311 Somerset Street, 234-236 O'Connor Street Transportation Impact Assessment

Step 1 Screening Report
Step 2 Scoping Report
Step 3 Forecasting Report
Step 4 Strategy Report

Prepared for:

Gemstone Corporation 252 Argyle Avenue Ottawa, ON K2P 1B9

Prepared by:



6 Plaza Court Ottawa, ON K2H 7W1

June 2022

PN: 2020-27

Table of Contents

L		Screening	1
2		Existing and Planned Conditions	1
	2.1	Proposed Development	1
	2.2	Existing Conditions	3
	2.2	2.1 Area Road Network	3
	2.2	2.2 Existing Intersections	3
	2.2	2.3 Existing Driveways	4
	2.2	2.4 Cycling and Pedestrian Facilities	4
	2.2	2.5 Existing Transit	7
	2.2	2.6 Existing Area Traffic Management Measures	8
	2.2	2.7 Existing Peak Hour Travel Demand	8
	2.2	2.8 Collision Analysis	10
	2.3	Planned Conditions	12
	2.3	3.1 Changes to the Area Transportation Network	12
	2.3	3.2 Other Study Area Developments	13
3		Study Area and Time Periods	14
	3.1	Study Area	14
	3.2	Time Periods	14
	3.3	Horizon Years	14
1		Exemption Review	14
5		Development-Generated Travel Demand	15
	5.1	Mode Shares	15
	5.2	Trip Generation	15
	5.3	Trip Distribution	16
	5.4	Trip Assignment	17
ŝ		Background Network Travel Demands	
	6.1	Transportation Network Plans	17
	6.2	Background Growth	
	6.3	Other Developments	18
7		Demand Rationalization	
	7.1	2024 Future Background Operations	
	7.2	2029 Future Background Operations	
	7.3	Modal Share Sensitivity	
3		Development Design	
	8.1	Design for Sustainable Modes	
	8.2	Circulation and Access	
9		Parking	
	9.1	Parking Supply	
	9.2	Spillover Parking	
L(Boundary Street Design	
11		Access Intersections Design	
	11.1	Location and Design of Access	23



11.2	Intersection Control	23
11.3	Access Intersection Design	23
11	L.3.1 2024 Future Total Access Intersection Operations	23
11	1.3.2 2029 Future Total Access Intersection Operations	24
11	L.3.3 Access Intersection MMLOS	25
11	L.3.4 Recommended Design Elements	25
12	Transportation Demand Management	26
12.1	Context for TDM	26
12.2	Need and Opportunity	26
12.3	TDM Program	26
13	Transit	26
13.1	Route Capacity	26
13.2	Transit Priority	27
14	Network Intersection Design	27
14.1	Network Intersection Control	27
14.2	Network Intersection Design	27
14	4.2.1 2024 Future Total Network Intersection Operations	27
14	1.2.2 2029 Future Total Network Intersection Operations	27
14	1.2.3 Network Intersection MMLOS	28
14	1.2.4 Recommended Design Elements	29
15	Summary of Improvements Indicated and Modifications Options	29
16	Conclusion	32
	of Figures 1: Area Context Plan	1
•	2: Concept Plan	
•	3: Study Area Pedestrian Facilities	
-	4: Study Area Cycling Facilities	
-	5:Existing Pedestrian Volumes	
•	6: Existing Cyclist Volumes	
U	7: Existing Study Area Transit Service	
_	8: Existing Study Area Transit Stops	
_	9: Existing Traffic Counts	
•	10: Study Area Collision Records – Representation of Study Area Collisions	
-	11: New Site Generation Auto Volumes	
-	12: 2024 Future Background Volumes	
-	13: 2029 Future Background Volumes	
-	14: 2024 Future Total Volumes	
-	15: 2029 Future Total Volumes	
U		
Table	e of Tables	
Tahla 1	· Intersection Count Date	0



Table 2: Existing Intersection Operations	9
Table 3: Study Area Collision Summary, 2016-2020	10
Table 4: Summary of Collision Locations, 2016-2020	11
Table 5: Somerset Street at O'Connor Street Collision Summary	12
Table 6: Exemption Review	14
Table 7: TRANS Trip Generation Manual Recommended Mode Shares – Ottawa Inner Area	15
Table 8: Trip Generation Person Trip Rates by Peak Period	15
Table 9: Total Residential Person Trip Generation by Peak Period	15
Table 10: Internal Capture Rates	16
Table 11: Trip Generation by Mode	16
Table 12: OD Survey Distribution – Ottawa Inner	17
Table 13: TRANS Regional Model Projections – Study Area Growth Rates	18
Table 14: 2024 Future Background Intersection Operations	19
Table 15: 2029 Future Background Intersection Operations	21
Table 16: Boundary Street MMLOS Analysis	22
Table 17: 2024 Future Total Access Intersection Operations	24
Table 18: 2029 Future Total Access Intersection Operations	25
Table 19: Trip Generation by Transit Mode	26
Table 20: 2024 Future Total Network Intersection Operations	27
Table 21: 2029 Future Total Network Intersection Operations	28
Table 22: Study Area Intersection MMLOS Analysis	28

List of Appendices

Appendix A – TIA Screening Form and Certification Form

Appendix B – Turning Movement Count Data

Appendix C – Synchro Intersection Worksheets – Existing Conditions

Appendix D - Collision Data

Appendix E - TRANS Model Plots

Appendix F – Background Development Volumes

Appendix G – Synchro Intersection Worksheets – 2024 Future Background Conditions

Appendix H – Synchro Intersection Worksheets – 2029 Future Background Conditions

Appendix I – MMLOS Analysis

Appendix J – Synchro Intersection Worksheets – 2024 Future Total Conditions

Appendix K – Synchro Intersection Worksheets – 2029 Future Total Conditions

Appendix L – TDM Checklist



1 Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for the TIA Study PM. As shown in the Screening Form, a TIA is required including the Design Review component and the Network Impact Component. This study is for a site plan application.

2 Existing and Planned Conditions

2.1 Proposed Development

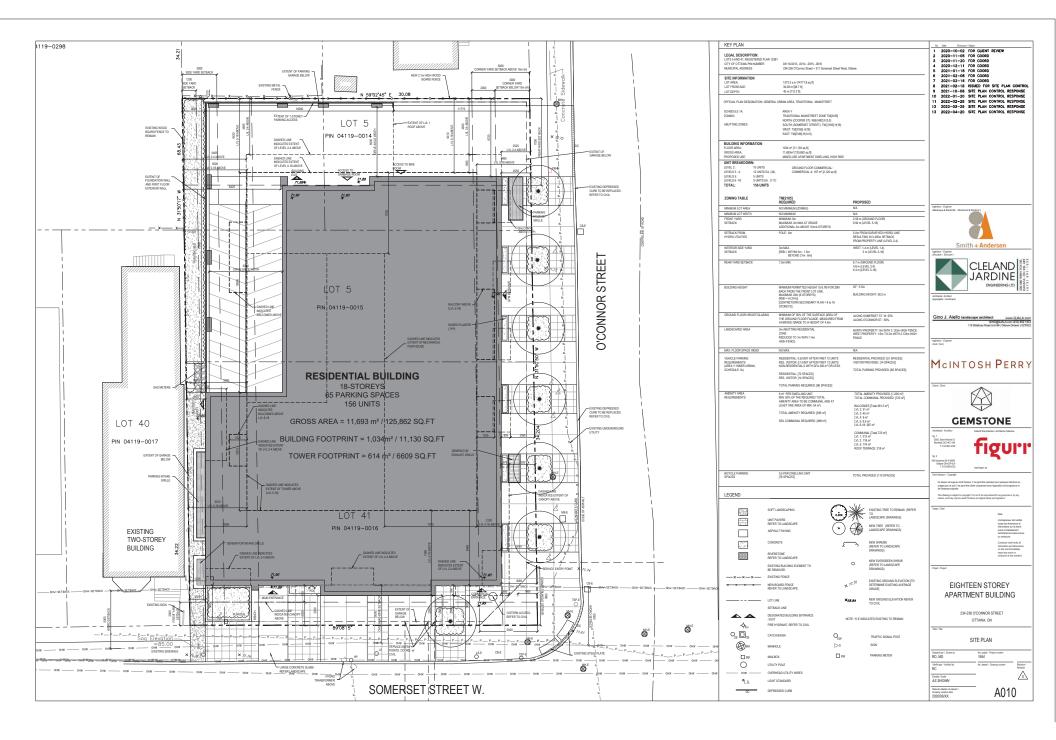
The proposed development includes a 18-storey, 156-unit apartment/mixed-use building with 2,120 sq. ft. of ground floor commercial space to be constructed in a single phase and occupied by 2024 and includes 51 resident parking spaces, 14 visitor parking spaces, and 110 bicycle parking spaces. The access is proposed as a right-in/right-out access onto O'Connor Street. The subject site is zoned as Traditional Mainstreet (TM[2185]) and Residential Fifth Density (R5B[482]), and is within the area of consideration of the Centretown SDP/CDP, the Somerset Traditional Mainstreet DPA, and the Downtown Ottawa Urban Design Strategy. Figure 1 illustrates the Study Area Context. Figure 2 illustrates the proposed concept plan.



Figure 1: Area Context Plan

Source: http://maps.ottawa.ca/geoOttawa/ Accessed: May 17, 2022





2.2 Existing Conditions

2.2.1 Area Road Network

Bank Street: Bank Street is a City of Ottawa arterial road with a two-lane urban cross-section, with sidewalks on both sides of the street and with on-street parking permitted on the west side of the road south of Lewis Street, and between Somerset Street and Lisgar Street and on the east side of the road between Gilmour Street and MacLaren Street, and between Lisgar Street and Gloucester Street (each no stopping 7:00-9:00AM and 3:30-5:30PM). The posted speed limit is 50 km/h and the Ottawa Official Plan reserves a 20.0 metre right of way within the study area.

O'Connor Street: O'Connor Street is a one-way, southbound City of Ottawa arterial road with a two-lane urban cross-section with a parking lane on the west side of the road and with a two-way curb-separated bike lane on the east side of the road and with sidewalks on both sides of the road south of Laurier Avenue W. North of Laurier Avenue W within the study area, it has a three lane urban cross-section with sidewalks are on both sides of the street and on-street parking permitted on the west side of the road (no stopping 3:30PM-5:30PM). The unposted speed limit is 50 km/h, and the Ottawa Official Plan reserves a 20.0 metre right of way within the study area.

Metcalfe Street: Metcalfe Street is a one-way, northbound City of Ottawa arterial road with a three-lane urban cross-section with sidewalks on both sides of the street and on-street parking permitted on the east side of the road (no stopping 7:00-9:00AM, 3:30PM-5:30PM). The unposted speed limit is 50 km/h and the Ottawa Official Plan reserves a 20.0 metre right of way within the study area.

Somerset Street W: Somerset Street W is a City of Ottawa arterial road west of Elgin Street and a collector road east of Elgin Street, each with a two-lane urban cross-section with sidewalks on both sides of the street. On-street parking is permitted on both sides of the road west of Bank Street (no stopping 7:00-9:00AM, 3:30PM-5:30PM), the south side of the road between Bank Street and O'Connor Street, between Metcalfe Street and Elgin Street (no stopping 7:00-9:00AM, 3:30PM-5:30PM), and on the south side of the road east of Elgin Street. The unposted speed limit is 50 km/h, the Ottawa Official Plan reserves a 20.0 metre right of way west of Elgin Street and the existing right of way varies between 18.0 and 20.5 metres to the east within the study area.

Gilmour Street: Gilmour Street is a City of Ottawa one-way, eastbound local road with a one-lane urban cross-section west of Metcalfe Street, and a two-way, two-lane urban cross-section to the east, each with sidewalks on both sides of the street. On-street parking is permitted on the south side of the road west of Bank Street, and on the north side of the road to the east. The unposted speed limit is 50 km/h, and the existing right of way is 17.0 metre within the study area.

2.2.2 Existing Intersections

Given the downtown environment, the following intersections were confirmed with City staff for inclusion and analysis:

Bank Street at Somerset Street W

The intersection of Bank Street at Somerset Street is a signalized intersection. The northbound and southbound approaches each consist of a shared through/right-turn lane. The eastbound approach consists of a shared left-turn/though lane and an auxiliary right-turn lane, and the westbound approach consists of an auxiliary left-turn lane and a shared through/right-turn lane. Northbound left turns are prohibited from 7:00-9:00AM and 3:30-5:30PM buses excepted,



southbound left turns are prohibited bicycles excepted, and right turns on red are prohibited on all approaches from 7:00AM-7:00PM.

O'Connor Street at Somerset Street W The intersection of O'Connor Street at Somerset Street is a signalized

intersection. The southbound approach consists of a curb-separated two-way bike lane, a shared left-turn/through lane, and a shared through/right-turn lane. The eastbound approach consists of a shared though/right-turn lane, and the westbound approach consists of a shared left-turn/through lane and a left-turn bike box. No turn

restrictions were noted.

Metcalfe Street at Somerset Street W The intersection of Metcalfe Street at Somerset Street is a signalized

intersection. The northbound approach consists of a shared left-turn/through lane, a through lane, and a shared left-turn/through lane. The eastbound approach consists of a shared left-turn/though lane, and the westbound approach consists of a shared through/right-

turn lane. No turn restrictions were noted.

O'Connor Street at Gilmour Street The intersection of O'Connor Street at Gilmour Street is a signalized

intersection. The southbound approach consists of a curb-separated two-way bike lane, a shared left-turn/through lane, and a through lane. The eastbound approach consists of a shared though/right-turn

lane. No turn restrictions were noted.

2.2.3 Existing Driveways

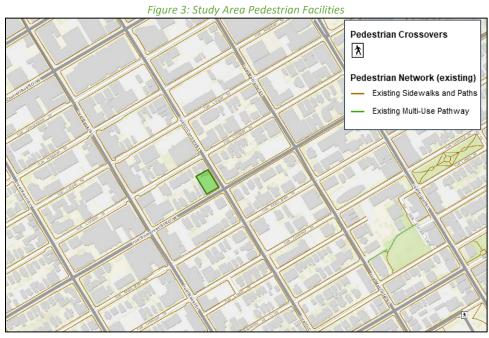
Six driveways on the west side and seven driveways on the east side of O'Connor Street, and nine driveways on the north side and 13 driveways on the south side of Somerset Street W are found along boundary streets within 200 metres of the proposed site access. Driveways access detached dwellings, low-rise, mid-rise, and high-rise residential land uses, and restaurants, office buildings, and embassies. Two-way accesses for the existing site are present, onto each O'Connor Street and Somerset Street W and are to be removed as part of the subject development plan.

2.2.4 Cycling and Pedestrian Facilities

Figure 3 illustrates the pedestrian facilities in the study area and Figure 4 illustrates the cycling facilities.

Sidewalks are provided along both sides of all study area streets. Cycling facilities include a two-way curb-separated bike lane along O'Connor Street, and curb-separated bike lanes on Laurier Avenue W. O'Connor Street, Metcalfe Street, Laurier Avenue W, and Somerset Street W are spine cycling routes, and Bank Street and Elgin Street are local routes.





Source: http://maps.ottawa.ca/geoOttawa/ Accessed: May 17, 2022



Figure 4: Study Area Cycling Facilities

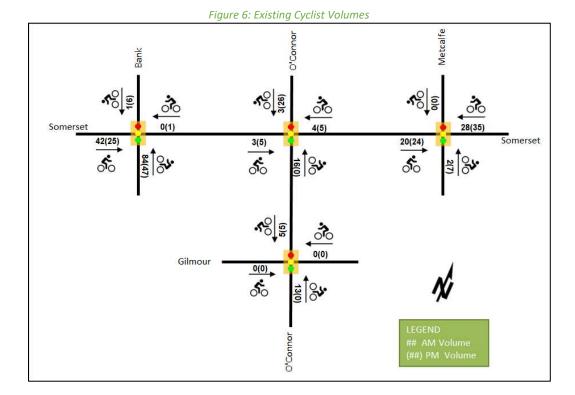
Source: http://maps.ottawa.ca/geoOttawa/ Accessed: May 17, 2022

Pedestrian and cyclist volumes included in study area intersection counts, presented in Section 2.2.7, have been compiled and are illustrated in Figure 5 and Figure 6, respectively.



Bank Somerset Somerset Gilmour

Figure 5:Existing Pedestrian Volumes



C|G|H

2.2.5 Existing Transit

Within the study area, the routes #6, #7, #11 travel along Bank Street with route #11 continuing along Somerset Street W, and routes #5, #14, 114 travel along Elgin Street. The frequency of these routes within proximity of the proposed site currently are:

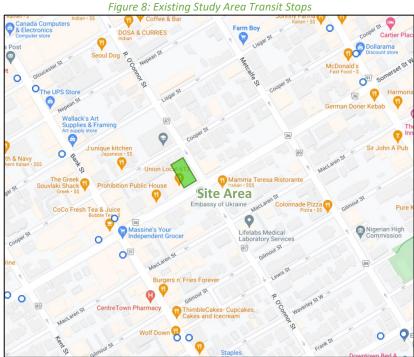
- Route #5 30-minute service all day
- Route #6 10-15-minute service all day, 30-minute service during the evening
- Route #7 10-15-minute service all day, 30-minute service during the evening
- Route #11 15-20-minute service all day
- Route #14 –15-minute service all day, 30-minute service during the evening
- Route #114 two buses per peak direction/period per day

Figure 7 illustrates the transit system map in the study area and Figure 8 illustrates nearby transit stops.



Source: http://www.octranspo.com/ Accessed: May 17, 2022





Source: http://www.octranspo.com/ Accessed: May 17, 2022

2.2.6 Existing Area Traffic Management Measures

Bulb-outs along the minor roads intersecting O'Connor Street, and tight corner radii at these intersections, textured crossings along arterial roads, turn restrictions at the intersection of Bank Street and Somerset Street W, and channelization on MacLaren Street at O'Connor Street constitute area traffic management measures.

2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa for the existing Study Area intersections. Table 1 summarizes the intersection count dates.

Intersection

Bank Street at Somerset Street

O'Connor Street at Somerset Street

Metcalfe Street at Somerset Street

O'Connor Street at Gilmour Street

Tuesday, March 21, 2017

Thursday, May 2, 2019

O'Connor Street at Gilmour Street

Tuesday, March 21, 2017

Table 1: Intersection Count Date

Figure 9 illustrates the existing traffic counts and Table 2 summarizes the existing intersection operations. The level of service for signalized intersections is based on volume to capacity ratio (v/c) calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. Detailed turning movement count data is included in Appendix B and the Synchro worksheets are provided in Appendix C.



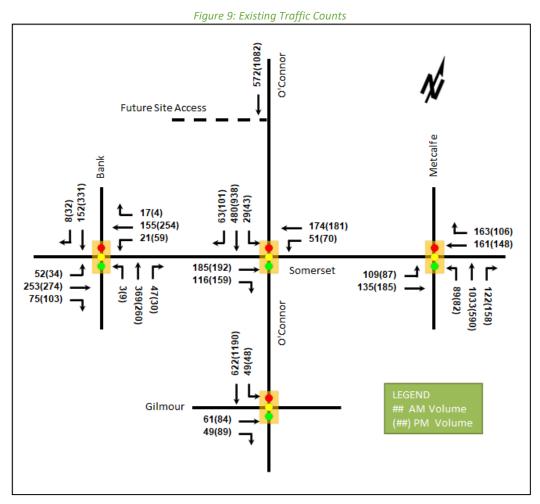


Table 2: Existing Intersection Operations

Intersection	Lana		AM P	eak Hour			PM Pe	eak Hour	
intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EBL/T	В	0.67	29.7	68.3	В	0.66	28.9	68.2
	EBR	Α	0.25	21.1	18.9	Α	0.29	21.3	24.2
Bank Street at	WBL	Α	0.11	9.2	m2.6	Α	0.31	15.7	m6.6
Somerset Street W	WBT/R	Α	0.35	9.9	15.0	Α	0.51	16.4	m34.3
Signalized	NBT/R	Α	0.54	14.7	65.6	Α	0.39	12.4	43.6
	SBT/R	Α	0.20	10.1	22.4	Α	0.47	13.5	54.7
	Overall	Α	0.59	17.6	-	Α	0.54	17.9	-
O/Common Chroat at	EBT/R	Α	0.49	18.6	46.9	С	0.72	45.8	#85.7
O'Connor Street at Somerset Street W	WBL/T	Α	0.41	24.9	m49.1	В	0.68	22.8	#34.5
Signalized	SB	Α	0.46	15.7	44.2	С	0.78	20.1	94.2
Signanzea	Overall	Α	0.46	18.4	-	С	0.75	25.9	-
Metcalfe Street at	EBL/T	С	0.71	24.6	#68.9	Α	0.60	22.4	m44.4
	WBT/R	В	0.63	23.9	66.3	Α	0.49	19.5	48.1
Somerset Street W Signalized	NB	В	0.69	17.8	67.9	Α	0.50	13.5	38.6
Signanzea	Overall	В	0.70	19.8	-	Α	0.54	16.4	-



Interception	Lana		AM P	eak Hour			PM Pe	eak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
O'Connor Street at	EBT/R	Α	0.33	19.0	22.3	Α	0.53	25.1	36.3
Gilmour Street	SBL/T	Α	0.35	3.7	15.5	В	0.64	5.8	24.3
Signalized	Overall	Α	0.32	5.9	-	Α	0.60	8.2	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres

Peak Hour Factor = 0.90

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours, the study area intersection operates well. The intersection of O'Connor Street and Somerset Street W shows potential for queuing on the eastbound and westbound movements during the PM peak hour, as does the eastbound movement at the intersection of Metcalfe Street and Somerset Street W during the AM peak hour.

2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study are road network. Table 3 summarizes the collisions types and conditions in the study area, Figure 10 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix D.

Table 3: Study Area Collision Summary, 2016-2020

		Number	%
Total (Collisions	56	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	15	27%
	Property Damage Only	41	73%
	Approaching	1	2%
	Angled	6	11%
	Rear end	4	7%
Initial Impact Tuno	Sideswipe	15	27%
Initial Impact Type	Turning Movement	10	18%
	SMV Unattended	11	20%
	SMV Other	8	14%
	Other	1	2%
	Dry	43	77%
	Wet	6	11%
Road Surface Condition	Loose Snow	4	7%
	Slush	2	4%
	Packed Snow	1	2%
Pedestrian Involved		6	11%
Cyclists Involved		8	14%



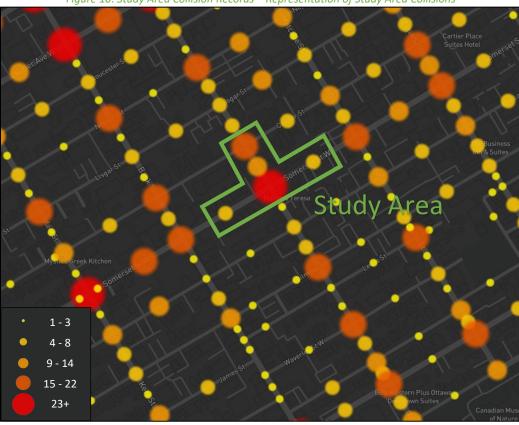


Figure 10: Study Area Collision Records – Representation of Study Area Collisions

Table 4: Summary of Collision Locations, 2016-2020

	Number	%
Intersections / Segments	56	100%
Cooper St at O'Connor St	12	21%
Somerset St at O'Connor St	22	39%
Somerset St W btwn Bank St & O'Connor St	5	9%
Somerset St W btwn O'Connor St & Metcalfe St	6	11%
O'Connor St btwn Cooper St & Somerset St	11	20%

Within the study area, the intersection of Somerset Street at O'Connor Street is noted to have experienced higher collisions than other locations. Table 5 summarizes the collision types and conditions for the intersection of Somerset Street at O'Connor Street.



Table 5: Somerset Street at O'Connor Street Collision Summary

		Number	%
Total Collisions		22	100%
	Fatality	0	0%
Classification	Non-Fatal Injury	6	27%
	Property Damage Only	16	73%
	Angle	2	9%
	Rear end	4	18%
Initial Impact	Sideswipe	5	23%
Type	Turning Movement	5	23%
	SMV Other	5	23%
	Other	1	5%
	Dry	15	68%
Road Surface	Wet	3	14%
Condition	Loose Snow	3	14%
	Slush	1	5%
Pedestrian Involved		3	14%
Cyclists Involved		3	14%

The Somerset Street at O'Connor Street intersection had a total of 22 collisions during the 2016-2020 time period, with 16 involving property damage only and the remaining six having non-fatal injuries. The collision types are most represented by sideswipe, turning movement, and SMV (other) each with five collisions, followed by rear end with four collisions, two angle and one collision as other. Vehicle, pedestrian, and cyclist volumes are high at this intersection, and no single collision type is overly represented at this location. Weather conditions may affect collisions at this location.

2.3 Planned Conditions

2.3.1 Changes to the Area Transportation Network

The subject development is within the Centretown CDP Area. As such, it is subject to the planning polices outlined in the CDP. The CDP makes the following general propositions: that a "Safe Crossing Project" be initiated along arterial route intersections, however makes no specific recommendations; to expand the cycle network with onstreet cycling routes and/or facilities along Bank Street, Metcalfe Street, Somerset Street W and Gladstone Avenue; pursue pedestrian comfort improvements along Metcalfe, Elgin, Bay, Somerset Streets; convert Metcalfe to a two-way road as a pilot study including the conversion Metcalfe Street's museum frontage to greenspace, with no specified timeline of implementation.

The subject development is also within the area considered by the Downtown Ottawa Urban Design Strategy, which includes several recommendations for the study area including priority consideration for the conversion of O'Connor Street and Metcalfe Street to two-way roads, the conversion of the museum frontage on Metcalfe Street to greenspace, and study area streetscape improvements, each with no specified timeline of implementation.

Within the Transportation Master Plan, the Rapid Transit and Transit Priority Network's Affordable Network diagram shows isolated transit priority measures on Bank Street, Somerset Street W west of Bank Street, and Elgin Street.

From the City of Ottawa's Planned Construction Projects Portal, the intersection of Nepean Street and O'Connor Street is due for intersection improvements this year and the intersection of Waverly Street W and Metcalfe Street is due for signalization within 1-2 years.



2.3.2 Other Study Area Developments

70 Gloucester Street, 89-91 Nepean Street

The application includes a zoning by-law amendment permitting the construction of two 27-storey residential apartment buildings consisting of 488 residential dwelling units and 2350 ft² of ground floor retail. The development is anticipated to generate 203 AM and 228 PM peak hour new two-way auto trips. (Novatech 2019)

96 Nepean Street

The application includes a site plan for a 27-storey residential building consisting of 201 residential dwelling units. The development is anticipated to generate an additional 59 AM and 57 PM peak hour new two-way auto trips. (Novatech, 2011) The file was last updated in 2012.

180 Metcalfe Street

The application includes an official plan amendment, zoning by-law amendment and a site plan control revision application permitting the construction of a 30-storey mixed-use building with 311 dwelling units and ground floor commercial uses. The development is anticipated to create a net increase of 40 AM peak hour outbound auto trips and 36 PM peak hour inbound auto trips. (Parsons 2018)

318-320 Lisgar Street, 235-241 Bank Street

The application includes a site plan for the construction of a six-storey mixed-use building and a zoning by-law amendment to permit office uses on the second floor. The development is anticipated to produce 11 AM and 16 PM peak hour new two-way auto trips. (Delcan, 2014)

390-394 Bank Street

The application includes a site plan for the construction of a 9-storey mixed use building with 127 residential dwelling units and 6,750 m² of ground floor commercial space. The development is anticipated to be built by 2024 and is forecasted to generate 16 AM and 19 PM peak hour new two-way vehicle trips. (CGH, 2021)

322 Waverley Street

The application includes an official plan and zoning by-law amendment application to permit the construction of a 6-storey building with 27 residential dwelling units. No TIA is available for this development.

257 Lisgar Street, 108 Nepean Street

The application includes a zoning by-law amendment to permit the construction of a 27-storey mixed-use building with 295 residential dwelling units and 120 m² of ground-floor commercial space. The development is anticipated to be built by 2022 and is forecasted to generate 31 AM and 31 PM peak hour new two-way vehicle trips. (Parsons, 2021)

331 Cooper Street

The application includes a zoning by-law amendment and site plan for converting an existing 7-storey non-residential building to an 8-storey mid-rise apartment building. The development is anticipated to be built by 2021. No TIA is available for this development.

267 O'Connor Street

The application includes a zoning by-law amendment to permit the construction of two 30-storey residential buildings with a total of 547 residential dwelling units in two phases. Phase 1 is anticipated to be built by 2023 and is forecasted to generate 59 new AM and 65 new PM two-way peak-hour auto trips. Phase 2 is anticipated to be built by 2025 and is forecasted to generate 62 new AM two-way peak hour auto trips and 69 new PM two-way peak hour auto trips. (Parsons, 2020)



359 Kent Street, 436-444 MacLaren Street

The application includes official plan amendment and zoning by-law amendment to permit the construction of a 35-storey mixed-use building with a total of 405 apartment units, 21,388 ft² of office space, and 7,833 ft² of commercial space. The development is assumed to be built by 2024 and is forecasted to generate 31 AM and 32 PM peak hour new two-way vehicle trips. (Parsons, 2021)

3 Study Area and Time Periods

3.1 Study Area

The study area will include the intersections of Bank Street at Somerset Street W, O'Connor Street at Somerset Street W, Metcalfe Street at Somerset Street W, O'Connor Street at Gilmour Street, and the intersection of site accesses and O'Connor Street.

The boundary roads will be O'Connor Street and Somerset Street W, and screenline 36 is south of Laurier Ave within proximity to the site however will not be reviewed as part of this study.

3.2 Time Periods

As the proposed development is composed primarily of residential units the AM and PM peak hours will be examined.

3.3 Horizon Years

The anticipated build-out year is 2024. As a result, the full build-out plus five years horizon year is 2029.

4 Exemption Review

Table 6 summarizes the exemptions for this TIA.

Table 6: Exemption Review

Module	Element	Explanation	Exempt/Required
Design Review Compo	nent		
4.1 Development	4.1.2 Circulation and Access	Only required for site plans	Required
Design	4.2.3 New Street Networks	Only required for plans of subdivision	Exempt
	4.2.1 Parking Supply	Only required for site plans	Required
4.2 Parking	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Required
Network Impact Comp	onent		
4.5 Transportation Demand Management	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Exempt
4.8 Network Concept		Only required when proposed development generates more than 200 person-trips during the peak hour in excess	Exempt



Module	Element	Explanation	Exempt/Required
		of equivalent volume permitted by	
		established zoning	

5 Development-Generated Travel Demand

5.1 Mode Shares

Examining the mode shares recommended in the TRANS Trip Generation Manual (2020) for the subject district, derived from the most recent National Capital Region Origin-Destination survey (OD Survey), the existing average district mode shares by land use for Ottawa Inner Area have been summarized in Table 7.

Table 7. TRANS Ti	in Generation	า Manual Recommend	ded Mode Shares	— Ottawa Inner Δrea

Travel Mode	Multi-Unit	(High-Rise)	Commercial Generator	
Travel Mode	AM	PM	AM	PM
Auto Driver	26%	25%	39%	22%
Auto Passenger	6%	8%	2%	4%
Transit	28%	21%	16%	12%
Cycling	5%	6%	3%	4%
Walking	34%	39%	40%	58%
Total	100%	100%	100%	100%

5.2 Trip Generation

This TIA has been prepared using the vehicle and person trip rates for the residential dwellings using the TRANS Trip Generation Manual (2020) and the vehicle trip rates and derived person trip rates for commercial component from the ITE Trip Generation Manual 10th Edition (2017) using the City-prescribed conversion factor of 1.28. Table 8 summarizes the person trip rates for the proposed residential land uses for each peak period and the person trip rates for the non-residential land uses by peak hour.

Table 8: Trip Generation Person Trip Rates by Peak Period

Land Use	Land Use Code	Peak Period	Vehicle Trip Rate	Person Trip Rates	
Marile: Haris (Hink Dine)	221 & 222	AM	-	0.80	
Multi-Unit (High-Rise)	(TRANS)	PM -		0.90	
	Land Use	Daale			
Land Use	Code	Peak Hour	Vehicle Trip Rate	Person Trip Rates	
Land Use Strip Retail Plaza			·	•	

Using the above person trip rates, the total person trip generation has been estimated. Table 9 summarizes the total person trip generation for the residential land uses and for the non-residential land uses.

Table 9: Total Residential Person Trip Generation by Peak Period

Land Use	Units	1	AM Peak Perio	d	PM Peak Period			
Land OSE		In	Out	Total	In	Out	Total	
Multi-Unit (High-Rise)	156	39	86	125	81	59	140	
Land Han	Units /		AM Peak Hou	r	PM Peak Hour			
Land Use	GFA	In	Out	Total	In	Out	Total	
Retail (<40k sq. ft.)	2,120	4	2	6	9	9	18	



Internal capture rates from the ITE Trip Generation Handbook 3rd Edition have been assigned to the development's retail component for mixed-use developments. The rates summarized in Table 10 represent the percentage of trips to/from the retail use based on the residential component.

Table 10: Internal Capture Rates

Landillea	Α	М	PM		
Land Use	In	Out	In	Out	
Residential to/from Retail	17%	14%	10%	26%	

Using the above mode share targets, the internal capture rates, and the person trip rates, the person trips by mode have been projected. Trip generation by peak hour has been forecasted using the prescribed peak period conversion factors presented in the TRANS Trip Generation Manual (2020) for the residential component. Table 11 summarizes the residential trip generation and the non-residential trip generation by mode and peak hour.

Table 11: Trip Generation by Mode

		F	M Peak F	lour	·	P	M Peak F	lour	
٦	Fravel Mode	Mode Share	In	Out	Total	Mode Share	In	Out	Total
	Auto Driver	26%	5	11	16	25%	9	7	15
it e)	Auto Passenger	6%	1	2	4	8%	3	2	5
Multi-Unit (High-Rise)	Transit	28%	6	13	19	21%	8	6	14
ulti igh	Cycling	5%	1	2	3	6%	2	2	4
ΣΞ	Walking	34%	8	17	25	39%	17	12	29
	Total	100%	20	43	63	100%	36	26	62
ft.)	Auto Driver	39%	1	1	2	22%	3	3	6
≠	Auto Passenger	2%	0	0	0	4%	0	0	1
Retail (<40k sq.	Transit	16%	0	0	1	12%	1	1	2
104	Cycling	3%	0	0	0	4%	0	0	1
<u>×</u>	Walking	40%	1	1	2	58%	5	4	9
etai	Internal Capture	varies	-1	0	-1	varies	-1	-2	-3
ž	Total	100%	3	2	5	100%	8	7	15
	Auto Driver	-	6	12	18	-	12	10	21
	Auto Passenger	-	1	2	4	-	3	2	6
<u>ta</u>	Transit	-	6	13	20	-	9	7	16
Total	Cycling	-	1	2	3	-	2	2	5
	Walking	-	9	18	27	-	22	16	38
	Total	-	23	45	68	-	44	33	77

As shown above, a total of 18 AM and 21 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

5.3 Trip Distribution

To understand the travel patterns of the subject development, the OD Survey has been reviewed to determine the travel for residential uses which were applied based on the build-out of Ottawa Inner. Table 12 below summarizes the distributions.



Table 12: OD Survey	v Distribution -	– Ottawa Inne	er
---------------------	------------------	---------------	----

To/From	Residential % of Trips	Via
North	10%	Metcalfe/O'Connor
South	35%	Metcalfe/O'Connor
East	25%	10% Somerset, 15% Metcalfe/O'Connor
West	30%	10% Somerset, 20% Metcalfe/O'Connor
Total	100%	-

5.4 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the study area road network. Figure 11 illustrates the new site generated auto volumes.

Future Site Access

12(10)

12(10)

12(10)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

11(1)

Figure 11: New Site Generation Auto Volumes

6 Background Network Travel Demands

6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3. None of the confirmed projects listed are expected to have any impact on the study area intersection operations.



6.2 Background Growth

A review of the background projections from the City's TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways. Table 13 summarizes the results of the model and the projections are provided in Appendix E.

Table 13: TRANS Regional Model Projections – Study Area Growth Rates

Chunch	Direction Growth %	from 2011 to 2031	Direction Growth % from Existing to 2031			
Street	Eastbound	Westbound	Eastbound	Westbound		
Gilmour St	-1.23%	-	-0.97%	-		
Somerset St W	-0.59%	-0.59% -3.38%		-3.62%		
	Northbound	Southbound	Northbound	Southbound		
Bank St	-0.50%	1.618%	-1.82%	-2.54%		
Metcalfe St	0.62%	-	-0.37%	-		

Consistent with a downtown development context, area growth has either been achieved, or is projected to be negative. Consequently, and in keeping with adjacent development TIAs, no growth will be applied to the study area road network.

6.3 Other Developments

The background developments explicitly considered in the background conditions (Section 6.2) include:

- 70 Gloucester Street, 89-91 Nepean Street
- 180 Metcalfe Street
- 390-394 Bank Street
- 257 Lisgar Street, 108 Nepean Street
- 267 O'Connor Street
- 359 Kent Street, 436, 444 MacLaren Street

The background development volumes within the study area have been provided in Appendix F.

7 Demand Rationalization

7.1 2024 Future Background Operations

Figure 13 illustrates the 2024 background volumes and Table 15 summarizes the 2024 background intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2024 future background horizon are provided in Appendix G.



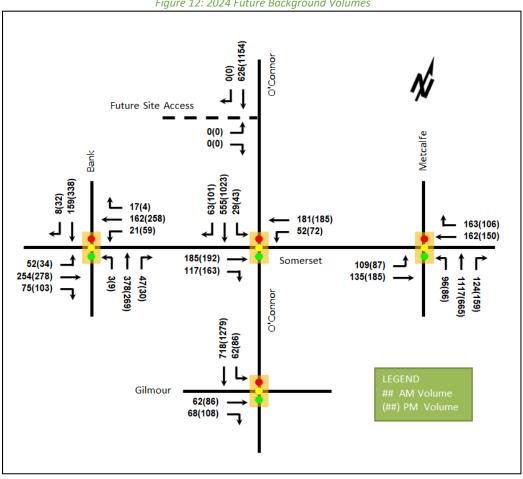


Figure 12: 2024 Future Background Volumes

Table 14: 2024 Future Background Intersection Operations

lusta na a ati a n	Laura		AM P	eak Hour			PM Pe	eak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EBL/T	Α	0.60	27.2	60.6	Α	0.60	26.9	61.1
	EBR	Α	0.23	20.7	17.5	Α	0.26	20.9	22.1
Bank Street at	WBL	Α	0.10	8.7	m2.4	Α	0.25	14.6	m6.4
Somerset Street W	WBT/R	Α	0.33	9.5	13.7	Α	0.46	16.0	m31.7
Signalized	NBT/R	Α	0.50	13.9	58.8	Α	0.37	12.0	39.8
	SBT/R	Α	0.19	10.0	21.2	Α	0.43	12.9	49.0
	Overall	Α	0.54	16.5	-	Α	0.49	17.0	-
O/Common Chroat at	EBT/R	Α	0.44	16.3	40.0	В	0.65	43.9	76.7
O'Connor Street at	WBL/T	Α	0.37	24.2	m45.8	Α	0.57	18.7	30.1
Somerset Street W Signalized	SB	Α	0.47	15.9	45.3	С	0.76	19.3	89.5
Signanzea	Overall	Α	0.44	17.7	-	С	0.71	24.1	-
Matalfa Ctroot at	EBL/T	Α	0.58	17.8	41.6	Α	0.52	19.8	m41.7
Metcalfe Street at Somerset Street W	WBT/R	Α	0.57	22.0	58.3	Α	0.45	18.4	42.7
Signalized	NB	В	0.67	17.3	64.8	Α	0.49	13.6	38.3
Signanzea	Overall	В	0.62	18.2	-	Α	0.50	15.6	-



Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
O'Connor Street at	EBT/R	Α	0.34	16.6	21.4	Α	0.54	24.6	36.0
Gilmour Street	SBL/T	Α	0.36	3.3	14.3	В	0.64	5.8	19.9
Signalized	Overall	Α	0.33	5.2	-	Α	0.59	8.1	-

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres

Peak Hour Factor = 1.00

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours, the study area intersections operate similarly to existing conditions, with operational improvement for all study area intersections due to the peak hour factor increasing from 0.90 to 1.00. No new capacity issues are noted

2029 Future Background Operations

Figure 13 illustrates the 2029 background volumes and Table 15 summarizes the 2029 background intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2029 future background horizon are provided in Appendix H.

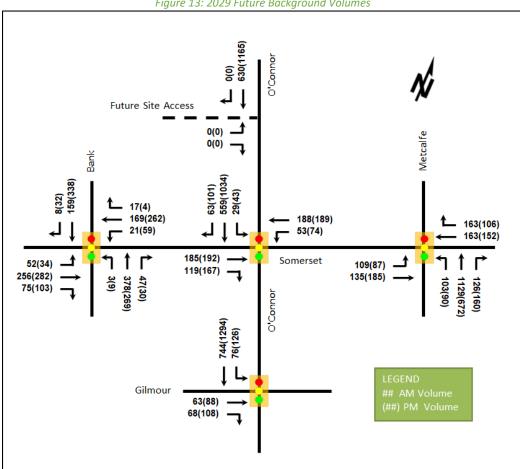


Figure 13: 2029 Future Background Volumes



Table 15: 2029 Future Background Intersection Operations

Intersection	Long		AM P	eak Hour			PM Pe	eak Hour	
Intersection	Lane	LOS	V/C	Delay (s)	Q (95 th)	LOS	V/C	Delay (s)	Q (95 th)
	EBL/T	В	0.61	27.4	60.9	В	0.61	27.1	62.1
	EBR	Α	0.23	20.7	17.5	Α	0.26	20.9	22.1
Bank Street at	WBL	Α	0.10	8.6	m2.3	Α	0.25	14.5	m6.0
Somerset Street W	WBT/R	Α	0.34	9.6	14.0	Α	0.47	15.8	m31.7
Signalized	NBT/R	Α	0.50	13.9	58.8	Α	0.37	12.0	39.8
	SBT/R	Α	0.19	10.0	21.2	Α	0.43	12.9	49.0
	Overall	Α	0.54	16.5	-	Α	0.50	17.0	-
O/Common Street of	EBT/R	Α	0.45	16.5	40.3	В	0.66	44.3	77.7
O'Connor Street at Somerset Street W	WBL/T	Α	0.38	23.8	m46.2	Α	0.59	19.5	30.9
Signalized	SB	Α	0.47	16.0	45.5	С	0.76	19.5	91.1
Signanzea	Overall	Α	0.44	17.7	-	С	0.71	24.5	-
Metcalfe Street at	EBL/T	Α	0.58	17.9	41.7	Α	0.52	20.0	m41.5
Somerset Street W	WBT/R	Α	0.57	22.1	58.7	Α	0.45	18.6	43.3
Signalized	NB	В	0.68	17.5	66.3	Α	0.50	13.7	39.1
Signanzea	Overall	В	0.63	18.3	-	Α	0.50	15.8	-
O'Connor Street at	EBT/R	Α	0.35	16.9	21.6	Α	0.54	25.1	36.7
Gilmour Street	SBL/T	Α	0.38	3.5	15.8	В	0.67	6.6	32.3
Signalized	Overall	Α	0.35	5.4	-	В	0.62	8.8	-

Notes: Saturation flow rate of 1800 veh/h/lane Queue is measured in metres

Peak Hour Factor = 1.00

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

During both the AM and PM peak hours, the study area intersections at the 2029 future background horizon operate similarly to the 2024 future background conditions. No new capacity issues are noted.

7.3 Modal Share Sensitivity

No capacity constraints have been noted at any study area intersections. Given the application of unmodified district mode shares and considering the downtown land-use context of the site, rationalization for adjusted demand is not required for this TIA.

8 Development Design

8.1 Design for Sustainable Modes

The proposed development is a mixed-use building with vehicle parking located underground and bicycle parking, located internal to the building, accessing the site's driveway. Bicycle parking is located in a secure storage room on the main floor and in the three underground parking levels. The ramps to underground parking are proposed as having 16% grades with 8% transition grades. Hard surface connections are provided from all proposed building entrances to the surrounding pedestrian facilities, additionally providing access to area transit. Stops for the routes #6, #7, #11, and the southbound routes #5, #14, and #114 are located within 400 metres of the proposed building entrances, with the northbound routes #5, #14, and #114 are just beyond this distance on the opposite side of Elgin Street.

8.2 Circulation and Access

Vehicle and bicycle access are proposed via the right-in/right-out access onto O'Connor Street. Any fence or wall separating the properties will need a setback of 3.5 metres from the near edge of the sidewalk to provide sight lines for exiting vehicles. The sight lines can be permitted through a maximum height of 0.75 metres through the setback or a transparent structure, e.g., chain link fence.



The internal garbage storage area accesses the site's driveway, and as such, garbage collection may either be collected in the vehicle driveway area or carted further to be collected on O'Connor Street. Emergency services are assumed to be able to access the site via its two arterial road frontages.

9 Parking

9.1 Parking Supply

The site provides 110 bicycle spaces (0.71 spaces per unit), 51 tenant vehicle parking spaces (0.35 spaces per unit after the first 12), and 14 visitor vehicle parking spaces (0.1 spaces per unit after the first 12). The site is seeking a parking exemption to permit the site to provide parking below the zoning by-law minimum parking provisions of 65 tenants spaces, given the site is in Area Y and as all parking is located underground. The visitor and bicycle parking minimum parking requirements are being met. All parking is located underground across three levels. The trip forecasts from the site, based on the surrounding area of the City, support the decreased parking rate and would serve the estimated auto travel to and from the site during peak hours.

9.2 Spillover Parking

Examining the trip generation presented in Section 5.2, based upon existing area mode shares, primary auto trips generated by the proposed development are 18 two-way AM peak hour auto trips, and 21 two-way PM peak hour auto trips. Given that the site is proposing a total of 51 tenant parking spaces and 14 visitor parking spaces, it is assumed that the parking demand will be satisfied even with the reduced parking rate for tenant parking. Notwithstanding the alignment of the modal share targets and proposed parking rates, effects from any remaining potential spillover from the tenant parking will be examined.

A review of the Centretown LAPS, over 600 spaces off-street parking spaces are provided within 400 metres of the site and were noted to have available capacity throughout the week. The on-street parking is generally limited to two-hours and are also noted to have residual capacity to support spill over parking. The weekends are noted to have reduced capacity during Saturdays and Sunday morning.

Overall, while the site is anticipated to meet the parking demands generated by the type of development in this area of the City, the surrounding area is expected to be able to support the minimal theoretical amount of residual parking demands from the site. Additionally, the marketing of the site and TDM measures should promote non-auto travel and limited parking for residents.

10 Boundary Street Design

Table 16 summarizes the MMLOS analysis for the boundary streets of O'Connor Street and Somerset Street West. The existing and future conditions for both streets will be the same and are considered in one row. The boundary street analysis is based on the designation of "General Urban Area" for O'Connor Street and Traditional Mainstreet for Somerset Street W. The MMLOS worksheets has been provided in Appendix I.

Table 16: Boundary Street MMLOS Analysis

Segment	Pedestrian LOS		Bicycle LOS		Trans	it LOS	Truck LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
O'Connor Street	С	С	Α	С	N/A	N/A	С	D
Somerset Street W	Α	В	Е	С	N/A	N/A	В	D

The boundary streets will meet MMLOS targets for all but the bicycle LOS on Somerset Street W due to mixed traffic conditions. To meet targets, Somerset Street W would require a curbside bike lane, however limited opportunity exists for improvements within the corridor given right of way constraints. Given the limits of the site



frontage, the performance of cycling facilities for the greater Somerset Street W corridor context should be investigated by the City to determine an appropriate treatment.

Crowding PLOS is not considered in the PLOS due to the excessively high-volume threshold. At the lowest threshold given, of 250 pedestrians per hour, the minimum effective sidewalk width required to achieve LOS A would be 3.0 metres, whereby nearly any sidewalk considered for installation in the City would not be able to meet this target.

11 Access Intersections Design

11.1 Location and Design of Access

The site will access the one-way southbound O'Connor Street via a right-in/right-out access at the northernmost extent of the site.

11.2 Intersection Control

Based upon the projected volumes, the site access will have stop-control on the minor approach. No further traffic control is necessary to address operational issues.

11.3 Access Intersection Design

11.3.1 2024 Future Total Access Intersection Operations

The 2024 future total intersection volumes are illustrated in Figure 14 and the access intersection operations are summarized below in Table 17. The level of service for unsignalized intersections is based on HCM 2010 delay. The synchro worksheets have been provided in Appendix J.



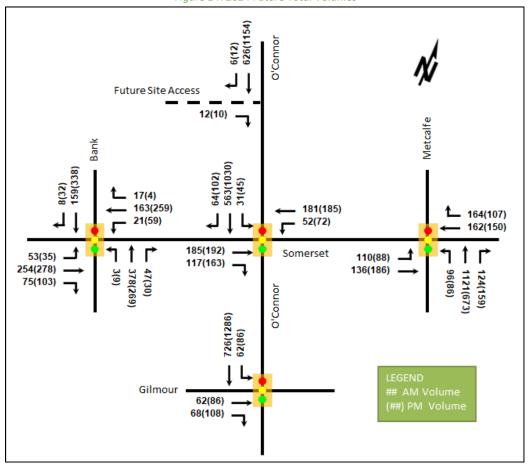


Figure 14: 2024 Future Total Volumes

Table 17: 2024 Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour				
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)	
Site Access at	EBR	В	0.02	10.4	0.8	В	0.02	13.1	0.8	
O'Connor Street	SBT/R	-	-	-	-	-	-	-	-	
Unsignalized	Overall	Α	-	0.2	-	Α	-	0.1	-	

Notes:

Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres

Peak Hour Factor = 1.00

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The access intersection on the 2024 future total horizons operates well. No capacity issues are noted.

11.3.2 2029 Future Total Access Intersection Operations

The 2029 future total intersection volumes are illustrated in Figure 14 and the access intersection operations are summarized below in Table 17. The level of service for unsignalized intersections is based on HCM 2010 delay. The synchro worksheets have been provided in Appendix K.



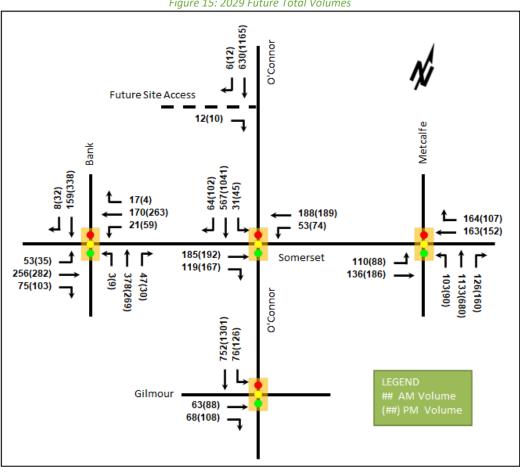


Figure 15: 2029 Future Total Volumes

Table 18: 2029 Future Total Access Intersection Operations

lutava attav	1	AM Peak Hour					PM Peak Hour				
Intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay 13.1 - 0.1	Q (95 th)		
Site Access at	EBR	В	0.02	10.4	0.8	В	0.02	13.1	0.8		
O'Connor Street	SBT/R	-	-	-	-	-	-	-	-		
Unsignalized	Overall	Α	-	0.2	-	Α	-	0.1	-		

Notes:

Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres

Peak Hour Factor = 1.00

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The access intersection on the 2029 future total horizons operates well. No capacity issues are noted.

11.3.3 Access Intersection MMLOS

As the access intersection will be unsignalized, no access intersection MMLOS analysis has been performed.

11.3.4 Recommended Design Elements

The exemption for the driveway from the adjacent property requires a 3.5-metre setback from the near edge of the sidewalk for any structure on the shared property line to maintain adequate sight lines between accesses and pedestrian facilities. For the length of this setback, it is recommended that any structure be either transparent or be no higher than 0.75 metres. No other access intersection design elements are proposed outside of the typical private approach considerations.



12 Transportation Demand Management

12.1 Context for TDM

The mode shares used within the TIA represent the unmodified district shares. The site further proposes a parking rate of 0.35 spaces per unit which should act as a constraint on the auto mode share beyond the level of auto use forecasted. Given the site context, supportive TDM measures should be provided and those aimed at increasing walk and bicycle modes should be of specific emphasis.

The subject site is within the Somerset Traditional Mainstreet Design Priority Area.

The total number of bedrooms is 187 across 125 studio and one-bedroom units and 31 two-bedroom. No age restrictions are noted.

12.2 Need and Opportunity

The foregoing analysis assumes a high level of walking, and similar levels of reliance on each the auto and transit modes. The proposed reduction in parking spaces reduces the risk of an increase in auto mode share above the area targets. Notwithstanding this constraint on additional vehicle trips, negligible impacts are anticipated from the potential failure to meet these targets.

12.3 TDM Program

The "suite of post occupancy TDM measures" has been summarized in the TDM checklists for the residential land uses. The checklist is provided in Appendix L. The key TDM measures recommended include:

- Display local area maps with walking/cycling access routes and key destinations at major entrances
- Display relevant transit schedules and route maps at entrances
- Provide a multimodal travel option information package to new residents
- Inclusion of a 1-month Presto card for first time new apartment rental, with a set time frame for this offer (e.g., 6-months) from the initial opening of the site
- Unbundle parking cost from purchase or rental costs

13 Transit

13.1 Route Capacity

In Section 5.1 the trip generation by mode was estimated, including an estimate of the number of transit trips that will be generated by the proposed development. Table 19 summarizes the transit trip generation.

Table 19: Trip Generation by Transit Mode

Travel	Residential Mode Share	AN	∕I Peak Peri	ak Period			
Mode	AM(PM)	In	Out	Total	In	Out	Total
Transit	28%(21%)	6	13	20	9	7	16

The proposed development is anticipated to generate an additional 20 AM peak hour transit trips and 16 PM peak hour transit trips. Of these trips, 13 outbound AM trips and 9 inbound PM trips are anticipated. From the trip distribution found in Section 5.3, the development is anticipated to generate less than five trips in each direction. As such and, given a minimum of 15-minute service in each direction, no service changes are anticipated as being required to accommodate site-generated transit trips.



13.2 Transit Priority

Examining the study area intersection delays, negligible impacts are noted on the transit movements at the study area intersections.

14 Network Intersection Design

14.1 Network Intersection Control

No change to the existing signalized control is recommended for the network intersections.

14.2 Network Intersection Design

14.2.1 2024 Future Total Network Intersection Operations

The 2024 future total network intersection operations are summarized below in Table 20. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets have been provided in Appendix J.

Table 20: 2024 Future Total Network Intersection Operations

Intersection	Lama	AM Peak Hour				PM Peak Hour					
intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay 27.0 21.0 14.7 16.1 12.0 12.9 17.1 44.1 18.7 19.6 24.3 19.9 18.6 13.7 15.7 24.8 5.8 8.1	Q (95 th)		
	EBL/T	В	0.61	27.4	60.8	Α	0.60	27.0	61.6		
	EBR	Α	0.23	20.9	17.6	Α	0.27	21.0	22.2		
Bank Street at	WBL	Α	0.10	8.8	m2.4	Α	0.25	14.7	m6.4		
Somerset Street W	WBT/R	Α	0.33	9.6	13.9	Α	0.47	16.1	m31.9		
Signalized	NBT/R	Α	0.50	13.9	58.9	Α	0.37	12.0	39.8		
	SBT/R	Α	0.19	10.0	21.2	Α	0.43	12.9	49.0		
	Overall	Α	0.54	16.6	-	Α	0.50	17.1	-		
0/6	EBT/R	Α	0.44	16.5	40.2	В	0.66	44.1	76.9		
O'Connor Street at	WBL/T	Α	0.37	24.2	m45.5	Α	0.57	18.7	30.0		
Somerset Street W Signalized	SB	Α	0.48	16.1	46.2	С	0.77	19.6	91.2		
Signanzea	Overall	Α	0.45	17.8	-	С	0.71	24.3	-		
Mataclfa Ctuant at	EBL/T	Α	0.59	18.3	42.2	Α	0.52	19.9	m42.0		
Metcalfe Street at	WBT/R	Α	0.57	22.2	58.8	Α	0.45	18.6	43.2		
Somerset Street W Signalized	NB	В	0.67	17.3	64.9	Α	0.49	13.7	38.9		
Signanzea	Overall	В	0.63	18.3	-	Α	0.50	15.7	-		
O'Connor Street at	EBT/R	Α	0.34	16.6	21.4	Α	0.54	24.8	36.2		
Gilmour Street	SBL/T	Α	0.37	3.3	14.4	В	0.64	5.8	19.9		
Signalized	Overall	Α	0.33	5.2	-	Α	0.60	8.1	-		

Notes: Saturation flow rate of 1800 veh/h/lane

Queue is measured in metres Peak Hour Factor = 1.00 Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The network intersections for the 2024 future total horizon continue to operate well, and similarly to the 2024 future background conditions. No new capacity issues are noted.

14.2.2 2029 Future Total Network Intersection Operations

The 2029 future total network intersection operations are summarized below in Table 20. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets have been provided in Appendix K.



Table 21: 2029 Future Total Network Intersection Operations

Intersection	Long		AM Pea	ak Hour		PM Peak Hour					
intersection	Lane	LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay 27.2 21.0 14.6 15.9 12.0 12.9 17.1 44.5 19.5 19.8 24.6 20.1 18.6 13.8 15.8 25.3 6.6 8.9	Q (95 th)		
	EBL/T	В	0.61	27.5	61.2	В	0.61	27.2	62.2		
	EBR	Α	0.23	20.9	17.6	Α	0.27	21.0	22.2		
Bank Street at	WBL	Α	0.10	8.7	m2.3	Α	0.26	14.6	m6.0		
Somerset Street W	WBT/R	Α	0.34	9.6	14.3	Α	0.47	15.9	m31.8		
Signalized	NBT/R	Α	0.50	13.9	58.9	Α	0.37	12.0	39.8		
	SBT/R	Α	0.19	10.0	21.2	Α	0.43	12.9	49.0		
	Overall	Α	0.54	16.6	-	Α	0.50	17.1	-		
0/0	EBT/R	Α	0.45	16.6	40.5	В	0.67	44.5	77.7		
O'Connor Street at	WBL/T	Α	0.38	23.8	m46.2	Α	0.59	19.5	30.9		
Somerset Street W	SB	Α	0.48	16.1	46.6	С	0.77	19.8	92.6		
Signalized	Overall	Α	0.45	17.8	-	С	0.72	Delay 27.2 21.0 14.6 15.9 12.0 12.9 17.1 44.5 19.5 19.8 24.6 20.1 18.6 13.8 15.8 25.3 6.6	-		
Matalfa Ctuant at	EBL/T	Α	0.59	18.4	42.1	Α	0.52	20.1	m41.7		
Metcalfe Street at Somerset Street W	WBT/R	Α	0.57	22.2	59.0	Α	0.45	18.6	43.5		
Signalized	NB	В	0.68	17.6	66.6	Α	0.50	13.8	39.5		
Signanzea	Overall	В	0.64	18.5	-	Α	0.50	15.8	-		
O'Connor Street at	EBT/R	Α	0.35	16.9	21.6	Α	0.55	25.3	36.8		
Gilmour Street	SBL/T	Α	0.39	3.5	15.8	В	0.67	6.6	32.3		
Signalized	Overall	Α	0.35	5.3	-	В	0.62	8.9	-		

Notes: Saturation flow rate of 1800 veh/h/lane Queue is measured in metres

Peak Hour Factor = 1.00

Delay is measured in seconds

m = metered queue

= volume for the 95th %ile cycle exceeds capacity

The network intersections for the 2029 future total horizon continue to operate well, and similarly to the 2029 future background conditions. No new capacity issues are noted.

14.2.3 Network Intersection MMLOS

Table 22 summarizes the MMLOS analysis for the study area network intersections below. The existing and future conditions for both intersections will be the same and are considered in one row. The intersection analysis is based on the designation of "Traditional Mainstreet" for the Somerset Street W at Bank Street and Somerset Street W at O'Connor Street intersections, and for "General Urban Area" for the Gilmour Street at O'Connor Street intersection and for the policy area of "Within 300m of a school" for the Somerset Street W at Metcalfe Street intersection. The MMLOS worksheets has been provided in Appendix I.

Table 22: Study Area Intersection MMLOS Analysis

Intersection	Pedesti	rian LOS	Bicyc	e LOS Transit		it LOS Truck		k LOS Aut		o LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target	
Somerset Street W at O'Connor Street	В	В	С	С	N/A	N/A	E	D	С	D	
Somerset Street W at Bank Street	С	В	F	С	D	D	F	D	Α	D	
Somerset Street W at Metcalfe Street	В	Α	E	С	N/A	N/A	D	D	В	E	
Gilmour Street at O'Connor Street	В	С	В	С	N/A	N/A	N/A	N/A	Α	D	

The MMLOS targets will not be met for the pedestrian and bicycle LOS at the intersection of Somerset Street W and Bank Street and the intersection of Somerset Street W and Metcalfe Street. Pedestrian delay LOS is not considered in the PLOS calculation.



To meet pedestrian LOS targets at both intersections, the roadways would need to be narrowed to no more than two lanes on all approaches.

To meet bicycle LOS targets, the eastbound approach at the intersection of Somerset Street W and Bank Street would require a configuration where cyclists do not have to shift left of the right-turning lane, such as a protected crossing, and the northbound approach at the intersection of Somerset Street and Metcalfe Street would require a two-stage left turn.

Truck LOS targets are not being met at the intersections of Somerset Street W at O'Connor Street and Somerset Street W and Bank Street. To meet targets, the southbound approach at the intersection of Somerset Street W and O'Connor Street and the eastbound and westbound approaches at the intersection of Somerset Street W and Bank Street would require either an additional receiving lane, or a greater than 15-metre effective turning radius. Given the trade-offs with pedestrian LOS and the downtown context, it is recommended that these mitigations not be implemented.

All other MMLOS targets are being met for the study area network intersections.

Pedestrian delay LOS is not considered in the PLOS calculation as it is not a suitable metric for the assessment of pedestrian LOS as formulated. This exclusion is consistent with City direction since 2015, and no alternative methodology has been provided for its assessment.

14.2.4 Recommended Design Elements

No study area intersection design elements are proposed as part of this study.

15 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

Proposed Site and Screening

- The proposed site includes 156 apartment dwelling units and 2,120 ft² of ground floor commercial space
- The site access is proposed as being right-in/right-out onto O'Connor Street
- The development is proposed to be completed as a single phase by 2024
- The Trip Generation, Location and Safety triggers were met for the TIA Screening
- This report accompanies a site plan application

Existing Conditions

- O'Connor Street, Somerset Street, Bank Street, and Metcalfe Street are arterial roads in the study area
- Sidewalks are provided on both sides of the study area roadways
- Two-way curb-separated bike lanes are on O'Connor Street, curb-separated bike lanes are on Laurier Avenue W, O'Connor Street, Metcalfe Street, Laurier Avenue W, and Somerset Street W are spine routes, and Bank Street and Elgin Street are local routes
- The high volumes on roadways produced elevated number of collisions at the intersection of Somerset Street W at O'Connor Street, however no collision type is overly represented
- Study area intersections operate well, with some queuing noted at the intersection of O'Connor Street and Somerset Street during the PM peak hour



Development Generated Travel Demand

- The proposed development is forecasted produce 68 two-way people trips during the AM peak hour and 77 two-way people trips during the PM peak hour
- Of the forecasted people trips, 18 two-way trips will be vehicle trips during the AM peak hour and 21 twoway trips will be vehicle trips during the PM peak hour
- Of the forecasted trips, 10% are anticipated to travel north, 35% south, 25% east, and 30% to travel west

Background Conditions

- The background developments were explicitly included in the background conditions, where from examination of the TRANS model, no growth was anticipated between existing conditions and 2031
- The study area intersections at both future horizons will operate similarly to the existing conditions

Development Design

- Auto parking will be underground across three parking levels, bicycle parking will be located internal to the building in both a secure storage room accessing the drive aisle and in the three parking levels
- Hard surface connections will be made along both site frontages to surrounding pedestrian facilities, transit stops for area routes are typically within 400 metres walk or just beyond this distance
- A setback of 3.5 metres from the near edge of the sidewalk is proposed for the wall separating the
 properties, where through the setback the structure be either transparent or the maximum height of the
 wall be 0.75 metres
- Garbage collection will either be via the site driveway or on O'Connor Street and emergency services are assumed to be able to access the site via its two arterial road frontages

Parking

- The site proposes bicycle parking at a rate of 0.71 spaces per unit for 110 spaces, tenant parking at a rate of 0.35 spaces after the first twelve units for a total of 51 spaces, and visitor parking at a rate of 0.1 spaces after the first twelve units for a total of 14 spaces
- Minimum visitor and bicycle parking provision rates are being met, and the site is proposing tenant parking at a deficit of 14 spaces from the minimum for which the development requires an exemption
- The proposed parking rate is supported by the forecasted trips and estimated site auto travel
- Residual capacity is noted in the surrounding area for private, paid, off-street, and on-street parking supply and can accommodate any minimal potential spillover parking from the site

Boundary Street Design

- The boundary streets will not meet bicycle MMLOS targets on Somerset Street W due to the mixed traffic conditions
- Limited opportunity for BLOS improvement exists due to corridor constraints, and the City should investigate treatment options for the greater corridor context



Access Intersections Design

- The site access is proposed as being right-in/right-out onto O'Connor Street and as being stop-controlled on the minor approach with O'Connor Street operating as a free flow corridor
- The access intersection operations perform well at both horizons
- The site is seeking an exemption for its access that does not provide minimum setbacks from the property line
- A 3.5-metre setback from the near edge of the sidewalk for any visual obstructions along the shared property line is recommended as part of access intersection design

TDM

- The site proposes providing parking at a rate commensurate with primary auto trip generation, and therefore, transit and active mode shares are likely to be achieved
- Supportive TDM measures to be included within the proposed development should include:
 - Display local area maps with walking/cycling access routes and key destinations at major entrances
 - Display relevant transit schedules and route maps at entrances
 - o Provide a multimodal travel option information package to new residents
 - o Inclusion of a 1-month Presto card for first time new townhome purchase and apartment rental, with a set time frame for this offer (e.g., 6-months) from the initial opening of the site
 - Unbundle parking cost from purchase or rental costs

Transit

- The site is anticipated to generate 13 outbound AM transit trips and 9 inbound PM transit trips
- Average site-generated ridership increases would translate to approximately one additional rider per bus
 per route in the area, thus no change is transit service is anticipated as being required
- Examining the study area intersection delays, negligible impacts are noted on the transit movements at the study area intersections

Network Intersection Design

- Generally, the network intersections will operate well at the future total horizons, where no capacity issues are noted
- The MMLOS targets will not be met for the pedestrian LOS and bicycle LOS at the intersections of Somerset Street W at Bank Street and Somerset Street W at Metcalfe Street intersections and for the truck LOS at the intersections of Somerset Street W at O'Connor Street and Somerset Street W and Bank Street
- Pedestrian LOS cannot be met with the existing approach geometries of the intersections
- The Somerset Street W at Bank Street intersection would require an approach where cyclists do not need
 to shift left of the right-turn lane, and the Somerset Street W at Metcalfe intersection would require a
 two-stage left turn
- Any mitigation aimed at meeting truck LOS would negatively impact pedestrian LOS, and given the downtown context, such treatments are not recommended



16 Conclusion

It is recommended that, from a transportation perspective, the proposed development applications proceed.

Prepared By:

John Kingsley, EIT

Transportation Engineering Intern

Reviewed By:

OFESSIONAL

A. J. HARTE
100149314

June 22, 2022

Andrew Harte, P.Eng.

Senior Transportation Engineer



Appendix A

TIA Screening Form and PM Certification Form





City of Ottawa 2017 TIA Guidelines Step 1 - Screening Form Date: 15-Sep-20
Project Number: 2020-27
Project Reference: 311 Somerset

1.1 Description of Proposed Development	
Municipal Address	311 Somerset Street W, 234-236 O'Connor Street
Description of Location	Lots 5 and 41, Registered Plan 12281
Land Use Classification	TM[2185], R5B[482] F(3.0)
Development Size	16-Storey, 139-Unit Aartment Building
Accesses	One right-in-right-out access onto O'Connor St
Phase of Development	One phase
Buildout Year	2024
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Townhomes or apartments
Development Size	139 Units
Trip Generation Trigger	Yes

1.3 Location Triggers		
Does the development propose a new driveway to a boundary street that is		
designated as part of the City's Transit Priority, Rapid Transit or Spine	Yes	
Bicycle Networks?		O'Connor Street Spine Route
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?	Yes	Somerset Traditional Mainstreet, Downtown Ottawa Urban Design Strategy
Location Trigger	Yes	

1.4. Safety Triggers		
Are posted speed limits on a boundary street 80 km/hr or greater?	No	
Are there any horizontal/vertical curvatures on a boundary street limits	No	
sight lines at a proposed driveway?	INO	
Is the proposed driveway within the area of influence of an adjacent traffic		
signal or roundabout (i.e. within 300 m of intersection in rural conditions,	Yes	Intersection of Somerset Street
or within 150 m of intersection in urban/ suburban conditions)?		
		W & O'Connor Street
Is the proposed driveway within auxiliary lanes of an intersection?	No	
Does the proposed driveway make use of an existing median break that	No	
serves an existing site?	No	
Is there is a documented history of traffic operations or safety concerns on	Voc	
the boundary streets within 500 m of the development?	Yes	
Does the development include a drive-thru facility?	No	
Safety Trigger	Yes	·



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$] is either transportation engineering $\sqrt{\text{or}}$ or transportation planning \square .
- License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at Ottawa (City)	this 20 day of September	, 2018
Name:	Andrew Harte (Places Print)	
Professional Title:	(Please Print) Professional Engineer	
Signatura	of Individed Market that she shows four criteria	
Signature	of Individual certifier that s/he meets the above four criteria	

Office Contact Information (Please Print)
Address: 13 Markham Avenue
City / Postal Code: Ottawa / K2G 3Z1
Telephone / Extension: (613) 697-3797
E-Mail Address: Andrew.Harte@CGHTransportation.com



Appendix B

Turning Movement Counts





Turning Movement Count - Full Study Peak Hour Diagram

BANK ST @ SOMERSET ST

Survey Date: Wednesday, August 05, 2015

Start Time: 07:00

WO No: Device:

34727 Jamar Technologies, Inc

		Heavy Vehicles Cars	8 1 7	160 152 11 141	0 0 0	992 0 0	432 432 33 399	126	⊘	w	> E
SOMEF 114 494 494	0 52 253 75	3 1111 0 0 5 47 4 249 5 70	1 1 1		I Periodak Hour	:	1 1 1	101 2	22 00 00 00 00 00 00 00 00 00 00 00 00 0	11 103 144 0	128 428 300
294	42	84	225 16 241	0 0 0 660	. [•	341 28 369	46 1 47		eavy ehicles	Total	l

Comments

Ottawa

Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

BANK ST @ SOMERSET ST

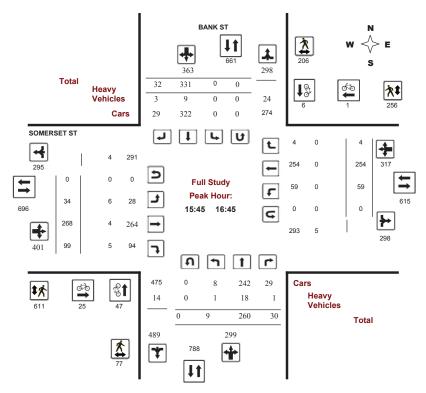
Survey Date: Wednesday, August 05, 2015

Start Time: 07:00

WO No: 34727

Device: Jamar Technolog

Technologies, Inc





Turning Movement Count - Full Study Peak Hour Diagram

BANK ST @ SOMERSET ST

Survey Date: Wednesday, August 05, 2015

Start Time: 07:00

WO No: Device:

34727 Jamar

Technologies, Inc

													IIIC
	Total	Heavy Vehicle: Ca		26 4 22	185 157 10 147	L	0 0 0	315 24 291	14	1	W	s -	> E
152 485	0 48 186 99	6 0 4 7 5	146 0 44 179 94	ן ן ן		Periodak Hour 0 12:	r:	1 1 1	28 96 14 0 204	2 2 0 0		30 98 14 0	142 357
1 / 5	11			255 15 270	0 0 0 562	, [219 18 237 292	23 4 27	Ca	rs Heavy Vehic	les	Tota	

Comments

Ottawa

Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

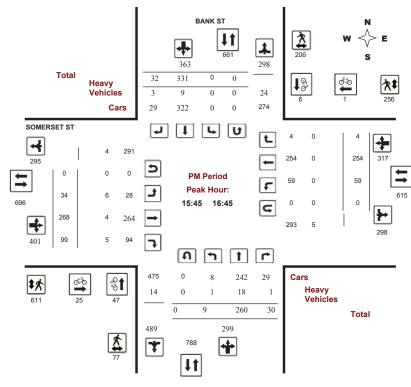
BANK ST @ SOMERSET ST

Survey Date: Wednesday, August 05, 2015

Start Time: 07:00

WO No: 34727 Device:

Jamar Technologies, Inc



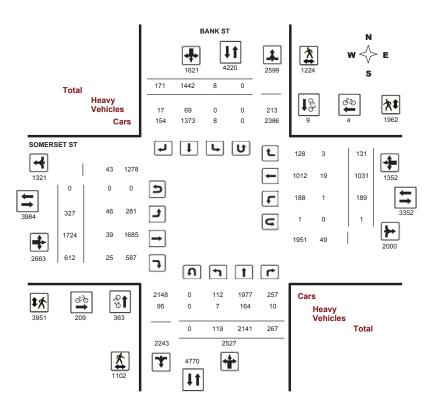


Transportation Services - Traffic Services Turning Movement Count - Full Study Diagram

BANK ST @ SOMERSET ST

Survey Date: Wednesday, August 05, 2015

WO#: Device: 34727 Jamar Technologies, Inc



Comments

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 34727

Turning Movement Count - Full Study Summary Report

BANK ST @ SOMERSET ST

 Survey Date:
 Wednesday, August 05, 2015
 Total Observed U-Turns
 AADT Factor

 Northbound:
 0
 Southbound:
 0
 .90

 Eastbound:
 0
 Westbound:
 1

								F	ull St	udy									
				BANK	ST							SC	OMERS	SET S	Т				
_	- 1	Northb	ound		,	Southb	ound		_		Eastb	ound			Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	3	251	36	290	0	103	13	116	406	46	160	39	245	14	58	11	83	328	734
08:00 09:00	3	369	47	419	0	152	8	160	579	52	253	75	380	14	103	11	128	508	1087
09:00 10:00	25	266	44	335	1	133	23	157	492	46	199	60	305	15	100	9	124	429	921
11:30 12:30	28	237	27	292	2	157	26	185	477	48	186	99	333	14	98	30	142	475	952
12:30 13:30	24	240	23	287	0	147	22	169	456	30	196	83	309	8	114	38	160	469	925
15:00 16:00	14	253	24	291	3	247	19	269	560	30	228	83	341	41	191	8	240	581	1141
16:00 17:00	7	260	30	297	0	287	29	316	613	38	253	96	387	39	222	10	271	658	1271
17:00 18:00	15	265	36	316	2	216	31	249	565	37	249	77	363	44	145	14	203	566	1131
Sub Total	119	2141	267	2527	8	1442	171	1621	4148	327	1724	612	2663	189	1031	131	1351	4014	8162
U Turns				0				0	0				0				1	1	1
Total	119	2141	267	2527	8	1442	171	1621	4148	327	1724	612	2663	189	1031	131	1352	4015	8163
EQ 12Hr	165	2976	371	3513	11	2004	238	2253	5766	455	2396	851	3702	263	1433	182	1879	5581	11347
Note: These	values a	ire calcu	lated b	y multiply	ing the	totals b	y the a	ppropriat	e expans	ion fac	tor.		1	.39					
AVG 12Hr	149	2678	334	3161	10	1804	214	2028	5189	409	2157	766	3331	236	1290	164	1691	5022	10211
Note: These	volumes	are cal	culated	by multip	olying t	he Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			90					
AVG 24Hr	195	3509	438	4141	13	2363	280	2657	6798	536	2825	1003	4364	310	1690	215	2216	6580	13378
Note: These	volumes	are cal	culated	by multip	olying t	he Avera	ige Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown



Survey Date:

Transportation Services - Traffic Services w.o.

Turning Movement Count - 15 Minute Summary Report

BANK ST @ SOMERSET ST

Wednesday, August 05, 2015 Total Observed U-Turns Northbound: 0 Southbound: 0

Note 10 Note N											astbour)		estboun	. 0					
					В	ANK S	т														
			١	Northbo	und		Sc	outhboun	ıd			Eas	stbound			Wes	stbound				
07:00 07:15	Time	Dariad	ıт	ST	RΤ		ΙT	ST	рT			ΙT	ST	RΤ		ΙT	ST	рT			Grand
07:15 07:30 07:45 2 72 99 83 0 28 2 30 113 17 52 18 11 30 11 52 6 16 5 27 79 173 07:30 07:45 2 72 99 83 0 28 2 30 113 17 52 12 81 3 17 1 1 21 102 215 07:45 08:00 0 75 10 85 0 24 3 27 112 13 48 6 6 67 1 1 15 1 17 84 186 08:00 08:15 0 91 15 106 0 44 1 45 151 10 65 25 100 65 23 2 30 130 281 186 186 187 187 187 187 187 187 187 187 187 187																					
07.45 07.45 07.45 2 72 9 83 0 28 2 30 113 17 52 12 81 3 17 1 21 102 215 07.45 08.00 0 75 10 85 0 24 3 27 112 13 48 6 6 67 1 1 15 1 17 84 196 08.00 08.15 0 91 15 10 60 0 44 1 45 151 10 65 25 100 5 23 2 3 0 30 30 30 08.15 0 91 15 10 60 0 44 1 4 45 151 10 65 25 100 5 23 2 3 0 30 30 30 08.15 0 91 15 10 80 0 44 1 4 45 151 10 65 25 100 5 23 2 3 0 30 30 30 08.15 0 93 0 1 95 11 108 0 44 1 5 46 154 11 59 24 94 5 14 1 20 114 22 88 08.03 08.05 7 7 72 13 92 0 26 1 27 119 13 60 10 10 11 11 1 1 13 83 20 09.05 09.15 7 72 13 92 0 26 1 27 119 13 63 14 70 1 1 11 1 1 1 3 83 20 09.05 09.15 7 7 72 13 92 0 26 1 27 119 13 53 13 14 70 1 1 11 1 1 13 83 20 09.05 09.05 7 8 6 1 10 81 0 40 13 53 134 13 50 15 78 4 18 1 2 3 101 235 09.45 10.00 8 6 8 13 86 1 4 91 0 57 10 67 158 17 48 22 87 5 23 7 35 122 280 11.45 12.00 9 41 7 57 7 2 44 8 54 111 10 44 23 77 1 1 15 2 18 95 26 12.00 12.15 6 57 11 74 0 9 9 2 11 18 85 9 52 2 28 86 5 3 3 43 129 112 12.15 12.30 7 68 5 9 81 0 0 14 1 22 3 104 92 8 44 12 1 73 0 3 3 12 8 12 8 12 12.30 12.45 6 84 76 0 5 9 81 0 1 14 12 2 3 104 92 8 44 12 1 73 0 3 3 1 2 8 12 12.30 12.45 6 87 7 67 80 81 0 81 1 12 2 3 104 92 8 44 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																					
0745 08:00 0 0 75 10 85 0 24 3 27 112 13 48 6 667 1 15 1 17 84 196 08:00 08:15 0 91 15 106 0 44 1 25 151 10 65 25 100 5 23 2 30 130 281 08:15 08:30 0 88 9 97 0 21 1 22 119 18 60 13 91 1 38 1 40 131 250 08:30 08:45 2 95 11 108 0 46 1 47 155 13 69 13 95 3 28 7 38 133 288 08:45 09:00 1 95 12 108 0 41 5 46 15 41 5 46 15 41 5 5 46 10 30 14 1 20 114 22 119 18 60 13 91 1 1 38 1 1 40 131 250 09:00 09:15 7 72 13 92 0 26 1 27 119 13 43 14 70 1 11 1 1 1 13 83 202 09:15 09:30 3 65 8 76 0 24 1 25 101 9 53 19 81 4 4 22 3 49 130 231 09:30 09:45 7 64 10 81 0 40 13 53 134 13 50 15 78 4 18 1 1 23 101 235 09:45 10:00 8 65 13 86 1 43 8 52 138 11 53 12 76 6 2 29 4 39 115 253 11:30 11:45 6 81 4 91 0 57 1 0 67 158 17 48 22 87 5 23 7 38 122 200 11:45 12:00 12:15 6 57 11 74 0 9 2 11 85 9 5 22 25 86 5 33 5 3 43 122 200 11:45 13:00 7 88 5 70 0 47 6 5 3 123 12 42 29 83 3 25 5 18 46 129 251 12:45 13:00 7 88 6 70 0 47 6 53 123 10 1 23 10 1 23 10 1 23 10 1 23 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1																					
08.00 08.15 0 0 91 1 15 106 0 44 1 1 45 151 10 65 25 100 5 23 2 30 130 281 08.15 08.30 0 88 9 97 0 2 11 1 22 119 18 60 13 91 1 38 12 1 40 131 250 08.30 08.45 2 95 11 108 0 46 1 47 155 13 69 13 95 3 28 7 38 133 288 08.45 09.00 1 95 7 72 13 92 0 26 1 27 119 13 43 14 70 1 11 1 1 1 13 83 202 09.15 7 72 13 92 0 26 1 27 119 13 43 14 70 1 11 1 1 1 1 38 83 202 09.15 09.30 3 65 8 76 0 24 1 25 119 13 53 19 81 4 4 2 3 49 130 231 09.30 09.45 7 6 4 10 81 0 40 131 55 13 14 15 5 14 1 1 1 1 1 1 1 1 1 1 1 1 1 1																					
08:15 08:30 0																					
08:30 08:45 2 9 95 11 108 0 46 1 47 155 13 69 13 95 3 28 7 38 133 288 08:45 09:00 1 95 12 108 0 41 5 46 154 11 59 24 94 5 14 1 20 114 268 09:00 09:15 7 72 13 92 0 26 1 27 119 13 43 14 70 1 1 11 1 1 13 83 202 09:15 09:30 3 65 8 76 0 24 1 25 101 9 53 19 81 4 12 3 49 130 231 09:30 09:45 7 64 10 81 0 40 13 53 134 13 50 15 78 4 18 1 23 101 235 09:45 10:00 8 65 13 86 1 43 8 52 138 11 53 12 76 6 29 4 39 115 233 11:30 11:45 6 81 4 91 0 57 10 67 158 17 48 22 87 5 23 7 35 122 280 11:45 12:00 9 4 1 7 5 7 1 7 4 0 9 2 11 85 9 3 12 42 2 8 7 5 2 3 4 3 43 129 214 11:45 12:00 7 5 58 5 70 0 47 6 53 123 12 42 2 9 83 3 25 18 46 129 24 12:20 12:45 6 4 40 5 5 1 0 3 4 7 6 53 123 12 42 2 9 83 3 2 5 18 46 129 24 12:45 13:00 7 65 9 81 0 1 1 12 2 3 104 7 51 19 77 4 4 41 11 56 133 237 13:30 13:15 14 6 8 4 76 0 29 0 29 0 29 103 5 44 7 7 56 4 4 40 4 4 48 104 207 15:15 15:30 6 6 6 7 7 8 4 0 9 9 5 7 9 0 88 7 9 9 1 1 1 12 23 15 14 14 14 14 14 14 14 14 14 14 14 14 14																					
08:45 09:00 1 1 95 12 108 0 41 5 46 154 11 59 24 94 5 14 1 20 114 268 09:00 09:15 7 7 72 13 92 0 26 1 27 119 13 43 14 70 1 111 1 1 13 83 202 09:15 09:30 3 65 8 76 0 24 1 25 101 9 53 19 81 4 4 22 3 49 130 231 09:30 09:45 7 64 10 81 0 40 13 53 134 13 50 15 78 4 18 1 23 101 235 09:45 10:00 8 65 13 86 1 43 8 52 138 11 53 12 76 6 6 29 4 39 115 253 11:30 11:45 6 81 4 91 0 57 10 67 158 17 48 22 87 5 23 7 35 122 280 11:45 6 81 4 91 0 57 2 44 8 54 111 10 44 23 77 1 1 15 2 18 95 206 11:45 6 5 7 11 74 0 9 2 11 85 9 52 25 86 5 5 35 3 43 129 214 12:15 12:30 12:45 6 40 5 5 11 74 0 9 2 11 185 92 28 84 13 3 25 12 13 13 13 13 13 13 13 13 13 13 13 13 13																					
09:00 09:15 7 7 72 13 92 0 26 1 27 119 13 43 14 70 1 111 1 1 13 83 202 09:15 09:30 3 65 8 76 0 24 1 25 101 9 53 19 81 4 4 22 3 49 130 231 09:30 09:45 7 64 10 81 0 40 13 53 134 13 50 15 78 4 18 1 23 101 235 09:45 10:00 8 65 13 86 1 43 8 8 52 138 11 53 12 76 6 6 29 4 39 115 253 11:30 11:45 6 81 4 91 0 57 10 67 158 17 48 22 87 5 23 7 35 122 280 11:45 12:00 9 41 7 57 2 44 8 554 111 10 44 23 77 1 1 15 2 2 18 95 206 12:00 12:15 6 5 7 11 74 0 9 2 2 11 85 9 52 28 86 5 3 3 25 18 46 129 252 12:30 12:45 6 40 5 5 11 0 34 7 4 1 92 8 44 21 73 0 33 7 40 113 205 12:45 13:00 7 56 9 81 0 11 12 2 33 104 7 51 12 73 0 33 7 40 113 205 12:45 13:00 7 65 9 81 0 11 12 2 33 104 7 51 12 77 4 41 11 15 56 133 237 13:16 13:30 7 67 5 8 5 79 0 2 29 0 29 103 5 44 7 7 80 29 29 103 5 44 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8																					
09:15 09:30																					
09:45																					
0945 10:00 8 65 13 86 1 43 8 52 138 11 53 12 76 6 29 4 39 115 253 11:30 11:45 6 81 4 91 0 57 10 67 158 17 48 22 87 5 23 7 35 122 280 11:45 12:00 9 41 7 57 2 44 8 54 111 10 44 23 77 1 15 2 18 95 206 12:15 6 57 11 74 0 9 2 11 85 9 52 25 86 5 3 43 129 21 44 120 2 18 42 29 83 3 25 18 46 129 2 23 104 7 5																					
11:30																					
1145 12:00 9 41 7 57 2 44 8 54 111 10 44 23 77 1 155 2 18 95 206 12:00 12:15 6 57 11 74 0 9 2 11 85 9 52 25 86 5 35 35 3 43 129 214 12:15 12:30 7 58 5 70 0 47 6 53 123 12 42 29 83 3 25 18 46 129 252 12:30 12:45 6 40 5 51 0 34 7 41 92 8 44 21 73 0 33 7 40 113 205 12:45 13:00 7 65 9 81 0 11 12 23 104 7 51 19 77 4 41 11 56 133 237 13:00 13:15 4 68 4 76 0 50 1 51 51 127 6 52 22 80 1 21 79 3 19 12 34 113 246 15:00 15:15 2 68 4 74 0 29 0 29 103 5 44 7 56 4 40 40 4 48 104 207 15:15 15:30 6 67 7 80 1 48 7 56 136 5 64 23 92 7 38 0 45 137 273 15:30 15:45 2 55 3 60 2 82 7 91 151 11 54 11 54 30 95 7 37 4 48 143 294 15:45 16:00 4 63 10 77 0 88 5 5 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 17 6 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 17 1 79 174 363 16:30 16:45 2 73 8 83 0 75 0 44 2 4 46 132 47 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 71 0 54 8 62 148 10 81 25 3 20 85 11 17 0 9 28 113 244 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 6 21 15 34 110 284																					
12:00																					
12:15																					
12:30																					
12:45 13:00 7 65 9 81 0 11 12 23 104 7 51 19 77 4 41 11 56 133 237 13:00 13:15 4 68 4 76 0 50 1 51 127 6 52 22 80 1 21 8 30 110 237 13:15 13:30 7 67 5 79 0 52 2 54 133 9 49 21 79 3 19 12 34 113 246 15:00 15:15 2 68 4 74 0 29 0 29 103 5 44 7 56 4 40 4 48 104 207 15:15 15:30 6 67 7 80 1 48 7 56 136 5 64 23 92 7 38 0 45 137 273 15:30 15:45 2 55 3 60 2 82 7 91 151 11 54 30 95 7 37 4 48 143 294 15:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 311 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:46 17:00 2 63 10 75 0 44 2 4 46 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 71 0 54 6 60 131 154 11 44 21 76 8 21 5 34 110 264																					
13:00 13:15																					
13:15 13:30 7 67 5 79 0 52 2 54 133 9 49 21 79 3 19 12 34 113 246 15:00 15:15 2 68 4 74 0 29 0 29 103 5 44 7 7 56 4 40 4 48 104 207 15:15 15:30 6 67 7 80 1 48 7 56 136 5 64 23 92 7 38 0 45 137 273 15:30 15:45 2 55 3 60 2 82 7 91 151 11 54 30 95 7 37 4 48 143 294 15:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 258 17:00 17:15 1 76 9 86 0 42 4 46 132 47 71 16 91 3 60 7 70 161 293 17:15 17:30 17:45 7 58 6 7 7 7 7 85 6 13 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 8 6 6 71 0 54 6 6 13 15 14 14 21 76 8 21 75 34 110 264 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264																					
15:00 15:15 2 68 4 74 0 29 0 29 103 5 44 7 56 4 40 4 48 104 207 15:15 15:30 6 6 7 7 80 1 48 7 56 136 5 64 23 92 7 38 0 45 137 273 15:30 15:45 2 55 3 60 2 82 7 91 151 11 54 30 95 7 37 4 48 143 294 15:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 4 46 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 244 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264			7		5		0	52						21		3	19				
15:30 15:45 2 55 3 60 2 82 7 91 151 11 54 30 95 7 37 4 48 143 294 15:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 46 121 13 51 20 84 3 44 6 53 137 258 17:00 17:15 1 76 9 86 0 42 4 46 132 47 1 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 154 11 44 21 76 8 21 5 34 110 264 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264	15:00	15:15	2	68		74	0	29		29	103	5		7	56	4	40	4	48	104	207
16:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 46 121 13 51 20 84 3 44 6 53 137 258 17:00 17:15 1 76 9 86 0 42 4 46 132 471 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 154 11 44 21 76 8 21 5 34 110 264	15:15	15:30	6	67	7	80	1	48	7	56	136	5	64	23	92	7	38	0	45	137	273
16:45 16:00 4 63 10 77 0 88 5 93 170 9 66 23 98 23 76 0 99 197 367 16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 46 121 13 51 20 84 3 44 6 53 137 258 17:00 17:15 1 76 9 86 0 42 4 46 132 471 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 154 11 44 21 76 8 21 5 34 110 264																					
16:00 16:15 2 48 5 55 0 70 10 80 135 7 72 23 102 16 58 2 76 178 313 16:15 16:30 1 76 7 84 0 95 10 105 189 10 64 21 95 7 71 1 79 174 363 16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:40 7:70 7 7 4 4 4 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15	15:45	16:00	4	63	10	77	0	88	5	93	170	9	66	23	98	23	76	0	99	197	367
16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 46 121 13 51 20 84 3 44 6 53 137 258 17:00 17:15 1 76 9 86 0 42 4 46 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 244 17:45		16:15	2	48	5		0	70	10			7		23		16	58	2			
16:30 16:45 2 73 8 83 0 78 7 85 168 8 66 32 106 13 49 1 63 169 337 16:45 17:00 2 63 10 75 0 44 2 46 121 13 51 20 84 3 44 6 53 137 258 17:00 17:15 1 76 9 86 0 42 4 46 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 244 17:45	16:15	16:30	1	76	7	84	0	95	10	105	189	10	64	21	95	7	71	1	79	174	363
17:00 17:15 1 76 9 86 0 42 4 46 132 4 71 16 91 3 60 7 70 161 293 17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 244 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264	16:30		2	73	8	83	0	78	7	85	168	8	66	32	106	13	49	1	63	169	337
17:15 17:30 2 69 15 86 0 54 8 62 148 10 81 20 111 22 47 2 71 182 330 17:30 17:45 7 58 6 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 24 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264	16:45	17:00	2	63	10	75	0	44	2	46	121	13	51	20	84	3	44	6	53	137	258
17:30 17:45 7 58 6 71 0 54 6 60 131 12 53 20 85 11 17 0 28 113 244 17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264	17:00	17:15	1	76	9	86	0	42	4	46	132	4	71	16	91	3	60	7	70	161	293
17:45 18:00 5 62 6 73 2 66 13 81 154 11 44 21 76 8 21 5 34 110 264	17:15	17:30	2	69	15	86	0	54	8	62	148	10	81	20	111	22	47	2	71	182	330
	17:30	17:45	7	58	6	71	0	54	6	60	131	12	53	20	85	11	17	0	28	113	244
TOTAL: 119 2141 267 2527 8 1442 171 1621 4148 327 1724 612 2663 189 1031 131 1352 4015 816	17:45	18:00	5	62	6	73	2	66	13	81	154	11	44	21	76	8	21	5	34	110	264
	TOTAL	L:	119	2141	267	2527	8	1442	171	1621	4148	327	1724	612	2663	189	1031	131	13	52 4015	8163

Note: U-Turns are included in Totals. Comment:

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services

Turning Movement Count - Cyclist Volume Report

Work Order 34727

BANK ST @ SOMERSET ST

Count Date: Wednesday, August 05, 2015

Start Time: 07:00

_		BANK ST			SOMERSET S	Г	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	42	1	43	25	0	25	68
08:00 09:00	84	1	85	42	0	42	127
09:00 10:00	69	0	69	34	2	36	105
11:30 12:30	20	0	20	10	0	10	30
12:30 13:30	42	0	42	17	0	17	59
15:00 16:00	34	3	37	18	0	18	55
16:00 17:00	48	4	52	23	1	24	76
17:00 18:00	24	0	24	40	1	41	65
Total	363	9	372	209	4	213	585

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.



W.O. 34727

Turning Movement Count - Heavy Vehicle Report

BANK ST @ SOMERSET ST

Survey Date: Wednesday, August 05, 2015

			1	BAN	(ST							SC	MER	SET S	Т					
		Northb	ound			Southb	oound	_			Eastb	ound		1	Westbo	ound	_			
Time	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Gran Tota
7:00	08:00	3	23	0	26	0	13	3	16	42	7	6	1	14	0	2	1	3	17	59
8:00	09:00	0	28	1	29	0	11	1	12	41	5	4	5	14	0	2	0	2	16	57
9:00	10:00	1	22	3	26	0	3	2	5	31	7	6	5	18	1	2	0	3	21	52
1:30	12:30	0	18	4	22	0	10	4	14	36	4	7	5	16	0	2	2	4	20	56
2:30	13:30	2	21	1	24	0	6	2	8	32	5	5	3	13	0	7	0	7	20	52
5:00	16:00	0	17	1	18	0	11	1	12	30	6	5	2	13	0	2	0	2	15	45
6:00	17:00	1	21	0	22	0	9	4	13	35	6	3	4	13	0	1	0	1	14	49
7:00	18:00	0	14	0	14	0	6	0	6	20	6	3	0	9	0	1	0	1	10	30
Sub	Total	7	164	10	181	0	69	17	86	267	46	39	25	110	1	19	3	23	133	400
-Turr	ıs (Heav	y Vel	nicles)		0				0	0				0				0	0	0
То	tal	7	164	10	0	0	69	17	86	267	46	39	25	110	1	19	3	23	133	400

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 34727

Turning Movement Count - Pedestrian Volume Report

Count Dat	e: Wednesday,	August 05, 2015				Start Time:	07:00
Γime Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
7:00 07:15	16	21	37	25	22	47	84
07:15 07:30	15	17	32	27	46	73	105
07:30 07:45	17	10	27	38	41	79	106
07:45 08:00	27	21	48	45	50	95	143
07:00 08:00	75	69	144	135	159	294	438
08:00 08:15	22	30	52	55	56	111	163
08:15 08:30	36	30	66	78	50	128	194
08:30 08:45	29	32	61	73	41	114	175
08:45 09:00	57	34	91	88	48	136	227
08:00 09:00	144	126	270	294	195	489	759
09:00 09:15	32	29	61	79	42	121	182
09:15 09:30	33	22	55	61	23	84	139
09:30 09:45	31	50	81	78	38	116	197
09:45 10:00	33	43	76	66	35	101	177
09:00 10:00	129	144	273	284	138	422	695
11:30 11:45	24	42	66	108	80	188	254
11:45 12:00	42	50	92	159	32	191	283
12:00 12:15	61	19	80	194	108	302	382
12:15 12:30	61	30	91	213	82	295	386
11:30 12:30	188	141	329	674	302	976	1305
12:30 12:45	70	37	107	209	58	267	374
12:45 13:00	58	37	95	202	61	263	358
13:00 13:15	58	34	92	182	115	297	389
13:15 13:30	61	35	96	187	96	283	379
12:30 13:30	247	143	390	780	330	1110	1500
15:00 15:15	51	17	68	83	50	133	201
15:15 15:30	12	22	34	118	45	163	197
15:30 15:45	28	52	80	129	44	173	253
15:45 16:00	21	48	69	133	54	187	256
15:00 16:00	112	139	251	463	193	656	907
16:00 16:15	12	40	52	146	101	247	299
16:15 16:30	19	63	82	169	44	213	295
16:30 16:45	25	55	80	163	57	220	300
16:45 17:00	11	65	76	164	78	242	318
16:00 17:00	67	223	290	642	280	922	1212
7:00 17:15	31	83	114	179	88	267	381
17:15 17:30	39	72	111	186	86	272	383
17:30 17:45	34	44	78	160	93	253	331
17:45 18:00	36	40	76	154	98	252	328
17:00 18:00	140	239	379	679	365	1044	1423
Total	1102	1224	2326	3951	1962	5913	8239

Comment:



Work Order 34727

Turning Movement Count - 15 Min U-Turn Total Report

BANK ST @ SOMERSET ST

Cumran Def	147		25 2015	COMILITOR	101	
Survey Date:	vve	dnesday, August (
Time Pe	riod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	1	1
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Tota	1	0	0	0	1	1

2019-Jul-04 Page 1 of 1

Ottawa

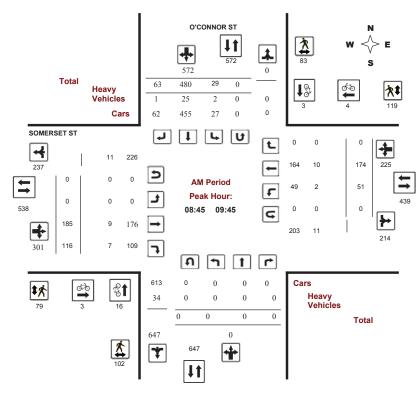
Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

O'CONNOR ST @ SOMERSET ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36787

 Start Time:
 07:00
 Device:
 Miovision



Comments

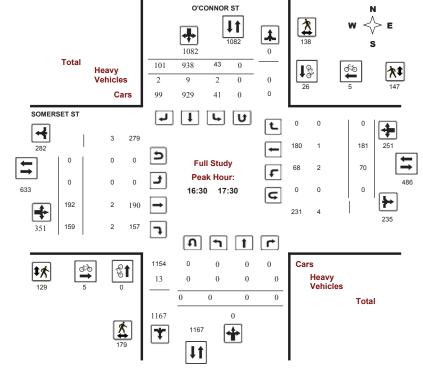


Turning Movement Count - Full Study Peak Hour Diagram

O'CONNOR ST @ SOMERSET ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36787

 Start Time:
 07:00
 Device:
 Miovision



Comments

Ottawa

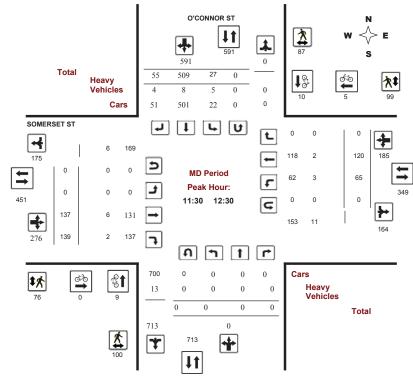
Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

O'CONNOR ST @ SOMERSET ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36787

 Start Time:
 07:00
 Device:
 Miovision

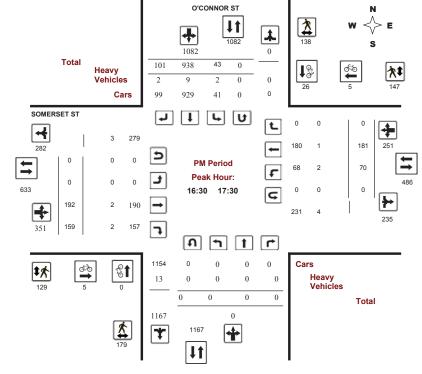




Turning Movement Count - Full Study Peak Hour Diagram

O'CONNOR ST @ SOMERSET ST

Survey Date: Tuesday, March 21, 2017 WO No: 36787
Start Time: 07:00 Device: Miovision



Comments

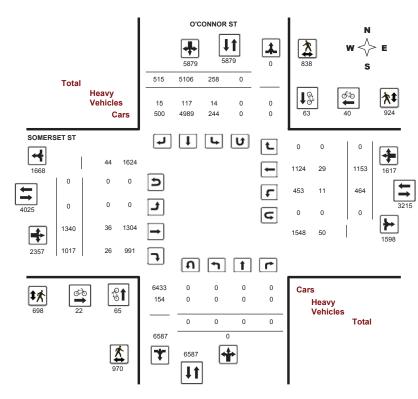


Transportation Services - Traffic Services Turning Movement Count - Full Study Diagram

O'CONNOR ST @ SOMERSET ST

Survey Date: Tuesday, March 21, 2017 WO#: 36787

Device: Miovision





Work Order 36787

Turning Movement Count - Full Study Summary Report

O'CONNOR ST @ SOMERSET ST

Survey Date: Tuesday, March 21, 2017 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 1.00

								Eastbou	ind: 0		Wes	tbound	: 0						
								F	ull Stu	ıdy									
_			0'0	CONNO	OR ST	Γ			_			SC	OMERS	SET S	T				
	N	lorthbo	ound		5	Southb	ound				Eastb	ound			Westb	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	0	0	0	19	443	45	507	507	0	143	78	221	36	109	0	145	366	873
08:00 09:00	0	0	0	0	42	466	50	558	558	0	197	110	307	57	164	0	221	528	1086
09:00 10:00	0	0	0	0	20	484	68	572	572	0	170	108	278	56	165	0	221	499	1071
11:30 12:30	0	0	0	0	27	509	55	591	591	0	137	139	276	65	120	0	185	461	1052
12:30 13:30	0	0	0	0	31	497	57	585	585	0	145	105	250	41	116	0	157	407	992
15:00 16:00	0	0	0	0	37	872	64	973	973	0	168	181	349	79	142	0	221	570	1543
16:00 17:00	0	0	0	0	45	916	88	1049	1049	0	198	155	353	60	161	0	221	574	1623
17:00 18:00	0	0	0	0	37	919	88	1044	1044	0	182	141	323	70	176	0	246	569	1613
Sub Total	0	0	0	0	258	5106	515	5879	5879	0	1340	1017	2357	464	1153	0	1617	3974	9853
U Turns				0				0	0				0				0	0	0
Total	0	0	0	0	258	5106	515	5879	5879	0	1340	1017	2357	464	1153	0	1617	3974	9853
EQ 12Hr	0	0	0	0	359	7097	716	8172	8172	0	1863	1414	3276	645	1603	0	2248	5524	13696
Note: These v	alues are	e calcul	ated by	multiply	ing the	totals b	y the a	ppropriat	e expansi	ion fact	tor.		1	1.39					
AVG 12Hr	0	0	0	0	359	7097	716	8172	8172	0	1863	1414	3276	645	1603	0	2248	5524	13696
Note: These v	olumes a	are calc	ulated	by multip	olying t	he Equiv	alent 1	2 hr. tota	als by the	AADT	factor.			1.00					
AVG 24Hr	0	0	0	0	470	9298	938	10705	10705	0	2440	1852	4292	845	2099	0	2944	7236	17941
Note: These v	olumes a	are calc	ulated	by multip	olying t	he Avera	age Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services w.o.

00707

Turning Movement Count - 15 Minute Summary Report

O'CONNOR ST @ SOMERSET ST

Survey Date: Tuesday, March 21, 2017 Total Observed U-Turns

Northbound: 0 Southbound: 0

Eastbound: 0 Westbound: 0

									E	astbour	d: ()	VVe	estboun	d: ()					
				O,CC	NNO	R ST						5	SOME	RSET	ST					
		N	orthbou	und		So	uthbour	ıd			Eas	stbound			Wes	stbound				
Time	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	0	0	0	4	90	9	103	103	0	21	12	33	3	13	0	16	49	152
07:15	07:30	0	0	0	0	2	102	8	112	112	0	37	16	53	6	21	0	27	80	192
07:30	07:45	0	0	0	0	6	127	5	138	138	0	37	27	64	17	45	0	62	126	264
07:45	08:00	0	0	0	0	7	124	23	154	154	0	48	23	71	10	30	0	40	111	265
08:00	08:15	0	0	0	0	8	134	14	156	156	0	60	25	85	13	40	0	53	138	294
08:15	08:30	0	0	0	0	7	99	15	121	121	0	41	23	64	18	41	0	59	123	244
08:30	08:45	0	0	0	0	12	113	10	135	135	0	52	29	81	17	38	0	55	136	271
08:45	09:00	0	0	0	0	15	120	11	146	146	0	44	33	77	9	45	0	54	131	277
09:00	09:15	0	0	0	0	8	127	13	148	148	0	52	31	83	13	41	0	54	137	285
09:15	09:30	0	0	0	0	4	104	16	124	124	0	45	26	71	16	44	0	60	131	255
09:30	09:45	0	0	0	0	2	129	23	154	154	0	44	26	70	13	44	0	57	127	281
09:45	10:00	0	0	0	0	6	124	16	146	146	0	29	25	54	14	36	0	50	104	250
11:30	11:45	0	0	0	0	3	130	21	154	154	0	29	31	60	17	31	0	48	108	262
11:45	12:00	0	0	0	0	9	123	14	146	146	0	46	50	96	19	31	0	50	146	292
12:00	12:15	0	0	0	0	7	125	11	143	143	0	26	34	60	20	29	0	49	109	252
12:15	12:30	0	0	0	0	8	131	9	148	148	0	36	24	60	9	29	0	38	98	246
12:30	12:45	0	0	0	0	4	115	16	135	135	0	45	29	74	12	23	0	35	109	244
12:45	13:00	0	0	0	0	13	134	15	162	162	0	34	25	59	11	37	0	48	107	269
13:00	13:15	0	0	0	0	11	129	9	149	149	0	33	28	61	9	26	0	35	96	245
13:15	13:30	0	0	0	0	3	119	17	139	139	0	33	23	56	9	30	0	39	95	234
15:00	15:15	0	0	0	0	13	227	17	257	257	0	35	48	83	24	32	0	56	139	396
15:15	15:30	0	0	0	0	10	224	11	245	245	0	37	47	84	18	35	0	53	137	382
15:30	15:45	0	0	0	0	6	210	25	241	241	0	51	39	90	20	32	0	52	142	383
15:45	16:00	0	0	0	0	8	211	11	230	230	0	45	47	92	17	43	0	60	152	382
16:00	16:15	0	0	0	0	7	230	16	253	253	0	52	34	86	11	44	0	55	141	394
16:15	16:30	0	0	0	0	9	236	24	269	269	0	53	41	94	16	38	0	54	148	417
16:30	16:45	0	0	0	0	14	240	27	281	281	0	51	38	89	16	43	0	59	148	429
16:45	17:00	0	0	0	0	15	210	21	246	246	0	42	42	84	17	36	0	53	137	383
17:00	17:15	0	0	0	0	7	242	24	273	273	0	47	40	87	24	56	0	80	167	440
17:15	17:30	0	0	0	0	7	246	29	282	282	0	52	39	91	13	46	0	59	150	432
17:30	17:45	0	0	0	0	13	238	18	269	269	0	31	30	61	12	45	0	57	118	387
17:45	18:00	0	0	0	0	10	193	17	220	220	0	52	32	84	21	29	0	50	134	354
TOTAL	L:	0	0	0	0	258	5106	515	5879	5879	0	1340	1017	2357	464	1153	0	161	17 3974	9853

Note: U-Turns are included in Totals.

Comment:



Turning Movement Count - Cyclist Volume Report

Work Order 36787

O'CONNOR ST @ SOMERSET ST

Count Date: Tuesday, March 21, 2017 Start Time: 07:00

		O'CONNOR ST			SOMERSET S	Г	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	8	1	9	3	3	6	15
08:00 09:00	30	1	31	2	6	8	39
09:00 10:00	10	3	13	3	2	5	18
11:30 12:30	9	10	19	0	5	5	24
12:30 13:30	5	1	6	0	3	3	9
15:00 16:00	2	10	12	3	3	6	18
16:00 17:00	1	18	19	6	5	11	30
17:00 18:00	0	19	19	5	13	18	37
Total	65	63	128	22	40	62	190

Comment:



Transportation Services - Traffic Services

W.O. 36787

Turning Movement Count - Heavy Vehicle Report

O'CONNOR ST @ SOMERSET ST

Survey Date: Tuesday, March 21, 2017

			0'0	CONN	IOR S	Т						sc	MER	SET S	Т					
		Northb	ound		;	Southb	ound	_			Eastb	ound		1	Westbo	ound				
Time P	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
7:00	08:00	0	0	0	0	1	20	3	24	24	0	8	3	11	0	4	0	4	15	39
08:00	09:00	0	0	0	0	1	15	2	18	18	0	5	2	7	3	3	0	6	13	31
9:00	10:00	0	0	0	0	1	26	1	28	28	0	6	6	12	1	9	0	10	22	50
11:30	12:30	0	0	0	0	5	8	4	17	17	0	6	2	8	3	2	0	5	13	30
12:30	13:30	0	0	0	0	1	16	2	19	19	0	3	4	7	1	4	0	5	12	31
15:00	16:00	0	0	0	0	1	8	1	10	10	0	5	3	8	1	3	0	4	12	22
16:00	17:00	0	0	0	0	3	15	2	20	20	0	3	5	8	2	3	0	5	13	33
17:00	18:00	0	0	0	0	1	9	0	10	10	0	0	1	1	0	1	0	1	2	12
Sub T	Γotal	0	0	0	0	14	117	15	146	146	0	36	26	62	11	29	0	40	102	248
-Turns	s (Hea	vy Vel	nicles)		0				0	0				0				0	0	0
Tota	al	0	0	0	0	14	117	15	146	146	0	36	26	62	11	29	0	40	102	248



Work Order 36787

Turning Movement Count - Pedestrian Volume Report

		0'0	CONNOF	R ST @ SOMER	RSET ST		
Count Dat	e: Tuesday, Ma	arch 21, 2017				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	7	4	11	13	3	16	27
07:15 07:30	11	14	25	8	16	24	49
07:30 07:45	11	22	33	19	22	41	74
07:45 08:00	27	22	49	25	34	59	108
07:00 08:00	56	62	118	65	75	140	258
08:00 08:15	36	27	63	31	42	73	136
08:15 08:30	39	16	55	26	47	73	128
08:30 08:45	34	39	73	35	53	88	161
08:45 09:00	44	37	81	28	58	86	167
08:00 09:00	153	119	272	120	200	320	592
09:00 09:15	24	22	46	18	34	52	98
09:15 09:30	19	18	37	21	15	36	73
09:30 09:45	15	6	21	12	12	24	45
09:45 10:00	19	9	28	13	18	31	59
09:00 10:00	77	55	132	64	79	143	275
11:30 11:45	27	22	49	16	15	31	80
11:45 12:00	22	22	44	17	27	44	88
12:00 12:15	30	21	51	19	28	47	98
12:15 12:30	21	22	43	24	29	53	96
11:30 12:30	100	87	187	76	99	175	362
12:30 12:45	32	23	55	16	28	44	99
12:45 13:00	27	43	70	13	28	41	111
13:00 13:15	29	30	59	14	24	38	97
13:15 13:30	24	18	42	23	18	41	83
12:30 13:30	112	114	226	66	98	164	390
15:00 15:15	29	26	55	18	15	33	88
15:15 15:30	23	21	44	13	20	33	77
15:30 15:45	37	23	60	19	19	38	98
15:45 16:00	31	27	58	7	26	33	91
15:00 16:00	120	97	217	57	80	137	354
16:00 16:15	33	32	65	24	34	58	123
16:15 16:30	37	34	71	38	43	81	152
16:30 16:45	47	36	83	32	34	66	149
16:45 17:00	41	37	78	27	34	61	139
16:00 17:00	158	139	297	121	145	266	563
17:00 17:15	52	36	88	44	38	82	170
17:15 17:30	39	29	68	26	41	67	135
17:30 17:45	53	39	92	35	34	69	161
17:45 18:00	50	61	111	24	35	59	170
17:00 18:00	194	165	359	129	148	277	636
Total	970	838	1808	698	924	1622	3430

Comment:

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 36787

Turning Movement Count - 15 Min U-Turn Total Report

O'CONNOR ST @ SOMERSET ST

Time Period Northbound UTum Total Southbound UTum Total Eastbound UTum Total Westbound UTum Total Total 07:00 07:15 0 0 0 0 0 0 07:15 07:30 0 0 0 0 0 0 07:30 07:45 0 0 0 0 0 0 07:45 08:00 0 0 0 0 0 0 08:00 08:15 0 0 0 0 0 0 08:30 08:45 0 0 0 0 0 0 08:45 09:00 0 0 0 0 0 0 0 09:00 09:15 0 <th>Survey Date:</th> <th>Tu</th> <th>uesday, March 21</th> <th>, 2017</th> <th></th> <th></th> <th></th>	Survey Date:	Tu	uesday, March 21	, 2017			
07:15 07:30 0	Time Pe	riod					Total
07:30 07:45 0	07:00	07:15	0	0	0	0	0
07:45 08:00 0	07:15	07:30	0	0	0	0	0
08:00 08:15 0	07:30	07:45	0	0	0	0	0
08:15 08:30 0 0 0 0 0 08:30 08:45 0 0 0 0 0 08:45 09:00 0 0 0 0 0 09:00 09:15 0 0 0 0 0 09:15 09:30 0 0 0 0 0 0 09:30 09:45 0 0 0 0 0 0 0 09:45 10:00 0 0 0 0 0 0 0 0 11:30 11:45 0<	07:45	08:00	0	0	0	0	0
08:30 08:45 0 0 0 0 0 08:45 09:00 0 0 0 0 0 09:00 09:15 0 0 0 0 0 0 09:15 09:30 0	08:00	08:15	0	0	0	0	0
08:45 09:00 0	08:15	08:30	0	0	0	0	0
09:00 09:15 0	08:30	08:45	0	0	0	0	0
09:15 09:30 0	08:45	09:00	0	0	0	0	0
09:30 09:45 0	09:00	09:15	0	0	0	0	0
09:45 10:00 0 0 0 0 0 11:30 11:45 0 0 0 0 0 11:45 12:00 0 0 0 0 0 12:00 12:15 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:45 16:00 0 0 0	09:15	09:30	0	0	0	0	0
11:30 11:45 0 0 0 0 0 11:45 12:00 0 0 0 0 0 12:00 12:15 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:15 0 0 0 0 0 <td>09:30</td> <td>09:45</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td>	09:30	09:45	0	0	0	0	0
11:45 12:00 0 0 0 0 0 12:00 12:15 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:45 16:00 0 0 0 0 0 16:15 16:30 0 0 0	09:45	10:00	0	0	0	0	0
12:00 12:15 0 0 0 0 0 12:15 12:30 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:45 15:30 0 0 0 0 0 15:45 16:00 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:15 0 0 0 0 0 0 16:30 16:45 0 0 0 0 <td>11:30</td> <td>11:45</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td>	11:30	11:45	0	0	0	0	0
12:15 12:30 0 0 0 0 0 12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:10 17:15 0 0 0	11:45	12:00	0	0	0	0	0
12:30 12:45 0 0 0 0 0 12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:45 0 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 0 16:00 16:15 0	12:00	12:15	0	0	0	0	0
12:45 13:00 0 0 0 0 0 13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:15 0 0 0 0 0 0 17:15 17:30 0 0 0 0 <td>12:15</td> <td>12:30</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td>	12:15	12:30	0	0	0	0	0
13:00 13:15 0 0 0 0 0 13:15 13:30 0 0 0 0 0 15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0	12:30	12:45	0	0	0	0	0
13:15 13:30 0 0 0 0 0 0 15:00 15:15 0 0 0 0 0 0 15:15 15:30 0 0 0 0 0 0 15:30 15:45 0 0 0 0 0 0 16:45 16:00 0 0 0 0 0 0 0 16:00 16:15 0 <td< td=""><td>12:45</td><td>13:00</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></td<>	12:45	13:00	0	0	0	0	0
15:00 15:15 0 0 0 0 0 15:15 15:30 0 0 0 0 0 15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	13:00	13:15	0	0	0	0	0
15:15 15:30 0 0 0 0 0 0 15:30 15:45 0 0 0 0 0 0 15:45 16:00 0 0 0 0 0 0 16:00 16:15 0 0 0 0 0 0 16:15 16:30 0 0 0 0 0 0 16:30 16:45 0 0 0 0 0 0 16:45 17:00 0 0 0 0 0 0 17:00 17:15 0 0 0 0 0 0 17:30 17:45 0 0 0 0 0 0 17:45 18:00 0 0 0 0 0 0	13:15	13:30	0	0	0	0	0
15:30 15:45 0 0 0 0 0 15:45 16:00 0 0 0 0 0 16:00 16:15 0 0 0 0 0 16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	15:00	15:15	0	0	0	0	0
15:45 16:00 0 0 0 0 0 0 16:00 16:15 0 0 0 0 0 0 16:15 16:30 0 0 0 0 0 0 16:30 16:45 0 0 0 0 0 0 16:45 17:00 0 0 0 0 0 0 17:00 17:15 0 0 0 0 0 0 17:15 17:30 0 0 0 0 0 0 17:45 18:00 0 0 0 0 0 0	15:15	15:30	0	0	0	0	0
16:00 16:15 0 0 0 0 0 16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	15:30	15:45	0	0	0	0	0
16:15 16:30 0 0 0 0 0 16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	15:45	16:00	0	0	0	0	0
16:30 16:45 0 0 0 0 0 16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	16:00	16:15	0	0	0	0	0
16:45 17:00 0 0 0 0 0 17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	16:15	16:30	0	0	0	0	0
17:00 17:15 0 0 0 0 0 17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	16:30	16:45	0	0	0	0	0
17:15 17:30 0 0 0 0 0 17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	16:45	17:00	0	0	0	0	0
17:30 17:45 0 0 0 0 0 17:45 18:00 0 0 0 0 0	17:00	17:15	0	0	0	0	0
17:45 18:00 0 0 0 0	17:15	17:30	0	0	0	0	0
	17:30	17:45	0	0	0	0	0
Total 0 0 0 0 0	17:45	18:00	0	0	0	0	0
	Total		0	0	0	0	0



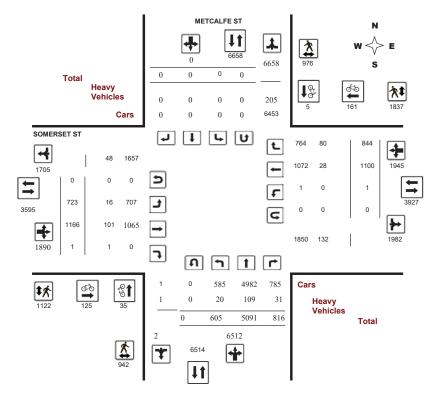
Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study Diagram





Transportation Services - Traffic Services

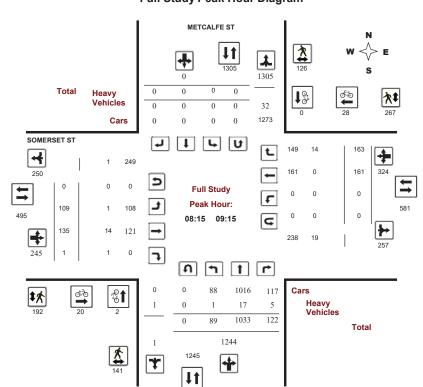
Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study Peak Hour Diagram



October 27, 2020 Page 1 of 8 October 27, 2020 Page 2 of 8

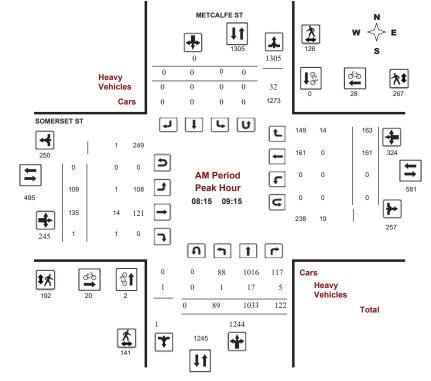


Turning Movement Count - Peak Hour Diagram

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision



Comments



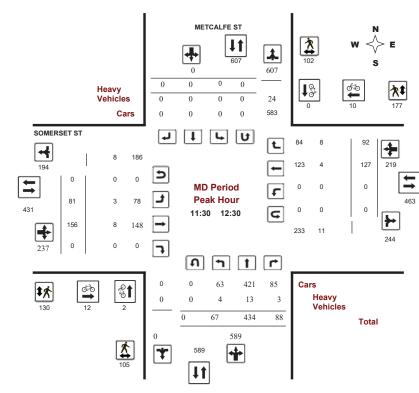
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

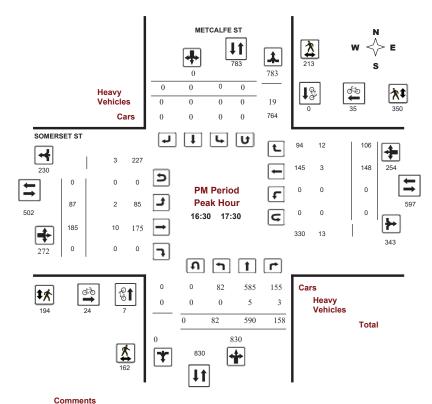




Turning Movement Count - Peak Hour Diagram

METCALFE ST @ SOMERSET ST

Survey Date: Thursday, May 02, 2019 WO No: 38599 Start Time: 07:00 Device: Miovision





Transportation Services - Traffic Services

Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

Survey Date: Thursday, May 02, 2019 WO No: 38599 Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, May 02, 2019 **Total Observed U-Turns AADT Factor** .90

Eastbound: Westbound:

									Ü										
_				CALFE					_			_	ИERSI						
	No	rthbou	nd		Sou	uthbou	nd			Е	astbou	ınd		W	/estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Gran Tota
07:00 08:00	67	999	99	1165	0	0	0	0	1165	96	77	0	173	0	106	84	190	363	152
08:00 09:00	84	1054	121	1259	0	0	0	0	1259	115	131	1	247	0	154	153	307	554	181
09:00 10:00	78	690	70	838	0	0	0	0	838	109	146	0	255	0	143	121	264	519	135
11:30 12:30	67	434	88	589	0	0	0	0	589	81	156	0	237	0	127	92	219	456	104
12:30 13:30	54	408	69	531	0	0	0	0	531	94	126	0	220	1	129	85	215	435	96
15:00 16:00	87	428	97	612	0	0	0	0	612	61	162	0	223	0	139	92	231	454	106
16:00 17:00	93	549	137	779	0	0	0	0	779	91	190	0	281	0	142	114	256	537	131
17:00 18:00	75	529	135	739	0	0	0	0	739	76	178	0	254	0	160	103	263	517	125
Sub Total	605	5091	816	6512	0	0	0	0	6512	723	1166	1	1890	1	1100	844	1945	3835	1034
U Turns	0			0	0			0	0	0			0	0			0	0	(
Total	605	5091	816	6512	0	0	0	0	6512	723	1166	1	1890	1	1100	844	1945	3835	1034
EQ 12Hr	841	7076	1134	9051	0	0	0	0	9051	1005	1621	1	2627	1	1529	1173	2703	5330	1438
Note: These v	alues a	re calcu	lated by	y multiply	ring the	totals b	y the ap	propriat	e expans	sion fac	tor.			1.39					
AVG 12Hr	757	6368	1021	8146	0	0	0	0	8146	904	1459	1	2364	1	1376	1056	2433	4797	1294
Note: These v	olumes	are cal	culated	by multip	lying th	e Equiv	alent 1	2 hr. tota	ls by the	AADT	factor.			.90					
AVG 24Hr	992	8342	1338	10672	0	0	0	0	10672	1184	1911	1	3096	1	1803	1383	3187	6283	1695
Note: These v	olumes	are cal	culated	by multip	olying th	e Avera	ge Dail	y 12 hr.	totals by	12 to 2	4 expans	sion fac	ctor.	1.31					
Note: U-Turr	ns prov	ided fo	r appro	oach tot	als. Re	fer to 'l	J-Turn	' Repor	t for spe	ecific b	reakdov	vn.							

2020-Oct-27 Page 3 of 3 October 27, 2020 Page 3 of 8



Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

METCALFE ST

Full Study 15 Minute Increments SOMERSET ST

TOT LT ST LT ST RT LT ST RT 442 **328** 21 29 297 33 353 24 487 **282** 27 **159** 21 34 **211** 34 37 308 186 27 39 **127** 26 46 259 **141** 18 36 **152** 20 23 237 42 85 **121** 30 243 138 25 68 121 259 **120** 19 227 **153** 15 264 **149** 18 44 293 175 349 **189** 32 45

223 15 53

171 23

0 0 **145** 20 31 0 0 **6512** 723 1166 44 25 69 137

360

320

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study Cyclist Volume

		METCALFE ST			SOMERSET S	ST	
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	1	6	7	7
07:15 07:30	0	0	0	0	1	1	1
07:30 07:45	0	0	0	7	4	11	11
07:45 08:00	1	0	1	3	2	5	6
08:00 08:15	0	0	0	1	4	5	5
08:15 08:30	0	0	0	4	5	9	9
08:30 08:45	0	0	0	6	10	16	16
08:45 09:00	2	0	2	5	9	14	16
9:00 09:15	0	0	0	5	4	9	9
9:15 09:30	1	1	2	4	3	7	9
09:30 09:45	0	0	0	1	1	2	2
9:45 10:00	1	0	1	1	4	5	6
11:30 11:45	1	0	1	4	5	9	10
11:45 12:00	0	0	0	1	2	3	3
2:00 12:15	1	0	1	4	3	7	8
12:15 12:30	0	0	0	3	0	3	3
12:30 12:45	1	0	1	3	3	6	7
12:45 13:00	1	0	1	5	5	10	11
13:00 13:15	0	0	0	2	0	2	2
13:15 13:30	3	0	3	3	3	6	9
15:00 15:15	3	1	4	3	1	4	8
15:15 15:30	3	0	3	5	5	10	13
15:30 15:45	3	0	3	9	10	19	22
15:45 16:00	1	0	1	4	7	11	12
16:00 16:15	2	0	2	5	5	10	12
16:15 16:30	2	2	4	3	10	13	17
17:15 17:30	1	0	1	5	8	13	14
16:30 16:45	2	0	2	8	13	21	23
16:45 17:00	2	0	2	5	8	13	15
17:00 17:15	2	0	2	6	6	12	14
17:30 17:45	1	0	1	4	8	12	13
17:45 18:00	1	1	2	5	6	11	13
Total	35	5	40	125	161	286	326

October 27, 2020 Page 4 of 8 October 27, 2020 Page 5 of 8



Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study Pedestrian Volume

METCALFE ST SOMERSET ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	11	9	20	15	21	36	56
07:15 07:30	14	10	24	15	34	49	73
07:30 07:45	27	12	39	26	45	71	110
07:45 08:00	23	20	43	25	67	92	135
08:00 08:15	30	25	55	34	71	105	160
08:15 08:30	37	33	70	56	71	127	197
08:30 08:45	35	28	63	45	67	112	175
08:45 09:00	28	34	62	54	66	120	182
09:00 09:15	41	31	72	37	63	100	172
09:15 09:30	27	19	46	23	37	60	106
09:30 09:45	15	13	28	19	36	55	83
09:45 10:00	16	16	32	25	31	56	88
11:30 11:45	34	20	54	21	41	62	116
1:45 12:00	18	29	47	43	40	83	130
2:00 12:15	22	27	49	37	49	86	135
12:15 12:30	31	26	57	29	47	76	133
2:30 12:45	27	27	54	39	55	94	148
2:45 13:00	23	29	52	35	71	106	158
3:00 13:15	34	26	60	44	51	95	155
3:15 13:30	20	30	50	35	42	77	127
5:00 15:15	30	32	62	36	54	90	152
5:15 15:30	16	33	49	21	34	55	104
5:30 15:45	40	32	72	28	52	80	152
5:45 16:00	21	36	57	29	51	80	137
6:00 16:15	27	55	82	42	63	105	187
6:15 16:30	37	34	71	38	88	126	197
7:15 17:30	36	56	92	50	97	147	239
6:30 16:45	30	46	76	54	82	136	212
6:45 17:00	42	48	90	37	88	125	215
7:00 17:15	54	63	117	53	83	136	253
7:30 17:45	55	27	82	42	82	124	206
7:45 18:00	41	50	91	35	58	93	184
Total	942	976	1918	1122	1837	2959	4877



Transportation Services - Traffic Services

Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study Heavy Vehicles

METCALFE ST SOMERSET ST

		NI.	orthbo	und		۰.	outhbou	nd			_	astbour	nd.		10/	estbour	ad.			
		IN			N				s	STR				_		esibour		w	STR	Grand
Time	Period	LT	ST	RT	N TOT	LT	ST	RT	тот	TOT	LT	ST	RT	E TOT	LT	ST	RT	TOT	TOT	Total
07:00	07:15	0	3	1	4	0	0	0	0	4	0	1	0	1	0	0	1	1	2	6
07:15	07:30	0	3	3	6	0	0	0	0	6	0	2	0	2	0	0	3	3	5	11
07:30	07:45	1	3	3	7	0	0	0	0	7	1	1	0	2	0	0	2	2	4	11
07:45	08:00	0	3	2	5	0	0	0	0	5	0	5	0	5	0	1	4	5	10	15
08:00	08:15	2	6	1	9	0	0	0	0	9	1	2	0	3	0	0	2	2	5	14
08:15	08:30	1	3	0	4	0	0	0	0	4	0	4	0	4	0	0	3	3	7	11
08:30	08:45	0	7	1	8	0	0	0	0	8	1	2	0	3	0	0	6	6	9	17
08:45	09:00	0	4	1	5	0	0	0	0	5	0	6	1	7	0	0	3	3	10	15
09:00	09:15	0	3	3	6	0	0	0	0	6	0	2	0	2	0	0	2	2	4	10
09:15	09:30	1	6	0	7	0	0	0	0	7	0	4	0	4	0	4	4	8	12	19
09:30	09:45	2	8	0	10	0	0	0	0	10	3	4	0	7	0	2	2	4	11	21
09:45	10:00	2	9	0	11	0	0	0	0	11	0	9	0	9	0	0	2	2	11	22
11:30	11:45	1	2	0	3	0	0	0	0	3	0	2	0	2	0	1	2	3	5	8
11:45	12:00	0	5	0	5	0	0	0	0	5	1	2	0	3	0	2	3	5	8	13
12:00	12:15	1	1	1	3	0	0	0	0	3	0	2	0	2	0	0	2	2	4	7
12:15	12:30	2	5	2	9	0	0	0	0	9	2	2	0	4	0	1	1	2	6	15
12:30	12:45	1	8	0	9	0	0	0	0	9	0	2	0	2	0	0	2	2	4	13
12:45	13:00	0	2	0	2	0	0	0	0	2	2	6	0	8	0	1	0	1	9	11
13:00	13:15	2	6	3	11	0	0	0	0	11	1	2	0	3	0	1	3	4	7	18
13:15	13:30	1	5	0	6	0	0	0	0	6	0	4	0	4	0	6	0	6	10	16
15:00	15:15	1	1	0	2	0	0	0	0	2	1	5	0	6	0	2	2	4	10	12
15:15	15:30	0	3	0	3	0	0	0	0	3	0	3	0	3	0	0	3	3	6	9
15:30	15:45	1	3	1	5	0	0	0	0	5	0	2	0	2	0	0	3	3	5	10
15:45	16:00	0	0	1	1	0	0	0	0	1	1	2	0	3	0	2	3	5	8	9
16:00	16:15	0	1	3	4	0	0	0	0	4	0	4	0	4	0	2	5	7	11	15
16:15	16:30	0	2	1	3	0	0	0	0	3	0	2	0	2	0	0	1	1	3	6
17:15	17:30	0	1	1	2	0	0	0	0	2	0	2	0	2	0	0	3	3	5	7
16:30	16:45	0	1	1	2	0	0	0	0	2	1	2	0	3	0	1	1	2	5	7
16:45	17:00	0	1	0	1	0	0	0	0	1	1	4	0	5	0	0	5	5	10	11
17:00	17:15	0	2	1	3	0	0	0	0	3	0	2	0	2	0	2	3	5	7	10
17:30	17:45	1	1	1	3	0	0	0	0	3	0	4	0	4	0	0	3	3	7	10
17:45	18:00	0	1	0	1	0	0	0	0	1	0	5	0	5	0	0	1	1	6	7
Total:	None	20	109	31	160	0	0	0	0	160	16	101	1	118	0	28	80	108	226	386

October 27, 2020 Page 6 of 8 October 27, 2020 Page 7 of 8



Turning Movement Count - Study Results

METCALFE ST @ SOMERSET ST

 Survey Date:
 Thursday, May 02, 2019
 WO No:
 38599

 Start Time:
 07:00
 Device:
 Miovision

Full Study 15 Minute U-Turn Total METCALFE ST SOMERSET ST

Northbound Southbound Eastbound Westbound Time Period Total U-Turn Total **U-Turn Total U-Turn Total U-Turn Total** 07:00 07:15 07:15 07:30 0 0 0 07:30 07:45 0 07:45 08:00 08:00 08:15 08:15 08:30 08:30 08:45 08:45 09:00 0 09:00 09:15 09:15 09:30 09:30 09:45 0 0 09:45 10:00 0 0 0 0 11:30 11:45 11:45 12:00 12:00 12:15 12:30 12:30 12:45 13:00 12:45 13:00 13:15 0 13:15 13:30 0 0 15:00 15:15 15:30 0 0 15:30 15:45 0 0 0 16:00 15:45 16:15 16:00 0 0 0 16:15 16:30 17:15 16:45 16:30 16:45 17:00 0 0 0 17:00 17:15 Ω Ω 17:30 17:45 0 0 0 17:45 18:00 Total

October 27, 2020 Page 8 of 8

Ottawa

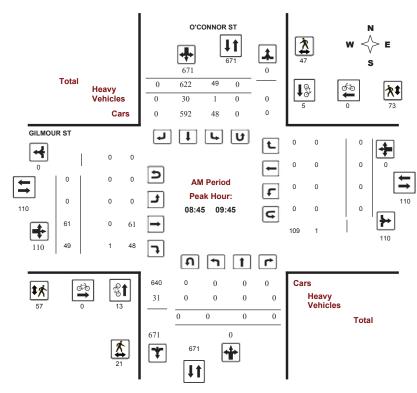
Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36785

 Start Time:
 07:00
 Device:
 Miovision



Comments

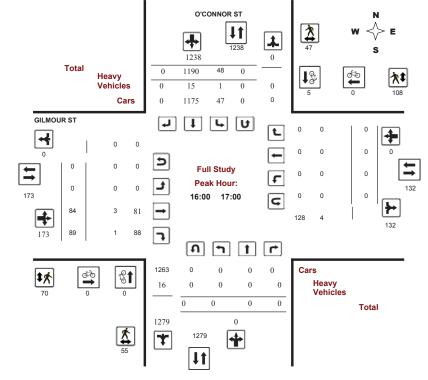


Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36785

 Start Time:
 07:00
 Device:
 Miovision



Comments

Ottawa

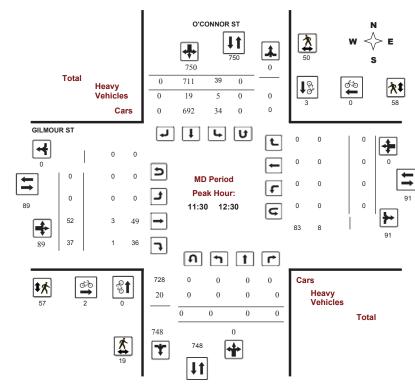
Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36785

 Start Time:
 07:00
 Device:
 Miovision



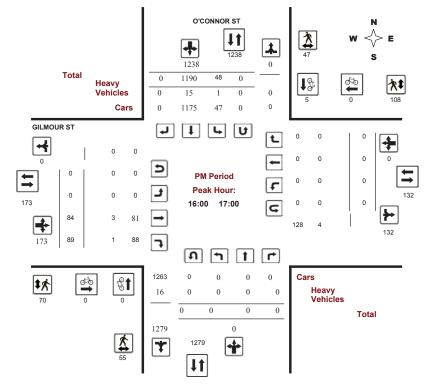


Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST

 Survey Date:
 Tuesday, March 21, 2017
 WO No:
 36785

 Start Time:
 07:00
 Device:
 Miovision



Comments

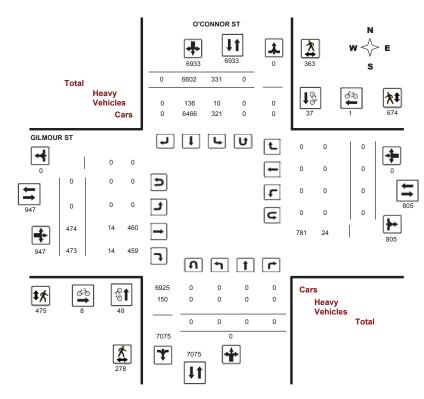


Transportation Services - Traffic Services Turning Movement Count - Full Study Diagram

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017 WO#: 36785

Device: Miovision





Work Order 36785

Turning Movement Count - Full Study Summary Report

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017 **Total Observed U-Turns AADT Factor** Northbound: () Southbound: () 1.00

								-	ull Stu	ıay									
_			0'0	CONN	OR S	Γ			_			G	ILMOL	JR ST					
	N	orthbo	ound		,	Southbo	ound			Eastbound		ound	t		Westbound				
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Tota
07:00 08:00	0	0	0	0	39	555	0	594	594	0	39	26	65	0	0	0	0	65	659
08:00 09:00	0	0	0	0	45	601	0	646	646	0	79	43	122	0	0	0	0	122	768
09:00 10:00	0	0	0	0	51	619	0	670	670	0	52	41	93	0	0	0	0	93	763
11:30 12:30	0	0	0	0	39	711	0	750	750	0	52	37	89	0	0	0	0	89	839
12:30 13:30	0	0	0	0	25	615	0	640	640	0	41	50	91	0	0	0	0	91	731
15:00 16:00	0	0	0	0	38	1192	0	1230	1230	0	54	96	150	0	0	0	0	150	1380
16:00 17:00	0	0	0	0	48	1190	0	1238	1238	0	84	89	173	0	0	0	0	173	1411
17:00 18:00	0	0	0	0	46	1119	0	1165	1165	0	73	91	164	0	0	0	0	164	1329
Sub Total	0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880
U Turns				0				0	0				0				0	0	0
Total	0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880
EQ 12Hr	0	0	0	0	460	9177	0	9637	9637	0	659	657	1316	0	0	0	0	1316	10953
Note: These va	alues are	e calcula	ated by	multiply	ying the	totals by	the a	ppropriat	e expansi	on fact	or.		1	.39					
AVG 12Hr	0	0	0	0	460	9177	0	9637	9637	0	659	657	1316	0	0	0	0	1316	10953
Note: These vo	olumes a	are calc	ulated I	oy multi	plying t	he Equiv	alent 1	2 hr. tota	als by the	AADT f	actor.		1	.00					
AVG 24Hr	0	0	0	0	603	12022	0	12624	12624	0	863	861	1724	0	0	0	0	1724	14348

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

Page 1 of 1 2019-Jul-04



Transportation Services - Traffic Services w.o.

Turning Movement Count - 15 Minute Summary Report

GILMOUR ST @ O'CONNOR ST Survey Date: Tuesday, March 21, 2017 **Total Observed U-Turns**

Northbound: 0 Southbound: 0

Eastbound: () Westbound: 0

O'CONNOR ST **GILMOUR ST**

		N	orthbou	ınd		So	uthboun	d			Eas	stbound			Wes	stbound	d			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	0	0	0	8	103	0	111	111	0	1	2	3	0	0	0	0	3	114
07:15	07:30	0	0	0	0	12	129	0	141	141	0	9	3	12	0	0	0	0	12	153
07:30	07:45	0	0	0	0	10	162	0	172	172	0	15	12	27	0	0	0	0	27	199
07:45	08:00	0	0	0	0	9	161	0	170	170	0	14	9	23	0	0	0	0	23	193
08:00	08:15	0	0	0	0	14	156	0	170	170	0	24	9	33	0	0	0	0	33	203
08:15	08:30	0	0	0	0	13	127	0	140	140	0	19	10	29	0	0	0	0	29	169
08:30	08:45	0	0	0	0	7	159	0	166	166	0	14	7	21	0	0	0	0	21	187
08:45	09:00	0	0	0	0	11	159	0	170	170	0	22	17	39	0	0	0	0	39	209
09:00	09:15	0	0	0	0	13	158	0	171	171	0	15	9	24	0	0	0	0	24	195
09:15	09:30	0	0	0	0	11	137	0	148	148	0	20	11	31	0	0	0	0	31	179
09:30	09:45	0	0	0	0	14	168	0	182	182	0	4	12	16	0	0	0	0	16	198
09:45	10:00	0	0	0	0	13	156	0	169	169	0	13	9	22	0	0	0	0	22	191
11:30	11:45	0	0	0	0	6	184	0	190	190	0	13	5	18	0	0	0	0	18	208
11:45	12:00	0	0	0	0	17	178	0	195	195	0	9	11	20	0	0	0	0	20	215
12:00	12:15	0	0	0	0	8	179	0	187	187	0	19	9	28	0	0	0	0	28	215
12:15	12:30	0	0	0	0	8	170	0	178	178	0	11	12	23	0	0	0	0	23	201
12:30	12:45	0	0	0	0	7	152	0	159	159	0	11	15	26	0	0	0	0	26	185
12:45	13:00	0	0	0	0	10	160	0	170	170	0	11	15	26	0	0	0	0	26	196
13:00	13:15	0	0	0	0	3	154	0	157	157	0	7	9	16	0	0	0	0	16	173
13:15	13:30	0	0	0	0	5	149	0	154	154	0	12	11	23	0	0	0	0	23	177
15:00	15:15	0	0	0	0	5	335	0	340	340	0	19	30	49	0	0	0	0	49	389
15:15	15:30	0	0	0	0	14	288	0	302	302	0	14	20	34	0	0	0	0	34	336
15:30	15:45	0	0	0	0	7	277	0	284	284	0	10	23	33	0	0	0	0	33	317
15:45	16:00	0	0	0	0	12	292	0	304	304	0	11	23	34	0	0	0	0	34	338
16:00	16:15	0	0	0	0	16	298	0	314	314	0	22	29	51	0	0	0	0	51	365
16:15	16:30	0	0	0	0	13	305	0	318	318	0	18	22	40	0	0	0	0	40	358
16:30	16:45	0	0	0	0	7	291	0	298	298	0	19	14	33	0	0	0	0	33	331
16:45	17:00	0	0	0	0	12	296	0	308	308	0	25	24	49	0	0	0	0	49	357
17:00	17:15	0	0	0	0	16	303	0	319	319	0	21	24	45	0	0	0	0	45	364
17:15	17:30	0	0	0	0	11	288	0	299	299	0	28	23	51	0	0	0	0	51	350
17:30	17:45	0	0	0	0	13	285	0	298	298	0	13	23	36	0	0	0	0	36	334
17:45	18:00	0	0	0	0	6	243	0	249	249	0	11	21	32	0	0	0	0	32	281
TOTAL	:	0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880

Note: U-Turns are included in Totals.

Comment:



Turning Movement Count - Cyclist Volume Report

Work Order 36785

GILMOUR ST @ O'CONNOR ST

Count Date: Tuesday, March 21, 2017 Start Time: 07:00

		O'CONNOR ST			GILMOUR ST		
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	5	0	5	1	0	1	6
08:00 09:00	26	1	27	2	0	2	29
09:00 10:00	8	5	13	0	0	0	13
11:30 12:30	0	3	3	2	0	2	5
12:30 13:30	3	0	3	1	1	2	5
15:00 16:00	5	2	7	0	0	0	7
16:00 17:00	0	5	5	0	0	0	5
17:00 18:00	2	21	23	2	0	2	25
Total	49	37	86	8	1	9	95

Comment:



Transportation Services - Traffic Services

W.O. 36785

Turning Movement Count - Heavy Vehicle Report

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017

			0'0	CONN	IOR S	Т						G	ILMO	UR ST						
		Northb	ound			Southb	ound	_		-	Eastb	ound		1	Vestbo	ound	_			
Time F	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	0	0	0	0	0	18	0	18	18	0	0	0	0	0	0	0	0	0	18
08:00	09:00	0	0	0	0	1	22	0	23	23	0	2	1	3	0	0	0	0	3	26
09:00	10:00	0	0	0	0	0	29	0	29	29	0	0	1	1	0	0	0	0	1	30
11:30	12:30	0	0	0	0	5	19	0	24	24	0	3	1	4	0	0	0	0	4	28
12:30	13:30	0	0	0	0	2	16	0	18	18	0	4	3	7	0	0	0	0	7	25
15:00	16:00	0	0	0	0	0	8	0	8	8	0	1	3	4	0	0	0	0	4	12
16:00	17:00	0	0	0	0	1	15	0	16	16	0	3	1	4	0	0	0	0	4	20
17:00	18:00	0	0	0	0	1	9	0	10	10	0	1	4	5	0	0	0	0	5	15
Sub 1	Total	0	0	0	0	10	136	0	146	146	0	14	14	28	0	0	0	0	28	174
J-Turn:	s (Heav	/y Veh	nicles)		0				0	0				0				0	0	0
Tot	al	0	0	0	0	10	136	0	146	146	0	14	14	28	0	0	0	0	28	174



Work Order 36785

Turning Movement Count - Pedestrian Volume Report

				ST @ O'CON			
Count Dat	e: Tuesday, Ma	arch 21, 2017				Start Time:	07:00
Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	2	5	9	7	16	21
07:15 07:30	3	2	5	9	12	21	26
07:30 07:45	6	8	14	9	22	31	45
07:45 08:00	4	10	14	15	16	31	45
07:00 08:00	16	22	38	42	57	99	137
08:00 08:15	18	12	30	16	17	33	63
08:15 08:30	4	18	22	29	33	62	84
08:30 08:45	6	16	22	31	37	68	90
08:45 09:00	8	18	26	19	30	49	75
08:00 09:00	36	64	100	95	117	212	312
09:00 09:15	5	8	13	16	18	34	47
09:15 09:30	2	8	10	13	15	28	38
09:30 09:45	6	13	19	9	10	19	38
09:45 10:00	2	6	8	5	12	17	25
09:00 10:00	15	35	50	43	55	98	148
11:30 11:45	6	6	12	15	12	27	39
11:45 12:00	3	8	11	8	11	19	30
12:00 12:15	5	18	23	15	20	35	58
12:15 12:30	5	18	23	19	15	34	57
11:30 12:30	19	50	69	57	58	115	184
12:30 12:45	13	16	29	18	31	49	78
12:45 13:00	4	11	15	10	17	27	42
13:00 13:15	5	10	15	10	9	19	34
13:15 13:30	5	14	19	4	16	20	39
12:30 13:30	27	51	78	42	73	115	193
15:00 15:15	13	12	25	13	14	27	52
15:15 15:30	8	12	20	15	12	27	47
15:30 15:45	9	13	22	21	18	39	61
15:45 16:00	15	10	25	5	19	24	49
15:00 16:00	45	47	92	54	63	117	209
16:00 16:15	17	15	32	14	29	43	75
16:15 16:30	10	8	18	18	20	38	56
16:30 16:45	13	10	23	19	23	42	65
16:45 17:00	15	14	29	19	36	55	84
16:00 17:00	55	47	102	70	108	178	280
17:00 17:15	14	18	32	19	34	53	85
17:15 17:30	25	9	34	16	33	49	83
17:30 17:45	13	11	24	22	30	52	76
17:45 18:00	13	9	22	15	46	61	83
17:00 18:00	65	47	112	72	143	215	327
Total	278	363	641	475	674	1149	1790

Comment:

2019-Jul-04 Page 1 of 1



Transportation Services - Traffic Services

Work Order 36785

Turning Movement Count - 15 Min U-Turn Total Report

GILMOUR ST @ O'CONNOR ST

Survey Date:	Τι	iesday, March 21	, 2017		01(01	
Time Pe	riod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Total	l	0	0	0	0	0

Appendix C

Synchro Intersection Worksheets – Existing Conditions



Lanes, Volumes, Timings 1: Bank & Somerset

Existing 311 Somerset St W

	•	\rightarrow	*	•	—	1	1	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		4	7	*	1		ĵ,	f.	Τ
Traffic Volume (vph)	52	253	75	21	155	3	369	152	
Future Volume (vph)	52	253	75	21	155	3	369	152	
Lane Group Flow (vph)	0	339	83	23	191	0	465	178	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	144	144	144	126	126	195	195	294	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.67	0.25	0.11	0.35		0.54	0.20	
Control Delay		29.7	21.1	9.2	9.9		14.7	10.1	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		29.7	21.1	9.2	9.9		14.7	10.1	
LOS		С	С	Α	Α		В	В	
Approach Delay		28.0			9.8		14.7	10.1	
Approach LOS		С			Α		В	В	
Queue Length 50th (m)		40.8	8.5	1.0	8.7		40.5	12.4	
Queue Length 95th (m)		68.3	18.9	m2.6	15.0		65.6	22.4	
Internal Link Dist (m)		161.3			160.8		255.6	215.8	
Turn Bay Length (m)			25.0	10.0					
Base Capacity (vph)		504	329	202	547		860	888	
Starvation Cap Reductn		0	0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.67	0.25	0.11	0.35		0.54	0.20	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 46 (61%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

10-27-2020 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.67 Intersection Signal Delay: 17.6 Intersection Capacity Utilization 74.4% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



Lanes, Volumes, Timings 2: O'Connor & Somerset

Existing 311 Somerset St W

	-	1	-	↓
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	<u> </u>		4	473
Traffic Volume (vph)	185	51	174	480
Future Volume (vph)	185	51	174	480
Lane Group Flow (vph)	335	0	250	635
Turn Type	NA	Perm	NA	NA
Protected Phases	4	1 01111	8	6
Permitted Phases	7	8	0	0
Detector Phase	4	8	8	6
Switch Phase	4	0	0	0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	37.0	37.0	37.0	38.0
Total Split (%)	49.3%	49.3%	49.3%	50.7%
Maximum Green (s)	31.5	31.5	31.5	32.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
	102	83	83	119
Pedestrian Calls (#/hr)		83		
Act Effct Green (s)	31.5		31.5	32.6
Actuated g/C Ratio	0.42		0.42	0.43
v/c Ratio	0.49		0.41	0.46
Control Delay	18.6		24.9	15.7
Queue Delay	0.0		0.0	0.0
Total Delay	18.6		24.9	15.7
LOS	В		С	В
Approach Delay	18.6		24.9	15.7
Approach LOS	В		С	В
Queue Length 50th (m)	21.5		30.7	30.8
Queue Length 95th (m)	46.9		m49.1	44.2
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)	100.0		100.1	170.7
Base Capacity (vph)	681		617	1376
	001		017	0
Starvation Cap Reductn				0
Spillback Cap Reductn	0		0	-
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.49		0.41	0.46
Intersection Summary				

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 44 (59%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 45

 10-27-2020
 CGH Transportation

 JK
 Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 18.4

Intersection LOS: B

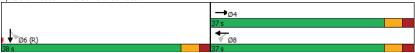
Intersection Capacity Utilization 67.3%

ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: O'Connor & Somerset



 10-27-2020
 CGH Transportation

 JK
 Page 4

Lanes, Volumes, Timings 3: Metcalfe & Somerset

Existing 311 Somerset St W

	*	-	—	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	<u> </u>	414
Traffic Volume (vph)	109	135	161	1033
Future Volume (vph)	109	135	161	1033
Lane Group Flow (vph)	0	271	360	1383
Turn Type	Perm	NA	NA	NA
Protected Phases	r ciiii	2	6	4
Permitted Phases	2	2	U	4
Detector Phase	2	2	6	4
Switch Phase	2	2	0	4
	10.0	10.0	10.0	10.0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	141	141	126	267
Act Effct Green (s)		29.5	29.5	34.8
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.71	0.63	0.69
Control Delay		24.6	23.9	17.8
Queue Delay		0.0	0.0	0.0
Total Delay		24.6	23.9	17.8
LOS		24.0 C	23.9 C	17.0 B
		24.6	23.9	17.8
Approach Delay				
Approach LOS		C	C	B
Queue Length 50th (m)		24.7	39.0	52.7
Queue Length 95th (m)		#68.9	66.3	67.9
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)				400
Base Capacity (vph)		382	573	1991
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.71	0.63	0.69
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				

Offset: 29 (39%), Referenced to phase 2:EBTL, Start of Green

Natural Cycle: 45

10-27-2020 CGH Transportation Page 5 JK

Lanes, Volumes, Timings 3: Metcalfe & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.71 Intersection Signal Delay: 19.8 Intersection Capacity Utilization 79.0% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Metcalfe & Somerset **↑** Ø4 **←** Ø6

Lanes, Volumes, Timings
4: O'Connor & Gilmour

Existing 311 Somerset St W

	-	↓
Lane Group	EBT	SBT
Lane Configurations	1 2	414
Traffic Volume (vph)	61	622
Future Volume (vph)	61	622
Lane Group Flow (vph)	122	745
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase	-	U
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag	0.0	J. I
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max 7.0	C-Max 16.0
Walk Time (s)		
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	47	73
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.33	0.35
Control Delay	19.0	3.7
Queue Delay	0.0	0.0
Total Delay	19.0	3.7
LOS	В	Α
Approach Delay	19.0	3.7
Approach LOS	В	Α
Queue Length 50th (m)	8.8	9.3
Queue Length 95th (m)	22.3	15.5
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)		
Base Capacity (vph)	368	2145
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.33	0.35
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 7:	5	
Offset: 46 (61%) Referen		2: and 6

Offset: 46 (61%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 50

 10-27-2020
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

Existing 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.35

Intersection Signal Delay: 5.9
 Intersection Capacity Utilization 44.4%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



 10-27-2020
 CGH Transportation

 JK
 Page 8

Lanes, Volumes, Timings 1: Bank & Somerset

Existing 311 Somerset St W

	<i>></i>	-	*	•	+	4	†	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		ની	7	ሻ	4		ĵ,	f.	_
Traffic Volume (vph)	34	274	103	59	254	9	260	331	
Future Volume (vph)	34	274	103	59	254	9	260	331	
Lane Group Flow (vph)	0	342	114	66	286	0	332	404	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	77	77	77	206	206	256	256	500	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.66	0.29	0.31	0.51		0.39	0.47	
Control Delay		28.9	21.3	15.7	16.4		12.4	13.5	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		28.9	21.3	15.7	16.4		12.4	13.5	
LOS		C	C C	В	В		В	В	
Approach Delay		27.0			16.2		12.4	13.5	
Approach LOS		C			В.		В.	В	
Queue Length 50th (m)		41.0	11.8	4.0	21.5		26.1	33.5	
Queue Length 95th (m)		68.2	24.2	m6.6	m34.3		43.6	54.7	
Internal Link Dist (m)		161.3			160.8		255.6	215.8	
Turn Bay Length (m)		101.0	25.0	10.0	100.0		200.0	210.0	
Base Capacity (vph)		519	394	216	565		842	856	
Starvation Cap Reductn		0.0	0	0	0		0.2	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.66	0.29	0.31	0.51		0.39	0.47	
		0.00	0.20	0.01	0.01		0.00	V11	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

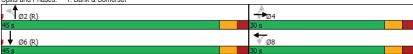
Offset: 71 (95%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

10-27-2020 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.66 Intersection Signal Delay: 17.9
Intersection Capacity Utilization 72.6%
Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



Lanes, Volumes, Timings 2: O'Connor & Somerset

Existing 311 Somerset St W

	→	1	+	Ţ
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	4		4	413
Traffic Volume (vph)	192	70	181	938
Future Volume (vph)	192	70	181	938
Lane Group Flow (vph)	390	0	279	1202
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8		
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	33.0	33.0	33.0	42.0
Total Split (%)	44.0%	44.0%	44.0%	56.0%
Maximum Green (s)	27.5	27.5	27.5	36.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0	2.2	0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag	5.5		5.5	5.4
Lead/Lag Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	179	138	138	147
Act Effct Green (s)	27.5		27.5	36.6
Actuated g/C Ratio	0.37		0.37	0.49
v/c Ratio	0.72		0.68	0.78
Control Delay	45.8		22.8	20.1
Queue Delay	0.0		0.0	0.0
Total Delay	45.8		22.8	20.1
LOS	D		С	С
Approach Delay	45.8		22.8	20.1
Approach LOS	D		С	С
Queue Length 50th (m)	56.3		21.0	68.9
Queue Length 95th (m)	#85.7		#34.5	94.2
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)				
Base Capacity (vph)	544		413	1536
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.72		0.68	0.78
	0.72		0.00	0.70
Intersection Summary				

Cycle Length: 75
Actuated Cycle Length: 75
Offset: 58 (77%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 55

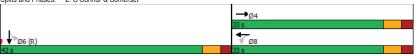
10-27-2020 **CGH Transportation** JK Page 3 Lanes, Volumes, Timings 2: O'Connor & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.78 Intersection Signal Delay: 25.9 Intersection Capacity Utilization 88.2% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service E # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: O'Connor & Somerset



Lanes, Volumes, Timings 3: Metcalfe & Somerset

Existing 311 Somerset St W

	*	-	•	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations	LUL	4	<u> </u>	414
Traffic Volume (vph)	87	185	148	590
Future Volume (vph)	87	185	148	590
Lane Group Flow (vph)	0	303	282	923
Turn Type	Perm	NA	NA	NA
Protected Phases	1 01111	2	6	4
Permitted Phases	2		0	4
Detector Phase	2	2	6	4
Switch Phase			0	4
	10.0	10.0	10.0	10.0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)		35.0		40.0
Total Split (s)	35.0		35.0	
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	162	162	213	350
Act Effct Green (s)		29.5	29.5	34.8
Actuated q/C Ratio		0.39	0.39	0.46
v/c Ratio		0.60	0.49	0.50
Control Delay		22.4	19.5	13.5
Queue Delay		0.0	0.0	0.0
Total Delay		22.4	19.5	13.5
LOS		C	В	В
Approach Delay		22.4	19.5	13.5
Approach LOS		22.4 C	13.3 B	13.3 B
Queue Length 50th (m)		25.0	26.9	28.1
Queue Length 95th (m)		m44.4	48.1	38.6
		155.7	145.3	134.2
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)		505	F70	4040
Base Capacity (vph)		505	572	1843
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.60	0.49	0.50
Intersection Summary				
Cycle Length: 75				

Offset: 20 (27%), Referenced to phase 2:EBTL, Start of Green

Natural Cycle: 40

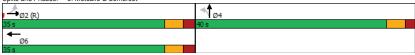
10-27-2020 CGH Transportation Page 5 JK

Lanes, Volumes, Timings 3: Metcalfe & Somerset

Existing 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.60 Intersection Signal Delay: 16.4 Intersection Capacity Utilization 68.7% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Metcalfe & Somerset



Lanes, Volumes, Timings
4: O'Connor & Gilmour

Existing 311 Somerset St W

	\rightarrow	ţ
Lane Group	EBT	SBT
Lane Configurations	1	414
Traffic Volume (vph)	84	1190
Future Volume (vph)	84	1190
Lane Group Flow (vph)	192	1375
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag	0.0	0.1
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	55	108
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.53	0.64
Control Delay	25.1	5.4
Queue Delay	0.0	0.4
Total Delay	25.1	5.8
LOS	25.1 C	3.6 A
Approach Delay	25.1	5.8
Approach LOS	25.1 C	3.6 A
Queue Length 50th (m)	17.2	13.7
Queue Length 95th (m)	36.3	24.3
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)	121.1	143.0
	360	2154
Base Capacity (vph)		
Starvation Cap Reductn	0	303
Spillback Cap Reductn	-	-
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.53	0.74
Intersection Summary		
Cycle Length: 75	_	
Actuated Cycle Length: 75		
Offset: 71 (95%), Referen Natural Cycle: 55	ced to phase	e 2: and 6

 10-27-2020
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

Existing 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 8.2
Intersection Capacity Utilization 61.0%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



 10-27-2020
 CGH Transportation

 JK
 Page 8

Appendix D

Collision Data



Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition	# Vehicles	# Motorcycles	# Bicycles	# Pedestrians
2016-01-22	2016	19:41	COOPER ST @ O'CONNOR ST (0006954)	02 - Rain	07 - Dark	02 - Stop sign	01 - Functioning	03 - P.D. only	04 - Sideswipe	02 - Wet	2	0	0	0
2017-11-23	2017	21:47	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	07 - Dark	02 - Stop sign	01 - Functioning	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2017-06-21	2017	16:13	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	02 - Non-fatal injury	05 - Turning movement	01 - Dry	2	0	1	0
2017-06-22	2017	10:58	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	01 - Functioning	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2017-07-23	2017	2:54	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	07 - Dark	02 - Stop sign	00 - Unknown	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2018-05-17	2018	17:46	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	02 - Non-fatal injury	05 - Turning movement	01 - Dry	2	0	1	0
2018-09-18	2018	11:40	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	01 - Functioning	03 - P.D. only	02 - Angle	01 - Dry	2	0	1	0
2019-12-16	2019	14:00	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	01 - Functioning	03 - P.D. only	05 - Turning movement	04 - Slush	2	0	0	0
2019-04-03	2019	15:10	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	02 - Non-fatal injury	05 - Turning movement	01 - Dry	2	0	1	0
2019-06-25	2019	10:59	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	02 - Non-fatal injury	07 - SMV other	01 - Dry	1	0	0	1
2020-03-13	2020	11:32	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2020-11-10	2020	8:35	COOPER ST @ O'CONNOR ST (0006954)	01 - Clear	01 - Daylight	02 - Stop sign	00 - Unknown	03 - P.D. only	05 - Turning movement	01 - Dry	2	0	0	0
2016-12-04	2016	2:59	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	07 - Dark	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2016-02-11	2016	14:26	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	99 - Other	02 - Wet	2	0	0	0
2016-03-14	2016	17:28	O'CONNOR ST @ SOMERSET ST (0002690)	02 - Rain	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	02 - Angle	02 - Wet	2	0	0	0
2016-05-02	2016	12:36	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	02 - Angle	01 - Dry	2	o o	1	0
2016-01-14	2016	10:10	O'CONNOR ST @ SOMERSET ST (0002690)	03 - Snow	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	03 - Loose snow	2	0	0	0
2016-08-17	2016	11:32	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2016-08-25	2016	17:15	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	01 - Dry	,	0	0	0
2017-03-09	2017	15:18	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	05 - Turning movement	01 - Dry	2	ō	1	0
2017-05-05	2017	18:31	O'CONNOR ST @ SOMERSET ST (0002690)	02 - Rain	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	05 - Turning movement	02 - Wet	2	0	0	0
2017-06-17	2017	0:12	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	07 - Dark	01 - Traffic signal	01 - Functioning	03 - P.D. only	07 - SMV other	01 - Dry	1	0	0	0
2017-09-14	2017	23:00	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	07 - Dark	01 - Traffic signal	00 - Unknown	03 - P.D. only	05 - Turning movement	01 - Dry	2			
2018-09-26	2018	17:25	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2018-12-19	2018	11:41	O'CONNOR ST @ SOMERSET ST (0002690)	03 - Snow	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	03 - Loose snow	2			0
2018-08-31	2018	16:36	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	03 - Rear end	01 - Dry	2	0	0	0
2019-11-23	2019	19:00	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	07 - Dark	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury	07 - SMV other	01 - Dry		0	0	1
2019-03-03	2019	13:30	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	05 - Turning movement	01 - Dry	2	0	0	0
2019-04-06	2019	12:46	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	07 - SMV other	01 - Dry		0	0	
2019-04-00	2019	13:00	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	01 - Daylight	01 - Traffic signal	01 - Functioning	03 - P.D. only	04 - Sideswipe	03 - Loose snow		0	0	0
2019-06-20	2019	21:00	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear	05 - Dusk	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury		01 - Dry	2	0		0
2020-01-25	2019	18:07	O'CONNOR ST @ SOMERSET ST (0002690)	03 - Snow	07 - Dusk	01 - Traffic signal	01 - Functioning	02 - Non-fatal injury 02 - Non-fatal injury	05 - Turning movement 07 - SMV other	01 - Dry 04 - Slush	2	0	0	1
2020-01-23	2020	10:54	O'CONNOR ST @ SOMERSET ST (0002690)					02 - Non-fatal injury	07 - SMV other			0	0	1
2020-01-09	2020			01 - Clear	01 - Daylight 07 - Dark	01 - Traffic signal	01 - Functioning	02 - Non-ratal Injury 03 - P.D. only		01 - Dry	1	0	0	0
		21:00	O'CONNOR ST @ SOMERSET ST (0002690)	01 - Clear		01 - Traffic signal	01 - Functioning		04 - Sideswipe	01 - Dry	2			0
2016-02-25	2016	15:34	SOMERSET ST W blwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	02 - Angle	02 - Wet	2	0	0	0
2016-04-12	2016	16:38	SOMERSET ST W btwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	01 - Daylight	10 - No control	0	02 - Non-fatal injury	07 - SMV other	01 - Dry	1	0	U	1
2019-10-29	2019	16:15	SOMERSET ST W btwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	01 - Daylight	10 - No control	0	02 - Non-fatal injury	04 - Sideswipe	01 - Dry	2	0	1	0
2019-06-06	2019	21:13	SOMERSET ST W btwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	05 - Dusk	10 - No control	0	02 - Non-fatal injury	02 - Angle	01 - Dry	2	0	U	0
2020-02-25	2020	Unknown	SOMERSET ST W btwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	00 - Unknown	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2020-05-29	2020	11:05	SOMERSET ST W btwn O'CONNOR ST & METCALFE ST (3ZBOEW)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	U	0
2016-10-07	2016	17:06	SOMERSET ST W btwn BANK ST & O'CONNOR ST (_3ZA3AV)	01 - Clear	01 - Daylight	10 - No control	0	02 - Non-fatal injury	07 - SMV other	01 - Dry	1	0	0	1
2016-12-07	2016	20:26	SOMERSET ST W btwn BANK ST & O'CONNOR ST (_3ZA3AV)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2016-03-05	2016	1:57	SOMERSET ST W btwn BANK ST & O'CONNOR ST (_3ZA3AV)	01 - Clear	07 - Dark	10 - No control	0	02 - Non-fatal injury	01 - Approaching	01 - Dry	2	0	0	0
2019-02-01	2019	Unknown	SOMERSET ST W btwn BANK ST & O'CONNOR ST (_3ZA3AV)	01 - Clear	00 - Unknown	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2019-03-26	2019	20:00	SOMERSET ST W btwn BANK ST & O'CONNOR ST (_3ZA3AV)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	U	0
2016-10-21	2016	12:04	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	02 - Rain	01 - Daylight	10 - No control	0	03 - P.D. only	02 - Angle	02 - Wet	2	0	0	0
2016-01-03	2016	21:29	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (_3ZA34V)	01 - Clear	07 - Dark	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2016-09-09	2016	13:18	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2017-11-09	2017	12:10	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (_3ZA34V)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2018-12-10	2018	11:45	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	01 - Daylight	10 - No control	0	03 - P.D. only	04 - Sideswipe	01 - Dry	2	0	0	0
2018-12-13	2018	16:30	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	05 - Dusk	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2019-04-15	2019	Unknown	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	00 - Unknown	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2019-05-18	2019	Unknown	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	00 - Unknown		0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2019-05-18	2019	Unknown	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	00 - Unknown		0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2020-03-02	2020	Unknown	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	00 - Unknown		0	03 - P.D. only	06 - SMV unattended vehicle		1	0	0	0
2020-12-23	2020	Unknown	O'CONNOR ST btwn COOPER ST & SOMERSET ST W (3ZA34V)	01 - Clear	00 - Unknown	10 - No control	0	03 - P.D. only	06 - SMV unattended vehicle	e 01 - Dry	1	0	0	0

Appendix E

TRANS Model Plots



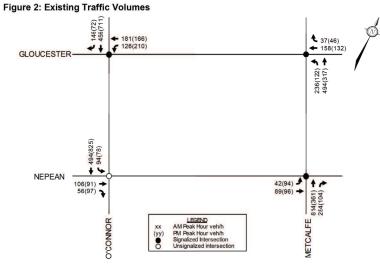




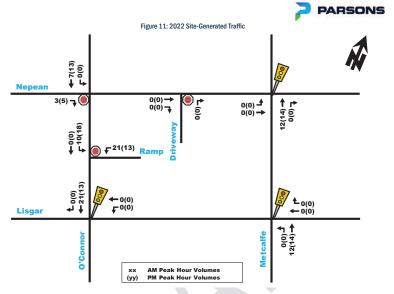
Appendix F

Background Development Volumes





Novatech Page 7



3.2. Background Network Traffic

3.2.1. Transportation network plans

Refer to Section 2.1.3: Planned Study Area Transportation Network Changes.

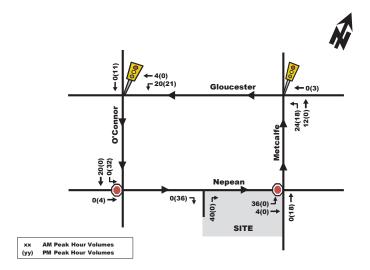
3.2.2. Background Growth

Given that the proposed development will be located in the well-developed core downtown area of the city of Ottawa, traffic along study area roadways is not anticipated to increase drastically within the future horizon years. Also, since the development is located in a TOD area, within 600m of LRT's Parliament Station, transit usage is expected to continuously increase, and auto usage is expected to decrease. Major other area developments within the study area are accounted for in Section 3.2.3. Nonetheless, a background growth rate of 1% has been applied to the study area roadways to account for trips that may be generated by future other area developments that are minor or located outside the scope of the study area. Figure 12 provides the future background traffic at 2022 and Figure 13 provides the future background traffic at 2027. Note that a traffic signal is anticipated to be constructed at the intersection of O'Connor/Nepean prior to the construction of the proposed development and the EBT movement is assumed to be reinstated as a result.

16

O'Connor - Nepean Development - Strategy Report

180 Metcalfe Street Apartment Development TIA Strategy Report



PARSONS Page | 11

PARSONS

Figure 7: Phase I 'New' Site Generated Traffic

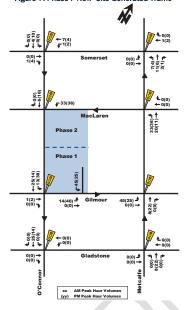
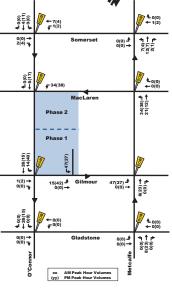
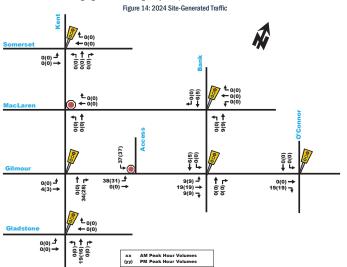


Figure 8: Phase II 'New' Site Generated Traffic





 Departing traffic is assumed to use Hwy 417 WB primarily by travelling east on Gilmour St away from the site driveway and turning right on O'Connor St to travel southbound to Catherine St, then turning right onto the highway ramp.

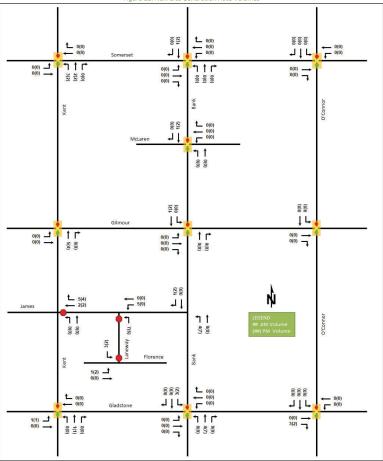


Based on the site-generated vehicle trips of the existing office building (provided in Table 4), study area traffic volumes are expected to decrease as shown in Figure 15. A similar trip distribution and assignment has been assumed for the existing office building's vehicle trips as the proposed residential development. This reduction in traffic volumes will be applied to the total projected traffic volumes for horizon years 2024 and 2029.

359 Kent Street - Strategy Report 19

390-394 Bank Street Transportation Impact Assessment

Figure 11: New Site Generation Auto Volumes





Appendix G

Synchro Intersection Worksheets – 2024 Future Background Conditions



Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

Carne Configurations Carnet Colume Col		•	-	*	1	—	1	†	Į.	
Traffic Volume (vph) 52 254 75 21 162 3 378 159 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Traffic Volume (vph) 52 254 75 21 162 3 378 159 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Lane Configurations		4	7	*	ĵ.		44	ĵ.	_
Carne Group Flow (vph)	Traffic Volume (vph)	52		75	21		3			
Turn Type	Future Volume (vph)	52	254	75	21	162	3	378	159	
Turn Type Perm NA Perm Perm NA Perm NA Perm NA Perm NA Perm NA NA NA NA Perm Todected Phases 4 4 4 8 2 2 6 Petiector Phase 4 4 4 8 8 2 2 6 Witch Phase 4 4 4 8 8 2 2 6 Minimum Initial (s) 10.0 <	Lane Group Flow (vph)	0	306	75	21	179	0	428	167	
Permitted Phases	Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Detector Phase 4	Protected Phases		4			8		2	6	
Switch Phase Amount of the properties of the	Permitted Phases	4		4	8		2			
Minimum Initial (s)	Detector Phase	4	4	4	8	8	2	2	6	
Affinimum Split (s) 25.5 39.5 </td <td>Switch Phase</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Switch Phase									
Total Split (s) 30.0 30.0 30.0 30.0 30.0 45.0 45.0 45.0	Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
fotal Split (%) 40.0% 40.0% 40.0% 40.0% 40.0% 40.0% 60.0% 60.0% 60.0% Alaximum Green (s) 24.5	Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Maximum Green (s) 24.5 24.5 24.5 24.5 24.5 24.5 39.5 33.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.0 0.0 <td>Total Split (s)</td> <td>30.0</td> <td>30.0</td> <td>30.0</td> <td>30.0</td> <td>30.0</td> <td>45.0</td> <td>45.0</td> <td>45.0</td> <td></td>	Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Vellow Time (s) 3.3	Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
All-Red Time (s)	Maximum Green (s)	24.5	24.5	24.5	24.5		39.5	39.5		
cost Time Adjust (s) 0.0	Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
Total Lost Time (s) 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5	All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Read/Lag Optimize? Pead Care	Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Lead-Lag Optimize? Vehicle Extension (s) 3.0 7.0	Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Vehicle Extension (s) 3.0 7.0	Lead/Lag									
Recall Mode Max Max Max Max Max Max C-Max C-Max C-Max	Lead-Lag Optimize?									
Valik Time (s) 7.0	Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Flash Dont Walk (s) 13.0 </td <td>Recall Mode</td> <td>Max</td> <td>Max</td> <td>Max</td> <td>Max</td> <td>Max</td> <td>C-Max</td> <td>C-Max</td> <td>C-Max</td> <td></td>	Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
December of Pedestrian Calls (#/hr) 144 144 144 126 126 195 195 294 Act Effic Green (s) 24.5 24.5 24.5 24.5 24.5 39.5 39.5 39.5 39.5 39.5 39.5 39.5 39.5 39.5 39.5 30.53 0.53 0.53 0.53 0.53 0.53 0.50 0.19 0.0 0.0 0.33 0.50 0.19 0.0 <	Walk Time (s)									
ket Effet Green (s) 24.5 24.5 24.5 24.5 39.5 39.5 Actuated g/C Ratio 0.33 0.33 0.33 0.33 0.53 0.53 /c Ratio 0.60 0.23 0.10 0.33 0.53 0.53 Obontrol Delay 27.2 20.7 8.7 9.5 13.9 10.0 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 20.7 8.7 9.5 13.9 10.0 0.0	Flash Dont Walk (s)	13.0	13.0	13.0		13.0	13.0	13.0		
Actuated g/C Ratio 0.33 0.33 0.33 0.33 0.53 0.53 0.53 0.53	Pedestrian Calls (#/hr)	144					195	195		
Recommendation 0.60 0.23 0.10 0.33 0.50 0.19 Control Delay 27.2 20.7 8.7 9.5 13.9 10.0 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Octal Delay 27.2 20.7 8.7 9.5 13.9 10.0 OS C C A A B B Approach LoS C C A A B B Queue Length 50th (m) 35.9 7.6 0.9 8.1 36.1 11.6 Queue Length 95th (m) 60.6 17.5 m2.4 13.7 58.8 21.2 Tetral Link Dist (m) 161.3 160.8 255.6 215.8 Turn Bay Length (m) 25.0 10.0 Starvation Cap Reductn 0 0 0 0 Sibillback Cap Reductn 0 0 0 0 0 Sibrage Cap Reductn 0 0 0<	Act Effct Green (s)		24.5	24.5	24.5			39.5	39.5	
27.2 20.7 8.7 9.5 13.9 10.0	Actuated g/C Ratio				0.33			0.53	0.53	
Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 27.2 20.7 8.7 9.5 13.9 10.0 Osc C C A A B B Approach Delay 26.0 9.4 13.9 10.0 Approach LOS C A B B Dueue Length 50th (m) 35.9 7.6 0.9 8.1 36.1 11.6 Dueue Length 95th (m) 60.6 17.5 m2.4 13.7 58.8 21.2 Internal Link Dist (m) 161.3 160.8 255.6 215.8 Turn Bay Length (m) 25.0 10.0 Base Capacity (vph) 506 329 221 548 860 890 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0	v/c Ratio									
27.2 20.7 8.7 9.5 13.9 10.0 OS C C A A B B Approach Delay 26.0 9.4 13.9 10.0 Approach LOS C A A B B Approach LOS C A B B Approach LOS A B B A B A B B A B B A B B A B B A B B A B B A B B A B B A B B A B B A B B A B B A B B A B A B B A B B A B B A B B A B B A B B A B B A B B A B B	Control Delay				8.7			13.9		
COS C	Queue Delay									
Approach Delay 26.0 9.4 13.9 10.0 Approach LOS C A B B B Daueue Length 50th (m) 35.9 7.6 0.9 8.1 36.1 11.6 Queue Length 95th (m) 60.6 17.5 m2.4 13.7 58.8 21.2 (m) Thermal Link Dist (m) 161.3 160.8 255.6 215.8 (m) Bay Length (m) 25.0 10.0 (m) 25.0 10.0 (m) 25.0 (m)	Total Delay									
Deproach LOS	LOS			С	Α					
Augueue Length 50th (m) 35.9 7.6 0.9 8.1 36.1 11.6 Jueue Length 95th (m) 60.6 17.5 m2.4 13.7 58.8 21.2 nternal Link Dist (m) 161.3 160.8 255.6 215.8 rum Bay Length (m) 25.0 10.0 329 221 548 860 890 starvation Cap Reductn 0 0 0 0 0 0 0 slillback Cap Reductn 0 0 0 0 0 0 0 storage Cap Reductn 0 0 0 0 0 0 0	Approach Delay									
Queue Length 95th (m) 60.6 17.5 m2.4 13.7 58.8 21.2 Internal Link Dist (m) 161.3 160.8 255.6 215.8 Turn Bay Length (m) 25.0 10.0 325.0	Approach LOS							В		
nternal Link Dist (m) 161.3 160.8 255.6 215.8 Turn Bay Length (m) 25.0 10.0 Jase Capacity (vph) 506 329 221 548 860 890 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Queue Length 50th (m)							36.1		
Turn Bay Length (m) 25.0 10.0 Jase Capacity (vph) 506 329 221 548 860 890 Starvation Cap Reductn 0 0 0 0 0 0 0 Sibiliback Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0	Queue Length 95th (m)			17.5	m2.4					
Base Capacity (vph) 506 329 221 548 860 890 Starvation Cap Reductn 0	Internal Link Dist (m)		161.3			160.8		255.6	215.8	
Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0	Turn Bay Length (m)									
Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0	Base Capacity (vph)									
Storage Cap Reductn 0 0 0 0 0 0	Starvation Cap Reductn									
	Spillback Cap Reductn					-		-		
Reduced v/c Ratio 0.60 0.23 0.10 0.33 0.50 0.19	Storage Cap Reductn									
	Reduced v/c Ratio		0.60	0.23	0.10	0.33		0.50	0.19	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

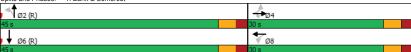
Offset: 46 (61%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

05/24/2022 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.60 Intersection Signal Delay: 16.5 Intersection Capacity Utilization 75.0% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

	-	1	-	Ţ
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	1		4	473
Traffic Volume (vph)	185	52	181	555
Future Volume (vph)	185	52	181	555
Lane Group Flow (vph)	302	0	233	647
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8		
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	37.0	37.0	37.0	38.0
Total Split (%)	49.3%	49.3%	49.3%	50.7%
Maximum Green (s)	31.5	31.5	31.5	32.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	102	83	83	119
Act Effct Green (s)	31.5		31.5	32.6
Actuated g/C Ratio	0.42		0.42	0.43
v/c Ratio	0.44		0.37	0.47
Control Delay	16.3		24.2	15.9
Queue Delay	0.0		0.0	0.0
Total Delay	16.3		24.2	15.9
LOS	В		С	В
Approach Delay	16.3		24.2	15.9
Approach LOS	В		С	В
Queue Length 50th (m)	14.3		27.7	31.7
Queue Length 95th (m)	40.0		m45.8	45.3
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)				
Base Capacity (vph)	681		627	1384
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.44		0.37	0.47
Intersection Summary				

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 44 (59%), Referenced to phase 2: and 6:SBTL, Start of Green

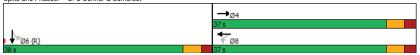
Natural Cycle: 45

05/24/2022 **CGH Transportation** JK Page 3 Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.47 Intersection Signal Delay: 17.7 Intersection Capacity Utilization 70.0% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: O'Connor & Somerset



Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

	•	\rightarrow	-	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	1	414
Traffic Volume (vph)	109	135	162	4 T P
	109	135	162	1117
Future Volume (vph)	0		325	1337
Lane Group Flow (vph)	-	244		
Turn Type	Perm	NA	NA	NA
Protected Phases		2	6	4
Permitted Phases	2			
Detector Phase	2	2	6	4
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)	2.2	0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	141	141	126	267
Act Effct Green (s)		29.5	29.5	34.8
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.58	0.57	0.67
Control Delay		17.8	22.0	17.3
Queue Delay		0.0	0.0	0.0
Total Delay		17.8	22.0	17.3
LOS		17.0 B	22.0 C	17.3 B
		17.8	22.0	17.3
Approach Delay				
Approach LOS		В	C	В
Queue Length 50th (m)		20.3	34.0	50.1
Queue Length 95th (m)		41.6	58.3	64.8
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)				
Base Capacity (vph)		423	573	1997
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.58	0.57	0.67
Internation Owner				
Intersection Summary				
Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75				

Offset: 29 (39%), Referenced to phase 2:EBTL, Start of Green

Natural Cycle: 40

 05/24/2022
 CGH Transportation

 JK
 Page 5

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Background AM Peak Hour 311 Somerset St W

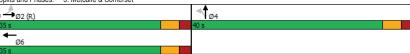
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 18.2
Intersection Capacity Utilization 81.0%

Analysis Period (min) 15

Splits and Phases: 3: Metcalfe & Somerset



Lanes, Volumes, Timings 4: O'Connor & Gilmour

2024 Future Background AM Peak Hour 311 Somerset St W

	-	†
ane Group	EBT	SBT
ane Configurations	1	414
raffic Volume (vph)	62	718
uture Volume (vph)	62	718
ane Group Flow (vph)	130	780
urn Type	NA	NA
rotected Phases	4	6
ermitted Phases		
etector Phase	4	6
witch Phase		
finimum Initial (s)	10.0	10.0
linimum Split (s)	20.6	26.1
otal Split (s)	21.0	54.0
otal Split (%)	28.0%	72.0%
laximum Green (s)	15.4	48.9
ellow Time (s)	3.3	3.3
II-Red Time (s)	2.3	1.8
ost Time Adjust (s)	0.0	0.0
otal Lost Time (s)	5.6	5.1
ead/Lag	5.0	J. I
ead-Lag Optimize?		
ead-Lag Optimize? ehicle Extension (s)	3.0	3.0
enicle Extension (s)	Max	C-Max
Valk Time (s)	7.0	16.0
	8.0	5.0
lash Dont Walk (s)	47	
edestrian Calls (#/hr)		73 48.9
ct Effct Green (s)	15.4	
ctuated g/C Ratio	0.21	0.65
/c Ratio	0.34	0.36
ontrol Delay	16.6	3.3
lueue Delay	0.0	0.0
otal Delay	16.6	3.3
OS	В	Α
pproach Delay	16.6	3.3
pproach LOS	В	Α
(ueue Length 50th (m)	7.6	8.4
(ueue Length 95th (m)	21.4	14.3
nternal Link Dist (m)	127.1	143.6
urn Bay Length (m)		
ase Capacity (vph)	377	2143
tarvation Cap Reductn	0	0
pillback Cap Reductn	0	0
torage Cap Reductn	0	0
leduced v/c Ratio	0.34	0.36
ntersection Summary		
ycle Length: 75		
ctuated Cycle Length: 7	75	
offset: 46 (61%), Refere	nced to phase	2: and 6
latural Cycle: 50		

 05/24/2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2024 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.36
Intersection Signal Delay: 5.2
Intersection Capacity Utilization 47.6%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



Lanes, Volumes, Timings
1: Bank & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

	•	→	*	•	+	4	†	Į.	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		ની	7	ሻ	4		fa fa	f.	
Traffic Volume (vph)	34	278	103	59	258	9	269	338	
Future Volume (vph)	34	278	103	59	258	9	269	338	
Lane Group Flow (vph)	0	312	103	59	262	0	308	370	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	77	77	77	206	206	256	256	500	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.60	0.26	0.25	0.46		0.37	0.43	
Control Delay		26.9	20.9	14.6	16.0		12.0	12.9	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		26.9	20.9	14.6	16.0		12.0	12.9	
LOS		С	С	В	В		В	В	
Approach Delay		25.4			15.7		12.0	12.9	
Approach LOS		C			В		В	В	
Queue Length 50th (m)		36.5	10.6	3.5	19.1		23.8	29.9	
Queue Length 95th (m)		61.1	22.1	m6.4	m31.7		39.8	49.0	
Internal Link Dist (m)		161.3	0=0	400	160.8		255.6	215.8	
Turn Bay Length (m)		501	25.0	10.0	50-		0.40	0.55	
Base Capacity (vph)		521	394	236	565		843	857	
Starvation Cap Reductn		0	0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.60	0.26	0.25	0.46		0.37	0.43	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 71 (95%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 55

 05/24/2022
 CGH Transportation

 JK
 Page 1

Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.60
Intersection Signal Delay: 17.0
Intersection LOS: B
Intersection Capacity Utilization 73.3%
ICU Level of Service D
Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

	-	1	—	Ţ
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	4		4	473
Traffic Volume (vph)	192	72	185	1023
Future Volume (vph)	192	72	185	1023
Lane Group Flow (vph)	355	0	257	1167
Turn Type	NA	Perm	NA	NA
Protected Phases	4	1 61111	8	6
Permitted Phases	7	8	0	0
Detector Phase	4	8	8	6
Switch Phase	4	- 0	- 0	- 0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	33.0	33.0	33.0	42.0
Total Split (%)	44.0%	44.0%	44.0%	56.0%
Maximum Green (s)	27.5	27.5	27.5	36.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	179	138	138	147
Act Effct Green (s)	27.5		27.5	36.6
Actuated g/C Ratio	0.37		0.37	0.49
v/c Ratio	0.65		0.57	0.76
Control Delay	43.9		18.7	19.3
Queue Delay	0.0		0.0	0.0
Total Delay	43.9		18.7	19.3
LOS	43.9 D		10.7 B	19.3 B
Approach Delay	43.9		18.7	19.3
	43.9 D		18.7 B	19.3 B
Approach LOS				
Queue Length 50th (m)	50.7		19.3	65.6
Queue Length 95th (m)	76.7		30.1	89.5
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)				
Base Capacity (vph)	543		454	1541
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.65		0.57	0.76
Intersection Summary				
Cycle Length: 75				

Cycle Length: 75
Actuated Cycle Length: 75
Offset: 58 (77%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 55

05/24/2022 CGH Transportation JK Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76 Intersection Signal Delay: 24.1 Intersection Capacity Utilization 91.3% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service F

Splits and Phases: 2: O'Connor & Somerset



Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

	•	-	—	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	1	414
Traffic Volume (vph)	87	185	150	665
Future Volume (vph)	87	185	150	665
Lane Group Flow (vph)	0	272	256	910
Turn Type	Perm	NA	NA	NA
Protected Phases	1 61111	2	6	4
Permitted Phases	2	2	U	4
Detector Phase	2	2	6	4
Switch Phase	2		U	4
	10.0	40.0	10.0	40.0
Minimum Initial (s)		10.0		10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	162	162	213	350
Act Effct Green (s)		29.5	29.5	34.8
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.52	0.45	0.49
Control Delay		19.8	18.4	13.6
Queue Delay		0.0	0.0	0.0
Total Delay		19.8	18.4	13.6
LOS		13.0 B	10.4 B	13.0 B
Approach Delay		19.8	18.4	13.6
Approach LOS		19.0 B	10.4 B	13.0 B
Queue Length 50th (m)		20.3	23.7	28.0
			42.7	38.3
Queue Length 95th (m)		m41.7		
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)		50.	570	4001
Base Capacity (vph)		524	573	1864
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.52	0.45	0.49
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Actuated Cycle Length: 75				

Actuated Cycle Length: 75
Offset: 20 (27%), Referenced to phase 2:EBTL, Start of Green

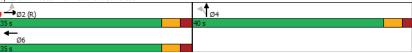
Natural Cycle: 40

05/24/2022 CGH Transportation JK Page 5 Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.52 Intersection Signal Delay: 15.6 Intersection Capacity Utilization 70.2% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Metcalfe & Somerset



Lanes, Volumes, Timings 4: O'Connor & Gilmour

Natural Cycle: 55

2024 Future Background PM Peak Hour 311 Somerset St W

	\rightarrow	¥
Lane Group	EBT	SBT
Lane Configurations	ą.	414
Traffic Volume (vph)	86	1279
Future Volume (vph)	86	1279
Lane Group Flow (vph)	194	1365
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	55	108
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.54	0.64
Control Delay	24.6	5.4
Queue Delay	0.0	0.4
Total Delay	24.6	5.8
LOS	С	Α
Approach Delay	24.6	5.8
Approach LOS	С	Α
Queue Length 50th (m)	16.8	12.5
Queue Length 95th (m)	36.0	19.9
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)		
Base Capacity (vph)	361	2142
Starvation Cap Reductn	0	285
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.54	0.74
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 71 (95%), Reference		2: and

 05/24/2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2024 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 8.1
Intersection Capacity Uffization 66.3%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



Appendix H

Synchro Intersection Worksheets – 2029 Future Background Conditions



Lanes, Volumes, Timings 1: Bank & Somerset

2029 Future Background AM Peak Hour 311 Somerset St W

	•	-	*	1	—	1	†	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		4	7	*	î,		4	î,	
Traffic Volume (vph)	52	256	75	21	169	3	378	159	
Future Volume (vph)	52	256	75	21	169	3	378	159	
Lane Group Flow (vph)	0	308	75	21	186	0	428	167	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	144	144	144	126	126	195	195	294	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.61	0.23	0.10	0.34		0.50	0.19	
Control Delay		27.4	20.7	8.6	9.6		13.9	10.0	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.4	20.7	8.6	9.6		13.9	10.0	
LOS		C	20.7 C	Α.	J.0		В	В	
Approach Delay		26.1	- 3	,,,	9.5		13.9	10.0	
Approach LOS		C			Α.		В	В	
Queue Length 50th (m)		36.1	7.6	0.9	8.2		36.1	11.6	
Queue Length 95th (m)		60.9	17.5	m2.3	14.0		58.8	21.2	
Internal Link Dist (m)		161.3	0	0	160.8		255.6	215.8	
Turn Bay Length (m)			25.0	10.0			200.0	2.0.0	
Base Capacity (vph)		506	329	220	549		860	890	
Starvation Cap Reductn		0	0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.61	0.23	0.10	0.34		0.50	0.19	
		0.01	0.20	00	0.01		0.00	00	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

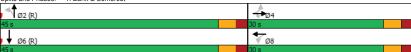
Offset: 46 (61%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

05/24/2022 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

2029 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.61 Intersection Signal Delay: 16.5 Intersection Capacity Utilization 75.1% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Background AM Peak Hour 311 Somerset St W

	→	•	←	1
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	1 ₃	WDL		100 414
Traffic Volume (vph)	185	53	વ 188	↔ → 559
	185	53	188	559
Future Volume (vph)	304	53		
Lane Group Flow (vph)		-	241	651
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8	_	_
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	37.0	37.0	37.0	38.0
Total Split (%)	49.3%	49.3%	49.3%	50.7%
Maximum Green (s)	31.5	31.5	31.5	32.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0	2.2	0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
	5.5		0.0	0.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	102	83	83	119
Act Effct Green (s)	31.5		31.5	32.6
Actuated g/C Ratio	0.42		0.42	0.43
v/c Ratio	0.45		0.38	0.47
Control Delay	16.5		23.8	16.0
Queue Delay	0.0		0.0	0.0
Total Delay	16.5		23.8	16.0
LOS	10.5 B		23.0 C	10.0 B
	16.5		23.8	16.0
Approach Delay				
Approach LOS	В		C	В
Queue Length 50th (m)	14.6		28.5	32.0
Queue Length 95th (m)	40.3		m46.2	45.5
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)				
Base Capacity (vph)	680		627	1384
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.45		0.38	0.47
	3.10		3.00	3.17

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 44 (59%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 45

05/24/2022 CGH Transportation JK Page 3 Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.47 Intersection Signal Delay: 17.7 Intersection Capacity Utilization 70.7% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: O'Connor & Somerset



Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Background AM Peak Hour 311 Somerset St W

	•	-	-	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations	LUL	4	7	414
Traffic Volume (vph)	109	135	163	1129
Future Volume (vph)	109	135	163	1129
Lane Group Flow (vph)	0	244	326	1358
Turn Type	Perm	NA	JZ6 NA	NA
Protected Phases	reim	NA 2	NA 6	NA 4
	_	2	6	4
Permitted Phases	2	^	^	
Detector Phase	2	2	6	4
Switch Phase	10.7	10.7	10 -	40.7
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	141	141	126	267
Act Effct Green (s)	141	29.5	29.5	34.8
		0.39	0.39	0.46
Actuated g/C Ratio				
v/c Ratio		0.58	0.57	0.68
Control Delay		17.9	22.1	17.5
Queue Delay		0.0	0.0	0.0
Total Delay		17.9	22.1	17.5
LOS		В	С	В
Approach Delay		17.9	22.1	17.5
Approach LOS		В	С	В
Queue Length 50th (m)		20.3	34.2	51.3
Queue Length 95th (m)		41.7	58.7	66.3
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)				
Base Capacity (vph)		422	574	1993
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.58	0.57	0.68
Neuroeu V/C Nalio		0.50	0.07	0.00
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 29 (39%), Reference		2:EBTI	Start of 0	Green
Natural Cycle: 40	ou to pridot	,U (L,	Clair Of C	3. 3011
Hatarai Oyolo. 70				

 05/24/2022
 CGH Transportation

 JK
 Page 5

Lanes, Volumes, Timings 3: Metcalfe & Somerset

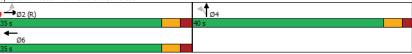
2029 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.68
Intersection Signal Delay: 18.3
Intersection Capacity Utilization 81.4%

Analysis Period (min) 15

Splits and Phases: 3: Metcalfe & Somerset



Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Background AM Peak Hour 311 Somerset St W

	-	¥
Lane Group	EBT	SBT
Lane Configurations	1>	414
Traffic Volume (vph)	63	744
Future Volume (vph)	63	744
Lane Group Flow (vph)	131	820
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	47	73
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.35	0.38
Control Delay	16.9	3.5
Queue Delay	0.0	0.0
Total Delay	16.9 B	3.5 A
LOS Approach Dolov	16.9	3.5
Approach Delay Approach LOS	16.9 B	3.5 A
Queue Length 50th (m)	7.8	9.5
Queue Length 95th (m)	21.6	15.8
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)	121.1	143.0
Base Capacity (vph)	377	2138
Starvation Cap Reductn	0	2130
Spillback Cap Reductin	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.35	0.38
	0.00	0.00
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		0 10
Offset: 46 (61%), Reference	ed to phase	2: and 6
Natural Cycle: 50		

 05/24/2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Background AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.38
Intersection Signal Delay: 5.4
Intersection Capacity Utilization 48.8%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



Lanes, Volumes, Timings
1: Bank & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

	•	-	*	•	+	4	†	Į.	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		ની	7	ሻ	4		fa fa	f.	_
Traffic Volume (vph)	34	282	103	59	262	9	269	338	
Future Volume (vph)	34	282	103	59	262	9	269	338	
Lane Group Flow (vph)	0	316	103	59	266	0	308	370	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	77	77	77	206	206	256	256	500	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.61	0.26	0.25	0.47		0.37	0.43	
Control Delay		27.1	20.9	14.5	15.8		12.0	12.9	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.1	20.9	14.5	15.8		12.0	12.9	
LOS		C	C	В	В		В	В	
Approach Delay		25.6			15.6		12.0	12.9	
Approach LOS		С			В		В	В	
Queue Length 50th (m)		37.0	10.6	3.5	19.1		23.8	29.9	
Queue Length 95th (m)		62.1	22.1	m6.0	m31.7		39.8	49.0	
Internal Link Dist (m)		161.3			160.8		255.6	215.8	
Turn Bay Length (m)			25.0	10.0					
Base Capacity (vph)		522	394	234	565		843	857	
Starvation Cap Reductn		0	0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.61	0.26	0.25	0.47		0.37	0.43	
				0				20	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 71 (95%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 55

 05/24/2022
 CGH Transportation

 JK
 Page 1

Lanes, Volumes, Timings
1: Bank & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 17.0

Intersection LOS: B

Intersection Capacity Utilization 73.5%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

	→	1	←	↓
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	<u> </u>		4	413
Traffic Volume (vph)	192	74	189	1034
Future Volume (vph)	192	74	189	1034
Lane Group Flow (vph)	359	0	263	1178
Turn Type	NA	Perm	NA	NA
Protected Phases	INA 4	Felill	NA 8	NA 6
Protected Phases Permitted Phases	4	8	ð	0
	4	8	8	6
Detector Phase	4	8	8	Ö
Switch Phase	40.0	40.0	40.0	40.0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	33.0	33.0	33.0	42.0
Total Split (%)	44.0%	44.0%	44.0%	56.0%
Maximum Green (s)	27.5	27.5	27.5	36.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	179	138	138	147
Act Effct Green (s)	27.5	130	27.5	36.6
	0.37		0.37	0.49
Actuated g/C Ratio				
v/c Ratio	0.66		0.59	0.76
Control Delay	44.3		19.5	19.5
Queue Delay	0.0		0.0	0.0
Total Delay	44.3		19.5	19.5
LOS	D		В	В
Approach Delay	44.3		19.5	19.5
Approach LOS	D		В	В
Queue Length 50th (m)	51.4		20.0	66.5
Queue Length 95th (m)	77.7		30.9	91.1
Internal Link Dist (m)	160.8		155.7	145.7
Turn Bay Length (m)				
Base Capacity (vph)	541		446	1542
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.66		0.59	0.76
	0.00		0.08	0.70
Intersection Summary				

Cycle Length: 75
Actuated Cycle Length: 75
Offset: 58 (77%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 55

05/24/2022 CGH Transportation JK Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76 Intersection Signal Delay: 24.5 Intersection Capacity Utilization 92.2% Analysis Period (min) 15 Intersection LOS: C ICU Level of Service F

Splits and Phases: 2: O'Connor & Somerset



Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

	•	-	-	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	<u> </u>	414
Traffic Volume (vph)	87	185	152	672
Future Volume (vph)	87	185	152	672
Lane Group Flow (vph)	0	272	258	922
Turn Type	Perm	NA	NA	NA
Protected Phases	r Cilli	2	6	4
Permitted Phases	2	2	U	4
Detector Phase	2	2	6	4
	2	2	0	4
Switch Phase	40.0	40.0	40.0	40.0
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	162	162	213	350
Act Effct Green (s)	102	29.5	29.5	34.8
()		0.39	0.39	0.46
Actuated g/C Ratio		0.59	0.39	0.46
v/c Ratio				
Control Delay		20.0	18.6	13.7
Queue Delay		0.0	0.0	0.0
Total Delay		20.0	18.6	13.7
LOS		В	В	В
Approach Delay		20.0	18.6	13.7
Approach LOS		В	В	В
Queue Length 50th (m)		20.6	24.0	28.6
Queue Length 95th (m)		m41.5	43.3	39.1
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)				
Base Capacity (vph)		524	573	1861
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.52	0.45	0.50
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75	,			
gui. 70				

Offset: 20 (27%), Referenced to phase 2:EBTL, Start of Green

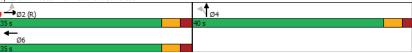
Natural Cycle: 40

05/24/2022 CGH Transportation JK Page 5 Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Background PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.52 Intersection Signal Delay: 15.8 Intersection Capacity Utilization 70.6% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Metcalfe & Somerset



Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Background PM Peak Hour 311 Somerset St W

	-	¥
Lane Group	EBT	SBT
Lane Configurations	f)	414
Traffic Volume (vph)	88	1294
Future Volume (vph)	88	1294
Lane Group Flow (vph)	196	1420
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	55	108
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.54	0.67
Control Delay	25.1	6.2
Queue Delay	0.0	0.4
Total Delay	25.1	6.6
LOS	С	Α
Approach Delay	25.1	6.6
Approach LOS	C	A
Queue Length 50th (m)	17.3	14.0
Queue Length 95th (m)	36.7	32.3
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)		1 10.0
Base Capacity (vph)	360	2129
Starvation Cap Reductn	0	269
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.54	0.76
	0.01	00
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 71 (95%), Referen	ced to phase	2: and 6

Natural Cycle: 60

 05/24/2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Background PM Peak Hour 311 Somerset St W

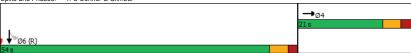
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 8.8 Intersection Capacity Uffization 68.1%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



Appendix I

MMLOS Analysis



Multi-Modal Level of Service - Segments Form

Consultant
Scenario
Comments

GH Transportation	Project
xisting and Future Conditions	Date

erset

SEGMENTS		Street A	O'Connor	Somerset	Section
		Olicel A	1	2	3
	Sidewalk Width Boulevard Width		≥ 2 m 0.5 - 2 m	≥ 2 m 0.5 - 2 m	
Pedestrian	Avg Daily Curb Lane Traffic Volume		> 3000	≤ 3000	
	Operating Speed On-Street Parking		> 50 to 60 km/h yes	> 50 to 60 km/h no	
est	Exposure to Traffic PLoS	С	С	Α	-
þ	Effective Sidewalk Width				
P	Pedestrian Volume				
	Crowding PLoS		Α	Α	-
	Level of Service		С	A	-
	Type of Cycling Facility		Physically Separated	Mixed Traffic	
	Number of Travel Lanes			2-3 lanes total	
	Operating Speed			≥ 50 to 60 km/h	
	# of Lanes & Operating Speed LoS		-	E	-
Bicycle	Bike Lane (+ Parking Lane) Width				
) S	Bike Lane Width LoS	E	-	-	-
Bic	Bike Lane Blockages				
	Blockage LoS		-	-	-
	Median Refuge Width (no median = < 1.8 m)			< 1.8 m refuge	
	No. of Lanes at Unsignalized Crossing			≤ 3 lanes	
	Sidestreet Operating Speed Unsignalized Crossing - Lowest LoS		A	≤ 40 km/h A	
	Olisigilalized Glossing - Lowest Loo			^	
	Level of Service		Α	Е	-
Ħ	Facility Type				
sui	Friction or Ratio Transit:Posted Speed	_			
Transit	Level of Service		-	-	-
	Truck Lane Width		≤ 3.3 m	> 3.7 m	
ck	Travel Lanes per Direction		> 1	1	
Truck	Level of Service	С	С	В	-

Multi-Modal Level of Service - Intersections Form

Consultant Scenario Comments

GH Transportation
xisting and Future Conditions
_

Project Date 2020-27 311 Somerset 2022-05-24

	INTERSECTIONS	Somerset St W at O'Connor St			Somerset St W at Bank St					Somerset St V	V at Metcalfe St		Gilmour St at O'Connor St					
	Crossing Side	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	
	Lanes	3	3	0 - 2	3	4	3	3	4	3	3	3	3	3	3	0 - 2	0 - 2	
	Median	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	
	Conflicting Left Turns	No left turn / Prohib.	Permissive	Permissive	No left turn / Prohib.	Permissive	Permissive	No left turn / Prohib.	No left turn / Prohib.	Permissive	No left turn / Prohib.	No left turn / Prohib.	Permissive	No left turn / Prohib.	No left turn / Prohib.	Permissive	No left turn / Prohib.	
	Conflicting Right Turns	No right turn	Permissive or yield control	No right turn	Permissive or yield control	No right turn	Permissive or yield control	No right turn	No right turn	Permissive or yield control	No right turn	No right turn						
	Right Turns on Red (RToR) ?	RTOR allowed	RTOR prohibited	RTOR prohibited	RTOR allowed	RTOR prohibited	RTOR allowed	RTOR allowed	RTOR prohibited	RTOR prohibited	RTOR prohibited	RTOR prohibited	RTOR allowed					
	Ped Signal Leading Interval?	No	No	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No	Yes	Yes	
an	Right Turn Channel	No Right Turn	No Channel	No Right Turn	No Channel	No Right Turn	No Right Turn	No Channel	No Right Turn	No Channel	No Right Turn	No Right Turn						
str	Corner Radius	No Right Turn	3-5m	No Right Turn	3-5m	3-5m	5-10m	5-10m	5-10m	0-3m	No Right Turn	No Right Turn	3-5m	No Right Turn	3-5m	No Right Turn	No Right Turn	
edestrian	Crosswalk Type	Std transverse markings	Std transverse markings	Textured/coloured pavement	Textured/coloured pavement	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Textured/coloured pavement	Zebra stripe hi-vis markings	Zebra stripe hi-vis markings	Zebra stripe hi-vis markings	Std transverse markings	
<u> </u>	PETSI Score	93	75	108	85	60	76	84	67	76	93	88	83	99	86	108	110	
	Ped. Exposure to Traffic LoS	A	В	A	В	C	В	В	C	В	A	В	В	A	В	A	A	
	Cycle Length	75	75	75	75	75	75	75	75	75	75	75	75	75	75	75	75	
	Effective Walk Time	26	26	24	24	27	27	12	12	29	29	23	23	44	44	7	7	
	Average Pedestrian Delay	16	16	17	17	15	15	26	26	14	14	18	18	6	6	31	31	
	Pedestrian Delay LoS	В	В	В	В	В	В	С	С	В	В	В	В	Α	Α	D	D	
		В	В	В	В	С	В	С	С	В	В	В	В	Α	В	D	D	
	Level of Service		E	3				C				В			[)		
	Approach From	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	
	Bicycle Lane Arrangement on Approach	Curb Bike Lane, Cycletrack or MUP	Curb Bike Lane, Cycletrack or MUP						Mixed Traffic					Curb Bike Lane, Cycletrack or MUP	Curb Bike Lane, Cycletrack or MUP			
	Right Turn Lane Configuration	Not Applicable	Not Applicable						> 50 m					Not Applicable	Not Applicable			
	Right Turning Speed	Not Applicable	Not Applicable						≤ 25 km/h					Not Applicable	Not Applicable			
Φ	Cyclist relative to RT motorists	Not Applicable	Not Applicable	-	Not Applicable	Α	Α	Α	F	-	Α	-	Α	Not Applicable	Not Applicable	-	-	
<u>ا</u>	Separated or Mixed Traffic	Separated	Separated	-	Separated	-	-	-	Mixed Traffic	-	-	•	-	Separated	Separated	-	-	
Bicycle	Left Turn Approach	2-stage, LT box	2-stage, LT box	No lane crossed	2-stage, LT box	No lane crossed	No lane crossed	No lane crossed	No lane crossed		One lane crossed		No lane crossed	No lane crossed				
	Operating Speed	> 50 to < 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	> 40 to ≤ 50 km/h		> 50 to < 60 km/h		> 50 to < 60 km/h	≤ 40 km/h							
	Left Turning Cyclist	Α	Α	С	Α	В	В	В	В	-	E	-	С	В	-	-	-	
	Level of Service	Α	Α	С	Α	В	В	В	F	•	E	-	С	В	-	-	-	
	Level of Service		C				1	F				E			E	3		
+	Average Signal Delay					≤ 20 sec	≤ 20 sec	≤ 30 sec										
nsit		-	-	-	-	С	С	D	-	-	-	-	-	-	-	-	-	
Trai	Level of Service							 D										
	Effective Corner Radius	10 - 15 m			< 10 m			< 10 m	< 10 m			< 10 m						
	Number of Receiving Lanes on Departure																	
5	from Intersection	1			≥ 2			1	1			≥ 2						
Truck	Level of Service	E	-	-	D	-	-	F	F	-	-	D	-	-	-	-	-	
	Level of Service		E			F				D				-				
0.	Volume to Capacity Ratio		0.71 -	0.80			0.0 - 0.60				0.61 - 0.70				0.0 - 0.60			
Auto	Level of Service		(;				Ą				В			-	\		

Appendix J

Synchro Intersection Worksheets – 2024 Future Total Conditions



Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Total AM Peak Hour 311 Somerset St W

	•	-	*	•	←	1	†	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		4	7	*	₽		ĵ,	f >	
Traffic Volume (vph)	53	254	75	21	163	3	378	159	
Future Volume (vph)	53	254	75	21	163	3	378	159	
Lane Group Flow (vph)	0	307	75	21	180	0	428	167	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	151	151	151	130	130	200	200	296	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.61	0.23	0.10	0.33		0.50	0.19	
Control Delay		27.4	20.9	8.8	9.6		13.9	10.0	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.4	20.9	8.8	9.6		13.9	10.0	
LOS		C	C	A	A		В	В	
Approach Delay		26.1			9.5		13.9	10.0	
Approach LOS		C			A		В	В	
Queue Length 50th (m)		36.0	7.6	0.9	8.2		36.2	11.6	
Queue Length 95th (m)		60.8	17.6	m2.4	13.9		58.9	21.2	
Internal Link Dist (m)		161.3			160.8		255.6	215.8	
Turn Bay Length (m)			25.0	10.0					
Base Capacity (vph)		504	323	219	548		860	890	
Starvation Cap Reductn		0	0_0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.61	0.23	0.10	0.33		0.50	0.19	
reduced we really		0.01	0.23	0.10	0.00		0.50	0.13	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 46 (61%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

05-31-2022 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.61 Intersection Signal Delay: 16.6 Intersection Capacity Utilization 75.0% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



05-31-2022 CGH Transportation JK Page 2

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Total AM Peak Hour 311 Somerset St W

EBT	WBL		
	VVDL	WBT	SBT
1>		4	413-
185	52	181	563
185	52	181	563
302	0	233	658
NA	Perm	NA	NA
4		8	6
	8		
4	8	8	6
10.0	10.0	10.0	10.0
20.5	20.5	20.5	22.4
37.0	37.0	37.0	38.0
49.3%	49.3%	49.3%	50.7%
31.5	31.5	31.5	32.6
3.3	3.3	3.3	3.3
2.2	2.2	2.2	2.1
0.0		0.0	0.0
5.5		5.5	5.4
3.0	3.0	3.0	3.0
Max	Max	Max	C-Max
7.0	7.0	7.0	10.0
8.0	8.0	8.0	7.0
106	89	89	120
31.5		31.5	32.6
0.42		0.42	0.43
0.44		0.37	0.48
16.5		24.2	16.1
			0.0
			16.1
В			В
16.5			16.1
			В
			32.4
			46.2
			33.6
100.0		100.7	55.0
679		626	1381
			0
			0
0		0	0
		0	
0.44		0.37	0.48
	185 185 185 302 NA 4 10.0 20.5 37.0 49.3% 31.5 3.3 2.2 0.0 5.5 3.0 Max 7.0 8.0 106 31.5 0.42 0.44 16.5 0.0 16.5 B 14.6 40.2 160.8	185 52 185 52 185 52 302 0 NA Perm 4	185 52 181 185 52 181 185 52 181 302 0 233 NA Perm NA 4 8 8 4 8 8 4 8 8 10.0 10.0 10.0 20.5 20.5 20.5 20.5 20.5 37.0 37.0 37.0 37.0 49.3% 49.3% 49.3% 31.5 31.5 31.5 3.3 3.3 3.3 3.3 2.2 2.2 2.2 0.0 0.0 0.0 5.5 5.5 3.0 3.0 3.0 3.0 Max Max Max 7.0 7.0 7.0 8.0 8.0 8.0 106 89 89 31.5 31.5 0.42 0.42 0.44 0.37 16.5 24.2 B C C 14.6 27.8 40.2 m45.5 160.8 155.7 679 626 0 0 0

Actuated Cycle Length: 75

Offset: 44 (59%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 45

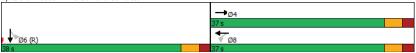
05-31-2022 **CGH Transportation** JK Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.48 Intersection Signal Delay: 17.8 Intersection Capacity Utilization 70.3% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: O'Connor & Somerset



05-31-2022 CGH Transportation JK Page 4 Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Total AM Peak Hour 311 Somerset St W

	•	-	•	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	<u> </u>	414
Traffic Volume (vph)	110	136	162	1121
Future Volume (vph)	110	136	162	1121
Lane Group Flow (vph)	0	246	326	1341
Turn Type	Perm	NA	NA	NA
Protected Phases	1 61111	2	6	4
Permitted Phases	2		0	4
Detector Phase	2	2	6	4
Switch Phase			0	4
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
	35.0	35.0	35.0	40.0
Total Split (s)	46.7%	46.7%	46.7%	53.3%
Total Split (%)				
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	145	145	130	268
Act Effct Green (s)		29.5	29.5	34.8
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.59	0.57	0.67
Control Delay		18.3	22.2	17.3
Queue Delay		0.0	0.0	0.0
Total Delay		18.3	22.2	17.3
LOS		В	С	В
Approach Delay		18.3	22.2	17.3
Approach LOS		В	С	В
Queue Length 50th (m)		20.7	34.2	50.3
Queue Length 95th (m)		42.2	58.8	64.9
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)		100.1	140.0	10-1.2
Base Capacity (vph)		420	571	1998
Starvation Cap Reductn		0	0	0
Spillback Cap Reductn		0	0	0
Storage Cap Reductin		0	0	0
Reduced v/c Ratio		0.59	0.57	0.67
		0.00	0.01	0.01
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				

Offset: 29 (39%), Referenced to phase 2:EBTL, Start of Green

Natural Cycle: 40

 05-31-2022
 CGH Transportation

 JK
 Page 5

Lanes, Volumes, Timings 3: Metcalfe & Somerset

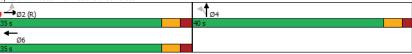
2024 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67
Intersection Signal Delay: 18.3 Intersection LOS: B
Intersection Capacity Utilization 81.3% ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: Metcalfe & Somerset



05-31-2022 CGH Transportation JK Page 6

Lanes, Volumes, Timings 4: O'Connor & Gilmour

Natural Cycle: 50

2024 Future Total AM Peak Hour 311 Somerset St W

	-	¥
Lane Group	EBT	SBT
Lane Configurations	1>	414
Traffic Volume (vph)	62	726
Future Volume (vph)	62	726
Lane Group Flow (vph)	130	788
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		• • • •
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	48	74
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.34	0.37
Control Delay	16.6	3.3
Queue Delay	0.0	0.0
Total Delay	16.6	3.3
LOS	В	Α.
Approach Delay	16.6	3.3
Approach LOS	В	A
Queue Length 50th (m)	7.6	8.4
Queue Length 95th (m)	21.4	14.4
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)	121.1	140.0
Base Capacity (vph)	377	2143
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.34	0.37
	0.04	0.51
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 46 (61%), Reference		2: and 6
10 1 50		

 05-31-2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

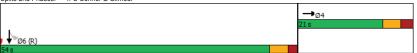
2024 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.37
Intersection Signal Delay: 5.2
Intersection Capacity Utilization 47.8%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



05-31-2022 CGH Transportation JK Page 8

Int Delay, s/veh

Movement Lane Configurations

Traffic Vol, veh/h

Future Vol, veh/h

RT Channelized

Storage Length

Peak Hour Factor

Heavy Vehicles, %

Conflicting Flow All

Stage 1 Stage 2 Critical Hdwy

Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy

Pot Cap-1 Maneuver

Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2

HCM Control Delay, s 10.4

Sign Control

Grade, %

Mvmt Flow

Conflicting Peds, #/hr

Veh in Median Storage, # 0

0.2

0 12 0

12

0 0

0

- 316

- 6.94

- 3.32

0 680 0 -

0.018

В

- -

EBL EBR NBL NBT SBT SBR

Stop Stop Free Free Free Free

- None - None - None

100 100 100 100 100 100

2 2 2 2

- - - 0 -0

0 626

0 626

0 0

0

SB

0

	•	-	•	•	+	4	†	ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		4	7	ች	1>		1>	1>	_
Traffic Volume (vph)	35	278	103	59	259	9	269	338	
Future Volume (vph)	35	278	103	59	259	9	269	338	
Lane Group Flow (vph)	0	313	103	59	263	0	308	370	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases	1 61111	4	1 Cilli	I CIIII	8	1 61111	2	6	
Permitted Phases	4	7	4	8	0	2	2	U	
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase	7	4	4	U	0	2		U	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (s)									
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	84	84	84	210	210	261	261	500	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.60	0.27	0.25	0.47		0.37	0.43	
Control Delay		27.0	21.0	14.7	16.1		12.0	12.9	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.0	21.0	14.7	16.1		12.0	12.9	
LOS		C C	C C	В.	В		12.0 B	12.3 B	
Approach Delay		25.5	- 3	5	15.8		12.0	12.9	
Approach LOS		23.5 C			13.0 B		12.0 B	12.3 B	
Queue Length 50th (m)		36.6	10.6	3.6	19.4		23.8	29.9	
Queue Length 95th (m)		61.6	22.2	m6.4	m31.9		39.8	49.0	
			22.2	1110.4			255.6	215.8	
Internal Link Dist (m)		161.3	05.0	40.0	160.8		255.6	215.8	
Turn Bay Length (m)		500	25.0	10.0	505		0.40	0.57	
Base Capacity (vph)		520	388	234	565		842	857	
Starvation Cap Reductn		0	0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.60	0.27	0.25	0.47		0.37	0.43	
Intersection Summary									
0 1 1 11 75									

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 71 (95%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 55

05-31-2022

JK

HCM LOS

Capacity (veh/h) HCM Lane V/C Ratio

HCM Control Delay (s) HCM Lane LOS

HCM 95th %tile Q(veh)

CGH Transportation Page 10 05-31-2022 JK

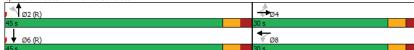
CGH Transportation Page 1

Lanes, Volumes, Timings 1: Bank & Somerset

2024 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.60 Intersection LOS: B
ICU Level of Service D Intersection Signal Delay: 17.1 Intersection Capacity Utilization 73.3% Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



05-31-2022 CGH Transportation Page 2 JK

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Total PM Peak Hour 311 Somerset St W

	-	•	-	↓
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	î,		4	414
Traffic Volume (vph)	192	72	185	1030
Future Volume (vph)	192	72	185	1030
Lane Group Flow (vph)	355	0	257	1177
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8	-	-
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	33.0	33.0	33.0	42.0
Total Split (%)	44.0%	44.0%	44.0%	56.0%
Maximum Green (s)	27.5	27.5	27.5	36.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag				
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	183	144	144	148
Act Effct Green (s)	27.5		27.5	36.6
Actuated g/C Ratio	0.37		0.37	0.49
v/c Ratio	0.66		0.57	0.77
Control Delay	44.1		18.7	19.6
Queue Delay	0.0		0.0	0.0
Total Delay	44.1		18.7	19.6
LOS	D		В	В
Approach Delay	44.1		18.7	19.6
Approach LOS	D		В	В
Queue Length 50th (m)	50.9		19.2	66.6
Queue Length 95th (m)	76.9		30.0	91.2
Internal Link Dist (m)	160.8		155.7	31.6
Turn Bay Length (m)				
Base Capacity (vph)	540		453	1538
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.66		0.57	0.77
Intersection Summary				

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 58 (77%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 55

JK

Lanes, Volumes, Timings 2: O'Connor & Somerset

2024 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated		
Maximum v/c Ratio: 0.77		
Intersection Signal Delay: 24.3	Intersection LOS: C	
Intersection Capacity Utilization 91.6%	ICU Level of Service F	
Analysis Period (min) 15		

Splits and Phases: 2: O'Connor & Somerset



05-31-2022 CGH Transportation Page 4 JK

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Total PM Peak Hour 311 Somerset St W

	*	-	←	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	<u></u>	414
Traffic Volume (vph)	88	186	150	673
Future Volume (vph)	88	186	150	673
Lane Group Flow (vph)	0	274	257	918
Turn Type	Perm	NA	NA	NA NA
	reiiii	2	6	4
Protected Phases	_	2	р	4
Permitted Phases	2	0	^	
Detector Phase	2	2	6	4
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag		0.0	0.0	0.2
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
	166	166	218	351
Pedestrian Calls (#/hr)	100	29.5	29.5	34.8
Act Effet Green (s)				
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.52	0.45	0.49
Control Delay		19.9	18.6	13.7
Queue Delay		0.0	0.0	0.0
Total Delay		19.9	18.6	13.7
LOS		В	В	В
Approach Delay		19.9	18.6	13.7
Approach LOS		В	В	В
Queue Length 50th (m)		20.7	23.9	28.4
Queue Length 95th (m)		m42.0	43.2	38.9
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)				
Base Capacity (vph)		523	571	1865
Starvation Cap Reductn		0_0	0	0
Spillback Cap Reductn		0	0	0
		0	0	0
Storage Cap Reductn				
Reduced v/c Ratio		0.52	0.45	0.49
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 20 (27%), Reference	nd to phace	2-EBTI	Start of (2roon
	eu to priase	Z.EDIL,	Start or t	JIEEH
latural Cycle: 40				

JK

05-31-2022 CGH Transportation Page 5

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2024 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52
Intersection Signal Delay: 15.7
Intersection LOS: B
Intersection Capacity Utilization 70.6%
ICU Level of Service C
Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Metcalfe & Somerset

Opinio and i nases. S. Meteane & Comerset		
≠ø2 (R)	↑ 04	
35 s	40 s	
← Ø6		
25 -		

 05-31-2022
 CGH Transportation

 JK
 Page 6

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2024 Future Total PM Peak Hour 311 Somerset St W

1 0	EDT	↓ ODT
Lane Group	EBT	SBT
Lane Configurations	4	410000
Traffic Volume (vph)	86	1286
Future Volume (vph)	86	1286
Lane Group Flow (vph)	194	1372
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases Detector Phase	4	6
Switch Phase	4	0
	10.0	10.0
Minimum Initial (s) Minimum Split (s)	10.0	10.0 26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag	0.0	0.1
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	56	109
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.54	0.64
Control Delay	24.8	5.4
Queue Delay	0.0	0.4
Total Delay	24.8	5.8
LOS	С	Α
Approach Delay	24.8	5.8
Approach LOS	С	Α
Queue Length 50th (m)	16.9	12.5
Queue Length 95th (m)	36.2	19.9
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)		
Base Capacity (vph)	360	2142
Starvation Cap Reductn	0	286
	0	0
Spillback Cap Reductn	0	0
Spillback Cap Reductn Storage Cap Reductn		
Spillback Cap Reductn	0.54	0.74
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		0.74
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary		0.74
Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio		0.74

Natural Cycle: 55

05-31-2022 CGH Transportation JK Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2024 Future Total PM Peak Hour 311 Somerset St W

HCM 2010 TWSC 5: O'Connor & Site Access 2024 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.64	
Intersection Signal Delay: 8.1	Intersection LOS: A
Intersection Capacity Utilization 66.5%	ICU Level of Service C
Analysis Period (min) 15	

		→ Ø4 21 s
Ø6 (R)	_	

Internation						
Intersection Int Delay, s/veh	0.1					
iiit Delay, S/Veri						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7			∱ ∱	
Traffic Vol, veh/h	0	10	0	0	1154	12
Future Vol, veh/h	0	10	0	0	1154	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	-	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	10	0	0	1154	12
	1inor2				Major2	
Conflicting Flow All	-	583			-	0
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Critical Hdwy	-	6.94			-	-
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	3.32			-	-
Pot Cap-1 Maneuver	0	456			-	-
Stage 1	0	-			-	-
Stage 2	0				_	-
Platoon blocked, %	U	_				_
Mov Cap-1 Maneuver		456				_
Mov Cap-1 Maneuver		400				-
	-					-
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Approach	EB				SB	
HCM Control Delay, s	13.1				0	
HCM LOS	В				U	
I ICIVI LOG	ь					
Minor Lane/Major Mvmt		EBLn1	SBT	SBR		
Capacity (veh/h)		456	-	-		
HCM Lane V/C Ratio		0.022				
HCM Control Delay (s)		13.1	_			
HCM Lane LOS		В				
HCM 95th %tile Q(veh)		0.1				
		0.1	-	-		

Appendix K

Synchro Intersection Worksheets – 2029 Future Total Conditions



Lanes, Volumes, Timings 1: Bank & Somerset

2029 Future Total AM Peak Hour 311 Somerset St W

	•	-	*	•	-	1	†	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBT	
Lane Configurations		4	7	ች	1 >		f,	f.	
Traffic Volume (vph)	53	256	75	21	170	3	378	159	
Future Volume (vph)	53	256	75	21	170	3	378	159	
Lane Group Flow (vph)	0	309	75	21	187	0	428	167	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	NA	
Protected Phases		4			8		2	6	
Permitted Phases	4		4	8		2			
Detector Phase	4	4	4	8	8	2	2	6	
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	
Total Split (s)	30.0	30.0	30.0	30.0	30.0	45.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	39.5	39.5	39.5	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.5	5.5	5.5	5.5		5.5	5.5	
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	Max	Max	Max	Max	Max	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	
Pedestrian Calls (#/hr)	151	151	151	130	130	200	200	296	
Act Effct Green (s)		24.5	24.5	24.5	24.5		39.5	39.5	
Actuated g/C Ratio		0.33	0.33	0.33	0.33		0.53	0.53	
v/c Ratio		0.61	0.23	0.10	0.34		0.50	0.19	
Control Delay		27.5	20.9	8.7	9.6		13.9	10.0	
Queue Delay		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		27.5	20.9	8.7	9.6		13.9	10.0	
LOS		C	C	A	A		В	В	
Approach Delay		26.2			9.5		13.9	10.0	
Approach LOS		C			A		В	В	
Queue Length 50th (m)		36.3	7.6	0.9	8.4		36.2	11.6	
Queue Length 95th (m)		61.2	17.6	m2.3	14.3		58.9	21.2	
nternal Link Dist (m)		161.3	0	0	160.8		255.6	215.8	
Turn Bay Length (m)			25.0	10.0			200.0	2.0.0	
Base Capacity (vph)		504	323	217	548		860	890	
Starvation Cap Reductn		0	0_0	0	0		0	0	
Spillback Cap Reductn		0	0	0	0		0	0	
Storage Cap Reductn		0	0	0	0		0	0	
Reduced v/c Ratio		0.61	0.23	0.10	0.34		0.50	0.19	
Nouvoca W/C (Natio		0.01	0.23	0.10	0.54		0.50	0.19	

Intersection Summary

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 46 (61%), Referenced to phase 2:NBTL and 6:SBT, Start of Green Natural Cycle: 55

05-31-2022 CGH Transportation JK Page 1 Lanes, Volumes, Timings 1: Bank & Somerset

2029 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.61 Intersection Signal Delay: 16.6 Intersection Capacity Utilization 75.2% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



05-31-2022 CGH Transportation JK Page 2

Lanes, Volumes, Timings 2: O'Connor & Somerset

2029	Future	Total	ΑM	Peak Hour
			311	Somerset St W

	→	1	+	
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	<u> </u>		4	47>
Traffic Volume (vph)	185	53	188	567
Future Volume (vph)	185	53	188	567
Lane Group Flow (vph)	304	0	241	662
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8		
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	37.0	37.0	37.0	38.0
Total Split (%)	49.3%	49.3%	49.3%	50.7%
Maximum Green (s)	31.5	31.5	31.5	32.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0	2.2	0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag	0.0		0.0	0.1
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	106	89	89	120
Act Effct Green (s)	31.5	09	31.5	32.6
Actuated g/C Ratio	0.42		0.42	0.43
				0.43
v/c Ratio	0.45		0.38	16.1
Control Delay	16.6			
Queue Delay	0.0		0.0	0.0
Total Delay	16.6		23.8	16.1
LOS	В		С	В
Approach Delay	16.6		23.8	16.1
Approach LOS	В		С	В
Queue Length 50th (m)	15.0		28.4	32.7
Queue Length 95th (m)	40.5		m46.2	46.6
Internal Link Dist (m)	160.8		155.7	33.6
Turn Bay Length (m)				
Base Capacity (vph)	678		626	1382
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
Storage Cap Reductn	0		0	0
Reduced v/c Ratio	0.45		0.38	0.48
Internation Comme				
Intersection Summary				

Cycle Length: 75
Actuated Cycle Length: 75

Offset: 44 (59%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 45

05-31-2022 **CGH Transportation** JK Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.48 Intersection Signal Delay: 17.8 Intersection Capacity Utilization 71.1% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: O'Connor & Somerset



05-31-2022 CGH Transportation JK Page 4

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Total AM Peak Hour 311 Somerset St W

•	-	—	†
EBL	EBT	WBT	NBT
			414
110			1133
			1133
			1362
-			NA
Perm			NA 4
^	2	6	4
	^	^	
2	2	6	4
			10.0
19.5	19.5	19.5	18.2
35.0	35.0	35.0	40.0
46.7%	46.7%	46.7%	53.3%
29.5	29.5	29.5	34.8
3.3	3.3	3.3	3.3
2.2	2.2	2.2	1.9
	0.0	0.0	0.0
			5.2
	0.0	0.0	U.L
3.0	3.0	3.0	3.0
			Max
			7.0
			6.0
145			268
			34.8
			0.46
			0.68
			17.6
	0.0	0.0	0.0
	18.4	22.2	17.6
	В	С	В
	18.4	22.2	17.6
	В	С	В
	20.7	34.3	51.5
			66.6
			134.2
	155.7	140.0	104.2
	420	571	1994
			1994
			0
			0
	0.59	0.57	0.68
	EBL 110 110 0 Perm 2 2 10.0 19.5 35.0 46.7% 29.5 3.3	EBL EBT 110 136 110 136 0 246 Perm NA 2 2 2 2 10.0 10.0 19.5 19.5 35.0 35.0 35.0 46.7% 46.7% 29.5 29.5 3.3 3.3 2.2 2.2 0.0 5.5 3.0 3.0 C-Max C-Max 7.0 7.0 7.0 7.0 145 145 29.5 0.39 0.59 18.4 B 18.4	EBL EBT WBT 110 136 163 110 136 163 0 246 327 Perm NA NA 2 6 2 2 2 6 10.0 10.0 10.0 19.5 19.5 19.5 35.0 35.0 35.0 35.0 46.7% 46.7% 46.7% 29.5 29.5 29.5 3.3 3.3 3.3 3.3 2.2 2.2 2.2 0.0 0.0 5.5 5.5 3.0 3.0 3.0 3.0 C-Max C-Max Max 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 145 145 130 29.5 29.5 0.39 0.39 0.59 0.57 18.4 22.2 0.0 0.0 18.4 22.2 0.0 0.0 18.4 22.2 0.0 18.4 22.2 0.0 18.4 22.2 0.0 18.4 22.2 0.1 18.4 22.2 0.2 18.4 22.2 0.3 18.4 22.2 0.5 18.4 22.2 0.6 18.4 22.2 0.7 34.3 0.7 145.3

Offset: 29 (39%), Referenced to phase 2:EBTL, Start of Green

Natural Cycle: 40

05-31-2022 CGH Transportation JK Page 5 Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.68 Intersection Signal Delay: 18.5 Intersection Capacity Utilization 81.8% Analysis Period (min) 15 Intersection LOS: B
ICU Level of Service D

Splits and Phases: 3: Metcalfe & Somerset



05-31-2022 CGH Transportation JK Page 6 Lanes, Volumes, Timings 4: O'Connor & Gilmour

Natural Cycle: 50

2029 Future Total AM Peak Hour 311 Somerset St W

	-	¥
Lane Group	EBT	SBT
Lane Configurations	1→	414
Traffic Volume (vph)	63	752
Future Volume (vph)	63	752
Lane Group Flow (vph)	131	828
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	48	74
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.35	0.39
Control Delay	16.9	3.5
Queue Delay	0.0	0.0
Total Delay	16.9	3.5
LOS	В	A
Approach Delay	16.9	3.5
Approach LOS	В	A
Queue Length 50th (m)	7.8	9.6
Queue Length 95th (m)	21.6	15.8
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)	127.1	140.0
Base Capacity (vph)	377	2138
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.35	0.39
	0.00	0.00
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 46 (61%), Reference	ed to phase	2: and 6
11 10 1 50		

 05-31-2022
 CGH Transportation

 JK
 Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

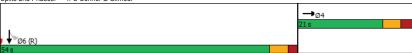
2029 Future Total AM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.39
Intersection Signal Delay: 5.3
Intersection Capacity Utilization 49.0%

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour



05-31-2022 CGH Transportation JK Page 8

HCM Control Delay (s)

HCM 95th %tile Q(veh)

В -

HCM Lane LOS

Intersection Summa

Cycle Length: 75

Actuated Cycle Length: 75

Lanes, Volumes, Timings

1: Bank & Somerset

Offset: 71 (95%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Natural Cycle: 55

JK

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			† \$	
Traffic Vol, veh/h	0	12	0	0	630	6
Future Vol. veh/h	0	12	0	0	630	6
Conflicting Peds, #/hr		.2	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-		-	None	-	None
Storage Length		0		-		-
Veh in Median Storag		-		0	0	
Grade. %	0	-	-	0	0	
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	12	0	0	630	6
IVIVIIIL FIOW	U	IZ	U	U	030	0
Major/Minor	Minor2			N	Major2	
Conflicting Flow All	-	318			-	0
Stage 1	-	-				-
Stage 2	-	-				-
Critical Hdwy	-	6.94			-	-
Critical Hdwy Stg 1	-	-				-
Critical Hdwy Stg 2		-				_
Follow-up Hdwy	-	3.32				-
Pot Cap-1 Maneuver	0	678				_
Stage 1	0	-				-
Stage 2	0	-				_
Platoon blocked. %	U	_				
Mov Cap-1 Maneuver		678				
Mov Cap-1 Maneuver		0/0				
Stage 1	-	-				-
Stage 2	-	-			-	-
Approach	EB				SB	
HCM Control Delay, s	10.4				0	
HCM LOS	В					
Minor Long/Major Mar	mt	EBLn1	SBT	SBR		
Minor Lane/Major Mvi	IIIC					
Capacity (veh/h)		678	-	-		
HCM Lane V/C Ratio		0.018	-	-		

05-31-2022 CGH Transportation JK Page 10

05-31-2022

CGH Transportation Page 1

Lanes, Volumes, Timings 1: Bank & Somerset

2029 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 17.1

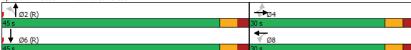
Intersection LOS: B

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Bank & Somerset



 05-31-2022
 CGH Transportation

 JK
 Page 2

Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Total PM Peak Hour 311 Somerset St W

	-	•	-	↓
Lane Group	EBT	WBL	WBT	SBT
Lane Configurations	î,		4	414
Traffic Volume (vph)	192	74	189	1041
Future Volume (vph)	192	74	189	1041
Lane Group Flow (vph)	359	0	263	1188
Turn Type	NA	Perm	NA	NA
Protected Phases	4		8	6
Permitted Phases		8		
Detector Phase	4	8	8	6
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	20.5	20.5	20.5	22.4
Total Split (s)	33.0	33.0	33.0	42.0
Total Split (%)	44.0%	44.0%	44.0%	56.0%
Maximum Green (s)	27.5	27.5	27.5	36.6
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1
Lost Time Adjust (s)	0.0		0.0	0.0
Total Lost Time (s)	5.5		5.5	5.4
Lead/Lag	0.0		0.0	0.1
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	Max	Max	Max	C-Max
Walk Time (s)	7.0	7.0	7.0	10.0
Flash Dont Walk (s)	8.0	8.0	8.0	7.0
Pedestrian Calls (#/hr)	183	144	144	148
Act Effct Green (s)	27.5		27.5	36.6
Actuated g/C Ratio	0.37		0.37	0.49
v/c Ratio	0.67		0.59	0.77
Control Delay	44.5		19.5	19.8
Queue Delay	0.0		0.0	0.0
Total Delay	44.5		19.5	19.8
LOS	TT.5		В	В
Approach Delay	44.5		19.5	19.8
Approach LOS	TT.5		В	В
Queue Length 50th (m)	51.6		20.0	67.6
Queue Length 95th (m)	77.7		30.9	92.6
Internal Link Dist (m)	160.8		155.7	31.6
Turn Bay Length (m)	100.0		100.1	01.0
Base Capacity (vph)	539		446	1539
Starvation Cap Reductn	0		0	0
Spillback Cap Reductn	0		0	0
	0		0	0
Storage Cap Reductin				
Storage Cap Reductn Reduced v/c Ratio	0.67		0.59	0.77

ntersection Summary

Cycle Length: 75

Actuated Cycle Length: 75

Offset: 58 (77%), Referenced to phase 2: and 6:SBTL, Start of Green

Natural Cycle: 55

05-31-2022 CGH Transportation JK Page 3

Lanes, Volumes, Timings 2: O'Connor & Somerset

2029 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated		
Maximum v/c Ratio: 0.77		
Intersection Signal Delay: 24.6	Intersection LOS: C	
Intersection Capacity Utilization 92.5%	ICU Level of Service F	
Analysis Period (min) 15		

Splits and Phases: 2: O'Connor & Somerset



05-31-2022 CGH Transportation Page 4 JK

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Total PM Peak Hour 311 Somerset St W

	۶	-	←	†
Lane Group	EBL	EBT	WBT	NBT
Lane Configurations		4	1>	414
Traffic Volume (vph)	88	186	152	680
Future Volume (vph)	88	186	152	680
Lane Group Flow (vph)	00	274	259	930
Turn Type	Perm	NA	NA	NA
	reiiii		6	4
Protected Phases	_	2	б	4
Permitted Phases	2			
Detector Phase	2	2	6	4
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	10.0
Minimum Split (s)	19.5	19.5	19.5	18.2
Total Split (s)	35.0	35.0	35.0	40.0
Total Split (%)	46.7%	46.7%	46.7%	53.3%
Maximum Green (s)	29.5	29.5	29.5	34.8
Yellow Time (s)	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	1.9
Lost Time Adjust (s)		0.0	0.0	0.0
Total Lost Time (s)		5.5	5.5	5.2
Lead/Lag		0.0	0.0	0.2
Lead-Lag Optimize?				
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	6.0
Pedestrian Calls (#/hr)	166	166	218	351
Act Effct Green (s)		29.5	29.5	34.8
Actuated g/C Ratio		0.39	0.39	0.46
v/c Ratio		0.52	0.45	0.50
Control Delay		20.1	18.6	13.8
Queue Delay		0.0	0.0	0.0
Total Delay		20.1	18.6	13.8
LOS		С	В	В
Approach Delay		20.1	18.6	13.8
Approach LOS		С	В	В
Queue Length 50th (m)		21.0	24.2	29.0
Queue Length 95th (m)		m41.7	43.5	39.5
Internal Link Dist (m)		155.7	145.3	134.2
Turn Bay Length (m)		155.7	140.0	104.2
Base Capacity (vph)		523	572	1864
				1004
Starvation Cap Reductn		0	0	
Spillback Cap Reductn		0	0	0
Storage Cap Reductn		0	0	0
Reduced v/c Ratio		0.52	0.45	0.50
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75		0 EDTI	01 1 (_
Offset: 20 (27%), Reference	d to phase	e 2:EBTL,	Start of 0	Green
Natural Cycle: 40				

JK

05-31-2022 CGH Transportation Page 5

Lanes, Volumes, Timings 3: Metcalfe & Somerset

2029 Future Total PM Peak Hour 311 Somerset St W

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.52 Intersection Signal Delay: 15.8 Intersection LOS: B Intersection Capacity Utilization 70.9% Analysis Period (min) 15 ICU Level of Service C m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Metcalfe & Somerset

opino ana i nacco.		
→ Ø2 (R)	↑ ø4	
35 s	40 s	
←		
Ø6		
35 c		

05-31-2022 CGH Transportation Page 6 JK

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Total PM Peak Hour 311 Somerset St W

	-	↓ ODT
Lane Group	EBT	SBT
Lane Configurations	fì	41
Traffic Volume (vph)	88	1301
Future Volume (vph)	88	1301
Lane Group Flow (vph)	196	1427
Turn Type	NA	NA
Protected Phases	4	6
Permitted Phases		
Detector Phase	4	6
Switch Phase		
Minimum Initial (s)	10.0	10.0
Minimum Split (s)	20.6	26.1
Total Split (s)	21.0	54.0
Total Split (%)	28.0%	72.0%
Maximum Green (s)	15.4	48.9
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.3	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.6	5.1
Lead/Lag		
Lead-Lag Optimize?		
Vehicle Extension (s)	3.0	3.0
Recall Mode	Max	C-Max
Walk Time (s)	7.0	16.0
Flash Dont Walk (s)	8.0	5.0
Pedestrian Calls (#/hr)	56	109
Act Effct Green (s)	15.4	48.9
Actuated g/C Ratio	0.21	0.65
v/c Ratio	0.55	0.67
Control Delay	25.3	6.2
Queue Delay	0.0	0.4
Total Delay	25.3	6.6
LOS	С	Α
Approach Delay	25.3	6.6
Approach LOS	С	Α
Queue Length 50th (m)	17.5	14.0
Queue Length 95th (m)	36.8	32.3
Internal Link Dist (m)	127.1	143.6
Turn Bay Length (m)		
Base Capacity (vph)	359	2129
Starvation Cap Reductn	0	269
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
	0.55	0.77
Reduced v/c Ratio		• · · ·
Reduced v/c Ratio		
Intersection Summary		

Natural Cycle: 60

JK

05-31-2022 CGH Transportation Page 7

Lanes, Volumes, Timings 4: O'Connor & Gilmour

2029 Future Total PM Peak Hour 311 Somerset St W

CGH Transportation

Page 8

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67
Intersection Signal Delay: 8.9
Intersection Capacity Utilization 68.3%
Intersection Capacity Utilization 68.3%
ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 4: O'Connor & Gilmour

05-31-2022

JK



HCM 2010 TWSC 5: O'Connor & Site Access

2029 Future Total PM Peak Hour 311 Somerset St W

Intersection						
Int Delay, s/veh	0.1					
•			NIDI	NID.T		000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7			∱ ⊅	
Traffic Vol, veh/h	0	10	0	0	1165	12
Future Vol, veh/h	0	10	0	0	1165	12
Conflicting Peds, #/hr	. 0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storag	je,# 0	-	-	-	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	10	0	0	1165	12
M 1 / / / / / / / / / / / / / / / / / /	14: 0					
Major/Minor	Minor2				Major2	
Conflicting Flow All	-	589			-	0
Stage 1	-	-			-	-
Ctogo 2						

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	589	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.94	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	3.32	-	-	
Pot Cap-1 Maneuver	0	452	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuver	-	452	-	-	
Mov Cap-2 Maneuver	-	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
			0.0		

HCM Control Delay, s 13.1 0	Approach	EB	SB	3	
HCM LOS B	HCM Control Delay, s	13.1	0		
TIOM EGG B	HCM LOS	В			

Minor Lane/Major Mymt	EBLn1	SBT	SBR
Capacity (veh/h)	452	-	-
HCM Lane V/C Ratio	0.022	-	-
HCM Control Delay (s)	13.1	-	-
HCM Lane LOS	В	-	-
HCM 95th %tile Q(veh)	0.1	-	-

Appendix L

TDM Checklist



TDM Measures Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

	Legend
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
*	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	★ 1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & destin	ations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances	abla
	2.2	Bicycle skills training	
		Commuter travel	
BETTER	★ 2.2.1	Offer on-site cycling courses for commuters, or subsidize off-site courses	
	2.3	Valet bike parking	
		Visitor travel	
BETTER	2.3.1	Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances	
BASIC	3.1.2	Provide online links to OC Transpo and STO information	♥
BETTER	3.1.3	Provide real-time arrival information display at entrances	
	3.2	Transit fare incentives	
		Commuter travel	
BETTER	3.2.1	Offer preloaded PRESTO cards to encourage commuters to use transit	
BETTER ★	3.2.2	Subsidize or reimburse monthly transit pass purchases by employees	
		Visitor travel	
BETTER	3.2.3	Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	
	3.3	Enhanced public transit service	
		Commuter travel	
BETTER	3.3.1	Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.3.2	Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	
	3.4	Private transit service	
		Commuter travel	
BETTER	3.4.1	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.4.2	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for feetivals, concerts, games)	

Version 1.0 (30 June 2017)

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	4.	RIDESHARING	
	4.1	Ridematching service	
		Commuter travel	
BASIC	4.1.1	Provide a dedicated ridematching portal at OttawaRideMatch.com	
	4.2	Carpool parking price incentives	
		Commuter travel	
BETTER	4.2.1	Provide discounts on parking costs for registered carpools	
	4.3	Vanpool service	
		Commuter travel	
BETTER	4.3.1	Provide a vanpooling service for long-distance commuters	
	5.	CARSHARING & BIKESHARING	
	5.1	Bikeshare stations & memberships	
BETTER	5.1.1	Contract with provider to install on-site bikeshare station for use by commuters and visitors	
		Commuter travel	
BETTER	5.1.2	Provide employees with bikeshare memberships for local business travel	
	5.2	Carshare vehicles & memberships	
		Commuter travel	
BETTER	5.2.1	Contract with provider to install on-site carshare vehicles and promote their use by tenants	
BETTER	5.2.2	Provide employees with carshare memberships for local business travel	
	6.	PARKING	
	6.1	Priced parking	
		Commuter travel	
BASIC #	6.1.1	Charge for long-term parking (daily, weekly, monthly)	Ø
BASIC	6.1.2	Unbundle parking cost from lease rates at multi-tenant sites	
		Visitor travel	
BETTER	6.1.3	Charge for short-term parking (hourly)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	7.	TDM MARKETING & COMMUNICATIONS	
	7.1	Multimodal travel information	
		Commuter travel	
BASIC *	7.1.1	Provide a multimodal travel option information package to new/relocating employees and students	abla
		Visitor travel	:
BETTER ★	7.1.2	Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	
	7.2	Personalized trip planning	
		Commuter travel	
BETTER ★	7.2.1	Offer personalized trip planning to new/relocating employees	
	7.3	Promotions	
		Commuter travel	
BETTER	7.3.1	Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	
	8.	OTHER INCENTIVES & AMENITIES	
	8.1	Emergency ride home	
		Commuter travel	
BETTER ★	8.1.1	Provide emergency ride home service to non-driving commuters	
	8.2	Alternative work arrangements	
		Commuter travel	
BASIC ★	8.2.1	Encourage flexible work hours	
BETTER	8.2.2	Encourage compressed workweeks	
BETTER *	8.2.3	Encourage telework	
	8.3	Local business travel options	
		Commuter travel	
BASIC *	8.3.1	Provide local business travel options that minimize the need for employees to bring a personal car to work	
	8.4	Commuter incentives	
		Commuter travel	
BETTER	8.4.1	Offer employees a taxable, mode-neutral commuting allowance	
	8.5	On-site amenities	
		Commuter travel	
BETTER	8.5.1	Provide on-site amenities/services to minimize	

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

	Legend
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
*	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC *	1.1.1	Designate an internal coordinator, or contract with an external coordinator	abla
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	Ø
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

		TDM	measures: Residential developments	Check if proposed & add descriptions
		3.	TRANSIT	
		3.1	Transit information	
BASIC		3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	abla
BETTER		3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
		3.2	Transit fare incentives	
BASIC	*	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	abla
BETTER		3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
		3.3	Enhanced public transit service	
BETTER	*	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
		3.4	Private transit service	
BETTER		3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
		4.	CARSHARING & BIKESHARING	
		4.1	Bikeshare stations & memberships	
BETTER		4.1.1	Contract with provider to install on-site bikeshare station (multi-family)	
BETTER		4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
		4.2	Carshare vehicles & memberships	
BETTER		4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER		4.2.2	Provide residents with carshare memberships, either free or subsidized	
		5.	PARKING	
		5.1	Priced parking	
BASIC	*	5.1.1	Unbundle parking cost from purchase price (condominium)	abla
BASIC	*	5.1.2	Unbundle parking cost from monthly rent (multi-family)	abla

Version 1.0 (30 June 2017)

TDM measures: Residential developments			Check if proposed & add descriptions
	6.	TDM MARKETING & COMMUNICATIONS	
	6.1	Multimodal travel information	
BASIC	6.1.1	Provide a multimodal travel option information package to new residents	abla
	6.2	Personalized trip planning	
BETTER	6.2.1	Offer personalized trip planning to new residents	П

TDM-Supportive Development Design and Infrastructure Checklist: *Non-Residential Developments (office, institutional, retail or industrial)*

Legend		
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed	
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users	
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance	

TDM-supportive design & infrastructure measures: Non-residential developments		•	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	abla
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	∀
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	∀
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	₹
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	Ø
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	\triangledown
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	upportive design & infrastructure measures: Non-residential developments	add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	Ø
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	✓
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	abla
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	
	2.3	Shower & change facilities	
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	
	2.4	Bicycle repair station	
BETTER	2.4.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	4.2	Carpool parking	
BASIC	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide carshare parking spaces in permitted non- residential zones, occupying either required or provided parking spaces (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct well-way connection.	

	TDM-s	upportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	abla
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	
	7.	OTHER	
	7.1	On-site amenities to minimize off-site trips	
BETTER	7.1.1	Provide on-site amenities to minimize mid-day or mid-commute errands	

TDM-Supportive Development Design and Infrastructure Checklist: Residential Developments (multi-family or condominium)

Legend		
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed	
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users	
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	abla
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	Ø
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	♥
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	☑
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	✓

	TDM-s	supportive design & infrastructure measures: Residential developments	add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	abla
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	♥
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	∇′
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	♥
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	♥
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 1111)	✓
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any op-site transit stops into the building	

	TDM-supportive design & infrastructure measures: Residential developments			Check if completed & add descriptions, explanations or plan/drawing references	
		4.	RIDESHARING		
ı		4.1	Pick-up & drop-off facilities		
	BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones		
		5.	CARSHARING & BIKESHARING		
		5.1	Carshare parking spaces		
	BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)		
		5.2	Bikeshare station location		
	BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection		
		6.	PARKING		
I		6.1	Number of parking spaces		
	REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for		
	BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking		
	BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)		
	BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)		
		6.2	Separate long-term & short-term parking areas		
	BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice verse)		