

Site Servicing Brief

Greystone Village Phase 3 Condos

Greystone Village Phase 3 Condos Site Servicing Brief

Prepared For:

Greystone Village Inc.

Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
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August 19, 2022

Novatech File: 114025

Ref: R-2022-020

August 19, 2022

City of Ottawa
Planning, Infrastructure and Economic Development Department
Infrastructure Approvals Division,
110 Laurier Avenue West, 4th Floor
Ottawa, ON K1P 1J1

Attention: Nishant Jhamb

**Reference: Greystone Village Phase 3 Condos
Site Servicing Brief
Novatech File No.: 114025**

Please find enclosed a copy of the revised Site Servicing Brief for the Greystone Village Phase 3 Condos, located at 375 Deschâtelets Avenue in Old Ottawa East, east of Main Street/Deschâtelets Avenue, south of des Oblats Avenue, west of Scholastic Drive and north of Deschâtelets Avenue within the City of Ottawa. The report demonstrates how the proposed site will be serviced with storm, sanitary, watermain, utilities, and stormwater management and is submitted for your review and approval.

This report is supplementary to the following reports to provide specifics related to the Greystone Village Phase 3 Condo Buildings which is part of the overall Greystone Village subdivision development:

- *“Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief” dated February 24, 2016*
- *“Greystone Village - 175 Main Street: Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief (Phase 2 and 3), R-2017-089”, dated May 26, 2017*

If you have any questions or comments, please do not hesitate to contact us.

Sincerely,

NOVATECH



Steve Zorgel, P. Eng.
Project Coordinator | Land Development Engineering

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This report should be read in conjunction with the engineering drawing set which includes the following drawings, dated August 19, 2022:

- 114025-GP (PH3) - General Plans of Services
- 114025-GR (PH3) - Grading, Erosion and Sediment Control Plan
- 114025-STM (PH3) - Storm Drainage Area Plan
- 114025-SAN1-2 – Sanitary Drainage Area Plan
- 114025-SAN1-B – Sanitary Drainage Area Plan

CD

- Hydraulic Model – Phase 3

1.0 INTRODUCTION

Novatech has been retained by Greystone Village Inc. to prepare this Site Servicing Brief in support of the site plan application of the Greystone Village Phase 3 Condos at 375 Deschâtelets Avenue in Old Ottawa East. The key plan (**Figure 1**) highlights the Greystone Village and Phase 3 Condo site location. The existing property is currently vacant. The proposed re-development of this portion of the site will consist of two (2) 7-storey buildings that will contain 153 and 118 units respectively. A total of approximately 170 underground parking spaces will be provided within underground parking. This Site Servicing Brief will confirm how the proposed Greystone Village Phase 3 Condos will be serviced with sanitary, water, stormwater management, and utilities. Refer to **Figure 2 – Concept Plan – Phase 3 Condos** for proposed site layout.

1.1 Geotechnical Investigation

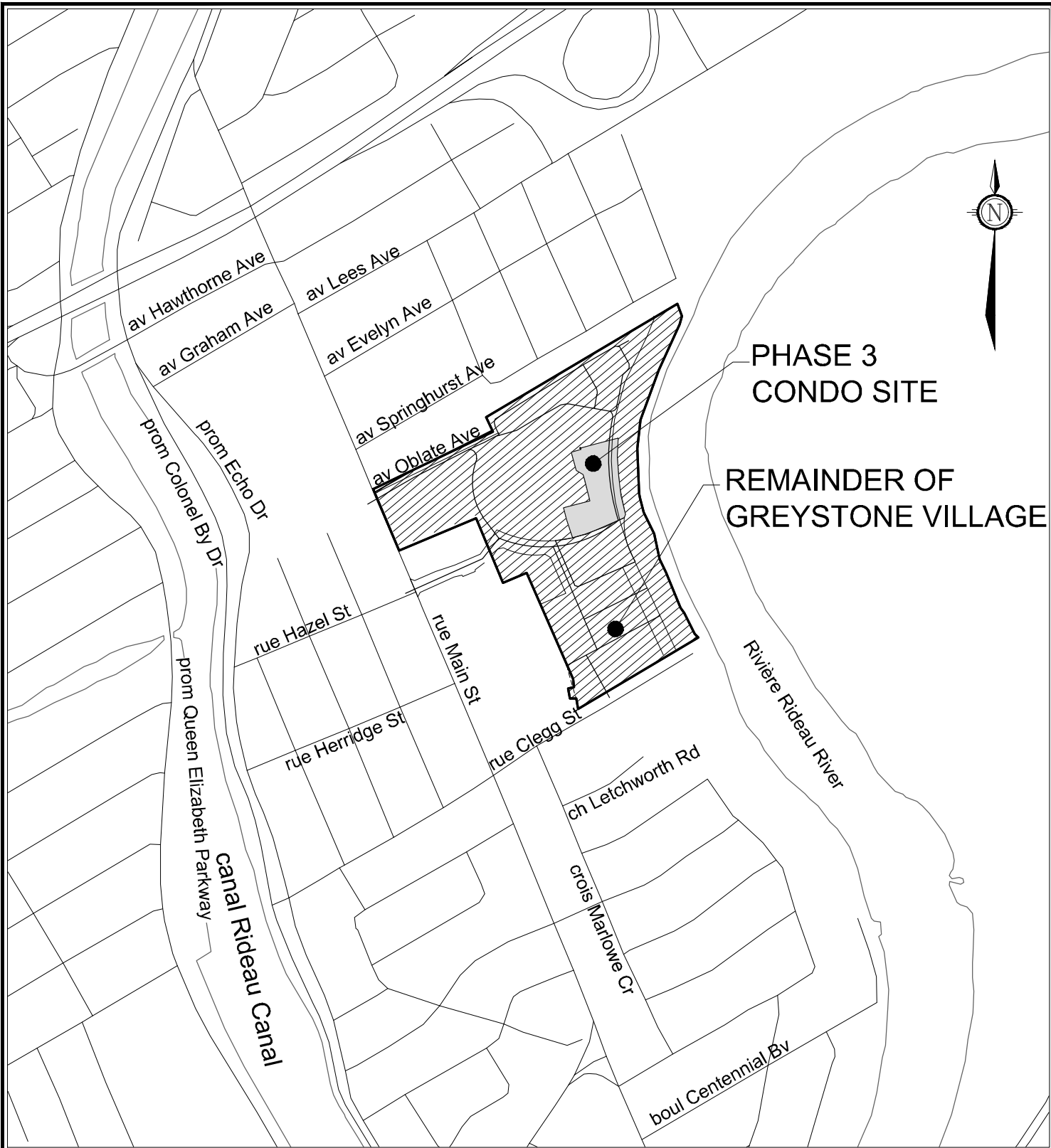
Refer to Paterson's geotechnical report (*Geotechnical Investigation – Proposed Multi-Storey Buildings – Greystone Village - Phase 3 – Scholastic Drive – Ottawa, Ontario*; PG5383-1; dated August 11, 2020) for geotechnical considerations.

1.2 Additional Reports

This report provides information on the considerations and approach by which Novatech has designed and evaluated the proposed servicing for the Greystone Village Phase 3 Condos. This report should be read in conjunction with the following:

- *Greystone Village, Phase 3, Scholastic Drive – Site Servicing Plan Review Memorandum, dated June 6, 2022, PG5383.MEMO.03 Revision 1;*
- *Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief, dated February 24, 2016 (Referred to as Master Servicing Study 2016);*
- *Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief – Phase 2 and 3, dated May 26, 2017 (Referred to as Master Servicing Study 2017);*
- *10 Des Oblats Avenue – Greystone Village 2A / 2B Mixed Use Building: Site Servicing And Stormwater Management Memorandum, dated March 11 2020;*
- *360 Deschatelets Ave – The Spencer (Greystone Village 1c Building): Site Servicing And Stormwater Management Memorandum, dated August 6, 2020;*
- *225 Scholastic Drive – Greystone Village Deschatelets Building: Site Servicing And Stormwater Management Memorandum, dated May 15, 2020;*
- *225 Scholastic Drive – Greystone Village Retirement Residence: Site Servicing And Stormwater Management Memorandum, dated March 23, 2018.*
- *Greystone Village – 175 Main Street – Potential Low Impact Development Opportunities, Prepared by Novatech, dated November 25, 2015, Ref. R-2015-182.*

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CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

KEY PLAN

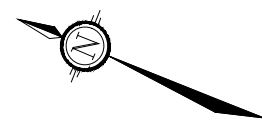
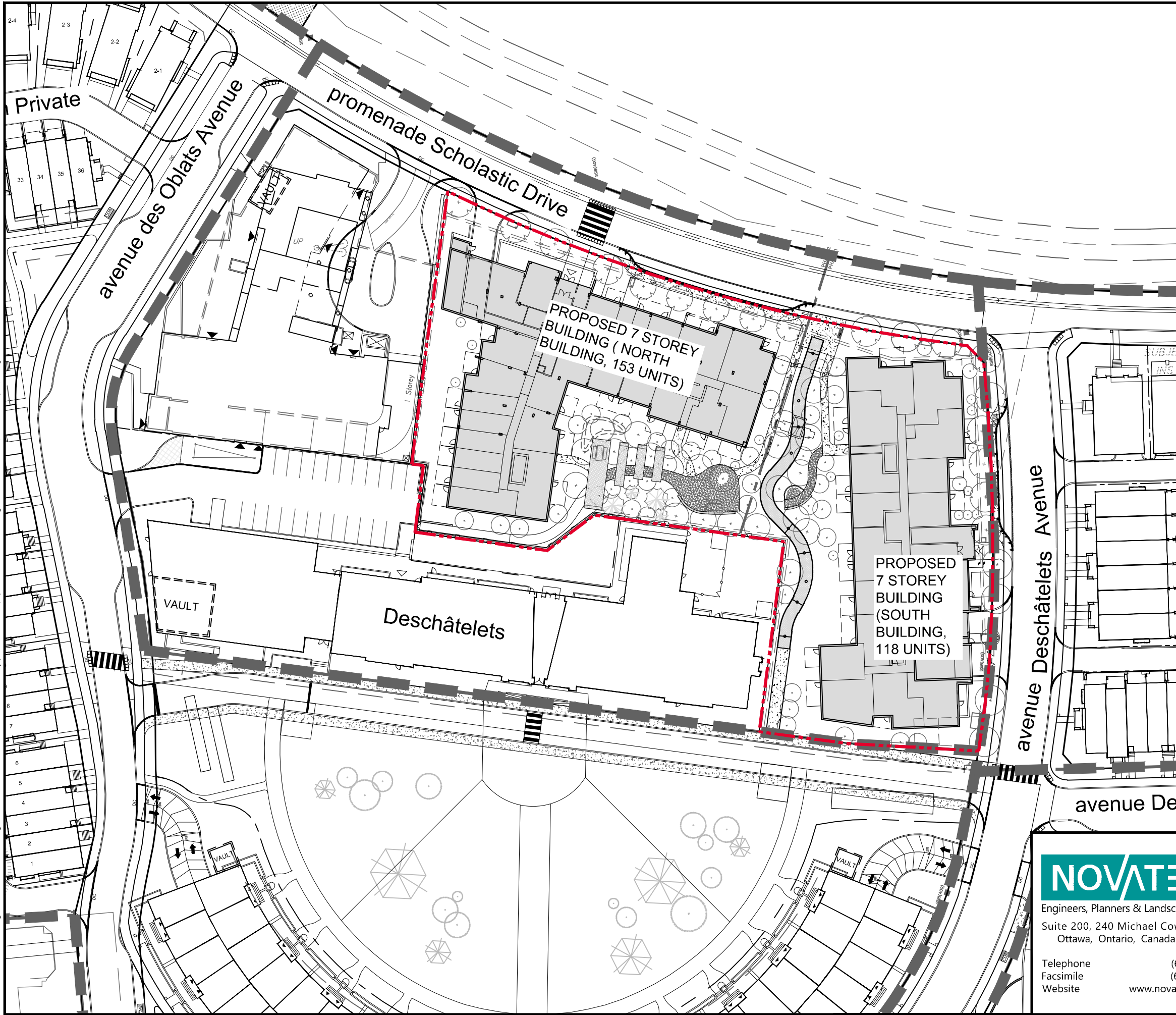
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FIGURE 1

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LEGEND

-  PROPERTY LINE
-  PHASE LINE
-  PHASE 3 CONDO SITE PROPERTY LINE

PROPOSED 7 STOREY BUILDING (NORTH BUILDING, 153 UNITS)

PROPOSED 7 STOREY BUILDING (SOUTH BUILDING, 118 UNITS)

Deschâtelets

VAULT

avenue Deschâtelets Avenue

avenue De


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175 MAIN STREET

CONCEPT PLAN - PHASE 3 CONDOS

1:750 

JUNE 2022 114025 FIGURE 2

1.3 Approvals

There have been several approvals by the Rideau Valley Conservation Authority and the Ministry of the Environment, Conservation and Parks and the City of Ottawa, which includes the phase 3 condo site. Relevant approvals are as follows. Refer to **Appendix D** for details.

MOECP

- ECA Number 0292-AP6PWR – Storm and Sanitary Sewers within Phase 2 & 3;
- ECA Number 3454-APEHFQ – Stormwater Outfall and Oil / Grit Separator within Phase 2 & 3;
- ECA Number 4082-AAZQ6P – Storm and Sanitary Sewers within Phase 1;
- ECA Number 8946-ACUP7W – Stormwater Outfall and Oil / Grit Separator within Phase 1;

RVCA

- File Number RV3-08/17 – Stormwater Outlet and Soil Remediation
- File Number RV3-56/15 – Soil Remediation Amended

The RVCA has confirmed they have no further comments and additional approvals are required for the phase 3 condo site. Refer to **Appendix D** for correspondence.

2.0 SANITARY SERVICING

The two (2) proposed 7-storey buildings at the corner of Scholastic **Avenue**/Deschâtelets Avenue will each be serviced by two 200mm dia. sanitary services that connects to the existing 250mm diameter sanitary sewers on Scholastic **Avenue** and Deschâtelets Avenue respectively complete with backwater flow valves. Refer to **Figure 3** – Hydrant Coverage and Service location Plan for locations.

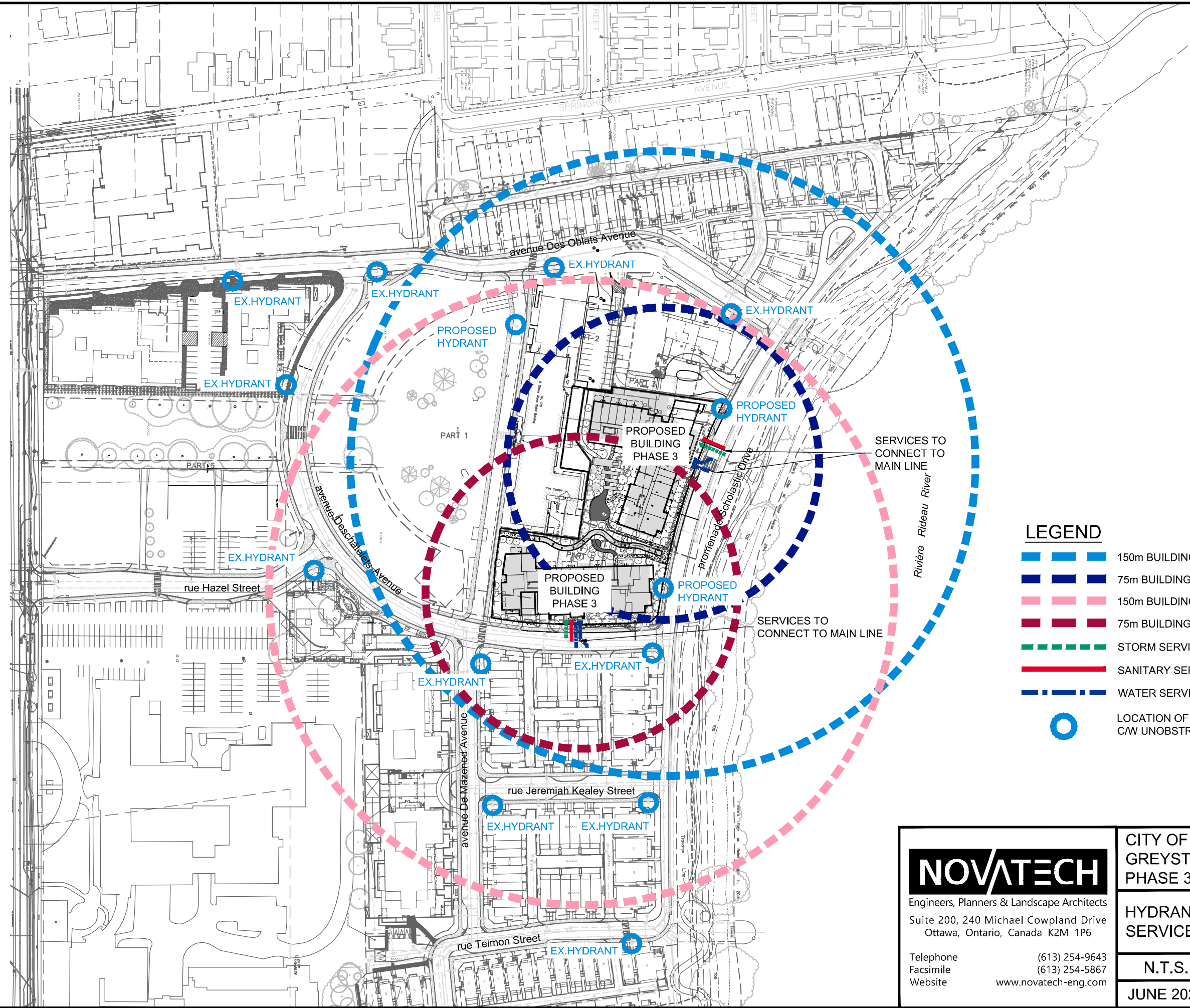
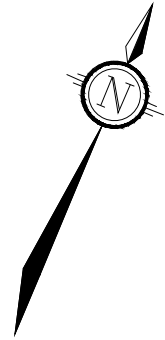
2.1 Design Criteria

2.1.1 Existing System

The master Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Briefs (2016 & 2017) as listed above were completed prior to the City of Ottawa issuing Technical Bulletin ISTB 2018-01. Therefore, the master servicing studies for Greystone Village were based on the following City of Ottawa design criteria:

- Residential Average Sewage Flow = 350 L/capita/day
- Residential Peaking Factor = Harmon Equation
- Max Peaking Factor = 4.0
- ICI Peaking factor = 1.5
- Infiltration Allowance = 0.28 L/s/ha
- Population Density:
 - 3.4/unit (Singles)
 - 2.7/unit (Towns)
 - 2.1/unit (Two Bedroom Apartment)
 - 3.1/unit (Three Bedroom Apartment)
 - 2.0/unit (School Residence)
 - 1.4/unit (Retirement Residence or Studio Apartment)

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LEGEND

- 150m BUILDING OFFSET
- 75m BUILDING OFFSET
- 150m BUILDING OFFSET
- 75m BUILDING OFFSET
- STORM SERVICE
- SANITARY SERVICE
- WATER SERVICE
- LOCATION OF EXISTING FIRE HYDRANT
C/W UNOBSTRUCTED PATH OF TRAVEL TO BUILDING

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CITY OF OTTAWA
GREYSTONE VILLAGE
PHASE 3

HYDRANT COVERAGE AND
SERVICE LOCATION PLAN

N.T.S.

JUNE 2022 | 114025-PH3 | FIG-3

- Minimum Pipe Slope (200mm) = 0.32%
- Minimum Full Flow Velocity = 0.6m/s
- Maximum Full Flow Velocity = 3.0m/s

2.1.2 Proposed System

The current sanitary design is based on design criteria outlined in the City of Ottawa's Technical Bulletin ISTB 2018-01 and are as follows:

- Residential Average Sewage Flow = 280 L/capita/day
- Residential Peaking Factor = Harmon Equation
- Max Peaking Factor = 4.0
- ICI Peaking factor = 1.5 if ICI >20%, 1.0 <20%
- Infiltration Allowance = 0.33 L/s/ha
- Population Density:
 - 3.4/unit (Singles)
 - 2.7/unit (Towns)
 - 2.1/unit (Two Bedroom Apartment)
 - 3.1/unit (Three Bedroom Apartment)
 - 2.0/unit (School Residence)
 - 1.4/unit (Retirement Residence or Studio Apartment)
- Minimum Pipe Slope (200mm) = 0.32%
- Minimum Full Flow Velocity = 0.6m/s
- Maximum Full Flow Velocity = 3.0m/s

2.2 Proposed Sanitary Design – Phase 3 Condos

2.2.1 Master Servicing Study Flows

Based on the criteria listed in Section 2.1.1 the calculated peak sanitary design flows for Greystone Village Phase 3 Condo Area as per the Master Servicing Studies was determined to be:

- Phase 1A/1B (Outlet 1) = 1.83 L/s
- Phase 2/3 (Outlet 2) = 1.15 L/s

The peak sanitary flows are summarized below in **Table 2.1**. Refer to **Appendix A** for approved Sanitary Design Sheets and to the Sanitary Drainage Areas Plans (Drawings **114025-SAN1**, **SAN2** and **SAN1-B**) for additional information. Note that Area B13 for phase 2/3 outlet includes the existing Deschatelets building outletting to Scholastic Drive in the MSS. This value has been netted out of the calculations for peak sanitary flow from the phase 3 site.

Table 2.1: Phase 3 Condo Site - Sanitary Flow Summary As per Master Servicing Studies
**

Development Condition	Population	Peak Res. Flow (L/s)	Peak Ext. Flow (L/s)	Peak Design Flow (L/s)
Total Flow Outlet 1	107	1.73	0.10	1.83*
Total Flow Outlet 2	63	1.02	0.13	1.15

* Peak Design Flow includes extraneous flows, population flows as well as forecourt flows (not listed in above table, refer to design sheets and Sanitary Drainage Areas Plans for further details).

**Refer to *Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief – Phase 2 and 3* for further details for further details.

2.2.2 Proposed System

Based on the new criteria listed in Section 2.1.2 the calculated peak sanitary design flow for the proposed Greystone Village Phase 3 Condo site was determined to be:

- Phase 1A/1B (Outlet 1) = 2.38 L/s
- Phase 2/3 (Outlet 2) = 3.08 L/s

The peak sanitary flows are summarized below in **Table 2.2**. Refer to **Appendix A** for proposed Sanitary Design Sheets and to the revised Sanitary Drainage Areas Plans (Drawings **114025-SAN1**, **SAN2** and **SAN1-B**) for additional information.

Table 2.2: Phase 3 Condos Site - Proposed Sanitary Flow Summary

Development Condition	Population	Peak Res. Flow (L/s)	Peak Ext. Flow (L/s)	Peak Design Flow (L/s)
Total Flow Outlet 1**	198	2.26	0.13	2.38
Total Flow Outlet 2***	256	2.89	0.20	3.08

* Peak Design Flow include extraneous flows, population flows as well as forecourt flows (not listed in above table, refer to design sheets and Sanitary Drainage Areas Plans for further details).

**Refer to Sanitary Sewer Design Sheet Greystone Village, 175 Main Street 3 dated June 3, 2022

***Refer to Current Sanitary Sewer Design Sheet Greystone Village, 175 Main-Phase 2 and 3 (Outlet 2) dated June 3, 2022

The proposed sanitary design flows for Greystone Village Phase 3 site are higher than the flows accounted for in the previous master servicing studies. Therefore, a review of the overall Greystone downstream sanitary system is required and has been completed below.

2.3 Overall Downstream Sanitary System – Greystone Village

2.3.1 Existing System

Based on the criteria listed in Section 2.1.1 the calculated peak sanitary design flows for Greystone Village as per the Master Servicing Studies was determined to be:

- Phase 1A/1B (Outlet 1) = 24.03 L/s
- Phase 2/3 (Outlet 2) = 10.72 L/s

The peak sanitary flows are summarized below in **Table 2.3**. Refer to **Appendix A** for approved Sanitary Design Sheets and to the Sanitary Drainage Areas Plans (Drawings **114025-SAN1**, **SAN2** and **SAN1-B**) for additional information.

Table 2.3: Overall Greystone Sanitary Flow Summary As per Master Servicing Studies**

Development Condition	Population	Peak Res. Flow (L/s)	Peak Ext. Flow (L/s)	Peak Design Flow (L/s)
Total Flow Outlet 1	1491	22.23	1.52	24.03*

Total Flow Outlet 2	624	9.91	0.81	10.72
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* Peak Design Flow includes extraneous flows, population flows as well as forecourt flows (not listed in above table, refer to design sheets and Sanitary Drainage Areas Plans for further details).

**Refer to *Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief – Phase 2 and 3* for further details.

2.3.1 Proposed System

Based on the new criteria listed in Section 2.1.2 the calculated peak sanitary design flow for the proposed Greystone Village Phase 3 Condos was determined to be:

- Phase 1A/1B (Outlet 1) = 18.05 L/s
- Phase 2/3 (Outlet 2) = 10.56 L/s

The peak sanitary flows are summarized below in **Table 2.4**. Refer to **Appendix A** for proposed Sanitary Design Sheets and to the revised Sanitary Drainage Areas Plans (Drawings **114025-SAN1, SAN2 and SAN1-B**) for additional information.

Table 2.4: Overall Greystone Proposed Sanitary Flow Summary

Development Condition	Population	Peak Res. Flow (L/s)	Peak Ext. Flow (L/s)	Peak Design Flow (L/s)
Total Flow Outlet 1**	1567	15.91	1.83	18.05
Total Flow Outlet 2***	909	9.61	0.95	10.56

* Peak Design Flow include extraneous flows, population flows as well as forecourt flows (not listed in above table, refer to design sheets and Sanitary Drainage Areas Plans for further details).

**Refer to Sanitary Sewer Design Sheet Greystone Village, 175 Main Street 3 dated June 3, 2022

***Refer to Current Sanitary Sewer Design Sheet Greystone Village, 175 Main-Phase 2 and 3 (Outlet 2) dated June 3, 2022

2.4 Sanitary Conclusion

For the proposed Phase 3 Condo site there is a 0.55 L/s increase of peak sanitary flow into the existing Outlet 1 and an increase of 2.01 L/s of peak sanitary flow into the existing Outlet 2 compared to the sanitary flows accounted for in the master servicing studies. An analysis of the downstream system was completed.

An analysis of the overall Greystone Village downstream system showed that there is a 5.98 L/s reduction of peak sanitary flow into the existing Outlet 1 and a reduction of 0.16 L/s for Outlet 2. Sanitary flows have been updated for the entire development including as-built information for past site plans to reflect current unit counts and design criteria as per Technical Bulletin ISTB 2018-01. The flows have decreased in comparison to the previously accounted flows in the master servicing reports (2016, 2017). Therefore, the existing sanitary sewers adequate capacity to accommodate the peak sanitary flow from the Greystone Village Phase 3 Condos. Refer to **Appendix A** for proposed Sanitary Sewer Design sheet for details.

3.0 WATERMAIN

The two (2) proposed 7-storey buildings at the corner of Scholastic Drive /Deschâtelets Avenue will each be serviced by two 150mm diameter water services that will be interconnected for redundancy that connects to the existing 250mm diameter watermain on Scholastic Drive and

Deschâtelets Avenue respectively. Each building will be sprinklered using modern fire fighting equipment. Refer to **Figure 3** – Hydrant Coverage and Service location Plan for locations.

3.1 Previous Studies

The master Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Briefs (2016 & 2017) as listed above were completed prior to the City of Ottawa issuing Technical Bulletin ISTB 2018-01 & ISTB-2021-03. Therefore, the master servicing studies for Greystone Village were based on the following City of Ottawa design criteria:

Demands:

- Average Daily Demand = 350L/capita/day
- Maximum Daily Demand = 2.5 x Average Daily Demand
- Peak Hour Demand = 2.2 x Maximum Daily Demand
- Fire Flow = Fire Underwriter's Survey

Residential

- School Residence Average Flow = 70 L/student/day
- Population Density:
 - 3.4/unit (Singles)
 - 2.7/unit (Towns)
 - 2.1/unit (Apartment)
 - 2.0/unit (School Residence)
 - 1.4/unit (Retirement Residence)

System Requirements

- Maximum Allowable Pressure = 100psi (690 kPa)
- Minimum Allowable Pressure (excluding fire flow conditions) = 40psi (276 kPa)
- Minimum Allowable Pressure during fire flow conditions = 20psi (138 kPa)
- Maximum Allowable Age = 5 days (residence time = 8 days, 192 hours)

Fire Flow (maximum):

- 141.40L/s (Singles)
- 259.36L/s (Row Towns)
- 219.97L/s (Back to Back Row Towns)
- 230.67L/s (4 Storey Combustible Construction Condo)
- 251.52L/s (6 Storey Non-Combustible Construction Condo)
- 286.45L/s (9 Storey Non-Combustible Construction Condo)
- 300.24L/s (Retirement Residence) (**Fronting Phase 3 Scholastic Drive**)
- 229.03L/s (4 Storey Condo) (**Fronting Phase 3 Deschatelets Avenue**)
- 249.76L/s (School Residence)
- 133.33 (Domicile Building - offsite)
- 173.37 (Sister's Building – offsite)

Friction Factors:

Watermain Size:	C-Factor:
300mm diameter	120
200mm and 250mm diameter	110

150mm to 50mm diameter 100

The water distribution network could provide adequate system pressures under the maximum day plus fireflow and under peak hour conditions within the development.

3.2 Proposed Watermain Design

The current sanitary design is based on design criteria outlined in the City of Ottawa's Technical Bulletin ISTB 2018-01 & ISTB 2021-03 and are as follows. Fireflows are based on the FUS 2020:

Demands:

- Average Daily Demand = 280L/capita/day
- Maximum Daily Demand = 2.5 x Average Daily Demand
- Peak Hour Demand = 2.2 x Maximum Daily Demand
- Fire Flow = Fire Underwriter's Survey 2020

Residential

- School Residence Average Flow = 70 L/student/day
- Population Density:
 - 3.4/unit (Singles)
 - 2.7/unit (Towns)
 - 2.1/unit (Apartment)
 - 2.0/unit (School Residence)
 - 1.4/unit (Retirement Residence)

System Requirements:

- Maximum Pressure (System) = 690kPa (100psi)
- Maximum Pressure (Service) = 552kPa (80psi)
- Minimum Pressure (w/o fire flow) = 275kPa (40psi)
- Minimum Pressure (w/ fire flow) = 140kPa (20psi)
- Maximum Age Onsite (Quality) = 192 hours
- Friction Factor: 200mm/300mm = 110/120

Fire Flow (FUS 2020)

- Phase 3 Condos (maximum):
 - 233 L/s (7 Storey building Fronting Scholastic Drive)
 - 200 L/s (7 Storey building Fronting Deschatelets Avenue)

It should be noted that both services will need to be interconnected for redundancy at the entry into the building.

There is adequate hydrant coverage to provide 233 L/s under the fireflow operating conditions, refer to **Figure 3** – Hydrant Coverage and Service Location plan for coverage areas.

Boundary Conditions

Boundary conditions were requested based on current demand and fireflows for the phase 3 condo site. The results indicate an increase in the boundary conditions under all operating conditions. It should be noted that the maximum fireflows for the phase 3 buildings (north and south) have decreased from the fireflows accounted for in the master servicing studies fronting the phase 3 site on Scholastic Drive (N21) and Deschatelets Avenue (N22). The increase in

population demand would have a negligible effect on the peak hour condition. Therefore, it can be concluded that the existing watermain system has adequate capacity to service the proposed phase 3 condo site.

A copy of the City of Ottawa provided boundary conditions, boundary condition request and information from the master servicing studies are included in **Appendix B**.

As per correspondence from the City of Ottawa, no further analysis is required for the phase 3 condo site.

4.0 STORMWATER MANAGEMENT

The two (2) proposed 7-storey buildings at the corner of Scholastic Drive /Deschâtelets Avenue will each be serviced by a 200mm diameter storm service that connects to the existing 375mm diameter storm sewer on Scholastic Drive and 375mm diameter storm sewer and Deschâtelets Avenue respectively complete with backwater flow valves. Refer to **Figure 3 – Hydrant Coverage and Service location Plan** for locations.

4.1 Stormwater Management Criteria

The stormwater management criteria used in the design of the Greystone Village Phase 3 Condos have been based on the following:

- Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief – Phase 2 and 3 dated May 26, 2017, (Novatech, May 2017/Ref. # R-2017-089)
 - This report outlines the design criteria for all future development Greystone Village, including the proposed Greystone Village Phase 3 Condos,
- City of Ottawa Sewer Design Guidelines (October, 2012).

4.1.1 *Minor System (Storm Sewers)*

- Storm sewers on Scholastic Drive and Deschâtelets Avenue were to be designed using the Rational Method for a 1:5-year return period;
- Storm sewers on site (area drains) in the courtyard will be connected to the underground parking structure and outlet to either Deschâtelets Avenue (phase 1, south building) or Scholastic Drive (phase 2, north building). Area drains are designed to capture the 100-year flow.
- Runoff from the flat roof buildings will utilize rooftop controls to control and store up to the 100-year event prior to entering the storm sewers on Scholastic Drive and Deschâtelets Avenue. Runoff that exceeds these controls will outlet via scuppers on the roof.
- Provide additional storage (if necessary) to control runoff for all storms up to the 100-year event to the allowable minor system release rate (5-year storm).
- Ensure 100-year controlled flow rates do not exceed or have adverse effects on the existing storm system.

4.1.2 *Major System (Emergency Overland Flow)*

- Runoff that exceeds the 100-year storm event will be conveyed overland to Scholastic Drive and Deschâtelets Avenue.

4.1.3 *Water Quality Control*

- Water quality control will be provided via two Vortechnic type structures and previously approved. Refer to Section 1.2 for approved master servicing reports (2016 & 2017)

4.1.4 *Erosion and Sediment Control*

- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accord with the design drawings and that mitigation measures are being implemented as specified;
- Inserts and filter fabric are to be placed under all proposed and existing catchbasins and storm manhole covers;
- After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.

4.2 **Proposed Stormwater System**

4.2.1 *Storm Drainage Areas – Previous Studies*

The proposed sewers on Scholastic Drive and Deschâtelets Avenue were sized using the Rational Method based on a 5-year level of service. Stormwater flows from the proposed site were split between :

- Outlet 1 located within phase 1 of the Greystone Village development;
- Outlet 2 located within phase 2-3 of the Greystone Village development.

The 5-year uncontrolled stormwater flows were calculated using the new property line boundaries and the previous storm drainage areas identified in the master servicing reports. Refer to the Appendix C for Figure – Storm Drainage Areas (Fig-SDA) for previous drainage boundaries compared to phase 3 condo property line. Refer to **Appendix C** for 5-year uncontrolled release rate calculations.

The 5-year uncontrolled release rate was determined to be **67.74L/s** for Outlet 1 and **86.48L/s** for Outlet 2. Refer to **Appendix C** for release rate summary.

4.2.2 *Proposed Storm System*

Runoff from the site will be captured by a combination of area drains in the courtyard or along the building, controlled roof drains, trench drains or will directly drain overland to Scholastic Drive or Deschâtelets Avenue.

Area drains 1-6 within the courtyard and the roof drains on the northern building will outlet to the existing 375mm storm sewer on Scholastic Drive.

Area drains 7-9 within the courtyard and controlled roof drains on the southern building are proposed to outlet to the existing 375mm storm sewer on Deschâtelets Avenue. This approach is consistent with the phasing of the site.

A trench drain will be required at the bottom the underground parking ramp along Deschâtelets Avenue to capture any flows entering the parking structure. This drain will not be controlled and will be pumped at the same rate as the incoming flow, therefore no ECA will be required.

Stormwater flows will be captured via area drains, controlled roof and trench drains and conveyed to the underground parking structure where it will be gravity fed or pumped to the service lateral. Foundation drains and sub floor drains will be conveyed to the underground parking sump where it will be pumped with appropriate back up power, sufficient sized pump and back flow prevention to the existing service lateral provided. Therefore, all runoff can be connected to this service lateral.

There are no external storm flows from neighbouring properties, see **Appendix D** for correspondence.

Stormwater flows from the proposed site were calculated using the 100-year storm event for both the roof and courtyard. The courtyard area drains will convey uncontrolled flow while the roof will control and store up to the 100-year event on the roof. Refer to the Storm Drainage Area Plan (**114025-STM-PH3**) for drainage boundaries. Refer to **Appendix C** for release rate calculations.

Table 4.1 provides a summary of the proposed controlled and uncontrolled stormwater flows to Scholastic Drive and Deschâtelets Avenue.

Table 4.1 – Stormwater Flow Summary – 100-year Storm Event

Outlet	Street	Uncontrolled Flow Areas (L/s)	Roof Controlled Flow (L/s)	Total Flow (L/s)
Outlet 1 – Phase 1	Deschâtelets Avenue	49.52 (Area Drains 7-9 A06, Area A04, A05)	10.49	60.01
Outlet 2 – Phase 2-3	Scholastic Drive	64.65 (Area Drains 1-6 A02, Area A01, A03)	12.29	76.94

There is a 7.73 L/s reduction of stormwater flow into the existing Outlet 1 and a reduction of 9.54 L/s for Outlet 2 compared to the 5-year uncontrolled flow accounted for in the master servicing studies. Therefore, the existing storm sewers have adequate capacity to accommodate the stormwater flows from the Greystone Village Phase 3 Condos and will have no adverse effects on the downstream system. Refer to **Appendix C** for details.

4.2.3 Overall Storm Flows – Existing System

The master servicing studies considered controlled flows under the 100-year storm event to the minor system. Therefore, an additional check was completed to ensure there is no adverse effects on HGLs and the minor system when considering all controlled flows under the 100-year design event.

A portion of the uncontrolled flow listed in Table 4.1 (A01, A03, A04, & A05) is directly draining to either Scholastic Drive or Deschâtelets Avenue and will eventually be captured within an existing roadside catchbasin, which is controlled by an Inlet Control Device (ICD). Therefore, it is difficult to quantify the controlled release rate from the phase 3 condo site under the 100-year event without a hydraulic model.

The approved hydraulic model in the master servicing study for phase 2-3 (2017) was used as a baseline to consider the effects of the phase 3 condo site flows on the existing storm system.

Drainage patterns and flows were updated in the model for the phase 3 condo site and surrounding area only. Refer to Appendix C for model schematics. Changes in drainage patterns

include the existing Deschatelets building outletting to Oblats Avenue instead of Scholastic Drive as per the MSS. Therefore, the HGLs and flows were checked for all 3 roadways, Oblats Avenue, Scholastic Drive and Deschatelets Avenue.

Results are provided in Appendix C, which compare the updated model results (Phase 3 Condo Site) with the approved model results as per the MSS (2017).

The overall storm flows were compared at each outlet to ensure there was no increase in flows compared to the master servicing studies. The results are as follows in **Table 4.2**:

Table 4.2 – Storm Flow Summary at Outlets

Outlet	Minor System Flows – Proposed Phase 3 Condo Site Plan Application (2022)	Minor System Flows - Master Servicing Study (2016, 2017)
Outlet 1 – Phase 1	763.65 L/s	798.36 L/s
Outlet 2 – Phase 2-3	1,575.36 L/s	1,578.35 L/s

The results indicate a decrease in flow in the minor system compared to the MSS once all onsite and roadside controls are utilized.

The hydraulic model has been provided with the updated drainage patterns and flows for the phase 3 condo site and surrounding area.

4.2.4 HGL Check – Existing System

A review of the HGL levels within the pipes along Deschatelets Avenue, Scholastic Drive and Oblats Avenue were checked to ensure there are no adverse effects to the storm system. Results are provided in Appendix C. A summary of the HGLs within the pipes are provided in **Table 4.3**.

Table 4.3 – HGL Levels

Street	MH Number	HGL – Proposed Phase 3 Model (m)	HGL – Approved MSS Model (m)	Difference Compared to MSS Model (m)
Oblats Avenue	304	61.06	61.07	-0.01
	306	59.62	59.58	+0.04
	308	58.90	58.85	+0.05
	310	58.65	58.69	-0.04
	334	58.55	58.61	-0.06
Scholastic Drive	324	60.71	60.75	-0.04
	326	59.31	59.31	0.00
	328	58.73	58.78	-0.05
	334	58.55	58.61	-0.06
Deschatelets Avenue	128	61.78	61.80	-0.02
	126	61.58	61.55	+0.03
	124	60.88	60.86	+0.02
	114	60.01	60.01	0.00
	118	59.50	59.51	-0.01

The results indicate the following:

- HGLs has decreased upstream and downstream of the connections to the mainline on Oblats Avenue, Scholastic Drive and Deschatelets Avenue.
- HGLs have slightly increased in the vicinity of the connection points along Oblats (Deschatelets building) and Deschatelets Avenue (south building).
 - HGLs are below the obvert of the pipe or fronting units with waterproof basements (no connection to storm sewer);
 - USFs are based on the obvert of the pipe + 0.3m.

Even though the HGL have increased slightly at the connection points there is no risk of basement flooding under the 100-year storm event.

The hydraulic model has been provided with the updated drainage patterns and flows for the phase 3 condo site and surrounding area.

4.2.5 Major System Flows

Since a portion of the uncontrolled flow listed in Table 4.1 (A01, A03, A04, & A05) is directly draining to either Scholastic Drive or Deschatelets Avenue, overland flow depths were checked under the updated model to ensure depths of flow within the roadway have not increased.

Results indicated the depths of flow within the roadway were at or below the previously calculated flow depths in the master servicing studies with the exception of CB75 at the intersection of Oblats Avenue / Scholastic Drive, which increased by 0.01m. The phase 3 condo site has no adverse effects on the major system flows under the 100-year storm event. Results are provided in Appendix C, which compare the updated model results (Phase 3 Condo Site) with the approved model results as per the MSS (2017).

4.2.6 Major System (Emergency Overland Flow) – Phase 3

Since the minor system is capturing or controlling (roof) up to the 100-year flow, the major system has been designed to convey the emergency overland flow to either Scholastic Drive or Deschâtelets Avenue. The northern building perimeter and courtyard will be conveyed to Scholastic Avenue and the southern building (south and west) will be conveyed to Deschâtelets Avenue. This approach is generally consistent with the previous master servicing studies.

Worst case channel depth calculations were completed using the stress test (100-year +20%) near pinch points of the building and verified that the theoretical flow depth was less than the difference between the building envelope grade and the spill point. Therefore, it can be concluded that the overland flow will not touch any part of the building envelope while conveying the calculated flow during the stress test (100-year +20%). Calculations can be found in **Appendix C**.

Emergency Overland flow from Scholastic Drive and Deschâtelets Avenue ultimately outlet to the Rideau River.

There will be no storm water drainage that will enter the subject site from the neighbouring property. A site plan application of the neighbouring property has been submitted and comments received. The adjacent landowner will capture and convey all storm flow within the adjacent property and no drainage will enter the phase 3 condo site. See Appendix D – Correspondence for details from adjacent landowner.

4.2.7 Major Overland Flow Stress Test – Deschatelets Avenue

The ramp highpoint has a current elevation of 63.73m. At CB34 the total of the static and dynamic depth during the stress test (100yr + 20%) event is 0.06 m based on the revised model for the Phase 3 development. The edge of pavement elevation in front of the underground parking ramp is 63.52m, giving a flow depth of 63.58m in the roadway. There is 0.15m freeboard between the depth of flow and highpoint therefore there will be no spillage into the underground parking ramp.

4.2.8 LID Features

No specific LID features were considered for the site and were previously considered not feasible. Refer to the report, *Greystone Village – 175 Main Street – Potential Low Impact Development Opportunities*, Prepared by Novatech, dated November 25, 2015, Ref. R-2015-182.

4.2.9 Groundwater Infiltration

Since the building foundation will be waterproof and an infiltration control system and tank and being implemented for the phase 3 development, it is expected that groundwater flows post construction will be low to negligible (2,000L/day). The infiltration control system / tank will be approved by the geotechnical consultant at the time of construction.

5.0 NOISE

An analysis of the roadway traffic along Mainstreet to the West and Highway 417 to the North indicates that the indoor sound levels for all condo buildings north of the existing Deschâtelets building will not exceed the maximum allowable limits outlined in the City of Ottawa's Environmental Noise Control Guidelines and therefore noise attenuation measures for the buildings will not be necessary for the Greystone Village Phase 3 Condos.

All condo buildings and site plans south of the existing Deschâtelets building are outside the 500m limit and are not subject to noise analysis.

The detailed results are included in the Noise Impact Assessment Report. Refer to "Greystone Village, 175 Main Street – Site Servicing, Stormwater Management, Noise, Erosion and Sediment Control Brief – Phase 2 and 3" dated May 26, 2017 by Novatech Engineering

6.0 UTILITIES

The phase 3 condo site will be serviced with hydro, gas, bell and rogers with connections to Scholastic Drive (north building) and Deschâtelets Avenue (south building). Canada Post will service the site with community mailboxes, as well as lobby mailboxes (condos). Site lighting will be provided along roadways, sidewalks and walkways as per City standards. OC Transpo will have a temporary turnaround at the end of Hazel Street until such time as the phase 2 is completed, which will provide a loop back to Main Street using Hazel Street, Deschâtelets Avenue as well as Oblats Avenue.

For additional information existing utility servicing, refer to the Phase 1A/1B utility plans (Drawings **114025-U1** to **U7**) as well as the Phase 2/3 utility plan (Drawing **114025-U-B**) which has been circulated to all the utilities (included as part of the drawings).

7.0 EROSION AND SEDIMENT CONTROL

Temporary erosion and sediment control measures will be implemented during construction in accordance with the "Guidelines on Erosion and Sediment Control for Urban Construction Sites" (Government of Ontario, May 1987). Details will be provided on the Erosion and Sediment Control Plan. Erosion and sediment control measures may include:

- Placement of insert in catchbasins and filter fabric under all maintenance holes;
- Silt fences around the area under construction placed as per OPSS 577 and OPSD 219.110;
- Light duty straw bale check dam per OPSD 219.180; and
- Application of topsoil and sod to disturbed areas.

The erosion and sediment control measures are to be installed to the satisfaction of the engineer, the City, and conservation authority prior to construction and will remain in place during construction until vegetation is established. The erosion and sediment control measures will also be subject to regular inspection to ensure the measures are operational.

8.0 CONCLUSIONS

This report confirms the proposed Greystone Village Phase 3 Condo development can be adequately serviced with storm and sanitary sewers and watermain. The report is summarized below:

Sanitary Servicing

- The sanitary flows have increased for the phase 3 condo site compared to the calculated flows in the master servicing studies, therefore an analysis of the overall Greystone system was completed.
- Sanitary flows have been updated to reflect current unit counts and design criteria as per Technical Bulletin ISTB 2018-01. The sanitary flows for outlet 1 and outlet 2 have decreased in comparison to the previously accounted flows in the master servicing reports (2016, 2017). Therefore, the existing sanitary sewers with Greystone and downstream of Greystone have adequate capacity to accommodate the peak sanitary flow from the Greystone Village Phase 3 Condo site.

Watermain

- The existing 250mm dia. watermain on Deschâtelets Avenue and Scholastic Drive are looped from the existing 200mm dia. watermain on Clegg Street to the new 400mm dia. watermain on Main Street.
- The existing onsite watermain can adequately service the site under all operating conditions including fireflow. There are sufficient existing hydrants to provide the required fireflow for the phase 3 condo buildings.

Stormwater Management

- The two (2) proposed 7-storey buildings at the corner of Scholastic Drive /Deschâtelets Avenue will each be serviced by a 200mm diameter storm service that connects to the existing 375mm diameter storm sewer on Scholastic Drive and 375mm diameter storm sewer and Deschâtelets Avenue respectively complete with backwater flow valves.
- Runoff from the site will be captured by a combination of area drains in the courtyard or along the building, controlled roof drains, trench drains or will drain overland to Scholastic Drive or Deschâtelets Avenue.
- There is a 7.73 L/s reduction of stormwater flow under the 100-year event into the existing Outlet 1 and a reduction of 9.54 L/s for Outlet 2 compared to the 5-year uncontrolled flows from master servicing studies. Therefore, the existing storm sewers have adequate capacity to accommodate the stormwater flows from the Greystone Village Phase 3 Condos and will have no adverse effects on the downstream system.
- Overall stormwater flows entering the minor system under the 100-year event have decreased compared to 100-year controlled flows in the master servicing studies.
- The HGLs have increased slightly in the vicinity of the connection points and have decreased upstream and downstream of the connection points. The HGLs that have increased are below the obvert of the pipe, therefore no basement flooding is anticipated.
- Depth of flow within Oblats Avenue, Scholastic Drive and Deschâtelets Avenue are at or below depths of flows calculated in the master servicing studies with the exception of CB75, which increase by 0.01m.
- Emergency Overland flow from Scholastic Drive and Deschâtelets Avenue ultimately outlet to the Rideau River. Overland flow will not touch any part of the building envelope while conveying the calculated flow during the stress test (100-year +20%).

Noise

- The indoor noise levels will be mitigated to achieve the required sound levels. The outdoor amenity areas are below the required noise level threshold. Further conclusions are provided in the Noise Impact Assessment Report dated dated May 26, 2017 by Novatech Engineering.

Utilities

- The development will be serviced by hydro, phone, gas, and cable from the existing services Deschâtelets Avenue and Scholastic Drive.

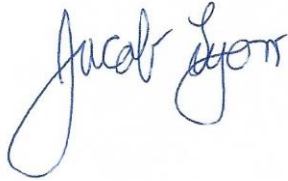
Erosion and Sediment Control

- Erosion and sediment control measures will be implemented prior to construction and remain in place until vegetation is established.

This report is respectfully submitted for site plan approval. Please contact the undersigned should you have questions or require additional information.

NOVATECH

Prepared by:



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Engineering Intern | Land Development Engineering

Reviewed by:



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Project Coordinator | Land Development Engineering

Appendix A
Sanitary Design

Approved Material as per Master Servicing Studies

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street
Developer: Greystone Village Inc.
Additional Condo Units



PROJECT # : 114025
DESIGNED BY : SZ
CHECKED BY : JAG
DATE PREPARED : 15-Dec-15
DATE REVISED : 04-Apr-16
DATE REVISED : 21-Jun-16
DATE REVISED : 15-Mar-17
DATE REVISED : 26-May-17

LOCATION				INDIVIDUAL							CUMULATIVE		PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER								
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	Condo Units	Future School Residence	Retirement Home Units	Population (in 1000's)	AREA (ha.)	Population (in 1000's)	AREA (ha.)					LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap	
*DESCHATELETS AVENUE	151	149	1&2		21	80			0.225	0.64	0.225	0.640	4.0	3.64	0.18	3.82	30.6	200	203.20	DR 35	0.65	27.6	0.85	14%	
*DESCHATELETS AVENUE	149	147	3							0.05	0.225	0.690	4.0	3.64	0.19	3.83	27.8	200	203.20	DR 35	0.40	21.6	0.67	18%	
*DESCHATELETS AVENUE	147	145	4							0.31	0.225	1.000	4.0	3.64	0.28	3.92	33.6	200	203.20	DR 35	0.40	21.6	0.67	18%	
*DESCHATELETS AVENUE	145	193	5		21		112		0.281	0.74	0.505	1.740	4.0	8.13	0.49	8.62	20.2	200	203.20	DR 35	0.40	21.6	0.67	40%	
*DESCHATELETS AVENUE	193	143									0.505	1.74	4.0	8.13	0.49	8.62	20.2	200	203.20	DR 35	0.40	21.6	0.67	40%	
DESCHATELETS AVENUE	143	141	6			75			0.158	0.21	0.663	1.95	3.9	10.49	0.55	11.04	31.1	200	203.20	DR 35	0.40	21.6	0.67	51%	
DESCHATELETS AVENUE	141	139	7							0.08	0.663	2.030	3.9	10.49	0.57	11.06	27.0	200	203.20	DR 35	0.40	21.6	0.67	51%	
DESCHATELETS AVENUE	139	133	8							0.09	0.663	2.120	3.9	10.49	0.59	11.09	21.8	200	203.20	DR 35	0.40	21.6	0.67	51%	
**FORECOURT			9						0.149	0.80			1.5	0.05	0.22	0.28									
DE MAZENOD AVENUE	133	131	10		12	102			0.247	0.47	0.910	2.590	3.8	14.10	0.73	14.82	75.3	200	203.20	DR 35	0.40	21.6	0.67	68%	
DE MAZENOD AVENUE	105	131	11		12	102			0.247	0.48	0.247	0.480	4.0	4.00	0.13	4.13	73.6	200	203.20	DR 35	0.40	21.6	0.67	19%	
JEREMIAH KEALEY STREET	131	129	12		6				0.016	0.19	1.172	3.260	3.8	17.83	0.91	19.02	47.7	250	254.00	DR 35	0.40	39.2	0.77	48%	
JEREMIAH KEALEY STREET	129	127	13		6				0.016	0.19	1.189	3.450	3.8	18.06	0.97	19.30	48.7	250	254.00	DR 35	0.40	39.2	0.77	49%	
DESCHATELETS AVENUE	133	135	14		3	47			0.107	0.34	0.107	0.340	4.0	1.73	0.10	1.83	51.2	200	203.20	DR 35	0.65	27.6	0.85	7%	
DESCHATELETS AVENUE	135	137	15		3	20			0.050	0.13	0.157	0.470	4.0	2.54	0.13	2.67	49.3	200	203.20	DR 35	0.40	21.6	0.67	12%	
SCHOLASTIC DRIVE	137	127	16	4					0.014	0.19	0.171	0.660	4.0	2.76	0.18	2.95	69.9	200	203.20	DR 35	0.40	21.6	0.67	14%	
SCHOLASTIC DRIVE	127	125	17	4					0.014	0.17	1.373	4.280	3.7	20.61	1.20	22.09	59.6	250	254.00	DR 35	0.40	39.2	0.77	56%	
SCHOLASTIC DRIVE	125	109									1.373	4.280	3.7	20.61	1.20	22.09	13.6	250	254.00	DR 35	0.40	39.2	0.77	56%	
PHILOSOPHER PRIVATE	101	111	18	4					0.014	0.17	0.014	0.170	4.0	0.22	0.05	0.27	24.8	200	203.20	DR 35	0.65	27.6	0.85	1%	
TELMON STREET	111	103	19							0.07	0.014	0.240	4.0	0.22	0.07	0.29	17.1	200	203.20	DR 35	0.40	21.6	0.67	1%	
TELMON STREET	103	105	20	1					0.003	0.03	0.017	0.270	4.0	0.28	0.08	0.35	8.4	200	203.20	DR 35	0.40	21.6	0.67	2%	
TELMON STREET	105	107	21	7	3				0.032	0.26	0.049	0.530	4.0	0.79	0.15	0.94	46.3	200	203.20	DR 35	0.40	21.6	0.67	4%	
TELMON STREET	107	109	22	4	3				0.022	0.21	0.071	0.740	4.0	1.14	0.21	1.35	39.7	200	203.20	DR 35	0.40	21.6	0.67	6%	
OUTLET	109	113									1.443	5.020	3.7	21.58	1.41	23.26	11.9	250	254.00	DR 35	0.40	39.2	0.77	59%	
OUTLET	113	115	23							0.04	1.443	5.060	3.7	21.58	1.42	23.27	43.3	250	254.00	DR 35	5.13	140.5	2.77	17%	
CLEGG	123	121	24	6					0.020	0.19	0.020	0.190	4.0	0.33	0.05	0.38	72.5	200	203.20	DR 35	3.16	60.8	1.88	1%	
CLEGG	121	117	25	8					0.027	0.18	0.048	0.370	4.0	0.77	0.10	0.87	77.0	200	203.20	DR 35	0.40	21.6	0.67	4%	
CLEGG	117	115									0.048	0.370	4.0	0.77	0.10	0.87	9.5	200	203.20	DR 35	0.42	22.2	0.68	4%	
OUTLET	115	119									1.491	5.430	3.7	22.23	1.52	24.03	10.6	250	254.00	DR 35	0.41	39.7	0.78	60%	

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street
Developer: Greystone Village Inc.
Additional Condo Units



PROJECT # : 114025
 DESIGNED BY : SZ
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 DATE PREPARED : 15-Dec-15
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LOCATION			INDIVIDUAL								CUMULATIVE			PROPOSED SEWER										
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	Condo Units	Future School Residence	Retirement Home Units	Population (in 1000's)	AREA (ha.)	Population (in 1000's)	AREA (ha.)	PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap

*Part of future phase 2 outletting through phase 1A at outlet 1.

Notes:

1. $Q(d) = Q(p) + Q(i)$
2. $Q(i) = 0.28 \text{ L/sec/ha}$
3. $Q(p) = (P \times q \times M / 86,400)$

Definitions:

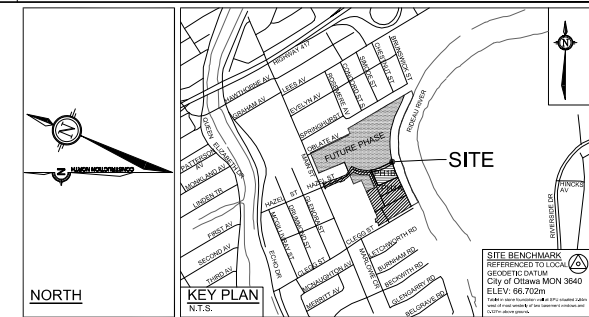
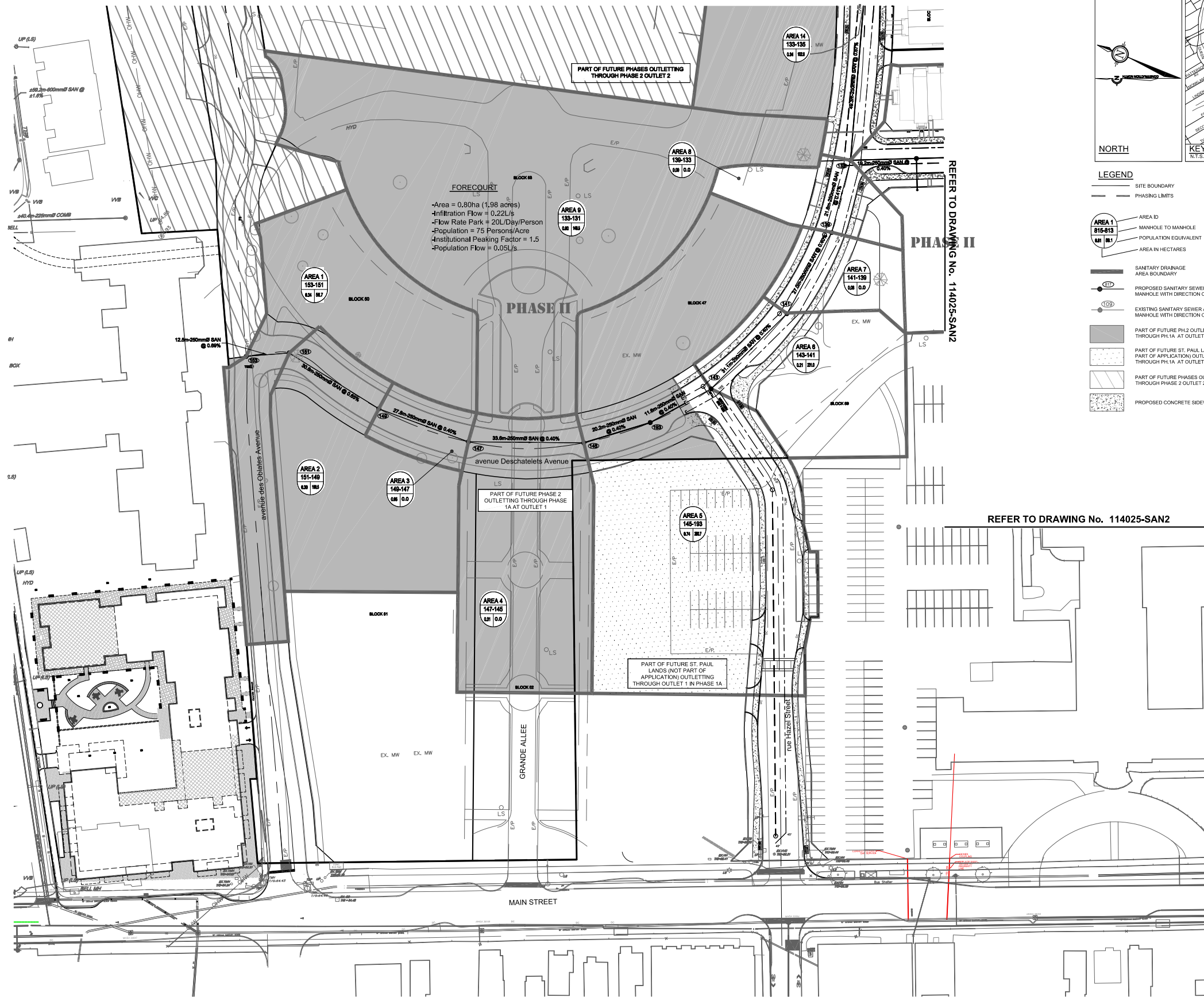
- $Q(d)$ = Design Flow (L/sec)
 $Q(p)$ = Population Flow (L/sec)
 $Q(i)$ = Extraneous Flow (L/sec)

P = Population (3.4 persons/single unit, 2.7 persons/townhouse, 2.1 persons/apartment, 2.0 persons/ school residence, 1.4 persons/retirement residence)

- q = Average per capita flow = 350 L/cap/day - Residential
 q = Average per gross ha. flow = 35000 L/gross ha/day - Light industrial
 q = Average per gross ha. flow = 50000 L/gross ha/day - Commercial/Mixed use
 M = Harmon Formula (maximum of 4.0)
 Min pipe size 200mm @ min. slope 0.32%

** Parkland: Area = 0.91 ha, Flow Rate for parks with flush toilets = 20L/Day/Person, peak design flow from parkland to be added to peak design flow of subsequent pipes.

Population = 75 Persons/acre Details from Appendix 4-A OSDG
 Institutional Peaking factor = 1.5



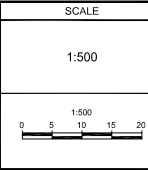
- LEGEND**
- SITE BOUNDARY
 - - - PHASING LIMITS
 - AREA ID
 - MANHOLE TO MANHOLE
 - POPULATION EQUIVALENT
 - AREA IN HECTARES
 - SANITARY DRAINAGE AREA BOUNDARY
 - PROPOSED SANITARY SEWER AND MANHOLE WITH DIRECTION OF FLOW
 - EXISTING SANITARY SEWER AND MANHOLE WITH DIRECTION OF FLOW
 - ▨ PART OF FUTURE PH2 OUTLETTING THROUGH PH 1A AT OUTLET 1
 - ▨ PART OF FUTURE ST. PAUL LANDS (NOT PART OF APPLICATION) OUTLETTING THROUGH PH 1A AT OUTLET 1
 - ▨ PART OF FUTURE PHASES OUTLETTING THROUGH PHASE 2 OUTLET 2
 - ▨ PROPOSED CONCRETE SIDEWALK

REFER TO DRAWING No. 114025-SAN2

REFER TO DRAWING No. 114025-SAN2

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY
4.	REVISED AS PER CITY COMMENTS & ISSUED FOR E.C.A.	MAY 24/16	JAG
3.	ISSUED FOR TENDER	APR 29/16	JAG
2.	REVISED AS PER CITY COMMENTS	APR 13/16	JAG
1.	ISSUED FOR CITY OF OTTAWA REVIEW	DEC 18/15	JAG



DESIGN	DESIGNER
DESIGNED	JAG
CHECKED	MSP
DRAWN	MTM
CHECKED	JAG
APPROVED	JGR

FOR REVIEW ONLY



NOVATECH
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CITY OF OTTAWA
GREYSTONE VILLAGE
175 MAIN STREET

DRAWING NAME
**SANITARY DRAINAGE AREAS PLAN
PHASE 1A AND 1B**

PROJECT No. 114025-00
REV # 4
DRAWING No. 114025-SAN1

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street - Phase 2 and 3 (Outlet 2)
Developer: Greystone Village Inc.



PROJECT # : 114025
 DESIGNED BY : SZ
 CHECKED BY : JAG
 DATE PREPARED : 18-Nov-16
 DATE REVISED: 15-Mar-17
 DATE REVISED: 26-May-17

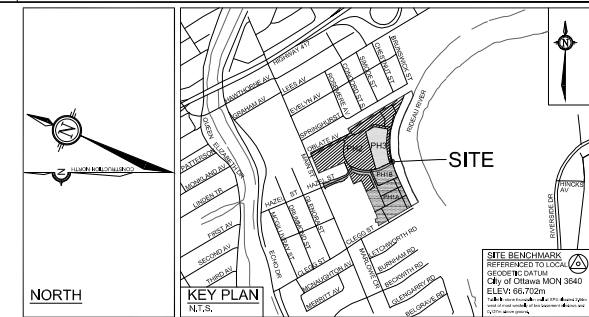
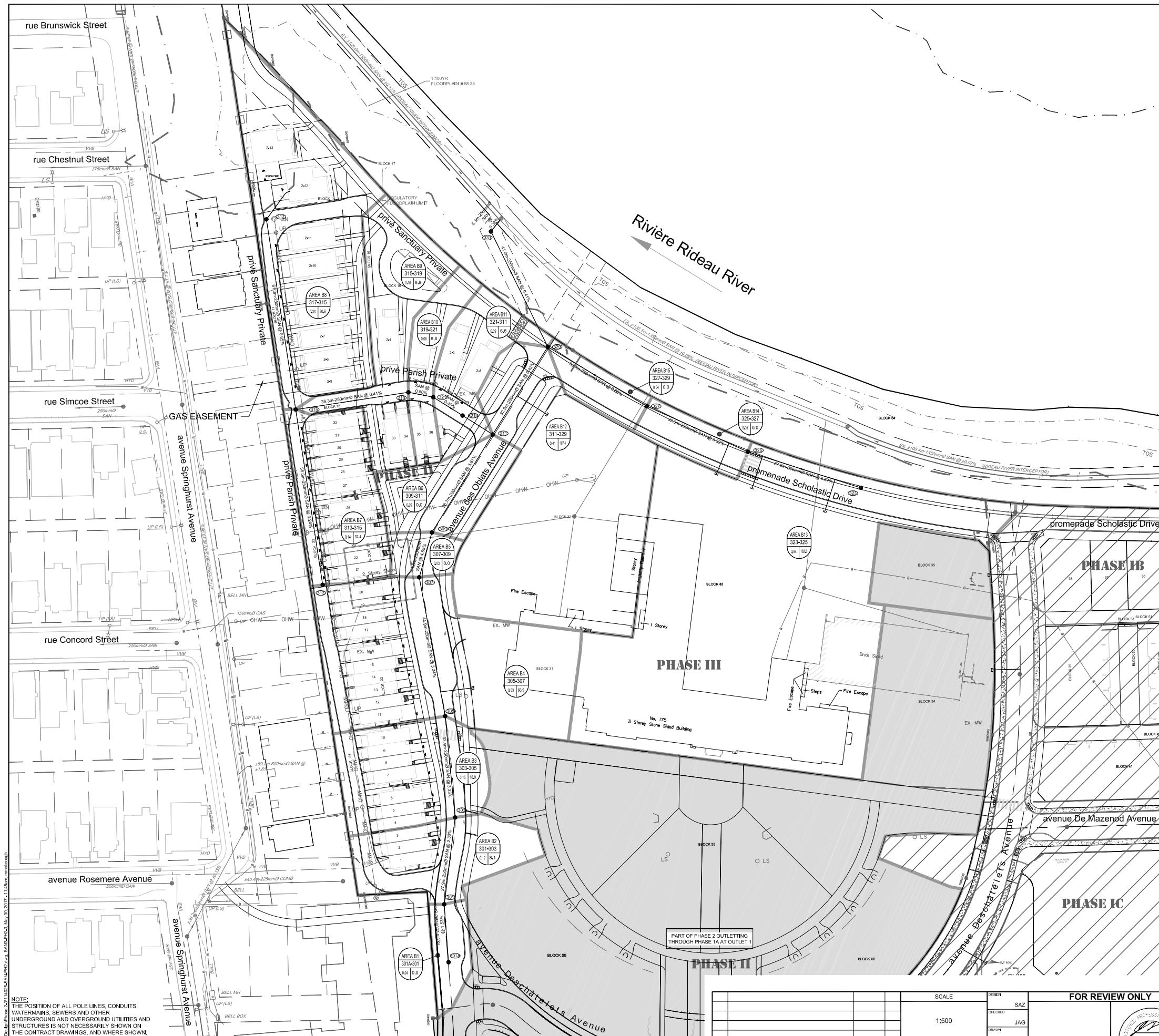
LOCATION				INDIVIDUAL						CUMULATIVE		PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER							
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	Condo Units	Retirement Home Units	Population (in 1000's)	AREA (ha.)	Population (in 1000's)	AREA (ha.)					LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap
OBLATS AVENUE	301A	301	B1					0.000	0.04	0.000	0.040	4.0	0.00	0.01	0.01	18.5	250	254.00	DR 35	5.25	142.1	2.81	0%
OBLATS AVENUE	301	303	B2		3			0.008	0.12	0.008	0.160	4.0	0.13	0.04	0.18	27.9	250	254.00	DR 35	2.30	94.1	1.86	0%
OBLATS AVENUE	303	305	B3		7			0.019	0.15	0.027	0.310	4.0	0.44	0.09	0.52	32.4	250	254.00	DR 35	3.33	113.2	2.23	0%
OBLATS AVENUE	305	307	B4		10	19		0.067	0.33	0.094	0.640	4.0	1.52	0.18	1.70	44.9	250	254.00	DR 35	3.34	113.4	2.24	2%
OBLATS AVENUE	307	309	B5					0.000	0.03	0.094	0.670	4.0	1.52	0.19	1.71	14.8	250	254.00	DR 35	4.99	138.6	2.73	1%
OBLATS AVENUE	309	311	B6					0.000	0.09	0.094	0.760	4.0	1.52	0.21	1.73	36.7	250	254.00	DR 35	3.81	121.1	2.39	1%
PARISH PRIVATE	313	315	B7		12			0.032	0.14	0.032	0.140	4.0	0.53	0.04	0.56	56.5	250	254.00	DR 35	3.24	111.7	2.20	1%
SANCTUARY PRIVATE	317	315	B8	9				0.031	0.33	0.031	0.330	4.0	0.50	0.09	0.59	61.3	250	254.00	DR 35	0.65	50.0	0.99	1%
SANCTUARY PRIVATE	315	319	B9	1	2			0.009	0.15	0.072	0.620	4.0	1.16	0.17	1.34	36.3	250	254.00	DR 35	0.41	39.7	0.78	3%
SANCTUARY PRIVATE	319	321	B10	1	2			0.009	0.06	0.081	0.680	4.0	1.31	0.19	1.50	8.1	250	254.00	DR 35	0.50	43.9	0.87	3%
SANCTUARY PRIVATE	321	311	B11	2				0.007	0.09	0.087	0.770	4.0	1.42	0.22	1.63	22.4	250	254.00	DR 35	0.40	39.2	0.77	4%
OBLATS AVENUE	311	329	B12				141	0.197	0.41	0.379	1.940	4.0	6.14	0.54	6.68	32.9	250	254.00	DR 35	0.42	40.2	0.79	17%
SCHOLASTIC DRIVE	323	325	B13			30	85	0.182	0.84	0.182	0.840	4.0	2.95	0.24	3.18	37.8	250	254.00	DR 35	3.87	122.0	2.41	3%
SCHOLASTIC DRIVE	325	327	B14			30		0.063	0.05	0.245	0.890	4.0	3.97	0.25	4.22	35.3	250	254.00	DR 35	3.32	113.0	2.23	4%
SCHOLASTIC DRIVE	327	329	B15					0.000	0.04	0.245	0.930	4.0	3.97	0.26	4.23	36.7	250	254.00	DR 35	3.88	122.2	2.41	3%
OUTLET	329	331								0.624	2.870	3.9	9.91	0.80	10.72	41.0	250	254.00	DR 35	0.41	39.7	0.78	27%
OUTLET	331	EXMH								0.624	2.870	3.9	9.91	0.80	10.72	5.3	250	254.00	DR 35	0.38	38.2	0.75	28%

- Notes:**
1. Q(d) = Q(p) + Q(i)
 2. Q(i) = 0.28 L/sec/ha
 3. Q(p) = (P x q x M / 86,400)

- Definitions:**
- Q(d) = Design Flow (L/sec)
 - Q(p) = Population Flow (L/sec)
 - Q(i) = Extraneous Flow (L/sec)

Population = 75 Persons/acre
 Institutional Peaking factor = 1.5

- P = Population (3.4 persons/single unit, 2.7 persons/townhouse, 2.1 persons/apartment, 1.4 persons/retirement residence)
- q = Average per capita flow = 350 L/cap/day - Residential
- q = Average per gross ha. flow = 35000 L/gross ha/day - Light industrial
- q = Average per gross ha. flow = 50000 L/gross ha/day - Commercial/Mixed use
- M = Harmon Formula (maximum of 4.0)
- Min pipe size 250mm @ min. slope 0.24%



- LEGEND**
- SITE BOUNDARY
 - - - PHASING LIMITS
 - PROPOSED BARRIER CURB
 - AREA ID
 - MANHOLE TO MANHOLE
 - POPULATION EQUIVALENT
 - AREA IN HECTARES
 - SANITARY DRAINAGE AREA BOUNDARY
 - PROPOSED SANITARY SEWER AND MANHOLE
 - POPOSED SANITARY SEWER WITH DIRECTION OF FLOW
 - EXISTING SANITARY SEWER AND MANHOLE
 - EXISTING SANITARY SEWER WITH DIRECTION OF FLOW
 - PART OF PH2 OUTLETTING THROUGH PH1A AT OUTLET 1
 - PHASE 1A AND 1B OUTLETTING THOUGH PH1A AT OUTLET 1
 - PROPOSED CONCRETE SIDEWALK

REFER TO DRAWING No. 114025-SAN2

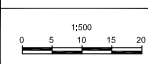
NOTE:
 THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED, BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

REFER TO DRAWING No. 114025-SAN1

PART OF PHASE 2 OUTLETTING THROUGH PHASE 1A AT OUTLET 1

REFER TO 114025-SAN1 & SAN2 FOR ADDITIONAL DRAINAGE DETAILS OUTLETTING TO PHASE 1A OUTLET 1

		SCALE	DESIGN	FOR REVIEW ONLY	
		1:500	SAZ		
			CHECKED		
			JAG		
			DRAWN		
			SAZ		
			CHECKED		
			JAG		
			APPROVED		
			MSP		
NO.	REVISION	DATE	BY		
3.	REVISED AS PER CITY COMMENTS AND ISSUED FOR I.C.A.	MAY 26/17	JAG		
2.	RE-ISSUED AS PER CITY COMMENTS	MAR 6/17	JAG		
1.	ISSUED FOR CITY OF OTTAWA REVIEW	NOV 21/16	JAG		



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CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

DRAWING NAME
**SANITARY DRAINAGE AREAS PLAN
 PHASE 2 AND 3
 (OUTLETTING THROUGH SANITARY
 OUTLET 2)**

PROJECT NO. 114025-SAN2
 REV #3
 DRAWING NO. 114025-SAN1-B

D07-16-15-0001 PHASE 2 AND 3

Proposed Material for Phase 3 Condo Site

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street
Developer: Greystone Village Inc.
Additional Condo Units



PROJECT # : 114025
 DESIGNED BY : SZ
 CHECKED BY : MSP
 DATE PREPARED : 15-Dec-15
 DATE REVISED : 4-Apr-16
 DATE REVISED : 21-Jun-16 17-Oct-16 5-Jan-17 27-Apr-17 As-Built
 DATE REVISED : 3-Jun-22

LOCATION			INDIVIDUAL								CUMULATIVE				PROPOSED SEWER												
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	3 Bedroom Condo Units	2 Bedroom Condo Units	Future School Residence	Retirement Home Units or Studio Apartment	Population (in 1000's)	AREA (ha.)	Population (in 1000's)	AREA (ha.)	PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap		
*DESCHATELETS AVENUE	153	151																									
Not Installed																											
*DESCHATELETS AVENUE	151	149	1							0.000	0.21	0.000	0.210	3.8	0.00	0.07	0.07	30.4	200	203.20	DR 35	0.72	29.0	0.90	0%		
*DESCHATELETS AVENUE	149	147	2		21		119			0.307	0.48	0.307	0.690	3.5	3.44	0.23	3.67	28.0	200	203.20	DR 35	0.29	18.4	0.57	20%		
**FORECOURT			9							0.000	0.80			1.5	0.05	0.26	0.32										
*DESCHATELETS AVENUE	147	145	3							0.000	0.31	0.307	1.000	3.5	3.44	0.33	4.08	33.8	200	203.20	DR 35	0.30	18.7	0.58	22%		
*DESCHATELETS AVENUE	145	193	4		21			112		0.281	0.68	0.587	1.680	3.3	6.38	0.55	7.25	20.1	200	203.20	DR 35	0.40	21.6	0.67	33%		
*DESCHATELETS AVENUE	193	143	5							0.000	0.06	0.587	1.74	3.3	6.38	0.57	7.27	21.2	200	203.20	DR 35	0.19	14.9	0.46	49%		
DESCHATELETS AVENUE	143	141	6							0.000	0.05	0.587	1.79	3.3	6.38	0.59	7.28	30.9	200	203.20	DR 35	0.46	23.2	0.72	31%		
DESCHATELETS AVENUE	141	139	7				43		42	0.149	0.23	0.736	2.020	3.3	7.89	0.67	8.87	26.3	200	203.20	DR 35	0.38	21.1	0.65	42%		
DESCHATELETS AVENUE	139	133	8							0.000	0.09	0.736	2.110	3.3	7.89	0.70	8.90	21.6	200	203.20	DR 35	0.55	25.4	0.78	35%		
					N																						
DE MAZENOD AVENUE	133	195	10A		1					0.003	0.04	0.739	2.150	3.3	7.92	0.71	8.94	17.9	200	203.20	DR 35	0.67	28.0	0.86	32%		
DE MAZENOD AVENUE	195	131	10B		11		61		45	0.221	0.44	0.960	2.590	3.2	10.11	0.85	11.28	56.6	200	203.20	DR 35	0.42	22.2	0.68	51%		
DE MAZENOD AVENUE	105	197	11A		1					0.003	0.04	0.003	0.040	3.8	0.03	0.01	0.05	17.5	200	203.20	DR 35	0.57	25.8	0.80	0%		
DE MAZENOD AVENUE	197	131	11B		11		61		45	0.221	0.44	0.224	0.480	3.5	2.54	0.16	2.70	56.5	200	203.20	DR 35	0.42	22.2	0.68	12%		
JEREMIAH KEALEY STREET	131	129	12		6					0.016	0.19	1.200	3.260	3.2	12.43	1.08	13.82	47.5	250	254.00	DR 35	0.23	29.8	0.59	46%		
JEREMIAH KEALEY STREET	129	127	13		6					0.016	0.19	1.216	3.450	3.2	12.59	1.14	14.04	49.1	250	254.00	DR 35	0.35	36.7	0.72	38%		
DESCHATELETS AVENUE	133	135	14		3		35		83	0.198	0.39	0.198	0.390	3.5	2.26	0.13	2.38	49.6	200	203.20	DR 35	0.71	28.8	0.89	8%		
DESCHATELETS AVENUE	135	137	15		3					0.008	0.13	0.206	0.520	3.5	2.35	0.17	2.52	51.5	200	203.20	DR 35	0.39	21.4	0.66	12%		
SCHOLASTIC DRIVE	137	127	16	4						0.014	0.23	0.220	0.750	3.5	2.49	0.25	2.74	70.4	200	203.20	DR 35	0.41	21.9	0.68	13%		
SCHOLASTIC DRIVE	127	125	17	4						0.014	0.18	1.449	4.380	3.2	14.80	1.45	16.56	59.1	250	254.00	DR 35	0.59	47.7	0.94	35%		
SCHOLASTIC DRIVE	125	109								0.000		1.449	4.380	3.2	14.80	1.45	16.56	12.5	250	254.00	DR 35	0.48	43.0	0.85	39%		
PHILOSOPHER PRIVATE	101	111	18	4						0.014	0.16	0.014	0.160	3.7	0.16	0.05	0.22	24.6	200	203.20	DR 35	0.81	30.8	0.95	1%		
TELMON STREET	111	103	19							0.000	0.08	0.014	0.240	3.7	0.16	0.08	0.24	15.5	200	203.20	DR 35	0.45	23.0	0.71	1%		
TELMON STREET	103	105	20	1						0.003	0.04	0.017	0.280	3.7	0.20	0.09	0.30	10.0	200	203.20	DR 35	0.40	21.6	0.67	1%		
TELMON STREET	105	107	21	6	3					0.029	0.25	0.046	0.530	3.7	0.54	0.17	0.71	45.7	200	203.20	DR 35	0.39	21.4	0.66	3%		
TELMON STREET	107	109	22	5	3					0.025	0.23	0.071	0.760	3.6	0.83	0.25	1.08	42.3	200	203.20	DR 35	0.57	25.8	0.80	4%		
OUTLET	109	113								0.000		1.520	5.140	3.1	15.46	1.70	17.48	11.6	250	254.00	DR 35	0.52	44.7	0.88	39%		
OUTLET	113	115	23							0.000	0.04	1.520	5.180	3.1	15.46	1.71	17.49	43.6	250	254.00	DR 35	3.81	121.1	2.39	14%		
CLEGG	123	121	24	6						0.020	0.18	0.020	0.180	3.7	0.24	0.06	0.30	74.8	200	203.20	DR 35	3.09	60.1	1.85	1%		
CLEGG	121	117	25	8						0.027	0.18	0.048	0.360	3.7	0.56	0.12	0.68	78.6	200	203.20	DR 35	0.38	21.1	0.65	3%		
CLEGG	117	115								0.000		0.048	0.360	3.7	0.56	0.12	0.68	6.6	200	203.20	DR 35	1.20	37.5	1.16	2%		
OUTLET	115	119								0.000		1.567	5.540	3.1	15.91	1.83	18.05	11.2	250	254.00	DR 35	0.36	37.2	0.73	48%		

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street
Developer: Greystone Village Inc.
 Additional Condo Units



PROJECT # : 114025
 DESIGNED BY : SZ
 CHECKED BY : MSP
 DATE PREPARED : 15-Dec-15
 DATE REVISED : 4-Apr-16
 DATE REVISED : 21-Jun-16 17-Oct-16 5-Jan-17 27-Apr-17 As-Built
 DATE REVISED : 3-Jun-22

LOCATION			INDIVIDUAL									CUMULATIVE			PROPOSED SEWER										
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	3 Bedroom Condo Units	2 Bedroom Condo Units	Future School Residence	Retirement Home Units or Studio Apartment	Population (in 1000's)	AREA (ha.)	Population (in 1000's)	AREA (ha.)	PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap

*Part of future phase 2 outletting through phase 1A at outlet 1.

Phase 3 Condo

Notes:

1. $Q(d) = Q(p) + Q(i)$
2. $Q(i) = 0.33 \text{ L/sec/ha}$
3. $Q(p) = (P \times q \times M / 86,400)$

Definitions:

- Q(d) = Design Flow (L/sec)
 Q(p) = Population Flow (L/sec)
 Q(i) = Extraneous Flow (L/sec)

** Parkland: Area = 0.80 ha, Flow Rate for parks with flush toilets = 20L/Day/Person, peak design flow from parkland to be added to peak design flow of subsequent pipes.

Population = 75 Persons/acre Details from Appendix 4-A OSDG
 Institutional Peaking factor = 1.5 if ICI >20%, 1.0 <20%

P = Population (3.4 persons/single unit, 2.7 persons/townhouse, 3.1 persons/3-bed apartment, 2.1 persons/2-bed apartment, 2.0 persons/ school residence, 1.4 persons/retirement residence or studio apartment)
 q = Average per capita flow = 280 L/cap/day - Residential
 q = Average per gross ha. flow = 35000 L/gross ha/day - Light industrial
 q = Average per gross ha. flow = 28000 L/gross ha/day - Commercial/Mixed use/Institutional
 M = Harmon Formula (maximum of 4.0)
 Min pipe size 200mm @ min. slope 0.32%

SANITARY SEWER DESIGN SHEET
Greystone Village - 175 Main Street - Phase 2 and 3 (Outlet 2)
Developer: Greystone Village Inc.



PROJECT # : 114025
 DESIGNED BY : SZ
 CHECKED BY : JAG
 DATE PREPARED : 18-Nov-16 DATE REVISED : 3-Jun-22
 DATE REVISED: 15-Mar-17
 DATE REVISED: 26-May-17
 ASBLT - DATE REVISED: 15-Sep-17

LOCATION			INDIVIDUAL							CUMULATIVE		PEAK FACTOR M	POPULATION FLOW Q(p) (L/s)	PEAK EXTRAN. FLOW Q(i) (L/s)	PEAK DESIGN FLOW Q(d) (L/s)	PROPOSED SEWER								
STREET	FROM MH	TO MH	Area	Single Units	Townhouse Units	3 Bedroom Condo Units	2 Bedroom Condo Units	Retirement Home Units or Studio Apartment	Population (in 1000's)	AREA (ha.)	Population (in 1000's)					AREA (ha.)	LENGTH (m)	PIPE SIZE (mm)	PIPE ID (mm)	TYPE OF PIPE	GRADE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	Qpeak/Qcap
OBLATS AVENUE	301A	301	B1						0.000	0.04	0.000	0.040	3.8	0.00	0.01	0.01	18.8	250	254.00	DR 35	1.81	83.5	1.65	0%
OBLATS AVENUE	301	303	B2		3				0.008	0.12	0.008	0.160	3.7	0.10	0.05	0.15	27.2	250	254.00	DR 35	2.43	96.7	1.91	0%
OBLATS AVENUE	303	305	B3		7				0.019	0.15	0.027	0.310	3.7	0.32	0.10	0.43	33.0	250	254.00	DR 35	3.40	114.4	2.26	0%
OBLATS AVENUE*	305	307	B4		10			220	0.335	0.67	0.362	0.980	3.4	4.03	0.32	4.35	45.4	250	254.00	DR 35	3.11	109.4	2.16	4%
OBLATS AVENUE	307	309	B5						0.000	0.03	0.362	1.010	3.4	4.03	0.33	4.36	14.9	250	254.00	DR 35	4.70	134.5	2.65	3%
OBLATS AVENUE	309	311	B6						0.000	0.09	0.362	1.100	3.4	4.03	0.36	4.39	36.8	250	254.00	DR 35	3.76	120.3	2.37	4%
PARISH PRIVATE	313	315	B7		12				0.032	0.14	0.032	0.140	3.7	0.39	0.05	0.43	57.6	250	254.00	DR 35	3.11	109.4	2.16	0%
SANCTUARY PRIVATE	317	315	B8	9					0.031	0.33	0.031	0.330	3.7	0.37	0.11	0.47	61.2	250	254.00	DR 35	0.61	48.5	0.96	1%
SANCTUARY PRIVATE	315	319	B9	1	2				0.009	0.15	0.072	0.620	3.6	0.84	0.20	1.05	36.5	250	254.00	DR 35	0.41	39.7	0.78	3%
SANCTUARY PRIVATE	319	321	B10	1	2				0.009	0.06	0.081	0.680	3.6	0.94	0.22	1.17	7.7	250	254.00	DR 35	0.52	44.7	0.88	3%
SANCTUARY PRIVATE	321	321b	B11	2					0.007	0.09	0.087	0.770	3.6	1.02	0.25	1.28	11.1	250	254.00	DR 35	0.45	41.6	0.82	3%
SANCTUARY PRIVATE	321b	311									0.087	0.770	3.6	1.02	0.25	1.28	11.6	250	254.00	DR 35	0.60	48.1	0.95	3%
OBLATS AVENUE	311	329	B12					146	0.204	0.36	0.654	2.230	3.3	7.05	0.74	7.79	32.9	250	254.00	DR 35	0.39	38.7	0.76	20%
SCHOLASTIC DRIVE	323	325	B13						0.000	0.08	0.000	0.080	3.8	0.00	0.03	0.03	37.4	250	254.00	DR 35	3.82	121.3	2.39	0%
SCHOLASTIC DRIVE	325	327	B14				59	94	0.256	0.52	0.256	0.600	3.5	2.89	0.20	3.08	35.0	250	254.00	DR 35	3.46	115.4	2.28	3%
SCHOLASTIC DRIVE	327	329	B15						0.000	0.04	0.256	0.640	3.5	2.89	0.21	3.10	37.6	250	254.00	DR 35	3.78	120.6	2.38	3%
OUTLET	329	331									0.909	2.870	3.3	9.61	0.95	10.56	40.8	250	254.00	DR 35	0.37	37.7	0.74	28%
OUTLET	331	EXMH									0.909	2.870	3.3	9.61	0.95	10.56	5.2	250	254.00	DR 35	0.77	54.4	1.07	19%

Phase 3 Condo

Notes:

- Q(d) = Q(p) + Q(i)
- Q(i) = 0.33 L/sec/ha
- Q(p) = (PqxM/86,400)

Definitions:

- Q(d) = Design Flow (L/sec)
 Q(p) = Population Flow (L/sec)
 Q(i) = Extraneous Flow (L/sec)

Institutional Peaking factor = 1.5 if ICI >20%, 1.0 <20%

P = Population (3.4 persons/single unit, 2.7 persons/townhouse, 3.1 persons/3-bed apartment, 2.1 persons/2-bed apartment, 2.0 persons/ school residence, 1.4 persons/retirement residence or studio apartment)
 q = Average per capita flow = 280 L/cap/day - Residential
 q = Average per gross ha. flow = 35000 L/gross ha/day - Light industrial
 q = Average per gross ha. flow =28000 L/gross ha/day - Commercial/Mixed use/Institutional
 M = Harmon Formula (maximum of 4.0)
 Min pipe size 200mm @ min. slope 0.32%

*Refer to technical memo 225 Deschatelets Avenue - Greystone Village, Site Servicing and Stormwater Management Memorandum for

details. Population of 195 retirement residence equivalent to flow demand from:

- 401 Student @ 90L/cap/day
- 40 Staff, 45 Daycare, 155 Community Centre @ 75L/cap/day
- 38 apartment units at 1.8 persons/unit @ 280L/cap/day
- 363 Gym @ 36L/cap/day

Appendix B
Boundary Conditions, Fire Flow Calculations, and Hydraulic Analysis Results

Steve Zorgel

From: Jhamb, Nishant <nishant.jhamb@ottawa.ca>
Sent: Thursday, August 4, 2022 9:49 AM
To: Steve Zorgel
Cc: Marc St.Pierre
Subject: FW: 375 Deschatelets - Greystone Phase 3
Attachments: 375 Deschatelets Avenue July 2022.pdf

Hello Steve

The following are boundary conditions, HGL, for hydraulic analysis at 375 Deschatelets Avenue (zone 1W) assumed to be connected to the 254 mm watermain on Deschatelets OR the 254 mm on Scholastic Drive (see attached PDF for location).

Both Connections:

Minimum HGL: 105.6 m

Maximum HGL: 115.1 m

Max Day + Fire Flow (233 L/s): 99.3 m (Connection 1)

Max Day + Fire Flow (200 L/s): 103.6 m (Connection 2)

These are for current conditions and are based on computer model simulation.

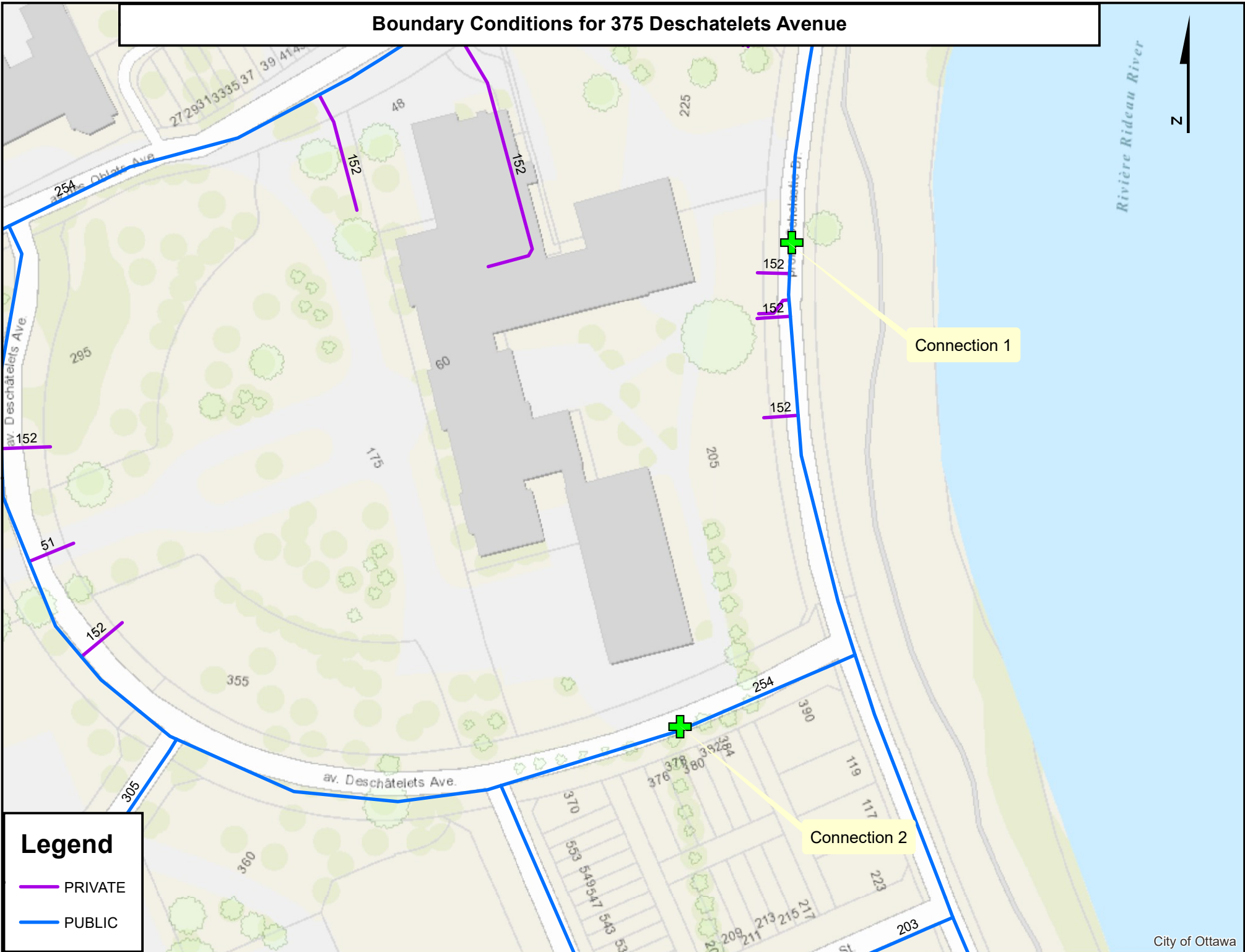
Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Nishant Jhamb, P.Eng
Project Manager | Gestionnaire de projet
Planning, Real Estate and Economic Development Department
Development Review - Central Branch
City of Ottawa | Ville d'Ottawa
110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1
613.580.2424 ext./poste 23112, nishant.jhamb@ottawa.ca

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Boundary Conditions for 375 Deschatelets Avenue



**Greystone Phase 3 Condos
Water Demand**

	Units	Population	Average Day Demand (L/s)	Maximum Day Demand (L/s)	Peak Hour Demand (L/s)
North Tower					
2 Bedroom	59	123.9	0.402	1.004	2.208
1 Bedroom or Studio	94	131.6	0.426	1.066	2.346
South Tower					
2 Bedroom	35	73.5	0.238	0.595	1.310
1 Bedroom or Studio	83	116.2	0.377	0.941	2.071
Total	271.0	445.2	1.44	3.61	7.94

Water Demand Parameters

Apartment Units (2 bedroom)	2.1	persons/unit
Apartment Units (studio)	1.4	persons/unit
Residential Demand	280	L/c/day
Residential Max Day	2.5	x Avg Day
Residential Peak Hour	2.2	x Max Day
North Condo Fireflow (#16)	233	L/s
South Condo Fireflow (#15)	200	L/s

21-Jul-22

FUS - Fire Flow Calculations

As per 2020 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

Novatech Project #: 114025 - Phase 3 Site Plan
 Project Name: Greystone Village - Phase 3 Site Plan
 Date: 7/21/2022
 Input By: Steve Zorgel
 Reviewed By: Marc St. Pierre

Legend
 Input by User
 No Information or Input Required

Building Description: 7 Storey Building Fronting Scholastic Avenue
 Non-Combustible Construction - 2020 FUS

Step		Choose		Value Used	Total Fire Flow (L/min)	
Base Fire Flow						
1	Construction Material		Multiplier		0.8	
	Coefficient related to type of construction C	Wood frame		1.5		
		Ordinary construction		1		
		Non-combustible construction	Yes	0.8		
		Modified Fire resistive construction (2 hrs)		0.6		
Fire resistive construction (> 3 hrs)			0.6			
2	Floor Area				16,000	
	A	Building Footprint (m ²)	1800			
		Number of Floors/Storeys	7			
		Protected Openings (1 hr)				
		Area of structure considered (m ²)		8,100		
F	Base fire flow without reductions					
		F = 220 C (A)^{0.5}				
Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge		Reduction/Surcharge		13,600	
	(1)	Non-combustible		-25%		-15%
		Limited combustible	Yes	-15%		
		Combustible		0%		
		Free burning		15%		
Rapid burning			25%			
4	Sprinkler Reduction		Reduction		-5,440	
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%		-30%
		Standard Water Supply	Yes	-10%		-10%
		Fully Supervised System	No	-10%		
		Cumulative Total		-40%		
5	Exposure Surcharge (cumulative %)		Surcharge		6,120	
	(3)	North Side	10.1 - 20 m			15%
		East Side	> 45.1m			0%
		South Side	10.1 - 20 m			15%
		West Side	10.1 - 20 m			15%
		Cumulative Total		45%		
Results						
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min		L/min	14,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	L/s	233
				or	USGPM	3,699
7	Storage Volume	Required Duration of Fire Flow (hours)		Hours	3	
		Required Volume of Fire Flow (m ³)		m ³	2520	

FUS - Fire Flow Calculations

As per 2020 Fire Underwriter's Survey Guidelines



Engineers, Planners & Landscape Architects

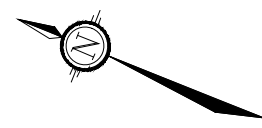
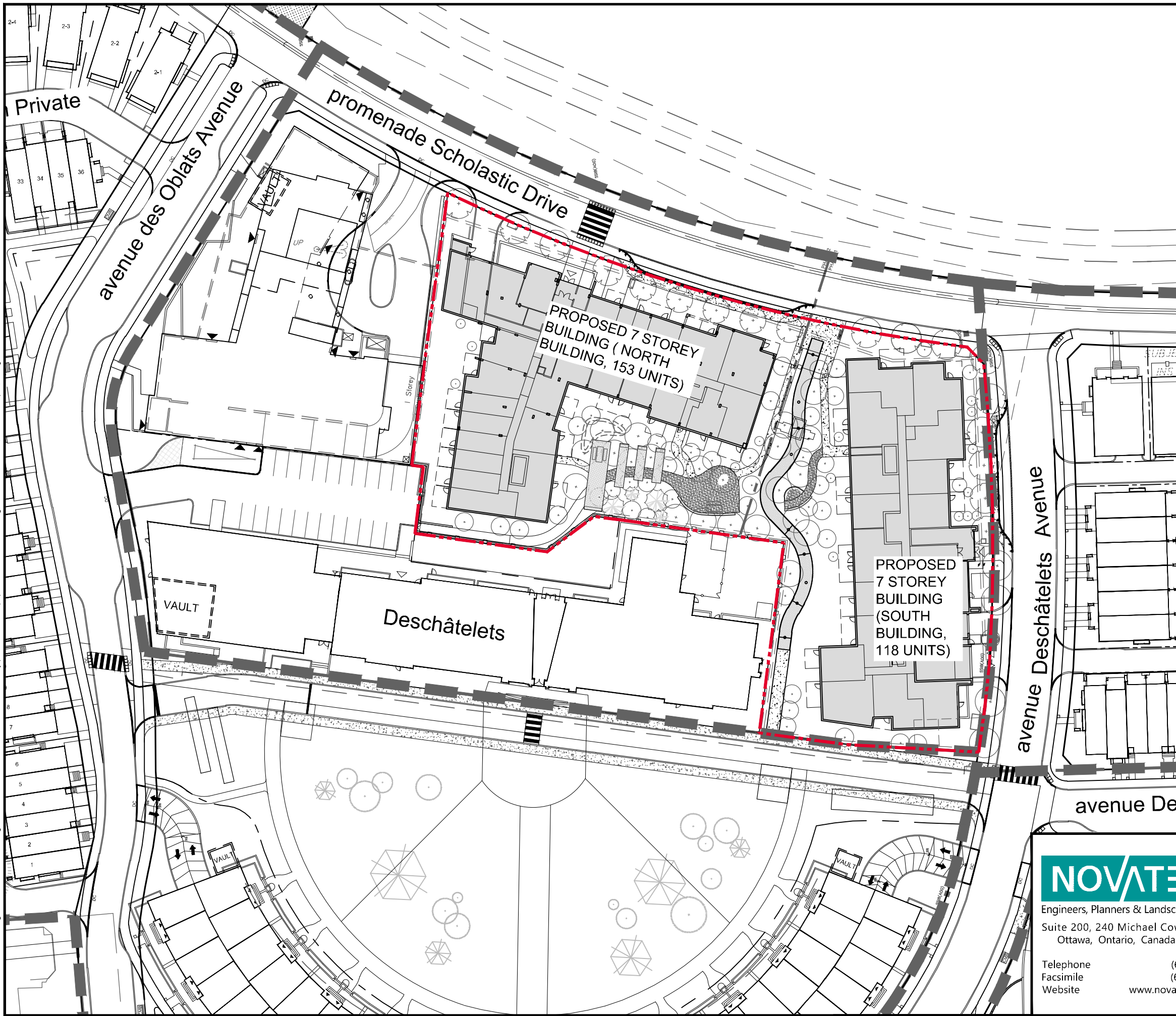
Novatech Project #: 114025 - Phase 3 Site Plan
 Project Name: Greystone Village - Phase 3 Site Plan
 Date: 7/21/2022
 Input By: Steve Zorgel
 Reviewed By: Marc St. Pierre

Legend
 Input by User
 No Information or Input Required

Building Description: 7 Storey Building Fronting Deschatelets Avenue
 Non-Combustible Construction - 2020 FUS

Step		Choose		Value Used	Total Fire Flow (L/min)		
Base Fire Flow							
1	Construction Material			Multiplier			
	Coefficient related to type of construction C	Wood frame		1.5	0.8		
		Ordinary construction		1			
		Non-combustible construction	Yes	0.8			
		Modified Fire resistive construction (2 hrs)		0.6			
Fire resistive construction (> 3 hrs)			0.6				
2	Floor Area						
	A	Building Footprint (m ²)	1680		7,560		
		Number of Floors/Storeys	7				
		Protected Openings (1 hr)					
		Area of structure considered (m ²)					
F	Base fire flow without reductions			15,000			
Reductions or Surcharges							
3	Occupancy hazard reduction or surcharge			Reduction/Surcharge			
	(1)	Non-combustible		-25%	-15%		
		Limited combustible	Yes	-15%			
		Combustible		0%			
		Free burning		15%			
Rapid burning			25%				
4	Sprinkler Reduction			Reduction			
	(2)	Adequately Designed System (NFPA 13)	Yes	-30%	-30%		
		Standard Water Supply	Yes	-10%	-10%		
		Fully Supervised System		-10%			
Cumulative Total			-40%				
5	Exposure Surcharge (cumulative %)			Surcharge			
	(3)	North Side	10.1 - 20 m		15%		
		East Side	> 45.1m		0%		
		South Side	20.1 - 30 m		10%		
		West Side	20.1 - 30 m		10%		
Cumulative Total			35%				
Results							
6	(1) + (2) + (3)	Total Required Fire Flow, rounded to nearest 1000L/min			L/min	12,000	
		(2,000 L/min < Fire Flow < 45,000 L/min)			or	L/s	200
					or	USGPM	3,170
7	Storage Volume	Required Duration of Fire Flow (hours)			Hours	2.5	
		Required Volume of Fire Flow (m ³)			m ³	1800	

M:\2014\114025\CAD\Design\Phase 2-3\Phase 3\Figures\20220603-Submission\20220603-Key plan and Concept plan\PH3-FIG-2-CP.dwg, FIG-2-CP, Jun 02, 2022 - 1:22pm, szorgal



LEGEND

-  PROPERTY LINE
-  PHASE LINE
-  PHASE 3 CONDO SITE PROPERTY LINE

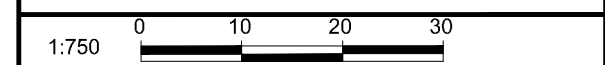


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 Website www.novatech-eng.com

CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

CONCEPT PLAN - PHASE 3 CONDOS



JUNE 2022 114025 FIGURE 2

Steve Zorgel

From: Chris Ilg <cilg@neufarchitectes.com>
Sent: Wednesday, January 26, 2022 12:44 PM
To: Steve Zorgel
Cc: Marc St.Pierre
Subject: RE: Greystone Phase 3 Buildings

Follow Up Flag: Follow up
Flag Status: Flagged

Hey Steve,

I'll describe the construction system and maybe you can confirm the class based on this.

Structure = concrete frame – floor assembly and columns to be 2hr rated

Exterior walls –

Masonry veneer finishes (90mm standard) and metal panels/siding – both non-combustible materials.

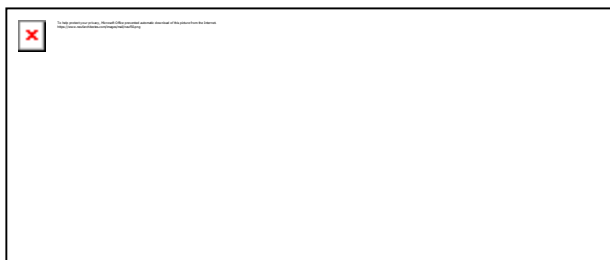
Steel stud assembly

Exterior gypsum and exterior gypsum. We likely will only require certain walls to be classified 1hr due to limiting distances with other buildings, but otherwise we are not specifying 1hr even though they could be considered 1hr.

Roof will be inverted roof with plastic XPS insulation above a rubberized membrane.

Let me know if this helps clarify the construction. I'm not sure the implications on the fire protection system if we qualify between classes, so let me know if there is a benefit to push either way in terms of cost or system complexity.

Thanks,



CHRIS ILG, OAQ, OAA, MRAIC, LEED AP

Architecte associé, Partner Architect

T 514 847 1117 #226 F 514 847 2287 C 514.512.1647

630, boul. René-Lévesque O. 32^e étage, Montréal (QC) H3B 1S6

NEUF ARCHITECTES SENCRL Confidentialité + Transmission
Montréal. Ottawa. Toronto

50 ANS ET TOUJOURS NEUF . 50 YEARS AND STILL NEUF

From: Steve Zorgel <s.zorgel@novatech-eng.com>
Sent: Monday, January 24, 2022 7:51 AM
To: Chris Ilg <cilg@neufarchitectes.com>
Cc: Marc St.Pierre <m.stpierre@novatech-eng.com>
Subject: Greystone Phase 3 Buildings

Hi Chris,

I am working on calculating estimated fireflows for the two 7-storey buildings in phase 3 of Greystone. I wanted to confirm the following for the two buildings. Do you anticipate

- Building class 3, 4, 5 or 6? The building class definitions are attached;
 - The fire rating of the floors, exterior walls and roof. 1-hr minimum fire rating to be classified as Modified Fire Resistive, otherwise Non-Combustible construction (example assemblies for Modified Fire Resistive attached but not necessary as long as rating is >1-hr);
- Sprinklered system will be a fully supervised/monitored system and utilize standard firefighting equipment (hoses, nozzles, etc.).

Please let us know if you have this information available (or if unknown) so we can make the appropriate assumptions for fireflows. Thank you.

Steve Zorgel, P.Eng., Project Coordinator | Engineering

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x298 | Fax: 613.254.5867

The information contained in this email message is confidential and is for exclusive use of the addressee

Fireflows at Node Locations (if applicable)

Date: May 10, 2017

Node ID	Fireflows Applied	Governing Fireflow
N1	Single Unit 1 - 141.40L/s	141.40L/s
	Single Unit 2 - 120.44L/s	
	Single Unit 3 - 104.77L/s	
N2	n/a 50mm lead	
N3	Condo 9a - full area = 399.88L/s	286.45L/s
	Condo 9b - Firebreak at Phase Line = 286.45L/s	
	Townhouse 4a - 1 firebreak = 219.53L/s	
	Townhouse 4b - 2 firebreaks = 176.85L/s	
	Townhouse 4c - 3 firebreaks = 154.12L/s	
	Single Unit 1 - 141.40L/s	
	Single Unit 3 - 104.77L/s	
N4	Townhouse 5 - (268.88) capped at 167L/s	167L/s
	Single Unit 23 - 137.65L/s	
	Single Unit 1 - 141.40L/s	
	Single Unit 2 - 120.44L/s	
	Single Unit 3 - 104.77L/s	
N5	Townhouse 5 - (268.88) capped at 167L/s	167L/s
	Single Unit 23 - 137.65L/s	
N6	Condo 9a - full area = 399.88L/s	286.45L/s
	Condo 9b - Firebreak at Phase Line = 286.45L/s	
	Townhouse 4a - 1 firebreak = 219.53L/s	
	Townhouse 4b - 2 firebreaks = 176.85L/s	
	Townhouse 4c - 3 firebreaks = 154.12L/s	
N7	Townhouse 5 - (268.88) capped at 167L/s	167L/s
	Condo 6 - 145.15 L/s	
	Single Unit 23 - 137.65L/s	
N8	Townhouse 4a - 1 firebreak = 219.53L/s	399.88L/s or 286.45L/s
	Townhouse 4b - 2 firebreaks = 176.85L/s	
	Townhouse 4c - 3 firebreaks = 154.12L/s	
	Townhouse 5 - (268.88) capped at 167L/s	
	Townhouse 7a - 219.97L/s	
	Townhouse 7b - 202.94L/s	
	Condo 9a - full area = 399.88L/s	
	Condo 9b - Firebreak at Phase Line = 286.45L/s	
	Condo 13a - 229.03L/s	
Condo 13b - 229.03L/s		
N9	Townhouse 7a - 219.97L/s	219.97L/s
	Townhouse 7b - 202.94L/s	
N10	School 12 - 249.76L/s	249.76L/s

Fireflows at Node Locations (if applicable)

Date: May 10, 2017

Node ID	Fireflows Applied	Governing Fireflow
N11	Townhouse 7a - 219.97L/s	249.76L/s
	Townhouse 7b - 202.94L/s	
	School 12 - 249.76L/s	
	Condo 10 - 181.08L/s	
N12	Townhouse 7a - 219.97L/s	219.97L/s
	Townhouse 7b - 202.94L/s	
	Condo 10 - 181.08L/s	
N13	Townhouse 7a - 219.97L/s	230.67L/s
	Townhouse 7b - 202.94L/s	
	Townhouse 18 (259.36) - capped at 167L/s	
	Condo Building 16 - 230.67L/s	
N14	Condo Building 16 - 230.67L/s	300.24L/s
	Retirement Residence Unit 17 - 300.24L/s	
	Townhouse 18 (259.36) - capped at 167L/s	
	Townhouse 19 - 259.36L/s	
N15	Retirement Residence Unit 17 - 300.24L/s	300.24L/s
	Townhouse 19 - 259.36L/s	
	Single Unit 21 - 103.75L/s	
N16*	Townhouse 19 - 259.36L/s	259.36L/s
	Single Unit 20 - 103.49L/s	
	Single Unit 21 - 103.75L/s	
N17	Retirement Residence Unit 17 - 300.24L/s	300.24L/s
	Single Unit 21 - 103.75L/s	
N18*	Townhouse 19 - 259.36L/s	259.36L/s
	Single Unit 20 - 103.49L/s	
	Single Unit 22 - 121.25L/s	
N19*	Townhouse 19 - 259.36L/s	259.36L/s
	Single Unit 20 - 103.49L/s	
N20	n/a 50mm lead	
N21	Deschatelets Building 14 - 248.24L/s	300.24L/s
	Condo 15a - 139.73L/s	
	Condo 15b 145.26L/s	
	Retirement Residence Unit 17 - 300.24L/s	
N22	Townhouse 5 - (268.88) capped at 167L/s	229.03L/s
	Condo 13a - 229.03L/s	
	Condo 13b - 229.03L/s	
	Townhouse 4a - 1 firebreak = 219.53L/s	
	Townhouse 4b - 2 firebreaks = 176.85L/s	
	Townhouse 4c - 3 firebreaks = 154.12L/s	
N23	Single Unit 1 - 141.40L/s	141.40L/s
	Single Unit 2 - 120.44L/s	
	Single Unit 3 - 104.77L/s	
N24*	Single Unit 1 - 141.40L/s	141.40L/s
	Single Unit 2 - 120.44L/s	
	Single Unit 3 - 104.77L/s	
N25	n/a 50mm lead	

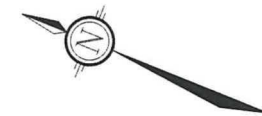
Fireflows at Node Locations (if applicable)

Date: May 10, 2017







Node ID	Fireflows Applied	Governing Fireflow
N26	Condo 11 - 251.52L/s	251.52L/s
N27*	Single Unit 1 - 141.40L/s	141.40L/s
	Single Unit 2 - 120.44L/s	
	Single Unit 3 - 104.77L/s	
N28	Condo 10 - 181.08L/s	181.08L/s
	Domicile Building	133.33L/s
	Sister's Building	173.37L/s
N29	School 12 - 249.76L/s	249.76L/s
N30	n/a 50mm lead	
N31	Single Unit 20 - 103.49L/s	121.25L/s
	Single Unit 21 - 103.75L/s	
	Single Unit 22 - 121.25L/s	

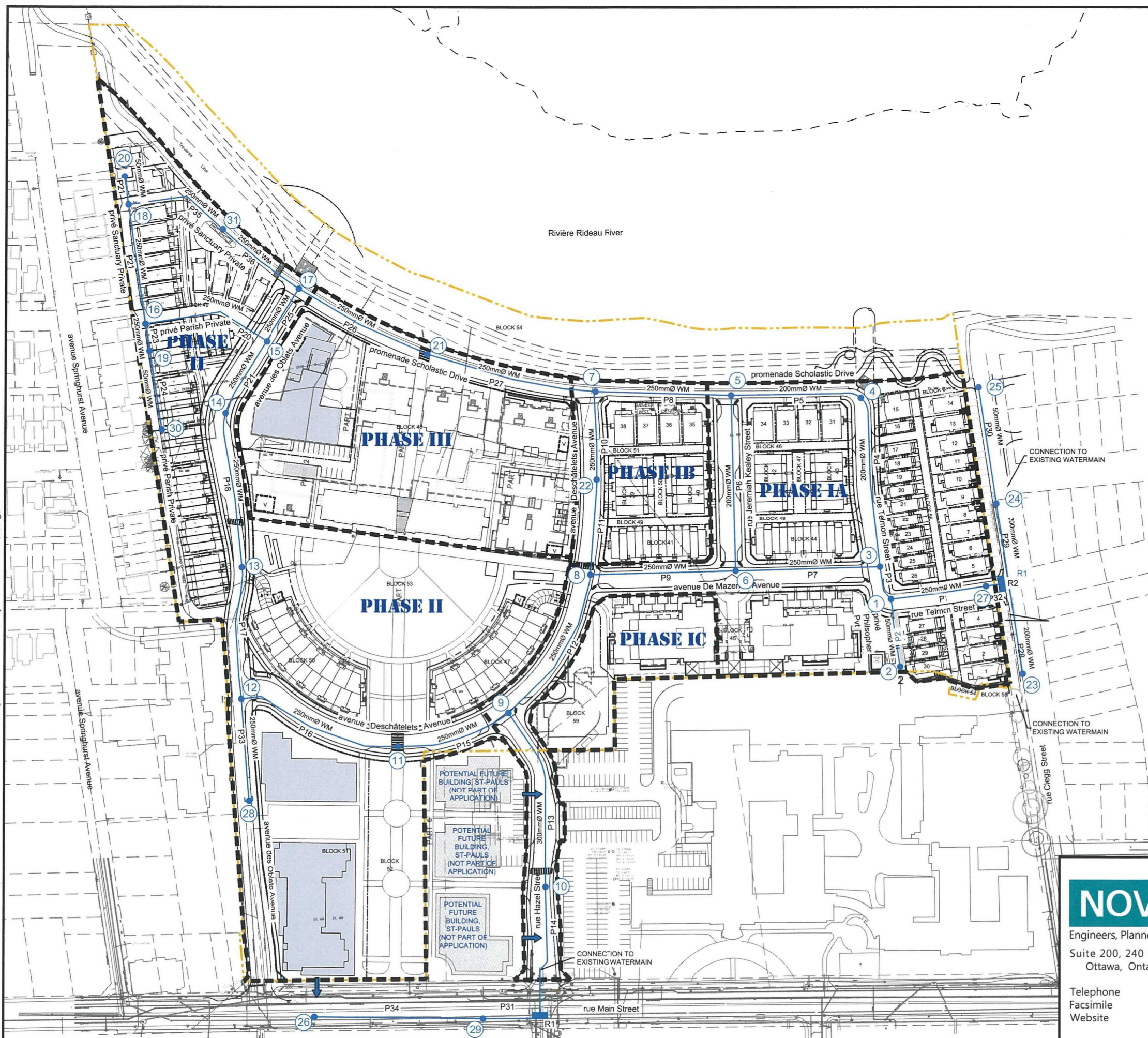
Notes: *No fire hydrant near location, therefore not subject to fireflow analysis

M:\2017\114025\CAD\Design\Figures\Design Brief\2017-05-SEWER-ALIGN.dwg, FIG-14-WM-NODE, May 31, 2017 - 8:08am, szogel



LEGEND

-  PROPERTY LINE
-  PROPOSED WATERMAIN AND ID NUMBER
-  PROPOSED NODE AND ID NUMBER
-  EXISTING RESERVOIR AND ID NUMBER
-  PROPOSED SERVICE LOCATION
-  PHASING LIMITS

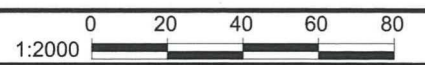


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Telephone (613) 254-9643
 Facsimile (613) 254-5867
 Website www.novatech-eng.com

CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

OVERALL WATERMAIN NODE LOCATIONS



MAY 2017 114025 FIGURE 14

SHT11X17 DWG - 279mmX432mm

Appendix C
Stormwater Management

GREYSTONE VILLAGE PHASE 3 CONDOS

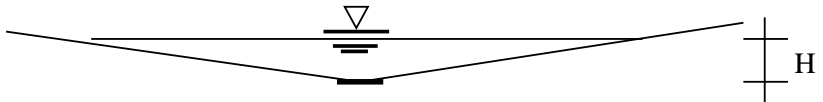


Runoff Coefficients

Drainage Area	Total Area (m ²)	Ponding Area (m ²)	Hard Surface Area		Gravel Area		Grass Area		5-Year Runoff Coefficient	100-Year Runoff Coefficient	
			Area (m ²)	C	Area (m ²)	C	Area (m ²)	C			
North Building/Outlet 2											
RD1	86.9	80.9	86.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD2	47.4	44.4	47.4	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD3	48.1	44.4	48.1	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD4	92.5	84.5	92.5	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD5	43.0	43.0	43.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD6	101.8	96.7	101.8	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD7	114.1	109.1	114.1	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD8	17.3	14.5	17.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD9	41.8	64.0	41.8	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD10	153.6	134.9	153.6	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD11	104.0	84.6	104.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD12	63.0	57.4	63.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD13	48.7	38.2	48.7	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD14	59.0	60.8	59.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD15	44.0	80.4	44.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD16	84.1	80.9	84.1	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD17	79.8	56.1	79.8	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD18	37.0	20.9	37.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD19	35.3	28.5	35.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD20	53.7	31.8	53.7	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD21	99.3	65.4	99.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD22	99.0	69.1	99.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD40	No Contributing Area	Roof Drain for Any Unintended Rain Entering near Edge of Building									
A01	345.0	-	65.0	0.90	0.0	0.70	280	0.20	0.33	0.39	
A02	1188.0	-	272.0	0.90	141.0	0.70	775	0.20	0.42	0.50	
A03	888.0	-	475.0	0.90	0.0	0.70	413	0.20	0.57	0.65	
South Building/Outlet 1											
RD23	87.2	75.0	87.2	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD24	73.9	60.2	73.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD25	63.9	60.3	63.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD26	97.0	87.7	97.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD27	92.3	74.4	92.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD28	91.9	80.6	91.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD29	73.2	83.6	73.2	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD30	126.3	106.6	126.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD31	41.9	37.4	41.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD32	124.2	116.7	124.2	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD33	64.0	59.7	64.0	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD34	60.8	54.2	60.8	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD35	101.3	62.4	101.3	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD36	54.5	83.5	54.5	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD37	95.9	67.7	95.9	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD38	104.2	53.6	104.2	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
RD39	82.7	42.9	82.7	0.90	0.0	0.70	0.0	0.20	0.90	1.00	
A04	441.0	-	189.0	0.90	0.0	0.70	252	0.20	0.50	0.57	
A05	459.0	-	261.0	0.90	0.0	0.70	198	0.20	0.60	0.68	
A06	924.0	-	247.0	0.90	30.0	0.70	647	0.20	0.40	0.47	
Total	7233.60		4250.6		141.0		1918.0				

Zurn Roof Drains

Opening	G.P.M. Per Inch of Head	L.P.M. Per Inch (25 mm) of Head	L/s Per Metre of Head	L/s Per 0.15 m of Head
Standard - X1	5.00	22.730	14.915	2.237
Reduced - X2	3.75	17.048	11.186	1.678
Reduced - X3	2.50	11.365	7.458	1.119
Max Reduced - X4	1.25	5.683	3.729	0.559



SAMPLE CALCULATION:

AREA R-01

Number of notches (N) = 1

Head (H) = 0.105 m for 5-year event

Head (H) = 0.138 m for 100-year event

$$Q_{5 \text{ all}} = 3.729 \text{ L/s/m/notch} \times H \times N$$

$$Q_{5 \text{ all}} = 3.729 \text{ L/s/m/notch} \times 0.105 \text{ m} \times 1 \text{ notch}$$

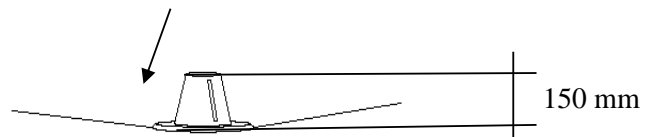
$$Q_{5 \text{ all}} = 0.39 \text{ L/s}$$

$$Q_{100 \text{ all}} = 3.729 \text{ L/s/m/notch} \times H \times N$$

$$Q_{100 \text{ all}} = 3.729 \text{ L/s/m/notch} \times 0.138 \text{ m} \times 1 \text{ notch}$$

$$Q_{100 \text{ all}} = 0.51 \text{ L/s}$$

No. of Notches



Controlled Flow

5 YR

Area No.	Area (ha)	C _{5yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zum Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
North Building/Outlet 2													
Roof Drains													
RD1	0.0087	0.90	10.00	104.19	2.26	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.105	0.39	4.043	1.41
RD2	0.0047	0.90	10.00	104.19	1.24	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.095	0.36	2.220	0.57
RD3	0.0048	0.90	10.00	104.19	1.25	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.096	0.36	2.219	0.58
RD4	0.0093	0.90	10.00	104.19	2.41	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.107	0.40	4.223	1.53
RD5	0.0043	0.90	10.00	104.19	1.12	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.096	0.36	2.151	0.48
RD6	0.0102	0.90	10.00	104.19	2.65	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.107	0.40	4.837	1.76
RD7	0.0114	0.90	10.00	104.19	2.98	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	5.457	2.07
RD8	0.0017	0.90	10.00	104.19	0.45	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.077	0.29	0.727	0.10
RD9	0.0042	0.90	10.00	104.19	1.09	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.082	0.30	3.200	0.52
RD10	0.0154	0.90	10.00	104.19	4.00	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.115	0.43	6.746	3.06
RD11	0.0104	0.90	10.00	104.19	2.71	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.112	0.42	4.230	1.77
RD12	0.0063	0.90	10.00	104.19	1.64	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.101	0.38	2.868	0.87
RD13	0.0049	0.90	10.00	104.19	1.27	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.096	0.36	1.911	0.58
RD14	0.0059	0.90	10.00	104.19	1.54	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.096	0.36	3.039	0.81
RD15	0.0044	0.90	10.00	104.19	1.15	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.079	0.29	4.019	0.58
RD16	0.0084	0.90	10.00	104.19	2.19	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.104	0.39	4.046	1.34
RD17	0.0080	0.90	10.00	104.19	2.08	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.113	0.42	2.805	1.19
RD18	0.0037	0.90	10.00	104.19	0.96	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.103	0.39	1.044	0.34
RD19	0.0035	0.90	10.00	104.19	0.92	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.094	0.35	1.425	0.35
RD20	0.0054	0.90	10.00	104.19	1.40	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.096	0.71	1.590	0.41
RD21	0.0099	0.90	10.00	104.19	2.59	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.106	0.79	3.270	1.16
RD22	0.0099	0.90	10.00	104.19	2.58	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.104	0.78	3.453	1.16
RD40	No Contributing Area				Roof Drain for Any Unintended Rain	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.000	0.00	0.000	0.00
Roof Subtotal:	0.16				40.50						9.31	69.52	22.65
Courtyard													
A01	0.0345	0.33	10.00	104.19	3.32								
A02	0.1188	0.42	10.00	104.19	14.44								
A03	0.0888	0.57	10.00	104.19	14.78								
Courtyard Subtotal:	0.24				32.53								
Total Outlet 2 (Roof+Courtyard):	0.40				73.03								
South Building/Outlet 1													
Roof Drains													
RD23	0.0087	0.90	10.00	104.19	2.27	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	3.752	1.40
RD24	0.0074	0.90	10.00	104.19	1.93	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.107	0.40	3.008	1.09
RD25	0.0064	0.90	10.00	104.19	1.67	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.100	0.37	3.013	0.91
RD26	0.0097	0.90	10.00	104.19	2.53	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	4.386	1.63
RD27	0.0092	0.90	10.00	104.19	2.41	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.111	0.41	3.721	1.50
RD28	0.0092	0.90	10.00	104.19	2.40	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	4.031	1.51
RD29	0.0073	0.90	10.00	104.19	1.91	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.097	0.36	4.181	1.13
RD30	0.0126	0.90	10.00	104.19	3.29	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.114	0.42	5.330	2.34
RD31	0.0042	0.90	10.00	104.19	1.09	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.095	0.35	1.871	0.47
RD32	0.0124	0.90	10.00	104.19	3.24	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.110	0.41	5.833	2.31
RD33	0.0064	0.90	10.00	104.19	1.67	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.101	0.38	2.985	0.90
RD34	0.0061	0.90	10.00	104.19	1.58	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.101	0.38	2.709	0.83
RD35	0.0101	0.90	10.00	104.19	2.64	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.108	0.81	3.122	1.18
RD36	0.0055	0.90	10.00	104.19	1.42	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.085	0.32	4.175	0.77
RD37	0.0096	0.90	10.00	104.19	2.50	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.103	0.77	3.383	0.58
RD38	0.0104	0.90	10.00	104.19	2.72	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.114	0.85	2.681	1.19
RD39	0.0083	0.90	10.00	104.19	2.16	Zum Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.109	0.81	2.146	0.83
Roof Subtotal:	0.14				37.41						8.25	60.32	20.58
Courtyard													
A04	0.0441	0.50	10.00	104.19	6.39								
A05	0.0459	0.60	10.00	104.19	7.95								
A06	0.0924	0.40	10.00	104.19	10.80								
Courtyard Subtotal:	0.18				25.13								
Total Outlet 1 (Roof + Courtyard):	0.33				62.55								
Total 5 Year (Outlet 1 + Outlet 2):	0.72				135.57						17.56	129.84	43.23

100 YR

Area ID	Area (ha)	C _{100yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zurn Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
North Building/Outlet 2													
Roof Drains													
RD1	0.0087	1.00	10.00	178.56	4.31	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.138	0.51	4.043	3.13
RD2	0.0047	1.00	10.00	178.56	2.35	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.127	0.47	2.220	1.35
RD3	0.0048	1.00	10.00	178.56	2.39	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.128	0.48	2.219	1.37
RD4	0.0093	1.00	10.00	178.56	4.59	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.139	0.52	4.223	3.39
RD5	0.0043	1.00	10.00	178.56	2.13	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.122	0.46	2.151	1.17
RD6	0.0102	1.00	10.00	178.56	5.05	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.139	0.52	4.837	3.87
RD7	0.0114	1.00	10.00	178.56	5.67	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.141	0.52	5.457	4.52
RD8	0.0017	1.00	10.00	178.56	0.86	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	0.727	0.27
RD9	0.0042	1.00	10.00	178.56	2.07	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.108	0.40	3.200	1.21
RD10	0.0154	1.00	10.00	178.56	7.63	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.149	0.56	6.746	6.60
RD11	0.0104	1.00	10.00	178.56	5.16	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.146	0.54	4.230	3.92
RD12	0.0063	1.00	10.00	178.56	5.13	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.133	0.50	2.868	2.00
RD13	0.0049	1.00	10.00	178.56	2.42	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.134	0.50	1.911	1.36
RD14	0.0059	1.00	10.00	178.56	2.93	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.127	0.47	3.039	1.87
RD15	0.0044	1.00	10.00	178.56	2.18	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.103	0.39	4.019	1.32
RD16	0.0084	1.00	10.00	178.56	4.18	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.135	0.51	4.046	2.99
RD17	0.0080	1.00	10.00	178.56	3.96	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.148	0.55	2.805	2.69
RD18	0.0037	1.00	10.00	178.56	1.83	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.140	0.52	1.044	0.85
RD19	0.0035	1.00	10.00	178.56	1.75	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.126	0.47	1.425	0.85
RD20	0.0054	1.00	10.00	178.56	2.67	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.131	0.97	1.590	1.05
RD21	0.0099	1.00	10.00	178.56	4.93	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.131	0.97	3.270	2.72
RD22	0.0099	1.00	10.00	178.56	4.91	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.141	1.05	3.453	2.74
RD40	No Contributing Area		Roof Drain for Any Unintended Rain Entering near Edge of Building			Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.000	0.00	0.000	0.00
Roof Subtotal:	0.16				77.11						12.29	69.52	51.25
Courtyard													
A01	0.0345	0.39	10.00	178.56	6.70								
A02	0.1188	0.50	10.00	178.56	29.24								
A03	0.0888	0.65	10.00	178.56	28.70								
Courtyard Subtotal:	0.24				64.65								
Total Outlet 2 (Roof+ Courtyard):	0.40				141.76								
South Building/Outlet 1													
Roof Drains													
RD23	0.0087	1.00	10.00	178.56	4.33	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.141	0.53	3.752	3.11
RD24	0.0074	1.00	10.00	178.56	3.67	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.140	0.52	3.008	2.47
RD25	0.0064	1.00	10.00	178.56	3.17	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.132	0.49	3.013	2.06
RD26	0.0097	1.00	10.00	178.56	4.81	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.141	0.52	4.386	3.62
RD27	0.0092	1.00	10.00	178.56	4.58	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.127	0.47	3.721	3.33
RD28	0.0092	1.00	10.00	178.56	4.56	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.141	0.53	4.031	3.35
RD29	0.0073	1.00	10.00	178.56	3.64	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.127	0.47	4.181	2.54
RD30	0.0126	1.00	10.00	178.56	6.27	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.148	0.55	5.330	5.09
RD31	0.0042	1.00	10.00	178.56	2.08	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.126	0.47	1.871	1.11
RD32	0.0124	1.00	10.00	178.56	6.16	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.143	0.53	5.833	5.06
RD33	0.0064	1.00	10.00	178.56	3.18	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.132	0.49	2.985	2.06
RD34	0.0061	1.00	10.00	178.56	3.02	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.133	0.50	2.709	1.89
RD35	0.0101	1.00	10.00	178.56	5.03	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.144	1.07	3.122	2.78
RD36	0.0055	1.00	10.00	178.56	2.71	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	1	0.112	0.42	4.175	1.75
RD37	0.0096	1.00	10.00	178.56	4.76	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.138	1.03	3.383	2.62
RD38	0.0104	1.00	10.00	178.56	5.17	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.108	0.80	2.681	1.00
RD39	0.0083	1.00	10.00	178.56	4.10	Zurn Roof Drain	ZCF121-1W-X4-Z-105-10-77	3.73	2	0.146	1.09	2.146	2.00
Roof Subtotal	0.14				71.24						10.49	60.32	45.85
Courtyard													
A04	0.0441	0.57	10.00	178.56	12.51								
A05	0.0459	0.68	10.00	178.56	15.41								
A06	0.0924	0.47	10.00	178.56	21.59								
Courtyard Subtotal	0.18				49.52								
Total Outlet 1 (Roof + Courtyard)	0.33				120.76								
Total 100 Year (Outlet 1 + Outlet 2):	0.72				262.52						22.79	129.84	97.10

Outlet 1 Summary Table (Runoff to Deschatelets Avenue)

Area ID	Area (ha)	Runoff		Storage available (m ³)	Storage used (m ³)	
		5 year event L/s	100 year L/s		5 year event (m ³)	100 year event (m ³)
Controlled						
Roof	0.144	8.25	10.49	60.32	20.58	45.85
Uncontrolled						
A04	0.04	6.39	12.51	-	-	-
A05	0.05	7.95	15.41	-	-	-
A06	0.09	10.80	21.59	-	-	-
Total	0.33	33.39	60.01	60.32	20.58	45.85

Outlet 2 Summary Table (Runoff to Scholastic Avenue)

Area ID	Area (ha)	Runoff		Storage available (m ³)	Storage used (m ³)	
		5 year event L/s	100 year event L/s		5 year event (m ³)	100 year event (m ³)
Controlled						
Roof	0.1553	9.31	12.29	69.52	22.65	51.25
Uncontrolled						
A01	0.035	3.32	6.70	-	-	-
A02	0.119	14.44	29.24	-	-	-
A03	0.089	14.78	28.70	-	-	-
Total	0.40	41.84	76.94	69.52	22.65	51.25

GREYSTONE VILLAGE PHASE 3 CONDOS

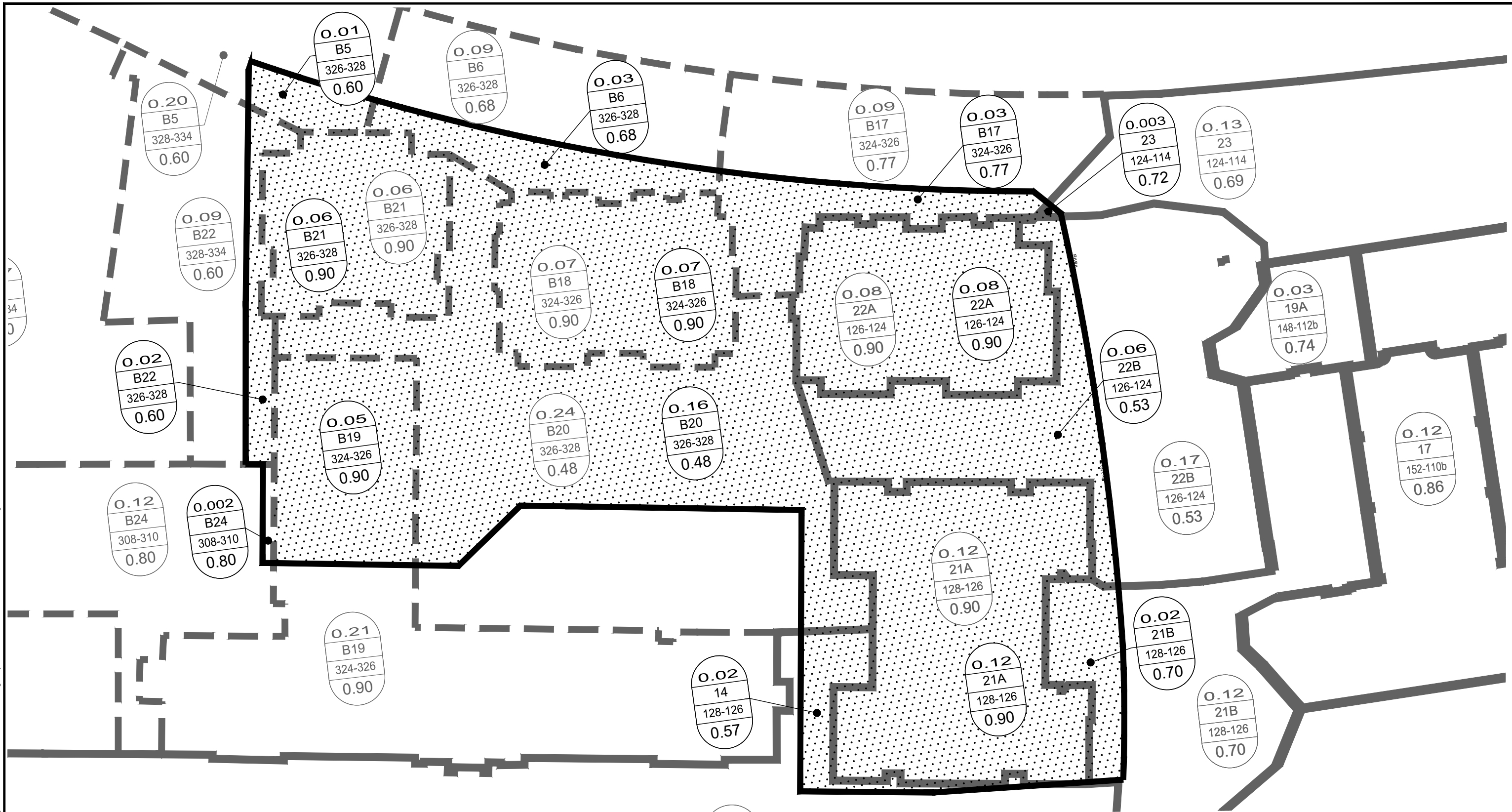


5-Year Uncontrolled Release Rate

Area ID	Area*	C	Time (tc)	intensity	Qallow
	(ha)	Coefficient	min	mm/hr	(L/s)
Outlet 1					
14	0.018	0.57	10.00	104.19	2.97
21A	0.120	0.90	10.00	104.19	31.28
21B	0.020	0.70	10.00	104.19	4.01
22A	0.075	0.90	10.00	104.19	19.53
22B	0.061	0.53	10.00	104.19	9.33
23	0.003	0.69	10.00	104.19	0.60
Subtotal	0.30				67.73
Outlet 2					
B24	0.002	0.80	10.00	104.19	0.46
B17	0.030	0.77	10.00	104.19	6.69
B18	0.070	0.90	10.00	104.19	18.20
B19	0.052	0.90	10.00	104.19	13.45
B6	0.030	0.68	10.00	104.19	5.91
B20	0.163	0.48	10.00	104.19	22.68
B21	0.056	0.90	10.00	104.19	14.62
B5	0.010	0.60	10.00	104.19	1.76
B22	0.016	0.60	10.00	104.19	2.71
Subtotal	0.43				86.48
Grand Total	0.72				154.21

*Contributing areas from the Master Storm Drainage Plans 114025-STM1,114025-STM1-B,114025-STM2 within the Phase 3 Condo site boundary

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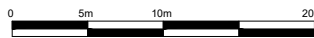
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City of Ottawa
Greystone Village
175 Main Street

Storm Drainage Areas

1 : 500



JULY 2022

114025

FIG-SDA

REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-1 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.009	ha	Qallow =	0.39
C =	0.90		Vol(max) =	1.41
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.07	2.68	0.80
10	104.19	2.26	1.87	1.12
15	83.56	1.82	1.43	1.28
20	70.25	1.53	1.14	1.36
25	60.90	1.32	0.93	1.40
30	53.93	1.17	0.78	1.41
35	48.52	1.05	0.66	1.40
40	44.18	0.96	0.57	1.37
45	40.63	0.88	0.49	1.33
50	37.65	0.82	0.43	1.29
55	35.12	0.76	0.37	1.23
60	32.94	0.72	0.33	1.17
65	31.04	0.67	0.28	1.11
70	29.37	0.64	0.25	1.04
75	27.89	0.61	0.22	0.97
80	26.56	0.58	0.19	0.90
85	25.37	0.55	0.16	0.82
90	24.29	0.53	0.14	0.74

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.20	0.10
43	1.59	0.11
52	2.07	0.12
61	2.63	0.13
70	3.29	0.14
81	4.04	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.105 m
1.59	1.41	1.20	Qallow =	0.39 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

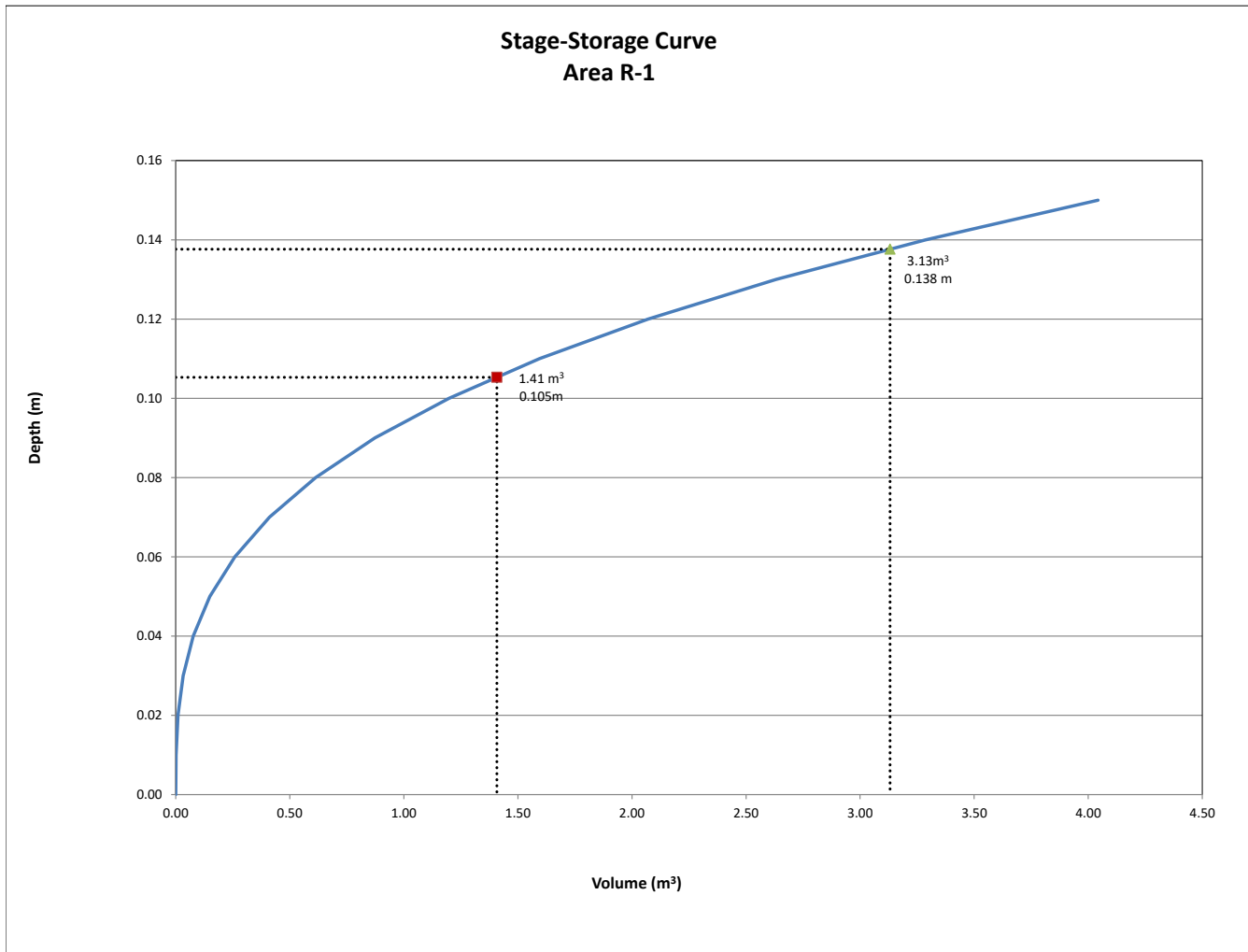
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-1 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.009	ha	Qallow =	0.51
C =	1.00		Vol(max) =	3.13
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	5.86	5.35	1.61
10	178.56	4.31	3.80	2.28
15	142.89	3.45	2.94	2.65
20	119.95	2.90	2.39	2.86
25	103.85	2.51	2.00	3.00
30	91.87	2.22	1.71	3.08
35	82.58	1.99	1.48	3.12
40	75.15	1.81	1.30	3.13
45	69.05	1.67	1.16	3.13
50	63.95	1.54	1.03	3.10
55	59.62	1.44	0.93	3.07
60	55.89	1.35	0.84	3.02
65	52.65	1.27	0.76	2.97
70	49.79	1.20	0.69	2.91
75	47.26	1.14	0.63	2.84
80	44.99	1.09	0.58	2.77
85	42.95	1.04	0.53	2.69
90	41.11	0.99	0.48	2.61

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.20	0.10
43	1.59	0.11
52	2.07	0.12
61	2.63	0.13
70	3.29	0.14
81	4.04	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.138 m
3.29	3.13	2.63	Qallow =	0.51 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-2 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.005	ha	Qallow =	0.36
C =	0.90		Vol(max) =	0.57
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.67	1.32	0.40
10	104.19	1.24	0.88	0.53
15	83.56	0.99	0.64	0.57
20	70.25	0.83	0.48	0.57
25	60.90	0.72	0.37	0.55
30	53.93	0.64	0.28	0.51
35	48.52	0.58	0.22	0.46
40	44.18	0.52	0.17	0.41
45	40.63	0.48	0.13	0.34
50	37.65	0.45	0.09	0.27
55	35.12	0.42	0.06	0.20
60	32.94	0.39	0.04	0.13
65	31.04	0.37	0.01	0.05
70	29.37	0.35	-0.01	-0.03
75	27.89	0.33	-0.02	-0.11
80	26.56	0.31	-0.04	-0.19
85	25.37	0.30	-0.05	-0.28
90	24.29	0.29	-0.07	-0.36

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
10	0.23	0.07
13	0.34	0.08
16	0.48	0.09
20	0.66	0.10
24	0.88	0.11
28	1.14	0.12
33	1.45	0.13
39	1.80	0.14
44	2.22	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.095 m
0.66	0.57	0.48	Qallow =	0.36 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

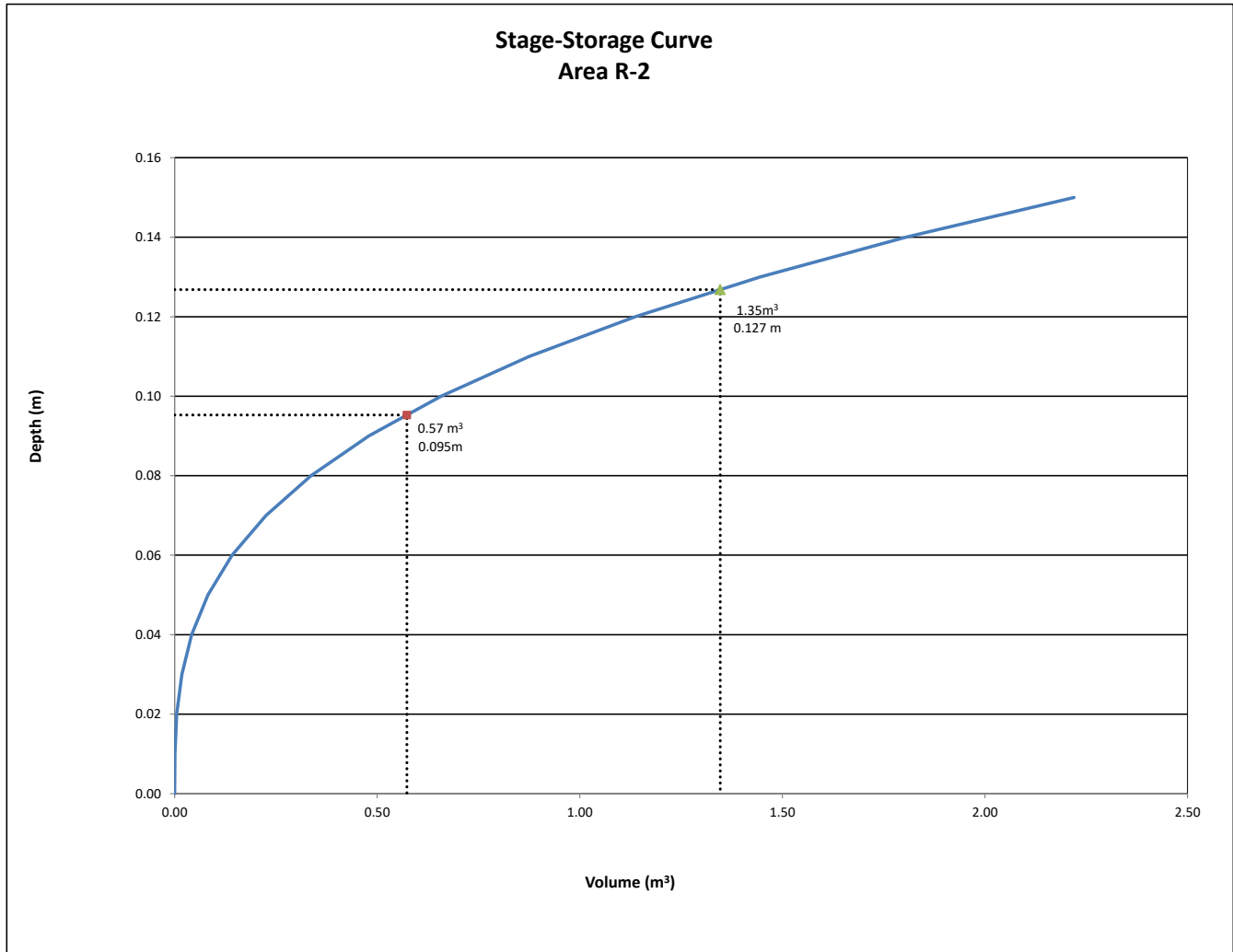
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-2 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.005	ha	Qallow =	0.47
C =	1.00		Vol(max) =	1.35
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.20	2.73	0.82
10	178.56	2.35	1.88	1.13
15	142.89	1.88	1.41	1.27
20	119.95	1.58	1.11	1.33
25	103.85	1.37	0.90	1.35
30	91.87	1.21	0.74	1.33
35	82.58	1.09	0.62	1.30
40	75.15	0.99	0.52	1.25
45	69.05	0.91	0.44	1.19
50	63.95	0.84	0.37	1.12
55	59.62	0.79	0.32	1.04
60	55.89	0.74	0.27	0.96
65	52.65	0.69	0.22	0.87
70	49.79	0.66	0.19	0.78
75	47.26	0.62	0.15	0.69
80	44.99	0.59	0.12	0.59
85	42.95	0.57	0.10	0.49
90	41.11	0.54	0.07	0.39

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
10	0.23	0.07
13	0.34	0.08
16	0.48	0.09
20	0.66	0.10
24	0.88	0.11
28	1.14	0.12
33	1.45	0.13
39	1.80	0.14
44	2.22	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.127 m
1.45	1.35	1.14	Qallow =	0.47 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-3 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.005	ha	Qallow =	0.36
C =	0.90		Vol(max) =	0.58
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.70	1.34	0.40
10	104.19	1.25	0.89	0.54
15	83.56	1.01	0.65	0.58
20	70.25	0.85	0.49	0.58
25	60.90	0.73	0.37	0.56
30	53.93	0.65	0.29	0.52
35	48.52	0.58	0.22	0.47
40	44.18	0.53	0.17	0.41
45	40.63	0.49	0.13	0.35
50	37.65	0.45	0.09	0.28
55	35.12	0.42	0.06	0.21
60	32.94	0.40	0.04	0.13
65	31.04	0.37	0.01	0.05
70	29.37	0.35	-0.01	-0.03
75	27.89	0.34	-0.02	-0.11
80	26.56	0.32	-0.04	-0.19
85	25.37	0.31	-0.05	-0.28
90	24.29	0.29	-0.07	-0.37

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
10	0.23	0.07
13	0.34	0.08
16	0.48	0.09
20	0.66	0.10
24	0.88	0.11
28	1.14	0.12
33	1.44	0.13
39	1.80	0.14
44	2.22	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.096 m
0.66	0.58	0.48	Qallow =	0.36 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

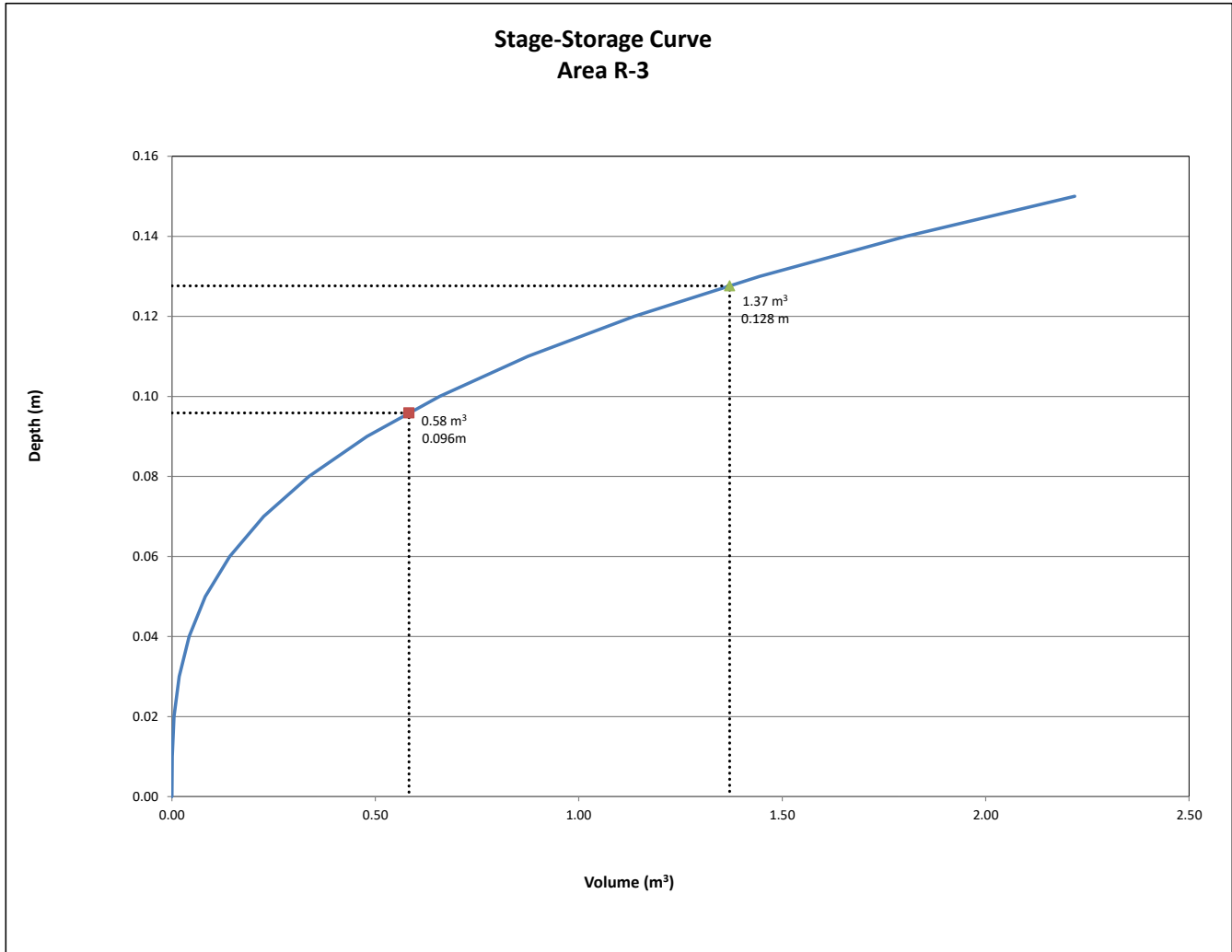
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-3 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.005	ha	Qallow =	0.48
C =	1.00		Vol(max) =	1.37
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.25	2.77	0.83
10	178.56	2.39	1.91	1.15
15	142.89	1.91	1.44	1.29
20	119.95	1.60	1.13	1.36
25	103.85	1.39	0.91	1.37
30	91.87	1.23	0.75	1.36
35	82.58	1.10	0.63	1.32
40	75.15	1.01	0.53	1.27
45	69.05	0.92	0.45	1.21
50	63.95	0.86	0.38	1.14
55	59.62	0.80	0.32	1.06
60	55.89	0.75	0.27	0.98
65	52.65	0.70	0.23	0.89
70	49.79	0.67	0.19	0.80
75	47.26	0.63	0.16	0.71
80	44.99	0.60	0.13	0.61
85	42.95	0.57	0.10	0.51
90	41.11	0.55	0.07	0.40

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
10	0.23	0.07
13	0.34	0.08
16	0.48	0.09
20	0.66	0.10
24	0.88	0.11
28	1.14	0.12
33	1.44	0.13
39	1.80	0.14
44	2.22	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.128 m
1.44	1.37	1.14	Qallow =	0.48 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-4 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.009	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.53
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.27	2.87	0.86
10	104.19	2.41	2.01	1.21
15	83.56	1.93	1.53	1.38
20	70.25	1.63	1.23	1.47
25	60.90	1.41	1.01	1.51
30	53.93	1.25	0.85	1.53
35	48.52	1.12	0.72	1.52
40	44.18	1.02	0.62	1.49
45	40.63	0.94	0.54	1.46
50	37.65	0.87	0.47	1.41
55	35.12	0.81	0.41	1.36
60	32.94	0.76	0.36	1.30
65	31.04	0.72	0.32	1.24
70	29.37	0.68	0.28	1.18
75	27.89	0.65	0.25	1.10
80	26.56	0.61	0.21	1.03
85	25.37	0.59	0.19	0.95
90	24.29	0.56	0.16	0.88

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.16	0.05
14	0.27	0.06
18	0.43	0.07
24	0.64	0.08
30	0.91	0.09
38	1.25	0.10
45	1.67	0.11
54	2.16	0.12
63	2.75	0.13
74	3.43	0.14
84	4.22	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.107 m
1.67	1.53	1.25	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

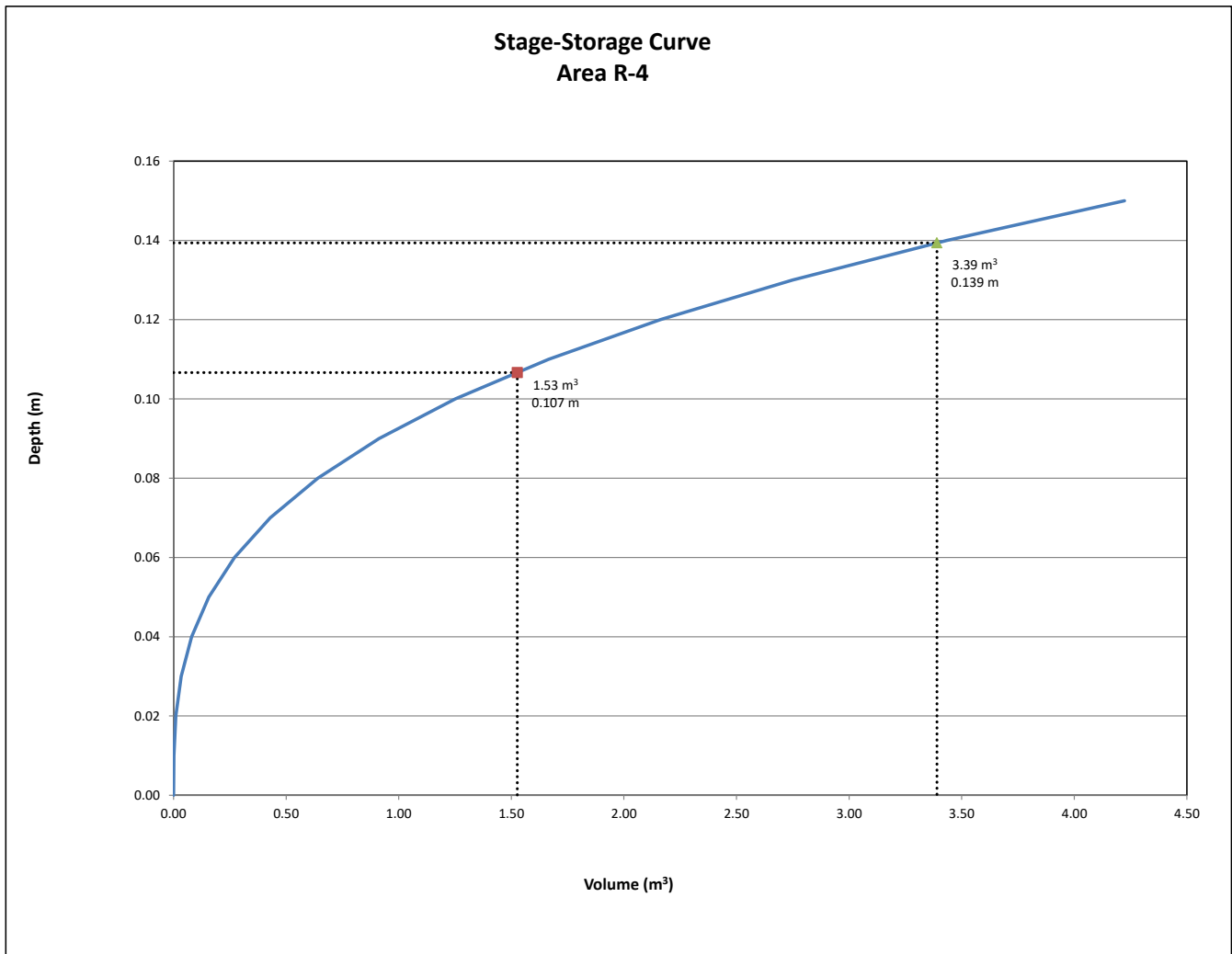
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-4 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.009	ha	Qallow =	0.52
C =	1.00		Vol(max) =	3.39
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.24	5.72	1.72
10	178.56	4.59	4.07	2.44
15	142.89	3.67	3.15	2.84
20	119.95	3.08	2.56	3.08
25	103.85	2.67	2.15	3.23
30	91.87	2.36	1.84	3.32
35	82.58	2.12	1.60	3.37
40	75.15	1.93	1.41	3.39
45	69.05	1.78	1.26	3.39
50	63.95	1.64	1.12	3.37
55	59.62	1.53	1.01	3.34
60	55.89	1.44	0.92	3.30
65	52.65	1.35	0.83	3.25
70	49.79	1.28	0.76	3.19
75	47.26	1.22	0.70	3.13
80	44.99	1.16	0.64	3.06
85	42.95	1.10	0.58	2.98
90	41.11	1.06	0.54	2.90

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.16	0.05
14	0.27	0.06
18	0.43	0.07
24	0.64	0.08
30	0.91	0.09
38	1.25	0.10
45	1.67	0.11
54	2.16	0.12
63	2.75	0.13
74	3.43	0.14
84	4.22	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.139 m
3.43	3.39	2.75	Qallow =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-5 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0043	ha	Qallow =	0.36
C =	0.90		Vol(max) =	0.48
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.52	1.16	0.35
10	104.19	1.12	0.76	0.46
15	83.56	0.90	0.54	0.48
20	70.25	0.75	0.39	0.47
25	60.90	0.65	0.29	0.44
30	53.93	0.58	0.22	0.40
35	48.52	0.52	0.16	0.34
40	44.18	0.47	0.11	0.28
45	40.63	0.44	0.08	0.21
50	37.65	0.40	0.04	0.13
55	35.12	0.38	0.02	0.06
60	32.94	0.35	-0.01	-0.02
65	31.04	0.33	-0.03	-0.10
70	29.37	0.32	-0.04	-0.19
75	27.89	0.30	-0.06	-0.27
80	26.56	0.29	-0.07	-0.36
85	25.37	0.27	-0.09	-0.45
90	24.29	0.26	-0.10	-0.53

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
9	0.22	0.07
12	0.33	0.08
15	0.46	0.09
19	0.64	0.10
23	0.85	0.11
28	1.10	0.12
32	1.40	0.13
37	1.75	0.14
43	2.15	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.096 m
1.10	0.48	0.85	Qallow =	0.36 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

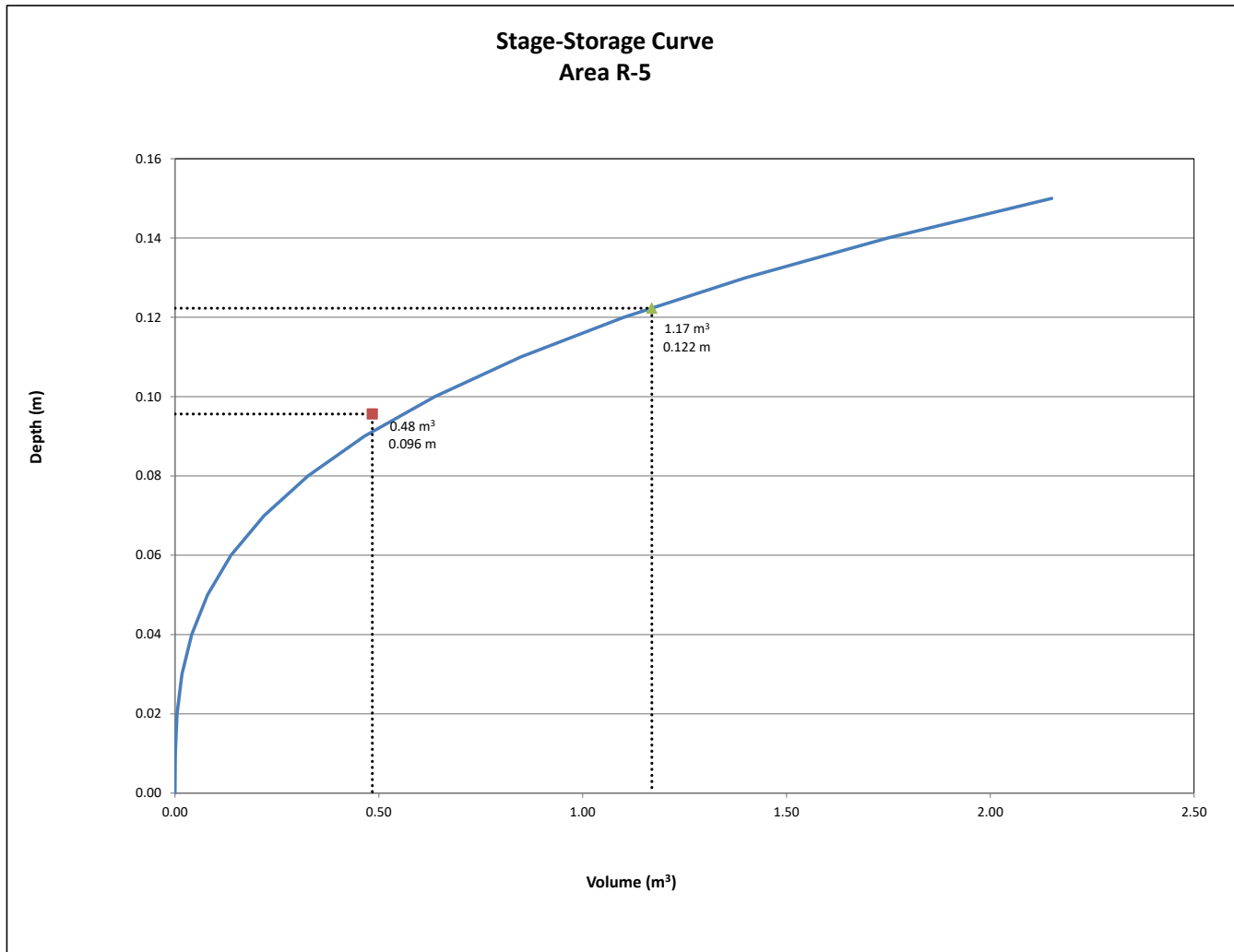
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-5 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0043	ha	Qallow =	0.46
C =	1.00		Vol(max) =	1.17
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.90	2.44	0.73
10	178.56	2.13	1.67	1.00
15	142.89	1.71	1.25	1.12
20	119.95	1.43	0.97	1.17
25	103.85	1.24	0.78	1.17
30	91.87	1.10	0.64	1.15
35	82.58	0.99	0.53	1.10
40	75.15	0.90	0.44	1.05
45	69.05	0.82	0.36	0.98
50	63.95	0.76	0.30	0.91
55	59.62	0.71	0.25	0.83
60	55.89	0.67	0.21	0.75
65	52.65	0.63	0.17	0.66
70	49.79	0.59	0.13	0.56
75	47.26	0.56	0.10	0.47
80	44.99	0.54	0.08	0.37
85	42.95	0.51	0.05	0.27
90	41.11	0.49	0.03	0.17

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
9	0.22	0.07
12	0.33	0.08
15	0.46	0.09
19	0.64	0.10
23	0.85	0.11
28	1.10	0.12
32	1.40	0.13
37	1.75	0.14
43	2.15	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.122 m
1.40	1.17	1.10	Qallow =	0.46 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-6 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0102	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.76
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.60	3.20	0.96
10	104.19	2.65	2.25	1.35
15	83.56	2.13	1.73	1.56
20	70.25	1.79	1.39	1.67
25	60.90	1.55	1.15	1.73
30	53.93	1.37	0.97	1.75
35	48.52	1.24	0.84	1.76
40	44.18	1.13	0.73	1.74
45	40.63	1.03	0.63	1.71
50	37.65	0.96	0.56	1.68
55	35.12	0.89	0.49	1.63
60	32.94	0.84	0.44	1.58
65	31.04	0.79	0.39	1.52
70	29.37	0.75	0.35	1.46
75	27.89	0.71	0.31	1.40
80	26.56	0.68	0.28	1.33
85	25.37	0.65	0.25	1.26
90	24.29	0.62	0.22	1.18

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
7	0.09	0.04
11	0.18	0.05
15	0.31	0.06
21	0.49	0.07
28	0.73	0.08
35	1.04	0.09
43	1.43	0.10
52	1.91	0.11
62	2.48	0.12
73	3.15	0.13
84	3.93	0.14
97	4.84	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.107 m
1.91	1.76	1.43	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

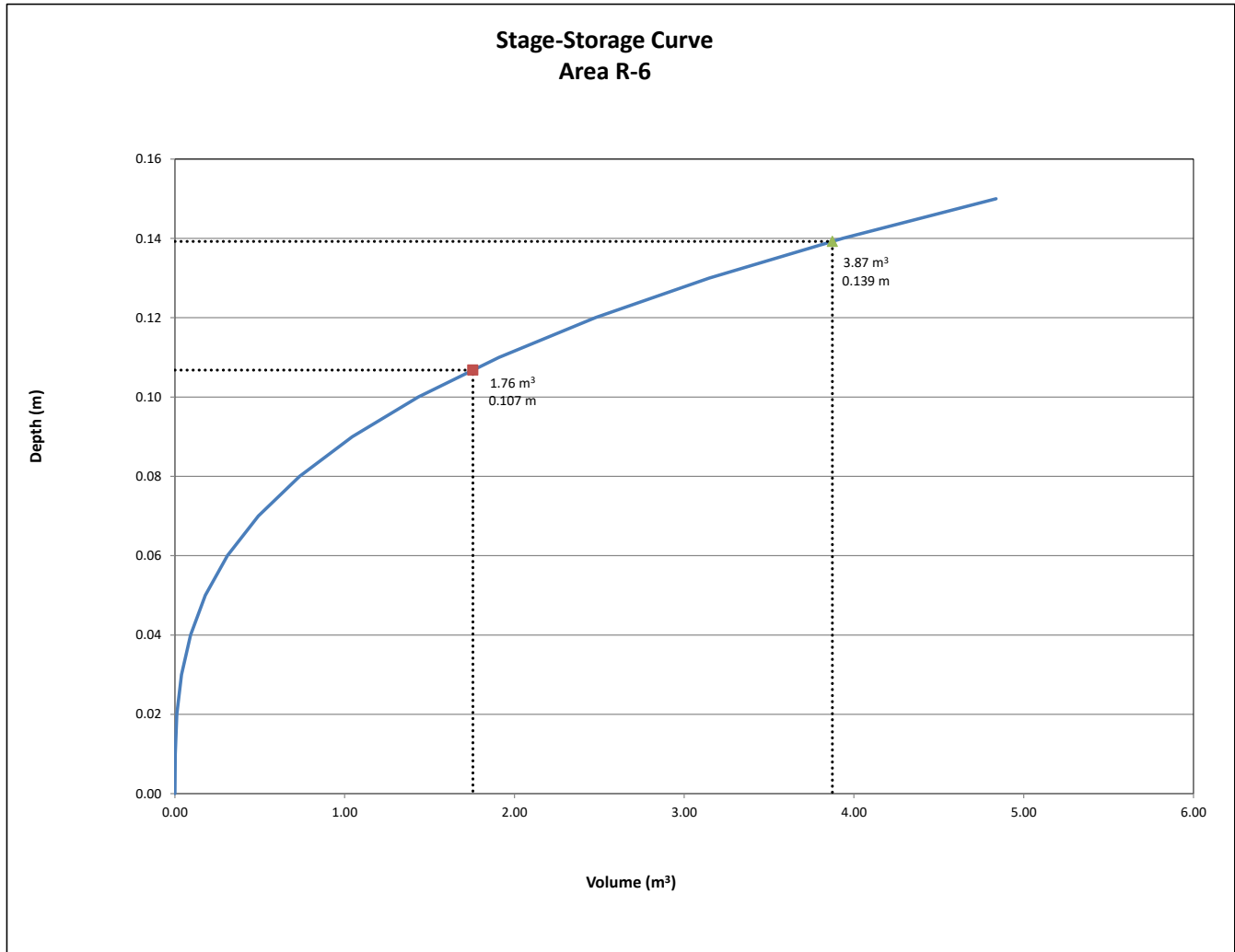
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-6 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0102	ha	Qallow =	0.52
C =	1.00		Vol(max) =	3.87
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.87	6.35	1.90
10	178.56	5.05	4.53	2.72
15	142.89	4.04	3.52	3.17
20	119.95	3.39	2.87	3.45
25	103.85	2.94	2.42	3.63
30	91.87	2.60	2.08	3.74
35	82.58	2.34	1.82	3.82
40	75.15	2.13	1.61	3.86
45	69.05	1.95	1.43	3.87
50	63.95	1.81	1.29	3.87
55	59.62	1.69	1.17	3.85
60	55.89	1.58	1.06	3.82
65	52.65	1.49	0.97	3.78
70	49.79	1.41	0.89	3.73
75	47.26	1.34	0.82	3.68
80	44.99	1.27	0.75	3.62
85	42.95	1.22	0.70	3.55
90	41.11	1.16	0.64	3.48

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
7	0.09	0.04
11	0.18	0.05
15	0.31	0.06
21	0.49	0.07
28	0.73	0.08
35	1.04	0.09
43	1.43	0.10
52	1.91	0.11
62	2.48	0.12
73	3.15	0.13
84	3.93	0.14
97	4.84	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.139 m
3.93	3.87	3.15	Qallow =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-7 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0114	ha	Qallow =	0.40
C =	0.90		Vol(max) =	2.07
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	4.03	3.63	1.09
10	104.19	2.98	2.58	1.55
15	83.56	2.39	1.99	1.79
20	70.25	2.01	1.61	1.93
25	60.90	1.74	1.34	2.01
30	53.93	1.54	1.14	2.05
35	48.52	1.39	0.99	2.07
40	44.18	1.26	0.86	2.07
45	40.63	1.16	0.76	2.05
50	37.65	1.08	0.68	2.03
55	35.12	1.00	0.60	1.99
60	32.94	0.94	0.54	1.95
65	31.04	0.89	0.49	1.90
70	29.37	0.84	0.44	1.84
75	27.89	0.80	0.40	1.78
80	26.56	0.76	0.36	1.72
85	25.37	0.72	0.32	1.65
90	24.29	0.69	0.29	1.59

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
8	0.10	0.04
12	0.20	0.05
17	0.35	0.06
24	0.55	0.07
31	0.83	0.08
39	1.18	0.09
49	1.62	0.10
59	2.15	0.11
70	2.79	0.12
82	3.55	0.13
95	4.44	0.14
109	5.46	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.108 m
2.15	2.07	1.62	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

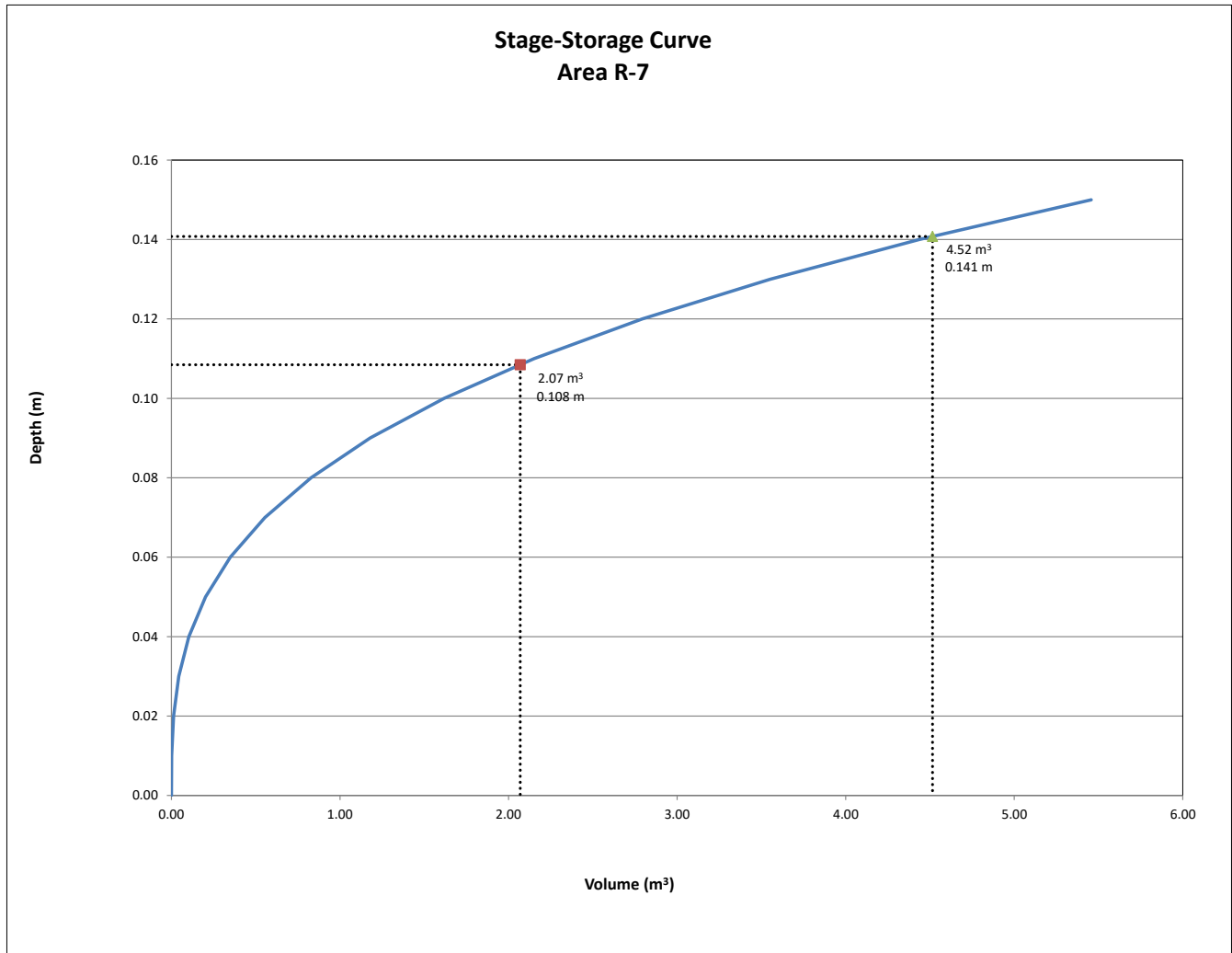
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-7 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0114	ha	Qallow =	0.52
C =	1.00		Vol(max) =	4.52
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	7.70	7.18	2.15
10	178.56	5.67	5.14	3.08
15	142.89	4.53	4.01	3.61
20	119.95	3.81	3.28	3.94
25	103.85	3.29	2.77	4.16
30	91.87	2.91	2.39	4.30
35	82.58	2.62	2.10	4.40
40	75.15	2.38	1.86	4.46
45	69.05	2.19	1.67	4.50
50	63.95	2.03	1.51	4.52
55	59.62	1.89	1.37	4.51
60	55.89	1.77	1.25	4.50
65	52.65	1.67	1.15	4.47
70	49.79	1.58	1.06	4.43
75	47.26	1.50	0.98	4.39
80	44.99	1.43	0.90	4.34
85	42.95	1.36	0.84	4.28
90	41.11	1.30	0.78	4.21

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
8	0.10	0.04
12	0.20	0.05
17	0.35	0.06
24	0.55	0.07
31	0.83	0.08
39	1.18	0.09
49	1.62	0.10
59	2.15	0.11
70	2.79	0.12
82	3.55	0.13
95	4.44	0.14
109	5.46	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.141 m
5.46	4.52	4.44	Qallow =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-8 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0017	ha	Qallow =	0.29
C =	0.90		Vol(max) =	0.10
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	0.61	0.33	0.10
10	104.19	0.45	0.17	0.10
15	83.56	0.36	0.08	0.07
20	70.25	0.30	0.02	0.02
25	60.90	0.26	-0.02	-0.03
30	53.93	0.23	-0.05	-0.09
35	48.52	0.21	-0.08	-0.16
40	44.18	0.19	-0.09	-0.23
45	40.63	0.18	-0.11	-0.29
50	37.65	0.16	-0.12	-0.37
55	35.12	0.15	-0.13	-0.44
60	32.94	0.14	-0.14	-0.51
65	31.04	0.13	-0.15	-0.59
70	29.37	0.13	-0.16	-0.66
75	27.89	0.12	-0.16	-0.74
80	26.56	0.11	-0.17	-0.82
85	25.37	0.11	-0.18	-0.89
90	24.29	0.11	-0.18	-0.97

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
0	0.00	0.02
1	0.01	0.03
1	0.01	0.04
2	0.03	0.05
2	0.05	0.06
3	0.07	0.07
4	0.11	0.08
5	0.16	0.09
6	0.22	0.10
8	0.29	0.11
9	0.37	0.12
11	0.47	0.13
13	0.59	0.14
15	0.73	0.15

Linear Interpolation				
0.08	H	0.07	H =	0.077 m
0.11	0.10	0.07	Q _{allow} =	0.29 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

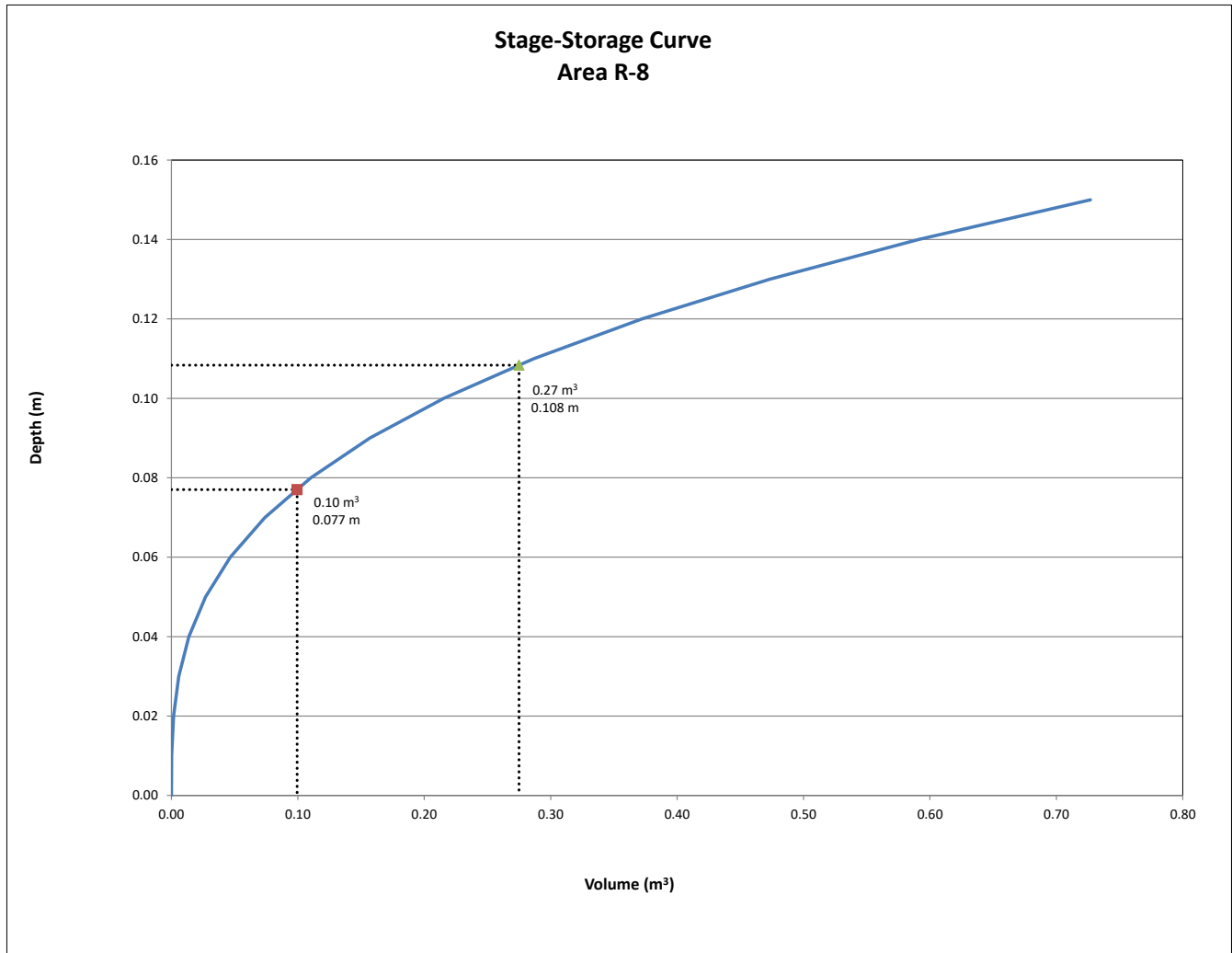
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-8 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0017	ha	Qallow =	0.40
C =	1.00		Vol(max) =	0.27
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	1.17	0.77	0.23
10	178.56	0.86	0.46	0.27
15	142.89	0.69	0.29	0.26
20	119.95	0.58	0.18	0.21
25	103.85	0.50	0.10	0.15
30	91.87	0.44	0.04	0.07
35	82.58	0.40	0.00	-0.01
40	75.15	0.36	-0.04	-0.09
45	69.05	0.33	-0.07	-0.18
50	63.95	0.31	-0.09	-0.28
55	59.62	0.29	-0.11	-0.37
60	55.89	0.27	-0.13	-0.47
65	52.65	0.25	-0.15	-0.57
70	49.79	0.24	-0.16	-0.67
75	47.26	0.23	-0.17	-0.78
80	44.99	0.22	-0.18	-0.88
85	42.95	0.21	-0.19	-0.99
90	41.11	0.20	-0.20	-1.09

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
0	0.00	0.02
1	0.01	0.03
1	0.01	0.04
2	0.03	0.05
2	0.05	0.06
3	0.07	0.07
4	0.11	0.08
5	0.16	0.09
6	0.22	0.10
8	0.29	0.11
9	0.37	0.12
11	0.47	0.13
13	0.59	0.14
15	0.73	0.15

Linear Interpolation				
0.11	H	0.1	H =	0.108 m
0.29	0.27	0.22	Q _{allow} =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-9 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0042	ha	Qallow =	0.30
C =	0.90		Vol(max) =	0.52
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.48	1.18	0.35
10	104.19	1.09	0.79	0.47
15	83.56	0.87	0.57	0.52
20	70.25	0.73	0.43	0.52
25	60.90	0.64	0.34	0.50
30	53.93	0.56	0.26	0.47
35	48.52	0.51	0.21	0.43
40	44.18	0.46	0.16	0.39
45	40.63	0.42	0.12	0.34
50	37.65	0.39	0.09	0.28
55	35.12	0.37	0.07	0.22
60	32.94	0.34	0.04	0.16
65	31.04	0.32	0.02	0.09
70	29.37	0.31	0.01	0.03
75	27.89	0.29	-0.01	-0.04
80	26.56	0.28	-0.02	-0.11
85	25.37	0.27	-0.03	-0.18
90	24.29	0.25	-0.05	-0.25

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
7	0.12	0.05
10	0.20	0.06
14	0.33	0.07
18	0.49	0.08
23	0.69	0.09
28	0.95	0.10
34	1.26	0.11
41	1.64	0.12
48	2.08	0.13
56	2.60	0.14
64	3.20	0.15

Linear Interpolation				
0.09	H	0.08	H =	0.082 m
0.69	0.52	0.49	Qallow =	0.30 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

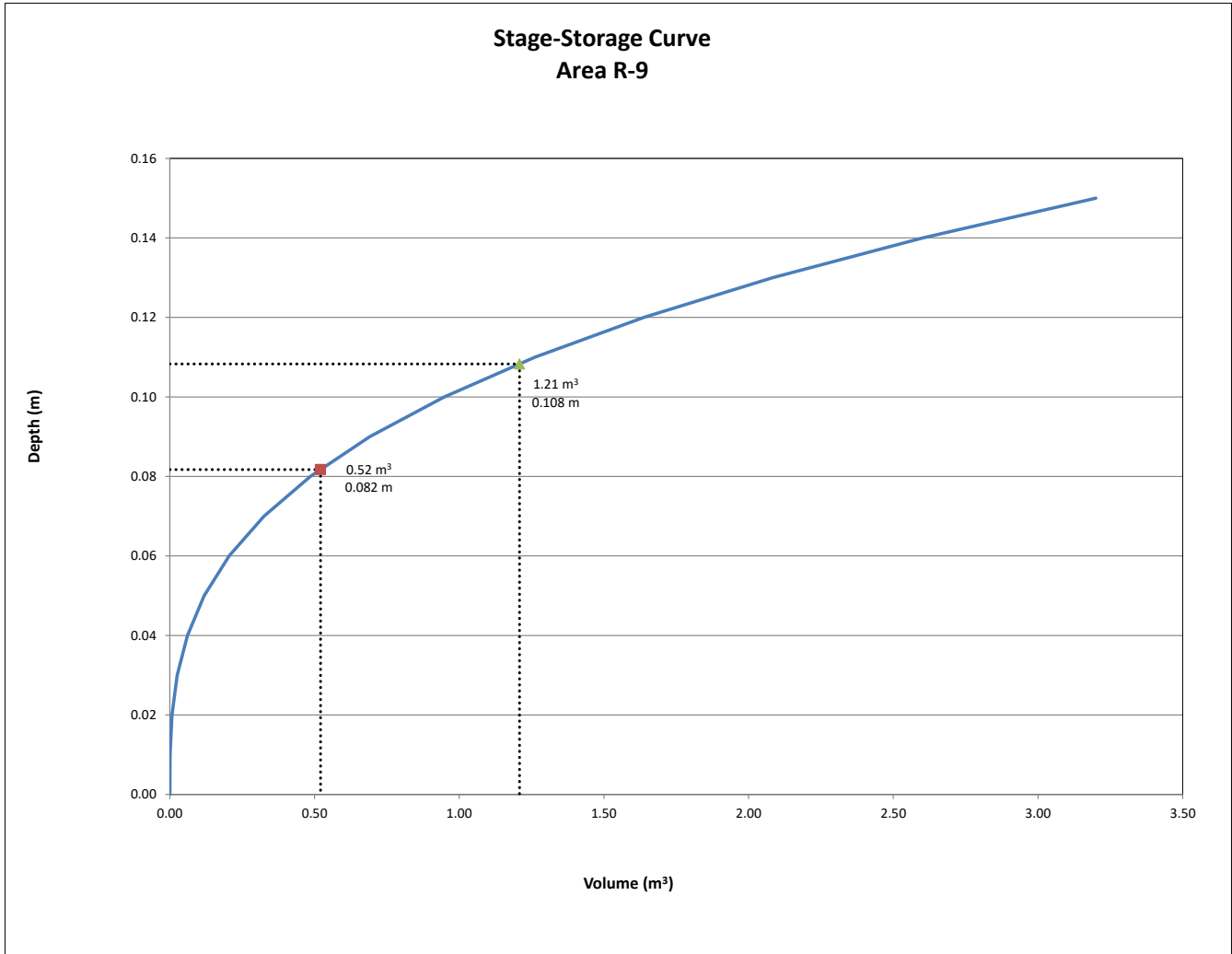
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-9 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0042	ha	Qallow =	0.40
C =	1.00		Vol(max) =	1.21
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.82	2.42	0.73
10	178.56	2.07	1.67	1.00
15	142.89	1.66	1.26	1.13
20	119.95	1.39	0.99	1.19
25	103.85	1.21	0.81	1.21
30	91.87	1.07	0.67	1.20
35	82.58	0.96	0.56	1.17
40	75.15	0.87	0.47	1.13
45	69.05	0.80	0.40	1.08
50	63.95	0.74	0.34	1.03
55	59.62	0.69	0.29	0.96
60	55.89	0.65	0.25	0.90
65	52.65	0.61	0.21	0.82
70	49.79	0.58	0.18	0.75
75	47.26	0.55	0.15	0.67
80	44.99	0.52	0.12	0.59
85	42.95	0.50	0.10	0.50
90	41.11	0.48	0.08	0.42

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
7	0.12	0.05
10	0.20	0.06
14	0.33	0.07
18	0.49	0.08
23	0.69	0.09
28	0.95	0.10
34	1.26	0.11
41	1.64	0.12
48	2.08	0.13
56	2.60	0.14
64	3.20	0.15

Linear Interpolation				
0.11	H	0.1	H =	0.108 m
1.26	1.21	0.95	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-10 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0154	ha	Qallow =	0.43
C =	0.90		Vol(max) =	3.06
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	5.43	5.00	1.50
10	104.19	4.00	3.57	2.14
15	83.56	3.21	2.78	2.50
20	70.25	2.70	2.27	2.72
25	60.90	2.34	1.91	2.87
30	53.93	2.07	1.64	2.96
35	48.52	1.86	1.43	3.01
40	44.18	1.70	1.27	3.04
45	40.63	1.56	1.13	3.06
50	37.65	1.45	1.02	3.05
55	35.12	1.35	0.92	3.04
60	32.94	1.27	0.84	3.01
65	31.04	1.19	0.76	2.98
70	29.37	1.13	0.70	2.94
75	27.89	1.07	0.64	2.89
80	26.56	1.02	0.59	2.84
85	25.37	0.98	0.55	2.78
90	24.29	0.93	0.50	2.72

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
1	0.00	0.01
2	0.02	0.02
5	0.05	0.03
10	0.13	0.04
15	0.25	0.05
22	0.43	0.06
29	0.69	0.07
38	1.02	0.08
49	1.46	0.09
60	2.00	0.10
73	2.66	0.11
86	3.45	0.12
101	4.39	0.13
118	5.48	0.14
135	6.75	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.115 m
3.45	3.06	2.66	Q _{allow} =	0.43 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

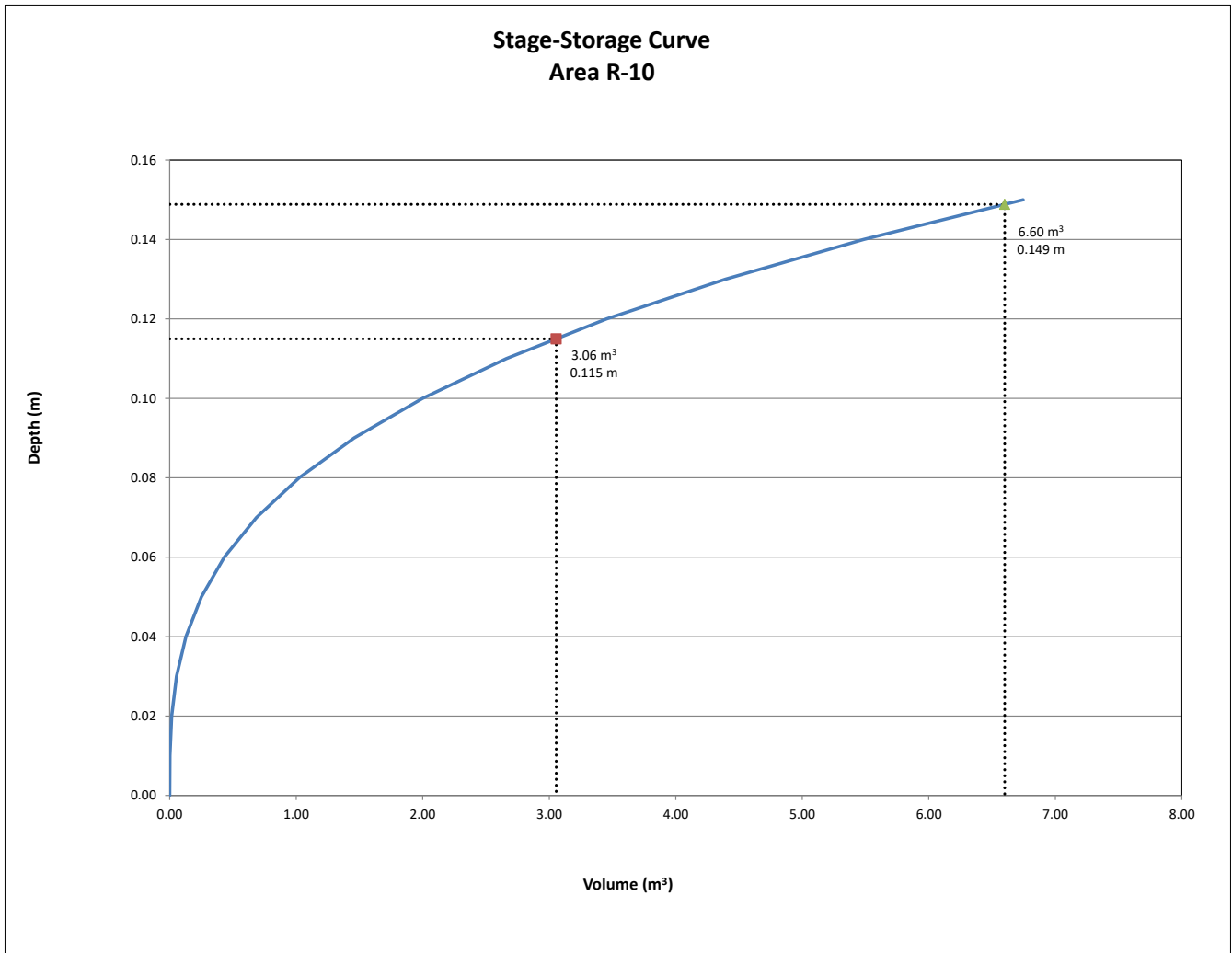
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-10 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0154	ha	Qallow =	0.56
C =	1.00		Vol(max) =	6.60
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	10.36	9.81	2.94
10	178.56	7.63	7.07	4.24
15	142.89	6.10	5.55	4.99
20	119.95	5.12	4.57	5.48
25	103.85	4.43	3.88	5.82
30	91.87	3.92	3.37	6.06
35	82.58	3.53	2.97	6.24
40	75.15	3.21	2.65	6.37
45	69.05	2.95	2.39	6.46
50	63.95	2.73	2.18	6.53
55	59.62	2.55	1.99	6.57
60	55.89	2.39	1.83	6.59
65	52.65	2.25	1.69	6.60
70	49.79	2.13	1.57	6.60
75	47.26	2.02	1.46	6.58
80	44.99	1.92	1.37	6.55
85	42.95	1.83	1.28	6.52
90	41.11	1.76	1.20	6.48

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
1	0.00	0.01
2	0.02	0.02
5	0.05	0.03
10	0.13	0.04
15	0.25	0.05
22	0.43	0.06
29	0.69	0.07
38	1.02	0.08
49	1.46	0.09
60	2.00	0.10
73	2.66	0.11
86	3.45	0.12
101	4.39	0.13
118	5.48	0.14
135	6.75	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.149 m
6.75	6.60	5.48	Q _{allow} =	0.56 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-11 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0104	ha	Qallow =	0.42
C =	0.90		Vol(max) =	1.77
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.67	3.25	0.98
10	104.19	2.71	2.29	1.37
15	83.56	2.17	1.75	1.58
20	70.25	1.83	1.41	1.69
25	60.90	1.58	1.16	1.75
30	53.93	1.40	0.98	1.77
35	48.52	1.26	0.84	1.77
40	44.18	1.15	0.73	1.75
45	40.63	1.06	0.64	1.72
50	37.65	0.98	0.56	1.68
55	35.12	0.91	0.49	1.63
60	32.94	0.86	0.44	1.57
65	31.04	0.81	0.39	1.51
70	29.37	0.76	0.34	1.45
75	27.89	0.73	0.31	1.38
80	26.56	0.69	0.27	1.30
85	25.37	0.66	0.24	1.22
90	24.29	0.63	0.21	1.14

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.16	0.05
14	0.27	0.06
18	0.43	0.07
24	0.64	0.08
30	0.91	0.09
38	1.25	0.10
45	1.67	0.11
54	2.17	0.12
64	2.75	0.13
74	3.44	0.14
85	4.23	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.112 m
2.17	1.77	1.67	Q _{allow} =	0.42 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

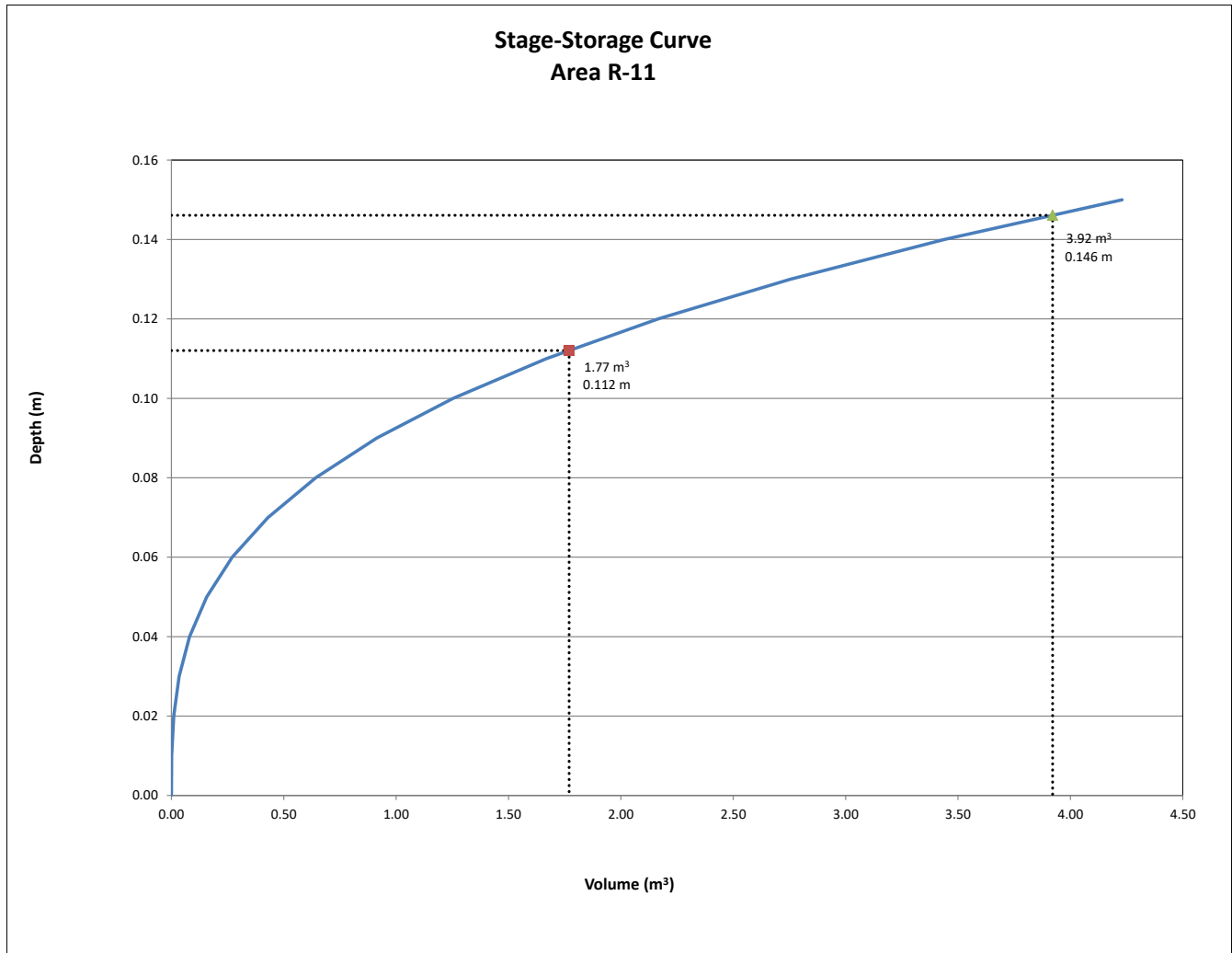
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-11 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0104	ha	Qallow =	0.54
C =	1.00		Vol(max) =	3.92
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	7.02	6.47	1.94
10	178.56	5.16	4.62	2.77
15	142.89	4.13	3.59	3.23
20	119.95	3.47	2.92	3.51
25	103.85	3.00	2.46	3.69
30	91.87	2.66	2.11	3.80
35	82.58	2.39	1.84	3.87
40	75.15	2.17	1.63	3.91
45	69.05	2.00	1.45	3.92
50	63.95	1.85	1.30	3.91
55	59.62	1.72	1.18	3.89
60	55.89	1.62	1.07	3.86
65	52.65	1.52	0.98	3.81
70	49.79	1.44	0.90	3.76
75	47.26	1.37	0.82	3.70
80	44.99	1.30	0.76	3.63
85	42.95	1.24	0.70	3.56
90	41.11	1.19	0.64	3.48

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.16	0.05
14	0.27	0.06
18	0.43	0.07
24	0.64	0.08
30	0.91	0.09
38	1.25	0.10
45	1.67	0.11
54	2.17	0.12
64	2.75	0.13
74	3.44	0.14
85	4.23	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.146 m
4.23	3.92	3.44	Q _{allow} =	0.54 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-12 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0063	ha	Qallow =	0.38
C =	0.90		Vol(max) =	0.87
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.23	1.85	0.55
10	104.19	1.64	1.26	0.76
15	83.56	1.32	0.94	0.84
20	70.25	1.11	0.73	0.87
25	60.90	0.96	0.58	0.87
30	53.93	0.85	0.47	0.85
35	48.52	0.77	0.39	0.81
40	44.18	0.70	0.32	0.76
45	40.63	0.64	0.26	0.70
50	37.65	0.59	0.21	0.64
55	35.12	0.55	0.17	0.57
60	32.94	0.52	0.14	0.50
65	31.04	0.49	0.11	0.43
70	29.37	0.46	0.08	0.35
75	27.89	0.44	0.06	0.27
80	26.56	0.42	0.04	0.19
85	25.37	0.40	0.02	0.10
90	24.29	0.38	0.00	0.02

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.11	0.05
9	0.18	0.06
12	0.29	0.07
16	0.44	0.08
21	0.62	0.09
25	0.85	0.10
31	1.13	0.11
37	1.47	0.12
43	1.87	0.13
50	2.33	0.14
57	2.87	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.101 m
1.13	0.87	0.85	Q _{allow} =	0.38 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

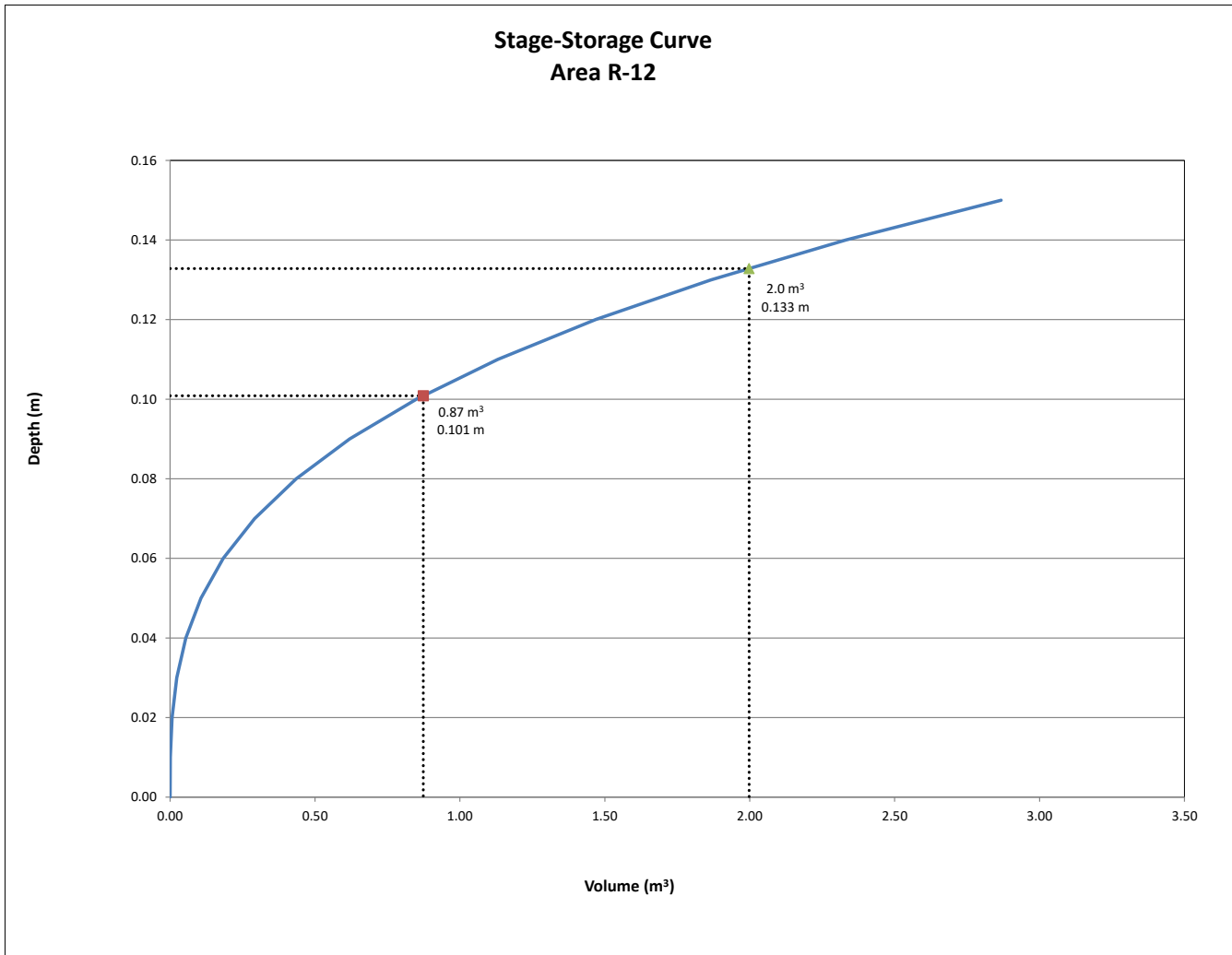
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-12 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0063	ha	Qallow =	0.50
C =	1.00		Vol(max) =	2.00
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.25	3.75	1.13
10	178.56	3.13	2.63	1.58
15	142.89	2.50	2.00	1.80
20	119.95	2.10	1.60	1.92
25	103.85	1.82	1.32	1.98
30	91.87	1.61	1.11	2.00
35	82.58	1.45	0.95	1.99
40	75.15	1.32	0.82	1.96
45	69.05	1.21	0.71	1.92
50	63.95	1.12	0.62	1.86
55	59.62	1.04	0.54	1.80
60	55.89	0.98	0.48	1.73
65	52.65	0.92	0.42	1.65
70	49.79	0.87	0.37	1.56
75	47.26	0.83	0.33	1.48
80	44.99	0.79	0.29	1.38
85	42.95	0.75	0.25	1.29
90	41.11	0.72	0.22	1.19

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.11	0.05
9	0.18	0.06
12	0.29	0.07
16	0.44	0.08
21	0.62	0.09
25	0.85	0.10
31	1.13	0.11
37	1.47	0.12
43	1.87	0.13
50	2.33	0.14
57	2.87	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.133 m
2.33	2.00	1.87	Q _{allow} =	0.50 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-13 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0049	ha	Qallow =	0.38
C =	0.90		Vol(max) =	0.58
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.72	1.34	0.40
10	104.19	1.27	0.89	0.54
15	83.56	1.02	0.64	0.58
20	70.25	0.86	0.48	0.58
25	60.90	0.74	0.37	0.55
30	53.93	0.66	0.28	0.51
35	48.52	0.59	0.22	0.45
40	44.18	0.54	0.16	0.39
45	40.63	0.49	0.12	0.32
50	37.65	0.46	0.08	0.25
55	35.12	0.43	0.05	0.17
60	32.94	0.40	0.03	0.09
65	31.04	0.38	0.00	0.01
70	29.37	0.36	-0.02	-0.07
75	27.89	0.34	-0.04	-0.16
80	26.56	0.32	-0.05	-0.25
85	25.37	0.31	-0.07	-0.34
90	24.29	0.30	-0.08	-0.43

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
2	0.02	0.03
3	0.04	0.04
4	0.07	0.05
6	0.12	0.06
8	0.19	0.07
11	0.29	0.08
14	0.41	0.09
17	0.57	0.10
21	0.75	0.11
24	0.98	0.12
29	1.24	0.13
33	1.55	0.14
38	1.91	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.101 m
0.75	0.58	0.57	Qallow =	0.38 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

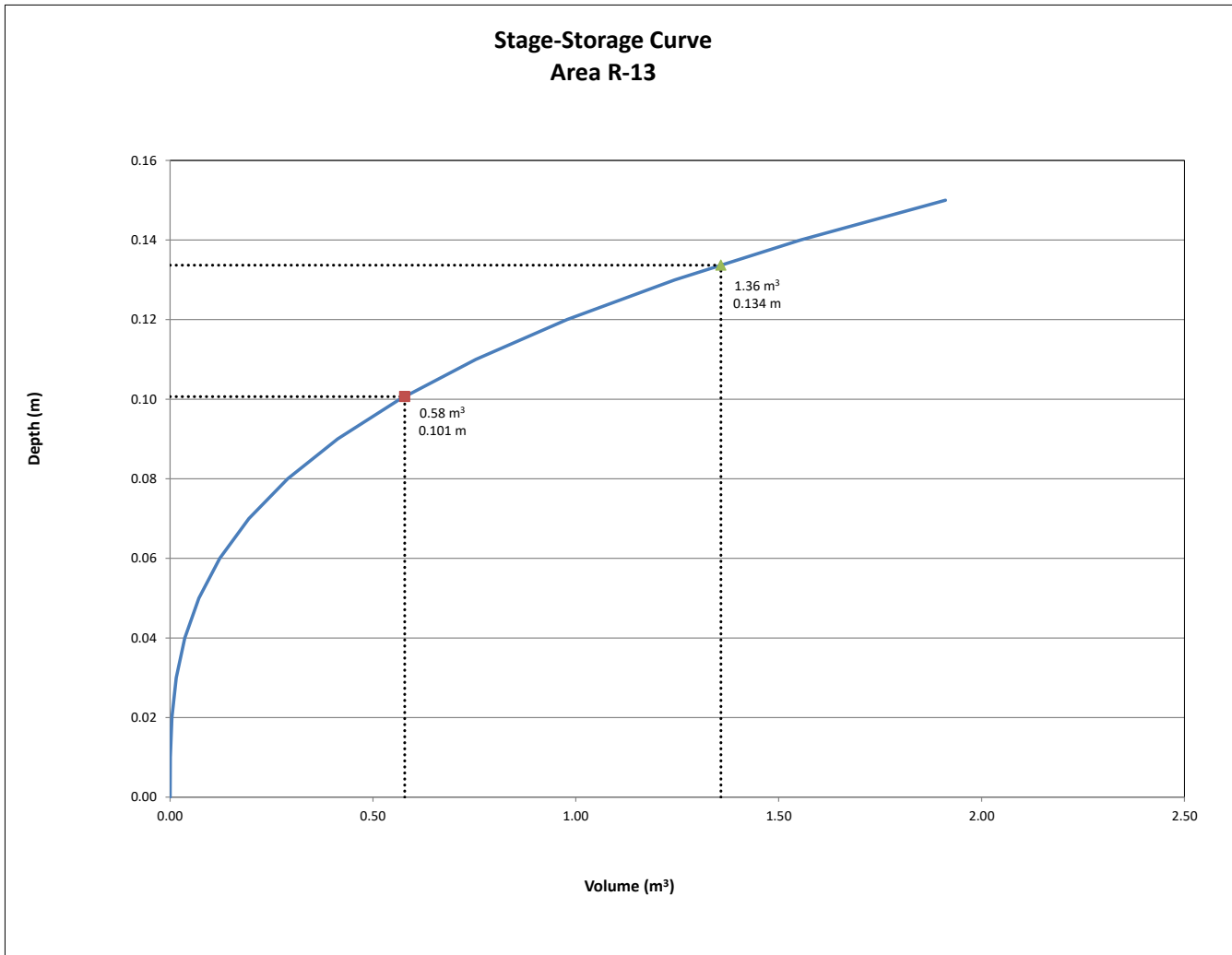
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-13 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0049	ha	Qallow =	0.50
C =	1.00		Vol(max) =	1.36
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.28	2.78	0.84
10	178.56	2.42	1.92	1.15
15	142.89	1.93	1.43	1.29
20	119.95	1.62	1.12	1.35
25	103.85	1.41	0.91	1.36
30	91.87	1.24	0.74	1.34
35	82.58	1.12	0.62	1.30
40	75.15	1.02	0.52	1.24
45	69.05	0.93	0.43	1.17
50	63.95	0.87	0.37	1.10
55	59.62	0.81	0.31	1.01
60	55.89	0.76	0.26	0.92
65	52.65	0.71	0.21	0.83
70	49.79	0.67	0.17	0.73
75	47.26	0.64	0.14	0.63
80	44.99	0.61	0.11	0.52
85	42.95	0.58	0.08	0.41
90	41.11	0.56	0.06	0.30

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
2	0.02	0.03
3	0.04	0.04
4	0.07	0.05
6	0.12	0.06
8	0.19	0.07
11	0.29	0.08
14	0.41	0.09
17	0.57	0.10
21	0.75	0.11
24	0.98	0.12
29	1.24	0.13
33	1.55	0.14
38	1.91	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.134 m
1.55	1.36	1.24	Qallow =	0.50 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-14 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0059	ha	Qallow =	0.36
C =	0.90		Vol(max) =	0.81
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.08	1.72	0.52
10	104.19	1.54	1.18	0.71
15	83.56	1.23	0.87	0.79
20	70.25	1.04	0.68	0.81
25	60.90	0.90	0.54	0.81
30	53.93	0.80	0.44	0.79
35	48.52	0.72	0.36	0.75
40	44.18	0.65	0.29	0.70
45	40.63	0.60	0.24	0.65
50	37.65	0.56	0.20	0.59
55	35.12	0.52	0.16	0.52
60	32.94	0.49	0.13	0.46
65	31.04	0.46	0.10	0.38
70	29.37	0.43	0.07	0.31
75	27.89	0.41	0.05	0.23
80	26.56	0.39	0.03	0.15
85	25.37	0.37	0.01	0.07
90	24.29	0.36	0.00	-0.01

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.66	0.09
27	0.90	0.10
33	1.20	0.11
39	1.56	0.12
46	1.98	0.13
53	2.47	0.14
61	3.04	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.096 m
0.90	0.81	0.66	Q _{allow} =	0.36 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

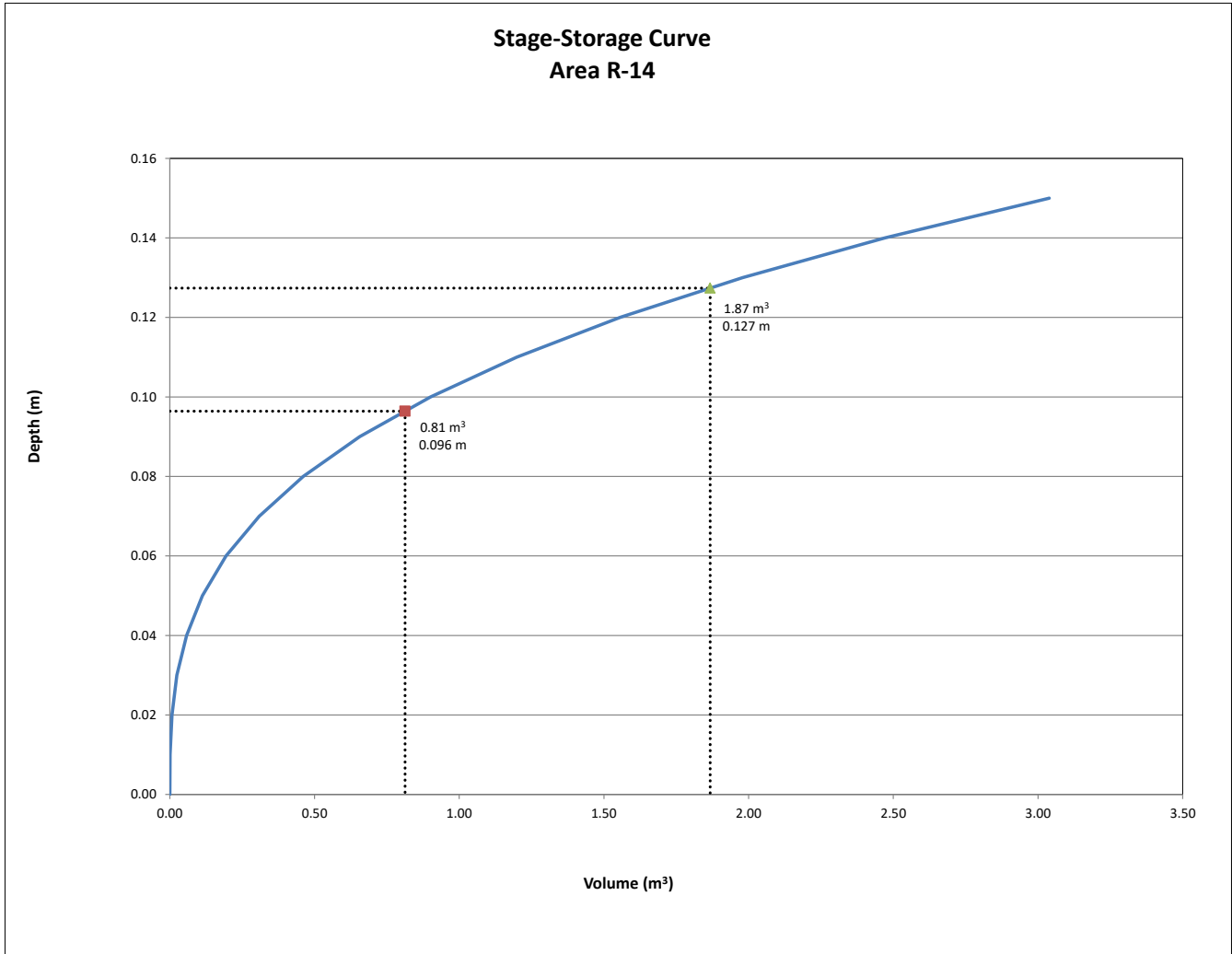
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-14 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0059	ha	Qallow =	0.47
C =	1.00		Vol(max) =	1.87
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.98	3.51	1.05
10	178.56	2.93	2.46	1.48
15	142.89	2.34	1.87	1.69
20	119.95	1.97	1.50	1.80
25	103.85	1.70	1.23	1.85
30	91.87	1.51	1.04	1.87
35	82.58	1.35	0.88	1.86
40	75.15	1.23	0.76	1.83
45	69.05	1.13	0.66	1.79
50	63.95	1.05	0.58	1.74
55	59.62	0.98	0.51	1.68
60	55.89	0.92	0.45	1.61
65	52.65	0.86	0.39	1.54
70	49.79	0.82	0.35	1.46
75	47.26	0.78	0.31	1.37
80	44.99	0.74	0.27	1.29
85	42.95	0.70	0.23	1.20
90	41.11	0.67	0.20	1.10

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.66	0.09
27	0.90	0.10
33	1.20	0.11
39	1.56	0.12
46	1.98	0.13
53	2.47	0.14
61	3.04	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.127 m
1.98	1.87	1.56	Q _{allow} =	0.47 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-15 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0044	ha	Qallow =	0.29
C =	0.90		Vol(max) =	0.58
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.55	1.26	0.38
10	104.19	1.15	0.86	0.51
15	83.56	0.92	0.63	0.57
20	70.25	0.77	0.48	0.58
25	60.90	0.67	0.38	0.57
30	53.93	0.59	0.30	0.55
35	48.52	0.53	0.24	0.51
40	44.18	0.49	0.20	0.47
45	40.63	0.45	0.16	0.42
50	37.65	0.41	0.12	0.37
55	35.12	0.39	0.10	0.32
60	32.94	0.36	0.07	0.26
65	31.04	0.34	0.05	0.20
70	29.37	0.32	0.03	0.14
75	27.89	0.31	0.02	0.08
80	26.56	0.29	0.00	0.01
85	25.37	0.28	-0.01	-0.06
90	24.29	0.27	-0.02	-0.12

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.19	0.10
43	1.58	0.11
51	2.06	0.12
60	2.62	0.13
70	3.27	0.14
80	4.02	0.15

Linear Interpolation				
0.08	H	0.07	H =	0.079 m
0.61	0.58	0.41	Q _{allow} =	0.29 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

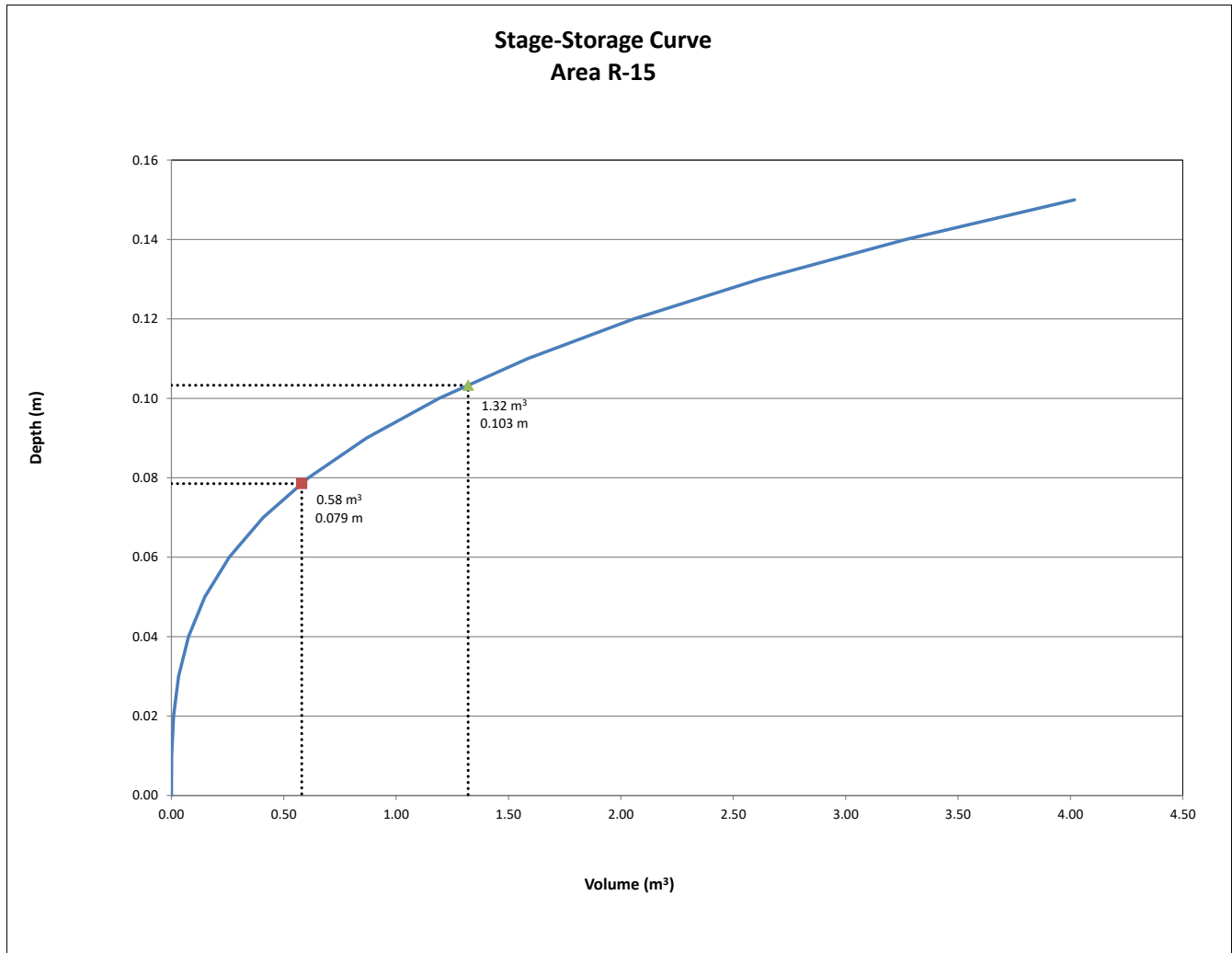
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-15 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0044	ha	Qallow =	0.39
C =	1.00		Vol(max) =	1.32
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.97	2.58	0.77
10	178.56	2.18	1.79	1.08
15	142.89	1.75	1.36	1.22
20	119.95	1.47	1.08	1.29
25	103.85	1.27	0.88	1.32
30	91.87	1.12	0.73	1.32
35	82.58	1.01	0.62	1.30
40	75.15	0.92	0.53	1.27
45	69.05	0.84	0.45	1.23
50	63.95	0.78	0.39	1.18
55	59.62	0.73	0.34	1.12
60	55.89	0.68	0.29	1.06
65	52.65	0.64	0.25	0.99
70	49.79	0.61	0.22	0.92
75	47.26	0.58	0.19	0.85
80	44.99	0.55	0.16	0.77
85	42.95	0.53	0.14	0.69
90	41.11	0.50	0.11	0.61

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.19	0.10
43	1.58	0.11
51	2.06	0.12
60	2.62	0.13
70	3.27	0.14
80	4.02	0.15

Linear Interpolation				
0.11	H	0.1	H =	0.103 m
1.58	1.32	1.19	Q _{allow} =	0.39 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-16 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0084	ha	Qallow =	0.39
C =	0.90		Vol(max) =	1.34
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.97	2.58	0.77
10	104.19	2.19	1.80	1.08
15	83.56	1.76	1.37	1.23
20	70.25	1.48	1.09	1.31
25	60.90	1.28	0.89	1.34
30	53.93	1.13	0.74	1.34
35	48.52	1.02	0.63	1.33
40	44.18	0.93	0.54	1.30
45	40.63	0.86	0.47	1.26
50	37.65	0.79	0.40	1.21
55	35.12	0.74	0.35	1.15
60	32.94	0.69	0.30	1.09
65	31.04	0.65	0.26	1.03
70	29.37	0.62	0.23	0.96
75	27.89	0.59	0.20	0.89
80	26.56	0.56	0.17	0.81
85	25.37	0.53	0.14	0.73
90	24.29	0.51	0.12	0.65

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.20	0.10
44	1.60	0.11
52	2.07	0.12
61	2.63	0.13
70	3.29	0.14
81	4.05	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.104 m
1.60	1.34	1.20	Q _{allow} =	0.39 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

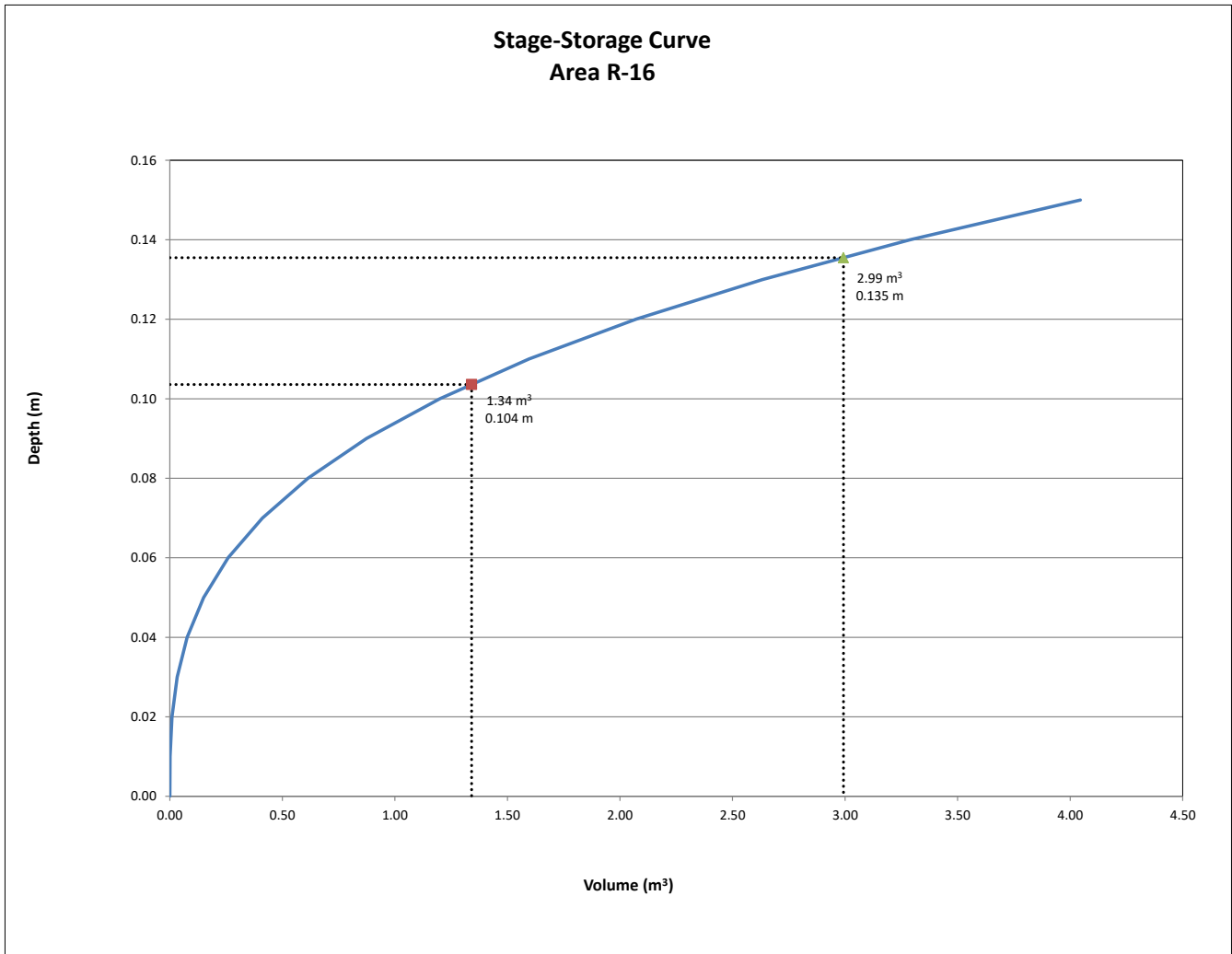
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-16 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0084	ha	Qallow =	0.51
C =	1.00		Vol(max) =	2.99
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	5.68	5.17	1.55
10	178.56	4.18	3.67	2.20
15	142.89	3.34	2.83	2.55
20	119.95	2.80	2.29	2.75
25	103.85	2.43	1.92	2.88
30	91.87	2.15	1.64	2.95
35	82.58	1.93	1.42	2.98
40	75.15	1.76	1.25	2.99
45	69.05	1.61	1.10	2.98
50	63.95	1.50	0.99	2.96
55	59.62	1.39	0.88	2.92
60	55.89	1.31	0.80	2.87
65	52.65	1.23	0.72	2.81
70	49.79	1.16	0.65	2.75
75	47.26	1.10	0.59	2.68
80	44.99	1.05	0.54	2.60
85	42.95	1.00	0.49	2.52
90	41.11	0.96	0.45	2.44

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.20	0.10
44	1.60	0.11
52	2.07	0.12
61	2.63	0.13
70	3.29	0.14
81	4.05	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.135 m
3.29	2.99	2.63	Q _{allow} =	0.51 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-17 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0080	ha	Qallow =	0.42
C =	0.90		Vol(max) =	1.19
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.82	2.40	0.72
10	104.19	2.08	1.66	1.00
15	83.56	1.67	1.25	1.12
20	70.25	1.40	0.98	1.18
25	60.90	1.22	0.80	1.19
30	53.93	1.08	0.66	1.18
35	48.52	0.97	0.55	1.15
40	44.18	0.88	0.46	1.11
45	40.63	0.81	0.39	1.06
50	37.65	0.75	0.33	1.00
55	35.12	0.70	0.28	0.93
60	32.94	0.66	0.24	0.86
65	31.04	0.62	0.20	0.78
70	29.37	0.59	0.17	0.70
75	27.89	0.56	0.14	0.62
80	26.56	0.53	0.11	0.53
85	25.37	0.51	0.09	0.44
90	24.29	0.49	0.07	0.35

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.18	0.06
12	0.29	0.07
16	0.43	0.08
20	0.61	0.09
25	0.83	0.10
30	1.11	0.11
36	1.44	0.12
42	1.83	0.13
49	2.28	0.14
56	2.80	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.113 m
1.44	1.19	1.11	Qallow =	0.42 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

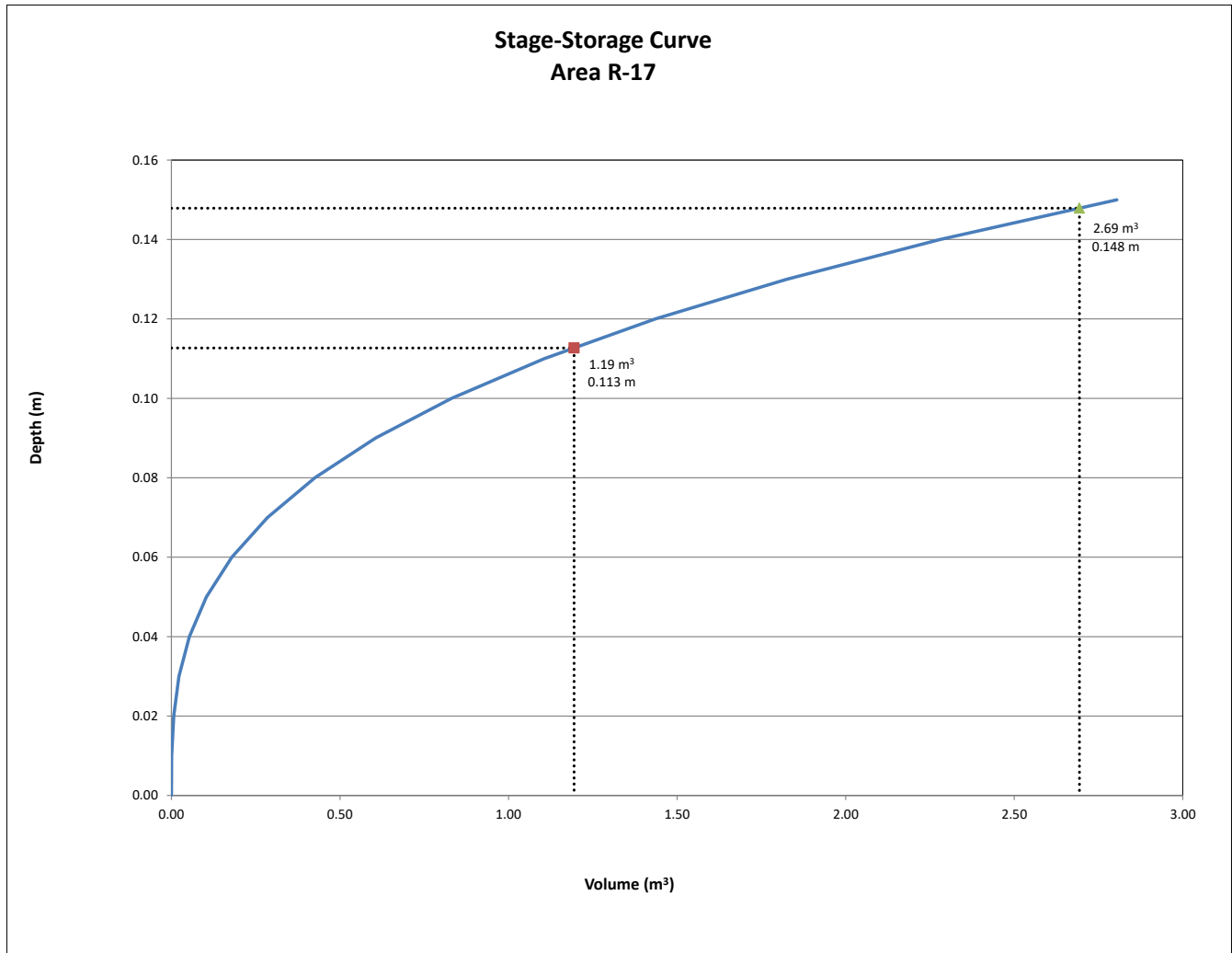
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-17 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0080	ha	Qallow =	0.55
C =	1.00		Vol(max) =	2.69
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	5.39	4.84	1.45
10	178.56	3.96	3.41	2.05
15	142.89	3.17	2.62	2.36
20	119.95	2.66	2.11	2.53
25	103.85	2.30	1.75	2.63
30	91.87	2.04	1.49	2.68
35	82.58	1.83	1.28	2.69
40	75.15	1.67	1.12	2.68
45	69.05	1.53	0.98	2.65
50	63.95	1.42	0.87	2.61
55	59.62	1.32	0.77	2.55
60	55.89	1.24	0.69	2.49
65	52.65	1.17	0.62	2.41
70	49.79	1.10	0.55	2.33
75	47.26	1.05	0.50	2.24
80	44.99	1.00	0.45	2.15
85	42.95	0.95	0.40	2.06
90	41.11	0.91	0.36	1.96

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.18	0.06
12	0.29	0.07
16	0.43	0.08
20	0.61	0.09
25	0.83	0.10
30	1.11	0.11
36	1.44	0.12
42	1.83	0.13
49	2.28	0.14
56	2.80	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.148 m
2.80	2.69	2.28	Qallow =	0.55 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-18 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0037	ha	Qallow =	0.39
C =	0.90		Vol(max) =	0.34
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.31	0.92	0.27
10	104.19	0.96	0.57	0.34
15	83.56	0.77	0.38	0.34
20	70.25	0.65	0.26	0.31
25	60.90	0.56	0.17	0.26
30	53.93	0.50	0.11	0.20
35	48.52	0.45	0.06	0.12
40	44.18	0.41	0.02	0.04
45	40.63	0.38	-0.01	-0.04
50	37.65	0.35	-0.04	-0.13
55	35.12	0.32	-0.07	-0.22
60	32.94	0.30	-0.09	-0.31
65	31.04	0.29	-0.10	-0.40
70	29.37	0.27	-0.12	-0.50
75	27.89	0.26	-0.13	-0.59
80	26.56	0.25	-0.14	-0.69
85	25.37	0.23	-0.16	-0.79
90	24.29	0.22	-0.17	-0.89

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
0	0.00	0.02
1	0.01	0.03
1	0.02	0.04
2	0.04	0.05
3	0.07	0.06
5	0.11	0.07
6	0.16	0.08
8	0.23	0.09
9	0.31	0.10
11	0.41	0.11
13	0.53	0.12
16	0.68	0.13
18	0.85	0.14
21	1.04	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.103 m
0.41	0.34	0.31	Qallow =	0.39 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

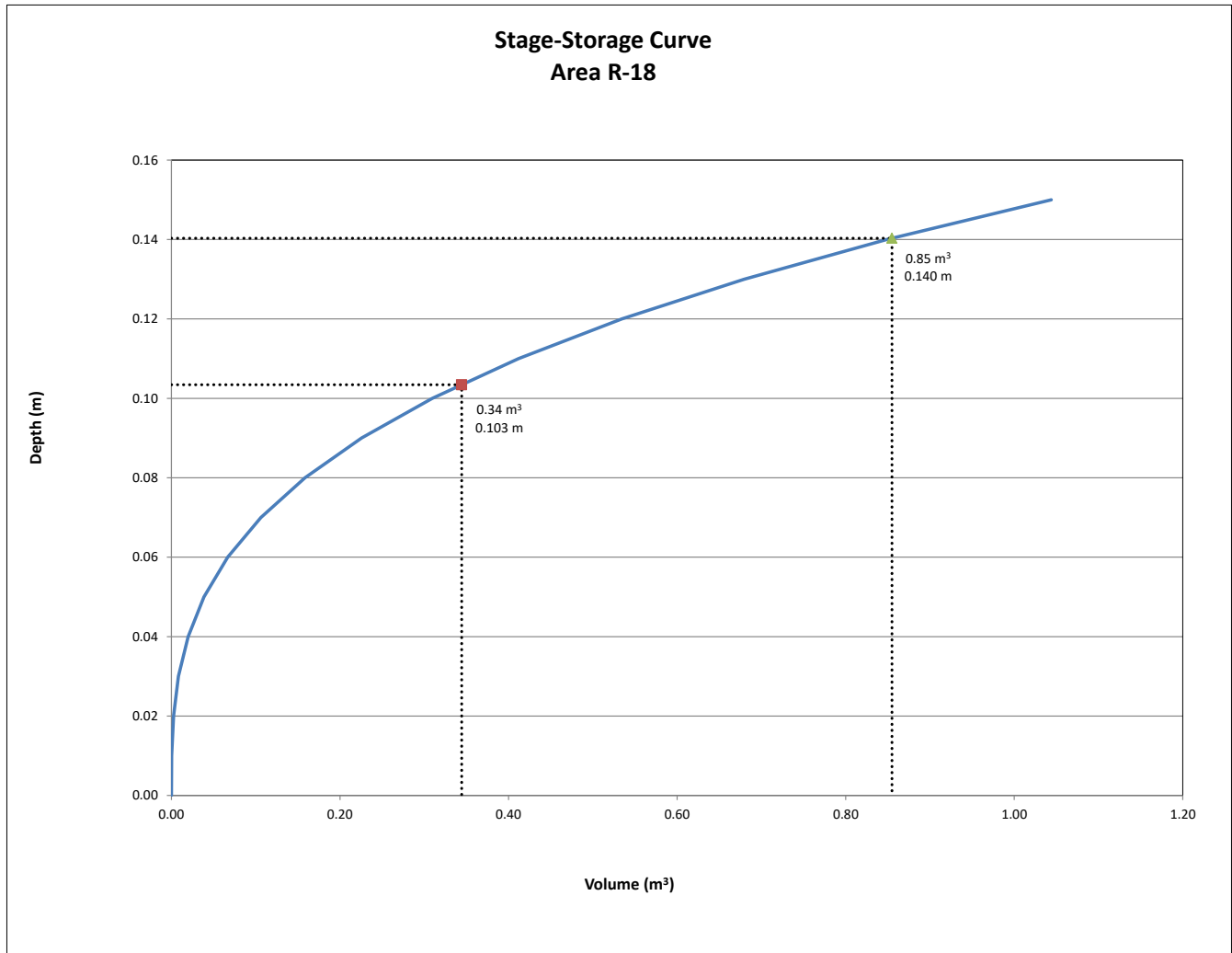
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-18 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0037	ha	Qallow =	0.52
C =	1.00		Vol(max) =	0.85
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.49	1.97	0.59
10	178.56	1.83	1.31	0.79
15	142.89	1.47	0.95	0.85
20	119.95	1.23	0.71	0.85
25	103.85	1.07	0.55	0.82
30	91.87	0.94	0.42	0.76
35	82.58	0.85	0.33	0.69
40	75.15	0.77	0.25	0.61
45	69.05	0.71	0.19	0.51
50	63.95	0.66	0.14	0.41
55	59.62	0.61	0.09	0.31
60	55.89	0.57	0.05	0.20
65	52.65	0.54	0.02	0.08
70	49.79	0.51	-0.01	-0.04
75	47.26	0.49	-0.03	-0.16
80	44.99	0.46	-0.06	-0.28
85	42.95	0.44	-0.08	-0.40
90	41.11	0.42	-0.10	-0.53

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
0	0.00	0.02
1	0.01	0.03
1	0.02	0.04
2	0.04	0.05
3	0.07	0.06
5	0.11	0.07
6	0.16	0.08
8	0.23	0.09
9	0.31	0.10
11	0.41	0.11
13	0.53	0.12
16	0.68	0.13
18	0.85	0.14
21	1.04	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.140 m
1.04	0.85	0.85	Qallow =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-19 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0035	ha	Qallow =	0.35
C =	0.90		Vol(max) =	0.35
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.25	0.90	0.27
10	104.19	0.92	0.57	0.34
15	83.56	0.74	0.39	0.35
20	70.25	0.62	0.27	0.33
25	60.90	0.54	0.19	0.28
30	53.93	0.48	0.13	0.23
35	48.52	0.43	0.08	0.17
40	44.18	0.39	0.04	0.10
45	40.63	0.36	0.01	0.02
50	37.65	0.33	-0.02	-0.05
55	35.12	0.31	-0.04	-0.13
60	32.94	0.29	-0.06	-0.21
65	31.04	0.27	-0.08	-0.29
70	29.37	0.26	-0.09	-0.38
75	27.89	0.25	-0.10	-0.47
80	26.56	0.23	-0.12	-0.55
85	25.37	0.22	-0.13	-0.64
90	24.29	0.21	-0.14	-0.73

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
2	0.03	0.04
3	0.05	0.05
5	0.09	0.06
6	0.14	0.07
8	0.22	0.08
10	0.31	0.09
13	0.42	0.10
15	0.56	0.11
18	0.73	0.12
21	0.93	0.13
25	1.16	0.14
28	1.42	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.094 m
0.42	0.35	0.31	Q _{allow} =	0.35 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

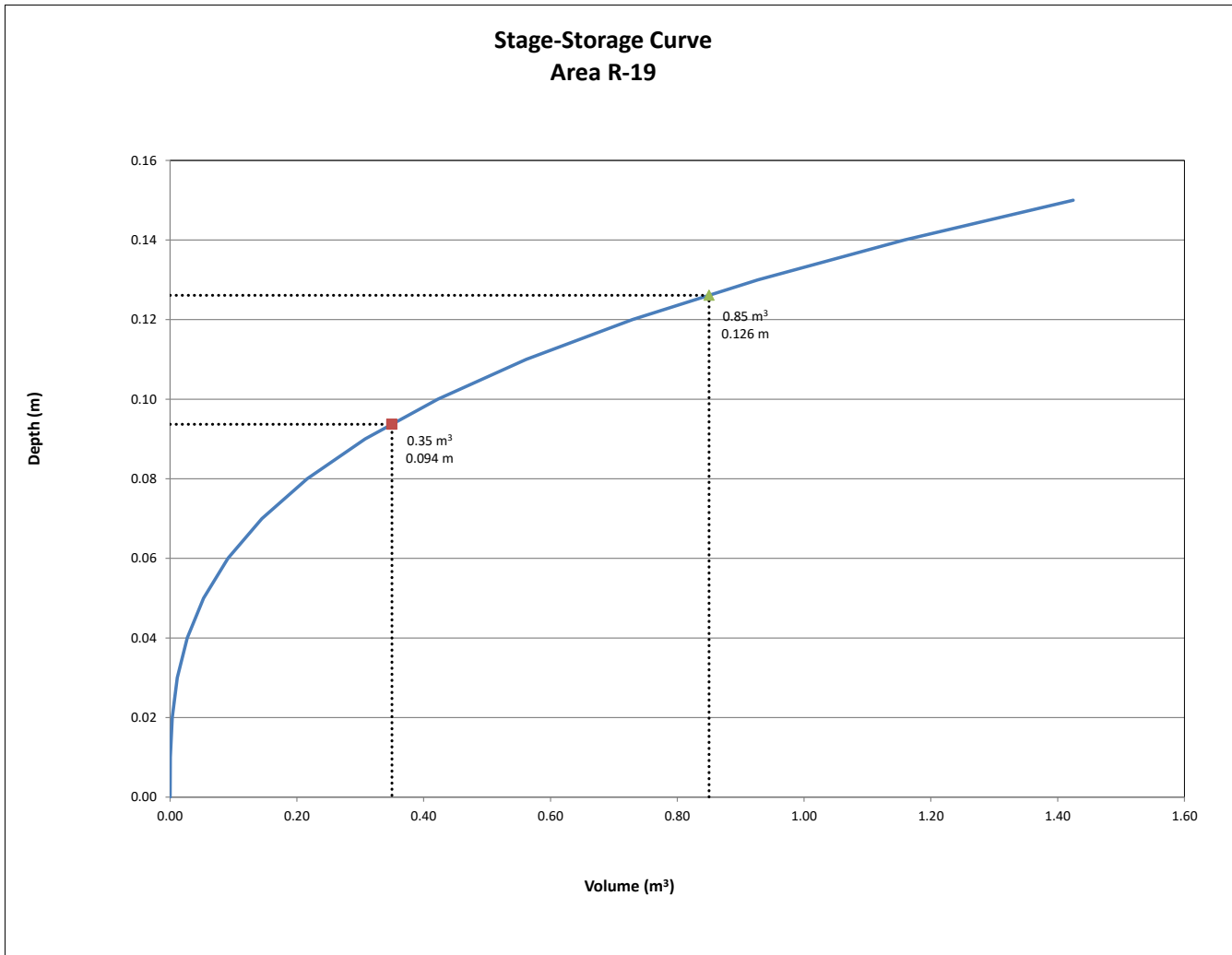
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-19 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0035	ha	Qallow =	0.47
C =	1.00		Vol(max) =	0.85
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.38	1.91	0.57
10	178.56	1.75	1.28	0.77
15	142.89	1.40	0.93	0.84
20	119.95	1.18	0.71	0.85
25	103.85	1.02	0.55	0.83
30	91.87	0.90	0.43	0.78
35	82.58	0.81	0.34	0.72
40	75.15	0.74	0.27	0.64
45	69.05	0.68	0.21	0.56
50	63.95	0.63	0.16	0.47
55	59.62	0.59	0.12	0.38
60	55.89	0.55	0.08	0.28
65	52.65	0.52	0.05	0.18
70	49.79	0.49	0.02	0.08
75	47.26	0.46	-0.01	-0.03
80	44.99	0.44	-0.03	-0.13
85	42.95	0.42	-0.05	-0.24
90	41.11	0.40	-0.07	-0.36

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
2	0.03	0.04
3	0.05	0.05
5	0.09	0.06
6	0.14	0.07
8	0.22	0.08
10	0.31	0.09
13	0.42	0.10
15	0.56	0.11
18	0.73	0.12
21	0.93	0.13
25	1.16	0.14
28	1.42	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.126 m
0.93	0.85	0.73	Q _{allow} =	0.47 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-20 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0054	ha	Qallow =	0.71
C =	0.90		Vol(max) =	0.41
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.90	1.19	0.36
10	104.19	1.40	0.69	0.41
15	83.56	1.12	0.41	0.37
20	70.25	0.94	0.23	0.28
25	60.90	0.82	0.11	0.16
30	53.93	0.72	0.01	0.03
35	48.52	0.65	-0.06	-0.12
40	44.18	0.59	-0.12	-0.28
45	40.63	0.55	-0.16	-0.44
50	37.65	0.51	-0.20	-0.61
55	35.12	0.47	-0.24	-0.78
60	32.94	0.44	-0.27	-0.96
65	31.04	0.42	-0.29	-1.14
70	29.37	0.39	-0.32	-1.32
75	27.89	0.37	-0.34	-1.51
80	26.56	0.36	-0.35	-1.69
85	25.37	0.34	-0.37	-1.88
90	24.29	0.33	-0.38	-2.07

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
2	0.03	0.04
4	0.06	0.05
5	0.10	0.06
7	0.16	0.07
9	0.24	0.08
11	0.34	0.09
14	0.47	0.10
17	0.63	0.11
20	0.81	0.12
24	1.04	0.13
28	1.29	0.14
32	1.59	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.096 m
0.47	0.41	0.34	Qallow =	0.71 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

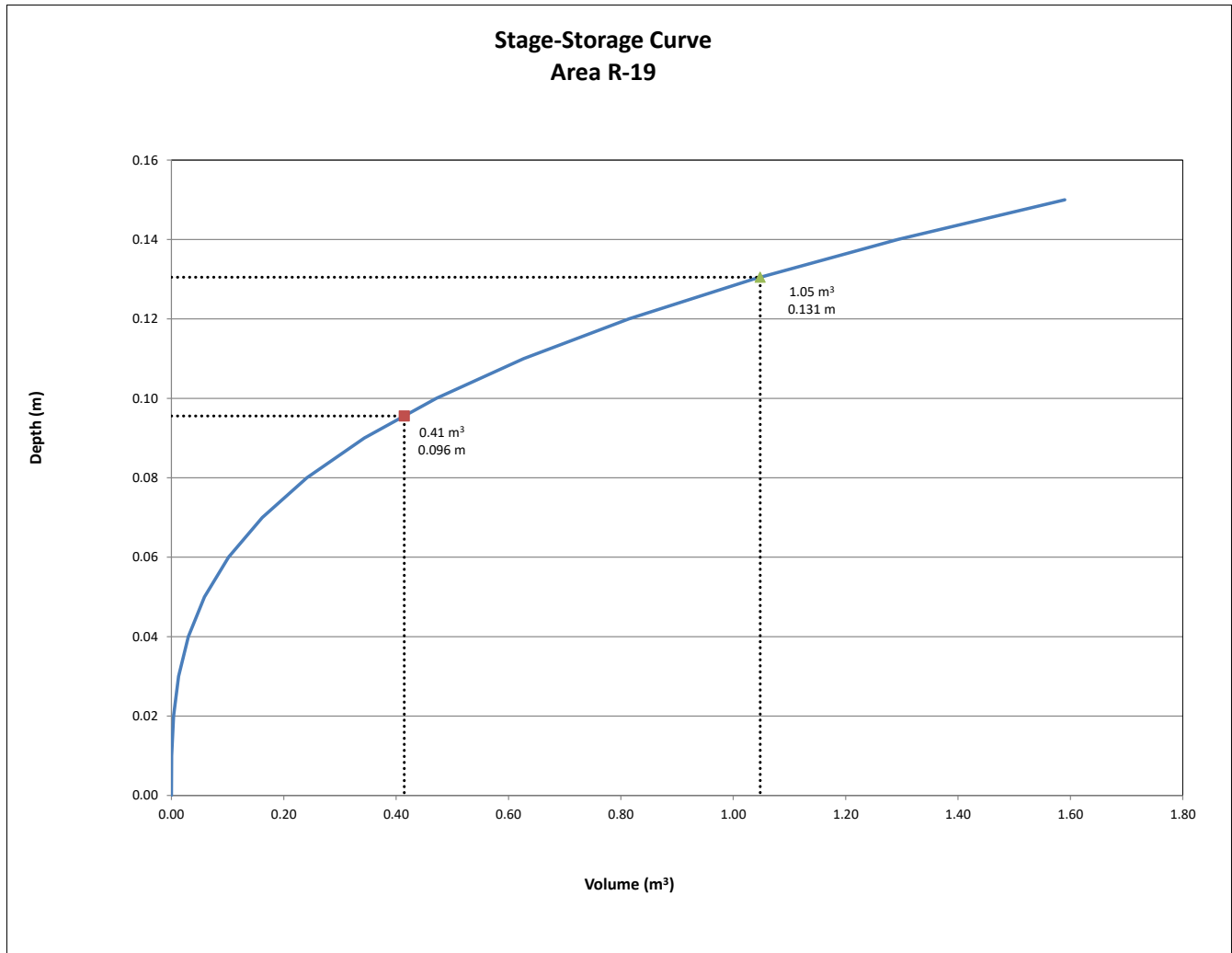
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-20 : BUILDING ROOF			
OTTAWA IDF CURVE				
Area =	0.0054	ha	Qallow =	0.97
C =	1.00		Vol(max) =	1.05
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.63	2.66	0.80
10	178.56	2.67	1.70	1.02
15	142.89	2.13	1.16	1.05
20	119.95	1.79	0.82	0.99
25	103.85	1.55	0.58	0.87
30	91.87	1.37	0.40	0.72
35	82.58	1.23	0.26	0.55
40	75.15	1.12	0.15	0.37
45	69.05	1.03	0.06	0.17
50	63.95	0.96	-0.01	-0.04
55	59.62	0.89	-0.08	-0.26
60	55.89	0.83	-0.14	-0.49
65	52.65	0.79	-0.18	-0.72
70	49.79	0.74	-0.23	-0.95
75	47.26	0.71	-0.26	-1.19
80	44.99	0.67	-0.30	-1.43
85	42.95	0.64	-0.33	-1.67
90	41.11	0.61	-0.36	-1.92

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
2	0.03	0.04
4	0.06	0.05
5	0.10	0.06
7	0.16	0.07
9	0.24	0.08
11	0.34	0.09
14	0.47	0.10
17	0.63	0.11
20	0.81	0.12
24	1.04	0.13
28	1.29	0.14
32	1.59	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.131 m
1.29	1.05	1.04	Qallow =	0.97 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA	R-21	: BUILDING ROOF		
OTTAWA IDF CURVE				
Area =	0.0099	ha	Qallow =	0.79
C =	0.90		Vol(max) =	1.16
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.51	2.72	0.82
10	104.19	2.59	1.80	1.08
15	83.56	2.08	1.29	1.16
20	70.25	1.75	0.96	1.15
25	60.90	1.51	0.72	1.08
30	53.93	1.34	0.55	0.99
35	48.52	1.21	0.42	0.87
40	44.18	1.10	0.31	0.74
45	40.63	1.01	0.22	0.59
50	37.65	0.94	0.15	0.44
55	35.12	0.87	0.08	0.27
60	32.94	0.82	0.03	0.10
65	31.04	0.77	-0.02	-0.07
70	29.37	0.73	-0.06	-0.25
75	27.89	0.69	-0.10	-0.44
80	26.56	0.66	-0.13	-0.62
85	25.37	0.63	-0.16	-0.82
90	24.29	0.60	-0.19	-1.01

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
7	0.12	0.05
10	0.21	0.06
14	0.33	0.07
19	0.50	0.08
24	0.71	0.09
29	0.97	0.10
35	1.29	0.11
42	1.67	0.12
49	2.13	0.13
57	2.66	0.14
65	3.27	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.106 m
1.29	1.16	0.97	Q _{allow} =	0.79 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

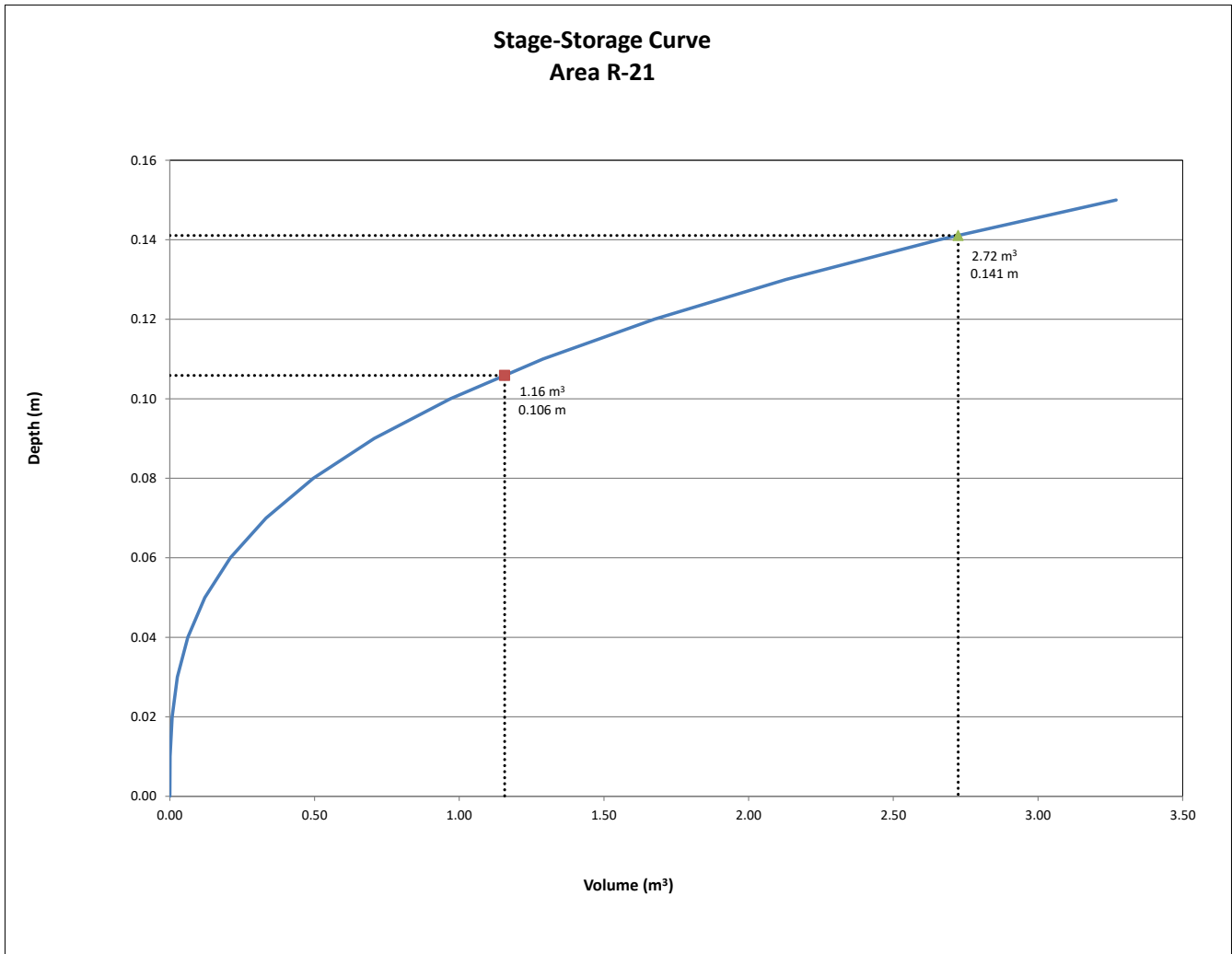
REQUIRED STORAGE - 100-YEAR EVENT				
AREA	R-21	: BUILDING ROOF		
OTTAWA IDF CURVE				
Area =	0.0099	ha	Qallow =	1.05
C =	1.00		Vol(max) =	2.72
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.70	5.65	1.69
10	178.56	4.93	3.88	2.33
15	142.89	3.94	2.89	2.60
20	119.95	3.31	2.26	2.71
25	103.85	2.87	1.82	2.72
30	91.87	2.54	1.49	2.67
35	82.58	2.28	1.23	2.58
40	75.15	2.07	1.02	2.46
45	69.05	1.91	0.86	2.31
50	63.95	1.77	0.72	2.15
55	59.62	1.65	0.60	1.97
60	55.89	1.54	0.49	1.77
65	52.65	1.45	0.40	1.57
70	49.79	1.37	0.32	1.36
75	47.26	1.30	0.25	1.14
80	44.99	1.24	0.19	0.92
85	42.95	1.19	0.14	0.69
90	41.11	1.13	0.08	0.46

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
7	0.12	0.05
10	0.21	0.06
14	0.33	0.07
19	0.50	0.08
24	0.71	0.09
29	0.97	0.10
35	1.29	0.11
42	1.67	0.12
49	2.13	0.13
57	2.66	0.14
65	3.27	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.141 m
3.27	2.72	2.66	Q _{allow} =	1.05 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-22 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0099	ha	Qallow =	0.78
C =	0.90		Vol(max) =	1.16
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.50	2.72	0.82
10	104.19	2.58	1.80	1.08
15	83.56	2.07	1.29	1.16
20	70.25	1.74	0.96	1.15
25	60.90	1.51	0.73	1.09
30	53.93	1.34	0.56	1.00
35	48.52	1.20	0.42	0.89
40	44.18	1.09	0.31	0.75
45	40.63	1.01	0.23	0.61
50	37.65	0.93	0.15	0.46
55	35.12	0.87	0.09	0.30
60	32.94	0.82	0.04	0.13
65	31.04	0.77	-0.01	-0.04
70	29.37	0.73	-0.05	-0.22
75	27.89	0.69	-0.09	-0.40
80	26.56	0.66	-0.12	-0.59
85	25.37	0.63	-0.15	-0.77
90	24.29	0.60	-0.18	-0.96

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.13	0.05
11	0.22	0.06
15	0.35	0.07
20	0.52	0.08
25	0.75	0.09
31	1.02	0.10
37	1.36	0.11
44	1.77	0.12
52	2.25	0.13
60	2.81	0.14
69	3.45	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.104 m
1.36	1.16	1.02	Q _{allow} =	0.78 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

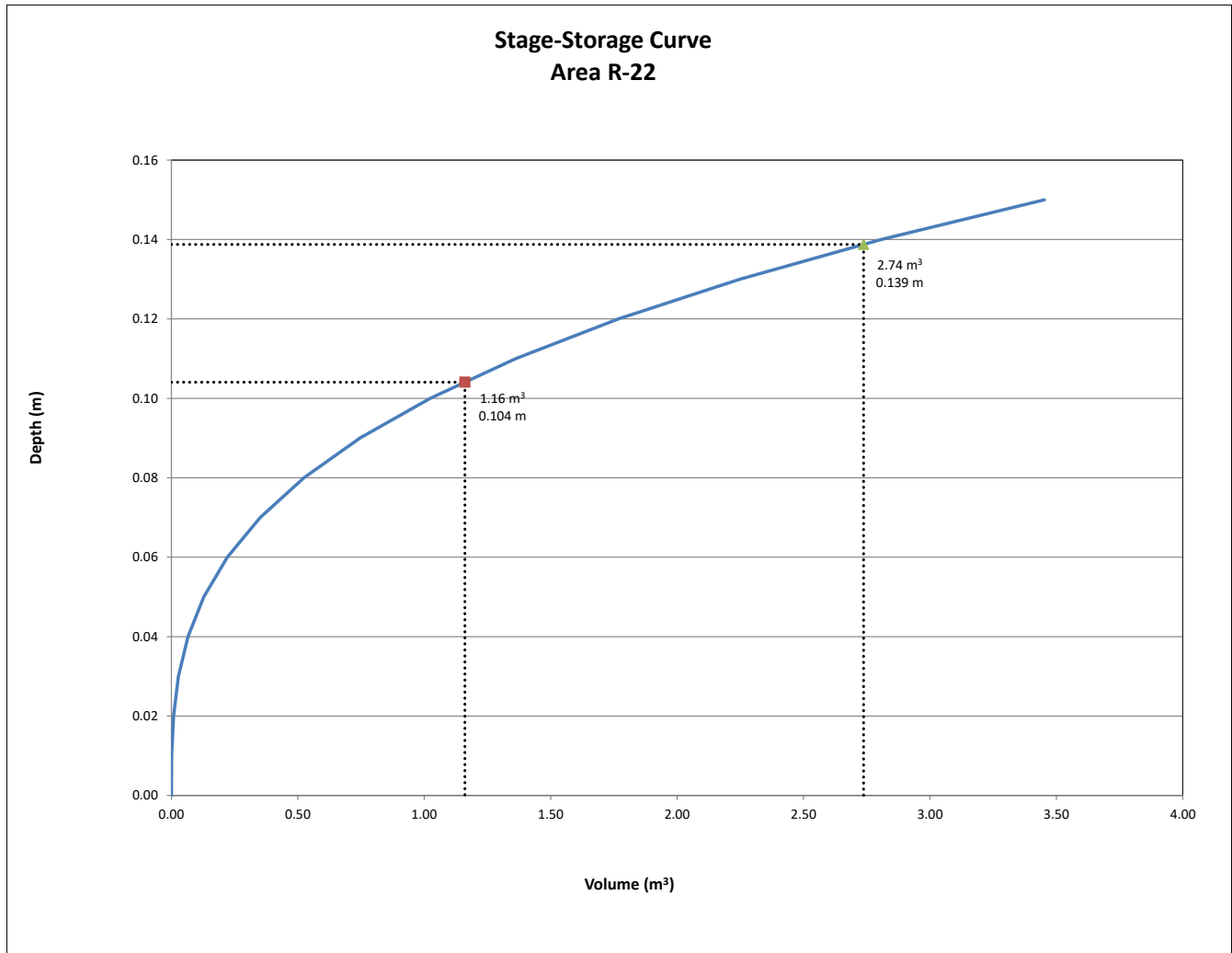
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-22 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0099	ha	Qallow =	1.03
C =	1.00		Vol(max) =	2.74
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.68	5.65	1.69
10	178.56	4.91	3.88	2.33
15	142.89	3.93	2.90	2.61
20	119.95	3.30	2.27	2.72
25	103.85	2.86	1.83	2.74
30	91.87	2.53	1.50	2.69
35	82.58	2.27	1.24	2.60
40	75.15	2.07	1.04	2.48
45	69.05	1.90	0.87	2.34
50	63.95	1.76	0.73	2.18
55	59.62	1.64	0.61	2.01
60	55.89	1.54	0.51	1.82
65	52.65	1.45	0.42	1.62
70	49.79	1.37	0.34	1.42
75	47.26	1.30	0.27	1.20
80	44.99	1.24	0.21	0.99
85	42.95	1.18	0.15	0.76
90	41.11	1.13	0.10	0.53

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.13	0.05
11	0.22	0.06
15	0.35	0.07
20	0.52	0.08
25	0.75	0.09
31	1.02	0.10
37	1.36	0.11
44	1.77	0.12
52	2.25	0.13
60	2.81	0.14
69	3.45	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.139 m
2.81	2.74	2.25	Q _{allow} =	1.03 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-23 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0087	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.40
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.08	2.68	0.80
10	104.19	2.27	1.87	1.12
15	83.56	1.82	1.42	1.28
20	70.25	1.53	1.13	1.36
25	60.90	1.33	0.93	1.39
30	53.93	1.18	0.78	1.40
35	48.52	1.06	0.66	1.38
40	44.18	0.96	0.56	1.35
45	40.63	0.89	0.49	1.31
50	37.65	0.82	0.42	1.27
55	35.12	0.77	0.37	1.21
60	32.94	0.72	0.32	1.15
65	31.04	0.68	0.28	1.08
70	29.37	0.64	0.24	1.01
75	27.89	0.61	0.21	0.94
80	26.56	0.58	0.18	0.86
85	25.37	0.55	0.15	0.78
90	24.29	0.53	0.13	0.70

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.14	0.05
12	0.24	0.06
16	0.38	0.07
21	0.57	0.08
27	0.81	0.09
33	1.11	0.10
40	1.48	0.11
48	1.92	0.12
56	2.44	0.13
65	3.05	0.14
75	3.75	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.108 m
1.48	1.40	1.11	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

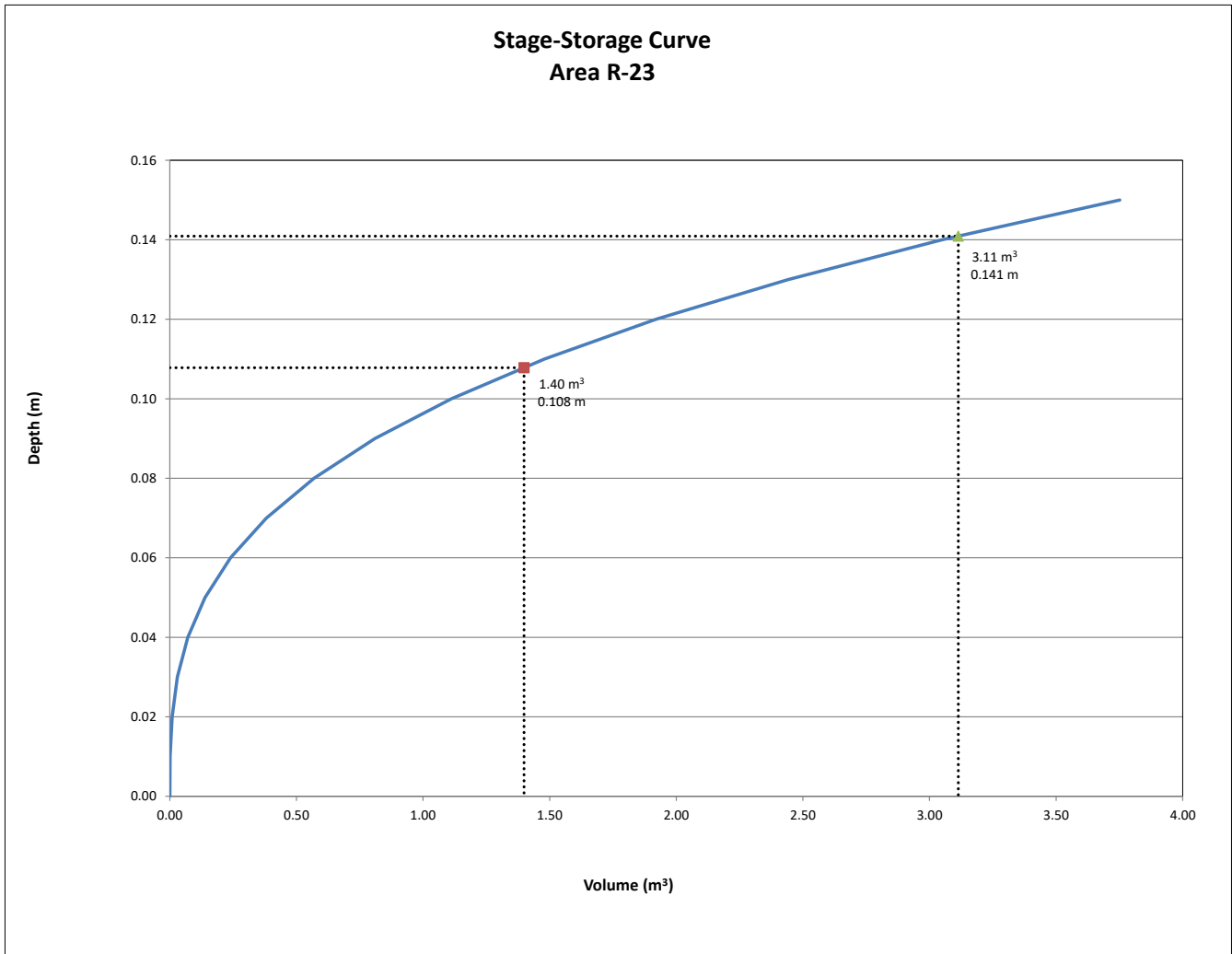
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-23 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0087	ha	Qallow =	0.53
C =	1.00		Vol(max) =	3.11
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	5.89	5.36	1.61
10	178.56	4.33	3.81	2.28
15	142.89	3.47	2.94	2.65
20	119.95	2.91	2.38	2.86
25	103.85	2.52	1.99	2.99
30	91.87	2.23	1.70	3.07
35	82.58	2.00	1.48	3.10
40	75.15	1.82	1.30	3.11
45	69.05	1.67	1.15	3.10
50	63.95	1.55	1.03	3.08
55	59.62	1.45	0.92	3.04
60	55.89	1.36	0.83	2.99
65	52.65	1.28	0.75	2.93
70	49.79	1.21	0.68	2.87
75	47.26	1.15	0.62	2.79
80	44.99	1.09	0.57	2.72
85	42.95	1.04	0.52	2.64
90	41.11	1.00	0.47	2.55

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.14	0.05
12	0.24	0.06
16	0.38	0.07
21	0.57	0.08
27	0.81	0.09
33	1.11	0.10
40	1.48	0.11
48	1.92	0.12
56	2.44	0.13
65	3.05	0.14
75	3.75	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.141 m
3.75	3.11	3.05	Qallow =	0.53 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-24 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0074	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.09
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.61	2.21	0.66
10	104.19	1.93	1.53	0.92
15	83.56	1.54	1.14	1.03
20	70.25	1.30	0.90	1.08
25	60.90	1.13	0.73	1.09
30	53.93	1.00	0.60	1.07
35	48.52	0.90	0.50	1.04
40	44.18	0.82	0.42	1.00
45	40.63	0.75	0.35	0.95
50	37.65	0.70	0.30	0.89
55	35.12	0.65	0.25	0.82
60	32.94	0.61	0.21	0.75
65	31.04	0.57	0.17	0.68
70	29.37	0.54	0.14	0.60
75	27.89	0.52	0.12	0.52
80	26.56	0.49	0.09	0.44
85	25.37	0.47	0.07	0.35
90	24.29	0.45	0.05	0.27

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.65	0.09
27	0.89	0.10
32	1.19	0.11
39	1.54	0.12
45	1.96	0.13
52	2.45	0.14
60	3.01	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.107 m
1.19	1.09	0.89	Q _{allow} =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

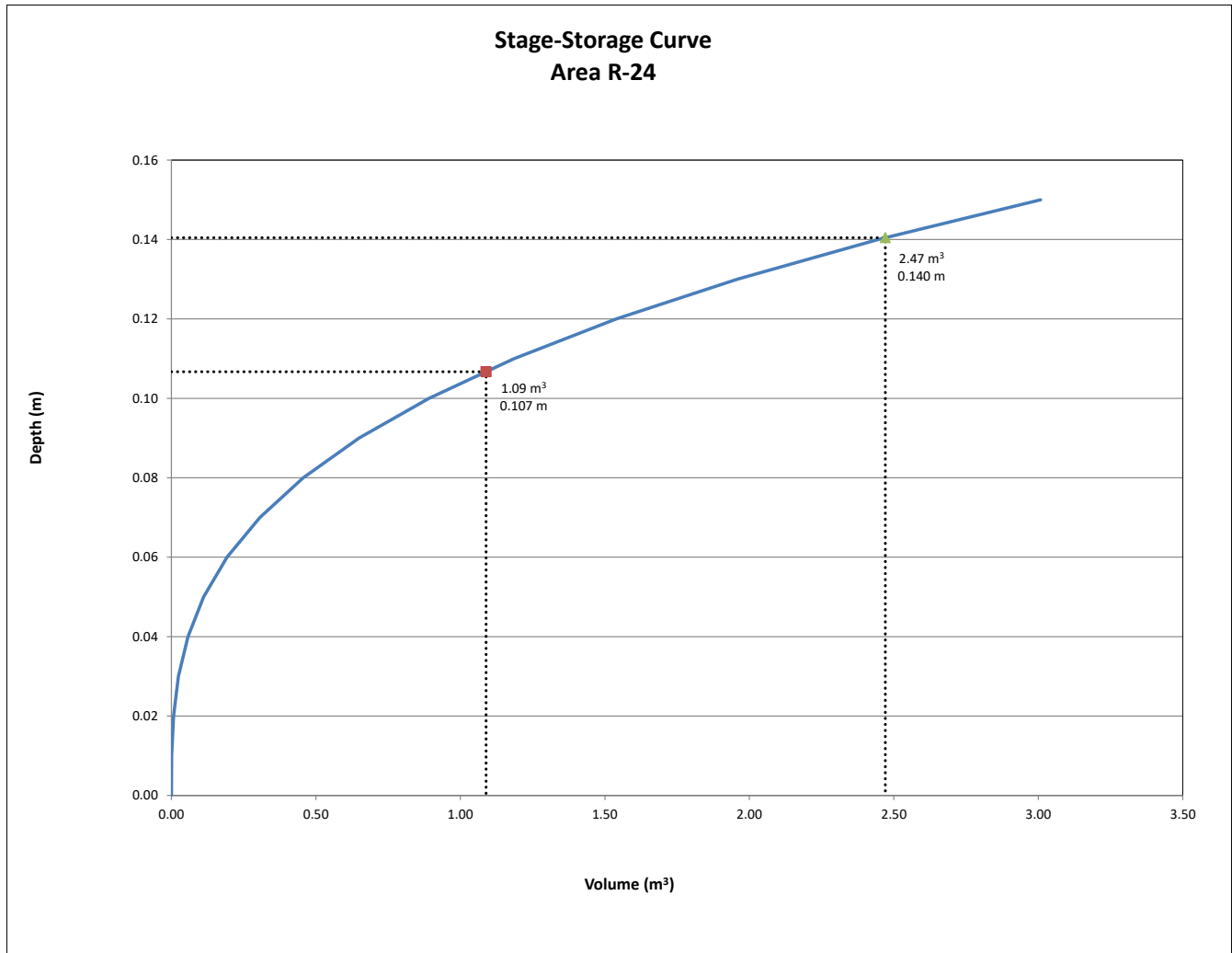
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-24 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0074	ha	Qallow =	0.52
C =	1.00		Vol(max) =	2.47
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.99	4.47	1.34
10	178.56	3.67	3.15	1.89
15	142.89	2.94	2.42	2.17
20	119.95	2.46	1.94	2.33
25	103.85	2.13	1.61	2.42
30	91.87	1.89	1.37	2.46
35	82.58	1.70	1.18	2.47
40	75.15	1.54	1.02	2.46
45	69.05	1.42	0.90	2.43
50	63.95	1.31	0.79	2.38
55	59.62	1.22	0.70	2.33
60	55.89	1.15	0.63	2.26
65	52.65	1.08	0.56	2.19
70	49.79	1.02	0.50	2.11
75	47.26	0.97	0.45	2.03
80	44.99	0.92	0.40	1.94
85	42.95	0.88	0.36	1.85
90	41.11	0.84	0.32	1.75

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.65	0.09
27	0.89	0.10
32	1.19	0.11
39	1.54	0.12
45	1.96	0.13
52	2.45	0.14
60	3.01	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.140 m
3.01	2.47	2.45	Q _{allow} =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-25 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0064	ha	Qallow =	0.37
C =	0.90		Vol(max) =	0.91
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.26	1.89	0.57
10	104.19	1.67	1.30	0.78
15	83.56	1.34	0.97	0.87
20	70.25	1.12	0.75	0.90
25	60.90	0.97	0.60	0.91
30	53.93	0.86	0.49	0.89
35	48.52	0.78	0.41	0.85
40	44.18	0.71	0.34	0.81
45	40.63	0.65	0.28	0.76
50	37.65	0.60	0.23	0.70
55	35.12	0.56	0.19	0.63
60	32.94	0.53	0.16	0.56
65	31.04	0.50	0.13	0.49
70	29.37	0.47	0.10	0.42
75	27.89	0.45	0.08	0.34
80	26.56	0.42	0.05	0.26
85	25.37	0.41	0.04	0.18
90	24.29	0.39	0.02	0.10

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.65	0.09
27	0.89	0.10
32	1.19	0.11
39	1.54	0.12
45	1.96	0.13
52	2.45	0.14
60	3.01	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.100 m
1.19	0.91	0.89	Q _{allow} =	0.37 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

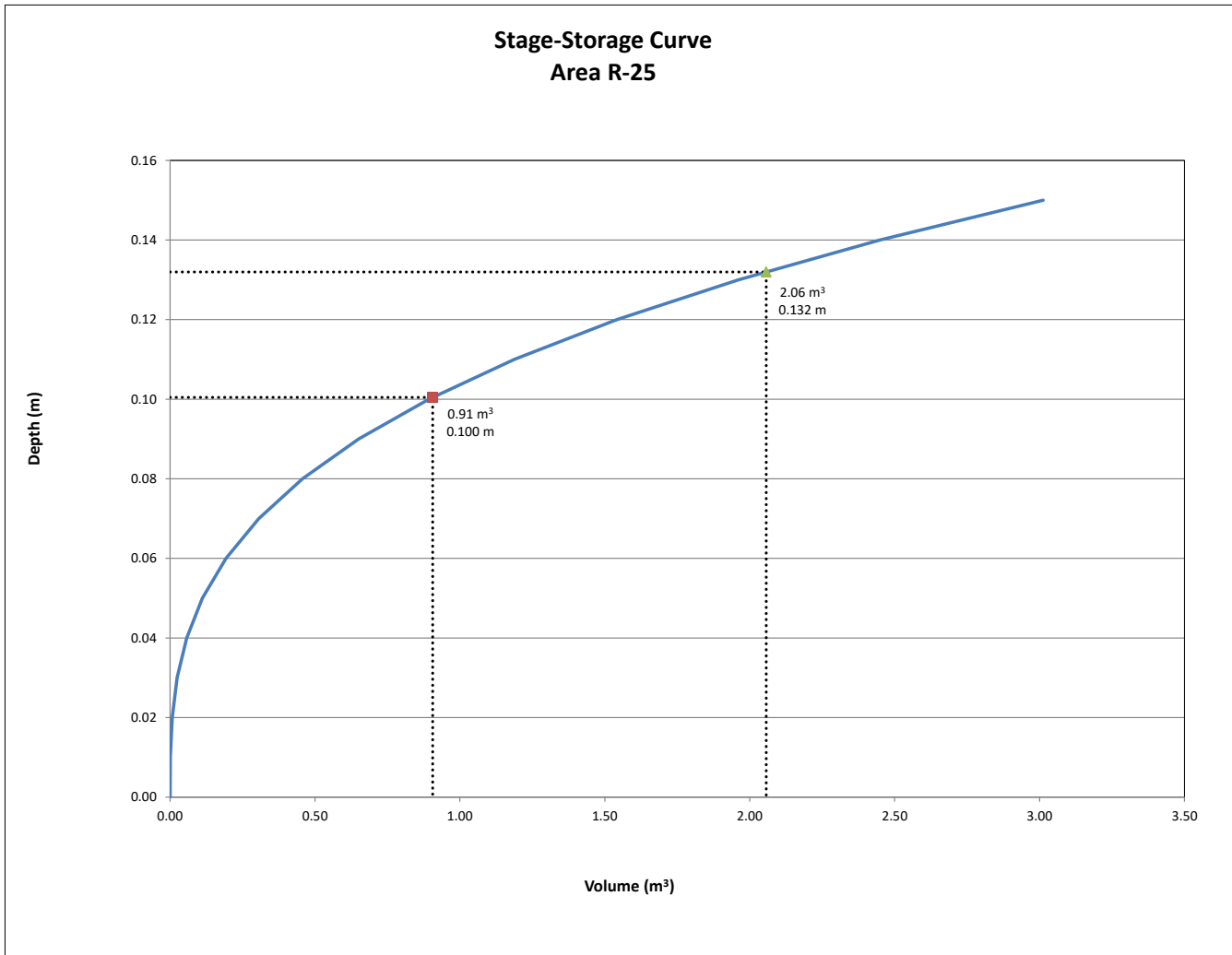
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-25 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0064	ha	Qallow =	0.49
C =	1.00		Vol(max) =	2.06
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.31	3.82	1.15
10	178.56	3.17	2.68	1.61
15	142.89	2.54	2.05	1.84
20	119.95	2.13	1.64	1.97
25	103.85	1.85	1.36	2.03
30	91.87	1.63	1.14	2.06
35	82.58	1.47	0.98	2.05
40	75.15	1.34	0.85	2.03
45	69.05	1.23	0.74	1.99
50	63.95	1.14	0.65	1.94
55	59.62	1.06	0.57	1.88
60	55.89	0.99	0.50	1.81
65	52.65	0.94	0.45	1.74
70	49.79	0.88	0.39	1.66
75	47.26	0.84	0.35	1.57
80	44.99	0.80	0.31	1.49
85	42.95	0.76	0.27	1.39
90	41.11	0.73	0.24	1.30

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.31	0.07
17	0.46	0.08
22	0.65	0.09
27	0.89	0.10
32	1.19	0.11
39	1.54	0.12
45	1.96	0.13
52	2.45	0.14
60	3.01	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.132 m
2.45	2.06	1.96	Q _{allow} =	0.49 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-26 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0097	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.63
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.42	3.02	0.91
10	104.19	2.53	2.13	1.28
15	83.56	2.03	1.63	1.46
20	70.25	1.70	1.30	1.57
25	60.90	1.48	1.08	1.62
30	53.93	1.31	0.91	1.63
35	48.52	1.18	0.78	1.63
40	44.18	1.07	0.67	1.61
45	40.63	0.99	0.59	1.58
50	37.65	0.91	0.51	1.54
55	35.12	0.85	0.45	1.49
60	32.94	0.80	0.40	1.44
65	31.04	0.75	0.35	1.38
70	29.37	0.71	0.31	1.31
75	27.89	0.68	0.28	1.24
80	26.56	0.64	0.24	1.17
85	25.37	0.62	0.22	1.10
90	24.29	0.59	0.19	1.02

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
6	0.08	0.04
10	0.16	0.05
14	0.28	0.06
19	0.45	0.07
25	0.67	0.08
32	0.95	0.09
39	1.30	0.10
47	1.73	0.11
56	2.25	0.12
66	2.86	0.13
76	3.57	0.14
88	4.39	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.108 m
1.73	1.63	1.30	Q _{allow} =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

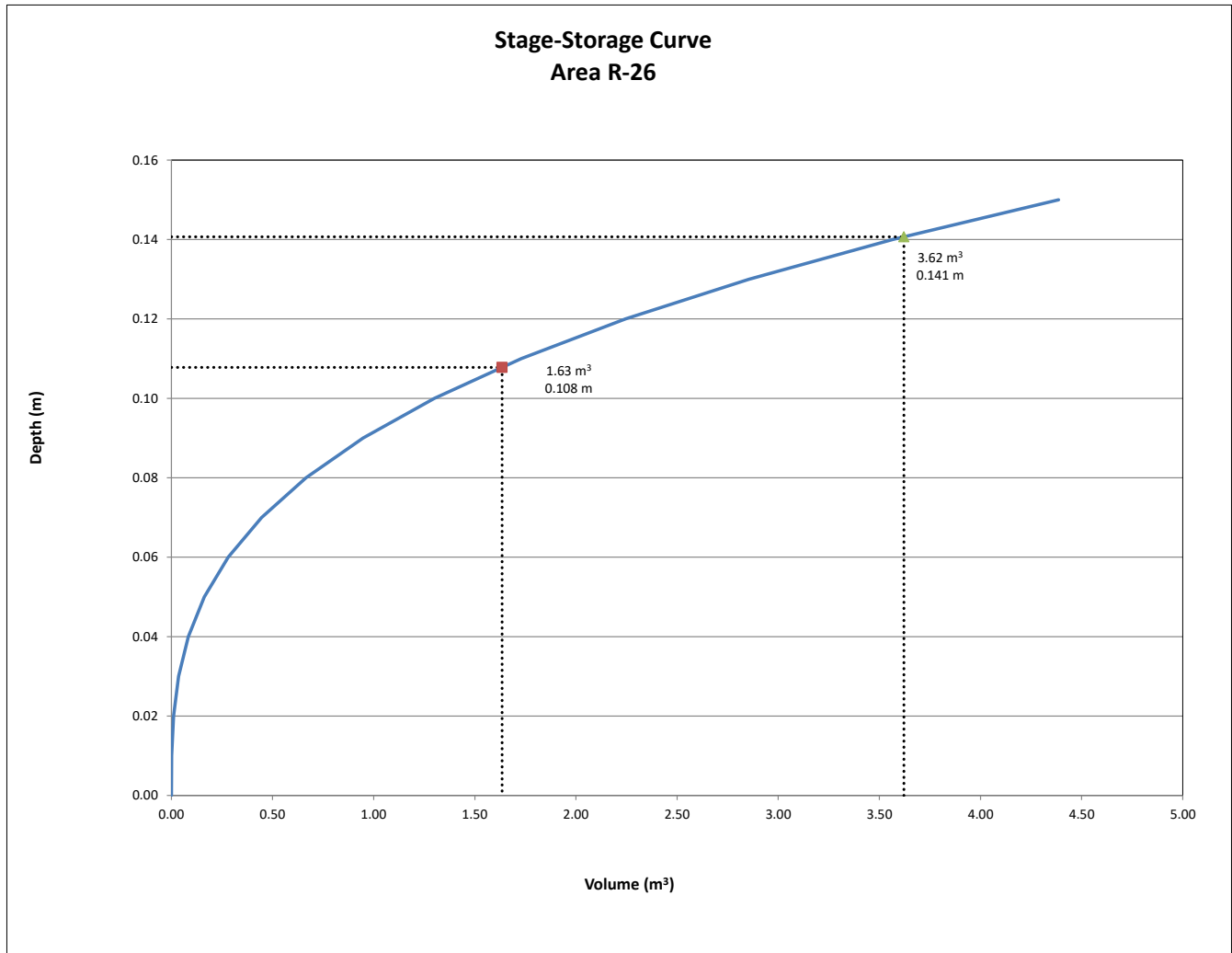
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-26 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0097	ha	Qallow =	0.52
C =	1.00		Vol(max) =	3.62
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.54	6.02	1.81
10	178.56	4.81	4.29	2.58
15	142.89	3.85	3.33	3.00
20	119.95	3.23	2.71	3.26
25	103.85	2.80	2.28	3.42
30	91.87	2.48	1.96	3.52
35	82.58	2.23	1.71	3.58
40	75.15	2.03	1.51	3.61
45	69.05	1.86	1.34	3.62
50	63.95	1.72	1.20	3.61
55	59.62	1.61	1.09	3.59
60	55.89	1.51	0.99	3.55
65	52.65	1.42	0.90	3.51
70	49.79	1.34	0.82	3.45
75	47.26	1.27	0.75	3.39
80	44.99	1.21	0.69	3.33
85	42.95	1.16	0.64	3.25
90	41.11	1.11	0.59	3.18

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
6	0.08	0.04
10	0.16	0.05
14	0.28	0.06
19	0.45	0.07
25	0.67	0.08
32	0.95	0.09
39	1.30	0.10
47	1.73	0.11
56	2.25	0.12
66	2.86	0.13
76	3.57	0.14
88	4.39	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.141 m
4.39	3.62	3.57	Q _{allow} =	0.52 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-27 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0092	ha	Qallow =	0.41
C =	0.90		Vol(max) =	1.50
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.26	2.85	0.85
10	104.19	2.41	2.00	1.20
15	83.56	1.93	1.52	1.37
20	70.25	1.62	1.21	1.45
25	60.90	1.41	1.00	1.49
30	53.93	1.25	0.84	1.50
35	48.52	1.12	0.71	1.49
40	44.18	1.02	0.61	1.46
45	40.63	0.94	0.53	1.43
50	37.65	0.87	0.46	1.38
55	35.12	0.81	0.40	1.32
60	32.94	0.76	0.35	1.26
65	31.04	0.72	0.31	1.20
70	29.37	0.68	0.27	1.13
75	27.89	0.64	0.23	1.05
80	26.56	0.61	0.20	0.98
85	25.37	0.59	0.18	0.90
90	24.29	0.56	0.15	0.81

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.14	0.05
12	0.24	0.06
16	0.38	0.07
21	0.56	0.08
27	0.80	0.09
33	1.10	0.10
40	1.47	0.11
48	1.91	0.12
56	2.42	0.13
65	3.03	0.14
74	3.72	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.111 m
1.91	1.50	1.47	Qallow =	0.41 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

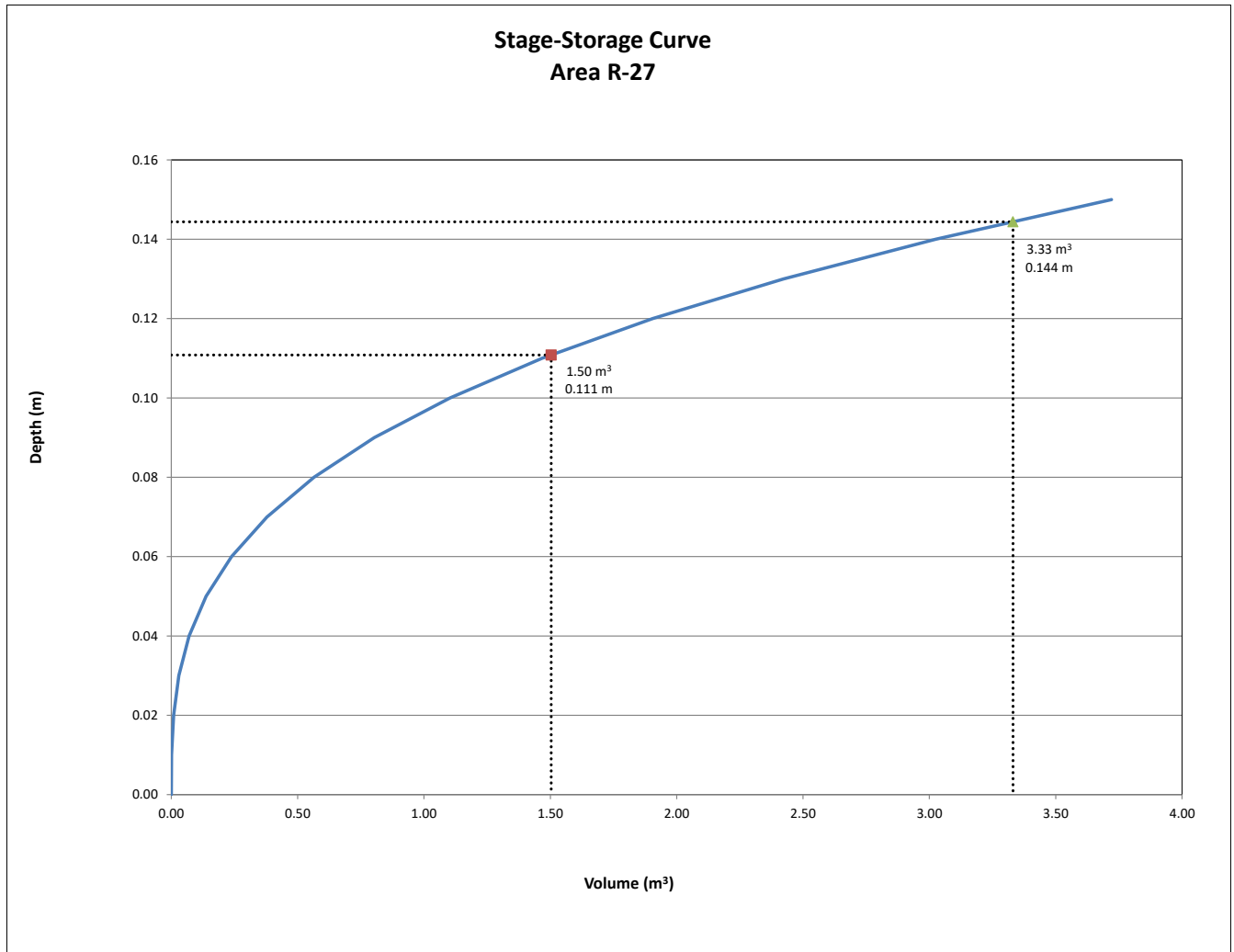
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-27 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0092	ha	Qallow =	0.54
C =	1.00		Vol(max) =	3.33
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.23	5.69	1.71
10	178.56	4.58	4.04	2.42
15	142.89	3.67	3.13	2.81
20	119.95	3.08	2.54	3.05
25	103.85	2.66	2.12	3.19
30	91.87	2.36	1.82	3.27
35	82.58	2.12	1.58	3.32
40	75.15	1.93	1.39	3.33
45	69.05	1.77	1.23	3.33
50	63.95	1.64	1.10	3.30
55	59.62	1.53	0.99	3.27
60	55.89	1.43	0.89	3.22
65	52.65	1.35	0.81	3.16
70	49.79	1.28	0.74	3.10
75	47.26	1.21	0.67	3.03
80	44.99	1.15	0.61	2.95
85	42.95	1.10	0.56	2.87
90	41.11	1.05	0.51	2.78

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.07	0.04
8	0.14	0.05
12	0.24	0.06
16	0.38	0.07
21	0.56	0.08
27	0.80	0.09
33	1.10	0.10
40	1.47	0.11
48	1.91	0.12
56	2.42	0.13
65	3.03	0.14
74	3.72	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.144 m
3.72	3.33	3.03	Qallow =	0.54 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-28 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0092	ha	Qallow =	0.40
C =	0.90		Vol(max) =	1.51
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.25	2.85	0.85
10	104.19	2.40	2.00	1.20
15	83.56	1.92	1.52	1.37
20	70.25	1.61	1.21	1.46
25	60.90	1.40	1.00	1.50
30	53.93	1.24	0.84	1.51
35	48.52	1.12	0.72	1.50
40	44.18	1.02	0.62	1.48
45	40.63	0.93	0.53	1.44
50	37.65	0.87	0.47	1.40
55	35.12	0.81	0.41	1.34
60	32.94	0.76	0.36	1.29
65	31.04	0.71	0.31	1.22
70	29.37	0.68	0.28	1.16
75	27.89	0.64	0.24	1.08
80	26.56	0.61	0.21	1.01
85	25.37	0.58	0.18	0.93
90	24.29	0.56	0.16	0.86

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.19	0.10
43	1.59	0.11
52	2.06	0.12
61	2.62	0.13
70	3.28	0.14
81	4.03	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.108 m
1.59	1.51	1.19	Qallow =	0.40 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

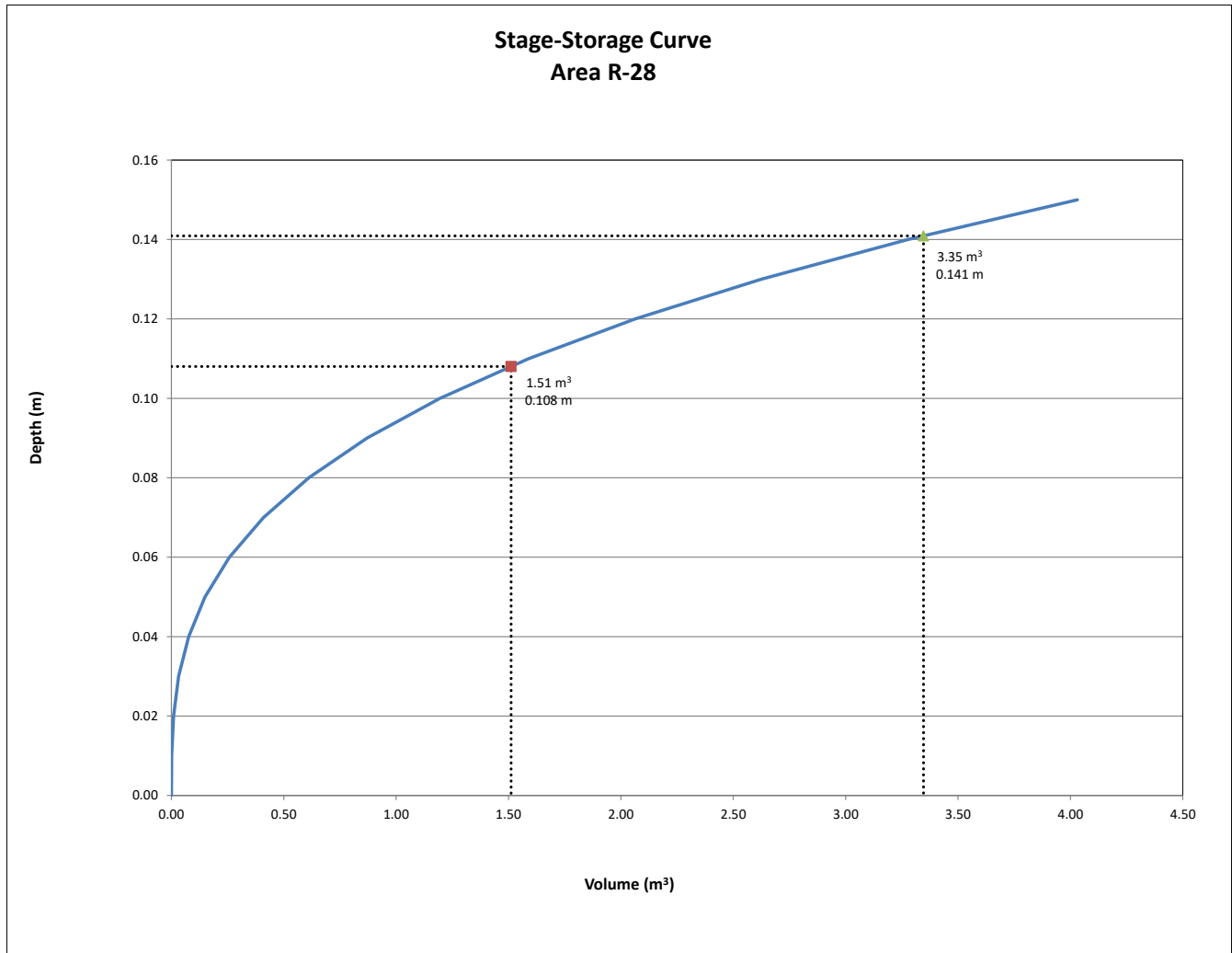
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-28 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0092	ha	Qallow =	0.53
C =	1.00		Vol(max) =	3.35
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.20	5.67	1.70
10	178.56	4.56	4.04	2.42
15	142.89	3.65	3.12	2.81
20	119.95	3.06	2.54	3.05
25	103.85	2.65	2.13	3.19
30	91.87	2.35	1.82	3.28
35	82.58	2.11	1.58	3.33
40	75.15	1.92	1.39	3.35
45	69.05	1.76	1.24	3.34
50	63.95	1.63	1.11	3.33
55	59.62	1.52	1.00	3.29
60	55.89	1.43	0.90	3.25
65	52.65	1.34	0.82	3.20
70	49.79	1.27	0.75	3.14
75	47.26	1.21	0.68	3.07
80	44.99	1.15	0.62	3.00
85	42.95	1.10	0.57	2.92
90	41.11	1.05	0.53	2.84

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.26	0.06
18	0.41	0.07
23	0.61	0.08
29	0.87	0.09
36	1.19	0.10
43	1.59	0.11
52	2.06	0.12
61	2.62	0.13
70	3.28	0.14
81	4.03	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.141 m
4.03	3.35	3.28	Qallow =	0.53 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-29 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0073	ha	Qallow =	0.36
C =	0.90		Vol(max) =	1.13
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.59	2.23	0.67
10	104.19	1.91	1.55	0.93
15	83.56	1.53	1.17	1.05
20	70.25	1.29	0.93	1.11
25	60.90	1.12	0.76	1.13
30	53.93	0.99	0.63	1.13
35	48.52	0.89	0.53	1.11
40	44.18	0.81	0.45	1.08
45	40.63	0.74	0.38	1.04
50	37.65	0.69	0.33	0.99
55	35.12	0.64	0.28	0.94
60	32.94	0.60	0.24	0.88
65	31.04	0.57	0.21	0.81
70	29.37	0.54	0.18	0.75
75	27.89	0.51	0.15	0.68
80	26.56	0.49	0.13	0.61
85	25.37	0.46	0.10	0.53
90	24.29	0.45	0.09	0.46

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.27	0.06
18	0.42	0.07
24	0.63	0.08
30	0.90	0.09
37	1.24	0.10
45	1.65	0.11
54	2.14	0.12
63	2.72	0.13
73	3.40	0.14
84	4.18	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.097 m
1.24	1.13	0.90	Qallow =	0.36 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

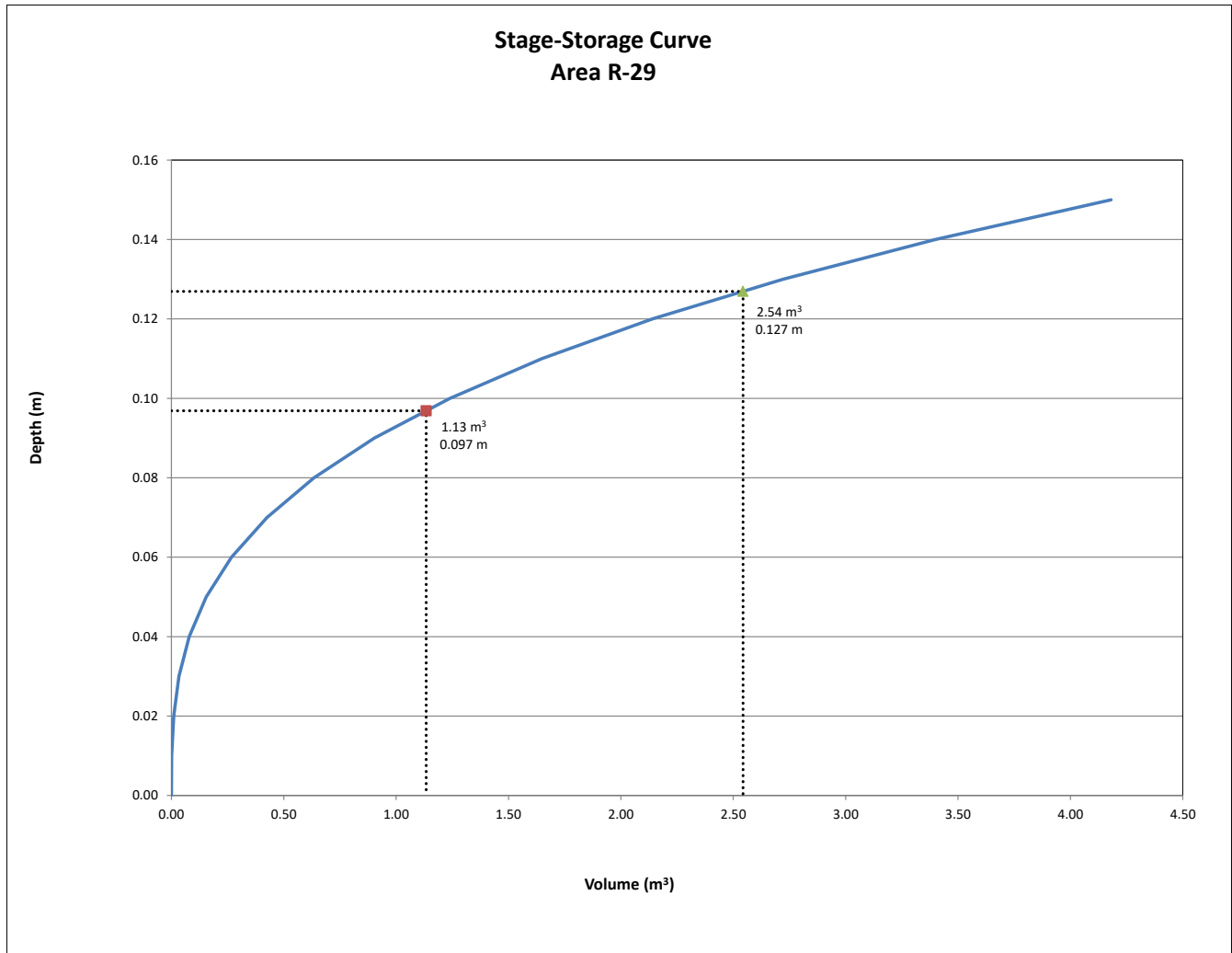
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-29 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0073	ha	Qallow =	0.47
C =	1.00		Vol(max) =	2.54
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.94	4.47	1.34
10	178.56	3.64	3.17	1.90
15	142.89	2.91	2.44	2.20
20	119.95	2.44	1.97	2.37
25	103.85	2.11	1.64	2.47
30	91.87	1.87	1.40	2.52
35	82.58	1.68	1.21	2.54
40	75.15	1.53	1.06	2.54
45	69.05	1.41	0.94	2.53
50	63.95	1.30	0.83	2.50
55	59.62	1.21	0.74	2.45
60	55.89	1.14	0.67	2.40
65	52.65	1.07	0.60	2.35
70	49.79	1.01	0.54	2.28
75	47.26	0.96	0.49	2.21
80	44.99	0.92	0.45	2.14
85	42.95	0.87	0.40	2.06
90	41.11	0.84	0.37	1.98

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.27	0.06
18	0.42	0.07
24	0.63	0.08
30	0.90	0.09
37	1.24	0.10
45	1.65	0.11
54	2.14	0.12
63	2.72	0.13
73	3.40	0.14
84	4.18	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.127 m
2.72	2.54	2.14	Qallow =	0.47 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-30 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0126	ha	Qallow =	0.42
C =	0.90		Vol(max) =	2.34
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	4.46	4.04	1.21
10	104.19	3.29	2.87	1.72
15	83.56	2.64	2.22	2.00
20	70.25	2.22	1.80	2.16
25	60.90	1.92	1.50	2.26
30	53.93	1.70	1.28	2.31
35	48.52	1.53	1.11	2.34
40	44.18	1.40	0.98	2.34
45	40.63	1.28	0.86	2.33
50	37.65	1.19	0.77	2.31
55	35.12	1.11	0.69	2.28
60	32.94	1.04	0.62	2.24
65	31.04	0.98	0.56	2.19
70	29.37	0.93	0.51	2.13
75	27.89	0.88	0.46	2.08
80	26.56	0.84	0.42	2.01
85	25.37	0.80	0.38	1.95
90	24.29	0.77	0.35	1.88

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
8	0.10	0.04
12	0.20	0.05
17	0.34	0.06
23	0.54	0.07
30	0.81	0.08
38	1.15	0.09
47	1.58	0.10
57	2.10	0.11
68	2.73	0.12
80	3.47	0.13
93	4.33	0.14
107	5.33	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.114 m
2.73	2.34	2.10	Qallow =	0.42 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

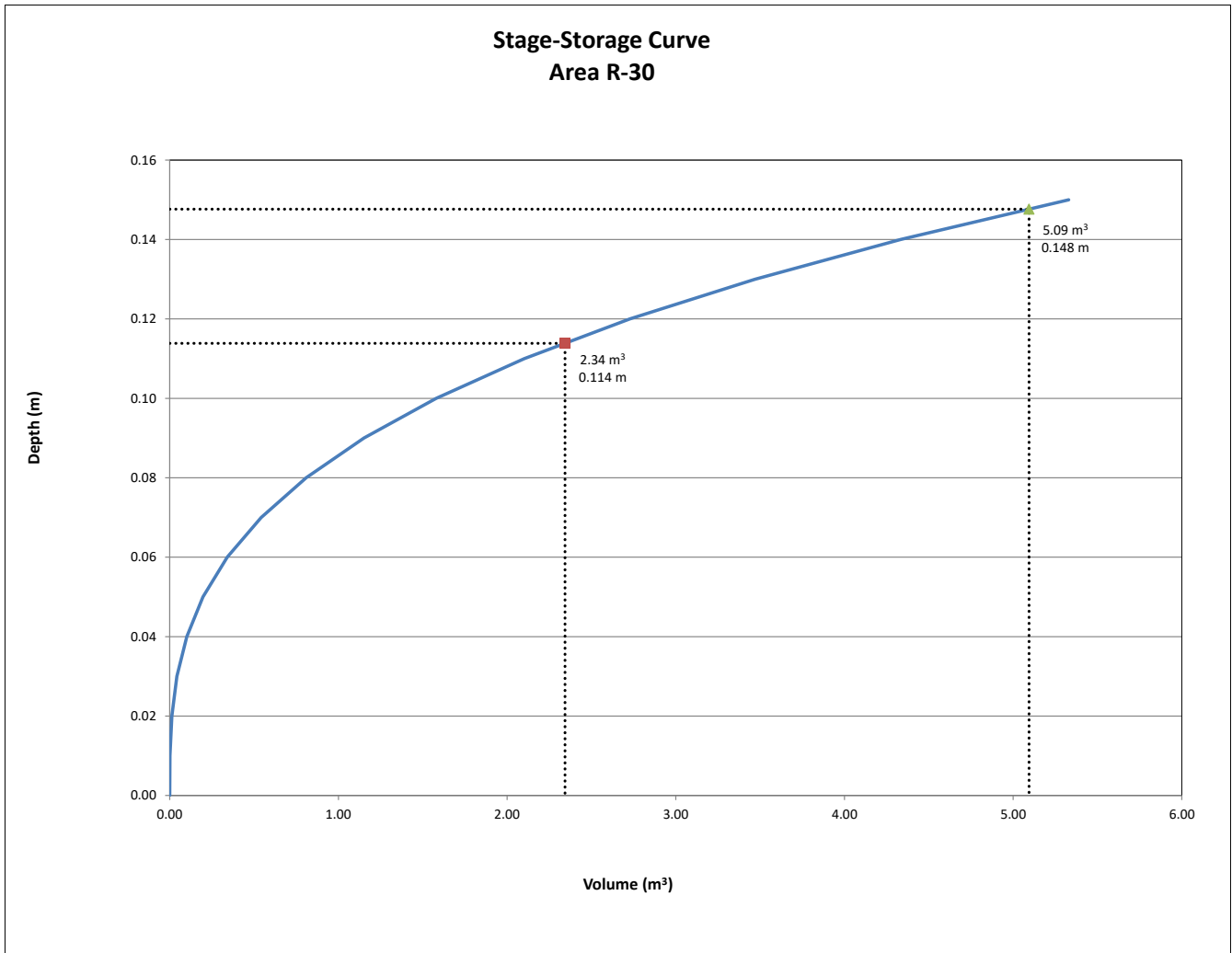
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-30 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0126	ha	Qallow =	0.55
C =	1.00		Vol(max) =	5.09
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	8.52	7.97	2.39
10	178.56	6.27	5.72	3.43
15	142.89	5.02	4.47	4.02
20	119.95	4.21	3.66	4.39
25	103.85	3.65	3.10	4.65
30	91.87	3.23	2.68	4.82
35	82.58	2.90	2.35	4.93
40	75.15	2.64	2.09	5.01
45	69.05	2.42	1.87	5.06
50	63.95	2.25	1.70	5.09
55	59.62	2.09	1.54	5.09
60	55.89	1.96	1.41	5.09
65	52.65	1.85	1.30	5.07
70	49.79	1.75	1.20	5.03
75	47.26	1.66	1.11	4.99
80	44.99	1.58	1.03	4.94
85	42.95	1.51	0.96	4.89
90	41.11	1.44	0.89	4.83

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
2	0.01	0.02
4	0.04	0.03
8	0.10	0.04
12	0.20	0.05
17	0.34	0.06
23	0.54	0.07
30	0.81	0.08
38	1.15	0.09
47	1.58	0.10
57	2.10	0.11
68	2.73	0.12
80	3.47	0.13
93	4.33	0.14
107	5.33	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.148 m
5.33	5.09	4.33	Qallow =	0.55 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-31 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0042	ha	Qallow =	0.35
C =	0.90		Vol(max) =	0.47
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.48	1.13	0.34
10	104.19	1.09	0.74	0.45
15	83.56	0.88	0.53	0.47
20	70.25	0.74	0.39	0.46
25	60.90	0.64	0.29	0.43
30	53.93	0.57	0.22	0.39
35	48.52	0.51	0.16	0.33
40	44.18	0.46	0.11	0.27
45	40.63	0.43	0.08	0.20
50	37.65	0.39	0.04	0.13
55	35.12	0.37	0.02	0.06
60	32.94	0.35	0.00	-0.02
65	31.04	0.33	-0.02	-0.10
70	29.37	0.31	-0.04	-0.18
75	27.89	0.29	-0.06	-0.26
80	26.56	0.28	-0.07	-0.34
85	25.37	0.27	-0.08	-0.43
90	24.29	0.25	-0.10	-0.52

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
3	0.04	0.04
4	0.07	0.05
6	0.12	0.06
8	0.19	0.07
11	0.28	0.08
13	0.40	0.09
17	0.55	0.10
20	0.74	0.11
24	0.96	0.12
28	1.22	0.13
33	1.52	0.14
37	1.87	0.15

Linear Interpolation				
0.10	H	0.09	H =	0.095 m
0.55	0.47	0.40	Qallow =	0.35 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

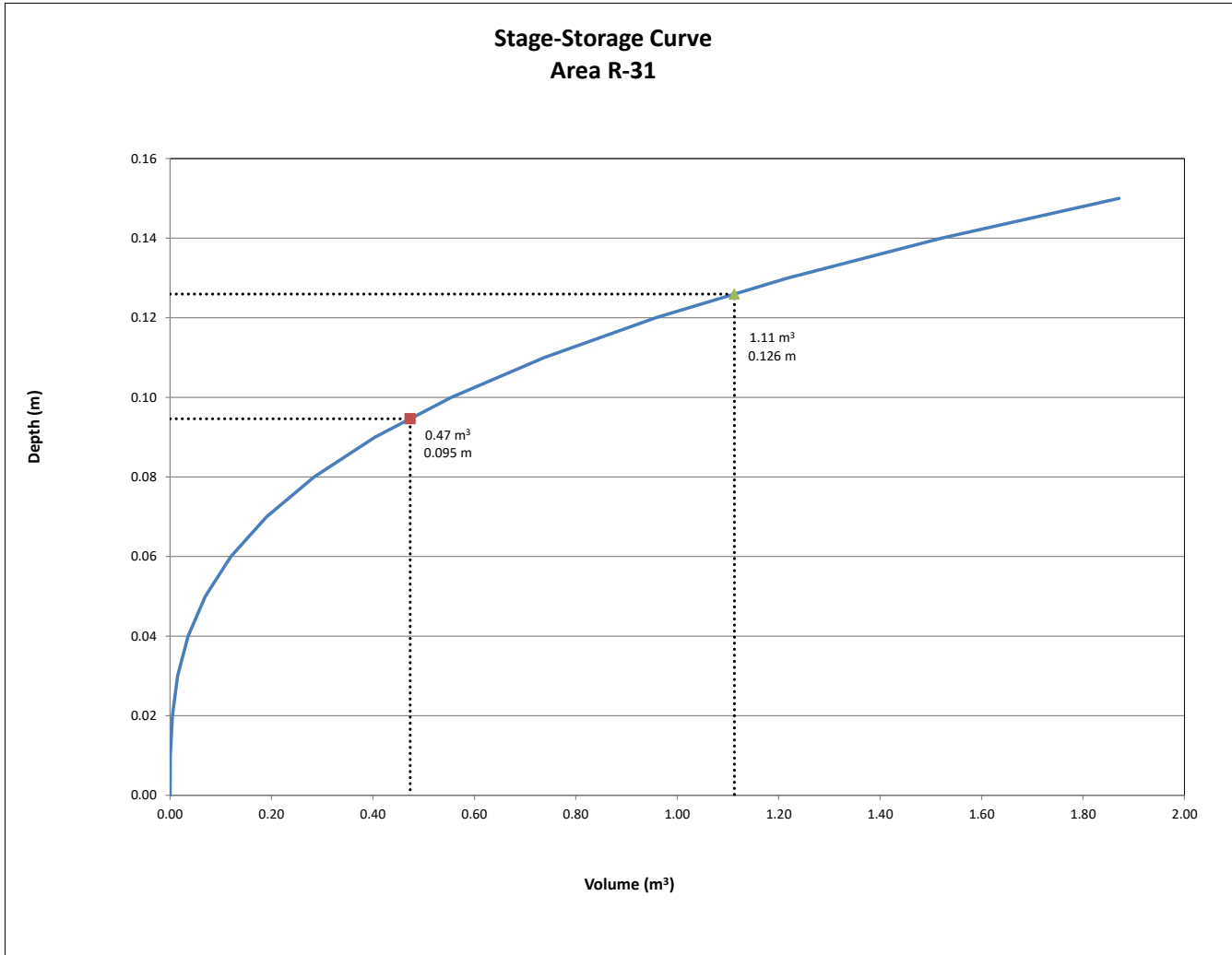
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-31 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0042	ha	Qallow =	0.47
C =	1.00		Vol(max) =	1.11
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	2.83	2.36	0.71
10	178.56	2.08	1.61	0.97
15	142.89	1.66	1.19	1.08
20	119.95	1.40	0.93	1.11
25	103.85	1.21	0.74	1.11
30	91.87	1.07	0.60	1.08
35	82.58	0.96	0.49	1.03
40	75.15	0.88	0.41	0.97
45	69.05	0.80	0.33	0.90
50	63.95	0.74	0.27	0.82
55	59.62	0.69	0.22	0.74
60	55.89	0.65	0.18	0.65
65	52.65	0.61	0.14	0.56
70	49.79	0.58	0.11	0.46
75	47.26	0.55	0.08	0.36
80	44.99	0.52	0.05	0.26
85	42.95	0.50	0.03	0.15
90	41.11	0.48	0.01	0.05

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.00	0.02
1	0.01	0.03
3	0.04	0.04
4	0.07	0.05
6	0.12	0.06
8	0.19	0.07
11	0.28	0.08
13	0.40	0.09
17	0.55	0.10
20	0.74	0.11
24	0.96	0.12
28	1.22	0.13
33	1.52	0.14
37	1.87	0.15

Linear Interpolation				
0.13	H	0.12	H =	0.126 m
1.22	1.11	0.96	Qallow =	0.47 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-32 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0124	ha	Qallow =	0.41
C =	0.90		Vol(max) =	2.31
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	4.39	3.98	1.19
10	104.19	3.24	2.83	1.70
15	83.56	2.60	2.19	1.97
20	70.25	2.18	1.77	2.13
25	60.90	1.89	1.48	2.22
30	53.93	1.68	1.27	2.28
35	48.52	1.51	1.10	2.30
40	44.18	1.37	0.96	2.31
45	40.63	1.26	0.85	2.30
50	37.65	1.17	0.76	2.28
55	35.12	1.09	0.68	2.25
60	32.94	1.02	0.61	2.21
65	31.04	0.96	0.55	2.16
70	29.37	0.91	0.50	2.11
75	27.89	0.87	0.46	2.05
80	26.56	0.83	0.42	1.99
85	25.37	0.79	0.38	1.93
90	24.29	0.75	0.34	1.86

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
1	0.00	0.01
2	0.01	0.02
5	0.05	0.03
8	0.11	0.04
13	0.22	0.05
19	0.37	0.06
25	0.59	0.07
33	0.88	0.08
42	1.26	0.09
52	1.73	0.10
63	2.30	0.11
75	2.99	0.12
88	3.80	0.13
102	4.74	0.14
117	5.83	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.110 m
2.99	2.31	2.30	Q _{allow} =	0.41 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

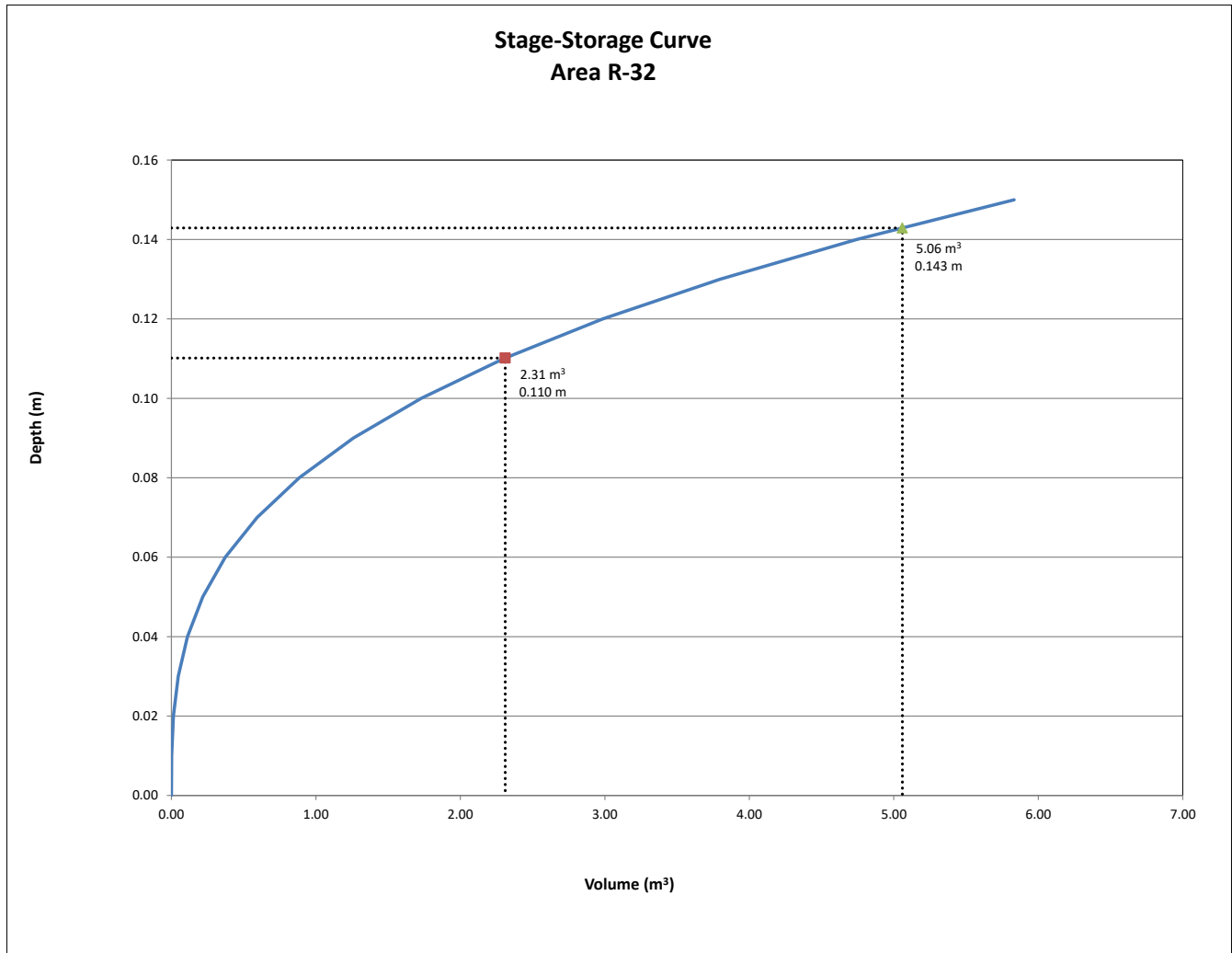
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-32 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0124	ha	Qallow =	0.53
C =	1.00		Vol(max) =	5.06
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	8.38	7.85	2.36
10	178.56	6.16	5.64	3.38
15	142.89	4.93	4.41	3.97
20	119.95	4.14	3.62	4.34
25	103.85	3.58	3.06	4.59
30	91.87	3.17	2.65	4.76
35	82.58	2.85	2.33	4.88
40	75.15	2.59	2.07	4.97
45	69.05	2.38	1.86	5.02
50	63.95	2.21	1.68	5.05
55	59.62	2.06	1.53	5.06
60	55.89	1.93	1.40	5.06
65	52.65	1.82	1.29	5.04
70	49.79	1.72	1.19	5.01
75	47.26	1.63	1.11	4.98
80	44.99	1.55	1.03	4.93
85	42.95	1.48	0.96	4.88
90	41.11	1.42	0.89	4.83

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
1	0.00	0.01
2	0.01	0.02
5	0.05	0.03
8	0.11	0.04
13	0.22	0.05
19	0.37	0.06
25	0.59	0.07
33	0.88	0.08
42	1.26	0.09
52	1.73	0.10
63	2.30	0.11
75	2.99	0.12
88	3.80	0.13
102	4.74	0.14
117	5.83	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.143 m
5.83	5.06	4.74	Q _{allow} =	0.53 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-33 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0064	ha	Qallow =	0.38
C =	0.90		Vol(max) =	0.90
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.26	1.89	0.57
10	104.19	1.67	1.29	0.78
15	83.56	1.34	0.96	0.87
20	70.25	1.13	0.75	0.90
25	60.90	0.98	0.60	0.90
30	53.93	0.86	0.49	0.88
35	48.52	0.78	0.40	0.85
40	44.18	0.71	0.33	0.80
45	40.63	0.65	0.28	0.75
50	37.65	0.60	0.23	0.68
55	35.12	0.56	0.19	0.62
60	32.94	0.53	0.15	0.55
65	31.04	0.50	0.12	0.48
70	29.37	0.47	0.10	0.40
75	27.89	0.45	0.07	0.32
80	26.56	0.43	0.05	0.24
85	25.37	0.41	0.03	0.16
90	24.29	0.39	0.01	0.08

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.30	0.07
17	0.45	0.08
21	0.64	0.09
27	0.88	0.10
32	1.18	0.11
38	1.53	0.12
45	1.94	0.13
52	2.43	0.14
60	2.98	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.101 m
1.18	0.90	0.88	Q _{allow} =	0.38 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

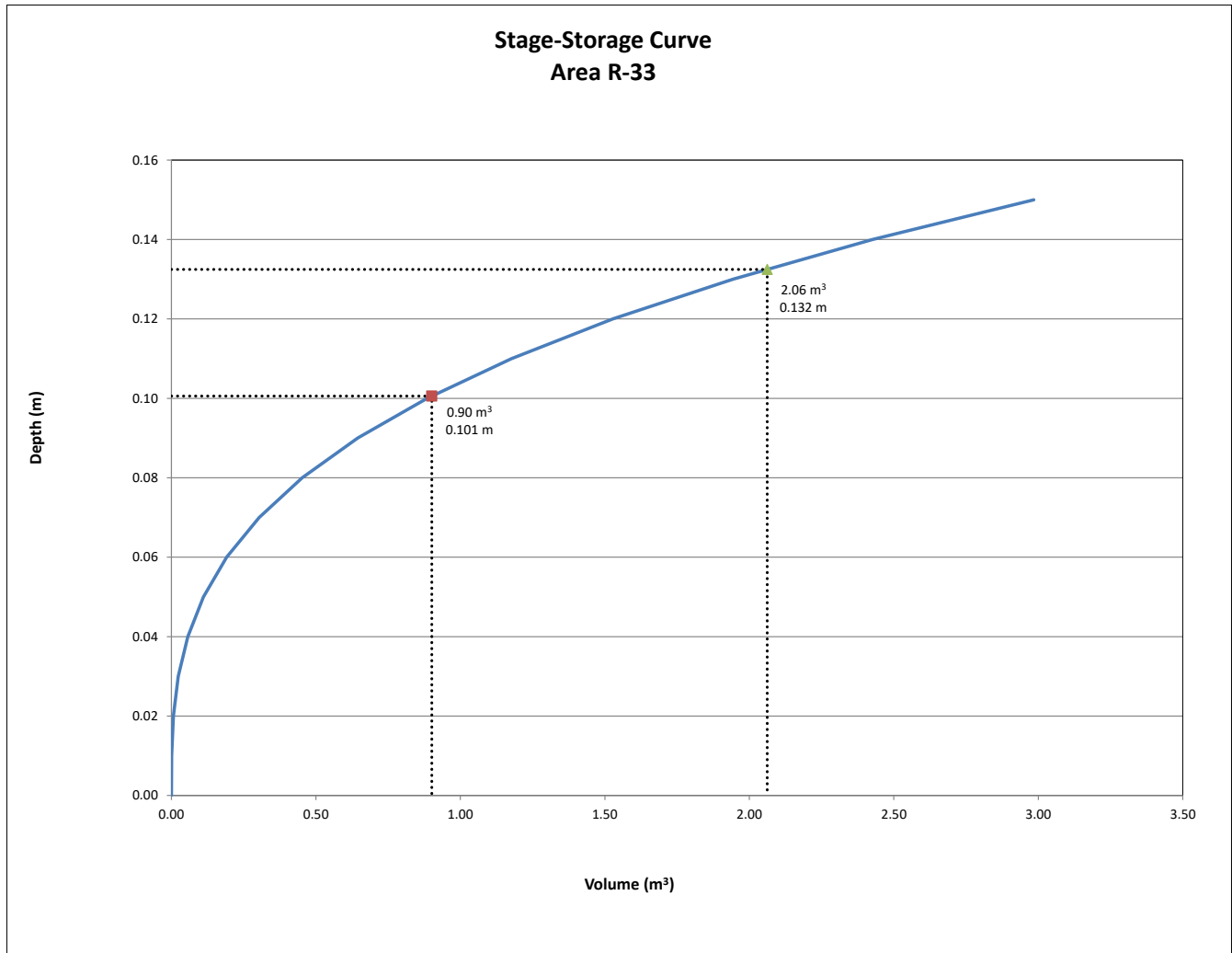
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-33 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0064	ha	Qallow =	0.49
C =	1.00		Vol(max) =	2.06
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.32	3.83	1.15
10	178.56	3.18	2.69	1.61
15	142.89	2.54	2.05	1.85
20	119.95	2.14	1.65	1.97
25	103.85	1.85	1.36	2.04
30	91.87	1.64	1.15	2.06
35	82.58	1.47	0.98	2.06
40	75.15	1.34	0.85	2.03
45	69.05	1.23	0.74	2.00
50	63.95	1.14	0.65	1.95
55	59.62	1.06	0.57	1.89
60	55.89	1.00	0.51	1.82
65	52.65	0.94	0.45	1.74
70	49.79	0.89	0.40	1.66
75	47.26	0.84	0.35	1.58
80	44.99	0.80	0.31	1.49
85	42.95	0.76	0.27	1.40
90	41.11	0.73	0.24	1.31

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.11	0.05
10	0.19	0.06
13	0.30	0.07
17	0.45	0.08
21	0.64	0.09
27	0.88	0.10
32	1.18	0.11
38	1.53	0.12
45	1.94	0.13
52	2.43	0.14
60	2.98	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.132 m
2.43	2.06	1.94	Q _{allow} =	0.49 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-34 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0061	ha	Qallow =	0.38
C =	0.90		Vol(max) =	0.83
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.15	1.77	0.53
10	104.19	1.58	1.20	0.72
15	83.56	1.27	0.89	0.80
20	70.25	1.07	0.69	0.83
25	60.90	0.93	0.55	0.82
30	53.93	0.82	0.44	0.79
35	48.52	0.74	0.36	0.75
40	44.18	0.67	0.29	0.70
45	40.63	0.62	0.24	0.64
50	37.65	0.57	0.19	0.58
55	35.12	0.53	0.15	0.51
60	32.94	0.50	0.12	0.44
65	31.04	0.47	0.09	0.36
70	29.37	0.45	0.07	0.28
75	27.89	0.42	0.04	0.20
80	26.56	0.40	0.02	0.12
85	25.37	0.39	0.01	0.03
90	24.29	0.37	-0.01	-0.06

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.17	0.06
12	0.28	0.07
15	0.41	0.08
20	0.59	0.09
24	0.80	0.10
29	1.07	0.11
35	1.39	0.12
41	1.76	0.13
47	2.20	0.14
54	2.71	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.101 m
1.07	0.83	0.80	Qallow =	0.38 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

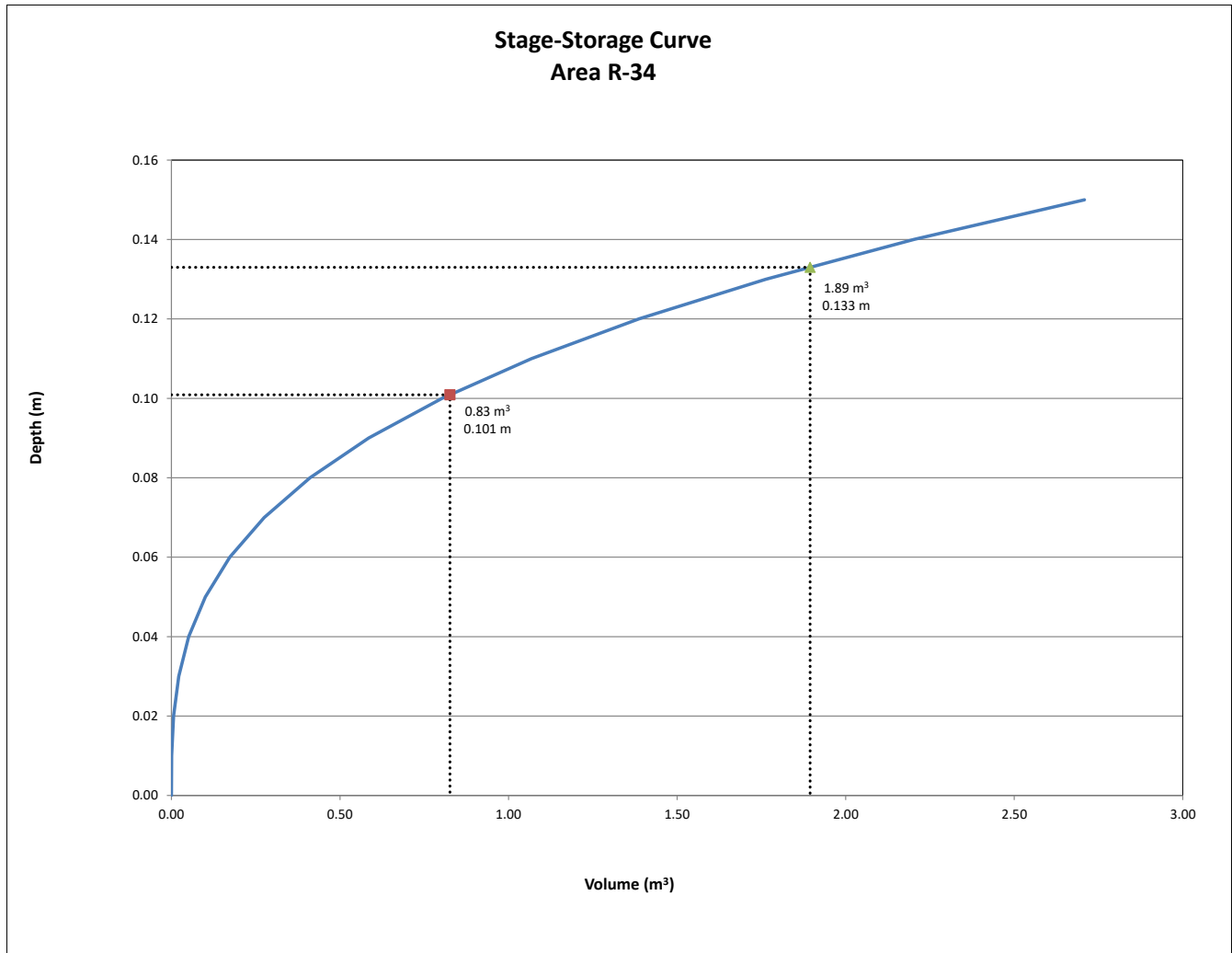
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-34 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0061	ha	Qallow =	0.50
C =	1.00		Vol(max) =	1.89
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	4.10	3.60	1.08
10	178.56	3.02	2.52	1.51
15	142.89	2.41	1.91	1.72
20	119.95	2.03	1.53	1.83
25	103.85	1.75	1.25	1.88
30	91.87	1.55	1.05	1.89
35	82.58	1.40	0.90	1.88
40	75.15	1.27	0.77	1.85
45	69.05	1.17	0.67	1.80
50	63.95	1.08	0.58	1.74
55	59.62	1.01	0.51	1.68
60	55.89	0.94	0.44	1.60
65	52.65	0.89	0.39	1.52
70	49.79	0.84	0.34	1.43
75	47.26	0.80	0.30	1.34
80	44.99	0.76	0.26	1.25
85	42.95	0.73	0.23	1.15
90	41.11	0.69	0.19	1.05

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.17	0.06
12	0.28	0.07
15	0.41	0.08
20	0.59	0.09
24	0.80	0.10
29	1.07	0.11
35	1.39	0.12
41	1.76	0.13
47	2.20	0.14
54	2.71	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.133 m
2.20	1.89	1.76	Qallow =	0.50 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-35 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0101	ha	Qallow =	0.81
C =	0.90		Vol(max) =	1.18
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.58	2.77	0.83
10	104.19	2.64	1.83	1.10
15	83.56	2.12	1.31	1.18
20	70.25	1.78	0.97	1.16
25	60.90	1.54	0.73	1.10
30	53.93	1.37	0.56	1.00
35	48.52	1.23	0.42	0.88
40	44.18	1.12	0.31	0.74
45	40.63	1.03	0.22	0.59
50	37.65	0.95	0.14	0.43
55	35.12	0.89	0.08	0.26
60	32.94	0.83	0.02	0.09
65	31.04	0.79	-0.02	-0.09
70	29.37	0.74	-0.07	-0.28
75	27.89	0.71	-0.10	-0.46
80	26.56	0.67	-0.14	-0.66
85	25.37	0.64	-0.17	-0.85
90	24.29	0.62	-0.19	-1.05

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.12	0.05
10	0.20	0.06
14	0.32	0.07
18	0.47	0.08
22	0.67	0.09
28	0.93	0.10
34	1.23	0.11
40	1.60	0.12
47	2.03	0.13
54	2.54	0.14
62	3.12	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.108 m
1.23	1.18	0.93	Qallow =	0.81 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

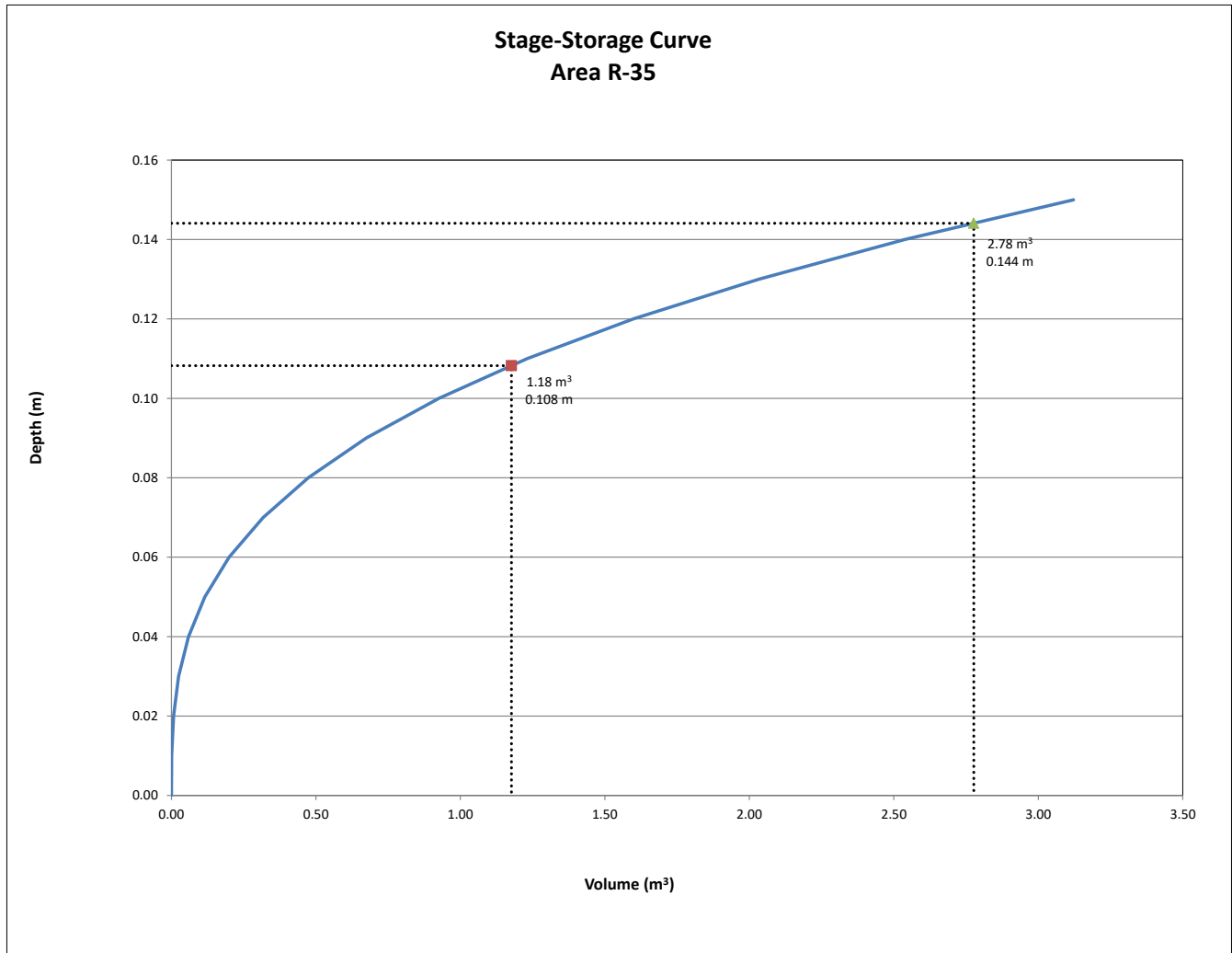
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-35 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0101	ha	Qallow =	1.07
C =	1.00		Vol(max) =	2.78
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.83	5.76	1.73
10	178.56	5.03	3.95	2.37
15	142.89	4.02	2.95	2.66
20	119.95	3.38	2.30	2.77
25	103.85	2.92	1.85	2.78
30	91.87	2.59	1.51	2.72
35	82.58	2.33	1.25	2.63
40	75.15	2.12	1.04	2.50
45	69.05	1.94	0.87	2.35
50	63.95	1.80	0.73	2.18
55	59.62	1.68	0.61	2.00
60	55.89	1.57	0.50	1.80
65	52.65	1.48	0.41	1.60
70	49.79	1.40	0.33	1.38
75	47.26	1.33	0.26	1.16
80	44.99	1.27	0.19	0.93
85	42.95	1.21	0.14	0.70
90	41.11	1.16	0.08	0.46

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.06	0.04
7	0.12	0.05
10	0.20	0.06
14	0.32	0.07
18	0.47	0.08
22	0.67	0.09
28	0.93	0.10
34	1.23	0.11
40	1.60	0.12
47	2.03	0.13
54	2.54	0.14
62	3.12	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.144 m
3.12	2.78	2.54	Qallow =	1.07 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-36 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0055	ha	Qallow =	0.32
C =	0.90		Vol(max) =	0.77
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	1.93	1.61	0.48
10	104.19	1.42	1.10	0.66
15	83.56	1.14	0.82	0.74
20	70.25	0.96	0.64	0.77
25	60.90	0.83	0.51	0.77
30	53.93	0.74	0.42	0.75
35	48.52	0.66	0.34	0.72
40	44.18	0.60	0.28	0.68
45	40.63	0.55	0.23	0.63
50	37.65	0.51	0.19	0.58
55	35.12	0.48	0.16	0.53
60	32.94	0.45	0.13	0.47
65	31.04	0.42	0.10	0.40
70	29.37	0.40	0.08	0.34
75	27.89	0.38	0.06	0.27
80	26.56	0.36	0.04	0.20
85	25.37	0.35	0.03	0.13
90	24.29	0.33	0.01	0.06

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.27	0.06
18	0.42	0.07
24	0.63	0.08
30	0.90	0.09
37	1.24	0.10
45	1.65	0.11
53	2.14	0.12
63	2.72	0.13
73	3.39	0.14
84	4.18	0.15

Linear Interpolation			
0.09	H	0.08	H = 0.085 m
0.90	0.77	0.63	Qallow = 0.32 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

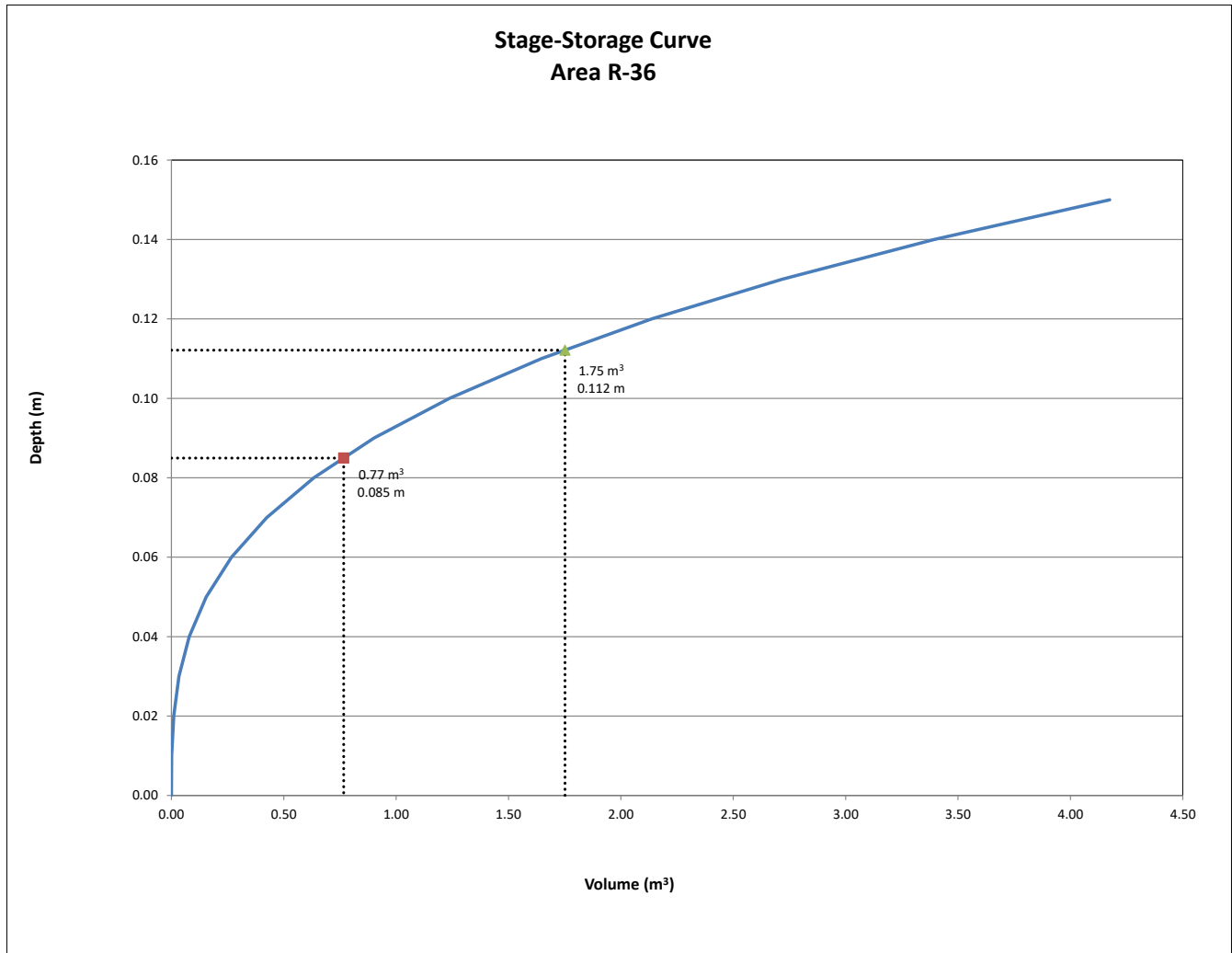
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-36 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0055	ha	Qallow =	0.42
C =	1.00		Vol(max) =	1.75
			Notches =	1
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	3.68	3.26	0.98
10	178.56	2.71	2.29	1.37
15	142.89	2.17	1.75	1.57
20	119.95	1.82	1.40	1.68
25	103.85	1.57	1.15	1.73
30	91.87	1.39	0.97	1.75
35	82.58	1.25	0.83	1.75
40	75.15	1.14	0.72	1.73
45	69.05	1.05	0.63	1.69
50	63.95	0.97	0.55	1.65
55	59.62	0.90	0.48	1.60
60	55.89	0.85	0.43	1.54
65	52.65	0.80	0.38	1.48
70	49.79	0.75	0.33	1.41
75	47.26	0.72	0.30	1.33
80	44.99	0.68	0.26	1.26
85	42.95	0.65	0.23	1.18
90	41.11	0.62	0.20	1.10

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
6	0.08	0.04
9	0.15	0.05
13	0.27	0.06
18	0.42	0.07
24	0.63	0.08
30	0.90	0.09
37	1.24	0.10
45	1.65	0.11
53	2.14	0.12
63	2.72	0.13
73	3.39	0.14
84	4.18	0.15

Linear Interpolation			
0.12	H	0.11	H = 0.112 m
2.14	1.75	1.65	Qallow = 0.42 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-37 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0096	ha	Qallow =	0.77
C =	0.90		Vol(max) =	1.11
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.39	2.62	0.79
10	104.19	2.50	1.73	1.04
15	83.56	2.00	1.23	1.11
20	70.25	1.69	0.92	1.10
25	60.90	1.46	0.69	1.04
30	53.93	1.29	0.52	0.94
35	48.52	1.16	0.39	0.83
40	44.18	1.06	0.29	0.70
45	40.63	0.97	0.20	0.55
50	37.65	0.90	0.13	0.40
55	35.12	0.84	0.07	0.24
60	32.94	0.79	0.02	0.07
65	31.04	0.74	-0.03	-0.10
70	29.37	0.70	-0.07	-0.27
75	27.89	0.67	-0.10	-0.45
80	26.56	0.64	-0.13	-0.64
85	25.37	0.61	-0.16	-0.82
90	24.29	0.58	-0.19	-1.01

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
8	0.13	0.05
11	0.22	0.06
15	0.34	0.07
19	0.51	0.08
24	0.73	0.09
30	1.00	0.10
36	1.33	0.11
43	1.73	0.12
51	2.20	0.13
59	2.75	0.14
68	3.38	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.103 m
1.33	1.11	1.00	Qallow =	0.77 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

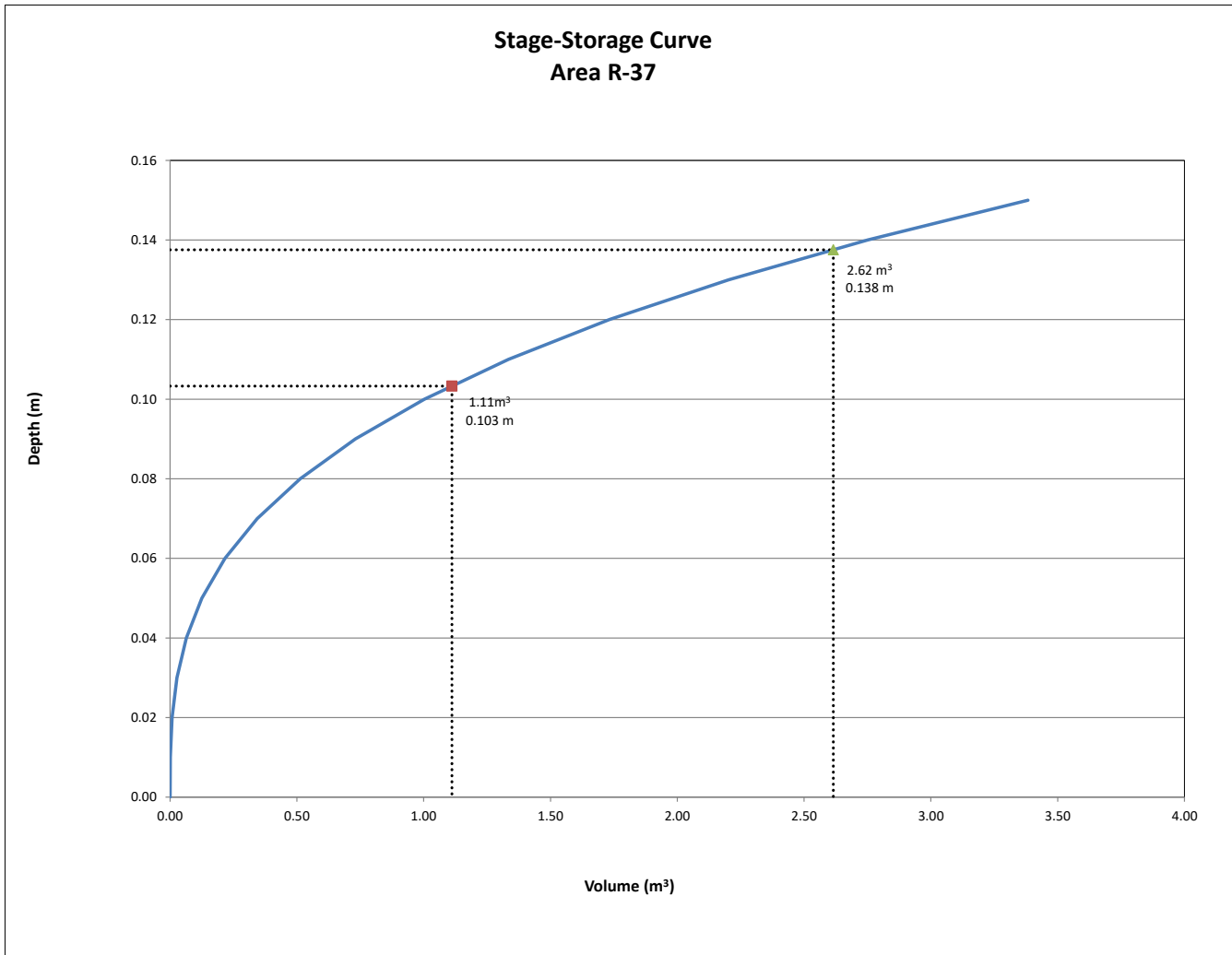
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-37 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0096	ha	Qallow =	1.03
C =	1.00		Vol(max) =	2.62
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	6.47	5.45	1.63
10	178.56	4.76	3.74	2.24
15	142.89	3.81	2.78	2.51
20	119.95	3.20	2.17	2.61
25	103.85	2.77	1.74	2.62
30	91.87	2.45	1.42	2.56
35	82.58	2.20	1.18	2.47
40	75.15	2.00	0.98	2.35
45	69.05	1.84	0.82	2.20
50	63.95	1.71	0.68	2.04
55	59.62	1.59	0.56	1.86
60	55.89	1.49	0.47	1.67
65	52.65	1.40	0.38	1.48
70	49.79	1.33	0.30	1.27
75	47.26	1.26	0.23	1.06
80	44.99	1.20	0.17	0.84
85	42.95	1.15	0.12	0.61
90	41.11	1.10	0.07	0.38

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
3	0.03	0.03
5	0.06	0.04
8	0.13	0.05
11	0.22	0.06
15	0.34	0.07
19	0.51	0.08
24	0.73	0.09
30	1.00	0.10
36	1.33	0.11
43	1.73	0.12
51	2.20	0.13
59	2.75	0.14
68	3.38	0.15

Linear Interpolation				
0.14	H	0.13	H =	0.138 m
2.75	2.62	2.20	Qallow =	1.03 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-38 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0104	ha	Qallow =	0.85
C =	0.90		Vol(max) =	1.19
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	3.68	2.83	0.85
10	104.19	2.72	1.87	1.12
15	83.56	2.18	1.33	1.19
20	70.25	1.83	0.98	1.18
25	60.90	1.59	0.74	1.11
30	53.93	1.41	0.56	1.00
35	48.52	1.26	0.41	0.87
40	44.18	1.15	0.30	0.72
45	40.63	1.06	0.21	0.56
50	37.65	0.98	0.13	0.39
55	35.12	0.92	0.07	0.22
60	32.94	0.86	0.01	0.03
65	31.04	0.81	-0.04	-0.16
70	29.37	0.77	-0.08	-0.36
75	27.89	0.73	-0.12	-0.55
80	26.56	0.69	-0.16	-0.76
85	25.37	0.66	-0.19	-0.96
90	24.29	0.63	-0.22	-1.17

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.17	0.06
12	0.27	0.07
15	0.41	0.08
19	0.58	0.09
24	0.79	0.10
29	1.06	0.11
34	1.37	0.12
40	1.74	0.13
47	2.18	0.14
54	2.68	0.15

Linear Interpolation				
0.12	H	0.11	H =	0.114 m
1.37	1.19	1.06	Qallow =	0.85 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

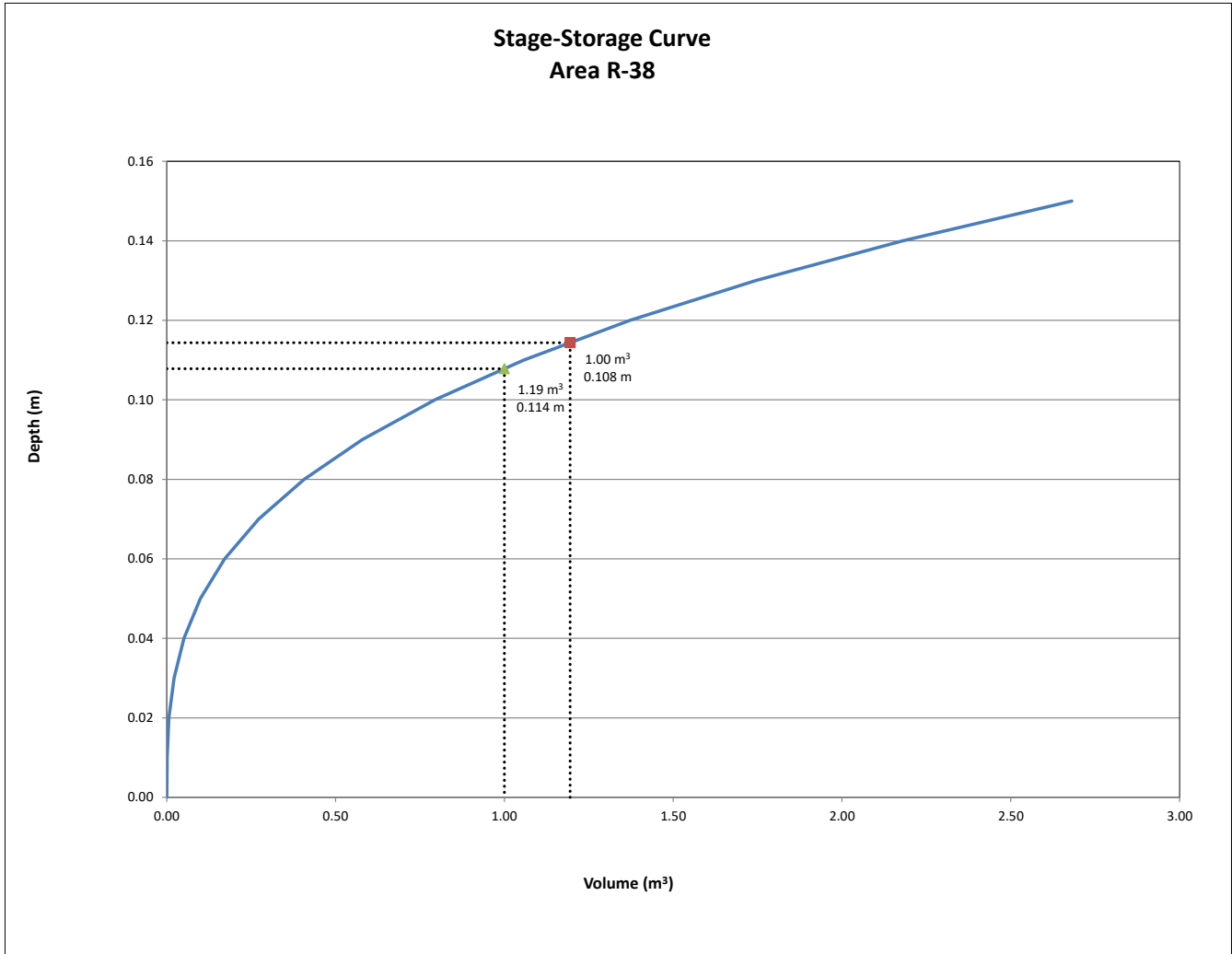
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-38 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0104	ha	Qallow =	0.80
C =	1.00		Vol(max) =	1.00
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	7.03	6.23	1.87
10	178.56	5.17	4.37	2.62
15	142.89	4.14	3.34	3.00
20	119.95	3.47	2.67	3.21
25	103.85	3.01	2.21	3.31
30	91.87	2.66	1.86	3.35
35	82.58	2.39	1.59	3.34
40	75.15	2.18	1.38	3.30
45	69.05	2.00	1.20	3.24
50	63.95	1.85	1.05	3.16
55	59.62	1.73	0.93	3.06
60	55.89	1.62	0.82	2.95
65	52.65	1.52	0.72	2.82
70	49.79	1.44	0.64	2.69
75	47.26	1.37	0.57	2.56
80	44.99	1.30	0.50	2.41
85	42.95	1.24	0.44	2.26
90	41.11	1.19	0.39	2.11

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
4	0.05	0.04
6	0.10	0.05
9	0.17	0.06
12	0.27	0.07
15	0.41	0.08
19	0.58	0.09
24	0.79	0.10
29	1.06	0.11
34	1.37	0.12
40	1.74	0.13
47	2.18	0.14
54	2.68	0.15

Linear Interpolation				
0.11	H	0.1	H =	0.108 m
1.06	1.00	0.79	Qallow =	0.80 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)



REQUIRED STORAGE - 5-YEAR EVENT				
AREA R-39 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0083	ha	Qallow =	0.81
C =	0.90		Vol(max) =	0.83
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	141.18	2.92	2.11	0.63
10	104.19	2.16	1.35	0.81
15	83.56	1.73	0.92	0.83
20	70.25	1.45	0.64	0.77
25	60.90	1.26	0.45	0.67
30	53.93	1.12	0.31	0.55
35	48.52	1.00	0.19	0.41
40	44.18	0.91	0.10	0.25
45	40.63	0.84	0.03	0.08
50	37.65	0.78	-0.03	-0.09
55	35.12	0.73	-0.08	-0.27
60	32.94	0.68	-0.13	-0.46
65	31.04	0.64	-0.17	-0.65
70	29.37	0.61	-0.20	-0.85
75	27.89	0.58	-0.23	-1.05
80	26.56	0.55	-0.26	-1.25
85	25.37	0.52	-0.29	-1.45
90	24.29	0.50	-0.31	-1.66

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (5-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
9	0.22	0.07
12	0.33	0.08
15	0.46	0.09
19	0.64	0.10
23	0.85	0.11
27	1.10	0.12
32	1.40	0.13
37	1.74	0.14
43	2.15	0.15

Linear Interpolation				
0.11	H	0.10	H =	0.109 m
0.85	0.83	0.64	Qallow =	0.81 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

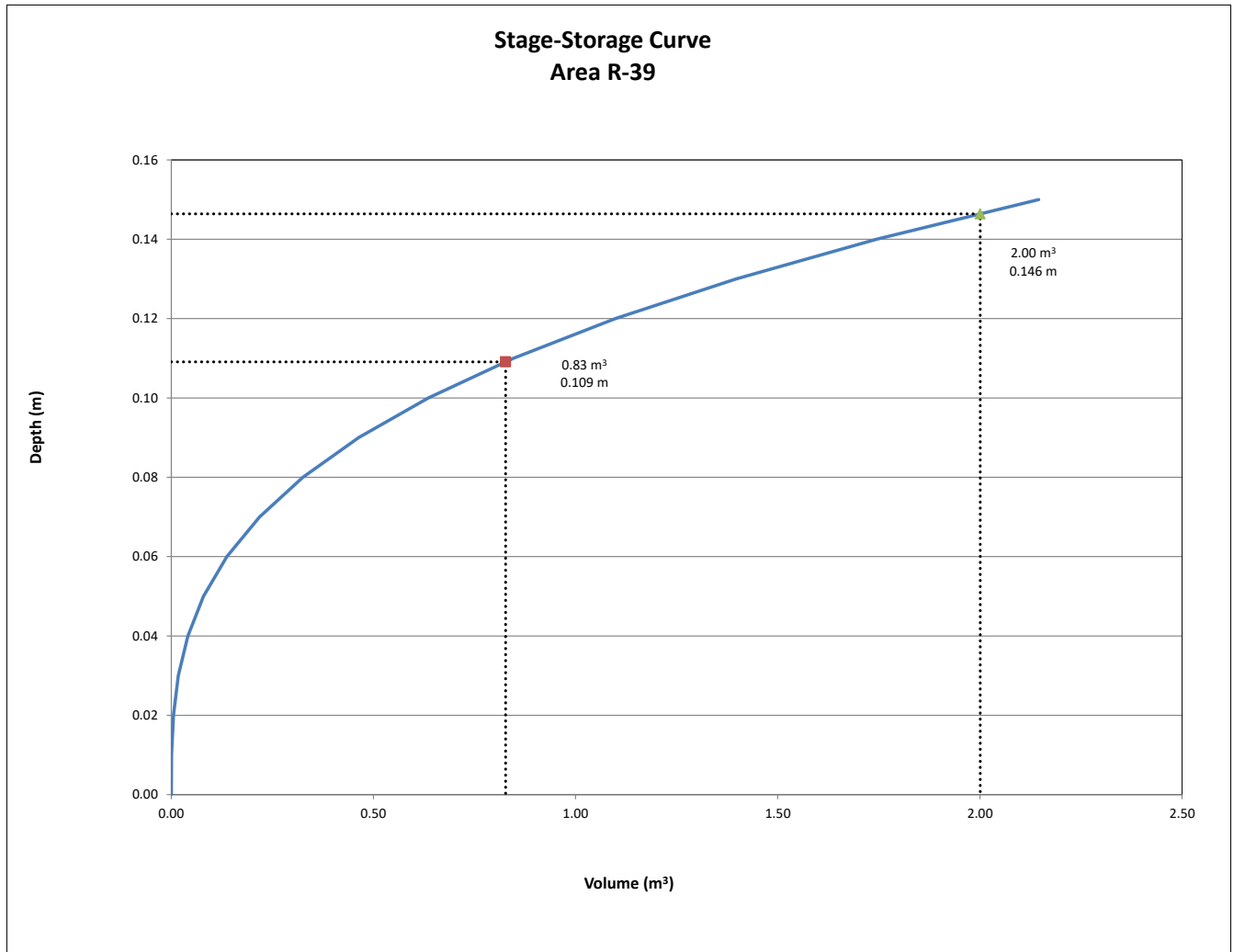
REQUIRED STORAGE - 100-YEAR EVENT				
AREA R-39 : BUILDING ROOF				
OTTAWA IDF CURVE				
Area =	0.0083	ha	Qallow =	1.09
C =	1.00		Vol(max) =	2.00
			Notches =	2
Time (min)	Intensity (mm/hr)	Q (L/s)	Qnet (L/s)	Vol (m ³)
5	242.70	5.58	4.49	1.35
10	178.56	4.10	3.01	1.81
15	142.89	3.28	2.19	1.98
20	119.95	2.76	1.67	2.00
25	103.85	2.39	1.30	1.95
30	91.87	2.11	1.02	1.84
35	82.58	1.90	0.81	1.70
40	75.15	1.73	0.64	1.53
45	69.05	1.59	0.50	1.34
50	63.95	1.47	0.38	1.14
55	59.62	1.37	0.28	0.93
60	55.89	1.28	0.19	0.70
65	52.65	1.21	0.12	0.47
70	49.79	1.14	0.05	0.23
75	47.26	1.09	0.00	-0.02
80	44.99	1.03	-0.06	-0.27
85	42.95	0.99	-0.10	-0.52
90	41.11	0.95	-0.14	-0.78

Notes: Vol = Qnet x time
Qnet = Q - Qallow

Ponding Depth (100-Year Storm)		
Area m ²	V m ³	H m
0	0.00	0.00
0	0.00	0.01
1	0.01	0.02
2	0.02	0.03
3	0.04	0.04
5	0.08	0.05
7	0.14	0.06
9	0.22	0.07
12	0.33	0.08
15	0.46	0.09
19	0.64	0.10
23	0.85	0.11
27	1.10	0.12
32	1.40	0.13
37	1.74	0.14
43	2.15	0.15

Linear Interpolation				
0.15	H	0.14	H =	0.146 m
2.15	2.00	1.74	Qallow =	1.09 L/s

Note: Qallow is the flow rate through an overcontrolled Zurn Roof Drain (3.73 L/s/m of head.)

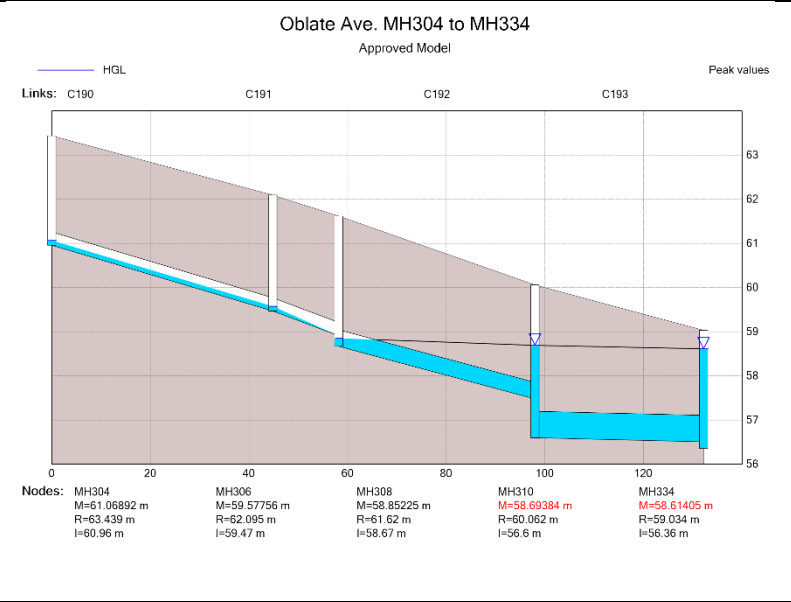
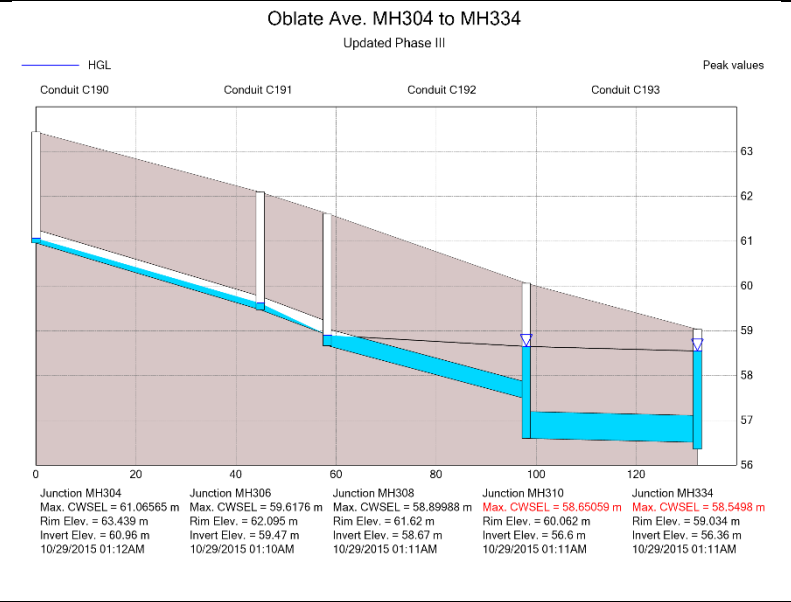


Flow rate at MH334 for Updated phase III is 763.65 L/s while it is 798.36 L/s in the approved model.

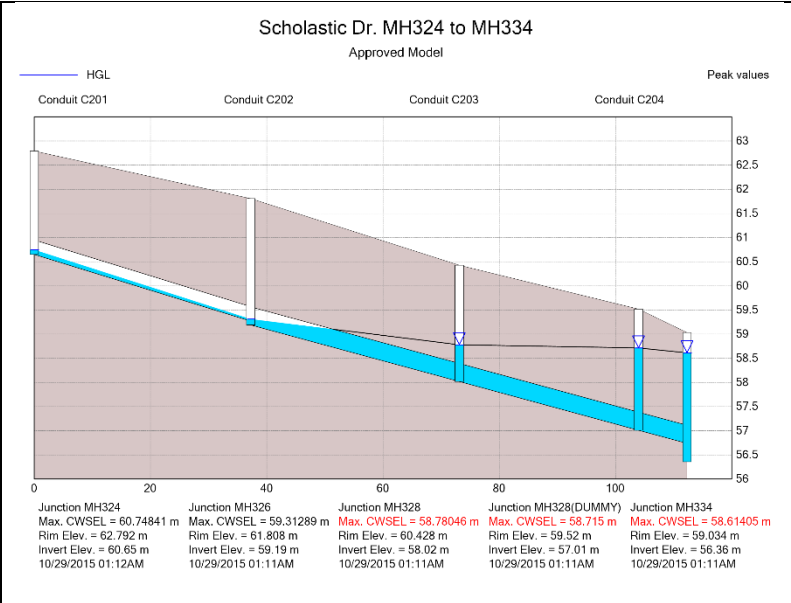
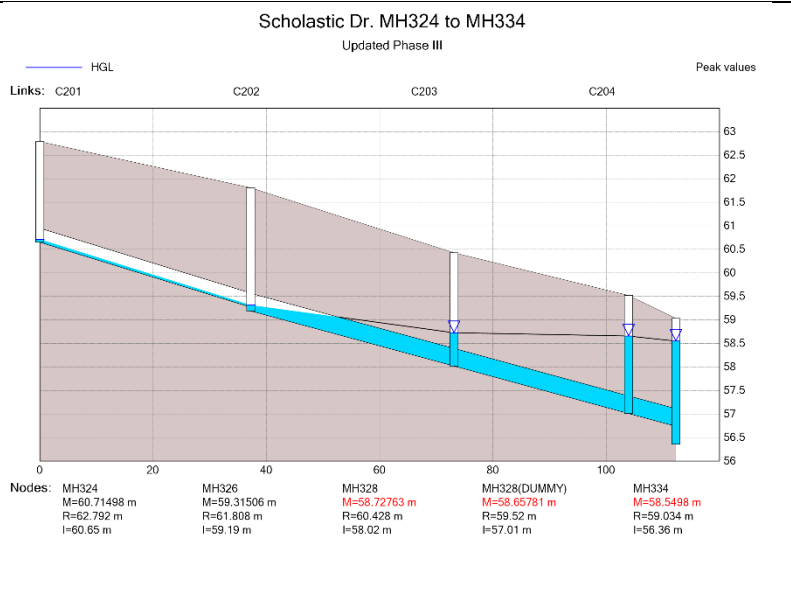
Flow rate at MH118 for Updated phase III is 1575.36 L/s while it is 1578.35 L/s in the approved model.

100-yr Profiles Minor System:

MH304 to MH334 (Oblate Avenue):

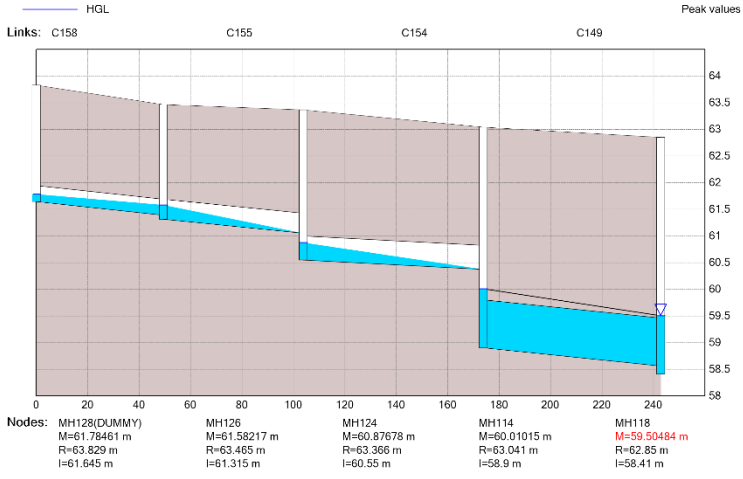


MH324 to MH334 (Scholastic Drive):

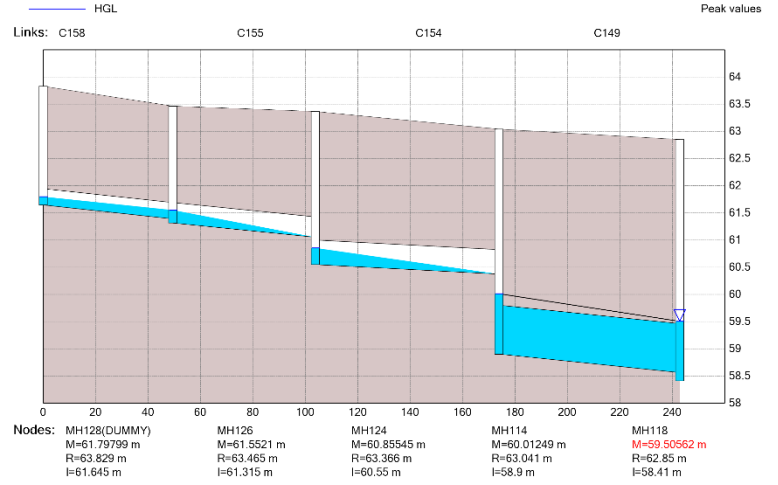


MH128 to MH118 (Deschâtelets Avenue):

Deschâtelets Ave. MH128 to MH118
Updated Phase III



Deschâtelets Ave. MH128 to MH118
Approved Model

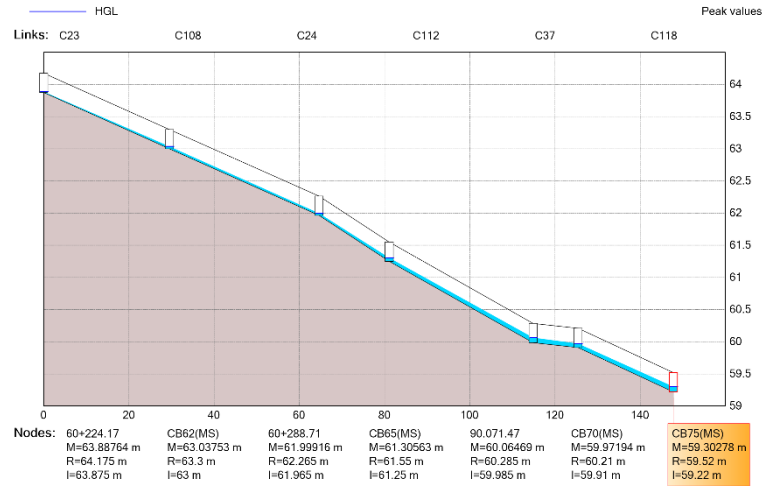


100-yr Profiles Major System:

Oblate Avenue:

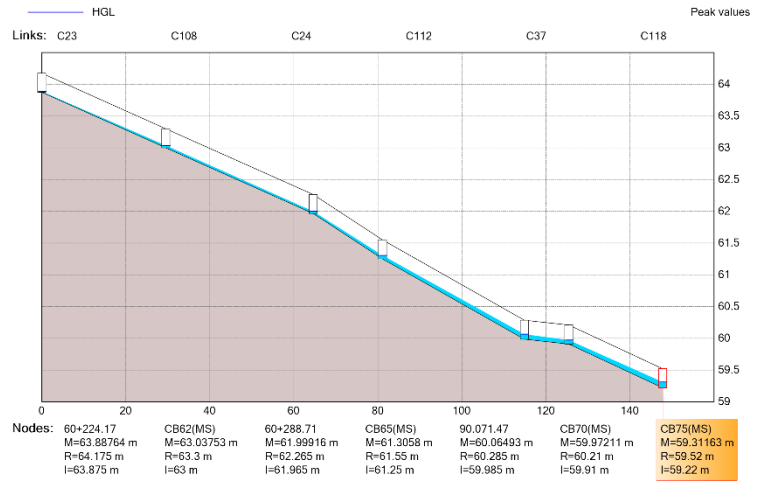
Oblate Ave. Major System 100-yr Profile

Updated Phase III



Oblate Ave. Major System 100-yr Profile

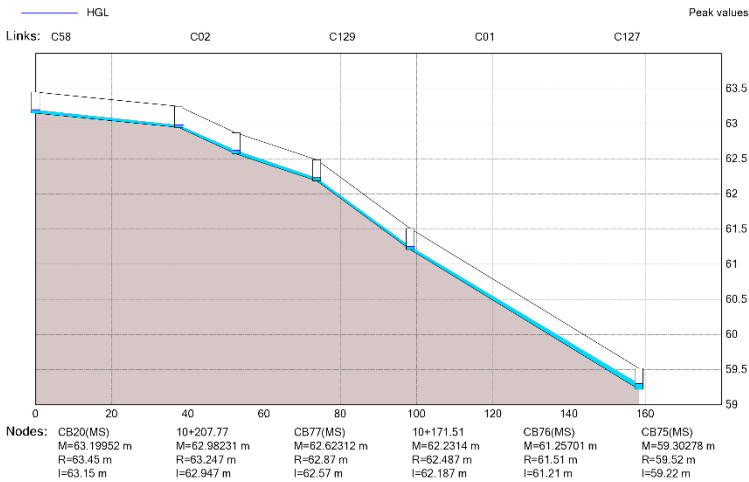
Approved Model



Scholastic Drive:

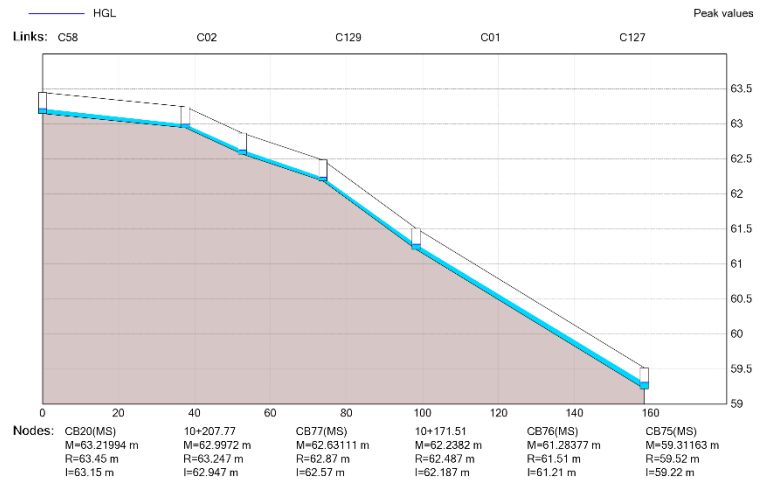
Scholastic Dr. Major System 100-yr Profile

Updated Phase III



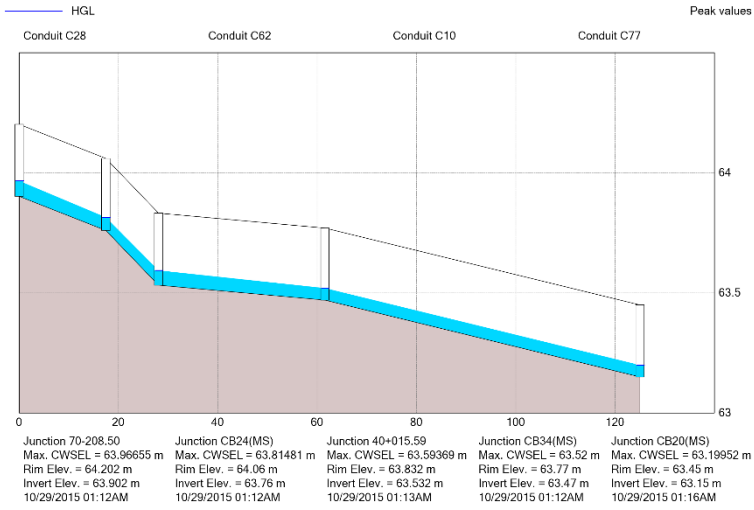
Scholastic Dr. Major System 100-yr Profile

Approved Model

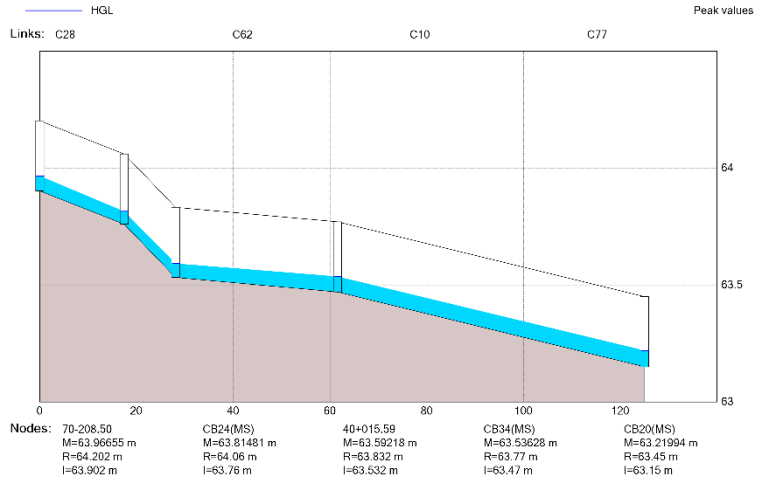


Deschâtelets Avenue:

Deschatelets Ave. Major System 100-yr Profile
Updated Phase III

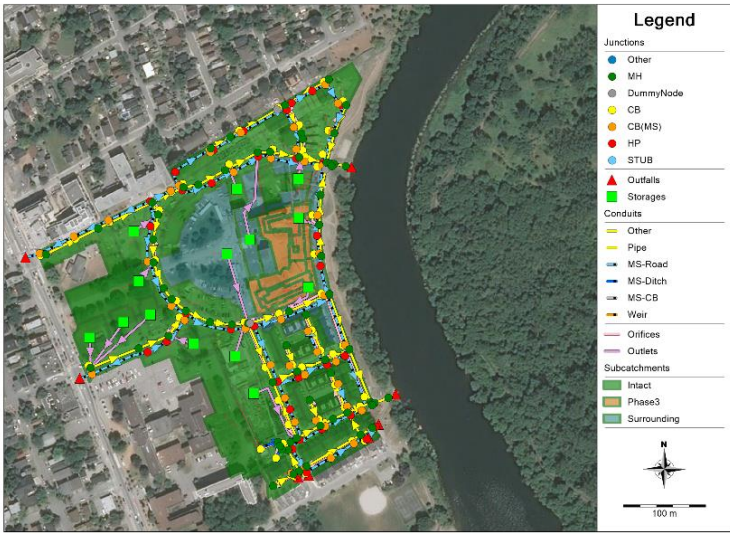


Deschatelets Ave. Major System 100-yr Profile
Approved Model



Updated vs. Approved Model Schematics:

Overall Model Schematics

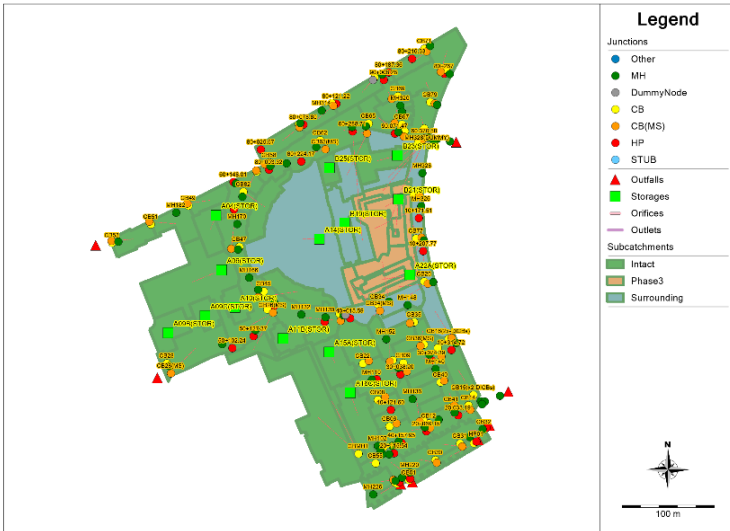


Updated Phase III

Overall Model Schematic

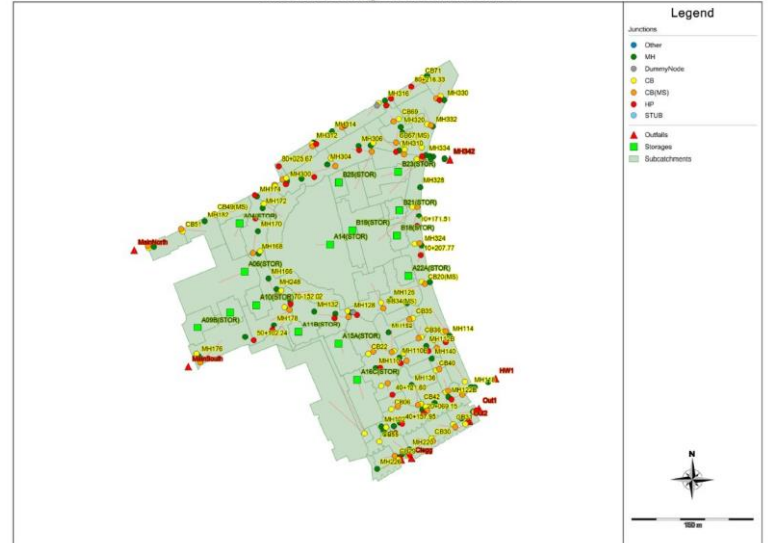


Junctions, Storage Nodes and Outfalls

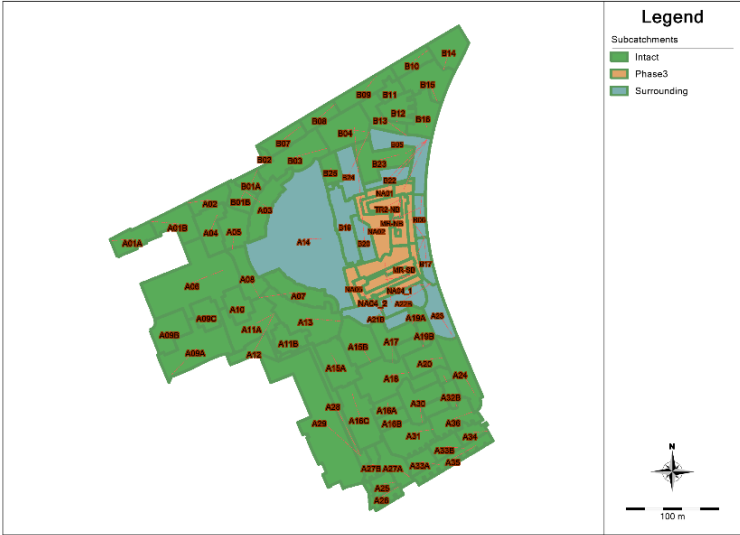


Updated Phase III

Junctions, Storage Nodes and Outfalls

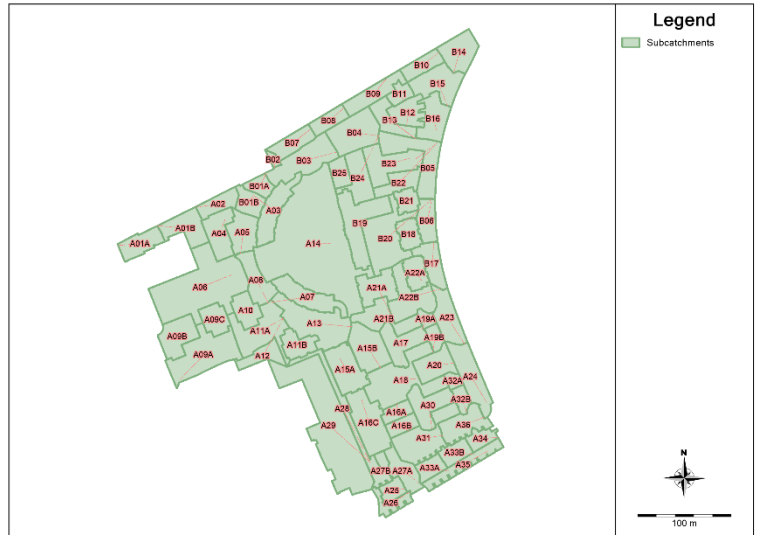


Subcatchments



Updated Phase III

Subcatchments - Approved Model



Mannings Equation Calculations - Worst Case Scenarios

Pinch Point North Building

North Side V-Bottom Ditch

Depth	m	0.2
Side slopes	1 to X	3
Top Width	m	1.2
Area	m ²	0.120
Perimeter	m	1.26
R=A/P	m	0.09
n		0.035
Slope	m/m	0.015
Q _{max}	m ³ /s	0.087
V _{max}	m/s	0.728

Courtyard - West Side North Building V-Bottom Ditch

Depth	m	0.08
Side slopes	1 to X	25
Top Width	m	4
Area	m ²	0.160
Perimeter	m	4.00
R=A/P	m	0.04
n		0.035
Slope	m/m	0.02
Q _{max}	m ³ /s	0.076
V _{max}	m/s	0.472

Post Development Runoff Coefficient "C"

Area	Surface	Ha	*C*	C _{avg}	*C ₁₀₀
Total	Hard	0.070	0.90	0.40	0.46
0.245	Soft	0.175	0.20		

Post-Development Free Flows

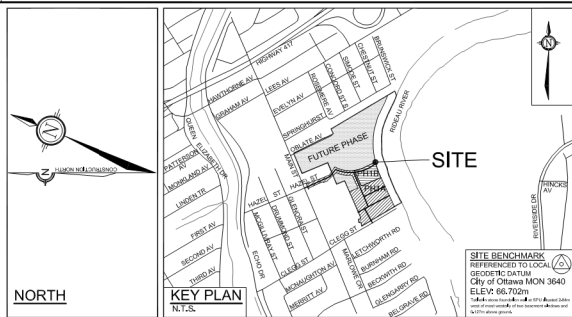
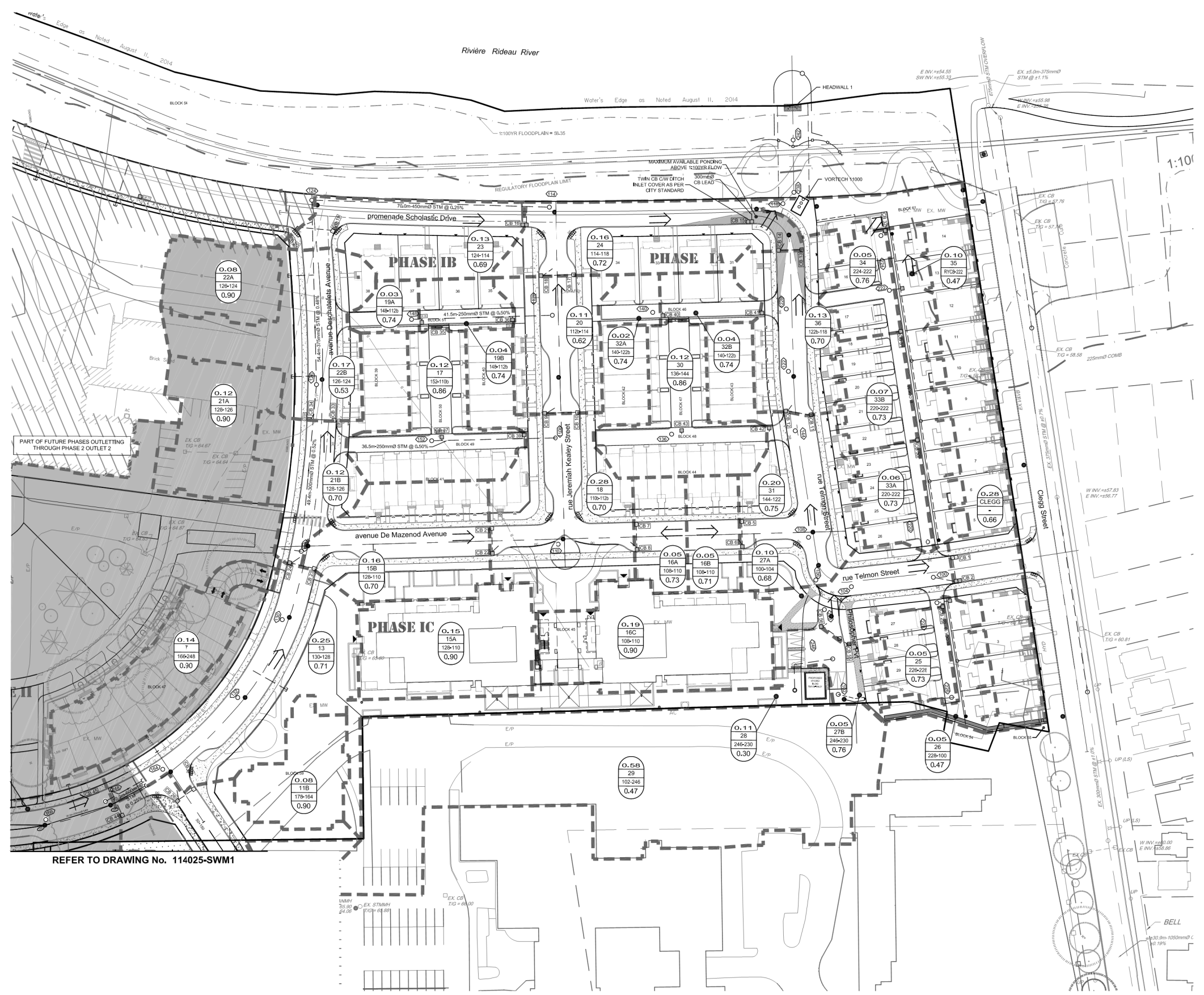
Total Emergency Overland to Scholastic Avenue

Outlet Options	Area (ha)	Q _{2 Year} (L/s)	Q _{5 Year} (L/s)	Q _{100 Year} (L/s)	Q _{100 Year +20%} (L/s)
	0.245	20.9	28.4	56.5	67.8

Time of Concentration	T _C =	10	min
Rainfall Intensity (2 Year Event)	I ₂ =	76.81	mm/hr
Rainfall Intensity (5 Year Event)	I ₅ =	104.19	mm/hr
Rainfall Intensity (10 Year Event)	I ₁₀ =	122.14	mm/hr
Rainfall Intensity (25 Year Event)	I ₂₅ =	144.69	mm/hr
Rainfall Intensity (50 Year Event)	I ₅₀ =	161.47	mm/hr
Rainfall Intensity (100 Year Event)	I ₁₀₀ =	178.56	mm/hr
Rainfall Intensity (100 Year +20% Event)	I ₁₀₀ =	214.27	mm/hr

100 year Intensity = $1735.688 / (\text{Time in min} + 6.014)^{0.820}$
 10 year Intensity = $1174.184 / (\text{Time in min} + 6.014)^{0.816}$
 5 year Intensity = $998.071 / (\text{Time in min} + 6.053)^{0.814}$
 2 year Intensity = $732.951 / (\text{Time in min} + 6.199)^{0.810}$

For 25 year storms add 10% to C value
 For 50 year storms add 20% to C value
 For 100 year storms add 25% to C value



- LEGEND**
- PHASING LIMITS
 - PROPERTY LINE
 - DRAINAGE AREA BOUNDARY
 - PROPOSED STORM MH & SEWER WITH DIRECTION OF FLOW
 - PROPOSED CATCH BASIN
 - MAJOR OVERLAND FLOW ROUTE
 - DRAINAGE AREA (hectares)
 - AREA IDENTIFICATION
 - MANHOLE TO MANHOLE
 - RUN-OFF COEFFICIENT
 - PART OF FUTURE PHASE 3 OUTLETTING THROUGH PH1A AT OUTLET 1
 - PART OF FUTURE ST. PAUL LANDS (NOT PART OF APPLICATION) OUTLETTING THROUGH PH1A AT OUTLET 1
 - PART OF FUTURE PHASES OUTLETTING THROUGH PHASE 2 OUTLET 2
 - PROPOSED CONCRETE SIDEWALK

REFER TO DRAWING No. 114025-SWM1

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY
6.	UPDATED TO REFLECT PH2 DRAINAGE AREAS	MAY 26/17	MJP
5.	ISSUED FOR CONSTRUCTION	JULY 7/16	JAG
4.	REVISED AS PER CITY COMMENTS & ISSUED FOR E.C.A.	MAY 24/16	JAG
3.	ISSUED FOR TENDER	APR 29/16	JAG
2.	REVISED AS PER CITY COMMENTS	APR 13/16	JAG
1.	ISSUED FOR CITY OF OTTAWA REVIEW	DEC 18/15	JAG

SCALE	DESIGN
1:500	SAZ
	CHECKED
	JAG
	DRAWN
	SAZ
	CHECKED
	JAG
	APPROVED
	MSP

FOR REVIEW ONLY

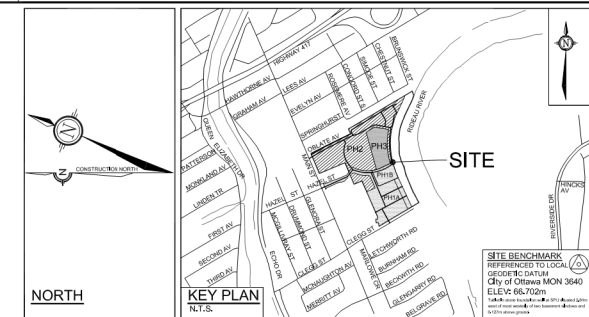
REG. PROF. ENGINEER
J.G. RIDDELL
PROVINCE OF ONTARIO

NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowland Drive
Ottawa, Ontario, Canada K2M 1P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

CITY OF OTTAWA
GREYSTONE VILLAGE
175 MAIN STREET

DRAWING NAME
**STORM DRAINAGE AREAS PLAN
PHASE 1A AND 1B**

PROJECT No.	114025-00
REV #	REV # 5
DRAWING No.	114025-STM2



- LEGEND**
- PHASING LIMITS
 - PROPERTY LINE
 - APPROVED PHASE 1A/8 DRAINAGE AREA BOUNDARY
 - PHASE 2/3 DRAINAGE AREA BOUNDARY
 - PROPOSED STORM MM & SEWER WITH DIRECTION OF FLOW
 - PROPOSED CATCH BASIN
 - MAJOR OVERLAND FLOW ROUTE
 - DRAINAGE AREA (hectares)
 - AREA IDENTIFICATION
 - MANHOLE TO MANHOLE
 - RUN-OFF COEFFICIENT
 - PART OF PH2 OUTLETTING THROUGH PH1A AT OUTLET 1
 - PHASE 1A AND 1B OUTLETTING THROUGH PH1A AT OUTLET 1
 - PROPOSED CONCRETE SIDEWALK
 - PROPOSED WATERPROOF BASEMENT LIMITS (NO WEERING TILE)

REFER TO DRAWING No. 114025-STM2

D07-16-15-0001 PHASE 2 AND 3

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

REFER TO DRAWING No. 114025-STM1

REFER TO 114025-STM1 AND STM2 FOR ADDITIONAL DRAINAGE DETAILS OUTLETTING TO PHASE 1A OUTLET 1

REVISION		DATE	BY	SCALE	DESIGN	FOR REVIEW ONLY
3.	REVISED AS PER CITY COMMENTS AND ISSUED FOR E.C.A.	MAY 26/17	JAG	1:500 0 5 10 15 20	CHECKED	SAZ
2.	RE-ISSUED AS PER CITY COMMENTS	MAR 6/17	JAG		DRAWN	JAG
1.	ISSUED FOR CITY OF OTTAWA REVIEW	NOV 21/16	JAG		CHECKED	SAZ
No.	REVISION	DATE	BY	APPROVED	JAG	MSP



NOVATECH
 Engineers, Planners & Landscape Architects
 Suite 200, 240 Michal Cowland Drive
 Ottawa, Ontario, Canada K2M 3J6
 Telephone: (613) 254-9643
 Facsimile: (613) 254-5867
 Website: www.novatech-eng.com

CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

DRAWING NAME
**STORM DRAINAGE AREAS PLAN
 PHASE 2 AND 3
 (OUTLETTING THROUGH
 STORM OUTLET 2)**

PROJECT No. 114025-00
 REV # 3
 DRAWING No. 114025-STM1-B

Appendix D
Correspondence

From: Evan Garfinkel <egarfinkel@regionalgroup.com>
Sent: Monday, August 15, 2022 12:10 PM
To: Steve Zorgel
Cc: Marc St.Pierre
Subject: FW: Greystone Phase 3

Hi Steve,

See below as discussed. Please let me know if this would be sufficient.

Also, if you have an example of a 'permission letter' that you are able to provide for reference, that would be greatly appreciated.

Best,

Evan Garfinkel
Manager, Land Development



Regional Group
1737 Woodward Drive
Ottawa, ON K2C 0P9
T: 613-230-2100 x 6004
C: 613-884-5574

egarfinkel@regionalgroup.com
www.regionalgroup.com

From: Jonathan Bruneau <brunej@ecolecatholique.ca>
Sent: Monday, August 15, 2022 10:08 AM
To: Evan Garfinkel <egarfinkel@regionalgroup.com>
Cc: Luc Poulin <pouuil@ecolecatholique.ca>; Annick Prud'homme <prudha@ecolecatholique.ca>
Subject: Re: Greystone Phase 3

External Email – Confirm Sender and Beware of Links and Attachments

Good day Evan,

Following your email, yes you are welcome to send us a draft letter for our review to address the retaining wall. I will issue it for review.

For the storm management water, I confirm that the CECCE addresses the issue to ensure water drains directly to our land sewer. The building connections to the sewer are temporary until the permanent

conduits and risers are in place. This solution works properly and no water from the storm system drains on your land behind Deschatelets.

Let me know if you require additional information, we will wait for your draft letter.

Thank you

Le ven. 12 août 2022, à 11 h 53, Evan Garfinkel <egarfinkel@regionalgroup.com> a écrit :

Hi Jonathan,

We are currently making our way through the Site Plan Control process for the Greystone Phase 3 application and we have received a few comments that apply to your lands.

Firstly, the City has asked us for a permission letter for the construction of the retaining wall, as it is likely there will be some excavation for the construction of the wall required on your side of the shared property line. The City has asked for a detailed permission letter to be signed, dated, and witnessed in order for us to re-submit for our Site Plan Control re-submission. I would be happy to provide a draft letter for you to include on CECCE letterhead if you would prefer.

Secondly, the City has asked for confirmation that your approved on-site SWM design is controlling only your on-site drainage and that you will not be sending any drainage into the phase 3 onsite storm system.

We are hopeful that you would be able to provide us with something for next week. We would be happy to set up a quick meeting early next week if you'd like to discuss further.

Best,

Evan Garfinkel

Manager, Land Development



Regional Group

1737 Woodward Drive

Ottawa, ON K2C 0P9

T: 613-230-2100 x 6004

C: 613-884-5574

egarfinkel@regionalgroup.com

www.regionalgroup.com

--

Jonathan Bruneau

Gestionnaire de projets

Conseil des écoles catholiques du Centre-Est

O. 613-744-2555 Ext:33348

M. 613-299-9242

brunej@ecolecatholique.ca

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From: Renaud, Jean-Charles <Jean-Charles.Renaud@ottawa.ca>
Sent: Thursday, August 4, 2022 1:57 PM
To: Evan Garfinkel
Cc: Jhamb, Nishant; Steve Zorgel
Subject: RE: 375 Deschâtelets Avenue - 2nd Review Comments
Attachments: D07-12-21-0111 - 375 Deschatelets Ave - RVCA Comments

Hi Evan,

While the RVCA was not circulated on the last submission, they did send the attached email in response to the initial circulation, and expressed no concern.

Have a good day.

JC

Jean-Charles Renaud, MCIP/MICU, RPP/UPC

Planner II | Urbaniste II

Development Review, Central | *Examen des projets d'aménagement, Central*

Planning, Real Estate and Economic Development Department | *Services de la planification, des biens immobiliers et du développement économique*

City of Ottawa | *Ville d'Ottawa*

110 Laurier Avenue West. Ottawa, ON | *110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1*

613.580.2424 ext./poste 27629

From: Evan Garfinkel <egarfinkel@regionalgroup.com>
Sent: August 4, 2022 1:55 PM
To: Renaud, Jean-Charles <Jean-Charles.Renaud@ottawa.ca>
Cc: Jhamb, Nishant <nishant.jhamb@ottawa.ca>; Steve Zorgel <s.zorgel@novatech-eng.com>
Subject: RE: 375 Deschâtelets Avenue - 2nd Review Comments

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Hi Jean-Charles,

Can you confirm if the RVCA was circulated on the last submission and if they had any comments?

Thanks,

Evan Garfinkel

Manager, Land Development



Regional Group
1737 Woodward Drive
Ottawa, ON K2C 0P9
T: 613-230-2100 x 6004
C: 613-884-5574

egarfinkel@regionalgroup.com
www.regionalgroup.com

From: Renaud, Jean-Charles <Jean-Charles.Renaud@ottawa.ca>
Sent: Thursday, July 7, 2022 12:59 PM
To: Evan Garfinkel <egarfinkel@regionalgroup.com>
Cc: Jhamb, Nishant <nishant.jhamb@ottawa.ca>; Dubyk, Wally <Wally.Dubyk@ottawa.ca>; Laplante, André <Andre.Laplante@ottawa.ca>; Moise, Christopher <christopher.moise@ottawa.ca>
Subject: 375 Deschâtelets Avenue - 2nd Review Comments

External Email – Confirm Sender and Beware of Links and Attachments

Good afternoon Evan,

Please find attached the second review comments for the above-noted file.

JC

Jean-Charles Renaud, MCIP/MICU, RPP/UPC

Planner II | Urbaniste II

Development Review, Central | *Examen des projets d'aménagement, Central*

Planning, Real Estate and Economic Development Department | *Services de la planification, des biens immobiliers et du développement économique*

City of Ottawa | *Ville d'Ottawa*

110 Laurier Avenue West. Ottawa, ON | *110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1*

613.580.2424 ext./poste 27629

***** Please be aware that I will be away from the office from July 11 to July 15, 2022 *****

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'

Steve Zorgel

From: Eric Lalande <eric.lalande@rvca.ca>
Sent: Wednesday, September 15, 2021 8:47 PM
To: Renaud, Jean-Charles
Subject: D07-12-21-0111 - 375 Deschatelets Ave - RVCA Comments

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Hi J-C,

The RVCA has reviewed the above noted Site Plan Control Application and have no objections.

Thank you,

Eric Lalande, MCIP, RPP
Planner | x1137



3889 Rideau Valley Drive
PO Box 599, Manotick ON K4M 1A5
T 613-692-3571 | 1-800-267-3504 F 613-692-0831 | www.rvca.ca

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Steve Zorgel

From: Scott Dennis <SDennis@patersongroup.ca>
Sent: Wednesday, August 10, 2022 9:37 AM
To: Steve Zorgel
Cc: Evan Garfinkel; Kevin Pickard
Subject: RE: PG5383: Grading Plan - Greystone PH.3 Condos
Attachments: Paterson Group Report PG5383-1 Rev. 1 dated March 9, 2022.pdf

Follow Up Flag: Follow up
Flag Status: Flagged

Steve,

Since we're waterproofing the foundation, long-term groundwater flows will be very low. Section 6.5 of the attached Geotechnical Investigation Report indicates the following:

Provided the proposed groundwater infiltration control system and the tanked system are properly implemented and approved by the geotechnical consultant at the time of construction, it is expected that groundwater flow will be very low to negligible (less than 2,000 L/day).

Hopefully this answers the City's question, but if they have follow up questions, please let us know.

Regards,



SCOTT DENNIS, P.Eng., ing.
Senior Project Manager – Geotechnical
TEL: (613) 226-7381 ext. 332

9 AURIGA DRIVE
OTTAWA ON K2E 7T9
patersongroup.ca

OUR DIRECT LINE FOR MATERIALS TESTING INSPECTION BOOKING HAS BEEN UPDATED,
PLEASE CALL **613-696-9677** TO BOOK AN INSPECTION.

From: Steve Zorgel <s.zorgel@novatech-eng.com>
Sent: August 10, 2022 8:41 AM
To: Scott Dennis <SDennis@patersongroup.ca>
Subject: RE: PG5383: Grading Plan - Greystone PH.3 Condos

Hi Scott,

We received a comment from the City regarding groundwater infiltration for the phase 3 condo site within Greystone Village. Comment as follows:

7. How much ground water is anticipated to drain into storm sewer post construction, please discuss in the body of the report.

Are you able to provide a response to this that we can add to our servicing report.



ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 4082-AAZQ6P

Issue Date: June 24, 2016

Greystone Village Inc.
1737 Woodward Drive, 2nd Floor
Ottawa, Ontario
K2C 0P9

Site Location: 175 Main Street
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

sanitary and storm sewers to be constructed in the City of Ottawa, as follows:

- sanitary sewers on Hazel Street (from Station 50+0000 to Station 50+175), Deschatelets Avenue (from Station 70+125 to Station 70+335), Scholastic Drive (from Station 10+225 to Station 10+392), Jeremiah Kealey Street (from Station 30+000 to Station 30+108), De Mazenod Avenue (from Station 40+000 to Station 40+168), Telmon Street (from Station 20+000 to Station 20+189), Clegg Street (from Station 90+000 to Station 90+179), and Easement (Block 61) (from Station 10+392 to Station 10+435); and
- storm sewers on Hazel Street (from Station 50+0000 to Station 50+175), Deschatelets Avenue (from Station 70+125 to Station 70+335), Scholastic Drive (from Station 10+225 to Station 10+392), Jeremiah Kealey Street (from Station 30+000 to Station 30+108), De Mazenod Avenue (from Station 40+000 to Station 40+168), and Telmon Street (from Station 20+000 to Station 20+189);

all in accordance with the application from Greystone Village Inc., dated May 18, 2016, including final plans and specifications prepared by Novatech Engineering.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The environmental compliance approval number;
6. The date of the environmental compliance approval;
7. The name of the Director, and;
8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

CONTENT COPY OF ORIGINAL

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5

AND

The Director appointed for the purposes of Part II.1 of
the Environmental Protection Act
Ministry of the Environment and Climate Change
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 24th day of June, 2016

Gregory Zimmer, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

YH/

c: District Manager, MOECC Ottawa District Office
M. Rick O'Connor, City Clerk, City of Ottawa
Joshua White, P.Eng., Project Manager, Development Review, City of Ottawa
Linda Carkner, Program Manager, Infrastructure, City of Ottawa
J.G. Riddell, P.Eng., Novatech Engineering

AMENDED ENVIRONMENTAL COMPLIANCE APPROVALNUMBER 8946-ACUP7W
Issue Date: August 17, 2016Greystone Village Inc.
1737 Woodward Drive, Unit. 2
Ottawa, Ontario
K2C 0P9Site Location: 175 Main Street
Lot H, Concession D
City of Ottawa,

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

an amendment of stormwater management Works for the Phase I of Greystone Village subdivision development, located on the north side of Clegg Street, south side of Springhurst Avenue, between Main Street and Rideau River within the Rideau watershed, in the City of Ottawa, for the collection, treatment and disposal of stormwater run-off, to add stormwater management facilities, to service approximately 7.48 hectares, discharging to Rideau River, providing Enhanced Level of quality control and erosion protection, consisting of the following:

Proposed Works:

oil and grit separator (catchment area 7.48 hectares): - one (1) oil and grit separator (Vortechs 11000 or Equivalent), located at the intersection of Telmon Street and Scholastic Drive, west side of Rideau River, receiving inflows from the storm sewers of the subdivision development, identified below, having a sediment storage capacity of approximately 4.280 m³, an oil storage capacity of approximately 2,378 L, a total storage volume of approximately 13.592 m³, and a maximum treatment flow rate of approximately 495 L/s, discharging via a 600 mm diameter outflow pipe to the storm sewer outfall, identified below;

storm sewer outfall (Outlet#1-catchment area 7.48 hectares): - one (1) 825 mm diameter storm sewer outfall with a concrete headwall and rip-rap protection, receiving inflows from the oil and grit separator, identified above, discharging to the Rideau River;

Previous Works:

sanitary sewers on Hazel Street (from Station 50+0000 to Station 50+175), Deschatelets Avenue (from Station 70+125 to Station 70+335), Scholastic Drive (from Station 10+225 to Station 10+392), Jeremiah Kealey Street (from Station 30+000 to Station 30+108), De Mazenod Avenue (from Station 40+000 to Station 40+168), Telmon Street (from Station 20+000 to Station 20+189), Clegg Street (from Station 90+000 to Station 90+179), and Easement (Block 61) (from Station 10+392 to Station 10+435); and

storm sewers on Hazel Street (from Station 50+0000 to Station 50+175), Deschatelets Avenue (from Station 70+125 to Station 70+335), Scholastic Drive (from Station 10+225 to Station 10+392), Jeremiah Kealey Street (from Station 30+000 to Station 30+108), De Mazenod Avenue (from Station 40+000 to Station 40+168), and Telmon Street (from Station 20+000 to Station 20+189);

including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned Works;

all in accordance with the submitted supporting documents listed in Schedule "A" forming part of this Approval.

For the purpose of this environmental compliance approval, the following definitions apply:

"Approval" means this entire document including the application and any supporting documents listed in any schedules in this Approval;

"Director" means a person appointed by the Minister pursuant to section 5 of the Environmental Protection Act for the purposes of Part II.1 of the Environmental Protection Act;

"Equivalent" means a substituted product that meets the required quality and performance standards of a named product;

"Ministry" means the ministry of the government of Ontario responsible for the Environmental Protection Act and the Ontario Water Resources Act and includes all officials, employees or other persons acting on its behalf;

"Owner" means the Greystone Village Inc., and includes their successors and assignees;

"Previous Works" means those portions of the sewage Works previously approved under an Approval;

"Works" means the sewage works described in the Owner's application(s) and this Approval.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL PROVISIONS

(1) The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the Conditions herein and shall take all reasonable measures to ensure any such person complies with the same.

(2) The designation of the City of Ottawa as the operating authority of the site on the application for approval of the Works does not relieve the owner from the responsibility of complying with any and all of the this approval.

(3) Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application for approval of the Works.

(4) Where there is a conflict between a provision of any submitted document referred to in this Approval and the Conditions of this Approval, the Conditions in this Approval shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.

(5) Where there is a conflict between the listed submitted documents, and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.

(6) The Conditions of this Approval are severable. If any Condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such Condition to other circumstances and the remainder of this Approval shall not be affected thereby.

(7) The issuance of, and compliance with the Conditions of this Approval does not:

(a) relieve any person of any obligation to comply with any provision of any applicable statute, regulation or other legal requirement, including, but not limited to, the obligation to obtain approval from the local conservation authority necessary to construct or operate the sewage Works;
or

(b) limit in any way the authority of the Ministry to require certain steps be taken to require the Owner to furnish any further information related to compliance with this Approval.

2. EXPIRY OF APPROVAL

(1) This Approval will cease to apply to those parts of the Works which have not been constructed within **five (5) years** of the date of this Approval.

3. CHANGE OF OWNER

(1) The Owner shall notify the Director, in writing, of any of the following changes within **thirty (30) days** of the change occurring:

(a) change of Owner;

(b) change of address of the Owner;

(c) change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the Business Names Act, R.S.O. 1990, c. B17 shall be included in the notification to the Director;

(d) change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the Corporations Information Act, R.S.O. 1990, c. C39 shall be included in the notification to the Director.

4. OPERATION AND MAINTENANCE

(1) The Owner shall inspect the Works at least **once a year** and, if necessary, clean and maintain the Works to prevent the excessive build-up of sediments and/or vegetation.

(2) The Owner shall maintain a record of the results of these inspections and any cleaning and maintenance operations undertaken, and shall make the record available for inspection by the Ministry. The record shall include the following:

(a) the name of the Works; and

(b) the date and results of each inspection, maintenance and cleaning, including an estimate of the quantity of any materials removed.

5. MONITORING AND REPORTING

(1) The Owner shall carry out a monitoring program for the inspection and maintenance of the Works as outline in this Approval and shall make the information available to the Ministry staff upon request. The monitoring program shall consist of annul maintenance logs listing the depth of sediment in the oil and grit separator and shall note the date of each inspection, maintenance and cleaning including an estimate of the quantity of materials removed, and maintenance operations undertaken.

6. TEMPORARY EROSION AND SEDIMENT CONTROL

(1) The Owner shall install and maintain temporary sediment and erosion control measures during construction and conduct inspections once every **two (2) weeks** and after each significant storm event (a significant storm event is defined as a minimum of 25 mm of rain in any 24 hours period). The inspections and maintenance of the temporary sediment and erosion control

measures shall continue until they are no longer required and at which time they shall be removed and all disturbed areas reinstated properly.

(2) The Owner shall maintain records of inspections and maintenance which shall be made available for inspection by the Ministry, upon request. The record shall include the name of the inspector, date of inspection, and the remedial measures, if any, undertaken to maintain the temporary sediment and erosion control measures.

7. RECORD KEEPING

The Owner shall retain for a minimum of **five (5) years** from the date of their creation, all records and information related to or resulting from the operation, maintenance and monitoring activities required by this Approval.

Schedule "A"

1. Application for Environmental Compliance Approval, dated March 9, 2016, received on March 31, 2016, submitted by Novatech;
2. Site Servicing, Stormwater Management, Noise Erosion and Sediment Control Brief, for Greystone Village 175 Main Street, Ottawa, Ontario, dated December 18, 2015, prepared by Novatech;
3. Pipe Data Form and Storm and Sanitary Sewer Design Sheets, prepared by Novatech;
4. Set of Engineering Drawings (8 drawings) for Greystone Village Phase 1A & 1B , City of Ottawa, dated December, 2015, prepared by Novatech;
5. E-mail from Justin Gauthier of Novatech to the Ministry, dated August 15, 2016; and
6. E-mail from Justin Gauthier of Novatech to the Ministry, dated August 16, 2016.

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the Works are built and operated in the manner in which they were described for review and upon which approval was granted. This Condition is also included to emphasize the precedence of Conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
2. Condition 2 is included to ensure that, when the Works are constructed, the Works will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to approved Works and to ensure that any subsequent Owner of the Works is made aware of the Approval and continue to operate the Works in compliance with it.
4. Condition 4 is included to require that the Works be properly operated and maintained such that the environment is protected.
5. Condition 5 is included to enable the Owner to evaluate and demonstrate the performance of the Works on a continual basis, so that the Works are properly operated and maintained at a level which is consistent with the design objectives specified in the Approval and that the Works do not cause any impairment of the receiving watercourse.
6. Condition 6 is included as installation, regular inspection and maintenance of the temporary sediment and erosion control measures is required to mitigate the impact on the downstream receiving watercourse during construction, until they are no longer required.
7. Condition 7 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the Works.

Upon issuance of the environmental compliance approval, I hereby revoke Approval No(s). 4082-AAZQ6P issued on June 24, 2016.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me, the Environmental Review Tribunal and in accordance with Section 47 of the Environmental Bill of Rights, 1993, S.O. 1993, c. 28 (Environmental Bill of Rights), the Environmental Commissioner, within 15 days after receipt of this Notice, require a hearing by the Tribunal. The Environmental Commissioner will place notice of your appeal on the Environmental Registry. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in

- respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

Pursuant to subsection 139(3) of the Environmental Protection Act, a hearing may not be required with respect to any terms and conditions in this environmental compliance approval, if the terms and conditions are substantially the same as those contained in an approval that is amended or revoked by this environmental compliance approval.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The environmental compliance approval number;
6. The date of the environmental compliance approval;
7. The name of the Director, and;
8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5

AND

The Environmental Commissioner
1075 Bay Street, Suite 605
Toronto, Ontario
M5S 2B1

AND

The Director appointed for the purposes of
Part II.1 of the Environmental Protection Act
Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca**

This instrument is subject to Section 38 of the Environmental Bill of Rights, 1993, that allows residents of Ontario to seek leave to appeal the decision on this instrument. Residents of Ontario may seek leave to appeal within 15 days from the date this decision is placed on the Environmental Registry. By accessing the Environmental Registry at www.ebr.gov.on.ca, you can determine when the leave to appeal period ends.

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 17th day of August, 2016



Gregory Zimmer, P.Eng.

Director

appointed for the purposes of Part II.1 of the
Environmental Protection Act

TN/

c: District Manager, MOECC Ottawa Office
M. Rick O'Connor, City Clerk, City of Ottawa
Joshua White, P.Eng., Project Manager, Development Review, City of Ottawa
Linda Carkner, Program Manager, Infrastructure, City of Ottawa
J.G. Riddell, Novatech Engineering
Justin Gauthier, Novatech Engineering

ENVIRONMENTAL COMPLIANCE APPROVALNUMBER 3454-APEHFQ
Issue Date: July 31, 2017

Greystone Village Inc.
1737 Woodward Drive, 2nd Floor
Ottawa, Ontario
K2C 0P9

Site Location: Greystone Village Phase 2 and 3
175 Main Street
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

storm sewers and an associated **stormwater outfall** to be constructed in the City of Ottawa on Block 58, from Station (0+024.35) to Station (0+056.7), and discharging to the Rideau River;

one (1) oil/grit separator (catchment area - 2.7 hectares): - the establishment of an off-line oil/grit separator (model stormceptor 5000 or Equivalent) in the City of Ottawa, for the treatment and disposal of stormwater run-off for all storm events up to and including the 100-year storm event, to provide Enhanced Level water quality protection for a total catchment area of approximately 2.7 hectares, having a sediment storage capacity of 20,940 litres, an oil storage capacity of 3,360 litres, a total holding capacity of 24,710 litres, and a maximum treatment flow rate of 61 litres/second, discharging to Rideau River;

including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned Works;

all in accordance with the submitted supporting documents listed in Schedule "A" forming part of this Approval.

For the purpose of this environmental compliance approval, the following definitions apply:

1. "Approval " means this entire document and any schedules attached to it, and the application;
2. "Director " means a person appointed by the Minister pursuant to section 5 of the *EPA* for the

purposes of Part II.1 of the *EPA*;

3. "*District Manager* " means the *District Manager* of the appropriate local District Office of the *Ministry* , where the *Works* are geographically located;
4. "*EPA* " means the Environmental Protection Act, R.S.O. 1990, c.E.19, as amended;
5. "*Equivalent* " means a substituted oil and grit separator that meets the required quality and performance standards of the approved oil and grit separator;
6. "*Ministry* " means the ministry of the government of Ontario responsible for the *EPA* and *OWRA* and includes all officials, employees or other persons acting on its behalf;
7. "*Owner* " means Greystone Village Inc., and includes its successors and assignees;
8. "*OWRA* " means the Ontario Water Resources Act, R.S.O. 1990, c. O.40, as amended;
9. "*Water Supervisor* " means the *Water Supervisor* of the appropriate local office of the Safe Drinking Water Branch of the *Ministry*, where the *Works* are geographically located;
10. "*Works* " means the sewage works described in the *Owner's* application, and this *Approval*.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL CONDITIONS

1. The *Owner* shall ensure that any person authorized to carry out work on or operate any aspect of the *Works* is notified of this *Approval* and the conditions herein and shall take all reasonable measures to ensure any such person complies with the same.
2. Except as otherwise provided by these Conditions, the *Owner* shall design, build, install, operate and maintain the *Works* in accordance with the description given in this *Approval*, and the application for approval of the *Works*.
3. Where there is a conflict between a provision of any document in the schedule referred to in this *Approval* and the conditions of this *Approval*, the conditions in this *Approval* shall take precedence, and where there is a conflict between the documents in the schedule, the document bearing the most recent date shall prevail.
4. Where there is a conflict between the documents listed in Schedule 'A' and the application, the

application shall take precedence unless it is clear that the purpose of the document was to amend the application.

5. The conditions of this *Approval* are severable. If any condition of this *Approval*, or the application of any requirement of this *Approval* to any circumstance, is held invalid or unenforceable, the application of such condition to other circumstances and the remainder of this *Approval* shall not be affected thereby.
6. The issuance of, and compliance with the conditions of, this *Approval* does not:
 - a. relieve any person of any obligation to comply with any provision of any applicable statute, regulation or other legal requirement, including, but not limited to, the obligation to obtain approval from the local conservation authority/MNR necessary to construct or operate the sewage works; or
 - b. limit in any way the authority of the *Ministry* to require certain steps be taken to require the *Owner* to furnish any further information related to compliance with this *Approval*.

2. EXPIRY OF APPROVAL

1. This *Approval* will cease to apply to those parts of the *Work* which have not been constructed within five (5) years of the date of this *Approval*.
2. In the event that completion and commissioning of any portion of the *Works* is anticipated to be delayed beyond the specified expiry period, the *Owner* shall submit an application of extension to the expiry period, at least twelve (12) months prior to the end of the period. The application for extension shall include the reason(s) for the delay, whether there is any design change(s) and a review of whether the standards applicable at the time of *Approval* of the *Works* are still applicable at the time of request for extension, to ensure the ongoing protection of the environment.

3. CHANGE OF OWNER

1. The *Owner* shall notify the District Manager and the *Director*, in writing, of any of the following changes within thirty (30) days of the change occurring:
 - a. change of *Owner*;
 - b. change of address of the *Owner*;
 - c. change of partners where the *Owner* is or at any time becomes a partnership, and a copy of the most recent declaration filed under the Business Names Act, R.S.O. 1990, c.B17 shall be included in the notification to the District Manager; or
 - d. change of name of the corporation where the *Owner* is or at any time becomes a corporation, and a copy of the most current information filed under the Corporations Information Act,

R.S.O. 1990, c. C39 shall be included in the notification to the District Manager.

2. In the event of any change in ownership of the *Works* , other than a change to a successor municipality, the *Owner* shall notify in writing the succeeding owner of the existence of this *Approval* , and a copy of such notice shall be forwarded to the District Manager and the *Director*.
3. The *Owner* shall ensure that all communications made pursuant to this condition refer to the number at the top of this *Approval*.
4. Notwithstanding any other requirements in this *Approval* , upon transfer of the ownership or assumption of the *Works* to a municipality if applicable, any reference to the *District Manager* shall be replaced with the *Water Supervisor*.

4. OPERATION AND MAINTENANCE

1. If applicable, any proposed storm sewers or other stormwater conveyance in this *Approval* can be constructed but not operated until the proposed stormwater management facilities in this *Approval* or any other *Approval* that are designed to service the storm sewers or other stormwater conveyance are in operation.
2. The *Owner* shall make all necessary investigations, take all necessary steps and obtain all necessary approvals so as to ensure that the physical structure, siting and operations of the *Works* do not constitute a safety or health hazard to the general public.
3. The *Owner* shall undertake an inspection of the condition of the *Works*, at least once a year, and undertake any necessary cleaning and maintenance to ensure that sediment, debris and excessive decaying vegetation are removed from the *Works* to prevent the excessive build-up of sediment, oil/grit, debris and/or decaying vegetation, to avoid reduction of the capacity and/or permeability of the *Works*, as applicable. The *Owner* shall also regularly inspect and clean out the inlet to and outlet from the *Works* to ensure that these are not obstructed.
4. The *Owner* shall design, construct and operate the *Works* with the objective that the effluent from the *Works* is essentially free of floating and settleable solids and does not contain oil or any other substance in amounts sufficient to create a visible film, sheen, foam or discoloration on the receiving waters.
5. The *Owner* shall maintain a logbook to record the results of these inspections and any cleaning and maintenance operations undertaken, and shall keep the logbook at the *Owner's* administration office for inspection by the *Ministry*. The logbook shall include the following:
 - a. the name of the *Works*; and
 - b. the date and results of each inspection, maintenance and cleaning, including an estimate of the quantity of any materials removed and method of clean-out of the *Works*.

6. The *Owner* shall prepare an operations manual prior to the commencement of operation of the *Works* that includes, but is not necessarily limited to, the following information:
 - a. operating and maintenance procedures for routine operation of the *Works*;
 - b. inspection programs, including frequency of inspection, for the *Works* and the methods or tests employed to detect when maintenance is necessary;
 - c. repair and maintenance programs, including the frequency of repair and maintenance for the *Works*;
 - d. contingency plans and procedures for dealing with potential spills and any other abnormal situations and for notifying the District Manager; and
 - e. procedures for receiving, responding and recording public complaints, including recording any follow-up actions taken.
7. The *Owner* shall maintain the operations manual current and retain a copy at the location of the *Works* for the operational life of the *Works*. Upon request, the *Owner* shall make the manual available to *Ministry* staff.

5. TEMPORARY EROSION AND SEDIMENT CONTROL

1. The *Owner* shall install and maintain temporary sediment and erosion control measures during construction and conduct inspections once every two (2) weeks and after each significant storm event (a significant storm event is defined as a minimum of 25 mm of rain in any 24 hours period). The inspections and maintenance of the temporary sediment and erosion control measures shall continue until they are no longer required and at which time they shall be removed and all disturbed areas reinstated properly.
2. The *Owner* shall maintain records of inspections and maintenance which shall be made available for inspection by the *Ministry*, upon request. The record shall include the name of the inspector, date of inspection, and the remedial measures, if any, undertaken to maintain the temporary sediment and erosion control measures.

6. REPORTING

1. One (1) week prior to the start-up of the operation of the *Works*, the *Owner* shall notify the District Manager (in writing) of the pending start-up date.
2. The *Owner* shall, upon request, make all manuals, plans, records, data, procedures and supporting documentation available to *Ministry* staff.
3. The *Owner* shall prepare and submit a performance report to the District Manager on an annual basis, within ninety (90) days following the end of the period being reported upon. The first such report shall cover the first annual period following the commencement of operation of the *Works* and subsequent reports shall be submitted to cover successive annual periods following

thereafter. The reports shall contain, but shall not be limited to, the following information:

- a. a description of any operating problems encountered and corrective actions taken;
- b. a summary of all maintenance carried out on any major structure, equipment, apparatus, mechanism or thing forming part of the *Works*, including an estimate of the quantity of any materials removed from the *Works*;
- c. a summary of any complaints received during the reporting period and any steps taken to address the complaints;
- d. a summary of all spill or abnormal discharge events; and
- e. any other information the District Manager requires from time to time.

Schedule "A"

1. Application for Environmental Compliance Approval under M&P Sewage Works, dated May 15, 2017 and received on June 29, 2017, submitted by The Greystone Village Inc.;
2. Greystone Village Phase 2 and 3, 175 Main Street, Plan and Profile, Storm Outlet 2 (including Grading, Erosion and Sediment Control) Revision 4, dated May 26, 2017, prepared by Novatech Engineering;
3. Greystone Village Phase 2 and 3, 175 Main Street, Site Servicing, stormwater management, Noise, Erosion & Sediment Control design beirf, revised May 26, 2017, prepared by Novatech Engineering;

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the *Works* are constructed and operated in the manner in which they were described and upon which approval was granted. This condition is also included to emphasize the precedence of conditions in the *Approval* and the practice that the *Approval* is based on the most current document, if several conflicting documents are submitted for review. Condition 1.6 is included to emphasize that the issuance of this *Approval* does not diminish any other statutory and regulatory obligations to which the *Owner* is subject in the construction, maintenance and operation of the *Works*. The Condition specifically highlights the need to obtain any necessary conservation authority approvals. The Condition also emphasizes the fact that this *Approval* doesn't limit the authority of the *Ministry* to require further information.
2. Condition 2 is included to ensure that, when the *Works* are constructed, the *Works* will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
3. Condition 3 is included to ensure that the *Ministry* records are kept accurate and current with respect to approved *Works* and to ensure that subsequent owners of the *Works* are made aware of the *Approval* and continue to operate the *Works* in compliance with it.
4. Condition 4 is included as regular inspection and necessary removal of sediment and excessive decaying vegetation from the *Works* are required to mitigate the impact of sediment, debris and/or decaying vegetation on the treatment capacity of the *Works*. The Condition also ensures that adequate storage is maintained in the *Works* at all times as required by the design. Furthermore, this Condition is included to ensure that the *Works* are operated and maintained to function as designed. Condition 4.1 is included to prevent the operation of stormwater pipes and other conveyance until such time that their required associated stormwater management *Works* are also constructed.
5. Condition 5 is included as installation, regular inspection and maintenance of the temporary sediment and erosion control measures is required to mitigate the impact on the downstream receiving watercourse during construction until they are no longer required.
6. Condition 6 is included to provide a performance record for future references, to ensure that the *Ministry* is made aware of problems as they arise, and to provide a compliance record for all the terms and conditions outlined in this *Approval*, so that the *Ministry* can work with the *Owner* in resolving any problems in a timely manner.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

- a. The portions of the environmental compliance approval or each term or condition in the environmental compliance

- approval in respect of which the hearing is required, and;
- b. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

1. The name of the appellant;
2. The address of the appellant;
3. The environmental compliance approval number;
4. The date of the environmental compliance approval;
5. The name of the Director, and;
6. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5

AND

The Director appointed for the purposes of
Part II.1 of the Environmental Protection Act
Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

*** Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca**

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 31st day of July, 2017



Christina Labarge, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

MS/

- c: District Manager, MOECC Ottawa office
Justin Gauthier, Project Manager, Novatech Engineering
City Clerk, City of Ottawa (File No. D07-16-15-0001)
Joshua White, P.Eng., Senior Engineer, Development Review, City of Ottawa
Linda Carkner, Program Manager, Row Unit, City of Ottawa

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 0292-AP6PWR

Issue Date: July 12, 2017

Greystone Village Inc.
1737 Woodward Drive, Unit 2
Ottawa, Ontario
K2C 0P9

Site Location: Greystone Village, Phase 2 and 3
175 Main Street
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

storm and sanitary sewers to be constructed in the City of Ottawa, as follows:

- sanitary sewers on Oblates Avenue (from Station 60+007.53 to Station 60+373.35), Scholastics Drive (from Station 10+0075 to Station 10+195.89), Deschatelets Avenue (from Station 70+000 to Station 70+132), and Block 58 (from Station 0+002 to Station 0+048.5); and
- storm sewers on Oblates Avenue (from Station 60+007.53 to Station 60+373.35), Scholastics Drive (from Station 10+0075 to Station 10+195.89), and Deschatelets Avenue (from Station 70+000 to Station 70+132);

all in accordance with the submitted application and supporting documents listed in Schedule "A" forming part of this Approval.

For the purpose of this environmental compliance approval, the following definitions apply:

1. "Approval " means this entire document and any schedules attached to it, and the application;
2. "Director " means a person appointed by the Minister pursuant to section 5 of the EPA for the purposes of Part II.1 of the EPA;
3. "District Manager " means the District Manager of the appropriate local District Office of the Ministry, where the Works are geographically located;

4. "*EPA* " means the Environmental Protection Act, R.S.O. 1990, c.E.19, as amended;
5. "*Ministry* " means the ministry of the government of Ontario responsible for the EPA and OWRA and includes all officials, employees or other persons acting on its behalf;
6. "*Owner* " means Greystone Village Inc., and includes their successors and assignees;
7. "*OWRA* " means the Ontario Water Resources Act, R.S.O. 1990, c. O.40, as amended;
8. "*Works* " means the sewage works described in the Owner's application, and this Approval;
9. "*Professional Engineer* " means a person entitled to practice as a Professional Engineer in the Province of Ontario under a licence issued under the *Professional Engineers Act*.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL CONDITIONS

1. The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the conditions herein and shall take all reasonable measures to ensure any such person complies with the same.
2. Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application for approval of the Works.
3. Where there is a conflict between a provision of any document in the schedule referred to in this Approval and the conditions of this Approval, the conditions in this Approval shall take precedence, and where there is a conflict between the documents in the schedule, the document bearing the most recent date shall prevail.
4. Where there is a conflict between the documents listed in Schedule 'A' and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.
5. The conditions of this Approval are severable. If any condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such condition to other circumstances and the remainder of this Approval shall not be affected thereby.

6. The issuance of, and compliance with the conditions of, this Approval does not:
 - a. relieve any person of any obligation to comply with any provision of any applicable statute, regulation or other legal requirement, including, but not limited to, the obligation to obtain approval from the local conservation authority/MNR necessary to construct or operate the sewage works; or
 - b. limit in any way the authority of the Ministry to require certain steps be taken to require the Owner to furnish any further information related to compliance with this Approval.

2. EXPIRY OF APPROVAL

1. This Approval will cease to apply to those parts of the Work which have not been constructed within five (5) years of the date of this Approval.
2. In the event that completion and commissioning of any portion of the Works is anticipated to be delayed beyond the specified expiry period, the Owner shall submit an application of extension to the expiry period, at least twelve (12) months prior to the end of the period. The application for extension shall include the reason(s) for the delay, whether there is any design change(s) and a review of whether the standards applicable at the time of Approval of the Works are still applicable at the time of request for extension, to ensure the ongoing protection of the environment.

3. CHANGE OF OWNER

1. The Owner shall notify the District Manager and the Director, in writing, of any of the following changes within thirty (30) days of the change occurring:
 - a. change of Owner;
 - b. change of address of the Owner;
 - c. change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the Business Names Act, R.S.O. 1990, c.B17 shall be included in the notification to the District Manager; or
 - d. change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the Corporations Information Act, R.S.O. 1990, c. C39 shall be included in the notification to the District Manager.
2. In the event of any change in ownership of the Works, other than a change to a successor municipality, the Owner shall notify in writing the succeeding owner of the existence of this Approval, and a copy of such notice shall be forwarded to the District Manager and the Director.
3. The Owner shall ensure that all communications made pursuant to this condition refer to the

number at the top of this Approval.

4. Notwithstanding any other requirements in this Approval, upon transfer of the ownership or assumption of the Works to a municipality if applicable, any reference to the District Manager shall be replaced with the Water Supervisor.

4. OPERATION AND MAINTENANCE

1. If applicable, any proposed storm sewers or other stormwater conveyance in this Approval can be constructed but not operated until the proposed stormwater management facilities in this Approval or any other Approval that are designed to service the storm sewers or other stormwater conveyance are in operation.

Schedule "A"

1. Application for Environmental Compliance Approval for Municipal and Private Sewage Works, dated May 17, 2017 and received on June 14, 2017, submitted by Greystone Village Inc.

The reasons for the imposition of these terms and conditions are as follows:

1. Condition 1 is imposed to ensure that the Works are constructed and operated in the manner in which they were described and upon which approval was granted. This condition is also included to emphasize the precedence of conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review. Condition 1.6 is included to emphasize that the issuance of this Approval does not diminish any other statutory and regulatory obligations to which the Owner is subject in the construction, maintenance and operation of the Works. The Condition specifically highlights the need to obtain any necessary conservation authority approvals. The Condition also emphasizes the fact that this Approval doesn't limit the authority of the Ministry to require further information.
2. Condition 2 is included to ensure that, when the Works are constructed, the Works will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
3. Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to approved Works and to ensure that subsequent owners of the Works are made aware of the Approval and continue to operate the Works in compliance with it.
4. Condition 4 is included to prevent the operation of stormwater pipes and other conveyance until such time that their required associated stormwater management Works are also constructed.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

- a. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
- b. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

1. The name of the appellant;
2. The address of the appellant;
3. The environmental compliance approval number;
4. The date of the environmental compliance approval;
5. The name of the Director, and;
6. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario

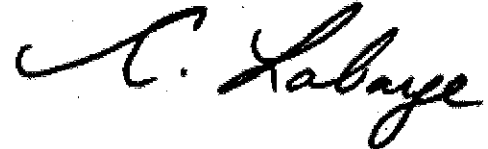
AND

The Director appointed for the purposes of
Part II.1 of the Environmental Protection Act
Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st Floor

* Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

DATED AT TORONTO this 12th day of July, 2017



Christina Labarge, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

SW/

c: District Manager, MOECC Ottawa District Office
City Clerk, City of Ottawa (File No. D07-16-15-0001)
Justin Gauthier, Project Manager, Novatech Engineering
Joshua White, P.Eng., Senior Engineer, Development Review, City of Ottawa
Linda Carkner, Program Manager, ROW Unit, City of Ottawa

Athens

Augusta

Beckwith

Central Frontenac

Clarence-Rockland

Drummond/North Elmsley

Elizabethtown-Kitley

Merrickville-Wolford

Montague

North Dundas

North Grenville

Ottawa

Perth

Rideau Lakes

Smiths Falls

South Frontenac

Tay Valley

Westport

**AMENDED LETTER OF PERMISSION – ONTARIO REGULATION 174/06,
SECTION 28 CONSERVATION AUTHORITIES ACT 1990, AS AMENDED.**

Date: April 14, 2016
File: RV3-56/15
Contact: Hal Stimson
(613) 692-3571 ext. 1127
hal.stimson@rvca.ca

Mr. David Kardish
Greystone Village Inc.
c/o The Regional Group
1737 Woodward Dr. 2nd Flr
Ottawa, Ontario
K2C 0P9

Permit for development under Section 28 of the Conservation Authorities Act for fill remediation in a regulated area at Lot Part H Concession D (old Nepean Township) City of Ottawa known municipally as 175 Main Street

Dear Mr. Kardish,

The Rideau Valley Conservation Authority has reviewed your application and understands the proposal to be for the removal of fill in the regulated limits area of the Rideau River in the vicinity of Clegg Street. The work involves removal of contaminated soil and replacement with suitable clean fill material for future development. The work will be carried out under a Brownfield application and falls outside the 1:100 year flood plain elevation of 58.35m geodetic. This amended permit also authorizes excavation work as described without shoring within the RVCA 30m setback zone provided the work is only for the soil remediation work and all grades are restored to existing and stabilized upon completion of the remediation work.

This proposal was reviewed under Ontario Regulation 174/06, the “Development, Interference with Wetlands and Alterations to Shorelines and Watercourses” regulation.

PERMISSION AND CONDITIONS

By this letter the Rideau Valley Authority hereby grants you approval to undertake this project as outlined in your permit application but subject to the following conditions:

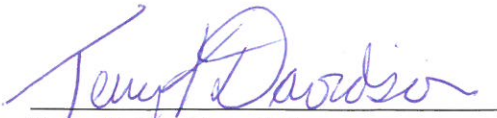
1. Approval is subject to the understanding of the project as described above and outlined in the application and submitted plans including:
 - Drawing No. 114025-LG2 for Project No. 114025 titled RVCA Remediation Permit Plan Phase 1A and 1B, dated Mar 07/16, revision No. 2, as prepared by Novatech Engineering and stamped by J. G. Riddell, P. Eng.
 - Technical Memorandum for project No. 1525113 dated March 10, 2016 by S. A. Trickey, P. Eng and M. Cunningham, P. Eng of Golder and Associates titled Excavation Side Slope Recommendations
No conditions are subject to change/revision by the on-site contractor(s).
2. No encroachment for fill remediation purposes is to occur within 15m of the top of the river bank. Construction access fencing should be installed at this 15m boundary to clearly demarcate the construction access limits.
3. All grades within the 30m setback are to be restored to existing and stabilized upon completion of the remediation work.
4. It is recommended that you retain the services of a professional engineer to conduct on-site inspections to ensure adequacy of the work, verify stability of the final grade and slopes and confirm all imported fill is of suitable type and has been adequately placed and compacted.
5. No in-water work is proposed, however work in-water shall not be conducted at times when flows are elevated due to local rain events, storms or seasonal floods.
6. **A Sediment and Erosion Control Plan must be submitted by the contractor to this office for review prior to construction activities commencing.**
7. It is recommended that you ensure your contractor(s) are provided with a copy of this letter so as to ensure compliance with the conditions listed herein.
8. Only clean non-contaminated fill material will be used.
9. **Any excess excavated material, as a result of the work, must be disposed of in a suitable location outside any regulatory floodplain and fill regulated area and local area grades to match as proposed.**
10. Sediment and erosion control measures shall be in place before any excavation or construction works commence. All sediment/erosion control measures are to be monitored regularly by experienced personnel and maintained as necessary to ensure good working order. In the event that the erosion and sedimentation control measures are deemed not to be performing adequately, the contractor shall undertake immediate additional measures as appropriate to the situation to the satisfaction of the Conservation Authority.
11. Sediment barriers should be used on site in an appropriate method according to the Ontario Provincial Standard Specifications (OPSS) for silt barriers as a minimum. If the sediment and erosion control methods include silt fence it should be placed along the shoreline to prevent overland flow on disturbed areas from

entering any watercourse. Soil type, slope of land, drainage area, weather, predicted sediment load and deposition should be considered when selecting the type of sediment/erosion control.

12. All materials and equipment used for the purpose of site preparation and project completion must be operated and stored in a manner that prevents any deleterious substance (e.g. petroleum products, silt, debris etc.) from entering the water.
13. The waters of the river are NOT to be considered as machine staging areas. Activities such as equipment refuelling and maintenance must be conducted away from the water to prevent entry of petroleum products, debris, or other deleterious substances into the water.
14. All disturbed soil areas must be appropriately stabilized to prevent erosion.
15. **There will be no in-water works between March 15 and July 1, of any given year to protect local aquatic species populations during their spawning and nursery time periods.**
16. Develop a response plan that is to be implemented immediately in the event of a sediment release or spill of a deleterious substance. This plan is to include measures to: a) stop work, contain sediment-laden water and other deleterious substances and prevent their further migration into the watercourse and downstream receiving watercourses; b) notify the RVCA and all applicable authorities in the area c) promptly clean-up and appropriately dispose of the sediment-laden water and deleterious substances; and d) ensure clean-up measures are suitably applied so as not to result in further alteration of the bed and/or banks of the watercourse.
17. That the Conservation Authority be given 48 hours notice prior to the start of construction and within 48 hours of project completion.
18. The applicant agrees that Authority staff may visit the subject property before, during and after project completion to ensure compliance with the conditions as set out in this letter of permission.
19. A new application must be submitted should any work as specified in this letter be ongoing or planned for or after January 18, 2018.
20. All other approvals as might be required from the Municipality, and/or other Provincial or Federal Agencies must be obtained prior to initiation of work. This includes but is not limited to the Endangered Species Act, the Ontario Water Resources Act, Environmental Protection Act, Public Lands Act, or the Fisheries Act.

By this letter the Rideau Valley Conservation Authority assumes no responsibility or liability for any flood, erosion, or slope failure damage which may occur either to your property or the structures on it or if any activity undertaken by you adversely affects the property or interests of adjacent landowners. This letter does not relieve you of the necessity or responsibility for obtaining any other federal, provincial or municipal permits. This permit is not transferable to subsequent property owners.

Should you have any questions regarding this letter please contact Hal Stimson at our Manotick office.



Terry K. Davidson P.Eng
Conservation Authority S. 28 Signing delegate
O. Reg. 174/06

Cc: M. St. Pierre, P. Eng. Novatech

- Pursuant to the provisions of S. 28(12) of the Conservation Authorities Act (R.S.O.1990, as amended.) any or all of the conditions set out above may be appealed to the Executive Committee of the Conservation Authority in the event that they are not satisfactory or cannot be complied with.
- Failure to comply with the conditions of approval or the scope of the project may result in the cancelling of the permission and/or initiation of legal action under S. 28(16) of the Act.
- This letter of permission does not come into full force and effect until the attached copy of this letter is returned to the Authority offices in Manotick signed and dated which return shall be taken as indicating acceptance of the conditions of the Authority's approval and acknowledgement that the details of the proposal as described in this letter are a fair and accurate representation of the proposed undertaking.

Name: _____ (print)

Signed: _____ Date: _____

DATE March 10, 2016**PROJECT No.** 1525113**TO** Trevor Mackay
Novatech Engineering Consultants Ltd.**FROM** Susan Trickey, P.Eng.
Mike Cunningham, P.Eng.**EMAIL** strickey@golder.com
mcunningham@golder.com

**EXCAVATION SIDE SLOPE RECOMMENDATIONS
EASTERN PROPERTY LIMIT
GREYSTONE VILLAGE REMEDIATION/ENGINEERED FILL PLACEMENT
175 MAIN STREET, OTTAWA, ONTARIO**

The following memo provides recommendations for excavation side slopes for the eastern property limit for the remediation and engineered fill placement at the Greystone Village development located at 175 Main Street in Ottawa, Ontario.

It is understood that consideration is being given to carrying out the excavation for the Greystone Village remediation without the use of shoring along the eastern edge of the site. It is also understood that the excavations would extend from the development line back into the 30 m buffer area along the Rideau River.

Golder Associates completed a previous geotechnical investigation on the property for the Draft Plan of Subdivision Application to the City of Ottawa. The results of that investigation were provided in a report titled "Geotechnical Investigation, Proposed Development, Oblates Property, 175 Main Street, Ottawa, Ontario", dated December 2014 (Report No. 14-1122-0005-5100). Based on the results of that investigation as well as observation of the construction activities that are currently taking place as part of the site remediation, the subsurface conditions on the site generally consist of up to about 7 to 8 metres of fill overlying a thick deposit of sensitive silty clay, which is underlain by layers of silty sand and glacial till with the bedrock surface between 25 and 30 metres depth.

Based on observation of excavations made in the fill to date, this material would generally be classified as Type 3 soil in accordance with the Occupational Health and Safety Act (OHSA) and therefore unsupported side slopes cut back at an inclination no steeper than 1 horizontal to 1 vertical (1H:1V) would be considered feasible. However, there is potential that the excavation slopes may ultimately slough to a somewhat flatter inclination depending on the length of time that they remain open as well as due to freeze-thaw of the exposed soil face. Therefore, consideration should be given to setting back the construction fencing by a distance of about 4 metres from the crest of the excavation as an added safety measure for the public. In addition, ongoing inspection of the excavation side slopes should be made as the remediation activities continue, to confirm the above recommendations.

Golder Associates Ltd.

1931 Robertson Road, Ottawa, Ontario, Canada, K2H 5B7
Tel: +1 (613) 592 9600 Fax: +1 (613) 592 9601 www.golder.com

Golder Associates: Operations in Africa, Asia, Australasia, Europe, North America and South America


We trust that this memo contains sufficient information for your present requirements. If you have any questions concerning this memo, please contact undersigned.

Yours truly,

GOLDER ASSOCIATES LTD.

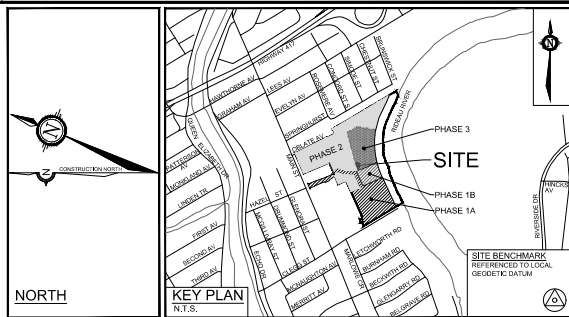

Susan Trickey, P.Eng.
Geotechnical Engineer




Mike Cunningham, P.Eng.
Geotechnical Engineer, Principal

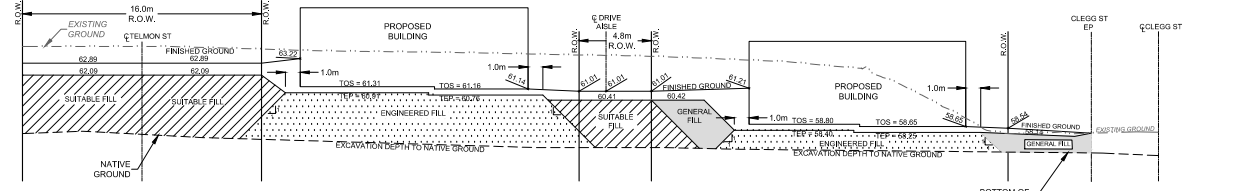
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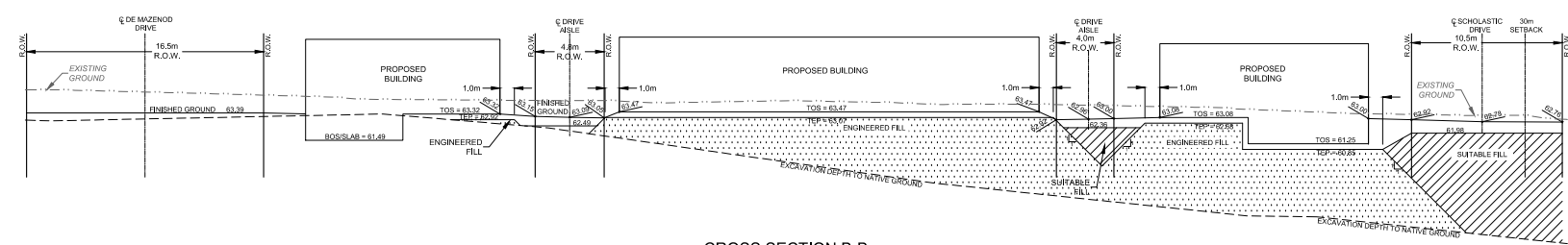


LEGEND

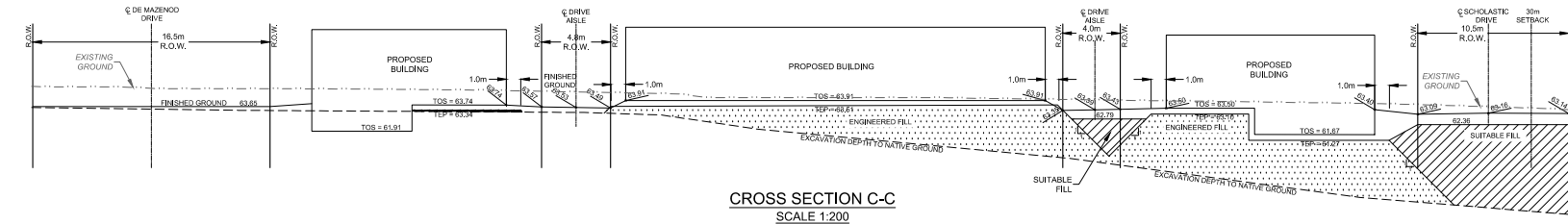
- SITE BOUNDARY
- PHASING LIMITS
- APPROXIMATE DEBRIS FILL BOUNDARY LIMIT
- 1:100YR FLOODPLAIN LIMIT
- FLOODPLAIN REGULATORY LIMIT
- ORIGINAL GROUND CONTOUR LINE AND CONTOUR ELEVATION
- REMEDIATION AREA REQUIRING A RVCA PERMIT



CROSS SECTION A-A
SCALE 1:200



CROSS SECTION B-B
SCALE 1:200



CROSS SECTION C-C
SCALE 1:200

NOTE:
THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY
2	REVISED REGULATORY REMEDIATION AREA	MAR 07/16	MSP
1	ISSUED FOR RVCA PERMIT	NOV 23/15	MSP

SCALE	1:400
SCALE	1:400
SCALE	1:400

FOR REVIEW ONLY	JAG
FOR REVIEW ONLY	MSP
FOR REVIEW ONLY	RBG
FOR REVIEW ONLY	JAG
FOR REVIEW ONLY	JGR



NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowland Drive
Ottawa, Ontario, Canada K2M 3P6
Telephone: (613) 254-6643
Facsimile: (613) 254-5867
Website: www.novatech-weg.com

CITY OF OTTAWA
GREYSTONE VILLAGE
175 MAIN STREET

DRAWING NAME
RVCA REMEDIATION PERMIT PLAN
PHASE 1A AND 1B

PROJECT No. 114025
REV # 2
DRAWING No. 114025-LG2

**LETTER OF PERMISSION – ONT. REG. 174/06,
SECTION 28 CONSERVATION AUTHORITIES ACT 1990, AS AMENDED.**

Date: 21 April, 2017.
File: RV3-08/17
Contact: Hal Stimson
(613) 692-3571 Ext 1127
hal.stimson@rvca.ca

Mr. David Kardish
Greystone Village Inc.
c/o The Regional Group
1737 Woodward Dr.
Ottawa, Ontario
K2C 0P9

Permit for development under Section 28 of the Conservation Authorities Act for storm water outlet and soil remediation in a regulated area at Lot Part H Concession D (old Nepean Township) City of Ottawa known municipally as 175 Main Street

Dear Mr. Kardish

The Rideau Valley Conservation Authority has reviewed your application on behalf of Regional Group and understands the proposal to be for: 1) the installation of a new 750 mm diameter concrete stormwater outlet pipe including headwall and river stone plunge pool discharging to the Rideau River east of the intersection of Oblate Avenue and Scholastic Drive and including a compensatory cut of fill previously approved. 2) removal and replacement of contaminated soil in the RVCA regulated area with existing grades to be re-established.

This proposal was reviewed under Ontario Regulation 174/06, the “*Development, Interference with Wetlands and Alterations to Shorelines and Watercourses*” regulation and is approved in an amended form noting that the construction of buildings request (lots 12 and 13) will need to form a separate application pending registration of the lots and verification of appropriate flood proofing measures in the final building design.

PERMISSION AND CONDITIONS

By this letter the Rideau Valley Conservation Authority hereby grants you approval to undertake this project as outlined in your permit application but subject to the following conditions:

1. Approval is subject to the understanding of the project as described above and outlined in the application and submitted plans including:
 - Drawing No. 114025-PR6-B for Project No. 114025-00 titled Plan and Profile Phase 2 and 3 Storm Outlet 2 (Incl. Grading, Erosion and Sediment Control) Station 0+000 to 0+54, dated Nov 21/16, revision No. 1, as prepared by Novatech Engineering and stamped by J. G. Riddell, P. Eng.
 - Drawing No. 114025-GR3-B for Project No. 114025-00 titled Grading, Erosion and Sediment Control Plan Phase 2 and 3, dated Nov 21/16, revision No. 1, as prepared by Novatech Engineering and stamped by J. G. Riddell, P. Eng.
 - Drawing No. 114025-GP3-B for Project No. 114025-00 titled General Plan of Services Phase 2 and 3, dated Nov 21/16, revision No. 1, as prepared by Novatech Engineering.
 - Drawing No. 114025-LG-B for Project No. 114025-00 titled RVCA Remediation Permit Plan, dated Feb 13/17, revision No. 1, as prepared by Novatech Engineering.
 - Technical memorandum for project 14-1122-0005 dated February 3, 2017 from Susan Trickey, P. Eng. of Golder Associates.

No conditions are subject to change/revision by the on-site contractor(s).

2. **There will be no in-water works between March 15 and July 1, of any given year to protect local aquatic species populations during their spawning and nursery time periods.**
3. No encroachment for fill remediation purposes is to occur within 15m of the top of the river bank. Construction access fencing should be installed to clearly demarcate the construction access limits.
4. All grades within the 30m setback are to be restored to existing and stabilized upon completion of the remediation work.
5. It is recommended that you retain the services of an engineer to conduct on-site inspections to ensure adequacy of the work, verify stability of the final grade and confirm all imported fill is of a suitable type and has been adequately placed and compacted and that the recommendations of the geotechnical technical memorandum are followed.
6. **A De-watering Plan and Sediment and Erosion Control Plan must be submitted by the contractor to this office for review prior to construction activities commencing on the storm outlet.**
7. It is recommended that you ensure your contractor(s) are provided with a copy of this letter so as to ensure compliance with the conditions listed herein.
8. All disturbed soil areas must be appropriately stabilized to prevent erosion.

9. Any excess excavated material, as a result of the work, must be disposed of in a suitable location outside any regulatory floodplain and fill regulated area. No changes to area grades are to occur as a result of the work.
10. A final as built grading plan shall be submitted immediately upon completion of the approved works prepared by an Ontario Land Surveyor or Professional Engineer licensed to practice in Ontario indicating that grades achieved on the site conform to those indicated on the approved plan. Only clean material free from particulate matter may be placed in the water.
11. Operate machinery from outside the water, or on the water in a manner that minimizes disturbance to the banks or bed of the watercourse. Equipment shall not be cleaned in the watercourse or where wash-water can enter any watercourse. Machinery is to arrive on site in a clean condition and is to be maintained free of fluid leaks.
12. All materials and equipment used for the purpose of site preparation and project completion must be operated (washed, refuelled, and serviced) and all fuel stored in a manner that prevents any deleterious substance (e.g. petroleum products, silt, debris etc.) from entering any watercourse.
13. Any stockpiled materials shall be stored and stabilized away from the water.
14. Work in water shall not be conducted at times when flows are elevated due to local rain events, storms or seasonal floods.
15. Sediment barriers should be used on site in an appropriate method according to the Ontario Provincial Standard Specifications (OPSS) for silt barriers as a minimum. If the sediment and erosion control methods include silt fence it should be placed along the shoreline to prevent overland flow on disturbed areas from entering the watercourse. Soil type, slope of land, drainage area, weather, predicted sediment load and deposition should be considered when selecting the type of sediment/erosion control.
16. Sediment and erosion control measures shall be in place before any excavation or construction works commence. All sediment/erosion control measures are to be monitored regularly by experienced personnel and maintained as necessary to ensure good working order. In the event that the erosion and sedimentation control measures are deemed not to be performing adequately, the contractor shall undertake immediate additional measures as appropriate to the situation to the satisfaction of the Conservation Authority.
17. Develop a response plan that is to be implemented immediately in the event of flooding, a sediment release or spill of a deleterious substance. This plan is to include measures to: a) stop work, contain sediment-laden water and other deleterious substances and prevent their further migration into the watercourse and downstream receiving watercourses; b) notify the RVCA and all applicable authorities in the area c) promptly clean-up and appropriately dispose of the sediment-laden water and deleterious

substances; and d) ensure clean-up measures are suitably applied so as not to result in further alteration of the bed and/or banks of the watercourse.

18. The owner is ultimately responsible for failure to comply with any and/or all of these conditions and must take all precautions to ensure no sediment runoff from the work site into any watercourse during and after the construction period. Failure to comply with the approval and/or conditions of this letter will result in the permit being revoked and may also result in legal action being initiated to resolve the matter to the Conservation Authority's satisfaction.
19. The applicant agrees that Authority staff may visit the subject property, before, during and after project completion, to ensure compliance with the conditions as set out in this letter of permission.
20. A new application must be submitted should any work as specified in this letter be ongoing or planned for or after April 25, 2019.
21. That the Authority be given twenty-four hours notice prior to the start of construction and within twenty-four hours of project completion.
22. All other approvals as might be required from the Municipality, and/or other Provincial or Federal Agencies must be obtained prior to initiation of work. This includes but is not limited to the Endangered Species Act., the Ontario Water Resources Act., Environmental Protection Act., Public Lands Act, and the Fisheries Act.

By this letter the Rideau Valley Conservation Authority assumes no responsibility or liability for any flood, erosion, or slope failure damage which may occur either to your property or the structures on it or if any activity undertaken by you adversely affects the property or interests of adjacent landowners. This letter does not relieve you of the necessity or responsibility for obtaining any other federal, provincial or municipal permits. This permit is not transferable to subsequent property owners.

Should you have any questions regarding this letter, please contact Hal Stimson at our Manotick office.



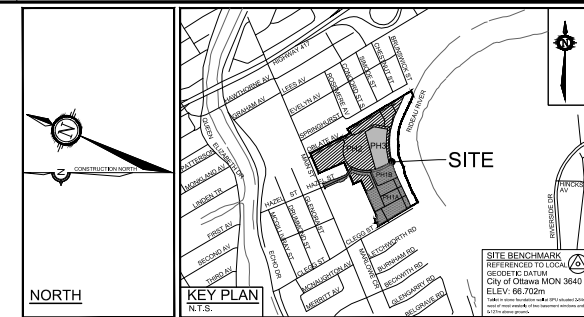
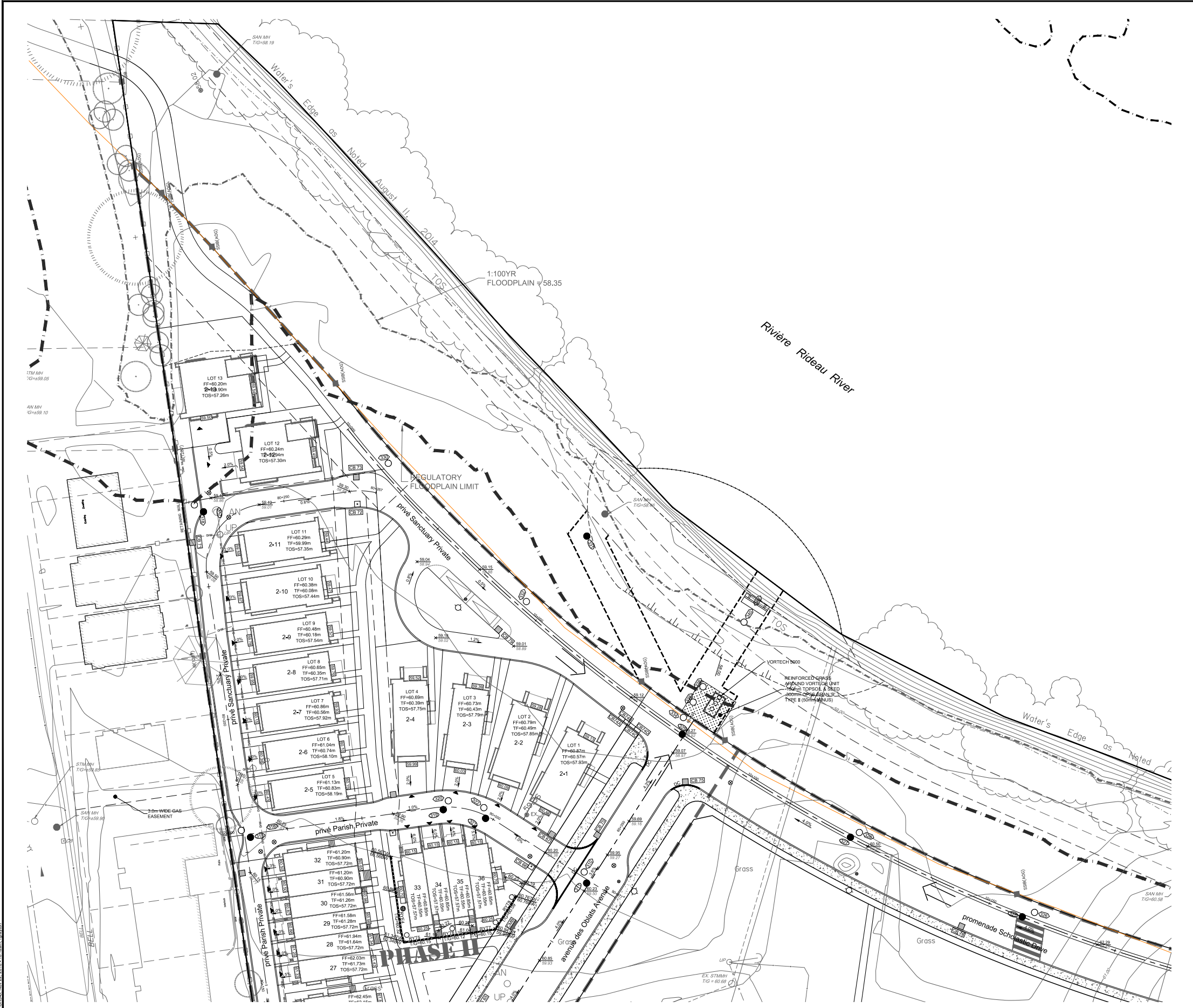
Terry K. Davidson, P. Eng.
Conservation Authority S. 28 Signing delegate
O. Reg. 174/06

Cc: J. Gauthier, E.I.T. Novatech
T. McLaurin, MNRF Kemptville

- Pursuant to the provisions of S. 28(12) of the Conservation Authorities Act (R.S.O.1990, as amended.) any or all of the conditions set out above may be appealed to the Executive Committee of the Conservation Authority in the event that they are not satisfactory or cannot be complied with.
- Failure to comply with the conditions of approval or the scope of the project may result in the cancelling of the permission and/or initiation of legal action under S. 28(16) of the Act.
- This letter of permission does not come into full force and effect until the attached copy of this letter is returned to the Authority offices in Manotick signed and dated which return shall be taken as indicating acceptance of the conditions of the Authority's approval and acknowledgement that the details of the proposal as described in this letter are a fair and accurate representation of the proposed undertaking.

Name: _____ (print)

Signed: _____ Date: _____



LEGEND

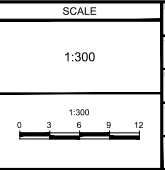
---	SITE BOUNDARY	◇ HYD	PROPOSED HYDRANT LOCATION
---	PHASING LIMITS	TF=127.55	PROPOSED TOP OF BOTTOM FLANGE
---	PROPOSED ELEVATION	● VALV	PROPOSED VALVE AND VALVE BOX
---	EXISTING ELEVATION	●	PROPOSED SANITARY MANHOLE
---	PROPOSED SWALE ELEVATION	○	PROPOSED STORM MANHOLE
---	PROPOSED TOP OF SLOPE	○	PROPOSED ROADSIDE CATCH BASIN
---	PROPOSED HIGH POINT	○	PROPOSED ROADSIDE CATCH BASIN WITH INLET CONTROL DEVICE
---	PROPOSED TOP OF RETAINING WALL	□ CB 10	PROPOSED REAR YARD CATCHBASIN MANHOLE WITH TOP OF GRATE ELEVATION
---	PROPOSED BOTTOM OF RETAINING WALL	□ CB 20	PROPOSED REAR YARD CATCHBASIN WITH TOP OF GRATE ELEVATION
---	FFE= FINISHED FLOOR ELEVATION	○ RYCBM 1	PROPOSED REAR YARD CATCHBASIN WITH TOP OF GRATE ELEVATION
---	TF= TOP OF FOOTING ELEVATION	○ RYCBM 1	PROPOSED REAR YARD CATCHBASIN WITH TOP OF GRATE ELEVATION
---	TOS= TOP OF SLAB ELEVATION	○ RY-1	REAR YARD ELBOW WITH TOP OF GRATE
---	USF= UNDERSIDE OF FOOTING ELEVATION	○ RY-1	REAR YARD TEE WITH TOP OF GRATE
---	MUSF= MINIMUM UNDERSIDE OF FOOTING ELEVATION	○ RYT-1	REAR YARD TEE WITH TOP OF GRATE
---	SD INDICATES A STANDARD UNIT	○	PROPOSED COMMUNITY MAIL BOX
---	L/O INDICATES A LOOK OUT UNIT	○	PROPOSED STREET LIGHT
---	W/O INDICATES A WALK OUT UNIT	○	PROPOSED SERVICE LOCATION (REFER TO DETAIL)
---	127.55 PROPOSED TERRACE ELEVATION		
---	2.0% PROPOSED GRADE AND DIRECTION		
---	MAJOR OVERLAND FLOW ROUTE		
---	MAXIMUM 3:1 SIDESLOPE		
---	PROPOSED CENTRELINE SWALE		
---	PART OF PH.2 OUTLETING THROUGH PH.1A AT OUTLET 1		
---	PHASE 1A AND 1B OUTLETING THROUGH PH.1A AT OUTLET 1		

NOTE: SILT FENCE TO BE INSTALLED AROUND ENTIRE SITE. (SEE OPSD 219.110 ON SHEET 114025-D3-B)

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**PRELIMINARY
NOT FOR
CONSTRUCTION**

No.	REVISION	DATE	BY
1.	ISSUED FOR CITY OF OTTAWA REVIEW	NOV 21/16	JAG



FOR REVIEW ONLY

DESIGN	JAG
CHECKED	MSP
DRAWN	MTM
CHECKED	JAG
APPROVED	JGR



CITY OF OTTAWA GREYSTONE VILLAGE 175 MAIN STREET	
DRAWING NAME	GRADING, EROSION AND SEDIMENT CONTROL PLAN PHASE 2 AND 3
PROJECT No.	114025-00
REV	REV # 1
DRAWING No.	114025-GR3-B

D07-16-15-0001 PHASE 2 AND 3

REFER TO 114025-N&L-B FOR ADDITIONAL NOTES AND CATCHBASIN TABLES

REFER TO MATCHLINE A

1:100YR FLOODPLAIN = 58.35

MATCHLINE A

CATCHBASIN TABLE				
CB No.	STATION	T/G ELEVATION	INVERT	ICD DIA.
CB 63	80+116.98	62.66	61.06	83mmØ
CB 64	80+306.59	61.25	59.71	1:100YR CAPTURE RATE=18L/s
CB 65	80+306.59	61.25	59.60	127mmØ
CB 66	90+060.73	60.25	58.51	1:100YR CAPTURE RATE=40L/s
CB 67	90+060.73	60.11	58.43	83mmØ
CB 68	80+030.30	60.06	58.46	1:100YR CAPTURE RATE=18L/s
CB 69	90+031.41	59.97	58.38	83mmØ
CB 70	80+346.26	59.95	58.25	1:100YR CAPTURE RATE=27L/s
CB 71	80+232.19	58.44	56.84	108mmØ
CB 72	80+263.73	58.90	57.54	1:100YR CAPTURE RATE=18L/s
CB 73	80+263.73	59.20	57.60	83mmØ
CB 74a	10+073.78	59.00	57.05	127mmØ LEAD
CB 74b	10+074.96	59.01	57.51	1:100YR CAPTURE RATE=495L/s
CB 74c	10+076.14	59.03	57.04	375mmØ LEAD
CB 74d	10+077.33	59.06	57.56	1:100YR CAPTURE RATE=495L/s
CB 75	80+363.05	59.04	57.44	127mmØ
CB 76	10+146.90	61.18	59.58	1:100YR CAPTURE RATE=28L/s
CB 77	10+192.63	62.57	60.97	83mmØ
CB 78	80+171.29	60.11	58.51	1:100YR CAPTURE RATE=18L/s

REARYARD MANHOLE TABLE			
MANHOLE ID	MANHOLE SIZE	T/G ELEV	INVERT
RYE4	300Ø	60.32	SW=59.12
RYT5	300Ø	60.15	NE=59.10 SE=59.10
RYT6	300Ø	60.12	NW=59.02 SE=59.08
RYT7	300Ø	60.15	NW=59.02 E=59.02
RYT8	300Ø	60.09	W=58.99 E=58.99

LEGEND

- SITE BOUNDARY
- PHASING LIMITS
- PROPOSED ELEVATION
- PROPOSED SWALE ELEVATION
- PROPOSED TOP OF SLOPE
- PROPOSED HIGH POINT
- PROPOSED TOP OF RETAINING WALL
- PROPOSED BOTTOM OF RETAINING WALL
- FFE= FINISHED FLOOR ELEVATION
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- USF= UNDERSIDE OF FOOTING ELEVATION
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- PHASE 1A AND 1B OUTLETTING THROUGH PH.1A AT OUTLET 1

- HYD PROPOSED HYDRANT LOCATION
- TF=127.55 PROPOSED TOP OF BOTTOM FLANGE
- V&VB PROPOSED VALVE AND VALVE BOX
- Ø 400 PROPOSED SANITARY MANHOLE
- Ø 150 PROPOSED STORM MANHOLE
- CB 10 PROPOSED ROADSIDE CATCH BASIN
- CB 20 PROPOSED ROADSIDE CATCH BASIN WITH INLET CONTROL DEVICE
- RYCB 1 PROPOSED REARYARD CATCHBASIN MANHOLE WITH TOP OF GRATE ELEVATION
- RYCBM 1 PROPOSED REARYARD CATCHBASIN WITH TOP OF GRATE ELEVATION
- RYE-1 PROPOSED REAR YARD ELBOW WITH TOP OF GRATE
- RYT-1 PROPOSED REAR YARD TEE WITH TOP OF GRATE
- PROPOSED COMMUNITY MAIL BOX
- PROPOSED STREET LIGHT
- PROPOSED SERVICE LOCATION (REFER TO DETAIL)

NOTE: SILT FENCE TO BE INSTALLED AROUND ENTIRE SITE. (SEE OPSD 219.110 ON SHEET 114025-03-B)

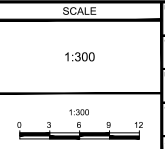
REFER TO DRAWING No. 114025-GP2-B

REFER TO 114025-N&L-B FOR ADDITIONAL NOTES AND CATCHBASIN TABLES

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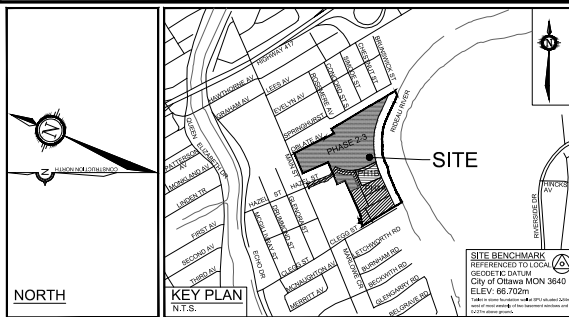
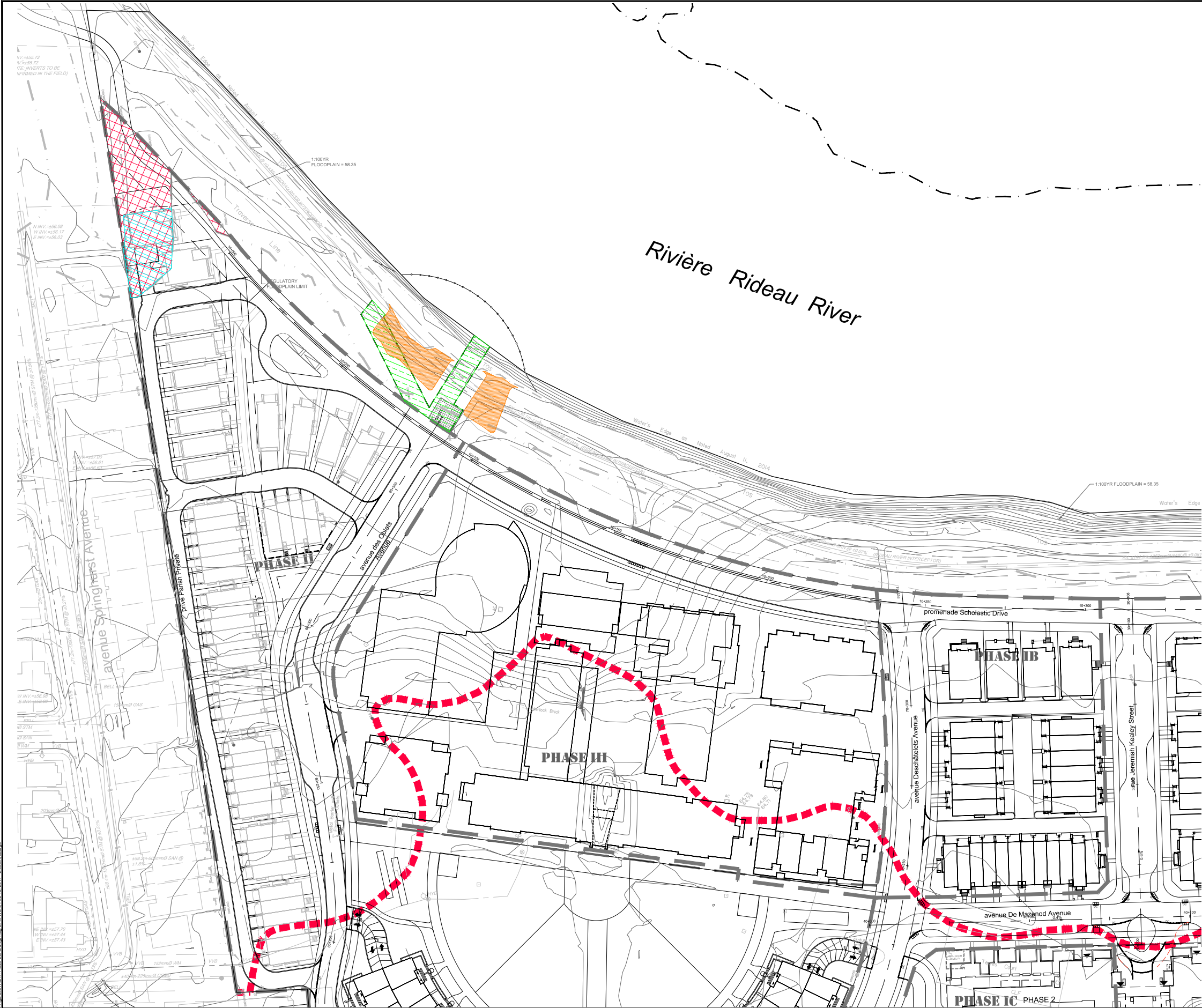
NOVATECH
Engineers, Planners & Landscape Architects
Suite 200, 240 Michael Cowland Drive
Ottawa, Ontario, Canada K2M 3P6
Telephone: (613) 254-9643
Facsimile: (613) 254-5867
Website: www.novatech-eng.com

CITY OF OTTAWA
GREYSTONE VILLAGE
175 MAIN STREET

DRAWING NAME
**GENERAL PLAN OF SERVICES
PHASE 2 AND 3**

PROJECT No. 114025-00
REV # 1
DRAWING No. 114025-GP3-B

D07-16-15-0001 PHASE 2 AND 3



LEGEND

- SITE BOUNDARY
- PHASING LIMITS
- APPROXIMATE DEBRIS FILL BOUNDARY LIMIT
- 1:100YR FLOODPLAIN LIMIT
- FLOODPLAIN REGULATORY LIMIT
- 61.00' ORIGINAL GROUND CONTOUR LINE AND CONTOUR ELEVATION
- REMEDATION AREA REQUIRING A RVCA PERMIT
- RESIDENTIAL DWELLING TO BE CONSTRUCTED REQUIRING A RVCA PERMIT
- STORM AND SANITARY OUTLETS TO BE CONSTRUCTED REQUIRING A RVCA PERMIT
- PREVIOUSLY APPROVED CUT COMPENSATION AREA

NOTE:
 THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY
1.	ISSUED FOR RVCA PERMIT	FEB 13/17	JAG

SCALE	1:500
0	5 10 15 20

FOR REVIEW ONLY										
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CITY OF OTTAWA
 GREYSTONE VILLAGE
 175 MAIN STREET

DRAWING NAME
 RVCA REMEDIATION PERMIT PLAN
 (OUTLET WORK, RESIDENTIAL
 CONSTRUCTION AND PREVIOUSLY
 APPROVED CUT COMPENSATION
 LIMITS WITHIN REGULATORY LIMITS)

PROJECT No. 114025-00
 REV # 1
 DRAWING No. 114025-LG-B

Appendix E
Development Servicing Study Checklist

Development Servicing Study Checklist

4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Executive Summary (for larger reports only).	NA		
Date and revision number of the report.	Y	Cover	
Location map and plan showing municipal address, boundary, and layout of proposed development.	Y	Fig 1 & 2	
Plan showing the site and location of all existing services.	Y	Fig 3	& General Plan of Services, 114025-GP (PH3)
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Y	1.0	
Summary of Pre-consultation Meetings with City and other approval agencies.	Y	Appendix	Appendix D - Correspondence
Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Y	1.2, 3.0-5.0	
Statement of objectives and servicing criteria.	Y	3.0-5.0	
Identification of existing and proposed infrastructure available in the immediate area.	Y		General Plan of Services, 114025-GP (PH3)
Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Y	1.3	
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighboring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Y		Grading, Erosion and Sediment Control Plan, 114025-GR (PH3)

4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	NA		
Proposed phasing of the development, if applicable.	Y		General Plan of Services, 114025-GP (PH3)
Reference to geotechnical studies and recommendations concerning servicing.	Y	1.1	
All preliminary and formal site plan submissions should have the following information:			
Metric scale	Y		
North arrow (including construction North)	Y		
Key plan	Y		
Name and contact information of applicant and property owner	Y		
Property limits including bearings and dimensions	Y		
Existing and proposed structures and parking areas	Y		
Easements, road widening and rights-of-way	Y		
Adjacent street names	Y		

Development Servicing Study Checklist

4.2 Water	Addressed (Y/N/NA)	Section	Comments
Confirm consistency with Master Servicing Study, if available.	Y	3.0	
Availability of public infrastructure to service proposed development.	Y	3.0	
Identification of system constraints.	Y	3.0	
Identify boundary conditions.	Y	3.0	Appendix B
Confirmation of adequate domestic supply and pressure.	Y	3.0	Appendix B
Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Y	3.0	Appendix B
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Y	3.0	Appendix B
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design.	NA		
Address reliability requirements such as appropriate location of shut-off valves.	Y		General Plan of Services, 114025-GP (PH3)
Check on the necessity of a pressure zone boundary modification.	Y	3.0	
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Y	3.0	Appendix B
Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Y	3.0	Appendix B
Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	Y	3.0	
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Y	3.0	
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Y		Appendix B

Development Servicing Study Checklist

4.3 Wastewater	Addressed (Y/N/NA)	Section	Comments
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Y	2.0	
Confirm consistency with Master Servicing Study and/or justifications for deviations.	Y	2.0	
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	Y	2.0	
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Y	2.0	
Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Y	2.0	
Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Y		Appendix A
Description of proposed sewer network including sewers, pumping stations, and forcemains.	Y	2.0	
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	NA		
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	NA		
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow	NA		
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	NA		
Special considerations such as contamination, corrosive environment etc.	NA		

Development Servicing Study Checklist

4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Description of drainage outlets and downstream constraints including legality of outlet (i.e. municipal drain, right-of-way, watercourse, or private property).	Y	4.0	
Analysis of the available capacity in existing public infrastructure.	Y	4.0	
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns and proposed drainage patterns.	Y		Storm Drainage Area Plan, 114025-STM (PH3)
Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Y	4.0	
Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Y	4.0	
Description of stormwater management concept with facility locations and descriptions with references and supporting information.	Y	4.0	
Set-back from private sewage disposal systems.	NA		
Watercourse and hazard lands setbacks.	Y	4.0	
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Y		Appendix D
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	Y	4.0	
Storage requirements (complete with calcs) and conveyance capacity for 5 yr and 100 yr events.	Y		Appendix C
Identification of watercourse within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Y	4.0	
Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Y	4.0	
Any proposed diversion of drainage catchment areas from one outlet to another.	Y	4.0	
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and SWM facilities.	Y	4.0	
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Y	4.0	

Development Servicing Study Checklist

4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Identification of municipal drains and related approval requirements.	Y	4.0	
Description of how the conveyance and storage capacity will be achieved for the development.	Y	4.0	
100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Y	4.0	
Inclusion of hydraulic analysis including HGL elevations.	N/A		See Master Servicing Report
Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Y	7.0	
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	Y	4.0	
Identification of fill constrains related to floodplain and geotechnical investigation.	Y	1.0, 4.0	

4.5 Approval and Permit Requirements	Addressed (Y/N/NA)	Section	Comments
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	Y	1.0	
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	NA		
Changes to Municipal Drains.	NA		
Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	NA		

4.6 Conclusion	Addressed (Y/N/NA)	Section	Comments
Clearly stated conclusions and recommendations.	Y	2.0-5.0	
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	Y		Included as separate letter.
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario.	Y		