

March 13, 2020

Our File Ref.: 170132.04

The Hindu Temple of Ottawa Carleton Inc. 4835 Bank Street Ottawa, Ontario K1X 1G6

Attention: Mr. Harish Gupta

Subject: Terrain Analysis and Private Sewage Disposal System Impact Assessment - Proposed Assembly Hall The Hindu Heritage Centre of Ottawa Carleton, 4835 Bank Street, Ottawa, Ontario

Dear Mr. Gupta,

LRL Associates Ltd. (LRL) has conducted a Terrain Analysis and Private Sewage Disposal System Impact Study for the proposed Hindu Heritage Centre of Ottawa Carleton Assembly Hall to be constructed on the property located at 4835 Bank Street, Ottawa, Ontario (herein referred to as the "Site"). It is understood that it is proposed that a 1,593 m² assembly hall be constructed at the eastern portion of the existing developed property which will have an available capacity of approximately 500 individuals, increasing the total occupancy of the Site to approximately 750.

The proposed assembly hall will be supplied by municipal water supply and a private septic as is the existing development on the Site.

The assessment was carried out to determine if the proposed development:

- Has soil conditions that are suitable for onsite sewage disposal; and
- Will not impair the use of groundwater resources on the Site or on adjacent lands.

The proposed sewage system for the proposed assembly hall building will be designed for approximately 4.0 m^3 /day of wastewater, as outlined below. No changes to the two existing sewage systems that service the existing temple are proposed.

The assessment involved a desktop review of available information on the geology and hydrogeology of the Site and adjacent lands in addition to an intrusive subsurface investigation (test pitting program). The Site is serviced by municipal water supply, however, neighbouring properties within 500 m of the Site were found to have records of supply wells present.

1 SITE AND AREA DESCRIPTION

The property is situated at the southern extent of the City of Ottawa at 4385 Bank Street. The property is legally described as Part Lot 22, Concession 5RF Gloucester Parts 1 & 2, 5R3156.

The location of the subject site is shown in **Figure 1**. The Site's area is approximately 3.8 hectares (9.4 acres). The property is currently occupied by the Hindu Heritage Centre of Ottawa Carleton. The footprints of the existing temple building and garage are approximately 1,168 m² and 105 m², respectively. The existing temple is located at the western extent of the Site with the associated septic systems to the north and south of the temple. Based on the previously prepared Use Permit, dated December 5, 1985, issued by the MECP, and associated application, the existing sewage disposal system includes two septic systems: one (1) to service the existing kitchen and washrooms and one (1) to service the remainder of the existing temple building. Each system was originally designed for 3,750 L/day, based on an assumed occupancy of 250 individuals and the use of 15 L/day per individual. However, since no food services are present in the building and none are proposed the use of 8 L/day per individual instead of 15 L/day per individual is deemed more appropriate. This yields a daily sewage flow of 2,000 L/day for each of the existing systems.

The neighbouring land use is as follows:

- Bank street, followed by light industrial/commercial business to the west; and
- Vacant/treed land to the north, south and east.

The topography of the land is generally flat with an approximate elevation of 97 m above mean sea level.

These site features are shown in the **Figure 2**.

2 PROPOSED DEVELOPMENT

It is anticipated that an assembly hall will be constructed at the eastern portion of the Site with the associated septic system along the south of the proposed structure. The estimated proposed building footprint is 1,593 m². The proposed assembly hall is anticipated to include a dining area, a lobby and two (2) halls. No food services are proposed. The proposed development will be equipped with a full basement. Water supply will be obtained from municipal services.

It is proposed that 14 additional parking spaces be created, for a total of 187 parking spaces with a total parking and circulation area of 4,996 m².

The approximate preliminary proposed development plan is shown in **Figure 3**. Further revisions with regards to the proposed septic system footprint and components may be required at a later date.

3 FIELDWORK

On May 8th, 2017, eight (8) test pits were advanced across the Site. The test pits were placed around the general perimeter of the Site so not to disrupt existing Site activities and services. The rationale for the test pits was to determine the general upper soil and perched water conditions. The test pits were advanced using a backhoe operated by a local contractor (Yelle Excavation, Ottawa) and under direct supervision by LRL field staff. The locations of the test pits are presented in **Figure 4** with the test pit logs included in **Appendix A**.

An open tube piezometer was installed in select test pits (TP1, TP3, TP5 and TP7) to allow for the groundwater elevation measurement and sampling of the perched water found in the overburden, herein referred to as groundwater. Groundwater samples were collected from each of the piezometers on May 8th, 2017, with the exception of TP5 which was found to have insufficient water available for sampling (i.e., dry). The samples collected were submitted for laboratory analyses of select nitrogen species parameters. The laboratory Certificate of Analysis is included in **Appendix B**.

Soil samples from two test pits were submitted to LRL's material testing laboratory for sieve and hydrometer analysis certificates are included in **Appendix C**.

A ground surface elevation survey was carried out at each test pit location to obtain the elevation of the test pit ground surface and the piezometer stick-up. These elevations would aid in determining the groundwater elevations across the Site. A locally referenced benchmark was established as the top of the east arm of the hydrant located along the west of the southern entrance to the Site. The benchmark was assigned an arbitrary elevation of 100.00 m. The elevations are summarized in **Table 1** and are presented in the test pit logs included in **Appendix A**.

4 TOPOGRAPHY, GEOLOGY AND HYDROGEOLOGY

Local topography indicates that the inferred overburden groundwater flow direction is east towards the North Castor River. The nearest open water body to the Site is an unnamed tributary that flows into the North Castor River, approximately 1.1 km east of the Site.

Surficial soil deposit mapping¹ indicates that the overburden consists of till, plain with local relief less than 5 m. Bedrock mapping² indicates that the underlying bedrock consists of dolomite and limestone, of the Oxford Formation.

The test pits completed across the Site were found to have a thin layer of topsoil over fill material which extended to depths between 0.7 and 1.5 m thick. The fill was underlain with silty sand in TP1. The fill layer generally extended to bedrock refusal, encountered at depths from 0.8 to 2.1 m bgs. Waste debris was observed in the fill material in TP2, TP3 and TP5, which included metal, tire debris and asphalt.

A representative till sample collected during the test pitting activities (TP3-6) was submitted for sieve analysis. The till sample was reported to be 39% silt & clay, 40% sand, and 21% gravel. This represents fine silty sand. A second representative till sample collected (TP1-3) was submitted for hydrometer analysis. The sample was reported to be 22% clay, 64% silt, 9% sand and 5% gravel. This represents a silt loam. These results are presented in the sieve and hydrometer certificates of analysis that are included in **Appendix C** and are summarized in **Table 2**.

LRL was provided with a servicing plan showing the properties within 500 m with a municipal water connection. The plan shows that the majority of properties are on municipal services, with some properties that are spread over multiple parcels having only one (1) connection. It is likely that if these properties are developed in the future, connection to municipal water would be

¹ St-Onge, D.A. (compilation), 2009: Surficial geology, lower Ottawa valley, Ontario-Quebec; Geological Survey of Canada, Map 2140A, scale 1:125000

² Harrison, J.E., 1976. Geological Survey of Canada, Generalized Bedrock Geology, Ottawa-Hull, Ontario and Quebec, Map 1508A, scale 1:125000.

required. One (1) residential property and various industrial properties are situated along the south side of Blais Road to the north, which are unserviced.

A search was conducted of the available water well records from the MECP Water Well Record Department. The search by UTM coordinates covered a 500 m radius from the site. The search returned records for twenty-three (23) wells. The well records are included in **Appendix D** and their locations are presented in **Figure 5**.

Review of the records of the wells within 500 m of the site retrieved revealed that the wells are drilled wells extending to depths between 8.2 and 67.1 m. The well records show that that the geological conditions within 500 m are relatively similar, and consist generally of mixed till materials including sand, clay, gravel and boulders from 0 to 8.0 m. Unidentified soil conditions, "soil" was described in one (1) of the well records, as noted in the table below. The described bedrock conditions varied slightly between limestone, sandstone and occasionally shale. Bedrock starting depths also vary from 0.6 to 7.9 m.

The general subsurface conditions indicated in the well records within 500 m of the site are as follows:

MOE	Distance and	Depth	C	Overburden Deta	ails	Bedrock Details	Groundwater	Static Water	Type of
Well Number	Direction from Site (m)	(m)	Sand/ Fill (m)	Clay/ Loam (m)	Gravel/ Till (m)	Bedrock	Encountered (m)	Level (m)	water
1502181	210 N	14.0			0 - 6.4	6.4- 14.0 (Limestone)	14.0	2.4	Fresh
7112950	485 N	52.7		0 – 3.3		3.3 – 52.7 (Limestone)	51.5	4.7	Unspecified
1533566	385 N					2.1 – 29.8 (Sandstone)			
		67.1	0 – 2.1			29.8 - 38.7 (Limestone)	65.8	4.8	Unspecified
						38.7 - 67.1 (Sandstone)			
1531693	385 N	67.1			0 – 0.9	0.9 – 67.1 (Sandstone)	62.7	9.1	Fresh
1502249	370 N	25.9	0 – 1.2			1.2 – 25.9 (Sandstone)	25.2	4.5	Unspecified
1502248	330 N	29.9	0 - 0.3	0.3 – 1.8		1.8 – 29.9 (Sandstone)	24.3, 29.5	4.2	Fresh
1502246	335 N	24.4			0 – 1.5	1.5 – 24.4 (Sandstone)	9.1, 18.2, 30.1	1.5	Fresh
1517349	260 N	8.2	0 - 2.4			2.4 – 8.2 (Granite)	8.2	1.5	Fresh
1509925	215 N	19.2			0 – 3.9 "Boulders"	3.9 – 19.2 (Sandstone)	18.2	0.6	Fresh
1502175	360 NW	18.3	0-6.0			6.0 – 18.3 (Sandstone)	18.3	3.0	Fresh
1502176	250 NM	13.7		0-5.4		5.4 – 13.7 (Limestone)	13.7	1.8	Fresh
1502179	50 W	27.1			0 – 4.8	4.8- 7.62 (Limestone) 7.62 – 27.1 (Sandstone)	27.1	6.1	Fresh

MOE and Distance Well Direction		Depth	(Overburden Deta	ails	Bedrock Details	Groundwater	Static Water	Type of
Number	from Site (m)	(m)	Sand/ Fill (m)	Clay/ Loam (m)	Gravel/ Till (m)	Bedrock	Encountered (m)	Level (m)	water
1513436	100 SW	15.0		0 – 3.6 "Soil"	3.6 - 4.8	4.8 – 15 (Limestone)	14.6	4.3	Fresh
1502180	140 S	16.8		0 – 1.8 "Loam"		1.8 – 16.8 (Limestone)	16.8	1.8	Fresh
1502177	195 S	18.2	0 – 2.1		2.1 – 6.1	6.1 – 18.2 (Sandstone)	18.2	1.8	Fresh
1512375	230 S	22.5	0 - 2.7			2.7 – 22.5 (Sandstone)	22.5	3.6	Fresh
1512265	245 S	14.6		0 – 0.9		0.9 – 14.6 (Limestone)	2.4, 6.4, 10.3	1.2	Fresh
1514664	220 SW	15.2			0 – 3.9	3.9 – 9.1 (Shale) 9.1 – 38.1 (Limestone)	9.7, 16.7	6.1	Fresh
1516052	15 S	54.2	0 - 2.8		2.8 – 7.9	7.9 – 13.1 (Limestone) 13.1 – 54.4 (Sandstone)	53.3	9.1	Fresh
1502178	310 SW	15.2			0 – 5.4	5.4 – 15.2 (Limestone)	14.6	3.9	Fresh
1510717	400 S	15.8	0 – 1.8			1.8 – 15.8 (Limestone)	15.2	2.1	Fresh
1514840	370 S	41.1	0 – 0.9 "Topsoil"			0.9 – 41.1 (Limestone)	32.0	6.0	Fresh
1502250	370 S	24.1		0 – 0.6 "Loam"		0.6 – 19.8 (Sandstone) 19.8 – 24.0 (Granite)	18.2, 24.0	6.0	Fresh

4.1 Groundwater from Test Pits

Table 3 summarizes the water quality analysis from the test pit piezometers for nitrates, nitrites, ammonia and total kjeldahl nitrogen (TKN). The Laboratory Certificate of Analysis is included in **Appendix B**.

Nitrites were not detected (<0.05 mg/L) in any of the groundwater samples collected. Nitrate levels were reported to be 0.5 mg/L in TP3 and <0.1 mg/L in both TP1 and TP7, below the ODWS of 10 mg/L. Ammonia was reported to be 0.28, 0.39 and 1.66 mg/L in TP1, TP3 and TP7, respectively. There are no set ODWS for ammonia.

TKN values were reported as 78.1, 65.3 and 131 mg/L in groundwater samples collected from TP1, TP3 and TP7, respectively. There are no set ODWS for TKN; however, based on the measured groundwater levels and corresponding elevations, the overburden groundwater flow direction is inferred to the north. TP7 is located along the extents of one of the existing septic beds on the property, and additionally, during the advancement of TP7, organic material including a tree stump was encountered. Both of which (septic and other organic decomposition) could contribute to the elevated levels of TKN across the central portion of the Site.

At the time of construction, it is recommended that an additional intrusive investigation be implemented (i.e., installation of groundwater monitoring wells) to further delineate and establish the extents of the elevated TKN and to determine whether the previously elevated level is an isolated event. The results of the additional investigation will be used in the design of the sewage disposal systems.

5 RECEIVING GROUNDWATER

The current and potential uses of the aquifers are identified below.

5.1 Overburden Groundwater

The overburden groundwater is unlikely to be used as a water supply based on the following:

- The Site and the adjacent properties are currently serviced by municipal water although water well records were identified in the area.
- Based on the well records reviewed and the shallow overburden conditions, no shallow wells were identified on the subject site or adjacent lands. Generally, the overburden conditions are not suitable for construction of a well.
- The buildings in this area are serviced by private septic systems; therefore, the current use of the overburden groundwater is for the attenuation of the septic system effluent.

5.2 Bedrock Aquifer

Twenty-three (23) well records were available for properties located within a 500 m radius of the Site. The records indicate that all twenty-three (23) wells tap into bedrock aquifer. Although it is our understanding that municipal water is available for the neighbouring properties, it is unknown at this time if these wells are still present or continue to be used for potable purposes.

6 TERRAIN ANALYSIS AND SEPTIC DESIGN

The terrain analysis was conducted to demonstrate that the unconsolidated material on the Site is appropriate for the construction of an on-site subsurface sewage disposal system, with consideration taken regarding the existing installation.

The subsurface conditions indicated for the Site are considered suitable for a Class IV sewage disposal system with a fully raised leaching bed depending on the lot specific soil and groundwater conditions at the actual location of the proposed septic system leaching bed. The leaching bed should be constructed to conform to the specifications set out in the Ontario Building Code (OBC). As part of this assessment, an analysis was carried out to ensure that sufficient space exists at the Site for the construction of a third septic system in accordance with the OBC which will service the proposed assembly hall.

As previously mentioned, the existing temple is serviced with two (2) sewage disposal systems located at the north and south sides of the buildings, respectively. Both are constructed with 9,000 L fibreglass septic tanks and 8 runs of 13.3 m length piping. One (1) of the systems services the kitchen and washrooms and the other services the remainder of the existing temple building. The existing septic systems were each designed for a sewage flow of 3,750 L/day, based on the assumption of 250 individuals and the use of 15 L/day per individual. However, since no food services are present in the building and none are proposed, the use of 8 L/day per individual

instead of 15 L/day per individual is deemed more appropriate. This yields a daily sewage flow of 2,000 L for each of the existing systems.

The daily sewage flow for the proposed assembly hall is based on the assumption that 500 individuals will occupy the building. In accordance with Schedule 8 of the OBC, it is assumed that 8 L/day will be discharged into the septic system for each individual that occupies the building. This is the set value for an assembly hall not equipped with food services. As a conservative approach to determine the expected largest septic system envelope required to service the proposed assembly hall, a septic system envelope size was calculated assuming a fully raised bed with mantle, a percolation rate of 12 min/cm for the imported sand required and a daily sewage flow of 4,000 L. The total length of pipe required for the proposed septic bed for the proposed assembly hall, assuming imported fill, was calculated as approximately 240 m using the following equation:

L = QT/200

where L = length of pipe (m);

Q = daily sewage flow for the proposed assembly hall (L/day); and

T = percolation rate of the imported sand fill material (min/cm).

Therefore, an area of approximately 360 m² is required for the septic bed assuming 16 pipes, each having a length of 15 m and a spacing of 1.6 m between the pipes. A mantle of 15 m in length would be required along the downgradient portion of the bed. Based on the total coverage of the septic bed (raised portion and mantle plus a replacement area) an area of approximately 1,215 m² would be required. This is a conservative approach based on the OBC.

However, due to the total sewage demand of the existing and proposed buildings (8,000 L/day) and available infiltration area on the site (15,888 m²), a conventional system for the proposed assembly hall is not adequate and tertiary treatment is necessary. It is proposed that a tertiary system such as an Ecoflo® Biofilter be considered for the new assembly hall. No changes to the existing systems servicing the existing building are proposed.

A preliminary design has been provided by Premier Tech Aqua for an Ecoflo® Biofilter system. The Ecoflo® system includes one (1) 12,000 L septic tank and two (2) STB730PR Ecoflo® Biofilters. The effluent will be pumped from the biofilters to an absorption system, consisting of a 0.3 m thick stone layer underlain with a 0.3 m thick sand layer. The stone layer shall be such that the loading on the surface of the stone layer does not exceed 50 L/m² per day for a total daily design sanitary sewage flow exceeding 3,000 L. Therefore, the minimum stone layer area is calculated as follows:

A = Q/50

where Q = daily sewage flow for the proposed assembly hall (L/day).

This gives a minimum area of the stone layer of 80 m^2 . It is proposed that a stone layer of 8 m length by 10 m width be used. The effluent would be pumped through eight (8) distribution pipes installed on top of the stone layer, each of 9 m in length and spaced 1 m apart.

The stone layer is to be installed on a sand bed. The minimum area of the sand layer is calculated as follows:

A = QT/400

Where Q = daily sewage flow for the proposed assembly hall (L/day)

T = percolation rate of the imported sand fill material (min/cm); assumed as worst case of 50 min/cm².

This gives a minimum area required for the sand layer of 500 m². It is proposed that a sand layer of 16 m wide by 31.25 m length be used. This gives a mantle length of 20.25 m.

The preliminary configuration for this design is presented in the included **Figure 7**. It is stressed that this is strictly for discussion purposes at this time and the final design may change, however it is anticipated that the approximate size requirement will not vary significantly.

6.1 Average Daily Water Demand Variance

It should be noted that the average daily water demand presented in the Site Servicing Report prepared by LRL, dated September 18, 2017 was calculated for the entire property using Section 7 of the OBC. The demand was calculated assuming a worst-case scenario where all fixtures at the property, both the existing and the proposed buildings, are turned on simultaneously at the applicable flowrate for each fixture as specified in the OBC. The purpose of this calculation is to size the piping required to service the site.

7 PRIVATE SEWAGE DISPOSAL SYSTEM IMPACT STUDY

The groundwater impact assessment addresses the ability of the land to attenuate the sewage effluent created by the development. Three methods for conducting the assessment are outlined in MOE's *Procedure D-5-4 Technical Guideline for Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment* (1996):

- Lot Size Consideration for lot greater than 10 000 m² (1 hectare);
- System Isolation Consideration for areas where the septic system is hydrogeologically isolated from the potable water source; and
- Contaminant Attenuation Consideration for sites that do not meet the above two points.

Bedrock was encountered at depths between less than 2.0 m across the site, therefore the site is considered hydrogeologically sensitive with areas of thin soil over highly permeable soils (i.e., bedrock).

The overburden material generally consisted of a fill material in the test pits with a stratum of till (TP3) or silty sand (TP1) above the bedrock in areas. The receiving groundwater for the septic system effluent is identified as the fill, silty sand and till. This groundwater is not considered an aquifer as it was encountered at depths less than 2.0 m below grade. As stated in Section 5.1, this groundwater is not a suitable supply aquifer for potable water based on its assumed poor yield, poor quality, shallow depth and likely used for the attenuation of the Site's existing and the neighbouring properties septic effluents. This groundwater is considered a suitable attenuation zone because alternative sources of water are available (i.e., municipal water or bedrock aquifer).

As mentioned above, the lot size is $38,000 \text{ m}^2$, with approximately $15,888 \text{ m}^2$ available for the installation of the proposed septic system. The lot size consideration for lots greater than $10,000 \text{ m}^2$ does not apply based on the anticipated total sewage demand of 8,000 L/day, including the existing systems. Therefore, "*Contamination Attenuation*" was considered in this terrain analysis.

The Site has a total area of 38,000 m². In accordance with Section 22.5.8 of the MECP Design Guidelines for Sewage Works, the stream which is identified to bisect the Site immediately east of the proposed development must be considered the extent of the allowable dilution area. It is

understood that a 20 m setback is required from the bank of the stream and any development on the Site. When the area of the proposed and existing building, septic systems, and other site features (parking facility), are taken into consideration, an area of approximately 3,300 m² is available for the placement of the septic disposal system. This is a sufficient area to accommodate the 500 m² "proposed" septic system, as shown in the proposed site development plan in **Figure 3** and the Available Area for Sewage Disposal (Conservative Approach) in **Figure 6**.

7.1 Contaminant Attenuation Method (Predictive Assessment)

The Contaminant Attenuation Method (Predictive Assessment) was used to determine the impact of the proposed on-Site septic systems at the boundary of the Site. This procedure assesses the risk that the individual on-site systems will cause the concentration of the nitrate-nitrogen exceed 10 mg/L at the property boundaries. Dilution is the attenuation mechanism considered for nitrates, with precipitation being the only source of infiltration. The following parameters and assumptions were used in the nitrate attenuation calculations:

- Infiltration factors for the site;
 - Flat topography;
 - Infiltration Factors:
 - i. An assumption of Sand was used for this calculation;
 - ii. Approximately 15,888 m² of the site is considered Cultivated Land;
 - Moisture Surplus:
 - i. The remaining cultivated land is considered Shallow Rooted Crops;
 - ii. An assumption of Fine Sand was used for this calculation;
 - The average background nitrate concentration was calculated to be 0.2 mg/L;
 - Impervious areas (existing and proposed) were calculated to be of 2,866 m² for the buildings and 4,996 m² of paved driveway and parking areas; and
 - Moisture surplus values from the Ottawa weather station (Environment Canada, 2011).

The moisture surplus printout is included in **Appendix E**. This location is considered representative of the site located at the south-central extent of the City of Ottawa, Ontario.

Based on the total proposed sewage volume for the entire Site of 8,000 L/day, the existing lot size, soil conditions, a nitrate concentration of the sewage of 40 mg/L, the calculated levels of nitrates at the property limits is estimated as 15.1 mg/L as presented in the attached **Table 4A**. This is above the procedure's guideline limit of 10 mg/L at the property line. Based on the *"Contaminant Attenuation Method"*, without tertiary treatment the current lot size and soil conditions are not suitable to attenuate the nitrate impacts generated by the septic systems of the development in accordance with D-5-4 guideline.

The above calculations are based on the current D-5-4 guideline which requires the use of 40 mg/L as the contaminant source as per Section 5.6.2 (a). Therefore, the use of an advanced tertiary treatment system such as Ecoflo Biofilter is necessary to reduce the levels of nitrates prior to discharge to the disposal field. This particular system is approved by the OBC and the Building Materials Evaluation Commission of the Ontario Ministry of Municipal Affairs and Housing. Furthermore, Section 5.7 of the D-5-4 guideline states that the Ministry recognises "that as

research continues, information and technologies may become available which warrant minor or substantial revisions to this guideline".

According to the report titled Wastewater Technology, NSF/ANSI standard 245 – Wastewater Treatment Systems – Nitrogen Reduction, prepared by Premier Tech Aqua, the Ecoflo Biofilter tertiary treatment system is capable of reduction of nitrates in the effluent nitrate concentrations to between 0.35 mg/L to 8.54 mg/L. A copy of the report is included in **Appendix F**. For the purpose of this assessment a conservative nitrate effluent concentration of 12 mg/L was used.

The calculated nitrates at the property line is estimated based on the daily sewage volume of the existing systems (2 x 2,000 L) and the daily sewage volume of the proposed system of 4,000 L, treated with an Ecoflo Bioreactor. The detailed calculations for the proposed development are presented in the attached **Table 4B**. It is assumed that the level of nitrates in the effluent from the existing system and the proposed Ecoflo Bioreactor are 40 mg/L and 12 mg/L, respectively. Based on these assumptions the nitrates at the property limits is estimated as 9.9 mg/L. This is below the procedure's guideline of 10.0 mg/L. Based on the "*Contaminant Attenuation Method*" the current lot size and soil conditions are suitable to attenuate the nitrate impacts generated by the septic systems on the development in accordance with current D-5-4 guidelines, provided an appropriate tertiary treatment is used for the proposed system.

8 CONCLUSIONS

Based on our review of available information and the results of the groundwater sampling and laboratory analytical programs, we conclude the following:

- 1. Sufficient area exists on the property for the installation of a septic system in accordance with the OBC to service the proposed Assembly Hall with a design sewage flow of up to 4,000 L/day.
- Pre-treatment of the sewage from the proposed sewage disposal systems with an Ecoflo Biofilter certified treatment system, which has a documented and measured output of between 0.35 mg/L and 8.54 mg/L yields a calculated nitrate concentration at the property line of 9.9 mg/L, based on the *"Contaminant Attenuation Method"*.
 - 3. Hydrogeologically sensitive conditions are present on the site due to thin overburden. The overburden generally consists of fill to bedrock, with till or silty sand observed at two (2) of the test pits.
 - 4. Records of domestic wells were retrieved within 500 m of the site. The potable water source of these wells is the bedrock aquifer. A thin layer of either clay, gravel or till, with some sand in areas, being between 0.9 and 7.6 m thick over bedrock.

9 RECOMMENDATIONS

- 1. The septic system should be placed at least 15 m from any drilled wells/water service and 30 m from any dug well. It is recommended that the water table be surveyed prior to installation. The 20 m setback from the normal high water mark of the identified stream east of the proposed development footprint.
- 2. Due to the thin soils and sensitive site conditions it is recommended that the leaching bed of the proposed system be fully raised and an appropriate groundwater monitoring program be implemented. It is recommended that the groundwater monitoring wells be installed in compliance with *O. Reg. 903: Wells* to aid in the interpretation of groundwater flow direction and monitoring potential impacts to the identified supply aquifers. When

no longer required the wells should be decommissioned in accordance with O. Reg. 903.

3. It is recommended that a geodetic benchmark be used for further investigations on the site, including the proposed monitoring wells and groundwater elevations.

10 LIMITATIONS

The findings contained in this report are based on data and information collected during the Terrain Analysis of the subject property conducted by LRL Associates Ltd. The conclusions and recommendations are based solely on site conditions encountered at the time of our fieldwork on May 8th, 2017, supplemented by historical information and data obtained as described in this report. The information presented in this report represents the groundwater conditions at the locations sampled. Due to natural variations in geological conditions, no inference is made to the soil or groundwater conditions between sampling points. No assurance is made regarding changes in conditions subsequent to the time of this investigation. If additional information is discovered or obtained, LRL Associates Ltd. should be requested to re-evaluate the conclusions presented in this report and to provide amendments as required.

In evaluating the subject property, LRL Associates Ltd. has relied in good faith on information provided by individuals as noted in this report. We assume that the information provided is factual and accurate. We accept no responsibility for any deficiencies, misstatements or inaccuracies contained in this report as a result of omissions, misinterpretation or fraudulent acts of the persons contacted.

EP PROFESSIONAL Yours truly, LRL Associates Ltd NEER 100110298 2020/03/13 ROLINCE OF ONTARIO Matthew Whitney, P. Eng.

Encl.

- Figure 1 Site Location
- Figure 2 Site Plan
- Figure 3 Proposed Site Layout
- Figure 4 Test Pit Locations, Groundwater Elevations and Groundwater Contours
- Figure 5 Well Locations, Ontario Well Records Within 500 m of the Site
- Figure 6 Available Area for Sewage Disposal (Conservative Approach)
- Figure 7 Proposed Ecoflow Septic Disposal System Layout
- Table 1 Summary of Groundwater Elevations in Test Pits
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- Appendix C Sieve & Hydrometer Analysis
- Appendix D Ontario Well Record Printouts
- Appendix E Moisture Surplus Printout
- Appendix F Premier Tech Aqua Report

FIGURES



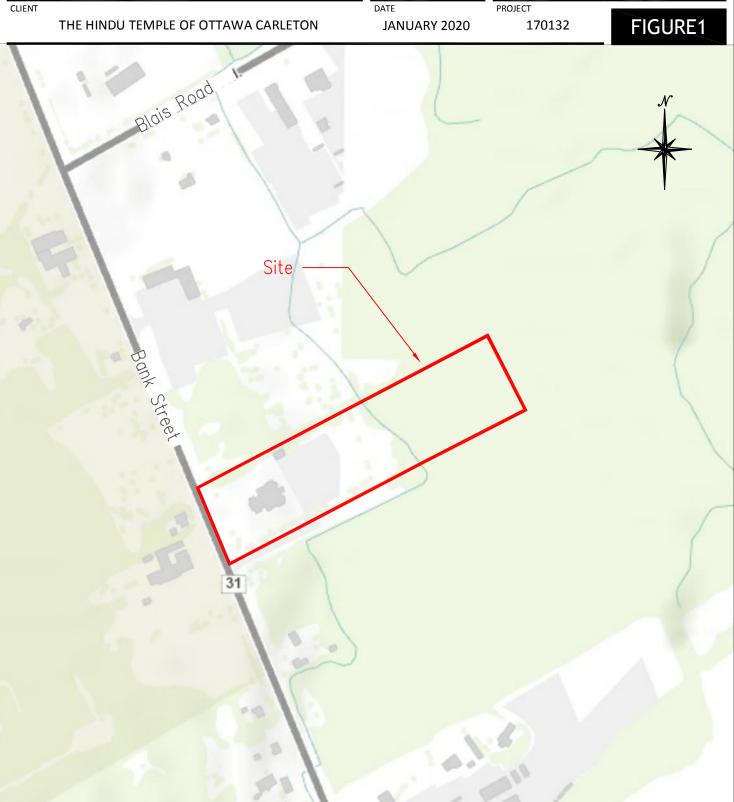
PROJECT

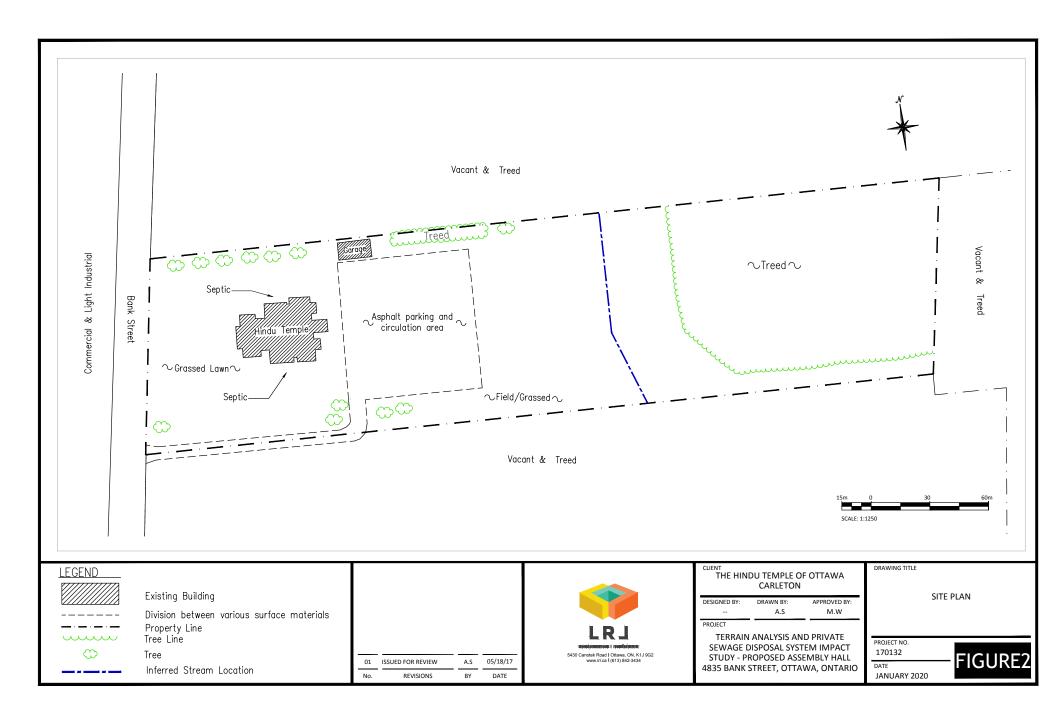
TERRAIN ANALYSIS AND PRIVATE SEWAGE DISPOSAL SYSTEM IMPACT STUDY PROPOSED ASSEMBLY HALL 4835 BANK STREET, OTTAWA, ONTARIO

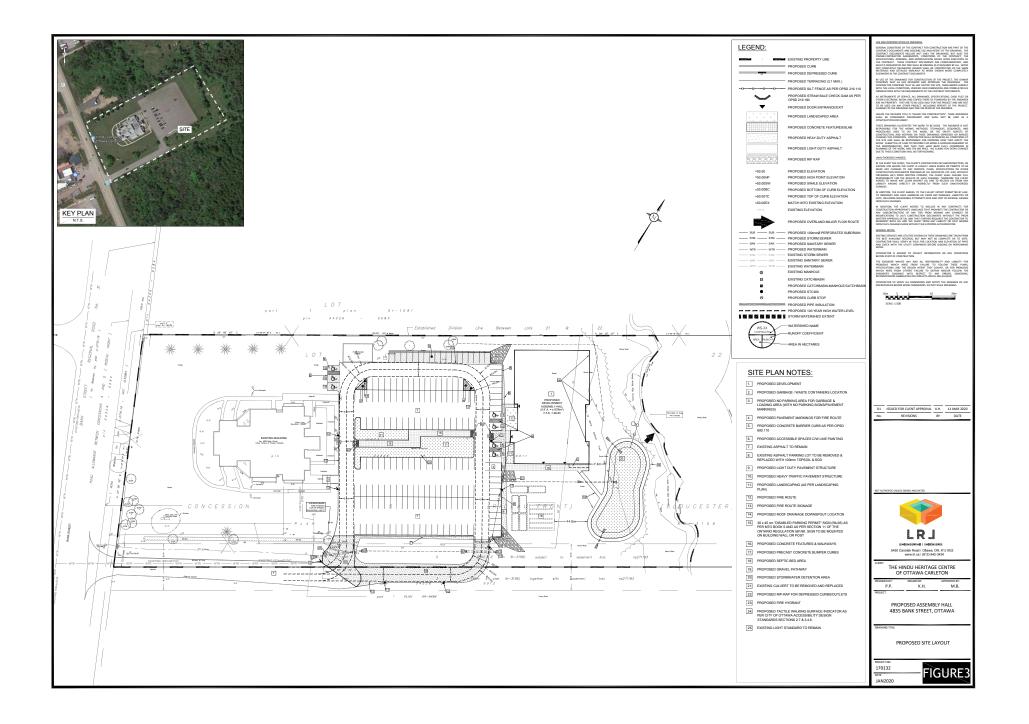
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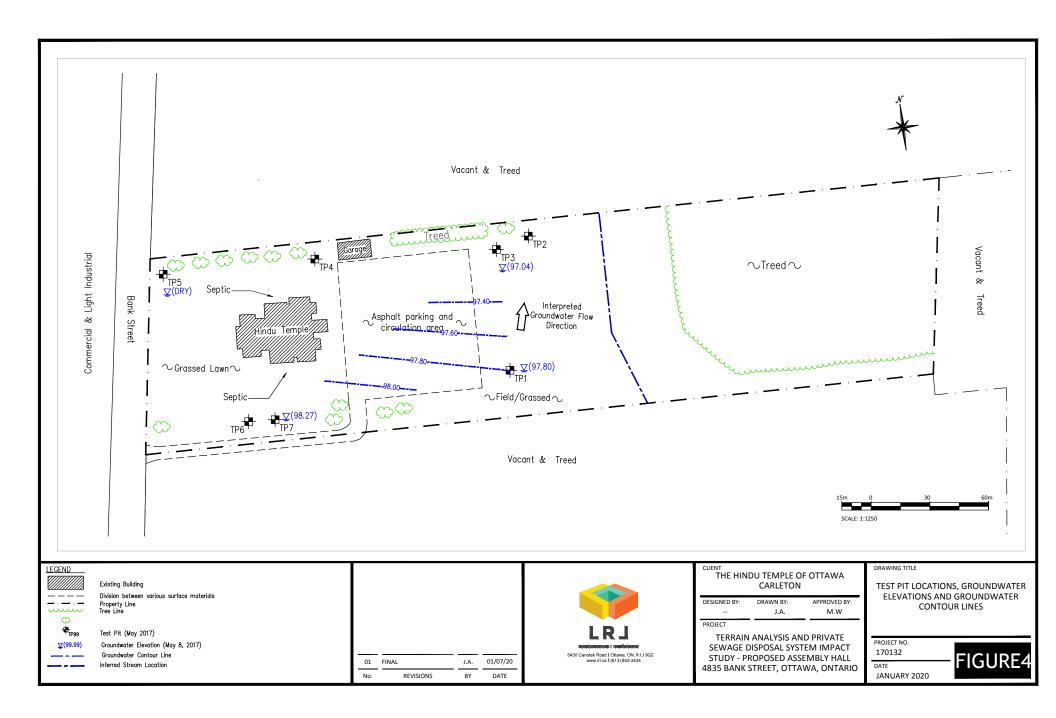
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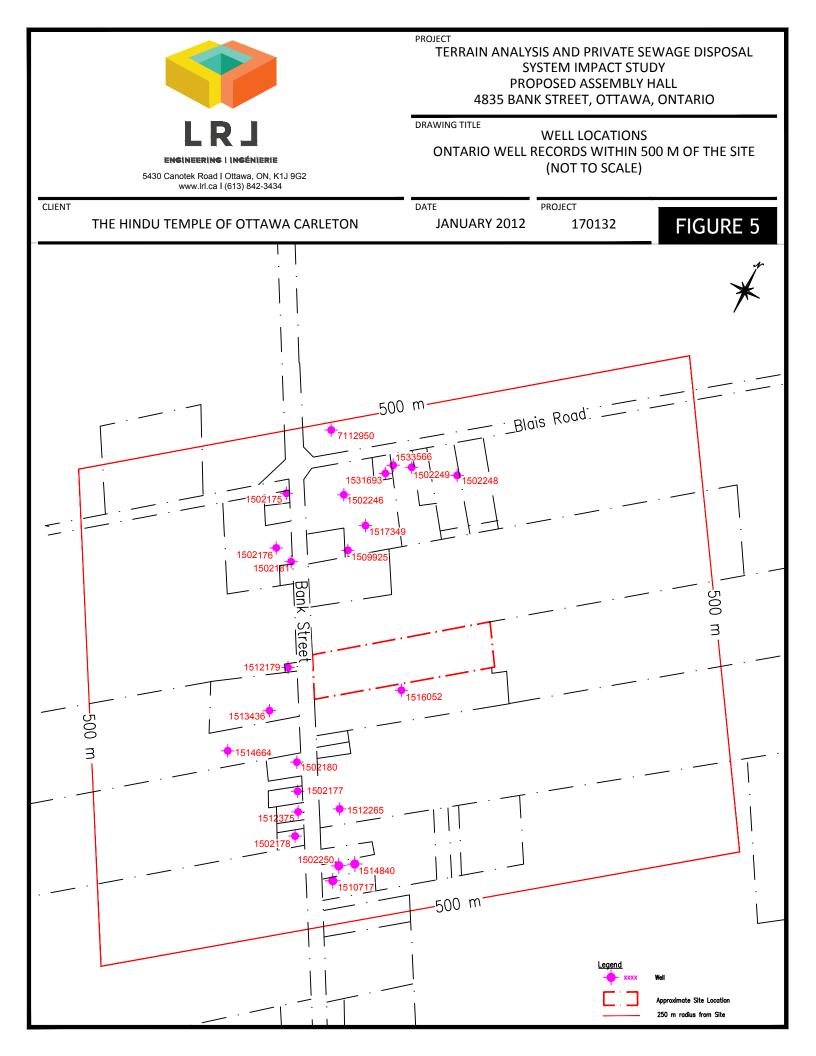
CLIENT

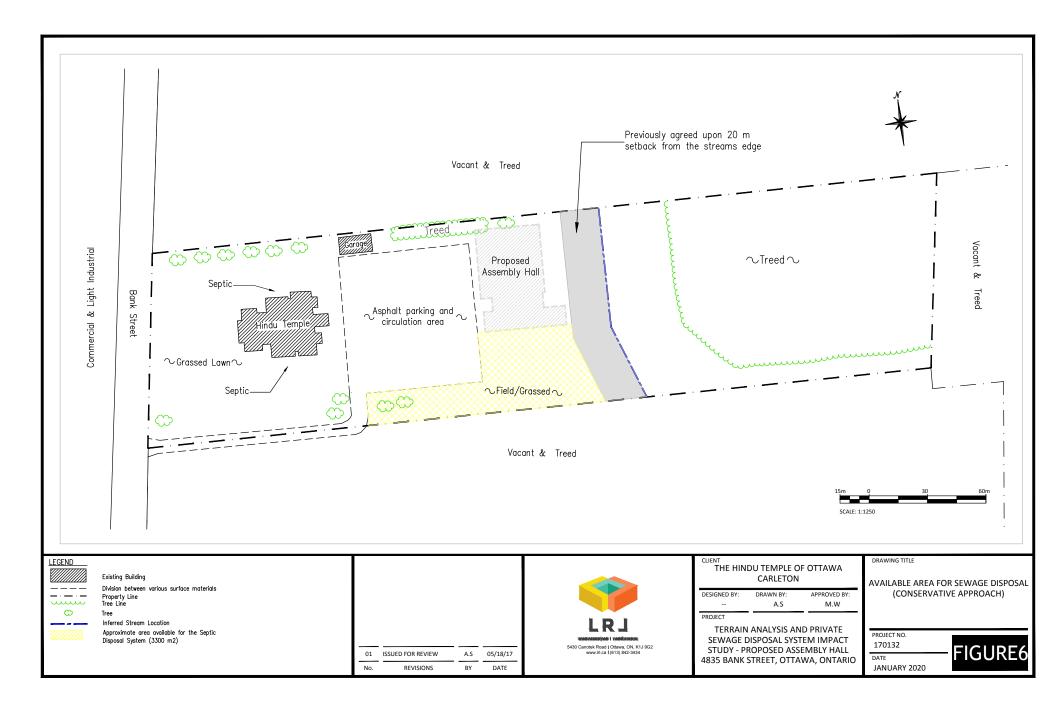


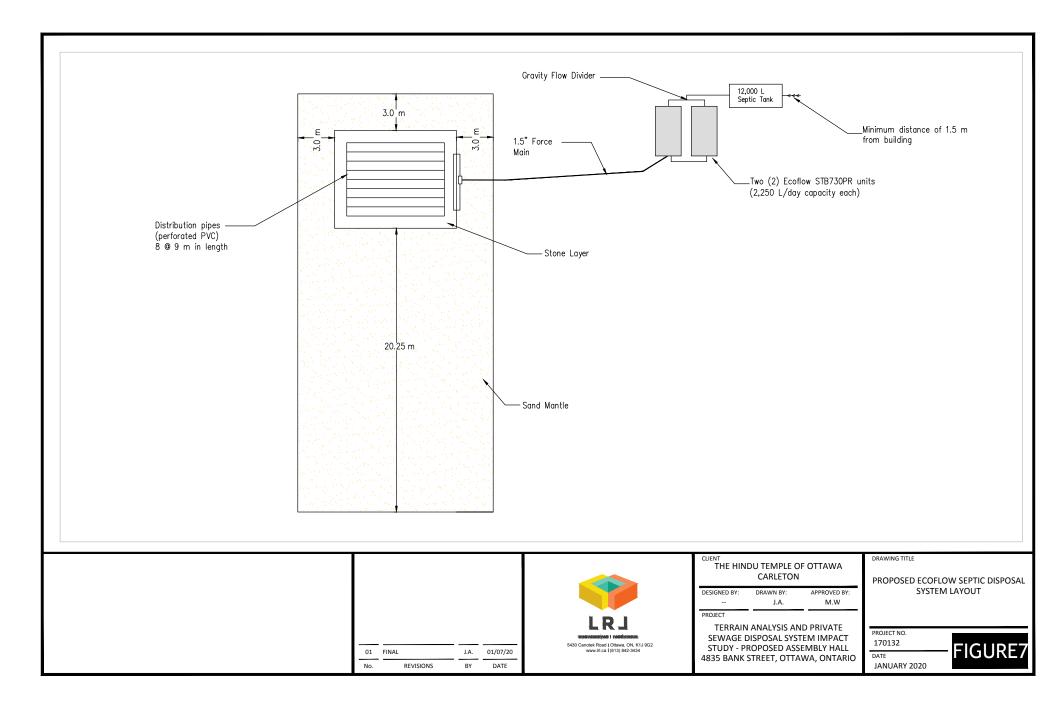












TABLES

Table 1
Summary of Groundwater Elevations in Test Pits

Terrain Analysis - Proposed Assembly Hall

4835 Bank Street, Ottawa, Ontario

LRL File: 170132

	Ground Surface Elevation ¹	Reference Elevation ²	Depth To W	ater Table (m)	Groundwater Elevation
Test Pit	(m)	(m)	Reference Point	Ground Surface	(m)
TP1	98.21	99.15	1.35	0.41	97.80
TP2	97.09				
TP3	97.75	98.98	1.94	0.71	97.04
TP4	99.54				
TP5	98.78	99.02	DRY		
TP6	99.38				
TP7	99.60	100.79	2.52	1.33	98.27

NOTES

¹ Elevations are based off of a temporary benchmark established at the top of the east arm of the fire hydrant along the southof the Site (100.00 m).

² Reference elevation is top of piezometer.

Table 2Summary of Sieve & Hydrometer AnalysesTerrain Analysis - Proposed Assembly HallPart of Lot 16, Concession 7, Hammond, Ontario

LRL File: 160833

	Sample Gradation ¹ Percent Particles in Each Fraction									
		Oracial	0	Sand	F ire e	0:14	0			
		Gravel	Coarse 2.0 - 4.75	Medium 425 µm - 2.0	Fine	Silt	Clay	Soil Texture		
Sample	Depth (m)	>4.75 mm	mm	mm	75 - 425 µm	2 - 75 µm	< 2µm	Classification		
TP1-3	1.8 - 2.0	4.8	1.2	1.8	6.5	63.8	22.0	Silt Loam		
TP3-6	1.4 - 1.6	21.3	7.0	12.7	20.1	39	.0	Fine Silty Sand		

NOTES:

¹ Unified Soil Classification System

Table 3Summary of analysis of water samples collected from the test pits.Terrain Analysis - Proposed Assembly Hall

4835 Bank Street, Ottawa, Ontario

		Ontario Drin Stano	-	Sample			
Parameter	Units	MRL	Standard	Туре	TP1	TP3	TP5
Sample Date (d/m/y)					05/08/2017	05/08/2017	05/08/2017
Ammonia	mg/L	0.01			0.28	0.39	1.66
Total Kjeldahl Nitrogen	mg/L	0.1			78.1	65.3	131
Nitrate as N	mg/L	0.1	10	MAC	<0.1	0.5	<0.1
Nitrite as N	mg/L	0.05	1	MAC	<0.05	<0.05	<0.05

NOTES

MAC Maximum Acceptable Concentration

MRL Minimum Reportable Limit

Table 4A

Nitrate Attenuation Calculations Terrain Analysis and Private Sewage Disposal System Impact Study - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario LRL File: 170132

1. Potential Infiltration

Maathaa Otatian

<u>____</u>

vveather a	Station	Ottawa													
			Infiltration Factor (IF) ¹							Moisture Surplus (MS)				Potential Infiltration (PI) (IF*MS) (mm)	
No.	Section Area (m ²)	Topography	Value	Soil	Value	Cover	Value	Total	Ground Cover	Soil Type	Moisture Retention ² (mm)	Moisture Surplus ³ (mm)	Section	Weighted	
1	23,750	Flat	0.3	Sand	0.4	Cultivated Land	0.1	0.8	Moderately Rooted Crops	1 Fine Sand	75	384	307.2	307.2	
Total ⁶	23,750												Total	307.2	

2. Area Available for Infil	tration					
Approximate footprint of the exis	itng assembly hall		Н		1,168	m ²
Approximate footprint of the exis	itng garage		Н		105	m ²
Approximate footprint of the prop	oosed assembly hall		Н		1,593	m²
Approximate area of paved park	ing and circulation (Existing &	Proposed)	d ⁴		4,996	m ²
Approximate Length of Road			L			m
Approximate Width of Road			w			m
Total Area of Property					23,750	m ²
Impervious Area					7,862	m ²
	Roads	l x w	-	m ²		
	Parking and Circulation	d	4,996.2	m ²		
	Buidling	Sum of H's	2,866.0	m ²		
Area available Infiltration			А		15,888	m ²

3. Nitrate Diluation Calculations			
Nitrate Concentration of Infiltration ⁷	C _i	0.2	mg/L
Site Infiltration	Q _i = A*PI	4,881	m³
Existing Development (Status Quo)			
Daily Sewage Volume - Exisitng Development	Q _d	4.00	m³
Maximum Yearly Sewage Volume - Existing Development	$Q_e = 365 * Q_d$	1,460	m³
Nitrate Concentration in Sewage - Existing Development	C _e	40	mg/L
Proposed Development (Eco-Flow System)			
Daily Sewage Volume - Proposed New Development ⁸	Qd	4.00	m³
Maximum Yearly Sewage Volume (water) - Proposed New Development	Qe=365*Qd	1,460	m³
Nitrate Concentration in Sewage - Proposed New Development	Ce	40	mg/L
Maximum Allowable Nitrate Concentration at Boundary ⁵	C _m	10.0	mg/L
Increase in Nitrate Concentration at Boundaries	C = (Qe,1Ce,1+Qe,2Ce,2+QiCi)/(Qe,1+Qe,2+Qi)	15.1	mg/L

NOTES

1 Table 2: Infiltration Factors, Hydrogical Technical Information Requirements for Land Development Applications, Ministry of the Energy and Environment, April 1995.

² Thornthwaite and Mather's (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance.

3 Moisture surplus for data for Ottawa ON (Environment Canada Meteorological Service of Canada, 2010).

4 The vaule is a calculation of the total existing parking & circulation area foot print, and the proposed 202 Vehicle parking & circulation area presented Vector Design Architects site plan, May 2019.

As per Technical Guideline for Individual On-Site Sewage Systems: Water Quality and Impact Risk Assessment, Ministry of the Energy and Environment, August 1996. 5

6 The total area of the property used in this calculation is limited to the area of the Site located west of the stream.

7 The nitrate concentration of infiltration is calculated based on the average nitrate concentration reported at the time of the 2017 Terrain Analysis. Values reported as <0.1 mg/L were interpreted into the calcualtion as 0.05 mg/L.

Table 4B **Nitrate Attenuation Calculations - Tertiary Treatment**

Terrain Analysis and Private Sewage Disposal System Impact Study - Proposed Assembly Hall 4835 Bank Street, Ottawa, Ontario LRL File: 170132

1. Potential Infiltration

Weather \$	Station	Ottawa												
			Infiltration Factor (IF) ¹ Moisture Surpl									us (MS)		
											Moisture Retention ²	Moisture Surplus ³		
No.	Section Area (m ²)	Topography	Value	Soil	Value	Cover	Value	Total	Ground Cover	Soil Type	(mm)	(mm)	Section	Weighted
1	23,750	Flat	0.3	Sand	0.4	Cultivated Land	0.1	0.8	Moderately Rooted Crops	1 Fine Sand	75	384	307.2	307.2
Total ⁶	23,750												Total	307.2

2. Area Available for Infiltration												
Approximate footprint of the exisitng assembly hall		Н	1,168 m ²									
Approximate footprint of the exisitng garage		Н	105 m ²									
Approximate footprint of the proposed assembly hall		Н	1,593 m ²									
Approximate area of paved parking and circulation (Existing &	Proposed)	d ⁴	4,996 m ²									
Approximate Length of Road		L	m									
Approximate Width of Road		w	m									
Total Area of Property			23,750 m ²									
Impervious Area			7,862 m ²									
Roads	l x w	- m ²										
Parking and Circulation	d	4,996.2 m ²										
Buidling	Sum of H's	2,866.0 m ²										
Area available Infiltration		A	15,888 m ²									

Nitrate Concentration of Infiltration ⁷	Ci	0.2	mg/L
Site Infiltration	Q _i = A*PI	4,881	m³
Existing Development (Status Quo - Existing Kitchen)			
Daily Sewage Volume - Exisitng Development	Q _{d1}	2.0	m³
Maximum Yearly Sewage Volume - Existing Development	Q _{e1} =365*Q _{d1}	730	m³
Nitrate Concentration in Sewage - Existing Development	C _{e1}	40	mg/L
Existing Development (Status Quo - Existing Temple)			
Daily Sewage Volume - Exisitng Development	Q _{d2}	2.0	m ³
Maximum Yearly Sewage Volume - Existing Development	$Q_{e2} = 365 * Q_{d2}$	730	m³
Nitrate Concentration in Sewage - Existing Development	C _{e2}	40	mg/L
Proposed Development (Eco-Flow System)			
Daily Sewage Volume - Proposed New Development ⁸	Q _{d3}	4.00	m³
Maximum Yearly Sewage Volume (water) - Proposed New Development	$Q_e = 365 * Q_d$	1,460	m ³
Nitrate Concentration in Sewage - Proposed New Development	C _e	12	mg/L
Maximum Allowable Nitrate Concentration at Boundary ⁵	C _m	10.0	mg/L
Increase in Nitrate Concentration at Boundaries	$C = (Q_iC_i + Q_{e1}*C_{e1} + Q_{e2}*C_{e2} + Q_{e3}*C_{e3})/(Q_i + Q_{e1} + Q_{e2} + Q_{e3})$	9.9	mg/L

NOTES

Table 2: Infiltration Factors, Hydrogical Technical Information Requirements for Land Development Applications, Ministry of the Energy and Environment, April 1995. 1

Thornthwaite and Mather's (1957) Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance. 2

3 Moisture surplus for data for Ottawa ON (Environment Canada Meteorological Service of Canada, 2010).

The vaule is a calculation of the total existing parking & circulation area foot print, and the proposed 202 Vehicle parking & circulation area presented Vector Design Architects site plan, May 2019. 4

5 As per Technical Guideline for Individual On-Site Sewage Systems: Water Quality and Impact Risk Assessment, Ministry of the Energy and Environment, August 1996.

6 The total area of the property used in this calculation is limited to the area of the Site located west of the stream.

7 The nitrate concentration of infiltration is calculated based on the average nitrate concentration reported at the time of the 2017 Terrain Analysis. Values reported as <0.1 mg/L were interpreted into the calculation as 0.05 mg/L.

APPENDIX A Test Pit Logs



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SA	MPLE C	ATA				
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (kF	Strength Pa) 150	Water Content ▼ (%) ▼ 25 50 75 Liquid Limit □ (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m 0 0	Ground Surface	98.21 0.00	~~ ~ .					12
	TOPSOIL Sandy, dark brown, dry.	98.01						
- - 1 - - 2 - - - - - - - - - - - - - - -	FILL Sandy clay, dark brown, dry.	97.31						▲ 0.4 m bgs (08/05/17)
3 1 - 4 -	Silty Sand Trace clay, with clay seam from 1.7 to 1.8 m bgs, brown, dry. Sieve analysis completed.	0.90		1				
				2				
2								
- 7 - - - - 8	End of Test Pit Refusal over inferred bedrock.	<u>96.11</u> 2.10		3				
_								
Eastin	g: N/M	Northing	: N/M			NOTES:		
Site Da	atum: Top east arm of hydrant at south	entrance (100.00 m)			BGS- B	Below Ground Surface	
Groun	dsurface Elevation: 98.21	Top of R	iser Elev.	: 99.15				
Excavation Width: 1.2 m Excavation Length: 1.5 m								



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SAMPLE DATA						
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (ki	Strength Pa) 150	Water Content ▼ (%) ▼ 25 50 75 Liquid Limit (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 tt m 0 - 0 1 2 -	Ground Surface FILL Silty sand with some clay, brown, saturated with water infiltration at 0.4 m bgs. Buried metal structure/waste at approximately 0.9 m bgs.	97.09 0.00						
3- - - - - -	End of Test Pit	96.19 0.90		4				
4 - - 								
5 - -								
6								
7								
8								-
Eastin	g: N/M	Northing:	N/M			NOTES: Test pit ter	minated at 0.9 meters due to v	olume of water in
Site Da	atum: Top east arm of hydrant at south er	ntrance (1	00.00 m)		pit.	w Ground Surface	oranie of water in
Groun	dsurface Elevation: 97.09	op of Ri	ser Elev	.:				
Excava	Excavation Width: 1.2 m Excavation Length: 1.5 m							



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SAI	MPLE D	ATA			Water Content	
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	She	ar Strength (kPa) 150	Water Content ▼ (%) ▼ 25 50 75 Liquid Limit (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m 0 0	Ground Surface	97.75	~~!					
	TOPSOIL Sandy loam, dark brown, dry. Brick debris found in top 0.2 m bgs.	0.00 97.55						
-	FILL	0.20						4
1	Sandy silt, trace boulders, brown, dry. Tire debris found at approximately 0.8							08/05/1
	m bgs.			5	-) sốc
- 2 -								0.71 m bgs (08/05/17)
_		96 95						
3-	TILL Silty sand, trace gravel, cobbles and boulders, brown, dry.	96.95 0.80						
- 1 - -	Sieve analysis completed.							
4 - -								
-]-]-					
5				6	-			_
_	End of Toot Dit	96.05 1.70	<u>•[[•][</u>					
6-	End of Test Pit Refusal at 1.7 m bgs over inferred bedrock.							
_ _— 2								_
7-								_
-								
8- -								_
Eastin	g: 0454091	Northing	: 5017670			NOTES:		
Site Da	atum: Top east arm of hydrant at south e	ntrance (´	100.00 m)			BGS- Belo	ow Ground Surface	
Groun	dsurface Elevation: 97.75	Top of Ri	ser Elev.:	98.98				
Excav	Excavation Width: 1.2 m Excavation Length: 1.5 m							



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SA		ATA					
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (kl	Strength Pa) 150	▼ (25 5 Liqui	Content %) ∇ 50 75 d Limit %) □ 50 75	Water Level (Standpipe or Open Excavation)
0 ft m 0 0	Ground Surface	99.54							
	FILL Silty loam, trace clay,dark brown, dry. FILL Silty sand, trace cobbles and gravel, light brown, dry. Changing to dark brown sandy fill with trace boulders at approximately 0.8 m bgs. End of Test Pit Refusal at 1.4 m bgs over inferred bedrock or large concrete structure.	99.54 0.00 99.04 0.50 98.14 1.40		7					
- 8 -									
Eastin	g: 0454005	Northing	: 5017628	1		NOTES:			
	atum: Top east arm of hydrant at south e		100.00 m)			BGS	6- Below Gr	ound Surfac	e
		Excavatio							
LAGAV			Longu			1			



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SAM	IPLE D	ATA				
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	She	ar Strength (kPa) 150	Water Content ▼ (%) ▼ 25 50 75 Liquid Limit (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m 0	Ground Surface	98.78						
	TOPSOIL Silty loam some sand, dark brown, dry. FILL Sand, some silt, trace cobbles, brown, dry. Waste debris of metal and asphalt pieces at approximately 0.9 m bgs. End of Test Pit Refusal at 1.5 m bgs over inferred bedrock.	98.78 0.00 98.63 0.15 97.28 1.50		10 9 11				Dry at 1.53 m bgs
8— _								
Eastin	g: 0453945	Northing:	5017595			NOTES:		
Site D	atum: Top east arm of hydrant at south	entrance (1	00.00 m)			BGS	- Below Ground Surface	
Groun	idsurface Elevation: 98.78	Top of Ri	ser Elev.	: 99.02				
Excav	Excavation Width: N/M Excavation Length: N/M							



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

S	UBSURFACE PROFILE	SAI	MPLE D	ATA				
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	Shear S (ki	Strength Pa) 150	Water Content ▼ (%) ▼ 25 50 75 Liquid Limit (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m	Ground Surface	99.38						
	TOPSOIL Sandy loam, dark brown, dry.	0.00	22222					
 - 1 - - -	FILL Sand, some gravel, cobbles, boulders, silty seam at 0.7 m bgs, brown, dry. Refusal at 0.8 m bgs over inferred bedrock.	<u>99.23</u> 0.15						
2_	Deulock.							
-				12	-			
		98.58 0.80		13				
	End of Test Pit							
								_
5 - -								
6								
2 2 								_
-								_
 8 -								_
Easting	Easting: 0454003 Northing: 5017542				NOTES:			
	atum: Top east arm of hydrant at south dsurface Elevation: 99.38	entrance ([*] Top of Ri				BGS	S- Below Ground Surfa	ce
	ation Width: N/M	Excavatio						



Project No.: 170132

Client: Hindu Temple of Ottawa Carleton

Date: May 08, 2017

Excavation Method: Backhoe

Project: Terrain Analysis

Location: 4835 Bank Street, Ottawa, ON

Field Personnel: JA

		1			-			
S	UBSURFACE PROFILE	SAI	MPLE DA	TA			Weter Orationt	
Depth	Soil Description	Elev./Depth (m)	Lithology	Sample Number	She	ar Strength (kPa) 150	Water Content ▽ (%) ▽ 25 50 75 Liquid Limit (%) □ 25 50 75	Water Level (Standpipe or Open Excavation)
0 ft m 0 0	Ground Surface	99.60						
	TOPSOIL Sandy loam, dark brown, dry.	0.00						
- 1 - - - 2	FILL Sand, brown, trace metal debris, dry.	0.20						
_	TILL	98.90 0.70						
 3- 	Silty sand, trace clay, boulders, grey, organics including tree stump, roots, bleRefusal due to obstruction (tree ⁿ bg _{stump}).							1.33 m bgs (08/05/17)
4								1.33 m bg
- 5 - - -								- - - - -
6-	End of Test Pit	97.80 1.80						
-	End of Test Pit							
2 								
7								-
 8 -								
Easting: 0454051 Northing: 5017564					NOTES:	1		
Site Da	atum: Top east arm of hydrant at south er	ntrance (*	100.00 m)			BGS	S- Below Ground Surfac	e
Groun	dsurface Elevation: 99.60	op of R	iser Elev.: 1	100.79				
Excav	ation Width: N/M	Excavati	on Length:	N/M				

APPENDIX B

Laboratory Certificates of Analysis



RELIABLE.

300 - 2319 St. Laurent Blvd Ottawa, ON, K1G 4J8 1-800-749-1947 www.paracellabs.com

Certificate of Analysis

LRL Associates Ltd.

5430 Canotek Road Ottawa, ON K1J 9G2 Attn: Jessica Arthurs

Client PO: Project: 170132 Custody: 32310

Report Date: 15-May-2017 Order Date: 11-May-2017

Order #: 1719377

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
1719377-01	TP1
1719377-02	TP3
1719377-03	TP7

Approved By:



Dale Robertson, BSc Laboratory Director

Any use of these results implies your agreement that our total liability in connection with this work, however arising, shall be limited to the amount paid by you for this work, and that our employees or agents shall not under any circumstances be liable to you in connection with this work.



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017 Project Description: 170132

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date Analysis Date
Ammonia, as N	EPA 351.2 - Auto Colour	12-May-17 12-May-17
Anions	EPA 300.1 - IC	12-May-17 12-May-17
Total Kjeldahl Nitrogen	EPA 351.2 - Auto Colour, digestion	12-May-17 15-May-17



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017

Project Description: 170132

	Client ID:	TP1	TP3	TP7	-				
	Sample Date:	08-May-17	08-May-17	08-May-17	-				
	Sample ID:	1719377-01	1719377-02	1719377-03	-				
	MDL/Units	Water	Water	Water	-				
General Inorganics									
Ammonia as N	0.01 mg/L	0.28	0.39	1.66	-				
Total Kjeldahl Nitrogen	0.1 mg/L	78.1	65.3	131	-				
Anions									
Nitrate as N	0.1 mg/L	<0.1	0.5	<0.1	-				
Nitrite as N	0.05 mg/L	<0.05	<0.05	<0.05	-				



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions Nitrate as N Nitrite as N	ND ND	0.1 0.05	mg/L mg/L						
General Inorganics Ammonia as N Total Kjeldahl Nitrogen	ND ND	0.01 0.1	mg/L mg/L						



Order #: 1719377

Report Date: 15-May-2017 Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions Nitrate as N Nitrite as N	ND ND	0.1 0.05	mg/L mg/L	ND ND				20 20	
General Inorganics Ammonia as N Total Kjeldahl Nitrogen	0.021 1.50	0.01 0.1	mg/L mg/L	0.022 1.52			2.4 1.8	8 10	



Report Date: 15-May-2017

Order Date: 11-May-2017

Project Description: 170132

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions					101				
Nitrate as N Nitrite as N	1.01 1.02	0.1 0.05	mg/L mg/L	ND ND	101 102	81-112 76-117			
General Inorganics	1.02	0.00	ing/L	ND	102	70-117			
Ammonia as N	0.280	0.01	mg/L	0.022	103	81-124			
Total Kjeldahl Nitrogen	1.91	0.1	mg/L		95.7	81-126			



Login Qualifiers :

Samples received submerged in water, possibly melted ice. This condition can compromise sample integrity. *Applies to samples: TP1, TP3, TP7*

Sample Data Revisions

None

Work Order Revisions / Comments:

None

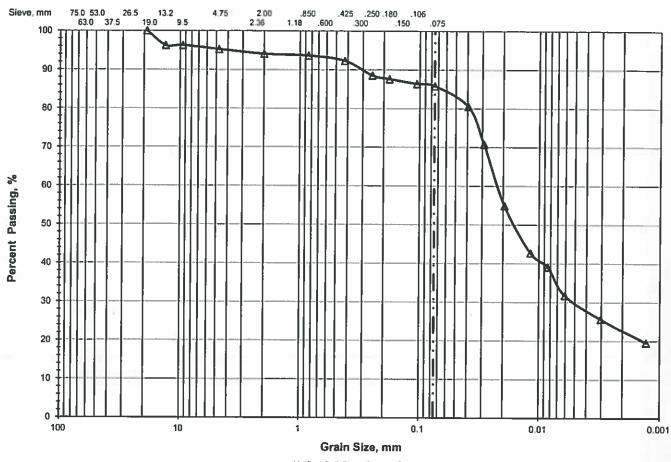
Other Report Notes:

n/a: not applicable ND: Not Detected MDL: Method Detection Limit Source Result: Data used as source for matrix and duplicate samples %REC: Percent recovery. RPD: Relative percent difference. Report Date: 15-May-2017 Order Date: 11-May-2017 Project Description: 170132 APPENDIX C

Sieve & Hydrometer Analysis

LRL Associates Ltd. PARTICLE SIZE ANALYSIS

I D I	Client:	Lloyd Phillips & Associates Ltd.	File No.:	170132
	Project:	Hydrogeological Assessment & Terrain Analysis	Report No.:	1
GINEERING INGENIERIE	Location:	4835 Bank Street., Ottawa, ON.	Date:	May 8, 2017



Unified Soil Classification System

	> 75 mm	% GR	AVEL		% SAND)	% FINES	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
Δ	0.0	0.0	4.8	1.2	1.8	6.5	63.8	22.0
								-

Δ

	Location	Sample	Depth, m	D ₆₀	D ₅₀	D ₃₀	D ₁₅	D ₁₀	Cc	Cu
△	TP-1	3	1.80 - 2.00	0.0226	0.0164	0.0052				
		_								

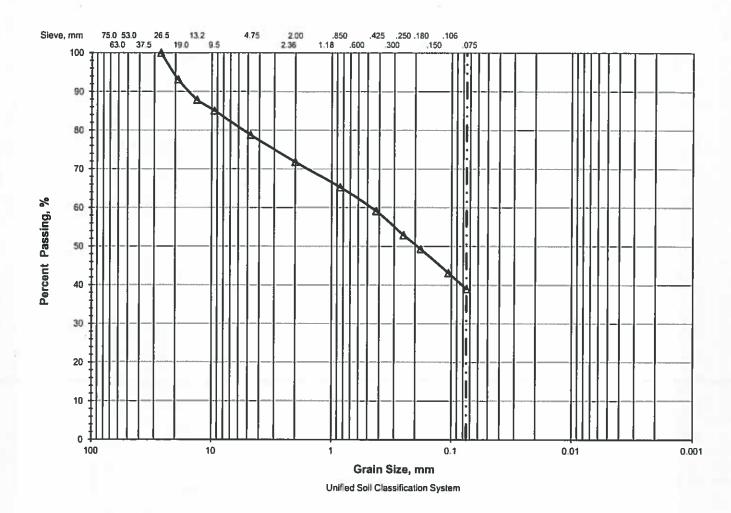


LRL Associates Ltd.

PARTICLE SIZE ANALYSIS

ASTM D 422 / LS-702

DI	Client:	Lloyd Phillips & Associates Ltd.	File No.:	170132	
RJ	Project:	Hydrogeological Assessment & Terrain Analysis	Report No.:	2	_
NG I INGÉNIÈRIE	Location:	4835 Bank Street., Ottawa, ON.	Date:	May 8, 2017	



ļ,	> 75 mm	% GR/	AVEL		% SAND		% FINES
Ľ		Coarse	Fine	Coarse	Medium	Fine	Silt & Clay
7	0.0	6.0	15.3	7.0	12.7	20.1	39.0
Γ							
Г							
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Ē					X		· · · · · · · · · · · · · · · · · · ·

Λ.	
_	

	Location	Sample	Depth, m	D ₆₀	D ₅₀	D ₃₀	D ₁₅	D ₁₀	Cc	Cu
△	TP3	6	1.4 - 1.6	0.4855	0.1932					
	_									

APPENDIX D Ontario Well Record Printouts

	314/52.				
UTM $ / 8 ^{z}$ $ 4 5 3 7 2$	OE	滴		GROUN 5VATE	NOBRANCIZ X
5 R 501171711	10 N	ONT		SEP - 9	
Elev. $ 4 ^{R}$ $ 0 3 0 6 $	The Wa		illers Act, 1954	ONTARIO V	
Basia 265 1 From +		Department		RESOURCES CO	
ConTV	Water	-We	ll Recor	d	
lot 21 County or Territorial District	Carliton	Town	shin Village Town of	City Alon	eester
			in Village, Town or	City)	
· · · · ·			Address Build	ing Brady	£
(day)	(month)	(year)			
Pipe and Casing		T		Pumping Test	
Casing diameter(s)			Static level Pumping rate	10 ft	
Length (s)			Pumping rate Pumping level	200 JIR	-/
Length of screen			Duration of test		
Well Log		<u>l</u>		W (D)	
Well Log		1	Depth(s)	Water Record	· · · · · · · · · · · · · · · · · · ·
Overburden and Bedrock Record	From ft.	To ft.	at which water (s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
Bolders and sand	0	20	68	50	Irnh
Aand stone	20	68			
······································					· · · · · · · · · · · · · · · · · · ·
				-	
					Ľ.r
For what purpose(s) is the water	11			ocation of Well	
Is water clear or cloudy?				v show distances of e. Indicate north	
Is well on upland, in valley, or on					sy allow.
	mitte		Barrett D	R	
Address 16 52 B a		4 2	and descent and a second s	1	
Name of Driller	contle.			1 hz	
Address			ź	IK	
Licence Number. 3.9.5			.3	k	
I certify that the f	oregoing				
statements of fact :	are true.		•		
Date J 9 aug 5-7 F. L.	Remute nature of Licensee	c	608+	//	
Form 5					
			1	C 25,83	
			/		

$\frac{31c/5z}{UIM} 18 ^{z} 4 5 3 7 6 0 ^{E}$ $\frac{15}{15} 5 6 17 5 6 0 ^{N}$ The Ontario Water Resc Elev: $ 4 ^{R} 0 3 15 $ WATER WEI Basin 2 5 0 BIE 7 on County or District 0 BIE 7 on Con. HRF Lot 21 I	LL REC	Act ORDES Fown or City	GLG0 Jury month	2 SION SION SION SION SION SION SION SION
Casing and Screen Record		Pumpi	ng Test	
Inside diameter of casing Total length of casing Type of screen Length of screen Depth to top of screen Diameter of finished hole	Pumping level Duration of test Water clear or c Recommended	ate pumping loudy at end o pumping rate	G G 8 JHR of test T	G.P.M.
Well Log	· · · · · ·		Wate	r Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
Limestae	18	45	45	A
For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside? Drilling or Boring Firm MEDSHER Address		m below show	of Well w distances of we adicate north by	ده
Licence Number Name of Driller or Borer Address Date (Signature of Licensed Drilling or Boring Contractor) Form 7 10M-62-1152		600	4.8	·
OWRC COPY			CSS.58	

UTM $1/16^{ z } 4 5 3 9 7 $ $1/16^{ z } 4 5 3 9 7 $ $15^{ R } 5 0+7 7 0 8 $ Elev. $ 4 ^{ R } 0 3 3 0 $ Basin $ 2' 5 $	≥_N The Wa		ARIO rillers Act, 1954 of Mines	15 N GROUND WATER B 194 MAY 20 19	BANCH X
V			ll Recor		TER MISSION
County or Territorial District	Carleton	STown	ship, Village, Town or in Village, Town or C .Address		sector
(day)	(month)	(year)			2022/02/2022 4 (24.02/04/4) 200
Pipe and Casing	Record			Pumping Test	
Casing diameter(s)			Static level Pumping rate Pumping level Duration of test	25 JT	<u>t/</u>
Well Log				Water Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
Aand	ð	7	60	54	Inch
Bolders and Sand	7	20			
Wit Band store	20	<u> </u>			
For what purpose(s) is the water of Is water clear or cloudy?	hillside? hillsi	nd	In diagram below road and lot line	cation of Well show distances of . Indicate north 1 Aporth1	
Form 5			ors	CSE	.33

316/50 F UTM 1/8 Z 41513101710 E GEDEND WATER BRANC 9 R 5101/16191910 N AUG 1 9 1957 Eley. 0133131 The Water-well Drillers Act, 1954 ONTARIO WATER **Department** of Mines RESOURCES COMMISSION Basin 215 Con IV Water-Well Record 10+ 22 arleton ruestorTownship, Village, Town or City..... County or Territorial District. n Village, Toyon or Gity)..... (day) (month) (year) **Pumping Test** Pipe and Casing Record Casing diameter(s), Static level Length(s) Pumping rate Pumping level Type of screen nour Duration of test Length of screen Water Record Well Log Depth(s) at which water(s) Kind of water No. of feet From To (fresh, salty, or sulphur) Overburden and Bedrock Record ft. ft. water rises found 18 D 50 AF For what purpose(s) is the water to be used? Location of Well house he duge ily In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside?...... uth flockest uplands Drilling firm Address rould Name of Driller ame 1. am/a Address Licence Number 337 I certify that the foregoing statements of fact are true. Date Angal 5 Signature of Licensee CSS.58 Form 5

L.I. 310/5-2.		GR	DUND WATER	BRANCH	
UTM $ Y B ^{Z}$ $ 4 S 3 8 6 0 ^{E}$ $ S ^{R}$ $ S 0 1 2 3 3 0 ^{N}$ The Ontario Water Resources	urces (Commission A	NOV 14		2100
Elev. 12 R 10131215 WATER WEL	L	RECC	STILL S CO	MMISSION	
Basin 215 Correton To County or District Correton To Con 4 R F Lot P. T.22 D	ownshi	p, Village, To	wn or City G	TO	
Con 4 R F	Iress.	28 01	day	month . Ottawa	year) 2, Ont.
Casing and Screen Record			Pumping		
Inside diameter of casing 6 3/16	Stat	IC AVAL	201		U
Total length of casing 21'	Test	-pumping rat			G.P.M.
Type of screen	Pun	nping level	10	1 hm	
Length of screen	Dur	ation of test p	umping	clea	r
Depth to top of screen	Wa	ter clear or clo	udy at end of	test clea 80	Н G.P. M .
Diameter of finished hole 6"	Red	commended p	umping rate 80	fert below	w ground surface
	wit	h pump settin	g of		Record
Well Log				Depth(s) at	Kind of water
Overburden and Bedrock Record		From ft.	To ft.	which water(s) found	(fresh, salty, sulphur)
Till and Bilders rest. Grey hard lime e	tone	•		85	fresh
and sand stone		10	27		
		25	07		
SANG STONE	Charles in such	25			
BOULDER TILL	Chailte Strate in Statistic X and an and the Statistic	25 O	16		
BOULDER TILL		25 0 /6	16		
SANG STONE			16 25 89	85	Facst
BOULDER TILL HARD GREY LIMESTONE SANDSTONE		/6 25	25 89 Location		Farst
SAND STONE BOULDER TILL HARD GREY LIMESTONE SMADSTONE For what purpose(s) is the water to be used?		16 25 In diagra	25 S 9 Location m below show	distances of we	Fats#
SAND STONE BOULDER TILL HARD GREY LIMESTONE SANDSTONE For what purpose(s) is the water to be used? Co-operative Valley		16 25 In diagra	25 S 9 Location m below show		Fats#
Sand Store BOULDER TILL HARD GREY LIMESTONE SMADSTONE SMADSTONE For what purpose(s) is the water to be used? Co-operative Valley		16 25 In diagra	25 S 9 Location m below show	distances of we	Fars#
SAND STONE BOULDER TILL HARD GREY LIMESTONE SANDSTONE For what purpose(s) is the water to be used? Co-operative Valley		16 25 In diagra	25 S 9 Location m below show	distances of we	Fats#
Sand Store BOULDER TILL HARD GREY LIMESTONE SMADSTONE SMADSTONE For what purpose(s) is the water to be used? Co-operative Valley	ding to a second	16 25 In diagra	25 Eocation m below show lot line. Inc A	distances of we	Fats#
Sand Store Boulder Till Hard brey Limestone Spandstone Spandstone <td>ding to a second</td> <td>16 25 In diagra</td> <td>25 Eocation m below show lot line. Ind /50'</td> <td>distances of we</td> <td>Fats# Il from arrow.</td>	ding to a second	16 25 In diagra	25 Eocation m below show lot line. Ind /50'	distances of we	Fats# Il from arrow.
Sand Storing BOULDER TILL HARD GREY LIMESTONE SANDSTONE For what purpose(s) is the water to be used? CO-Operative Is well on upland, in valley, or on hillside? Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Licence Number 194	ding to a second	16 25 In diagra	25 Eocation m below show lot line. Ind /50'	distances of we	Fats#
Sand Stown Boulder Till Hard GREY LIMESTONE SMADSTONE For what purpose(s) is the water to be used? Co-operative Is well on upland, in valley, or on hillside? Valley Is well on upland, in valley, or on hillside? Valley Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Address 194 Licence Number 194 Name of Driller or Borer W. Roy	ding to a second	In diagra road and	25 Eocation m below show lot line. Ind /50'	distances of we	Fats#
Sand Store BOULDER TILL HARD GREY LIMESTONE SMADSTone SMADSTone For what purpose(s) is the water to be used? CO-operative Valley Is well on upland, in valley, or on hillside? Valley Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Licence Number 194 Name of Driller or Borer W. Roy Address Hull	ding to a second	In diagra road and	25 Eocation m below show lot line. Ind /50'	distances of we	Facs# Il from arrow.
Sand Storm BOULDER TILL HARD GREY LIMESTONE SMADSTONE For what purpose(s) is the water to be used? CO-Operative Valley Is well on upland, in valley, or on hillside? Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Licence Number 194 Name of Driller or Borer W. Roy Address Hull Date Oct 10466	ding to a second	16 25 In diagra	25 Eocation m below show lot line. Ind /50'	distances of we	Fats#
Sand Store BOULDER TILL HARD GREY LIMESTONE SMADSTone SMADSTone For what purpose(s) is the water to be used? CO-operative Valley Is well on upland, in valley, or on hillside? Valley Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Licence Number 194 Name of Driller or Borer W. Roy Address Hull	ding to a second	In diagra road and	25 Eocation m below show lot line. Ind /50'	distances of we	Fats#
Sand Storm BOULDER TILL HARD GREY LIMESTONE SMADSTONE For what purpose(s) is the water to be used? CO-Operative Valley Is well on upland, in valley, or on hillside? Drilling or Boring Firm J. B. Dufresne Co. Ltd. Address Ottawa, Ontario. Licence Number 194 Name of Driller or Borer W. Roy Address Hull Date Oct 10466	ding to a second	In diagra road and	25 Eocation m below show lot line. Ind /50'	distances of we	Fats#

$\begin{array}{c} 316/52\\ \hline 316/52\\ \hline 316/52\\ \hline 1000\\ \hline 1$	LL REC Township, Village, T	ORD	JUNE	2180 1961 NATER MMISSION ET TER 61 year)
Casing and Screen Record		Pumpin	a Test	
	Static level			4
Inside diameter of casing	Test pumping r	ate		4 G.P.M.
Total length of casing	Pumping level		8	
Type of screen	Duration of test	aumping	14	tr
Length of screen				CEAR
Depth to top of screen Diameter of finished hole 4 °				♀ G.P.M.
Diameter of finished hole				w ground surface
	with pump setti			r Record
Well Log	Duran	Ta	Depth(s) at	Kind of water
Overburden and Bedrock Record	From ft.	To ft.	which water(s) found	(fresh, salty, sulphur)
LOAM	0	6		
GAEY LOMESTANE	6	55	55	Forest
For what purpose(s) is the water to be used? HOUSE Is well on upland, in valley, or on hillside? Drilling or Boring Firm NI MEAGHER Address	road and		of Well distances of we dicate north by	
Address Licence Number Name of Driller or Borer Address Date (Signature of Licensed Datting or Boring Contractor) Form 7 15M Sets 60-5930	Stor 2 Stor 2 St	, ,	H H	¹ δ. 20 2 μ. 10 μ.
OWRC COPY	T. C.			

$\frac{3}{5^{ \mathbf{r} }} = \frac{3}{5^{ $	Durres Commission		ad water bran 15 N 500 5 1962	
Elev. 4 R 031/15 WATER WEI Basin 25 County or District	LL REC	ORD	FLOU.	e ESTER
Con. 4 R = Lot 21 22	Date completed			
Casing and Screen Record		Pumpi	ng Test	
Inside diameter of casing	Static level		×	
Total length of casing			5	G.P.M.
Type of screen			10	
Length of screen	Duration of test	pumping	1HR	
C Servit			of test C	
Depth to top of screen Diameter of finished hole			<u>ج</u>	
				w ground surface
Well Log				r Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
CLAY	0	21		
LIMESTM	21	46	46	F
For what purpose(s) is the water to be used?		Location	of Well	
Hone	0		w distances of we	
Is well on upland, in valley, or on hillside?	road and	lot line. Ir	ndicate north by	arrow.
Drilling or Boring Firm				5
MMEAGHER			eor	, 1
Address.			Lo	20
1.9		N		
Licence Number 6/8		<u>(</u>)		
Name of Driller or Borer SAME	· 7	· V/		
Address		45	1 #	
Date		-į •	"	
(Signature of Licensed Dilling or Boring Contractor)			,	
Form 7 10M-62-1152	1			

. 125 1	ONTARIO Well Drillers A f Mines, Provin			DEC ~ G 19 EOLOGICAL S	
Water			, DE	PARTMENT N	
C. I. T. T. I. Disting (ARILE TAN)	Town	or City)			
Date Completed	ss L.	ei//://///	\$3.37.00	·	
Pipe and Casing Record		Pu	imping Test		
Casing diameter (s) 5 Length (s) of casing (s) 6. Type of screen. Length of screen Distance from top of screen to ground level.	 Static level Pumping level Pumping rate Duration of t 	el. 20. 8 G/ rest. 30	214 4 in		
Is well a gravel-wall type?		n cylinder or	bowls to ground	l level	
Kind (fresh or mineral)	Water Record		1	1	
Quality (hard, soft, contains iron, sulphur, etc.)	r.d		Depth(s) to Water Horizon(s)	Water	Water Ris
Appearance (clear, cloudy, coloured)	n 100 ¹ B ur	77	30 60' 19'	<u> </u>	30 55 74'
For what purpose(s) is the water to be used? $Pare$ How far is well from possible source of contamination? What is the source of contamination? β HR.N.	n 100 ¹ B ur	77	<u> </u>	J .,'	
For what purpose(s) is the water to be used?	nade of water	77.	<u> </u>	J .,'	1
For what purpose(s) is the water to be used?	77. 	77. To .5ft. \$0'	<u>co'</u> <u>19'</u> <u>Loc:</u> In diagram b	ation of Well pelow show disc poad and lot li by arrow.	l tances of
For what purpose(s) is the water to be used?	77. 100' <i>Bar</i> made of water From 0 ft.	77. To .5ft. \$0'	CO' 19' Loc: In diagram t well from ro dicate north wy 31 Of/au 70 550 y Ps	ation of Well below show dist boad and lot li by arrow. $y_a \rightarrow - \gg n$.	l tances of ne. In-
For what purpose(s) is the water to be used?	77. 100' <i>Bar</i> made of water From 0 ft.	77 To 5. ft. 9 0' <u>4</u>	CO' 19' Loc: In diagram b well from re dicate north wy 3: 07/44	ation of Well below show dist boad and lot li by arrow. $y_a \rightarrow - \gg n$.	l tances of ne. In-
For what purpose(s) is the water to be used?	77. made of water From 0 ft. 5 ⁻ 	77 To .5. ft. 90' 1/3 1/3	CO' 19' Loca In diagram to well from ra dicate north wy 31 Offau 70 D Soype 4 -2CC	ation of Well below show disc boad and lot li by arrow. $y_a \rightarrow - > n$.	I tances of ne. In-

N.F. 316/52 UTRONEZ 4154101610 E			15 N	· 2248
Elev. 4 R OBILO WATER WEI Basinty of District CARLETON	LL RECO	ORD	0 1440 1 % nIScuriczi colen GLOUC	STER
Con. 5 R.P. Lot 21	Date completed	?5 (day 7/2	4 month R.R#6 01	1966 _{year)} TAWA, ONT
Casing and Screen Record			ng Test	
Inside diameter of casing 6 1/4 "	Static level			
Total length of casing 21' 3"	Test-pumping ra	te 3		G.P.M.
Type of screen	Pumping level	40		
Length of screen	Duration of test p	umping	1 1/2 HRS	
Depth to top of screen	Water clear or clo	oudy at end o	f test CLEA	R
Diameter of finished hole 6	Recommended p			
	with pump setting	g of 🛛 🖌	6 feet belo	w ground surface
Well Log				r Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
TOP Soil	0			Fresh
SAND STONE	<u></u>	98	80 -97	1703/1
·			_	
For what purpose(s) is the water to be used? $hd USTry$		Location	of Well	
/	0		distances of wel dicate north by	
Is well on upland, in valley, or on hillside? $Up/a \gamma d$	Toat and		dicate north by	anow.
Drilling or Boring Firm $MCLEAN$ WATER SUPPLY LTP. Address 1532 RAVEN AVE	UN .	N R	10/20	
OTTAWA, ONT.		707	51	
Licence Number 2154	I	~ -10		
Name of Driller or Borer LOUIS BULTOWS	2	(i	10 2150	,
Address			•	
Date APR. 26-1966 a. J. Scharl				
(Signature of Licensed Drilling or Boring Contractor)				
Form 7 15M-60-4138				
OWRC COPY			C 53.	58

-9 xi		V//	TER RESOURCES DIVISION	e
UTM 18^{2} 453960^{2}		n.	15 N 141355	· 2249
5 R 5011786 Gthe Ontario Water Res		Act	,	
Elev. 4 R 0300 WATER WE	LL REC	ORDOU	ITARIO WATER RCES CONTERPORT	
Basinty 2 bistrict Li Ciarl			A A	lester
Con. 5 17 Lot 21	Date completed	19	nov.	1966
*	Idress RR#	(day	month	vear) D. X
			U	
Casing and Screen Record Inside diameter of casing 5"		Pumpir	ig Test	
Total length of casing 20°	Static level	15		~ ~ ~ ~
	Test-pumping r			G.P.M.
Type of screen				
Length of screen	Water clear or cl			
Depth to top of screen $5''$	Recommended		~	G.P.M.
	1		- /	w ground surface
Well Log	with pump setting			r Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
sand fill	0	4	83	pech
	4	85		0
Sandertione	/	00		
For what purpose(s) is the water to be used?		Location	of Well	
old house	In diagra		distances of we	ll from
Is well on upland, in valley, or on hillside? upland			licate north by	
Drilling or Boring Firm Capital Water		1.1		
Supply				
Address 14 ashford Dr 3				
ottawa 6			- 1 1	
Licence Number 2158		ひー く	ALU	
Licence Number $\mathcal{L} \mathcal{I} \mathcal{I} \mathcal{I}$ Name of Driller or Borer $\mathcal{H} \mathcal{I} \mathcal{O} \mathcal{O} \mathcal{O} \mathcal{O} \mathcal{O}$			<u> </u>	
Address			15 mi	60.
Date, nov 19, 1966		JAG .	15 mil	21
Halter Lavan agh (Signature of Licensed Drilling or Boring Contractor)		21		~
(Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138				
OWRC COPY	<i>a</i>	•	C55.38	
	9			

$\frac{31G/5\sigma}{UTM} = \frac{118}{118} = \frac{415141/110}{16912161} = 66$ $\frac{5}{16912161912161} = 66$ $\frac{5}{16912161912161} = 66$ $\frac{14}{16912161} = 66$	Townsh	nip, Village, T	ORD own or City. /4 (day	15 N JAN ONTARI RESOURCES Glocc Dec month	RÉSOURCES (ISION 2251 191965 $0 WATER COMMISSION e \le Tei1964year)Wy$
Casing and Screen Record			Pumpi	ng Test	
Inside diameter of casing 5"	Sta	tic level			
Total length of casing 10 '					G.P.M.
Type of screen					0.1.1.1.
Length of screen	1	•		197 3	5
Depth to top of screen					vdy
Diameter of finished hole 5"					G.P.M.
					ow ground surface
Well Log	wit				
wen Log				Depth(s) at	r Record Kind of water
Overburden and Bedrock Record		From ft.	To ft.	which water(s) found	
/o am		0	2		suprur)
Hard Sandstone		2	65		
Red Granile		65	- 79	60 - 79	fies17
For what purpose(s) is the water to be used? house Is well on upland, in valley, or on hillside? Drilling or Boring Firm Mchean Warei-Supply LTC/ Address 1532 Raven Ave OTTawa Licence Number 1328			lot line. In	of Well distances of we dicate north by Poad beTwe Sot 20421	arrow.
Name of Driller or Borer H. Sq //y Address Date De GA7, 1969 (Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138				- 0.55 Miles	
OWRC COPY	172 . ?	·/ • /····		Sec. 199	5

U.n. 118 4 53 8 9 0 $\frac{2}{2}$ 54 50 17 640 Elov. 4 0 308 The Ontario Water Res San 257 WATER WE County or District Catteron Con RF 5 Lot $\frac{2}{2}$	LL Towns Date c	REC hip, Village, to completed	ORD	month	Уеат)
	ddres	s. Long	Daurt, Unte		
Casing and Screen Record	-	<u> </u>	Pumping		
Inside diameter of casing 6"	Sta	atic level	21		
Total length of casing 15 •					G.P.M.
Type of screen	Pu	mping level		51	
Length of screen	Du	ration of test	pumping	Hour	
Depth to top of screen	Wa	ater clear or cl	oudy at end of	test cloudy	
Diameter of finished hole	1				G.P.M.
	wi	th pump settin	ng of 25 •	feet belo	w ground surface
Well Log			•	Water	r Record
Overburden and Bedrock Record		From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
Closely packed Boulders Very Abrassive Sandstone		0'	13 * 63 *	60 *	
For what purpose(s) is the water to be used?	[Location a	of Well	
Twailer Sales Depot		In diagram	n below show.	distances of wel	from
Is well on upland, in valley, or on hillside? Valley	C	LOJCES	and the second se	cate north by	
Drilling or Boring Firm	-	WN H	FALL	NORT	[* .
Blair Phillips Drilling Co. Ltd.,	ſ		2	(P	
Address			G.		
Licence Number 2779		3	£,	1	
Name of Driller or Borer J. Noo re		- V	60		
Address				7 10 LOT	LINE
Date 6 December 1958 (Signature of Licensed Drilling or Boring Contractor)					
Form 7 OWRC COPY				cs d	

	Ontario 1. PRINT ONLY IN S 2. CHECK X CORRE	CT BOX WHERE APPLICABLE	11 1 2	151	0717 10 CON., BLOCK, 1	TRACT. SURVEY	15 ETC.		22 23 LOT 25-
OUNTY OR DISTRICT	JON	TOWNSHIP, BOROUGH, CI	KCESTER	x ³	9	Y	- RF		23
		3/	HIGHW	AY L	EITRUN	1	DAY 15	MO. DO	KYR.
		ING A/K	19.20 4	0131412 26	RC. BASIN CO 4 25 30 31				
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10-13 20-23 15-18 12-23 12	FRESH 3 SULPHUR SALTY 4 MINERAL HOD 10 PUMPING 2 BAILER WATEI WWATER ISMINTES DOUP 2 BAILER FEET PUMPING 76 TEST HOLE PUMPING GPM FEET PUMPING GPM FEET DEEEP SETTING GOMESTIC 3 IRGATION 3 3 </td <td>DMG. MATERIAL THICKNESS FR INCHES INCHES INCHES INCHES FR IG011 INSTELL 12 GALVANIZED INCHES FR IG011 INSTELL 12 GALVANIZED INCHES INCHES FR IG011 ISTELL 19 INCHES INCHES OH IG011 ISTELL 250 OH INCHES OH IG011 ISTELL 250 OH INCHES OH IG011 STELL 26 INCHES INCHES INCHES IG011 STELL 26 INCHES INCHES INCHES IG011 STELL IST6 INTES INCHES INCHES IG012 IST6 OFEN HOLE INCHES INCHES INCHES IG011 STELL INCHES INCHES INCHES INCHES IG012 IST6 OFEN HOLE INCHES INCHES INCHES IG012</td> <td>DEPTH - FEET 10M TO 13-16 00/2 61 1 20-25 0048 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 12 14 10 10 12 10 10 10 10 10 10 10 10 10 10</td> <td>PLUGGING & SEA SET AT - FEET TO 0-13 14-17 8-21 22-25 30-33 80 OCATION OF WE SLOW SHOW DISTANCES OF WELL FI ICATE HORTH BY ARROW.</td> <td>LLL ROM ROAD AND</td> <td>41-44 80 FEET CORD ENT GROUT, KCKER, ETC.)</td>	DMG. MATERIAL THICKNESS FR INCHES INCHES INCHES INCHES FR IG011 INSTELL 12 GALVANIZED INCHES FR IG011 INSTELL 12 GALVANIZED INCHES INCHES FR IG011 ISTELL 19 INCHES INCHES OH IG011 ISTELL 250 OH INCHES OH IG011 ISTELL 250 OH INCHES OH IG011 STELL 26 INCHES INCHES INCHES IG011 STELL 26 INCHES INCHES INCHES IG011 STELL IST6 INTES INCHES INCHES IG012 IST6 OFEN HOLE INCHES INCHES INCHES IG011 STELL INCHES INCHES INCHES INCHES IG012 IST6 OFEN HOLE INCHES INCHES INCHES IG012	DEPTH - FEET 10M TO 13-16 00/2 61 1 20-25 0048 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 27-30 11 12 14 10 10 12 10 10 10 10 10 10 10 10 10 10	PLUGGING & SEA SET AT - FEET TO 0-13 14-17 8-21 22-25 30-33 80 OCATION OF WE SLOW SHOW DISTANCES OF WELL FI ICATE HORTH BY ARROW.	LLL ROM ROAD AND	41-44 80 FEET CORD ENT GROUT, KCKER, ETC.)
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Well ID Number: 1512375 Well Audit Number: Well Tag Number:

This table contains information from the original well record and any subsequent updates.

Well Location

Address of Well Location	
Township	GLOUCESTER TOWNSHIP
Lot	022
Concession	RF 04
County/District/Municipality	OTTAWA-CARLETON
City/Town/Village	
Province	ON
Postal Code	n/a
	NAD83 — Zone 18
UTM Coordinates	Easting: 454020.70
Municipal Dian and Sublat Number	Northing: 5017262.00
Municipal Plan and Sublot Number	_
Other	

Overburden and Bedrock Materials Interval

General Colour	Most Common Material	Other Materials	General Description	Depth From	Depth To
BRWN	OBDN	SAND		0 ft	9 ft
WHIT	SNDS			9 ft	74 ft

Annular Space/Abandonment Sealing Record

DepthDepthType of Sealant UsedVolumeFromTo(Material and Type)Placed

Method of Construction & Well Use

Method of Construction Well Use Diamond

Domestic

Status of Well

Water Supply

Construction Record - Casing

Inside	Open Hole or material	Depth	Depth
Diameter		From	To
2 inch	GALVANIZED OPEN HOLE		20 ft 74 ft

Construction Record - Screen

Outside Diameter Material Depth Depth From To

Well Contractor and Well Technician Information

Results of Well Yield Testing

Duration of Pumping 2 h Final water level 12 If flowing give rate 35 Recommended pump depth 35 Recommended pump rate 8 G	EAR
Pumping Rate8 GDuration of Pumping2 hFinal water level12If flowing give rate35Recommended pump depth35Well ProductionPU	
Duration of Pumping2 hFinal water level12If flowing give rateRecommended pump depth35Recommended pump rate8 GWell ProductionPU	
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Recommended pump depth35Recommended pump rate8 GWell ProductionPU	ft
Recommended pump rate8 GWell ProductionPU.	
Well ProductionPU	ft
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Draw Down & Recovery

Draw Down Time(min)	Draw Down Water level	Recovery Time(min)	Recovery Water level
SWL	6 ft		
1		1	
2		2	
3		3	
4		4	
5		5	
10		10	
15	12 ft	15	
20		20	
25		25	
30	12 ft	30	
40		40	
45	12 ft	45	
50		50	
60	12 ft	60	

Water Details

Water Found at Depth	Kind
74 ft	Fresh

Hole Diameter

Depth Depth From To Diameter

Audit Number:

Date Well Completed: November 27, 1972

Date Well Record Received by MOE: March 07, 1973

Updated: February 2, 2018 Rate <u>Rate</u> Share <u>facebook twitter Print</u> Tags

• Environment and energy,

		The Ontario Water R	VIRONMENT Resources Act	
				31 G/5a
Ontario	1. PRINT ONLY IN SI 2. CHECK 🛛 CORRE	CT BOX WHERE APPLICABLE	1513436	S.FC_
LETRIN	Alama - Culito	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	IV RFC	COMPLETED 44-53
UNITED CO	- OP OF CNTARE	The second	NTARIO. DAY.	16 NO. 08 YR.
21)	LISIS	5.01/17.21/15	ELEVATION RC. BASIN CODE II	
L'	LO	G OF OVERBURDEN AND BEDROC	K MATERIALS (SEE INSTRUCTIONS)	
GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET
Brown	Top Soil		Seft	0 4
Brown	Seil	Boulder	Hard	4 12
Grey White	Limestone	Clay Limestone Grev	Soft Percus Medium Hard	12 16 16 50
WATER FOUND AT - FEET 00 48 10-13 1 2 2 2 15-18 1 2 2 2	AG Q2	51 CASING & OPEN HOLE RI INSIDE DIAM INCHES MATERIAL INCHES MATERIAL INCHES FROM O6 10-11 2 STEEL 12 STEEL 12 STEEL 2 GAUVANIZED 3 GORCRETE 4 OPEN HOLE	CORD PTH - FEET M 22 13-16 0 0 2 2 61 PLUGGING & S	DIAMETER 34-38 LENGTH INCHES DEFINITO TOP OF SCREEN FR BEALING RECORD
25-28 1 [2 [30-33]1 [] FRESH 3 _ SULPHUR ²⁴] SALTY 4 _ MINERAL] FRESH 3 _ SULPHUR ²⁹] SALTY 4 _ MINERAL] FRESH 3 _ SULPHUR ³⁴ 80] SALTY 4 _ MINERAL	17-16 1 □ STEEL 19 2 □ GALVANIZED 3 □ CONCETE 4 □ OPEN HOLE 24-25 1 □ STEEL 26 2 □ GALVANIZED 3 □ CONCETE 4 □ OPEN HOLE	Z0-23 DEPTH SET AT - FEET MATERIA FROM TO MATERIA 27-30 18-21 22-25 26-29 30-33 60	AL AND TYPE (CEMENT GROU
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Homes Admission bare Licence number 2557	
SIGNATIONS CONTRACTOR SUBMISSION DATE 2552.	
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WATER	RECORD	51 CASING	S & OPEN HOL	ERECORD	Z SIZE (S, OF	OPENING 31-33 0	IAMETER 34-38	LENGTH 39-4
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BHALLOW C	DEEP SETTING	23 FLET RATE			11-	and the New Y	14 1=	*
FINAL	1 D WATER SUPPLY	L D AGANDONE	D. INSUFFICIENT SUPP D POOR QUALITY	LY			12	·
STATUS OF WELL	3 TEST HOLE						E	. <u>(. * /</u> *)
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10 14 1	RECORD	51 C/	ASING & OF	PEN HOLE					31-33 Diamet	er ³⁴⁻³⁸ Lei	75 ngth 3!
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2 🗌 :	Salty 6 Gas	2	Steel 26 Galvanized Concrete		-	27-3	18-21	22.25	PMO.	you	J
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¹ □ Cable tool ² □ Rotary (com ³ □ Rotary (reve ⁴ □ Rotary (air) ³ Name of Well Contrac AIF - Ru	sentional) ⁵ ☐ Air percussion ⁶ ☐ Boring ⁷ ☐ Diamond ⁸ ☐ Jetting	,(tel	¹⁰ Digging ¹¹ Other) or's Licence No.	S NL	ata burce	11	19	59-62 Date r	eceived	862 2001
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grey Sands " Cones " Sands	tone Dre stone						0 7 98 127	7 98 12 22
31][111]	1.1.1.1.1		
32 41 WATER RECORD 41 WATER RECORD Vater found at - feet 32 41 WATER RECORD Water found at - feet Kind of water 32 41 Winerals 32 41 Fresh 32 Sulphur 14 32 Sulphur 14 32 Sulphur 19 32 Sulphur 19 32 Sulphur 19 32 Sulphur 14 32 Sulphur 14 32 Sulphur 14 33 Sulphur 19 34 Fresh 3 Sulphur 20-23 1 Fresh 3 Sulphur	Inside diam inches 10-11 1 Steel 2 Galvani 3 Concre 4 Open h 3 Plastic 17-18	12 inches ized te ole 19	From	- feet To 13-16 Z Z 20-23 61		ype	ALING RECORI	fe o of screen 41-44 feet
2 Salty 4 Minerals 2 Salty 6 Gas 25-28 1 Fresh 3 Sulphur 29 2 Salty 4 Minerals 6 Gas 30-33 1 Fresh 3 Sulphur 34 2 Salty 4 Minerals 24 2 Salty 6 Gas 34	60 60 60 5 0 Plastic	te ole ized te ole	0 20	ω			type (Cement grout, t Internet grout, t	Dentonite, et
71 Pumping test method 10 Pumping rate, Pumping test method 1 Pumping test method 10 Pumping rate, Bailor 1 Static level end of pumping 25 Water level end of pumping 1 1921 12.00 15 minutes 56.28 1 10.01 17.00 16 feet 1 feet 16 feet 16 feet 1 forming give rate 38-11 Pump intake s 1 Recommended pump type Recommended Recommended	GPM	15-16 Hours Mins 32-34 do flest 45-37 Reet 60 minutes 15-37 Reet 46-49 anded → 46-49		In diagram bel Indicate north	low show dis	ION OF WELL stances of well	from road and k	ot line.
Cobservation well Cobservation well Cobservation Cobserva	d, poor quality ¹⁰ d (Other)	g GPM Unfinished Replacement well			Bla	SRC 80	<u>k</u>	
WATEP USE 55-56 1° Domestic 5 2 Stock 6 3 Irrigation 7 4 Industrial 8 Cooling & METHOD OF CONSTRUCTION 57 1 Cable tool 5// Air percus 2 Rotary (conventional) 6 Boring	10 🗌 (pply air conditioning sion 9 🗌 [Driving Digging		/		•		
A Detroin (reverse) A Detroin (ar) B Detroin B Detroin S Detroin S		ontractor's Licence No.			contractor		248	63-68
Addess R# Richw Name of Well Technician	glottal 1 Wind, Dr	119 F chnician's Licence No	Date	ce of inspection narks	II	ector		2003
Shannon-Hus Signatupe of Technician/Contractor	IRONMENT CO	mo yr	MINISTF				CSS.H	

	tario Ministry of the Environm		/ell Tag No	o. (Place Sti A05156			elow) 1 569	egulation	903 On		er Reso	ecord
Well Owner's				Le.	JI Add						Nell Con	structed
First Name Last Name E-mail Address Airport Golfland											by Well	Owner
Mailing Address (Street Number/Name, RR) Municipality						Provin	ario	Postal Code	cra 12	elephone N 13 85	202010200	168
6357 Emera	ald Links uction and/or Major Alter:	ation of a We	Greely			Ont	arro			1-1		
Address of Well L	Location (Street Number/Name	e, RR)	Tow	Inship				Lot 20	C	Concession 5		
Hwy 31 Gloucester County/District/Municipality City/Town/Village								20	Provinc		Postal	Code
	Ottawa Carleton Gloucester					Ontario						ЦЦ
UTM Coordinates		lorthing 5018088	GPS L	Unit Make	Model Garmi	n	Mode of 0	Deration:	Undiffer	entiated	Ave	raged
NAD 8 3 Overburden and	d Bedrock Materials (see ins		back of this fo	orm)	ourmit						0 4	144-4
General Colour	Most Common Material		ther Material				General D	lescription			From	(Metres) To
Brown	Clay	Stone	Stones			Pa	acked		-		0	3.35
Grey	Limestone					B	roken				3.35	4.57
Grey	Limestone					M	edium l	Hard				42.66
Grey	Limestone	Sands	stone La	tone Layers			ard			2	42.66	52.72
					Veron							
Donth Col -1 ///-	Annular Space/Aband	Ionment Seali ealant Used	ing Record	Volume	Placed			Results of We st of well yield,		d Testing aw Down		ecovery
			nd Type) (Cubic		Metres)	water wa			Time (Min)	Water Lew (Metres)	el Time (Min)	Water Level (Metres)
6.40 0) Grouted Bento	onite Slu	rry	.13	2m ³	Car stat		p to sand-free	Static Level	10.00	5 Static	
						10 Mar 1 10 10 10 10		ued, give reason	1	6.4	4	18.19
						Dumoin	g test meth	od	2	8.5		17.26
							ubmers		3		-	Takes She
Method	of Construction		Water Use	e		Pump in	ntake set at		4	9.9	0	15.67
Cable Tool			Commerci	and the second s	lot used lewatering	Pumpin	45. g rate (Litre			11.1	8	14.50
Rotary (Conve	rse) Driving	Livestock	Test Hole	🗆 N	Anitoring	18	54.		5	12.2	1000	13.32
Rotary (Air)		Irrigation Industrial	Cooling &	. Air Condition	ing		n of pumpir hrs +	ng min	10	16.1		9.44
Other, specify	/ D	Other, specify _			_	Final wa	iter level en	d of pumping	15	18.2		7.38
Water Supply		s of Well	Observatio	on and/or Mon	itoring Hole	(Metres)	21. mended pu		20	19.5	1 20	6.24
Replacement		Contraction of the second	Other, sp		1)	Sh	allow 🔀	Deep	25	20.3	6 25	5.61
Recharge Wei		Contraction of the second s					mended pu		30	20.9	30	5.18
		on of Well				30.4 Recom	mended pu		40	21.6	40	4.75
- all property bou	a map below showing: undaries, and measurements suffi	icient to locate th	ne well in relat	tion to fixed po	oints,	(Litres/r	45.		50	22.0	50	
- detailed drawing	uting the North direction gs can be provided as attachmen	its no larger than	n legal size (8	5° by 14")	£	(Litres/r	ng give rate nin)		60	22.1	00	
- vidigital pictures	s of inside of well can also be pro	VIDED			1			Wate	er Deta			
-	Golf Shop						found at D		of Wate	Sally	Test	ed
t	Gold Shap					51L Water	50 Metres	Gas Gas Kind	of Wate		Sulphui	
25							Metres	Gas F			Sulphur	Minerals
5						Water	found at E Metres		of Wati resh		Sulphur	Minerals
X						Cas	ing Used					II Details
, SA	Blais Rd.						anized	Galvanized		iameter of th	ne Hole (C	Centimetres)
P						Stee	il eglass	Steel	D	15 Depth of the l	.39 Hole (Met	res)
Date Well Com	npleted Was the well owner's in		ate the Well R			Plas	1 m 1 m 1 m 1 m 1 m 1 m 1 m 1 m 1 m 1 m	Plastic			.72	
(yyyy/mm/dd)	package delivered?	22 Ditta /	elivered to We 2008/07		vy/mm/dd)	Cor		Concrete		Vall Thickne	.48	5)
_2008/07/	Well Contractor and V		an Informat	tion				and Screen Use	lr	nside Diame	ter of the	Casing (Metres
	e of Well Contractor			Contractor's L	100	Disinfe	pen Hole		0	25 Depth of the	.86 Casing (A	(etres)
	Water Supply Ltd. ess (Street No /Name, number,		Municipa	lity	5 8	100000000000000000000000000000000000000	es 🗌 No	,		a value da da	to 6	
Box 490				tsville		A	la		Well	Only Contractor	No	
Province Ontario Bus.Telephone		ness E-mail Ad fice @ ca Il Technician (L	pitalwa	iter.ca irst Name)		Audit M	Z / I			of Inspectio		m/dd)
613 836 Well Technician's	5 1766 Mille s Licence No. Signature of Tech	er, Stepj	hen	te Submitted (2008/07	yyyy/mm/dk /16	d) Rema		2008				
0 0 0506E (11/2006)	9 7 Mulufme	rat			ry's Cop	by				© Que	en's Printe	r for Ontario, 20

APPENDIX E Moisture Surplus Printout

Ottawa	Airport, C	N	O WATE	ttawa_ R BUDG	50mm_V ET MEA	VBNRMSI ANS FOI	D.txt R THE F	PERIOD	1950-2	010	DC20492
	45.32 5 75.67			LDING			50 MM 30 MM		AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10.6 -8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1 1.3 -7.0 5.9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 79 97 94 83 66 35 10 1 503	$\begin{array}{c} 0\\ 0\\ -1\\ -19\\ -41\\ -34\\ -9\\ -1\\ 0\\ 0\\ -105\\ \end{array}$	27 29 107 104 13 4 3 1 7 24 50 38 407	83 110 64 0 0 0 0 0 0 9 47	50 50 50 32 14 5 20 37 49 50	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, O	N	STAN	DARD D	EVIAT	IONS F	OR THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4 1	1 4 8 11 26 30 30 14 7 4 1	0 0 5 26 31 32 13 3 0 0	31 37 53 84 21 17 12 5 20 27 30 29	43 59 83 0 0 0 0 0 0 13 34	0 0 2 19 19 14 16 21 19 6 0	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_75mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20									DC20492		
	45.32 5 75.67			LDING			75 MM 45 MM		AT IND		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10.6 -8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1 1.3 -7.0 5.9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 107 104 85 66 35 10 1 526	$\begin{array}{c} 0 \\ 0 \\ 0 \\ -10 \\ -32 \\ -32 \\ -9 \\ -1 \\ 0 \\ 0 \\ -84 \end{array}$	27 29 107 104 13 4 2 1 4 15 42 36 384	83 110 64 0 0 0 0 0 0 9 47	75 75 75 28 10 12 26 52 71 75	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIATI	IONS FO	OR THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4	1 4 8 12 19 28 29 14 7 4	0 0 0 19 30 31 14 2 0 0	30 37 53 84 21 17 11 5 17 23 33 30	43 59 83 0 0 0 0 0 0 13 34	0 0 2 22 28 22 23 29 28 11 3	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_100mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20										DC20492	
	45.32 G 75.67							HE A.	AT IND		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 112 115 88 66 35 10 1 545	0 0 0 -4 -21 -29 -8 -1 0 0 -63	25 28 106 104 13 4 2 1 3 10 34 33 363	83 110 64 0 0 0 0 0 0 9 47	99 99 100 100 81 47 19 18 32 63 91 97	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIATI	ONS FO	OR THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4 1	1 4 8 12 25 29 14 6 4 1	0 0 0 11 26 30 13 2 0 0	30 37 53 84 21 17 11 5 21 34 30	43 59 83 0 0 0 0 0 0 13 34	5 3 2 22 34 30 35 36 19 8	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_125mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20										DC20492	
	45.32 G 75.67							HE A.	AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 115 122 92 67 35 10 1 560	$\begin{array}{c} 0 \\ 0 \\ 0 \\ -1 \\ -13 \\ -25 \\ -7 \\ -1 \\ 0 \\ 0 \\ -47 \end{array}$	24 28 105 104 13 4 2 1 3 9 27 29 349	83 110 64 0 0 0 0 0 0 9 47	122 123 125 125 106 69 33 28 41 74 108 119	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIATI	IONS FO	R THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4 1	1 4 8 12 9 21 26 13 6 4 1	0 0 0 4 23 28 11 2 0 0	31 37 54 84 21 17 11 5 14 20 32 30	43 59 83 0 0 0 0 0 0 13 34	10 8 0 22 39 37 38 42 42 25 14	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_150mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20										DC20492	
	45.32 G 75.67							HE A.	AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1	64 57 66 72 74 89 87 84 77 80 78 911	13 12 32 67 74 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 127 97 68 36 10 1 573	$\begin{array}{c} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -8 \\ -19 \\ -6 \\ -1 \\ 0 \\ 0 \\ -34 \end{array}$	23 26 103 104 13 4 2 1 3 8 23 26 336	83 110 64 0 0 0 0 0 0 0 9 47	144 146 150 131 93 52 41 54 88 126 140	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIATI	IONS FO	R THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4	1 4 8 12 8 18 22 12 6 4 1	0 0 0 1 18 23 10 2 0 0	31 37 54 84 21 17 11 5 14 19 30 29	43 59 83 0 0 0 0 0 0 13 34	15 12 0 22 41 42 44 49 47 31 20	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_200mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20										DC20492	
	45.32 G 75.67							HE. A.	AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12	-8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 132 106 70 36 10 1 589	0 0 0 -3 -11 -4 0 0 0 -18	21 24 99 103 13 4 2 1 3 7 19 22 318	83 110 64 0 0 0 0 0 0 9 47	187 191 199 200 181 143 97 78 89 123 164 182	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIATI	CONS FO	R THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4 1	1 4 8 12 9 11 16 10 6 4 1	0 0 0 0 10 16 8 1 0 0	30 36 55 83 21 17 11 5 14 19 29 28	43 59 83 0 0 0 0 0 0 13 34	24 20 4 22 41 48 54 59 55 41 29	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_225mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20										DC20492	
LAT. LONG	45.32 5 75.67	WA LO	TER HO WER ZO	LDING	CAPAC	ΙΤΥ2 1	225 MM L35 MM	HE A.			
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12		64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 133 109 71 36 10 1 594	0 0 0 0 -2 -8 -4 0 0 0 -14	21 24 97 103 13 4 2 1 3 7 18 21 314	83 110 64 0 0 0 0 0 0 9 47	209 214 224 225 206 168 121 99 109 143 185 204	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, O	N	STAN	DARD D	EVIAT	CONS FO	OR THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4	1 4 8 12 9 10 14 10 6 4 1	0 0 0 0 7 13 7 1 0 0	30 36 56 82 21 17 11 5 14 19 29 28	43 59 83 0 0 0 0 0 0 13 34	28 24 7 22 41 49 58 63 58 44 33	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_250mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20									DC20492		
	45.32 5 75.67							HE. A.	AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12	-8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 134 111 72 36 10 1 598	0 0 0 0 -1 -6 -3 0 0 0 0 -10	20 23 96 102 13 4 2 1 3 7 18 20 309	83 110 64 0 0 0 0 0 0 9 47	232 238 248 250 231 193 145 121 130 164 207 226	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, O	N	STAN	DARD D	EVIATI	ONS FO	OR THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4 1	1 4 8 12 9 9 12 9 7 4 1	0 0 0 0 5 11 6 1 0 0	29 36 56 21 17 11 5 14 19 29 28	43 59 83 0 0 0 0 0 0 13 34	32 27 22 41 50 61 66 47 36	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_265mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC20									DC20492		
	45.32 G 75.67		TER HO WER ZO	LDING	CAPAC	ITY2	65 MM 59 MM	HE/ A.	AT INC		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
31- 1 28- 2 31- 3 30- 4 31- 5 30- 6 31- 7 31- 8 30- 9 31-10 30-11 31-12 AVE	-10.6 -8.8 -2.7 5.9 13.0 18.3 20.8 19.5 14.6 8.1 1.3 -7.0 5.9 TTL	64 57 66 72 74 82 89 87 84 77 80 78 911	13 12 32 67 74 82 89 87 84 76 63 26 705	15 18 80 69 0 0 0 0 0 0 8 15 205	0 1 5 32 80 116 135 117 75 36 10 1 608	0 1 5 32 80 116 134 112 72 36 10 1 599	0 0 0 0 -1 -5 -3 0 0 0 -9	20 23 96 102 13 4 2 1 3 7 18 20 309	83 110 64 0 0 0 0 0 0 9 47	246 252 263 265 246 208 160 135 144 177 221 240	299 356 422 494 568 651 740 827 912 77 157 236
Ottawa	Airport, C	N	STAN	DARD D	EVIAT	ONS FO	R THE	PERIOD	1950-	2010	DC20492
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3.0 2.6 2.3 1.7 1.9 1.2 1.2 1.3 1.5 1.4 1.7 3.0	26 29 28 31 32 38 42 39 38 37 27 30	16 15 22 31 32 38 42 39 38 37 28 22	18 27 47 84 0 0 0 0 0 2 9 14	1 4 8 12 9 8 8 8 7 4	1 4 8 12 9 8 11 9 7 4	0 0 0 0 4 10 5 1 0 0	29 36 56 21 17 11 5 14 19 29 28	43 59 83 0 0 0 0 0 0 13 34	34 29 10 22 41 51 62 68 62 49 38	55 59 65 74 85 93 93 107 110 37 45 56

Ottawa_275mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DC204										DC20492	
	45.32 75.67		TER HO WER ZO	LDING	CAPACI	ΙΤΥ2 1	75 MM 65 MM	HE. A.	AT IND		36.41 1.075
DATE	TEMP (C)	PCPN	RAIN	MELT	PE	AE	DEF	SURP	SNOW	SOIL	ACC P
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Ottawa_400mm_WBNRMSD.txt Ottawa Airport, ON WATER BUDGET MEANS FOR THE PERIOD 1950-2010 DO							DC20492				
			TER HOLDING CAPACITY400 MM WER ZONE240 MM							36.41 1.075	
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APPENDIX F Premier Tech Aqua Report

WASTEWATER TECHNOLOGY

NSF/ANSI Standard 245 - *Wastewater Treatment Systems – Nitrogen Reduction*

Final Report:

Premier Tech Aqua Ecoflo Coco Filter ECDn Model Series 15/03/055/0030



NSF International 789 N. Dixboro Road PO Box 130140 Ann Arbor, Michigan 48113-0140 USA Evaluation Report: Ecoflo Coco Filter ECDn Model Series Wastewater Treatment System

Under the provisions of NSF/ANSI Standard 245 Wastewater Treatment Systems – Nitrogen Reduction

January 2016

EXECUTIVE SUMMARY

Testing of the Ecoflo Coco Filter ECDn Model Series was conducted under the provisions of NSF/ANSI Standard 245 for Residential Wastewater Treatment Systems (April 2013 revision). NSF/ANSI Standard 245 was developed by the NSF Joint Committee on Wastewater Technology.

The performance evaluation was conducted at the NSF Wastewater Technology Testing Facility located in Waco, Texas, using wastewater diverted from the Waco municipal wastewater collection system, which serves predominantly residential development. The evaluation consisted of sixteen weeks of dosing at design flow, seven and one half weeks of stress testing and an additional two and one half weeks of dosing at design flow. The stress weeks were repeated due to sampling error and the test was extended for 35 weeks. Sampling started in the spring and continued through summer and fall, covering a range of operating temperatures.

Over the course of the evaluation, the average influent Total Nitrogen was 40.4 mg/L, ranging between 20.9 and 77.4 mg/L. The Ecoflo Coco Filter ECDn Model Series produced an average effluent Total Nitrogen of 18.6 mg/L, which resulted in a 53.89% reduction in the influent Total Nitrogen. The Ecoflo Coco Filter ECDn Model Series produced an effluent that successfully met the performance requirements established by NSF/ANSI Standard 245.

The Ecoflo Coco Filter ECDn Model Series produced an effluent that successfully met the performance requirements established by NSF/ANSI Standard 40 for Class I effluent:

The maximum 7-day arithmetic mean was 13 mg/L for CBOD₅ and 9 mg/L for total suspended solids, both below the allowed maximums of 40 and 45 mg/L, respectively. The maximum 30-day arithmetic mean was 5 mg/L for CBOD₅ and 5 mg/L for total suspended solids, both below the allowed maximums of 25 mg/L and 30 mg/L, respectively.

The effluent pH during the entire evaluation ranged between 6.6 and 7.3, within the required range of 6.0 to 9.0. The Ecoflo Coco Filter ECDn Model Series met the requirements for noise levels (less than 60 dbA at a distance of 20 feet), color, threshold odor, oily film and foam.

PREFACE

Performance evaluation of nitrogen reduction for residential wastewater treatment systems is achieved within the provisions of NSF/ANSI Standard 245: Wastewater Treatment Systems – Nitrogen Reduction (April 2013), prepared by the NSF Joint Committee on Wastewater Technology and adopted by the NSF Board of Trustees.

Conformance with the Standard is recognized by issuance of the NSF Mark. This is not to be construed as an approval of the equipment, but a certification of the data provided by the test and an indication of compliance with the requirements expressed in the Standard.

Systems conforming to Standard 245 are classified as having met the requirements of the Standard. Permission to use the NSF Mark is granted only after the equipment has been tested and found to perform satisfactorily, and all other requirements of the Standard have been satisfied. Continued use of the Mark is dependent upon evidence of compliance with the Standard and NSF General and Program Specific Policies, as determined by periodic reinspection of the equipment at the factory, distributors and reports from the field.

NSF Standard 245 requires the testing laboratory to provide the manufacturer of a residential wastewater treatment system a report including significant data and appropriate commentary relative to the performance evaluation of the plant. NSF policy specifies provision of performance evaluation reports to appropriate state regulatory agencies at publication. Subsequent direct distribution of the report by NSF is made only at the specific request of or by permission of the manufacturer.

The following report contains results of the entire testing program, a description of the plant, its operation and key process control equipment, and a narrative summary of the test program, including test location, procedures and significant occurrences. The plant represented herein reflects the equipment authorized to bear the NSF Mark.

CERTIFICATION

NSF International has determined by performance evaluation under the provisions of NSF/ANSI Standard 245 (revised April 2013) that the Model Number Ecoflo Coco Filter ECDn Model Series manufactured by Premier Tech Aqua has fulfilled the requirements of NSF/ANSI Standard 245. The Ecoflo Coco Filter ECDn Model Series has therefore been authorized to bear the NSF Mark so long as Manufacture continues to meet the requirements of Standard 245 and NSF General and Program Specific Policies.

General performance evaluation and stress tests were performed at the Wastewater Technology Site located at the NSF Wastewater Technology Testing Facility located in Waco, Texas. The raw wastewater used in the test was residential wastewater. The characteristics of the wastewater during the test are included in the tabulated data of this report.

The observations and analyses included in this report are certified to be correct and true copies of the data secured during the performance tests conducted by NSF on the wastewater treatment system described herein. The manufacturer has agreed to present the data in this certification in its entirety whenever it is used in advertising, prospectuses, bids or similar uses.

Sharon Steiner

Jenny Oorbeck General Manager Sustainability

Sharon Stiener Business Unit Manager Wastewater

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Appendices

Appendix A -	Plant Specifications and Drawings
Appendix B -	Standard 245 Section 8 - Performance testing and evaluation
Appendix C -	Analytical Results – BOD ₅ , CBOD ₅ , TSS, pH and Temperature
Appendix D -	Analytical Results – Nitrogen Analyses
Appendix E -	Owner's Manual

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1.0 PROCESS DESCRIPTION

To be treated, the wastewater first flows into a Primary tank where a primary treatment, gross solids sedimentation, takes place. The Ecoflo® Coco ECDn Model Series is based on a pre-denitrification approach: The ammonia is first converted into nitrates inside the Ecoflo® Coco filtering media (nitrification); then, nitrates are recirculated in a Primary tank and transformed in gaseous nitrogen (denitrification).

The wastewater entering the Ecoflo® Coco Filter is directed to the tipping bucket and evenly distributed onto distribution plates. These plates include channels and orifices that uniformly distribute the primary tank effluent over the surface of the filtering media. Distributed wastewater trickles downward into the filtering media where microorganisms, naturally attached onto the filtering media, degrade the contaminants through their metabolic reactions. A fraction of the treated wastewater is returned to the Primary/anoxic tank via the pumping station provided for that matter and the remaining fraction is directed toward the final disposal. The recirculation ratio is approximately two (2) times the daily flow (2Q).

2.0 PERFORMANCE EVALUATION

2.1 Description of Plant Evaluated

The Ecoflo® Coco Filter ECDn tested in this evaluation has a rated capacity of 14.1 gpd/ft² for and applied flow rate of 460 gallons per day (gpd). Specifications and drawings are included in Appendix A. The system is composed of a 920 gallon primary/septic tank equipped with an effluent filter, followed by the Ecoflo® Coco Filter operated in recirculation mode. The Ecoflo® Coco Filter was housed in a concrete shell. The filtering media consisted in a natural coco composed of fragments of coconut husks especially shaped and sized to treat residential wastewater. A securely fastened polyethylene lid limited the access to the filter.

The wastewater entered the primary/septic tank for primary treatment (separation of settleable solids). From the Primary tank, the pretreated wastewater flows to the Ecoflo® Coco Filter. Effluent from the primary/septic tank is gravity fed to a tipping bucket to alternately apply wastewater to the distribution plates. These plates included channels and orifices that uniformly distributed the settled wastewater to the top of the filtering media. The wastewater trickled down into the filtering media where microorganisms, naturally attached onto the coco fragments, degraded the contaminants through their metabolic reactions.

The treated effluent was collected at the bottom of the filter and directed to a pumping station with a minimum working capacity of 150 gallons. The pump, controlled by a time dosing control panel, allowed the recirculation, via a specially designed Premier Tech Aqua pressure flow divider (PFS-200DN), of 2/3 of the dose at the beginning of the treatment train (Primary tank) and 1/3 of the dose to the outlet pipe located on the side of the shell.

Normally, an Ecoflo® Coco Filter model with integrated pump would be used to ensure the recirculation. However, for the purpose of the certification, a pumping station was installed downstream of the Ecoflo® Coco Filter in order to recirculate part of the treated water to the Primary tank. To regulate the recirculation rate a minimum working capacity of 0.3Q (Q being the design daily flow rate) is required. As mentioned above, this volume of treated effluent can be provided either at the bottom of the Ecoflo® Coco Filter tank with an integrated pump or in an independent pumping station installed downstream of the filter.

Flow regulation can be achieved either by using a time dosing unit controlling the recirculation pump or, by a gravity flow regulator mounted on the outlet of the septic tank. Both Approaches provide equivalent flow regulation performance.

2.2 Test Protocol

Section 8 of NSF/ANSI Standard 40 protocol, "Performance Testing and Evaluation", is included in Appendix B. Start up of the plant was accomplished by filling the primary tank with 2/3 water and 1/3 raw sewage. The plant was then dosed at the design loading rate of 460 gpd as follows:

6 a.m. to 9 a.m. - 35 percent of daily rated capacity (160 gallons) 11 a.m. to 2 p.m. - 25 percent of daily rated capacity (120 gallons) 5 p.m. to 8 p.m. - 40 percent of daily rated capacity (180 gallons)

Dosing was accomplished by opening an electrically actuated valve to feed wastewater to the test plant. Ten gallon doses were spread uniformly over each dosing period to comprise the total dose volume for the period.

After a start up period (up to three weeks at the manufacturer's discretion), the plant is subjected to the following loading sequence:

Design loading-16 weeksStress loading-7.5 weeksDesign loading-2.5 weeksNote this test was extended to 35 weeks, stress was repeated due to sampling error.

During the design loading periods, flow proportioned 24-hour composite influent and effluent samples are collected three times per week. The influent samples are analyzed for five-day biochemical oxygen demand (BOD₅), total suspended solids (TSS), alkalinity, total Kjeldahl nitrogen (TKN), and ammonia-N. The effluent samples are analyzed for carbonaceous five-day biochemical oxygen demand (CBOD₅), TSS, alkalinity, TKN, ammonia-N and nitrite/nitrate-N concentrations. Onsite determinations of the influent and effluent pH, temperature and dissolved oxygen are made five days per week on grab samples.

Stress testing is designed to evaluate how the plant performs under non-ideal conditions, including varied hydraulic loadings and electrical or system failure. The test sequence includes (1) Wash Day stress, (2) Working Parent stress, (3) Power/Equipment Failure stress, and (4) Vacation stress. Detailed descriptions of the stress sequences are shown in Appendix B.

During the stress test sequences, 24-hour composite samples are collected before and after each stress dosing pattern. The analyses and on-site determinations completed on the samples are the same as described for the design load testing. Each stress is followed by seven consecutive days of dosing at design rated capacity before beginning the next stress test. Sample collection is initiated twenty-four hours after completion of Wash Day, Working Parent, and Vacation stresses, and beginning 48 hours after completion of the Power/Equipment Failure stress.

In order for the system to successfully pass the Standard 245 evaluation:

- (1) CBOD₅: The average of all effluent samples shall not exceed 25 mg/L.
- (2) TSS: The average of all effluent samples shall not exceed 30 mg/L.
- (3) Total Nitrogen: The average total nitrogen concentration of all effluent samples shall be less than 50% of the average total nitrogen concentration of all influent samples.
- (4) pH: Individual effluent values shall remain between 6.0 and 9.0 SU.

2.3 Test Chronology

The system was installed under the direction of the manufacturer from March 3, 2015 through March 11, 2015. The infiltration/exfiltration test, during which the entire system was tested for leaks, was completed on March 2, 2015. The unit was completely pumped out then filled with fresh water to allow set up and adjustment prior to the start of dosing. The fresh water was then pumped down by approximately one-third volume in the treatment unit. Dosing was initiated at the rate of 460 gallons per day beginning March 16, 2015. After a three-week start up period, the test was officially started on April 6, 2015. The stress test sequence was started on July 27, 2015 to September 8, 2015, and repeated September 21, 2015 to November 6, 2015. The stress weeks were repeated due to test site error and the test was extended to 35 weeks. During the second wash day stress, the system was mistakenly dosed at 520 gpd on each of the three wash days. Testing was completed on December 4, 2015.

3.0 ANALYTICAL RESULTS

3.1 Summary

Chemical analyses of samples collected during the evaluation were completed using the procedures in *Standard Methods for the Examination of Water and Wastewater*21st edition. Copies of the data generated during the evaluation are included in Appendix C. Results of the chemical analyses and on-site observations and measurements made during the evaluation are summarized in Table I.

Biochemical Oxygen Demand (n	Average	<u>Std. Dev.</u>	<u>Minimum</u>	Maximum	Median	Interquartile <u>Range</u>	
Influent (BOD ₅)	200	88	39	590	200	200-290	
Effluent (CBOD ₅)	4	3	1	34	3	3-6	
	-	U	•	04	0	00	
Total Suspended Solids (mg/L)							
Influent	190	83	26	600	180	180-250	
Effluent	2	2	1	10	2	2-4	
рН							
Influent	-	-	6.8	7.9	7.4	7.3-7.5	
Effluent	-	-	6.6	7.3	7.1	7.0-7.2	
Temperature (°C)							
Influent	28	3	22	32	28	28-31	
Effluent	28	3	20	33	28	28-30	
Dissolved Oxygen (mg/L)							
Primary Tank	1.0	1.0	0.2	2.4	0.5	0.4-1.3	
Effluent	4.0	2.0	0.5	8.0	4.0	3.9-5.4	
Alkalinity (mg/L)							
Influent	320	42	230	420	320	300-350	
Effluent	280	42	190	360	270	250-310	
,							
Total Kjeldahl Nitrogen							
Influent	40.1	12.6	20.7	76.9	37.8	39.9-47.0	
Effluent	14.9	9.3	2.3	33.3	11.5	8.6-21.1	
Ammonia-N	o= /				o / =		
Influent	25.1	8.3	7.1	44.3	24.7	18.4-30.5	
Effluent	12.5	8.4	1.0	28.9	10.2	6.8-18.4	
Nitrite/nitrate-N (mg/L)	0.05	0.40	0.05	0.00	0.40	0.00.0.50	
Influent	0.35	0.46	0.05	2.20	0.10	0.06-0.53	
Effluent	3.77	2.01	0.30	8.54	4.20	2.34-5.01	
Total Nitrogen							
Influent	40.4	12.6	20.9	77.4	37.9	30.3-47.7	
Effluent	40.4 18.6	8.1	20.9 6.7	77.4 34.4	37.9 15.9	12.6-24.8	
Linuent	10.0	0.1	0.7	34.4	10.9	12.0-24.0	

TABLE I. SUMMARY OF ANALYTICAL RESULTS

Notes: The median is the point where half of the values are greater and half are less. The interquartile range is the range of values about the median between the upper and lower 25 percent of all values. Criteria for evaluating the analytical results from the testing are described in Section 8.5 of NSF/ANSI Standard 40. In completing the pass/fail determination for the data, an allowance is made for effluent TSS and CBOD₅ during the first month of testing. The 30- and 7-day averages during this time may not equal or exceed 1.4 times the effluent limits required for the rest of the test. This provision recognizes that an immature culture of microorganisms within the system may require additional time to achieve adequate treatment efficiency. Effluent CBOD₅ and TSS concentrations from the Ecoflo Coco Filter ECDn Model Series during the first calendar month of testing were within the normal limits and did not need to use this provision.

Section 8.5.1.1 of the Standard provides guidance addressing the impact of unusual testing conditions, including sampling, dosing, or influent characteristics, on operation of a system under test. Specific data points may be excluded from 7- and 30-day average calculations where determined to have an adverse impact on performance of the system, with rationale for the exclusion to be documented in the final report. During the second wash day stress, the system was mistakenly dosed at 520 gpd on each of the three wash days. No impact was observed on the system under test and no data was excluded because of this testing error.

Sections 3.6 and 8.2.1 of the Standard define influent wastewater characteristics as they apply to testing under the Standard. Typical domestic wastewater is defined as having a 30-day average BOD₅ concentration between 100 and 300 mg/L and a 30-day average TSS concentration between 100 and 350 mg/L. The 30-day average influent remained inside this specified range for the duration of the test.

3.2 Biochemical Oxygen Demand

The five-day biochemical oxygen demand (BOD₅) and five-day carbonaceous biochemical oxygen demand (CBOD₅) analyses were completed using *Standard Methods for the Examination of Water and Wastewater* 21st edition. The results of both analyses are shown in Figure 1.

Influent BOD₅:

Individual influent BOD₅ concentrations ranged from 39 to 590 mg/L during the evaluation, with average concentration of 200 mg/L and a median concentration of 200 mg/L. Thirty day average concentrations ranged from 160 to 280 mg/L. The average influent BOD₅ delivered to the treatment unit was within the influent characteristics defined under Section 8.2.1 of NSF/ANSI Standard 245.

Effluent CBOD5:

Effluent CBOD₅ concentrations ranged from 1 to 34 mg/L over the course of the evaluation, with an average and median effluent CBOD₅ concentrations of 4 and 3 mg/L.

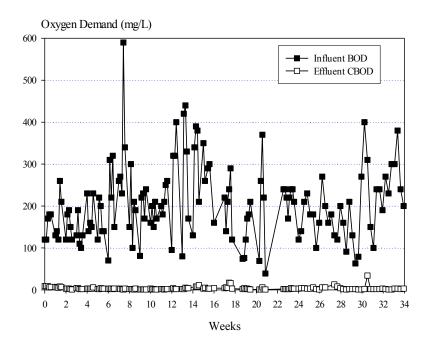


Figure 1. Biochemical Oxygen Demand

3.3 Total Suspended Solids

TSS analyses were completed using *Standard Methods for the Examination of Water and Wastewater*21st edition. The TSS results over the entire evaluation are shown in Figure 2. Data from the TSS analyses are summarized in Table I.

Influent TSS:

The influent TSS ranged from 26 to 600 mg/L during the evaluation, with an average and median concentrations of 190 and 180 mg/L. The 30-day average concentrations during the test ranged from 130-260 mg/L. The average influent TSS delivered to the treatment unit was within the influent characteristics defined under Section 8.2.1 of NSF/ANSI Standard 245.

Effluent TSS:

The effluent TSS concentration ranged from 1 to 10 mg/L during the evaluation, with an average and median concentrations of 2 mg/L.

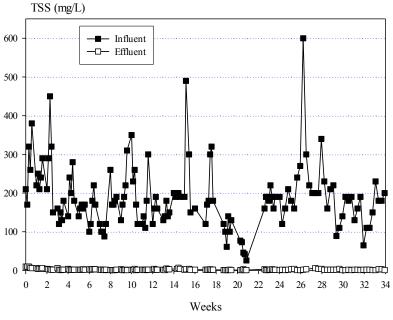


Figure 2. Total Suspended Solids

3.4 pH

Over the entire evaluation period, the influent pH ranged from 6.8 to 7.9 (median of 7.4). The effluent pH ranged 6.6 to 7.3 during the evaluation (median of 7.1); within the 6 to 9 range required by NSF/ANSI Standard 245. The pH data for the evaluation are shown in Appendix C.

3.5 Temperature

Influent temperatures over the evaluation period ranged from 22 to 32°C (median of 29°C). The temperature data are shown in Appendix C. The average influent temperature was within the characteristics defined under Section 8.2.1 of NSF/ANSI Standard 245.

3.6 Dissolved Oxygen

Dissolved Oxygen (DO) was measured in the primary tank effluent and effluent during the evaluation. The primary tank effluent DO ranged between 0.2 and 2.4 mg/L (median of 0.5 mg/L), while the effluent DO ranged between 0.5 and 8.0 mg/L (median of 4.0 mg/L). All dissolved oxygen data are shown in Appendix C.

3.7 Alkalinity

Alkalinity analyses were completed using *Standard Methods for the Examination of Water and Wastewater* 21st edition. The alkalinity results over the entire evaluation are shown in Figure 3. The influent and effluent alkalinities were all well within the range required by the Standard, and review of the nitrogen data indicates that alkalinity was not a limiting factor for nitrification in the system.

Influent Alkalinity

The influent alkalinity averaged 320 mg/L, ranging from 230 to 420 mg/L, with a median concentration of 320 mg/L. The influent alkalinity delivered to the treatment unit was within the influent characteristics defined under Section 8.2.1 of NSF/ANSI Standard 245.

Effluent Alkalinity

The effluent Alkalinity concentration ranged from 190 to 360 mg/L during the period when alkalinity samples were collected, with an average concentration of 280 mg/L and a median concentration of 270 mg/L.

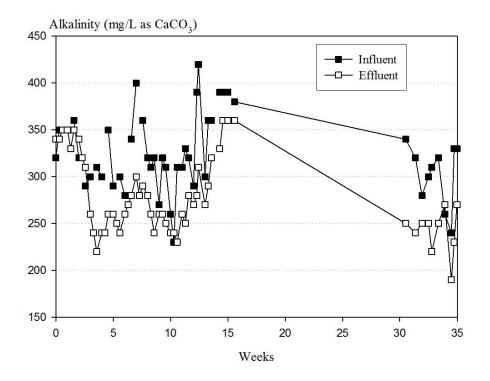


Figure 3: Alkalinity

3.8 Total Kjeldahl Nitrogen (TKN)

TKN analyses were completed using *Standard Methods for the Examination of Water and Wastewater* 21st edition. The TKN results over the entire evaluation are shown in Figure 4.

Influent TKN:

The influent TKN ranged from 20.7 to 76.9 mg/L during the evaluation, with average of 40.1 mg/L and a median concentration of 37.8 mg/L. The influent TKN delivered to the treatment unit was within the influent characteristics defined under Section 8.2.1 of NSF/ANSI Standard 245.

Effluent TKN:

The effluent TKN concentration ranged from 2.3 to 33.3 mg/L during the evaluation, with an average concentration of 14.9 mg/L and a median concentration of 11.5 mg/L.

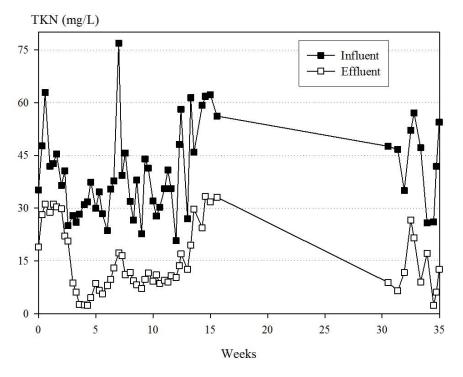


Figure 4: Total Kjeldahl Nitrogen

3.9 Ammonia-N

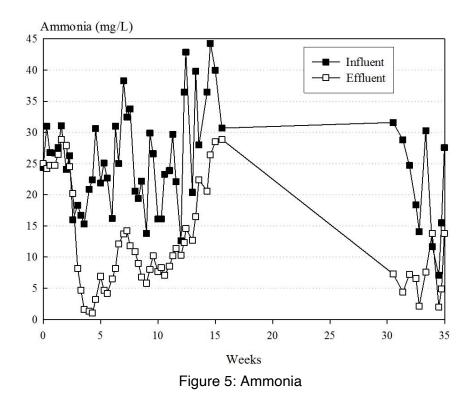
Ammonia-N analyses were completed using *Standard Methods for the Examination of Water and Wastewater* 21st edition. The Ammonia-N results over the entire evaluation are shown in Figure 5.

Influent Ammonia-N:

The influent Ammonia-N ranged from 7.1 to 44.3 mg/L during the evaluation, with an average and median concentrations of 25.1 and 24.7 mg/L.

Effluent Ammonia-N:

The effluent Ammonia-N concentration ranged from 1.0 to 28.9 mg/L during the evaluation, with an average of 12.5 mg/L and a median concentration of 10.2 mg/L.



3.10 Nitrite/nitrate-N

Nitrite/nitrate-N analyses were completed using *Standard Methods for the Examination of Water and Wastewater* 21st edition. The Nitrite/nitrate-N results over the entire evaluation are shown in Figure 6.

Influent Nitrite/nitrate-N:

The influent Nitrite/nitrate-N ranged from 0.05 to 2.2 mg/L during the evaluation, with average and median concentrations of 0.35 and 0.1 mg/L.

Effluent Nitrite/nitrate-N:

The effluent Nitrite/nitrate-N concentration ranged from 0.3 to 8.5 mg/L during the evaluation, with an average of 3.8 mg/L and a median concentration of 4.2 mg/L.

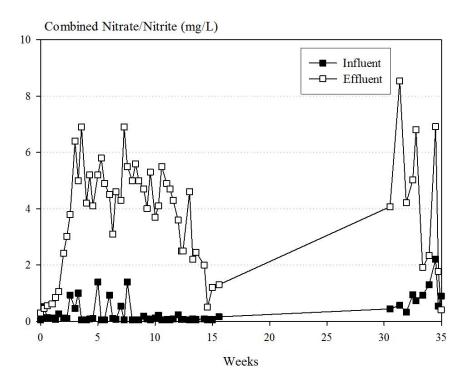


Figure 6: Effluent Nitrate/Nitrite

3.11 Total Nitrogen

Total Nitrogen (TN) is the sum of the total Kjeldahl nitrogen (TKN), nitrite (NO₂) and nitrate (NO₃) in a sample, and is expressed as mg/L as N. The TN results over the entire evaluation are shown in Figure 7.

Influent Total Nitrogen

The influent TN ranged from 20.9 to 77.4 mg/L during the evaluation, with average and median concentrations of 40.4 and 37.9 mg/L.

Effluent Total Nitrogen:

The effluent TN concentration ranged from 6.7 to 34.4 mg/L during the evaluation, with an average concentration of 18.6 mg/L and a median concentration of 15.9 mg/L. The Premier Tech Aqua Ecoflo Coco

Filter ECDn Model Series successfully met the requirements of Standard 245 by reducing the influent TN by 53.89%, which exceeds the pass/fail criteria of 50%.

Nitrogen Loading:

Over the course of the evaluation the influent Total Nitrogen loading averaged 0.15 lb/day. The Premier Tech Aqua Ecoflo Coco Filter ECDn Model Series achieved an average reduction of 0.08 lbs/day.

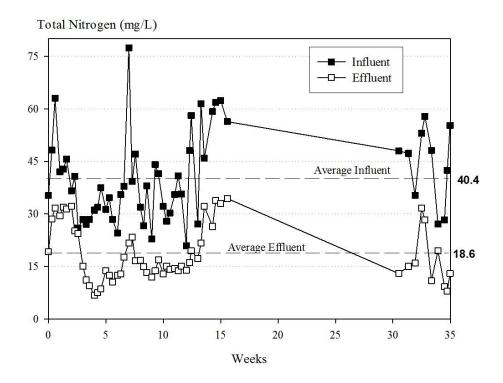


Figure 7: Total Nitrogen

4.0 **REFERENCES**

- American Public Health Association (APHA), American Water Works Association (AWWA) & Water Environment Federation (WEF): *Standard Methods for the Examination of Water and Wastewater*, 21st Edition, 2005 (hereinafter referred to as *Standard Methods*)
- 2. ANSI/AWS D.1.1/D1.1M:2010, *Structural Welding Code Steel and* ANSI/AWS D1.3/D1.3M:2008, *Structural Welding Code Sheet Steel*, 5th Edition, with Errata
- 3. NFPA 70®: National Electrical Code® (NEC®), 2011
- 4. NSF/ANSI 40, Residential Wastewater Treatment Systems
- 5. US EPA, Code of Federal Regulations (CFR), Title 40: Protection of Environment, July 1, 2010

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APPENDIX A

PLANT SPECIFICATIONS

PLANT SPECIFICATIONS Premier Tech Aqua Ecoflo Coco Filter ECDn Model Series <u>460 gpd</u>

<u>Plant Capacity</u> Design Flow	460 gpd
System Hydraulic Capacity Total Hydraulic Capacity	920 gallons
<u>Hydraulic Retention Time (at design flow))</u> Primary/septic tank	48 hours
Ecoflo Coco Filter Media	
Coco Shell Dimensions	Height:62 '' Width: 43-1/8 '' Length: 115-1/4''
Material: Fragments of natural coconut husk base Volume: 62.4 ft ³ Hydraulic loading rate: 14.1 gpd/ft ²	d media
Effluent Filter	
Manufacture Model #	Polylok PL-122
Pump	
Manufacture ABS	120V 60Hz 0.3 HP
Recirculation	
Recirculation rate: 2Q Hydraulic loading rate including recirculation: 42.3 Minimum working volume required for recirculatior	

Alarm & Time dosing Panel²

Manufacturer	Premier Tech Aqua
Model	TPA-350 I/E

¹ The minimum working volume can either be built in at the bottom of the Ecoflo[®] Coco Filter unit or in an independent pumping station located downstream.

² or equivalent time dosing panel

APPENDIX B

NSF STANDARD 245 PERFORMANCE EVALUATION METHOD AND REQUIREMENTS

8 Performance testing and evaluation

This section describes the methods used to evaluate the performance of residential wastewater treatment systems designed to remove nitrogen from residential wastewater. Performance testing and evaluation shall not be restricted to specific seasons.

8.1 Preparations for testing and evaluation

The system shall be assembled, installed, and filled in accordance with the manufacturer's instructions.

The manufacturer shall inspect the system for proper installation. If no defects are detected and the system is judged to be structurally sound, it shall be placed into operation in accordance with the manufacturer's start-up procedures. If the manufacturer does not provide a start-up procedure, $\frac{2}{3}$ of the system's capacity shall be filled with water and the remaining $\frac{1}{3}$ shall be filled with residential wastewater.

The system shall undergo design loading (see 8.2.2.1) until testing and evaluations are initiated. Sample collection and analysis shall be initiated within three weeks of filling the system and shall continue without interruption until the end of the evaluation period, except as specified in 8.4.2.

If conditions at the test site preclude installation of the system at its normally prescribed depth, the manufacturer shall be permitted to cover the system with soil to achieve normal installation depth.

When possible, electrical or mechanical defects shall be repaired to prevent delays. All repairs made during the performance testing and evaluation shall be documented in the final report.

The system shall be operated in accordance with the manufacturer's instructions. However, routine service and maintenance of the system shall not be allowed during the testing and evaluation period.

NOTE – The manufacturer may recommend or offer more frequent service and maintenance of the system, but for purpose of performance testing and evaluation, the service and maintenance shall not be performed beyond what is specified in this Standard.

8.2 Testing conditions, hydraulic loading and schedules

8.2.1 Influent wastewater characteristics

Except as required by NSF/ANSI 40 for systems seeking concurrent NSF/ANSI 40 and Nitrogen Reduction certification, the average wastewater characteristics delivered to the system over the course of the testing shall fall within:

- BOD5: 100 to 300 mg/L
- TSS: 100 to 350 mg/L
- TKN: 35 to 70 mg/L as N
- alkalinity: > 175 mg/L as CaCO3 (alkalinity may be adjusted if inadequate)
- temperature: 10 to 30 °C (50 to 86 °F)
- pH: 6.5 to 9 SU

Unless requested by the manufacturer, the raw influent shall be supplemented with sodium bicarbonate if the wastewater is found to be deficient in alkalinity. In addition, the influent shall be supplemented with urea to meet the required influent TKN concentration. The influent may also be supplemented with methanol to maintain a carbon:nitrogen ratio of no less than 5:1.

NOTE – For this testing, minimum alkalinity may be calculated as described in Annex A. If the influent temperature drops below 10 °C (50 °F), impacting the nitrification process, sample collection may be suspended until the influent temperature returns to 10 °C (50 °F).

8.2.2 Hydraulic loading

The performance of the system shall be evaluated for a minimum of 26 wks. During the testing and evaluation period, the system shall be subjected to 16 wks of design loading, followed by 7.5 wks (52 d) of stress loading, and an additional period of design loading to obtain a minimum of 55 influent and effluent data sets collected during non-stress dosing period.

8.2.2.1 Design loading

The system shall be dosed 7 d/wk with a wastewater volume equivalent to the daily hydraulic capacity of the system. The following schedule shall be adhered to for dosing:

Time Frame	Approximate % rated daily hydraulic capacity
6 a. m. – 9 a. m.	35
11 a. m. – 2 p. m.	25
5 p. m. – 8 p. m.	40

NOTE – An individual dose shall be no more than 10 gal (37.9 L), unless the dosage system is based on a continuous flow, and the doses shall be uniformly applied over the dosing period.

8.2.2.2 Stress loading

Stress loading sequences shall begin in week 17 of the testing and will be completed in the order listed in the following sections. Each stress sequence shall be separated by 7 d of design loading, as described in 8.2.2.1.

8.2.2.2.1 Wash-day stress

The wash-day stress shall consist of 3 wash-days in a 5-d period. Each wash-day shall be separated by a 24h period. During a wash-day, the system shall be loaded at times and capacities similar to those delivered during design loading (see 8.2.2.1). However, during the first two dosing periods per day, the design loading shall include 3 wash loads (3 wash cycles and 6 rinse cycles).

8.2.2.2.2 Working-parent stress

For five consecutive days, the system shall be subjected to a working-parent stress. During this stress, the system shall be dosed with 40% of its daily hydraulic capacity between 6:00 a. m. and 9:00 a. m. Between 5:00 p. m. and 8:00 p. m., the system shall be dosed with the remaining 60% of its daily hydraulic capacity,

which shall include 1 wash load (1 wash cycle and 2 rinse cycles).

8.2.2.2.3 Power/equipment failure stress

Power/equipment failure stress simulation shall consist of a flow pattern where approximately 40% of the total daily flow is received between 5 p. m. and 8 p. m. on the day when the power/equipment failure stress is initiated. Power to the system shall then be turned off at 9 p. m. and the flow pattern shall be discontinued for 48 h. After the 48-h period, power shall be restored and the system shall receive approximately 60% of the total daily flow over a 3-h period which shall include 1 wash load (1 wash cycle and 2 rinse cycles).

8.2.2.2.4 Vacation stress

Vacation stress simulation shall consist of a flow pattern where approximately 35% of the total daily flow is received between 6 a. m. and 9 a. m. and approximately 25% of the total daily flow is received between 11 a. m. and 2 p. m. on the day that the vacation stress is initiated. The flow pattern shall be discontinued for 8 consecutive days with power continuing to be supplied to the system. Between 5 p. m. and 8 p. m. of the ninth day, the system shall receive 60% of the total daily flow, which shall include 3 wash loads (3 wash cycles and 6 rinse cycles).

8.2.3 Dosing volumes

The 30-d average volume of the wastewater delivered to the system shall be within $100\% \pm 10\%$ of the system's rated hydraulic capacity.

NOTE – All dosing days, except those with dosing requirements less than the daily hydraulic capacity, shall be included in the 30-d average calculation.

8.3 Sample collection

8.3.1 Sampling frequency

Influent and effluent samples shall be collected three times per week during design loading periods and twice during each stress recovery period (the week following completion of each of the stress simulations described in 8.2.2.2). This schedule shall be continued in the event that testing is extended beyond the 26-wk minimum.

8.3.2 Collection methods

All sample collection shall be in accordance with *Standard Methods*, unless otherwise specified. Influent wastewater samples shall be flow-proportional, 24-h composites obtained during periods of system dosing. Effluent samples shall be flow-proportional, 24-h composites obtained during periods of system discharge. Effluent samples shall be representative of all treated effluent discharged from the system, as sampled from a central point of collection of all treated effluent. Grab samples shall be collected for pH, temperature, and dissolved oxygen (DO). The location of the grab sample shall be appropriate to provide a sample that is representative of the influent or effluent, and shall be determined in conjunction with the manufacturer. Grab samples shall be collected during the morning dosing period for gravity flow systems and during a time of discharge for systems that are pump discharged.

8.3.3 Analyses

The samples collected as described in 8.3.1 and 8.3.2 shall be analyzed as follows:

	Sample location							
Parameter	Sample type Raw influent Treated effluent Testing location							
BOD₅	24 h composite X Laboratory		Laboratory					
CBOD₅	24 h composite		Х	Laboratory				
Total suspended solids	24 h composite	Х	Х	Laboratory				
PH	Grab	Х	Х	Test site				
Temperature (°C)	Grab	Х	Х	Test site				
Dissolved oxygen	Grab		Х	Test site				
Alkalinity (as CaCO ₃)	24 h composite	Х	Х	Laboratory				
TKN (as N)	24 h composite	Х	Х	Laboratory				
Ammonia-N (as N)	24 h composite	Х	Х	Laboratory				
Nitrite/nitrate-N (as N)	24 h composite	Х	Х	Laboratory				

8.3.4 Analytical methods

The appropriate methods in *Standard Methods* shall be used to complete the analyses indicated in 8.3.3.

8.4 Criteria

8.4.1 Testing conditions

If conditions during the testing and evaluation period result in system upset, improper sampling, improper dosing, or influent characteristics outside the ranges specified in 8.2.1, an assessment shall be conducted to determine the extent to which these conditions adversely affected the performance of the system. Based on this assessment, specific data points may be excluded from the averages. Rationale for all data exclusions shall be documented in the final report.

8.4.2 Catastrophic site problems

In the event that a catastrophic site problem not described in the Standard including, but not limited to, influent characteristics, malfunctions of test site apparatus and acts of God, jeopardizes the validity of the performance testing, manufacturers shall be given the choice to:

- perform maintenance on the system, reinitiate system start-up procedures, and restart the performance testing; or

- with no routine maintenance performed, have the system brought back to pre-existing conditions and resume testing within 3 wks after the site problem has been identified and corrected. Data collected during the system recovery period shall be excluded from the effluent averages. NOTE – "Pre-existing conditions" shall be defined as the point when the results of 1 wk's worth of sampling are within 15% of the averages of the samples from the previous 3 wks of sampling.

8.4.3 Effluent quality

For purposes of determining system performance, only samples collected during design loading periods, described in 8.2.2, shall be used in the calculations. The data collected during the stress sequences shall not be included in the calculations, but shall be included in the final report.

8.4.3.1 CBOD5

The average CBOD5 of all effluent samples shall not exceed 25 mg/L.

8.4.3.2 TSS

The average TSS of all effluent samples shall not exceed 30 mg/L.

8.4.3.3 Total nitrogen

The average total nitrogen concentration of all effluent samples shall be less than 50% of the average total nitrogen concentration of all influent samples.

8.4.3.4 pH

The pH of individual effluent samples shall be between 6.0 and 9.0 SU.

8.5 Final report

A final report shall be prepared that presents the following:

- all data collected in accordance with the testing and evaluations within this Standard;

- a table indicating the actual percent reduction over the course of the test (included in the Executive Summary, as well as in the body, of the report);

- observations made during the testing;

- an estimation of the pounds of nitrogen loaded during the test and the pounds removed;
- any adjustments made to the alkalinity of the influent wastewater;
- a copy of the current edition of the Owner's Manual; and
- process description and detailed dimensioned drawings of the system evaluated.

A supplemental report shall be prepared for any system(s) approved under the performance classification section (1.4) of this Standard, including process description(s) and dimensioned drawings.

APPENDIX C

ANALYTICAL RESULTS

Plant Effluent

1

Weeks Into Test:

Weekend Dosing:	Sunday	460	gallons	Saturday	460	gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gallons)		460	460	460	460	460
Dissolved	aeration chamber	0.59	0.63	0.62	0.57	0.64
Oxygen (mg/L)	effluent	6.61	6.97	8.02	6.15	7.43
	influent	23	23	24	24	24
Temperature (C)	aeration chamber	21	22	22	23	22
	effluent	21	22	22	22	22
	influent	7.1	7.0	7.2	7.0	7.3
pН	aeration chamber	7.2	7.2	7.1	7.1	7.3
	effluent	7.2	7.2	7.1	7.1	7.2
Biochemical	influent (BOD ₅)	120	120	170	180	180
Oxygen Demand (mg/L)	effluent (CBOD ₅)	9	9	9	6	7
Suspended	influent	210	170	320	260	380
Solids (mg/L)	effluent	10	10	10	7	6

(a) Site problem

Notes:

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Week Beginning: <u>12-Apr-15</u> Plant Code: <u>Premier Tech Coco DN</u>

Weeks Into Test:

2

Weekend Dosing: Sunday <u>460</u> gallons Saturday <u>460</u> gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gallons)		460	460	460	460	460
Dissolved	aeration chamber	0.46	0.75	0.59	0.55	0.46
Oxygen (mg/L)	effluent	5.41	5.88	4.71	4.63	4.61
	influent	25	24	24	25	24
Temperature (C)	aeration chamber	23	23	22	23	23
	effluent	22	22	22	23	23
	influent	7.2	7.1	6.9	7.3	7.0
pH	aeration chamber	7.1	7.2	7.2	7.2	7.1
	effluent	7.1	7.2	7.2	7.1	7.0
Biochemical Oxygen Demand	influent (BOD ₅)	130	140	120	260	210
(mg/L)	effluent (CBOD ₅)	7	6	5	8	7
Suspended	influent	220	250	210	240	290
Solids (mg/L)	effluent	6	4	5	4	6

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

Plant Effluent

Week Beginning:	<u>19-Apr-15</u>	Plant Code: Premier Tech Coco DN
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3

Weeks Into Test:

Weekend Dosing:	Sunday	460	gallons	Saturday	460	gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gallons)		460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.49	0.57	0.36	0.48	0.40
Oxygen (mg/L)	effluent	4.81	6.29	4.87	4.57	5.07
	influent	22	25	25	24	25
Temperature (C)	aeration chamber	23	23	23	23	23
	effluent	22	22	23	23	23
	influent	6.8	7.2	7.0	6.9	7.2
рН	aeration chamber	7.2	7.2	7.1	7.2	7.1
	effluent	7.1	7.2	7.1	7.2	7.2
Biochemical Oxygen Demand	influent (BOD ₅)	120	180	190	150	120
(mg/L)	effluent (CBOD ₅)	3	3	3	2	1
Suspended	influent	210	290	450	320	150
Solids (mg/L)	effluent	4	3	3	2	2

(a) Site problem

Notes:

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent

 Week Beginning:
 <u>26-Apr-15</u>
 Plant Code:
 <u>Premier Tech Coco DN</u>

Weeks Into Test:

4

Weekend Dosing: Sunday <u>460</u> gallons Saturday <u>460</u> gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved	aeration chamber	0.45	0.55	0.59	0.69	0.57
Oxygen (mg/L)	effluent	5.31	5.17	6.89	6.32	6.60
	influent	25	24	23	24	22
Temperature (C)	aeration chamber	24	23	23	23	23
	effluent	24	23	23	23	22
	influent	7.1	6.8	7.1	7.2	7.0
pH	aeration chamber	7.0	7.0	7.2	7.1	7.1
	effluent	7.0	7.0	7.2	7.0	7.1
Biochemical Oxygen Demand	influent (BOD ₅)	130	190	110	99	130
(mg/L)	effluent (CBOD ₅)	4	4	2	2	2
Suspended	influent	160	120	150	130	180
Solids (mg/L)	effluent	6	2	1	2	2

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

Week Beginning:	<u>3-May-15</u>	Plant Code:	Premier Tech Coco DN
Weeks Into Test:	<u>5</u>		

Weeks Into Test:

Weekend Dosing:	Sunday	<u>460</u>	gallons	Saturday	<u>460</u>	gallons
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		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.57	0.49	0.58	0.65	0.57
	effluent	5.49	6.27	6.71	6.86	6.33
	influent	25	25	26	26	25
Temperature (C)	aeration chamber	24	24	25	25	24
	effluent	24	24	25	25	24
	influent	6.9	6.8	7.0	7.1	6.9
рН	aeration chamber	7.1	7.0	7.0	7.0	7.1
	effluent	7.1	7.0	7.0	7.0	7.0
Biochemical	influent (BOD ₅)	230	140	160	150	230
Oxygen Demand (mg/L)	effluent (CBOD ₅)	3	3	3	3	6
Suspended	influent	140	240	200	280	180
Solids (mg/L)	effluent	4	2	2	2	2

(a) Site problem

Notes:

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent

Week Beginning: <u>10-May-15</u> Plant Code: Premier Tech Coco DN

Weeks Into Test:

6

Weekend Dosing: gallons Sunday 460 gallons Saturday <u>460</u>

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.50	0.46	0.64	0.59	0.55
	effluent	5.27	5.52	6.40	7.27	6.12
	influent	26	26	26	25	26
Temperature (C)	aeration chamber	25	25	24	24	25
	effluent	25	24	24	24	24
	influent	7.0	6.9	7.1	6.8	7.0
pН	aeration chamber	7.1	7.0	7.1	7.0	7.0
	effluent	7.0	7.0	7.1	7.0	7.0
Biochemical	influent (BOD ₅)	120	220	200	140	140
Oxygen Demand (mg/L)	effluent (CBOD ₅)	2	2	4	3	3
Suspended	influent	140	160	170	160	170
Solids (mg/L)	effluent	2	2	2	3	2

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

 Plant Effluent

 Week Beginning:
 <u>17-May-15</u>

 Plant Code:
 <u>Premier Tech Coco DN</u>

Weeks Into Test:

7

Weekend Dosing:	Sunday	460	gallons	Saturday	460	gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	Dosed Volume (gallons)		460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.39	0.45	0.49	0.40	0.55
Oxygen (mg/L)	effluent	5.07	4.89	4.74	4.65	4.95
	influent	26	26	26	27	26
Temperature (C)	aeration chamber	25	25	25	26	25
	effluent	25	25	25	25	25
	influent	6.9	7.0	7.0	6.9	7.1
рН	aeration chamber	7.2	7.2	7.2	7.2	7.1
	effluent	7.0	7.1	7.1	7.1	7.1
Biochemical Oxygen Demand	influent (BOD ₅)	70	310	220	320	150
(mg/L)	effluent (CBOD ₅)	2	3	3	2	3
Suspended	influent	100	120	180	220	170
Solids (mg/L)	effluent	3	2	3	3	3

(a) Site problem

Notes:

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

 Week Beginning:
 24-May-15
 Plant Code:
 Premier Tech Coco DN

Weeks Into Test:

8

Weekend Dosing: Sunday <u>460</u> gallons Saturday <u>460</u> gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	d	0.49	0.45	0.69	0.54
	effluent	d	3.89	4.89	4.31	4.36
	influent	d	25	26	26	26
Temperature (C)	aeration chamber	d	25	25	25	25
	effluent	d	25	25	25	25
	influent	d	7.0	7.0	7.5	7.2
pН	aeration chamber	d	6.8	7.2	6.7	6.9
	effluent	d	6.7	7.1	6.6	6.8
Biochemical	influent (BOD ₅)	260	270	230	590	340
Oxygen Demand (mg/L)	effluent (CBOD ₅)	3	2	2	2	<3
Suspended	influent	120	100	100	88	120
Solids (mg/L)	effluent	2	2	2	1	2

(a) Site problem

Notes: No field readings on 5/25 due to the holiday.

(b) Malfunction of system under test

(c) Weather problem

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent Week Beginning: Plant Code: Premier Tech Coco DN 31-May-15

9 Weeks Into Test:

Weekend Dosing:	Sunday	<u>460</u>	gallons	Saturday	460	gallons
U	5		0	2		0

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.39	a	0.55	0.47	0.47
Oxygen (mg/L)	effluent	5.07	а	4.33	4.30	4.57
	influent	26	а	27	26	26
Temperature (C)	aeration chamber	26	a	27	27	26
	effluent	26	а	26	26	26
	influent	6.9	а	а	а	а
pН	aeration chamber	7.2	a	а	a	а
	effluent	7.0	а	а	а	а
Biochemical Oxygen Demand	influent (BOD ₅)	150	300	97	210	190
(mg/L)	effluent (CBOD ₅)	2	2	2	1	<3
Suspended	influent	260	170	170	180	190
Solids (mg/L)	effluent	<1	1	1	<1	<2

(a) Site problem (b) Malfunction of Notes: The pH meter failed on 6/2, resulting in loss of pH, temperature, and D.O. data on that day. pH measurements were not completed until the problem was resolved on 6/12.

system under test (c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent Week Beginning: Plant Code: Premier Tech Coco DN 7-Jun-15 10

Weeks Into Test:

Weekend Dosing: gallons Sunday 460 gallons Saturday <u>460</u>

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.57	0.46	0.41	0.48	0.75
	effluent	4.38	4.30	3.88	4.19	4.13
	influent	27	27	27	27	27
Temperature (C)	aeration chamber	28	28	28	28	28
	effluent	27	25	28	27	28
	influent	а	а	а	а	7.4
pН	aeration chamber	a	а	а	а	7.0
	effluent	а	а	а	а	6.9
Biochemical	influent (BOD ₅)	81	220	230	170	240
Oxygen Demand (mg/L)	effluent (CBOD ₅)	1	1	1	1	1
Suspended	influent	130	170	190	220	310
Solids (mg/L)	effluent	3	1	1	2	<1

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

Week Beginning:	<u>14-Jun-15</u>	Plant Code:	Premier Tech Coco DN
Weeks Into Test:	<u>11</u>		

 Weekend Dosing:
 Sunday
 460
 gallons
 Saturday
 460
 gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.83	1.28	1.09	1.01	1.05
	effluent	4.11	4.27	4.35	4.53	4.32
	influent	27	27	27	27	27
Temperature (C)	aeration chamber	28	28	28	27	28
(0)	effluent	28	28	28	28	28
	influent	7.5	7.4	7.5	7.6	7.5
pH	aeration chamber	7.0	7.0	7.0	7.0	7.0
	effluent	7.0	6.9	7.0	6.9	6.9
Biochemical	influent (BOD ₅)	160	200	150	210	170
Oxygen Demand (mg/L)	effluent (CBOD ₅)	3	1	2	1	1
Suspended	influent	350	230	260	170	120
Solids (mg/L)	effluent	2	1	<4	<1	2

(a) Site problem

Notes:

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent Week Beginning: 21-Jun-15 Plant Code: Premier Tech Coco DN Weeks Into Test: 12

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Weekend Dosing:Sunday $\underline{460}$ gallonsSaturday $\underline{460}$ gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	1.60	1.58	1.74	1.63	1.64
Oxygen (Ing/L)	effluent	5.36	5.45	5.27	5.18	5.32
	influent	27	27	27	28	28
Temperature (C)	aeration chamber	27	27	28	28	28
	effluent	28	28	28	28	28
	influent	7.6	7.3	7.5	7.4	7.5
pН	aeration chamber	7.1	7.0	7.0	7.0	7.0
	effluent	6.9	6.9	7.0	6.9	6.9
Biochemical Oxygen Demand	influent (BOD ₅)	200	180	210	250	260
(mg/L)	effluent (CBOD ₅)	2	2	1	2	2
Suspended	influent	120	140	110	180	300
Solids (mg/L)	effluent	2	2	1	2	2

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent Week Beginning: <u>28-Jun-15</u> Plant Code: <u>Premier Tech Coco DN</u>

Weeks Into Test: <u>13</u>

Weekend Dosing:	Sunday	460	gallons	Saturday	460	gallons
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		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	1.71	1.69	1.87	1.76	d
Oxygen (mg/L)	effluent	4.96	4.72	5.11	4.93	d
	influent	28	28	28	28	d
Temperature (C)	aeration chamber	28	28	29	28	d
	effluent	28	28	28	28	d
	influent	7.6	7.5	7.6	7.5	d
рН	aeration chamber	7.0	7.0	7.0	7.0	d
	effluent	7.0	6.9	7.0	7.0	d
Biochemical Oxygen Demand	influent (BOD ₅)	95	320	320	400	d
(mg/L)	effluent (CBOD ₅)	2	4	2	2	d
Suspended	influent	120	160	190	160	d
Solids (mg/L)	effluent	2	4	2	2	d

Notes: No samples on 7/3 due to the holiday.

(a) Site problem

(b) Malfunction of system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent

 Week Beginning:
 5-Jul-15
 Plant Code:
 Premier Tech Coco DN

Weeks Into Test: <u>14</u>

Weekend Dosing: Sunday <u>460</u> gallons Saturday <u>460</u>

gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved	aeration chamber	2.24	2.04	2.36	2.42	2.27
Oxygen (mg/L)	effluent	4.69	5.53	4.82	4.17	4.80
	influent	28	28	28	28	28
Temperature (C)	aeration chamber	29	29	29	29	29
	effluent	29	29	29	29	29
	influent	7.7	7.7	7.8	7.8	7.9
pH	aeration chamber	7.1	7.1	7.1	7.1	7.1
	effluent	7.1	7.1	7.1	7.1	7.1
Biochemical	influent (BOD ₅)	80	420	440	330	170
Oxygen Demand (mg/L)	effluent (CBOD ₅)	2	2	5	4	4
Suspended	influent	130	140	180	140	150
Solids (mg/L)	effluent	2	<1	5	3	3

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent Week Beginning: <u>12-Jul-15</u> Plant Code: <u>Premier Tech Coco DN</u>

Weeks Into Test: 15

Weekend Dosing:	Sunday	460	gallons	Saturday	460	gallons
e	2		0			0

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	d	1.29	1.43	1.39	1.37
Oxygen (mg/L)	effluent	d	4.68	2.91	2.73	3.44
	influent	d	29	29	29	29
Temperature (C)	aeration chamber	d	30	30	30	30
	effluent	d	30	30	30	30
	influent	d	7.5	7.5	7.3	7.4
pН	aeration chamber	d	7.1	7.2	7.0	7.1
	effluent	d	7.1	7.2	7.1	7.1
Biochemical Oxygen Demand	influent (BOD ₅)	130	340	390	380	210
(mg/L)	effluent (CBOD ₅)	а	а	8	8	11
Suspended	influent	200	190	200	200	190
Solids (mg/L)	effluent	а	а	4	7	4

(a) Site problem(b) Malfunction of

Notes: On site measurements not completed on 7/13 due to lab error. Effluent TSS and CBOD samples not collected on 7/13 and 7/14 due to a problem with the sampling system.

system under test (c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent

 Week Beginning:
 19-Jul-15
 Plant Code:
 Premier Tech Coco DN

Weeks Into Test: <u>16</u>

Weekend Dosing: Sunday gallons Saturday <u>460</u> gallons

		Monday	Tuesday	Wednesday	Thursday	Friday
Dosed Volume (gall	ons)	460	276	299	460	460
Dissolved	aeration chamber	1.27	1.57	1.87	2.15	2.03
Oxygen (mg/L)	effluent	3.32	3.41	3.29	3.38	3.35
	influent	29	31	30	31	32
Temperature (C)	aeration chamber	30	30	30	31	31
	effluent	30	30	30	31	30
	influent	7.2	7.4	7.2	7.3	7.3
pН	aeration chamber	7.0	7.1	7.2	7.1	7.1
	effluent	7.1	7.1	7.2	7.1	7.0
Biochemical	influent (BOD ₅)	350	260	a	290	300
Oxygen Demand (mg/L)	effluent (CBOD ₅)	3	5	a	3	3
Suspended	influent	190	490	а	300	150
Solids (mg/L)	effluent	2	3	а	4	2

(a) Site problem(b) Malfunction of

Notes: Evening dosing was missed on 7/21 and morning dosing was missed on 7/22 due to problems with the Waco test site dosing system.

(c) Weather problem

system under test

(d) Other

TSS, BOD, and CBOD samples were not collected on 7/22 due to the problems with the dosing system.

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: <u>26-Jul-15</u>

Weeks Into Test: <u>17</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (gal	lons)	460	460	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	-	1.69	1.56	1.59	1.65	1.57	1.61
Oxygen (mg/L)	effluent	-	2.98	2.81	1.89	1.72	1.96	2.27
	influent	-	30	30	30	30	30	30
Temperature (C)	aeration chamber	-	31	31	31	31	31	31
	effluent	-	31	30	30	30	30	30
	influent	-	7.4	7.7	7.6	7.4	7.4	7.5
рН	aeration chamber	-	7.1	7.2	7.2	7.2	7.1	7.1
	effluent	-	7.1	7.2	7.2	7.2	7.1	7.2
Biochemical Oxygen Demand	influent (BOD ₅)		160	170	180	230	160	
(mg/L)	effluent (CBOD ₅)		4	6	5	5	7	
Suspended	influent		160	130	140	150	140	
Solids (mg/L)	effluent		2	3	2	2	3	

(a) Site problem

Notes: Wash Day Stress 7/27 through 7/31. Additiional samples were collected on 7/28, 29, and 30 at the

request of the manufacturer.

(b) Malfunction of system under test

(c) Weather problem (d) Other NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

2-Aug-15 Plant Code: Premier Tech Coco DN

Weeks Into Test:

Week Beginning:

t: <u>18</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday
Dosed Volume (gal	lons)	460	460	460	460	460
Dissolved	aeration chamber	1.18	1.26	1.38	1.40	1.21
Oxygen (mg/L)	effluent	1.57	1.42	1.74	1.68	1.71
Temperature (C)	influent	31	30	30	30	31
	aeration chamber	31	31	31	31	31
	effluent	30	30	31	31	30
	influent	7.2	7.6	7.5	7.3	7.3
pH	aeration chamber	7.2	7.2	7.2	7.2	7.1
	effluent	7.2	7.2	7.2	7.2	7.2
Biochemical	influent (BOD ₅)		220	140	210	240
Oxygen Demand (mg/L)	effluent (CBOD ₅)		4	6	5	17
Suspended	influent		120	170	180	300
Solids (mg/L)	effluent		2	2	2	3

(a) Site problem

Notes: Working Parent Stress started on 8/8.

(b) Malfunction of

system under test (c) Weather problem

Plant Effluent

Plant Code: Premier Tech Coco DN

Week Beginning: 9-Aug-15

Weeks Into Test: 19

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (gall	lons)	460	460	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	1.29	1.01	0.47	0.52	0.38	0.37	0.40
Oxygen (mg/L)	effluent	2.48	1.97	2.13	2.59	2.82	1.72	1.83
	influent	31	32	32	32	32	31	32
Temperature (C)	aeration chamber	32	32	32	32	32	32	32
	effluent	32	31	32	33	33	33	32
	influent	7.3	7.5	7.4	7.4	7.5	7.4	7.4
рН	aeration chamber	7.1	7.2	7.2	7.2	7.2	7.2	7.2
	effluent	7.2	7.2	7.2	7.3	7.3	7.2	7.2
Biochemical Oxygen Demand	influent (BOD ₅)		76	160	200	180		74
(mg/L)	effluent (CBOD ₅)		3	3	4	5		2
Suspended	influent		110	150	120	160		120
Solids (mg/L)	effluent		1	<1	1	1		<1

(a) Site problem

Notes: Working Parent Stress completed on 8/12.

(b) Malfunction of

Additiional samples were collected on 8/11 and 8/13 at the request of the

manufacturer.

system under test (c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent Plant Code: Premier Tech Coco DN 16-Aug-15

Weeks Into Test:

Week Beginning:

<u>20</u>

Sunday Monday Tuesday Wednesday Thursday Dosed Volume (gallons) 460 460 460 460 180 aeration Dissolved 0.42 0.33 0.33 0.36 0.39 chamber Oxygen (mg/L) effluent 1.67 1.58 2.31 3.13 2.43 31 32 influent 31 31 31 Temperature aeration 32 32 32 32 32 (C) chamber effluent 32 32 32 32 32 7.3 7.4 7.6 7.5 7.5 influent aeration 7.2 7.2 7.1 7.1 pН 7.2 chamber 7.3 7.2 7.3 7.3 7.2 effluent influent Biochemical 76 120 170 180 210 (BOD_5) Oxygen Demand effluent (mg/L) 4 2 3 2 1 (CBOD₅) Suspended influent 95 61 140 99 130 Solids (mg/L) effluent 1 <1 1 2 1

(a) Site problem

Notes: Power/Equipment Failure Stress 8/20th through 8/22.

(b) Malfunction of

system under test (c) Weather problem

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: <u>23-Aug-15</u>

Weeks Into Test: 21

Sunday Monday Tuesday Wednesday Thursday Friday Saturday Dosed Volume (gallons) 460 460 460 460 460 460 460 aeration Dissolved 0.51 0.48 0.40 0.55 0.55 0.42 0.49 chamber Oxygen (mg/L) effluent 3.68 3.82 3.73 4.22 3.75 3.04 3.21 32 influent 31 31 31 31 31 31 Temperature aeration 30 31 30 30 30 30 31 (C) chamber effluent 31 31 31 31 30 30 31 influent 7.4 7.6 7.5 7.5 7.5 7.5 7.5 aeration 7.1 7.1 7.1 7.1 7.1 7.1 7.2 pН chamber 7.1 7.1 7.1 7.1 7.1 7.1 7.1 effluent influent Biochemical 69 260 370 220 (BOD_5) Oxygen Demand effluent (mg/L) 1 1 6 1 (CBOD₅) Suspended influent 77 73 46 42 Solids (mg/L) effluent <1 <1 3 <1

Notes: Odor:2 T.O.N were measured on Wed 8/26

(a) Site problem

(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

<u>30-Aug-15</u> Plant Code: <u>Premier Tech Coco DN</u>

Weeks Into Test: 22

Week Beginning:

<u>22</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday
Dosed Volume (gal	lons)	280	0	0	0	0
Dissolved	aeration chamber	0.49	0.47	0.38	0.26	0.42
Oxygen (mg/L)	effluent	3.21	-	-	-	-
	influent	31	-	-	-	-
Temperature (C)	aeration chamber	31	30	30	30	30
	effluent	31	-	-	-	-
	influent	7.3	-	-	-	-
рН	aeration chamber	7.1	7.1	7.2	7.2	7.3
	effluent	7.1	-	-	-	-
Biochemical	influent (BOD ₅)	<39				
Oxygen Demand (mg/L)	effluent (CBOD ₅)	1				
Suspended	influent	26				
Solids (mg/L)	effluent	<1				

(a) Site problem

Notes: Vacation Stress started on 8/30.

(b) Malfunction of system under test(c) Weather problem

(d) Other

a) Other

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: <u>6-Sep-15</u>

Weeks Into Test: 23

Sunday Monday Tuesday Wednesday Thursday Friday Saturday Dosed Volume (gallons) 0 0 276 460 460 460 460 aeration Dissolved 0.27 0.33 0.25 0.35 0.31 0.30 0.28 chamber Oxygen (mg/L) effluent 4.09 4.15 4.02 3.89 ---30 influent ---31 31 31 Temperature aeration 31 31 30 30 30 30 31 (C) chamber effluent 31 30 30 30 --influent -7.4 7.1 7.3 7.3 -aeration 7.4 7.0 7.1 7.2 7.5 7.0 7.2 pН chamber 7.0 7.0 7.0 7.1 effluent --influent Biochemical 240 240 (BOD_5) Oxygen Demand effluent (mg/L) 2 2 (CBOD₅) Suspended influent 160 190 Solids (mg/L) effluent 3 1

Notes: Vacation Stress completed on 9/8.

(a) Site problem(b) Malfunction of

system under test

(c) Weather problem

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Plant Code: Premier Tech Coco DN

Weeks Into Test: 24

13-Sep-15

Week Beginning:

		Sunday	Monday	Tuesday	Wednesday	Thursday
Dosed Volume (ga	llons)	460	460	460	460	460
Dissolved	aeration chamber	0.29	0.29	0.33	0.31	0.25
Oxygen (mg/L)	effluent	3.37	3.34	3.38	2.64	2.42
Temperature (C)	influent	30	30	30	30	30
	aeration chamber	30	29	29	30	30
	effluent	30	30	29	30	30
	influent	7.5	7.5	7.4	7.4	7.4
pH	aeration chamber	7.2	7.2	7.2	7.2	7.2
	effluent	7.1	7.1	7.1	7.1	7.2
Biochemical Oxygen Demand (mg/L)	influent (BOD ₅)	220	170	220	240	240
	effluent (CBOD ₅)	2	2	2	4	3
Suspended	influent	190	180	220	190	160
Solids (mg/L)	effluent	1	2	2	3	2

(a) Site problem

Notes: Odor:5 T.O.N were measured on Wed 9/16

(b) Malfunction of

system under test

(c) Weather problem

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: <u>20-Sep-15</u>

Weeks Into Test: 25

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (gal	lons)	460	560	460	560	460	560	460
Dissolved Oxygen (mg/L)	aeration chamber	0.32	0.29	0.31	0.27	0.30	0.33	0.30
Oxygen (mg/L)	effluent	1.92	1.89	1.97	1.91	1.48	1.94	1.85
	influent	30	30	30	30	29	29	29
Temperature (C)	aeration chamber	30	30	30	30	30	30	30
	effluent	30	30	30	30	30	30	30
	influent	7.4	7.4	7.4	7.4	7.3	7.3	7.3
pН	aeration chamber	7.2	7.2	7.2	7.2	7.2	7.2	7.2
	effluent	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Biochemical	influent (BOD ₅)		120		260		140	
Oxygen Demand (mg/L)	effluent (CBOD ₅)		3		7		8	
Suspended	influent		190		220		160	
Solids (mg/L)	effluent		1		2		3	

(a) Site problem(b) Malfunction of

Notes: The stress sequences were repeated, starting in week 25 because some of the required sampling was missed during the first set of stress sequences. Wash Day Stress 9/21 through 9/25.

system under test (c) Weather problem (d) Other Wash Day Stress 9/21 through 9/25. Wash loads were added on the wash days, without adjusting the normal dosing, due to lab error. This resulted in 100 extra gallons of dosing on 9/21, 23, and 25. NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Plant Code: Premier Tech Coco DN

Weeks Into Test: 26

Week Beginning:

g: <u>27-Sep-15</u> t: <u>26</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday
Dosed Volume (gal	460	460	460	460	460	
Dissolved	aeration chamber	0.41	0.35	0.36	0.41	0.34
Oxygen (mg/L)	effluent	2.05	2.05	3.14	2.11	2.03
	influent	29	29	29	30	30
Temperature (C)	aeration chamber	29	29	29	29	29
	effluent	30	29	29	29	29
	influent	7.1	7.3	7.6	7.4	7.3
pH	aeration chamber	7.1	7.1	7.2	7.2	7.2
	effluent	7.2	7.2	7.2	7.2	7.2
Biochemical	influent (BOD ₅)		140	210	230	180
Oxygen Demand (mg/L)	effluent (CBOD ₅)		4	4	3	3
Suspended	influent		120	160	210	180
Solids (mg/L)	effluent		2	1	2	3

(a) Site problem

(b) Malfunction of

system under test (c) Weather problem (d) Other Notes: Working Parent Stress didn't start on 10/3 as scheduled, due technical issue with the influent.

Odor:4 T.O.N were measured on Wed 9/30

Plant Effluent

Plant Cod Premier Tech Coco DN

Week Beginning: 4-Oct-15

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Plant Coc Premier Tech Coco DN

Weeks Into Test: 27 Weeks Into Test: 28

Week Beginning: 11-Oct-15

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (g	Dosed Volume (gallons)			460	460	460	460	460
Dissolved	aeration chamber	0.47	0.51	0.44	0.39	0.41	0.40	0.44
Oxygen (mg/L)	effluent	3.78	3.84	4.02	3.16	1.28	1.41	1.87
	influent	29	29	30	30	30	30	30
Temperature (C)	aeration chamber	28	28	28	28	28	28	28
	effluent	28	28	28	27	28	28	28
	influent	7.2	7.3	7.6	7.3	7.3	7.4	7.4
рН	aeration chamber	7.1	7.1	7.2	7.2	7.1	7.2	7.2
	effluent	7.2	7.1	7.1	7.2	7.2	7.2	7.2
Biochemical	influent (BOD ₅)	а	<39	160	220	250	180	210
(mg/L)	mg/L) (CBOD ₅)		1	1	1	7	7	4
Suspended	influent	а	31	270	470	190	230	280
Solids (mg/L)	effluent	а	<1	1	<1	6	4	2

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (g	460	460	460	276	184	460	460	
Dissolved	aeration chamber	0.41	0.38	0.73	0.33	0.38	0.42	0.39
Oxygen (mg/L)	effluent	1.94	2.02	2.45	2.21	1.16	1.78	1.95
	influent	30	30	30	30	30	30	30
Temperature (C)	aeration chamber	29	29	29	28	28	28	28
. ,	effluent	28	28	28	28	25	27	27
	influent	7.3	7.3	7.3	7.3	7.4	7.3	7.3
рН	aeration chamber	7.1	7.2	7.2	7.0	7.1	7.1	7.1
	effluent	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Biochemical	influent (BOD ₅)			270	200	160	180	130
Oxygen Demand (mg/L)	effluent (CBOD ₅)			6	6	b	b	13
Suspended	influent			600	300	220	200	200
Solids (mg/L)	effluent			2	3	b	b	6

(a) Site problem (b) Malfunction of

Notes: No sampling on 10/4 because there was site technical issue with the influent on 10/3 and 4, which was resolved on 10/5.

Working Parent Stress 10/6 through 10/10.

(c) Weather problem (d) Other

system under test

Additional sampling during the stress was at the request of the manufacturi (c) Weather problem

(b) Malfunction of system under test Notes: The septic tank effluent filter, which is part of the system under test, was found clogged on 10/13, resulting in the treatment system backing up. Dosing was suspended following the mid-day dosing on 10/14. The effluent filter was serviced according to the manufacturer's instructions on 10/15 after allowing the water level in the septic tank to return to normal. Dosing resumed just prior to evening dosing on 10/15. This meant full 24-hour effluent samples could not be collected on 10/15 and 10/16. Color: 40 Pt-Co units

10/14 measurements:

Odor: 10 T.O.N Oily film and foam: Not detected

(a) Site problem

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: <u>18-Oct-15</u>

Weeks Into Test: 29

Sundav Monday Tuesday Wednesday Thursday Friday Saturday Dosed Volume (gallons) 460 0 280 460 460 460 460 aeration Dissolved 0.42 0.50 0.41 0.41 0.36 0.38 0.33 chamber Oxygen (mg/L) 0.54 3.35 3.42 3.67 effluent 2.13 -influent 29 --29 29 29 28 Temperature aeration 28 28 28 28 27 28 27 (C) chamber effluent 27 24 27 28 28 -influent 7.4 7.3 7.3 7.3 7.3 -aeration 7.2 7.3 7.2 7.2 7.2 7.3 pН 7.2 chamber 7.1 7.1 7.1 7.0 7.1 effluent -influent Biochemical 120 200 (BOD_5) Oxygen Demand effluent (mg/L) 8 4 (CBOD₅) Suspended influent 200 340 Solids (mg/L) effluent 4 4

Notes: Power/Equipment Failure Stress 10/18 through 10/20.

(a) Site problem

(b) Malfunction of

system under test (c) Weather problem

(c) weather pro

(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Plant Code: Premier Tech Coco DN

Weeks Into Test: <u>30</u>

25-Oct-15

effluent

Week Beginning:

Sunday Monday Tuesday Wednesday Thursday Dosed Volume (gallons) 460 460 460 280 0 aeration Dissolved 0.38 0.46 1.50 0.53 0.60 chamber Oxygen (mg/L) effluent 3.84 4.17 4.22 1.06 influent 27 27 27 27 -Temperature aeration 25 25 25 25 25 (C) chamber effluent 25 25 25 25 influent 7.3 7.3 7.5 7.4 aeration 7.2 7.4 7.1 7.2 pН 6.9 chamber 7.0 7.3 7.1 effluent 7.0 influent Biochemical 160 91 210 130 (BOD_5) Oxygen Demand effluent (mg/L) 2 1 2 2 (CBOD₅) Suspended influent 230 160 210 220

(a) Site problem

Solids (mg/L)

Notes: Vacation Stress started on 10/28.

2

2

2

2

(b) Malfunction of

system under test (c) Weather problem

Plant Code: Premier Tech Coco DN

Plant Effluent

Week Beginning: 1-Nov-15

Weeks Into Test: <u>31</u>

Sunday Monday Tuesday Wednesday Thursday Friday Saturday Dosed Volume (gallons) 275 460 0 0 0 0 0 aeration Dissolved 0.64 0.62 0.58 0.45 0.61 0.47 0.63 chamber Oxygen (mg/L) 2.17 effluent -----influent ----27 --Temperature aeration 23 23 23 23 23 23 23 (C) chamber effluent 20 -----influent 7.4 --_ --aeration pН 7.1 7.2 7.2 7.2 7.2 7.2 7.2 chamber 7.2 effluent influent Biochemical (BOD_5) Oxygen Demand effluent (mg/L) (CBOD₅) Suspended influent Solids (mg/L) effluent

(a) Site problem

Notes: Vacation Stress completed on 11/6.

(b) Malfunction of

system under test

(c) Weather problem (d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems Plant Effluent

Week Beginning: 8-Nov-15

Plant Code: Premier Tech Coco DN

Weeks Into Test: 32

Sunday Monday Tuesday Wednesday Thursday Dosed Volume (gallons) 460 460 460 460 460 aeration Dissolved 0.58 0.54 0.61 0.46 0.39 chamber Oxygen (mg/L) 3.89 5.62 5.72 5.47 5.12 effluent influent 26 26 26 26 26 Temperature aeration 23 23 23 23 23 (C) chamber 22 23 effluent 21 22 23 7.3 7.3 7.4 7.3 7.4 influent aeration pН 7.2 7.2 7.1 7.2 7.2 chamber 7.2 7.1 7.1 7.1 7.1 effluent influent Biochemical 79 270 400 63 (BOD_5) Oxygen Demand effluent (mg/L) 2 2 1 1 (CBOD₅) Suspended influent 89 110 140 190 Solids (mg/L) effluent 2 4

(a) Site problem

(b) Malfunction of system under test (c) Weather problem (d) Other

Notes: 11/11 measurements:

Color: 20 Pt-Co units Odor 10 T.O.N Oily film and foam: Not detected

Plant Effluent

Plant Code: Premier Tech Coco DN

Week Beginning: <u>15-Nov-15</u>

Weeks Into Test: <u>33</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
Dosed Volume (gal	lons)	460	460	460	460	460	460	460
Dissolved Oxygen (mg/L)	aeration chamber	0.39	0.44	0.58	0.52	0.57	0.66	
Oxygen (mg/L)	effluent	4.19	4.01	3.91	3.80	5.55	5.78	
	influent	26	26	27	31	28	28	
Temperature (C)	aeration chamber	23	23	22	22	22	22	
	effluent	22	23	22	22	22	22	
	influent	7.4	7.6	7.4	7.1	7.2	7.3	
pН	aeration chamber	7.2	7.2	7.2	7.2	7.2	7.1	
	effluent	7.1	7.1	7.1	7.1	7.2	7.1	
Biochemical Oxygen Demand	influent (BOD ₅)	100	240	240	190	270	230	
(mg/L)	effluent (CBOD ₅)	2	2	2	3	2	1	
Suspended	influent	130	160	190	65	110	110	
Solids (mg/L)	effluent	2	2	1	2	2	2	

(a) Site problem

(b) Malfunction of

system under test

Notes: D.O., temperature, and pH data collected from 11/14 through 11/20 was measured with an instrument that was past it's calibration due date. However, the instrument was calibrated in house for both pH and DO each day before collecting data.

(c) Weather problem(d) Other

NSF International Standard 40 - Residential Wastewater Treatment Systems

Plant Effluent

Week Beginning: <u>22-Nov-15</u> Plant Code: <u>Premier Tech Coco DN</u>

Weeks Into Test: 34

<u>34</u>

		Sunday	Monday	Tuesday	Wednesday	Thursday
Dosed Volume (gal	lons)	460	460	460	460	460
Dissolved	aeration chamber		1.41	1.83	2.21	2.17
Oxygen (mg/L)	effluent		4.99	5.23	5.44	5.37
	influent		23	23	24	25
Temperature (C)	aeration chamber		20	21	21	21
	effluent		20	20	21	21
	influent		7.5	7.6	7.7	7.5
pH	aeration chamber		7.2	7.2	7.1	7.1
	effluent		7.1	7.1	7.2	7.1
Biochemical	influent (BOD ₅)		300	300	380	240
Oxygen Demand (mg/L)	effluent (CBOD ₅)		1	3	3	2
Suspended	influent		150	230	180	180
Solids (mg/L)	effluent		1	1	3	3

(a) Site problem Notes:

(b) Malfunction of

system under test

(c) Weather problem

APPENDIX D

ANALYTICAL RESULTS – Nitrogen Analyses

	Date	Ammonia (mg			Kjeldahl n (mg/L)		e/Nitrite g/L)	Total Nitro	gen (mg/L)		lkalinity CaCO3)	Days	Daily TN Reduction	Avg TN Reduction
		Influent	Effluent	Influent	Effluent	Influent	Effluent	Influent	Effluent	Influent	Effluent			'
1	04/06/15	24.4	25	35.2	18.9	0.06	0.3	35.3	19.2	320	340	1	45.55	
Week	04/08/15	31.0	24.2	47.7	28.1	0.52	0.46	48.2	28.6	350	340	2	40.77	
Ŵ	04/10/15	26.8	24.7	62.9	31.1	0.14	0.55	63.0	31.7	350	350	3	49.79	
2	04/13/15	26.7	24.7	41.9	28.8	0.12	0.61	42.0	29.4	350	350	4	30.01	
Week	04/15/15	27.6	26.5	42.7	31.1	0.07	0.84	42.8	31.9		330	5	25.32	
M	04/17/15	31.1	28.9	45.4	30.4	0.26	1.05	45.7	31.5	360	350	6	31.12	
: 3	04/20/15	24.1	27.9	36.5	29.8	0.12	2.42	36.6	32.2	320	340	7	12.02	
Week (04/22/15	26.3	24.5	40.6	22.1	0.1	3.02	40.7	25.1		320	8	38.28	
M	04/24/15	16.0	20.2	25.0	20.6	0.93	3.79	25.9	24.4	290	310	9	5.94	
4	04/27/15	18.3	8.2	27.9	8.7	0.46	6.4	28.4	15.1	300	260	10	46.76	
Week	04/29/15	16.7	4.7	26.0	6.1	1.0	5.0	27.0	11.1		240	11	58.89	
M	05/01/15	15.3	1.6	28.3	2.6	0.05	6.9	28.4	9.5	310	220	12	66.49	
: 5	05/04/15	20.9	1.3	31.0	2.5	0.05	4.2	31.1	6.7	300	240	13	78.42	
Week :	05/06/15	22.4	1.0	31.8	2.3	0.08	5.2	31.9	7.5		240	14	76.47	
M	05/08/15	30.6	3.2	37.4	4.5	0.1	4.1	37.5	8.6	350	260	15	77.07	
6	05/11/15	21.9	6.9	29.9	8.6	1.4	5.2	31.3	13.8	290	260	16	55.91	
Week	05/13/15	25.1	4.7	34.6	6.6	0.05	5.8	34.7	12.4		250	17	64.21	
Μ	05/15/15	22.7	4.2	28.4	5.6	0.05	4.9	28.5	10.5	300	240	18	63.09	
7	05/18/15	16.2	6.5	23.6	8.0	0.93	4.5	24.5	12.5	280	260	19	49.04	
Week '	05/20/15	31.0	8.2	35.4	9.7	0.1	3.1	35.5	12.8		270	20	63.94	
M	05/22/15	25.0	12.1	37.8	13.0	0.07	4.6	37.9	17.6	340	280	21	53.53	
8	05/25/15	38.3	13.7	76.9	17.3	0.54	4.3	77.4	21.6	400	300	22	72.11	
Week	05/27/15	32.5	14.2	39.3	16.5	0.05	6.9	39.4	23.4		280	23	40.53	
W	05/29/15	33.8	11.8	45.7	11.1	1.4	5.5	47.1	16.6	360	290	24	64.76	
6	06/01/15	20.6	10.9	31.9	11.7	0.05	5	32.0	16.7	320	280	24	47.73	
Week 9	06/03/15	19.4	9.0	26.6	9.3	0.05	5.6	26.7	14.9	310	260	26	44.09	
	06/05/15	22.2	6.8	38.0	8.2	0.05	5.0	38.1	13.2	320	240	27	65.31	
10	06/08/15	13.8	5.8	22.7	7.2	0.18	4.7	22.9	11.9	270	260	28	47.99	50.93
Week 10	06/10/15	29.9	8.0	44.0	9.7	0.09	4	44.1	13.7	320	260	29	68.93	
We	06/12/15	26.6	10.2	41.4	11.5	0.06	5.3	41.5	16.8	310	250	30	59.48	
Week 11	06/15/15	16.1	7.7	32.0	9.2	0.12	3.7	32.1	12.9	260	240	31	59.84	
eek	06/17/15	16.1	8.3	27.7	11	0.22	4.1	27.9	15.1	230	240	32	45.92	
W	06/19/15	23.3	7.1	30.2	8.6	0.05	5.5	30.3	14.1	310	230	33	53.39	
12	06/22/15	23.9	8.5	35.5	9.5	0.06	4.9	35.6	14.4	310	260	34	59.51	52.27
Week 12	06/24/15	29.7	10.2	40.8	9.0	0.06	4.7	40.9	13.7	330	250	35	66.47	
	06/26/15	22.1	11.4	35.6	10.8	0.08	4.3	35.7	15.1	320	280	36	57.68	
13	06/29/15	12.6	10.3	20.7	10.3	0.23	3.6	20.9	13.9	290	270	37	33.59	
Week 13	07/01/15	36.5	12.3	48.1	13.6	0.07	2.5	48.2	16.1	390	280	38	66.58	
Wέ	07/02/15	42.9	14.6	58.1	17.0	0.07	2.5	58.2	19.5	420	310	39	66.48	
14	07/06/15	20.4	12.7	27	12.6	0.05	4.6	27.1	17.2	300	270	40	36.41	53.26
Week 14	07/08/15	39.8	16.5	61.4	19.5	0.09	2.2	61.5	21.7	360	290	41	64.71	
Wé	07/10/15	28.0	22.4	45.9	29.7	0.06	2.45	46.0	32.2	360	320	42	30.05	

5	07/13/15					-								1
Week 15	07/15/15	36.5	20.6	59.2	24.4	0.08	2	59.3	26.4	390	330	43	55.47	
Vee	07/17/15	44.3	26.4	61.8	33.3	0.08	0.5	<u> </u>	33.8	390	360	43	45.36	
	07/20/15	40.0	28.5	62.3	31.8	0.06	1.2	62.4	33.0	390	360	44	43.36	
k 1	07/22/15	40.0	20.5	02.5	51.0	0.00	1.2	02.4	33.0	570	500	-Т.	47.00	
Week 16	07/24/15	30.7	28.9	56.2	33.1	0.16	1.3	56.4	34.4	380	360	46	38.96	52.22
-	08/03/15	50.1	20.9	50.2	55.1	0.10	1.5	50.4	J.T.T	500	500	10	50.70	
	08/05/15													
	08/07/15													Extra
	08/10/15		24.5		26.7		0.19	0	26.9			2		samples
	08/11/15	19.8	22.8	30.6	26.5	2.07	0.83	25.5	29.4			24		per
<mark>23</mark>	08/12/15	18.7	20.7	33.6	26.9	1.6	0.44	24.1	26.7			for		Premier
1 <mark>7-</mark>	08/13/15	16.6	20.2	26.1	21.7	0.62	0.48	21.4	26.0			ıse		Tech
week 17-	08/15/15											Stress data, do not use for 245		
wei	08/17/15											ů o		
	08/19/15											, de		
	08/26/15											ata		
	08/28/15											b s		
	08/29/15											res		
_	09/11/15											St		
<mark>week 24</mark>	09/14/15	35.1	15.9	47.3	16.4	0.13	5.98	47.4	22.4	350	280			
sek	09/16/15	39.5	20	55.6	20.8	0.25	4.79	55.9	25.6	310	290			
	09/18/15	31.5	25.5	47.4	24.4	0.09	3.58	47.5	28.0	340	300			
Week 26 Week 25	09/21/15	32.7	25.8	45.6	24.2	0.12	3.27	45.7	27.5	320	310		39.92	wash day
eek	09/23/15	44.6	26.7	63.9	29.7	0.11	1.87	64.0	31.6	370	330		50.68	stress
M	09/25/15	25.8	29.6	43.4	35.3	0.09	0.87	43.5	36.2	330	370		16.83	311033
t 2(09/28/15	23.2	33.5	38.9	34.3	1.69	1.32	40.6	35.6	290	340		12.24	
eel	09/30/15	31.3	25.6	50.6	26.9	0.2	2.01	50.8	28.9	310	320		43.09	
	10/02/15	30.3	27.4	46.0	31.5	0.23	2.21	46.2	33.7	350	320		27.08	
Week 27	10/06/15	28.1	11.1	44.3	13.1	0.27	4.73	44.6	17.8	280	230		60.00	working
eel	10/07/15	29.0	13.0	52.0	16.5	0.11	3.46	52.1	20.0	300	240	245	61.70	parent
	10/09/15	29.9	20.5	47.9	25.5	0.16	1.38	48.1	26.9	310	300	do not use for 245	44.07	stress
Week 28	10/13/15	36.9	25.6	76.2	25.7	0.13	0.68	76.3	26.4	350	320	e f	65.44	
eek	10/14/15	23.3	27.7	46.7	33.3	0.14	0.64	46.8	33.9	300	320	t us	27.54	
	10/17/15	31.4	29.2	49.5	34.3	0.11	2.61	49.6	36.9	340	330	not	25.60	
Week 29	10/18/15	32.9	29.2	40.6	30	0.5	3.03	41.1	33.0	330	320	do	19.64	power
/ee]	10/21/15	20.0	16.6	22.6	10.0	0.14	5.21			2(0	2(0	ta,		failure
M (10/24/15	20.0	16.6	33.6	18.0	0.14	5.31	33.7	23.3	260	260	Stress data,	30.91	stress
Week30	10/26/15	7.4	6.0	18.5	7.3	2.03	9.56	20.5	16.9	200	190	SSS	17.88	
Vee	10/27/15 10/28/15	19.6	5.3 4.95	30.7 34.8	10.7 8.3	2.31	5.88 4.93	33.0	16.6	280 260	200 200	Stre	49.77	
		17.6	4.95	34.8	8.3	0.8	4.93	35.6	13.2	200	200		62.84	
ek 3	11/02/15 11/04/15													vacation
Week 2	11/04/15													
\geq	11/00/13									<u> </u>				stress

32	11/09/15	10.7	0.52	24.3	0.94	1.79	12.7	26.1	13.6	260	190		47.72
eek	11/11/15	21.8	0.08	46.3	0.93	0.05	10.4	46.4	11.3	320	190		75.56
We	11/13/15	31.6	7.3	47.6	8.88	0.44	4.07	48.0	13.0	340	250		73.04
33	11/16/15	28.8	4.4	46.7	6.45	0.57	8.54	47.3	15.0	320	240	47	68.29
eek	11/18/15	24.7	7.2	35	11.7	0.32	4.21	35.3	15.9	280	250	48	54.95
We	11/20/15	18.4	6.6	52.1	26.6	0.94	5.02	53.0	31.6	300	250	49	40.38
34	11/23/15	14.1	2.15	57.1	21.5	0.73	6.81	57.8	28.3	310	220	50	51.05
eek	11/25/15	30.3	7.6	47.2	9.0	0.92	1.91	48.1	10.9	320	250	51	77.33
W,	11/27/15	11.7	13.8	25.8	17.1	1.3	2.34	27.1	19.4	260	270	52	28.27
35	11/30/15	7.1	2.0	26.1	2.3	2.2	6.92	28.3	9.2	240	190	53	67.42
eek	12/02/15	15.5	4.9	41.9	6.1	0.54	1.77	42.4	7.9	330	230	54	81.46
W,	12/04/15	27.6	13.8	54.4	12.6	0.9	0.41	55.3	13.0	330	270	55	76.47
												_	
	Median	24.7	10.2	37.8	11.5	0.1	4.2	37.9	15.91	320	270		
	Min	7.1	1.0	20.7	2.3	0.1	0.3	20.9	6.7	230	190		
	Max	44.3	28.9	76.9	33.3	2.2	8.5	77.4	34.4	420	360		

3.8

2.0

40.4

12.6

18.6

8.1

324

42

278

42

Avg Std Dev 25.1

8.3

12.5

8.4

40.1

12.6

14.9

9.3

0.3

0.5

53.89

APPENDIX E

OWNERS MANUAL



Residential Owner's Manual - USA

Congratulations on your purchase of an Ecoflo[®] Coco Filter- ECDn unit from Premier Tech Aqua (PTA). The Ecoflo[®] Coco Filter-ECDn unit has been tested and listed under NSF standard 40 & 245 and meets requirements for Class I systems.

With the Ecoflo® Coco Filter –ECDn unit, you have wisely chosen to protect your health as well as the environment. This manual contains information on the operation, operating guidelines, maintenance and warranties of the Ecoflo® Coco Filter-ECDn unit. For additional information, contact our customer service at 1 800 632-6356 or visit our website at PREMIERTECHAQUA.COM.

Operating Principle

Onsite wastewater treatment systems must respect applicable local rules and regulations. These systems are specifically designed to treat residential wastewater to such a level that treated effluent can be safely returned to the environment. Typically, an onsite wastewater treatment system is composed of 2 to 3 main treatment steps depending on site constraints prior to final dispersal of treated effluent: primary treatment, treatment system and if required polishing unit.

Primary treatment

The Primary tank is the first element of this nitrification-denitrification system. The primary tank's main functions are to accomplish a primary treatment which is to retain solids and let only a clarified effluent enter further treatment (Ecoflo[®] Coco Filter-ECDn unit) as well as to offer an anoxic zone in order to promote total nitrogen removal (denitrification).

Treatment system

The wastewater first goes into the Primary tank through an inlet device (tee or baffle) that directs it into the tank. The Primary tank promotes total nitrogen removal (denitrification) under anoxic conditions by creating a rapid mix of the recirculated treated effluent (from the Ecoflo[®] Coco Filter- ECDn unit) with the organic content of the raw wastewater. The recirculation line between the Ecoflo[®] Coco Filter- ECDn unit and the Primary tank is simply connected to the inlet pipe of the Primary tank. From the Primary tank, the pretreated wastewater flows by gravity to the Ecoflo[®] Coco Filter- ECDn unit by first passing into an effluent filter that promotes scum and solids retention in the tank.

Once the wastewater reaches the Ecoflo[®] Coco Filter- ECDn unit, a tipping bucket equally disperses the wastewater on specially designed plates which evenly distribute the wastewater on top of the filtering media. The wastewater then trickles through the natural fibrous filtering media.

The dosing control unit used in this system controls the pump located in the bottom of the biofilter that feeds a flow divider (PFS-200DN). This control unit consists of a "simplex" control panel allowing management of the dosing pump's cycles (start and pause). The controller totalizes and keeps records of the different pump's operating times either in normal cycle or in critical high level situations. It also totalizes and records the number of critical high level events occurred on the system since its installation. This last count allows validation of the dosing cycle used (operation time and pause time).

The flow divider (PFS-200DN) allow a fraction of the treated wastewater to return to the Primary/anoxic tank via the pump located in the bottom of the biofilter and the remaining fraction is directed toward the dispersal/disposal mean in accordance to local regulations. The recirculation ratio is approximately two (2) times the daily flow (2Q).

The Ecoflo[®] Coco Filter-ECDn unit's operating principle allows the system to be used continuously or intermittently without requiring any special precaution or having any impact on the quality of the treatment. No specific action from the owner is required to start the system.

The model and the number of Ecoflo® Coco Filter-ECDn unit are determined by the domestic wastewater flow per

day. Other factors such as the available space, the topography of the lot, as well as the type, permeability and depth of the natural occurring soils could influence model selection.

Ecoflo® Coco Filter ECDn unit Models

There are many different models of Ecoflo[®] Coco Filter-ECDn unit and each model has different characteristics. The letters and numbers associated with the Ecoflo[®] Coco Filter-ECDn unit specify the model's characteristics, as presented in the following table with model **ECDn-500-P** (**PACK**) as reference:

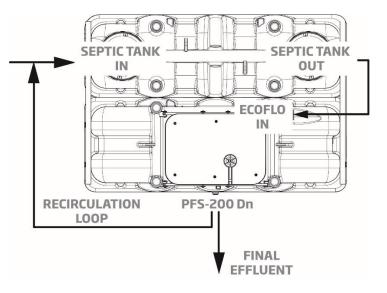
EC refers to the Ecoflo [®] model	EC = Ecoflo [®] Coco Filter
Dn	Nitrogen removal product with a maximum applicable HLR 575 L/m ² -d, i.e. of 14.1 gal /ft ²
500 refers to the daily flow capacity	500 = Capacity of 500 US gallons per day
	600 = Capacity of 600 US gallons per day
	865 = Capacity of 850 US gallons per day
	1000 = Capacity of 1000 US gallons per day
	1100 = Capacity of 1100 US gallons per day
P refers to the material of the shell	C = Concrete
	P = Plastic (Polyethylene)
PACK refers to configuration of the primary tank	PACK = monobloc configuration, both tight together
and biofilter	No mention = In line

Therefore, according to this nomenclature, the **ECDn-500-P (PACK)** model refers to an Ecoflo Coco Filter, Nitrogen removal version, with a daily flow capacity of 500 US gallons, in a polyethylene shell. Both primary tank and biofilter come in a monobloc configuration (pack).

For models that doesn't come into PACK configuration, the recommended Hydraulic Retention Time of the primary/septic tank is a minimum of two days at the design daily flow.

Installation Diagrams

NOTE: The installation diagrams below show the Ecoflo[®] Coco Filter-ECDn unit with polyethylene shell-PACK configuration.



Operating Guidelines

Type of wastewater that can be treated by an Ecoflo® Coco Filter-ECDn unit:

Domestic wastewater (for example: wastewater from isolated dwellings).

It is NOT RECOMMENDED to discharge any of the following substances into the septic system:

- Oil and grease (motor oil, cooking oil, etc.);
- Wax and resins;
- Paints and solvents;
- Any kind of petroleum product;
- Any kind of pesticide;
- Any kind of primary tank additive;
- Any kind of toxic substance;
- Anything not easily biodegradable (for example, coffee beans, cigarette butts, sanitary napkins, tampons, condoms, cotton swab, etc.).



AND

- NEVER open or go inside the primary tank or the Ecoflo® Coco Filter-ECDn unit.
- Keep all lids of the septic system accessible at all times. NEVER cover them with mulch, dirt or any permanent structure (patio, swing, shed, etc.).
- Make sure all lids of the septic system are at least 50 mm (2") above the surface of the landscaped lot.
- NEVER install a riser on polyethylene Ecoflo® Coco Filter ECDn-865, 1000 and 1100-P models.
- NEVER install more than one (1) 6 inch riser on a polyethylene Ecoflo® Coco Filter-ECDn-500 and 600 models.
- NEVER install more than ONE (1) 8 inch RISER on a concrete Ecoflo[®] Coco Filter-ECDn unit main access. Use only PTA products.
- NEVER plant trees within 6 m (20') of the Ecoflo[®] Coco Filter-ECDn unit lid and within 2 m (6' 6") of the absorption bed.
- NEVER connect a drain pipe, roof gutter, sump pump or air conditioner drain to the septic system.
- NEVER discharge content or water from a water softener backwash, a spa or pool in your septic system.
- NEVER discharge wastewater from a recreation vehicle (camping trailer, caravan, etc.) into any of the components of your septic system.
- NEVER use automatic toilet bowl cleaners.
- DO NOT let anything accumulate on top of the septic system (for example, blown snow, backfill, landscaping, rocks, etc.) less than 5 m (16' 5") of your septic system's lid.
- Maintain a minimal distance of 6 m (20') between the bottom of a slope, an embankment or a retaining wall and the lids of your septic installation.

By respecting these guidelines, you contribute to the proper operation of your septic system and help prolong the life of your Ecoflo[®] Coco Filter-ECDn unit filtering media. Failure to abide by these guidelines may, at Premier Tech Aqua's discretion, render the warranty invalid.

Owner's responsibility

The owner must respect all existing laws and regulations regarding the system's effluent quality and its discharge into the environment. The owner of the wastewater treatment system is responsible for its installation, operation and maintenance.

The system's warranty begins upon purchase. Should the start-up be delayed, it is the customer's responsibility to inform Premier Tech Aqua about it so the first maintenance, which is included in the purchase price, is postponed. If the first maintenance has been performed prior to the client's call, Premier Tech Aqua reserves the right to decide whether another maintenance, free of charge or not, will be carried out the following year. No request for delayed

start-up will be accepted any later than one (1) year after the purchase date without it affecting the product's warranties.

Keep heavy objects off your septic system

Never drive a vehicle or place objects weighing more than 225 kg (500 lb) within 5 m (16' 5") of the lid of your Ecoflo[®] Coco Filter-ECDn unit. If you are planning any kind of landscaping or any other type of work on the property (i.e.: snow removal, lawn mowing, excavation, etc.), **make sure you advise all those involved,** so they do not damage your septic system. It is recommended to note where of your septic system elements are located.

About your home

Your home must be equipped with an air vent that is in proper working order and all plumbing must comply with the applicable standards of the building code in your location. Every septic tank must be ventilated by an air duct with a diameter of at least 100 mm (4") or be connected to the air vent of the isolated dwelling being served. Premier Tech Aqua strongly recommends using a pipe with a diameter of 100 mm (4") for the air vent.

Any change in the use of your home or any modification to your Ecoflo[®] Coco Filter-ECDn unit must be authorized by the local authorities, and Premier Tech Aqua must be advised. If this requirement is not fully met, the warranty for your Ecoflo[®] Coco Filter-ECDn unit will be null and void.

Maintenance

Primary tank

Empty your primary/septic tank every two to four years or if the level of sludge measured exceed the 2/3 of the total height of water in the tank. This helps to keep your septic system in proper working order. Every primary/septic tank and effluent filter shall be inspected and maintained as prescribed by local regulations.

If your home is equipped with a garbage disposal or a sewage pump, we strongly recommend emptying your primary/septic tank more frequently than the frequency noted above. Using this kind of equipment increases the amount of sludge in the primary/septic tank.

To have complete records of the maintenance performed on your septic system, we recommend that you to keep the proof of maintenance (invoice) with this Owner's Manual.

IMPORTANT: Primary tanks can be emptied in several ways that can be classified into two categories: **complete emptying and selective emptying.** Complete emptying, the most common, consists of completely pumping the contents of the primary tank. It's easy to check if the work was properly done because the primary tank will be completely empty when the vacuum truck leaves the site. Selective emptying is divided into two sub-categories: with a filter (or recycled) or without a filter. The method with a filter requires a truck that has been adapted for this type of emptying, that is, one that separates and retains the solids from the wastewater. The mechanically clarified water is then returned to the primary tank. The selective method without a filter allows the solids to settle while in the truck before the water is returned to the primary tank. As such, in an effort to ensure the Ecoflo[®] Coco Filter-ECDn unit continues to perform optimally, **it is very important that you ensure than the water that is returned to the primary tank has been properly clarified and does not contain or contains very few suspended solids**. We also recommend you to call one of the members of PTA's local partners. He will assist and verify if the work is done according to your specific needs to best protect your Ecoflo[®] Coco Filter-ECDn unit system.

Effluent filter

Under normal operating conditions, as described in this manual, an effluent filter that complies with local regulations should operate efficiently for many years. It must be cleaned every time the primary tank is emptied, as established or recommended by local authorities.

Ecoflo[®] Coco Filter-ECDn unit

The owner of a biofiltration system shall follow the manufacturer's recommendations regarding the maintenance of the system. For that purpose, he shall at all times have a valid contract with the manufacturer or its local representative and, depending on the local regulations, a copy of the contract may have to be filed to the authorities.

Annual maintenance is important to ensure optimal performance of your Ecoflo[®] Coco Filter-ECDn unit and essential to maintain its warranty. Therefore, your biofilter must be serviced annually for the duration of its useful life. According to local regulations, more than 1 visit per year may be required.

The maintenance of your Ecoflo[®] Coco Filter-ECDn unit shall be carried out by one of our duly trained service providers. This service includes a visual inspection of all components and a verification of the operation, as well as maintenance of the filtering media. For maintenance purposes and to replace the filtering media, you must ensure that your system's lid is easily accessible at all time. Never cover or bury the lid of the Ecoflo[®] Coco Filter-ECDn unit. After each inspection, you will be given a maintenance record. Keep it with this manual in a safe place.

After a minimum of eight (10) years, the filtering media is analyzed by one of our authorized agents. Under normal usage, if the filtering media has not been abused and the operating guidelines have been respected, the filtering media might not have to be replaced and can be used for some additional years. However, your Ecoflo[®] Coco Filter-

ECDn unit's filtering media must be replaced before the system's treatment capacity and performance begins to deteriorate. The filtering media is easily pumped out using a truck adapted to emptying primary tanks. The new filtering media is then installed by an authorized agent or the pumper.

To know more about the maintenance of your Ecoflo[®] Coco Filter-ECDn unit, refer to your Maintenance Agreement. If you need help or more information, please call our Customer Service Department at **1 800 632-6356** or visit our website at **PREMIERTECHAQUA.COM**. Information regarding service of the unit are also available on the dataplate of the unit.



Ecoflo[®] Coco Filter-ECDn with Pump

Ecoflo[®] Coco Filters-ECDn unit are equipped with a pump that directs the treated effluent to an appropriate disposal mean according to local regulations and the primary tank via the recirculation line. The electro-mechanical components are included in this system. To learn more about electro-mechanical components, consult the Timed Dosing Units TPA-350DN Installation Guide and Owner's Manual.

Electrical connections

All electrical connections must be done by a **certified electrician** and using seal connectors is mandatory. Premier Tech Aqua recommends installing the power box on top of the pump vault insulating board to avoid humidity problems.

Use two (2) separate circuit-breakers, one to operate the pump and the other to connect the control unit. Do not connect anything else to these circuit-breakers (for example, a household appliance). They must be used exclusively for the pump and the alarm box.

What to do in case of...

An activated alarm

If an alarm is activated, unrelated to a power failure, contact Premier Tech Aqua's After-Sales Service Department so the problem can be identified and corrected.

A prolonged power failure

If a power failure that occurs during winter is prolonged, protect the components of your septic system against freezing. If you have any questions to restart your system, contact Premier Tech Aqua's After-Sales Service Department.

Flooding

Certain sites are prone to flooding or to rises in groundwater levels. This can lead to a malfunction in your septic system or alter the performance of your Ecoflo[®] Coco Filter-ECDn unit. If this happens, contact Premier Tech Aqua's After-Sales Service Department.

Backflow

Backflow rarely occurs. But if it does happen, the primary tank is usually the cause. Your primary tank installer or primary tank pumper can generally take care of the situation.

Odours

All septic systems are apt to generate gases and odours. The position of the air vent, as well as other factors unrelated to the Ecoflo[®] Coco Filter-ECDn unit itself, can prevent septic gases from dispersing properly and lead to odours. If this happens, contact Premier Tech Aqua's After-Sales Service Department.

If you have any questions or comments, do not hesitate to contact Premier Tech Aqua at 1 800 632-6356.

PREMIER TECH AQUA

1800 632-6356
 418 862-6642
 pta@premiertech.com
 PREMIERTECHAQUA.COM

The information contained in this document is based upon the latest information available at the time of publication and is designed to provide you with a general introduction to our products. We make no warranties or representations as to its accuracy. We are continually upda-ting and improving our products and reserve the right to amend, discontinue, alter or change specifications and prices without prior notice. Ecoflo[®] is a brand of Premier Tech Ltd. The Ecoflo[®] Biofilter is protected under patents: CA2499637; US7097768; ES2285173; EP1539325 (BE, FR). Notice issued on 2016-01-12. For current data regarding all patent application(s) and patent(s) for this product or any part thereof, consult the website patentmarking.premiertech.com (references: 3685).

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1. PREAMBLE

Premier Tech Technologies Ltd. (hereinafter called "Premier Tech") is proud to provide its customers with an exclusive wastewater treatment system guaranteed by an innovative Warranty.

For the application and interpretation of this Warranty, "Customer" shall mean the person who has purchased an Ecoflo[®] Coco Filter (hereinafter called "Initial Purchaser"), for a residential installation, as well as any subsequent purchaser (hereinafter called "Subsequent Purchaser(s) "), in accordance with the provisions of section 8 of this Warranty. "Successor(s)" shall mean any other person entitled to exercise the same rights as the Customer under the law.

2. NATURE OF THE WARRANTY

2.1. Ecoflo® Coco Filter

Premier Tech warrants to the Customer that the filtering media of the Ecoflo[®] Coco Filter shall function properly for a period of eight (10) years from the date of purchase by the Initial Purchaser (proof of purchase required).

Except as provided in sections 2.2 and 2.3 below, Premier Tech also warrants all parts of the Ecoflo[®] Coco Filter components against any manufacturing defect for a period of ten (10) years from the date of purchase by the Initial Purchaser (proof of purchase required). The first two years of the warranty also cover the labour.

2.2. Concrete

Premier Tech does not offer any additional Warranty on the shell of the concrete Ecoflo[®] Coco Filter. Accordingly, the Customer shall rely on the local concrete manufacturer's Warranty policy.

2.3. Pump, floats, alarm box and junction box

The pump, floats, alarm box and junction box included with the Ecoflo[®] Coco Filter are guaranteed for two (2) years (parts only), from the date of purchase by the Initial Purchaser (proof of purchase required). The first year of the warranty also covers the labour.

Premier Tech's conventional Warranty is expressly limited to the text of this Certificate and valid provided the Ecoflo[®] Coco Filter was installed in accordance with applicable regulations and with the manufacturer's recommendations.

3. NOTICE

For this Warranty to be valid, the Customer must notify Premier Tech in writing immediately upon the appearance of any indication of an anomaly or irregularity in the Ecoflo[®] Coco Filter.

Such notice shall be mailed to Premier Tech's Head Office at 1, avenue Premier, Rivière-du-Loup, Québec, G5R 6C1, CANADA or by facsimile at (418) 862-6642.

Upon receipt of this notice, Premier Tech shall examine the situation and, if necessary, take appropriate corrective measures in accordance with the terms of this Warranty.

4. GENERAL EXCLUSIONS

The following damages or problems are excluded from the Warranty:

(a) Any damage or problem caused by a fortuitous event or "force majeure", such as, without limiting the generality of

the foregoing, an earthquake, a flood, frost, hurricane, landslide, explosion or dynamiting;

(b) Any damage or problem caused by the fault or act of a third party including, without limiting the generality of the foregoing, the execution of landscaping work;

(c) Any damage or problem arising from a defective installation carried out by a person trained by Premier Tech, or any installation, modification, correction or addition carried out by a person not trained by Premier Tech;

(d) Any damage or problem arising from any installation, modification, correction or addition to the treatment system carried out after installation of the Ecoflo[®] Coco Filter without prior written approval from Premier Tech;

(e) Any damage or problem caused by the use of a septic tank that does not comply with the applicable regulations and/or with Premier Tech's specifications, as described in the Owner's Manual;

(f) Any damage or problem, if it is shown that the usage of the Ecoflo[®] Coco Filter was not in accordance with the instructions and guidelines described in the Owner's Manual;

(g) Any damage or problem, if the maintenance of the Ecoflo[®] Coco Filter was not carried out by a person authorized by Premier Tech, in accordance with the Maintenance Agreement;

(h) Any damage or problem caused by an omission or act of the Customer or the Customer's Successors including, without limiting the generality of the foregoing, refusal to allow access to the system for maintenance;

(i) Any damage or problem, if it is found that the Customer or the Customer's Successors have modified or changed the use of the property serviced by the Ecoflo[®] Coco Filter resulting in the alteration of the nature or quality of wastewater being treated and/or that constitutes a violation of the applicable regulations;

(j) Any damage or problem caused by and/or resulting from the work carried out to access to the Ecoflo[®] Coco Filter, including, without limiting the generality of the foregoing, excavation, snow removal or demolition;

(k) Any damage or problem resulting from the condition of the site or of the soil and not reported or not properly reported to Premier Tech by the Customer or the person undertaking the site investigation.

5. PARTICULAR EXCLUSIONS

It is further expressly understood that the Customer may not carry out or cause to be carried out any repair or verification of the Ecoflo[®] Coco Filter sold to him, or attempt to carry out any work or to apply any corrective measures whatsoever to said work, before notifying Premier Tech in accordance with the provisions of section 3 of this Warranty and before Premier Tech has visited the site, within a reasonable time following receipt of said notice, to assess the situation.

If the Customer carries out or causes to be carried out repairs, or attempts to repair or to apply corrective measures of any kind whatsoever to the Ecoflo® Coco Filter sold to him without prior authorization by Premier Tech, this Warranty shall be considered null and void and Premier Tech shall be considered completely discharged from any and all of its obligations under this Warranty.

6. INDEMNITIES AND DAMAGES

Subject to the application of the provisions and exclusions provided for in this Warranty, Premier Tech's liability and obligations regarding any corrective measure carried out or any attempt to correct an indicated problem shall be limited to replacing the filtering media and/or one or several components of the Ecoflo[®] Coco Filter and to supplying the required labour, if applicable.

7. LIMITATION OF LIABILITY

Premier Tech's compensation or indemnification obligation shall be limited to the provisions of section 6 of this Certificate of Warranty and Premier Tech shall not be held liable for any other damage or loss that may have been suffered or incurred by the Customer or any third party in connection with the Ecoflo[®] Coco Filter, its parts and/or components which originate thereof.

No additional warranty, express or implied, hence excluding any direct or indirect consequential damages (not limited to but including third parties loss) concerning the design, sale or use of the Ecoflo[®] Coco Filter and/or services provided by Premier Tech is hereby granted. Premier Tech's liability under its warranty obligation shall in no case exceed the cost of the Ecoflo[®] Coco Filter.

8. TRANSFER OF OWNERSHIP

In the event of transfer of ownership, sale, assignment or disposal in any way whatsoever of the Customer's property to a third party, this Warranty shall continue to apply if and only if the Subsequent Purchaser or the Successor confirms, by forwarding the attached "Notice of New Property Owner" to Premier Tech within a reasonable delay, that he/she is the new owner of the property, he/she understands and is aware of the content of this Certificate of Warranty and accepts its terms and conditions.

The person who proceeds with the transfer, sale, assignment or disposal of any way whatsoever of the property undertakes to hand over to the Subsequent Purchaser or the Successor the Certificate of Warranty provided upon completion of the work, as well as the Owner's Manual and, if applicable, the Maintenance and Environmental Monitoring Program for the Ecoflo[®] Coco Filter.

Failure to abide by the terms and conditions of section 8 of this Certificate of Warranty may, at Premier Tech's discretion, render it invalid or to be rejected.

9. INSPECTION

The Customer and/or the Customer's Successors shall allow Premier Tech or its duly authorized representatives to carry out all necessary monitoring and inspections, as required, for implementation of this Warranty.

If the Customer and/or the Customer's Successors notify Premier Tech of an alleged defect or malfunction of the Ecoflo® Coco Filter and that, after inspection, it is found that no such defect or malfunction exists or that such defect or malfunction is excluded from or does not apply to the Warranty, a minimum charge of \$150.00 plus direct expenses shall be paid by the Customer and/or the Customer's Successors for the cost of the inspection.

10. INTERPRETATION

The terms and conditions of this Warranty shall be interpreted according to and governed by the provisions of this Warranty and the legislation in effect in the Province of Quebec.

11. PRIORITY OF THE CERTIFICATE OF WARRANTY

This Warranty supersedes any contract or understanding, written or verbal, entered into between the Customer and Premier Tech. In the event of contradiction between this Warranty and any other documents and/or contracts entered into between the Customer and Premier Tech, this Warranty shall prevail.

12. PURCHASERS AND SUCCESSORS

Subject to the provisions of this Warranty and especially those of section 8, this Warranty shall continue to be valid for Subsequent Purchasers and Successors and shall continue to have full effect until the end of the agreed Warranty period provided for in section 2 of this Certificate.

	Notice of New Property O	wner
Send a copy to Premier Tech	n Aqua.	
Name of previous the owner:		
I, the undersigned,	hereby declare that I h	nave acquired the property located at
Civic Number Street	City	Province or State
ZIP or Postal Code	() Phone number	
wish to benefit from this Warranty is, undertakings and conditions set f	for the remaining period, if any, and fro I accept to be bound by this Wa forth therein. I have had the opportur	echnologies Ltd for the Ecoflo [®] Coco Filter. I om the date of the transfer of ownership, that rranty and by any and all of the sections, nity to examine the Ecoflo [®] Coco Filter and r Tech Technologies Ltd. to take note of this
Signature:	Date: _	
Name of new owner:(block lette	ers)	
Language preference: 🗅 English 🕻	☐ French New owner's e-n	nail address: