target	Actual	Target	Actual	Target	Actual	Target
Tweedsmuir Avenue	Existing	C	A	D	B	N/A	N/A	N/A	N/A
	2022								
	2027								
Scott Street	Existing	D	A	D	D	E	D	B	D
	2022								
	2027								

McRae Avenue	Existing	F	A	D	D	D	D	F	N/A
	2022								
	2027								

As the Study Area is considered to be a TOD area for all horizons and no changes to the boundary road segments are planned within the study horizons, the existing and future horizons are the same.

The road segment of Tweedsmuir Avenue will not meet its pedestrian LOS target due to small sidewalk and boulevard widths and will not meet its bicycle LOS targets due to mixed traffic conditions. As Tweedsmuir Avenue is not a truck route and is not part of a transit route, no truck or transit LOS could be determined.

The road segment of Scott Street will not meet the pedestrian LOS target due to small sidewalk and boulevard widths and a high average daily curb lane traffic volume. The transit LOS was not met due to mixed traffic conditions. Scott Street meets both the bicycle and truck LOS targets.

The road segment of McRae Avenue will not meet its pedestrian LOS target due to small sidewalk and boulevard widths. It meets the bicycle and truck LOS targets. As McRae Avenue is a local road, no truck LOS target is available.

Future network plans mentioned in both Section 2.3.1 and Section 6.1 are anticipated to increase connectivity and improve the active mode network within the Study Area. These changes have the potential to improve the MMLOS of the boundary streets.

## 11 Access Intersections Design

### 11.1 Location and Design of Access

Two unsignalized full-movement site accesses are planned for the proposed development. Site Access #1 is approximately 40 metres south of Scott Street on Tweedsmuir Avenue and Site Access #2 is approximately 120 metres south of Scott Street on McRae Avenue. Site Access #2 is a loading access and is intended for truck use only.

### 11.2 Intersection Control

Based on the projected volumes, the two site accesses will have stop-control on the minor approach for both future total horizons. No further traffic control is warranted to address operational issues. Signalization warrants for Site Access #1 can be found in Appendix G.

### 11.3 Intersection Design

Left-turn lane warrants for unsignalized intersections were examined at Site Access #1 for both 2022 and 2027 total future horizons. To determine if a left-turn lane is warranted, the MTO Geometric Design Standards for Ontario Highways, Section E. A left-turn lane was not found to be warranted. Left-turn lane warrant nomographs can be found in Appendix H.

## 12 Transportation Demand Management

Transportation Demand Management measures are implemented to encourage the use of non-auto modes of travel. This is aimed at reducing the reliance on single occupant auto trips in the City of Ottawa. The proposed development adheres to the City’s TDM principles by facilitating connections to adjacent pedestrian, cycling and

transit facilities. As the proposed development will be in a designated Transit-oriented Development (TOD) zone a TOD mode share has been used for the 2027 horizon.

The following measures, consistent with the TDM Checklist included in Appendix D, could be implemented to ensure that the travel mode shares meet the TOD targets.

- Designate an internal coordinator, or contract with an external coordinator.
- Display local area maps with walking/cycling access routes and key destinations at major entrances.
- Display relevant transit schedules and route maps at entrances.
- Contract with provider to install on-site carshare vehicles and promote their use by residents.
- Unbundle parking cost from monthly rent.

### 13 Neighbourhood Traffic Management

#### 13.1 McRae Avenue

Table 16 summarizes the McRae Avenue peak hour volumes, estimated Average Annual Daily Traffic (AADT) in the peak direction for the proposed development as well as future background volumes.

Table 16: McRae Avenue Road Volumes - NTM Review

North of the Site Access				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	8 (80 AADT)	3	6	8 (80 AADT)
2027 FB Volumes	135	155 (1550 AADT)	252 (2520 AADT)	182
<b>Total</b>	<b>142</b>	<b>158 (1580 AADT)</b>	<b>258 (2580 AADT)</b>	<b>190</b>

South of the Site Access				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	6	16 (160 AADT)	16 (160 AADT)	10
2027 FB Volumes	263 (2240 AADT)	176	331 (3310 AADT)	303
<b>Total</b>	<b>269 (2690 AADT)</b>	<b>194</b>	<b>347 (3470 AADT)</b>	<b>313</b>

South of Richmond Road				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	2	5 (50 AADT)	5 (50 AADT)	3
2027 FB Volumes	56	81 (810 AADT)	201 (2010 AADT)	182
<b>Total</b>	<b>58</b>	<b>86 (860 AADT)</b>	<b>206 (2060 AADT)</b>	<b>185</b>

Notes:

1 – AADT generated using a conservative 10:1 ratio

The TIA guidelines outline a local road threshold of 1,000 vehicles per day (AADT), or 120 vehicles in a given peak hour for Neighbourhood Traffic Management review. As illustrated above, McRae Avenue will exceed these volume thresholds in 2027. As these thresholds are exceeded independently of the subject development, which makes up 5% or less of the total future volumes on McRae Avenue, no Neighbourhood Traffic Management Plans are recommended as a result of the proposed development’s site generated traffic.

### 13.2 Tweedsmuir Avenue

Table 17 summarizes the Tweedsmuir Avenue peak hour volumes, estimated AADT in the peak direction for the proposed development as well as future background volumes.

*Table 17: Tweedsmuir Avenue Road Volumes - NTM Review*

North of Scott Street				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	0	0	0	0
2027 FB Volumes	6	7 (70 AADT)	6	7 (70 AADT)
<b>Total</b>	<b>6</b>	<b>7 (70 AADT)</b>	<b>6</b>	<b>7 (70 AADT)</b>

South of Scott Street				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	0	0	0	0
2027 FB Volumes	44 (440 AADT)	16	47 (470 AADT)	24
<b>Total</b>	<b>44 (440 AADT)</b>	<b>16</b>	<b>47 (470 AADT)</b>	<b>240</b>

South of Richmond Road				
Segment	AM Peak		PM Peak	
	North	South	North	South
320 McRae Avenue	1	4 (40 AADT)	4 (40 AADT)	2
2027 FB Volumes	47 (470 AADT)	29	48	113 (1130 AADT)
<b>Total</b>	<b>48 (480 AADT)</b>	<b>33</b>	<b>52</b>	<b>115 (1150 AADT)</b>

Notes:  
1 – AADT generated using a conservative 10:1 ratio

The TIA guidelines outline a local road threshold of 1,000 vehicles per day (AADT), or 120 vehicles in a given peak hour for Neighbourhood Traffic Management review. As illustrated above, Tweedsmuir Avenue will not exceed these volume thresholds in 2027 with the exception of the southbound PM Peak volumes south of Richmond Road. As this threshold is exceeded independently of the subject development, which makes up approximately 2% of the total future the southbound PM Peak volumes south of Richmond Road on Tweedsmuir Avenue. As such, no Neighbourhood Traffic Management Plans are recommended as a result of the proposed development’s site generated traffic.

## 14 Transit

In Section 5.1 the trip generation by mode was estimated, including the number of transit trips that will be generated by the proposed development. Table 18 summarizes the transit trip generation for the ultimate future horizon year of 2027.

*Table 18: Trip Generation by Transit Mode*

Travel Mode	Mode Share	AM Peak Period			PM Peak Period		
		In	Out	Total	In	Out	Total
	65%	38	108	146	100	64	164

The Westboro LRT Station is expected to provide adequate transit capacity to support the increase in travel demand by the proposed development.

## 15 Review of Network Concept

This TIA is exempt from this Module (See Table 7).

## 16 Intersection Design

### 16.1 Intersection Control

A signal warrant analysis was performed for the intersections of Richmond Road and Tweedsmuir Avenue and McRae Avenue and Scott Street. Signalization was not warranted at either of the intersections analyzed. As such, the intersection method of control will remain consistent with existing methods of control for all Study Area intersections at both future horizons. Signal warrants can be found in Appendix G.

### 16.2 Intersection Design

To understand the intersection design, an MMLOS analysis of existing, 2022 future horizon, and 2027 future horizon demands is required. The existing and future segment MMLOS has been discussed in Section 10. The following sections will discuss the vehicle LOS at Study Area intersections which is based on the HCM criteria for average delay at unsignalized intersections. At signalized intersections, the level of service is based on the V/C ratio as required by the City of Ottawa.

The intersection of Scott Street and Tweedsmuir Avenue is made up of a two-way stop-controlled intersection with stop control on the north and south legs and a signalized pedestrian crossing directly to the east on Scott Street. The limitations of Synchro are such that this intersection is required to be modelled as two intersections as close to one another as is allowable in Synchro. This approach was confirmed by the City of Ottawa.

This will be followed by a discussion of the intersection MMLOS for other modes.

Additionally, left-turn lane warrants for unsignalized intersections were not examined as a result of the low volume increases and acceptable intersection operations.

#### 16.2.1 Existing Conditions

The existing intersection volumes have been analyzed to establish a baseline condition and determine the impact of the subject development on the Study Area road network. Table 19 summarizes the operational analysis of the 2019 existing conditions. Appendix I contains the 2019 Existing Conditions Synchro sheets.

Table 19: Existing Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	Delay	V/C	Q (95 <sup>th</sup> )	LOS	Delay	V/C	Q (95 <sup>th</sup> )
<b>Scott Street &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	0	-	0	A	0	-	0
	WBL/T/R	A	9	0.01	<1	A	8	0.01	<1
	NBL/T/R	B	13	0.08	2	B	14	0.10	3
	SBL/T/R	D	27	0.04	1	A	29	0.04	1
<b>Scott Street &amp; Pedestrian Crossing</b> <i>Signalized</i>	EBT	A	12	0.55	69	A	11	0.47	54
	WBT	A	9	0.30	31	A	11	0.50	58
	<b>Overall</b>	<b>B</b>	<b>11</b>	-	-	<b>B</b>	<b>11</b>	-	-
<b>Scott Street &amp; McRae Avenue</b> <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBL/T	A	9	0.11	3	A	9	0.10	3
	NBL/R	C	16	0.22	6	C	20	0.49	19
<b>Richmond Road &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	8	0.02	<1	A	9	0.03	1
	WBL/T	A	0	0.01	<1	A	9	0.07	2
	WBR	-	-	-	-	-	-	-	-
	NBL/T/R	B	15	0.11	3	C	18	0.14	4
	SBL/T/R	B	13	0.04	1	D	29	0.19	7
<b>Richmond Road &amp; McRae Avenue</b> <i>Signalized</i>	EBL	A	7	0.19	14	A	18	0.30	22
	EBT/R	A	9	0.43	54	A	15	0.43	73
	WBL	A	6	0.02	2	A	7	0.10	8
	WBT/R	A	8	0.41	47	C	20	0.79	#195
	NBL	A	25	0.13	8	A	34	0.44	21
	NBT/R	A	14	0.10	7	A	16	0.30	18
	SBL/T/R	B	35	0.63	32	C	41	0.80	51
	<b>Overall</b>	<b>B</b>	<b>12</b>	-	-	<b>C</b>	<b>22</b>	-	-
<b>Notes:</b>	Saturation flow rate of 1800 veh/h/lane								
	PHF = 0.90								
	# indicates the volume for the 95 <sup>th</sup> percentile cycle exceeds capacity								

In general, the existing intersections operated well during the peak hours with no high delays or capacity issues noted. At the intersection of Richmond Road and McRae Avenue, the 95<sup>th</sup> percentile cycle exceeds capacity for the westbound shared through / right movement in the PM peak. The V/C ratio for this movement is less than one and it can therefore be assumed that in practice the 95<sup>th</sup> percentile queue will rarely be exceeded.

16.2.2 2022 Future Background

The 2022 future background intersection volumes and other development traffic has been analyzed to allow a comparison between the future volumes with and without the proposed development. Table 20 summarizes the operational analysis of 2022 future background conditions. Appendix J contains the 2022 Future Background Synchro sheets.

Table 20: 2022 Future Background Conditions Operational Analysis

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	Delay	V/C	Q (95 <sup>th</sup> )	LOS	Delay	V/C	Q (95 <sup>th</sup> )
<b>Scott Street &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	0	-	0	A	0	-	0
	WBL/T/R	A	9	0.01	<1	A	8	0.01	<1
	NBL/T/R	B	13	0.09	2	B	14	0.10	3
	SBL/T/R	D	30	0.04	1	D	27	0.04	1
<b>Scott Street &amp; Pedestrian Crossing</b> <i>Signalized</i>	EBT	A	13	0.59	77	A	11	0.45	52
	WBT	A	9	0.33	35	A	11	0.48	56
	<b>Overall</b>	<b>B</b>	<b>11</b>	-	-	<b>B</b>	<b>11</b>	-	-
<b>Scott Street &amp; McRae Avenue</b> <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBL/T	A	9	0.13	4	A	9	0.11	3
	NBL/R	C	19	0.34	11	C	21	0.51	20
<b>Richmond Road &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	8	0.02	<1	A	9	0.03	1
	WBL/T	A	0	0.02	<1	A	9	0.07	2
	WBR	-	-	-	-	-	-	-	-
	NBL/T/R	C	15	0.12	3	C	17	0.12	3
	SBL/T/R	B	13	0.05	1	D	27	0.17	5
<b>Richmond Road &amp; McRae Avenue</b> <i>Signalized</i>	EBL	A	9	0.26	18	A	20	0.33	23
	EBT/R	A	10	0.47	62	A	15	0.43	71
	WBL	A	7	0.02	2	A	8	0.09	8
	WBT/R	A	9	0.49	61	C	21	0.80	#193
	NBL	A	23	0.13	8	A	30	0.39	19
	NBT/R	A	13	0.10	7	A	15	0.28	18
	SBL/T/R	B	36	0.67	37	C	40	0.80	54
	<b>Overall</b>	<b>B</b>	<b>14</b>	-	-	<b>C</b>	<b>22</b>	-	-
<b>Notes:</b>	Saturation flow rate of 1800 veh/h/lane								
	PHF = 1.00								
	# indicates the volume for the 95 <sup>th</sup> percentile cycle exceeds capacity								

With the addition of background growth to reflect the 2022 horizon as well as traffic generated from surrounding developments, the existing intersections are anticipated to operate with similar operational characteristics to the existing conditions, and well within the City of Ottawa operational thresholds.

At the intersection of Richmond Road and McRae Avenue, the 95<sup>th</sup> percentile cycle exceeds capacity for the westbound shared through / right movement in the PM peak. The V/C ratio for this movement is less than one and it can therefore be assumed that in practice the 95<sup>th</sup> percentile queue will rarely be exceeded.

16.2.3 2027 Future Background

The 2027 future background intersection volumes and other development traffic has been analyzed to allow a comparison between the future volumes with and without the proposed development. Table 21 summarizes the operational analysis of 2027 future background conditions. Appendix K contains the 2027 Future Background Synchro sheets.

Table 21: 2027 Future Background Conditions Operational Analysis

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	Delay	V/C	Q (95 <sup>th</sup> )	LOS	Delay	V/C	Q (95 <sup>th</sup> )
<b>Scott Street &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	0	-	0	A	0	-	0
	WBL/T/R	A	9	0.01	<1	A	8	0.01	<1
	NBL/T/R	B	14	0.11	3	C	15	0.11	3
	SBL/T/R	E	35	0.06	2	D	33	0.05	1
<b>Scott Street &amp; Pedestrian Crossing</b> <i>Signalized</i>	EBT	B	15	0.66	#106	A	11	0.50	59
	WBT	A	10	0.36	39	A	12	0.53	64
	<b>Overall</b>	<b>B</b>	<b>11</b>	-	-	<b>B</b>	<b>12</b>	-	-
<b>Scott Street &amp; McRae Avenue</b> <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBL/T	A	10	0.15	4	A	9	0.12	3
	NBL/R	C	22	0.41	15	D	27	0.62	26
<b>Richmond Road &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	8	0.02	1	A	9	0.03	1
	WBL/T	A	9	0.02	1	A	9	0.08	2
	WBR	-	-	-	-	A	0	-	-
	NBL/T/R	C	17	0.15	4	C	20	0.17	6
	SBL/T/R	B	15	0.06	2	D	34	0.24	10
<b>Richmond Road &amp; McRae Avenue</b> <i>Signalized</i>	EBL	A	10	0.31	21	A	32	0.51	#36
	EBT/R	A	11	0.53	72	A	17	0.50	81
	WBL	A	7	0.02	3	A	9	0.12	9
	WBT/R	A	11	0.54	70	E	32	0.91	#223
	NBL	A	23	0.14	9	A	29	0.39	21
	NBT/R	A	13	0.10	7	A	15	0.28	20
	SBL/T/R	C	37	0.71	40	D	41	0.82	61
	<b>Overall</b>	<b>B</b>	<b>15</b>	-	-	<b>C</b>	<b>28</b>	-	-
<b>Notes:</b>	Saturation flow rate of 1800 veh/h/lane								
	PHF = 1.00								
	# indicates the volume for the 95 <sup>th</sup> percentile cycle exceeds capacity								

With the addition of background growth to reflect the 2027 horizon as well as traffic generated from surrounding developments, the existing intersections are anticipated to operate with similar operational characteristics to the existing conditions, and well within the City of Ottawa operational thresholds.

The AM peak eastbound through at the intersection of Scott Street and the pedestrian signal, and the eastbound left-turn and westbound shared through / right movement in the PM peak at the intersection of Richmond Road and McRae Avenue, exceed the 95<sup>th</sup> percentile cycle capacity. The V/C ratio for these movements is less than one and it can therefore be assumed that in practice the 95<sup>th</sup> percentile queue will rarely be exceeded.

16.2.4 2022 Total Future

The 2022 total future intersection volumes, including the site generated traffic and other development traffic, has been analyzed to understand the impact of the subject development on the Study Area intersections. Table 22 summarizes the operational analysis of the 2022 total future conditions. Appendix L contains the 2022 Future Total Synchro Sheets.



Table 22: 2022 Total Future Conditions Operational Analysis

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	Delay	V/C	Q (95 <sup>th</sup> )	LOS	Delay	V/C	Q (95 <sup>th</sup> )
<b>Scott Street &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	0	-	0	A	0	-	0
	WBL/T/R	A	9	0.01	<1	A	8	0.01	<1
	NBL/T/R	B	13	0.09	2	B	14	0.10	3
	SBL/T/R	D	30	0.04	1	D	27	0.04	<1
<b>Scott Street &amp; Pedestrian Crossing</b> <i>Signalized</i>	EBT	A	13	0.59	77	A	11	0.45	52
	WBT	A	9	0.33	35	A	11	0.48	56
	<b>Overall</b>	<b>B</b>	<b>12</b>	-	-	<b>B</b>	<b>11</b>	-	-
<b>Scott Street &amp; McRae Avenue</b> <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBL/T	A	9	0.14	4	A	9	0.14	4
	NBL/R	C	19	0.41	19	C	23	0.56	24
<b>Richmond Road &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	8	0.02	<1	A	9	0.03	1
	WBL/T	A	9	0.03	1	A	9	0.07	2
	WBR	-	-	-	-	-	-	-	-
	NBL/T/R	C	16	0.14	4	C	16	0.15	4
	SBL/T/R	B	14	0.05	1	D	29	0.19	6
<b>Richmond Road &amp; McRae Avenue</b> <i>Signalized</i>	EBL	A	11	0.31	20	A	28	0.49	#36
	EBT/R	A	12	0.50	62	A	17	0.45	81
	WBL	A	8	0.02	2	A	9	0.10	9
	WBT/R	A	11	0.52	61	D	26	0.85	#223
	NBL	A	22	0.12	8	A	28	0.36	21
	NBT/R	A	14	0.10	8	A	17	0.29	20
	SBL/T/R	C	38	0.75	49	D	41	0.82	61
	<b>Overall</b>	<b>B</b>	<b>16</b>	-	-	<b>C</b>	<b>25</b>	-	-
<b>McRae Avenue &amp; Site Access #1</b> <i>Unsignalized</i>	EBL/R	B	10	0.10	3	B	11	0.07	2
	NBL/T	A	8	0.01	<1	A	8	0.04	1
	SBT/R	-	-	-	-	-	-	-	-
<b>Notes:</b>	Saturation flow rate of 1800 veh/h/lane								
	PHF = 1.00								
	# indicates the volume for the 95 <sup>th</sup> percentile cycle exceeds capacity								

With the addition of the site generated traffic, the Study Area intersections are expected to operate with similar operational characteristics as the 2022 future background conditions, and well within the City of Ottawa operational thresholds.

The eastbound left-turn and westbound shared through / right movement in the PM peak at the intersection of Richmond Road and McRae Avenue, exceed the 95<sup>th</sup> percentile cycle capacity. The V/C ratio for these movements is less than one and it can therefore be assumed that in practice the 95<sup>th</sup> percentile queue will rarely be exceeded.

16.2.5 2027 Total Future

The 2027 total future intersection volumes, including the site generated traffic and other development traffic, has been analyzed to understand the impact of the subject development on the Study Area intersections. Table 23 summarizes the operational analysis of the 2027 total future conditions. Appendix M contains the 2027 Future Total Synchro Sheets.

Table 23: 2027 Total Future Conditions Operational Analysis

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	Delay	V/C	Q (95 <sup>th</sup> )	LOS	Delay	V/C	Q (95 <sup>th</sup> )
<b>Scott Street &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	0	-	0	A	0	-	0
	WBL/T/R	A	9	0.01	<1	A	8	0.02	<1
	NBL/T/R	B	14	0.11	3	C	15	0.12	3
	SBL/T/R	E	35	0.06	2	D	33	0.05	1
<b>Scott Street &amp; Pedestrian Crossing</b> <i>Signalized</i>	EBT	B	15	0.66	#106	A	11	0.50	59
	WBT	A	10	0.36	39	A	12	0.53	64
	<b>Overall</b>	<b>B</b>	<b>13</b>	-	-	<b>B</b>	<b>12</b>	-	-
<b>Scott Street &amp; McRae Avenue</b> <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBL/T	A	10	0.16	5	A	9	0.13	4
	NBL/R	C	22	0.44	16	D	28	0.64	31
<b>Richmond Road &amp; Tweedsmuir Avenue</b> <i>Unsignalized</i>	EBL/T/R	A	8	0.02	1	A	9	0.03	1
	WBL/T	A	9	0.02	1	A	9	0.08	2
	WBR	-	-	-	-	-	-	-	-
	NBL/T/R	C	17	0.16	4	C	20	0.18	7
	SBL/T/R	B	15	0.06	2	D	35	0.24	10
<b>Richmond Road &amp; McRae Avenue</b> <i>Signalized</i>	EBL	A	11	0.33	21	A	38	0.58	#40
	EBT/R	A	12	0.54	72	A	18	0.51	81
	WBL	A	7	0.02	3	A	9	0.12	9
	WBT/R	A	11	0.55	70	E	35	0.93	#225
	NBL	A	23	0.14	9	A	28	0.38	21
	NBT/R	A	13	0.10	7	A	16	0.29	21
	SBL/T/R	C	38	0.73	44	D	41	0.82	64
	<b>Overall</b>	<b>B</b>	<b>16</b>	-	-	<b>C</b>	<b>30</b>	-	-
<b>McRae Avenue &amp; Site Access #1</b> <i>Unsignalized</i>	EBL/R	A	10	0.03	1	B	10	0.02	1
	NBL/T	A	8	0.00	<1	A	8	0.01	<1
	SBT/R	-	-	-	-	-	-	-	-
<b>Notes:</b>	Saturation flow rate of 1800 veh/h/lane								
	PHF = 1.00								
	# indicates the volume for the 95 <sup>th</sup> percentile cycle exceeds capacity								

With the addition of the site generated traffic, the Study Area intersections are expected to operate with similar operational characteristics as the 2027 future background conditions, and well within the City of Ottawa operational thresholds.

The AM peak eastbound through at the intersection of Scott Street and the pedestrian signal, and the eastbound left-turn and westbound shared through / right movement in the PM peak at the intersection of Richmond Road and McRae Avenue, exceed the 95<sup>th</sup> percentile cycle capacity. The V/C ratio for these movements is less than one and it can therefore be assumed that in practice the 95<sup>th</sup> percentile queue will rarely be exceeded.

16.2.6 Intersection MMLOS

As intersection MMLOS is only undertaken at signalized intersections, only the intersection of Richmond Road and McRae Avenue and the intersection of Tweedsmuir Avenue and Scott Street can be analyzed. As no changes that will impact the MMLOS are expected at this intersection, the existing and future horizons are the same and can be considered in one row. The MMLOS worksheets have been provided in Appendix F.

Table 24 summarizes the MMLOS analysis for the network signalized intersection in the Study Area for all horizons.

Table 24: Study Area Intersection MMLOS Analysis-All Horizons

Intersection	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target
Richmond Road & McRae Avenue	E	A	F	D	E(F)	D	E	D	B(D)	E
Tweedsmuir Avenue & Scott Street	B	A	F	D	C(C)	D	F	D	A(A)	E

The target levels of service within 600 metres of a rapid transit station were used to evaluate the Study Area intersections.

The existing pedestrian LOS does not meet the target at either intersection due to large pedestrian crossing distances. The bicycle LOS does not meet the targets due to mixed traffic conditions along Tweedsmuir Avenue and McRae Avenue, as well as high vehicle operating speeds along Scott Street and Richmond Road. Transit LOS is limited due to signal delay and as such the target is only met in both peak periods at the intersection of Tweedsmuir Avenue and Scott Street. The truck LOS is not met at either intersection due to low number of receiving lanes on the departure from the intersections. Auto LOS meets the targets at both intersections.

16.2.7 Intersection Design

No intersection changes are recommended based on the above PLOS, BLOS, TLOS, TkLOS or vehicle LOS analysis above.

17 Conclusions

This Transportation Impact Assessment has documented the existing and future transportation conditions, for all travel modes, in the Study Area. The following conclusions can be offered based on the foregoing:

- A. The proposed development, located at 320 McRae Avenue, is a mixed-use development which will consist of a four-storey commercial / residential tower, and a commercial / residential tower with both a 26-storey and a six-storey component. The development is expected to have 882 square metres of commercial space, 307 apartment units, 11 townhouse units, 185 underground automobile parking spaces and 163 bicycle parking spaces
- B. The proposed development will have two full-movement accesses, one approximately 40 metres, curb to curb, south of Scott Street on Tweedsmuir Avenue (Site Access #1) and the second approximately 120 metres, curb to curb, south of Scott Street on McRae Avenue (Site Access #2). Site Access #2 is a loading access and is intended for truck use only. A drop-off area is located on McRae Avenue, approximately 23 metres, curb to curb, south of Scott Street.
- C. The proposed development is within 400 metres of the existing Westboro Transitway Station and will be within 400 metres of the future Westboro LRT Station. As such, can be considered a Transit Oriented Development area.
- D. The existing Study Area is currently served by bus routes #11, 50, 81 and 153 as well as the Westboro Transitway Station.
- E. The previous five years of collision history at the existing Study Area intersections has been reviewed. No patterns emerged that indicated that mitigation measures or further monitoring was required.
- F. Using the TRANS study, the residential trip generation rates were calculated and using the ITE trip generation equations, the retail rates were calculated.

- G. The Ottawa West area mode shares were used for the 2022 horizon and the TOD mode shares were used for the 2027 horizon to determine the trip generation by mode.
- H. The proposed development is anticipated to generate an estimated 112 AM and 127 PM peak hour two-way person trips in the 2022 horizon and an estimated 33 AM and 40 PM peak hour two-way person trips in the 2027 horizon.
- I. TOD areas do not require resident or retail vehicle parking, however, 185 parking spaces will be provided of which 30 are required to be designated as residential visitor parking. Additionally, 163 bicycle parking spots will be provided. All minimum parking requirements are met or exceeded.
- J. The vertical geometry of the underground parking ramp has been examined and analyzed.
- K. Turning templates indicate that the proposed accesses and circulation route within the development can accommodate the expected garbage trucks and vehicles.
- L. It was found that the road segments of Tweedsmuir Avenue, Scott Street and McRae Avenue will not meet the PLOS target. Additionally, Tweedsmuir Avenue will not meet the BLOS target and Scott Street will not meet the Scott Street target. No resulting improvements are recommended.
- M. Signal warrants and turning lane warrants were examined for Site Access #1. Neither signalization or turning lanes were warranted.
- N. Signal warrants have been examined at the unsignalized intersections within the Study Area. Signalization was not found to be warranted.
- O. The Study Area intersections operate satisfactorily during the peak hours in the existing conditions operational analysis.
- P. The Study Area intersections operate satisfactorily during the peak hours in the 2022 future background operational analysis.
- Q. The Study Area intersections operate satisfactorily during the peak hours in the 2022 future total operational analysis with similar operational characteristics as the 2022 future background conditions.
- R. The Study Area intersections operate satisfactorily during the peak hours in the 2027 future background operational analysis.
- S. The Study Area intersections operate satisfactorily during the peak hours in the 2027 future total operational analysis with similar operational characteristics as the 2027 future background conditions.
- T. The PLOS, BLOS, TLOS, and TkLOS were evaluated at both signalized Study Area intersections (Scott Street at Tweedsmuir Avenue and Richmond Road at McRae Avenue). In some cases, the MMLOS targets were not met. No intersection alterations or mitigation measures are suggested as changes to these intersections are not feasible.

The proposed development will function within the Study Area Road Network. It is recommended that, from a transportation perspective, the proposed development application process proceed.

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# Appendix A

TIA Screening Form and PM Certification Form

City of Ottawa 2017 TIA Guidelines  
Step 1 - Screening Form

Date: 31-Jan-20  
Project Number: 2019-29  
Project Reference: GWL 320 McRae

1.1 Description of Proposed Development	
Municipal Address	320 McRae Avenue
Description of Location	PLAN 263 LOTS 24 AND 25 PLAN; 273 LOT 12 TO 19
Land Use Classification	Traditional Mainstreet Zone, Parks and Open Spcae Zo
Development Size	318 residential units, 882 square metres of commercial retail, 185 parking spaces, 163 bicycle parking spaces
Accesses	Access on McRae Ave (approximately 120 metres south of the Scott St / McRae Ave intersection). Access on Tweedsmuir Ave (approximately 40 metres south of Scott St / Tweedsmuir Ave intersection)
Phase of Development	Single Phase
Buildout Year	2022
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Townhomes or apartments
Development Size	307 Units
Trip Generation Trigger	Yes

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	No
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?	Yes
Location Trigger	Yes

1.4. Safety Triggers	
Are posted speed limits on a boundary street 80 km/hr or greater?	No
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	No
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	Yes
Is the proposed driveway within auxiliary lanes of an intersection?	No
Does the proposed driveway make use of an existing median break that serves an existing site?	No
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development development?	No
Does the development include a drive-thru facility?	No
Safety Trigger	Yes



## **TIA Plan Reports**

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

### **CERTIFICATION**

1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
4. I am either a licensed<sup>1</sup> or registered<sup>2</sup> professional in good standing, whose field of expertise [check  appropriate field(s)] is either transportation engineering  or transportation planning .

**1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.**

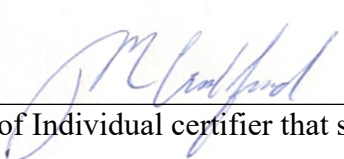
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Urbanisme et Gestion de la croissance  
110, avenue Laurier Ouest  
Ottawa (Ontario) K1P 1J1  
Tél. : 613-580-2424  
Télécopieur: 613-560-6006

Dated at Newmarket this 26 day of July, 2019.  
(City)

Name: Mark Crockford  
(Please Print)

Professional Title: Professional Engineer

  
\_\_\_\_\_  
Signature of Individual certifier that s/he meets the above four criteria

**Office Contact Information (Please Print)**

Address: 628 Haines Road

City / Postal Code: Newmarket / L3Y 6V5

Telephone / Extension: (905) 251-4070

E-Mail Address: Mark.Crockford@CGHTransportation.com





# Appendix B

Traffic Data

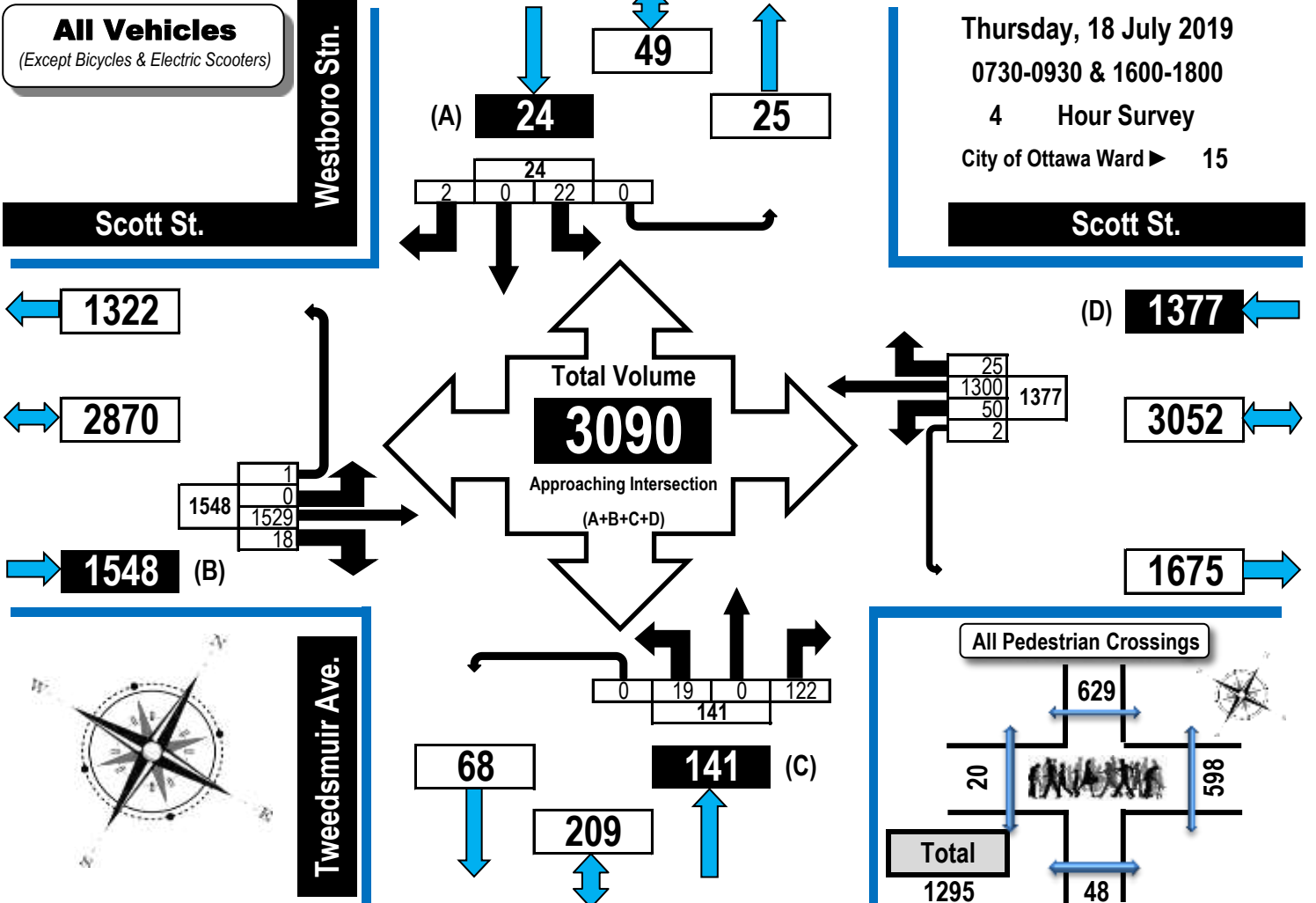


# Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

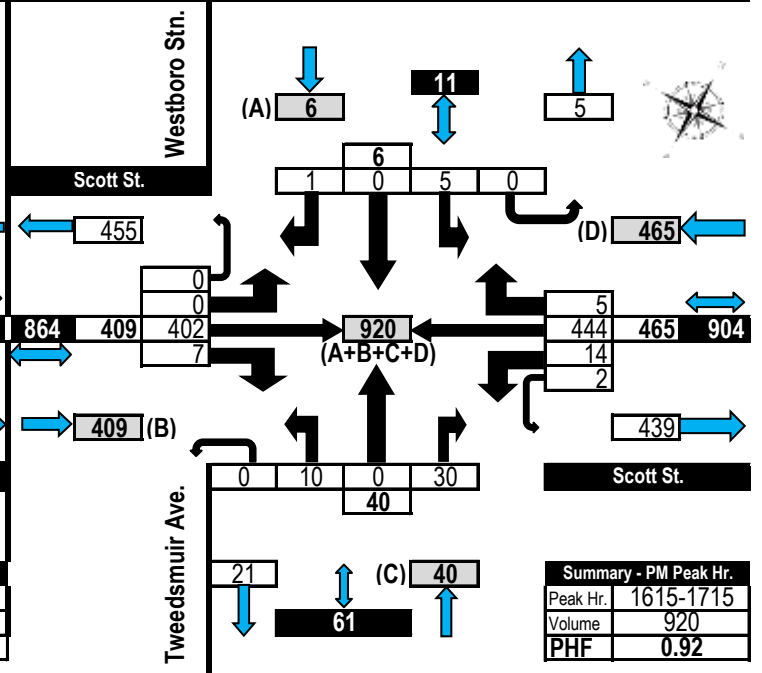
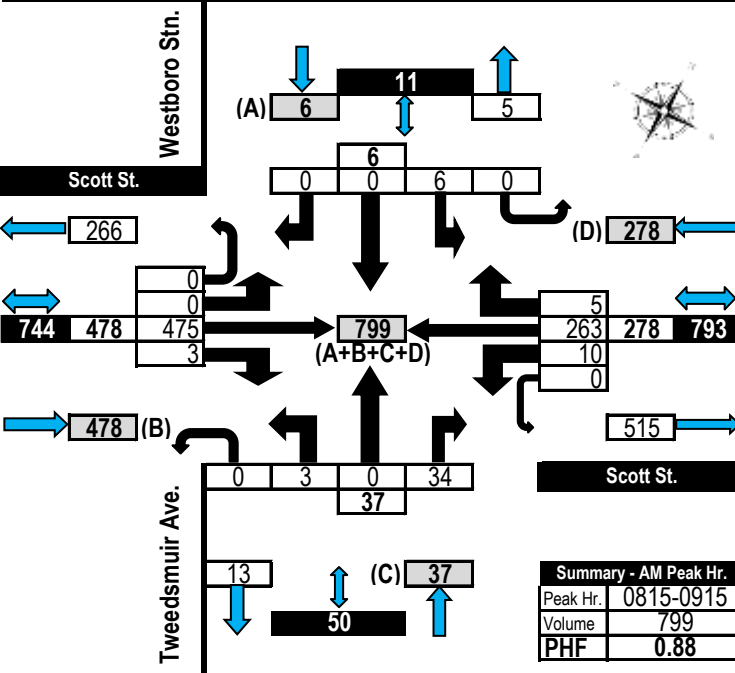
Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

## Scott Street & Tweedsmuir Avenue

## Ottawa, ON



### AM Peak Hour Flow Diagram      PM Peak Hour Flow Diagram

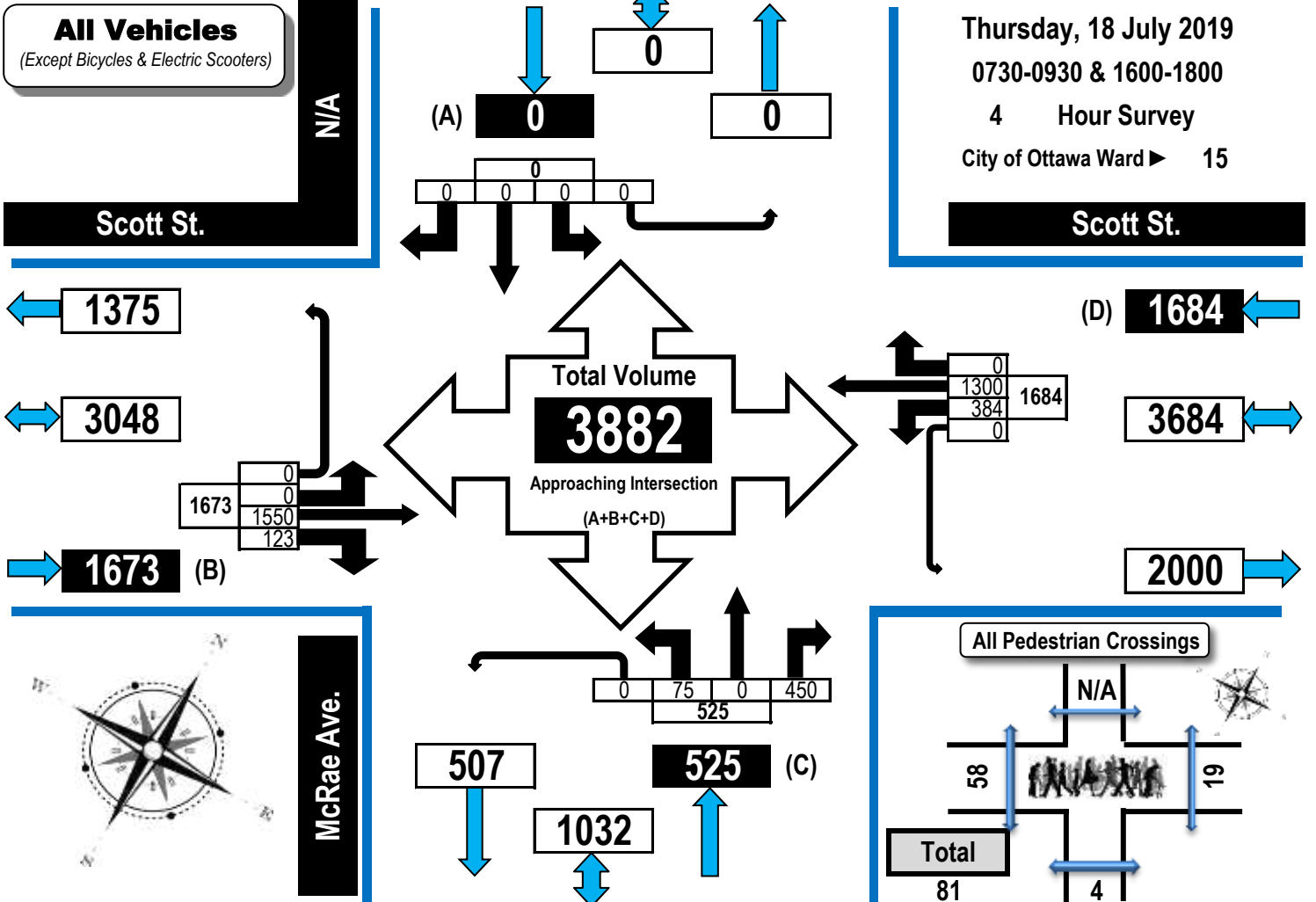




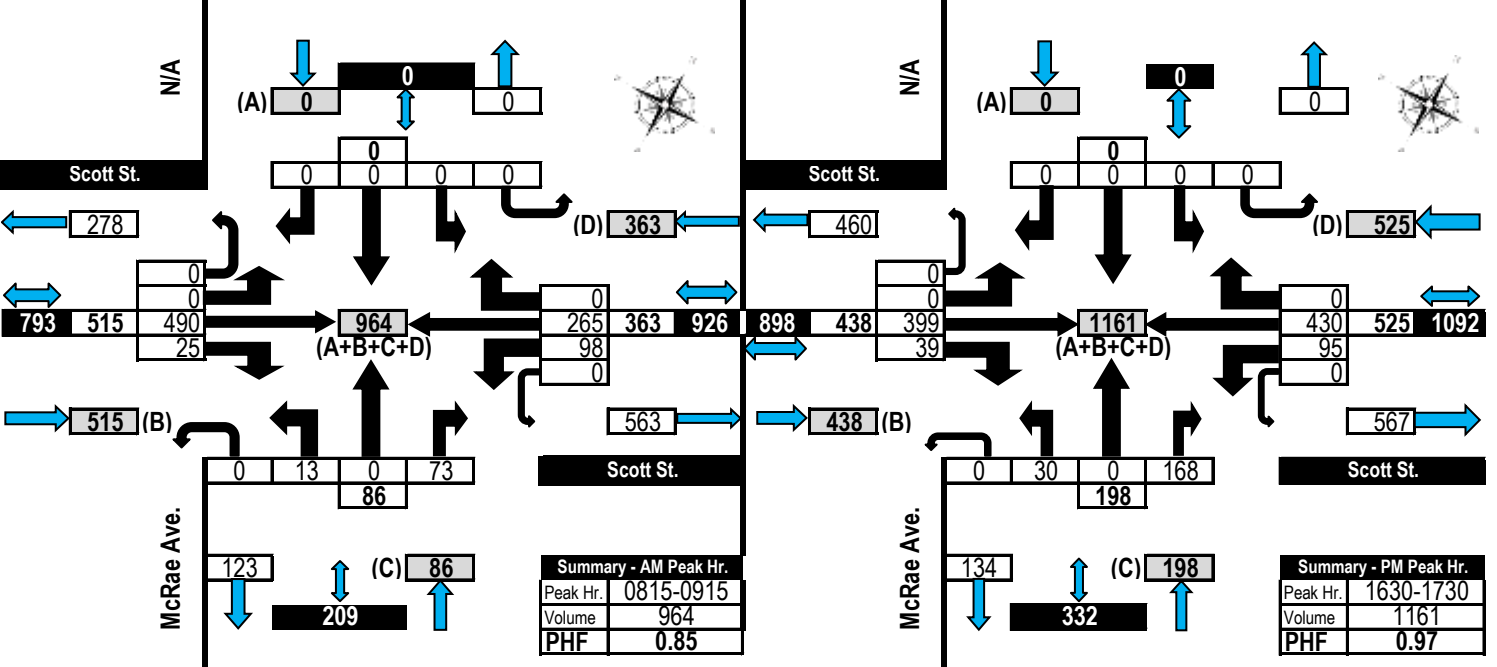
# Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

## McRae Avenue & Scott Street Ottawa, ON



### AM Peak Hour Flow Diagram PM Peak Hour Flow Diagram

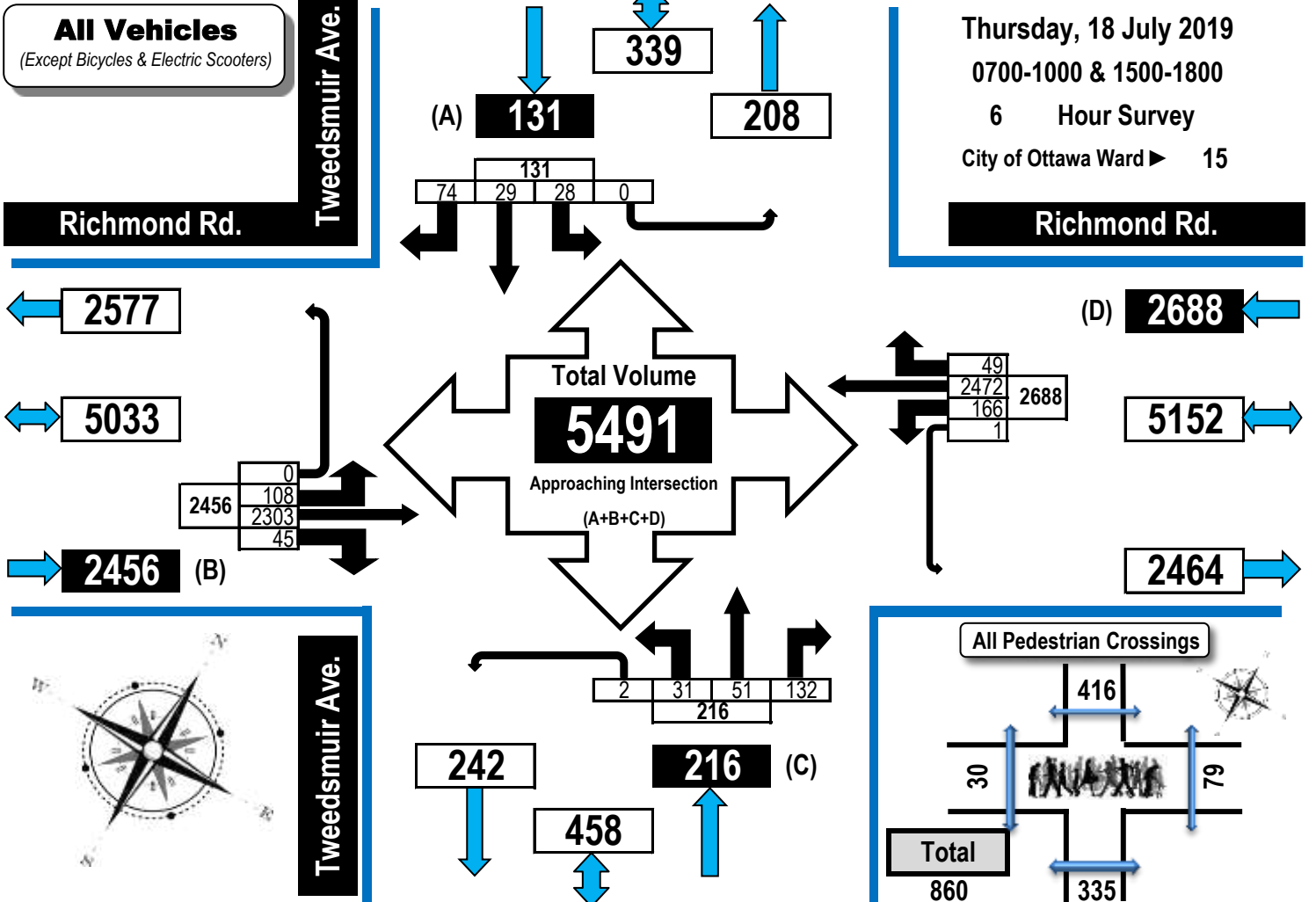




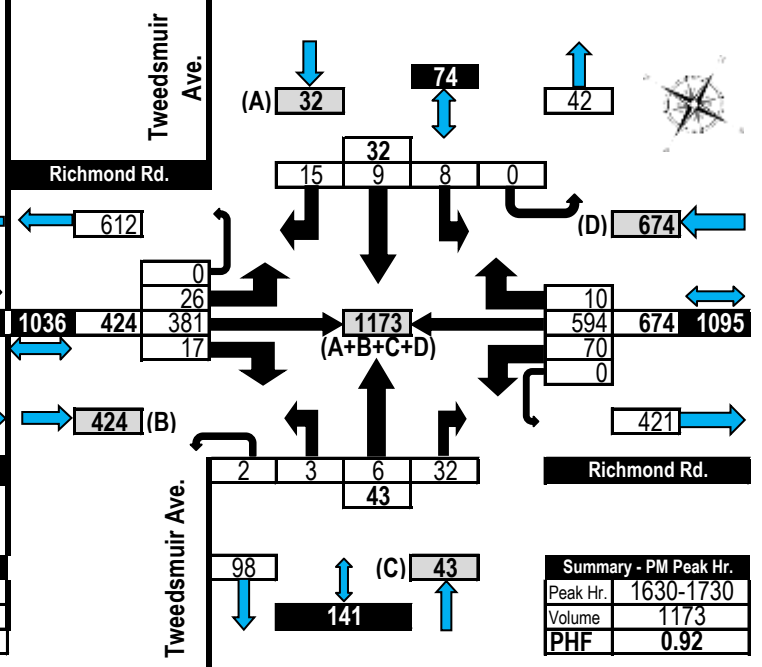
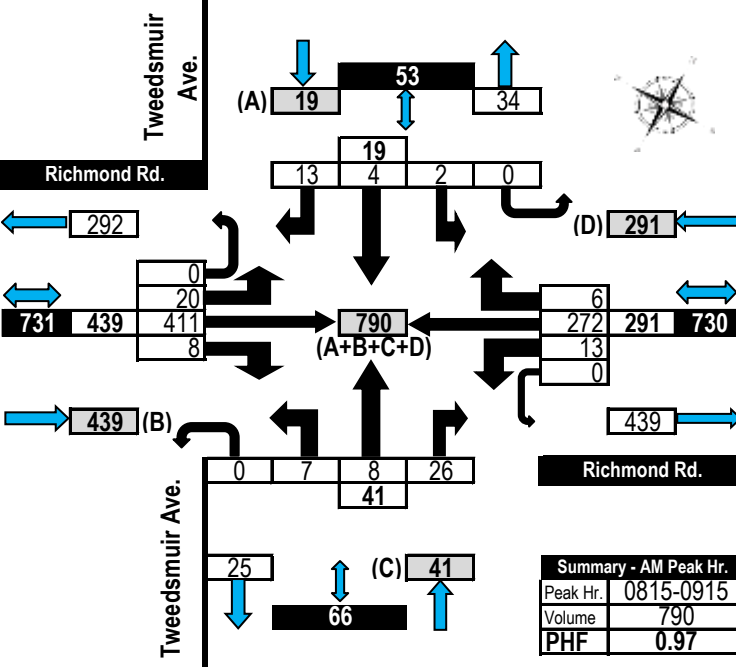
# Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

## Richmond Road & Tweedsmuir Avenue Ottawa, ON



### AM Peak Hour Flow Diagram PM Peak Hour Flow Diagram





# Transportation Services - Traffic Services

## Turning Movement Count - Full Study Peak Hour Diagram

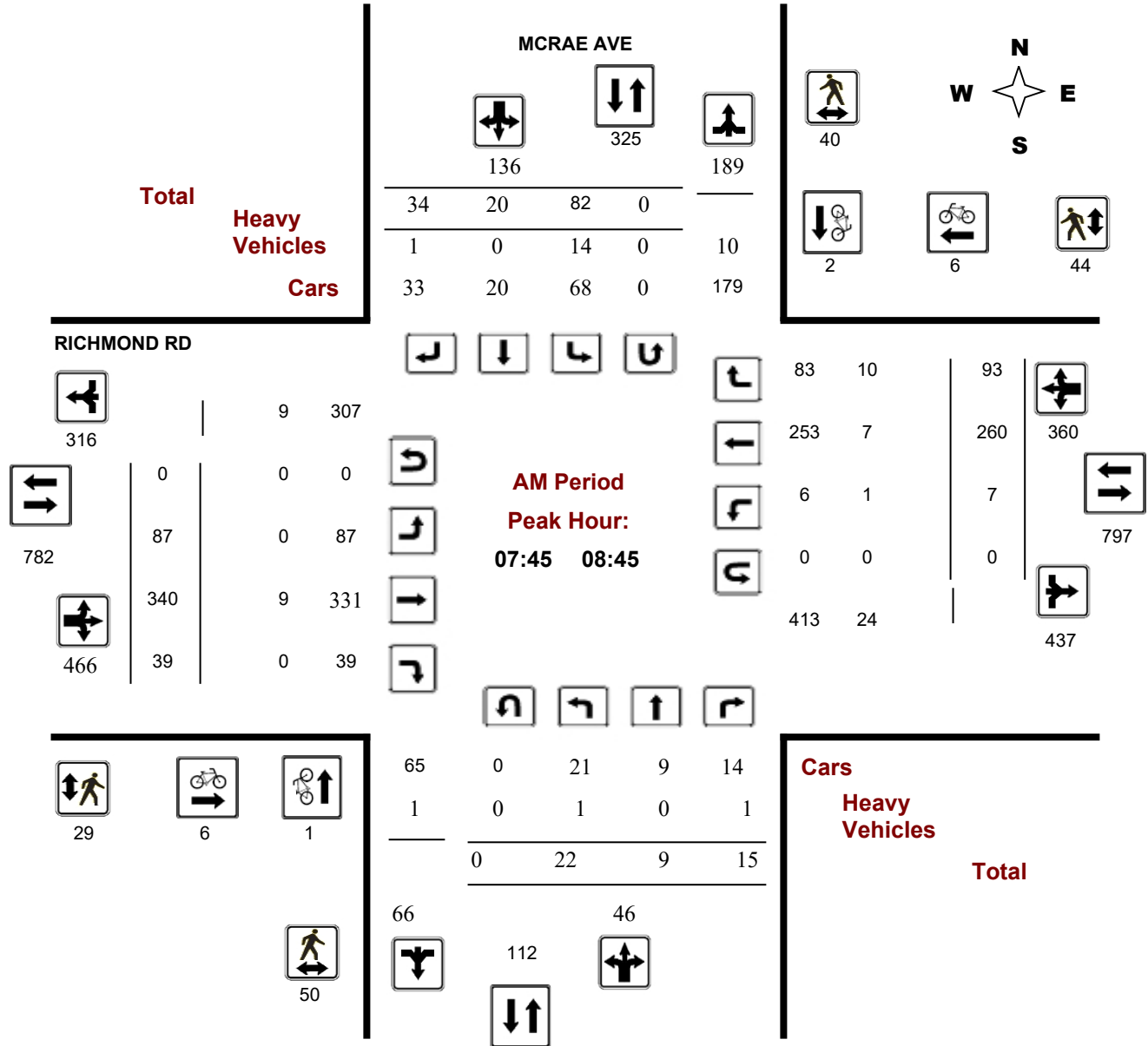
### MCRAE AVE @ RICHMOND RD

**Survey Date:** Thursday, April 20, 2017

**Start Time:** 07:00

**WO No:** 36957

**Device:** Miovision



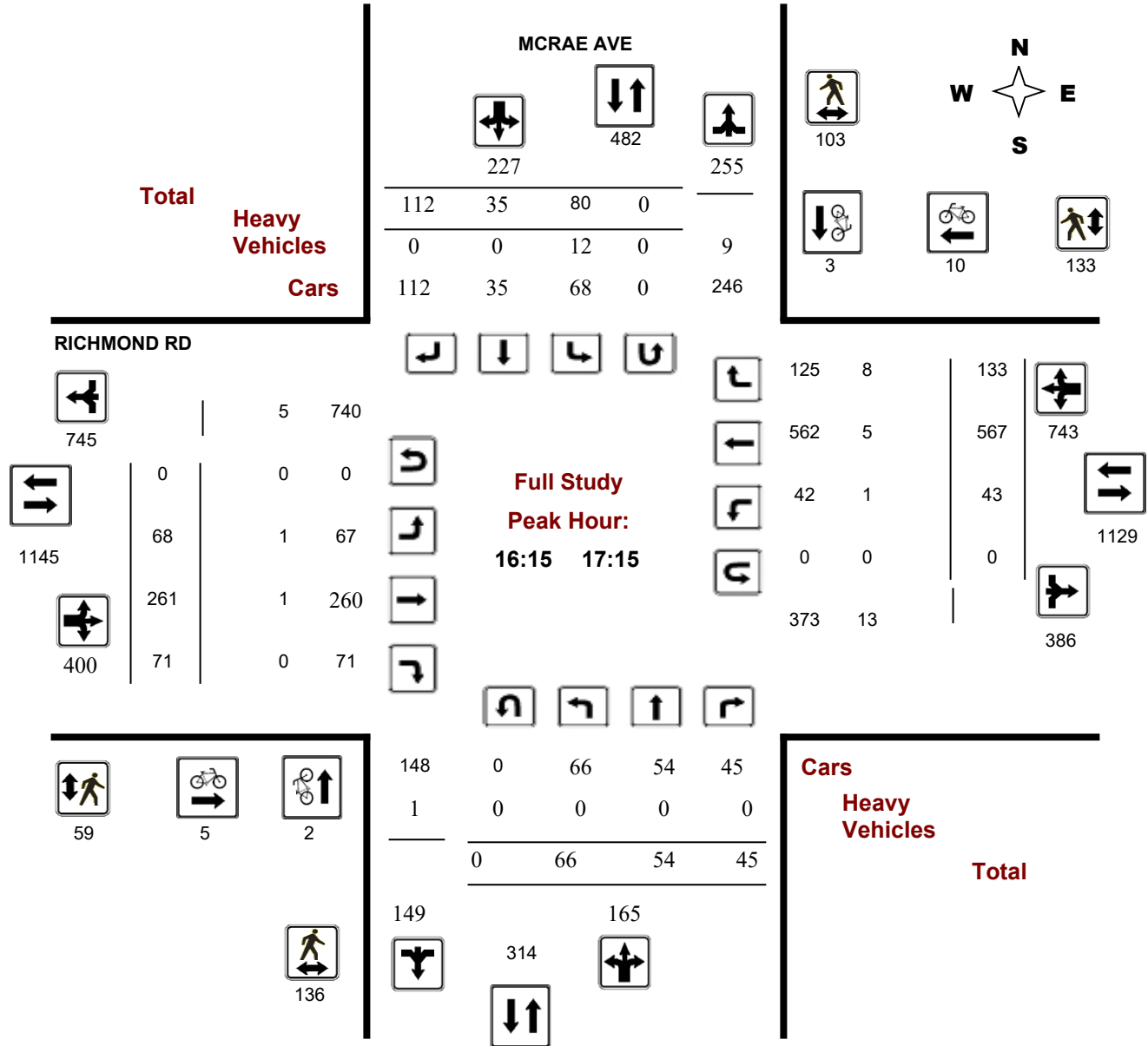
**Comments**

**Survey Date:** Thursday, April 20, 2017

**Start Time:** 07:00

**WO No:** 36957

**Device:** Miovision



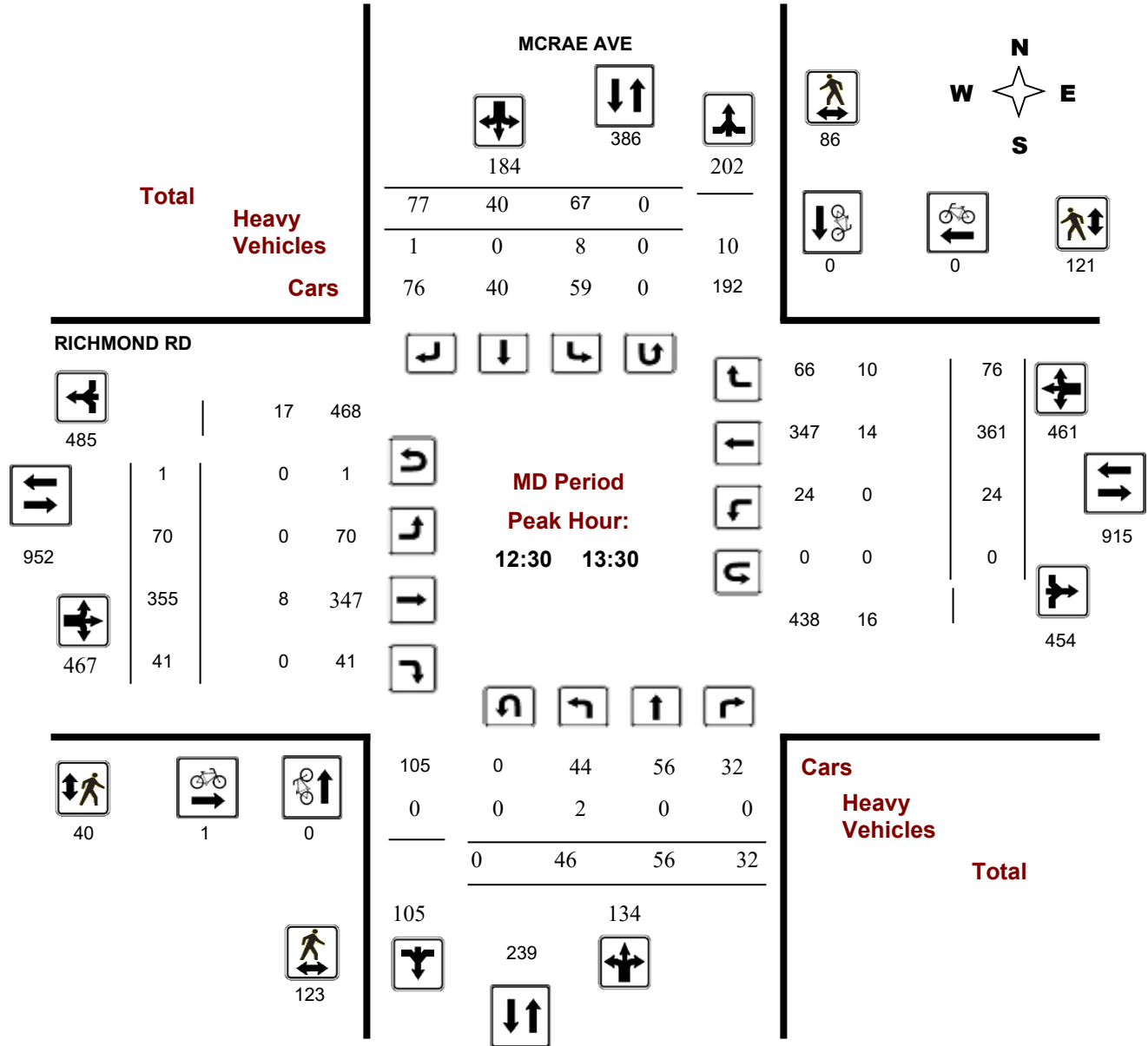
**Comments**

**Survey Date:** Thursday, April 20, 2017

**Start Time:** 07:00

**WO No:** 36957

**Device:** Miovision



**Comments**

## Turning Movement Count - Full Study Peak Hour Diagram

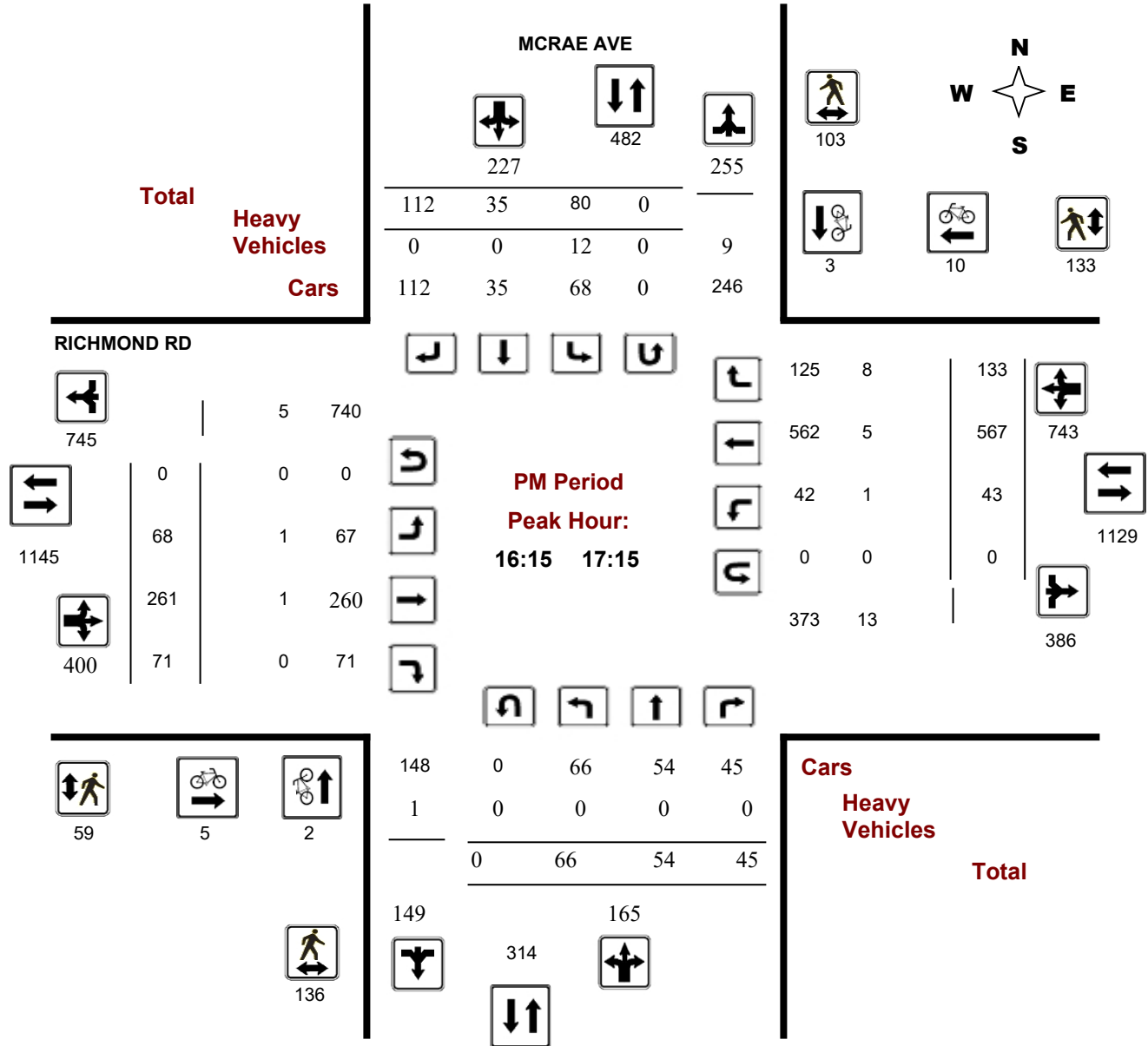
### MCRAE AVE @ RICHMOND RD

**Survey Date:** Thursday, April 20, 2017

**Start Time:** 07:00

**WO No:** 36957

**Device:** Miovision





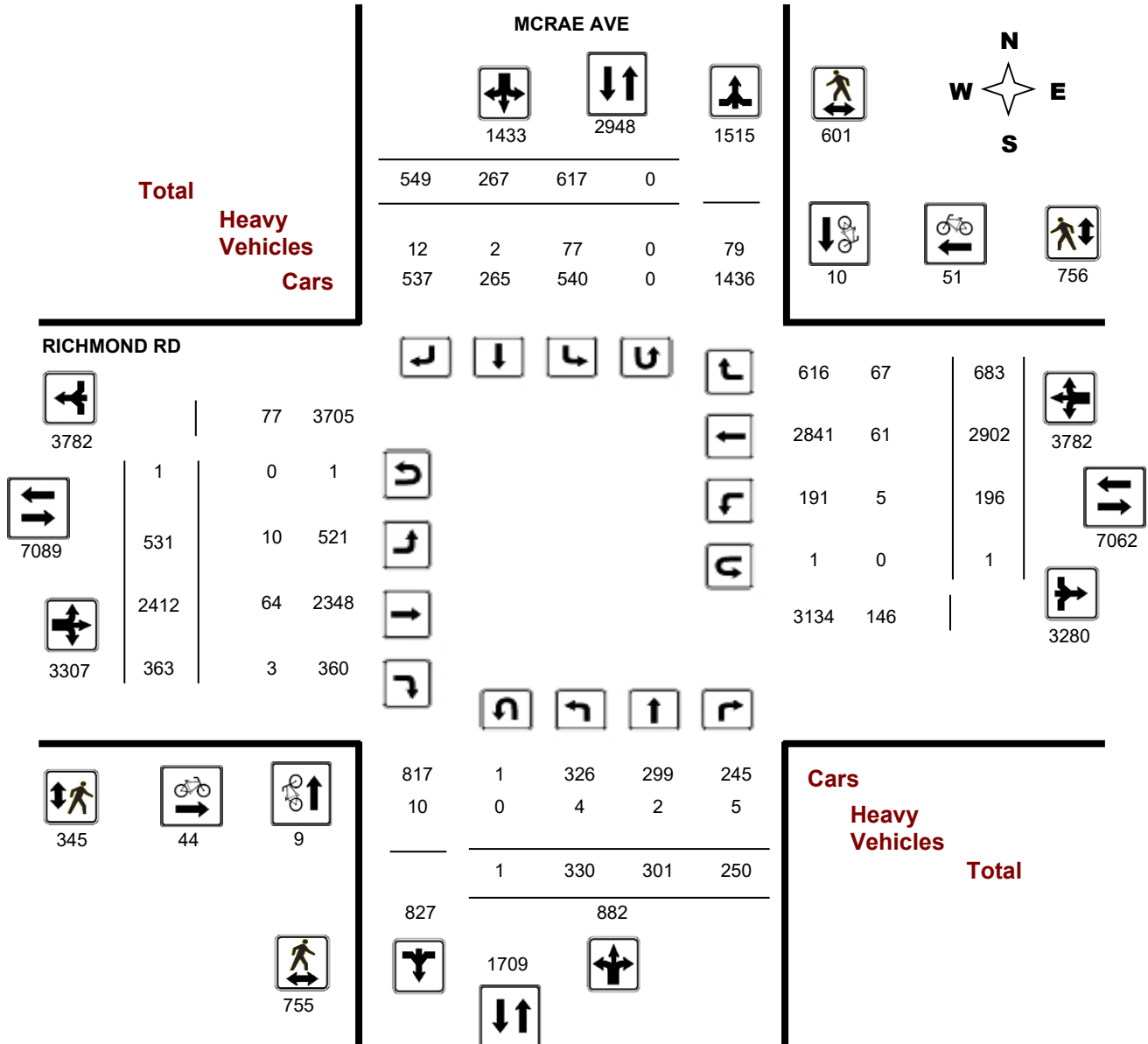
# Transportation Services - Traffic Services

## Turning Movement Count - Full Study Diagram

### MCRAE AVE @ RICHMOND RD

**Survey Date:** Thursday, April 20, 2017

**WO#:** 36957  
**Device:** Miovision



**Comments**

## Turning Movement Count - Full Study Summary Report

### MCRAE AVE @ RICHMOND RD

**Survey Date:** Thursday, April 20, 2017

**Total Observed U-Turns**

Northbound: 1      Southbound: 0  
Eastbound: 1      Westbound: 1

**AADT Factor**

.90

**Full Study**

Period	MCRAE AVE								RICHMOND RD								STR TOT	Grand Total	
	Northbound				Southbound				Eastbound				Westbound						
	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT			
07:00 08:00	17	6	12	35	61	16	25	102	137	67	310	29	406	6	194	67	267	673	810
08:00 09:00	19	12	12	43	88	19	41	148	191	80	318	36	434	7	252	93	352	786	977
09:00 10:00	25	18	11	54	53	31	41	125	179	65	277	34	376	14	273	66	353	729	908
11:30 12:30	43	36	46	125	82	37	72	191	316	68	327	52	447	31	353	68	452	899	1215
12:30 13:30	46	56	32	134	67	40	77	184	318	70	355	41	466	24	361	76	461	927	1245
15:00 16:00	53	51	42	146	87	33	84	204	350	54	284	50	388	32	423	77	532	920	1270
16:00 17:00	65	54	50	169	82	37	106	225	394	59	255	63	377	43	528	137	708	1085	1479
17:00 18:00	62	68	45	175	97	54	103	254	429	68	286	58	412	39	518	99	656	1068	1497
<b>Sub Total</b>	330	301	250	881	617	267	549	1433	2314	531	2412	363	3306	196	2902	683	3781	7087	9401
<b>U Turns</b>				1				0	1				1				1	2	3
<b>Total</b>	330	301	250	882	617	267	549	1433	2315	531	2412	363	3307	196	2902	683	3782	7089	9404
<b>EQ 12Hr</b>	459	418	348	1226	858	371	763	1992	3218	738	3353	505	4597	272	4034	949	5257	9854	13072
Note: These values are calculated by multiplying the totals by the appropriate expansion factor.													<b>1.39</b>						
<b>AVG 12Hr</b>	413	377	313	1103	772	334	687	1793	2896	664	3017	454	4137	245	3630	854	4731	8868	11764
Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.													<b>.90</b>						
<b>AVG 24Hr</b>	541	493	410	1445	1011	438	900	2348	3793	870	3953	595	5420	321	4756	1119	6198	11618	15411
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.													<b>1.31</b>						

**Comments:**

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Turning Movement Count - 15 Minute Summary Report

MCRAE AVE @ RICHMOND RD

Survey Date: Thursday, April 20, 2017

Total Observed U-Turns

Northbound: 1 Southbound: 0
Eastbound: 1 Westbound: 1

MCRAE AVE

RICHMOND RD

Table with columns for Time Period, Northbound (LT, ST, RT, N TOT), Southbound (LT, ST, RT, S TOT, STR TOT), Eastbound (LT, ST, RT, E TOT), Westbound (LT, ST, RT, W TOT, STR TOT), and Grand Total. Rows represent 15-minute intervals from 07:00 to 18:00.

Note: U-Turns are included in Totals.

Comment:



# Transportation Services - Traffic Services

## Turning Movement Count - Cyclist Volume Report

**Work Order**  
**36957**

### MCRAE AVE @ RICHMOND RD

**Count Date:** Thursday, April 20, 2017

**Start Time:** 07:00

Time Period	MCRAE AVE			RICHMOND RD			Grand Total
	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	
07:00 08:00	1	1	2	12	4	16	18
08:00 09:00	2	1	3	7	8	15	18
09:00 10:00	2	0	2	4	6	10	12
11:30 12:30	0	1	1	3	1	4	5
12:30 13:30	0	0	0	1	0	1	1
15:00 16:00	0	2	2	5	5	10	12
16:00 17:00	2	1	3	8	10	18	21
17:00 18:00	2	4	6	4	17	21	27
<b>Total .....</b>	<b>9</b>	<b>10</b>	<b>19</b>	<b>44</b>	<b>51</b>	<b>95</b>	<b>114</b>

**Comment:**

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.



# Transportation Services - Traffic Services

W.O.  
36957

## Turning Movement Count - Heavy Vehicle Report

### MCRAE AVE @ RICHMOND RD

**Survey Date:** Thursday, April 20, 2017

Time Period	MCRAE AVE									RICHMOND RD									Grand Total
	Northbound			Southbound			S TOT	STR TOT	Eastbound			Westbound			W TOT	STR TOT			
	LT	ST	RT	N TOT	LT	ST			RT	LT	ST	RT	E TOT	LT			ST	RT	
07:00 08:00	0	0	0	0	10	0	2	12	12	1	10	0	11	1	7	10	18	29	41
08:00 09:00	1	0	1	2	14	0	1	15	17	0	10	0	10	1	6	13	20	30	47
09:00 10:00	0	1	0	1	9	1	3	13	14	4	8	1	13	0	7	7	14	27	41
11:30 12:30	1	0	3	4	7	1	2	10	14	4	18	1	23	2	14	7	23	46	60
12:30 13:30	2	0	0	2	8	0	1	9	11	0	8	0	8	0	14	10	24	32	43
15:00 16:00	0	0	1	1	9	0	3	12	13	0	8	0	8	0	5	6	11	19	32
16:00 17:00	0	1	0	1	11	0	0	11	12	1	0	1	2	1	4	7	12	14	26
17:00 18:00	0	0	0	0	9	0	0	9	9	0	2	0	2	0	4	7	11	13	22
<b>Sub Total</b>	<b>4</b>	<b>2</b>	<b>5</b>	<b>11</b>	<b>77</b>	<b>2</b>	<b>12</b>	<b>91</b>	<b>102</b>	<b>10</b>	<b>64</b>	<b>3</b>	<b>77</b>	<b>5</b>	<b>61</b>	<b>67</b>	<b>133</b>	<b>210</b>	<b>312</b>
<b>U-Turns (Heavy Vehicles)</b>				<b>0</b>				<b>0</b>	<b>0</b>				<b>0</b>				<b>0</b>	<b>0</b>	<b>0</b>
<b>Total</b>	<b>4</b>	<b>2</b>	<b>5</b>	<b>0</b>	<b>77</b>	<b>2</b>	<b>12</b>	<b>91</b>	<b>102</b>	<b>10</b>	<b>64</b>	<b>3</b>	<b>77</b>	<b>5</b>	<b>61</b>	<b>67</b>	<b>133</b>	<b>210</b>	<b>312</b>

Heavy Vehicles include Buses, Single-Unit Trucks and Articulated Trucks. Further, they ARE included in the Turning Movement Count Summary.



# Transportation Services - Traffic Services

Work Order

36957

## Turning Movement Count - Pedestrian Volume Report

### MCRAE AVE @ RICHMOND RD

Count Date: Thursday, April 20, 2017

Start Time: 07:00

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	7	1	8	5	3	8	16
07:15 07:30	2	6	8	5	2	7	15
07:30 07:45	8	7	15	7	9	16	31
07:45 08:00	17	8	25	16	14	30	55
<b>07:00 08:00</b>	<b>34</b>	<b>22</b>	<b>56</b>	<b>33</b>	<b>28</b>	<b>61</b>	<b>117</b>
08:00 08:15	14	12	26	5	13	18	44
08:15 08:30	10	11	21	5	8	13	34
08:30 08:45	9	9	18	3	9	12	30
08:45 09:00	12	17	29	9	21	30	59
<b>08:00 09:00</b>	<b>45</b>	<b>49</b>	<b>94</b>	<b>22</b>	<b>51</b>	<b>73</b>	<b>167</b>
09:00 09:15	19	4	23	2	8	10	33
09:15 09:30	12	6	18	2	18	20	38
09:30 09:45	13	19	32	3	14	17	49
09:45 10:00	13	25	38	0	20	20	58
<b>09:00 10:00</b>	<b>57</b>	<b>54</b>	<b>111</b>	<b>7</b>	<b>60</b>	<b>67</b>	<b>178</b>
11:30 11:45	42	19	61	9	31	40	101
11:45 12:00	21	20	41	11	27	38	79
12:00 12:15	19	25	44	7	24	31	75
12:15 12:30	21	34	55	10	34	44	99
<b>11:30 12:30</b>	<b>103</b>	<b>98</b>	<b>201</b>	<b>37</b>	<b>116</b>	<b>153</b>	<b>354</b>
12:30 12:45	42	31	73	12	34	46	119
12:45 13:00	37	17	54	9	29	38	92
13:00 13:15	21	18	39	8	32	40	79
13:15 13:30	23	20	43	11	26	37	80
<b>12:30 13:30</b>	<b>123</b>	<b>86</b>	<b>209</b>	<b>40</b>	<b>121</b>	<b>161</b>	<b>370</b>
15:00 15:15	16	20	36	6	23	29	65
15:15 15:30	43	24	67	20	33	53	120
15:30 15:45	27	15	42	6	22	28	70
15:45 16:00	30	26	56	11	36	47	103
<b>15:00 16:00</b>	<b>116</b>	<b>85</b>	<b>201</b>	<b>43</b>	<b>114</b>	<b>157</b>	<b>358</b>
16:00 16:15	26	25	51	10	42	52	103
16:15 16:30	32	21	53	9	41	50	103
16:30 16:45	39	25	64	14	36	50	114
16:45 17:00	30	29	59	12	23	35	94
<b>16:00 17:00</b>	<b>127</b>	<b>100</b>	<b>227</b>	<b>45</b>	<b>142</b>	<b>187</b>	<b>414</b>
17:00 17:15	35	28	63	24	33	57	120
17:15 17:30	41	26	67	36	40	76	143
17:30 17:45	28	30	58	34	27	61	119
17:45 18:00	46	23	69	24	24	48	117
<b>17:00 18:00</b>	<b>150</b>	<b>107</b>	<b>257</b>	<b>118</b>	<b>124</b>	<b>242</b>	<b>499</b>
<b>Total .....</b>	<b>755</b>	<b>601</b>	<b>1356</b>	<b>345</b>	<b>756</b>	<b>1101</b>	<b>2457</b>

Comment:

## Turning Movement Count - 15 Min U-Turn Total Report

### MCRAE AVE @ RICHMOND RD

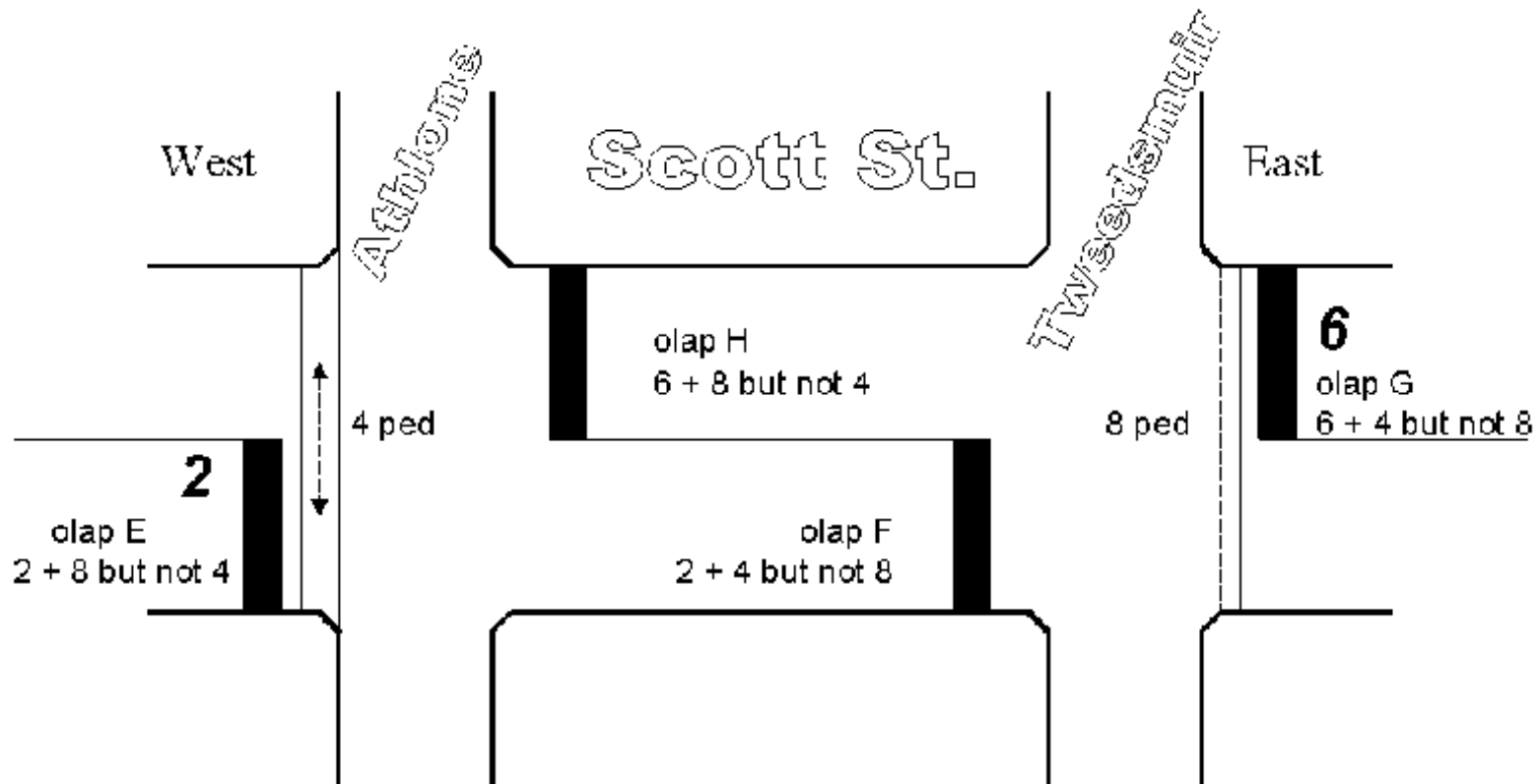
**Survey Date:** Thursday, April 20, 2017

Time Period		Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	1	0	1
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	1	1
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	1	0	0	0	1
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Total		1	0	1	1	3

**Scott St & Tweedsmuir/Athlone (int #5781)**

09-Jul-2019

The one MS3200 controller provides phasing for these 2 intersections (both are Intersection Ped Signals).



**NOTES:**



1. When the NS ped at Athlone (4 ped) is actuated, Olap F and G have a green display. Olap E and H have a red display.
2. When the NS ped at Tweedsmuir (8 ped) is actuated, Olap E and H have a green display. Olap F and G have a red display.
3. When both NS peds are actuated, all Olaps have a red display.

**TIMING:**

Please see excel file for Timing info.

# Traffic Signal Timing

City of Ottawa, Transportation Services Department

## Traffic Signal Operations Unit

<b>Intersection:</b>	<b>Main:</b> Scott	<b>Side:</b> Tweedsmuir/Athlone
<b>Controller:</b>	<b>MS-3200</b>	<b>TSD:</b> 5781
<b>Author:</b>	Matthew Anderson	<b>Date:</b> 11-Jul-2019

## Existing Timing Plans†

	Plan					Ped Minimum Time		
	AM Peak 1	Off Peak 2	PM Peak 3	Night 4	Weekend 5	Walk	DW	A+R
<b>Cycle</b>	FREE	FREE	FREE	FREE	FREE			
<b>Offset</b>	X	X	X	X	X			
EB Thru	min=30.8	min=30.8	min=30.8	min=30.8	min=30.8	-	-	3.3+2.5
WB Thru	min=30.8	min=30.8	min=30.8	min=30.8	min=30.8	-	-	3.3+2.5
West Walk	max=24.0	max=24.0	max=24.0	max=24.0	max=24.0	7	11	3.0+1.0
East Walk	max=24.0	max=24.0	max=24.0	max=24.0	max=24.0	7	11	3.0+1.0

### NOTE:

Please see attached PDF file for detailed signal phasing information.

## Schedule

### Weekday

Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
21:30	4

### Weekend

Time	Plan
0:15	4
6:30	2
11:00	5
19:30	2
22:00	4

## Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

◀.....▶ Pedestrian signal

Cost is \$57.63 (\$51 + HST)

# Traffic Signal Timing

City of Ottawa, Transportation Services Department

## Traffic Signal Operations Unit

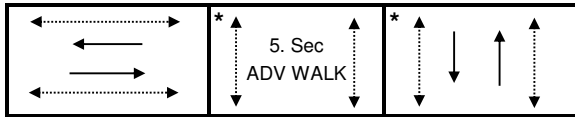
<b>Intersection:</b>	Main: Richmond	Side: McRae	
<b>Controller:</b>	MS-3200	<b>TSD:</b>	6589
<b>Author:</b>	Jean Nabolle	<b>Date:</b>	11-Jul-2019

### Existing Timing Plans<sup>†</sup>

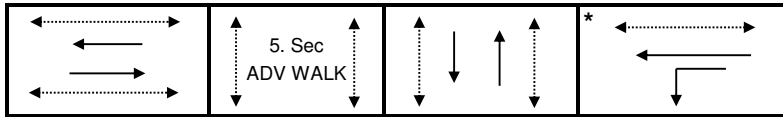
	Plan							Ped Minimum Time		
	AM Peak 1	Evening 2	PM Peak 3	Night 4	Weekend 5	AM Heavy 11	Off Peak 12	Walk	DW	A+R
<b>Cycle</b>	75	80	85	65	80	75	75			
<b>Offset</b>	X	X	1	X	X	16	X			
<b>EB Thru</b>	48	35	40	35	35	48	35	9	20	3.3+2.9
<b>WB Thru</b>	48	35	51	35	48	48	48	9	20	3.3+2.9
<b>NB Thru</b>	27	32	34	30	32	27	27	7	13	3.3+2.2
<b>SB Thru</b>	27	32	34	30	32	27	27	7	13	3.3+2.2
<b>WB Left</b>	-	13	11	-	13	-	13	-	-	3.3+2.8

### Phasing Sequence<sup>‡</sup>

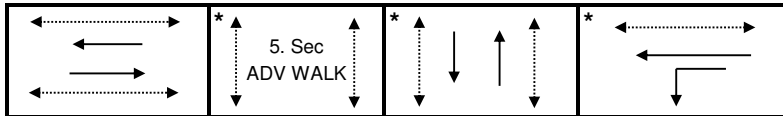
**Plans:** 1,4,11



**Plans:** 3,12



**Plans:** 2,5



### Schedule

Weekday		Saturday		Sunday	
Time	Plan	Time	Plan	Time	Plan
0:15	4	0:15	4	0:15	4
6:30	1	6:30	2	6:30	2
7:30	11	9:00	5	9:00	5
9:30	12	18:30	2	18:00	2
15:00	3	22:30	4	22:30	4
18:30	2				
22:30	4				

### Notes

- †: Time for each direction includes amber and all red intervals
- ‡: Start of first phase should be used as reference point for offset
- Asterisk (\*) Indicates actuated phase
- (fp): Fully Protected Left Turn
- ◄.....► Pedestrian signal

Cost is \$57.63 (\$51 + HST)

# Appendix C

Collision Data

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Classification Of Accident	Initial Impact Type	Road Surface Condition
2013-02-01	2013	1:56:00 PM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	03 - Rear end	01 - Dry
2013-06-15	2013	9:20:00 AM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	03 - Rear end	01 - Dry
2014-11-23	2014	1:30:00 PM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	03 - Rear end	01 - Dry
2015-02-05	2015	1:15:00 PM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	02 - Angle	02 - Wet
2016-09-02	2016	10:10:00 AM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	02 - Angle	01 - Dry
2016-08-08	2016	1:00:00 PM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	02 - Angle	01 - Dry
2017-06-05	2017	1:09:00 PM	MCRAE AVE @ SCOTT ST	01 - Clear	01 - Daylight	02 - Stop sign	02 - Non-fatal injury	02 - Angle	01 - Dry

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Classification Of Accident	Initial Impact Type	Road Surface Condition
2014-03-02	2014	4:00:00 PM	TWEEDSMUIR AVE @ SCOTT ST	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	02 - Wet
2015-06-21	2015	12:50:00 PM	TWEEDSMUIR AVE @ SCOTT ST	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	01 - Dry
2016-10-03	2016	8:03:00 AM	TWEEDSMUIR AVE @ SCOTT ST	01 - Clear	01 - Daylight	01 - Traffic signal	02 - Non-fatal injury	03 - Rear end	01 - Dry
2016-06-09	2016	12:15:00 PM	TWEEDSMUIR AVE @ SCOTT ST	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	02 - Angle	01 - Dry

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Classification Of Accident	Initial Impact Type	Road Surface Condition
2013-03-26	2013	3:27:00 PM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	05 - Turning movement	01 - Dry
2013-06-01	2013	6:12:00 PM	MCRAE AVE @ RICHMOND RD	02 - Rain	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	02 - Wet
2013-09-12	2013	6:00:00 PM	MCRAE AVE @ RICHMOND RD	02 - Rain	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	02 - Wet
2013-11-17	2013	5:16:00 PM	MCRAE AVE @ RICHMOND RD	01 - Clear	07 - Dark	01 - Traffic signal	02 - Non-fatal injury	07 - SMV other	02 - Wet
2014-08-26	2014	6:41:00 PM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	01 - Dry
2014-11-04	2014	5:46:00 PM	MCRAE AVE @ RICHMOND RD	03 - Snow	07 - Dark	01 - Traffic signal	03 - P.D. only	03 - Rear end	02 - Wet
2015-08-29	2015	8:15:00 AM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	01 - Dry
2015-12-12	2015	8:55:00 AM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	03 - P.D. only	02 - Angle	01 - Dry
2017-07-15	2017	1:28:00 PM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	02 - Non-fatal injury	07 - SMV other	01 - Dry
2017-01-30	2017	1:48:00 PM	MCRAE AVE @ RICHMOND RD	01 - Clear	01 - Daylight	01 - Traffic signal	02 - Non-fatal injury	07 - SMV other	01 - Dry
2017-12-23	2017	3:30:00 PM	MCRAE AVE @ RICHMOND RD	03 - Snow	01 - Daylight	01 - Traffic signal	03 - P.D. only	03 - Rear end	03 - Loose snow

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Classification Of Accident	Initial Impact Type	Road Surface Condition
2013-11-30	2013	9:19:00 AM	RICHMOND RD @ TWEEDSMUIR AVE	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	06 - SMV unattended vehicle	02 - Wet
2015-08-22	2015	2:58:00 PM	RICHMOND RD @ TWEEDSMUIR AVE	02 - Rain	01 - Daylight	02 - Stop sign	02 - Non-fatal injury	07 - SMV other	02 - Wet
2015-06-10	2015	12:17:00 PM	RICHMOND RD @ TWEEDSMUIR AVE	01 - Clear	01 - Daylight	02 - Stop sign	02 - Non-fatal injury	05 - Turning movement	01 - Dry
2015-03-03	2015	6:55:00 PM	RICHMOND RD @ TWEEDSMUIR AVE	03 - Snow	07 - Dark	02 - Stop sign	03 - P.D. only	02 - Angle	05 - Packed snow
2017-09-16	2017	6:42:00 PM	RICHMOND RD @ TWEEDSMUIR AVE	01 - Clear	01 - Daylight	02 - Stop sign	03 - P.D. only	02 - Angle	01 - Dry

LOCATION & GEOID	TOTAL_COLLISIONS	TOTAL_CYCLIST_COLLISIONS	TOTAL_PEDESTRIAN_COLLISIONS
MCRAE AVE @ SCOTT ST (0006868)	7	0	0
TWEEDSMUIR AVE @ SCOTT ST (0006371)	4	0	0
MCRAE AVE @ RICHMOND RD (0006867)	11	0	3
RICHMOND RD @ TWEEDSMUIR AVE (0006681)	5	1	0

# Appendix D

TDM Checklists

**TDM Measures Checklist:**  
*Residential Developments (multi-family, condominium or subdivision)*

<b>Legend</b>	
<b>BASIC</b>	The measure is generally feasible and effective, and in most cases would benefit the development and its users
<b>BETTER</b>	The measure could maximize support for users of sustainable modes, and optimize development performance
<b>★</b>	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

<b>TDM measures: Residential developments</b>		<b>Check if proposed &amp; add descriptions</b>
<b>1. TDM PROGRAM MANAGEMENT</b>		
<b>1.1 Program coordinator</b>		
<b>BASIC</b> ★	1.1.1 Designate an internal coordinator, or contract with an external coordinator	<input checked="" type="checkbox"/>
<b>1.2 Travel surveys</b>		
<b>BETTER</b>	1.2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	<input type="checkbox"/>
<b>2. WALKING AND CYCLING</b>		
<b>2.1 Information on walking/cycling routes &amp; destinations</b>		
<b>BASIC</b>	2.1.1 Display local area maps with walking/cycling access routes and key destinations at major entrances ( <i>multi-family, condominium</i> )	<input checked="" type="checkbox"/>
<b>2.2 Bicycle skills training</b>		
<b>BETTER</b>	2.2.1 Offer on-site cycling courses for residents, or subsidize off-site courses	<input type="checkbox"/>



TDM measures: <i>Residential developments</i>		Check if proposed & add descriptions
<b>3. TRANSIT</b>		
<b>3.1 Transit information</b>		
BASIC	3.1.1 Display relevant transit schedules and route maps at entrances ( <i>multi-family, condominium</i> )	<input checked="" type="checkbox"/>
BETTER	3.1.2 Provide real-time arrival information display at entrances ( <i>multi-family, condominium</i> )	<input type="checkbox"/>
<b>3.2 Transit fare incentives</b>		
BASIC ★	3.2.1 Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	<input type="checkbox"/>
BETTER	3.2.2 Offer at least one year of free monthly transit passes on residence purchase/move-in	<input type="checkbox"/>
<b>3.3 Enhanced public transit service</b>		
BETTER ★	3.3.1 Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels ( <i>subdivision</i> )	<input type="checkbox"/>
<b>3.4 Private transit service</b>		
BETTER	3.4.1 Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	<input type="checkbox"/>
<b>4. CARSHARING &amp; BIKESHARING</b>		
<b>4.1 Bikeshare stations &amp; memberships</b>		
BETTER	4.1.1 Contract with provider to install on-site bikeshare station ( <i>multi-family</i> )	<input type="checkbox"/>
BETTER	4.1.2 Provide residents with bikeshare memberships, either free or subsidized ( <i>multi-family</i> )	<input type="checkbox"/>
<b>4.2 Carshare vehicles &amp; memberships</b>		
BETTER	4.2.1 Contract with provider to install on-site carshare vehicles and promote their use by residents	<input checked="" type="checkbox"/>
BETTER	4.2.2 Provide residents with carshare memberships, either free or subsidized	<input type="checkbox"/>
<b>5. PARKING</b>		
<b>5.1 Priced parking</b>		
BASIC ★	5.1.1 Unbundle parking cost from purchase price ( <i>condominium</i> )	<input type="checkbox"/>
BASIC ★	5.1.2 Unbundle parking cost from monthly rent ( <i>multi-family</i> )	<input checked="" type="checkbox"/>

<b>TDM measures: <i>Residential developments</i></b>		<b>Check if proposed &amp; add descriptions</b>
<b>6. TDM MARKETING &amp; COMMUNICATIONS</b>		
<b>6.1 Multimodal travel information</b>		
<b>BASIC</b> ★	6.1.1 Provide a multimodal travel option information package to new residents	<input type="checkbox"/>
<b>6.2 Personalized trip planning</b>		
<b>BETTER</b> ★	6.2.1 Offer personalized trip planning to new residents	<input type="checkbox"/>

**TDM-Supportive Development Design and Infrastructure Checklist:**  
*Residential Developments (multi-family or condominium)*

<b>Legend</b>	
<b>REQUIRED</b>	The Official Plan or Zoning By-law provides related guidance that must be followed
<b>BASIC</b>	The measure is generally feasible and effective, and in most cases would benefit the development and its users
<b>BETTER</b>	The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
<b>1. WALKING &amp; CYCLING: ROUTES</b>		
<b>1.1 Building location &amp; access points</b>		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	<input checked="" type="checkbox"/>
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	<input checked="" type="checkbox"/>
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	<input checked="" type="checkbox"/>
<b>1.2 Facilities for walking &amp; cycling</b>		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations ( <i>see Official Plan policy 4.3.3</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible ( <i>see Official Plan policy 4.3.12</i> )	<input checked="" type="checkbox"/>

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks ( <i>see Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps ( <i>see Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians ( <i>see Official Plan policy 4.3.11</i> )	<input checked="" type="checkbox"/>
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	<input type="checkbox"/>
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	<input type="checkbox"/>
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	<input type="checkbox"/>
<b>1.3 Amenities for walking &amp; cycling</b>		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	<input checked="" type="checkbox"/>
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
<b>2. WALKING &amp; CYCLING: END-OF-TRIP FACILITIES</b>		
<b>2.1 Bicycle parking</b>		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	<input type="checkbox"/>
<b>2.2 Secure bicycle parking</b>		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multi-family residential developments	<input type="checkbox"/>
<b>2.3 Bicycle repair station</b>		
BETTER	2.3.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	<input checked="" type="checkbox"/>
<b>3. TRANSIT</b>		
<b>3.1 Customer amenities</b>		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	<input type="checkbox"/>
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	<input type="checkbox"/>
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	<input type="checkbox"/>

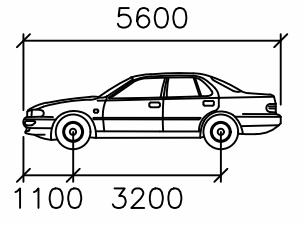
TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
<b>4. RIDESHARING</b>		
<b>4.1 Pick-up &amp; drop-off facilities</b>		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	<input checked="" type="checkbox"/>
<b>5. CARSHARING &amp; BIKESHARING</b>		
<b>5.1 Carshare parking spaces</b>		
BETTER	5.1.1 Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see <i>Zoning By-law Section 94</i> )	<input type="checkbox"/>
<b>5.2 Bikeshare station location</b>		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	<input type="checkbox"/>
<b>6. PARKING</b>		
<b>6.1 Number of parking spaces</b>		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	<input checked="" type="checkbox"/>
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	<input checked="" type="checkbox"/>
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see <i>Zoning By-law Section 104</i> )	<input type="checkbox"/>
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see <i>Zoning By-law Section 111</i> )	<input type="checkbox"/>
<b>6.2 Separate long-term &amp; short-term parking areas</b>		
BETTER	6.2.1 Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	<input checked="" type="checkbox"/>

# Appendix E

Turning Templates



Notes:



P  
 Width : 2000  
 Track : 2000  
 Lock to Lock Time : 6.0  
 Steering Angle : 35.9

REV:	DESCRIPTION:	BY:	DATE:

**CGH Transportation**  
 13 Markham Ave  
 Ottawa, ON  
 K2G 3Z1  
 (343) 999-9117

CLIENT: GWL Realty Advisors

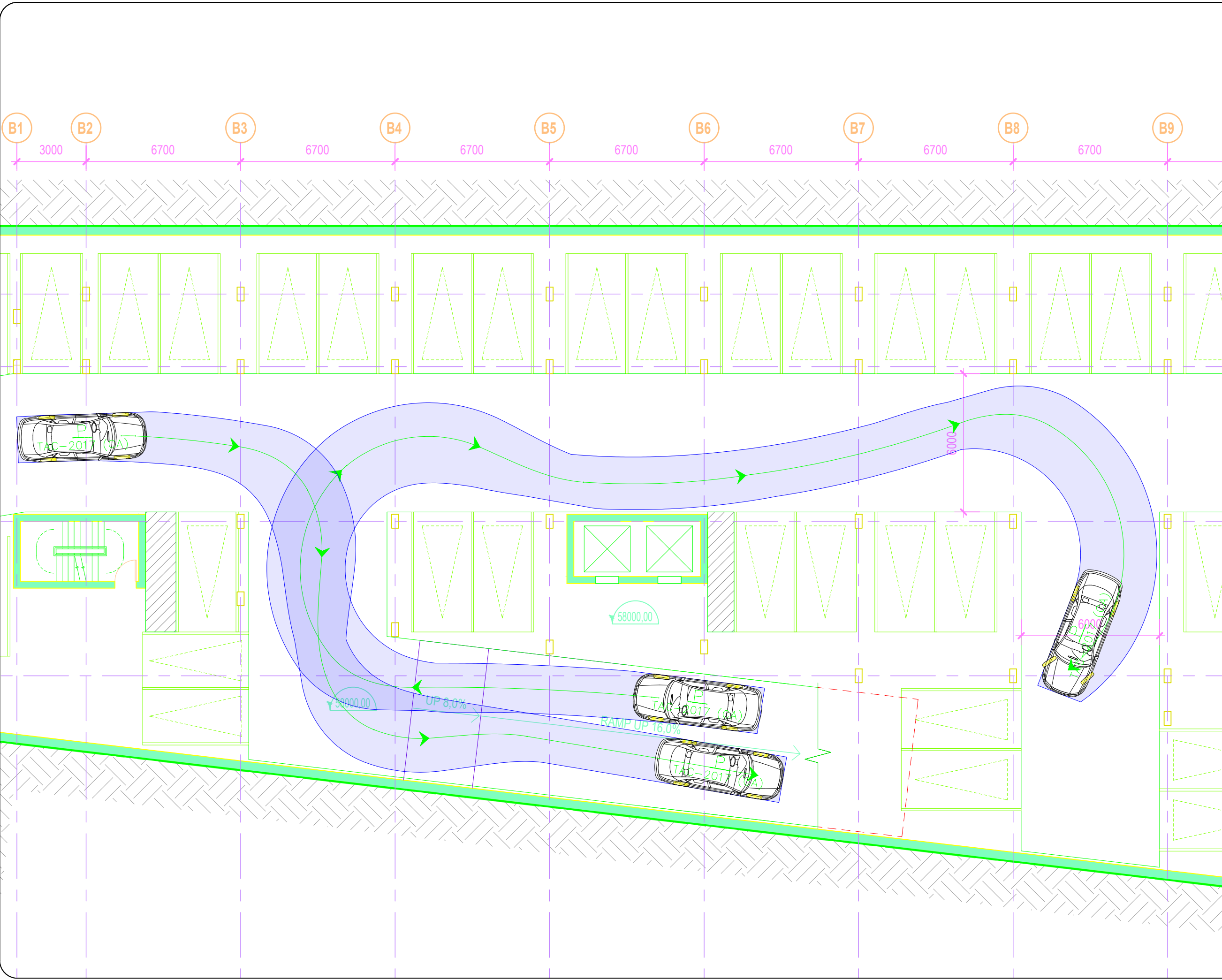
ARCHITECT: NEUF Architect(e)s

SITE: 320 McRae Avenue

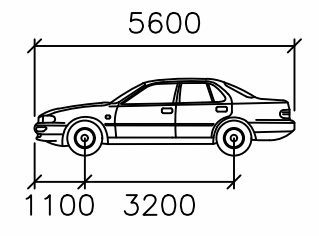
TITLE: P1 Corner Templates

SCALE AT A3:	DATE:	DRAWN:	CHECKED:
NTS	2020/01/31	JK	MC
PROJECT NO:	DRAWING NO:	REVISION:	
2019-29	002	-	





Notes:



P  
 Width : 2000  
 Track : 2000  
 Lock to Lock Time : 6.0  
 Steering Angle : 35.9

REV:	DESCRIPTION:	BY:	DATE:
STATUS:			



CGH Transportation  
 13 Markham Ave  
 Ottawa, ON  
 K2G 3Z1  
 (343) 999-9117

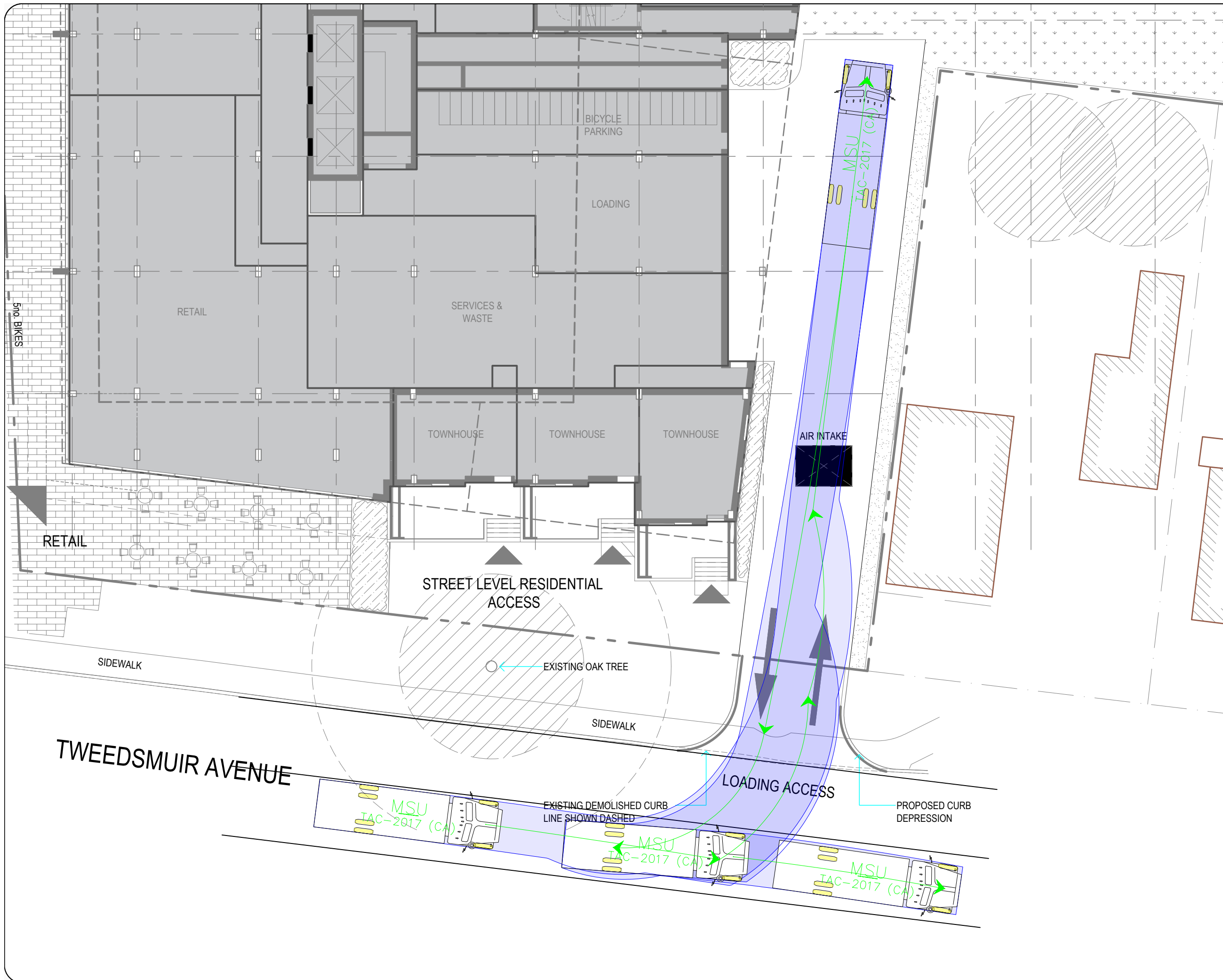
CLIENT: GWL Realty Advisors

ARCHITECT: NEUF Architect(e)s

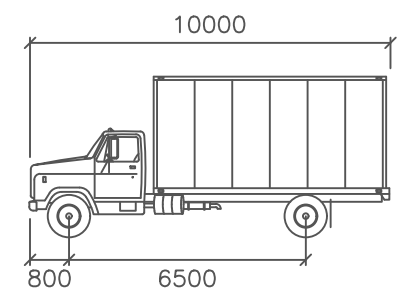
SITE:  
 320 McRae Avenue

TITLE:  
 P2 Corner Templates

SCALE AT A3: NTS	DATE: 20/01/31	DRAWN: JK	CHECKED: MC
PROJECT NO: 2019-29	DRAWING NO: 003	REVISION:	-



Notes:



MSU  
mm  
Width : 2600  
Track : 2600  
Lock to Lock Time : 6.0  
Steering Angle : 40.2

REV:	DESCRIPTION:	BY:	DATE:
STATUS:			



CGH Transportation  
13 Markham Ave  
Ottawa, ON  
K2G 3Z1  
(343) 999-9117

CLIENT: GWL Realty Advisors

ARCHITECT: NEUF Architect(e)s

SITE:  
320 McRae Avenue

TITLE:  
Garbage Collection

SCALE AT A3: NTS	DATE: 2020/01/31	DRAWN: JK	CHECKED: MC
PROJECT NO: 2019-29	DRAWING NO: 001	REVISION:	-

# Appendix F

MMLOS Worksheets

# Multi-Modal Level of Service - Segments Form

Consultant Scenario Comments	CGH Transportation	Project Date	2019-29
	All		20-01-2020

SEGMENTS		Segment	Tweedsmuir Ave 1	McRae Ave 2	Scott St 3
Pedestrian	Sidewalk Width	-	1.5 m	1.5 m	1.8 m
	Boulevard Width		> 2 m	< 0.5 m	< 0.5 m
	Avg Daily Curb Lane Traffic Volume		≤ 3000	≤ 3000	> 3000
	Operating Speed		> 50 to 60 km/h	> 50 to 60 km/h	> 50 to 60 km/h
	On-Street Parking		yes	yes	yes
	<b>Exposure to Traffic PLoS</b>		<b>C</b>	<b>F</b>	<b>D</b>
	Effective Sidewalk Width				
	Pedestrian Volume				
<b>Crowding PLoS</b>	-	-	-		
<b>Level of Service</b>	-	-	-		
Bicycle	Type of Cycling Facility	-	Mixed Traffic	Mixed Traffic	Parking beside Bike Lane
	Number of Travel Lanes		≤ 2 (no centreline)	≤ 2 (no centreline)	1 each direction
	Operating Speed		≥ 50 to 60 km/h	≥ 50 to 60 km/h	>50 to <70 km/h
	<b># of Lanes &amp; Operating Speed LoS</b>		<b>D</b>	<b>D</b>	<b>D</b>
	Bike Lane (+ Parking Lane) Width				≤ 4 m biking + parking width
	<b>Bike Lane Width LoS</b>		-	-	<b>C</b>
	Bike Lane Blockages				Frequent
	<b>Blockage LoS</b>		-	-	<b>C</b>
	Median Refuge Width (no median = < 1.8 m)				
	No. of Lanes at Unsignalized Crossing				
	Sidestreet Operating Speed				
	<b>Unsignalized Crossing - Lowest LoS</b>		-	-	-
<b>Level of Service</b>	-	-	-		
Transit	Facility Type	E		Mixed Traffic	Mixed Traffic
	Friction or Ratio Transit:Posted Speed			Vt/Vp ≥ 0.8	Vt/Vp ≤ 0.6
	<b>Level of Service</b>		-	<b>D</b>	<b>E</b>
Truck	Truck Lane Width	F		≤ 3.0 m	> 3.7 m
	Travel Lanes per Direction			1	1
	<b>Level of Service</b>		-	<b>F</b>	<b>B</b>
Auto	<b>Level of Service</b>	<b>Not Applicable</b>			

### Multi-Modal Level of Service - Intersections Form

Consultant Scenario Comments	CGH Transportation	Project Date	2019-29
	All Horizons-AM		22-01-2020

INTERSECTIONS		McRae Avenue and Richmond Road				Tweedsmuir Avenue and Scott Street			
Crossing Side		NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
Pedestrian	Lanes	0 - 2	3	4	5	0 - 2	0 - 2	0 - 2	
	Median	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	
	Conflicting Left Turns	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	
	Conflicting Right Turns	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	No right turn	Permissive or yield control	Permissive or yield control	
	Right Turns on Red (RTOR) ?	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR prohibited	
	Ped Signal Leading Interval?	No	No	Yes	Yes	No	No	Yes	
	Right Turn Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	
	Corner Radius	5-10m	10-15m	10-15m	10-15m	5-10m	5-10m	5-10m	
	Crosswalk Type	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	
	<b>PETSI Score</b>	<b>86</b>	<b>70</b>	<b>55</b>	<b>39</b>	<b>91</b>	<b>86</b>	<b>91</b>	
	<b>Ped. Exposure to Traffic LoS</b>	<b>B</b>	<b>C</b>	<b>D</b>	<b>E</b>	<b>A</b>	<b>B</b>	<b>A</b>	<b>-</b>
	Cycle Length								
Effective Walk Time									
<b>Average Pedestrian Delay</b>									
<b>Pedestrian Delay LoS</b>									
<b>Level of Service</b>	<b>B</b>	<b>C</b>	<b>D</b>	<b>E</b>	<b>A</b>	<b>B</b>	<b>A</b>	<b>-</b>	
	<b>E</b>				<b>B</b>				
<b>Approach From</b>		<b>NORTH</b>	<b>SOUTH</b>	<b>EAST</b>	<b>WEST</b>	<b>NORTH</b>	<b>SOUTH</b>	<b>EAST</b>	<b>WEST</b>
Bicycle	Bicycle Lane Arrangement on Approach	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic		Mixed Traffic	Curb Bike Lane, Cycletrack or MUP	
	Right Turn Lane Configuration	> 50 m	> 50 m	> 50 m	> 50 m		> 50 m	Not Applicable	
	Right Turning Speed	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h		≤ 25 km/h	Not Applicable	
	<b>Cyclist relative to RT motorists</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>	<b>Not Applicable</b>	
	<b>Separated or Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>-</b>	<b>Mixed Traffic</b>	<b>Separated</b>	
	Left Turn Approach	No lane crossed	No lane crossed	No lane crossed	No lane crossed		No lane crossed	No lane crossed	
	Operating Speed	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h		> 40 to ≤ 50 km/h	> 50 to < 60 km/h	
	<b>Left Turning Cyclist</b>	<b>B</b>	<b>B</b>	<b>C</b>	<b>C</b>	<b>-</b>	<b>B</b>	<b>C</b>	<b>-</b>
<b>Level of Service</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>	<b>C</b>	<b>-</b>	
	<b>F</b>				<b>F</b>				
Transit	Average Signal Delay	≤ 40 sec	≤ 20 sec	≤ 20 sec	≤ 20 sec			≤ 10 sec	≤ 20 sec
	<b>Level of Service</b>	<b>E</b>	<b>C</b>	<b>C</b>	<b>C</b>	<b>-</b>	<b>-</b>	<b>B</b>	<b>C</b>
	<b>E</b>				<b>C</b>				
Truck	Effective Corner Radius	10 - 15 m	10 - 15 m	10 - 15 m	10 - 15 m	< 10 m	< 10 m		< 10 m
	Number of Receiving Lanes on Departure from Intersection	1	≥ 2	1	1	1	1		1
<b>Level of Service</b>	<b>E</b>	<b>B</b>	<b>E</b>	<b>E</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>	
	<b>E</b>				<b>F</b>				
Auto	Volume to Capacity Ratio	0.61 - 0.70				0.0 - 0.60			
	<b>Level of Service</b>	<b>B</b>				<b>A</b>			

**Multi-Modal Level of Service - Intersections Form**

Consultant  
Scenario  
Comments

<b>CGH Transportation</b>
<b>All Horizons-PM</b>

Project  
Date

<b>2019-29</b>
<b>22-01-2020</b>

INTERSECTIONS		McRae Avenue and Richmond Road				Tweedsmuir Avenue and Scott Street			
Crossing Side		NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
<b>Pedestrian</b>	Lanes	0 - 2	3	4	5	0 - 2	0 - 2	0 - 2	
	Median	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	
	Conflicting Left Turns	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	
	Conflicting Right Turns	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	No right turn	Permissive or yield control	Permissive or yield control	
	Right Turns on Red (RTOR) ?	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR prohibited	
	Ped Signal Leading Interval?	No	No	Yes	Yes	No	No	Yes	
	Right Turn Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	
	Corner Radius	5-10m	10-15m	10-15m	10-15m	5-10m	5-10m	5-10m	
	Crosswalk Type	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	
	<b>PETSI Score</b>	<b>86</b>	<b>70</b>	<b>55</b>	<b>39</b>	<b>91</b>	<b>86</b>	<b>91</b>	
	<b>Ped. Exposure to Traffic LoS</b>	<b>B</b>	<b>C</b>	<b>D</b>	<b>E</b>	<b>A</b>	<b>B</b>	<b>A</b>	<b>-</b>
	Cycle Length								
Effective Walk Time									
<b>Average Pedestrian Delay</b>									
<b>Pedestrian Delay LoS</b>									
<b>Level of Service</b>	<b>B</b>	<b>C</b>	<b>D</b>	<b>E</b>	<b>A</b>	<b>B</b>	<b>A</b>	<b>-</b>	
	<b>E</b>				<b>B</b>				
Approach From		NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
<b>Bicycle</b>	Bicycle Lane Arrangement on Approach	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic		Mixed Traffic	Curb Bike Lane, Cycletrack or MUP	
	Right Turn Lane Configuration	> 50 m	> 50 m	> 50 m	> 50 m		> 50 m	Not Applicable	
	Right Turning Speed	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h		≤ 25 km/h	Not Applicable	
	<b>Cyclist relative to RT motorists</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>	<b>Not Applicable</b>	
	<b>Separated or Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>Mixed Traffic</b>	<b>-</b>	<b>Mixed Traffic</b>	<b>Separated</b>	
	Left Turn Approach	No lane crossed	No lane crossed	No lane crossed	No lane crossed		No lane crossed	No lane crossed	
	Operating Speed	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h		> 40 to ≤ 50 km/h	> 50 to < 60 km/h	
	<b>Left Turning Cyclist</b>	<b>B</b>	<b>B</b>	<b>C</b>	<b>C</b>	<b>-</b>	<b>B</b>	<b>C</b>	<b>-</b>
<b>Level of Service</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>	<b>C</b>	<b>-</b>	
	<b>F</b>				<b>F</b>				
<b>Transit</b>	Average Signal Delay	> 40 sec	≤ 20 sec	≤ 30 sec	≤ 30 sec			≤ 20 sec	≤ 20 sec
	<b>Level of Service</b>	<b>F</b>	<b>C</b>	<b>D</b>	<b>D</b>	<b>-</b>	<b>-</b>	<b>C</b>	<b>C</b>
	<b>F</b>				<b>C</b>				
<b>Truck</b>	Effective Corner Radius	10 - 15 m	10 - 15 m	10 - 15 m	10 - 15 m	< 10 m	< 10 m		< 10 m
	Number of Receiving Lanes on Departure from Intersection	1	≥ 2	1	1	1	1		1
	<b>Level of Service</b>	<b>E</b>	<b>B</b>	<b>E</b>	<b>E</b>	<b>F</b>	<b>F</b>	<b>-</b>	<b>F</b>
	<b>E</b>				<b>F</b>				
<b>Auto</b>	Volume to Capacity Ratio	0.81 - 0.90				0.0 - 0.60			
	<b>Level of Service</b>	<b>D</b>				<b>A</b>			

# Appendix G

Signal Warrants

McRae Avenue & Scott Street  
2022 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	613	85%	59%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	101	59%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	513	71%	17%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	13	17%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes,  $AHV = PM/2$  or  $(AM + PM) / 4$
4. T-intersection factor corrected, applies only to 1B



McRae Avenue & Scott Street  
2027 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	657	91%	59%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	100	59%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	557	77%	18%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	14	18%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes,  $AHV = PM/2$  or  $(AM + PM) / 4$
4. T-intersection factor corrected, applies only to 1B

Tweedsmuir Avenue and Richmond Road  
2022 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	545	76%	23%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	39	23%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	506	70%	11%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	9	11%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes,  $AHV = PM/2$  or  $(AM + PM) / 4$
4. T-intersection factor corrected, applies only to 1B

Tweedsmuir Avenue and Richmond Road  
2027 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	587	82%	24%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	40	24%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	547	76%	13%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	10	13%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4
4. T-intersection factor corrected, applies only to 1B

McRae Ave & Site Access #1  
2022 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	222	31%	30%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	50	30%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	188	26%	16%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	12	16%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes,  $AHV = PM/2$  or  $(AM + PM) / 4$
4. T-intersection factor corrected, applies only to 1B

McRae Ave & Site Access #1  
2027 FT

**Justification #7**

Justification	Description	Minimum Requirement		Minimum Requirement		Compliance		Signal	
		1 Lane Highway		2 or More Lanes		Sectional			Entire %
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%		
1. Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	195	27%	9%	No
	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	15	9%		
2. Delay to Cross Traffic	A. Vehicle volumes, major street (average hour)	480	720	600	900	185	26%	5%	No
	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	4	5%		

Notes

1. Refer to OTM Book 12, pg 88, Nov 2007
2. Lowest section percentage governs justification
3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4
4. T-intersection factor corrected, applies only to 1B

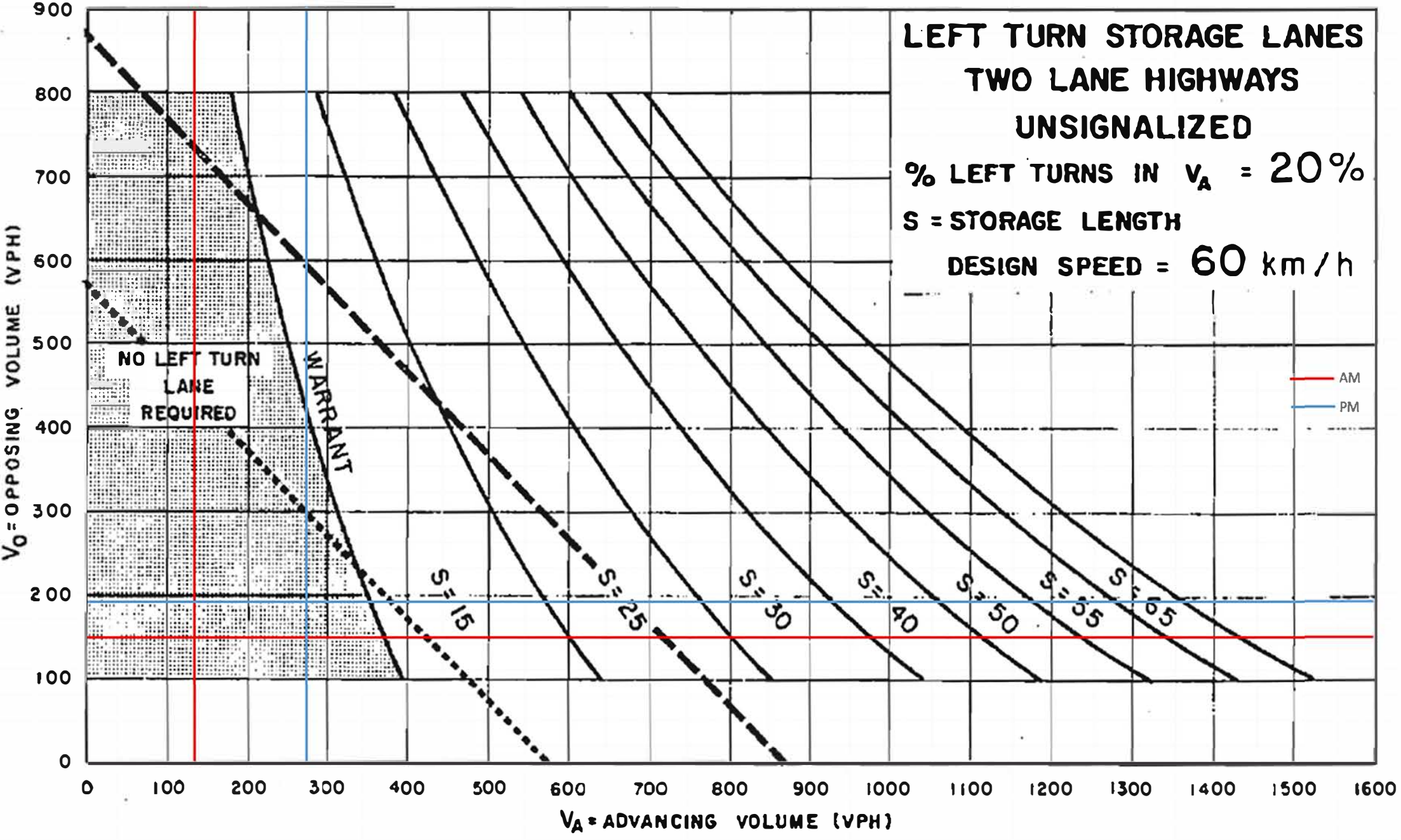
# Appendix H

Left-turn Lane Warrants

**2022 FT**

Design Speed	Northbound Left	EBL	EBT	EBR	WBL	WBT	WBR	Yes	NBL	NBT	NBR	SBL	SBT	SBR	%Left Turn	Volume Advancing	Volume Opposing
60	km/h																
	AM	29		54					19	114			142	10	14.3%	133	152
	PM	18		33					50	223			167	27	18.3%	273	194

**LEFT TURN STORAGE LANES  
TWO LANE HIGHWAYS  
UNSIGNALIZED**  
 % LEFT TURNS IN  $v_A = 20\%$   
 S = STORAGE LENGTH  
 DESIGN SPEED = 60 km/h

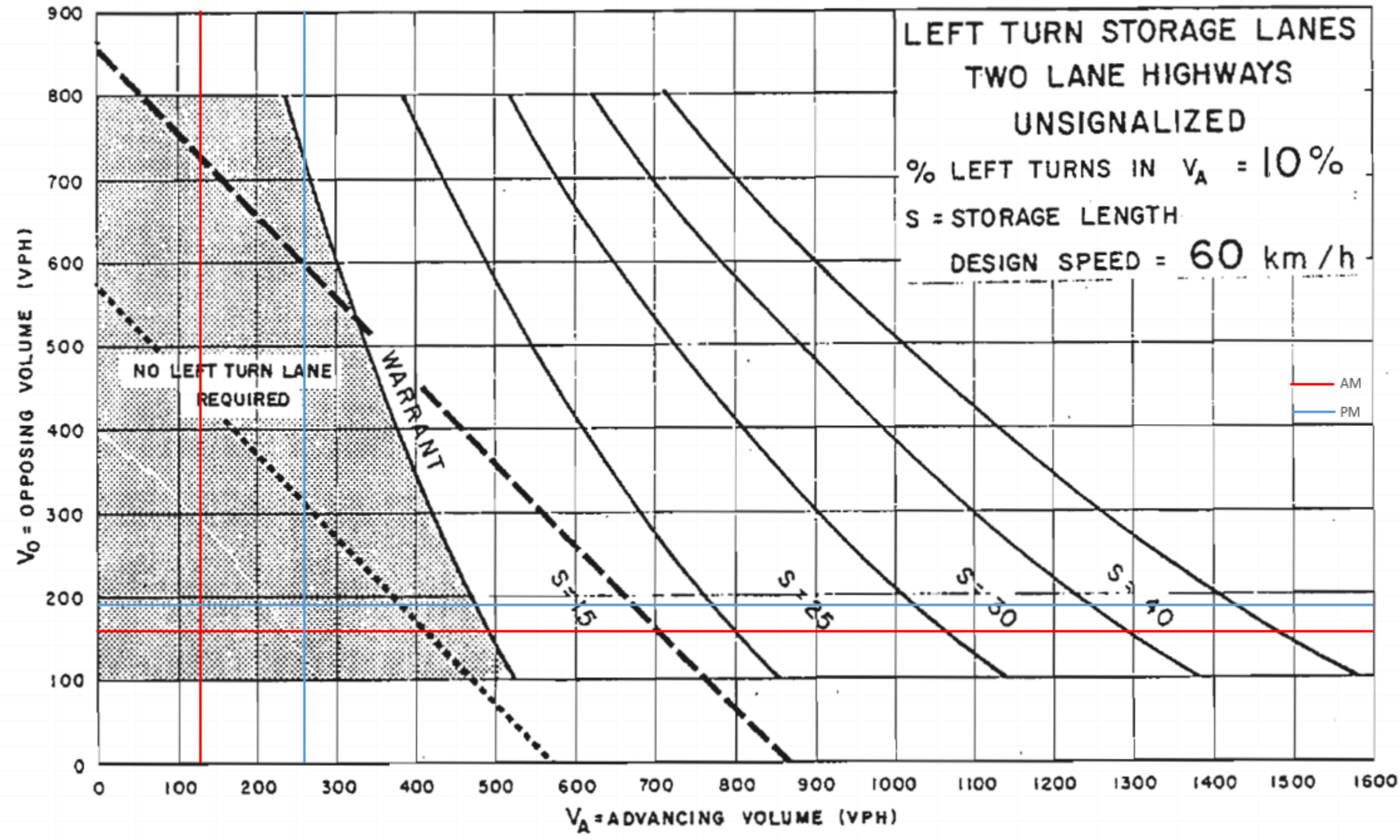




**2027 FT**

Design Speed	Northbound Left	EBL	EBT	EBR	WBL	WBT	WBR	Yes	NBL	NBT	NBR	SBL	SBT	SBR	%Left Turn	Volume Advancing	Volume Opposing
60	km/h								6	124			155	3	4.6%	130	158
	AM	8		16					6	124			155	3	4.6%	130	158
	PM	6		10					16	245			182	8	6.1%	261	190

LEFT TURN STORAGE LANES  
 TWO LANE HIGHWAYS  
 UNSIGNALIZED  
 % LEFT TURNS IN  $V_A = 10\%$   
 S = STORAGE LENGTH  
 DESIGN SPEED = 60 km/h



















# Appendix I

2019 Existing Conditions Synchro Worksheets

Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2019 Existing-AM  
320 McRae

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	475	3	10	263	5	3	0	34	6	0	0
Future Volume (vph)	0	475	3	10	263	5	3	0	34	6	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.999			0.997			0.875				
Fl <sub>t</sub> Protected					0.998			0.996			0.950	
Satd. Flow (prot)	0	1569	0	0	1534	0	0	1369	0	0	761	0
Fl <sub>t</sub> Permitted					0.998			0.996			0.950	
Satd. Flow (perm)	0	1569	0	0	1534	0	0	1369	0	0	761	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	528	3	11	292	6	3	0	38	7	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	531	0	0	309	0	0	41	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	39.5%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	475	3	10	263	5	3	0	34	6	0	0
Future Vol, veh/h	0	475	3	10	263	5	3	0	34	6	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	528	3	11	292	6	3	0	38	7	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	298	0	0	531	0	0	847	850	530	866	848	295
Stage 1	-	-	-	-	-	-	530	530	-	317	317	-
Stage 2	-	-	-	-	-	-	317	320	-	549	531	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	863	-	-	1036	-	-	282	211	549	189	212	562
Stage 1	-	-	-	-	-	-	533	396	-	528	510	-
Stage 2	-	-	-	-	-	-	694	509	-	380	396	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	863	-	-	1036	-	-	279	208	549	174	209	562
Mov Cap-2 Maneuver	-	-	-	-	-	-	279	208	-	174	209	-
Stage 1	-	-	-	-	-	-	533	396	-	528	503	-
Stage 2	-	-	-	-	-	-	685	502	-	354	396	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			12.7			26.5		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	509	863	-	-	1036	-	-	174
HCM Lane V/C Ratio	0.081	-	-	-	0.011	-	-	0.038
HCM Control Delay (s)	12.7	0	-	-	8.5	0	-	26.5
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	490	25	98	265	13	73
Future Volume (vph)	490	25	98	265	13	73
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.993			0.885		
Flt Protected				0.987	0.993	
Satd. Flow (prot)	1560	0	0	1395	1380	0
Flt Permitted				0.987	0.993	
Satd. Flow (perm)	1560	0	0	1395	1380	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	544	28	109	294	14	81
Shared Lane Traffic (%)						
Lane Group Flow (vph)	572	0	0	403	95	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	70.9%
	ICU Level of Service C
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	2.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	490	25	98	265	13	73
Future Vol, veh/h	490	25	98	265	13	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	544	28	109	294	14	81

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	572	0	1070
Stage 1	-	-	-	-	558
Stage 2	-	-	-	-	512
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1001	-	245
Stage 1	-	-	-	-	573
Stage 2	-	-	-	-	602
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1001	-	213
Mov Cap-2 Maneuver	-	-	-	-	213
Stage 1	-	-	-	-	573
Stage 2	-	-	-	-	524

Approach	EB	WB	NB
HCM Control Delay, s	0	2.4	15.7
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	432	-	-	1001	-
HCM Lane V/C Ratio	0.221	-	-	0.109	-
HCM Control Delay (s)	15.7	-	-	9	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	0.8	-	-	0.4	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2019 Existing-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	20	411	8	13	272	6	7	8	26	2	4	13
Future Volume (vph)	20	411	8	13	272	6	7	8	26	2	4	13
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.915			0.905	
Flt Protected		0.998			0.998			0.991			0.995	
Satd. Flow (prot)	0	1564	0	0	1567	1201	0	1424	0	0	1414	0
Flt Permitted		0.998			0.998			0.991			0.995	
Satd. Flow (perm)	0	1564	0	0	1567	1201	0	1424	0	0	1414	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	22	457	9	14	302	7	8	9	29	2	4	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	488	0	0	316	7	0	46	0	0	20	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	56.2%
ICU Level of Service	B
Analysis Period (min)	15



Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	20	411	8	13	272	6	7	8	26	2	4	13
Future Vol, veh/h	20	411	8	13	272	6	7	8	26	2	4	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	457	9	14	302	7	8	9	29	2	4	14

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	309	0	0	466	0	0	849	843	462	855	840	302
Stage 1	-	-	-	-	-	-	506	506	-	330	330	-
Stage 2	-	-	-	-	-	-	343	337	-	525	510	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1252	-	-	1095	-	-	281	300	600	278	302	738
Stage 1	-	-	-	-	-	-	549	540	-	683	646	-
Stage 2	-	-	-	-	-	-	672	641	-	536	538	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1252	-	-	1095	-	-	264	288	600	251	290	738
Mov Cap-2 Maneuver	-	-	-	-	-	-	264	288	-	251	290	-
Stage 1	-	-	-	-	-	-	536	527	-	667	636	-
Stage 2	-	-	-	-	-	-	644	631	-	490	525	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.4			14.6			12.8		
HCM LOS							B			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	420	1252	-	-	1095	-	-	483
HCM Lane V/C Ratio	0.108	0.018	-	-	0.013	-	-	0.044
HCM Control Delay (s)	14.6	7.9	0	-	8.3	0	-	12.8
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.4	0.1	-	-	0	-	-	0.1

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2019 Existing-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	91	354	41	7	271	97	23	9	16	85	21	35
Future Volume (vph)	91	354	41	7	271	97	23	9	16	85	21	35
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.984			0.960			0.904				0.966
Flt Protected	0.950			0.950			0.950					0.971
Satd. Flow (prot)	1492	1545	0	1492	1508	0	1492	1420	0	0	1473	0
Flt Permitted	0.507			0.486			0.671					0.799
Satd. Flow (perm)	796	1545	0	763	1508	0	1054	1420	0	0	1212	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		13			39			18				21
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	101	393	46	8	301	108	26	10	18	94	23	39
Shared Lane Traffic (%)												
Lane Group Flow (vph)	101	439	0	8	409	0	26	28	0	0	156	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2019 Existing-AM  
320 McRae

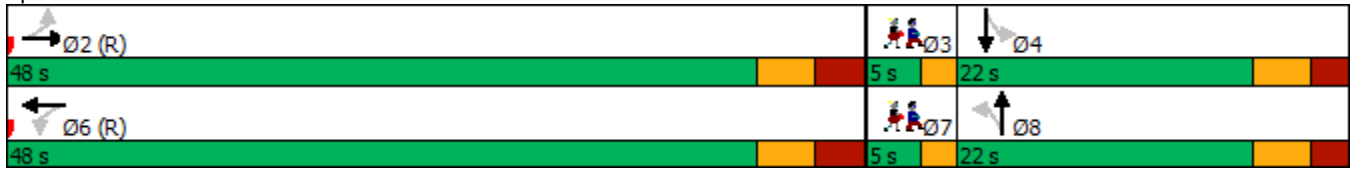


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.2	24.2		24.2	24.2		22.0	22.0		22.0	22.0	
Total Split (s)	48.0	48.0		48.0	48.0		22.0	22.0		22.0	22.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		29.3%	29.3%		29.3%	29.3%	
Maximum Green (s)	41.8	41.8		41.8	41.8		16.5	16.5		16.5	16.5	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.9	2.9		2.9	2.9		2.2	2.2		2.2	2.2	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.5	5.5				5.5
Lead/Lag							Lag	Lag		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0		7.0	7.0		5.5	5.5		5.5	5.5	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)	49.1	49.1		49.1	49.1		14.2	14.2				14.2
Actuated g/C Ratio	0.65	0.65		0.65	0.65		0.19	0.19				0.19
v/c Ratio	0.19	0.43		0.02	0.41		0.13	0.10				0.63
Control Delay	7.4	8.5		6.1	7.8		24.6	14.1				35.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	7.4	8.5		6.1	7.8		24.6	14.1				35.2
LOS	A	A		A	A		C	B				D
Approach Delay		8.3			7.7			19.1				35.2
Approach LOS		A			A			B				D
Queue Length 50th (m)	4.7	24.2		0.3	20.1		3.1	1.2				17.8
Queue Length 95th (m)	14.0	53.6		2.1	46.5		8.4	6.6				32.4
Internal Link Dist (m)		80.2			77.3			36.6				296.9
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	521	1016		499	1000		241	339				294
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.19	0.43		0.02	0.41		0.11	0.08				0.53

Intersection Summary

Area Type:	CBD
Cycle Length:	75
Actuated Cycle Length:	75
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.63
Intersection Signal Delay:	12.2
Intersection LOS:	B
Intersection Capacity Utilization:	60.5%
ICU Level of Service:	B
Analysis Period (min):	15

Splits and Phases: 4: McRae Ave & Richmond Rd



Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	7%	7%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2019 Existing-AM  
320 McRae



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Lane Configurations		↑	↑				
Traffic Volume (vph)	0	515	278	0	0	0	
Future Volume (vph)	0	515	278	0	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1745	1745	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	1745	1745	0	0	0	
Right Turn on Red				No		Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)		50	50		50		
Link Distance (m)		22.1	57.4		18.9		
Travel Time (s)		1.6	4.1		1.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	572	309	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	572	309	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		0.0		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		3.0	3.0		3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	25			15	25	15	
Number of Detectors		2	2				
Detector Template		Thru	Thru				
Leading Detector (m)		10.0	10.0				
Trailing Detector (m)		0.0	0.0				
Detector 1 Position(m)		0.0	0.0				
Detector 1 Size(m)		0.6	0.6				
Detector 1 Type		Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)		0.0	0.0				
Detector 1 Queue (s)		0.0	0.0				
Detector 1 Delay (s)		0.0	0.0				
Detector 2 Position(m)		9.4	9.4				
Detector 2 Size(m)		0.6	0.6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type		NA	NA				
Protected Phases		2	6			4	
Permitted Phases							
Detector Phase		2	6				
Switch Phase							
Minimum Initial (s)		5.0	5.0			10.0	

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2019 Existing-AM  
320 McRae

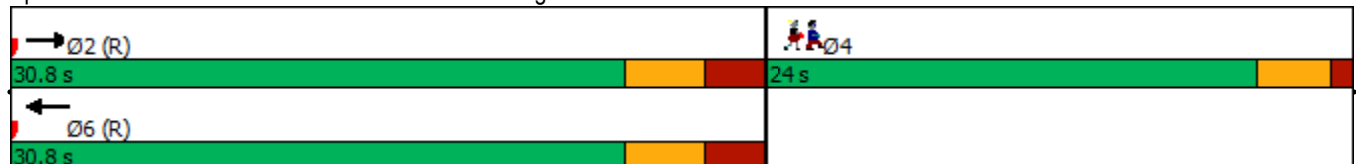


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Minimum Split (s)		23.8	23.8				22.0
Total Split (s)		30.8	30.8				24.0
Total Split (%)		56.2%	56.2%				44%
Maximum Green (s)		25.0	25.0				20.0
Yellow Time (s)		3.3	3.3				3.0
All-Red Time (s)		2.5	2.5				1.0
Lost Time Adjust (s)		0.0	0.0				
Total Lost Time (s)		5.8	5.8				
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0	3.0				3.0
Recall Mode		C-Max	C-Max				None
Walk Time (s)		7.0	7.0				7.0
Flash Dont Walk (s)		11.0	11.0				11.0
Pedestrian Calls (#/hr)		0	0				122
Act Effct Green (s)		32.6	32.6				
Actuated g/C Ratio		0.59	0.59				
v/c Ratio		0.55	0.30				
Control Delay		12.3	9.1				
Queue Delay		0.0	0.0				
Total Delay		12.3	9.1				
LOS		B	A				
Approach Delay		12.3	9.1				
Approach LOS		B	A				
Queue Length 50th (m)		39.2	17.3				
Queue Length 95th (m)		68.5	31.3				
Internal Link Dist (m)		0.1	33.4		0.1		
Turn Bay Length (m)							
Base Capacity (vph)		1036	1036				
Starvation Cap Reductn		0	0				
Spillback Cap Reductn		0	0				
Storage Cap Reductn		0	0				
Reduced v/c Ratio		0.55	0.30				

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 55  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.55  
 Intersection Signal Delay: 11.2  
 Intersection Capacity Utilization 33.4%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

















Splits and Phases: 6: Scott St & Pedestrian Crossing





Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2019 Existing-PM  
320 McRae

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	402	7	14	444	5	10	0	30	5	0	1
Future Volume (vph)	0	402	7	14	444	5	10	0	30	5	0	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.998			0.998			0.899			0.981	
Fl <sub>t</sub> Protected					0.998			0.988			0.959	
Satd. Flow (prot)	0	1567	0	0	1547	0	0	1395	0	0	754	0
Fl <sub>t</sub> Permitted					0.998			0.988			0.959	
Satd. Flow (perm)	0	1567	0	0	1547	0	0	1395	0	0	754	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	447	8	16	493	6	11	0	33	6	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	455	0	0	515	0	0	44	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	51.0%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	402	7	14	444	5	10	0	30	5	0	1
Future Vol, veh/h	0	402	7	14	444	5	10	0	30	5	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	447	8	16	493	6	11	0	33	6	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	499	0	0	455	0	0	980	982	451	996	983	496
Stage 1	-	-	-	-	-	-	451	451	-	528	528	-
Stage 2	-	-	-	-	-	-	529	531	-	468	455	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	705	-	-	1106	-	-	229	172	608	150	172	419
Stage 1	-	-	-	-	-	-	588	435	-	392	397	-
Stage 2	-	-	-	-	-	-	533	396	-	427	433	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	705	-	-	1106	-	-	225	169	608	140	169	419
Mov Cap-2 Maneuver	-	-	-	-	-	-	225	169	-	140	169	-
Stage 1	-	-	-	-	-	-	588	435	-	392	389	-
Stage 2	-	-	-	-	-	-	521	388	-	404	433	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			14.4			28.9		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	427	705	-	-	1106	-	-	157
HCM Lane V/C Ratio	0.104	-	-	-	0.014	-	-	0.042
HCM Control Delay (s)	14.4	0	-	-	8.3	0	-	28.9
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	399	39	95	430	30	168
Future Volume (vph)	399	39	95	430	30	168
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.988			0.885		
Flt Protected				0.991	0.993	
Satd. Flow (prot)	1552	0	0	1401	1380	0
Flt Permitted				0.991	0.993	
Satd. Flow (perm)	1552	0	0	1401	1380	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	443	43	106	478	33	187
Shared Lane Traffic (%)						
Lane Group Flow (vph)	486	0	0	584	220	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	84.2%
ICU Level of Service	E
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	4.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	399	39	95	430	30	168
Future Vol, veh/h	399	39	95	430	30	168
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	443	43	106	478	33	187

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	486	0	1155 465
Stage 1	-	-	-	-	465 -
Stage 2	-	-	-	-	690 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1077	-	218 597
Stage 1	-	-	-	-	632 -
Stage 2	-	-	-	-	498 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1077	-	189 597
Mov Cap-2 Maneuver	-	-	-	-	189 -
Stage 1	-	-	-	-	632 -
Stage 2	-	-	-	-	431 -

Approach	EB	WB	NB
HCM Control Delay, s	0	1.6	20.4
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	450	-	-	1077	-
HCM Lane V/C Ratio	0.489	-	-	0.098	-
HCM Control Delay (s)	20.4	-	-	8.7	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2.6	-	-	0.3	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2019 Existing-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	26	381	17	70	594	10	3	6	32	8	9	15
Future Volume (vph)	26	381	17	70	594	10	3	6	32	8	9	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.894			0.936	
Flt Protected		0.997			0.995			0.997			0.988	
Satd. Flow (prot)	0	1558	0	0	1563	1201	0	1400	0	0	1452	0
Flt Permitted		0.997			0.995			0.997			0.988	
Satd. Flow (perm)	0	1558	0	0	1563	1201	0	1400	0	0	1452	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	29	423	19	78	660	11	3	7	36	9	10	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	471	0	0	738	11	0	46	0	0	36	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	74.0%
ICU Level of Service	D
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	26	381	17	70	594	10	3	6	32	8	9	15
Future Vol, veh/h	26	381	17	70	594	10	3	6	32	8	9	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	29	423	19	78	660	11	3	7	36	9	10	17

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	671	0	0	442	0	0	1326	1318	433	1328	1316	660
Stage 1	-	-	-	-	-	-	491	491	-	816	816	-
Stage 2	-	-	-	-	-	-	835	827	-	512	500	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	919	-	-	1118	-	-	133	157	623	132	158	463
Stage 1	-	-	-	-	-	-	559	548	-	371	391	-
Stage 2	-	-	-	-	-	-	362	386	-	545	543	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	919	-	-	1118	-	-	107	134	623	106	135	463
Mov Cap-2 Maneuver	-	-	-	-	-	-	107	134	-	106	135	-
Stage 1	-	-	-	-	-	-	536	525	-	355	348	-
Stage 2	-	-	-	-	-	-	301	343	-	486	520	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0.9			17.6			29.4		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	330	919	-	-	1118	-	-	183
HCM Lane V/C Ratio	0.138	0.031	-	-	0.07	-	-	0.194
HCM Control Delay (s)	17.6	9	0	-	8.5	0	-	29.4
HCM Lane LOS	C	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0.2	-	-	0.7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2019 Existing-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	71	272	74	45	590	138	69	56	47	83	36	117
Future Volume (vph)	71	272	74	45	590	138	69	56	47	83	36	117
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.968			0.972			0.932				0.933
Flt Protected	0.950			0.950			0.950					0.983
Satd. Flow (prot)	1492	1520	0	1492	1527	0	1492	1464	0	0	1440	0
Flt Permitted	0.284			0.431			0.487					0.840
Satd. Flow (perm)	446	1520	0	677	1527	0	765	1464	0	0	1231	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		20			22			50				59
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	79	302	82	50	656	153	77	62	52	92	40	130
Shared Lane Traffic (%)												
Lane Group Flow (vph)	79	384	0	50	809	0	77	114	0	0	262	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		2		1	6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7



Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2019 Existing-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		1	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		9.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	40.0	40.0		11.0	51.0		29.0	29.0		29.0	29.0	
Total Split (%)	47.1%	47.1%		12.9%	60.0%		34.1%	34.1%		34.1%	34.1%	
Maximum Green (s)	35.5	35.5		6.5	46.5		24.5	24.5		24.5	24.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5				4.5
Lead/Lag	Lag	Lag		Lead			Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0		0	0	
Act Effct Green (s)	49.6	49.6		56.4	56.4		19.6	19.6				19.6
Actuated g/C Ratio	0.58	0.58		0.66	0.66		0.23	0.23				0.23
v/c Ratio	0.30	0.43		0.10	0.79		0.44	0.30				0.80
Control Delay	18.4	14.5		7.3	19.8		33.6	16.3				40.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	18.4	14.5		7.3	19.8		33.6	16.3				40.6
LOS	B	B		A	B		C	B				D
Approach Delay		15.1			19.1			23.3				40.6
Approach LOS		B			B			C				D
Queue Length 50th (m)	6.8	34.2		2.5	79.7		10.7	8.4				31.3
Queue Length 95th (m)	22.3	72.6		8.2	#195.0		20.5	18.4				50.9
Internal Link Dist (m)		80.2			77.3			36.6				296.9
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	260	894		514	1021		227	471				407
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.30	0.43		0.10	0.79		0.34	0.24				0.64

Intersection Summary

Area Type:	CBD
Cycle Length:	85
Actuated Cycle Length:	85
Offset:	1 (1%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.80
Intersection Signal Delay:	21.7
Intersection LOS:	C
Intersection Capacity Utilization:	84.8%
ICU Level of Service:	E
Analysis Period (min):	15

Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

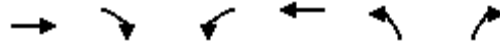
# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 4: McRae Ave & Richmond Rd



Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2019 Existing-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	↑			↑			
Traffic Volume (vph)	437	0	0	463	0	0	
Future Volume (vph)	437	0	0	463	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1745	0	0	1745	0	0	
Flt Permitted							
Satd. Flow (perm)	1745	0	0	1745	0	0	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)	50			50	50		
Link Distance (m)	22.1			57.4	22.8		
Travel Time (s)	1.6			4.1	1.6		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	486	0	0	514	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	486	0	0	514	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(m)	0.0			0.0	0.0		
Link Offset(m)	0.0			0.0	0.0		
Crosswalk Width(m)	3.0			3.0	3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)		15	25		25	15	
Number of Detectors	2			2			
Detector Template	Thru			Thru			
Leading Detector (m)	10.0			10.0			
Trailing Detector (m)	0.0			0.0			
Detector 1 Position(m)	0.0			0.0			
Detector 1 Size(m)	0.6			0.6			
Detector 1 Type	Cl+Ex			Cl+Ex			
Detector 1 Channel							
Detector 1 Extend (s)	0.0			0.0			
Detector 1 Queue (s)	0.0			0.0			
Detector 1 Delay (s)	0.0			0.0			
Detector 2 Position(m)	9.4			9.4			
Detector 2 Size(m)	0.6			0.6			
Detector 2 Type	Cl+Ex			Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA			NA			
Protected Phases	2			6		4	
Permitted Phases							
Detector Phase	2			6			
Switch Phase							
Minimum Initial (s)	5.0			5.0		10.0	

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2019 Existing-PM  
320 McRae

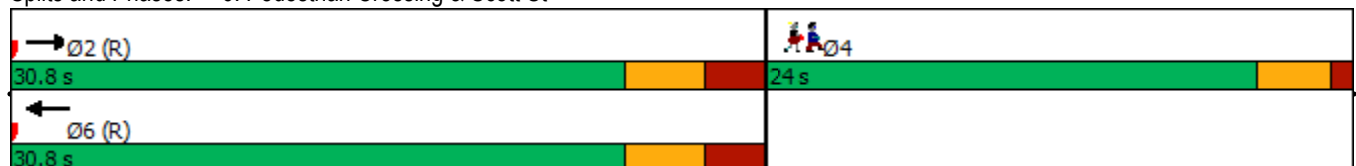


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Minimum Split (s)	23.8			23.8			22.0
Total Split (s)	30.8			30.8			24.0
Total Split (%)	56.2%			56.2%			44%
Maximum Green (s)	25.0			25.0			20.0
Yellow Time (s)	3.3			3.3			3.0
All-Red Time (s)	2.5			2.5			1.0
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.8			5.8			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0			3.0			3.0
Recall Mode	C-Max			C-Max			None
Walk Time (s)	7.0			7.0			7.0
Flash Dont Walk (s)	11.0			11.0			11.0
Pedestrian Calls (#/hr)	0			0			187
Act Effct Green (s)	32.6			32.6			
Actuated g/C Ratio	0.59			0.59			
v/c Ratio	0.47			0.50			
Control Delay	11.0			11.4			
Queue Delay	0.0			0.0			
Total Delay	11.0			11.4			
LOS	B			B			
Approach Delay	11.0			11.4			
Approach LOS	B			B			
Queue Length 50th (m)	31.1			33.6			
Queue Length 95th (m)	54.2			58.3			
Internal Link Dist (m)	0.1			33.4	0.1		
Turn Bay Length (m)							
Base Capacity (vph)	1036			1036			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.47			0.50			

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 50  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.50  
 Intersection Signal Delay: 11.2  
 Intersection LOS: B  
 Intersection Capacity Utilization 30.6%  
 ICU Level of Service A  
 Analysis Period (min) 15

Splits and Phases: 6: Pedestrian Crossing & Scott St



















# Appendix J

2022 Future Background Synchro Worksheets

Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2022 FB-AM  
320 McRae

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	512	3	11	287	5	3	0	36	6	0	0
Future Volume (vph)	0	512	3	11	287	5	3	0	36	6	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.999			0.998			0.874				
Fl <sub>t</sub> Protected					0.998			0.997			0.950	
Satd. Flow (prot)	0	1569	0	0	1538	0	0	1369	0	0	761	0
Fl <sub>t</sub> Permitted					0.998			0.997			0.950	
Satd. Flow (perm)	0	1569	0	0	1538	0	0	1369	0	0	761	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	569	3	12	319	6	3	0	40	7	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	572	0	0	337	0	0	43	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	41.8%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	512	3	11	287	5	3	0	36	6	0	0
Future Vol, veh/h	0	512	3	11	287	5	3	0	36	6	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	569	3	12	319	6	3	0	40	7	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	325	0	0	572	0	0	917	920	571	937	918	322
Stage 1	-	-	-	-	-	-	571	571	-	346	346	-
Stage 2	-	-	-	-	-	-	346	349	-	591	572	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	840	-	-	1001	-	-	253	190	520	167	190	540
Stage 1	-	-	-	-	-	-	506	377	-	507	493	-
Stage 2	-	-	-	-	-	-	670	491	-	358	376	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	840	-	-	1001	-	-	250	187	520	152	187	540
Mov Cap-2 Maneuver	-	-	-	-	-	-	250	187	-	152	187	-
Stage 1	-	-	-	-	-	-	506	377	-	507	486	-
Stage 2	-	-	-	-	-	-	660	484	-	330	376	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			13.2			29.8		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	480	840	-	-	1001	-	-	152
HCM Lane V/C Ratio	0.09	-	-	-	0.012	-	-	0.044
HCM Control Delay (s)	13.2	0	-	-	8.6	0	-	29.8
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1



Lanes, Volumes, Timings  
2: McRae Ave & Scott St

2022 FB-AM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	527	28	115	289	16	109
Future Volume (vph)	527	28	115	289	16	109
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.993			0.882		
Flt Protected				0.986	0.994	
Satd. Flow (prot)	1560	0	0	1394	1377	0
Flt Permitted				0.986	0.994	
Satd. Flow (perm)	1560	0	0	1394	1377	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	586	31	128	321	18	121
Shared Lane Traffic (%)						
Lane Group Flow (vph)	617	0	0	449	139	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	78.8%
ICU Level of Service	D
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	3.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	527	28	115	289	16	109
Future Vol, veh/h	527	28	115	289	16	109
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	586	31	128	321	18	121

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	617	0	1179 602
Stage 1	-	-	-	-	602 -
Stage 2	-	-	-	-	577 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	963	-	211 500
Stage 1	-	-	-	-	547 -
Stage 2	-	-	-	-	562 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	963	-	177 500
Mov Cap-2 Maneuver	-	-	-	-	177 -
Stage 1	-	-	-	-	547 -
Stage 2	-	-	-	-	471 -

Approach	EB	WB	NB
HCM Control Delay, s	0	2.6	18.5
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	405	-	-	963	-
HCM Lane V/C Ratio	0.343	-	-	0.133	-
HCM Control Delay (s)	18.5	-	-	9.3	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	1.5	-	-	0.5	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2022 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	21	445	8	14	294	6	7	8	28	2	4	14
Future Volume (vph)	21	445	8	14	294	6	7	8	28	2	4	14
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.913			0.902	
Flt Protected		0.998			0.998			0.992			0.995	
Satd. Flow (prot)	0	1564	0	0	1567	1201	0	1422	0	0	1410	0
Flt Permitted		0.998			0.998			0.992			0.995	
Satd. Flow (perm)	0	1564	0	0	1567	1201	0	1422	0	0	1410	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	23	494	9	16	327	7	8	9	31	2	4	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	526	0	0	343	7	0	48	0	0	22	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	59.4%
ICU Level of Service	B
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	21	445	8	14	294	6	7	8	28	2	4	14
Future Vol, veh/h	21	445	8	14	294	6	7	8	28	2	4	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	23	494	9	16	327	7	8	9	31	2	4	16

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	334	0	0	503	0	0	918	911	499	924	908	327
Stage 1	-	-	-	-	-	-	545	545	-	359	359	-
Stage 2	-	-	-	-	-	-	373	366	-	565	549	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1225	-	-	1061	-	-	252	274	572	250	275	714
Stage 1	-	-	-	-	-	-	523	519	-	659	627	-
Stage 2	-	-	-	-	-	-	648	623	-	510	516	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1225	-	-	1061	-	-	235	262	572	223	263	714
Mov Cap-2 Maneuver	-	-	-	-	-	-	235	262	-	223	263	-
Stage 1	-	-	-	-	-	-	509	506	-	642	615	-
Stage 2	-	-	-	-	-	-	617	611	-	461	503	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.4			15.4			13.3		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	394	1225	-	-	1061	-	-	457
HCM Lane V/C Ratio	0.121	0.019	-	-	0.015	-	-	0.049
HCM Control Delay (s)	15.4	8	0	-	8.4	0	-	13.3
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.4	0.1	-	-	0	-	-	0.2

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	106	379	43	8	301	126	24	10	17	97	22	43
Future Volume (vph)	106	379	43	8	301	126	24	10	17	97	22	43
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.985			0.956			0.905				0.964
Flt Protected	0.950			0.950			0.950					0.971
Satd. Flow (prot)	1492	1547	0	1492	1501	0	1492	1421	0	0	1470	0
Flt Permitted	0.456			0.459			0.651					0.799
Satd. Flow (perm)	716	1547	0	721	1501	0	1022	1421	0	0	1210	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			45			19				22
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	118	421	48	9	334	140	27	11	19	108	24	48
Shared Lane Traffic (%)												
Lane Group Flow (vph)	118	469	0	9	474	0	27	30	0	0	180	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

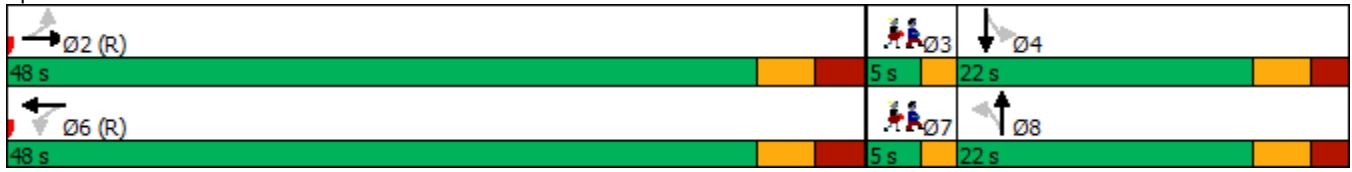
2022 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.2	24.2		24.2	24.2		22.0	22.0		22.0	22.0	
Total Split (s)	48.0	48.0		48.0	48.0		22.0	22.0		22.0	22.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		29.3%	29.3%		29.3%	29.3%	
Maximum Green (s)	41.8	41.8		41.8	41.8		16.5	16.5		16.5	16.5	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.9	2.9		2.9	2.9		2.2	2.2		2.2	2.2	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.5	5.5				5.5
Lead/Lag							Lag	Lag		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0		7.0	7.0		5.5	5.5		5.5	5.5	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)	47.8	47.8		47.8	47.8		15.5	15.5				15.5
Actuated g/C Ratio	0.64	0.64		0.64	0.64		0.21	0.21				0.21
v/c Ratio	0.26	0.47		0.02	0.49		0.13	0.10				0.67
Control Delay	9.1	9.8		6.9	9.4		23.4	13.3				36.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	9.1	9.8		6.9	9.4		23.4	13.3				36.0
LOS	A	A		A	A		C	B				D
Approach Delay		9.7			9.4			18.1				36.0
Approach LOS		A			A			B				D
Queue Length 50th (m)	6.3	28.9		0.4	27.0		3.2	1.3				20.7
Queue Length 95th (m)	18.0	61.9		2.4	60.5		8.4	6.7				36.8
Internal Link Dist (m)		80.2			77.3			36.6				296.9
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	456	990		459	973		242	352				304
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.26	0.47		0.02	0.49		0.11	0.09				0.59

Intersection Summary	
Area Type:	CBD
Cycle Length:	75
Actuated Cycle Length:	75
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.67
Intersection Signal Delay:	13.6
Intersection LOS:	B
Intersection Capacity Utilization:	66.8%
ICU Level of Service:	C
Analysis Period (min):	15

Splits and Phases: 4: McRae Ave & Richmond Rd





Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	7%	7%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2022 FB-AM  
320 McRae



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Lane Configurations		↑	↑				
Traffic Volume (vph)	0	554	303	0	0	0	
Future Volume (vph)	0	554	303	0	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1745	1745	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	1745	1745	0	0	0	
Right Turn on Red				No		Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)		50	50		50		
Link Distance (m)		22.1	57.4		18.9		
Travel Time (s)		1.6	4.1		1.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	616	337	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	616	337	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		0.0		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		3.0	3.0		3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	25			15	25	15	
Number of Detectors		2	2				
Detector Template		Thru	Thru				
Leading Detector (m)		10.0	10.0				
Trailing Detector (m)		0.0	0.0				
Detector 1 Position(m)		0.0	0.0				
Detector 1 Size(m)		0.6	0.6				
Detector 1 Type		Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)		0.0	0.0				
Detector 1 Queue (s)		0.0	0.0				
Detector 1 Delay (s)		0.0	0.0				
Detector 2 Position(m)		9.4	9.4				
Detector 2 Size(m)		0.6	0.6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type		NA	NA				
Protected Phases		2	6			4	
Permitted Phases							
Detector Phase		2	6				
Switch Phase							
Minimum Initial (s)		5.0	5.0			10.0	

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2022 FB-AM  
320 McRae

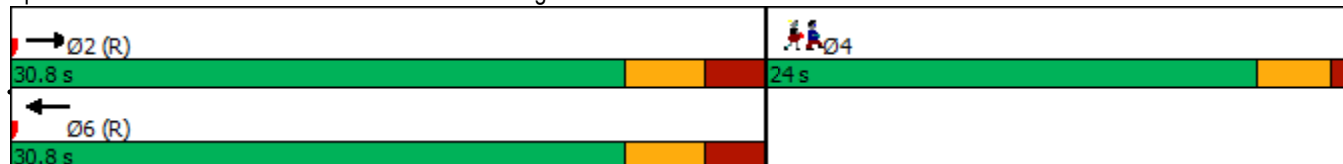


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Minimum Split (s)		23.8	23.8				22.0
Total Split (s)		30.8	30.8				24.0
Total Split (%)		56.2%	56.2%				44%
Maximum Green (s)		25.0	25.0				20.0
Yellow Time (s)		3.3	3.3				3.0
All-Red Time (s)		2.5	2.5				1.0
Lost Time Adjust (s)		0.0	0.0				
Total Lost Time (s)		5.8	5.8				
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0	3.0				3.0
Recall Mode		C-Max	C-Max				None
Walk Time (s)		7.0	7.0				7.0
Flash Dont Walk (s)		11.0	11.0				11.0
Pedestrian Calls (#/hr)		0	0				122
Act Effct Green (s)		32.6	32.6				
Actuated g/C Ratio		0.59	0.59				
v/c Ratio		0.59	0.33				
Control Delay		13.3	9.3				
Queue Delay		0.0	0.0				
Total Delay		13.3	9.3				
LOS		B	A				
Approach Delay		13.3	9.3				
Approach LOS		B	A				
Queue Length 50th (m)		43.9	19.3				
Queue Length 95th (m)		76.9	34.6				
Internal Link Dist (m)		0.1	33.4		0.1		
Turn Bay Length (m)							
Base Capacity (vph)		1036	1036				
Starvation Cap Reductn		0	0				
Spillback Cap Reductn		0	0				
Storage Cap Reductn		0	0				
Reduced v/c Ratio		0.59	0.33				

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 60  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.59  
 Intersection Signal Delay: 11.9  
 Intersection Capacity Utilization 35.6%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

Splits and Phases: 6: Scott St & Pedestrian Crossing



Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2022 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Future Volume (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.998			0.999			0.900			0.977	
Fl <sub>t</sub> Protected					0.998			0.987			0.960	
Satd. Flow (prot)	0	1567	0	0	1551	0	0	1395	0	0	751	0
Fl <sub>t</sub> Permitted					0.998			0.987			0.960	
Satd. Flow (perm)	0	1567	0	0	1551	0	0	1395	0	0	751	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	441	0	0	498	0	0	43	0	0	6	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	54.1%
Analysis Period (min)	15
	ICU Level of Service A

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	434	7	15	478	5	11	0	32	5	0	1
Future Vol, veh/h	0	434	7	15	478	5	11	0	32	5	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	434	7	15	478	5	11	0	32	5	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	483	0	0	441	0	0	949	951	438	965	952	481
Stage 1	-	-	-	-	-	-	438	438	-	511	511	-
Stage 2	-	-	-	-	-	-	511	513	-	454	441	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	716	-	-	1119	-	-	240	181	619	159	180	428
Stage 1	-	-	-	-	-	-	597	442	-	402	405	-
Stage 2	-	-	-	-	-	-	545	404	-	435	441	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	716	-	-	1119	-	-	236	178	619	149	177	428
Mov Cap-2 Maneuver	-	-	-	-	-	-	236	178	-	149	177	-
Stage 1	-	-	-	-	-	-	597	442	-	402	398	-
Stage 2	-	-	-	-	-	-	534	397	-	413	441	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			14.1			27.4		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	437	716	-	-	1119	-	-	167
HCM Lane V/C Ratio	0.098	-	-	-	0.013	-	-	0.036
HCM Control Delay (s)	14.1	0	-	-	8.3	0	-	27.4
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	428	43	120	463	35	195
Future Volume (vph)	428	43	120	463	35	195
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.988			0.886		
Flt Protected				0.990	0.992	
Satd. Flow (prot)	1552	0	0	1399	1380	0
Flt Permitted				0.990	0.992	
Satd. Flow (perm)	1552	0	0	1399	1380	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)	0			0		
Adj. Flow (vph)	428	43	120	463	35	195
Shared Lane Traffic (%)						
Lane Group Flow (vph)	471	0	0	583	230	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	92.2%
ICU Level of Service	F
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	4.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	428	43	120	463	35	195
Future Vol, veh/h	428	43	120	463	35	195
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	428	43	120	463	35	195

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	471	0	1153
Stage 1	-	-	-	-	450
Stage 2	-	-	-	-	703
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1091	-	218
Stage 1	-	-	-	-	642
Stage 2	-	-	-	-	491
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1091	-	186
Mov Cap-2 Maneuver	-	-	-	-	186
Stage 1	-	-	-	-	642
Stage 2	-	-	-	-	418

Approach	EB	WB	NB
HCM Control Delay, s	0	1.8	20.9
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	452	-	-	1091	-
HCM Lane V/C Ratio	0.509	-	-	0.11	-
HCM Control Delay (s)	20.9	-	-	8.7	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2.8	-	-	0.4	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2022 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	28	409	18	74	641	11	3	6	34	8	10	16
Future Volume (vph)	28	409	18	74	641	11	3	6	34	8	10	16
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.893			0.936	
Flt Protected		0.997			0.995			0.997			0.988	
Satd. Flow (prot)	0	1558	0	0	1563	1201	0	1398	0	0	1452	0
Flt Permitted		0.997			0.995			0.997			0.988	
Satd. Flow (perm)	0	1558	0	0	1563	1201	0	1398	0	0	1452	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	28	409	18	74	641	11	3	6	34	8	10	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	455	0	0	715	11	0	43	0	0	34	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	78.5%
ICU Level of Service	D
Analysis Period (min)	15



Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	28	409	18	74	641	11	3	6	34	8	10	16
Future Vol, veh/h	28	409	18	74	641	11	3	6	34	8	10	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	28	409	18	74	641	11	3	6	34	8	10	16

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	652	0	0	427	0	0	1282	1274	418	1283	1272	641
Stage 1	-	-	-	-	-	-	474	474	-	789	789	-
Stage 2	-	-	-	-	-	-	808	800	-	494	483	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	935	-	-	1132	-	-	142	167	635	142	168	475
Stage 1	-	-	-	-	-	-	571	558	-	384	402	-
Stage 2	-	-	-	-	-	-	375	397	-	557	553	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	935	-	-	1132	-	-	116	144	635	116	145	475
Mov Cap-2 Maneuver	-	-	-	-	-	-	116	144	-	116	145	-
Stage 1	-	-	-	-	-	-	549	536	-	369	361	-
Stage 2	-	-	-	-	-	-	316	356	-	501	531	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0.9			16.5			26.9		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	355	935	-	-	1132	-	-	198
HCM Lane V/C Ratio	0.121	0.03	-	-	0.065	-	-	0.172
HCM Control Delay (s)	16.5	9	0	-	8.4	0	-	26.9
HCM Lane LOS	C	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.4	0.1	-	-	0.2	-	-	0.6

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	82	301	78	47	636	160	73	60	50	101	39	136
Future Volume (vph)	82	301	78	47	636	160	73	60	50	101	39	136
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.969			0.970			0.932				0.933
Flt Protected	0.950			0.950			0.950					0.982
Satd. Flow (prot)	1492	1522	0	1492	1523	0	1492	1464	0	0	1439	0
Flt Permitted	0.281			0.429			0.492					0.835
Satd. Flow (perm)	441	1522	0	674	1523	0	773	1464	0	0	1224	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19			24			50				58
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0						
Adj. Flow (vph)	82	301	78	47	636	160	73	60	50	101	39	136
Shared Lane Traffic (%)												
Lane Group Flow (vph)	82	379	0	47	796	0	73	110	0	0	276	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		2		1	6			8				4

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		1	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		9.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	40.0	40.0		11.0	51.0		29.0	29.0		29.0	29.0	
Total Split (%)	47.1%	47.1%		12.9%	60.0%		34.1%	34.1%		34.1%	34.1%	
Maximum Green (s)	35.5	35.5		6.5	46.5		24.5	24.5		24.5	24.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5				4.5
Lead/Lag	Lag	Lag		Lead			Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0		0	0	
Act Effct Green (s)	48.4	48.4		55.1	55.1		20.9	20.9				20.9
Actuated g/C Ratio	0.57	0.57		0.65	0.65		0.25	0.25				0.25
v/c Ratio	0.33	0.43		0.09	0.80		0.39	0.28				0.80
Control Delay	19.7	15.0		7.9	20.9		30.4	15.1				40.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	19.7	15.0		7.9	20.9		30.4	15.1				40.4
LOS	B	B		A	C		C	B				D
Approach Delay		15.9			20.2			21.2				40.4
Approach LOS		B			C			C				D
Queue Length 50th (m)	7.5	35.2		2.5	82.1		9.9	7.6				33.4
Queue Length 95th (m)	23.3	71.3		8.0	#192.9		19.2	17.5				54.0
Internal Link Dist (m)		80.2			77.3			36.6				296.9
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	251	875		500	996		232	475				409
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.33	0.43		0.09	0.80		0.31	0.23				0.67

Intersection Summary

Area Type:	CBD
Cycle Length:	85
Actuated Cycle Length:	85
Offset:	1 (1%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.80
Intersection Signal Delay:	22.3
Intersection LOS:	C
Intersection Capacity Utilization:	92.6%
ICU Level of Service:	F
Analysis Period (min):	15

Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 4: McRae Ave & Richmond Rd



Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

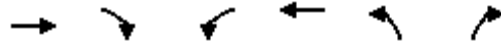
2022 FB-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	↑			↑			
Traffic Volume (vph)	471	0	0	498	0	0	
Future Volume (vph)	471	0	0	498	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1745	0	0	1745	0	0	
Flt Permitted							
Satd. Flow (perm)	1745	0	0	1745	0	0	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)	50			50	50		
Link Distance (m)	22.1			57.4	22.8		
Travel Time (s)	1.6			4.1	1.6		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	471	0	0	498	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	471	0	0	498	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(m)	0.0			0.0	0.0		
Link Offset(m)	0.0			0.0	0.0		
Crosswalk Width(m)	3.0			3.0	3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)		15	25		25	15	
Number of Detectors	2			2			
Detector Template	Thru			Thru			
Leading Detector (m)	10.0			10.0			
Trailing Detector (m)	0.0			0.0			
Detector 1 Position(m)	0.0			0.0			
Detector 1 Size(m)	0.6			0.6			
Detector 1 Type	Cl+Ex			Cl+Ex			
Detector 1 Channel							
Detector 1 Extend (s)	0.0			0.0			
Detector 1 Queue (s)	0.0			0.0			
Detector 1 Delay (s)	0.0			0.0			
Detector 2 Position(m)	9.4			9.4			
Detector 2 Size(m)	0.6			0.6			
Detector 2 Type	Cl+Ex			Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA			NA			
Protected Phases	2			6		4	
Permitted Phases							
Detector Phase	2			6			
Switch Phase							
Minimum Initial (s)	5.0			5.0		10.0	

Lanes, Volumes, Timings  
 6: Pedestrian Crossing & Scott St

2022 FB-PM  
 320 McRae

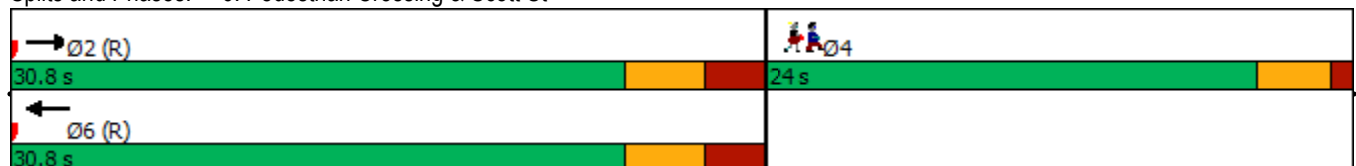


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Minimum Split (s)	23.8			23.8			22.0
Total Split (s)	30.8			30.8			24.0
Total Split (%)	56.2%			56.2%			44%
Maximum Green (s)	25.0			25.0			20.0
Yellow Time (s)	3.3			3.3			3.0
All-Red Time (s)	2.5			2.5			1.0
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.8			5.8			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0			3.0			3.0
Recall Mode	C-Max			C-Max			None
Walk Time (s)	7.0			7.0			7.0
Flash Dont Walk (s)	11.0			11.0			11.0
Pedestrian Calls (#/hr)	0			0			187
Act Effct Green (s)	32.6			32.6			
Actuated g/C Ratio	0.59			0.59			
v/c Ratio	0.45			0.48			
Control Delay	10.8			11.1			
Queue Delay	0.0			0.0			
Total Delay	10.8			11.1			
LOS	B			B			
Approach Delay	10.8			11.1			
Approach LOS	B			B			
Queue Length 50th (m)	29.7			32.1			
Queue Length 95th (m)	51.9			56.0			
Internal Link Dist (m)	0.1			33.4	0.1		
Turn Bay Length (m)							
Base Capacity (vph)	1036			1036			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.45			0.48			

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 50  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.48  
 Intersection Signal Delay: 11.0 Intersection LOS: B  
 Intersection Capacity Utilization 32.5% ICU Level of Service A  
 Analysis Period (min) 15

Splits and Phases: 6: Pedestrian Crossing & Scott St





# Appendix K

2027 Future Background Synchro Worksheets

Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2027 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	565	4	12	316	6	4	0	40	7	0	0
Future Volume (vph)	0	565	4	12	316	6	4	0	40	7	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.999			0.997			0.876				
Fl <sub>t</sub> Protected					0.998			0.996			0.950	
Satd. Flow (prot)	0	1569	0	0	1535	0	0	1370	0	0	761	0
Fl <sub>t</sub> Permitted					0.998			0.996			0.950	
Satd. Flow (perm)	0	1569	0	0	1535	0	0	1370	0	0	761	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	628	4	13	351	7	4	0	44	8	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	632	0	0	371	0	0	48	0	0	8	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	45.2%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	565	4	12	316	6	4	0	40	7	0	0
Future Vol, veh/h	0	565	4	12	316	6	4	0	40	7	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	628	4	13	351	7	4	0	44	8	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	358	0	0	632	0	0	1011	1014	630	1033	1013	355
Stage 1	-	-	-	-	-	-	630	630	-	381	381	-
Stage 2	-	-	-	-	-	-	381	384	-	652	632	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	813	-	-	951	-	-	218	164	482	141	164	515
Stage 1	-	-	-	-	-	-	470	351	-	483	473	-
Stage 2	-	-	-	-	-	-	641	472	-	328	350	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	813	-	-	951	-	-	215	161	482	126	161	515
Mov Cap-2 Maneuver	-	-	-	-	-	-	215	161	-	126	161	-
Stage 1	-	-	-	-	-	-	470	351	-	483	465	-
Stage 2	-	-	-	-	-	-	630	464	-	298	350	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.3	14.4	35.4
HCM LOS			B	E

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	433	813	-	-	951	-	-	126
HCM Lane V/C Ratio	0.113	-	-	-	0.014	-	-	0.062
HCM Control Delay (s)	14.4	0	-	-	8.8	0	-	35.4
HCM Lane LOS	B	A	-	-	A	A	-	E
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2

Lanes, Volumes, Timings  
2: McRae Ave & Scott St

2027 FB-AM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	581	30	126	318	17	118
Future Volume (vph)	581	30	126	318	17	118
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.993			0.882		
Flt Protected				0.986	0.994	
Satd. Flow (prot)	1560	0	0	1394	1377	0
Flt Permitted				0.986	0.994	
Satd. Flow (perm)	1560	0	0	1394	1377	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	646	33	140	353	19	131
Shared Lane Traffic (%)						
Lane Group Flow (vph)	679	0	0	493	150	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	85.4%
ICU Level of Service	E
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	3.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	581	30	126	318	17	118
Future Vol, veh/h	581	30	126	318	17	118
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	646	33	140	353	19	131

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	679	0	1296 663
Stage 1	-	-	-	-	663 -
Stage 2	-	-	-	-	633 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	913	-	179 461
Stage 1	-	-	-	-	512 -
Stage 2	-	-	-	-	529 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	913	-	145 461
Mov Cap-2 Maneuver	-	-	-	-	145 -
Stage 1	-	-	-	-	512 -
Stage 2	-	-	-	-	428 -

Approach	EB	WB	NB
HCM Control Delay, s	0	2.7	21.8
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	362	-	-	913	-
HCM Lane V/C Ratio	0.414	-	-	0.153	-
HCM Control Delay (s)	21.8	-	-	9.7	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2	-	-	0.5	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2027 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	23	491	9	15	324	7	8	9	30	2	5	15
Future Volume (vph)	23	491	9	15	324	7	8	9	30	2	5	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.914			0.908	
Flt Protected		0.998			0.998			0.991			0.996	
Satd. Flow (prot)	0	1564	0	0	1567	1201	0	1423	0	0	1420	0
Flt Permitted		0.998			0.998			0.991			0.996	
Satd. Flow (perm)	0	1564	0	0	1567	1201	0	1423	0	0	1420	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	26	546	10	17	360	8	9	10	33	2	6	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	582	0	0	377	8	0	52	0	0	25	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	64.9%
	ICU Level of Service C
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	23	491	9	15	324	7	8	9	30	2	5	15
Future Vol, veh/h	23	491	9	15	324	7	8	9	30	2	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	26	546	10	17	360	8	9	10	33	2	6	17

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	368	0	0	556	0	0	1013	1005	551	1019	1002	360
Stage 1	-	-	-	-	-	-	603	603	-	394	394	-
Stage 2	-	-	-	-	-	-	410	402	-	625	608	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1191	-	-	1015	-	-	217	241	534	215	242	684
Stage 1	-	-	-	-	-	-	486	488	-	631	605	-
Stage 2	-	-	-	-	-	-	619	600	-	473	486	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1191	-	-	1015	-	-	199	228	534	187	229	684
Mov Cap-2 Maneuver	-	-	-	-	-	-	199	228	-	187	229	-
Stage 1	-	-	-	-	-	-	470	472	-	611	592	-
Stage 2	-	-	-	-	-	-	586	587	-	420	470	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.4			17.2			14.5		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	346	1191	-	-	1015	-	-	404
HCM Lane V/C Ratio	0.151	0.021	-	-	0.016	-	-	0.061
HCM Control Delay (s)	17.2	8.1	0	-	8.6	0	-	14.5
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0.1	-	-	0.2

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FB-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	116	418	48	9	331	136	27	11	18	106	24	46
Future Volume (vph)	116	418	48	9	331	136	27	11	18	106	24	46
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.985			0.956			0.906				0.965
Flt Protected	0.950			0.950			0.950					0.971
Satd. Flow (prot)	1492	1547	0	1492	1501	0	1492	1423	0	0	1472	0
Flt Permitted	0.421			0.422			0.640					0.797
Satd. Flow (perm)	661	1547	0	663	1501	0	1005	1423	0	0	1208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			44			20				22
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	129	464	53	10	368	151	30	12	20	118	27	51
Shared Lane Traffic (%)												
Lane Group Flow (vph)	129	517	0	10	519	0	30	32	0	0	196	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8				4



Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FB-AM  
320 McRae

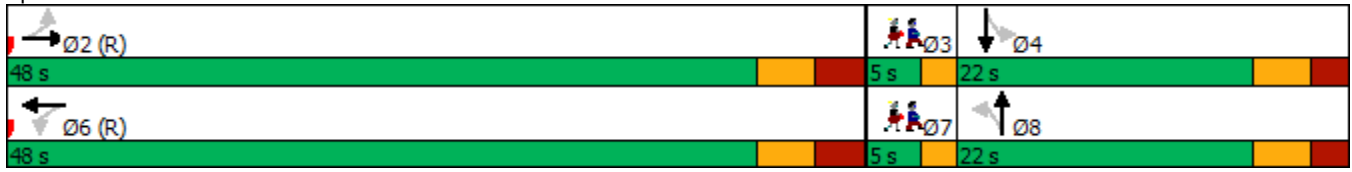


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2				6		8				4	
Detector Phase	2	2			6	6	8	8			4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	10.0	10.0			10.0	10.0
Minimum Split (s)	24.2	24.2			24.2	24.2	22.0	22.0			22.0	22.0
Total Split (s)	48.0	48.0			48.0	48.0	22.0	22.0			22.0	22.0
Total Split (%)	64.0%	64.0%			64.0%	64.0%	29.3%	29.3%			29.3%	29.3%
Maximum Green (s)	41.8	41.8			41.8	41.8	16.5	16.5			16.5	16.5
Yellow Time (s)	3.3	3.3			3.3	3.3	3.3	3.3			3.3	3.3
All-Red Time (s)	2.9	2.9			2.9	2.9	2.2	2.2			2.2	2.2
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				0.0
Total Lost Time (s)	6.2	6.2			6.2	6.2	5.5	5.5				5.5
Lead/Lag							Lag	Lag			Lag	Lag
Lead-Lag Optimize?							Yes	Yes			Yes	Yes
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0			3.0	3.0
Recall Mode	C-Max	C-Max			C-Max	C-Max	None	None			None	None
Walk Time (s)	7.0	7.0			7.0	7.0	5.5	5.5			5.5	5.5
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0			11.0	11.0
Pedestrian Calls (#/hr)	0	0			0	0	0	0			0	0
Act Effct Green (s)	47.1	47.1			47.1	47.1	16.2	16.2				16.2
Actuated g/C Ratio	0.63	0.63			0.63	0.63	0.22	0.22				0.22
v/c Ratio	0.31	0.53			0.02	0.54	0.14	0.10				0.71
Control Delay	10.4	11.1			7.2	10.8	23.0	13.0				37.2
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0				0.0
Total Delay	10.4	11.1			7.2	10.8	23.0	13.0				37.2
LOS	B	B			A	B	C	B				D
Approach Delay	11.0				10.7		17.9				37.2	
Approach LOS	B				B		B				D	
Queue Length 50th (m)	7.6	35.5			0.5	33.4	3.4	1.3			22.6	
Queue Length 95th (m)	20.5	71.6			2.5	69.8	9.1	7.0			40.3	
Internal Link Dist (m)	80.2				77.3		36.6				296.9	
Turn Bay Length (m)	45.0				60.0							
Base Capacity (vph)	414	975			416	958	243	358			308	
Starvation Cap Reductn	0	0			0	0	0	0			0	
Spillback Cap Reductn	0	0			0	0	0	0			0	
Storage Cap Reductn	0	0			0	0	0	0			0	
Reduced v/c Ratio	0.31	0.53			0.02	0.54	0.12	0.09			0.64	

Intersection Summary

Area Type:	CBD
Cycle Length:	75
Actuated Cycle Length:	75
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.71
Intersection Signal Delay:	14.8
Intersection LOS:	B
Intersection Capacity Utilization:	70.9%
ICU Level of Service:	C
Analysis Period (min):	15

Splits and Phases: 4: McRae Ave & Richmond Rd



Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	7%	7%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

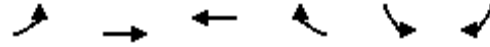
Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2027 FB-AM  
320 McRae



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Lane Configurations		↑	↑				
Traffic Volume (vph)	0	612	334	0	0	0	
Future Volume (vph)	0	612	334	0	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1745	1745	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	1745	1745	0	0	0	
Right Turn on Red				No		Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)		50	50		50		
Link Distance (m)		22.1	57.4		18.9		
Travel Time (s)		1.6	4.1		1.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	680	371	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	680	371	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		0.0		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		3.0	3.0		3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	25			15	25	15	
Number of Detectors							
Detector Template		Thru	Thru				
Leading Detector (m)		10.0	10.0				
Trailing Detector (m)		0.0	0.0				
Detector 1 Position(m)		0.0	0.0				
Detector 1 Size(m)		0.6	0.6				
Detector 1 Type		Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)		0.0	0.0				
Detector 1 Queue (s)		0.0	0.0				
Detector 1 Delay (s)		0.0	0.0				
Detector 2 Position(m)		9.4	9.4				
Detector 2 Size(m)		0.6	0.6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type							
Protected Phases		2	6			4	
Permitted Phases							
Detector Phase		2	6				
Switch Phase							
Minimum Initial (s)		5.0	5.0			10.0	

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Minimum Split (s)		23.8	23.8				22.0
Total Split (s)		30.8	30.8				24.0
Total Split (%)		56.2%	56.2%				44%
Maximum Green (s)		25.0	25.0				20.0
Yellow Time (s)		3.3	3.3				3.0
All-Red Time (s)		2.5	2.5				1.0
Lost Time Adjust (s)		0.0	0.0				
Total Lost Time (s)		5.8	5.8				
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0	3.0				3.0
Recall Mode		C-Max	C-Max				None
Walk Time (s)		7.0	7.0				7.0
Flash Dont Walk (s)		11.0	11.0				11.0
Pedestrian Calls (#/hr)		0	0				122
Act Effct Green (s)		32.6	32.6				
Actuated g/C Ratio		0.59	0.59				
v/c Ratio		0.66	0.36				
Control Delay		15.4	9.6				
Queue Delay		0.0	0.0				
Total Delay		15.4	9.6				
LOS		B	A				
Approach Delay		15.4	9.6				
Approach LOS		B	A				
Queue Length 50th (m)		51.3	21.7				
Queue Length 95th (m)		#106.3	38.5				
Internal Link Dist (m)		0.1	33.4		0.1		
Turn Bay Length (m)							
Base Capacity (vph)		1036	1036				
Starvation Cap Reductn		0	0				
Spillback Cap Reductn		0	0				
Storage Cap Reductn		0	0				
Reduced v/c Ratio		0.66	0.36				

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 60  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.66  
 Intersection Signal Delay: 13.4    Intersection LOS: B  
 Intersection Capacity Utilization 38.8%                                    ICU Level of Service A  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Scott St & Pedestrian Crossing



Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2027 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Future Volume (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.998			0.999			0.899			0.981	
Fl <sub>t</sub> Protected					0.999			0.987			0.959	
Satd. Flow (prot)	0	1567	0	0	1551	0	0	1394	0	0	754	0
Fl <sub>t</sub> Permitted					0.999			0.987			0.959	
Satd. Flow (perm)	0	1567	0	0	1551	0	0	1394	0	0	754	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	486	0	0	549	0	0	47	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	58.1%
ICU Level of Service	B
Analysis Period (min)	15



Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	478	8	16	527	6	12	0	35	6	0	1
Future Vol, veh/h	0	478	8	16	527	6	12	0	35	6	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	478	8	16	527	6	12	0	35	6	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	533	0	0	486	0	0	1045	1047	482	1062	1048	530
Stage 1	-	-	-	-	-	-	482	482	-	562	562	-
Stage 2	-	-	-	-	-	-	563	565	-	500	486	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	681	-	-	1077	-	-	207	156	584	134	155	398
Stage 1	-	-	-	-	-	-	565	420	-	373	381	-
Stage 2	-	-	-	-	-	-	511	380	-	408	418	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	681	-	-	1077	-	-	203	153	584	124	152	398
Mov Cap-2 Maneuver	-	-	-	-	-	-	203	153	-	124	152	-
Stage 1	-	-	-	-	-	-	565	420	-	373	373	-
Stage 2	-	-	-	-	-	-	499	372	-	384	418	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			15.3			32.5		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	395	681	-	-	1077	-	-	138
HCM Lane V/C Ratio	0.119	-	-	-	0.015	-	-	0.051
HCM Control Delay (s)	15.3	0	-	-	8.4	0	-	32.5
HCM Lane LOS	C	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	472	48	130	511	38	214
Future Volume (vph)	472	48	130	511	38	214
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.988			0.885		
Flt Protected				0.990	0.993	
Satd. Flow (prot)	1552	0	0	1399	1380	0
Flt Permitted				0.990	0.993	
Satd. Flow (perm)	1552	0	0	1399	1380	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	320.9	
Travel Time (s)	4.1			10.0	23.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)	0			0		
Adj. Flow (vph)	472	48	130	511	38	214
Shared Lane Traffic (%)						
Lane Group Flow (vph)	520	0	0	641	252	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	100.5%
ICU Level of Service	G
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	5.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	472	48	130	511	38	214
Future Vol, veh/h	472	48	130	511	38	214
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	472	48	130	511	38	214

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	520	0	1267
Stage 1	-	-	-	-	496
Stage 2	-	-	-	-	771
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1046	-	186
Stage 1	-	-	-	-	612
Stage 2	-	-	-	-	456
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1046	-	154
Mov Cap-2 Maneuver	-	-	-	-	154
Stage 1	-	-	-	-	612
Stage 2	-	-	-	-	377

Approach	EB	WB	NB
HCM Control Delay, s	0	1.8	27.2
HCM LOS			D

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	407	-	-	1046	-
HCM Lane V/C Ratio	0.619	-	-	0.124	-
HCM Control Delay (s)	27.2	-	-	8.9	0
HCM Lane LOS	D	-	-	A	A
HCM 95th %tile Q(veh)	4	-	-	0.4	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2027 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	30	451	20	82	707	12	4	7	37	9	11	18
Future Volume (vph)	30	451	20	82	707	12	4	7	37	9	11	18
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.896			0.936	
Flt Protected		0.997			0.995			0.996			0.988	
Satd. Flow (prot)	0	1558	0	0	1563	1201	0	1402	0	0	1452	0
Flt Permitted		0.997			0.995			0.996			0.988	
Satd. Flow (perm)	0	1558	0	0	1563	1201	0	1402	0	0	1452	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	30	451	20	82	707	12	4	7	37	9	11	18
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	501	0	0	789	12	0	48	0	0	38	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	86.3%
ICU Level of Service	E
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	30	451	20	82	707	12	4	7	37	9	11	18
Future Vol, veh/h	30	451	20	82	707	12	4	7	37	9	11	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	30	451	20	82	707	12	4	7	37	9	11	18

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	719	0	0	471	0	0	1413	1404	461	1414	1402	707
Stage 1	-	-	-	-	-	-	521	521	-	871	871	-
Stage 2	-	-	-	-	-	-	892	883	-	543	531	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	882	-	-	1091	-	-	115	140	600	115	140	435
Stage 1	-	-	-	-	-	-	539	532	-	346	368	-
Stage 2	-	-	-	-	-	-	337	364	-	524	526	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	882	-	-	1091	-	-	89	117	600	90	117	435
Mov Cap-2 Maneuver	-	-	-	-	-	-	89	117	-	90	117	-
Stage 1	-	-	-	-	-	-	514	508	-	330	322	-
Stage 2	-	-	-	-	-	-	273	319	-	463	502	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0.9			20			34.1		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	288	882	-	-	1091	-	-	161
HCM Lane V/C Ratio	0.167	0.034	-	-	0.075	-	-	0.236
HCM Control Delay (s)	20	9.2	0	-	8.6	0	-	34.1
HCM Lane LOS	C	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.6	0.1	-	-	0.2	-	-	0.9

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	90	331	87	52	701	175	80	66	55	111	43	149
Future Volume (vph)	90	331	87	52	701	175	80	66	55	111	43	149
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.969			0.970			0.932				0.934
Flt Protected	0.950			0.950			0.950					0.982
Satd. Flow (prot)	1492	1522	0	1492	1523	0	1492	1464	0	0	1441	0
Flt Permitted	0.209			0.389			0.486					0.830
Satd. Flow (perm)	328	1522	0	611	1523	0	763	1464	0	0	1218	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19			23			50				58
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				320.9
Travel Time (s)		7.5			7.3			4.4				23.1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0						
Adj. Flow (vph)	90	331	87	52	701	175	80	66	55	111	43	149
Shared Lane Traffic (%)												
Lane Group Flow (vph)	90	418	0	52	876	0	80	121	0	0	303	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		2		1	6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FB-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		1	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		9.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	40.0	40.0		11.0	51.0		29.0	29.0		29.0	29.0	
Total Split (%)	47.1%	47.1%		12.9%	60.0%		34.1%	34.1%		34.1%	34.1%	
Maximum Green (s)	35.5	35.5		6.5	46.5		24.5	24.5		24.5	24.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5				4.5
Lead/Lag	Lag	Lag		Lead			Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0		0	0	
Act Effct Green (s)	46.4	46.4		53.2	53.2		22.8	22.8				22.8
Actuated g/C Ratio	0.55	0.55		0.63	0.63		0.27	0.27				0.27
v/c Ratio	0.51	0.50		0.12	0.91		0.39	0.28				0.82
Control Delay	31.6	17.2		8.6	31.8		29.2	15.1				41.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	31.6	17.2		8.6	31.8		29.2	15.1				41.0
LOS	C	B		A	C		C	B				D
Approach Delay		19.8			30.5			20.7				41.0
Approach LOS		B			C			C				D
Queue Length 50th (m)	9.9	43.7		3.1	113.2		10.4	8.7				37.0
Queue Length 95th (m)	#35.8	81.2		8.6	#223.2		20.8	19.5				61.3
Internal Link Dist (m)		80.2			77.3			36.6				296.9
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	178	839		451	961		233	484				414
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.51	0.50		0.12	0.91		0.34	0.25				0.73

Intersection Summary

Area Type:	CBD
Cycle Length:	85
Actuated Cycle Length:	85
Offset:	1 (1%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.91
Intersection Signal Delay:	28.3
Intersection LOS:	C
Intersection Capacity Utilization:	105.5%
ICU Level of Service:	G
Analysis Period (min):	15



Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 4: McRae Ave & Richmond Rd



Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2027 FB-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	↑			↑			
Traffic Volume (vph)	519	0	0	549	0	0	
Future Volume (vph)	519	0	0	549	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1745	0	0	1745	0	0	
Flt Permitted							
Satd. Flow (perm)	1745	0	0	1745	0	0	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)	50			50	50		
Link Distance (m)	22.1			57.4	22.8		
Travel Time (s)	1.6			4.1	1.6		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	519	0	0	549	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	519	0	0	549	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(m)	0.0			0.0	0.0		
Link Offset(m)	0.0			0.0	0.0		
Crosswalk Width(m)	3.0			3.0	3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)		15	25		25	15	
Number of Detectors	2			2			
Detector Template	Thru			Thru			
Leading Detector (m)	10.0			10.0			
Trailing Detector (m)	0.0			0.0			
Detector 1 Position(m)	0.0			0.0			
Detector 1 Size(m)	0.6			0.6			
Detector 1 Type	Cl+Ex			Cl+Ex			
Detector 1 Channel							
Detector 1 Extend (s)	0.0			0.0			
Detector 1 Queue (s)	0.0			0.0			
Detector 1 Delay (s)	0.0			0.0			
Detector 2 Position(m)	9.4			9.4			
Detector 2 Size(m)	0.6			0.6			
Detector 2 Type	Cl+Ex			Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA			NA			
Protected Phases	2			6		4	
Permitted Phases							
Detector Phase	2			6			
Switch Phase							
Minimum Initial (s)	5.0			5.0		10.0	

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2027 FB-PM  
320 McRae

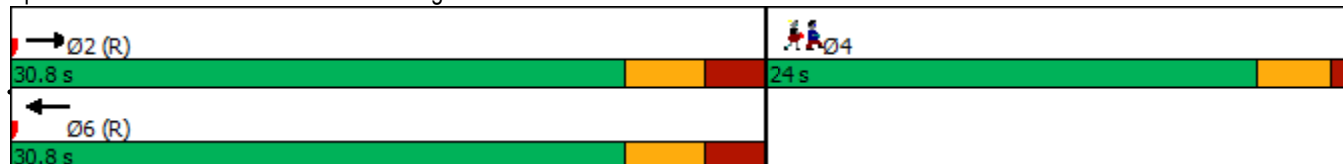


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Minimum Split (s)	23.8			23.8			22.0
Total Split (s)	30.8			30.8			24.0
Total Split (%)	56.2%			56.2%			44%
Maximum Green (s)	25.0			25.0			20.0
Yellow Time (s)	3.3			3.3			3.0
All-Red Time (s)	2.5			2.5			1.0
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.8			5.8			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0			3.0			3.0
Recall Mode	C-Max			C-Max			None
Walk Time (s)	7.0			7.0			7.0
Flash Dont Walk (s)	11.0			11.0			11.0
Pedestrian Calls (#/hr)	0			0			187
Act Effct Green (s)	32.6			32.6			
Actuated g/C Ratio	0.59			0.59			
v/c Ratio	0.50			0.53			
Control Delay	11.4			11.9			
Queue Delay	0.0			0.0			
Total Delay	11.4			11.9			
LOS	B			B			
Approach Delay	11.4			11.9			
Approach LOS	B			B			
Queue Length 50th (m)	34.0			36.9			
Queue Length 95th (m)	59.4			64.2			
Internal Link Dist (m)	0.1			33.4	0.1		
Turn Bay Length (m)							
Base Capacity (vph)	1036			1036			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.50			0.53			

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 55  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.53  
 Intersection Signal Delay: 11.7  
 Intersection Capacity Utilization 35.3%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

Splits and Phases: 6: Pedestrian Crossing & Scott St



# Appendix L

2022 Future Total Synchro Worksheets

Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2022 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	512	3	11	287	5	3	0	36	6	0	0
Future Volume (vph)	0	512	3	11	287	5	3	0	36	6	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999			0.998			0.874				
Flt Protected					0.998			0.997			0.950	
Satd. Flow (prot)	0	1569	0	0	1538	0	0	1369	0	0	761	0
Flt Permitted					0.998			0.997			0.950	
Satd. Flow (perm)	0	1569	0	0	1538	0	0	1369	0	0	761	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	569	3	12	319	6	3	0	40	7	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	572	0	0	337	0	0	43	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	41.8%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	512	3	11	287	5	3	0	36	6	0	0
Future Vol, veh/h	0	512	3	11	287	5	3	0	36	6	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	569	3	12	319	6	3	0	40	7	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	325	0	0	572	0	0	917	920	571	937	918	322
Stage 1	-	-	-	-	-	-	571	571	-	346	346	-
Stage 2	-	-	-	-	-	-	346	349	-	591	572	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	840	-	-	1001	-	-	253	190	520	167	190	540
Stage 1	-	-	-	-	-	-	506	377	-	507	493	-
Stage 2	-	-	-	-	-	-	670	491	-	358	376	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	840	-	-	1001	-	-	250	187	520	152	187	540
Mov Cap-2 Maneuver	-	-	-	-	-	-	250	187	-	152	187	-
Stage 1	-	-	-	-	-	-	506	377	-	507	486	-
Stage 2	-	-	-	-	-	-	660	484	-	330	376	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			13.2			29.8		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	480	840	-	-	1001	-	-	152
HCM Lane V/C Ratio	0.09	-	-	-	0.012	-	-	0.044
HCM Control Delay (s)	13.2	0	-	-	8.6	0	-	29.8
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1

Lanes, Volumes, Timings  
2: McRae Ave & Scott St

2022 FT-AM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	527	28	125	289	16	138
Future Volume (vph)	527	28	125	289	16	138
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.993			0.879		
Flt Protected				0.985	0.995	
Satd. Flow (prot)	1560	0	0	1392	1374	0
Flt Permitted				0.985	0.995	
Satd. Flow (perm)	1560	0	0	1392	1374	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	125.8	
Travel Time (s)	4.1			10.0	9.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	586	31	139	321	18	153
Shared Lane Traffic (%)						
Lane Group Flow (vph)	617	0	0	460	171	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	81.5%
Analysis Period (min)	15
	ICU Level of Service D



Intersection						
Int Delay, s/veh	3.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	527	28	125	289	16	138
Future Vol, veh/h	527	28	125	289	16	138
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	586	31	139	321	18	153

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	617	0	1201
Stage 1	-	-	-	-	602
Stage 2	-	-	-	-	599
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	963	-	204
Stage 1	-	-	-	-	547
Stage 2	-	-	-	-	549
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	963	-	168
Mov Cap-2 Maneuver	-	-	-	-	168
Stage 1	-	-	-	-	547
Stage 2	-	-	-	-	452

Approach	EB	WB	NB
HCM Control Delay, s	0	2.8	19.6
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	415	-	-	963	-
HCM Lane V/C Ratio	0.412	-	-	0.144	-
HCM Control Delay (s)	19.6	-	-	9.4	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2	-	-	0.5	-

Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2022 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	21	449	8	26	306	6	7	8	32	2	4	14
Future Volume (vph)	21	449	8	26	306	6	7	8	32	2	4	14
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.908			0.902	
Flt Protected		0.998			0.996			0.993			0.995	
Satd. Flow (prot)	0	1564	0	0	1564	1201	0	1416	0	0	1410	0
Flt Permitted		0.998			0.996			0.993			0.995	
Satd. Flow (perm)	0	1564	0	0	1564	1201	0	1416	0	0	1410	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	23	499	9	29	340	7	8	9	36	2	4	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	531	0	0	369	7	0	53	0	0	22	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	59.9%
ICU Level of Service	B
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	21	449	8	26	306	6	7	8	32	2	4	14
Future Vol, veh/h	21	449	8	26	306	6	7	8	32	2	4	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	23	499	9	29	340	7	8	9	36	2	4	16

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	347	0	0	508	0	0	962	955	504	970	952	340
Stage 1	-	-	-	-	-	-	550	550	-	398	398	-
Stage 2	-	-	-	-	-	-	412	405	-	572	554	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1212	-	-	1057	-	-	235	258	568	233	259	702
Stage 1	-	-	-	-	-	-	519	516	-	628	603	-
Stage 2	-	-	-	-	-	-	617	598	-	505	514	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1212	-	-	1057	-	-	216	243	568	203	244	702
Mov Cap-2 Maneuver	-	-	-	-	-	-	216	243	-	203	244	-
Stage 1	-	-	-	-	-	-	506	503	-	612	582	-
Stage 2	-	-	-	-	-	-	578	578	-	453	501	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.7			15.8			13.8		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	386	1212	-	-	1057	-	-	433
HCM Lane V/C Ratio	0.135	0.019	-	-	0.027	-	-	0.051
HCM Control Delay (s)	15.8	8	0	-	8.5	0	-	13.8
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0.1	-	-	0.2

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	115	379	43	8	301	130	24	16	17	109	39	68
Future Volume (vph)	115	379	43	8	301	130	24	16	17	109	39	68
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.985			0.955			0.923				0.957
Flt Protected	0.950			0.950			0.950					0.975
Satd. Flow (prot)	1492	1547	0	1492	1500	0	1492	1450	0	0	1465	0
Flt Permitted	0.440			0.447			0.587					0.822
Satd. Flow (perm)	691	1547	0	702	1500	0	922	1450	0	0	1236	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			47			19				29
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				195.0
Travel Time (s)		7.5			7.3			4.4				14.0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	128	421	48	9	334	144	27	18	19	121	43	76
Shared Lane Traffic (%)												
Lane Group Flow (vph)	128	469	0	9	478	0	27	37	0	0	240	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8				4

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FT-AM  
320 McRae

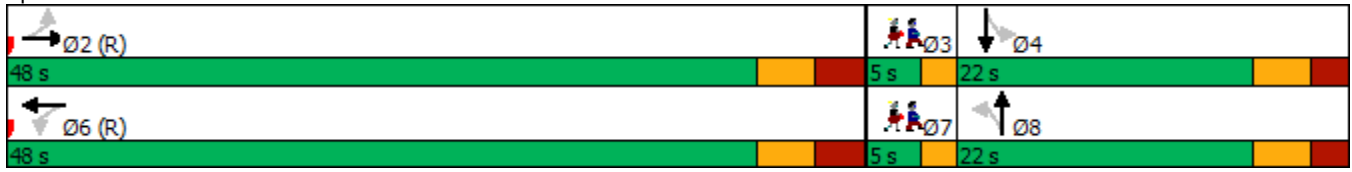


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		6	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.2	24.2		24.2	24.2		22.0	22.0		22.0	22.0	
Total Split (s)	48.0	48.0		48.0	48.0		22.0	22.0		22.0	22.0	
Total Split (%)	64.0%	64.0%		64.0%	64.0%		29.3%	29.3%		29.3%	29.3%	
Maximum Green (s)	41.8	41.8		41.8	41.8		16.5	16.5		16.5	16.5	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.9	2.9		2.9	2.9		2.2	2.2		2.2	2.2	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	6.2	6.2		6.2	6.2		5.5	5.5				5.5
Lead/Lag							Lag	Lag		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		C-Max	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0		7.0	7.0		5.5	5.5		5.5	5.5	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)	45.1	45.1		45.1	45.1		18.2	18.2				18.2
Actuated g/C Ratio	0.60	0.60		0.60	0.60		0.24	0.24				0.24
v/c Ratio	0.31	0.50		0.02	0.52		0.12	0.10				0.75
Control Delay	11.0	11.5		7.6	11.0		21.8	13.5				37.5
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	11.0	11.5		7.6	11.0		21.8	13.5				37.5
LOS	B	B		A	B		C	B				D
Approach Delay		11.4			11.0			17.0				37.5
Approach LOS		B			B			B				D
Queue Length 50th (m)	8.5	35.0		0.5	33.1		2.9	1.9				26.8
Queue Length 95th (m)	19.9	61.9		2.4	60.9		8.4	8.1				49.3
Internal Link Dist (m)		80.2			77.3			36.6				171.0
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	416	935		422	921		235	384				337
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.31	0.50		0.02	0.52		0.11	0.10				0.71

Intersection Summary

Area Type:	CBD
Cycle Length:	75
Actuated Cycle Length:	75
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	60
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.75
Intersection Signal Delay:	16.0
Intersection LOS:	B
Intersection Capacity Utilization:	71.3%
ICU Level of Service:	C
Analysis Period (min):	15

Splits and Phases: 4: McRae Ave & Richmond Rd



Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	7%	7%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		



Lanes, Volumes, Timings  
5: McRae Ave & Site Access #1

2022 FT-AM  
320 McRae



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	29	54	19	114	142	10
Future Volume (vph)	29	54	19	114	142	10
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.912				0.991	
Flt Protected	0.983			0.993		
Satd. Flow (prot)	1564	0	0	1733	1729	0
Flt Permitted	0.983			0.993		
Satd. Flow (perm)	1564	0	0	1733	1729	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	54.2			195.0	125.8	
Travel Time (s)	6.5			14.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	29	54	19	114	142	10
Shared Lane Traffic (%)						
Lane Group Flow (vph)	83	0	0	133	152	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	31.2% ICU Level of Service A
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	2.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		
Traffic Vol, veh/h	29	54	19	114	142	10
Future Vol, veh/h	29	54	19	114	142	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	29	54	19	114	142	10

Major/Minor	Minor2	Major1		Major2	
Conflicting Flow All	299	147	152	0	0
Stage 1	147	-	-	-	-
Stage 2	152	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-
Pot Cap-1 Maneuver	692	900	1429	-	-
Stage 1	880	-	-	-	-
Stage 2	876	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	682	900	1429	-	-
Mov Cap-2 Maneuver	682	-	-	-	-
Stage 1	868	-	-	-	-
Stage 2	876	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10	1.1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1429	-	810	-	-
HCM Lane V/C Ratio	0.013	-	0.102	-	-
HCM Control Delay (s)	7.6	0	10	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.3	-	-

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2022 FT-AM  
320 McRae



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Lane Configurations		↑	↑				
Traffic Volume (vph)	0	554	303	0	0	0	
Future Volume (vph)	0	554	303	0	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1745	1745	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	1745	1745	0	0	0	
Right Turn on Red				No		Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)		50	50		50		
Link Distance (m)		22.1	57.4		18.9		
Travel Time (s)		1.6	4.1		1.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	616	337	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	616	337	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		0.0		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		3.0	3.0		3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	25			15	25	15	
Number of Detectors		2	2				
Detector Template		Thru	Thru				
Leading Detector (m)		10.0	10.0				
Trailing Detector (m)		0.0	0.0				
Detector 1 Position(m)		0.0	0.0				
Detector 1 Size(m)		0.6	0.6				
Detector 1 Type		Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)		0.0	0.0				
Detector 1 Queue (s)		0.0	0.0				
Detector 1 Delay (s)		0.0	0.0				
Detector 2 Position(m)		9.4	9.4				
Detector 2 Size(m)		0.6	0.6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type		NA	NA				
Protected Phases		2	6			4	
Permitted Phases							
Detector Phase		2	6				
Switch Phase							
Minimum Initial (s)		5.0	5.0			10.0	

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2022 FT-AM  
320 McRae

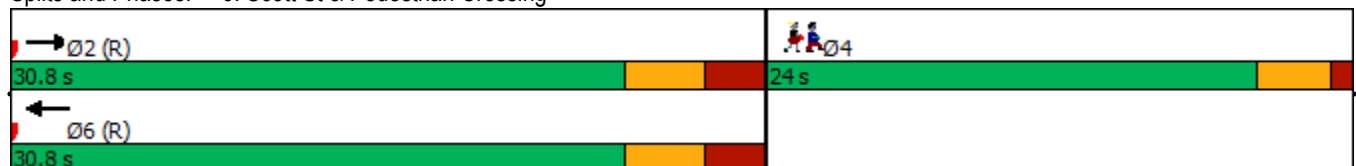


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Minimum Split (s)		23.8	23.8				22.0
Total Split (s)		30.8	30.8				24.0
Total Split (%)		56.2%	56.2%				44%
Maximum Green (s)		25.0	25.0				20.0
Yellow Time (s)		3.3	3.3				3.0
All-Red Time (s)		2.5	2.5				1.0
Lost Time Adjust (s)		0.0	0.0				
Total Lost Time (s)		5.8	5.8				
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0	3.0				3.0
Recall Mode		C-Max	C-Max				None
Walk Time (s)		7.0	7.0				7.0
Flash Dont Walk (s)		11.0	11.0				11.0
Pedestrian Calls (#/hr)		0	0				122
Act Effct Green (s)		32.6	32.6				
Actuated g/C Ratio		0.59	0.59				
v/c Ratio		0.59	0.33				
Control Delay		13.3	9.3				
Queue Delay		0.0	0.0				
Total Delay		13.3	9.3				
LOS		B	A				
Approach Delay		13.3	9.3				
Approach LOS		B	A				
Queue Length 50th (m)		43.9	19.3				
Queue Length 95th (m)		76.9	34.6				
Internal Link Dist (m)		0.1	33.4		0.1		
Turn Bay Length (m)							
Base Capacity (vph)		1036	1036				
Starvation Cap Reductn		0	0				
Spillback Cap Reductn		0	0				
Storage Cap Reductn		0	0				
Reduced v/c Ratio		0.59	0.33				

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 60  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.59  
 Intersection Signal Delay: 11.9  
 Intersection Capacity Utilization 35.6%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

Splits and Phases: 6: Scott St & Pedestrian Crossing



Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2022 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Future Volume (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.998			0.999			0.900			0.977	
Fl <sub>t</sub> Protected					0.998			0.987			0.960	
Satd. Flow (prot)	0	1567	0	0	1551	0	0	1395	0	0	751	0
Fl <sub>t</sub> Permitted					0.998			0.987			0.960	
Satd. Flow (perm)	0	1567	0	0	1551	0	0	1395	0	0	751	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	434	7	15	478	5	11	0	32	5	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	441	0	0	498	0	0	43	0	0	6	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	54.1%
Analysis Period (min)	15
	ICU Level of Service A

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	434	7	15	478	5	11	0	32	5	0	1
Future Vol, veh/h	0	434	7	15	478	5	11	0	32	5	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	434	7	15	478	5	11	0	32	5	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	483	0	0	441	0	0	949	951	438	965	952	481
Stage 1	-	-	-	-	-	-	438	438	-	511	511	-
Stage 2	-	-	-	-	-	-	511	513	-	454	441	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	716	-	-	1119	-	-	240	181	619	159	180	428
Stage 1	-	-	-	-	-	-	597	442	-	402	405	-
Stage 2	-	-	-	-	-	-	545	404	-	435	441	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	716	-	-	1119	-	-	236	178	619	149	177	428
Mov Cap-2 Maneuver	-	-	-	-	-	-	236	178	-	149	177	-
Stage 1	-	-	-	-	-	-	597	442	-	402	398	-
Stage 2	-	-	-	-	-	-	534	397	-	413	441	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			14.1			27.4		
HCM LOS							B			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	437	716	-	-	1119	-	-	167
HCM Lane V/C Ratio	0.098	-	-	-	0.013	-	-	0.036
HCM Control Delay (s)	14.1	0	-	-	8.3	0	-	27.4
HCM Lane LOS	B	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.1

Lanes, Volumes, Timings  
2: McRae Ave & Scott St

2022 FT-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	428	43	147	463	35	213
Future Volume (vph)	428	43	147	463	35	213
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.988			0.884		
Flt Protected				0.988	0.993	
Satd. Flow (prot)	1552	0	0	1397	1379	0
Flt Permitted				0.988	0.993	
Satd. Flow (perm)	1552	0	0	1397	1379	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	126.3	
Travel Time (s)	4.1			10.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)	0			0		
Adj. Flow (vph)	428	43	147	463	35	213
Shared Lane Traffic (%)						
Lane Group Flow (vph)	471	0	0	610	248	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	95.3%
ICU Level of Service	F
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	5.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	428	43	147	463	35	213
Future Vol, veh/h	428	43	147	463	35	213
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	428	43	147	463	35	213

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	471	0	1207
Stage 1	-	-	-	-	450
Stage 2	-	-	-	-	757
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1091	-	203
Stage 1	-	-	-	-	642
Stage 2	-	-	-	-	463
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1091	-	166
Mov Cap-2 Maneuver	-	-	-	-	166
Stage 1	-	-	-	-	642
Stage 2	-	-	-	-	379

Approach	EB	WB	NB
HCM Control Delay, s	0	2.1	23.1
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	442	-	-	1091	-
HCM Lane V/C Ratio	0.561	-	-	0.135	-
HCM Control Delay (s)	23.1	-	-	8.8	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	3.4	-	-	0.5	-



Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2022 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	28	421	18	82	649	11	3	6	46	8	10	16
Future Volume (vph)	28	421	18	82	649	11	3	6	46	8	10	16
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.887				0.936
Flt Protected		0.997			0.994			0.997				0.988
Satd. Flow (prot)	0	1558	0	0	1561	1201	0	1389	0	0	1452	0
Flt Permitted		0.997			0.994			0.997				0.988
Satd. Flow (perm)	0	1558	0	0	1561	1201	0	1389	0	0	1452	0
Link Speed (k/h)		50			50			50				50
Link Distance (m)		72.3			104.2			72.7				319.9
Travel Time (s)		5.2			7.5			5.2				23.0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	28	421	18	82	649	11	3	6	46	8	10	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	467	0	0	731	11	0	55	0	0	34	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0				0.0
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop				Stop

Intersection Summary	
Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	83.2%
	ICU Level of Service E
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	28	421	18	82	649	11	3	6	46	8	10	16
Future Vol, veh/h	28	421	18	82	649	11	3	6	46	8	10	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	28	421	18	82	649	11	3	6	46	8	10	16

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	660	0	0	439	0	0	1318	1310	430	1325	1308	649
Stage 1	-	-	-	-	-	-	486	486	-	813	813	-
Stage 2	-	-	-	-	-	-	832	824	-	512	495	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	928	-	-	1121	-	-	134	159	625	133	159	470
Stage 1	-	-	-	-	-	-	563	551	-	372	392	-
Stage 2	-	-	-	-	-	-	363	387	-	545	546	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	928	-	-	1121	-	-	108	135	625	105	135	470
Mov Cap-2 Maneuver	-	-	-	-	-	-	108	135	-	105	135	-
Stage 1	-	-	-	-	-	-	540	529	-	357	347	-
Stage 2	-	-	-	-	-	-	301	342	-	479	524	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.5			0.9			16.2			28.9		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	377	928	-	-	1121	-	-	184
HCM Lane V/C Ratio	0.146	0.03	-	-	0.073	-	-	0.185
HCM Control Delay (s)	16.2	9	0	-	8.5	0	-	28.9
HCM Lane LOS	C	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0.2	-	-	0.7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	105	301	78	47	636	172	73	75	50	109	49	151
Future Volume (vph)	105	301	78	47	636	172	73	75	50	109	49	151
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.969			0.968			0.940				0.934
Flt Protected	0.950			0.950			0.950					0.983
Satd. Flow (prot)	1492	1522	0	1492	1520	0	1492	1476	0	0	1442	0
Flt Permitted	0.254			0.417			0.479					0.835
Satd. Flow (perm)	399	1522	0	655	1520	0	752	1476	0	0	1225	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19			25			40				57
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				194.6
Travel Time (s)		7.5			7.3			4.4				14.0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0						
Adj. Flow (vph)	105	301	78	47	636	172	73	75	50	109	49	151
Shared Lane Traffic (%)												
Lane Group Flow (vph)	105	379	0	47	808	0	73	125	0	0	309	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		2		1	6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2022 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		1	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		9.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	40.0	40.0		11.0	51.0		29.0	29.0		29.0	29.0	
Total Split (%)	47.1%	47.1%		12.9%	60.0%		34.1%	34.1%		34.1%	34.1%	
Maximum Green (s)	35.5	35.5		6.5	46.5		24.5	24.5		24.5	24.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5				4.5
Lead/Lag	Lag	Lag		Lead			Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0		0	0	
Act Effct Green (s)	46.0	46.0		52.8	52.8		23.2	23.2				23.2
Actuated g/C Ratio	0.54	0.54		0.62	0.62		0.27	0.27				0.27
v/c Ratio	0.49	0.45		0.10	0.85		0.36	0.29				0.82
Control Delay	28.0	16.5		8.6	25.5		27.9	16.7				40.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	28.0	16.5		8.6	25.5		27.9	16.7				40.9
LOS	C	B		A	C		C	B				D
Approach Delay		19.0			24.5			20.9				40.9
Approach LOS		B			C			C				D
Queue Length 50th (m)	11.5	38.4		2.8	95.6		9.4	10.4				38.1
Queue Length 95th (m)	#37.9	71.3		8.0	#197.6		19.2	21.7				63.1
Internal Link Dist (m)		80.2			77.3			36.6				170.6
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	216	833		472	953		231	482				417
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.49	0.45		0.10	0.85		0.32	0.26				0.74

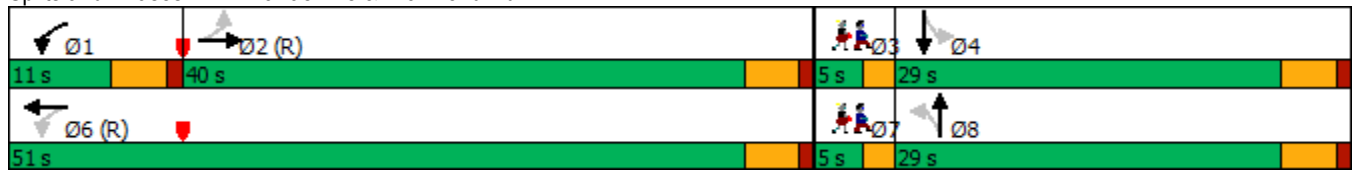
Intersection Summary

Area Type:	CBD
Cycle Length:	85
Actuated Cycle Length:	85
Offset:	1 (1%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.85
Intersection Signal Delay:	25.4
Intersection LOS:	C
Intersection Capacity Utilization:	102.6%
ICU Level of Service:	G
Analysis Period (min):	15

Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 4: McRae Ave & Richmond Rd



Lanes, Volumes, Timings  
5: McRae Ave & Site Access #1

2022 FT-PM  
320 McRae



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	18	33	50	223	167	27
Future Volume (vph)	18	33	50	223	167	27
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.913				0.981	
Flt Protected	0.983			0.991		
Satd. Flow (prot)	1566	0	0	1729	1712	0
Flt Permitted	0.983			0.991		
Satd. Flow (perm)	1566	0	0	1729	1712	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	55.1			194.6	126.3	
Travel Time (s)	6.6			14.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	18	33	50	223	167	27
Shared Lane Traffic (%)						
Lane Group Flow (vph)	51	0	0	273	194	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	39.6%
Analysis Period (min)	15
	ICU Level of Service A



Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		T
Traffic Vol, veh/h	18	33	50	223	167	27
Future Vol, veh/h	18	33	50	223	167	27
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	18	33	50	223	167	27

Major/Minor	Minor2	Major1		Major2	
Conflicting Flow All	504	181	194	0	0
Stage 1	181	-	-	-	-
Stage 2	323	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-
Pot Cap-1 Maneuver	528	862	1379	-	-
Stage 1	850	-	-	-	-
Stage 2	734	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	506	862	1379	-	-
Mov Cap-2 Maneuver	506	-	-	-	-
Stage 1	815	-	-	-	-
Stage 2	734	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.6	1.4	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1379	-	691	-	-
HCM Lane V/C Ratio	0.036	-	0.074	-	-
HCM Control Delay (s)	7.7	0	10.6	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0.1	-	0.2	-	-

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2022 FT-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	↑			↑			
Traffic Volume (vph)	471	0	0	498	0	0	
Future Volume (vph)	471	0	0	498	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1745	0	0	1745	0	0	
Flt Permitted							
Satd. Flow (perm)	1745	0	0	1745	0	0	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)	50			50	50		
Link Distance (m)	22.1			57.4	22.8		
Travel Time (s)	1.6			4.1	1.6		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	471	0	0	498	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	471	0	0	498	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(m)	0.0			0.0	0.0		
Link Offset(m)	0.0			0.0	0.0		
Crosswalk Width(m)	3.0			3.0	3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)		15	25		25	15	
Number of Detectors	2			2			
Detector Template	Thru			Thru			
Leading Detector (m)	10.0			10.0			
Trailing Detector (m)	0.0			0.0			
Detector 1 Position(m)	0.0			0.0			
Detector 1 Size(m)	0.6			0.6			
Detector 1 Type	Cl+Ex			Cl+Ex			
Detector 1 Channel							
Detector 1 Extend (s)	0.0			0.0			
Detector 1 Queue (s)	0.0			0.0			
Detector 1 Delay (s)	0.0			0.0			
Detector 2 Position(m)	9.4			9.4			
Detector 2 Size(m)	0.6			0.6			
Detector 2 Type	Cl+Ex			Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA			NA			
Protected Phases	2			6		4	
Permitted Phases							
Detector Phase	2			6			
Switch Phase							
Minimum Initial (s)	5.0			5.0		10.0	

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2022 FT-PM  
320 McRae

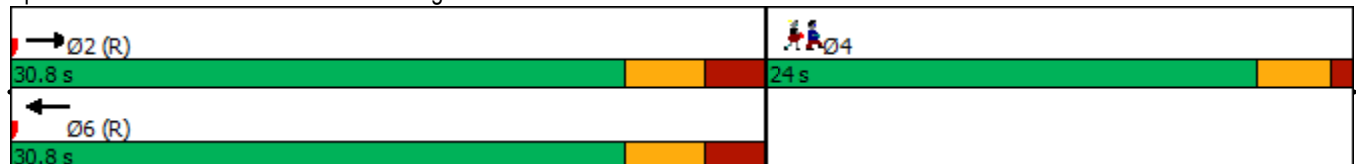


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Minimum Split (s)	23.8			23.8			22.0
Total Split (s)	30.8			30.8			24.0
Total Split (%)	56.2%			56.2%			44%
Maximum Green (s)	25.0			25.0			20.0
Yellow Time (s)	3.3			3.3			3.0
All-Red Time (s)	2.5			2.5			1.0
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.8			5.8			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0			3.0			3.0
Recall Mode	C-Max			C-Max			None
Walk Time (s)	7.0			7.0			7.0
Flash Dont Walk (s)	11.0			11.0			11.0
Pedestrian Calls (#/hr)	0			0			187
Act Effct Green (s)	32.6			32.6			
Actuated g/C Ratio	0.59			0.59			
v/c Ratio	0.45			0.48			
Control Delay	10.8			11.1			
Queue Delay	0.0			0.0			
Total Delay	10.8			11.1			
LOS	B			B			
Approach Delay	10.8			11.1			
Approach LOS	B			B			
Queue Length 50th (m)	29.7			32.1			
Queue Length 95th (m)	51.9			56.0			
Internal Link Dist (m)	0.1			33.4	0.1		
Turn Bay Length (m)							
Base Capacity (vph)	1036			1036			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.45			0.48			

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 50  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.48  
 Intersection Signal Delay: 11.0  
 Intersection Capacity Utilization 32.5%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

Splits and Phases: 6: Pedestrian Crossing & Scott St



# Appendix M

2027 Future Total Synchro Worksheets

Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2027 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	565	4	12	316	6	4	0	40	7	0	0
Future Volume (vph)	0	565	4	12	316	6	4	0	40	7	0	0
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.999			0.997			0.876				
Fl <sub>t</sub> Protected					0.998			0.996			0.950	
Satd. Flow (prot)	0	1569	0	0	1535	0	0	1370	0	0	761	0
Fl <sub>t</sub> Permitted					0.998			0.996			0.950	
Satd. Flow (perm)	0	1569	0	0	1535	0	0	1370	0	0	761	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	628	4	13	351	7	4	0	44	8	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	632	0	0	371	0	0	48	0	0	8	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	45.2%
ICU Level of Service	A
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	565	4	12	316	6	4	0	40	7	0	0
Future Vol, veh/h	0	565	4	12	316	6	4	0	40	7	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	628	4	13	351	7	4	0	44	8	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	358	0	0	632	0	0	1011	1014	630	1033	1013	355
Stage 1	-	-	-	-	-	-	630	630	-	381	381	-
Stage 2	-	-	-	-	-	-	381	384	-	652	632	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	813	-	-	951	-	-	218	164	482	141	164	515
Stage 1	-	-	-	-	-	-	470	351	-	483	473	-
Stage 2	-	-	-	-	-	-	641	472	-	328	350	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	813	-	-	951	-	-	215	161	482	126	161	515
Mov Cap-2 Maneuver	-	-	-	-	-	-	215	161	-	126	161	-
Stage 1	-	-	-	-	-	-	470	351	-	483	465	-
Stage 2	-	-	-	-	-	-	630	464	-	298	350	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			14.4			35.4		
HCM LOS							B			E		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	433	813	-	-	951	-	-	126
HCM Lane V/C Ratio	0.113	-	-	-	0.014	-	-	0.062
HCM Control Delay (s)	14.4	0	-	-	8.8	0	-	35.4
HCM Lane LOS	B	A	-	-	A	A	-	E
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	581	30	129	318	17	126
Future Volume (vph)	581	30	129	318	17	126
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.993			0.881		
Flt Protected				0.986	0.994	
Satd. Flow (prot)	1560	0	0	1394	1375	0
Flt Permitted				0.986	0.994	
Satd. Flow (perm)	1560	0	0	1394	1375	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	125.8	
Travel Time (s)	4.1			10.0	9.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)	0			0		
Adj. Flow (vph)	646	33	143	353	19	140
Shared Lane Traffic (%)						
Lane Group Flow (vph)	679	0	0	496	159	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	86.2%
ICU Level of Service	E
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	3.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	581	30	129	318	17	126
Future Vol, veh/h	581	30	129	318	17	126
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	646	33	143	353	19	140

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	679	0	1302 663
Stage 1	-	-	-	-	663 -
Stage 2	-	-	-	-	639 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	913	-	177 461
Stage 1	-	-	-	-	512 -
Stage 2	-	-	-	-	526 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	913	-	142 461
Mov Cap-2 Maneuver	-	-	-	-	142 -
Stage 1	-	-	-	-	512 -
Stage 2	-	-	-	-	423 -

Approach	EB	WB	NB
HCM Control Delay, s	0	2.8	22.3
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	364	-	-	913	-
HCM Lane V/C Ratio	0.437	-	-	0.157	-
HCM Control Delay (s)	22.3	-	-	9.7	0
HCM Lane LOS	C	-	-	A	A
HCM 95th %tile Q(veh)	2.1	-	-	0.6	-



Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2027 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	23	492	9	19	328	7	8	9	31	2	5	15
Future Volume (vph)	23	492	9	19	328	7	8	9	31	2	5	15
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998				0.850		0.913			0.908	
Flt Protected		0.998			0.997			0.992			0.996	
Satd. Flow (prot)	0	1564	0	0	1566	1201	0	1422	0	0	1420	0
Flt Permitted		0.998			0.997			0.992			0.996	
Satd. Flow (perm)	0	1564	0	0	1566	1201	0	1422	0	0	1420	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	26	547	10	21	364	8	9	10	34	2	6	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	583	0	0	385	8	0	53	0	0	25	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	65.1%
	ICU Level of Service C
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	23	492	9	19	328	7	8	9	31	2	5	15
Future Vol, veh/h	23	492	9	19	328	7	8	9	31	2	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	26	547	10	21	364	8	9	10	34	2	6	17

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	372	0	0	557	0	0	1026	1018	552	1032	1015	364
Stage 1	-	-	-	-	-	-	604	604	-	406	406	-
Stage 2	-	-	-	-	-	-	422	414	-	626	609	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1186	-	-	1014	-	-	213	237	533	211	238	681
Stage 1	-	-	-	-	-	-	485	488	-	622	598	-
Stage 2	-	-	-	-	-	-	609	593	-	472	485	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1186	-	-	1014	-	-	195	223	533	182	224	681
Mov Cap-2 Maneuver	-	-	-	-	-	-	195	223	-	182	224	-
Stage 1	-	-	-	-	-	-	469	472	-	602	582	-
Stage 2	-	-	-	-	-	-	573	578	-	418	469	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.5			17.4			14.6		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	344	1186	-	-	1014	-	-	398
HCM Lane V/C Ratio	0.155	0.022	-	-	0.021	-	-	0.061
HCM Control Delay (s)	17.4	8.1	0	-	8.6	0	-	14.6
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0.1	-	-	0.2

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	119	418	48	9	331	137	27	13	18	110	29	53
Future Volume (vph)	119	418	48	9	331	137	27	13	18	110	29	53
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.985			0.956			0.912				0.963
Flt Protected	0.950			0.950			0.950					0.972
Satd. Flow (prot)	1492	1547	0	1492	1501	0	1492	1432	0	0	1470	0
Flt Permitted	0.416			0.418			0.622					0.804
Satd. Flow (perm)	653	1547	0	657	1501	0	977	1432	0	0	1216	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			45			20				24
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				195.0
Travel Time (s)		7.5			7.3			4.4				14.0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Parking (#/hr)			0			0						
Adj. Flow (vph)	132	464	53	10	368	152	30	14	20	122	32	59
Shared Lane Traffic (%)												
Lane Group Flow (vph)	132	517	0	10	520	0	30	34	0	0	213	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8				4

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FT-AM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2				6		8				4	
Detector Phase	2	2			6	6	8	8			4	4
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0	5.0	10.0	10.0			10.0	10.0
Minimum Split (s)	24.2	24.2			24.2	24.2	22.0	22.0			22.0	22.0
Total Split (s)	48.0	48.0			48.0	48.0	22.0	22.0			22.0	22.0
Total Split (%)	64.0%	64.0%			64.0%	64.0%	29.3%	29.3%			29.3%	29.3%
Maximum Green (s)	41.8	41.8			41.8	41.8	16.5	16.5			16.5	16.5
Yellow Time (s)	3.3	3.3			3.3	3.3	3.3	3.3			3.3	3.3
All-Red Time (s)	2.9	2.9			2.9	2.9	2.2	2.2			2.2	2.2
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0				0.0
Total Lost Time (s)	6.2	6.2			6.2	6.2	5.5	5.5				5.5
Lead/Lag							Lag	Lag			Lag	Lag
Lead-Lag Optimize?							Yes	Yes			Yes	Yes
Vehicle Extension (s)	3.0	3.0			3.0	3.0	3.0	3.0			3.0	3.0
Recall Mode	C-Max	C-Max			C-Max	C-Max	None	None			None	None
Walk Time (s)	7.0	7.0			7.0	7.0	5.5	5.5			5.5	5.5
Flash Dont Walk (s)	11.0	11.0			11.0	11.0	11.0	11.0			11.0	11.0
Pedestrian Calls (#/hr)	0	0			0	0	0	0			0	0
Act Effct Green (s)	46.4	46.4			46.4	46.4	16.9	16.9				16.9
Actuated g/C Ratio	0.62	0.62			0.62	0.62	0.23	0.23				0.23
v/c Ratio	0.33	0.54			0.02	0.55	0.14	0.10				0.73
Control Delay	11.1	11.6			7.4	11.2	22.6	13.0				37.9
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0				0.0
Total Delay	11.1	11.6			7.4	11.2	22.6	13.0				37.9
LOS	B	B			A	B	C	B				D
Approach Delay	11.5				11.2		17.5				37.9	
Approach LOS	B				B		B				D	
Queue Length 50th (m)	8.2	37.3			0.5	35.0	3.4	1.5			24.5	
Queue Length 95th (m)	21.4	71.6			2.5	69.8	9.1	7.4			43.8	
Internal Link Dist (m)	80.2				77.3		36.6				171.0	
Turn Bay Length (m)	45.0				60.0							
Base Capacity (vph)	404	961			406	945	242	370			319	
Starvation Cap Reductn	0	0			0	0	0	0			0	
Spillback Cap Reductn	0	0			0	0	0	0			0	
Storage Cap Reductn	0	0			0	0	0	0			0	
Reduced v/c Ratio	0.33	0.54			0.02	0.55	0.12	0.09			0.67	

**Intersection Summary**

Area Type: CBD

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

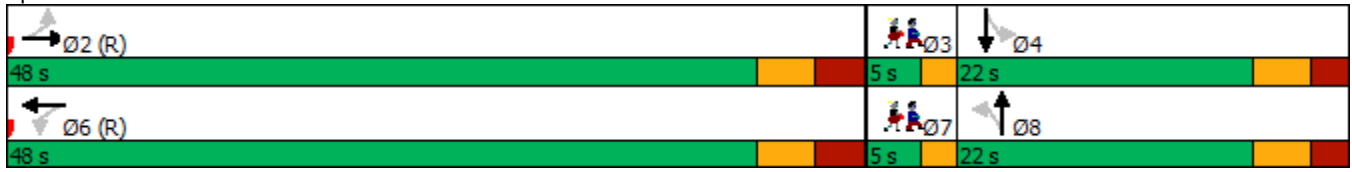
Maximum v/c Ratio: 0.73

Intersection Signal Delay: 15.5      Intersection LOS: B

Intersection Capacity Utilization 72.3%      ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 4: McRae Ave & Richmond Rd



Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	7%	7%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

Lanes, Volumes, Timings  
5: McRae Ave & Site Access #1

2027 FT-AM  
320 McRae



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	8	16	6	124	155	3
Future Volume (vph)	8	16	6	124	155	3
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.910				0.997	
Flt Protected	0.984			0.998		
Satd. Flow (prot)	1563	0	0	1742	1740	0
Flt Permitted	0.984			0.998		
Satd. Flow (perm)	1563	0	0	1742	1740	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	54.2			195.0	125.8	
Travel Time (s)	6.5			14.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	8	16	6	124	155	3
Shared Lane Traffic (%)						
Lane Group Flow (vph)	24	0	0	130	158	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	22.0%
Analysis Period (min)	15
	ICU Level of Service A



Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		T
Traffic Vol, veh/h	8	16	6	124	155	3
Future Vol, veh/h	8	16	6	124	155	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	16	6	124	155	3

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	293	157	158	0	-	0
Stage 1	157	-	-	-	-	-
Stage 2	136	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	698	889	1422	-	-	-
Stage 1	871	-	-	-	-	-
Stage 2	890	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	695	889	1422	-	-	-
Mov Cap-2 Maneuver	695	-	-	-	-	-
Stage 1	867	-	-	-	-	-
Stage 2	890	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.6	0.3	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1422	-	813	-	-
HCM Lane V/C Ratio	0.004	-	0.03	-	-
HCM Control Delay (s)	7.5	0	9.6	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Lanes, Volumes, Timings  
6: Scott St & Pedestrian Crossing

2027 FT-AM  
320 McRae

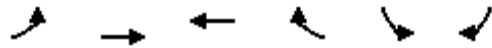


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Lane Configurations		↑	↑				
Traffic Volume (vph)	0	612	334	0	0	0	
Future Volume (vph)	0	612	334	0	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1745	1745	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	1745	1745	0	0	0	
Right Turn on Red				No		Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)		50	50		50		
Link Distance (m)		22.1	57.4		18.9		
Travel Time (s)		1.6	4.1		1.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	0	680	371	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	680	371	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(m)		0.0	0.0		0.0		
Link Offset(m)		0.0	0.0		0.0		
Crosswalk Width(m)		3.0	3.0		3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)	25			15	25	15	
Number of Detectors		2	2				
Detector Template		Thru	Thru				
Leading Detector (m)		10.0	10.0				
Trailing Detector (m)		0.0	0.0				
Detector 1 Position(m)		0.0	0.0				
Detector 1 Size(m)		0.6	0.6				
Detector 1 Type		Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)		0.0	0.0				
Detector 1 Queue (s)		0.0	0.0				
Detector 1 Delay (s)		0.0	0.0				
Detector 2 Position(m)		9.4	9.4				
Detector 2 Size(m)		0.6	0.6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type		NA	NA				
Protected Phases		2	6			4	
Permitted Phases							
Detector Phase		2	6				
Switch Phase							
Minimum Initial (s)		5.0	5.0			10.0	

# Lanes, Volumes, Timings

## 6: Scott St & Pedestrian Crossing

2027 FT-AM  
320 McRae

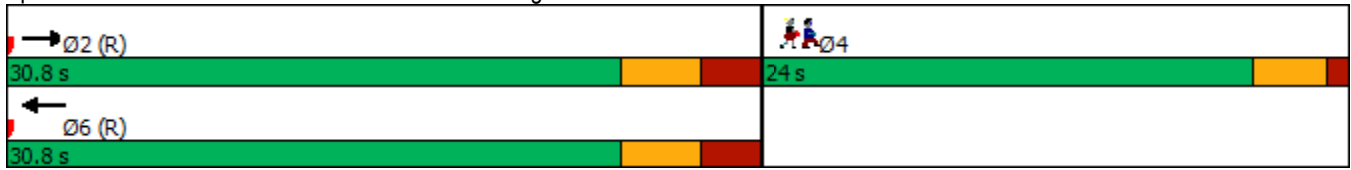


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	Ø4
Minimum Split (s)		23.8	23.8				22.0
Total Split (s)		30.8	30.8				24.0
Total Split (%)		56.2%	56.2%				44%
Maximum Green (s)		25.0	25.0				20.0
Yellow Time (s)		3.3	3.3				3.0
All-Red Time (s)		2.5	2.5				1.0
Lost Time Adjust (s)		0.0	0.0				
Total Lost Time (s)		5.8	5.8				
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)		3.0	3.0				3.0
Recall Mode		C-Max	C-Max				None
Walk Time (s)		7.0	7.0				7.0
Flash Dont Walk (s)		11.0	11.0				11.0
Pedestrian Calls (#/hr)		0	0				122
Act Effct Green (s)		32.6	32.6				
Actuated g/C Ratio		0.59	0.59				
v/c Ratio		0.66	0.36				
Control Delay		15.4	9.6				
Queue Delay		0.0	0.0				
Total Delay		15.4	9.6				
LOS		B	A				
Approach Delay		15.4	9.6				
Approach LOS		B	A				
Queue Length 50th (m)		51.3	21.7				
Queue Length 95th (m)		#106.3	38.5				
Internal Link Dist (m)		0.1	33.4		0.1		
Turn Bay Length (m)							
Base Capacity (vph)		1036	1036				
Starvation Cap Reductn		0	0				
Spillback Cap Reductn		0	0				
Storage Cap Reductn		0	0				
Reduced v/c Ratio		0.66	0.36				

### Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 60  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.66  
 Intersection Signal Delay: 13.4  
 Intersection LOS: B  
 Intersection Capacity Utilization 38.8%  
 ICU Level of Service A  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Scott St & Pedestrian Crossing



Lanes, Volumes, Timings  
1: Tweedsmuir Ave & Scott St

2027 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Future Volume (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fr <sub>t</sub>		0.998			0.999			0.899			0.981	
Fl <sub>t</sub> Protected					0.999			0.987			0.959	
Satd. Flow (prot)	0	1567	0	0	1551	0	0	1394	0	0	754	0
Fl <sub>t</sub> Permitted					0.999			0.987			0.959	
Satd. Flow (perm)	0	1567	0	0	1551	0	0	1394	0	0	754	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		78.0			22.1			319.9			53.1	
Travel Time (s)		5.6			1.6			23.0			3.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	100%	2%	2%	2%	2%	100%	2%	100%	2%	100%	100%	100%
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	0	478	8	16	527	6	12	0	35	6	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	486	0	0	549	0	0	47	0	0	7	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	58.1%
ICU Level of Service	B
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	478	8	16	527	6	12	0	35	6	0	1
Future Vol, veh/h	0	478	8	16	527	6	12	0	35	6	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	2	2	2	2	100	2	100	2	100	100	100
Mvmt Flow	0	478	8	16	527	6	12	0	35	6	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	533	0	0	486	0	0	1045	1047	482	1062	1048	530
Stage 1	-	-	-	-	-	-	482	482	-	562	562	-
Stage 2	-	-	-	-	-	-	563	565	-	500	486	-
Critical Hdwy	5.1	-	-	4.12	-	-	7.12	7.5	6.22	8.1	7.5	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	6.5	-	7.1	6.5	-
Follow-up Hdwy	3.1	-	-	2.218	-	-	3.518	4.9	3.318	4.4	4.9	4.2
Pot Cap-1 Maneuver	681	-	-	1077	-	-	207	156	584	134	155	398
Stage 1	-	-	-	-	-	-	565	420	-	373	381	-
Stage 2	-	-	-	-	-	-	511	380	-	408	418	-
Platoon blocked, %		-	-	-	-	-						
Mov Cap-1 Maneuver	681	-	-	1077	-	-	203	153	584	124	152	398
Mov Cap-2 Maneuver	-	-	-	-	-	-	203	153	-	124	152	-
Stage 1	-	-	-	-	-	-	565	420	-	373	373	-
Stage 2	-	-	-	-	-	-	499	372	-	384	418	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			15.3			32.5		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	395	681	-	-	1077	-	-	138
HCM Lane V/C Ratio	0.119	-	-	-	0.015	-	-	0.051
HCM Control Delay (s)	15.3	0	-	-	8.4	0	-	32.5
HCM Lane LOS	C	A	-	-	A	A	-	D
HCM 95th %tile Q(veh)	0.4	0	-	-	0	-	-	0.2



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	472	48	138	511	38	220
Future Volume (vph)	472	48	138	511	38	220
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.988			0.885		
Flt Protected				0.989	0.993	
Satd. Flow (prot)	1552	0	0	1398	1380	0
Flt Permitted				0.989	0.993	
Satd. Flow (perm)	1552	0	0	1398	1380	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	57.4			138.5	126.3	
Travel Time (s)	4.1			10.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)	0			0		
Adj. Flow (vph)	472	48	138	511	38	220
Shared Lane Traffic (%)						
Lane Group Flow (vph)	520	0	0	649	258	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.5	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.24	1.24	1.24	1.41	1.24	1.24
Turning Speed (k/h)	15		25	25		15
Sign Control	Free			Free	Stop	

**Intersection Summary**

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	101.4%
ICU Level of Service	G
Analysis Period (min)	15

Intersection						
Int Delay, s/veh	6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	472	48	138	511	38	220
Future Vol, veh/h	472	48	138	511	38	220
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	472	48	138	511	38	220

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	520	0	1283 496
Stage 1	-	-	-	-	496 -
Stage 2	-	-	-	-	787 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1046	-	182 574
Stage 1	-	-	-	-	612 -
Stage 2	-	-	-	-	449 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1046	-	149 574
Mov Cap-2 Maneuver	-	-	-	-	149 -
Stage 1	-	-	-	-	612 -
Stage 2	-	-	-	-	366 -

Approach	EB	WB	NB
HCM Control Delay, s	0	1.9	28.4
HCM LOS			D

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	404	-	-	1046	-
HCM Lane V/C Ratio	0.639	-	-	0.132	-
HCM Control Delay (s)	28.4	-	-	9	0
HCM Lane LOS	D	-	-	A	A
HCM 95th %tile Q(veh)	4.3	-	-	0.5	-



Lanes, Volumes, Timings  
3: Tweedsmuir Ave & Richmond Rd

2027 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Volume (vph)	30	455	20	84	709	12	4	7	41	9	11	18
Future Volume (vph)	30	455	20	84	709	12	4	7	41	9	11	18
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		10.0	0.0		0.0	0.0		0.0
Storage Lanes	0		0	0		1	0		0	0		0
Taper Length (m)	15.0			15.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.894			0.936	
Flt Protected		0.997			0.995			0.996			0.988	
Satd. Flow (prot)	0	1558	0	0	1563	1201	0	1398	0	0	1452	0
Flt Permitted		0.997			0.995			0.996			0.988	
Satd. Flow (perm)	0	1558	0	0	1563	1201	0	1398	0	0	1452	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		72.3			104.2			72.7			319.9	
Travel Time (s)		5.2			7.5			5.2			23.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0			0			0
Adj. Flow (vph)	30	455	20	84	709	12	4	7	41	9	11	18
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	505	0	0	793	12	0	52	0	0	38	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		3.0			3.0			3.0			3.0	
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.41	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	87.5%
ICU Level of Service	E
Analysis Period (min)	15

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕		↕			↕	
Traffic Vol, veh/h	30	455	20	84	709	12	4	7	41	9	11	18
Future Vol, veh/h	30	455	20	84	709	12	4	7	41	9	11	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	30	455	20	84	709	12	4	7	41	9	11	18

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	721	0	0	475	0	0	1423	1414	465	1426	1412	709
Stage 1	-	-	-	-	-	-	525	525	-	877	877	-
Stage 2	-	-	-	-	-	-	898	889	-	549	535	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	881	-	-	1087	-	-	114	138	597	113	138	434
Stage 1	-	-	-	-	-	-	536	529	-	343	366	-
Stage 2	-	-	-	-	-	-	334	361	-	520	524	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	881	-	-	1087	-	-	88	115	597	87	115	434
Mov Cap-2 Maneuver	-	-	-	-	-	-	88	115	-	87	115	-
Stage 1	-	-	-	-	-	-	511	505	-	327	319	-
Stage 2	-	-	-	-	-	-	269	314	-	456	500	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	0.5		0.9		19.7		34.9	
HCM LOS					C		D	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	297	881	-	-	1087	-	-	158
HCM Lane V/C Ratio	0.175	0.034	-	-	0.077	-	-	0.241
HCM Control Delay (s)	19.7	9.2	0	-	8.6	0	-	34.9
HCM Lane LOS	C	A	A	-	A	A	-	D
HCM 95th %tile Q(veh)	0.6	0.1	-	-	0.3	-	-	0.9

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	97	331	87	52	701	179	80	71	55	113	46	154
Future Volume (vph)	97	331	87	52	701	179	80	71	55	113	46	154
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	45.0		0.0	60.0		0.0	0.0		0.0	0.0		0.0
Storage Lanes	1		0	1		0	1		0	0		0
Taper Length (m)	30.0			55.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.969			0.969			0.935				0.934
Flt Protected	0.950			0.950			0.950					0.982
Satd. Flow (prot)	1492	1522	0	1492	1522	0	1492	1468	0	0	1441	0
Flt Permitted	0.198			0.384			0.482					0.831
Satd. Flow (perm)	311	1522	0	603	1522	0	757	1468	0	0	1219	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		19			24			46				58
Link Speed (k/h)		50			50			50				50
Link Distance (m)		104.2			101.3			60.6				194.6
Travel Time (s)		7.5			7.3			4.4				14.0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking (#/hr)			0			0						
Adj. Flow (vph)	97	331	87	52	701	179	80	71	55	113	46	154
Shared Lane Traffic (%)												
Lane Group Flow (vph)	97	418	0	52	880	0	80	126	0	0	313	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.5			3.5			3.5				3.5
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		3.0			3.0			3.0				3.0
Two way Left Turn Lane												
Headway Factor	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24	1.24
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		2		1	6			8			4	

Lane Group	Ø3	Ø7
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (m)		
Storage Lanes		
Taper Length (m)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (k/h)		
Link Distance (m)		
Travel Time (s)		
Peak Hour Factor		
Parking (#/hr)		
Adj. Flow (vph)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Enter Blocked Intersection		
Lane Alignment		
Median Width(m)		
Link Offset(m)		
Crosswalk Width(m)		
Two way Left Turn Lane		
Headway Factor		
Turning Speed (k/h)		
Number of Detectors		
Detector Template		
Leading Detector (m)		
Trailing Detector (m)		
Detector 1 Position(m)		
Detector 1 Size(m)		
Detector 1 Type		
Detector 1 Channel		
Detector 1 Extend (s)		
Detector 1 Queue (s)		
Detector 1 Delay (s)		
Detector 2 Position(m)		
Detector 2 Size(m)		
Detector 2 Type		
Detector 2 Channel		
Detector 2 Extend (s)		
Turn Type		
Protected Phases	3	7

Lanes, Volumes, Timings  
4: McRae Ave & Richmond Rd

2027 FT-PM  
320 McRae



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			8			4		
Detector Phase	2	2		1	6		8	8		4	4	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		9.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	40.0	40.0		11.0	51.0		29.0	29.0		29.0	29.0	
Total Split (%)	47.1%	47.1%		12.9%	60.0%		34.1%	34.1%		34.1%	34.1%	
Maximum Green (s)	35.5	35.5		6.5	46.5		24.5	24.5		24.5	24.5	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5				4.5
Lead/Lag	Lag	Lag		Lead			Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Max	C-Max		None	C-Max		None	None		None	None	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0		0	0	
Act Effct Green (s)	45.6	45.6		52.4	52.4		23.6	23.6				23.6
Actuated g/C Ratio	0.54	0.54		0.62	0.62		0.28	0.28				0.28
v/c Ratio	0.58	0.51		0.12	0.93		0.38	0.29				0.82
Control Delay	38.0	17.8		8.9	34.8		28.4	15.7				40.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0				0.0
Total Delay	38.0	17.8		8.9	34.8		28.4	15.7				40.7
LOS	D	B		A	C		C	B				D
Approach Delay		21.6			33.3			20.6				40.7
Approach LOS		C			C			C				D
Queue Length 50th (m)	11.6	44.9		3.2	118.2		10.3	9.7				38.4
Queue Length 95th (m)	#40.1	81.2		8.6	#224.6		20.9	20.9				64.3
Internal Link Dist (m)		80.2			77.3			36.6				170.6
Turn Bay Length (m)	45.0			60.0								
Base Capacity (vph)	166	825		441	947		235	487				418
Starvation Cap Reductn	0	0		0	0		0	0				0
Spillback Cap Reductn	0	0		0	0		0	0				0
Storage Cap Reductn	0	0		0	0		0	0				0
Reduced v/c Ratio	0.58	0.51		0.12	0.93		0.34	0.26				0.75

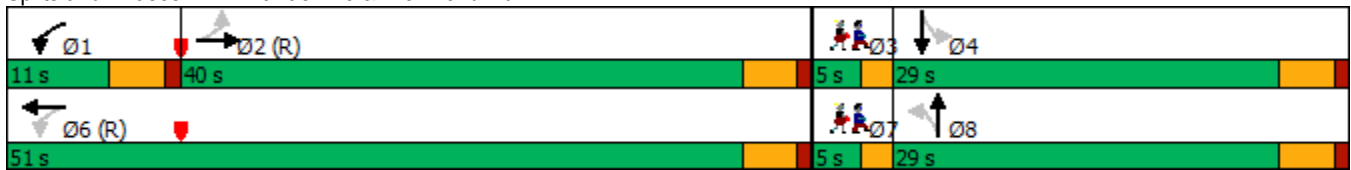
Intersection Summary

Area Type:	CBD
Cycle Length:	85
Actuated Cycle Length:	85
Offset:	1 (1%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.93
Intersection Signal Delay:	30.1
Intersection LOS:	C
Intersection Capacity Utilization:	106.9%
ICU Level of Service:	G
Analysis Period (min):	15

Lane Group	Ø3	Ø7
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	3.0	3.0
Minimum Split (s)	5.0	5.0
Total Split (s)	5.0	5.0
Total Split (%)	6%	6%
Maximum Green (s)	3.0	3.0
Yellow Time (s)	2.0	2.0
All-Red Time (s)	0.0	0.0
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes
Vehicle Extension (s)	3.0	3.0
Recall Mode	None	None
Walk Time (s)		
Flash Dont Walk (s)		
Pedestrian Calls (#/hr)		
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (m)		
Queue Length 95th (m)		
Internal Link Dist (m)		
Turn Bay Length (m)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Splits and Phases: 4: McRae Ave & Richmond Rd



Lanes, Volumes, Timings  
5: McRae Ave & Site Access #1

2027 FT-PM  
320 McRae



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	6	10	16	245	182	8
Future Volume (vph)	6	10	16	245	182	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.916			0.994		
Flt Protected	0.982			0.997		
Satd. Flow (prot)	1570	0	0	1740	1735	0
Flt Permitted	0.982			0.997		
Satd. Flow (perm)	1570	0	0	1740	1735	0
Link Speed (k/h)	30			50	50	
Link Distance (m)	55.1			194.6	126.3	
Travel Time (s)	6.6			14.0	9.1	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	6	10	16	245	182	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	0	261	190	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	3.5			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	3.0			3.0	3.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	37.5%
Analysis Period (min)	15
	ICU Level of Service A



Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T			T		T
Traffic Vol, veh/h	6	10	16	245	182	8
Future Vol, veh/h	6	10	16	245	182	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	10	16	245	182	8

Major/Minor	Minor2	Major1		Major2	
Conflicting Flow All	463	186	190	0	0
Stage 1	186	-	-	-	-
Stage 2	277	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-
Pot Cap-1 Maneuver	557	856	1384	-	-
Stage 1	846	-	-	-	-
Stage 2	770	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	550	856	1384	-	-
Mov Cap-2 Maneuver	550	-	-	-	-
Stage 1	835	-	-	-	-
Stage 2	770	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.2	0.5	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1384	-	708	-	-
HCM Lane V/C Ratio	0.012	-	0.023	-	-
HCM Control Delay (s)	7.6	0	10.2	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2027 FT-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	↑			↑			
Traffic Volume (vph)	519	0	0	549	0	0	
Future Volume (vph)	519	0	0	549	0	0	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1745	0	0	1745	0	0	
Flt Permitted							
Satd. Flow (perm)	1745	0	0	1745	0	0	
Right Turn on Red		Yes				Yes	
Satd. Flow (RTOR)							
Link Speed (k/h)	50			50	50		
Link Distance (m)	22.1			57.4	22.8		
Travel Time (s)	1.6			4.1	1.6		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	519	0	0	549	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	519	0	0	549	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(m)	0.0			0.0	0.0		
Link Offset(m)	0.0			0.0	0.0		
Crosswalk Width(m)	3.0			3.0	3.0		
Two way Left Turn Lane							
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	
Turning Speed (k/h)		15	25		25	15	
Number of Detectors	2			2			
Detector Template	Thru			Thru			
Leading Detector (m)	10.0			10.0			
Trailing Detector (m)	0.0			0.0			
Detector 1 Position(m)	0.0			0.0			
Detector 1 Size(m)	0.6			0.6			
Detector 1 Type	Cl+Ex			Cl+Ex			
Detector 1 Channel							
Detector 1 Extend (s)	0.0			0.0			
Detector 1 Queue (s)	0.0			0.0			
Detector 1 Delay (s)	0.0			0.0			
Detector 2 Position(m)	9.4			9.4			
Detector 2 Size(m)	0.6			0.6			
Detector 2 Type	Cl+Ex			Cl+Ex			
Detector 2 Channel							
Detector 2 Extend (s)	0.0			0.0			
Turn Type	NA			NA			
Protected Phases	2			6		4	
Permitted Phases							
Detector Phase	2			6			
Switch Phase							
Minimum Initial (s)	5.0			5.0		10.0	

Lanes, Volumes, Timings  
6: Pedestrian Crossing & Scott St

2027 FT-PM  
320 McRae



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Minimum Split (s)	23.8			23.8			22.0
Total Split (s)	30.8			30.8			24.0
Total Split (%)	56.2%			56.2%			44%
Maximum Green (s)	25.0			25.0			20.0
Yellow Time (s)	3.3			3.3			3.0
All-Red Time (s)	2.5			2.5			1.0
Lost Time Adjust (s)	0.0			0.0			
Total Lost Time (s)	5.8			5.8			
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0			3.0			3.0
Recall Mode	C-Max			C-Max			None
Walk Time (s)	7.0			7.0			7.0
Flash Dont Walk (s)	11.0			11.0			11.0
Pedestrian Calls (#/hr)	0			0			187
Act Effct Green (s)	32.6			32.6			
Actuated g/C Ratio	0.59			0.59			
v/c Ratio	0.50			0.53			
Control Delay	11.4			11.9			
Queue Delay	0.0			0.0			
Total Delay	11.4			11.9			
LOS	B			B			
Approach Delay	11.4			11.9			
Approach LOS	B			B			
Queue Length 50th (m)	34.0			36.9			
Queue Length 95th (m)	59.4			64.2			
Internal Link Dist (m)	0.1			33.4	0.1		
Turn Bay Length (m)							
Base Capacity (vph)	1036			1036			
Starvation Cap Reductn	0			0			
Spillback Cap Reductn	0			0			
Storage Cap Reductn	0			0			
Reduced v/c Ratio	0.50			0.53			

Intersection Summary

Area Type: Other  
 Cycle Length: 54.8  
 Actuated Cycle Length: 54.8  
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green  
 Natural Cycle: 55  
 Control Type: Actuated-Coordinated  
 Maximum v/c Ratio: 0.53  
 Intersection Signal Delay: 11.7  
 Intersection Capacity Utilization 35.3%  
 Analysis Period (min) 15  
 Intersection LOS: B  
 ICU Level of Service A

Splits and Phases: 6: Pedestrian Crossing & Scott St

