

# Geotechnical Investigation

## Proposed Industrial Building

2167 McGee Side Road  
Ottawa, Ontario

Prepared for Stoked Industries Inc.

Report PG6662-1 Revision 1 dated September 14, 2023

## Table of Contents

	<b>PAGE</b>
<b>1.0 Introduction .....</b>	<b>1</b>
<b>2.0 Proposed Development.....</b>	<b>1</b>
<b>3.0 Method of Investigation .....</b>	<b>2</b>
3.1 Field Investigation .....	2
3.2 Field Survey .....	3
3.3 Laboratory Testing .....	3
3.4 Analytical Testing .....	3
<b>4.0 Observations .....</b>	<b>4</b>
4.1 Surface Conditions.....	4
4.2 Subsurface Profile.....	4
4.3 Groundwater .....	4
<b>5.0 Discussion .....</b>	<b>6</b>
5.1 Geotechnical Assessment.....	6
5.2 Site Grading and Preparation.....	6
5.3 Foundation Design .....	7
5.4 Design for Earthquakes.....	8
5.5 Slab on Grade Construction .....	8
5.6 Pavement Design.....	8
<b>6.0 Design and Construction Precautions.....</b>	<b>10</b>
6.1 Foundation Drainage and Backfill .....	10
6.2 Protection Against Frost Action .....	10
6.3 Excavation Side Slopes .....	11
6.4 Pipe Bedding and Backfill .....	11
6.5 Groundwater Control.....	12
6.6 Winter Construction.....	13
6.7 Corrosion Potential and Sulphate.....	13
<b>7.0 Recommendations .....</b>	<b>14</b>
<b>8.0 Statement of Limitations.....</b>	<b>15</b>

---

## Appendices

- Appendix 1**      Soil Profile and Test Data Sheets  
                     Symbols and Terms  
                     Analytical Test Results
- Appendix 2**      Figure 1 – Key Plan  
                     Drawing PG6662-1 – Test Hole Location Plan

## 1.0 Introduction

Paterson Group (Paterson) was commissioned by Stoked Industries Inc. to prepare a geotechnical investigation report for the proposed industrial building to be located at 2167 McGee Side Road, Ottawa, Ontario (refer to Figure 1 - Key Plan presented in Appendix 2).

The objective of the geotechnical investigation was to:

- determine the subsoil and groundwater conditions at the site by means of boreholes.
- provide geotechnical recommendations for the design of the proposed building including construction considerations which may affect its design.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. It contains our findings and includes geotechnical recommendations pertaining to the design and construction of the subject development as they are understood at the time of writing this report.

## 2.0 Proposed Development

Based on the available conceptual drawings, it is understood that the proposed building will consist of a low-rise industrial building of slab-on-grade construction with an approximate footprint of 1635 m<sup>2</sup>. It is also understood that there will be a mezzanine. Associated asphalt-paved access lanes, parking areas with landscaped margins, and garbage enclosure are also proposed surrounding the building. It is further understood that the proposed building will be municipally serviced.

## **3.0 Method of Investigation**

### **3.1 Field Investigation**

#### **Field Program**

The field program for the current investigation was carried out on November 20, 2020. At that time, seven (7) boreholes were advanced to a maximum depth of 4.8m. The borehole locations were distributed in a manner to provide general coverage of the subject site taking into consideration site features and underground utilities. The test hole locations of the boreholes are shown on Drawing PG6662-1 - Test Hole Location Plan included in Appendix 2.

The boreholes were completed using a track-mounted auger drill rig operated by a two-person crew. All fieldwork was conducted under the full-time supervision of Paterson personnel under the direction of a senior engineer from the geotechnical division. The drilling procedure consisted of augering to the required depth at the selected locations, sampling and testing the overburden.

#### **Sampling and In Situ Testing**

The soil samples were recovered from the auger flights and using a 50 mm diameter split-spoon sampler. The samples were initially classified on site, placed in sealed plastic bags and transported to our laboratory. The depths at which the auger, split-spoon and grab samples were recovered from the boreholes are shown as AU and SS, respectively, on the Soil Profile and Test Data sheets in Appendix 1.

The Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as “N” values on the Soil Profile and Test Data sheets. The “N” value is the number of blows required to drive the split-spoon sampler 300 mm into the soil after a 150 mm initial penetration using a 63.5 kg hammer falling from a height of 760 mm.

The subsurface conditions observed in the test holes were recorded in detail in the field. The soil profiles are logged on the Soil Profile and Test Data sheets in Appendix 1 of this report.

---

## **Groundwater**

Flexible polyethylene standpipes were installed in all boreholes during the filed investigation to permit monitoring of the groundwater levels subsequent to the completion of the sampling program. The groundwater observations are discussed in subsection 4.3 and presented in the Soil Profile and Test Data Sheets in Appendix 1

### **3.2 Field Survey**

The test hole locations were selected by Paterson to provide general coverage of the subject site. The test hole locations and ground surface elevation at each test hole location were surveyed by Paterson using a high precision GPS and referenced to a geodetic datum. The location of the test holes is presented on Drawing PG6662-1 - Test Hole Location Plan in Appendix 2.

### **3.3 Laboratory Testing**

Soil samples were recovered from the subject site and visually examined in our laboratory to review the results of the field logging.

### **3.4 Analytical Testing**

One (1) soil sample was submitted for analytical testing to assess the corrosion potential for exposed ferrous metals and the potential of sulphate attacks against subsurface concrete structures by others. The sample was submitted to determine the concentration of sulphate and chloride, the resistivity, and the pH of the samples. The results are presented in Appendix 1 and discussed further in Subsection 6.7.

## **4.0 Observations**

### **4.1 Surface Conditions**

The majority of the subject site is occupied by a gravel-surface parking area. A culvert system, which runs parallel with McGee Side Road and John Cavanaugh Drive, is located along the eastern and southern limits of the site, respectively. The subject site is bordered by undeveloped, densely treed, land to the north, John Cavanaugh Drive to the east, McGee Side Road to the south and a commercial property to the west. The existing ground surface is relatively flat within the central portion of the site at approximate geodetic elevations of 117 to 118 m. At the eastern and southern boundaries of the site, the ground surface slopes downwards from approximate geodetic elevations of 119 to 116.5 m, where the culvert system is present.

### **4.2 Subsurface Profile**

#### **Overburden**

Generally, the subsurface profile observed at the test hole locations consists of an approximate 0.3 to 0.6 m thick layer of fill which is composed of a brown silty sand with crushed stone. A glacial till deposit was encountered underlying the fill and was observed to consist of a compact to very dense, brown silty sand with gravel, cobbles and boulders.

Practical refusal to Practical refusal to augering was encountered in all boreholes at depths ranging from 1.6 to 4.8 m.

Reference should be made to the Soil Profile and Test Data sheets in Appendix 1 for specific details of the soil profiles encountered at each test hole location.

#### **Bedrock**

Based on the available geological mapping and on the findings of the investigation, the site is located in an area where the bedrock consists of interbedded limestone and shale of the Verulam formation with a drift thickness of 3 to 5 m.

### **4.3 Groundwater**

Groundwater levels were measured in the installed piezometers on December 2, 2020. The groundwater readings obtained are summarised in Table 1 below and are also presented on the Soil Profile and Test Data sheets in Appendix 1.

<b>Table 1 – Summary of Groundwater Levels</b>				
<b>Test Hole</b>	<b>Ground Surface Elevation (m)</b>	<b>Measured Groundwater Level</b>		<b>Date Recorded</b>
		<b>Depth (m)</b>	<b>Elevation (m)</b>	
BH 1	117.98	3.96	114.02	December 2, 2020
BH 2	117.96	Blocked	-	December 2, 2020
BH 3	117.39	2.26	115.13	December 2, 2020
BH 4	117.07	0.94	116.13	December 2, 2020
BH 5	117.98	1.33	116.65	December 2, 2020
BH 6	117.23	2.09	115.14	December 2, 2020
BH 7	118.08	Blocked	-	December 2, 2020

**Note:** The ground surface elevation at each borehole location was surveyed using a high precision GPS and referenced to a geodetic datum.

It is important to note that groundwater level readings could be influenced by surface water infiltrating the backfilled borehole. Long-term groundwater levels can also be estimated based on recovered soils samples moisture levels and recovered soil sample coloring and consistency. Based on the existing groundwater information and our knowledge of the groundwater within the area, the long-term groundwater level is estimated to be at **2 to 3 m** depth below the existing grade.

It should be noted that groundwater levels are subject to seasonal fluctuations. Therefore, the groundwater level could vary at the time of construction.



## **5.0 Discussion**

### **5.1 Geotechnical Assessment**

From a geotechnical perspective, the subject site is considered suitable for the proposed building. The proposed industrial building can be founded using conventional shallow footings placed on over an undisturbed, compact to very dense glacial till bearing surface.

Dependent on the founding depths of the site servicing, it is anticipated that localized bedrock removal may be required. However, it is expected that bedrock removal, if required, would be in relatively small quantities and could be completed using hoe ramming. This is discussed further in Section 5.2

The above and other considerations are further discussed in the following sections.

### **5.2 Site Grading and Preparation**

#### **Stripping Depth**

Topsoil and deleterious fill, such as those containing significant amounts of organic materials, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures.

#### **Bedrock Removal**

Bedrock removal could be carried out by hoe-ramming where only small quantities of bedrock need to be removed. It is anticipated that bedrock may be encountered within the depth of excavation required for the installation of site services. It is expected that bedrock removal will be possible using hoe ramming in conjunction with conventional excavation techniques.

#### **Fill Placement**

Fill used for grading beneath the building areas should consist, unless otherwise specified, of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A, Granular B Type II. This material should be tested and approved prior to delivery to the site. The fill should be placed in lifts no greater than 300 mm thick and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building areas should be compacted to at least 98% of its standard Proctor maximum dry density (SPMDD).

Non-specified existing fill, along with site-excavated soil, can be used as general landscaping fill where settlement of the ground surface is of minor concern. These materials should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If these materials are to be used to build up the subgrade level for areas to be paved, they should be compacted in thin lifts to a minimum density of 95% of their respective SPMDD.

Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless used in conjunction with a geocomposite drainage membrane, such as Miradrain G100N or Delta Drain 6000.

## 5.3 Foundation Design

### **Bearing Resistance Values (Conventional Shallow Footings)**

Strip footings, up to 2 m wide, and pad footings, up to 5 m wide, placed on an undisturbed, compact to very dense glacial till bearing surface can be designed using a bearing resistance value at serviceability limit states (SLS) of **300 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **450 kPa**. A geotechnical resistance factor of 0.5 was applied to the bearing resistance value at ULS.

An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

Footings designed using the bearing resistance values at SLS given above will be subjected to potential post construction total and differential settlements of 25 and 20 mm, respectively

### **Lateral Support**

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to the encountered overburden material above the groundwater table when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil or engineered fill of the same or higher capacity as the bearing medium soil.

## 5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class C**. If a higher seismic site class is required (Class A or B), a site-specific shear wave velocity test may be completed to accurately determine the applicable seismic site classification for foundation design of the proposed building, as presented in Table 4.1.8.4.A of the 2012 Ontario Building Code (OBS 2012).

The soils underlying the subject site are not susceptible to liquefaction. Reference should be made to the latest revision of the 2012 Ontario Building Code for a full discussion of the earthquake design requirements.

## 5.5 Slab on Grade Construction

With the removal of any topsoil and fill, such as those containing significant amounts of deleterious materials, the existing fill or native soil subgrade approved by Paterson at the time of excavation will be considered an acceptable subgrade surface on which to commence backfilling for floor slab construction. It is recommended that the slab-on-grade subgrade be proof-rolled with a suitably sized roller making several passes under dry conditions prior to fill placement, and which is approved by Paterson personnel. Any poor performing areas should be removed and replaced with an engineered fill, such as Granular B Type II.

The upper 200 mm of sub-slab fill is recommended to consist of OPSS Granular A crushed stone. All backfill material within the footprint of the proposed building should be placed in maximum 300 mm thick loose layers and compacted to a minimum of 98% of the SPMDD.

## 5.6 Pavement Design

The pavement structures presented in the following tables can be used for the design of car only parking, access lanes and heavy truck parking areas.

<b>Table 2 - Recommended Pavement Structure – Car Only Parking Areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
50	<b>Wear Course</b> - HL-3 or Superpave 12.5 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
300	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> – Either fill, in situ soils or OPSS Granular B Type I or II material placed over in situ soil, or fill	

<b>Table 3 - Recommended Pavement Structure – Access Lanes and Heavy Truck Parking Areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
40	<b>Wear Course</b> - HL-3 or Superpave 12.5 Asphaltic Concrete
50	<b>Binder Course</b> - HL-8 or Superpave 19.0 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
400	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> – Either fill, in situ soils or OPSS Granular B Type I or II material placed over in situ soil, or fill	

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type I or II material. The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

## **6.0 Design and Construction Precautions**

### **6.1 Foundation Drainage and Backfill**

#### **Foundation Drainage**

It is recommended that a perimeter foundation drainage system be provided for the proposed structures. The system should consist of a 150 mm diameter perforated corrugated plastic pipe, surrounded on all sides by 150 mm of 10 mm clear crushed stone, placed at the footing level around the exterior perimeter of the structure. The pipe should have a positive outlet, such as a gravity connection to the sump pump pit or storm sewer.

#### **Foundation Backfilling**

Backfill against the exterior sides of the foundation walls should consist of free-draining non frost susceptible granular materials. The greater part of the site excavated materials will be frost susceptible and, as such, are not recommended for re-use as backfill against the foundation walls, unless used in conjunction with a drainage geocomposite, such as Miradrain G100N or Delta Drain 6000, connected to the perimeter foundation drainage system. Imported granular materials, such as clean sand or OPSS Granular B Type I granular material, should otherwise be used for this purpose.

#### **Concrete Sidewalks and Walkways**

Backfill material below sidewalks and walkway subgrade areas throughout the subject site, including along the building, should be provided with a minimum 300 mm thick layer of OPSS Granular A or OPSS Granular B Type II crushed stone. This material should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 98% of the materials SPMDD. The subgrade for walkway structures against the building should be shaped to promote drainage towards the buildings perimeter drainage system.

### **6.2 Protection Against Frost Action**

Perimeter footings of heated structures are required to be insulated against the deleterious effect of frost action. A minimum of 1.5 m thick soil cover (or equivalent) should be provided in this regard.

Exterior unheated footings, such as those for isolated exterior piers, are more prone to deleterious movement associated with frost action than the exterior walls of the structure proper and require additional protection, such as soil cover of 2.1 m or a combination of soil cover and foundation insulation.

### **6.3 Excavation Side Slopes**

The side slopes of excavations in the overburden materials should either be cut back at acceptable slopes or should be retained by shoring systems from the start of the excavation until the structure is backfilled. It is assumed that sufficient room will be available for the greater part of the excavation to be undertaken by open-cut methods (i.e. unsupported excavations).

#### **Unsupported Excavations**

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be cut back at 1H:1V or flatter. The flatter slope is required for excavation below groundwater level. The subsoil at this site is considered to be mainly a Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

It is recommended that a trench box be used at all times to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

### **6.4 Pipe Bedding and Backfill**

Bedding and backfill materials should be in accordance with the most recent Material Specifications & Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Service Branch of the City of Ottawa.

The pipe bedding for sewer and water pipes should consist of at least 150 mm of OPSS Granular A material. The material should be placed in maximum 300 mm thick lifts and compacted to a minimum of 98% of its SPMDD. The bedding should extend at least to the spring line of the pipe.

The cover material, which should consist of OPSS Granular A, should extend from the spring line of the pipe to at least 300 mm above the top of the pipe. The material should be placed in maximum 300 mm thick lifts and compacted to 98% of the SPMDD.

It should generally be possible to re-use the site materials above the cover material if the operations are carried out in dry weather conditions.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in maximum 225 mm thick loose lifts and compacted to a minimum of 95% of the material standard Proctor maximum dry density.

## **6.5 Groundwater Control**

### **Groundwater Control for Building Construction**

Based on our observations, it is anticipated that groundwater infiltration into the excavations should be controllable using open sumps. Pumping from open sumps should be sufficient to control the groundwater influx through the sides of shallow excavations. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

### **Permit to Take Water**

A temporary Ministry of the Environment, Conservation and Parks (MECP) permit to take water (PTTW) may be required for this project if more than 400,000 L/day of ground and/or surface water is to be pumped during the construction phase. A minimum 4 to 5 months should be allowed for completion of the PTTW application package and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16.

However, based on the groundwater level readings at the borehole locations, as well as the observed colour, moisture content and consistency of the recovered soil samples, it is anticipated that the long-term groundwater levels are below the founding elevation of the proposed building. Therefore, an EASR or PTTW will not be required for dewatering during the construction of the proposed development.

## **6.6 Winter Construction**

Precautions must be taken if winter construction is considered for this project.

The subsoil conditions at this site mostly consist of frost susceptible materials. In the presence of water and freezing conditions, ice could form within the soil mass. Heaving and settlement upon thawing could occur.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters and tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are also difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be taken if such activities are to be carried out during freezing conditions.

## **6.7 Corrosion Potential and Sulphate**

The results of analytical testing show that the sulphate content is less than 0.01%. This result is indicative that Type 10 Portland cement (normal cement) would be appropriate for this site. The chloride content and the pH of the sample indicate they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity is indicative of a non-aggressive to slightly aggressive corrosive environment.



## 7.0 Recommendations

It is a requirement for the foundation design data provided herein to be applicable that the following material testing and observation program be performed by the geotechnical consultant.

- Observation of all bearing surfaces prior to the placement of concrete.
- Sampling and testing of the concrete and fill materials.
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- Observation of all subgrades prior to backfilling.
- Field density tests to ensure that the specified level of compaction has been achieved.
- Sampling and testing of the bituminous concrete including mix design reviews.

All excess soils generated by construction activities should be handled as per *Ontario Regulation 406/19: On-Site and Excess Soil Management*.

A report confirming that these works have been conducted in general accordance with our recommendations could be issued upon the completion of a satisfactory inspection program by the geotechnical consultant.

## 8.0 Statement of Limitations

The recommendations provided are in accordance with the present understanding of the project. Paterson requests permission to review the recommendations when the drawings and specifications are completed.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, Paterson requests immediate notification to permit reassessment of our recommendations.

The recommendations provided herein should only be used by the design professionals associated with this project. They are not intended for contractors bidding on or undertaking the work. The latter should evaluate the factual information provided in this report and determine the suitability and completeness for their intended construction schedule and methods. Additional testing may be required for their purposes.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Stoked Industries Inc. or their agent(s) is not authorized without review by Paterson Group for the applicability of our recommendations to the altered use of the report.

### Paterson Group Inc.

Zubaida Al-Moselly, P.Eng.



Maha K. Saleh, M.A.Sc., P.Eng.

### Report Distribution:

- Stoked Industries Inc. (email copy)
- Paterson Group (1 copy)

# APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

SYMBOLS AND TERMS

ANALYTICAL TEST RESULTS

## Table of Contents

	<b>PAGE</b>
<b>1.0 Introduction .....</b>	<b>1</b>
<b>2.0 Proposed Development.....</b>	<b>1</b>
<b>3.0 Method of Investigation .....</b>	<b>2</b>
3.1 Field Investigation .....	2
3.2 Field Survey .....	3
3.3 Laboratory Testing .....	3
3.4 Analytical Testing .....	3
<b>4.0 Observations .....</b>	<b>4</b>
4.1 Surface Conditions.....	4
4.2 Subsurface Profile.....	4
4.3 Groundwater .....	4
<b>5.0 Discussion .....</b>	<b>6</b>
5.1 Geotechnical Assessment.....	6
5.2 Site Grading and Preparation.....	6
5.3 Foundation Design .....	7
5.4 Design for Earthquakes.....	8
5.5 Slab on Grade Construction .....	8
5.6 Pavement Design.....	8
<b>6.0 Design and Construction Precautions.....</b>	<b>10</b>
6.1 Foundation Drainage and Backfill .....	10
6.2 Protection Against Frost Action.....	10
6.3 Excavation Side Slopes .....	11
6.4 Pipe Bedding and Backfill .....	11
6.5 Groundwater Control.....	12
6.6 Winter Construction.....	13
6.7 Corrosion Potential and Sulphate.....	13
<b>7.0 Recommendations .....</b>	<b>14</b>
<b>8.0 Statement of Limitations.....</b>	<b>15</b>

## Appendices

- Appendix 1**      Soil Profile and Test Data Sheets  
                         Symbols and Terms  
                         Analytical Test Results
- Appendix 2**      Figure 1 – Key Plan  
                         Drawing PG6662-1 – Test Hole Location Plan

## 1.0 Introduction

Paterson Group (Paterson) was commissioned by Stoked Industries Inc. to prepare a geotechnical investigation report for the proposed industrial building to be located at 2167 McGee Side Road, Ottawa, Ontario (refer to Figure 1 - Key Plan presented in Appendix 2).

The objective of the geotechnical investigation was to:

- determine the subsoil and groundwater conditions at the site by means of boreholes.
- provide geotechnical recommendations for the design of the proposed building including construction considerations which may affect its design.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. It contains our findings and includes geotechnical recommendations pertaining to the design and construction of the subject development as they are understood at the time of writing this report.

## 2.0 Proposed Development

Based on the available conceptual drawings, it is understood that the proposed building will consist of a low-rise industrial building of slab-on-grade construction with an approximate footprint of 1635 m<sup>2</sup>. It is also understood that there will be a mezzanine. Associated asphalt-paved access lanes, parking areas with landscaped margins, and garbage enclosure are also proposed surrounding the building. It is further understood that the proposed building will be municipally serviced.

## **3.0 Method of Investigation**

### **3.1 Field Investigation**

#### **Field Program**

The field program for the current investigation was carried out on November 20, 2020. At that time, seven (7) boreholes were advanced to a maximum depth of 4.8m. The borehole locations were distributed in a manner to provide general coverage of the subject site taking into consideration site features and underground utilities. The test hole locations of the boreholes are shown on Drawing PG6662-1 - Test Hole Location Plan included in Appendix 2.

The boreholes were completed using a track-mounted auger drill rig operated by a two-person crew. All fieldwork was conducted under the full-time supervision of Paterson personnel under the direction of a senior engineer from the geotechnical division. The drilling procedure consisted of augering to the required depth at the selected locations, sampling and testing the overburden.

#### **Sampling and In Situ Testing**

The soil samples were recovered from the auger flights and using a 50 mm diameter split-spoon sampler. The samples were initially classified on site, placed in sealed plastic bags and transported to our laboratory. The depths at which the auger, split-spoon and grab samples were recovered from the boreholes are shown as AU and SS, respectively, on the Soil Profile and Test Data sheets in Appendix 1.

The Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as “N” values on the Soil Profile and Test Data sheets. The “N” value is the number of blows required to drive the split-spoon sampler 300 mm into the soil after a 150 mm initial penetration using a 63.5 kg hammer falling from a height of 760 mm.

The subsurface conditions observed in the test holes were recorded in detail in the field. The soil profiles are logged on the Soil Profile and Test Data sheets in Appendix 1 of this report.

---

## **Groundwater**

Flexible polyethylene standpipes were installed in all boreholes during the filed investigation to permit monitoring of the groundwater levels subsequent to the completion of the sampling program. The groundwater observations are discussed in subsection 4.3 and presented in the Soil Profile and Test Data Sheets in Appendix 1

### **3.2 Field Survey**

The test hole locations were selected by Paterson to provide general coverage of the subject site. The test hole locations and ground surface elevation at each test hole location were surveyed by Paterson using a high precision GPS and referenced to a geodetic datum. The location of the test holes is presented on Drawing PG6662-1 - Test Hole Location Plan in Appendix 2.

### **3.3 Laboratory Testing**

Soil samples were recovered from the subject site and visually examined in our laboratory to review the results of the field logging.

### **3.4 Analytical Testing**

One (1) soil sample was submitted for analytical testing to assess the corrosion potential for exposed ferrous metals and the potential of sulphate attacks against subsurface concrete structures by others. The sample was submitted to determine the concentration of sulphate and chloride, the resistivity, and the pH of the samples. The results are presented in Appendix 1 and discussed further in Subsection 6.7.



## **4.0 Observations**

### **4.1 Surface Conditions**

The majority of the subject site is occupied by a gravel-surface parking area. A culvert system, which runs parallel with McGee Side Road and John Cavanaugh Drive, is located along the eastern and southern limits of the site, respectively. The subject site is bordered by undeveloped, densely treed, land to the north, John Cavanaugh Drive to the east, McGee Side Road to the south and a commercial property to the west. The existing ground surface is relatively flat within the central portion of the site at approximate geodetic elevations of 117 to 118 m. At the eastern and southern boundaries of the site, the ground surface slopes downwards from approximate geodetic elevations of 119 to 116.5 m, where the culvert system is present.

### **4.2 Subsurface Profile**

#### **Overburden**

Generally, the subsurface profile observed at the test hole locations consists of an approximate 0.3 to 0.6 m thick layer of fill which is composed of a brown silty sand with crushed stone. A glacial till deposit was encountered underlying the fill and was observed to consist of a compact to very dense, brown silty sand with gravel, cobbles and boulders.

Practical refusal to Practical refusal to augering was encountered in all boreholes at depths ranging from 1.6 to 4.8 m.

Reference should be made to the Soil Profile and Test Data sheets in Appendix 1 for specific details of the soil profiles encountered at each test hole location.

#### **Bedrock**

Based on the available geological mapping and on the findings of the investigation, the site is located in an area where the bedrock consists of interbedded limestone and shale of the Verulam formation with a drift thickness of 3 to 5 m.

### **4.3 Groundwater**

Groundwater levels were measured in the installed piezometers on December 2, 2020. The groundwater readings obtained are summarised in Table 1 below and are also presented on the Soil Profile and Test Data sheets in Appendix 1.

<b>Table 1 – Summary of Groundwater Levels</b>				
<b>Test Hole</b>	<b>Ground Surface Elevation (m)</b>	<b>Measured Groundwater Level</b>		<b>Date Recorded</b>
		<b>Depth (m)</b>	<b>Elevation (m)</b>	
BH 1	117.98	3.96	114.02	December 2, 2020
BH 2	117.96	Blocked	-	December 2, 2020
BH 3	117.39	2.26	115.13	December 2, 2020
BH 4	117.07	0.94	116.13	December 2, 2020
BH 5	117.98	1.33	116.65	December 2, 2020
BH 6	117.23	2.09	115.14	December 2, 2020
BH 7	118.08	Blocked	-	December 2, 2020

**Note:** The ground surface elevation at each borehole location was surveyed using a high precision GPS and referenced to a geodetic datum.

It is important to note that groundwater level readings could be influenced by surface water infiltrating the backfilled borehole. Long-term groundwater levels can also be estimated based on recovered soils samples moisture levels and recovered soil sample coloring and consistency. Based on the existing groundwater information and our knowledge of the groundwater within the area, the long-term groundwater level is estimated to be at **2 to 3 m** depth below the existing grade.

It should be noted that groundwater levels are subject to seasonal fluctuations. Therefore, the groundwater level could vary at the time of construction.

## **5.0 Discussion**

### **5.1 Geotechnical Assessment**

From a geotechnical perspective, the subject site is considered suitable for the proposed building. The proposed industrial building can be founded using conventional shallow footings placed on over an undisturbed, compact to very dense glacial till bearing surface.

Dependent on the founding depths of the site servicing, it is anticipated that localized bedrock removal may be required. However, it is expected that bedrock removal, if required, would be in relatively small quantities and could be completed using hoe ramming. This is discussed further in Section 5.2

The above and other considerations are further discussed in the following sections.

### **5.2 Site Grading and Preparation**

#### **Stripping Depth**

Topsoil and deleterious fill, such as those containing significant amounts of organic materials, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures.

#### **Bedrock Removal**

Bedrock removal could be carried out by hoe-ramming where only small quantities of bedrock need to be removed. It is anticipated that bedrock may be encountered within the depth of excavation required for the installation of site services. It is expected that bedrock removal will be possible using hoe ramming in conjunction with conventional excavation techniques.

#### **Fill Placement**

Fill used for grading beneath the building areas should consist, unless otherwise specified, of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A, Granular B Type II. This material should be tested and approved prior to delivery to the site. The fill should be placed in lifts no greater than 300 mm thick and compacted using suitable compaction equipment for the lift thickness. Fill placed beneath the building areas should be compacted to at least 98% of its standard Proctor maximum dry density (SPMDD).

Non-specified existing fill, along with site-excavated soil, can be used as general landscaping fill where settlement of the ground surface is of minor concern. These materials should be spread in thin lifts and at least compacted by the tracks of the spreading equipment to minimize voids. If these materials are to be used to build up the subgrade level for areas to be paved, they should be compacted in thin lifts to a minimum density of 95% of their respective SPMDD.

Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless used in conjunction with a geocomposite drainage membrane, such as Miradrain G100N or Delta Drain 6000.

## 5.3 Foundation Design

### **Bearing Resistance Values (Conventional Shallow Footings)**

Strip footings, up to 2 m wide, and pad footings, up to 5 m wide, placed on an undisturbed, compact to very dense glacial till bearing surface can be designed using a bearing resistance value at serviceability limit states (SLS) of **300 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **450 kPa**. A geotechnical resistance factor of 0.5 was applied to the bearing resistance value at ULS.

An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

Footings designed using the bearing resistance values at SLS given above will be subjected to potential post construction total and differential settlements of 25 and 20 mm, respectively

### **Lateral Support**

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to the encountered overburden material above the groundwater table when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V passes only through in situ soil or engineered fill of the same or higher capacity as the bearing medium soil.

## 5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class C**. If a higher seismic site class is required (Class A or B), a site-specific shear wave velocity test may be completed to accurately determine the applicable seismic site classification for foundation design of the proposed building, as presented in Table 4.1.8.4.A of the 2012 Ontario Building Code (OBS 2012).

The soils underlying the subject site are not susceptible to liquefaction. Reference should be made to the latest revision of the 2012 Ontario Building Code for a full discussion of the earthquake design requirements.

## 5.5 Slab on Grade Construction

With the removal of any topsoil and fill, such as those containing significant amounts of deleterious materials, the existing fill or native soil subgrade approved by Paterson at the time of excavation will be considered an acceptable subgrade surface on which to commence backfilling for floor slab construction. It is recommended that the slab-on-grade subgrade be proof-rolled with a suitably sized roller making several passes under dry conditions prior to fill placement, and which is approved by Paterson personnel. Any poor performing areas should be removed and replaced with an engineered fill, such as Granular B Type II.

The upper 200 mm of sub-slab fill is recommended to consist of OPSS Granular A crushed stone. All backfill material within the footprint of the proposed building should be placed in maximum 300 mm thick loose layers and compacted to a minimum of 98% of the SPMDD.

## 5.6 Pavement Design

The pavement structures presented in the following tables can be used for the design of car only parking, access lanes and heavy truck parking areas.

<b>Table 2 - Recommended Pavement Structure – Car Only Parking Areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
50	<b>Wear Course</b> - HL-3 or Superpave 12.5 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
300	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> – Either fill, in situ soils or OPSS Granular B Type I or II material placed over in situ soil, or fill	

<b>Table 3 - Recommended Pavement Structure – Access Lanes and Heavy Truck Parking Areas</b>	
<b>Thickness (mm)</b>	<b>Material Description</b>
40	<b>Wear Course</b> - HL-3 or Superpave 12.5 Asphaltic Concrete
50	<b>Binder Course</b> - HL-8 or Superpave 19.0 Asphaltic Concrete
150	<b>BASE</b> - OPSS Granular A Crushed Stone
400	<b>SUBBASE</b> - OPSS Granular B Type II
<b>SUBGRADE</b> – Either fill, in situ soils or OPSS Granular B Type I or II material placed over in situ soil, or fill	

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type I or II material. The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

## **6.0 Design and Construction Precautions**

### **6.1 Foundation Drainage and Backfill**

#### **Foundation Drainage**

It is recommended that a perimeter foundation drainage system be provided for the proposed structures. The system should consist of a 150 mm diameter perforated corrugated plastic pipe, surrounded on all sides by 150 mm of 10 mm clear crushed stone, placed at the footing level around the exterior perimeter of the structure. The pipe should have a positive outlet, such as a gravity connection to the sump pump pit or storm sewer.

#### **Foundation Backfilling**

Backfill against the exterior sides of the foundation walls should consist of free-draining non frost susceptible granular materials. The greater part of the site excavated materials will be frost susceptible and, as such, are not recommended for re-use as backfill against the foundation walls, unless used in conjunction with a drainage geocomposite, such as Miradrain G100N or Delta Drain 6000, connected to the perimeter foundation drainage system. Imported granular materials, such as clean sand or OPSS Granular B Type I granular material, should otherwise be used for this purpose.

#### **Concrete Sidewalks and Walkways**

Backfill material below sidewalks and walkway subgrade areas throughout the subject site, including along the building, should be provided with a minimum 300 mm thick layer of OPSS Granular A or OPSS Granular B Type II crushed stone. This material should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 98% of the materials SPMDD. The subgrade for walkway structures against the building should be shaped to promote drainage towards the buildings perimeter drainage system.

### **6.2 Protection Against Frost Action**

Perimeter footings of heated structures are required to be insulated against the deleterious effect of frost action. A minimum of 1.5 m thick soil cover (or equivalent) should be provided in this regard.

Exterior unheated footings, such as those for isolated exterior piers, are more prone to deleterious movement associated with frost action than the exterior walls of the structure proper and require additional protection, such as soil cover of 2.1 m or a combination of soil cover and foundation insulation.

### **6.3 Excavation Side Slopes**

The side slopes of excavations in the overburden materials should either be cut back at acceptable slopes or should be retained by shoring systems from the start of the excavation until the structure is backfilled. It is assumed that sufficient room will be available for the greater part of the excavation to be undertaken by open-cut methods (i.e. unsupported excavations).

#### **Unsupported Excavations**

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be cut back at 1H:1V or flatter. The flatter slope is required for excavation below groundwater level. The subsoil at this site is considered to be mainly a Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress.

It is recommended that a trench box be used at all times to protect personnel working in trenches with steep or vertical sides. It is expected that services will be installed by “cut and cover” methods and excavations will not be left open for extended periods of time.

### **6.4 Pipe Bedding and Backfill**

Bedding and backfill materials should be in accordance with the most recent Material Specifications & Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Service Branch of the City of Ottawa.

The pipe bedding for sewer and water pipes should consist of at least 150 mm of OPSS Granular A material. The material should be placed in maximum 300 mm thick lifts and compacted to a minimum of 98% of its SPMDD. The bedding should extend at least to the spring line of the pipe.



The cover material, which should consist of OPSS Granular A, should extend from the spring line of the pipe to at least 300 mm above the top of the pipe. The material should be placed in maximum 300 mm thick lifts and compacted to 98% of the SPMDD.

It should generally be possible to re-use the site materials above the cover material if the operations are carried out in dry weather conditions.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to minimize differential frost heaving. The trench backfill should be placed in maximum 225 mm thick loose lifts and compacted to a minimum of 95% of the material standard Proctor maximum dry density.

## **6.5 Groundwater Control**

### **Groundwater Control for Building Construction**

Based on our observations, it is anticipated that groundwater infiltration into the excavations should be controllable using open sumps. Pumping from open sumps should be sufficient to control the groundwater influx through the sides of shallow excavations. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

### **Permit to Take Water**

A temporary Ministry of the Environment, Conservation and Parks (MECP) permit to take water (PTTW) may be required for this project if more than 400,000 L/day of ground and/or surface water is to be pumped during the construction phase. A minimum 4 to 5 months should be allowed for completion of the PTTW application package and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16.

However, based on the groundwater level readings at the borehole locations, as well as the observed colour, moisture content and consistency of the recovered soil samples, it is anticipated that the long-term groundwater levels are below the founding elevation of the proposed building. Therefore, an EASR or PTTW will not be required for dewatering during the construction of the proposed development.

## **6.6 Winter Construction**

Precautions must be taken if winter construction is considered for this project.

The subsoil conditions at this site mostly consist of frost susceptible materials. In the presence of water and freezing conditions, ice could form within the soil mass. Heaving and settlement upon thawing could occur.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters and tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

Trench excavations and pavement construction are also difficult activities to complete during freezing conditions without introducing frost in the subgrade or in the excavation walls and bottoms. Precautions should be taken if such activities are to be carried out during freezing conditions.

## **6.7 Corrosion Potential and Sulphate**

The results of analytical testing show that the sulphate content is less than 0.01%. This result is indicative that Type 10 Portland cement (normal cement) would be appropriate for this site. The chloride content and the pH of the sample indicate they are not significant factors in creating a corrosive environment for exposed ferrous metals at this site, whereas the resistivity is indicative of a non-aggressive to slightly aggressive corrosive environment.

## 7.0 Recommendations

It is a requirement for the foundation design data provided herein to be applicable that the following material testing and observation program be performed by the geotechnical consultant.

- Observation of all bearing surfaces prior to the placement of concrete.
- Sampling and testing of the concrete and fill materials.
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- Observation of all subgrades prior to backfilling.
- Field density tests to ensure that the specified level of compaction has been achieved.
- Sampling and testing of the bituminous concrete including mix design reviews.

All excess soils generated by construction activities should be handled as per *Ontario Regulation 406/19: On-Site and Excess Soil Management*.

A report confirming that these works have been conducted in general accordance with our recommendations could be issued upon the completion of a satisfactory inspection program by the geotechnical consultant.

## 8.0 Statement of Limitations

The recommendations provided are in accordance with the present understanding of the project. Paterson requests permission to review the recommendations when the drawings and specifications are completed.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, Paterson requests immediate notification to permit reassessment of our recommendations.

The recommendations provided herein should only be used by the design professionals associated with this project. They are not intended for contractors bidding on or undertaking the work. The latter should evaluate the factual information provided in this report and determine the suitability and completeness for their intended construction schedule and methods. Additional testing may be required for their purposes.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Stoked Industries Inc. or their agent(s) is not authorized without review by Paterson Group for the applicability of our recommendations to the altered use of the report.

### Paterson Group Inc.



September 14, 2023

Zubaida Al-Moselly, P.Eng.

Maha K. Saleh, M.A.Sc., P.Eng.

### Report Distribution:

- Stoked Industries Inc. (email copy)
- Paterson Group (1 copy)

# APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

SYMBOLS AND TERMS

ANALYTICAL TEST RESULTS

DATUM Geodetic

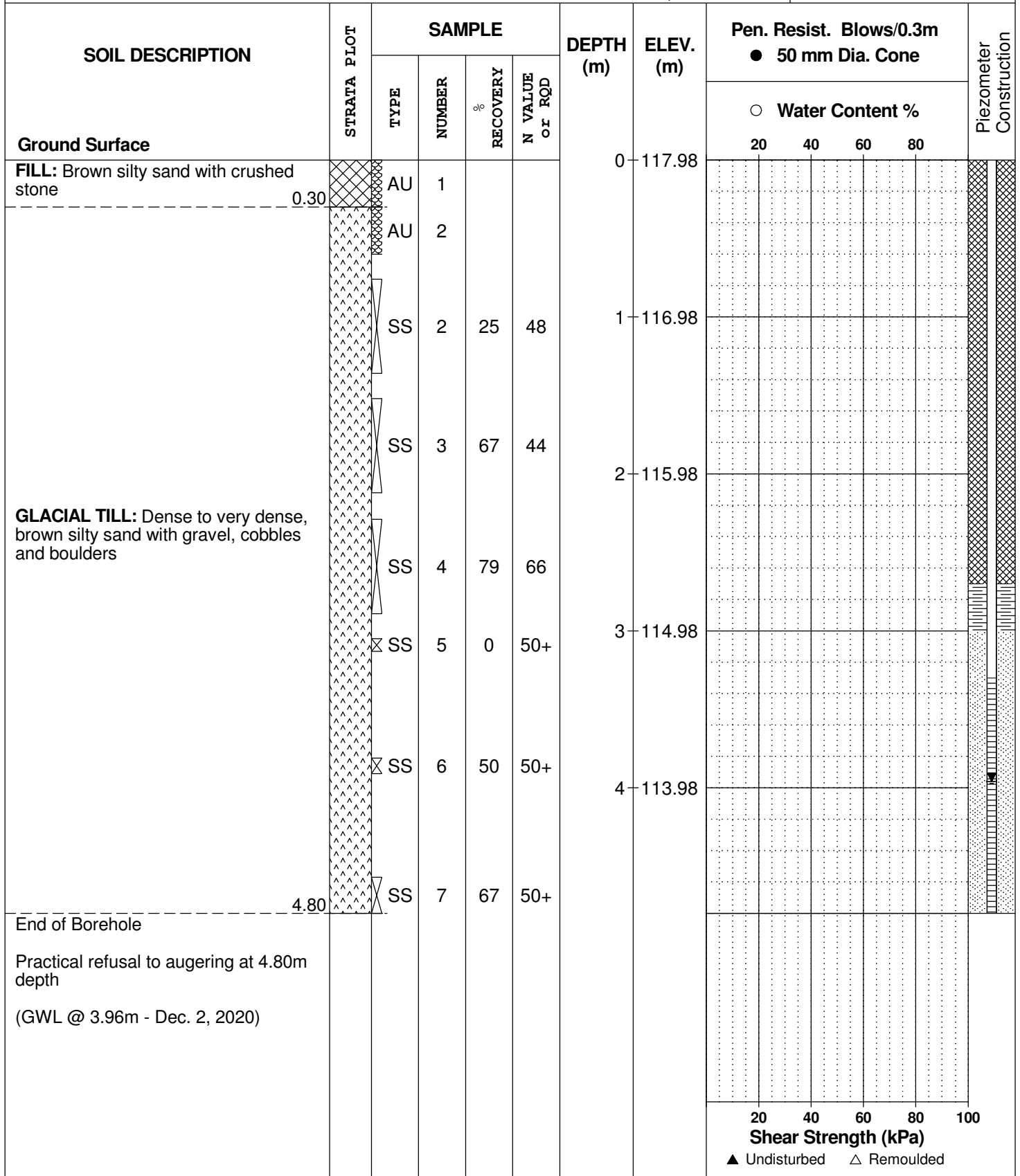
REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 1-20**



DATUM Geodetic

REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 2-20**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80	
<b>Ground Surface</b>												
<b>FILL:</b> Crushed stone with silty sand and gravel	0.30	AU	1			0	117.96					
		AU	2									
<b>GLACIAL TILL:</b> Compact to very dense, brown silty sand with gravel, cobbles and boulders		SS	3	75	28	1	116.96					
		SS	4	60	50+							
		SS	5	27	50+							
		SS	6		50+							
<b>End of Borehole</b>	2.82					2	115.96					
Practical refusal to augering at 2.82m depth  (Piezometer blocked and dry at 2.04m depth - Dec. 2, 2020)												

20 40 60 80 100  
**Shear Strength (kPa)**  
▲ Undisturbed    △ Remoulded

DATUM Geodetic

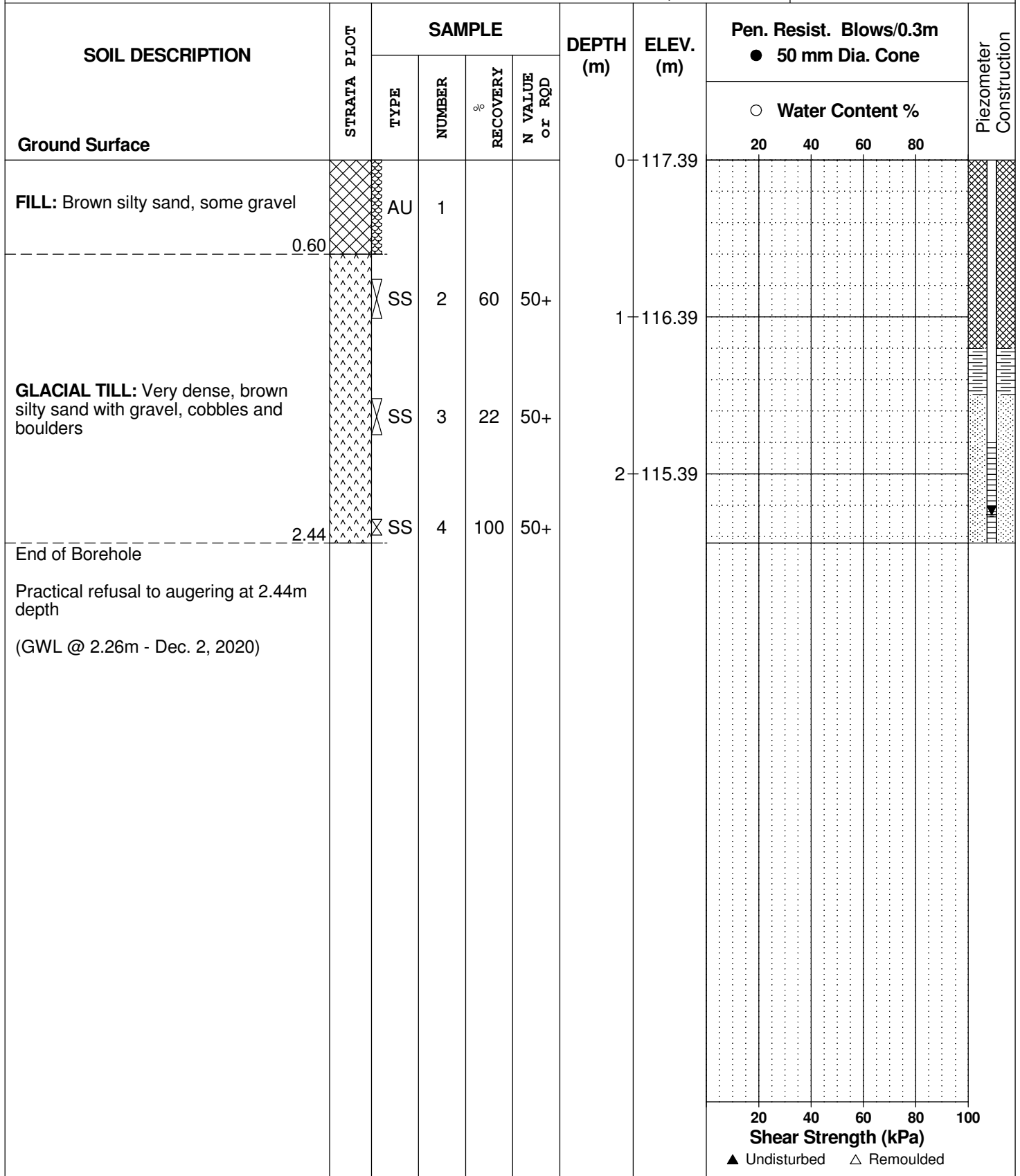
REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 3-20**





DATUM Geodetic



REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 4-20**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
Ground Surface								20	40	60	80		
<b>FILL:</b> Brown silty sand with crushed stone		AU	1			0	117.07						
	0.30	AU	2										
<b>GLACIAL TILL:</b> Compact to very dense, brown silty sand with gravel, cobbles and boulders		SS	3	50	28	1	116.07						
		SS	4	75	34								
		SS	5	50	50+	2	115.07						
	2.69												
End of Borehole													
Practical refusal to augering at 2.69m depth													
(GWL @ 0.94m - Dec. 2, 2020)													
								20	40	60	80	100	
								<b>Shear Strength (kPa)</b>					
								▲ Undisturbed    △ Remoulded					

## SOIL PROFILE AND TEST DATA

Geotechnical Investigation  
Proposed Building - 2167 McGee Side Road  
Ottawa, Ontario

DATUM Geodetic

REMARKS

FILE NO.  
**PG6662**

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

HOLE NO.  
**BH 5-20**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			○ Water Content %					
Ground Surface								20	40	60	80		
<b>FILL:</b> Brown silty sand, some gravel, trace organics	[Cross-hatched symbol]	AU	1			0	117.98						
0.60													
<b>GLACIAL TILL:</b> Compact to very dense, brown silty sand with gravel, cobbles and boulders	[Stippled symbol]	SS	2	83	17	1	116.98						
1.62													
End of Borehole		SS	3	100	50+								
Practical refusal to augering at 1.62m depth (GWL @ 1.33m - Dec. 2, 2020)													
								20	40	60	80	100	
								<b>Shear Strength (kPa)</b>					
								▲ Undisturbed    △ Remoulded					

DATUM Geodetic

REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 6-20**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
<b>Ground Surface</b>						0	117.23						
<b>FILL:</b> Crushed stone with silty sand, trace organics	0.30	AU	1										
<b>GLACIAL TILL:</b> Compact to very dense, light brown silty sand with gravel, cobbles and boulders		SS	2	67	26	1	116.23						
		SS	3	75	57								
		SS	4	73	50+	2	115.23						
<b>End of Borehole</b>	2.57												
Practical refusal to augering at 2.57m depth (GWL @ 2.09m - Dec. 2, 2020)													

20 40 60 80 100  
**Shear Strength (kPa)**  
▲ Undisturbed    △ Remoulded

DATUM Geodetic

REMARKS

BORINGS BY CME-55 Low Clearance Drill

DATE November 20, 2020

FILE NO.  
**PG6662**

HOLE NO.  
**BH 7-20**

SOIL DESCRIPTION	STRATA PLOT	SAMPLE				DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				Piezometer Construction	
		TYPE	NUMBER	RECOVERY %	N VALUE or RQD			20	40	60	80		
Ground Surface						0	118.08						
<b>GLACIAL TILL:</b> Dense to very dense, brown silty sand with gravel, cobbles and boulders		AU	1										
		AU	2										
		SS	3	92	34	1	117.08						
		SS	4	75	50+								
End of Borehole	2.06					2	116.08						
Practical refusal to augering at 2.06m depth  (Piezometer blocked and dry at 1.38m depth - Dec. 2, 2020)													

20 40 60 80 100  
**Shear Strength (kPa)**  
▲ Undisturbed    △ Remoulded

# SYMBOLS AND TERMS

## SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

## SYMBOLS AND TERMS (continued)

### SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

### ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

<b>RQD %</b>	<b>ROCK QUALITY</b>
90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

### SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard Penetration Test (SPT))
TW	-	Thin wall tube or Shelby tube
PS	-	Piston sample
AU	-	Auger sample or bulk sample
WS	-	Wash sample
RC	-	Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.

## SYMBOLS AND TERMS (continued)

### GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)
PL	-	Plastic limit, % (water content above which soil behaves plastically)
PI	-	Plasticity index, % (difference between LL and PL)
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size
D10	-	Grain size at which 10% of the soil is finer (effective grain size)
D60	-	Grain size at which 60% of the soil is finer
Cc	-	Concavity coefficient = $(D_{30})^2 / (D_{10} \times D_{60})$
Cu	-	Uniformity coefficient = $D_{60} / D_{10}$

Cc and Cu are used to assess the grading of sands and gravels:

Well-graded gravels have:  $1 < Cc < 3$  and  $Cu > 4$

Well-graded sands have:  $1 < Cc < 3$  and  $Cu > 6$

Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded.

Cc and Cu are not applicable for the description of soils with more than 10% silt and clay (more than 10% finer than 0.075 mm or the #200 sieve)

### CONSOLIDATION TEST

$p'_o$	-	Present effective overburden pressure at sample depth
$p'_c$	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below $p'_c$ )
Cc	-	Compression index (in effect at pressures above $p'_c$ )
OC Ratio		Overconsolidation ratio = $p'_c / p'_o$
Void Ratio		Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

### PERMEABILITY TEST

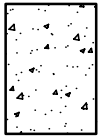
k	-	Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.
---	---	--

## SYMBOLS AND TERMS (continued)

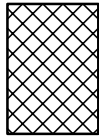
### STRATA PLOT



Topsoil



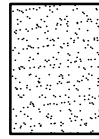
Asphalt



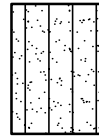
Fill



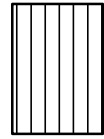
Peat



Sand



Silty Sand



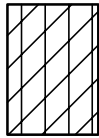
Silt



Sandy Silt



Clay



Silty Clay



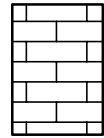
Clayey Silty Sand



Glacial Till



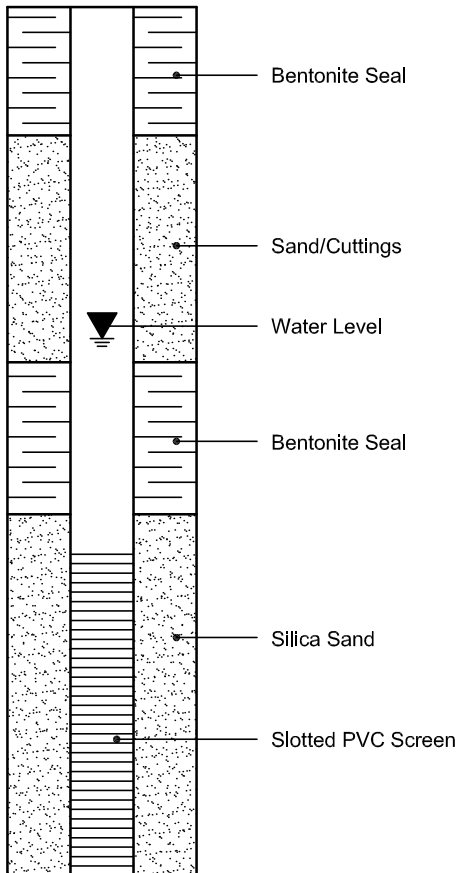
Shale



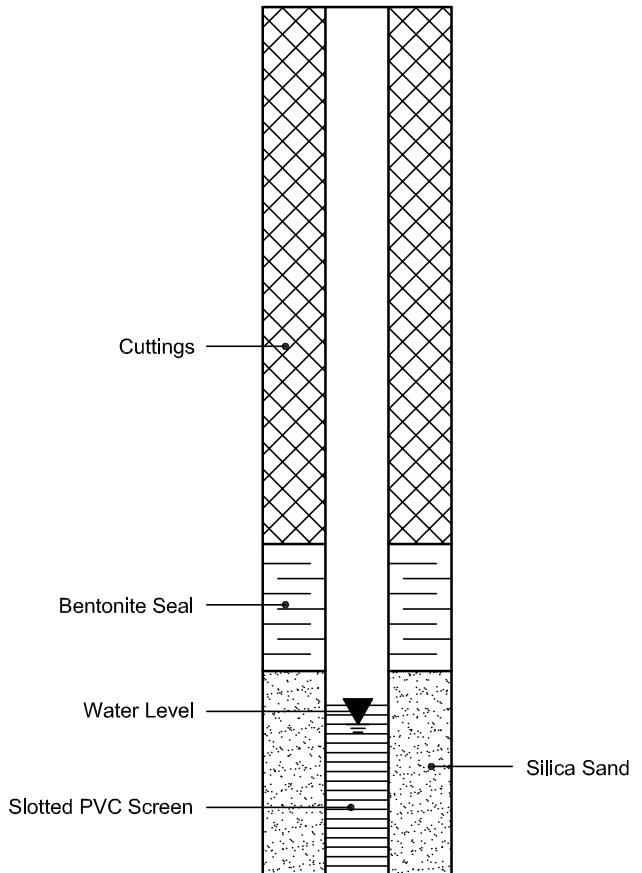
Bedrock

### MONITORING WELL AND PIEZOMETER CONSTRUCTION

#### MONITORING WELL CONSTRUCTION



#### PIEZOMETER CONSTRUCTION





Certificate of Analysis

Report Date: 27-Nov-2020

Client: Paterson Group Consulting Engineers

Order Date: 20-Nov-2020

Client PO: 31283

Project Description: PG5602

<b>Client ID:</b>	BH3-20-SS2	-	-	-
<b>Sample Date:</b>	20-Nov-20 12:00	-	-	-
<b>Sample ID:</b>	2047669-01	-	-	-
<b>MDL/Units</b>	Soil	-	-	-

**Physical Characteristics**

% Solids	0.1 % by Wt.	91.3	-	-	-
----------	--------------	------	---	---	---

**General Inorganics**

pH	0.05 pH Units	7.75	-	-	-
Resistivity	0.10 Ohm.m	78.8	-	-	-

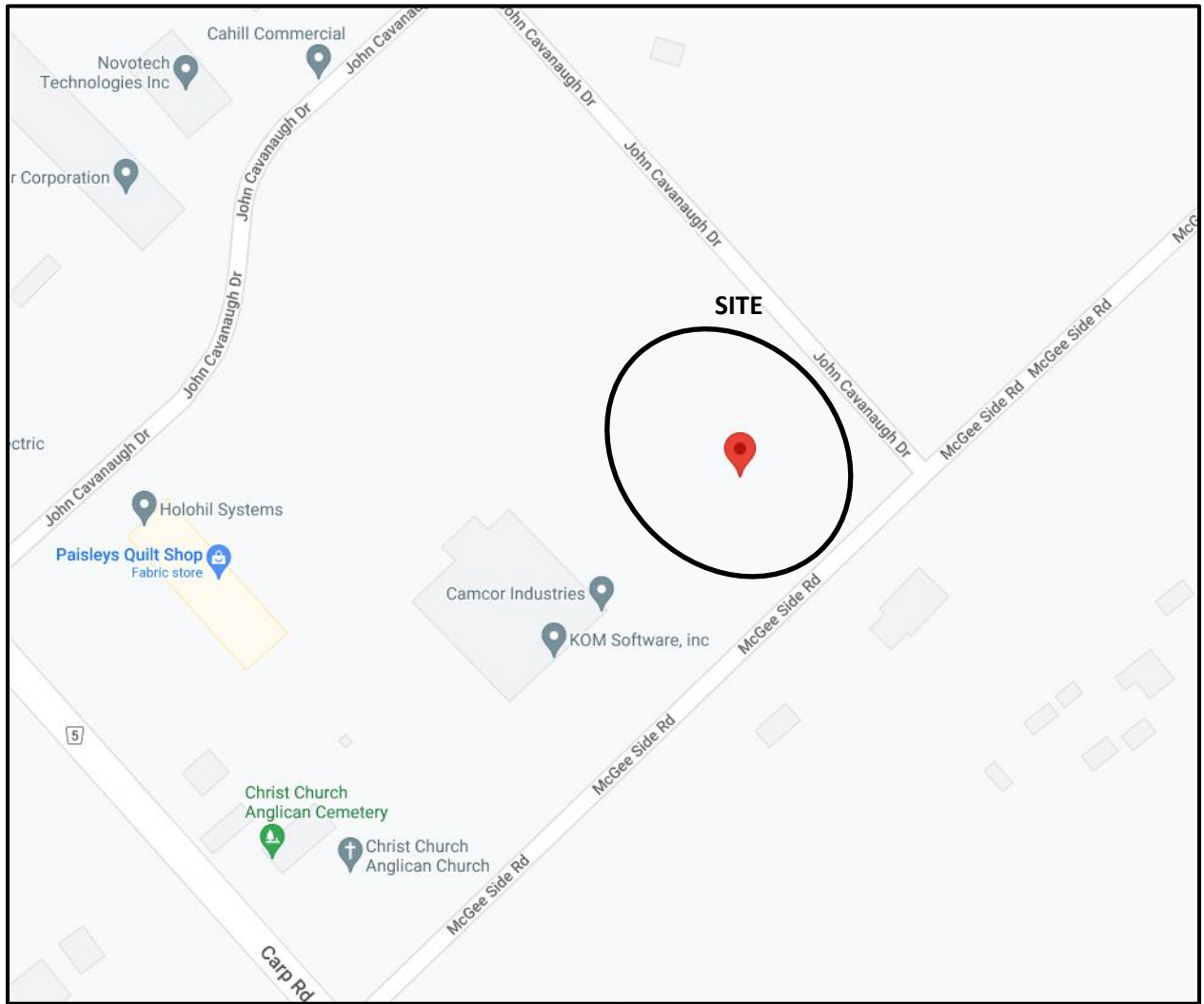
**Anions**

Chloride	5 ug/g dry	9	-	-	-
Sulphate	5 ug/g dry	13	-	-	-

# APPENDIX 2

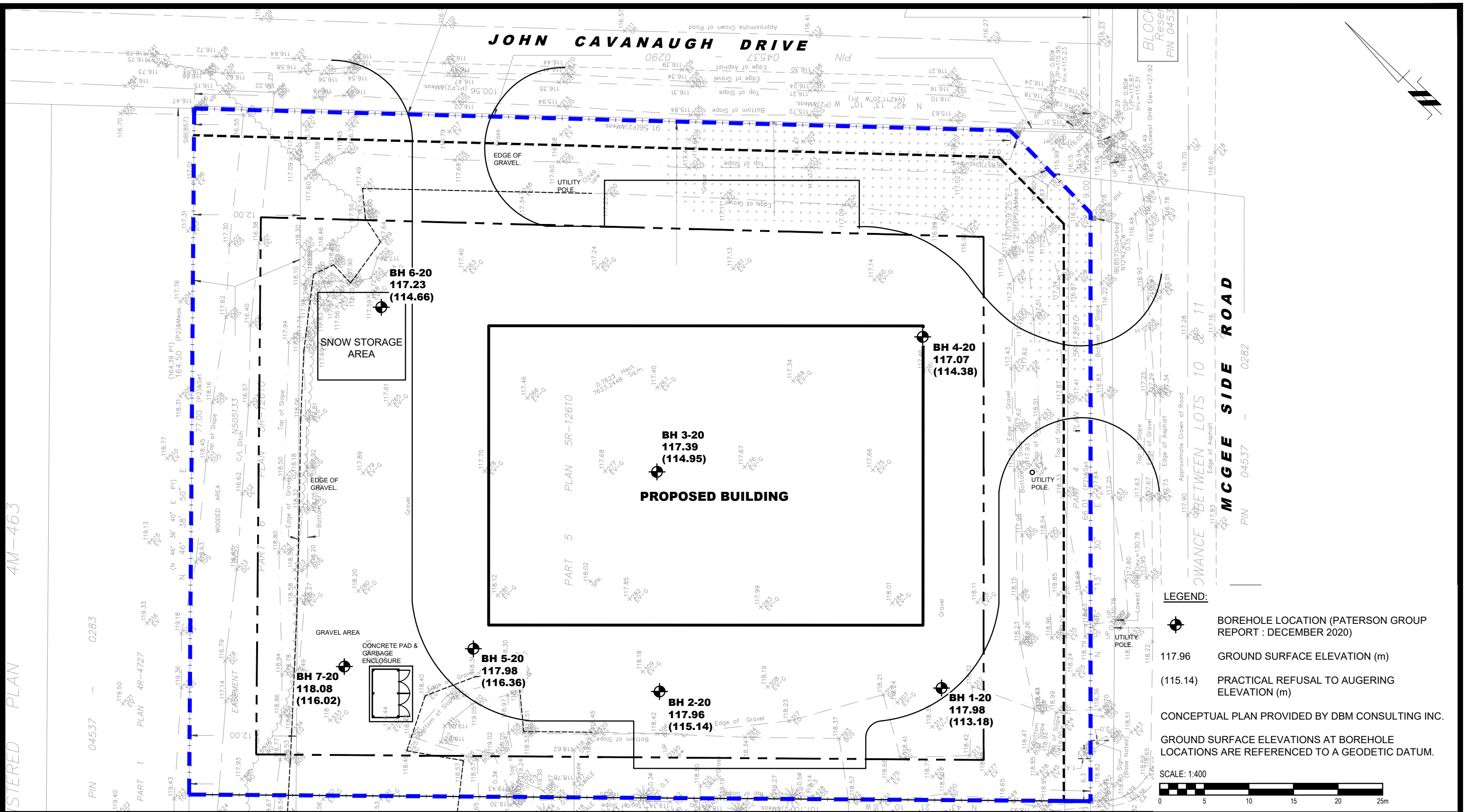
FIGURE 1 – KEY PLAN

DRAWING PG6662-1 – TEST HOLE LOCATION PLAN



# FIGURE 1

## KEY PLAN



**LEGEND:**

- BOREHOLE LOCATION (PATERSON GROUP REPORT : DECEMBER 2020)
- 117.96 GROUND SURFACE ELEVATION (m)
- (115.14) PRACTICAL REFUSAL TO AUGERING ELEVATION (m)

CONCEPTUAL PLAN PROVIDED BY DBM CONSULTING INC.

GROUND SURFACE ELEVATIONS AT BOREHOLE LOCATIONS ARE REFERENCED TO A GEODETIC DATUM.

SCALE: 1:400

9 AURIGA DRIVE  
OTTAWA, ON  
K2E 7T9  
TEL: (613) 226-7381

NO.	REVISIONS	DATE	INITIAL

**STOKED INDUSTRIES INC.**  
**GEOTECHNICAL INVESTIGATION**  
**PROPOSED BUILDING**  
**2167 MCGEE SIDE ROAD**  
**ONTARIO**

**CARP,**  
**Title:**  
**TEST HOLE LOCATION PLAN**

Scale:	1:400	Date:	05/2023
Drawn by:	YA	Report No.:	PG6662-1
Checked by:	ZA	Dwg. No.:	<b>PG6662-1</b>
Approved by:	MAS	Revision No.:	