



**South Nepean Town Centre
(SNTC) Block 4 – Site Servicing
and Stormwater Management
Report**

Project #160401085

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Prepared for:

Caivan Development Corporation

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SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

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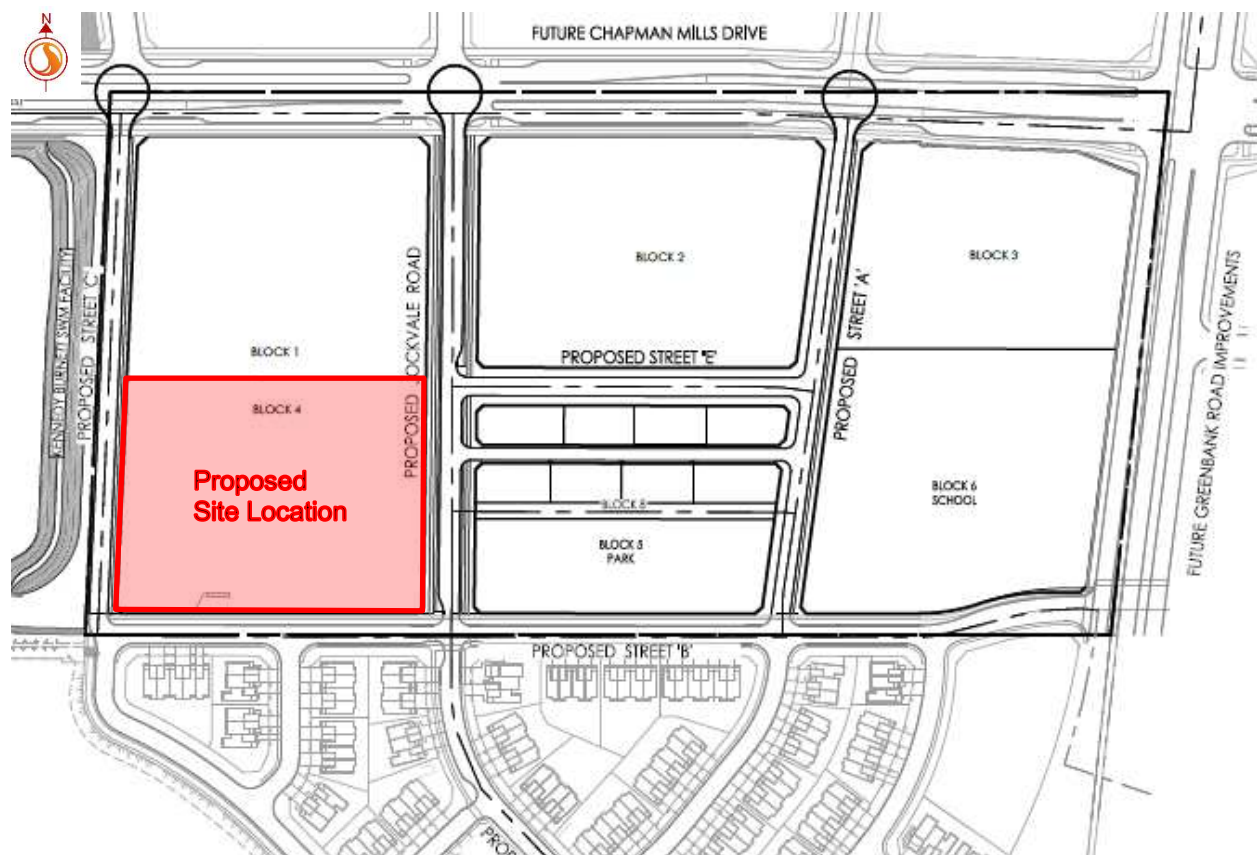
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Introduction

1.0 INTRODUCTION

Caivan Development Corporation has commissioned Stantec Consulting Ltd. to prepare the following Servicing and Stormwater Management Report for the South Nepean Town Centre (SNTC) Block 4. The subject property is located at the southwest quadrant of the SNTC subdivision, east of the Kennedy-Burnett Stormwater Management facility within the City of Ottawa, as indicated in **Figure 1.1**. The proposed residential development comprises approximately 1.59 ha of land, and consists of 116 back to back townhome units, associated private streets, and a parking area. The objective of this report is to provide a servicing scenario for the site that is free of conflicts, provides on-site servicing in accordance with City of Ottawa design guidelines, and utilizes the existing local infrastructure in accordance with the various background studies as well as the South Nepean Town Centre Site Servicing and Stormwater Management Report as outlined in **Section 2.0**.

Figure 1.1: Approximate Location of SNTC Block 4



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References

2.0 REFERENCES

The following documents were referenced in the preparation of this report:

- South Nepean Town Centre (SNTC) Site Servicing and Stormwater Management Report, Stantec Consulting Ltd., July 2020
- South Nepean Town Centre (SNTC) – Functional Servicing Report, Stantec Consulting Ltd., October 2019
- Geotechnical Investigation – Proposed Residential Development – 3288 Greenbank Road, Paterson Group Inc., March 6, 2019
- City of Ottawa Design Guidelines – Water Distribution, Infrastructure Services Department, City of Ottawa, First Edition, July 2010
- City of Ottawa Sewer Design Guidelines, 2nd Ed., City of Ottawa, October 2012
- Technical Bulletin ISTB-2018-01 Revision to Ottawa Design Guidelines – Sewer, City of Ottawa, March 2018
- Technical Bulletin ISTB-2018-02 Revision to Ottawa Design Guidelines – Water Distribution, City of Ottawa, March 2018



Potable Water

3.0 POTABLE WATER

A detailed potable water servicing hydraulic analysis was completed as part of Stantec's SNTC Development Site Servicing and SWM Report (February 2020), which included the proposed Block 4 development.

3.1 BACKGROUND

The proposed site comprises 116 back to back townhome units, complete with associated infrastructure and access areas. The site will be serviced via two watermain connections to the 200mm diameter watermains within Street B and Street C (see **Drawing SSP-1**). The future watermains on Streets B and C will service 17 back-to-back townhomes fronting the ROWs. Services will be dropped off for each unit when Streets B and C are constructed. A district metering chamber will be installed on the water service connection at Street C in accordance with the Water Distribution Guidelines to facilitate leak detection within the private site by the City of Ottawa.

The SNTC development is currently located within Zone BARR of the City of Ottawa's water distribution system. This zone is fed by the Barrhaven Pump Station and Barrhaven Reservoir Pump Station, with the Moodie elevated storage tank providing balancing storage for peak flows and demands. The development is located within the future Zone 3C pressure zone reconfiguration which will be completed by the City of Ottawa in the future.

The overall development lies within the Nepean Town Centre lands (NTC). The Kennedy-Burnett Potable Water Master Servicing Study (MSS) completed by Stantec in March 2014 assessed the proposed water distribution system within the Kennedy-Burnett development and the NTC lands and provided servicing recommendations during the interim scenario with both development areas serviced by Zone BARR pressures and during future conditions with the NTC area converted to Zone 3C pressure zone.

3.2 PROPOSED WATERMAIN SIZING AND LAYOUT

The proposed watermain alignment and sizing for Block 4 is based on the proposed SNTC Development. Dual 400mm diameter watermains exist north of the subdivision lands at the intersection of Greenbank Road and Jockvale Road. Both the proposed Caivan SNTC Development and Claridge's Burnett Lands Development to the south require a 300mm watermain connection to be made at this location (Boundary Connection #1) which will require the extension of the distribution system along future Chapman Mills Drive and Greenbank Road. Looping for both developments will be provided through a second connection to an existing 300mm watermain stub located south of the Claridge lands at the intersection of Greenbank



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Road and Bending Way (Boundary Connection #2). **Figure 3.1** shows the existing and proposed potable water distribution network required to service the proposed site.



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3.2.1 Water Demands

Water demands for the proposed site were estimated using the City of Ottawa Design Guidelines – Water Distribution (2010). A daily rate of 350 L/cap/day has been applied for the population of the proposed site. Population densities have been assumed as 2.7 pers./townhome unit. See **Appendix A** for detailed domestic water demand estimates.

The potable water analysis estimated for Block 4 in the overall SNTC development by Stantec assumed 140 units/ha resulting in 225 units with 2.7 pers./townhome, generating a total population of 608 persons. Excerpts from this report are included in **Appendix A**. The proposed Block 4 consists of 116 units which results in a population of 313 persons.

The average day demand (AVDY) for the proposed site was determined to be 1.27 L/s. The maximum daily demand (MXDY) is 2.5 times the AVDY for residential areas, which results in 3.17 L/s. The peak hour demand (PKHR) is 2.2 times the MXDY for residential areas totaling 6.98 L/s. Each unit per block will be individually metered.

3.2.2 Fire Flow Requirements

Wood frame construction was considered in the assessment for fire flow requirements according to the FUS Guidelines. The FUS Guidelines indicate that low hazard occupancies include apartments, dwellings, dormitories, hotels, and schools, and as such, a low hazard occupancy / limited combustible building contents credit was applied. Based on calculations per the FUS Guidelines (**Appendix A**), the worst case required fire flows for this site occur at Block 5 with a required fire flow of 14,000 L/min (233 L/s).

Fire separation walls are required for Blocks 10 and 11 to reduce maximum fire flows requirements in order to obtain minimum residual pressures of 20 psi.

As per the City's Technical Bulletin ISTB-2018-02, the maximum flow contribution from one given hydrant is 5,700 L/min (95 L/s) within a distance of 75 m, and 3,800 L/min (63 L/s) between 75 m and 150 m. As a result, hydrant placement in the vicinity of the townhome units within Block 5 was considered to ensure the maximum required fire flow of 14,000 L/min can be achieved.

On site fire protection will be provided by private hydrants located with a maximum of 90m spacing and within 90m of all building entrances. The internal private streets have been designed with a fire route providing access to all hydrants and residential units. The fire route is shown on the proposed site plan prepared by Korsiak.

3.2.3 Boundary Conditions

Boundary conditions for both existing and zone reconfiguration conditions were provided for the entire SNTC development from the City of Ottawa as included in **Appendix A.1** and summarized



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in **Table 3.1** and **Table 3.2**. These boundary conditions have been used to evaluate the level of service based on domestic design flows.

As noted in the correspondence from the City included in Appendix A.1, the maximum day plus fire flow scenario assumed a fire flow requirement of 267 L/s. The Burnett Subdivision watermain sizing for Street B has since been updated which directly impacts the fire flow analysis for Block 4. Boundary conditions for a fire flow requirement of 233 L/s have been interpolated from the boundary conditions provided for the Burnett Subdivision scenario 2 (Appendix A.1). These have been summarized in **Table 3.3** below. Updated boundary conditions have been requested from the City to confirm the interpolated values.

Table 3.1: Hydraulic Analysis Existing Boundary Conditions (SNTC Subdivision)

Connection	Maximum HGL (m)	Peak Hour HGL (m)	Max. Day plus Fire HGL (m) 267 L/s (16,000 L/min)	Ground Elevation (m)
Jockvale Road and Greenbank Road (Connection #1)	157.4	140.2	145.0	96.4
Greenbank road and Bending Way (Connection #2)	157.3	139.8	128.8	94.3

Table 3.2: Hydraulic Analysis Post SUC Zone Reconfiguration Boundary Conditions (SNTC Subdivision)

Connection	Maximum HGL (m)	Peak Hour HGL (m)	Max. Day plus Fire HGL (m) 267 L/s (16,000 L/min)	Ground Elevation (m)
Jockvale Road and Greenbank Road (Connection #1)	147.8	145.3	144.2	96.4
Greenbank road and Bending Way (Connection #2)	147.6	144.5	128.3	94.3



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Table 3.3: Maximum Day + Fire Flow Boundary Conditions (Burnett Lands Subdivision)

	Connection	Max. Day plus Fire HGL (m) 200 L/s (12,000 L/min)	Max. Day plus Fire HGL (m) 250 L/s (15,000 L/min)	Max. Day plus Fire HGL (m) 233 L/s (14,000 L/min)	Ground Elevation (m)
PRE-Configuration	Jockvale Road and Greenbank Road (Connection #1)	138.2	133.70	135.23	112.7
	Jockvale Road and Bending Way (Connection #2)	136.8	131.80	133.50	108.2
POST-Configuration	Jockvale Road and Greenbank Road (Connection #1)	146.4	146.30	146.35	112.7
	Jockvale Road and Bending Way (Connection #2)	145.4	144.90	145.07	108.2

*300mm watermain along Bending Way from Greenbank Road to Jockvale Road was included in MXDY + Fire Flow modeling analysis

3.3 HYDRAULIC ASSESSMENT

Level of Service

The City of Ottawa Water Distribution Design Guidelines state that the desired range of system pressures under normal demand conditions (i.e. basic day, maximum day and peak hour) should be in the range of 350 to 480 kPa (50 to 70 psi) and no less than 275 kPa (40 psi) at the ground elevation on the streets (i.e. at hydrant level). The maximum pressure at any point in the distribution system in occupied areas outside of the public right-of-way is 552 kPa (80 psi). As per the Ontario Building Code (OBC) & Guide for Plumbing, if pressures greater than 552 kPa (80 psi)



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are anticipated, pressure relief measures are required. The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi). Under emergency fire flow conditions, the minimum pressure objective in the distribution system is 138 kPa (20 psi).

Model Development

The proposed watermains within the Block 4 Development were added to the H2OMAP hydraulic model created for the entire SNTC Development (Stantec, February 2020) to simulate the proposed distribution system under both the existing and post reconfiguration conditions. Hazen-Williams coefficients (“C-Factors”) were applied to the new watermain in accordance with the City of Ottawa’s Water Distribution Design Guidelines (**Table 3.4**).

Table 3.4: Proposed Watermain C-Factors

Pipe Diameter (mm)	C-Factor
150	100
200 to 250	110
300 to 600	120
> 600	130

3.4 HYDRAULIC MODEL RESULTS

The Kennedy-Burnett Potable Water MSS concluded that development that proceeds in the interim prior to the reconfiguration of Zone 3C will have to ensure pressure reducing valves are installed where required.

The H2OMAP model for the proposed site consists of both existing and post reconfiguration scenarios. The existing scenario assumes ultimate development conditions under existing Zone BARR conditions, and the post reconfiguration scenario assumes ultimate development conditions under SUC Zone reconfiguration (3C). The overall results can be found in **Appendix A.4**.

The results from the existing zone - ultimate development conditions analysis show that the maximum pressure modeled is approximately 90.7 psi (625 kPa) and the minimum pressure during the peak hour scenario was approximately 64.9 psi (447 kPa) within the proposed Block 4 development as shown in **Figure 3.1** and **Figure 3.2** respectively. These pressures are above the serviceable limit of 50 to 80 psi (345 to 552 kPa) and therefore all proposed units will require pressure reducing valves.

Figure 3.1: Existing Zone - Ultimate Condition AVDY Pressure Results (psi)



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Figure 3.2: Existing Zone - Ultimate Condition PKHR Pressure Results (psi)



Post SUC zone reconfiguration under ultimate development conditions, the maximum pressure modeled was approximately 77.0 psi (530 kPa) and the minimum pressure during peak hour was approximately 71.8 psi (494 kPa) within the proposed SNTC development as shown in **Figure 3.3** and **Figure 3.4** respectively. These pressures are within the City of Ottawa allowable serviceable limits of 50 to 80 psi (345 to 552 kPa). Should the pressure zones be converted from Zone BARR to Zone 3C prior to construction, the dwellings will not require pressure reducing valves.

Figure 3.3: Post SUC Zone Reconfiguration - Ultimate Condition AVDY Pressure Results (psi)



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Figure 3.4: Post SUC Zone Reconfiguration - Ultimate Condition PKHR Pressure Results (psi)



The hydraulic model was used to assess the fire flow conditions of the proposed site. The model was carried out to determine the anticipated amount of flow that could be provided under maximum day demands and a fire flow requirement of 233 L/s for Blocks 4 (back-to-back townhomes) on Street B and Street C (nodes 20 to 26). As boundary condition parameters for the overall SNTC Development model only assessed a maximum day plus fire flow based on a fire flow demand of 267 L/s, the Burnett Lands boundary conditions were assumed to analysis the maximum day plus fire flow scenario.



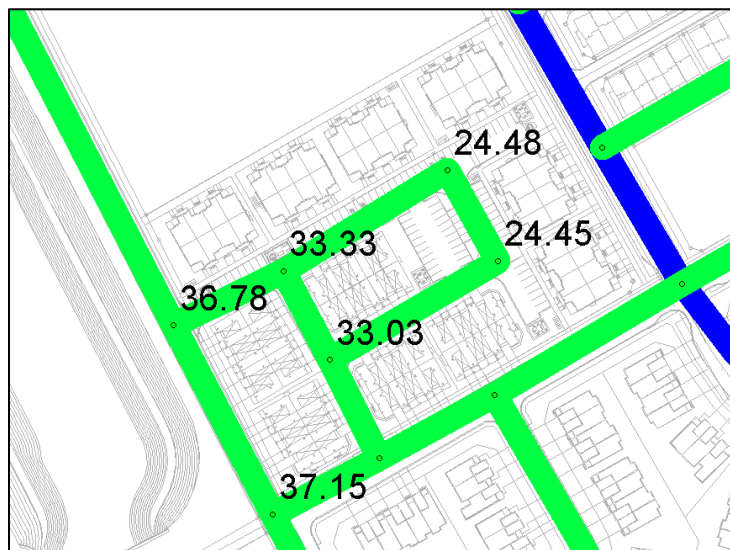
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Scenario 2 was considered, for both pre and post configuration. A fire flow of 233L/s was interpolated as detailed in section 3.2.3 above.

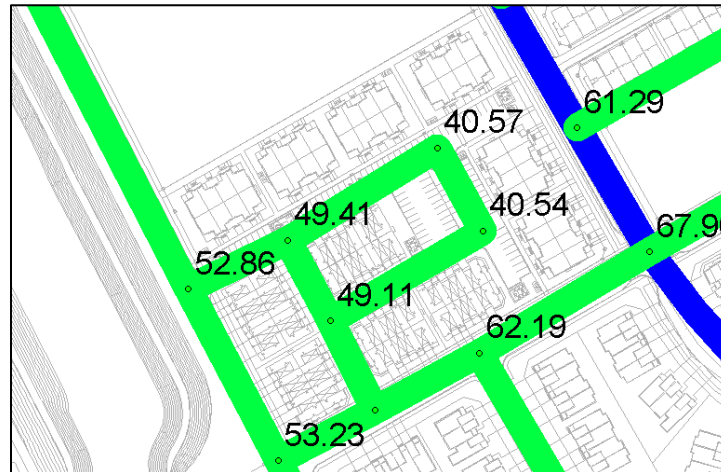
Results of the fire flow analysis indicate that flows in excess of 15.000 L/min (250 L/s) for the existing zone condition and 18,350 L/min (306 L/s) for the post reconfiguration zone c condition can be delivered for the units that require 233 L/s while still maintaining a residual pressure of 140 kPa (20 psi).

Figure 3.5: Existing Zone - Ultimate Condition Fire Flow Results – Residual Pressure (psi)



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Figure 3.6: Post SUC Zone Reconfiguration - Ultimate Condition Fire Flow Results – Residual Pressure (psi)



3.5 SUMMARY OF FINDINGS

Based on the findings of the report, pressure reducing valves will be required in all proposed units under existing BARR zone conditions to meet maximum pressure guidelines as per City of Ottawa standards under typical demand conditions (peak hour and average day conditions). If construction of the development occurs post SUC Zone reconfiguration, pressure reducing valves will not be required.

The results indicate that sufficient fire flows are available within the proposed watermain network under emergency fire demand conditions (maximum day + fire flow) for both existing and post zone reconfiguration scenarios, while meeting the minimum pressure requirements as per City of Ottawa standards. Fire walls are required for Blocks 10 and 11.



4.0 WASTEWATER SERVICING

4.1 BACKGROUND

As shown on **Drawing SA-1**, the proposed Block 4 will be serviced by the 200 mm diameter sanitary sewer on Street C with a connection to the existing 200mm stub dropped inside the property line. The network of 200mm diameter sanitary sewers is proposed along the private streets. Servicing requirements for Block 4 were outlined in Stantec's SNTC Site Servicing and Stormwater Management Report (February 2020) which included an estimated sanitary peak flow allocation for Block 4 of 7.1 L/s assuming high density residential land use with 140 units/ha and 2.7 person/unit for a total of 608 persons (Site Area = 1.59 ha).

The proposed site consists of eleven townhome blocks consists of 116 back-to-back townhome units with a population density of 313 persons. Nine units within Blocks 9 and 10 will be serviced through the sanitary sewer on Street C, and eight units within Block 7 and 8 will be serviced with individual service connections to Street B.

4.2 DESIGN CRITERIA

As outlined in the City's Sewer Design Guidelines, the following design parameters were used to calculate estimated wastewater flow rates and to size on-site sanitary sewers for the proposed phase of the development:

- Minimum Full Flow Velocity – 0.6 m/s
- Maximum Full Flow Velocity – 3.0 m/s
- Manning's roughness coefficient for all smooth walled pipes – 0.013
- Townhouse Persons per unit – 2.7
- Extraneous Flow Allowance – 0.33 L/s/ha
- Residential Average Flows – 280 L/cap/day
- Manhole Spacing – 120 m
- Minimum Cover – 2.5m

In addition, a residential peak factor based on Harmon's Equation in conjunction with a correction factor of 0.8 was used to determine the peak design flows.

4.3 PROPOSED SERVICING

The majority of the proposed site will be serviced by gravity sewers which will direct wastewater peak flows (approx. 3.5 L/s with allowance for infiltration) to the existing 200 mm diameter sanitary sewer stub. The proposed units within Blocks 7, 8, 9 and 10 that front Street B and Street C will be serviced through individual service connections to the sanitary sewers on Street C and Street B will generate a flow of approximately 0.6 L/s. Units within Blocks 1 to 6 and the units within Blocks 7-10 fronting the internal streets will be serviced through the private sanitary sewer system.



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Wastewater Servicing

A sanitary sewer design sheets for the proposed sanitary sewers within the development and the sewer design sheet for the SNTC Subdivision are included in **Appendix B**.

Full port backwater valves are to be installed on all sanitary services within the site to prevent any surcharge from the downstream sewer main from impacting the proposed property.



5.0 STORMWATER MANAGEMENT

The following section describes the stormwater management (SWM) design for Block 4 in accordance with the background documents and governing criteria within the SNTC subdivision based on Stantec's Site Servicing and Stormwater Management Report (February 2020).

5.1 PROPOSED CONDITIONS

The proposed 1.59 ha development is located within the southwestern quadrant of the SNTC subdivision and comprises 116 townhome units. The storm sewer collection system for the site will discharge to the 1200 mm diameter storm sewer on Street C that ultimately directs runoff to the Kennedy Burnett SWM Facility outlet channel. Quality control of stormwater runoff from the proposed Block 4 will be provided by hydrodynamic separator / Oil-Grit Separator (HDS) designed as part of Claridge's Development to the south to provide 'Enhanced' level of treatment (80% TSS Removal) prior to discharging into the outlet channel for the Kennedy-Burnett SWM Facility.

5.2 CRITERIA AND CONSTRAINTS

The overall approach for storm servicing and stormwater management for the proposed development was outlined in the Stantec SNTC Servicing and SWM Report (February 2020), excerpts can be found in **Appendix C.5**. The following summarizes the SWM criteria and constraints that will govern the detailed design of the proposed site as per the latest City of Ottawa Storm Sewer Guidelines as well as the conclusions made within the SNTC Servicing and SWM Report.

- Design using the dual drainage principle. (City)
- Minor system capture rate from Block 4 up to the 100-year storm to be restricted to 215 L/s. (Stantec - SNTC)
- Major system peak overflows to be controlled to a rate of 369 L/s during the 100-year storm event. (Stantec - SNTC)
- Size storm sewers to fully capture 2-year storm event under free-flow conditions (i.e. no ponding during 2-year storm event) using 2012 City of Ottawa Sewer Design Guidelines I-D-F parameters. (City)
- Separation of at least 0.3 m between the 100-year hydraulic grade line (HGL) and building under side of footing (USF) must be provided. (City)
- Maximum 'climate change' HGL to be lower than proposed basement elevations. (City)
- Inlet control devices (ICDs) to have a minimum orifice diameter of 83 mm. (City)
- Depth of flow may extend adjacent to the right-of-way provided that the water level does not touch any part of the building envelope and remains below the lowest building opening during the stress test event (100-year increased by 20%). (City)



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Stormwater Management

- Total maximum depth of flow under static and dynamic conditions shall be less than 0.35 m. (City)
- 100-year hydraulic grade line (HGL) to be a minimum 0.30 m below lowest building underside of footing elevation. The proposed townhome units will not have basements due to grade raise restrictions and 100-year HGL elevations. (City)
- Design storm sewers along local and collector roadways to convey the 2-year and 5-year peak flow respectively under free-flow conditions using 2004 City of Ottawa I-D-F parameters and an inlet time of 10 minutes. (City)
- Assess impact of 2-year storm, 5-year storm, and the worst case 100-year storm events, and climate change scenarios with a 20% increase of rainfall intensity, on the major & minor drainage system. (City)
- Building openings to be above the 100-year water level. (City)
- There must be at least 30 cm of vertical clearance between the spill elevation on the private street and the lowest building opening that is in the proximity of the flow route or ponding area. (City)
- There must be at least 30 cm of vertical clearance between the spill elevation on rear yard swales and the ground elevation at the building envelope that is in the proximity of the flow route or ponding area. (City)
- Minimum swale grades at 1.5% (subgrade provided for grades < 1.5%). (City)
- Minimum roadway profile grades at 0.5%. (City)
- Minimum roadway slope of 0.1% from crest-to-crest for overland flow route. (City)
- Provide adequate emergency overflow conveyance off-site. (City)

5.3 DESIGN METHODOLOGY

The design methodology for the SWM component of the development is as follows:

- Create a PCSWMM model that generates major and minor system hydrographs and assesses the minor system hydraulic grade line and the major system flow depths.
- Size inlet control devices for the proposed catchbasins to avoid surface ponding during the 2-year storm while meeting the required 0.3m 100-year HGL to USF clearance and the 214.6 L/s minor system allowable release rate in the 100-year storm.
- Ensure that total dynamic and static surface ponding depths do not exceed 0.35 m during the 100-year storm scenario.
- Confirm that climate change storm simulation does not result in flooding of properties.

The site is designed using the “dual drainage” principle, whereby the minor (pipe) system is designed to convey the peak rate of runoff from the 2-year design storm and runoff from larger



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events is conveyed by both minor (pipe) and major (overland) channels, such as roadways and walkways, safely to the appropriate outlet without impacting proposed or existing downstream properties.

In keeping with the minor system target peak outflow, Inlet Control Devices (ICDs) or orifice plates have been specified for all catchbasins to limit the inflow to the minor system which outlets to the 1200 mm diameter storm sewer on Street C. Restricted inlet rates to the sewer are necessary to meet the target peak outflows.

Drawing SD-1 outlines the proposed storm sewer alignment, ICD locations, drainage divides, and labels. The storm sewer design sheet is included in **Appendix C.1**.

5.4 MODELING RATIONALE

A comprehensive hydrologic modeling exercise was completed with PCSWMM, accounting for the estimated major and minor systems to evaluate the storm sewer infrastructure and major system segments. The use of PCSWMM for modeling of the site hydrology and hydraulics allowed for an analysis of the systems' response during various storm events. The following assumptions were applied to the detailed model:

- Hydrologic parameters as per Ottawa Sewer Design Guidelines, including Horton infiltration, Manning's 'n', and depression storage values.
- 3-hour Chicago Storm distribution for the 2-year, 5-year and 100-year analysis.
- To 'stress test' the system a 'climate change' scenario was created by adding 20% of the individual intensity values of the 100-year storm at their specified time step.
- Percent imperviousness calculated based on actual soft and hard surfaces for the proposed catchments and converted to equivalent Runoff Coefficient using the relationship $C = (\text{Imp.} \times 0.7) + 0.2$.
- Subcatchment areas are defined from high-point to high-point where sags occur.
- Width parameter was taken as twice the length of the street/swale segment for two-sided catchments and as the length of the street/swale segment for one-sided catchments.
- Catchbasin inflow restricted with inlet-control devices (ICDs) as necessary to maintain the minor system target peak outflow.
- Surface storage in road sags calculated based on grading plans (**Drawing SD-1**).

5.4.1 SWMM Dual Drainage Methodology

The proposed development is modeled in one modeling program as a dual conduit system (see **Figure 5.1**), with: 1) circular conduits representing the sewers & storage nodes representing manholes; 2) irregular conduits using street-shaped cross-sections to represent the approximate overland road network and storage nodes representing catchbasins. The dual drainage systems

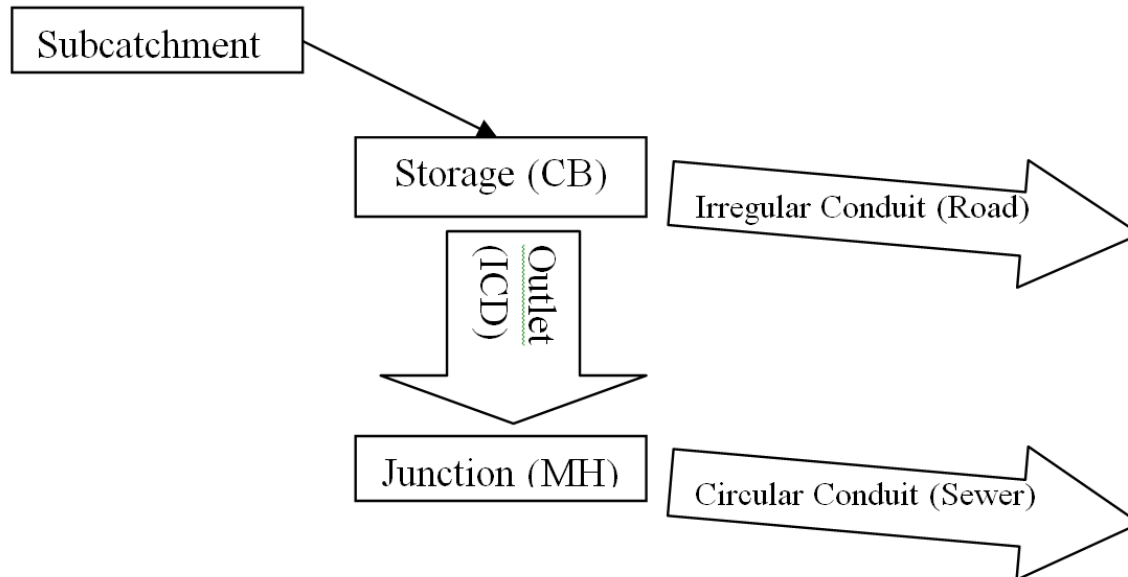


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are connected via outlet/orifice link objects from storage node (i.e. CB) to storage node (i.e. MH), and represent inlet control devices (ICDs). Subcatchments are linked to the storage node on the surface so that generated hydrographs are directed there firstly.

Figure 5.1: Schematic Representing Model Object Roles



Storage nodes are used in the model to represent catchbasins as well as major system junctions. For storage nodes representing catchbasins (CBs), the invert of the storage node represents the invert of the CB and the rim of the storage node represents the top of the CB plus an allowable flow depth on the segment. For the purpose of this SWM plan, CB inverts have been set 1.38 m below the top of the CB. The additional depth has been added to rim elevations to allow routing from one surface storage to the next. Storage nodes that represent catchbasins at sags, are surrounded by two transects that represent the road segments forming the sag. The storage value assigned to the storage node represents the available ponding volume above the catch basin. If the available storage volume in a storage node is exceeded, flows spill above the storage node and into the sag in the irregular conduits (representing roads). The volume stored within the road sags is represented as flood volume in the model and includes the total static volume and the ponded depth above the node representing the dynamic flow depth. Flow storage volumes exceeding the sag storage available in the transect (roadway) will spill at the downstream highpoint into the next sag and continue routing through the system until ultimately flows either re-enter the minor system or reach the outfall of the major system. Storage nodes representing high points are assigned an invert elevation equal to the transect invert (spill elevation at edge of pavement) and a rim elevation equal to the maximum allowable flow depth elevation above the storage node plus 0.35 m. A Storage value of 0 has been assigned to these nodes to disable linear volume calculations. No storage has been accounted for within



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storage nodes at high points. In this manner, storage will accumulate according to the actual ponding depths before spilling along the roadway conduit, and to the next downstream road conduit.

Inlet control devices, as represented by orifice links, have been used to represent the proposed vertical circular orifices sized to restrict minor system capture rates to the 2-year and 5-year rate for local streets, and collector roads (Jockvale Road) respectively. A minimum orifice diameter of 83 mm has been specified.

5.4.2 Design Storms

The 3-hour Chicago distribution was selected to estimate the 2-year capture rates for the proposed subcatchments, and to assess the 100-year HGL across the proposed development.

To 'stress test' the system a 'climate change' scenario was created by adding 20% of the individual intensity values of the 100-year storms at their specified time step.

5.4.3 Boundary Conditions

The detailed PCSWMM hydrology and the proposed storm sewers were used to assess the peak inflows and hydraulic grade line (HGL) in the proposed site. Fixed backwater elevation was obtained from Stantec's SNTC PCSWMM model (July 2020) from the stubbed outlet for Block 4 (Node 221) as 91.10 m 2-year storm.

As requested by City of Ottawa staff, the following scenarios have been used for the subject site to demonstrate compliance with City Guidelines:

- 5-year runoff with 100-year Jock River spring freshet boundary condition (91.90 m). Increased release rates to the minor system from Block 4 were required in order to minimize ponding during the 2-year event as the imperviousness assumed for Block 4 in the subdivision design is lower than that of the proposed Block 4 site plan. The revised Block 4 discharge rates were modeled in the overall SNTC subdivision for the worst case 100-year event. As the 5-year runoff with 100-year boundary condition provides the worse HGL results, the new boundary condition (from PCSWMM storage node ID-DICB221) generated from the revised subdivision model, was then used to model the Block 4 site plan.
- 100-year runoff with 5-year Jock River spring freshet boundary condition (91.36 m)

A stress test scenario using the 100-year rainfall intensities with 20% increase and the 5-year Jock River boundary condition should be assessed to ensure the maximum surface water level is below the proposed building openings. If the 5-year rainfall event with the 100-year boundary condition results in the governing hydraulic grade line (HGL), an additional climate change scenario based on the 5-year rainfall intensities with 20% increase should be assessed with the



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100-year Jock River boundary condition to ensure the HGL does not touch the proposed under side of footings (USFs).

- 5-year +20% runoff with 100-year Jock River spring freshet boundary condition (91.92 m)
- 100-year + 20% runoff with 5-year Jock River spring freshet boundary condition (91.38 m)

5.4.4 Modeling Parameters

Table 5.1 presents the general subcatchment parameters used:

Table 5.1: General Subcatchment Parameters

Subcatchment Parameter	Value
Infiltration Method	Horton
Max. Infil. Rate (mm/hr)	76.2
Min. Infil. Rate (mm/hr)	13.2
Decay Constant (1/hr)	4.14
N Imperv	0.013
N Perv	0.25
Dstore Imperv (mm)	1.57
Dstore Perv (mm)	4.67

Table 5.2 presents the individual parameters that vary for each of the subcatchments tributary to the storm outlet.

Table 5.2: Subcatchment Parameters

Area ID	Area (ha)	Width (m)	Slope (%)	% Impervious	Runoff Coefficient
L101A	0.37	370	3.0	88.57	0.82
L102A	0.16	116	3.0	87.14	0.81
L102B	0.42	314	3.0	84.29	0.79
L102C	0.09	40	3.0	91.43	0.84
L102D	0.12	54	3.0	88.57	0.82
L105A	0.11	56	3.0	87.14	0.81
UNC-1	0.14	321	2.0	60.00	0.62
UNC-2	0.03	200	3.0	42.86	0.50
UNC-3	0.12	305	3.0	42.86	0.50
UNC-4	0.03	200	3.0	42.86	0.50



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Table 5.3 summarizes the storage node parameters used in the model. All catchbasins have been modeled as having an outlet invert as depicted on **Drawings SSP-1**. Static ponding depths, areas, and volumes within the proposed development area are as per **Drawings SD-1**.

Table 5.3: Storage Node Parameters

Storage Node	Invert Elevation (m)	Rim Elevation (m)	Total Depth (m)
L101A1-S	93.99	94.34	0.35
L101A-S	92.12	93.85	1.73
L101A-S1	93.69	94.04	0.35
L102A-S	92.29	94.02	1.73
L102A-S1	93.77	94.12	0.35
L102B-S	91.94	93.87	1.93
L102B-S1	93.87	94.22	0.35
L102B-S2	93.97	94.32	0.35
L102C-S	92.52	94.25	1.73
L102C-S1	94.10	94.45	0.35
L102D-S	92.26	93.99	1.73
L102D-S1	93.94	94.29	0.35
L105A-S	92.08	93.81	1.73
L105A-S1	93.91	94.26	0.35

*The rim of the storage node represents the maximum allowable flow depth elevation above the storage node (equal to the top of the CB plus an additional 0.35 m).

5.4.5 Hydraulic Parameters

As per the City of Ottawa Sewer Design Guidelines, 2012, Manning's roughness values of 0.013 were used for sewer modeling and overland flow corridors representing roadways.

Storm sewers were modeled to confirm flow capacities, assess hydraulic grade lines (HGLs) and to determine minor system peak outflows to the outlet. The detailed storm sewer design sheet is included in **Appendix C.1**.

The table below presents the parameters for the orifice link objects within the proposed residential blocks which represent ICDs. It should be noted that the proposed ICDs will consist of slide type vertical circular orifices. A coefficient of 0.572 was applied when using orifices to conform to head/discharge curves as supplied by the manufacturer for IPEX Tempest HF model ICDs.



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Table 5.4: Orifice Parameters for Proposed Catchments

Orifice Name	Catchbasin ID	Tributary Area ID	Minor System Node	ICD Type
L101A-IC	CB101A	L101A	STM-101	178mm Orifice
L102A-IC	CB102A	L102A	STM-101	108mm Orifice
L102B-IC	CB102B	L102B	STM-102	178mm Orifice
L102C-IC	CB102C	L102C	STM-102	83mm Orifice
L102D-IC	CB102D	L102D	STM-102	102mm Orifice
L105A-IC	CB105A	L105A	STM-105	83mm Orifice

5.5 MODELING RESULTS AND DISCUSSION

The following sections summarize the key hydrologic and hydraulic model results. For detailed model results or inputs please refer to the electronic model files.

To allow for minimal ponding during the 2-year storm event, ICD sizes were maximized resulting in higher release rates during the worst case 100-year condition (100 year runoff using 5 year boundary conditions).

5.5.1 Proposed Inlet Control Devices

Table 5.5 summarizes the orifice link maximum flow rates and heads across the proposed development.

Table 5.5: Proposed Phase Orifice Link Results

Orifice Name	Catchbasin ID	Tributary Area ID	ICD Type	2yr Head (m)	100yr Runoff-5-yr BC Head (m)	2yr Flow (L/s)	100yr Runoff 5-yr BC Flow (L/s)
L101A-IC	CB101A	L101A	178mm Orifice	1.35	1.58	70.91	76.90
L102A-IC	CB102A	L102A	108mm Orifice	1.41	1.54	27.07	28.27
L102B-IC	CB102B	L102B	178mm Orifice	1.52	1.77	75.49	81.84
L102C-IC	CB102C	L102C	83mm Orifice	1.41	1.57	16.06	16.96
L102D-IC	CB102D	L102D	102mm Orifice	1.32	1.56	23.34	25.44
L105A-IC	CB105A	L105A	83mm Orifice	1.41	1.44	16.02	16.20



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5.5.2 Proposed Development Hydraulic Grade Line Analysis

The 100-year hydraulic grade line (HGL) elevation across the proposed development and Street B and Street C in the adjacent Claridge development was estimated using the PCSWMM model for the worst-case HGL using the 3 hour Chicago storm for the 5-year runoff with the 100-year Jock River water level as boundary condition and the 100-year runoff with the 5-year Jock River water level as boundary condition. The 5-year runoff with 100-year boundary conditions is based on the SNTC subdivision model adjusted values to account for a worst case minor system peak outflow of 245 L/s as detailed in section 5.5.4 below.

The climate change scenario was assessed using the 5-year runoff intensities (worst-case HGL) increased by 20% with the 100-year Jock River water level as a boundary condition. The 100 year HGL along storm sewers on Street B and C will service units within Blocks 7-10. The HGL values were obtained from Stantec's SNTC PCSWMM model (July 2020), excerpts of the stormwater management section can be found in **Appendix C.5. Table 5.6** below presents the clearance between the proposed storm sewers worst case HGL and the nearest proposed USF. The storm sewer design sheet is included in **Appendix C.1**.

Table 5.6: 100-Year HGL Results

STM MH	USF (m)	Worst-Case 100-Year HGL				5-year+20%, 3hr Chicago Storm	
		100-year Runoff HGL (m)	5-year Runoff HGL (m)	Worst-Case HGL (m)	Prop. Grade/USF-HGL Clearance (m)	Climate Change HGL (m)	Prop. Grade/USF-HGL Clearance (m)
101	92.27	91.41	91.95	91.95	0.32	91.97	0.30
102	92.43	91.73	92.09	92.09	0.34	92.12	0.31
103	92.85	91.48	91.98	91.98	0.87	92.00	0.85
104	92.43	91.40	91.99	91.99	0.44	92.02	0.41
105	92.85	91.63	91.99	91.99	0.86	92.01	0.84
218 (Street C)	92.24	91.33	91.85	91.85	0.39	91.89	0.35
EX 100 (Street B)	92.85	91.36	91.87	91.87	0.98	91.91	0.94

The model results indicate that there is sufficient clearance between the worst-case HGL and the proposed USFs within Block 4. Detailed grading of the site has been completed to ensure that the maximum hydraulic grade line is kept at least 0.30 m below the underside-of-footing (USF) of the adjacent units connected to the storm sewer during the worst case 100-year storm



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event and below proposed basement or first floor elevations during the 'climate change' event (Blocks 7- 11 do not have basements).

5.5.3 Overland Flow

Table 5.7 presents the maximum total surface water depths (static ponding depth + dynamic flow) above the top-of-grate of the proposed catchbasins for the 100-year, 3-hr Chicago storm and the 'climate change' storm. Based on the model results, the total ponding depth (static + dynamic) does not exceed the required 0.35 m maximum during the 100-year event. Tables summarizing the total surface water depths over the proposed catchbasins are included in **Appendix C.2**, which show that little to no ponding occurs over the proposed local streets during the 2-year storms event, with minimal ponding in subcatchment areas L102A and L102C.

Table 5.7: Proposed Phase – Ultimate Maximum Static and Dynamic Surface Water Depths

Storage node ID	Structure ID	Top of Grate Elevation (m)	2-year, 3-hour Chicago		100-year, 3-hour Chicago		100-year, 3-hour Chicago+20%	
			Max HGL (m)	Total Surface Water Depth (m)	Max HGL (m)	Total Surface Water Depth (m)	Max HGL (m)	Total Surface Water Depth (m)
L101A-S	CB101A	93.50	93.47	0.00	93.70	0.20	93.74	0.24
L102A-S	CB102A	93.67	93.70	0.03	93.83	0.16	93.84	0.17
L102B-S	CB102B	93.52	93.46	0.00	93.71	0.19	93.75	0.23
L102C-S	CB102C	93.90	93.93	0.03	94.09	0.19	94.11	0.21
L102D-S	CB102D	93.64	93.58	0.00	93.82	0.18	93.85	0.21
L105A-S	CB105A	93.46	93.46	0.00	93.52	0.06	93.52	0.06

5.5.4 Minor and Major System Peak Outflows and Deviations

Maximizing ICD sizes to allow for minimal ponding in the 2-year storm event resulted in greater minor system release rates during the 100-year storm event than targeted in the SNTC – Site Servicing and Stormwater Management Report (Stantec July 2020). The assumed imperviousness for Block 4 in the subdivision design was 71%, while the current site plan indicated 88%. As the SNTC subdivision plan at this time is under detail design approval, minor system alterations to Block 4's outflow rate will need to be reflected in the subdivision SWM model prior to final submission.

An analysis was completed on the overall SNTC subdivision model applying updated minor system discharge rates provided in **Table 5.8**. The PCSWMM modeling results and HGL comparison table can be found in **Appendix C.4**. Based on the overall model, the slight increase in minor system peak flows from Block 4 is negligible and will not have a significant impact on the downstream system as Block 4 is within close proximity to the outlet channel for the Kennedy-



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Burnett SWMF. A 0.29m clearance to USF during the 5-year runoff with 100-year boundary conditions can be provided across the subdivision with the increased peak flow from Block 4.

Minor system peak flows from the site are directed to the 1200 mm diameter storm sewer on Street C. Based on the PCSWMM model for the proposed development the worst case 100-year minor system peak outflow from the proposed site is equal to 245.5 L/s, 30 L/s greater than the 215.2 L/s minor system target for Block 4. The increase had minimal effect on the downstream water levels.

Table 5.8: Minor System Runoffs Results

Storm Event	Required Minor System Release Rate	Boundary Condition (DICB221)	Minor System Release Rate Target
2-year Chicago Storm	228.1	91.10	190.0
5-year runoff with 100-year Boundary Conditions Chicago Storm	237.3	91.90	214.5
100-year runoff with 5-year Boundary Conditions Chicago Storm	245.5	91.36	215.2
5-year+20%, 3hr Chicago Storm	238.8	91.92	219.5
100-year+20%, 3hr Chicago Storm	252.73	91.38	215.7

Similarly, major system peak flows from the site are directed to Street C. Due to grading restrictions, minor areas surrounding the site cannot be graded to enter the storm sewer system and will sheet flow uncontrolled to the adjacent Street B (UNC-2 & UNC-4), Street C (UNC-1), and Jockvale Road ROW (UNC-3). Peak discharges from uncontrolled areas have not been considered in the overall SWM plan for Block 4. The uncontrolled areas within Block 4 have been included as part of the SNTC subdivision plan and sheet drain to the adjacent streets (subcatchments L102A, L102C, L208A, L209A, L218A, L218B and L219C). The overall uncontrolled discharge rate for the proposed site is 149.6 L/s. Major system outflows from the proposed site onto Street C and Street B equal to 0 L/s and 37.9 L/s respectively, well below the 369 L/s target.



Grading

6.0 GRADING

The proposed development site measures approximately 1.59 ha in area. The topography across the site under existing conditions is relatively flat and generally slopes towards the south west direction. The objective of the grading design strategy is to satisfy the stormwater management requirements, adhere to permissible grade raise restrictions as much as possible (see Section 10.0), and provide for minimum cover requirements for sewers. The grading plan has been provided for reference in **Appendix E**.

The grading design also follows the recommendations outlined in the SNTC Site Servicing and Stormwater Management Report (Stantec Consulting Ltd., February 2020) and directs overland drainage towards Street B and C ROW ultimately into the Kennedy Burnett SWM Facility.

The grading plan (**Drawing GP-1**) identifies grade raise restrictions identified in the geotechnical investigation. Areas where grades are expected to exceed the maximum permissible grade raise will be subject to either a pre-loading/surcharge program, or lightweight fill and/or other approved means outside of proposed rights-of-way to reduce the risks of unacceptable long-term post construction differential settlements.



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Utilities

7.0 UTILITIES

As the subject site lies within residential development community, Hydro, Bell, Gas and Cable servicing for the proposed site will be readily available within subsurface infrastructure within Street B and Street C ROW. Exact size, location and routing of hydro utilities will be finalized after design circulation.

8.0 APPROVALS

Ontario Ministry of Environment, Conservation and Parks (MECP) Environmental Compliance Approvals (ECAs, formerly Certificates of Approval (CofA)) under the Ontario Water Resources Act is not expected to be required for proposed site.

An MECP Permit to Take Water (PTTW) or reporting on the Environmental Activity and Sector Registry (EASR) may be required for the site as some of the proposed works may be below the groundwater elevation shown in the geotechnical report. The geotechnical consultant shall determine whether a PTTW or EASR reporting is required prior to construction.



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Erosion Control

9.0 EROSION CONTROL

Erosion and sediment controls must be in place during construction. The following recommendations to the contractor will be included in contract documents.

1. Implement best management practices to provide appropriate protection of the existing and proposed drainage system and the receiving water course(s).
2. Limit extent of exposed soils at any given time.
3. Re-vegetate exposed areas as soon as possible.
4. Minimize the area to be cleared and grubbed.
5. Protect exposed slopes with plastic or synthetic mulches.
6. Provide sediment traps and basins during dewatering.
7. Install sediment traps (such as SiltSack® by Terrafix) between catch basins and frames.
8. Plan construction at proper time to avoid flooding.

The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:

9. Verification that water is not flowing under silt barriers.
10. Clean and change silt traps at catch basins.

Refer to Erosion and Sediment Control Plan included in **Appendix E** for the proposed location of silt fences, cutoff swales, temporary sediment basins and other erosion control structures.



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Geotechnical Investigation

10.0 GEOTECHNICAL INVESTIGATION

A geotechnical investigation for the development was completed by Paterson Group Inc. in March 2019. The report summarizes the existing soil conditions within the overall SNTC subdivision including the subject site and construction recommendations. For details which are not summarized below, please see the original Paterson report included in **Appendix D**.

Subsurface soil conditions within the subject area were determined through field investigations in February 2019 and October 2012. In total 4 boreholes were drilled, and 1 test pits excavated throughout the subject lands. In general soil stratigraphy consisted of topsoil and/or a silty clay deposit overlaying glacial till. Bedrock was estimated to occur between depths of 5-15m. The thickness of the existing topsoil ranged from 300 to 330mm.

Groundwater levels were encountered between 1.08m and 2.71m in depth. It is expected that construction may occur below the existing groundwater table and therefore a permit to take water may be required.

Based on the observed soil conditions, a grade raise restriction of between 1.4m and 2.0m above existing grade was recommended for housing / roadways. Areas where grades are expected to exceed the maximum permissible grade raise will be subject to either a pre-loading/surcharge program, or lightweight fill and/or other approved means outside of proposed rights-of-way to reduce the risks of unacceptable long-term post construction differential settlements.

The required pavement structure for the local roadways is outlined in **Table 10.1** and

Table 10.2 below.

Table 10.1: Recommended Pavement Structure – Car Parking Areas

Thickness (mm)	Material Description
50	Wear Course – HL 3 or Superpave 12.5 AC
150	Base - OPSS Granular 'A' crushed stone
300	Subbase - OPSS Granular 'B' Type II
	Subgrade – either fill, insitu soil or OPSS Granular B Type I or II material place over insitu soil or fill.



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**Table 10.2: Recommended Pavement Structure – Local Residential
Roadways**

Thickness (mm)	Material Description
40	Wear Course - Superpave 12.5 AC
50	Binder Course - Superpave 19.0 AC
150	Base – OPSS Granular A Crushed Stone
400	Subbase - OPSS Granular B Type II
	Subgrade – either fill, insitu soil or OPSS Granular B Type I or II material place over insitu soil or fill.



11.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the preceding information, the following conclusions are summarized below:

11.1 POTABLE WATER ANALYSIS

Based on the findings of the report, pressure reducing valves will be required in all proposed units under existing BARR zone conditions to meet maximum pressure guidelines as per City of Ottawa standards under typical demand conditions (peak hour and average day conditions). If construction of the development occurs post SUC Zone reconfiguration, pressure reducing valves will not be required.

The results indicate that sufficient fire flows are available within the proposed watermain network under emergency fire demand conditions (maximum day + fire flow) for both existing and post zone reconfiguration scenarios, while meeting the minimum pressure requirements as per City of Ottawa standards. Fire separation walls are required to reduce maximum fire flows for Blocks 10 and 11.

11.2 WASTEWATER SERVICING

Block 4 will be serviced by a network of gravity sewers which will direct wastewater flows to the Street C. The proposed sanitary sewer design indicates a total estimated peak outflow of 3.5 L/s will be discharged to the Street C sewer. A total of 0.6 L/s will be generated by the units with direct service connections to the Street B and Street C sewers. The receiving sewer system has sufficient available capacity to receive the design flows. Design guidelines for slope and velocity have been met within the proposed sewers.

11.3 STORMWATER MANAGEMENT

- The proposed stormwater management plan is in compliance with the goals specified in the background reports and the 2012 City of Ottawa Sewer Guidelines.
- Inlet control devices will be proposed to limit inflow from the site area into the minor system to the 2-year storm (5-year for collector roads) event based on City of Ottawa IDF curves.
- All dynamic surface water depths are to be less than 0.35 m during all storm events up to the 100-year storm event.
- The storm sewer hydraulic grade line will be maintained at least 0.30 m below the underside of footing in the subdivision during design storm events.
- Minor system peak flows from the proposed site will be captured and directed to an HDS for quality control (designed by others) and will ultimately discharge into the outlet channel for the Kennedy-Burnett SWM Facility.



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Conclusions and Recommendations

- The minor system outflow is greater than the proposed target by 30 L/s. The increased discharge rate has been modeled through PCSWMM and generates minimal impact to the downstream storm sewers.
- Major system peak outflow target have been met with the proposed design.

11.4 GRADING

A grading plan has been prepared taking into account required overland flow conveyance, cover over sewers, hydraulic grade line requirements, and grade raise restrictions as identified in the geotechnical investigation.

11.5 UTILITIES

Utility infrastructure exists in the general area of the subject site. Exact size, location and routing of utilities will be finalized at the detailed design stage.



**SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER
MANAGEMENT REPORT**

Appendix A Potable Water analysis

APPENDICES

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

Appendix A POTABLE WATER ANALYSIS

A.1 BOUNDARY CONDITIONS



Boundary Conditions South Nepean Town Centre

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	570.6	9.51
Maximum Daily Demand	1426.8	23.78
Peak Hour	3139.2	52.32
Fire Flow Demand	16020	267

Location



Results – Existing Conditions

Connection 1 - Jockvale Rd. (N20319)

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.4	86.8
Peak Hour	140.2	62.3
Max Day plus Fire	145.0	69.1

¹ Ground Elevation = 96.4 m

Connection 2 - Greenbank Rd. (N20084)

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.3	89.6
Peak Hour	139.8	64.7
Max Day plus Fire	128.8	49.0

¹ Ground Elevation = 94.3 m

Results – SUC Zone Reconfiguration**Connection 1 - Cambrian Rd. (N20319)**

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.8	73.1
Peak Hour	145.3	69.6
Max Day plus Fire	144.2	68.0

¹ Ground Elevation = 96.4 m

Connection 2 - Greenbank Rd. (N20084)

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.6	75.8
Peak Hour	144.5	71.4
Max Day plus Fire	128.3	48.3

¹ Ground Elevation = 94.3 m

Notes:

1. As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a. If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.
 - b. Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.
2. Under Existing Condition BARR PUMP #3 had to be turned on during Fire Hours.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Boundary Conditions 3370 Greenbank Road (Burnett Lands)

Information Provided

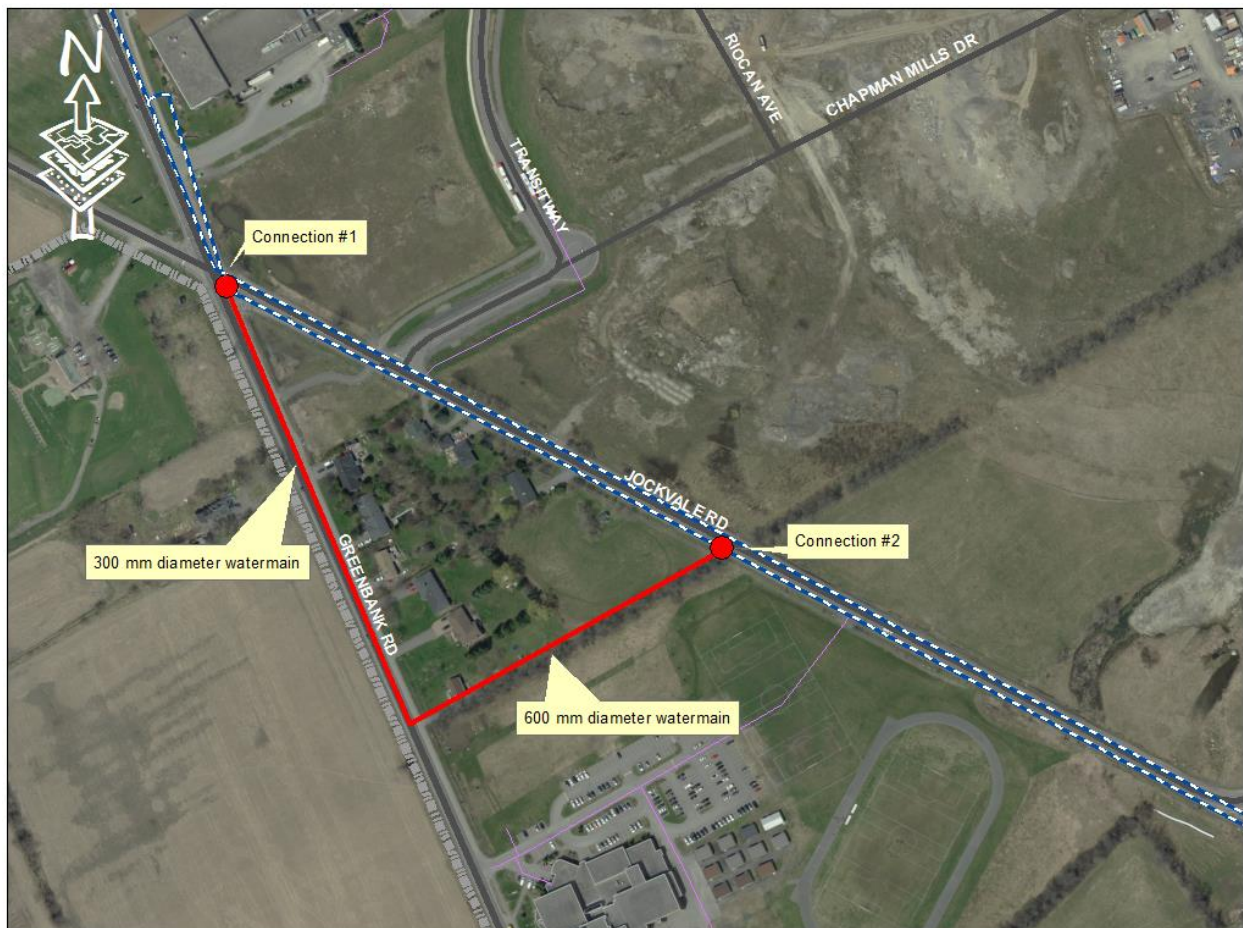
Date provided: 27 December 2017

Scenario	Demand	
	L/min	L/s
Average Daily Demand	348.6	5.81
Maximum Daily Demand	871.2	14.52
Peak Hour	1917	32.0
Fire Flow Demand # 1	12000	200.0
Fire Flow Demand # 2	15000	250.0
Fire Flow Demand # 3	18000	300.0

of connections

2

Connection Location Scenario 1



Connection Location Scenario 2



Results

SCENARIO 1

PRE-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	97.2
Peak Hour	141.2	62.6
Max Day plus Fire (12,000 l/min)	138.2	58.4
Max Day plus Fire (15,000 l/min)	133.7	52.0
Max Day plus Fire (18,000 l/min)	129.3	45.6

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Private Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	153.0	95.0
Peak Hour	140.9	60.2
Max Day plus Fire (12,000 l/min)	137.1	54.8
Max Day plus Fire (15,000 l/min)	132.2	47.8
Max Day plus Fire (18,000 l/min)	127.3	40.8

¹ Ground Elevation = 108.2 m

SCENARIO 2

PRE-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	97.2
Peak Hour	141.2	62.6
Max Day plus Fire (12,000 l/min)	138.2	58.4
Max Day plus Fire (15,000 l/min)	133.7	52.0
Max Day plus Fire (18,000 l/min)	129.2	45.6

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Bren-Maur Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	157.2	104.1
Peak Hour	140.5	69.0
Max Day plus Fire (12,000 l/min)	136.8	63.7
Max Day plus Fire (15,000 l/min)	131.8	56.7
Max Day plus Fire (18,000 l/min)	126.9	49.7

¹ Ground Elevation = 108.2 m

SCENARIO 1

POST-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	72.1
Peak Hour	146.3	70.0
Max Day plus Fire (12,000 l/min)	146.4	70.2
Max Day plus Fire (15,000 l/min)	146.2	69.9
Max Day plus Fire (18,000 l/min)	145.9	69.5

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Private Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	70.0
Peak Hour	146.0	67.5
Max Day plus Fire (12,000 l/min)	145.2	66.5
Max Day plus Fire (15,000 l/min)	144.6	65.5
Max Day plus Fire (18,000 l/min)	143.8	64.5

¹ Ground Elevation = 108.2 m

SCENARIO 2

POST-configured Zone

Connection 1 - Greenbank Road and Jockvale Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	72.1
Peak Hour	146.3	70.0
Max Day plus Fire (12,000 l/min)	146.4	70.2
Max Day plus Fire (15,000 l/min)	146.3	69.9
Max Day plus Fire (18,000 l/min)	145.9	69.5

¹ Ground Elevation = 112.7 m

Connection 2 - Jockvale Road and Bren-Maur Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.7	79.2
Peak Hour	145.7	76.2
Max Day plus Fire (12,000 l/min)	145.4	75.9
Max Day plus Fire (15,000 l/min)	144.9	75.1
Max Day plus Fire (18,000 l/min)	144.3	74.2

¹ Ground Elevation = 108.2 m

Notes:

- 1) As per the Ontario Building Code in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi.) Pressure control measures to be considered are as follows, in order of preference:
 - a) If possible, systems to be designed to residual pressures of 345 to 552 kPa (50 to 80 psi) in all occupied areas outside of the public right-of-way without special pressure control equipment.

- b) Pressure reducing valves to be installed immediately downstream of the isolation valve in the home/ building, located downstream of the meter so it is owner maintained.

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.2 WATER DEMAND CALCULATIONS



SNTC Lands Block 4 - Domestic Water Demand Estimates

- Based on Proposed Site Plan (160401085)

Densities as per City Guidelines:

B2B units 2.7 ppu

Building ID	Units	Population ¹	Daily Rate of Demand ² (L/m ² /day)	Avg Day Demand		Max Day Demand ³		Peak Hour Demand ³	
				(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Area 1 - Junction 23	39	105.3	350	25.6	0.43	64.0	1.07	140.8	2.35
Area 2 - Junction 24	8	21.6	350	5.3	0.09	13.1	0.22	28.9	0.48
Area 3 - Junction 25	36	97.2	350	23.6	0.39	59.1	0.98	129.9	2.17
Area 4 - Junction 26	16	43.2	350	10.5	0.18	26.3	0.44	57.8	0.96
Area 7 - Junction 7	17	45.9	350	11.2	0.19	27.9	0.46	61.4	1.02
Total Site :		313.2		76.1	1.27	190.3	3.17	418.7	6.98

1 Population counts based on a conversion factor of 2.7 persons/back-to-back townhome unit

2 Average day water demand for residential areas equal to 350 L/cap/d

3 The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

 maximum day demand rate = 2.5 x average day demand rate

 peak hour demand rate = 2.2 x maximum day demand rate

Referenced from the City of Ottawa Design Guidelines: Water Distribution (July 2010)

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.3 FUS CALCULATIONS





FUS Fire Flow Calculation Sheet

Stantec Project #: 160401085

Project Name: Nepean Town Centre Development Corporation - SNTC Lands Block 4

Date: 7/30/2020

Fire Flow Calculation #: 2

Description: Residential Back to Back Townhome Units (Block 5)

Notes: Worst case townhome units

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Wood Frame	1.5	-					
2	Determine Ground Floor Area of One Unit	-	456.0	-					
	Determine Number of Adjoining Units	Includes adjacent wood frame structures separated by 3m or less	1	-					
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space	3	-					
4	Determine Required Fire Flow	($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min	-	12000					
5	Determine Occupancy Charge	Limited Combustible	-15%	10200					
6	Determine Sprinkler Reduction	None	0%	0					
		Non-Standard Water Supply or N/A	0%						
		Not Fully Supervised or N/A	0%						
		% Coverage of Sprinkler System	0%						
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	20.1 to 30	14	3	31-60	Wood Frame or Non-Combustible	8%	3672
		East	30.1 to 45	28	3	61-90	Wood Frame or Non-Combustible	5%	
		South	3.1 to 10	14	3	31-60	Wood Frame or Non-Combustible	18%	
		West	30.1 to 45	28	3	61-90	Wood Frame or Non-Combustible	5%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							14000
		Total Required Fire Flow in L/s							233.3
		Required Duration of Fire Flow (hrs)							3.00
		Required Volume of Fire Flow (m ³)							2520



FUS Fire Flow Calculation Sheet

Stantec Project #: 160401085
 Project Name: Nepean Town Centre Development Corporation - SNIC Lands Block 4
 Date: 7/30/2020
 Fire Flow Calculation #: 5
 Description: Residential Back to Back Townhome Units (Block 10)

Notes: Worst case townhome units

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Wood Frame	1.5	-					
2	Determine Ground Floor Area of One Unit	-	58.0	-					
	Determine Number of Adjoining Units	Includes adjacent wood frame structures separated by 3m or less	6	-					
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space	3	-					
4	Determine Required Fire Flow	($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min	-	11000					
5	Determine Occupancy Charge	Limited Combustible	-15%	9350					
6	Determine Sprinkler Reduction	None	0%	0					
		Non-Standard Water Supply or N/A	0%						
		Not Fully Supervised or N/A	0%						
		% Coverage of Sprinkler System	0%						
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	20.1 to 30	20	3	31-60	Wood Frame or Non-Combustible	8%	2151
		East	10.1 to 20	33	3	91-120	Wood Frame or Non-Combustible	15%	
		South	3.1 to 10	20	3	31-60	Ordinary or Fire Resistive (Blank Wall)	0%	
		West	> 45	33	3	91-120	Wood Frame or Non-Combustible	0%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							12000
		Total Required Fire Flow in L/s							200.0
		Required Duration of Fire Flow (hrs)							2.50
		Required Volume of Fire Flow (m ³)							1800



FUS Fire Flow Calculation Sheet

Stantec Project #: 160401085
 Project Name: Nepean Town Centre Development Corporation - SNIC Lands Block 4
 Date: 7/30/2020
 Fire Flow Calculation #: 1
 Description: Residential Back to Back Townhome Units (Block 11)

Notes: Worst case townhome units

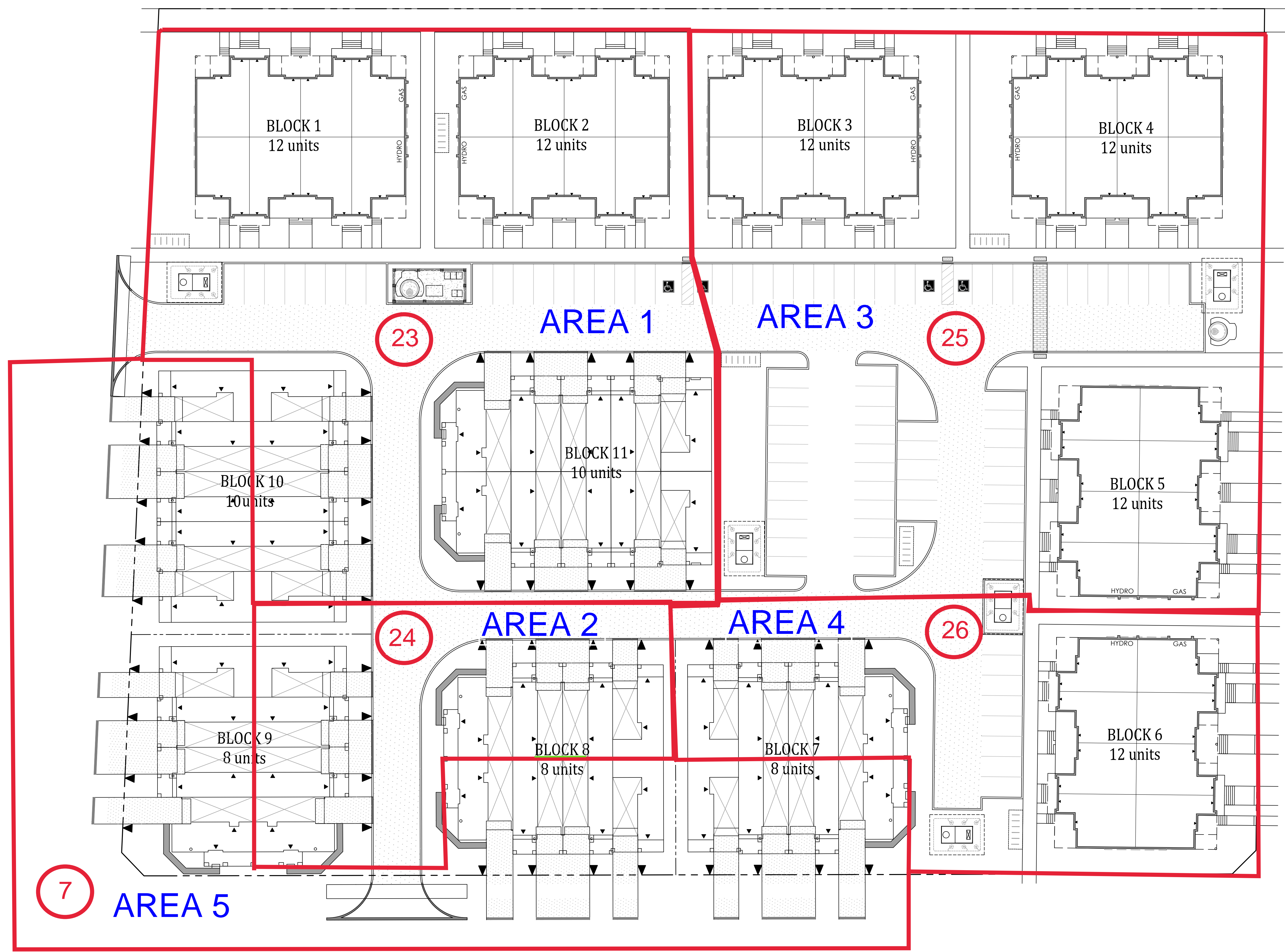
Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Wood Frame	1.5	-					
2	Determine Ground Floor Area of One Unit	-	58.0	-					
	Determine Number of Adjoining Units	Includes adjacent wood frame structures separated by 3m or less	6	-					
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space	3	-					
4	Determine Required Fire Flow	($F = 220 \times C \times A^{1/2}$). Round to nearest 1000 L/min	-	11000					
5	Determine Occupancy Charge	Limited Combustible	-15%	9350					
6	Determine Sprinkler Reduction	None	0%	0					
		Non-Standard Water Supply or N/A	0%						
		Not Fully Supervised or N/A	0%						
		% Coverage of Sprinkler System	0%						
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	20.1 to 30	33	3	91-120	Wood Frame or Non-Combustible	10%	2805
		East	30.1 to 45	20	3	31-60	Wood Frame or Non-Combustible	5%	
		South	10.1 to 20	33	3	91-120	Wood Frame or Non-Combustible	15%	
		West	10.1 to 20	20	3	31-60	Ordinary or Fire Resistive (Blank Wall)	0%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							12000
		Total Required Fire Flow in L/s							200.0
		Required Duration of Fire Flow (hrs)							2.50
		Required Volume of Fire Flow (m ³)							1800

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.3 HYDRAULIC ANALYSIS





EXISTING CONDITIONS

Hydraulic Model Results - Average Day Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
7	0.19	93.51	157.31	90.69	625.29
20	0.00	93.48	157.31	90.74	625.63
21	0.00	93.50	157.31	90.71	625.43
23	0.43	93.73	157.31	90.38	623.15
24	0.09	93.95	157.31	90.07	621.01
25	0.39	93.94	157.31	90.08	621.08
26	0.18	93.97	157.31	90.04	620.81

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
20	7	21	43.3	204	110	0.16	0.00
50	20	7	76.8	204	110	0.73	0.02
54	25	23	72.2	204	110	-0.32	0.01
56	23	20	40.0	204	110	-1.02	0.03
58	25	26	32.2	204	110	-0.07	0.00
60	26	24	72.2	204	110	-0.25	0.01
62	23	24	32.2	204	110	0.28	0.01
64	24	21	47.6	204	110	-0.07	0.00

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
7	1.02	93.51	139.65	65.59	452.23
20	0.00	93.48	139.65	65.63	452.51
21	0.00	93.50	139.65	65.61	452.37
23	2.35	93.73	139.65	65.27	450.02
24	0.48	93.95	139.65	64.96	447.89
25	2.17	93.94	139.64	64.97	447.96
26	0.96	93.97	139.64	64.93	447.68

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
20	7	21	43.26	204	110	-0.32	0.01
50	20	7	76.79	204	110	-1.30	0.04
54	25	23	72.20	204	110	-1.52	0.05
56	23	20	40.00	204	110	-2.88	0.09
58	25	26	32.20	204	110	-0.65	0.02
60	26	24	72.20	204	110	-1.61	0.05
62	23	24	32.20	204	110	-0.99	0.03
64	24	21	47.60	204	110	-3.08	0.09

Hydraulic Model Results -Fire Flow Analysis 233L/s

ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
7	0.46	57.69	397.76	134.09	233	37.15	256.14	327.08	20	137.90
20	0.00	57.74	398.11	134.10	233	36.78	253.59	323.2	20	137.90
23	1.07	57.38	395.62	134.09	233	33.33	229.80	298.78	20	137.90
24	0.22	57.06	393.42	134.09	233	33.03	227.74	296.59	20	137.90
25	0.98	57.08	393.56	134.09	233	24.48	168.78	251.19	20	137.90
26	0.44	57.03	393.21	134.09	233	24.45	168.58	250.54	20	137.90

RECONFIGURATION CONDITIONS

Hydraulic Model Results - Average Day Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
7	0.19	93.51	147.63	76.94	530.49
20	0.00	93.48	147.63	76.98	530.76
21	0.00	93.50	147.63	76.95	530.55
23	0.43	93.73	147.63	76.63	528.35
24	0.09	93.95	147.63	76.31	526.14
25	0.39	93.94	147.63	76.33	526.28
26	0.18	93.97	147.63	76.29	526.00

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
20	7	21	43.26	204	110	0.26	0.01
50	20	7	76.79	204	110	1.25	0.04
54	25	23	72.20	204	110	-0.46	0.01
56	23	20	40.00	204	110	-1.57	0.05
58	25	26	32.20	204	110	0.07	0.00
60	26	24	72.20	204	110	-0.11	0.00
62	23	24	32.20	204	110	0.68	0.02
64	24	21	47.60	204	110	0.48	0.01

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
7	1.02	93.51	144.45	72.42	499.32
20	0.00	93.48	144.45	72.46	499.60
21	0.00	93.50	144.45	72.43	499.39
23	2.35	93.73	144.45	72.10	497.12
24	0.48	93.95	144.45	71.79	494.98
25	2.17	93.94	144.45	71.80	495.05
26	0.96	93.97	144.45	71.76	494.77

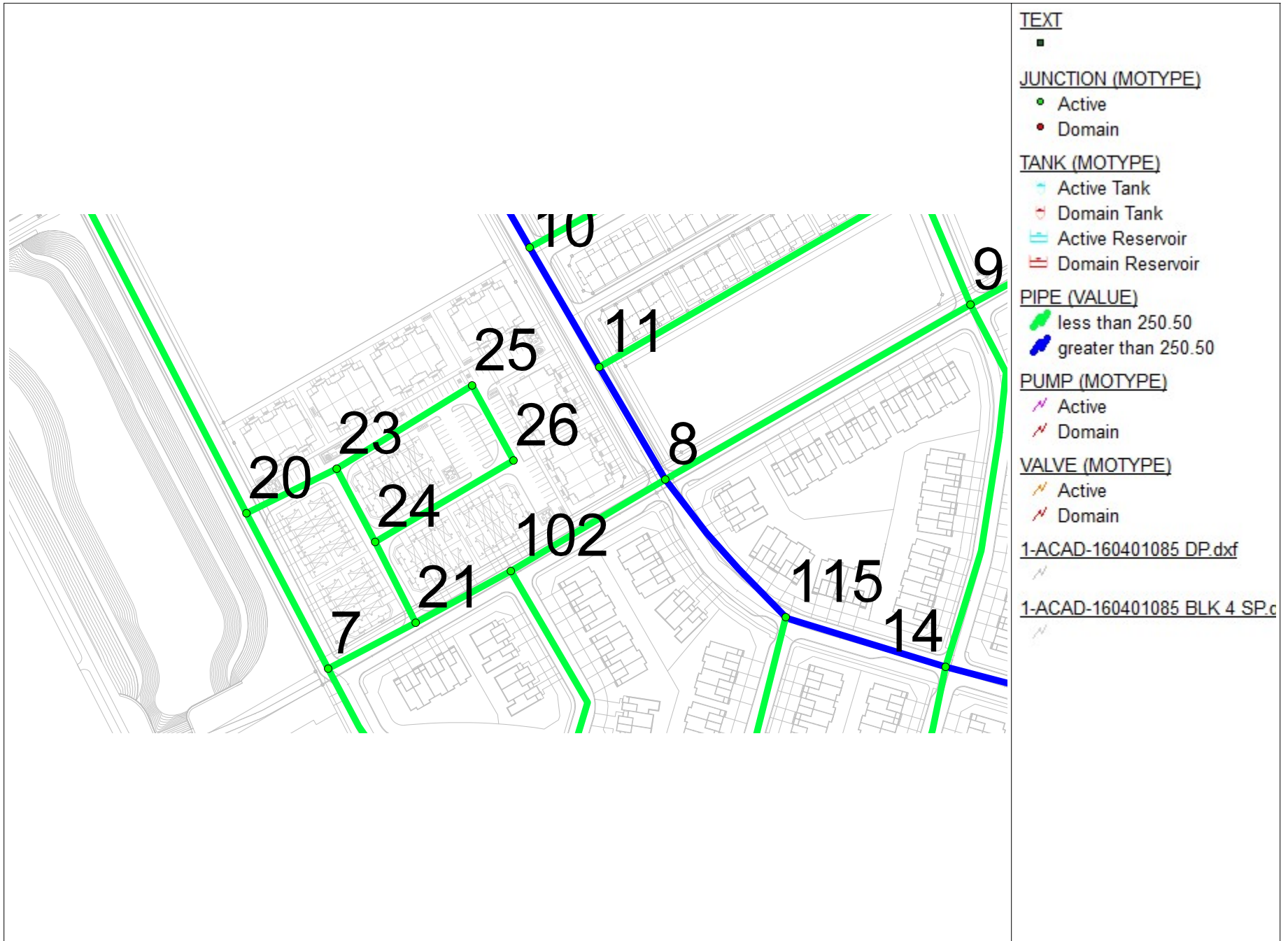
Pipe Results

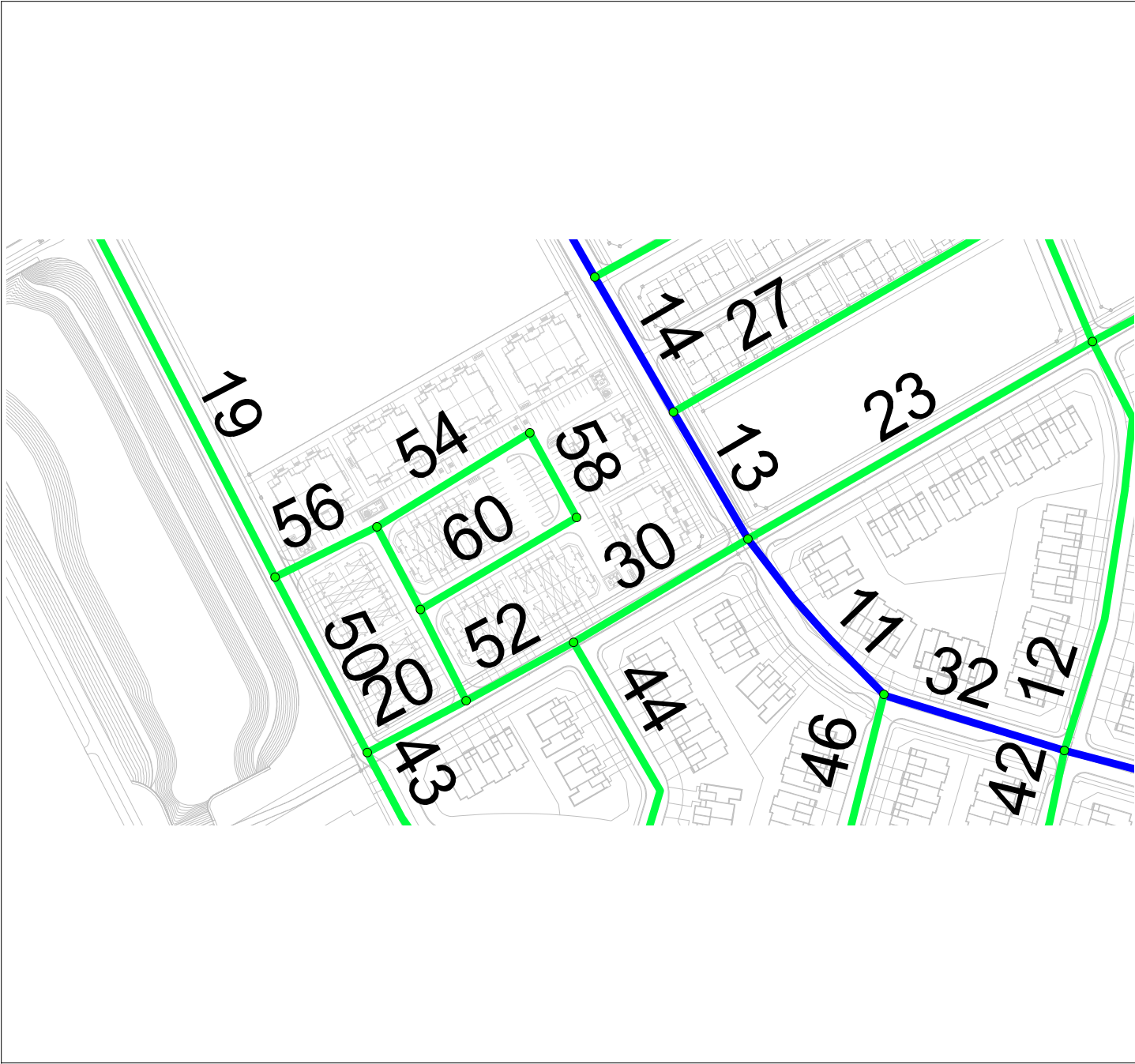
ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
20	7	21	43.26	204	110	0.39	0.01
50	20	7	76.79	204	110	0.07	0.00
54	25	23	72.20	204	110	-1.55	0.05
56	23	20	40.00	204	110	-3.18	0.10
58	25	26	32.20	204	110	-0.62	0.02
60	26	24	72.20	204	110	-1.58	0.05
62	23	24	32.20	204	110	-0.72	0.02
64	24	21	47.60	204	110	-2.78	0.08

Hydraulic Model Results -Fire Flow Analysis 233 L/s

ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
7	0.46	73.82	141.33	145.44	233	53.23	367.01	398.09	20	137.90
20	0.00	73.87	141.39	145.44	233	52.86	364.46	393.39	20	137.90
23	1.07	73.51	141.36	145.44	233	49.41	340.67	363.97	20	137.90
24	0.22	73.19	141.35	145.44	233	49.11	338.60	361.99	20	137.90
25	0.98	73.21	141.35	145.44	233	40.57	279.72	306.21	20	137.90
26	0.44	73.16	141.35	145.44	233	40.54	279.52	305.58	20	137.90

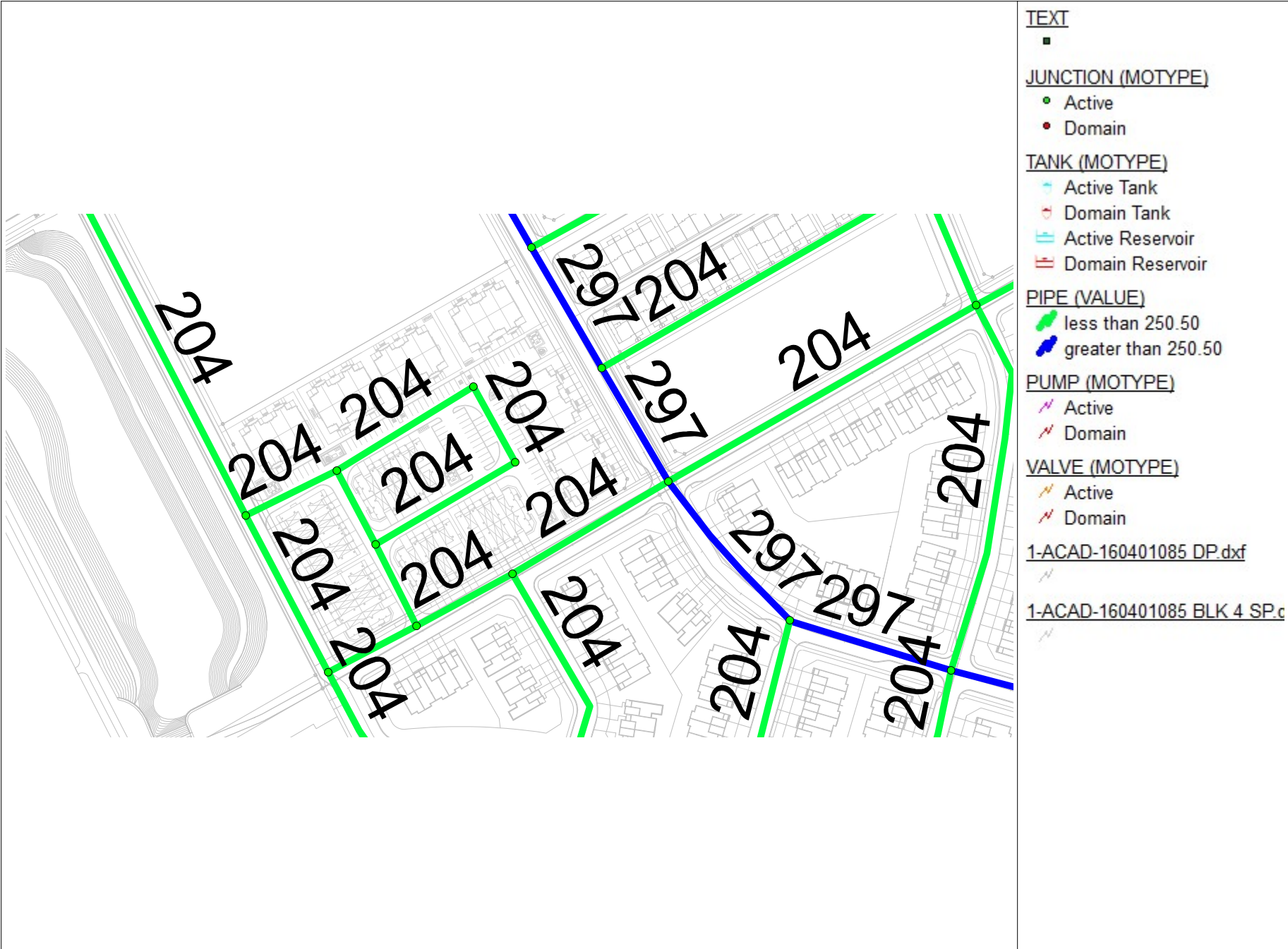
160401085_SNTC_Block4-2020-07-30- JUNCTION ID





- TEXT
-
- JUNCTION (MOTYPE)
- Active
- Domain
- TANK (MOTYPE)
- Active Tank
- Domain Tank
- Active Reservoir
- Domain Reservoir
- PIPE (VALUE)
- less than 250.50
- greater than 250.50
- PUMP (MOTYPE)
- Active
- Domain
- VALVE (MOTYPE)
- Active
- Domain
- 1-ACAD-160401085 DP.dxf
- 1-ACAD-160401085 BLK 4 SP.c

160401085_SNTC_Block4-2020-07-30-PIPE DIAMETER



SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.5 BACKGROUND REPORT EXCERPTS



Potable Water

3.2.1 Ground Elevations

The proposed ground elevations of the development range from approximately 95.50m to 93.30m. Proposed grading and elevations have been determined for the site and are included on **Drawing GP-1** and **Drawing GP-2**.

3.2.2 Water Demands

The current subdivision plan for the development consists of four public roadways with two rows of rear-lane townhomes, 4 blocks intended for future residential development, a community park block and a school block. The residential blocks lie within CDP areas noted as mid-rise residential and mid-rise mixed-use areas (2-4 and 4-6 storeys buildings), as well as high density mixed-use areas. Net unit density targets have been applied to each block to develop estimated domestic demand rates for the region in consideration with an average townhouse unit population density of 2.7ppu and average apartment population density of 1.8ppu.

The contributing area was assessed at a residential density of 100 units/ha for mid-rise 2-4 storey residential areas (Block 2), 200 units/ha for mid-rise 4-6 storey residential areas, and 250 units/ha for high-rise residential areas (Block 3). A residential density of 140 units/ha was assumed for Blocks 1 and 3. Detailed design for Block 4 is currently under review at the City of Ottawa and as such, the actual unit count of 116 townhomes was used in the calculations in accordance with the proposed site plan.

Water demands for the development were estimated using the City of Ottawa's Water Distribution Design Guidelines. For residential developments, the average day (AVDY) per capita water demand is 350 L/cap/day. For maximum day (MXDY) demand, AVDY was multiplied by a factor of 2.5 and for peak hour (PKHR) demand, MXDY was multiplied by a factor of 2.2. For commercial and institutional use, the AVDY is based on the area of land use at 28,000 L/ha/day as shown in the following tables. For institutional use, AVDY was multiplied by a factor of 1.5 for MXDY demand and MXDY was multiplied by a factor of 1.8 for PKHR demand (see detailed calculations in **Appendix A.2**). The calculated domestic water consumption for the proposed SNTC Development is represented in **Table 3.1** and **Table 3.2**.

A 300mm watermain connection through Claridge's Burnett Lands located to the south of the site is required to maintain looping. As such, water demands for Claridge's development to the south have also been included in the hydraulic model. The water demands for Claridge's Burnett Lands Development were taken from the latest Novatech Site Servicing and Stormwater Management Report completed in June 2020(see **Appendix A.5**). Claridge's domestic demands are represented in **Table 3.3**.

Table 3.1: SNTC Development Residential Water Demands

Area ID	Units	Person/Unit	Population	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Block 1	225	2.7	608	2.46	6.15	13.54



Potable Water

Area ID	Units	Person/Unit	Population	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Block 2	172	2.7	464	1.88	4.70	10.35
Block 3	310	1.8	558	2.26	5.65	12.43
Block 4	116	2.7	313	1.27	3.17	6.98
Block 8-15	42	2.7	113	0.46	1.15	2.53
		Total	2,056	8.33	20.82	45.83

Table 3.2: SNTC Development Institutional Water Demands

Area ID	Area (ha)	Demand (L/ha/day)	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Block 5	0.62	28,000	0.20	0.30	0.54
Block 6	1.62	28,000	0.52	0.79	1.42
		Total	0.72	1.09	1.96

Table 3.3: Claridge’s Burnett Lands Water Demands

Area ID	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Claridge Homes (3370 Greenbank Road)	4.53	11.33	24.93

3.2.3 Fire Flow Requirements

As part of the Kennedy-Burnett Potable Water Master Servicing Study, an assessment using the City’s 2013 Water Master Plan Update model was carried out. The MSS analysis concluded that under both pre and post zone reconfiguration, available fire flows in the NTC lands are projected to be greater than 15,000 L/min along all the larger diameter watermain (305mm and greater). Background report excerpts are included in **Appendix A.5**.

A maximum fire flow of 16,000 L/min (267 L/s) was estimated for the worst-case townhome units (Block 10) within the proposed Blocks 8 to 15. FUS calculations can be found in **Appendix A.3**. A fire flow requirement of 10,000 L/min has been assumed for the future development blocks. However, it is recommended that the maximum fire flow requirement assumption be revisited at the detailed design stage of each block as development proceeds to ensure sufficient fire flows are available within the adjacent watermains.

As per the City’s Technical Bulletin ISTB-2018-02, the maximum flow contribution from one given hydrant is 5,700 L/min (95 L/s) within a distance of 75 m, and 3,800 L/min (63 L/s) between 75 m and 150 m. As a result, hydrant placement in the vicinity of the townhome units within Block 10 was considered to ensure the maximum required fire flow of 16,000 L/min can be achieved.



From: Bougadis, John
To: Rathnasooriya, Thakshika
Subject: Re: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)
Date: Friday, February 14, 2020 5:22:44 PM
Attachments: image001.png

No, the bes did not include the claridge lands to the south or future development to the north. The 305 mm watermain was sized in past master planning studies (kevin Alemany did the work as part of the kB water servicing update).

Fire demands govern the sizing of local water mains that you will be designing. Also, the domestic demands from claridge and your development are relatively small compared to the entire pressure zone (hgls will not significantly decrease during peak hour).

John

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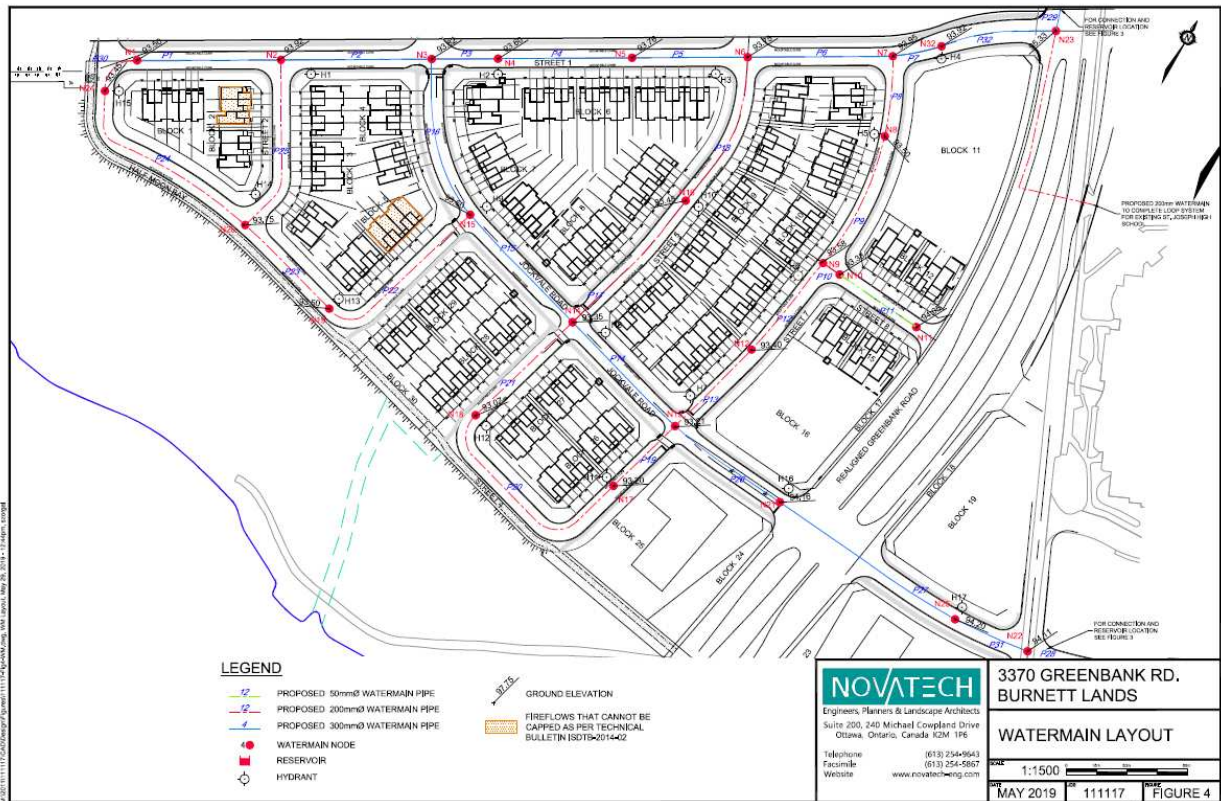
From: Rathnasooriya, Thakshika <Thakshika.Rathnasooriya@stantec.com>
Sent: Friday, February 14, 2020 3:15:03 PM
To: Bougadis, John <John.Bougadis@ottawa.ca>
Subject: RE: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Hi John,

To clarify, did the boundary conditions provided include demands from the development to the south, Burnett Lands (3370 Greenbank Road)?



Thanks,

Shika Rathnasooriya, P.Eng.

Direct: 613 724-4081
Thakshika.Rathnasooriya@stantec.com

Stantec
400 - 1331 Clyde Avenue
Ottawa ON K2C 3G4



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From: Bougadis, John <John.Bougadis@ottawa.ca>
Sent: Wednesday, February 12, 2020 12:31 PM
To: Rathnasooriya, Thakshika <Thakshika.Rathnasooriya@stantec.com>
Cc: Shillington, Jeffrey <jeff.shillington@ottawa.ca>; Simard, Lyndsey <lyndsey.simard@ottawa.ca>
Subject: RE: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)

Hi Shika,

I have attached boundary conditions for the SNTC development.

Thanks

John
x14990

From: Rathnasooriya, Thakshika <Thakshika.Rathnasooriya@stantec.com>
Sent: February 04, 2020 10:16 AM
To: Shillington, Jeffrey <jeff.shillington@ottawa.ca>
Subject: RE: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)

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Hi Jeff,

As per my voice mail, we are leaning towards going with conservative population counts which were used during draft plan. If revising the demands will trigger an additional 2 weeks wait, please continue to use the values provided in the previous request.

The connection locations and pipes diameters will remain the same, however, the demands have increased. Please see revised domestic demands below:

Estimated domestic demands and fire flow requirements for the site are as follows:
Average Day Demand - 9.51L/s
Max Day Demand - 23.78 L/s
Peak Hour Demand - 52.32L/s
Fire Flow Requirement per FUS for townhome units - 267 L/s

Thank you,

Shika Rathnasooriya, P.Eng.

Direct: 613 724-4081
Thakshika.Rathnasooriya@stantec.com

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From: Rathnasooriya, Thakshika
Sent: Friday, January 31, 2020 2:36 PM
To: Shillington, Jeffrey <jeff.shillington@ottawa.ca>
Subject: RE: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)

Good afternoon Jeff,

Could you please provide me with a status update of the boundary conditions request for South Nepean Town Centre.

Thank you,

Shika Rathnasooriya, P.Eng.

Direct: 613 724-4081
Thakshika.Rathnasooriya@stantec.com

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From: Rathnasooriya, Thakshika
Sent: Wednesday, January 22, 2020 1:35 PM
To: Shillington, Jeffrey <jeff.shillington@ottawa.ca>
Cc: Paerez, Ana <Ana.Paerez@stantec.com>
Subject: Hydraulic Boundary Conditions - South Nepean Town Centre (SNTC)

Hi Jeff,

I am looking for watermain hydraulic boundary conditions for the South Nepean Town Centre. The development is located southwest of the intersection of future extension of Chapman Mills Drive and Greenbank Road. The development consists of townhomes, back-to-back units, apartment buildings, a school and a park.

We anticipate connecting to the existing twinning 400mm watermains on Jockvale Road south of the intersection with Greenbank Road (connection 1), and a 300mm diameter main exits at the intersection of Bending Way and Greenbank Road to maintain looping (connection 2). The development for Burnett Lands by Claridge to the south will require the extension of the Jockvale Road realignment (stated as "proposed Jockvale Rd" in proposed connection plan) to Street B.

Greenbank Road is proposed to be widened within the near future and minimal construction of the road prior to the widening is recommended through pre-consultation meetings with the City.

Estimated domestic demands and fire flow requirements for the site are as follows:
Average Day Demand - 7.07L/s

Max Day Demand - 16.95L/s
Peak Hour Demand - 36.85L/s
Fire Flow Requirement per FUS for townhome units - 267 L/s

Thank you.

Shika Rathnasooriya, P.Eng.

Direct: 613 724-4081
Thakshika.Rathnasooriya@stantec.com

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400 - 1331 Clyde Avenue
Ottawa ON K2C 3G4



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,

SOUTH NEPEAN TOWN CENTRE (SNTC) – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.2 WATER DEMAND CALCULATIONS



South Nepean Town Centre - Domestic Water Demand Estimates

Densities as per City Guidelines:

Apartments	1.8	ppu
Townhomes	2.7	ppu
Mid-Rise (2-4)	100	units/ha
Mid-Rise (4-6)	200	units/ha
High Density	250	units/ha
Block 1 & 4	140	units/ha

Area ID	Residential Area	# of Units	Population	Institutional Area (ha)	Daily Rate of Demand (L/cap/day)	Daily Rate of Demand (L/ha/day)	Avg Day Demand		Max Day Demand ¹		Peak Hour Demand ²	
							(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Block 1	1.60	225	608	0.00	350	0	147.7	2.46	369.1	6.15	812.1	13.54
Block 2	1.72	172	464	0.00	350	0	112.9	1.88	282.2	4.70	620.8	10.35
Block 3	1.24	310	558	0.00	350	0	135.6	2.26	339.1	5.65	745.9	12.43
Block 4	1.59	116	313	0.00	350	0	76.1	1.27	190.3	3.17	418.7	6.98
Block 7	0.00	0	0	0.00	350	0	0.0	0.00	0.0	0.00	0.0	0.00
Block 8	0.00	0	0	0.00	350	0	0.0	0.00	0.0	0.00	0.0	0.00
Block 9-16	0.78	42	113	0.00	350	0	27.6	0.46	68.9	1.15	151.6	2.53
Block 5	0.00	0	0	0.62	0	28000	12.1	0.20	18.1	0.30	32.6	0.54
Block 6	0.00	0	0	1.62	0	28000	31.5	0.52	47.2	0.79	85.0	1.42
Claridge Homes (3370 Greenbank Road)								4.53		11.33		24.93
Total Site :			2057				543	13.59	1315	33.25	2867	72.71

Average day water demand for residential areas: 350 L/cap/d

The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

- 1 maximum day demand rate = 2.5 x average day demand rate for residential
- 2 peak hour demand rate = 2.2 x maximum day demand rate for residential

Water demand criteria used to estimate peak demand rates for commercial/institutional areas are as follows:

- 1 maximum day demand rate = 1.5 x average day demand rate
- 2 peak hour demand rate = 1.8 x maximum day demand rate

SOUTH NEPEAN TOWN CENTRE (SNTC) – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.3 FUS CALCULATIONS





FUS Fire Flow Calculation Sheet

Stantec Project #: 160401085
 Project Name: Nepean Town Centre Development Corporation
 Date: 2/19/2020
 Fire Flow Calculation #: 1
 Description: Residential Townhome Units (Block 10)

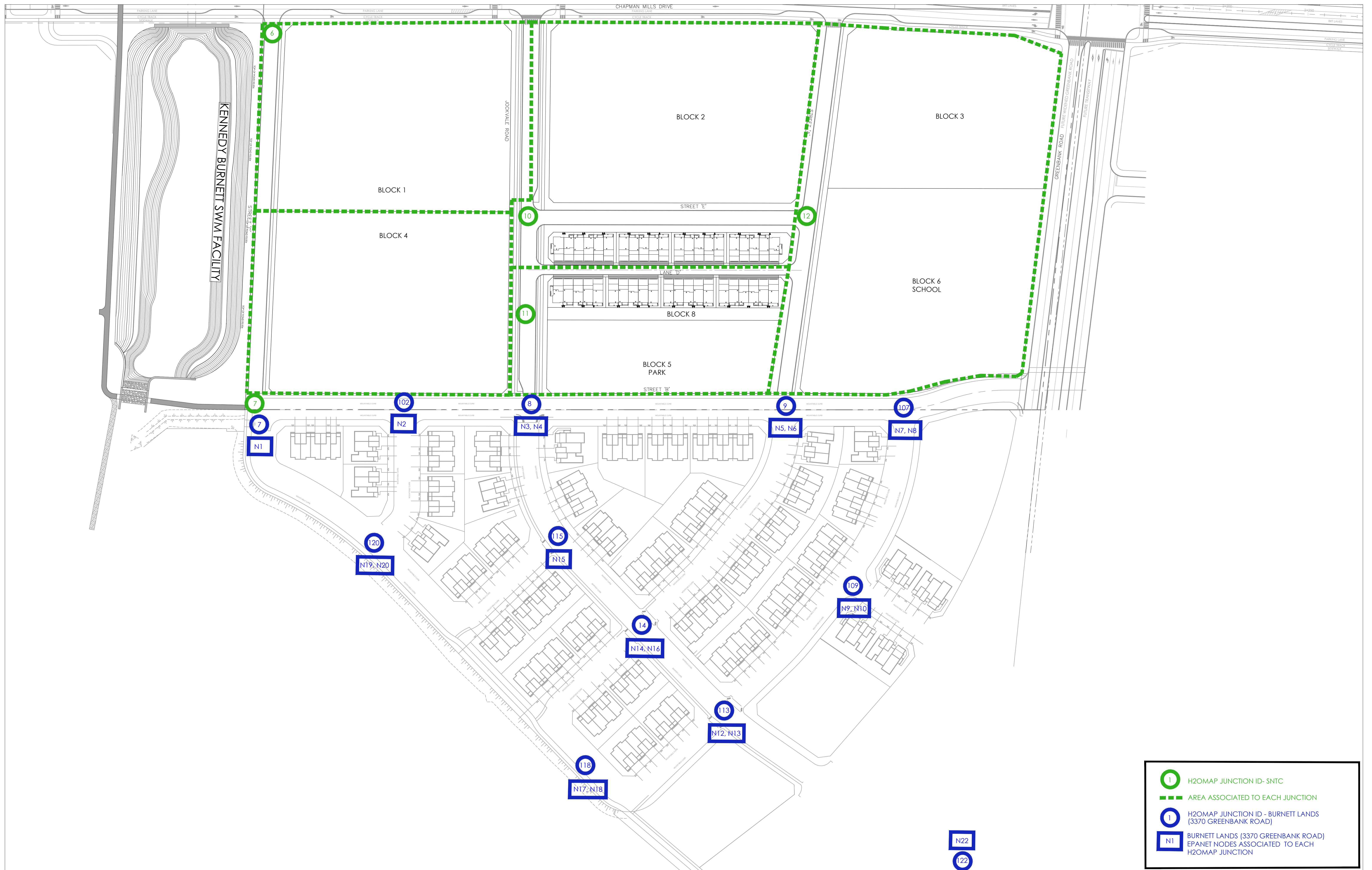
Notes: Worst case townhome units

Step	Task	Notes	Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Wood Frame	1.5	-					
2	Determine Ground Floor Area of One Unit	-	447.6	-					
	Determine Number of Adjoining Units	Includes adjacent wood frame structures separated by 3m or less	1	-					
3	Determine Height in Storeys	Does not include floors >50% below grade or open attic space	3	-					
4	Determine Required Fire Flow	(F = 220 x C x A ^{1/2}). Round to nearest 1000 L/min	-	12000					
5	Determine Occupancy Charge	Limited Combustible	-15%	10200					
6	Determine Sprinkler Reduction	None	0%	0					
		Non-Standard Water Supply or N/A	0%						
		Not Fully Supervised or N/A	0%						
		% Coverage of Sprinkler System	0%						
7	Determine Increase for Exposures (Max. 75%)	Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	-	-
		North	20.1 to 30	30	3	61-90	Wood Frame or Non-Combustible	9%	6018
		East	3.1 to 10	12	3	31-60	Wood Frame or Non-Combustible	18%	
		South	10.1 to 20	30	3	61-90	Wood Frame or Non-Combustible	14%	
		West	3.1 to 10	12	3	31-60	Wood Frame or Non-Combustible	18%	
8	Determine Final Required Fire Flow	Total Required Fire Flow in L/min, Rounded to Nearest 1000L/min							16000
		Total Required Fire Flow in L/s							266.7
		Required Duration of Fire Flow (hrs)							3.50
		Required Volume of Fire Flow (m ³)							3360

Appendix A Potable Water analysis

A.4 ULTIMATE CONDITIONS HYDRAULIC ANALYSIS RESULTS





EXISTING CONDITIONS
Hydraulic Model Results - Average Day Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	157.38	88.53	610.40
4	0.00	94.21	157.34	89.74	618.74
5	0.00	93.15	157.32	91.22	628.94
6	2.46	93.93	157.31	90.10	621.22
7	0.19	93.51	157.31	90.69	625.29
8	0.15	93.82	157.31	90.25	622.26
9	0.15	93.74	157.31	90.36	623.01
10	2.11	93.82	157.31	90.26	622.32
11	0.43	93.61	157.31	90.55	624.32
12	2.78	94.11	157.31	89.84	619.43
13	0.00	93.99	157.31	90.01	620.60
14	0.43	93.35	157.30	90.92	626.88
15	0.00	95.33	157.30	88.10	607.43
20	0.00	93.48	157.31	90.74	625.63
21	0.00	93.50	157.31	90.71	625.43
102	0.08	93.92	157.31	90.11	621.29
107	0.38	93.95	157.30	90.06	620.95
109	0.23	93.58	157.30	90.59	624.60
113	1.13	93.21	157.30	91.11	628.19
115	0.20	93.60	157.31	90.56	624.39
118	0.15	92.82	157.30	91.67	632.05
120	0.20	93.75	157.31	90.35	622.95
122	1.39	94.11	157.30	89.83	619.36

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
10	14	113	76.07	297	120	3.46	0.05
11	8	115	80.12	297	120	3.25	0.05
12	9	14	165.53	204	110	0.96	0.03
13	11	8	57.06	297	120	4.38	0.06
14	10	11	60.55	297	120	4.77	0.07
15	5	10	125.25	297	120	6.48	0.09
16	4	12	128.80	204	110	5.03	0.15
17	12	13	57.59	204	110	1.85	0.06
18	13	9	57.74	204	110	1.81	0.06
19	6	20	167.03	204	110	1.76	0.05
20	7	21	43.26	204	110	0.16	0.00
21	6	5	158.01	297	120	-4.22	0.06
22	4	5	186.77	297	120	10.70	0.15
23	9	8	153.93	204	110	-0.57	0.02
24	3	4	146.18	297	120	15.73	0.23
25	1000	3	92.09	297	120	15.73	0.23
26	10	12	170.91	204	110	-0.40	0.01
27	11	13	161.29	204	110	-0.04	0.00
28	9	107	72.78	204	110	1.27	0.04
30	102	8	78.61	204	110	-0.42	0.01
32	115	14	73.28	297	120	3.66	0.05
34	122	113	199.82	297	120	-3.57	0.05
36	107	15	78.35	204	110	0.00	0.00
38	113	109	106.78	204	110	-0.66	0.02
39	109	107	116.92	204	110	-0.89	0.03
41	118	113	114.97	204	110	0.58	0.02
42	14	118	109.64	204	110	0.73	0.02
43	7	120	128.07	204	110	0.38	0.01
44	120	102	92.43	204	110	-0.43	0.01
46	120	115	154.08	204	110	0.61	0.02
48	122	1001	10.00	297	120	2.18	0.03
50	20	7	76.79	204	110	0.73	0.02
52	21	102	47.41	204	110	0.10	0.00

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	140.06	63.91	440.65
4	0.00	94.21	139.83	64.86	447.20
5	0.00	93.15	139.69	66.16	456.16
6	13.54	93.93	139.65	65.00	448.16
7	1.02	93.51	139.65	65.59	452.23
8	0.85	93.82	139.67	65.18	449.40
9	0.80	93.74	139.67	65.29	450.16
10	11.61	93.82	139.67	65.18	449.40
11	1.79	93.61	139.67	65.47	451.40
12	13.85	94.11	139.67	64.76	446.51
13	0.00	93.99	139.67	64.93	447.68
14	2.36	93.35	139.68	65.86	454.09
15	0.00	95.33	139.67	63.03	434.58
20	0.00	93.48	139.65	65.63	452.51
21	0.00	93.50	139.65	65.61	452.37
102	0.42	93.92	139.66	65.02	448.30
107	2.07	93.95	139.67	64.99	448.09
109	1.25	93.58	139.67	65.53	451.82
113	6.27	93.21	139.69	66.07	455.54
115	1.09	93.60	139.67	65.49	451.54
118	0.85	92.82	139.68	66.61	459.26
120	1.09	93.75	139.66	65.26	449.95
122	7.62	94.11	139.79	64.94	447.75

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
10	14	113	76.07	297	120	-11.16	0.16
11	8	115	80.12	297	120	-4.33	0.06
12	9	14	165.53	204	110	-2.06	0.06
13	11	8	57.06	297	120	-0.09	0.00
14	10	11	60.55	297	120	1.45	0.02
15	5	10	125.25	297	120	13.41	0.19
16	4	12	128.80	204	110	12.94	0.40
17	12	13	57.59	204	110	-0.56	0.02
18	13	9	57.74	204	110	-0.81	0.02
19	6	20	167.03	204	110	1.58	0.05
20	7	21	43.26	204	110	-0.32	0.01
21	6	5	158.01	297	120	-15.12	0.22
22	4	5	186.77	297	120	28.53	0.41
23	9	8	153.93	204	110	0.55	0.02
24	3	4	146.18	297	120	41.47	0.60
25	1000	3	92.09	297	120	41.47	0.60
26	10	12	170.91	204	110	0.35	0.01
27	11	13	161.29	204	110	-0.25	0.01
28	9	107	72.78	204	110	-0.10	0.00
30	102	8	78.61	204	110	-3.93	0.12
32	115	14	73.28	297	120	-8.39	0.12
34	122	113	199.82	297	120	23.35	0.34
36	107	15	78.35	204	110	0.00	0.00
38	113	109	106.78	204	110	3.42	0.10
39	109	107	116.92	204	110	2.17	0.07
41	118	113	114.97	204	110	-2.50	0.08
42	14	118	109.64	204	110	-1.65	0.05
43	7	120	128.07	204	110	-2.00	0.06
44	120	102	92.43	204	110	-0.11	0.00
46	120	115	154.08	204	110	-2.97	0.09
48	122	1001	10.00	297	120	-30.97	0.45
50	20	7	76.79	204	110	-1.30	0.04
52	21	102	47.41	204	110	-3.40	0.10

Hydraulic Model Results -Fire Flow Analysis 267 L/s

ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
3	0.00	67.54	465.67	142.61	167	61.71	425.48	1020.86	20	137.90
4	0.00	63.40	437.13	138.81	167	54.04	372.59	731.07	20	137.90
5	0.00	61.54	424.31	136.44	167	52.14	359.49	631.84	20	137.90
6	6.15	60.13	414.58	136.23	167	48.29	332.95	460.82	20	137.90
8	0.38	58.16	401.00	134.73	167	50.33	347.02	640.93	20	137.90
9	0.36	57.89	399.14	134.46	167	49.05	338.19	505.95	20	137.90
10	5.27	59.42	409.69	135.62	267	48.17	332.12	625.67	20	137.90
11	0.87	59.06	407.21	135.16	267	48.55	334.74	634.00	20	137.90
12	6.44	59.30	408.86	135.83	167	48.93	337.36	474.54	20	137.90
13	0.00	58.52	403.48	135.15	167	48.75	336.12	483.88	20	137.90
14	1.08	57.15	394.04	133.55	167	50.66	349.29	674.29	20	137.90
15	0.00	55.01	379.28	134.03	167	24.71	170.37	183.01	20	137.90
102	0.19	58.08	400.45	134.77	183	45.49	313.64	409.70	20	137.90
107	0.95	56.97	392.80	134.03	167	43.17	297.65	331.50	20	137.90
109	0.56	56.56	389.97	133.36	167	42.70	294.41	322.03	20	137.90
113	2.83	56.26	387.90	132.78	167	50.76	349.98	720.12	20	137.90
115	0.50	57.76	398.24	134.23	200	49.90	344.05	639.17	20	137.90
118	0.38	57.36	395.49	133.17	167	45.09	310.89	358.29	20	137.90
120	0.49	58.24	401.55	134.71	167	45.70	315.09	376.85	20	137.90
122	3.46	49.57	341.78	128.98	167	49.31	339.98	2290.76	20	137.90

RECONFIGURATION CONDITIONS
Hydraulic Model Results - Average Day Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	147.76	74.86	516.14
4	0.00	94.21	147.69	76.03	524.21
5	0.00	93.15	147.65	77.48	534.21
6	2.46	93.93	147.65	76.36	526.49
7	0.19	93.51	147.63	76.94	530.49
8	0.15	93.82	147.63	76.5	527.45
9	0.15	93.74	147.63	76.61	528.21
10	2.11	93.82	147.64	76.51	527.52
11	0.43	93.61	147.64	76.8	529.52
12	2.78	94.11	147.64	76.1	524.69
13	0.00	93.99	147.64	76.26	525.80
14	0.43	93.35	147.62	77.16	532.00
15	0.00	95.33	147.63	74.34	512.56
20	0.00	93.48	147.63	76.98	530.76
21	0.00	93.50	147.63	76.95	530.55
102	0.08	93.92	147.63	76.36	526.49
107	0.38	93.95	147.63	76.31	526.14
109	0.23	93.58	147.62	76.83	529.73
113	1.13	93.21	147.62	77.35	533.31
115	0.20	93.60	147.63	76.81	529.59
118	0.15	92.82	147.62	77.91	537.17
120	0.20	93.75	147.63	76.6	528.14
122	1.39	94.11	147.60	76.04	524.28

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow (L/s)	Velocity (m/s)
			(m)	(mm)			
10	14	113	76.07	297	120	7.35	0.11
11	8	115	80.12	297	120	6.20	0.09
12	9	14	165.53	204	110	1.91	0.06
13	11	8	57.06	297	120	7.52	0.11
14	10	11	60.55	297	120	7.94	0.11
15	5	10	125.25	297	120	9.22	0.13
16	4	12	128.80	204	110	6.84	0.21
17	12	13	57.59	204	110	3.23	0.10
18	13	9	57.74	204	110	3.22	0.10
19	6	20	167.03	204	110	2.82	0.09
20	7	21	43.26	204	110	0.26	0.01
21	6	5	158.01	297	120	-5.28	0.08
22	4	5	186.77	297	120	14.50	0.21
23	9	8	153.93	204	110	-1.08	0.03
24	3	4	146.18	297	120	21.34	0.31
25	1000	3	92.09	297	120	21.34	0.31
26	10	12	170.91	204	110	-0.83	0.03
27	11	13	161.29	204	110	-0.01	0.00
28	9	107	72.78	204	110	2.25	0.07
30	102	8	78.61	204	110	-0.08	0.00
32	115	14	73.28	297	120	7.35	0.11
34	122	113	199.82	297	120	-9.18	0.13
36	107	15	78.35	204	110	0.00	0.00
38	113	109	106.78	204	110	-1.64	0.05
39	109	107	116.92	204	110	-1.87	0.06
41	118	113	114.97	204	110	1.32	0.04
42	14	118	109.64	204	110	1.47	0.05
43	7	120	128.07	204	110	0.80	0.02
44	120	102	92.43	204	110	-0.75	0.02
46	120	115	154.08	204	110	1.35	0.04
48	122	1001	10.00	297	120	7.79	0.11
50	20	7	76.79	204	110	1.25	0.04
52	21	102	47.41	204	110	0.74	0.02

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	145.08	71.05	489.88
4	0.00	94.21	144.73	71.82	495.18
5	0.00	93.15	144.52	73.02	503.46
6	13.54	93.93	144.47	71.85	495.39
7	1.02	93.51	144.45	72.42	499.32
8	0.85	93.82	144.46	71.99	496.36
9	0.80	93.74	144.46	72.11	497.18
10	11.61	93.82	144.47	72.00	496.43
11	1.79	93.61	144.47	72.30	498.49
12	13.85	94.11	144.47	71.59	493.60
13	0.00	93.99	144.47	71.76	494.77
14	2.36	93.35	144.46	72.66	500.98
15	0.00	95.33	144.46	69.84	481.53
20	0.00	93.48	144.45	72.46	499.60
21	0.00	93.50	144.45	72.43	499.39
102	0.42	93.92	144.46	71.84	495.32
107	2.07	93.95	144.46	71.81	495.12
109	1.25	93.58	144.46	72.33	498.70
113	6.27	93.21	144.46	72.86	502.36
115	1.09	93.60	144.46	72.31	498.56
118	0.85	92.82	144.46	73.41	506.15
120	1.09	93.75	144.46	72.08	496.98
122	7.62	94.11	144.50	71.63	493.87

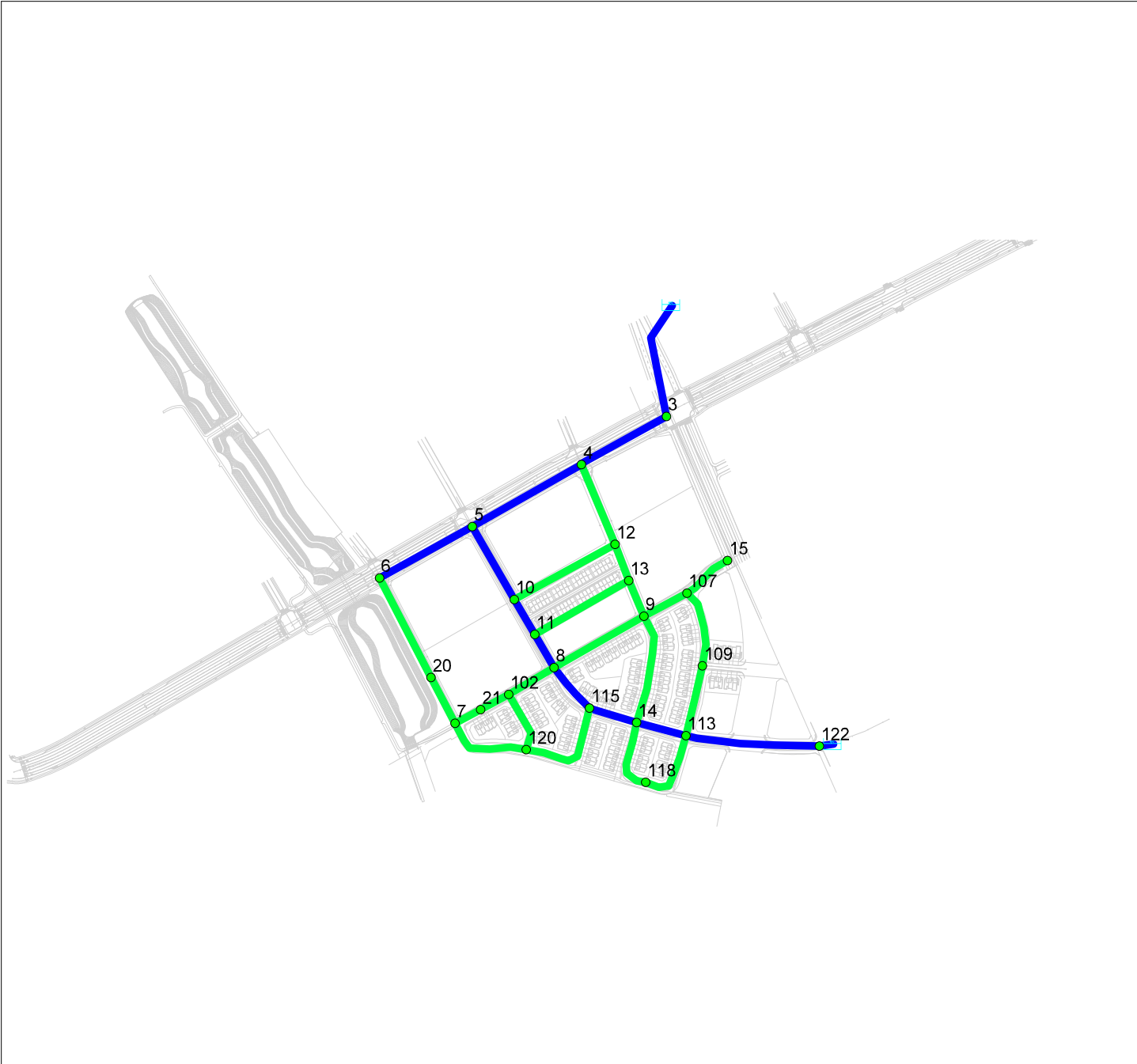
Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow (L/s)	Velocity (m/s)
			(m)	(mm)			
10	14	113	76.07	297	120	-3.46	0.05
11	8	115	80.12	297	120	1.91	0.03
12	9	14	165.53	204	110	0.11	0.00
13	11	8	57.06	297	120	6.18	0.09
14	10	11	60.55	297	120	7.64	0.11
15	5	10	125.25	297	120	19.11	0.28
16	4	12	128.80	204	110	16.59	0.51
17	12	13	57.59	204	110	2.61	0.08
18	13	9	57.74	204	110	2.28	0.07
19	6	20	167.03	204	110	3.26	0.10
20	7	21	43.26	204	110	0.39	0.01
21	6	5	158.01	297	120	-16.80	0.24
22	4	5	186.77	297	120	35.91	0.52
23	9	8	153.93	204	110	-0.31	0.01
24	3	4	146.18	297	120	52.50	0.76
25	1000	3	92.09	297	120	52.50	0.76
26	10	12	170.91	204	110	-0.13	0.00
27	11	13	161.29	204	110	-0.33	0.01
28	9	107	72.78	204	110	1.67	0.05
30	102	8	78.61	204	110	-3.12	0.10
32	115	14	73.28	297	120	-1.30	0.02
34	122	113	199.82	297	120	12.32	0.18
36	107	15	78.35	204	110	0.00	0.00
38	113	109	106.78	204	110	1.65	0.05
39	109	107	116.92	204	110	0.40	0.01
41	118	113	114.97	204	110	-0.94	0.03
42	14	118	109.64	204	110	-0.09	0.00
43	7	120	128.07	204	110	-1.34	0.04
44	120	102	92.43	204	110	-0.31	0.01
46	120	115	154.08	204	110	-2.12	0.06
48	122	1001	10.00	297	120	-19.94	0.29
50	20	7	76.79	204	110	0.07	0.00
52	21	102	47.41	204	110	-2.39	0.07

Hydraulic Model Results -Fire Flow Analysis 267 L/s

ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
3	0.00	66.46	458.23	141.85	167	60.67	418.31	1007.92	20	137.90
4	0.00	62.42	430.37	138.12	167	53.14	366.39	721.82	20	137.90
5	0.00	60.62	417.96	135.79	167	51.33	353.91	624.04	20	137.90
6	6.15	59.21	408.24	135.58	167	47.47	327.30	455.00	20	137.90
8	0.38	57.28	394.93	134.11	167	49.56	341.71	632.76	20	137.90
9	0.36	57.02	393.14	133.85	167	48.27	332.81	499.53	20	137.90
10	5.27	58.51	403.41	134.98	267	47.38	326.68	617.77	20	137.90
11	0.87	58.17	401.07	134.53	267	47.77	329.36	626.00	20	137.90
12	6.44	58.39	402.59	135.18	167	48.11	331.71	468.53	20	137.90
13	0.00	57.63	397.35	134.53	167	47.96	330.67	477.69	20	137.90
14	1.08	56.30	388.18	132.95	167	49.91	344.12	665.84	20	137.90
15	0.00	54.15	373.35	133.42	167	23.94	165.06	180.49	20	137.90
102	0.19	57.19	394.31	134.15	183	44.72	308.34	404.45	20	137.90
107	0.95	56.11	386.87	133.42	167	42.40	292.34	327.25	20	137.90
109	0.56	55.71	384.11	132.77	167	41.93	289.10	317.95	20	137.90
113	2.83	55.43	382.18	132.20	167	50.02	344.88	711.16	20	137.90
115	0.50	56.89	392.25	133.62	200	49.17	339.02	631.09	20	137.90
118	0.38	56.52	389.69	132.58	167	44.34	305.72	353.90	20	137.90
120	0.49	57.35	395.42	134.09	167	44.91	309.65	372.06	20	137.90
122	3.46	48.86	336.88	128.48	167	48.60	335.09	2260.76	20	137.90

160401085_SNTC-2020-06-2-Ultimate-JUNCTION ID



- TEXT
-
- JUNCTION (MOTYPE)
- Active
- Domain
- TANK (MOTYPE)
- Active Tank
- Domain Tank
- Active Reservoir
- Domain Reservoir
- PIPE (VALUE)
- less than 250.50
- greater than 250.50
- PUMP (MOTYPE)
- Active
- Domain
- VALVE (MOTYPE)
- Active
- Domain
- 1-ACAD-160401085 DP.dxf

160401085_SNTC-2020-06-2-Ultimate - PIPE ID



- TEXT
-
- JUNCTION (MOTYPE)
- Active
- Domain
- TANK (MOTYPE)
- ⊕ Active Tank
- ⊕ Domain Tank
- ⊕ Active Reservoir
- ⊕ Domain Reservoir
- PIPE (VALUE)
- less than 250.50
- greater than 250.50
- PUMP (MOTYPE)
- ⚡ Active
- ⚡ Domain
- VALVE (MOTYPE)
- ⚡ Active
- ⚡ Domain
- 1-ACAD-160401085 DP.dxf

160401085_SNTC-2020-06-2-Ultimate - PIPE DIAMTER



- TEXT
-
- JUNCTION (MOTYPE)
- Active
- Domain
- TANK (MOTYPE)
- ↗ Active Tank
- ↖ Domain Tank
- ↗ Active Reservoir
- ↖ Domain Reservoir
- PIPE (VALUE)
- less than 250.50
- greater than 250.50
- PUMP (MOTYPE)
- ↗ Active
- ↖ Domain
- VALVE (MOTYPE)
- ↗ Active
- ↖ Domain
- 1-ACAD-160401085 DP.dxf

SOUTH NEPEAN TOWN CENTRE (SNTC) – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A Potable Water analysis

A.5 HYDRAULIC ANALYSIS RESULTS – SINGLE FEED OPTION



EXISTING CONDITIONS
Hydraulic Model Results - Average Day Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	157.39	88.55	610.53
4	0.00	94.21	157.38	89.80	619.15
5	0.00	93.15	157.37	91.30	629.50
6	2.46	93.93	157.37	90.19	621.84
7	0.19	93.51	157.37	90.79	625.98
8	0.15	93.82	157.37	90.35	622.95
9	0.15	93.74	157.37	90.46	623.70
10	2.11	93.82	157.37	90.35	622.95
11	0.43	93.61	157.37	90.64	624.94
12	2.78	94.11	157.37	89.93	620.05
13	0.00	93.99	157.37	90.10	621.22
20	0.00	93.60	157.37	90.66	625.08

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
13	11	8	57.06	297	120	0.15	0.00
14	10	11	60.55	297	120	0.70	0.01
15	5	10	125.25	297	120	3.00	0.04
16	4	12	128.80	204	110	2.62	0.08
17	12	13	57.59	204	110	0.03	0.00
18	13	9	57.74	204	110	0.15	0.00
19	6	20	167.03	204	110	0.19	0.01
21	6	5	158.01	297	120	-2.65	0.04
22	4	5	186.77	297	120	5.65	0.08
24	3	4	146.18	297	120	8.27	0.12
25	1000	3	92.09	297	120	8.27	0.12
26	10	12	170.91	204	110	0.19	0.01
27	11	13	161.29	204	110	0.12	0.00
50	20	7	76.79	204	110	0.19	0.01

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	140.05	63.89	440.51
4	0.00	94.21	139.80	64.81	446.85
5	0.00	93.15	139.65	66.10	455.75
6	13.54	93.93	139.61	64.94	447.75
7	1.02	93.51	139.61	65.53	451.82
8	0.85	93.82	139.62	65.10	448.85
9	0.80	93.74	139.62	65.22	449.68
10	11.61	93.82	139.62	65.10	448.85
11	1.79	93.61	139.62	65.40	450.92
12	13.85	94.11	139.62	64.69	446.02
13	0.00	93.99	139.62	64.86	447.20
20	0.00	93.60	139.61	65.41	450.99

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
13	11	8	57.06	297	120	0.85	0.01
14	10	11	60.55	297	120	2.98	0.04
15	5	10	125.25	297	120	15.21	0.22
16	4	12	128.80	204	110	13.69	0.42
17	12	13	57.59	204	110	0.46	0.01
18	13	9	57.74	204	110	0.80	0.02
19	6	20	167.03	204	110	1.02	0.03
21	6	5	158.01	297	120	-14.56	0.21
22	4	5	186.77	297	120	29.77	0.43
24	3	4	146.18	297	120	43.46	0.63
25	1000	3	92.09	297	120	43.46	0.63
26	10	12	170.91	204	110	0.62	0.02
27	11	13	161.29	204	110	0.34	0.01
50	20	7	76.79	204	110	1.02	0.03

Hydraulic Model Results -Fire Flow Analysis 267 L/s

ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
3	0.00	70.89	488.77	144.96	167	67.67	466.57	804.23	20	137.90
4	0.00	72.07	496.91	144.91	167	63.76	439.61	479.92	20	137.90
5	0.00	73.52	506.91	144.87	167	61.46	423.75	394.30	20	137.90
6	6.15	72.40	499.18	144.86	167	55.47	382.45	324.94	20	137.90
8	0.38	72.56	500.29	144.86	167	56.35	388.52	327.35	20	137.90
9	0.36	72.67	501.05	144.86	167	41.49	286.07	224.23	20	137.90
10	5.27	72.56	500.29	144.86	267	42.23	291.17	370.29	20	137.90
11	0.87	72.86	502.36	144.86	267	40.07	276.27	350.89	20	137.90
12	6.44	72.15	497.46	144.86	167	56.57	390.04	340.06	20	137.90
13	0.00	72.32	498.63	144.86	167	53.35	367.84	297.03	20	137.90

RECONFIGURATION CONDITIONS**Hydraulic Model Results - Average Day Analysis****Junction Results**

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	147.79	74.91	516.49
4	0.00	94.21	147.78	76.16	525.11
5	0.00	93.15	147.77	77.65	535.38
6	2.46	93.93	147.77	76.54	527.73
7	0.19	93.51	147.77	77.14	531.86
8	0.15	93.82	147.77	76.70	528.83
9	0.15	93.74	147.77	76.81	529.59
10	2.11	93.82	147.77	76.70	528.83
11	0.43	93.61	147.77	77.00	530.90
12	2.78	94.11	147.77	76.29	526.00
13	0.00	93.99	147.77	76.46	527.18
20	0.00	93.60	147.77	77.01	530.97

Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
13	11	8	57.06	297	120	0.15	0.00
14	10	11	60.55	297	120	0.70	0.01
15	5	10	125.25	297	120	3.00	0.04
16	4	12	128.80	204	110	2.62	0.08
17	12	13	57.59	204	110	0.03	0.00
18	13	9	57.74	204	110	0.15	0.00
19	6	20	167.03	204	110	0.19	0.01
21	6	5	158.01	297	120	-2.65	0.04
22	4	5	186.77	297	120	5.65	0.08
24	3	4	146.18	297	120	8.27	0.12
25	1000	3	92.09	297	120	8.27	0.12
26	10	12	170.91	204	110	0.19	0.01
27	11	13	161.29	204	110	0.12	0.00
50	20	7	76.79	204	110	0.19	0.01

Hydraulic Model Results -Peak Hour Analysis

Junction Results

ID	Demand	Elevation	Head	Pressure	
	(L/s)	(m)	(m)	(psi)	(Kpa)
3	0.00	95.10	145.15	71.14	490.50
4	0.00	94.21	144.90	72.06	496.84
5	0.00	93.15	144.75	73.35	505.73
6	13.54	93.93	144.71	72.19	497.74
7	1.02	93.51	144.71	72.78	501.80
8	0.85	93.82	144.72	72.35	498.84
9	0.80	93.74	144.72	72.47	499.67
10	11.61	93.82	144.72	72.35	498.84
11	1.79	93.61	144.72	72.65	500.91
12	13.85	94.11	144.72	71.94	496.01
13	0.00	93.99	144.72	72.11	497.18
20	0.00	93.60	144.71	72.66	500.98

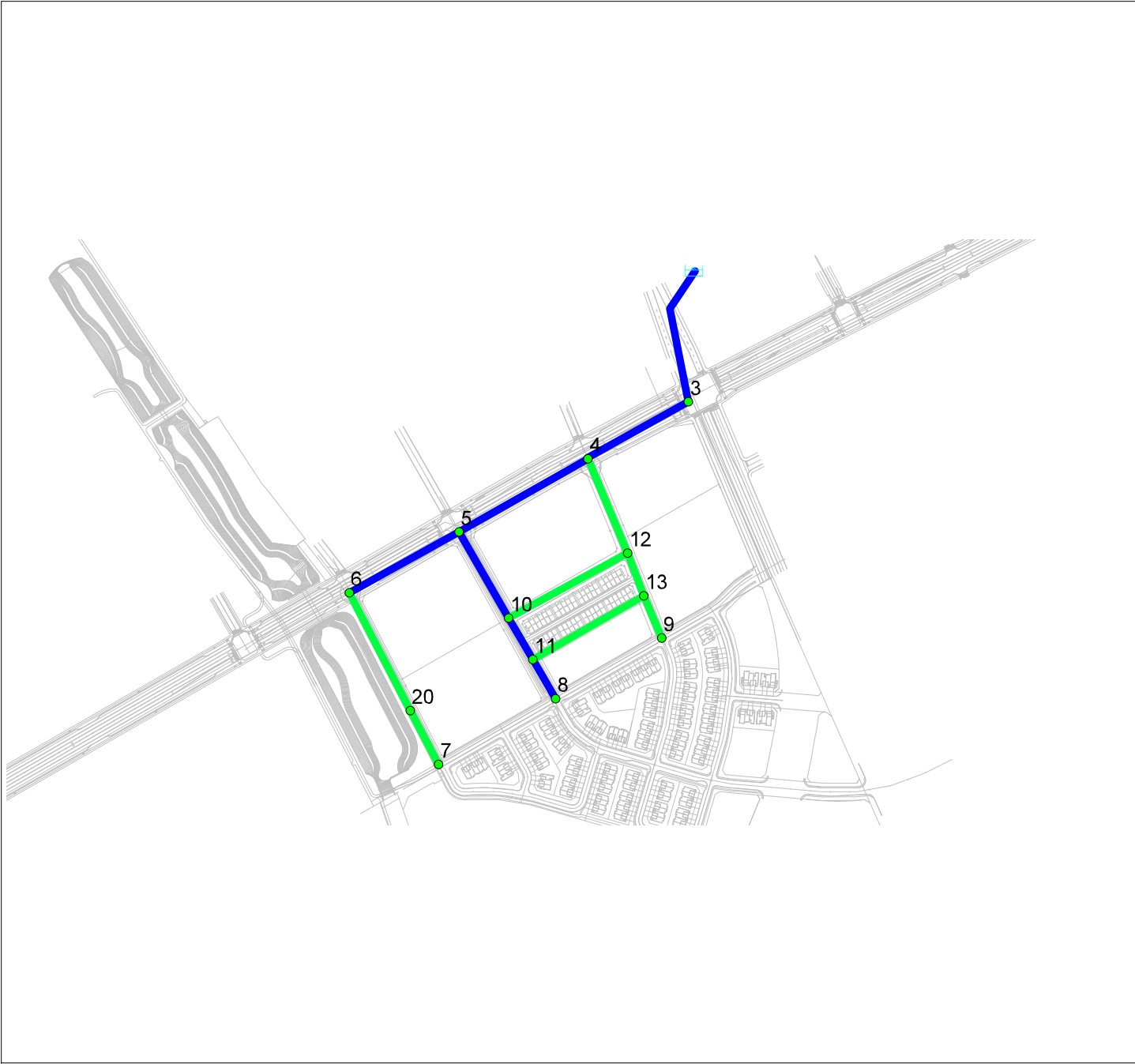
Pipe Results

ID	From Node	To Node	Length	Diameter	Roughness	Flow	Velocity
			(m)	(mm)		(L/s)	(m/s)
13	11	8	57.06	297	120	0.85	0.01
14	10	11	60.55	297	120	2.98	0.04
15	5	10	125.25	297	120	15.21	0.22
16	4	12	128.80	204	110	13.69	0.42
17	12	13	57.59	204	110	0.46	0.01
18	13	9	57.74	204	110	0.80	0.02
19	6	20	167.03	204	110	1.02	0.03
21	6	5	158.01	297	120	-14.56	0.21
22	4	5	186.77	297	120	29.77	0.43
24	3	4	146.18	297	120	43.46	0.63
25	1000	3	92.09	297	120	43.46	0.63
26	10	12	170.91	204	110	0.62	0.02
27	11	13	161.29	204	110	0.34	0.01
50	20	7	76.79	204	110	1.02	0.03

Hydraulic Model Results -Fire Flow Analysis 267 L/s

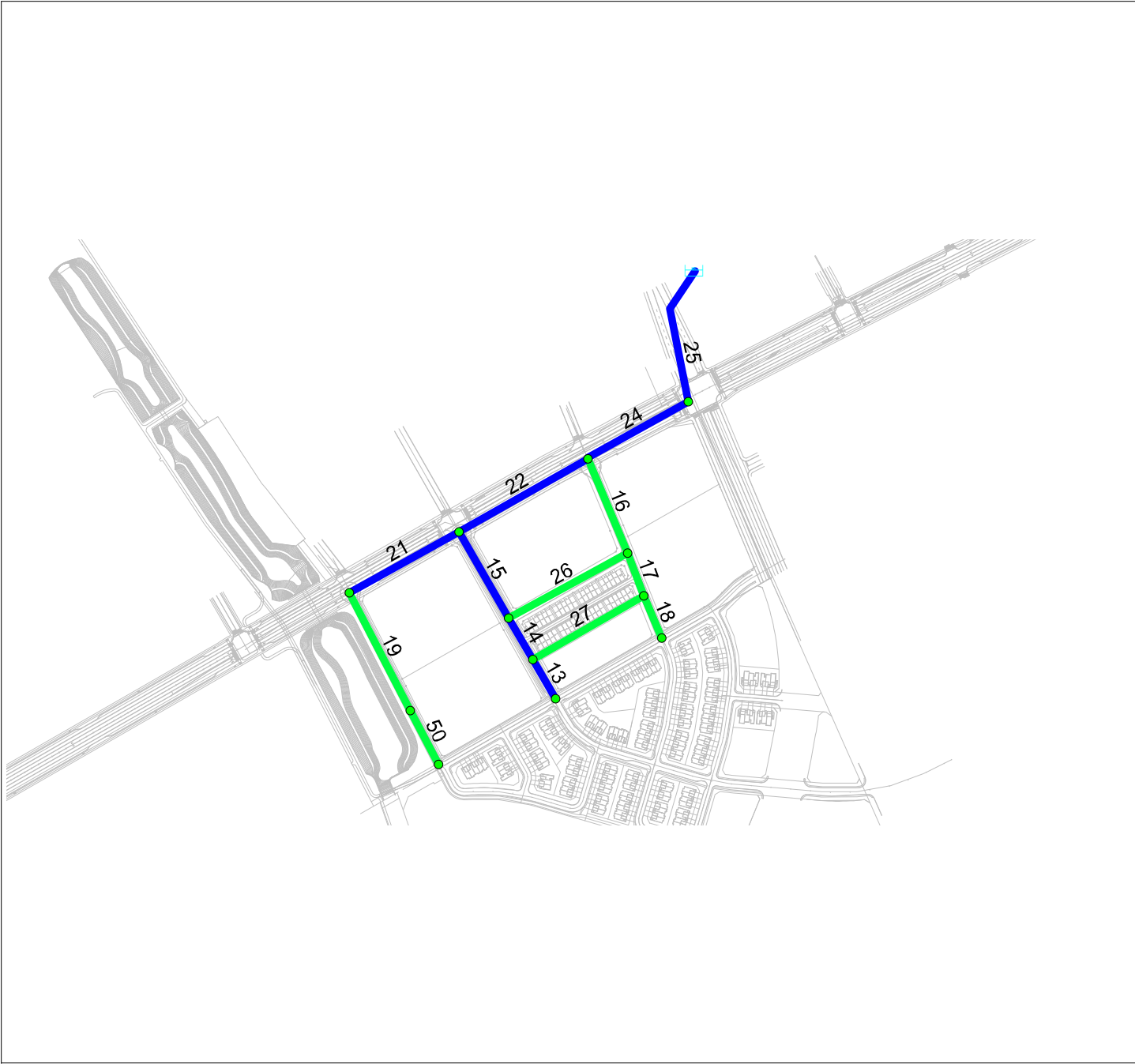
ID	Static Demand	Static Pressure		Static Head	Fire-Flow Demand	Residual Pressure		Available Flow at Hydrant	Available Flow Pressure	
	(L/s)	(psi)	(Kpa)	(m)	(L/s)	(psi)	(Kpa)	(L/s)	(psi)	(Kpa)
3	0.00	69.75	480.91	144.16	167	66.54	458.78	794.24	20	137.90
4	0.00	70.93	489.05	144.11	167	62.62	431.75	474.01	20	137.90
5	0.00	72.39	499.11	144.07	167	60.32	415.89	389.55	20	137.90
6	6.15	71.27	491.39	144.06	167	54.33	374.59	321.02	20	137.90
8	0.38	71.42	492.43	144.06	167	55.21	380.66	323.35	20	137.90
9	0.36	71.54	493.25	144.06	167	40.36	278.27	221.53	20	137.90
10	5.27	71.42	492.43	144.06	267	41.10	283.38	365.81	20	137.90
11	0.87	71.72	494.50	144.06	267	38.93	268.41	346.62	20	137.90
12	6.44	71.01	489.60	144.06	167	55.43	382.18	335.94	20	137.90
13	0.00	71.18	490.77	144.06	167	52.21	359.98	293.39	20	137.90

160401085_SNTC-2020-06-2-JUNCTION ID



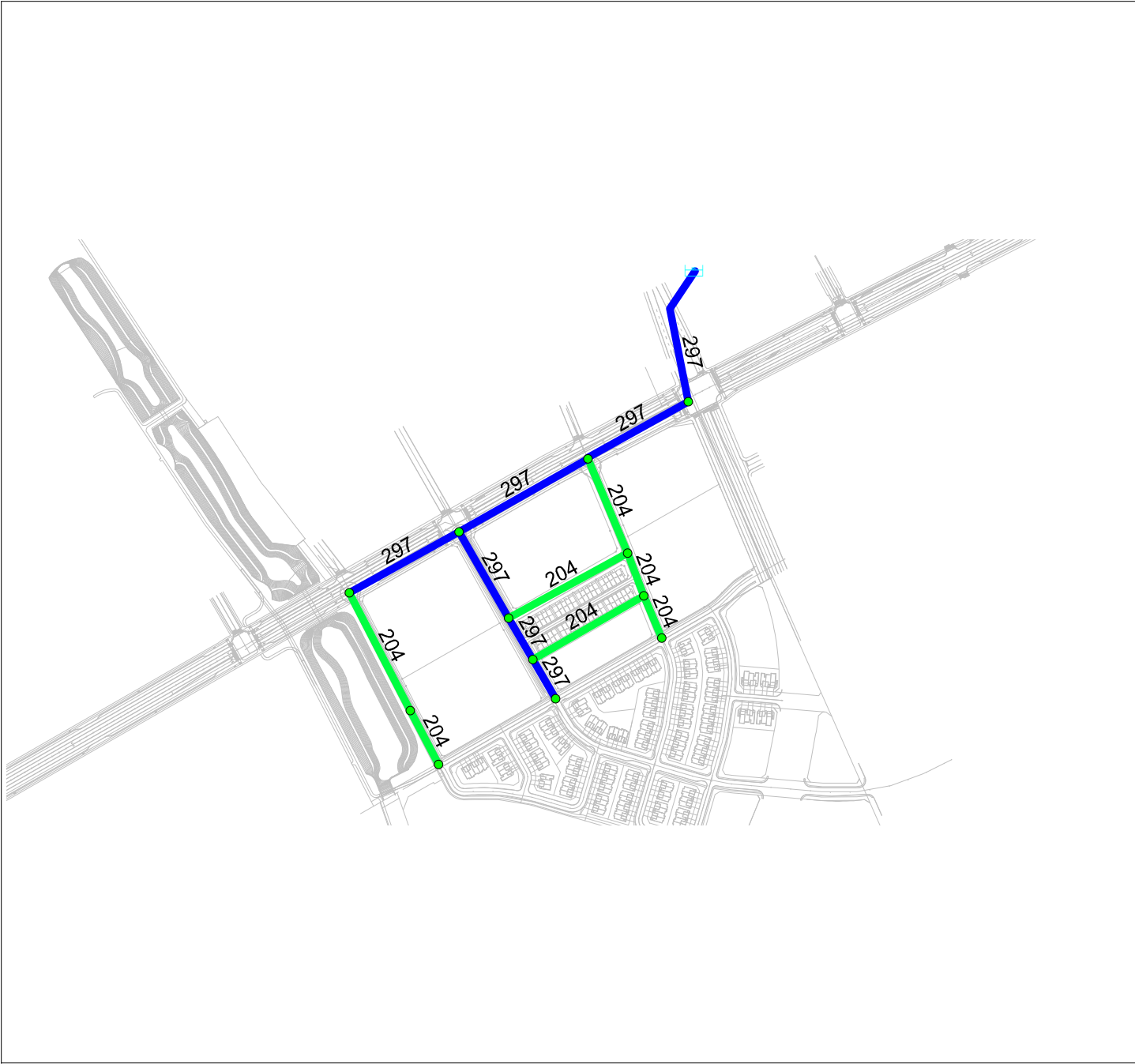
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- VALVE (MOTYPE)
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 - ↗

160401085_SNTC-2020-06-2-PIPE ID



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160401085_SNTC-2020-06-2- PIPE DIAMETER



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- TANK (MOTYPE)
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- Domain Tank
- Active Reservoir
- Domain Reservoir
- PIPE (VALUE)
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- greater than 250.50
- PUMP (MOTYPE)
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- Domain
- VALVE (MOTYPE)
- Active
- Domain
- 1-ACAD-160401085 DP.dxf

4.0 WATERMAIN

4.1 Previous Studies

Stantec consulting was retained to analyze the regional-level impact of developments in the area including the Burnett Lands. Their report, *Kennedy-Burnett Potable Water Master Servicing Study* (April 29, 2014) proposed 3 connections for the Burnett Lands development. At the northeast limits, the watermain will connect to a 300 mm watermain to be located in the realigned Greenbank Road at the Street 1 intersection. At the southeast limits, the watermain will connect to a future 300mm watermain (by others) located within Jockvale Road and current Greenbank Road. At the northwest limits of the site, the watermain will connect to a future watermain located at the Street 1 / Jockvale Road intersection.

4.2 Proposed Watermain System

The ultimate watermain connections and backbone watermain (300mm) throughout the development are consistent with Stantec's *Kennedy-Burnett Potable Water Master Servicing Study* (April 29, 2014) and have not been constructed to date. It has been noted by the City that the southeast and northern connections (by others) should advance and be constructed prior to development of the Burnett Lands. These two connections will serve as a backbone loop system for the proposed Burnett development until such time Greenbank Road is realigned and the 3rd connection can be established as per the ultimate condition of the development.

Since the construction of realigned Greenbank Road is unknown at this time, alternative looping and servicing will be provided through the lands to the north. In addition to the 300mm connection to the north, two connections to 200mm watermains are proposed to the lands to the north at two separate intersections.

The Burnett Lands will be serviced with a combination of 50mm, 200mm and 300 mm looped watermain with three connections to the site to the north and one connection at the southeast limits of the site. Refer to the following figures:

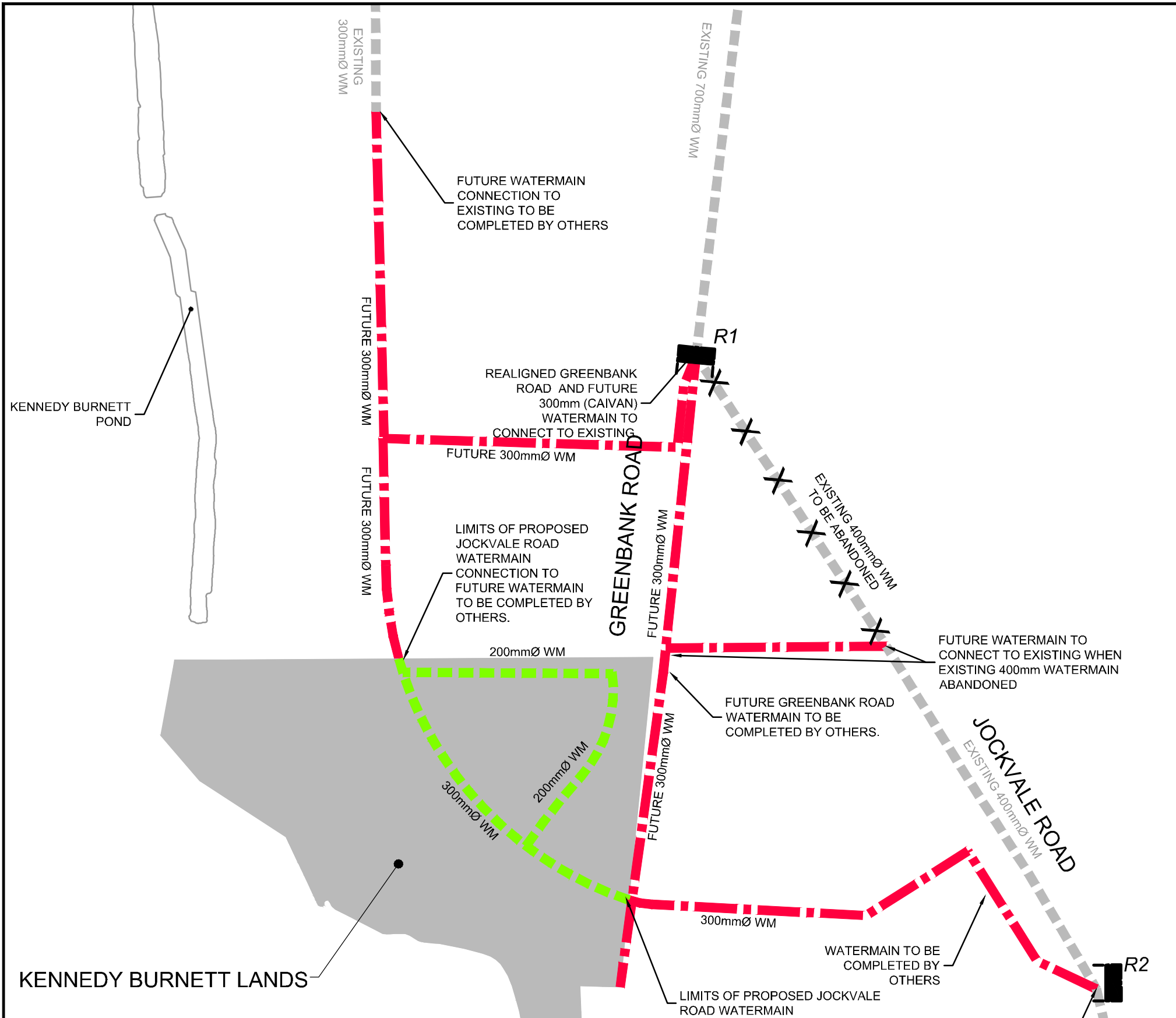
- **Figure 3** – Realigned Greenbank Road Watermain
- **Figure 4A** – Watermain Layout
- **Figure 4B** – Watermain Layout – Offsite (By Others)

4.3 Design Criteria

It is noted the proposed watermain works are located in a future Zone 3C pressure zone. The realignment of the pressure zone will be completed by the City of Ottawa and once complete will alter the boundary conditions for the development. Timing for the realignment is unknown at this time. The City of Ottawa has provided boundary conditions for the pre and post-realignment conditions. This report considers both conditions.

Boundary conditions were provided at the intersection of current Greenbank Road and Jockvale Road as well as the intersection of Jockvale Road and Bren-Maur Road. These two connections were utilized and modelled, however, since the connection to Jockvale Road / Greenbank Road occurs through the site to the north, demand and the proposed pipe network were also taken into account and modelled. Boundary conditions do not take into account the demand from the site to the north, however, it is anticipated the demand will have minimal impact on the hydraulic results.

Fire flow demands have been calculated as per the Fire Underwriter's Survey (FUS) and are included in **Appendix C**. However as per the City of Ottawa's technical bulletin ISDTB-2014-02 (Revisions to Ottawa Design Guidelines – Water), the majority of the townhouse fireflows have



- LEGEND:**
- - - FUTURE WATERMAIN BY OTHERS
 - - - PROPOSED WATERMAIN
 - ~~---~~ EXISTING WATERMAIN TO BE ABANDONED
 -
 EXISTING WATERMAIN
 - R1**
|
 BOUNDARY CONDITION LOCATION

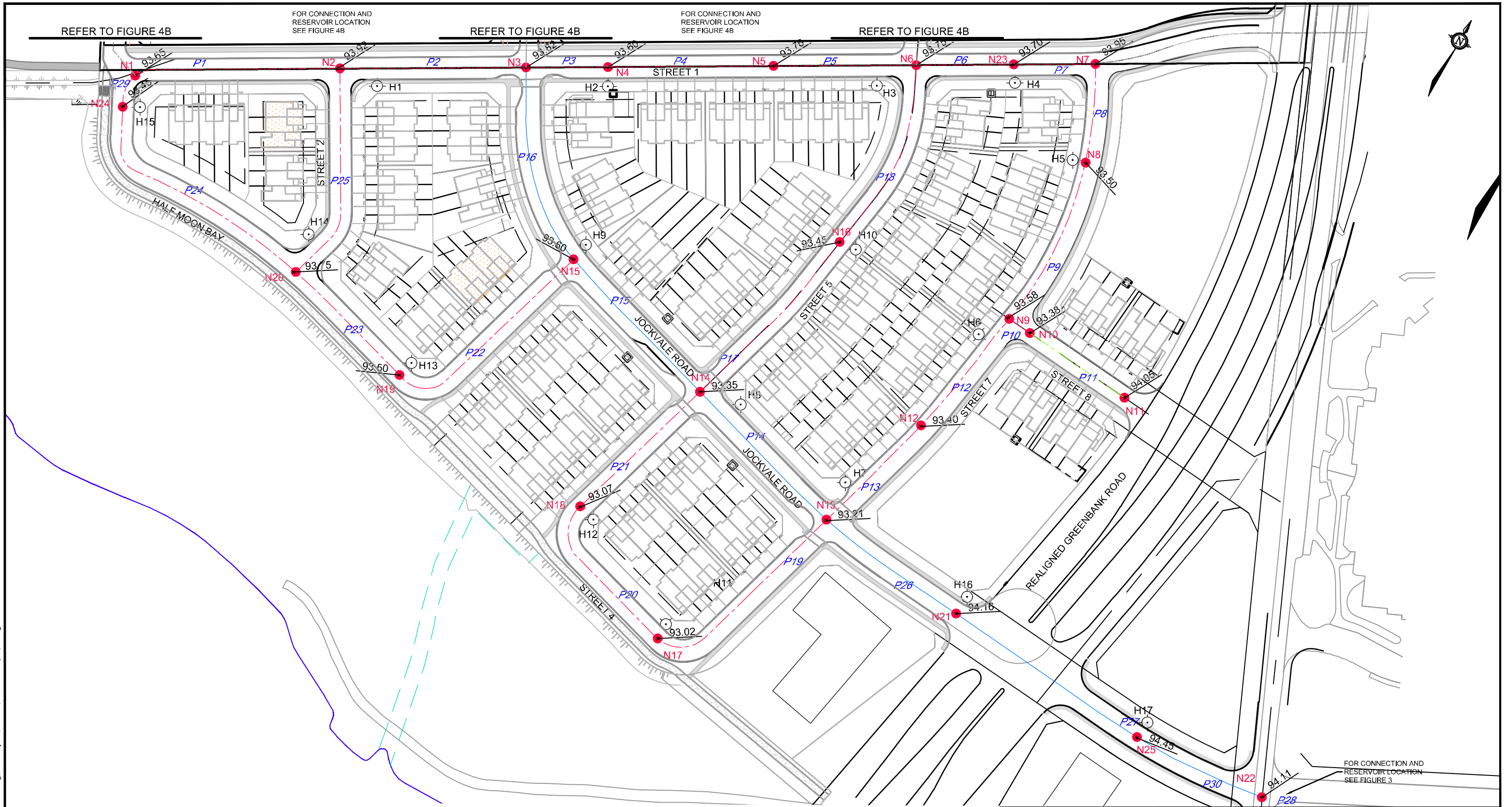
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KENNEDY BURNETT LANDS

KENNEDY BURNETT POND

 Engineers, Planners & Landscape Architects Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6 Telephone (613) 254-9643 Facsimile (613) 254-5867 Website www.novatech-eng.com	3370 GREENBANK RD. BURNETT LANDS	
	REALIGNED GREENBANK ROAD WATERMAIN	
SCALE NOT TO SCALE		
DATE	JUNE 2020	FIGURE
JOB	111117	FIGURE 3

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LEGEND

- P12 PROPOSED 50mmØ WATERMAIN PIPE
- P12 PROPOSED 200mmØ WATERMAIN PIPE
- P4 PROPOSED 300mmØ WATERMAIN PIPE
- OP4 OFFSITE WATERMAIN PIPE ID AND SIZE (BY OTHERS)
- 300mmØ
- N4 ● WATERMAIN NODE - ONSITE
- ON4 ● WATERMAIN NODE - OFFSITE
- RESERVOIR
- HYDRANT
- ↗ 97.75 GROUND ELEVATION
- ▨ FIREFLOWS THAT CANNOT BE CAPPED AS PER TECHNICAL BULLETIN ISDTB-2014-02



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Ottawa, Ontario, Canada K2M 1P6

Telephone (613) 254-9643
Facsimile (613) 254-5867
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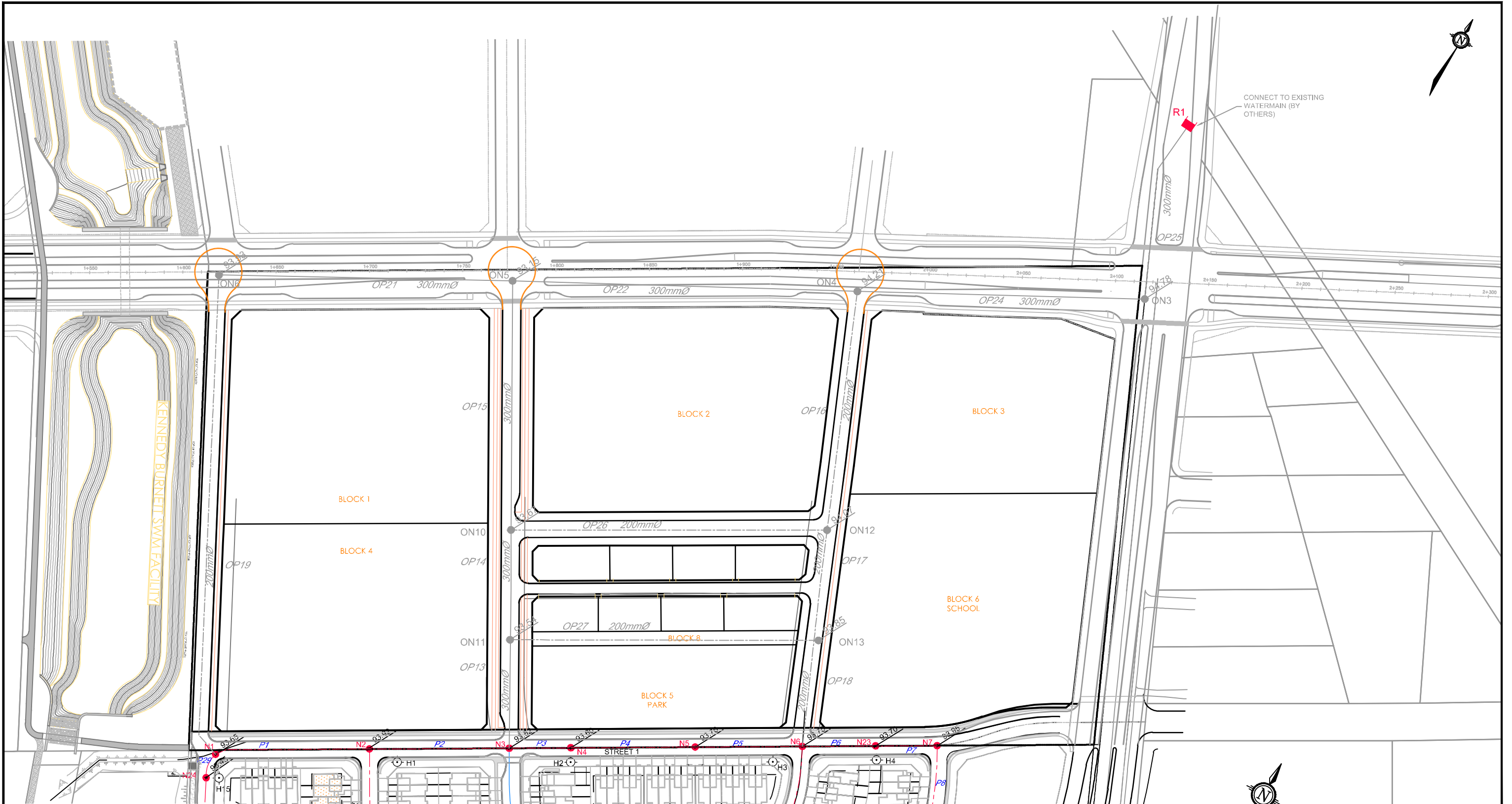
**3370 GREENBANK RD.
BURNETT LANDS**

WATERMAIN LAYOUT

SCALE 1:1500

DATE JUNE 2020 JOB 111117 FIGURE FIGURE 4A

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LEGEND

- P12 PROPOSED 50mmØ WATERMAIN PIPE
- P12 PROPOSED 200mmØ WATERMAIN PIPE
- P4 PROPOSED 300mmØ WATERMAIN PIPE
- OP4
300mmØ OFFSITE WATERMAIN PIPE ID AND SIZE (BY OTHERS)
- N4 ● WATERMAIN NODE - ONSITE
- ON4 ● WATERMAIN NODE - OFFSITE
- RESERVOIR
- ⊙ HYDRANT
- ↗ 91.75 GROUND ELEVATION
- ▨ FIREFLOWS THAT CANNOT BE CAPPED AS PER TECHNICAL BULLETIN ISDTB-2014-02

REFER TO FIGURE 4A

REFER TO FIGURE 4A

NOVATECH

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3370 GREENBANK RD.
BURNETT LANDS

**WATERMAIN LAYOUT -
OFFSITE (BY OTHERS)**

SCALE 1 : 2000

DATE JUNE 2020 JOB 111117 FIGURE FIGURE 4B

been capped at 10,000 L/min (167 L/s). Fireflows that cannot be capped are highlighted in **Figure 4A – Watermain Layout**. Watermain analysis was completed based on the following criteria:

Demands:

- Townhouse Density 2.7 persons/unit
- Condo/Apartment Density 1.8 persons/unit
- Average Daily Demand 350 L/capita/day
- Max. Daily Demand 2.5 x Average Daily Demand
- Peak Hour Demand 2.2 x Maximum Daily Demand
- Fire Flow Demand Fire Underwriters Survey

System Requirements:

- Max. Pressure (Unoccupied Areas) 690 kPa (100 psi)
- Max. Pressure (Occupied Areas) 552 kPa (80 psi)
- Min. Pressure 276 kPa (40 psi) excluding fire flows
- Min. Pressure (Fire) 138 kPa (20 psi) including fire flows
- Max. Age (Quality) 192 hours (onsite)

Friction Factors:

- Watermain Size C-Factor
- 50mm 100
- 200-250 mm 110
- 300-400 mm 120

4.4 Watermain Analysis

Hydraulic modelling of the proposed 3370 Greenbank Road was completed using EPANET 2.0. EPANET is public domain software capable of modeling municipal water distribution systems by performing simulations of the water movement within a pressurized system. EPANET utilized the Hazen-Williams equation to predict the performance of the proposed watermain and considered the following input parameters: water demand, pipe length, pipe diameter, pipe roughness, and pipe elevation. Table 1 (Water Demand Calculations) in Appendix C confirms the water demands at each node in the system. Table 2 (Pipe Data) in Appendix C confirms the length, diameter, and roughness of each pipe in the system. Tables 3 to 8 in Appendix C confirms the elevation of each node in the system.

The high pressure condition (average daily demand) was analyzed to ensure the system meets the design criteria for maximum pressure and quality. The maximum daily demand plus fire flow and peak hour conditions were analyzed to ensure the system meets the design criteria for maximum flow and minimum pressure.

The hydraulic modelling results for the development prior to the City reconfiguring the watermain are listed in **Table 4.1**.

Table 4.1: Water Demand Summary (Pre-Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	4.53	N/A	690/100 (Max)	626.7/90.9	22.5
Maximum Daily Demand	11.33	167, 183, 200	138/20 (Min)	303.4/44.0 (167L/s at N10)	N/A
Peak Hour	24.93	N/A	276/40 (Min)	447.7/64.9	N/A

The analysis confirms the proposed watermain can service the Burnett Lands prior to the site being included into the realigned Zone 3C pressure zone. It is noted that pressure in the main is greater than 552 kPa/80psi for the entire development. Since the timing for the realignment and reconfiguration is unknown at this time, the use of pressure reducing values will be required throughout the entire development.

The hydraulic modelling results for the development after the watermain realignment of the watermain are listed in **Table 4.2**.

Table 4.2: Water Demand Summary (Post Watermain Reconfiguration)

Condition	Demand (L/s)	Fire Flow (L/s)	Allowable Max/Min Pressure (kPa/psi)	Max/Min Pressure (kPa/psi)	Time (hrs)
High Pressure	4.53	N/A	690/100 (Max)	533.7/77.4	22.5
Maximum Daily Demand	11.33	167, 183, 200	138/20 (Min)	385.4/55.9 (167L/s at N10)	N/A
Peak Hour	24.93	N/A	276/40 (Min)	497.8/72.2	N/A

The analysis confirms the proposed watermain can service the Burnett Lands after the realigned Zone 3C pressure zone under all operating conditions.

A copy of the boundary conditions provided by the City of Ottawa, fire flow calculations, detailed hydraulic analysis results, and watermain layout figure are included in **Appendix C**.

Deviations from the City of Ottawa Design Guidelines – Water Distribution (2010) include:

- As per discussions with the City of Ottawa, Jockvale Road has a 22.0m ROW with a 9.5m wide roadway, with servicing including a sanitary trunk sewer, local sanitary sewer, storm sewer and watermain. A collector road typically has a 24.0m ROW with a 11.0m wide roadway.
 - Due to the geometry (bend, curb bump outs) of Jockvale Road and following the existing sanitary trunk sewer while maintaining sufficient offsets, a portion of the watermain is located under curb. Additionally, hydrant shut off valves cannot be located within the roadway and will be located under the sidewalk along Jockvale

Road. This deviation is a result of reducing the roadway width to 9.5m as per City comments.

- A specific cross section has been developed for Street 1 (shared road) based on comments received from utilities and to accommodate large elliptical storm sewers within the roadway.
- A specific cross section has been developed for all other roadway based on comments received from utilities.
- Isolation valves are to be located 2.0m away from the intersection, from the point where the projection of the property line intersects the watermain. This distance has been increased to accommodate intersection narrowing along the collector road to improve pedestrian crossings and to ensure a valve chamber is not located under curb and located within the roadway. This occurs in several intersections along Jockvale Road.

EXECUTIVE SUMMARY

The following report identifies and evaluates the proposed water distribution system for a development located in the southern area of the City of Ottawa. The proposed Kennedy-Burnett (KB) development and the Nepean Town Centre (NTC) are located south of Strandherd Drive and are bound by Longfields Drive to the east, Cedarview Road to the west and the Jock River to the south (Figure 1-1). The area is currently serviced by the Zone BARR pressure zone within the City of Ottawa water distribution system.

A hydraulic assessment was performed using the City's 2013 Water Master Plan Update model for existing conditions and future conditions. Since this development is located in close proximity to the future 3C boundary, the hydraulic assessment reviewed scenarios in which the development is built in existing conditions, interim conditions, and future network configurations.

In the interim, both development areas are to be serviced by Zone BARR pressures. Based on the evaluation presented herein, it is recommended for the future that the KB area remain within Zone BARR and the NTC area be converted to the Zone 3C pressure zone post zone reconfiguration.

The proposed servicing plan recommends that the KB area will be fed by the existing small diameter Zone Barr watermain that currently feeds a small development in this area and/or by the future Strandherd 406mm diameter watermain. It is recommended that the NTC area continue to be fed by the 762mm diameter watermain along Greenbank and converted to Zone 3C pressures as previously planned. This configuration eliminates the need to connect the two developments with looping watermain across the future Kennedy-Burnett stormwater facilities – a savings of potentially \$2.847 million.

Under pre zone reconfiguration conditions, certain areas within both the KB lands and the NTC lands experience maximum pressures greater than 80psi. Per the Ontario Building Code, services with pressures greater than 80 psi require pressure mitigation measures to be implemented, in most cases this involves a pressure reduction valve (PRV) on the individual service lines. Any development that proceeds in the interim prior to zone reconfiguration will have to ensure pressure reduction measures are in place as required depending on ground elevations. Under post zone reconfiguration conditions, lands within the KB area remain in the higher pressure zone and will continue to require pressure reduction measures accordingly. Lands within NTC will be located within a lower operating pressure zone and according to the results, maximum pressures are not anticipated to exceed 80 psi and therefore would likely not require pressure reduction measures – the requirement for PRVs in the NTC lands post zone reconfigurations will ultimately be decided once the final zone reconfiguration hydraulics are confirmed and implemented.

With respect to minimum pressure constraints, the results are similar for both areas under pre and post zone reconfiguration. The KB lands are anticipated to maintain minimum pressures greater than 60psi. This value is greater than the City's Design Guideline minimum pressure objective whereas the minimum pressures in NTC are anticipated to be maintained greater than 50psi at the highest ground elevations. This value is greater than the 40 psi minimum allowable pressure per the design guidelines.

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A fire flow assessment under maximum day demand conditions was carried out. Under both pre and post zone reconfiguration, available fire flows in the NTC lands are projected to be greater than 15,000L/ min along all the larger diameter watermain (305mm and greater). A hydraulic analysis for individual developments will still be required to determine the fire flow requirements and capabilities of the local watermain to provide the necessary fire flow amounts.

Under pre zone reconfiguration conditions, fire flows in the KB lands are limited by the small diameter watermains that feed the area. Available fire flows are limited to approximately 10,080L/ min at a residual pressure of 20 psi. As development grows in the KB area, a third connection across Strandherd to an existing watermain along Frasier Fields will help to increase fire flows to a minimum of 12,120L/ min along the larger diameter pipe network. Under future network conditions if and when the KB lands connect directly to the future 406mm diameter watermain along Strandherd, the available fire flows in the KB lands are expected to increase significantly and will be capable of providing greater than 15,000L/ min.

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Hydraulic Assessment
March 25, 2014

Scenario 1A:

- Entire study area serviced by Zone BARR with no direct connections between KB and NTC;
- Represents a scenario where KB is developed independently of NTC (Figure 2-3);
- Existing (2012) demands under existing network conditions;
- KB area connected to existing development north of Strandherd;

Scenario 1B:

- Entire study area serviced by Zone BARR with no direct connections between KB and NTC;
- Represents a scenario where KB is developed independently of NTC (Figure 2-3);
- Existing (2012) demands under existing conditions;
- KB area connected to existing development north of Strandherd and a new watermain connection to the existing 254mm diameter pipe on Fraser Fields Way (for additional fire flow support);

Scenario 1C:

- Study area serviced by Zone BARR with direct connections between KB and NTC;
- Represents a scenario where both areas are developed concurrently (Figure 2-4);
- Existing (2012) demands under existing conditions;
- KB area connected to existing development north of Strandherd and two new 305mm diameter watermain connections across the future stormwater facilities to the NTC lands.

Model results are summarized in Table 2-3. Under existing network conditions, 100% of the nodes in the KB & NTC lands exceed the 80 psi threshold requiring pressure reduction measures per the Ontario Building/Plumbing Code.

With respect to minimum pressures under peak demand conditions, all pressures at nodes in the NTC lands remain greater than 51 psi whereas the minimum pressure in the KB lands is 64psi. These minimum pressure values are within acceptable guideline ranges.

Available fire flow to the KB lands is restricted by existing smaller diameter watermain, as shown in Scenario 1A, the minimum fire flow observed is 10,080L/min. With a third connection along Fraser Fields, the fire flow increases to 12,120. If the KB lands were connected to the NTC lands (Scenario 1C) the available fire flow would exceed 15,000L/min.

Table 2-3: Pre Zone Reconfiguration - Results Under Various Scenarios

Scenario & Area	Zone	AVDY (psi)		Available Fire Flow (L/min) @ 20 psi
		Max	Min	
Scenario 1A: KB	BARR	99-102	64-67	10,080
Scenario 1A: NTC	BARR	82-103	51-72	> 15,000
Scenario 1B: KB	BARR	99-102	66-69	12,120
Scenario 1B: NTC	BARR	82-103	51-72	> 15,000
Scenario 1C: KB	BARR	99-102	67-70	> 15,000
Scenario 1C: NTC	BARR	82-103	51-72	> 15,000

* Pressures greater than 80psi exceed the allowable range as per the OBC

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Hydraulic Assessment
March 25, 2014

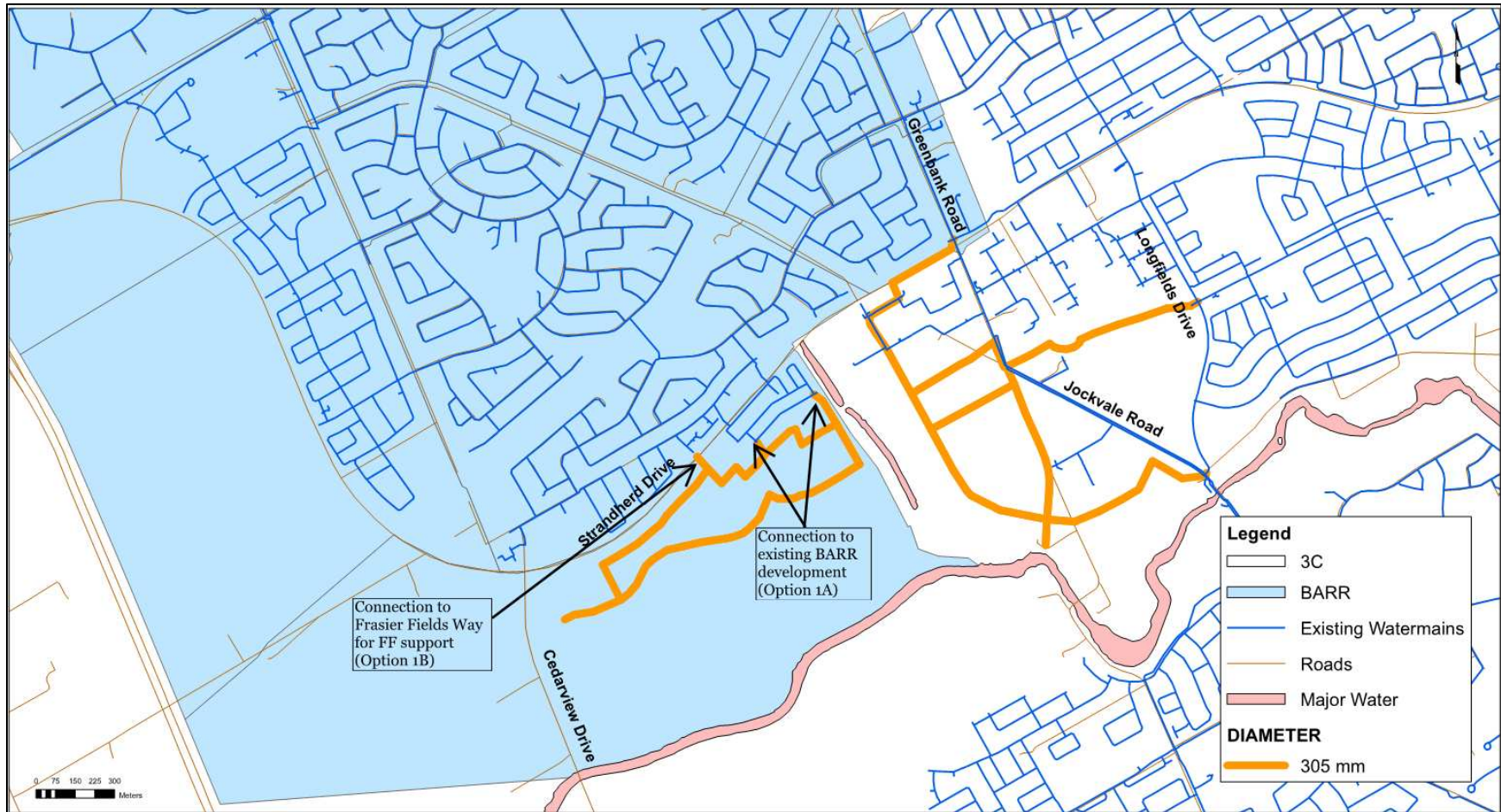


Figure 2-3: Proposed Pipe Layout Pre Zone Reconfiguration – Scenarios 1A and 1B

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March 25, 2014

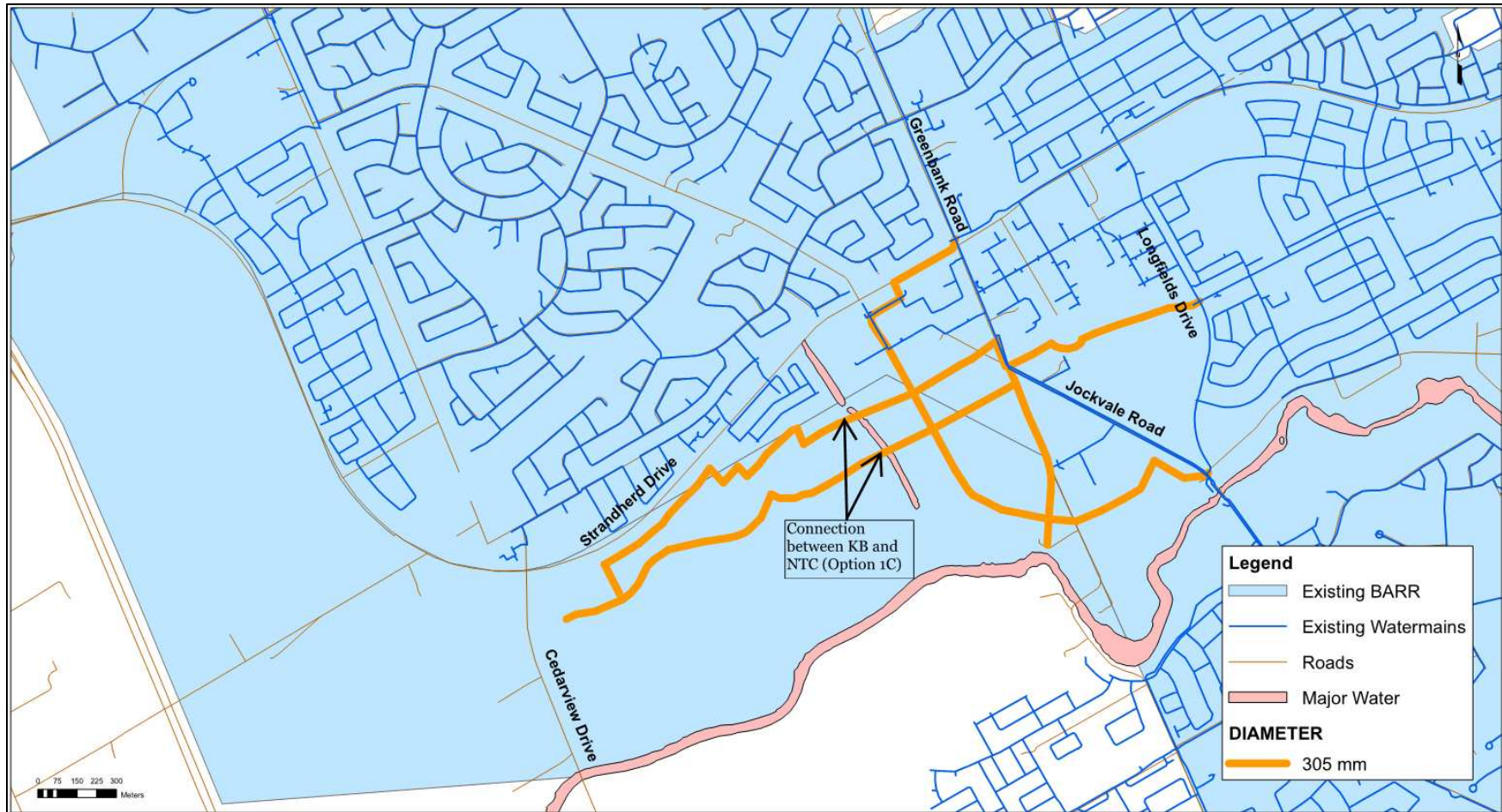


Figure 2-4: Proposed Pipe Layout Pre Zone Reconfiguration – Scenario 1C

2.7.2 Post Zone Reconfiguration – Future Demand Conditions

Scenario 2A: Prior to 406mm diameter watermain along Strandherd

- Represents scenario where KB operates at BARR pressure (*blue*) and NTC operates at 3C pressure (*white*) post zone reconfiguration (Figure 2-5);
- 2031 demands under 2031 network conditions;
- KB area connected to existing development north of Strandherd and a new watermain connection to the existing 254mm diameter pipe on Fraser Fields Way;

Scenario 2B: Post construction of 406mm watermain along Strandherd

- Represents scenario where KB operates at BARR pressure (*blue*) and NTC operates at 3C pressure (*white*) post zone reconfiguration (Figure 2-6);
- 2031 demands under 2031 network conditions;
- KB area connected to existing development north of Strandherd and to a future BARR 406mm diameter watermain along Strandherd;

Scenario 3: KB and NTC both serviced by Zone 3C

- Represents scenario where study area operates at 3C pressure (*white*) post zone reconfiguration (Figure 2-7);
- 2031 demands under 2031 network conditions;
- Two 305mm diameter watermains connecting KB and NTC across future stormwater facilities;
- KB area not connected to existing development along Strandherd.

Model results are summarized in Table 2-4. As shown in Scenario 2A and 2B, keeping the KB lands in Zone Barr results in maximum pressures exceeding the 80 psi threshold and would require pressure reduction measures per the Ontario Building/Plumbing Code (similar to existing development conditions). If the KB development is switched to Zone 3C post reconfiguration, the maximum pressures drop below the threshold. For all scenarios, the maximum pressures within the NTC lands remain just below the maximum pressures threshold and therefore would not require pressure reduction measures.

Available fire flow to the KB lands is restricted by existing smaller diameter watermain in the development to the north. In order for fire flows to increase to greater than 15,000 L/min, connections to a new larger diameter watermain along Strandherd would be required. Similarly, if the KB lands were to be directly connected to the NTC lands and disconnected from Zone Barr, this alternative would also increase fire flows to KB lands to greater than 15,000L/min.

Table 2-4: Post Zone Reconfiguration - Results Under Various Scenarios

Scenario & Area	Zone	AVDY (psi)	PKHR (psi)	Available Fire Flow (L/min) @ 20 psi
		Max	Min	
Scenario 2A: KB	BARR	95-98	78-81	10,980
Scenario 2A: NTC	3C	59-80	51-70	> 15,000
Scenario 2B: KB	BARR	91-94	80-83	> 15,000
Scenario 2B: NTC	3C	59-80	51-70	> 15,000
Scenario 3: KB	3C	76-79	66-69	> 15,000
Scenario 3: NTC	3C	59-80	51-70	> 15,000

* Pressures greater than 80psi exceeds the allowable maximum pressure per the OBC

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Hydraulic Assessment
March 25, 2014

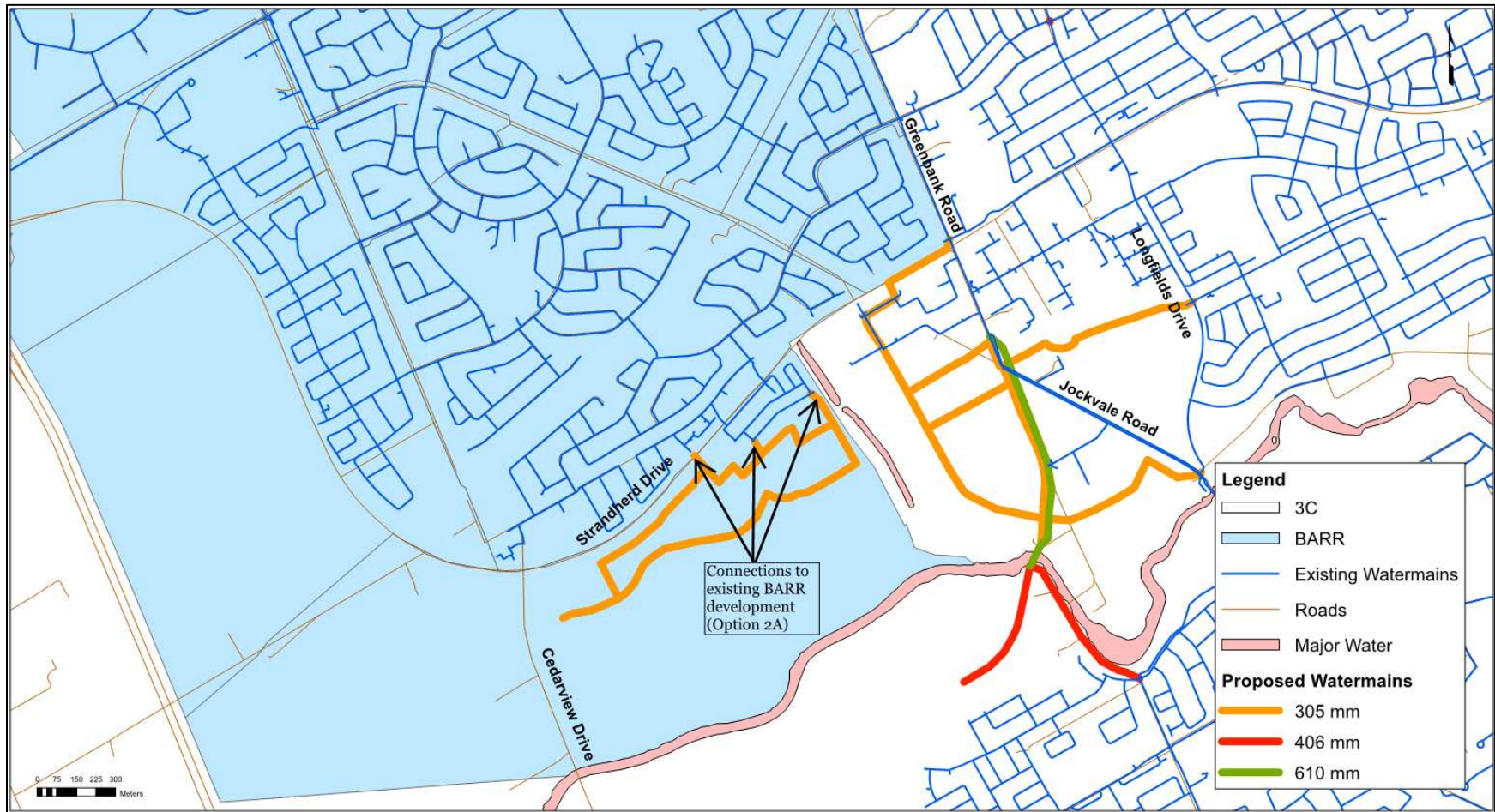


Figure 2-5: Proposed Pipe Layout Post Zone Reconfiguration – Scenario 2A

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March 25, 2014

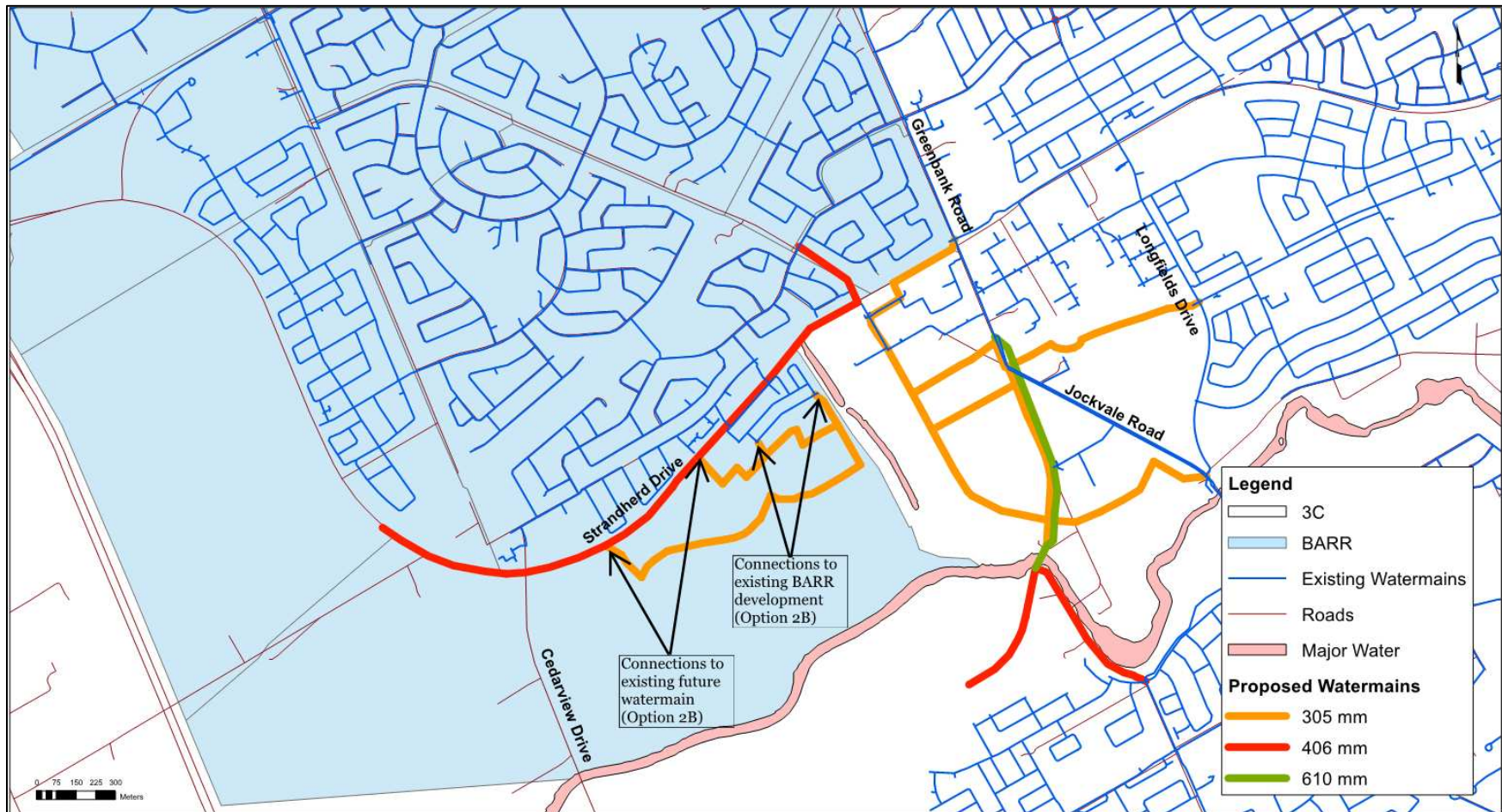


Figure 2-6: Proposed Pipe Layout Post Zone Reconfiguration – Scenario 2B

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Hydraulic Assessment
March 25, 2014

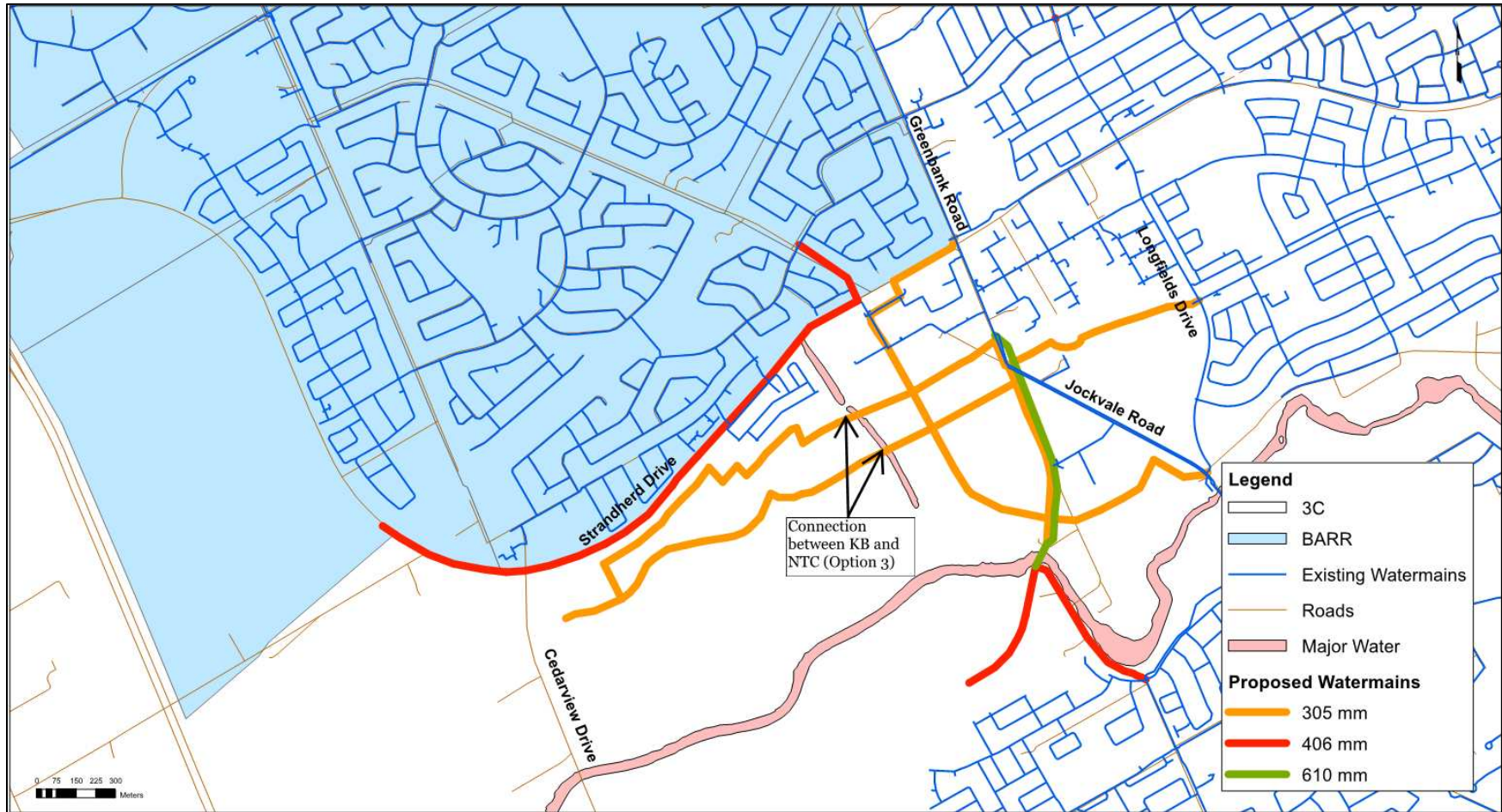


Figure 2-7: Proposed Pipe Layout Post Zone Reconfiguration – Scenario 3

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix B Sanitary Sewer Calculations

Appendix B **SANITARY SEWER CALCULATIONS**

B.1 SANITARY SEWER DESIGN SHEET



SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix B Sanitary Sewer Calculations

B.2 BACKGROUND REPORT EXCERPTS



SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix C Stormwater Management Calculations

Appendix C STORMWATER MANAGEMENT CALCULATIONS

C.1 STORM SEWER DESIGN SHEET





SNTC Lands - Block 4

STORM SEWER DESIGN SHEET (City of Ottawa)

DESIGN PARAMETERS

I = a / (t+b)^c (As per City of Ottawa Guidelines, 2012)

Table with design parameters: a, b, c, MANNING'S n, MINIMUM COVER, TIME OF ENTRY, BEDDING CLASS.

Main data table with columns: LOCATION, DRAINAGE AREA, and PIPE SELECTION. Includes rows for Block 4, Block 5, Block 6, and Block 1 drainage systems.

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix C Stormwater Management Calculations

C.2 PCSWMM MODEL INPUT



160401085-2020-07-30-5yr-100BC.inp

[TITLE]
160401085 - SNTC Lands Assessment
Detailed Design Block 4, located north of the Jock River and west of Greenbank Road.

```

[OPTIONS]
;;Options      Value
;;-----
FLOW_UNITS     LPS
INFILTRATION   HORTON
FLOW_ROUTING   DYNWAVE
LINK_OFFSETS   ELEVATION
MIN_SLOPE      0
ALLOW_PONDING  YES
SKIP_STEADY_STATE NO
START_DATE     01/02/2018
START_TIME     00:00:00
REPORT_START_DATE 01/02/2018
REPORT_START_TIME 00:00:00
END_DATE       01/03/2018
END_TIME       00:00:00
SWEEP_START    01/01
SWEEP_END      12/31
DRY_DAYS       0
REPORT_STEP    00:01:00
WET_STEP       00:01:00
DRY_STEP       00:01:00
ROUTING_STEP   2
RULE_STEP      00:00:00
INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP  0
LENGTHENING_STEP 0
MIN_SURFAREA   0
MAX_TRIALS     8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL   5
LAT_FLOW_TOL   5
MINIMUM_STEP   0.5
THREADS        6

```

```

[EVAPORATION]
;;Type      Parameters
;;-----
CONSTANT    0.0
DRY_ONLY    NO

```

160401085-2020-07-30-5yr-100BC.inp

```

[RAINGAGES]
;;      Rain      Time      Snow      Data
;;Name   Type      Intrvl  Catch      Source
;;-----
Raingage INTENSITY 0:10    1.0      TIMESERIES Chicago_5y_3h_10m_City

```

```

[SUBCATCHMENTS]
;;      Curb      Snow      Total      Pcnt.      Pcnt.
;;Name   Length  Pack      Raingage   Outlet      Area      Imperv  Width  Slope
;;-----
L101A    Raingage      L101A-S    0.371714  88.571    370    3
0
L102A    Raingage      L102A-S    0.155965  87.143    116    3
0
L102B    Raingage      L102B-S    0.416519  84.286    314    3
0
L102C    Raingage      L102C-S    0.091812  91.429    40     3
0
L102D    Raingage      L102D-S    0.124257  88.571    54     3
0
L105A    Raingage      L105A-S    0.111158  87.143    56     3
0
UNC-1    Raingage      OF1        0.142651  60        321    2
0
UNC-2    Raingage      OF2        0.03007   42.857    200    3
0
UNC-3    Raingage      OF3        0.1184    42.857    305    3
0
UNC-4    Raingage      OF4        0.02893   42.857    200    3
0

```

```

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo
PctRouted
;;-----
L101A    0.013    0.25    1.57    4.67    0      OUTLET
L102A    0.013    0.25    1.57    4.67    0      OUTLET
L102B    0.013    0.25    1.57    4.67    0      OUTLET
L102C    0.013    0.25    1.57    4.67    0      OUTLET
L102D    0.013    0.25    1.57    4.67    0      OUTLET
L105A    0.013    0.25    1.57    4.67    0      OUTLET
UNC-1    0.013    0.25    1.57    4.67    0      PERVIOUS
90

```

```

160401085-2020-07-30-5yr-100BC.inp
UNC-2      0.013    0.25    1.57    4.67    0      PERVIOUS
70
UNC-3      0.013    0.25    1.57    4.67    0      PERVIOUS
95
UNC-4      0.013    0.25    1.57    4.67    0      PERVIOUS
90

[INFILTRATION]
;;Subcatchment MaxRate MinRate Decay DryTime MaxInfil
;;-----
L101A      76.2     13.2    4.14    7        0
L102A      76.2     13.2    4.14    7        0
L102B      76.2     13.2    4.14    7        0
L102C      76.2     13.2    4.14    7        0
L102D      76.2     13.2    4.14    7        0
L105A      76.2     13.2    4.14    7        0
UNC-1      76.2     13.2    4.14    7        0
UNC-2      76.2     13.2    4.14    7        0
UNC-3      76.2     13.2    4.14    7        0
UNC-4      76.2     13.2    4.14    7        0

```

```

[OUTFALLS]
;;
;;Name Invert Elev. Outfall Type Stage/Table Time Series Tide Gate Route To
;;-----
DICB221  90.72   FIXED   91.9      NO
MAJ-1    93.33   FREE
MAJ-2    93.24   FREE
OF1      0       FREE
OF2      0       FREE
OF3      93.52   FREE
OF4      93.44   FREE

```

```

[STORAGE]
;;
;; Name Evap. Invert Max. Init. Storage Curve
;; Name Elev. Depth Depth Curve Params
;; Name Frac. Infiltration parameters
;;-----
L101A1-S  93.99  0.35  0      FUNCTIONAL 0 0 0 0
0
L101A-S  92.12  1.73  0      TABULAR L101A-V 0
0
L101A-S1  93.69  0.35  0      FUNCTIONAL 0 0 0 0
0
L102A-S  92.29  1.73  0      TABULAR L102A-V 0
0

```

```

160401085-2020-07-30-5yr-100BC.inp
L102A-S1  93.77  0.35  0      FUNCTIONAL 0 0 0 0
0
L102B-S  91.94  1.93  0      TABULAR L102B-V 0
0
L102B-S1  93.87  0.35  0      FUNCTIONAL 0 0 0 0
0
L102B-S2  93.97  0.35  0      FUNCTIONAL 0 0 0 0
0
L102C-S  92.52  1.73  0      TABULAR 102C-V 0
0
L102C-S1  94.1    0.35  0      FUNCTIONAL 0 0 0 0
0
L102D-S  92.26  1.73  0      TABULAR L102D-V 0
0
L102D-S1  93.94  0.35  0      FUNCTIONAL 0 0 0 0
0
L105A-S  92.08  1.73  0      FUNCTIONAL 0 0 0 0
0
L105A-S1  93.91  0.35  0      FUNCTIONAL 0 0 0 0
0
STM-101   90.496  3.214  1.404  FUNCTIONAL 0 0 0 0
0
STM-102   91.16  2.977  0.74   FUNCTIONAL 0 0 0 0
0
STM-103   91.071  2.863  0.829  FUNCTIONAL 0 0 0 0
0
STM-104   91.404  2.583  0.496  FUNCTIONAL 0 0 0 0
0
STM-105   91.232  2.306  0.668  FUNCTIONAL 0 0 0 0
0

```

```

[CONDUITS]
;;
;; Inlet Inlet Outlet Manning Inlet
;; Name Init. Max. Node Length N Offset
;; Name Offset Flow Flow
;;-----
C1 93.46 0 L105A-S1 L105A-S 29.884 0.013 93.91
0
C10 93.94 0 L102D-S L102D-S1 18.031 0.013 93.64
0
C11 93.64 0 L102B-S2 L102D-S 10.552 0.013 93.97
0
C12 93.52 0 L102B-S2 L102B-S 21.381 0.013 93.97
0
C13 93.87 0 L102D-S1 L102B-S1 24.144 0.013 93.94
0

```

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C14		L101A1-S	L101A-S	75.133	0.013	93.99
93.5	0	0				
C15		L101A1-S	L102B-S	31.108	0.013	93.99
93.52	0	0				
C16		L101A-S1	MAJ-1	18.852	0.013	93.69
93.33	0	0				
C17		L105A-S	MAJ-2	9.672	0.013	93.46
93.24	0	0				
C3		L101A-S	L101A-S1	14.841	0.013	93.5
93.69	0	0				
C4		L102A-S1	L101A-S	18.287	0.013	93.77
93.5	0	0				
C5		L102A-S	L102A-S1	18.719	0.013	93.67
93.77	0	0				
C6		L102B-S1	L102A-S	20.072	0.013	93.87
93.67	0	0				
C7		L102C-S	L102C-S1	10.31	0.013	93.9
94.1	0	0				
C8		L102C-S1	L102B-S1	46.399	0.013	94.1
93.87	0	0				
C9		L102B-S	L102B-S1	21.768	0.013	93.52
93.87	0	0				
Pipe_3		STM-102	STM-101	82.253	0.013	91.46
91.131	0	0				
Pipe_4		STM-103	STM-101	41.48	0.013	91.371
91.206	0	0				
Pipe_46		STM-101	DICB221	25.2	0.013	90.8
90.72	0	0				
Pipe_5		STM-104	STM-103	68.185	0.013	91.704
91.431	0	0				
Pipe_6		STM-105	STM-103	26.256	0.013	91.532
91.401	0	0				

[ORIFICES]

;;	Inlet	Outlet	Orifice	Crest	Disch.
;;	Flap Open/Close				
;;	Name	Node	Type	Height	Coeff.
;;	Gate Time				
;;	-----	-----	-----	-----	-----
L101A-IC	L101A-S	STM-101	SIDE	92.12	0.572
NO 0					
L102A-IC	L102A-S	STM-101	SIDE	92.29	0.572
NO 0					
L102C-IC	L102C-S	STM-102	SIDE	92.52	0.572
NO 0					
L102D-IC	L102D-S	STM-102	SIDE	92.26	0.572
NO 0					

160401085-2020-07-30-5yr-100BC.inp

L103B-IC	L102B-S	STM-102	SIDE	91.94	0.572
NO 0					
L105A-IC	L105A-S	STM-105	SIDE	92.08	0.572
NO 0					

[XSECTIONS]

;;	Link	Shape	Geom1	Geom2	Geom3	Geom4
;;	Barrels					
;;	-----	-----	-----	-----	-----	-----
C1		IRREGULAR	Half-12m-MC	0	0	0
C10		IRREGULAR	Half-17m-MC	0	0	0
C11		IRREGULAR	Half-17m-MC	0	0	0
C12		IRREGULAR	Half-17m-MC	0	0	0
C13		IRREGULAR	Half-17m-MC	0	0	0
C14		IRREGULAR	Half-12m-MC	0	0	0
C15		IRREGULAR	Half-17m-MC	0	0	0
C16		IRREGULAR	Half-17m-MC	0	0	0
C17		IRREGULAR	Half-8.5mROW	0	0	0
C3		IRREGULAR	Half-17m-MC	0	0	0
C4		IRREGULAR	Half-17m-MC	0	0	0
C5		IRREGULAR	Half-17m-MC	0	0	0
C6		IRREGULAR	Half-17m-MC	0	0	0
C7		IRREGULAR	Half-17m-MC	0	0	0
C8		IRREGULAR	Half-17m-MC	0	0	0
C9		IRREGULAR	Half-17m-MC	0	0	0
Pipe_3		CIRCULAR	0.45	0	0	0
Pipe_4		CIRCULAR	0.3	0	0	0
Pipe_46		CIRCULAR	0.675	0	0	0

```

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Pipe_5      CIRCULAR    0.3      0      0      1
Pipe_6      CIRCULAR    0.3      0      0      1
L101A-IC   CIRCULAR    0.178    0      0      0
L102A-IC   CIRCULAR    0.108    0      0      0
L102C-IC   CIRCULAR    0.083    0      0      0
L102D-IC   CIRCULAR    0.102    0      0      0
L103B-IC   CIRCULAR    0.178    0      0      0
L105A-IC   CIRCULAR    0.083    0      0      0

```

[TRANSECTS]

;Full street, width = 8.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.245m.

```

NC 0.025 0.025 0.013
X1 18mROW 7 10 18.5 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.13 14.25 0
18.5
GR 0.15 18.5 0.35 28.5

```

;Full street, width = 11m, curb = 0.15m , cross-slope = 0.016m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

```

NC 0.025 0.025 0.013
X1 24mROW 7 10 21 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.165 15.5 0
21
GR 0.15 21 0.35 31

```

;Full street, width = 5.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

```

NC 0.025 0.025 0.013
X1 8.5m-ROW 7 10 15.5 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.13 12.75 0
15.5
GR 0.15 15.5 0.35 25.5

```

```

NC 0.013 0.025 0.013
X1 Half-12m-MC 6 3 9 0.0 0.0 0.0 0.0
0.0
GR 0.17 0 0.11 3 0.06 3 0 9 0.05
9
GR 0.11 12

```

```

NC 0.013 0.025 0.013

```

```

160401085-2020-07-30-5yr-100BC.inp
X1 Half-17m-MC 6 3 14 0.0 0.0 0.0 0.0
0.0
GR 0.36 0 0.27 3 0.22 3 0 14 0.05
14
GR 0.14 17
NC 0.013 0.025 0.013
X1 Half-18mROW 5 0.0 4.25 0.0 0.0 0.0 0.0
0.0
GR 0.13 0 0 4.25 0.15 4.25 0.35 10 0.4
12.5
NC 0.013 0.025 0.013
X1 Half-8.5mROW 5 0.0 2.75 0.0 0.0 0.0 0.0
0.0
GR 0.13 0 0 2.75 0.15 2.75 0.35 10 0.4
12.5

```

[LOSSES]

;;Link	Inlet	Outlet	Average	Flap Gate	SeepageRate
Pipe_4	0	1.32	0	NO	0
Pipe_46	0.02	1.32	0	NO	0
Pipe_5	0	1.32	0	NO	0
Pipe_6	0	0.02	0	NO	0

[CURVES]

;;Name	Type	X-Value	Y-Value
C202A-Q	Rating	0	0
C202A-Q		1.38	116.6
C202A-Q		1.73	128.3
C202A-Q		2.08	130.6
L201A-Q	Rating	0	0
L201A-Q		1.38	79.8
L201A-Q		1.73	87.8
L201A-Q		2.08	89.4
L201C-Q	Rating	0	0
L201C-Q		1.38	27.6
L201C-Q		1.73	30.4
L201C-Q		2.08	30.9
L202B-Q	Rating	0	0
L202B-Q		1.38	127
L202B-Q		1.73	139.7
L202B-Q		2.08	142.2

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L202C-Q	Rating	0	0
L202C-Q		1.38	20.3
L202C-Q		1.68	22.3
L203A-Q	Rating	0	0
L203A-Q		1.38	52.3
L203A-Q		1.73	57.5
L203A-Q		2.08	58.6
L204A-Q	Rating	0	0
L204A-Q		1.38	246.7
L204A-Q		1.73	271.4
L204A-Q		2.08	276.3
L205AA-Q	Rating	0	0
L205AA-Q		1.38	88.2
L205AA-Q		1.73	97
L205A-Q	Rating	0	0
L205A-Q		1.38	208.6
L205A-Q		1.73	229.5
L205A-Q		2.08	233.6
L205C-Q	Rating	0	0
L205C-Q		1.38	123.6
L205C-Q		1.73	136
L205C-Q		2.08	138.4
L215A-Q	Rating	0	0
L215A-Q		1.38	262.8
L215A-Q		1.73	289.1
L215A-Q		2.08	294.3
L216A-Q	Rating	0	0
L216A-Q		2.3	76
L216A-Q		2.6	83.6
L216A-Q		2.9	85.12
L218A-Q	Rating	0	0
L218A-Q		1.38	60.3
L218A-Q		1.73	66.3
L218A-Q		2.08	67.5
L221A-Q	Rating	0	0
L221A-Q		1.38	243.5
L221A-Q		1.73	267.9
L221A-Q		2.08	272.7

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L222A-Q	Rating	0	0
L222A-Q		1.38	245.1
L222A-Q		1.73	269.6
L222A-Q		2.08	274.5
102C-V	Storage	0	0
102C-V		1.38	0
102C-V		1.58	115
102C-V		1.581	0
102C-V		1.73	0
C202A-V	Storage	0	0
C202A-V		1.38	0
C202A-V		1.73	933
C202A-V		1.731	0
C202A-V		2.08	0
C300A-V	Storage	0	0
C300A-V		1.38	0
C300A-V		1.39	1
C300A-V		1.4	0
C300A-V		1.73	0
C302A-V	Storage	0	0
C302A-V		1.38	0
C302A-V		1.39	1
C302A-V		1.4	0
C302A-V		1.73	0
L101A-V	Storage	0	0
L101A-V		1.38	0
L101A-V		1.57	105
L101A-V		1.571	0
L101A-V		1.73	0
L102A-V	Storage	0	0
L102A-V		1.38	0
L102A-V		1.48	44
L102A-V		1.481	0
L102A-V		1.73	0
L102B-V	Storage	0	0
L102B-V		1.58	0
L102B-V		1.93	600
L102B-V		1.931	0
L102D-V	Storage	0	0

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L102D-V		1.38	0
L102D-V		1.68	138
L102D-V		1.681	0
L102D-V		1.73	0
L201A-V	Storage	0	0
L201A-V		1.38	0
L201A-V		1.73	88
L201A-V		1.731	0
L201A-V		2.08	0
L201C-V	Storage	0	0
L201C-V		1.38	0
L201C-V		1.73	44
L201C-V		1.731	0
L201C-V		2.08	0
L203A-V	Storage	0	0
L203A-V		1.38	0
L203A-V		1.73	78
L203A-V		1.731	0
L203A-V		2.08	0
L204AA-V	Storage	0	0
L204AA-V		1.38	0
L204AA-V		1.73	116
L204AA-V		1.731	0
L204AA-V		2.08	0
L204A-V	Storage	0	0
L204A-V		1.38	0
L204A-V		1.73	1700
L204A-V		1.731	0
L204A-V		2.08	0
L205AA-V	Storage	0	0
L205AA-V		1.38	0
L205AA-V		1.73	693
L205A-V	Storage	0	0
L205A-V		1.38	0
L205A-V		1.73	1130
L205A-V		1.731	0
L205A-V		2.08	0
L205C-V	Storage	0	0
L205C-V		1.38	0
L205C-V		1.73	180

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L205C-V		1.731	0
L205C-V		2.08	0
L215A-V	Storage	0	0
L215A-V		1.38	0
L215A-V		1.73	487
L215A-V		1.731	0
L215A-V		2.08	0
L216A-V	Storage	0	0
L216A-V		2.3	0
L216A-V		2.6	22000
L216A-V		2.601	0
L216A-V		2.9	0
L221A-V	Storage	0	0
L221A-V		1.38	0
L221A-V		1.73	452
L221A-V		1.731	0
L221A-V		2.08	0
L222A-V	Storage	0	0
L222A-V		1.38	0
L222A-V		1.73	458
L222A-V		1.731	0
L222A-V		2.08	0
L400A-V	Storage	0	0
L400A-V		1.38	0
L400A-V		1.39	1
L400A-V		1.4	0
L400A-V		1.73	0
L402A-V	Storage	0	0
L402A-V		1.38	0
L402A-V		1.39	4700
L402A-V		1.4	0
L402A-V		1.73	0

[TIMESERIES]

;;Name	Date	Time	Value
;-----			
;100yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)			
C100yr-4hr		0:00	0.00
C100yr-4hr		0:10	4.39
C100yr-4hr		0:20	5.07
C100yr-4hr		0:30	6.05
C100yr-4hr		0:40	7.54

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C100yr-4hr	0:50	10.16
C100yr-4hr	1:00	15.97
C100yr-4hr	1:10	40.65
C100yr-4hr	1:20	178.56
C100yr-4hr	1:30	54.05
C100yr-4hr	1:40	27.32
C100yr-4hr	1:50	18.24
C100yr-4hr	2:00	13.74
C100yr-4hr	2:10	11.06
C100yr-4hr	2:20	9.29
C100yr-4hr	2:30	8.02
C100yr-4hr	2:40	7.08
C100yr-4hr	2:50	6.35
C100yr-4hr	3:00	5.76
C100yr-4hr	3:10	5.28
C100yr-4hr	3:20	4.88
C100yr-4hr	3:30	4.54
C100yr-4hr	3:40	4.25
C100yr-4hr	3:50	3.99
C100yr-4hr	4:00	3.77

;100yr +20% - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C100yr-4hr+20%	0:00	0.00
C100yr-4hr+20%	0:10	5.27
C100yr-4hr+20%	0:20	6.08
C100yr-4hr+20%	0:30	7.26
C100yr-4hr+20%	0:40	9.05
C100yr-4hr+20%	0:50	12.19
C100yr-4hr+20%	1:00	19.16
C100yr-4hr+20%	1:10	48.78
C100yr-4hr+20%	1:20	214.27
C100yr-4hr+20%	1:30	64.86
C100yr-4hr+20%	1:40	32.78
C100yr-4hr+20%	1:50	21.89
C100yr-4hr+20%	2:00	16.49
C100yr-4hr+20%	2:10	13.27
C100yr-4hr+20%	2:20	11.15
C100yr-4hr+20%	2:30	9.62
C100yr-4hr+20%	2:40	8.50
C100yr-4hr+20%	2:50	7.62
C100yr-4hr+20%	3:00	6.91
C100yr-4hr+20%	3:10	6.34
C100yr-4hr+20%	3:20	5.86
C100yr-4hr+20%	3:30	5.45
C100yr-4hr+20%	3:40	5.10
C100yr-4hr+20%	3:50	4.79
C100yr-4hr+20%	4:00	4.52

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;25mm - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C25mm-4hr	0:00	0.00
C25mm-4hr	0:10	1.51
C25mm-4hr	0:20	1.75
C25mm-4hr	0:30	2.07
C25mm-4hr	0:40	2.58
C25mm-4hr	0:50	3.46
C25mm-4hr	1:00	5.39
C25mm-4hr	1:10	13.44
C25mm-4hr	1:20	56.67
C25mm-4hr	1:30	17.77
C25mm-4hr	1:40	9.12
C25mm-4hr	1:50	6.14
C25mm-4hr	2:00	4.65
C25mm-4hr	2:10	3.76
C25mm-4hr	2:20	3.17
C25mm-4hr	2:30	2.74
C25mm-4hr	2:40	2.43
C25mm-4hr	2:50	2.18
C25mm-4hr	3:00	1.98
C25mm-4hr	3:10	1.81
C25mm-4hr	3:20	1.68
C25mm-4hr	3:30	1.56
C25mm-4hr	3:40	1.47
C25mm-4hr	3:50	1.38
C25mm-4hr	4:00	1.31

;2yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C2yr-4hr	0:00	0.00
C2yr-4hr	0:10	2.05
C2yr-4hr	0:20	2.37
C2yr-4hr	0:30	2.81
C2yr-4hr	0:40	3.50
C2yr-4hr	0:50	4.69
C2yr-4hr	1:00	7.30
C2yr-4hr	1:10	18.21
C2yr-4hr	1:20	76.81
C2yr-4hr	1:30	24.08
C2yr-4hr	1:40	12.36
C2yr-4hr	1:50	8.32
C2yr-4hr	2:00	6.30
C2yr-4hr	2:10	5.09
C2yr-4hr	2:20	4.29
C2yr-4hr	2:30	3.72
C2yr-4hr	2:40	3.29
C2yr-4hr	2:50	2.95
C2yr-4hr	3:00	2.68
C2yr-4hr	3:10	2.46

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C2yr-4hr	3:20	2.28
C2yr-4hr	3:30	2.12
C2yr-4hr	3:40	1.99
C2yr-4hr	3:50	1.87
C2yr-4hr	4:00	1.77

;5yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C5yr-4hr	0:00	0.00
C5yr-4hr	0:10	2.68
C5yr-4hr	0:20	3.10
C5yr-4hr	0:30	3.68
C5yr-4hr	0:40	4.58
C5yr-4hr	0:50	6.15
C5yr-4hr	1:00	9.61
C5yr-4hr	1:10	24.17
C5yr-4hr	1:20	104.19
C5yr-4hr	1:30	32.04
C5yr-4hr	1:40	16.34
C5yr-4hr	1:50	10.96
C5yr-4hr	2:00	8.29
C5yr-4hr	2:10	6.69
C5yr-4hr	2:20	5.63
C5yr-4hr	2:30	4.87
C5yr-4hr	2:40	4.30
C5yr-4hr	2:50	3.86
C5yr-4hr	3:00	3.51
C5yr-4hr	3:10	3.22
C5yr-4hr	3:20	2.98
C5yr-4hr	3:30	2.77
C5yr-4hr	3:40	2.60
C5yr-4hr	3:50	2.44
C5yr-4hr	4:00	2.31

Chicago_2yr_3hr_10m_ottawa	0:10	2.81459
Chicago_2yr_3hr_10m_ottawa	0:20	3.49824
Chicago_2yr_3hr_10m_ottawa	0:30	4.68718
Chicago_2yr_3hr_10m_ottawa	0:40	7.30485
Chicago_2yr_3hr_10m_ottawa	0:50	18.20881
Chicago_2yr_3hr_10m_ottawa	1:00	76.805
Chicago_2yr_3hr_10m_ottawa	1:10	24.07906
Chicago_2yr_3hr_10m_ottawa	1:20	12.36376
Chicago_2yr_3hr_10m_ottawa	1:30	8.32403
Chicago_2yr_3hr_10m_ottawa	1:40	6.30341
Chicago_2yr_3hr_10m_ottawa	1:50	5.09498
Chicago_2yr_3hr_10m_ottawa	2:00	4.29133
Chicago_2yr_3hr_10m_ottawa	2:10	3.71786
Chicago_2yr_3hr_10m_ottawa	2:20	3.28762
Chicago_2yr_3hr_10m_ottawa	2:30	2.95254

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Chicago_2yr_3hr_10m_ottawa	2:40	2.68388
Chicago_2yr_3hr_10m_ottawa	2:50	2.46348
Chicago_2yr_3hr_10m_ottawa	3:00	2.27921

Chicago_5y_3h_10m_City	0:00	3.68223
Chicago_5y_3h_10m_City	0:10	4.58232
Chicago_5y_3h_10m_City	0:20	6.15055
Chicago_5y_3h_10m_City	0:30	9.6141
Chicago_5y_3h_10m_City	0:40	24.17035
Chicago_5y_3h_10m_City	0:50	104.193
Chicago_5y_3h_10m_City	1:00	32.03692
Chicago_5y_3h_10m_City	1:10	16.3375
Chicago_5y_3h_10m_City	1:20	10.96479
Chicago_5y_3h_10m_City	1:30	8.28693
Chicago_5y_3h_10m_City	1:40	6.68897
Chicago_5y_3h_10m_City	1:50	5.6279
Chicago_5y_3h_10m_City	2:00	4.87167
Chicago_5y_3h_10m_City	2:10	4.30483
Chicago_5y_3h_10m_City	2:20	3.8637
Chicago_5y_3h_10m_City	2:30	3.51028
Chicago_5y_3h_10m_City	2:40	3.22046
Chicago_5y_3h_10m_City	2:50	2.97831

Chicago100y_3h_10m_City	00:00	6.04573
Chicago100y_3h_10m_City	00:10	7.54219
Chicago100y_3h_10m_City	00:20	10.15880
Chicago100y_3h_10m_City	00:30	15.96889
Chicago100y_3h_10m_City	00:40	40.65497
Chicago100y_3h_10m_City	00:50	178.55900
Chicago100y_3h_10m_City	01:00	54.04853
Chicago100y_3h_10m_City	01:10	27.31870
Chicago100y_3h_10m_City	01:20	18.24039
Chicago100y_3h_10m_City	01:30	13.73692
Chicago100y_3h_10m_City	01:40	11.05876
Chicago100y_3h_10m_City	01:50	9.28521
Chicago100y_3h_10m_City	02:00	8.02389
Chicago100y_3h_10m_City	02:10	7.08022
Chicago100y_3h_10m_City	02:20	6.34698
Chicago100y_3h_10m_City	02:30	5.76029
Chicago100y_3h_10m_City	02:40	5.27978
Chicago100y_3h_10m_City	02:50	4.87871

Chicago100y+20%_3h_10m_City	00:00	7.255
Chicago100y+20%_3h_10m_City	00:10	9.051
Chicago100y+20%_3h_10m_City	00:20	12.191
Chicago100y+20%_3h_10m_City	00:30	19.163
Chicago100y+20%_3h_10m_City	00:40	48.786
Chicago100y+20%_3h_10m_City	00:50	214.271

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Chicago100y+20%_3h_10m_City	01:00	64.858
Chicago100y+20%_3h_10m_City	01:10	32.782
Chicago100y+20%_3h_10m_City	01:20	21.888
Chicago100y+20%_3h_10m_City	01:30	16.484
Chicago100y+20%_3h_10m_City	01:40	13.271
Chicago100y+20%_3h_10m_City	01:50	11.142
Chicago100y+20%_3h_10m_City	02:00	9.629
Chicago100y+20%_3h_10m_City	02:10	8.496
Chicago100y+20%_3h_10m_City	02:20	7.616
Chicago100y+20%_3h_10m_City	02:30	6.912
Chicago100y+20%_3h_10m_City	02:40	6.336
Chicago100y+20%_3h_10m_City	02:50	5.854

;100yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S100yr-12hr	0:00	0.00
S100yr-12hr	0:30	2.82
S100yr-12hr	1:00	1.31
S100yr-12hr	1:30	2.44
S100yr-12hr	2:00	2.44
S100yr-12hr	2:30	3.19
S100yr-12hr	3:00	2.82
S100yr-12hr	3:30	3.76
S100yr-12hr	4:00	3.76
S100yr-12hr	4:30	5.07
S100yr-12hr	5:00	6.39
S100yr-12hr	5:30	10.14
S100yr-12hr	6:00	80.38
S100yr-12hr	6:30	20.47
S100yr-12hr	7:00	9.02
S100yr-12hr	7:30	6.01
S100yr-12hr	8:00	5.26
S100yr-12hr	8:30	4.13
S100yr-12hr	9:00	4.32
S100yr-12hr	9:30	2.82
S100yr-12hr	10:00	2.25
S100yr-12hr	10:30	3.19
S100yr-12hr	11:00	2.07
S100yr-12hr	11:30	1.88
S100yr-12hr	12:00	1.88

;2yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S2yr-12hr	0:00	0.00
S2yr-12hr	0:30	1.27
S2yr-12hr	1:00	0.59
S2yr-12hr	1:30	1.10
S2yr-12hr	2:00	1.10
S2yr-12hr	2:30	1.44
S2yr-12hr	3:00	1.27

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S2yr-12hr	3:30	1.69
S2yr-12hr	4:00	1.69
S2yr-12hr	4:30	2.29
S2yr-12hr	5:00	2.88
S2yr-12hr	5:30	4.57
S2yr-12hr	6:00	36.24
S2yr-12hr	6:30	9.23
S2yr-12hr	7:00	4.06
S2yr-12hr	7:30	2.71
S2yr-12hr	8:00	2.37
S2yr-12hr	8:30	1.86
S2yr-12hr	9:00	1.95
S2yr-12hr	9:30	1.27
S2yr-12hr	10:00	1.02
S2yr-12hr	10:30	1.44
S2yr-12hr	11:00	0.93
S2yr-12hr	11:30	0.85
S2yr-12hr	12:00	0.85

;5yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S5yr-12hr	0:00	0.00
S5yr-12hr	0:30	1.69
S5yr-12hr	1:00	0.79
S5yr-12hr	1:30	1.46
S5yr-12hr	2:00	1.46
S5yr-12hr	2:30	1.91
S5yr-12hr	3:00	1.69
S5yr-12hr	3:30	2.25
S5yr-12hr	4:00	2.25
S5yr-12hr	4:30	3.03
S5yr-12hr	5:00	3.82
S5yr-12hr	5:30	6.07
S5yr-12hr	6:00	48.08
S5yr-12hr	6:30	12.25
S5yr-12hr	7:00	5.39
S5yr-12hr	7:30	3.60
S5yr-12hr	8:00	3.15
S5yr-12hr	8:30	2.47
S5yr-12hr	9:00	2.58
S5yr-12hr	9:30	1.69
S5yr-12hr	10:00	1.35
S5yr-12hr	10:30	1.91
S5yr-12hr	11:00	1.24
S5yr-12hr	11:30	1.12
S5yr-12hr	12:00	1.12

[REPORT]
INPUT YES

CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]

Node	DICB221	MH-C
Node	L101A-S	CB
Node	L102A-S	CB
Node	L102B-S	CB
Node	L102C-S	CB
Node	L102D-S	CB
Node	L105A-S	CB
Node	STM-101	MH-C
Node	STM-102	MH-C
Node	STM-103	MH-C
Node	STM-104	MH-C
Node	STM-105	MH-C
Link	C1	MAJ
Link	C10	MAJ
Link	C11	MAJ
Link	C12	MAJ
Link	C13	MAJ
Link	C14	MAJ
Link	C15	MAJ
Link	C16	MAJ
Link	C17	MAJ
Link	C3	MAJ
Link	C4	MAJ
Link	C5	MAJ
Link	C6	MAJ
Link	C7	MAJ
Link	C8	MAJ
Link	C9	MAJ
Link	Pipe_3	STM
Link	Pipe_4	STM
Link	Pipe_46	STM
Link	Pipe_5	STM
Link	Pipe_6	STM
Link	L101A-IC	RoadCB
Link	L102A-IC	RoadCB
Link	L102C-IC	RoadCB
Link	L102D-IC	RoadCB
Link	L103B-IC	RoadCB
Link	L105A-IC	RoadCB

[MAP]
DIMENSIONS 363780.9547 5013769.74325 363974.0113 5013954.18575

UNITS Meters

[COORDINATES]

;;Node	X-Coord	Y-Coord
;;-----		
DICB221	363809.615	5013844.287
MAJ-1	363801.696	5013833.174
MAJ-2	363872.589	5013785.283
OF1	363815.06	5013801.877
OF2	363927.001	5013812.438
OF3	363935.684	5013936.112
OF4	363951.599	5013825.948
L101A1-S	363885.226	5013841.386
L101A-S	363830.675	5013850.347
L101A-S1	363817.757	5013843.042
L102A-S	363859.441	5013857.072
L102A-S1	363850.337	5013854.166
L102B-S	363890.759	5013864.523
L102B-S1	363873.778	5013871.118
L102B-S2	363911.787	5013860.656
L102C-S	363915.2	5013903
L102C-S1	363904.9	5013903.432
L102D-S	363910.176	5013871.083
L102D-S1	363900.795	5013886.481
L105A-S	363868.4	5013794
L105A-S1	363848.35	5013816.157
STM-101	363833.3	5013856
STM-102	363904.6	5013897
STM-103	363853.9	5013820
STM-104	363913	5013854
STM-105	363866.9	5013798

[VERTICES]

;;Link	X-Coord	Y-Coord
;;-----		
C14	363850.866	5013819.484
C14	363846.381	5013820.418
C15	363897.883	5013851.268
C8	363889.802	5013891.91
C9	363886.162	5013871.512
C9	363880.374	5013873.874

[POLYGONS]

;;Subcatchment	X-Coord	Y-Coord
;;-----		
L101A	363824.67	5013885.528
L101A	363827.444	5013886.934

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L101A	363827.444	5013886.934
L101A	363835.034	5013873.662
L101A	363835.034	5013873.662
L101A	363843.407	5013862.561
L101A	363843.407	5013862.561
L101A	363848.277	5013858.427
L101A	363848.277	5013858.427
L101A	363855.956	5013845.052
L101A	363855.956	5013845.052
L101A	363876.545	5013856.872
L101A	363876.545	5013856.872
L101A	363884.203	5013843.532
L101A	363884.203	5013843.532
L101A	363891.025	5013831.652
L101A	363891.025	5013831.652
L101A	363889.68	5013830.88
L101A	363889.68	5013830.88
L101A	363898.703	5013815.164
L101A	363898.703	5013815.164
L101A	363895.839	5013813.52
L101A	363895.839	5013813.52
L101A	363895.586	5013813.961
L101A	363895.586	5013813.961
L101A	363892.811	5013812.367
L101A	363892.811	5013812.367
L101A	363893.007	5013812.026
L101A	363893.007	5013812.026
L101A	363890.308	5013810.476
L101A	363890.308	5013810.476
L101A	363890.004	5013811.005
L101A	363890.004	5013811.005
L101A	363884.299	5013807.73
L101A	363884.299	5013807.73
L101A	363884.602	5013807.201
L101A	363884.602	5013807.201
L101A	363881.904	5013805.652
L101A	363881.904	5013805.652
L101A	363881.297	5013806.709
L101A	363881.297	5013806.709
L101A	363878.795	5013805.273
L101A	363878.795	5013805.273
L101A	363869.724	5013821.065
L101A	363869.724	5013821.065
L101A	363866.6	5013826.504
L101A	363866.6	5013826.504
L101A	363853.592	5013819.03
L101A	363853.592	5013819.03
L101A	363843.731	5013813.398

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L101A	363843.731	5013813.398
L101A	363842.663	5013815.246
L101A	363842.663	5013815.246
L101A	363839.177	5013813.243
L101A	363839.177	5013813.243
L101A	363830.126	5013808.042
L101A	363830.126	5013808.042
L101A	363828.481	5013810.905
L101A	363828.481	5013810.905
L101A	363828.921	5013811.158
L101A	363828.921	5013811.158
L101A	363827.327	5013813.933
L101A	363827.327	5013813.933
L101A	363825.347	5013816.589
L101A	363825.347	5013816.589
L101A	363822.246	5013821.985
L101A	363822.246	5013821.985
L101A	363822.775	5013822.288
L101A	363822.775	5013822.288
L101A	363819.497	5013827.992
L101A	363819.497	5013827.992
L101A	363818.969	5013827.689
L101A	363818.969	5013827.689
L101A	363817.419	5013830.386
L101A	363817.419	5013830.386
L101A	363817.759	5013830.585
L101A	363817.759	5013830.585
L101A	363816.164	5013833.36
L101A	363816.164	5013833.36
L101A	363815.724	5013833.107
L101A	363815.724	5013833.107
L101A	363814.079	5013835.97
L101A	363814.079	5013835.97
L101A	363819.989	5013839.365
L101A	363819.989	5013839.365
L101A	363815.759	5013846.754
L101A	363815.759	5013846.754
L101A	363811.72	5013852.739
L101A	363811.72	5013852.739
L101A	363805.735	5013856.908
L101A	363805.735	5013856.908
L101A	363798.145	5013870.18
L101A	363798.145	5013870.18
L101A	363800.151	5013871.327
L101A	363800.151	5013871.327
L101A	363800	5013871.592
L101A	363800	5013871.592
L101A	363801.764	5013872.6

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L101A	363801.764	5013872.6
L101A	363800.554	5013874.717
L101A	363800.554	5013874.717
L101A	363805.051	5013877.289
L101A	363805.051	5013877.289
L101A	363806.261	5013875.172
L101A	363806.261	5013875.172
L101A	363808.036	5013876.187
L101A	363808.036	5013876.187
L101A	363813.14	5013879.106
L101A	363813.14	5013879.106
L101A	363811.93	5013881.222
L101A	363811.93	5013881.222
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L105A	363833.315	5013802.491
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UNC-1	363816.164	5013833.36
UNC-1	363816.164	5013833.36
UNC-1	363817.759	5013830.585
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UNC-1	363817.419	5013830.386
UNC-1	363817.419	5013830.386
UNC-1	363818.969	5013827.689
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UNC-1	363819.497	5013827.992
UNC-1	363819.497	5013827.992
UNC-1	363822.775	5013822.288
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UNC-1	363822.246	5013821.985
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UNC-1	363825.347	5013816.589
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UNC-1	363827.327	5013813.933
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UNC-1	363828.921	5013811.158
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UNC-1	363828.481	5013810.905
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UNC-1	363830.126	5013808.042

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UNC-1	363830.126	5013808.042
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UNC-1	363842.663	5013815.246
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UNC-1	363843.731	5013813.398
UNC-1	363843.731	5013813.398
UNC-1	363844.208	5013812.558
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UNC-1	363831.67	5013805.354
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UNC-1	363833.315	5013802.491
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UNC-1	363836.56	5013797.075
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UNC-1	363840.366	5013791.674
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UNC-1	363839.837	5013791.371
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UNC-1	363841.387	5013788.673
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UNC-1	363843.006	5013785.677
UNC-1	363844.803	5013782.55
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UNC-1	363843.215	5013778.127
UNC-1	363843.215	5013778.127
UNC-1	363839.241	5013779.3
UNC-1	363839.241	5013779.3
UNC-1	363789.73	5013875.495
UNC-1	363789.73	5013875.495
UNC-1	363863.823	5013917.865

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UNC-1	363863.823	5013917.865
UNC-1	363868.036	5013910.495
UNC-1	363868.036	5013910.495
UNC-1	363863.278	5013907.774
UNC-1	363863.278	5013907.774
UNC-1	363862.068	5013909.891
UNC-1	363862.068	5013909.891
UNC-1	363857.57	5013907.32
UNC-1	363857.57	5013907.32
UNC-1	363858.78	5013905.203
UNC-1	363858.78	5013905.203
UNC-1	363857.016	5013904.195
UNC-1	363857.016	5013904.195
UNC-1	363857.167	5013903.93
UNC-1	363857.167	5013903.93
UNC-1	363855.161	5013902.783
UNC-1	363855.161	5013902.783
UNC-1	363855.954	5013901.396
UNC-1	363855.954	5013901.396
UNC-1	363851.651	5013898.935
UNC-1	363851.651	5013898.935
UNC-1	363850.858	5013900.323
UNC-1	363850.858	5013900.323
UNC-1	363848.851	5013899.176
UNC-1	363848.851	5013899.176
UNC-1	363848.7	5013899.44
UNC-1	363848.7	5013899.44
UNC-1	363846.936	5013898.432
UNC-1	363846.936	5013898.432
UNC-1	363845.726	5013900.548
UNC-1	363845.726	5013900.548
UNC-1	363841.229	5013897.976
UNC-1	363841.229	5013897.976
UNC-1	363842.439	5013895.86
UNC-1	363842.439	5013895.86
UNC-1	363835.56	5013891.927
UNC-1	363835.56	5013891.927
UNC-1	363834.35	5013894.043
UNC-1	363834.35	5013894.043
UNC-1	363829.852	5013891.471
UNC-1	363829.852	5013891.471
UNC-1	363831.063	5013889.355
UNC-1	363831.063	5013889.355
UNC-1	363829.299	5013888.346
UNC-1	363829.299	5013888.346
UNC-1	363829.45	5013888.082
UNC-1	363829.45	5013888.082
UNC-1	363827.444	5013886.934

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UNC-1	363827.444	5013886.934
UNC-1	363824.67	5013885.528
UNC-1	363824.67	5013885.528
UNC-1	363823.107	5013884.635
UNC-1	363823.107	5013884.635
UNC-1	363821.559	5013883.569
UNC-1	363821.559	5013883.569
UNC-1	363819.552	5013882.422
UNC-1	363819.552	5013882.422
UNC-1	363819.401	5013882.686
UNC-1	363819.401	5013882.686
UNC-1	363817.637	5013881.678
UNC-1	363817.637	5013881.678
UNC-1	363816.427	5013883.794
UNC-1	363816.427	5013883.794
UNC-1	363811.93	5013881.222
UNC-1	363811.93	5013881.222
UNC-1	363813.14	5013879.106
UNC-1	363813.14	5013879.106
UNC-1	363808.036	5013876.187
UNC-1	363808.036	5013876.187
UNC-1	363806.261	5013875.172
UNC-1	363806.261	5013875.172
UNC-1	363805.051	5013877.289
UNC-1	363805.051	5013877.289
UNC-1	363800.554	5013874.717
UNC-1	363800.554	5013874.717
UNC-1	363801.764	5013872.6
UNC-1	363801.764	5013872.6
UNC-1	363800	5013871.592
UNC-1	363800	5013871.592
UNC-1	363800.151	5013871.327
UNC-1	363800.151	5013871.327
UNC-1	363798.145	5013870.18
UNC-1	363798.145	5013870.18
UNC-1	363805.735	5013856.908
UNC-1	363805.735	5013856.908
UNC-1	363811.72	5013852.739
UNC-1	363811.72	5013852.739
UNC-1	363815.759	5013846.754
UNC-1	363815.759	5013846.754
UNC-1	363819.989	5013839.365
UNC-1	363819.989	5013839.365
UNC-1	363814.079	5013835.97
UNC-1	363814.079	5013835.97
UNC-1	363815.724	5013833.107
UNC-2	363917.402	5013826.037
UNC-2	363919.785	5013822.096

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UNC-2	363874.339	5013795.992
UNC-2	363875.868	5013802.085
UNC-2	363878.996	5013803.88
UNC-2	363878.345	5013805.015
UNC-2	363878.795	5013805.273
UNC-2	363881.297	5013806.709
UNC-2	363881.904	5013805.652
UNC-2	363884.602	5013807.201
UNC-2	363884.299	5013807.73
UNC-2	363890.004	5013811.005
UNC-2	363890.308	5013810.476
UNC-2	363893.007	5013812.026
UNC-2	363892.811	5013812.367
UNC-2	363895.586	5013813.961
UNC-2	363895.839	5013813.52
UNC-2	363898.703	5013815.164
UNC-2	363889.68	5013830.88
UNC-2	363891.025	5013831.652
UNC-2	363892.37	5013832.424
UNC-2	363901.397	5013816.711
UNC-2	363904.26	5013818.356
UNC-2	363904.007	5013818.796
UNC-2	363906.782	5013820.39
UNC-2	363906.978	5013820.049
UNC-2	363909.677	5013821.599
UNC-2	363909.373	5013822.128
UNC-2	363915.078	5013825.405
UNC-2	363915.381	5013824.876
UNC-2	363917.402	5013826.037
UNC-3	363964.106	5013847.567
UNC-3	363958.387	5013847.945
UNC-3	363957.275	5013849.936
UNC-3	363957.541	5013850.085
UNC-3	363956.55	5013851.859
UNC-3	363958.679	5013853.048
UNC-3	363956.153	5013857.571
UNC-3	363954.024	5013856.382
UNC-3	363950.165	5013863.303
UNC-3	363952.289	5013864.489
UNC-3	363949.763	5013869.012
UNC-3	363947.634	5013867.824
UNC-3	363946.654	5013869.585
UNC-3	363946.378	5013869.449
UNC-3	363945.251	5013871.467
UNC-3	363938.576	5013867.739
UNC-3	363936.012	5013872.332
UNC-3	363942.686	5013876.059
UNC-3	363941.561	5013878.077

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UNC-3	363941.826	5013878.225
UNC-3	363940.836	5013879.999
UNC-3	363942.965	5013881.188
UNC-3	363940.44	5013885.712
UNC-3	363938.31	5013884.523
UNC-3	363934.448	5013891.442
UNC-3	363936.577	5013892.63
UNC-3	363934.051	5013897.154
UNC-3	363931.922	5013895.966
UNC-3	363930.932	5013897.74
UNC-3	363930.666	5013897.591
UNC-3	363929.539	5013899.609
UNC-3	363928.855	5013902.193
UNC-3	363924.785	5013909.312
UNC-3	363921.391	5013913.361
UNC-3	363920.497	5013914.924
UNC-3	363917.371	5013920.395
UNC-3	363918.077	5013920.799
UNC-3	363917.925	5013921.063
UNC-3	363919.932	5013922.21
UNC-3	363912.342	5013935.483
UNC-3	363910.336	5013934.335
UNC-3	363910.185	5013934.6
UNC-3	363908.421	5013933.591
UNC-3	363907.211	5013935.708
UNC-3	363902.713	5013933.136
UNC-3	363903.923	5013931.019
UNC-3	363897.045	5013927.086
UNC-3	363895.834	5013929.202
UNC-3	363891.337	5013926.631
UNC-3	363892.547	5013924.514
UNC-3	363890.783	5013923.506
UNC-3	363890.935	5013923.241
UNC-3	363888.929	5013922.094
UNC-3	363889.722	5013920.706
UNC-3	363885.061	5013918.04
UNC-3	363879.367	5013914.785
UNC-3	363878.576	5013916.168
UNC-3	363876.57	5013915.021
UNC-3	363876.419	5013915.286
UNC-3	363874.655	5013914.278
UNC-3	363873.445	5013916.394
UNC-3	363868.947	5013913.823
UNC-3	363870.157	5013911.707
UNC-3	363868.036	5013910.495
UNC-3	363863.823	5013917.865
UNC-3	363912.677	5013945.802
UNC-3	363926.744	5013920.609

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UNC-3	363932.862	5013909.651
UNC-3	363965.236	5013851.67
UNC-3	363964.106	5013847.567
UNC-4	363919.785	5013822.096
UNC-4	363917.402	5013826.037
UNC-4	363918.079	5013826.426
UNC-4	363917.472	5013827.483
UNC-4	363920.423	5013829.179
UNC-4	363921.075	5013828.045
UNC-4	363924.202	5013829.841
UNC-4	363922.288	5013833.174
UNC-4	363932.242	5013838.731
UNC-4	363936.591	5013841.609
UNC-4	363938.164	5013842.488
UNC-4	363940.541	5013842.918
UNC-4	363942.67	5013844.107
UNC-4	363943.66	5013842.333
UNC-4	363943.929	5013842.483
UNC-4	363945.053	5013840.464
UNC-4	363958.402	5013847.919
UNC-4	363958.387	5013847.945
UNC-4	363964.106	5013847.567
UNC-4	363964.101	5013847.55
UNC-4	363919.785	5013822.096

[SYMBOLS]
;;Gage X-Coord Y-Coord
;;-----

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[TITLE]
 160401085 - SNTC Lands Assessment
 Detailed Design Block 4, located north of the Jock River and west of Greenbank Road.

```
[OPTIONS]
;;Options      Value
;;-----
FLOW_UNITS     LPS
INFILTRATION   HORTON
FLOW_ROUTING   DYNWAVE
LINK_OFFSETS   ELEVATION
MIN_SLOPE      0
ALLOW_PONDING  YES
SKIP_STEADY_STATE NO
START_DATE     01/02/2018
START_TIME     00:00:00
REPORT_START_DATE 01/02/2018
REPORT_START_TIME 00:00:00
END_DATE       01/03/2018
END_TIME       00:00:00
SWEEP_START    01/01
SWEEP_END      12/31
DRY_DAYS       0
REPORT_STEP    00:01:00
WET_STEP       00:01:00
DRY_STEP       00:01:00
ROUTING_STEP   2
RULE_STEP      00:00:00
INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP  0
LENGTHENING_STEP 0
MIN_SURFAREA   0
MAX_TRIALS     8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL   5
LAT_FLOW_TOL   5
MINIMUM_STEP   0.5
THREADS        6
```

```
[EVAPORATION]
;;Type      Parameters
;;-----
CONSTANT    0.0
DRY_ONLY    NO
```

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```
[RAINGAGES]
;;      Rain      Time      Snow      Data
;;Name   Type      Intrvl  Catch  Source
;;-----
Raingage INTENSITY 0:10   1.0    TIMESERIES Chicago100y_3h_10m_City

[SUBCATCHMENTS]
;;      Curb      Snow      Total      Pcnt.      Pcnt.
;;Name   Length  Pack  Raingage  Outlet  Area  Imperv  Width  Slope
;;-----
L101A    Raingage      L101A-S    0.371714  88.571  370  3
0
L102A    Raingage      L102A-S    0.155965  87.143  116  3
0
L102B    Raingage      L102B-S    0.416519  84.286  314  3
0
L102C    Raingage      L102C-S    0.091812  91.429  40  3
0
L102D    Raingage      L102D-S    0.124257  88.571  54  3
0
L105A    Raingage      L105A-S    0.111158  87.143  56  3
0
UNC-1    Raingage      OF1        0.142651  60  321  2
0
UNC-2    Raingage      OF2        0.03007  42.857  200  3
0
UNC-3    Raingage      OF3        0.1184  42.857  305  3
0
UNC-4    Raingage      OF4        0.02893  42.857  200  3
0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo
PctRouted
;;-----
L101A    0.013  0.25  1.57  4.67  0  OUTLET
L102A    0.013  0.25  1.57  4.67  0  OUTLET
L102B    0.013  0.25  1.57  4.67  0  OUTLET
L102C    0.013  0.25  1.57  4.67  0  OUTLET
L102D    0.013  0.25  1.57  4.67  0  OUTLET
L105A    0.013  0.25  1.57  4.67  0  OUTLET
UNC-1    0.013  0.25  1.57  4.67  0  PERVIOUS
90
```

```

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UNC-2      0.013    0.25    1.57    4.67    0      PERVIOUS
70
UNC-3      0.013    0.25    1.57    4.67    0      PERVIOUS
95
UNC-4      0.013    0.25    1.57    4.67    0      PERVIOUS
90

```

[INFILTRATION]

```

;;Subcatchment MaxRate MinRate Decay DryTime MaxInfil
;;-----
L101A      76.2     13.2    4.14    7        0
L102A      76.2     13.2    4.14    7        0
L102B      76.2     13.2    4.14    7        0
L102C      76.2     13.2    4.14    7        0
L102D      76.2     13.2    4.14    7        0
L105A      76.2     13.2    4.14    7        0
UNC-1      76.2     13.2    4.14    7        0
UNC-2      76.2     13.2    4.14    7        0
UNC-3      76.2     13.2    4.14    7        0
UNC-4      76.2     13.2    4.14    7        0

```

[OUTFALLS]

```

;;
;;Name Invert Elev. Outfall Type Stage/Table Time Series Tide Gate Route To
;;-----
DICB221 90.72 FIXED 91.36 NO
MAJ-1 93.33 FREE NO
MAJ-2 93.24 FREE NO
OF1 0 FREE NO
OF2 0 FREE NO
OF3 93.52 FREE NO
OF4 93.44 FREE NO

```

[STORAGE]

```

;;
;; Name Evap. Invert Max. Init. Storage Curve
;; Name Elev. Depth Depth Curve Params
;; Name Frac. Infiltration parameters
;;-----
L101A1-S 93.99 0.35 0 FUNCTIONAL 0 0 0 0
0
L101A-S 92.12 1.73 0 TABULAR L101A-V 0
0
L101A-S1 93.69 0.35 0 FUNCTIONAL 0 0 0 0
0
L102A-S 92.29 1.73 0 TABULAR L102A-V 0
0

```

```

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L102A-S1 93.77 0.35 0 FUNCTIONAL 0 0 0 0
0
L102B-S 91.94 1.93 0 TABULAR L102B-V 0
0
L102B-S1 93.87 0.35 0 FUNCTIONAL 0 0 0 0
0
L102B-S2 93.97 0.35 0 FUNCTIONAL 0 0 0 0
0
L102C-S 92.52 1.73 0 TABULAR 102C-V 0
0
L102C-S1 94.1 0.35 0 FUNCTIONAL 0 0 0 0
0
L102D-S 92.26 1.73 0 TABULAR L102D-V 0
0
L102D-S1 93.94 0.35 0 FUNCTIONAL 0 0 0 0
0
L105A-S 92.08 1.73 0 FUNCTIONAL 0 0 0 0
0
L105A-S1 93.91 0.35 0 FUNCTIONAL 0 0 0 0
0
STM-101 90.496 3.214 0.864 FUNCTIONAL 0 0 0 0
0
STM-102 91.16 2.977 0.2 FUNCTIONAL 0 0 0 0
0
STM-103 91.071 2.863 0.289 FUNCTIONAL 0 0 0 0
0
STM-104 91.404 2.583 0 FUNCTIONAL 0 0 0 0
0
STM-105 91.232 2.306 0.128 FUNCTIONAL 0 0 0 0
0

```

[CONDUITS]

```

;;
;; Inlet Inlet Outlet Manning Inlet
;; Name Init. Max. Node Length N Offset
;; Name Offset Flow Flow
;;-----
C1 93.46 0 L105A-S1 L105A-S 29.884 0.013 93.91
0
C10 93.94 0 L102D-S L102D-S1 18.031 0.013 93.64
0
C11 93.64 0 L102B-S2 L102D-S 10.552 0.013 93.97
0
C12 93.52 0 L102B-S2 L102B-S 21.381 0.013 93.97
0
C13 93.87 0 L102D-S1 L102B-S1 24.144 0.013 93.94
0

```


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C14		L101A1-S	L101A-S	75.133	0.013	93.99
93.5	0	0				
C15		L101A1-S	L102B-S	31.108	0.013	93.99
93.52	0	0				
C16		L101A-S1	MAJ-1	18.852	0.013	93.69
93.33	0	0				
C17		L105A-S	MAJ-2	9.672	0.013	93.46
93.24	0	0				
C3		L101A-S	L101A-S1	14.841	0.013	93.5
93.69	0	0				
C4		L102A-S1	L101A-S	18.287	0.013	93.77
93.5	0	0				
C5		L102A-S	L102A-S1	18.719	0.013	93.67
93.77	0	0				
C6		L102B-S1	L102A-S	20.072	0.013	93.87
93.67	0	0				
C7		L102C-S	L102C-S1	10.31	0.013	93.9
94.1	0	0				
C8		L102C-S1	L102B-S1	46.399	0.013	94.1
93.87	0	0				
C9		L102B-S	L102B-S1	21.768	0.013	93.52
93.87	0	0				
Pipe_3		STM-102	STM-101	82.253	0.013	91.46
91.131	0	0				
Pipe_4		STM-103	STM-101	41.48	0.013	91.371
91.206	0	0				
Pipe_46		STM-101	DICB221	25.2	0.013	90.8
90.72	0	0				
Pipe_5		STM-104	STM-103	68.185	0.013	91.704
91.431	0	0				
Pipe_6		STM-105	STM-103	26.256	0.013	91.532
91.401	0	0				

[ORIFICES]

;;	Inlet	Outlet	Orifice	Crest	Disch.
;;	Flap Open/Close				
;;	Name	Node	Type	Height	Coeff.
;;	Gate Time				
;;	-----				

L101A-IC	L101A-S	STM-101	SIDE	92.12	0.572
NO 0					
L102A-IC	L102A-S	STM-101	SIDE	92.29	0.572
NO 0					
L102B-IC	L102B-S	STM-102	SIDE	91.94	0.572
NO 0					
L102C-IC	L102C-S	STM-102	SIDE	92.52	0.572
NO 0					

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L102D-IC	L102D-S	STM-102	SIDE	92.26	0.572
NO 0					
L105A-IC	L105A-S	STM-105	SIDE	92.08	0.572
NO 0					

[XSECTIONS]

;;	Link	Shape	Geom1	Geom2	Geom3	Geom4
;;	Barrels					
;;	-----					

C1	IRREGULAR	Half-12m-MC	0	0	0	1
C10	IRREGULAR	Half-17m-MC	0	0	0	1
C11	IRREGULAR	Half-17m-MC	0	0	0	1
C12	IRREGULAR	Half-17m-MC	0	0	0	1
C13	IRREGULAR	Half-17m-MC	0	0	0	1
C14	IRREGULAR	Half-12m-MC	0	0	0	1
C15	IRREGULAR	Half-17m-MC	0	0	0	1
C16	IRREGULAR	Half-17m-MC	0	0	0	1
C17	IRREGULAR	Half-8.5mROW	0	0	0	1
C3	IRREGULAR	Half-17m-MC	0	0	0	1
C4	IRREGULAR	Half-17m-MC	0	0	0	1
C5	IRREGULAR	Half-17m-MC	0	0	0	1
C6	IRREGULAR	Half-17m-MC	0	0	0	1
C7	IRREGULAR	Half-17m-MC	0	0	0	1
C8	IRREGULAR	Half-17m-MC	0	0	0	1
C9	IRREGULAR	Half-17m-MC	0	0	0	1
Pipe_3	CIRCULAR	0.45	0	0	0	1
Pipe_4	CIRCULAR	0.3	0	0	0	1
Pipe_46	CIRCULAR	0.675	0	0	0	1

```

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Pipe_5      CIRCULAR    0.3      0      0      0      1
Pipe_6      CIRCULAR    0.3      0      0      0      1
L101A-IC   CIRCULAR    0.178    0      0      0
L102A-IC   CIRCULAR    0.108    0      0      0
L102B-IC   CIRCULAR    0.178    0      0      0
L102C-IC   CIRCULAR    0.083    0      0      0
L102D-IC   CIRCULAR    0.102    0      0      0
L105A-IC   CIRCULAR    0.083    0      0      0

```

[TRANSECTS]

;Full street, width = 8.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.245m.

```

NC 0.025 0.025 0.013
X1 18mROW 7 10 18.5 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.13 14.25 0
18.5
GR 0.15 18.5 0.35 28.5

```

;Full street, width = 11m, curb = 0.15m , cross-slope = 0.016m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

```

NC 0.025 0.025 0.013
X1 24mROW 7 10 21 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.165 15.5 0
21
GR 0.15 21 0.35 31

```

;Full street, width = 5.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

```

NC 0.025 0.025 0.013
X1 8.5m-ROW 7 10 15.5 0.0 0.0 0.0 0.0
0.0
GR 0.35 0 0.15 10 0 10 0.13 12.75 0
15.5
GR 0.15 15.5 0.35 25.5

```

```

NC 0.013 0.025 0.013
X1 Half-12m-MC 6 3 9 0.0 0.0 0.0 0.0
0.0
GR 0.17 0 0.11 3 0.06 3 0 9 0.05
9
GR 0.11 12

```

```

NC 0.013 0.025 0.013

```

```

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X1 Half-17m-MC 6 3 14 0.0 0.0 0.0 0.0
0.0
GR 0.36 0 0.27 3 0.22 3 0 14 0.05
14
GR 0.14 17
NC 0.013 0.025 0.013
X1 Half-18mROW 5 0.0 4.25 0.0 0.0 0.0 0.0
0.0
GR 0.13 0 0 4.25 0.15 4.25 0.35 10 0.4
12.5
NC 0.013 0.025 0.013
X1 Half-8.5mROW 5 0.0 2.75 0.0 0.0 0.0 0.0
0.0
GR 0.13 0 0 2.75 0.15 2.75 0.35 10 0.4
12.5

```

[LOSSES]

;;Link	Inlet	Outlet	Average	Flap Gate	SeepageRate
Pipe_4	0	1.32	0	NO	0
Pipe_46	0.02	1.32	0	NO	0
Pipe_5	0	1.32	0	NO	0
Pipe_6	0	0.02	0	NO	0

[CURVES]

;;Name	Type	X-Value	Y-Value
C202A-Q	Rating	0	0
C202A-Q		1.38	116.6
C202A-Q		1.73	128.3
C202A-Q		2.08	130.6
L201A-Q	Rating	0	0
L201A-Q		1.38	79.8
L201A-Q		1.73	87.8
L201A-Q		2.08	89.4
L201C-Q	Rating	0	0
L201C-Q		1.38	27.6
L201C-Q		1.73	30.4
L201C-Q		2.08	30.9
L202B-Q	Rating	0	0
L202B-Q		1.38	127
L202B-Q		1.73	139.7
L202B-Q		2.08	142.2

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L202C-Q	Rating	0	0
L202C-Q		1.38	20.3
L202C-Q		1.68	22.3
L203A-Q	Rating	0	0
L203A-Q		1.38	52.3
L203A-Q		1.73	57.5
L203A-Q		2.08	58.6
L204A-Q	Rating	0	0
L204A-Q		1.38	246.7
L204A-Q		1.73	271.4
L204A-Q		2.08	276.3
L205AA-Q	Rating	0	0
L205AA-Q		1.38	88.2
L205AA-Q		1.73	97
L205A-Q	Rating	0	0
L205A-Q		1.38	208.6
L205A-Q		1.73	229.5
L205A-Q		2.08	233.6
L205C-Q	Rating	0	0
L205C-Q		1.38	123.6
L205C-Q		1.73	136
L205C-Q		2.08	138.4
L215A-Q	Rating	0	0
L215A-Q		1.38	262.8
L215A-Q		1.73	289.1
L215A-Q		2.08	294.3
L216A-Q	Rating	0	0
L216A-Q		2.3	76
L216A-Q		2.6	83.6
L216A-Q		2.9	85.12
L218A-Q	Rating	0	0
L218A-Q		1.38	60.3
L218A-Q		1.73	66.3
L218A-Q		2.08	67.5
L221A-Q	Rating	0	0
L221A-Q		1.38	243.5
L221A-Q		1.73	267.9
L221A-Q		2.08	272.7

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L222A-Q	Rating	0	0
L222A-Q		1.38	245.1
L222A-Q		1.73	269.6
L222A-Q		2.08	274.5
102C-V	Storage	0	0
102C-V		1.38	0
102C-V		1.58	115
102C-V		1.581	0
102C-V		1.73	0
C202A-V	Storage	0	0
C202A-V		1.38	0
C202A-V		1.73	933
C202A-V		1.731	0
C202A-V		2.08	0
C300A-V	Storage	0	0
C300A-V		1.38	0
C300A-V		1.39	1
C300A-V		1.4	0
C300A-V		1.73	0
C302A-V	Storage	0	0
C302A-V		1.38	0
C302A-V		1.39	1
C302A-V		1.4	0
C302A-V		1.73	0
L101A-V	Storage	0	0
L101A-V		1.38	0
L101A-V		1.57	105
L101A-V		1.571	0
L101A-V		1.73	0
L102A-V	Storage	0	0
L102A-V		1.38	0
L102A-V		1.48	44
L102A-V		1.481	0
L102A-V		1.73	0
L102B-V	Storage	0	0
L102B-V		1.58	0
L102B-V		1.93	600
L102B-V		1.931	0
L102D-V	Storage	0	0

```

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L102D-V      1.38      0
L102D-V      1.68     138
L102D-V      1.681     0
L102D-V      1.73      0

L201A-V      Storage    0          0
L201A-V      1.38      0
L201A-V      1.73     88
L201A-V      1.731     0
L201A-V      2.08      0

L201C-V      Storage    0          0
L201C-V      1.38      0
L201C-V      1.73     44
L201C-V      1.731     0
L201C-V      2.08      0

L203A-V      Storage    0          0
L203A-V      1.38      0
L203A-V      1.73     78
L203A-V      1.731     0
L203A-V      2.08      0

L204AA-V     Storage    0          0
L204AA-V     1.38      0
L204AA-V     1.73    116
L204AA-V     1.731     0
L204AA-V     2.08      0

L204A-V     Storage    0          0
L204A-V     1.38      0
L204A-V     1.73   1700
L204A-V     1.731     0
L204A-V     2.08      0

L205AA-V     Storage    0          0
L205AA-V     1.38      0
L205AA-V     1.73    693

L205A-V     Storage    0          0
L205A-V     1.38      0
L205A-V     1.73   1130
L205A-V     1.731     0
L205A-V     2.08      0

L205C-V     Storage    0          0
L205C-V     1.38      0
L205C-V     1.73    180

```

```

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L205C-V      1.731     0
L205C-V      2.08      0

L215A-V      Storage    0          0
L215A-V      1.38      0
L215A-V      1.73    487
L215A-V      1.731     0
L215A-V      2.08      0

L216A-V      Storage    0          0
L216A-V      2.3        0
L216A-V      2.6   22000
L216A-V      2.601     0
L216A-V      2.9        0

L221A-V      Storage    0          0
L221A-V      1.38      0
L221A-V      1.73    452
L221A-V      1.731     0
L221A-V      2.08      0

L222A-V      Storage    0          0
L222A-V      1.38      0
L222A-V      1.73    458
L222A-V      1.731     0
L222A-V      2.08      0

L400A-V     Storage    0          0
L400A-V     1.38      0
L400A-V     1.39      1
L400A-V     1.4        0
L400A-V     1.73      0

L402A-V     Storage    0          0
L402A-V     1.38      0
L402A-V     1.39   4700
L402A-V     1.4        0
L402A-V     1.73      0

[TIMESERIES]
; ;Name      Date      Time      Value
;-----
;100yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)
C100yr-4hr      0:00      0.00
C100yr-4hr      0:10      4.39
C100yr-4hr      0:20      5.07
C100yr-4hr      0:30      6.05
C100yr-4hr      0:40      7.54

```

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C100yr-4hr	0:50	10.16
C100yr-4hr	1:00	15.97
C100yr-4hr	1:10	40.65
C100yr-4hr	1:20	178.56
C100yr-4hr	1:30	54.05
C100yr-4hr	1:40	27.32
C100yr-4hr	1:50	18.24
C100yr-4hr	2:00	13.74
C100yr-4hr	2:10	11.06
C100yr-4hr	2:20	9.29
C100yr-4hr	2:30	8.02
C100yr-4hr	2:40	7.08
C100yr-4hr	2:50	6.35
C100yr-4hr	3:00	5.76
C100yr-4hr	3:10	5.28
C100yr-4hr	3:20	4.88
C100yr-4hr	3:30	4.54
C100yr-4hr	3:40	4.25
C100yr-4hr	3:50	3.99
C100yr-4hr	4:00	3.77

;100yr +20% - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C100yr-4hr+20%	0:00	0.00
C100yr-4hr+20%	0:10	5.27
C100yr-4hr+20%	0:20	6.08
C100yr-4hr+20%	0:30	7.26
C100yr-4hr+20%	0:40	9.05
C100yr-4hr+20%	0:50	12.19
C100yr-4hr+20%	1:00	19.16
C100yr-4hr+20%	1:10	48.78
C100yr-4hr+20%	1:20	214.27
C100yr-4hr+20%	1:30	64.86
C100yr-4hr+20%	1:40	32.78
C100yr-4hr+20%	1:50	21.89
C100yr-4hr+20%	2:00	16.49
C100yr-4hr+20%	2:10	13.27
C100yr-4hr+20%	2:20	11.15
C100yr-4hr+20%	2:30	9.62
C100yr-4hr+20%	2:40	8.50
C100yr-4hr+20%	2:50	7.62
C100yr-4hr+20%	3:00	6.91
C100yr-4hr+20%	3:10	6.34
C100yr-4hr+20%	3:20	5.86
C100yr-4hr+20%	3:30	5.45
C100yr-4hr+20%	3:40	5.10
C100yr-4hr+20%	3:50	4.79
C100yr-4hr+20%	4:00	4.52

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;25mm - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C25mm-4hr	0:00	0.00
C25mm-4hr	0:10	1.51
C25mm-4hr	0:20	1.75
C25mm-4hr	0:30	2.07
C25mm-4hr	0:40	2.58
C25mm-4hr	0:50	3.46
C25mm-4hr	1:00	5.39
C25mm-4hr	1:10	13.44
C25mm-4hr	1:20	56.67
C25mm-4hr	1:30	17.77
C25mm-4hr	1:40	9.12
C25mm-4hr	1:50	6.14
C25mm-4hr	2:00	4.65
C25mm-4hr	2:10	3.76
C25mm-4hr	2:20	3.17
C25mm-4hr	2:30	2.74
C25mm-4hr	2:40	2.43
C25mm-4hr	2:50	2.18
C25mm-4hr	3:00	1.98
C25mm-4hr	3:10	1.81
C25mm-4hr	3:20	1.68
C25mm-4hr	3:30	1.56
C25mm-4hr	3:40	1.47
C25mm-4hr	3:50	1.38
C25mm-4hr	4:00	1.31

;2yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C2yr-4hr	0:00	0.00
C2yr-4hr	0:10	2.05
C2yr-4hr	0:20	2.37
C2yr-4hr	0:30	2.81
C2yr-4hr	0:40	3.50
C2yr-4hr	0:50	4.69
C2yr-4hr	1:00	7.30
C2yr-4hr	1:10	18.21
C2yr-4hr	1:20	76.81
C2yr-4hr	1:30	24.08
C2yr-4hr	1:40	12.36
C2yr-4hr	1:50	8.32
C2yr-4hr	2:00	6.30
C2yr-4hr	2:10	5.09
C2yr-4hr	2:20	4.29
C2yr-4hr	2:30	3.72
C2yr-4hr	2:40	3.29
C2yr-4hr	2:50	2.95
C2yr-4hr	3:00	2.68
C2yr-4hr	3:10	2.46

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C2yr-4hr	3:20	2.28
C2yr-4hr	3:30	2.12
C2yr-4hr	3:40	1.99
C2yr-4hr	3:50	1.87
C2yr-4hr	4:00	1.77

;5yr - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)

C5yr-4hr	0:00	0.00
C5yr-4hr	0:10	2.68
C5yr-4hr	0:20	3.10
C5yr-4hr	0:30	3.68
C5yr-4hr	0:40	4.58
C5yr-4hr	0:50	6.15
C5yr-4hr	1:00	9.61
C5yr-4hr	1:10	24.17
C5yr-4hr	1:20	104.19
C5yr-4hr	1:30	32.04
C5yr-4hr	1:40	16.34
C5yr-4hr	1:50	10.96
C5yr-4hr	2:00	8.29
C5yr-4hr	2:10	6.69
C5yr-4hr	2:20	5.63
C5yr-4hr	2:30	4.87
C5yr-4hr	2:40	4.30
C5yr-4hr	2:50	3.86
C5yr-4hr	3:00	3.51
C5yr-4hr	3:10	3.22
C5yr-4hr	3:20	2.98
C5yr-4hr	3:30	2.77
C5yr-4hr	3:40	2.60
C5yr-4hr	3:50	2.44
C5yr-4hr	4:00	2.31

Chicago_2yr_3hr_10m_ottawa	0:10	2.81459
Chicago_2yr_3hr_10m_ottawa	0:20	3.49824
Chicago_2yr_3hr_10m_ottawa	0:30	4.68718
Chicago_2yr_3hr_10m_ottawa	0:40	7.30485
Chicago_2yr_3hr_10m_ottawa	0:50	18.20881
Chicago_2yr_3hr_10m_ottawa	1:00	76.805
Chicago_2yr_3hr_10m_ottawa	1:10	24.07906
Chicago_2yr_3hr_10m_ottawa	1:20	12.36376
Chicago_2yr_3hr_10m_ottawa	1:30	8.32403
Chicago_2yr_3hr_10m_ottawa	1:40	6.30341
Chicago_2yr_3hr_10m_ottawa	1:50	5.09498
Chicago_2yr_3hr_10m_ottawa	2:00	4.29133
Chicago_2yr_3hr_10m_ottawa	2:10	3.71786
Chicago_2yr_3hr_10m_ottawa	2:20	3.28762
Chicago_2yr_3hr_10m_ottawa	2:30	2.95254

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Chicago_2yr_3hr_10m_ottawa	2:40	2.68388
Chicago_2yr_3hr_10m_ottawa	2:50	2.46348
Chicago_2yr_3hr_10m_ottawa	3:00	2.27921

Chicago_5y_3h_10m_City	0:00	3.68223
Chicago_5y_3h_10m_City	0:10	4.58232
Chicago_5y_3h_10m_City	0:20	6.15055
Chicago_5y_3h_10m_City	0:30	9.6141
Chicago_5y_3h_10m_City	0:40	24.17035
Chicago_5y_3h_10m_City	0:50	104.193
Chicago_5y_3h_10m_City	1:00	32.03692
Chicago_5y_3h_10m_City	1:10	16.3375
Chicago_5y_3h_10m_City	1:20	10.96479
Chicago_5y_3h_10m_City	1:30	8.28693
Chicago_5y_3h_10m_City	1:40	6.68897
Chicago_5y_3h_10m_City	1:50	5.6279
Chicago_5y_3h_10m_City	2:00	4.87167
Chicago_5y_3h_10m_City	2:10	4.30483
Chicago_5y_3h_10m_City	2:20	3.8637
Chicago_5y_3h_10m_City	2:30	3.51028
Chicago_5y_3h_10m_City	2:40	3.22046
Chicago_5y_3h_10m_City	2:50	2.97831

Chicago100y_3h_10m_City	00:00	6.04573
Chicago100y_3h_10m_City	00:10	7.54219
Chicago100y_3h_10m_City	00:20	10.15880
Chicago100y_3h_10m_City	00:30	15.96889
Chicago100y_3h_10m_City	00:40	40.65497
Chicago100y_3h_10m_City	00:50	178.55900
Chicago100y_3h_10m_City	01:00	54.04853
Chicago100y_3h_10m_City	01:10	27.31870
Chicago100y_3h_10m_City	01:20	18.24039
Chicago100y_3h_10m_City	01:30	13.73692
Chicago100y_3h_10m_City	01:40	11.05876
Chicago100y_3h_10m_City	01:50	9.28521
Chicago100y_3h_10m_City	02:00	8.02389
Chicago100y_3h_10m_City	02:10	7.08022
Chicago100y_3h_10m_City	02:20	6.34698
Chicago100y_3h_10m_City	02:30	5.76029
Chicago100y_3h_10m_City	02:40	5.27978
Chicago100y_3h_10m_City	02:50	4.87871

Chicago100y+20%_3h_10m_City	00:00	7.255
Chicago100y+20%_3h_10m_City	00:10	9.051
Chicago100y+20%_3h_10m_City	00:20	12.191
Chicago100y+20%_3h_10m_City	00:30	19.163
Chicago100y+20%_3h_10m_City	00:40	48.786
Chicago100y+20%_3h_10m_City	00:50	214.271

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Chicago100y+20%_3h_10m_City	01:00	64.858
Chicago100y+20%_3h_10m_City	01:10	32.782
Chicago100y+20%_3h_10m_City	01:20	21.888
Chicago100y+20%_3h_10m_City	01:30	16.484
Chicago100y+20%_3h_10m_City	01:40	13.271
Chicago100y+20%_3h_10m_City	01:50	11.142
Chicago100y+20%_3h_10m_City	02:00	9.629
Chicago100y+20%_3h_10m_City	02:10	8.496
Chicago100y+20%_3h_10m_City	02:20	7.616
Chicago100y+20%_3h_10m_City	02:30	6.912
Chicago100y+20%_3h_10m_City	02:40	6.336
Chicago100y+20%_3h_10m_City	02:50	5.854

;100yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S100yr-12hr	0:00	0.00
S100yr-12hr	0:30	2.82
S100yr-12hr	1:00	1.31
S100yr-12hr	1:30	2.44
S100yr-12hr	2:00	2.44
S100yr-12hr	2:30	3.19
S100yr-12hr	3:00	2.82
S100yr-12hr	3:30	3.76
S100yr-12hr	4:00	3.76
S100yr-12hr	4:30	5.07
S100yr-12hr	5:00	6.39
S100yr-12hr	5:30	10.14
S100yr-12hr	6:00	80.38
S100yr-12hr	6:30	20.47
S100yr-12hr	7:00	9.02
S100yr-12hr	7:30	6.01
S100yr-12hr	8:00	5.26
S100yr-12hr	8:30	4.13
S100yr-12hr	9:00	4.32
S100yr-12hr	9:30	2.82
S100yr-12hr	10:00	2.25
S100yr-12hr	10:30	3.19
S100yr-12hr	11:00	2.07
S100yr-12hr	11:30	1.88
S100yr-12hr	12:00	1.88

;2yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S2yr-12hr	0:00	0.00
S2yr-12hr	0:30	1.27
S2yr-12hr	1:00	0.59
S2yr-12hr	1:30	1.10
S2yr-12hr	2:00	1.10
S2yr-12hr	2:30	1.44
S2yr-12hr	3:00	1.27

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S2yr-12hr	3:30	1.69
S2yr-12hr	4:00	1.69
S2yr-12hr	4:30	2.29
S2yr-12hr	5:00	2.88
S2yr-12hr	5:30	4.57
S2yr-12hr	6:00	36.24
S2yr-12hr	6:30	9.23
S2yr-12hr	7:00	4.06
S2yr-12hr	7:30	2.71
S2yr-12hr	8:00	2.37
S2yr-12hr	8:30	1.86
S2yr-12hr	9:00	1.95
S2yr-12hr	9:30	1.27
S2yr-12hr	10:00	1.02
S2yr-12hr	10:30	1.44
S2yr-12hr	11:00	0.93
S2yr-12hr	11:30	0.85
S2yr-12hr	12:00	0.85

;5yr - 12hr SCS Design Storm - City of Ottawa IDF Data (1967-1997)

S5yr-12hr	0:00	0.00
S5yr-12hr	0:30	1.69
S5yr-12hr	1:00	0.79
S5yr-12hr	1:30	1.46
S5yr-12hr	2:00	1.46
S5yr-12hr	2:30	1.91
S5yr-12hr	3:00	1.69
S5yr-12hr	3:30	2.25
S5yr-12hr	4:00	2.25
S5yr-12hr	4:30	3.03
S5yr-12hr	5:00	3.82
S5yr-12hr	5:30	6.07
S5yr-12hr	6:00	48.08
S5yr-12hr	6:30	12.25
S5yr-12hr	7:00	5.39
S5yr-12hr	7:30	3.60
S5yr-12hr	8:00	3.15
S5yr-12hr	8:30	2.47
S5yr-12hr	9:00	2.58
S5yr-12hr	9:30	1.69
S5yr-12hr	10:00	1.35
S5yr-12hr	10:30	1.91
S5yr-12hr	11:00	1.24
S5yr-12hr	11:30	1.12
S5yr-12hr	12:00	1.12

[REPORT]
INPUT YES

CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]

Node	DICB221	MH-C
Node	L101A-S	CB
Node	L102A-S	CB
Node	L102B-S	CB
Node	L102C-S	CB
Node	L102D-S	CB
Node	L105A-S	CB
Node	STM-101	MH-C
Node	STM-102	MH-C
Node	STM-103	MH-C
Node	STM-104	MH-C
Node	STM-105	MH-C
Link	C1	MAJ
Link	C10	MAJ
Link	C11	MAJ
Link	C12	MAJ
Link	C13	MAJ
Link	C14	MAJ
Link	C15	MAJ
Link	C16	MAJ
Link	C17	MAJ
Link	C3	MAJ
Link	C4	MAJ
Link	C5	MAJ
Link	C6	MAJ
Link	C7	MAJ
Link	C8	MAJ
Link	C9	MAJ
Link	Pipe_3	STM
Link	Pipe_4	STM
Link	Pipe_46	STM
Link	Pipe_5	STM
Link	Pipe_6	STM
Link	L101A-IC	RoadCB
Link	L102A-IC	RoadCB
Link	L102B-IC	RoadCB
Link	L102C-IC	RoadCB
Link	L102D-IC	RoadCB
Link	L105A-IC	RoadCB

[MAP]
DIMENSIONS 363780.9547 5013769.74325 363974.0113 5013954.18575

UNITS Meters

[COORDINATES]

;;Node	X-Coord	Y-Coord
;;-----		
DICB221	363809.615	5013844.287
MAJ-1	363801.696	5013833.174
MAJ-2	363872.589	5013785.283
OF1	363815.06	5013801.877
OF2	363927.001	5013812.438
OF3	363935.684	5013936.112
OF4	363951.599	5013825.948
L101A1-S	363885.226	5013841.386
L101A-S	363830.675	5013850.347
L101A-S1	363817.757	5013843.042
L102A-S	363859.441	5013857.072
L102A-S1	363850.337	5013854.166
L102B-S	363890.759	5013864.523
L102B-S1	363873.778	5013871.118
L102B-S2	363911.787	5013860.656
L102C-S	363915.2	5013903
L102C-S1	363904.9	5013903.432
L102D-S	363910.176	5013871.083
L102D-S1	363900.795	5013886.481
L105A-S	363868.4	5013794
L105A-S1	363848.35	5013816.157
STM-101	363833.3	5013856
STM-102	363904.6	5013897
STM-103	363853.9	5013820
STM-104	363913	5013854
STM-105	363866.9	5013798

[VERTICES]

;;Link	X-Coord	Y-Coord
;;-----		
C14	363850.866	5013819.484
C14	363846.381	5013820.418
C15	363897.883	5013851.268
C8	363889.802	5013891.91
C9	363886.162	5013871.512
C9	363880.374	5013873.874

[POLYGONS]

;;Subcatchment	X-Coord	Y-Coord
;;-----		
L101A	363824.67	5013885.528
L101A	363827.444	5013886.934

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L101A	363827.444	5013886.934
L101A	363835.034	5013873.662
L101A	363835.034	5013873.662
L101A	363843.407	5013862.561
L101A	363843.407	5013862.561
L101A	363848.277	5013858.427
L101A	363848.277	5013858.427
L101A	363855.956	5013845.052
L101A	363855.956	5013845.052
L101A	363876.545	5013856.872
L101A	363876.545	5013856.872
L101A	363884.203	5013843.532
L101A	363884.203	5013843.532
L101A	363891.025	5013831.652
L101A	363891.025	5013831.652
L101A	363889.68	5013830.88
L101A	363889.68	5013830.88
L101A	363898.703	5013815.164
L101A	363898.703	5013815.164
L101A	363895.839	5013813.52
L101A	363895.839	5013813.52
L101A	363895.586	5013813.961
L101A	363895.586	5013813.961
L101A	363892.811	5013812.367
L101A	363892.811	5013812.367
L101A	363893.007	5013812.026
L101A	363893.007	5013812.026
L101A	363890.308	5013810.476
L101A	363890.308	5013810.476
L101A	363890.004	5013811.005
L101A	363890.004	5013811.005
L101A	363884.299	5013807.73
L101A	363884.299	5013807.73
L101A	363884.602	5013807.201
L101A	363884.602	5013807.201
L101A	363881.904	5013805.652
L101A	363881.904	5013805.652
L101A	363881.297	5013806.709
L101A	363881.297	5013806.709
L101A	363878.795	5013805.273
L101A	363878.795	5013805.273
L101A	363869.724	5013821.065
L101A	363869.724	5013821.065
L101A	363866.6	5013826.504
L101A	363866.6	5013826.504
L101A	363853.592	5013819.03
L101A	363853.592	5013819.03
L101A	363843.731	5013813.398

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L101A	363843.731	5013813.398
L101A	363842.663	5013815.246
L101A	363842.663	5013815.246
L101A	363839.177	5013813.243
L101A	363839.177	5013813.243
L101A	363830.126	5013808.042
L101A	363830.126	5013808.042
L101A	363828.481	5013810.905
L101A	363828.481	5013810.905
L101A	363828.921	5013811.158
L101A	363828.921	5013811.158
L101A	363827.327	5013813.933
L101A	363827.327	5013813.933
L101A	363825.347	5013816.589
L101A	363825.347	5013816.589
L101A	363822.246	5013821.985
L101A	363822.246	5013821.985
L101A	363822.775	5013822.288
L101A	363822.775	5013822.288
L101A	363819.497	5013827.992
L101A	363819.497	5013827.992
L101A	363818.969	5013827.689
L101A	363818.969	5013827.689
L101A	363817.419	5013830.386
L101A	363817.419	5013830.386
L101A	363817.759	5013830.585
L101A	363817.759	5013830.585
L101A	363816.164	5013833.36
L101A	363816.164	5013833.36
L101A	363815.724	5013833.107
L101A	363815.724	5013833.107
L101A	363814.079	5013835.97
L101A	363814.079	5013835.97
L101A	363819.989	5013839.365
L101A	363819.989	5013839.365
L101A	363815.759	5013846.754
L101A	363815.759	5013846.754
L101A	363811.72	5013852.739
L101A	363811.72	5013852.739
L101A	363805.735	5013856.908
L101A	363805.735	5013856.908
L101A	363798.145	5013870.18
L101A	363798.145	5013870.18
L101A	363800.151	5013871.327
L101A	363800.151	5013871.327
L101A	363800	5013871.592
L101A	363800	5013871.592
L101A	363801.764	5013872.6

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L101A	363801.764	5013872.6
L101A	363800.554	5013874.717
L101A	363800.554	5013874.717
L101A	363805.051	5013877.289
L101A	363805.051	5013877.289
L101A	363806.261	5013875.172
L101A	363806.261	5013875.172
L101A	363808.036	5013876.187
L101A	363808.036	5013876.187
L101A	363813.14	5013879.106
L101A	363813.14	5013879.106
L101A	363811.93	5013881.222
L101A	363811.93	5013881.222
L101A	363816.427	5013883.794
L101A	363816.427	5013883.794
L101A	363817.637	5013881.678
L101A	363817.637	5013881.678
L101A	363819.401	5013882.686
L101A	363819.401	5013882.686
L101A	363819.552	5013882.422
L101A	363819.552	5013882.422
L101A	363821.559	5013883.569
L101A	363821.559	5013883.569
L101A	363823.107	5013884.635
L101A	363823.107	5013884.635
L101A	363824.67	5013885.528
L102A	363851.651	5013898.935
L102A	363855.954	5013901.396
L102A	363855.954	5013901.396
L102A	363862.748	5013889.51
L102A	363862.748	5013889.51
L102A	363867.398	5013882.261
L102A	363867.398	5013882.261
L102A	363872.957	5013872.538
L102A	363872.957	5013872.538
L102A	363875.408	5013868.257
L102A	363875.408	5013868.257
L102A	363880.606	5013859.203
L102A	363880.606	5013859.203
L102A	363876.545	5013856.872
L102A	363876.545	5013856.872
L102A	363855.956	5013845.052
L102A	363855.956	5013845.052
L102A	363848.277	5013858.427
L102A	363848.277	5013858.427
L102A	363843.407	5013862.561
L102A	363843.407	5013862.561
L102A	363835.034	5013873.662

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L102A	363835.034	5013873.662
L102A	363827.444	5013886.934
L102A	363827.444	5013886.934
L102A	363829.45	5013888.082
L102A	363829.45	5013888.082
L102A	363829.299	5013888.346
L102A	363829.299	5013888.346
L102A	363831.063	5013889.355
L102A	363831.063	5013889.355
L102A	363829.852	5013891.471
L102A	363829.852	5013891.471
L102A	363834.35	5013894.043
L102A	363834.35	5013894.043
L102A	363835.56	5013891.927
L102A	363835.56	5013891.927
L102A	363842.439	5013895.86
L102A	363842.439	5013895.86
L102A	363841.229	5013897.976
L102A	363841.229	5013897.976
L102A	363845.726	5013900.548
L102A	363845.726	5013900.548
L102A	363846.936	5013898.432
L102A	363846.936	5013898.432
L102A	363848.7	5013899.44
L102A	363848.7	5013899.44
L102A	363848.851	5013899.176
L102A	363848.851	5013899.176
L102A	363850.858	5013900.323
L102A	363850.858	5013900.323
L102A	363851.651	5013898.935
L102B	363885.061	5013918.04
L102B	363889.722	5013920.706
L102B	363889.722	5013920.706
L102B	363896.518	5013908.822
L102B	363896.518	5013908.822
L102B	363898.524	5013909.969
L102B	363898.524	5013909.969
L102B	363898.676	5013909.704
L102B	363898.676	5013909.704
L102B	363900.439	5013910.713
L102B	363900.439	5013910.713
L102B	363901.65	5013908.596
L102B	363901.65	5013908.596
L102B	363903.565	5013905.243
L102B	363903.565	5013905.243
L102B	363904.723	5013903.601
L102B	363904.723	5013903.601
L102B	363910.282	5013893.878

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L102B	363910.282	5013893.878
L102B	363906.9	5013891.83
L102B	363906.9	5013891.83
L102B	363897.182	5013883.324
L102B	363897.182	5013883.324
L102B	363894.808	5013881.989
L102B	363894.808	5013881.989
L102B	363904.068	5013865.403
L102B	363904.068	5013865.403
L102B	363909.654	5013861.144
L102B	363909.654	5013861.144
L102B	363921.1	5013858.279
L102B	363921.1	5013858.279
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L102B	363938.576	5013867.739
L102B	363938.576	5013867.739
L102B	363945.251	5013871.467
L102B	363945.251	5013871.467
L102B	363946.378	5013869.449
L102B	363946.378	5013869.449
L102B	363946.654	5013869.585
L102B	363946.654	5013869.585
L102B	363947.634	5013867.824
L102B	363947.634	5013867.824
L102B	363949.763	5013869.012
L102B	363949.763	5013869.012
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L102B	363952.289	5013864.489
L102B	363950.165	5013863.303
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L102B	363956.153	5013857.571
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L102B	363958.679	5013853.048
L102B	363958.679	5013853.048
L102B	363956.55	5013851.859
L102B	363956.55	5013851.859
L102B	363957.541	5013850.085
L102B	363957.541	5013850.085
L102B	363957.275	5013849.936
L102B	363957.275	5013849.936
L102B	363958.402	5013847.919
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L102B	363945.053	5013840.464
L102B	363945.053	5013840.464
L102B	363943.929	5013842.483

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L102B	363943.929	5013842.483
L102B	363943.66	5013842.333
L102B	363943.66	5013842.333
L102B	363942.67	5013844.107
L102B	363942.67	5013844.107
L102B	363940.541	5013842.918
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L102B	363938.164	5013842.488
L102B	363938.164	5013842.488
L102B	363936.591	5013841.609
L102B	363936.591	5013841.609
L102B	363932.242	5013838.731
L102B	363932.242	5013838.731
L102B	363922.288	5013833.174
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L102B	363924.202	5013829.841
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L102B	363921.075	5013828.045
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L102B	363920.423	5013829.179
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L102B	363917.472	5013827.483
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L102B	363918.079	5013826.426
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L102B	363915.381	5013824.876
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L102B	363909.373	5013822.128
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L102B	363909.677	5013821.599
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L102B	363906.978	5013820.049
L102B	363906.782	5013820.39
L102B	363906.782	5013820.39
L102B	363904.007	5013818.796
L102B	363904.007	5013818.796
L102B	363904.26	5013818.356
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L102B	363901.397	5013816.711
L102B	363901.397	5013816.711
L102B	363892.37	5013832.424
L102B	363892.37	5013832.424
L102B	363891.025	5013831.652
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L102B	363884.203	5013843.532

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L102B	363884.203	5013843.532
L102B	363876.545	5013856.872
L102B	363876.545	5013856.872
L102B	363880.606	5013859.203
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L102B	363875.408	5013868.257
L102B	363875.408	5013868.257
L102B	363872.957	5013872.538
L102B	363872.957	5013872.538
L102B	363867.398	5013882.261
L102B	363867.398	5013882.261
L102B	363862.748	5013889.51
L102B	363862.748	5013889.51
L102B	363855.954	5013901.396
L102B	363855.954	5013901.396
L102B	363855.161	5013902.783
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L102B	363857.167	5013903.93
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L102B	363858.78	5013905.203
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L102B	363857.57	5013907.32
L102B	363857.57	5013907.32
L102B	363862.068	5013909.891
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L102B	363863.278	5013907.774
L102B	363863.278	5013907.774
L102B	363868.036	5013910.495
L102B	363868.036	5013910.495
L102B	363870.157	5013911.707
L102B	363870.157	5013911.707
L102B	363868.947	5013913.823
L102B	363868.947	5013913.823
L102B	363873.445	5013916.394
L102B	363873.445	5013916.394
L102B	363874.655	5013914.278
L102B	363874.655	5013914.278
L102B	363876.419	5013915.286
L102B	363876.419	5013915.286
L102B	363876.57	5013915.021
L102B	363876.57	5013915.021
L102B	363878.576	5013916.168
L102B	363878.576	5013916.168
L102B	363879.367	5013914.785
L102B	363879.367	5013914.785
L102B	363885.061	5013918.04

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L102C	363912.342	5013935.483
L102C	363919.932	5013922.21
L102C	363919.932	5013922.21
L102C	363917.925	5013921.063
L102C	363917.925	5013921.063
L102C	363918.077	5013920.799
L102C	363918.077	5013920.799
L102C	363917.371	5013920.395
L102C	363917.371	5013920.395
L102C	363920.497	5013914.924
L102C	363920.497	5013914.924
L102C	363921.391	5013913.361
L102C	363921.391	5013913.361
L102C	363924.785	5013909.312
L102C	363924.785	5013909.312
L102C	363928.855	5013902.193
L102C	363928.855	5013902.193
L102C	363929.539	5013899.609
L102C	363929.539	5013899.609
L102C	363916.19	5013892.157
L102C	363916.19	5013892.157
L102C	363910.282	5013893.878
L102C	363910.282	5013893.878
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L102C	363904.723	5013903.601
L102C	363903.565	5013905.243
L102C	363903.565	5013905.243
L102C	363901.65	5013908.596
L102C	363901.65	5013908.596
L102C	363900.439	5013910.713
L102C	363900.439	5013910.713
L102C	363898.676	5013909.704
L102C	363898.676	5013909.704
L102C	363898.524	5013909.969
L102C	363898.524	5013909.969
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L102C	363896.518	5013908.822
L102C	363889.722	5013920.706
L102C	363889.722	5013920.706
L102C	363888.929	5013922.094
L102C	363888.929	5013922.094
L102C	363890.935	5013923.241
L102C	363890.935	5013923.241
L102C	363890.783	5013923.506
L102C	363890.783	5013923.506
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L102C	363907.211	5013935.708
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L102D	363931.902	5013864.012
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L102D	363894.808	5013881.989
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L102D	363897.182	5013883.324
L102D	363906.9	5013891.83
L102D	363906.9	5013891.83
L102D	363910.282	5013893.878
L102D	363910.282	5013893.878
L102D	363916.19	5013892.157
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L102D	363929.539	5013899.609
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L102D	363930.932	5013897.74
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L102D	363931.922	5013895.966
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L102D	363936.577	5013892.63

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L102D	363936.577	5013892.63
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L102D	363938.31	5013884.523
L102D	363940.44	5013885.712
L102D	363940.44	5013885.712
L102D	363942.965	5013881.188
L102D	363942.965	5013881.188
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L102D	363940.836	5013879.999
L102D	363941.826	5013878.225
L102D	363941.826	5013878.225
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L105A	363844.803	5013782.55
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L105A	363842.444	5013789.28
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L105A	363840.366	5013791.674
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L105A	363837.088	5013797.378
L105A	363836.56	5013797.075
L105A	363836.56	5013797.075
L105A	363835.009	5013799.773
L105A	363835.009	5013799.773
L105A	363835.35	5013799.969
L105A	363835.35	5013799.969
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L105A	363833.315	5013802.491
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L105A	363831.67	5013805.354
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L105A	363844.208	5013812.558
L105A	363843.731	5013813.398
L105A	363843.731	5013813.398
L105A	363853.592	5013819.03
L105A	363853.592	5013819.03
L105A	363866.6	5013826.504
L105A	363866.6	5013826.504
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L105A	363869.724	5013821.065
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UNC-1	363817.419	5013830.386
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UNC-1	363818.969	5013827.689
UNC-1	363819.497	5013827.992
UNC-1	363819.497	5013827.992
UNC-1	363822.775	5013822.288
UNC-1	363822.775	5013822.288
UNC-1	363822.246	5013821.985
UNC-1	363822.246	5013821.985
UNC-1	363825.347	5013816.589
UNC-1	363825.347	5013816.589
UNC-1	363827.327	5013813.933
UNC-1	363827.327	5013813.933
UNC-1	363828.921	5013811.158
UNC-1	363828.921	5013811.158
UNC-1	363828.481	5013810.905
UNC-1	363828.481	5013810.905
UNC-1	363830.126	5013808.042

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UNC-1	363830.126	5013808.042
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UNC-1	363839.177	5013813.243
UNC-1	363842.663	5013815.246
UNC-1	363842.663	5013815.246
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UNC-1	363843.731	5013813.398
UNC-1	363844.208	5013812.558
UNC-1	363844.208	5013812.558
UNC-1	363831.67	5013805.354
UNC-1	363831.67	5013805.354
UNC-1	363833.315	5013802.491
UNC-1	363833.315	5013802.491
UNC-1	363833.756	5013802.744
UNC-1	363833.756	5013802.744
UNC-1	363835.35	5013799.969
UNC-1	363835.35	5013799.969
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UNC-1	363835.009	5013799.773
UNC-1	363836.56	5013797.075
UNC-1	363836.56	5013797.075
UNC-1	363837.088	5013797.378
UNC-1	363837.088	5013797.378
UNC-1	363840.366	5013791.674
UNC-1	363840.366	5013791.674
UNC-1	363839.837	5013791.371
UNC-1	363839.837	5013791.371
UNC-1	363841.387	5013788.673
UNC-1	363841.387	5013788.673
UNC-1	363842.444	5013789.28
UNC-1	363842.444	5013789.28
UNC-1	363844.14	5013786.329
UNC-1	363844.14	5013786.329
UNC-1	363843.006	5013785.677
UNC-1	363843.006	5013785.677
UNC-1	363844.803	5013782.55
UNC-1	363844.803	5013782.55
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UNC-1	363862.862	5013792.927
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UNC-1	363868.693	5013792.758
UNC-1	363843.215	5013778.127
UNC-1	363843.215	5013778.127
UNC-1	363839.241	5013779.3
UNC-1	363839.241	5013779.3
UNC-1	363789.73	5013875.495
UNC-1	363789.73	5013875.495
UNC-1	363863.823	5013917.865

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UNC-1	363863.823	5013917.865
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UNC-1	363863.278	5013907.774
UNC-1	363863.278	5013907.774
UNC-1	363862.068	5013909.891
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UNC-1	363857.57	5013907.32
UNC-1	363857.57	5013907.32
UNC-1	363858.78	5013905.203
UNC-1	363858.78	5013905.203
UNC-1	363857.016	5013904.195
UNC-1	363857.016	5013904.195
UNC-1	363857.167	5013903.93
UNC-1	363857.167	5013903.93
UNC-1	363855.161	5013902.783
UNC-1	363855.161	5013902.783
UNC-1	363855.954	5013901.396
UNC-1	363855.954	5013901.396
UNC-1	363851.651	5013898.935
UNC-1	363851.651	5013898.935
UNC-1	363850.858	5013900.323
UNC-1	363850.858	5013900.323
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UNC-1	363848.851	5013899.176
UNC-1	363848.7	5013899.44
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UNC-1	363846.936	5013898.432
UNC-1	363846.936	5013898.432
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UNC-1	363845.726	5013900.548
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UNC-1	363842.439	5013895.86
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UNC-1	363835.56	5013891.927
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UNC-1	363831.063	5013889.355
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UNC-1	363829.299	5013888.346
UNC-1	363829.45	5013888.082
UNC-1	363829.45	5013888.082
UNC-1	363827.444	5013886.934

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UNC-1	363824.67	5013885.528
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UNC-1	363823.107	5013884.635
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UNC-1	363821.559	5013883.569
UNC-1	363819.552	5013882.422
UNC-1	363819.552	5013882.422
UNC-1	363819.401	5013882.686
UNC-1	363819.401	5013882.686
UNC-1	363817.637	5013881.678
UNC-1	363817.637	5013881.678
UNC-1	363816.427	5013883.794
UNC-1	363816.427	5013883.794
UNC-1	363811.93	5013881.222
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UNC-1	363813.14	5013879.106
UNC-1	363813.14	5013879.106
UNC-1	363808.036	5013876.187
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UNC-1	363806.261	5013875.172
UNC-1	363805.051	5013877.289
UNC-1	363805.051	5013877.289
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UNC-1	363800.554	5013874.717
UNC-1	363801.764	5013872.6
UNC-1	363801.764	5013872.6
UNC-1	363800	5013871.592
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UNC-1	363800.151	5013871.327
UNC-1	363800.151	5013871.327
UNC-1	363798.145	5013870.18
UNC-1	363798.145	5013870.18
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UNC-1	363805.735	5013856.908
UNC-1	363811.72	5013852.739
UNC-1	363811.72	5013852.739
UNC-1	363815.759	5013846.754
UNC-1	363815.759	5013846.754
UNC-1	363819.989	5013839.365
UNC-1	363819.989	5013839.365
UNC-1	363814.079	5013835.97
UNC-1	363814.079	5013835.97
UNC-1	363815.724	5013833.107
UNC-2	363917.402	5013826.037
UNC-2	363919.785	5013822.096

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UNC-2	363874.339	5013795.992
UNC-2	363875.868	5013802.085
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UNC-2	363878.345	5013805.015
UNC-2	363878.795	5013805.273
UNC-2	363881.297	5013806.709
UNC-2	363881.904	5013805.652
UNC-2	363884.602	5013807.201
UNC-2	363884.299	5013807.73
UNC-2	363890.004	5013811.005
UNC-2	363890.308	5013810.476
UNC-2	363893.007	5013812.026
UNC-2	363892.811	5013812.367
UNC-2	363895.586	5013813.961
UNC-2	363895.839	5013813.52
UNC-2	363898.703	5013815.164
UNC-2	363889.68	5013830.88
UNC-2	363891.025	5013831.652
UNC-2	363892.37	5013832.424
UNC-2	363901.397	5013816.711
UNC-2	363904.26	5013818.356
UNC-2	363904.007	5013818.796
UNC-2	363906.782	5013820.39
UNC-2	363906.978	5013820.049
UNC-2	363909.677	5013821.599
UNC-2	363909.373	5013822.128
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UNC-2	363915.381	5013824.876
UNC-2	363917.402	5013826.037
UNC-3	363964.106	5013847.567
UNC-3	363958.387	5013847.945
UNC-3	363957.275	5013849.936
UNC-3	363957.541	5013850.085
UNC-3	363956.55	5013851.859
UNC-3	363958.679	5013853.048
UNC-3	363956.153	5013857.571
UNC-3	363954.024	5013856.382
UNC-3	363950.165	5013863.303
UNC-3	363952.289	5013864.489
UNC-3	363949.763	5013869.012
UNC-3	363947.634	5013867.824
UNC-3	363946.654	5013869.585
UNC-3	363946.378	5013869.449
UNC-3	363945.251	5013871.467
UNC-3	363938.576	5013867.739
UNC-3	363936.012	5013872.332
UNC-3	363942.686	5013876.059
UNC-3	363941.561	5013878.077

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UNC-3	363941.826	5013878.225
UNC-3	363940.836	5013879.999
UNC-3	363942.965	5013881.188
UNC-3	363940.44	5013885.712
UNC-3	363938.31	5013884.523
UNC-3	363934.448	5013891.442
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UNC-3	363931.922	5013895.966
UNC-3	363930.932	5013897.74
UNC-3	363930.666	5013897.591
UNC-3	363929.539	5013899.609
UNC-3	363928.855	5013902.193
UNC-3	363924.785	5013909.312
UNC-3	363921.391	5013913.361
UNC-3	363920.497	5013914.924
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UNC-3	363918.077	5013920.799
UNC-3	363917.925	5013921.063
UNC-3	363919.932	5013922.21
UNC-3	363912.342	5013935.483
UNC-3	363910.336	5013934.335
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UNC-3	363908.421	5013933.591
UNC-3	363907.211	5013935.708
UNC-3	363902.713	5013933.136
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UNC-3	363892.547	5013924.514
UNC-3	363890.783	5013923.506
UNC-3	363890.935	5013923.241
UNC-3	363888.929	5013922.094
UNC-3	363889.722	5013920.706
UNC-3	363885.061	5013918.04
UNC-3	363879.367	5013914.785
UNC-3	363878.576	5013916.168
UNC-3	363876.57	5013915.021
UNC-3	363876.419	5013915.286
UNC-3	363874.655	5013914.278
UNC-3	363873.445	5013916.394
UNC-3	363868.947	5013913.823
UNC-3	363870.157	5013911.707
UNC-3	363868.036	5013910.495
UNC-3	363863.823	5013917.865
UNC-3	363912.677	5013945.802
UNC-3	363926.744	5013920.609

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UNC-3	363932.862	5013909.651
UNC-3	363965.236	5013851.67
UNC-3	363964.106	5013847.567
UNC-4	363919.785	5013822.096
UNC-4	363917.402	5013826.037
UNC-4	363918.079	5013826.426
UNC-4	363917.472	5013827.483
UNC-4	363920.423	5013829.179
UNC-4	363921.075	5013828.045
UNC-4	363924.202	5013829.841
UNC-4	363922.288	5013833.174
UNC-4	363932.242	5013838.731
UNC-4	363936.591	5013841.609
UNC-4	363938.164	5013842.488
UNC-4	363940.541	5013842.918
UNC-4	363942.67	5013844.107
UNC-4	363943.66	5013842.333
UNC-4	363943.929	5013842.483
UNC-4	363945.053	5013840.464
UNC-4	363958.402	5013847.919
UNC-4	363958.387	5013847.945
UNC-4	363964.106	5013847.567
UNC-4	363964.101	5013847.55
UNC-4	363919.785	5013822.096

[SYMBOLS]

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;;Gage X-Coord Y-Coord  
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SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix C Stormwater Management Calculations

C.3 PCSWMM MODEL OUTPUT



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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

160401085 - SNTC Lands Assessment
 Detailed Design Block 4, located north of the Jock River and west of Greenbank Road.

Element Count

Number of rain gages 1
 Number of subcatchments ... 10
 Number of nodes 26
 Number of links 27
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
Raingage	Chicago_5y_3h_10m_City	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
L101A	0.37	370.00	88.57	3.0000	Raingage
L101A-S					
L102A	0.16	116.00	87.14	3.0000	Raingage
L102A-S					
L102B	0.42	314.00	84.29	3.0000	Raingage
L102B-S					
L102C	0.09	40.00	91.43	3.0000	Raingage
L102C-S					
L102D	0.12	54.00	88.57	3.0000	Raingage
L102D-S					
L105A	0.11	56.00	87.14	3.0000	Raingage

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L105A-S

UNC-1	0.14	321.00	60.00	2.0000	Raingage
OF1					
UNC-2	0.03	200.00	42.86	3.0000	Raingage
OF2					
UNC-3	0.12	305.00	42.86	3.0000	Raingage
OF3					
UNC-4	0.03	200.00	42.86	3.0000	Raingage
OF4					

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
DICB221	OUTFALL	90.72	0.68	0.0	
MAJ-1	OUTFALL	93.33	0.36	0.0	
MAJ-2	OUTFALL	93.24	0.40	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
OF3	OUTFALL	93.52	0.00	0.0	
OF4	OUTFALL	93.44	0.00	0.0	
L101A1-S	STORAGE	93.99	0.35	0.0	
L101A-S	STORAGE	92.12	1.73	0.0	
L101A-S1	STORAGE	93.69	0.35	0.0	
L102A-S	STORAGE	92.29	1.73	0.0	
L102A-S1	STORAGE	93.77	0.35	0.0	
L102B-S	STORAGE	91.94	1.93	0.0	
L102B-S1	STORAGE	93.87	0.35	0.0	
L102B-S2	STORAGE	93.97	0.35	0.0	
L102C-S	STORAGE	92.52	1.73	0.0	
L102C-S1	STORAGE	94.10	0.35	0.0	
L102D-S	STORAGE	92.26	1.73	0.0	
L102D-S1	STORAGE	93.94	0.35	0.0	
L105A-S	STORAGE	92.08	1.73	0.0	
L105A-S1	STORAGE	93.91	0.35	0.0	
STM-101	STORAGE	90.50	3.21	0.0	
STM-102	STORAGE	91.16	2.98	0.0	
STM-103	STORAGE	91.07	2.86	0.0	
STM-104	STORAGE	91.40	2.58	0.0	
STM-105	STORAGE	91.23	2.31	0.0	

Link Summary

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Name	From Node	To Node	Type	Length
%Slope	Roughness			

C1	L105A-S1	L105A-S	CONDUIT	29.9
1.5060	0.0130			
C10	L102D-S	L102D-S1	CONDUIT	18.0
-1.6640	0.0130			
C11	L102B-S2	L102D-S	CONDUIT	10.6
3.1289	0.0130			
C12	L102B-S2	L102B-S	CONDUIT	21.4
2.1051	0.0130			
C13	L102D-S1	L102B-S1	CONDUIT	24.1
0.2899	0.0130			
C14	L101A1-S	L101A-S	CONDUIT	75.1
0.6522	0.0130			
C15	L101A1-S	L102B-S	CONDUIT	31.1
1.5110	0.0130			
C16	L101A-S1	MAJ-1	CONDUIT	18.9
1.9100	0.0130			
C17	L105A-S	MAJ-2	CONDUIT	9.7
2.2752	0.0130			
C3	L101A-S	L101A-S1	CONDUIT	14.8
-1.2803	0.0130			
C4	L102A-S1	L101A-S	CONDUIT	18.3
1.4766	0.0130			
C5	L102A-S	L102A-S1	CONDUIT	18.7
-0.5342	0.0130			
C6	L102B-S1	L102A-S	CONDUIT	20.1
0.9965	0.0130			
C7	L102C-S	L102C-S1	CONDUIT	10.3
-1.9402	0.0130			
C8	L102C-S1	L102B-S1	CONDUIT	46.4
0.4957	0.0130			
C9	L102B-S	L102B-S1	CONDUIT	21.8
-1.6081	0.0130			
Pipe_3	STM-102	STM-101	CONDUIT	82.3
0.4000	0.0130			
Pipe_4	STM-103	STM-101	CONDUIT	41.5
0.3978	0.0130			
Pipe_46	STM-101	DICB221	CONDUIT	25.2
0.3175	0.0130			
Pipe_5	STM-104	STM-103	CONDUIT	68.2
0.4004	0.0130			
Pipe_6	STM-105	STM-103	CONDUIT	26.3
0.4989	0.0130			
L101A-IC	L101A-S	STM-101	ORIFICE	

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L102A-IC	L102A-S	STM-101	ORIFICE			
L102C-IC	L102C-S	STM-102	ORIFICE			
L102D-IC	L102D-S	STM-102	ORIFICE			
L103B-IC	L102B-S	STM-102	ORIFICE			
L105A-IC	L105A-S	STM-105	ORIFICE			

Cross Section Summary						

Full Conduit Flow	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels

C1	Half-12m-MC	0.17	1.20	0.08	12.00	1
2179.29						
C10	Half-17m-MC	0.36	3.68	0.18	17.00	1
11600.04						
C11	Half-17m-MC	0.36	3.68	0.18	17.00	1
15906.50						
C12	Half-17m-MC	0.36	3.68	0.18	17.00	1
13047.26						
C13	Half-17m-MC	0.36	3.68	0.18	17.00	1
4841.99						
C14	Half-12m-MC	0.17	1.20	0.08	12.00	1
1434.14						
C15	Half-17m-MC	0.36	3.68	0.18	17.00	1
11053.93						
C16	Half-17m-MC	0.36	3.68	0.18	17.00	1
12427.71						
C17	Half-8.5mROW	0.40	2.07	0.14	12.50	1
6338.51						
C3	Half-17m-MC	0.36	3.68	0.18	17.00	1
10175.17						
C4	Half-17m-MC	0.36	3.68	0.18	17.00	1
10927.31						
C5	Half-17m-MC	0.36	3.68	0.18	17.00	1
6572.65						
C6	Half-17m-MC	0.36	3.68	0.18	17.00	1
8976.55						
C7	Half-17m-MC	0.36	3.68	0.18	17.00	1
12525.80						
C8	Half-17m-MC	0.36	3.68	0.18	17.00	1
6331.27						
C9	Half-17m-MC	0.36	3.68	0.18	17.00	1

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11403.33						
Pipe_3	CIRCULAR	0.45	0.16	0.11	0.45	1
180.33						
Pipe_4	CIRCULAR	0.30	0.07	0.07	0.30	1
60.99						
Pipe_46	CIRCULAR	0.68	0.36	0.17	0.68	1
473.65						
Pipe_5	CIRCULAR	0.30	0.07	0.07	0.30	1
61.19						
Pipe_6	CIRCULAR	0.30	0.07	0.07	0.30	1
68.31						

 Transect Summary

Transect 18mROW

Area:	0.0004	0.0014	0.0033	0.0058	0.0091
	0.0130	0.0177	0.0232	0.0293	0.0362
	0.0438	0.0522	0.0612	0.0710	0.0815
	0.0927	0.1047	0.1174	0.1307	0.1441
	0.1576	0.1712	0.1859	0.2016	0.2185
	0.2364	0.2555	0.2757	0.2970	0.3194
	0.3429	0.3675	0.3932	0.4201	0.4480
	0.4770	0.5072	0.5385	0.5708	0.6043
	0.6389	0.6746	0.7114	0.7493	0.7883
	0.8284	0.8697	0.9120	0.9554	1.0000
Hrad:					
	0.0226	0.0452	0.0679	0.0905	0.1131
	0.1357	0.1583	0.1810	0.2036	0.2262
	0.2488	0.2715	0.2941	0.3167	0.3393
	0.3619	0.3846	0.4072	0.4392	0.4836
	0.5279	0.5717	0.6114	0.6466	0.6779
	0.7058	0.7307	0.7531	0.7733	0.7916
	0.8083	0.8236	0.8378	0.8510	0.8633
	0.8748	0.8858	0.8962	0.9062	0.9158
	0.9251	0.9341	0.9428	0.9514	0.9598
	0.9680	0.9762	0.9842	0.9921	1.0000
Width:					
	0.0161	0.0321	0.0482	0.0642	0.0803
	0.0964	0.1124	0.1285	0.1445	0.1606
	0.1767	0.1927	0.2088	0.2248	0.2409
	0.2570	0.2730	0.2891	0.2982	0.2982
	0.2982	0.3123	0.3368	0.3614	0.3860
	0.4105	0.4351	0.4596	0.4842	0.5088

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0.5333	0.5579	0.5825	0.6070	0.6316
0.6561	0.6807	0.7053	0.7298	0.7544
0.7789	0.8035	0.8281	0.8526	0.8772
0.9018	0.9263	0.9509	0.9754	1.0000

Transect 24mROW
 Area:

0.0003	0.0013	0.0030	0.0053	0.0083
0.0119	0.0162	0.0211	0.0268	0.0330
0.0400	0.0476	0.0558	0.0648	0.0744
0.0846	0.0955	0.1071	0.1193	0.1322
0.1457	0.1601	0.1760	0.1936	0.2122
0.2318	0.2524	0.2740	0.2966	0.3202
0.3448	0.3703	0.3969	0.4244	0.4530
0.4825	0.5130	0.5445	0.5770	0.6105
0.6450	0.6805	0.7170	0.7544	0.7929
0.8323	0.8727	0.9142	0.9566	1.0000

Hrad:

0.0220	0.0439	0.0659	0.0879	0.1099
0.1318	0.1538	0.1758	0.1978	0.2197
0.2417	0.2637	0.2857	0.3076	0.3296
0.3516	0.3735	0.3955	0.4175	0.4395
0.4614	0.4831	0.5016	0.5262	0.5600
0.5912	0.6202	0.6471	0.6723	0.6958
0.7179	0.7388	0.7585	0.7773	0.7952
0.8122	0.8286	0.8443	0.8594	0.8740
0.8881	0.9018	0.9152	0.9281	0.9408
0.9531	0.9652	0.9770	0.9886	1.0000

Width:

0.0151	0.0301	0.0452	0.0602	0.0753
0.0903	0.1054	0.1204	0.1355	0.1505
0.1656	0.1806	0.1957	0.2108	0.2258
0.2409	0.2559	0.2710	0.2860	0.3011
0.3161	0.3441	0.3817	0.4129	0.4355
0.4581	0.4806	0.5032	0.5258	0.5484
0.5710	0.5935	0.6161	0.6387	0.6613
0.6839	0.7065	0.7290	0.7516	0.7742
0.7968	0.8194	0.8419	0.8645	0.8871
0.9097	0.9323	0.9548	0.9774	1.0000

Transect 8.5m-ROW
 Area:

0.0003	0.0012	0.0026	0.0046	0.0073
0.0105	0.0142	0.0186	0.0235	0.0291
0.0352	0.0418	0.0491	0.0569	0.0654
0.0744	0.0840	0.0941	0.1048	0.1156
0.1264	0.1374	0.1497	0.1633	0.1783
0.1947	0.2125	0.2316	0.2521	0.2740

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	0.2973	0.3219	0.3479	0.3752	0.4040
	0.4341	0.4656	0.4985	0.5327	0.5683
	0.6053	0.6437	0.6834	0.7245	0.7670
	0.8108	0.8561	0.9027	0.9507	1.0000
Hrad:					
	0.0273	0.0546	0.0819	0.1092	0.1365
	0.1638	0.1911	0.2184	0.2458	0.2731
	0.3004	0.3277	0.3550	0.3823	0.4096
	0.4369	0.4642	0.4915	0.5300	0.5831
	0.6360	0.6880	0.7322	0.7685	0.7980
	0.8220	0.8415	0.8574	0.8704	0.8813
	0.8905	0.8984	0.9053	0.9116	0.9173
	0.9228	0.9280	0.9331	0.9382	0.9432
	0.9484	0.9536	0.9589	0.9643	0.9699
	0.9756	0.9815	0.9875	0.9937	1.0000
Width:					
	0.0116	0.0232	0.0348	0.0465	0.0581
	0.0697	0.0813	0.0929	0.1045	0.1161
	0.1278	0.1394	0.1510	0.1626	0.1742
	0.1858	0.1974	0.2090	0.2157	0.2157
	0.2157	0.2314	0.2588	0.2863	0.3137
	0.3412	0.3686	0.3961	0.4235	0.4510
	0.4784	0.5059	0.5333	0.5608	0.5882
	0.6157	0.6431	0.6706	0.6980	0.7255
	0.7529	0.7804	0.8078	0.8353	0.8627
	0.8902	0.9176	0.9451	0.9725	1.0000
Transect Half-12m-MC Area:					
	0.0005	0.0019	0.0043	0.0077	0.0120
	0.0173	0.0236	0.0308	0.0390	0.0482
	0.0583	0.0694	0.0814	0.0944	0.1084
	0.1237	0.1405	0.1586	0.1774	0.1967
	0.2165	0.2368	0.2576	0.2788	0.3005
	0.3227	0.3454	0.3686	0.3922	0.4163
	0.4409	0.4660	0.4916	0.5177	0.5442
	0.5712	0.5987	0.6267	0.6551	0.6841
	0.7135	0.7434	0.7738	0.8047	0.8360
	0.8679	0.9002	0.9330	0.9662	1.0000
Hrad:					
	0.0199	0.0397	0.0596	0.0795	0.0993
	0.1192	0.1391	0.1589	0.1788	0.1987
	0.2185	0.2384	0.2583	0.2781	0.2980
	0.3167	0.3343	0.3577	0.3924	0.4261
	0.4588	0.4905	0.5213	0.5513	0.5805
	0.6089	0.6366	0.6635	0.6899	0.7155
	0.7406	0.7651	0.7779	0.7855	0.7942
	0.8040	0.8146	0.8261	0.8381	0.8508

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	0.8641	0.8778	0.8919	0.9064	0.9213
	0.9365	0.9520	0.9678	0.9838	1.0000
Width:					
	0.0283	0.0567	0.0850	0.1133	0.1417
	0.1700	0.1983	0.2267	0.2550	0.2833
	0.3117	0.3400	0.3683	0.3967	0.4292
	0.4717	0.5142	0.5467	0.5608	0.5750
	0.5892	0.6033	0.6175	0.6317	0.6458
	0.6600	0.6742	0.6883	0.7025	0.7167
	0.7308	0.7450	0.7592	0.7733	0.7875
	0.8017	0.8158	0.8300	0.8442	0.8583
	0.8725	0.8867	0.9008	0.9150	0.9292
	0.9433	0.9575	0.9717	0.9858	1.0000
Transect Half-17m-MC Area:					
	0.0004	0.0014	0.0032	0.0056	0.0088
	0.0127	0.0173	0.0228	0.0295	0.0374
	0.0465	0.0567	0.0681	0.0807	0.0945
	0.1094	0.1255	0.1428	0.1613	0.1808
	0.2011	0.2221	0.2439	0.2663	0.2894
	0.3132	0.3378	0.3630	0.3890	0.4156
	0.4429	0.4703	0.4977	0.5251	0.5524
	0.5798	0.6072	0.6347	0.6625	0.6909
	0.7197	0.7489	0.7787	0.8089	0.8396
	0.8707	0.9023	0.9344	0.9670	1.0000
Hrad:					
	0.0196	0.0393	0.0589	0.0785	0.0982
	0.1178	0.1374	0.1551	0.1703	0.1844
	0.1980	0.2114	0.2248	0.2383	0.2518
	0.2653	0.2790	0.2927	0.3065	0.3218
	0.3389	0.3565	0.3745	0.3929	0.4115
	0.4304	0.4494	0.4686	0.4879	0.5073
	0.5330	0.5665	0.6000	0.6334	0.6669
	0.7002	0.7336	0.7602	0.7800	0.7999
	0.8198	0.8398	0.8597	0.8797	0.8997
	0.9198	0.9398	0.9599	0.9799	1.0000
Width:					
	0.0212	0.0424	0.0635	0.0847	0.1059
	0.1271	0.1490	0.1843	0.2196	0.2549
	0.2902	0.3255	0.3608	0.3961	0.4314
	0.4667	0.5020	0.5373	0.5725	0.6080
	0.6212	0.6424	0.6635	0.6847	0.7059
	0.7271	0.7482	0.7694	0.7906	0.8118
	0.8235	0.8235	0.8235	0.8235	0.8235
	0.8235	0.8235	0.8306	0.8447	0.8588
	0.8729	0.8871	0.9012	0.9153	0.9294
	0.9435	0.9576	0.9718	0.9859	1.0000

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Transect Half-18mROW
Area:

0.0004	0.0018	0.0040	0.0071	0.0111
0.0160	0.0218	0.0285	0.0361	0.0445
0.0539	0.0641	0.0753	0.0873	0.1002
0.1140	0.1285	0.1429	0.1574	0.1725
0.1884	0.2050	0.2224	0.2406	0.2596
0.2793	0.2999	0.3212	0.3433	0.3662
0.3899	0.4144	0.4396	0.4657	0.4925
0.5201	0.5484	0.5776	0.6076	0.6383
0.6698	0.7021	0.7352	0.7691	0.8042
0.8406	0.8784	0.9176	0.9581	1.0000

Hrad:

0.0212	0.0424	0.0637	0.0849	0.1061
0.1273	0.1486	0.1698	0.1910	0.2122
0.2334	0.2547	0.2759	0.2971	0.3183
0.3395	0.3757	0.4165	0.4571	0.4959
0.5317	0.5649	0.5957	0.6244	0.6512
0.6763	0.6998	0.7219	0.7428	0.7625
0.7812	0.7990	0.8159	0.8321	0.8476
0.8624	0.8767	0.8904	0.9037	0.9165
0.9289	0.9410	0.9527	0.9633	0.9708
0.9777	0.9840	0.9897	0.9951	1.0000

Width:

0.0209	0.0418	0.0628	0.0837	0.1046
0.1255	0.1465	0.1674	0.1883	0.2092
0.2302	0.2511	0.2720	0.2929	0.3138
0.3348	0.3400	0.3400	0.3446	0.3630
0.3814	0.3998	0.4182	0.4366	0.4550
0.4734	0.4918	0.5102	0.5286	0.5470
0.5654	0.5838	0.6022	0.6206	0.6390
0.6574	0.6758	0.6942	0.7126	0.7310
0.7494	0.7678	0.7862	0.8080	0.8400
0.8720	0.9040	0.9360	0.9680	1.0000

Transect Half-8.5mROW
Area:

0.0003	0.0013	0.0029	0.0052	0.0082
0.0118	0.0160	0.0209	0.0265	0.0327
0.0395	0.0471	0.0552	0.0641	0.0735
0.0837	0.0943	0.1049	0.1155	0.1270
0.1396	0.1533	0.1681	0.1841	0.2011
0.2193	0.2386	0.2590	0.2806	0.3032
0.3270	0.3519	0.3779	0.4051	0.4333
0.4627	0.4932	0.5249	0.5576	0.5915
0.6264	0.6625	0.6998	0.7381	0.7779
0.8192	0.8621	0.9065	0.9525	1.0000

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Hrad:

0.0281	0.0561	0.0842	0.1122	0.1403
0.1683	0.1964	0.2245	0.2525	0.2806
0.3086	0.3367	0.3647	0.3928	0.4209
0.4489	0.4959	0.5488	0.6012	0.6488
0.6890	0.7232	0.7521	0.7768	0.7979
0.8161	0.8320	0.8460	0.8585	0.8697
0.8800	0.8894	0.8983	0.9067	0.9147
0.9224	0.9298	0.9371	0.9443	0.9514
0.9584	0.9655	0.9725	0.9787	0.9825
0.9861	0.9896	0.9931	0.9965	1.0000

Width:

0.0135	0.0271	0.0406	0.0542	0.0677
0.0812	0.0948	0.1083	0.1218	0.1354
0.1489	0.1625	0.1760	0.1895	0.2031
0.2166	0.2200	0.2200	0.2258	0.2490
0.2722	0.2954	0.3186	0.3418	0.3650
0.3882	0.4114	0.4346	0.4578	0.4810
0.5042	0.5274	0.5506	0.5738	0.5970
0.6202	0.6434	0.6666	0.6898	0.7130
0.7362	0.7594	0.7826	0.8080	0.8400
0.8720	0.9040	0.9360	0.9680	1.0000

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS

Process Models:

Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO

Infiltration Method HORTON
Flow Routing Method DYNWAVE

Surcharge Method EXTRAN

Starting Date 01/02/2018 00:00:00

Ending Date 01/03/2018 00:00:00

Antecedent Dry Days 0.0

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Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 2.00 sec
 Variable Time Step NO
 Maximum Trials 8
 Number of Threads 6
 Head Tolerance 0.001500 m

```
*****
Volume      Depth
Runoff Quantity Continuity
hectare-m   mm
-----
Total Precipitation ..... 0.068 42.514
Evaporation Loss ..... 0.000 0.000
Infiltration Loss ..... 0.013 8.229
Surface Runoff ..... 0.053 33.110
Final Storage ..... 0.002 1.252
Continuity Error (%) ..... -0.179
*****
```

```
*****
Volume      Volume
Flow Routing Continuity
hectare-m   10^6 ltr
-----
Dry Weather Inflow ..... 0.000 0.000
Wet Weather Inflow ..... 0.053 0.527
Groundwater Inflow ..... 0.000 0.000
RDII Inflow ..... 0.000 0.000
External Inflow ..... 0.000 0.004
External Outflow ..... 0.053 0.532
Flooding Loss ..... 0.000 0.000
Evaporation Loss ..... 0.000 0.000
Exfiltration Loss ..... 0.000 0.000
Initial Stored Volume .... 0.003 0.031
Final Stored Volume ..... 0.003 0.031
Continuity Error (%) ..... 0.000
*****
```

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

Minimum Time Step : 2.00 sec
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Average Time Step : 2.00 sec
 Maximum Time Step : 2.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.02

 Subcatchment Runoff Summary

Perv	Total	Total	Total	Total	Total	Imperv	
Runoff	Runoff	Runoff	Peak	Runoff	Evap	Infil	
Subcatchment	Runoff	Runoff	Precip	Runoff	mm	mm	
mm	mm	10^6 ltr	mm	Coeff		mm	

			LPS				
L101A			42.51	0.00	0.00	4.11	36.32
0.76	37.09	0.14	103.53	0.872			
L102A			42.51	0.00	0.00	4.63	35.74
0.84	36.58	0.06	43.08	0.860			
L102B			42.51	0.00	0.00	5.67	34.56
1.02	35.58	0.15	113.49	0.837			
L102C			42.51	0.00	0.00	3.09	37.48
0.56	38.04	0.03	25.74	0.895			
L102D			42.51	0.00	0.00	4.13	36.31
0.73	37.04	0.05	34.31	0.871			
L105A			42.51	0.00	0.00	4.65	35.73
0.82	36.55	0.04	30.53	0.860			
UNC-1			42.51	0.00	0.00	19.25	24.60
20.00	22.46	0.03	37.41	0.528			
UNC-2			42.51	0.00	0.00	24.10	17.55
12.59	17.86	0.01	7.37	0.420			
UNC-3			42.51	0.00	0.00	25.16	17.56
15.91	16.79	0.02	29.16	0.395			
UNC-4			42.51	0.00	0.00	24.90	17.56
15.36	17.11	0.00	7.16	0.403			

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
DICB221	OUTFALL	1.18	1.18	91.90	0 00:00	1.18
MAJ-1	OUTFALL	0.00	0.00	93.33	0 00:00	0.00
MAJ-2	OUTFALL	0.00	0.04	93.28	0 01:00	0.04
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	93.52	0 00:00	0.00
OF4	OUTFALL	0.00	0.00	93.44	0 00:00	0.00
L101A1-S	STORAGE	0.00	0.00	93.99	0 00:00	0.00
L101A-S	STORAGE	0.03	1.46	93.58	0 01:00	1.46
L101A-S1	STORAGE	0.00	0.00	93.69	0 00:00	0.00
L102A-S	STORAGE	0.03	1.48	93.77	0 01:00	1.48
L102A-S1	STORAGE	0.00	0.00	93.77	0 01:01	0.00
L102B-S	STORAGE	0.03	1.68	93.62	0 01:00	1.67
L102B-S1	STORAGE	0.00	0.00	93.87	0 00:00	0.00
L102B-S2	STORAGE	0.00	0.00	93.97	0 00:00	0.00
L102C-S	STORAGE	0.03	1.48	94.00	0 01:00	1.48
L102C-S1	STORAGE	0.00	0.00	94.10	0 00:00	0.00
L102D-S	STORAGE	0.02	1.46	93.72	0 01:00	1.46
L102D-S1	STORAGE	0.00	0.00	93.94	0 00:00	0.00
L105A-S	STORAGE	0.02	1.42	93.50	0 01:00	1.42
L105A-S1	STORAGE	0.00	0.00	93.91	0 00:00	0.00
STM-101	STORAGE	1.40	1.45	91.95	0 00:53	1.45
STM-102	STORAGE	0.74	0.93	92.09	0 01:00	0.93
STM-103	STORAGE	0.83	0.91	91.98	0 00:53	0.90
STM-104	STORAGE	0.50	0.59	92.00	0 00:52	0.59
STM-105	STORAGE	0.67	0.76	91.99	0 00:53	0.75

Node Inflow Summary

Total	Flow	Maximum	Maximum	Lateral
Inflow	Balance	Lateral	Total	Time of Max
Volume	Error	Inflow	Inflow	Occurrence
Node	Type	LPS	LPS	days hr:min
				10^6 ltr
				10^6

ltr	Percent					
DICB221		OUTFALL	0.00	237.33	0 00:54	0
0.468	0.000					
MAJ-1		OUTFALL	0.00	0.00	0 00:00	0
0	0.000 ltr					
MAJ-2		OUTFALL	0.00	14.14	0 01:00	0
0.00586	0.000					
OF1		OUTFALL	37.41	37.41	0 01:00	0.032
0.032	0.000					
OF2		OUTFALL	7.37	7.37	0 01:00	0.00537
0.00537	0.000					
OF3		OUTFALL	29.16	29.16	0 01:00	0.0199
0.0199	0.000					
OF4		OUTFALL	7.16	7.16	0 01:00	0.00495
0.00495	0.000					
L101A1-S		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L101A-S		STORAGE	103.53	103.53	0 01:00	0.138
0.138	-0.127					
L101A-S1		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L102A-S		STORAGE	43.08	43.08	0 01:00	0.057
0.0571	-0.034					
L102A-S1		STORAGE	0.00	2.02	0 01:00	0
8.76e-05	117.870					
L102B-S		STORAGE	113.49	113.49	0 01:00	0.148
0.148	-0.059					
L102B-S1		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L102B-S2		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L102C-S		STORAGE	25.74	25.74	0 01:00	0.0349
0.0349	0.053					
L102C-S1		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L102D-S		STORAGE	34.31	34.31	0 01:00	0.046
0.046	0.069					
L102D-S1		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
L105A-S		STORAGE	30.53	30.53	0 01:00	0.0406
0.0406	0.035					
L105A-S1		STORAGE	0.00	0.00	0 00:00	0
0	0.000 ltr					
STM-101		STORAGE	0.00	236.81	0 00:53	0
0.469	0.000					

```

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STM-102 STORAGE 0.00 118.86 0 01:00 0
0.232 0.000
STM-103 STORAGE 0.00 19.59 0 00:53 0
0.0417 -0.001
STM-104 STORAGE 0.00 8.08 0 00:52 0
0.00327 -0.076
STM-105 STORAGE 0.00 16.10 0 01:00 0
0.0355 -0.002

```

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

of Max Occurrence	Maximum Outflow Storage Unit	Average Volume	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume	Max Pcnt Full	Time days
-----	-----	-----	-----	-----	-----	-----	-----	-----
L101A1-S	0.000	0	0	0	0	0.000	0	0
00:00	0.00							
L101A-S	0.000	0	0	0	0	0.002	18	0
01:00	73.88							
L101A-S1	0.000	0	0	0	0	0.000	0	0
00:00	0.00							
L102A-S	0.000	1	0	0	0	0.002	100	0
01:00	29.77							

```

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L102A-S1 0.000 0 0 0 0.000 0 0
00:00 0.06
L102B-S 0.000 0 0 0 0.008 7 0
01:00 77.82
L102B-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L102B-S2 0.000 0 0 0 0.000 0 0
00:00 0.00
L102C-S 0.000 0 0 0 0.003 24 0
01:00 16.43
L102C-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L102D-S 0.000 0 0 0 0.002 8 0
01:00 24.61
L102D-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L105A-S 0.000 0 0 0 0.000 0 0
00:00 30.24
L105A-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
STM-101 0.000 0 0 0 0.000 0 0
00:00 237.33
STM-102 0.000 0 0 0 0.000 0 0
00:00 118.89
STM-103 0.000 0 0 0 0.000 0 0
00:00 20.53
STM-104 0.000 0 0 0 0.000 0 0
00:00 3.96
STM-105 0.000 0 0 0 0.000 0 0
00:00 16.77

```

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
-----	-----	-----	-----	-----
DICB221	84.50	6.41	237.33	0.468
MAJ-1	0.00	0.00	0.00	0.000
MAJ-2	0.71	9.57	14.14	0.006
OF1	11.04	3.36	37.41	0.032
OF2	10.95	0.57	7.37	0.005
OF3	8.19	2.80	29.16	0.020
OF4	4.87	1.16	7.16	0.005

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 System 17.18 23.86 7.33 0.536

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
C10	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
C11	CHANNEL	0.00	0 00:00	0.00	0.00	0.12
C12	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
C13	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C14	CHANNEL	0.00	0 00:00	0.00	0.00	0.24
C15	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
C16	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C17	CHANNEL	14.14	0 01:00	0.83	0.00	0.10
C3	CHANNEL	0.00	0 00:00	0.00	0.00	0.11
C4	CHANNEL	0.04	0 01:01	0.08	0.00	0.12
C5	CHANNEL	2.02	0 01:00	0.03	0.00	0.15
C6	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
C7	CHANNEL	0.00	0 00:00	0.00	0.00	0.14
C8	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C9	CHANNEL	0.00	0 00:00	0.00	0.00	0.13
Pipe_3	CONDUIT	118.89	0 01:00	0.75	0.66	1.00
Pipe_4	CONDUIT	20.53	0 00:53	0.29	0.34	1.00
Pipe_46	CONDUIT	237.33	0 00:54	0.66	0.50	1.00
Pipe_5	CONDUIT	8.08	0 00:52	0.12	0.13	0.99
Pipe_6	CONDUIT	16.77	0 00:54	0.24	0.25	1.00
L101A-IC	ORIFICE	73.88	0 01:00			1.00
L102A-IC	ORIFICE	27.75	0 01:00			1.00
L102C-IC	ORIFICE	16.43	0 01:00			1.00
L102D-IC	ORIFICE	24.61	0 01:00			1.00
L103B-IC	ORIFICE	77.82	0 01:00			1.00
L105A-IC	ORIFICE	16.10	0 01:00			1.00

 Flow Classification Summary

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 Adjusted ----- Fraction of Time in Flow Class

 /Actual Up Down Sub Sup Up Down Norm
 Inlet Length Dry Dry Dry Crit Crit Crit Crit Ltd
 Conduit
 Ctrl

C1	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C10	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C11	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C12	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C14	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C15	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C16	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C17	1.00	0.99	0.00	0.00	0.00	0.01	0.00	0.00	0.00
0.00									
C3	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C4	1.00	0.04	0.01	0.00	0.00	0.00	0.00	0.95	0.00
0.00									
C5	1.00	0.04	0.01	0.00	0.01	0.00	0.00	0.95	0.01
0.00									
C6	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C7	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C8	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
C9	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
Pipe_3	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_4	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_46	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									

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Pipe_5 1.00 0.00 0.00 0.00 1.00 0.00 0.00 0.00 0.00
0.00
Pipe_6 1.00 0.00 0.00 0.00 1.00 0.00 0.00 0.00 0.00
0.00

Conduit Surcharge Summary

Conduit	----- Both Ends	Hours Full Upstream	----- Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
Pipe_3	0.53	0.53	24.00	0.01	0.01
Pipe_4	24.00	24.00	24.00	0.01	0.01
Pipe_46	24.00	24.00	24.00	0.01	0.01
Pipe_5	0.01	0.01	24.00	0.01	0.01
Pipe_6	24.00	24.00	24.00	0.01	0.01

Analysis begun on: Fri Aug 7 12:31:32 2020
Analysis ended on: Fri Aug 7 12:31:36 2020
Total elapsed time: 00:00:04

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

160401085 - SNTC Lands Assessment
 Detailed Design Block 4, located north of the Jock River and west of Greenbank Road.

 Element Count

Number of rain gages 1
 Number of subcatchments ... 10
 Number of nodes 26
 Number of links 27
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
Raingage	Chicago100y_3h_10m_City	INTENSITY	10 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
L101A	0.37	370.00	88.57	3.0000	Raingage
L101A-S					
L102A	0.16	116.00	87.14	3.0000	Raingage
L102A-S					
L102B	0.42	314.00	84.29	3.0000	Raingage
L102B-S					
L102C	0.09	40.00	91.43	3.0000	Raingage
L102C-S					
L102D	0.12	54.00	88.57	3.0000	Raingage
L102D-S					
L105A	0.11	56.00	87.14	3.0000	Raingage

Node	Invert Elev.	Max. Depth	Ponded Area	External Inflow
L105A-S				
UNC-1	0.14	321.00	60.00	2.0000 Raingage
OF1				
UNC-2	0.03	200.00	42.86	3.0000 Raingage
OF2				
UNC-3	0.12	305.00	42.86	3.0000 Raingage
OF3				
UNC-4	0.03	200.00	42.86	3.0000 Raingage
OF4				

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
DICB221	OUTFALL	90.72	0.68	0.0	
MAJ-1	OUTFALL	93.33	0.36	0.0	
MAJ-2	OUTFALL	93.24	0.40	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
OF3	OUTFALL	93.52	0.00	0.0	
OF4	OUTFALL	93.44	0.00	0.0	
L101A1-S	STORAGE	93.99	0.35	0.0	
L101A-S	STORAGE	92.12	1.73	0.0	
L101A-S1	STORAGE	93.69	0.35	0.0	
L102A-S	STORAGE	92.29	1.73	0.0	
L102A-S1	STORAGE	93.77	0.35	0.0	
L102B-S	STORAGE	91.94	1.93	0.0	
L102B-S1	STORAGE	93.87	0.35	0.0	
L102B-S2	STORAGE	93.97	0.35	0.0	
L102C-S	STORAGE	92.52	1.73	0.0	
L102C-S1	STORAGE	94.10	0.35	0.0	
L102D-S	STORAGE	92.26	1.73	0.0	
L102D-S1	STORAGE	93.94	0.35	0.0	
L105A-S	STORAGE	92.08	1.73	0.0	
L105A-S1	STORAGE	93.91	0.35	0.0	
STM-101	STORAGE	90.50	3.21	0.0	
STM-102	STORAGE	91.16	2.98	0.0	
STM-103	STORAGE	91.07	2.86	0.0	
STM-104	STORAGE	91.40	2.58	0.0	
STM-105	STORAGE	91.23	2.31	0.0	

 Link Summary

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Name	From Node	To Node	Type	Length
%Slope	Roughness			

C1	L105A-S1	L105A-S	CONDUIT	29.9
1.5060	0.0130			
C10	L102D-S	L102D-S1	CONDUIT	18.0
-1.6640	0.0130			
C11	L102B-S2	L102D-S	CONDUIT	10.6
3.1289	0.0130			
C12	L102B-S2	L102B-S	CONDUIT	21.4
2.1051	0.0130			
C13	L102D-S1	L102B-S1	CONDUIT	24.1
0.2899	0.0130			
C14	L101A1-S	L101A-S	CONDUIT	75.1
0.6522	0.0130			
C15	L101A1-S	L102B-S	CONDUIT	31.1
1.5110	0.0130			
C16	L101A-S1	MAJ-1	CONDUIT	18.9
1.9100	0.0130			
C17	L105A-S	MAJ-2	CONDUIT	9.7
2.2752	0.0130			
C3	L101A-S	L101A-S1	CONDUIT	14.8
-1.2803	0.0130			
C4	L102A-S1	L101A-S	CONDUIT	18.3
1.4766	0.0130			
C5	L102A-S	L102A-S1	CONDUIT	18.7
-0.5342	0.0130			
C6	L102B-S1	L102A-S	CONDUIT	20.1
0.9965	0.0130			
C7	L102C-S	L102C-S1	CONDUIT	10.3
-1.9402	0.0130			
C8	L102C-S1	L102B-S1	CONDUIT	46.4
0.4957	0.0130			
C9	L102B-S	L102B-S1	CONDUIT	21.8
-1.6081	0.0130			
Pipe_3	STM-102	STM-101	CONDUIT	82.3
0.4000	0.0130			
Pipe_4	STM-103	STM-101	CONDUIT	41.5
0.3978	0.0130			
Pipe_46	STM-101	DICB221	CONDUIT	25.2
0.3175	0.0130			
Pipe_5	STM-104	STM-103	CONDUIT	68.2
0.4004	0.0130			
Pipe_6	STM-105	STM-103	CONDUIT	26.3
0.4989	0.0130			
L101A-IC	L101A-S	STM-101	ORIFICE	

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L102A-IC	L102A-S	STM-101	ORIFICE			
L102B-IC	L102B-S	STM-102	ORIFICE			
L102C-IC	L102C-S	STM-102	ORIFICE			
L102D-IC	L102D-S	STM-102	ORIFICE			
L105A-IC	L105A-S	STM-105	ORIFICE			

Cross Section Summary						

Full Conduit Flow	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels

C1	Half-12m-MC	0.17	1.20	0.08	12.00	1
2179.29						
C10	Half-17m-MC	0.36	3.68	0.18	17.00	1
11600.04						
C11	Half-17m-MC	0.36	3.68	0.18	17.00	1
15906.50						
C12	Half-17m-MC	0.36	3.68	0.18	17.00	1
13047.26						
C13	Half-17m-MC	0.36	3.68	0.18	17.00	1
4841.99						
C14	Half-12m-MC	0.17	1.20	0.08	12.00	1
1434.14						
C15	Half-17m-MC	0.36	3.68	0.18	17.00	1
11053.93						
C16	Half-17m-MC	0.36	3.68	0.18	17.00	1
12427.71						
C17	Half-8.5mROW	0.40	2.07	0.14	12.50	1
6338.51						
C3	Half-17m-MC	0.36	3.68	0.18	17.00	1
10175.17						
C4	Half-17m-MC	0.36	3.68	0.18	17.00	1
10927.31						
C5	Half-17m-MC	0.36	3.68	0.18	17.00	1
6572.65						
C6	Half-17m-MC	0.36	3.68	0.18	17.00	1
8976.55						
C7	Half-17m-MC	0.36	3.68	0.18	17.00	1
12525.80						
C8	Half-17m-MC	0.36	3.68	0.18	17.00	1
6331.27						
C9	Half-17m-MC	0.36	3.68	0.18	17.00	1

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11403.33						
Pipe_3	CIRCULAR	0.45	0.16	0.11	0.45	1
180.33						
Pipe_4	CIRCULAR	0.30	0.07	0.07	0.30	1
60.99						
Pipe_46	CIRCULAR	0.68	0.36	0.17	0.68	1
473.65						
Pipe_5	CIRCULAR	0.30	0.07	0.07	0.30	1
61.19						
Pipe_6	CIRCULAR	0.30	0.07	0.07	0.30	1
68.31						

 Transect Summary

Transect 18mROW

Area:	0.0004	0.0014	0.0033	0.0058	0.0091
	0.0130	0.0177	0.0232	0.0293	0.0362
	0.0438	0.0522	0.0612	0.0710	0.0815
	0.0927	0.1047	0.1174	0.1307	0.1441
	0.1576	0.1712	0.1859	0.2016	0.2185
	0.2364	0.2555	0.2757	0.2970	0.3194
	0.3429	0.3675	0.3932	0.4201	0.4480
	0.4770	0.5072	0.5385	0.5708	0.6043
	0.6389	0.6746	0.7114	0.7493	0.7883
	0.8284	0.8697	0.9120	0.9554	1.0000
Hrad:					
	0.0226	0.0452	0.0679	0.0905	0.1131
	0.1357	0.1583	0.1810	0.2036	0.2262
	0.2488	0.2715	0.2941	0.3167	0.3393
	0.3619	0.3846	0.4072	0.4392	0.4836
	0.5279	0.5717	0.6114	0.6466	0.6779
	0.7058	0.7307	0.7531	0.7733	0.7916
	0.8083	0.8236	0.8378	0.8510	0.8633
	0.8748	0.8858	0.8962	0.9062	0.9158
	0.9251	0.9341	0.9428	0.9514	0.9598
	0.9680	0.9762	0.9842	0.9921	1.0000
Width:					
	0.0161	0.0321	0.0482	0.0642	0.0803
	0.0964	0.1124	0.1285	0.1445	0.1606
	0.1767	0.1927	0.2088	0.2248	0.2409
	0.2570	0.2730	0.2891	0.2982	0.2982
	0.2982	0.3123	0.3368	0.3614	0.3860
	0.4105	0.4351	0.4596	0.4842	0.5088

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0.5333	0.5579	0.5825	0.6070	0.6316
0.6561	0.6807	0.7053	0.7298	0.7544
0.7789	0.8035	0.8281	0.8526	0.8772
0.9018	0.9263	0.9509	0.9754	1.0000

Transect 24mROW
 Area:

0.0003	0.0013	0.0030	0.0053	0.0083
0.0119	0.0162	0.0211	0.0268	0.0330
0.0400	0.0476	0.0558	0.0648	0.0744
0.0846	0.0955	0.1071	0.1193	0.1322
0.1457	0.1601	0.1760	0.1936	0.2122
0.2318	0.2524	0.2740	0.2966	0.3202
0.3448	0.3703	0.3969	0.4244	0.4530
0.4825	0.5130	0.5445	0.5770	0.6105
0.6450	0.6805	0.7170	0.7544	0.7929
0.8323	0.8727	0.9142	0.9566	1.0000

Hrad:

0.0220	0.0439	0.0659	0.0879	0.1099
0.1318	0.1538	0.1758	0.1978	0.2197
0.2417	0.2637	0.2857	0.3076	0.3296
0.3516	0.3735	0.3955	0.4175	0.4395
0.4614	0.4831	0.5016	0.5262	0.5600
0.5912	0.6202	0.6471	0.6723	0.6958
0.7179	0.7388	0.7585	0.7773	0.7952
0.8122	0.8286	0.8443	0.8594	0.8740
0.8881	0.9018	0.9152	0.9281	0.9408
0.9531	0.9652	0.9770	0.9886	1.0000

Width:

0.0151	0.0301	0.0452	0.0602	0.0753
0.0903	0.1054	0.1204	0.1355	0.1505
0.1656	0.1806	0.1957	0.2108	0.2258
0.2409	0.2559	0.2710	0.2860	0.3011
0.3161	0.3441	0.3817	0.4129	0.4355
0.4581	0.4806	0.5032	0.5258	0.5484
0.5710	0.5935	0.6161	0.6387	0.6613
0.6839	0.7065	0.7290	0.7516	0.7742
0.7968	0.8194	0.8419	0.8645	0.8871
0.9097	0.9323	0.9548	0.9774	1.0000

Transect 8.5m-ROW
 Area:

0.0003	0.0012	0.0026	0.0046	0.0073
0.0105	0.0142	0.0186	0.0235	0.0291
0.0352	0.0418	0.0491	0.0569	0.0654
0.0744	0.0840	0.0941	0.1048	0.1156
0.1264	0.1374	0.1497	0.1633	0.1783
0.1947	0.2125	0.2316	0.2521	0.2740

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	0.2973	0.3219	0.3479	0.3752	0.4040
	0.4341	0.4656	0.4985	0.5327	0.5683
	0.6053	0.6437	0.6834	0.7245	0.7670
	0.8108	0.8561	0.9027	0.9507	1.0000
Hrad:					
	0.0273	0.0546	0.0819	0.1092	0.1365
	0.1638	0.1911	0.2184	0.2458	0.2731
	0.3004	0.3277	0.3550	0.3823	0.4096
	0.4369	0.4642	0.4915	0.5300	0.5831
	0.6360	0.6880	0.7322	0.7685	0.7980
	0.8220	0.8415	0.8574	0.8704	0.8813
	0.8905	0.8984	0.9053	0.9116	0.9173
	0.9228	0.9280	0.9331	0.9382	0.9432
	0.9484	0.9536	0.9589	0.9643	0.9699
	0.9756	0.9815	0.9875	0.9937	1.0000
Width:					
	0.0116	0.0232	0.0348	0.0465	0.0581
	0.0697	0.0813	0.0929	0.1045	0.1161
	0.1278	0.1394	0.1510	0.1626	0.1742
	0.1858	0.1974	0.2090	0.2157	0.2157
	0.2157	0.2314	0.2588	0.2863	0.3137
	0.3412	0.3686	0.3961	0.4235	0.4510
	0.4784	0.5059	0.5333	0.5608	0.5882
	0.6157	0.6431	0.6706	0.6980	0.7255
	0.7529	0.7804	0.8078	0.8353	0.8627
	0.8902	0.9176	0.9451	0.9725	1.0000
Transect Half-12m-MC Area:					
	0.0005	0.0019	0.0043	0.0077	0.0120
	0.0173	0.0236	0.0308	0.0390	0.0482
	0.0583	0.0694	0.0814	0.0944	0.1084
	0.1237	0.1405	0.1586	0.1774	0.1967
	0.2165	0.2368	0.2576	0.2788	0.3005
	0.3227	0.3454	0.3686	0.3922	0.4163
	0.4409	0.4660	0.4916	0.5177	0.5442
	0.5712	0.5987	0.6267	0.6551	0.6841
	0.7135	0.7434	0.7738	0.8047	0.8360
	0.8679	0.9002	0.9330	0.9662	1.0000
Hrad:					
	0.0199	0.0397	0.0596	0.0795	0.0993
	0.1192	0.1391	0.1589	0.1788	0.1987
	0.2185	0.2384	0.2583	0.2781	0.2980
	0.3167	0.3343	0.3577	0.3924	0.4261
	0.4588	0.4905	0.5213	0.5513	0.5805
	0.6089	0.6366	0.6635	0.6899	0.7155
	0.7406	0.7651	0.7779	0.7855	0.7942
	0.8040	0.8146	0.8261	0.8381	0.8508

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	0.8641	0.8778	0.8919	0.9064	0.9213
	0.9365	0.9520	0.9678	0.9838	1.0000
Width:					
	0.0283	0.0567	0.0850	0.1133	0.1417
	0.1700	0.1983	0.2267	0.2550	0.2833
	0.3117	0.3400	0.3683	0.3967	0.4292
	0.4717	0.5142	0.5467	0.5608	0.5750
	0.5892	0.6033	0.6175	0.6317	0.6458
	0.6600	0.6742	0.6883	0.7025	0.7167
	0.7308	0.7450	0.7592	0.7733	0.7875
	0.8017	0.8158	0.8300	0.8442	0.8583
	0.8725	0.8867	0.9008	0.9150	0.9292
	0.9433	0.9575	0.9717	0.9858	1.0000
Transect Half-17m-MC Area:					
	0.0004	0.0014	0.0032	0.0056	0.0088
	0.0127	0.0173	0.0228	0.0295	0.0374
	0.0465	0.0567	0.0681	0.0807	0.0945
	0.1094	0.1255	0.1428	0.1613	0.1808
	0.2011	0.2221	0.2439	0.2663	0.2894
	0.3132	0.3378	0.3630	0.3890	0.4156
	0.4429	0.4703	0.4977	0.5251	0.5524
	0.5798	0.6072	0.6347	0.6625	0.6909
	0.7197	0.7489	0.7787	0.8089	0.8396
	0.8707	0.9023	0.9344	0.9670	1.0000
Hrad:					
	0.0196	0.0393	0.0589	0.0785	0.0982
	0.1178	0.1374	0.1551	0.1703	0.1844
	0.1980	0.2114	0.2248	0.2383	0.2518
	0.2653	0.2790	0.2927	0.3065	0.3218
	0.3389	0.3565	0.3745	0.3929	0.4115
	0.4304	0.4494	0.4686	0.4879	0.5073
	0.5330	0.5665	0.6000	0.6334	0.6669
	0.7002	0.7336	0.7602	0.7800	0.7999
	0.8198	0.8398	0.8597	0.8797	0.8997
	0.9198	0.9398	0.9599	0.9799	1.0000
Width:					
	0.0212	0.0424	0.0635	0.0847	0.1059
	0.1271	0.1490	0.1843	0.2196	0.2549
	0.2902	0.3255	0.3608	0.3961	0.4314
	0.4667	0.5020	0.5373	0.5725	0.6080
	0.6212	0.6424	0.6635	0.6847	0.7059
	0.7271	0.7482	0.7694	0.7906	0.8118
	0.8235	0.8235	0.8235	0.8235	0.8235
	0.8235	0.8235	0.8306	0.8447	0.8588
	0.8729	0.8871	0.9012	0.9153	0.9294
	0.9435	0.9576	0.9718	0.9859	1.0000

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Transect Half-18mROW

Area:

0.0004	0.0018	0.0040	0.0071	0.0111
0.0160	0.0218	0.0285	0.0361	0.0445
0.0539	0.0641	0.0753	0.0873	0.1002
0.1140	0.1285	0.1429	0.1574	0.1725
0.1884	0.2050	0.2224	0.2406	0.2596
0.2793	0.2999	0.3212	0.3433	0.3662
0.3899	0.4144	0.4396	0.4657	0.4925
0.5201	0.5484	0.5776	0.6076	0.6383
0.6698	0.7021	0.7352	0.7691	0.8042
0.8406	0.8784	0.9176	0.9581	1.0000

Hrad:

0.0212	0.0424	0.0637	0.0849	0.1061
0.1273	0.1486	0.1698	0.1910	0.2122
0.2334	0.2547	0.2759	0.2971	0.3183
0.3395	0.3757	0.4165	0.4571	0.4959
0.5317	0.5649	0.5957	0.6244	0.6512
0.6763	0.6998	0.7219	0.7428	0.7625
0.7812	0.7990	0.8159	0.8321	0.8476
0.8624	0.8767	0.8904	0.9037	0.9165
0.9289	0.9410	0.9527	0.9633	0.9708
0.9777	0.9840	0.9897	0.9951	1.0000

Width:

0.0209	0.0418	0.0628	0.0837	0.1046
0.1255	0.1465	0.1674	0.1883	0.2092
0.2302	0.2511	0.2720	0.2929	0.3138
0.3348	0.3400	0.3400	0.3446	0.3630
0.3814	0.3998	0.4182	0.4366	0.4550
0.4734	0.4918	0.5102	0.5286	0.5470
0.5654	0.5838	0.6022	0.6206	0.6390
0.6574	0.6758	0.6942	0.7126	0.7310
0.7494	0.7678	0.7862	0.8046	0.8230
0.8720	0.9040	0.9360	0.9680	1.0000

Transect Half-8.5mROW

Area:

0.0003	0.0013	0.0029	0.0052	0.0082
0.0118	0.0160	0.0209	0.0265	0.0327
0.0395	0.0471	0.0552	0.0641	0.0735
0.0837	0.0943	0.1049	0.1155	0.1270
0.1396	0.1533	0.1681	0.1841	0.2011
0.2193	0.2386	0.2590	0.2806	0.3032
0.3270	0.3519	0.3779	0.4051	0.4333
0.4627	0.4932	0.5249	0.5576	0.5915
0.6264	0.6625	0.6998	0.7381	0.7779
0.8192	0.8621	0.9065	0.9525	1.0000

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Hrad:

0.0281	0.0561	0.0842	0.1122	0.1403
0.1683	0.1964	0.2245	0.2525	0.2806
0.3086	0.3367	0.3647	0.3928	0.4209
0.4489	0.4959	0.5488	0.6012	0.6488
0.6890	0.7232	0.7521	0.7768	0.7979
0.8161	0.8320	0.8460	0.8585	0.8697
0.8800	0.8894	0.8983	0.9067	0.9147
0.9224	0.9298	0.9371	0.9443	0.9514
0.9584	0.9655	0.9725	0.9787	0.9825
0.9861	0.9896	0.9931	0.9965	1.0000

Width:

0.0135	0.0271	0.0406	0.0542	0.0677
0.0812	0.0948	0.1083	0.1218	0.1354
0.1489	0.1625	0.1760	0.1895	0.2031
0.2166	0.2200	0.2200	0.2258	0.2490
0.2722	0.2954	0.3186	0.3418	0.3650
0.3882	0.4114	0.4346	0.4578	0.4810
0.5042	0.5274	0.5506	0.5738	0.5970
0.6202	0.6434	0.6666	0.6898	0.7130
0.7362	0.7594	0.7826	0.8058	0.8400
0.8720	0.9040	0.9360	0.9680	1.0000

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method HORTON
 Flow Routing Method DYNWAVE
 Surcharge Method EXTRAN
 Starting Date 01/02/2018 00:00:00
 Ending Date 01/03/2018 00:00:00
 Antecedent Dry Days 0.0

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Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 2.00 sec
 Variable Time Step NO
 Maximum Trials 8
 Number of Threads 6
 Head Tolerance 0.001500 m

```

*****
Volume      Depth
Runoff Quantity Continuity
hectare-m   mm
-----
Total Precipitation ..... 0.114 71.665
Evaporation Loss ..... 0.000 0.000
Infiltration Loss ..... 0.015 9.663
Surface Runoff ..... 0.097 60.875
Final Storage ..... 0.002 1.252
Continuity Error (%) ..... -0.175
  
```

```

*****
Volume      Volume
Flow Routing Continuity
hectare-m   10^6 ltr
-----
Dry Weather Inflow ..... 0.000 0.000
Wet Weather Inflow ..... 0.097 0.969
Groundwater Inflow ..... 0.000 0.000
RDII Inflow ..... 0.000 0.000
External Inflow ..... 0.000 0.004
External Outflow ..... 0.097 0.973
Flooding Loss ..... 0.000 0.000
Evaporation Loss ..... 0.000 0.000
Exfiltration Loss ..... 0.000 0.000
Initial Stored Volume .... 0.001 0.012
Final Stored Volume ..... 0.001 0.013
Continuity Error (%) ..... -0.107
  
```

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

Minimum Time Step : 2.00 sec
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Average Time Step : 2.00 sec
 Maximum Time Step : 2.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.01

 Subcatchment Runoff Summary

Perv	Total	Total	Total	Total	Total	Imperv
Runoff	Runoff	Peak	Runoff	Evap	Infil	Runoff
Subcatchment	Runoff	Precip	Runoff	Runoff	mm	mm
mm	mm	10^6 ltr	mm	mm	mm	mm

			LPS			
L101A		71.66	0.00	0.00	5.00	62.19
3.21	65.40	0.24	181.43	0.913		
L102A		71.66	0.00	0.00	5.63	61.19
3.60	64.79	0.10	75.95	0.904		
L102B		71.66	0.00	0.00	6.89	59.18
4.39	63.57	0.26	201.97	0.887		
L102C		71.66	0.00	0.00	3.76	64.18
2.40	66.58	0.06	44.98	0.929		
L102D		71.66	0.00	0.00	5.02	62.17
3.19	65.36	0.08	60.61	0.912		
L105A		71.66	0.00	0.00	5.64	61.18
3.59	64.77	0.07	54.11	0.904		
UNC-1		71.66	0.00	0.00	21.56	42.10
45.08	49.29	0.07	67.67	0.688		
UNC-2		71.66	0.00	0.00	28.09	30.06
34.14	43.16	0.01	13.86	0.602		
UNC-3		71.66	0.00	0.00	28.93	30.06
40.69	42.20	0.05	54.67	0.589		
UNC-4		71.66	0.00	0.00	28.72	30.06
39.43	42.44	0.01	13.36	0.592		

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
DICB221	OUTFALL	0.64	0.64	91.36	0 00:00	0.64
MAJ-1	OUTFALL	0.00	0.01	93.34	0 01:01	0.01
MAJ-2	OUTFALL	0.00	0.06	93.30	0 01:00	0.06
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	93.52	0 00:00	0.00
OF4	OUTFALL	0.00	0.00	93.44	0 00:00	0.00
L101A1-S	STORAGE	0.00	0.00	93.99	0 00:00	0.00
L101A-S	STORAGE	0.05	1.58	93.70	0 01:02	1.58
L101A-S1	STORAGE	0.00	0.01	93.70	0 01:01	0.01
L102A-S	STORAGE	0.04	1.54	93.83	0 01:00	1.54
L102A-S1	STORAGE	0.00	0.05	93.82	0 01:00	0.05
L102B-S	STORAGE	0.06	1.77	93.71	0 01:01	1.77
L102B-S1	STORAGE	0.00	0.00	93.87	0 00:00	0.00
L102B-S2	STORAGE	0.00	0.00	93.97	0 00:00	0.00
L102C-S	STORAGE	0.05	1.57	94.09	0 01:02	1.57
L102C-S1	STORAGE	0.00	0.00	94.10	0 00:00	0.00
L102D-S	STORAGE	0.05	1.56	93.82	0 01:01	1.56
L102D-S1	STORAGE	0.00	0.00	93.94	0 00:00	0.00
L105A-S	STORAGE	0.04	1.44	93.52	0 01:00	1.44
L105A-S1	STORAGE	0.00	0.00	93.91	0 00:00	0.00
STM-101	STORAGE	0.87	0.92	91.41	0 01:01	0.92
STM-102	STORAGE	0.31	0.57	91.73	0 01:02	0.57
STM-103	STORAGE	0.31	0.41	91.48	0 01:00	0.41
STM-104	STORAGE	0.00	0.00	91.40	0 00:00	0.00
STM-105	STORAGE	0.30	0.40	91.63	0 01:00	0.40

Node Inflow Summary

Total Inflow Volume Node	Flow Balance Error Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr

ltr	Percent	Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr
0.811	0.000	DICB221	OUTFALL	0.00	245.45	0 01:01	0
8.55e-05	0.000	MAJ-1	OUTFALL	0.00	0.21	0 01:01	0
0.0208	0.000	MAJ-2	OUTFALL	0.00	37.85	0 01:00	0
0.0703	0.000	OF1	OUTFALL	67.67	67.67	0 01:00	0.0703
0.013	0.000	OF2	OUTFALL	13.86	13.86	0 01:00	0.013
0.05	0.000	OF3	OUTFALL	54.67	54.67	0 01:00	0.05
0.0123	0.000	OF4	OUTFALL	13.36	13.36	0 01:00	0.0123
0	0.000 ltr	L101A1-S	STORAGE	0.00	0.00	0 00:00	0
0.26	-0.112	L101A-S	STORAGE	181.43	224.09	0 01:00	0.243
0.000204	36.454	L101A-S1	STORAGE	0.00	5.82	0 01:01	0
0.101	-0.280	L102A-S	STORAGE	75.95	75.95	0 01:00	0.101
0.0165	-2.156	L102A-S1	STORAGE	0.00	44.47	0 01:00	0
0.265	0.033	L102B-S	STORAGE	201.97	201.97	0 01:00	0.265
0	0.000 ltr	L102B-S1	STORAGE	0.00	0.00	0 00:00	0
0.0611	-0.006	L102B-S2	STORAGE	0.00	0.00	0 00:00	0
0.0812	0.087	L102C-S	STORAGE	44.98	44.98	0 01:00	0.0611
0.072	0.039	L102C-S1	STORAGE	0.00	0.00	0 00:00	0
0.807	0.019	L102D-S	STORAGE	60.61	60.61	0 01:00	0.0812
		L102D-S1	STORAGE	0.00	0.00	0 00:00	0
		L105A-S	STORAGE	54.11	54.11	0 01:00	0.072
		L105A-S1	STORAGE	0.00	0.00	0 00:00	0
		STM-101	STORAGE	0.00	245.43	0 01:01	0

```

160401085-2020-07-30-100yr5yrBC.rpt
STM-102 STORAGE 0.00 124.24 0 01:01 0
0.407 0.029
STM-103 STORAGE 0.00 16.20 0 01:00 0
0.0509 0.023
STM-104 STORAGE 0.00 0.00 0 00:00 0
0 0.000 ltr
STM-105 STORAGE 0.00 16.20 0 01:00 0
0.0511 0.389

```

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

of Max Occurrence	Maximum Outflow Storage Unit	Average Volume	Avg Pcnc Full	Evap Loss	Exfil Loss	Maximum Volume	Max Pcnc Full	Time days
hr:min	LPS	1000 m3	Full	Loss	Loss	1000 m3	Full	
L101A1-S		0.000	0	0	0	0.000	0	0
00:00	0.00							
L101A-S		0.000	2	0	0	0.010	100	0
01:00	82.66							
L101A-S1		0.000	0	0	0	0.000	0	0
00:00	0.39							
L102A-S		0.000	2	0	0	0.002	100	0
00:53	72.75							

```

160401085-2020-07-30-100yr5yrBC.rpt
L102A-S1 0.000 0 0 0 0.000 0 0
00:00 43.40
L102B-S 0.000 0 0 0 0.032 30 0
01:01 81.84
L102B-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L102B-S2 0.000 0 0 0 0.000 0 0
00:00 0.00
L102C-S 0.000 2 0 0 0.011 91 0
01:02 16.96
L102C-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L102D-S 0.000 1 0 0 0.007 36 0
01:01 25.44
L102D-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
L105A-S 0.000 0 0 0 0.000 0 0
00:00 54.06
L105A-S1 0.000 0 0 0 0.000 0 0
00:00 0.00
STM-101 0.000 0 0 0 0.000 0 0
00:00 245.45
STM-102 0.000 0 0 0 0.000 0 0
00:00 124.23
STM-103 0.000 0 0 0 0.000 0 0
00:00 16.20
STM-104 0.000 0 0 0 0.000 0 0
00:00 0.00
STM-105 0.000 0 0 0 0.000 0 0
00:00 16.20

```

Outfall Loading Summary

Outfall Node	Flow Freq Pcnc	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
DICB221	89.98	10.43	245.45	0.811
MAJ-1	0.89	0.09	0.21	0.000
MAJ-2	1.34	17.95	37.85	0.021
OF1	11.60	7.02	67.67	0.070
OF2	11.53	1.30	13.86	0.013
OF3	11.45	5.05	54.67	0.050
OF4	7.78	1.82	13.36	0.012

 System 19.22 43.66 13.86 0.977

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CHANNEL	0.00	0 00:00	0.00	0.00	0.17
C10	CHANNEL	0.00	0 00:00	0.00	0.00	0.25
C11	CHANNEL	0.00	0 00:00	0.00	0.00	0.25
C12	CHANNEL	0.00	0 00:00	0.00	0.00	0.27
C13	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C14	CHANNEL	0.00	0 00:00	0.00	0.00	0.50
C15	CHANNEL	0.00	0 00:00	0.00	0.00	0.27
C16	CHANNEL	0.21	0 01:01	0.24	0.00	0.02
C17	CHANNEL	37.85	0 01:00	1.06	0.01	0.15
C3	CHANNEL	5.82	0 01:01	0.02	0.00	0.28
C4	CHANNEL	43.40	0 01:00	0.12	0.00	0.33
C5	CHANNEL	44.47	0 01:00	0.14	0.01	0.28
C6	CHANNEL	0.00	0 00:00	0.00	0.00	0.22
C7	CHANNEL	0.00	0 00:00	0.00	0.00	0.27
C8	CHANNEL	0.00	0 00:00	0.00	0.00	0.00
C9	CHANNEL	0.00	0 00:00	0.00	0.00	0.27
Pipe_3	CONDUIT	124.23	0 01:02	1.22	0.69	0.62
Pipe_4	CONDUIT	16.20	0 01:00	0.44	0.27	0.52
Pipe_46	CONDUIT	245.45	0 01:01	0.71	0.52	0.93
Pipe_5	CONDUIT	0.00	0 00:00	0.00	0.00	0.08
Pipe_6	CONDUIT	16.20	0 01:00	0.81	0.24	0.33
L101A-IC	ORIFICE	76.90	0 01:02			1.00
L102A-IC	ORIFICE	28.27	0 01:00			1.00
L102B-IC	ORIFICE	81.84	0 01:01			1.00
L102C-IC	ORIFICE	16.96	0 01:02			1.00
L102D-IC	ORIFICE	25.44	0 01:01			1.00
L105A-IC	ORIFICE	16.20	0 01:00			1.00

 Flow Classification Summary

 Adjusted ----- Fraction of Time in Flow Class

 /Actual Up Down Sub Sup Up Down Norm
 Inlet Length Dry Dry Dry Crit Crit Crit Crit Ltd
 Conduit
 Ctrl1

C1	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
C10	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00
C11	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00
C12	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00
C13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
C14	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00
C15	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00
C16	1.00	0.04	0.00	0.00	0.91	0.05	0.00	0.00	0.01
C17	1.00	0.99	0.00	0.00	0.00	0.01	0.00	0.00	0.00
C3	1.00	0.04	0.01	0.00	0.02	0.00	0.00	0.94	0.02
C4	1.00	0.04	0.00	0.00	0.03	0.00	0.00	0.94	0.03
C5	1.00	0.04	0.00	0.00	0.02	0.00	0.00	0.95	0.01
C6	1.00	0.98	0.02	0.00	0.00	0.00	0.00	0.00	0.00
C7	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00
C8	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
C9	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00
Pipe_3	1.00	0.00	0.20	0.00	0.80	0.00	0.00	0.00	0.98
Pipe_4	1.00	0.00	0.39	0.00	0.61	0.00	0.00	0.00	0.99
Pipe_46	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00

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Pipe_5 1.00 0.97 0.03 0.00 0.00 0.00 0.00 0.00 0.00
0.00
Pipe_6 1.00 0.01 0.00 0.00 0.00 0.00 0.00 0.99 0.00
0.00

Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Fri Aug 7 12:49:29 2020
Analysis ended on: Fri Aug 7 12:49:35 2020
Total elapsed time: 00:00:06

**SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER
MANAGEMENT REPORT**

Appendix C Stormwater Management Calculations

**C.4 REVISED SNTC SUBDIVISION PCSWMM MODEL AND HGL
COMPARISON TABLE**



160401085_2020-06-30_5CHI-HGL_SNTC.inp

[TITLE]
160401085 - SNTC Lands Assessment
Detailed Design Blocks 8 to 15, located north of the Jock River and west of
Greenbank Road.

[OPTIONS]
;;Options Value
;;-----
FLOW_UNITS LPS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING YES
SKIP_STEADY_STATE NO
START_DATE 01/02/2018
START_TIME 00:00:00
REPORT_START_DATE 01/02/2018
REPORT_START_TIME 00:00:00
END_DATE 01/03/2018
END_TIME 00:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 2
RULE_STEP 00:00:00
INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 6

[FILES]
USE HOTSTART "C:\2020\1085 - Block 4\2020-07-30\SNTC PCWMM\Submitted\5CHI-HGL.HSF"

[EVAPORATION]
;;Type Parameters

160401085_2020-06-30_5CHI-HGL_SNTC.inp

;;-----
CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;; Rain Time Snow Data
;;Name Type Intrvl Catch Source
;;-----
Raingage INTENSITY 0:10 1.0 TIMESERIES Chicago_5y_3h_10m_City

[SUBCATCHMENTS]
;; Total Pcnt. Pcnt.
;; Curb Snow
;;Name Raingage Outlet Area Imperv Width Slope
;; Length Pack
;;-----
;0.65
C208A Raingage C208A-S 0.331731 75.714 195 3
0
;0.60
C209A Raingage C209A-S 0.251315 61.429 142.8 5
0
;0.59
C210A Raingage C210A-S 0.215972 75.714 90 4
0
;0.59
C210B Raingage C210B-S 0.250004 72.857 104 4
0
CM1 Raingage GREENBANK 0.59 85.714 136.9 2
0
CM2 Raingage LP 0.7 85.714 204 2
0
CM3 Raingage STREETC 0.83 85.714 146 2
0
;C=0.6
L100A Raingage CB69-70 0.1 57.1 57.074 1.4
0
;C=0.69
L102A Raingage CB39-40 0.23 70 96.201 5.5
0
;C=0.47
L102B Raingage RYCB-01 0.03 38.6 20.515 1.8
0
;C=0.62
L102C Raingage CB41-42 0.1825 60 120.041 0.7
0
;C=0.49

160401085_2020-06-30_5CHI-HGL_SNTC.inp						
L102D 0 ;C=0.68	Raingage	RYT-01	0.11	41.4	31.402	1.5
L104A 0 ;C=0.42	Raingage	CB43-44	0.2	68.6	85.721	4.7
L104B 0 ;C=0.73	Raingage	RYT-05	0.2	31.4	41.287	2
L106A 0 ;C=0.45	Raingage	CB45-46	0.23	75.7	103.091	4.5
L106B 0 ;C=0.62	Raingage	RYT-11	0.08	35.7	28.135	1.6
L108A 0 ;C=0.62	Raingage	CB47-48	0.15	60	119.261	0.8
L118A 0 ;0.68	Raingage	CB49-50	0.14	60	150.602	2.8
L201A 0 ;0.68	Raingage	L201A-S	0.054236	78.571	40.5	2
L201B 0 ;0.68	Raingage	L201B-S	0.059995	92.857	47.6	6
L201C 0 ;0.68	Raingage	L201C-S	0.153167	47.143	111.1	3
L202A 0 ;0.65	Raingage	L202A-S	0.354367	68.571	142.3	4.5
L203A 0 ;0.70	Raingage	L203A-S	0.0657	74.286	49.5	3
L204A 0 ;0.70	Raingage	L204A-S	1.556739	71.429	742.8	2
L205A 0 ;0.85	Raingage	L205A-S	1.102616	71.429	523	2
L211A 0 ;0.84	Raingage	L211A-S	0.047633	90	35.5	5
L211B 0 ;0.65	Raingage	L211B-S	0.106429	91.429	79.4	6

160401085_2020-06-30_5CHI-HGL_SNTC.inp						
L212A 0 ;0.65	Raingage	L212A-S	0.245866	72.857	119	3.5
L213A 0 ;0.63	Raingage	L213A-S	0.245087	72.857	117.4	3.5
L214A 0 ;0.70	Raingage	L214A-S	0.245928	62.857	178.5	4.5
L215A 0 ;0.60	Raingage	L215A-S	1.14885	71.429	547	1
L216A 0 ;0.70	Raingage	L216A-S	0.619558	25	111.8	0.5
L216B 0 ;0.73	Raingage	L216A-S	0.240143	50	143.7	2.5
L218A 0 ;0.73	Raingage	L218A-S	0.087369	75.714	45	4
L218B 0 ;0.73	Raingage	L218B-S	0.044699	74.286	21	4
L219A 0 ;0.70	Raingage	L219A-S	0.23166	72.857	92	4
L219B 0 ;0.73	Raingage	L219B-S	1.061507	71.428	506	1
L219C 0 ;0.73	Raingage	L219C-S	0.300748	75.714	135	3
L220A 0 ;0.70	Raingage	L220A-S	0.074565	90	36	2
L221A 0 ;0.90	Raingage	L221A-S	1.271426	87.857	635	1
L223A 0 ;0.70	Raingage	L223A-S	0.039392	95.714	29.4	4.5
UNC-1 0 ;0.70	Raingage	STREETC	0.102069	71.429	125	3
UNC-2 0 ;0.70	Raingage	LP	0.15082	71.429	160	3

160401085_2020-06-30_5CHI-HGL_SNTC.inp						
UNC-3	Raingage	GREENBANK	0.104615	71.429	111.11	3
[SUBAREAS]	N-Imperv	N-Perv	S-Imperv	S-Perv	PctZero	RouteTo
;;Subcatchment	-----					
PctRouted	-----					
;;	-----					
-----	-----					
C208A 100	0.013	0.25	1.57	4.67	15	IMPERVIOUS
C209A 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
C210A 100	0.013	0.25	1.57	4.67	40	IMPERVIOUS
C210B 100	0.013	0.25	1.57	4.67	40	IMPERVIOUS
CM1 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
CM2 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
CM3 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L100A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L102A 100	0.013	0.25	1.57	4.67	15	IMPERVIOUS
L102B 100	0.013	0.25	1.57	4.67	100	PERVIOUS
L102C 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L102D 100	0.013	0.25	1.57	4.67	100	PERVIOUS
L104A 100	0.013	0.25	1.57	4.67	15	IMPERVIOUS
L104B 100	0.013	0.25	1.57	4.67	100	PERVIOUS
L106A 100	0.013	0.25	1.57	4.67	25	IMPERVIOUS
L106B 100	0.013	0.25	1.57	4.67	100	PERVIOUS
L108A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L118A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L201A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L201B 100	0.013	0.25	1.57	4.67	60	IMPERVIOUS

160401085_2020-06-30_5CHI-HGL_SNTC.inp						
L201C	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L202A 100	0.013	0.25	1.57	4.67	30	IMPERVIOUS
L203A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L204A 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L205A 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L211A 100	0.013	0.25	1.57	4.67	60	IMPERVIOUS
L211B 100	0.013	0.25	1.57	4.67	60	IMPERVIOUS
L212A 100	0.013	0.25	1.57	4.67	40	IMPERVIOUS
L213A 100	0.013	0.25	1.57	4.67	40	IMPERVIOUS
L214A 100	0.013	0.25	1.57	4.67	40	IMPERVIOUS
L215A 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L216A 100	0.013	0.25	1.57	4.67	25	IMPERVIOUS
L216B 100	0.013	0.25	1.57	4.67	100	PERVIOUS
L218A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L218B 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L219A 100	0.013	0.25	1.57	4.67	15	IMPERVIOUS
L219B 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L219C 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L220A 100	0.013	0.25	1.57	4.67	0	IMPERVIOUS
L221A 100	0.013	0.25	1.57	4.67	50	IMPERVIOUS
L223A 100	0.013	0.25	1.57	4.67	60	IMPERVIOUS
UNC-1 100	0.013	0.25	1.57	4.67	100	IMPERVIOUS
UNC-2 100	0.013	0.25	1.57	4.67	100	IMPERVIOUS
UNC-3 100	0.013	0.25	1.57	4.67	100	IMPERVIOUS

160401085_2020-06-30_5CHI-HGL_SNTC.inp

[INFILTRATION]					
;;Subcatchment	MaxRate	MinRate	Decay	DryTime	MaxInfil
;;-----					
C208A	76.2	13.2	4.14	7	0
C209A	76.2	13.2	4.14	7	0
C210A	76.2	13.2	4.14	7	0
C210B	76.2	13.2	4.14	7	0
CM1	76.2	13.2	4.14	7	0
CM2	76.2	13.2	4.14	7	0
CM3	76.2	13.2	4.14	7	0
L100A	76.2	13.2	4.14	7	0
L102A	76.2	13.2	4.14	7	0
L102B	76.2	13.2	4.14	7	0
L102C	76.2	13.2	4.14	7	0
L102D	76.2	13.2	4.14	7	0
L104A	76.2	13.2	4.14	7	0
L104B	76.2	13.2	4.14	7	0
L106A	76.2	13.2	4.14	7	0
L106B	76.2	13.2	4.14	7	0
L108A	76.2	13.2	4.14	7	0
L118A	76.2	13.2	4.14	7	0
L201A	76.2	13.2	4.14	7	0
L201B	76.2	13.2	4.14	7	0
L201C	76.2	13.2	4.14	7	0
L202A	76.2	13.2	4.14	7	0
L203A	76.2	13.2	4.14	7	0
L204A	76.2	13.2	4.14	7	0
L205A	76.2	13.2	4.14	7	0
L211A	76.2	13.2	4.14	7	0
L211B	76.2	13.2	4.14	7	0
L212A	76.2	13.2	4.14	7	0
L213A	76.2	13.2	4.14	7	0
L214A	76.2	13.2	4.14	7	0
L215A	76.2	13.2	4.14	7	0
L216A	76.2	13.2	4.14	7	0
L216B	76.2	13.2	4.14	7	0
L218A	76.2	13.2	4.14	7	0
L218B	76.2	13.2	4.14	7	0
L219A	76.2	13.2	4.14	7	0
L219B	76.2	13.2	4.14	7	0
L219C	76.2	13.2	4.14	7	0
L220A	76.2	13.2	4.14	7	0
L221A	76.2	13.2	4.14	7	0
L223A	76.2	13.2	4.14	7	0
UNC-1	76.2	13.2	4.14	7	0
UNC-2	76.2	13.2	4.14	7	0
UNC-3	76.2	13.2	4.14	7	0

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[JUNCTIONS]					
;;Name	Invert Elev.	Max. Depth	Init. Depth	Surcharge Depth	Ponded Area
;;-----					
1+010	93.47	0.35	0	0	0
1+096	93.79	0.35	0	0	0
1+173	93.61	0.35	0	0	0
1+250	93.79	0.35	0	0	0
1+329	93.57	0.36	0	0	0
1+400	93.76	0.35	0	0	0
1+466	94.97	0.35	0	0	0
3+057	93.71	0.35	0	0	0
;T/G = 93.27					
CB39-40	91.49	2.13	0	0.3	0
;T/G = 93.44					
CB41-42	91.75	2.04	0	0	0
;T/G = 93.4					
CB43-44	91.72	2.03	0	0	0
;T/G = 93.41					
CB45-46	91.73	2.03	0	0	0
;T/G = 93.47					
CB47-48	91.79	2.03	0	0	0
;T/G = 93.68					
CB49-50	91.97	2.06	0	0	0
;T/G = 93.32					
CB69-70	91.52	2.15	0	0	0
HP-RYCB01	93.75	0.3	0	0	0
HP-RYCB04	93.83	0.24	0	0	0
HP-RYT01	93.85	0.3	0	0	0
HP-RYT05	93.85	0.3	0	0	0
HP-RYT11	93.74	0.3	0	0	0
J4	93.53	0.42	0	0	0
;T/G = 93.53					
RYCB-72	91.42	2.41	0	0.3	0
;T/G = 93.51					
RYCB-73	91.79	2.02	0	0.3	0
;T/G = 93.55					
RYCB-76	91.62	2.23	0	0	0
[OUTFALLS]					
;;Name	Invert Elev.	Outfall Type	Stage/Table Time Series	Tide Gate	Route To
;;-----					
400	89.7	FIXED	91.94		NO
;T/G = 93.33					
CB09	91.66	FREE			NO
;T/G = 93.3					

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 CB13-14 91.62 FREE NO
 ;T/G = 93.35
 CB37-38 91.67 FREE NO
 ;NWL = 89.90, 2-year WL = 90.55
 ;5-year WL = 90.93, 100-year WL = 91.58
 HW-02 89.87 FIXED 91.58 NO
 MAJ-OUT2 93.4 NORMAL NO

[STORAGE]
 ;; Invert Max. Init. Storage Curve
 ;; Evap. Elev. Depth Depth Curve Params
 ;; Name Infiltration parameters
 ;;-----

 ;Mcon MH - Concentric
 10000 90.04 3.38 0 FUNCTIONAL 1.13 0 0 0
 0
 102 89.956 3.806 0 FUNCTIONAL 0 0 1.13 0
 0
 106 90.139 3.686 0 FUNCTIONAL 0 0 1.13 0
 0
 200 90.51 2.9 0 FUNCTIONAL 0 0 0 0
 0
 201 90.321 3.701 0 FUNCTIONAL 0 0 1.13 0
 0
 201A-S(2) 94.03 0.4 0 FUNCTIONAL 0 0 0 0
 0
 201B-S(1) 94.08 0.4 0 FUNCTIONAL 0 0 0 0
 0
 202 90.794 3.226 0 FUNCTIONAL 0 0 1.13 0
 0
 202A-S(1) 93.96 0.4 0 FUNCTIONAL 0 0 0 0
 0
 203 91.359 2.807 0 FUNCTIONAL 0 0 1.13 0
 0
 203A-S(1) 94.31 0.4 0 FUNCTIONAL 0 0 0 0
 0
 206 90.33 3.24 0 FUNCTIONAL 0 0 0 0
 0
 207 90.053 3.359 0 FUNCTIONAL 0 0 1.13 0
 0
 208 90.105 3.675 0 FUNCTIONAL 0 0 1.13 0
 0
 209 90.142 3.573 0 FUNCTIONAL 0 0 1.13 0
 0
 209A-S(1) 93.76 0.4 0 FUNCTIONAL 0 0 0 0
 0

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 210 91.115 1.922 0 FUNCTIONAL 0 0 1.13 0
 0
 210A-S(1) 93.91 0.4 0 FUNCTIONAL 0 0 0 0
 0
 210B-S(1) 93.83 0.4 0 FUNCTIONAL 0 0 0 0
 0
 211 91.194 2.96 0 FUNCTIONAL 0 0 1.13 0
 0
 212 90.683 3.205 0 FUNCTIONAL 0 0 1.13 0
 0
 212A-S(1) 93.64 0.4 0 FUNCTIONAL 0 0 0 0
 0
 213 90.868 3.096 0 FUNCTIONAL 0 0 1.13 0
 0
 213A-S(1) 93.87 0.4 0 FUNCTIONAL 0 0 0 0
 0
 214 91.248 2.567 0 FUNCTIONAL 0 0 1.13 0
 0
 214A-S(1) 93.93 0.4 0 FUNCTIONAL 0 0 0 0
 0
 215A 91.31 3.09 0 FUNCTIONAL 0 0 1.13 0
 0
 216 91.132 2.308 0 FUNCTIONAL 0 0 1.13 0
 0
 217 90.08 3.22 0 FUNCTIONAL 0 0 0 0
 0
 218 89.88 3.726 0 FUNCTIONAL 0 0 1.13 0
 0
 218A-S(1) 93.67 0.4 0 FUNCTIONAL 0 0 0 0
 0
 218B-S(1) 93.55 0.4 0 FUNCTIONAL 0 0 0 0
 0
 219 90.446 3.28 0 FUNCTIONAL 0 0 1.13 0
 0
 219A-S(1) 93.95 0.4 0 FUNCTIONAL 0 0 0 0
 0
 219B 91.18 3.12 0 FUNCTIONAL 0 0 1.13 0
 0
 219C-S(1) 93.6 0.4 0 FUNCTIONAL 0 0 0 0
 0
 220 91.179 2.704 0 FUNCTIONAL 0 0 1.13 0
 0
 221A 90.81 3.31 0 FUNCTIONAL 0 0 1.13 0
 0
 223 91.411 2.683 0 FUNCTIONAL 0 0 1.13 0
 0
 402 90 3.5 0 FUNCTIONAL 0 0 0 0
 0

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404	90.2	3.94	0	FUNCTIONAL	0	0	0	0
0								
406	90.5	5.08	0	FUNCTIONAL	0	0	0	0
0								
C208A-S	91.97	1.78	0	FUNCTIONAL	0	0	0	0
0								
C209A-S	92.23	1.78	0	FUNCTIONAL	0	0	0	0
0								
C210A-S	92.12	1.78	0	FUNCTIONAL	0	0	0	0
0								
C210B-S	92.36	1.78	0	FUNCTIONAL	0	0	0	0
0								
DICB204	91.27	2.95	0	FUNCTIONAL	0	0	0	0
0								
DICB205	91.366	2.854	0	FUNCTIONAL	0	0	0	0
0								
DICB215	91.279	2.881	0	FUNCTIONAL	0	0	0	0
0								
DICB221	90.751	2.949	0	FUNCTIONAL	0	0	0	0
0								
DICB222	90.942	2.938	0	FUNCTIONAL	0	0	0	0
0								
GREENBANK	93.765	2.4	0	FUNCTIONAL	0	0	0	0
0								
L201A-S	92.52	1.78	0	FUNCTIONAL	0	0	0	0
0								
L201B-S	92.69	1.78	0	FUNCTIONAL	0	0	0	0
0								
L201B-S(1)	94.12	0.4	0	FUNCTIONAL	0	0	0	0
0								
L201C-S	92.03	1.78	0	FUNCTIONAL	0	0	0	0
0								
L202A-S	92.51	1.78	0	FUNCTIONAL	0	0	0	0
0								
L203A-S	92.41	1.78	0	FUNCTIONAL	0	0	0	0
0								
L204A-S	92.62	2.08	0	TABULAR	L204A-V			0
0								
L205A-S	92.88	2.08	0	TABULAR	L205A-V			0
0								
L211A-S	92.68	1.78	0	FUNCTIONAL	0	0	0	0
0								
L211B-S	92.32	1.78	0	FUNCTIONAL	0	0	0	0
0								
L212A-S	92.12	1.78	0	FUNCTIONAL	0	0	0	0
0								
L213A-S	92.35	1.78	0	FUNCTIONAL	0	0	0	0
0								

160401085_2020-06-30_5CHI-HGL_SNTC.inp								
L214A-S	92.37	1.78	0	FUNCTIONAL	0	0	0	0
0								
L215A-S	92.67	2.08	0	TABULAR	L215A-V			0
0								
L216A-S	91.43	2.9	0	TABULAR	L216A-V			0
0								
L218A-S	92.06	1.78	0	FUNCTIONAL	0	0	0	0
0								
L218B-S	91.92	1.78	0	FUNCTIONAL	0	0	0	0
0								
L219A-S	92.21	1.78	0	FUNCTIONAL	0	0	0	0
0								
L219B-S	92.57	2.08	0	TABULAR	L219B-V			0
0								
L219C-S	92.09	1.78	0	FUNCTIONAL	0	0	0	0
0								
L220A-S	92.38	1.78	0	FUNCTIONAL	0	0	0	0
0								
L221A-S	92.39	2.08	0	TABULAR	L221A-V			0
0								
L223A-S	92.71	1.78	0	FUNCTIONAL	0	0	0	0
0								
LP	91.56	2.4	0	FUNCTIONAL	0	0	0	0
0								
;Mcon MH - Concentric								
MH-100	90.1	3.34	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-104	90.32	3.28	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-108	91.4	2.43	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-110	91.47	3.04	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-x10002	89.99	3.62	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-x10004	89.97	3.58	0	FUNCTIONAL	1.13	0	0	0
0								
;Mcon MH - Concentric								
MH-x10006	89.88	3.28	0	FUNCTIONAL	1.13	0	0	0
0								
OF2	93.94	0.4	0	FUNCTIONAL	0	0	0	0
0								
;T/G = 93.65								

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RYCB-01      92      1.95      0      FUNCTIONAL 0.1      0      0      0
0
;T/G = 93.72
RYT-01      91.55      2.47      0      FUNCTIONAL 0.1      0      0      0
0
;T/G = 93.7
RYT-05      91.92      2.08      0      FUNCTIONAL 0.1      0      0      0
0
;T/G = 93.58
RYT-11      91.7      2.18      0      FUNCTIONAL 0.1      0      0      0
0
STM-430     90      2.9      0      FUNCTIONAL 1.13      0      0      0
0
STREETC     91.9      2.4      0      FUNCTIONAL 0      0      0      0
0

```

[CONDUITS]

```

;;
Outlet      Init.      Inlet      Max.      Outlet      Manning      Inlet
;;Name      Offset      Flow      Flow      Node      Length      N      Offset
;-----
;-----
200-201     221A      DICB221     166      0.013      91.09
90.82      0
C1          CB69-70     MAJ-OUT2     23.864     0.035      93.61
93.53      0
C10         201A-S(2)     L214A-S      26         0.013      94.03
93.75      0
C11         203A-S(1)     L203A-S      30.62      0.016      94.31
93.79      0
C12         L203A-S      OF2          2         0.016      93.79
93.94      0
C13         L214A-S      214A-S(1)    26.18      0.013      93.75
93.93      0
C14         214A-S(1)     L213A-S      34.07      0.013      93.93
93.73      0
C15         L213A-S      213A-S(1)    25.3       0.013      93.73
93.87      0
C16         213A-S(1)     L212A-S      43.76      0.013      93.87
93.5       0
C17         L212A-S      212A-S(1)    13.5       0.013      93.5
93.63      0
C18         C210A-S      210A-S(1)    53         0.016      93.5
93.91      0
C19         LP           C210A-S      16.4       0.013      93.56
93.5       0
C2          L219C-S      219C-S(1)    27.05      0.013      93.47

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93.6      0      0
C20      C210B-S      210B-S(1)     18      0.013      93.74
93.83     0
C21      212A-S(1)     C209A-S      3.5      0.013      93.63
93.61     0
C22      C209A-S      209A-S(1)    14.8     0.013      93.61
93.76     0
C23      209A-S(1)     C208A-S      41.3     0.013      93.76
93.35     0
C24      C208A-S      1+173        42.6     0.013      93.35
93.61     0
C25      OF2          LP           15       0.013      93.94
93.56     0
C26      202A-S(1)     L214A-S      21.2     0.013      93.96
93.75     0
C27      L223A-S      L211B-S      76.8     0.013      94.09
93.7       0
C28      L211B-S      209A-S(1)    4         0.013      93.7
93.76     0
C29      L211A-S      C208A-S      112      0.013      94.06
93.35     0
C3        215A        DICB215      160      0.013      91.59
91.27     0
C30      L201B-S(1)     L201A-S      30.4     0.013      94.12
93.9       0
C32      219A-S(1)     L220A-S      12.9     0.013      93.95
93.76     0
C33      L220A-S      STREETC      14.5     0.013      93.76
93.9       0
C34      219A-S(1)     L219A-S      71.4     0.013      93.95
93.59     0
C35      L219A-S      218A-S(1)    14.5     0.013      93.59
93.67     0
C36      218A-S(1)     L219C-S      63.3     0.013      93.67
93.47     0
C37      L218A-S      218B-S(1)    15.6     0.013      93.44
93.55     0
C38      218B-S(1)     L218B-S      14.4     0.013      93.55
93.3       0
C39      L218B-S      1+010        10       0.016      93.3
93.47     0
C4        L205A-S      L202A-S      51       0.016      94.61
93.89     0
C40      L219B-S      L219A-S      142      0.016      94.3
93.59     0
C41      219C-S(1)     L218A-S      27.9     0.013      93.6
93.44     0
C42      217         10000        12.9     0.013      90.08

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90.07	0	0				
C43		210A-S(1)	C210B-S	34.3	0.013	93.91
93.74	0	0				
C44		210B-S(1)	212A-S(1)	42.7	0.013	93.83
93.63	0	0				
C45		L201B-S	201B-S(1)	3	0.013	94.07
94.08	0	0				
C46		201B-S(1)	201A-S(2)	23	0.013	94.08
94.03	0	0				
C47		L201C-S	J4	18.7	0.013	93.41
93.53	0	0				
C5		L201B-S(1)	L201C-S	56.9	0.013	94.12
93.41	0	0				
C6		L201C-S	1+329	23.05	0.016	93.37
93.57	0	0				
C7		203A-S(1)	L202A-S	84.2	0.013	94.31
93.89	0	0				
C8		219B	DICB222	149	0.013	91.18
90.94	0	0				
C9		L201A-S	201A-S(2)	4.25	0.016	93.9
94.03	0	0				
CM1		406	404	175	0.013	90.72
90.54	0	0				
CM2		404	402	140	0.013	90.54
90.4	0	0				
CM3		402	400	150	0.013	90.18
90.03	0	0				
CM4		STREETC	LP	68	0.013	93.9
93.56	0	0				
CM6		GREENBANK	LP	235.036	0.013	95.765
93.56	0	0				
L201B-W		L221A-S	L218A-S	135	0.013	94.12
93.44	0	0				
L203C-W		L216A-S	C208A-S	10	0.025	94.03
93.68	0	0				
L204B-M		L215A-S	L214A-S	131	0.016	94.4
93.75	0	0				
L205B-W		L202A-S	202A-S(1)	10.2	0.013	93.89
93.96	0	0				
L205D-W		L204A-S	L201A-S	51	0.016	94.35
93.9	0	0				
;CONC. 50-D						
MH10000-MH10002		10000	MH-x10002	9.8	0.013	90.04
90.02	0	0				
;PVC DR 35						
MH10002-STM430		MH-x10002	STM-430	5	0.013	90.01
90	0	0				
;CONC. 50-D						

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MH10004-MH10006	MH-x10004	MH-x10006		40.3	0.013	89.97
89.91	0	0				
;CONC. 50-D						
MH10006-STM469	MH-x10006	HW-02		7.3	0.013	89.88
89.87	0	0				
;CONC. 50-D						
MH100-MH10000	MH-100	10000		22.2	0.013	90.1
90.07	0	0				
;CONC. 50-D						
MH102-MH100	102	MH-100		142.6	0.013	90.26
90.1	0	0				
;CONC. 50-D						
MH104-MH102	MH-104	102		50.4	0.013	90.32
90.26	0	0				
;CONC. 50-D						
MH106-MH104	106	MH-104		105.8	0.013	90.44
90.32	0	0				
;CONC. 50-D						
MH108-MH106	MH-108	106		85.4	0.013	91.4
90.96	0	0				
;CONC. 50-D						
MH110-MH108	MH-110	MH-108		37.4	0.013	91.47
91.4	0	0				
OVF-HP-RYCB01	HP-RYCB01	CB39-40		3	0.035	93.75
93.27	0	0				
OVF-HP-RYCB04	HP-RYCB04	RYT-01		3	0.035	93.83
93.72	0	0				
OVF-HP-RYT01	HP-RYT01	CB41-42		3	0.035	93.85
93.44	0	0				
OVF-HP-RYT05	HP-RYT05	CB43-44		3	0.035	93.85
93.4	0	0				
OVF-HP-RYT11	HP-RYT11	CB47-48		3	0.035	93.74
93.47	0	0				
OVF-RYCB01	HP-RYCB01	RYCB-01		3	0.035	93.75
93.65	0	0				
OVF-RYT01	HP-RYT01	RYT-01		3	0.035	93.85
93.72	0	0				
OVF-RYT05	HP-RYT05	RYT-05		3	0.035	93.85
93.7	0	0				
OVF-RYT11	HP-RYT11	RYT-11		3	0.035	93.74
93.58	0	0				
Pipe_28	201	200		105.988	0.013	90.621
90.515	0	0				
Pipe_29	202	201		22.766	0.013	91.094
91.071	0	0				
Pipe_30	203	202		88.399	0.013	91.659
91.394	0	0				
Pipe_31	DICB204	201		16.519	0.013	91.27

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91.221	0	0				
Pipe_32	DICB205	202	15.788	0.013	91.366	
91.319	0	0				
Pipe_33	207	206	21.63	0.013	90.353	
90.331	0	0				
Pipe_34	208	207	49.292	0.013	90.405	
90.356	0	0				
Pipe_35	209	208	34.456	0.013	90.442	
90.408	0	0				
Pipe_36	210	209	111.526	0.013	91.415	
91.192	0	0				
Pipe_37	211	208	88.049	0.013	91.494	
91.23	0	0				
Pipe_37_(1)	223	211	69.187	0.013	91.731	
91.523	0	0				
Pipe_38	212	209	60.649	0.013	90.983	
90.892	0	0				
Pipe_39	213	212	64.432	0.013	91.168	
91.058	0	0				
Pipe_40	214	213	26.61	0.013	91.548	
91.468	0	0				
Pipe_41	DICB215	213	12.061	0.013	91.279	
91.243	0	0				
Pipe_42	216	207	17.974	0.013	91.432	
91.253	0	0				
Pipe_43	218	217	67.08	0.013	90.15	
90.083	0	0				
Pipe_44	219	218	77.639	0.013	90.716	
90.6	0	0				
Pipe_45	220	219	71.543	0.013	91.449	
91.091	0	0				
Pipe_46	DICB221	218	15.248	0.013	90.721	
90.675	0	0				
Pipe_47	DICB222	219	13.971	0.013	90.912	
90.87	0	0				
Pipe_58	206	102	11.964	0.013	90.328	
90.316	0	0				
Pipe_59	200	106	12.27	0.013	90.512	
90.499	0	0				
;Ipex PVC DR 35						
RYT01-RYCB72	RYT-01	RYCB-72	14	0.013	91.55	
91.42	0	0				
;Ipex PVC DR 35						
RYT02-RYCB73	RYT-05	RYCB-73	14.9	0.013	91.92	
91.79	0	0				
;Ipex PVC DR 35						
RYT11-RYCB76	RYT-11	RYCB-76	9.5	0.013	91.7	
91.62	0	0				

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ST01-01	1+010	CB69-70	5.2	0.016	93.47	
93.32	0	0				
ST01-02	1+010	CB39-40	21.2	0.016	93.47	
93.27	0	0				
ST01-03	1+096	CB39-40	59.9	0.016	93.79	
93.27	0	0				
ST01-04	1+096	CB41-42	51.3	0.016	93.79	
93.44	0	0				
ST01-05	1+173	CB41-42	25.4	0.016	93.61	
93.44	0	0				
ST01-06	1+173	CB43-44	28.7	0.016	93.61	
93.4	0	0				
ST01-07	1+250	CB43-44	50.4	0.016	93.79	
93.4	0	0				
ST01-08	1+250	CB45-46	50.2	0.016	93.79	
93.41	0	0				
ST01-09_1	J4	1+329	9.88	0.016	93.53	
93.57	0	0				
ST01-09_2	J4	CB45-46	18.32	0.016	93.533	
93.41	0	0				
ST01-10	1+329	CB47-48	28.4	0.016	93.57	
93.47	0	0				
ST01-11	1+400	CB47-48	46.6	0.016	93.76	
93.47	0	0				
ST01-12	1+400	CB49-50	18.3	0.016	93.76	
93.68	0	0				
ST01-13	1+466	CB49-50	46.4	0.016	94.97	
93.68	0	0				
ST03-01	3+057	CB69-70	25.7	0.016	93.71	
93.32	0	0				
ST05-01	1+329	CB13-14	32	0.016	93.57	
93.3	0	0				
ST06-01	1+173	CB09	35.3	0.016	93.61	
93.33	0	0				
ST07-01	1+400	CB37-38	36.9	0.016	93.76	
93.35	0	0				
;PVC DR 35						
STM431-MH10004	STM-430	MH-x10004	5	0.013	90	
89.99	0	0				
[ORIFICES]						
;;	Inlet	Outlet	Orifice	Crest	Disch.	
;;	Flap Open/Close					
;;Name	Node	Node	Type	Height	Coeff.	
Gate Time						
;;						

C208A-IC1	C208A-S	208	SIDE	91.97	0.61	

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NO 0						
C208A-IC2	C208A-S	208	SIDE	91.97	0.61	
NO 0						
C209A-IC1	C209A-S	209	SIDE	92.23	0.61	
NO 0						
C209A-IC2	C209A-S	209	SIDE	92.23	0.61	
NO 0						
C210A-IC1	C210A-S	210	SIDE	92.12	0.61	
NO 0						
C210A-IC2	C210A-S	210	SIDE	92.12	0.61	
NO 0						
C210B-IC1	C210B-S	210	SIDE	92.36	0.61	
NO 0						
C210B-IC2	C210B-S	210	SIDE	92.36	0.61	
NO 0						
L201A-IC	L201A-S	201	SIDE	92.52	0.61	
NO 0						
L201B-IC1	L201B-S	201	SIDE	92.69	0.61	
NO 0						
L201C-IC1	L201C-S	201	SIDE	92.03	0.61	
NO 0						
L201C-IC2	L201C-S	201	SIDE	92.03	0.61	
NO 0						
L202A-IC1	L202A-S	202	SIDE	92.51	0.61	
NO 0						
L202A-IC2	L202A-S	202	SIDE	92.51	0.61	
NO 0						
L203A-IC1	L203A-S	203	SIDE	92.41	0.61	
NO 0						
L203A-IC2	L203A-S	203	SIDE	92.41	0.61	
NO 0						
L211A-IC	L211A-S	211	SIDE	92.68	0.61	
NO 0						
L211B-IC	L211B-S	211	SIDE	92.13	0.61	
NO 0						
L212A-IC1	L212A-S	212	SIDE	92.12	0.61	
NO 0						
L212A-IC2	L212A-S	212	SIDE	92.12	0.61	
NO 0						
L213A-IC1	L213A-S	213	SIDE	92.35	0.61	
NO 0						
L213A-IC2	L213A-S	213	SIDE	92.35	0.61	
NO 0						
L214A-IC1	L214A-S	214	SIDE	92.37	0.61	
NO 0						
L214A-IC2	L214A-S	214	SIDE	92.37	0.61	
NO 0						
L216A-IC1	L216A-S	216	SIDE	91.43	0.61	

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NO 0						
L218A-IC1	L218A-S	218	SIDE	92.06	0.61	
NO 0						
L218A-IC2	L218A-S	218	SIDE	92.06	0.61	
NO 0						
L218B-IC1	L218B-S	218	SIDE	91.92	0.61	
NO 0						
L218B-IC2	L218B-S	218	SIDE	91.92	0.61	
NO 0						
L219A-IC1	L219A-S	219	SIDE	92.21	0.61	
NO 0						
L219A-IC2	L219A-S	219	SIDE	92.21	0.61	
NO 0						
L219C-IC1	L219C-S	219	SIDE	92.09	0.61	
NO 0						
L219C-IC2	L219C-S	219	SIDE	92.09	0.61	
NO 0						
L220A-IC1	L220A-S	220	SIDE	92.38	0.61	
NO 0						
L220A-IC2	L220A-S	220	SIDE	92.38	0.61	
NO 0						
L223A-IC	L223A-S	223	SIDE	92.71	0.61	
NO 0						
OCB-39	CB39-40	MH-100	SIDE	91.49	0.61	
NO 0						
OCB-40	CB39-40	MH-100	SIDE	91.49	0.61	
NO 0						
OCB-41	CB41-42	102	SIDE	91.75	0.61	
NO 0						
OCB-42	CB41-42	102	SIDE	91.75	0.61	
NO 0						
OCB-43	CB43-44	MH-104	SIDE	91.72	0.61	
NO 0						
OCB-44	CB43-44	MH-104	SIDE	91.72	0.61	
NO 0						
OCB-45	CB45-46	106	SIDE	91.73	0.61	
NO 0						
OCB-46	CB45-46	106	SIDE	91.73	0.61	
NO 0						
OCB-47	CB47-48	MH-108	SIDE	91.79	0.61	
NO 0						
OCB-48	CB47-48	MH-108	SIDE	91.79	0.61	
NO 0						
OCB-49	CB49-50	MH-110	SIDE	91.97	0.61	
NO 0						
OCB-50	CB49-50	MH-110	SIDE	91.97	0.61	
NO 0						
OCB-69	CB69-70	10000	SIDE	91.52	0.61	

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NO 0
OCB-70 CB69-70 10000 SIDE 91.52 0.61
NO 0
ORYCB-01 RYCB-01 102 SIDE 92 0.61
NO 0
ORYCB-72 RYCB-72 102 SIDE 91.42 0.61
NO 0
ORYCB-73 RYCB-73 MH-104 SIDE 91.79 0.61
NO 0
ORYCB-76 RYCB-76 MH-108 SIDE 91.62 0.61
NO 0

[WEIRS]
;;
; Flap End Inlet Outlet Weir Crest Disch.
; ; Name Node Node Type Height Coeff.
; Gate Con. Coeff. Surchage RoadWidth RoadSurf Coeff. Curve
; ; -----
; ; -----
; CONC. 50-D
MH10002-MH10004 MH-x10002 MH-x10004 TRANSVERSE 90.68 1.84
NO 0 0 YES

[OUTLETS]
;;
; Qcoeff/ Inlet Flap Outlet Outflow Outlet
; ; Name Node Node Node Height Type
; QTable Qexpon Gate
; ; -----
; ; -----
CM-1 GREENBANK 406 93.765 TABULAR/HEAD S1-S
NO
CM-2 LP 404 91.56 TABULAR/HEAD S2-S
NO
CM-3 STREETC 402 91.9 TABULAR/HEAD S3-S
NO
L204A-0 L204A-S DICB204 92.62 TABULAR/DEPTH
L204A-Q NO
L205A-0 L205A-S DICB205 92.88 TABULAR/DEPTH
L205A-Q NO
L215A-0 L215A-S 215A 92.67 TABULAR/DEPTH
L215A-Q NO
L219B-0 L219B-S 219B 92.57 TABULAR/DEPTH
L219B-Q NO
L221A-0 L221A-S 221A 92.39 TABULAR/DEPTH
L221A-Q NO

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[XSECTIONS]
;;Link Shape Geom1 Geom2 Geom3 Geom4
; ; Barrels
; ; -----
; ; -----
200-201 CIRCULAR 0.6 0 0 0 1
C1 TRIANGULAR 0.3 1.8 0 0 1
C10 IRREGULAR Half-18mROW 0 0 0 1
C11 IRREGULAR 18mROW 0 0 0 1
C12 IRREGULAR 18mROW 0 0 0 1
C13 IRREGULAR Street-E-18m-ROW 0 0 0 1
C14 IRREGULAR Street-E-18m-ROW 0 0 0 1
C15 IRREGULAR Street-E-18m-ROW 0 0 0 1
C16 IRREGULAR 18mROW 0 0 0 1
C17 IRREGULAR 18mROW 0 0 0 1
C18 IRREGULAR Jockvale 0 0 0 1
C19 IRREGULAR Jockvale 0 0 0 1
C2 IRREGULAR Street-C-mount-14m-ROW 0 0 0 1
C20 IRREGULAR Jockvale 0 0 0 1
C21 IRREGULAR Jockvale 0 0 0 1
C22 IRREGULAR Jockvale 0 0 0 1
C23 IRREGULAR Jockvale 0 0 0 1
C24 IRREGULAR Jockvale 0 0 0 1
C25 IRREGULAR Street-A-18mROW 0 0 0 1
C26 IRREGULAR Half-18mROW 0 0 0 1
C27 IRREGULAR Half-LaneD-8.5mROW 0 0 0 1
C28 IRREGULAR Half-LaneD-8.5mROW 0 0 0 1

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C29	IRREGULAR	Half-LaneD-8.5mROW	0	0	0	1
C3	CIRCULAR	0.6	0	0	0	1
C30	IRREGULAR	Half-18mROW	0	0	0	1
C32	IRREGULAR	Street-C-14m-ROW	0	0	0	1
C33	IRREGULAR	Street-C-14m-ROW	0	0	0	1
C34	IRREGULAR	Street-C-14m-ROW	0	0	0	1
C35	IRREGULAR	Street-C-14m-ROW	0	0	0	1
C36	IRREGULAR	Street-C-mount-14m-ROW	0	0	0	1
C37	IRREGULAR	Street-C-mount-14m-ROW	0	0	0	1
C38	IRREGULAR	Street-C-mount-14m-ROW	0	0	0	1
C39	IRREGULAR	18mROW	0	0	0	1
C4	IRREGULAR	18mROW	0	0	0	1
C40	IRREGULAR	8.5m-ROW	0	0	0	1
C41	IRREGULAR	Street-C-mount-14m-ROW	0	0	0	1
C42	CIRCULAR	1.2	0	0	0	1
C43	IRREGULAR	Jockvale	0	0	0	1
C44	IRREGULAR	Jockvale	0	0	0	1
C45	IRREGULAR	Half-LaneD-8.5mROW	0	0	0	1
C46	IRREGULAR	Half-18mROW	0	0	0	1
C47	IRREGULAR	Street-A-18mROW	0	0	0	1
C5	IRREGULAR	Street-A-18mROW	0	0	0	1
C6	IRREGULAR	18mROW	0	0	0	1
C7	IRREGULAR	Street-A-18mROW	0	0	0	1
C8	CIRCULAR	0.6	0	0	0	1

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C9	IRREGULAR	Half-18mROW	0	0	0	1
CM1	CIRCULAR	1.05	0	0	0	1
CM2	CIRCULAR	1.05	0	0	0	1
CM3	HORIZ_ELLIPSE	10	0	0	0	1
CM4	IRREGULAR	ChapmanMills	0	0	0	1
CM6	IRREGULAR	ChapmanMills	0	0	0	1
L201B-W	IRREGULAR	8.5m-ROW	0	0	0	1
L203C-W	TRIANGULAR	0.3	1.8	0	0	1
L204B-M	IRREGULAR	8.5m-ROW	0	0	0	1
L205B-W	IRREGULAR	Street-A-18mROW	0	0	0	1
L205D-W	IRREGULAR	8.5m-ROW	0	0	0	1
MH10000-MH10002	HORIZ_ELLIPSE	1.219	1.93	0	0	1
MH10002-STM430	CIRCULAR	1.219	0	0	0	1
MH10004-MH10006	HORIZ_ELLIPSE	1.219	1.93	0	0	1
MH10006-STM469	HORIZ_ELLIPSE	1.219	1.93	0	0	1
MH100-MH10000	HORIZ_ELLIPSE	1.219	1.93	0	0	1
MH102-MH100	HORIZ_ELLIPSE	1.219	1.93	0	0	1
MH104-MH102	CIRCULAR	1.219	0	0	0	1
MH106-MH104	CIRCULAR	1.219	0	0	0	1
MH108-MH106	CIRCULAR	0.686	0	0	0	1
MH110-MH108	CIRCULAR	0.686	0	0	0	1
OVF-HP-RYCB01	TRIANGULAR	0.3	1.8	0	0	1
OVF-HP-RYCB04	TRIANGULAR	0.3	1.8	0	0	1
OVF-HP-RYT01	TRIANGULAR	0.3	1.8	0	0	1

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OVF-HP-RYT05	TRIANGULAR	0.3	1.8	0	0	1
OVF-HP-RYT11	TRIANGULAR	0.3	1.8	0	0	1
OVF-RYCB01	TRIANGULAR	0.3	1.8	0	0	1
OVF-RYT01	TRIANGULAR	0.3	1.8	0	0	1
OVF-RYT05	TRIANGULAR	0.3	1.8	0	0	1
OVF-RYT11	TRIANGULAR	0.3	1.8	0	0	1
Pipe_28	CIRCULAR	1.2	0	0	0	1
Pipe_29	CIRCULAR	0.75	0	0	0	1
Pipe_30	CIRCULAR	0.45	0	0	0	1
Pipe_31	CIRCULAR	0.6	0	0	0	1
Pipe_32	CIRCULAR	0.525	0	0	0	1
Pipe_33	CIRCULAR	1.2	0	0	0	1
Pipe_34	CIRCULAR	1.2	0	0	0	1
Pipe_35	CIRCULAR	1.2	0	0	0	1
Pipe_36	CIRCULAR	0.45	0	0	0	1
Pipe_37	CIRCULAR	0.375	0	0	0	1
Pipe_37_(1)	CIRCULAR	0.375	0	0	0	1
Pipe_38	CIRCULAR	0.75	0	0	0	1
Pipe_39	CIRCULAR	0.675	0	0	0	1
Pipe_40	CIRCULAR	0.375	0	0	0	1
Pipe_41	CIRCULAR	0.6	0	0	0	1
Pipe_42	CIRCULAR	0.3	0	0	0	1
Pipe_43	CIRCULAR	1.2	0	0	0	1
Pipe_44	CIRCULAR	0.75	0	0	0	1

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Pipe_45	CIRCULAR	0.375	0	0	0	1
Pipe_46	CIRCULAR	0.675	0	0	0	1
Pipe_47	CIRCULAR	0.6	0	0	0	1
Pipe_58	CIRCULAR	1.2	0	0	0	1
Pipe_59	CIRCULAR	1.2	0	0	0	1
RYT01-RYCB72	CIRCULAR	0.25	0	0	0	1
RYT02-RYCB73	CIRCULAR	0.25	0	0	0	1
RYT11-RYCB76	CIRCULAR	0.25	0	0	0	1
ST01-01	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-02	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-03	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-04	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-05	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-06	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-07	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-08	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-09_1	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-09_2	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-10	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-11	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-12	IRREGULAR	8.5m_ROW	0	0	0	1
ST01-13	IRREGULAR	8.5m_ROW	0	0	0	1
ST03-01	IRREGULAR	8.5m_ROW	0	0	0	1
ST05-01	IRREGULAR	8.5m_ROW	0	0	0	1

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ST06-01	IRREGULAR	9.5m_ROW	0	0	0	1
ST07-01	IRREGULAR	8.5m_ROW	0	0	0	1
STM431-MH10004	CIRCULAR	1.219	0	0	0	1
C208A-IC1	CIRCULAR	0.127	0	0	0	
C208A-IC2	CIRCULAR	0.127	0	0	0	
C209A-IC1	CIRCULAR	0.127	0	0	0	
C209A-IC2	CIRCULAR	0.127	0	0	0	
C210A-IC1	CIRCULAR	0.102	0	0	0	
C210A-IC2	CIRCULAR	0.102	0	0	0	
C210B-IC1	CIRCULAR	0.127	0	0	0	
C210B-IC2	CIRCULAR	0.127	0	0	0	
L201A-IC	CIRCULAR	0.083	0	0	0	
L201B-IC1	CIRCULAR	0.083	0	0	0	
L201C-IC1	CIRCULAR	0.094	0	0	0	
L201C-IC2	CIRCULAR	0.094	0	0	0	
L202A-IC1	CIRCULAR	0.108	0	0	0	
L202A-IC2	CIRCULAR	0.108	0	0	0	
L203A-IC1	CIRCULAR	0.083	0	0	0	
L203A-IC2	CIRCULAR	0.083	0	0	0	
L211A-IC	CIRCULAR	0.083	0	0	0	
L211B-IC	CIRCULAR	0.094	0	0	0	
L212A-IC1	CIRCULAR	0.094	0	0	0	
L212A-IC2	CIRCULAR	0.094	0	0	0	
L213A-IC1	CIRCULAR	0.094	0	0	0	
L213A-IC2	CIRCULAR	0.094	0	0	0	
L214A-IC1	CIRCULAR	0.094	0	0	0	
L214A-IC2	CIRCULAR	0.094	0	0	0	
L216A-IC1	CIRCULAR	0.152	0	0	0	
L218A-IC1	CIRCULAR	0.094	0	0	0	
L218A-IC2	CIRCULAR	0.094	0	0	0	
L218B-IC1	CIRCULAR	0.083	0	0	0	
L218B-IC2	CIRCULAR	0.083	0	0	0	
L219A-IC1	CIRCULAR	0.094	0	0	0	
L219A-IC2	CIRCULAR	0.094	0	0	0	
L219C-IC1	CIRCULAR	0.108	0	0	0	
L219C-IC2	CIRCULAR	0.108	0	0	0	
L220A-IC1	CIRCULAR	0.094	0	0	0	
L220A-IC2	CIRCULAR	0.094	0	0	0	
L223A-IC	CIRCULAR	0.083	0	0	0	
OCB-39	CIRCULAR	0.083	0	0	0	
OCB-40	CIRCULAR	0.083	0	0	0	
OCB-41	CIRCULAR	0.083	0	0	0	
OCB-42	CIRCULAR	0.083	0	0	0	
OCB-43	CIRCULAR	0.083	0	0	0	

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OCB-44	CIRCULAR	0.083	0	0	0
OCB-45	CIRCULAR	0.083	0	0	0
OCB-46	CIRCULAR	0.083	0	0	0
OCB-47	CIRCULAR	0.083	0	0	0
OCB-48	CIRCULAR	0.083	0	0	0
OCB-49	CIRCULAR	0.083	0	0	0
OCB-50	CIRCULAR	0.083	0	0	0
OCB-69	CIRCULAR	0.083	0	0	0
OCB-70	CIRCULAR	0.083	0	0	0
ORYCB-01	CIRCULAR	0.083	0	0	0
ORYCB-72	CIRCULAR	0.102	0	0	0
ORYCB-73	CIRCULAR	0.127	0	0	0
ORYCB-76	CIRCULAR	0.083	0	0	0
MH10002-MH10004	RECT_OPEN	1	2.4	0	0

[TRANSECTS]

;Full street, width = 8.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.245m.

NC 0.025	0.025	0.013						
X1 18mROW	7	10	18.5	0.0	0.0	0.0	0.0	
GR 0.35	0	0.15	10	0	10	0.13	14.25	0
GR 0.15	18.5	0.35	28.5					

;Full street, width = 11m, curb = 0.15m , cross-slope = 0.016m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

NC 0.025	0.025	0.013						
X1 24mROW	7	10	21	0.0	0.0	0.0	0.0	
GR 0.35	0	0.15	10	0	10	0.165	15.5	0
GR 0.15	21	0.35	31					

;NOVATECH

NC 0.016	0.016	0.016						
X1 8.5m_ROW	9	5	15.5	0.0	0.0	0.0	0.0	0.0
GR 0.35	0	0.16	5	0.15	6	0	6.01	0.13
GR 0	14.49	0.15	14.5	0.16	15.5	0.35	20.5	

;Full street, width = 5.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.02m/m, bank-height = 0.23m.

NC 0.025	0.025	0.013						
X1 8.5m-ROW	7	10	15.5	0.0	0.0	0.0	0.0	0.0
GR 0								

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GR 0.35	0	0.15	10	0	10	0.13	12.75	0
15.5								
GR 0.15	15.5	0.35	25.5					
NC 0.01	0.01	0.01						
X1 9.5m_ROW	9	5	16.5	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.35	0	0.16	5	0.15	6	0	6.01	0.14
10.75								
GR 0	15.49	0.15	15.5	0.16	16.5	0.35	21.5	
;41mROW								
NC 0.025	0.025	0.013						
X1 ChapmanMills	16	6	35	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.27	0	0.17	4.8	0.15	6	0	6	0.18
12								
GR 0.23	12	0.22	16.5	0.17	16.5	0.29	20.5	0.17
24.5								
GR 0.22	24.5	0.23	29	0.18	29	0	35	0.15
35								
GR 0.27	41							
NC 0.013	0.025	0.013						
X1 Half-18mROW	5	0.0	4.25	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.13	0	0	4.25	0.15	4.25	0.35	10	0.4
12.5								
NC 0.013	0.025	0.013						
X1 Half-8.5mROW	5	0.0	2.75	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.13	0	0	2.75	0.15	2.75	0.35	10	0.4
12.5								
NC 0.013	0.013	0.013						
X1 Half-LaneD-8.5mROW	4	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.07	0	0	2.75	0.05	2.75	0.1	4.25	
;Full street, width = 9.5m, curb = 0.15m , cross-slope = 3%, bank-slope = 1.8-2.6%, bank-height = 0.20m.								
NC 0.025	0.025	0.013						
X1 Jockvale	11	2.7	26.7	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.35	0	0.28	2.7	0.23	5.45	0.15	9.95	0
9.95								
GR 0.13	14.7	0	19.45	0.15	19.45	0.23	23.95	0.28

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26.7								
GR 0.35	29.4							
NC 0.016	0.016	0.016						
X1 novatech-8.5m_ROW	9	5	15.5	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.35	0	0.16	5	0.15	6	0	6.01	0.13
10.25								
GR 0	14.49	0.15	14.5	0.16	15.5	0.35	20.5	
;Full street, width = 8.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 2-3%, bank-height = 0.19m.								
NC 0.025	0.025	0.013						
X1 Street-A-18mROW	10	7.65	17.65	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.34	0	0.25	2.9	0.15	7.65	0	7.65	0.13
11.9								
GR 0	16.15	0.15	16.15	0.18	17.65	0.25	20.9	0.34
23.8								
;Barrier Curb, 8.5m-wide pavement, 1.8m-sidewalk on one side, 1.5m-wide boulevard on the other side, followed by K-B SWMF								
NC 0.025	0.025	0.013						
X1 Street-C-14m-ROW	8	1.5	11.8	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.18	0	0.15	1.5	0	1.5	0.13	5.75	0
10								
GR 0.15	10	0.19	11.8	0.23	14			
;Barrier/Mountable Curb, 8.5m-wide pavement, 1.8m-sidewalk on one side, 1.5m-wide boulevard on the other side, followed by K-B SWMF								
NC 0.025	0.025	0.013						
X1 Street-C-mount-14m-ROW	9	1.5	11.8	0.0	0.0	0.0	0.0	0.0
0.0	0.0							
GR 0.18	0	0.15	1.5	0	1.5	0.13	5.75	0
10								
GR 0.05	10	0.09	11.8	0.13	14	0.23	18.8	
;Full street, width = 8.5m, curb = 0.15m , cross-slope = 0.03m/m, bank-slope = 0.027m/m, bank-height = 0.13m.								
NC 0.025	0.025	0.013						
X1 Street-E-18m-ROW	7	4.75	13.25	0.0	0.0	0.0	0.0	0.0
0.0								
GR 0.28	0	0.15	4.75	0	4.75	0.13	9	0
13.25								
GR 0.15	13.25	0.28	18					
[LOSSES]								

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;;Link	Inlet	Outlet	Average	Flap Gate	SeepageRate
;;-----					
200-201	0	1.26	0	NO	0
C3	0	0.02	0	NO	0
C42	0	0.02	0	NO	0
C8	0	0.02	0	NO	0
CM1	0	0.06	0	NO	0
CM2	0	0.06	0	NO	0
MH10000-MH10002	0.5	0.39	0	NO	0
MH10004-MH10006	0.5	0.39	0	NO	0
MH10006-STM469	0.5	1	0.5	NO	0
MH100-MH10000	0.5	0.39	0	NO	0
MH102-MH100	0.5	0	0	NO	0
MH104-MH102	0.5	1.32	0	NO	0
MH106-MH104	0.5	0	0	NO	0
MH108-MH106	0.5	0	0	NO	0
MH110-MH108	0.5	0.09	0	NO	0
Pipe_28	0	0.06	0	NO	0
Pipe_29	0	0.06	0	NO	0
Pipe_30	0	0.06	0	NO	0
Pipe_31	0	0.02	0	NO	0
Pipe_32	0	1.32	0	NO	0
Pipe_33	0	0.06	0	NO	0
Pipe_34	0	0.06	0	NO	0
Pipe_35	0	0.06	0	NO	0
Pipe_36	0	0.06	0	NO	0
Pipe_37	0	1.32	0	NO	0
Pipe_37_(1)	0	0.06	0	NO	0
Pipe_38	0	1.32	0	NO	0
Pipe_39	0	0.06	0	NO	0
Pipe_40	0	0.06	0	NO	0
Pipe_41	0	1.32	0	NO	0
Pipe_42	0	1.32	0	NO	0
Pipe_43	0	0.06	0	NO	0
Pipe_44	0	0.06	0	NO	0
Pipe_45	0	0.06	0	NO	0
Pipe_46	0	1.32	0	NO	0
Pipe_47	0	0.02	0	NO	0
RYT01-RYCB72	0.5	1.32	0	NO	0
RYT02-RYCB73	0.5	1.32	0	NO	0
RYT11-RYCB76	0.5	1.32	0	NO	0

[CURVES]			
;;Name	Type	X-Value	Y-Value
;;-----			
C202A-Q	Rating	0	0
C202A-Q		1.38	116.6
C202A-Q		1.73	128.3

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C202A-Q		2.08	130.6
L201A-Q	Rating	0	0
L201A-Q		1.38	79.8
L201A-Q		1.73	87.8
L201A-Q		2.08	89.4
L201C-Q	Rating	0	0
L201C-Q		1.38	27.6
L201C-Q		1.73	30.4
L201C-Q		2.08	30.9
L202B-Q	Rating	0	0
L202B-Q		1.38	127
L202B-Q		1.73	139.7
L202B-Q		2.08	142.2
L202C-Q	Rating	0	0
L202C-Q		1.38	20.3
L202C-Q		1.68	22.3
L203A-Q	Rating	0	0
L203A-Q		1.38	52.3
L203A-Q		1.73	57.5
L203A-Q		2.08	58.6
L204A-Q	Rating	0	0
L204A-Q		1.38	237
L204A-Q		1.73	261
L204A-Q		2.08	265
L205AA-Q	Rating	0	0
L205AA-Q		1.38	88.2
L205AA-Q		1.73	97
L205A-Q	Rating	0	0
L205A-Q		1.38	168
L205A-Q		1.73	185
L205A-Q		2.08	188
L205C-Q	Rating	0	0
L205C-Q		1.38	123.6
L205C-Q		1.73	136
L205C-Q		2.08	138.4
L215A-Q	Rating	0	0
L215A-Q		1.38	175
L215A-Q		1.73	193

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L215A-Q		2.08	196
L216A-Q	Rating	0	0
L216A-Q		2.3	76
L216A-Q		2.6	83.6
L216A-Q		2.9	85.12
L218A-Q	Rating	0	0
L218A-Q		1.38	60.3
L218A-Q		1.73	66.3
L218A-Q		2.08	67.5
L219B-Q	Rating	0	0
L219B-Q		1.38	162
L219B-Q		1.73	178
L219B-Q		2.08	181
L221A-Q	Rating	0	0
L221A-Q		1.38	230
L221A-Q		1.73	245
L221A-Q		2.08	250
L222A-Q	Rating	0	0
L222A-Q		1.38	245.1
L222A-Q		1.73	269.6
L222A-Q		2.08	274.5
S1-S	Rating	0	0
S1-S		2	292
S1-S		2.2	453
S1-S		2.4	498
S2-S	Rating	0	0
S2-S		2	327
S2-S		2.2	495
S2-S		2.4	544
S3-S	Rating	0	0
S3-S		2	296
S3-S		2.2	457
S3-S		2.4	502
BLOCK11	Storage	0	0.36
BLOCK11		1.4	0.36
BLOCK11		1.75	320
BLOCK16	Storage	0	0.36
BLOCK16		1.4	0.36

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BLOCK16		1.75	230
BLOCK19	Storage	0	0.36
BLOCK19		1.4	0.36
BLOCK19		1.75	250
BLOCK20	Storage	0	0.36
BLOCK20		1.4	0.36
BLOCK20		1.75	515
BLOCK25	Storage	0	0.36
BLOCK25		1.4	0.36
BLOCK25		1.75	350
C202A-V	Storage	0	0
C202A-V		1.38	0
C202A-V		1.73	933
C202A-V		1.731	0
C202A-V		2.08	0
C300A-V	Storage	0	0
C300A-V		1.38	0
C300A-V		1.39	1
C300A-V		1.4	0
C300A-V		1.73	0
C302A-V	Storage	0	0
C302A-V		1.38	0
C302A-V		1.39	1
C302A-V		1.4	0
C302A-V		1.73	0
L201A-V	Storage	0	0
L201A-V		1.38	0
L201A-V		1.73	88
L201A-V		1.731	0
L201A-V		2.08	0
L201C-V	Storage	0	0
L201C-V		1.38	0
L201C-V		1.73	44
L201C-V		1.731	0
L201C-V		2.08	0
L203A-V	Storage	0	0
L203A-V		1.38	0
L203A-V		1.73	78
L203A-V		1.731	0


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L203A-V          2.08      0
L204AA-V        Storage  0          0
L204AA-V          1.38      0
L204AA-V          1.73     116
L204AA-V          1.731     0
L204AA-V          2.08      0
L204A-V          Storage  0          0
L204A-V          1.38      0
L204A-V          1.73     1700
L204A-V          1.731     0
L204A-V          2.08      0
L205AA-V         Storage  0          0
L205AA-V          1.38      0
L205AA-V          1.73     693
L205A-V          Storage  0          0
L205A-V          1.38      0
L205A-V          1.73     1130
L205A-V          1.731     0
L205A-V          2.08      0
L205C-V          Storage  0          0
L205C-V          1.38      0
L205C-V          1.73     180
L205C-V          1.731     0
L205C-V          2.08      0
L215A-V          Storage  0          0
L215A-V          1.38      0
L215A-V          1.73     326
L215A-V          1.731     0
L215A-V          2.08      0
L216A-V          Storage  0          0
L216A-V          2.3        0
L216A-V          2.6       1000
L216A-V          2.601     0
L216A-V          2.9        0
L219B-V          Storage  0          0
L219B-V          1.38      0
L219B-V          1.73     304
L219B-V          1.731     0
L219B-V          2.08      0

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L221A-V          Storage  0          0
L221A-V          1.38      0
L221A-V          1.73     360
L221A-V          1.731     0
L221A-V          2.08      0
L400A-V          Storage  0          0
L400A-V          1.38      0
L400A-V          1.39      1
L400A-V          1.4        0
L400A-V          1.73      0
L402A-V          Storage  0          0
L402A-V          1.38      0
L402A-V          1.39     4700
L402A-V          1.4        0
L402A-V          1.73      0

```

```

[TIMESERIES]
;;Name          Date          Time          Value
;-----
;25mm - 4hr Chicago Design Storm - City of Ottawa IDF Data (1967-1997)
01-C25mm-4      0:00          0.00
01-C25mm-4      0:10          1.51
01-C25mm-4      0:20          1.75
01-C25mm-4      0:30          2.07
01-C25mm-4      0:40          2.58
01-C25mm-4      0:50          3.46
01-C25mm-4      1:00          5.39
01-C25mm-4      1:10          13.44
01-C25mm-4      1:20          56.67
01-C25mm-4      1:30          17.77
01-C25mm-4      1:40          9.12
01-C25mm-4      1:50          6.14
01-C25mm-4      2:00          4.65
01-C25mm-4      2:10          3.76
01-C25mm-4      2:20          3.17
01-C25mm-4      2:30          2.74
01-C25mm-4      2:40          2.43
01-C25mm-4      2:50          2.18
01-C25mm-4      3:00          1.98
01-C25mm-4      3:10          1.81
01-C25mm-4      3:20          1.68
01-C25mm-4      3:30          1.56
01-C25mm-4      3:40          1.47
01-C25mm-4      3:50          1.38
01-C25mm-4      4:00          1.31

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UNC-1	363857.006	5014032.4
UNC-1	363857.006	5014032.4
UNC-1	363852.804	5014037.995
UNC-1	363852.804	5014037.995
UNC-1	363843.736	5014032.93
UNC-1	363843.736	5014032.93
UNC-1	363834.667	5014027.864
UNC-1	363834.667	5014027.864
UNC-1	363838.491	5014019.091
UNC-1	363838.491	5014019.091
UNC-1	363831.471	5014015.06
UNC-1	363831.471	5014015.06
UNC-1	363826.734	5014023.308
UNC-1	363826.734	5014023.308
UNC-1	363803.345	5014009.875
UNC-1	363803.345	5014009.875
UNC-1	363808.518	5014000.868
UNC-1	363808.518	5014000.868
UNC-1	363802.072	5013997.166
UNC-1	363802.072	5013997.166
UNC-1	363796.899	5014006.173
UNC-1	363796.899	5014006.173
UNC-1	363773.51	5013992.741
UNC-1	363773.51	5013992.741
UNC-1	363776.103	5013988.226
UNC-1	363776.103	5013988.226
UNC-1	363764.209	5013981.528
UNC-1	363764.209	5013981.528
UNC-1	363762.222	5013985.39
UNC-1	363762.222	5013985.39
UNC-1	363752.985	5013980.639
UNC-1	363752.985	5013980.639
UNC-1	363752.985	5013980.639
UNC-1	363752.985	5013980.639
UNC-1	363743.748	5013975.887
UNC-1	363743.748	5013975.887
UNC-1	363750.733	5013962.31
UNC-1	363750.733	5013962.31
UNC-1	363746.283	5013960.021
UNC-1	363746.283	5013960.021
UNC-1	363743.995	5013964.428
UNC-1	363743.995	5013964.428
UNC-1	363739.473	5013973.14
UNC-1	363739.473	5013973.14
UNC-1	363741.52	5013980.076
UNC-2	363883.585	5014047.135
UNC-2	363879.867	5014053.793
UNC-2	363879.867	5014053.793

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UNC-2	363881.765	5014060.65
UNC-2	363881.765	5014060.65
UNC-2	363986.826	5014121.009
UNC-2	363986.826	5014121.009
UNC-2	364015.507	5014135.958
UNC-2	364015.507	5014135.958
UNC-2	364021.861	5014133.652
UNC-2	364021.861	5014133.652
UNC-2	364027.07	5014121.039
UNC-2	364027.07	5014121.039
UNC-2	364023.349	5014119.509
UNC-2	364023.349	5014119.509
UNC-2	364021.671	5014123.542
UNC-2	364021.671	5014123.542
UNC-2	364020.804	5014123.181
UNC-2	364020.804	5014123.181
UNC-2	364019.135	5014127.191
UNC-2	364019.135	5014127.191
UNC-2	364009.544	5014123.199
UNC-2	364009.544	5014123.199
UNC-2	363999.955	5014119.209
UNC-2	363999.955	5014119.209
UNC-2	364000.756	5014114.838
UNC-2	364000.756	5014114.838
UNC-2	364001.587	5014112.842
UNC-2	364001.587	5014112.842
UNC-2	363999.199	5014111.849
UNC-2	363999.199	5014111.849
UNC-2	363991.114	5014108.182
UNC-2	363991.114	5014108.182
UNC-2	363987.142	5014115.097
UNC-2	363987.142	5014115.097
UNC-2	363952.39	5014095.138
UNC-2	363952.39	5014095.138
UNC-2	363962.736	5014077.123
UNC-2	363962.736	5014077.123
UNC-2	363959.998	5014073.533
UNC-2	363959.998	5014073.533
UNC-2	363958.437	5014072.636
UNC-2	363958.437	5014072.636
UNC-2	363953.956	5014072.081
UNC-2	363953.956	5014072.081
UNC-2	363943.609	5014090.096
UNC-2	363943.609	5014090.096
UNC-2	363908.857	5014070.136
UNC-2	363908.857	5014070.136
UNC-2	363919.203	5014052.121
UNC-2	363919.203	5014052.121

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UNC-2	363914.369	5014047.327
UNC-2	363914.369	5014047.327
UNC-2	363912.798	5014046.45
UNC-2	363912.798	5014046.45
UNC-2	363900.452	5014065.023
UNC-2	363900.452	5014065.023
UNC-2	363891.383	5014059.958
UNC-2	363891.383	5014059.958
UNC-2	363882.314	5014054.892
UNC-2	363882.314	5014054.892
UNC-2	363883.612	5014050.643
UNC-2	363883.612	5014050.643
UNC-2	363885.098	5014047.981
UNC-2	363885.098	5014047.981
UNC-2	363883.585	5014047.135
UNC-3	364156.658	5014197.145
UNC-3	364163.863	5014180.004
UNC-3	364163.863	5014180.004
UNC-3	364161.003	5014178.76
UNC-3	364161.003	5014178.76
UNC-3	364155.936	5014190.813
UNC-3	364155.936	5014190.813
UNC-3	364153.764	5014189.914
UNC-3	364153.764	5014189.914
UNC-3	364152.685	5014192.521
UNC-3	364152.685	5014192.521
UNC-3	364150.263	5014193.524
UNC-3	364150.263	5014193.524
UNC-3	364080.679	5014164.705
UNC-3	364080.679	5014164.705
UNC-3	364074.599	5014162.187
UNC-3	364074.599	5014162.187
UNC-3	364081.162	5014146.342
UNC-3	364081.162	5014146.342
UNC-3	364062.664	5014138.681
UNC-3	364062.664	5014138.681
UNC-3	364056.773	5014152.904
UNC-3	364056.773	5014152.904
UNC-3	364047.484	5014149.057
UNC-3	364047.484	5014149.057
UNC-3	364047.112	5014149.953
UNC-3	364047.112	5014149.953
UNC-3	364043.773	5014148.57
UNC-3	364043.773	5014148.57
UNC-3	364042.877	5014146.961
UNC-3	364042.877	5014146.961
UNC-3	364041.09	5014142.033
UNC-3	364041.09	5014142.033

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UNC-3	364046.469	5014129.045
UNC-3	364046.469	5014129.045
UNC-3	364043.718	5014127.905
UNC-3	364043.718	5014127.905
UNC-3	364037.882	5014142.003
UNC-3	364037.882	5014142.003
UNC-3	364040.396	5014148.93
UNC-3	364040.396	5014148.93
UNC-3	364060.249	5014159.278
UNC-3	364060.249	5014159.278
UNC-3	364060.711	5014158.391
UNC-3	364060.711	5014158.391
UNC-3	364126.131	5014192.489
UNC-3	364126.131	5014192.489
UNC-3	364156.658	5014197.145

[SYMBOLS]

;;Gage X-Coord Y-Coord
 ;;-----

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.013)

160401085 - SNTC Lands Assessment
 Detailed Design Blocks 8 to 15, located north of the Jock River and west of
 Greenbank Road.

Element Count

Number of rain gages 1
 Number of subcatchments ... 44
 Number of nodes 120
 Number of links 184
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
Raingage	Chicago_5y_3h_10m_City	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
C208A	0.33	195.00	75.71	3.0000	Raingage
C208A-S					
C209A	0.25	142.80	61.43	5.0000	Raingage
C209A-S					
C210A	0.22	90.00	75.71	4.0000	Raingage
C210A-S					
C210B	0.25	104.00	72.86	4.0000	Raingage
C210B-S					
CM1	0.59	136.90	85.71	2.0000	Raingage
GREENBANK					
CM2	0.70	204.00	85.71	2.0000	Raingage

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LP

CM3	0.83	146.00	85.71	2.0000	Raingage
STREETC					
L100A	0.10	57.07	57.10	1.4000	Raingage
CB69-70					
L102A	0.23	96.20	70.00	5.5000	Raingage
CB39-40					
L102B	0.03	20.52	38.60	1.8000	Raingage
RYCB-01					
L102C	0.18	120.04	60.00	0.7000	Raingage
CB41-42					
L102D	0.11	31.40	41.40	1.5000	Raingage
RYT-01					
L104A	0.20	85.72	68.60	4.7000	Raingage
CB43-44					
L104B	0.20	41.29	31.40	2.0000	Raingage
RYT-05					
L106A	0.23	103.09	75.70	4.5000	Raingage
CB45-46					
L106B	0.08	28.14	35.70	1.6000	Raingage
RYT-11					
L108A	0.15	119.26	60.00	0.8000	Raingage
CB47-48					
L118A	0.14	150.60	60.00	2.8000	Raingage
CB49-50					
L201A	0.05	40.50	78.57	2.0000	Raingage
L201A-S					
L201B	0.06	47.60	92.86	6.0000	Raingage
L201B-S					
L201C	0.15	111.10	47.14	3.0000	Raingage
L201C-S					
L202A	0.35	142.30	68.57	4.5000	Raingage
L202A-S					
L203A	0.07	49.50	74.29	3.0000	Raingage
L203A-S					
L204A	1.56	742.80	71.43	2.0000	Raingage
L204A-S					
L205A	1.10	523.00	71.43	2.0000	Raingage
L205A-S					
L211A	0.05	35.50	90.00	5.0000	Raingage
L211A-S					
L211B	0.11	79.40	91.43	6.0000	Raingage
L211B-S					
L212A	0.25	119.00	72.86	3.5000	Raingage
L212A-S					
L213A	0.25	117.40	72.86	3.5000	Raingage
L213A-S					
L214A	0.25	178.50	62.86	4.5000	Raingage

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L214A-S					
L215A	1.15	547.00	71.43	1.0000	Raingage
L215A-S					
L216A	0.62	111.80	25.00	0.5000	Raingage
L216A-S					
L216B	0.24	143.70	50.00	2.5000	Raingage
L216A-S					
L218A	0.09	45.00	75.71	4.0000	Raingage
L218A-S					
L218B	0.04	21.00	74.29	4.0000	Raingage
L218B-S					
L219A	0.23	92.00	72.86	4.0000	Raingage
L219A-S					
L219B	1.06	506.00	71.43	1.0000	Raingage
L219B-S					
L219C	0.30	135.00	75.71	3.0000	Raingage
L219C-S					
L220A	0.07	36.00	90.00	2.0000	Raingage
L220A-S					
L221A	1.27	635.00	87.86	1.0000	Raingage
L221A-S					
L223A	0.04	29.40	95.71	4.5000	Raingage
L223A-S					
UNC-1	0.10	125.00	71.43	3.0000	Raingage
STREETC					
UNC-2	0.15	160.00	71.43	3.0000	Raingage
LP					
UNC-3	0.10	111.11	71.43	3.0000	Raingage
GREENBANK					

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
1+010	JUNCTION	93.47	0.35	0.0	
1+096	JUNCTION	93.79	0.35	0.0	
1+173	JUNCTION	93.61	0.35	0.0	
1+250	JUNCTION	93.79	0.35	0.0	
1+329	JUNCTION	93.57	0.36	0.0	
1+400	JUNCTION	93.76	0.35	0.0	
1+466	JUNCTION	94.97	0.35	0.0	
3+057	JUNCTION	93.71	0.35	0.0	
CB39-40	JUNCTION	91.49	2.13	0.0	
CB41-42	JUNCTION	91.75	2.04	0.0	
CB43-44	JUNCTION	91.72	2.03	0.0	

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CB45-46	JUNCTION	91.73	2.03	0.0
CB47-48	JUNCTION	91.79	2.03	0.0
CB49-50	JUNCTION	91.97	2.06	0.0
CB69-70	JUNCTION	91.52	2.39	0.0
HP-RYCB01	JUNCTION	93.75	0.30	0.0
HP-RYCB04	JUNCTION	93.83	0.30	0.0
HP-RYT01	JUNCTION	93.85	0.30	0.0
HP-RYT05	JUNCTION	93.85	0.30	0.0
HP-RYT11	JUNCTION	93.74	0.30	0.0
J4	JUNCTION	93.53	0.42	0.0
RYCB-72	JUNCTION	91.42	2.41	0.0
RYCB-73	JUNCTION	91.79	2.02	0.0
RYCB-76	JUNCTION	91.62	2.23	0.0
400	OUTFALL	89.70	1.42	0.0
CB09	OUTFALL	91.66	2.02	0.0
CB13-14	OUTFALL	91.62	2.03	0.0
CB37-38	OUTFALL	91.67	2.03	0.0
HW-02	OUTFALL	89.87	1.22	0.0
MAJ-OUT2	OUTFALL	93.40	0.43	0.0
10000	STORAGE	90.04	3.38	0.0
102	STORAGE	89.96	3.81	0.0
106	STORAGE	90.14	3.69	0.0
200	STORAGE	90.51	2.90	0.0
201	STORAGE	90.32	3.70	0.0
201A-S(2)	STORAGE	94.03	0.40	0.0
201B-S(1)	STORAGE	94.08	0.40	0.0
202	STORAGE	90.79	3.23	0.0
202A-S(1)	STORAGE	93.96	0.40	0.0
203	STORAGE	91.36	2.81	0.0
203A-S(1)	STORAGE	94.31	0.40	0.0
206	STORAGE	90.33	3.24	0.0
207	STORAGE	90.05	3.36	0.0
208	STORAGE	90.10	3.67	0.0
209	STORAGE	90.14	3.57	0.0
209A-S(1)	STORAGE	93.76	0.40	0.0
210	STORAGE	91.11	1.92	0.0
210A-S(1)	STORAGE	93.91	0.40	0.0
210B-S(1)	STORAGE	93.83	0.40	0.0
211	STORAGE	91.19	2.96	0.0
212	STORAGE	90.68	3.20	0.0
212A-S(1)	STORAGE	93.64	0.40	0.0
213	STORAGE	90.87	3.10	0.0
213A-S(1)	STORAGE	93.87	0.40	0.0
214	STORAGE	91.25	2.57	0.0
214A-S(1)	STORAGE	93.93	0.40	0.0
215A	STORAGE	91.31	3.09	0.0
216	STORAGE	91.13	2.31	0.0
217	STORAGE	90.08	3.22	0.0

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218	STORAGE	89.88	3.73	0.0
218A-S(1)	STORAGE	93.67	0.40	0.0
218B-S(1)	STORAGE	93.55	0.40	0.0
219	STORAGE	90.45	3.28	0.0
219A-S(1)	STORAGE	93.95	0.40	0.0
219B	STORAGE	91.18	3.12	0.0
219C-S(1)	STORAGE	93.60	0.40	0.0
220	STORAGE	91.18	2.70	0.0
221A	STORAGE	90.81	3.31	0.0
223	STORAGE	91.41	2.68	0.0
402	STORAGE	90.00	3.50	0.0
404	STORAGE	90.20	3.94	0.0
406	STORAGE	90.50	5.08	0.0
C208A-S	STORAGE	91.97	1.78	0.0
C209A-S	STORAGE	92.23	1.78	0.0
C210A-S	STORAGE	92.12	1.78	0.0
C210B-S	STORAGE	92.36	1.78	0.0
DICB204	STORAGE	91.27	2.95	0.0
DICB205	STORAGE	91.37	2.85	0.0
DICB215	STORAGE	91.28	2.88	0.0
DICB221	STORAGE	90.75	2.95	0.0
DICB222	STORAGE	90.94	2.94	0.0
GREENBANK	STORAGE	93.77	2.40	0.0
L201A-S	STORAGE	92.52	1.78	0.0
L201B-S	STORAGE	92.69	1.78	0.0
L201B-S(1)	STORAGE	94.12	0.40	0.0
L201C-S	STORAGE	92.03	1.78	0.0
L202A-S	STORAGE	92.51	1.78	0.0
L203A-S	STORAGE	92.41	1.78	0.0
L204A-S	STORAGE	92.62	2.08	0.0
L205A-S	STORAGE	92.88	2.08	0.0
L211A-S	STORAGE	92.68	1.78	0.0
L211B-S	STORAGE	92.32	1.78	0.0
L212A-S	STORAGE	92.12	1.78	0.0
L213A-S	STORAGE	92.35	1.78	0.0
L214A-S	STORAGE	92.37	1.78	0.0
L215A-S	STORAGE	92.67	2.08	0.0
L216A-S	STORAGE	91.43	2.90	0.0
L218A-S	STORAGE	92.06	1.78	0.0
L218B-S	STORAGE	91.92	1.78	0.0
L219A-S	STORAGE	92.21	1.78	0.0
L219B-S	STORAGE	92.57	2.08	0.0
L219C-S	STORAGE	92.09	1.78	0.0
L220A-S	STORAGE	92.38	1.78	0.0
L221A-S	STORAGE	92.39	2.08	0.0
L223A-S	STORAGE	92.71	1.78	0.0
LP	STORAGE	91.56	2.40	0.0
MH-100	STORAGE	90.10	3.34	0.0

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MH-104	STORAGE	90.32	3.28	0.0
MH-108	STORAGE	91.40	2.43	0.0
MH-110	STORAGE	91.47	3.04	0.0
MH-x10002	STORAGE	89.99	3.62	0.0
MH-x10004	STORAGE	89.97	3.58	0.0
MH-x10006	STORAGE	89.88	3.28	0.0
OF2	STORAGE	93.94	0.40	0.0
RYCB-01	STORAGE	92.00	1.95	0.0
RYT-01	STORAGE	91.55	2.47	0.0
RYT-05	STORAGE	91.92	2.08	0.0
RYT-11	STORAGE	91.70	2.18	0.0
STM-430	STORAGE	90.00	2.90	0.0
STREETC	STORAGE	91.90	2.40	0.0

Link Summary

Name	From Node	To Node	Type	Length

200-201	221A	DICB221	CONDUIT	166.0
0.1627 0.0130				
C1	CB69-70	MAJ-OUT2	CONDUIT	23.9
0.3352 0.0350				
C10	201A-S(2)	L214A-S	CONDUIT	26.0
1.0770 0.0130				
C11	203A-S(1)	L203A-S	CONDUIT	30.6
1.6985 0.0130				
C12	L203A-S	OF2	CONDUIT	2.0
-7.5212 0.0130				
C13	L214A-S	214A-S(1)	CONDUIT	26.2
-0.6876 0.0130				
C14	214A-S(1)	L213A-S	CONDUIT	34.1
0.5870 0.0130				
C15	L213A-S	213A-S(1)	CONDUIT	25.3
-0.5534 0.0130				
C16	213A-S(1)	L212A-S	CONDUIT	43.8
0.8456 0.0130				
C17	L212A-S	212A-S(1)	CONDUIT	13.5
-1.0371 0.0130				
C18	C210A-S	210A-S(1)	CONDUIT	53.0
-0.7736 0.0130				
C19	LP	C210A-S	CONDUIT	16.4
0.3659 0.0130				
C2	L219C-S	219C-S(1)	CONDUIT	27.0

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-0.4806	0.0130				
C20		C210B-S	210B-S(1)	CONDUIT	18.0
-0.5000	0.0130				
C21		212A-S(1)	C209A-S	CONDUIT	3.5
0.8572	0.0130				
C22		C209A-S	209A-S(1)	CONDUIT	14.8
-1.0136	0.0130				
C23		209A-S(1)	C208A-S	CONDUIT	41.3
0.9928	0.0130				
C24		C208A-S	1+173	CONDUIT	42.6
-0.6103	0.0130				
C25		OF2	LP	CONDUIT	15.0
2.5341	0.0130				
C26		202A-S(1)	L214A-S	CONDUIT	21.2
0.9906	0.0130				
C27		L223A-S	L211B-S	CONDUIT	76.8
0.5078	0.0130				
C28		L211B-S	209A-S(1)	CONDUIT	4.0
-1.5002	0.0130				
C29		L211A-S	C208A-S	CONDUIT	112.0
0.6339	0.0130				
C3		215A	DICB215	CONDUIT	160.0
0.1944	0.0130				
C30		L201B-S(1)	L201A-S	CONDUIT	30.4
0.7237	0.0130				
C32		219A-S(1)	L220A-S	CONDUIT	12.9
1.4730	0.0130				
C33		L220A-S	STREETC	CONDUIT	14.5
-0.9656	0.0130				
C34		219A-S(1)	L219A-S	CONDUIT	71.4
0.5042	0.0130				
C35		L219A-S	218A-S(1)	CONDUIT	14.5
-0.5517	0.0130				
C36		218A-S(1)	L219C-S	CONDUIT	63.3
0.3160	0.0130				
C37		L218A-S	218B-S(1)	CONDUIT	15.6
-0.7051	0.0130				
C38		218B-S(1)	L218B-S	CONDUIT	14.4
1.7364	0.0130				
C39		L218B-S	1+010	CONDUIT	10.0
-1.7002	0.0130				
C4		L205A-S	L202A-S	CONDUIT	51.0
1.4119	0.0130				
C40		L219B-S	L219A-S	CONDUIT	142.0
0.5000	0.0130				
C41		219C-S(1)	L218A-S	CONDUIT	27.9
0.5735	0.0130				
C42		217	10000	CONDUIT	12.9

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0.0775	0.0130				
C43		210A-S(1)	C210B-S	CONDUIT	34.3
0.4956	0.0130				
C44		210B-S(1)	212A-S(1)	CONDUIT	42.7
0.4450	0.0130				
C45		L201B-S	201B-S(1)	CONDUIT	3.0
-0.3333	0.0130				
C46		201B-S(1)	201A-S(2)	CONDUIT	23.0
0.2174	0.0130				
C47		L201C-S	J4	CONDUIT	18.7
-0.6417	0.0130				
C5		L201B-S(1)	L201C-S	CONDUIT	56.9
1.2479	0.0130				
C6		L201C-S	1+329	CONDUIT	23.1
-0.8677	0.0130				
C7		203A-S(1)	L202A-S	CONDUIT	84.2
0.4988	0.0130				
C8		219B	DICB222	CONDUIT	149.0
0.1597	0.0130				
C9		L201A-S	201A-S(2)	CONDUIT	4.3
-3.0603	0.0130				
CM1		406	404	CONDUIT	175.0
0.1029	0.0130				
CM2		404	402	CONDUIT	140.0
0.1000	0.0130				
CM3		402	400	CONDUIT	150.0
0.1000	0.0130				
CM4		STREETC	LP	CONDUIT	68.0
0.5000	0.0130				
CM6		GREENBANK	LP	CONDUIT	235.0
0.9382	0.0130				
L201B-W		L221A-S	L218A-S	CONDUIT	135.0
0.5037	0.0130				
L203C-W		L216A-S	C208A-S	CONDUIT	10.0
3.5021	0.0250				
L204B-M		L215A-S	L214A-S	CONDUIT	131.0
0.4962	0.0130				
L205B-W		L202A-S	202A-S(1)	CONDUIT	10.2
-0.6863	0.0130				
L205D-W		L204A-S	L201A-S	CONDUIT	51.0
0.8824	0.0130				
MH10000-MH10002		10000	MH-x10002	CONDUIT	9.8
0.2041	0.0130				
MH10002-STM430		MH-x10002	STM-430	CONDUIT	5.0
0.2000	0.0130				
MH10004-MH10006		MH-x10004	MH-x10006	CONDUIT	40.3
0.1489	0.0130				
MH10006-STM469		MH-x10006	HW-02	CONDUIT	7.3

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0.1370	0.0130					
MH100-MH10000	MH-100	10000	CONDUIT	22.2		
0.1351	0.0130					
MH102-MH100	102	MH-100	CONDUIT	142.6		
0.1122	0.0130					
MH104-MH102	MH-104	102	CONDUIT	50.4		
0.1190	0.0130					
MH106-MH104	106	MH-104	CONDUIT	105.8		
0.1134	0.0130					
MH108-MH106	MH-108	106	CONDUIT	85.4		
0.5152	0.0130					
MH110-MH108	MH-110	MH-108	CONDUIT	37.4		
0.1872	0.0130					
OVF-HP-RYCB01	HP-RYCB01	CB39-40	CONDUIT	3.0		
16.2088	0.0350					
OVF-HP-RYCB04	HP-RYCB04	RYT-01	CONDUIT	3.0		
3.6691	0.0350					
OVF-HP-RYT01	HP-RYT01	CB41-42	CONDUIT	3.0		
13.7961	0.0350					
OVF-HP-RYT05	HP-RYT05	CB43-44	CONDUIT	3.0		
15.1717	0.0350					
OVF-HP-RYT11	HP-RYT11	CB47-48	CONDUIT	3.0		
9.0367	0.0350					
OVF-RYCB01	HP-RYCB01	RYCB-01	CONDUIT	3.0		
3.3352	0.0350					
OVF-RYT01	HP-RYT01	RYT-01	CONDUIT	3.0		
4.3374	0.0350					
OVF-RYT05	HP-RYT05	RYT-05	CONDUIT	3.0		
5.0063	0.0350					
OVF-RYT11	HP-RYT11	RYT-11	CONDUIT	3.0		
5.3409	0.0350					
Pipe_28	201	200	CONDUIT	106.0		
0.1000	0.0130					
Pipe_29	202	201	CONDUIT	22.8		
0.1010	0.0130					
Pipe_30	203	202	CONDUIT	88.4		
0.2998	0.0130					
Pipe_31	DICB204	201	CONDUIT	16.5		
0.2966	0.0130					
Pipe_32	DICB205	202	CONDUIT	15.8		
0.2977	0.0130					
Pipe_33	207	206	CONDUIT	21.6		
0.1017	0.0130					
Pipe_34	208	207	CONDUIT	49.3		
0.0994	0.0130					
Pipe_35	209	208	CONDUIT	34.5		
0.0987	0.0130					
Pipe_36	210	209	CONDUIT	111.5		

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0.2000	0.0130					
Pipe_37	211	208	CONDUIT	88.0		
0.2998	0.0130					
Pipe_37_(1)	223	211	CONDUIT	69.2		
0.3006	0.0130					
Pipe_38	212	209	CONDUIT	60.6		
0.1500	0.0130					
Pipe_39	213	212	CONDUIT	64.4		
0.1707	0.0130					
Pipe_40	214	213	CONDUIT	26.6		
0.3006	0.0130					
Pipe_41	DICB215	213	CONDUIT	12.1		
0.2985	0.0130					
Pipe_42	216	207	CONDUIT	18.0		
0.9959	0.0130					
Pipe_43	218	217	CONDUIT	67.1		
0.0999	0.0130					
Pipe_44	219	218	CONDUIT	77.6		
0.1494	0.0130					
Pipe_45	220	219	CONDUIT	71.5		
0.5004	0.0130					
Pipe_46	DICB221	218	CONDUIT	15.2		
0.4984	0.0130					
Pipe_47	DICB222	219	CONDUIT	14.0		
0.5154	0.0130					
Pipe_58	206	102	CONDUIT	12.0		
0.1170	0.0130					
Pipe_59	200	106	CONDUIT	12.3		
0.1059	0.0130					
RYT01-RYCB72	RYT-01	RYCB-72	CONDUIT	14.0		
0.9286	0.0130					
RYT02-RYCB73	RYT-05	RYCB-73	CONDUIT	14.9		
0.8725	0.0130					
RYT11-RYCB76	RYT-11	RYCB-76	CONDUIT	9.5		
0.8421	0.0130					
ST01-01	1+010	CB69-70	CONDUIT	5.2		
2.8858	0.0160					
ST01-02	1+010	CB39-40	CONDUIT	21.2		
0.9434	0.0160					
ST01-03	1+096	CB39-40	CONDUIT	59.9		
0.8681	0.0160					
ST01-04	1+096	CB41-42	CONDUIT	51.3		
0.6823	0.0160					
ST01-05	1+173	CB41-42	CONDUIT	25.4		
0.6693	0.0160					
ST01-06	1+173	CB43-44	CONDUIT	28.7		
0.7317	0.0160					
ST01-07	1+250	CB43-44	CONDUIT	50.4		

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0.7738	0.0160				
ST01-08		1+250	CB45-46	CONDUIT	50.2
0.7570	0.0160				
ST01-09_1		J4	1+329	CONDUIT	9.9
-0.4049	0.0160				
ST01-09_2		J4	CB45-46	CONDUIT	18.3
0.6714	0.0160				
ST01-10		1+329	CB47-48	CONDUIT	28.4
0.3521	0.0160				
ST01-11		1+400	CB47-48	CONDUIT	46.6
0.6223	0.0160				
ST01-12		1+400	CB49-50	CONDUIT	18.3
0.4372	0.0160				
ST01-13		1+466	CB49-50	CONDUIT	46.4
2.7812	0.0160				
ST03-01		3+057	CB69-70	CONDUIT	25.7
1.5177	0.0160				
ST05-01		1+329	CB13-14	CONDUIT	32.0
0.8438	0.0160				
ST06-01		1+173	CB09	CONDUIT	35.3
0.7932	0.0100				
ST07-01		1+400	CB37-38	CONDUIT	36.9
1.1112	0.0160				
STM431-MH10004		STM-430	MH-x10004	CONDUIT	5.0
0.2000	0.0130				
C208A-IC1	C208A-S	208		ORIFICE	
C208A-IC2	C208A-S	208		ORIFICE	
C209A-IC1	C209A-S	209		ORIFICE	
C209A-IC2	C209A-S	209		ORIFICE	
C210A-IC1	C210A-S	210		ORIFICE	
C210A-IC2	C210A-S	210		ORIFICE	
C210B-IC1	C210B-S	210		ORIFICE	
C210B-IC2	C210B-S	210		ORIFICE	
L201A-IC	L201A-S	201		ORIFICE	
L201B-IC1	L201B-S	201		ORIFICE	
L201C-IC1	L201C-S	201		ORIFICE	
L201C-IC2	L201C-S	201		ORIFICE	
L202A-IC1	L202A-S	202		ORIFICE	
L202A-IC2	L202A-S	202		ORIFICE	
L203A-IC1	L203A-S	203		ORIFICE	
L203A-IC2	L203A-S	203		ORIFICE	
L211A-IC	L211A-S	211		ORIFICE	
L211B-IC	L211B-S	211		ORIFICE	
L212A-IC1	L212A-S	212		ORIFICE	
L212A-IC2	L212A-S	212		ORIFICE	
L213A-IC1	L213A-S	213		ORIFICE	
L213A-IC2	L213A-S	213		ORIFICE	
L214A-IC1	L214A-S	214		ORIFICE	

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L214A-IC2	L214A-S	214	ORIFICE
L216A-IC1	L216A-S	216	ORIFICE
L218A-IC1	L218A-S	218	ORIFICE
L218A-IC2	L218A-S	218	ORIFICE
L218B-IC1	L218B-S	218	ORIFICE
L218B-IC2	L218B-S	218	ORIFICE
L219A-IC1	L219A-S	219	ORIFICE
L219A-IC2	L219A-S	219	ORIFICE
L219C-IC1	L219C-S	219	ORIFICE
L219C-IC2	L219C-S	219	ORIFICE
L220A-IC1	L220A-S	220	ORIFICE
L220A-IC2	L220A-S	220	ORIFICE
L223A-IC	L223A-S	223	ORIFICE
OCB-39	CB39-40	MH-100	ORIFICE
OCB-40	CB39-40	MH-100	ORIFICE
OCB-41	CB41-42	102	ORIFICE
OCB-42	CB41-42	102	ORIFICE
OCB-43	CB43-44	MH-104	ORIFICE
OCB-44	CB43-44	MH-104	ORIFICE
OCB-45	CB45-46	106	ORIFICE
OCB-46	CB45-46	106	ORIFICE
OCB-47	CB47-48	MH-108	ORIFICE
OCB-48	CB47-48	MH-108	ORIFICE
OCB-49	CB49-50	MH-110	ORIFICE
OCB-50	CB49-50	MH-110	ORIFICE
OCB-69	CB69-70	10000	ORIFICE
OCB-70	CB69-70	10000	ORIFICE
ORYCB-01	RYCB-01	102	ORIFICE
ORYCB-72	RYCB-72	102	ORIFICE
ORYCB-73	RYCB-73	MH-104	ORIFICE
ORYCB-76	RYCB-76	MH-108	ORIFICE
MH10002-MH10004	MH-x10002	MH-x10004	WEIR
CM-1	GREENBANK	406	OUTLET
CM-2	LP	404	OUTLET
CM-3	STREETC	402	OUTLET
L204A-0	L204A-S	DICB204	OUTLET
L205A-0	L205A-S	DICB205	OUTLET
L215A-0	L215A-S	215A	OUTLET
L219B-0	L219B-S	219B	OUTLET
L221A-0	L221A-S	221A	OUTLET

Cross Section Summary

Full	Full	Hyd.	Max.	No. of
Conduit	Shape	Rad.	Width	Barrels
		Depth	Area	

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Flow						

200-201	CIRCULAR	0.60	0.28	0.15	0.60	1
247.65						
C1	TRIANGULAR	0.30	0.27	0.14	1.80	1
121.75						
C10	Half-18mROW	0.40	2.35	0.18	12.50	1
6023.64						
C11	18mROW	0.35	4.42	0.15	28.50	1
12487.10						
C12	18mROW	0.35	4.42	0.15	28.50	1
26276.91						
C13	Street-E-18m-ROW	0.28	2.45	0.15	18.00	1
4405.97						
C14	Street-E-18m-ROW	0.28	2.45	0.15	18.00	1
4071.16						
C15	Street-E-18m-ROW	0.28	2.45	0.15	18.00	1
3952.68						
C16	18mROW	0.35	4.42	0.15	28.50	1
8810.51						
C17	18mROW	0.35	4.42	0.15	28.50	1
9757.53						
C18	Jockvale	0.35	4.86	0.18	29.40	1
10563.86						
C19	Jockvale	0.35	4.86	0.18	29.40	1
7264.70						
C2	Street-C-mount-14m-ROW	0.23	2.29	0.12	18.80	
1 2944.07						
C20	Jockvale	0.35	4.86	0.18	29.40	1
8492.78						
C21	Jockvale	0.35	4.86	0.18	29.40	1
11119.80						
C22	Jockvale	0.35	4.86	0.18	29.40	1
12091.72						
C23	Jockvale	0.35	4.86	0.18	29.40	1
11967.13						
C24	Jockvale	0.35	4.86	0.18	29.40	1
9383.14						
C25	Street-A-18mROW	0.34	3.93	0.16	23.80	1
14467.20						
C26	Half-18mROW	0.40	2.35	0.18	12.50	1
5777.05						
C27	Half-LaneD-8.5mROW	0.10	0.22	0.05	4.25	1
160.30						
C28	Half-LaneD-8.5mROW	0.10	0.22	0.05	4.25	1
275.51						

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C29	Half-LaneD-8.5mROW	0.10	0.22	0.05	4.25	1
179.10						
C3	CIRCULAR	0.60	0.28	0.15	0.60	1
270.72						
C30	Half-18mROW	0.40	2.35	0.18	12.50	1
4937.81						
C32	Street-C-14m-ROW	0.23	1.65	0.13	14.00	1
3926.52						
C33	Street-C-14m-ROW	0.23	1.65	0.13	14.00	1
3179.02						
C34	Street-C-14m-ROW	0.23	1.65	0.13	14.00	1
2297.25						
C35	Street-C-14m-ROW	0.23	1.65	0.13	14.00	1
2403.07						
C36	Street-C-mount-14m-ROW	0.23	2.29	0.12	18.80	
1 2387.11						
C37	Street-C-mount-14m-ROW	0.23	2.29	0.12	18.80	
1 3566.13						
C38	Street-C-mount-14m-ROW	0.23	2.29	0.12	18.80	
1 5596.02						
C39	18mROW	0.35	4.42	0.15	28.50	1
12493.58						
C4	18mROW	0.35	4.42	0.15	28.50	1
11385.02						
C40	8.5m-ROW	0.35	3.57	0.12	25.50	1
4767.52						
C41	Street-C-mount-14m-ROW	0.23	2.29	0.12	18.80	
1 3216.03						
C42	CIRCULAR	1.20	1.13	0.30	1.20	1
1085.56						
C43	Jockvale	0.35	4.86	0.18	29.40	1
8455.56						
C44	Jockvale	0.35	4.86	0.18	29.40	1
8011.75						
C45	Half-LaneD-8.5mROW	0.10	0.22	0.05	4.25	1
129.87						
C46	Half-18mROW	0.40	2.35	0.18	12.50	1
2706.30						
C47	Street-A-18mROW	0.34	3.93	0.16	23.80	1
7280.19						
C5	Street-A-18mROW	0.34	3.93	0.16	23.80	1
10152.16						
C6	18mROW	0.35	4.42	0.15	28.50	1
8925.22						
C7	Street-A-18mROW	0.34	3.93	0.16	23.80	1
6418.59						
C8	CIRCULAR	0.60	0.28	0.15	0.60	1
245.41						

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C9	Half-18mROW	0.40	2.35	0.18	12.50	1
10153.89						
CM1	CIRCULAR	1.05	0.87	0.26	1.05	1
875.83						
CM2	CIRCULAR	1.05	0.87	0.26	1.05	1
863.58						
CM3	HORIZ_ELLIPSE	1.09	1.54	0.34	1.73	1
1817.19						
CM4	ChapmanMills	0.29	4.44	0.09	41.00	1
5020.25						
CM6	ChapmanMills	0.29	4.44	0.09	41.00	1
6876.77						
L201B-W	8.5m-ROW	0.35	3.57	0.12	25.50	1
4785.15						
L203C-W	TRIANGULAR	0.30	0.27	0.14	1.80	1
550.92						
L204B-M	8.5m-ROW	0.35	3.57	0.12	25.50	1
4749.29						
L205B-W	Street-A-18mROW	0.34	3.93	0.16	23.80	1
7528.74						
L205D-W	8.5m-ROW	0.35	3.57	0.12	25.50	1
6333.37						
MH10000-MH10002	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1
3397.04						
MH10002-STM430	CIRCULAR	1.22	1.17	0.30	1.22	1
1818.27						
MH10004-MH10006	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1
2901.49						
MH10006-STM469	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1
2783.15						
MH100-MH10000	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1
2764.28						
MH102-MH100	HORIZ_ELLIPSE	1.22	1.89	0.37	1.93	1
2518.83						
MH104-MH102	CIRCULAR	1.22	1.17	0.30	1.22	1
1402.82						
MH106-MH104	CIRCULAR	1.22	1.17	0.30	1.22	1
1369.27						
MH108-MH106	CIRCULAR	0.69	0.37	0.17	0.69	1
629.99						
MH110-MH108	CIRCULAR	0.69	0.37	0.17	0.69	1
379.70						
OVF-HP-RYCB01	TRIANGULAR	0.30	0.27	0.14	1.80	1
846.59						
OVF-HP-RYCB04	TRIANGULAR	0.30	0.27	0.14	1.80	1
402.79						
OVF-HP-RYT01	TRIANGULAR	0.30	0.27	0.14	1.80	1
781.04						

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OVF-HP-RYT05	TRIANGULAR	0.30	0.27	0.14	1.80	1
819.06						
OVF-HP-RYT11	TRIANGULAR	0.30	0.27	0.14	1.80	1
632.12						
OVF-RYCB01	TRIANGULAR	0.30	0.27	0.14	1.80	1
384.02						
OVF-RYT01	TRIANGULAR	0.30	0.27	0.14	1.80	1
437.94						
OVF-RYT05	TRIANGULAR	0.30	0.27	0.14	1.80	1
470.49						
OVF-RYT11	TRIANGULAR	0.30	0.27	0.14	1.80	1
485.97						
Pipe_28	CIRCULAR	1.20	1.13	0.30	1.20	1
1233.03						
Pipe_29	CIRCULAR	0.75	0.44	0.19	0.75	1
353.88						
Pipe_30	CIRCULAR	0.45	0.16	0.11	0.45	1
156.11						
Pipe_31	CIRCULAR	0.60	0.28	0.15	0.60	1
334.43						
Pipe_32	CIRCULAR	0.53	0.22	0.13	0.53	1
234.66						
Pipe_33	CIRCULAR	1.20	1.13	0.30	1.20	1
1243.46						
Pipe_34	CIRCULAR	1.20	1.13	0.30	1.20	1
1229.30						
Pipe_35	CIRCULAR	1.20	1.13	0.30	1.20	1
1224.78						
Pipe_36	CIRCULAR	0.45	0.16	0.11	0.45	1
127.50						
Pipe_37	CIRCULAR	0.38	0.11	0.09	0.38	1
96.01						
Pipe_37_(1)	CIRCULAR	0.38	0.11	0.09	0.38	1
96.14						
Pipe_38	CIRCULAR	0.75	0.44	0.19	0.75	1
431.26						
Pipe_39	CIRCULAR	0.68	0.36	0.17	0.68	1
347.34						
Pipe_40	CIRCULAR	0.38	0.11	0.09	0.38	1
96.14						
Pipe_41	CIRCULAR	0.60	0.28	0.15	0.60	1
335.48						
Pipe_42	CIRCULAR	0.30	0.07	0.07	0.30	1
96.51						
Pipe_43	CIRCULAR	1.20	1.13	0.30	1.20	1
1232.23						
Pipe_44	CIRCULAR	0.75	0.44	0.19	0.75	1
430.35						

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Pipe_45	CIRCULAR	0.38	0.11	0.09	0.38	1
124.03						
Pipe_46	CIRCULAR	0.68	0.36	0.17	0.68	1
593.49						
Pipe_47	CIRCULAR	0.60	0.28	0.15	0.60	1
440.82						
Pipe_58	CIRCULAR	1.20	1.13	0.30	1.20	1
1333.75						
Pipe_59	CIRCULAR	1.20	1.13	0.30	1.20	1
1269.11						
RYT01-RYCB72	CIRCULAR	0.25	0.05	0.06	0.25	1
57.31						
RYT02-RYCB73	CIRCULAR	0.25	0.05	0.06	0.25	1
55.55						
RYT11-RYCB76	CIRCULAR	0.25	0.05	0.06	0.25	1
54.58						
ST01-01	8.5m_ROW	0.35	3.76	0.18	20.50	1
12745.74						
ST01-02	8.5m_ROW	0.35	3.76	0.18	20.50	1
7287.65						
ST01-03	8.5m_ROW	0.35	3.76	0.18	20.50	1
6990.80						
ST01-04	8.5m_ROW	0.35	3.76	0.18	20.50	1
6197.42						
ST01-05	8.5m_ROW	0.35	3.76	0.18	20.50	1
6138.23						
ST01-06	8.5m_ROW	0.35	3.76	0.18	20.50	1
6418.08						
ST01-07	8.5m_ROW	0.35	3.76	0.18	20.50	1
6600.16						
ST01-08	8.5m_ROW	0.35	3.76	0.18	20.50	1
6527.95						
ST01-09_1	8.5m_ROW	0.35	3.76	0.18	20.50	1
4774.02						
ST01-09_2	8.5m_ROW	0.35	3.76	0.18	20.50	1
6147.88						
ST01-10	8.5m_ROW	0.35	3.76	0.18	20.50	1
4452.18						
ST01-11	8.5m_ROW	0.35	3.76	0.18	20.50	1
5918.90						
ST01-12	8.5m_ROW	0.35	3.76	0.18	20.50	1
4960.81						
ST01-13	8.5m_ROW	0.35	3.76	0.18	20.50	1
12512.68						
ST03-01	8.5m_ROW	0.35	3.76	0.18	20.50	1
9243.18						
ST05-01	8.5m_ROW	0.35	3.76	0.18	20.50	1
6892.00						

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ST06-01	9.5m_ROW	0.35	4.00	0.18	21.50	1
11475.53						
ST07-01	8.5m_ROW	0.35	3.76	0.18	20.50	1
7909.02						
STM431-MH10004	CIRCULAR	1.22	1.17	0.30	1.22	1
1818.27						

Transect Summary						

Transect 18mROW						
Area:						
	0.0004	0.0014	0.0033	0.0058	0.0091	
	0.0130	0.0177	0.0232	0.0293	0.0362	
	0.0438	0.0522	0.0612	0.0710	0.0815	
	0.0927	0.1047	0.1174	0.1307	0.1441	
	0.1576	0.1712	0.1859	0.2016	0.2185	
	0.2364	0.2555	0.2757	0.2970	0.3194	
	0.3429	0.3675	0.3932	0.4201	0.4480	
	0.4770	0.5072	0.5385	0.5708	0.6043	
	0.6389	0.6746	0.7114	0.7493	0.7883	
	0.8284	0.8697	0.9120	0.9554	1.0000	
Hrad:						
	0.0226	0.0452	0.0679	0.0905	0.1131	
	0.1357	0.1583	0.1810	0.2036	0.2262	
	0.2488	0.2715	0.2941	0.3167	0.3393	
	0.3619	0.3846	0.4072	0.4392	0.4836	
	0.5279	0.5717	0.6114	0.6466	0.6779	
	0.7058	0.7307	0.7531	0.7733	0.7916	
	0.8083	0.8236	0.8378	0.8510	0.8633	
	0.8748	0.8858	0.8962	0.9062	0.9158	
	0.9251	0.9341	0.9428	0.9514	0.9598	
	0.9680	0.9762	0.9842	0.9921	1.0000	
Width:						
	0.0161	0.0321	0.0482	0.0642	0.0803	
	0.0964	0.1124	0.1285	0.1445	0.1606	
	0.1767	0.1927	0.2088	0.2248	0.2409	
	0.2570	0.2730	0.2891	0.2982	0.2982	
	0.2982	0.3123	0.3368	0.3614	0.3860	
	0.4105	0.4351	0.4596	0.4842	0.5088	
	0.5333	0.5579	0.5825	0.6070	0.6316	
	0.6561	0.6807	0.7053	0.7298	0.7544	
	0.7789	0.8035	0.8281	0.8526	0.8772	
	0.9018	0.9263	0.9509	0.9754	1.0000	

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CM4	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
CM6	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
L201B-W	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
L203C-W	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
L204B-M	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
L205B-W	1.00	0.04	0.01	0.00	0.01	0.00	0.00	0.95	0.00
0.00									
L205D-W	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
MH1000-MH1002	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH1002-STM430	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH1004-MH1006	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH1006-STM469	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH100-MH1000	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH102-MH100	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH104-MH102	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH106-MH104	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH108-MH106	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
MH110-MH108	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
OVF-HP-RYCB01	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-HP-RYCB04	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-HP-RYT01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-HP-RYT05	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-HP-RYT11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-RYCB01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-RYT01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									

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OVF-RYT05	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
OVF-RYT11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
Pipe_28	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_29	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_30	1.00	0.00	0.58	0.00	0.42	0.00	0.00	0.00	0.99
0.00									
Pipe_31	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_32	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_33	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_34	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_35	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_36	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_37	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_37_(1)	1.00	0.00	0.60	0.00	0.40	0.00	0.00	0.00	0.99
0.00									
Pipe_38	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_39	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_40	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.07
0.00									
Pipe_41	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_42	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_43	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_44	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_45	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_46	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_47	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
Pipe_58	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									

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Pipe_59	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
RYT01-RYCB72	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									
RYT02-RYCB73	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.04
0.00									
RYT11-RYCB76	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.97
0.00									
ST01-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-02	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-03	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-04	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-05	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-06	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-07	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-08	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-09_1	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-09_2	1.00	0.99	0.01	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-10	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-11	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-12	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST01-13	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST03-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST05-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST06-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
ST07-01	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00									
STM431-MH10004	1.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
0.00									

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 Conduit Surchage Summary

Conduit	Hours Full			Hours	Hours
	Both Ends	Upstream	Dnstream	Above Normal Flow	Capacity Limited
200-201	0.38	0.38	24.00	0.01	0.23
C3	0.17	0.17	0.30	0.01	0.01
C42	24.00	24.00	24.00	0.01	0.20
C8	0.31	0.31	24.00	0.01	0.01
CM1	24.00	24.00	24.00	0.01	0.01
CM2	24.00	24.00	24.00	0.01	0.01
CM3	24.00	24.00	24.00	0.01	0.01
MH10000-MH10002	24.00	24.00	24.00	0.01	21.64
MH10002-STM430	24.00	24.00	24.00	0.01	1.48
MH10004-MH10006	24.00	24.00	24.00	0.01	0.20
MH10006-STM469	24.00	24.00	24.00	0.01	0.50
MH100-MH10000	24.00	24.00	24.00	0.01	0.09
MH102-MH100	24.00	24.00	24.00	0.01	0.01
MH104-MH102	24.00	24.00	24.00	0.01	0.01
MH106-MH104	0.39	0.39	24.00	0.01	0.01
MH108-MH106	0.01	0.01	0.40	0.01	0.01
Pipe_28	0.28	0.28	0.36	0.01	0.01
Pipe_29	0.28	0.28	0.28	0.01	0.01
Pipe_30	0.01	0.01	0.28	0.01	0.01
Pipe_31	0.27	0.27	0.28	0.01	0.01
Pipe_32	0.27	0.28	0.28	0.01	0.27
Pipe_33	24.00	24.00	24.00	0.01	0.01
Pipe_34	0.82	0.82	24.00	0.01	0.01
Pipe_35	0.39	0.39	0.78	0.01	0.01
Pipe_36	0.28	0.28	0.39	0.01	0.01
Pipe_37	0.26	0.26	0.82	0.01	0.01
Pipe_37_(1)	0.01	0.01	0.20	0.01	0.01
Pipe_38	0.34	0.34	0.39	0.01	0.05
Pipe_39	0.31	0.31	0.34	0.01	0.01
Pipe_40	0.28	0.28	0.31	0.01	0.01
Pipe_41	0.30	0.30	0.31	0.01	0.15
Pipe_42	0.40	0.40	24.00	0.01	0.01
Pipe_43	24.00	24.00	24.00	0.01	0.01
Pipe_44	24.00	24.00	24.00	0.01	0.01
Pipe_45	0.24	0.24	24.00	0.01	0.01
Pipe_46	24.00	24.00	24.00	0.01	0.01
Pipe_47	24.00	24.00	24.00	0.01	0.01
Pipe_58	24.00	24.00	24.00	0.01	0.01
Pipe_59	0.36	0.36	0.36	0.01	0.01

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RYT01-RYCB72	0.35	0.35	0.45	0.01	0.01
RYT02-RYCB73	0.20	0.20	0.27	0.01	0.01
RYT11-RYCB76	0.30	0.30	0.34	0.01	0.01
STM431-MH10004	24.00	24.00	24.00	0.01	7.48

Analysis begun on: Thu Aug 6 10:09:29 2020

Analysis ended on: Thu Aug 6 10:09:43 2020

Total elapsed time: 00:00:14

STM MH	Prop. Grade (m)	USF (m)	Worst-Case 100-Year HGL			
			100-year Runoff HGL (m)	5-year Runoff HGL (m)	Worst-Case HGL (m)	Prop. Grade/USF-HGL Clearance (m)
200	93.41	N/A	91.54	92.02	92.02	1.39
201	94.02	N/A	91.56	92.04	92.04	1.98
202	94.02	N/A	91.59	92.05	92.05	1.97
203	94.17	N/A	91.8	92.06	92.06	2.11
DICB204	94.22	N/A	91.64	92.07	92.07	2.15
DICB205	94.22	N/A	91.8	92.12	92.12	2.1
206	93.57	N/A	91.44	91.94	91.94	1.63
207	93.41	N/A	91.45	91.95	91.95	1.46
208	93.78	92.45	91.46	91.96	91.96	0.49
209	93.72	92.48	91.47	91.96	91.96	0.52
210	93.04	N/A	91.81	92.13	92.13	0.91
211	94.15	92.45	91.73	92.02	92.02	0.43
212	93.89	92.48	91.59	92.04	92.04	0.44
213	93.96	92.61	91.67	92.12	92.12	0.49
214	93.82	92.61	91.74	92.13	92.13	0.48
DICB215	94.16	N/A	91.74	92.16	92.16	2
216	93.44	N/A	91.69	92.09	92.09	1.35
217	93.3	92.85	91.32	91.85	91.85	1
218	93.61	92.24	91.34	91.86	91.86	0.38
219	93.73	N/A	91.4	91.91	91.91	1.82
220	93.88	N/A	91.61	91.91	91.91	1.97
DICB221	93.7	N/A	91.38	91.9	91.9	1.8
DICB222	93.88	N/A	91.41	91.92	91.92	1.96
223	94.09	92.79	91.84	92.02	92.02	0.77
215A	94.4	N/A	91.97	92.31	92.31	2.09
219B	94.3	N/A	91.57	92.04	92.04	2.26
221A	94.12	N/A	91.64	92.19	92.19	1.93
Claridge Burnett Lands Subdivision – Novatech June 2020 Submission						
102	93.76	N/A	91.44	91.94	91.94	1.82
MH-100	93.44	92.21	91.36	91.88	91.88	0.33
MH-104	93.6	92.31	91.5	91.99	91.99	0.32
106	93.83	92.31	91.54	92.02	92.02	0.29

MH-108	93.83	N/A	91.58	92.03	92.03	1.8
MH-110	94.51	N/A	91.63	92.04	92.04	2.47

SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix C Stormwater Management Calculations

C.5 BACKGROUND REPORT EXCERPTS



Table 5.12: V x D Summary

Storage Node ID	Drainage Area ID	Conduit 100-yr V (m/s)	Conduit Depth (m)	VxD (m²/s)
C208A-S	C208A	0.16	0.14	0.02
C209A-S	C209A	0.53	0.17	0.09
C210A-S	C210A	0.24	0.08	0.02
C210B-S	C210B	0.18	0.13	0.02
L201A-S	L201A	0.64	0.10	0.06
L201B-S	L201B	0.46	0.07	0.03
L201C-S	L201C	0.26	0.11	0.03
L202A-S	L202A	0.17	0.15	0.03
L203A-S	L203A	0.00	0.00	0.00
L211A-S	L211A	0.05	0.03	0.00
L211B-S	L211B	0.32	0.12	0.04
L212A-S	L212A	0.15	0.27	0.04
L213A-S	L213A	0.18	0.21	0.04
L214A-S	L214A	0.16	0.26	0.04
L218A-S	L218A	0.24	0.20	0.05
L218B-S	L218B	0.77	0.23	0.18
L219A-S	L219A	0.33	0.19	0.06
L219C-S	L219C	0.19	0.20	0.04
L220A-S	L220A	0.19	0.21	0.04
L223A-S	L223A	0.01	0.01	0.00

5.3.4 Future Development Blocks SWM Criteria

Table 5.13 below presents the parameters for the outlet link objects in the model, which represent the minor system capture rate for the future private blocks within the Caivan SNTC Development.

Table 5.13: SWM Criteria of Future Blocks

Storm Drainage Area	Description	Minor System Outlet	100-Year Minor System Capture (L/s)	Major System Flow Direction	100-Year Major System Overflows (L/s)
L204A	Block6-School	STM201	259.0	100-yr on-site storage. Emergency overland to Street A.	N/A
L205A	Block3-HD-RES	STM202	184.1	100-yr on-site storage. Emergency overland to Street A.	N/A



Stormwater Management

Storm Drainage Area	Description	Minor System Outlet	100-Year Minor System Capture (L/s)	Major System Flow Direction	100-Year Major System Overflows (L/s)
L215A	Block2-B2BTH	STM213	194.1	50m ³ /ha on-site storage. Major Flow to Street E.	328
L219B	Block1-RL-TH	STM219	179.1	50m ³ /ha on-site storage. Major Flow to Street C.	309
L221A	Block4-RL-TH	STM218	214.6	50m ³ /ha on-site storage. Major Flow to Street C.	369
L216A	Block5-Park	STM207	74.2	100-yr on-site storage. Emergency overland to Jockvale/Street A.	N/A

5.4 QUALITY CONTROL

Street 1 is a shared road between the Claridge Burnett Lands Development and the Caivan SNTC Lands Development which will be constructed as a part of the Burnett Lands Development. Approximately 1.59 ha of the Burnett Lands development and all of the Caivan Lands will drain to Street 1, the K-B SWMF outlet channel, and ultimately to the Fraser Clarke Drain that discharges into the Jock River.

Storm runoff from the proposed Caivan SNTC Lands Development and Street 1 will be directed to the outlet channel for the Kennedy-Burnett SWMF by a shared storm sewer. An Enhanced (80% TSS removal) level of water quality control will be provided by using a hydrodynamic separator (HDS) (i.e. Vortechs units or approved equivalent) upstream of the storm outfall as designed by Novatech in their *Stormwater Management Report Burnett Lands – 3370 Greenbank Road* submitted in June 2020 and summarized as follows.

- A total drainage area of 12.34 ha at 70% imperviousness was used to size the HDS unit as shown in excerpts from Novatech's SWM Report for Burnett Lands included in **Appendix C.5**.
- Storm runoff from Street 1 and the proposed Caivan SNTC Development will be treated by an Off-line Vortechs Model PC1421 unit located upstream of the outfall to the Kennedy-Burnett SWM Facility outlet channel as shown on **Drawing SD-1**.

5.5 INTERIM CONDITION EXTERNAL DRAINAGE MANAGEMENT

The existing Burnett Municipal Drain, which is tributary to the Jock River bisects the site from north to south. The drain consists primarily of an open channel between the Barrhaven Town Centre





SNTC SUBDIVISION
 DATE: 2020-07-10
 REVISION: 1
 DESIGNED BY: MJS
 CHECKED BY: AMP

STORM SEWER DESIGN SHEET
 (City of Ottawa)
 FILE NUMBER: 160401085

DESIGN PARAMETERS
 $I = a / (t+b)^c$ (As per City of Ottawa Guidelines, 2012)
 a = 732.951, 998.071, 1174.184, 1735.688
 b = 6.199, 6.053, 6.014, 6.014
 c = 0.810, 0.814, 0.816, 0.820
 MANNING'S n = 0.013
 MINIMUM COVER: 2.00 m
 TIME OF ENTRY: 10 min
 BEDDING CLASS = B

LOCATION			DRAINAGE AREA																PIPE SELECTION																				
AREA ID NUMBER	FROM M.H.	TO M.H.	AREA (2-YEAR)	AREA (5-YEAR)	AREA (10-YEAR)	AREA (100-YEAR)	AREA (ROOF)	C (2-YEAR)	C (5-YEAR)	C (10-YEAR)	C (100-YEAR)	A x C (2-YEAR)	ACCUM (2YR)	A x C (5-YEAR)	ACCUM (5YR)	A x C (10YR)	ACCUM (10YR)	A x C (100YR)	ACCUM (100YR)	T of C (min)	I ₂ YEAR (mm/h)	I ₅ YEAR (mm/h)	I ₁₀ YEAR (mm/h)	I ₁₀₀ YEAR (mm/h)	Q _{CONTROL} (L/s)	ACCUM. Q _{CONTROL} (L/s)	Q _{ACT} (CIA/360) (L/s)	LENGTH (m)	PIPE WIDTH OR DIAMETER (mm)	PIPE HEIGHT (mm)	PIPE SHAPE (-)	MATERIAL (-)	CLASS (-)	SLOPE (%)	Q _{CAP} (FULL) (L/s)	% FULL (-)	VEL. (FULL) (m/s)	VEL. (ACT) (m/s)	TIME OF FLOW (min)
L110A (Claridge)	110	108	0.00	0.14	0.00	0.00	0.00	0.62	0.00	0.00	0.00	0.000	0.000	0.087	0.087	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	25.1	37.4	675	675	CIRCULAR	CONCRETE	-	0.20	392.2	6.41%	1.06	0.50	1.25
L108A, L106B (Claridge)	108	106	0.00	0.23	0.00	0.00	0.00	0.59	0.00	0.00	0.00	0.000	0.000	0.135	0.222	0.000	0.000	0.000	0.000	11.25	72.31	98.01	114.86	167.87	0.0	0.0	60.4	84.4	675	675	CIRCULAR	CONCRETE	-	0.51	626.3	9.64%	1.70	0.88	1.59
L203A	203	202	0.07	0.00	0.00	0.00	0.00	0.72	0.00	0.00	0.00	0.047	0.047	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	10.1	88.4	450	450	CIRCULAR	CONCRETE	-	0.30	162.9	6.19%	0.99	0.45	3.26
L205A	TEMP 205	202	1.11	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	0.777	0.777	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	165.8	15.8	525	525	CIRCULAR	CONCRETE	-	0.30	245.7	67.46%	1.10	1.03	0.26
L202A	202	201	0.35	0.00	0.00	0.00	0.00	0.68	0.00	0.00	0.00	0.241	1.065	0.000	0.000	0.000	0.000	0.000	0.000	13.26	66.20	89.62	104.99	153.37	0.0	0.0	195.9	22.8	750	750	CIRCULAR	CONCRETE	-	0.10	367.3	53.32%	0.81	0.70	0.54
L204A	TEMP 204	201	1.56	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	1.092	1.092	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	233.0	16.5	600	600	CIRCULAR	CONCRETE	-	0.30	350.8	66.40%	1.20	1.12	0.25
L201A, L201C, L201B	201	106	0.27	0.00	0.00	0.00	0.00	0.65	0.00	0.00	0.00	0.173	2.330	0.000	0.000	0.000	0.000	0.000	0.000	13.80	64.74	87.63	102.65	149.93	0.0	0.0	419.0	118.3	1200	1200	CIRCULAR	CONCRETE	-	0.10	1294.0	32.38%	1.11	0.83	2.37
L106A (Claridge)	106	104	0.00	0.23	0.00	0.00	0.00	0.00	0.73	0.00	0.00	0.000	2.330	0.168	0.390	0.000	0.000	0.000	0.000	16.18	59.13	79.94	93.60	136.66	0.0	0.0	469.2	106.8	1200	1200	CIRCULAR	CONCRETE	-	0.11	1363.4	34.42%	1.17	0.89	2.01
L104A, L104B (Claridge)	104	102	0.00	0.40	0.00	0.00	0.00	0.00	0.55	0.00	0.00	0.000	2.330	0.220	0.610	0.000	0.000	0.000	0.000	18.18	55.15	74.51	87.21	127.29	0.0	0.0	483.1	50.4	1200	1200	CIRCULAR	CONCRETE	-	0.12	1403.7	34.42%	1.20	0.91	0.92
C210A, C210B	210	209	0.00	0.47	0.00	0.00	0.00	0.72	0.00	0.00	0.00	0.000	0.000	0.337	0.337	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	97.7	111.5	450	450	CIRCULAR	CONCRETE	-	0.20	133.0	73.43%	0.81	0.78	2.39
L214A	214	213	0.25	0.00	0.00	0.00	0.00	0.64	0.00	0.00	0.00	0.157	0.157	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	33.6	26.6	375	375	CIRCULAR	PVC	-	0.30	90.3	37.19%	0.86	0.67	0.67
L215A	TEMP 215	213	1.15	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	0.805	0.805	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	171.7	12.1	600	600	CIRCULAR	CONCRETE	-	0.30	350.8	48.95%	1.20	1.02	0.20
L213A	213	212	0.25	0.00	0.00	0.00	0.00	0.71	0.00	0.00	0.00	0.174	1.136	0.000	0.000	0.000	0.000	0.000	0.000	10.67	74.34	100.80	118.15	172.69	0.0	0.0	234.7	64.4	675	675	CIRCULAR	CONCRETE	-	0.17	361.6	64.90%	0.98	0.90	1.19
L212A	212	209	0.25	0.00	0.00	0.00	0.00	0.71	0.00	0.00	0.00	0.175	1.311	0.000	0.000	0.000	0.000	0.000	0.000	11.86	70.35	95.32	111.69	163.21	0.0	0.0	256.2	60.6	750	750	CIRCULAR	CONCRETE	-	0.15	449.8	56.95%	0.99	0.87	1.16
C209A	209	208	0.00	0.25	0.00	0.00	0.00	0.63	0.00	0.00	0.00	0.000	1.311	0.158	0.496	0.000	0.000	0.000	0.000	13.01	66.90	90.58	106.12	155.03	0.0	0.0	368.4	34.5	1200	1200	CIRCULAR	CONCRETE	-	0.10	1286.2	28.64%	1.10	0.79	0.72
L223A	223	211	0.04	0.00	0.00	0.00	0.00	0.87	0.00	0.00	0.00	0.034	0.034	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	7.3	69.2	375	375	CIRCULAR	PVC	-	0.30	90.3	8.10%	0.86	0.42	2.72
L211B, L211A	211	208	0.15	0.00	0.00	0.00	0.00	0.84	0.00	0.00	0.00	0.129	0.163	0.000	0.000	0.000	0.000	0.000	0.000	12.72	67.74	91.74	107.48	157.03	0.0	0.0	30.7	88.0	375	375	CIRCULAR	PVC	-	0.30	90.3	34.01%	0.86	0.65	2.26
C208A	208	207	0.00	0.33	0.00	0.00	0.00	0.73	0.00	0.00	0.00	0.000	1.474	0.242	0.738	0.000	0.000	0.000	0.000	14.97	61.83	83.64	97.95	143.03	0.0	0.0	424.6	49.3	1200	1200	CIRCULAR	CONCRETE	-	0.10	1286.2	33.01%	1.10	0.83	0.99
L216B, L216A	216	207	0.86	0.00	0.00	0.00	0.00	0.59	0.00	0.00	0.00	0.504	0.504	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	107.5	18.0	375	375	CIRCULAR	PVC	-	1.00	164.8	65.21%	1.56	1.45	0.21
L102C, L102A, L102B, L102D (Claridge)	102	100	0.54	0.00	0.00	0.00	0.00	0.62	0.00	0.00	0.00	0.337	4.645	0.000	1.348	0.000	0.000	0.000	0.000	19.10	53.52	72.28	84.60	123.45	0.0	0.0	961.1	142.6	1920	1220	ELLIPTICAL	CONCRETE	-	0.11	2434.5	39.48%	1.32	1.05	2.26
L220A	220	219	0.07	0.00	0.00	0.00	0.00	0.83	0.00	0.00	0.00	0.062	0.062	0.000	0.000	0.000	0.000	0.000	0.000	11.98	76.81	104.19	122.14	178.56	0.0	0.0	13.2	71.5	375	375	CIRCULAR	PVC	-	0.50	116.6	11.32%	1.11	0.60	1.98
L219B	TEMP 222	219	1.06	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	0.742	0.742	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	158.3	14.0	600	600	CIRCULAR	CONCRETE	-	0.30	350.8	45.12%	1.20	1.00	0.23
L219C, L219A	219	218	0.53	0.00	0.00	0.00	0.00	0.73	0.00	0.00	0.00	0.389	1.193	0.000	0.000	0.000	0.000	0.000	0.000	11.98	69.97	94.80	111.08	162.32	0.0	0.0	231.8	77.6	750	750	CIRCULAR	CONCRETE	-	0.15	449.8	51.53%	0.99	0.85	1.52
L221A	TEMP 221	218	1.27	0.00	0.00	0.00	0.00	0.70	0.00	0.00	0.00	0.889	0.889	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	189.7	15.2	675	675	CIRCULAR	CONCRETE	-	0.30	480.3	39.49%	1.30	1.03	0.25
L218B, L218A	218	10000	0.13	0.00	0.00	0.00	0.00	0.73	0.00	0.00	0.00	0.096	2.177	0.000	0.000	0.000	0.000	0.000	0.000	13.50	65.56	88.76	103.97	151.87	0.0	0.0	396.6	80.0	1200	1200	CIRCULAR	CONCRETE	-	0.10	1286.2	30.83%	1.10	0.82	1.64
L100A (Claridge)	10000	10002	0.10	0.00	0.00	0.00	0.00	0.60	0.00	0.00	0.00	0.060	6.882	0.000	1.348	0.000	0.000	0.000	0.000	21.69	49.46	66.74	78.08	113.90	0.0	0.0	1195.3	9.8	1920	1220	ELLIPTICAL	CONCRETE	-	0.16	2907.0	41.12%	1.58	1.27	0.13

Summary of Subcatchment Parameters - Proposed Development Areas

Area ID	Area (ha)	Width (m)	Slope (%)	%IMP	Runoff Coefficient	Subarea Routing	% Routed		Minor System Capture (L/s)	STORAGE- (cu.m)
C208A	0.33	195.0	3.0	75.7%	0.73	IMPERVIOUS	100	Jockvale	82.3	-
C209A	0.25	142.8	5.0	61.4%	0.63	IMPERVIOUS	100	Jockvale	55.2	-
C210A	0.22	90.0	4.0	75.7%	0.73	IMPERVIOUS	100	Jockvale	52.9	-
C210B	0.25	104.0	4.0	72.9%	0.71	IMPERVIOUS	100	Jockvale	59.4	-
L100A	0.10	57.1	1.4	57.1%	0.60	IMPERVIOUS	100	CLARIDGE-RD	12.2	-
L102A	0.23	96.2	5.5	70.0%	0.69	IMPERVIOUS	100	CLARIDGE-RD	34.4	-
L102B	0.03	20.5	1.8	38.6%	0.47	PERVIOUS	100	CLARIDGE-RY	2.8	-
L102C	0.18	120.0	0.7	60.0%	0.62	IMPERVIOUS	100	CLARIDGE-RD	23.4	-
L102D	0.11	31.4	1.5	41.4%	0.49	PERVIOUS	100	CLARIDGE-RY	5.9	-
L104A	0.20	85.7	4.7	68.6%	0.68	IMPERVIOUS	100	CLARIDGE-RD	29.3	-
L104B	0.20	41.3	2.0	31.4%	0.42	PERVIOUS	100	CLARIDGE-RY	5.4	-
L106A	0.23	103.1	4.5	75.7%	0.73	IMPERVIOUS	100	CLARIDGE-RD	37.2	-
L106B	0.08	28.1	1.6	35.7%	0.45	PERVIOUS	100	CLARIDGE-RY	3.9	-
L108A	0.15	119.3	0.8	60.0%	0.62	IMPERVIOUS	100	CLARIDGE-RD	19.2	-
L118A	0.14	150.6	2.8	60.0%	0.62	IMPERVIOUS	100	CLARIDGE-RD	17.9	-
L201A	0.05	40.5	2.0	78.6%	0.75	IMPERVIOUS	100	Street-A	9.1	-
L201B	0.06	47.6	6.0	92.9%	0.85	IMPERVIOUS	100	Lane-D	11.9	-
L201C	0.15	111.1	3.0	47.1%	0.53	IMPERVIOUS	100	Street-A	15.4	-
L202A	0.35	142.3	4.5	68.6%	0.68	IMPERVIOUS	100	Street-A	51.9	-
L203A	0.07	49.5	3.0	74.3%	0.72	IMPERVIOUS	100	Street-A	10.4	-
L204A	1.56	742.8	2.0	71.4%	0.70	IMPERVIOUS	100	Block6-School	237.3	250
L205A	1.10	523.0	2.0	71.4%	0.70	IMPERVIOUS	100	Block3-HD-RES	168.1	177
L211A	0.05	35.5	5.0	90.0%	0.83	IMPERVIOUS	100	Lane-D	9.2	-
L211B	0.11	79.4	6.0	91.4%	0.84	IMPERVIOUS	100	Lane-D	20.8	-
L212A	0.25	119.0	3.5	72.9%	0.71	IMPERVIOUS	100	Street-E	38.2	-
L213A	0.25	117.4	3.5	72.9%	0.71	IMPERVIOUS	100	Street-E	38.1	-
L214A	0.25	178.5	4.5	62.9%	0.64	IMPERVIOUS	100	Street-E	33.0	-
L215A	1.15	547.0	1.0	71.4%	0.70	IMPERVIOUS	100	Block2-B2BTH	175.1	57
L216A	0.62	111.8	0.5	25.0%	0.38	IMPERVIOUS	100	Block5-Park	65.7	104
L216B	0.24	143.7	2.5	50.0%	0.55	PERVIOUS	100	Block-7	32.6	-
L218A	0.09	45.0	4.0	75.7%	0.73	IMPERVIOUS	100	Street-C	14.1	-
L218B	0.04	21.0	4.0	74.3%	0.72	IMPERVIOUS	100	Street-C	7.1	-
L219A	0.23	92.0	4.0	72.9%	0.71	IMPERVIOUS	100	Street-C	36.0	-
L219B	1.06	506.0	1.0	71.4%	0.70	IMPERVIOUS	100	Block1-RL-TH	161.8	53
L219C	0.30	135.0	3.0	75.7%	0.73	IMPERVIOUS	100	Street-C	48.6	-
L220A	0.07	36.0	2.0	90.0%	0.83	IMPERVIOUS	100	StreetC	14.3	-
L221A	1.27	635.0	1.0	71.4%	0.70	IMPERVIOUS	100	Block4-RL-TH	193.8	64
L223A	0.04	29.4	4.5	95.7%	0.87	IMPERVIOUS	100	Lane-D	8.1	-
UNC-1	0.10	125.0	3.0	71.4%	0.70	IMPERVIOUS	100	UNC-CM	0.0	-
UNC-2	0.15	160.0	3.0	71.4%	0.70	IMPERVIOUS	100	UNC-CM	0.0	-
UNC-3	0.10	111.1	3.0	71.4%	0.70	IMPERVIOUS	100	UNC-CM	0.0	-
12.06 ha - Total Sewershed Area				67.1%	0.67					
10.76 ha - Total Proposed Site				68.6%	0.68					

Total Surface Flow Depth Summary

Storage node ID	Drainage Area	Top of Grate Elevation (m)	Lowest Adjacent Building Opening (m)	Rim Elevation (m)	2 year, 3 hour Chicago		5 year, 3 hour Chicago		100 year, 3 hour Chicago		100 year, 3 hour Chicago +20%	
					Max Surface HGL (m)	Total Surface Ponding Depth (m)	Max Surface HGL (m)	Total Surface Ponding Depth (m)	Max Surface HGL (m)	Total Surface Ponding Depth (m)	Max Surface HGL (m)	Total Surface Ponding Depth (m)
C208A-S	C208A	93.35	93.95	93.75	92.78	0.00	93.37	0.02	93.49	0.14	93.66	0.31
C209A-S	C209A	93.61	93.98	94.01	92.63	0.00	93.11	0.00	93.78	0.17	93.86	0.25
C210A-S	C210A	93.61	N/A	93.90	92.90	0.00	93.52	0.00	93.69	0.08	93.75	0.14
C210B-S	C210B	93.74	N/A	94.14	92.82	0.00	93.29	0.00	93.87	0.13	93.89	0.15
L201A-S	L201A	93.90	N/A	94.30	93.00	0.00	93.46	0.00	94.00	0.10	94.14	0.24
L201B-S	L201B	94.07	94.26	94.47	93.43	0.00	94.03	0.00	94.14	0.07	94.14	0.07
L201C-S	L201C	93.41	N/A	93.81	92.35	0.00	92.83	0.00	93.52	0.11	93.59	0.18
L202A-S	L202A	93.89	N/A	94.29	93.88	0.00	93.96	0.07	94.04	0.15	94.08	0.19
L203A-S	L203A	93.79	N/A	94.19	92.61	0.00	92.81	0.00	93.61	0.00	93.84	0.05
L211A-S	L211A	94.06	94.15	94.46	93.15	0.00	93.54	0.00	94.09	0.03	94.09	0.03
L211B-S	L211B	93.70	93.95	94.10	93.67	0.00	93.77	0.07	93.82	0.12	93.84	0.14
L212A-S	L212A	93.50	93.87	93.90	93.39	0.00	93.59	0.09	93.77	0.27	93.86	0.36
L213A-S	L213A	93.73	94.00	94.13	93.61	0.00	93.82	0.09	93.94	0.21	94.00	0.27
L214A-S	L214A	93.75	94.07	94.15	93.48	0.00	93.80	0.05	94.01	0.26	94.07	0.32
L218A-S	L218A	93.44	93.80	93.84	92.28	0.00	92.49	0.00	93.64	0.20	93.67	0.23
L218B-S	L218B	93.30	94.05	93.70	92.03	0.00	92.12	0.00	93.53	0.23	93.67	0.37
L219A-S	L219A	93.59	93.81	93.99	93.32	0.00	93.63	0.04	93.78	0.19	93.81	0.22
L219C-S	L219C	93.47	94.00	93.87	93.26	0.00	93.54	0.07	93.67	0.20	93.71	0.24
L220A-S	L220A	93.76	N/A	94.16	92.59	0.00	92.74	0.00	93.97	0.21	93.99	0.23
L223A-S	L223A	94.09	94.31	94.49	93.07	0.00	93.34	0.00	94.10	0.01	94.12	0.03
CB69-70	L100A	93.32	93.90	93.67	91.78	0.00	92.08	0.00	93.52	0.20	93.68	0.36
CB39-40	L102A	93.27	93.90	93.62	93.14	0.00	93.35	0.08	93.50	0.23	93.69	0.42
CB41-42	L102C	93.44	94.04	93.79	92.52	0.00	93.43	0.00	93.58	0.14	93.63	0.19
CB43-44	L104A	93.40	94.15	93.75	92.94	0.00	93.45	0.05	93.58	0.18	93.63	0.23
CB45-46	L106A	93.41	94.05	93.76	93.43	0.02	93.51	0.10	93.59	0.18	93.60	0.19
CB47-48	L108A	93.47	94.09	93.82	92.35	0.00	93.04	0.00	93.58	0.11	93.61	0.14
CB49-50	L118A	93.68	N/A	94.03	92.57	0.00	93.35	0.00	93.79	0.11	93.81	0.13

**SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER
MANAGEMENT REPORT**

Appendix D Geotechnical Investigation

Appendix D **GEOTECHNICAL INVESTIGATION**



Geotechnical
Engineering

Environmental
Engineering

Hydrogeology

Geological
Engineering

Materials Testing

Building Science

Archaeological Studies

Geotechnical Investigation

Proposed Residential Development
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March 6, 2019

Report: PG2743-2

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Appendices

Appendix 1 Soil Profile and Test Data Sheets

 Symbols and Terms

 Record of Borehole by Others

 Consolidation Test Sheets

 Atterberg Test Results Sheets

Appendix 2 Figure 1 - Key Plan

 Drawing PG2743-2 - Permissible Grade Raise Areas - Housing

 Drawing PG2743-3 - Permissible Grade Raise Areas - Apartment Buildings

 Drawing PG2743-4 - Test Hole Location Plan

1.0 Introduction

Paterson Group (Paterson) was commissioned by Caivan Communities to conduct a geotechnical investigation for the proposed residential development to be located at 3288 Greenbank Road, in the City of Ottawa (refer to Figure 1 - Key Plan presented in Appendix 2).

The objective of the investigation was to:

- determine the subsoil and groundwater conditions at this site by means of test holes.
- provide geotechnical recommendations for the design of the proposed development including construction considerations which may affect the design.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. The report contains our findings and includes geotechnical recommendations pertaining to the design and construction of the proposed development as understood at the time of this report.

Investigating the presence or potential presence of contamination on the proposed development was not part of the scope of work. Therefore, the present report does not address environmental issues.

2.0 Proposed Development

It is understood that the proposed development consists of townhouse style housing blocks and multi-storey apartment buildings. Local roadways, car parking and landscaped areas are further anticipated for the proposed development.

3.0 Method of Investigation

3.1 Field Investigation

The field program for our investigation was carried out in February 2019 and October 2012. As part of our investigations, eleven (11) boreholes and 8 test pits were completed across the subject site extending to a maximum 10 m depth. The test hole location was placed in a manner to provide general coverage of the subject site taking into account existing test holes completed by others. The test hole locations are illustrated on Drawing PG2743-4 - Test Hole Location Plan presented in Appendix 2.

The boreholes were completed using a track-mounted auger drill rig operated by a two person crew. All fieldwork was conducted under the full-time supervision of Paterson personnel under the direction of a senior engineer from the geotechnical division. The testing procedure consisted of augering to the required depths and at the selected locations sampling the overburden.

Sampling and In Situ Testing

Soil samples were collected from the borehole using a 50 mm diameter split-spoon (SS) sampler, using 73 mm diameter thin walled (TW) Shelby tubes in conjunction with a piston sampler, or from the auger flights. All soil samples were visually inspected and initially classified on site. The split-spoon samples were placed in sealed plastic bags and the Shelby tubes were sealed at both ends on site. All samples were transported to the our laboratory for examination and classification. The depths at which the split-spoon, Shelby tube and auger samples were recovered from the test holes are shown as SS, TW and AU, respectively, on the Soil Profile and Test Data sheets presented in Appendix 1.

The Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as “N” values on the Soil Profile and Test Data sheets. The “N” value is the number of blows required to drive the split-spoon sampler 300 mm into the soil after a 150 mm initial penetration using a 63.5 kg hammer falling from a height of 760 mm.

Undrained shear strength testing was conducted in cohesive soils using a field vane apparatus.

The overburden thickness was evaluated by a dynamic cone penetration testing (DCPT) at BH 6, BH 8 and BH 10. The DCPT consists of driving a steel drill rod, equipped with a 50 mm diameter cone at the tip, using a 63.5 kg hammer falling from a height of 760 mm. The number of blows required to drive the cone into the soil is recorded for each 300 mm increment.

The subsurface conditions observed at the test hole locations were recorded in detail in the field. The soil profiles are presented on the Soil Profile and Test Data sheets in Appendix 1 of this report.

Groundwater

Flexible standpipes were installed in the boreholes to monitor the groundwater level subsequent to the completion of the sampling program. The groundwater observations are noted on the Soil Profile and Test Data sheets presented in Appendix 1.

Sample Storage

All samples will be stored in the laboratory for a period of one month after issuance of the report. They will then be discarded unless we are otherwise directed.

3.2 Field Survey

The test holes were located in the field by JD Barnes. It is understood that the elevations are referenced to a geodetic datum. The ground surface elevation and location of the test holes are presented on Drawing PG2743-4 - Test Hole Location Plan in Appendix 2.

3.3 Laboratory Testing

The soil samples recovered from the field investigation were examined in our laboratory to review field notes and soil samples.

Seven (7) Shelby tube samples were submitted for unidimensional consolidation and one (1) sample submitted for Atterberg limit testing from our test holes completed during our investigation. Eight (8) additional soil samples were submitted for atterberg limit testing as part of our current investigation.

The results of the consolidation testing are presented on the Consolidation Test sheets presented in Appendix 1 and are further discussed in Sections 4 and 5.

4.0 Observations

4.1 Surface Conditions

Currently, the subject site consists of agricultural lands and associated farmhouse and outbuildings. The majority of the ground surface across the subject site is relatively flat and slopes gradually downwards to the south.

4.2 Subsurface Profile

Generally, the soil conditions encountered at the test hole locations consist of a cultivated topsoil/organic layer followed by a stiff, brown silty clay deposit overlying a glacial till layer. Practical refusal to augering or DCPT was encountered at BH 1, BH 6, BH 8 and BH 10 at depths varying between 8.2 and 14.8 m. It should be noted that BH 2 was terminated due to damage of drilling augers on dense till material at a 5.3 m depth. Reference should be made to the Soil Profile and Test Data sheets in Appendix 1 for specific details of the soil profiles encountered at each test hole location.

Based on available geological mapping, the bedrock in this area mostly consists of interbedded limestone and dolomite of the Gull River formation with an overburden drift thickness of 5 to 15 m depth.

4.3 Groundwater

Groundwater levels (GWL) were measured in the piezometers installed in the boreholes and results are summarized in Table 1. It should be noted that surface water can become perched within a backfilled borehole, which can lead to higher than normal groundwater level readings. The long-term groundwater level can also be estimated based on moisture levels and colour of the recovered soil samples. Based on these observations, the long-term groundwater table is expected between 1.5 to 2.5 m below original ground surface. It should be noted that groundwater levels are subject to seasonal fluctuations. Therefore, the groundwater levels could vary at the time of construction.

Table 1 - Summary of Groundwater Level Readings				
Borehole Number	Ground Surface Elevation (m)	Groundwater Level (m)	Groundwater Elevation (m)	Recording Date
BH 1	92.89	1.30	91.59	November 7, 2012
BH 2	92.37	1.08	91.29	November 7, 2012
BH 3	92.66	0.55	92.11	November 7, 2012
BH 4	92.81	1.76	91.05	November 7, 2012
BH 5	92.06	-	92.06	November 7, 2012
BH 6	92.19	1.08	91.11	November 7, 2012
BH 7	92.38	2.71	89.67	November 7, 2012
BH 8	92.88	1.35	91.53	November 7, 2012
BH 9	92.64	3.32	89.32	November 7, 2012
BH 10	92.40	1.63	90.77	November 7, 2012
BH 11	92.19	1.08	91.11	November 7, 2012

5.0 Discussion

5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is adequate for the proposed residential development. However, due to the presence of the sensitive silty clay layer, the proposed development will be subjected to grade raise restrictions.

Our permissible grade raise recommendations are discussed in Subsection 5.3 and the recommended permissible grade raise areas for housing are presented in Drawing PG2743-2 - Permissible Grade Raise Areas - Housing in Appendix 2. Also, the recommended permissible grade raise areas for apartment buildings are presented in Drawing PG2743-3 - Permissible Grade Raise Areas - Apartment Buildings in Appendix 2. If higher than permissible grade raises are required, preloading with or without a surcharge, lightweight fill and/or other measures should be investigated to reduce the risks of unacceptable long-term post construction total and differential settlements.

The above and other considerations are further discussed in the following sections.

5.2 Site Grading and Preparation

Stripping Depth

Topsoil and deleterious fill, such as those containing organic materials, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures.

Fill Placement

Fill used for grading beneath the building areas should consist, unless otherwise specified, of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. Granular material should be tested and approved prior to delivery to the site. The fill should be placed in lifts of 300 mm thick or less and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building areas should be compacted to at least 98% of the Standard Proctor Maximum Dry Density (SPMDD).

Non-specified existing fill along with site-excavated soil can be used as general landscaping fill and beneath parking areas where settlement of the ground surface is of minor concern. In landscaped areas, these materials should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If these materials are to be used to build up the subgrade level for areas to be paved, they should be compacted in thin lifts to a minimum density of 95% of the SPMDD. Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless a composite drainage blanket connected to a perimeter drainage system is provided.

5.3 Foundation Design

Bearing Resistance Values

Strip footings, up to 2.5 m wide, and pad footings, up to 5 m wide, placed on an undisturbed, stiff silty clay bearing surface can be designed using a bearing resistance value at SLS of **100 kPa** and a factored bearing resistance value at ULS of **200 kPa**. Footings designed using the bearing resistance value at SLS given above will be subjected to potential post construction total and differential settlements of 25 and 20 mm, respectively.

The bearing resistance values are provided on the assumption that the footings will be placed on undisturbed soil bearing surfaces. An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to the in-situ bearing medium soils above the groundwater table when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil of the same or higher capacity as the bearing medium soil.

Settlement/Grade Raise

Consideration must be given to potential settlements which could occur due to the presence of the silty clay deposit and the combined loads from the proposed footings, any groundwater lowering effects, and grade raise fill. The foundation loads to be considered for the settlement case are the continuously applied loads which consist of the unfactored dead loads and the portion of the unfactored live load that is considered to be continuously applied. For dwellings, a minimum value of 50% of the live load is recommended by Paterson.

Generally, the potential long term settlement is evaluated based on the compressibility characteristics of the silty clay. These characteristics are estimated in the laboratory by conducting unidimensional consolidation tests on undisturbed soil samples collected using Shelby tubes in conjunction with a piston sampler. Seven (7) site specific consolidation tests were conducted. The results of the consolidation tests are presented in Table 2 and in Appendix 1.

The value for p'_c is the preconsolidation pressure and p'_o is the effective overburden pressure of the test samples. The difference between these values is the available preconsolidation. The increase in stress on the soil due to the cumulative effects of the fill surcharge, the footing pressures, the slab loadings and the lowering of the groundwater should not exceed the available preconsolidation if unacceptable settlements are to be avoided.

The values for C_{cr} and C_c are the recompression and compression indices, respectively. These soil parameters are a measure of the compressibility due to stress increases below and above the preconsolidation pressures. The higher values for the C_c , as compared to the C_{cr} , illustrate the increased settlement potential above, as compared to below, the preconsolidation pressure.

Table 2 - Summary of Consolidation Test Results							
Borehole	Sample	Elevation	p'_c	p'_o	C_{cr}	C_c	Q
BH 3	TW 4	88.45	104	50	0.021	2.253	A
BH 4	TW 4	88.53	103	55	0.019	2.146	A
BH 6	TW 3	87.16	119	63	0.022	1.064	A
BH 7	TW 3	87.35	113	68	0.016	1.683	A
BH 8	TW 3	87.78	111	62	0.015	2.000	A
BH 11	TW 1	88.41	119	58	0.014	1.253	A
* - Q - Quality assessment of sample - G: Good A: Acceptable P: Likely disturbed							

The values of p'_c , p'_o , C_{cr} and C_c are determined using standard engineering testing procedures and are estimates only. Natural variations within the soil deposit will affect the results. The p'_o parameter is directly influenced by the groundwater level. Groundwater levels were measured during the site investigation. Groundwater levels vary seasonally which has an impact on the available preconsolidation. Lowering the groundwater level increases the p'_o and therefore reduces the available preconsolidation. Unacceptable settlements could be induced by a significant lowering of the groundwater level. The p'_o values for the consolidation tests during the investigation are based on the long term groundwater level being at 0.5 m below the existing groundwater table. The groundwater level is based on the colour and undrained shear strength profile of the silty clay.

The total and differential settlements will be dependent on characteristics of the proposed buildings. For design purposes, the total and differential settlements are estimated to be 25 and 20 mm, respectively. A post-development groundwater lowering of 0.5 m was assumed.

The potential post construction total and differential settlements are dependent on the position of the long term groundwater level when buildings are situated over deposits of compressible silty clay. Efforts can be made to reduce the impacts of the proposed development on the long term groundwater level by placing clay dykes in the service trenches, reducing the sizes of paved areas, leaving green spaces to allow for groundwater recharge or limiting planting of trees to areas away from the buildings. However, it is not economically possible to control the groundwater level.

To reduce potential long term liabilities, consideration should be given to accounting for a larger groundwater lowering and to provide means to reduce long term groundwater lowering (e.g. clay dykes, restriction on planting around the dwellings, etc). Buildings on silty clay deposits increases the likelihood of movements and therefore of cracking. The use of steel reinforcement in foundations placed at key structural locations will tend to reduce foundation cracking compared to unreinforced foundations.

Based on the consolidation testing results and subsurface profile encountered at the borehole locations, a permissible grade raise restriction was calculated for loadings associated with housing and for loadings associated with a 4 storey apartment building with an underground parking level. The recommended permissible grade raise areas for housing and apartment buildings are defined in Drawing PG2743-2 - Permissible Grade Raise Areas - Housing and Drawing PG2743-3 - Permissible Grade Raise Areas - Apartment Buildings in Appendix 2.

If higher grade raises and/or higher loading conditions are required, post construction settlements can be reduced by several methods. The following options can be considered and are further discussed in Subsection 5.4:

- preloading and surcharging
- lightweight fill (LWF)

Bearing resistance values for footing designs should be determined on a per lot basis at the time of construction.

5.4 Foundation Options

Based on the above discussion, several options could be considered for the foundation support of the proposed buildings:

Scenario A

Where the grade raise is close to, but below, the maximum permissible grade raise, consideration should be given to using more reinforcement in the design of the foundation (footings and walls) to reduce the risks of cracking in the concrete foundation. The use of control joints within the brick work between the garage and basement area should also be considered.

Scenario B

Where the grade raise cannot be accommodated with soil fill, the following options could be used alone or in combination.

Option 1 - Use of Lightweight Fill

Lightweight fill (LWF) can be used, consisting of EPS (expanded polystyrene) Type 19 or 22 blocks or other light weight materials which allow for raising the grade without adding a significant load to the underlying soils. However, these materials are expensive and, in the case of the EPS, are more difficult to use under the groundwater level, as they are buoyant, and must be protected against potential hydrocarbon spills. Use lightweight fill within the interior of the garage and porch areas to reduce the fill-related loads.

As an alternative to lightweight fill in the interior of the garage and porch, a structural slab can be designed to create a void beneath the floor slab and therefore reduce fill-related loads. Additional information can be provided once the design of the buildings is known.

Option 2 - Preloading or Surcharging

It is possible to preload or surcharge the proposed site in localized areas provided sufficient time is available to achieve the desired settlements based on theoretical values from the settlement analysis. If this option is considered, a monitoring program using settlement plates and electronic piezometers will have to be implemented. This program will determine the amount of settlement in the preloaded or surcharged areas. Obviously, preloading to proposed finished grades will allow for consolidation of the underlying clays over a longer time period. Surcharging the site with additional fill above the proposed finished grade will add additional load to the underlying clays accelerating the consolidation process and allowing for accelerated settlements. Once the desired settlements are achieved, the site can be unloaded and the fill can be used elsewhere on site.

With both the preloading and surcharging methods, the loading period can be reduced by installing vertical wick drains or sand drains in the silty clay layer to promote the movement of groundwater towards the ground surface. However, vertical drains are expensive for this type of residential project.

Underground Utilities

The underground services may be subjected to unacceptable total or differential settlements. In particular, the joints at the interface building/soil may be subjected to excessive stress if the differential settlements between the building and the services are excessive. This should be considered in the design of the underground services.

Once the required grade raises are established, the above options could be further discussed along with further recommendations on specific requirements.

5.5 Design for Earthquakes

A seismic site response **Class D** is applicable for foundations designed for the subject site according to the OBC 2012. A higher site class, such as Class C, may be applicable for foundations constructed within the east portion of the subject site. However, the higher site class should be confirmed by a site specific shear wave velocity test. The soils underlying the site are not susceptible to liquefaction.

5.6 Basement Slab

With the removal of all topsoil and deleterious fill, containing organic matter, within the footprints of the proposed buildings, undisturbed native soil surface will be considered acceptable subgrade on which to commence backfilling for floor slab construction. Any soft areas should be removed and backfilled with appropriate backfill material. OPSS Granular B Type II, with a maximum particle size of 50 mm, are recommended for backfilling below the floor slab. It is recommended that the upper 200 mm of sub-slab fill consist of 19 mm clear crushed stone.

5.7 Pavement Structure

For design purposes, the pavement structure presented in the following tables could be used for the design of car parking areas and access lanes/local residential streets. These guidelines should be reviewed once the details of the development are known.

Table 4 - Recommended Pavement Structure - Car Parking Areas	
Thickness (mm)	Material Description
50	Wear Course - HL 3 or Superpave 12.5 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
300	SUBBASE - OPSS Granular B Type II
SUBGRADE - Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil or fill	

Table 5 - Recommended Pavement Structure - Local Residential Roadways	
Thickness (mm)	Material Description
40	Wear Course - Superpave 12.5 Asphaltic Concrete
50	Binder Course - Superpave 19.0 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
400	SUBBASE - OPSS Granular B Type II
SUBGRADE - Either fill, in situ soil or OPSS Granular B Type I or II material placed over in situ soil	

Table 6 - Recommended Pavement Structure - Roadways with Bus Traffic	
Thickness mm	Material Description
40	Wear Course - Superpave 12.5 Asphaltic Concrete
50	Upper Binder Course - Superpave 19.0 Asphaltic Concrete
50	Lower Binder Course - Superpave 19.0 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
600	SUBBASE - OPSS Granular B Type II
SUBGRADE - Either in situ soil or OPSS Granular B Type II material placed over in situ soil	

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type I or II material. Weak subgrade conditions may be experienced over service trench fill materials. This may require the use of a geotextile, thicker subbase or other measures that can be recommended at the time of construction as part of the field observation program.

The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

Pavement Structure Drainage

Satisfactory performance of the pavement structure is largely dependent on the contact zone between the subgrade material and the base stone in a dry condition. Failure to provide adequate drainage under conditions of heavy wheel loading can result in the fine subgrade soil being pumped into the voids in the stone subbase, thereby reducing load carrying capacity.

Due to the low permeability of the subgrade materials consideration should be given to installing subdrains during the pavement construction as per City of Ottawa standards. The subdrain inverts should be approximately 300 mm below subgrade level. The subgrade surface should be crowned to promote water flow to the drainage lines.

6.0 Design and Construction Precautions

6.1 Foundation Drainage and Backfill

A perimeter foundation drainage system is recommended for proposed structures. The system should consist of a 100 to 150 mm diameter, geotextile-wrapped, perforated, corrugated, plastic pipe, surrounded on all sides by 150 mm of 10 mm clear crushed stone, placed at the footing level around the exterior perimeter of the structure. The pipe should have a positive outlet, such as a gravity connection to the storm sewer.

Backfill against the exterior sides of the foundation walls should consist of free-draining, non frost susceptible granular materials. The site materials will be frost susceptible and, as such, are not recommended for re-use as backfill unless a composite drainage system (such as system Platon or Miradrain G100N) connected to a drainage system is provided.

6.2 Protection Against Frost Action

Perimeter footings of heated structures are required to be insulated against the deleterious effect of frost action. A minimum 1.5 m thick soil cover (or equivalent) should be provided in this regard.

A minimum of 2.1 m thick soil cover (or equivalent) should be provided for other exterior unheated footings.

6.3 Excavation Side Slopes

The excavation for the current phase of the proposed development will be mostly through sandy silt and/or clayey silt/silty clay. Above the groundwater level, for excavations to depths of approximately 3 m, the excavation side slopes should be stable in the short term at 1H:1V. Flatter slopes could be required for deeper excavations or for excavation below the groundwater level. Where such side slopes are not permissible or practical, temporary shoring should be used. The subsoil at this site is considered to be mainly a Type 2 or 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

The slope cross-sections recommended above are for temporary slopes. Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

It is recommended that a trench box be used at all times to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

6.4 Pipe Bedding and Backfill

Bedding and backfill materials should be in accordance with the most recent Material Specifications and Standard Detail Drawings from the City of Ottawa.

The pipe bedding for sewer and water pipes should consist of at least 150 mm of OPSS Granular A material. Where the bedding is located within the firm grey silty clay, the thickness of the bedding material should be increased to a minimum of 300 mm. The material should be placed in maximum 300 mm thick lifts and compacted to a minimum of 95% of its SPMDD. The bedding material should extend at least to the spring line of the pipe.

The cover material, which should consist of OPSS Granular A, should extend from the spring line of the pipe to at least 300 mm above the obvert of the pipe. The material should be placed in maximum 300 mm thick lifts and compacted to a minimum of 95% of its SPMDD.

Generally, it should be possible to re-use the moist (not wet) brown silty clay above the cover material if the excavation and filling operations are carried out in dry weather conditions. Wet silty clay materials will be difficult to re-use, as the high water contents make compacting impractical without an extensive drying period.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 95% of the material's SPMDD.

To reduce long-term lowering of the groundwater level at this site, clay seals should be provided in the service trenches. The seals should be at least 1.5 m long (in the trench direction) and should extend from trench wall to trench wall. The seals should extend from the frost line and fully penetrate the bedding, subbedding and cover material. The barriers should consist of relatively dry and compactable brown silty clay placed in maximum 225 mm thick loose layers and compacted to a minimum of 95% of the SPMDD. The clay seals should be placed at the site boundaries and at strategic locations at no more than 60 m intervals in the service trenches.

6.5 Groundwater Control

Due to the relatively impervious nature of the silty clay materials, it is anticipated that groundwater infiltration into the excavations should be low and controllable using open sumps. A perched groundwater condition may be encountered within the silty sand/sandy silt deposit which may produce significant temporary groundwater infiltration levels. Pumping from open sumps should be sufficient to control the groundwater influx through the sides of shallow excavations.

A temporary MOE permit to take water (PTTW) will be required for this project if more than 50,000 L/day are to be pumped during the construction phase. At least 3 to 4 months should be allowed for completion of the application and issuance of the permit by the MOE.

The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

6.6 Winter Construction

The subsurface conditions at this site mostly consist of frost susceptible materials. In presence of water and freezing conditions ice could form within the soil mass. Heaving and settlement upon thawing could occur. Precautions should be taken if winter construction is considered for this project.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters, tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

The trench excavations should be constructed in a manner that will avoid the introduction of frozen materials into the trenches. As well, pavement construction is difficult during winter. The subgrade consists of frost susceptible soils which will experience total and differential frost heaving as the work takes place. In addition, the introduction of frost, snow or ice into the pavement materials, which is difficult to avoid, could adversely affect the performance of the pavement structure. Additional information could be provided, if required.

6.7 Landscaping Considerations

Tree Planting Restrictions

In accordance with the City of Ottawa Tree Planting in Sensitive Marine Clay Soils (2017 Guidelines), Paterson completed a soils review of the site to determine applicable tree planting setbacks. Atterberg limits testing was completed for recovered silty clay samples at selected locations throughout the subject site. A shrinkage limit test and sieve analysis testing was also completed on selected soil samples. The shrinkage limit testing indicates a shrinkage limit of 21% with a shrinkage ratio of 1.78. The results of our atterberg limit and sieve testing are presented in Appendix 1.

During our field investigation, it was noted that the silty clay deposit across the site consists of a brown, stiff to very stiff silty clay, which is not considered to be a sensitive marine clay soil. Therefore, the Tree Planting Guidelines not required to be followed for the subject site. **Based on our review of the silty clay deposit, a tree planting setback limit of 4.5 m for small (mature tree height up to 7.5m) and medium size trees (mature tree height 7.5 m to 14 m) is recommended across the subject site provided that the following conditions are met.**

- ❑ The underside of footing (USF) is 2.1 m or greater below the lowest finished grade must be satisfied for footings within 10 m from the tree, as measured from the centre of the tree trunk and verified by means of the Grading Plan as indicated procedural changes below.

- ❑ A small tree must be provided with a minimum of 25 m³ of available soil volume while a medium tree must be provided with a minimum of 30 m³ of available soil volume, as determined by the Landscape Architect. The developer is to ensure that the soil is generally un-compacted when backfilling in street tree planting locations.

- ❑ The tree species must be small (mature tree height up to 7.5 m) to medium size (mature tree height 7.5 m to 14 m) as confirmed by the Landscape Architect. The foundation walls are to be reinforced at least nominally (minimum of two upper and two lower 15M bars in the foundation wall).
- ❑ Grading surround the tree must promote drainage to the tree root zone (in such a manner as not to be detrimental to the tree), as noted on the subdivision Grading Plan.

Swimming Pools

The in-situ soils are considered to be acceptable for swimming pools. Above ground swimming pools must be placed at least 4 m away from the residence foundation and neighbouring foundations. Otherwise, pool construction is considered routine, and can be constructed in accordance with the manufacturer's requirements.

Aboveground Hot Tubs

If consideration is given to construction of an aboveground hot tub, a geotechnical consultant should be retained by the homeowner to review the site conditions. Additional grading around the hot tub should not exceed permissible grade raises. Otherwise, hot tub construction is considered routine, and can be constructed in accordance with the manufacturer's specifications.

Installation of Decks or Additions

If consideration is given to construction of a deck or addition, a geotechnical consultant should be retained by the homeowner to review the site conditions. Additional grading around proposed deck or addition should not exceed permissible grade raises. Otherwise, standard construction practices are considered acceptable.

7.0 Recommendations

It is recommended that the following be completed once the master plan and site development are determined:

- Review detailed grading plan(s) from a geotechnical perspective.
- Observation of all bearing surfaces prior to the placement of concrete.
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- Observation of all subgrades prior to placing backfilling materials.
- Field density tests to ensure that the specified level of compaction has been achieved.
- Sampling and testing of the bituminous concrete including mix design reviews.

A report confirming that these works have been conducted in general accordance with Paterson's recommendations could be issued upon request, following the completion of a satisfactory material testing and observation program by the geotechnical consultant.

8.0 Statement of Limitations

The recommendations made in this report are in accordance with Paterson's present understanding of the project. Paterson requests permission to review the grading plan once available. Paterson's recommendations should be reviewed when the drawings and specifications are complete.

The client should be aware that any information pertaining to soils and the test hole log are furnished as a matter of general information only. Test hole descriptions or logs are not to be interpreted as descriptive of conditions at locations other than those of the test holes.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, Paterson requests to be notified immediately in order to permit reassessment of the recommendations.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Caivan Communities or their agent(s) is not authorized without review by this firm for the applicability of our recommendations to the altered use of the report.

Paterson Group Inc.



Joey R Villeneuve, M.A.Sc, EIT.



David J. Gilbert, P.Eng.

APPENDIX 1

SOIL PROFILE AND TEST DATA SHEET

SYMBOLS AND TERMS

RECORDS OF BOREHOLE BY OTHERS

CONSOLIDATION TEST RESULTS

ATTERBERG LIMITS' TESTING RESULTS

GRAIN SIZE DISTRIBUTION RESULTS

DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

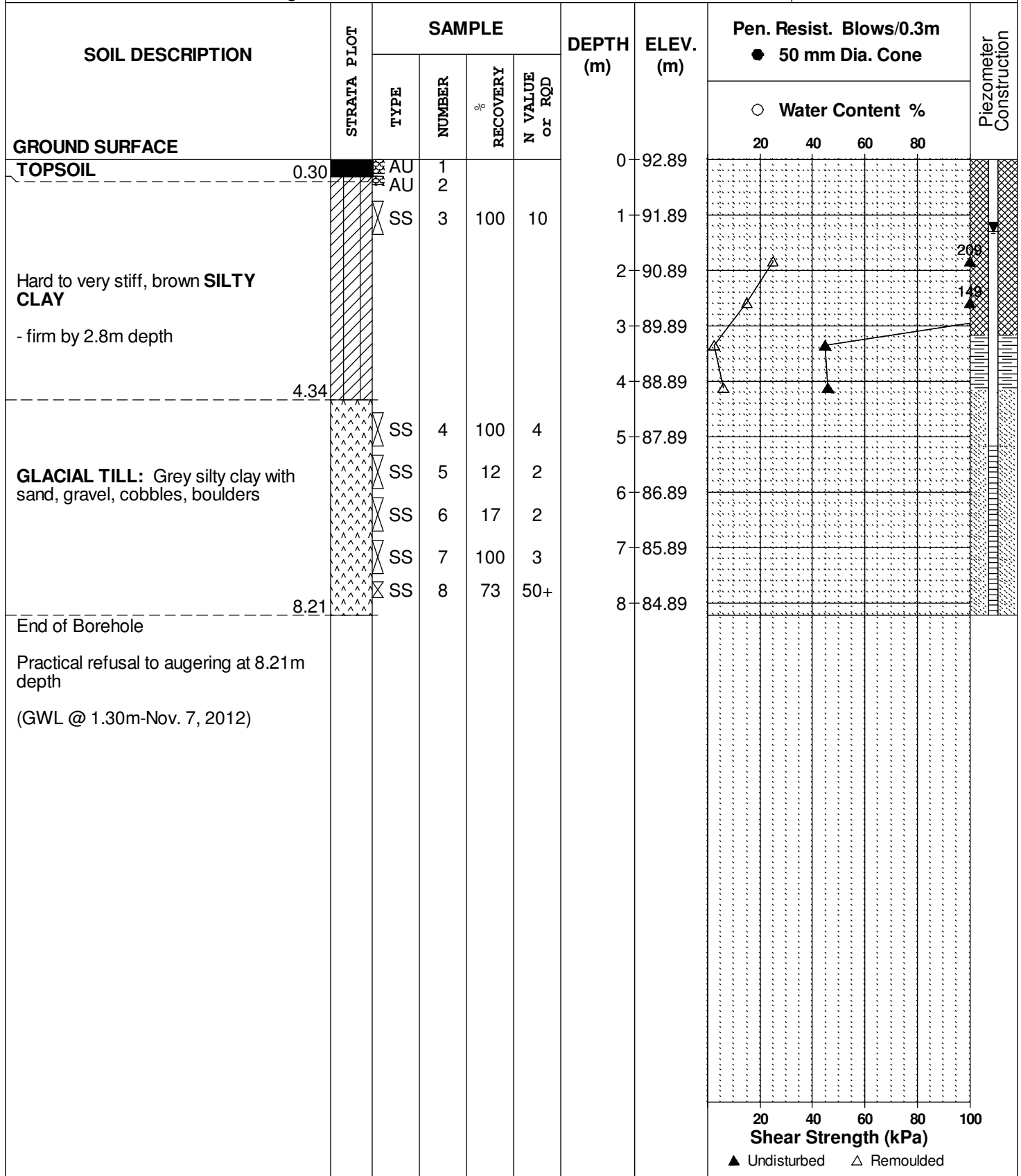
FILE NO. PG2743

REMARKS

HOLE NO. BH 1

BORINGS BY CME 850X Power Auger

DATE October 15, 2012



SOIL PROFILE AND TEST DATA

Geotechnical Investigation
 Prop. Residential Development - 3288 Greenbank Rd.
 Ottawa, Ontario

DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

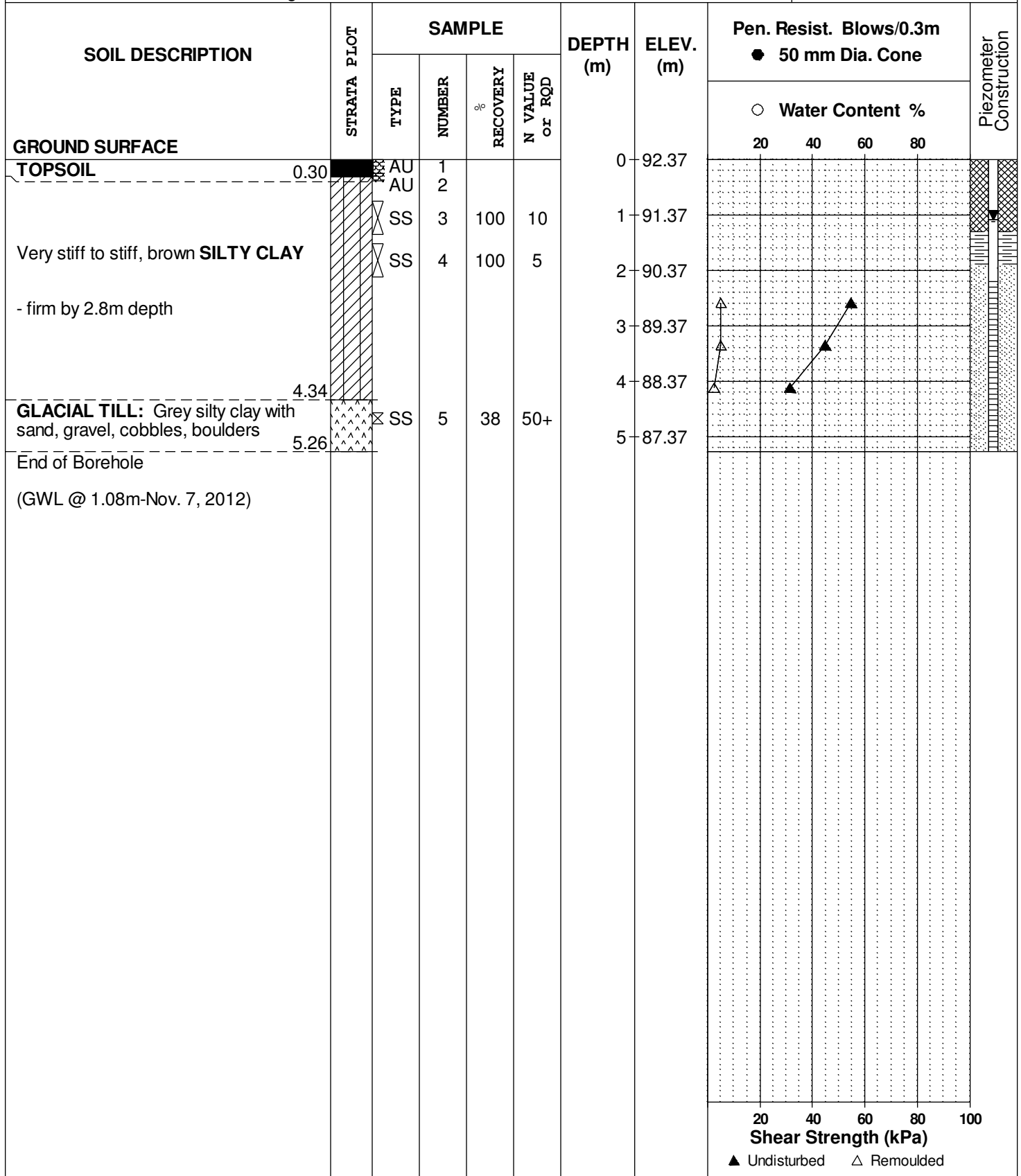
FILE NO. PG2743

REMARKS

HOLE NO. BH 2

BORINGS BY CME 850X Power Auger

DATE October 16, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

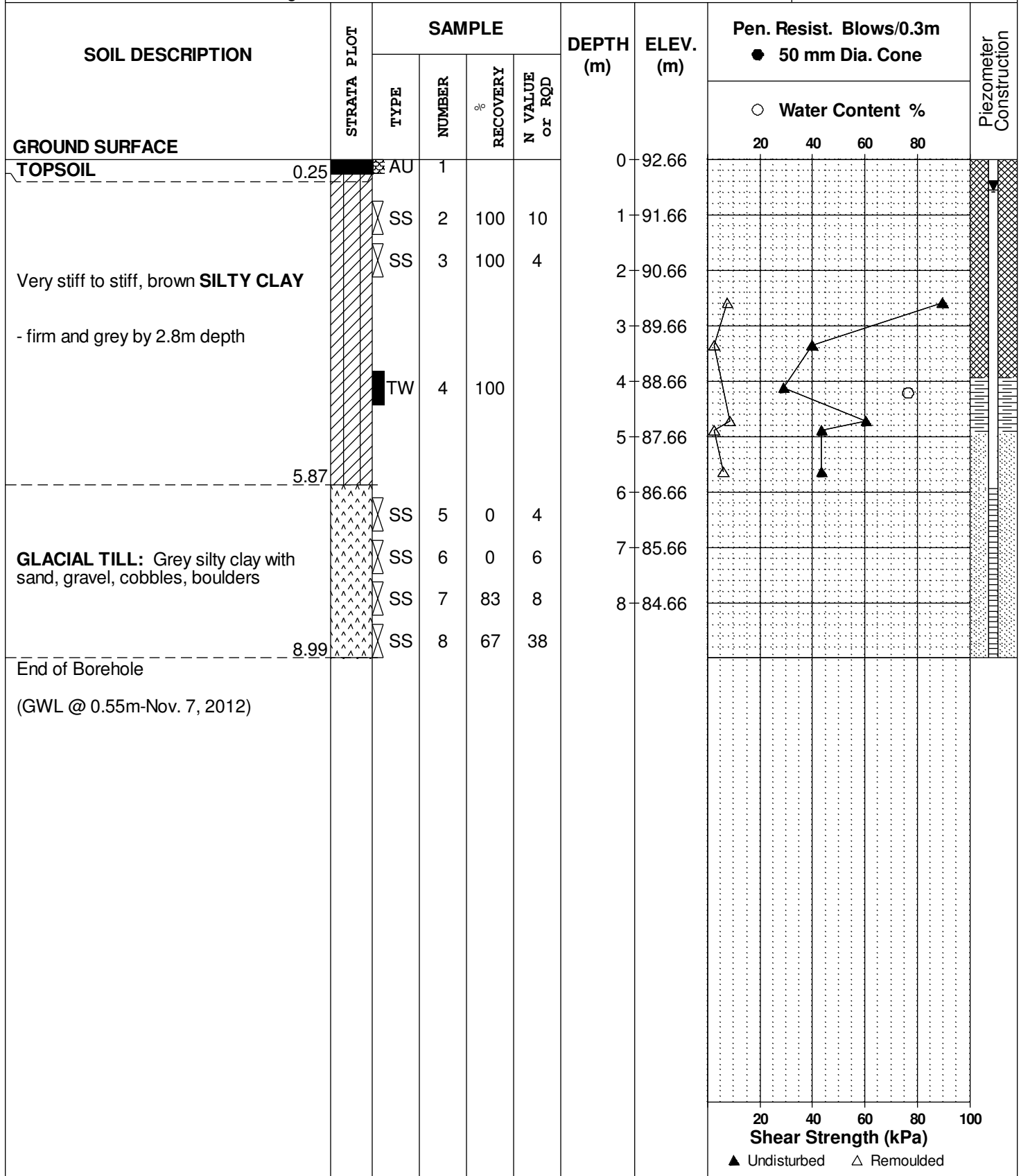
FILE NO. PG2743

REMARKS

HOLE NO. BH 3

BORINGS BY CME 850X Power Auger

DATE October 16, 2012



SOIL PROFILE AND TEST DATA

Geotechnical Investigation
 Prop. Residential Development - 3288 Greenbank Rd.
 Ottawa, Ontario

DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

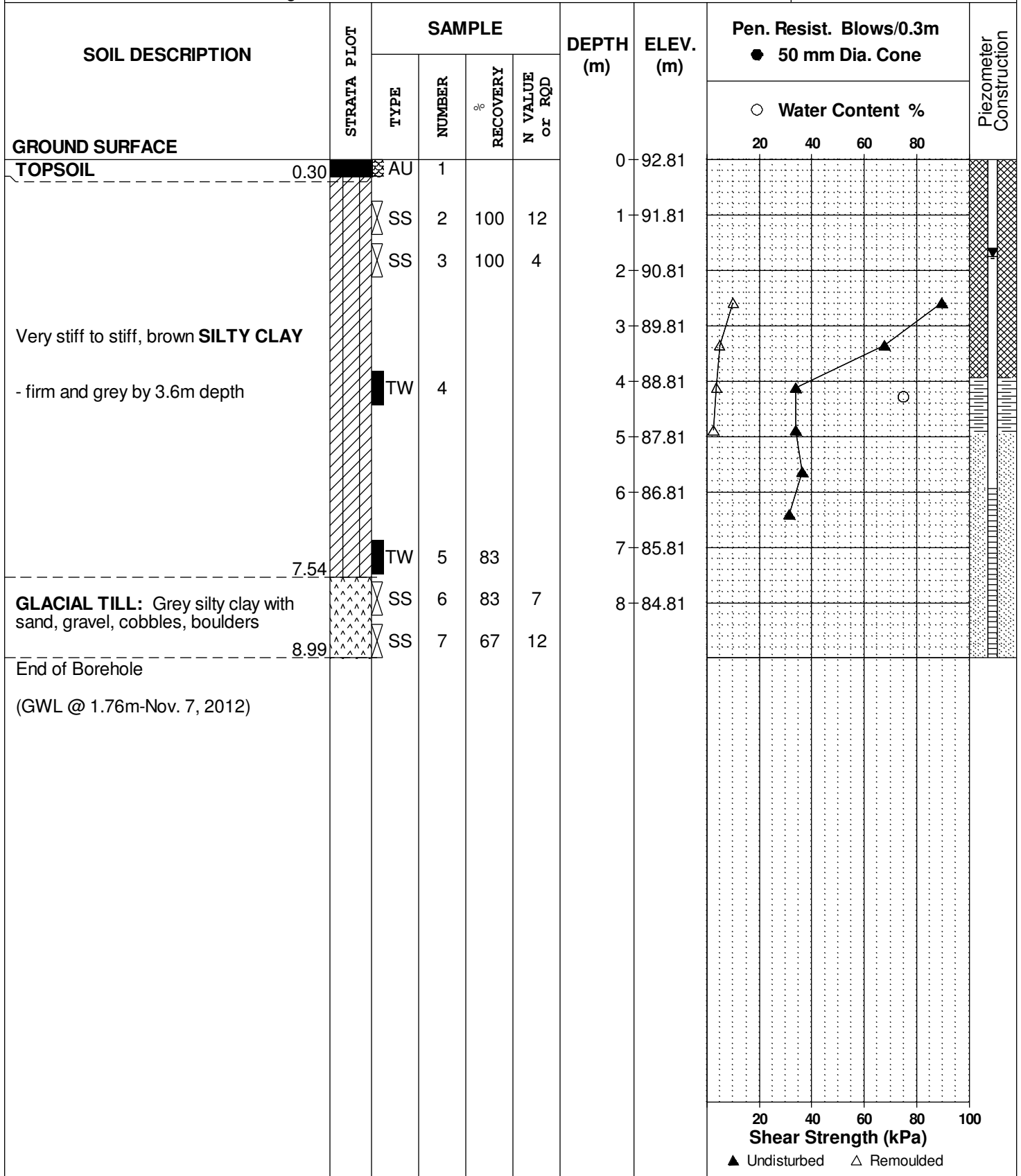
FILE NO. PG2743

REMARKS

HOLE NO. BH 4

BORINGS BY CME 850X Power Auger

DATE October 16, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

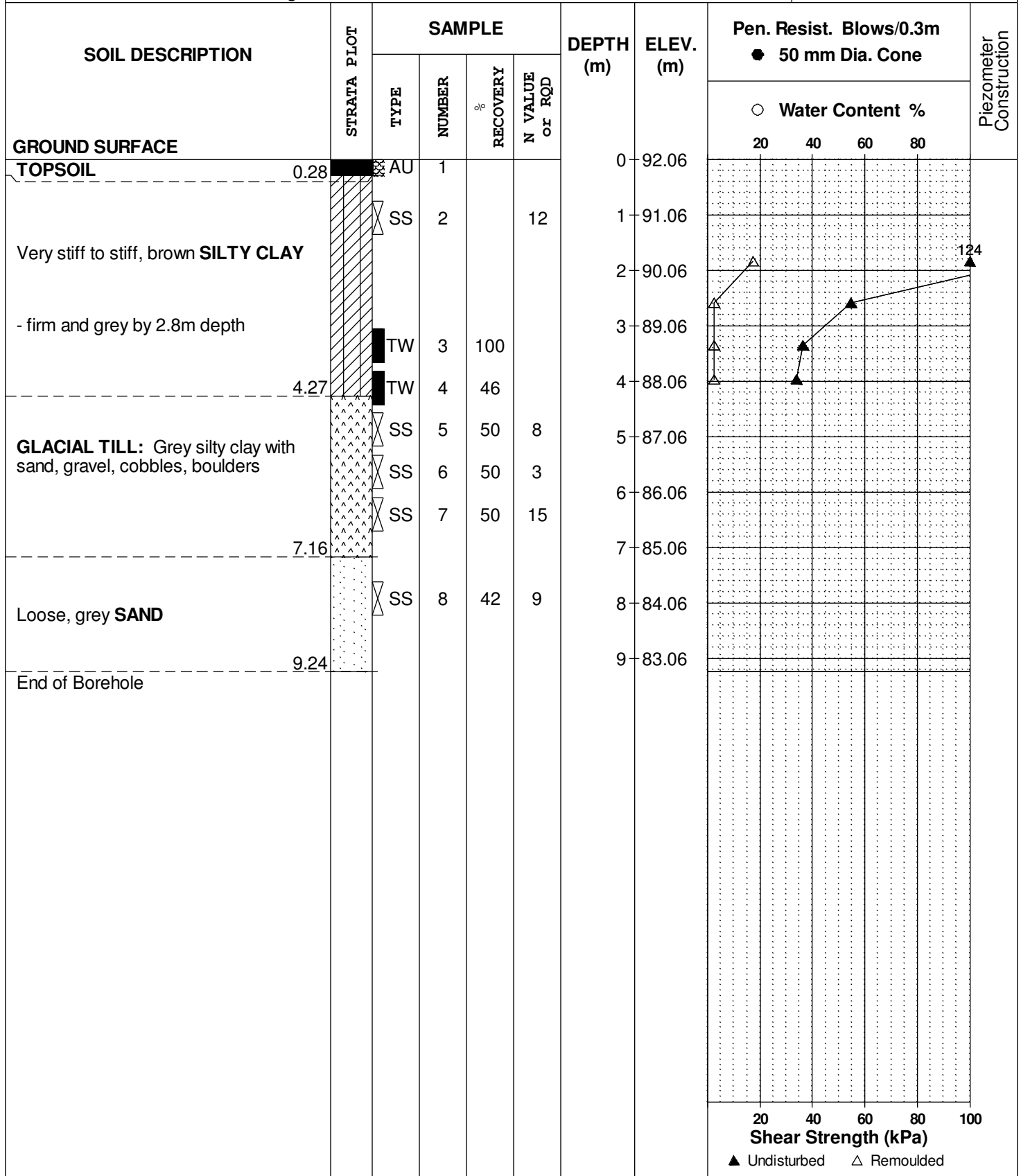
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REMARKS

HOLE NO. BH 5

BORINGS BY CME 850X Power Auger

DATE October 16, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

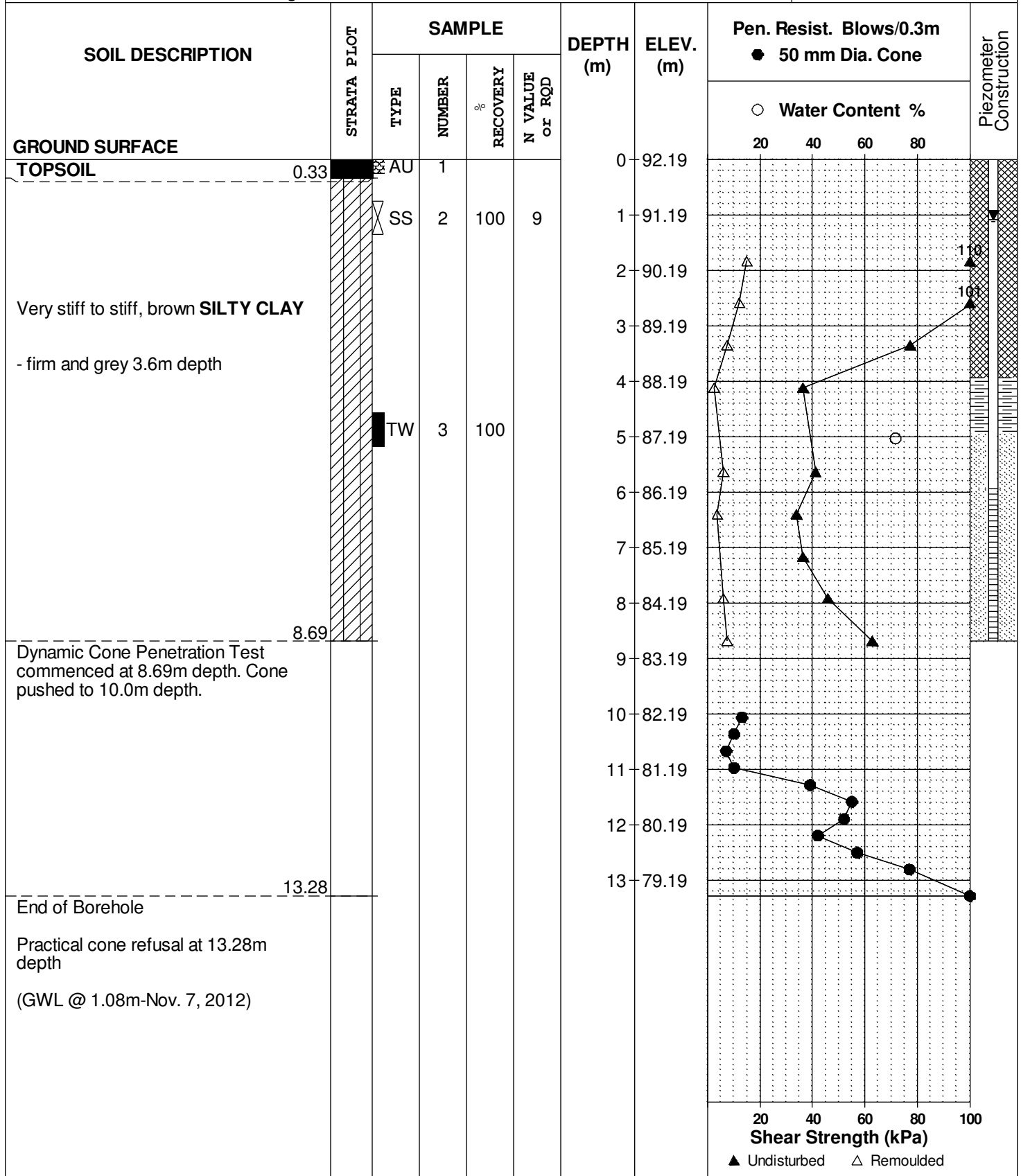
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REMARKS

HOLE NO. BH 6

BORINGS BY CME 850X Power Auger

DATE October 17, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

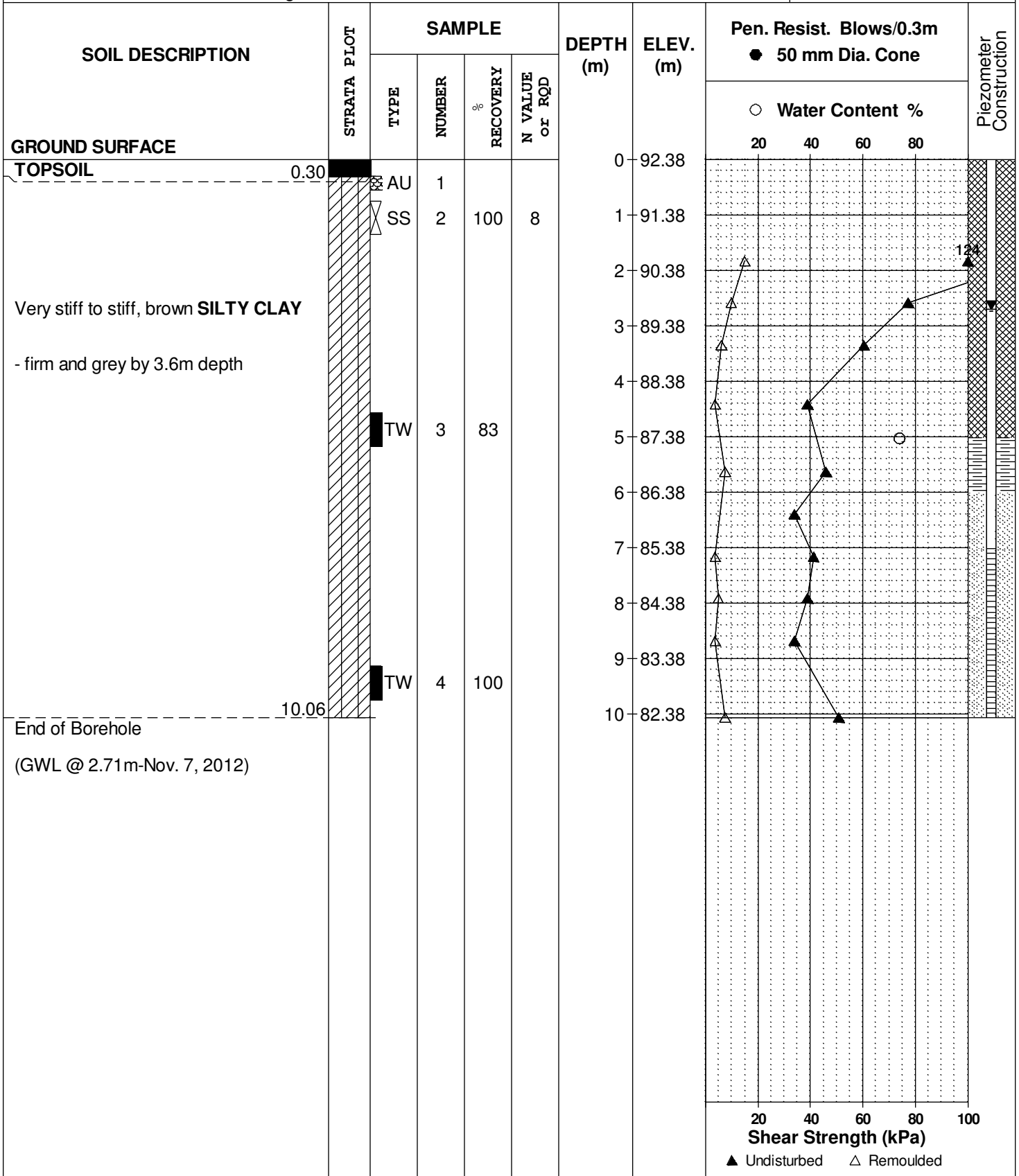
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REMARKS

HOLE NO. BH 7

BORINGS BY CME 850X Power Auger

DATE October 17, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

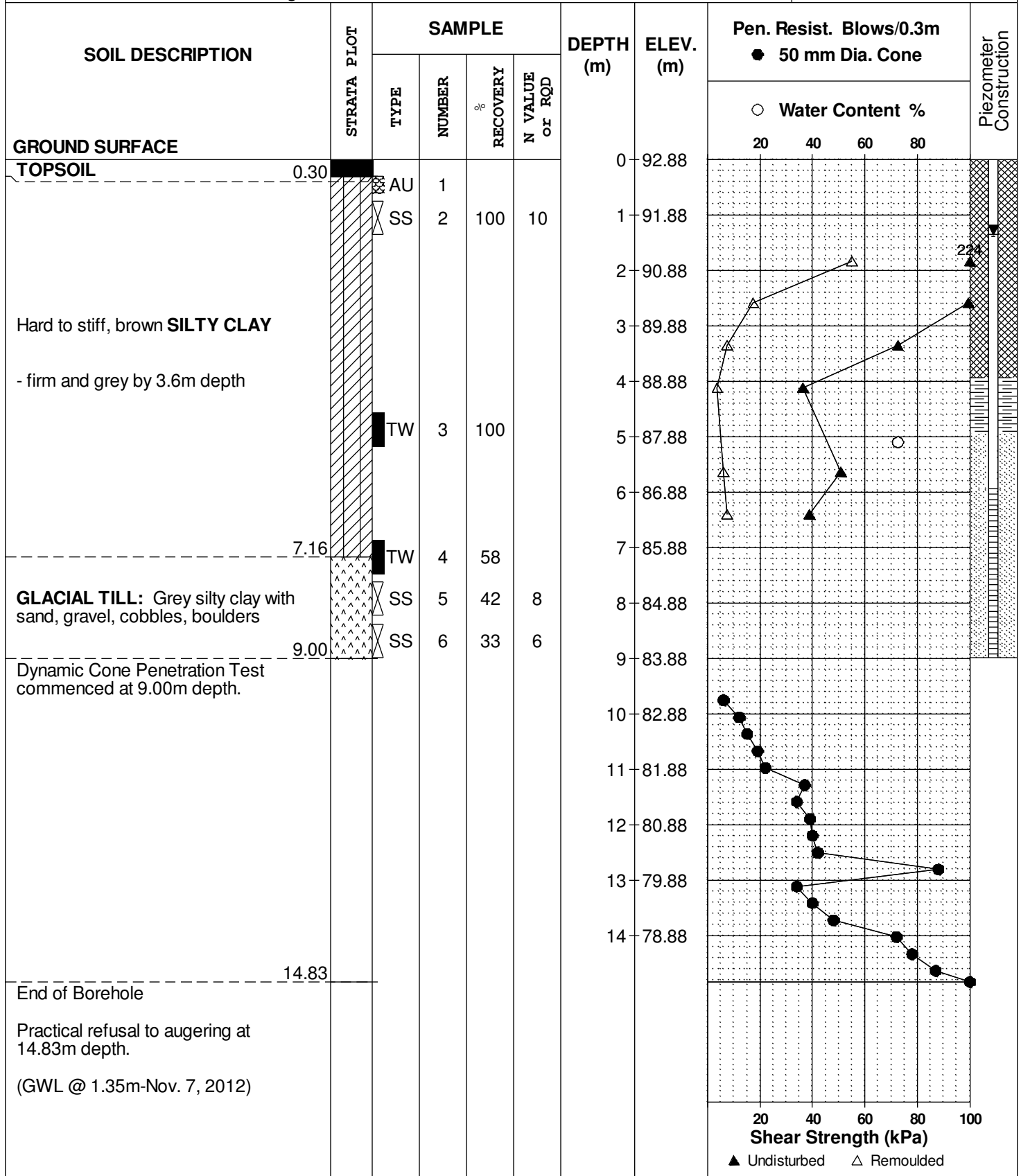
FILE NO. PG2743

REMARKS

HOLE NO. BH 8

BORINGS BY CME 850X Power Auger

DATE October 17, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

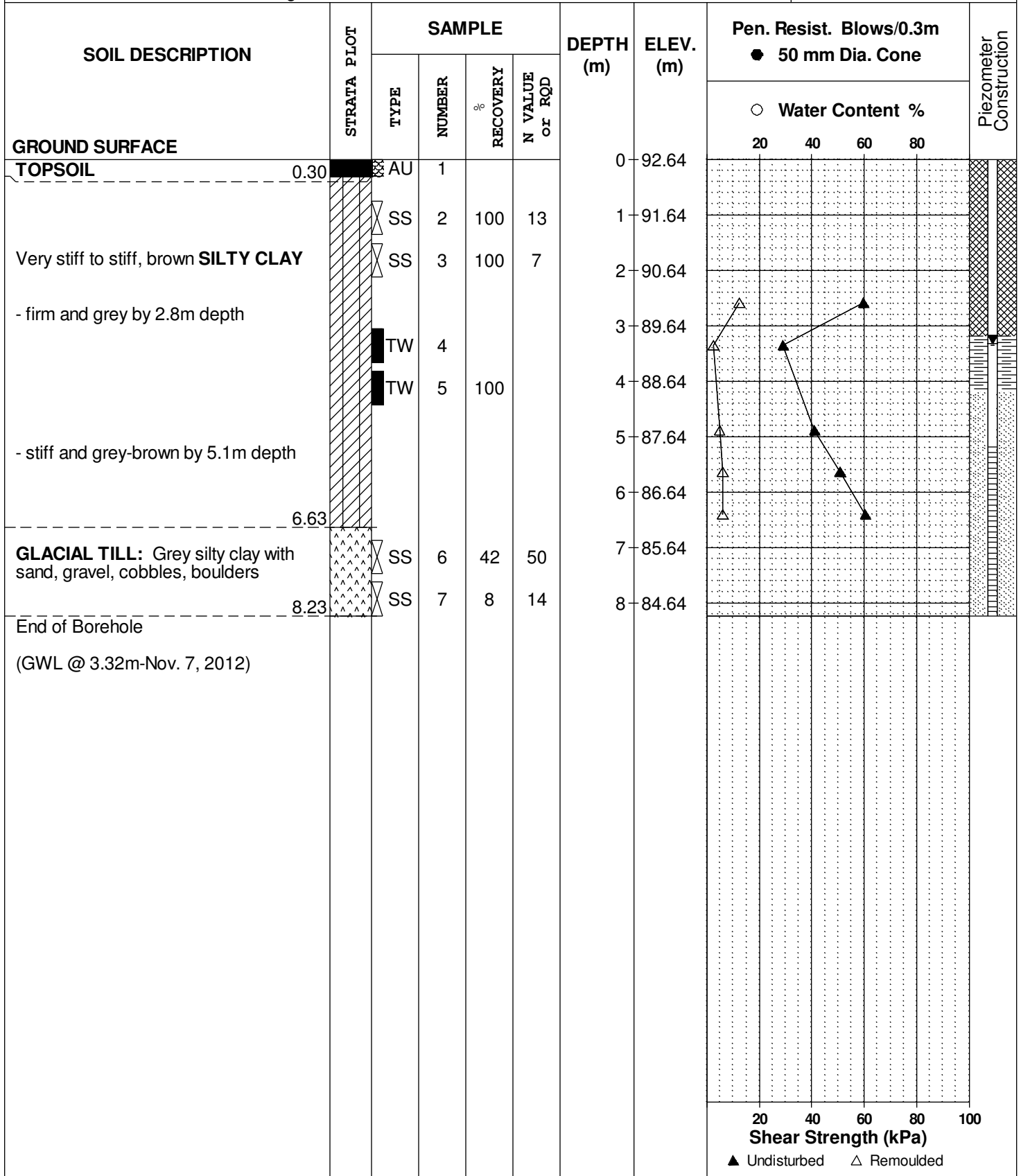
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REMARKS

HOLE NO. BH 9

BORINGS BY CME 850X Power Auger

DATE October 18, 2012



DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

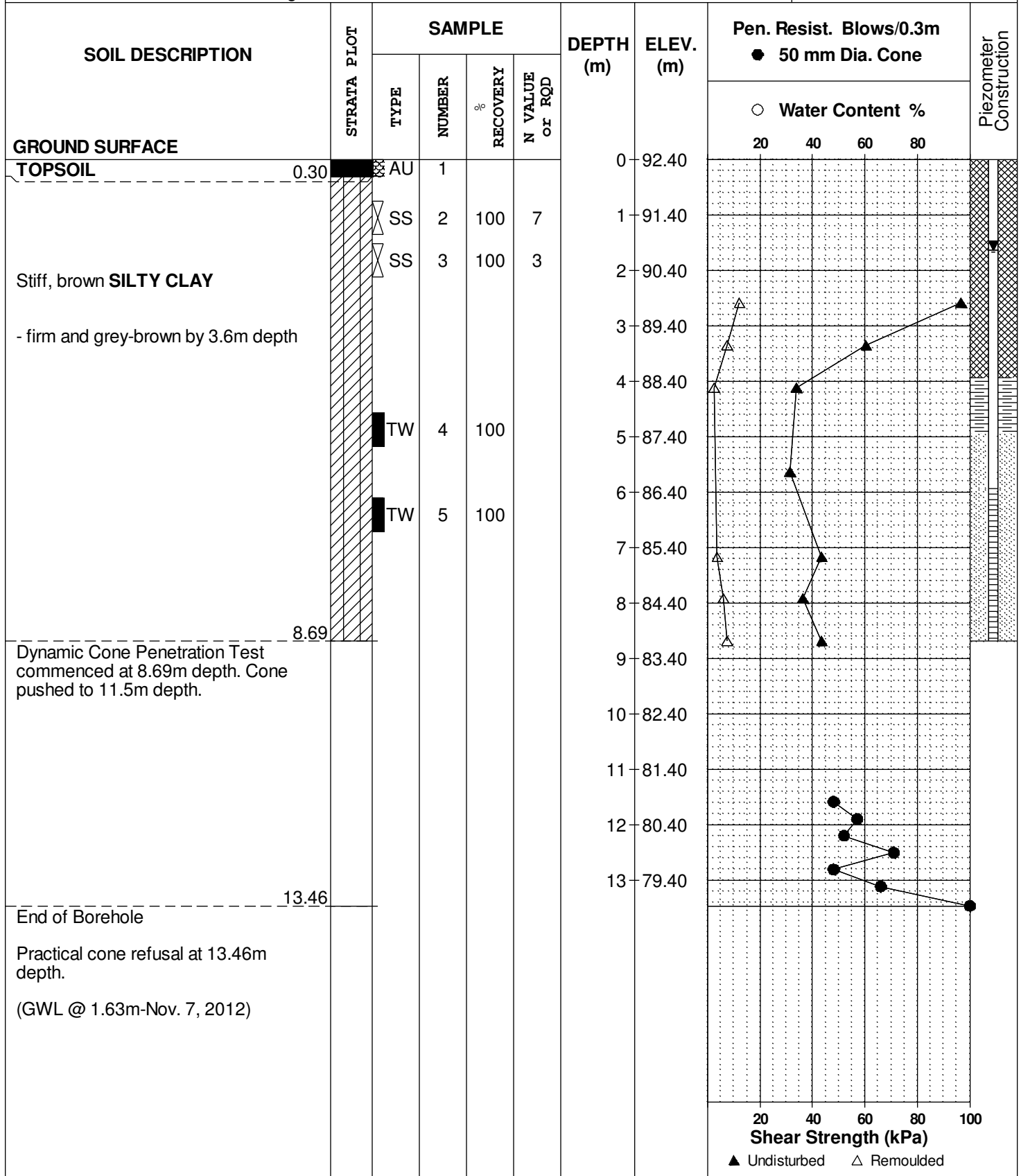
FILE NO. PG2743

REMARKS

HOLE NO. BH10

BORINGS BY CME 850X Power Auger

DATE October 18, 2012



SOIL PROFILE AND TEST DATA

Geotechnical Investigation
 Prop. Residential Development - 3288 Greenbank Rd.
 Ottawa, Ontario

DATUM Ground surface elevations at borehole locations provided by J.D. Barnes Limited.

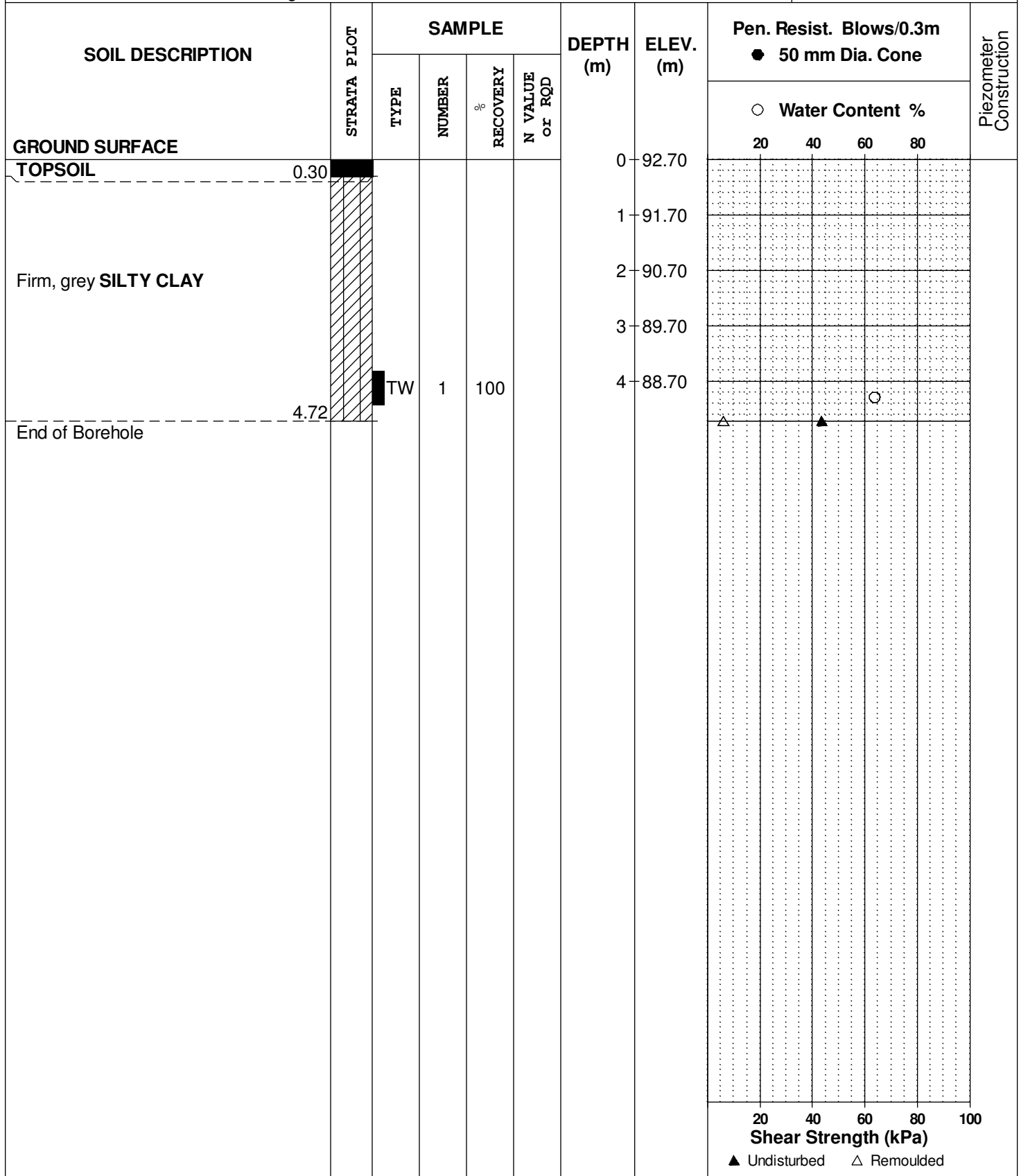
FILE NO. PG2743

REMARKS

HOLE NO. BH11

BORINGS BY CME 850X Power Auger

DATE October 18, 2012



DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 1**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY	N VALUE or RQD			○ Water Content %					
GROUND SURFACE								20	40	60	80		
TOPSOIL	[REDACTED]					0	93.85						
Brown SILTY SAND with clay	[REDACTED]												
Stiff, brown CLAYEY SILT	[REDACTED]					1	92.85						
GLACIAL TILL: Brown clayey silt with sand, gravel, cobbles and boulders	[REDACTED]	G	1			2	91.85						∇
End of Test Pit (GWL @ 2.0m depth based on field observations)	[REDACTED]	G	2										
	[REDACTED]	G	3										

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 2**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE						0	92.72						
TOPSOIL	0.25												
Very stiff to stiff, brown SILTY CLAY - grey by 1.0m depth	G	1											
	G	2				1	91.72						▽
	G	3											
	G	4				2	90.72						
End of Test Pit (Groundwater infiltration at 1.5m depth)	2.70												

○ Water Content %
20 40 60 80
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

REMARKS

BORINGS BY Excavator

DATE February 19, 2019

FILE NO. **PG2743**

HOLE NO. **TP 3**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE						0	92.90						
TOPSOIL	[REDACTED]												
	0.30												
Very stiff to stiff, brown SILTY CLAY - grey by 1.0m depth		G	1			1	91.90						
		G	2										
		G	3			2	90.90						
		G	4										
End of Test Pit (Groundwater infiltration at 1.8m depth)	2.50												

○ Water Content %

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 4**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %				
GROUND SURFACE						0	92.95	20	40	60	80	
TOPSOIL	[REDACTED]					0	92.95					
	0.25											
Very stiff, brown CLAYEY SILT/SILTY CLAY												
	1.00					1	91.95					
Very stiff, brown SILTY CLAY		G	1									
		G	2			2	90.95					
		G	3									
	2.60											
End of Test Pit (Groundwater infiltration at 1.9m depth)												

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 5**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE						0	92.40						
TOPSOIL	[REDACTED]												
	0.30												
Stiff, brown SILTY CLAY		G	1			1	91.40						▽
		G	2			2	90.40						
		G	3										
End of Test Pit (Groundwater infiltration at 1.7m depth)	2.60												

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 7**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE						0	92.63						
TOPSOIL													
Very stiff, brown SILTY CLAY with sand		G	1										
Stiff, brown SILTY CLAY		G	2										
End of Test Pit (Groundwater infiltration at 1.5m depth)		G	3										▽

○ Water Content %
20 40 60 80
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

DATUM Ground surface elevations provided by J.D. Barnes Limited.

FILE NO. **PG2743**

REMARKS

HOLE NO. **TP 8**

BORINGS BY Excavator

DATE February 19, 2019

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
GROUND SURFACE						0	93.20						
TOPSOIL	[REDACTED]												
	0.30												
Very stiff, brown SILTY CLAY with sand	[Hatched Pattern]												
	1.10					1	92.20						
Very stiff, brown SILTY CLAY	[Hatched Pattern]	G	1										▽
		G	2			2	91.20						
		G	3										
	2.60												
End of Test Pit (Groundwater infiltration at 1.6m depth)													

20 40 60 80 100
Shear Strength (kPa)
▲ Undisturbed △ Remoulded

SYMBOLS AND TERMS

SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

SYMBOLS AND TERMS (continued)

SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

RQD %	ROCK QUALITY
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.

SYMBOLS AND TERMS (continued)

GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic limit, % (water content above which soil behaves plastically)
PI	-	Plasticity index, % (difference between LL and PL)
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D10	-	Grain size at which 10% of the soil is finer (effective grain size)
D60	-	Grain size at which 60% of the soil is finer
Cc	-	Concavity coefficient = $(D_{30})^2 / (D_{10} \times D_{60})$
Cu	-	Uniformity coefficient = D_{60} / D_{10}

Cc and Cu are used to assess the grading of sands and gravels:

Well-graded gravels have: $1 < Cc < 3$ and $Cu > 4$

Well-graded sands have: $1 < Cc < 3$ and $Cu > 6$

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

Cc and Cu are not applicable for the description of soils with more than 10% silt and clay (more than 10% finer than 0.075 mm or the #200 sieve)

CONSOLIDATION TEST

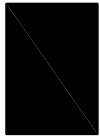
p'_o	-	Present effective overburden pressure at sample depth
p'_c	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below p'_c)
Cc	-	Compression index (in effect at pressures above p'_c)
OC Ratio		Overconsolidation ratio = p'_c / p'_o
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

PERMEABILITY TEST

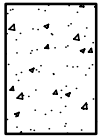
k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.
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SYMBOLS AND TERMS (continued)

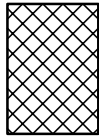
STRATA PLOT



Topsoil



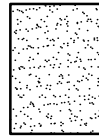
Asphalt



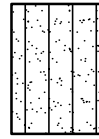
Fill



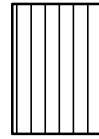
Peat



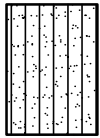
Sand



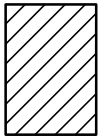
Silty Sand



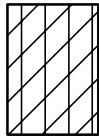
Silt



Sandy Silt



Clay



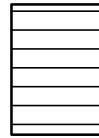
Silty Clay



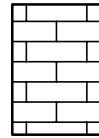
Clayey Silty Sand



Glacial Till



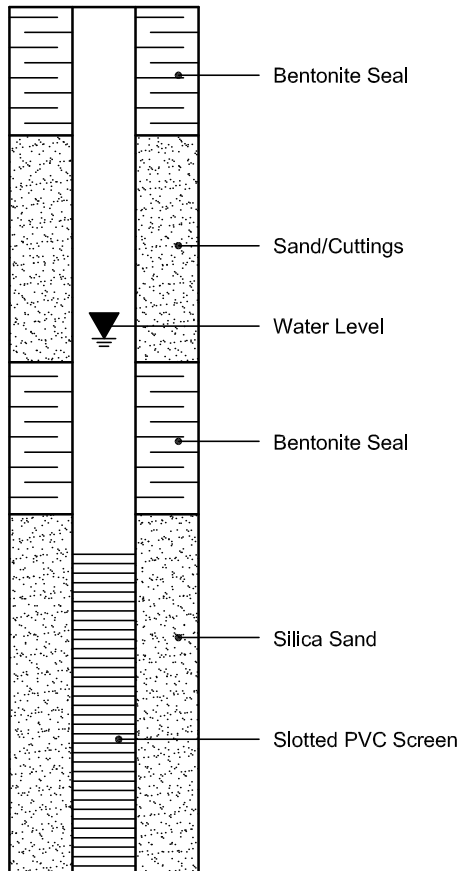
Shale



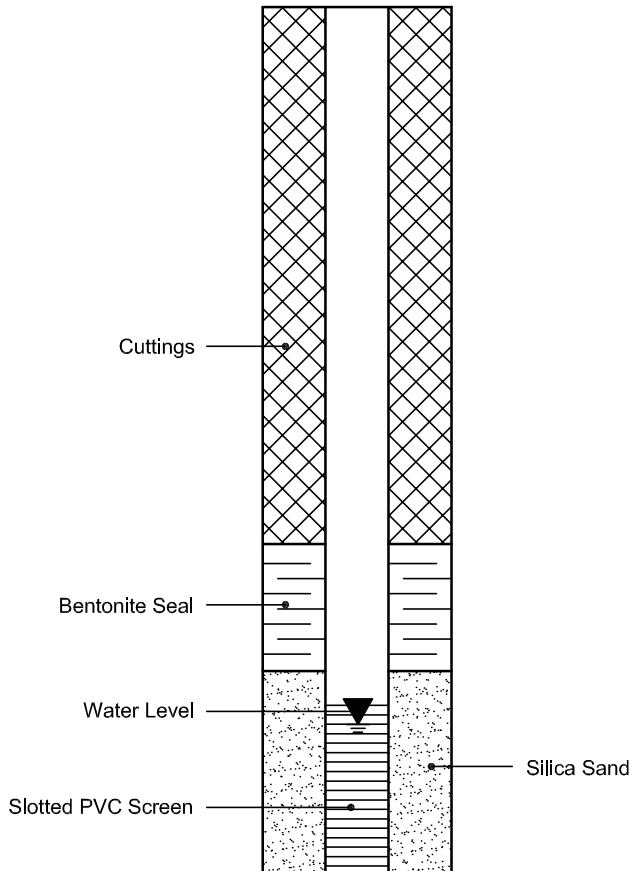
Bedrock

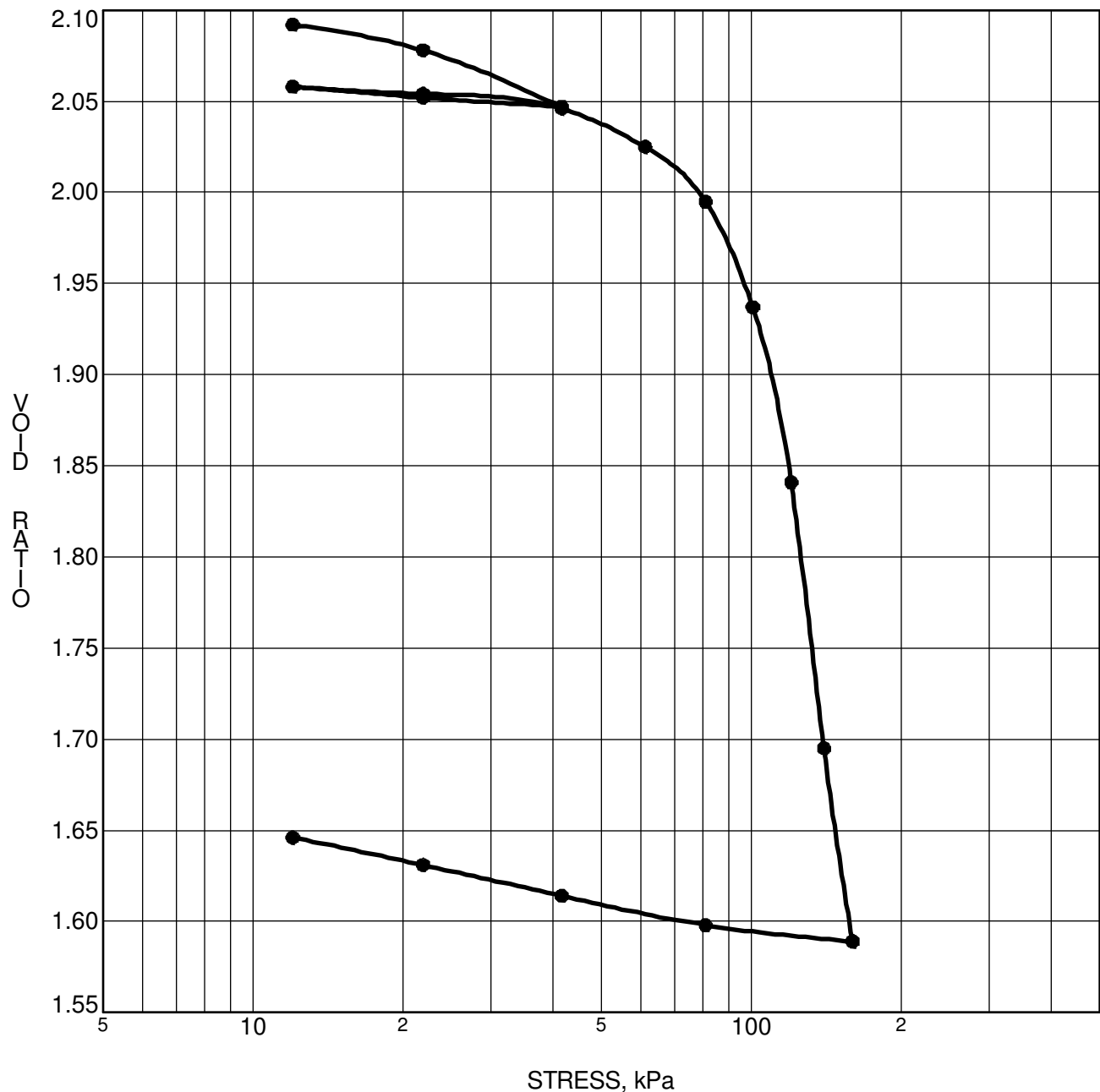
MONITORING WELL AND PIEZOMETER CONSTRUCTION

MONITORING WELL CONSTRUCTION



PIEZOMETER CONSTRUCTION





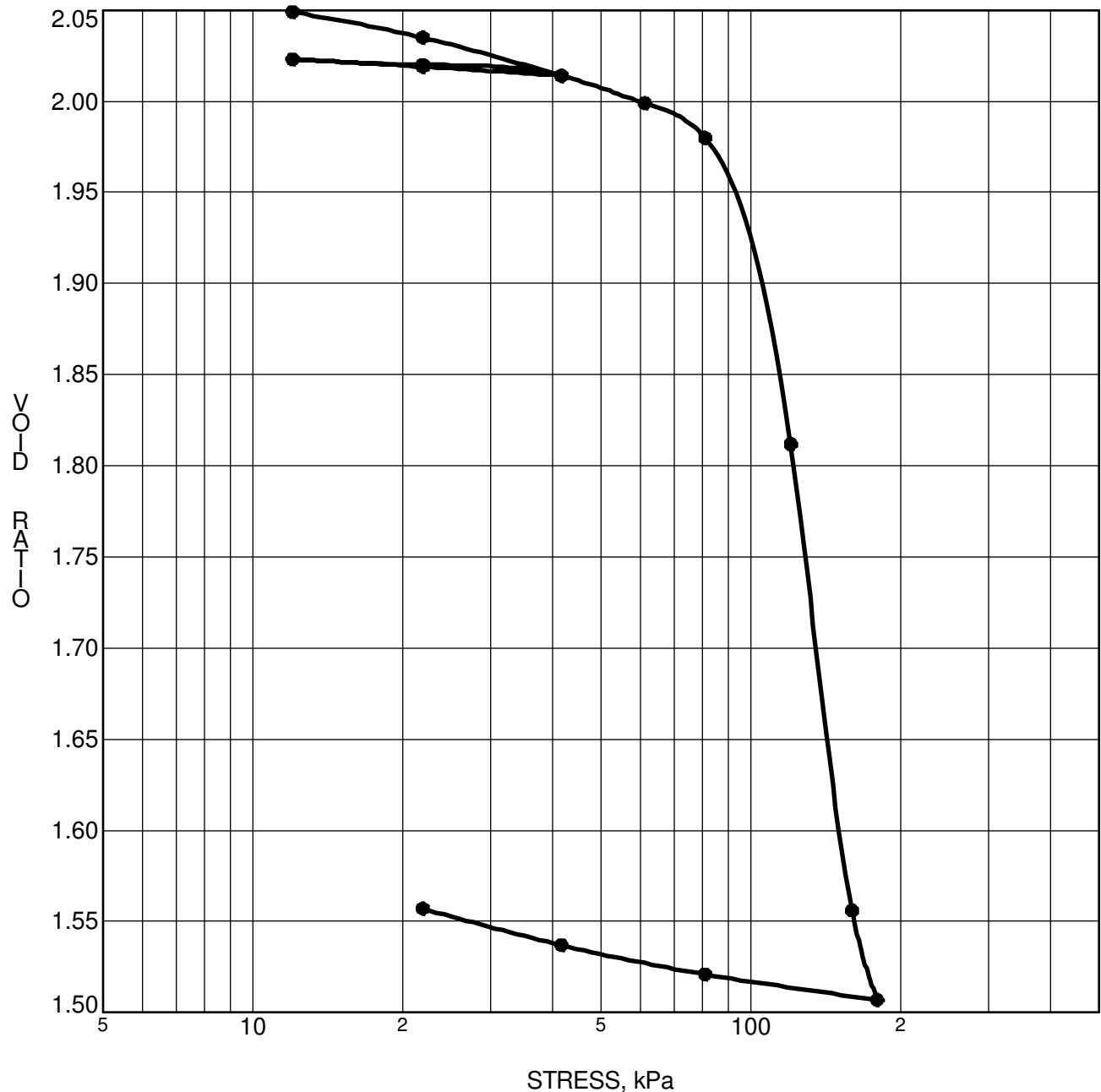
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 3	p'_o	50 kPa	C_{cr}	0.021
Sample No.	TW 4	p'_c	104 kPa	C_c	2.253
Sample Depth	4.21 m	OC Ratio	2.1	W_o	76.5 %
Sample Elev.	88.45 m	Void Ratio	2.103	Unit Wt.	15.8 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **26/10/2012**

patersongroup Consulting Engineers
 154 Colonnade Road South, Ottawa, Ontario K2E 7J5

CONSOLIDATION TEST



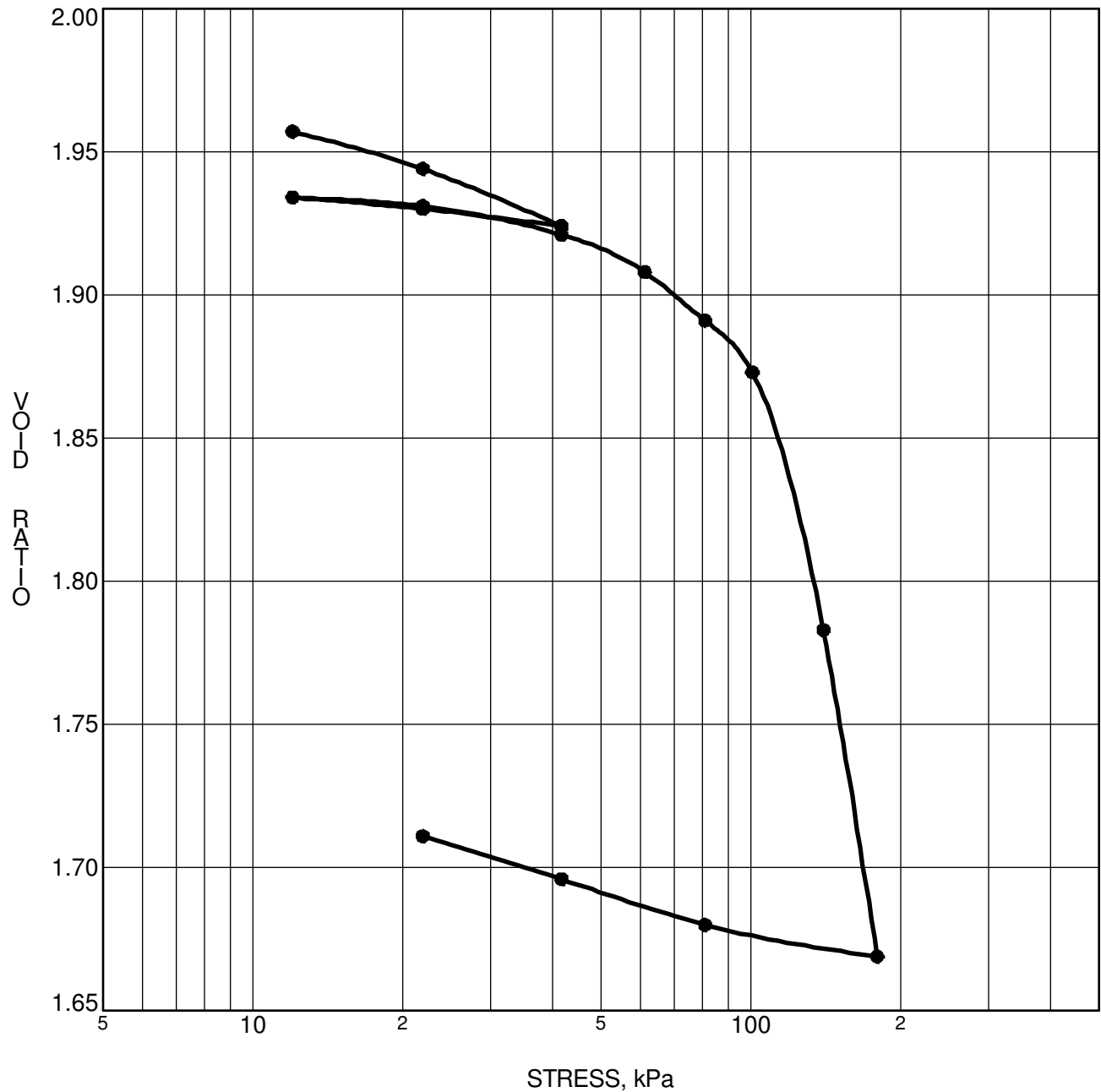
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 4	p'_o	55 kPa	C_{cr}	0.019
Sample No.	TW 4	p'_c	103 kPa	C_c	2.146
Sample Depth	4.28 m	OC Ratio	1.9	W_o	74.9 %
Sample Elev.	88.53 m	Void Ratio	2.061	Unit Wt.	15.8 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **22/10/2012**

patersongroup Consulting Engineers
 154 Colonnade Road South, Ottawa, Ontario K2E 7J5

CONSOLIDATION TEST



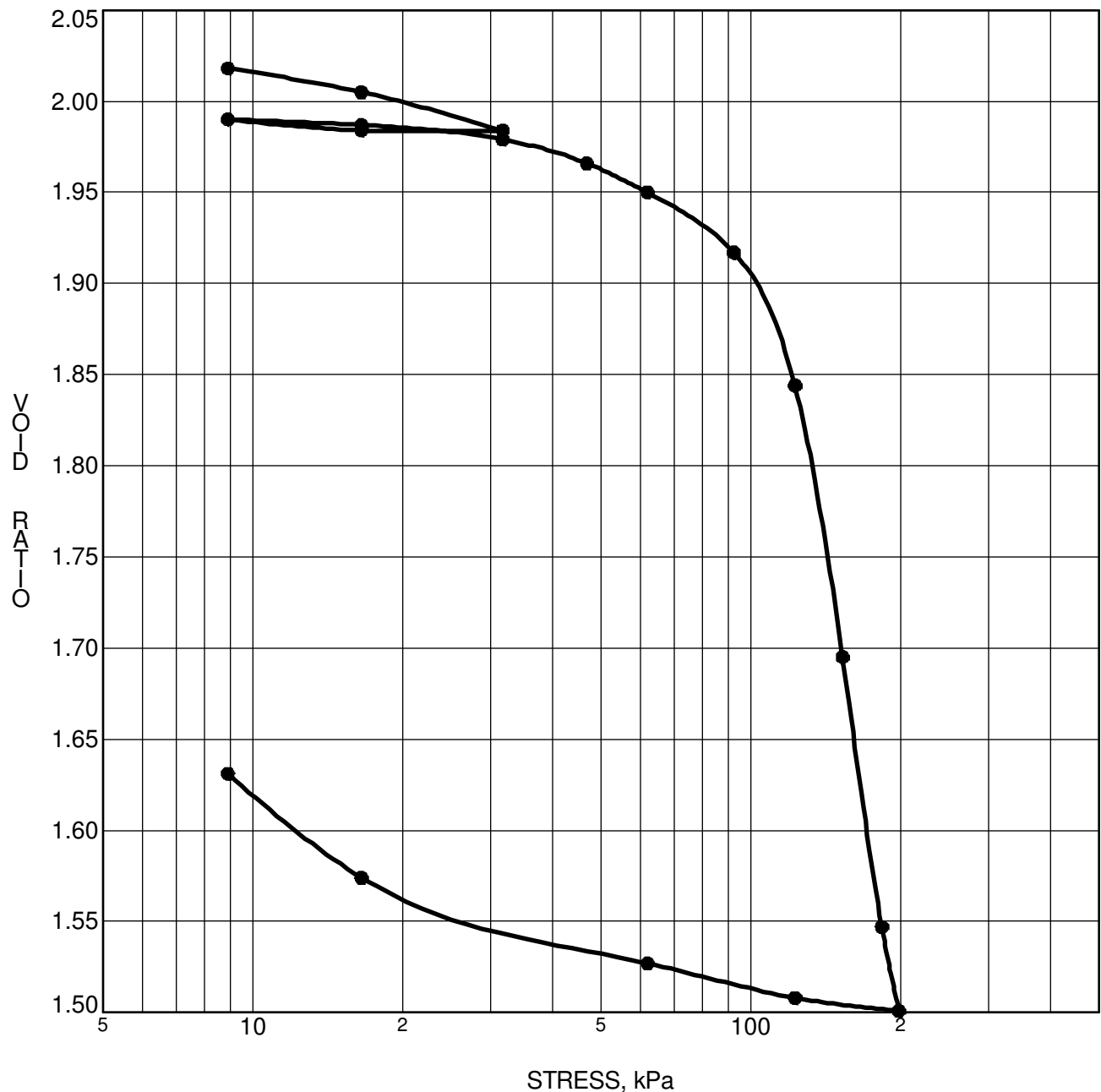
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 6	p'_o	63 kPa	C_{cr}	0.022
Sample No.	TW 3	p'_c	119 kPa	C_c	1.064
Sample Depth	5.03 m	OC Ratio	1.9	W_o	71.7 %
Sample Elev.	87.16 m	Void Ratio	1.971	Unit Wt.	16.0 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **22/12/2012**

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CONSOLIDATION TEST



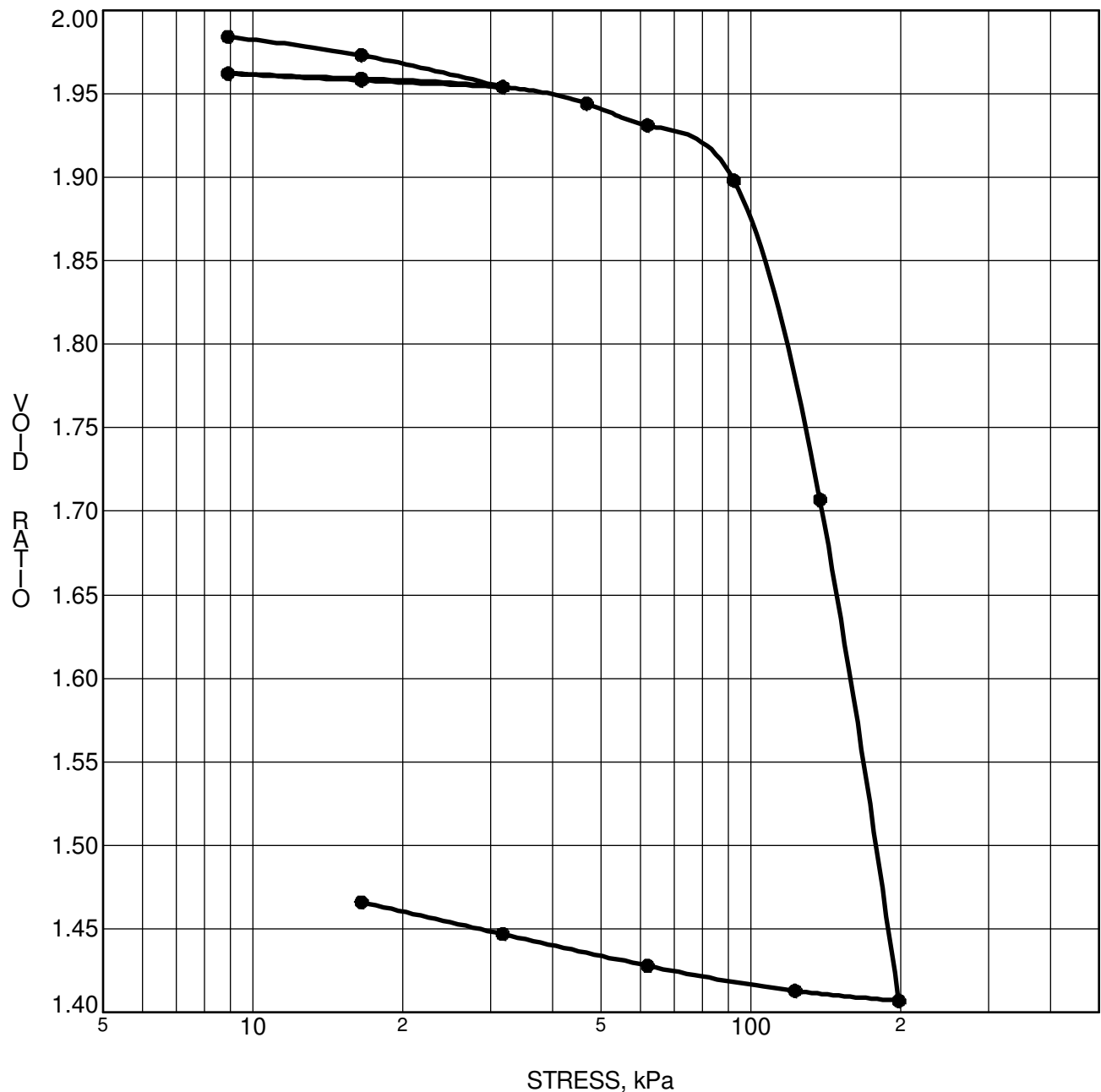
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 7	p'_o	68 kPa	C_{cr}	0.016
Sample No.	TW 3	p'_c	113 kPa	C_c	1.683
Sample Depth	5.03 m	OC Ratio	1.7	W_o	74.0 %
Sample Elev.	87.35 m	Void Ratio	2.034	Unit Wt.	15.8 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **29/10/2012**

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 154 Colonnade Road South, Ottawa, Ontario K2E 7J5

CONSOLIDATION TEST



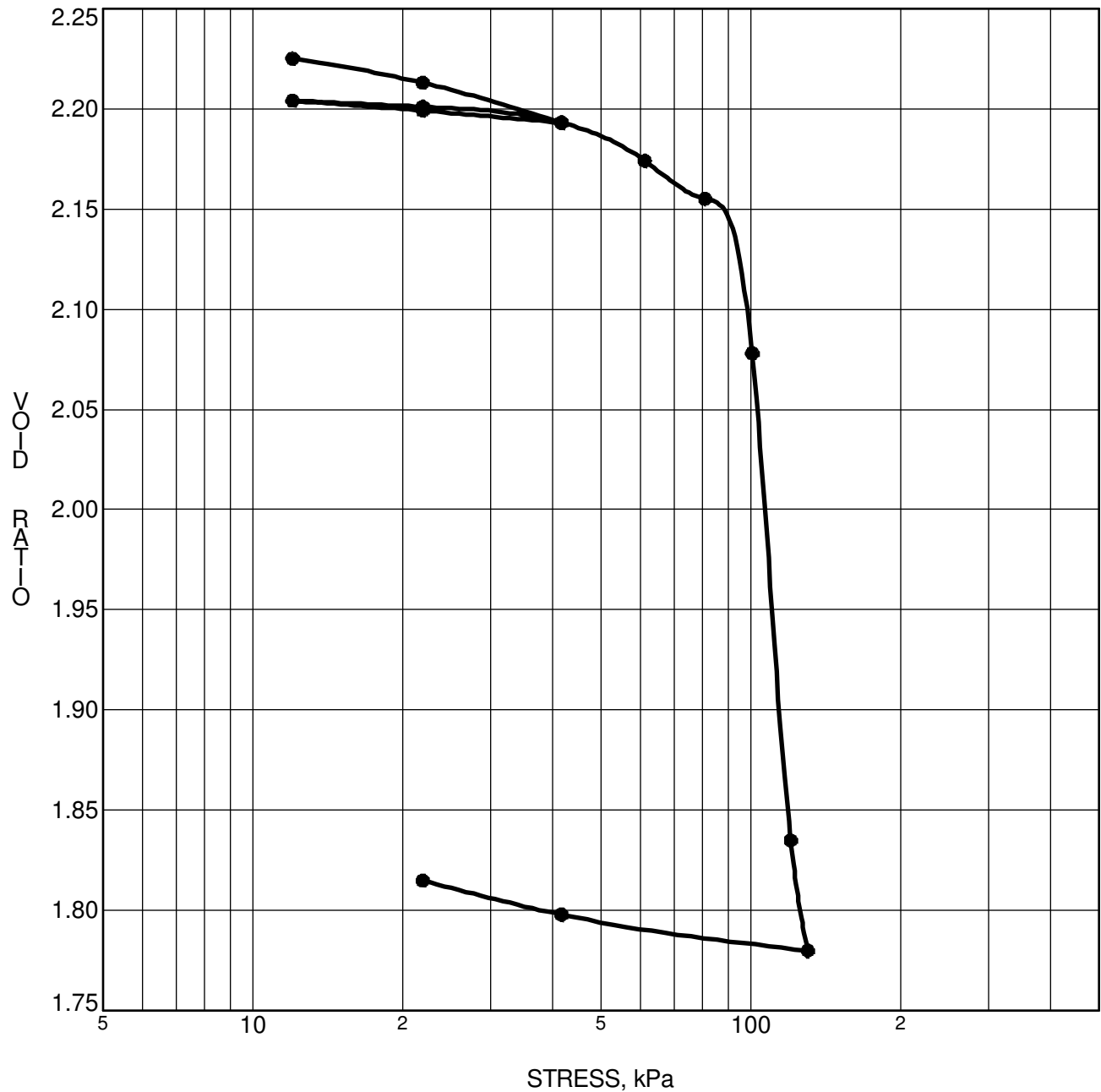
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 8	p'_o	62 kPa	C_{cr}	0.015
Sample No.	TW 3	p'_c	111 kPa	C_c	2.000
Sample Depth	5.10 m	OC Ratio	1.8	W_o	72.6 %
Sample Elev.	87.78 m	Void Ratio	1.996	Unit Wt.	15.9 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **22/10/2012**

patersongroup Consulting Engineers
 154 Colonnade Road South, Ottawa, Ontario K2E 7J5

CONSOLIDATION TEST



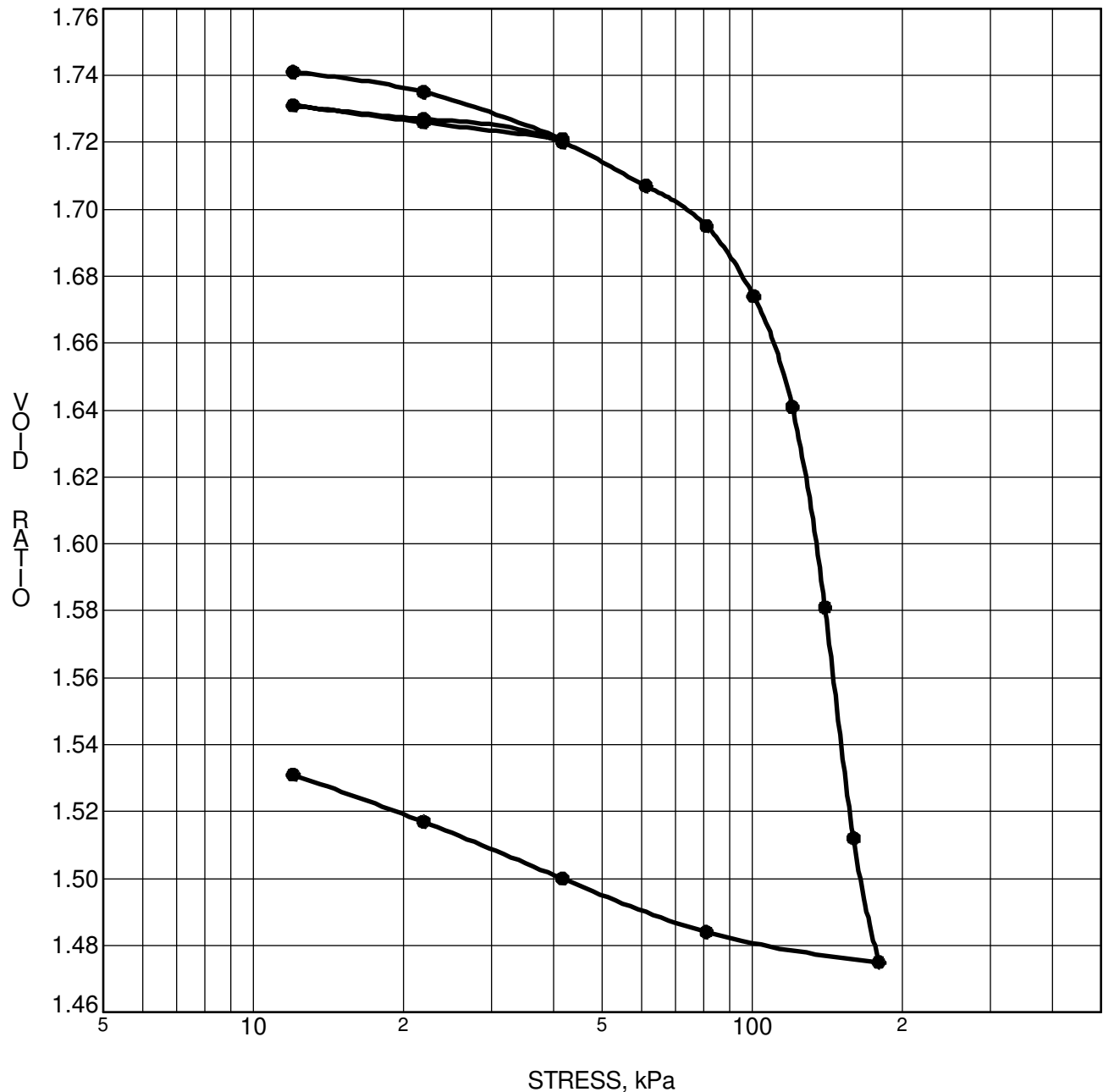
CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH 9	p'_o	54 kPa	C_{cr}	0.022
Sample No.	TW 5	p'_c	97 kPa	C_c	3.224
Sample Depth	4.32 m	OC Ratio	1.8	W_o	81.4 %
Sample Elev.	88.32 m	Void Ratio	2.237	Unit Wt.	15.5 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **12/11/2012**

patersongroup Consulting Engineers
 154 Colonnade Road South, Ottawa, Ontario K2E 7J5

CONSOLIDATION TEST



CONSOLIDATION TEST DATA SUMMARY					
Borehole No.	BH11	p'_o	58 kPa	C_{cr}	0.014
Sample No.	TW 1	p'_c	119 kPa	C_c	1.253
Sample Depth	4.29 m	OC Ratio	2.1	W_o	63.7 %
Sample Elev.	88.41 m	Void Ratio	1.753	Unit Wt.	16.4 kN/m³

CLIENT **Mattamy Homes**
 PROJECT **Geotechnical Investigation - Prop. Residential**
Development - 3288 Greenbank Rd.

FILE NO. **PG2743**
 DATE **29/10/2012**

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CONSOLIDATION TEST

APPENDIX 2

FIGURE 1 - KEY PLAN

DRAWING PG2743-2 - PERMISSIBLE GRADE RAISE AREAS - HOUSING

DRAWING PG2743-3 - PERMISSIBLE GRADE RAISE AREAS - APARTMENT BLDG.

DRAWING PG2743-4 - TEST HOLE LOCATION PLAN

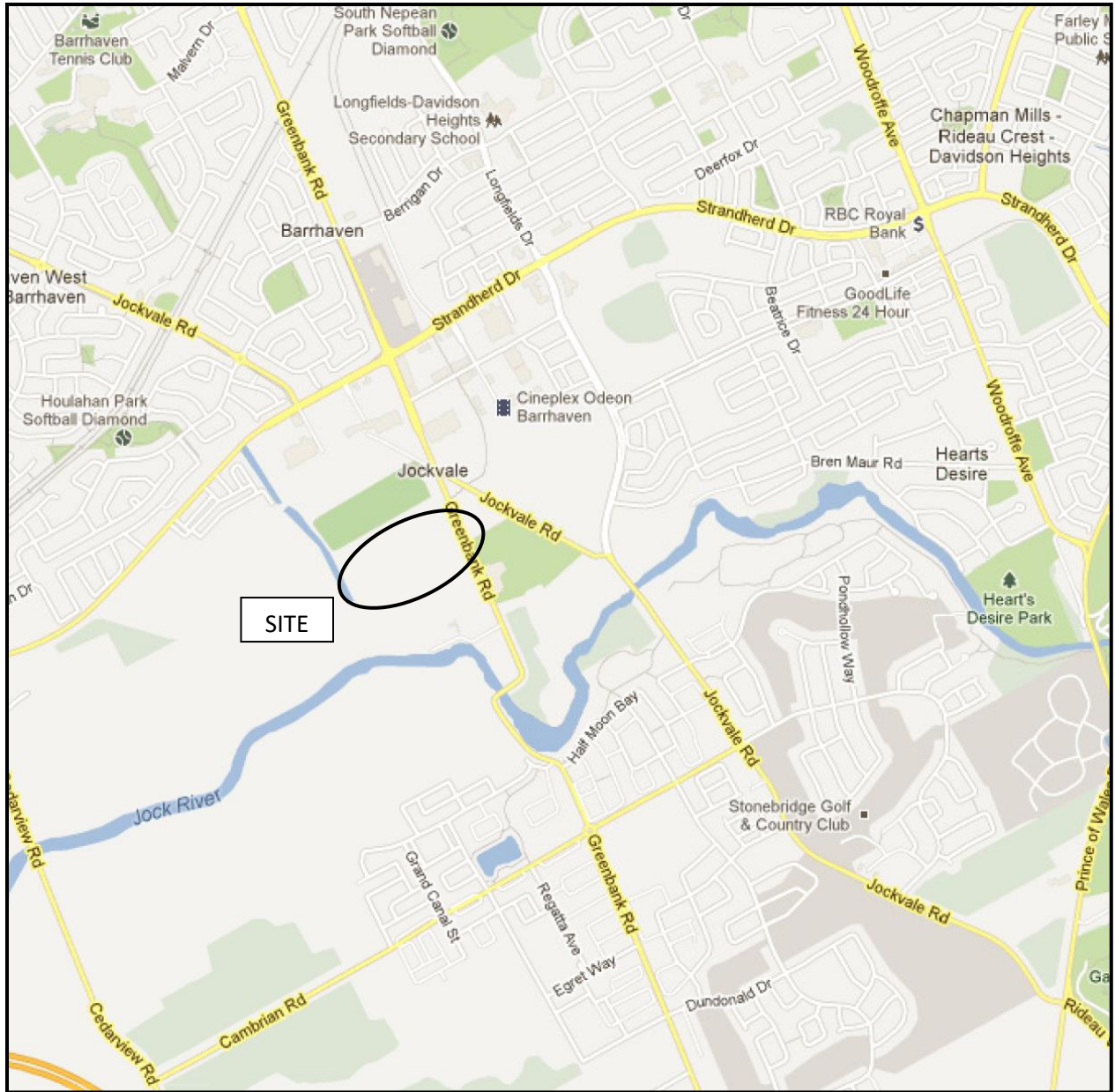
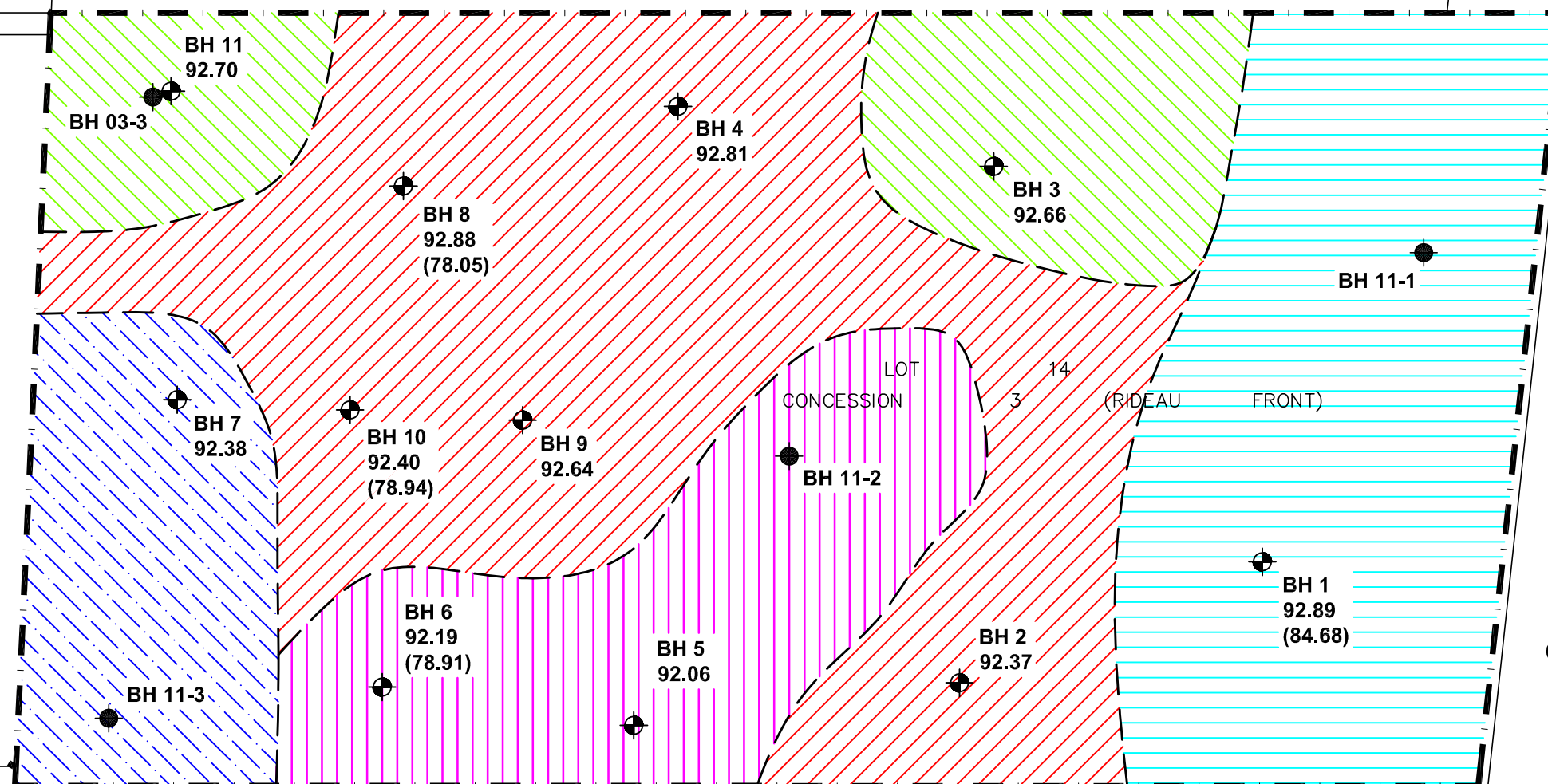
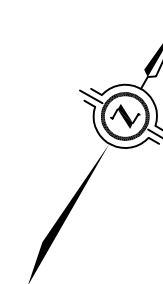


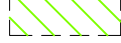

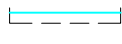




FIGURE 1
KEY PLAN



PERMISSIBLE GRADE RAISES:

-  UP TO 1.4m
-  UP TO 1.6m
-  UP TO 1.8m
-  UP TO 2.0m
-  UP TO 3.0m

LEGEND:

-  BOREHOLE LOCATION
-  APPROX. BOREHOLE LOCATIONS BY GOLDER ASSOCIATED LTD.
- 92.89 GROUND SURFACE ELEVATION (m)
- (84.68) PRACTICAL REFUSAL TO AUGERING / DCPT ELEVATION (m)

BASE PLAN, TEST HOLE LOCATIONS AND GROUND SURFACE ELEVATIONS AT TEST HOLE LOCATIONS PROVIDED BY J.D. BARNES LIMITED.

SCALE: 1:2000



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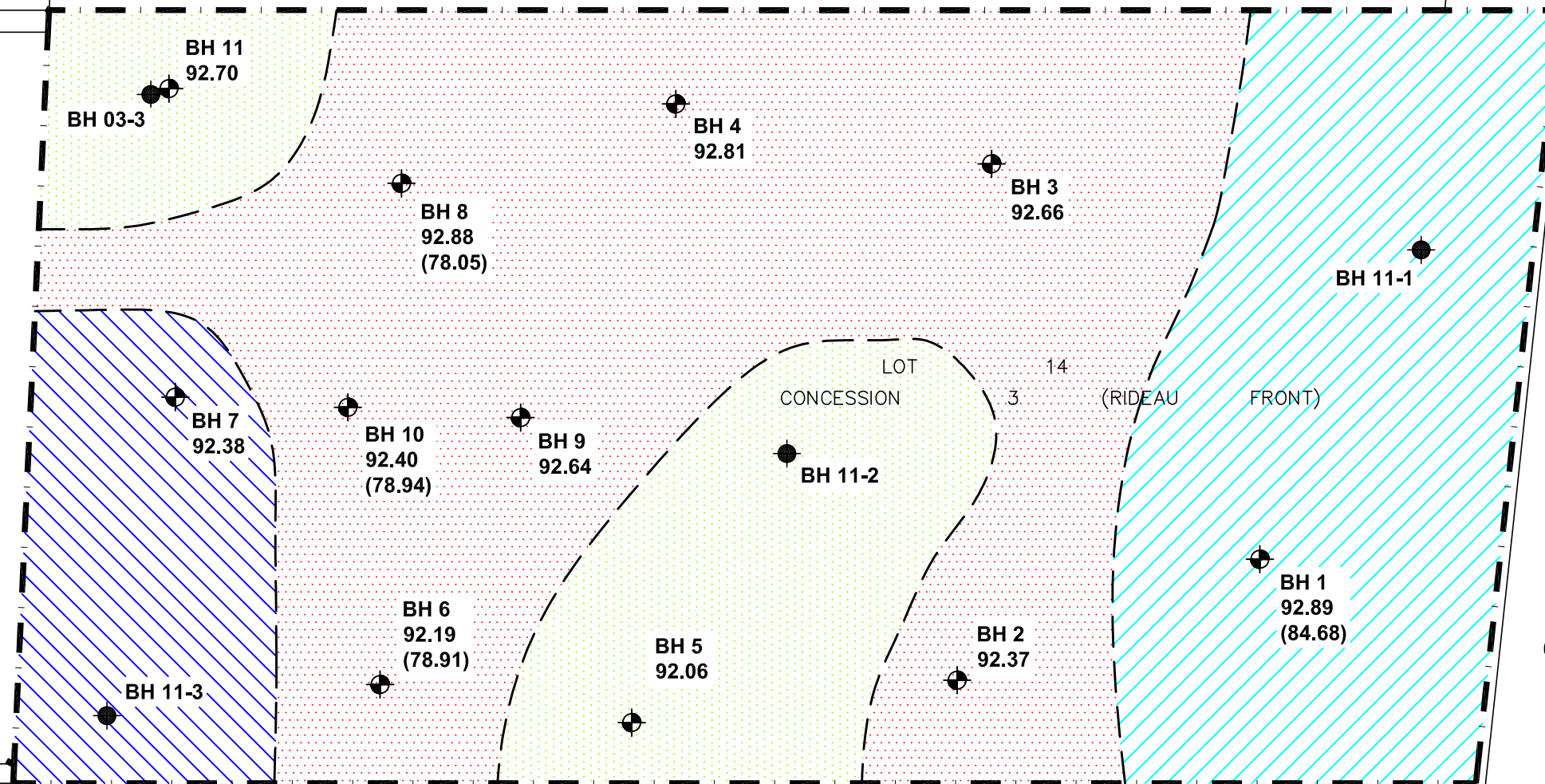
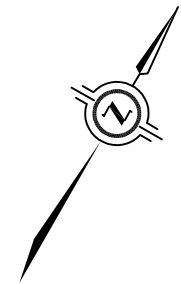
154 Colonnade Road South
Ottawa, Ontario K2E 7J5
Tel: (613) 226-7381 Fax: (613) 226-6344

NO.	REVISIONS	DATE	INITIAL

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - 2388 GREENBANK ROAD
OTTAWA, ONTARIO
Title: **PERMISSIBLE GRADE RAISE PLAN - HOUSING**

Scale:	1:2000	Date:	11/2012
Drawn by:	MPG	Report No.:	PG2743-1
Checked by:	DJG	PG2743-2	Revision No.:
Approved by:	DJG		

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GREENBANK ROAD

PERMISSIBLE GRADE RAISES:

- UP TO 0.8m
- UP TO 1.2m
- UP TO 1.5m
- UP TO 2.5m

LEGEND:

- BOREHOLE LOCATION
- APPROX. BOREHOLE LOCATIONS BY GOLDER ASSOCIATED LTD.
- 92.89 GROUND SURFACE ELEVATION (m)
- (84.68) PRACTICAL REFUSAL TO AUGERING / DCPT ELEVATION (m)

BASE PLAN, TEST HOLE LOCATIONS AND GROUND SURFACE ELEVATIONS AT TEST HOLE LOCATIONS PROVIDED BY J.D. BARNES LIMITED.

SCALE: 1:2000



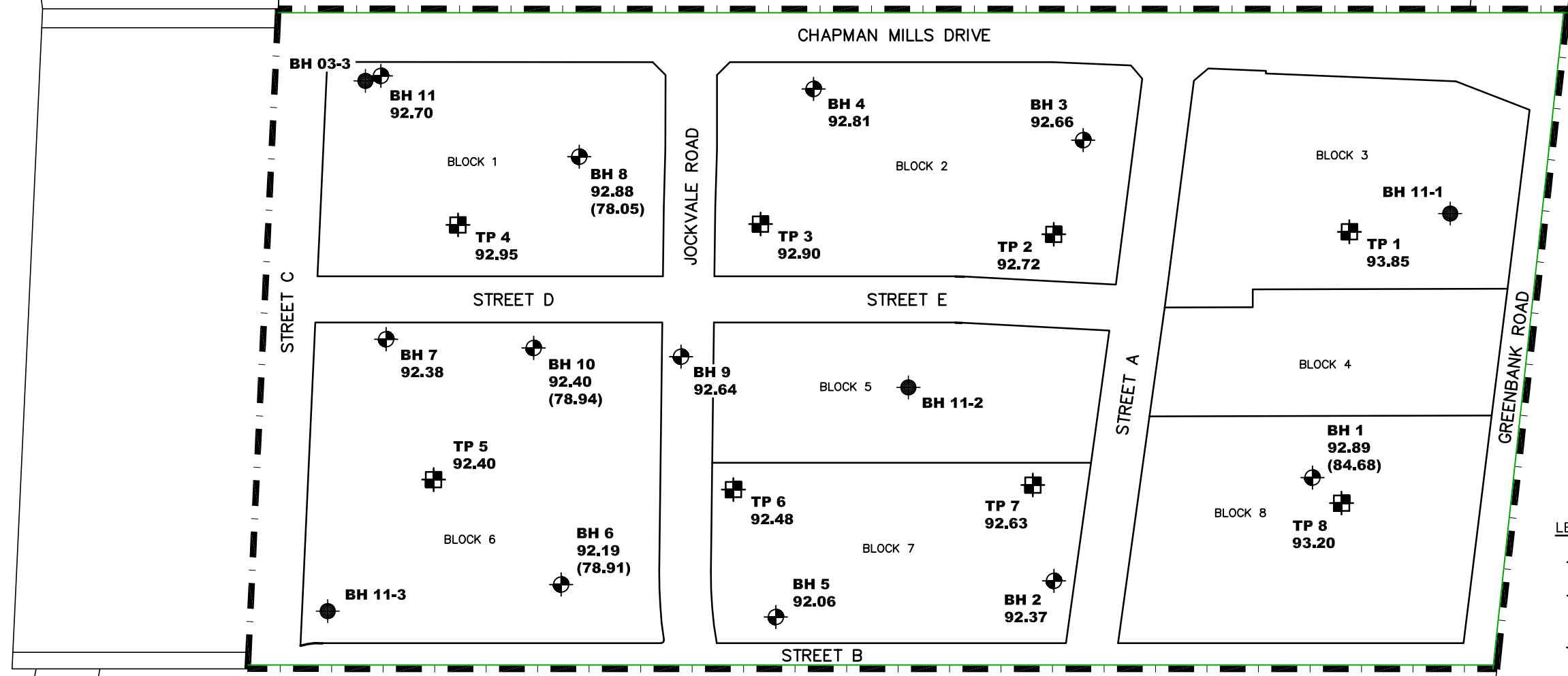
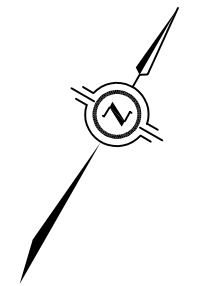
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NO.	REVISIONS	DATE	INITIAL

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - 2388 GREENBANK ROAD
OTTAWA, ONTARIO
Title: **PERMISSIBLE GRADE RAISE PLAN - APARTMENT BLDG.**

Scale:	1:2000	Date:	11/2012
Drawn by:	MPG	Report No.:	PG2743-1
Checked by:	DJG	PG2743-3	Revision No.:
Approved by:	DJG		



- LEGEND:**
- TEST PIT LOCATION, CURRENT INVESTIGATION
 - BOREHOLE LOCATION, PATERSON GROUP REPORT PG2743, 2012
 - BOREHOLE LOCATION BY OTHERS
 - 92.89 GROUND SURFACE ELEVATION (m)
 - (84.68) PRACTICAL REFUSAL TO DCPT/AUGERING ELEVATION (m)

BASE PLAN, TEST HOLE LOCATIONS AND GROUND SURFACE ELEVATIONS PROVIDED BY J.D. BARNES LTD.

SCALE: 1:2000

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Ottawa, Ontario K2E 7J5
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NO.	REVISIONS	DATE	INITIAL

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROP. RESIDENTIAL DEVELOPMENT - 2388 GREENBANK ROAD

OTTAWA, ONTARIO

Title: **TEST HOLE LOCATION PLAN**

Scale:	1:2000	Date:	03/2019
Drawn by:	MPG	Report No.:	PG2743
Checked by:	JV	PG2743-4	Revision No.:
Approved by:	DJG		

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**SOUTH NEPEAN TOWN CENTRE (SNTC) BLOCK 4 – SITE SERVICING AND STORMWATER
MANAGEMENT REPORT**

Appendix E Drawings

Appendix E **DRAWINGS**

