

2582-2600, 2626 Bank Street Transportation Impact Assessment

Step 1 Screening Report

Step 2 Scoping Report

Step 3 Forecasting Report

Step 4 Strategy Report

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1 Screening

This study has been prepared according to the City of Ottawa’s 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for the TIA Study PM. As shown in the Screening Form, a TIA is required including the Design Review component and the Network Impact Component. This report accompanies a site plan application.

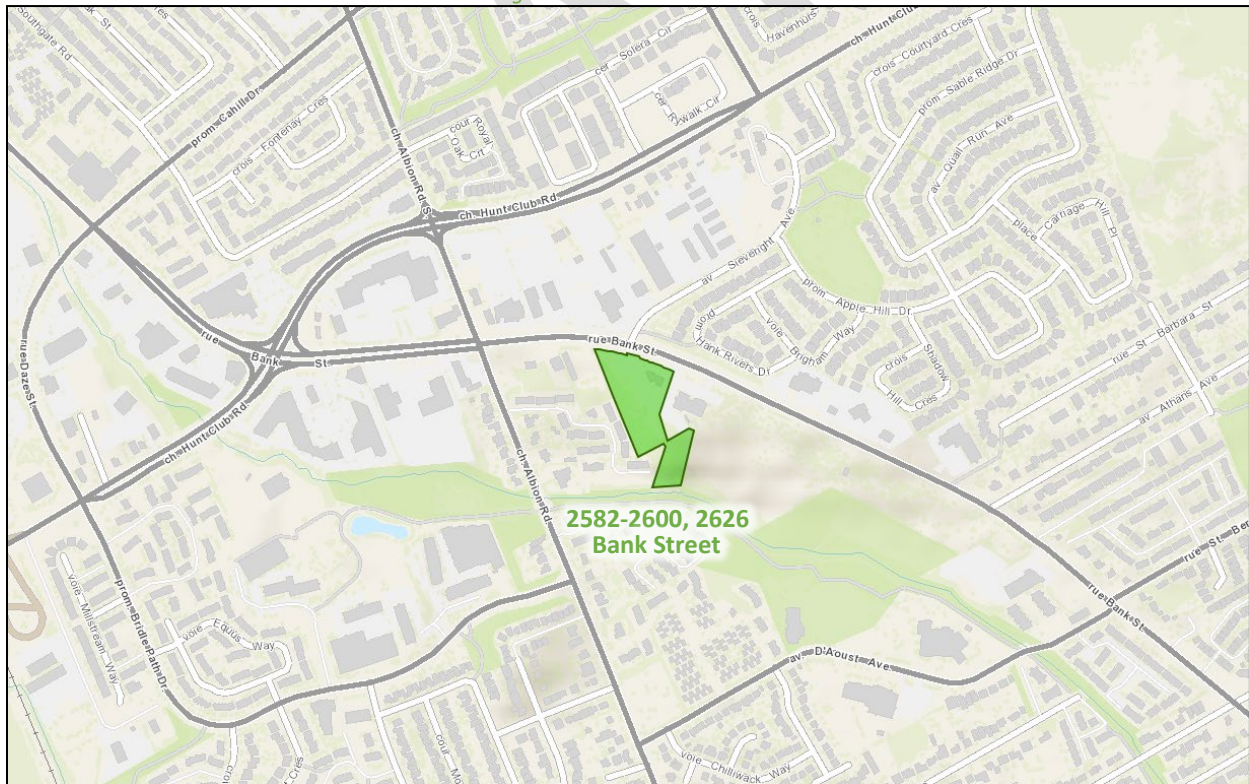
2 Existing and Planned Conditions

2.1 Proposed Development

The subject site currently occupied by a used car dealership/car rental centre, is zoned as Arterial Mainstreet (AM H(30)) and Residential Third Density (R3Y[708]) and intersects the Bank Arterial Mainstreet Design Priority Area. The proposed development includes three new mixed-use buildings on the 2582 and 2600 Bank Street parcels, comprising 4,232.8 m² of commercial space and 7,718.0 m² of office space with the retention and repurposing of the existing car sales/rental building. The construction is to be phased, with the number of phases yet to be determined, where full build-out and occupation is expected by 2025. The existing full-movement site access is to be retained and the development proposes a new right-in/right-out site access onto Bank Street in line with a median which would prevent left-turns to and from the access.

Figure 1 illustrates the Study Area Context. Figure 2 illustrates the proposed concept plan.

Figure 1: Area Context Plan



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: February 8, 2021

2.2 Existing Conditions

2.2.1 Area Road Network

Bank Street: Bank Street is a City of Ottawa arterial road with a divided four-lane urban cross-section including sidewalks on both sides of the road to the west of Albion Road. Between Albion Road and Sieveright Avenue, Bank Street has a five-lane urban cross-section including a two-way left-turn lane, with sidewalks on both sides of the road, where the sidewalk is discontinuous on the south side of the road along the frontage of the Petro-Canada. Between Sieveright Avenue and Councillors Way, the cross-section is: semi-urban for 175 metres, curbed with a sidewalk on the south side and with a paved shoulder on the north side; fully rural for 40 metres with paved shoulders on both sides; semi-urban for 90 metres, curbed with sidewalk on the north side and with a paved shoulder on the south side; semi-urban for 145 metres, curbed on the north side with a paved shoulder on the south side of the road, and a functional paved shoulder on the north side of the road, given nearly half of the frontage is driveways and the shoulder is bounded by curbed sidewalks; fully rural for 50 metres with paved shoulders on both sides; semi-urban for 300 metres curbed with a sidewalk on the north side and with a paved shoulder on the south side. East of Councillors Way within the study area, the cross-section urban with sidewalks on both sides of the road. The posted speed limit is 60 km/h, and City-protected right of way within the study area is 37.5 metres, north of Hunt Club Road, and is 44.5 metres, south of Hunt Club Road. Bank Street is a truck route.

Hunt Club Road: Hunt Club Road is a City of Ottawa arterial road with a divided six-lane urban cross-section including sidewalks and bike lanes on both sides of the road to the west of Bank Street within the study area. East of Bank Street, Hunt Club Road has a divided four-lane cross-section with sidewalks on both sides of the road. Outside lane transit priority lanes are present at the western extent of the study area. The posted speed limit is 60 km/h, and City-protected right of way within the study area is 44.5 metres. Hunt Club Road is a truck route.

Albion Road: Albion Road is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks and curbside bike lanes on both sides of the road south of Bank Street. North of Bank Street, Albion Road's cross-section includes a sidewalk on the west side of the road along its entire length within the study area and a sidewalk on the east side of the road for: 50 metres north of Bank Street; 50 metres south of Hunt Club Road; and 165 metres north of Hunt Club Road. The posted speed limit is 50 km/h, and City-protected right of way within the study area is 24.0 metres to the south of Bank Street, and the measured right of way is 20.0 metres to the north.

Sieveright Avenue: Sieveright Avenue is a City of Ottawa local road with a two-lane urban cross-section including a sidewalk on the east side of the road. The posted speed limit is 50 km/h, and measured right of way is 24.0 metres to the south of Apple Hill Drive, and 23.0 metres to the north.

2.2.2 Existing Intersections

The key existing signalized area intersections with 400 metres of the site and along Bank Street or Hunt Club Road have been summarized below:

Bank Street at Hunt Club Road

The intersection of Bank Street at Hunt Club Road is a signalized intersection. The northbound and southbound approaches of Bank Street each consist of two auxiliary left-turn lanes, two through lanes, and an auxiliary channelized right-turn lane. The eastbound approach consists of two auxiliary left-turn lanes, two through lanes, and a channelized right-turn lane and the westbound approach consists of an auxiliary left-turn lane, two through lanes, and an auxiliary channelized right-turn lane. No turn restrictions were noted.

Albion Road at Hunt Club Road

The intersection of Albion Road at Hunt Club Road is a signalized intersection. The northbound and southbound approaches each consist of an auxiliary left-turn lane and a shared through/channelized right-turn lane. The eastbound and westbound approaches each consist of an auxiliary left-turn lane, two through lanes, and an auxiliary channelized right-turn lane. No turn restrictions were noted.

Bank Street at Towngate Plaza

The intersection of Bank Street at the Towngate Plaza shopping centre access/Petro-Canada access is a signalized intersection. The northbound approach of Bank Street consists of an auxiliary left-turn lane that functions as through/left-turn lane, an auxiliary through lane, two through lanes, and an auxiliary right-turn lane and the southbound approach consists of two through lanes and a right-turn lane. The eastbound approach consists of a shared left/through/channelized right-turn lane and the westbound approach consists of a shared all-movements lane. Southbound left turns are prohibited at this intersection.

Albion Road at Bank Street

The intersection of Albion Road at Bank Street is a signalized intersection. The northbound and southbound approaches of Albion Road each consist of an auxiliary left-turn lane and a shared through/channelized right-turn lane. The eastbound approach consists of an auxiliary left-turn lane, two through lanes, and an auxiliary right-turn lane and the westbound approach consists of an auxiliary left-turn lane, two through lanes, and an auxiliary channelized right-turn lane. No turn restrictions were noted.

Sieveright Avenue at Bank Street

The intersection of Sieveright Avenue at Bank Street is an unsignalized T-intersection, stop-controlled on the minor approach of Sieveright Avenue. The southbound approach of Sieveright Avenue consists of an auxiliary left-turn lane and a right-turn lane. The eastbound approach consists of an auxiliary left-turn lane and two through lanes, and the westbound approach consists of a through lane and a shared through/channelized right-turn lane. No turn restrictions were noted.

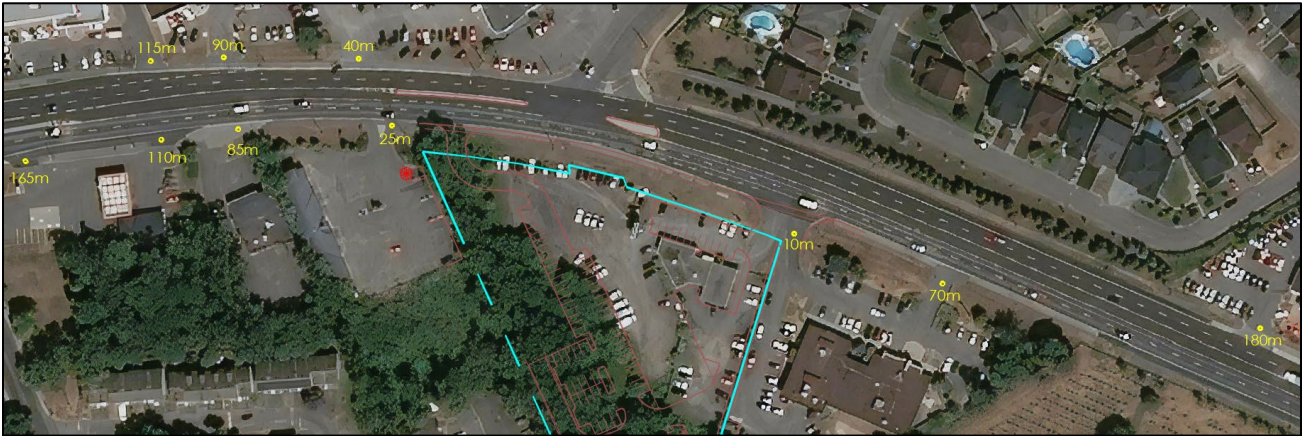
2.2.3 Existing Driveways

Within 200 metres of the proposed site accesses, four driveways exist along the south side of Bank Street and three driveways exist on the north side of Bank Street to the west of the site, and one on the north side of Bank Street and two on the south side of Bank Street exist to the east of the site, with all driveways accessing a number of commercial land uses.

The existing site access permits full movements and is shared with the adjacent parcel's full-movement access, a veterinary hospital, where a 2.5-metre median separates the driveways.

Driveways onto Bank Street within proximity of the site are illustrated in Figure 3.

Figure 3: Exiting Area Driveways

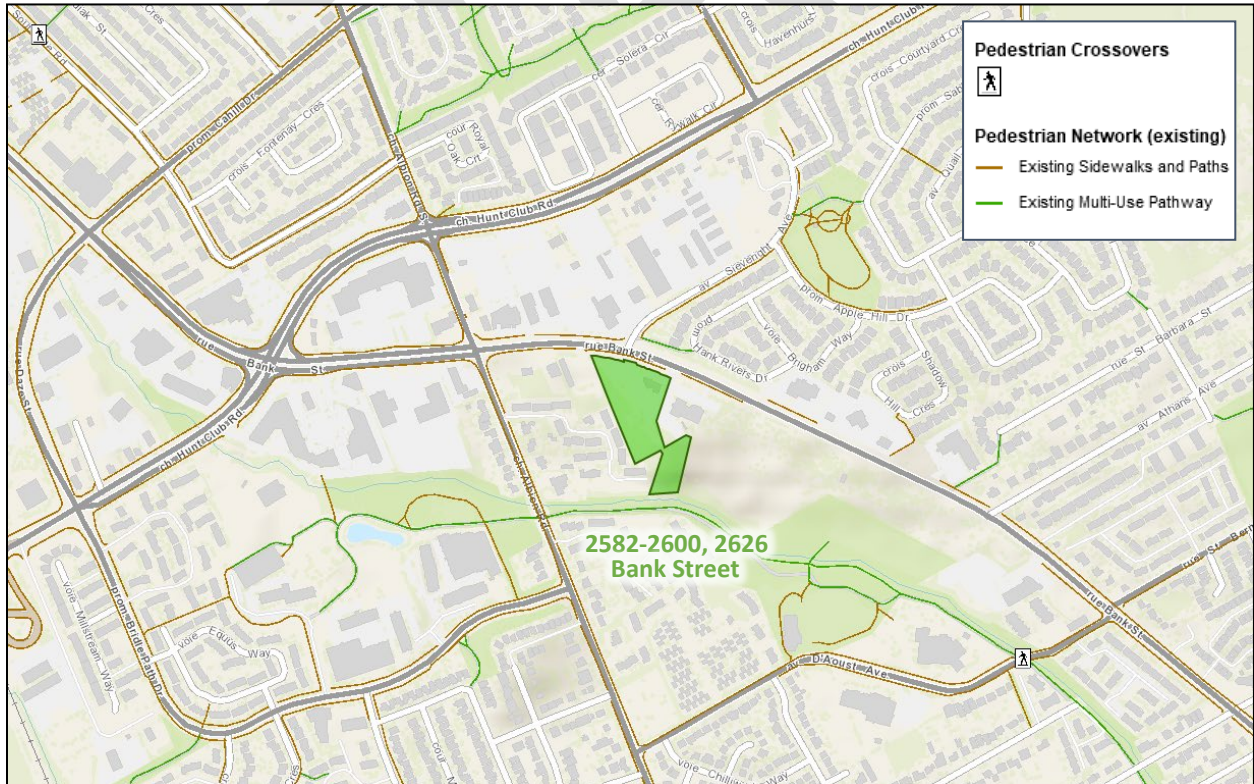


2.2.4 Cycling and Pedestrian Facilities

Figure 4 illustrates the pedestrian facilities in the study area and Figure 5 illustrates the cycling facilities.

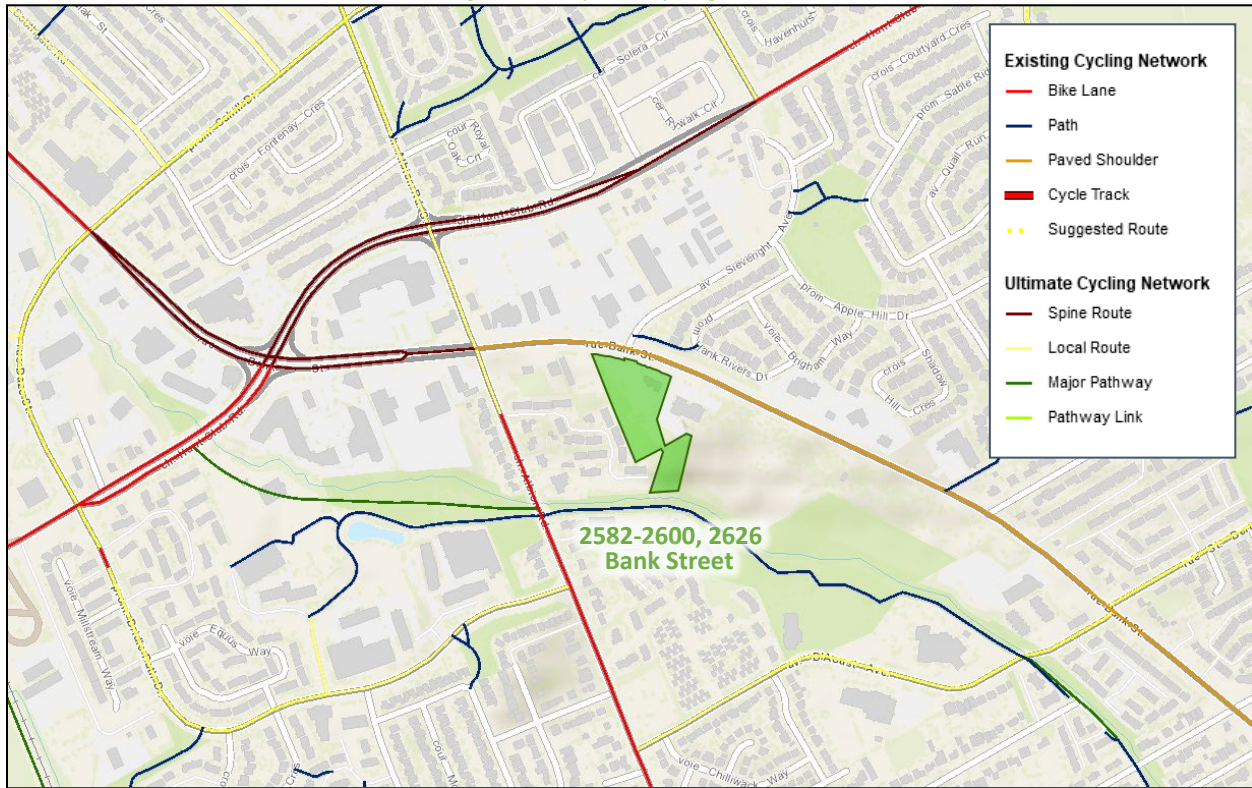
Sidewalks are provided along both sides of Hunt Club Road and both sides of Bank Street to the west of the site and both sides of Albion Road South of Bank Street. A single sidewalk is found along the west side of Albion Road north of Bank Street. A sidewalk exists along the site frontage which connects to the pedestrian facilities to the west. Cycling facilities include curbside bike lanes on Albion Road south of Bank Street and on Hunt Club Road west of Bank Street, and discontinuous paved shoulders along Bank Street east of Albion Road. Bank Street and Hunt Club Road are spine routes, and Albion Road, Bridle Path Drive/Dazé Street/Cahill Drive, and D’Aoust Avenue are local routes.

Figure 4: Study Area Pedestrian Facilities



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: February 8, 2021

Figure 5: Study Area Cycling Facilities



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: February 8, 2021

Pedestrian and cyclist volumes included in study area intersection counts, presented in Section 2.2.7, have been compiled and are illustrated in Figure 6 and Figure 7 respectively.

Figure 6: Existing Pedestrian Counts

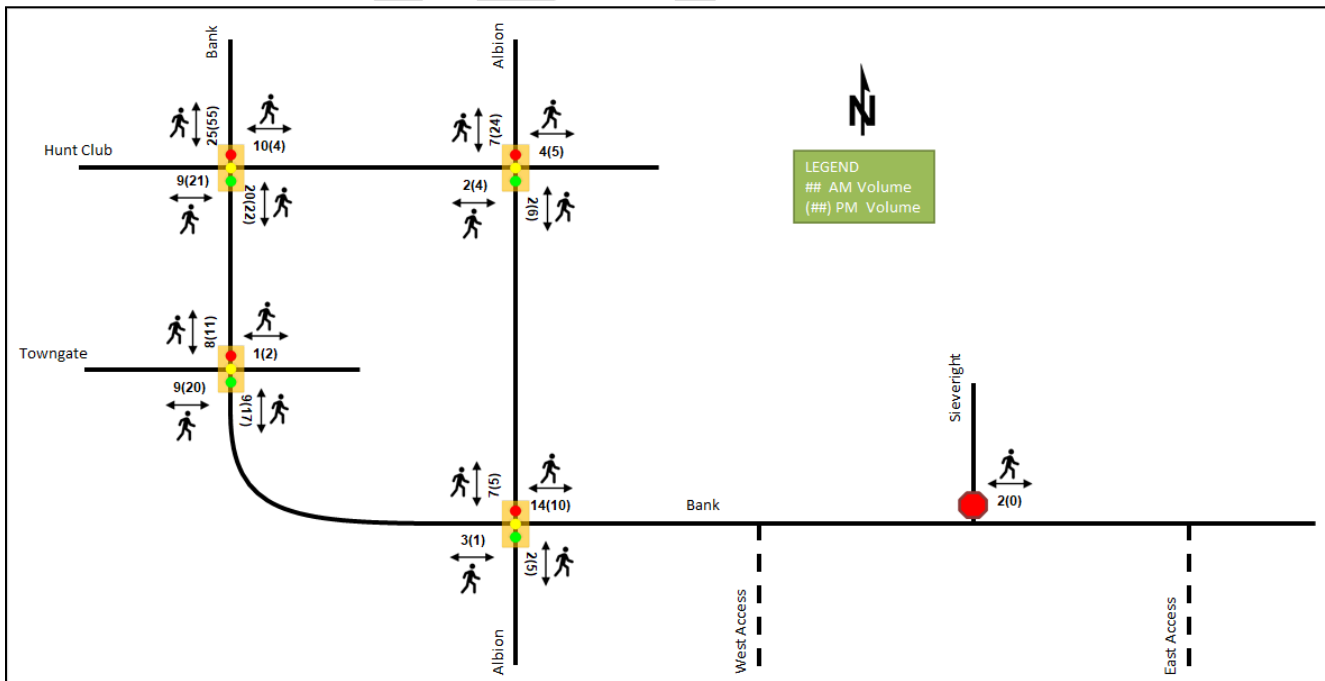
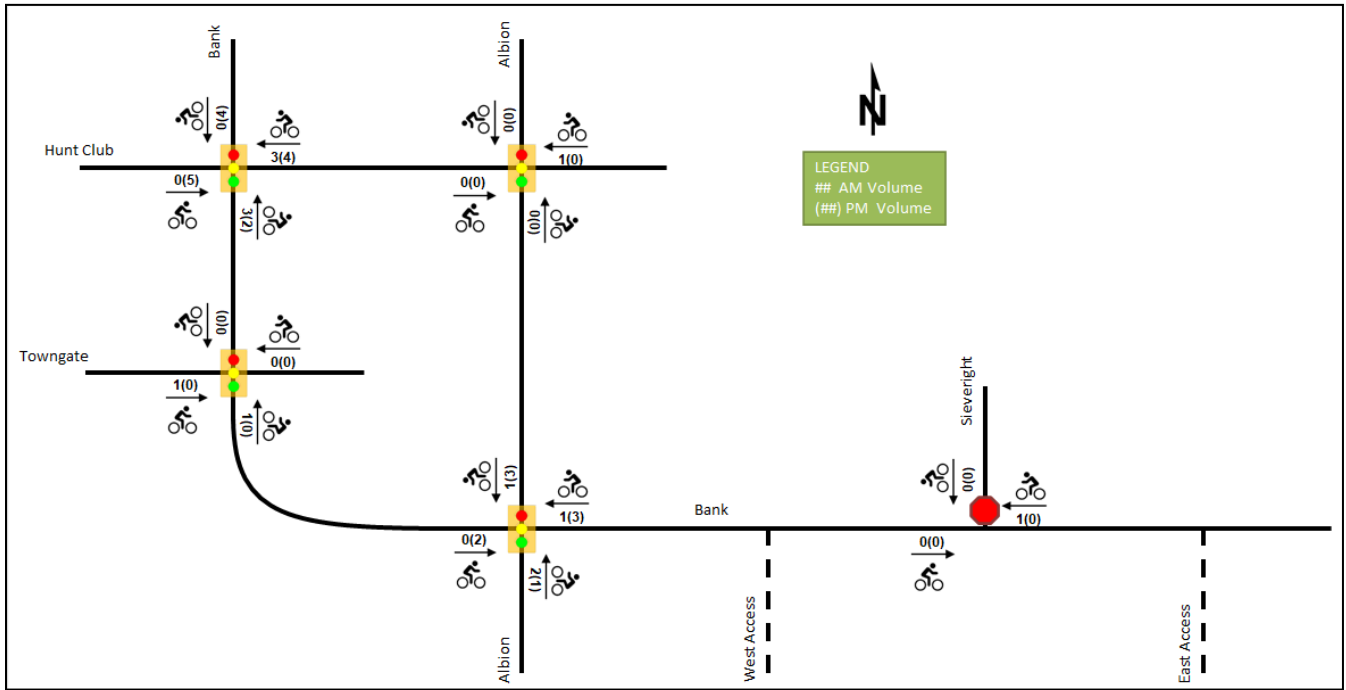


Figure 7: Existing Cyclist Counts



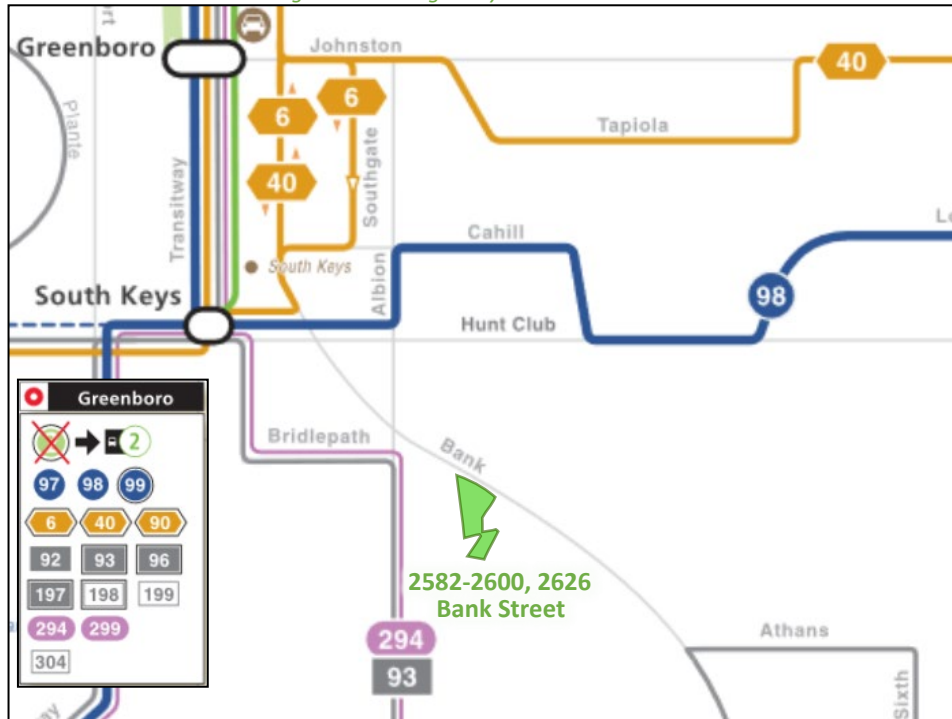
2.2.5 Existing Transit

The site is approximately 550 metres-walk to the intersection of Albion Road and Hunt Club Road, and 600 metres-walk to the intersection of Bank Street and Hunt Club Road, around which the route #98 stops. The site is additionally 950 metres-walk from the intersection of Bridle Path Drive at Albion Road where the routes #93 and 294 stop and 900 metres-walk from Bank Street at St. Bernard Street where the route #93 stops. The frequency of these routes within proximity of the proposed site currently are:

- Route # 93 – 10-15-minute service during peak period/direction, 30-minute service all day
- Route # 98 – 10-15-minute service during peak period/direction, 30-minute service all day
- Route # 294 – 15-30-minute service in peak period/direction only

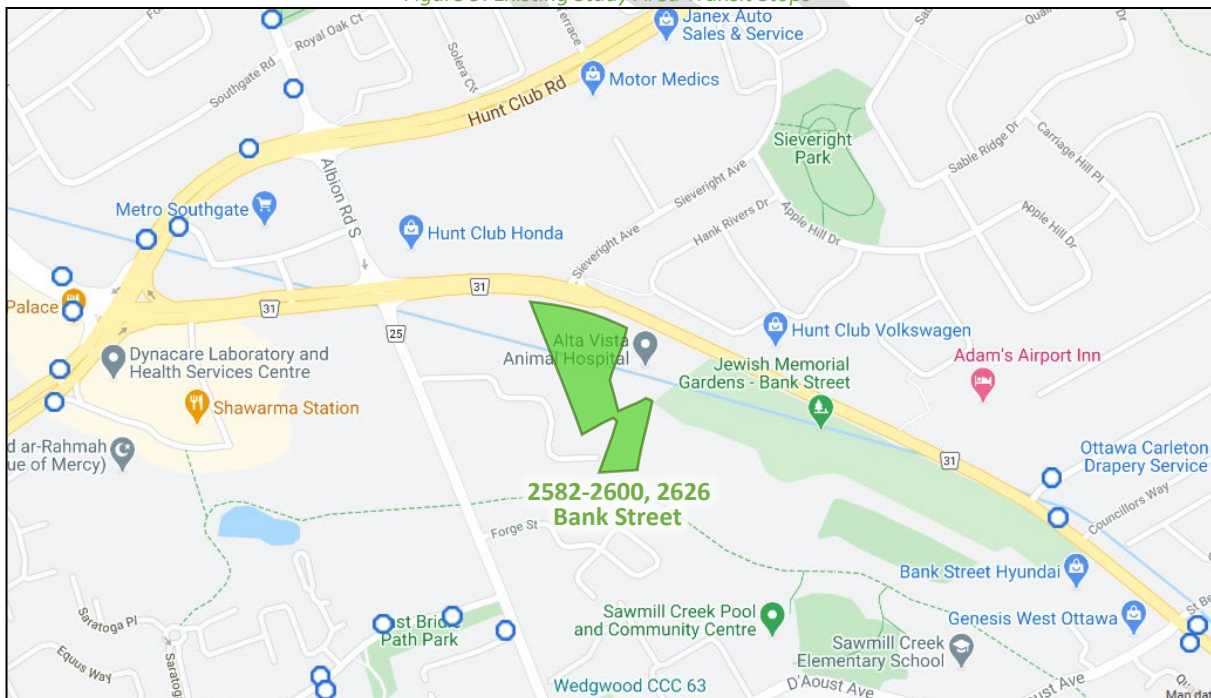
Figure 8 illustrates the transit system map in the study area and Figure 9 illustrates nearby transit stops.

Figure 8: Existing Study Area Transit Service



Source: <http://www.octranspo.com/> Accessed: February 8, 2021

Figure 9: Existing Study Area Transit Stops



Source: <http://www.octranspo.com/> Accessed: February 8, 2021

2.2.6 Existing Area Traffic Management Measures

There are no existing area traffic management measures within the Study Area.

2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa for the existing Study Area intersections. Table 1 summarizes the intersection count dates.

Table 1: Intersection Count Date

Intersection	Count Date
Bank Street at Hunt Club Road	Wednesday, June 12, 2019
Albion Road at Hunt Club Road	Thursday, April 5, 2018
Bank Street at Towngate Mall	Thursday, April 5, 2018
Albion Road at Bank Street	Thursday, June 20, 2019
Sieveright Avenue at Bank Street	Thursday, November 30, 2017

Figure 10 illustrates the existing traffic counts and Table 2 summarizes the existing intersection operations. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. Detailed turning movement count data is included in Appendix B and the Synchro worksheets are provided in Appendix C.

Figure 10: Existing Traffic Counts

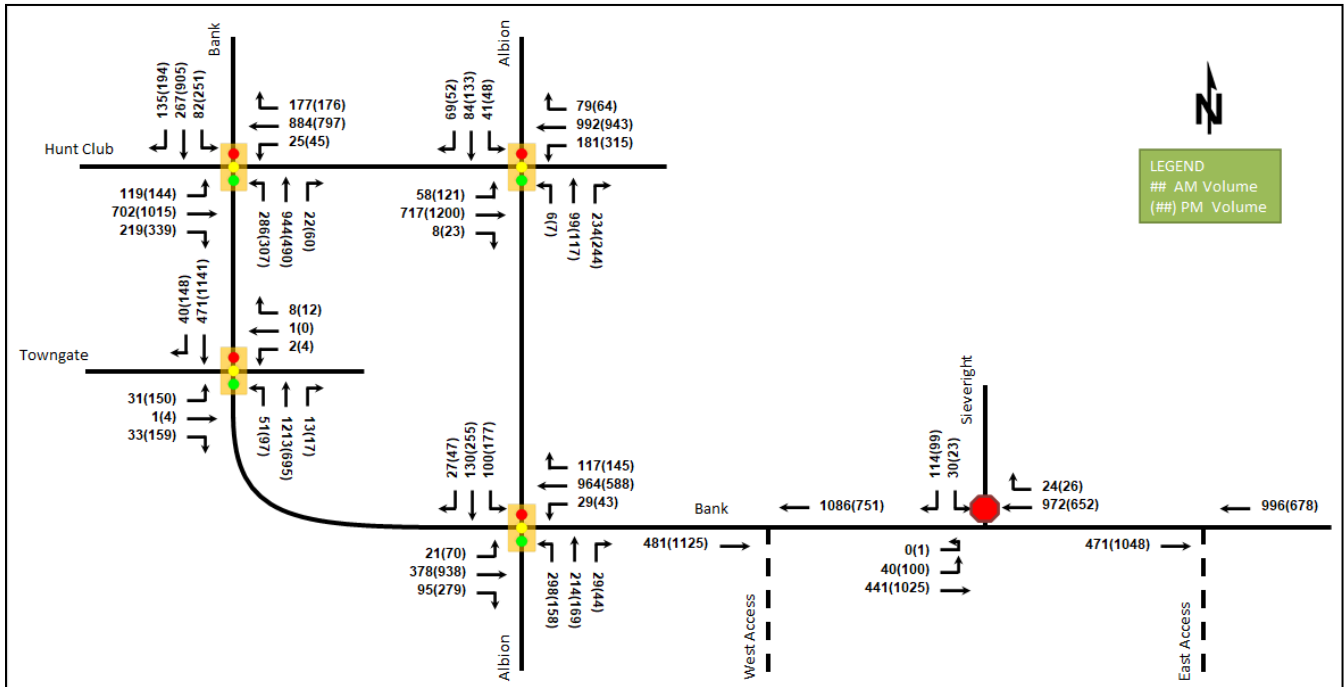


Table 2: Existing Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road <i>Signalized</i>	EBL	A	0.50	58.3	24.8	B	0.63	64.6	30.2
	EBT	B	0.69	37.5	111.1	F	1.12	107.5	#212.3
	EBR	A	0.36	5.3	17.6	A	0.60	14.6	51.9
	WBL	A	0.34	53.2	m11.7	A	0.52	73.8	m21.4
	WBT	F	1.09	114.7	#194.8	F	1.01	83.8	#150.7
	WBR	A	0.32	16.5	29.7	A	0.37	22.1	37.4
	NBL	C	0.75	66.4	55.9	F	1.07	126.3	m#71.4
	NBT	E	0.92	57.2	#159.2	A	0.50	38.9	m87.8
	NBR	A	0.04	0.1	0.0	A	0.12	4.5	m6.1
	SBL	A	0.50	64.9	19.4	D	0.88	80.8	#57.2
	SBT	A	0.35	37.4	43.2	E	0.92	53.2	#159.2
	SBR	A	0.27	1.2	0.0	A	0.39	10.1	26.7
Overall	E	0.98	59.2	-	F	1.04	69.0	-	
Albion Road at Hunt Club Road <i>Signalized</i>	EBL	A	0.22	3.3	m0.3	A	0.45	10.3	m4.9
	EBT	A	0.44	4.6	2.4	E	0.92	14.2	m26.6
	EBR	A	0.01	0.0	m0.0	A	0.04	0.1	m0.0
	WBL	A	0.46	9.6	22.5	E	0.99	81.1	#133.3
	WBT	A	0.57	15.5	101.6	A	0.58	20.7	113.9
	WBR	A	0.10	3.1	7.6	A	0.09	3.2	6.4
	NBL	A	0.04	62.0	m5.0	A	0.04	30.6	m4.0
	NBT/R	E	1.00	102.2	#129.0	E	0.94	63.3	#129.5
	SBL	C	0.79	117.7	#32.8	D	0.88	133.6	#36.9
	SBT/R	A	0.54	42.1	51.9	A	0.52	41.6	62.2
	Overall	B	0.68	25.9	-	E	1.00	30.8	-
Bank Street at Towngate Mall <i>Signalized</i>	EB	A	0.39	30.6	18.3	E	0.95	78.3	#127.1
	WB	A	0.07	24.5	5.4	A	0.05	8.8	4.3
	NBT/L	A	0.33	2.7	m23.6	A	0.32	7.2	18.1
	NBR	A	0.01	0.0	m0.0	A	0.02	0.4	m0.3
	SBT	A	0.20	3.9	24.6	A	0.59	5.6	m32.8
	SBR	A	0.04	1.7	0.9	A	0.17	0.8	m0.0
	Overall	A	0.33	4.0	-	B	0.69	15.2	-
Albion Road at Bank Street <i>Signalized</i>	EBL	A	0.23	66.0	14.2	A	0.53	74.9	m26.4
	EBT	A	0.25	20.4	52.1	B	0.67	14.4	m#155.6
	EBR	A	0.13	5.9	9.2	A	0.37	1.5	m6.4
	WBL	A	0.31	60.6	17.2	A	0.39	61.5	22.6
	WBT	B	0.61	24.9	#173.3	A	0.44	26.3	86.7
	WBR	A	0.16	5.5	14.5	A	0.22	5.0	14.6
	NBL	F	1.06	103.0	#87.0	D	0.83	59.8	#48.1
	NBT/R	A	0.53	36.7	61.8	A	0.42	30.7	56.0
	SBL	B	0.63	60.1	30.7	C	0.78	45.2	m54.6
	SBT/R	A	0.59	50.3	39.1	D	0.82	41.7	m88.7
Overall	D	0.81	37.3	-	C	0.77	25.6	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Sieveright Avenue at Bank Street Signalized	EBL	B	0.07	11.2	1.5	A	0.13	9.9	3.8
	EBT	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	-	-	-
	SBL	D	0.16	25.0	3.8	C	0.12	23.5	3.0
	SBR	C	0.27	15.3	8.3	B	0.18	12.0	4.5
	Overall	A	-	1.8	-	A	-	1.4	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 0.90

m = metered queue
= queue exceeds storage or mid-block length

During both the AM and PM peak hours, various capacity issues are noted throughout the study area.

The intersection of Bank Street and Hunt Club Road during the AM peak hour is shown to experience extended queuing on the northbound through movement, the westbound through movement is shown as being over capacity with high delay and extended queuing, and the overall intersection is shown to be at capacity. During the PM peak hour at this intersection, the eastbound through, westbound through, and northbound left movements each show as being over capacity with high delay and queueing, the southbound left movement shows high delay and extended queuing, where the southbound through movement is shown as exhibiting extended queuing, and the overall intersection is shown as being over capacity.

The intersection of Albion Road and Hunt Club Road during the AM peak hour shows the northbound through/right and southbound left movements as experiencing high delay and extended queuing, with the northbound through/right movement as being at capacity. During the PM peak hour, the westbound left and southbound left movements are shown as experiencing high delay and extended queuing, where the westbound left is also at capacity, the northbound through/right is shown as experiencing extended queuing, and the overall intersection is at capacity.

The intersection of Bank Street and the Towngate Plaza shopping centre is shown to experience high delay, and extended queuing and is at capacity during the PM peak hour, however this performance is a function of priority being given to the performance of the Bank Street approaches.

The intersection of Albion Road and Bank Street during the AM peak hour shows the northbound left movement as being over capacity and experiencing high delay and extended queuing, and the westbound through movement as exhibiting extended queueing. During the PM peak hour, the eastbound through movement and the northbound left movements are shown as exhibiting extended queuing.

2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study area road network. Table 3 summarizes the collisions types and conditions in the study area, Figure 11 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix D.

Table 3: Study Area Collision Summary, 2015-2019

		Number	%
Total Collisions		71	100%
Classification	Fatality	0	0%
	Non-Fatal Injury	19	27%
	Property Damage Only	52	73%
Initial Impact Type	Angled	19	27%
	Rear end	24	34%
	Sideswipe	8	11%
	Turning Movement	12	17%
	SMV Unattended	2	3%
	SMV Other	6	8%
	Road Surface Condition	Dry	52
Wet		11	15%
Loose Snow		5	7%
Slush		2	3%
Ice		1	1%
Pedestrian Involved		4	6%
Cyclists Involved		2	3%

Figure 11: Study Area Collision Records – Representation of 2015-2019

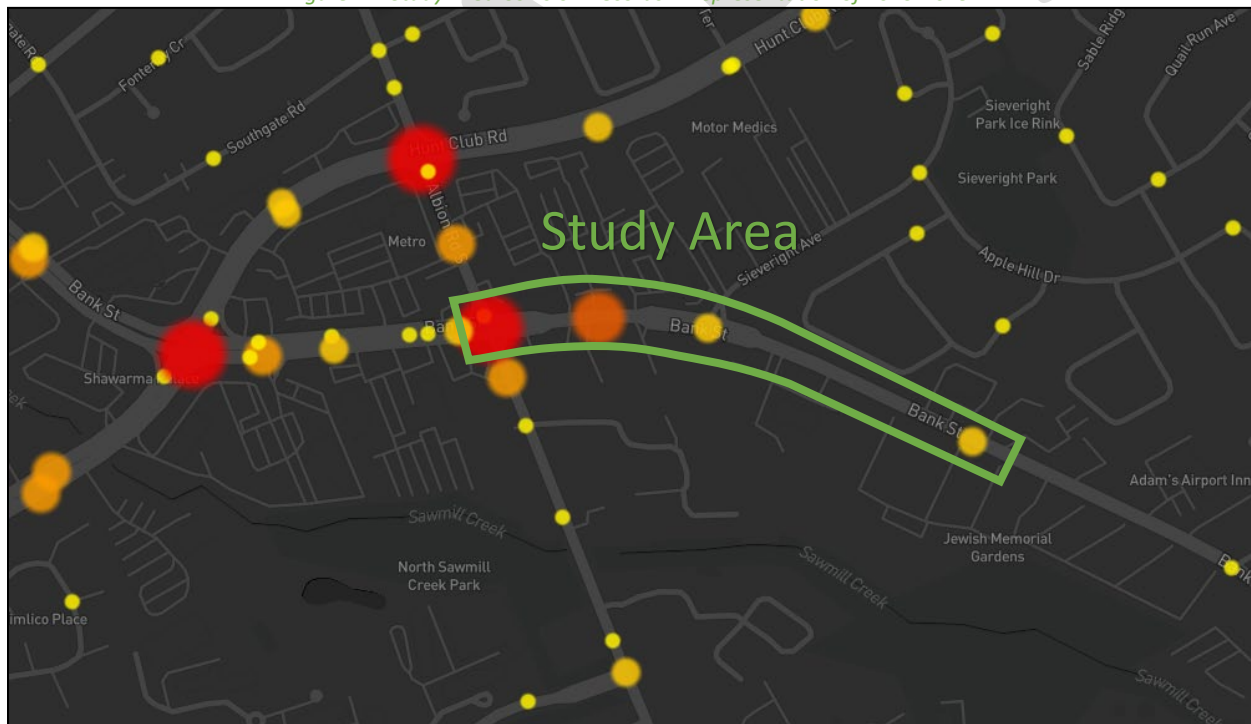


Table 4: Summary of Collision Locations, 2015-2019

	Number	%
Intersections / Segments	71	100%
Albion Road at Bank Street	39	55%
Bank Street at Sieveright Avenue	7	10%
Albion Road between Albion Road S and Bank Street	2	3%
Bank Street between Albion Road and Sieveright Avenue	15	21%
Bank Street between Sieveright Avenue and Athans Avenue	8	11%

Within the study area, the intersection of Albion Road at Bank Street and the segment of Bank Street between Albion Road and Sieveright Avenue are noted to have experienced higher collisions than other locations. Table 5 and Table 6 summarize the collision types and conditions for each of the intersection of Albion Road at Bank Street and segment of Bank Street between Albion Road and Sieveright Avenue.

Table 5: Albion Road at Bank Street Collision Summary

		Number	%
Total Collisions		39	100%
Classification	Fatality	0	0%
	Non-Fatal Injury	9	23%
	Property Damage Only	30	77%
Initial Impact Type	Angle	7	18%
	Rear end	17	44%
	Sideswipe	4	10%
	Turning Movement	8	21%
	SMV Other	2	5%
	SMV Unattended	1	3%
	Road Surface Condition	Dry	26
Wet		8	21%
Loose Snow		3	8%
Slush		2	5%
Pedestrian Involved		2	5%
Cyclists Involved		1	3%

The Albion Road at Bank Street intersection had a total of 39 collisions during the 2015-2019 time period, with 30 involving property damage only and the remaining nine having non-fatal injuries. The collision types are most represented by rear end with 17 collisions, followed by turning movement with eight and angle with seven, and four or fewer for sideswipe, SMV (other), and SMV (unattended). Rear end collisions are generally associated with congestion, and the turning movements may be influenced by the right-turn channels present on all but the eastbound approach. Weather conditions are not considered to influence collisions at this location. No mitigation is recommended at this time as a more detailed review would be required by the City to identify specific or full intersection upgrades.

Table 6: Bank Street between Albion Road and Sieveright Avenue Collision Summary

		Number	%
Total Collisions		15	100%
Classification	Fatality	0	0%
	Non-Fatal Injury	3	20%
	Property Damage Only	12	80%
Initial Impact Type	Angle	5	33%

		Number	%
Total Collisions		15	100%
	Rear end	3	20%
	Sideswipe	2	13%
	Turning Movement	2	13%
	SMV Other	2	13%
	SMV Unattended	1	7%
Road Surface Condition	Dry	13	87%
	Wet	1	7%
	Ice	1	7%
Pedestrian Involved		1	7%
Cyclists Involved		0	0%

The segment of Bank Street between Albion Road and Sieveright Avenue had a total of 15 collisions during the 2015-2019 time period, with 12 involving property damage only and the remaining three having non-fatal injuries. The collision types are most represented by angle with five collisions, followed by a relatively even split of rear end, sideswipe, turning movement, SMV (other) and SMV (unattended) with three or fewer collisions each. No discernible pattern is noted along the segment. Weather conditions are not considered to influence collisions at this location. No mitigation is recommended due to lack of specific collision type to be addressed.

2.3 Planned Conditions

2.3.1 Changes to the Area Transportation Network

Within the Transportation Master Plan, the Road Network’s Network Concept diagram shows the widening of Hunt Club Road, however this improvement is not included in the Affordable Network.

The TMP’s Rapid Transit and Transit Priority’s Affordable Network diagram identifies a continuous transit priority corridor on Hunt Club Road west of Albion Road, and the Network Concept diagram depicts isolated transit priority measures on Hunt Club Road east of Albion Road and on Bank Street North of Hunt Club Road.

Stage 2 LRT is due to extend the Trillium Line beyond Greenboro Station notably to South Keys Station just outside of the study area.

From the Ottawa Cycling Plan, Hunt Club Road between Bank Street and Lorry Greenburg Drive is to receive bike lanes as part of the Phase 2 Affordable Cycling Project List.

From the Planned Construction Projects portal, Bank Street is due to receive new sidewalks south of Sieveright Avenue to commence within four-to-seven years.

2.3.2 Other Study Area Developments

20 Mountain Crescent

The proposed development application includes a zoning by-law amendment to allow the construction of a 12-storey residential building comprising 151 residential dwelling units. The development is anticipated to generate 36 new AM and 41 new PM peak hour two-way auto trips and to be built out by 2022. (Parsons, 2020)

2425-2431 Bank Street

The proposed development application includes a site plan for the construction of a one-, seven-, and fourteen-storey addition to an existing retirement community. The development would add 144 units to the site, generate seven new AM and 17 new PM peak hour two-way auto trips and is anticipated to be built out by 2021. (Novatech, 2020)

3776-3780 Albion Road

The proposed development application includes a zoning by-law amendment to rezone the property from R1 to R4. No TIA is available for this development.

3 Study Area and Time Periods

3.1 Study Area

The study area will include the intersections of:

- Bank Street at:
 - Hunt Club Road
 - Towngate Plaza Access/Petro-Canada Access
 - Albion Road
 - Site Access West (Future Conditions)
 - Site Access East (Future Conditions)
- Albion Road at Hunt Club Road

The boundary road will be Bank Street and no screenlines are present within proximity to the site.

3.2 Time Periods

As the proposed development is composed primarily of office space the AM and PM peak hours will be examined.

3.3 Horizon Years

The anticipated build-out year is 2025. As a result, the full build-out plus five years horizon year is 2030.

4 Exemption Review

Table 7 summarizes the exemptions for this TIA.

Table 7: Exemption Review

Module	Element	Explanation	Exempt/Required
Design Review Component			
4.1 Development Design	4.1.2 Circulation and Access	Only required for site plans	Required
	4.1.3 New Street Networks	Only required for plans of subdivision	Exempt
4.2 Parking	4.2.1 Parking Supply	Only required for site plans	Required
	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
Network Impact Component			
4.5 Transportation Demand Management	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required
4.6 Neighbourhood Traffic Management	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Exempt
4.8 Network Concept		Only required when proposed development generates more than 200	Exempt

Module	Element	Explanation	Exempt/Required
		person-trips during the peak hour in excess of equivalent volume permitted by established zoning	

5 Development-Generated Travel Demand

5.1 Trip Generation and Mode Shares

This TIA has been prepared using the vehicle and person trip rates for the land uses of General Office using the fitted curve equation and of Shopping Centre using the average rates from the ITE Trip Generation Manual 10th Edition (2017). As the source rates are provided in vehicle trips alone, conversion to person trips is via the City-prescribed adjustment factor of 1.28. Table 8 summarizes the person trip rates for the proposed land use.

Table 8: Trip Generation Person Trip Rates

Dwelling Type	Land Use Code	Peak Hour	Vehicle Trip Rate	Person Trip Rates
General Office	710 (ITE)	AM	1.19	1.52
		PM	1.13	1.45
Shopping Centre	820 (ITE)	AM	0.94	1.20
		PM	3.81	4.88

Using the above Person Trip rates, the total person trip generation has been estimates. Table 9 below illustrates the total person trip generation for the proposed land uses.

Table 9: Total Person Trip Generation

Land Use	Units / GFA	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
General Office	83,076 ft ²	115	19	134	20	102	122
Shopping Centre	45,561 ft ²	34	21	55	107	115	222

Using the most recent National Capital Region Origin-Destination survey (OD Survey), the existing mode shares for Hunt Club have been determined and compared to various modes share breakdowns identified by City Staff as potential interpretations of the data. Table 10 summarizes these modal shares.

Table 10: Mode Shares

Travel Mode	Hunt Club (average)	Hunt Club (AM to/within)	Hunt Club (PM from/within)
Auto Driver	65%	75%	70%
Auto Passenger	15%	10%	20%
Transit	15%	5%	5%
Cycling	0%	1%	0%
Walking	5%	9%	5%
Total	100%	100%	100%

Internal capture rates from the ITE Trip Generation Handbook 3rd Edition have been assigned for the retail component for mixed-use developments. The rates summarized in Table 11 represent the percentage of trips to/from the retail use based on the office component.

Table 11: Internal Capture Rates

Land Use	AM		PM	
	In	Out	In	Out
Office to/from Shopping Centre	32%	29%	8%	2%

Pass-by reductions applied to the retail trip generation at a rate of 35% have been included, a value taken as a moderately conservative interpretation from the rates presented in the ITE Trip Generation Handbook 3rd Edition.

Using the above mode share targets by peak hour and from the person trip rates, the person trips by mode, internal capture, and pass-by reductions have been projected. Table 12 summarizes the trip generation by mode and the appropriate reductions.

Table 12: Trip Generation by Mode

Travel Mode	Mode Share (AM)	AM Peak Hour			Mode Share (PM)	PM Peak Hour		
		In	Out	Total		In	Out	Total
Auto Driver	75%	97	22	120	70%	59	122	180
Auto Passenger	10%	14	3	16	20%	17	35	51
Transit	5%	7	2	8	5%	4	9	13
Cycling	1%	1	0	1	0%	0	0	0
Walking	9%	11	3	14	5%	4	9	13
Internal Capture	<i>varies</i>	-7	-4	-11	<i>varies</i>	-6	-2	-8
Pass-by	35%	-12	-7	-19	35%	-37	-40	-78
Total	100%	130	30	159	100%	84	175	258

As shown above, 120 AM and 180 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

5.2 Trip Distribution

To understand the travel patterns of the subject development, the OD Survey has been reviewed to determine the existing district travel and these patterns were applied based on the build-out of Hunt Club. Table 13 below summarizes the distributions.

Table 13: OD Survey Distribution – Hunt Club

To/From	Residential % of Trips	Via
North	55%	40% Bank St, 10% Hunt Club Rd (W), 5% Hunt Club Rd (E)
South	10%	Bank St
East	10%	Hunt Club Rd
West	25%	15% Albion Road (S), 5% Hunt Club Rd, 5% Bank St (N)
Total	100%	-

5.3 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the study area road network. Figure 12 illustrates the new site generated volumes and Figure 13 illustrates pass-by volumes.

Figure 12: New Site Generation Auto Volumes

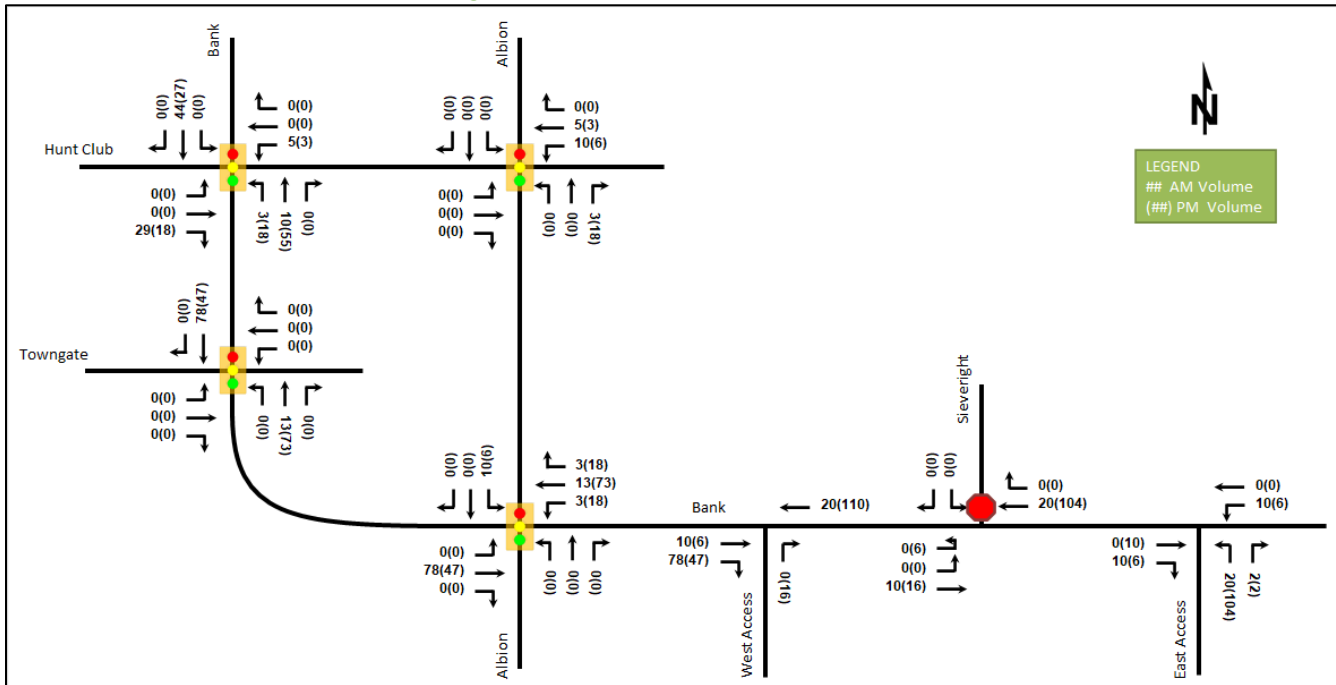


Figure 13: Pass-By Auto Volumes



6 Background Network Travel Demands

6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3. None of the listed projects are anticipated to impact traffic operations within the study area.

6.2 Background Growth

A review of the background projections from the City’s TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways. Table 14 summarizes the results of the model, and the projections are provided in Appendix E.

Table 14: TRANS Regional Model Projections – Study Area Growth Rates

Street	Direction Growth % from 2011 to 2031		Direction Growth % from Existing to 2031	
	Eastbound	Westbound	Eastbound	Westbound
Hunt Club Rd	-0.89%	-0.38%	-0.09%	-0.56%
	Northbound	Southbound	Northbound	Southbound
Bank St	0.29%	-0.38%	1.37%	-2.05%
Albion Rd	0.66%	-0.29%	0.04%	-2.19%

Growth during the AM peak hour within the study area is forecasted only to occur in the northbound direction. When accounting for the existing volumes, it can be seen that the growth predicted on Albion Road has been largely achieved and the growth on Bank Street has not yet occurred. As such, growth rates rounded to the nearest 0.25% have been applied to peak direction mainline volumes with negative growth rates taken as zero.

6.3 Other Developments

As the only active development files with TIAs, the background developments that are explicitly considered in the background conditions (Section 6.2) are:

- 20 Mountain Crescent
- 2425 Bank Street

The background development volumes within the study area have been provided in Appendix F.

7 Demand Rationalization

7.1 2025 Future Background Operations

Figure 14 illustrates the 2025 background volumes and Table 15 summarizes the 2025 background intersection operations. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2025 future background horizon are provided in Appendix G.

Figure 14: 2025 Future Background Volumes

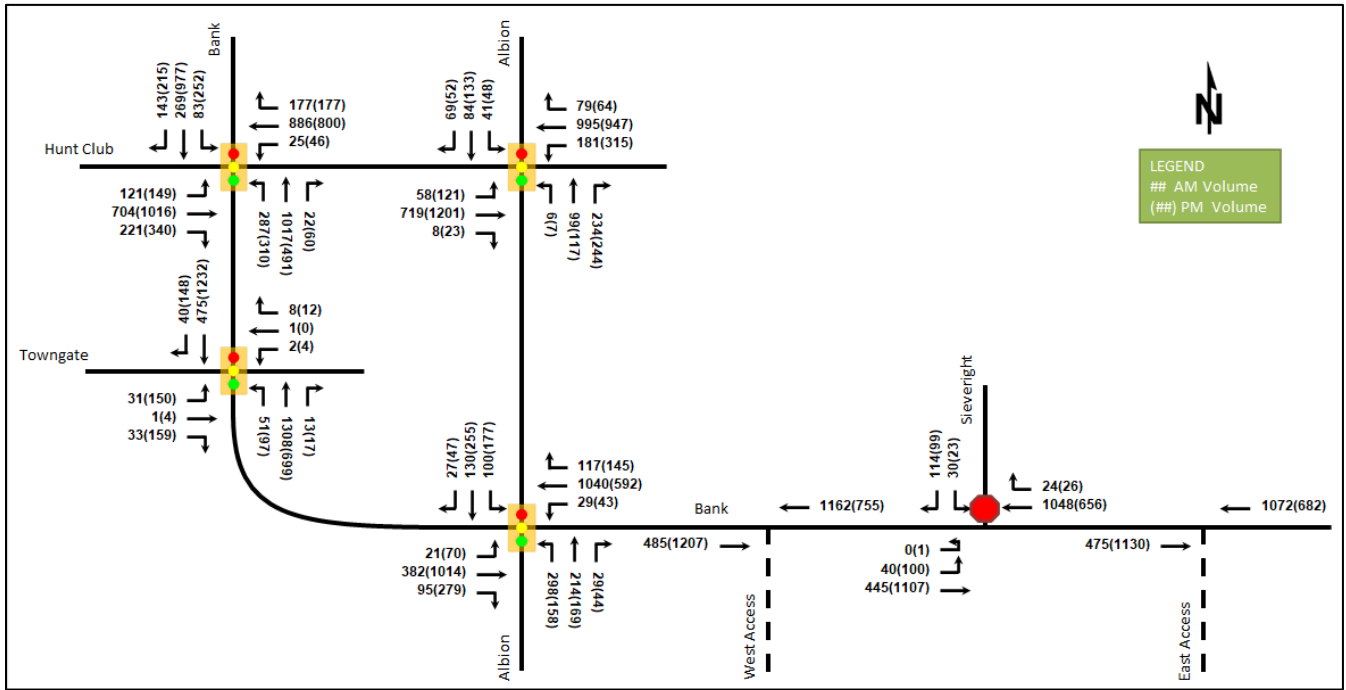


Table 15: 2025 Future Background Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road <i>Signalized</i>	EBL	A	0.48	58.3	23.2	A	0.59	63.2	28.4
	EBT	B	0.62	35.6	98.0	F	1.01	72.9	#182.6
	EBR	A	0.34	5.4	17.0	A	0.54	10.7	37.6
	WBL	A	0.30	53.1	m11.5	A	0.48	74.5	m22.7
	WBT	E	0.97	85.1	#166.6	E	0.91	61.6	#126.6
	WBR	A	0.29	14.3	24.7	A	0.34	19.1	31.1
	NBL	C	0.71	59.9	51.2	E	0.97	101.9	m#64.4
	NBT	D	0.84	40.2	#121.4	A	0.45	35.3	m81.4
	NBR	A	0.03	0.1	0.0	A	0.10	3.8	m5.4
	SBL	A	0.46	63.4	18.1	D	0.81	72.7	#49.6
	SBT	A	0.31	36.4	39.4	D	0.90	50.1	#151.4
SBR	A	0.26	1.1	0.0	A	0.39	9.4	25.4	
Overall	D	0.90	47.0	-	E	0.97	54.1	-	
Albion Road at Hunt Club Road <i>Signalized</i>	EBL	A	0.18	1.9	m0.3	A	0.36	6.6	m4.8
	EBT	A	0.39	3.4	2.3	C	0.77	11.0	m26.1
	EBR	A	0.01	0.0	m0.0	A	0.03	0.1	m0.0
	WBL	A	0.39	8.2	20.3	D	0.86	47.2	#97.0
	WBT	A	0.50	14.1	87.2	A	0.50	18.0	95.9
	WBR	A	0.08	2.6	6.2	A	0.08	2.5	5.0
	NBL	A	0.03	61.3	m4.3	A	0.04	30.7	m4.0
	NBT/R	E	0.94	89.5	#107.7	D	0.90	56.9	#108.4
	SBL	C	0.72	103.5	#28.7	C	0.72	93.2	#29.7
SBT/R	A	0.50	40.4	46.6	A	0.51	41.8	55.8	
Overall	A	0.60	22.9	-	D	0.88	24.1	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Towngate Mall Signalized	EB	A	0.36	30.5	17.2	D	0.90	67.8	#107.6
	WB	A	0.07	25.9	5.2	A	0.05	8.2	3.9
	NBT/L	A	0.31	2.6	m22.8	A	0.28	6.8	16.9
	NBR	A	0.01	0.0	m0.0	A	0.02	0.3	m0.0
	SBT	A	0.18	3.8	22.7	A	0.56	4.6	m29.4
	SBR	A	0.04	1.8	0.7	A	0.15	0.7	m0.0
	Overall	A	0.32	3.9	-	B	0.65	12.9	-
Albion Road at Bank Street Signalized	EBL	A	0.21	67.1	13.6	A	0.49	75.0	m25.7
	EBT	A	0.23	17.9	46.4	B	0.63	12.3	102.4
	EBR	A	0.12	4.3	7.3	A	0.33	1.3	m4.6
	WBL	A	0.29	60.1	16.0	A	0.37	61.1	21.1
	WBT	A	0.56	21.9	#164.3	A	0.38	24.2	77.3
	WBR	A	0.14	4.4	11.5	A	0.20	5.0	13.9
	NBL	E	0.94	74.8	68.7	C	0.72	48.3	38.6
	NBT/R	A	0.49	36.3	55.3	A	0.40	31.1	50.6
	SBL	A	0.59	58.2	28.4	C	0.73	48.3	m54.2
	SBT/R	A	0.56	49.8	0.0	C	0.79	45.8	m87.9
	Overall	C	0.75	31.6	-	B	0.70	24.2	-
Sieveright Avenue at Bank Street Signalized	EBL	B	0.06	10.9	1.5	A	0.11	9.5	3.0
	EBT	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	-	-	-
	SBL	C	0.13	23.4	3.8	C	0.10	21.4	2.3
	SBR	B	0.23	14.6	6.8	B	0.15	11.5	3.8
	Overall	A	-	1.6	-	A	-	1.3	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

During both the AM and PM peak hours at the 2025 future background horizon, the study area intersections operate similarly to the existing conditions, with operational improvement for all study area intersections due to the peak hour factor increasing from 0.90 to 1.00. No new capacity issues are noted.

7.2 2030 Future Background Operations

Figure 15 illustrates the 2030 background volumes and Table 16 summarizes the 2030 background intersection operations. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection. The synchro worksheets for the 2030 future background horizon are provided in Appendix H.

Figure 15: 2030 Future Background Volumes

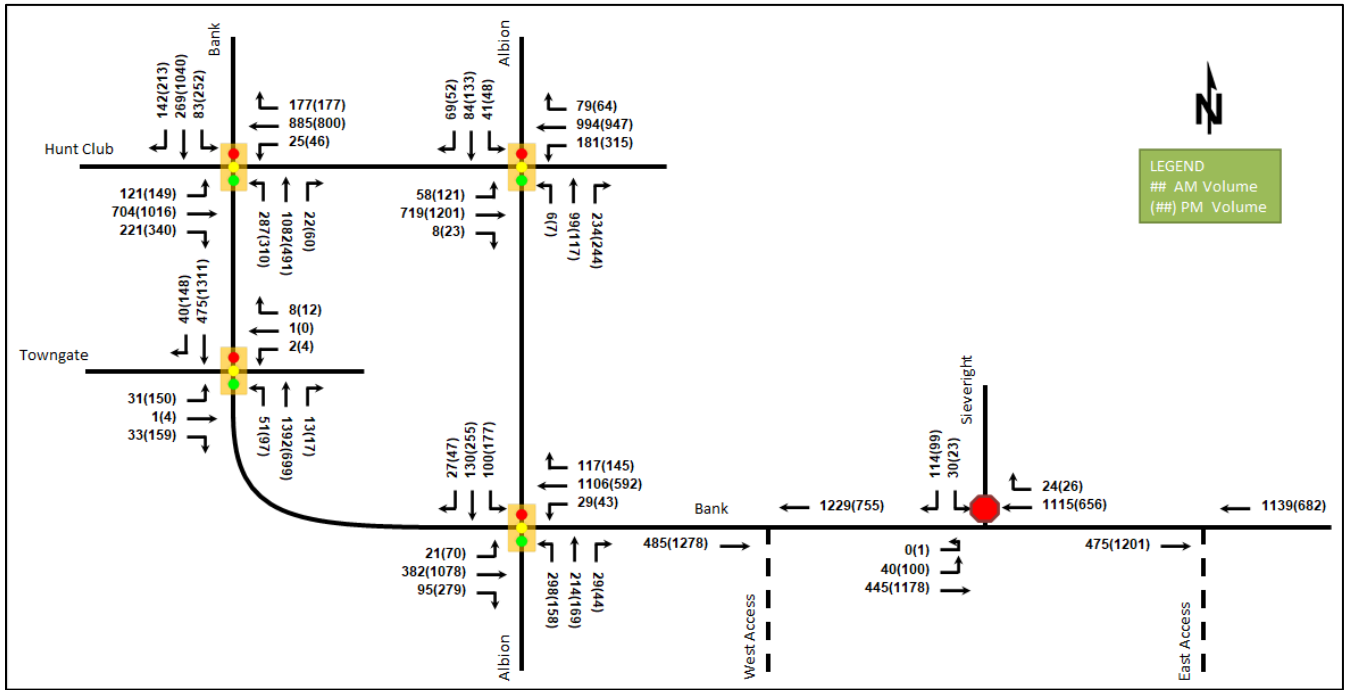


Table 16: 2030 Future Background Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road <i>Signalized</i>	EBL	A	0.48	58.3	23.2	A	0.59	63.2	28.4
	EBT	B	0.62	35.6	98.0	F	1.01	72.9	#182.6
	EBR	A	0.34	5.4	17.0	A	0.54	10.9	38.1
	WBL	A	0.30	53.0	m11.5	A	0.48	74.5	m22.7
	WBT	E	0.97	84.9	#166.3	E	0.91	61.6	#126.6
	WBR	A	0.29	14.3	24.7	A	0.34	19.1	31.1
	NBL	C	0.71	60.5	51.2	E	0.97	101.9	m#64.4
	NBT	D	0.89	45.7	#167.3	A	0.45	35.3	m81.4
	NBR	A	0.03	0.1	0.0	A	0.10	3.8	m5.4
	SBL	A	0.46	63.4	18.1	D	0.81	72.7	#49.6
	SBT	A	0.31	36.4	39.4	E	0.95	57.8	#168.2
SBR	A	0.26	1.1	0.0	A	0.39	9.3	24.9	
Overall	E	0.93	48.4	-	E	0.99	55.8	-	
Albion Road at Hunt Club Road <i>Signalized</i>	EBL	A	0.18	1.9	m0.3	A	0.36	6.6	m4.8
	EBT	A	0.39	3.4	2.3	C	0.77	11.0	m26.1
	EBR	A	0.01	0.0	m0.0	A	0.03	0.1	m0.0
	WBL	A	0.39	8.2	20.3	D	0.86	47.2	#97.0
	WBT	A	0.50	14.1	87.2	A	0.50	18.0	95.9
	WBR	A	0.08	2.6	6.2	A	0.08	2.5	5.0
	NBL	A	0.03	61.3	m4.3	A	0.04	30.7	m4.0
	NBT/R	E	0.94	89.5	#107.7	D	0.90	56.9	#108.5
	SBL	C	0.72	103.5	#28.7	C	0.72	93.2	#29.7
	SBT/R	A	0.50	40.4	46.6	A	0.51	41.8	55.8
Overall	B	0.60	22.9	-	D	0.88	24.1	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Towngate Mall Signalized	EB	A	0.36	30.5	17.2	D	0.90	67.8	#107.6
	WB	A	0.07	25.9	5.2	A	0.05	8.2	3.9
	NBT/L	A	0.33	2.5	m23.5	A	0.28	6.8	16.9
	NBR	A	0.01	0.0	m0.0	A	0.02	0.3	m0.0
	SBT	A	0.18	3.8	22.7	A	0.60	5.0	m29.1
	SBR	A	0.04	1.8	0.7	A	0.15	0.7	m0.0
	Overall	A	0.33	3.8	-	B	0.68	12.8	-
Albion Road at Bank Street Signalized	EBL	A	0.21	67.1	13.6	A	0.49	74.7	m24.2
	EBT	A	0.23	17.9	46.4	B	0.67	13.9	#162.6
	EBR	A	0.12	4.3	7.3	A	0.33	1.4	m4.5
	WBL	A	0.29	60.1	16.0	A	0.37	61.1	21.1
	WBT	A	0.60	22.6	#181.5	A	0.38	24.2	77.3
	WBR	A	0.14	4.4	11.5	A	0.20	5.0	13.9
	NBL	E	0.94	74.8	68.7	C	0.72	48.3	38.6
	NBT/R	A	0.49	36.3	55.3	A	0.40	31.1	50.6
	SBL	A	0.59	58.1	28.4	C	0.73	48.3	m54.2
	SBT/R	A	0.56	49.8	0.0	C	0.79	45.8	m87.9
Overall	C	0.77	31.6	-	C	0.73	24.5	-	
Sieveright Avenue at Bank Street Signalized	EBL	B	0.07	11.3	1.5	A	0.11	9.5	3.0
	EBT	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	-	-	-
	SBL	D	0.14	25.2	3.8	C	0.10	21.8	2.3
	SBR	C	0.25	15.2	7.5	B	0.15	11.5	3.8
Overall	A	-	1.7	-	A	-	1.3	-	

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

During both the AM and PM peak hours at the 2030 future background horizon, the study area intersections operate similarly to the 2025 future background conditions. No new capacity issues are noted.

7.3 Modal Share Sensitivity and Demand Rationalization Conclusions

Capacity constraints have been noted at the Bank Street at Hunt Club Road intersection. With specific concern to the subject development, capacity constraints are noted on the northbound left-turn movement during the PM peak hour, where v/c for existing conditions is 1.07 on this movement. Area modal shift may occur with the extension of the LRT line to and beyond South Keys Station, and operations may improve at this intersection as a result. Given the existing transit service in the immediate site context, a modal shift exceeding the existing area mode shares for site traffic is not anticipated. The changes to area transit service that support the new LRT stations would likely be the pretext to a network auto demand reduction within the study area.

As with initial redevelopments along arterial roadways, mode share constraints and a reliance on auto modes is typically the impact documented with the TIAs. Subsequent to these development locations, the City road rehabilitation to construct additional active mode facilities and an increase in transit service in the area will need lead to a shift in the area mode share. This may reduce the future trips from the site or decrease the impact of additional developments that align more closely with more supportive infrastructure and service. The Bank Street improvements currently envision by the City are a step towards this progression but are anticipated to be on a mid to long term implementation timeline. Previous transit service models have included routes along Bank Street between Albion Road and Athans Avenue and could be a near term solution for the City to support further redevelopment in the area and increase connectivity to the Trillium LRT line.

Within the site plan itself, the road impacts are anticipated to result in queuing and delays within the internal drive aisles and have limited impact on the operation of Bank Street as a whole. This condition would be conducive to additional mode share adoption for non-auto modes if the supporting services are provided. Due to the current transportation environment, it is anticipated that active mode demand management features will provide potential reductions (if required) and support alternative modes, and transit management will be ineffectual at this time.

7.3.1 Review of Design Considerations for U-Turns on Bank Street at Sieveright Avenue

Given the proposed access locations and volumes along Bank Street, it is anticipated that there will be potential for increased U-turn volumes at the Bank Street and Sieveright Avenue intersection. The U-turn would be in the eastbound left-turn lane to head west and north towards Hunt Club Road.

A desktop review was completed to review the sight line distance from the eastbound left-turn lane on Bank Street at Sieveright Avenue. Using the contour lines from geoOttawa, the westbound approach along Bank Street to Sieveright Avenue has an approximately 3% downgrade. Based on TAC equations 2.5.2 and 2.5.3 from section 2.5.3, the stopping sight distance is approximately 110 metres for a design speed of 70 km/h with the given grade. The available sight distance is approximately 120 metres. While the location meets the +10 km/h design speed for Bank Street, the City's desired design speed will need to be confirmed if a higher design speed is required. For example, the 75 km/h stopping sight distance is approximately 122 metres and would require a subsequent site visit, once permitted, to confirm it is currently provided at this intersection. An illustration of the stopping sight distance for the westbound approach is provided in Appendix I.

8 Development Design

8.1 Design for Sustainable Modes

Parking for vehicles is proposed both via surface lots surrounding the site buildings and underground via the ramp to the garage entrance below Building B. Bicycle parking for the respective buildings is proposed via racks in front of the existing building and Building C, and underground for Buildings A and B, in proportion to the required spaces per building.

Walkways circulate the site surrounding the buildings and parking facilities and connect to the existing pedestrian facilities on Bank Street via two connections between the site accesses and another at the eastern site access.

Bus stop distances are provided in Section 2.2.5. Area bus stops are all over 400 metres walk from the building entrances, with the nearest stop approximately 550 metres walk at the intersection of Albion Road and Hunt Club Road.

8.2 Circulation and Access

Access is to be provided via a proposed right-in/right-out western access and an existing full-movement eastern access, each with 6.7-metre-wide drive aisles. Waste collection is anticipated as occurring on-site, and emergency vehicles will access the site via both accesses.

9 Parking

9.1 Parking Supply

The site plan proposes 441 vehicle parking spaces of which 92 are proposed as being above ground and 349 as being below. Bicycle parking is proposed as 48 spaces, of which 16 are proposed as being above ground and 32 as being below.

Per the zoning by-law, the office space requires 185 vehicle spaces (based upon 2.4 vehicle spaces per 100 m² of gross floor area) and 31 bicycle spaces (based upon one bicycle space per 250 m² of gross floor area), and the retail component requires 152 vehicle spaces (based upon 3.6 vehicle spaces per 100 m² of gross floor area) and 17 bicycle spaces (based upon one bicycle space per 250 m² of gross floor area).

The minimum vehicle and bicycle parking is being proposed by the development.

10 Boundary Street Design

Table 17 summarizes the MMLOS analysis for the boundary street of Bank Street. The existing and future conditions will be the same and are considered in one row. The boundary street analysis is based on the land use designation of “Arterial Main Street”. The MMLOS worksheets has been provided in Appendix J.

Table 17: Boundary Street MMLOS Analysis

Segment	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Bank Street	F	C	F	C	-	-	A	D

Bank Street does not meet the pedestrian and cycling MMLOS targets. The pedestrian LOS targets cannot be met with any sidewalk configuration due to the volumes and operating speeds on the roadway. Bicycle LOS is limited by the mixed traffic conditions and targets may be met through the introduction of a dedicated bike facility. The appropriateness of cycling treatments along Bank Street would be within a corridor context beyond the scope of this report.

11 Access Intersections Design

11.1 Location and Design of Access

The site will connect to the adjacent arterial road network via the existing full-movement access at the eastern property line and via a proposed right-in/right-out access near the western property line. No sightline issues are present for the proposed western site access, as illustrated in the sightline analysis in Appendix I.

The proposed right-in/right-out access, constituting the second two-way private approach for the frontage of over 138 metres, has a width of 6.7 metres, a throat length of 41.7 metres and is over 3.0 metres from the property line. This access will have through and left-turn movements restricted through the median on Bank Street.

The existing full movement access is shared with the adjacent property and will not be modified through this site plan.

11.2 Intersection Control

Both accesses are to be stop controlled on the minor approach with Bank Street operating under free-flow conditions.

11.3 Access Intersection Design

11.3.1 2025 Future Total Access Intersection Operations

The 2025 future total intersection volumes are illustrated in Figure 16 and the access intersection operations are summarized below in Table 18. The level of service for is based on HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix K.

Figure 16: 2025 Future Total Volumes

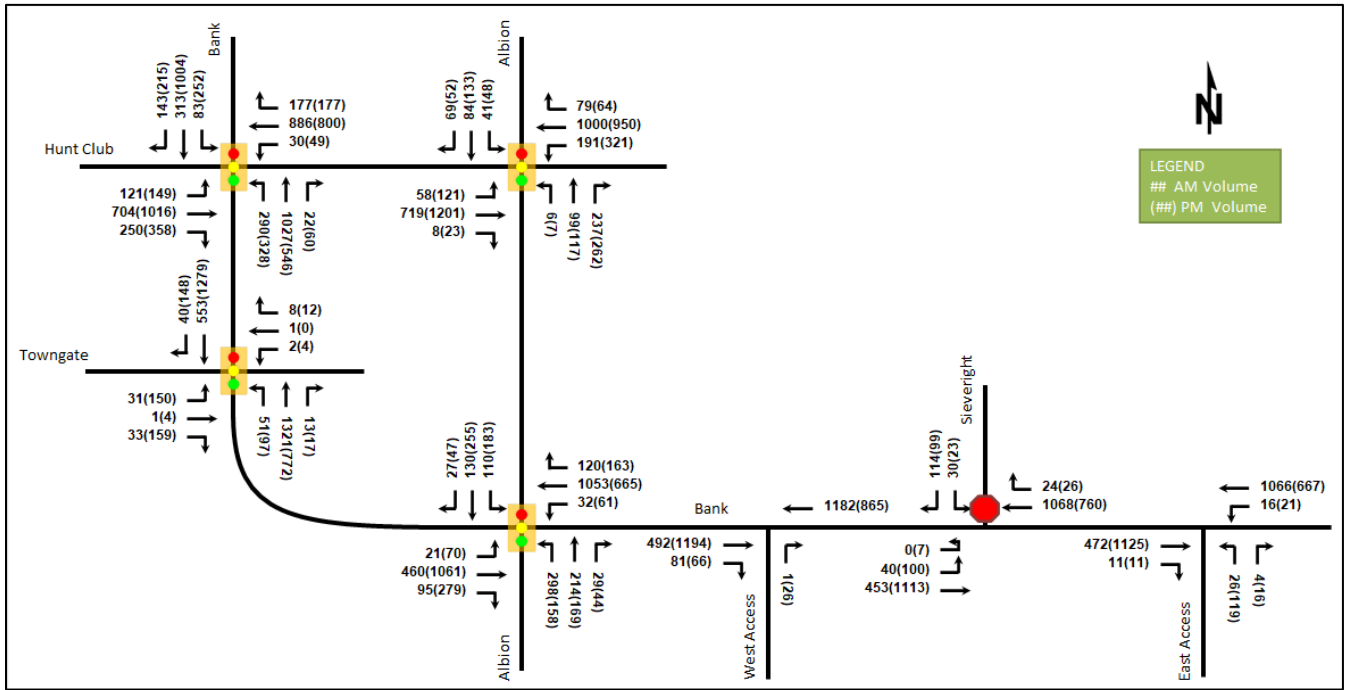


Table 18: 2025 Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at West Access <i>Signalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBT	-	-	-	-	-	-	-	-
	NBR	B	0.00	10.1	0.0	B	0.06	14.0	1.5
	Overall	A	-	0.0	-	A	-	0.2	-
Bank Street and East Access <i>Signalized</i>	EBT	-	-	-	-	-	-	-	-
	EBR	-	-	-	-	-	-	-	-
	WBL	A	0.02	8.4	0.0	B	0.03	11.1	0.8
	WBT	-	-	-	-	-	-	-	-
	NBL/R	C	0.08	15.2	2.3	E	0.59	41.4	25.5
	Overall	A	-	0.4	-	A	-	3.0	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

The access intersections at the 2025 future total horizon operate satisfactorily with the forecasted average delays on the northbound left/right movement at the eastern access of just over 40 seconds. All other movements operate well at both intersections, notably including the westbound left movement at the east access.

11.3.2 2030 Future Total Access Intersection Operations

The 2030 future total intersection volumes are illustrated in Figure 17 and the access intersection operations are summarized below in Table 19. The level of service for is based on HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix L.

Figure 17: 2030 Future Total Volumes

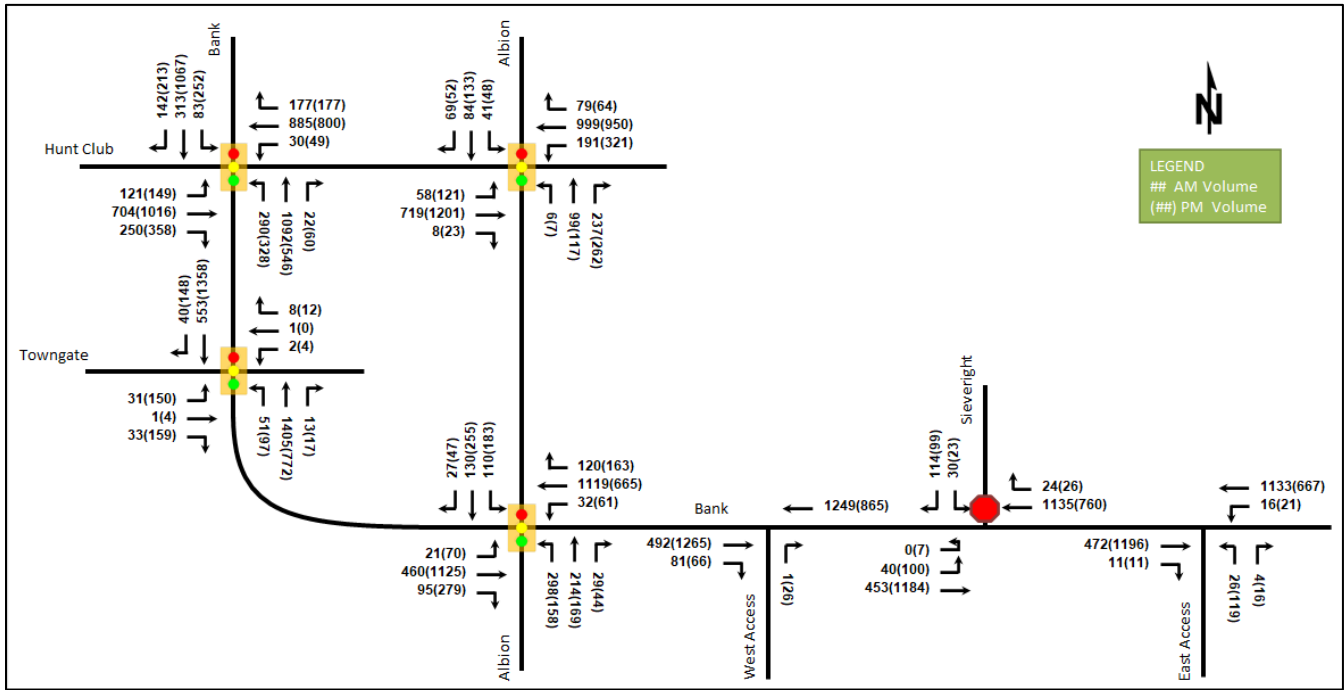


Table 19: 2030 Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at West Access <i>Signalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBT	-	-	-	-	-	-	-	-
	NBR	B	0.00	10.1	0.0	B	0.07	14.6	1.5
	Overall	A	-	0.0	-	A	-	0.2	-
Bank Street and East Access <i>Signalized</i>	EBT	-	-	-	-	-	-	-	-
	EBR	-	-	-	-	-	-	-	-
	WBL	A	0.02	8.4	0.0	B	0.04	11.5	0.8
	WBT	-	-	-	-	-	-	-	-
	NBL/R	C	0.08	15.5	2.3	E	0.64	48.2	28.5
	Overall	A	-	0.3	-	A	-	3.3	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

The access intersections at the 2030 future total horizon operate similarly to the 2025 future total conditions, with an increase in average delay on the northbound left/right movement to just under 50 seconds. The remaining movements at both intersections continue to operate well.

11.3.3 Access Intersection MMLoS

As the access intersections are not signalized, no access intersection MMLoS analysis has been performed.

11.3.4 Recommended Design Elements

No design elements are recommended for the existing site accesses and the new access location will meet the private approach bylaw and be restricted to right-in/right-out by the centre median on Bank Street.

12 Transportation Demand Management

12.1 Context for TDM

The mode shares used within the TIA represent a reliance on auto travel and reflect the limited site access to transit. Overall, the modal shares are likely to be achieved, and limited opportunity for modal shifts exist in the absence of transit stops in the vicinity of the site.

The subject site is within the Bank Arterial Mainstreet design priority area and no age restrictions are noted for the occupants.

12.2 Need and Opportunity

The subject site has been assumed to rely predominantly on auto travel and those assumptions have been carried through the analysis. The risk for not meeting the mode share targets is seen as being low due to their conservative nature.

12.3 TDM Program

The “suite of post occupancy TDM measures” has been summarized in the TDM checklists for the residential land uses. The checklist is provided in Appendix M. The key TDM measures recommended include:

- Display local area walking, cycling, and transit maps and area route schedules at entrances
- Provide a multimodal travel option information package to new employees as well as online links to transit information
- Provide a dedicated ridematching portal
- Charge for long-term parking and unbundle parking cost from lease rates

Should the City plan to revise the transit service in the area, a PRESTO bus pass may be incorporated into the site TDM measures but will have limited impact at this stage.

13 Transit

13.1 Route Capacity

In Section 5.1 the trip generation by mode was estimated, including an estimate of the number of transit trips that will be generated by the proposed development. Table 20 summarizes the transit trip generation.

Table 20: Trip Generation by Transit Mode

Travel Mode	Mode Share	AM Peak Period			PM Peak Period		
		In	Out	Total	In	Out	Total
Transit	5%	7	2	8	4	9	13

The proposed development is anticipated to generate an additional eight AM peak hour transit trips and 13 PM peak hour transit trips. Of these trips, seven inbound AM trips and nine outbound PM trips are anticipated.

As previously discussed within this report, transit service is limited within the study area. The surrounding routes should be able to accommodate any site transit trips at the 5% mode share assumed for this site. Per Section 2.2.5, the site is approximately 550 metres-walk to the intersection of Albion Road and Hunt Club Road, and 600 metres-walk to the intersection of Bank Street and Hunt Club Road, around which the route #98 stops. The site is additionally 950 metres-walk from the intersection of Bridle Path Drive at Albion Road where the routes #93 and 294 stop and 900 metres-walk from Bank Street at St. Bernard Street where the route #93 stops. The low volume of transit riders can be expected to walk to and from these stops. Transit service changes should be explored by

OC Transpo to service this redevelopment and other land uses on the arterial mainstreet corridor between Hunt Club Road and St. Bernard Street with connections to South Keys Station.

13.2 Transit Priority

No transit priority is required explicitly for this study.

14 Network Intersection Design

14.1 Network Intersection Control

No change to the existing signalized control is recommended for the network intersections.

14.2 Network Intersection Design

14.2.1 2025 Future Total Network Intersection Operations

The 2025 future total network intersection operations are summarized below in Table 21. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix K.

Table 21: 2025 Future Total Network Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road <i>Signalized</i>	EBL	A	0.48	58.3	23.2	A	0.59	63.2	28.4
	EBT	B	0.62	35.6	98.0	F	1.01	73.2	#182.6
	EBR	A	0.37	5.3	17.8	A	0.57	12.7	44.5
	WBL	A	0.36	55.4	m14.1	A	0.51	75.8	m23.9
	WBT	E	0.97	85.1	#166.8	E	0.91	62.9	#127.0
	WBR	A	0.29	14.2	24.7	A	0.34	19.7	31.1
	NBL	C	0.71	61.0	51.7	F	1.03	114.7	m#69.8
	NBT	D	0.85	40.4	#153.1	A	0.50	37.3	m89.5
	NBR	A	0.03	0.1	0.0	A	0.10	3.9	m5.7
	SBL	A	0.46	63.4	18.1	D	0.81	72.7	#49.6
	SBT	A	0.36	37.2	45.4	E	0.92	52.9	#158.4
SBR	A	0.26	1.1	0.0	A	0.39	9.5	25.6	
Overall	E	0.91	46.8	-	E	0.99	56.0	-	
Albion Road at Hunt Club Road <i>Signalized</i>	EBL	A	0.18	2.0	m0.3	A	0.36	6.7	m4.8
	EBT	A	0.39	3.6	2.3	C	0.78	11.2	m26.0
	EBR	A	0.01	0.0	m0.0	A	0.03	0.1	m0.0
	WBL	A	0.40	8.5	21.4	D	0.88	51.6	#101.5
	WBT	A	0.51	14.2	87.8	A	0.51	18.4	96.3
	WBR	A	0.08	2.6	6.2	A	0.08	2.5	5.0
	NBL	A	0.03	61.3	m4.4	A	0.03	30.6	m3.9
	NBT/R	E	0.94	89.5	#108.6	E	0.92	58.8	#115.3
	SBL	C	0.72	102.9	#28.7	C	0.79	109.2	#31.7
	SBT/R	A	0.50	40.4	46.6	A	0.49	41.0	55.8
	Overall	B	0.61	23.0	-	C	0.91	25.4	-

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Towngate Mall Signalized	EB	A	0.36	30.5	17.2	D	0.90	69.7	#107.6
	WB	A	0.07	25.9	5.2	A	0.05	8.2	3.9
	NBT/L	A	0.32	2.6	m23.1	A	0.30	6.6	17.4
	NBR	A	0.01	0.0	m0.0	A	0.02	0.2	m0.2
	SBT	A	0.21	3.8	25.7	A	0.58	5.1	m31.5
	SBR	A	0.04	1.7	0.9	A	0.15	0.7	m0.0
	Overall	A	0.32	3.9	-	B	0.67	13.0	-
Albion Road at Bank Street Signalized	EBL	A	0.21	66.0	13.7	A	0.49	74.9	m24.3
	EBT	A	0.28	20.5	57.0	B	0.67	14.4	#164.8
	EBR	A	0.12	5.4	8.6	A	0.33	1.4	m5.5
	WBL	A	0.31	60.5	17.2	A	0.46	62.4	27.0
	WBT	A	0.57	22.5	#167.6	A	0.43	24.9	88.2
	WBR	A	0.14	4.6	12.2	A	0.22	4.8	14.6
	NBL	E	0.92	69.3	68.7	C	0.72	48.3	38.6
	NBT/R	A	0.48	35.5	55.3	A	0.40	31.1	50.6
	SBL	B	0.62	60.6	31.3	C	0.76	49.1	m55.1
	SBT/R	A	0.54	49.3	36.0	C	0.79	44.8	m86.1
	Overall	C	0.75	31.4	-	C	0.72	25.0	-
Sieveright Avenue at Bank Street Signalized	EBL	B	0.06	11.1	1.5	B	0.14	10.4	3.8
	EBT	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	-	-	-
	SBL	C	0.14	23.9	3.8	C	0.11	23.5	3.0
	SBR	B	0.24	14.8	6.8	B	0.16	12.1	4.5
	Overall	A	-	1.6	-	A	-	1.3	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

The network intersection operations for the 2025 future total horizon operate similarly to the 2025 future background conditions. As in the existing conditions, the northbound left-turn movement at the intersection of Bank Street and Hunt Club Road is forecasted to be over theoretical capacity, and the eastbound through movement at the intersection of Albion Road at Bank Street may exhibit extended queues, each during the PM peak hour at this horizon.

It is noted that the eastbound left-turn lane queue from Bank Street onto Sieveright Avenue is contained in the existing left-turn lane and will not extend back to the proposed site access.

14.2.2 2030 Future Total Network Intersection Operations

The 2030 future total network intersection operations are summarized below in Table 22. The level of service for signalized intersections is based on HCM 2010 v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix L.

Table 22: 2030 Future Total Network Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road <i>Signalized</i>	EBL	A	0.48	58.3	23.2	A	0.59	63.2	28.4
	EBT	B	0.62	35.6	98.0	F	1.01	73.2	#182.6
	EBR	A	0.37	5.3	17.8	A	0.57	12.9	45.1
	WBL	A	0.36	55.6	m14.1	A	0.51	75.8	m23.9
	WBT	E	0.97	84.9	#166.5	E	0.91	62.9	#127.0
	WBR	A	0.29	14.2	24.7	A	0.34	19.7	31.1
	NBL	C	0.71	61.6	51.6	F	1.03	114.7	m#69.8
	NBT	D	0.90	46.5	#170.2	A	0.50	37.3	m89.5
	NBR	A	0.03	0.1	0.0	A	0.10	3.9	m5.7
	SBL	A	0.46	63.4	18.1	D	0.81	72.7	#49.6
	SBT	A	0.36	37.2	45.4	E	0.98	62.6	#175.1
	SBR	A	0.26	1.1	0.0	A	0.39	9.3	25.1
Overall	E	0.93	48.3	-	F	1.01	58.1	-	
Albion Road at Hunt Club Road <i>Signalized</i>	EBL	A	0.18	2.0	m0.3	A	0.36	6.7	m4.8
	EBT	A	0.39	3.7	2.3	C	0.78	11.2	m26.0
	EBR	A	0.01	0.0	m0.0	A	0.03	0.1	m0.0
	WBL	A	0.40	8.5	21.4	D	0.88	51.6	#101.5
	WBT	A	0.51	14.1	87.7	A	0.51	18.4	96.3
	WBR	A	0.08	2.6	6.2	A	0.08	2.5	5.0
	NBL	A	0.03	61.3	m4.4	A	0.03	30.6	m3.9
	NBT/R	E	0.94	89.5	#108.6	E	0.92	58.8	#115.3
	SBL	C	0.72	102.9	#28.7	C	0.79	109.2	#31.7
	SBT/R	A	0.50	40.4	46.6	A	0.49	41.0	55.8
	Overall	B	0.61	23.0	-	E	0.91	25.4	-
Bank Street at Towngate Mall <i>Signalized</i>	EB	A	0.36	30.5	17.2	D	0.90	69.8	#107.6
	WB	A	0.07	25.9	5.2	A	0.05	8.2	3.9
	NBT/L	A	0.34	2.5	m23.8	A	0.30	6.6	17.4
	NBR	A	0.01	0.0	m0.0	A	0.02	0.2	m0.2
	SBT	A	0.21	3.8	25.7	B	0.62	5.5	m31.1
	SBR	A	0.04	1.7	0.9	A	0.15	0.7	m0.0
	Overall	A	0.34	3.8	-	B	0.69	13.0	-
Albion Road at Bank Street <i>Signalized</i>	EBL	A	0.21	66.0	13.7	A	0.49	74.1	m23.4
	EBT	A	0.28	20.5	57.0	C	0.71	16.0	#182.0
	EBR	A	0.12	5.4	8.6	A	0.33	1.6	m5.2
	WBL	A	0.31	60.5	17.2	A	0.46	62.4	27.0
	WBT	B	0.61	23.2	#184.9	A	0.43	24.9	88.2
	WBR	A	0.14	4.6	12.2	A	0.22	4.8	14.6
	NBL	E	0.92	69.3	68.7	C	0.72	48.3	38.6
	NBT/R	A	0.48	35.5	55.3	A	0.40	31.1	50.6
	SBL	B	0.62	60.5	31.3	C	0.76	49.1	m55.1
	SBT/R	A	0.54	49.4	36.0	C	0.79	44.8	m86.1
Overall	C	0.78	31.4	-	C	0.74	25.4	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)	LOS	V/C	Delay	Q (95 th)
Sieveright Avenue at Bank Street Signalized	EBL	B	0.07	11.4	1.5	B	0.14	10.4	3.8
	EBT	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	-	-	-
	SBL	D	0.15	25.8	3.8	C	0.11	24.0	3.0
	SBR	C	0.25	15.4	7.5	B	0.16	12.1	4.5
	Overall	A	-	1.7	-	A	-	1.3	-

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

The network intersection operations for the 2030 future total horizon operate similarly to the 2030 future background conditions. As in the existing and 2025 future total conditions, the northbound left-turn movement at the intersection of Bank Street and Hunt Club Road is forecasted to be over theoretical capacity, and as in the existing conditions, the overall intersection is forecasted to be over theoretical capacity, each during the PM peak hour at this horizon.

It is noted that the eastbound left-turn lane queue from Bank Street onto Sieveright Avenue is contained in the existing left-turn lane and will not extend back to the proposed site access.

Mitigation for the capacity issues at the intersection of Bank Street and Hunt Club Road during the PM peak hour may be partially achieved through signal timing optimization, and Table 23 summarizes these operations. The synchro worksheets have been provided in Appendix L.

Table 23: 2030 Future Total Optimized Intersection Operations

Intersection	Lane	PM Peak Hour			
		LOS	V/C	Delay	Q (95 th)
Bank Street at Hunt Club Road Signalized	EBL	D	0.90	103.7	#37.8
	EBT	E	0.97	62.4	#175.6
	EBR	A	0.55	10.6	39.3
	WBL	B	0.66	90.7	m#29.3
	WBT	D	0.82	61.4	114.2
	WBR	A	0.32	23.4	35.6
	NBL	E	0.95	97.7	m#68.5
	NBT	A	0.53	41.7	m90.0
	NBR	A	0.11	4.2	m5.7
	SBL	B	0.67	59.9	42.7
	SBT	E	1.00	69.4	#178.7
	SBR	A	0.40	11.7	29.3
Overall	F	1.01	57.2	-	

Notes: Saturation flow rate of 1800 veh/h/lane
PHF = 1.00

m = metered queue
= queue exceeds storage or mid-block length

Through signal timing changes, the v/c ratio of each individual movement at the intersection of Bank Street and Hunt Club Road may reduce to 1.00 or below, and the overall intersection v/c remains 1.01.

14.2.3 Network Intersection MMLOS

Table 24 summarizes the MMLOS analysis for the network intersections of Bank Street at Hunt Club Road, Albion Road at Hunt Club Road, Bank Street at the Towngate Mall access, and Albion Road at Bank Street. The existing and future conditions for the intersections will be the same and are considered in one row. The intersection analysis is based on the land use designation of Arterial Mainstreet for the Bank Street intersections, and for

general urban area for the intersection of Albion Road at Hunt Club Road. The MMLOS worksheets has been provided in Appendix J.

Table 24: Study Area Intersection MMLOS Analysis

Intersection	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target
Bank Street at Hunt Club Road	F	C	F	C	F	D	A	D	F	D
Albion Road at Hunt Club Road	F	C	F	D	F	C	-	-	E	D
Bank Street at Towngate Mall	F	C	F	C	-	-	-	-	B	D
Albion Road at Bank Street	F	C	F	C	-	-	-	-	D	D

The MMLOS targets will not be met for the pedestrian and bicycle LOS at all study area intersections. Auto and transit LOS will not be met at the intersections of Bank Street at Hunt Club Road and Albion Road at Hunt Club Road.

Meeting the pedestrian level of service targets would require a reduction of crossing distance to four lanes on all crossings at the Bank Street at Hunt Club Road intersection due the protected turns, and to three lanes on the crossings at all other intersections.

To meet bicycle level of service targets, the eastbound and westbound approaches at the Albion Road at Hunt Club intersection and all other study area intersection approaches with auxiliary right-turn lanes would require separated facilities and two-stage left turns or left-turn boxes.

Transit level of service targets require that the eastbound through and left movements and the westbound through movement at the Bank Street at Hunt Club Road intersection be reduced to 30 seconds or below, and that the southbound through/right movement at the Albion Road at Hunt Club Road intersection be reduced to 20 seconds or less.

14.2.4 Recommended Design Elements

No study area intersection design elements are proposed as part of this study.

15 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

Proposed Site and Screening

- The proposed site includes 4,232.8 m² of commercial space and 7,718.0 m² of office space
- Accesses to Bank Street is proposed via the retention of the existing access on the east side of the parcel and the addition of a new right-in/right-out access at the west side of the parcel
- The development is proposed to be completed as a single phase by 2025
- The Trip Generation, Location, and Safety triggers were met for the TIA Screening
- This report is in support of a site plan application

Existing Conditions

- Bank Street and Hunt Club Road are arterial roads, and Albion Road is a collector road in the study area

- Sidewalks are provided on both sides of Hunt Club Road, on both sides of Bank Street west of the site and on Albion Road south of Bank Street and are provided on one side of Albion Road north of Bank Street and on Bank Street sporadically east of the site
- Curbside bike lanes are on Albion Road south of Bank Street and on Hunt Club west of Bank Street, and Bank Street and Hunt Club Road are spine routes, Albion Road, Bridle Path Drive/Dazé Street/Cahill Drive, and D'Aoust Avenue are local routes
- The high volumes roadways have produced a high number of collisions at the study area intersections, primarily at the Albion Road at Bank Street intersection where the collisions are predominantly rear end collisions indicating that they are lower speed and a result of congestion, and turning movement and angle collisions which may be influenced by the right-turn channels
- Capacity constraints are noted at the Bank Street at Hunt Club intersection, especially during the PM peak hour, and high delays and extended queuing are noted at the Bank Street at Hunt Club and Albion Road at Hunt Club intersections, and extended queues are noted at the Albion Road at Bank Street intersection

Development Generated Travel Demand

- The proposed development is forecasted produce 189 two-way people trips during the AM peak hour and 344 two-way people trips during the PM peak hour
- Of the forecasted people trips, 120 two-way trips will be vehicle trips during the AM peak hour and 180 two-way trips will be vehicle trips during the PM peak hour based on a 75% AM and 70% PM auto modal share target
- Of the forecasted trips, 55% are anticipated to travel north, 10% each to travel east and south, and 25% to travel west

Background Conditions

- The background developments were explicitly included in the background conditions, along with a total background growth of 1.50% per annum along the mainline volumes on Bank Street
- The study area intersections at both horizons will operate similarly to the existing conditions
- High demand is forecasted on the outbound left-turn onto Bank Street and increased demand on the eastbound U-turn on Bank Street at the Sieveright Avenue intersection will be resultant from any potential on-site impacts from associated delays

Development Design

- The auto parking will be in surface lots surrounding the buildings and in an underground parking level, bike parking will be in surface racks for two of the buildings and will be in the underground parking facilities for the other two newly proposed buildings
- Pedestrian connections will be made from all buildings on the property to the facilities on Bank Street
- Area bus stops are all beyond 400 metres-walk from the site entrances

Parking

- Parking for 441 vehicles is proposed, 92 above ground and 349 below ground, and parking for 48 bicycles is proposed, 16 above ground and 32 below ground
- The minimum vehicle and parking rates from the zoning by-law are met by the proposed parking

Boundary Street Design

- The boundary street will not meet pedestrian LOS targets, due to high volumes and operating speeds on the arterial roadway and bicycle LOS due to mixed traffic conditions
- Bicycle LOS may or may not be met through the provision of a dedicated facility and should be considered within the City's review of the Bank Street corridor

Access Intersections Design

- One new access proposed onto Bank Street as right-in/right-out and one existing full-movements access is to be retained
- The accesses are assumed to be minor stop-controlled, with Bank Street operating as a free flow corridor
- No specific recommendations or design elements are required outside of typical site design
- The northbound left-turn movement at the existing east site access is forecasted to experience moderate delays during the PM peak hour

TDM

- Supportive TDM measures to be included within the proposed development should include:
 - Display local area walking, cycling, and transit maps and area route schedules at entrances
 - Provide a multimodal travel option information package to new employees as well as online links to transit information
 - Provide a dedicated ridematching portal
 - Charge for long-term parking and unbundle parking cost from lease rates

Transit

- The forecasted site-generated transit trips are eight AM and 13 PM peak hour two-way riders
- Transit service is available within 600-950-metres-walk of the site and the provision of transit along Bank Street would support the arterial mainstreet
- No specific transit priority measures were considered as part of this development

Network Intersection Design

- Generally, the network intersections at the future total horizons will operate similarly to background conditions, notably with capacity issues during the PM peak hour at the intersection of Bank Street and Hunt Club Road, as in the existing conditions
- The MMLOS targets will not be met for the pedestrian and bicycle LOS at all network intersections, and for the transit and Auto LOS at the Hunt Club Road intersections
- Pedestrian LOS targets would require shorter crossing distances, and bicycle LOS targets would require separated facilities on the eastbound and westbound approaches at the Albion Road at Hunt Club intersection and where right-turn lanes are present throughout the study area, and two-stage left-turns and bike boxes on these same approaches
- Transit LOS targets would require transit approach delays to be reduced to 30 seconds at the Bank Street at Hunt Club Road intersection and to 20 seconds at the Albion Road at Hunt Club Road intersection

16 Next Steps

Following the circulation and review of the TIA, any outstanding comments will be documents within the context of the site plan application in the Step 4 Strategy Report. Once remaining TIA Steps are completed and sign-off has been received from City Transportation Project Manager, a signed and stamped final report will be provided to City staff.

DRAFT

Appendix A

TIA Screening Form and PM Certification Form

DRAFT

City of Ottawa 2017 TIA Guidelines
Step 1 - Screening Form

Date: 21-Jan-21
Project Number: 2021-010
Project Reference: 2582-2626 Bank Street

1.1 Description of Proposed Development	
Municipal Address	2582, 2600, 2626 (back parcel) Bank Street
Description of Location	Existing Auto Centre and Rental
Land Use Classification	Arterial Mainstreet (AM) for 2582 and 2600 Bank Street Residential Third Zone (R3Y) for 2626 Bank Street
Development Size	15,000 sq. m. of commercial/retail, existing building on 2600 Bank Street to remain
Accesses	Existing access for 2600 Bank Street, new right-in/right-out access on 2582 Bank Street
Phase of Development	Phased construction
Buildout Year	2025
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger		
Land Use Type	Destination retail	
Development Size	15000	G.F.A.
Trip Generation Trigger	Yes	

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	Yes
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?	Yes
Location Trigger	Yes

1.4. Safety Triggers		
Are posted speed limits on a boundary street 80 km/hr or greater?	No	
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	Yes	
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	No	
Is the proposed driveway within auxiliary lanes of an intersection?	Yes	
Does the proposed driveway make use of an existing median break that serves an existing site?	No	
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	Yes	Collisions noted along Bank Street
Does the development include a drive-thru facility?	No	
Safety Trigger	Yes	



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check appropriate field(s)] is either transportation engineering or transportation planning .

1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.


City Of Ottawa
Infrastructure Services and Community
Sustainability
Planning and Growth Management
110 Laurier Avenue West, 4th fl.
Ottawa, ON K1P 1J1
Tel. : 613-580-2424
Fax: 613-560-6006

Ville d'Ottawa
Services d'infrastructure et Viabilité des
collectivités
Urbanisme et Gestion de la croissance
110, avenue Laurier Ouest
Ottawa (Ontario) K1P 1J1
Tél. : 613-580-2424
Télécopieur: 613-560-6006

Dated at Ottawa this 20 day of September, 2018.
(City)

Name: Andrew Harte
(Please Print)

Professional Title: Professional Engineer



Signature of Individual certifier that s/he meets the above four criteria

Office Contact Information (Please Print)
Address: 13 Markham Avenue
City / Postal Code: Ottawa / K2G 3Z1
Telephone / Extension: (613) 697-3797
E-Mail Address: Andrew.Harte@CGHTransportation.com



Appendix B

Turning Movement Counts

DRAFT



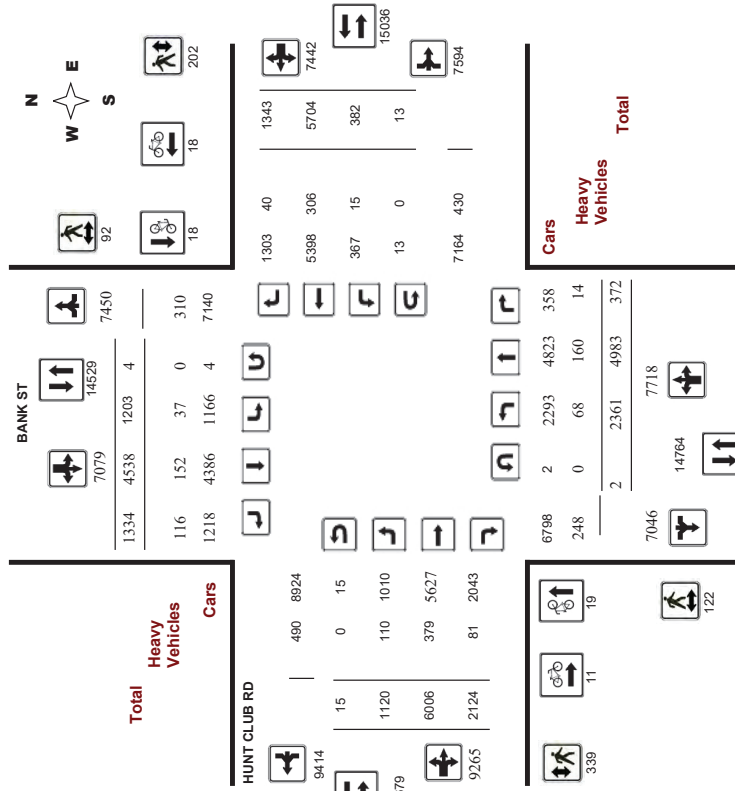
Transportation Services - Traffic Services
Turning Movement Count - Study Results

BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

Full Study Diagram



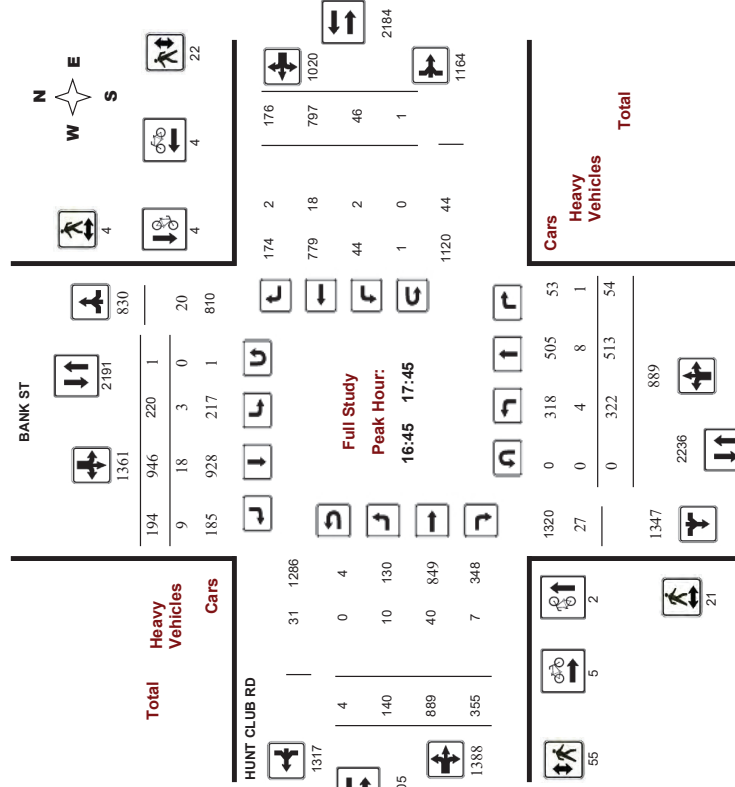
Transportation Services - Traffic Services
Turning Movement Count - Study Results

BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

Full Study Peak Hour Diagram





Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

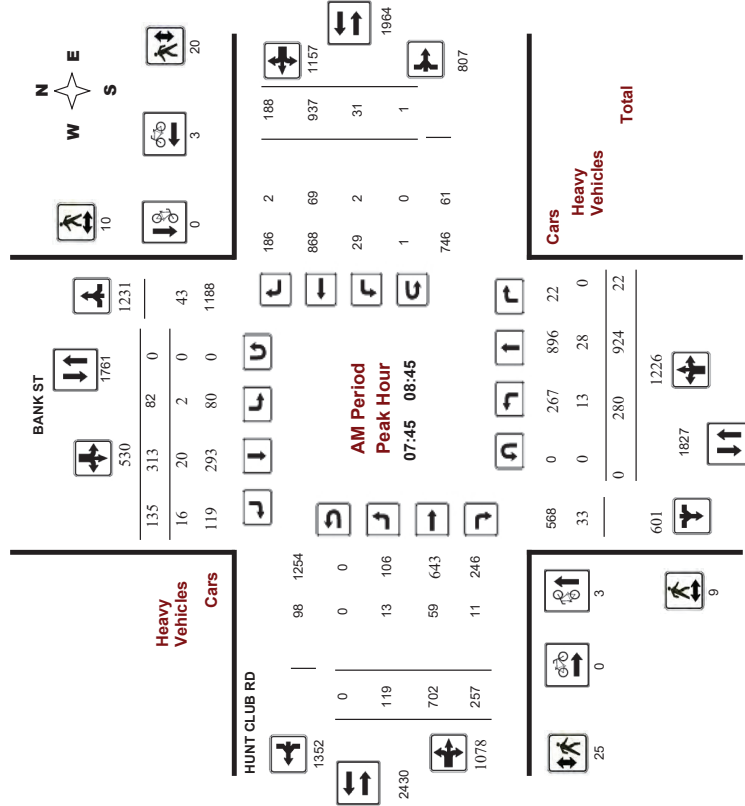
BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019

WO No: 38656

Start Time: 07:00

Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

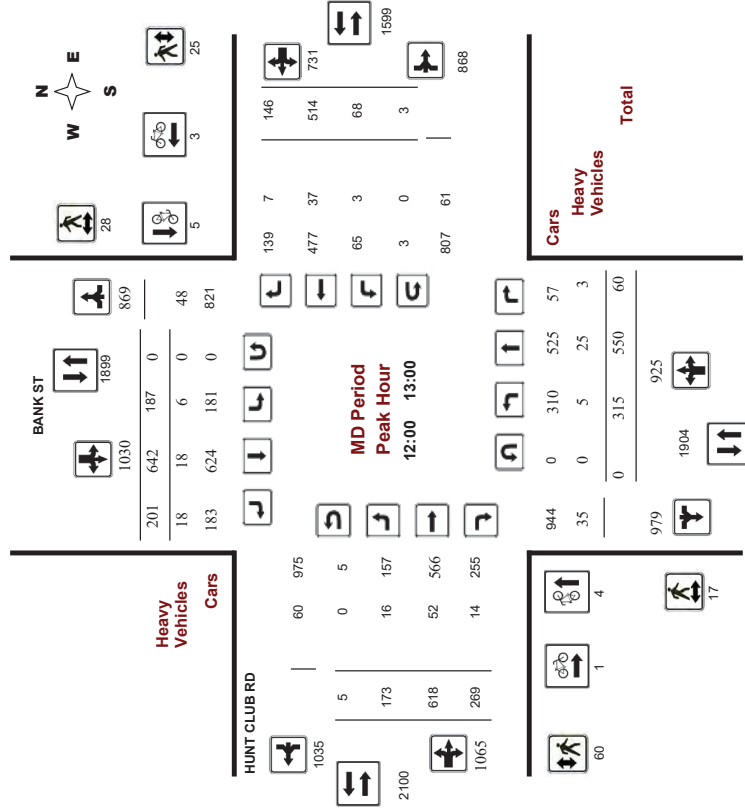
BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019

WO No: 38656

Start Time: 07:00

Device: Miovision



Comments



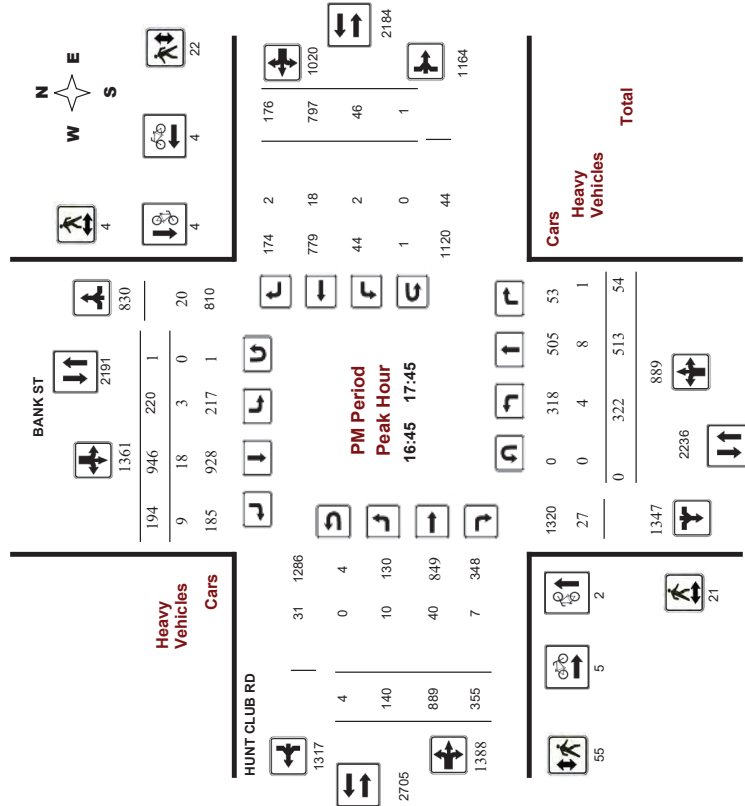
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, June 12, 2019
Total Observed U-Turns: 90
Northbound: 2
Southbound: 4
Eastbound: 15
Westbound: 13

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	NB	LT	ST	RT	TOT	SB	LT	ST	RT	TOT	EB	LT			
07:00-08:00	236	885	11	1132	54	281	114	449	1581	119	647	248	1014	17	935	183	1135	2149	3730
08:00-09:00	292	852	27	1171	72	318	135	525	1696	126	748	257	1131	35	881	190	1106	2237	3933
09:00-10:00	272	571	36	879	94	364	143	601	1480	143	602	192	937	62	672	183	917	1854	3334
11:30-12:30	316	536	56	908	145	571	192	908	1816	180	591	276	1047	58	581	151	790	1837	3653
12:30-13:30	286	567	55	918	175	585	185	955	1873	156	611	251	1018	56	567	143	766	1784	3657
15:00-16:00	348	581	63	992	211	547	175	933	1925	121	920	234	1275	58	494	157	709	1984	3909
16:00-17:00	291	502	65	858	240	945	175	1360	2218	122	955	312	1429	55	773	158	986	2415	4633
17:00-18:00	310	489	59	858	212	917	215	1344	2202	153	892	354	1399	41	801	178	1020	2419	4621
Sub Total	2361	4983	372	7716	1203	4538	1334	7075	14791	1120	6006	2124	9250	382	5704	1343	7429	16679	31470
U-Turns	2	4	2	8	4	4	6	14	30	15	13	13	41	13	13	13	28	28	34
Total	2363	4983	372	7718	1207	4538	1334	7079	14797	1135	6006	2124	9265	395	5704	1343	7442	16707	31504
EQ 12hr	3285	6926	517	10728	1678	6308	1854	9840	20588	1578	8348	2952	12878	549	7929	1867	10345	23223	43791
Note: These values are calculated by multiplying the totals by the appropriate expansion factor: 1.39																			
AVG 12hr	2956	6233	465	9654	1510	5677	1689	8856	18510	1420	7513	2657	11590	494	7136	1680	9310	20900	38410
Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor: .90																			
AVG 24hr	3872	8165	609	12646	1978	7437	2186	11601	24247	1860	9842	3481	15183	647	9348	2201	12186	27379	51626
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor: 1.31																			
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.																			



Transportation Services - Traffic Services
Turning Movement Count - Study Results
BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

Full Study Pedestrian Volume
HUNT CLUB RD

BANK ST

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)	Total	Grand Total
	E or W	W or E	N or S	S or N			
07:00	0	2	3	1	1	4	6
07:15	0	0	4	3	3	7	7
07:30	0	3	3	3	3	6	9
07:45	0	1	5	3	3	8	9
08:00	1	2	11	5	16	16	19
08:15	2	4	4	5	9	9	15
08:30	6	3	5	7	7	12	21
08:45	1	3	4	3	3	8	12
09:00	2	2	9	6	6	15	19
09:15	2	4	10	6	6	16	20
09:30	3	1	7	2	2	9	13
09:45	0	1	11	9	9	20	21
11:30	7	2	17	2	2	19	28
11:45	7	6	16	6	6	22	35
12:00	8	2	17	6	23	23	43
12:15	4	5	9	3	9	15	24
12:30	2	3	5	8	28	28	33
12:45	3	8	11	8	19	19	30
13:00	5	3	8	3	11	11	19
13:15	5	5	17	15	32	32	42
15:00	2	1	23	16	39	39	42
15:15	3	7	14	9	23	23	33
15:30	11	4	3	5	8	8	23
15:45	4	0	0	6	6	6	10
16:00	7	7	17	15	32	32	46
16:15	5	0	5	8	21	21	26
16:30	11	1	12	10	23	23	35
16:45	9	0	9	7	22	22	31
17:00	6	1	7	4	16	16	23
17:15	5	2	12	9	26	26	33
17:30	1	1	11	2	13	13	15
17:45	0	0	6	7	13	13	13
Total	122	92	339	202	541	541	755



Transportation Services - Traffic Services
Turning Movement Count - Study Results
BANK ST @ HUNT CLUB RD

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

Full Study Heavy Vehicles
HUNT CLUB RD

BANK ST

Time Period	Northbound			Southbound			Eastbound			Westbound			W STR TOT	STR TOT	Grand Total			
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT						
07:00	3	7	0	10	1	3	7	11	21	3	6	8	17	4	21	38	59	
07:15	3	8	0	11	1	8	2	11	22	0	5	3	8	0	14	2	46	
07:30	3	6	0	9	0	6	3	9	18	5	10	4	19	1	10	2	32	
07:45	2	3	0	5	0	9	2	11	16	1	11	1	13	0	17	1	47	
08:00	3	9	0	12	0	6	5	11	23	2	9	4	15	0	22	0	60	
08:15	5	4	0	9	1	3	4	8	17	6	20	1	18	0	19	4	63	
08:30	3	12	0	15	1	2	5	8	23	4	19	5	28	1	12	1	65	
08:45	6	5	1	12	2	7	1	10	22	3	15	1	19	0	9	4	54	
09:00	2	9	0	11	1	5	5	11	22	4	14	1	19	0	7	3	51	
09:15	4	9	2	15	1	7	6	14	29	2	16	2	20	2	17	0	68	
09:30	3	8	0	11	2	5	4	11	22	5	11	1	17	0	13	2	54	
09:45	2	10	0	12	1	2	6	9	21	4	14	2	20	2	7	1	51	
11:30	2	3	1	6	2	10	2	14	20	2	16	0	18	1	7	2	48	
11:45	3	4	0	7	1	8	6	15	22	4	13	2	19	1	15	0	57	
12:00	6	6	0	6	1	5	4	10	16	3	9	7	19	1	11	1	48	
12:15	5	8	2	12	2	3	7	12	24	3	15	2	20	0	9	3	56	
12:30	2	6	1	9	2	7	3	12	21	5	12	2	19	2	10	1	53	
12:45	2	6	1	9	2	7	3	12	21	5	12	2	19	2	10	1	53	
13:00	4	3	0	4	1	4	3	8	12	6	12	3	16	0	8	2	43	
13:15	3	3	0	7	1	6	3	10	17	1	12	3	16	0	17	0	50	
15:00	0	2	0	2	1	1	4	6	8	5	14	1	20	0	10	1	39	
15:15	1	4	0	5	1	4	4	9	14	7	6	2	15	0	10	0	39	
15:30	1	4	0	6	1	6	4	11	17	6	11	5	22	0	7	0	46	
15:45	5	3	2	10	4	0	4	8	18	0	13	0	13	0	2	15	33	
16:00	4	1	5	2	4	3	9	14	5	9	4	18	0	6	1	7	39	
16:15	6	0	6	3	2	1	6	12	4	12	2	18	1	3	0	4	34	
16:30	2	2	6	0	7	1	8	14	1	11	3	15	0	1	2	3	32	
16:45	1	2	0	3	2	7	3	12	15	2	14	4	20	2	4	2	43	
17:00	2	2	0	4	1	2	3	6	10	5	12	0	17	0	6	0	33	
17:15	1	2	1	4	0	2	1	3	7	1	9	1	11	0	4	0	22	
17:30	0	2	0	2	0	7	2	9	11	2	5	2	9	0	4	1	24	
17:45	0	0	0	0	0	1	4	5	5	4	8	2	14	0	1	5	24	
Total	68	160	14	242	37	152	116	305	547	110	379	81	570	15	306	40	931	1,478

Survey Date: Wednesday, June 12, 2019
Start Time: 07:00

WO No: 38656
Device: Miovision

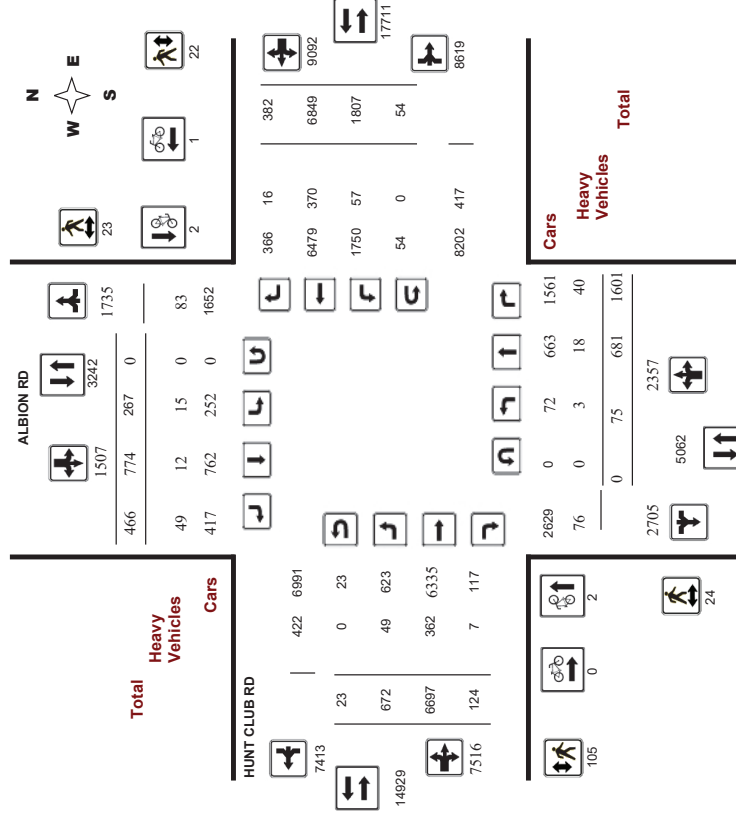
Full Study 15 Minute U-Turn Total
BANK ST

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	1	1	1
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	1	0	0	0	0	0	1
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	1	1	1
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	0	0	1	0	0	0	1	2	2
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	2	0	0	2	2
12:15	0	0	0	0	0	0	2	2	2
12:30	0	0	0	0	1	0	1	2	2
12:45	0	0	0	0	2	0	0	2	2
13:00	0	0	0	0	1	0	0	1	1
13:15	0	0	0	0	2	0	1	3	3
13:30	1	0	0	0	0	0	1	2	2
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	1	1	1
15:45	0	0	0	0	0	0	0	0	0
16:00	1	1	1	1	1	1	1	4	4
16:15	0	0	0	0	1	1	1	2	2
16:30	0	0	0	0	1	1	1	2	2
16:45	0	0	0	0	2	0	0	2	2
17:00	0	0	1	0	0	0	1	2	2
17:15	0	0	1	0	1	0	0	1	1
17:30	0	0	0	0	1	0	0	1	1
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	2	4	4	15	13	13	34	34	34

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37697
Device: Miovision

Full Study Diagram





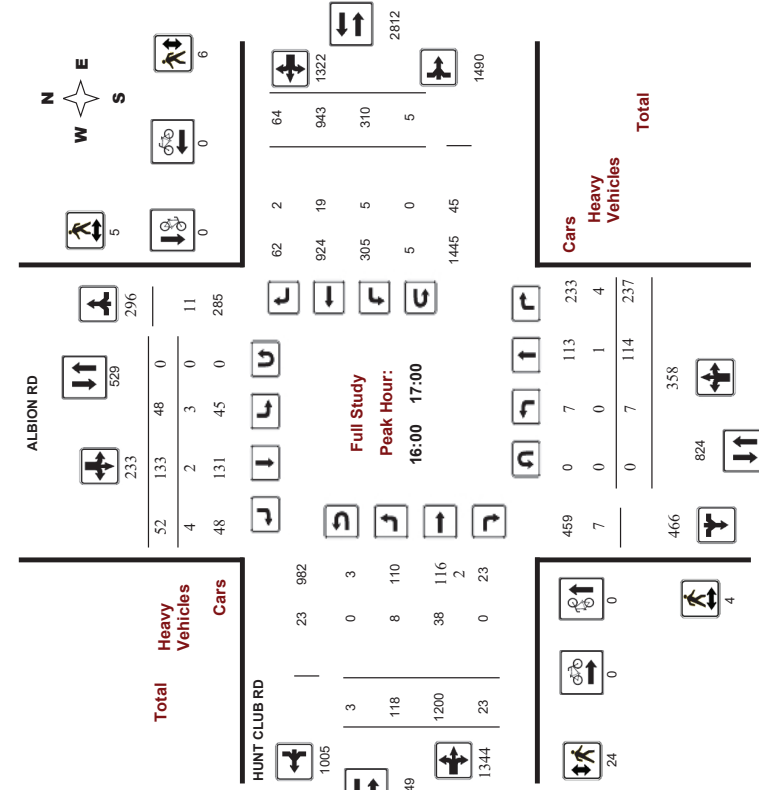
Transportation Services - Traffic Services
Turning Movement Count - Study Results

ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
 Start Time: 07:00

WO No: 37697
 Device: Miovision

Full Study Peak Hour Diagram



Comments

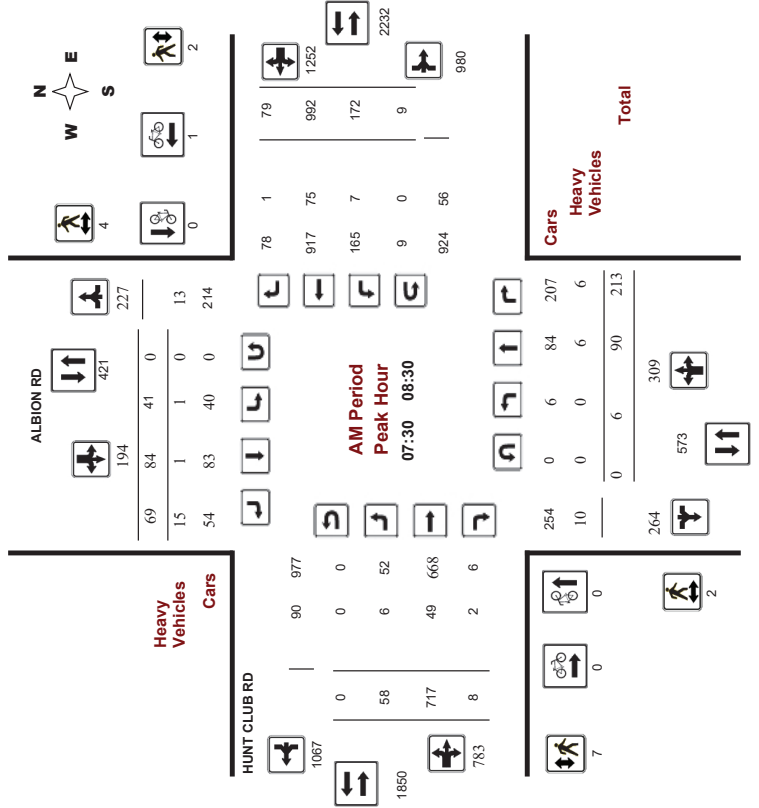


Transportation Services - Traffic Services
Turning Movement Count - Peak Hour Diagram

ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
 Start Time: 07:00

WO No: 37697
 Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

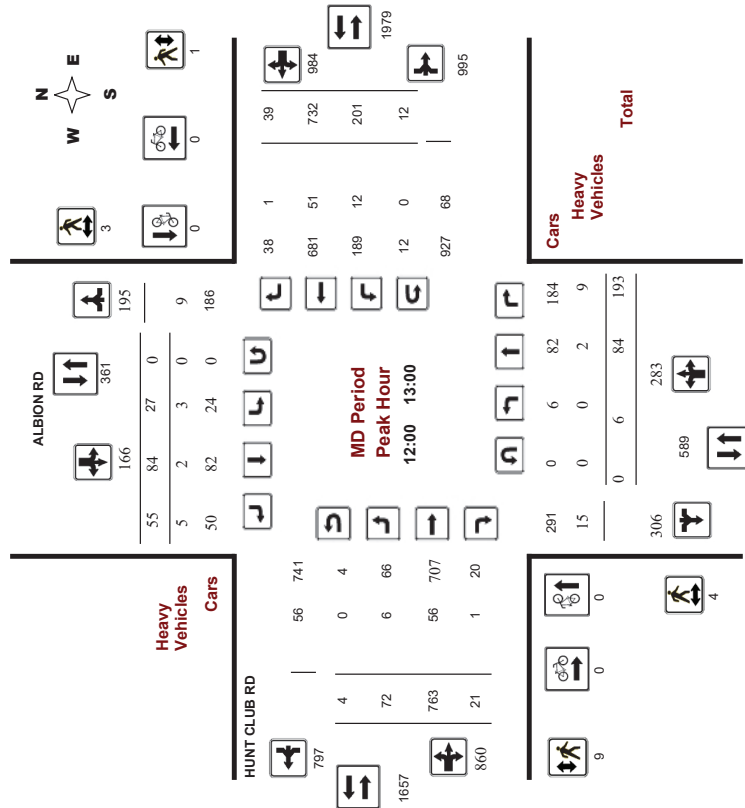
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018

WO No: 37697

Start Time: 07:00

Device: MiVision



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

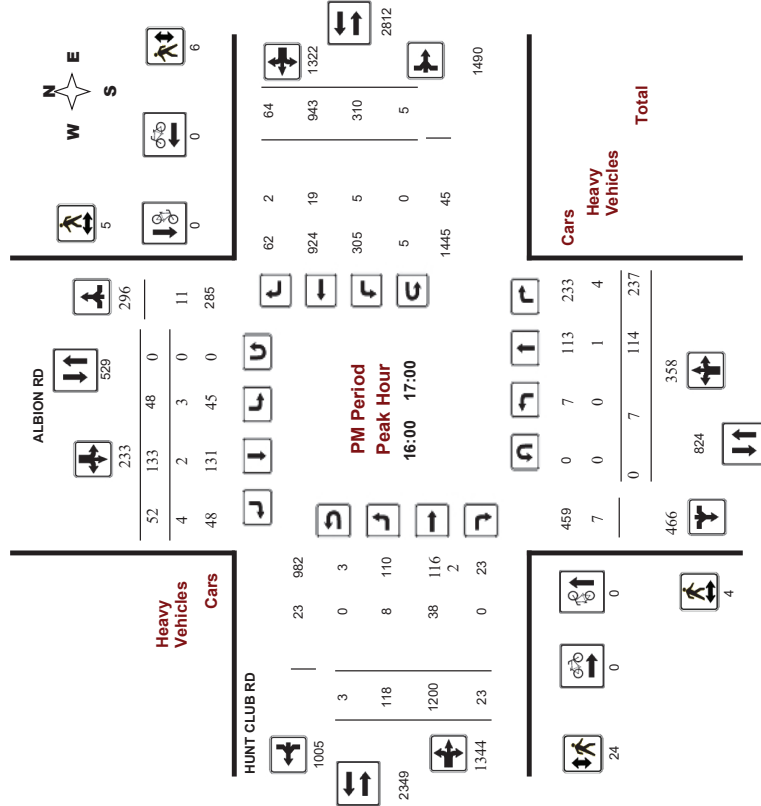
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018

WO No: 37697

Start Time: 07:00

Device: MiVision





Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018 **WO No:** 37697
Start Time: 07:00 **Device:** Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, April 05, 2018 **Total Observed U-Turns** **AAADT Factor**
 Northbound: 0 Southbound: 0 Eastbound: 23 Westbound: 54 90

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00 08:00	6	76	174	256	29	68	69	166	422	50	635	11	686	155	1007	66	1228	2346	
08:00 09:00	6	84	211	301	43	89	61	193	494	63	734	7	804	189	949	60	1188	2496	
09:00 10:00	2	73	139	214	23	79	56	158	372	50	599	8	657	184	840	34	1058	2087	
11:30 12:30	10	79	186	275	26	87	63	176	451	77	659	16	752	192	734	37	963	2166	
12:30 13:30	10	74	199	283	24	95	57	176	459	85	779	25	889	204	685	30	919	1808	
15:00 16:00	21	87	221	329	33	131	57	221	550	106	1045	15	1166	288	855	46	1189	2355	
16:00 17:00	7	114	237	358	48	133	52	233	591	118	1200	23	1341	310	943	64	1317	2658	
17:00 18:00	13	94	234	341	41	92	51	184	525	123	1046	19	1188	285	836	45	1166	2354	
Sub Total	75	681	1601	2357	267	774	466	1507	3864	672	6697	124	7483	1807	6849	382	9038	16331	
U-Turns	0	0	0	0	0	23	54	54	54	54	54	54	54	54	54	54	54	77	
Total	75	681	1601	2357	267	774	466	1507	3864	695	6697	124	7516	1861	6849	382	9092	16608	
EQ 12hr	104	947	2225	3276	371	1076	648	2095	5371	966	9309	172	10447	2587	9520	531	12638	23456	
Note: These values are calculated by multiplying the totals by the appropriate expansion factor.																			
AVG 12hr	94	852	2002	2948	334	968	583	1885	4833	869	8378	155	9402	2328	8568	478	11374	20776	
Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.																			
AVG 24hr	123	1116	2623	3862	438	1268	764	2470	6332	1138	10975	203	12316	3050	11224	626	14900	27216	
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.																			
Note: U-Turns provided for approach totals. Refer to "U-Turn" Report for specific breakdown.																			



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018 **WO No:** 37697
Start Time: 07:00 **Device:** Miovision

Full Study 15 Minute Increments

Survey Date: Thursday, April 05, 2018 **Total Observed U-Turns** **AAADT Factor**
 Northbound: 0 Southbound: 0 Eastbound: 23 Westbound: 54 90

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00 07:15	1	12	28	42	5	18	15	38	80	9	139	3	151	36	259	7	302	453	
07:15 07:30	1	20	43	64	9	13	14	36	100	10	146	4	160	34	240	10	284	444	
07:30 07:45	2	27	52	81	7	22	24	53	134	19	174	2	195	46	264	27	337	532	
07:45 08:00	2	17	50	69	8	15	16	39	108	13	176	2	191	45	244	22	311	502	
08:00 08:15	1	25	59	85	14	28	13	55	140	14	182	3	199	50	230	20	300	489	
08:15 08:30	1	21	52	74	12	19	16	47	121	12	185	1	198	40	254	10	304	502	
08:30 08:45	2	21	54	77	14	29	19	62	139	23	190	1	214	49	251	9	309	523	
08:45 09:00	2	17	46	65	3	13	13	29	94	14	177	2	193	59	214	21	284	487	
09:00 09:15	1	18	37	56	7	29	13	49	105	13	155	1	169	59	213	14	286	455	
09:15 09:30	0	19	34	53	4	13	13	30	83	11	152	1	164	43	225	7	275	439	
09:30 09:45	0	14	33	47	8	17	14	39	86	15	130	5	150	37	214	6	257	407	
09:45 10:00	1	22	35	58	4	20	16	40	98	12	162	1	175	46	188	7	241	416	
10:00 10:15	1	16	52	71	7	24	9	40	111	25	145	6	176	58	189	6	253	429	
10:15 10:30	4	18	37	59	5	27	22	54	113	20	146	4	170	45	177	8	236	400	
10:30 10:45	3	23	43	69	8	15	14	37	106	15	164	5	184	49	207	11	267	451	
10:45 11:00	2	22	54	76	6	21	18	45	121	20	204	1	225	51	161	12	224	449	
11:00 11:15	2	17	54	73	4	19	10	33	106	21	190	10	221	52	192	11	255	476	
11:15 11:30	1	22	42	65	9	29	13	51	116	20	205	5	230	61	172	5	238	468	
11:30 11:45	3	22	50	75	6	26	10	42	117	20	195	3	218	47	148	7	202	420	
11:45 12:00	4	13	53	70	5	21	24	50	120	20	199	7	226	52	173	7	232	458	
12:00 12:15	3	19	49	76	11	34	15	60	136	19	287	4	310	72	212	12	296	606	
12:15 12:30	8	19	49	76	11	34	15	60	136	19	287	4	310	72	212	12	296	606	
12:30 12:45	2	22	48	72	5	28	9	42	114	27	248	4	279	74	245	3	322	601	
12:45 13:00	1	24	69	97	8	42	20	70	167	37	247	1	285	72	186	19	277	562	
13:00 13:15	3	22	50	75	6	26	10	42	117	20	195	3	202	99	214	27	340	642	
13:15 13:30	4	13	53	70	5	21	24	50	120	20	199	7	226	52	173	7	232	458	
13:30 13:45	8	19	49	76	11	34	15	60	136	19	287	4	310	72	212	12	296	606	
13:45 14:00	7	22	55	84	9	27	13	49	133	28	263	6	297	77	212	12	301	598	
14:00 14:15	2	22	48	72	5	28	9	42	114	27	248	4	279	74	245	3	322	601	
14:15 14:30	4	24	69	97	8	42	20	70	167	37	247	1	285	72	186	19	277	562	
14:30 14:45	1	33	57	91	11	39	10	60	151	27	266	9	302	99	214	27	340	642	
14:45 15:00	5	32	53	90	21	32	8	61	151	24	307	5	336	76	236	12	324	660	
15:00 15:15	0	23	60	83	8	33	17	58	141	34	309	4	347	66	264	14	343	690	
15:15 15:30	1	26	67	94	8	29	17	54	148	36	318	5	359	75	229	11	315	674	
15:30 15:45	6	22	71	99	13	18	23	54	153	30	259	4	293	88	235	11	334	627	
15:45 16:00	3	31	55	89	13	35	13	61	150	40	286	3	329	76	196	6	278	607	
16:00 16:15	1	18	50	69	9	15	7	31	100	33	286	6	295	63	218	10	291	586	
16:15 16:30	3	23	58	84	6	24	8	38	122	24	245	6	275	65	187	18	270	548	
16:30 16:45	1	23	60	83	8	33	17	58	141	34	309	4	347	66	264	14	343	690	
16:45 17:00	1	26	67	94	8	29	17	54	148	36	318	5	359	75	229	11	315	674	
17:00 17:15	6	22	71	99	13	18	23	54	153	30	259	4	293	88	235	11	334	627	
17:15 17:30	3	31	55	89	13	35	13	61	150	40	286	3	329	76	196	6	278	607	
17:30 17:45	1	18	50	69	9	15	7	31	100	33	286	6	295	63	218	10	291	586	
17:45 18:00	3	23	58	84	6	24	8	38	122	24	245	6	275	65	187	18	270	548	
Total:	75	681	1601	2357	267	774	466	1507	3864	695	6697	124	7516	1861	6849	382	9092	16644	

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37697
Device: Miovision

Full Study Cyclist Volume

Time Period	ALBION RD		HUNT CLUB RD		Street Total	Grand Total
	Northbound	Southbound	Eastbound	Westbound		
07:00 07:15	0	0	0	0	0	0
07:15 07:30	1	0	0	0	1	1
07:30 07:45	0	0	0	1	1	1
07:45 08:00	0	0	0	0	0	0
08:00 08:15	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0
10:00 10:15	0	0	0	0	0	0
10:15 10:30	0	0	0	0	0	0
10:30 10:45	0	0	0	0	0	0
10:45 11:00	0	0	0	0	0	0
11:00 11:15	0	0	0	0	0	0
11:15 11:30	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0
13:15 13:30	0	1	0	0	1	1
13:30 13:45	0	0	0	0	0	0
13:45 14:00	0	0	0	0	0	0
14:00 14:15	0	0	0	0	0	0
14:15 14:30	0	0	0	0	0	0
14:30 14:45	0	0	0	0	0	0
14:45 15:00	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0
17:15 17:30	0	1	0	0	1	1
17:30 17:45	0	0	0	0	0	0
17:45 18:00	1	0	0	0	1	1
Total	2	2	4	1	9	5



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37697
Device: Miovision

Full Study Pedestrian Volume

Time Period	ALBION RD		HUNT CLUB RD		Total	Grand Total
	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)		
07:00 07:15	1	0	0	0	1	1
07:15 07:30	0	0	0	1	1	2
07:30 07:45	1	1	2	0	4	3
07:45 08:00	0	1	1	1	3	3
08:00 08:15	1	1	2	1	5	5
08:15 08:30	0	1	1	3	5	4
08:30 08:45	0	0	2	0	2	2
08:45 09:00	1	0	1	0	2	3
09:00 09:15	1	0	1	3	5	4
09:15 09:30	1	1	2	0	4	3
09:30 09:45	1	0	1	3	5	5
09:45 10:00	0	0	0	3	3	3
10:00 10:15	0	0	7	0	7	7
10:15 10:30	1	0	1	2	4	4
10:30 10:45	2	1	3	1	7	7
10:45 11:00	1	0	1	2	4	4
11:00 11:15	2	1	3	3	9	7
11:15 11:30	1	0	1	2	4	7
11:30 11:45	1	0	1	2	4	4
11:45 12:00	2	1	3	3	9	7
12:00 12:15	1	0	1	2	4	3
12:15 12:30	1	0	1	2	4	3
12:30 12:45	1	2	3	0	6	6
12:45 13:00	0	0	0	0	0	1
13:00 13:15	1	1	2	0	4	3
13:15 13:30	0	0	0	6	6	7
13:30 13:45	1	1	2	1	5	7
13:45 14:00	2	1	3	5	11	10
14:00 14:15	0	1	1	4	6	6
14:15 14:30	1	1	2	4	8	7
14:30 14:45	0	1	1	9	11	12
14:45 15:00	0	1	1	4	6	5
15:00 15:15	2	2	4	2	10	10
15:15 15:30	2	1	3	7	13	12
15:30 15:45	1	2	3	6	12	10
15:45 16:00	0	0	0	1	1	4
16:00 16:15	0	0	0	4	4	4
16:15 16:30	0	1	1	2	4	4
16:30 16:45	2	2	4	2	10	10
16:45 17:00	2	1	3	7	13	12
17:00 17:15	1	2	3	6	12	10
17:15 17:30	1	0	1	2	4	4
17:30 17:45	0	0	0	4	4	4
17:45 18:00	1	3	4	5	13	10
Total	24	23	47	105	127	174



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37697
Device: Miovision

Full Study Heavy Vehicles

Time Period	Northbound						Southbound						Eastbound						Westbound						Grand Total
	RT		ST		LT		RT		ST		LT		RT		ST		LT		RT		ST		LT		
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	1	1	0	0	1	1	0	0	1	1	2	3	7	0	10	3	22	1	26	36	38		
07:15	0	1	2	3	0	0	1	4	0	5	0	5	2	17	1	20	25	29	2	24	39	42	29		
07:30	0	1	1	2	0	0	2	3	1	13	1	15	2	21	1	24	39	42	1	25	40	43	42		
07:45	0	3	0	3	0	1	5	6	9	0	10	0	17	0	17	27	36	36	0	17	27	36	36		
08:00	0	1	3	4	1	0	4	5	9	2	13	0	17	0	20	35	44	44	0	20	35	44	44		
08:15	0	2	2	4	0	0	4	4	8	3	13	1	17	2	20	39	47	47	0	20	39	47	47		
08:30	0	1	1	2	2	1	2	5	7	3	14	1	18	1	19	0	20	38	45	0	20	38	45		
08:45	0	1	2	0	1	1	3	0	17	0	17	2	16	0	18	35	38	38	0	18	35	38	38		
09:00	0	1	3	4	0	0	1	1	5	1	17	0	18	8	12	0	20	38	43	0	20	38	43		
09:15	0	0	1	0	1	1	2	3	0	15	0	15	2	10	0	12	27	30	0	12	27	30	30		
09:30	0	0	0	0	2	0	1	3	1	11	2	14	1	13	0	14	28	31	0	14	28	31	31		
09:45	0	0	0	0	0	0	1	1	1	0	15	0	16	0	18	33	34	34	0	18	33	34	34		
10:00	0	1	0	1	0	1	1	2	1	10	1	12	0	15	1	16	28	30	0	16	28	30	30		
10:15	0	0	1	1	0	2	3	4	1	8	0	9	2	14	0	16	25	29	0	16	25	29	29		
11:45	0	1	0	1	0	1	0	2	3	4	1	8	0	9	2	14	0	16	25	0	16	25	29		
12:00	0	1	0	1	0	1	0	1	0	1	0	1	3	10	0	13	24	26	0	13	24	26	26		
12:15	0	1	1	2	1	1	1	3	5	1	16	0	17	5	16	1	22	39	44	0	22	39	44		
12:30	0	0	4	4	0	0	1	1	5	1	12	1	14	1	12	0	13	27	32	0	13	27	32		
12:45	0	0	4	4	0	0	3	5	9	2	19	0	21	3	13	0	16	37	46	0	16	37	46		
13:00	0	0	2	3	1	2	0	3	6	1	10	0	11	0	8	0	18	19	25	0	18	19	25		
13:15	0	0	4	4	1	0	3	4	8	2	12	0	14	3	9	1	13	27	35	0	13	27	35		
13:30	0	0	2	2	0	0	2	2	4	4	18	0	22	1	9	2	12	34	38	0	12	34	38		
15:00	0	0	2	2	0	0	2	2	4	4	18	0	22	1	9	2	12	34	38	0	12	34	38		
15:15	0	2	1	3	1	0	2	3	6	3	13	0	16	3	20	36	42	42	0	20	36	42	42		
15:30	0	2	3	0	0	1	1	4	1	14	0	15	0	16	0	10	25	29	0	10	25	29	29		
15:45	0	0	0	0	1	3	4	4	2	7	0	9	2	6	1	9	18	22	0	9	18	22	22		
16:00	0	0	0	0	0	1	2	3	3	1	5	0	6	1	4	1	6	12	15	0	6	12	15		
16:15	0	0	1	1	2	0	0	2	3	2	11	0	13	1	3	0	4	17	20	0	4	17	20		
16:30	0	1	1	2	0	1	0	1	3	3	16	0	19	2	10	0	12	31	34	0	12	31	34		
16:45	0	0	2	2	1	0	2	3	5	2	6	0	8	1	2	1	4	12	17	0	4	12	17		
17:00	0	0	1	1	0	0	1	1	2	2	8	0	10	1	4	0	5	15	17	0	5	15	17		
17:15	0	1	1	2	0	1	0	1	3	1	9	0	10	0	5	0	5	15	18	0	5	15	18		
17:30	0	0	0	0	0	0	2	2	2	2	6	0	8	1	2	0	3	11	13	0	3	11	13		
17:45	0	0	0	0	0	0	0	0	0	1	3	0	4	1	3	1	5	9	9	0	5	9	9		
Total	3	18	40	61	15	12	49	76	137	49	362	7	418	57	370	16	443	861	998	0	443	861	998		



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ HUNT CLUB RD

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37697
Device: Miovision

Full Study 15 Minute U-Turn Total

Time Period	ALBION RD		Southbound		Eastbound		Westbound		Total
	U-Turn Total		U-Turn Total		U-Turn Total		U-Turn Total		
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	1	1	1	2
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0



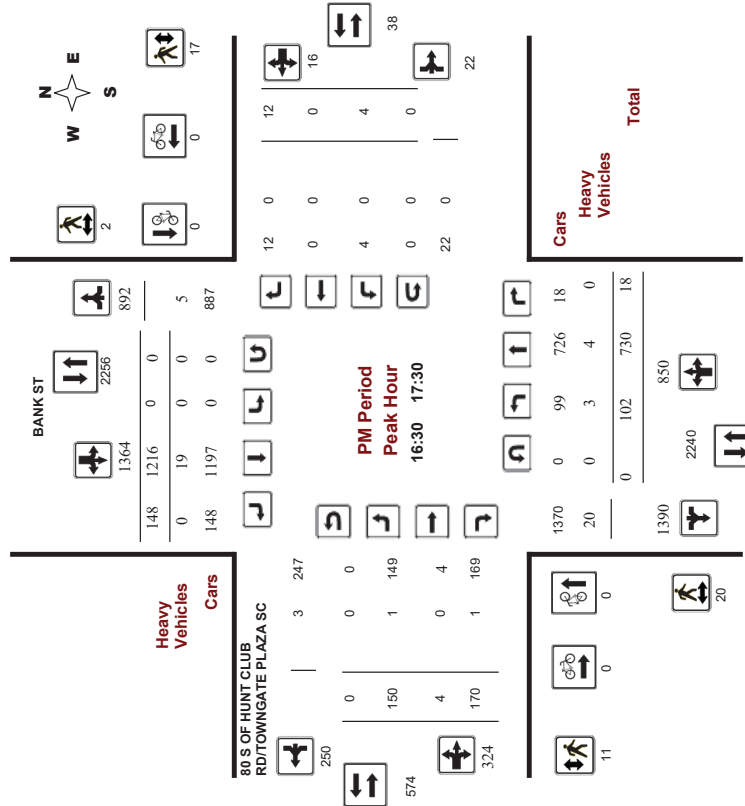
Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, April 05, 2018
Total Observed U-Turns: 90
Northbound: 5
Southbound: 1
Eastbound: 0
Westbound: 0

BANK ST

Period	Northbound			Southbound			Eastbound			Westbound			STR TOT	WB TOT	Grand Total				
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT							
07:00-08:00	40	1120	12	443	30	473	18	0	18	36	0	1	4	5	41	1686			
08:00-09:00	75	1021	15	1111	0	449	66	515	1626	54	2	51	107	3	0	9	1745		
09:00-10:00	102	732	6	840	1	555	115	672	1512	115	2	88	205	2	1	3	1723		
11:30-12:30	99	741	14	854	0	686	192	878	1732	173	1	172	346	2	1	11	14	360	2092
12:30-13:30	113	697	25	835	0	728	181	909	1744	207	0	161	368	0	0	10	10	378	2122
15:00-16:00	120	645	15	780	0	1041	136	1177	1957	174	0	196	370	5	1	4	10	380	2337
16:00-17:00	106	720	14	840	0	1177	142	1319	2159	154	3	168	325	2	0	13	15	340	2499
17:00-18:00	93	694	15	802	1	1178	140	1319	2121	147	2	168	317	2	0	8	10	327	2448
Sub Total	748	6370	116	7234	2	6258	1002	7262	14496	1042	10	1022	2074	16	4	62	82	2156	16652
U-Turns	5	5	1	1	6	0	0	0	0	0	0	0	0	0	0	0	0	0	6
Total	753	6370	116	7239	3	6258	1002	7263	14502	1042	10	1022	2074	16	4	62	82	2156	16658
EQ 12hr	1047	8854	161	10062	4	8639	1393	10096	20158	1448	14	1421	2883	22	6	86	114	2897	23155
Note: These values are calculated by multiplying the totals by the appropriate expansion factor.																			
AVG 12hr	942	7989	145	9056	4	7829	1254	9087	18443	1303	13	1279	2595	20	5	77	102	2897	20840
Note: These values are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.																			
AVG 24hr	1234	10439	190	11863	5	10256	1643	11904	23767	1707	17	1675	3399	26	7	101	134	3533	27300
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.																			
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.																			



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study 15 Minute Increments

80 S OF HUNT CLUB RD/TOWNGATE PLAZA SC

BANK ST

Time Period	Northbound				Southbound				Eastbound				Westbound				Grand Total		
	LT	ST	RT	TOT	N	LT	ST	RT	TOT	E	LT	ST	RT	TOT	W	ST		RT	TOT
07:00 07:15	11	239	0	250	0	80	5	85	335	4	0	4	8	0	0	0	0	8	343
07:15 07:30	5	282	4	291	0	103	7	110	401	2	0	3	5	0	0	2	2	7	408
07:30 07:45	6	305	6	317	0	116	6	122	439	6	0	6	12	0	0	2	2	14	483
07:45 08:00	18	294	2	314	0	144	12	156	470	6	0	5	11	0	1	0	1	12	482
08:00 08:15	9	311	4	324	0	114	13	127	451	12	0	7	19	0	0	2	2	21	472
08:15 08:30	17	259	1	277	0	108	9	117	394	7	1	16	24	2	0	4	6	30	424
08:30 08:45	26	254	4	284	0	105	20	125	409	17	0	15	32	1	0	0	1	33	442
08:45 09:00	23	197	6	226	0	122	24	146	372	18	1	13	32	0	0	3	3	35	407
09:00 09:15	27	217	2	246	0	141	24	165	411	31	1	13	45	0	1	2	47	458	
09:15 09:30	25	198	2	224	0	140	34	174	398	22	0	31	53	0	0	0	1	54	482
09:30 09:45	22	153	2	177	0	128	26	154	331	28	1	22	51	1	0	1	2	53	384
09:45 10:00	28	164	1	193	1	147	31	179	372	34	0	22	56	0	0	1	1	57	429
10:00 10:15	21	176	5	202	0	186	48	234	436	41	0	45	86	1	0	0	1	87	523
10:15 10:30	27	175	4	206	0	173	46	219	425	42	0	36	78	0	0	3	3	81	506
10:30 10:45	25	181	4	210	0	188	49	237	427	42	1	46	89	1	0	7	8	97	524
10:45 11:00	29	209	1	239	0	159	49	208	447	48	0	45	93	0	1	1	2	95	542
11:00 11:15	27	212	3	242	0	177	54	231	473	49	0	39	88	0	0	0	0	88	561
11:15 11:30	34	157	7	198	0	188	49	247	445	46	0	40	86	0	0	4	4	90	555
11:30 11:45	29	159	8	196	0	184	41	225	421	55	0	33	88	0	0	3	3	91	512
11:45 12:00	23	169	7	199	0	169	37	206	405	57	0	49	106	0	0	3	3	109	514
12:00 12:15	27	182	3	212	0	242	34	276	488	44	0	39	83	1	1	0	2	85	573
12:15 12:30	28	172	8	206	0	262	29	291	497	44	0	60	104	2	0	2	4	108	605
12:30 12:45	31	146	2	179	0	256	40	296	475	46	0	31	77	1	0	0	1	78	553
12:45 13:00	38	145	2	185	0	281	33	314	499	40	0	66	106	1	0	2	3	109	608
13:00 13:15	23	160	4	187	0	278	29	307	494	35	0	53	88	0	0	4	4	92	586
13:15 13:30	28	174	4	206	0	287	31	318	524	39	1	36	76	0	0	1	1	77	601
13:30 13:45	26	186	4	216	0	330	43	373	569	38	1	34	73	1	0	5	6	79	668
13:45 14:00	29	200	2	231	0	282	39	321	552	42	1	45	88	1	0	3	4	92	644
14:00 14:15	30	172	7	209	0	305	32	337	546	38	1	46	85	1	0	3	4	89	635
14:15 14:30	17	172	5	194	0	299	34	333	527	32	1	45	78	1	0	1	2	80	607
14:30 14:45	18	176	2	196	2	303	38	343	559	31	0	37	68	0	0	2	2	70	609
14:45 15:00	28	174	1	203	0	271	36	307	510	46	0	40	86	0	0	2	2	88	598
Total:	753	6370	116	7239	3	6258	1002	7263	14502	1042	10	1022	2074	16	4	62	82	14502	16,658

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study Cyclist Volume

80 S OF HUNT CLUB RD/TOWNGATE PLAZA SC

BANK ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	1	0	1	0	0	1	1
07:45 08:00	0	0	0	1	0	1	1
08:00 08:15	0	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
10:00 10:15	0	0	0	0	0	0	0
10:15 10:30	0	0	0	0	0	0	0
10:30 10:45	0	0	0	0	0	0	0
10:45 11:00	0	0	0	0	0	0	0
11:00 11:15	0	0	0	0	0	0	0
11:15 11:30	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	1	1	0	0	1	1
12:45 13:00	0	0	0	0	0	0	0
13:00 13:15	0	4	4	0	0	4	4
13:15 13:30	0	3	3	0	0	3	3
13:30 13:45	0	0	0	0	0	0	0
13:45 14:00	0	0	0	0	0	0	0
14:00 14:15	0	0	0	0	0	0	0
14:15 14:30	0	0	0	0	0	0	0
14:30 14:45	0	0	0	0	0	0	0
14:45 15:00	1	0	1	0	0	1	1
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	1	1	0	0	1	1
16:30 16:45	0	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	0	0	0	2	0	2	2
Total	2	9	11	3	0	3	14



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study Pedestrian Volume

BANK ST 80 S OF HUNT CLUB RD/TOWNGATE PLAZA SC

Time Period	NB Approach (E or W Crossing)		SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)		Total	Grand Total
	E or W	W or E	E or W	W or E	N or S	S or N	N or S	S or N		
07:00 07:15	2	0	0	0	0	0	1	1	1	3
07:15 07:30	0	0	0	0	0	0	0	0	0	0
07:30 07:45	2	0	0	0	0	0	3	3	3	5
07:45 08:00	1	0	0	4	1	4	1	5	6	6
08:00 08:15	3	1	0	2	1	2	3	3	7	7
08:15 08:30	3	0	0	3	3	3	6	6	9	9
08:30 08:45	9	0	5	8	5	8	13	13	22	22
08:45 09:00	3	0	3	3	3	3	4	4	7	7
09:00 09:15	2	0	0	2	0	2	2	2	4	4
09:15 09:30	3	1	0	3	3	3	3	3	7	7
09:30 09:45	8	0	4	3	4	3	7	7	15	15
09:45 10:00	4	1	0	4	1	4	4	4	9	9
10:00 10:15	6	0	0	3	3	3	6	6	12	12
10:15 10:30	10	0	0	7	7	7	8	8	18	18
10:30 10:45	7	0	0	4	4	4	5	5	12	12
10:45 11:00	6	0	0	6	0	0	6	6	12	12
11:00 11:15	4	0	0	4	0	0	4	4	8	8
11:15 11:30	4	0	0	3	3	3	4	4	8	8
11:30 11:45	14	0	0	14	3	0	3	3	17	17
11:45 12:00	8	0	0	8	4	4	8	8	16	16
12:00 12:15	10	0	0	10	7	5	12	12	22	22
12:15 12:30	8	0	0	8	1	2	3	3	11	11
12:30 12:45	10	0	0	10	1	1	2	2	12	12
12:45 13:00	8	0	0	8	0	0	3	3	11	11
13:00 13:15	8	0	0	8	0	0	3	3	11	11
13:15 13:30	10	0	0	10	0	0	2	2	12	12
13:30 13:45	8	0	0	8	0	0	1	1	9	9
13:45 14:00	10	0	0	10	0	0	3	3	13	13
14:00 14:15	6	0	0	6	0	0	2	2	8	8
14:15 14:30	6	0	0	6	0	0	3	3	9	9
14:30 14:45	4	0	0	4	0	0	4	4	8	8
14:45 15:00	4	0	0	4	0	0	5	5	9	9
15:00 15:15	4	0	0	4	0	0	8	8	12	12
15:15 15:30	6	0	0	6	0	0	9	9	15	15
15:30 15:45	4	0	0	4	0	0	5	5	9	9
15:45 16:00	6	0	0	6	0	0	6	6	12	12
16:00 16:15	4	0	0	4	0	0	5	5	9	9
16:15 16:30	4	0	0	4	0	0	4	4	8	8
16:30 16:45	4	0	0	4	0	0	5	5	9	9
16:45 17:00	7	0	0	7	0	0	7	7	14	14
17:00 17:15	6	0	0	6	0	0	6	6	12	12
17:15 17:30	3	0	0	3	0	0	4	4	7	7
17:30 17:45	1	0	0	1	0	0	4	4	5	5
17:45 18:00	12	0	0	12	0	0	4	4	16	16
Total	181	15	82	196	83	83	165	165	361	361



Transportation Services - Traffic Services

Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study Heavy Vehicles

BANK ST 80 S OF HUNT CLUB RD/TOWNGATE PLAZA SC

Time Period	Northbound			Southbound			Eastbound			Westbound			W TOT	STR TOT	Grand Total		
	LT	ST	RT	N TOT	LT	ST	RT	S TOT	LT	ST	RT	E TOT				LT	ST
07:00 07:15	0	7	0	7	0	4	0	4	11	2	0	0	0	0	0	2	13
07:15 07:30	2	9	0	11	0	8	1	9	20	0	0	0	0	0	0	0	20
07:30 07:45	0	6	0	6	0	7	0	7	13	1	0	2	3	0	0	3	16
07:45 08:00	0	6	0	6	0	2	2	4	10	1	0	0	0	0	0	1	11
08:00 08:15	0	5	0	5	0	11	1	12	17	0	0	0	0	0	0	0	17
08:15 08:30	0	9	0	9	0	5	2	7	16	0	0	1	1	0	0	1	18
08:30 08:45	1	5	0	6	0	3	3	6	12	1	0	1	2	0	0	2	14
08:45 09:00	0	8	0	8	0	4	0	4	12	1	0	0	1	0	0	0	13
09:00 09:15	1	4	0	5	0	8	0	8	13	2	0	0	2	0	0	2	15
09:15 09:30	1	4	0	5	0	8	2	10	15	0	0	0	0	0	0	0	15
09:30 09:45	0	5	0	5	0	5	0	5	10	2	0	1	3	0	0	3	13
09:45 10:00	1	3	0	4	0	9	1	10	14	1	0	0	1	0	0	1	15
10:00 10:15	0	8	0	8	0	3	11	19	1	0	0	1	0	0	0	1	20
10:15 10:30	1	6	0	7	0	9	0	9	16	2	0	1	3	0	0	3	19
10:30 10:45	0	1	0	1	0	9	3	12	13	0	0	1	1	0	0	1	14
10:45 11:00	0	5	0	5	0	6	4	12	21	2	0	0	2	0	0	2	23
11:00 11:15	0	5	0	5	0	6	0	6	11	1	0	2	3	0	0	3	14
11:15 11:30	2	3	0	5	0	6	4	10	15	1	0	1	2	0	0	2	17
11:30 11:45	0	8	0	8	0	6	3	9	17	2	0	0	2	0	0	2	19
11:45 12:00	0	8	0	8	0	3	0	3	11	0	0	0	0	0	0	0	11
12:00 12:15	0	2	0	2	0	4	1	5	7	1	0	0	1	0	0	1	8
12:15 12:30	0	4	0	4	0	3	0	3	7	1	0	1	2	0	0	2	9
12:30 12:45	0	1	0	1	0	6	0	6	7	0	0	0	0	0	0	0	5
12:45 13:00	0	2	0	2	0	7	0	7	9	0	0	0	0	0	0	0	7
13:00 13:15	0	3	0	3	0	3	1	4	7	0	0	1	1	0	0	1	8
13:15 13:30	0	0	0	0	0	5	0	5	6	0	0	0	0	0	0	0	6
13:30 13:45	0	0	0	0	0	5	0	5	5	0	0	0	0	0	0	0	5
13:45 14:00	2	2	0	4	0	3	0	3	7	1	0	0	1	0	0	1	8
14:00 14:15	1	0	0	1	0	6	0	6	8	0	0	1	1	0	0	1	9
14:15 14:30	0	1	0	1	0	1	0	1	2	1	0	0	1	0	0	1	3
14:30 14:45	1	1	0	2	0	1	0	1	2	1	0	0	1	0	0	1	3
14:45 15:00	1	1	0	2	0	1	0	1	3	0	0	0	0	0	0	0	3
15:00 15:15	14	136	0	150	0	178	31	209	359	24	0	13	37	1	0	1	387



Transportation Services - Traffic Services
Turning Movement Count - Study Results

BANK ST @ 80 S OF HUNT CLUB RD/TOWNGATE PLAZA

Survey Date: Thursday, April 05, 2018
Start Time: 07:00

WO No: 37698
Device: Miovision

Full Study 15 Minute U-Turn Total

BANK ST
80 S OF HUNT CLUB RD/TOWNGATE PLAZA SC Eastbound Westbound

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	1	0	0	0	0	0	0	0	1
11:45	1	0	0	0	0	0	0	0	1
12:00	1	0	0	0	0	0	0	0	1
12:15	1	0	0	0	0	0	0	0	1
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	1	0	0	0	0	0	0	0	1
15:45	1	0	0	0	0	0	0	0	1
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	1	0	0	0	0	0	0	1
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	5	1	0	0	0	0	0	0	6



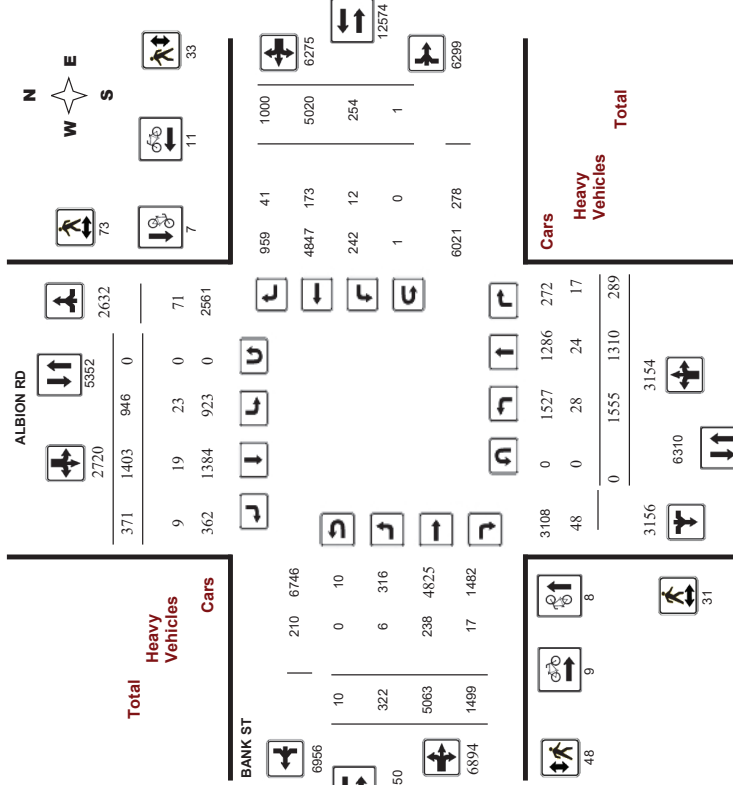
Transportation Services - Traffic Services
Turning Movement Count - Study Results

ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
Start Time: 07:00

WO No: 38667
Device: Miovision

Full Study Diagram





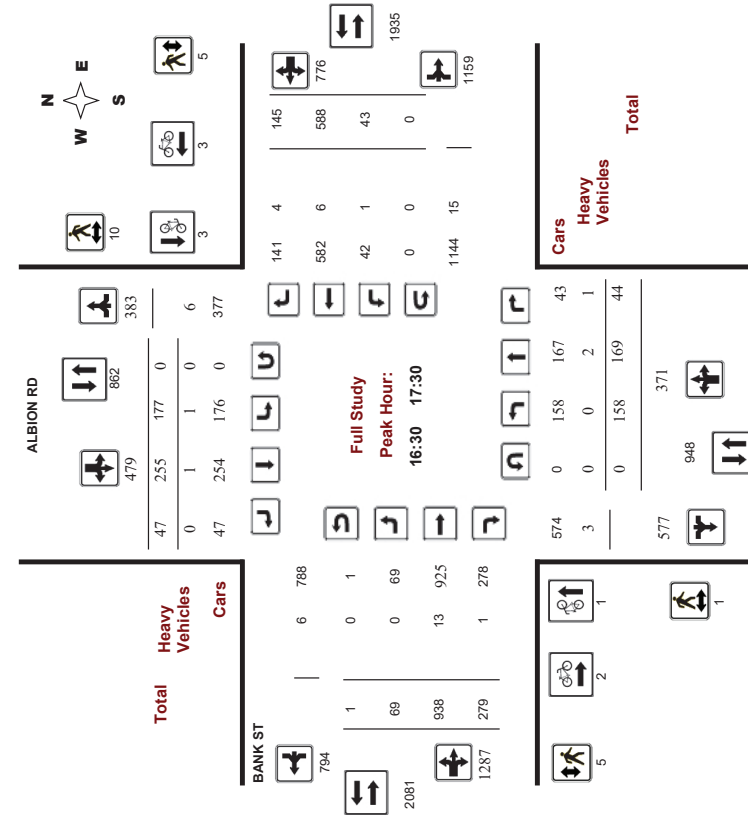
Transportation Services - Traffic Services
Turning Movement Count - Study Results

ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
 Start Time: 07:00

WO No: 38667
 Device: Miovision

Full Study Peak Hour Diagram



Comments

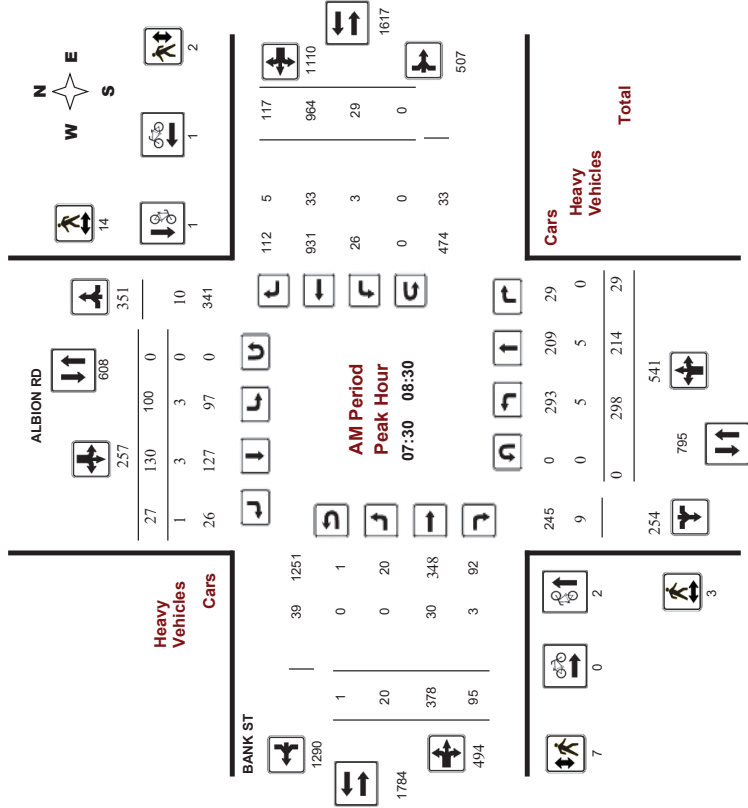


Transportation Services - Traffic Services
Turning Movement Count - Peak Hour Diagram

ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
 Start Time: 07:00

WO No: 38667
 Device: Miovision



Comments



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

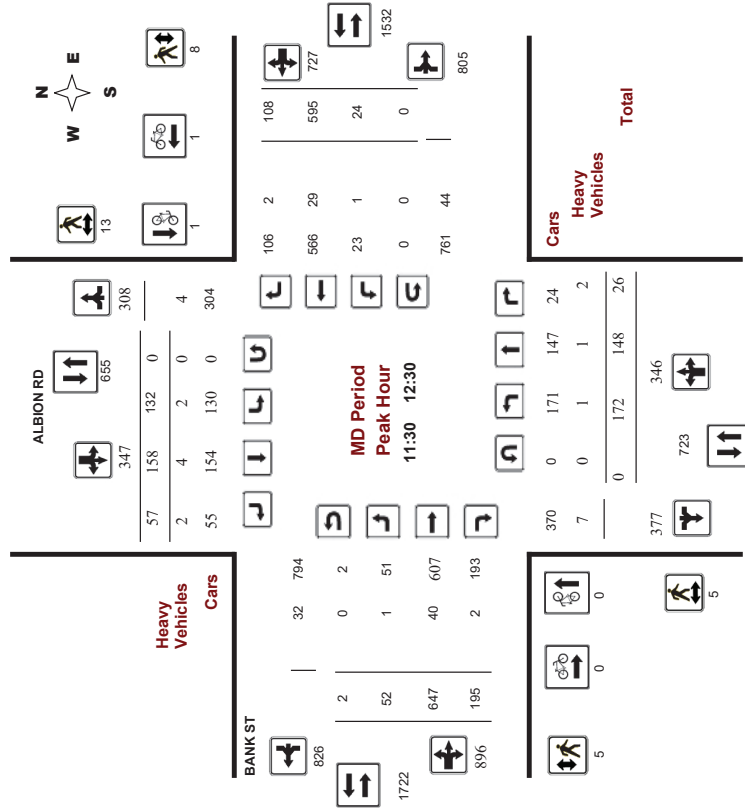
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019

WO No: 38667

Start Time: 07:00

Device: Miovision



Transportation Services - Traffic Services

Turning Movement Count - Peak Hour Diagram

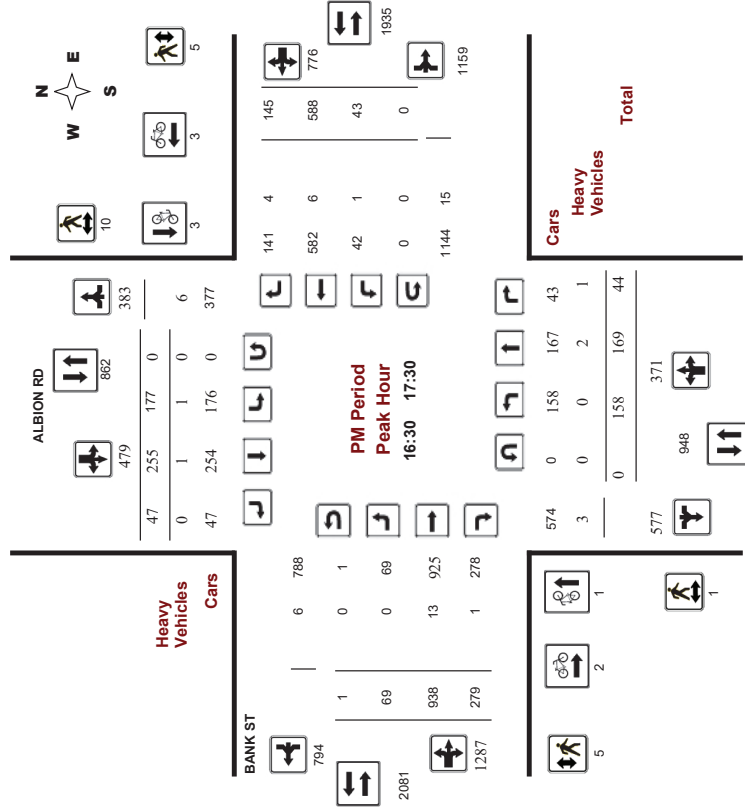
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019

WO No: 38667

Start Time: 07:00

Device: Miovision





Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019 **WO No:** 38667
Start Time: 07:00 **Device:** Miovision

Full Study Summary (8 HR Standard)

Survey Date: Thursday, June 20, 2019 **Total Observed U-Turns** **AAADT Factor**
Northbound: 0 Southbound: 0 90
Eastbound: 10 Westbound: 1

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	NB	LT	ST	RT	SB	STR	LT	ST	EB	EB	LT	ST			
07:00-08:00	259	189	27	475	69	120	24	213	688	14	380	81	475	22	883	97	1002	1477	2165
08:00-09:00	281	201	29	511	97	118	41	256	767	27	397	108	532	29	857	108	994	1526	2293
09:00-10:00	177	140	47	364	83	108	47	238	602	21	419	126	566	23	623	110	756	1322	1924
11:30-12:30	172	148	26	346	132	158	57	347	693	52	647	195	894	24	595	108	727	1621	2314
12:30-13:30	178	144	39	361	111	157	54	322	683	44	565	185	794	25	462	125	612	1406	2089
15:00-16:00	178	149	43	370	141	246	52	439	809	56	834	260	1150	41	495	141	677	1827	2636
16:00-17:00	159	173	38	370	150	233	49	432	802	54	931	258	1243	46	517	149	712	1955	2757
17:00-18:00	151	166	40	357	163	263	47	473	830	54	890	286	1230	44	588	162	794	2024	2854
Sub Total	1555	1310	289	3154	946	1403	371	2720	3874	322	5063	1499	6884	254	5020	1000	6274	13158	19032
U-Turns	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	11	11
Total	1555	1310	289	3154	946	1403	371	2720	3874	322	5063	1499	6884	254	5020	1000	6275	13169	19043
EQ 12hr	2161	1821	402	4384	1315	1950	516	3781	8165	461	7038	2084	9583	354	6976	1390	8722	18305	26470
Note: These values are calculated by multiplying the totals by the appropriate expansion factor.																			
AVG 12hr	1945	1639	362	3946	1184	1755	464	3403	7349	415	6334	1876	8625	319	6280	1251	7850	16475	23824
Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.																			
AVG 24hr	2548	2147	474	5169	1551	2299	608	4458	9627	544	8298	2458	11300	418	8227	1639	10284	21884	31211
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.																			
Note: U-Turns provided for approach totals. Refer to "U-Turn" Report for specific breakdown.																			



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019 **WO No:** 38667
Start Time: 07:00 **Device:** Miovision

Full Study 15 Minute Increments

Survey Date: Thursday, June 20, 2019 **Total Observed U-Turns** **AAADT Factor**
Northbound: 0 Southbound: 0 90
Eastbound: 10 Westbound: 1

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	N	LT	ST	RT	S	STR	LT	ST	E	EB	LT	ST			
07:00	45	32	6	83	12	27	7	46	129	3	91	19	113	5	216	9	230	343	472
07:15	74	46	7	127	12	28	5	45	172	2	100	21	123	2	201	25	228	351	523
07:30	59	50	4	113	14	26	7	47	160	6	100	18	124	9	223	35	267	391	551
07:45	81	61	10	152	31	39	5	75	227	4	89	23	116	6	243	28	271	393	620
08:00	84	44	9	137	25	31	7	63	200	3	101	22	126	5	256	28	289	415	615
08:15	74	59	6	139	30	34	8	72	211	8	88	32	128	9	242	26	277	405	616
08:30	50	46	9	105	14	23	11	48	153	7	107	18	132	10	192	31	233	365	518
08:45	52	52	5	130	28	30	15	73	203	9	101	38	146	5	167	23	195	341	544
09:00	73	52	5	130	28	30	15	73	203	9	101	38	146	5	167	23	195	341	544
09:15	49	40	9	98	18	36	15	69	167	5	99	34	138	5	169	10	184	322	489
09:30	43	29	12	84	23	29	11	63	147	7	104	30	141	5	133	36	174	315	462
09:45	37	37	13	87	25	20	12	57	141	5	122	36	163	7	129	25	161	324	482
10:00	49	40	5	94	35	46	18	99	193	18	151	53	222	6	146	27	179	401	594
10:15	41	31	5	77	27	37	14	78	155	12	152	40	204	5	148	23	176	380	535
10:30	45	36	8	89	38	34	15	87	176	12	170	48	230	9	148	34	189	419	595
10:45	41	42	13	96	26	42	17	85	183	8	155	49	212	7	130	39	176	388	571
11:00	47	41	12	100	28	47	11	86	186	12	139	43	194	7	114	36	157	351	537
11:15	34	6	8	48	29	34	16	79	168	9	135	51	195	7	99	29	135	330	498
11:30	39	27	8	74	28	34	10	72	146	15	136	42	193	5	119	21	145	338	484
11:45	46	33	11	90	34	57	10	101	191	10	172	62	244	8	142	38	188	432	623
12:00	49	38	11	98	33	71	15	119	217	11	176	59	246	8	88	26	122	368	585
12:15	45	40	5	90	34	59	9	102	192	19	253	77	349	12	133	32	177	526	718
12:30	38	38	16	92	40	59	18	117	209	18	233	62	313	13	132	45	190	503	712
12:45	31	42	6	79	35	46	9	90	169	5	252	54	311	13	119	34	166	477	646
13:00	33	45	12	90	28	73	17	118	208	12	214	66	292	11	123	48	182	474	682
13:15	50	43	11	104	38	62	7	107	211	20	244	62	326	11	148	37	196	522	733
13:30	45	43	9	97	49	52	16	117	214	17	221	76	314	11	127	30	168	482	696
13:45	33	36	17	86	41	70	11	122	208	16	233	75	324	10	150	40	200	524	732
14:00	30	47	7	84	49	71	13	133	217	17	240	66	323	11	163	38	212	535	752
14:15	37	38	9	84	35	64	13	112	196	11	241	69	321	13	127	41	181	502	698
14:30	51	45	7	103	38	58	10	106	209	13	176	76	285	10	148	43	201	466	675
Total:	1555	1310	289	3154	946	1403	371	2720	3874	322	5063	1499	6884	255	5020	1000	6275	19,043	27,574

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
Start Time: 07:00

WO No: 38667
Device: Miovision

Full Study Cyclist Volume

Time Period	ALBION RD		BANK ST		Street Total	Grand Total
	Northbound	Southbound	Eastbound	Westbound		
07:00 07:15	2	0	0	0	2	2
07:15 07:30	0	0	0	0	0	0
07:30 07:45	2	0	0	0	2	2
07:45 08:00	0	1	1	1	3	3
08:00 08:15	0	0	0	0	0	0
08:15 08:30	0	0	0	0	0	0
08:30 08:45	1	1	0	0	2	2
08:45 09:00	0	0	0	1	1	1
09:00 09:15	0	0	0	3	3	3
09:15 09:30	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0
09:45 10:00	0	0	3	0	3	3
10:00 10:15	0	0	0	0	0	0
10:15 10:30	0	1	1	0	2	2
10:30 10:45	0	0	0	0	0	0
10:45 11:00	0	0	0	0	0	0
11:00 11:15	0	0	0	0	0	0
11:15 11:30	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0
12:15 12:30	0	0	0	1	1	1
12:30 12:45	0	0	0	0	0	0
12:45 13:00	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0
13:30 13:45	0	0	0	0	0	0
13:45 14:00	0	0	0	0	0	0
14:00 14:15	0	0	0	0	0	0
14:15 14:30	0	0	0	0	0	0
14:30 14:45	1	0	1	0	2	2
14:45 15:00	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0
15:30 15:45	1	0	1	0	2	2
15:45 16:00	0	1	1	0	2	2
16:00 16:15	1	0	1	0	2	2
16:15 16:30	0	0	0	0	0	0
16:30 16:45	0	1	1	0	2	2
16:45 17:00	1	1	0	0	2	2
17:00 17:15	0	0	1	1	2	2
17:15 17:30	0	1	0	2	3	3
17:30 17:45	0	1	0	2	3	3
17:45 18:00	0	0	0	0	0	0
18:00 18:15	0	0	0	0	0	0
18:15 18:30	0	0	0	0	0	0
18:30 18:45	0	0	0	0	0	0
18:45 19:00	0	0	0	0	0	0
Total	8	7	15	9	20	35



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
Start Time: 07:00

WO No: 38667
Device: Miovision

Full Study Pedestrian Volume

Time Period	ALBION RD		BANK ST		Total	Grand Total
	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)		
07:00 07:15	0	2	1	0	3	3
07:15 07:30	0	0	0	0	0	0
07:30 07:45	0	1	1	1	3	3
07:45 08:00	0	2	2	1	5	5
08:00 08:15	2	1	0	1	4	4
08:15 08:30	1	10	3	0	14	14
08:30 08:45	1	2	3	1	7	7
08:45 09:00	5	0	3	1	9	9
09:00 09:15	1	1	2	0	4	4
09:15 09:30	2	3	5	2	12	12
09:30 09:45	1	3	4	1	9	9
09:45 10:00	1	1	2	0	4	4
10:00 10:15	0	5	1	4	10	10
10:15 10:30	3	2	5	4	14	14
10:30 10:45	1	3	4	0	8	8
10:45 11:00	1	3	4	0	8	8
11:00 11:15	0	2	0	3	5	5
11:15 11:30	0	1	2	1	4	4
11:30 11:45	0	2	0	1	3	3
11:45 12:00	0	1	2	0	3	3
12:00 12:15	1	3	4	0	8	8
12:15 12:30	1	3	4	0	8	8
12:30 12:45	0	2	2	0	4	4
12:45 13:00	1	1	2	2	6	6
13:00 13:15	1	0	1	0	2	2
13:15 13:30	0	1	1	2	4	4
13:30 13:45	0	1	1	0	2	2
13:45 14:00	0	1	1	3	5	5
14:00 14:15	2	3	5	5	15	15
14:15 14:30	0	1	1	0	2	2
14:30 14:45	1	5	2	1	9	9
14:45 15:00	1	2	2	1	6	6
15:00 15:15	1	2	3	2	8	8
15:15 15:30	4	3	7	0	14	14
15:30 15:45	1	5	6	1	13	13
15:45 16:00	0	1	1	0	2	2
16:00 16:15	0	1	1	5	7	7
16:15 16:30	0	1	1	0	2	2
16:30 16:45	0	1	1	0	2	2
16:45 17:00	0	1	1	5	7	7
17:00 17:15	0	1	1	0	2	2
17:15 17:30	0	3	3	0	6	6
17:30 17:45	0	2	2	0	4	4
17:45 18:00	1	3	4	1	9	9
Total	31	73	104	48	81	185



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
Start Time: 07:00

WO No: 38667
Device: Miovision

ALBION RD

Full Study Heavy Vehicles

Time Period	Northbound			Southbound			Eastbound			Westbound			W	STR	Grand				
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT				RT	TOT	TOT	
07:00	0	1	0	1	2	0	4	5	0	4	1	5	0	13	1	14	19	24	
07:15	0	2	5	0	0	0	5	0	17	1	18	1	9	1	11	29	34	34	
07:30	0	1	0	1	0	1	1	2	0	7	1	8	1	8	0	9	17	19	
07:45	0	3	0	5	2	2	0	4	9	0	4	1	5	0	8	1	9	14	23
08:00	0	0	0	0	1	0	0	1	1	0	11	1	8	4	13	24	25	25	
08:15	0	3	1	0	4	0	1	1	5	0	8	1	9	1	9	10	19	24	
08:30	0	2	0	4	1	1	2	4	8	0	10	0	4	1	5	15	23	23	
08:45	0	2	0	1	3	2	1	0	3	6	0	9	0	4	1	5	14	20	
09:00	0	1	2	5	0	0	0	5	0	7	4	11	0	14	2	16	27	32	
09:15	0	1	0	3	0	1	1	4	8	1	8	0	9	0	9	2	11	20	28
09:30	0	1	2	4	2	1	1	4	8	1	8	0	9	0	9	2	11	20	28
09:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	28	24	17	69	23	19	9	51	120	6	238	17	261	12	173	41	226	487	607



Transportation Services - Traffic Services
Turning Movement Count - Study Results
ALBION RD @ BANK ST

Survey Date: Thursday, June 20, 2019
Start Time: 07:00

WO No: 38667
Device: Miovision

ALBION RD

Full Study 15 Minute U-Turn Total

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0

Appendix C

Synchro Intersection Worksheets – Existing Conditions

DRAFT

Lanes, Volumes, Timings
1: Bank & Hunt Club

ExistingAM Peak Hour
2600 Bank Street

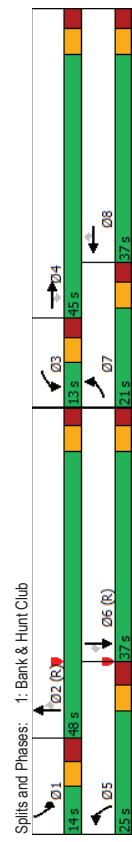
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	119	702	219	25	884	177	286	944	22	82	267	135
Future Volume (vph)	119	702	219	25	884	177	286	944	22	82	267	135
Lane Group Flow (vph)	132	780	243	28	982	197	318	1049	24	91	297	150
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.7	43.5	43.5	6.3	34.1	34.1	16.3	41.6	41.6	6.8	32.1	32.1
Actuated G/C Ratio	0.09	0.36	0.36	0.05	0.28	0.28	0.14	0.35	0.35	0.06	0.27	0.27
v/c Ratio	0.50	0.69	0.36	0.34	1.09	0.32	0.75	0.92	0.04	0.50	0.35	0.27
Control Delay	58.3	37.5	5.3	53.2	114.7	16.5	66.4	43.9	0.1	64.9	37.4	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.3	37.5	5.3	53.2	114.7	16.5	66.4	43.9	0.1	64.9	37.4	1.2
LOS	E	D	A	D	F	B	E	E	A	E	D	A
Approach Delay	33.1			97.3			58.3			31.9		
Approach LOS	C			F			E			C		
Queue Length 50th (m)	15.5	87.4	0.0	6.6	~142.1	12.1	36.7	129.4	0.0	10.9	30.2	0.0
Queue Length 95th (m)	24.8	111.1	17.6	m11.7	#194.8	29.7	55.9	#159.2	0.0	19.4	43.2	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	669	86	898	609	465	1138	635	184	854	551
Starvation Cap Reductn	0	0	0	0	0	0	0	101	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.69	0.36	0.33	1.09	0.32	0.68	1.01	0.04	0.49	0.35	0.27

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	105

Lanes, Volumes, Timings
1: Bank & Hunt Club

ExistingAM Peak Hour
2600 Bank Street

Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.09
Intersection Signal Delay:	59.2
Intersection LOS:	E
ICU Level of Service:	E
Intersection Capacity Utilization:	84.2%
Analysis Period (min):	15
Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
Volume for 95th percentile queue is metered by upstream signal.	



Splits and Phases: 1: Bank & Hunt Club	
Phase	Duration (s)
D1	13
D2	13
D3	13
D4	15
D5	25
D6	17
D7	21
D8	17

Lanes, Volumes, Timings
2: Albion & Hunt Club

ExistingAM Peak Hour
2600 Bank Street

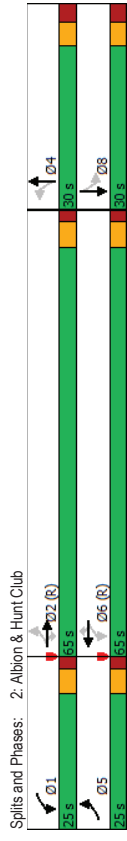
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	58	717	8	181	992	79	6	99	41	84
Future Volume (vph)	58	717	8	181	992	79	6	99	41	84
Lane Group Flow (vph)	64	797	9	201	1102	88	7	370	46	170
Turn Type	pm-pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.0	65.0	65.0	25.0	65.0	65.0	30.0	30.0	30.0	30.0
Total Split (%)	20.8%	54.2%	54.2%	20.8%	54.2%	54.2%	25.0%	25.0%	25.0%	25.0%
Maximum Green (s)	19.6	59.5	59.5	19.6	59.5	59.5	23.8	23.8	23.8	23.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	2	2	2	4	4	4	2	2	2	2
Act Effr Green (s)	75.7	68.6	68.6	83.4	74.3	74.3	23.8	23.8	23.8	23.8
Actuated G/C Ratio	0.63	0.57	0.57	0.70	0.62	0.62	0.20	0.20	0.20	0.20
v/c Ratio	0.22	0.44	0.01	0.46	0.57	0.10	0.04	1.00	0.79	0.54
Control Delay	3.3	4.6	0.0	9.6	15.5	3.1	62.0	102.2	117.7	42.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	3.3	4.6	0.0	9.6	15.5	3.1	62.0	102.2	117.7	42.1
LOS	A	A	A	A	B	A	E	F	F	D
Approach Delay	4.4	4.4	4.4	13.9	13.9	13.9	101.4	101.4	101.4	58.2
Approach LOS	A	A	A	B	B	B	F	F	F	E
Queue Length 50th (m)	0.2	1.3	0.0	14.2	79.2	0.9	1.4	-66.9	10.4	29.5
Queue Length 95th (m)	m0.3	2.4	m0.0	22.5	101.6	7.6	m5.0	#129.0	#32.8	51.9
Internal Link Dist (m)	334.1			554.6	554.6	554.6	188.3	188.3	188.3	429.6
Turn Bay Length (m)	65.0	40.0	40.0	100.0	100.0	40.0	35.0	30.0	30.0	30.0
Base Capacity (vph)	437	1806	707	525	1938	918	187	370	58	317
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.44	0.01	0.38	0.57	0.10	0.04	1.00	0.79	0.54

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 27 (23%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
 Natural Cycle: 75

Lanes, Volumes, Timings
2: Albion & Hunt Club

ExistingAM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.00	
Intersection Signal Delay: 25.9	Intersection LOS: C
Intersection Capacity Utilization 81.7%	ICU Level of Service D
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	31	1	2	1	51	1213	13	471	40
Traffic Volume (vph)	31	1	2	1	51	1213	13	471	40
Future Volume (vph)	0	72	0	12	0	1405	14	523	44
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Minimum Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (s)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Total Split (%)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Maximum Green (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
Yellow Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Total Lost Time (s)									
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	1	1	9	9	9	9	9	8	8
Act Effr Green (s)	14.4	14.4	14.4	14.4	97.6	97.6	97.6	97.6	97.6
Actuated g/C Ratio	0.12	0.12	0.12	0.12	0.81	0.81	0.81	0.81	0.81
v/c Ratio	0.39	0.07	0.33	0.33	0.01	0.20	0.04	0.04	0.04
Control Delay	30.6	24.5	24.5	24.5	0.0	0.0	3.7	1.7	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	30.6	24.5	24.5	24.5	2.7	0.0	3.9	1.7	1.7
LOS	C	C	C	C	A	A	A	A	A
Approach Delay	30.6	24.5	24.5	24.5	2.6	0.0	3.7	1.7	1.7
Approach LOS	C	C	C	C	A	A	A	A	A
Queue Length 50th (m)	7.9	0.7	0.7	0.7	14.2	0.0	8.2	0.0	0.0
Queue Length 95th (m)	18.3	5.4	5.4	5.4	m23.6	m0.0	24.6	0.9	0.9
Internal Link Dist (m)	64.2	37.0	37.0	37.0	227.9	67.1			
Turn Bay Length (m)					15.0				
Base Capacity (vph)	362	362	362	362	4292	1172	2621	1062	1062
Starvation Cap Reductn	0	0	0	0	0	0	1253	0	0
Spillback Cap Reductn	2	0	0	0	382	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.03	0.03	0.03	0.36	0.01	0.38	0.04	0.04

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	30 (25%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	85

Control Type: Actuated-Coordinated	
Maximum v/c Ratio:	0.39
Intersection Signal Delay:	4.0
Intersection LOS:	A
Intersection Capacity Utilization:	69.2%
Analysis Period (min):	15
m. Volume for 95th percentile queue is metered by upstream signal.	



Lanes, Volumes, Timings
4: Albion & Bank

Lanes, Volumes, Timings
4: Albion & Bank

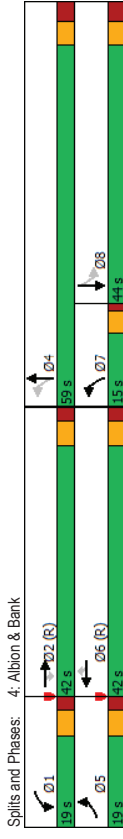
ExistingAM Peak Hour
2600 Bank Street

ExistingAM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	2	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	21	378	95	29	964	117	298	214	100	130
Future Volume (vph)	21	378	95	29	964	117	298	214	100	130
Lane Group Flow (vph)	23	420	106	32	1071	130	331	270	111	174
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prn+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phases	5	2	2	1	6	6	7	4	4	8
Detector Phase	5	2	2	1	6	6	7	4	4	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.3	63.3	63.3	8.0	64.0	64.0	37.7	35.6	20.6	20.6
Actuated g/C Ratio	0.06	0.53	0.53	0.07	0.53	0.53	0.31	0.30	0.17	0.17
v/c Ratio	0.23	0.25	0.13	0.31	0.61	0.16	1.06	0.53	0.63	0.59
Control Delay	66.0	20.4	5.9	60.6	24.9	5.5	103.0	36.7	60.1	50.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.0	20.4	5.9	60.6	24.9	5.5	103.0	36.7	60.1	50.3
LOS	E	C	A	E	C	A	F	D	E	D
Approach Delay	19.5			23.7			73.2		54.1	
Approach LOS	B			C			E		D	
Queue Length 50th (m)	5.5	26.3	0.0	7.3	92.4	0.9	~77.5	53.1	27.3	41.0
Queue Length 95th (m)	14.2	52.1	9.2	17.2	#173.3	14.5	#87.0	61.8	30.7	39.1
Internal Link Dist (m)	227.9			198.3			328.9		188.3	
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	183	1652	813	170	1750	796	313	753	320	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.25	0.13	0.19	0.61	0.16	1.06	0.36	0.35	0.33

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 56 (47%), Referenced to phase 2,EBT and 6,WBT, Start of Green
 Natural Cycle: 105

Control Type: Actuated-Coordinated	Intersection LOS: D
Maximum v/c Ratio: 1.06	ICU Level of Service C
Intersection Signal Delay: 37.3	
Intersection Capacity Utilization: 72.8%	
Analysis Period (min): 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



ExistingAM Peak Hour
2600 Bank Street

ExistingPM Peak Hour
2600 Bank Street

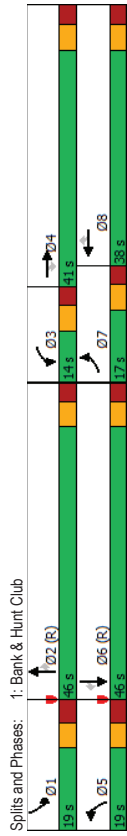
Lanes, Volumes, Timings
1: Bank & Hunt Club

Intersection	Major1	Major2	Minor2	EBL	EBT	WBT	WBR	SBL	SBR
Int Delay, s/veh				1.8					
Movement				EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				4	4	4	4	4	4
Traffic Vol, veh/h	40	441	972	24	30	114			
Future Vol, veh/h	40	441	972	24	30	114			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	None	-	None	-	None	-	None	-
Storage Length	10	-	-	-	-	350	0	-	-
Veh in Median Storage, #	0	0	0	0	0	0	0	0	0
Grade, %									
Peak Hour Factor	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2
Mvmt Flow	44	490	1080	27	33	127			
Major/Minor	Major1	Major2	Minor2						
Conflicting Flow All	1107	0	0	1427	554				
Stage 1	-	-	-	1094	-	-	-	333	-
Stage 2	-	-	-	-	-	-	-	-	684
Critical Hwy	4.14	-	-	6.84	6.94				
Critical Hwy Stg 1	-	-	-	5.84	-				
Critical Hwy Stg 2	-	-	-	3.52	3.32				
Follow-up Hwy	2.22	-	-	3.52	3.32				
Pot Cap-1 Maneuver	626	-	-	126	476				
Stage 1	-	-	-	282	-				
Stage 2	-	-	-	-	698				
Platoon blocked, %	-	-	-	-	-				
Mov Cap-1 Maneuver	626	-	-	117	476				
Mov Cap-2 Maneuver	-	-	-	213	-				
Stage 1	-	-	-	262	-				
Stage 2	-	-	-	-	698				
Approach	EB	WB	SB						
HCM Control Delay, s	0.9	0	17.3						
HCM LOS	C								
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2			
Capacity (veh/h)	626	-	-	-	213	476			
HCM Lane V/C Ratio	0.071	-	-	-	0.156	0.266			
HCM Control Delay (s)	11.2	-	-	-	25	15.3			
HCM Lane LOS	B	-	-	-	D	C			
HCM 95th %ile Q(veh)	0.2	-	-	-	0.5	1.1			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	144	1015	339	45	797	176	307	490	60	251	905	194
Traffic Volume (vph)	144	1015	339	45	797	176	307	490	60	251	905	194
Future Volume (vph)	160	1128	377	50	886	196	341	544	67	279	1006	216
Turn Type	Prot	NA	Perm	Prot	NA	Perm	NA	Prot	NA	Perm	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	17.0	41.0	41.0	14.0	38.0	38.0	19.0	46.0	46.0	19.0	46.0	46.0
Total Split (%)	14.2%	34.2%	34.2%	11.7%	31.7%	31.7%	15.8%	38.3%	38.3%	15.8%	38.3%	38.3%
Maximum Green (s)	10.5	34.3	34.3	7.5	31.3	31.3	11.9	39.5	39.5	11.9	39.5	39.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21		4	4		22	22	22	22	55	55
Act Effr Green(s)	10.0	37.1	37.1	7.1	31.8	31.8	11.9	39.5	39.5	11.9	39.5	39.5
Actuated g/C Ratio	0.08	0.31	0.31	0.06	0.26	0.26	0.10	0.33	0.33	0.10	0.33	0.33
v/c Ratio	0.63	1.12	1.12	0.60	0.52	0.52	1.07	0.50	0.50	1.07	0.88	0.92
Control Delay	64.6	107.5	107.5	14.6	74.2	74.2	83.4	21.6	21.6	83.4	4.5	80.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.6	107.5	107.5	14.6	74.2	74.2	83.4	21.6	21.6	83.4	4.5	80.8
LOS	E	F	B	E	F	C	F	D	A	F	D	B
Approach Delay	82.3			72.3			67.8			52.1		
Approach LOS	F			E			E			D		
Queue Length 50th (m)	19.0	~171.0	18.5	12.5	~90.4	10.4	~45.4	49.4	0.5	33.9	119.5	7.5
Queue Length 95th (m)	30.2	#212.3	51.9	m21.7	#150.7	35.5	m#71.4	m87.8	m6.1	#57.2	#159.2	26.7
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		
Base Capacity (vph)	268	1005	629	101	879	529	318	1091	573	318	1091	548
Starvation Cap Reductn	0	0	0	0	0	0	0	347	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.60	1.12	0.60	0.50	1.01	0.37	1.07	0.73	0.12	0.88	0.92	0.39
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 23 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Natural Cycle: 135												

Lanes, Volumes, Timings
 1: Bank & Hunt Club

Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.12	
Intersection Signal Delay: 68.9	Intersection LOS: E
Intersection Capacity Utilization 91.8%	ICU Level of Service F
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
~ Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Lanes, Volumes, Timings
 2: Albion & Hunt Club

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	121	1200	23	315	943	64	7	117	48	133
Future Volume (vph)	121	1200	23	315	943	64	7	117	48	133
Lane Group Flow (vph)	134	1333	26	350	1048	71	8	401	53	206
Turn Type	pm-pt	NA	Perm	pm-pt	NA	Perm	NA	Perm	NA	NA
Protected Phases	5	2	1	6	6	4	4	8	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.4	58.6	58.6	25.4	58.6	58.6	36.0	36.0	36.0	36.0
Total Split (%)	21.2%	48.8%	48.8%	21.2%	48.8%	48.8%	30.0%	30.0%	30.0%	30.0%
Maximum Green (s)	20.0	53.1	53.1	20.0	53.1	53.1	29.8	29.8	29.8	29.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	4	4	4	5	5	5	6	6	6	6
Ad Effr Green (s)	62.5	53.1	53.1	80.1	65.3	65.3	28.3	28.3	28.3	28.3
Actuated g/C Ratio	0.52	0.44	0.44	0.67	0.54	0.54	0.24	0.24	0.24	0.24
v/c Ratio	0.45	0.92	0.04	0.99	0.58	0.09	0.04	0.94	0.88	0.52
Control Delay	10.2	14.2	0.1	81.1	20.5	3.1	30.6	63.3	133.6	41.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.2	14.2	0.1	81.1	20.5	3.1	30.6	63.3	133.6	41.6
LOS	B	B	A	F	C	A	C	E	F	D
Approach Delay	13.6			34.1			62.6			60.5
Approach LOS	B			C			E			E
Queue Length 50th (m)	4.6	29.0	0.0	~75.0	85.5	0.0	1.5	82.4	11.9	38.5
Queue Length 95th (m)	m4.9	m26.6	m0.0	#133.3	111.4	6.3	m4.0	#129.5	#36.9	62.2
Internal Link Dist (m)	334.1			564.6			188.3			429.6
Turn Bay Length (m)	65.0			100.0			40.0			30.0
Base Capacity (vph)	439	1452	679	355	1805	807	212	446	63	414
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.92	0.04	0.99	0.58	0.09	0.04	0.90	0.84	0.50

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 120	
Offset: 96 (80%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green	
Natural Cycle: 90	

Lanes, Volumes, Timings
2: Albion & Hunt Club

ExistingPM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.99
Intersection LOS: C
Intersection Signal Delay: 30.8
IOU Level of Service G
Intersection Capacity Utilization 103.9%
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Albion & Hunt Club



Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

ExistingPM Peak Hour
2600 Bank Street

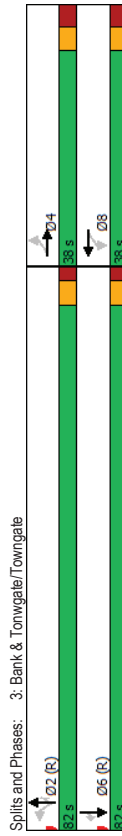
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	150	4	4	0	97	695	17	1141	148
Future Volume (vph)	150	4	4	0	97	695	17	1141	148
Lane Group Flow (vph)	0	348	0	17	0	880	19	1268	164
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	2	6
Permitted Phases	4	4	8	8	2	2	2	2	6
Detector Phase	4	4	8	8	2	2	2	2	6
Switch Phase	4	4	8	8	2	2	2	2	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Total Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	20	20	17	17	17	17	17	17	17
Act Effr Green (s)	30.1	30.1	30.1	30.1	77.4	77.4	77.4	77.4	77.4
v/c Ratio	0.25	0.25	0.25	0.25	0.64	0.64	0.64	0.64	0.64
Control Delay	74.5	8.8	8.8	8.8	7.2	7.2	7.2	7.2	7.2
Queue Delay	3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.3	8.8	8.8	8.8	7.2	7.2	7.2	7.2	7.2
LOS	E	A	A	A	A	A	A	A	A
Approach Delay	78.3	8.8	8.8	8.8	7.1	7.1	7.1	7.1	7.1
Approach LOS	E	A	A	A	A	A	A	A	A
Queue Length 50th (m)	71.5	0.0	0.0	0.0	18.9	18.9	18.9	18.9	18.9
Queue Length 95th (m)	#127.1	4.3	4.3	4.3	18.1	18.1	18.1	18.1	18.1
Internal Link Dist (m)	64.2	37.0	37.0	37.0	227.9	227.9	227.9	227.9	227.9
Turn Bay Length (m)					15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	380	388	388	388	2762	2762	2762	2762	2762
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	12	12	12	12	123	123	123	123	123
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.95	0.05	0.05	0.05	0.33	0.33	0.33	0.33	0.33

Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 120
Offset: 9 (8%), Referenced to phase 2:NBL and 6:SBT, Start of Green
Natural Cycle: 85

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

2600 Bank Street

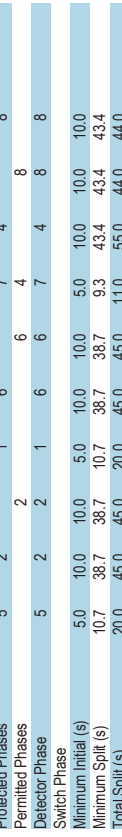
Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 15.2
 Intersection LOS: B
 ICU Level of Service G
 Intersection Capacity Utilization 108.0%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
4: Albion & Bank

2600 Bank Street

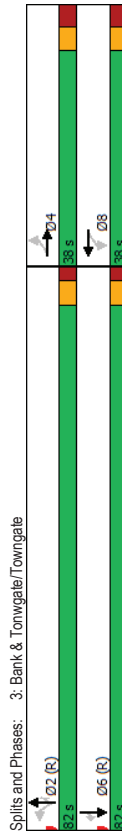
Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 15.2
 Intersection LOS: B
 ICU Level of Service G
 Intersection Capacity Utilization 108.0%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
ExistingPM Peak Hour

2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.95
 Intersection Signal Delay: 15.2
 Intersection LOS: B
 ICU Level of Service G
 Intersection Capacity Utilization 108.0%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
ExistingPM Peak Hour

2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	70	938	279	43	588	145	158	169	177	255
Future Volume (vph)	70	938	279	43	588	145	158	169	177	255
Lane Group Flow (vph)	78	1042	310	48	653	161	176	237	197	335
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	7	4	8	8

Detector Phase	Switch Phase	Minimum Initial (s)	Minimum Split (s)	Total Split (s)	Total Split (%)	Maximum Green (s)	Yellow Time (s)	All-Red Time (s)	Lost Time Adjust (s)	Total Lost Time (s)	Lead/Lag	Lead/Lag	Lead/Lag	Vehicle Extension (s)	Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None
5	5	10.0	10.0	10.0	10.0	10.0	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
10.7	38.7	38.7	10.7	38.7	38.7	38.7	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
20.0	45.0	45.0	20.0	45.0	45.0	45.0	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
16.7%	37.5%	37.5%	16.7%	37.5%	37.5%	37.5%	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
14.3	39.3	39.3	14.3	39.3	39.3	39.3	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.7	5.7	5.7	3.0	None	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7	5.7
Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
None	C-Max	C-Max	None	C-Max	C-Max	C-Max	None	C-Max	C-Max	C-Max	None	None	None	None	None	None	None	None	None	None	None	None	None
20.0	20.0	20.0	13.0	13.0	13.0	13.0	10	10	10	10	10	10	10	10	10	10	10	10	10	10	10	10	
13.0	13.0	13.0	13.0	13.0	13.0	13.0	10	10	10	10	10	10	10	10	10	10	10	10	10	10	10	10	
1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
10.8	56.0	56.0	8.9	54.3	54.3	41.7	39.6	28.6	28.6	28.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6	39.6
0.09	0.47	0.47	0.07	0.45	0.45	0.35	0.33	0.24	0.24	0.24	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33
0.63	0.67	0.37	0.39	0.44	0.44	0.22	0.83	0.42	0.82	0.82	0.42	0.78	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
74.9	14.4	1.5	61.5	26.3	5.0	59.8	30.7	45.2	41.7	41.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
74.9	14.4	1.5	61.5	26.3	5.0	59.8	30.7	45.2	41.7	41.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7	30.7
E	B	A	E	C	A	E	C	A	E	C	A	E	C	A	E	C	A	E	C	A	E	C	A
14.9	14.9	14.9	24.3	24.3	24.3	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1	43.1
B	B	B	C	C	C	D	D	D	D	D	D	D	D	D	D	D	D	D	D	D	D	D	D
0.0	44.5	0.0	11.0	56.4	0.0	30.0	40.8	47.8	80.2	80.2	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8	40.8
m26.4	m#155.6	m6.4	22.6	86.7	14.6	#48.1	56.0	m54.6	m88.7	m88.7	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0	56.0
227.9	227.9	227.9	198.3	198.3	198.3	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9	328.9
30.0	100.0	100.0	65.0	30.0	30.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
197	1547	841	197	1501	725	213	690	333	538	538	213	213	213	213	213	213	213	213	213	213	213	213	213
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0.40	0.67	0.37	0.24	0.44	0.22	0.83	0.34	0.59	0.62	0.62	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34

Intersection Summary	Cycle Length: 120	Actuated Cycle Length: 120	Offset: 42 (35%), Referenced to phase 2:EBT and 6:WBT, Start of Green	Natural Cycle: 105						
Reduced v/c Ratio	0.40	0.67	0.37	0.24	0.44	0.22	0.83	0.34	0.59	0.62

Lanes, Volumes, Timings
4: Albion & Bank

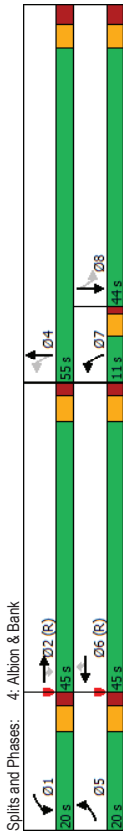
HCM 2010 TWSC
7: Bank & Steveright

ExistingPM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 25.6
 Intersection LOS: C
 Intersection Capacity Utilization: 78.7%
 ICU Level of Service D
 Analysis Period (min): 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.

Intersection
 In/Delay, s/veh 1.4

Movement	EBU	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔
Traffic Vol, veh/h	100	1025	652	26	23	99	99
Future Vol, veh/h	100	1025	652	26	23	99	99
Conflicting Peds. #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	None	-	None
Storage Length	-	10	-	-	-	350	0
Veh in Median Storage, #	-	0	-	-	-	0	-
Grade, %	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	1	111	1139	724	29	26	110



Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	753	0	0
Stage 1	-	-	1533
Stage 2	-	-	377
Critical Hdwy	6.44	4.14	-
Critical Hdwy Stg 1	-	-	794
Critical Hdwy Stg 2	-	-	6.84
Follow-up Hdwy	2.52	2.22	-
Pot Cap-1 Maneuver	478	853	-
Stage 1	-	-	107
Stage 2	-	-	621
Platoon blocked, %	-	-	433
Mov Cap-1 Maneuver	844	-	-
Mov Cap-2 Maneuver	-	-	406
Stage 1	-	-	93
Stage 2	-	-	621

Approach	EB	WB	SB
HCM Control Delay, s	0.9	0	14.2
HCM LOS	B	B	B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	844	-	-	-	220	621
HCM Lane V/C Ratio	0.133	-	-	-	0.116	0.177
HCM Control Delay (s)	9.9	-	-	-	23.5	12
HCM Lane LOS	A	-	-	-	C	B
HCM 95th %tile Q(veh)	0.5	-	-	-	0.4	0.6

Appendix D

Collision Data

DRAFT

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition
2015-08-24	2015	13:31	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	02 - Wet
2015-09-19	2015	14:47	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-09-17	2015	15:13	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	01 - Dry
2015-12-14	2015	13:14	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	01 - Dry
2015-09-05	2015	12:09	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2015-01-09	2015	17:06	ALBION RD @ BANK ST	03 - Snow	05 - Dusk	01 - Traffic signal		03 - P.D. only	03 - Rear end	03 - Loose snow
2015-06-05	2015	14:18	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2015-08-01	2015	14:14	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-06-18	2016	19:37	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	01 - Dry
2016-12-15	2016	17:32	ALBION RD @ BANK ST	01 - Clear	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2016-02-21	2016	16:46	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-05-09	2016	21:35	ALBION RD @ BANK ST	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-05-04	2016	19:00	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2016-10-31	2016	13:03	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2016-12-22	2016	7:56	ALBION RD @ BANK ST	03 - Snow	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	03 - Loose snow
2016-12-07	2016	19:29	ALBION RD @ BANK ST	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-06-12	2017	17:48	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	01 - Dry
2017-07-22	2017	11:11	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-11-19	2017	17:27	ALBION RD @ BANK ST	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	02 - Angle	01 - Dry
2017-01-12	2017	17:34	ALBION RD @ BANK ST	02 - Rain	05 - Dusk	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2017-02-14	2017	8:39	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	04 - Slush
2017-03-16	2017	13:10	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	02 - Wet
2017-04-13	2017	17:55	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-04-13	2017	16:10	ALBION RD @ BANK ST	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-01-20	2018	12:52	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-01-30	2018	16:04	ALBION RD @ BANK ST (0012409)	01 - Clear	03 - Snow	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2018-03-27	2018	14:52	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2018-06-08	2018	22:31	ALBION RD @ BANK ST (0012409)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-06-23	2018	3:26	ALBION RD @ BANK ST (0012409)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2018-07-07	2018	23:00	ALBION RD @ BANK ST (0012409)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-11-09	2018	14:05	ALBION RD @ BANK ST (0012409)	03 - Snow	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	02 - Wet
2018-11-17	2018	20:00	ALBION RD @ BANK ST (0012409)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2018-12-11	2018	10:31	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	04 - Slush
2019-03-08	2019	13:00	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2019-03-31	2019	23:20	ALBION RD @ BANK ST (0012409)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	03 - Loose snow
2019-08-09	2019	9:02	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	02 - Angle	01 - Dry
2019-10-15	2019	13:00	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2019-11-09	2019	16:04	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	02 - Wet
2019-12-17	2019	14:38	ALBION RD @ BANK ST (0012409)	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	02 - Wet
2017-08-11	2017	18:08	ALBION RD btwn ALBION RD S & BANK ST	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	02 - Angle	01 - Dry
2019-03-08	2019	19:35	ALBION RD btwn ALBION RD S & BANK ST (_3ZA2PX)	01 - Clear	07 - Dark	10 - No control		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-10-31	2015	19:26	BANK ST @ SIEVERIGHT AVE	01 - Clear	07 - Dark	02 - Stop sign		02 - Non-fatal injury	02 - Angle	01 - Dry
2017-11-11	2017	11:01	BANK ST @ SIEVERIGHT AVE	01 - Clear	01 - Daylight	02 - Stop sign		02 - Non-fatal injury	02 - Angle	01 - Dry
2017-02-17	2017	17:54	BANK ST @ SIEVERIGHT AVE	01 - Clear	07 - Dark	02 - Stop sign		03 - P.D. only	02 - Angle	01 - Dry
2019-02-26	2019	9:45	BANK ST @ SIEVERIGHT AVE (0012408)	01 - Clear	01 - Daylight	02 - Stop sign		03 - P.D. only	02 - Angle	01 - Dry
2019-04-26	2019	20:19	BANK ST @ SIEVERIGHT AVE (0012408)	02 - Rain	07 - Dark	02 - Stop sign		02 - Non-fatal injury	07 - SMV other	02 - Wet
2019-05-29	2019	17:30	BANK ST @ SIEVERIGHT AVE (0012408)	01 - Clear	01 - Daylight	02 - Stop sign		02 - Non-fatal injury	02 - Angle	01 - Dry
2019-10-10	2019	21:00	BANK ST @ SIEVERIGHT AVE (0012408)	01 - Clear	07 - Dark	02 - Stop sign		03 - P.D. only	05 - Turning movement	01 - Dry
2015-10-07	2015	17:15	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2016-06-25	2016	5:56	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	07 - SMV other	01 - Dry
2016-04-21	2016	20:25	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry
2017-02-17	2017	14:04	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	05 - Turning movement	01 - Dry
2017-02-28	2017	18:29	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2017-04-09	2017	17:09	BANK ST btwn ALBION RD & SIEVERIGHT AVE	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - Sideswipe	01 - Dry
2018-03-28	2018	16:40	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry
2018-06-15	2018	23:06	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2018-11-02	2018	7:55	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	02 - Rain	01 - Daylight	10 - No control		02 - Non-fatal injury	05 - Turning movement	02 - Wet
2019-01-06	2019	17:30	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	05 - Dusk	10 - No control		03 - Rear end	03 - Rear end	06 - Ice
2019-02-28	2019	7:57	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2019-04-06	2019	20:17	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	07 - Dark	10 - No control		02 - Non-fatal injury	04 - Sideswipe	01 - Dry
2019-05-29	2019	19:30	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	06 - SMV unattended vehicle	01 - Dry
2019-11-14	2019	12:09	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	07 - SMV other	01 - Dry
2019-12-28	2019	17:20	BANK ST btwn ALBION RD & SIEVERIGHT AVE (_3ZA295)	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2015-02-09	2015	7:56	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE	03 - Snow	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	03 - Loose snow
2015-08-17	2015	17:07	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - Sideswipe	01 - Dry
2017-06-09	2017	18:30	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	03 - Rear end	01 - Dry
2018-03-08	2018	1:31	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE (_3ZAYF2)	03 - Snow	07 - Dark	10 - No control		03 - P.D. only	07 - SMV other	03 - Loose snow
2018-05-29	2018	19:20	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE (_3ZAYF2)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	04 - Sideswipe	01 - Dry
2018-05-31	2018	15:32	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE (_3ZAYF2)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	02 - Angle	01 - Dry
2018-08-22	2018	20:55	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE (_3ZAYF2)	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	05 - Turning movement	01 - Dry
2019-11-13	2019	14:45	BANK ST btwn SIEVERIGHT AVE & ATHANS AVE (_3ZAYF2)	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	02 - Wet

Appendix E

TRANS Model Plots

DRAFT

TRANS Regional Model

Version 2.15 - Assigned June 16, 2020

AM Peak Hour Total Traffic Volume

2600 Bank Street

2011 Model - Basecase

N/A

User Initials: TIMW

Plot Prepared: Feb 2, 2020

EMME Scenario: 21711



Legend

AM Peak Hour Total Traffic Volume



Distance (m)



The TRANS model is continuously refined & maintained, and all information is provided in good faith. However, model outputs are provided "as is", and no warranty or guarantee is provided as to the accuracy, reliability, or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.



TRANS Regional Model

Version 2.15 - Assigned June 16, 2020

AM Peak Hour Total Traffic Volume

Mooney's Bay

2031 Model - Basecase

N/A

User Initials: TIMW

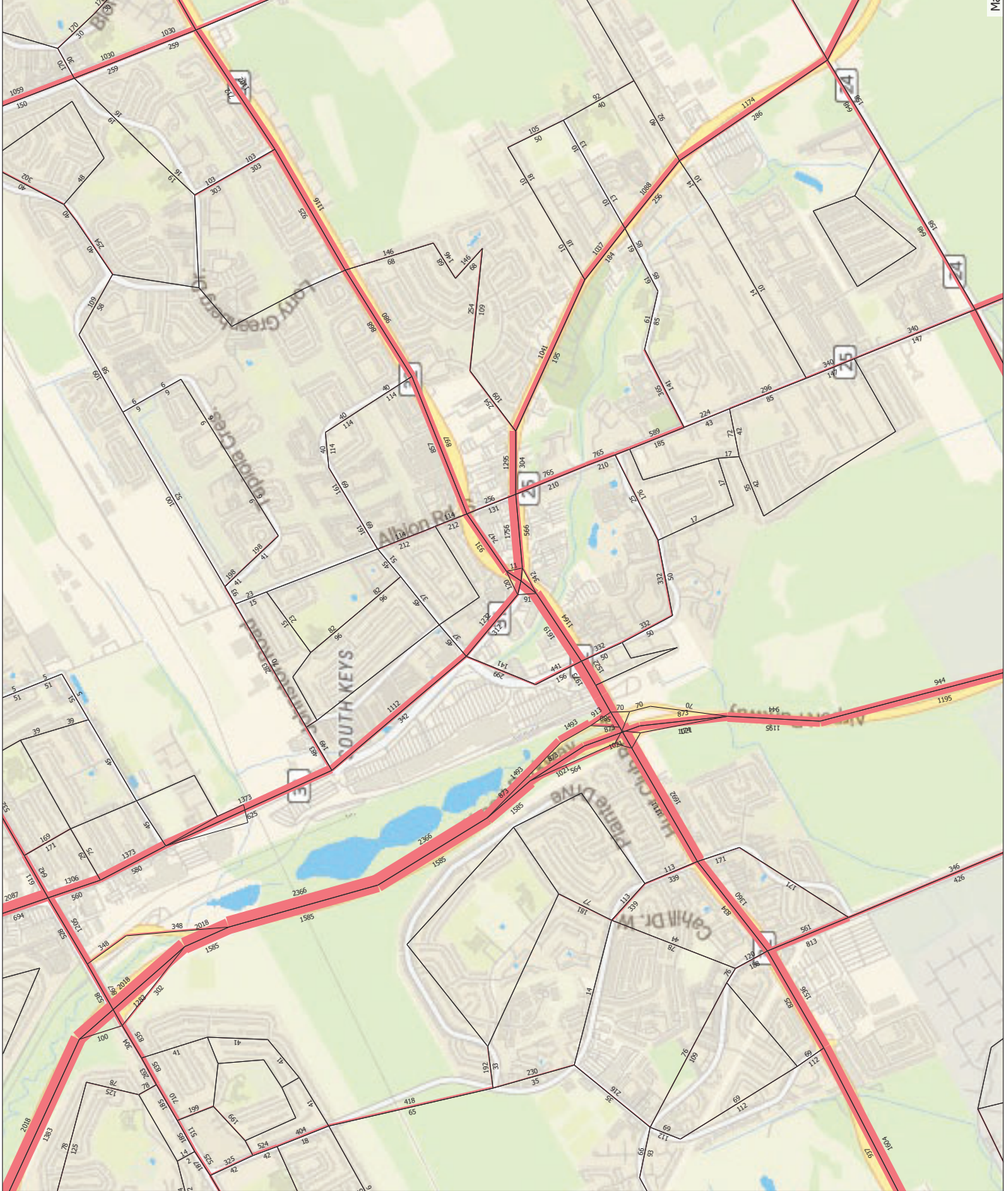
Plot Prepared: Feb 2, 2020

EMME Scenario: 21711



Legend

AM Peak Hour Total Traffic Volume



The TRANS model is continuously refined & maintained, and all information is provided in good faith. However, model outputs are provided "as is", and no warranty or guarantee is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.

Appendix F

Background Development Traffic

DRAFT

Figure 12: 2022 Site-Generated Traffic

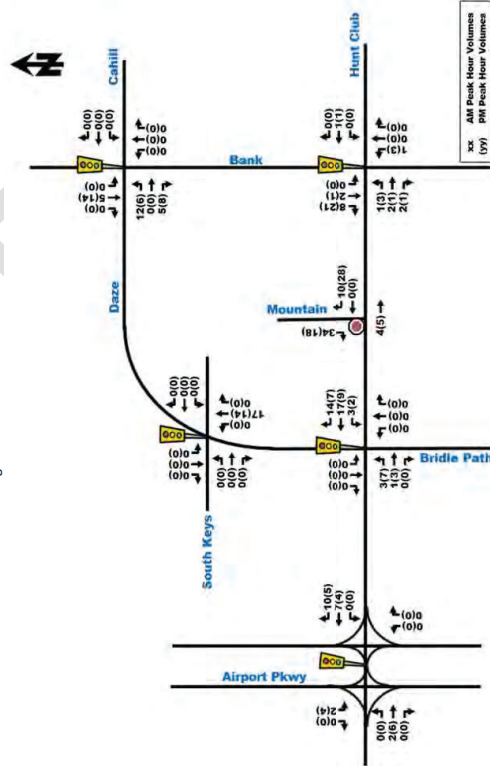


Figure 13: 2027 Site-Generated Traffic

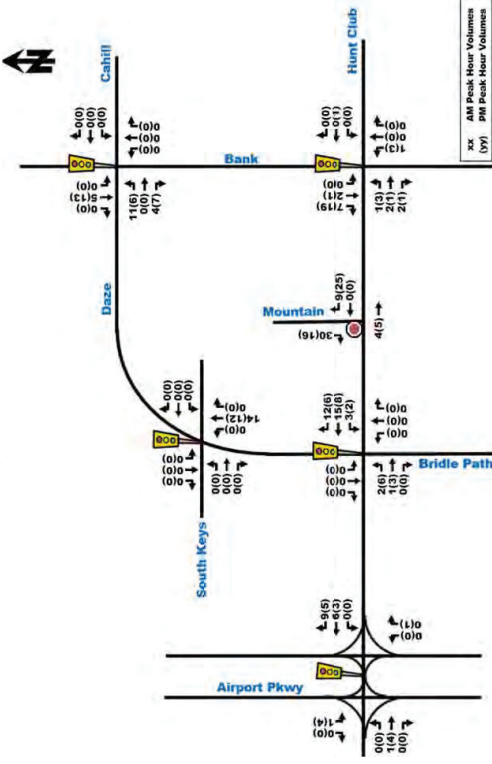
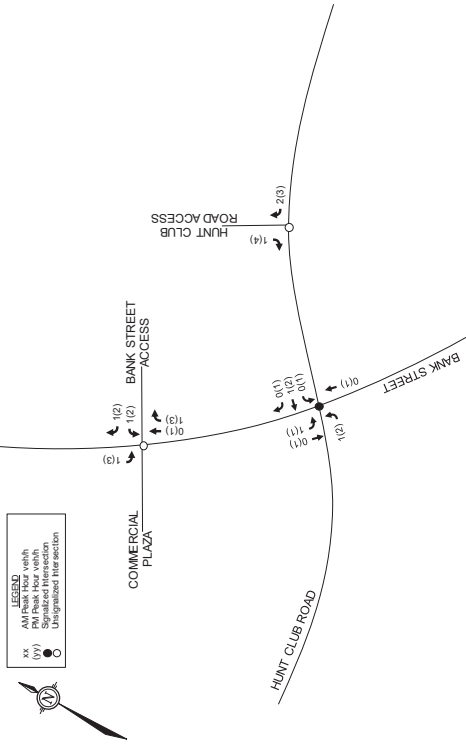


Figure 7: Site Generated Traffic Volumes



Appendix G

Synchro Intersection Worksheets – 2025 Future Background Conditions

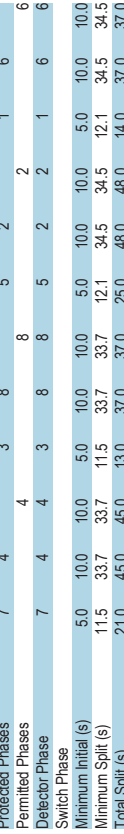
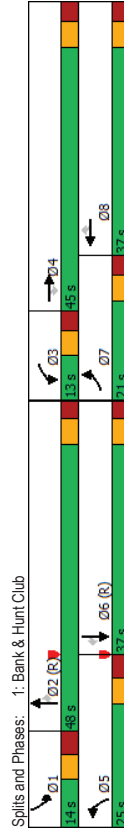
DRAFT

Lanes, Volumes, Timings
1: Bank & Hunt Club

Lanes, Volumes, Timings
1: Bank & Hunt Club

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.97
Intersection Signal Delay: 47.0
Intersection LOS: D
IOU Level of Service E
Intersection Capacity Utilization 86.2%
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.97
Intersection Signal Delay: 47.0
Intersection LOS: D
IOU Level of Service E
Intersection Capacity Utilization 86.2%
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	221	25	886	177	287	1017	22	83	269	143
Future Volume (vph)	121	704	221	25	886	177	287	1017	22	83	269	143
Lane Group Flow (vph)	121	704	221	25	886	177	287	1017	22	83	269	143
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phase	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.6	44.3	44.3	6.7	32.8	32.8
Actuated g/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.34	0.30	0.97	0.29	0.71	0.84	0.03	0.46	0.31	0.26
Control Delay	58.3	35.6	5.4	53.1	85.1	14.3	59.9	36.9	0.1	63.4	36.4	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.4	53.1	85.1	14.3	59.9	40.2	0.1	63.4	36.4	1.1
LOS	E	D	A	D	F	B	E	D	A	E	D	A
Approach Delay	31.8			72.9			43.8			30.7		
Approach LOS	C			E			D			C		
Queue Length 50th (m)	14.2	76.3	0.0	5.9	117.8	9.8	27.0	124.1	0.0	9.8	26.7	0.0
Queue Length 95th (m)	23.2	98.0	17.0	m11.5	#166.6	24.7	51.2	#121.4	0.0	18.1	39.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	655	86	910	613	465	1212	662	184	871	556
Starvation Cap Reductn	0	0	0	0	0	0	0	117	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.34	0.29	0.97	0.29	0.62	0.93	0.03	0.45	0.31	0.26

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 120
Offset: 65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 95

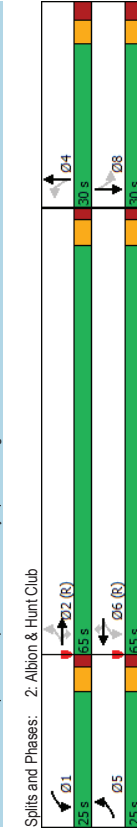
Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Background 2025AM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔↔	↔↔	↔	↔↔	↔↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	58	719	8	181	995	79	6	99	41	84
Future Volume (vph)	58	719	8	181	995	79	6	99	41	84
Lane Group Flow (vph)	58	719	8	181	995	79	6	333	41	153
Turn Type	pm-pt	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	25.0	65.0	65.0	25.0	65.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	20.8%	54.2%	54.2%	20.8%	54.2%	25.0%	25.0%	25.0%	25.0%	25.0%
Maximum Green (s)	19.6	59.5	59.5	19.6	59.5	23.8	23.8	23.8	23.8	23.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	2	2	2	4	4	2	2	2	2	2
Act Effr Green (s)	77.3	70.3	70.3	84.2	75.6	75.6	22.7	22.7	22.7	22.7
Actuated G/C Ratio	0.64	0.59	0.59	0.70	0.63	0.63	0.19	0.19	0.19	0.19
v/c Ratio	0.18	0.39	0.01	0.39	0.50	0.08	0.03	0.94	0.72	0.50
Control Delay	1.9	3.4	0.0	8.2	14.1	2.6	61.3	89.5	103.5	40.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	1.9	3.4	0.0	8.2	14.1	2.6	61.3	89.5	103.5	40.4
LOS	A	A	A	A	B	A	E	F	F	D
Approach Delay	3.2	3.2	3.2	12.5	12.5	89.0	89.0	89.0	89.0	53.7
Approach LOS	A	A	A	B	B	F	F	F	F	D
Queue Length 50th (m)	0.1	0.7	0.0	12.7	67.6	0.1	1.3	56.4	9.0	25.5
Queue Length 95th (m)	m0.3	2.3	m0.0	20.3	87.2	6.2	m4.3	#107.7	#28.7	46.6
Internal Link Dist (m)	334.1			554.6	554.6	188.3	188.3	429.6	429.6	429.6
Turn Bay Length (m)	65.0	40.0	40.0	100.0	40.0	35.0	30.0	30.0	30.0	30.0
Base Capacity (vph)	474	1852	723	564	1971	933	198	370	60	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.39	0.01	0.32	0.50	0.08	0.03	0.90	0.68	0.48

04-20-2021 JK CGH Transportation Page 3

Control Type: Actuated-Coordinated	Intersection LOS: C
Maximum v/c Ratio: 0.94	ICU Level of Service D
Intersection Signal Delay: 22.9	
Intersection Capacity Utilization 61.8%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



04-20-2021 JK CGH Transportation Page 4

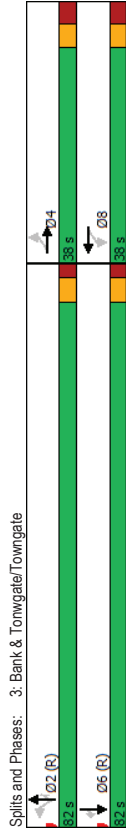
Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBR	SBR
Lane Configurations	31	1	2	1	51	1308	13	475
Traffic Volume (vph)	31	1	2	1	51	1308	13	475
Future Volume (vph)	0	65	0	11	0	1359	13	475
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	8	8	2	2	2	2	6
Protected Phases	4	4	8	8	2	2	2	6
Permitted Phases	4	4	8	8	2	2	2	6
Detector Phase	4	4	8	8	2	2	2	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8
Minimum Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0
Total Split (s)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%
Total Split (%)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2
Maximum Green (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7
Yellow Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8
Total Lost Time (s)								
Lead/Lag								
Lead-Lag Optimize?	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	None	None	None	None	C-Max	C-Max	C-Max	C-Max
Recall Mode	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0
Walk Time (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0
Flash Dont Walk (s)	1	1	9	9	9	9	8	8
Pedestrian Calls (#/hr)	14.3	14.3	0.12	0.12	0.81	0.81	0.81	0.81
Act Effr Green (s)	0.12	0.12	0.07	0.07	0.31	0.31	0.18	0.04
Actuated g/C Ratio	30.5	25.9	2.6	2.6	0.0	0.0	3.6	1.8
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0
Queue Delay	30.5	25.9	2.6	2.6	0.0	0.0	3.8	1.8
Total Delay	C	C	A	A	A	A	A	A
LOS	C	C	A	A	A	A	A	A
Approach Delay	30.5	25.9	2.6	2.6	3.7	3.7		
Approach LOS	C	C	A	A	A	A		
Queue Length 50th (m)	7.2	0.7	13.1	0.0	7.3	0.0		
Queue Length 95th (m)	17.2	5.2	m22.8	m0.0	22.7	0.7		
Internal Link Dist (m)	64.2	37.0	227.9		67.1			
Turn Bay Length (m)					15.0			
Base Capacity (vph)	359	359	4340	1173	2623	1062		
Starvation Cap Reductn	0	0	0	0	1322	0		
Spillback Cap Reductn	3	0	540	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.18	0.03	0.36	0.01	0.37	0.04		

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	30 (25%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	85

Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.36
Intersection Signal Delay:	3.9
Intersection LOS:	A
Intersection Capacity Utilization:	69.2%
Analysis Period (min):	15
m:	Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
4: Albion & Bank

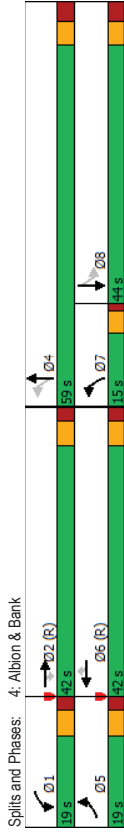
Future Background 2025AM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	21	382	95	29	1040	117	298	214	100	130
Future Volume (vph)	21	382	95	29	1040	117	298	214	100	130
Lane Group Flow (vph)	21	382	95	29	1040	117	298	243	100	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prt+prt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phase	5	2	2	1	6	6	7	4	4	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	42.0	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	64.6	64.6	7.8	67.7	67.7	36.6	34.5	19.5	19.5
Actuated G/C Ratio	0.06	0.54	0.54	0.06	0.56	0.56	0.30	0.29	0.16	0.16
v/c Ratio	0.21	0.23	0.12	0.29	0.56	0.14	0.94	0.49	0.59	0.56
Control Delay	67.1	17.9	4.3	60.1	21.9	4.4	74.8	36.3	58.2	49.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.1	17.9	4.3	60.1	21.9	4.4	74.8	36.3	58.2	49.8
LOS	E	B	A	E	C	A	E	D	E	D
Approach Delay	17.4			21.1			57.5			53.1
Approach LOS	B			C			E			D
Queue Length 50th (m)	0.0	19.3	0.0	6.6	64.2	0.0	61.4	47.6	24.7	36.9
Queue Length 95th (m)	13.6	46.4	7.3	16.0	#164.3	11.5	68.7	55.3	28.4	0.0
Internal Link Dist (m)	227.9			198.3			328.9			188.3
Turn Bay Length (m)	30.0	100.0	100.0	65.0	30.0					45.0
Base Capacity (vph)	183	1684	826	170	1852	836	316	753	328	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.23	0.12	0.17	0.56	0.14	0.94	0.32	0.30	0.29

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 56 (47%), Referenced to phase 2,EBT and 6,WBT, Start of Green
 Natural Cycle: 105

Lanes, Volumes, Timings
4: Albion & Bank

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.94
Intersection Signal Delay: 31.6
Intersection LOS: C
ICU Level of Service D
Intersection Capacity Utilization 75.0%
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



HCM 2010 TWSC
7: Bank & Steveright

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Background 2025AM Peak Hour
2600 Bank Street

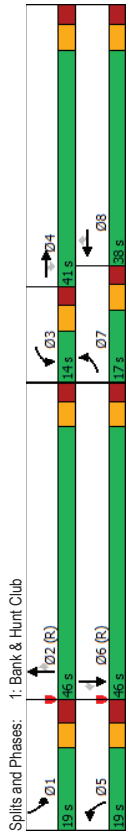
Future Background 2025PM Peak Hour
2600 Bank Street

Intersection	EBL	EBT	WBT	WBR	SBL	SBR
Int Delay, s/veh	1.6					
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Vol, veh/h	40	445	1048	24	30	114
Future Vol, veh/h	40	445	1048	24	30	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	10	-	-	-	350	0
Veh in Median Storage, #	0	0	0	0	0	0
Grade, %	-	-	-	-	-	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	445	1048	24	30	114
Major/Minor	Major1	Major2	Major2	Minor2	Minor2	Minor2
Conflicting Flow All	1072	0	0	0	1363	536
Stage 1	-	-	-	-	1060	-
Stage 2	-	-	-	-	303	-
Critical Hwy	4.14	-	-	-	6.84	6.94
Critical Hwy Stg 1	-	-	-	-	5.84	-
Critical Hwy Stg 2	-	-	-	-	3.52	3.32
Follow-up Hwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	646	-	-	-	139	489
Stage 1	-	-	-	-	294	-
Stage 2	-	-	-	-	723	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	646	-	-	-	130	489
Mov Cap-2 Maneuver	-	-	-	-	226	-
Stage 1	-	-	-	-	276	-
Stage 2	-	-	-	-	723	-
Approach	EB	WB	SB	SB	EB	EB
HCM Control Delay, s	0.9	0	16.4	16.4	0	0
HCM LOS	C	C	C	C	C	C
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	646	-	-	-	226	489
HCM Lane V/C Ratio	0.062	-	-	-	0.133	0.233
HCM Control Delay (s)	10.9	-	-	-	23.4	14.6
HCM Lane LOS	B	-	-	-	C	B
HCM 95th %ile Q(veh)	0.2	-	-	-	0.5	0.9

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	1016	340	46	800	177	310	491	60	252	977
Future Volume (vph)	149	1016	340	46	800	177	310	491	60	252	977
Lane Group Flow (vph)	149	1016	340	46	800	177	310	491	60	252	977
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2	2	1	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6
Switch Phase	7	4	4	3	8	8	5	2	2	1	6
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5
Total Split (s)	17.0	41.0	41.0	14.0	38.0	38.0	19.0	46.0	46.0	19.0	46.0
Total Split (%)	14.2%	34.2%	34.2%	11.7%	31.7%	31.7%	15.8%	38.3%	38.3%	15.8%	38.3%
Maximum Green (s)	10.5	34.3	34.3	7.5	31.3	31.3	11.9	39.5	39.5	11.9	39.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag
Lead/Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21	21	4	4	4	22	22	22	22	22
Act Effr Green(s)	9.8	37.1	37.1	7.1	32.0	32.0	11.9	39.7	39.7	11.7	39.5
Actuated g/C Ratio	0.08	0.31	0.31	0.06	0.27	0.27	0.10	0.33	0.33	0.10	0.33
v/c Ratio	0.59	1.01	0.54	0.48	0.91	0.34	0.97	0.45	0.10	0.80	0.39
Control Delay	63.2	72.9	10.7	74.5	61.6	19.1	101.9	34.1	3.8	72.7	50.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.2	0.0	0.0	0.0
Total Delay	63.2	72.9	10.7	74.5	61.6	19.1	101.9	35.3	3.8	72.7	50.1
LOS	E	E	B	E	E	B	F	D	A	E	D
Approach Delay	57.9	57.9	57.9	54.8	54.8	54.8	57.1	57.1	57.1	54.8	54.8
Approach LOS	E	E	B	D	D	D	E	E	D	E	D
Queue Length 50th (m)	17.6	~142.1	10.2	11.5	71.4	4.4	38.0	38.5	0.3	30.3	114.6
Queue Length 95th (m)	28.4	#182.6	37.6	m#22.7	#126.6	31.1	m#64.4	m#1.4	m#5.4	#151.4	25.4
Internal Link Dist (m)	358.7	358.7	358.7	334.1	334.1	334.1	358.7	358.7	358.7	334.1	358.7
Turn Bay Length (m)	150.0	150.0	150.0	60.0	60.0	60.0	90.0	90.0	90.0	60.0	115.0
Base Capacity (vph)	268	1006	633	101	882	516	318	1096	575	318	1091
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	1.01	0.54	0.46	0.91	0.34	0.97	0.68	0.10	0.79	0.90
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 23 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle: 115											

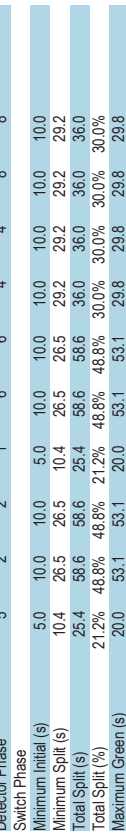
Lanes, Volumes, Timings
1: Bank & Hunt Club

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 54.1
 Intersection Capacity Utilization 94.0%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



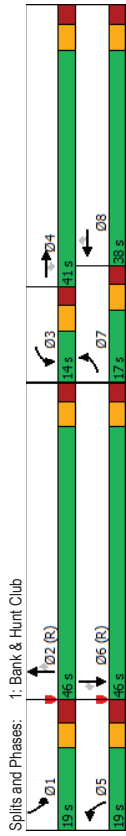
Lanes, Volumes, Timings
2: Albion & Hunt Club

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 54.1
 Intersection Capacity Utilization 94.0%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



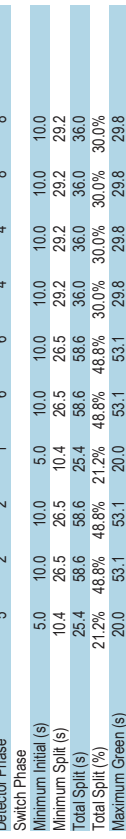
Lanes, Volumes, Timings
Future Background 2025PM Peak Hour

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 54.1
 Intersection Capacity Utilization 94.0%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
Future Background 2025PM Peak Hour

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 54.1
 Intersection Capacity Utilization 94.0%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	121	1201	23	315	947	64	7	117	48	133
Future Volume (vph)	121	1201	23	315	947	64	7	117	48	133
Lane Group Flow (vph)	121	1201	23	315	947	64	7	361	48	185
Turn Type	pm-pt	NA	Perm	pm-pt	NA	Perm	NA	Perm	NA	Perm
Protected Phases	5	2	1	6	6	4	4	8	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.4	58.6	58.6	25.4	58.6	58.6	36.0	36.0	36.0	36.0
Total Split (%)	21.2%	48.8%	48.8%	21.2%	48.8%	48.8%	30.0%	30.0%	30.0%	30.0%
Maximum Green (s)	20.0	53.1	53.1	20.0	53.1	53.1	29.8	29.8	29.8	29.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	4	4	4	5	5	5	6	6	6	24
Ad Effr Green (s)	66.0	57.1	57.1	82.2	67.9	67.9	26.2	26.2	26.2	26.2
Actuated g/C Ratio	0.55	0.48	0.48	0.68	0.57	0.57	0.22	0.22	0.22	0.22
v/c Ratio	0.36	0.77	0.03	0.86	0.50	0.08	0.04	0.90	0.72	0.51
Control Delay	6.6	11.0	0.1	47.2	18.0	2.5	30.7	56.9	93.2	41.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.6	11.0	0.1	47.2	18.0	2.5	30.7	56.9	93.2	41.8
LOS	A	B	A	D	B	A	C	E	F	D
Approach Delay	10.4			24.2			56.4			52.4
Approach LOS	B			C			E			D
Queue Length 50th (m)	4.0	25.0	0.0	47.7	71.6	0.0	1.5	71.7	10.2	34.1
Queue Length 95th (m)	m4.8	m26.1	m0.0	#97.0	#95.9	5.0	m4.0	#108.4	#29.7	55.8
Internal Link Dist (m)	334.1			564.6			188.3			429.6
Turn Bay Length (m)	65.0		40.0	100.0		40.0	35.0		30.0	
Base Capacity (vph)	487	1563	725	377	1876	836	223	446	76	414
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.77	0.03	0.84	0.50	0.08	0.03	0.81	0.63	0.45

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 96 (80%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
 Natural Cycle: 90

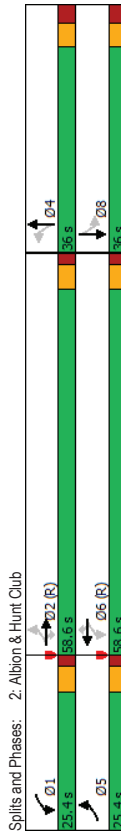
Lanes, Volumes, Timings
2: Albion & Hunt Club

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 24.1
 Intersection LOS: C
 ICU Level of Service G
 Intersection Capacity Utilization 104.0%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.

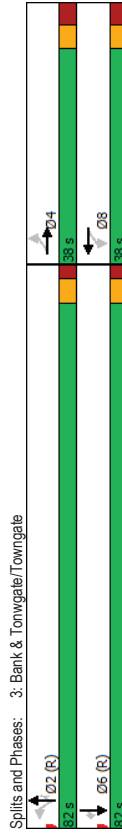
Future Background 2025PM Peak Hour
 2600 Bank Street

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	4	4	0	97	699	17	1232
Traffic Volume (vph)	150	4	4	4	0	97	699	17	1232
Future Volume (vph)	150	4	4	4	0	97	699	17	1232
Lane Group Flow (vph)	0	313	0	16	0	796	17	1232	148
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	6	6
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Minimum Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (s)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	20	20	17	17	17	17	17	11	11
Act Effr Green (s)	28.2	28.2	28.2	28.2	79.3	79.3	79.3	79.3	79.3
v/c Ratio	0.24	0.24	0.24	0.24	0.66	0.66	0.66	0.66	0.66
Control Delay	67.7	67.7	8.2	8.2	6.8	6.8	6.8	6.8	6.8
Queue Delay	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.4	0.4
Total Delay	67.8	67.8	8.2	8.2	6.8	6.8	6.8	7.2	7.2
LOS	E	E	A	A	A	A	A	A	A
Approach Delay	67.8	67.8	8.2	8.2	6.7	6.7	6.7	4.2	4.2
Approach LOS	E	E	A	A	A	A	A	A	A
Queue Length 50th (m)	61.2	61.2	0.0	0.0	17.3	0.0	19.7	0.1	0.1
Queue Length 95th (m)	#107.6	#107.6	3.9	3.9	16.9	m0.0	m29.4	m0.0	m0.0
Internal Link Dist (m)	64.2	64.2	37.0	37.0	227.9	67.1	67.1	67.1	67.1
Turn Bay Length (m)					15.0				
Base Capacity (vph)	380	380	389	389	2873	947	2192	994	994
Starvation Cap Reductn	0	0	0	0	0	0	461	524	524
Spillback Cap Reductn	1	1	0	0	119	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.83	0.04	0.04	0.29	0.02	0.71	0.31	0.31



Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 12.9
 Intersection LOS: B
 ICU Level of Service H
 Intersection Capacity Utilization 110.6%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.



Splits and Phases: 3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
4: Albion & Bank

Future Background 2025PM Peak Hour
 2600 Bank Street

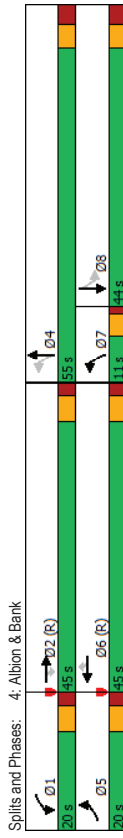
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	70	1014	279	43	592	145	158	169	177	255
Future Volume (vph)	70	1014	279	43	592	145	158	169	177	255
Lane Group Flow (vph)	70	1014	279	43	592	145	158	169	177	255
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	7	4	8	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	20.0	45.0	45.0	20.0	45.0	45.0	11.0	55.0	44.0	44.0
Total Split (%)	16.7%	37.5%	37.5%	16.7%	37.5%	37.5%	9.2%	45.8%	36.7%	36.7%
Maximum Green (s)	14.3	39.3	39.3	14.3	39.3	39.3	6.7	48.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	1	1	1	10	10	10	5	5	5	5
Ad Effr Green (s)	10.4	58.2	58.2	8.5	56.5	56.5	39.8	37.7	26.7	26.7
Actuated g/C Ratio	0.09	0.48	0.48	0.07	0.47	0.47	0.33	0.31	0.22	0.22
v/c Ratio	0.49	0.63	0.33	0.37	0.38	0.20	0.72	0.40	0.73	0.79
Control Delay	75.0	12.3	1.3	61.1	24.2	5.0	48.3	31.1	48.3	45.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.0	12.3	1.3	61.1	24.2	5.0	48.3	31.1	48.3	45.8
LOS	E	B	A	E	C	A	D	C	D	D
Approach Delay	13.3			22.6			38.4			46.8
Approach LOS	B			C			D			D
Queue Length 50th (m)	17.5	38.2	0.0	9.8	47.6	0.0	27.5	36.9	42.4	71.0
Queue Length 95th (m)	m25.7	102.4	m4.6	21.1	77.3	13.9	38.6	50.6	m54.2	m87.9
Internal Link Dist (m)	227.9			198.3			328.9			188.3
Turn Bay Length (m)	30.0	100.0	100.0	100.0	65.0	30.0	45.0			45.0
Base Capacity (vph)	198	1608	846	197	1560	739	220	690	340	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.63	0.33	0.22	0.38	0.20	0.72	0.31	0.52	0.56
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 42 (35%), Referenced to phase 2,EBT and 6:WBT, Start of Green										
Natural Cycle: 105										

Lanes, Volumes, Timings
4: Albion & Bank

HCM 2010 TWSC
7: Bank & Steveright

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 24.2
 Intersection LOS: C
 Intersection Capacity Utilization: 60.8%
 Analysis Period (min): 15
 Volume for 95th percentile queue is metered by upstream signal.

Intersection
 In/Delay, s/veh 1.3



Movement	EBU	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔
Traffic Vol. veh/h	100	1107	656	26	23	99	99
Future Vol. veh/h	100	1107	656	26	23	99	99
Conflicting Peds. #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	None	-	None
Storage Length	-	10	-	-	-	350	0
Veh in Median Storage	-	-	0	0	-	0	-
Grade, %	-	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	1	100	1107	656	26	23	99

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	682	682	0
Stage 1	-	-	669
Stage 2	-	-	756
Critical Hdwy	6.44	4.14	-
Critical Hdwy Stg 1	-	-	6.84
Critical Hdwy Stg 2	-	-	5.84
Follow-up Hdwy	2.52	2.22	-
Pot Cap-1 Maneuver	530	907	-
Stage 1	-	-	471
Stage 2	-	-	424
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	898	898	-
Mov Cap-2 Maneuver	-	-	243
Stage 1	-	-	418
Stage 2	-	-	424

Approach	EB	WB	SB
HCM Control Delay, s	0.8	0	13.4
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBL	SBR
Capacity (veh/h)	898	-	-	-	243	655
HCM Lane V/C Ratio	0.112	-	-	-	0.095	0.151
HCM Control Delay (s)	9.5	-	-	-	21.4	11.5
HCM Lane LOS	A	-	-	-	C	B
HCM 95th %tile Q(veh)	0.4	-	-	-	0.3	0.5

Appendix H

Synchro Intersection Worksheets – 2030 Future Background Conditions

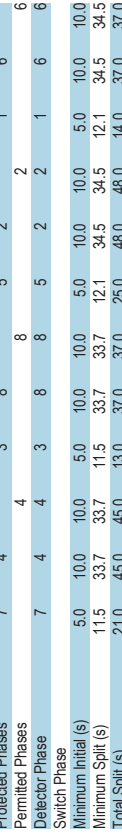
DRAFT

Lanes, Volumes, Timings
1: Bank & Hunt Club

Lanes, Volumes, Timings
1: Bank & Hunt Club

Control Type: Actuated-Coordinated	Intersection LOS: D
Maximum v/c Ratio: 0.97	ICU Level of Service E
Intersection Signal Delay: 48.4	ICU Level of Service E
Intersection Capacity Utilization 88.1%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	

Control Type: Actuated-Coordinated	Intersection LOS: D
Maximum v/c Ratio: 0.97	ICU Level of Service E
Intersection Signal Delay: 48.4	ICU Level of Service E
Intersection Capacity Utilization 88.1%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Splits and Phases:	1: Bank & Hunt Club
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Splits and Phases:	1: Bank & Hunt Club
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Splits and Phases:	1: Bank & Hunt Club
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Splits and Phases:	1: Bank & Hunt Club
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Future Volume (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Future Flow (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.6	44.3	44.3	6.7	32.8	32.8
Actuated g/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.34	0.30	0.97	0.29	0.71	0.89	0.03	0.46	0.31	0.26
Control Delay	58.3	35.6	5.4	53.0	84.9	14.3	60.5	40.7	0.1	63.4	36.4	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.0	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.4	53.0	84.9	14.3	60.5	45.7	0.1	63.4	36.4	1.1
LOS	E	D	A	D	F	B	E	D	A	E	D	A
Approach Delay	31.8			72.7			48.1			30.8		
Approach LOS	C			E			D			C		
Queue Length 50th (m)	14.2	76.3	0.0	5.9	117.8	9.7	27.7	134.5	0.0	9.8	26.7	0.0
Queue Length 95th (m)	23.2	98.0	17.0	m11.5	#166.3	24.7	51.2	#167.3	0.0	18.1	39.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	655	86	910	613	465	1212	662	184	871	556
Starvation Cap Reductn	0	0	0	0	0	0	0	90	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.34	0.29	0.97	0.29	0.62	0.96	0.03	0.45	0.31	0.26

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Future Volume (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Future Flow (vph)	121	704	221	25	885	177	287	1082	22	83	269	142
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.6	44.3	44.3	6.7	32.8	32.8
Actuated g/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.34	0.30	0.97	0.29	0.71	0.89	0.03	0.46	0.31	0.26
Control Delay	58.3	35.6	5.4	53.0	84.9	14.3	60.5	40.7	0.1	63.4	36.4	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.0	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.4	53.0	84.9	14.3	60.5	45.7	0.1	63.4	36.4	1.1
LOS	E	D	A	D	F	B	E	D	A	E	D	A
Approach Delay	31.8			72.7			48.1			30.8		
Approach LOS	C			E			D			C		
Queue Length 50th (m)	14.2	76.3	0.0	5.9	117.8	9.7	27.7	134.5	0.0	9.8	26.7	0.0
Queue Length 95th (m)	23.2	98.0	17.0	m11.5	#166.3	24.7	51.2	#167.3	0.0	18.1	39.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	655	86	910	613	465	1212	662	184	871	556
Starvation Cap Reductn	0	0	0	0	0	0	0	90	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.34	0.29	0.97	0.29	0.62	0.96	0.03	0.45	0.31	0.26

Intersection Summary

Intersection Summary

Cycle Length: 120

Cycle Length: 120

Actuated Cycle Length: 120

Actuated Cycle Length: 120

Offset: 65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Offset: 65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 95

Natural Cycle: 95

Lanes, Volumes, Timings
2: Albion & Hunt Club

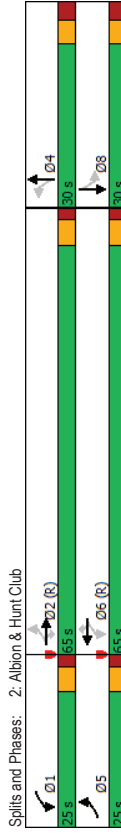
Future Background 2030AM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	58	719	8	181	994	79	6	99	41	84
Future Volume (vph)	58	719	8	181	994	79	6	99	41	84
Lane Group Flow (vph)	58	719	8	181	994	79	6	333	41	153
Turn Type	pm-pt	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	25.0	65.0	65.0	25.0	65.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	20.8%	54.2%	54.2%	20.8%	54.2%	25.0%	25.0%	25.0%	25.0%	25.0%
Maximum Green (s)	19.6	59.5	59.5	19.6	59.5	23.8	23.8	23.8	23.8	23.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	2	2	2	4	4	2	2	2	2	2
Act Effr Green (s)	77.3	70.3	70.3	84.2	75.6	75.6	22.7	22.7	22.7	22.7
Actuated g/C Ratio	0.64	0.59	0.59	0.70	0.63	0.63	0.19	0.19	0.19	0.19
v/c Ratio	0.18	0.39	0.01	0.39	0.50	0.08	0.03	0.94	0.72	0.50
Control Delay	1.9	3.4	0.0	8.2	14.1	2.6	61.3	89.5	103.5	40.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	1.9	3.4	0.0	8.2	14.1	2.6	61.3	89.5	103.5	40.4
LOS	A	A	A	A	B	A	E	F	F	D
Approach Delay	3.3			12.5			89.0			53.7
Approach LOS	A			B			F			D
Queue Length 50th (m)	0.1	0.7	0.0	12.7	67.5	0.1	1.3	56.4	9.0	25.5
Queue Length 95th (m)	m0.3	2.3	m0.0	20.3	87.2	6.2	m4.3	#107.7	#28.7	46.6
Internal Link Dist (m)	334.1			554.6			188.3			429.6
Turn Bay Length (m)	65.0			100.0			40.0			30.0
Base Capacity (vph)	475	1852	723	564	1971	933	198	370	60	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.39	0.01	0.32	0.50	0.08	0.03	0.90	0.68	0.48

Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Background 2030AM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.94
Intersection Signal Delay: 22.9
Intersection LOS: C
ICU Level of Service D
Intersection Capacity Utilization 81.8%
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

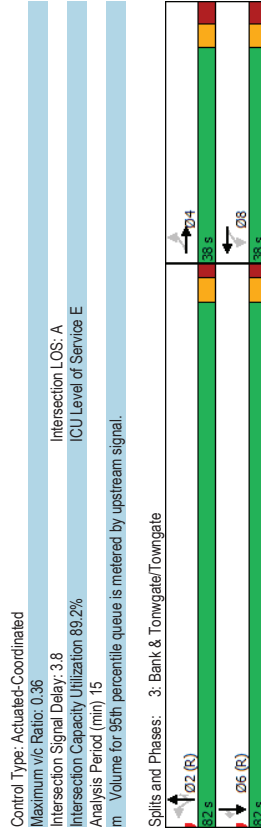


Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	31	1	2	1	51	1392	13	475	40
Traffic Volume (vph)	31	1	2	1	51	1392	13	475	40
Future Volume (vph)	0	65	0	11	0	1443	13	475	40
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	2	6
Permitted Phases	4	4	8	8	2	2	2	2	6
Detector Phase	4	4	8	8	2	2	2	2	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Minimum Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (s)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Total Split (%)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Maximum Green (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
Yellow Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Total Lost Time (s)									
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	1	1	9	9	9	9	9	9	9
Act Effr Green (s)	14.3	14.3	14.3	14.3	97.7	97.7	97.7	97.7	97.7
Actuated g/C Ratio	0.12	0.12	0.12	0.12	0.81	0.81	0.81	0.81	0.81
v/c Ratio	0.36	0.07	0.33	0.33	0.01	0.18	0.04	0.04	0.04
Control Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.6	1.8	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.8	1.8	1.8
LOS	C	C	C	C	A	A	A	A	A
Approach Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.7	1.8	1.8
Approach LOS	C	C	C	C	A	A	A	A	A
Queue Length 50th (m)	7.2	0.7	0.7	0.7	13.4	0.0	7.3	0.0	0.0
Queue Length 95th (m)	17.2	5.2	5.2	5.2	m23.5	m0.0	22.7	0.7	0.7
Internal Link Dist (m)	64.2	37.0	37.0	37.0	227.9	67.1			
Turn Bay Length (m)					15.0				
Base Capacity (vph)	359	359	359	359	4350	1173	2623	1062	1062
Starvation Cap Reductn	0	0	0	0	0	0	1322	0	0
Spillback Cap Reductn	3	0	0	0	556	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.03	0.03	0.03	0.38	0.01	0.37	0.04	0.04

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	30 (25%), Referenced to phase 2:NBTL and 6:SBT, Start of Green
Natural Cycle:	85



Lanes, Volumes, Timings
4: Albion & Bank

Future Background 2030AM Peak Hour
2600 Bank Street

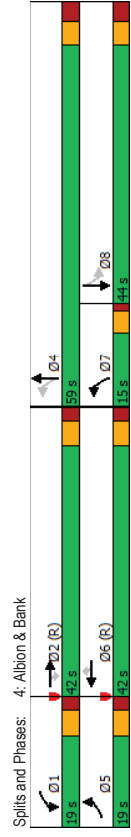
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	21	382	95	29	1106	117	298	214	100	130
Future Volume (vph)	21	382	95	29	1106	117	298	214	100	130
Lane Group Flow (vph)	21	382	95	29	1106	117	298	243	100	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prot+yp	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phase	5	2	2	1	6	6	7	4	4	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	64.6	64.6	7.8	67.7	67.7	36.6	34.5	19.5	19.5
Actuated g/C Ratio	0.06	0.54	0.54	0.06	0.56	0.56	0.30	0.29	0.16	0.16
v/c Ratio	0.21	0.23	0.12	0.29	0.60	0.14	0.94	0.49	0.59	0.56
Control Delay	67.1	17.9	4.3	60.1	22.6	4.4	74.8	36.3	58.1	49.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.1	17.9	4.3	60.1	22.6	4.4	74.8	36.3	58.1	49.8
LOS	E	B	A	E	C	A	E	D	E	D
Approach Delay	17.4			21.7			57.5		53.1	
Approach LOS	B			C			E		D	
Queue Length 50th (m)	0.0	19.3	0.0	6.6	70.3	0.0	61.4	47.6	24.6	36.7
Queue Length 95th (m)	13.6	46.4	7.3	16.0	#181.5	11.5	68.7	55.3	28.4	0.0
Internal Link Dist (m)	227.9			198.3			328.9		188.3	
Turn Bay Length (m)	30.0	100.0	100.0	65.0	30.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	183	1684	826	170	1852	836	316	753	328	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.23	0.12	0.17	0.60	0.14	0.94	0.32	0.30	0.29

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 56 (47%), Referenced to phase 2,EBT and 6,WBT, Start of Green
 Natural Cycle: 105

Lanes, Volumes, Timings
4: Albion & Bank

Future Background 2030AM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 31.6
 Intersection LOS: C
 ICU Level of Service D
 Intersection Capacity Utilization 77.0%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.



HCM 2010 TWSC
7: Bank & Steveright

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Background 2030AM Peak Hour
2600 Bank Street

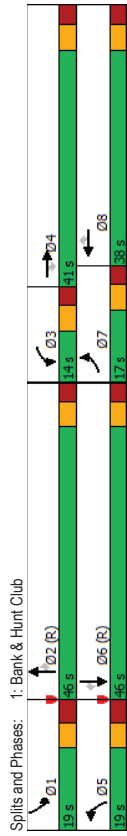
Future Background 2030PM Peak Hour
2600 Bank Street

Intersection	EBL	EBT	WBT	WBR	SBL	SBR
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	4	4	4	4	4
Traffic Vol, veh/h	40	445	1115	24	30	114
Future Vol, veh/h	40	445	1115	24	30	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	10	-	-	-	350	0
Veh in Median Storage, #	-	0	0	-	-	-
Grade, %	-	0	0	-	-	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	445	1115	24	30	114
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	1139	0	0	1430	570	
Stage 1	-	-	-	1127	-	-
Stage 2	-	-	-	303	-	-
Critical Hwy	4.14	-	-	6.84	6.94	-
Critical Hwy Stg 1	-	-	-	5.84	-	-
Critical Hwy Stg 2	-	-	-	3.52	3.32	-
Follow-up Hwy	2.22	-	-	3.52	3.32	-
Pot Cap-1 Maneuver	609	-	-	125	465	-
Stage 1	-	-	-	271	-	-
Stage 2	-	-	-	723	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	609	-	-	117	465	-
Mov Cap-2 Maneuver	-	-	-	208	-	-
Stage 1	-	-	-	253	-	-
Stage 2	-	-	-	723	-	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.9	0	17.3			
HCM LOS	C		C			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	609	-	-	208	465	
HCM Lane V/C Ratio	0.066	-	-	0.144	0.245	
HCM Control Delay (s)	11.3	-	-	25.2	15.2	
HCM Lane LOS	B	-	-	D	C	
HCM 95th %ile Q(veh)	0.2	-	-	0.5	1	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Traffic Volume (vph)	149	1016	340	46	800	177	310	491	60	252	1040
Future Volume (vph)	149	1016	340	46	800	177	310	491	60	252	1040
Lane Group Flow (vph)	149	1016	340	46	800	177	310	491	60	252	1040
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2	2	1	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6
Switch Phase											
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5
Total Split (s)	17.0	41.0	41.0	14.0	38.0	38.0	19.0	46.0	46.0	19.0	46.0
Total Split (%)	14.2%	34.2%	34.2%	11.7%	31.7%	31.7%	15.8%	38.3%	38.3%	15.8%	38.3%
Maximum Green (s)	10.5	34.3	34.3	7.5	31.3	31.3	11.9	39.5	39.5	11.9	39.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21		4	4		22	22	22	22	22
Act Effort Green(s)	9.8	37.1	37.1	7.1	32.0	32.0	11.9	39.7	39.7	11.7	39.5
Actuated g/C Ratio	0.08	0.31	0.31	0.06	0.27	0.27	0.10	0.33	0.33	0.10	0.33
v/c Ratio	0.59	1.01	0.54	0.48	0.91	0.34	0.97	0.45	0.10	0.81	0.95
Control Delay	63.2	72.9	10.9	74.5	61.6	19.1	101.9	34.1	3.8	72.7	57.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.2	0.0	0.0	0.0
Total Delay	63.2	72.9	10.9	74.5	61.6	19.1	101.9	35.3	3.8	72.7	57.8
LOS	E	E	B	E	E	B	F	D	A	E	A
Approach Delay	57.9			54.8			57.1			53.4	
Approach LOS	E			D			E			D	
Queue Length 50th (m)	17.6	~142.1	10.6	11.5	71.4	4.4	38.0	38.4	0.3	30.3	125.4
Queue Length 95th (m)	28.4	#182.6	38.1	m#22.7	#126.6	31.1	m#64.4	m#1.4	m#5.4	#168.2	24.9
Internal Link Dist (m)	358.7			334.1			67.1			340.8	
Turn Bay Length (m)	150.0			60.0			90.0			40.0	115.0
Base Capacity (vph)	268	1006	632	101	882	516	318	1096	575	318	1091
Starvation Cap Reductn	0	0	0	0	0	0	0	372	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	1.01	0.54	0.46	0.91	0.34	0.97	0.68	0.10	0.79	0.95
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 23 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle: 125											

Lanes, Volumes, Timings
 1: Bank & Hunt Club

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.01
 Intersection Signal Delay: 55.8
 Intersection Capacity Utilization 95.8%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 ~ Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
 2: Albion & Hunt Club

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	121	1201	23	315	947	64	7	117	48	133
Future Volume (vph)	121	1201	23	315	947	64	7	117	48	133
Lane Group Flow (vph)	121	1201	23	315	947	64	7	361	48	185
Turn Type	pm-pt	NA	Perm	pm-pt	NA	Perm	NA	Perm	NA	Perm
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.4	58.6	58.6	25.4	58.6	58.6	36.0	36.0	36.0	36.0
Total Split (%)	21.2%	48.8%	48.8%	21.2%	48.8%	48.8%	30.0%	30.0%	30.0%	30.0%
Maximum Green (s)	20.0	53.1	53.1	20.0	53.1	53.1	29.8	29.8	29.8	29.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	4	4	4	5	5	5	6	6	6	24
Ad Effr Green (s)	66.0	57.1	57.1	82.2	67.9	67.9	26.2	26.2	26.2	26.2
Actuated g/C Ratio	0.55	0.48	0.48	0.68	0.57	0.57	0.22	0.22	0.22	0.22
v/c Ratio	0.36	0.77	0.03	0.86	0.50	0.08	0.04	0.90	0.72	0.51
Control Delay	6.6	11.0	0.1	47.2	18.0	2.5	30.7	56.9	93.2	41.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.6	11.0	0.1	47.2	18.0	2.5	30.7	56.9	93.2	41.8
LOS	A	B	A	D	B	A	C	E	F	D
Approach Delay	10.4			24.2			56.4			52.4
Approach LOS	B			C			E			D
Queue Length 50th (m)	4.0	25.0	0.0	47.7	71.6	0.0	1.5	71.7	10.2	34.1
Queue Length 95th (m)	m4.8	m26.1	m0.0	#97.0	#95.9	5.0	m4.0	#108.5	#29.7	55.8
Internal Link Dist (m)	334.1			564.6			188.3			429.6
Turn Bay Length (m)	65.0		40.0	100.0		40.0	35.0		30.0	
Base Capacity (vph)	487	1563	725	377	1876	836	223	446	76	414
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.77	0.03	0.84	0.50	0.08	0.03	0.81	0.63	0.45

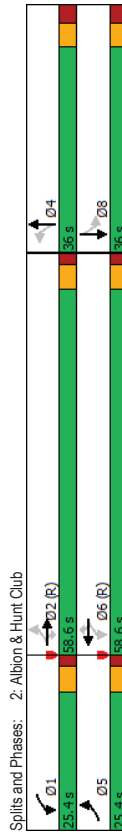
Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 96 (80%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
 Natural Cycle: 90

Lanes, Volumes, Timings
2: Albion & Hunt Club

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 24.1
 Intersection LOS: C
 Intersection Capacity Utilization: 104.0%
 ICU Level of Service: G
 Analysis Period (min): 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.

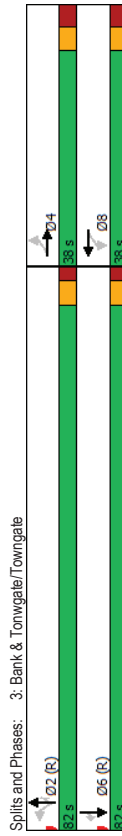
Future Background 2030PM Peak Hour
 2600 Bank Street



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	150	4	4	0	97	699	17	1311	148
Future Volume (vph)	150	4	4	0	97	699	17	1311	148
Lane Group Flow (vph)	0	313	0	16	0	796	17	1311	148
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	6	6
Switch Phase	4	4	8	8	2	2	2	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Total Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	20	20	20	17	17	17	17	11	11
Act Effr Green (s)	28.2	28.2	28.2	28.2	79.3	79.3	79.3	79.3	79.3
v/c Ratio	0.24	0.24	0.24	0.24	0.66	0.66	0.66	0.66	0.66
Control Delay	67.7	67.7	8.2	8.2	0.28	0.28	0.28	0.60	0.15
Queue Delay	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.6	0.5
Total Delay	67.8	67.8	8.2	8.2	6.8	6.8	6.8	5.0	0.7
LOS	E	E	A	A	A	A	A	A	A
Approach Delay	67.8	67.8	8.2	8.2	6.7	6.7	6.7	4.5	0.7
Approach LOS	E	E	A	A	A	A	A	A	A
Queue Length 50th (m)	61.2	61.2	0.0	0.0	17.3	0.0	20.7	0.0	0.0
Queue Length 95th (m)	#107.6	#107.6	3.9	3.9	16.9	m0.0	m29.1	m0.0	m0.0
Internal Link Dist (m)	64.2	64.2	37.0	37.0	227.9	227.9	67.1	67.1	67.1
Turn Bay Length (m)					15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	380	380	389	389	2853	947	2192	994	994
Starvation Cap Reductn	0	0	0	0	0	0	446	532	532
Spillback Cap Reductn	1	1	0	0	119	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.83	0.04	0.04	0.29	0.02	0.75	0.32	0.32
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 9 (8%), Referenced to phase 2:NBL and 6:SBT, Start of Green									
Natural Cycle: 85									

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 12.8
 Intersection LOS: B
 ICU Level of Service H
 Intersection Capacity Utilization 112.9%
 Analysis Period (min) 15
 # 96th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 96th percentile queue is metered by upstream signal.



Splits and Phases: 3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
4: Albion & Bank

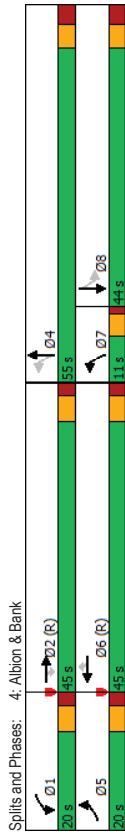
Future Background 2030PM Peak Hour
 2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	70	1078	279	43	592	145	158	169	177	255
Future Volume (vph)	70	1078	279	43	592	145	158	169	177	255
Lane Group Flow (vph)	70	1078	279	43	592	145	158	169	177	255
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	7	4	8	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	20.0	45.0	45.0	20.0	45.0	45.0	11.0	55.0	44.0	44.0
Total Split (%)	16.7%	37.5%	37.5%	16.7%	37.5%	37.5%	9.2%	45.8%	36.7%	36.7%
Maximum Green (s)	14.3	39.3	39.3	14.3	39.3	39.3	6.7	48.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	1	1	1	10	10	10	5	5	5	5
Ad Effr Green (s)	10.4	58.2	58.2	8.5	56.5	56.5	39.8	37.7	26.7	26.7
Actuated g/C Ratio	0.09	0.48	0.48	0.07	0.47	0.47	0.33	0.31	0.22	0.22
v/c Ratio	0.49	0.67	0.33	0.37	0.38	0.20	0.72	0.40	0.73	0.79
Control Delay	74.7	13.9	1.4	61.1	24.2	5.0	48.3	31.1	48.3	45.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.7	13.9	1.4	61.1	24.2	5.0	48.3	31.1	48.3	45.8
LOS	E	B	A	E	C	A	D	C	D	D
Approach Delay	14.4			22.6			38.4		46.8	
Approach LOS	B			C			D		D	
Queue Length 50th (m)	17.5	41.1	0.0	9.8	47.6	0.0	27.5	36.9	42.4	71.0
Queue Length 95th (m)	m24.2	#162.6	m4.5	21.1	77.3	13.9	38.6	50.6	m54.2	m87.9
Internal Link Dist (m)	227.9			198.3			328.9		188.3	
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	198	1608	846	197	1560	739	220	690	340	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.67	0.33	0.22	0.38	0.20	0.72	0.31	0.52	0.56
Intersection Summary										
Cycle Length: 120										
Actuated Cycle Length: 120										
Offset: 42 (35%), Referenced to phase 2,EBT and 6:WBT, Start of Green										
Natural Cycle: 105										

Lanes, Volumes, Timings
4: Albion & Bank

HCM 2010 TWSC
7: Bank & Steveright

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 24.5
 Intersection LOS: C
 Intersection Capacity Utilization: 82.6%
 ICU Level of Service: E
 Analysis Period (min): 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Intersection
 In Delay, s/veh 1.3

Movement	EBU	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔
Traffic Vol, veh/h	100	1178	656	26	23	99	99
Future Vol, veh/h	100	1178	656	26	23	99	99
Conflicting Peds. #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	None	-	None	-	None
Storage Length	-	10	-	-	-	350	0
Veh in Median Storage, #	-	-	0	-	-	0	-
Grade, %	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	1	100	1178	656	26	23	99

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	682	682	0
Stage 1	-	-	669
Stage 2	-	-	791
Critical Hdwy	6.44	4.14	-
Critical Hdwy Stg 1	-	-	6.84
Critical Hdwy Stg 2	-	-	5.84
Follow-up Hdwy	2.52	2.22	-
Pot Cap-1 Maneuver	530	907	-
Stage 1	-	-	471
Stage 2	-	-	407
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	898	898	-
Mov Cap-2 Maneuver	-	-	237
Stage 1	-	-	418
Stage 2	-	-	407

Approach	EB	WB	SB
HCM Control Delay, s	0.8	0	13.4
HCM LOS		B	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBL	SBR
Capacity (veh/h)	898	-	-	-	237	655
HCM Lane V/C Ratio	0.112	-	-	-	0.097	0.151
HCM Control Delay (s)	9.5	-	-	-	21.8	11.5
HCM Lane LOS	A	-	-	-	C	B
HCM 95th %tile Q(veh)	0.4	-	-	-	0.3	0.5

Appendix I

Sightline Analysis

DRAFT



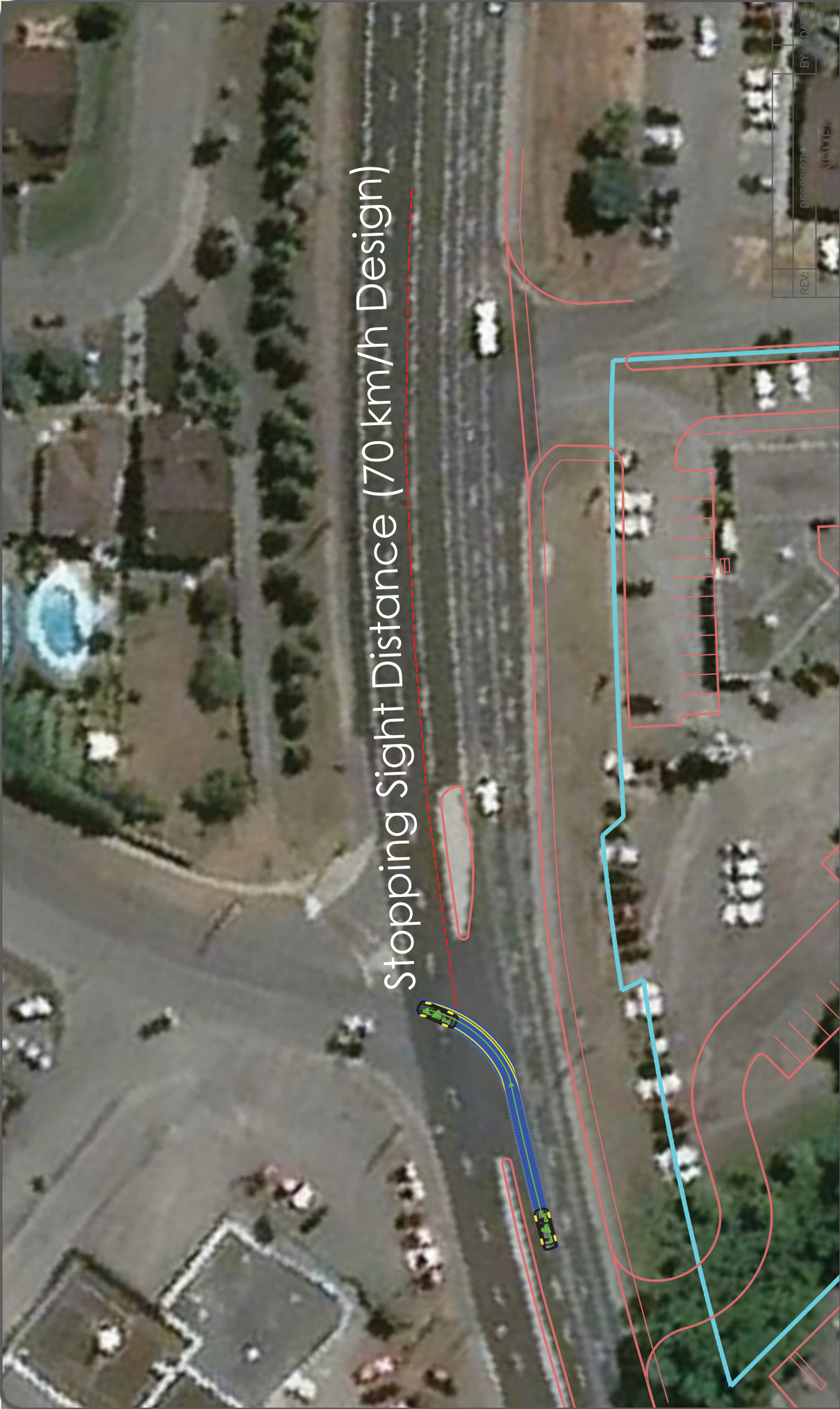
Required Sightline (70 km/h Design)

APP: []
 DESCRIPTION: []
 REVISIONS: []

SITE:	2600 Bank Street		DRAWING NO.	001		PROJECT NO.	2021-010		DATE.	21/06/09	
	TITLE:	West Access Sightlines		NTS	SCALE AT A4.		JK	DRAWN.		AH	CHECKED.




CGH Transportation Inc.
 13 Markham Drive, Ottawa
 ON K2G 3Z1



Stopping Sight Distance (70 km/h Design)

REV.	DESCRIPTION	BY	DATE

SITE:	2600 Bank Street		DRAWING NO.	002	PROJECT NO.	2021-010	DATE.	21/06/09
	TITLE:		NTS SCALE AT A4.	JK DRAWN.	AH CHECKED.	01 REVISION.		


CGH Transportation Inc.
 13 Markham Drive, Ottawa
 ON K2G 3Z1

Appendix J

MMLOS Analysis

DRAFT

Multi-Modal Level of Service - Segments Form

Consultant Scenario Comments	CGH Transportation Inc.	Project Date	2021-010
	Existing/Future Conditions		2021-03-18

SEGMENTS			Bank St Ex/Fut	Section 2	Section 3
Pedestrian	Sidewalk Width	F	≥ 2 m		
	Boulevard Width		< 0.5		
	Avg Daily Curb Lane Traffic Volume		> 3000		
	Operating Speed		> 60 km/h		
	On-Street Parking		no		
	Exposure to Traffic PLoS		F	-	-
	Effective Sidewalk Width				
Pedestrian Volume					
Crowding PLoS	A	-	-		
Level of Service	F	-	-		
Bicycle	Type of Cycling Facility	F	Mixed Traffic		
	Number of Travel Lanes		4-5 lanes total		
	Operating Speed		≥ 60 km/h		
	# of Lanes & Operating Speed LoS		F	-	-
	Bike Lane (+ Parking Lane) Width				
	Bike Lane Width LoS		-	-	-
	Bike Lane Blockages				
	Blockage LoS		-	-	-
	Median Refuge Width (no median = < 1.8 m)		< 1.8 m refuge		
	No. of Lanes at Unsignalized Crossing		4-5 lanes		
	Sidestreet Operating Speed		≤ 40 km/h		
	Unsignalized Crossing - Lowest LoS		B	-	-
Level of Service	F	-	-		
Transit	Facility Type	-			
	Friction or Ratio Transit:Posted Speed				
	Level of Service		-	-	-
Truck	Truck Lane Width	A	≤ 3.5 m		
	Travel Lanes per Direction		> 1		
	Level of Service		A	-	-

Appendix K

Synchro Intersection Worksheets – 2025 Future Total Conditions

DRAFT

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Total 2025AM Peak Hour
2600 Bank Street

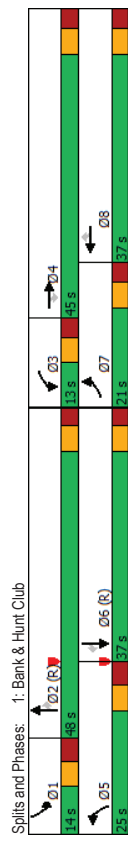
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	7	4	4	3	8	8	5	2	2	1	6	
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	250	30	886	177	290	1027	22	83	313	143
Future Volume (vph)	121	704	250	30	886	177	290	1027	22	83	313	143
Lane Group Flow (vph)	121	704	250	30	886	177	290	1027	22	83	313	143
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	
Permitted Phase	7	4	4	3	8	8	5	2	2	1	6	
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.7	44.3	44.3	6.7	32.7	32.7
Actuated G/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.37	0.36	0.97	0.29	0.71	0.85	0.03	0.46	0.36	0.26
Control Delay	58.3	35.6	5.3	55.4	85.1	14.2	61.0	37.0	0.1	63.4	37.2	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.4	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.3	55.4	85.1	14.2	61.0	40.4	0.1	63.4	37.2	1.1
LOS	E	D	A	E	F	B	E	D	A	E	D	A
Approach Delay	31.1			72.8			44.2				31.7	
Approach LOS	C			E			D				C	
Queue Length 50th (m)	14.2	76.3	0.0	7.1	117.8	9.7	27.4	125.4	0.0	9.8	31.5	0.0
Queue Length 95th (m)	23.2	98.0	17.8	m14.1	#166.8	24.7	51.7	#153.1	0.0	18.1	45.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1				340.8	
Turn Bay Length (m)	150.0			60.0			90.0				40.0	
Base Capacity (vph)	357	1135	674	86	910	613	465	1212	662	184	870	556
Starvation Cap Reductn	0	0	0	0	0	0	0	112	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.37	0.35	0.97	0.29	0.62	0.93	0.03	0.45	0.36	0.26

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 95

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Total 2025AM Peak Hour
2600 Bank Street

Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.97
Intersection Signal Delay:	46.8
Intersection LOS:	D
IOU Level of Service E	
Intersection Capacity Utilization:	66.5%
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Splits and Phases: 1: Bank & Hunt Club

Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Total 2025AM Peak Hour
2600 Bank Street

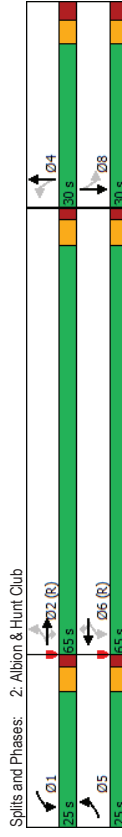
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	58	719	8	191	1000	79	6	99	41	84
Future Traffic Volume (vph)	58	719	8	191	1000	79	6	99	41	84
Lane Group Flow (vph)	58	719	8	191	1000	79	6	366	41	153
Turn Type	pm-pt	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	25.0	65.0	65.0	25.0	65.0	65.0	30.0	30.0	30.0	30.0
Total Split (%)	20.8%	54.2%	54.2%	20.8%	54.2%	54.2%	25.0%	25.0%	25.0%	25.0%
Maximum Green (s)	19.6	59.5	59.5	19.6	59.5	59.5	23.8	23.8	23.8	23.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	2	2	2	4	4	4	2	2	2	2
Act Effr Green (s)	76.9	70.0	70.0	84.4	75.5	75.5	22.7	22.7	22.7	22.7
Actuated G/C Ratio	0.64	0.58	0.58	0.70	0.63	0.63	0.19	0.19	0.19	0.19
v/c Ratio	0.18	0.39	0.01	0.40	0.51	0.08	0.03	0.94	0.72	0.50
Control Delay	2.0	3.6	0.0	8.5	14.2	2.6	61.3	89.5	102.9	40.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	2.0	3.6	0.0	8.5	14.2	2.6	61.3	89.5	102.9	40.4
LOS	A	A	A	A	B	A	E	F	F	D
Approach Delay	3.5			12.6			89.0			53.6
Approach LOS	A			B			F			D
Queue Length 50th (m)	0.1	0.7	0.0	13.5	68.0	0.1	1.3	57.0	9.0	25.5
Queue Length 95th (m)	m0.3	2.3	m0.0	21.4	87.8	6.2	m4.4	#108.6	#28.7	46.6
Internal Link Dist (m)	334.1			554.6			188.3			429.6
Turn Bay Length (m)	65.0			40.0	100.0		40.0	35.0		30.0
Base Capacity (vph)	474	1843	720	563	1970	932	198	371	60	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.39	0.01	0.34	0.51	0.08	0.03	0.91	0.68	0.48

Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 120
Offset: 27 (23%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
Natural Cycle: 70

Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Total 2025AM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.94
Intersection Signal Delay: 23.0
Intersection LOS: C
Intersection Capacity Utilization 82.1%
IOU Level of Service E
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

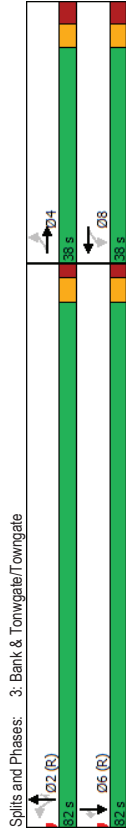
Future Total 2025AM Peak Hour
2600 Bank Street

Future Total 2025AM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	31	1	2	1	51	1321	13	563	40
Traffic Volume (vph)	31	1	2	1	51	1321	13	563	40
Future Volume (vph)	0	65	0	11	0	1372	13	563	40
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	2	6
Permitted Phases	4	4	8	8	2	2	2	2	6
Detector Phase	4	4	8	8	2	2	2	2	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Minimum Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (s)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Total Split (%)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Maximum Green (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
Yellow Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Total Lost Time (s)									
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	1	1	9	9	9	9	9	9	9
Act Effr Green (s)	14.3	14.3	14.3	14.3	97.7	97.7	97.7	97.7	97.7
Actuated g/C Ratio	0.12	0.12	0.07	0.07	0.81	0.81	0.81	0.81	0.81
v/c Ratio	0.36	0.36	0.02	0.02	0.32	0.01	0.21	0.04	0.04
Control Delay	30.5	25.9	2.6	2.6	0.0	0.0	3.7	1.7	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	30.5	25.9	2.6	2.6	0.0	0.0	3.8	1.7	1.7
LOS	C	C	A	A	A	A	A	A	A
Approach Delay	30.5	25.9	2.6	2.6	3.7	3.7	3.7	3.7	3.7
Approach LOS	C	C	A	A	A	A	A	A	A
Queue Length 50th (m)	7.2	0.7	0.7	0.7	13.1	0.0	8.6	0.0	0.0
Queue Length 95th (m)	17.2	5.2	5.2	5.2	m23.1	m0.0	25.7	0.9	0.9
Internal Link Dist (m)	64.2	37.0	37.0	37.0	227.9	67.1	67.1	67.1	67.1
Turn Bay Length (m)					15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	359	359	359	359	4311	1173	2623	1062	1062
Starvation Cap Reductn	0	0	0	0	0	0	1208	0	0
Spillback Cap Reductn	3	0	0	0	524	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.03	0.03	0.03	0.36	0.01	0.39	0.04	0.04

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	30 (25%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	85

Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.36
Intersection Signal Delay:	3.9
Intersection LOS:	A
IOU Level of Service E	
Intersection Capacity Utilization:	69.2%
Analysis Period (min):	15
m:	Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
4: Albion & Bank

Future Total 2025AM Peak Hour
2600 Bank Street

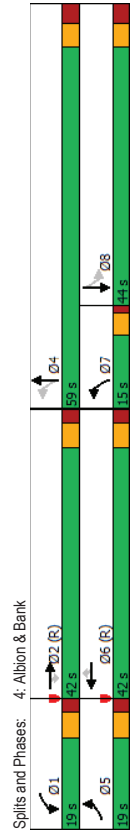
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	21	460	95	32	1053	120	298	214	110	130
Future Volume (vph)	21	460	95	32	1053	120	298	214	110	130
Lane Group Flow (vph)	21	460	95	32	1053	120	298	243	110	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prt+yp	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phase	5	2	2	1	6	6	7	4	4	8
Detector Phase	5	2	2	1	6	6	7	4	4	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	63.7	63.7	8.0	67.0	67.0	37.3	35.2	20.2	20.2
Actuated g/C Ratio	0.06	0.53	0.53	0.07	0.56	0.56	0.31	0.29	0.17	0.17
v/c Ratio	0.21	0.28	0.12	0.31	0.57	0.14	0.92	0.48	0.62	0.54
Control Delay	66.0	20.5	5.4	60.5	22.5	4.6	69.3	35.5	60.6	49.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.0	20.5	5.4	60.5	22.5	4.6	69.3	35.5	60.6	49.3
LOS	E	C	A	E	C	A	E	D	E	D
Approach Delay	19.7			21.7			54.1		53.9	
Approach LOS	B			C			D		D	
Queue Length 50th (m)	5.2	26.6	0.0	7.3	67.3	0.0	60.6	46.9	27.2	36.8
Queue Length 95th (m)	13.7	57.0	8.6	17.2	#167.6	12.2	68.7	55.3	31.3	36.0
Internal Link Dist (m)	227.9			198.3			328.9		188.3	
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	183	1661	817	170	1833	828	324	753	328	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.28	0.12	0.19	0.57	0.14	0.92	0.32	0.34	0.29

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 56 (47%), Referenced to phase 2,EBT and 6:WBT, Start of Green
 Natural Cycle: 105

Lanes, Volumes, Timings
4: Albion & Bank

Future Total 2025AM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 31.4
 Intersection LOS: C
 IOU Level of Service D
 Intersection Capacity Utilization 75.4%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.



HCM 2010 TWSC
5: West Access & Bank

HCM 2010 TWSC
6: East Access & Bank

Future Total 2025AM Peak Hour
2600 Bank Street

Future Total 2025AM Peak Hour
2600 Bank Street

Intersection													
Int Delay, s/veh												0	
Movement	EBT	EBR	WBL	WBT	NBL	NBR							
Lane Configurations	↔	↔	↔	↔	↔	↔							
Traffic Vol, veh/h	492	81	0	1182	0	1							
Future Vol, veh/h	492	81	0	1182	0	1							
Conflicting Peds, #/hr	0	0	0	0	0	0							
Sign Control	Free	Free	Free	Free	Stop	Stop							
RT Channelized	-	None	-	None	-	None							
Storage Length	-	-	-	-	-	0							
Veh in Median Storage, #	0	-	-	0	0	-							
Grade, %	0	-	-	0	0	-							
Peak Hour Factor	100	100	100	100	100	100							
Heavy Vehicles, %	7	2	2	4	2	2							
Mvmt Flow	492	81	0	1182	0	1							
Major/Minor	Major1	Major2	Minor1										
Conflicting Flow All	0	0	-	-	-	287							
Stage 1	-	-	-	-	-	-							
Stage 2	-	-	-	-	-	-							
Critical Hdwy	-	-	-	-	-	6.94							
Critical Hdwy Stg 1	-	-	-	-	-	-							
Critical Hdwy Stg 2	-	-	-	-	-	-							
Follow-up Hdwy	-	-	-	-	-	3.32							
Pot Cap-1 Maneuver	-	0	0	0	0	710							
Stage 1	-	0	0	0	0	-							
Stage 2	-	0	0	0	0	-							
Platoon blocked, %	-	-	-	-	-	-							
Mov Cap-1 Maneuver	-	-	-	-	-	710							
Mov Cap-2 Maneuver	-	-	-	-	-	-							
Stage 1	-	-	-	-	-	-							
Stage 2	-	-	-	-	-	-							
Approach	EB	WB	NB										
HCM Control Delay, s	0	0	10.1										
HCM LOS				B									
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT									
Capacity (veh/h)	710	-	-	-									
HCM Lane V/C Ratio	0.001	-	-	-									
HCM Control Delay (s)	10.1	-	-	-									
HCM Lane LOS	B	-	-	-									
HCM 95th %tile Q(veh)	0	-	-	-									

Intersection													
Int Delay, s/veh												0.4	
Movement	EBT	EBR	WBL	WBT	NBL	NBR							
Lane Configurations	↔	↔	↔	↔	↔	↔							
Traffic Vol, veh/h	472	11	16	1066	26	4							
Future Vol, veh/h	472	11	16	1066	26	4							
Conflicting Peds, #/hr	0	0	0	0	0	0							
Sign Control	Free	Free	Free	Free	Stop	Stop							
RT Channelized	-	None	-	None	-	None							
Storage Length	-	250	500	-	-	0							
Veh in Median Storage, #	0	-	-	0	0	-							
Grade, %	0	-	-	0	0	-							
Peak Hour Factor	100	100	100	100	100	100							
Heavy Vehicles, %	7	2	2	4	2	2							
Mvmt Flow	472	11	16	1066	26	4							
Major/Minor	Major1	Major2	Minor1										
Conflicting Flow All	0	0	483	0	1037	236							
Stage 1	-	-	-	-	472	-							
Stage 2	-	-	-	-	-	565							
Critical Hdwy	-	-	4.14	-	6.84	6.94							
Critical Hdwy Stg 1	-	-	-	-	5.84	-							
Critical Hdwy Stg 2	-	-	-	-	5.84	-							
Follow-up Hdwy	-	-	2.22	-	3.52	3.32							
Pot Cap-1 Maneuver	-	-	1076	-	227	766							
Stage 1	-	-	-	-	594	-							
Stage 2	-	-	-	-	532	-							
Platoon blocked, %	-	-	-	-	-	-							
Mov Cap-1 Maneuver	-	-	1076	-	224	766							
Mov Cap-2 Maneuver	-	-	-	-	356	-							
Stage 1	-	-	-	-	594	-							
Stage 2	-	-	-	-	524	-							
Approach	EB	WB	NB										
HCM Control Delay, s	0	0.1	15.2										
HCM LOS				C									
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT									
Capacity (veh/h)	383	-	-	1076									
HCM Lane V/C Ratio	0.078	-	-	0.015									
HCM Control Delay (s)	15.2	-	-	8.4									
HCM Lane LOS	C	-	-	A									
HCM 95th %tile Q(veh)	0.3	-	-	0									

HCM 2010 TWSC
7: Bank & Steveright

Future Total 2025AM Peak Hour
2600 Bank Street

Intersection	1.6										
Int Delay, s/veh											
Movement	EBL	EBT	WBT	WBR	SBL	SBR					
Lane Configurations	↔	↔	↔	↔	↔	↔					
Traffic Vol, veh/h	40	453	1068	24	30	114					
Future Vol, veh/h	40	453	1068	24	30	114					
Conflicting Peds, #/hr	0	0	0	0	0	0					
Sign Control	Free	Free	Free	Free	Stop	Stop					
RT Channelized	-	None	-	None	-	None					
Storage Length	10	-	-	-	-	350					
Veh in Median Storage, #	0	0	0	0	0	0					
Grade, %	-	-	-	-	-	-					
Peak Hour Factor	100	100	100	100	100	100					
Heavy Vehicles, %	2	2	2	2	2	2					
Mvmt Flow	40	453	1068	24	30	114					
Major/Minor	Major1	Major2	Minor2								
Conflicting Flow All	1092	0	0	1387	546						
Stage 1	-	-	-	-	1080	-					
Stage 2	-	-	-	-	-	307					
Critical Hwy	4.14	-	-	-	6.84	6.94					
Critical Hwy Stg 1	-	-	-	-	5.84	-					
Critical Hwy Stg 2	-	-	-	-	3.52	3.32					
Follow-up Hwy	2.22	-	-	-	5.84	-					
Pot Cap-1 Maneuver	635	-	-	-	184	482					
Stage 1	-	-	-	-	287	-					
Stage 2	-	-	-	-	719	-					
Platoon blocked, %	-	-	-	-	-	-					
Mov Cap-1 Maneuver	635	-	-	-	126	482					
Mov Cap-2 Maneuver	-	-	-	-	220	-					
Stage 1	-	-	-	-	269	-					
Stage 2	-	-	-	-	719	-					
Approach	EB	WB	SB								
HCM Control Delay, s	0.9	0	16.7								
HCM LOS	C	C	C								
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2					
Capacity (veh/h)	635	-	-	-	220	482					
HCM Lane V/C Ratio	0.063	-	-	-	0.136	0.237					
HCM Control Delay (s)	11.1	-	-	-	23.9	14.8					
HCM Lane LOS	B	-	-	-	C	B					
HCM 95th %ile Q(veh)	0.2	-	-	-	0.5	0.9					

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Total 2025PM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1004
Future Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1004
Lane Group Flow (vph)	149	1016	358	49	800	177	328	546	60	252	1004
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2	2	1	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6
Switch Phase											
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5
Total Split (s)	17.0	41.0	41.0	14.0	38.0	38.0	19.0	46.0	46.0	19.0	46.0
Total Split (%)	14.2%	34.2%	34.2%	11.7%	31.7%	31.7%	15.8%	38.3%	38.3%	15.8%	38.3%
Maximum Green (s)	10.5	34.3	34.3	7.5	31.3	31.3	11.9	39.5	39.5	11.9	39.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21	21	4	4	4	22	22	22	22	22
Ad Effct Green (s)	9.8	37.1	37.1	7.1	32.0	32.0	11.9	39.7	39.7	11.7	39.5
Actuated g/C Ratio	0.08	0.31	0.31	0.06	0.27	0.27	0.10	0.33	0.33	0.10	0.33
v/c Ratio	0.59	1.01	0.57	0.51	0.91	0.91	0.34	1.03	0.50	0.81	0.92
Control Delay	63.2	73.2	12.7	75.8	62.9	19.7	114.7	36.0	3.9	72.7	52.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	0.0	0.0
Total Delay	63.2	73.2	12.7	75.8	62.9	19.7	114.7	37.3	3.9	72.7	52.9
LOS	E	E	B	E	E	B	F	D	A	E	D
Approach Delay	58.0	56.1	62.3								
Approach LOS	E	E	E								
Queue Length 50th (m)	17.6	~142.1	14.3	12.3	75.1	6.5	~41.7	43.4	0.3	30.3	119.2
Queue Length 95th (m)	28.4	#182.6	44.5	m#23.9	#127.0	31.1	m#69.8	m#9.5	m#5.7	#158.4	25.6
Internal Link Dist (m)	358.7	334.1	67.1								
Turn Bay Length (m)	150.0	60.0	60.0	90.0	90.0	40.0					
Base Capacity (vph)	268	1005	631	101	882	516	318	1096	575	318	1091
Starvation Cap Reductn	0	0	0	0	0	0	0	342	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	1.01	0.57	0.49	0.91	0.34	1.03	0.72	0.10	0.79	0.92
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 23 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle: 115											

Lanes, Volumes, Timings
 1: Bank & Hunt Club

Lanes, Volumes, Timings
 2: Albion & Hunt Club

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.03
 Intersection Signal Delay: 56.0
 Intersection LOS: E
 ICU Level of Service F
 Intersection Capacity Utilization 95.3%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 ~ Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	121	1201	23	321	950	64	7	117	48	133
Traffic Volume (vph)	121	1201	23	321	950	64	7	117	48	133
Future Volume (vph)	121	1201	23	321	950	64	7	117	48	133
Lane Group Flow (vph)	121	1201	23	321	950	64	7	379	48	185
Turn Type	pm-pt	NA	Perm	pm-pt	NA	Perm	NA	Perm	NA	Perm
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8

Splits and Phases: 1: Bank & Hunt Club



Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.4	58.6	58.6	25.4	58.6	58.6	36.0	36.0	36.0	36.0
Total Split (%)	21.2%	48.8%	48.8%	21.2%	48.8%	48.8%	30.0%	30.0%	30.0%	30.0%
Maximum Green (s)	20.0	53.1	53.1	20.0	53.1	53.1	29.8	29.8	29.8	29.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	4	4	4	5	5	5	6	6	6	24
Act Effort Green (s)	65.2	56.3	56.3	81.5	67.1	67.1	26.9	26.9	26.9	26.9
Actuated g/C Ratio	0.54	0.47	0.47	0.68	0.56	0.56	0.22	0.22	0.22	0.22
v/c Ratio	0.36	0.78	0.03	0.88	0.51	0.08	0.03	0.92	0.79	0.49
Control Delay	6.7	11.2	0.1	51.6	18.4	2.5	30.6	58.8	109.2	41.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.7	11.2	0.1	51.6	18.4	2.5	30.6	58.8	109.2	41.0
LOS	A	B	A	D	B	A	C	E	F	D
Approach Delay	10.6	10.6	25.6	25.6	25.6	25.6	58.3	58.3	55.0	55.0
Approach LOS	B	B	C	C	C	C	E	E	E	E
Queue Length 50th (m)	4.0	25.1	0.0	51.3	73.7	0.0	1.5	74.9	10.3	33.7
Queue Length 95th (m)	m4.8	m26.0	m0.0	#101.5	96.3	5.0	m3.9	#115.3	#31.7	55.8
Internal Link Dist (m)	334.1	334.1	564.6	564.6	564.6	188.3	188.3	429.6	429.6	429.6
Turn Bay Length (m)	65.0	40.0	100.0	40.0	100.0	40.0	35.0	30.0	30.0	30.0
Base Capacity (vph)	481	1539	715	373	1854	828	225	449	68	414
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.78	0.03	0.86	0.51	0.08	0.03	0.84	0.71	0.45

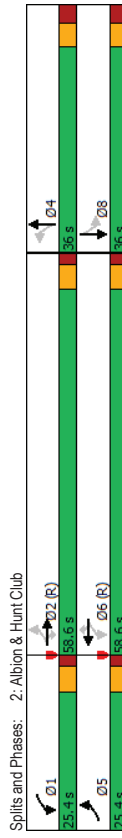
Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 96 (80%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
 Natural Cycle: 90

Lanes, Volumes, Timings
2: Albion & Hunt Club

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 25.4
 Intersection LOS: C
 ICU Level of Service G
 Intersection Capacity Utilization 105.5%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

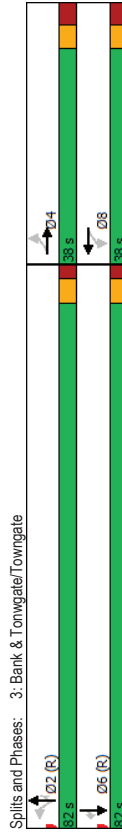


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	150	4	4	0	97	772	17	1279	148
Future Volume (vph)	150	4	4	0	97	772	17	1279	148
Lane Group Flow (vph)	0	313	0	16	0	869	17	1279	148
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	6	6
Switch Phase	4	4	8	8	2	2	2	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Total Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	20	20	17	17	17	17	17	17	17
Act Effr Green (s)	28.2	28.2	28.2	28.2	79.3	79.3	79.3	79.3	79.3
v/c Ratio	0.24	0.24	0.24	0.24	0.66	0.66	0.66	0.66	0.66
Control Delay	67.7	67.7	8.2	8.2	0.30	0.30	0.30	0.30	0.30
Queue Delay	1.9	1.9	0.0	0.0	0.0	0.0	0.0	0.5	0.4
Total Delay	69.7	69.7	8.2	8.2	6.6	6.6	6.6	5.1	0.7
LOS	E	E	A	A	A	A	A	A	A
Approach Delay	69.7	69.7	8.2	8.2	6.5	6.5	6.5	4.6	0.7
Approach LOS	E	E	A	A	A	A	A	A	A
Queue Length 50th (m)	61.2	61.2	0.0	0.0	17.9	0.0	22.3	0.0	0.0
Queue Length 95th (m)	#107.6	#107.6	3.9	3.9	17.4	m0.2	m31.5	m0.0	0.0
Internal Link Dist (m)	64.2	64.2	37.0	37.0	227.9	227.9	67.1	67.1	67.1
Turn Bay Length (m)					15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	380	380	389	389	2869	947	2192	994	994
Starvation Cap Reductn	0	0	0	0	0	0	454	529	529
Spillover Cap Reductn	16	16	16	16	124	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.86	0.86	0.04	0.04	0.32	0.32	0.02	0.74	0.32
Intersection Summary									
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 9 (8%), Referenced to phase 2:NBL and 6:SBT, Start of Green									
Natural Cycle: 85									

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 13.0
 Intersection LOS: B
 Intersection Capacity Utilization: 112.0%
 ICU Level of Service: H
 Analysis Period (min): 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings
4: Albion & Bank

2600 Bank Street

Future Total 2025PM Peak Hour

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
↔	↔	↔	↔	↔	↔	↔	↔	↔	↔

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	70	1061	279	61	665	163	158	169	183	255
Future Volume (vph)	70	1061	279	61	665	163	158	169	183	255
Lane Group Flow (vph)	70	1061	279	61	665	163	158	213	183	302
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	7	4	8	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	20.0	45.0	45.0	20.0	45.0	45.0	11.0	55.0	44.0	44.0
Total Split (%)	16.7%	37.5%	37.5%	16.7%	37.5%	37.5%	9.2%	45.8%	36.7%	36.7%
Maximum Green (s)	14.3	39.3	39.3	14.3	39.3	39.3	6.7	48.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	1	1	1	1	1	1	10	10	5	5
Act Effr Green (s)	10.4	57.0	57.0	9.8	56.5	56.5	39.8	37.7	26.7	26.7
Actuated g/C Ratio	0.09	0.48	0.48	0.08	0.47	0.47	0.33	0.31	0.22	0.22
v/c Ratio	0.49	0.67	0.33	0.46	0.43	0.22	0.72	0.40	0.76	0.79
Control Delay	74.9	14.4	1.4	62.4	24.9	4.8	48.3	31.1	49.1	44.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.9	14.4	1.4	62.4	24.9	4.8	48.3	31.1	49.1	44.8
LOS	E	B	A	E	C	A	D	C	D	D
Approach Delay	14.8			23.8			38.4			46.4
Approach LOS	B			C			D			D
Queue Length 50th (m)	17.6	42.7	0.0	14.0	55.0	0.0	27.5	36.9	44.0	71.2
Queue Length 95th (m)	m24.3	#164.8	m5.5	27.0	88.2	14.6	38.6	50.6	m55.1	m86.1
Internal Link Dist (m)	227.9			188.3			328.9			188.3
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	198	1576	834	197	1560	748	220	690	340	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.67	0.33	0.31	0.43	0.22	0.72	0.31	0.54	0.56

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 42 (35%), Referenced to phase 2,EBT and 6,WBT, Start of Green
 Natural Cycle: 105

Lanes, Volumes, Timings
4: Albion & Bank

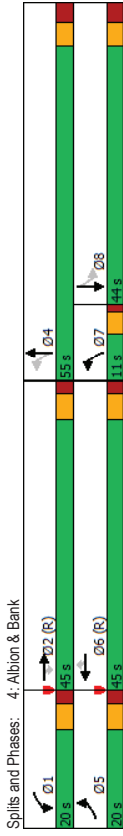
HCM 2010 TWSC
5: West Access & Bank

Future Total 2025PM Peak Hour
2600 Bank Street

Future Total 2025PM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.79
Intersection Signal Delay: 25.0
Intersection LOS: C
Intersection Capacity Utilization 82.1%
ICU Level of Service E
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

Intersection
In/Delay, s/veh 0.2
Movement
EBT EBR WBL WBT NBL NBR
Lane Configurations
Traffic Vol, veh/h 1194 66 0 865 0 26
Future Vol, veh/h 1194 66 0 865 0 26
Conflicting Peds, #/hr 0 0 0 0 0 0
Sign Control Free Free Free Free Stop Stop
RT Channelized - None - None - None
Storage Length - - - - -
Veh in Median Storage, # 0 0 0 0 0
Grade, % 0 0 0 0 0
Peak Hour Factor 100 100 100 100 100
Heavy Vehicles, % 2 2 2 2 2
Mvmt Flow 1194 66 0 865 0 26



Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	630
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	-	0	424
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	424
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-
Approach	EB	WB	NB
HCM Control Delay, s	0	0	14
HCM LOS			B
Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	424	-	-
HCM Lane V/C Ratio	0.061	-	-
HCM Control Delay (s)	14	-	-
HCM Lane LOS	B	-	-
HCM 95th %tile Q(veh)	0.2	-	-

HCM 2010 TWSC
6: East Access & Bank

HCM 2010 TWSC
7: Bank & Sieveright

Future Total 2025PM Peak Hour
2600 Bank Street

Future Total 2025PM Peak Hour
2600 Bank Street

Intersection												
Int Delay, s/veh												3
Movement	EBT	EBR	WBL	WBT	NBL	NBR						
Lane Configurations	↔	↔	↔	↔	↔	↔						
Traffic Vol, veh/h	1125	11	21	667	119	16						
Future Vol, veh/h	1125	11	21	667	119	16						
Conflicting Peds, #/hr	0	0	0	0	0	0						
Sign Control	Free	Free	Free	Free	Free	Stop						
RT Channelized	-	None	-	None	-	None						
Storage Length	-	250	500	-	-	0						
Veh in Median Storage, #	0	-	-	0	0	-						
Grade, %	0	-	-	0	0	-						
Peak Hour Factor	100	100	100	100	100	100						
Heavy Vehicles, %	2	2	2	2	2	2						
Mvmt Flow	1125	11	21	667	119	16						
Major/Minor	Major1	Major2	Minor1									
Conflicting Flow All	0	0	1136	0	1501	563						
Stage 1	-	-	-	-	-	-						
Stage 2	-	-	-	-	-	-						
Critical Hdwy	-	-	4.14	-	-	6.84						
Critical Hdwy Stg 1	-	-	-	-	-	5.84						
Critical Hdwy Stg 2	-	-	-	-	-	5.84						
Follow-up Hdwy	-	-	2.22	-	-	3.32						
Pot Cap-1 Maneuver	-	-	611	-	-	113						
Stage 1	-	-	-	-	-	272						
Stage 2	-	-	-	-	-	664						
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	611	-	-	109						
Mov Cap-2 Maneuver	-	-	-	-	-	213						
Stage 1	-	-	-	-	-	272						
Stage 2	-	-	-	-	-	641						
Approach	EB	WB	NB									
HCM Control Delay, s	0	0.3	41.4									
HCM LOS	E											
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT							
Capacity (veh/h)	228	-	-	611	-							
HCM Lane V/C Ratio	0.592	-	-	0.034	-							
HCM Control Delay (s)	41.4	-	-	11.1	-							
HCM Lane LOS	E	-	-	B	-							
HCM 95th %tile Q(veh)	3.4	-	-	0.1	-							
Notes	-											
- Volume exceeds capacity \$ Delay exceeds 300s +- Computation Not Defined *- All major volume in platoon												

Intersection													
Int Delay, s/veh												1.3	
Movement	EBU	EBL	EBT	WBT	WBR	SBL	SBR						
Lane Configurations	↔	↔	↔	↔	↔	↔	↔						
Traffic Vol, veh/h	7	100	1113	760	26	23	99						
Future Vol, veh/h	7	100	1113	760	26	23	99						
Conflicting Peds, #/hr	0	0	0	0	0	0	0						
Sign Control	Free	Free	Free	Free	Free	Stop	Stop						
RT Channelized	-	-	None	-	None	-	None						
Storage Length	-	10	-	-	-	-	350						
Veh in Median Storage, #	-	-	0	0	0	-	0						
Grade, %	-	-	0	0	0	-	0						
Peak Hour Factor	100	100	100	100	100	100	100						
Heavy Vehicles, %	2	2	2	2	2	2	2						
Mvmt Flow	7	100	1113	760	26	23	99						
Major/Minor	Major1	Major2	Minor2										
Conflicting Flow All	786	786	0	-	0	1544	393						
Stage 1	-	-	-	-	-	-	773						
Stage 2	-	-	-	-	-	-	771						
Critical Hdwy	6.44	4.14	-	-	-	-	6.84						
Critical Hdwy Stg 1	-	-	-	-	-	-	5.84						
Critical Hdwy Stg 2	-	-	-	-	-	-	5.84						
Follow-up Hdwy	2.52	2.22	-	-	-	-	3.52						
Pot Cap-1 Maneuver	455	829	-	-	-	-	105						
Stage 1	-	-	-	-	-	-	416						
Stage 2	-	-	-	-	-	-	417						
Platoon blocked, %	-	-	-	-	-	-	-						
Mov Cap-1 Maneuver	770	770	-	-	-	-	90						
Mov Cap-2 Maneuver	-	-	-	-	-	-	217						
Stage 1	-	-	-	-	-	-	358						
Stage 2	-	-	-	-	-	-	417						
Approach	EB	WB	SB										
HCM Control Delay, s	0.9	0	14.2										
HCM LOS	B												
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2							
Capacity (veh/h)	770	-	-	-	217	606							
HCM Lane V/C Ratio	0.139	-	-	-	0.106	0.163							
HCM Control Delay (s)	10.4	-	-	-	23.5	12.1							
HCM Lane LOS	B	-	-	-	C	B							
HCM 95th %tile Q(veh)	0.5	-	-	-	0.4	0.6							
Notes	-												

Appendix L

Synchro Intersection Worksheets – 2030 Future Total Conditions

DRAFT

Lanes, Volumes, Timings
1: Bank & Hunt Club

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Total 2030AM Peak Hour

Future Total 2030AM Peak Hour

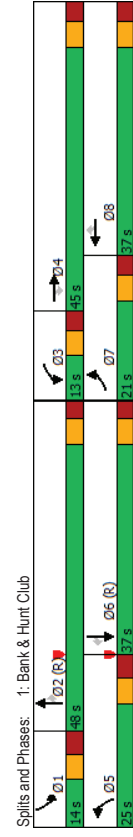
2600 Bank Street

2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Future Volume (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Lane Group Flow (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phase	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.7	44.3	44.3	6.7	32.7	32.7
Actuated G/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.37	0.36	0.97	0.29	0.71	0.90	0.03	0.46	0.36	0.26
Control Delay	58.3	35.6	5.3	55.6	84.9	14.2	61.6	41.0	0.1	63.4	37.2	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.4	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.3	55.6	84.9	14.2	61.6	46.5	0.1	63.4	37.2	1.1
LOS	E	D	A	E	F	B	E	D	A	E	D	A
Approach Delay	31.1			72.6			48.9			31.7		
Approach LOS	C			E			D			C		
Queue Length 50th (m)	14.2	76.3	0.0	7.0	117.6	9.7	28.1	136.0	0.0	9.8	31.5	0.0
Queue Length 95th (m)	23.2	98.0	17.8	m14.1	#166.5	24.7	51.6	#170.2	0.0	18.1	45.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	674	86	910	613	465	1212	662	184	870	556
Starvation Cap Reductn	0	0	0	0	0	0	0	86	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.37	0.35	0.97	0.29	0.62	0.97	0.03	0.45	0.36	0.26

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 65 (54%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 95

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT	TT
Traffic Volume (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Future Volume (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Lane Group Flow (vph)	121	704	250	30	885	177	290	1092	22	83	313	142
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phase	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	21.0	45.0	45.0	13.0	37.0	37.0	25.0	48.0	48.0	14.0	37.0	37.0
Total Split (%)	17.5%	37.5%	37.5%	10.8%	30.8%	30.8%	20.8%	40.0%	40.0%	11.7%	30.8%	30.8%
Maximum Green (s)	14.5	38.3	38.3	6.5	30.3	30.3	17.9	41.5	41.5	6.9	30.5	30.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	9	9	9	10	10	10	20	20	20	25	25	25
Act Effr Green (s)	10.2	43.5	43.5	6.3	34.6	34.6	15.7	44.3	44.3	6.7	32.7	32.7
Actuated G/C Ratio	0.08	0.36	0.36	0.05	0.29	0.29	0.13	0.37	0.37	0.06	0.27	0.27
v/c Ratio	0.48	0.62	0.37	0.36	0.97	0.29	0.71	0.90	0.03	0.46	0.36	0.26
Control Delay	58.3	35.6	5.3	55.6	84.9	14.2	61.6	41.0	0.1	63.4	37.2	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.4	0.0	0.0	0.0	0.0
Total Delay	58.3	35.6	5.3	55.6	84.9	14.2	61.6	46.5	0.1	63.4	37.2	1.1
LOS	E	D	A	E	F	B	E	D	A	E	D	A
Approach Delay	31.1			72.6			48.9			31.7		
Approach LOS	C			E			D			C		
Queue Length 50th (m)	14.2	76.3	0.0	7.0	117.6	9.7	28.1	136.0	0.0	9.8	31.5	0.0
Queue Length 95th (m)	23.2	98.0	17.8	m14.1	#166.5	24.7	51.6	#170.2	0.0	18.1	45.4	0.0
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	357	1135	674	86	910	613	465	1212	662	184	870	556
Starvation Cap Reductn	0	0	0	0	0	0	0	86	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.62	0.37	0.35	0.97	0.29	0.62	0.97	0.03	0.45	0.36	0.26



Splits and Phases: 1: Bank & Hunt Club

Control Type	Actuated-Coordinated
Maximum v/c Ratio	0.97
Intersection Signal Delay	48.3
Intersection LOS	D
IOU Level of Service E	
Intersection Capacity Utilization	88.4%
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	

Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Total 2030AM Peak Hour
2600 Bank Street

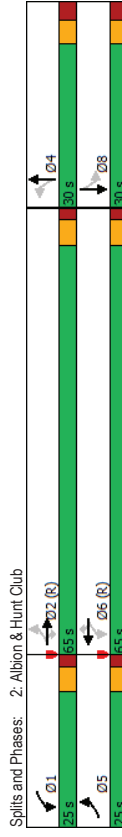
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	58	719	8	191	999	79	6	99	41	84
Future Volume (vph)	58	719	8	191	999	79	6	99	41	84
Lane Group Flow (vph)	58	719	8	191	999	79	6	366	41	153
Turn Type	pm-pt	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	2	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	25.0	65.0	65.0	25.0	65.0	65.0	30.0	30.0	30.0	30.0
Total Split (%)	20.8%	54.2%	54.2%	20.8%	54.2%	54.2%	25.0%	25.0%	25.0%	25.0%
Maximum Green (s)	19.6	59.5	59.5	19.6	59.5	59.5	23.8	23.8	23.8	23.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	5.5	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	2	2	2	4	4	4	2	2	2	7
Act Effr Green (s)	76.9	70.0	70.0	84.4	75.5	75.5	22.7	22.7	22.7	22.7
Actuated G/C Ratio	0.64	0.58	0.58	0.70	0.63	0.63	0.19	0.19	0.19	0.19
v/c Ratio	0.18	0.39	0.01	0.40	0.51	0.08	0.03	0.94	0.72	0.50
Control Delay	2.0	3.7	0.0	8.5	14.1	2.6	61.3	89.5	102.9	40.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	2.0	3.7	0.0	8.5	14.1	2.6	61.3	89.5	102.9	40.4
LOS	A	A	A	A	B	A	E	F	F	D
Approach Delay	3.5			12.6			89.0			53.6
Approach LOS	A			B			F			D
Queue Length 50th (m)	0.1	0.7	0.0	13.5	68.0	0.1	1.3	57.0	9.0	25.5
Queue Length 95th (m)	m0.3	2.3	m0.0	21.4	87.7	6.2	m4.4	#108.6	#28.7	46.6
Internal Link Dist (m)	334.1			554.6			188.3			429.6
Turn Bay Length (m)	65.0	40.0	40.0	100.0	40.0	35.0	30.0			30.0
Base Capacity (vph)	474	1843	720	563	1970	932	198	371	60	318
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.39	0.01	0.34	0.51	0.08	0.03	0.91	0.68	0.48

Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 120
Offset: 27 (23%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
Natural Cycle: 70

Lanes, Volumes, Timings
2: Albion & Hunt Club

Future Total 2030AM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.94	
Intersection Signal Delay: 23.0	Intersection LOS: C
Intersection Capacity Utilization 82.1%	IOU Level of Service E
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

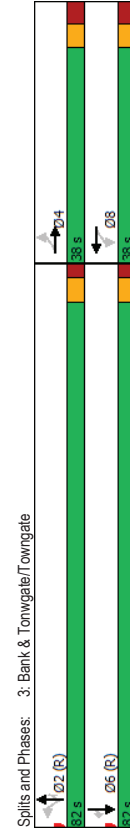
Future Total 2030AM Peak Hour
2600 Bank Street

Future Total 2030AM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	8	8	2	2	2	2	6
Traffic Volume (vph)	31	1	2	1	51	1405	13	563	40
Future Volume (vph)	31	1	2	1	51	1405	13	563	40
Lane Group Flow (vph)	0	65	0	11	0	1456	13	563	40
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	2	6
Permitted Phases	4	4	8	8	2	2	2	2	6
Detector Phase	4	4	8	8	2	2	2	2	6
Switch Phase	4	4	8	8	2	2	2	2	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Total Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	1	1	9	9	9	9	9	9	9
Act Effr Green (s)	14.3	14.3	14.3	14.3	97.7	97.7	97.7	97.7	97.7
Actuated g/C Ratio	0.12	0.12	0.07	0.07	0.81	0.81	0.81	0.81	0.81
v/c Ratio	0.36	0.36	0.07	0.07	0.34	0.01	0.21	0.04	0.04
Control Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.7	1.7	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0
Total Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.8	1.7	1.7
LOS	C	C	C	C	A	A	A	A	A
Approach Delay	30.5	25.9	25.9	25.9	2.5	0.0	3.7	1.7	1.7
Approach LOS	C	C	C	C	A	A	A	A	A
Queue Length 50th (m)	7.2	0.7	0.7	0.7	13.4	0.0	8.7	0.0	0.0
Queue Length 95th (m)	17.2	5.2	5.2	5.2	m23.8	m0.0	25.7	0.9	0.9
Internal Link Dist (m)	64.2	37.0	37.0	37.0	227.9	67.1			
Turn Bay Length (m)					15.0				
Base Capacity (vph)	359	359	359	359	4321	1173	2623	1062	1062
Starvation Cap Reductn	0	0	0	0	0	0	1208	0	0
Spillback Cap Reductn	3	0	0	0	540	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.03	0.03	0.03	0.39	0.01	0.39	0.04	0.04

Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	30 (25%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	85

Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.36
Intersection Signal Delay:	3.8
Intersection LOS:	A
IOU Level of Service E	
Intersection Capacity Utilization:	69.2%
Analysis Period (min):	15
m. Volume for 95th percentile queue:	is metered by upstream signal.



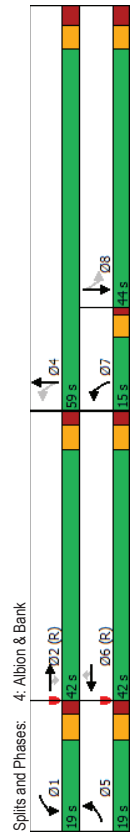
Lanes, Volumes, Timings
4: Albion & Bank

Lanes, Volumes, Timings
4: Albion & Bank

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Future Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Lane Group Flow (vph)	21	460	95	32	1119	120	298	243	110	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prn+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phases	5	2	2	1	6	6	7	4	4	8
Detector Phase	5	2	2	1	6	6	7	4	4	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	63.7	63.7	8.0	67.0	67.0	37.3	35.2	20.2	20.2
Actuated g/C Ratio	0.06	0.53	0.53	0.07	0.56	0.56	0.31	0.29	0.17	0.17
v/c Ratio	0.21	0.28	0.12	0.31	0.61	0.14	0.92	0.48	0.62	0.54
Control Delay	66.0	20.5	5.4	60.5	23.2	4.6	69.3	35.5	60.5	49.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.0	20.5	5.4	60.5	23.2	4.6	69.3	35.5	60.5	49.4
LOS	E	C	A	E	C	A	E	D	E	D
Approach Delay	19.7			22.4			54.1			
Approach LOS	B			C			D			
Queue Length 50th (m)	5.2	26.7	0.0	7.3	73.7	0.0	60.6	46.9	27.4	36.9
Queue Length 95th (m)	13.7	57.0	8.6	17.2	#184.9	12.2	68.7	55.3	31.3	36.0
Internal Link Dist (m)	227.9			198.3			328.9			188.3
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0		
Base Capacity (vph)	183	1661	817	170	1833	828	324	753	328	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.28	0.12	0.19	0.61	0.14	0.92	0.32	0.34	0.29

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 56 (47%), Referenced to phase 2,EBT and 6:WBT, Start of Green
 Natural Cycle: 105

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Future Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Lane Group Flow (vph)	21	460	95	32	1119	120	298	243	110	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prn+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phases	5	2	2	1	6	6	7	4	4	8
Detector Phase	5	2	2	1	6	6	7	4	4	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	63.7	63.7	8.0	67.0	67.0	37.3	35.2	20.2	20.2
Actuated g/C Ratio	0.06	0.53	0.53	0.07	0.56	0.56	0.31	0.29	0.17	0.17
v/c Ratio	0.21	0.28	0.12	0.31	0.61	0.14	0.92	0.48	0.62	0.54
Control Delay	66.0	20.5	5.4	60.5	23.2	4.6	69.3	35.5	60.5	49.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.0	20.5	5.4	60.5	23.2	4.6	69.3	35.5	60.5	49.4
LOS	E	C	A	E	C	A	E	D	E	D
Approach Delay	19.7			22.4			54.1			
Approach LOS	B			C			D			
Queue Length 50th (m)	5.2	26.7	0.0	7.3	73.7	0.0	60.6	46.9	27.4	36.9
Queue Length 95th (m)	13.7	57.0	8.6	17.2	#184.9	12.2	68.7	55.3	31.3	36.0
Internal Link Dist (m)	227.9			198.3			328.9			188.3
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0		
Base Capacity (vph)	183	1661	817	170	1833	828	324	753	328	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.28	0.12	0.19	0.61	0.14	0.92	0.32	0.34	0.29



	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Future Volume (vph)	21	460	95	32	1119	120	298	214	110	130
Lane Group Flow (vph)	21	460	95	32	1119	120	298	243	110	157
Turn Type	Prot	NA	Perm	Prot	NA	Perm	prn+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	7	4	4	8
Permitted Phases	5	2	2	1	6	6	7	4	4	8
Detector Phase	5	2	2	1	6	6	7	4	4	8
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	19.0	42.0	42.0	19.0	42.0	42.0	15.0	59.0	44.0	44.0
Total Split (%)	15.8%	35.0%	35.0%	15.8%	35.0%	35.0%	12.5%	49.2%	36.7%	36.7%
Maximum Green (s)	13.3	36.3	36.3	13.3	36.3	36.3	10.7	52.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	3	3	3	14	14	14	2	2	2	2
Act Effr Green (s)	7.1	63.								

HCM 2010 TWSC
5: West Access & Bank

HCM 2010 TWSC
6: East Access & Bank

Future Total 2030AM Peak Hour
2600 Bank Street

Future Total 2030AM Peak Hour
2600 Bank Street

Intersection													
Int Delay, s/veh												0	
Movement													
EBT	EBR	WBL	WBT	NBL	NBR								
492	81	0	1249	0	1	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	
Lane Configurations													
Traffic Vol, veh/h												492	
Future Vol, veh/h												492	
Conflicting Peds, #/hr												0	
Sign Control													
RT Channelized												- None -	
Storage Length												- None -	
Veh in Median Storage, #												0	
Grade, %												0	
Peak Hour Factor												100	
Heavy Vehicles, %												7	
Mvmt Flow												492	
Major/Minor													
Major1												Major2	
Minor1												Minor2	
Conflicting Flow All													0
Stage 1													0
Stage 2													287
Critical Hdwy													-
Critical Hdwy Stg 1													-
Critical Hdwy Stg 2													6.94
Follow-up Hdwy													-
Pot Cap-1 Maneuver													-
Stage 1													0
Stage 2													710
Platoon blocked, %													-
Mov Cap-1 Maneuver													-
Mov Cap-2 Maneuver													710
Stage 1													-
Stage 2													-
Approach													EB WB NB
HCM Control Delay, s												0	
HCM LOS												B	
Minor Lane/Major Mvmt													NBLn1
Capacity (veh/h)												710	
HCM Lane V/C Ratio												0.001	
HCM Control Delay (s)												10.1	
HCM Lane LOS												B	
HCM 95th %tile Q(veh)												0	

Intersection													
Int Delay, s/veh												0.3	
Movement													
EBT	EBR	WBL	WBT	NBL	NBR								
472	11	16	1133	26	4	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	
Lane Configurations													
Traffic Vol, veh/h												472	
Future Vol, veh/h												472	
Conflicting Peds, #/hr												0	
Sign Control													
RT Channelized												- None -	
Storage Length												250	
Veh in Median Storage, #												0	
Grade, %												0	
Peak Hour Factor												100	
Heavy Vehicles, %												7	
Mvmt Flow												472	
Major/Minor													
Major1												Major2	
Minor1												Minor2	
Conflicting Flow All													0
Stage 1													472
Stage 2													599
Critical Hdwy													-
Critical Hdwy Stg 1													4.14
Critical Hdwy Stg 2													6.94
Follow-up Hdwy													-
Pot Cap-1 Maneuver													-
Stage 1													2.22
Stage 2													3.32
Platoon blocked, %													-
Mov Cap-1 Maneuver													1076
Mov Cap-2 Maneuver													594
Stage 1													-
Stage 2													503
Approach													EB WB NB
HCM Control Delay, s												0	
HCM LOS												C	
Minor Lane/Major Mvmt													NBLn1
Capacity (veh/h)												372	
HCM Lane V/C Ratio												0.081	
HCM Control Delay (s)												15.5	
HCM Lane LOS												C	
HCM 95th %tile Q(veh)												0.3	

HCM 2010 TWSC
7: Bank & Steveright

Future Total 2030AM Peak Hour
2600 Bank Street

Intersection	EBL	EBT	WBT	WBR	SBL	SBR
Int Delay, s/veh	1.7					
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Vol, veh/h	40	453	1135	24	30	114
Future Vol, veh/h	40	453	1135	24	30	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	10	-	-	-	350	0
Veh in Median Storage, #	0	0	0	0	0	0
Grade, %	-	0	0	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	453	1135	24	30	114
Major/Minor	Major1	Major2	Minor2	Minor2	Minor2	Minor2
Conflicting Flow All	1159	0	0	1454	580	580
Stage 1	-	-	-	1147	-	-
Stage 2	-	-	-	307	-	-
Critical Hwy	4.14	-	-	6.84	6.94	-
Critical Hwy Stg 1	-	-	-	5.84	-	-
Critical Hwy Stg 2	-	-	-	3.52	3.32	-
Follow-up Hwy	2.22	-	-	3.52	3.32	-
Pot Cap-1 Maneuver	599	-	-	121	458	-
Stage 1	-	-	-	265	-	-
Stage 2	-	-	-	719	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	599	-	-	113	458	-
Mov Cap-2 Maneuver	-	-	-	203	-	-
Stage 1	-	-	-	247	-	-
Stage 2	-	-	-	719	-	-
Approach	EB	WB	SB	SB	SB	SB
HCM Control Delay, s	0.9	0	17.6	17.6	17.6	17.6
HCM LOS	C	C	C	C	C	C
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	599	-	-	203	458	458
HCM Lane V/C Ratio	0.067	-	-	0.148	0.249	0.249
HCM Control Delay (s)	11.4	-	-	25.8	15.4	15.4
HCM Lane LOS	B	-	-	D	C	C
HCM 95th %ile Q(veh)	0.2	-	-	0.5	1	1

Lanes, Volumes, Timings
1: Bank & Hunt Club

Future Total 2030PM Peak Hour
2600 Bank Street

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1067
Future Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1067
Lane Group Flow (vph)	149	1016	358	49	800	177	328	546	60	252	1067
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2	2	1	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6
Switch Phase	7	4	4	3	8	8	5	2	2	1	6
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5
Total Split (s)	17.0	41.0	41.0	14.0	38.0	38.0	19.0	46.0	46.0	19.0	46.0
Total Split (%)	14.2%	34.2%	34.2%	11.7%	31.7%	31.7%	15.8%	38.3%	38.3%	15.8%	38.3%
Maximum Green (s)	10.5	34.3	34.3	7.5	31.3	31.3	11.9	39.5	39.5	11.9	39.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21	21	4	4	4	22	22	22	22	22
Ad Effct Green (s)	9.8	37.1	37.1	7.1	32.0	32.0	11.9	39.7	39.7	11.7	39.5
Actuated g/C Ratio	0.08	0.31	0.31	0.06	0.27	0.27	0.10	0.33	0.33	0.10	0.33
v/c Ratio	0.59	1.01	0.57	0.51	0.91	0.91	0.34	1.03	0.50	0.81	0.98
Control Delay	63.2	73.2	73.2	12.9	75.8	75.8	19.7	114.7	114.7	36.0	93
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	0.0
Total Delay	63.2	73.2	73.2	12.9	75.8	75.8	19.7	114.7	114.7	37.3	93
LOS	E	E	B	E	E	B	F	D	A	E	A
Approach Delay	58.0	58.0	58.0	56.1	56.1	56.1	62.3	62.3	62.3	56.8	56.8
Approach LOS	E	E	E	E	E	E	E	E	E	E	E
Queue Length 50th (m)	17.6	~142.1	14.7	12.3	75.1	6.5	~41.8	43.4	0.3	30.3	130.2
Queue Length 95th (m)	28.4	#182.6	45.1	m#23.9	#127.0	31.1	m#69.8	m89.5	m5.7	#49.6	#175.1
Internal Link Dist (m)	358.7	358.7	358.7	334.1	334.1	334.1	67.1	67.1	67.1	340.8	340.8
Turn Bay Length (m)	150.0	150.0	150.0	60.0	60.0	60.0	90.0	90.0	90.0	40.0	115.0
Base Capacity (vph)	268	1005	629	101	882	516	318	1096	575	318	1091
Starvation Cap Reductn	0	0	0	0	0	0	0	342	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	1.01	0.57	0.49	0.91	0.34	1.03	0.72	0.10	0.79	0.98
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 23 (19%), Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle: 125											

Lanes, Volumes, Timings
1: Bank & Hunt Club

Lanes, Volumes, Timings
2: Albion & Hunt Club

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.03
 Intersection Signal Delay: 58.1
 Intersection LOS: E
 ICU Level of Service F
 Intersection Capacity Utilization 97.2%
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	121	1201	23	321	950	64	7	117	48	133
Traffic Volume (vph)	121	1201	23	321	950	64	7	117	48	133
Future Volume (vph)	121	1201	23	321	950	64	7	117	48	133
Lane Group Flow (vph)	121	1201	23	321	950	64	7	379	48	185
Turn Type	pm-pt	NA	Perm	pm-pt	NA	Perm	NA	Perm	NA	Perm
Protected Phases	5	2	1	6	6	4	4	8	8	8
Permitted Phases	5	2	2	1	6	6	4	4	8	8
Detector Phase	5	2	2	1	6	6	4	4	8	8

Splits and Phases: 1: Bank & Hunt Club



Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	10.4	26.5	26.5	10.4	26.5	26.5	29.2	29.2	29.2	29.2
Total Split (s)	25.4	58.6	58.6	25.4	58.6	58.6	36.0	36.0	36.0	36.0
Total Split (%)	21.2%	48.8%	48.8%	21.2%	48.8%	48.8%	30.0%	30.0%	30.0%	30.0%
Maximum Green (s)	20.0	53.1	53.1	20.0	53.1	29.8	29.8	29.8	29.8	29.8
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	1.7	1.8	1.8	1.7	1.8	1.8	2.9	2.9	2.9	2.9
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.5	5.5	5.4	5.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	14.0	14.0	14.0	14.0	14.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	7.0	7.0	7.0	7.0	7.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	4	4	4	5	5	6	6	6	6	24
Act Effort Green (s)	65.2	56.3	56.3	81.5	67.1	67.1	26.9	26.9	26.9	26.9
Actuated g/C Ratio	0.54	0.47	0.47	0.68	0.56	0.56	0.22	0.22	0.22	0.22
v/c Ratio	0.36	0.78	0.03	0.88	0.51	0.08	0.03	0.92	0.79	0.49
Control Delay	6.7	11.2	0.1	51.6	18.4	2.5	30.6	58.8	109.2	41.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.7	11.2	0.1	51.6	18.4	2.5	30.6	58.8	109.2	41.0
LOS	A	B	A	D	B	A	C	E	F	D
Approach Delay	10.6	10.6	25.6	25.6	25.6	58.3	58.3	55.0	55.0	55.0
Approach LOS	B	B	C	C	C	E	E	E	E	E
Queue Length 50th (m)	4.0	25.1	0.0	51.3	73.7	0.0	1.5	74.9	10.3	33.7
Queue Length 95th (m)	m4.8	m26.0	m0.0	#101.5	96.3	5.0	m3.9	#115.3	#31.7	55.8
Internal Link Dist (m)	334.1	334.1	564.6	564.6	564.6	188.3	188.3	429.6	429.6	429.6
Turn Bay Length (m)	65.0	40.0	100.0	40.0	100.0	40.0	35.0	30.0	30.0	30.0
Base Capacity (vph)	481	1539	715	373	1854	828	225	449	68	414
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.78	0.03	0.86	0.51	0.08	0.03	0.84	0.71	0.45

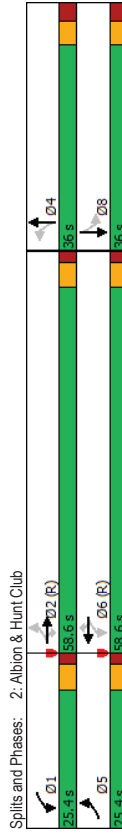
Intersection Summary	
Cycle Length:	120
Actuated Cycle Length:	120
Offset:	96 (80%), Referenced to phase 2,EBTL and 6,WBTL, Start of Green
Natural Cycle:	90

Lanes, Volumes, Timings
2: Albion & Hunt Club

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 25.4
 Intersection LOS: C
 ICU Level of Service G
 Intersection Capacity Utilization 105.5%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Future Total 2030PM Peak Hour
 2600 Bank Street



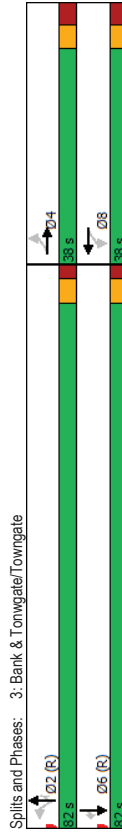
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	150	4	4	0	97	772	17	1368	148
Future Volume (vph)	150	4	4	0	97	772	17	1368	148
Lane Group Flow (vph)	0	313	0	16	0	869	17	1368	148
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	6	6
Switch Phase	4	4	8	8	2	2	2	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	16.7	16.7	37.7	37.7	44.8	44.8	44.8	44.8	44.8
Total Split (s)	38.0	38.0	38.0	38.0	82.0	82.0	82.0	82.0	82.0
Total Split (%)	31.7%	31.7%	31.7%	31.7%	68.3%	68.3%	68.3%	68.3%	68.3%
Maximum Green (s)	31.3	31.3	31.3	31.3	76.2	76.2	76.2	76.2	76.2
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.4	3.4	3.4	3.4	2.1	2.1	2.1	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.7	6.7	6.7	6.7	5.8	5.8	5.8	5.8	5.8
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Flash Dont Walk (s)	24.0	24.0	14.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	20	20	17	17	17	17	17	17	17
Act Effr Green (s)	28.2	28.2	28.2	28.2	79.3	79.3	79.3	79.3	79.3
Actuated g/C Ratio	0.24	0.24	0.24	0.24	0.66	0.66	0.66	0.66	0.66
v/c Ratio	0.90	0.90	0.05	0.30	0.02	0.62	0.15	0.02	0.15
Control Delay	67.7	8.2	8.2	6.6	0.2	4.9	0.2	0.2	4.9
Queue Delay	2.1	0.0	0.0	0.0	0.0	0.0	0.7	0.5	0.7
Total Delay	69.8	8.2	8.2	6.6	0.2	5.5	0.7	0.7	5.5
LOS	E	A	A	A	A	A	A	A	A
Approach Delay	69.8	8.2	8.2	6.5	5.1	5.1	5.1	5.1	5.1
Approach LOS	E	A	A	A	A	A	A	A	A
Queue Length 50th (m)	61.2	0.0	0.0	17.9	0.0	23.5	0.0	0.0	23.5
Queue Length 95th (m)	#107.6	3.9	3.9	17.4	m0.2	m31.1	m0.0	m0.0	m31.1
Internal Link Dist (m)	64.2	37.0	37.0	227.9	67.1	67.1	67.1	67.1	67.1
Turn Bay Length (m)					15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	380	389	389	2854	947	2192	994	994	994
Starvation Cap Reductn	0	0	0	0	0	440	537	537	537
Spillback Cap Reductn	17	17	17	123	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.86	0.04	0.04	0.32	0.02	0.78	0.32	0.32	0.78

Intersection Summary	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBT	SBR
Cycle Length: 120									
Actuated Cycle Length: 120									
Offset: 9 (8%), Referenced to phase 2:NBT and 6:SBT, Start of Green									
Natural Cycle: 85									

Lanes, Volumes, Timings
3: Bank & Tonwgate/Towngate

Lanes, Volumes, Timings
4: Albion & Bank

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.90
 Intersection Signal Delay: 13.0
 Intersection LOS: B
 Intersection Capacity Utilization 114.3%
 ICU Level of Service H
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	70	1125	279	61	665	163	158	169	183	255
Future Volume (vph)	70	1125	279	61	665	163	158	169	183	255
Lane Group Flow (vph)	70	1125	279	61	665	163	158	213	183	302
Turn Type	Prot	NA	Perm	Prot	NA	Perm	pm+pt	NA	Perm	NA
Protected Phases	5	2	2	1	6	6	4	4	8	8
Permitted Phases	5	2	2	1	6	6	7	4	8	8
Detector Phase										
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	10.7	38.7	38.7	10.7	38.7	38.7	9.3	43.4	43.4	43.4
Total Split (s)	20.0	45.0	45.0	20.0	45.0	45.0	11.0	55.0	44.0	44.0
Total Split (%)	16.7%	37.5%	37.5%	16.7%	37.5%	37.5%	9.2%	45.8%	36.7%	36.7%
Maximum Green (s)	14.3	39.3	39.3	14.3	39.3	39.3	6.7	48.6	37.6	37.6
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	3.1	3.1	3.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.7	5.7	5.7	5.7	5.7	5.7	4.3	6.4	6.4	6.4
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None
Walk Time (s)	20.0	20.0	20.0	20.0	20.0	20.0	10.0	10.0	10.0	10.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	27.0	27.0	27.0	27.0
Pedestrian Calls (#/hr)	1	1	1	1	1	1	10	10	5	5
Act Effct Green (s)	10.4	57.0	57.0	9.8	56.5	56.5	39.8	37.7	26.7	26.7
Actuated g/C Ratio	0.09	0.48	0.48	0.08	0.47	0.47	0.33	0.31	0.22	0.22
v/c Ratio	0.49	0.71	0.33	0.46	0.43	0.22	0.72	0.40	0.76	0.79
Control Delay	74.1	16.0	1.6	62.4	24.9	4.8	48.3	31.1	49.1	44.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	74.1	16.0	1.6	62.4	24.9	4.8	48.3	31.1	49.1	44.8
LOS	E	B	A	E	C	A	D	C	D	D
Approach Delay	16.1			23.8			38.4		46.4	
Approach LOS	B			C			D		D	
Queue Length 50th (m)	17.6	45.1	0.0	14.0	55.0	0.0	27.5	36.9	44.0	71.2
Queue Length 95th (m)	m23.4	#182.0	m5.2	27.0	88.2	14.6	38.6	50.6	m55.1	m86.1
Internal Link Dist (m)	227.9			188.3			328.9		188.3	
Turn Bay Length (m)	30.0	100.0	100.0	100.0	100.0	65.0	30.0	45.0	45.0	45.0
Base Capacity (vph)	198	1576	834	197	1560	748	220	690	340	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.71	0.33	0.31	0.43	0.22	0.72	0.31	0.54	0.56

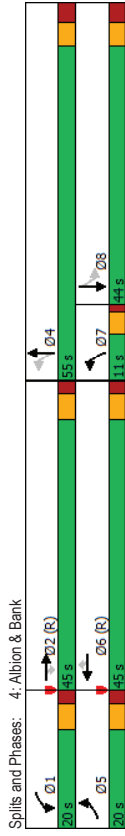
Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 42 (35%), Referenced to phase 2,EBT and 6,WBT, Start of Green
 Natural Cycle: 105

Lanes, Volumes, Timings
4: Albion & Bank

HCM 2010 TWSC
5: West Access & Bank

Future Total 2030PM Peak Hour
2600 Bank Street

Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 25.4
 Intersection LOS: C
 ICU Level of Service E
 Intersection Capacity Utilization 84.0%
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.



Intersection
 Int Delay, s/veh 0.2

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑↑		↑↑	↑↑	↑↑
Traffic Vol, veh/h	1265	66	0	865	0	26
Future Vol, veh/h	1265	66	0	865	0	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	- None	- None	- None	- None	- None	- None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	0
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1265	66	0	865	0	26

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	666
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	6.94
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	3.32
Pot Cap-1 Maneuver	-	0	402
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	402
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	14.6
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	402	-	-	-
HCM Lane V/C Ratio	0.065	-	-	-
HCM Control Delay (s)	14.6	-	-	-
HCM Lane LOS	B	-	-	-
HCM 95th %tile Q(veh)	0.2	-	-	-

HCM 2010 TWSC
6: East Access & Bank
Future Total 2030PM Peak Hour
2600 Bank Street

Intersection												
Int Delay, s/veh											3.3	
Movement	EBT	EBR	WBL	WBT	NBL	NBR						
Lane Configurations	↔	↔	↔	↔	↔	↔						
Traffic Vol, veh/h	1196	11	21	667	119	16						
Future Vol, veh/h	1196	11	21	667	119	16						
Conflicting Peds, #/hr	0	0	0	0	0	0						
Sign Control	Free	Free	Free	Free	Free	Stop						
RT Channelized	-	None	-	None	-	None						
Storage Length	-	250	500	-	0	-						
Veh in Median Storage, #	0	-	-	0	0	-						
Grade, %	0	-	-	0	0	-						
Peak Hour Factor	100	100	100	100	100	100						
Heavy Vehicles, %	2	2	2	2	2	2						
Mvmt Flow	1196	11	21	667	119	16						
Major/Minor	Major1	Major2	Minor1									
Conflicting Flow All	0	0	1207	0	1572	598						
Stage 1	-	-	-	-	1196	-						
Stage 2	-	-	-	-	376	-						
Critical Hdwy	-	-	4.14	-	6.84	6.94						
Critical Hdwy Stg 1	-	-	-	-	5.84	-						
Critical Hdwy Stg 2	-	-	-	-	5.84	-						
Follow-up Hdwy	-	-	2.22	-	3.52	3.32						
Pot Cap-1 Maneuver	-	-	574	-	~101	445						
Stage 1	-	-	-	-	249	-						
Stage 2	-	-	-	-	664	-						
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	-	-	574	-	~97	445						
Mov Cap-2 Maneuver	-	-	-	-	197	-						
Stage 1	-	-	-	-	249	-						
Stage 2	-	-	-	-	639	-						
Approach	EB	WB	NB									
HCM Control Delay, s	0	0.4	48.2									
HCM LOS	E											
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT							
Capacity (veh/h)	211	-	-	574	-							
HCM Lane V/C Ratio	0.64	-	-	0.037	-							
HCM Control Delay (s)	48.2	-	-	11.5	-							
HCM Lane LOS	E	-	-	B	-							
HCM 95th %tile Q(veh)	3.8	-	-	0.1	-							
Notes	-											
- Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon												

HCM 2010 TWSC
7: Bank & Sieveright
Future Total 2030PM Peak Hour
2600 Bank Street

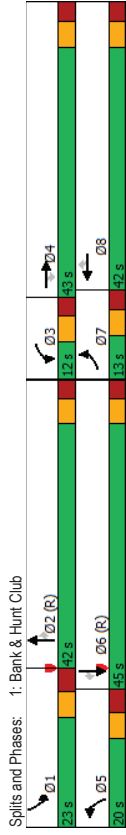
Intersection													
Int Delay, s/veh											1.3		
Movement	EBU	EBL	EBT	WBT	WBR	SBL	SBR						
Lane Configurations	↔	↔	↔	↔	↔	↔	↔						
Traffic Vol, veh/h	7	100	1184	760	26	23	99						
Future Vol, veh/h	7	100	1184	760	26	23	99						
Conflicting Peds, #/hr	0	0	0	0	0	0	0						
Sign Control	Free	Free	Free	Free	Free	Stop	Stop						
RT Channelized	-	-	None	-	None	-	None						
Storage Length	-	10	-	-	-	350	0						
Veh in Median Storage, #	-	-	0	0	-	0	-						
Grade, %	-	-	0	0	-	0	-						
Peak Hour Factor	100	100	100	100	100	100	100						
Heavy Vehicles, %	2	2	2	2	2	2	2						
Mvmt Flow	7	100	1184	760	26	23	99						
Major/Minor	Major1	Major2	Minor2										
Conflicting Flow All	786	786	0	-	0	1579	393						
Stage 1	-	-	-	-	-	773	-						
Stage 2	-	-	-	-	-	806	-						
Critical Hdwy	6.44	4.14	-	-	-	6.84	6.94						
Critical Hdwy Stg 1	-	-	-	-	-	5.84	-						
Critical Hdwy Stg 2	-	-	-	-	-	5.84	-						
Follow-up Hdwy	2.52	2.22	-	-	-	3.52	3.32						
Pot Cap-1 Maneuver	455	829	-	-	-	100	606						
Stage 1	-	-	-	-	-	416	-						
Stage 2	-	-	-	-	-	400	-						
Platoon blocked, %	-	-	-	-	-	-	-						
Mov Cap-1 Maneuver	770	770	-	-	-	86	606						
Mov Cap-2 Maneuver	-	-	-	-	-	212	-						
Stage 1	-	-	-	-	-	358	-						
Stage 2	-	-	-	-	-	400	-						
Approach	EB	WB	SB										
HCM Control Delay, s	0.9	0	14.3										
HCM LOS	B												
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2							
Capacity (veh/h)	770	-	-	-	212	606							
HCM Lane V/C Ratio	0.139	-	-	-	0.108	0.163							
HCM Control Delay (s)	10.4	-	-	-	24	12.1							
HCM Lane LOS	B	-	-	-	C	B							
HCM 95th %tile Q(veh)	0.5	-	-	-	0.4	0.6							
Notes	-												

Lanes, Volumes, Timings
1: Bank & Hunt Club - Optimized

Lanes, Volumes, Timings
1: Bank & Hunt Club - Optimized

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1067	213
Future Volume (vph)	149	1016	358	49	800	177	328	546	60	252	1067	213
Lane Group Flow (vph)	149	1016	358	49	800	177	328	546	60	252	1067	213
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	4	3	8	8	5	2	2	1	6	6
Permitted Phases	7	4	4	3	8	8	5	2	2	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	11.5	33.7	33.7	11.5	33.7	33.7	12.1	34.5	34.5	12.1	34.5	34.5
Total Split (s)	13.0	43.0	43.0	12.0	42.0	42.0	20.0	49.0	42.0	23.0	45.0	45.0
Total Split (%)	10.8%	35.8%	35.8%	10.0%	35.0%	35.0%	16.7%	35.0%	35.0%	19.2%	37.5%	37.5%
Maximum Green (s)	6.5	36.3	36.3	5.5	35.3	35.3	12.9	35.5	35.5	15.9	38.5	38.5
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.8	3.0	3.0	2.8	3.0	3.0	3.4	2.8	2.8	3.4	2.8	2.8
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.7	6.7	6.5	6.7	6.7	7.1	6.5	6.5	7.1	6.5	6.5
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Max	None	Max	None	Max	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	20.0	20.0	20.0	20.0	20.0	20.0	21.0	21.0	21.0	21.0	21.0	21.0
Pedestrian Calls (#/hr)	21	21	21	4	4	4	22	22	22	22	55	55
Act Effr Green (s)	6.5	38.7	38.7	5.5	35.3	35.3	12.9	37.4	37.4	14.0	38.5	38.5
Actuated G/C Ratio	0.05	0.32	0.32	0.05	0.29	0.29	0.11	0.31	0.31	0.12	0.32	0.32
v/c Ratio	0.90	0.97	0.95	0.66	0.82	0.82	0.95	0.83	0.83	0.11	0.67	1.00
Control Delay	103.7	62.4	10.6	90.7	61.4	23.4	97.7	40.2	4.2	59.9	69.4	11.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	0.0	0.0
Total Delay	103.7	62.4	10.6	90.7	61.4	23.4	97.7	41.7	4.2	59.9	69.4	11.7
LOS	F	E	B	F	E	C	F	D	A	E	E	B
Approach Delay	54.2			56.2			58.9			59.8		
Approach LOS	D			E			E			E		E
Queue Length 50th (m)	18.2	~135.1	11.2	12.1	86.6	11.1	42.6	51.1	0.3	29.5	~132.6	9.3
Queue Length 95th (m)	#37.8	#175.6	39.3	m#29.3	114.2	35.6	m#68.5	m#90.0	m#5.7	42.7	#178.7	29.3
Internal Link Dist (m)	358.7			334.1			67.1			340.8		
Turn Bay Length (m)	150.0			60.0			90.0			40.0		115.0
Base Capacity (vph)	166	1048	666	74	975	552	345	1033	551	426	1063	550
Starvation Cap Reductn	0	0	0	0	0	0	0	294	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.97	0.95	0.66	0.82	0.82	0.95	0.74	0.11	0.59	1.00	0.40

Control Type: Actuated-Coordinated	Intersection LOS: E
Maximum v/c Ratio: 1.00	IOU Level of Service F
Intersection Signal Delay: 57.2	
Intersection Capacity Utilization 97.2%	
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Appendix M

TDM Checklist

DRAFT

TDM Measures Checklist:
Non-Residential Developments (office, institutional, retail or industrial)

Legend

BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users

BETTER The measure could maximize support for users of sustainable modes, and optimize development performance

★ The measure is one of the most dependably effective tools to encourage the use of sustainable modes

TDM measures: <i>Non-residential developments</i>		Check if proposed & add descriptions
1. TDM PROGRAM MANAGEMENT		
1.1 Program coordinator		
BASIC ★	1.1.1 Designate an internal coordinator, or contract with an external coordinator	<input type="checkbox"/>
1.2 Travel surveys		
BETTER	1.2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	<input type="checkbox"/>
2. WALKING AND CYCLING		
2.1 Information on walking/cycling routes & destinations		
BASIC	2.1.1 Display local area maps with walking/cycling access routes and key destinations at major entrances	<input checked="" type="checkbox"/>
2.2 Bicycle skills training		
<i>Commuter travel</i>		
BETTER ★	2.2.1 Offer on-site cycling courses for commuters, or subsidize off-site courses	<input type="checkbox"/>
2.3 Valet bike parking		
<i>Visitor travel</i>		
BETTER	2.3.1 Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	<input type="checkbox"/>

TDM measures: *Non-residential developments*

TDM measures: <i>Non-residential developments</i>		Check if proposed & add descriptions
3. TRANSIT		
3.1 Transit information		
BASIC	3.1.1 Display relevant transit schedules and route maps at entrances	<input checked="" type="checkbox"/>
BASIC	3.1.2 Provide online links to OC Transpo and STO information	<input checked="" type="checkbox"/>
BETTER	3.1.3 Provide real-time arrival information display at entrances	<input type="checkbox"/>
3.2 Transit fare incentives		
<i>Commuter travel</i>		
BETTER	3.2.1 Offer preloaded PRESTO cards to encourage commuters to use transit	<input type="checkbox"/>
BETTER ★	3.2.2 Subsidize or reimburse monthly transit pass purchases by employees	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.2.3 Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	<input type="checkbox"/>
3.3 Enhanced public transit service		
<i>Commuter travel</i>		
BETTER	3.3.1 Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.3.2 Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	<input type="checkbox"/>
3.4 Private transit service		
<i>Commuter travel</i>		
BETTER	3.4.1 Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.4.2 Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	<input type="checkbox"/>

TDM measures: Non-residential developments		Check if proposed & add descriptions
4. RIDESHARING		
<i>Commuter travel</i>		
BASIC ★	4.1.1 Provide a dedicated ride-matching portal at OttawaRideMatch.com	<input checked="" type="checkbox"/>
4.2 Carpool parking price incentives		
<i>Commuter travel</i>		
BETTER	4.2.1 Provide discounts on parking costs for registered carpools	<input type="checkbox"/>
4.3 Vanpool service		
<i>Commuter travel</i>		
BETTER	4.3.1 Provide a vanpooling service for long-distance commuters	<input type="checkbox"/>
5. CARSHARING & BIKESHARING		
5.1 Bikeshare stations & memberships		
BETTER	5.1.1 Contract with provider to install on-site bikeshare station for use by commuters and visitors	<input type="checkbox"/>
<i>Commuter travel</i>		
BETTER	5.1.2 Provide employees with bikeshare memberships for local business travel	<input type="checkbox"/>
5.2 Carshare vehicles & memberships		
<i>Commuter travel</i>		
BETTER	5.2.1 Contract with provider to install on-site carshare vehicles and promote their use by tenants	<input type="checkbox"/>
BETTER	5.2.2 Provide employees with carshare memberships for local business travel	<input type="checkbox"/>
6. PARKING		
<i>Commuter travel</i>		
BASIC ★	6.1.1 Charge for long-term parking (daily, weekly, monthly)	<input checked="" type="checkbox"/>
BASIC	6.1.2 Unbundle parking cost from lease rates at multi-tenant sites	<input checked="" type="checkbox"/>
<i>Visitor travel</i>		
BETTER	6.1.3 Charge for short-term parking (hourly)	<input type="checkbox"/>

TDM measures: Non-residential developments		Check if proposed & add descriptions
7. TDM MARKETING & COMMUNICATIONS		
7.1 Multimodal travel information		
<i>Commuter travel</i>		
BASIC ★	7.1.1 Provide a multimodal travel option information package to new/relocating employees and students	<input checked="" type="checkbox"/>
<i>Visitor travel</i>		
BETTER	7.1.2 Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	<input type="checkbox"/>
7.2 Personalized trip planning		
<i>Commuter travel</i>		
BETTER	7.2.1 Offer personalized trip planning to new/relocating employees	<input type="checkbox"/>
7.3 Promotions		
<i>Commuter travel</i>		
BETTER	7.3.1 Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	<input type="checkbox"/>
8. OTHER INCENTIVES & AMENITIES		
8.1 Emergency ride home		
<i>Commuter travel</i>		
BETTER	8.1.1 Provide emergency ride home service to non-driving commuters	<input type="checkbox"/>
8.2 Alternative work arrangements		
<i>Commuter travel</i>		
BASIC ★	8.2.1 Encourage flexible work hours	<input checked="" type="checkbox"/>
BETTER	8.2.2 Encourage compressed workweeks	<input type="checkbox"/>
BETTER	8.2.3 Encourage telework	<input checked="" type="checkbox"/>
8.3 Local business travel options		
<i>Commuter travel</i>		
BASIC ★	8.3.1 Provide local business travel options that minimize the need for employees to bring a personal car to work	<input type="checkbox"/>
8.4 Commuter incentives		
<i>Commuter travel</i>		
BETTER	8.4.1 Offer employees a taxable, mode-neutral commuting allowance	<input type="checkbox"/>
8.5 On-site amenities		
<i>Commuter travel</i>		
BETTER	8.5.1 Provide on-site amenities/services to minimize mid-day or mid-commute errands	<input type="checkbox"/>

**TDM-Supportive Development Design and Infrastructure Checklist:
Non-Residential Developments (office, institutional, retail or industrial)**

Legend	
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
1. WALKING & CYCLING: ROUTES		
1.1 Building location & access points		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	<input type="checkbox"/>
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	<input type="checkbox"/>
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	<input type="checkbox"/>
1.2 Facilities for walking & cycling		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see <i>Official Plan policy 4.3.3</i>)	<input type="checkbox"/>
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see <i>Official Plan policy 4.3.12</i>)	<input checked="" type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see <i>Official Plan policy 4.3.10</i>)	<input checked="" type="checkbox"/>
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see <i>Official Plan policy 4.3.10</i>)	<input checked="" type="checkbox"/>
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see <i>Official Plan policy 4.3.11</i>)	<input checked="" type="checkbox"/>
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	<input type="checkbox"/>
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	<input type="checkbox"/>
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	<input type="checkbox"/>
1.3 Amenities for walking & cycling		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	<input type="checkbox"/>
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
2. WALKING & CYCLING: END-OF-TRIP FACILITIES		
2.1 Bicycle parking		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i>)	<input checked="" type="checkbox"/>
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i>)	<input checked="" type="checkbox"/>
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions, that no more than 50% of spaces are vertical spaces, and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i>)	<input checked="" type="checkbox"/>
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	<input type="checkbox"/>
BETTER	2.1.5 Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	<input type="checkbox"/>
2.2 Secure bicycle parking		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i>)	<input checked="" type="checkbox"/>
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	<input type="checkbox"/>
2.3 Shower & change facilities		
BASIC	2.3.1 Provide shower and change facilities for the use of active commuters	<input type="checkbox"/>
BETTER	2.3.2 In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	<input type="checkbox"/>
2.4 Bicycle repair station		
BETTER	2.4.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
3. TRANSIT		
3.1 Customer amenities		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	<input type="checkbox"/>
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	<input type="checkbox"/>
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	<input type="checkbox"/>
4. RIDESHARING		
4.1 Pick-up & drop-off facilities		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	<input type="checkbox"/>
4.2 Carpool parking		
BASIC	4.2.1 Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	<input type="checkbox"/>
BETTER	4.2.2 At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	<input type="checkbox"/>
5. CARSHARING & BIKESHARING		
5.1 Carshare parking spaces		
BETTER	5.1.1 Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (see <i>Zoning By-law Section 94</i>)	<input type="checkbox"/>
5.2 Bikeshare station location		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
6. PARKING		
6.1 Number of parking spaces		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	<input checked="" type="checkbox"/>
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	<input type="checkbox"/>
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see <i>Zoning By-law Section 704</i>)	<input type="checkbox"/>
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see <i>Zoning By-law Section 111</i>)	<input type="checkbox"/>
6.2 Separate long-term & short-term parking areas		
BETTER	6.2.1 Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	<input type="checkbox"/>
7. OTHER		
7.1 On-site amenities to minimize off-site trips		
BETTER	7.1.1 Provide on-site amenities to minimize mid-day or mid-continute errands	<input type="checkbox"/>