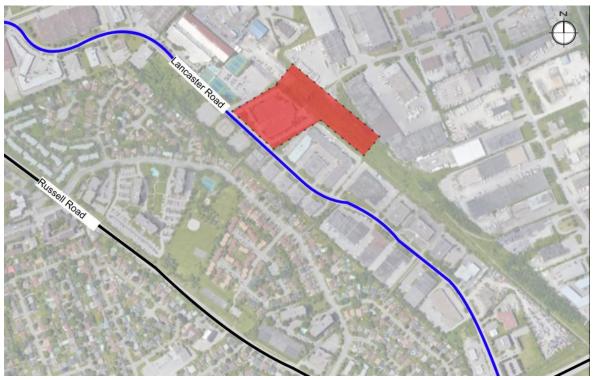
CITY OF OTTAWA

2571 LANCASTER ROAD TRANSPORTATION IMPACT ASSESSMENT FINAL REPORT

FEBRUARY 24, 2022

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CITY OF OTTAWA

DRAFT

PROJECT NO.: OUR REF. NO. 211-06996-00 DATE: FEBRUARY 24, 2022

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1 SCREENING

This Transportation Impact Assessment (TIA) has been prepared to support the <u>Site Plan Control Application</u> for the development at 2571 Lancaster Road in Ottawa. The TIA follows the City of Ottawa guidelines which potentially includes five steps:

- 1 Screening
- 2 Scoping
- 3 Forecasting
- 4 Analysis
- 5 TIA Report

The Screening Step determines the need to continue with a TIA Study. The development is assessed against three triggers: trip generation, location, and safety to identify the next step of the study. If one or more of the triggers is satisfied, the Scoping Step must be completed. If none of the triggers are satisfied, the TIA is deemed complete. If one or more triggers are satisfied, specific TIA components are required to be carried out depending on the combination of triggers (**Table 1-1**) that have been satisfied.

The proposed development at 2571 Lancaster Road **satisfied the Trip Generation trigger** indicating that, as part of Steps Two through Five of the TIA process, the Design Review and Network Impact components should be completed. For reference, the completed Screening Form is provided in **Appendix B**.

Table 1-1. Transportation Impact Assessment (TIA) Screening Triggers

	TIA TRIGGERS SATISFIED			
Next Step of the TIA Process	Trip Generation	Location	Safety	
Design Review and Network Impact	Yes	No	No	

2 SCOPING

2.1 SCREENING FORM

The completed Screening Form is provided in Appendix B.

2.2 DESCRIPTION OF PROPOSED DEVELOPMENT

This Transportation Impact Assessment (TIA) has been prepared in support of the Site Plan Control Application for the proposed development at 2571 Lancaster Road at the Enbridge Ottawa Operation Centre.

The 2571 Lancaster Road site is a currently developed skating rink (The Minto Skating Centre) in the Alta Vista area of the city. The skating rink is slated to be demolished starting in the fall of 2021. It is located mid block on Lancaster Road. The property consists of approximately 16,663 m² of land which is currently zoned as a Light Industrial Zone (IL). **Figure 2-1** illustrates the Study Area Context.

The draft site plan, attached as **Appendix C**, includes $3,598 \text{ m}^2$ of office/operation centre space, 689 m^2 of warehouse space, and 528 m^2 of shop space for a total building gross floor area of $5,704 \text{ m}^2$. It is noted that the proposed Shop space is not intended for typical commercial use, but will be used by Enbridge field staff primarily for the storage of tools, equipment and parts needed for work in the filed; staff would typically access this space at the start of their shift in the morning prior to leaving for the field and return at the end of the day. Compared with the 189 parking spaces currently available on site for the Skating Centre, a total of 289 parking spaces is being proposed on site consisting of 98 spots for staff parking, among which 6 are barrier-free spots, and 191 spots for equipment and fleet vehicles. The existing two site accesses on Lancaster Road will remain as the access/egress points to the proposed development. No additional new access is being proposed.

The facility will be built as a single phase with an estimated date of completion in 2022 and full occupancy in mid 2023.



Figure 2-1. Area Context Plan

2.3 EXISTING CONDITIONS

2.3.1 ROADWAYS AND PEDESTRIAN / CYCLING FACILITIES

The five existing roadways that the TIA will consider are Lancaster Road, Russell Road, Walkley Road, St. Laurent Boulevard, and Smyth Road. These roads are all designated Full Loads urban truck routes identified on the Ottawa Urban Truck Routes map (March 2017) under the jurisdiction of the City of Ottawa. The road classification for City of Ottawa roadways are defined in the City of Ottawa Official Plan, 2013, Volume 1, Section 7, Annex 1 Road Classifications and Rights-of-Way.

Lancaster Road is an urban collector road that generally runs in a north-south alignment with a posted speed limit of 50 km/h. It has one traffic lane in each direction. The right-of-way adjacent to the proposed development is approximately 18 metres. Street parking is prohibited on both sides of the road expect for a short section on the west side adjacent to the subject development site where parking restriction is only in effect during December 1 to March 21.

Russell Road is an urban arterial road that runs north-south with a posted speed limit of 50 km/h. Russel Road is a four-lane road on the section between Smyth Road and St. Laurent Boulevard, transitions to a two-lane configuration from St. Laurent Boulevard to Lancaster Road and widens to four-lane when approaching Walkley Road. The existing right-of-way width varies along the section between Smyth Road and Walkley Road, ranging from approximately 22 meters to 27 metres. The Official Plan reserves a 37.5 metre right-of-way in the study area.

Walkley Road is an urban arterial road that runs east-west with two traffic lanes in each direction for a total of four lanes throughout the study area. The posted speed limit transitions from 50 km/h to 80km/h eastbound between Russell Road and Lancaster Road, while the westbound remains as 50 km/h on this section of road. The existing right-of-way width is approximately 30 metres. The Official Plan reserves a 44.5 metre right-of-way in the study area.

St. Laurent Boulevard is an urban arterial road that runs north-south with a posted speed limit of 50 km/h. It has two traffic lanes in each direction. The existing right-of-way in the study area is approximately 27 metres, with no right-of-way protection south of Smyth Road.

Smyth Road is an urban arterial that runs east-west with a posted speed limit of 50 km/h. It has two traffic lanes in each direction for a total of four lanes throughout the study area. The existing Right-of-Way in the study area is approximately 26 metres, which is the same as the reserved amount in the Official Plan.

The existing pedestrian and cycling facilities providing direct connections to the proposed development are shown in **Figure 2-2**. In addition to multiple pathways linking Lancaster Road to Southvale Crescent, the available pedestrian and cycling facilities in the vicinity of the development site along Lancaster Road include:

- 1.6 metres concrete sidewalk on both sides of the road
- 1.5 metres curb cycle track on both sides of the road starting Walkley Road and terminating near 2500 Lancaster Road
- The northern section does not have a sidewalk on the northern curb for about a 620m section



Figure 2-2: Bicycle and Pedestrian Facilities

2.3.2 INTERSECTIONS

The TIA will consider two (2) intersections as described in Table 2-1.

Table 2-1. Description of Study Area Intersections

INTERSECTION (DESCRIPTION)

Lancaster Road/Smyth Road and Russell Road/St. Laurent Boulevard is a signalized intersection.

North Approach: One left turn lane, two through lanes, one channelized right turn lane

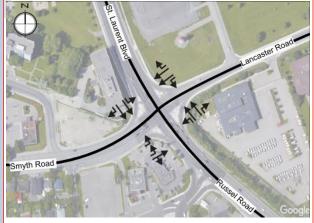
East Approach: One left turn lane, two through lanes, one channelized right turn lane

South Approach: One left turn lane, two through lanes, one channelized right turn lane

West Approach: Two left turn lanes, one through lane, one channelized right turn lane

Pedestrian/Bicycle: Bicycle lanes and pedestrian crossings on all legs

LANE CONFIGURATION



INTERSECTION (DESCRIPTION)

LANE CONFIGURATION

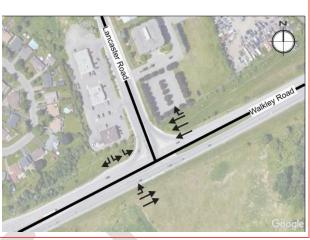
Walkley Road and Lancaster Road is a signalized intersection.

North Approach: Two left turn lanes, one channelized right turn lane

East Approach: three through lanes, one channelized right turn lane

West Approach: One left turn lane, two through lanes

Pedestrian/Bicycle: Pedestrian crossings on all legs



2.3.3 DRIVEWAYS

There are several private accesses existing along Lancaster Road within 200 m from the development site.

- Three accesses to 2525 Lancaster Road (Ottawa Athletic Club) north of the development site. The south
 access to this property is approximately 15 m from the proposed northern access of the subject
 development site.
- Four accesses to the IL site at 2615 Lancaster Road south of the development site. The north access to this
 property is approximately 45 metres from the proposed southern access of the subject development site.
- One access to a surface parking lot at 2510 Lancaster Road across street from the development site.
- One access to 2540 Lancaster Road (Canada Blood Services) on the opposite side of Lancaster Road from the development site.
- Two accesses to 2600 Lancaster Road (mobility equipment supplier) across Lancaster Road from the development site.
- Four accesses to a general employment site at 2620 Lancaster Road south of the development site across Lancaster Road.

2.3.4 TRANSIT FACILITIES

OC Transpo does not provide transit service along Lancaster Road. The nearest transit stops are on Southvale Crescent servicing Route #46 which goes to Hurdman O-Train Station and Billings Bridge Station, and connects to the SE Transitway. Route 46 is a Local Route that operates seven days a week with a 15-minute frequency during peak hours. There is an existing walkway connecting the proposed development at 2571 Lancaster Road to the bus stops on Southvale Crescent (i.e. Stop #0642, #7299 etc.). There are four bus stops within 400 metres of the proposed development that all service Route #46 and are on Southvale Crescent:

- Bus Stop #0642 and Bus Stop #7299 are both within approximately 250 metres of the proposed site,
- Bus Stop #7298 and Bus Stop #0641 are both within approximately 400 metres of the proposed site.

There Additional Local Routes are available on the adjacent roadways of the proposed development site, including

- Route #46 and Route #48 on Russell Road:
- Northbound Transit Stop #7294, #7293, #7295, #8281
- Southbound Transit Stop #7297, #7296
- Route #47 Walkley Road:
- Eastbound Transit Stop #6899
- Westbound Transit Stop #6898

Route #47 is a local route that operates from Hawthorne to St-Laurent O-Train Station, while Route #48 (also local) runs between Elmvale S.C. and Hurdman O-Train Station, and like Route #46, also connects with the SE Transitway.

Route #40 runs along St Laurent Boulevard that provides 15-minute service from 6 am to 6 pm on weekdays and operates seven days per week.

Figure 2-3 highlights all OC Transpo bus routes on adjacent roadways in close proximity of the proposed development.



Figure 2-3: OC Transpo Bus Routes (Source: OC Transpo Map Network Map)

2.3.5 AREA TRAFFIC MANAGEMENT MEASURES

The existing area traffic management measures identified adjacent to the proposed development site include:

- Centre medians near the intersections with Walkley Road and St. Laurent Boulevard as well as where the road curves near 2400 Lancaster Road.
- Sidewalks are on both sides of Lancaster Road.
- Lancaster Road has curbside bike lanes on both sides of the southern section with painted bike sharrows on the northern section.
- Pedestrian crossings on all study intersections.

2.3.6 PEAK HOUR TRAVEL DEMANDS

The TRANS Committee was established to co-ordinate transportation planning efforts among various planning agencies located within the National Capital Region. In the most recent TRANS Trip Generation Manual, an employment mode share by district was developed. For this analysis, the employment mode share of the Alta Vista district was taken as the existing mode share. A map of the district area is provided in **Appendix D**. The TRANS mode share for the Alta Vista Area is summarized in **Table 2-2**.

TRAVEL MODE	AM PEAK PERIOD (7:00 A.M. – 9:30 A.M.)	PM PEAK PERIOD (3:30 P.M. - 6:00 P.M.)	
Auto-Driver	69%	69%	
Auto-Passenger	7%	7%	
Transit	18%	18%	
Bicycle	3%	3%	
Walk	3%	3%	

Table 2-2. Existing Employment Mode Share – Alta Vista District

Source: 2021 TRANS Trip Generation Manual

Reviewing the mode share for the new development, the majority of users are expected to use their personal vehicles to commute. During both AM and PM peak hour periods, auto-driver and auto-passenger modes account for between 76% of the total vehicles that are travelling to and from the aforementioned TAZs. The remaining 24% are split between transit, bicycle, walk, or other modes of transportation.

2.3.7 FIVE-YEAR COLLISION HISTORY

The boundary road for the proposed development is Lancaster Road between Walkley Road and Russell Road. Using the collision history from the City of Ottawa Open Data, WSP reviewed the number and types of collisions (January 1, 2015 through December 31, 2019) on the boundary road. More recent and detailed five-year collision data will be requested from the City in support of a more thorough collision review. **Table 2-3** summarizes the five-year collision history on the boundary road.

Table 2-3. Five Year Collision History Summary

LOCATION	SUMMARY	TRENDS	
Segment : Lancaster Road between Walkley Road and Russell Road	17 crashes on Lancaster Road segment between northern intersection with Russell Road to the southern intersection with Walkley Road.	There were no trends for the collision on the segments as they varied in location throughout the road segment and impact type.	
Intersection: Lancaster Road & Walkley Road	41 crashes at this location with no fatalities. Out of the 41 collisions, 34 were classified as Property Damage Only while the remaining 7 were classified as Non-Fatal Injury.	More than half of the collisions were rear-end collisions. In addition, majority of the collisions occurred in daylight with dry surface condition and clear environment.	
Intersection: Smyth Road/Lancaster Road & St. Laurent Boulevard/Russell Road	96 crashes at this location with no fatalities. Out of the 96 collisions, 82 were classified as Property Damage Only while the remaining 14 were classified as Non-Fatal Injury.		

2.4 PLANNED CONDITIONS

2.4.1 CHANGES TO THE STUDY AREA TRANSPORTATION NETWORK

Based on the City of Ottawa's Construction and Infrastructure projects, the only major project near the adjacent roads expected to commence within three to five years is the resurfacing of **Walkley Road**. This may impact the traffic entering and existing Lancaster Road. However, this will be dependent on the expected build-out of the proposed development and should be further reviewed.

The City of Ottawa's Transportation Master Plan (2013) includes proposed transit improvements as part of the 2031 Network Concept of Rapid Transit and Transit Priority Network. They include:

- Walkley Road and Russell Road have a proposed at-grade BRT service connecting Bayshore Station to St. Laurent Station
- **St. Laurent Boulevar**d is proposed as a designated transit priority corridor with transit signal priority and queue jump lanes implemented at locations between Russell Road and Walkley Road

2.4.2 OTHER STUDY AREA DEVELOPMENTS

Two developments are noted in the City of Ottawa's Development Application Search tool that could have an influence on the study area:

- 2790 Lancaster Road (App# D07-12-20-0177): The City of Ottawa received a site plan control application for the construction of a 1-storey light industrial building.
- 2380 Walkley Road (App# D07-12-19-0164): The City of Ottawa received a site plan control application for the renovation of an existing building and construction of a new parking area.

2.5 STUDY AREA

The limits for the Transportation Impact Assessment (TIA) study area are shown in Figure 2-4.



Figure 2-4: Study Area

2.6 TIME PERIOD

The time periods identified for the traffic analysis are:

- Weekday AM Peak Hour: 7:15 a.m. to 8:15 a.m.
- Weekday PM Peak Hour: 4:00 p.m. to 5:00 p.m.

These are consistent with the AM and PM peak hours identified in the turning movement counts that were collected at the two study intersections dated between 2018 and 2020.

2.7 HORIZON YEARS

The proposed facility is expected to be completed in one phase with a target build-out year of 2022 and full occupancy in mid 2023. In accordance with the TIA Guidelines, the following horizons will be considered for analysis:

- 2023, which represents the anticipated buildout horizon,
- 2028, which represents the buildout year plus five years.

2.8 EXEMPTIONS REVIEW

Based on the review of the development and network conditions, the following elements shown in **Table 2-4** qualify for an exemption from this Transportation Impact Assessment.

Table 2-4. Exemptions Summary

MODULE ELEMENT		EXEMPTIONS		
DESIGN REVIEW CO	MPONENT			
4.1 Development	4.1.2 Circulation and Access	Not Exempted. This element is only required for site plans.		
Design	4.1.3 New Street Networks	Exempted This element is only required for plans of subdivision.		
	4.2.1 Parking Supply	Not Exempted. This element is required for site plans.		
4.2 Parking	4.2.2 Spillover Parking	Exempted This element is only required for site plans where parking supply is 15% below unconstrained demand.		
NETWORK IMPACT	COMPONENT			
4.5 Transportation Demand Management	All Elements	Not Exempted Required for site plans expected to have more than 60 employees on location at any given time.		
4.6 Neighborhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Not Exempted Required when the development relies on local or collector access and total volumes exceed ATM capacity threshold.		

MODULE	ELEMENT	EXEMPTIONS
4.8 Network Concept		Exempted
		Required when proposed development generates more than
		200 person-trips during the peak hour in excess of the
		equivalent volume permitted by established zoning.

3 FORECASTING

3.1 DEVELOPMENT-GENERATED TRAVEL DEMAND

3.1.1 TRIP GENERATION

Select Base Trip Generation. Due to the nature of operations at the Enbridge Ottawa Operation Centre, the following base principals were assumed, through discussion with Enbridge about daily operations and head counts.

AM Peak Hour:

- 98 staff (field and office staff) arrive
- 54 field staff depart (heavy vehicles)

PM Peak Hour:

- 54 field staff arrive (heavy vehicles)
- 98 staff (field and office staff) depart

Estimate Total Development-Generated Person-Trips. Based on the base principal's trip generation estimates presented above, the peak hour person-trips generated by this development are summarized in Table 3-1.

 Table 3-1: Estimate Total Development-Generated Person-Trips Generated

	TIME	TOTAL PERSON-	DIRECTIONAL DISTRIBUTION			
BUILDING	PERIOD	TRIPS GENERATED	% ENTER	% EXIT	ENTER	EXIT
Enbridge Ottawa Operation Centre	AM Peak Hour	152	67	33	98	54
	PM Peak Hour	152	33	67	54	98

Identify Existing Mode Share. The existing peak hour mode share at a place of employment was identified from the most recent TRANS Origin-Destination Survey (2011) and is shown in **Table 3-2**.

Table 3-2: Existing Mode Share (Trans O-D Survey)

PEAK HOUR AUTO-DRIVER		AUTO- PASSENGER	TRANSIT	BICYCLE	WALK
AM Peak Hour	69%	7%	18%	3%	3%
PM Peak Hour	69%	7%	18%	3%	3%

Future Mode Share Targets. The future mode share targets are provided in Table 3-3.

Table 3-3: Future Travel Mode Share Targets

TRAVEL MODE	COMMUTER TARGET MODE SHARE	RATIONALE
Transit	10%	Staff Trips: The future mode share considers the most conservative approach
Walking	0%	and assumes that most trips generated by employees will be by vehicle (auto
Cycling	0%	driver) and that it is noticeably higher than the district average for employee auto driver mode share.
Auto-Passenger	5%	
Auto-Driver	85%	Fleet Trips: All trips generated by field staff will be in fleet vehicles by
Other	0%	nature.

Project Development Trips by Mode. Based on the estimated future employment mode share, the number of person trips to and from the development at 2571 Lancaster Road are shown in **Table 3-4**.

Table 3-4: Trips Generated by Mode

MODE	TRIPS GENERATED			
MODE	AM Peak Hour	PM Peak Hour		
Auto Driver	137	137		
Auto Passenger	5	5		
Transit	10	10		
Bicycle	0	0		
Walk	0	0		

Trip Reduction Factors. No trip reduction factors were considered for this development based on the existing operations at the Minto Skate Centre. This will increase volumes conservatively and account for a scenario where all field and office staff enter and exit during the peak hour.

3.1.2 TRIP DISTRIBUTION

Currently, there are two vehicle accesses to the development, both from Lancaster Road. Vehicles will approach and depart from the proposed development following existing travel patterns. The trip distribution for the proposed development is presented as follows:

Table 3-5: Trip Distribution

DIRECTION	DIRECTIONAL SPLIT (%)					
DIRECTION	AM Peak Hour – Inbound	AM Peak Hour – Outbound	PM Peak Hour – Inbound	PM Peak Hour - Outbound		
North	14%	24%	17%	27%		
West	63%	50%	55%	16%		
East	6%	13%	5%	42%		
South	17%	13%	23%	15%		

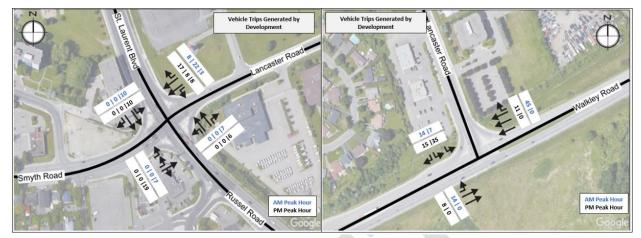
3.1.3 TRIP ASSIGNMENT

Trips were assigned to adjacent the transportation network and have been based upon a good understanding of existing travel patterns as identified from a review of intersection turning movement counts; including those at the following locations:

- Lancaster Rd/Smyth Rd/St. Laurent Blvd.
- Lancaster Rd/Walkley Rd
- The distribution of generated vehicle trips from the development at 2571 Lancaster Road is shown in Table 3-6.

Table 3-6: Trip Assignment by Intersection and Peak Hour

INTERSECTION	TRIP ASSIGNMENT (%)					
INTERSECTION	AM Peak Hour – Inbound	AM Peak Hour – Outbound	PM Peak Hour – Inbound	PM Peak Hour - Outbound		
Lancaster/Smyth/St. Laurent	29%	62%	64%	40%		
Lancaster/Walkley	71%	38%	36%	60%		



The overall development-generated demand at the two nearby intersections is shown in Figure 3-1.

Figure 3-1: Vehicle Trips Generated by Development

3.2 BACKGROUND NETWORK TRAFFIC

3.2.1 CHANGES TO THE BACKGROUND TRANSPORTATION NETWORK

There are no road projects identified along the border streets in the study area. Furthermore, neither the Ottawa Pedestrian Plan (2013) nor the Ottawa Cycling Plan (2013) nor the Ottawa Cycling Plan (2013) identify connectivity or infrastructure improvements along Lancaster Road.

3.2.2 GENERAL BACKGROUND GROWTH RATES

A 0.6% background growth rate was calculated based on a review of the 10-year historical screenline data across Screenline #54 (Alta Vista Hydro). For a more conservative approach, a background growth rate of 1% was applied for 2023 and 2028. Data from the 2031 TRANS model was acquired from the City of Ottawa to confirm the growth rate. The trip growth from the 2011 O-D Survey to the 2031 TRANS model indicated a similar growth rate.

The background traffic volumes for 2023 and 2028 are provided in Figure 3-2.

3.2.3 OTHER AREA DEVELOPMENTS

As noted in Section 2.4.2, there are two active development applications near the proposed development at 2571 Lancaster Road. The trips generated from these developments are expected to be accounted for through application of the background traffic growth rate and through the any trips captured in the 2020 traffic counts that were generated by the Minto Skating Centre. Therefore, specific trips were not added for these two developments.

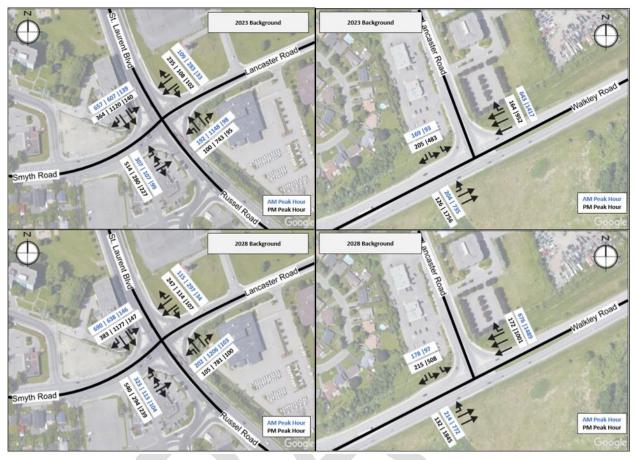
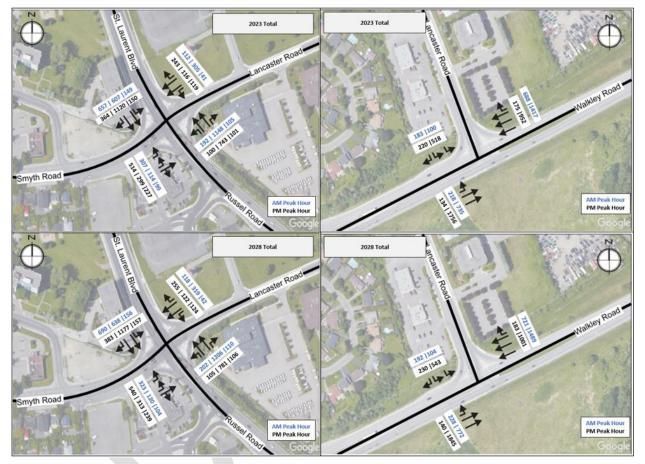


Figure 3-2: 2023 and 2028 Background Volumes

3.3 DEMAND RATIONALIZATION

3.3.1 DESCRIPTION OF CAPACITY ISSUES

Total traffic volumes for the 2023 and 2028 planning horizons were estimated by and adding trips generated by the proposed development and other area developments to the background traffic. The estimated total traffic volumes are shown in **Figure 3-3**.





Recent studies in this area have noted that the intersections at the boundary roads are operating well and under capacity. It is anticipated that similar results will be observed as part of this Transportation Impact Assessment.

3.3.2 ADJUSTMENTS TO DEVELOPMENT GENERATED DEMANDS

There are no proposed adjustments to development generated demands.

3.3.3 ADJUSTMENTS TO BACKGROUND NETWORK DEMANDS

There are no proposed adjustments to background network demands.

4 STRATEGY

4.1 DEVELOPMENT DESIGN

4.1.1 DESIGN FOR SUSTAINABLE MODES

The TDM-supportive Development Design and Infrastructure Checklist includes two checklists, one for non-residential developments and one for residential developments. The non-residential development checklist was completed to assess the opportunity to implement facilities that are supportive of sustainable modes. The completed checklist is attached as **Appendix E**.

4.1.2 CIRCULATION AND ACCESS

The proposed site plan (**Appendix C**) provides a continuous drive movement through the property between the two access points for service vehicles and delivery trucks. Turning movements in and out of the property are also provided in **Appendix F**. Additionally, the layout of the site successfully separates passenger vehicles (via the south entrance) and operational vehicles (via the north entrance). The site circulation was assessed to confirm suitability of the layout for a variety of design vehicles. The results are provided in the following table.

Table 4-1. Swept Path Assessment

DESIGN VEHICLE	VEHICLE REPRESENTING	FINDINGS
MSU (<i>TAC 1999</i>)	Dump Truck	The north proposed entrance configuration on Lancaster Road accommodates the movements of an MSU design vehicle without impacting any built features or parking spaces
WB-20M (ASSHTOM 2011)	Delivery Vehicle	To enter the site at the north access making a northbound right turning movement, the WB-20M design vehicle will encroach completely into the opposite lane to complete the maneuver.
53-foot with Axles Pulled-Up (Custom)	Delivery Vehicle Moving Truck	Circulation: The north proposed entrance configuration on Lancaster Road accommodates the movements of the custom truck without impacting any built features or parking spaces Access to Loading Dock: A custom 53-foot truck with axles pulled- up will be able to maneuver to and from the loading dock without conflicting with nearby parking spaces.
LZ RAS-K-EAE (DE)	Equipment Truck	The proposed configuration of the parking lot accommodates the movements of the equipment trucks that are anticipated to park towards the back of the parking lot.
Enbridge F-350 (Custom)	Fleet Pick-Up Truck	The proposed configuration of the parking lot accommodates the movements of the fleet pick-up trucks that are anticipated to park in the parking lot overnight.

4.1.3 NEW STREET NETWORKS

This section was exempted in the Transportation Impact Assessment Scoping Report submitted on July 13, 2021 and approved by the City of Ottawa on July 28, 2021. The approved exemptions table is found in **Section 2.8**.

4.2 PARKING

4.2.1 PARKING SUPPLY

The proposed development parking requirements, based on its location, will be assessed in accordance with the Suburban Area (Area 'C') for minimum parking requirements as part of Schedule 1A to the City of Ottawa's Zoning By-Law 2008-250. The Zoning By-Law requires an office space in Area 'C', provide a minimum parking rate of 2.4 spaces per 100 m² of gross floor area (*Table 101, Row N59, Column IV*). The Zoning By-Law indicates that the warehouse requires a minimum parking rate of 0.8 spaces per 100 m² of gross floor area (*Table 101, Row N59, Column IV*). The Zoning By-Law further requires a minimum bicycle parking rate of 1 space per 1,500 m² of gross floor area (*Table 111A, i, II*).

As stated in Section 2.2, a total of 288 parking spaces are planned. However, 191 spaces are for equipment and fleet vehicles:

- Backhoe/Mini Excavator 15'x25': 24 spaces
- Small Equipment Attachment 12'x20': 15 spaces
- Small Equipment Trailer 12'x20': 23 spaces
- Large International / Dump Truck 15'x25': 32 spaces
- Fleet Car / Van / Pickup Truck 12'x20': 96 spaces

The remaining 98 spaces are planned for staff parking which includes 6 barrier free parking spots. The minimum parking supply requirements for this development compared wit the proposed parking supply are highlighted in the following table.

Table 4-2. Minimum Bylaw Requirements for Parking and Proposed Parking Supply

ТҮРЕ	REQUIRED	CALCULATION	SITE PLAN	COMPLIANCE WITH BY-LAW
Vehicle: Office	92	3,828 m ² * 2.4 spaces per 100 m ² of gross floor area	- 98	Meets the minimum
Vehicle: Warehouse	6	710 m ² * 0.8 spaces per 100 m ² of gross floor area	98	requirements of the Zoning By-Law
Bicycle	3	4,123 m ² * 1 space per 1,500 m ² of gross floor area	3	Meets the minimum requirements of the Zoning By-Law

4.2.2 SPILLOVER PARKING

This section was exempted in the Transportation Impact Assessment Scoping Report submitted on July 13, 2021 and approved by the City of Ottawa on July 28, 2021. The approved exemptions table is found in **Section 2.8**.

4.3 BOUNDARY STREETS

4.3.1 LANCASTER ROAD

Lancaster Road is a collector with an existing 20-metre protected right-of-way in the study area adjacent to the proposed development site. The existing cross-section (Figure 4-1) includes a paved area approximately 11m wide with sidewalks in each direction with no median. The right-of-way allocation does not meet the minimum width requirements of 22m for a neighbourhood collector street as per the City of Ottawa's Designing Neighbourhood Collector Streets (2019) document.

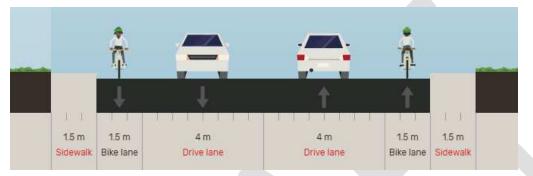


Figure 4-1. Lancaster Road - Existing Cross-Section

4.3.1.1 MOBILITY

The segment of Lancaster Road within the study area is identified as an Urban Employment Area in the City of Ottawa's Official Plan (2013), Schedule B (Urban Policy Plan). The development is not proposing any changes to the boundary street. The resulting MMLOS targets and segment scores are indicated in **Table 4-3**.

Table 4-3. Segment MMLOS

	PLOS	BLOS	TLOS	TKLOS	VLOS
Target	С	Е	D	В	VLOS Not
Status Quo	Е	В	-	В	Reported for Segments

The PLOS 'C' is not achieved, in part, because of the width of the sidewalks are narrow. If the sidewalks were widened to 2m, a PLOS 'C' could be attained.

4.3.1.2 *SAFETY*

Historical crash records for the study area were obtained from the City of Ottawa for the five years between January 2015 and December 2019. The TIA Guidelines indicate that patterns with six or more crashes should be identified. In this timeframe, here were three collisions along the roadway segment on Lancaster Road, and thus do not meet the TIA Guideline criteria.

4.4 ACCESS INTERSECTION

4.4.1 LOCATION AND DESIGN OF ACCESS

There are two access points proposed for this development and are indicated in the Site Plan (Appendix C), both on Lancaster Road.

Both entrances are proposed as full movements access intersections. While both entrances have been tested for large delivery trucks, it is anticipated that the north access will be the preferred access point for large delivery trucks while the south access will be the preferred access point for staff parking.

A design compliance check was carried out for each of the two accesses following guidelines prepared by the City of Ottawa and the Transportation Association of Canada's Geometric Design Guidelines for Canadian Roads (2017). The design compliance check is summarized in **Table 4-4** with elements not in compliance with the requirements in red.

Table 4-4. Access Intersection Design Elements

DESIGN ELEMENTS	MINIMUM REQUIRED	SOUTH ACCESS	NORTH ACCESS
Access Type	-	Full Movement	Full Movement
One-way vs. Two-way	<25 vpd = one-way driveway <750 vpd = two-way driveway	<200 peak hour trips 250-300 daily trips Two-Way	<200 peak hour trips 250-300 daily trips Two-Way
Entrance Width	9.0 – 15.0 (TAC 2017)	9.0m	9.0m
Corner Clearance	70m to traffic signals (TAC 2017)	>70m	>70m
Right Turn Radius	9.0m – 15.0m	5.0m	5.0
Sight Distance (Intersection	Left turn:130m (TAC 2017)	No obstruction	No obstruction
with No Control)	Right turn: 110m (TAC 2017)	No obstruction	No obstruction
Throat Length	15m (TAC 2017)	10m	15m+
Angle of Intersection	At or near 90°	Access to Lancaster at intersection is 90°	Access to Lancaster at intersection is 90°
Proximity to Adjacent Driveways	3m between driveways (TAC 2017)	Private driveway 10 m north of access	Private driveway 35 m south of access

DESIGN ELEMENTS	MINIMUM REQUIRED	SOUTH ACCESS	NORTH ACCESS	
Pedestrian Crossing Consideration	Ottawa Standard Drawing SC7.1 (Curb Return Private Entrance - Unsignalized)	12.0m pedestrian crossing Ottawa Standard SC7.1 (Curb Return at a Private Entrance)	12.0m pedestrian crossing Ottawa Standard SC7.1 (Curb Return at a Private Entrance)	
Cycling Crossing Consideration	Large curb return radii with narrow driveway to minimize crossing distance	12.0m cycling crossing	12.0m cycling crossing	

4.4.2 INTERSECTION CONTROL

An all-way stop minimum volume warrant for the minor road was conducted in accordance with the Ontario Traffic Manual (OTM) Book 5 for both proposed accesses under the future total planning horizon. Both of the following conditions must be met to warrant a stop at the two access points:

Table 4-5. All-Way Stop (Minor Road) Requirements

	FUTURE TOTAL PLANNING HORIZON		
CONDITION	North Access South Access		
Total vehicle volume on all intersection approaches exceeds 350 for the highest hour recorded	Estimated Volumes: AM: 460; PM: 460		
Volume split does not exceed 75/25 for three-way control or 65/35 for four-way control. Volume is defined as vehicles only	Volume Splits: AM: 89/11; PM: 90/10		

While the total vehicle volume is expected to exceed 350 for the highest hour recorded, the volume split is anticipated to greatly exceed 75/25 for a three-way control intersection. Therefore, a stop control on the minor road was not warranted, and accordingly a traffic signal warrant was not analyzed.

Pedestrian accommodations at the accesses should follow the City of Ottawa's Standard Drawing SC7.1 for Curb Return Entrances utilizing the Private / Unsignalized entrance option which includes a continuous depressed sidewalk across the access as shown below.

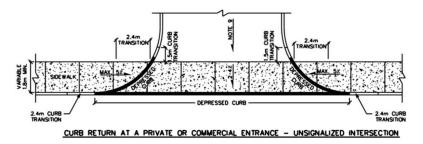


Figure 4-2. City of Ottawa, Standard Drawing SC7.1 Curb Return Entrances, Private / Unsignalized (March 2017)

Signage options for the two driveways are provided below:

North Access: Wayfinding signage indicating that this is the entrance nearest the delivery truck entrance.

South Access: Wayfinding signage indicating that this is the entrance with immediate access to the employee and visitor parking.

4.4.3 INTERSECTION DESIGN

The City of Ottawa's Multi-Modal Level of Service (MMLOS) Guidelines (2015) are to be applied at signalized intersections only. The proposed site accesses are both uncontrolled and do not warrant an MMLOS evaluation.

4.5 TRANSPORTATION DEMAND MANAGEMENT

4.5.1 CONTEXT FOR TDM

Transportation Demand Management (TDM) describes a broad range of policies, programs and services designed to reduce the demand for vehicle use by influencing individual travel behaviour and providing expanded options. As mentioned in Section 3.1.1, the forecasted commuter mode share is: 85% auto driver, 10% transit and 5% auto passenger.

The forecasted auto driver mode share is much greater than the existing and target mode share set out in the City's Official Plan as well as the existing employment mode share to the Alta Vista district.

4.5.2 NEED AND OPPORTUNITY

The existing road network has available capacity should the mode share targets not be met. The anticipated Vehicle Level of Service (VLOS) in the area is provided in the Multi-Modal Level of Service (MMLOS) analysis (Section 4.9.2.2).

4.5.3 TDM PROGRAM

The TDM-supportive Development Design and Infrastructure Checklist was completed to assess the opportunity to implement facilities that are supportive of sustainable modes. The completed checklist is attached to this report as **Appendix G**.

The sustainable mode share at the proposed development can be maximized by including the following TDM measures and amenities:

- Exterior bicycle rack under cover near main entrance
- Subsidize or reimburse monthly transit pass purchases by employees
- Provide emergency ride home service to non-driving commuters
- Provide a dedicated ride-matching portal at ottawaridematch.com

4.6 NEIGBOURHOOD TRAFFIC MANAGEMENT

4.6.1 ADJACENT NEIGHBOURHOODS

The development will increase traffic traveling along Lancaster Road, which is classified as a collector road in the City of Ottawa's Official Plan (2013). The traffic volume threshold provided in the TIA Guidelines (2017) for the various classifications of road are:

- Local Road: 1,000 vehicles per day or 120 vehicles per peak hour
- Collector Road: 2,500 vehicles per day or 300 vehicles per peak hour
- Major Collector Road: 5,000 vehicles per day or 600 vehicles per peak hour

As shown in **Table 4-6**, the Collector Road thresholds are exceeded for existing conditions. Additionally, both future background and future total exceed the Collector Road threshold. Thus, while the thresholds are exceeded for all scenarios, it is not anticipated that the new trips generated from the proposed development will change the results.

Table 4-6. Collector Traffic Volume Thresholds

PLANNING HORIZON	DAILY THRESHOLD 2,500 VPD	PEAK HOUR THRESHOLD 300 VPH	
2021 Existing	3,200	460	
2023 Background	3,264	469	
2028 Background	3,430	493	
2023 Total	3,401	606	
2028 Total	3,567	630	

4.7 TRANSIT

4.7.1 ROUTE CAPACITY

The trip generation estimates have assumed a 10% transit mode share given the nature of the employment and services provided at the new development. From **Section 3.1.1**, 10 transit trips are expected during each of the peak hours.

The nearest bus station to the development is located on Southvale Crescent (200m walking distance). Route 46 passes four times in each direction during the peak hours and connects to the Hurdman LRT Station. The next closest transit route to the development is located at the intersection of Southvale Crescent and Russell Road (650m walking distance). Route 48 passes twice an hour in each direction during the peak hours and, like Route 46, connects to the Hurdman LRT Station. All other bus routes along St. Laurent Boulevard and Walkley Road are located at a walking distance at or greater than one kilometre.

Given the frequency of the nearby routes and the number of trips expected from the new development, the capacity of the existing transit routes is likely sufficient.

4.7.2 TRANSIT PRIORITY

Neither Lancaster Road nor Southvale Crescent have been identified as a Transit Priority Corridor in the City's Transportation Master Plan (2013). As mentioned in **Section 4.7.1**, with the minimal amount of additional transit users, it is not anticipated that additional transit measures would be required.

However, St. Laurent Boulevard, which is located just over one kilometre from the new development, was identified as a Transit Priority Corridor (Isolated Measures) with a 44.5m protected right-of-way. Improvements, such as a complete street concept could prove beneficial for transit users willing to walk over a kilometre to the routes available on St. Laurent Boulevard.

4.8 REVIEW OF NETWORK CONCEPT

This section was exempted in the Transportation Impact Assessment Scoping Report submitted on July 13, 2021 and approved by the City of Ottawa on July 28, 2021. The approved exemptions table is found in **Section 2.8**.

4.9 INTERSECTION DESIGN

4.9.1 INTERSECTION CONTROL

The identification of appropriate intersection controls to serve future background and future total travel demands included an all-way stop control warrant, and a cursory review of transit priority measures. For this assessment we reviewed the 2028 total traffic volumes which would provide the worst-case scenario in terms of area traffic demands. The results are provided in **Table 4-7**.

The **all-way stop control warrant** was carried out in accordance with the Ontario Traffic Manual Book 5 (2000) methodology for all-way stop minimum volume warrant (minor roads) for the two accesses into the new development. Based on 2028 total traffic volumes, the warrant was not met for the unsignalized intersections analyzed.

While there is no transit service along Lancaster Road, there is service nearby on Russell Road and Southvale Crescent. Additionally, St. Laurent Boulevard has been identified in the City of Ottawa's Transportation Master Plan (2013) as a candidate for transit priority measures as part of the 2031 Ultimate Network. However, at this time, the intersections within the study area are not considered as immediate candidates for **transit priority measures**.

Table 4-7. Intersection Control Summary (2028 Total)

INTERSECTION	EXISTING CONTROL	TAC STOP WARRANT	ISOLATED TRANSIT PRIORITY	FUTURE CONTROL
North Access	Unsignalized	Not Warranted	Not Warranted	No change
South Access	Unsignalized	Not Warranted	Not Warranted	No change
Lancaster/Smyth/St. Laurent	Signalized	-	Not at this Time	No change
Lancaster/Walkley	Signalized	-	Not Warranted	No charge

4.9.2 INTERSECTION DESIGN

4.9.2.1 MULTI-MODAL LEVEL OF SERVICE ANALYSIS

A Multi-Modal Level of Service (MMLOS) analysis was carried in accordance with the methodology outlined in the City of Ottawa's MMLOS Guidelines (2015). The Guidelines state that intersection LOS measures are to be evaluated at signalized intersections. We have prepared a MMLOS analysis for the existing conditions (2021) and future total (2028) time horizon to provide a comparison between the baseline and future condition (beyond the development period). An intersection MMLOS analysis was completed for the two intersections within the study area, and a segment MMLOS was completed for Lancaster Road.

From Schedule B and E of the City's Official Plan, Lancaster Road is a collector road within an Urban Employment Area. However, the two intersections within the study area are both bordering different land uses:

- Lancaster Road/ Smyth Road /St. Laurent Boulevard:
- Lancaster Road: Collector road within an Urban Employment
- Smyth Road: Arterial road within a General Urban Area

- St. Laurent Boulevard: Arterial Mainstreet
- Lancaster Rd/Walkley Rd:
- Lancaster Road: Collector road within an Urban Employment
- Walkley Road: Arterial road within a General Urban Area

For the intersection MMLOS analysis, the recommended targets for the intersections will be based on the leg of the intersection with the most rigorous evaluation criteria.

The intersection MMLOS results for the existing and future total conditions at the Lancaster/Smyth/St. Laurent intersection (**Table 4-8**) and the Lancaster/Walkley intersection (**Table 4-9**) indicate that pedestrian, bicycle and transit modes do not meet their target LOS for both intersections with the exception of transit LOS at Lancaster/Walkley in future total conditions.

Table 4.9. Intersection MMLOC Evicting	n and Euture Canditions	(Longester/Cmuth/Ct. Lourent)
Table 4-8. Intersection MMLOS Existing	g and Future Conditions	(Lancaster/Smyth/St. Laurent)

	PLOS	BLOS	TLOS	TKLOS	VLOS
Time Horizon	С	С	В	В	D
Existing (2021)	F	Е	F	А	С
Future Total (2028)	F	Е	F	А	D

Table 4-9. Intersection MMLOS Existing and Future Conditions (Lancaster/Walkley)

	PLOS	BLOS	TLOS	TKLOS	VLOS
Time Horizon	С	С	D	В	D
Existing (2021)	F	E	E	А	С
Future Total (2028)	F	Е	D	А	С

The **Pedestrian Level of Service** (PLOS) targets were not met. Following, the City of Ottawa's MMLOS Guidelines, the reported PLOS is governed by the worst of the PETSI (Pedestrian Exposure to Traffic) LOS and average pedestrian delay LOS which are reporting the same LOS 'F' for both intersections.

The **Bicycle Level of Service** (BLOS) target of 'C' could be met if separated bicycle facilities were provided on St. Laurent Boulevard and Walkley Road.

The **Transit Level of Service** (TLOS) target of 'D' was not met for the Lancaster/Smyth/St. Laurent intersection. However, the TLOS is met for the future total scenario at the Lancaster/Smyth intersection. This is a result of the change in Peak Hour Factor (PHF) that the City's TIA Guidelines prescribe for existing conditions (0.9) and future conditions (1.0).

The **Truck Level of Service** (TKLOS) target of 'B' and the **Vehicle Level of Service** (VLOS) targets of 'D' were met for both intersections. The detailed VLOS intersection performance analysis is provided in **Section 4.9.2.2**.

4.9.2.2 DETAILED INTERSECTION PERFORMANCE ANALYSIS

The existing and future conditions were analyzed based upon the peak hour traffic volumes presented in **Section 3**. The City of Ottawa's MMLOS Guidelines assigns the vehicle level of service (VLOS) based on ranges of volume to capacity ratio, as indicated in **Table 4-10**. The City's MMLOS Guidelines recommend targets for the VLOS based on their Official Plan Policy/Designation and Road Class and is a LOS D for the two intersections in the study area.

VLOS	VOLUME TO CAPACITY RATIO
Α	0-0.60
В	0.61 - 0.70
С	0.71 - 0.80
D	0.81 - 0.90
E	0.91 – 1.00
F	>1.00

Table 4-10. City of Ottawa MMLOS Guidelines, V/C Criteria

The intersections were analyzed using Synchro v. 11 following the analysis parameters in the TIA Guidelines. **Appendix H** contains the detailed Synchro analysis sheets.

EXISTING CONDITIONS

The existing (2021) intersection capacity analysis results are summarized in **Table 4-11**. Under existing conditions, all intersections operate with a VLOS that does not exceed the VLOS of 'D'.

INTERSECTION	ION INTERSECTION CONTROL	AM PEAK HOUR			PM PEAK HOUR		
INTERSECTION		V/C Ratio	VLOS	Critical Movement	V/C Ratio	VLOS	Critical Movement
Lancaster/Smyth/St. Laurent	Signalized	0.78	С	-	0.79	С	EBL = 1.01
Lancaster/Walkley	Signalized	0.63	В	-	0.74	С	-

Table 4-11. Intersection Capacity Summary for Existing Conditions (2021)

FUTURE BACKGROUND CONDITIONS

The 2023 future background intersection VLOS remain largely unchanged when compared to the existing conditions with the exception of the critical EBL turning movement from the existing conditions no longer being over capacity. As mentioned in **Section 4.9.2.1**, this is a result of the Peak Hour Factor (PHF) that the City's TIA Guidelines prescribes for existing conditions (0.9) and future conditions (1.0). When volumes do not significantly change, the future VLOS can be reported as improving since the PHF of 1.0 assumes peak spreading.

Overall, the intersections continue to operate below the target VLOS. The intersection capacity results are summarized in **Table 4-12**.

INTERSECTION	N INTERSECTION CONTROL	AM PEAK HOUR			PM PEAK HOUR		
INTERSECTION		V/C Ratio	VLOS	Critical Movement	V/C Ratio	VLOS	Critical Movement
Lancaster/Smyth/St. Laurent	Signalized	0.78	С	-	0.79	С	-
Lancaster/Walkley	Signalized	0.64	В	-	0.76	С	-

Table 4-12. Intersection Capacity Summary for Future Background Conditions (2023)

The intersection capacity results for the 2028 future background conditions are summarized in **Table 4-13.** The background growth does cause the VLOS of the Lancaster/Smyth/St. Laurent intersection to decrease from a LOS 'C' to LOS 'D'. However, the intersection continues to operate at or below the target VLOS of LOS 'D'.

Table 4-13. Intersection Capacity Summary for Future Background Conditions (2028)

INTERSECTION		AM PEAK HOUR			PM PEAK HOUR		
INTERSECTION	CONTROL	V/C Ratio	VLOS	Critical Movement	V/C Ratio	VLOS	Critical Movement
Lancaster/Smyth/St. Laurent	Signalized	0.81	D	-	0.82	D	-
Lancaster/Walkley	Signalized	0.67	В	-	0.79	С	-

FUTURE TOTAL CONDITIONS

The traffic operation results for the 2023 future total are unchanged from the 2023 future background scenario with all intersections below the target VLOS 'D'. The intersection capacity results are summarized in **Table 4-14**.

Table 4-14. Intersection Capacity Summary for Future Total Conditions (2023)

INTERSECTION	INTERSECTION CONTROL	AM PEAK HOUR			PM PEAK HOUR		
INTERSECTION		V/C Ratio	VLOS	Critical Movement	V/C Ratio	VLOS	Critical Movement
Lancaster/Smyth/St. Laurent	Signalized	0.78	С	-	0.79	С	-
Lancaster/Walkley	Signalized	0.64	В	-	0.76	С	-

The intersection results for the 2028 future total conditions are summarized in **Table 4-15**. The traffic operation results for the 2028 future total are unchanged from the 2028 future background scenario with all intersections at or below the target VLOS 'D'.

INTERSECTION	INTERSECTION CONTROL	AM PEAK HOUR			PM PEAK HOUR		
INTERSECTION		V/C Ratio	VLOS	Critical Movement	V/C Ratio	VLOS	Critical Movement
Lancaster/Smyth/St. Laurent	Signalized	0.81	D	-	0.82	D	-
Lancaster/Walkley	Signalized	0.67	В		0.79	С	-

Table 4-15. Intersection Capacity Summary for Future Total Conditions (2028)

4.10 SUMMARY OF IMPROVEMENTS INDICATED AND MODIFICATION OPTIONS

A summary of transportation improvements proposed as part of this Transportation Impact Assessment are presented as follows:

Development Design

- a) Provisions for sustainable modes on-site have been identified through the development design checklist. The proposed site plan could be improved to provide direct pedestrian access from the public sidewalk and potentially consider assigned parking spaces for carpools to encourage carpooling.
- b) A review of the design vehicle turning templates indicated that vehicles will be able to circulate around the site without impacting on built features.
- c) A review of the WB-20M design vehicle making a northbound right from Lancaster Road into the north access indicates that the WB-20M will encroach entirely into the oncoming southbound lane to complete the turning maneuvre.

Reference: Section 4.1

Parking

- a) The 98 proposed staff parking spaces satisfies the requirements of the Zoning By-Law (2018-250).
- b) Six of the 98 proposed staff parking spaces are barrier-free (three Type A and three Type B) which exceed the requirement of 4 barrier-free parking spaces.

Reference: Section 4.2

Boundary Street Design

a) There are no proposed changes to the cross-section of Lancaster Road adjacent to the proposed development.

b) The status quo MMLOS assessment of Lancaster Road resulted in the BLOS and TKLOS meeting the MMLOS targets for the Urban Employment policy area; while the PLOS did not achieve the target because of the narrow width of the existing sidewalks.

Reference: Section 4.3

Access Intersections

- a) North Access: The north access is proposed as full movement with unimpeded traffic flow along Lancaster Road. It provides gated access to the fleet parking area for Enbridge fleet vehicles. Generally, the access design meets the minimum requirements provided by the TAC Geometric Design Guide for Canadian Roads, however the right turn radius of 5.0m may not easily accommodate the larger fleet vehicles.
- b) South Access: The south access is proposed as full movement with unimpeded traffic flow along Lancaster Road. It provides public access to the staff parking area south of the main building. Generally, the access design meets the minimum requires provided by the TAC Geometric Design Guide for Canadian Roads, however the right turn radius of 5.0m may not accommodate the occasional heavy vehicle.

Reference: Section 4.4

Transportation Demand Management

a) The existing road network has available capacity should the anticipated mode share targets not be met.

Reference: Section 4.5

Neighborhood Traffic Management

a) The existing traffic volumes along Lancaster Road exceed the volume thresholds for both the collector road and major collector road designations. It is not anticipated that traffic volumes along Lancaster Road will decrease in the future planning horizons.

Reference: Section 4.6

Transit

a) The development is forecasted to generate a low number of transit trips during the peak periods. These trips can be accommodated by the existing transit service.

Reference: Section 4.7

Intersection Design

- a) Lancaster/Smyth/St. Laurent: No modifications are proposed. The Synchro analysis indicates that the intersection has reserve capacity during the existing and future planning horizons. The level of service for pedestrians, bicycles and transit were not achieved considering the existing or future traffic conditions.
- b) Lancaster/Walkley: No modifications are proposed. The Synchro analysis indicates that the intersection has reserve capacity during the existing and future planning horizons. The level of service for pedestrians, bicycles and transit were not achieved considering the existing or future traffic conditions.

Reference: Section 4.9



A CIRCULATION COMMENT / RESPONSE



COMMENT AND RESPONSE LOG

TO: File

FROM: Jordan Papazoglou, P. Eng., ing.

SUBJECT: 2571 Lancaster Road Transportation Impact Assessment (current to February 16, 2022)

DATE: February 16, 2022

SCOPING REPORT COMMENT / RESPONSE

WSP Submission July 13, 2021 City Comments Received July 28, 2021

Section 2.3.1 Roadways and Pedestrian/Cycling Facilities

- 1 Correct the section about Russell Road, the four-lane configuration changes south of St. Laurent, and it is unclear where the existing 18m ROW has been measured, as it is wider in the four-lane portion. Corrected in TIA Forecasting submission.
- 2 Walkley's speed limit transitions from 50 km/h to 80 km/h between Russell Road and Lancaster. Corrected in TIA Forecasting submission.
- 3 There is no ROW protection for St. Laurent Boulevard south of Smyth. Corrected in TIA Forecasting submission.
- 4 Smyth's existing ROW is approximately 26m. Corrected in TIA Forecasting submission.

Section 2.3.2 Intersections

5 Include the through lanes on the north approach of the Walkley/Lancaster intersection.
 Walkley / Lancaster is a T-intersection with no south leg.

Section 2.3.6 Peak Hour Travel Demands

6 Note that a new TRANS Trip Generation Report is now available to be used for employment uses (office), see attached. I've also attached a calculator for your use should you wish to use it. Thank you for this information, we have incorporated it in to the TIA report



Section 2.8 Exemptions Review

7 If employee count is expected to be below 60, consider including the TDM checklist from section 4.5 alongside the TDM-supportive development design and infrastructure checklist review in section 4.1. Corrected in TIA Forecasting submission. The anticipated daily employee count is 98.

Next Steps

8 Please proceed to the forecasting report. Thank you. Forecasting Report completed and submitted for comment on August 24, 2021.

FORECASTING REPORT COMMENT / RESPONSE

WSP Submission August 24, 2021 City Comments Received September 20, 2021

Section 3.1.1 Trip Generation

1 While a conservative approach is understandable and it is appreciated that transit service to the site isn't robust, having 0% targets for all other modes other than auto driver is not supported. Consider at least a nominal amount for transit and auto passenger as these can and should both be incentivized through TDM measures. Feel free to contact myself the City's TDM Officer at travelwise@ottawa.ca for strategies to improve these metrics.

Consider using ITE rates to reduce development travel demand in the case that demand rationalization is required. This would also allow for quantified inclusion of the other area development traffic in section 3.2.3. The mode share targets were revised to include 10% transit and 5% auto passenger for staff trips and several TDM measures were discussed in the Strategy Report completed and submitted for comment on October 15, 2021.

Next Steps

Please proceed to the strategy report.
 Thank you. Strategy Report completed and submitted for comment on October 15, 2021.

STRATEGY REPORT COMMENT / RESPONSE

WSP Submission October 15, 2021

City Comments Received January 28, 2022



- Provide truck turning movements for egress and access at site access points onto Lancaster Road. Illustrate largest vehicle and a template for a more frequently used vehicle (list was provided on site plan). Truck turning template has been incorporated in to the TIA report.
- 2 Pedestrian Facilities should be included between the main entrance of the building and the sidewalk on Lancaster to facilitate access to the building for transit customers. (Required item 1.2.2 in TDM-Supportive Development Design and Infrastructure Checklist) Pedestrian facilities have been added to updated Site Plan and thus have been updated in TDM-Supportive Development Design and Infrastructure Checklist.
- Related to item 3.3.1 in the TDM Checklist Please clarify needs for enhanced transit service. The TIA indicates that existing transit service is sufficient in accommodating the anticipated transit trips.
 TDM Checklist has been updated to reflect previous comments in TIA regarding existing transit services.
- 4 Section 2.3.4:

a) Proposed development is incorrectly shown on Transit Network Map.
b) To clarify: Route 46 operates between Billings Bridge Station and Hurdman Station. Route 48 operates between Elmvale S.C. and Hurdman Station.
c) The connection of routes 46 and 48 to the SE Transitway could also be mentioned.
Wording has been updated in TIA Report.

- 5 Wording in section 4.7.1 could be reworked slightly for clarity.
 a) Nearest bus stops (not transit station) located on Southvale.
 - b) Next closest transit route (not transit stop) located on Russell.

Wording has been updated in TIA Report.

Prepared by:

Jordan Papazoglou, P.Eng., ing. Engineer, Transportation Planning and Science



B SCREENING FORM



City of Ottawa 2017 TIA Guidelines Screening Form

1. Description of Proposed Development			
Municipal Address	2571 Lancaster Road		
Description of Location	Lancaster Road - Mid-block between Russell Road and Walkley Road		
Land Use Classification	Mixed - Office/operation space, warehouse and shop spaces		
Development Size (units)			
Development Size (m ²)	4403 square metres		
Number of Accesses and Locations	2 existing accesses on Lancaster Road; No new proposed accesses		
Phase of Development	Single Phase		
Buildout Year	TBD		

If available, please attach a sketch of the development or site plan to this form.

2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size	
Single-family homes	40 units	- Office
Townhomes or apartments	90 units	/Operation
Office	3,500 m ²	Centre: 3,828 m2
Industrial	5,000 m ²	- Shop: 536 m2
Fast-food restaurant or coffee shop	100 m ²	- Warehouse:
Destination retail	1,000 m ²	710 m2
Gas station or convenience market	75 m ²	

* If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, <u>the Trip Generation</u> <u>Trigger is satisfied.</u> Land Use: 150 - Warehouse (710 sq m - 7,641 sq ft)

710 - General Office Building (3,828 sq m - 41,215 sq ft)

AM Peak Hour: - Warehouse: 26 (Enter: 77%; Exit: 23%) - Office: 91 (Enter: 87%; Exit: 13%)

PM Peak Hour: - Warehouse: 29 (Enter: 27%; Exit: 73%)**71** -Office: 97 (Enter: 15%; Exit: 85%)

Revision Date: June, 2017



ocation Trigger

S. Location mggers		
	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		\checkmark
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		\checkmark

*DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

4. Safety Triggers		
	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		\checkmark
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?		\checkmark
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?		\checkmark
Is the proposed driveway within auxiliary lanes of an intersection?		\checkmark
Does the proposed driveway make use of an existing median break that serves an existing site?		\checkmark
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		\checkmark
Does the development include a drive-thru facility?		\checkmark

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

5. Summary		
	Yes	No
Does the development satisfy the Trip Generation Trigger?	\checkmark	
Does the development satisfy the Location Trigger?		\checkmark
Does the development satisfy the Safety Trigger?		\checkmark

If none of the triggers are satisfied, the TIA Study is complete. If one or more of the triggers is satisfied, the TIA Study must continue into the next stage (Screening and Scoping).



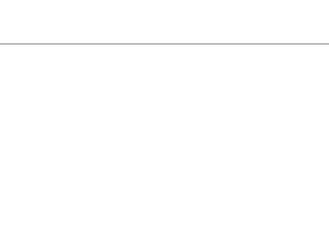
C DRAFT SITE PLAN



Staff Parking	92
Staff Barier-Free Parking	6
Small Equipmet - Attachment 12'x20'	14
Small Equiment - Trailer 12'x20'	23
Large International / Dump Truck 15'x25'	32
Fleet Car / Van / Pickup Truck 12'x20'	96
Backhoe / Mini Excavator 15'x25'	24
Grand total: 287	

SITE STATISTICS				
ite Description: Part of Lot 25, Concession 3 (Ottawa Front) and Part of Block "B" Registered Plan 4M-121				
Assigned Address	2571 Lancaster Rd, Ottawa, Ontario			
Zoning By-Law	Ottawa Zoning By-law 2008-250			
Permitted Use				
	Proposed IL Light Industrial	Required Office / Warehouse Use is		
Zoning Category Definition: Utility	has been given the e duty to supply the ge commodity, or servic water, waste water,	Permitted within a regulated industry that xpress right or subsequent legal meral public with a product, se such as natural gas, electricity, sewer, rail service, or ice. (service public) (By-law 2013		
Definition: Storage yard:	the storage of vehicle maintenance materia and the storage of equipment, and inclu	outdoor storage, including: esthe storage of road al such as gravel or sand; 'heavy vehicles or construction ides an accessory maintenance service and repair of the stored ent.		
Permitted Uses	Office / Storage	Permitted		
Outdoor Storage	Utility Service Yard with Equipment Storage	Permitted at Rear of Site in IH Zone. Zone Change will be aplrequired for storage in IL portion of the site.		
Areas	Proposed	Required		
Lot Area	3.30	0.8 ha		
Building Area	5109			
Industrial Area	557			
Warehouse Area	723	2		
Office Area	3829	N		
Parking	Proposed	Required		
Shops / Warehouse Office Area	10	5		
Parking total	108	97		
Accessible Parking (Min)	6	4		
Type A -	3	2		
Туре 8 -	3	2		
Bicycle Parking (Min)	6	4		
Loading Spaces (Min)	2	2		
Site Coverage	Proposed	Required		
Lot Coverage (Max)	16%	65%		
% Landscape area (Min)	12%	10%		
Landspaced Buffer to Street (Min)	7.5	1.5		
% Outdoor Storage	3%	0% in IL, Permitted in IH		
% Utility Service Yard	72%	N/A		
Floor Space Index	0.16	2		
Setbacks	Proposed			
	20.001	Required		
Front Yard	7.5	7.5		
Interior Side Yard	17.4 & 58.2	75		
Exterior Side Yard	N/A	7.5		
Rear Yard	7.5	7.5		
Open Storage	3	0		
Canopy Front Yard Setback	N/A	3.75		
Height	Proposed	Required		
Max Principal Bldg (m)	10.5	18		
Max Accessory Bidg (m)	6	6		
Walter And the				
Parking Dim's	Proposed	Required		
Standard (m)	5.8m x 2.75m	5.5m x 2.7m		
Type A -	5.8m x 3.4m	5.2m x 3.4m		
Type B -	5.8m x 3.4m	5.2m x 2.4m		
Type b -	S Development of the second			
Loading Space Dim's	12m x 3.5mx4.2	12m x 3.5mx4.2		

я	a	Regulated Zone	



CLIENT LOGO

о.	ISSUANCE	DATE
	ISSUED FOR SITE PLAN CONTROL	2021-08-25
2	ISSUED FOR OWNER REVIEW	2021-11-22
3	ISSUED FOR MASS TIMBER BID	2021-11-26
ŀ	ISSUED FOR OWNER REVIEW	2021-12-13
5	60% OWNER REVIEW SET	2022-01-20
5	90% OWNER REVIEW SET	2022-02-02

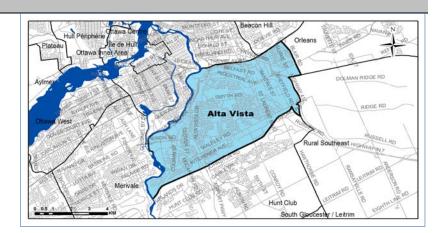


D TRANS O-D SURVEY



Demographic Characteristics

Population Employed Population Households	74,770 32,910 32,590		Actively Travelled Number of Vehicles Area (km ²)	
Occupation				
Status (age 5+)		Male	Female	Total
Full Time Employed		15,840	12,940	28,780
Part Time Employed		1,660	2,470	4,130
Student		8,130	8,750	16,870
Retiree		6,200	8,840	15,030
Unemployed		1,200	950	2,150
Homemaker		50	2,150	2,200
Other		630	900	1,530
Total:		33,700	36,990	70,700
Traveller Characteristics		Male	Female	Total
Transit Pass Holders		7,620	9,140	16,760
Licensed Drivers		25,060	24,810	49,870
Telecommuters		140	60	200
Trips made by residents		92,440	98,770	191,210

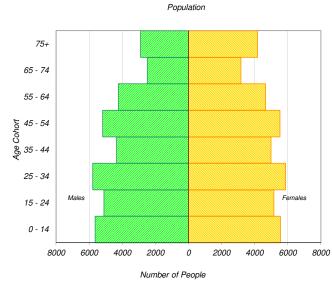


Household Size		
1 person	10,780	33%
2 persons	11,010	34%
3 persons	4,790	15%
4 persons	3,880	12%
5+ persons	2,130	7%
Total:	32,590	100%

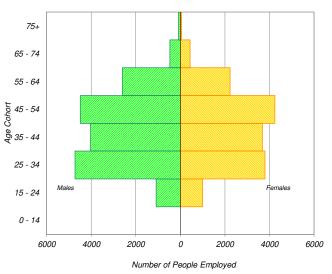
Households by Vehicle Availability					
0 vehicles 6,320 19					
1 vehicle	16,930	52%			
2 vehicles	8,030	25%			
3 vehicles	1,030	3%			
4+ vehicles	290	1%			
Total:	32,590	100%			

Households by Dwelling Type						
Single-detached	12,320	38%				
Semi-detached	1,790	5%				
Townhouse	4,700	14%				
Apartment/Condo	13,780	42%				
Total:	32,590	100%				

Selected Indicators	
Daily Trips per Person (age 5+)	2.70
Vehicles per Person	0.50
Number of Persons per Household	2.29
Daily Trips per Household	5.87
Vehicles per Household	1.14
Workers per Household	1.01
Population Density (Pop/km2)	1940



Employed Population



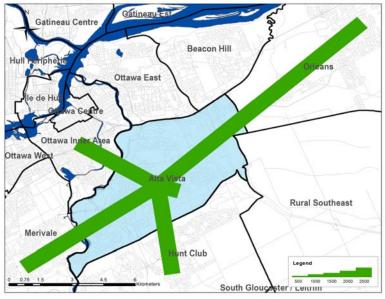
* In 2005 data was only collected for household members aged 11^{*} therefore these results cannot be compared to the 2011 data.



Travel Patterns

Top Five Origins of Trips to Alta Vista

AM Peak Period



Summary of Trips to and from Alta Vista					
AM Peak Period (6:30 - 8:59)	Destinations of	C	Drigins of		
	Trips From		Trips To		
Districts	District	% Total	District	% Total	
Ottawa Centre	4,180	10%	680	1%	
Ottawa Inner Area	4,970	12%	4,270	7%	
Ottawa East	1,940	5%	2,370	4%	
Beacon Hill	2,690	7%	1,850	3%	
Alta Vista	16,220	39%	16,220	27%	
Hunt Club	1,980	5%	7,990	13%	
Merivale	3,010	7%	3,690	6%	
Ottawa West	1,160	3%	1,550	3%	
Bayshore / Cedarview	830	2%	2,330	4%	
Orléans	1,050	3%	5,890	10%	
Rural East	110	0%	430	1%	
Rural Southeast	140	0%	1,550	3%	
South Gloucester / Leitrim	160	0%	1,970	3%	
South Nepean	460	1%	2,360	4%	
Rural Southwest	160	0%	690	1%	
Kanata / Stittsvile	660	2%	1,810	3%	
Rural West	20	0%	180	0%	
Île de Hull	710	2%	190	0%	
Hull Périphérie	360	1%	420	1%	
Plateau	0	0%	680	1%	
Aylmer	40	0%	480	1%	
Rural Northwest	40	0%	300	1%	
Pointe Gatineau	20	0%	740	1%	
Gatineau Est	220	1%	270	0%	
Rural Northeast	10	0%	320	1%	
Buckingham / Masson-Angers	10	0%	70	0%	
Ontario Sub-Total:	39,740	97%	55,830	94%	
Québec Sub-Total:	1,410	3%	3,470	6%	
Total:	41,150	100%	59,300	100%	

Trips by Trip Purpose

24 Hours	From District		To District	Wi	thin District	
Work or related	22,370	15%	46,540	31%	10,770	13%
School	8,550	6%	8,090	5%	6,440	8%
Shopping	16,500	11%	16,600	11%	14,550	17%
Leisure	11,940	8%	13,340	9%	7,720	9%
Medical	2,990	2%	7,860	5%	2,380	3%
Pick-up / drive passenger	9,390	6%	9,900	6%	6,990	8%
Return Home	75,570	50%	44,070	29%	33,060	39%
Other	4,870	3%	6,050	4%	3,240	4%
Total:	152,180	100%	152,450	100%	85,150	100%
AM Peak (06:30 - 08:59)	From District		To District	Wi	thin District	
Work or related	13,920	56%	28,300	66%	5,390	33%
School	5,340	21%	7,330	17%	5,600	35%
Shopping	510	2%	530	1%	320	2%
Leisure	570	2%	990	2%	480	3%
Medical	500	2%	1,760	4%	460	3%
Pick-up / drive passenger	1,790	7%	2,490	6%	2,110	13%
Return Home	1,380	6%	730	2%	910	6%
Other	910	4%	940	2%	930	6%
Total:	24,920	100%	43,070	100%	16,200	100%
PM Peak (15:30 - 17:59)	From District		To District	Wi	thin District	
Work or related	820	2%	1,340	5%	740	4%
School	550	1%	90	0%	70	0%
Shopping	3,920	9%	3,630	13%	2,830	14%
Leisure	2,550	6%	2,440	9%	1,580	8%
Medical	260	1%	670	2%	300	2%
Pick-up / drive passenger	3,310	7%	2,550	9%	2,390	12%
Return Home	31,900	72%	15,950	57%	11,310	58%
Other	1,270	3%	1,230	4%	440	2%
Total:	44,580	100%	27,900	100%	19,660	100%
Peak Period (%)	Total:		% of 24 Hours	v	/ithin Distric	ct (%)
24 Hours	389,780				22%	
AM Peak Period	84,190		22%		19%	
PM Peak Period	92,140		24%		21%	

Trips by Primary Travel Mode

24 Hours	From District		To District	Wit	thin District	:
Auto Driver	92,240	61%	92,670	61%	43,390	51%
Auto Passenger	24,030	16%	24,040	16%	13,430	16%
Transit	27,890	18%	27,220	18%	6,520	8%
Bicycle	2,180	1%	2,110	1%	1,390	2%
Walk	1,440	1%	1,510	1%	15,170	18%
Other	4,420	3%	4,890	3%	5,260	6%
Total:	152,200	100%	152,440	100%	85,160	100%
AM Peak (06:30 - 08:59)	From District		To District	Wit	thin District	:
Auto Driver	12,430	50%	26,810	62%	6,330	39%
Auto Passenger	3,040	12%	5,100	12%	2,500	15%
Transit	7,540	30%	7,300	17%	1,700	10%
Bicycle	750	3%	750	2%	340	2%
Walk	280	1%	280	1%	3,210	20%
Other	880	4%	2,850	7%	2,140	13%
Total:	24,920	100%	43,090	100%	16,220	100%
PM Peak (15:30 - 17:59)	From District		To District	Wit	thin District	:
Auto Driver	28,570	64%	15,990	57%	9,640	49%
Auto Passenger	5,930	13%	4,230	15%	3,570	18%
Transit	7,460	17%	6,420	23%	1,500	8%
Bicycle	630	1%	610	2%	470	2%
Walk	340	1%	310	1%	3,280	17%
Other	1,660	4%	340	1%	1,210	6%
Total:	44,590	100%	27,900	100%	19,670	100%
Avg Vehicle Occupancy	From District		To District	Wit	thin District	:
24 Hours	1.26		1.26		1.31	
AM Peak Period	1.24		1.19		1.39	
PM Peak Period	1.21		1.26		1.37	
Transit Modal Split	From District		To District	Wit	thin District	
			19%		10%	
	19%					
24 Hours AM Peak Period	19% 33%		19%		16%	



E DEVELOPMENT DESIGN CHECKLIST

TDM-Supportive Development Design and Infrastructure Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

Legend				
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed			
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users			
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance			

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	□ _{N/A}
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official <i>Plan policy 4.3.12</i>)	

	TDM-s	supportive design & infrastructure measures: Non-residential developments		Check if completed & descriptions, explanations plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	X	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)		
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)		N/A
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	X	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	X	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility		
	1.3	Amenities for walking & cycling		
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails		N/A
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	X	

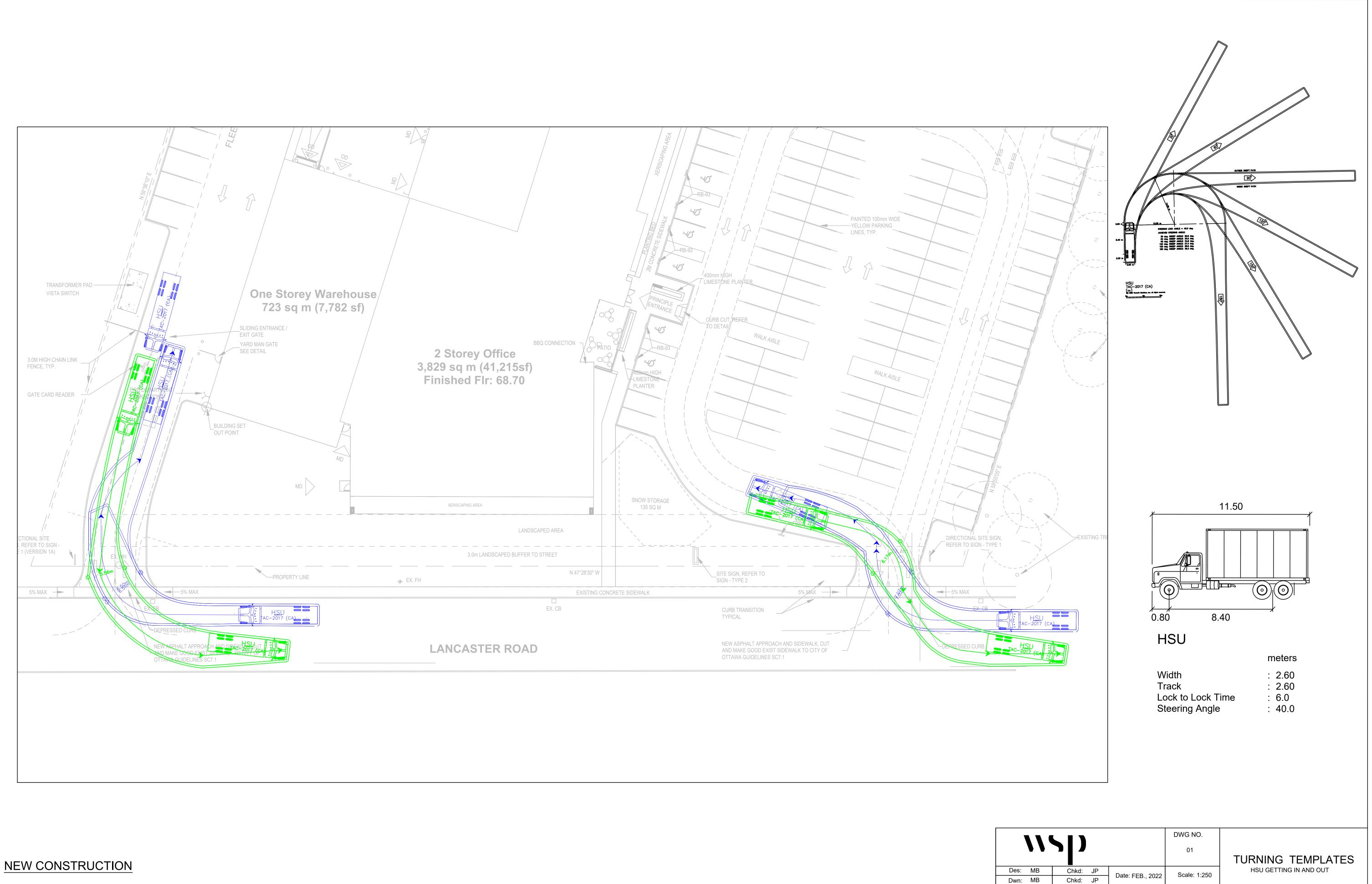
	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	X
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well- used areas (<i>see Zoning By-law Section 111</i>)	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored <i>(see Zoning By-law Section 111)</i>	□ N/A
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	
	2.3	Shower & change facilities	
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	X
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	
	2.4	Bicycle repair station	
BETTER	2.4.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	□ N/A
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	□ N/A
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	□ _{N/A}
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	4.2	Carpool parking	
BASIC	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide carshare parking spaces in permitted non- residential zones, occupying either required or provided parking spaces (see Zoning By-law Section 94)	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly <i>(see Zoning By-law</i> <i>Section 104)</i>	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking <i>(see Zoning By-law Section 111)</i>	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	
	7.	OTHER	
	7.1	On-site amenities to minimize off-site trips	
BETTER	7.1.1	Provide on-site amenities to minimize mid-day or mid-commute errands	



TURNING TEMPLATE





G TDM CHECKLIST

TDM Measures Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

Legend

BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users

BETTER The measure could maximize support for users of sustainable modes, and optimize development performance

The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC ★	1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	\square
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & destin	ations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances	
	2.2	Bicycle skills training	
		Commuter travel	
BETTER ★	2.2.1	Offer on-site cycling courses for commuters, or subsidize off-site courses	
	2.3	Valet bike parking	
		Visitor travel	
BETTER	2.3.1	Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	

TDM Measures Checklist

Version 1.0 (30 June 2017)

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances	\square
BASIC	3.1.2	Provide online links to OC Transpo and STO information	
BETTER	3.1.3	Provide real-time arrival information display at entrances	
	3.2	Transit fare incentives	
		Commuter travel	
BETTER	3.2.1	Offer preloaded PRESTO cards to encourage commuters to use transit	
BETTER	★ 3.2.2	Subsidize or reimburse monthly transit pass purchases by employees	
		Visitor travel	
BETTER	3.2.3	Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	
	3.3	Enhanced public transit service	
		Commuter travel	
BETTER	3.3.1	Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.3.2	Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	
	3.4	Private transit service	
		Commuter travel	
BETTER	3.4.1	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.4.2	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments		Check if proposed & add descriptions
	4.	RIDESHARING		
	4.1	Ridematching service		
		Commuter travel		
BASIC ★	4.1.1	Provide a dedicated ridematching portal at OttawaRideMatch.com	X	
	4.2	Carpool parking price incentives		
		Commuter travel		
BETTER	4.2.1	Provide discounts on parking costs for registered carpools		N/A
	4.3	Vanpool service		
		Commuter travel		
BETTER	4.3.1	Provide a vanpooling service for long-distance commuters		
	5.	CARSHARING & BIKESHARING		
	5.1	Bikeshare stations & memberships		
BETTER	5.1.1	Contract with provider to install on-site bikeshare station for use by commuters and visitors		N/A
		Commuter travel	1	
BETTER	5.1.2	Provide employees with bikeshare memberships for local business travel		N/A
	5.2	Carshare vehicles & memberships		
		Commuter travel		
BETTER	5.2.1	Contract with provider to install on-site carshare vehicles and promote their use by tenants		
BETTER	5.2.2	Provide employees with carshare memberships for local business travel		
	6.	PARKING		
	6.1	Priced parking		
		Commuter travel		
BASIC ★	6.1.1	Charge for long-term parking (daily, weekly, monthly)		
	~	Unbundle parking cost from lease rates at multi-tenant		
BASIC	6.1.2	sites		
BASIC	6.1.2			

TDM Measures Checklist

Version 1.0 (30 June 2017)

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	7.	TDM MARKETING & COMMUNICATIONS	
	7.1	Multimodal travel information	
		Commuter travel	
BASIC ★	7.1.1	Provide a multimodal travel option information package to new/relocating employees and students <i>Visitor travel</i>	
BETTER ★	712	Include multimodal travel option information in	
BETTER	1.1.2	invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	
	7.2	Personalized trip planning	
		Commuter travel	
BETTER ★	7.2.1	Offer personalized trip planning to new/relocating employees	
	7.3	Promotions	
		Commuter travel	
BETTER	7.3.1	Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	
	8.	OTHER INCENTIVES & AMENITIES	
	8.1	Emergency ride home	
		Commuter travel	
BETTER ★	8.1.1	Provide emergency ride home service to non-driving commuters	
	8.2	Alternative work arrangements	
		Commuter travel	
BASIC ★	8.2.1	Encourage flexible work hours	
BETTER	8.2.2	Encourage compressed workweeks	
BETTER ★	8.2.3	Encourage telework	X
	8.3	Local business travel options	
		Commuter travel	
BASIC ★	8.3.1	Provide local business travel options that minimize the need for employees to bring a personal car to work	
	8.4	Commuter incentives	
		Commuter travel	
BETTER	8.4.1	Offer employees a taxable, mode-neutral commuting allowance	
	8.5	On-site amenities	
		Commuter travel	
BETTER	8.5.1	Provide on-site amenities/services to minimize mid-day or mid-commute errands	



SYNCHRO

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

10-1	4-2021
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	ሻ	- 44	1	ሻ	- 44	1	ሻ	- 44	1
Traffic Volume (vph)	304	106	98	32	280	108	137	601	650	190	1136	97
Future Volume (vph)	304	106	98	32	280	108	137	601	650	190	1136	97
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			132			132			323			130
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Adj. Flow (vph)	338	118	109	36	311	120	152	668	722	211	1262	108
Shared Lane Traffic (%)												
Lane Group Flow (vph)	338	118	109	36	311	120	152	668	722	211	1262	108
Enter Blocked Intersection	No	No	No									
Lane Alignment	Left	Left	Right									
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25	0	15	25	0	15	25	0	15	25	0	15
Number of Detectors	1	2	1 Diabt									
Detector Template	Left	Thru	Right									
Leading Detector (m)	2.0 0.0	10.0 0.0	2.0 0.0									
Trailing Detector (m) Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	9.4	0.0	0.0	9.4	0.0	0.0	9.4	0.0	0.0	9.4	0.0
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		CI+Ex			CI+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm									
Protected Phases	7	4		3	8		1	6	1 0111	5	2	
Permitted Phases	1	+	4	5	0	8	1	0	6	J	2	2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase	'	+	-	5	0	0	1	0	0	J	2	2
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2021 - AM Peak Hour 8:42 am 07-07-2021 Baseline

Synchro 11 Report Page 1

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	20.0	32.4	32.4	15.6	28.0	28.0	20.0	52.0	52.0	20.0	52.0	52.0
Total Split (%)	16.7%	27.0%	27.0%	13.0%	23.3%	23.3%	16.7%	43.3%	43.3%	16.7%	43.3%	43.3%
Maximum Green (s)	14.4	26.7	26.7	10.0	22.3	22.3	14.0	46.1	46.1	14.0	46.1	46.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	14.3	27.3	27.3	8.1	16.4	16.4	15.4	46.1	46.1	20.0	50.7	50.7
Actuated g/C Ratio	0.12	0.23	0.23	0.07	0.14	0.14	0.13	0.38	0.38	0.17	0.42	0.42
v/c Ratio	0.87	0.30	0.27	0.37	0.68	0.40	0.72	0.54	0.93	0.84	0.93	0.16
Control Delay	75.1	42.3	5.7	63.8	56.9	10.1	69.5	30.7	38.5	77.7	47.5	3.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.1	42.3	5.7	63.8	56.9	10.1	69.5	30.7	38.5	77.7	47.5	3.1
LOS	E	D	Α	E	E	В	E	С	D	E	D	A
Approach Delay		54.8			45.4			38.2			48.5	
Approach LOS		D			D			D			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 120												
Actuated Cycle Length: 1												
Offset: 0 (0%), Reference	d to phase 2	2:NWT a	nd 6:SET	, Start of	Green							
Natural Cycle: 115												
Control Type: Actuated-C	oordinated											
Maximum v/c Ratio: 0.93												
Intersection Signal Delay:						on LOS: E						
Intersection Capacity Utili	zation 77.8	%		10	CU Level	of Servic	e D					
Analysis Period (min) 15												
Calita and Dhasas 2. F		t louropi		have the Dat	/l anaaat	or Dd						

Splits and Phases: 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

∕•ø1	Ø2 (R)	€ ø3	-	Ø4	
20 s	52 s	15.6 s	32.4	łs	
★ _{Ø5}	🛰 Ø6 (R)	<u>ک</u> ₀₇		Ø8	
20 s	52 s	20 s		28 s	

10-14-2022	1
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	↑	1	٦	- † †	1	۳.	- † †	1	٦.	- † †	1
Traffic Volume (vph)	304	106	98	32	280	108	137	601	650	190	1136	97
Future Volume (vph)	304	106	98	32	280	108	137	601	650	190	1136	97
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	338	118	109	36	311	120	152	668	722	211	1262	108
RTOR Reduction (vph)	0	0	84	0	0	101	0	0	205	0	0	64
Lane Group Flow (vph)	338	118	25	36	311	19	152	668	517	211	1262	44
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	14.3	27.3	27.3	5.6	18.6	18.6	15.4	43.9	43.9	20.0	48.5	48.5
Effective Green, g (s)	14.3	27.3	27.3	5.6	18.6	18.6	15.4	43.9	43.9	20.0	48.5	48.5
Actuated g/C Ratio	0.12	0.23	0.23	0.05	0.16	0.16	0.13	0.37	0.37	0.17	0.40	0.40
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	387	397	308	67	519	209	210	1180	554	250	1291	583
v/s Ratio Prot	c0.10	0.07		0.03	c0.09		0.09	0.21		c0.14	c0.39	
v/s Ratio Perm			0.02			0.01			0.34			0.03
v/c Ratio	0.87	0.30	0.08	0.54	0.60	0.09	0.72	0.57	0.93	0.84	0.98	0.07
Uniform Delay, d1	52.0	38.4	36.5	55.9	47.2	43.4	50.3	30.4	36.6	48.5	35.2	22.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.0	0.4	0.1	8.1	1.9	0.2	11.7	2.0	24.9	22.1	20.2	0.2
Delay (s)	71.0	38.8	36.6	64.0	49.1	43.6	61.9	32.4	61.5	70.5	55.4	22.2
Level of Service	E	D	D	E	D	D	E	С	E	E	E	С
Approach Delay (s)		57.6			48.8			49.0			55.2	
Approach LOS		E			D			D			E	
Intersection Summary												
HCM 2000 Control Delay			52.5	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.90									
Actuated Cycle Length (s)			120.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		77.8%	IC	CU Level	of Service	Э		D			
Analysis Period (min)			15									
c Critical Lane Group												

	≯	-	+	×	1	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	† †	111	1	<u>ነ</u> ካ	1
Traffic Volume (vph)	200	720	1389	630	91	166
Future Volume (vph)	200	720	1389	630	91	166
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			25.0	1	130.0
Taper Length (m)	7.5				7.5	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	1.00	0.90	0.91	0.850	0.97	0.850
Fit Protected	0.950			0.000	0.950	0.000
	1613	2040	1502	1500	0.950 3072	1391
Satd. Flow (prot)	0.085	2948	4593	1500		1291
Flt Permitted		2040	4502	1500	0.950	1204
Satd. Flow (perm)	144	2948	4593	1500	3072	1391
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			- ^	316		184
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	6%	16%	7%	2%	8%	10%
Adj. Flow (vph)	222	800	1543	700	101	184
Shared Lane Traffic (%)						
Lane Group Flow (vph)	222	800	1543	700	101	184
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6	Ŭ	7.2	Ŭ
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25	1.01	1.01	1.07	25	1.07
Number of Detectors	1	2	2	1	25	1
Detector Template	Left	∠ Thru	∠ Thru	-	Left	
	2.0	10.0	10.0	Right 2.0	2.0	Right
Leading Detector (m)						2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		CI+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4		3	8	J	6
Detector Phase	7	4	8	8	6	6
Switch Phase	'	т	0	0	U	0
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
	5.0	0.0	5.0	0.0	J.U	5.0

2021 - AM Peak Hour 8:42 am 07-07-2021 Baseline

	∕	-	+	×	1	4	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7	
Total Split (s)	24.0	122.0	98.0	98.0	28.0	28.0	
Total Split (%)	16.0%	81.3%	65.3%	65.3%	18.7%	18.7%	
Maximum Green (s)	17.8	115.8	91.8	91.8	22.3	22.3	
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3	
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	C-Max	C-Max	
Walk Time (s)			20.0	20.0	7.0	7.0	
Flash Dont Walk (s)			7.0	7.0	15.0	15.0	
Pedestrian Calls (#/hr)			0	0	0	0	
Act Effct Green (s)	96.1	96.1	72.5	72.5	42.0	42.0	
Actuated g/C Ratio	0.64	0.64	0.48	0.48	0.28	0.28	
v/c Ratio	0.85	0.42	0.69	0.79	0.12	0.35	
Control Delay	58.7	13.6	31.4	22.6	45.0	8.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	58.7	13.6	31.4	22.6	45.0	8.8	
LOS	E	В	С	С	D	A	
Approach Delay		23.4	28.7		21.7		
Approach LOS		С	С		С		
Intersection Summary							
Area Type:	Other						
Cycle Length: 150	2						
Actuated Cycle Length: 15	50						
Offset: 0 (0%), Reference		2: and 6:	SBL, Star	t of Gree	n		
Natural Cycle: 90		unu 0.					
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.85							
Intersection Signal Delay:	26.6			h	ntersectio	on LOS: C	
Intersection Capacity Utiliz		26				of Servic	
Analysis Period (min) 15					00 2070	0.00.00	
Splits and Phases: 14: Walkley Rd & Lancaster Rd							

	122 s		
Ø6 (R)		4 [♠] Ø8	
28 s	24 s	98.5	

10-14-2021	
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Movement EBL EBT WBT WBR SBL SBR Lane Configurations 1 1 1 1 1 1
Lane Configurations 🎽 🛧 🛧 👬 🎢
Traffic Volume (vph) 200 720 1389 630 91 166
Future Volume (vph) 200 720 1389 630 91 166
Ideal Flow (vphpl) 1800 1800 1800 1800 1800 1800
Total Lost time (s) 6.2 6.2 6.2 6.2 5.7 5.7
Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00
Frt 1.00 1.00 1.00 0.85 1.00 0.85
Flt Protected 0.95 1.00 1.00 1.00 0.95 1.00
Satd. Flow (prot) 1613 2948 4593 1500 3072 1391
Flt Permitted 0.09 1.00 1.00 0.95 1.00
Satd. Flow (perm) 144 2948 4593 1500 3072 1391
Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90
Adj. Flow (vph) 222 800 1543 700 101 184
RTOR Reduction (vph) 0 0 0 163 0 132
Lane Group Flow (vph) 222 800 1543 537 101 52
Heavy Vehicles (%) 6% 16% 7% 2% 8% 10%
Turn Type pm+pt NA NA Perm Prot Perm
Protected Phases 7 4 8 6
Permitted Phases 4 8 6
Actuated Green, G (s) 96.1 96.1 72.5 72.5 42.0 42.0
Effective Green, g (s) 96.1 96.1 72.5 72.5 42.0 42.0
Actuated g/C Ratio 0.64 0.64 0.48 0.48 0.28 0.28
Clearance Time (s) 6.2 6.2 6.2 6.2 5.7 5.7
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0
Lane Grp Cap (vph) 262 1888 2219 725 860 389
v/s Ratio Prot c0.10 0.27 0.34 0.03
v/s Ratio Perm c0.44 0.36 c0.04
v/c Ratio 0.85 0.42 0.70 0.74 0.12 0.13
Uniform Delay, d1 36.4 13.3 30.2 31.2 40.2 40.4
Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
Incremental Delay, d2 21.6 0.2 1.0 4.1 0.3 0.7
Delay (s) 57.9 13.4 31.1 35.3 40.5 41.1
Level of Service E B C D D D
Approach Delay (s) 23.1 32.4 40.9
Approach LOS C C D
Intersection Summary
HCM 2000 Control Delay 30.4 HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio 0.64
Actuated Cycle Length (s) 150.0 Sum of lost time (s)
Intersection Capacity Utilization 63.2% ICU Level of Service
Analysis Period (min) 15
c Critical Lane Group

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

10-1	14-2	021
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	<u>۲</u>	- † †	1	- ሽ	- ††	1	- ሽ	- ††	1
Traffic Volume (vph)	509	277	225	101	107	232	138	1109	361	99	735	94
Future Volume (vph)	509	277	225	101	107	232	138	1109	361	99	735	94
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			223			209			401			181
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Adj. Flow (vph)	566	308	250	112	119	258	153	1232	401	110	817	104
Shared Lane Traffic (%)												
Lane Group Flow (vph)	566	308	250	112	119	258	153	1232	401	110	817	104
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			Cl+Ex	_
Detector 2 Channel		0.0			0.0			0.0			0.0	
Detector 2 Extend (s)		0.0	D	- ·	0.0	D	D (0.0	D	D (0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8	~	1	6	-	5	2	
Permitted Phases	-	4	4	^	^	8	4	^	6	-	^	2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase	F 0	F 0	F 0	F A	F 0	F 0	F 0	F 0	F 0	F 0	F 0	F A
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2021 - PM Peak Hour 9:43 am 09-22-2021

Lanes, Volumes, Timings <u>3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd</u>

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	26.0	38.4	38.4	15.6	28.0	28.0	17.0	49.0	49.0	17.0	49.0	49.0
Total Split (%)	21.7%	32.0%	32.0%	13.0%	23.3%	23.3%	14.2%	40.8%	40.8%	14.2%	40.8%	40.8%
Maximum Green (s)	20.4	32.7	32.7	10.0	22.3	22.3	11.0	43.1	43.1	11.0	43.1	43.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	20.4	25.7	25.7	9.9	15.2	15.2	15.5	48.8	48.8	12.4	45.7	45.7
Actuated g/C Ratio	0.17	0.21	0.21	0.08	0.13	0.13	0.13	0.41	0.41	0.10	0.38	0.38
v/c Ratio	1.01	0.81	0.51	0.82	0.28	0.69	0.71	0.90	0.47	0.69	0.65	0.15
Control Delay	91.1	60.8	10.5	95.1	47.3	21.0	69.2	44.9	4.6	73.9	34.3	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	91.1	60.8	10.5	95.1	47.3	21.0	69.2	44.9	4.6	73.9	34.3	0.5
LOS Annrageh Delay	F	E	В	F	D 44.4	С	E	D 37.9	А	E	C 35.1	A
Approach Delay		64.9										
Approach LOS		E			D			D			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 120												
Actuated Cycle Length: 12												
Offset: 0 (0%), Referenced	d to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 115												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.01												
Intersection Signal Delay:						n LOS: E						
Intersection Capacity Utiliz	zation 78.8	%		10	CU Level	of Servic	ce D					_
Analysis Period (min) 15												

Splits and Phases: 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

∖ ₀ _{Ø1}	Ø2 (R)	€ Ø3	₩ Ø4
17 s	49 s	15.6 s 3	8.4s
★ _{Ø5}	🚽 🍡 Ø6 (R)	<u>ک</u> ₀₇	Ø8
17 s	49 s	26 s	28 s

10-1	4-2021
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	↑	1	٦.	- † †	1	- ሻ	- † †	1	ሻ	- † †	1
Traffic Volume (vph)	509	277	225	101	107	232	138	1109	361	99	735	94
Future Volume (vph)	509	277	225	101	107	232	138	1109	361	99	735	94
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	566	308	250	112	119	258	153	1232	401	110	817	104
RTOR Reduction (vph)	0	0	175	0	0	183	0	0	238	0	0	64
Lane Group Flow (vph)	566	308	75	112	119	75	153	1232	163	110	817	40
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	20.4	25.7	25.7	9.9	15.2	15.2	15.5	48.8	48.8	12.4	45.7	45.7
Effective Green, g (s)	20.4	25.7	25.7	9.9	15.2	15.2	15.5	48.8	48.8	12.4	45.7	45.7
Actuated g/C Ratio	0.17	0.21	0.21	0.08	0.13	0.13	0.13	0.41	0.41	0.10	0.38	0.38
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	558	381	318	136	428	191	216	1363	616	160	1252	576
v/s Ratio Prot	c0.17	c0.17		0.07	0.04		c0.09	c0.37		0.07	0.25	
v/s Ratio Perm			0.05			0.05			0.11			0.03
v/c Ratio	1.01	0.81	0.24	0.82	0.28	0.40	0.71	0.90	0.26	0.69	0.65	0.07
Uniform Delay, d1	49.8	44.8	39.0	54.2	47.4	48.2	50.1	33.4	23.7	51.9	30.6	23.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	41.7	11.9	0.4	31.5	0.4	1.3	10.1	10.1	1.0	11.6	2.7	0.2
Delay (s)	91.5	56.7	39.4	85.7	47.8	49.5	60.2	43.5	24.7	63.5	33.3	23.9
Level of Service	F	E	D	F	D	D	E	D	С	E	С	С
Approach Delay (s)		70.4			57.4			40.7			35.5	
Approach LOS		E			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			48.9	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.93									
Actuated Cycle Length (s)			120.0	S	um of los	t time (s)			23.2			
Intersection Capacity Utiliza	tion		78.8%	IC	CU Level	of Service	е		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	1	^	1	ኘካ	1
Traffic Volume (vph)	123	1721	933	161	474	201
Future Volume (vph)	123	1721	933	161	474	201
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			1	1	100.0
Taper Length (m)	7.5				7.5	1
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	1.00	0.95	0.91	0.850	0.97	0.850
Fit Protected				0.000	0.950	0.000
	0.950 1676	2057	1500	1404	3252	1485
Satd. Flow (prot)		3257	4508	1404		1400
Fit Permitted	0.224	2057	4500	4404	0.950	4405
Satd. Flow (perm)	395	3257	4508	1404	3252	1485
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				124		223
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	5%	9%	9%	2%	3%
Adj. Flow (vph)	137	1912	1037	179	527	223
Shared Lane Traffic (%)						
Lane Group Flow (vph)	137	1912	1037	179	527	223
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane		7.0	т.U			
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
	25	1.07	1.07	1.07	25	1.07
Turning Speed (k/h)		0	0	15		
Number of Detectors	1	2 Thru	2 Thru		1	1 Diaht
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel		J . L A	U . L /(
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	ριπ + ρι 7	4	8		6	1 CIIII
Permitted Phases	4	4	U	8	U	6
	4	4	0	0 8	C	6
Detector Phase	1	4	8	ð	6	Ø
Switch Phase	F 0	F 0	F 0	F 0	F 0	F 0
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0

2021 - PM Peak Hour 9:43 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	22.0	102.0	80.0	80.0	28.0	28.0
Total Split (%)	16.9%	78.5%	61.5%	61.5%	21.5%	21.5%
Maximum Green (s)	15.8	95.8	73.8	73.8	22.3	22.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	95.8	95.8	81.1	81.1	22.3	22.3
Actuated g/C Ratio	0.74	0.74	0.62	0.62	0.17	0.17
v/c Ratio	0.37	0.80	0.37	0.19	0.95	0.51
Control Delay	7.7	14.2	12.6	4.1	80.2	10.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	7.7	14.2	12.6	4.1	80.2	10.2
LOS	А	В	В	А	F	В
Approach Delay		13.7	11.3		59.4	
Approach LOS		В	В		E	
Intersection Summary						
Area Type:	Other					
Cycle Length: 130						
Actuated Cycle Length: 13	30					
Offset: 0 (0%), Reference		2: and 6:3	SBL, Star	t of Gree	n	
Natural Cycle: 90						
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.95						
Intersection Signal Delay:	21.5			l	ntersectio	n LOS: C
Intersection Capacity Utili		%		l	CU Level	of Service
Analysis Period (min) 15						

Splits and Phases: 14: Walkley Rd & Lancaster Rd

	 Ø4		
	102 s		
A	٠	<u>↓</u>	
Ø6 (R)	🖌 Ø7	Ø8	
28 s	22 s	80 s	

10-14-202	1
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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	۲	† †	^	1	ኘካ	1	
Traffic Volume (vph)	123	1721	933	161	474	201	
Future Volume (vph)	123	1721	933	161	474	201	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1676	3257	4508	1404	3252	1485	
Flt Permitted	0.22	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	396	3257	4508	1404	3252	1485	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	
Adj. Flow (vph)	137	1912	1037	179	527	223	
RTOR Reduction (vph)	0	0	0	47	0	185	
Lane Group Flow (vph)	137	1912	1037	132	527	38	
Heavy Vehicles (%)	2%	5%	9%	9%	2%	3%	
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm	
Protected Phases	7	4	8		6		
Permitted Phases	4			8		6	
Actuated Green, G (s)	95.8	95.8	81.1	81.1	22.3	22.3	
Effective Green, g (s)	95.8	95.8	81.1	81.1	22.3	22.3	
Actuated g/C Ratio	0.74	0.74	0.62	0.62	0.17	0.17	
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	375	2400	2812	875	557	254	
v/s Ratio Prot	0.02	c0.59	0.23		c0.16		
v/s Ratio Perm	0.24			0.09		0.03	
v/c Ratio	0.37	0.80	0.37	0.15	0.95	0.15	
Uniform Delay, d1	5.8	10.9	11.9	10.2	53.3	45.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.6	1.9	0.1	0.1	26.9	1.3	
Delay (s)	6.4	12.8	12.0	10.2	80.2	47.0	
Level of Service	A	B	B	В	F	D	
Approach Delay (s)		12.4	11.8		70.3		
Approach LOS		В	В		E		
Intersection Summary							
HCM 2000 Control Delay			23.0	Н	CM 2000	Level of Ser	vice
HCM 2000 Volume to Capa	city ratio		0.87				
Actuated Cycle Length (s)			130.0			t time (s)	
Intersection Capacity Utiliza	ition		74.4%	IC	U Level	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	↑	1	ሻ	- 44	1	ሻ	- 44	1	ሻ	- † †	1
Traffic Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Future Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			138			138			343			136
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Adj. Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Shared Lane Traffic (%)												
Lane Group Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2	0 -		7.2	J -		3.6	J ·		3.6	J -
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel						-	-	-				
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	0.0	0.0	9.4			9.4	0.0	0.0	9.4	0.0
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		U			0/			0/			U	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
()	Prot		Perm	Prot		Perm	Prot		Perm	Prot		Perm
			. 0.111						. 0.111			. 0.111
	,	Т	4	U	0	8		U	6	U	L	2
	7	4		3	8		1	6		5	2	2
	I	-	-	5	0	U	1	U	U	5	2	2
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s)	Prot 7 7 5.0	NA 4 4 5.0	Perm 4 4 5.0	Prot 3 3 5.0	NA 8 8 5.0	Perm 8 8 5.0	Prot 1 1 5.0	NA 6 5.0	Perm 6 6 5.0	Prot 5 5 5.0	NA 2 2 5.0	Perm 2 2 5.0

2023 - AM Peak Hour Background 8:37 am 09-22-2021

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	18.0	32.5	32.5	13.5	28.0	28.0	17.0	47.0	47.0	22.0	52.0	52.0
Total Split (%)	15.7%	28.3%	28.3%	11.7%	24.3%	24.3%	14.8%	40.9%	40.9%	19.1%	45.2%	45.2%
Maximum Green (s)	12.4	26.8	26.8	7.9	22.3	22.3	11.0	41.1	41.1	16.0	46.1	46.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	12.4	24.9	24.9	7.2	15.0	15.0	14.8	44.5	44.5	19.9	49.6	49.6
Actuated g/C Ratio	0.11	0.22	0.22	0.06	0.13	0.13	0.13	0.39	0.39	0.17	0.43	0.43
v/c Ratio	0.88	0.28	0.25	0.37	0.65	0.37	0.66	0.49	0.83	0.74	0.83	0.14
Control Delay	76.2	41.3	3.9	63.4	54.4	7.2	63.1	29.0	25.5	62.9	36.4	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.2	41.3	3.9	63.4	54.4	7.2	63.1	29.0	25.5	62.9	36.4	1.8
LOS	E	D	A	E	D	А	E	С	С	E	D	A
Approach Delay		55.0			43.0			30.8			37.5	
Approach LOS		D			D			С			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 115												
Actuated Cycle Length: 17	15											
Offset: 0 (0%), Reference	Offset: 0 (0%), Referenced to phase 2:NWT and 6:SET, Start of Green											
Natural Cycle: 95												
	Control Type: Actuated-Coordinated											
	Maximum v/c Ratio: 0.88											
Intersection Signal Delay:	38.0			h	ntersectio	on LOS: E)					
Intersection Capacity Utiliz	zation 78.5	6		10	CU Level	of Servic	e D					
Analysis Period (min) 15												

∕•ø1	102 (R)	€ø3	→ Ø4
17 s	52 s	13.5 s	32.5 s
★ _{Ø5}	📕 🍡 Ø6 (R)	<u>ک</u> _{Ø7}	₽ Ø8
22 s	47 s	18 s	28 s

10-14-2	021
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	٦.	- † †	1	ሻ	^	1	<u>۲</u>	- † †	1
Traffic Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Future Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
RTOR Reduction (vph)	0	0	78	0	0	93	0	0	217	0	0	58
Lane Group Flow (vph)	307	107	21	33	283	16	139	607	440	192	1148	40
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	12.4	24.9	24.9	4.7	17.2	17.2	14.8	42.3	42.3	19.9	47.4	47.4
Effective Green, g (s)	12.4	24.9	24.9	4.7	17.2	17.2	14.8	42.3	42.3	19.9	47.4	47.4
Actuated g/C Ratio	0.11	0.22	0.22	0.04	0.15	0.15	0.13	0.37	0.37	0.17	0.41	0.41
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	350	378	293	58	501	202	211	1186	557	259	1317	594
v/s Ratio Prot	c0.09	0.06		0.02	c0.08		0.08	0.19		c0.13	c0.36	
v/s Ratio Perm			0.02			0.01			0.29			0.03
v/c Ratio	0.88	0.28	0.07	0.57	0.56	0.08	0.66	0.51	0.79	0.74	0.87	0.07
Uniform Delay, d1	50.5	37.6	35.9	54.2	45.4	42.1	47.7	28.3	32.4	45.1	31.0	20.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.1	0.4	0.1	12.2	1.5	0.2	7.2	1.6	10.9	10.9	8.1	0.2
Delay (s)	71.6	38.0	36.0	66.3	46.9	42.3	54.9	29.9	43.3	56.0	39.1	20.7
Level of Service	E	D	D	E	D	D	D	С	D	E	D	С
Approach Delay (s)		57.7			47.2			38.7			40.1	
Approach LOS		E			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			42.8	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.82									
Actuated Cycle Length (s)			115.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		78.5%	IC	U Level	of Service	e		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	1	111	1	ካካ	1
Traffic Volume (vph)	204	735	1417	643	93	169
Future Volume (vph)	204	735	1417	643	93	169
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			25.0	1	130.0
Taper Length (m)	7.5			1	7.5	1
		0.05	0.04	1.00	7.5 0.97	1.00
Lane Util. Factor	1.00	0.95	0.91		0.97	
Frt Fit Droto etc.d	0.050			0.850	0.050	0.850
Flt Protected	0.950	00.40	4500	4500	0.950	4004
Satd. Flow (prot)	1613	2948	4593	1500	3072	1391
Flt Permitted	0.109				0.950	
Satd. Flow (perm)	185	2948	4593	1500	3072	1391
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				375		169
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	16%	7%	2%	8%	10%
Adj. Flow (vph)	204	735	1417	643	93	169
Shared Lane Traffic (%)						
Lane Group Flow (vph)	204	735	1417	643	93	169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)	Lon	3.6	3.6	. agric	7.2	. ugrit
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane		4.0	4.0		4.0	
	1.07	1.07	1.07	1.07	1.07	1.07
Headway Factor		1.07	1.07			
Turning Speed (k/h)	25	•	•	15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel		^	^			
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	ριπ+ρι 7	4	8		6	
Permitted Phases	4	4	U	8	U	6
		Λ	0		e	
Detector Phase	7	4	8	8	6	6
Switch Phase	F 0	F 0	F A	F 0	F 0	F 0
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0

2023 - AM Peak Hour Background 8:37 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	14.0	51.2	37.2	37.2	28.8	28.8
Total Split (%)	17.5%	64.0%	46.5%	46.5%	36.0%	36.0%
Maximum Green (s)	7.8	45.0	31.0	31.0	23.1	23.1
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	44.4	44.4	30.4	30.4	23.7	23.7
Actuated g/C Ratio	0.56	0.56	0.38	0.38	0.30	0.30
v/c Ratio	0.84	0.45	0.81	0.80	0.10	0.32
Control Delay	46.0	11.5	26.7	17.9	21.3	5.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.0	11.5	26.7	17.9	21.3	5.7
LOS	D	В	С	В	С	А
Approach Delay		19.0	24.0		11.2	
Approach LOS		В	С		В	
Intersection Summary						
Area Type:	Other					
Cycle Length: 80						
Actuated Cycle Length: 8	0					
Offset: 0 (0%), Reference		2: and 6:	SBL, Star	t of Gree	n	
Natural Cycle: 80			, otai			
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.84						
Intersection Signal Delay:	21.5			- h	ntersectio	on LOS: C
Intersection Capacity Utili		%				of Servic
Analysis Period (min) 15		, .		•	00 2070	0.00.00
Calita and Dhasaar 14:		0 00	otor Dd			
Splits and Phases: 14:	Walkley Rd	& Lanca	ster Kd			

	51.2 s		
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Ø6 (R)	Ø7	Ø8	
28.8 s	14 s	37.2 s	

10-14-2021	
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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	۲	† †	^	1	ኘካ	1	
Traffic Volume (vph)	204	735	1417	643	93	169	
Future Volume (vph)	204	735	1417	643	93	169	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1613	2948	4593	1500	3072	1391	
Flt Permitted	0.11	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	186	2948	4593	1500	3072	1391	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	204	735	1417	643	93	169	
RTOR Reduction (vph)	0	0	0	233	0	119	
Lane Group Flow (vph)	204	735	1417	411	93	50	
Heavy Vehicles (%)	6%	16%	7%	2%	8%	10%	
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm	
Protected Phases	7	4	8		6		
Permitted Phases	4			8		6	
Actuated Green, G (s)	44.4	44.4	30.4	30.4	23.7	23.7	
Effective Green, g (s)	44.4	44.4	30.4	30.4	23.7	23.7	
Actuated g/C Ratio	0.55	0.55	0.38	0.38	0.30	0.30	
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	242	1636	1745	570	910	412	
v/s Ratio Prot	c0.08	0.25	0.31		0.03		
v/s Ratio Perm	c0.39			0.27		c0.04	
v/c Ratio	0.84	0.45	0.81	0.72	0.10	0.12	
Uniform Delay, d1	15.7	10.6	22.2	21.2	20.4	20.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	22.5	0.2	3.0	4.5	0.2	0.6	
Delay (s)	38.2	10.7	25.2	25.6	20.7	21.2	
Level of Service	D	В	С	С	С	С	
Approach Delay (s)		16.7	25.4		21.0		
Approach LOS		В	С		С		
Intersection Summary			00 5		014 0000		
HCM 2000 Control Delay			22.5	H	CM 2000	Level of Ser	vice
HCM 2000 Volume to Capa	acity ratio		0.61	<u>_</u>	<i>.</i> .		
Actuated Cycle Length (s)			80.0			t time (s)	
Intersection Capacity Utiliza	ation		64.3%	IC	U Level	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

10-1	4-2021
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	ሻ	- † †	1	ሻ	- 44	1	ሻ	- † †	1
Traffic Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Future Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3252	1651	1354	1346	3109	1275	1527	3226	1515	1500	3196	1354
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3252	1651	1354	1346	3109	1275	1527	3226	1515	1500	3196	1354
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			138			191			322			189
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	9%	13%	27%	10%	20%	12%	6%	1%	14%	7%	13%
Adj. Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Shared Lane Traffic (%)												
Lane Group Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2023 - AM Peak Hour Total 11:21 am 09-22-2021

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	18.0	34.7	34.7	11.3	28.0	28.0	17.0	47.0	47.0	22.0	52.0	52.0
Total Split (%)	15.7%	30.2%	30.2%	9.8%	24.3%	24.3%	14.8%	40.9%	40.9%	19.1%	45.2%	45.2%
Maximum Green (s)	12.4	29.0	29.0	5.7	22.3	22.3	11.0	41.1	41.1	16.0	46.1	46.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	12.4	26.9	26.9	5.7	15.7	15.7	15.4	44.2	44.2	19.5	48.3	48.3
Actuated g/C Ratio	0.11	0.23	0.23	0.05	0.14	0.14	0.13	0.38	0.38	0.17	0.42	0.42
v/c Ratio	0.88	0.28	0.23	0.50	0.67	0.32	0.68	0.49	0.84	0.76	0.86	0.14
Control Delay	76.2	39.1	3.5	79.0	54.8	2.5	65.3	29.3	28.1	64.9	38.5	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.2	39.1	3.5	79.0	54.8	2.5	65.3	29.3	28.1	64.9	38.5	0.4
LOS	E	D	А	E	D	А	E	С	С	E	D	A
Approach Delay		54.5			43.3			32.3			39.4	
Approach LOS		D			D			С			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 115												
Actuated Cycle Length: 1												
Offset: 0 (0%), Reference	ed to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 95												
Control Type: Actuated-C	Coordinated											
Maximum v/c Ratio: 0.88												
Intersection Signal Delay						on LOS: E						
Intersection Capacity Utili	ization 78.59	%		10	CU Level	of Servic	e D					
Analysis Period (min) 15												
Oulite and Dharass 2. D					//							

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17 s	52 s	11.3 s	34.7 s	
★ ∕ø5	📕 🍡 Ø6 (R)	<mark>ک</mark> و	₩ Ø8	
22 s	47 s	18 s	28 s	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	<u>۲</u>	- † †	1	ሻ	^	1	<u>۲</u>	- † †	1
Traffic Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Future Volume (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3252	1651	1354	1346	3109	1275	1527	3226	1515	1500	3196	1354
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3252	1651	1354	1346	3109	1275	1527	3226	1515	1500	3196	1354
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	307	107	99	33	283	109	139	607	657	192	1148	98
RTOR Reduction (vph)	0	0	76	0	0	92	0	0	204	0	0	59
Lane Group Flow (vph)	307	107	23	33	283	17	139	607	453	192	1148	39
Heavy Vehicles (%)	2%	9%	13%	27%	10%	20%	12%	6%	1%	14%	7%	13%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	12.4	26.9	26.9	3.4	17.9	17.9	15.4	42.0	42.0	19.5	46.1	46.1
Effective Green, g (s)	12.4	26.9	26.9	3.4	17.9	17.9	15.4	42.0	42.0	19.5	46.1	46.1
Actuated g/C Ratio	0.11	0.23	0.23	0.03	0.16	0.16	0.13	0.37	0.37	0.17	0.40	0.40
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	350	386	316	39	483	198	204	1178	553	254	1281	542
v/s Ratio Prot	c0.09	0.06		0.02	c0.09		0.09	0.19		c0.13	c0.36	
v/s Ratio Perm			0.02			0.01			0.30			0.03
v/c Ratio	0.88	0.28	0.07	0.85	0.59	0.09	0.68	0.52	0.82	0.76	0.90	0.07
Uniform Delay, d1	50.5	36.1	34.3	55.5	45.1	41.5	47.5	28.5	33.0	45.5	32.2	21.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.1	0.4	0.1	84.9	1.8	0.2	9.0	1.6	12.7	12.1	10.0	0.3
Delay (s)	71.6	36.5	34.4	140.4	46.9	41.7	56.5	30.2	45.7	57.5	42.2	21.5
Level of Service	E	D	С	F	D	D	Е	С	D	E	D	С
Approach Delay (s)		57.1			52.9			40.1			42.8	
Approach LOS		E			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			44.9	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.84									
Actuated Cycle Length (s)			115.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		78.5%	IC	CU Level	of Service	9		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	1	111	1	ኘካ	1
Traffic Volume (vph)	204	735	1417	643	93	169
Future Volume (vph)	204	735	1417	643	93	169
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
				25.0 1	75.0 1	
Storage Lanes	1			I		1
Taper Length (m)	7.5	0.05	0.04	1.00	7.5	1.00
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	0.0-0			0.850	0.0-0	0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1527	2974	4636	1404	2885	1297
Flt Permitted	0.096				0.950	
Satd. Flow (perm)	154	2974	4636	1404	2885	1297
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				346		169
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	12%	15%	6%	9%	15%	18%
Adj. Flow (vph)	204	735	1417	9% 643	93	16%
	204	130	1417	043	93	109
Shared Lane Traffic (%)	004	705	4 4 4 7	040	00	400
Lane Group Flow (vph)	204	735	1417	643	93	169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
	2.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)						
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel		^	^			
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	рт+рі 7	10A	NA 8	1 CIIII	6	1 CIIII
	-	4	0	0	0	<u>^</u>
Permitted Phases	4		•	8	0	6
Detector Phase	7	4	8	8	6	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0

2023 - AM Peak Hour Total 11:21 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR		
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7		
Total Split (s)	18.0	61.0	43.0	43.0	29.0	29.0		
Total Split (%)	20.0%	67.8%	47.8%	47.8%	32.2%	32.2%		
Maximum Green (s)	11.8	54.8	36.8	36.8	23.3	23.3		
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3		
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7		
Lead/Lag	Lead		Lag	Lag				
Lead-Lag Optimize?	Yes		Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		
Recall Mode	None	None	None	None	C-Max	C-Max		
Walk Time (s)			20.0	20.0	7.0	7.0		
Flash Dont Walk (s)			7.0	7.0	15.0	15.0		
Pedestrian Calls (#/hr)			0	0	0	0		
Act Effct Green (s)	53.2	53.2	35.4	35.4	24.9	24.9		
Actuated g/C Ratio	0.59	0.59	0.39	0.39	0.28	0.28		
v/c Ratio	0.76	0.42	0.78	0.84	0.12	0.35		
Control Delay	37.6	10.7	27.2	22.9	25.8	6.7		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	37.6	10.7	27.2	22.9	25.8	6.7		
LOS	D	В	С	С	С	А		
Approach Delay		16.5	25.9		13.5			
Approach LOS		В	С		В			
Intersection Summary								
Area Type:	Other							
Cycle Length: 90								
Actuated Cycle Length: 90	0							
Offset: 0 (0%), Reference		2: and 6:9	SBL, Star	t of Gree	n			
Natural Cycle: 80			,					
Control Type: Actuated-C	oordinated							
Maximum v/c Ratio: 0.84								
Intersection Signal Delay:	22.2			l	ntersectio	n LOS: C		
Intersection Capacity Utili		6				of Service		
Analysis Period (min) 15								

Splits and Phases: 14: Walkley Rd & Lancaster F

	4 ₀₄		
	61s		
1. L		*	
Ø6 (R)	Ø7	Ø8	
29 s	18 s	43 s	

10-14-2021

Movement EBL EBT WBT WBR SBL SBR Lane Configurations 1 1 1 643 93 169 Future Volume (vph) 204 735 1417 643 93 169 Ideal Flow (vph) 1800 1800 1800 1800 1800 1800 1800 Total Lost time (s) 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.95 1.00 Fit Protected 0.95 1.00 1.00 0.95 1.00 1.00 885 1297 Fit Protected 0.10 1.0		۶	-	←	•	1	- ✓	
Lane Configurations Image: Configuration in the image: Configuratin the image: Configuration in the image: Configuration in the im	Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Traffic Volume (vph) 204 735 1417 643 93 169 Future Volume (vph) 1800 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00								
Future Volume (vph) 204 735 1417 643 93 169 Ideal Flow (vphpl) 1800 1800 1800 1800 1800 1800 1800 Total Lost time (s) 6.2 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Fit Factor 1.00 1.00 1.00 0.95 1.00 Std. Flow (prot) 1527 2974 4636 1404 2885 1297 Fit Permitted 0.10 1.00 1.00 1.00 1.00 1.00 Std. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18%								
Ideal Flow (vphpl) 1800 1800 1800 1800 1800 1800 1800 Total Lost time (s) 6.2 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Fit 0.00 1.00 1.00 0.85 1.00 0.85 Fit Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1527 2974 4636 1404 2885 1297 Fit Permitted 0.10 1.00 1.00 1.00 1.00 1.00 3.0 3.0 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18%	· · · <i>i</i>							
Total Lost time (s) 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (port) 1527 2974 4636 1404 2885 1297 Flt Permitted 0.10 1.00 1.00 1.00 1.00 1.00 1.00 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj Flow (ph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 122 Lane Group Flow (vph) 244 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% <td>(, , ,</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	(, , ,							
Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1527 2974 4636 1404 2885 1297 Flt Permitted 0.10 1.00 1.00 1.00 1.00 9.5 1.00 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn			6.2	6.2	6.2		5.7	
Fit Protected 0.95 1.00 1.00 1.00 2.95 1.00 Satd. Flow (port) 1527 2974 4636 1404 2885 1297 Fit Permitted 0.10 1.00 1.00 0.95 1.00 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Pot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 0 9 0.28 0.28 0.28 <td>Lane Util. Factor</td> <td>1.00</td> <td>0.95</td> <td>0.91</td> <td>1.00</td> <td>0.97</td> <td>1.00</td> <td></td>	Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Satd. Flow (prot) 1527 2974 4636 1404 2885 1297 Flt Permitted 0.10 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9 24.9	Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Filt Permitted 0.10 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.39 0.39	Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm) 155 2974 4636 1404 2885 1297 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 6.2 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757	Satd. Flow (prot)	1527	2974	4636	1404	2885	1297	
Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 53.2 53.2 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0	· · · /		1.00	1.00	1.00	0.95		
Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 4 4.24.9 24.9 Actuated Green, G (s) 53.2 53.2 35.4 24.9 24.9 24.9 Actuated g/C Ratio 0.59 0.39 0.39 0.28 0.28 C28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0	Satd. Flow (perm)	155	2974	4636	1404	2885	1297	
Adj. Flow (vph) 204 735 1417 643 93 169 RTOR Reduction (vph) 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 6 Actuated Green, G (s) 53.2 53.2 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 24.9 24.9 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Port c0.10 0.25 0.31 c0.04 v/c Ratio 0.76	U _ /	1.00	1.00	1.00	1.00		1.00	
RTOR Reduction (vph) 0 0 210 0 122 Lane Group Flow (vph) 204 735 1417 433 93 47 Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 c0.04 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>								
Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Perm c0.35 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0			0	0	210	0	122	
Heavy Vehicles (%) 12% 15% 6% 9% 15% 18% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Perm c0.35 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0	Lane Group Flow (vph)	204	735	1417	433	93	47	
Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 c0.04 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 <td></td> <td>12%</td> <td>15%</td> <td>6%</td> <td>9%</td> <td>15%</td> <td>18%</td> <td></td>		12%	15%	6%	9%	15%	18%	
Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 c0.04 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 <td></td> <td>pm+pt</td> <td></td> <td>NA</td> <td>Perm</td> <td>Prot</td> <td>Perm</td> <td></td>		pm+pt		NA	Perm	Prot	Perm	
Permitted Phases 4 8 6 Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 c0.04 0.76 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7								
Actuated Green, G (s) 53.2 53.2 35.4 35.4 24.9 24.9 Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C <		4			8		6	
Effective Green, g (s) 53.2 53.2 35.4 35.4 24.9 24.9 Actuated g/C Ratio 0.59 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 0.03 .0 .0 v/s Ratio Perm c0.35 0.31 c0.04 .0 .0 .0 .12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C	Actuated Green, G (s)		53.2	35.4	35.4	24.9	24.9	
Actuated g/C Ratio 0.59 0.39 0.39 0.28 0.28 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 0.03 v/s Ratio Perm c0.35 0.31 c0.04 V/s Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Se			53.2	35.4	35.4	24.9		
Clearance Time (s) 6.2 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 0.03		0.59	0.59	0.39	0.39	0.28	0.28	
Lane Grp Cap (vph) 268 1757 1823 552 798 358 v/s Ratio Prot c0.10 0.25 0.31 0.03 0.04 v/s Ratio Perm c0.35 0.31 c0.04 0.78 0.12 0.13 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C C Intersection Summary HCM 2000 Control Delay 23.7 HCM 2000 Level of Service Actuated Cycle Length (s) 90.0		6.2	6.2	6.2	6.2	5.7	5.7	
v/s Ratio Prot c0.10 0.25 0.31 0.03 v/s Ratio Perm c0.35 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C Intersection Summary 23.7 HCM 2000 Level of Service HCM 2000 Control Delay 23.7 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.57 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min	Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
v/s Ratio Prot c0.10 0.25 0.31 0.03 v/s Ratio Perm c0.35 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C Intersection Summary 23.7 HCM 2000 Level of Service HCM 2000 Control Delay 23.7 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.57 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min	Lane Grp Cap (vph)	268	1757	1823	552	798	358	
v/s Ratio Perm c0.35 0.31 c0.04 v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C C Intersection Summary 23.7 HCM 2000 Level of Service HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.57 0.57 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min) 15						0.03		
v/c Ratio 0.76 0.42 0.78 0.78 0.12 0.13 Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C Intersection Summary 23.7 HCM 2000 Level of Service HCM 2000 Control Delay 23.7 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.57 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min) 15 15					0.31		c0.04	
Uniform Delay, d1 19.7 10.0 23.9 24.0 24.3 24.4 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C Intersection Summary 23.7 HCM 2000 Level of Service HCM 2000 Control Delay 23.7 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.57 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min) 15			0.42	0.78	0.78	0.12		
Progression Factor 1.00 <th1.00< th=""> 1.00 1.00<td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th1.00<>								
Incremental Delay, d2 12.0 0.2 2.2 7.2 0.3 0.8 Delay (s) 31.8 10.2 26.0 31.2 24.6 25.2 Level of Service C B C C C Approach Delay (s) 14.8 27.6 25.0 Approach Delay (s) 14.8 27.6 25.0 Approach LOS B C C Intersection Summary B C C C C C HCM 2000 Control Delay 23.7 HCM 2000 Level of Service HCM 2000 Level of Service Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 64.3% ICU Level of Service Analysis Period (min) 15 15		1.00	1.00	1.00	1.00	1.00	1.00	
Level of ServiceCBCCCCApproach Delay (s)14.827.625.0Approach LOSBCCIntersection SummaryHCM 2000 Control Delay23.7HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15		12.0	0.2	2.2	7.2	0.3	0.8	
Approach Delay (s)14.827.625.0Approach LOSBCCIntersection Summary23.7HCM 2000 Level of ServiceHCM 2000 Control Delay23.7HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	Delay (s)	31.8	10.2	26.0	31.2	24.6	25.2	
Approach LOSBCCIntersection SummaryHCM 2000 Control Delay23.7HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15		С	В	С	С	С	С	
Intersection SummaryHCM 2000 Control Delay23.7HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	Approach Delay (s)		14.8	27.6		25.0		
HCM 2000 Control Delay23.7HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	Approach LOS		В	С		С		
HCM 2000 Volume to Capacity ratio0.57Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	Intersection Summary							
Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	HCM 2000 Control Delay			23.7	H	CM 2000	Level of Serv	vice
Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15	,	acity ratio		0.57				
Intersection Capacity Utilization64.3%ICU Level of ServiceAnalysis Period (min)15					S	um of los	t time (s)	
		ation		64.3%				
	Analysis Period (min)			15				

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ኘኘ	↑	1	٦	† †	1	٦	††	1	5	††	1
Traffic Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Future Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.950			0.950			0.950			0.950	0200	
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			227			191			364			189
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Adj. Flow (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Shared Lane Traffic (%)	0.11	200		102	100	200	110	1120	001	100	110	
Lane Group Flow (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	. ug. u
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	•••	•••	0	. <u>_</u> ,	0/	0/.	•••	•••	. <u>.</u>	••• =	•••	U . <u>-</u> /
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	9.4		0.0	9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		• •			0/			•••			•••	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	1 0111
Permitted Phases	,		4	Ű.	Ű.	8		v	6	Ŭ	<u> </u>	2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase	,	7	7	0	5	5		J	0	J		2
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	25.2	36.3	36.3	16.9	28.0	28.0	20.0	47.8	47.8	14.0	41.8	41.8
Total Split (%)	21.9%	31.6%	31.6%	14.7%	24.3%	24.3%	17.4%	41.6%	41.6%	12.2%	36.3%	36.3%
Maximum Green (s)	19.6	30.6	30.6	11.3	22.3	22.3	14.0	41.9	41.9	8.0	35.9	35.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	19.5	23.7	23.7	10.4	14.6	14.6	13.8	46.0	46.0	11.7	43.9	43.9
Actuated g/C Ratio	0.17	0.21	0.21	0.09	0.13	0.13	0.12	0.40	0.40	0.10	0.38	0.38
v/c Ratio	0.92	0.76	0.47	0.68	0.25	0.66	0.70	0.84	0.44	0.63	0.59	0.14
Control Delay	70.8	56.4	7.8	73.0	44.7	19.6	66.9	39.0	4.5	68.5	32.5	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.8	56.4	7.8	73.0	44.7	19.6	66.9	39.0	4.5	68.5	32.5	0.4
LOS	E	E	А	E	D	В	E	D	A	E	С	A
Approach Delay		52.9			37.9			33.6			33.1	
Approach LOS		D			D			С			С	
Intersection Summary												
Area Type:	Other											
Cycle Length: 115												
Actuated Cycle Length: 11												
Offset: 0 (0%), Referenced	to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.92												
Intersection Signal Delay:					ntersectio							
Intersection Capacity Utiliz	ation 79.49	%		10	CU Level	of Servic	e D					
Analysis Period (min) 15												

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20 s	41.8 s	16.9 s	36.3 s
★ ∕ø5	🛰 📭 (R)	⊅ _{∅7}	₩ Ø8
14 s	47.8 s	25.2 s	28 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	۳.	- ††	1	<u>۲</u>	- ††	1	۳.	- ††	1
Traffic Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Future Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
RTOR Reduction (vph)	0	0	180	0	0	167	0	0	218	0	0	59
Lane Group Flow (vph)	514	280	47	102	108	68	140	1120	146	100	743	36
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	19.5	23.7	23.7	10.4	14.6	14.6	13.8	46.0	46.0	11.7	43.9	43.9
Effective Green, g (s)	19.5	23.7	23.7	10.4	14.6	14.6	13.8	46.0	46.0	11.7	43.9	43.9
Actuated g/C Ratio	0.17	0.21	0.21	0.09	0.13	0.13	0.12	0.40	0.40	0.10	0.38	0.38
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	557	367	306	150	429	192	201	1341	606	158	1255	578
v/s Ratio Prot	c0.16	c0.16		0.06	0.03		c0.08	c0.33		0.06	0.23	
v/s Ratio Perm			0.03			0.05			0.10			0.02
v/c Ratio	0.92	0.76	0.15	0.68	0.25	0.36	0.70	0.84	0.24	0.63	0.59	0.06
Uniform Delay, d1	47.0	43.0	37.4	50.7	45.3	45.9	48.6	31.1	22.9	49.6	28.4	22.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.0	9.1	0.2	12.0	0.3	1.1	10.0	6.3	0.9	8.0	2.1	0.2
Delay (s)	68.0	52.1	37.7	62.6	45.6	47.0	58.6	37.4	23.8	57.6	30.5	22.7
Level of Service	E	D	D	E	D	D	E	D	С	E	С	С
Approach Delay (s)		56.9			50.3			36.2			32.6	
Approach LOS		E			D			D			С	
Intersection Summary												
HCM 2000 Control Delay			42.1	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.86									
Actuated Cycle Length (s)			115.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		79.4%	IC	CU Level	of Service	е		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	1	^	1	ኘካ	1
Traffic Volume (vph)	126	1756	952	164	483	205
Future Volume (vph)	126	1756	952	164	483	205
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0			25.0	75.0	150.0
Storage Lanes	1			1	1	1
Taper Length (m)	7.5				7.5	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	1.00	0.00	0.01	0.850	0.01	0.850
Flt Protected	0.950			0.000	0.950	0.000
Satd. Flow (prot)	1676	3257	4508	1404	3252	1485
Flt Permitted	0.212	5251	-500	1-10-1	0.950	1400
Satd. Flow (perm)	374	3257	4508	1404	3252	1485
Right Turn on Red	514	5257	4000	Yes	5252	Yes
Satd. Flow (RTOR)		50	50	147	50	205
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)	1	17.8	28.3	1.00	13.5	1.00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	5%	9%	9%	2%	3%
Adj. Flow (vph)	126	1756	952	164	483	205
Shared Lane Traffic (%)						
Lane Group Flow (vph)	126	1756	952	164	483	205
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.0	0.0	2.0	2.0	2.0
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Z.0 CI+Ex
Detector 1 Channel	OI+EX	UI+EX	UI+EX	UI+EX	OI+EX	OI+EX
	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Detector Phase	7	4	8	8	6	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
	5.0	0.0	0.0	0.0	5.0	0.0

2023 - PM Peak Hour Background 10:13 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7	
Total Split (s)	13.4	52.0	38.6	38.6	28.0	28.0	
Total Split (%)	16.8%	65.0%	48.3%	48.3%	35.0%	35.0%	
Maximum Green (s)	7.2	45.8	32.4	32.4	22.3	22.3	
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3	
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lead/Lag	Lead		Lag	Lag			
Lead-Lag Optimize?	Yes		Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	None	None	C-Max	C-Max	
Walk Time (s)			20.0	20.0	7.0	7.0	
Flash Dont Walk (s)			7.0	7.0	15.0	15.0	
Pedestrian Calls (#/hr)			0	0	0	0	
Act Effct Green (s)	45.6	45.6	34.8	34.8	22.5	22.5	
Actuated g/C Ratio	0.57	0.57	0.44	0.44	0.28	0.28	
v/c Ratio	0.39	0.95	0.48	0.24	0.53	0.36	
Control Delay	11.5	28.9	17.9	4.7	26.8	5.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.5	28.9	17.9	4.7	26.8	5.6	
LOS	В	С	В	А	С	А	
Approach Delay		27.7	16.0		20.5		
Approach LOS		С	В		С		
Intersection Summary							
Area Type:	Other						
Cycle Length: 80							
Actuated Cycle Length: 8	0						
Offset: 0 (0%), Reference	d to phase :	2: and 6:	SBL, Star	t of Gree	n		
Natural Cycle: 80							
Control Type: Actuated-C	oordinated						
Maximum v/c Ratio: 0.95							
Intersection Signal Delay:	22.8			h	ntersectio	on LOS: C	2
Intersection Capacity Utili	zation 75.7	%](CU Level	of Servic	ce D
Analysis Period (min) 15							
Splits and Phases: 14:	Walkley Rd	& Lanca	ster Rd				

opilio ana i nacio. I n. Walki	by 1 ta a Eanoaotor 1 ta		
	52 s		
Ø6 (R)	<u>∕</u> ≉ _{∅7}	4 [⊕] Ø8	
28 s	13.4 s	38.6 s	

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	٢	††	^	1	ኘካ	1		
Traffic Volume (vph)	126	1756	952	164	483	205		
Future Volume (vph)	126	1756	952	164	483	205		
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800		
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7		
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00		
Frt	1.00	1.00	1.00	0.85	1.00	0.85		
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (prot)	1676	3257	4508	1404	3252	1485		
Flt Permitted	0.21	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (perm)	374	3257	4508	1404	3252	1485		
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00		
Adj. Flow (vph)	126	1756	952	164	483	205		
RTOR Reduction (vph)	0	0	0	83	0	150		
Lane Group Flow (vph)	126	1756	952	81	483	55		
Heavy Vehicles (%)	2%	5%	9%	9%	2%	3%		
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm		
Protected Phases	7	4	8		6			
Permitted Phases	4			8		6		
Actuated Green, G (s)	46.8	46.8	34.8	34.8	21.3	21.3		
Effective Green, g (s)	46.8	46.8	34.8	34.8	21.3	21.3		
Actuated g/C Ratio	0.58	0.58	0.43	0.43	0.27	0.27		
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	313	1905	1960	610	865	395		
v/s Ratio Prot	0.03	c0.54	0.21		c0.15			
v/s Ratio Perm	0.21			0.06		0.04		
v/c Ratio	0.40	0.92	0.49	0.13	0.56	0.14		
Uniform Delay, d1	8.4	15.0	16.2	13.6	25.3	22.4		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	0.8	7.9	0.2	0.1	2.6	0.7		
Delay (s)	9.3	22.9	16.4	13.7	27.9	23.1		
Level of Service	A	С	B	В	С	С		
Approach Delay (s)		22.0	16.0		26.5			
Approach LOS		С	В		С			
Intersection Summary								
HCM 2000 Control Delay			21.0	H	CM 2000	Level of Servi	се	С
HCM 2000 Volume to Capa	acity ratio		0.89					
Actuated Cycle Length (s)			80.0	S	um of los	t time (s)		18.1
Intersection Capacity Utilization	ation		75.7%	IC	CU Level	of Service		D
Analysis Period (min)			15					
c Critical Lane Group								

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ኘኘ	1	1	۲	^	1	1	† †	1	۲	† †	1
Traffic Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Future Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0	1000	85.0	55.0	1000	30.0	130.0	1000	115.0	185.0	1000	35.0
Storage Lanes	1		1	1		1	100.0		1	100.0		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	0.57	1.00	0.850	1.00	0.00	0.850	1.00	0.00	0.850	1.00	0.00	0.850
Flt Protected	0.950		0.000	0.950		0.000	0.950		0.000	0.950		0.000
Satd. Flow (prot)	3285	1667	1485	1541	3167	1404	1569	3353	1515	1555	3320	1430
Flt Permitted	0.950	1007	1400	0.950	0107	1-0-1	0.950	0000	1010	0.950	0020	1400
Satd. Flow (perm)	3285	1667	1485	1541	3167	1404	1569	3353	1515	1555	3320	1430
Right Turn on Red	5205	1007	Yes	10-11	5107	Yes	1005	0000	Yes	1000	5520	Yes
Satd. Flow (RTOR)			227			248			364			246
Link Speed (k/h)		50	221		50	240		50	504		50	240
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			132.0			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	1.00	8%	3%	1.00	8%	9%	9%	2%	1.00	1.00	3%	7%
Heavy Vehicles (%)	514	280	227	102	0% 108	9% 235	9% 140	1120	364	10%	5% 743	95
Adj. Flow (vph)	514	200	221	102	100	235	140	1120	304	100	743	95
Shared Lane Traffic (%)	E14	200	007	100	100	025	140	1100	264	100	740	05
Lane Group Flow (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07	4.07
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25	0	15 1	25	0	15	25 1	0	15 1	25	0	15
Number of Detectors	1	2	-	1	2	1 Dialat	•	2	•	1	2	1 Dialet
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel	• •		• •									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	_
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	_
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2023 - PM Peak Hour Total 11:44 am 09-22-2021

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	25.2	35.5	35.5	17.7	28.0	28.0	21.0	47.8	47.8	14.0	40.8	40.8
Total Split (%)	21.9%	30.9%	30.9%	15.4%	24.3%	24.3%	18.3%	41.6%	41.6%	12.2%	35.5%	35.5%
Maximum Green (s)	19.6	29.8	29.8	12.1	22.3	22.3	15.0	41.9	41.9	8.0	34.9	34.9
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	19.5	24.2	24.2	11.0	15.7	15.7	14.2	45.5	45.5	11.0	42.4	42.4
Actuated g/C Ratio	0.17	0.21	0.21	0.10	0.14	0.14	0.12	0.40	0.40	0.10	0.37	0.37
v/c Ratio	0.92	0.80	0.46	0.69	0.25	0.58	0.73	0.84	0.44	0.67	0.61	0.14
Control Delay	70.8	59.8	7.7	74.0	43.8	10.4	69.6	39.7	4.5	73.3	33.9	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.8	59.8	7.7	74.0	43.8	10.4	69.6	39.7	4.5	73.3	33.9	0.4
LOS	E	E	А	E	D	В	E	D	А	E	С	A
Approach Delay		53.8			33.1			34.4			34.7	
Approach LOS		D			С			С			С	
Intersection Summary												
Area Type:	Other											
Cycle Length: 115												
Actuated Cycle Length: 11												
Offset: 0 (0%), Reference	d to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.92												
Intersection Signal Delay:						n LOS: E						
Intersection Capacity Utiliz	zation 79.4%	6		10	CU Level	of Servic	e D					
Analysis Period (min) 15												

∕•ø1	Ø2 (R)	€ ø3	1 04
21 s	40.8 s	17.7 s	35.5 s
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14 s	47.8 s	25.2 s	28 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	•	1	٦	<u></u>	1	٦	- † †	1	٦	<u></u>	1
Traffic Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Future Volume (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3285	1667	1485	1541	3167	1404	1569	3353	1515	1555	3320	1430
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3285	1667	1485	1541	3167	1404	1569	3353	1515	1555	3320	1430
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	514	280	227	102	108	235	140	1120	364	100	743	95
RTOR Reduction (vph)	0	0	179	0	0	203	0	0	220	0	0	60
Lane Group Flow (vph)	514	280	48	102	108	32	140	1120	144	100	743	35
Heavy Vehicles (%)	1%	8%	3%	11%	8%	9%	9%	2%	1%	10%	3%	7%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	19.5	24.2	24.2	11.0	15.7	15.7	14.2	45.6	45.6	11.0	42.4	42.4
Effective Green, g (s)	19.5	24.2	24.2	11.0	15.7	15.7	14.2	45.6	45.6	11.0	42.4	42.4
Actuated g/C Ratio	0.17	0.21	0.21	0.10	0.14	0.14	0.12	0.40	0.40	0.10	0.37	0.37
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	557	350	312	147	432	191	193	1329	600	148	1224	527
v/s Ratio Prot	c0.16	c0.17		0.07	0.03		c0.09	c0.33		0.06	0.22	
v/s Ratio Perm			0.03			0.02			0.10			0.02
v/c Ratio	0.92	0.80	0.15	0.69	0.25	0.17	0.73	0.84	0.24	0.68	0.61	0.07
Uniform Delay, d1	47.0	43.1	37.0	50.4	44.4	43.9	48.5	31.5	23.1	50.3	29.5	23.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.0	12.3	0.2	13.3	0.3	0.4	12.7	6.6	0.9	11.6	2.2	0.2
Delay (s)	68.0	55.4	37.3	63.6	44.7	44.3	61.2	38.1	24.1	61.8	31.8	23.7
Level of Service	E	E	D	E	D	D	E	D	С	E	С	С
Approach Delay (s)		57.7			48.8			36.9			34.2	
Approach LOS		E			D			D			С	
Intersection Summary												
HCM 2000 Control Delay			42.9	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.88									
Actuated Cycle Length (s)			115.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		79.4%	IC	U Level	of Servic	е		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	<u></u>	^	1	<u>ካ</u> ካ	7
Traffic Volume (vph)	126	1756	952	164	483	205
Future Volume (vph)	120	1756	952	164	483	205
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
	120.0	1000	1000	25.0	75.0	150.0
Storage Length (m)						
Storage Lanes	1			1	1	1
Taper Length (m)	7.5	0.05	0.04	4.00	7.5	4.00
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt				0.850		0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1583	3257	4550	1330	3043	1391
Flt Permitted	0.213				0.950	
Satd. Flow (perm)	355	3257	4550	1330	3043	1391
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				147		205
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	8%	5%	8%	15%	9%	10%
Adj. Flow (vph)	126	1756	952	164	483	205
Shared Lane Traffic (%)	120	1750	302	104	+03	200
Lane Group Flow (vph)	126	1756	952	164	483	205
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.0	0.0	2.0	2.0	2.0
	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Type	CI+EX	UI+EX	UI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4		J	8	Ĵ	6
Detector Phase	7	4	8	8	6	6
Switch Phase	1	4	U	U	U	U
	EO	5.0	E 0	ΕO	E 0	EO
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0

2023 - PM Peak Hour Total 11:44 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	13.2	52.0	38.8	38.8	28.0	28.0
Total Split (%)	16.5%	65.0%	48.5%	48.5%	35.0%	35.0%
Maximum Green (s)	7.0	45.8	32.6	32.6	22.3	22.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	45.6	45.6	35.0	35.0	22.5	22.5
Actuated g/C Ratio	0.57	0.57	0.44	0.44	0.28	0.28
v/c Ratio	0.41	0.95	0.48	0.25	0.56	0.38
Control Delay	12.2	28.9	17.7	4.7	27.7	5.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.2	28.9	17.7	4.7	27.7	5.9
LOS	В	С	В	А	С	А
Approach Delay		27.8	15.8		21.2	
Approach LOS		С	В		С	
Intersection Summary						
Area Type:	Other					
Cycle Length: 80						
Actuated Cycle Length: 8	0					
Offset: 0 (0%), Reference	ed to phase 2	2: and 6:8	SBL, Star	t of Gree	n	
Natural Cycle: 80						
Control Type: Actuated-C	coordinated					
Maximum v/c Ratio: 0.95						
Intersection Signal Delay						on LOS: C
Intersection Capacity Util	ization 75.7%	%		l	CU Level	of Servic
Analysis Period (min) 15						
Splits and Phases: 1/:	Walklov Dd	8 anaa	otor Dd			

Splits and Phases:	14: Walkley Rd & Lancaster Rd
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	52 s		
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Ø6 (R)	🖌 Ø7	Ø8	
28 s	13.2 s	38.8 s	

10-14-2021	
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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	۲	† †	^	1	ኘካ	1	
Traffic Volume (vph)	126	1756	952	164	483	205	
Future Volume (vph)	126	1756	952	164	483	205	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1583	3257	4550	1330	3043	1391	
Flt Permitted	0.21	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	354	3257	4550	1330	3043	1391	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	126	1756	952	164	483	205	
RTOR Reduction (vph)	0	0	0	83	0	150	
Lane Group Flow (vph)	126	1756	952	81	483	55	
Heavy Vehicles (%)	8%	5%	8%	15%	9%	10%	
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm	
Protected Phases	7	4	8		6		
Permitted Phases	4			8		6	
Actuated Green, G (s)	46.8	46.8	35.0	35.0	21.3	21.3	
Effective Green, g (s)	46.8	46.8	35.0	35.0	21.3	21.3	
Actuated g/C Ratio	0.58	0.58	0.44	0.44	0.27	0.27	
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	293	1905	1990	581	810	370	
v/s Ratio Prot	0.03	c0.54	0.21		c0.16		
v/s Ratio Perm	0.22			0.06		0.04	
v/c Ratio	0.43	0.92	0.48	0.14	0.60	0.15	
Uniform Delay, d1	8.5	15.0	16.0	13.5	25.6	22.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.0	7.9	0.2	0.1	3.2	0.8	
Delay (s)	9.5	22.9	16.2	13.6	28.8	23.3	
Level of Service	А	С	В	В	С	С	
Approach Delay (s)		22.0	15.8		27.2		
Approach LOS		С	В		С		
Intersection Summary							
HCM 2000 Control Delay			21.1	Н	CM 2000	Level of Serv	vice
HCM 2000 Volume to Capac	city ratio		0.90				
Actuated Cycle Length (s)			80.0		um of los		
Intersection Capacity Utiliza	tion		75.7%	IC	U Level	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	- ከ	- † †	1	<u>۲</u>	- † †	1	- ሽ	- † †	1
Traffic Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Future Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			127			175			328			174
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Adj. Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Shared Lane Traffic (%)												
Lane Group Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			CI+Ex			CI+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2028 - AM Peak Hour Background 9:09 am 09-22-2021

10-14-2021	10	-14-202	!1
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	19.7	33.9	33.9	13.8	28.0	28.0	18.0	53.3	53.3	24.0	59.3	59.3
Total Split (%)	15.8%	27.1%	27.1%	11.0%	22.4%	22.4%	14.4%	42.6%	42.6%	19.2%	47.4%	47.4%
Maximum Green (s)	14.1	28.2	28.2	8.2	22.3	22.3	12.0	47.4	47.4	18.0	53.4	53.4
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	14.1	27.7	27.7	7.4	16.3	16.3	15.6	50.1	50.1	21.3	55.8	55.8
Actuated g/C Ratio	0.11	0.22	0.22	0.06	0.13	0.13	0.12	0.40	0.40	0.17	0.45	0.45
v/c Ratio	0.88	0.29	0.26	0.40	0.68	0.35	0.71	0.49	0.86	0.79	0.85	0.14
Control Delay	79.9	44.1	5.7	70.2	59.7	4.0	72.0	30.2	30.2	72.0	38.2	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.9	44.1	5.7	70.2	59.7	4.0	72.0	30.2	30.2	72.0	38.2	0.4
LOS	E	D	А	E	E	А	E	С	С	E	D	Α
Approach Delay		58.1			46.2			34.4			40.2	
Approach LOS		E			D			С			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 125												
Actuated Cycle Length: 12												
Offset: 0 (0%), Reference	d to phase 2	2:NWT a	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.88												
Intersection Signal Delay:						on LOS: E						
Intersection Capacity Utiliz	zation 81.5	%](CU Level	of Servic	e D					
Analysis Period (min) 15												

∖ ₀ _{Ø1}	× 2 (R)	€ ø3	
18 s	59.3 s	13.8 s	33.9 s
★ _{Ø5}	🛛 🎽 Ø6 (R)	<u>ک</u> _{Ø7}	Ø8
24 s	53.3 s	19.7 s	28 s

10-14-2022	1
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	٦	- † †	1	٦	<u></u>	1	٦	- † †	1
Traffic Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Future Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3252	1748	1354	1437	3353	1354	1644	3226	1515	1500	3196	1443
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
RTOR Reduction (vph)	0	0	81	0	0	98	0	0	202	0	0	59
Lane Group Flow (vph)	323	113	23	34	297	17	146	638	488	202	1206	44
Heavy Vehicles (%)	2%	3%	13%	19%	2%	13%	4%	6%	1%	14%	7%	6%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	14.1	27.7	27.7	4.9	18.5	18.5	15.6	47.9	47.9	21.3	53.6	53.6
Effective Green, g (s)	14.1	27.7	27.7	4.9	18.5	18.5	15.6	47.9	47.9	21.3	53.6	53.6
Actuated g/C Ratio	0.11	0.22	0.22	0.04	0.15	0.15	0.12	0.38	0.38	0.17	0.43	0.43
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	366	387	300	56	496	200	205	1236	580	255	1370	618
v/s Ratio Prot	c0.10	0.06		0.02	c0.09		0.09	0.20		c0.13	c0.38	
v/s Ratio Perm			0.02			0.01			0.32			0.03
v/c Ratio	0.88	0.29	0.08	0.61	0.60	0.09	0.71	0.52	0.84	0.79	0.88	0.07
Uniform Delay, d1	54.6	40.5	38.5	59.1	49.8	45.9	52.5	29.6	35.1	49.7	32.8	21.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.3	0.4	0.1	17.2	2.0	0.2	11.1	1.5	13.8	15.4	8.4	0.2
Delay (s)	75.9	40.9	38.6	76.3	51.7	46.1	63.6	31.2	48.8	65.1	41.1	21.3
Level of Service	E	D	D	E	D	D	E	С	D	E	D	С
Approach Delay (s)		61.4			52.2			42.7			43.0	
Approach LOS		E			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			46.4	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.84									
Actuated Cycle Length (s)			125.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		81.5%	IC	CU Level	of Service	9		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	۲	† †	^	1	ኘካ	1
Traffic Volume (vph)	218	735	1489	676	97	178
Future Volume (vph)	218	735	1489	676	97	178
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			20.0	1	130.0
Taper Length (m)	7.5				7.5	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	1.00	0.95	0.91	0.850	0.97	0.850
	0.050			0.650	0.050	0.650
Fit Protected	0.950	0040	4500	4500	0.950	4004
Satd. Flow (prot)	1613	2948	4593	1500	3072	1391
Flt Permitted	0.095		1500	1500	0.950	1001
Satd. Flow (perm)	161	2948	4593	1500	3072	1391
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				346		178
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	6%	16%	7%	2%	8%	10%
Adj. Flow (vph)	218	735	1489	676	97	178
Shared Lane Traffic (%)	210		1.00	010	51	
Lane Group Flow (vph)	218	735	1489	676	97	178
Enter Blocked Intersection	No	No	No	No	No	No
	Left	Left	Left		Left	
Lane Alignment	Leit			Right		Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
()	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)						
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		CI+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Detector Phase	7	4	8	8	6	6
Switch Phase	,	· · ·	J	J	J	J
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
	5.0	0.0	0.0	5.0	5.0	5.0

2028 - AM Peak Hour Background 9:09 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	18.0	61.0	43.0	43.0	29.0	29.0
Total Split (%)	20.0%	67.8%	47.8%	47.8%	32.2%	32.2%
Maximum Green (s)	11.8	54.8	36.8	36.8	23.3	23.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	53.8	53.8	36.0	36.0	24.3	24.3
Actuated g/C Ratio	0.60	0.60	0.40	0.40	0.27	0.27
v/c Ratio	0.77	0.42	0.81	0.84	0.12	0.35
Control Delay	37.1	10.4	28.1	22.4	25.9	6.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.1	10.4	28.1	22.4	25.9	6.5
LOS	D	В	С	С	С	А
Approach Delay		16.5	26.3		13.3	
Approach LOS		В	С		В	
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 9	0					
Offset: 0 (0%), Reference		2: and 6:8	SBL, Star	t of Gree	n	
Natural Cycle: 80						
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.84						
Intersection Signal Delay:	22.5			l	ntersectio	on LOS: C
Intersection Capacity Utili		6				of Service
Analysis Period (min) 15						

Splits and Phases:	14: Walkley Rd & Lancaster Rd
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🖉 🖉 Ø6 (R)	Ø7	Ø8	
29 s	18 s	43 s	

10-14-2021	
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Movement EBL EBT WBT WBR SBL SBR Lane Configurations 1 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 1 0 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 1 1 1 1 1 1 1		۶	-	+	•	1	-	
Lane Configurations Image: Configurations <	Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Traffic Volume (vph) 218 735 1489 676 97 178 Future Volume (vph) 1800 1800 1800 1800 1800 1800 Ideal Flow (vphp) 1800 1800 1800 1800 1800 1800 Total Lost time (s) 6.2 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 1.00 1.00 0.95 1.00 0.97 1.00 Stat. Flow (prot) 1613 2948 4593 1500 3072 1391 Fit Permitted 0.09 1.00 1.00 1.00 1.00 1.00 1.00 Stat. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>								
Future Volume (vph) 218 735 1489 676 97 178 Ideal Flow (vphpl) 1800 1800 1800 1800 1800 1800 Total Lost time (s) 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Fit Factor 1.00 1.00 1.00 0.95 1.00 Satd. Flow (port) 1613 2948 4593 1500 3072 1391 Fit Protected 0.99 1.00 1.00 1.00 0.95 1.00 Satd. Flow (pern) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) <t< td=""><td></td><td></td><td></td><td>1489</td><td></td><td></td><td></td><td></td></t<>				1489				
Total Lost time (s) 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (port) 1613 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (pph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 466 97 48 Heavy Vehicles (%)		218	735		676	97	178	
Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1613 2948 4593 1500 3072 1391 Flt Permitted 0.09 1.00 1.00 1.00 1.00 1.00 Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt <td< td=""><td>Ideal Flow (vphpl)</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td></td></td<>	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1613 2948 4593 1500 3072 1391 Flt Permitted 0.09 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Perm Permited Phases 4 8 6	Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Fit Protected 0.95 1.00 1.00 1.00 0.95 1.00 Satd. Flow (port) 1613 2948 4593 1500 3072 1391 Fit Permitted 0.09 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 53.8 5	Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Satd. Flow (prot) 1613 2948 4593 1500 3072 1391 Flt Permitted 0.09 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60	Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Fit Permitted 0.09 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 36.0 24.3 24.3 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 </td <td>Flt Protected</td> <td>0.95</td> <td>1.00</td> <td>1.00</td> <td>1.00</td> <td>0.95</td> <td>1.00</td> <td></td>	Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm) 161 2948 4593 1500 3072 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated G/C Ratio 0.60 0.60 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0	Satd. Flow (prot)	1613	2948	4593	1500	3072	1391	
Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 53.8 53.8 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 <	Flt Permitted	0.09	1.00	1.00	1.00	0.95	1.00	
Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375	Satd. Flow (perm)	161	2948	4593	1500	3072	1391	
Adj. Flow (vph) 218 735 1489 676 97 178 RTOR Reduction (vph) 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375	Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
RTOR Reduction (vph) 0 0 208 0 130 Lane Group Flow (vph) 218 735 1489 468 97 48 Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03		218	735	1489	676	97	178	
Heavy Vehicles (%) 6% 16% 7% 2% 8% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Perm c0.36 0.31 c0.03 v/c Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.		0	0	0	208	0	130	
Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Perm c0.36 0.31 c0.03 v/s Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00	Lane Group Flow (vph)	218	735	1489	468	97	48	
Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03	Heavy Vehicles (%)	6%	16%	7%	2%	8%	10%	
Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 53.8 53.8 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Pert c0.10 0.25 0.32 0.03 V/s Ratio Pert c0.36 0.31 c0.03 V/s Ratio Pert 1.00 1.00 1.00 1.00 1.00 1.00 Inform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor	Turn Type	pm+pt	NA	NA	Perm	Prot	Perm	
Actuated Green, G (s) 53.8 53.8 36.0 36.0 24.3 24.3 Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03 V/s Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5				8		6		
Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03 .0 .0 v/s Ratio Perm c0.36 0.31 c0.03 .0 .0 .0 .0 .0 .0 V/s Ratio 0.77 0.42 0.81 0.78 0.12 0.13 .0	Permitted Phases	4			8		6	
Effective Green, g (s) 53.8 53.8 36.0 36.0 24.3 24.3 Actuated g/C Ratio 0.60 0.60 0.40 0.40 0.27 0.27 Clearance Time (s) 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03 .0 .0 v/s Ratio Perm c0.36 0.31 c0.03 .0 .0 .0 .0 V/s Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 <t< td=""><td>Actuated Green, G (s)</td><td>53.8</td><td>53.8</td><td>36.0</td><td>36.0</td><td>24.3</td><td>24.3</td><td></td></t<>	Actuated Green, G (s)	53.8	53.8	36.0	36.0	24.3	24.3	
Clearance Time (s) 6.2 6.2 6.2 6.2 5.7 5.7 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03		53.8	53.8	36.0	36.0	24.3	24.3	
Vehicle Extension (s) 3.0 1.00 1.00 1.00 <td>Actuated g/C Ratio</td> <td>0.60</td> <td>0.60</td> <td></td> <td>0.40</td> <td>0.27</td> <td>0.27</td> <td></td>	Actuated g/C Ratio	0.60	0.60		0.40	0.27	0.27	
Lane Grp Cap (vph) 283 1762 1837 600 829 375 v/s Ratio Prot c0.10 0.25 0.32 0.03 0.0		6.2	6.2	6.2	6.2	5.7	5.7	
v/s Ratio Prot c0.10 0.25 0.32 0.03 v/s Ratio Perm c0.36 0.31 c0.03 v/c Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C Approach Delay (s) 15.0 27.8 25.4 Approach LOS B C C Intersection Summary 24.0 HCM 2000 Level of Service HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15 <td>Vehicle Extension (s)</td> <td>3.0</td> <td>3.0</td> <td>3.0</td> <td>3.0</td> <td>3.0</td> <td>3.0</td> <td></td>	Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
v/s Ratio Prot c0.10 0.25 0.32 0.03 v/s Ratio Perm c0.36 0.31 c0.03 v/c Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C Approach Delay (s) 15.0 27.8 25.4 25.4 Approach LOS B C C C Intersection Summary 24.0 HCM 2000 Level of Service HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service	Lane Grp Cap (vph)	283	1762	1837	600		375	
v/s Ratio Perm c0.36 0.31 c0.03 v/c Ratio 0.77 0.42 0.81 0.78 0.12 0.13 Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C Approach Delay (s) 15.0 27.8 25.4 Approach LOS B C C Intersection Summary 24.0 HCM 2000 Level of Service HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15		c0.10	0.25	0.32		0.03		
Uniform Delay, d1 20.3 9.7 24.0 23.6 24.8 24.8 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C A Approach Delay (s) 15.0 27.8 25.4 Approach LOS B C C Intersection Summary B C C C Intersection Summary HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Level of Service Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15 15 15 15 15 15	v/s Ratio Perm	c0.36						
Progression Factor 1.00 <th1.00< th=""> 1.00 1.00<td>v/c Ratio</td><td>0.77</td><td>0.42</td><td>0.81</td><td>0.78</td><td>0.12</td><td>0.13</td><td></td></th1.00<>	v/c Ratio	0.77	0.42	0.81	0.78	0.12	0.13	
Incremental Delay, d2 12.2 0.2 2.8 6.5 0.3 0.7 Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C Approach Delay (s) 15.0 27.8 25.4 Approach LOS B C C Intersection Summary HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15	Uniform Delay, d1		9.7	24.0	23.6	24.8	24.8	
Delay (s) 32.5 9.9 26.8 30.1 25.1 25.5 Level of Service C A C C C C Approach Delay (s) 15.0 27.8 25.4 Approach LOS B C C Intersection Summary HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15			1.00	1.00	1.00	1.00	1.00	
Level of ServiceCACCCCApproach Delay (s)15.027.825.4Approach LOSBCCIntersection SummaryHCM 2000 Control Delay24.0HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.58Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15	Incremental Delay, d2	12.2	0.2	2.8	6.5	0.3	0.7	
Approach Delay (s)15.027.825.4Approach LOSBCCIntersection SummaryHCM 2000 Control Delay24.0HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.58CActuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)1515	Delay (s)	32.5	9.9	26.8	30.1	25.1	25.5	
Approach LOSBCCIntersection SummaryHCM 2000 Control Delay24.0HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.58Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15	Level of Service	С			С		С	
Intersection Summary HCM 2000 Control Delay 24.0 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.58 Actuated Cycle Length (s) 90.0 Sum of lost time (s) Intersection Capacity Utilization 67.3% ICU Level of Service Analysis Period (min) 15	Approach Delay (s)		15.0	27.8		25.4		
HCM 2000 Control Delay24.0HCM 2000 Level of ServiceHCM 2000 Volume to Capacity ratio0.58Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15	Approach LOS		В	С		С		
HCM 2000 Volume to Capacity ratio0.58Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15								
Actuated Cycle Length (s)90.0Sum of lost time (s)Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15	,				H	CM 2000	Level of Se	rvice
Intersection Capacity Utilization67.3%ICU Level of ServiceAnalysis Period (min)15		acity ratio						
Analysis Period (min) 15								
		ation			IC	U Level	of Service	
c Critical Lane Group				15				
	c Critical Lane Group							

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	<u> </u>	- ††	1	- ሽ	- ††	1	<u>۲</u>	- ††	1
Traffic Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Future Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3252	1651	1354	1357	3138	1286	1541	3257	1515	1513	3196	1354
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3252	1651	1354	1357	3138	1286	1541	3257	1515	1513	3196	1354
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			127			127			326			125
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	9%	13%	26%	9%	19%	11%	5%	1%	13%	7%	13%
Adj. Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Shared Lane Traffic (%)												
Lane Group Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel			• •									
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			CI+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel		0.0			0.0			0.0			0.0	
Detector 2 Extend (s)	D 1	0.0	-	.	0.0	-	D (0.0	-	D (0.0	-
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	-	1	6	~	5	2	
Permitted Phases		,	4	0	0	8	4	0	6	-	-	2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase	= ^		E ^			= ^	E ^	= ^			= ^	= ^
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2028 - AM Peak Hour Total 12:10 pm 09-22-2021

10-14-2021	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	19.4	33.2	33.2	14.2	28.0	28.0	19.0	53.6	53.6	24.0	58.6	58.6
Total Split (%)	15.5%	26.6%	26.6%	11.4%	22.4%	22.4%	15.2%	42.9%	42.9%	19.2%	46.9%	46.9%
Maximum Green (s)	13.8	27.5	27.5	8.6	22.3	22.3	13.0	47.7	47.7	18.0	52.7	52.7
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None		C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	13.8	27.9	27.9	7.7	17.0	17.0	15.8	50.3	50.3	20.6	55.2	55.2
Actuated g/C Ratio	0.11	0.22	0.22	0.06	0.14	0.14	0.13	0.40	0.40	0.16	0.44	0.44
v/c Ratio	0.90	0.31	0.26	0.41	0.70	0.40	0.75	0.49	0.86	0.81	0.86	0.15
Control Delay	82.9	44.3	5.7	70.9	60.1	10.5	76.5	29.9	30.2	74.9	39.3	2.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	82.9	44.3	5.7	70.9	60.1	10.5	76.5	29.9	30.2	74.9	39.3	2.8
LOS	F	D	А	E	E	В	E	С	С	E	D	A
Approach Delay		60.0			48.1			34.7			41.6	
Approach LOS		E			D			С			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 125												
Actuated Cycle Length: 12												
Offset: 0 (0%), Referenced	to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.90												
Intersection Signal Delay:						n LOS: E						
Intersection Capacity Utiliz	ation 81.5	%		10	CU Level	of Servic	e D					
Analysis Period (min) 15												
Splits and Phases: 3: R	ussell Rd/S	t Laurent	Blvd & S	myth Rd	/Lancaste	er Rd						

opilits and i mases.	J. Russell Ru/St Laurent Div	u a omyth Nu/Lancaster Nu			
∕•ø1	22 (R)		€ø3		
19 s	58.6 s		14.2 s	33.2 s	
★ _{Ø5}	🖉 🛰 Ø6 (R)		<u>ک</u> _{Ø7}	# Ø8	
24 s	53.6 s		19.4 s	28 s	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	↑	1	٦	- † †	1	٦	<u></u>	1	٦.	- † †	1
Traffic Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Future Volume (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3252	1651	1354	1357	3138	1286	1541	3257	1515	1513	3196	1354
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3252	1651	1354	1357	3138	1286	1541	3257	1515	1513	3196	1354
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	323	113	104	34	297	115	146	638	690	202	1206	103
RTOR Reduction (vph)	0	0	81	0	0	97	0	0	200	0	0	59
Lane Group Flow (vph)	323	113	23	34	297	18	146	638	490	202	1206	44
Heavy Vehicles (%)	2%	9%	13%	26%	9%	19%	11%	5%	1%	13%	7%	13%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	13.8	27.9	27.9	5.1	19.2	19.2	15.8	48.2	48.2	20.6	53.0	53.0
Effective Green, g (s)	13.8	27.9	27.9	5.1	19.2	19.2	15.8	48.2	48.2	20.6	53.0	53.0
Actuated g/C Ratio	0.11	0.22	0.22	0.04	0.15	0.15	0.13	0.39	0.39	0.16	0.42	0.42
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	359	368	302	55	481	197	194	1255	584	249	1355	574
v/s Ratio Prot	c0.10	0.07		0.03	c0.09		0.09	0.20		c0.13	c0.38	
v/s Ratio Perm			0.02			0.01			0.32			0.03
v/c Ratio	0.90	0.31	0.08	0.62	0.62	0.09	0.75	0.51	0.84	0.81	0.89	0.08
Uniform Delay, d1	54.9	40.5	38.4	59.0	49.5	45.4	52.7	29.3	34.9	50.3	33.3	21.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	24.1	0.5	0.1	18.9	2.4	0.2	15.2	1.5	13.5	17.9	9.1	0.3
Delay (s)	79.1	41.0	38.5	77.9	51.8	45.6	67.9	30.8	48.4	68.2	42.4	21.7
Level of Service	E	D	D	E	D	D	E	С	D	E	D	С
Approach Delay (s)		63.3			52.2			42.7			44.4	
Approach LOS		E			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			47.2	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.86									
Actuated Cycle Length (s)			125.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		81.5%	IC	CU Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	11	111	1	ኘካ	1
Traffic Volume (vph)	218	735	1489	676	97	178
Future Volume (vph)	218	735	1489	676	97	178
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			20.0	1	130.0
Taper Length (m)	7.5			1	7.5	1
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	1.00	0.95	0.91	0.850	0.97	0.850
	0.050			0.000	0.050	0.850
Fit Protected	0.950	0074	4000	4404	0.950	4000
Satd. Flow (prot)	1527	2974	4636	1404	2910	1308
Flt Permitted	0.095				0.950	
Satd. Flow (perm)	153	2974	4636	1404	2910	1308
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				346		178
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	12%	15%	6%	9%	14%	17%
Adj. Flow (vph)	218	735	1489	676	97	178
Shared Lane Traffic (%)	210	, 00	1100	010	51	
Lane Group Flow (vph)	218	735	1489	676	97	178
Enter Blocked Intersection	No	No	No	No	No	No
	Left	Left	Left		Left	
Lane Alignment	Leit			Right		Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane		4	1	1		
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	<u>. </u>	<u> </u>	<u>_</u> /	/	<u>_</u> ^	^
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
()	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Detector Phase	7	4	8	8	6	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
	5.0	0.0	0.0	0.0	0.0	0.0

2028 - AM Peak Hour Total 12:10 pm 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	18.0	61.0	43.0	43.0	29.0	29.0
Total Split (%)	20.0%	67.8%	47.8%	47.8%	32.2%	32.2%
Maximum Green (s)	11.8	54.8	36.8	36.8	23.3	23.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	54.0	54.0	36.0	36.0	24.1	24.1
Actuated g/C Ratio	0.60	0.60	0.40	0.40	0.27	0.27
v/c Ratio	0.80	0.41	0.80	0.88	0.12	0.37
Control Delay	42.0	10.3	27.9	26.7	26.1	6.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.0	10.3	27.9	26.7	26.1	6.8
LOS	D	В	С	С	С	А
Approach Delay		17.6	27.5		13.6	
Approach LOS		В	С		В	
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 9	0					
Offset: 0 (0%), Reference		2: and 6:	SBL, Star	t of Gree	n	
Natural Cycle: 80						
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.88						
Intersection Signal Delay:	23.6			l	ntersectio	on LOS: C
Intersection Capacity Utili		%				of Servic
Analysis Period (min) 15						
		<u>.</u> .				

Splits and Phases: 14: Walkley Rd & Lancaster R

	61s		
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🖉 🖉 Ø6 (R)	Ø7	Ø8	
29 s	18 s	43 s	

10-14-2021	
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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	۲	††	†††	1	ሻሻ	1	
Traffic Volume (vph)	218	735	1489	676	97	178	
Future Volume (vph)	218	735	1489	676	97	178	
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1527	2974	4636	1404	2910	1308	
Flt Permitted	0.09	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	152	2974	4636	1404	2910	1308	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	218	735	1489	676	97	178	
RTOR Reduction (vph)	0	0	0	208	0	130	
Lane Group Flow (vph)	218	735	1489	468	97	48	
Heavy Vehicles (%)	12%	15%	6%	9%	14%	17%	
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm	
Protected Phases	7	4	8		6		
Permitted Phases	4			8		6	
Actuated Green, G (s)	54.0	54.0	36.0	36.0	24.1	24.1	
Effective Green, g (s)	54.0	54.0	36.0	36.0	24.1	24.1	
Actuated g/C Ratio	0.60	0.60	0.40	0.40	0.27	0.27	
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	271	1784	1854	561	779	350	
v/s Ratio Prot	c0.11	0.25	0.32		0.03		
v/s Ratio Perm	c0.38			0.33		c0.04	
v/c Ratio	0.80	0.41	0.80	0.83	0.12	0.14	
Uniform Delay, d1	21.3	9.6	23.9	24.3	25.0	25.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	15.8	0.2	2.6	10.4	0.3	0.8	
Delay (s)	37.1	9.7	26.5	34.7	25.3	25.8	
Level of Service	D	Α	С	С	С	С	
Approach Delay (s)		16.0	29.0		25.7		
Approach LOS		В	С		С		
Intersection Summary							
HCM 2000 Control Delay			25.1	H	CM 2000	Level of Sei	rvice
HCM 2000 Volume to Capa	icity ratio		0.61				
Actuated Cycle Length (s)			90.0			t time (s)	
Intersection Capacity Utiliza	ation		67.3%	IC	U Level	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	- †	1	- ከ	- † †	1	<u>۲</u>	- † †	1	<u>۲</u>	- 44	1
Traffic Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Future Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			218			228			383			226
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Adj. Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Shared Lane Traffic (%)												
Lane Group Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0	_		0.0	_	_	0.0	_	_	0.0	_
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	-
Permitted Phases	_	,	4	_	_	8			6	_		2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2028 - PM Peak Hour Background 10:46 am 09-22-2021

Synchro 11 Report Page 1

Lanes, Volumes, Timings <u>3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd</u>

10-14-2021	10	-14-202	!1
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	28.0	38.0	38.0	18.0	28.0	28.0	22.0	54.0	54.0	15.0	47.0	47.0
Total Split (%)	22.4%	30.4%	30.4%	14.4%	22.4%	22.4%	17.6%	43.2%	43.2%	12.0%	37.6%	37.6%
Maximum Green (s)	22.4	32.3	32.3	12.4	22.3	22.3	16.0	48.1	48.1	9.0	41.1	41.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	22.2	25.9	25.9	11.4	15.2	15.2	15.1	51.8	51.8	12.7	49.3	49.3
Actuated g/C Ratio	0.18	0.21	0.21	0.09	0.12	0.12	0.12	0.41	0.41	0.10	0.39	0.39
v/c Ratio	0.93	0.80	0.50	0.71	0.28	0.64	0.73	0.85	0.45	0.67	0.60	0.14
Control Delay	74.0	62.9	10.5	79.7	49.9	16.0	73.1	40.9	4.3	75.4	34.2	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.0	62.9	10.5	79.7	49.9	16.0	73.1	40.9	4.3	75.4	34.2	0.4
LOS	E	E	В	E	D	В	E	D	А	E	С	A
Approach Delay		56.8			38.8			35.5			35.1	
Approach LOS		E			D			D			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 125												
Actuated Cycle Length: 12	25											
Offset: 0 (0%), Reference	d to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.93												
Intersection Signal Delay:	41.2			l I	ntersectio	on LOS: E)					
Intersection Capacity Utiliz	zation 82.4%	6		10	CU Level	of Servic	еE					
Analysis Period (min) 15												

Splits and Phases: 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

₩ø1	02 (R)	K Ø3	→ Ø4	
22 s	47 s	18 s	38 s	
◆ Ø5	🔪 🥶 (R)	y ₀₇	← Ø8	
15 s	54 s	28 s	28 s	

10-14-2022	1
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	٦	- † †	1	٦	- † †	1	۳.	- ††	1
Traffic Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Future Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3285	1782	1485	1660	3386	1515	1676	3353	1515	1555	3288	1515
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
RTOR Reduction (vph)	0	0	173	0	0	200	0	0	225	0	0	61
Lane Group Flow (vph)	540	294	66	107	114	47	147	1177	158	105	781	39
Heavy Vehicles (%)	1%	1%	3%	3%	1%	1%	2%	2%	1%	10%	4%	1%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6	-	5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	22.2	26.0	26.0	11.4	15.2	15.2	15.1	51.7	51.7	12.7	49.3	49.3
Effective Green, g (s)	22.2	26.0	26.0	11.4	15.2	15.2	15.1	51.7	51.7	12.7	49.3	49.3
Actuated g/C Ratio	0.18	0.21	0.21	0.09	0.12	0.12	0.12	0.41	0.41	0.10	0.39	0.39
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	583	370	308	151	411	184	202	1386	626	157	1296	597
v/s Ratio Prot	c0.16	c0.16	0.04	0.06	0.03	0.00	c0.09	c0.35	0.40	0.07	0.24	0.00
v/s Ratio Perm	0.00	0.70	0.04	0.74	0.00	0.03	0.70	0.05	0.10	0.07	0.00	0.03
v/c Ratio	0.93	0.79	0.22	0.71	0.28	0.25	0.73	0.85	0.25	0.67	0.60	0.07
Uniform Delay, d1	50.6	47.0	41.0	55.2	49.9	49.8	53.0	33.1	24.0	54.1	30.1	23.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	20.8	11.2	0.4	14.1	0.4	0.7	12.3	6.7	1.0	10.3	2.1	0.2
Delay (s)	71.4	58.1 E	41.4	69.3	50.3 D	50.5 D	65.3 E	39.8 D	25.0 C	64.4 E	32.2 C	23.7
Level of Service	E	⊏ 61.1	D	E	54.7	U	E	38.7	U	E	34.7	С
Approach Delay (s)		61.1 E			54.7 D			30.7 D			34.7 C	
Approach LOS		E			U			U			U	
Intersection Summary												
HCM 2000 Control Delay			45.2	Н	CM 2000	Level of	Service		D			_
HCM 2000 Volume to Capa	acity ratio		0.88									
Actuated Cycle Length (s)			125.0			t time (s)			23.2			_
Intersection Capacity Utiliza	ation		82.4%	IC	U Level	of Service	e		Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	1	^	1	ኘካ	1
Traffic Volume (vph)	132	1845	1001	172	508	215
Future Volume (vph)	132	1845	1001	172	508	215
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0			25.0	75.0	150.0
Storage Lanes	1			1	1	1
Taper Length (m)	7.5			•	7.5	
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt		0.00	0.01	0.850	0.01	0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1676	3257	4508	1404	3252	1485
Flt Permitted	0.202				0.950	. 100
Satd. Flow (perm)	356	3257	4508	1404	3252	1485
Right Turn on Red	000	0201	1000	Yes	0202	Yes
Satd. Flow (RTOR)				140		215
Link Speed (k/h)		50	50	140	50	215
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	17.0	20.3	1.00	13.5	1.00
			9%	9%		3%
Heavy Vehicles (%)	2%	5%			2%	
Adj. Flow (vph)	132	1845	1001	172	508	215
Shared Lane Traffic (%)	400	4045	1004	470	500	045
Lane Group Flow (vph)	132	1845	1001	172	508	215
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	9.4	9.4	0.0	0.0	0.0
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		CI+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	ριπ+ρι 7	4	8		6	I CIIII
Protected Phases Permitted Phases	4	4	0	8	U	6
		Λ	0		c	
Detector Phase	7	4	8	8	6	6
Switch Phase	F 0	F 0	F 0	F 0	F 0	F 0
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0

2028 - PM Peak Hour Background 10:46 am 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	15.4	62.0	46.6	46.6	28.0	28.0
Total Split (%)	17.1%	68.9%	51.8%	51.8%	31.1%	31.1%
Maximum Green (s)	9.2	55.8	40.4	40.4	22.3	22.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	55.5	55.5	41.1	41.1	22.6	22.6
Actuated g/C Ratio	0.62	0.62	0.46	0.46	0.25	0.25
v/c Ratio	0.39	0.92	0.49	0.24	0.62	0.40
Control Delay	10.6	24.4	18.1	5.0	33.9	6.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.6	24.4	18.1	5.0	33.9	6.6
LOS	В	С	В	А	С	А
Approach Delay		23.5	16.2		25.8	
Approach LOS		С	В		С	
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 9						
Offset: 0 (0%), Reference	d to phase 2	2: and 6:8	SBL, Star	t of Gree	n	
Natural Cycle: 90						
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.92						
Intersection Signal Delay:	: 21.7			l	ntersectio	on LOS: C
Intersection Capacity Utili	zation 79.09	6		l	CU Level	of Service
Analysis Period (min) 15						

Splits and Phases:	14: Walkley Rd & Lancaster Rd
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	62 s		
₩Ø6 (R)	▶ _{Ø7}	Ø8	
28 s	15.4 s	46.6 s	

10-14-202	1
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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	۲	† †	^	1	ኘካ	1		
Traffic Volume (vph)	132	1845	1001	172	508	215		
Future Volume (vph)	132	1845	1001	172	508	215		
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800		
Total Lost time (s)	6.2	6.2	6.2	6.2	5.7	5.7		
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00		
Frt	1.00	1.00	1.00	0.85	1.00	0.85		
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (prot)	1676	3257	4508	1404	3252	1485		
Flt Permitted	0.20	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (perm)	356	3257	4508	1404	3252	1485		
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00		
Adj. Flow (vph)	132	1845	1001	172	508	215		
RTOR Reduction (vph)	0	0	0	76	0	161		
Lane Group Flow (vph)	132	1845	1001	96	508	54		
Heavy Vehicles (%)	2%	5%	9%	9%	2%	3%		
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm		
Protected Phases	7	4	8		6			
Permitted Phases	4			8		6		
Actuated Green, G (s)	55.5	55.5	41.2	41.2	22.6	22.6		
Effective Green, g (s)	55.5	55.5	41.2	41.2	22.6	22.6		
Actuated g/C Ratio	0.62	0.62	0.46	0.46	0.25	0.25		
Clearance Time (s)	6.2	6.2	6.2	6.2	5.7	5.7		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	338	2008	2063	642	816	372		
v/s Ratio Prot	0.04	c0.57	0.22		c0.16			
v/s Ratio Perm	0.21			0.07		0.04		
v/c Ratio	0.39	0.92	0.49	0.15	0.62	0.15		
Uniform Delay, d1	8.4	15.3	17.0	14.2	29.9	26.2		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	0.7	7.2	0.2	0.1	3.6	0.8		
Delay (s)	9.1	22.5	17.2	14.3	33.5	27.0		
Level of Service	А	С	В	В	С	С		
Approach Delay (s)		21.6	16.8		31.6			
Approach LOS		С	В		С			
Intersection Summary								
HCM 2000 Control Delay			22.0	Н	CM 2000	Level of Se	rvice	
HCM 2000 Volume to Capa	city ratio		0.90					
Actuated Cycle Length (s)			90.0		um of los			
Intersection Capacity Utiliza	ition		79.0%	IC	U Level	of Service		
Analysis Period (min)			15					
c Critical Lane Group								

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ካካ	↑	1	- ከ	- † †	1	<u>۲</u>	- † †	1	<u>۲</u>	- 44	1
Traffic Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Future Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0		85.0	55.0		30.0	130.0		115.0	185.0		35.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3285	1682	1485	1555	3167	1404	1569	3353	1515	1555	3320	1430
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3285	1682	1485	1555	3167	1404	1569	3353	1515	1555	3320	1430
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			223			228			383			226
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		211.6			125.1			192.6			164.4	
Travel Time (s)		15.2			9.0			13.9			11.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	7%	3%	10%	8%	9%	9%	2%	1%	10%	3%	7%
Adj. Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Shared Lane Traffic (%)												
Lane Group Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		7.2			7.2			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4			9.4			9.4			9.4	
Detector 2 Size(m)		0.6			0.6			0.6			0.6	
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	_	0.0	_	_	0.0	_	_	0.0	_	_	0.0	_
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Detector Phase	7	4	4	3	8	8	1	6	6	5	2	2
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

2028 - PM Peak Hour Total 12:11 pm 09-22-2021

Lanes, Volumes, Timings 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

10-14-2021	10	-14-202	!1
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Minimum Split (s)	10.6	32.4	32.4	10.6	28.0	28.0	11.0	27.9	27.9	11.0	27.9	27.9
Total Split (s)	28.0	37.3	37.3	18.7	28.0	28.0	23.0	54.0	54.0	15.0	46.0	46.0
Total Split (%)	22.4%	29.8%	29.8%	15.0%	22.4%	22.4%	18.4%	43.2%	43.2%	12.0%	36.8%	36.8%
Maximum Green (s)	22.4	31.6	31.6	13.1	22.3	22.3	17.0	48.1	48.1	9.0	40.1	40.1
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	4.2	4.2	4.2	4.2	4.2	4.2
All-Red Time (s)	1.9	2.0	2.0	1.9	2.0	2.0	1.8	1.7	1.7	1.8	1.7	1.7
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0
Flash Dont Walk (s)		15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	0
Act Effct Green (s)	22.2	26.4	26.4	12.0	16.2	16.2	15.6	51.5	51.5	11.9	47.9	47.9
Actuated g/C Ratio	0.18	0.21	0.21	0.10	0.13	0.13	0.12	0.41	0.41	0.10	0.38	0.38
v/c Ratio	0.93	0.83	0.49	0.72	0.28	0.65	0.75	0.85	0.45	0.71	0.61	0.15
Control Delay	74.0	66.2	9.8	81.2	49.0	16.3	76.2	41.4	4.3	81.2	35.4	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.0	66.2	9.8	81.2	49.0	16.3	76.2	41.4	4.3	81.2	35.4	0.5
LOS	E	E	Α	F	D	В	E	D	А	F	D	A
Approach Delay		57.6			39.1			36.1			36.8	
Approach LOS		Е			D			D			D	
Intersection Summary												
Area Type:	Other											
Cycle Length: 125												
Actuated Cycle Length: 12	25											
Offset: 0 (0%), Reference	d to phase 2	2:NWT ai	nd 6:SET	, Start of	Green							
Natural Cycle: 105												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.93												
Intersection Signal Delay:	42.0			li li	ntersectio	on LOS: E)					
Intersection Capacity Utiliz	zation 82.4%	%		10	CU Level	of Servic	еE					
Analysis Period (min) 15												

Splits and Phases: 3: Russell Rd/St Laurent Blvd & Smyth Rd/Lancaster Rd

∕•ø1	🗸 🔪 🖉 2 (R)	€ Ø3 →Ø4	
23 s	46 s	18.7 s 37.3 s	
★ _{Ø5}	≫ Ø ⊕ (R)	≯ _{Ø7} ←	_ Ø8
15 s	54 s	28 s 28 s	

10-14-202	1
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations	ሻሻ	↑	1	٦	- † †	1	٦	- † †	1	۳.	- ††	1
Traffic Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Future Volume (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Total Lost time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3285	1682	1485	1555	3167	1404	1569	3353	1515	1555	3320	1430
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3285	1682	1485	1555	3167	1404	1569	3353	1515	1555	3320	1430
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	540	294	239	107	114	247	147	1177	383	105	781	100
RTOR Reduction (vph)	0	0	176	0	0	198	0	0	225	0	0	62
Lane Group Flow (vph)	540	294	63	107	114	49	147	1177	158	105	781	38
Heavy Vehicles (%)	1%	7%	3%	10%	8%	9%	9%	2%	1%	10%	3%	7%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		1	6		5	2	
Permitted Phases			4			8			6			2
Actuated Green, G (s)	22.2	26.4	26.4	12.0	16.2	16.2	15.6	51.5	51.5	11.9	47.8	47.8
Effective Green, g (s)	22.2	26.4	26.4	12.0	16.2	16.2	15.6	51.5	51.5	11.9	47.8	47.8
Actuated g/C Ratio	0.18	0.21	0.21	0.10	0.13	0.13	0.12	0.41	0.41	0.10	0.38	0.38
Clearance Time (s)	5.6	5.7	5.7	5.6	5.7	5.7	6.0	5.9	5.9	6.0	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	583	355	313	149	410	181	195	1381	624	148	1269	546
v/s Ratio Prot	c0.16	c0.17		0.07	0.04		c0.09	c0.35		0.07	0.24	
v/s Ratio Perm			0.04			0.03			0.10			0.03
v/c Ratio	0.93	0.83	0.20	0.72	0.28	0.27	0.75	0.85	0.25	0.71	0.62	0.07
Uniform Delay, d1	50.6	47.1	40.6	54.9	49.1	49.1	52.8	33.3	24.1	54.9	31.2	24.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	20.8	14.6	0.3	15.2	0.4	0.8	15.2	6.8	1.0	14.4	2.2	0.2
Delay (s)	71.4	61.8	40.9	70.1	49.5	49.9	68.0	40.1	25.1	69.3	33.4	24.7
Level of Service	E	E	D	E	D	D	E	D	С	E	С	С
Approach Delay (s)		62.0			54.4			39.2			36.4	
Approach LOS		E			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			46.0	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.90									
Actuated Cycle Length (s)			125.0			t time (s)			23.2			
Intersection Capacity Utiliza	ation		82.4%	IC	CU Level	of Service	е		E			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u> </u>	^	^	7	<u>ነ</u> ካ	7
Traffic Volume (vph)	132	1845	1001	172	508	215
Future Volume (vph)	132	1845	1001	172	508	215
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	120.0	1000	1000	25.0	75.0	150.0
Storage Lanes	120.0			25.0	1	150.0
	7.5			1	7.5	
Taper Length (m)		0.05	0.04	1.00		1.00
Lane Util. Factor	1.00	0.95	0.91	1.00	0.97	1.00
Frt	0.050			0.850	0.0=0	0.850
Flt Protected	0.950	005-	4===	1000	0.950	1001
Satd. Flow (prot)	1583	3257	4550	1330	3043	1391
Flt Permitted	0.235				0.950	
Satd. Flow (perm)	392	3257	4550	1330	3043	1391
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				124		215
Link Speed (k/h)		50	50		50	
Link Distance (m)		246.9	393.2		187.8	
Travel Time (s)		17.8	28.3		13.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	8%	5%	8%	15%	9%	10%
Adj. Flow (vph)	132	1845	1001	172	508	215
Shared Lane Traffic (%)	102	10-10	1001	172	500	210
Lane Group Flow (vph)	132	1845	1001	172	508	215
Enter Blocked Intersection	No			No		No
		No	No		No	
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		7.2	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.07	1.07	1.07	1.07	1.07	1.07
Turning Speed (k/h)	25			15	25	15
Number of Detectors	1	2	2	1	1	1
Detector Template	Left	Thru	Thru	Right	Left	Right
Leading Detector (m)	2.0	10.0	10.0	2.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	0.6	2.0	2.0	2.0
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4	9.4			
Detector 2 Size(m)		0.6	0.6			
Detector 2 Type		CI+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	pm+pt	NA	NA	Perm	Prot	Perm
Protected Phases	7	4	8		6	
Permitted Phases	4			8		6
Detector Phase	7	4	8	8	6	6
Switch Phase	,	T	U	U	V	V
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
	5.0	0.0	0.0	5.0	5.0	5.0

2028 - PM Peak Hour Total 12:11 pm 09-22-2021

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Minimum Split (s)	11.2	24.2	37.2	37.2	27.7	27.7
Total Split (s)	22.0	102.0	80.0	80.0	28.0	28.0
Total Split (%)	16.9%	78.5%	61.5%	61.5%	21.5%	21.5%
Maximum Green (s)	15.8	95.8	73.8	73.8	22.3	22.3
Yellow Time (s)	4.6	4.6	4.6	4.6	3.3	3.3
All-Red Time (s)	1.6	1.6	1.6	1.6	2.4	2.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.2	6.2	6.2	6.2	5.7	5.7
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max
Walk Time (s)			20.0	20.0	7.0	7.0
Flash Dont Walk (s)			7.0	7.0	15.0	15.0
Pedestrian Calls (#/hr)			0	0	0	0
Act Effct Green (s)	95.8	95.8	81.0	81.0	22.3	22.3
Actuated g/C Ratio	0.74	0.74	0.62	0.62	0.17	0.17
v/c Ratio	0.36	0.77	0.35	0.20	0.98	0.52
Control Delay	7.7	13.2	12.4	4.0	87.2	10.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	7.7	13.2	12.4	4.0	87.2	10.8
LOS	А	В	В	А	F	В
Approach Delay		12.8	11.2		64.5	
Approach LOS		В	В		E	
Intersection Summary						
Area Type:	Other					
Cycle Length: 130						
Actuated Cycle Length: 1	30					
Offset: 0 (0%), Reference	ed to phase 2	2: and 6:	SBL, Star	t of Gree	n	
Natural Cycle: 90						
Control Type: Actuated-C	Coordinated					
Maximum v/c Ratio: 0.98						
Intersection Signal Delay	: 22.0			l	ntersectio	on LOS: C
Intersection Capacity Util		%		l	CU Level	of Servic
Analysis Period (min) 15						

Splits and Phases: 14: Walkley Rd & Lancaster Rd

	 Ø4		
	102 s		
1		<u>↓</u>	
Ø6 (R)	Ø7	Ø8	
28 s	22 s	80 s	

10-14-2021	
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Movement EBL EBT WBT WBR SBL SBR Lane Configurations ***
Lane Configurations i
Traffic Volume (vph)13218451001172508215Future Volume (vph)13218451001172508215Ideal Flow (vphpl)180018001800180018001800Total Lost time (s)6.26.26.26.25.75.7Lane Util. Factor1.000.950.911.000.971.00Frt1.001.001.000.851.000.85Flt Protected0.951.001.001.000.951.00Satd. Flow (port)158332574550133030431391Flt Permitted0.231.001.001.000.951.00Satd. Flow (perm)39132574550133030431391Peak-hour factor, PHF1.001.001.001.001.00Adj. Flow (vph)13218451001172508215RTOR Reduction (vph)000470178Lane Group Flow (vph)1321845100112550837Heavy Vehicles (%)8%5%8%15%9%10%Turn Typepm+ptNANAPermProtPermProtected Phases74866Permitted Phases4866Actuated Green, G (s)95.895.881.081.022.322.3Effective Green, g (s) </td
Future Volume (vph)13218451001172508215Ideal Flow (vphpl)1800180018001800180018001800Total Lost time (s)6.26.26.26.25.75.7Lane Util. Factor1.000.950.911.000.971.00Frt1.001.001.000.851.000.85Flt Protected0.951.001.001.000.951.00Satd. Flow (prot)158332574550133030431391Flt Permitted0.231.001.001.000.951.00Satd. Flow (perm)39132574550133030431391Peak-hour factor, PHF1.001.001.001.001.001.00Adj. Flow (vph)13218451001172508215RTOR Reduction (vph)000470178Lane Group Flow (vph)1321845100112550837Heavy Vehicles (%)8%5%8%15%9%10%Turn Typepm+ptNANAPermProtPermProtected Phases74866Permitted Phases4866Actuated Green, G (s)95.895.881.081.022.322.3Effective Green, g (s)95.895.881.081.022.322.3
Ideal Flow (vphpl)180018001800180018001800Total Lost time (s)6.26.26.26.25.75.7Lane Util. Factor1.000.950.911.000.971.00Frt1.001.001.000.851.000.85Flt Protected0.951.001.001.000.951.00Satd. Flow (prot)158332574550133030431391Flt Permitted0.231.001.001.000.951.00Satd. Flow (perm)39132574550133030431391Peak-hour factor, PHF1.001.001.001.001.001.00Adj. Flow (vph)13218451001172508215RTOR Reduction (vph)000470178Lane Group Flow (vph)1321845100112550837Heavy Vehicles (%)8%5%8%15%9%10%Turn Typepm+ptNANAPermProtPermProtected Phases74866Permitted Phases4866Actuated Green, G (s)95.895.881.081.022.322.3Effective Green, g (s)95.895.881.081.022.322.3Actuated g/C Ratio0.740.740.620.620.170.17 </td
Total Lost time (s) 6.2 6.2 6.2 6.2 6.2 5.7 5.7 Lane Util. Factor 1.00 0.95 0.91 1.00 0.97 1.00 Frt 1.00 1.00 1.00 0.85 1.00 0.85 Filt Protected 0.95 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1583 3257 4550 1330 3043 1391 Fit Permitted 0.23 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Typepm+ptNANAPermProtPermProtected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62
Lane Util. Factor1.000.950.911.000.971.00Frt1.001.001.000.851.000.85Flt Protected0.951.001.001.000.951.00Satd. Flow (prot)158332574550133030431391Flt Permitted0.231.001.001.000.951.00Satd. Flow (perm)39132574550133030431391Peak-hour factor, PHF1.001.001.001.001.001.00Adj. Flow (vph)13218451001172508215RTOR Reduction (vph)000470178Lane Group Flow (vph)1321845100112550837Heavy Vehicles (%)8%5%8%15%9%10%Turn Typepm+ptNANAPermProtPermProtected Phases74866Permitted Phases48666Actuated Green, G (s)95.895.881.081.022.322.3Effective Green, g (s)95.895.881.081.022.322.3Actuated g/C Ratio0.740.740.620.620.170.17
Frt 1.00 1.00 1.00 0.85 1.00 0.85 Flt Protected 0.95 1.00 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1583 3257 4550 1330 3043 1391 Flt Permitted 0.23 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 <td< td=""></td<>
Flt Protected 0.95 1.00 1.00 1.00 0.95 1.00 Satd. Flow (prot) 1583 3257 4550 1330 3043 1391 Flt Permitted 0.23 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Perm Protected Phases 7 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0
Satd. Flow (prot) 1583 3257 4550 1330 3043 1391 Flt Permitted 0.23 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Perm Protected Phases 7 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0
Flt Permitted 0.23 1.00 1.00 1.00 0.95 1.00 Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
Satd. Flow (perm) 391 3257 4550 1330 3043 1391 Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17
Peak-hour factor, PHF 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 6 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
Adj. Flow (vph) 132 1845 1001 172 508 215 RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
RTOR Reduction (vph) 0 0 0 47 0 178 Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.62 0.62 0.17 0.17
Lane Group Flow (vph) 132 1845 1001 125 508 37 Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.62 0.62 0.17 0.17
Heavy Vehicles (%) 8% 5% 8% 15% 9% 10% Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.62 0.62 0.17 0.17
Turn Type pm+pt NA NA Perm Prot Perm Protected Phases 7 4 8 6 6 Permitted Phases 4 8 6 6 6 Actuated Phases 4 8 6 6 6 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
Protected Phases 7 4 8 6 Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.62 0.62 0.17 0.17
Permitted Phases 4 8 6 Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.62 0.62 0.17 0.17
Actuated Green, G (s) 95.8 95.8 81.0 81.0 22.3 22.3 Effective Green, g (s) 95.8 95.8 81.0 81.0 22.3 22.3 Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
Effective Green, g (s)95.895.881.081.022.322.3Actuated g/C Ratio0.740.740.620.620.170.17
Actuated g/C Ratio 0.74 0.74 0.62 0.62 0.17 0.17
\mathbf{v}
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0
Lane Grp Cap (vph) 366 2400 2835 828 521 238
v/s Ratio Prot 0.02 c0.57 0.22 c0.17
v/s Ratio Perm 0.24 0.09 0.03
v/c Ratio 0.36 0.77 0.35 0.15 0.98 0.15
Uniform Delay, d1 5.8 10.4 11.8 10.2 53.6 45.8
Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
Incremental Delay, d2 0.6 1.5 0.1 0.1 33.7 1.4
Delay (s) 6.4 11.9 11.9 10.3 87.3 47.2
Level of Service A B B F D
Approach Delay (s) 11.5 11.7 75.4
Approach LOS B B E
Intersection Summary
HCM 2000 Control Delay 23.5 HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio 0.85
Actuated Cycle Length (s) 130.0 Sum of lost time (s)
Intersection Capacity Utilization 79.0% ICU Level of Service
Analysis Period (min) 15
c Critical Lane Group