951 Gladstone Avenue and 145 Loretta Avenue North Transportation Impact Assessment

Step 1 Screening Report
Step 2 Scoping Report
Step 3 Forecasting Report
Step 4 Strategy Report

Prepared for:

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Prepared by:



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PN: 2020-25

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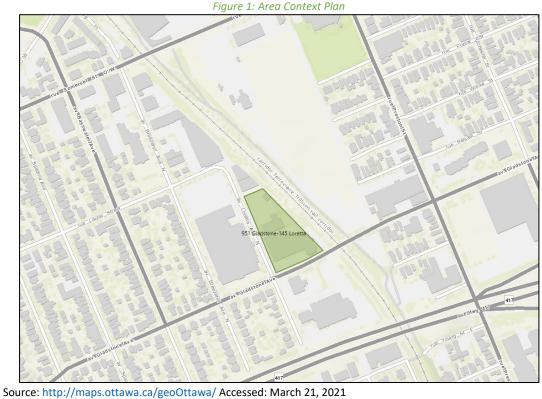
Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines. A TIA report was submitted and approved for the zoning bylaw amendment in 2019. This report provides an update on the existing conditions, forecasting and network impact component and the submission of the design review component. Accordingly, the Step 1 Screening Form has been revised and is included as Appendix A, along with the Certification Form for TIA Study PM. This TIA will support the site plan application.

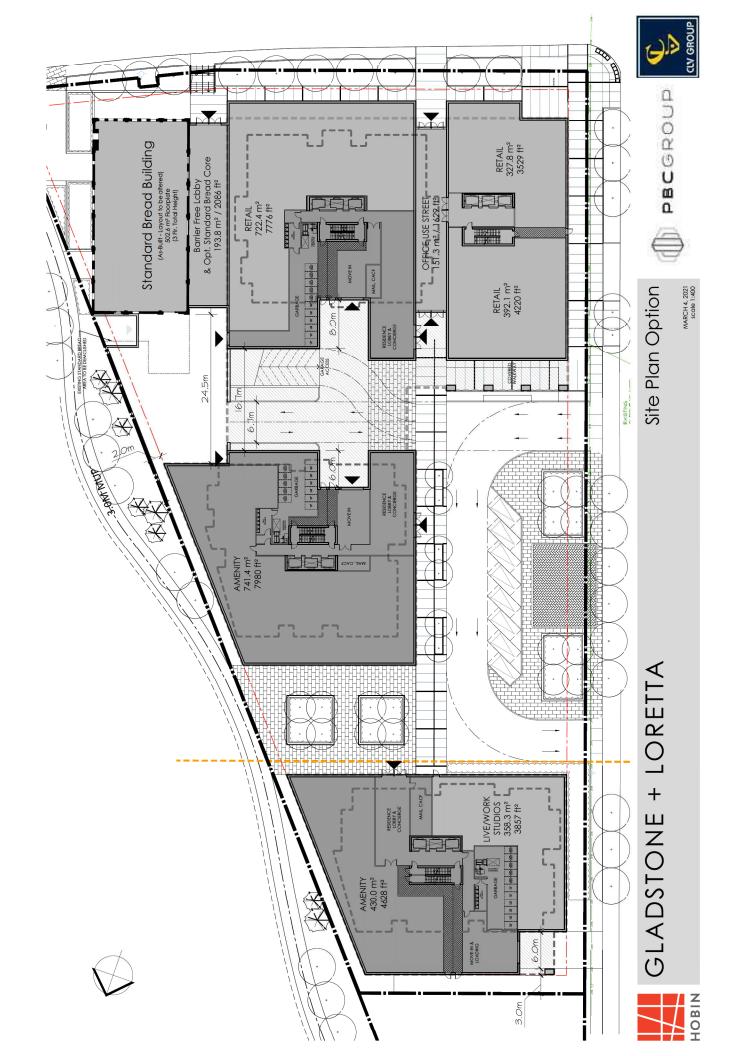
Existing and Planned Conditions

2.1 Proposed Development

The proposed development is located at 951 Gladstone Avenue and 145 Loretta Avenue North and has undergone a rezoning amendment to rezone the general industry (IG) zoning to mixed-use centre (MC). The existing land uses include brewery, jujitsu club, cross-fit gym, glass blowing, art studio, beer and wine supply, bread bakery, and other assorted industrial uses. The proposed site plan application consists of approximately 846 residential units, 198,165 sq. ft of office space (including live-work space), and 17,611 sq. ft. of retail space (including the existing Standard Bread building). A total of 570 parking spaces will be provided (30 are visitor spaces), with 8 at ground level and the remaining 562 in the two levels of underground parking. The existing accesses will be removed from both properties and a one-way general access loop along Loretta Avenue North will be provided for access to the underground garage. A loading/move-in areas will be provided adjacent to the underground parking ramp, and an additional loading/move-in access will be provided north of the one-way general access loop. The frontage along Loretta Avenue North would formalize the curb edge and remove the paved shoulder and open access along the building frontage. The anticipated full build-out and occupancy horizon is 2026. Figure 1 illustrates the Study Area Context. Figure 2 illustrates the proposed concept plan.







2.2 Existing Conditions

2.2.1 Area Road Network

Preston Street: Preston Street is a City of Ottawa arterial road with a two-lane urban cross-section, including parking lanes and auxiliary lanes at major intersections. The unposted speed limit is 50 km/h, and the Ottawa Official Plan reserves a 23.0 metre right-of-way.

Somerset Street West: Somerset Street West is a City of Ottawa arterial road with a two-lane cross-section, including sidewalks and on street parking. The unposted speed limit is 50 km/h, and the right-of-way is 20.0 metres. East of Breezehill Avenue, bike lanes are provided.

Gladstone Avenue: Gladstone Avenue is a City of Ottawa major collector road with a two-lane urban cross-section including sidewalks and a posted speed limit of 40 km/h. The current right-of-way is 20.0 metres, with additional width provided in proximity to the rail corridor.

Bayswater Avenue: Bayswater Avenue is a City of Ottawa collector road with a two-lane urban cross-section, including sidewalks and on-street parking. The unposted speed limit is 50 km/h, and the right-of-way is 25.0 metres.

Loretta Avenue North/Laurel Street: Loretta Avenue N is a City of Ottawa local road with a two-lane urban cross-section including paved shoulders on the east side and a sidewalk on the west side. The posted speed is 40 km/h, and the right-of-way is 20.0 metres.

Breezehill Avenue: Breezehill Avenue is a City of Ottawa local road with a two-lane urban cross-section, including sidewalks, and parking on the east side of the road. The posted speed limit is 40 km/h, and the right-of-way is 20.0 metres.

2.2.2 Existing Intersections

The existing signalized area intersections within one kilometre of the site have been summarized below:

Gladstone Avenue & Bayswater Avenue	The intersection of Gladstone Avenue and Bayswater
	Avenue is a signalized intersection with shared all
	movement lanes on each approach. No turn restrictions
	. 1

were noted.

Gladstone Avenue & Preston Street The intersection of Gladstone Avenue and Preston Street

is a signalized intersection with auxiliary left-turn lanes on the northbound, westbound, and southbound

approaches. No turn restrictions were noted.

Somerset Street West & Breezehill Avenue The intersection of Somerset Street West and Breezehill

Avenue is a minor stop-controlled intersection with shared movement lanes on all approaches. Bike lanes along Somerset Street West start/end on the east side of the

intersection. No turn restrictions were noted.

Gladstone Avenue & Loretta Avenue North

The intersection of Gladstone Avenue and Loretta Avenue

N is a minor stop-controlled intersection with shared movement lanes on all approaches. No turn restrictions

were noted.



Laurel Street & Breezehill Avenue

The intersection of Laurel Street and Breezehill Avenue is an all-way stop-controlled intersection with shared movement lanes on all approaches. No turn restrictions were noted.

2.2.3 Existing Driveways

Along Gladstone Avenue, a driveway to the City of Ottawa yard (175 Loretta Avenue North) is located opposite the existing Standard Bread access adjacent to the Trillium Rail Corridor, and an access to 950 Gladstone Avenue within 5.0 metres of the Loretta Avenue North intersection. Between Loretta Avenue North and Breezehill Avenue, an access loop is located on the north side of Gladstone Avenue to the Canadian Bank Note Limited, and five driveways are located on the south side.

Along Loretta Avenue North, two accesses are provided on the west side of the road for the Canadian Bank Note Limited site, and a single access is located north of the proposed site for 131 Loretta Avenue North. The paved shoulder is used for perpendicular parking along Loretta Avenue North as well.

2.2.4 Cycling and Pedestrian Facilities

Figure 3 illustrates the pedestrian facilities in the study area and Figure 4 illustrates the cycling facilities. Figure 5 and Figure 6 illustrate the existing pedestrian and cycling volumes within the study area.

Sidewalks are provided along both sides of the roadways in the study area with the exception of the east side of Breezehill Avenue between Gladstone Avenue and Laurel Street, on both sides of Laurel Street, and the east side of Loretta Avenue North. The Trillium Pathway is a multi-use pathway along the east side of the Trillium Rail Corridor.

The cycling network consists of the Trillium Pathway as a cross-town bikeway, suggested biking routes along Gladstone Avenue, Bayswater Avenue and Somerset Street W, including bike lanes on the bridge over the Trillium Rail Corridor.

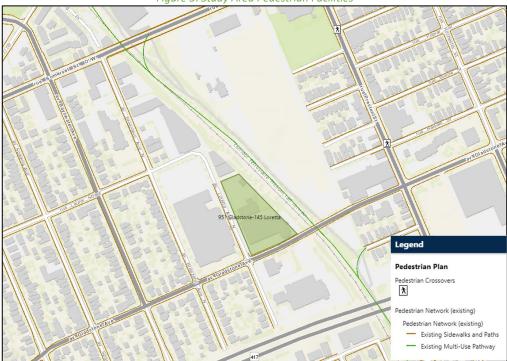


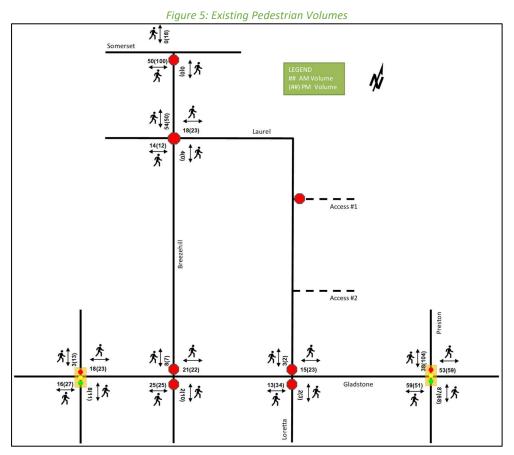
Figure 3: Study Area Pedestrian Facilities



Source: http://maps.ottawa.ca/geoOttawa/ Accessed: March 31, 2021



Source: http://maps.ottawa.ca/geoOttawa/ Accessed: March 31, 2021



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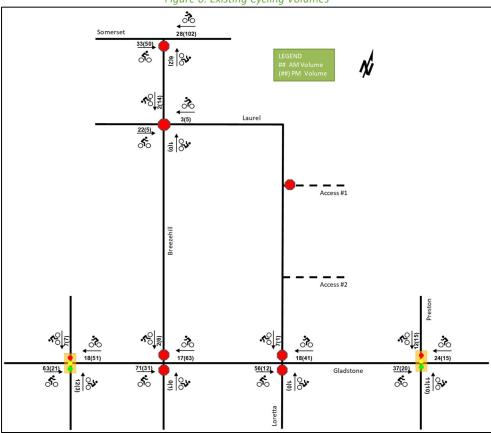


Figure 6: Existing Cycling Volumes

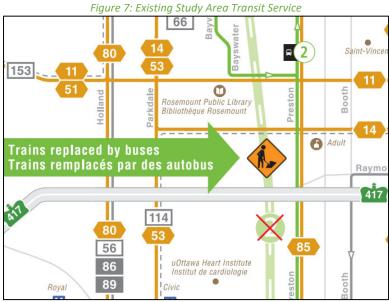
2.2.5 Existing Transit

Within the study area, the routes #11, #14 and #114 run along Gladstone Avenue, Somerset Street and Preston Street. The frequency of these routes within proximity of the proposed site currently are:

- Route #11 every 10-15 minutes during the day and 20-30 minutes during the evening
- Route # 14 every 15-20 minutes during the day and 20-30 minutes during the evening
- Route#85 every 10-20 minutes during the day and 20-30 minutes during the evening
- Route # 114 two trips during the AM peak to Rideau and two trips during the PM peak to Caldwell/Merivale

Figure 7 illustrates the transit system map in the study area and Figure 8 illustrates nearby transit stops.





Source: http://www.octranspo.com/ Accessed: March 16, 2021

Beaver Boxing Club lant Recreation Centre Centretown Veterinary Hospital 0 Plant & CuO Dollarama 😜 Sala San Marco Event 0 Happy Goat Coffee Co. (Laurel) 0 00 Adult H Vimy Brewing Company O Yogatown Q ranaShanti Yoga Centre 8 Starbucks iovanni's

Figure 8: Existing Study Area Transit Stops

Source: http://www.octranspo.com/ Accessed: March 16, 2021

2.2.6 Existing Area Traffic Management Measures

There are no existing area traffic management measures within the Study Area.

2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa and new traffic count surveys for the existing Study Area intersection. Table 1 summarizes the intersection count dates and sources.

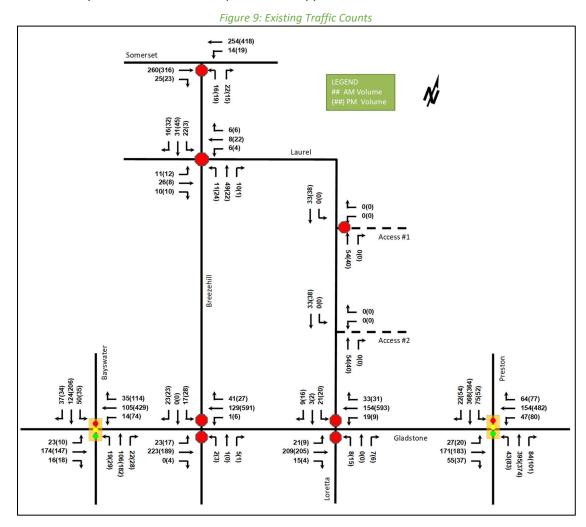
Table 1: Intersection Count Date

rabic 21 intersection country at				
Intersection	Count Date	Source		
Gladstone Avenue and Bayswater Avenue	Wednesday July 27, 2016	City of Ottawa		
Gladstone Avenue and Breezehill Avenue	Wednesday, July 18, 2018	City of Ottawa		



Intersection	Count Date	Source
Gladstone Avenue and Loretta Avenue	Tuesday, April 23, 2019	The Traffic Specialist
Gladstone Avenue and Preston Street	Tuesday June 20,2017	City of Ottawa
Laurel Street and Breezehill Avenue	Tuesday, April 23, 2019	The Traffic Specialist
Somerset Street West and Breezehill Avenue	Thursday August 12, 2015	City of Ottawa

Figure 9 illustrates the existing traffic counts and Table 2 summarizes the existing intersection operations. The level of service for signalized intersections is based on HCM 2010 calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The volumes have been balanced through the network. Detailed turning movement count data is included in Appendix B and the Synchro worksheets are provided in Appendix C.



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Table 2: Existing Intersection Operations

1			AM Peak Hour		PM Peak Hour				
Intersection	Lane	LOS	V/C	Delay	Q (95th)	LOS	V/C	Delay	Q (95th)
	EB	С	0.78	38.0	53.6	Α	0.50	19.0	46.0
	WBL	Α	0.29	25.3	13.7	Α	0.23	16.0	17.5
	WBT/R	В	0.62	27.7	43.1	Е	0.91	40.2	#140.8
Gladstone Avenue	NBL	Α	0.11	8.2	7.6	Α	0.44	22.6	20.5
& Preston Street	NBT/R	Α	0.54	11.5	68.1	С	0.76	24.4	85.2
Signalized	SBL	Α	0.22	9.8	12.8	Α	0.29	17.7	12.7
	SBT/R	Α	0.42	10.0	51.2	В	0.66	21.0	71.1
	Overall	В	0.60	18.2	_	С	0.83	27.0	_
	EB	Α	0.02	7.8	0.8	Α	0.01	9.2	0.0
Gladstone Avenue	WB	Α	0.02	7.8	0.8	Α	0.01	7.9	0.0
& Loretta Avenue	NB	В	0.03	11.9	0.8	С	0.09	20.3	2.3
North <i>Unsignalized</i>	SB	В	0.08	12.9	1.5	С	0.15	20.3	3.8
Unsignalizea	Overall	Α	-	1.8	-	Α	-	1.5	-
Cladatana Avanua	EB	Α	0.02	7.7	0.8	Α	0.02	9.2	0.8
Gladstone Avenue & Breezehill	WB	Α	0.00	7.8	0.0	Α	0.01	7.8	0.0
Avenue	NB	В	0.02	11.2	0.0	С	0.02	19.6	0.8
Unsignalized	SB	В	0.07	11.3	1.5	С	0.20	20.9	5.3
Onsignanzea	Overall	Α	-	1.5	-	Α	-	1.5	-
Gladstone Avenue	EB	Α	0.31	11.3	27.8	Α	0.23	8.5	19.8
& Bayswater	WB	Α	0.23	8.9	18.4	D	0.85	25.5	#121.1
Avenue	NB	Α	0.28	13.8	22.9	Α	0.52	20.7	41.7
Signalized	SB	Α	0.42	16.0	33.3	В	0.61	23.0	49.3
	Overall	В	0.35	12.7	-	С	0.75	21.8	-
Somerset Street W	EB	-	-	-	-	-	-	-	-
& Breezehill	WB	Α	0.01	8.2	0.0	Α	0.02	8.7	0.8
Avenue	NB	В	0.08	12.5	2.3	С	0.12	17.7	3.0
Unsignalized	Overall	Α	-	1.0	-	Α	-	1.0	-
	EB	Α	0.06	7.5	1.5	Α	0.04	7.3	0.8
Laurel Street &	WB	Α	0.03	7.3	0.8	Α	0.04	7.4	0.8
Breezehill Avenue	NB	Α	0.09	7.5	2.3	Α	0.06	7.5	1.5
Unsignalized	SB	Α	0.09	7.5	2.3	Α	0.10	7.3	2.3
	Overall	Α	-	7.5	-	Α	-	7.4	-

Notes: Saturation flow rate of 1800 veh/h/lane

PHF = 0.90

m = metered queue

= queue exceeds storage or mid-block length

During both the AM and PM peak hours, the study area intersection operates well. No capacity issues are noted. During the PM peak, the westbound queues at the Gladstone Avenue and Preston Street intersection may extend beyond the mid-block and to Rochester Street, and at the Gladstone Avenue and Bayswater Avenue intersection may extend beyond the Breezehill Avenue intersection.

2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study are road network. Table 3 summarizes the collision types and conditions in the study area, Figure 10 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix D.



Table 3: Study Area Collision Summary, 2015-2019

		Number	%
Total Co	ollisions	72	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	24	33%
	Total Collisions 72 Inn Fatality 0 Non-Fatal Injury 24 Property Damage Only 48 Approaching 1 Angled 17 Rear end 15 Sideswipe 9 Turning Movement 13 SMV Unattended 8 SMV Other 8 Other 1 Dry 45 Wet 14 Loose Snow 4 Slush 5 Packed Snow 1 Ice 3 Involved 7	67%	
	Approaching	1	1%
	Angled	17	24%
	Rear end	15	21%
Initial Impact Type	Sideswipe	9	13%
Initial Impact Type	Turning Movement	13	18%
	SMV Unattended	8	11%
	SMV Other	8	11%
	Other	1	1%
	Dry	45	63%
	Wet	14	19%
Road Surface Condition	Loose Snow	4	6%
Road Surface Condition	Slush	5	7%
	Packed Snow	1	1%
	Ice	3	4%
Pedestrian Involved		7	10%
Cyclists Involved		6	8%

Figure 10: Study Area Collision Records – Representation of 2015-2019

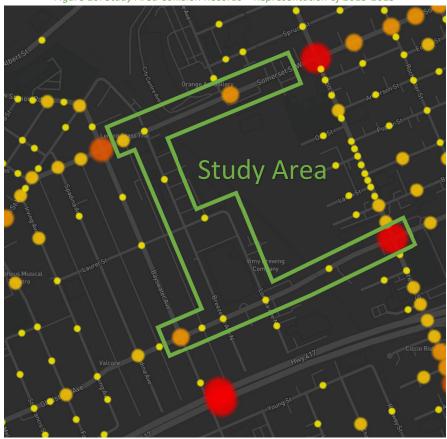




Table 4: Summary of Collision Locations, 2015-2019

	Number	%
Intersections / Segments	72	100%
Gladstone Avenue @ Bayswater Avenue	15	21%
Gladstone Avenue @ Breezehill Avenue	1	1%
Gladstone Avenue @ Loretta Avenue	2	3%
Gladstone Avenue @ Preston Street	23	32%
Somerset Street @ Breezehill Avenue	1	1%
Gladstone Avenue btwn Bayswater Avenue & Breezehill Avenue N	2	3%
Gladstone Avenue btwn Loretta Avenue N & Preston Street	3	4%
Somerset Street W btwn Bayswater Avenue & Breezehill Avenue N	7	10%
Somerset Street W btwn Breezehill Avenue N & Preston Street	14	19%
Breezehill Avenue N btwn Somerset Street W & Laurel Street	1	1%
Laurel Street btwn Breezehill Avenue N & Loretta Avenue N	2	3%
Loretta Avenue N btwn Gladstone Avenue & End	1	1%

Within the study area, the intersection of Gladstone Avenue at Bayswater Avenue and Gladstone Avenue at Preston Street, and the segment of Somerset Street W between Breezehill Avenue and Preston Street are noted to have experienced higher collisions than other intersections. Table 5, Table 6 and Table 7 summarize the collision types and conditions for each of these locations.

Table 5: Gladstone Avenue and Bayswater Avenue Collision Summary

		Number	%
Total Co	ollisions	15	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	5	33%
	Property Damage Only	10	67%
	Angle	6	40%
Initial Impact Type	Rear end	4	27%
	Turning Movement	3	20%
	SMV Other	2	13%
	Dry	9	60%
Road Surface Condition	Wet	3	20%
Road Surface Condition	Loose Snow	2	13%
	Packed Snow	1	7%
Pedestrian Involved		2	13%
Cyclists Involved		0	0%

The Gladstone Avenue and Bayswater Avenue intersection had a total of 15 collisions during the 2015-2019 time period, with ten involving property damage only and the remaining five having non-fatal injuries. The collision types are most represented by angled (six collisions) and rear-end (four collisions) and the remaining split between turning movement and single motor vehicle. The angled collisions may be subject to weather conditions as two are loose snow and one was wet weather conditions. No other patterns are noted within the collision data and no mitigation is recommended. Weather conditions do not affect collisions at this location.

Table 6: Gladstone Avenue and Preston Street Collision Summary

		Number	%
Total Collisions		23	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	7	30%
	Property Damage Only	16	70%



		Number	%
To	otal Collisions	23	100%
	Angle	1	4%
Initial Impact	Rear end	10	43%
Initial Impact Type	Sideswipe	4	17%
	Turning Movement	3	13%
	SMV Other	5	22%
	Dry	14	61%
Road Surface Condition	Wet	4	17%
	Loose Snow	1	4%
Condition	Slush	2	9%
	Ice	2	9%
Pedestrian Involved		4	17%
Cyclists Involved		1	4%

The Gladstone Avenue and Preston Street intersection had a total of 23 collisions during the 2015-2019 time period, with 16 involving property damage only and the remaining seven having non-fatal injuries. The collision types are most represented by rear-end (ten collisions), SMV other (five collisions), and sideswipe, turning movement and angled with four or less each. No patterns are noted within the collision data and no mitigation is recommended. Weather conditions do not affect collisions at this location.

Table 7: Somerset Street W between Breezehill Avenue and Preston Street Collision Summary

		Number	%
To	tal Collisions	14	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	6	43%
	Property Damage Only	8	57%
	Angle	4	29%
Initial Impact	Sideswipe	3	21%
Туре	Turning Movement	4	29%
	SMV Unattended	3	21%
Road Surface	Dry	11	79%
Condition	Wet	3	21%
Pedestrian Involv	ved	0	0%
Cyclists Involved		5	36%

The Somerset Street W segment between Breezehill Avenue and Preston Street had a total of 14 collisions during the 2015-2019 time period, with eight involving property damage only and the remaining six having non-fatal injuries. The collision types are evenly distributed with angled and turning movements with four each, and sideswipe and SMV unattended with three each. The collisions are assumed to be a result of the on-street parking and commercial/retail access located on the east side of the Trillium Line overpass, through a combination of parked cars, dooring, loading vehicles, infrequent access traffic and the Plant Recreation Centre access. This area also has shared road cycling facilities, where as the overpass has bike lanes west to Breezehill Avenue, and likely contributes to the cycling collision frequency of approximately one per year. Weather conditions do not affect collisions at this location.



2.3 Planned Conditions

2.3.1 Changes to the Area Transportation Network

The subject development is within the Gladstone Station District CDP (2014) and as such, is subject to the development and planning vision outlined with the CDP. The CDP visioning option for the transit-oriented development node, illustrated in Figure 11, has the following new transportation infrastructure elements:

- Trillium LRT station plaza identified as a node/landmark/gateway for the community
- a multi-use crossing is proposed over the rail line between Gladstone Avenue and Laurel Street W
- a new road connection across the rail line between Laurel Street W and Oak Street

Beyond the station plaza, these improvements are not identified in the City's affordable network and not time frame is available for their construction.



Figure 11: Gladstone Station District CDP Vision & Concept Options Report - Preferred Option: Perspective Looking North-East

2.3.2 Other Study Area Developments

1040 and 1050 Somerset Street

The combined site would include a 32-storey residential building between the Trillium Rail corridor and Breezehill Avenue, and a 23-storey residential building on the west side of Breezehill Avenue. Both sites would include ground floor commercial/retail and provide underground parking. Access to the 1040 site was proposed along Breezehill Avenue and a laneway access on Somerset Street West was proposed for the 1050 site. This application file has not advanced since 2013.

989 Somerset Street

The proposed development consists of a mixed-use building with ground floor retail and 127 residential units above. The transportation impact of this site will be primarily on Somerset Street West and Preston Street. In addition, this file has not advanced since 2014.

975 Gladstone Avenue



An addition to the existing Canada Bank Note building is proposed on the rear of the building, consisting of 947m² warehousing space and a 177m² secure interior loading bay. The planning rationale states that the addition supports the existing light industrial business and not generate any increase of vehicular trips.

139-143 Balsam Street and 20 Larch Street

The proposed demolish control applications was submitted to remove the existing buildings on site. The properties would remain in an interim condition until such time redevelopment occurs in accordance with the Gladstone Station CDP.

3 Study Area and Time Periods

3.1 Study Area

The study area will include the intersections of:

- Gladstone Avenue at:
 - Bayswater Avenue
 - o Breezehill Avenue
 - Loretta Avenue
 - o Preston Street
- Breezehill Avenue at:
 - Laurel Street
 - Somerset Street
- Loretta Avenue at:
 - Site Access #1 (outbound)
 - Site Access #2 (inbound)

The boundary roads will be Gladstone Avenue and Loretta Avenue. The TRANS screenline SL-29 was reviewed during the zoning bylaw amendment application and no issues were noted. No additional screenline analysis will be provided within this TIA.

3.2 Time Periods

As the proposed development is composed predominantly by residential and office, the AM and PM peak hours will be examined.

3.3 Horizon Years

The anticipated build-out year is 2026. As a result, the full build-out plus five years horizon year is 2031.

4 Exemption Review

Table 8 summarizes the exemptions for this TIA.

Table 8: Exemption Review

		rable 6. Exemption neview						
Module	Element	Explanation	Exempt/Required					
Design Review Component								
4.1 Development	4.1.2 Circulation and Access	Only required for site plans	Required					
Design	4.2.3 New Street Networks	Only required for plans of subdivision	Exempt					
4.2 Parking	4.2.1 Parking Supply	Only required for site plans	Required					



Module	Element	Explanation	Exempt/Required
	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
Network Impact Comp	onent		
4.5 Transportation Demand Management	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Required
4.8 Network Concept		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of equivalent volume permitted by established zoning	Exempt

5 Development-Generated Travel Demand

5.1 Trip Generation and Mode Shares

This TIA has been prepared using the vehicle and person trip rates for the residential components using the TRANS Trip Generation Study Report (2009) and person trip rates for general office and the vehicle trip rates for the retail components using the ITE Trip Generation Manual (10th Edition). To estimate person trip generation for the office and retail components, a factor of 1.28 has been applied to the ITE rates. Table 9 summarizes the person trip rates for the proposed land use.

Table 9: Trip Generation Person Trip Rates

Dwelling Type	Land Use	Peak	Vehicle	Person	Note
	Code	Hour	Trip Rate	Trip Rate	
High-rise condominiums (10+ floors)	232	AM	0.24	0.65	Urban
	(TRANS)	PM	0.27	0.68	Orbail
General Office (Dense Multi-Use)	710	AM	0.84	1.08	Fitted curve
General Office (Delise Multi-Ose)	710	PM	0.88	1.12	ritted curve
Shonning Contro (Urban Suburban)	820	AM	0.94	1.20	Average rate, low studies
Shopping Centre (Urban-Suburban))		PM	3.81	4.88	for Dense Multi-Use

Using the above Person Trip rates, the total person trip generation has been estimates. Table 10 below illustrates the total person trip generation for the proposed land-uses.

Table 10: Total Person Trip Generation

Landllan	Units /		AM Peak Hou	r	PM Peak Hour				
Land Use	GFA	In	Out	Total	In	Out	Total		
High-rise condominiums	846	132	418	550	356	219	575		
General Office	198,165	181	28	210	37	183	220		
Shopping Centre	17,611	13	8	21	41	45	86		
Total Pe	erson Trips	326	454	781	434	447	881		

Using the most recent National Capital Region Origin-Destination survey (OD Survey), the existing mode shares for the area have been determined and compared to various modes share breakdowns identified by City Staff as potential interpretations of the data. Table 11 and Table 12 summarizes the modal shares for the proposed land-uses.



Table 11: Mode Shares – Residential

Travel Mode	Ottawa West (average)	Ottawa West (AM from/ within)	Ottawa West (PM to/ within)	Ottawa Centre (average)	Ottawa Centre (AM from/within)	Ottawa Centre (PM to/within)	TOD Area
Auto Driver	50%	45%	45%	30%	35%	30%	15%
Auto Passenger	15%	10%	15%	10%	5%	10%	5%
Transit	20%	25%	15%	45%	20%	25%	65%
Cycling	5%	5%	5%	5%	0%	5%	5%
Walking	10%	15%	20%	10%	40%	30%	10%
Total	100%	100%	100%	100%	100%	100%	100%

Table 12: Mode Shares – Office/Retail

Travel Mode	Ottawa West (average)	Ottawa West (AM to/ within)	Ottawa West (PM from/ within)	Ottawa Centre (average)	Ottawa Centre (AM to/within)	Ottawa Centre (PM from/within)	TOD Area
Auto Driver	50%	50%	45%	30%	30%	30%	15%
Auto Passenger	15%	15%	15%	10%	10%	5%	5%
Transit	20%	20%	15%	45%	50%	50%	65%
Cycling	5%	5%	5%	5%	5%	5%	5%
Walking	10%	10%	20%	10%	10%	10%	10%
Total	100%	100%	100%	100%	105%	100%	100%

Internal capture rates from the ITE Trip Generation Handbook 3rd Edition assigned to the development for the office and retail components for mixed-use developments. The rates summarized in Table 13 represent the percentage of trips to/from the retail or office uses based on the residential component.

Table 13: Internal Capture Rates

Land Use	Α	M	PM		
	In	Out	In	Out	
General Office	3%	1%	57%	2%	
Shopping Centre	17%	14%	10%	26%	

Using the above mode share targets for a TOD area and person trip rates the person trips by mode have been projected. Table 14 summarizes the trip generation by mode.

Table 14: Trip Generation by Mode

Travel Mode	Mode Share	А	M Peak Ho	ur	PM Peak Hour		
	wode Share	In	Out	Total	In	Out	Total
Auto Driver	15%	48	68	116	62	64	125
Auto Passenger	5%	16	22	38	21	22	43
Transit	65%	207	296	503	266	278	545
Cycling	5%	16	22	38	21	22	43
Walking	10%	33	46	78	42	43	84
Internal Capture	varies	-7	-1	-8	-25	-16	-41
Total	100%	320	453	774	410	430	840

As shown above, 116 AM and 125 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.



5.2 Trip Distribution

To understand the travel patterns of the subject development the OD Survey has been reviewed to determine the travel patterns for the Ottawa West area. Table 15 below summarizes the distributions.

Table 15: OD Survey Distribution

To/From	Ottawa West	Routing
North	5%	via Somerset (2.5% east and west)
South	30%	via Gladstone (15% west), Preston (15% south)
East	30%	via Gladstone (15% east), Preston (5% south), Preston (5% north), Gladstone (5% west to Hwy 417)
West	35%	via Laurel (5%) west, Somerset (5% west), Gladstone (25% west)
Total	100%	

Note: internal trips applied to west direction

5.3 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the Study Area road network. Figure 12 illustrates the new site generated volumes.

Figure 12: New Site Generation Auto Volumes 0(0) 1(2) 0(0) 4(5) 7 2(2) 5(5) 0(0) 7(6) Laurel ኀ ሶ ሶ 0(0) (0)0 10(10)] L 0(0) 58(54) 7(9) Access #2 31(29) 0(0) 27(26) (0)0 (0)0 **1** 0(0) **1** 22(27) 0(0) 31(29) 31(29) **4**] L 1] [3(3) <u>↑</u>
10(10) → Gladstone 0(0) ኅ ሰ ሰ ר וֹן רֹ ጎ ሰ ሰ 0(0) 19(24) 22(27) 0(0) 0(0) 0(0) 0(0) 0(0) 0(0) 10(12) 0(0) 14(13) Loretta



6 Background Network Travel Demands

6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3.1. Beyond the opening of the Trillium LRT Gladstone Station, no addition network changes have been included the preparation of this TIA.

6.2 Background Growth

The auto demand within the inner area of Ottawa has been documented as decreasing over the past 10 years, resulting in reduced demand on many roadways. As such, no growth has been applied to the study area intersections. This is consistent with the zoning bylaw amendment TIA.

6.3 Other Developments

No background developments were explicitly considered as part of this TIA.

The future background traffic volumes are anticipated to remain the same as the existing conditions and no improvements are recommended.

7 Demand Rationalization

7.1 Modal Share Sensitivity

No capacity constraints have been noted at the study area intersections. As this development is targeted for a transit focus and meets the planned context of this area, rationalization for adjusted demand is not required for this TIA.

8 Development Design

8.1 Design for Sustainable Modes

The proposed development is a mixed-use development, with eight surface parking spaces and 562 underground parking spaces. Of the total 570 parking spaces, 30 will be designated for visitor parking, 422 spaces for the residential component, 101 for the office and live-work components, and 16 for the retail component. It is anticipated that 422 bicycle parking spaces will be provided for the residential component and 80 bicycle spaces shared for the office, retail, and live-work components.

As part of the site plan, the plaza located between Towers 2 and 3 will provide access to the MUP on the west side of the Trillium LRT line, and the area between the Standard Bread and Trillium LRT line will be reconstructed as a pedestrian plaza for active mode connectivity to the Gladstone Station.

8.2 Circulation and Access

Vehicle access is provided via a one-way loop on Loretta Avenue, with a two-way access on the south end of the loop towards the underground garage ramp, and the move-in and garbage pick up locations for Towers 1 and 2. The one-way loop is 8.0m wide on the inbound portion to allow inbound turning movements, with an aisle width of 6.7m within the remainder of the site. An additional access point is provided to Tower 3 from Loretta Avenue for move-in and garbage pick up. No general vehicle access is authorized at this access location. This additional access location is 6.0m in width. The one-way loop is designated as the fire route within the site.

The pedestrian and active mode access is provided along Gladstone Avenue for the Standard Bread building, retail components, and through connection into the site. Along Loretta Avenue, the lobby accesses are all located on the one-way loop.



The garbage truck, move in truck and fire truck turning movements can be accommodated on site.

9 Parking

9.1 Parking Supply

The site provides 423 spaces for the residential component, 30 visitor parking spaces, 101 for the office and livework components, and 16 for the retail component. It is anticipated that 422 bicycle parking spaces will be provided for the residential component and 80 bicycle spaces shared for the office, retail, and live-work components. The resulting parking ratios for the site are summarized in Table 16. It is noted that the parking provisions are within a TOD zone and are below the maximum parking provisions.

Table 16: Parking Ratios

	Residential	Office/Live-Work	Retail
Parking Ratio	0.50 per unit	0.55 per 1076 sq. ft.	1.00 per 1076 sq. ft.

10 Boundary Street Design

Table 17 summarizes the MMLOS analysis for the boundary streets of Gladstone Avenue and Loretta Avenue N. The Gladstone Avenue existing and future conditions will be the same and are considered in one row and the Loretta Avenue N will be split between existing and future conditions. The boundary street analysis is based on the policy area of "within 600m of a rapid transit station". The MMLOS worksheets has been provided in Appendix E.

Table 17: Boundary Street MMLOS Analysis

Segment	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Gladstone Avenue	С	Α	В	В	D	D	-	-
Loretta Avenue North (existing)	F	Α	В	D	-	-	-	-
Loretta Avenue North (future)	Α	Α	В	D	-	-	-	-

Gladstone Avenue and the existing Loretta Avenue N segments do not meet the pedestrian MMLOS target. Gladstone Avenue would require reduced traffic and a boulevard of 0.5m or greater to meet the PLOS A target. The addition of a sidewalk and landscaping along the Loretta Avenue N frontage will meet the PLOS target.

11 Access Intersections Design

11.1 Location and Design of Access

The vehicle access will be located along Loretta Avenue N on a one-way loop, and a secondary garbage truck/move-in access for Tower 3 located at the north of the site. The inbound access is located approximately 55 metres north of Gladstone Avenue and will be 8.0 metres wide to accommodate turning movements, and the outbound access is located approximately 105 metres north of Gladstone Avenue and will be 6.7 metres wide, permitting left and right turn lanes. The Tower 3 garbage/move-in access is located approximately 4 metres south of the northern property limit and is 6.0 metres wide.

The inbound throat length is approximately 7.4 metres to the turn of the one-way loop radius and 20.7 metres to the first parking stall. The outbound throat length is approximately 7.3 metre from the property line to the end the turn of the one-way loop radius and 26.2 metres to the first parking space. The Tower 3 garbage/move-in access provides approximately 10.6 metres from the property line to the loading door.



The 8.0m inbound access is noted to exceed the Private Approach Bylaw Section 25(d), but is required to permit truck northbound turning movements into the site from Loretta Avenue N.

11.2 Intersection Control

The outbound site access will include a stop sign control and one-way signage will be provided at each of the inbound and outbound access locations. No other access control is recommended for the site.

11.3 Access Intersection Design

11.3.1 Future Total Access Intersection Operations

The future total intersection volumes are illustrated in Figure 13 and the access intersection operations are summarized below in Table 18. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix F.

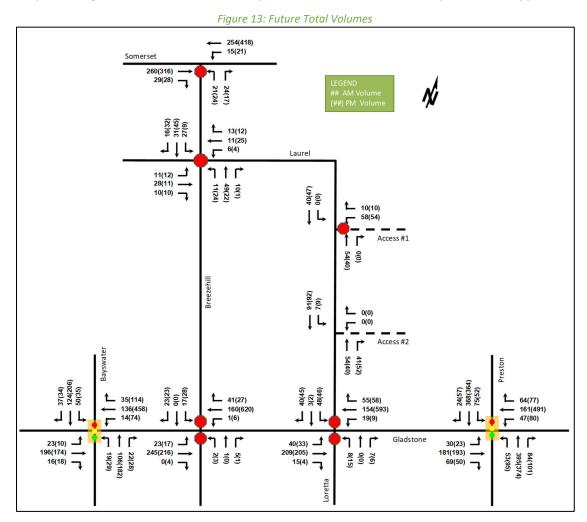




Table 18: Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95th)	LOS	V/C	Delay	Q (95th)
	WBL	Α	0.06	9.2	1.5	Α	0.06	9.2	1.5
Access #1 &	WBR	Α	0.01	8.6	0.0	Α	0.01	8.5	0.0
Loretta Avenue N	NB	-	-	-	-	-	-	-	-
Unsignalized	SB	-	-	-	-	-	-	-	-
	Overall	Α	-	3.8	-	Α	-	3.9	-

Notes:

Saturation flow rate of 1800 veh/h/lane

PHF = 1.00

m = metered queue

= queue exceeds storage or mid-block length

The access intersection operations for the future total horizon operate well.

11.3.2 Access Intersection MMLOS

No signalized intersections are used to access the proposed site.

11.3.3 Recommended Design Elements

The recommended access design elements remain consistent with City of Ottawa standards for access design for depressed curbs through the access locations and depressed sidewalks crossing the accesses. The inbound access is noted to be 8.0 metres for truck turning movements.

12 Transportation Demand Management

12.1 Context for TDM

The mode shares used within the TIA represent the planning level targets for a transit-oriented design (TOD) s the site is located within the Gladstone Station TOD design priority area.

For the residential land use, total bedrooms are estimated to be 1,242 and no age restrictions are noted. The retail and office land uses will be determined by lease options and are entirely dependant on tenants to determine the number and occupation of employees, and clients/customers travelling from the Ottawa-Gatineau area and within 2.0km of the site. It is noted that the existing tenants of the site may continue to stay and rent the provided retail/office space.

12.2 Need and Opportunity

The subject site has been assumed to rely predominantly on transit due to the proximity to the future Gladstone LRT Station. As mentioned above, a decrease in the assumed 15% auto mode share to the existing 45% auto mode share for the area, would see an increase of over 3 times the auto trips from the proposed development.

The development is planned to coincide with the Gladstone Station construction. The convenience of the transit station should provide the opportunity to reach the forecast transit mode share, although incentives for new residential tenants exists within the TDM framework. Hard measures, such as reduced parking provisions, would limit the risk of higher auto mode shares being produced from the site.

12.3 TDM Program

The "suite of post occupancy TDM measures" has been summarized in the TDM checklists for both the residential and non-residential land uses. The checklist is provided in Appendix G.

The key TDM measures recommended include:

 Enhanced connectivity of pedestrians and cyclists to the adjacent network and adjacent Gladstone LRT station



- Engagement with local bike share programs (e.g., VeloGO) to include onsite space for bike rack/storage
- Posting of pedestrian, cycling, and transit information and maps at primary entrances/exits
- Unbundle parking cost from purchase or rental costs
- Inclusion of a 1-year Presto card for first time new residential tenants, along with a set time frame for this offer (e.g., 6-months) from the 'opening' of the building/tower

The City and the proponent may engage beyond the scope of the TIA process to confirm the elements and conditions required as part of the site plan approvals.

13 Neighbourhood Traffic Management

Gladstone Avenue is a major collector road and has a 600-vehicle threshold for two-way traffic volumes per the City's TIA Guidelines. The existing Gladstone Avenue volumes exceed this threshold in both segments to the east and west of Loretta Avenue N by at least 140 percent. The site traffic will increase the two-way vehicles by 53 to the east of the site and 46 vehicles to the west of the site. The overall increase is not considered significant within the context of the existing Gladstone Avenue traffic. If a reduction is required to meet the TIA thresholds, the City will be required to explore options to reduce Gladstone Avenue volumes by 250-300 vehicles during the peak hours.

Along Loretta Avenue N, the existing volumes are below the local road thresholds of 120 two-way vehicles. The segment of Loretta Avenue N from Gladstone Avenue to the outbound site access will exceed the local road threshold with the addition of the site traffic and the segment north of the site will remain below the threshold. As the segment between Gladstone Avenue and the site accesses will not have any accesses or adjacent land uses that would be negatively affected by the volumes and no mitigation is recommended. It is noted that the MMLOS analysis meets the area targets for Loretta Avenue N.

14 Transit

14.1 Route Capacity

The existing Trillium line provides capacity for 1,000 passengers per direction per hour on a 15-minute service schedule. The future plans include a steady increase in service time to 8-minutes (1,750 passengers per direction per hour) as demand increases.

Based on the trip distribution and forecasted transit trips for the site, an upper limit of 205 northbound and 90 southbound outbound trips during the AM peak, and 80 northbound and 185 southbound inbound trips during the PM peak could be expected on the Trillium LRT. The range of the impacts are 8-21% of the existing service capacity of the Trillium LRT. Therefore, a revised service time schedule of 10-12 minutes may be required to support this development.

If a maximum of 10% of the transit mode share utilizes the existing route #14, this may see the need for an additional single bus (55-person capacity) during the peak hours to accommodate the additional demand.

14.2 Transit Priority

No transit priority is required explicitly for this study.



15 Network Intersection Design

15.1 Network Intersection Control

No change to the existing signalized control is recommended for the network intersections. A signal warrant was completed for the Gladstone Avenue and Loretta Avenue N intersection and is provided in Appendix H.

15.2 Network Intersection Design

15.2.1 Future Total Network Intersection Operations

The future total network intersection operations are summarized below in Table 19. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix F.

Table 19: Future Total Network Intersection Operations

I		AM Peak Hour				PM Peak Hour			
Intersection	Lane	LOS	V/C	Delay	Q (95th)	LOS	V/C	Delay	Q (95th)
	EB	С	0.73	32.9	48.6	Α	0.53	19.7	45.2
	WBL	Α	0.24	22.3	11.6	Α	0.22	16.1	16.0
	WBT/R	Α	0.55	24.2	36.7	D	0.89	37.9	#123.7
Gladstone Avenue	NBL	Α	0.11	9.1	9.2	Α	0.33	17.3	18.3
& Preston Street	NBT/R	Α	0.50	11.6	66.2	В	0.64	19.6	72.9
Signalized	SBL	Α	0.19	10.3	12.8	Α	0.20	14.9	10.7
	SBT/R	Α	0.39	10.5	51.3	Α	0.56	17.9	62.6
	Overall	Α	0.56	17.2	-	С	0.75	24.2	_
	EB	Α	0.03	7.8	0.8	A	0.04	9.2	0.8
Gladstone Avenue	WB	Α	0.01	7.8	0.0	Α	0.01	7.8	0.0
& Loretta Avenue	NB	В	0.03	12.2	0.8	С	0.09	21.0	2.3
North <i>Unsignalized</i>	SB	В	0.17	13.2	4.5	С	0.32	22.7	9.8
Unsignalizea	Overall	Α	-	3.1	-	Α	-	2.9	-
Cll-t	EB	Α	0.02	7.8	0.8	Α	0.02	9.1	0.8
Gladstone Avenue & Breezehill	WB	Α	0.00	7.8	0.0	Α	0.01	7.8	0.0
Avenue	NB	В	0.01	11.2	0.0	С	0.02	18.6	0.0
Unsignalized	SB	В	0.07	11.3	1.5	С	0.17	19.6	4.5
Onsignanzea	Overall	Α	-	1.4	-	Α	-	1.4	-
Gladstone Avenue & Bayswater	EB	Α	0.31	11.3	27.6	Α	0.24	8.7	20.6
	WB	Α	0.24	9.5	20.3	С	0.80	21.6	#109.9
Avenue	NB	Α	0.25	13.2	20.6	Α	0.47	19.6	37.4
Signalized	SB	Α	0.37	15.1	29.6	Α	0.55	21.3	43.5
	Overall	Α	0.33	12.3	-	В	0.70	19.3	-
Somerset Street W	EB	-	-	-	-	-	-	-	-
& Breezehill	WB	Α	0.01	8.1	0.0	Α	0.02	8.6	0.8
Avenue	NB	В	0.08	12.3	2.3	С	0.12	16.8	3.0
Unsignalized	Overall	Α	-	1.1	-	Α	-	1.0	-
	EB	Α	0.06	7.5	1.5	Α	0.04	7.3	0.8
Laurel Street &	WB	Α	0.03	7.2	0.8	Α	0.05	7.3	0.8
Breezehill Avenue	NB	Α	0.08	7.5	2.3	Α	0.06	7.5	1.5
Unsignalized	SB	Α	0.09	7.5	2.3	Α	0.09	7.3	2.3
	Overall	Α	-	7.5	-	Α	-	7.3	-

Notes:

Saturation flow rate of 1800 veh/h/lane

PHF = 1.00

m = metered queue

= queue exceeds storage or mid-block length



The network intersection operations for the future total conditions operate similarly to the existing conditions. A slight increase in operations is noted due to the peak hour factor of 1.00 applied when compared to the existing conditions.

15.2.2 Network Intersection MMLOS

Table 20 summarizes the MMLOS analysis for the signalized network intersections and the existing and future conditions for are assumed to be the same and are considered in one row. The intersection analysis is based on the policy area of "within 600m of a rapid transit station", which has the same targets as "within 330m of a school". The MMLOS worksheets has been provided in Appendix E.

Table 20: Study Area Intersection MMLOS Analysis

Intersection	Pedesti	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
intersection	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target	
Gladstone Avenue & Preston Street	С	А	В	В	E	D	F	D	С	E	
Gladstone Avenue & Bayswater Avenue	В	А	В	В	D	D	-	-	В	E	

The MMLOS targets will not be met for the pedestrian at both intersections and truck LOS at the Gladstone Avenue and Preston Street intersection.

To meet the PLOS targets, the intersection of Gladstone Avenue and Bayswater Avenue could meet the area targets on the east and west sides of the intersection with a signage change to prohibit right-turns on red, and on the north and south sides of the intersection through protected left-turns on Gladstone Avenue or a combination of zebra hi-vis crossing markings and the prohibition of right-turns on red. These are considered City improvements and can be reviewed internally for potential implementation.

At the intersection of Gladstone Avenue and Preston Street, the PLOS cannot be met due to arterial road intersection limitations. It would require reconstruction of the intersection to reduce all pedestrian crossing distances to approximately 7.0 metres and include the additional implementation of on of the following items: protected left-turns, right-turn on red prohibition or raised pedestrian crossings. The truck LOS would require additional receiving lanes or corner radii to be increased to greater than 15 metres to meet the targets. No improvements are recommended for this intersection to meet the MMLOS targets.

15.2.3 Recommended Design Elements

No study area intersection design elements are proposed as part of this study.

16 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

Proposed Site and Screening

- The proposed site includes 846 residential units, 198,165 sq. ft of office space (including live-work space), and 17,611 sq. ft. of retail space (including the existing Standard Bread building)
- Accesses will be provided along Loretta Avenue N via a one-way loop
- A pedestrian plaza is proposed at the corner of the development adjacent to Gladstone Avenue and the Trillium LRT corridor
- The development is proposed to be completed by 2026
- The trip generation and location triggers were met for the TIA Screening



Existing Conditions

- Preston Street and Somerset Street W are arterial roads, Gladstone Avenue is a major collector road, and Bayswater Avenue is a collector road in the study area
- Sidewalks are generally provided on both sides of the study area roadways, with the exception of limited facilities along Loretta Avenue, no sidewalks on Laurel Street to the east of Breezehill Avenue and on the east side of Breezehill Avenue between Laurel Street and Gladstone Avenue
- Bike lanes are provided on Somerset Street W over the Trillium LRT corridor and Gladstone Avenue,
 Bayswater Avenue and Somerset Street W are suggested bike routes
- The Trillium Pathway runs along the east side of the Trillium LRT corridor
- The existing transit route #14 travels along Gladstone Avenue, route #11 along Somerset Street W and #85 along Preston Street
- No operational issues are noted for the study area intersections
- The study area intersections with higher collisions were reviewed and no specific mitigation recommendations are noted for the Gladstone Avenue and Bayswater Avenue intersection or Gladstone Avenue and Preston Street intersection, but additional review may be required by the City for Somerset Street W between Breezehill Avenue and Preston Street for cycling collisions

Development Generated Travel Demand

- The proposed development is forecasted produce 779 two-way people trips during the AM peak hour and 879 two-way people trips during the PM peak hour
- Of the forecasted people trips, 116 two-way trips will be vehicle trips during the AM peak hour and 125 two-way trips will be vehicle trips during the PM peak hour based on a 15% modal share target
- Of the forecasted people trips, 502 two-way trips will be transit trips during the AM peak hour and 543 two-way trips will be transit trips during the PM peak hour based on a 65% modal share target
- Of the forecasted trips, 35% are anticipated to travel west, 30 to the east and south, and 5% to the north

Background Conditions

- Adjacent developments have either been on hold for extended periods of time with an unknown horizon, or are too small to have a noticeable impact on the adjacent road network
- Additionally, the background growth in the Ottawa core has been decreasing and a 0% growth was assumed for the area
- The future background intersection operations are the same as the existing intersections

Development Design

- The auto parking areas are to be located in two levels of underground parking and the bike parking will be internal to the building
- Pedestrian connections will be made along Gladstone Avenue and Loretta Avenue N, and a connection to the Trillium LRT corridor MUP will be made between Towers 2 and 3 and at the Gladstone Avenue pedestrian plaza to the LRT station

Parking

• The site is proposed to provide a total of 570 parking spaces, split into 30 visitor spaces, 423 residential spaces, 101 office/live-work spaces and 16 retail parking spaces



• In addition, 422 bicycle parking spaces will be provided for the residential component and 80 bicycle spaces for the retail, office, and live-work components

Boundary Street Design

- The boundary streets will not meet pedestrian MMLOS targets along Gladstone Avenue and the existing Loretta Avenue N, due to auto volumes and lack of boulevard space along Gladstone Avenue and no sidewalk currently provided along the frontage of Loretta Avenue N
- The addition of the sidewalk and boulevard on Loretta Avenue N will meet the pedestrian targets once the site is developed

Access Intersections Design

- A one-way loop access and garbage/move-in only access are proposed along Loretta Avenue N, the access will require a depressed curb and sidewalk through the access
- The outbound access will include a minor stop control and one-way signage will be provided on both the inbound and outbound accesses of the one-way loop
- No specific recommendations or design elements are required outside of typical site design

TDM

- Supportive TDM measures to be included within the proposed development should include:
 - Enhanced connectivity of pedestrians and cyclists to the adjacent network and adjacent Gladstone LRT station
 - Engagement with local bike share programs (e.g., VeloGO) to include onsite space for bike rack/storage
 - Posting of pedestrian, cycling, and transit information and maps at primary entrances/exits
 - Unbundle parking cost from purchase or rental costs
 - o Inclusion of a 1-year Presto card for first time new residential tenants, along with a set time frame for this offer (e.g., 6-months) from the 'opening' of the building/tower
- The City and the proponent may engage beyond the scope of the TIA process to confirm the elements and conditions required as part of the site plan approvals

Neighbourhood Traffic Management

- Gladstone Avenue currently exceeds the City's TIA Guideline thresholds for a major collector road, and the City would be required to reduce the two-way traffic along Gladstone Avenue by 250-300 vehicles during the peak hours to achieve the prescribed thresholds
- Loretta Avenue N is currently under the City's TIA Guideline thresholds for a local road and is projected to remain under the threshold to the north upon the site build-out
- Between Gladstone Avenue and the outbound access of the one-way loop is forecasted to exceed the City's TIA Guideline thresholds for a local road, and due to limited impacts along the segment, no mitigation is recommended

Transit

- To meet forecasted transit use, it is estimated that the Trillium LRT line may need to increase service times
 from a 15-minute service schedule to a 10–12-minute service schedule, and an additional bus in each
 direction may be needed along Gladstone Avenue during the peak hours for local routes
- No specific transit priority measures were considered as part of this development



Network Intersection Design

- Generally, the network intersections will operate similarly to the existing conditions
- The MMLOS targets will not be met for the pedestrian LOS at the Gladstone Avenue intersections at Bayswater Avenue and at Preston Street, and the truck LOS at Preston Street
- At the intersection of Gladstone Avenue and Bayswater Avenue, the prohibition of right-turns on read for all approaches and the addition of zebra hi-vis crossing markings on the north and south sides of the intersection would be required to meet the pedestrian LOS, and are considered the responsibility of the City to implement should they require
- No mitigation is recommended for the Gladstone Avenue and Preston Street intersection as reconstruction would be required to meet the pedestrian LOS targets and the truck LOS targets directly conflict with achieving the pedestrian LOS targets

17 Conclusion

It is recommended that, from a transportation perspective, the proposed development applications proceed.

Prepared By:

A. J. HARTE 100149314

April 21, 2021

April 21, 2021

Andrew Harte, P.Eng. Senior Transportation Engineer Reviewed By:

Christopher Gordon, P.Eng. Senior Transportation Engineer



Appendix A

TIA Screening Form and PM Certification Form





City of Ottawa 2017 TIA Guidelines Step 1 - Screening Form Date: 08-Apr-21
Project Number: 2020-25
Project Reference: 951 Gladstone & 145 Loretta

1.1 Description of Proposed Development	
Municipal Address	951 Gladstone Ave, 145 Loretta Ave N
Description of Location	Existing general indutrial or retail/commercial uses,
Description of Location	including the Standard Bread Building
Land Use Classification	Pending rezoning to Mixed-Use Centre
Decelerated Co.	Apartments: 843 units, Office: 198,165 sq. ft, Retail:
Development Size	17,611 sq.ft (includes Standard Bread Building)
Accesses	One-way loop and garbage/move-in access on Loretta
Phase of Development	Single Phase
Buildout Year	2026
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Townhomes or apartments
Development Size	843 Units
Trip Generation Trigger	Yes

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is	
designated as part of the City's Transit Priority, Rapid Transit or Spine	No
Bicycle Networks?	
Is the development in a Design Priority Area (DPA) or Transit-oriented	Voc
Development (TOD) zone?	Yes
Location Trigger	Yes

1.4. Safety Triggers	
Are posted speed limits on a boundary street 80 km/hr or greater?	No
Are there any horizontal/vertical curvatures on a boundary street limits	No
sight lines at a proposed driveway?	INO
Is the proposed driveway within the area of influence of an adjacent traffic	
signal or roundabout (i.e. within 300 m of intersection in rural conditions,	No
or within 150 m of intersection in urban/ suburban conditions)?	
Is the proposed driveway within auxiliary lanes of an intersection?	No
Does the proposed driveway make use of an existing median break that	No
serves an existing site?	INO
Is there is a documented history of traffic operations or safety concerns on	No
the boundary streets within 500 m of the development?	NO
Does the development include a drive-thru facility?	No
Safety Trigger	No



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$] is either transportation engineering $\sqrt{\text{or}}$ or transportation planning \square .
- License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at Ottawa (City)	this 20 day of September	_, 2018
Name:	Andrew Harte (Please Print)	_
Professional Title:	Professional Engineer	
Signature	of Individual certifier that s/he meets the above four criteria	

Office Contact Information (Please Print)
Address: 13 Markham Avenue
City / Postal Code: Ottawa / K2G 3Z1
Telephone / Extension: (613) 697-3797
E-Mail Address: Andrew.Harte@CGHTransportation.com



Appendix B

Turning Movement Count Data





Turning Movement Count - 15 Minute Summary Report

BAYSWATER AVE @ GLADSTONE AVE

Survey Date: Wednesday, July 27, 2016

Total Observed U-Turns

Northbound: 0 Eastbound: 0 Southbound () Westbound ()

BAYSWATER AVE

GLADSTONE AVE

				2010		RAV	-					G	LAUS	TONE	AVE					
		Ñ	orthbo	und		So	uthbour	nd.			Eas	thound			We	stound	d			
Time Pe	eriod .	LT	ST	RT	TOT	LT	st	RT	S TOT	STR	LT	ST	RT	TOT	LT	ST	RT	TOT	STR	Grand Total
07:00	07:15	0	13	3	16	11	31	6	48	64	5	22	3	30	2	11	6	19	49	113
07:15	07:30	2	21	2	25	3	23	3	29	54	5	21	3	29	2	18	5	25	54	108
07:30 0	07:45	$\pm t$	19	2	22	10	32	2	44	66	9	24	3	36	3	21	8	32	68	134
07:45	08:00	2	25	7	34	8	35	5	48	82	5	28	3	36	4	14	4	22	58	140
08:00	08:15	3	20	5	28	18	30	11	59	87	5	36	1	42	3	19	7	29	71	158
08:15	08:30	5	32	3	40		29	7	44	84	7	28	4	39	1	22	9	32	71	155
08:30 0	08:45	3	24	7	34	10	32	15	57	91	5	23	8	36	5	26	10	41	77	168
08:45	09:00	8	30	7	45	14	33	4	51	96	6	25	3	34	5	38	9	52	86	182
09:00	09:15	3	27	8	38	9	24	7	40	78	7	28	3	38	5	19	8	32	70	148
09:15	09:30	4	18	10	32	10	28	9	47	79	3	29	2	34	2	20	8	30	64	143
09:30	09:45	7	13	8	28	10	14	5	29	57	6	23	5	34	3	21	7	31	65	122
09:45	10:00	4	12	5	21	9	19	2	30	51	5	24	3	32	1	25	6	32	64	115
11:30	11:45	4	28	5	37	2	21	5	28	65	2	27	2	31	12	24	10	46	77	142
11:45	12:00	.5	15	14	34	10	23	6	39	73	1	30	3	34	5	46	14	65	99	172
12:00	12:15	6	25	9	40	8	28	8	44	84	5	28	0	33	7	35	8	50	83	167
12:15	12:30		17	5	28	8	23	13	44	72	4	26	4	34	6	38	15	59	93	165
12:30	12:45	7	28	10	45	5	18	3	26	71	8	29	4	41	3	27	8	36	77	148
12:45	13:00	7	27	13	47	10	15	4	29	76	3	26	5	34	7	28	10	45	79	155
13:00	13:15	3	14	3	20	7	24	4	35	55	4	30	3	37	4	32	16	52	89	144
13:15	13:30	5	20	8	33	10	17	4	31	64	9	33	4	46	2	38	10	50	96	160
15:00	15:15	4	48	5	57	10	30	9	49	106	6	32	3	41	7	27	11	45	86	192
15:15	15:30	2	53	3	58	6	42	8	56	114	8	20	8	36	9	35	26	70	106	220
15:30	15:45	1	52	7	60	4	38	2	44	104	7	28	2	37	11	60	35	106	143	247
15:45	16:00	4	45	5	55	8	25	6	39	94	5	25	5	35	9	46	23	78	113	207
16:00	16:15	.6	57	7	70	12	35	10	57	127	11	30	6	47	13	60	33	115	162	289
16:15	16:30	7	44	3	54	9	47	9	65	119	8	26	1	35	24	66	27	117	152	271
16:30	16:45	8	44	8	60	5	69	9	83	143	7	26	8	41	11	66	32	109	150	293
16:45	17:00	8	37	10	55	9	55	6	70	125	4	18	3	25	26	66	22	114	139	264
17:00	17:15	9	40	2	51	6	58	15	79	130	0	26	6	41	22	57	27	106	147	277
17:15	17:30	5	36	10	51	9	37	12	58	109	9	23	6	38	16	73	26	115	153	262
17:30	17:45	3	28	5	36	6	35	8	49	85	11	29	5	45	11	50	13	74	119	204
17:45	18:00	6	26	9	41	10	28	10	48	89	6	21	2	29	5	38	11	54	83	172

Note: U-Turns are included in Totals.

Comment

2018-Oct-04 Page 1 of 1



Transportation Services - Traffic Services

Turning Movement Count - Cyclist Volume Report

Work Order 36100

BAYSWATER AVE @ GLADSTONE AVE

Count Date: Wednesday, July 27, 2016

Start Time: 07:00

	B	AYSWATER AV	E		SLADSTONE A	VE	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	6	2	8	29	14	43	51
00:00 00:00	12	7	19	63	18	81	100
09:00 10:00	5	0	5	19	15	34	39
11:30 12:30	2	0	2	11	14	25	27
12:30 13:30	2	0	2	10	0	10	12
15:00 16:00	3	4	7	10	9	19	26
16:00 17:00	3	7	10	21	51	72	82
17:00 18:00	11	11	22	35	40	75	97
Total	.44	31	75	198	161	359	434

Comment

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

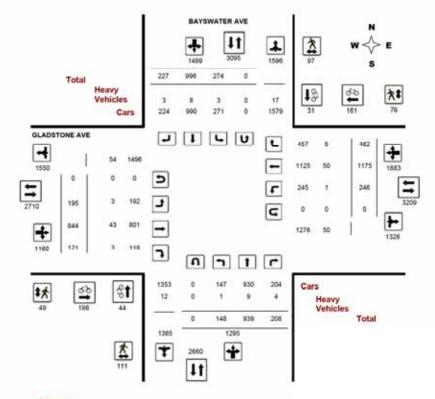


Turning Movement Count - Full Study Diagram

BAYSWATER AVE @ GLADSTONE AVE

Survey Date: Wednesday, July 27, 2016 WO#: 36100

Device: Miovision



Comments



Transportation Services - Traffic Services

W.O. 36100

Turning Movement Count - Heavy Vehicle Report

BAYSWATER AVE @ GLADSTONE AVE

Survey Date: Wednesday, July 27, 2016

			BAY	SWA	TER A	VE						GLA	DST	ONE A	VE					
		Northb	ound		- 1	South	bound	= 0		400	Eastb	ound		10.50	Westbo	ound				
Time	Period	LT.	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E	LT	ST	RT	W	STR TOT	Gran
07:00	08:00	0	0		1	1	1	0	2	3	2	2	0	4	1	8	0	9	13	16
08:00	09:00	0	2	0	2	0	3	1	4	6	0	4	2	6	0	7	1	8	14	20
09:00	10.00	0	1	0	1	0	1	0	1	2	0	4	0	4	0	5	0	5	9	11
11:30	12:30	0	1	3	4	t	٥	. 1	2	6	0	6	0	6	0	8	2	10	16	22
12:30	13:30	1	5	0	6	1	2	0	3	9	0	9	1	10	.0	9	0	9	19	28
15:00	16.00	0	0	0	0	0	0	0	0	0	0	9	0	9	0	3	2	5	14	14
16:00	17:00	0	0	0	0	0	0		1	1		6	0	7	0	6	0	6	13	14
17:00	18:00	0	0	0	0	0	10	0	1	1.	0	3	0	3	0	4	0	4	7	8
Sub	Total	1	9	4	14	3	ā	3	14	28	3	43	3	49	t	50	5	56	105	133
J-Turn	s (Hea	vy Vet	nicles)		0				0	0				0				0	0	0
To	tal	11	9	4	0	3	8	3	14	28	3	43	3	49	.1	50	5	56	105	133

2018-Oct-04 Page 1 of 1

2018-Oct-04

Page 1 of 1



Work Order 36100

Turning Movement Count - Pedestrian Volume Report

Count Dat	e: Wednesday,			AVE @ GLADS		Start Time:	07:00
Time Period	NB Approach	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	- 1	- 1	2	0	1	.1	3
07:15 07:30	3	0	3	-1	0	1	4
07:30 07:45	4	5	9	2	. 1	3	12
07:45 08:00	3	3	6	1	6	7	13
07:00 08:00	11	9	20	4	8	12	32
08:00 08:15	2	5	7	. 1	3	4	11
08:15 08:30	5	6	11	. 1	4	5	16
08:30 08:45	8	2	10	0	0	0	10
08:45 09:00	1	5	6	1	1	2	8
08:00 09:00	16	18	34	3	8	11	45
09:00 09:15	3	5	8	2	0	2	10
09:15 09:30	2	2	4	t	2	3	7
09:30 09:45	4	3	7	2	2	4	11
09:45 10:00	2	1	3	0	3	3	6
09:00 10:00	11	11	22	5	7	12	34
11:30 11:45	3.	4	5	0	1	1	6
11:45 12:00	3	0	3	0	4	4	7
12:00 12:15	3	0	3	0	9	9	12
12:15 12:30	4	1	5	2	1	3	
11:30 12:30	11	5	16	2	15	17	33
12:30 12:45	3	0	3	2	1.	3	6
12:45 13:00	5	1	6	0	0	0	6
13:00 13:15	4	1	5	0	2	2	7
13:15 13:30	5	2	7	1	3	4	11
12:30 13:30	17	4	21	3	6	9	30
15:00 15:15	0	- 1	1	0	1	- 1	2
15:15 15:30	3	-1	4	4	2		10
15:30 15:45	4	2	6	1	2	3	9
15:45 16:00	2	2	4	0	5	3	7
15:00 16:00	9	- 6	15	- 5	-	13	28
16:00 16:15	.6	9	15	3	. 4	7	22
16:15 16:30	2	1	3	4	0	4	7
16:30 16:45	8	7	15	5	5	10	25
16:45 17:00	11	6	17	1	2	3	20
6:00 17:00	27	23	50	13	- 11	24	74
17:00 17:15	3	8	11	3	1	4	15
17:15 17:30	1	7	8	3	7	10	18
17:30 17:45	2	3	5	2	5	7	12
17:45 18:00	3	3	6	6	0	6	12
17:00 18:00	9	21	30	14	13	27	57
Total	111	97	208	49	76	125	333

Comment

2018-Oct-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 36100

Turning Movement Count - Full Study Summary Report

BAYSWATER AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 27, 2016
 Total Observed U-Turns
 AADT Factor

 Northbound:
 0
 Southbound:
 0
 90

 Eastbound:
 0
 Westbound:
 0

								F	ull St	udy									
			BAY	SWAT	ER A	VΕ						GLA	ADSTO	NE A	VE				
_	1	Vorthb	ound	2.0	5	Southb	ound	2010		. 1	Eastbo	bnuo	1250		Westb	bnuo	According		
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	E8 TOT	LT	st	RT	TOT	STR	Gran Tota
00:00 00:10	5	78	14	97	32	121	16	163	266	24	95	12	131	11	64	23	56	229	495
00:00 00:00	19	106	22	147	50	124	37	211	358	23	112	16	151	14	105	35	154	305	663
09:00 10:00	18	70	31	119	38	85	23	146	265	21	104	13	138	11	85	29	125	263	525
11:30 12:30	21	85	33	133	28	95	32	155	294	12	111	9	132	30	143	47	220	352	640
12:30 13:30	22	89	34	145	32	74	15	121	266	24	118	16	158	16	125	42	183	341	607
15:00 16:00	11	199	20	230	28	135	25	188	415	26	105	18	149	36	168	95	299	448	866
16:00 17:00	29	182	28	239	35	206	34	275	514	30	100	18	148	74	267	114	455	603	1117
17:00 18:00	23	130	26	179	31	158	45	234	413	35	99	19	153	54	218	77	349	502	915
Sub Total	148	939	208	1295	274	998	227	1499	2794	195	844	121	1160	246	1175	462	1583	3043	5837
U Turns				0				9.0					0.5				.0	0.7	.0
Total	148	939	206	1295	276	996	227	1499	2794	195	844	121	1160	245	1175	492	1883	3043	5837
EQ 12Hr	206	1305	289	1800	381	1387	316	2084	3884	271	1173	168	1612	342	1633	642	2617	4229	8113
Note: These v	alues is	re calcu	lated by	multiply	ying the	totals b	y the a	ppropriati	e expans	ion tac	br.		- 1	.39					
AVG 129tr	185	1175	260	1620	343	1248	284	1875	3495	244	1056	151	1451	308	1470	578	2356	3887	7302
Note: These v	rolumes	are calc	ulated	by multip	plying t	he Equiv	alent 1	2 hr. tota	is by the	AADT	factor.			90					
AVG 24Hr	243	1539	341	2122	449	1636	372	2457	4579	320	1383	198	1901	403	1926	757	3086	4987	1566

Comments

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

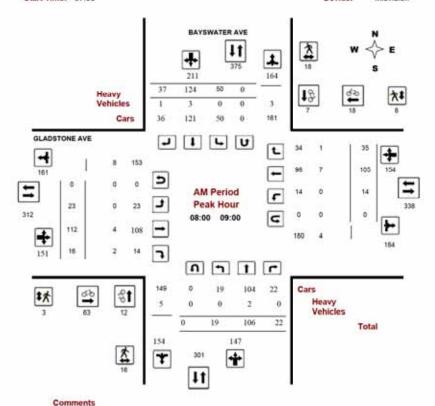


Turning Movement Count - Peak Hour Diagram

BAYSWATER AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 27, 2016
 WO No:
 36100

 Start Time:
 07:00
 Device:
 Miovision





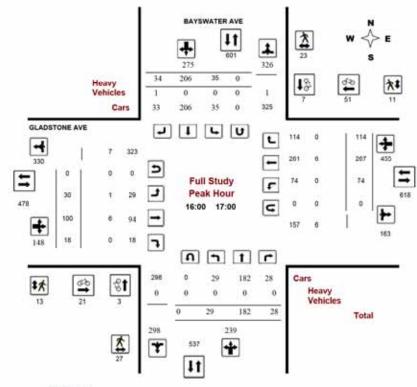
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BAYSWATER AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 27, 2016
 WO No:
 36100

 Start Time:
 07:00
 Device:
 Miovision



Comments

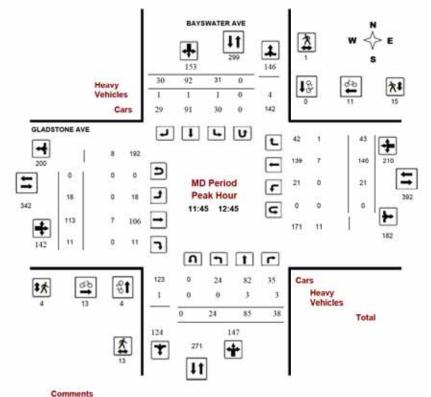


Turning Movement Count - Peak Hour Diagram

BAYSWATER AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 27, 2016
 WO No:
 36100

 Start Time:
 07:00
 Device:
 Miovision



Ottawa

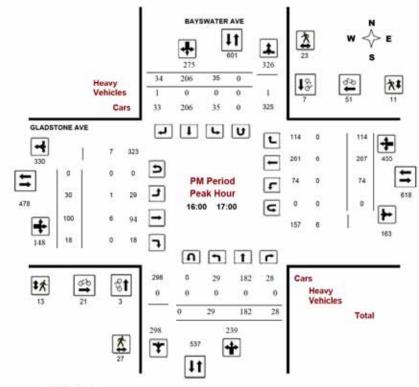
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BAYSWATER AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 27, 2016
 WO No:
 36100

 Start Time:
 07:00
 Device:
 Miovision



Comments



Work Order 36100

Turning Movement Count - 15 Min U-Turn Total Report

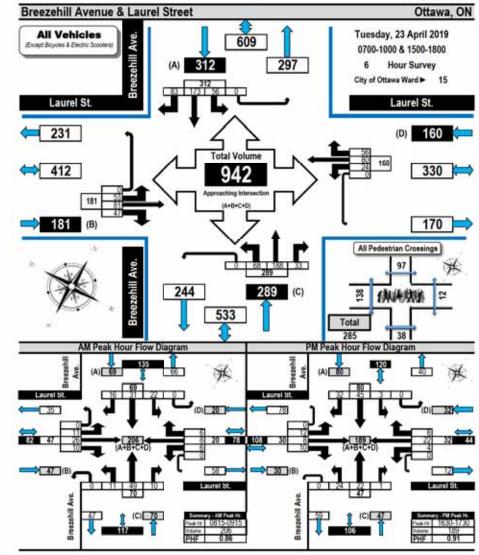
BAYSWATER AVE @ GLADSTONE AVE

Survey Date	e: yw	ednesday, July 27	, 2016			
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	.0
12:30	12:45	0	0	0	0	0
12:45	13:00	.0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	.0	0	0	0
15:00	15:15	0	.0	0.	0	0
15:15	15:30	0	.0	0	0	0
15:30	15:45	0	.0	0	0	0
15:45	16:00	0	.0	0	0	0
16:00	16:15	0	.0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
	tal	0	0	0	0	0

PRACTICE TRACTICE TRA

Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses



2018-Oct-04 Page 1 of 1 Printed on: 4/25/2019 Prepared by, thetrafficspecialist@gmail.com Flow Diagrams: AM PM Peak



Turning Movement Count Summary Report AADT and Expansion Factors

Automobiles, Taxis, Light Trucks, Vans. SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

Breeze	hill	Ave	enu	e &	Lau	rel	Stre	et													Otta	awa,	ON
Survey Da Weather Al Weather Pf	ıt:	Tues Partiy Overc	Cloud	ty +1(rill 201 PC		ırvey	Dura	tion:	6	Hrs.	Surv	Time ey Ho	urs:		0700 0700 Carm	1000	8 15	3.65310		ctor.		0.7
		1000	urel atbou					urel	10000	, j		В	reez	ehil thbo	7070.50	e.	В	reez	zehil uthbo	10000	e.		
Time Period	LT	ST	RT	UT	E/B Tot	LT	ST	RT	UT	W/B Tot	Street Total	LT	ST	RT	UT	N/B Tot	LT	ST	RT	UT		Street Total	Grand Total
0700-0800	4	16	- 4	. 0	24	2	- 5	7	- 0	14	38	- 7	26	7	- 0	40	10	17	2	.0	29	69	107
0800-0900	12	19	5	- 0	36	- 6	- 8	11	- 0	25	61	12	44	9	- 0	65	15	36	18	- 0	69	134	195
0900-1000	12	19	. 11	-0	42	- 4	- 11	. 10	- 0	25	67	. 4	39	- 8	- 0	51	14	21	9	- 0	44	95	162
1500-1600	- 4	9	12	- 0	25	- 6	19	16	- 0	41	66	14	29	6	- 0	49	- 8	29	- 8	- 0	45	94	160
1600-1700	12	11	10	. 0	33	- 4	19	7	0	30	63	. 9	27	0	- 0	36	- 6	33	18	- 0	57	93	156
1700-1800	9	. 7	5	- 0	21	2	18	5	- 0	25	46	22	23	3	0	48	3	37	28	- 0	68	116	162
Totals	53	-81	47	- 0	181	24	80	56	0	160	341	68	188	33	0	289	56	173	83	0	312	601	942

Equivalent 12 & 24-hour Vehicle Volumes Including the Annual Average Daily Traffic (AADT) Factor Applicable to the Day and Month of the Turning Movement Count

Expansion factors are applied exclusively to standard weekday 8-hour turning movement counts conducted during the hours of 0700h - 1000h, 1130h - 1330h and 1500h - 1800h

	E	pulvälen	d 12-ho	ur vehi	cle vol	umes.	These:	volume	t are c	alculate	d by m	ultiplyi	ng the	l-hour	totals b	y the 8	◆12 €	opansi	on fac	for of 1.	39		
Equ. 12 Hr	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	nia	n/a	n/a	n/a	n/a	n/a	n/a
										s ate ca													_
AADT 12-hr	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a
rtwe-sort	24.8	our AAD	T. The	se volu	mes a	e talc	alated b	y mult	plying	the ave	rage da	By 12-8	hour ve	hicle v	olumes	by the	12 42	4 екра	nsion 1	actor o	11.31		-
														100								n/a	

AADT and expansion factors provided by the City of Ottawa

AM Peak Ho	ur Fac	tor •	•	0.8	6									High	est H	ourly	Vehicle	Volu	ime E	etwe	en 071	00h &	1000h
AM Feak He	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	S.TOT	G.TOT
0815-0915	11	26	10	0	47	- 6	8	6	0	20	67	-11	49	10	0	70	22	31	16	0	69	139	206

PM Peak Ho	ur Fac	tor •	•	0,5	91							1		High	est H	ourly	Vehicle	Vol	ume 8	Betwe	en 15	00h &	1800h
PM Feak III	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	STOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	S.TOT	G.TOT
1630-1730	12	8	10	- 0	30	4	22	- 6	0	32	62	24	22	1	0	47	3	45	32	- 0	80	127	189

Comments:

A cedar hedge growing along the property frontage on the southwest quadrant is creating a serious sightline problem. The majority of the cyclists as well as some drivers ignore the all-way stop control. Vehicles parked too close to the intersection on both Laurel Street, east of Breezehill Avenue and Breezehill Avenue, south of Laurel Street create a sightline problem.

1. Includes all vehicle types except bicycles, electric bicycles, and electric scooters.

2 When expansion and AADT factors are applied, the results will differ slightly due to rounding.

Printed on: 4/25/2019 Prepared by: thetrafficspecialist@omail.com Summary: All Vehicles



Turning Movement Count Pedestrian Crossings Summary and Flow Diagram



approach, then another to reach their destination. Accordingly, one pedestrian crossing two approaches will be recorded as two crossings.

Breezehill Avenue & Laurel Street Ottawa, ON Tuesday, 23 April 2019 Pedestrian Crossings 0700-1000 & 1500-1800 Breezehill Ave. 6 Hour Survey City of Ottawa Ward ▶ 15 **Grand Total** Note The values in the summary table below and the flow diagram represent the number of pedestrian crossings NOT the number of individual pedestrians crossing. For example, some pedestrians will cross one

Time Period	West Side Crossing Laurel St.	East Side Crossing Laurel St.	Street	South Side Crossing Breezehill Ave.	North Side Crossing Breezehill Ave.	Street Total	Grand Total
0700-0800	5	1	6	3	- 8	11	17
0800-0900	54	4	58	14	18	32	90
0900-1000	2	0	2	0	4	4	- 6
1500-1600	50	0	50	12	23	35	85
1600-1700	12	1	13	2	22	24	37
1700-1800	15	6	21	7	22	29	50
Totals	138	12	150	38	97	135	285

38

Breezehill Ave.

Comments:

A cedar hedge growing along the property frontage on the southwest quadrant is creating a serious sightline problem. The majority of the cyclists as well as some drivers ignore the all-way stop control. Vehicles parked too close to the intersection on both Laurel Street, east of Breezehill Avenue and Breezehill Avenue, south of Laurel Street create a sightline problem.

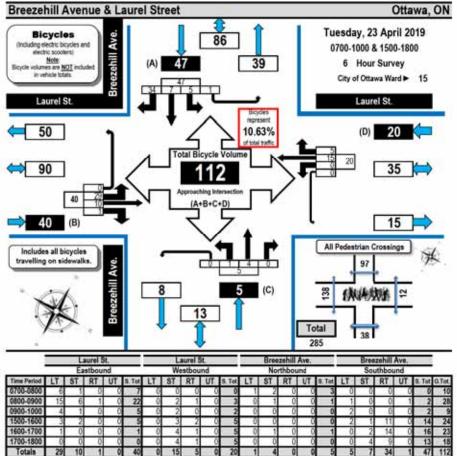
Printed on: 4/25/2019 Prepared by: thetrafficspecialist@gmail.com Summary: Pedestrian Crossings



Turning Movement Count

Bicycle Summary Flow Diagram





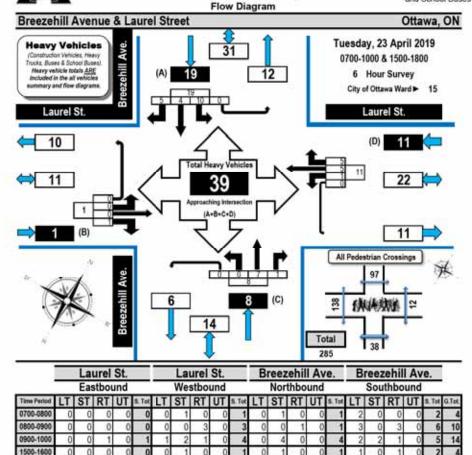
Comments:

A cedar hedge growing along the property frontage on the southwest quadrant is creating a serious sightline problem. The majority of the cyclists as well as some drivers ignore the all-way stop control. Vehicles parked too close to the intersection on both Laurel Street, east of Breezehill Avenue and Breezehill Avenue, south of Laurel Street create a sightline problem.



Turning Movement Count Heavy Vehicle Summary

Heavy Trucks, Buses, and School Buses



Totals Comments:

1600-1700

A cedar hedge growing along the property frontage on the southwest quadrant is creating a serious sightline problem. The majority of the cyclists as well as some drivers ignore the all-way stop control. Vehicles parked too close to the intersection on both Laurel Street, east of Breezehill Avenue and Breezehill Avenue, south of Laurel Street create a sightline problem.



Survey Date:

Transportation Services - Traffic Services w.o.

Turning Movement Count - 15 Minute Summary Report

BREEZEHILL AVE @ GLADSTONE AVE

Total Observed U-Turns

Northbound: 0 Southbound () Westbound: 0

Fastbound 0

Wednesday, July 18, 2018

			572	NLTSS: -		2770.000			E	astbour	1d: (estboun)				
			Е	REE	ZEHIL	L AVE						GL	ADS	TONE	AVE					
		No	orthbou	ind .	2.0	Soc	uthbour	nd	029	10000	Eas	thound		12	We	stbound		222	12000	24002
Time !	Period	LT	ST	RT	TOT	LT	ST	RT	TOT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	TOT	Grand Total
07:00	07:15	0	0	0	0	2	0	1	3	3	3	32	0	35	0	18	2	20	55	58
7:15	07:30	0	0	0	0	1	0	3	4	4	4	28	0	32	0	30	8	38	70	74
07:30	07:45	.0	1	13	2	0	0	3	3	5	3	36	0	39	0	18	4	22	61	66
07:45	08:00		o	1	1	5	1	3	9	10	7	36	0	43	0	35	2	37	80	90
00:80	08:15	0	0	1	1	4	0	4		9	4	40	0	44	0	30	4	34	78	87
08:15	08:30	. 0	0	1	1	4	0	7		9	10	49	0	59	0	40	8	48	107	116
06:30	08:45		1	0	2	3	0	8	11	13	4	53	0	57	0	42	7	49	106	119
18:45	09:00	0	0	1	1	2	0	1	3	4	5	60	0	65	1	44	8	53	118	122
09:00	09:15	. 1	0	1	2	2	0	2	4	6	4	38	0	42	0	27	14	41	83	89
19:15	09:30	0	0	0	0	6	0	6	12	12	8	42	0	50	٥	46	9	55	105	117
9:30	09:45	0	0	0	0	9	0	7	16	16	3	35	0	38	2	38	8	48	86	102
09:45	10:00	2	1	0	3	5	0	5	10	13	9	42	0	51	0	32.	1	33	84	97
11:30	11:45	2	0	1	3	5	0	5	10	13	3	44	0	47	1	39	4	44	91	104
1:45	12:00	1	0	0	1	7	0	4	11	12	9	43	1	53	1	39	2	42	95	107
2:00	12:15	1	0	0	1	7	0	7	14	15	7	40	1	48	0	42	6	48	96	111
2:15	12:30	1	0	-1	2	5	0	5	10	12	t	30	3	43	0	48	8	56	99	111
2:30	12:45	. 0	1	0	1	8	0	3	11	12	5	36	0	41	0	37	4	41	82	94
2:45	13:00	0	1	3	4	5	1	5	11	15	5	37	0	42	1	31	1	33	75	90
3.00	13:15	0	0	0	0	2	0	4	6	6	2	45	0	47	0	49	2	51	98	104
3:15	13:30	- 1	0	0	1	2	.1	3	6	7	1	39	1	41	1.	58	2	61	102	100
5:00	15:15	. 2	1	0	3	2	0	6	8	11	2	43	1	46	0	67	5	72	118	129
5:15	15:30	1	1	-1	3	4	0	3	7	10	Ø.	37	0	37	0	87	4	91	128	138
5:30	15:45	1	+	0	2	7	0	4	11	13	2	39	0	41	0	88	2	90	131	144
5:45	16:00	0	0	2	2	5	0	7	12	14	4	45	0	49	0	105	5	110	159	173
16,00	16:15	. 0	0	0	0	2	0	6		8	4	56	1	61	0	103	5	108	169	177
16:15	16:30	1	0	1	2	10	0	10	20	22	2	56	2	60	2	124	5	131	191	213
6:30	16:45	. 0	0	0	0	6	.0	2	8	8	8	46	2	56	0	118	9	127	183	191
16:45	17:00	. 0	0	0	0	7	0	6	13	13	3	41	0	44	1	133	5	139	183	196
7:00	17:15	2	0	0	2	5	0	5	10	12	4	44	0	48	3	122	8	133	181	193
17:15	17:30	. 1	0	2	3	4	1	2	7	10	0	49	1	50	2	112	3	117	167	177
17:30	17:45	2	0	1	3	1	0	6	7	10	2	34	2	38	1	114	2	117	155	165
17:45	18:00	0	0	0	0	6	0	t	7	7	0	30	0	30	0	93	1	94	124	131
TOTAL	9	20		18	46	140	4	144	288	334	128	1334	15	1477	10	2009	158	21	13 3660	3994

Note: U-Turns are included in Totals.

2018-Oct-04 Page 1 of 1



Transportation Services - Traffic Services

Turning Movement Count - Cyclist Volume Report

Work Order 37971

BREEZEHILL AVE @ GLADSTONE AVE

Count Date: Wednesday, July 18, 2018

Start Time: 07:00

	В	REEZEHILL AV	E		LADSTONE A	VE.	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	4	1	5	39	23	62	67
00:00 00:00	2	2	4	98	15	113	117
09:00 10:00	0	1	1	29	24	53	54
11:30 12:30	0	0	0	17	11	28	28
12:30 13:30	0	2	2	7	10	17	19
15:00 16:00	1	3	4	26	33	59	63
16:00 17:00	1	3	4	26	49	77	81
17:00 18:00	2	6	8	36	71	107	115
Total	10	18	28	280	236	516	544

Comment

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

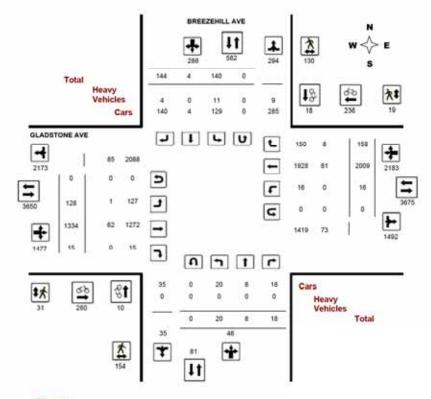


Turning Movement Count - Full Study Diagram

BREEZEHILL AVE @ GLADSTONE AVE

Survey Date: Wednesday, July 18, 2018 WO#: 37971

Device: Miovision



Page 1 of 1

Comments

Ottawa

Transportation Services - Traffic Services

W.O. 37971

Turning Movement Count - Heavy Vehicle Report

BREEZEHILL AVE @ GLADSTONE AVE

Surve	y Date	20	We	dneso	day, Ju	aly 18	, 201	В												
			BRE	EZEH	ILL A	VE						GLA	DST	ONE A	VE					
	· = 7)	Northb	ound			South	ound	= 0		A	Eastb	ound		10.5-0	Westbo	ound	7			
Time	Period	LT.	ST	RT	TOT	LT	ST	RT	TOT	STR	LT	ST	RT	TOT	LT	ST	RT	TOT	STR	Grand Total
07:00	98:00	0	0	0	0	0	0	. 1	+		0	11	0	11	0	12	1	13	24	25
08:00	09:00	0	0	0	0	0	0	0	0		0	11	٥	11	0	11	0	11	22	22
09:00	10.00	0	0	0	0	1	0	0	1	1	0	10	0	10	0	12	1	13	23	24
11:30	12:30	0	0	0	0	4	0	2	6	6	1	9	0	10	0	10	3	13	23	29
12:30	13:30	0	0	0	0	1	0	1	2	2	0	7	٥	7	.0	9	0	9	16	18
15:00	16.00	0	0	0	0	t	0	0	1	1	0	5	0	5	0	6	0	6	11	12
16:00	17:00	0	0	0	0	3	0	0	3	3	0	7	0	7	0	15	3	18	25	28
17:00	18:00	0	0	0	0	10	0	0	1	1	0	2	0	2	0	0	0	6	8	9
Sub	Total	0	0	0	0	11	0	4	15	15	1	62	0	63	0	81	8	89	152	167
U-Turn	s (Hear	ry Vet	nicles)		0				0	0				0				0	0	0
To	tal	0	0	0	0	11	0	. 4	15	15	.1	62	0	63	0	61	.8	89	152	167

2018-Oct-04 Page 1 of 1

2018-Oct-04



Work Order 37971

Turning Movement Count - Pedestrian Volume Report

			ZEHILL	AVE @ GLADS	STONE AVE		
Count Dat	te: Wednesday,	July 18, 2018				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	S8 Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	6	5	11	- 1	0	:1	12
07:15 07:30		3	8	1.	1	2	10
07:30 07:45		3	10	.0	2	2	12
07:45 08:00	2	3	5	0	2	2	7
07:00 08:00	20	14	34	2	5	7	41
08:00 08:15	5	4	9	0	0	0	9
08:15 08:30	8	2	10	3	0	3	13
08:30 08:45	11	6	17	2	1	3	20
08:45 09:00		9	14	4	0	4	18
08:00 09:00	29	21	50	9	1	10	60
09:00 09:15	.0	3	9	2	1	3	12
09:15 09:30	3	- 3	6	0	0	0	6
09:30 09:45	1	2	3	2	0	2	5
09:45 10:00	6	3	9	0	0	0	9
09:00 10:00	16	11	27	4	1	5	32
11:30 11:45	7	7	14	0	0	0	14
11:45 12:00	5	1	6	1	0	1	7
12:00 12:15	3	4	7	1	0	1	8
12:15 12:30	6	2	8	0	0	0	
11:30 12:30	21	14	35	2	0	2	37
12:30 12:45	2	5	7	2	0	2	9
12:45 13:00	2	2	4	1	0	1	5
13:00 13:15	4	3	7	0	0	0	7
13:15 13:30	2	3	5	0	0	0	5
12:30 13:30	10	13	23	3	0	3	26
15:00 15:15	9	3	12	0	0	0	12
15:15 15:30	3	1	4	1	0	1	5
15:30 15:45	6	3	9	0	0	0	9
15:45 16:00	4	7	11	0	0	0	11
15:00 16:00	22	14	36	1	0	1	37
16:00 16:15	2	7	9	0	0	0	9
16:15 16:30	3	7	10	1	5	6	16
16:30 16:45	5	6	11	4	3	7	18
16:45 17:00		3	10	0	1	1	11
16:00 17:00	17	23	40	5	0	14	54
17:00 17:15	10	6	16	2	1	3	19
17:15 17:30		7	12	0	1	4	13
17:30 17:45		2	2	0	1	4	3
17:45 18:00		5	9	3	0	3	12
17:00 18:00	19	20	39	- 5	3	8	47
Total	154	130	284	31	19	50	334
1,010	197	129	204	- 21		20	334

Comment

2018-Oct-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 37971

Turning Movement Count - Full Study Summary Report

BREEZEHILL AVE @ GLADSTONE AVE

Survey Date:	Wednesday, July 18, 2018	Tot	al Obs	erved U-Turns		AADT Factor
		Northbound:	0	Southbound	0	.90
		Eastbound	0	Westbound	0	

								F	ull Stu	ıdy									
			BRE	EZEH	ILL AV	E						GLA	DSTO	NE A	VE				
	N	lorthbo	bund	2.00	S	outhb	ound	82439		. 1	Eastbo	bund	14500		Westb	ound	Account		
Period	LT	ST	RT	NB TOT	LT	ST	RT	58 TOT	STR TOT	LT	ST	RT	EB TOT	LŤ	st	RT	TOT	STR	Gran Tota
07:00 08:00	0	1	2	3	8	1	10	19	22	17	132	0	149	0	101	18	117	266	280
00:00 00:00	1	1	3	5	10	0	20	30	35	23	202	0	225	1	156	27	184	429	444
09:00 10:00	3	1		5	22	0	20	42	47	24	157	ø	181	2	143	32	177	358	400
11:30 12:30	5	0	2	7	24	0	21	45	52	20	100	5	191	2	168	20	110	381	433
12:30 13:30	1	2	3	6	17	2	15	34	40	13	157	1	171	2	175	9	186	357	397
15:00 16:00	4	3	3	10	18	0	20	38	48	8	164	1	173	0	347	16	363	536	584
16:00 17:00	1	0	1	2	25	0	24	49	51	17	199	5	221	3	478	24	505	726	m
17:00 18:00	5	0	3		16	1	14	31	39	8	157	3	166	6	441	14	461	627	666
Sub Total	20	8	18	46	140	4	144	268	334	128	1334	15	1477	16	2009	158	2183	3660	3994
U Turns				0				9.0					- 65				୍ଞ	9.5	.0
Total	20	- 8	18	45	140	4	184	288	334	128	1334	15	1477	16	2009	158	2183	3660	3994
EQ 12Hr	28	11	25	64	195	-6	200	400	464	178	1854	21	2053	22	2793	220	3034	5087	5551
Note: These v	alues ar	e calcul	ated by	multiply	ying the	totals b	y the ap	propriate	expans	ion fact	br.		1	.39					
AVG 129tr	25	10	23	58	175	5	180	360	418	160	1669	19	1848	20	2513	198	2731	4579	4997
Note: These v	olumes :	are calc	ulated	by multip	plying th	e Equiv	alent 1	hr. total	s by the	AADT	factor.			90					
AVG 24Hr	33	13	29	75	229	7	236	472	547	210	2186	25	2421	26	3292	259	3578	5999	6546
Note: These v	olumes :	are calc	ulated	by multip	plying th	e Avera	ge Dail	y 12 hr. t	totals by	12 to 2	4 expans	sion fac	tor. 1	.31					

Comments

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

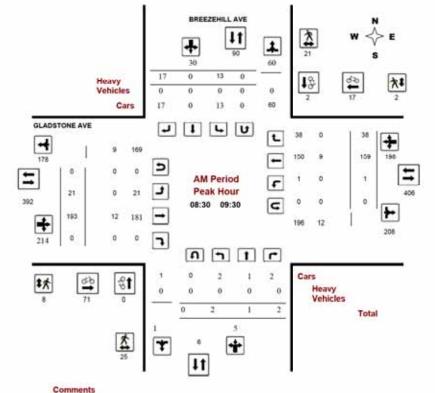


Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 18, 2018
 WO No:
 37971

 Start Time:
 07:00
 Device:
 Miovision



Ottawa

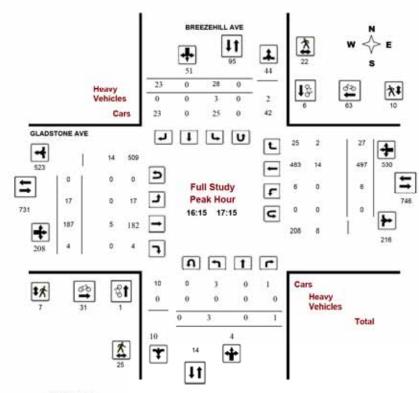
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

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Comments

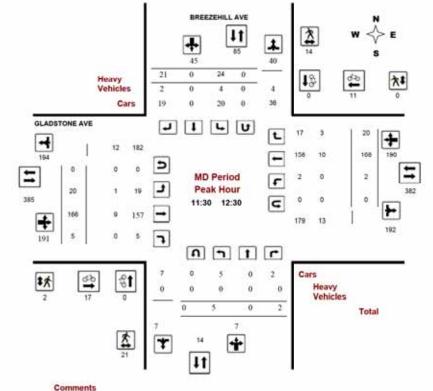


Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ GLADSTONE AVE

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Ottawa

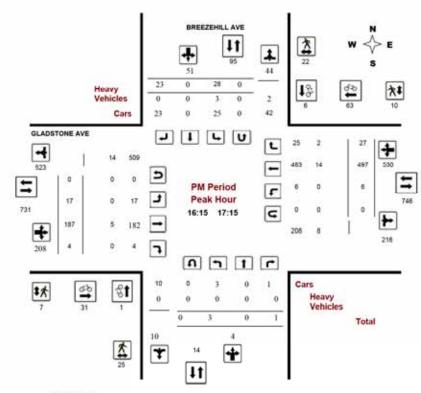
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ GLADSTONE AVE

 Survey Date:
 Wednesday, July 18, 2018
 WO No:
 37971

 Start Time:
 07:00
 Device:
 Miovision



Comments



Work Order 37971

Turning Movement Count - 15 Min U-Turn Total Report

BREEZEHILL AVE @ GLADSTONE AVE

Survey Date:	W	ednesday, July 18	3, 2018	- San (4)		
Time Pe	riod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	.0	0	0	0
15:15	15:30	0	.0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	.0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	.0
17:45	18:00	0	0	0	0	.0
Total	F:	0	0	0	0	0

2018-Oct-04 Page 1 of 1



Transportation Services - Traffic Services w.o.

25204

Turning Movement Count - 15 Minute Summary Report

BREEZEHILL AVE @ SOMERSET ST

Survey	Date:	1	hurse	day, A	ugust	13, 2	015			Tota	Obse	rved	U-Tur	ns					
									iorthbou	7/100	0	19	outhbou		0				
									Eastbou	nd:	4		Vestbou		2				
		Е	REE	ZEHIL	L AVI	E						SOM	ERSE	TST					
		iorthbou	und		So	uthbou	nd		or enaces	E	estboun	d	124	W	estbour	d		School I	02566
Time Period	LT	ST	RT	TOT	LT	ST	RT	TOT	STR	LT	ST	RT	TOT	LT	st	RT	TOT	STR	Grand
07:00 07:1	3 7 7 7	0	2	4	0	0	0	0	4	0	27	5	32	3	21	0	24	56	60
07:15 07:3	0 0	0	6	6	0	0	0	0	6	0	29	3	32	2	35	0	37	69	75
07:30 07:4	5 2	0	3	5	0	0	0	0	5	0	53	2	55	1	35	0	36	91	96
07:45 08:0	0 2	0	4	6	0	0	0	0	6	0	62	3	65	4	54	0	58	123	129
08:00 08:1	5 3	0	6	9	0	0	0	0	9	0	65	5	70	4	56	0	60	130	139
08:15 08:3	9 4	0	5	9	0	0	0	0	9	0	92	3	95	4	42	0	46	141	150
08:30 08:4	5 3	0	4	7	0	0	0	0	7	0	54	5	59	3	40	0	43	102	109
08:45 09:0	0 3	0	9	12	0	0	0	0	12	0	64	4	68	4	50	0	54	122	134
09:00 09:1	5 3	0	6	9	0	0	0	0	9	0	63	0	71	5	68	0	73	144	153
09:15 09:3	5	0	4	9	0	0	0	0	9	0	62	4	66	5	53	0	58	124	133
09:30 09:4	5 6	0	7	13	0	0	0	0	13	0	76	3	79	2	66	0	68	147	160
09:45 10:0	0 2	0	5	7	0	0	0	0	7	Q	59	10	70	2	67	0	69	139	146
11:30 11:4	5 5	0	2	7	0	0	0	0	7	0	81	2	83	3	74	0	77	160	167
11:45 12:0	9	0	3	12	0	0	0	0	12	0	81	3	84	5	69	0	74	158	170
12:00 12:1	5 7	0	12	19	0	0	0	0	19	0	78	8	86	12	80	0	92	178	197
12:15 12:3	0 4	0	8	12	0	۵	0	0	12	0	69	4	74	5	70	0	75	149	161
12:30 12:4	5 6	0	3	9	0	0	0	0	9	0	71	5	76	2	64	0	66	142	151
12:45 13:0	0 1	0	4	5	0	0	0	0	5	0	80	9	89	5	72	0	77	166	171
13:00 13:1	5 4	0	3	7	0	0	0	0	7	0	65	7	72	2	66	0	68	140	147
13:15 13:3	5	0	5	10	0	0	0	0	10	0	49	5	54	6	86	0	92	146	156
15:00 15:1	5 6	0	2	8	0	0	0	0	8	0	65	5	70	2	74	0	76	146	154
15:15 15:3	0 7	0	6	13	0	0	0	0	13	0	66	2	68	5	89	0	94	162	175
15:30 15:4	5 8	0	8	16	0	0	0	0	16	0	58	.4	62	3	93	0	96	158	174
15:45 16:0	0 4	0	2	6	0	0	0	0	6	0	61	4	66	6	75	0	81	147	153
16:00 16:1	5 2	0	7	9	0	ū	0	0	9	0	73	3	76	5	96	Ü	101	177	186
16:15 16:3	3	0	5	8	0	0	0	0	8	0	87	7	94	7	108	0	115	209	217
16:30 16:4	5 5	0	3	8	0	O	0	0	8	0	65	8	73	4	104	0	109	182	190
40.00			2.4						-		100	0.00					***		

Note: U-Turns are included in Totals.

Comment

287

0 2137 147 2288 136 2329

2018-Sep-27 Page 1 of 1



Turning Movement Count - Cyclist Volume Report

Work Order 35301

BREEZEHILL AVE @ SOMERSET ST

Count Date: Thursday, August 13, 2015 Start Time: 07:00

	В	REEZEHILL AV	E		SOMERSET S	r .	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	6	0	6	38	24	62	68
08:00 09:00	17	0	17	99	39	138	155
09:00 10:00	6	0	6	33	28	61	67
11:30 12:30	4	0	4	28	25	53	57
12:30 13:30	2	0	2	21	30	51	53
15:00 16:00	3	0	3	41	34	75	78
16:00 17:00	3	0	3	50	79	129	132
17:00 18:00	4	0	4	59	101	160	164
Total	45	0	45	369	360	729	774

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

2018-Sep-27 Page 1 of 1



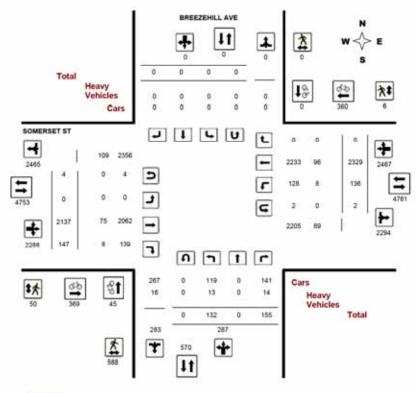
Transportation Services - Traffic Services

Turning Movement Count - Full Study Diagram

BREEZEHILL AVE @ SOMERSET ST

Survey Date: Thursday, August 13, 2015

WO#: 35301 Device: Miovision



Comments

2018-Sep-27 Page 1 of 1



W.O. 35301

Turning Movement Count - Heavy Vehicle Report

BREEZEHILL AVE @ SOMERSET ST

Survey Date: Thursday, August 13, 2015

			BRE	EZEH	ILL A	٧E						sc	MER	SET S	T					
	1	Northb	ound			Southb	ound				Eastb	ound			Westbo	bund				
Time	Period	LT	ST	RT	N	LT	ST	RT	S TOT	STR	LT	ST	RT	E	LT	ST	RT	TOT	STR	Grand
07:00	08:00	2	0	1	3	0	0	0	0	3	0	7	1		2	12	0	14	22	25
00.80	09:00	1	0	2	3	0	0	0	0	3	0	12	0	12	1	15	0	16	28	31
09:00	10:00	4	0	0	10	0	0	0	0	10	0	11	2	13	0	15	0	15	28	38
1:30	12:30	4	0	2	6	0	0	. 0	0	6	0	10	0	10	4	11	0	15	25	31
2:30	13:30	1	0	0	1	0	0	0	0		0	7	3	10	1	13	0	14	24	25
5.00	16:00	1	0	3	4	0	0	0	0	4	0	11	0	11	0	15	0	15	26	30
16.00	17:00	0	0	0	. 0	0	0	0	0	0	0	7	11	8	0	7	0	7	15	15
7:00	18:00	0	0	0	0	0	0	0	0	0	0	10	3.	11	0	8	0	8	19	19
Sub	Total	13	0	14	27	0	0	0	0	27	0	75	8	83	8	96	0	104	187	214
-Turn	s (Heav	ry Vet	icles)		0				0	0				0				0	0	0
To	tat	13	0	14	0	0	0	0	0	27	0	75	0	83		96	0.	104	187	214

2018-Sep-27 Page 1 of 1 2018-Sep-27



Transportation Services - Traffic Services

Work Order 35301

Page 1 of 1

Turning Movement Count - Pedestrian Volume Report

			-	AVE @ SOM			
Count Dat	e: Thursday, A	ugust 13, 2015				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	SS Approach (E or W Crossing)	Total	E8 Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	8	0	8	Ö	0	0	8
07:15 07:30	8	0	8	0	1	1	9
07:30 07:45	14	0	14	1.3	0	1	15
07:45 08:00	15	0	15	2	1	3	18
07:00 08:00	45	. 0	45	3	2	5	50
08:00 08:15	10	.0	10	- 1	1	2	12
08:15 08:30	25	0	25	0	0	0	25
08:30 08:45	14	0	14	3	0	3	17
08:45 09:00	10	0	10	0	0	0	10
00:00 00:00	59	0	59	4	100	5	64
09:00 09:15	13	0	13	0	0	0	13
09:15 09:30	12	0	12	0	0	0	12
09:30 09:45	16	0	16	0	0	0	16
09:45 10:00	9	0	9	0	0	0	9
09:00:10:00	50	0	50	0	0	0	50
11:30 11:45	18	0	18	2	0	2	20
11:45 12:00	8	0	8	3	0	3	11
12:00 12:15	21	0	21	2	0	2	23
12:15 12:30	19	0	19	0	1	1	20
11:30 12:30	66	0	66	7	- 1	8	74
12:30 12:45	21	0	21	0	0	0	21
12:45 13:00	16	0	16	0	0	0	16
13:00: 13:15	16	0	16	0	0	0	16
13:15 13:30	18	0	18	0	1	1	19
12:30 13:30	71	0	71	0	1.	1	72
15:00 15:15	19	0	19	2	0	2	21
15:15 15:30	26	0	26	1	0	1	27
15:30 15:45	21	0	21	1	0	1	22
15:45 16:00	21	0	21	0	0	0	21
15:00 16:00	87	.0	87	4	0	4	91
16:00 16:15	13	0	13	2	0	2	15
16:15 16:30	20	0	20	6	0	6	26
16:30 16:45	24	0	24	3	0	3	27
16:45 17:00	30	0	30	6	0	6	36
16:00 17:00	87	0	87	17	0	17	104
17:00 17:15	26	0	26	3	0	3	29
17:15 17:30	37	0	37	5	1	6	43
17:30 17:45	35	0	35	7	0	7	42
17:45 18:00	25	0	25	0	0	0	25
17:00 18:00	123	.0	123	15	12	16	139
Total	588	Ô	588	50	6	56	644

Comment



Work Order

35301

Turning Movement Count - Full Study Summary Report

BREEZEHILL AVE @ SOMERSET ST Survey Date: Thursday, August 13, 2015 Total Observed U-Turns AADT Factor Northbound: 0 Southbound: 0 90

								-	ull Stu	ldy .									
_			BRE	EZEHI	LL AV	E						SC	MERS	SETS	T				
	N	orthbo	ound		s	outhbo	bund	_	-	- 3	Eastbo	ound			Westbo	ound	_		
Period	LT	ST	RT	NB TOT	ĻŢ	ST	RT	SB TOT	STR	ĻΤ	ST	RT	EB	LT	51	RT	WB	STR	Gran
7:00 08:00	6	0	15	21	0	0	0		21	0	171	13	184	10	145	0	155	339	360
00:00 00:00	13	0	24	37	0	0	0		37	0	275	17	292	15	188	0	203	495	532
9:00 10:00	16	0	22	38	0		0		38	0	260	25	285	14	254	0	268	553	591
11:30 12:30	25	0	25	50	0	.0	0		50	0	309	17	326	25	293	0	318	644	694
12:30 13:30	16	0	15	31	0	.0	0		31	0	265	26	291	15	288	0	303	594	625
15:00 16:00	25	0	18	43	.0	0	0		43	0	250	15	265	16	331	0	347	612	655
16:00 17:00	13	0	19	32	.0	.0	0		32	0	311	21	332	22	422	0	444	776	806
17:00 18:00	18	0	17	35	0		0		35	0	296	13	309	19	408	0	427	736	771
Sub Total	132	0	155	287	0	0	0		287	0	2137	147	2284	138	2329	0	2465	4749	5036
U Turns				0				0	0				4				2	6	6
Total	132	0	155	287	0		0		287	٥	2137	147	2288	138	2329	0	2467	4755	5842
EQ 12Hr	183	0	215	399	0	0	0		399	0	2970	204	3180	189	3237	0	3429	6609	7008
lote: These va	lues an	e calcul	ated by	multiply	ing the t	totals by	the ap	propriate	expansi	on fact	tor.		1	.39					
AVG 12Hr	185	0	194	359	0	0	0		359	0	2673	184	2862	170	2914	0	3086	5948	6307
lote: These vo	lumes :	are calc	ulated	by multip	lying the	e Equiv	alent 12	hr. total	s by the	AADT	factor.		9	90					
AVG 24Hv lote: These vo	218	0	254	479	0	0	0		470	0	3502	241	3750	223	3817	0	4043	7793	8263

Comments

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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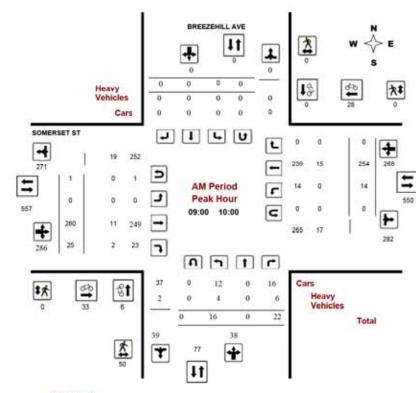
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ SOMERSET ST

 Survey Date:
 Thursday, August 13, 2015
 WO No:
 35301

 Start Time:
 07:00
 Device:
 Miovision



Comments

2018-Sep-27 Page 1 of 4

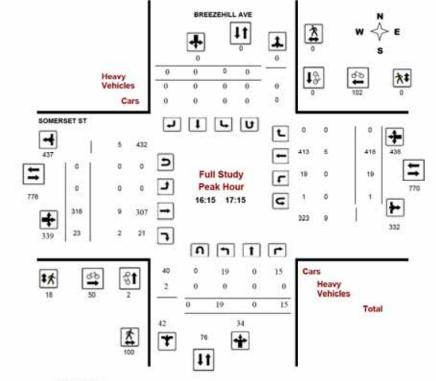


Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ SOMERSET ST

 Survey Date:
 Thursday, August 13, 2015
 WO No:
 35301

 Start Time:
 07:00
 Device:
 Miovision



Comments

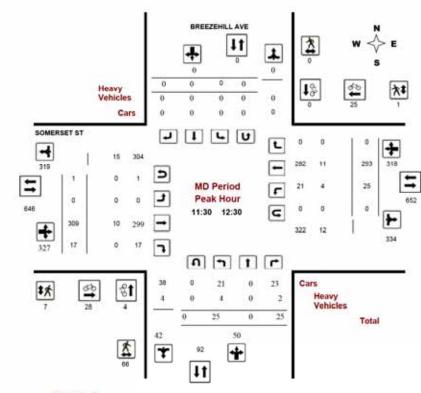


Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ SOMERSET ST

Survey Date: Thursday, August 13, 2015 WO No: 35301 Start Time: 07:00 Device: Miovision



Comments



Turning Movement Count - Peak Hour Diagram

BREEZEHILL AVE @ SOMERSET ST

Survey Date: Thursday, August 13, 2015 WO No: 35301 Start Time: 07:00 Device: Miovision **BREEZEHILL AVE** 1 Heavy Vehicles Cars SOMERSET ST 4 U 0 L 4 432 413 418 438 2 = PM Period 19 F 19 Peak Hour 0 0 770 G 776 1 16:15 17:15 316 307 323 23 7 2 21 339 7 1 6 15 Cars රේච → 50 \$1 Heavy Vehicles 0 15 0 19 0 Total 42 34 100 * + 11

Comments

2018-Sep-27 Page 4 of 4



17:45

Total

18:00

0

Transportation Services - Traffic Services

Work Order 35301

Turning Movement Count - 15 Min U-Turn Total Report

			ZEHILL AV	E@SOME	RSET ST	
Survey Date:	: Th	ursday, August 1	3, 2015			
Time Po	priod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	,0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	1	0	1
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	1	0	1
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13.15	13.30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	1	0	1
16:00	16:15	0	0	0	0	0
16:15	16:30	.0	0	0	0	0
16:30	16:45	0	0	0	1	1
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	1	1
17:30	17:45	0	0	1	0	1

2018-Sep-27

0

0

4

0

2

0

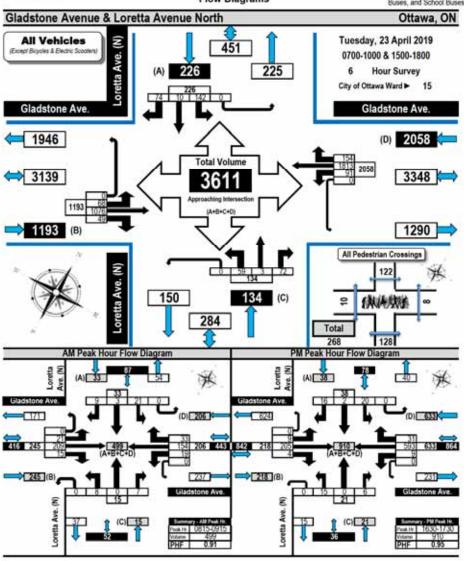
6



Printed on: 4/24/2019

Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses



Prepared by: thetrafficspecialist@gmail.com



Turning Movement Count Summary Report AADT and Expansion Factors

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

Gladst	one	Av	enu	e &	Lor	etta	a Av	eni	ie l	lort	h										Otta	awa,	ON
Survey Da Weather All Weather Pl	N;	Tueso Partly Overc	Clou	dy +10	oril 201 7°C		ırvey	Dura	tion:	6	Hrs.	Sun	t Tim	ours:		0700 0700 Carm	-1000	8 15	16527	T Fa 800	ctor:		0.7
	G	lads	ton		/e.	G	lads We	ton		/e.		L	oret	ta A	2000	N)	L	orett	a A		N)		
Time Period	LT	ST	RT	UT	E/B Tot	LT	ST	RT	UT	WB Tot	Street Total	LT	ST	RT	UT	N/B Tot	LT	ST	RT	UT		Street Total	
0700-0800	- 6	142	18	- 0	166	22	112	24	- 0	158	324	- 1	- 0	- 6	- 0	7	13	3	- 4	- 0	20	27	351
0800-0900	17	214	9	- 0	240	19	140	35	- 0	194	434	- 8	- 0	10	0	18	20	2	3	- 0	25	43	477
0900-1000	16	168	- 11	- 0	195	24	160	27	- 0	211	406	2	- 0	- 7	. 0	9	14	2	- 11	- 0	27	36	442
1500-1600	10	174	- 5	. 0	189	13	356	22	. 0	393	582	26	- 1	35	- 0	62	39	- 1	24	- 0	64	126	
1600-1700	9	188	2	- 0	199	- 4	525	18	0	547	746	11	1	10	0	22	37	. 1	20	- 0	58	80	826
1700-1800	10	190	- 4	0	204	9	518	28	. 0	555	759	- 11	1	4	0	16	19	1	12	.0	32	48	807
Totals	68	1076	49	- 0	1193	91	1813	154	0	2058	3251	59	- 3	72	0	134	142	10	74	- 0	226	360	3611

Equivalent 12 & 24-hour Vehicle Volumes Including the Annual Average Daily Traffic (AADT) Factor
Applicable to the Day and Month of the Turning Movement Count

Expansion factors are applied exclusively to standard weekday 8-hour turning movement counts conducted during the hours of 0700h - 1000h, 1130h - 1330h and 1500h - 1800h

Equ. 12 Hr	n/a	nía	n/a																				
										are cal													
AADT 12-hr	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	15/2									

AADT and expansion factors provided by the City of Ottawa

AM Peak Ho	our Fa	ctor *	•	0.9	31							1		High	est H	ourly	Vehicle	e Volk	ume E	etwe	en 070	10h &	1000h
AM Peak He	LT	ST	RT	UT	TOT	LT	BT	RT	UT	TOT	S.TOT	LT	87	RT	UT	TOT	LT	BT	RT	UT	TOT	8.TOT	G.TOT
0815-0915	21	209	15	0	245	19	154	33	- 0	206	451	8	0	.7	0	15	21	3	9	0	33	48	499

PM Peak Ho	ur Fa	ctor *	•	0.9	35									High	est H	ourly	Vehicle	Volu	ime i	Setwee	in 15	00h &	1800h
PM Peak Hr	LT	ST	RT	UT	TOT	LT	57	RT	UT	TOT	S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UŤ	TOT	S.TOT	6.101
1630-1730	9	205	4	0	218	. 9	593	31	.0	633	851	15	. 0	6	0	21	20	2	16	- 0	38	59	910

Comments:

No traffic issues noted during survey.

Notes

Flow Diagrams: AM PM Peak

1. Includes all vehicle types except bicycles, electric bicycles, and electric scooters.

2. When expansion and AADT factors are applied, the results will differ slightly due to rounding.

Printed on: 4/24/2019 Prepared by thetrafficspecialist@gmail.com Summary All Vehicles



Turning Movement Count Pedestrian Crossings Summary and Flow Diagram



Gladstone Avenue & Loretta Avenue North Ottawa, ON Tuesday, 23 April 2019 Pedestrian Crossings 0700-1000 & 1500-1800 Loretta Ave. (N) 6 Hour Survey City of Ottawa Ward ▶ 15 122 **Grand Total** œ Note The values in the auremary table below and the flow diagram represent the number of pedestrian crossing NOT the number of individual pedestrians crossing. For example, some pedestrians will cross one 128 approach, then another to reach their destination. Accordingly, one pedestrian crossing two approaches. will be recorded as two crossings. Loretta Ave. (N)

Time Period	West Side Crossing Gladstone Ave.	East Side Crossing Gladstone Ave.	Street	South Side Crossing Loretta Ave. (N)	North Side Crossing Loretta Ave. (N)	Street	Grand Total
0700-0800	6	2	8	13	15	28	36
0800-0900	2	2	4	14	16	30	34
0900-1000	0	0	0	16	17	33	34
1500-1600	0	1	111	24	23	47	48
1600-1700	0	0	0	27	28	55	55
1700-1800	2	3	5	34	23	57	55 62
Totals	10	- 8	18	128	122	250	268

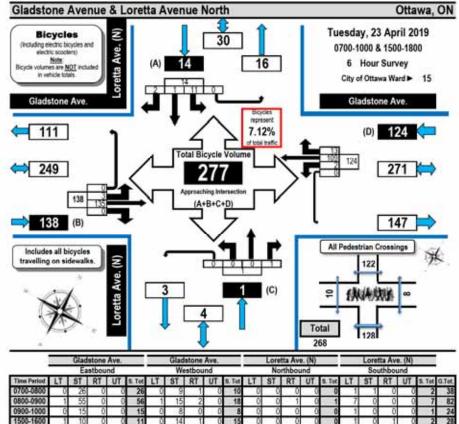
Comments:

No traffic issues noted during survey.



Turning Movement Count Bicycle Summary Flow Diagram





Totals Comments:

1600-1700

No traffic issues noted during survey.

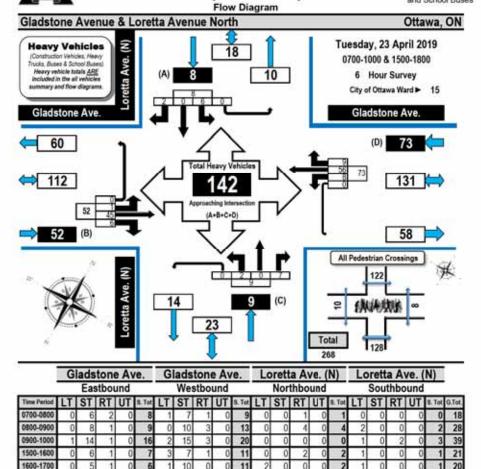
Printed on: 4/24/2019 Prepared by: thetrafficapecialist@gmail.com Summary. Pedestrian Crossings

Printed on: 4/24/2019 Prepared by: thetrafficspecialist@gmail.com Summary: Bicycles



Turning Movement Count Heavy Vehicle Summary

Heavy Trucks, Buses, and School Buses



Totals Comments:

1700-1800

No traffic issues noted during survey.

Printed on: 4/24/2019

Prepared by: thetrafficspecialist@gmail.com

Summary: Heavy Vehicles.



Transportation Services - Traffic Services wo.

37132

Turning Movement Count - 15 Minute Summary Report

GLADSTONE AVE @ PRESTON ST Survey Date: Tuesday, June 20, 2017

Total Observed U-Turns Northbound: 0

Eastbound: 1 Westbound: ()

PRESTON ST **GLADSTONE AVE**

			Northbo	und		So	uthbour	nd			Eas	stbound			We	stbound	1			
Time !	Period	LT	ST	RT	TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E	LT	st	RT	W	STR TOT	Grand Total
07:00	07:15	12	66	16	94	12	82	2	96	190	4	25	10	39	10	27	19	56	95	285
7.15	07:30	3	78	19	100	14	104	3	121	221	2	24	8	34	10	23	12	45	79	300
7:30	07:45	13	85	15	113	10	70	4	84	197	3	29	13	45	В	25	12	45	90	287
37:45	08:00	13	95	15	123	17	90	7	114	237	11	32	10	53	13	24	14	51	104	341
00:80	08:15	12	99	17	128	19	85	3	107	235	7	39	16	62	13	28	21	62	124	359
8 15	08:30	14	100	27	141	18	91	10	119	260	9	56	19	84	17	40	17	74	158	418
8:30	08:45	9	120	21	150	20	91	3	114	264	5	52	14	71	6	49	14	69	140	404
8:45	09:00	10	94	16	120	16	78	0	94	214	10	50	13	73	12	41	16	69	142	356
9:00	09:15	13	81	20	114	21	108	10	139	253	5	27	13	45	12	35	17	64	109	362
9:15	09:30	19	61	18	98	23	94	8	125	223	8	27	12	47	12	40	21	73	120	343
9:30	09:45	14	79	17	110	24	84	11	119	228	9	25	10	45	21	26	15	62	107	336
9:45	10:00	9	73	19	101	16	75	3	94	195	4	30	11	45	11	38	17	66	111	306
1:30	11:45	8	70	20	98	20	63	4	87	185	5	24	14	43	29	22	20	71	114	299
1:45	12:00	9	67	23	99	15	89	13	117	216	2	38	12	52	27	53	27	107	159	375
2:00	12:15	13	79	21	113	11	84	11	106	219	5	35	13	53	30	41	28	99	152	371
2:15	12:30	11	89	25	125	16	67	5	88	213	5	35	14	54	22	44	29	95	149	362
2:30	12:45	7	53	19	79	18	97	11	126	205	6	33	13	52	21	34	27	82	134	339
2:45	13:00	16	63	27	106	21	77	4	102	208	8	32	19	59	22	44	25	91	150	358
3:00	13:15	13	89	19	121	14	83	9	106	227	9	40	19	68	24	44	21	89	157	384
3:15	13:30	12	73	22	107	29	75	8	112	219	10	35	11	56	21	41	21	83	139	358
5:00	15:15	15	84	29	128		95	8	111	239	5	39	26	70	24	62	27	113	183	422
5:15	15:30	21	118	24	163	17	97	9	123	286	6	40	11	57	16	64	15	95	152	438
5:30	15:45	12	98	16	126	13	73	10	96	222	5	39	14	58	26	92	20	138	196	418
5:45	16:00	13	108	16	137	12	86	6	104	241	10	48	14	72	19	96	15	130	202	443
6:00	16:15	27	118	19	164	15	83	9	107	271	4	40	8	52	22	97	14	133	185	456
6:15	16:30	14	104	22	140	12	95	10	117	257	3	48	11	62	18	107	20	145	207	464
6:30	16:45	16	83	29	128	9	86	8	103	231	6	51	13	70	24	101	20	145	215	446
6:45	17:00	22	92	23	137	15	103	10	128	265	3	43	6	52	18	114	24	156	208	473
7:00	17:15	24	95	27	146	16	80	21	117	263	8	45	8	61	20	117	13	150	211	474
7:15	17:30	12	79	24	115	7	85	8	100	215	1	40	9	50	24	100	20	144	194	409
7:30	17:45	13	86	31	130	15	71	7	93	223	8	48	8	64	26	85	25	136	200	423
7:45	18:00	13	84	12	109	6	94	11	111	220	8	45	12	65	27	78	20	125	190	410
TOTAL	.:	432	2763	668	3863	499	2735	246	3480	7343	194	1214	404	1813	605	1832	626	300	53 4876	12219

Note: U-Turns are included in Totals



Turning Movement Count - Cyclist Volume Report

Work Order 37132

GLADSTONE AVE @ PRESTON ST

Count Date: Tuesday, June 20, 2017 Start Time: 07:00

		PRESTON ST			SLADSTONE A	VE	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	0	8	8	19	14	33	41
00:00 00:00	10	14	24	47	25	72	96
09:00 10:00	7	8	15	15	21	36	51
11:30 12:30	5	9	14	10	8	18	32
12:30 13:30	8	10	18	4	14	18	36
15:00 16:00	8	8	16	17	15	32	48
18:00 17:00	11	13	24	20	42	62	86
17:00 18:00	7	8	15	27	38	65	80
Total	56	78	134	159	177	336	470

Comment

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

2018-Oct-04 Page 1 of 1



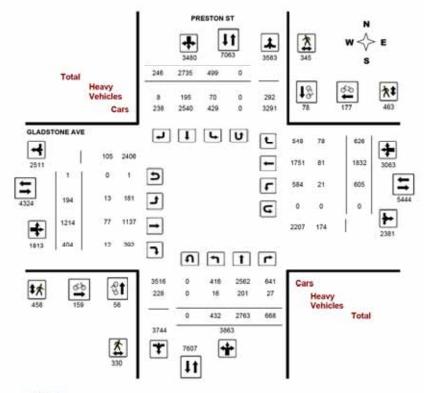
Transportation Services - Traffic Services

Turning Movement Count - Full Study Diagram

GLADSTONE AVE @ PRESTON ST

Survey Date: Tuesday, June 20, 2017.

WO#: 37132 Device: Miovision



Comments



W.O. 37132

Turning Movement Count - Heavy Vehicle Report

GLADSTONE AVE @ PRESTON ST

Survey Date: Tuesday, June 20, 2017

			PF	REST	ON ST							GLA	DST	ONE A	VE					
	-	Northi	ound			South	ound	=			Eastb	bund		1	Westbo	ound	-			
Time !	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E	LT	ST	RT	W	STR	Grand
07:00	08:00	1	26	5	32	13	22	1	36	68	2	12	1	15	0	9	5	14	29	97
00.80	09:00	1	28	5	34	11	26	0	37	71	4	10	3	17	4	12	10	26	43	114
00:00	10:00	4	33	5	42	12	40	0	52	94	2	12	4	18	3	13	12	28	46	140
11:30	12:30	1	25	2	28	8	29	4	39	67	1	9	1	11	3	9	14	26	37	104
12:30	13:30	5	27	2	34	15	22	0	37	71	1	10	2	13	7	11	11	29	42	113
15:00	16:00	3	27	4	34	3	24	3	30	64	3	6	1	10	3	12	8	23	33	97
16.00	17:00	1	21	3	25	7	18	0	25	50	0	11	0	11	0	9	11	20	31	81
17:00	18:00	0	14	1	15	3	14	0	17	32	0	7	0	7	1	6	70	14	21	53
Sub	Total	16	201	27	244	70	195	8	273	517	13	77	12	102	21	81	78	180	282	799
J-Turn	n (Heav	ry Vet	nicles)		0				0	0				0				0	0	0
To	tal	16	201	27	0	70	195	8	273	517	13	77	12	102	21	81	78	180	282	799

2018-Oct-04 Page 1 of 1 2018-Oct-04



Transportation Services - Traffic Services

Work Order 37132

Turning Movement Count - Pedestrian Volume Report

			ADSTON	E AVE @ PRE	210021		
Count Date: 1	Tuesday, Jur	ne 20, 2017				Start Time:	07:00
	B Approach or W Crossing)	SS Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	1	3	4	1	3	4	8
07:15 07:30	7	5	12	3	4	7	19
07:30 07:45	7	10	17	. 0	12	20	37
07:45 08:00	19	6	25	12	14	26	51
07:00 08:00	34	24	58	24	33	57	115
08:00 08:15	10	7	17	9	16	25	42
08:15 08:30	. 8	9	17	4	15	19	36
08:30 08:45	26	22	48	21	40	61	109
08:45 09:00	13	16	29	10	23	33	62
08:00 09:00	57	54	111	44	94	138	249
09:00 09:15	12	.6	18	3	9	12	30
09:15 09:30	8	6	14	5	14	19	33
09:30 09:45	4	9	13	9	3	12	25
09:45 10:00	9	10	19	10	12	22	41
09:00 10:00	33	31	64	27	38	65	129
11:30 11:45	9	4	13	8	14	22	35
11:45 12:00	8	9	17	11	9	20	37
12:00 12:15	14	13	27	18	13	31	58
12:15 12:30	14	9	23	27	8	35	58
11:30 12:30	45	35	80	64	44	108	188
12:30 12:45	10	9	19	16	20	36	55
12:45 13:00	8	8	16	13	16	29	45
13:00 13:15	6	11	17	20	14	34	51
13:15 13:30	4	6	10	12	12	24	34
12:30 13:30	28	34	62	61	62	123	185
15:00 15:15	6	7	13	16	14	30	43
15:15 15:30		11	20	10	19	29	49
15:30 15:45	10	5	15	17	11	29	43
15:45 16:00	21	11	32	11	13	24	56
15:00 16:00	46	34	80	54	57	111	191
16:00 16:15	8	14	22	22	14	36	58
16:15 16:30	10	15	25	20	16	36	61
16:30 16:45	10	16		33		55	81
	16	12	26 28		22	40	68
16:45 17:00	222			28	12	377	
16:00 17:00	44	57	101	103	64	167	268
17:00 17:15	15	16	31	23	18	41	72
17:15 17:30	13	17	30	18	22	40	70
17:30 17:45	9	18	27	13	8	21	48
17:45 18:00	6	25	31	27	23	50	81
17:00 18:00	43	76	119	81	71	152	271
Total	330	345	675	458	463	921	1596

Comment:



Work Order 37132

Turning Movement Count - Full Study Summary Report

GLADSTONE AVE @ PRESTON ST

Survey Date: Tuesday, June 20, 2017 Total Observed U-Turns AADT Factor
Northbound: 0 Southbound: 0 .90
Eastbound: 1 Wesbound: 0

								Eastboo	ind: 1		Wes	sbound	0						
								F	ull St	udy									
			P	RESTO	ON ST	-						GL	ADSTO	NE A	VE				
	- 9	Northb	ound	l viine		Southb	ound				Eastb	ound	w/ 1/2 f		Westb	ound	la cale		
Period	LT	ST	RT	NB TOT	ĹŦ	ST	RT	SB TOT	STR	LT	st	RT	EB	LT	ST	RT	WB	STR	Grand
07:00 06:00	41	324	65	430	53	345	16	415	845	20	110	41	171	41	99	57	197	368	1213
08:00 09:00	45	413	81	539	73	345	18	434	973	31	197	62	290	48	158	68	274	564	1537
09:00 10:00	55	294	74	423	84	361	32	477	900	26	109	46	181	58	139	70	265	445	1346
11:30 12:30	41	305	89	435	62	303	33	298	833	17	132	53	202	108	160	104	372	574	1407
12:30 13:30	48	278	87	413	82	332	32	445	859	33	140	62	235	88	163	94	345	580	1435
15:00 16:00	81	408	85	554	50	351	33	434	968	26	166	65	257	85	314	77	476	733	1721
18:00 17:00	79	397	93	569	51	367	37	455	1024	18	182	38	236	82	419	78	579	815	1839
17:00 18:00	62	344	94	500	44	330	47	421	921	25	178	37	240	97	380	78	555	795	1716
Sub Total	432	2763	888	3863	499	2735	248	3400	7343	194	1214	404	1812	605	1832	626	3063	4875	12218
U Turns				୍ଦ				.0	0				(1)					1.	- 1
Total	432	2763	668	3863	499	2735	246	3480	7343	194	1214	404	1813	605	1832	626	3063	4876	12219
EQ 12HV	900	3841	929	5370	894	3802	342	4837	10207	270	1587	562	2520	841	2548	870	4258	6778	16985
Note: These	values a	re calcu	lated b	y multipl	ying the	totals b	y the a	ppropria	te expans	ion fac	tor.		1	1.39					
AVG 12Hr	540	3457	836	4833	834	3421	308	4353	9186	243	1519	505	2268	757	2292	783	3832	6100	15286
Note: These	volumes	are cal	culated	by multi	plying t	he Equiv	valent 1	2 hr. tota	als by the	AADT	factor.			90					172.5.
AVG 24Hr	708	4528	1096	6331	818	4482	403	5703	12034	318	1990	662	2971	991	3002	1026	5020	7991	20025

Comments

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. 1,31

2018-Oct-04 Page 1 of 1



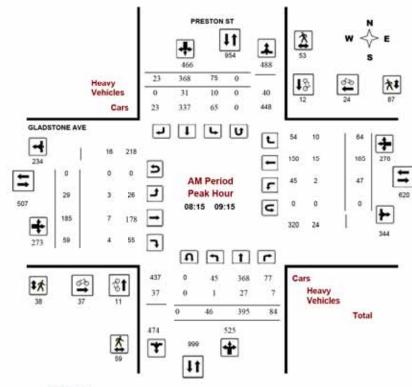
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ PRESTON ST

 Survey Date: Tuesday, June 20, 2017
 WO No:
 37132

 Start Time: 07:00
 Device:
 Miovision



Comments

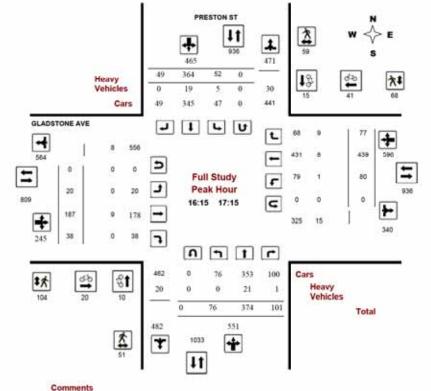


Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ PRESTON ST

 Survey Date:
 Tuesday, June 20, 2017
 WO No:
 37132

 Start Time:
 07:00
 Device:
 Miovision



Ottawa

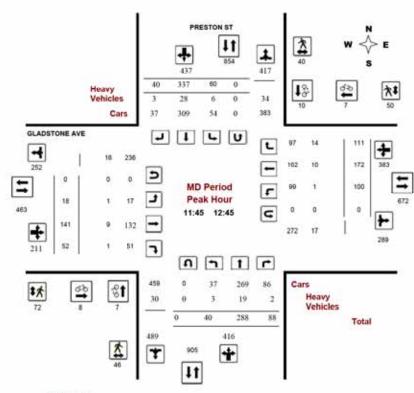
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ PRESTON ST

 Survey Date: Tuesday, June 20, 2017
 WO No:
 37132

 Start Time:
 07:00
 Device:
 Miovision



Comments

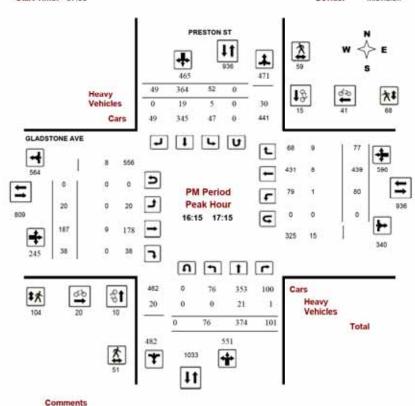


Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ PRESTON ST

 Survey Date:
 Tuesday, June 20, 2017
 WO No:
 37132

 Start Time:
 07:00
 Device:
 Microsion



2018-Oct-04 Page 4 of 4



Transportation Services - Traffic Services

Work Order 37132

Turning Movement Count - 15 Min U-Turn Total Report

GLADSTONE AVE @ PRESTON ST

Survey Dat	e:	Tuesday, June 20,	2017			
Time I	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07;30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	- 1	0	1
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12.15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	.0	0	0	0	0
15:15	15:30	.0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	.0	0	0	0
17:45	18:00	0	.0	0	0	0
To	otal	0	0	1	0	10

Appendix C

Synchro Intersection Worksheets – Existing Conditions



951 Gladstone & 145 Loretta Existing - AM Peak Hour

ntersection													
nt Delay, s/veh	1.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
ane Configurations	- 100	4			4			4			4	V Section 1	
Traffic Vol. veh/h	23	223	0	- 1	129	41	2	1	- 5	17	0	23	
Future Vol., veh/h	23	223	0	1	129	41	2	1	5	17	0	23	
Conflicting Peds, #hr	21	0	25	25	0	21	8	. 0	2	2	. 0	8	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	1000	-	None		-	None			None	econolis	and the last	None	
Storage Length	-		+					-	+				
Veh in Median Storage		- 0		-	- 0	- 4		0	- 4		- 0	-	
Grade, %		0			- 0		-	0			0		
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	26	248	0	- 1	143	46	2	- 1	6	19	- 0	26	
Major/Minor 1	lajor1		- 1	Major2		1	Minor1			Minor2			
Conflicting Flow All	210	0	0	273	0	. 0	514	537	275	495	514	195	
Stage 1	11100	- 9	-	and the	-	-	325	325	200	189	189	100	
Stage 2						- 4	189	212		306	325		
Critical Howy	4.12	12	- 2	4.12		9 64	7.12	6.52	6 22	7.12	6.52	8 22	
Critical Howy Stg 1			-		,		6.12	5.52		6.12	5.52		
Critical Howy Stg 2	- 14	- 4	- +	-		-	6.12	5.52	- 4	6.12	5.52		
Follow-up Hdwy	2.218	- 4	-	2.218			3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1361			1290			471	450	764	485	464	846	
Stage 1		-	- +	+		- 4	687	649		813	744		
Stage 2	1+	9	- 4			-	813	727	-	704	649	1.5	
Platoon blocked, %		-	-			-							
Mov Cap-1 Maneuver	1339	- 1	-	1265		-	437	424	748	463	437	827	
Mov Cap-2 Maneuver		-	-	+			437	424	+	463	437		
Stage 1		- 4			-	-	658	622		781	731		
Stage 2	-	- 1	-		-		782	715	-	680	622		
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.7			0			11.2			11,3			
HCM LOS							В			В			
			_		_								
Minor Lane/Major Mvm		NBLn1	EBL	EBT	EBR	WBL		WBR					
Capacity (veh/h)		587	1339	+		1265			620				
HCM Lane V/C Ratio		0.015			_	0,001			0.072				
HCM Control Delay (s)	1	11.2	7.7	0		7.8	0		11.3				
HCM Lane LOS		8	A	A		A	· A		В				
HCM 95th %tile Q(veh)	1	0	0.1			0		-	0.2				

	,	-	1	1	-	•	1	Ť	-	-	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	-	44.	4.05	7	1.	· variable	7	1.		7	1.	
Traffic Volume (vph)	27	171	55	47	154	64	43	395	84	75	368	2
Future Volume (vph)	27	171	55	47	154	64	43	395	84	75	368	22
Satd. Flow (prot)	0	1616	.0	1658	1588	0	1658	1643	0	1658	1722	6.0
Fit Permitted		0.935		0.477			0.468			0.394		
Satd. Flow (perm)	0	1506	.0	775	1588	0	787	1643	0	643	1722	- (
Satd. Flow (RTOR)		19			29			25			7	
Lane Group Flow (vph)	0	281	0	52	242	0	48	532	0	83	433	- (
Turn Type	Perm	NA:		Perm	NA.		Perm	NA.		Perm	NA.	
Protected Phases		4			8			2			6	
Permitted Phases	- 4			8			2			- 6		
Detector Phase	4	4		- 8	- 8		2	2		- 6	- 6	
Switch Phase				10.0	- 27							
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.5	24.5		24.5	24.5		23.7	23.7		23.7	23.7	
Total Split (s)	25.0	25.0		25.0	25.0		45.0	45.0		45.0	45.0	
Total Solit (%)	35.7%	35.7%		35.7%	35.7%		64.3%	64.3%		64.3%	64.3%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	3.5	3.5		3.5	3.5		2.4	2.4		24	2.4	
Lost Time Adjust (s)	1000	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.5		6.5	6.5		5.7	5.7		5.7	5.7	
Lead/Lag		0.0		- 0.5	0.0		0.1	2.1		9.1	0.1	
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C.Min	C-Mn		C-Min	C-Min	
Act Effct Green (s)	DALIER	16.6		16.6	16.6		41.2	41.2		41.2	41.2	
Actuated g/C Ratio		0.24		0.24	0.24		0.59	0.59		0.59	0.59	
v/c Ratio		0.76		0.28	0.61		0.10	0.54		0.22	0.43	
Control Delay		36.3		24.7	27.1		8.3	11.7		10.0	10.2	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		36.3		24.7	27.1		8.3	11.7		10.0	10.2	
LOS		100000000000000000000000000000000000000		100000000000000000000000000000000000000			0.3 A			10.0	10000	
9.0.0		36.3		С	26.7		A	11.4		A	10.1	
Approach Delay					20.7 C			11.4 B			10.1 B	
Approach LOS		D		55			2.6	37.0		112.00	28.4	
Queue Length 50th (m)		31.3			24.3					4.8	-	
Queue Length 95th (m)		53.6		13.7	43.1		7.6	68.1		12.8	51.2	
Internal Link Dist (m)		300.5		27.5	149.8			122.5			139.6	
Turn Bay Length (m)		40.0		37.5	-		24.0			28.0		
Base Capacity (vph)		424		211	454		470	992		384	1031	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	- 0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced v/c Ratio		0.66		0.25	0.53		0.10	0.54		0.22	0.42	
Intersection Summary												
Cycle Length: 70 Actuated Cycle Length: 70												
Offset: 37 (53%), Reference Natural Cycle: 60	ed to phase	2.NBTL	and 6:SB	TL, Start	of Green							
Control Type: Actuated-Co	ordinated											
CONTROL TYPE PROMISED CO	Comment.											

Synchro 11 Report Page 2 Synchro 11 Report Page 3

Lanes, Volumes, Timings 2: Preston & Gladstone

951 Gladstone & 145 Loretta Existing - AM Peak Hour

Maximum v/c Ratio: 0.76		
Intersection Signal Delay: 17.9	Intersection LOS: B	
Intersection Capacity Utilization 86.4%	ICU Level of Service E	
Analysis Period (min) 15		

Splits and Phases: 2: Preston & Gladstone

HCM 2010 TWSC 3: Breezehill & Somerset 951 Gladstone & 145 Loretta Existing - AM Peak Hour

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1.			4	Y	
Traffic Vol., veh/h	260	25	14	254	16	22
Future Vol., veh/h	260	25	14	254	16	22
Conflicting Peds, #hr	0	50	50	.0	0	. 0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	mining.	None	and the same	None	10000	None
Storage Length	-				0	
Veh in Median Storage.	# 0			- 0	- 0	-
Grade, %	0	-		0	0	
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	. 2	2	2	2	2	2
Myrnt Flow	289	28	16	282	18	24
	- Cherlan	-	-			-
Major/Minor N	lajor1		Major2		Minort	
Conflicting Flow All	0		367	0	667	353
Stage 1			307	-	353	333
Stage 2	-:				314	
Critical Hdwy			4 12	- 1	6.42	6.22
Critical Howy Stg 1	-:		4.12.		5.42	0.77
Critical Howy Stg 2					5.42	V V
Follow-up Hdwy			2.218		3.518	3.318
Pot Cap-1 Maneuver	:+		1192	-	CONTRACTOR AND	691
Stage 1			1192		711	693
Stage 1			-		741	
			-		:191	-
Platoon blocked, %	-	-	1146	-	401	201
Mov Cap-1 Maneuver	-		22,14			664
Mov Cap-2 Maneuver	-		-	-	401	+
Stage 1		-		-	683	
Stage 2	-	- 1	-		728	- +
Approach	EB		WB.		NB	4
HCM Control Delay, s	0		0.4		12.5	
HCM LOS			-,71.7		В	
E-10.00 (-10-	
Minor Lane/Major Mymt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	Married Mr.	520	CDI		1146	HOL
HCM Lane V/C Ratio		0.081			0.014	
		12.5	-		8.2	0
HCM Control Delay (s) HCM Lane LOS		12.5 B			O.Z	A
			-			
HCM 95th %tile Q(veh)		0.3	-	-	- 0	-

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			44.			-4-	
Traffic Vol. veh/h	- 21	209	15	19	154	33	8	0	7	21	3	9
Future Vol. yeh/h	21	209	15	19	154	33	- 8	0	7	21	3	9
Conflicting Peds, #hr	15	0	13	13	0	15	3	. 0	2	2	. 0	3
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	1000		None		-	None	THE REAL PROPERTY.	-	None	-comb	05000	None
Storage Length	-	-	+	-				-	-			
Veh in Median Storage		0	- 4	-	- 0		- 12	0	- 2		- 0	-
Grade, %		0	-	-	- 0		-	0		-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	23	232	17	21	171	37	9	0	- 8	23	3	10
1000	-	-		-			- 12		-	-		
Major/Minor 1	lajor (- 0	Major2		- 1	Minor1		- 13	Minor2		
Conflicting Flow All	223	0	0	262	. 0	0	541	565	256	540	555	208
Stage 1	0953			-			300	300	-	247	247	DANSE
Stage 2		-		-		-	241	265		293	308	
Critical Hdwy	4.12	- 5	- 2	4.12		8 84	7.12	6.52	6.22	7.12	6.52	8 22
Critical How Stg 1							6.12	5.52		6.12	5.52	
Critical Howy Stg 2	- 1			-		-	6.12	5.52	-	6.12	5.52	
	2.218	-		2.218			3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1346			1302	-		452	434	783	453	440	832
Stage 1	100,00		-			-	709	666	-	757	702	
Stage 2	1.0	- 9	- 4			-	762	689	-	715	660	
Platoon blocked, %		-	-			-		-				
Mov Cap-1 Maneuver	1330	-	-	1289		- 4	425	408	774	429	414	820
Mov Cap-2 Maneuver			-			,	425	408	-	429	414	The Parket
Stage 1		- 4	-			-	688	646	-	734	680	-
Stage 2						-	733	668		693	640	
				-								
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0.7			11.9			12.9		
HCM LOS	-			-			В			В		
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WER	58Ln1			
Capacity (veh/h)		538	1330	-		1289	11 (*		491			
HCM Lane V/C Ratio		0.031	0.018	,		0.016			0.075			
HCM Control Delay (s)		11.9	7.8	0		7.8	0	- 8	12.9			
HCM Lane LOS		8	A	A		A	A	-	8			
HCM 95th %tile Q(veh)		0.1	0.1			0.1	- 1	- 4	0.2			

Intersection												
Intersection Delay, siveh	7.5											
Intersection LOS	A											
	- 1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4.			44.	
Traffic Vol. veh/h	- 11	26	10	6	8	- 6	11	49	10	22	31	16
Future Vol., veh/h	- 11	26	10	6	- 8	- 6	11	49	10	- 22	31	16
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	12	29	- 11	7	9	7	12	54	11	24	34	18
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			-10			. 1.		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1.			1			51		
Conflicting Approach Right	NB			SB			WB			EB.		
Conflicting Lanes Right	1			1			1010			- 1		
HCM Control Delay	7.5			7.3			7.5			7.5		
HCM LOS	A			A			A			A		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		16%	23%	30%	32%							
Vol Thru, %		70%	55%	40%	45%							
Vol Föght, %		14%	21%	30%	23%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		70	47	20	69							
LT Vol		- 11	- 11	- 6	22							
Through Vol		49	26	8	31							
RT Vol		10	10	6	16							
Lane Flow Rate		78	52	22	77							
Geometry Grp		1	- 1	- 1	- 1							
Degree of Util (X)		0.088	0.06	0.025	0.086							
Departure Headway (Hd)		4.067	4.136	4.12	4.047							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		875	855	857	879							
Service Time		2.12	2.212	2.204	2.101							
HCM Lane V/C Ratio		0.089	0.061	0.026	0.088							
HCM Control Delay		7.5	7.5	7.3	7.5							
HCM Lane LOS		A	A	A	A							
HCM 95th-tile Q		0.3	0.2	0.1	0.3							

Lanes, Volumes, Timings 8: Bayswater & Gladstone 951 Gladstone & 145 Loretta Existing - AM Peak Hour

•	-	•	1	-	-	1	T	-	*	+	*
EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
	4			4			4			4	
23	174	16	14	105	35	19	106	22	50	124	3
23	174	16	14	105	35	19	106	22	50	124	3
0	1706	0	0	1657	0	0	1688	0	0	1673	
	0.960			0.966			0.946			0.894	
- 0	1641	0	0	1606	0	0	1606	0	0	1510	
	9	4700		32	-		16			20	
0	237	0	0	172	- 0	0	163	- 0	0	235	
Perm	NA.		Perm	NA.		Perm	NA.		Perm	NA.	
	2			6			- 8			4	
2			6	100		8	-		- 4		
2	2		- 6	- 6		8	- 8		4	- 4	
11.7				-							
10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
22.5	22.5		22.5	22.5		22.5	22.5		22.5	22.5	
33.0	33.0		33.0	33.0		27.0	27.0		27.0	27.0	
55.0%	55.0%		55.0%	55.0%		45.0%	45.0%		45.0%	45.0%	
-			-			-			NAME OF TAXABLE		
	0.0			2.0			0.0			0.0	
CMax	C-May		C-May	C-May		May	Max		Mare	Max	
									and a		
										0.42	
				70.00							
	99.1			01,0			110.0			20.4	
	757			753			501			658	
	The second second									1 1	
	0.51			0.23			0.60			0.46	
	-										
ed to phase	2 EBTL:	and 6:WE	TL, Star	of Green							
	1										
	23 23 0 0 0 Perm 2 2 2 10.0 22.5 33.0 55.0% 3.0 2.5	EBL EBT 23 174 23 174 0 1706 0 960 0 1641 9 0 237 Perm NA 2 2 2 2 2 100 100 225 225 330 330 550% 550% 30 30 25 25 00 555 C-Max C-Max 275 0 46 0 31 113 8 8 113 8 8 113 8 8 8 8	EBL EBT EBR 23 174 16 23 174 16 0 1706 0 0 960 0 1641 0 9 0 237 0 Perm NA 2 2 2 2 2 100 100 225 225 330 330 550% 550% 30 30 25 25 00 55 C-Max C-Max 275 046 031 113 8 113 8 113 8 148 278 95.1 757 0 0 0 0 0 0 0 31	EBL EBT EBR WBL 23 174 16 14 23 174 16 14 0 1706 0 0 0 0 960 0 1641 0 0 9 0 237 0 0 Perm NA Perm 2 2 2 6 100 100 100 100 225 225 225 330 330 330 550% 550% 550% 30 30 30 25 25 25 00 55 C-Max C-Max 275 046 031 113 B 113 B 113 B 113 B 148 278 95.1 757 0 0 0 0 0 031	EBL EBT EBR WBL WBT 23 174 16 14 105 23 174 16 14 105 0 1706 0 0 1657 0 960 0 966 0 1641 0 0 1606 9 32 0 237 0 0 172 Perm NA Perm NA 2 6 2 6 2 7 6 6 2 2 6 6 10.0 10.0 10.0 10.0 22.5 22.5 22.5 22.5 22.5 33.0 33.0 33.0 33.0 33.0 33.0 35.50% 55.0% 55.0% 55.0% 55.0% 55.0% 55.5 55 C-Max C-Max 27.5 27.5 0.6 0.46 0.46 0.31 0.23 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 0.0 11.3 8.9 0.0 0.0 0.0 0.0 11.3 8.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	EBL EBT EBR WBL WBT WBR 23 174 16 14 105 35 23 174 16 14 105 35 0 1706 0 0 1657 0 0 960 0 .966 0 1641 0 0 1606 0 9 32 0 237 0 0 172 0 Perm NA Perm NA 2 6 2 2 6 6 2 2 2 6 6 10.0 10.0 10.0 10.0 225 225 225 225 33.0 33.0 33.0 33.0 33.0 55.0% 55.0% 55.0% 55.0% 55.0% 3.0 3.0 3.0 3.0 3.0 2.5 2.5 2.5 2.5 2.5 0.0 0 0.0 5.5 CMax C-Max C-Max C-Max 27.5 0.46 0.46 0.31 0.23 11.3 8.9 8 A 11.3 8.9 8 B A 11.3 8.9 8 B A 11.3 8.9 8 B A 14.8 8.6 27.8 18.4 95.1 81.5	EBL EBT EBR WBL WBT WBR NBL 23 174 16 14 105 35 19 0 1706 0 0 1657 0 0 0 960 0 966 0 1641 0 0 1606 0 0 9 32 0 237 0 0 172 0 0 Perm NA Perm NA Perm NA Perm 2 6 8 2 2 6 6 6 8 2 2 2 6 6 6 8 40.0 10.0 10.0 10.0 10.0 225 225 22.5 22.5 33.0 33.0 33.0 33.0 33.0 37.0 55.0% 55.0% 55.0% 55.0% 55.0% 55.0% 55.0% 55.0% 55.0% 55.5 55 CMax C-Max C-Max C-Max 27.5 0.46 0.46 0.31 0.23 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 11.3 8.9 0.0 0.0 0.0 11.3 8.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	EBL EBT EBR WBL WBT WBR NBL NBT 23 174 16 14 105 35 19 106 0 1706 0 0 1657 0 0 1688 0.960 0.966 0.966 0 1641 0 0 1606 0 0 1606 9 32 16 0 237 0 0 172 0 0 163 Perm NA Perm NA Perm NA 2 6 8 2 2 6 6 8 8 2 2 2 6 6 8 8 2 2 2 6 6 8 8 33.0 33.0 33.0 33.0 27.0 27.0 55.0% 55.0% 55.0% 55.0% 45.0% 45.0% 45.0% 3.0 3.0 3.0 3.3 3.3 3.0 3.0 3.3 3.3 3.0 3.0	### Color Co	### Color Co	### CAMERY CAMERY CAMERY MAY MAY MAY MAY CAMERY CAM

Synchro 11 Report Page 11

Lanes, Volumes, Timings 8: Bayswater & Gladstone

951 Gladstone & 145 Loretta Existing - AM Peak Hour

Maximum v/c Ratio: 0.42		
Intersection Signal Delay: 12.7	Intersection LOS: B	
Intersection Capacity Utilization 46.5%	ICU Level of Service A	
Analysis Period (min) 15		

Splits and Phases: 8: Bayswater & Gladstone

Synchro 11 Report Page 12 951 Gladstone & 145 Loretta Existing - PM Peak Hour

Int Delay, s/veh	1.5											
			con	11000	Her	Hinn	No.	NO.	NO.	000	COOT	200
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44.	- 22		4		222	•4•	1
Traffic Vol. vehih	17	189	4	6	591	27	- 3	0	- 1	28	0	23
Future Vol., veh/h	17	189	4	6	591	27	- 3	0	1	28	0	23
Conflicting Peds, #hr	22	- 0	25	25	- 0	22	7	. 0	10	10	0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None		-	None	1		None	-	ASSESSED OF	None
Storage Length	-		-	-			1	-	-	- 4		
Veh in Median Storage		- 0	-		- 0		- 6	- 0		- 4	- 0	-
Grade, %		- 0		-	- 0	-	-	0			- 0	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	19	210	- 4	- 7	657	30	- 3	0	1	31	0	26
MajoriMinor	Majorf		14	Major2		- 1	Minor1		14	Minor2		
DE CONTRACTOR	709		_	-	-		981	998	247	969	985	701
Conflicting Flow All			0	239	0	0			_	708	708	-
Stage 1	107		- 1		-	- 4	275	275				
Stage 2	4.47			4.12			706	723 6.52	0.00	261	277 6.52	8 22
Critical Howy	4.12	- 2	-					5.52	6.22	7.12	5.52	
Critical Hdwy Stg 1	_			_	-	-	6.12		-	6.12		
Critical Howy Stg 2	0.040		-	-		- 4	6.12	5.52	2.040	6.12	5.52	0.040
Follow-up Hdwy	2.218	- 12	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	
Pot Cap-1 Maneuver	890			1328	-		229	244	792	233	248	439
Stage 1	- 10			+	-	-	731	683		426	438	
Stage 2	10	15					427	431	+	744	681	1.0
Platoon blocked, %	1000	-	-		-	-	1000			1000		
Mov Cap-1 Maneuver	875		-	1302		- 4	205	227	771	221	231	429
Mov Cap-2 Maneuver			+	+	-	-	205	227	+	221	231	
Stage 1		- 4	-		-	- 4	699	653	-	408	427	
Stage 2	-	- 27		•			396	420	-	719	651	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0.1			19.6			20.9		
HCM LOS							C			C		
Minor Lane/Major Mvin		NBLnt	EBL	EBT	EBR	WBL	MOT	WBR	001			
	m.				LOK		_	NON				
Capacity (veh/h)		251	875			1302			283			
HCM Lane V/C Ratio		0.018	0.022		_			-	0.2			
HCM Control Delay (s)	1	19.6	9.2	0		7.8	0		20.9			
HCM Lane LOS		C	. A	A		A	· A		C			
HCM 95th %tile Q(veh	1	0.1	0.1			0		-	0.7			

	,	-	1	•	-	•	1	1	-	-	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		44		7	1.		7	1.		7	1.	
Traffic Volume (vph)	20	183	37	80	482	77	83	374	101	52	364	- 5
Future Volume (vph)	20	183	37	80	482	77	83	374	101	52	364	5
Satd. Flow (prot)	0	1663	.0	1658	1671	0	1658	1633	0	1658	1661	10
Fit Permitted		0.778		0.595			0.355			0.290		
Satd. Flow (perm)	0	1285	.0	968	1671	- 0	494	1633	0	483	1661	-04
Satd. Flow (RTOR)		15			13	-		25	-		14	
Lane Group Flow (vph)	0	266	0	89	622	- 0	92	528	- 0	58	464	- 0.6
Turn Type	Perm	NA:		Perm	NA.		Perm	NA.		Perm	NA.	
Protected Phases		4		-	8			2			6	
Permitted Phases	4			8			2			- 6		
Detector Phase	4	4		. 8	- 8		2	- 2		- 6	- 6	
Switch Phase							-				-	
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.5	24.5		24.5	24.5		23.7	23.7		23.7	23.7	
Total Split (s)	33.0	33.0		33.0	33.0		37.0	37.0		37.0	37.0	
Total Split (%)	47.1%	47.1%		47.1%	47.1%		52.9%	52.9%		52.9%	52.9%	
Yellow Time (s)	3.0	3.0		3.0	3.0		33	3.3		33	3.3	
All-Red Time (s)	3.5	3.5		3.5	3.5		24	2.4		24	2.4	
Lost Time Adjust (s)	3,3	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.5		6.5	6.5		5.7	5.7		5.7	5.7	
Lead/Lag		6.5		. 6.5	6.0		3.7	D./		0.7	D./	
Lead-Lag Optimize? Recall Mode	None	None		None	None		C-Min	C-Min		C-Min	C-Min	
Printed by the second of the s	None											
Act Effct Green (s)		28.4		28.4	28.4		29.4	29.4		29.4	29.4	
Actuated g/C Ratio		0.41		0.41	0.41		0.42	0.42		0.42	0.42	
wic Ratio		0.50		0.23	0.91		0.44	0.76		0.29	0.66	
Control Delay		19.0		16.0	40.2		22.6	24.4		17.7	21.0	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		19.0		16.0	40.2		22.6	24.4		17.7	21.0	
LOS		8		В	D		C	C		В	C	
Approach Delay		19.0			37.2			24.1			20.6	
Approach LOS		В			D			C			C	
Queue Length 50th (m)		22.2		7.0	68.6		8.7	57.0		5.1	48.1	
Queue Length 95th (m)		46.0		17.5	#140.8		20.5	85.2		12.7	71.1	
Internal Link Dist (m)		300.5			149.8			122.5			139.6	
Turn Bay Length (m)				37.5			24.0			28.0		
Base Capacity (vph)		530		392	686		220	744		215	750	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		. 0	0	
Reduced v/c Ratio		0.50		0.23	0.91		0.42	0.71		0.27	0.62	
Intersection Summary												
Cycle Length; 70												
Actuated Cycle Length: 70												
Offset: 40 (57%), Reference	ed to phase	2 NBTL	and 6 SB	TL Starf	of Green							
Natural Cycle: 65		-	William Co.	Section 2	on contract of							

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Lanes, Volumes, Timings 2: Preston & Gladstone

951 Gladstone & 145 Loretta Existing - PM Peak Hour

Maximum v/c Ratio: 0.91 Maximum vic Rabo: 0.91 InterIntersection Signal Delay: 27.0 InterIntersection Capacity Usization 83.9% ICU
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles. Intersection LOS: C ICU Level of Service E

Splits and Phases: 2: Preston & Gladstone

02 (91)	-04	
1700	33 x	
1 06 (R)	₹ os	
374	13 6	

HCM 2010 TWSC 3: Breezehill & Somerset 951 Gladstone & 145 Loretta Existing - PM Peak Hour

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1.	C. School S.		4	Y	1,000,1
Traffic Vol. veh/h	316	23	19	418	19	15
Future Vol. yeh/h	316	23	19	418	19	15
Conflicting Peds, #hr	0	100	100	0	19	-0
Sign Control	Free	Free	Free	Free	Stop	
RT Channelized	marine or	None			TOWNS !	
Storage Length	-	COMMON.			0	
Veh in Median Storage.		-		0	- 0	-
Grade %	0	-	=:	0	0	
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mynt Flow	351	26	21	464	21	17
MANUAL PARK	901	600	- 51	194		- "
7.		-				
	lajort	_	Major2	_	Minor1	
Conflicting Flow All	0	0	477	0	989	464
Stage 1		-	- 2	-	464	1
Stage 2			-		525	
Critical Hdwy		-	4.12	-		6.22
Critical Howy Stg 1	-	-	-	-	5.42	-
Critical Howy Stg 2		-	-		5.42	-
Follow-up Hdwy	. + 1		2.218		3.518	
Pot Cap-1 Maneuver	-		1085	-		598
Stage 1	-	-	-	- +	633	
Stage 2			-	-	593) =
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver		-	1001	-	242	552
Mov Cap-2 Maneuver		4	-		242	-
Stage 1	-	-	-	- 4	584	
Stage 2		-	-		568	
	EB		WB.		NB	
Approach		_		_		_
HCM Control Delay, s	0		0.4		17.7	
HCM LOS					C	
Minor Lane/Major Mymt	- 1	NBLn1	EBT	EBR	WEL	TBW
Capacity (veh/h)		322	-	-	1001	
HCM Lane V/C Ratio		0.117	-	-	0.021	
HCM Control Delay (s)		17.7	-	- 4	8.7	0
					-	-
HCM Lane LOS		C	-		· A	A

Intersection	-0											
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4			44.			4	
Traffic Vol. veh/h	9	205	4	9	593	31	15	0	- 6	20	2	16
Future Vol. yeh/h	9	205	4	9	593	31	15	0	6	20	2	16
Conflicting Peds, #hr	23	0	34	34	0	23	- 2	. 0	3	3	. 0	2
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	The same	-	None	O PERSONAL PROPERTY.	-	None		-	None	-	ACCUPATION OF	None
Storage Length	-		+	-					+			
Veh in Median Storage		- 0	-	- 4	- 0		- 6	- 0	-		- 0	-
Grade, %		0		-	- 0	-	-	- 0			- 0	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	. 2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	10	228	- 4	10	659	34	17	0	- 7	22	2	18
Major/Minor 1	Majorf		- 11	Major2		-	Minor1		174	Minor2		
	716	_			-	_		1000			1005	701
Conflicting Flow All		0	0	266	0	0	992	1020	267	976 719	1005 719	
Stage 1	-		- 1			- 4	-,553	- 1905.2				-
Stage 2	4 12			4 12			708	736 6.52	6 22	257 7 12	286 6.52	8 22
Critical Hidwy Critical Hidwy Stg 1		- 2	-	4.12		- 4	6.12	5.52		6.12	5.52	622
	-	- 4	-	-	-		6.12	5.52	-	6.12	5.52	
Critical Howy Stg 2 Follow-up Howy	2.218	- 12		2.218		-	3.518	4.018	3.318	3,518	4 018	3.318
Pot Cap-1 Maneuver	885	- 4	-	1298	-	-	225	237	772	230	241	439
Stage 1	11000	- 1	-	1200	-:	- 1	723	676		420	433	439
Stage 2		- 5	- 6	-		-	426	425	- 1	748	675	
Platoon blocked. %		- 1	- 1		-		920	923		140	010	1 1 1
	869			1264		-	204	221	750	219	200	430
Mov Cap-1 Maneuver	-		-	1,100,71		- 4	204	221		219	225	10000
Mov Cap-2 Maneuver			-	+		-	695	650	-	407	420	
Stage 1		-	-		-		400	412		730	649	-
Stage 2				-			400	412	-	130	048	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.1			20.3			20.3		
HCM LOS	- 777						C			C		
Minor Lane/Major Mvm		NBLnt	CDI	EBT	EBR	WBL	MOT	WBR	001.11			
			EBL		LUK			HUM	277			
Capacity (veh/h)		258	869			1264						
HCM Lane V/C Ratio		0.09	9.2		-	7.9		-	0.152			
HCM Control Delay (s)	Ų.	20.3	(F. 1997)	0		-1775	0					
HCM Lane LOS		C	A	A		A	A		C			
HCM 95th %tile Q(veh)		0.3	- 0		-	0		-	0.5			

Intersection	_											
Intersection Delay, siveh	7.4											
Intersection LOS	A											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4			4			44.	
Traffic Vol., veh/h	12	8	10	4	22	- 6	24	22	1	3	45	32
Future Vol., veh/h	12	- 8	10	4	22	- 6	24	22	1	- 3	45	32
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	13	9	- 11	4	24	7	27	24	1	3	50	36
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	(
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1.			1.		
Conflicting Approach Left	SB			NB.			EB			WB		
Conflicting Lanes Left	1			1			1			10		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			- 1			- 15			- 1		
HCM Control Delay	7.3			7.4			7.5			7.3		
HCM LOS	A			A			A			A		
#4004H		NAME OF	-	and the	en e							
Lane		NBLn1		WBLn1								
Vol Left, %		51%	40%	12%	4%							
Vol Thru, %		47%	27%	69%	56%							
Vol Föght, %		2%	33%	19%	40%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		47	30	32	80							
LT Vol		24	12	4	3							
Through Vol		22	8	72	45							
RT Vol		. 1	10	6	32							
Lane Flow Rate		52	33	36	89							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.061	0.038	0.041	0.095							
Departure Headway (Hd)		4.212	4.085	4.116	3.862							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		846	867	861	923							
Service Time		2.26	2.153	2.183	1.908							
HCM Lane V/C Ratio		0.061	0.038	0.042	0.096							
HCM Control Delay		7.5	7.3	7.4	7.3							
HCM Lane LOS		A	A	A	A							
HCM 95th-tile Q		0.2	0.1	0.1	0.3							

Lanes, Volumes, Timings 8: Bayswater & Gladstone 951 Gladstone & 145 Loretta Existing - PM Peak Hour

-	r 1	1
NBR SBL	SBL SI	BT SBF
		4
28 35		06 3
28 35	35 2	06 3
0 0	0 16	94
	0.9	34
0 0	0 15	89 (
	100	12
0 0		06
Perm	erm t	NA.
		4
- 4	4	
	4	4
	7/	-
10.0	0.0 10	0.0
22.5		2.5
24.0		1.0
40.0%		~~
		3.3
		2.0
2.0		0.0
		5.3
	12	1.3
Max	Total Di	inv
Max		3.7
		31
		61
		7.7
		3.0
		0.0
	44	3.0
	-	C
	23	3.0
		С
		7.0
		9,3
	98	3.4
	- 5	03
		0
		0
		0
	0.	61
		V.

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Lanes, Volumes, Timings 8: Bayswater & Gladstone

951 Gladstone & 145 Loretta Existing - PM Peak Hour

Maximum vfc Ratio: 0.85 Intersection Signal Delay: 21.8	Intersection LOS: C	
Intersection Capacity Utilization 84.1%	ICU Level of Service E	
Analysis Period (min) 15		
# 95th percentile volume exceeds capacity, queue	may be longer.	
Queue shown is maximum after two cycles.	Mrs. Sic	

Splits and Phases: 8. Bayswater & Gladstone

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Appendix D

Collision Data



Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition
2015-02-05	2015-02-05	12:54	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	02 - Wet
2016-10-02	2016-10-02	13:33	BAYSWATER AVE @ GLADSTONE AVE	02 - Rain	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	02 - Wet
2016-06-14	2016-06-14	17:42	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-11-12	2016-11-12	18:01	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-08-21 2017-10-14	2017-08-21 2017-10-14	13:29 12:20	BAYSWATER AVE @ GLADSTONE AVE BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry 02 - Wet
2017-10-14	2017-10-14	12:20	BAYSWATER AVE @ GLADSTONE AVE BAYSWATER AVE @ GLADSTONE AVE	01 - Clear 01 - Clear	01 - Daylight 07 - Dark	01 - Traffic signal 01 - Traffic signal		03 - P.D. only 02 - Non-fatal injury	05 - Turning movement 03 - Rear end	02 - Wet 01 - Dry
2017-11-28	2017-11-28	13:39	BAYSWATER AVE @ GLADSTONE AVE	03 - Snow	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	03 - Loose snow
2018-05-16	2018-05-16	18:10	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	01 - Dry
2019-01-18	2019-01-18	9:50	BAYSWATER AVE @ GLADSTONE AVE	03 - Snow	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	03 - Loose snow
2019-02-14	2019-02-14	19:19	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	05 - Packed snow
2019-10-10	2019-10-10	8:47	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2019-10-24	2019-10-24	16:42	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	01 - Dry
2018-06-28	2018-06-28	16:27	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	01 - Daylight	02 - Stop sign		03 - P.D. only	02 - Angle	01 - Dry
2019-06-14	2019-06-14	23:13	BAYSWATER AVE @ GLADSTONE AVE	01 - Clear	07 - Dark	02 - Stop sign		03 - P.D. only	02 - Angle	01 - Dry
2016-09-29	2016-09-29	0:00	BREEZEHILL AVE N btwn SOMERSET ST W & LAUREL ST	01 - Clear	00 - Unknown	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2016-12-07 2015-12-14	2016-12-07 2015-12-14	15:25 16:00	BREEZEHILL AVE @ GLADSTONE AVE BREEZEHILL AVE @ SOMERSET ST	01 - Clear 01 - Clear	01 - Daylight 05 - Dusk	02 - Stop sign 02 - Stop sign		02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 02 - Angle	02 - Wet 01 - Dry
2015-12-14	2015-12-14	19:26	GLADSTONE AVE @ LORETTA AVE	02 - Rain	07 - Dark	02 - Stop sign		02 - Non-fatal injury	05 - Turning movement	02 - Wet
2018-06-15	2013-11-11	22:14	GLADSTONE AVE @ LORETTA AVE	01 - Clear	07 - Dark	02 - Stop sign		02 - Non-fatal injury	07 - SMV other	01 - Dry
2015-04-22	2015-04-22	22:52	GLADSTONE AVE @ PRESTON ST	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2015-04-15	2015-04-15	20:20	GLADSTONE AVE @ PRESTON ST	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2015-06-23	2015-06-23	16:20	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2015-06-08	2015-06-08	12:00	GLADSTONE AVE @ PRESTON ST	02 - Rain	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2015-07-10	2015-07-10	7:30	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2015-10-23	2015-10-23	14:02	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2015-08-11	2015-08-11	11:15	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-04-01	2016-04-01	6:23	GLADSTONE AVE @ PRESTON ST	01 - Clear	03 - Dawn	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	02 - Wet
2016-08-09 2016-08-10	2016-08-09 2016-08-10	18:13 9:45	GLADSTONE AVE @ PRESTON ST GLADSTONE AVE @ PRESTON ST	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	01 - Traffic signal 01 - Traffic signal		02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 03 - Rear end	01 - Dry 01 - Dry
2016-08-10	2016-08-10	15:25	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	01 - Dry 01 - Dry
2016-02-12	2016-02-12	17:40	GLADSTONE AVE @ PRESTON ST	03 - Snow	05 - Dusk	01 - Traffic signal		03 - P.D. only	03 - Rear end	03 - Loose snow
2016-06-29	2016-06-29	9:45	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	07 - SMV other	01 - Dry
2017-10-23	2017-10-23	14:18	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	01 - Dry
2017-12-27	2017-12-27	9:15	GLADSTONE AVE @ PRESTON ST	03 - Snow	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	06 - Ice
2018-01-10	2018-01-10	14:37	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	04 - Slush
2018-04-04	2018-04-04	14:25	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	01 - Dry
2018-06-29	2018-06-29	7:26	GLADSTONE AVE @ PRESTON ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2018-11-05	2018-11-05	17:14	GLADSTONE AVE @ PRESTON ST	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	02 - Wet
2019-01-26	2019-01-26	17:34	GLADSTONE AVE @ PRESTON ST	03 - Snow	05 - Dusk	01 - Traffic signal		03 - P.D. only	03 - Rear end	06 - Ice
2019-01-25 2019-06-03	2019-01-25 2019-06-03	8:27 9:04	GLADSTONE AVE @ PRESTON ST GLADSTONE AVE @ PRESTON ST	01 - Clear 01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only 03 - P.D. only	03 - Rear end	04 - Slush
2019-06-03	2019-06-03	9:04	GLADSTONE AVE @ PRESTON ST	01 - Clear 02 - Rain	01 - Daylight 01 - Daylight	01 - Traffic signal 01 - Traffic signal		03 - P.D. only 03 - P.D. only	04 - Sideswipe 04 - Sideswipe	01 - Dry 02 - Wet
2015-10-31	2019-10-31	17:05	GLADSTONE AVE W PRESTON ST GLADSTONE AVE btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - SidesWipe 02 - Angle	01 - Dry
2019-08-10	2019-08-10	16:41	GLADSTONE AVE btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	99 - Other	01 - Dry
2018-02-10	2018-02-10	19:11	GLADSTONE AVE btwn LORETTA AVE N & PRESTON ST	03 - Snow	07 - Dark	10 - No control		03 - P.D. only	01 - Approaching	03 - Loose snow
2018-11-17	2018-11-17	18:30	GLADSTONE AVE btwn LORETTA AVE N & PRESTON ST	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	04 - Sideswipe	02 - Wet
2019-02-14	2019-02-14	11:00	GLADSTONE AVE btwn LORETTA AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	04 - Slush
2015-06-06	2015-06-06	20:00	LAUREL ST btwn BREEZEHILL AVE N & LORETTA AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2019-02-05	2019-02-05	0:00	LAUREL ST btwn BREEZEHILL AVE N & LORETTA AVE N	03 - Snow	00 - Unknown	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	06 - Ice
2015-01-23	2015-01-23	13:49	LORETTA AVE N btwn GLADSTONE AVE & END	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2015-08-05	2015-08-05	10:59	SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - Sideswipe	01 - Dry
2017-05-09	2017-05-09	14:42	SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2017-12-07 2017-01-03	2017-12-07 2017-01-03	15:38 0:46	SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear 01 - Clear	01 - Daylight 07 - Dark	10 - No control 10 - No control		03 - P.D. only 02 - Non-fatal injury	02 - Angle 05 - Turning movement	01 - Dry 02 - Wet
2017-01-03	2018-01-08	14:15	SOMERSET ST W blwn BAYSWATER AVE & BREEZEHILL AVE N	03 - Snow	01 - Daylight	10 - No control		02 - Non-fatal injury	02 - Angle	04 - Slush
2018-01-08	2018-12-19	0:34	SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	07 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2019-01-08	2019-01-08	11:36	SOMERSET ST W btwn BAYSWATER AVE & BREEZEHILL AVE N	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	04 - Slush
2015-04-10	2015-04-10	12:00	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2015-03-04	2015-03-04	8:22	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	02 - Wet
2015-09-26	2015-09-26	9:23	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2016-05-08	2016-05-08	13:37	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	04 - Sideswipe	01 - Dry
2016-07-11	2016-07-11	18:43	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2016-08-03	2016-08-03	9:12	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - Sideswipe	01 - Dry
2017-06-08	2017-06-08	16:42	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2017-11-30	2017-11-30	11:17	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2017-01-20	2017-01-20 2018-01-17	13:01 18:10	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight 07 - Dark	10 - No control		03 - P.D. only	02 - Angle	02 - Wet
2018-01-17 2018-06-30	2018-01-17 2018-06-30	18:10	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear 01 - Clear	07 - Dark 01 - Daylight	10 - No control 10 - No control		03 - P.D. only 02 - Non-fatal injury	02 - Angle 05 - Turning movement	02 - Wet 01 - Dry
2018-06-30	2018-06-30	14:20	SOMERSET ST W DTWN BREEZEHILL AVE N & PRESTON ST	01 - Clear 01 - Clear	01 - Daylight 01 - Daylight	10 - No control		02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 04 - Sideswipe	01 - Dry 01 - Dry
2019-07-24	2019-07-24	17:28	SOMERSET ST W blum BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2019-09-12	2019-09-12	11:15	SOMERSET ST W btwn BREEZEHILL AVE N & PRESTON ST	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
-	· · · · · 				,					

Appendix E

MMLOS Analysis



Multi-Modal Level of Service - Intersections Form

Consultant CGH Transportation Inc
Scenario Existing and Future
Comments

Project Date 2020-25 2021-04-07

	INTERSECTIONS		Gladstone (Bayswater			Gladstone	@ Preston	
	Crossing Side	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
	Lanes	0 - 2	0 - 2	0 - 2	0 - 2	3	3	3	0 - 2
	Median	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m
	Conflicting Left Turns	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive
	Conflicting Right Turns	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control
	Right Turns on Red (RToR)?	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed
	Ped Signal Leading Interval?	No	No	No	No	no	no	no	No
ian	Right Turn Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel
stı	Corner Radius	5-10m	5-10m	5-10m	5-10m	5-10m	5-10m	10-15m	5-10m
Pedestrian	Crosswalk Type	Std transverse markings	Std transverse markings	Zebra stripe hi-vis markings	Zebra stripe hi-vis markings	Textured/coloured pavement	Textured/coloured pavement	Textured/coloured pavement	Textured/coloured pavement
4	PETSI Score	86	86	89	89	74	74	73	89
	Ped. Exposure to Traffic LoS	В	В	В	В	С	С	С	В
	Cycle Length								
	Effective Walk Time								
	Average Pedestrian Delay								
	Pedestrian Delay LoS	-	-	-	-	-	-	-	-
		В	В	В	В	С	С	С	В
	Level of Service		E	3			(3	
	Approach From	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
	Bicycle Lane Arrangement on Approach								
	Right Turn Lane Configuration								
	Right Turning Speed								
Θ	Cyclist relative to RT motorists	-	-	-	-	-	-	-	-
짗	Separated or Mixed Traffic	-	-	-	-	-	-	-	-
Bicycle	Left Turn Approach	No lane crossed	No lane crossed	No lane crossed	No lane crossed	No lane crossed	No lane crossed	No lane crossed	No lane crossed
ш	Operating Speed	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h
	Left Turning Cyclist	> 40 to ≤ 50 km/m	> 40 to ≤ 50 kH/H	> 40 to ≤ 50 kii/ii	> 40 to ≤ 50 km/m	> 40 to ≤ 50 kH/H	> 40 to ≤ 50 km/m	> 40 to ≤ 50 km/m	> 40 to ≤ 50 km/m
	Left furning Cyclist	-	-	-	-	-	-	-	-
	Level of Service	-				-			
	2010.0.000			-				-	
	Average Signal Delay			≤ 30 sec	≤ 20 sec	≤ 20 sec	≤ 20 sec	≤ 40 sec	≤ 40 sec
ısit	,	-	-	D	С	С	С	E	E
Transit	Level of Service								
	Effective Corner Radius					< 10 m	< 10 m	< 10 m	
×	Number of Receiving Lanes on Departure from Intersection					1	1	1	
Truck		-	-	-	-	F	F	F	-
	Level of Service			-				F	
0	Volume to Capacity Ratio		0.61	- 0.70			0.71	- 0.80	
- 3									
Auto	Level of Service			3				3	

Multi-Modal Level of Service - Segments Form

Consultant
Scenario
Comments

CGH Transportation Inc	
Existing and Future	

Project Date

2020-25
2021-04-07

SEGMENTS			Gladstone 1	Loretta (frontage) 2	Loretta (frontage) 3
	Sidewalk Width Boulevard Width		≥ 2 m < 0.5	no sidewalk n/a	≥ 2 m 0.5 - 2 m
	Avg Daily Curb Lane Traffic Volume		> 3000	≤ 3000	≤ 3000
Pedestrian	Operating Speed On-Street Parking		> 30 to 50 km/h no	> 30 to 50 km/h yes	> 30 to 50 km/h no
st	Exposure to Traffic PLoS	С	С	F	Α
β	Effective Sidewalk Width		2.0 m		
Pe	Pedestrian Volume		500 ped /hr		
	Crowding PLoS		В	-	-
	Level of Service		С	-	-
	Type of Cycling Facility		Mixed Traffic	Mixed Traffic	Mixed Traffic
	Number of Travel Lanes		≤ 2 (no centreline)	≤ 2 (no centreline)	≤ 2 (no centreline)
	Operating Speed		>40 to <50 km/h	>40 to <50 km/h	>40 to <50 km/h
	# of Lanes & Operating Speed LoS		В	В	В
Bicycle	Bike Lane (+ Parking Lane) Width				
င်	Bike Lane Width LoS	В	-	-	-
Θ̈	Bike Lane Blockages				
	Blockage LoS		-	-	-
	Median Refuge Width (no median = < 1.8 m)		< 1.8 m refuge	< 1.8 m refuge	< 1.8 m refuge
	No. of Lanes at Unsignalized Crossing		≤ 3 lanes >40 to 50 km/h	≤ 3 lanes ≤ 40 km/h	≤ 3 lanes ≤ 40 km/h
	Sidestreet Operating Speed Unsignalized Crossing - Lowest LoS		B	≤ 40 KIII/II	≤ 40 KIII/II
	Onsignanzed Orossing - Lowest Loo				
	Level of Service		В	В	В
it	Facility Type		Mixed Traffic		
ransit	Friction or Ratio Transit:Posted Speed	D	Vt/Vp ≥ 0.8		
Ĕ	Level of Service		D	-	-
×	Truck Lane Width				
Truck	Travel Lanes per Direction	-			
	Level of Service		-	-	-
Auto	Level of Service		Not Ap	plicable	

Appendix F

Synchro Intersection Worksheets – Future Total Conditions



HCM 95th %tile Q(veh)

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	1 00		4			-4-	
Traffic Vol. vehih	17	216	- 4	- 6	620	27	3	0	1	28	0	23
Future Vol., veh/h	17	216	- 4	6	620	27	- 3	0	- 1	28	0	23
Conflicting Peds, #hr	25	0	25	25	. 0	25	7	. 0	10	10	. 0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	- 117		None		-	None		-	None	on the same	1000	None
Storage Length	- 1		-	-			-	-	-			
Veh in Median Storage		. 0		- 4	- 0	- 4	- 6	- 0	-		- 0	- 4
Grade, %		. 0			- 0	- 4	-	0			- 0	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	- 2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	216	- 4	- 6	620	27	- 3	0	- 1	28	- 0	23
1107110000		-			-					-		
MajoriMinor	Majorf		- 0	Major2	7	- 1	Minor1		13	Minor2		
Conflicting Flow All	672		0	245	0	0	941	961	253	934	950	666
Stage 1							277	277		671	671	1000
Stage 2						-	664	684		263	279	
Critical Hdwy	4.12			4.12		8 84	7.12	6.52	6.22	7.12	6.52	8 22
Critical Hdwy Stg 1							6.12	5.52		6.12	5.52	
Critical Howy Stg 2							6.12	5.52		6.12	5.52	
Follow-up Hdwy	2.218	-		2.218			3.518	4.018	3.318	3.518	4 018	3.318
Pot Cap-1 Maneuver	919		-	1321	-		THE PARTY OF	256	786	246	260	459
Stage 1	-			I See !		-	729	681	1400	446	455	744
Stage 2			-			8 84	450	449		742	680	
Platoon blocked, %		- 1										
Mov Cap-1 Maneuver	901		_	1295		-	220	239	765	234	243	448
Mov Cap-2 Maneuver	901	-		1200			-	239	(00	234	243	440
Stage 1			- 2				699	653			443	
Stage 2						-	422	437		719	652	
Giage £							766	401		710	- 002	
Approach	EE			WB			NB			SB		
HCM Control Delay, s	0.7			0.1			18.6			19.6		
HCM LOS	V.			V.1			10.0 C			(a,0		
nom LU3							-0			-		
Minor Lane/Major Myn	nt	NBLnt	EBL	EBT	EBR	WBL	WRT	WER	581 e1			
Capacity (veh/h)		268	901	- Lui	LUIT	1295	1101	TTUNE.	298			
HCM Lane V/C Ratio		0.015	CONCENTRAL PARTY			Vienes.			0.171			
HCM Control Delay (s		18.6	9.1	0		7.8	0		19.6			
HCM Lane LOS	10	C	A	A		A	A		C			
THUM LIKE LUG		- 4	^			- ^	^	. +	- 0			

0 0.1 - - 0 - - 0.6

	,	-	1	1	-		1	†	-	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	1.		7	1.		7	7.	
Traffic Volume (vph)	23	193	50	80	491	77	95	374	101	52	364	57
Future Volume (vph)	23	193	50	80	491	77	95	374	101	52	364	57
Satd. Flow (prot)	0	1646	- 0	1658	1668	0	1658	1633	0	1658	1657	- 0
Fit Permitted	-	0.788		0.586		-	0.415		-	0.362		-
Satd Flow (perm)	0	1298	.0	953	1668	- 0	655	1633	0	598	1657	- 0
Satd. Flow (RTOR)		19			13	- 7	1755	25	7.0		15	
Lane Group Flow (vph)	0	266	0	80	568	- 0	95	475	- 0	52	421	- 0
Turn Type	Perm	NA:		Perm	NA.		Perm	NA.		Perm	NA.	
Protected Phases		4		-	8			2			- 6	
Permitted Phases	- 4	7.0		8			2			- 6		
Detector Phase	- 4	4		- 8	- 8		2	- 2		- 6	- 6	
Switch Phase				- 7			-				1	
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	24.5	24.5		24.5	24.5		23.7	23.7		23.7	23.7	
Total Split (s)	33.0	33.0		33.0	33.0		37.0	37.0		37.0	37.0	
Total Split (%)	47.1%	47.1%		47.1%	47.1%		52.9%	52.9%		52.9%	52.9%	
Yellow Time (s)	3.0	3.0		30	3.0		33	3.3		33	3.3	
All-Red Time (s)	3.5	3.5		3.5	3.5		2.4	2.4		24	2.4	
Lost Time Adjust (s)	3,3	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.5		65	6.5		5.7	5.7		5.7	5.7	
Lead/Lag		0.0		6.0	0.0		3.7	0.7		0.7	0.7	_
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Min	C-Min		C-Min	C-Min	
Act Effct Green (s)	None	26.6		26.6	26.6		31.2	31.2		31.2	31.2	
Actuated g/C Ratio		0.38		0.38	0.38		0.45	0.45		0.45	0.45	
vic Ratio		0.53		0.22	0.89		0.33	0.64		0.20	0.45	
and the second s		19.7		16.1	37.9		17.3	19.6		14.9	17.9	
Control Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Queue Delay		19.7		16.1	37.9		17.3	19.6		14.9	17.9	
Total Delay		1650010		2000			100000					
LOS		8		В	D		В	В		В	В	
Approach Delay		19.7			35.2			19.2			17.6	
Approach LOS		22.5			61.4			47.2		43	40.6	
Queue Length 50th (m)				6.4			8.3					
Queue Length 95th (m)		45.2		16.0	#123.7		18.3	72.9		10.7	62.6	
Internal Link Dist (m)		300.5		27.5	149.8			122.5			139.6	
Turn Bay Length (m)		2000		37.5	-		24.0	247		28.0	754	
Base Capacity (vph)		522		374	663		301	765		275	771	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced vic Ratio		0.51		0.21	0.86		0.32	0.62		0.19	0.55	
Intersection Summary												
Cycle Length: 70 Actuated Cycle Length: 70												
Offset: 40 (57%), Reference	ed to phase	2 NRTI	and 6 SE	TI Start	of Green							
Natural Cycle: 55	es to priase	E.MOIL	m q u 20	IL OWN	Ur Green							
Natural Cycle, 30												

Lanes, Volumes, Timings 2: Preston & Gladstone

951 Gladstone & 145 Loretta Future Total - PM Peak Hour

Maximum v/c Ratio: 0.89 Intersection Signal Delay: 24.2 Intersection Capacity Utilization 86.9% ICU Analysis Period (min) 15. # 95th percentile volume exceeds capacity, queue may be longer. Intersection LOS: C ICU Level of Service E Queue shown is maximum after two cycles. Colite and Dhases: 2: Decetor & Cladetone

Spirts and Phases:	Z: Preston &	Usadstone
- A		

To2(8)		
Particular (33 5	-
1 (D6 (R))	★ 08	

HCM 2010 TWSC 3: Breezehill & Somerset 951 Gladstone & 145 Loretta Future Total - PM Peak Hour

Intersection						
int Delay, s/veh	1					
Movement:	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1.			4	Y	
Traffic Vol. veh/h	316	28	21	418	24	- 17
Future Vol. veh/h	316	28	21	418	24	17
Conflicting Peds, #hr	0	100	100	0	19	-0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	Fibe	None	Free	None	alop	None
	-	None		rvone	0	none
Storage Length				0	0	
Veh in Median Storage						-
Grade, %	0			0	0	***
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	316	28	21	418	24	- 17
Major/Minor N	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	444	0	909	430
Stage 1	-			-	430	WAR.
Stage 2	-	-	-		479	
Critical Hdwy			4 12	2	6.42	822
Critical Howy Stg 1			-		5.42	-
Critical Howy Stg 2		-			5.42	VIII V
Follow-up Hdwy			2.218		3.518	3 318
Pot Cap-1 Maneuver			THE RESIDENCE		COLUMN THE REAL PROPERTY.	625
Stage 1			1110		656	444
Stage 2					623	
			-		023	1
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver			1029		270	576
Mov Cap-2 Maneuver	. +		-	-	270	+
Stage 1		-	-	-	605	-
Stage 2		-	-		597	
Approach	EB		WB.		NB	4
HCM Control Delay, s	0	_	0.4		16.8	
HCM LOS					C	
POLYMENT STATE						
Minor Lane/Major Mymi		NBLnT	EBT	EBR	WEL	WBT
Capacity (veh/h)		346	CDI	LUN	1029	HUI
HCM Lane V/C Ratio		0.118			0.02	
			-	-		
HCM Control Delay (s)		16.8			8.6	0
HCM Lane LOS		C	-	+	A	A
HCM 95th %tile Q(veh)		0.4	-	-	0.1	-

HCM 95th %tile Q(veh)

- 0.2 0 -

Intersection						
Int Delay, s/veh	3.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	1	4			+
Traffic Vol. vehih	54	10	40	- 0	0	47
Future Vol. yeh/h	54	10	40	0	0	47
Conflicting Peds, #hr	- 0	. 0	. 0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized		The second second	in no	None	1000	None
Storage Length	0	0				-
Veh in Median Storag	e # 0		0			- 0
Grade, %	0	-	0			0
Peak Hour Factor	100	100	100		100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mumt Flow	54	10	40		0	47
MINISTER SOM		10	40			- 31
	1000		35-0	1 10		-
MajorMinor	Minor1		Major1		Major2	
Conflicting Flow All	87	40	0	+		-
Stage 1	40	- 4				4
Stage 2	47				- 10	- 4
Critical Hdwy	6.42	6.22	- 2			9 64
Critical Howy Stg 1	5.42				,	-
Critical Howy Stg 2	5.42	- 4	- 4			- 4
Follow-up Hdwy	3,518	3.318	-			-
Pot Cap-1 Maneuver	914	1031		0	. 0	
Stage 1	982			0	0	-
Stage 2	975	- 4	- 4	0	0	1 14
Platoon blocked, %						-
Mov Cap-1 Maneuver	914	1031				-
Mov Cap-2 Maneuver		1001	-			-
Stage 1	982	- 4				
Stage 2	975	-			-	
Stage 2	010					-
Accessed	WB		NB		SB	
Approach						_
HCM Control Delay, s			0		0	
HCM LOS	A					
Minor Lane/Major Mvi	mi	NBTV		WBLn2	SBT	
Capacity (veh/h)		- 4		1031		
HCM Lane V/C Ratio		,	0.059		,	
HCM Control Delay (s	13	- 4	9.2	8.5		
HCM Lane LOS		-	A	A		
and the second section is a few parts of the second section in the second						

Conflicting Peds, #hz 31 0 35 35 0 31 2 0 4 4 0 2
Lane Configurations
Traffic Vol, vehith 33 205 4 9 593 58 15 0 6 46 2 45 Future Vol, vehith 33 205 4 9 593 58 15 0 6 46 2 45 Conflicting Peds, #hr 31 0 35 35 0 31 2 0 4 4 0 2 Stop Stop Stop Stop Stop Stop Stop Stop
Traffic Vol, vehith 33 205 4 9 593 58 15 0 6 46 2 45 Future Vol, vehith 33 205 4 9 593 58 15 0 6 46 2 45 Conflicting Peds, #hr 31 0 35 35 0 31 2 0 4 4 0 2 Sign Control Free Free Free Free Free Free Stop
Future Vol, veh/h 33 205 4 9 593 58 15 0 6 46 2 45 Conflicting Peds, #hr 31 0 35 35 0 31 2 0 4 4 0 2 2 5 Sign Control Free Free Free Free Free Free Stop Stop Stop Stop Stop RT Charmetzed None None None None None None None None
Conflicting Peds, #hr 31
Sign Control Free Stop
RT Charmelized - None - None - None - None - None Storage Length
Storage Length
Veh in Median Storage, # 0 - 0 0 - 0 0 - 0 0 4
Grade, % - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 -
Peak Hour Factor 100
Heavy Vehicles, % 2
Major/Minor Major! Major2 Minor! Minor2 Conflicting Flow All 682 0 0 244 0 0 974 1008 246 951 981 655
Major/Minor Major! Major2 Minor! Minor2 Conflicting Flow All 682 0 0 244 0 0 974 1008 246 951 981 655
Conflicting Flow All 682 0 0 244 0 0 974 1008 246 951 981 655
Conflicting Flow All 682 0 0 244 0 0 974 1008 246 951 981 655
Stage 1 306 308 - 671 671 -
Stage 2 666 700 - 280 310 -
Critical Holay 4 12 - 4 12 - 7 12 6 52 6 22 7 12 6 52 6 22
Critical Holwy Stg 1 6.12 5.52 - 6.12 5.52 -
Critical Howy Stg 2 6.12 5.52 - 6.12 5.52 -
Follow-up Howy 2218 2218 3518 4.018 3.318 3.518 4.018 3.318
Pot Cap-1 Maneuver 911 1322 231 240 793 240 249 466
Stage 1 702 660 - 446 455 -
Stage 2 449 441 - 727 659 -
Platoon blooked, %
Mov Cap-1 Maneuver 889 1286 193 216 769 222 224 454
Mov Cap-2 Maneuver 193 216 - 222 224 -
Stage 1 654 615 - 417 439 -
Stage 2 398 426 - 689 614 -
2 71 222 722 722
Approach EB WB NB SB
HCM Control Delay, s 1.3 0.1 21 22.7
HCM LOS C C
Control of the contro
Minor Lane/Major Minit NBLn1 EBL EBT EBR WBL WBT WBR SBLn1
Capacity (vehih) 246 889 - 1286 - 295 HCM Lane V/C Ratio 0.085 0.037 - 0.007 - 0.315
The state of the s
HCM Lane LOS C A A - A A - C HCM 95th %tile Q(veh) 0.3 0.1 0 1.3

	4000											
ntersection Delay, s/veh	7.3											
ntersection LOS	A											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
ane Configurations	Life	4	4485	STATE	4	HEAT	PAGE	4	INUIS	SALE.	4	SALES
Traffic Vol. veh/h	12	11	10	- 4	25	12	24	22		9	45	32
future Vol. vehih	12	- 11	10	4	25	12	24	22	-	9	45	32
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mynt Flow	12	- 11	10	4	25	12	24	22	1	9	45	32
Number of Lanes	0	1	0	0	1	0	0	1	0	0	40	0
Number of Lanes	0		U	v		0	v		v	0	,	0
Approach	EB			WB			NB.			58		- 1
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			- 1			-1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			58			WB			EB		
Conflicting Lanes Right	1			- 1			1			11		
HCM Control Delay	7.3			7.3			7.5			7.3		
HCM LOS				-			0.00					
HART LAND	PL.			A			A			A		
HAMP LLKS				A			A			A		
		NOVET	EDI-1		ent at		A			A		
Lane				WBLn1			Α.			A		
Lane Vol Left, %		51%	36%	WBLn1	10%		A			A		
Lane Vol Left, % Vol Thru, %		51% 47%	36% 33%	WBLn1 10% 61%	10% 52%		A			A		
Lene Vol Left, % Vol Thru, % Vol Right, %		51% 47% 2%	36% 33% 30%	WBLn1 10% 61% 29%	10% 52% 37%		A			A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		51% 47% 2% Stop	36% 33% 30% Stop	WBLn1 10% 61% 29% Stop	10% 52% 37% Stop		A			A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		51% 47% 2% Stop 47	36% 33% 30% Stop 33	WBLn1 10% 61% 29% Stop 41	10% 52% 37% Stop 86		A			A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sontrol Traffic Vol by Lane LT Vol		51% 47% 2% Stop 47 24	36% 33% 30% Stop 33 12	WBLn1 10% 61% 29% Stop 41 4	10% 52% 37% Stop 86 9		A			Α		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		51% 47% 2% Stop 47 24 22	36% 33% 30% Stop 33 12 11	WBLn1 10% 61% 29% Stop 41 4 25	10% 52% 37% Stop 86 9 45		A .			Α		
Lane Vol Left, % Vol Thu, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Trrough Vol RT Vol		51% 47% 2% Stop 47 24 22	36% 33% 30% Stop 33 12 11	WBLn1 10% 61% 29% Stop 41 4 25 12	10% 52% 37% Stop 86 9 45 32		A .			(A)		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		51% 47% 2% Stop 47 24 22	36% 33% 30% Stop 33 12 11	WBLn1 10% 61% 29% Stop 41 4 25	10% 52% 37% Stop 86 9 45 32 86		A			(A)		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Seometry Grp		51% 47% 2% Stop 47 24 22 1 47	36% 33% 30% Stop 33 12 11 10 33	WBLn1 10% 61% 29% Stop 41 4 25 12 41	10% 52% 37% Stop 86 9 45 32 86 1		A .			(A)		
Lane Vol Left, % Vol Tithu, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		51% 47% 2% Stop 47 24 22 1 47 1 0.055	36% 33% 30% Stop 33 12 11 10 33 1	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046	10% 52% 37% Stop 86 9 45 32 86 1		A .			(A)		
Lane Vol Left, % Vol Tinu, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Uhl (X) Departure Headway (Hd)		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216	36% 33% 30% Stop 33 12 11 10 33 1 0.037 4.066	W9Ln1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894		A -			(A)		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Through Vol RT Vol ane Flow Rate Seometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216 Yes	36% 33% 30% Stop 33 12 11 10 33 1 0 037 4 086 Yes	W9Ln1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4,032 Yes	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes		A -			(A)		
Lane Vol Left, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol ane Flow Rate Seometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Lap		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216 Yes 845	36% 33% 30% Stop 33 12 11 10 33 1 0 037 4 086 Yes 867	WBLn1 10% 61% 29% Stop 41 4 25 12 41 0.046 4.032 Yes 879	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915		A -			(A)		
Lane Vol Left, % Vol Tinu, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216 Yes 845 2.266	36% 33% 30% Stop 33 12 11 10 33 1 0 037 4 086 Yes 867 2.153	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032 Yes 879 2.097	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915 1.941		A -			(A)		
Lane Vol Left, % Vol Tinu, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Seometry Grp Degree of Uhl (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216 Yes 845 2.266 0.056	36% 33% 30% Stop 33 12 11 10 33 1 0.037 4.066 Yes 867 2.153 0.038	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032 Yes 879 2.097 0.047	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915 1.941 0.094		A			(A)		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Seometry Grp Degree of Uhl (X) Departure Headway (Hd) Convergence, Y/N Cap Senice Time HCM Lane V/C Ratio HCM Control Delay		51% 47% 2% Stop 47 24 22 1 47 0.055 4.216 Yes 845 2.266 0.056 7.5	36% 33% 30% Stop 33 12 11 10 33 1 0.037 4.086 Yes 867 2.153 0.038 7.3	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032 Yes 879 2.097 0.047 7.3	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915 1.941 0.094 7.3		A			(A)		
Lane Vol Left, % Vol Left, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane VIC Ratio HCM Control Delay HCM Lone LOS		51% 47% 2% Stop 47 24 22 1 47 1 0.055 4.216 Yes 845 2.266 0.056 7.5 A	36% 33% 30% Stop 33 12 11 10 33 1 0.037 4.066 Yes 867 2.153 0.038 7.3	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032 Yes 879 2.097 0.047 7.3 A	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915 1.941 0.094 7.3 A		A			(A)		
Lane Vol Left, % Vol Left, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Ratie Seometry Grp Degree of Uhi (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		51% 47% 2% Stop 47 24 22 1 47 0.055 4.216 Yes 845 2.266 0.056 7.5	36% 33% 30% Stop 33 12 11 10 33 1 0.037 4.086 Yes 867 2.153 0.038 7.3	WBLn1 10% 61% 29% Stop 41 4 25 12 41 1 0.046 4.032 Yes 879 2.097 0.047 7.3	10% 52% 37% Stop 86 9 45 32 86 1 0.093 3.894 Yes 915 1.941 0.094 7.3		A			(-A)		

	,	-	1	1	-	1	1	1	-	1	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	10	174	18	74	458	114	29	182	28	35	206	34
Future Volume (vph)	10	174	18	74	458	114	29	182	28	35	206	34
Satd. Flow (prot)	0	1707	.0	0	1662	0	0	1697	0	0	1694	100
Fit Permitted		0.968			0.940			0.935			0.934	
Satd. Flow (perm)	0	1655	.0	0	1565	0	0	1594	0	- 0	1589	- (
Satd. Flow (RTOR)		12			26			12			12	
Lane Group Flow (vph)	0	202	0	0	646	0	0	239	0	0	275	- (
Turn Type	Perm	NA.		Perm	NA.		Perm	NA.		Perm	NA.	
Protected Phases		2			6			- 8			4	
Permitted Phases	2			6			- 8			4		
Detector Phase	2	2		- 6	- 6		- 8	- 8		4	- 4	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		22.5	22.5		22.5	22.5	
Total Split (s)	36.0	36.0		36.0	36.0		24.0	24.0		24.0	24.0	
Total Split (%)	60.0%	60.0%		60.0%	60.0%		40.0%	40:0%		40.0%	40.0%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.3	3.3		3.3	3.3	
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.5			5.5			5.3			5.3	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		Max	Max		Max	Minc	
Act Effct Green (s)		30,5			30.5			18.7			18.7	
Actuated g/C Ratio		0.51			0.51			0.31			0.31	
wic Ratio		0.24			0.80			0.47			0.55	
Control Delay		8.7			21.6			19.6			21.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		8.7			21.6			19.6			21.3	
LOS		A			C			В			C	
Approach Delay		8.7			21.6			19.6			21.3	
Approach LOS		Α.			C			В			C	
Queue Length 50th (m)		10.7			51.9			19.8			23.6	
Queue Length 95th (m)		20.6			#109.9			37.4			43.5	
Internal Link Dist (m)		95.1			81.5			119.0			98.4	
Turn Bay Length (m)												
Base Capacity (vph)		847			808			505			503	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			-0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.24			0.80			0.47			0.55	
Intersection Summary												
Cycle Length; 60												
Actuated Cycle Length: 60												
Offset: 53 (88%), Reference		2 EBTL	and 6 WE	TL. Start	of Green							
Natural Cycle: 60		- secondari			-							

Natural Cycle: 60 Control Type: Actuated-Coordinated

Lanes, Volumes, Timings 8: Bayswater & Gladstone

951 Gladstone & 145 Loretta Future Total - PM Peak Hour

Maximum v/c Ratio: 0.80		
Intersection Signal Delay: 19.3	Intersection LOS: B	
Intersection Capacity Utilization 85.7%	ICU Level of Service E	
Analysis Period (min) 15		
# 95th percentile volume exceeds capacity, queue	may be longer.	
Queue shown is maximum after two cycles.		

Splits and Phases: 8: Bayswater & Gladstone



Appendix G

TDM Checklist



TDM-Supportive Development Design and Infrastructure Checklist: Non-Residential Developments (office, institutional, retail or industrial)

Legend					
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed				
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users				
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance				

	TDM-	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references		
	1.	WALKING & CYCLING: ROUTES			
	1.1	Building location & access points			
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	₩		
	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	♥		
BASIC	1,1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	♥		
	1.2	Facilities for walking & cycling			
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	♥		
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	♂		

	TDM-	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	Ø
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	Ø
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	✓
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	♥
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	Ø

5

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACIL	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	₽′
REQUIRED	212	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well- used areas (see Zoning By-law Section 111)	♥
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	
	2.2	Secure bicycle parking	100
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	♥
BETTER	222	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	
	2.3	Shower & change facilities	To a second
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	
	2.4	Bicycle repair station	The state of the s
BETTER	2.4,1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	♥

	TDM-	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
EASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	♥
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
EASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	4.2	Carpool parking	
	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide carshare parking spaces in permitted non- residential zones, occupying either required or provided parking spaces (see Zoning By-law Section 94)	5
1	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	✓

	TDM-	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references		
	6,	PARKING			
	6.1	Number of parking spaces			
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	∀		
MADIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking			
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	Ø		
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)			
	6.2	Separate long-term & short-term parking areas			
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)			
	7.	OTHER			
	7.1	On-site amenities to minimize off-site trips			
BETTER	7,1,1	Provide on-site amenities to minimize mid-day or mid-commute errands			

TDM-Supportive Development Design and Infrastructure Checklist: Residential Developments (multi-family or condominium)

	Legend
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references	
	18	WALKING & CYCLING: ROUTES		
	1.1	Building location & access points		
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances		
BANC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	♥	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	✓	
	1.2	Facilities for walking & cycling	y 47940	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres, minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	▽	
REQUIRED	1.22	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	♥	

	TDM-	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references			
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	Ø			
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	Ø			
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	d			
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	♥′			
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	♥′			
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility				
	1.3	Amenities for walking & cycling				
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and traits	♥			
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	♥			

Appendix H

Signal Warrant – OTM Justification 7



Gladstone Ave @ Loretta Ave N Future Total

Justification #7

	Description	Minimum Requirement		Minimum Requirement		Compliance			
Justification		1 Lane Highway		2 or More Lanes		Sectional		Entire %	Signal
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Elitile %	
1. Minimum Vehicular	A. Vehicle volume, all approaches (average hour)	480	720	600	900	404	56%	32%	No
Volume	B. Vehicle volume, along minor streets (average hour)	120	170	120	170	55	32%		
2. Delay to Cross	A. Vehicle volumes, major street (average hour)	480	720	600	900	349	48%	41%	No
Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	50	75	31	41%		

Notes

- 1. Refer to OTM Book 12, pg 92, Mar 2012
- 2. Lowest section percentage governs justification
- 3. Average hourly volumes estimated from peak hour volumes, AHV = PM/2 or (AM + PM) / 4, including amplification factors
- 4. T-intersection factor corrected, applies only to 1B