

# 2370 Tenth Line Road

## Transportation Impact Assessment

Step 1 Screening Report

Step 2 Scoping Report

Step 3 Forecasting Report

Step 4 Strategy Report

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## 2.2 Existing Conditions

### 2.2.1 Area Road Network

*Tenth Line Road:* Tenth Line Road is a City of Ottawa arterial road with a divided four-lane urban cross-section throughout the majority of the study area that transitions to a two-lane rural cross-section 135 metres south of Harvest Valley Avenue. Bike lanes are included on both sides of the road where the cross-section is urban and transition to paved shoulders where the cross-section is rural. North of Gerry Lalonde Drive, the cross-section includes a MUP on the west side and a sidewalk on the east side of the road, and between Gerry Lalonde Drive and the Shops of Tenth Line access, sidewalks are on both sides of the road. Between the Shops of Tenth Line access and 135 metres south of Harvest Valley Avenue, a MUP is present on the west side and a sidewalk is present on the east side of the road. The posted speed limit is 60 km/h and the measured right-of-way is 37.5 metres. Tenth Line Road is a truck route.

*Brian Coburn Boulevard:* Brian Coburn Boulevard is a City of Ottawa arterial road with a two-lane urban cross-section including a sidewalk on the north side of the road. The posted speed limit is 60 km/h and the measured right-of-way is 40.0 metres. Brian Coburn Boulevard is a truck route east of Tenth Line Road.

*Esprit Drive:* Esprit Drive is a City of Ottawa major collector road north and a collector road to the south of Brian Coburn Boulevard, each with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the posted speed limit is 50 km/h and the measured right-of-way is 26.0 metres.

*Decoeur Drive:* Decoeur Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the unposted speed limit is assumed to be 50 km/h and the measured right-of-way is 28.0 metres.

*Gerry Lalonde Drive:* Gerry Lalonde Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the posted speed limit is 50 km/h and the measured right-of-way is 26.0 metres.

*Jerome Jodoin Drive:* Jerome Jodoin Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the unposted speed limit is assumed to be 50 km/h and the measured right-of-way is 24.0 metres.

*Des Aubepines Drive:* des Aubepines Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on the west side of the road and in a layby in front of the Notre-Place Elementary School on the east side of the road. The unposted speed limit is assumed to be 50 km/h and the measured right-of-way is 28.0 metres.

*Aquaview Drive:* Aquaview Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road south of Lakepointe Drive and on the west side of the road to the north. On-street parking is permitted on both sides of the road to the south and one the west side of the road to the north of Lakepointe Drive. The posted speed limit is 40 km/h and the measured right-of-way is 26.0 metres.

*Lakeridge Drive:* Lakeridge Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road. The unposted speed limit is assumed to be 50 km/h and the measured right-of-way is 26.0 metres.

*Lakepointe Drive:* Lakepointe Drive is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the unposted speed limit is assumed to be 50 km/h and the measured right-of-way is 30.0 metres.

*Southfield Way:* Southfield Way is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on both sides of the road, the posted speed limit is 50 km/h and the measured right-of-way is 26.0 metres.

*Harvest Valley Avenue:* Harvest Valley Avenue is a City of Ottawa collector road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on the south side of the road, the posted speed limit is 50 km/h and the measured right-of-way is 26.0 metres.

*Strasbourg Street:* Strasbourg Street is a City of Ottawa local road with a two-lane urban cross-section including sidewalks on both sides of the road. On-street parking is permitted on the south side of the road, the posted speed limit is 50 km/h and the measured right-of-way is 24.0 metres.

*Sweetvalley Drive:* Sweetvalley Drive is a City of Ottawa local road with a two-lane urban cross-section including a sidewalk on the north and west sides of the road within the study area. On-street parking is permitted on both sides of the road, the posted speed limit is 40 km/h and the measured right-of-way is 26.0 metres.

### 2.2.2 Existing Intersections

The key existing signalized area within one kilometre of the site have been summarized below:

*Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road* The intersection of Gerry Lalonde Drive/Lakepointe Drive at Tenth Line Road is a signalized intersection. The northbound and southbound approaches each consist of an auxiliary left-turn lane, two through lanes, a bike lane, and an auxiliary right turn lane. The eastbound functionally consists of an unpainted auxiliary left-turn lane and a shared through/right-turn lane, and the westbound approach consists of an auxiliary left-turn lane, a through lane, and an auxiliary right-turn lane. No turn restrictions were noted.

*The Shops of Tenth Line Access at Tenth Line Road* The intersection of The Shops of Tenth Line access at Tenth Line Road is a signalized intersection. The northbound approach consists of an auxiliary left-turn lane, two through lanes, and a bike lane and the southbound approach consists of two through lanes, a bike lane, and an auxiliary right-turn lane. The private eastbound approach consists of an unmarked left-turn lane and right-turn lane. No turn restrictions were noted.

*Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive* The intersection of Brian Coburn Boulevard at Gerry Lalonde Drive/Jerome Jodoin Drive is a roundabout with shared all-movement lanes yield-controlled on all approaches. No turn restrictions were noted.

*Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive* The intersection of Brian Coburn Boulevard at Strasbourg Street/Des Aubepines Drive a roundabout with shared all-movement lanes yield-controlled on all approaches. No turn restrictions were noted.

*Brian Coburn Boulevard at Tenth Line Road* The intersection of Brian Coburn Boulevard at Tenth Line Road is a signalized intersection with an auxiliary left-turn lane on each approach. The northbound and southbound approaches each also



have a through lane, a shared through/right-turn lane, and a bike lane. The eastbound approach additionally has a shared through/right-turn lane, and the westbound approach additionally has a through lane and an auxiliary channelized right-turn lane. No turn restrictions were noted.

*Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive*

The intersection of Brian Coburn Boulevard at Aquaview Drive/Lakeridge Drive is a signalized intersection with an auxiliary left-turn lane and a shared through/right-turn lane on each approach. No turn restrictions were noted.

*Brian Coburn Boulevard at Esprit Drive*

The intersection of Brian Coburn Boulevard at Esprit Drive is a signalized intersection with an auxiliary left-turn lane and a shared through/right-turn lane on each approach. No turn restrictions were noted.

*Decoeur Drive / Southfield Way at Tenth Line Road*

The intersection of Decoeur Drive/Southfield Way at Tenth Line Road is a signalized intersection with an auxiliary left-turn lane on each approach. The northbound and southbound approaches each also have two through lanes, a bike lane, and an auxiliary right-turn lane. The eastbound and westbound approaches additionally each have a shared through/right-turn lane. No turn restrictions were noted.

*Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road*

The intersection of Sweetvalley Drive/Harvest Valley Avenue at Tenth Line Road is a signalized intersection with an auxiliary left-turn lane on each approach. The northbound and southbound approaches each also have a through lane, a shared through/right-turn lane, and a bike lane. The eastbound and westbound lanes additionally each have a shared through/right-turn lane. No turn restrictions were noted.

2.2.3 Existing Driveways

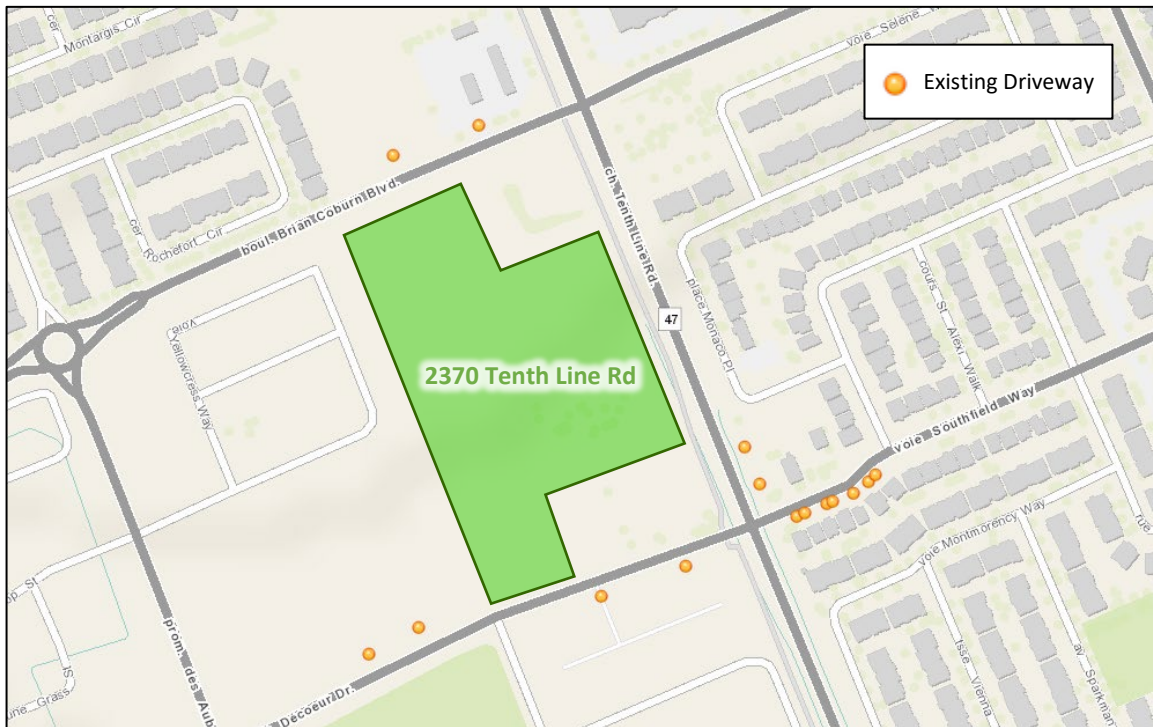
Within 200 metres of the proposed site access on Brian Coburn Boulevard, a driveway to The Shops of Tenth Line and a driveway to a gas station with drive-through coffee and drive-through carwash facilities are present on the north side of Brian Coburn Boulevard. Queueing from these drive-through facilities does not pose potential for conflict with the driveway onto Brian Coburn Boulevard.

Within 200 metres of the proposed site access on Decoeur Drive, a private road accessing a number of townhomes and a driveway to the community home sales centre are present on the south side of Decoeur Drive and a two-driveway loop is present on the north side of Decoeur Drive west of Tenth Line Road. East of Tenth Line Road, seven driveways to single detached homes are present on the south side of Southfield Way.

Within 200 metres of the proposed site access on Tenth Line Road, a two-driveway loop to a home cabinetry business and accompanying workshop is present on the east side of Tenth Line Road. As Tenth Line Road is separated by a median, no potential conflict exists with these driveways.

Figure 3 illustrates the existing area driveways.

Figure 3: Existing Driveways



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: October 1, 2021

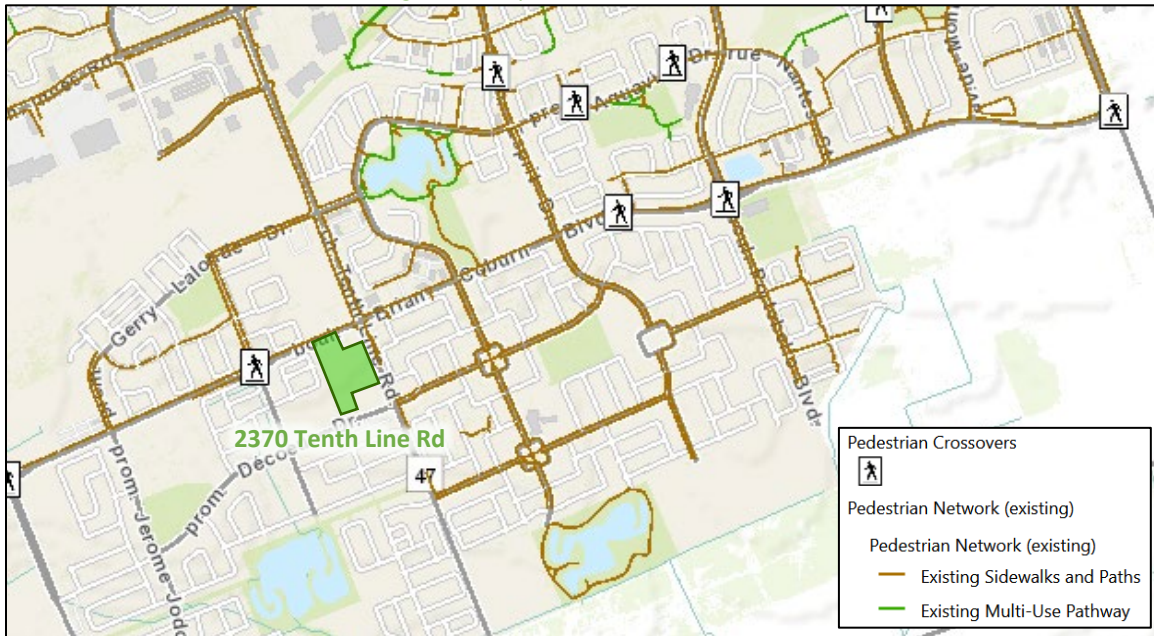
#### 2.2.4 Cycling and Pedestrian Facilities

Figure 4 illustrates the pedestrian facilities in the study area and Figure 5 illustrates the cycling facilities.

Sidewalks are provided along the north side of Brian Coburn Boulevard, the east side of Tenth Line, the west side of Aquaview Drive north of Lakepointe Drive, the north/west side of Sweetvalley Drive, and on both sides of all other study area roadways within the study area. A pathway is provided around the stormwater management facility within Aquaview Park and a mixed-use path (MUP) is provided on the west side of Tenth Line Road.

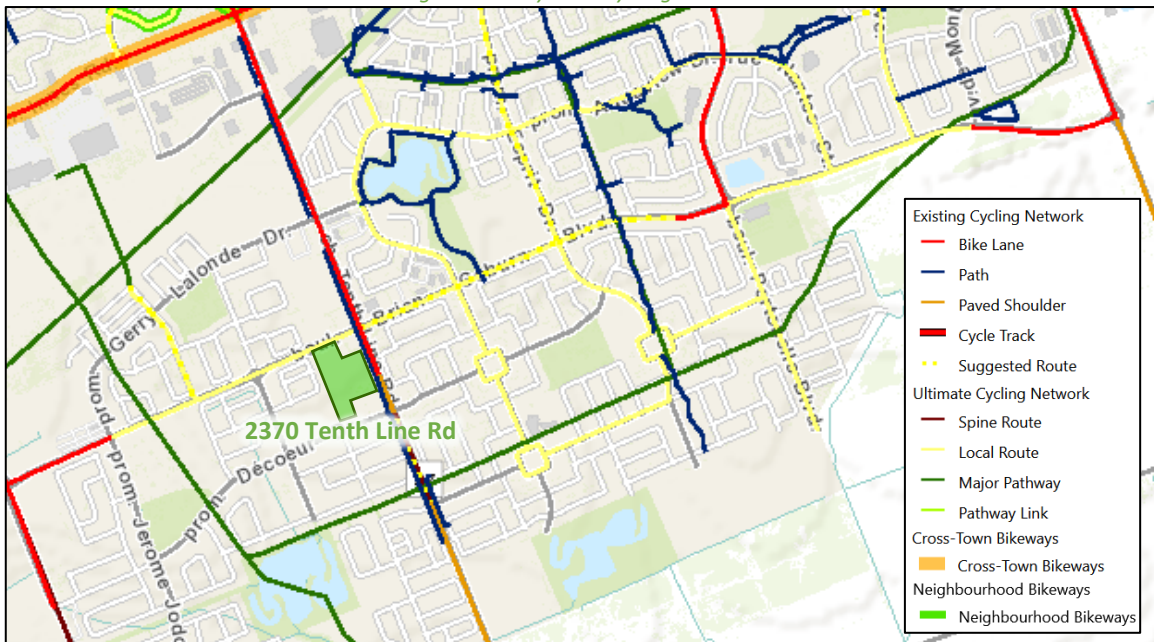
Cycling facilities include the MUP along the west side of Tenth Line Road, bike lanes on Tenth Line Road transitioning to paved shoulders 135 metres south of Harvest Valley Avenue. Tenth Line Road is a spine route, Brian Coburn Boulevard, Cabris Crescent/Azure Street/Trigoria Crescent, Aquaview Drive, Lakeridge Drive, and Esprit Drive are local routes.

Figure 4: Study Area Pedestrian Facilities



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: September 27, 2021

Figure 5: Study Area Cycling Facilities



Source: <http://maps.ottawa.ca/geoOttawa/> Accessed: September 27, 2021

Pedestrian and cyclist volumes included in study area intersection counts, presented in Section 2.2.7, have been compiled and are illustrated in Figure 6 and Figure 7 respectively.

Figure 6: Existing Study Area Pedestrian Volumes

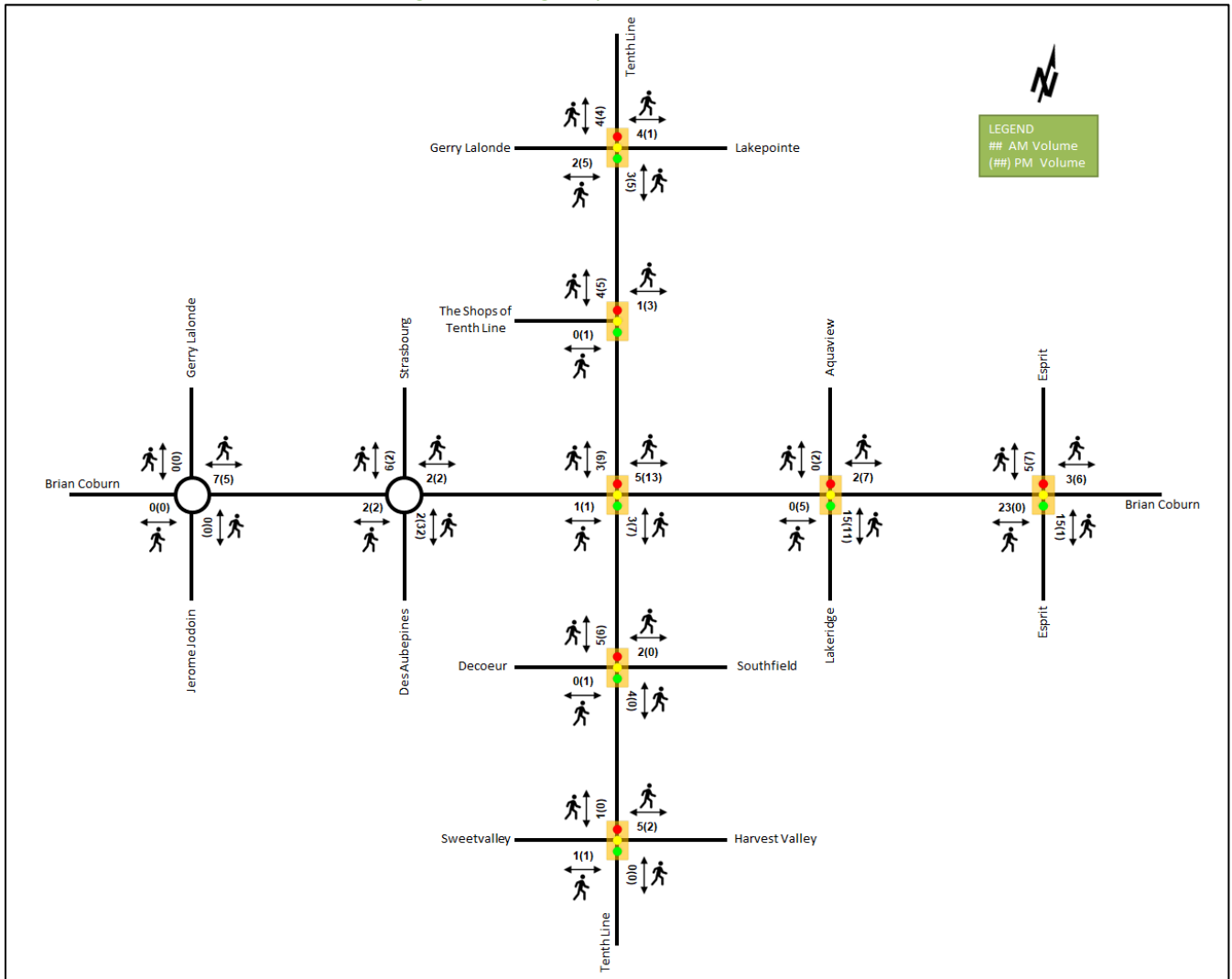
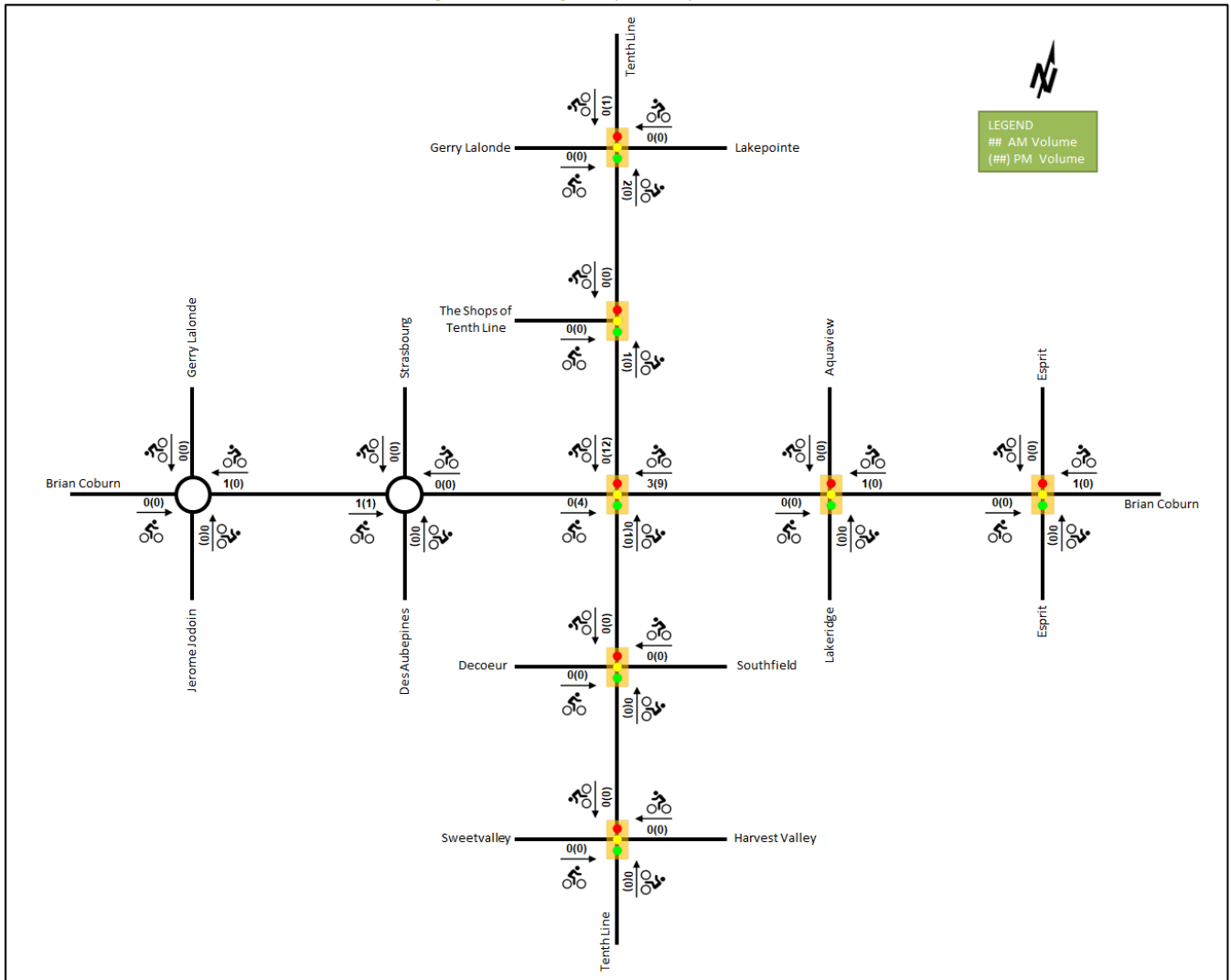


Figure 7: Existing Study Area Cyclist Volumes



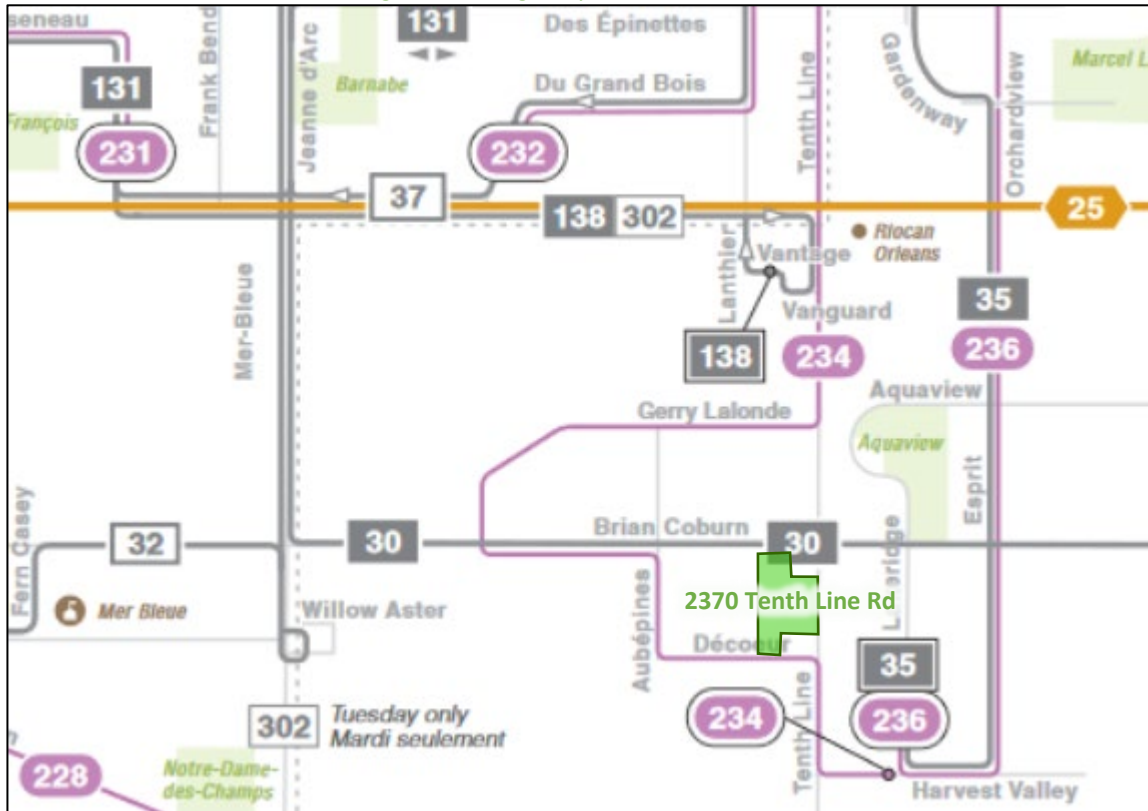
### 2.2.5 Existing Transit

Within the study area, the route #234, connecting to Place d’Orleans and Blair Stations, passes the site on Decoeur Drive and the route #30, connecting to Jeanne d’Arc and Blair Stations, travels along Brian Coburn Boulevard. The frequency of these routes within proximity of the proposed site currently are:

- Route # 30 – 15-minute service in the peak period/direction, 30-minute service all-day
- Route # 234 – 30-minute service operating in the peak period/direction

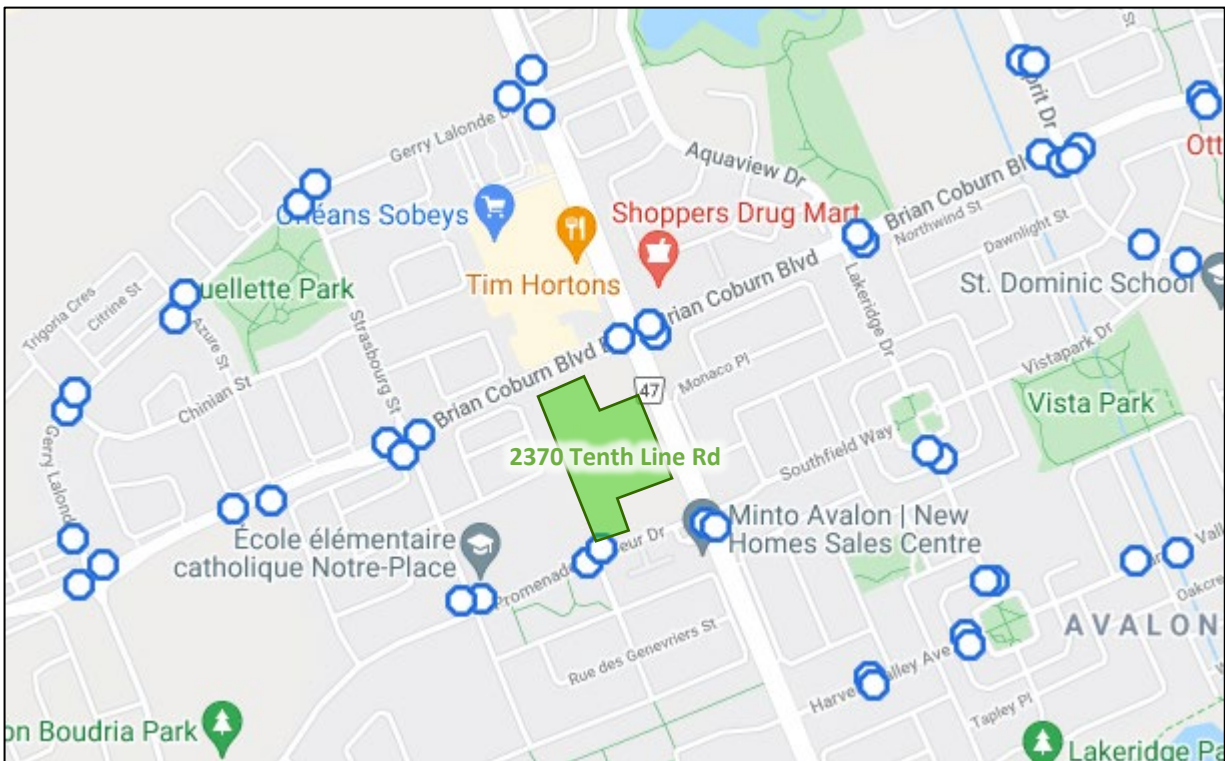
Figure 8 illustrates the transit system map in the study area and Figure 9 illustrates nearby transit stops.

Figure 8: Existing Study Area Transit Service



Source: <http://www.octranspo.com/> Accessed: September 28, 2021

Figure 9: Existing Study Area Transit Stops



Source: <http://www.octranspo.com/> Accessed: September 28, 2021

### 2.2.6 Existing Area Traffic Management Measures

Speed display devices are present on collector and local roads throughout the study area, and on-road messaging and vertical treatments in the form of flexible bollards are present along collector roads.

### 2.2.7 Existing Peak Hour Travel Demand

Existing turning movement counts were acquired from the City of Ottawa for the existing Study Area intersection. Table 1 summarizes the intersection count dates.

*Table 1: Intersection Count Date*

<b>Intersection</b>	<b>Count Date</b>
<b>Gerry Lalonde Drive/Lakepointe Drive at Tenth Line Road</b>	Thursday, April 19, 2018
<b>The Shops of Tenth Line Access at Tenth Line Road</b>	Tuesday, January 15, 2019
<b>Brian Coburn Boulevard at Gerry Lalonde Drive/Jerome Jodoin Drive</b>	Wednesday, October 17, 2018
<b>Brian Coburn Boulevard at Strasbourg Street/Des Aubepines Drive</b>	Thursday, April 20, 2017
<b>Brian Coburn Boulevard at Tenth Line Road</b>	Wednesday, September 19, 2018
<b>Brian Coburn Boulevard at Aquaview Drive/Lakeridge Drive</b>	Tuesday, February 26, 2019
<b>Brian Coburn Boulevard at Esprit Drive</b>	Tuesday, February 26, 2019
<b>Decoeur Drive/Southfield Way at Tenth Line Road</b>	Thursday, February 9, 2017
<b>Sweetvalley Drive/Harvest Valley Avenue at Tenth Line Road</b>	Thursday, April 19, 2018

Figure 10 illustrates the 2021 existing traffic volumes from the counts and forecasted traffic from interim development within the Avalon West and Summerside West communities, and Table 2 summarizes the existing intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection and HCM 2010 average delay for unsignalized intersections. Synchro 11 has been used to model the signalized intersections and Sidra 8 to model the study area roundabouts. Detailed turning movement count data is included in Appendix B, the Synchro and Sidra worksheets are provided in Appendix C, and the background developments included in the existing conditions are provided in Appendix D.

Figure 10: Existing Traffic Counts

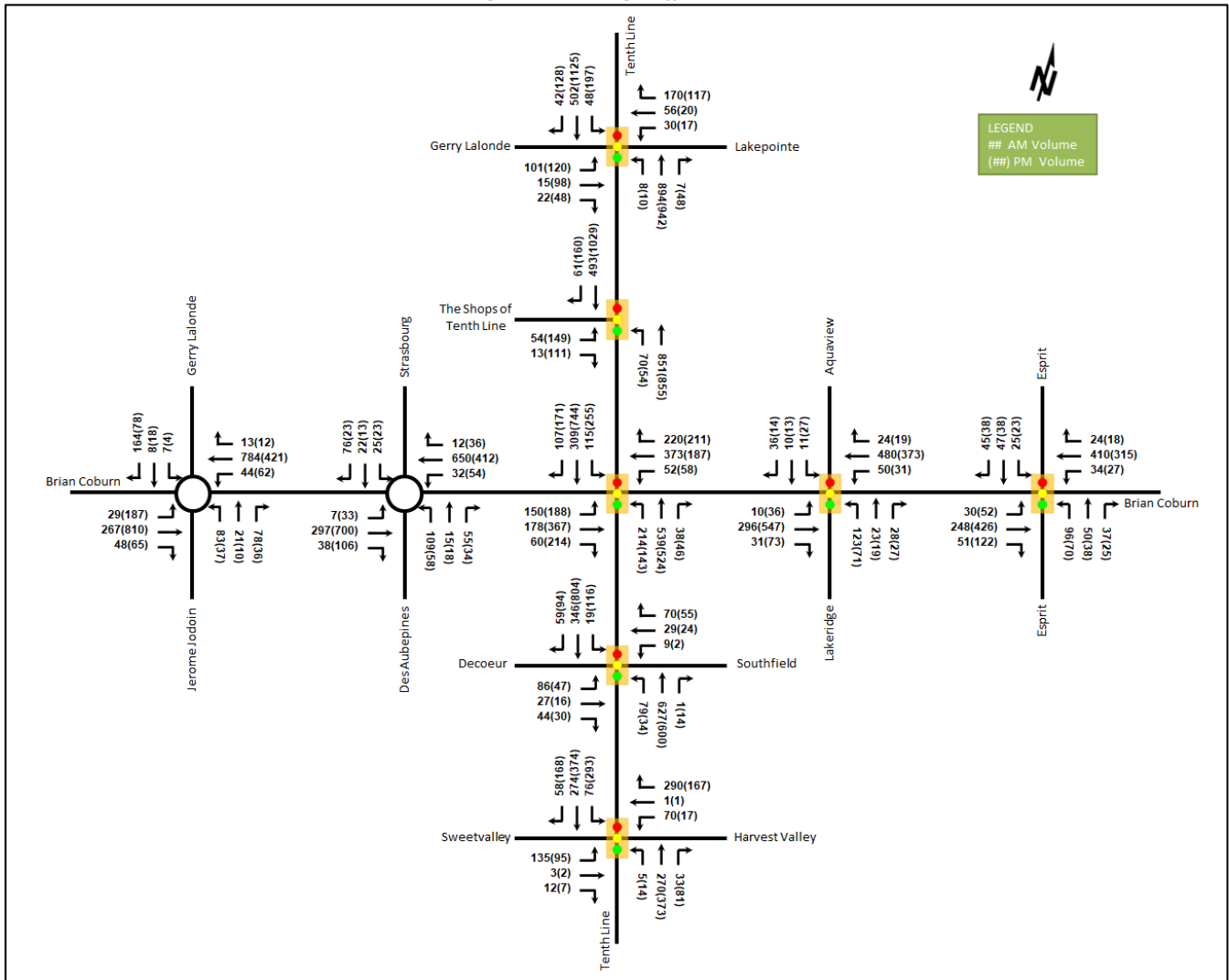


Table 2: Existing Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road <i>Signalized</i>	EBL	A	0.55	42.3	29.0	B	0.62	49.6	38.0
	EBT/R	A	0.14	16.4	9.2	A	0.55	38.0	39.1
	WBL	A	0.15	29.9	10.9	A	0.11	33.1	8.4
	WBT	A	0.20	30.7	16.9	A	0.08	32.1	9.0
	WBR	A	0.56	21.7	28.5	A	0.39	13.2	17.5
	NBL	A	0.02	4.2	m0.9	A	0.05	3.8	m0.9
	NBT	A	0.44	4.8	46.3	A	0.45	4.4	20.8
	NBR	A	0.01	0.0	m0.0	A	0.05	0.7	0.8
	SBL	A	0.17	9.0	11.2	C	0.73	28.9	#81.1
	SBT	A	0.26	6.8	35.3	A	0.54	9.1	96.1
	SBR	A	0.05	2.7	4.4	A	0.14	1.7	7.1
<b>Overall</b>		<b>A</b>	<b>0.46</b>	<b>10.3</b>	-	<b>B</b>	<b>0.70</b>	<b>12.1</b>	-



Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>The Shops of Tenth Line Access at Tenth Line Road Signalized</b>	EBL	A	0.31	41.1	20.9	A	0.58	44.4	41.8
	EBR	A	0.08	17.8	5.3	A	0.39	19.4	20.7
	NBL	A	0.13	2.9	m4.4	A	0.23	8.6	m13.4
	NBT	A	0.37	3.1	21.2	A	0.41	6.1	66.4
	SBT	A	0.22	2.1	9.6	A	0.50	5.8	34.9
	SBR	A	0.06	0.4	0.1	A	0.17	0.7	2.7
	<b>Overall</b>	<b>A</b>	<b>0.38</b>	<b>4.1</b>	<b>-</b>	<b>A</b>	<b>0.51</b>	<b>8.7</b>	<b>-</b>
<b>Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive Roundabout</b>	EB	A	0.27	4.3	12.8	A	0.83	5.9	99.8
	WB	A	0.71	5.3	57.2	A	0.30	6.1	27.7
	NB	A	0.20	6.5	7.8	B	0.24	15.3	12.7
	SB	B	0.41	13.2	23.4	A	0.14	5.9	6.0
	<b>Overall</b>	<b>A</b>	<b>0.71</b>	<b>6.2</b>	<b>-</b>	<b>A</b>	<b>0.83</b>	<b>6.4</b>	<b>-</b>
<b>Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive Roundabout</b>	EB	A	0.28	4.1	13.2	A	0.66	4.7	49.9
	WB	A	0.60	5.0	40.2	A	0.43	4.9	23.6
	NB	A	0.20	7.5	7.9	B	0.19	10.9	9.1
	SB	A	0.23	10.0	10.9	A	0.08	7.5	3.2
	<b>Overall</b>	<b>A</b>	<b>0.60</b>	<b>5.6</b>	<b>-</b>	<b>A</b>	<b>0.66</b>	<b>5.3</b>	<b>-</b>
<b>Brian Coburn Boulevard at Tenth Line Road Signalized</b>	EBL	<b>F</b>	<b>1.04</b>	<b>111.4</b>	<b>#61.5</b>	A	0.51	27.6	51.4
	EBT/R	A	0.52	25.7	49.9	E	0.96	55.5	<b>#184.7</b>
	WBL	A	0.22	22.1	14.7	C	0.71	68.9	<b>#33.6</b>
	WBT	C	0.76	36.6	85.3	A	0.30	21.8	43.3
	WBR	A	0.41	7.0	19.3	A	0.34	5.4	17.4
	NBL	A	0.52	16.0	21.1	<b>F</b>	<b>1.08</b>	<b>128.7</b>	<b>#70.9</b>
	NBT/R	A	0.36	10.3	23.5	A	0.40	17.5	53.7
	SBL	A	0.37	23.5	40.8	E	0.94	56.9	<b>#96.3</b>
	SBT/R	A	0.27	14.5	47.7	B	0.66	12.3	43.8
<b>Overall</b>	<b>B</b>	<b>0.70</b>	<b>24.3</b>	<b>-</b>	<b>F</b>	<b>1.02</b>	<b>32.4</b>	<b>-</b>	
<b>Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive Signalized</b>	EBL	A	0.02	6.5	2.5	A	0.06	4.8	5.2
	EBT/R	A	0.33	7.5	39.4	A	0.54	8.1	81.9
	WBL	A	0.09	6.8	7.8	A	0.08	5.1	4.8
	WBT/R	A	0.48	9.3	69.6	A	0.34	5.8	41.7
	NBL	A	0.54	30.4	28.5	A	0.41	34.7	21.4
	NBT/R	A	0.17	12.8	10.0	A	0.19	16.4	10.9
	SBL	A	0.05	19.7	4.7	A	0.17	28.9	10.3
	SBT/R	A	0.15	10.1	8.1	A	0.12	17.6	8.0
	<b>Overall</b>	<b>A</b>	<b>0.54</b>	<b>11.2</b>	<b>-</b>	<b>A</b>	<b>0.56</b>	<b>9.7</b>	<b>-</b>
<b>Brian Coburn Boulevard at Esprit Drive Signalized</b>	EBL	A	0.09	10.4	6.7	A	0.13	10.7	10.2
	EBT/R	A	0.39	12.2	43.8	B	0.68	18.1	97.2
	WBL	A	0.09	10.2	7.2	A	0.11	11.0	6.5
	WBT/R	A	0.54	15.3	71.0	A	0.41	13.1	50.6
	NBL	A	0.27	22.3	24.1	A	0.20	21.1	18.3
	NBT/R	A	0.19	13.2	16.1	A	0.13	13.6	12.9
	SBL	A	0.08	19.4	8.4	A	0.06	19.2	8.0
	SBT/R	A	0.19	12.0	15.8	A	0.16	12.0	13.8
<b>Overall</b>	<b>A</b>	<b>0.44</b>	<b>14.4</b>	<b>-</b>	<b>A</b>	<b>0.49</b>	<b>15.8</b>	<b>-</b>	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Decoeur Drive / Southfield Way at Tenth Line Road</b> <i>Signalized</i>	EBL	A	0.45	37.0	22.2	A	0.28	38.3	15.5
	EBT/R	A	0.27	14.6	12.0	A	0.19	17.1	10.1
	WBL	A	0.04	24.7	4.3	A	0.01	29.0	1.8
	WBT/R	A	0.32	12.3	13.6	A	0.31	15.8	13.8
	NBL	A	0.15	8.9	18.2	A	0.09	7.8	9.2
	NBT	A	0.31	7.7	55.1	A	0.26	6.2	50.2
	NBR	A	0.00	0.0	0.0	A	0.01	0.5	0.6
	SBL	A	0.05	6.3	m3.4	A	0.24	4.8	m10.5
	SBT	A	0.17	5.5	17.1	A	0.35	3.8	m32.0
	SBR	A	0.07	2.2	2.8	A	0.09	0.4	m0.0
<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>9.6</b>	<b>-</b>	<b>-</b>	<b>A</b>	<b>0.36</b>	<b>6.3</b>	<b>-</b>
<b>Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road</b> <i>Signalized</i>	EBL	D	0.82	56.2	#38.7	B	0.62	49.3	32.0
	EBT/R	A	0.04	10.6	4.1	A	0.04	17.4	4.1
	WBL	A	0.23	21.3	17.3	A	0.09	29.2	8.1
	WBT/R	A	0.53	6.0	15.7	A	0.46	8.4	15.5
	NBL	A	0.01	9.6	2.3	A	0.03	6.9	3.9
	NBT/R	A	0.20	8.6	21.3	A	0.23	5.9	29.0
	SBL	A	0.16	10.6	15.2	A	0.59	14.9	75.6
	SBT/R	A	0.21	8.2	22.0	A	0.28	5.4	31.2
	<b>Overall</b>	<b>A</b>	<b>0.39</b>	<b>14.0</b>	<b>-</b>	<b>-</b>	<b>A</b>	<b>0.59</b>	<b>10.6</b>

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 0.90

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

During both the AM and PM peak hours, the study area intersections generally operate well with some capacity issues noted at the intersection of Brian Coburn Boulevard and Tenth Line Road.

During the AM peak hour at the intersection of Brian Coburn Boulevard and Tenth Line Road, the eastbound left movement is over theoretical capacity and may be subject to high delays and extended queues. During the PM peak hour, the northbound left movement is over theoretical capacity and may be subject to high delays and extended queues, the eastbound through, westbound left, and southbound left movements may exhibit extended queues, and the overall intersection is over theoretical capacity.

Additionally, within the study area, the southbound left movement at the intersection of Gerry Lalonde Drive/Lakepointe Drive at Tenth Line Road may exhibit extended queues during the PM peak hour and the eastbound left movement at the intersection of Sweetvalley Drive/Harvest Valley Avenue at Tenth Line Road may exhibit extended queues during the AM peak hour.

### 2.2.8 Collision Analysis

Collision data have been acquired from the City of Ottawa open data website (data.ottawa.ca) for five years prior to the commencement of this TIA for the surrounding study area road network. Table 3 summarizes the collisions types and conditions in the study area, Figure 11 illustrates the intersections and segments analyzed, and Table 4 summarizes the total collisions for each of these locations. Collision data are included in Appendix E.

Table 3: Study Area Collision Summary, 2015-2019

		Number	%
<b>Total Collisions</b>		<b>63</b>	<b>100%</b>
<b>Classification</b>	<b>Fatality</b>	0	0%
	<b>Non-Fatal Injury</b>	13	21%
	<b>Property Damage Only</b>	50	79%
<b>Initial Impact Type</b>	<b>Approaching</b>	1	2%
	<b>Angle</b>	5	8%
	<b>Rear end</b>	30	48%
	<b>Sideswipe</b>	5	8%
	<b>Turning Movement</b>	13	21%
	<b>SMV Other</b>	8	13%
	<b>Other</b>	1	2%
<b>Road Surface Condition</b>	<b>Dry</b>	37	59%
	<b>Wet</b>	15	24%
	<b>Loose Snow</b>	5	8%
	<b>Ice</b>	6	10%
<b>Pedestrian Involved</b>		2	3%
<b>Cyclists Involved</b>		0	0%

Figure 11: Study Area Collision Records – Representation of 2015-2019

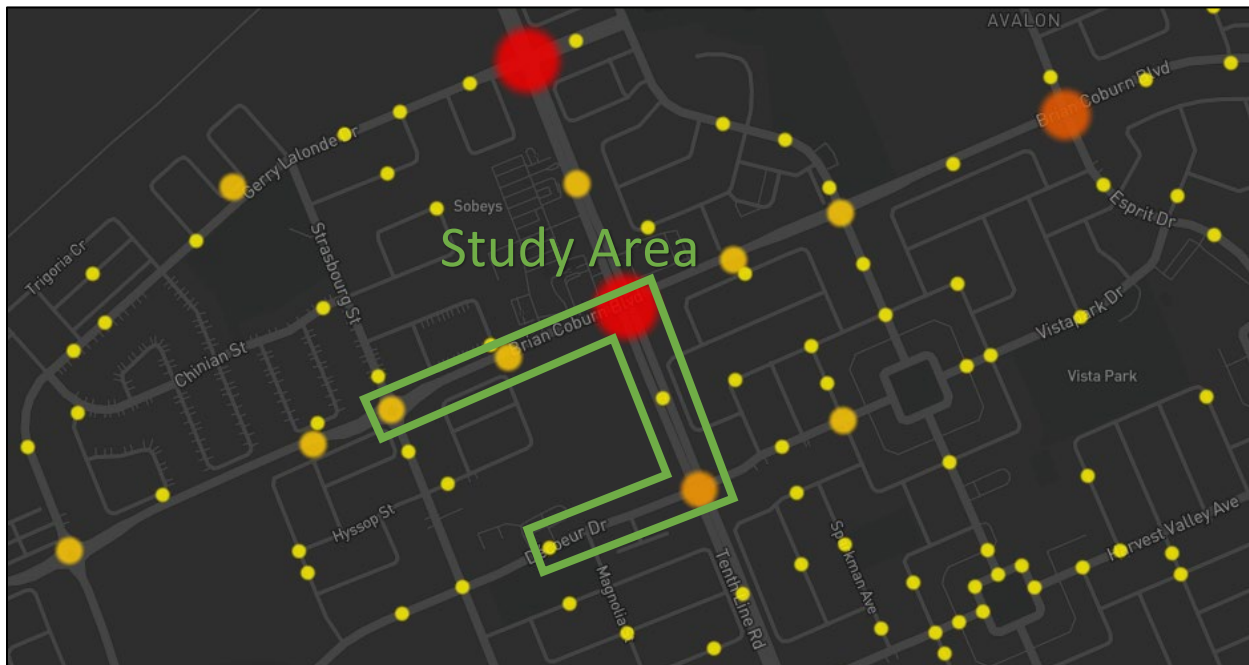


Table 4: Summary of Collision Locations, 2014-2018

Intersections / Segments	Number	%
<b>Brian Coburn Blvd @ Strasbourg St</b>	8	13%
<b>Brian Coburn Blvd @ Tenth Line Rd</b>	44	70%
<b>Decoeur Dr/Southfield Way @ Tenth Line Rd</b>	3	5%
<b>Brian Coburn Blvd btwn Strasbourg St &amp; Tenth Line Rd</b>	4	6%
<b>Tenth Line Rd btwn Brian Coburn Blvd &amp; Southfield Way</b>	3	5%
<b>Decoeur Dr btwn des Aubepines Dr &amp; Magnolia St</b>	1	2%

Within the study area, the intersection of Brian Coburn Boulevard at Tenth Line Road is noted to have experienced higher collisions than other locations. Table 5 summarizes the collision types and conditions for the intersection of Brian Coburn Boulevard at Tenth Line Road.

*Table 5: Brian Coburn Boulevard at Tenth Line Road Collision Summary*

		Number	%
<b>Total Collisions</b>		<b>44</b>	<b>100%</b>
<b>Classification</b>	<b>Fatality</b>	0	0%
	<b>Non-Fatal Injury</b>	11	25%
	<b>Property Damage Only</b>	33	75%
<b>Initial Impact Type</b>	<b>Angle</b>	2	5%
	<b>Rear end</b>	20	45%
	<b>Sideswipe</b>	5	11%
	<b>Turning Movement</b>	10	23%
	<b>SMV Other</b>	6	14%
	<b>Other</b>	1	2%
	<b>Road Surface Condition</b>		
	<b>Dry</b>	25	57%
	<b>Wet</b>	14	32%
	<b>Loose Snow</b>	2	5%
	<b>Ice</b>	3	7%
<b>Pedestrian Involved</b>		2	5%
<b>Cyclists Involved</b>		0	0%

The Brian Coburn Boulevard at Tenth Line Road intersection had a total of 44 collisions during the 2015-2019 time period, with 33 involving property damage only and the remaining 11 having non-fatal injuries. The collision types are most represented by rear end with 20 collisions, followed by turning movement with ten, SMV (other) with six, sideswipe with five, angle with two, and other with one. Rear end collisions are typically observed at congested intersections. Turning movement collisions may be influenced by the smart channel on the westbound approach. Weather conditions do may collisions at this location and the City may wish to investigate alternative paving treatments when the intersection is next rehabilitated or is converted to a roundabout. Should the intersection be converted to a roundabout, collision frequencies and types are not anticipated to be of concern.

## 2.3 Planned Conditions

### 2.3.1 Changes to the Area Transportation Network

The subject development is within the Mer Bleue CDP area, however no planning considerations contained within this document are considered relevant to the proposed development.

Within the Transportation Master Plan (TMP), the Rapid Transit and Transit Priority Network's (RTTP) Network Concept diagram shows a grade-separated BRT line along Brian Coburn Boulevard West of Mer Bleue Road, continuing north of the communities fronting Gerry Lalonde Drive and Aquaview Drive within the study area, however it is not included in the Affordable Network. It is anticipated that this project will be completed after 2031.

From the TMP's Road Network's Affordable Network diagram, Tenth Line Road is to be widened from 135 metres south of Harvest Valley Avenue southward to limit of the urban boundary as a Phase 2 (2020-2025) project.

Space has been reserved on the south side of Brian Coburn Boulevard within the study area for the widening of the roadway to four lanes. It is assumed that this improvement would take place outside of the study horizons.

### 2.3.2 Other Study Area Developments

#### *2168 Tenth Line Road*

The proposed development application includes a site plan for the construction of four buildings comprising 251 apartment units and 500m<sup>2</sup> of commercial space. The development is anticipated to be built out by 2026 and to generate 107 new AM and 132 new PM peak hour two-way vehicle trips. (Castleglenn, 2020)

#### *353 Gerry Lalonde Drive*

The proposed development application includes a zoning amendment to allow the construction of 190 new townhome dwellings and two roadway connections to Gerry Lalonde Drive. The development is anticipated to be built out by 2025 and to generate 102 new AM and 121 new PM peak hour two-way vehicle trips. (Novatech, 2021)

#### *2605 Tenth Line Road*

The proposed development application includes a Plan of Subdivision for the construction of 372 detached single dwellings and 194 townhome dwellings. Originally anticipated to be built out by 2019, the development is anticipated to generate 102 new AM and 121 new PM peak hour two-way vehicle trips. (Parsons, 2018)

#### *2275 Mer-Bleue Road*

The proposed development application includes a Plan of Subdivision for the construction of 112 townhome dwelling units and a 0.75-hectare mid-rise mixed-use development block. The development is anticipated to be built out by 2024 and to generate 237 new AM and 332 new PM peak hour two-way vehicle trips.

#### *352 Aquaview Drive*

The proposed development application included a Plan of Subdivision for the construction of 48 detached single dwellings and 274 townhome dwellings. The development was originally anticipated to be built out by 2020 and to generate 182 new AM and 238 new PM peak hour two-way auto trips. (Parsons, 2018).

## 3 Study Area and Time Periods

### 3.1 Study Area

The study area will include the intersections of:

- Tenth Line Road at:
  - Gerry Lalonde Drive/Lakepointe Drive
  - The Shops of Tenth Line Access
  - Brian Coburn Boulevard
  - Decoeur Drive/Southfield Way
  - Sweetvalley Drive/Harvest Drive
  - Site Access (Future Conditions)
- Brian Coburn Boulevard at:
  - Gerry Lalonde Drive/Jerome Jodoin Drive
  - Strasbourg Street/Des Aubepines Drive
  - Aquaview Drive/Lakeridge Drive
  - Esprit Drive
  - Site Access (Future Conditions)
- Site Access at Decoeur Drive (Future Conditions)

The boundary roads will be Tenth Line Road, Brian Coburn Boulevard, and Decoeur Drive, and no screenlines are present within proximity to the site.

### 3.2 Time Periods

As the proposed development is composed primarily of residential units the AM and PM peak hours will be examined.

### 3.3 Horizon Years

The anticipated build-out year is 2026. As a result, the full build-out plus five years horizon year is 2031.

## 4 Exemption Review

Table 6 summarizes the exemptions for this TIA.

*Table 6: Exemption Review*

Module	Element	Explanation	Exempt/Required
<b>Design Review Component</b>			
<b>4.1 Development Design</b>	4.1.2 Circulation and Access	Only required for site plans	Required
	4.2.3 New Street Networks	Only required for plans of subdivision	Exempt
<b>4.2 Parking</b>	4.2.1 Parking Supply	Only required for site plans	Required
	4.2.2 Spillover Parking	Only required for site plans where parking supply is 15% below unconstrained demand	Exempt
<b>Network Impact Component</b>			
<b>4.5 Transportation Demand Management</b>	All Elements	Not required for site plans expected to have fewer than 60 employees and/or students on location at any given time	Required
<b>4.6 Neighbourhood Traffic Management</b>	4.6.1 Adjacent Neighbourhoods	Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds	Required
<b>4.8 Network Concept</b>		Only required when proposed development generates more than 200 person-trips during the peak hour in excess of equivalent volume permitted by established zoning	Exempt

## 5 Development-Generated Travel Demand

### 5.1 Trip Generation and Mode Shares

Examining the mode shares recommended in the TRANS Trip Generation Manual (2020) for the subject district, derived from the most recent National Capital Region Origin-Destination survey (OD Survey), the existing average district mode shares by land use for Orleans have been summarized in Table 7.

Table 7: TRANS Trip Generation Manual Recommended Mode Shares – Orleans

Travel Mode	Multi-Unit (Low-Rise)		Commercial Generator	
	AM	PM	AM	PM
Auto Driver	47%	51%	77%	71%
Auto Passenger	15%	19%	14%	20%
Transit	29%	24%	3%	2%
Cycling	1%	1%	0%	1%
Walking	9%	6%	6%	5%
<b>Total</b>	<b>100%</b>	<b>100%</b>	<b>100%</b>	<b>100%</b>

## 5.2 Trip Generation

This TIA has been prepared using the vehicle and person trip rates for the residential dwellings using the TRANS Trip Generation Manual (2020) and the vehicle trip rates and derived person trip rates for commercial component from the ITE Trip Generation Manual 10th Edition (2017) using the City-prescribed conversion factor of 1.28. Table 8 summarizes the person trip rates for the proposed residential land use for each peak period and the person trip rates for the non-residential land use for each peak hour.

Table 8: Trip Generation Person Trip Rates by Peak Period

Land Use	Land Use Code	Peak Period	Vehicle Trip Rate	Person Trip Rate
Multi-Unit (Low-Rise)	220 (TRANS)	AM	-	1.35
		PM	-	1.58
Land Use	Land Use Code	Peak Hour	Vehicle Trip Rate	Person Trip Rate
Shopping Centre	820 (ITE)	AM	0.94	1.20
		PM	3.81	4.88

Using the above person trip rates, the total person trip generation has been estimated. Table 9 summarizes the total person trip generation for the residential land use and for the non-residential land use.

Table 9: Total Residential Person Trip Generation by Peak Period

Land Use	Units	AM Peak Period			PM Peak Period		
		In	Out	Total	In	Out	Total
Multi-Unit (Low-Rise)	240	97	227	324	212	167	379
Land Use	Units / GFA	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
Shopping Centre	34,117	25	16	41	80	86	166

Internal capture rates from the ITE Trip Generation Handbook 3<sup>rd</sup> Edition have been assigned to the development’s retail component for mixed-use developments. The rates summarized in Table 10 represent the percentage of trips to/from the retail use based on the residential component.

Table 10: Internal Capture Rates

Land Use	AM		PM	
	In	Out	In	Out
Residential to/from Shopping Centre	17%	14%	10%	26%

Pass-by reductions applied to the retail trip generation at a rate of 35% have been included, a value taken as a moderately conservative interpretation from the rates presented in the ITE Trip Generation Handbook 3<sup>rd</sup> Edition.

Using the above mode share targets, the internal capture and pass-by rates, and the person trip rates, the person trips by mode have been projected.

Table 11 summarizes the trip generation by mode and peak hour using the residential peak hour adjustment factor and the non-residential trip generation using the internal capture and pass-by reductions.

Table 11: Trip Generation by Mode

Travel Mode		AM Peak Hour				PM Peak Hour			
		Mode Share	In	Out	Total	Mode Share	In	Out	Total
Multi-Unit (Low-Rise)	Auto Driver	47%	22	51	73	51%	48	37	85
	Auto Passenger	15%	7	16	24	19%	18	14	32
	Transit	29%	15	36	52	24%	24	19	43
	Cycling	1%	1	1	2	1%	1	1	2
	Walking	9%	5	12	17	6%	7	5	12
	<b>Total</b>	<b>100%</b>	<b>49</b>	<b>114</b>	<b>162</b>	<b>100%</b>	<b>93</b>	<b>73</b>	<b>167</b>
Shopping Centre	Auto Driver	77%	10	7	18	71%	33	29	62
	Auto Passenger	14%	2	1	3	20%	9	8	18
	Transit	3%	0	0	1	2%	1	1	2
	Cycling	0%	0	0	0	1%	0	0	1
	Walking	6%	1	1	1	5%	2	2	4
	Internal Capture	varies	-9	-6	-14	varies	-28	-30	-58
	Pass-by	35%	-3	-1	-4	35%	-5	-15	-20
<b>Total</b>	<b>100%</b>	<b>13</b>	<b>9</b>	<b>23</b>	<b>100%</b>	<b>47</b>	<b>41</b>	<b>88</b>	
Total	Auto Driver	-	32	58	91	-	81	66	147
	Auto Passenger	-	9	17	27	-	27	22	50
	Transit	-	15	36	53	-	25	20	45
	Cycling	-	1	1	2	-	1	1	3
	Walking	-	6	13	18	-	9	7	16
	<b>Total</b>	<b>-</b>	<b>62</b>	<b>123</b>	<b>185</b>	<b>-</b>	<b>140</b>	<b>114</b>	<b>255</b>

As shown above, a total of 91 AM new and 147 PM new peak hour two-way vehicle trips are projected as a result of the proposed development.

### 5.3 Trip Distribution

To understand the travel patterns of the subject development, the OD Survey has been reviewed to determine the travel for the existing district travel and these patterns were applied based on the build-out of Orleans. Table 12 below summarizes the distributions.

Table 12: OD Survey Distribution – Orleans

To/From	% of Trips	Via
North	30%	20% Tenth Line Rd (N), 10% Brian Coburn Blvd (W)
South	5%	5% Tenth Line Rd (S)
East	5%	5% Tenth Line Rd (N)
West	60%	50% Brian Coburn boulevard (W), 10% Tenth Line Rd (N)
<b>Total</b>	<b>100%</b>	<b>100%</b>

### 5.4 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the primary and pass-by trips generated by the site have been assigned to the study area road network. Table 12 above summarizes the proportional assignment to the study area roadways, Figure 12 illustrates the new site generated primary auto trip volumes, Figure 13 illustrates the site pass-by trips.



Figure 12: New Site Generated Auto Volumes

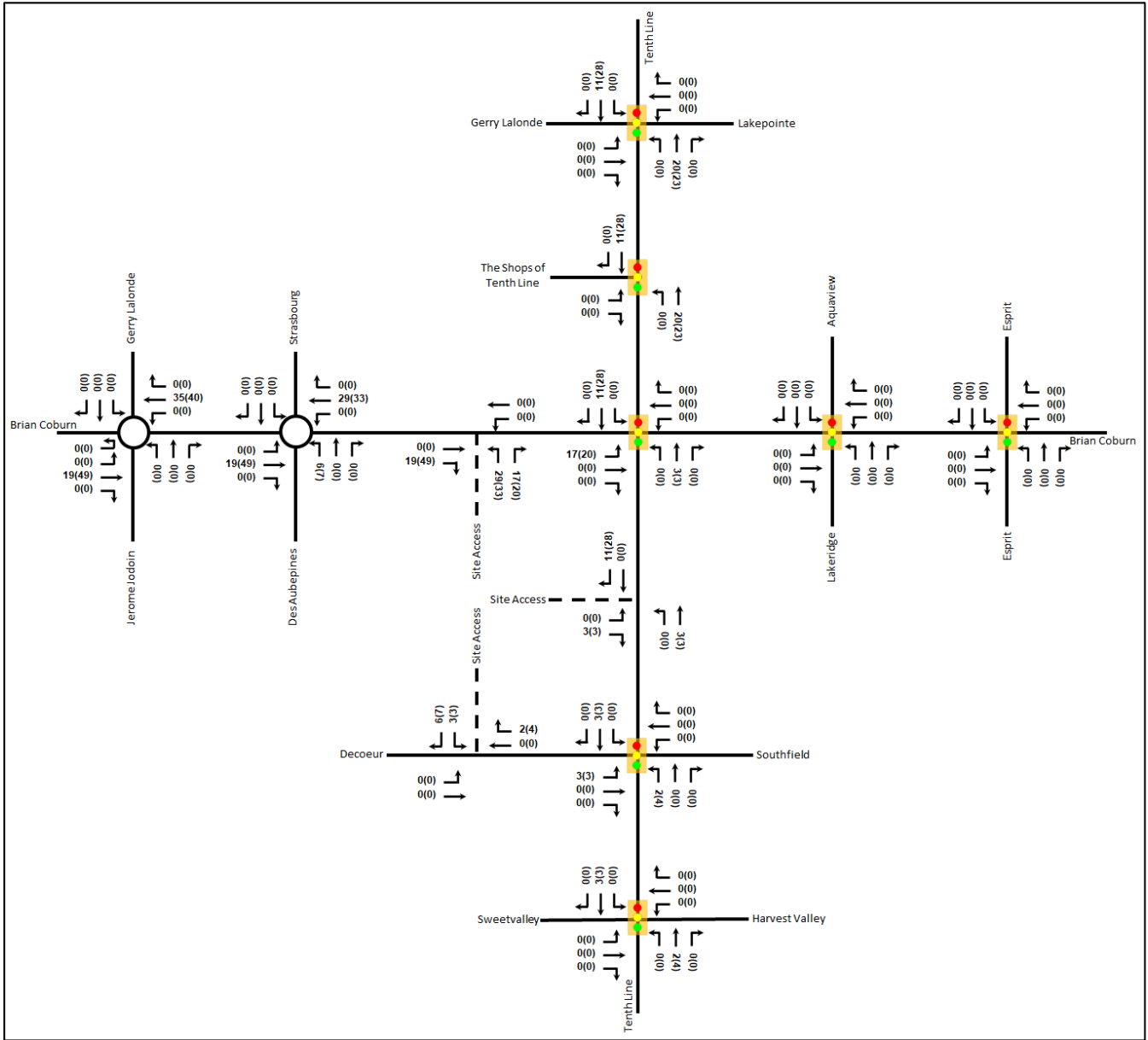
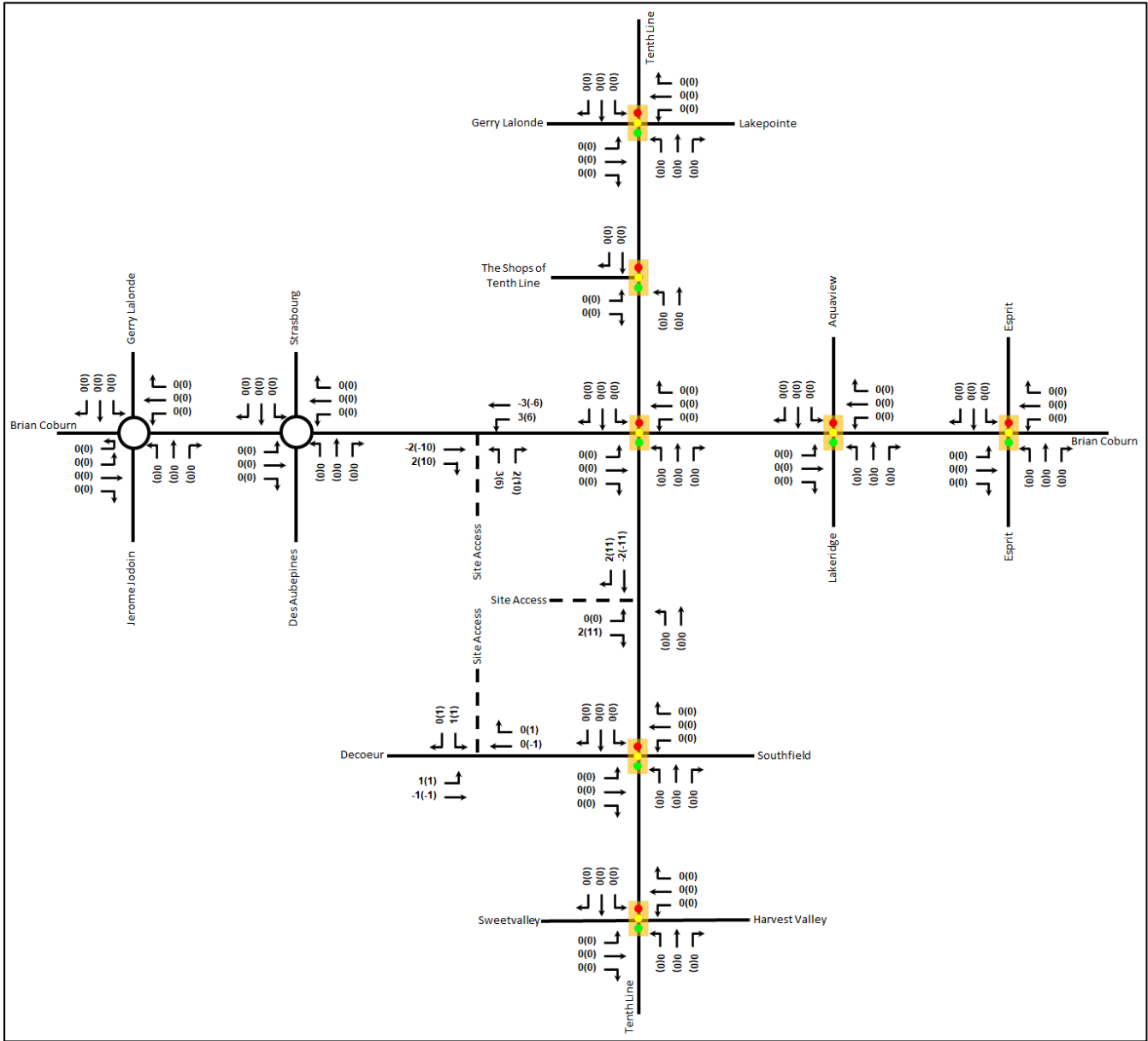


Figure 13: Site Pass-By Auto Volumes



## 6 Background Network Travel Demands

### 6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3. The Tenth Line Road widening south of Harvest Valley Avenue is the only confirmed project within the study horizons and these conditions have been incorporated into the analysis.

### 6.2 Background Growth

A review of the background projections from the City's TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways.

In general, the growth rates in the study area derived from the two TRANS model horizons are projected to be positive in all directions. When reviewing the existing volumes, however, it is noted that the majority of growth predicted within the study area has been achieved. Only volumes in the westbound direction in the AM peak hour on Brian Coburn Boulevard have not reached the anticipated 2031 values, and resultantly, growth rates derived from the existing horizon to the 2031 model horizon rounded to the nearest 0.25% will be peak-directionally applied to the roadway's mainline volumes. To account for development south of the study area, an annual rate of 0.50% will be applied in both directions along Tenth Line Road. Table 13 summarizes the growth rates applied within the study area and the TRANS model plots are provided in Appendix F.

*Table 13: TRANS Regional Model Projections – Study Area Growth Rates*

Street	AM Peak Hour		PM Peak Hour	
	Eastbound	Westbound	Eastbound	Westbound
<b>Brian Coburn Boulevard</b>	-	1.25%	1.25%	-
	Northbound	Southbound	Northbound	Southbound
<b>Tenth Line Road</b>	0.50%	0.50%	0.50%	0.50%

### 6.3 Other Developments

The background developments explicitly considered in the background conditions (Section 6.2) include:

- 2168 Tenth Line Road
- 353 Gerry Lalonde Drive
- 2605 Tenth Line Road
- 2275 Mer-Bleue Road
- 352 Aquaview Drive

The background development volumes within the study area have been provided in Appendix G.

## 7 Demand Rationalization

### 7.1 2026 Future Background Operations

Figure 14 illustrates the 2026 background volumes and Table 14 summarizes the 2026 background intersection operations. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM 2010 average delay for unsignalized intersections. Synchro 11 has been used to model the signalized intersections and Sidra 8 to model the study area roundabouts. The Synchro and Sidra worksheets for the 2026 future background horizon are provided in Appendix H.

Figure 14: 2026 Future Background Volumes

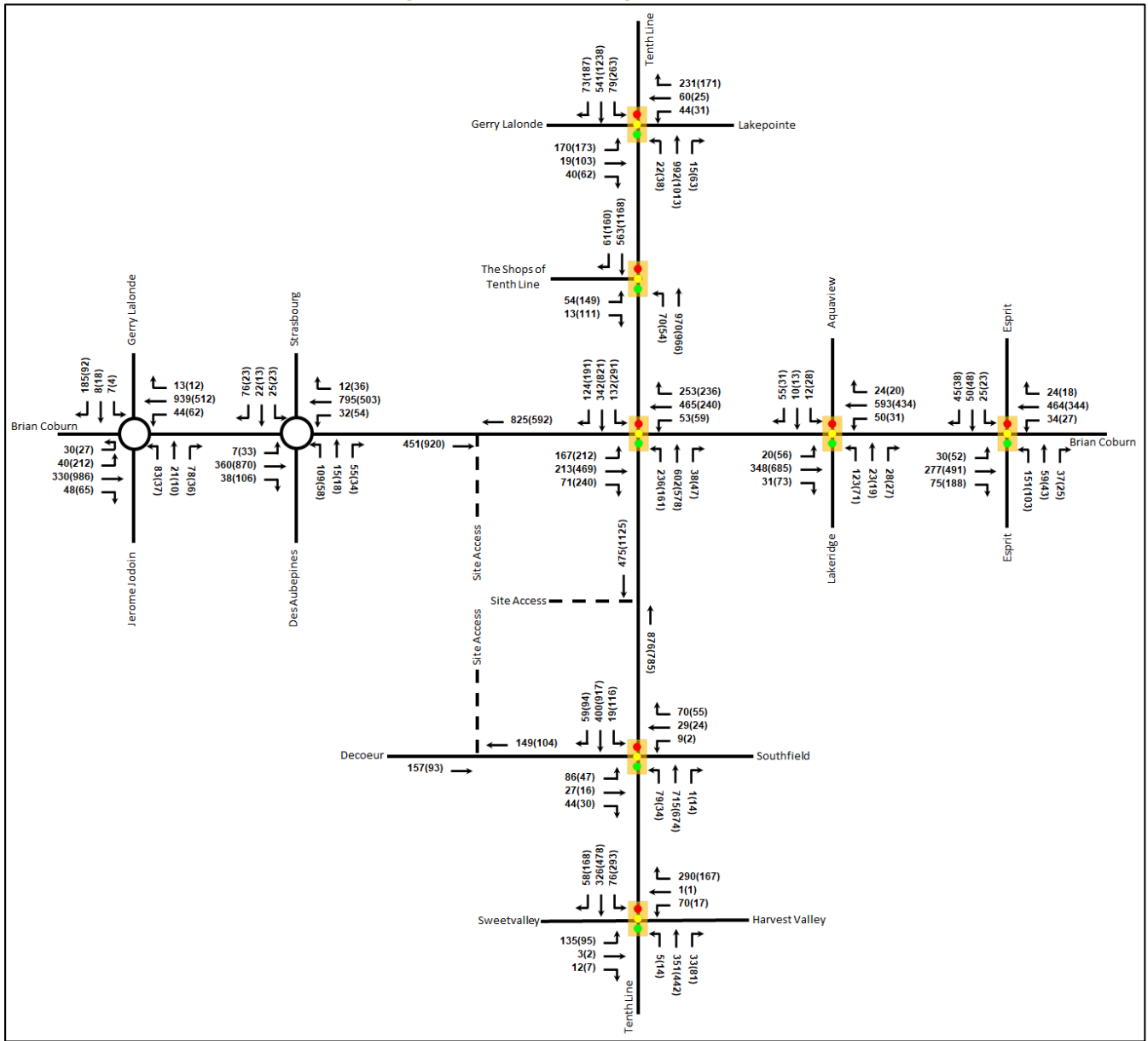


Table 14: 2026 Future Background Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road</b> <i>Signalized</i>	EBL	C	0.71	48.1	43.2	C	0.71	53.6	49.0
	EBT/R	A	0.18	13.4	10.9	A	0.50	33.5	38.8
	WBL	A	0.17	28.3	13.4	A	0.16	33.0	11.8
	WBT	A	0.17	27.8	16.5	A	0.08	30.8	9.9
	WBR	B	0.61	24.0	37.9	A	0.47	16.7	25.5
	NBL	A	0.05	4.6	m2.1	A	0.17	5.2	2.6
	NBT	A	0.46	5.3	46.4	A	0.45	4.9	18.2
	NBR	A	0.02	0.1	m0.1	A	0.06	0.6	0.7
	SBL	A	0.28	12.0	17.1	D	0.88	46.9	#100.9
	SBT	A	0.26	7.9	34.2	A	0.55	10.1	94.7
	SBR	A	0.08	2.5	5.5	A	0.18	1.7	8.0
<b>Overall</b>	<b>A</b>	<b>0.52</b>	<b>12.4</b>	-	<b>D</b>	<b>0.84</b>	<b>14.8</b>	-	
<b>The Shops of Tenth Line Access at Tenth Line Road</b> <i>Signalized</i>	EBL	A	0.29	40.6	19.3	A	0.54	43.4	37.7
	EBR	A	0.07	18.5	5.2	A	0.37	18.7	18.9
	NBL	A	0.12	2.8	m4.0	A	0.21	8.1	m11.6
	NBT	A	0.38	2.8	m21.3	A	0.42	5.9	66.3
	SBT	A	0.22	2.3	11.7	A	0.50	5.8	37.2
	SBR	A	0.05	0.5	0.1	A	0.15	0.7	2.5
	<b>Overall</b>	<b>A</b>	<b>0.39</b>	<b>3.9</b>	-	<b>A</b>	<b>0.51</b>	<b>8.3</b>	-
<b>Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive</b> <i>Roundabout</i>	EB	A	0.31	4.9	15.9	A	0.89	6.2	145.4
	WB	A	0.78	6.5	74.9	A	0.55	6.3	32.7
	NB	A	0.19	7.0	7.4	B	0.30	18.8	16.3
	SB	B	0.50	18.6	31.7	A	0.15	6.4	6.8
	<b>Overall</b>	<b>A</b>	<b>0.78</b>	<b>7.5</b>	-	<b>A</b>	<b>0.89</b>	<b>6.7</b>	-
<b>Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive</b> <i>Roundabout</i>	EB	A	0.30	4.1	14.2	A	0.70	5.2	8.2
	WB	A	0.63	4.9	46.1	A	0.45	4.7	25.5
	NB	A	0.18	7.6	7.2	B	0.20	12.1	9.4
	SB	B	0.22	10.7	10.8	A	0.07	7.7	2.9
	<b>Overall</b>	<b>A</b>	<b>0.63</b>	<b>5.5</b>	-	<b>A</b>	<b>0.71</b>	<b>5.2</b>	-
<b>Brian Coburn Boulevard at Tenth Line Road</b> <i>Signalized</i>	EBL	<b>F</b>	<b>1.06</b>	<b>118.5</b>	<b>#65.3</b>	A	0.53	28.6	53.5
	EBT/R	A	0.52	24.3	54.0	<b>F</b>	<b>1.03</b>	<b>72.4</b>	<b>#213.2</b>
	WBL	A	0.18	20.3	13.8	D	0.84	<b>104.0</b>	<b>#35.2</b>
	WBT	C	0.79	36.1	98.9	A	0.34	22.2	50.1
	WBR	A	0.40	7.1	21.2	A	0.33	5.3	17.4
	NBL	A	0.55	18.2	19.8	<b>F</b>	<b>1.13</b>	<b>145.6</b>	<b>#71.0</b>
	NBT/R	A	0.38	11.6	22.4	A	0.41	18.0	53.3
	SBL	A	0.41	27.7	42.2	E	0.98	65.4	<b>#99.1</b>
	SBT/R	A	0.29	16.7	46.2	B	0.67	11.9	43.7
<b>Overall</b>	<b>C</b>	<b>0.75</b>	<b>25.8</b>	-	<b>F</b>	<b>1.08</b>	<b>38.3</b>	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive <i>Signalized</i></b>	EBL	A	0.05	6.4	3.7	A	0.09	5.0	6.8
	EBT/R	A	0.34	7.3	40.2	A	0.59	8.9	97.9
	WBL	A	0.08	6.4	7.0	A	0.08	5.2	4.6
	WBT/R	A	0.53	9.7	78.0	A	0.36	5.9	44.2
	NBL	A	0.50	29.8	26.0	A	0.38	33.9	19.6
	NBT/R	A	0.16	13.0	9.4	A	0.18	16.4	10.2
	SBL	A	0.05	20.0	4.7	A	0.16	28.7	9.8
	SBT/R	A	0.19	9.4	9.0	A	0.17	14.8	9.2
	<b>Overall</b>	<b>A</b>	<b>0.57</b>	<b>10.9</b>	-	<b>A</b>	<b>0.60</b>	<b>9.7</b>	-
<b>Brian Coburn Boulevard at Esprit Drive <i>Signalized</i></b>	EBL	A	0.08	10.3	6.2	A	0.11	10.5	9.3
	EBT/R	A	0.41	12.3	46.3	C	0.76	21.0	116.6
	WBL	A	0.08	10.1	6.7	A	0.12	11.4	6.3
	WBT/R	A	0.55	15.4	72.2	A	0.40	12.9	49.1
	NBL	A	0.38	24.3	33.2	A	0.26	22.1	23.4
	NBT/R	A	0.19	13.8	16.5	A	0.13	14.1	12.9
	SBL	A	0.07	19.3	7.8	A	0.06	19.0	7.3
	SBT/R	A	0.18	12.3	15.3	A	0.17	12.9	14.5
	<b>Overall</b>	<b>A</b>	<b>0.48</b>	<b>15.1</b>	-	<b>A</b>	<b>0.56</b>	<b>17.6</b>	-
<b>Decoeur Drive / Southfield Way at Tenth Line Road <i>Signalized</i></b>	EBL	A	0.41	36.0	20.2	A	0.26	37.7	14.4
	EBT/R	A	0.25	14.9	11.3	A	0.18	17.2	9.6
	WBL	A	0.04	25.1	3.9	A	0.01	29.0	1.8
	WBT/R	A	0.30	12.6	12.9	A	0.28	15.8	12.8
	NBL	A	0.14	8.7	16.5	A	0.09	7.7	8.5
	NBT	A	0.31	7.6	56.7	A	0.26	6.1	50.8
	NBR	A	0.00	0.0	0.0	A	0.01	0.0	0.3
	SBL	A	0.04	6.1	m3.3	A	0.22	4.6	m9.3
	SBT	A	0.17	5.1	19.0	A	0.36	3.7	m32.8
	SBR	A	0.06	1.9	2.6	A	0.08	0.5	m0.0
<b>Overall</b>	<b>A</b>	<b>0.35</b>	<b>9.1</b>	-	<b>A</b>	<b>0.36</b>	<b>6.1</b>	-	
<b>Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road <i>Signalized</i></b>	EBL	C	0.71	44.1	32.7	A	0.55	44.6	28.5
	EBT/R	A	0.04	11.2	4.0	A	0.03	18.1	4.0
	WBL	A	0.23	21.6	15.9	A	0.08	29.4	7.5
	WBT/R	A	0.51	6.2	15.2	A	0.44	8.7	14.7
	NBL	A	0.01	9.0	2.1	A	0.03	6.7	3.6
	NBT/R	A	0.22	8.2	24.3	A	0.23	5.8	30.7
	SBL	A	0.15	9.8	13.9	A	0.53	13.0	63.7
	SBT/R	A	0.21	7.8	23.2	A	0.29	5.7	36.6
	<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>12.1</b>	-	<b>A</b>	<b>0.53</b>	<b>9.6</b>	-

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

During both AM and PM peak hours, the study area intersections at the 2026 future background horizon operate similarly to the existing conditions.

At the intersection of Brian Coburn Boulevard and Tenth Line Road during the PM peak hour, the eastbound through/right-turn movement is forecasted to be over theoretical capacity, and the westbound left movement may be subject to extended delays at this horizon.

As much development has recently occurred within the study area, and given the background growth and background development volumes anticipated at this intersection, it is proposed that changes to signal phasing

be considered at this intersection. Table 15 summarizes the study area intersection operations with a protected eastbound left-turn phase introduced at the intersection of Brian Coburn Boulevard and Tenth Line Road in the AM peak hour, and the introduction of protected eastbound, northbound, and southbound left-turn phases at this intersection during the PM peak hour. Additionally, during the PM peak hour, the cycle length has been increased at this intersection, although it is noted that coordination with other signals on the Tenth Line Corridor would be required to support this change. The synchro worksheets for the 2026 future background horizon with the phasing/cycle changes are provided in Appendix I.

Table 15: 2026 Future Background Intersection Operations with Phase/Cycle Changes

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Brian Coburn Boulevard at Tenth Line Road</b> <i>Signalized</i>	EBL	D	0.81	47.8	#40.6	A	0.55	26.7	46.1
	EBT/R	A	0.41	17.5	47.3	E	0.98	59.5	#220.7
	WBL	A	0.18	23.5	15.1	C	0.74	74.4	#34.1
	WBT	D	0.89	49.6	#121.7	A	0.48	26.9	52.7
	WBR	A	0.46	12.1	31.0	A	0.41	4.9	8.0
	NBL	B	0.69	40.2	#73.6	D	0.90	80.3	#64.3
	NBT/R	A	0.46	24.4	69.7	C	0.76	42.8	54.8
	SBL	A	0.54	23.7	29.1	D	0.83	53.7	#81.5
	SBT/R	A	0.34	9.5	21.1	E	0.95	45.5	#146.9
<b>Overall</b>	<b>C</b>	<b>0.78</b>	<b>27.1</b>	-	<b>F</b>	<b>1.06</b>	<b>45.5</b>	-	

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

At the intersection of Brian Coburn Boulevard at Tenth Line Road, with the addition of the protected eastbound left-turn phase during the AM peak hour, the eastbound left movement is forecasted to no longer be over theoretical capacity with extended queues on the eastbound left movement, however the westbound through and northbound left movements may exhibit extended queues. During the PM peak hour, with the addition of multiple protected left-turn phases and cycle length increase, the eastbound through/right and northbound left movements are anticipated to no longer be over theoretical capacity, however the southbound through/right movement may exhibit extended queues.

### 7.2 2031 Future Background Operations

Figure 15 illustrates the 2031 background volumes and Table 16 summarizes the 2031 background intersection operations, including the phasing and cycle length changes applied to the intersection of Brian Coburn Boulevard at Tenth Line Road at the 2026 background horizon. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM 2010 average delay for unsignalized intersections. Synchro 11 has been used to model the signalized intersections and Sidra 8 to model the study area roundabouts. The Synchro and Sidra worksheets for the 2031 future background horizon are provided in Appendix J.

Figure 15: 2031 Future Background Volumes

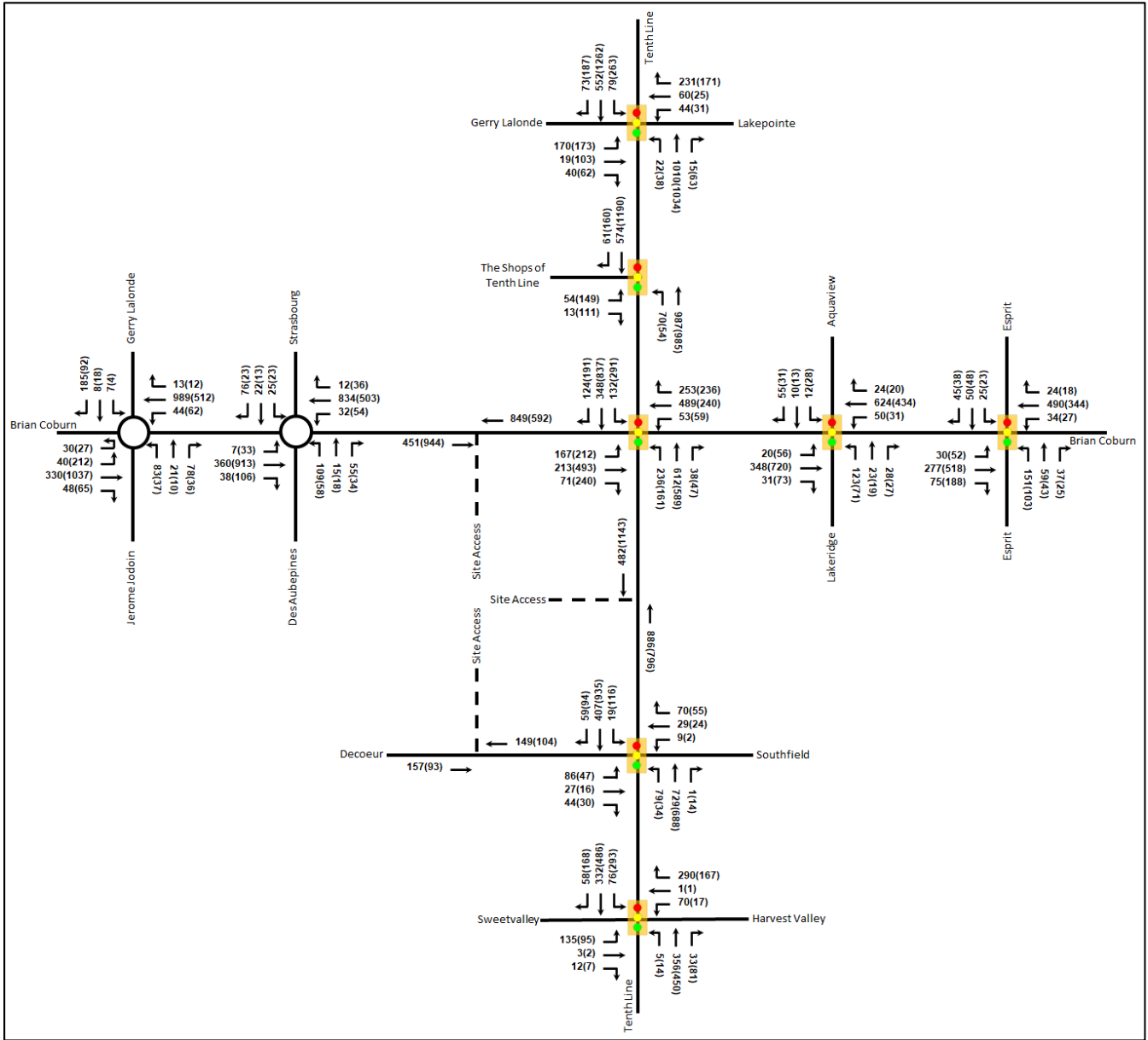




Table 16: 2031 Future Background Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road Signalized</b>	EBL	C	0.71	48.1	43.2	C	0.71	53.6	49.0
	EBT/R	A	0.18	13.4	10.9	A	0.50	33.5	38.8
	WBL	A	0.17	28.3	13.4	A	0.16	33.0	11.8
	WBT	A	0.17	27.8	16.5	A	0.08	30.8	9.9
	WBR	B	0.61	24.0	37.9	A	0.47	17.7	26.5
	NBL	A	0.05	8.9	m3.4	A	0.18	5.6	2.8
	NBT	A	0.46	12.7	90.6	A	0.46	5.6	19.8
	NBR	A	0.02	1.2	m0.4	A	0.06	0.6	0.7
	SBL	A	0.28	12.0	17.1	D	0.90	52.1	#102.5
	SBT	A	0.26	7.9	34.2	A	0.56	10.2	97.6
	SBR	A	0.08	2.5	5.5	A	0.18	1.7	8.0
<b>Overall</b>	<b>A</b>	<b>0.52</b>	<b>15.6</b>	-	<b>D</b>	<b>0.86</b>	<b>15.4</b>	-	
<b>The Shops of Tenth Line Access at Tenth Line Road Signalized</b>	EBL	A	0.29	40.6	19.3	A	0.54	43.4	37.7
	EBR	A	0.07	18.5	5.2	A	0.37	19.6	19.3
	NBL	A	0.12	6.0	m9.1	A	0.22	10.5	13.5
	NBT	A	0.38	5.6	43.2	A	0.42	8.2	76.0
	SBT	A	0.22	2.3	11.7	A	0.51	5.8	37.7
	SBR	A	0.05	0.5	0.1	A	0.15	0.7	2.5
	<b>Overall</b>	<b>A</b>	<b>0.39</b>	<b>5.6</b>	-	<b>A</b>	<b>0.52</b>	<b>9.2</b>	-
<b>Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive Roundabout</b>	EB	A	0.31	4.8	16.1	A	0.92	7.1	180.7
	WB	A	0.82	7.1	89.6	A	0.55	6.3	33.7
	NB	A	0.19	6.9	7.4	C	0.37	24.5	21.1
	SB	C	0.56	23.6	38.0	A	0.15	6.4	6.8
	<b>Overall</b>	<b>A</b>	<b>0.82</b>	<b>8.3</b>	-	<b>A</b>	<b>0.92</b>	<b>7.1</b>	-
<b>Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive Roundabout</b>	EB	A	0.30	4.1	14.3	A	0.73	4.7	65.0
	WB	A	0.66	5.0	50.5	A	0.45	4.7	25.7
	NB	A	0.18	7.6	7.2	B	0.21	12.8	10.3
	SB	B	0.23	11.4	11.7	A	0.07	7.7	2.9
	<b>Overall</b>	<b>A</b>	<b>0.66</b>	<b>5.5</b>	-	<b>A</b>	<b>0.73</b>	<b>5.3</b>	-
<b>Brian Coburn Boulevard at Tenth Line Road Signalized</b>	EBL	C	0.73	36.5	#36.5	A	0.54	26.5	45.3
	EBT/R	A	0.40	17.0	46.8	E	0.98	59.4	#228.6
	WBL	A	0.18	24.3	15.4	C	0.76	87.6	#35.2
	WBT	D	0.90	52.7	#125.5	A	0.42	31.7	62.2
	WBR	A	0.46	12.3	31.2	A	0.38	5.4	16.6
	NBL	C	0.71	42.7	#74.7	E	0.93	79.8	#61.5
	NBT/R	A	0.47	25.5	69.8	C	0.80	47.8	#90.3
	SBL	A	0.56	25.4	#30.2	D	0.87	49.2	#86.0
	SBT/R	A	0.35	10.0	22.1	E	1.00	64.2	#158.9
<b>Overall</b>	<b>C</b>	<b>0.79</b>	<b>27.5</b>	-	<b>F</b>	<b>1.09</b>	<b>51.9</b>	-	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive Signalized</b>	EBL	A	0.05	6.4	3.7	A	0.09	5.0	6.8
	EBT/R	A	0.34	7.3	40.2	B	0.62	9.5	107.6
	WBL	A	0.08	6.4	7.0	A	0.08	5.3	4.6
	WBT/R	A	0.53	9.7	78.0	A	0.36	5.9	44.2
	NBL	A	0.50	29.8	26.0	A	0.38	33.9	19.6
	NBT/R	A	0.16	13.0	9.4	A	0.18	16.4	10.2
	SBL	A	0.05	20.0	4.7	A	0.16	28.7	9.8
	SBT/R	A	0.19	9.4	9.0	A	0.17	14.8	9.2
	<b>Overall</b>	<b>A</b>	<b>0.57</b>	<b>10.9</b>	-	<b>B</b>	<b>0.62</b>	<b>10.0</b>	-
<b>Brian Coburn Boulevard at Esprit Drive Signalized</b>	EBL	A	0.08	10.3	6.2	A	0.11	10.5	9.3
	EBT/R	A	0.41	12.3	46.3	C	0.79	22.7	#126.8
	WBL	A	0.08	10.1	6.7	A	0.13	11.8	6.4
	WBT/R	A	0.55	15.4	72.2	A	0.40	12.9	49.1
	NBL	A	0.38	24.3	33.2	A	0.26	22.1	23.4
	NBT/R	A	0.19	13.8	16.5	A	0.13	14.1	12.9
	SBL	A	0.07	19.3	7.8	A	0.06	19.0	7.3
	SBT/R	A	0.18	12.3	15.3	A	0.17	12.9	14.5
	<b>Overall</b>	<b>A</b>	<b>0.48</b>	<b>15.1</b>	-	<b>A</b>	<b>0.58</b>	<b>18.5</b>	-
<b>Decoeur Drive / Southfield Way at Tenth Line Road Signalized</b>	EBL	A	0.41	36.0	20.2	A	0.26	37.7	14.4
	EBT/R	A	0.25	14.9	11.3	A	0.18	17.2	9.6
	WBL	A	0.04	25.1	3.9	A	0.01	29.0	1.8
	WBT/R	A	0.30	12.6	12.9	A	0.28	15.8	12.8
	NBL	A	0.14	8.7	16.5	A	0.09	7.8	8.6
	NBT	A	0.31	7.6	56.7	A	0.27	6.2	52.0
	NBR	A	0.00	0.0	0.0	A	0.01	0.0	0.3
	SBL	A	0.04	6.8	m6.7	A	0.22	8.3	24.5
	SBT	A	0.17	5.0	38.9	A	0.37	6.9	76.0
	SBR	A	0.06	2.3	10.6	A	0.08	2.2	7.0
<b>Overall</b>	<b>A</b>	<b>0.35</b>	<b>9.1</b>	-	<b>A</b>	<b>0.37</b>	<b>7.8</b>	-	
<b>Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road Signalized</b>	EBL	C	0.71	44.1	32.7	A	0.55	44.6	28.5
	EBT/R	A	0.04	11.2	4.0	A	0.03	18.1	4.0
	WBL	A	0.23	21.6	15.9	A	0.08	29.4	7.5
	WBT/R	A	0.51	6.2	15.2	A	0.44	8.7	14.7
	NBL	A	0.01	9.0	2.1	A	0.03	6.7	3.6
	NBT/R	A	0.22	8.2	24.3	A	0.24	5.8	31.3
	SBL	A	0.15	9.8	13.9	A	0.53	13.1	64.1
	SBT/R	A	0.21	7.8	23.2	A	0.30	5.8	37.3
	<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>12.1</b>	-	<b>A</b>	<b>0.54</b>	<b>9.6</b>	-

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = queue exceeds storage or mid-block length

During both AM and PM peak hours, the study area intersections at the 2031 future background horizon operate similarly to the 2026 background conditions, each with phase and cycle changes. At the intersection of Brian Coburn Boulevard at Tenth Line Road, the southbound left movement during the AM peak hour and the northbound through/right movement during the PM peak hour may exhibit extended queues, and the westbound left movement may be subject to high delays during the PM peak hour at this horizon. The eastbound through/right movement at the intersection of Brian Coburn Boulevard at Esprit Drive may also exhibit extended queueing during the PM peak hour at this horizon.

### 7.3 Modal Share Sensitivity and Demand Rationalization Conclusions

Although some movements are approaching capacity at the intersection of Brian Coburn Boulevard at Tenth Line Road during the PM peak hour at the future background horizons, residual capacity is generally available throughout the study area. Given the site has three accesses onto different study area roadways and thus impacts will be distributed, no rationalization for adjusted demand is considered to be necessary for this TIA. Modal share shifts towards transit are considered to be likely beyond the study area horizons with the buildout of the grade-separated BRT corridor to the north of the site.

## 8 Development Design

### 8.1 Design for Sustainable Modes

The proposed development is a mixed-use site with auto parking for the residential dwellings and commercial units located in surface lots surrounding the site buildings. Bicycle racks for all site buildings are located adjacent to each.

Walkways circulate the site, connecting each building to the others, the on-site amenities, and the surrounding pedestrian facilities on Tenth Line Road, and Decoeur Drive.

### 8.2 Circulation and Access

Access for vehicles and bicycles is provided via the three site driveways which connect to 6.5-metre drive aisles accessing the surface parking lots.

Emergency services may access each of the site buildings via the internal drive aisles and residential and commercial garbage collection is anticipated to take place on-site.

## 9 Parking

### 9.1 Parking Supply

The site proposes 134 bicycle parking spaces, and a total of 439 vehicle parking spaces. Vehicle parking spaces are allocated as 279 spaces for residents, 48 spaces for visitors, and 123 spaces for commercial, with 11 spaces shared between the visitor and commercial parking uses.

As presented in Figure 2, the zoning by-law prescribes a minimum bicycle parking provision of 133 spaces and a minimum vehicle parking provision of 439 spaces for the site.

The minimum parking provision from the zoning by-law is being met by the proposed parking on-site.

## 10 Boundary Street Design

Table 17 summarizes the MMLOS analysis for the boundary streets of Brian Coburn Boulevard, Tenth Line Road, and Decoeur Drive. The existing and future conditions for each street will be the same and are considered in one row. The boundary street analysis is based on the land use designation of “Developing Community” for Brian Coburn Boulevard and Tenth Line Road and on the policy area of “Within 300m of a School” for Decoeur Drive. The MMLOS worksheets has been provided in Appendix K.

Table 17: Boundary Street MMLOS Analysis

Segment	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Brian Coburn Blvd	F	C	F	B	D	D	B	D
Tenth Line Rd	D	C	C	C	-	-	A	D

Segment	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target
Decoeur Dr	B	A	D	D	D	D	-	-

The boundary streets do not meet pedestrian LOS targets, and Brian Coburn Boulevard does not meet bicycle LOS targets.

Pedestrian LOS targets cannot be met on Brian Coburn Boulevard and on Tenth Line Road due to vehicle volumes and operating speeds. It is anticipated that pedestrian LOS will improve, however will still not meet targets, with the provision of a sidewalk on the south side of the Brian Coburn Boulevard as part of an eventual road widening. To meet pedestrian LOS targets on Decoeur Drive the boulevard would need to be marginally increased. As the MMLOS scorings group discrete values, no improvement from “two metres” to “greater than two metres” is considered appropriate for Decoeur Drive, and the existing condition is considered adequate.

To meet bicycle LOS, Brian Coburn Boulevard would require physically separated facilities. The City should investigate the provision of such a facility as part of the design of a future widening of Brian Coburn Boulevard.

## 11 Access Intersections Design

### 11.1 Location and Design of Access

The site will connect to the adjacent arterial road network via a full-movement access on Brian Coburn Boulevard and a right-in/right-out access on Tenth Line Road. The site will also connect to the adjacent collector road Decoeur Drive via an access that will be shared with the adjacent parcel of 885 Decoeur Drive via a joint use and maintenance agreement.

The throat length for the accesses on Brian Coburn Boulevard in the existing condition and Decoeur Drive meet the suggested minimums of 40 metres and 25 metres, respectively, from Table 8.9.3 of the TAC Geometric Design Guidelines. The throat length of the right-in/right-out access on Tenth Line Road is proposed to be approximately 22.0 metres, does not meet the suggested minimum 40 metres. Additionally, the site access on Brian Coburn Boulevard will not meet the suggested minimum 40 metres once the roadway is widened.

As the site has three accesses, and as there is additional nine metres for queuing between the intersecting roadway and the ends of the driveway curb return radii, the future throat length condition with a widened Brian Coburn Boulevard is considered to be adequate. Further to the foregoing rationale for the site access on Brian Coburn Boulevard, as the Tenth Line access is right-in/right-out and the site is proposed as a mixed-use development, the throat length of approximately 22.0 metres is considered sufficient to ensure proper access operations and to prevent any queuing-related concerns.

### 11.2 Intersection Control

All three site accesses are proposed as being minor stop-controlled with Brian Coburn Boulevard, Tenth Line Road, and Decoeur Drive operating as free flow-corridors.

### 11.3 Access Intersection Design

#### 11.3.1 2026 Future Total Access Intersection Operations

The 2026 future total intersection volumes are illustrated in Figure 16 and the access intersection operations are summarized below in Table 18. The level of service is based on v HCM 2010 average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix L.

Figure 16: 2026 Future Total Volumes

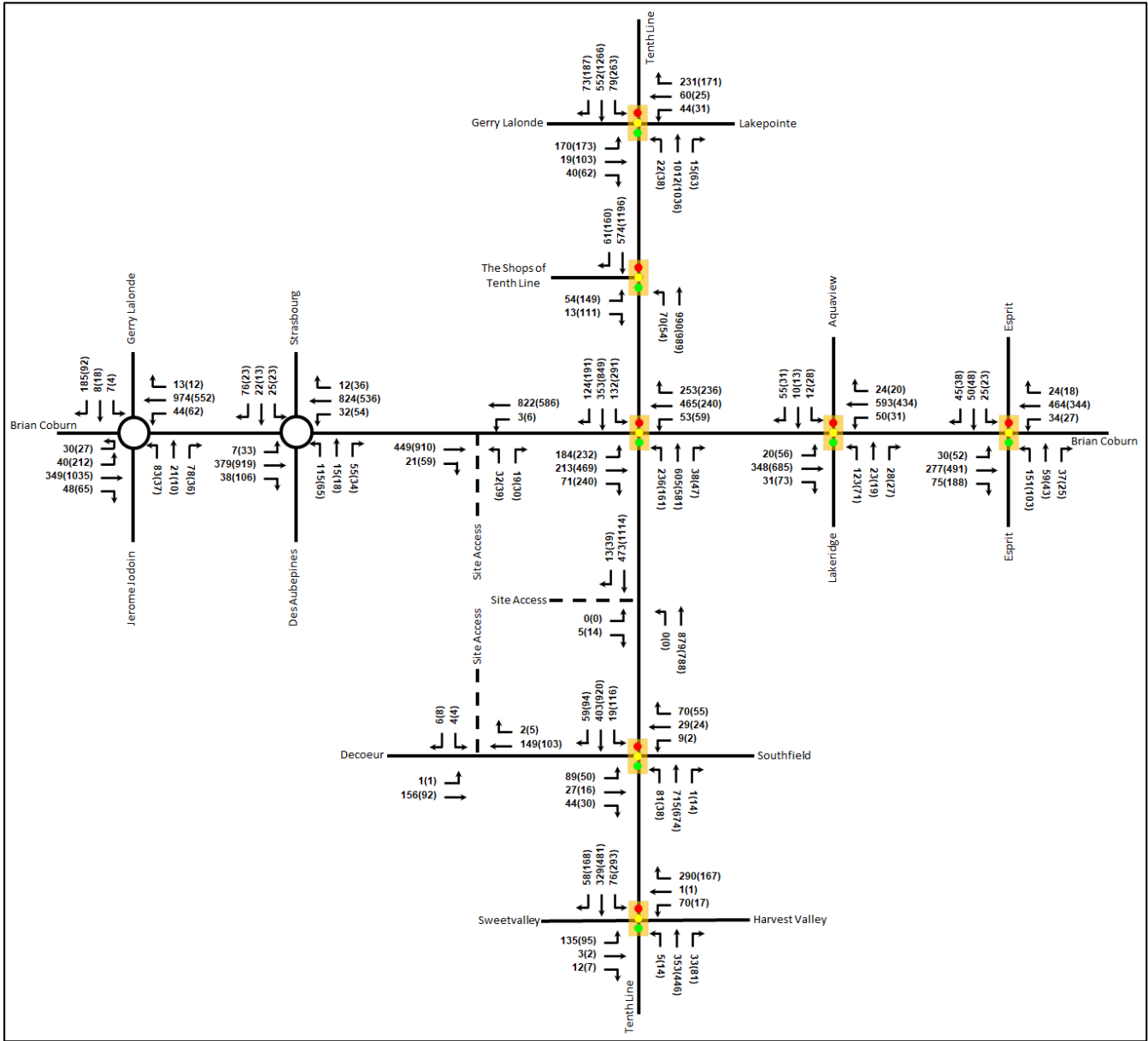


Table 18: 2026 Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
Brian Coburn Boulevard at Site Access <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBT/L	A	0.00	8.3	0.0	B	0.01	10.1	0.0
	NBL/R	C	0.21	23.6	6.0	E	0.41	39.9	13.5
	<b>Overall</b>	<b>A</b>	-	<b>0.9</b>	-	<b>A</b>	-	<b>1.7</b>	-
Site Access at Tenth Line Road <i>Unsignalized</i>	EBL/R	A	0.01	9.8	0.0	B	0.03	13.1	0.8
	NBT	-	-	-	-	-	-	-	-
	SBT/R	-	-	-	-	-	-	-	-
	<b>Overall</b>	<b>A</b>	-	<b>0.0</b>	-	<b>A</b>	-	<b>0.1</b>	-

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Decoeur Drive at Site Access Unsignalized</b>	EBL/T	A	0.00	7.5	0.0	A	0.00	7.4	0.0
	WBT/R	-	-	-	-	-	-	-	-
	SBL/R	A	0.01	9.6	0.0	A	0.01	9.1	0.0
	<b>Overall</b>	<b>A</b>	-	<b>0.3</b>	-	<b>A</b>	-	<b>0.6</b>	-

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

The access intersections for the 2026 future total horizon operate well. No capacity issues are noted.

### 11.3.2 2031 Future Total Access Intersection Operations

The 2031 future total intersection volumes are illustrated in Figure 17 and the access intersection operations are summarized below in Table 19. The level of service is based on HCM 2010 average delay for unsignalized intersections. The synchro worksheets have been provided in Appendix M.

Figure 17: 2031 Future Total Volumes

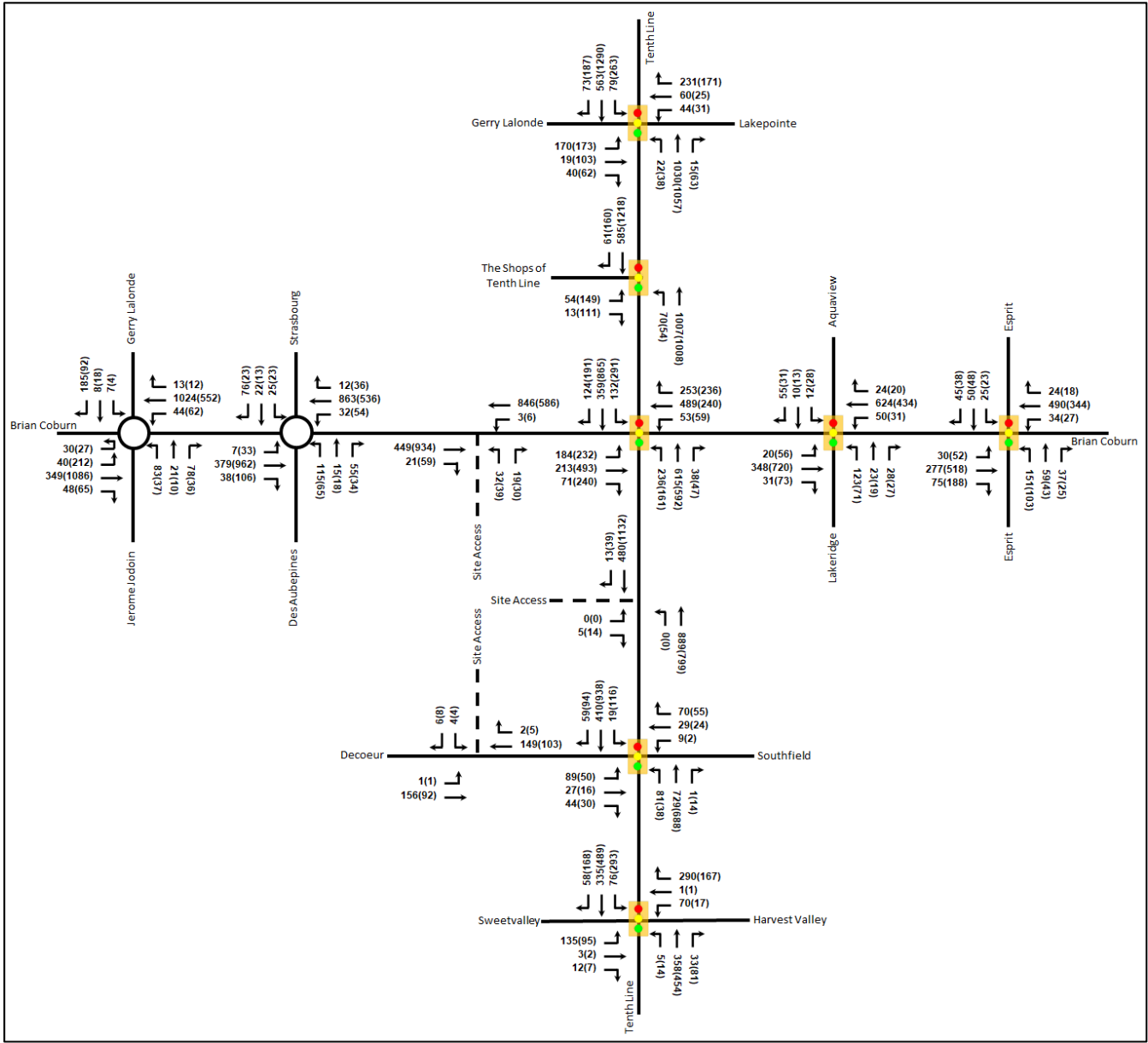


Table 19: 2031 Future Total Access Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
Brian Coburn Boulevard at Site Access <i>Unsignalized</i>	EBT/R	-	-	-	-	-	-	-	-
	WBT/L	A	0.00	8.3	0.0	B	0.01	10.2	0.0
	NBL/R	C	0.22	24.3	6.0	E	0.42	41.7	14.3
	<b>Overall</b>	<b>A</b>	-	<b>0.9</b>	-	<b>A</b>	-	<b>1.8</b>	-
Site Access at Tenth Line Road <i>Unsignalized</i>	EBL/R	A	0.01	9.8	0.0	B	0.03	13.2	0.8
	NBT	-	-	-	-	-	-	-	-
	SBT/R	-	-	-	-	-	-	-	-
	<b>Overall</b>	<b>A</b>	-	<b>0.0</b>	-	<b>A</b>	-	<b>0.1</b>	-

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Decoeur Drive at Site Access Unsignalized</b>	EBL/T	A	0.00	7.5	0.0	A	0.00	7.4	0.0
	WBT/R	-	-	-	-	-	-	-	-
	SBL/R	A	0.01	9.6	0.0	A	0.01	9.1	0.0
	<b>Overall</b>	<b>A</b>	-	<b>0.3</b>	-	<b>A</b>	-	<b>0.6</b>	-

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

The access intersection operations for the 2031 future total horizon operate similarly to the 2026 future total conditions. No capacity issues are noted.

### 11.3.3 Access Intersection MMLoS

As the access intersections are not signalized, no access intersection MMLoS analysis has been performed.

### 11.3.4 Recommended Design Elements

No additional design elements are proposed for the site access intersections beyond the typical private approach considerations.

## 12 Transportation Demand Management

### 12.1 Context for TDM

The mode shares used within the TIA represent the recommended shares for the Orleans district. Overall, the modal shares are likely to be achieved and supporting TDM measures should be provided.

The subject site is not within a design priority area, the bedroom count within the development is subject to the final unit count and layout selections by purchasers, and no age restrictions are noted.

### 12.2 Need and Opportunity

The subject site has been assumed to rely predominantly on auto travel and those assumptions have been carried through the analysis. An increase in transit ridership is likely beyond the study horizons with the proximity to the future BRT corridor. The study area intersections are anticipated to have residual capacity in the background conditions and the level of transit ridership is achievable.

### 12.3 TDM Program

The “suite of post occupancy TDM measures” has been summarized in the TDM checklists for the residential land uses. The checklist is provided in Appendix N. The key TDM measures recommended include:

- Post online links to OC Transpo and post transit routes and maps in employee areas
- Inclusion of a 1-year Presto card for first time new townhome purchase or apartment rental, with a set time frame for this offer (e.g. 6-months) from the initial opening of the site
- Unbundle parking cost from purchase or rental costs, charge for employee parking

## 13 Neighbourhood Traffic Management

The proposed development will connect to the arterial road network directly at Brian Coburn Boulevard and Tenth Line Road, and via the collector roads of Decoeur Drive and Des Aubepines Drive. The TIA guidelines describe a volume threshold for the classification of collector roads as 300 vehicles per peak hour, which are to be interpreted as two-way volumes according to City guidance.



The forecasted volumes along Decoeur Drive between the site access and Tenth Line Road are in the range of 204 -311 two-way vehicles per peak hour. The forecasted volumes along Des Aubepines Drive south of Brian Coburn Boulevard are in the range of 277-290 two-way vehicles per peak hour.

These volumes on Decoeur Drive exceed the TIA Guidelines threshold of 300 vehicles during the peak hour, equivalent to five total vehicles per minute in both directions. In general, the TIA thresholds are typically too low, and may be more appropriately interpreted as one-way volumes. Volumes on Decoeur Drive are approximately four percent higher than these thresholds however, and the subject development is only anticipated to contribute five AM and seven PM vehicles between the site access and Tenth Line. The collector road classifications are considered to be appropriate and site volumes are not considered to impact this classification.

## 14 Transit

### 14.1 Route Capacity

In Section 5.1 the trip generation by mode was estimated, including an estimate of the number of transit trips that will be generated by the proposed development. Table 20 summarizes the transit trip generation.

*Table 20: Trip Generation by Transit Mode*

Travel Mode	Mode Share	AM Peak Period			PM Peak Period		
		In	Out	Total	In	Out	Total
Transit	varies	15	36	53	25	20	45

The proposed development is anticipated to generate an additional 53 AM peak hour transit trips and 45 PM peak hour transit trips. Of these trips, 36 outbound AM trips and 25 inbound PM trips are anticipated. From the trip distribution found in Section 5.3, these values can be further broken down.

Site-generated outbound AM trips break down to 11 trips to the north, two trips to each the south and east, and 22 trips to the west. Site-generated inbound PM trips break down to eight trips from the north, one trip from each the south and east, and 15 trips from the west.

Transit impacts from the site are anticipated as constituting on the order of 10 riders per bus per peak hour on route #234 and on the order of five additional riders per bus on the route #30. To service increased demand the substitution of a single higher-capacity bus (i.e., an articulated bus in place of a standard bus) per peak hour for the routes #30 and #234 may be required.

### 14.2 Transit Priority

No impacts to transit operations are anticipated within the study area as a result of the subject development. The maximum increase in delay in any movement on the isolated measures transit priority corridor is 1.1 seconds at the network intersections. At the access intersection, no measurable average delays are anticipated for westbound through vehicles as a result of site traffic.

## 15 Network Intersection Design

### 15.1 Network Intersection Control

No change to the existing signalized control is recommended for the network intersections. No additional changes to the signal phasing beyond the mitigations included in the background conditions are proposed.

## 15.2 Network Intersection Design

### 15.2.1 2026 Future Total Network Intersection Operations

The 2026 future total network intersection operations, including the phasing proposed as mitigations for the background conditions, are summarized below in Table 21. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM 2010 average delay for unsignalized intersections. Synchro 11 has been used to model the signalized intersections and Sidra 8 to model the study area roundabouts. The Synchro and Sidra worksheets for the 2026 future total horizon have been provided in Appendix L.

Table 21: 2026 Future Total Network Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road</b> <i>Signalized</i>	EBL	C	0.71	48.1	43.2	C	0.71	53.6	49.0
	EBT/R	A	0.18	13.4	10.9	A	0.50	33.5	38.8
	WBL	A	0.17	28.3	13.4	A	0.16	33.0	11.8
	WBT	A	0.17	27.8	16.5	A	0.08	30.8	9.9
	WBR	B	0.62	25.1	38.8	A	0.47	17.7	26.5
	NBL	A	0.05	8.8	m3.2	A	0.18	5.6	2.8
	NBT	A	0.47	12.8	92.4	A	0.46	5.6	19.7
	NBR	A	0.02	1.1	m0.4	A	0.06	0.6	0.7
	SBL	A	0.29	12.3	17.3	D	0.90	52.1	#102.5
	SBT	A	0.27	7.9	34.8	A	0.56	10.3	98.0
	SBR	A	0.08	2.5	5.5	A	0.18	1.7	8.0
	<b>Overall</b>	<b>A</b>	<b>0.53</b>	<b>15.7</b>	-	<b>D</b>	<b>0.86</b>	<b>15.4</b>	-
<b>The Shops of Tenth Line Access at Tenth Line Road</b> <i>Signalized</i>	EBL	A	0.29	40.6	19.3	A	0.54	43.4	37.7
	EBR	A	0.07	18.5	5.2	A	0.37	19.9	19.5
	NBL	A	0.12	6.1	m8.9	A	0.22	10.6	13.5
	NBT	A	0.38	5.7	43.3	A	0.43	8.2	76.1
	SBT	A	0.23	2.3	11.7	A	0.51	5.8	37.9
	SBR	A	0.05	0.5	0.1	A	0.15	0.7	2.5
	<b>Overall</b>	<b>A</b>	<b>0.39</b>	<b>5.6</b>	-	<b>A</b>	<b>0.52</b>	<b>9.2</b>	-
<b>Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive</b> <i>Roundabout</i>	EB	A	0.33	4.8	17.0	A	0.92	6.5	180.2
	WB	A	0.81	7.0	85.6	A	0.59	6.5	38.7
	NB	A	0.19	7.1	7.6	C	0.36	24.3	21.0
	SB	C	0.54	22.0	36.1	A	0.16	6.8	7.3
	<b>Overall</b>	<b>A</b>	<b>0.81</b>	<b>8.0</b>	-	<b>A</b>	<b>0.92</b>	<b>7.2</b>	-
<b>Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive</b> <i>Roundabout</i>	EB	A	0.31	4.1	15.1	A	0.74	4.7	66.2
	WB	A	0.66	5.0	50.0	A	0.47	4.8	28.3
	NB	A	0.19	7.8	7.6	B	0.22	13.1	11.2
	SB	B	0.23	11.3	11.6	A	0.08	8.0	3.1
	<b>Overall</b>	<b>A</b>	<b>0.66</b>	<b>5.6</b>	-	<b>A</b>	<b>0.74</b>	<b>5.4</b>	-

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Brian Coburn Boulevard at Tenth Line Road</b> <i>Signalized</i>	EBL	C	0.75	35.7	#39.4	A	0.59	28.4	50.9
	EBT/R	A	0.39	16.2	45.8	E	0.98	59.9	#221.9
	WBL	A	0.18	24.3	15.4	C	0.74	86.3	#34.9
	WBT	D	0.90	52.7	#125.5	A	0.49	37.2	66.9
	WBR	A	0.47	13.0	32.5	A	0.41	6.4	17.9
	NBL	C	0.75	47.1	#77.5	E	0.93	78.6	#61.6
	NBT/R	A	0.49	26.9	70.1	C	0.75	44.0	86.2
	SBL	A	0.58	28.0	#34.0	D	0.84	43.1	#79.7
	SBT/R	A	0.37	10.9	25.6	E	0.97	57.6	#157.2
<b>Overall</b>	<b>D</b>	<b>0.81</b>	<b>28.4</b>	-	<b>F</b>	<b>1.07</b>	<b>49.3</b>	-	
<b>Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive</b> <i>Signalized</i>	EBL	A	0.05	6.4	3.7	A	0.09	5.0	6.8
	EBT/R	A	0.34	7.3	40.2	A	0.59	8.9	97.9
	WBL	A	0.08	6.4	7.0	A	0.08	5.2	4.6
	WBT/R	A	0.53	9.7	78.0	A	0.36	5.9	44.2
	NBL	A	0.50	29.8	26.0	A	0.38	33.9	19.6
	NBT/R	A	0.16	13.0	9.4	A	0.18	16.4	10.2
	SBL	A	0.05	20.0	4.7	A	0.16	28.7	9.8
	SBT/R	A	0.19	9.4	9.0	A	0.17	14.8	9.2
	<b>Overall</b>	<b>A</b>	<b>0.57</b>	<b>10.9</b>	-	<b>A</b>	<b>0.60</b>	<b>9.7</b>	-
<b>Brian Coburn Boulevard at Esprit Drive</b> <i>Signalized</i>	EBL	A	0.08	10.3	6.2	A	0.11	10.5	9.3
	EBT/R	A	0.41	12.3	46.3	C	0.76	21.0	116.6
	WBL	A	0.08	10.1	6.7	A	0.12	11.4	6.3
	WBT/R	A	0.55	15.4	72.2	A	0.40	12.9	49.1
	NBL	A	0.38	24.3	33.2	A	0.26	22.1	23.4
	NBT/R	A	0.19	13.8	16.5	A	0.13	14.1	12.9
	SBL	A	0.07	19.3	7.8	A	0.06	19.0	7.3
	SBT/R	A	0.18	12.3	15.3	A	0.17	12.9	14.5
	<b>Overall</b>	<b>A</b>	<b>0.48</b>	<b>15.1</b>	-	<b>A</b>	<b>0.56</b>	<b>17.6</b>	-
<b>Decoeur Drive / Southfield Way at Tenth Line Road</b> <i>Signalized</i>	EBL	A	0.42	36.3	20.7	A	0.27	38.0	15.0
	EBT/R	A	0.25	14.8	11.3	A	0.18	17.1	9.6
	WBL	A	0.04	25.0	3.9	A	0.01	29.0	1.8
	WBT/R	A	0.30	12.6	12.9	A	0.28	15.7	12.8
	NBL	A	0.14	8.8	17.1	A	0.10	7.8	9.3
	NBT	A	0.31	7.7	56.7	A	0.26	6.1	50.8
	NBR	A	0.00	0.0	0.0	A	0.01	0.0	0.3
	SBL	A	0.04	6.9	m6.7	A	0.22	8.2	24.3
	SBT	A	0.17	5.0	39.8	A	0.36	6.9	74.3
	SBR	A	0.06	2.3	10.7	A	0.08	2.2	7.0
	<b>Overall</b>	<b>A</b>	<b>0.35</b>	<b>9.2</b>	-	<b>A</b>	<b>0.36</b>	<b>7.8</b>	-

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road</b> <i>Signalized</i>	EBL	C	0.71	44.1	32.7	A	0.55	44.6	28.5
	EBT/R	A	0.04	11.2	4.0	A	0.03	18.1	4.0
	WBL	A	0.23	21.6	15.9	A	0.08	29.4	7.5
	WBT/R	A	0.51	6.2	15.2	A	0.44	8.7	14.7
	NBL	A	0.01	9.0	2.1	A	0.03	6.7	3.6
	NBT/R	A	0.22	8.3	24.4	A	0.23	5.8	31.1
	SBL	A	0.15	9.8	13.9	A	0.53	13.1	63.9
	SBT/R	A	0.21	7.8	23.4	A	0.29	5.8	37.0
<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>12.0</b>	<b>-</b>	<b>-</b>	<b>A</b>	<b>0.53</b>	<b>9.6</b>	<b>-</b>

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

The network intersections for the 2026 future total horizon operate similarly to the 2026 future background conditions.

At the intersection of Brian Coburn Boulevard at Tenth Line Road, the southbound left movement may exhibit extended queuing during the AM peak hour with the splits optimized for the total traffic conditions. During the PM peak hour, delays are anticipated to be increased on the westbound left movement and reduced on the northbound left movement with split optimization for the total traffic conditions.

### 15.2.2 2031 Future Total Network Intersection Operations

The 2031 future total network intersection operations, including the phasing proposed as mitigations for the background conditions, are summarized below in Table 22. The level of service for signalized intersections is based on v/c calculations for individual lane movements and HCM 2000 v/c calculations for the overall intersection, and HCM 2010 average delay for unsignalized intersections. Synchro 11 has been used to model the signalized intersections and Sidra 8 to model the study area roundabouts. The Synchro and Sidra worksheets for the 2031 future total horizon have been provided in Appendix M.

Table 22: 2031 Future Total Network Intersection Operations

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road</b> <i>Signalized</i>	EBL	C	0.71	48.1	43.2	C	0.71	53.6	49.0
	EBT/R	A	0.18	13.4	10.9	A	0.50	33.5	38.8
	WBL	A	0.17	28.3	13.4	A	0.16	33.0	11.8
	WBT	A	0.17	27.8	16.5	A	0.08	30.8	9.9
	WBR	B	0.62	25.9	39.5	A	0.48	18.7	27.4
	NBL	A	0.05	8.7	m3.1	A	0.18	5.7	m2.6
	NBT	A	0.48	12.8	95.1	A	0.47	5.6	19.8
	NBR	A	0.02	1.1	m0.3	A	0.06	0.5	0.6
	SBL	A	0.29	12.6	17.6	E	0.93	57.9	#103.9
	SBT	A	0.27	8.0	35.7	A	0.57	10.4	101.0
	SBR	A	0.08	2.5	5.5	A	0.18	1.7	8.0
<b>Overall</b>	<b>A</b>	<b>0.54</b>	<b>15.8</b>	<b>-</b>	<b>-</b>	<b>D</b>	<b>0.88</b>	<b>15.9</b>	<b>-</b>

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>The Shops of Tenth Line Access at Tenth Line Road Signalized</b>	EBL	A	0.29	40.6	19.3	A	0.54	43.4	37.7
	EBR	A	0.07	18.5	5.2	A	0.38	21.1	20.2
	NBL	A	0.12	6.1	m9.0	A	0.22	10.8	13.7
	NBT	A	0.39	5.8	44.4	A	0.43	8.3	78.2
	SBT	A	0.23	2.3	11.8	A	0.52	5.9	38.4
	SBR	A	0.05	0.5	0.1	A	0.15	0.7	2.5
	<b>Overall</b>	<b>A</b>	<b>0.40</b>	<b>5.6</b>	<b>-</b>	<b>A</b>	<b>0.53</b>	<b>9.3</b>	<b>-</b>
<b>Brian Coburn Boulevard at Gerry Lalonde Drive / Jerome Jodoin Drive Roundabout</b>	EB	A	0.33	4.8	17.1	A	0.92	6.5	180.2
	WB	A	0.84	7.7	103.2	A	0.59	6.5	38.7
	NB	A	0.19	7.1	7.6	C	0.36	24.3	21.0
	SB	C	0.61	29.0	44.1	A	0.16	6.8	7.3
	<b>Overall</b>	<b>A</b>	<b>0.84</b>	<b>9.2</b>	<b>-</b>	<b>A</b>	<b>0.92</b>	<b>7.2</b>	<b>-</b>
<b>Brian Coburn Boulevard at Strasbourg Street / Des Aubepines Drive Roundabout</b>	EB	A	0.31	4.1	15.2	A	0.77	4.8	73.8
	WB	A	0.69	5.1	54.8	A	0.48	4.8	28.5
	NB	A	0.19	7.8	7.6	B	0.24	13.9	12.3
	SB	B	0.25	12.1	12.6	A	0.08	8.0	3.1
	<b>Overall</b>	<b>A</b>	<b>0.69</b>	<b>5.6</b>	<b>-</b>	<b>A</b>	<b>0.77</b>	<b>5.5</b>	<b>-</b>
<b>Brian Coburn Boulevard at Tenth Line Road Signalized</b>	EBL	D	0.82	45.5	#43.9	A	0.60	29.1	50.6
	EBT/R	A	0.39	16.2	45.8	E	1.00	65.3	#232.2
	WBL	A	0.17	23.4	15.1	D	0.87	117.5	#37.6
	WBT	E	0.91	52.6	#131.8	A	0.45	34.1	64.4
	WBR	A	0.46	13.3	33.7	A	0.39	5.8	17.2
	NBL	C	0.76	48.4	#78.1	E	0.93	79.5	#61.5
	NBT/R	A	0.50	27.3	71.2	C	0.77	45.4	88.1
	SBL	A	0.60	29.0	#34.6	D	0.85	45.6	#82.5
	SBT/R	A	0.38	11.0	26.3	E	0.99	62.9	#162.4
	<b>Overall</b>	<b>D</b>	<b>0.83</b>	<b>29.5</b>	<b>-</b>	<b>F</b>	<b>1.10</b>	<b>52.8</b>	<b>-</b>
<b>Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive Signalized</b>	EBL	A	0.05	6.5	3.7	A	0.09	5.0	6.8
	EBT/R	A	0.34	7.3	40.2	B	0.62	9.5	107.6
	WBL	A	0.08	6.4	7.0	A	0.08	5.3	4.6
	WBT/R	A	0.55	10.2	84.6	A	0.36	5.9	44.2
	NBL	A	0.50	29.8	26.0	A	0.38	33.9	19.6
	NBT/R	A	0.16	13.0	9.4	A	0.18	16.4	10.2
	SBL	A	0.05	20.0	4.7	A	0.16	28.7	9.8
	SBT/R	A	0.19	9.4	9.0	A	0.17	14.8	9.2
	<b>Overall</b>	<b>A</b>	<b>0.59</b>	<b>11.1</b>	<b>-</b>	<b>B</b>	<b>0.62</b>	<b>10.0</b>	<b>-</b>
<b>Brian Coburn Boulevard at Esprit Drive Signalized</b>	EBL	A	0.09	10.4	6.3	A	0.11	10.5	9.3
	EBT/R	A	0.41	12.3	46.3	C	0.79	22.7	#126.8
	WBL	A	0.08	10.1	6.7	A	0.13	11.8	6.4
	WBT/R	A	0.58	16.0	77.7	A	0.40	12.9	49.1
	NBL	A	0.38	24.3	33.2	A	0.26	22.1	23.4
	NBT/R	A	0.19	13.8	16.5	A	0.13	14.1	12.9
	SBL	A	0.07	19.3	7.8	A	0.06	19.0	7.3
	SBT/R	A	0.18	12.3	15.3	A	0.17	12.9	14.5
<b>Overall</b>	<b>A</b>	<b>0.50</b>	<b>15.3</b>	<b>-</b>	<b>A</b>	<b>0.58</b>	<b>18.5</b>	<b>-</b>	

Intersection	Lane	AM Peak Hour				PM Peak Hour			
		LOS	V/C	Delay	Q (95 <sup>th</sup> )	LOS	V/C	Delay	Q (95 <sup>th</sup> )
<b>Decoeur Drive / Southfield Way at Tenth Line Road Signalized</b>	EBL	A	0.42	36.3	20.7	A	0.27	38.0	15.0
	EBT/R	A	0.25	14.8	11.3	A	0.18	17.1	9.6
	WBL	A	0.04	25.0	3.9	A	0.01	29.0	1.8
	WBT/R	A	0.30	12.6	12.9	A	0.28	15.7	12.8
	NBL	A	0.14	8.8	17.1	A	0.10	7.9	9.3
	NBT	A	0.32	7.7	58.1	A	0.27	6.2	52.0
	NBR	A	0.00	0.0	0.0	A	0.01	0.0	0.3
	SBL	A	0.04	6.6	m6.4	A	0.22	8.3	24.5
	SBT	A	0.18	4.8	39.8	A	0.37	6.9	76.3
	SBR	A	0.06	2.2	11.0	A	0.08	2.2	7.0
<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>9.1</b>	<b>-</b>	<b>-</b>	<b>A</b>	<b>0.37</b>	<b>7.8</b>	<b>-</b>
<b>Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road Signalized</b>	EBL	C	0.71	44.1	32.7	A	0.55	44.6	28.5
	EBT/R	A	0.04	11.2	4.0	A	0.03	18.1	4.0
	WBL	A	0.23	21.6	15.9	A	0.08	29.4	7.5
	WBT/R	A	0.51	6.2	15.2	A	0.44	8.7	14.7
	NBL	A	0.01	9.0	2.1	A	0.03	6.7	3.6
	NBT/R	A	0.22	8.3	24.8	A	0.24	5.9	31.7
	SBL	A	0.15	9.8	13.9	A	0.54	13.2	64.3
	SBT/R	A	0.21	7.8	23.8	A	0.30	5.8	37.6
	<b>Overall</b>	<b>A</b>	<b>0.36</b>	<b>12.0</b>	<b>-</b>	<b>-</b>	<b>A</b>	<b>0.54</b>	<b>9.7</b>

Notes: Saturation flow rate of 1800 veh/h/lane  
PHF = 1.00

m = metered queue  
# = volume for the 95<sup>th</sup> %ile cycle exceeds capacity

The network intersection operations for the 2031 future total horizon operate similarly to the 2026 future total conditions.

At the intersection of Brian Coburn Boulevard at Tenth Line Road, the eastbound through/right movement is forecasted to operate at its theoretical capacity.

### 15.2.3 Network Intersection MMLOS

Table 23 summarizes the MMLOS analysis for the signalized network intersections. The existing and future conditions for the intersections will be the same and are considered in one row. The intersection analysis is based on the land use designation of “Developing Community” for all but the Decoeur Drive/Southfield Way at Tenth Line Road intersection which will be based on the policy area of “Within 300m of a School.” The MMLOS worksheets has been provided in Appendix K.

Table 23: Study Area Intersection MMLOS Analysis

Intersection	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target
<b>Gerry Lalonde Dr / Lakepointe Dr at Tenth Line Rd</b>	<b>F</b>	C	<b>F</b>	C	<b>F</b>	D	-	-	D	D
<b>The Shops of Tenth Line Access at Tenth Line Rd</b>	<b>F</b>	C	<b>F</b>	C	-	-	-	-	A	D
<b>Brian Coburn Blvd at Tenth Line Rd</b>	<b>F</b>	C	<b>F</b>	B	<b>F</b>	D	C	D	<b>F</b>	D
<b>Brian Coburn Blvd at Aquaview Dr/ Lakeridge Dr</b>	<b>E</b>	C	<b>F</b>	B	C	D	-	-	B	D

Intersection	Pedestrian LOS		Bicycle LOS		Transit LOS		Truck LOS		Auto LOS	
	PLOS	Target	BLOS	Target	TLOS	Target	TrLOS	Target	ALOS	Target
Brian Coburn Blvd at Esprit Dr	E	C	F	B	C	D	-	-	B	D
Decoeur Dr / Southfield Way at Tenth Line Rd	F	A	F	C	E	D	-	-	A	E
Sweetvalley Dr / Harvest Valley Ave at Tenth Line Rd	F	C	F	C	C	D	-	-	A	D

The MMLOS targets will not be met for the pedestrian and bicycle LOS at all network intersections, transit LOS at the intersections of Gerry Lalonde Drive/Lakepoint Drive at Tenth Line Road, Brian Coburn Boulevard at Tenth Line Road, and Decoeur Drive/Southfield Way at Tenth Line Road, and auto LOS at the intersection of Brian Coburn Boulevard at Tenth Line Road.

The pedestrian level of service would require a maximum crossing distance of two lane-widths at each crossing to meet a LOS A and generally a maximum distance of three lane-widths to meet LOS C.

The presence of auxiliary right-turn lanes on mixed traffic approaches for cyclists and left-turn arrangements at the study area intersections each govern the bicycle LOS, requiring separated facilities and two-stage left-turns or left-turn boxes to meet the targets.

The transit LOS will not be met due to the intersection delays on the eastbound approach at the intersection of Gerry Lalonde Drive/Lakepoint Drive at Tenth Line Road, the east and westbound approaches at the intersection of Brian Coburn Boulevard at Tenth Line Road, and the eastbound approach at the intersection of Decoeur Drive/Southfield Way at Tenth Line Road. Delays on these transit movements would need to be reduced to below 30 seconds to meet targets.

The MMLOS improvements will need to be considered by the City during future rehabilitation and widening projects to improve the overall pedestrian, cycling and transit network levels of service.

#### 15.2.4 Recommended Design Elements

No study area intersection design elements are proposed as part of this study.

## 16 Summary of Improvements Indicated and Modifications Options

The following summarizes the analysis and results presented in this TIA report:

### Proposed Site and Screening

- The proposed site includes 240 townhome units and 3,170 m<sup>2</sup> of retail space
- Accesses will be provided along the Brian Coburn Boulevard and Decoeur Drive via full-movements accesses and along Tenth Line Road via a right-in/right-out access
- The development is proposed to be completed as a single phase by 2026
- The Trip Generation, Location, and Safety Triggers were met for the TIA Screening
- This report is in support of a site plan application

### Existing Conditions

- Brian Coburn Boulevard and Tenth Line Road are arterial roads, Esprit Drive, Decoeur Drive, Gerry Lalonde Drive, Jerome Jodoin Drive, Des Aubepines Drive, Aquaview Drive, Lakeridge Drive, Lakepointe Drive, Southfield Way, and Harvest Valley Avenue are collector roads in the study area
- Sidewalks/MUPS are generally provided on both sides of the study area roadways, with a sidewalk on only one side of Aquaview Drive north of Lakepointe Drive and of Sweetvalley Drive
- A MUP is along one side of Tenth Line Road, bike lanes are on both sides of Tenth Line Road transitioning to paved shoulders south of Harvest Valley Avenue
- Tenth Line Road is a spine route, Brian Coburn Boulevard, Cabris Crescent/Azure Street/Trigoria Crescent, Aquaview Drive, Lakeridge Drive, and Esprit Drive are local routes
- The high volumes roadways have produced a high number of collisions at the Brian Coburn Boulevard at Tenth Line Road intersection
- The collisions are predominantly rear end collisions indicating that they are lower speed and a result of congestion
- Movements at the intersection of Brian Coburn Boulevard at Tenth Line Road are over capacity in each peak hour, with queuing noted on multiple approaches during the PM peak hour with the overall intersection over capacity

### Development Generated Travel Demand

- The proposed development is forecasted produce 203 two-way people trips during the AM peak hour and 333 two-way people trips during the PM peak hour
- Of the forecasted people trips, 91 two-way trips will be vehicle trips during the AM peak hour and 147 two-way trips will be vehicle trips during the PM peak hour based on a 47%-51% residential auto modal share target
- Of the forecasted trips, 30% are anticipated to travel north, 5% to travel each south and east, and 60% to travel west

### Background Conditions

- The background developments were explicitly included in the background conditions, along with a total background growth of 1.25% unidirectionally per annum on Brian Coburn Boulevard and 0.50% bidirectionally per annum on Tenth Line Road
- Conditions are anticipated to deteriorate at the intersection of Brian Coburn Boulevard and Tenth Line Road with the background growth and buildout of the study area
- The introduction of protected turn phases at this intersection would reduce the v/c of all movements to 1.00 or below, however the overall intersection will still operate over its theoretical capacity during the PM peak hour

### Development Design

- The bike parking is to be located in racks adjacent to each building and auto parking areas are to be located in surface lots interspersed throughout the site
- Pedestrian connections will be made from all building entrances to surrounding pedestrian facilities or to internal site facilities that connect to the external facilities
- Emergency vehicles are anticipated to circulate the site and garbage collection will occur on-site



### **Parking**

- The development proposes the inclusion of 134 bicycle parking spaces and 439 vehicle parking spaces
- The proposed parking provision meets the minimums prescribed by the zoning by-law

### **Boundary Street Design**

- The boundary streets will not meet pedestrian MMLOS targets, due to auto volumes and operating speeds on Brian Coburn Boulevard and Tenth Line Road, and the conditions on Decoeur Drive are considered to be adequate given it being on the threshold of meeting PLOS targets
- Brian Coburn Boulevard will not meet bicycle LOS targets and would require separated facilities to do so

### **Access Intersections Design**

- A full-movements access is proposed onto each Brian Coburn Boulevard and Decoeur Drive, and a right-in/right-out access onto Tenth Line Road
- The throat length of the access on Tenth Line Road is approximately 22.0 metres and in the future conditions with a widened Brian Coburn Boulevard, this site access throat will be below the 40.0-metre suggested minimum length, both which are considered adequate given the access configuration and the site land use
- The access intersections along are proposed as being minor stop-controlled, with each boundary road operating as a free flow corridor
- The access intersections are anticipated to operate well with no capacity issues noted
- No specific recommendations or design elements are required outside of typical site design

### **TDM**

- Supportive TDM measures to be included within the proposed development should include:
  - Post online links to OC Transpo and post transit routes and maps in employee areas
  - Inclusion of a 1-year Presto card for first time new townhome purchase or apartment rental, with a set time frame for this offer (e.g. 6-months) from the initial opening of the site
  - Unbundle parking cost from purchase or rental costs, charge for employee parking

### **NTM**

- Volumes on Decoeur Drive east of the site access are forecasted to exceed collector road classification threshold by 11 vehicles and the classification is not impacted by the addition of site traffic

### **Transit**

- The site is forecasted to generate 53 new AM and 45 new PM peak hour two-way transit trips
- To meet forecasted transit use, the substitution of a single higher capacity bus per route per peak hour or equivalent capacity, would be required
- No impact to transit operations is anticipated as a result of the development and a maximum increase in delay of 1.1 seconds is forecasted for transit movements on the isolated transit priority measures corridor

### **Network Intersection Design**

- Generally, the network intersections will operate similarly to the background conditions
- The MMLOS targets will not be met for the pedestrian and bicycle LOS at all network intersections, transit LOS at the intersections of Gerry Lalonde Drive/Lakepoint Drive at Tenth Line Road, Brian Coburn

Boulevard at Tenth Line Road, and Decoeur Drive/Southfield Way at Tenth Line Road, and auto LOS at the intersection of Brian Coburn Boulevard at Tenth Line Road

- Improved cycling facilities, including separated facilities and left-turn configurations out of mixed flow could meet the LOS targets but due to the nature of arterials roadways, the pedestrian and transit LOS cannot be met

## 17 Conclusion

It is recommended that, from a transportation perspective, the proposed development applications proceed.

Prepared By:



John Kingsley, EIT  
Transportation Engineering-Intern

Reviewed By:



Andrew Harte, P.Eng.  
Senior Transportation Engineer

# Appendix A

TIA Screening Form and PM Certification Form

City of Ottawa 2017 TIA Guidelines  
Step 1 - Screening Form

Date: 29-Sep-21  
Project Number: 2021-052  
Project Reference: 2370 Tenth Line

1.1 Description of Proposed Development	
Municipal Address	2370 Tenth Line Rd
Description of Location	T-shaped parcel fronting Brian Coburn Blvd, Decoeur Dr, Tenth Line Rd
Land Use Classification	General Mixed Use (GM[950])
Development Size	236 dwelling units, 1900m2 of commercial space
Accesses	1 full-moves onto Brian Coburn Blvd, 1 full-moves onto Decoeur Dr, 1 RIRO onto Tenth Line Blvd
Phase of Development	One
Buildout Year	2025
TIA Requirement	Full TIA Required

1.2 Trip Generation Trigger	
Land Use Type	Townhomes or apartments
Development Size	236 Units
Trip Generation Trigger	Yes

1.3 Location Triggers	
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	Yes
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?	No
Location Trigger	Yes

1.4. Safety Triggers	
Are posted speed limits on a boundary street 80 km/hr or greater?	No
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?	No
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	Yes
Is the proposed driveway within auxiliary lanes of an intersection?	No
Does the proposed driveway make use of an existing median break that serves an existing site?	No
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	No
Does the development include a drive-thru facility?	No
Safety Trigger	Yes



## **TIA Plan Reports**

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

### **CERTIFICATION**

1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
4. I am either a licensed<sup>1</sup> or registered<sup>2</sup> professional in good standing, whose field of expertise [check  appropriate field(s)] is either transportation engineering  or transportation planning .

**1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.**


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Dated at Ottawa this 20 day of September, 2018.  
(City)

Name: Andrew Harte  
(Please Print)

Professional Title: Professional Engineer

  
\_\_\_\_\_  
Signature of Individual certifier that s/he meets the above four criteria

<b>Office Contact Information (Please Print)</b>
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E-Mail Address: Andrew.Harte@CGHTransportation.com



# Appendix B

Turning Movement Counts







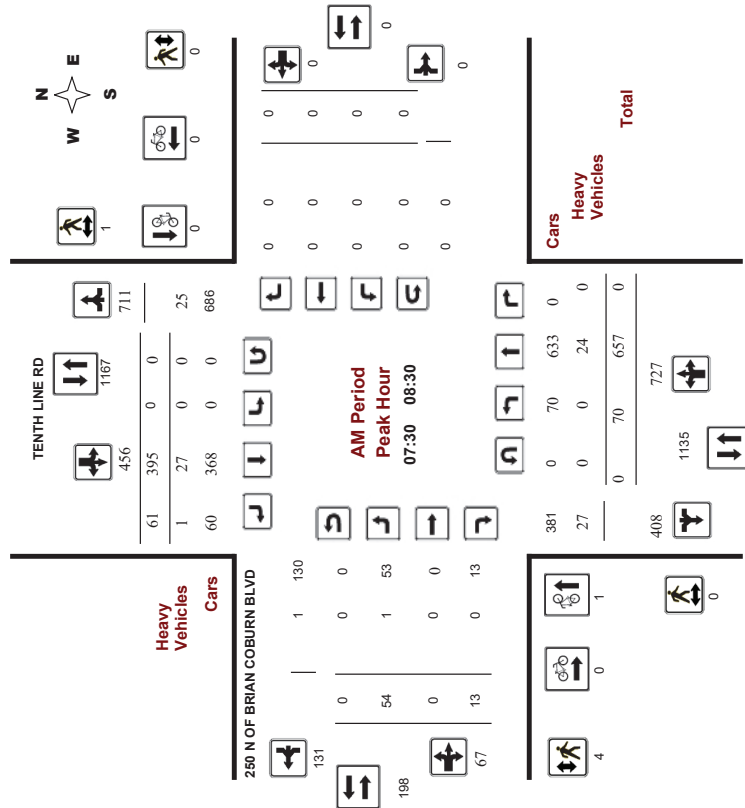
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision



Comments



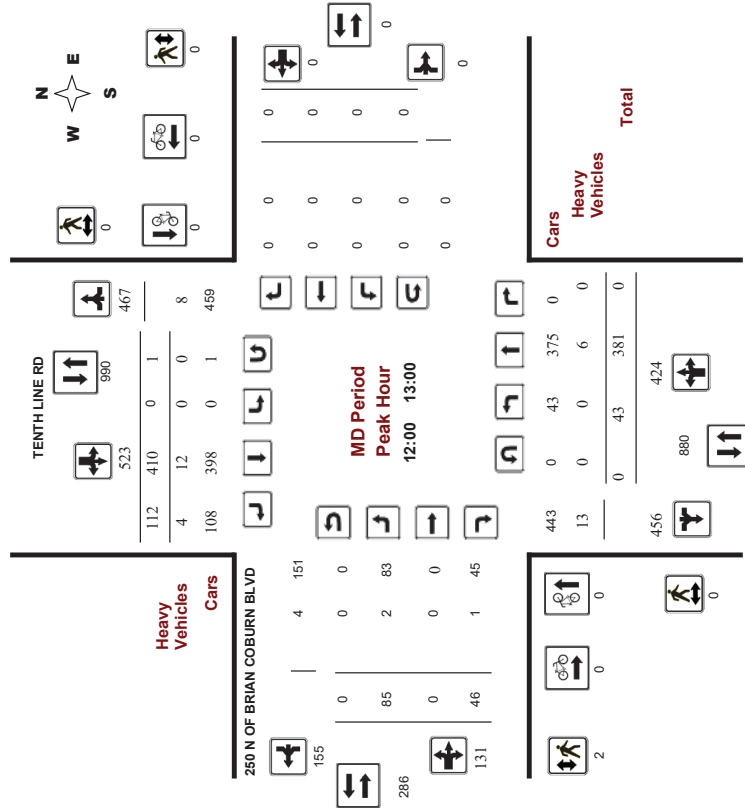
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision



Comments



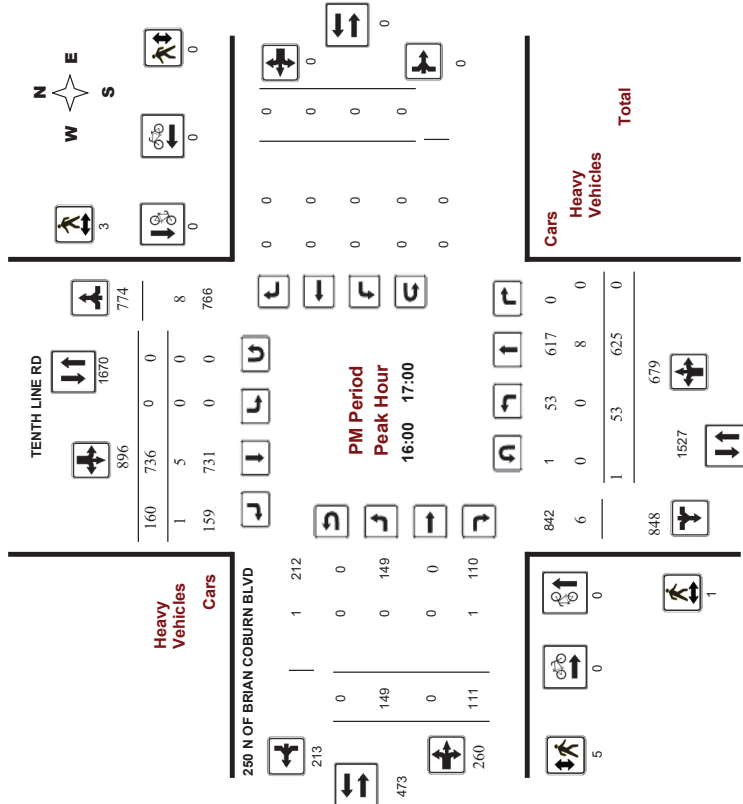
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

## Full Study Summary (8 HR Standard)

Survey Date: Tuesday, January 15, 2019  
Total Observed U-Turns: 1

Northbound: 1  
Southbound: 2  
Eastbound: 0  
Westbound: 0

AADT Factor: 1.10

### TENTH LINE RD

Period	Northbound				Southbound				Eastbound				Westbound				STR TOT	WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	NB	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT				
07:00-08:00	60	616	0	676	0	355	67	422	1088	56	0	9	65	0	0	0	0	0	65	1163
08:00-09:00	80	591	0	671	0	372	71	443	1114	49	0	17	66	0	0	0	0	0	66	1180
09:00-10:00	61	421	0	482	0	278	66	344	826	56	0	39	95	0	0	0	0	0	95	921
11:30-12:30	42	393	0	435	0	400	109	509	944	80	0	44	124	0	0	0	0	0	124	1068
12:30-13:30	35	367	0	402	0	416	111	527	929	81	0	45	126	0	0	0	0	0	126	1055
15:00-16:00	46	544	0	590	0	652	152	804	1394	118	0	62	180	0	0	0	0	0	180	1574
16:00-17:00	53	625	0	678	0	736	160	896	1574	149	0	111	260	0	0	0	0	0	260	1834
17:00-18:00	52	633	0	685	0	728	136	864	1549	121	0	94	215	0	0	0	0	0	215	1764
<b>Sub Total</b>	<b>429</b>	<b>4190</b>	<b>0</b>	<b>4619</b>	<b>0</b>	<b>3937</b>	<b>872</b>	<b>4809</b>	<b>9428</b>	<b>710</b>	<b>0</b>	<b>421</b>	<b>1131</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1131</b>	<b>10559</b>
<b>U-Turns</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>3</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>3</b>	
<b>Total</b>	<b>430</b>	<b>4190</b>	<b>0</b>	<b>4620</b>	<b>2</b>	<b>3937</b>	<b>872</b>	<b>4811</b>	<b>9431</b>	<b>710</b>	<b>0</b>	<b>421</b>	<b>1131</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1131</b>	<b>10562</b>
<b>EQ 12hr</b>	<b>588</b>	<b>5824</b>	<b>0</b>	<b>6422</b>	<b>3</b>	<b>5472</b>	<b>1212</b>	<b>6687</b>	<b>13109</b>	<b>987</b>	<b>0</b>	<b>585</b>	<b>1572</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1572</b>	<b>14681</b>
Note: These values are calculated by multiplying the totals by the appropriate expansion factor: 1.39																				
<b>AVG 12hr</b>	<b>658</b>	<b>6406</b>	<b>0</b>	<b>7064</b>	<b>3</b>	<b>6019</b>	<b>1333</b>	<b>7355</b>	<b>14419</b>	<b>1086</b>	<b>0</b>	<b>644</b>	<b>1730</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1730</b>	<b>16149</b>
Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor: 1.10																				
<b>AVG 24hr</b>	<b>862</b>	<b>8382</b>	<b>0</b>	<b>9254</b>	<b>4</b>	<b>7885</b>	<b>1746</b>	<b>9635</b>	<b>18889</b>	<b>1423</b>	<b>0</b>	<b>844</b>	<b>2267</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2267</b>	<b>21156</b>
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor: 1.31																				
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.																				



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

#### Full Study 15 Minute Increments

##### 250 N OF BRIAN COBURN BLVD

Time Period	Northbound				Southbound				Eastbound				Westbound				W	STR	TOT	TOT	Grand Total				
	LT		RT		LT		RT		LT		RT		LT		RT							W	STR	TOT	TOT
	LT	RT	LT	RT	LT	RT	LT	RT	LT	RT	LT	RT	LT	RT	LT	RT									
07:00	121	0	141	0	80	15	95	236	7	0	3	10	0	0	0	0	0	10	246						
07:15	8	152	0	160	0	89	21	110	270	13	0	4	17	0	0	0	0	17	287						
07:30	16	170	0	186	0	90	15	105	291	16	0	0	16	0	0	0	0	16	307						
07:45	16	173	0	189	0	96	16	112	301	20	0	2	22	0	0	0	0	22	323						
08:00	20	154	0	174	0	94	16	110	284	11	0	2	13	0	0	0	0	13	297						
08:15	18	160	0	178	0	115	14	129	307	7	0	9	16	0	0	0	0	16	323						
08:30	26	154	0	180	0	78	20	98	278	22	0	3	25	0	0	0	0	25	303						
08:45	16	123	0	139	0	85	21	106	245	9	0	3	12	0	0	0	0	12	257						
09:00	21	118	0	139	0	68	19	87	226	23	0	8	31	0	0	0	0	31	287						
09:15	6	107	0	113	0	69	13	82	195	9	0	6	15	0	0	0	0	15	210						
09:30	20	95	0	115	0	74	17	91	206	8	0	12	20	0	0	0	0	20	226						
09:45	14	101	0	110	0	67	17	84	199	16	0	13	29	0	0	0	0	29	228						
10:00	10	107	0	117	0	104	37	141	258	12	0	6	18	0	0	0	0	18	276						
10:15	8	102	0	110	0	80	20	100	210	21	0	12	33	0	0	0	0	33	243						
10:30	10	88	0	98	0	112	21	133	231	26	0	16	42	0	0	0	0	42	273						
10:45	14	96	0	110	0	104	31	135	245	21	0	10	31	0	0	0	0	31	276						
11:00	9	95	0	104	1	102	28	131	235	20	0	12	32	0	0	0	0	32	267						
11:15	10	102	0	112	0	92	32	124	236	18	0	8	26	0	0	0	0	26	262						
11:30	8	85	0	91	0	108	21	129	220	28	0	16	44	0	0	0	0	44	264						
11:45	10	85	0	95	0	114	30	144	239	15	0	9	24	0	0	0	0	24	263						
12:00	11	115	0	128	0	164	42	206	332	27	0	12	39	0	0	0	0	39	371						
12:15	12	140	0	152	1	151	39	191	343	26	0	15	41	0	0	0	0	41	384						
12:30	13	144	0	157	0	164	29	183	350	34	0	12	46	0	0	0	0	46	396						
12:45	10	145	0	155	0	173	42	215	370	31	0	23	54	0	0	0	0	54	424						
13:00	12	152	0	164	0	193	45	238	402	35	0	26	61	0	0	0	0	61	463						
13:15	14	152	0	166	0	173	53	226	392	32	0	24	56	0	0	0	0	56	448						
13:30	10	157	0	167	0	178	36	213	380	45	0	31	76	0	0	0	0	76	456						
13:45	18	164	0	182	0	192	27	219	401	37	0	30	67	0	0	0	0	67	468						
14:00	18	139	0	157	0	190	44	234	391	34	0	28	60	0	0	0	0	60	451						
14:15	13	179	0	192	0	173	27	200	392	28	0	30	58	0	0	0	0	58	450						
14:30	10	142	0	152	0	188	19	207	389	33	0	18	51	0	0	0	0	51	410						
14:45	11	173	0	184	0	177	46	223	407	26	0	20	46	0	0	0	0	46	453						
Total:	430	4190	0	4620	2	3937	872	4811	9431	710	0	421	1131	0	0	0	0	9431	10,562						

Note: U-Turns are included in Totals.



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

#### Full Study Cyclist Volume

##### 250 N OF BRIAN COBURN BLVD

Time Period	TENTH LINE RD			250 N OF BRIAN COBURN BLVD			Grand Total
	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	
07:00	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0
08:15	1	0	1	0	0	0	1
08:30	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0
Total	1	0	1	0	0	0	1



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

### Full Study Pedestrian Volume

#### TENTH LINE RD

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)		Total	Grand Total
	E or W	S or N	N or S	E or W	N or S	E or W		
07:00	0	0	1	0	0	0	1	1
07:15	0	1	0	0	0	0	1	1
07:30	0	0	2	0	0	0	2	2
07:45	0	1	1	0	0	0	2	2
08:00	0	0	1	0	0	0	1	1
08:15	0	0	0	0	0	0	0	0
08:30	0	0	1	0	0	0	1	1
08:45	0	1	1	0	0	0	2	2
09:00	0	0	0	0	0	0	0	0
09:15	0	0	1	0	0	0	1	1
09:30	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0
10:00	0	0	2	0	0	0	2	2
11:30	1	0	1	0	0	0	2	3
11:45	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0
12:45	0	0	2	0	0	0	2	2
13:00	0	0	1	0	0	0	1	1
13:15	0	0	2	0	0	0	2	2
15:00	1	0	3	0	0	0	4	4
15:15	0	0	2	0	0	0	2	2
15:30	1	0	1	0	0	0	2	2
15:45	0	0	2	0	0	0	2	2
16:00	0	1	1	0	0	0	2	1
16:15	0	0	1	0	0	0	1	1
16:30	1	0	2	0	0	0	3	3
16:45	0	2	2	0	0	0	4	4
17:00	0	2	0	0	0	0	2	2
17:15	0	0	0	0	0	0	0	0
17:30	0	1	2	0	0	0	3	3
17:45	0	0	0	0	0	0	0	0
17:50	0	0	0	0	0	0	0	0
Total	3	10	13	0	0	0	30	43



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

### Full Study Heavy Vehicles

#### 250 N OF BRIAN COBURN BLVD

Time Period	Northbound			Southbound			Eastbound			Westbound			W	STR	Grand Total	
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT				RT
07:00	0	2	0	2	0	5	0	5	0	0	0	0	0	0	0	7
07:15	0	1	0	1	0	7	0	7	0	1	0	0	0	0	0	10
07:30	0	1	0	1	0	6	0	6	0	0	0	0	0	0	0	7
07:45	0	7	0	7	0	5	1	6	13	0	0	0	0	0	0	13
08:00	0	10	0	10	0	7	0	7	17	0	0	0	0	0	0	17
08:15	0	6	0	6	0	9	0	9	15	1	0	0	0	0	1	16
08:30	0	7	0	7	0	5	1	6	13	0	0	0	0	0	0	13
08:45	0	4	0	4	0	3	1	4	9	2	0	0	0	0	2	11
09:00	0	1	0	1	0	4	0	4	5	0	0	0	0	0	0	5
09:15	0	5	0	5	0	2	1	3	8	0	0	0	0	0	0	8
09:30	0	2	0	2	0	2	0	2	3	1	0	1	2	0	0	5
09:45	0	2	0	2	0	2	0	2	4	2	0	0	2	0	0	6
10:00	0	2	0	2	0	0	1	1	3	0	0	0	0	0	0	3
11:30	0	2	0	2	0	4	1	5	7	0	0	0	0	0	0	7
11:45	0	0	0	0	0	3	1	4	2	0	1	3	0	0	0	4
12:00	0	1	0	1	0	3	0	3	4	0	0	0	0	0	0	4
12:15	0	3	0	3	0	3	1	4	7	0	0	0	0	0	0	7
12:30	0	2	0	2	0	3	2	5	7	0	0	0	0	0	0	7
12:45	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	8
13:00	0	3	0	3	0	2	1	3	6	0	0	0	0	0	0	6
13:15	0	5	0	5	0	2	0	2	7	0	0	0	0	0	0	7
15:00	0	5	0	5	0	4	0	4	9	0	0	0	0	0	0	9
15:15	0	5	0	5	0	3	0	3	8	0	0	0	0	0	0	8
15:30	0	0	0	0	0	4	0	4	4	0	0	0	0	0	0	4
15:45	0	4	0	4	0	3	1	4	8	0	0	0	0	0	0	8
16:00	0	4	0	4	0	3	1	4	8	0	0	0	0	0	0	8
16:15	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	2
16:30	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	2
16:45	0	2	0	2	0	0	0	0	2	0	0	1	1	0	0	3
17:00	0	2	0	2	0	0	0	0	2	5	0	0	0	0	0	5
17:15	0	5	0	5	0	3	0	3	8	0	0	0	0	0	0	8
17:30	0	1	0	1	0	3	0	3	4	0	0	0	0	0	0	4
17:45	0	2	0	2	0	1	0	1	3	0	0	0	0	0	0	3
17:50	0	2	0	2	0	1	0	1	3	0	0	0	0	0	0	3
Total	3	96	0	99	0	106	12	118	217	9	0	4	13	0	0	230



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### 250 N OF BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Tuesday, January 15, 2019  
Start Time: 07:00

WO No: 38271  
Device: Miovision

#### Full Study 15 Minute U-Turn Total

##### TENTH LINE RD

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	1	0	0	0	0	0	0	1
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	1	0	0	0	0	0	0	1
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	1	0	0	0	0	0	0	0	1
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	1	2	0	0	0	0	0	0	3



# Transportation Services - Traffic Services

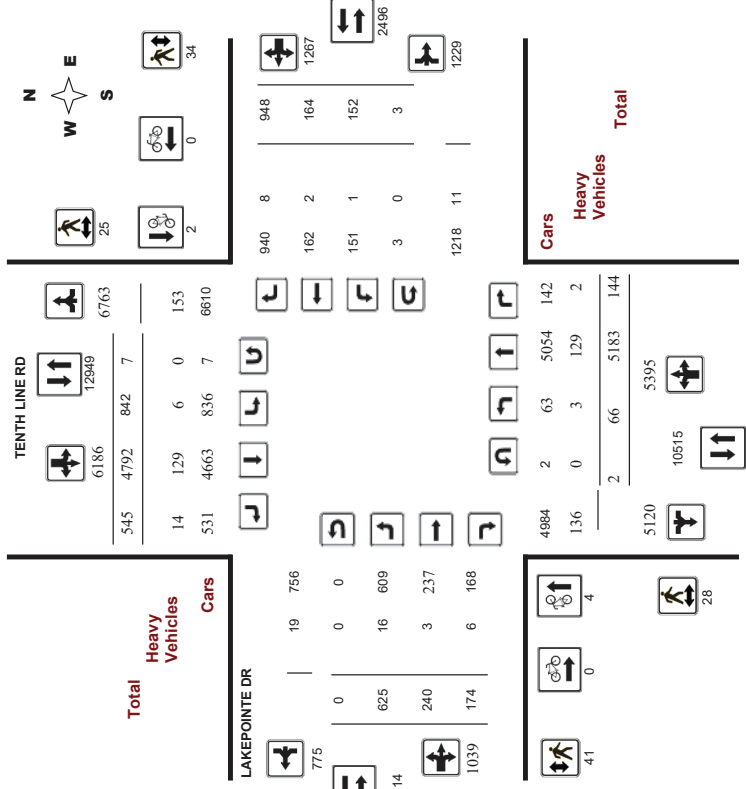
## Turning Movement Count - Study Results

### TENTH LINE RD @ LAKEPOINTE DR

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37742  
Device: Miovision

#### Full Study Diagram





# Transportation Services - Traffic Services

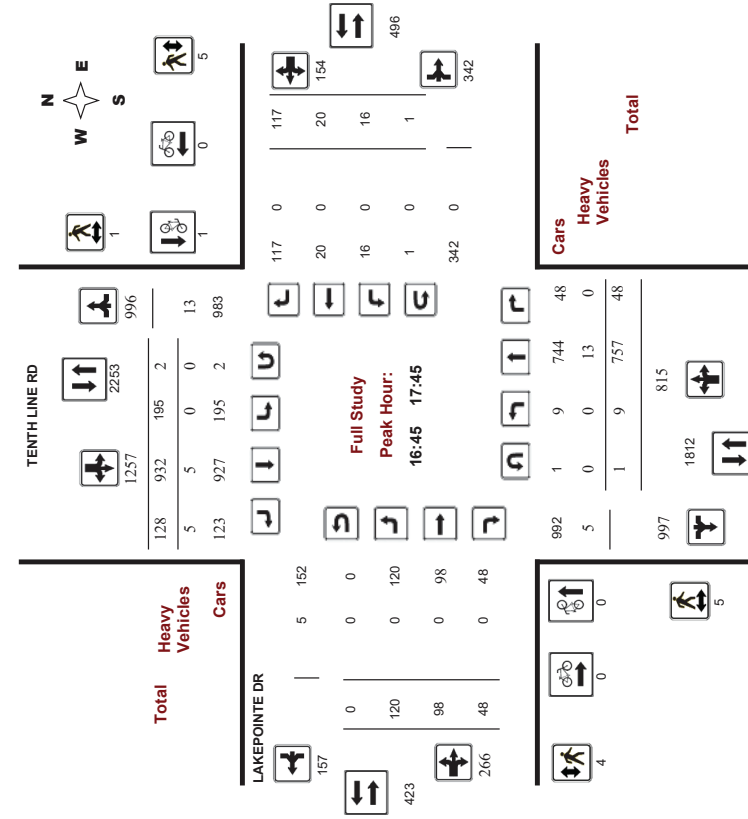
## Turning Movement Count - Study Results

### TENTH LINE RD @ LAKEPOINTE DR

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37742  
Device: Miovision

### Full Study Peak Hour Diagram



Comments



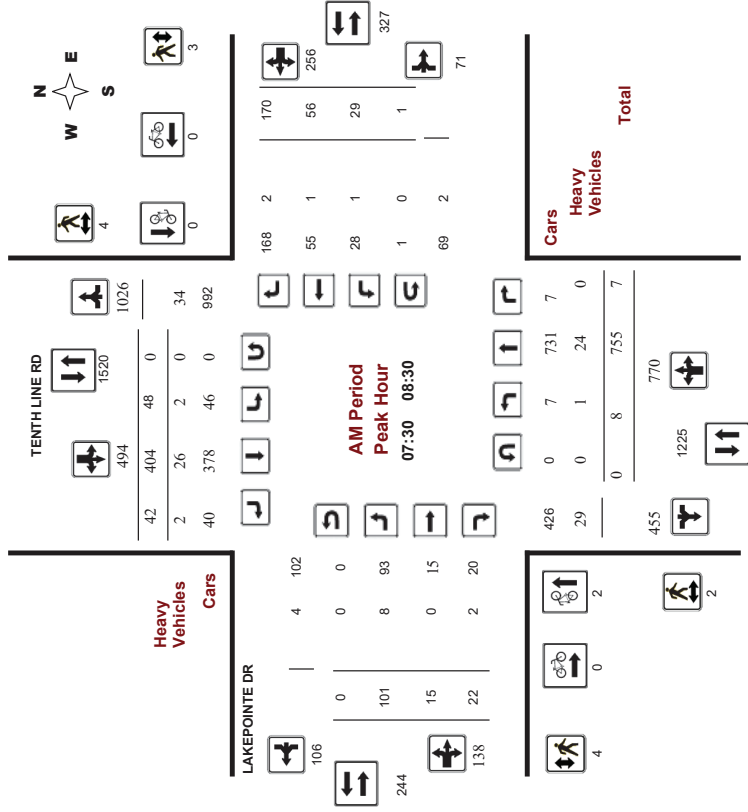
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### TENTH LINE RD @ LAKEPOINTE DR

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37742  
Device: Miovision



Comments



# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

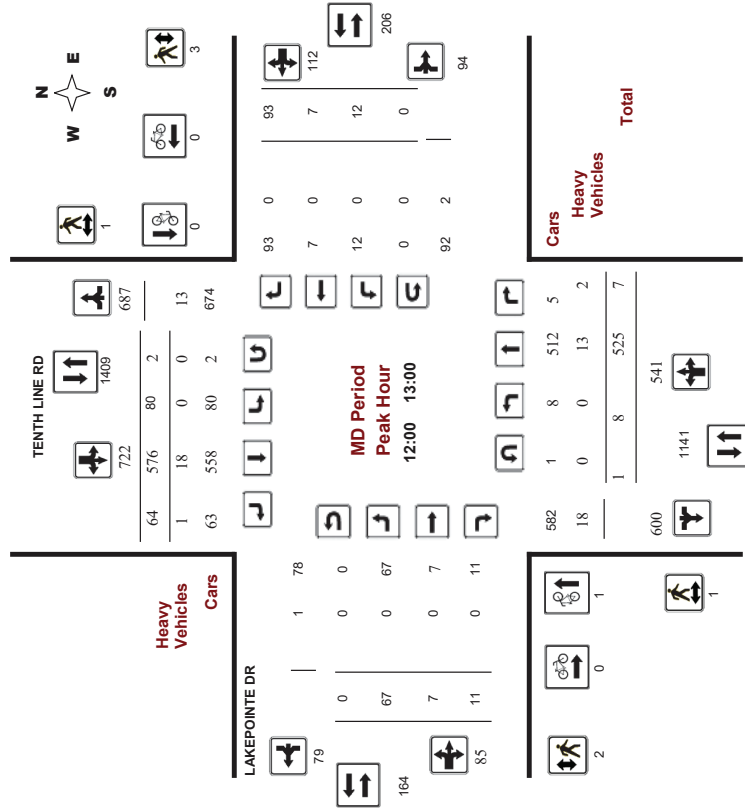
### TENTH LINE RD @ LAKEPOINTE DR

Survey Date: Thursday, April 19, 2018

WO No: 37742

Start Time: 07:00

Device: Miovision



Comments



# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

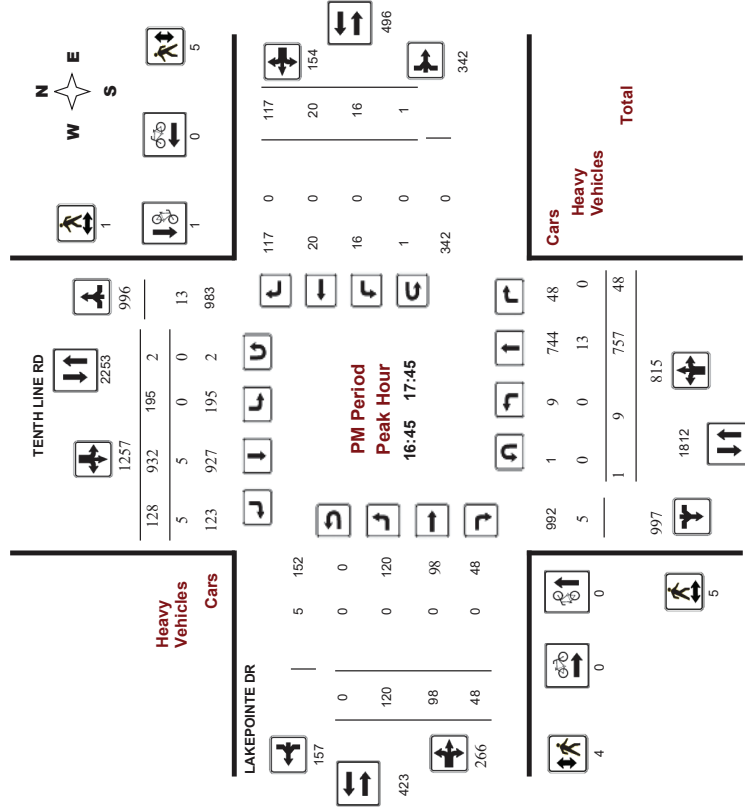
### TENTH LINE RD @ LAKEPOINTE DR

Survey Date: Thursday, April 19, 2018

WO No: 37742

Start Time: 07:00

Device: Miovision



Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Thursday, April 19, 2018

**Total Observed U-Turns**  
 Northbound: 2 Southbound: 7  
 Eastbound: 0 Westbound: 3

**AADT Factor**  
 90

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00-08:00	7	688	8	713	41	385	48	474	1187	84	17	15	116	29	59	191	279	395	1582
08:00-09:00	11	703	6	720	48	367	43	458	1178	85	13	21	119	24	37	156	217	336	1514
09:00-10:00	5	548	7	560	62	327	34	423	983	67	4	15	86	17	6	124	147	233	1216
11:30-12:30	4	543	11	558	87	543	48	678	1236	40	4	9	53	13	8	90	111	164	1400
12:30-13:30	8	524	8	540	82	564	55	701	1241	71	8	10	89	12	4	70	86	175	1416
15:00-16:00	10	673	23	706	133	810	96	1039	1745	60	25	20	105	18	17	95	130	235	1980
16:00-17:00	12	759	33	804	186	879	94	1159	1963	111	88	41	240	21	14	107	142	382	2345
17:00-18:00	9	735	48	792	203	917	127	1247	2039	107	81	43	231	18	19	115	152	383	2422
<b>Sub Total</b>	<b>66</b>	<b>5183</b>	<b>144</b>	<b>5393</b>	<b>842</b>	<b>4792</b>	<b>545</b>	<b>6179</b>	<b>11572</b>	<b>625</b>	<b>240</b>	<b>174</b>	<b>1039</b>	<b>152</b>	<b>164</b>	<b>948</b>	<b>1264</b>	<b>2303</b>	<b>13875</b>
<b>U-Turns</b>	<b>2</b>				<b>7</b>	<b>9</b>				<b>0</b>					<b>3</b>			<b>3</b>	<b>12</b>
<b>Total</b>	<b>66</b>	<b>5183</b>	<b>144</b>	<b>5395</b>	<b>842</b>	<b>4792</b>	<b>545</b>	<b>6186</b>	<b>11581</b>	<b>625</b>	<b>240</b>	<b>174</b>	<b>1039</b>	<b>152</b>	<b>164</b>	<b>948</b>	<b>1267</b>	<b>2306</b>	<b>13887</b>
<b>EQ 12hr</b>	<b>92</b>	<b>7204</b>	<b>200</b>	<b>7499</b>	<b>1170</b>	<b>6661</b>	<b>758</b>	<b>8599</b>	<b>16098</b>	<b>869</b>	<b>334</b>	<b>242</b>	<b>1444</b>	<b>211</b>	<b>228</b>	<b>1318</b>	<b>1761</b>	<b>3205</b>	<b>19303</b>

Note: These values are calculated by multiplying the totals by the appropriate expansion factor.

**AVG 12hr** 78 6111 170 6361 983 5650 643 7293 14488 737 283 205 1225 179 193 1118 1484 2884 17373  
 Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

**AVG 24hr** 102 8005 222 8333 1300 7401 842 9554 17887 965 371 269 1605 235 253 1464 1957 3562 21449  
 Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to U-Turn Report for specific breakdown.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

**Full Study 15 Minute Increments**

**Survey Date:** Thursday, April 19, 2018

**Total Observed U-Turns**  
 Northbound: 2 Southbound: 7  
 Eastbound: 0 Westbound: 3

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00-07:15	3	160	2	165	10	85	12	107	10	15	4	4	23	11	11	50	72	10	367
07:15-07:30	3	152	2	157	10	92	13	116	15	23	3	6	32	7	20	49	76	15	381
07:30-07:45	1	195	1	197	8	110	13	131	16	25	6	3	34	3	16	50	69	16	431
07:45-08:00	0	191	3	194	13	98	10	121	11	21	4	2	27	8	12	42	62	11	404
08:00-08:15	4	186	2	192	11	97	8	116	11	29	2	13	44	6	15	39	61	11	413
08:15-08:30	3	183	1	187	16	99	11	126	17	26	3	4	33	12	13	39	64	17	410
08:30-08:45	3	184	2	189	10	92	14	116	15	20	5	2	27	5	6	39	50	15	362
08:45-09:00	1	150	1	152	11	79	10	100	10	10	3	2	15	1	3	39	44	10	311
09:00-09:15	3	141	3	147	12	74	8	94	11	24	3	6	33	6	3	41	50	11	324
09:15-09:30	1	139	1	141	19	69	8	96	8	23	1	3	27	5	1	31	37	8	301
09:30-09:45	1	125	1	127	17	94	5	116	7	11	0	2	13	4	1	32	37	7	283
09:45-10:00	0	143	2	145	14	90	13	117	6	9	0	4	13	2	1	20	23	6	298
10:00-10:15	0	140	1	141	20	117	8	146	6	7	0	2	9	3	1	19	23	6	319
10:15-10:30	2	139	6	147	23	143	13	179	10	5	2	2	9	5	4	19	28	10	363
10:30-10:45	0	138	2	140	18	140	12	171	11	16	1	1	18	3	2	31	36	11	365
10:45-11:00	2	126	2	130	26	143	15	185	6	12	1	4	17	2	1	21	24	6	356
11:00-11:15	1	131	2	134	18	149	22	189	8	18	3	3	24	4	4	20	28	8	375
11:15-11:30	5	130	1	137	18	144	15	177	9	21	2	3	26	3	0	21	24	9	364
11:30-11:45	0	136	2	138	18	129	12	160	5	16	0	3	19	4	0	15	19	5	336
11:45-12:00	2	127	3	132	28	142	6	176	8	16	3	1	20	1	0	14	15	8	343
12:00-12:15	1	148	6	155	28	188	25	239	10	9	3	1	13	2	5	21	28	10	435
12:15-12:30	1	163	5	169	28	194	22	244	11	17	6	8	31	6	5	22	33	11	497
12:30-12:45	4	173	8	185	40	213	24	277	8	19	3	6	28	4	4	30	38	8	528
12:45-13:00	4	169	4	177	39	215	25	279	6	15	13	5	33	6	3	22	31	6	520
13:00-13:15	4	161	4	169	46	209	18	273	5	26	9	4	39	6	5	31	42	5	523
13:15-13:30	4	178	10	192	55	223	22	300	7	26	21	3	50	5	0	22	27	7	569
13:30-13:45	3	206	8	217	40	226	24	290	6	22	31	18	71	3	6	23	32	6	610
13:45-14:00	1	214	11	226	45	221	30	297	9	37	27	16	80	7	3	31	41	9	644
14:00-14:15	3	160	14	197	48	230	35	314	6	26	25	8	59	2	4	25	31	6	601
14:15-14:30	4	165	13	182	54	249	33	336	5	28	27	10	65	5	7	26	39	5	622
14:30-14:45	1	198	10	210	48	232	30	310	3	29	19	14	62	2	6	35	43	3	625
14:45-15:00	1	192	11	204	53	206	29	288	7	24	10	11	45	9	2	29	40	7	577
<b>Total:</b>	<b>66</b>	<b>5183</b>	<b>144</b>	<b>5395</b>	<b>842</b>	<b>4792</b>	<b>545</b>	<b>6186</b>	<b>283</b>	<b>625</b>	<b>240</b>	<b>174</b>	<b>1039</b>	<b>152</b>	<b>164</b>	<b>948</b>	<b>1267</b>	<b>2306</b>	<b>13887</b>

Note: U-Turns are included in Totals.





**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

**Full Study Cyclist Volume**  
**LAKEPOINTE DR**

Time Period	TENTH LINE RD		LAKEPOINTE DR		Grand Total
	Northbound	Southbound	Street Total	Westbound	
07:00 07:15	0	0	0	0	0
07:15 07:30	0	0	0	0	0
07:30 07:45	0	0	0	0	0
07:45 08:00	2	0	2	0	2
08:00 08:15	0	0	0	0	0
08:15 08:30	0	0	0	0	0
08:30 08:45	0	0	0	0	0
08:45 09:00	0	0	0	0	0
09:00 09:15	0	0	0	0	0
09:15 09:30	0	0	0	0	0
09:30 09:45	0	0	0	0	0
09:45 10:00	0	0	0	0	0
10:00 10:15	0	0	0	0	0
10:15 10:30	0	0	0	0	0
10:30 10:45	0	0	0	0	0
10:45 11:00	0	0	0	0	0
11:00 11:15	0	0	0	0	0
11:15 11:30	0	0	0	0	0
11:30 11:45	0	0	0	0	0
11:45 12:00	0	0	0	0	0
12:00 12:15	1	0	1	0	1
12:15 12:30	0	0	0	0	0
12:30 12:45	0	0	0	0	0
12:45 13:00	0	0	0	0	0
13:00 13:15	0	0	0	0	0
13:15 13:30	0	0	0	0	0
13:30 13:45	0	0	0	0	0
13:45 14:00	0	0	0	0	0
14:00 14:15	0	0	0	0	0
14:15 14:30	1	0	1	0	1
14:30 14:45	0	1	1	0	1
14:45 15:00	0	0	0	0	0
15:00 15:15	0	0	0	0	0
15:15 15:30	0	0	0	0	0
15:30 15:45	0	0	0	0	0
15:45 16:00	0	0	0	0	0
16:00 16:15	0	0	0	0	0
16:15 16:30	0	0	0	0	0
16:30 16:45	0	0	0	0	0
16:45 17:00	0	0	0	0	0
17:00 17:15	0	1	1	0	1
17:15 17:30	0	0	0	0	0
17:30 17:45	0	0	0	0	0
17:45 18:00	0	0	0	0	0
18:00 18:15	0	0	0	0	0
18:15 18:30	0	0	0	0	0
18:30 18:45	0	0	0	0	0
18:45 19:00	0	0	0	0	0
<b>Total</b>	<b>4</b>	<b>2</b>	<b>6</b>	<b>0</b>	<b>6</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

**Full Study Pedestrian Volume**  
**LAKEPOINTE DR**

Time Period	TENTH LINE RD		LAKEPOINTE DR		Grand Total
	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	
07:00 07:15	0	1	1	1	2
07:15 07:30	1	3	4	3	7
07:30 07:45	0	1	1	1	2
07:45 08:00	1	0	1	3	4
08:00 08:15	1	0	1	0	1
08:15 08:30	0	3	3	0	3
08:30 08:45	0	0	0	1	1
08:45 09:00	1	0	1	2	3
09:00 09:15	0	1	1	0	1
09:15 09:30	1	2	3	4	7
09:30 09:45	3	2	5	2	7
09:45 10:00	0	0	0	0	0
10:00 10:15	2	2	4	1	5
10:15 10:30	0	3	3	0	3
10:30 10:45	0	0	0	1	1
10:45 11:00	0	0	0	1	1
11:00 11:15	0	0	0	0	0
11:15 11:30	0	0	0	1	1
11:30 11:45	0	0	0	0	0
11:45 12:00	0	0	0	0	0
12:00 12:15	0	0	0	1	1
12:15 12:30	0	0	0	1	1
12:30 12:45	0	1	1	0	1
12:45 13:00	0	0	0	1	1
13:00 13:15	1	2	3	3	6
13:15 13:30	1	1	2	2	4
13:30 13:45	0	1	1	0	1
13:45 14:00	0	1	1	3	4
14:00 14:15	0	0	0	1	1
14:15 14:30	4	0	4	2	6
14:30 14:45	4	0	4	3	7
14:45 15:00	0	0	0	0	0
15:00 15:15	0	1	1	1	2
15:15 15:30	0	0	0	2	2
15:30 15:45	0	0	0	0	0
15:45 16:00	0	0	0	1	1
16:00 16:15	0	0	0	0	0
16:15 16:30	0	0	0	3	3
16:30 16:45	1	1	2	1	4
16:45 17:00	0	0	0	0	0
17:00 17:15	2	0	2	1	3
17:15 17:30	1	0	1	3	4
17:30 17:45	2	1	3	0	3
17:45 18:00	1	0	1	2	3
18:00 18:15	1	0	1	1	2
18:15 18:30	1	0	1	0	1
18:30 18:45	1	0	1	0	1
18:45 19:00	1	0	1	0	1
<b>Total</b>	<b>28</b>	<b>25</b>	<b>53</b>	<b>41</b>	<b>94</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

**Full Study Heavy Vehicles**

Time Period	Northbound				Southbound				Eastbound				Westbound				Grand Total			
	LT		RT		LT		RT		LT		ST		RT		LT			ST		
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total		U-Turn	Total	
07:00	0	4	0	4	0	6	0	6	10	1	0	0	0	1	0	0	0	0	1	11
07:15	0	4	0	4	0	11	0	11	16	1	0	0	0	0	0	0	0	0	1	16
07:30	0	7	0	7	0	9	0	9	16	2	0	0	0	2	0	0	0	0	2	18
07:45	0	4	0	4	0	7	0	7	11	1	0	0	0	1	1	1	1	1	2	13
08:00	0	5	0	5	1	5	0	6	11	3	0	0	0	3	0	0	1	1	4	15
08:15	0	8	0	8	0	9	1	5	2	8	17	2	0	2	4	1	1	0	2	23
08:30	0	8	0	8	0	7	0	7	15	0	1	1	2	0	0	1	1	3	18	
08:45	0	4	0	4	1	4	1	6	10	0	0	0	0	0	0	1	1	1	11	
09:00	0	6	0	6	1	4	0	5	11	1	0	0	1	0	0	0	0	1	12	
09:15	0	5	0	5	0	3	0	3	8	1	0	0	1	0	0	0	0	1	9	
09:30	0	4	0	4	0	3	0	3	7	1	0	0	1	0	0	0	0	1	8	
09:45	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	0	0	6	
10:00	0	5	0	5	0	1	0	1	6	0	0	0	0	0	0	0	0	0	6	
10:15	0	5	0	5	0	5	0	5	10	0	0	0	0	0	0	0	0	0	10	
10:30	0	3	1	4	0	6	1	7	11	0	0	0	0	0	0	0	0	0	11	
10:45	0	2	0	2	0	4	0	4	6	0	0	0	0	0	0	0	0	0	6	
11:00	0	4	1	5	0	3	0	3	8	0	0	0	0	0	0	0	0	0	8	
11:15	0	4	0	4	0	5	0	5	9	0	0	0	0	0	0	0	0	0	9	
11:30	0	3	0	3	0	2	0	2	5	0	0	0	0	0	0	1	1	1	6	
11:45	0	3	0	3	0	5	0	5	8	0	0	0	0	0	0	0	0	0	8	
12:00	0	3	0	3	0	7	0	7	10	0	0	0	0	0	0	0	0	0	10	
12:15	0	3	0	3	0	4	0	4	7	11	1	0	2	3	0	1	0	1	4	15
12:30	0	5	0	5	0	3	0	3	8	1	0	1	2	0	0	1	1	3	11	
12:45	0	1	0	1	3	1	5	6	0	0	0	0	0	0	0	1	1	7	7	
13:00	0	4	0	4	0	1	0	1	5	1	0	0	1	0	0	0	0	1	6	
13:15	0	4	0	4	0	3	0	3	7	0	1	0	1	0	0	0	0	1	8	
13:30	0	3	0	3	0	2	1	3	6	0	1	0	1	0	1	1	2	8		
13:45	0	6	0	6	0	2	1	3	9	0	0	0	0	0	0	0	0	0	9	
14:00	0	5	0	5	0	0	1	1	6	0	0	0	0	0	0	0	0	0	6	
14:15	0	2	0	2	0	2	1	3	5	0	0	0	0	0	0	0	0	0	5	
14:30	0	0	0	0	0	2	1	3	3	0	0	0	0	0	0	0	0	0	3	
14:45	0	3	0	3	1	2	1	4	7	0	0	0	0	0	0	0	0	0	7	
Total	3	129	2	134	6	129	14	149	283	16	3	6	25	1	2	8	11	36	319	



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**TENTH LINE RD @ LAKEPOINTE DR**

**Survey Date:** Thursday, April 19, 2018  
**Start Time:** 07:00

**WO No:** 37742  
**Device:** Miovision

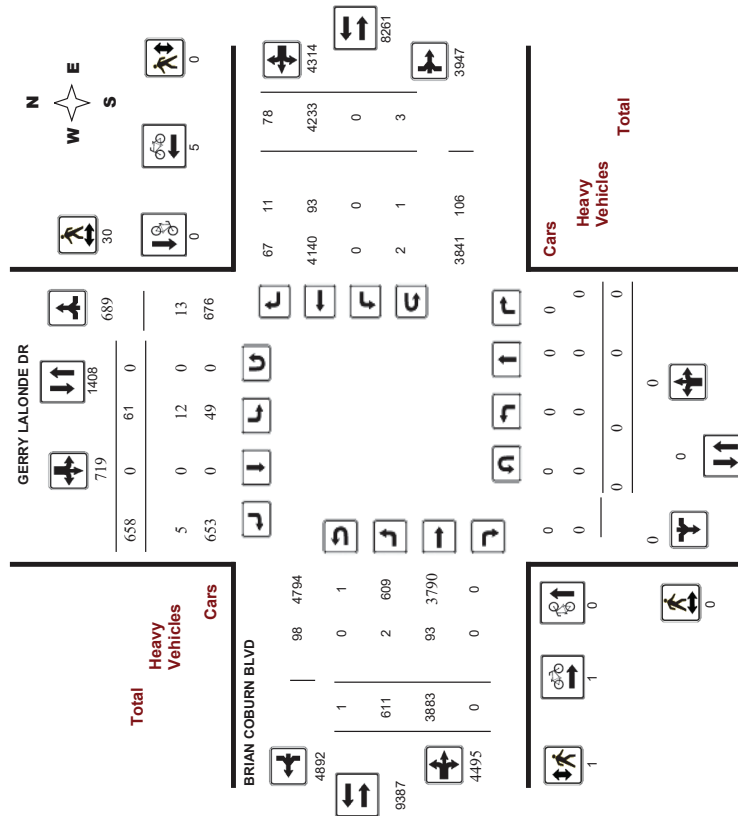
**Full Study 15 Minute U-Turn Total**

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn Total		U-Turn Total		U-Turn Total		U-Turn Total		
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	1	1	0	0	0	0	1
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	1	1
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	1	1
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
10:15	0	0	1	1	0	0	0	0	1
10:30	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0
11:00	0	0	1	1	0	0	0	0	1
11:15	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	1	1	0	0	0	0	1
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	1	1	0	0	0	0	1
12:45	0	0	0	0	0	0	0	0	0
13:00	1	1	0	0	0	0	0	0	1
13:15	0	0	1	1	0	0	0	0	1
13:30	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	1	1	0	0	0	0	1
17:15	0	0	1	1	0	0	0	0	1
17:30	0	0	0	0	0	0	0	0	0
17:45	1	1	0	0	0	0	0	0	1
18:00	0	0	0	0	0	0	0	0	0
18:15	0	0	0	0	0	0	0	0	0
18:30	0	0	0	0	0	0	0	0	0
18:45	0	0	0	0	0	0	0	0	0
Total	2	7	7	7	0	0	0	0	12

Survey Date: Wednesday, October 17, 2018  
 Start Time: 07:00

WO No: 38062  
 Device: Miovision

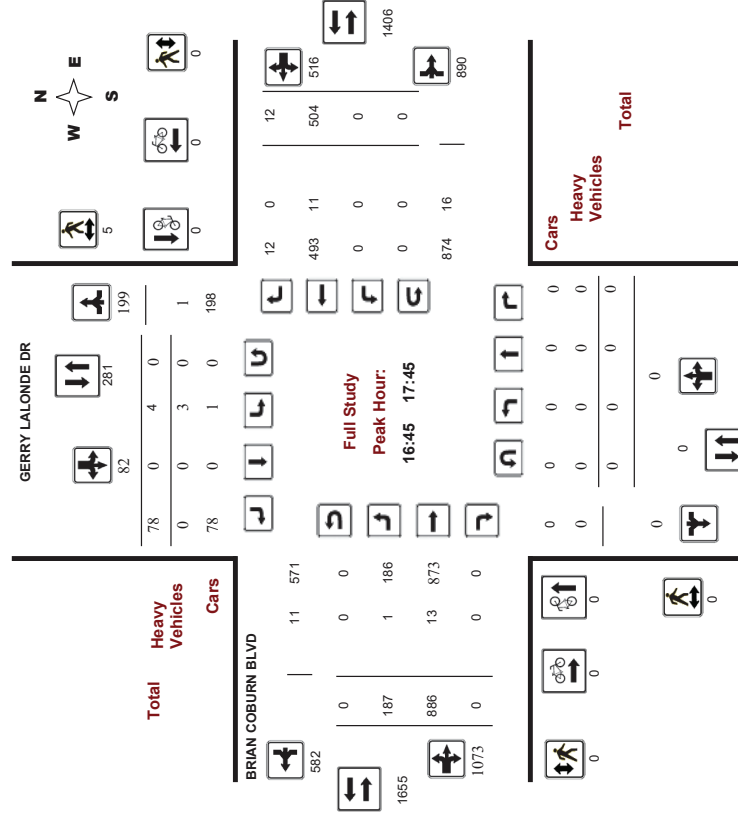
Full Study Diagram



Survey Date: Wednesday, October 17, 2018  
 Start Time: 07:00

WO No: 38062  
 Device: Miovision

Full Study Peak Hour Diagram





# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

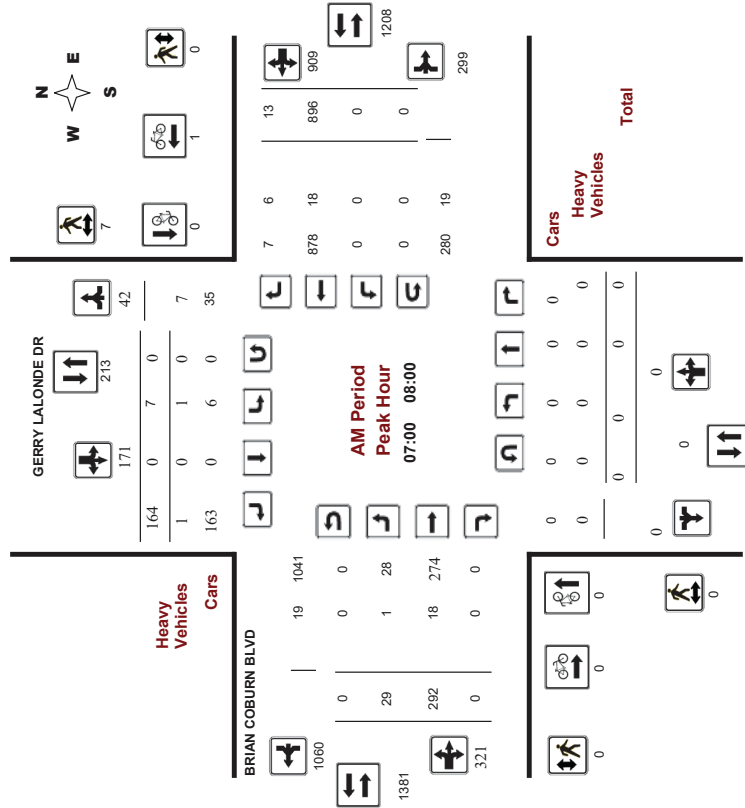
### BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018

WO No: 38062

Start Time: 07:00

Device: Miovision



Comments



# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

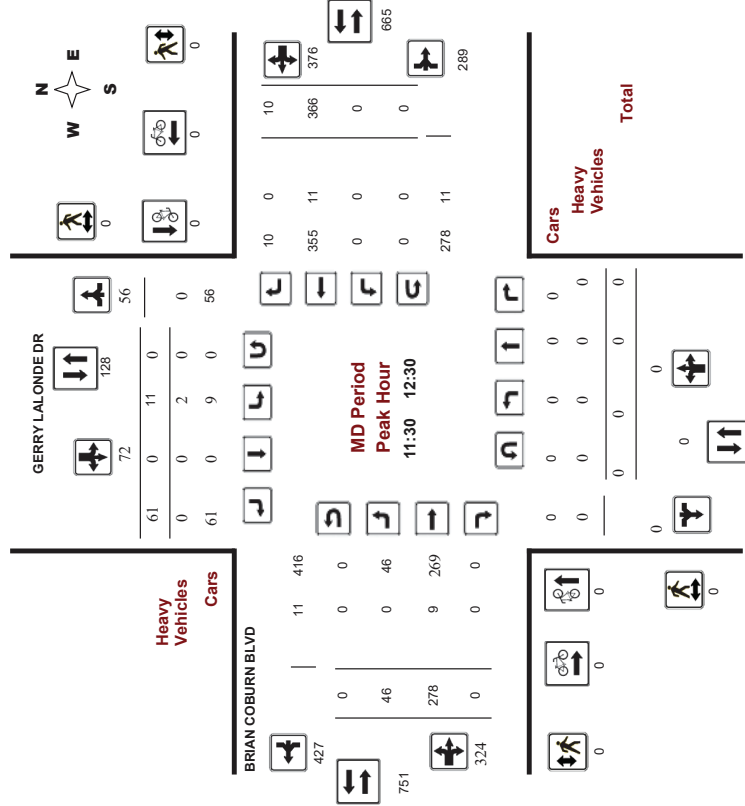
### BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018

WO No: 38062

Start Time: 07:00

Device: Miovision



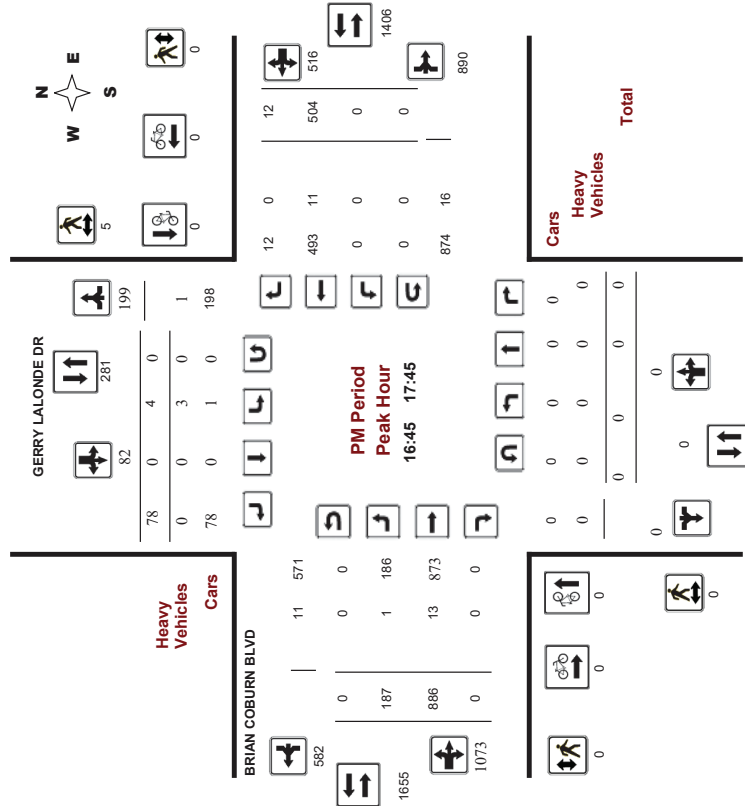
Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Peak Hour Diagram**  
**BRIAN COBURN BLVD @ GERRY LALONDE DR**

**Survey Date:** Wednesday, October 17, 2018  
**Start Time:** 07:00

**WO No:** 38062  
**Device:** Miovision



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ GERRY LALONDE DR**

**Survey Date:** Wednesday, October 17, 2018  
**Start Time:** 07:00

**WO No:** 38062  
**Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Wednesday, October 17, 2018  
**Total Observed U-Turns:** 90  
**AADT Factor:** 0.90  
 Northbound: 0 Southbound: 0  
 Eastbound: 1 Westbound: 3

Period	GERRY LALONDE DR				BRIAN COBURN BLVD				GERRY LALONDE DR				BRIAN COBURN BLVD			
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT
07:00-08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00-09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00-10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30-12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30-13:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00-16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00-17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00-18:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>Sub Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>U-Turns</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>EQ 12hr</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>AVG 12hr</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>AVG 24hr</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. 1.31  
 Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor. 0.9



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018  
Start Time: 07:00

WO No: 38062  
Device: Miovision

### Full Study 15 Minute Increments

GERRY LALONDE DR

Northbound

Southbound

Eastbound

Westbound

Time Period	Northbound			Southbound			Eastbound			Westbound			W	STR	Grand Total	
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT				RT
07:00	0	0	0	0	0	0	2	48	0	50	0	239	5	244	0	335
07:15	0	0	0	3	0	37	40	1	9	52	0	225	3	228	1	329
07:30	0	0	0	2	0	47	49	1	6	92	0	236	3	239	1	386
07:45	0	0	0	2	0	39	41	0	12	100	0	198	2	198	0	351
08:00	0	0	0	1	0	36	37	0	5	68	0	182	1	183	0	283
08:15	0	0	0	5	0	35	40	0	9	61	0	70	0	228	2	338
08:30	0	0	0	3	0	20	23	1	8	61	0	203	2	205	1	297
08:45	0	0	0	0	0	18	18	0	6	70	0	136	1	137	0	231
09:00	0	0	0	1	0	18	19	1	8	70	0	178	2	180	1	277
09:15	0	0	0	1	0	20	21	0	4	54	0	132	4	136	0	215
09:30	0	0	0	2	0	22	24	2	11	66	0	109	1	112	2	213
09:45	0	0	0	0	0	11	11	0	1	54	0	98	2	100	0	166
10:00	0	0	0	5	0	15	20	2	11	71	0	82	0	110	2	212
10:15	0	0	0	4	0	15	19	0	8	70	0	76	3	79	0	176
10:30	0	0	0	0	0	12	12	0	16	71	0	91	2	93	0	192
10:45	0	0	0	2	0	19	21	0	11	66	0	92	2	94	0	192
11:00	0	0	0	1	0	23	24	0	9	89	0	98	0	71	1	195
11:15	0	0	0	3	0	10	13	0	6	73	0	79	0	97	0	189
11:30	0	0	0	1	0	13	14	0	13	86	0	89	0	79	2	181
11:45	0	0	0	4	0	11	15	0	10	79	0	89	0	79	1	184
12:00	0	0	0	2	0	13	15	1	19	139	0	168	0	101	1	275
12:15	0	0	0	5	0	15	20	0	22	175	0	197	0	104	6	327
12:30	0	0	0	0	0	13	13	0	24	235	0	259	0	103	3	378
12:45	0	0	0	4	0	14	18	1	38	201	0	239	0	109	3	389
13:00	0	0	0	1	0	11	12	1	51	205	0	256	0	113	1	382
13:15	0	0	0	2	0	15	17	2	36	218	0	255	0	107	2	381
13:30	0	0	0	1	0	19	20	1	37	227	0	264	0	103	4	391
13:45	0	0	0	0	0	18	18	0	49	240	0	289	0	97	3	407
14:00	0	0	0	3	0	18	21	2	42	199	0	241	0	144	2	408
14:15	0	0	0	0	0	21	21	0	57	238	0	265	0	132	6	454
14:30	0	0	0	1	0	21	22	1	39	209	0	248	0	131	1	402
14:45	0	0	0	2	0	18	20	0	32	196	0	228	0	139	2	389
Total	0	0	0	61	0	658	719	17	611	3883	0	4495	0	4233	78	9,528

Note: U-Turns are included in Totals.



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018  
Start Time: 07:00

WO No: 38062  
Device: Miovision

### Full Study Cyclist Volume

GERRY LALONDE DR

Southbound

Street Total

Eastbound

Westbound

Street Total

Grand Total

Time Period	Southbound			Street Total			Eastbound			Westbound			Street Total			Grand Total
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018  
Start Time: 07:00

WO No: 38062  
Device: Miovision

### Full Study Pedestrian Volume

BRIAN COBURN BLVD

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		Total	Grand Total
	E or W	S or B	N or E	S or B		
07:00	0	1	0	0	1	1
07:15	0	2	0	0	2	2
07:30	0	3	0	0	3	3
07:45	0	1	0	0	1	1
08:00	0	5	0	0	5	5
08:15	0	0	0	0	0	0
08:30	0	0	0	0	0	0
08:45	0	0	1	0	1	1
09:00	0	0	0	0	0	0
09:15	0	2	0	0	2	2
09:30	0	0	0	0	0	0
09:45	0	0	0	0	0	0
10:00	0	0	0	0	0	0
10:15	0	0	0	0	0	0
10:30	0	0	0	0	0	0
10:45	0	0	0	0	0	0
11:00	0	0	0	0	0	0
11:15	0	0	0	0	0	0
11:30	0	0	0	0	0	0
11:45	0	0	0	0	0	0
12:00	0	0	0	0	0	0
12:15	0	0	0	0	0	0
12:30	0	0	0	0	0	0
12:45	0	1	0	0	1	1
13:00	0	0	0	0	0	0
13:15	0	0	0	0	0	0
13:30	0	0	0	0	0	0
13:45	0	0	0	0	0	0
14:00	0	0	0	0	0	0
14:15	0	0	0	0	0	0
14:30	0	1	0	0	1	1
14:45	0	0	0	0	0	0
15:00	0	0	0	0	0	0
15:15	0	0	0	0	0	0
15:30	0	0	0	0	0	0
15:45	0	0	0	0	0	0
16:00	0	2	0	0	2	2
16:15	0	0	0	0	0	0
16:30	0	5	0	0	5	5
16:45	0	2	0	0	2	2
17:00	0	1	0	0	1	1
17:15	0	1	0	0	1	1
17:30	0	1	0	0	1	1
17:45	0	1	0	0	1	1
17:59	0	2	0	0	2	2
<b>Total</b>	<b>0</b>	<b>30</b>	<b>1</b>	<b>0</b>	<b>31</b>	<b>31</b>



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018  
Start Time: 07:00

WO No: 38062  
Device: Miovision

### Full Study Heavy Vehicles

BRIAN COBURN BLVD

Time Period	Northbound			Southbound			Eastbound			Westbound			W STR TOT	STR TOT	Grand Total
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT			
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ GERRY LALONDE DR

Survey Date: Wednesday, October 17, 2018  
Start Time: 07:00

WO No: 38062  
Device: Miovision

### Full Study 15 Minute U-Turn Total

Time Period	GERRY LALONDE DR		BRIAN COBURN BLVD		Total
	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	
07:00	0	0	0	0	0
07:15	0	0	0	0	0
07:30	0	0	0	0	0
07:45	0	0	0	0	0
08:00	0	0	0	0	0
08:15	0	0	0	0	0
08:30	0	0	0	0	0
08:45	0	0	0	0	0
09:00	0	0	0	0	0
09:15	0	0	0	0	0
09:30	0	0	0	0	0
09:45	0	0	2	2	2
10:00	0	0	0	0	0
11:30	0	0	0	0	0
11:45	0	0	0	0	0
12:00	0	0	0	0	0
12:15	0	0	0	0	0
12:30	0	0	0	0	0
12:45	0	0	0	1	1
13:00	0	0	0	0	0
13:15	0	0	0	0	0
13:30	0	0	0	0	0
15:00	0	0	0	0	0
15:15	0	0	0	0	0
15:30	0	0	0	0	0
15:45	0	0	0	0	0
16:00	0	0	0	0	0
16:15	0	0	0	0	0
16:30	0	0	1	0	1
16:45	0	0	0	0	0
17:00	0	0	0	0	0
17:15	0	0	0	0	0
17:30	0	0	0	0	0
17:45	0	0	0	0	0
18:00	0	0	0	0	0
Total	0	0	1	3	4



# Transportation Services - Traffic Services

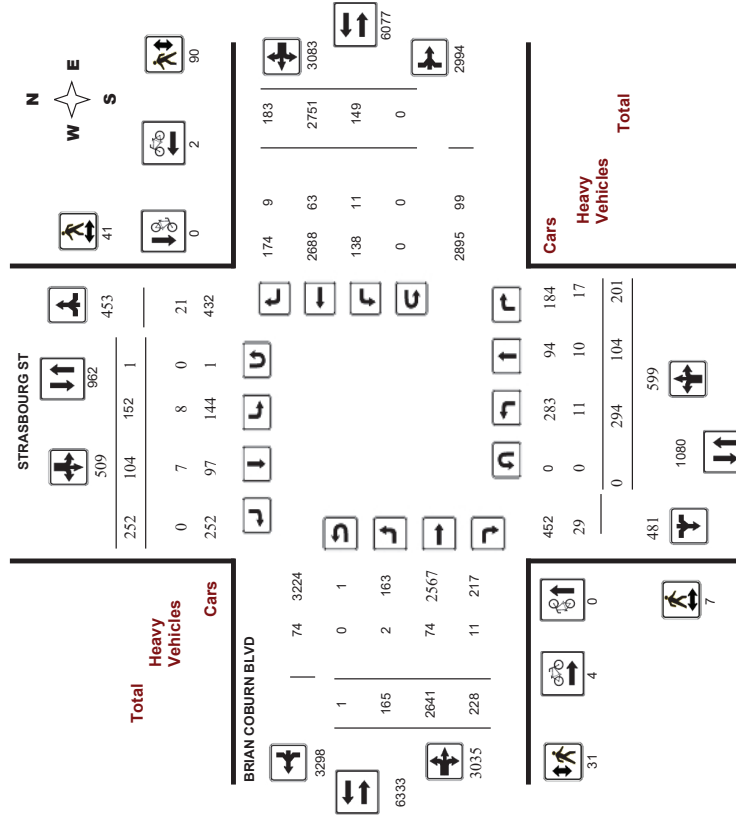
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ STRASBOURG ST

Survey Date: Thursday, April 20, 2017  
Start Time: 07:00

WO No: 36948  
Device: Miovision

### Full Study Diagram







# Transportation Services - Traffic Services

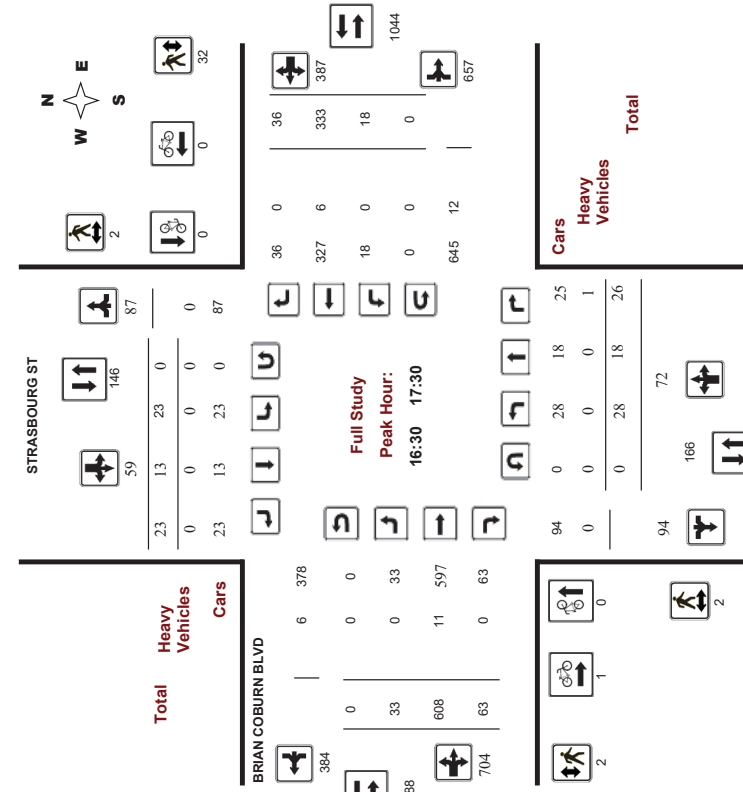
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ STRASBOURG ST

Survey Date: Thursday, April 20, 2017  
Start Time: 07:00

WO No: 36948  
Device: Miovision

#### Full Study Peak Hour Diagram



Comments



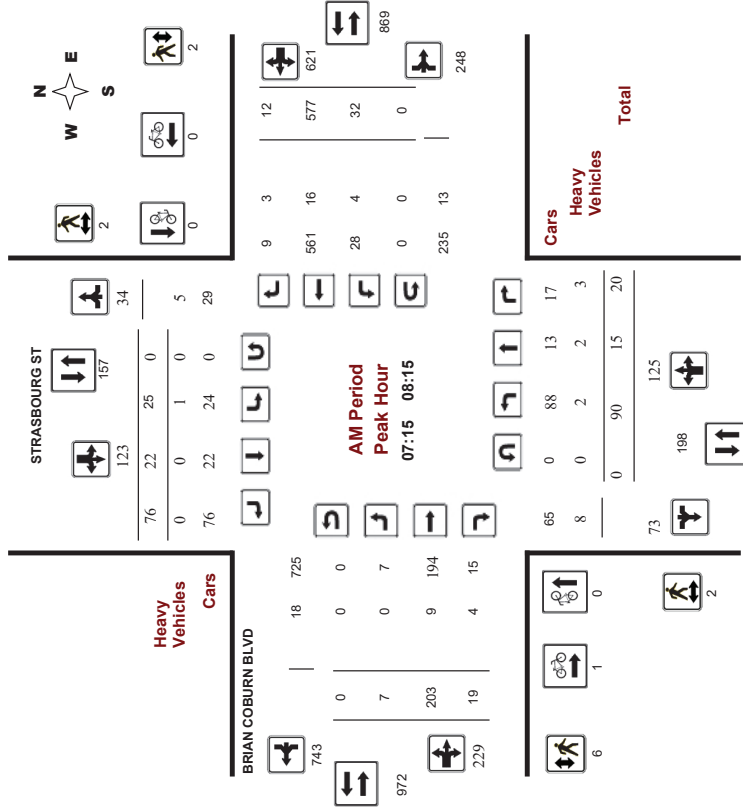
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### BRIAN COBURN BLVD @ STRASBOURG ST

Survey Date: Thursday, April 20, 2017  
Start Time: 07:00

WO No: 36948  
Device: Miovision

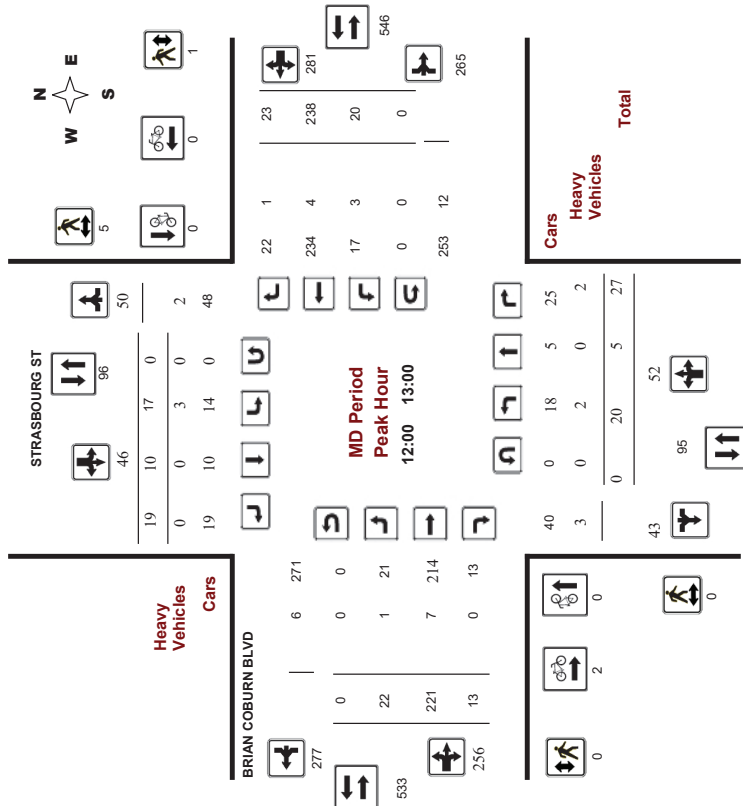




**Transportation Services - Traffic Services**  
**Turning Movement Count - Peak Hour Diagram**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

Survey Date: Thursday, April 20, 2017  
 Start Time: 07:00

WO No: 36948  
 Device: Miovision



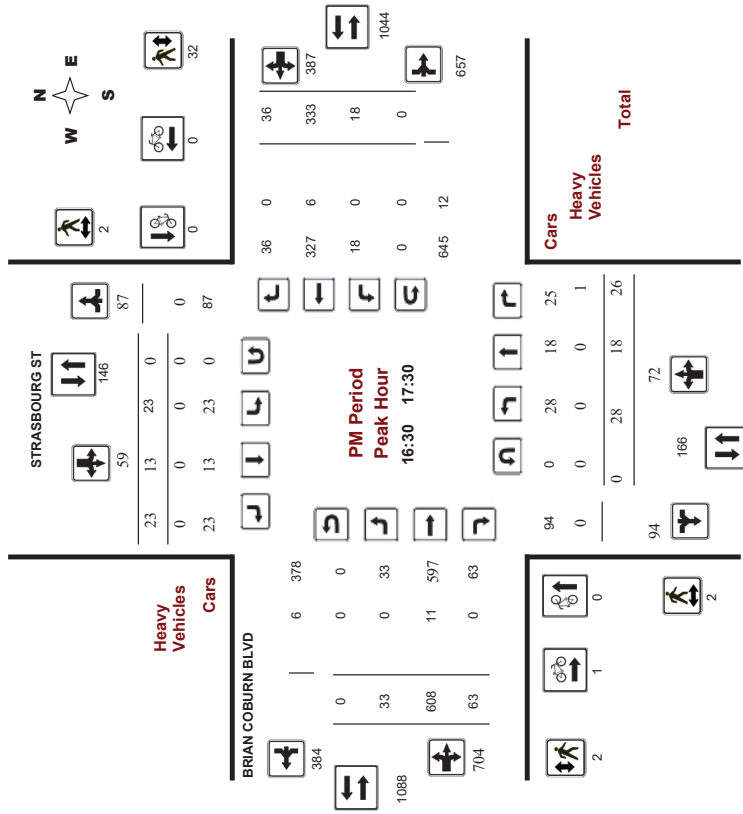
Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Peak Hour Diagram**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

Survey Date: Thursday, April 20, 2017  
 Start Time: 07:00

WO No: 36948  
 Device: Miovision



Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

**Survey Date:** Thursday, April 20, 2017 **WO No:** 36948  
**Start Time:** 07:00 **Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Thursday, April 20, 2017 **Total Observed U-Turns** **AAADT Factor**  
 Northbound: 0 Southbound: 1 Eastbound: 1 Westbound: 0 **90**

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	RT TOT	Grand Total
	LT	ST	RT	TOT	NB	LT	ST	RT	TOT	EB	LT	ST	RT	TOT	WB	STR				
07:00-08:00	101	15	20	136	23	22	74	119	255	4	172	17	183	32	565	14	611	804	1059	
08:00-09:00	56	18	22	96	18	27	46	91	187	11	186	19	216	21	525	9	555	771	958	
09:00-10:00	20	7	23	50	11	8	30	49	99	8	179	10	197	20	274	10	304	501	600	
11:30-12:30	21	7	32	60	17	12	18	47	107	19	220	12	251	21	220	23	264	515	622	
12:30-13:30	14	9	18	41	16	8	21	45	86	23	200	28	251	13	214	20	247	498	584	
15:00-16:00	26	13	40	79	25	8	18	51	130	32	483	34	549	10	273	29	312	861	991	
16:00-17:00	19	20	21	60	26	11	22	59	119	34	598	65	697	16	335	40	391	1088	1207	
17:00-18:00	37	15	25	77	16	8	23	47	124	34	603	43	680	16	345	38	399	1079	1203	
<b>Sub Total</b>	<b>294</b>	<b>104</b>	<b>201</b>	<b>599</b>	<b>152</b>	<b>104</b>	<b>252</b>	<b>508</b>	<b>1107</b>	<b>165</b>	<b>2641</b>	<b>228</b>	<b>3034</b>	<b>149</b>	<b>2751</b>	<b>183</b>	<b>3083</b>	<b>6117</b>	<b>7224</b>	
<b>U-Turns</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>2</b>	
<b>Total</b>	<b>294</b>	<b>104</b>	<b>201</b>	<b>599</b>	<b>153</b>	<b>104</b>	<b>252</b>	<b>509</b>	<b>1108</b>	<b>166</b>	<b>2641</b>	<b>228</b>	<b>3035</b>	<b>149</b>	<b>2751</b>	<b>183</b>	<b>3083</b>	<b>6118</b>	<b>7226</b>	
<b>EQ 12hr</b>	<b>409</b>	<b>145</b>	<b>279</b>	<b>833</b>	<b>213</b>	<b>145</b>	<b>350</b>	<b>708</b>	<b>1541</b>	<b>231</b>	<b>3671</b>	<b>317</b>	<b>4219</b>	<b>207</b>	<b>3824</b>	<b>254</b>	<b>4285</b>	<b>8504</b>	<b>10045</b>	

Note: These values are calculated by multiplying the totals by the appropriate expansion factor. **1.39**  
**AVG 12hr** 368 130 251 749 192 130 315 637 1386 208 3304 285 3797 186 3442 229 3857 7654 9040  
 Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor. **.90**  
**AVG 24hr** 482 170 329 881 252 170 413 835 1816 272 4328 373 4973 244 4509 300 5053 10026 11842  
 Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor. **1.31**  
 Note: U-Turns provided for approach totals. Refer to "U-Turn" Report for specific breakdown.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

**Survey Date:** Thursday, April 20, 2017 **WO No:** 36948  
**Start Time:** 07:00 **Device:** Miovision

**Full Study 15 Minute Increments**

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	RT TOT	Grand Total
	LT	ST	RT	TOT	N	LT	ST	RT	TOT	S	STR	LT	ST	RT	TOT	E				
07:00	28	3	5	37	3	6	15	24	61	0	21	4	25	10	135	2	147	172	233	
07:15	07:30	29	2	5	36	9	6	22	37	73	2	39	4	45	7	152	5	164	209	287
07:30	07:45	24	5	5	34	6	6	22	34	68	0	52	6	58	7	142	2	151	209	277
07:45	08:00	19	5	5	29	5	4	15	24	53	2	60	3	65	8	138	5	149	214	267
08:00	08:15	18	3	5	26	5	6	17	28	54	3	52	6	61	10	147	0	157	218	272
08:15	08:30	23	5	4	32	7	13	13	33	65	3	48	1	52	4	141	2	147	199	264
08:30	08:45	11	8	5	24	3	7	7	17	41	2	52	8	62	3	139	3	145	207	248
08:45	09:00	4	2	8	14	3	1	9	13	27	3	34	4	41	4	98	4	106	147	174
09:00	09:15	6	1	9	16	2	3	11	16	32	1	49	5	55	6	78	2	86	141	173
09:15	09:30	6	4	3	13	1	3	6	10	23	2	44	2	48	4	60	5	69	117	140
09:30	09:45	4	1	4	9	5	2	6	13	22	2	34	1	37	7	75	2	84	121	143
09:45	10:00	4	1	7	12	3	0	7	10	22	3	52	2	57	3	61	1	65	122	144
11:30	11:45	5	3	7	15	2	3	7	12	27	4	54	3	61	3	59	4	66	127	154
11:45	12:00	5	2	9	16	6	4	6	16	32	5	57	5	67	5	44	6	55	122	154
12:00	12:15	7	1	9	17	4	3	3	10	27	8	55	2	65	8	60	5	73	138	165
12:15	12:30	4	1	7	12	5	2	2	9	21	2	54	2	58	5	57	8	70	128	149
12:30	12:45	6	0	7	13	4	1	6	11	24	3	47	3	53	4	55	4	63	116	140
12:45	13:00	3	3	4	10	4	4	8	16	26	9	65	6	80	3	66	6	75	155	181
13:00	13:15	2	1	4	7	5	1	2	8	15	3	44	4	49	7	60	116	131	131	
13:15	13:30	3	5	3	11	3	2	5	10	21	8	44	10	62	2	44	3	49	111	132
15:00	15:15	9	4	9	22	3	2	1	6	28	7	89	5	101	2	59	2	63	164	192
15:15	15:30	3	3	12	18	9	4	7	20	38	9	127	10	146	3	75	6	84	230	268
15:30	15:45	9	4	15	28	7	0	3	10	38	7	119	9	135	3	60	12	75	210	248
15:45	16:00	5	2	4	11	7	2	7	16	27	9	148	10	167	2	79	9	90	257	284
16:00	16:15	2	3	6	11	3	2	5	10	21	13	149	12	174	4	89	13	106	280	301
16:15	16:30	6	6	7	19	5	1	5	11	30	6	151	15	172	3	82	11	96	268	298
16:30	16:45	5	6	7	18	6	0	7	13	31	12	151	15	178	2	84	5	91	269	300
16:45	17:00	6	5	1	12	12	8	5	25	37	4	147	23	174	7	80	11	98	272	309
17:00	17:15	11	5	10	26	3	4	8	15	41	9	154	9	172	7	82	12	101	273	314
17:15	17:30	6	2	8	16	2	1	3	6	22	8	156	16	180	2	87	8	97	277	299
17:30	17:45	13	6	5	24	6	1	5	12	36	8	150	7	165	4	82	6	92	257	283
17:45	18:00	7	2	2	11	5	2	7	14	25	9	143	11	163	3	94	12	109	272	297
<b>Total:</b>		<b>294</b>	<b>104</b>	<b>201</b>	<b>599</b>	<b>153</b>	<b>104</b>	<b>252</b>	<b>509</b>	<b>1108</b>	<b>166</b>	<b>2641</b>	<b>228</b>	<b>3035</b>	<b>149</b>	<b>2751</b>	<b>183</b>	<b>3083</b>	<b>6118</b>	<b>7226</b>

Note: U-Turns are included in Totals.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

**Survey Date:** Thursday, April 20, 2017  
**Start Time:** 07:00

**WO No:** 36948  
**Device:** Miovision

**Full Study Cyclist Volume**

STRASBOURG ST      BRIAN COBURN BLVD

Time Period	STRASBOURG ST		BRIAN COBURN BLVD		Street Total	Grand Total
	Northbound	Southbound	Eastbound	Westbound		
07:00 07:15	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0
08:00 08:15	0	0	1	0	1	1
08:15 08:30	0	0	0	0	0	0
08:30 08:45	0	0	0	0	0	0
08:45 09:00	0	0	0	0	0	0
09:00 09:15	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0
10:00 10:15	0	0	0	0	0	0
10:15 10:30	0	0	0	0	0	0
10:30 10:45	0	0	0	0	0	0
10:45 11:00	0	0	0	0	0	0
11:00 11:15	0	0	0	0	0	0
11:15 11:30	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0
12:15 12:30	0	0	1	0	1	1
12:30 12:45	0	0	1	0	1	1
12:45 13:00	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0
13:30 13:45	0	0	0	0	0	0
13:45 14:00	0	0	0	0	0	0
14:00 14:15	0	0	0	0	0	0
14:15 14:30	0	0	0	0	0	0
14:30 14:45	0	0	0	0	0	0
14:45 15:00	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0
15:15 15:30	0	0	0	0	0	0
15:30 15:45	0	0	0	0	0	0
15:45 16:00	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0
16:30 16:45	0	0	0	0	0	0
16:45 17:00	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0
17:15 17:30	0	0	0	0	0	0
17:30 17:45	0	0	0	0	0	0
17:45 18:00	0	0	0	0	0	0
<b>Total</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>2</b>	<b>6</b>	<b>6</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ STRASBOURG ST**

**Survey Date:** Thursday, April 20, 2017  
**Start Time:** 07:00

**WO No:** 36948  
**Device:** Miovision

**Full Study Pedestrian Volume**

STRASBOURG ST      BRIAN COBURN BLVD

Time Period	STRASBOURG ST		BRIAN COBURN BLVD		Total	Grand Total
	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)		
07:00 07:15	0	1	1	2	3	4
07:15 07:30	0	1	1	0	1	2
07:30 07:45	0	0	0	1	1	2
07:45 08:00	1	0	1	0	2	3
08:00 08:15	1	1	2	1	3	5
08:15 08:30	0	0	0	1	1	4
08:30 08:45	0	2	2	2	5	7
08:45 09:00	0	0	0	0	0	1
09:00 09:15	0	1	1	0	2	1
09:15 09:30	0	0	0	0	0	3
09:30 09:45	0	0	0	1	1	2
09:45 10:00	0	0	0	1	1	1
10:00 10:15	0	0	0	0	0	1
10:15 10:30	0	1	1	2	3	3
10:30 10:45	0	0	0	0	0	1
10:45 11:00	0	2	2	0	4	2
11:00 11:15	0	3	3	0	6	3
11:15 11:30	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0
12:15 12:30	0	2	2	0	4	2
12:30 12:45	0	3	3	0	6	3
12:45 13:00	0	0	0	0	0	0
13:00 13:15	0	0	0	0	0	0
13:15 13:30	0	2	2	0	4	1
13:30 13:45	0	7	7	0	14	2
13:45 14:00	0	8	8	0	16	9
14:00 14:15	0	1	1	0	2	10
14:15 14:30	0	1	1	0	2	3
14:30 14:45	0	1	1	0	2	6
14:45 15:00	0	1	1	0	2	5
15:00 15:15	0	5	5	1	11	4
15:15 15:30	0	1	1	0	2	9
15:30 15:45	0	0	0	0	0	4
15:45 16:00	0	0	0	0	0	4
16:00 16:15	0	0	0	0	0	4
16:15 16:30	0	1	1	0	2	9
16:30 16:45	0	1	1	0	2	8
16:45 17:00	1	0	0	0	1	7
17:00 17:15	1	1	2	0	4	16
17:15 17:30	0	0	0	2	2	6
17:30 17:45	0	1	1	2	4	24
17:45 18:00	3	2	5	3	13	18
<b>Total</b>	<b>7</b>	<b>41</b>	<b>48</b>	<b>31</b>	<b>121</b>	<b>169</b>



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ STRASBOURG ST

Survey Date: Thursday, April 20, 2017  
Start Time: 07:00

WO No: 36948  
Device: Miovision

#### Full Study Heavy Vehicles

##### STRASBOURG ST

Time Period	Northbound				Southbound				Eastbound				Westbound				W STR TOT	R STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	S	STR	RT	TOT	E	LT	ST	RT			
07:00	0	0	0	0	0	0	0	0	0	0	0	0	2	0	3	0	3	5	
07:15	0	2	3	1	0	0	0	2	0	2	0	4	0	5	1	6	8	12	
07:30	1	0	0	1	0	0	0	1	0	4	1	5	1	4	1	6	11	12	
07:45	0	1	0	1	0	0	0	1	0	2	1	3	2	4	1	7	10	11	
08:00	0	1	1	2	0	0	0	2	0	1	2	3	1	3	0	4	7	9	
08:15	0	1	1	2	1	2	0	3	5	0	0	0	0	3	1	4	4	9	
08:30	0	1	1	2	0	2	0	2	6	0	5	1	6	0	4	0	10	16	
08:45	0	0	0	0	0	0	0	0	0	0	1	1	0	1	0	1	2	2	
09:00	0	0	0	0	0	0	0	0	0	0	4	1	5	1	4	0	10	10	
09:15	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2	2	
09:30	0	0	1	1	0	0	0	1	0	1	0	1	0	6	0	6	7	8	
09:45	0	0	1	1	0	0	0	0	0	4	0	4	0	1	0	1	5	7	
10:00	0	0	1	1	0	0	0	0	0	1	0	1	1	1	1	3	4	5	
11:30	1	0	1	2	0	0	0	2	0	1	1	2	0	0	0	0	2	4	
11:45	1	0	1	1	1	0	0	1	1	2	0	2	0	2	0	5	7	9	
12:00	1	0	1	1	1	0	0	1	2	0	2	0	3	1	0	1	2	5	8
12:15	1	0	1	2	1	0	0	1	3	1	2	0	3	1	0	1	2	5	8
12:30	0	0	0	0	0	0	0	0	0	0	2	0	2	0	1	0	1	3	4
12:45	0	0	1	1	0	0	0	1	0	1	0	1	0	0	0	0	1	2	3
13:00	0	1	1	2	0	0	0	1	0	1	0	1	0	0	0	0	1	2	3
13:15	1	0	1	2	0	0	0	1	3	0	0	0	0	0	0	0	0	3	3
13:30	0	2	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	3	6
13:45	1	2	0	3	0	0	0	0	3	0	3	0	3	0	0	0	3	6	6
14:00	0	1	1	2	0	2	0	2	4	1	5	0	6	1	4	0	5	11	15
14:15	0	1	2	1	0	0	0	1	3	0	2	1	3	0	3	0	3	6	9
14:30	0	0	1	0	0	0	0	0	1	0	2	1	3	1	1	0	2	5	6
14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	1	0	0	1	0	0	0	1	1	0	2	0	2	0	2	0	4	6	7
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	1	0	0	1	0	0	0	1	1	0	2	0	2	0	2	0	4	6	7
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	1	0	2	1	0	0	0	1	3	0	4	0	4	0	1	0	1	5	8
16:45	0	0	1	1	0	0	0	0	1	0	2	0	2	0	2	0	2	4	5
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	11	10	17	38	8	7	0	15	53	2	74	11	87	11	63	9	83	170	223



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ STRASBOURG ST

Survey Date: Thursday, April 20, 2017  
Start Time: 07:00

WO No: 36948  
Device: Miovision

#### Full Study 15 Minute U-Turn Total

##### STRASBOURG ST

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
15:00	0	1	0	0	0	0	0	0	1
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	1	0	1
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	1	1	2



# Transportation Services - Traffic Services

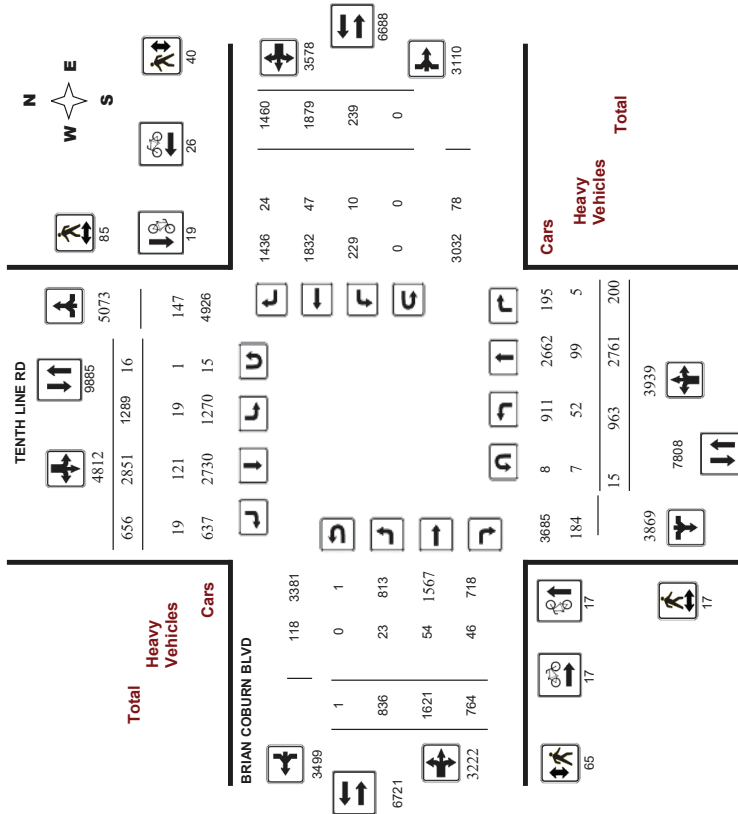
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Wednesday, September 19, 2018  
Start Time: 07:00

WO No: 38045  
Device: Miovision

#### Full Study Diagram



# Transportation Services - Traffic Services

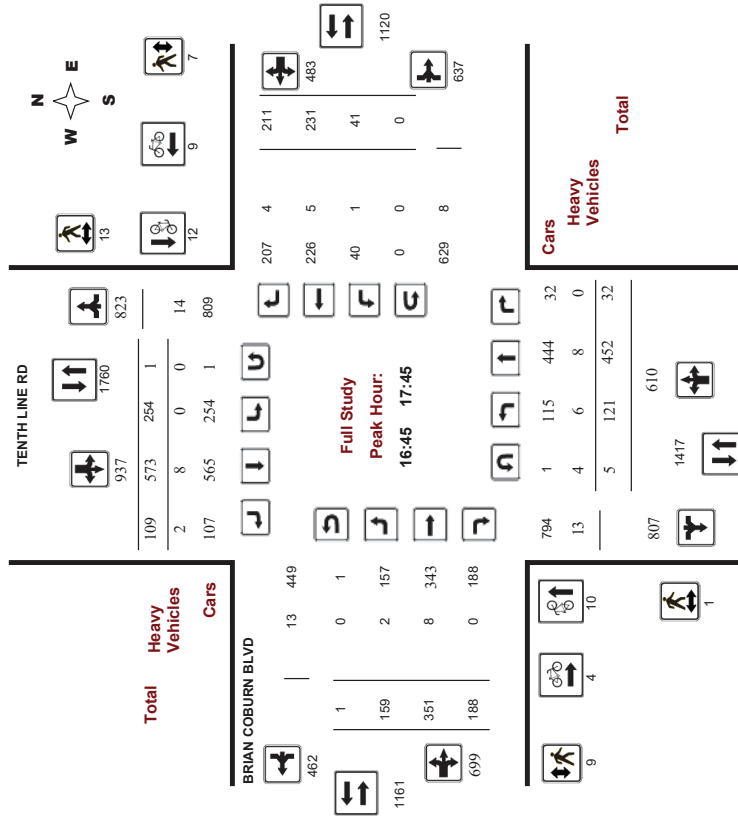
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Wednesday, September 19, 2018  
Start Time: 07:00

WO No: 38045  
Device: Miovision

#### Full Study Peak Hour Diagram





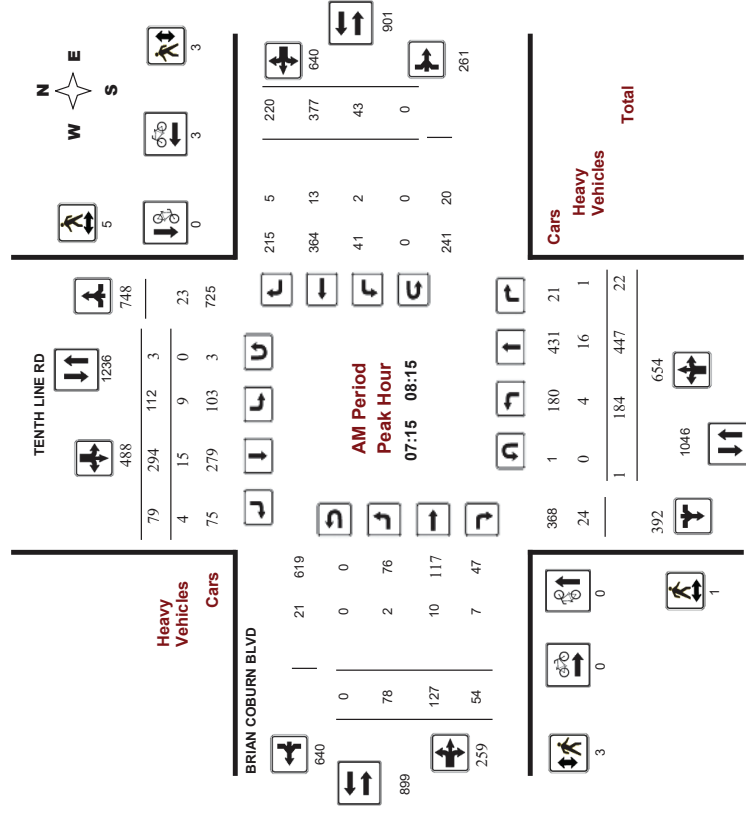
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Wednesday, September 19, 2018  
 Start Time: 07:00

WO No: 38045  
 Device: Miovision



Comments

Comments



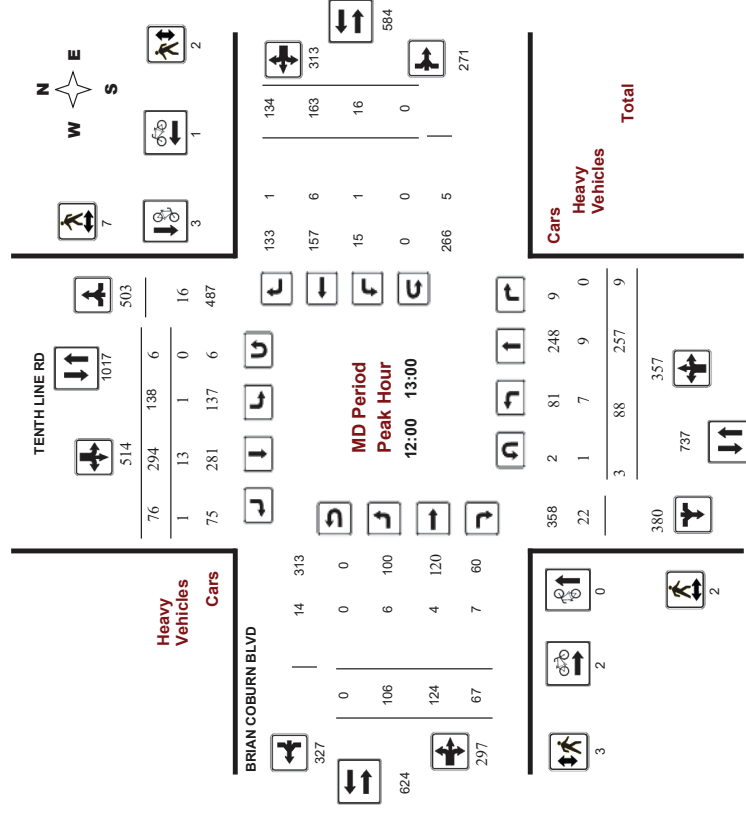
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### BRIAN COBURN BLVD @ TENTH LINE RD

Survey Date: Wednesday, September 19, 2018  
 Start Time: 07:00

WO No: 38045  
 Device: Miovision



Comments

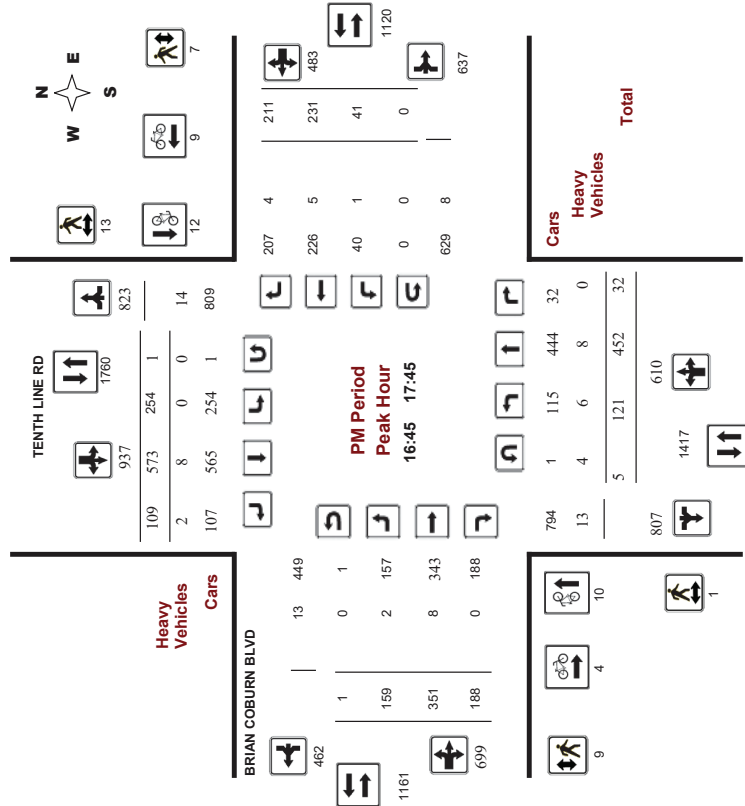
Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Peak Hour Diagram**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018  
**Start Time:** 07:00

**WO No:** 38045  
**Device:** Miovision



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018  
**Start Time:** 07:00

**WO No:** 38045  
**Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Wednesday, September 19, 2018  
**Total Observed U-Turns:** 1.00  
**Southbound:** 1.5  
**Eastbound:** 1  
**Westbound:** 0

Period	Northbound			Southbound			Eastbound			Westbound			WB TOT	STR TOT	Grand Total				
	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	LT	ST	RT	EB TOT				LT	ST	RT	
07:00-08:00	208	424	13	645	102	288	77	467	1112	67	129	56	252	41	370	233	644	896	2008
08:00-09:00	171	401	34	606	102	259	85	446	1052	94	119	44	257	35	330	206	571	828	1880
09:00-10:00	106	283	15	404	92	202	60	354	798	90	95	39	224	19	232	179	430	654	1412
11:30-12:30	77	289	9	355	120	283	66	489	824	101	128	67	296	11	156	122	289	585	1409
12:30-13:30	90	233	12	335	161	288	70	519	854	75	136	61	272	19	146	123	288	560	1414
15:00-16:00	98	322	47	467	210	409	100	719	1186	94	308	129	531	28	197	160	385	916	2102
16:00-17:00	90	388	28	506	257	581	86	924	1430	136	349	196	681	45	222	206	473	1154	2584
17:00-18:00	123	441	42	606	245	541	112	898	1584	179	357	172	708	41	226	231	488	1206	2710
<b>Sub Total</b>	<b>963</b>	<b>2761</b>	<b>200</b>	<b>3924</b>	<b>1289</b>	<b>2851</b>	<b>656</b>	<b>4796</b>	<b>8720</b>	<b>836</b>	<b>1621</b>	<b>764</b>	<b>3221</b>	<b>239</b>	<b>1879</b>	<b>1460</b>	<b>3578</b>	<b>6799</b>	<b>15519</b>
<b>U-Turns</b>	<b>15</b>	<b>200</b>	<b>200</b>	<b>3939</b>	<b>1289</b>	<b>2851</b>	<b>656</b>	<b>4812</b>	<b>8751</b>	<b>836</b>	<b>1621</b>	<b>764</b>	<b>3222</b>	<b>239</b>	<b>1879</b>	<b>1460</b>	<b>3578</b>	<b>6800</b>	<b>15551</b>
<b>Total</b>	<b>963</b>	<b>2761</b>	<b>200</b>	<b>3939</b>	<b>1289</b>	<b>2851</b>	<b>656</b>	<b>4812</b>	<b>8751</b>	<b>836</b>	<b>1621</b>	<b>764</b>	<b>3222</b>	<b>239</b>	<b>1879</b>	<b>1460</b>	<b>3578</b>	<b>6800</b>	<b>15551</b>
<b>EQ 12hr</b>	<b>1339</b>	<b>3838</b>	<b>278</b>	<b>5475</b>	<b>1792</b>	<b>3963</b>	<b>912</b>	<b>6889</b>	<b>12164</b>	<b>1162</b>	<b>2253</b>	<b>1062</b>	<b>4479</b>	<b>332</b>	<b>2612</b>	<b>2029</b>	<b>4973</b>	<b>9452</b>	<b>21616</b>

Note: These values are calculated by multiplying the totals by the appropriate expansion factor: **1.39**

**AVG 12hr** 1262 3617 262 5160 1689 3735 859 6304 12164 1095 2124 1001 4221 313 2461 1913 4687 9452 21616

Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor: **1**

**AVG 24hr** 1653 4738 343 6760 2212 4883 1126 8258 15018 1435 2782 1311 5529 410 3225 2506 6140 11689 26687

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor: **1.31**

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.





**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018  
**Start Time:** 07:00

**WO No:** 38045  
**Device:** Miovision

**Full Study 15 Minute Increments**

BRIAN COBURN BLVD

Time Period	Northbound				Southbound				Eastbound				Westbound				Grand Total		
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00	75	95	2	172	18	70	18	106	13	11	23	17	51	11	93	52	156	13	485
07:15	52	94	1	147	23	85	19	128	8	12	28	16	66	14	95	63	172	8	504
07:30	38	106	8	153	27	74	19	121	14	21	34	10	65	5	101	56	162	14	501
07:45	43	129	2	174	34	59	21	114	14	23	44	13	80	11	81	62	154	14	522
08:00	51	118	11	180	28	76	20	124	13	22	21	15	58	13	100	39	152	13	514
08:15	42	98	11	151	27	61	18	106	15	28	32	10	70	6	90	61	157	15	484
08:30	45	103	7	155	26	74	23	123	18	21	34	6	61	10	80	65	155	18	484
08:45	33	82	5	120	21	48	24	94	14	23	32	13	68	6	80	41	107	14	389
09:00	34	74	4	112	23	48	12	83	8	27	27	4	58	8	77	47	132	8	385
09:15	32	80	8	121	17	59	14	90	12	18	21	9	48	6	68	49	123	12	382
09:30	23	67	1	92	21	54	18	95	13	25	28	10	63	2	53	34	88	13	339
09:45	17	62	2	82	31	41	16	88	13	20	19	16	55	3	34	49	86	13	311
11:30	22	53	1	76	35	72	14	121	8	19	36	16	71	2	37	24	63	8	331
11:45	11	79	4	94	27	66	15	108	12	21	32	14	67	2	32	32	66	12	385
12:00	25	79	2	106	36	75	17	128	7	36	30	21	87	3	45	35	83	7	404
12:15	19	58	2	81	22	70	20	114	9	25	30	16	71	4	42	31	77	9	343
12:30	23	55	3	82	41	81	19	143	10	24	31	16	71	3	44	31	78	10	374
12:45	21	65	2	88	39	68	20	129	6	21	33	14	68	6	32	37	75	6	380
13:00	21	57	3	81	41	70	19	130	6	10	34	15	59	7	34	28	69	6	339
13:15	25	56	4	85	40	69	12	122	8	20	38	16	74	3	36	27	66	8	347
15:00	23	70	13	106	43	85	29	157	13	19	58	29	106	11	43	39	93	13	462
15:15	16	82	19	117	54	98	30	182	11	25	72	27	124	3	48	36	87	11	510
15:30	33	71	4	109	51	107	12	170	11	23	93	36	152	8	47	49	104	11	535
15:45	28	99	11	136	62	119	28	210	8	27	85	37	149	6	59	36	101	8	586
16:00	16	77	3	97	49	147	25	222	9	28	83	48	159	13	59	54	126	9	604
16:15	22	113	11	146	69	142	12	224	6	31	102	39	172	10	50	57	117	6	659
16:30	23	89	8	121	69	148	27	244	8	42	79	55	176	15	53	43	111	8	682
16:45	29	109	6	146	70	144	22	236	8	35	85	54	174	7	60	52	119	8	675
17:00	21	134	11	169	67	136	28	231	6	52	86	38	176	12	53	61	126	6	702
17:15	35	111	7	153	63	161	27	252	9	34	95	42	172	7	49	50	106	9	683
17:30	36	98	8	142	54	132	32	218	5	38	85	54	177	15	69	48	132	5	669
17:45	31	98	16	145	61	112	25	198	8	55	91	38	184	7	55	72	134	8	661
Total:	963	2761	200	3539	1289	2651	656	4812	323	836	1621	764	3222	239	1879	1460	3578	323	15,551

Note: U-Turns are included in Totals.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018  
**Start Time:** 07:00

**WO No:** 38045  
**Device:** Miovision

**Full Study Cyclist Volume**

BRIAN COBURN BLVD

Time Period	Northbound		Southbound		Street Total		Eastbound		Westbound		Street Total		Grand Total
	LT	ST	LT	ST	LT	ST	LT	ST	LT	ST	LT	ST	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15	0	0	0	0	0	0	0	0	0	0	0	0	3
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	1	0	0	0	1	0	1	0	0	0	0	1	2
09:15	0	0	0	0	0	0	0	0	0	0	0	0	1
09:30	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45	1	0	0	0	1	0	1	0	0	0	0	1	3
09:45	1	0	0	0	1	0	1	0	0	0	0	1	2
11:30	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30	1	0	0	0	1	0	1	0	0	0	0	1	1
15:00	0	0	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	6	0	0	0	6	0	6	0	0	0	0	6	6
17:15	2	0	0	0	2	0	2	0	0	0	0	2	2
17:30	4	0	0	0	4	0	4	0	0	0	0	4	4
17:45	3	0	0	0	3	0	3	0	0	0	0	3	3
Total	17	0	0	0	17	0	17	0	0	0	0	17	43



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018      **WO No:** 38045  
**Start Time:** 07:00      **Device:** Miovision

**Full Study Pedestrian Volume**  
**BRIAN COBURN BLVD**

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)	Total	Grand Total
	NB	SB	EB	WB			
07:00 07:15	1	0	0	0	2	2	3
07:15 07:30	0	1	0	0	2	2	3
07:30 07:45	0	2	1	1	2	2	4
07:45 08:00	0	1	0	0	0	0	1
08:00 08:15	1	1	2	0	2	2	4
08:15 08:30	0	2	2	2	1	3	5
08:30 08:45	1	1	2	2	1	3	5
08:45 09:00	0	2	5	3	3	8	10
09:00 09:15	2	1	5	0	0	1	8
09:15 09:30	0	2	4	2	2	6	8
09:30 09:45	0	3	4	4	0	4	7
09:45 10:00	1	3	4	0	2	2	6
11:30 11:45	0	1	1	0	0	2	3
11:45 12:00	1	3	4	2	2	6	10
12:00 12:15	0	1	0	0	0	0	1
12:15 12:30	0	4	4	0	1	1	5
12:30 12:45	2	1	3	3	0	3	6
12:45 13:00	0	1	1	1	1	1	2
13:00 13:15	0	6	0	0	0	0	6
13:15 13:30	1	4	5	0	0	0	5
15:00 15:15	0	1	1	4	2	6	7
15:15 15:30	1	3	4	2	0	2	6
15:30 15:45	0	2	2	1	1	2	4
15:45 16:00	1	3	7	4	4	11	15
16:00 16:15	2	8	4	2	2	6	16
16:15 16:30	0	5	6	1	1	7	12
16:30 16:45	2	4	6	2	4	6	12
16:45 17:00	1	5	3	3	3	6	12
17:00 17:15	0	6	2	1	2	3	9
17:15 17:30	0	2	2	2	2	3	5
17:30 17:45	0	0	3	0	1	4	4
17:45 18:00	0	2	2	1	1	1	3
Total	17	85	102	65	40	105	207



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ TENTH LINE RD**

**Survey Date:** Wednesday, September 19, 2018      **WO No:** 38045  
**Start Time:** 07:00      **Device:** Miovision

**Full Study Heavy Vehicles**  
**BRIAN COBURN BLVD**

Time Period	Northbound			Southbound			Eastbound			Westbound			W STR TOT	STR TOT	Grand Total	
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT				
07:00 07:15	3	6	0	9	1	3	4	13	1	1	4	6	0	3	9	22
07:15 07:30	0	1	0	1	4	2	1	7	8	0	0	1	1	2	4	13
07:30 07:45	1	3	0	4	2	6	2	10	14	0	3	1	4	0	7	11
07:45 08:00	3	7	1	11	2	1	0	3	14	1	5	1	7	0	2	24
08:00 08:15	0	5	0	5	1	6	1	8	13	1	2	4	7	1	3	26
08:15 08:30	0	4	0	4	4	4	1	11	15	2	1	1	4	2	0	22
08:30 08:45	3	8	0	11	0	7	0	7	18	0	2	0	2	0	3	23
08:45 09:00	5	2	0	7	1	4	2	7	14	2	2	5	9	1	0	25
09:00 09:15	3	2	0	5	0	3	0	3	8	1	2	0	3	0	3	17
09:15 09:30	2	4	0	6	0	6	0	6	12	0	3	0	3	0	3	18
09:30 09:45	3	5	0	8	0	5	0	5	13	2	2	1	5	0	1	19
09:45 10:00	1	4	0	6	0	6	1	7	13	1	0	4	5	0	0	18
11:30 11:45	2	2	0	4	0	2	2	4	8	0	1	1	2	0	2	12
11:45 12:00	1	5	1	7	0	5	0	5	12	1	1	2	4	0	0	16
12:00 12:15	2	2	0	4	0	3	0	3	7	3	1	3	7	0	1	16
12:15 12:30	2	2	0	4	1	4	0	5	9	1	1	1	3	1	2	15
12:30 12:45	2	2	0	5	0	4	1	5	10	2	2	2	6	0	2	18
12:45 13:00	1	3	0	4	0	2	0	2	6	0	0	1	1	0	1	8
13:00 13:15	1	2	0	3	1	1	1	3	6	1	2	4	0	1	0	11
13:15 13:30	2	1	0	3	0	3	1	5	8	0	0	3	3	1	0	12
15:00 15:15	2	5	0	7	0	6	0	6	13	0	1	4	5	1	1	21
15:15 15:30	1	2	0	3	0	6	2	8	11	0	1	0	1	0	0	12
15:30 15:45	4	4	0	8	0	3	0	3	11	0	0	2	2	0	2	15
15:45 16:00	1	2	0	3	0	5	0	5	8	0	0	0	0	0	3	11
16:00 16:15	0	2	0	3	0	5	1	6	9	0	2	3	5	1	0	15
16:15 16:30	0	2	0	2	1	3	0	4	6	1	6	0	7	0	2	16
16:30 16:45	1	3	1	5	0	3	0	3	8	1	2	0	3	1	1	14
16:45 17:00	3	1	0	6	0	1	1	2	8	0	3	0	3	1	1	14
17:00 17:15	0	2	0	4	0	1	1	2	6	1	0	0	1	0	2	10
17:15 17:30	2	3	0	5	0	4	0	4	9	1	2	0	3	0	1	13
17:30 17:45	1	2	0	3	0	2	0	2	5	0	3	0	3	0	2	11
17:45 18:00	0	3	0	3	1	3	1	5	8	0	4	0	4	0	3	15
Total	52	99	5	163	19	121	19	160	323	23	54	46	123	10	47	204

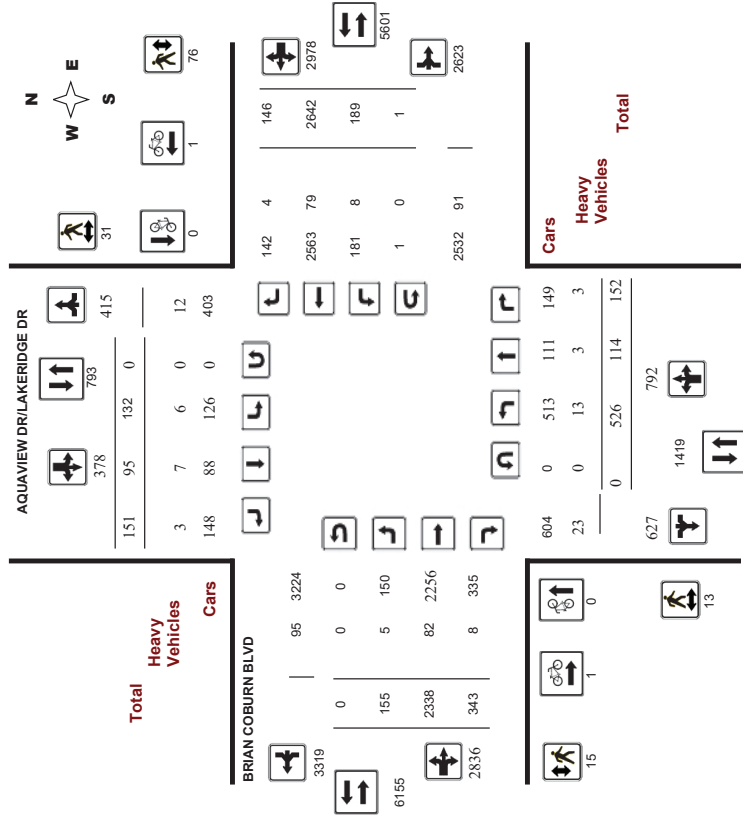
Survey Date: Wednesday, September 19, 2018  
 Start Time: 07:00  
 WO No: 38045  
 Device: Miovision

Full Study 15 Minute U-Turn Total

Time Period	TENTH LINE RD		BRIAN COBURN BLVD		Total
	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	
07:00	0	0	0	0	0
07:15	0	2	0	0	2
07:30	1	1	0	0	2
07:45	0	0	0	0	0
08:00	0	0	0	0	0
08:15	0	0	0	0	0
08:30	0	0	0	0	0
08:45	0	0	0	0	0
09:00	0	1	0	0	1
09:15	0	0	0	0	0
09:30	1	0	0	0	1
09:45	1	2	0	0	3
10:00	1	0	0	0	1
11:30	0	0	0	0	0
11:45	0	0	0	0	0
12:00	0	0	0	0	0
12:15	0	0	0	0	0
12:30	2	2	0	0	4
12:45	1	2	0	0	3
13:00	0	2	0	0	2
13:15	0	0	0	0	0
13:30	0	1	0	0	1
15:00	0	0	0	0	0
15:15	0	0	0	0	0
15:30	1	0	0	0	1
15:45	0	0	0	0	0
16:00	1	1	0	0	2
16:15	0	1	0	0	1
16:30	1	0	0	0	1
16:45	2	0	0	0	2
17:00	3	0	0	0	3
17:15	0	1	1	0	2
17:30	0	0	0	0	0
17:45	0	0	0	0	0
18:00	0	0	0	0	0
Total	15	16	1	0	32

Survey Date: Tuesday, February 26, 2019  
 Start Time: 07:00  
 WO No: 38372  
 Device: Miovision

Full Study Diagram





# Transportation Services - Traffic Services

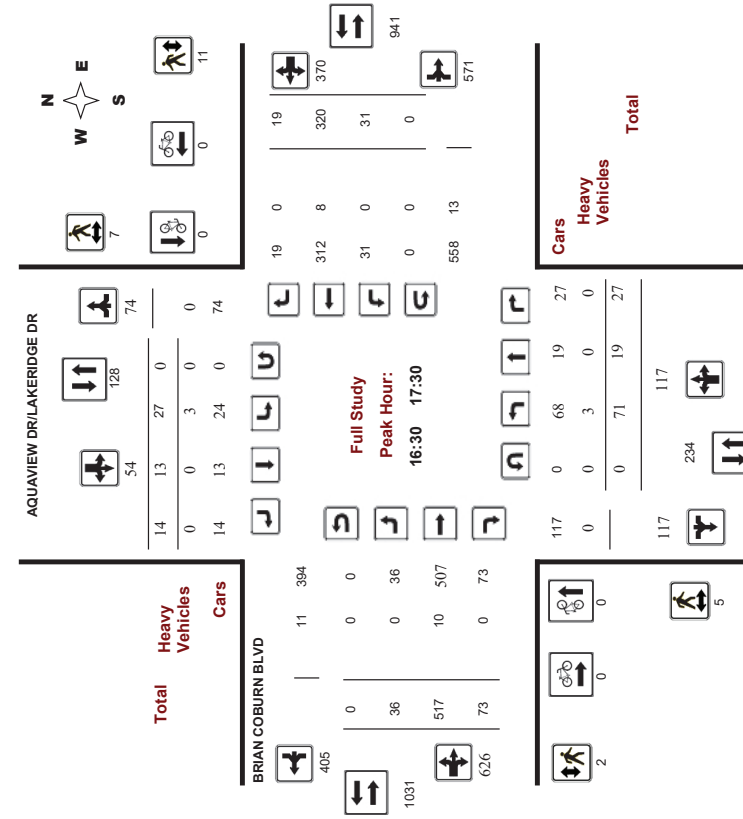
## Turning Movement Count - Study Results

### AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision

#### Full Study Peak Hour Diagram



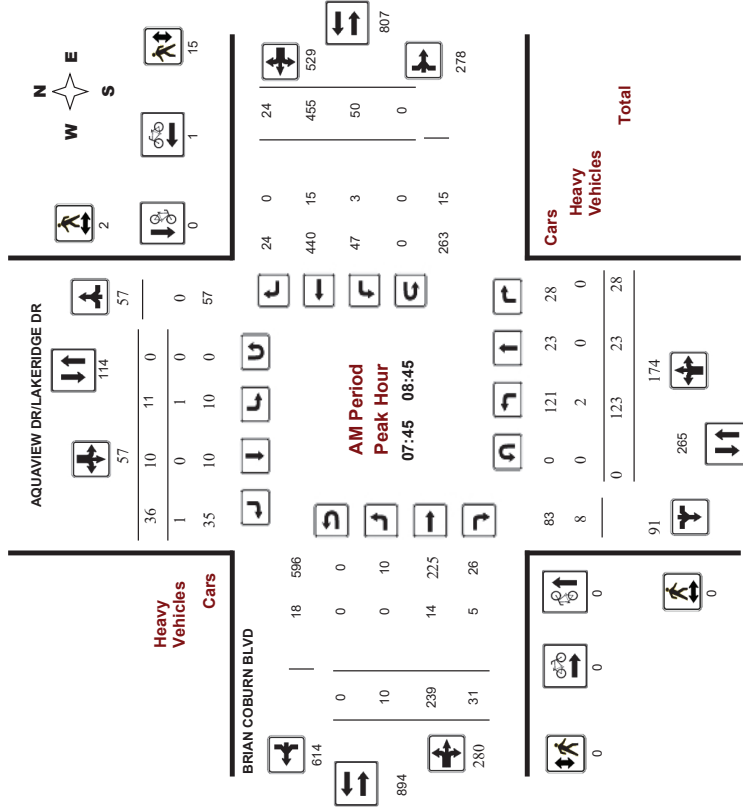
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision





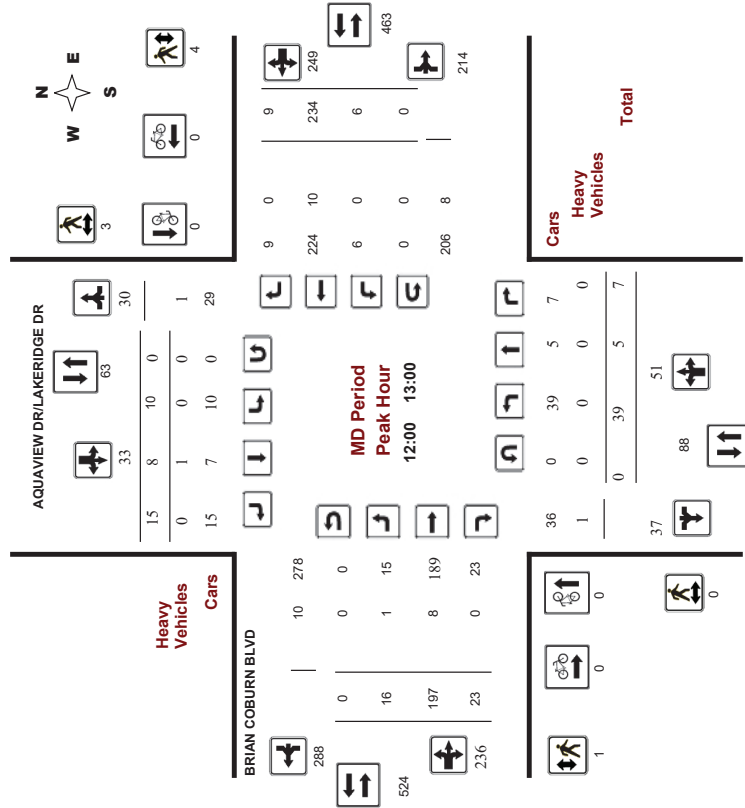
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision



Comments



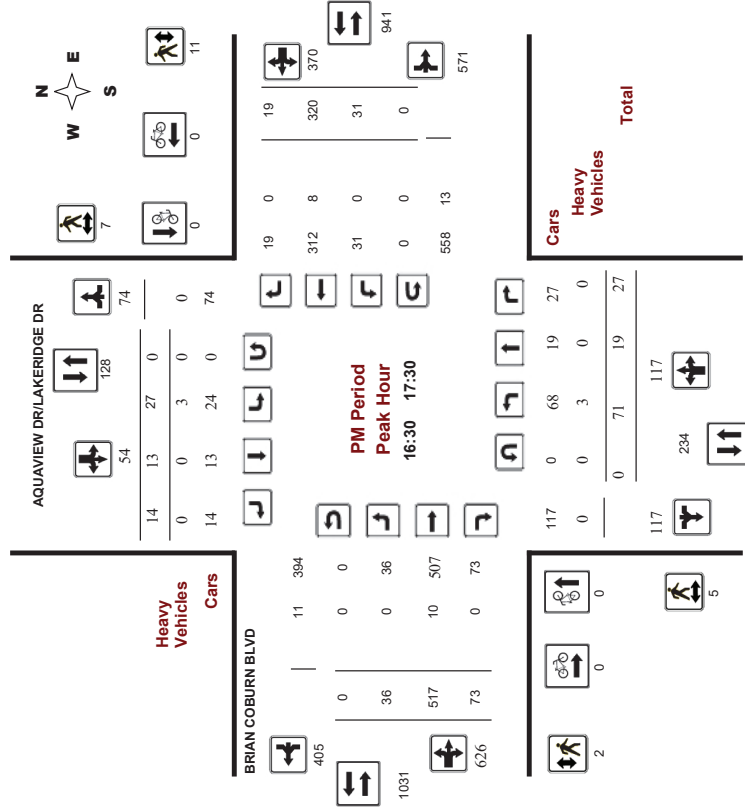
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision



Comments



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**AQUAVIEW DRLAKERIDGE DR @ BRIAN COBURN BLVD**

**Survey Date:** Tuesday, February 26, 2019 **WO No:** 38372  
**Start Time:** 07:00 **Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Tuesday, February 26, 2019 **Total Observed U-Turns** **AAADT Factor**  
 Northbound: 0 Southbound: 0 1.00  
 Eastbound: 0 Westbound: 1

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	NB	LT	ST	RT	TOT	EB	LT	ST	RT	TOT	WB	STR			
07:00-08:00	110	22	24	156	9	15	34	58	214	7	175	28	210	27	484	14	525	949	
08:00-09:00	113	22	28	163	12	10	31	53	216	12	246	29	287	48	442	22	512	1015	
09:00-10:00	57	5	11	73	14	6	18	38	111	5	159	23	187	8	315	30	353	651	
11:30-12:30	34	10	5	49	11	7	13	31	80	9	191	26	226	6	249	8	263	489	
12:30-13:30	27	5	3	35	10	7	13	30	65	14	195	31	240	4	208	13	225	530	
15:00-16:00	56	19	28	103	24	24	9	57	160	30	388	57	475	33	315	19	367	842	
16:00-17:00	73	17	34	124	25	14	13	52	176	42	498	83	623	30	297	15	342	965	
17:00-18:00	56	14	19	89	27	12	20	59	148	36	486	66	588	33	332	25	390	978	
<b>Sub Total</b>	<b>526</b>	<b>114</b>	<b>152</b>	<b>792</b>	<b>132</b>	<b>95</b>	<b>151</b>	<b>378</b>	<b>1170</b>	<b>155</b>	<b>2338</b>	<b>343</b>	<b>2836</b>	<b>189</b>	<b>2642</b>	<b>146</b>	<b>2977</b>	<b>5813</b>	
<b>U-Turns</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	
<b>Total</b>	<b>526</b>	<b>114</b>	<b>152</b>	<b>792</b>	<b>132</b>	<b>95</b>	<b>151</b>	<b>378</b>	<b>1170</b>	<b>155</b>	<b>2338</b>	<b>343</b>	<b>2836</b>	<b>190</b>	<b>2642</b>	<b>146</b>	<b>2978</b>	<b>5814</b>	
<b>EQ 12hr</b>	<b>731</b>	<b>158</b>	<b>211</b>	<b>1100</b>	<b>183</b>	<b>132</b>	<b>210</b>	<b>525</b>	<b>1625</b>	<b>215</b>	<b>3250</b>	<b>477</b>	<b>3942</b>	<b>284</b>	<b>3672</b>	<b>203</b>	<b>4139</b>	<b>8081</b>	

Note: These values are calculated by multiplying the totals by the appropriate expansion factor.

**AVG 12hr** 731 158 211 1100 183 132 210 525 1625 215 3250 477 3942 284 3672 203 4139 8081 9706  
 Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.

**AVG 24hr** 958 207 276 1441 240 173 275 688 2129 282 4258 625 5165 346 4810 266 5422 10887 12716  
 Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to "U-Turn" Report for specific breakdown.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**AQUAVIEW DRLAKERIDGE DR @ BRIAN COBURN BLVD**

**Survey Date:** Tuesday, February 26, 2019 **WO No:** 38372  
**Start Time:** 07:00 **Device:** Miovision

**Full Study 15 Minute Increments**

**Survey Date:** Tuesday, February 26, 2019 **Total Observed U-Turns** **AAADT Factor**  
 Northbound: 0 Southbound: 0 1.00  
 Eastbound: 0 Westbound: 1

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	N	LT	ST	RT	TOT	S	STR	LT	ST	RT	TOT	E			
07:00-07:15	32	7	7	46	2	0	7	9	55	0	26	4	30	9	141	4	154	184	239
07:15-07:30	20	4	2	26	2	5	8	15	41	2	40	8	50	8	118	5	131	181	222
07:30-07:45	32	7	9	48	3	6	8	17	65	3	58	6	67	4	111	0	115	182	247
07:45-08:00	26	4	6	36	2	4	11	17	53	2	51	10	63	6	114	5	125	188	241
08:00-08:15	22	8	5	35	2	2	11	15	50	5	71	3	79	10	93	3	106	185	235
08:15-08:30	36	6	7	49	3	2	10	15	64	1	61	10	72	17	133	7	157	229	293
08:30-08:45	39	5	10	54	4	2	4	10	64	2	56	8	66	17	115	9	141	207	271
08:45-09:00	16	3	6	25	3	4	6	13	38	4	58	8	70	4	101	3	108	178	216
09:00-09:15	21	2	5	28	2	4	2	8	36	2	37	8	47	5	98	14	117	164	200
09:15-09:30	15	0	0	15	5	1	6	12	27	1	52	6	59	2	81	8	91	150	177
09:30-09:45	12	1	3	16	4	0	4	8	24	2	38	6	46	0	89	6	75	121	145
09:45-10:00	9	2	3	14	3	1	6	10	24	0	32	3	35	2	67	2	71	106	130
10:00-10:15	5	3	1	9	6	1	5	12	21	1	45	7	53	2	50	4	56	109	130
10:15-10:30	7	4	0	11	1	2	3	6	17	0	51	7	58	0	72	2	74	132	149
10:30-10:45	13	2	2	17	1	3	2	6	23	4	50	10	64	4	70	1	75	139	162
10:45-11:00	9	1	2	12	3	1	3	7	19	4	45	2	51	0	57	1	58	109	128
11:00-11:15	10	1	2	13	4	1	5	10	23	5	44	6	55	2	45	5	52	107	130
11:15-11:30	7	1	1	9	2	3	5	10	19	3	58	5	66	0	62	2	64	130	149
11:30-11:45	2	0	0	2	0	1	3	5	14	4	48	9	61	1	44	4	49	110	124
11:45-12:00	3	1	0	4	3	0	2	5	9	2	45	11	58	1	57	2	60	118	127
12:00-12:15	7	5	4	16	5	4	0	9	25	5	81	10	96	9	87	4	100	196	221
12:15-12:30	16	4	7	27	5	7	2	14	41	6	102	10	118	4	62	3	69	187	228
12:30-12:45	14	3	8	25	8	8	2	18	43	10	97	18	125	12	65	3	80	205	248
12:45-13:00	19	7	9	35	6	5	5	16	51	9	108	19	136	8	101	9	118	254	305
13:00-13:15	15	5	6	26	6	4	5	15	41	9	104	22	135	7	74	7	88	223	264
13:15-13:30	16	2	8	26	6	2	3	11	37	9	131	19	159	6	68	0	74	233	270
13:30-13:45	20	4	12	36	10	3	4	17	53	14	126	24	164	6	78	5	89	253	306
13:45-14:00	22	6	8	36	3	5	1	9	45	10	137	18	165	11	77	3	91	256	301
14:00-14:15	15	4	4	23	9	2	4	15	38	6	135	20	161	7	79	5	91	252	290
14:15-14:30	14	5	3	22	5	3	5	13	35	6	119	11	136	7	86	6	99	235	270
14:30-14:45	11	4	11	26	8	6	7	21	47	15	119	23	157	10	81	8	99	236	303
14:45-15:00	16	1	1	18	5	1	4	10	28	9	113	12	134	9	86	6	101	235	263
<b>Total:</b>	<b>526</b>	<b>114</b>	<b>152</b>	<b>792</b>	<b>132</b>	<b>95</b>	<b>151</b>	<b>378</b>	<b>1170</b>	<b>155</b>	<b>2338</b>	<b>343</b>	<b>2836</b>	<b>190</b>	<b>2642</b>	<b>146</b>	<b>2978</b>	<b>1170</b>	<b>6,984</b>

Note: U-Turns are included in Totals.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38372  
**Device:** Miovision

**Full Study Cyclist Volume**

AQUAVIEW DR/LAKERIDGE DR      BRIAN COBURN BLVD

Time Period	Street Total		Street Total		Grand Total
	Northbound	Southbound	Eastbound	Westbound	
07:00 07:15	0	0	1	0	1
07:15 07:30	0	0	0	0	0
07:30 07:45	0	0	0	0	0
07:45 08:00	0	0	0	0	0
08:00 08:15	0	0	0	1	1
08:15 08:30	0	0	0	0	0
08:30 08:45	0	0	0	0	0
08:45 09:00	0	0	0	0	0
09:00 09:15	0	0	0	0	0
09:15 09:30	0	0	0	0	0
09:30 09:45	0	0	0	0	0
09:45 10:00	0	0	0	0	0
10:00 10:15	0	0	0	0	0
10:15 10:30	0	0	0	0	0
10:30 10:45	0	0	0	0	0
10:45 11:00	0	0	0	0	0
11:00 11:15	0	0	0	0	0
11:15 11:30	0	0	0	0	0
11:30 11:45	0	0	0	0	0
11:45 12:00	0	0	0	0	0
12:00 12:15	0	0	0	0	0
12:15 12:30	0	0	0	0	0
12:30 12:45	0	0	0	0	0
12:45 13:00	0	0	0	0	0
13:00 13:15	0	0	0	0	0
13:15 13:30	0	0	0	0	0
13:30 13:45	0	0	0	0	0
13:45 14:00	0	0	0	0	0
14:00 14:15	0	0	0	0	0
14:15 14:30	0	0	0	0	0
14:30 14:45	0	0	0	0	0
14:45 15:00	0	0	0	0	0
15:00 15:15	0	0	0	0	0
15:15 15:30	0	0	0	0	0
15:30 15:45	0	0	0	0	0
15:45 16:00	0	0	0	0	0
16:00 16:15	0	0	0	0	0
16:15 16:30	0	0	0	0	0
16:30 16:45	0	0	0	0	0
16:45 17:00	0	0	0	0	0
17:00 17:15	0	0	0	0	0
17:15 17:30	0	0	0	0	0
17:30 17:45	0	0	0	0	0
17:45 18:00	0	0	0	0	0
<b>Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>2</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38372  
**Device:** Miovision

**Full Study Pedestrian Volume**

AQUAVIEW DR/LAKERIDGE DR      BRIAN COBURN BLVD

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)		Total	Grand Total
	NB Approach (E or W Crossing)	Total	SB Approach (N or S Crossing)	Total	WB Approach (N or S Crossing)	Total		
07:00 07:15	0	1	0	0	0	8	8	9
07:15 07:30	0	2	0	0	0	8	8	10
07:30 07:45	0	3	0	0	0	7	7	10
07:45 08:00	0	0	0	0	0	6	6	6
08:00 08:15	0	0	0	0	0	4	4	4
08:15 08:30	0	1	0	0	0	4	4	5
08:30 08:45	0	1	0	0	0	1	1	2
08:45 09:00	0	1	0	0	0	2	2	3
09:00 09:15	0	0	0	0	0	2	2	2
09:15 09:30	0	2	0	0	0	0	0	2
09:30 09:45	0	1	0	0	0	0	0	1
09:45 10:00	0	2	0	0	0	0	0	2
10:00 10:15	0	2	0	0	0	0	0	2
10:15 10:30	0	0	0	0	0	0	0	2
10:30 10:45	0	0	0	0	0	0	0	3
10:45 11:00	0	0	0	0	0	0	0	1
11:00 11:15	0	2	0	0	0	1	1	3
11:15 11:30	0	2	0	0	0	1	1	3
11:30 11:45	0	1	0	0	0	1	1	2
11:45 12:00	0	1	0	0	0	2	2	2
12:00 12:15	0	2	0	0	0	2	2	2
12:15 12:30	0	2	0	0	0	2	2	2
12:30 12:45	0	1	0	0	0	1	1	2
12:45 13:00	0	0	0	0	0	2	2	2
13:00 13:15	0	2	0	0	0	0	0	2
13:15 13:30	0	0	0	0	0	0	0	1
13:30 13:45	0	0	0	0	0	0	0	0
13:45 14:00	0	0	0	0	0	0	0	0
14:00 14:15	2	1	3	2	3	5	8	8
14:15 14:30	0	0	0	0	0	2	2	5
14:30 14:45	0	1	0	0	0	5	5	6
14:45 15:00	0	1	0	0	0	2	2	4
15:00 15:15	1	0	1	2	1	3	4	4
15:15 15:30	0	2	2	0	0	4	4	5
15:30 15:45	0	2	2	0	0	3	3	5
15:45 16:00	0	2	2	0	0	5	5	6
16:00 16:15	0	2	2	0	0	4	4	4
16:15 16:30	0	2	2	0	0	3	3	5
16:30 16:45	0	2	2	0	0	5	5	7
16:45 17:00	0	2	2	0	0	2	2	2
17:00 17:15	1	1	1	0	0	0	0	2
17:15 17:30	4	2	6	4	0	3	3	9
17:30 17:45	3	1	4	1	0	2	2	6
17:45 18:00	2	0	2	0	0	1	1	3
<b>Total</b>	<b>13</b>	<b>31</b>	<b>44</b>	<b>15</b>	<b>76</b>	<b>91</b>	<b>91</b>	<b>135</b>



Transportation Services - Traffic Services

Turning Movement Count - Study Results

AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision

Full Study Heavy Vehicles

AQUAVIEW DR/LAKERIDGE DR

BRIAN COBURN BLVD

Southbound Westbound

Table with columns: Time Period, Northbound (LT, ST, RT, TOT), Southbound (LT, ST, RT, TOT), Eastbound (LT, ST, RT, TOT), Westbound (LT, ST, RT, TOT), W, STR, Grand Total.



Transportation Services - Traffic Services

Turning Movement Count - Study Results

AQUAVIEW DR/LAKERIDGE DR @ BRIAN COBURN BLVD

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38372  
Device: Miovision

Full Study 15 Minute U-Turn Total

AQUAVIEW DR/LAKERIDGE DR

BRIAN COBURN BLVD

Northbound Southbound Eastbound Westbound

Table with columns: Time Period, Northbound U-Turn Total, Southbound U-Turn Total, Eastbound U-Turn Total, Westbound U-Turn Total, Total.





# Transportation Services - Traffic Services

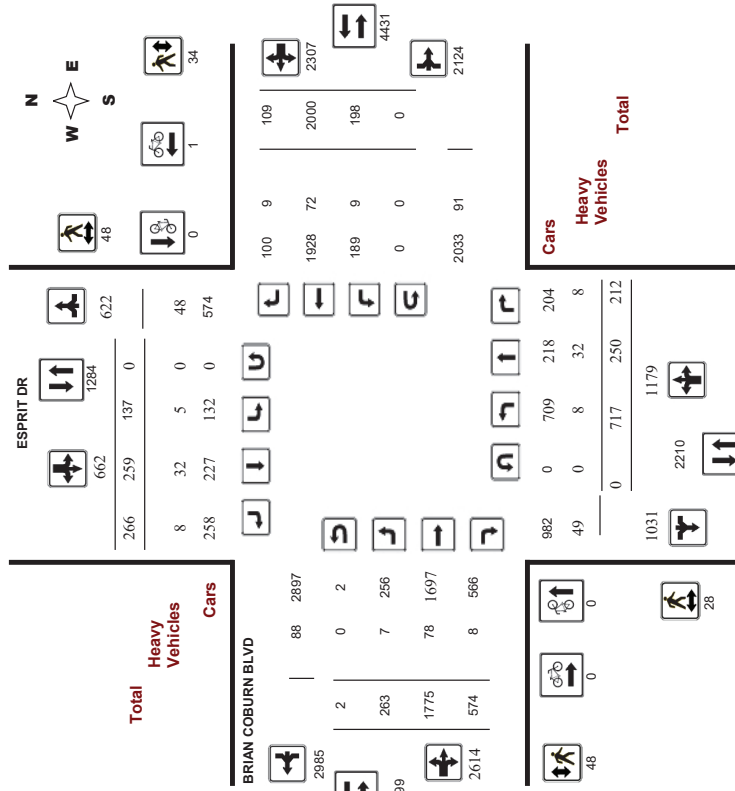
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38373  
Device: Miovision

#### Full Study Diagram



# Transportation Services - Traffic Services

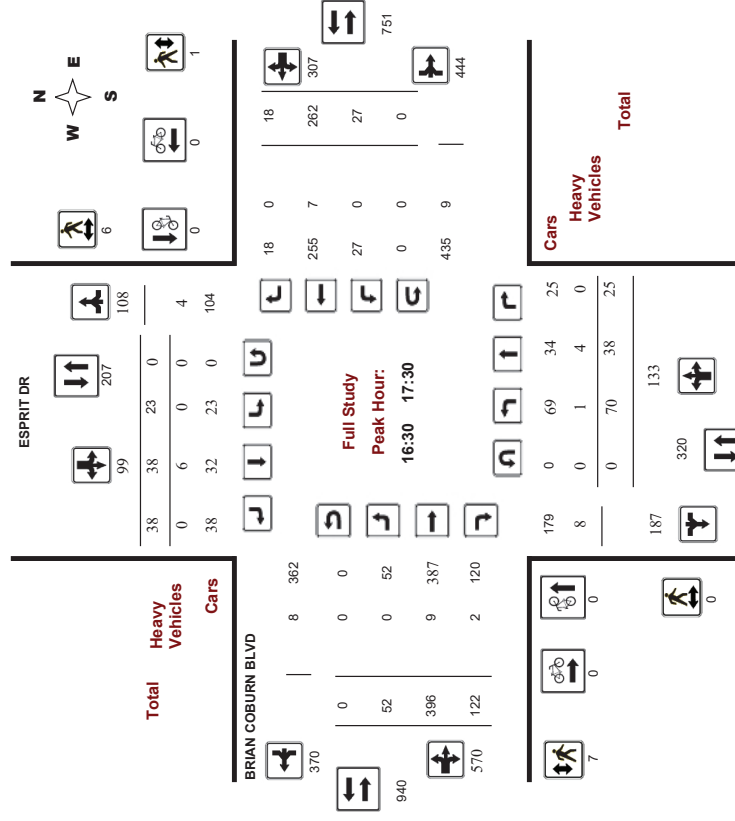
## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019  
Start Time: 07:00

WO No: 38373  
Device: Miovision

#### Full Study Peak Hour Diagram





# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

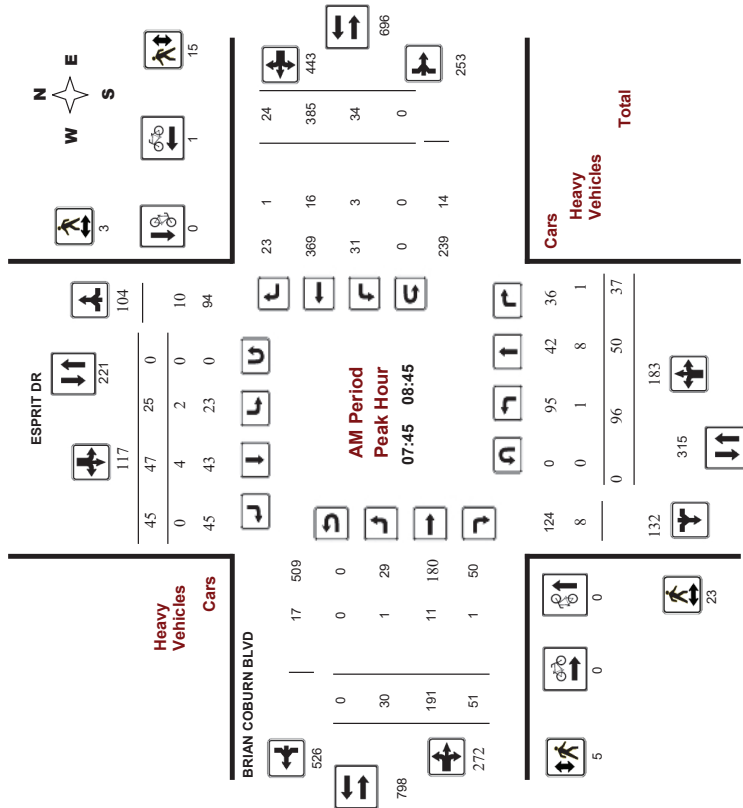
### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019

WO No: 38373

Device: Miovision

Start Time: 07:00



# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

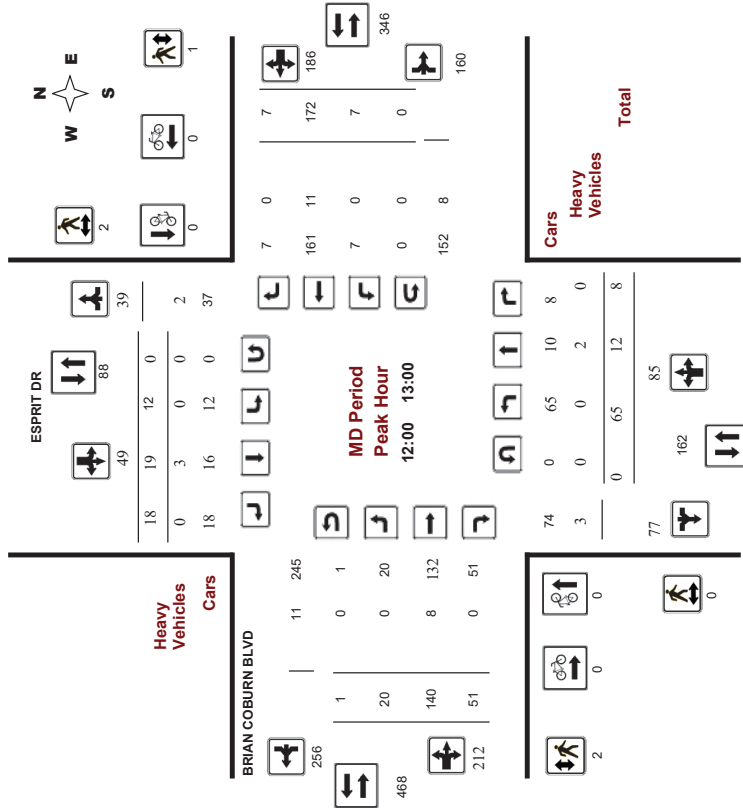
### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019

WO No: 38373

Device: Miovision

Start Time: 07:00





# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

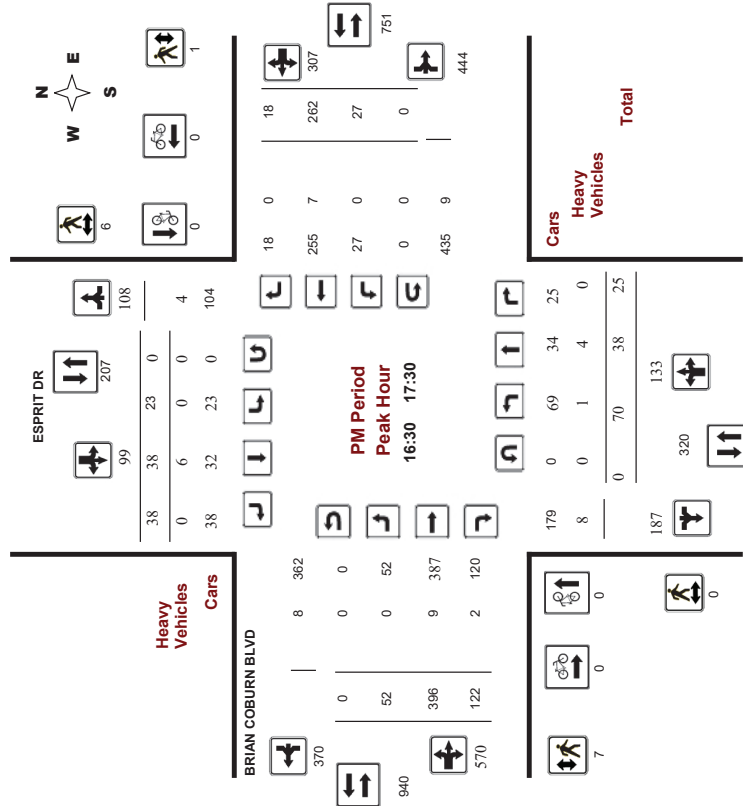
### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019

Start Time: 07:00

WO No: 38373

Device: Miovision



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### BRIAN COBURN BLVD @ ESPRIT DR

Survey Date: Tuesday, February 26, 2019

Start Time: 07:00

WO No: 38373

Device: Miovision

## Full Study Summary (8 HR Standard)

Survey Date: Tuesday, February 26, 2019

Total Observed U-Turns

Northbound: 0

Southbound: 0

Westbound: 0

AADT Factor

1.00

1.00

Eastbound: 2

Westbound: 0

ESPRIT DR

BRIAN COBURN BLVD

Period	Northbound					Southbound					Eastbound					Westbound					STR TOT	WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	RT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT				
07:00-08:00	119	62	38	219	18	33	55	106	325	24	165	23	212	24	355	28	407	619	944					
08:00-09:00	104	38	32	174	25	40	33	98	272	30	198	52	280	34	373	13	420	700	972					
09:00-10:00	129	25	23	177	12	31	29	72	249	14	127	46	187	21	195	6	222	409	658					
11:30-12:30	65	12	11	88	7	14	25	46	134	22	139	45	206	7	174	8	189	395	529					
12:30-13:30	58	13	7	78	12	19	15	46	124	19	141	50	210	13	157	8	178	388	512					
15:00-16:00	90	35	43	168	11	42	33	86	254	50	266	107	423	50	244	18	312	735	989					
16:00-17:00	73	35	36	144	34	37	40	111	255	53	384	128	585	26	227	15	288	833	1088					
17:00-18:00	79	30	22	131	18	43	36	97	228	51	355	123	529	23	275	13	311	840	1068					
<b>Sub Total</b>	<b>717</b>	<b>250</b>	<b>212</b>	<b>1179</b>	<b>137</b>	<b>259</b>	<b>266</b>	<b>662</b>	<b>1841</b>	<b>263</b>	<b>1775</b>	<b>574</b>	<b>2612</b>	<b>198</b>	<b>2000</b>	<b>109</b>	<b>2307</b>	<b>4919</b>	<b>6760</b>					
<b>U-Turns</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>2</b>	<b>2</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>2</b>					
<b>Total</b>	<b>717</b>	<b>250</b>	<b>212</b>	<b>1179</b>	<b>137</b>	<b>259</b>	<b>266</b>	<b>662</b>	<b>1841</b>	<b>263</b>	<b>1775</b>	<b>574</b>	<b>2614</b>	<b>198</b>	<b>2000</b>	<b>109</b>	<b>2307</b>	<b>4921</b>	<b>6762</b>					
<b>EQ 12hr</b>	<b>987</b>	<b>348</b>	<b>295</b>	<b>1639</b>	<b>190</b>	<b>360</b>	<b>370</b>	<b>920</b>	<b>2559</b>	<b>366</b>	<b>2467</b>	<b>798</b>	<b>3633</b>	<b>275</b>	<b>2780</b>	<b>152</b>	<b>3207</b>	<b>6840</b>	<b>9399</b>					
Note: These values are calculated by multiplying the totals by the appropriate expansion factor: 1.39																								
<b>AVG 12hr</b>	<b>939</b>	<b>328</b>	<b>278</b>	<b>1544</b>	<b>179</b>	<b>339</b>	<b>348</b>	<b>867</b>	<b>2539</b>	<b>345</b>	<b>2325</b>	<b>752</b>	<b>3424</b>	<b>259</b>	<b>2620</b>	<b>143</b>	<b>3022</b>	<b>6840</b>	<b>9399</b>					
Note: These values are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor: 1																								
<b>AVG 24hr</b>	<b>1230</b>	<b>429</b>	<b>364</b>	<b>2023</b>	<b>235</b>	<b>444</b>	<b>456</b>	<b>1136</b>	<b>3159</b>	<b>451</b>	<b>3046</b>	<b>985</b>	<b>4486</b>	<b>340</b>	<b>3432</b>	<b>187</b>	<b>3959</b>	<b>8445</b>	<b>11604</b>					
Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor: 1.31																								
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.																								



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ ESPRIT DR**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38373  
**Device:** Miovision

**Full Study 15 Minute Increments**

Time Period	Northbound				Southbound				Eastbound				Westbound				Grand Total			
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT				
07:00	41	14	5	60	1	4	17	22	3	2	25	7	34	2	96	6	104	3	220	
07:15	28	15	9	52	7	6	15	28	5	4	32	3	39	6	91	4	101	5	220	
07:30	31	16	13	60	6	6	7	19	6	9	64	5	78	8	80	5	93	6	250	
07:45	19	17	11	47	4	17	16	37	5	9	44	8	61	8	88	13	109	5	254	
08:00	28	12	7	47	8	13	7	28	4	7	47	15	69	8	71	2	81	4	225	
08:15	25	9	10	44	8	10	12	30	3	6	52	15	73	8	119	4	131	3	278	
08:30	24	12	9	45	5	7	10	22	4	8	48	13	69	10	107	5	122	4	258	
08:45	27	5	6	38	4	10	4	18	3	9	51	9	69	8	78	2	86	3	211	
09:00	15	15	8	38	3	13	6	22	3	2	36	7	45	12	62	0	74	3	215	
09:15	39	4	7	50	3	8	9	20	5	2	42	10	55	5	40	0	45	5	170	
09:30	23	2	5	30	3	4	9	16	2	5	25	17	47	1	43	4	48	2	141	
09:45	16	4	3	23	3	6	5	14	2	5	24	12	41	3	50	2	55	2	133	
11:30	11:45	8	4	4	16	1	2	3	6	2	5	36	10	51	4	46	2	52	2	125
11:45	12:00	20	3	2	25	1	4	10	15	3	5	35	9	49	0	43	2	45	3	134
12:00	12:15	22	2	3	27	2	5	8	15	1	5	37	12	54	1	45	3	49	1	145
12:15	12:30	15	3	2	20	3	4	10	1	7	31	14	52	2	40	1	43	1	125	
12:30	12:45	12	3	3	18	4	3	3	10	1	5	33	11	50	2	38	1	41	1	119
12:45	13:00	16	4	0	20	3	8	3	14	2	3	39	14	56	2	49	2	53	2	143
13:00	13:15	11	3	1	15	2	4	6	12	2	7	31	14	52	6	34	3	43	2	124
13:15	13:30	19	3	1	23	3	4	3	10	2	4	38	11	53	3	36	2	41	2	127
15:00	15:15	19	7	9	35	3	9	9	21	1	13	48	24	85	21	75	1	97	1	238
15:15	15:30	13	6	12	31	2	8	6	16	3	14	69	30	113	15	48	3	66	3	226
15:30	15:45	19	5	9	33	2	14	10	26	5	11	68	22	101	9	51	3	63	5	223
15:45	16:00	39	17	13	69	4	11	8	23	2	12	81	31	124	5	70	11	86	2	302
16:00	16:15	20	10	10	40	10	10	9	29	7	9	86	28	123	6	57	4	67	7	259
16:15	16:30	19	7	12	38	12	9	9	30	2	18	89	37	144	5	46	2	53	2	265
16:30	16:45	17	9	8	34	4	9	14	27	4	14	103	32	149	8	58	5	71	4	281
16:45	17:00	17	9	6	32	8	9	8	25	1	12	106	31	149	7	66	4	77	1	283
17:00	17:15	21	7	6	34	8	6	9	23	5	12	102	30	144	4	62	3	69	5	270
17:15	17:30	15	13	5	33	3	14	7	24	1	14	85	29	128	8	76	6	90	1	275
17:30	17:45	17	4	8	29	2	13	11	26	3	13	94	33	140	10	70	1	81	3	276
17:45	18:00	26	6	3	35	5	10	9	24	0	12	74	31	117	1	67	3	71	0	247
Total:	717	250	212	1179	137	259	266	662	93	263	1775	574	2614	198	2000	109	2307	93	6,762	

Note: U-Turns are included in Totals.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ ESPRIT DR**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38373  
**Device:** Miovision

**Full Study Cyclist Volume**

Time Period	Northbound		Southbound		Street Total		Eastbound		Westbound		Street Total		Grand Total
	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	ESPRIT DR	
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	1



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ ESPRIT DR**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38373  
**Device:** Miovision

**Full Study Pedestrian Volume**  
**BRIAN COBURN BLVD**

ESPRIT DR

Time Period	SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		Total	WB Approach (N or S Crossing)	Grand Total
	E or W	S or N	N or S	E or W			
07:00	0	0	0	0	0	1	1
07:15	0	1	0	0	1	2	3
07:30	0	0	0	0	0	4	4
07:45	1	0	1	1	2	5	7
08:00	0	1	0	0	1	8	9
08:15	0	1	0	0	1	1	2
08:30	1	1	0	0	2	1	3
08:45	22	4	4	1	23	5	28
08:45	0	4	5	4	9	4	13
09:00	0	1	0	0	1	0	1
09:15	0	2	0	2	2	2	4
09:30	1	2	1	0	3	1	4
09:45	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0
11:30	0	0	0	0	0	1	1
11:45	0	0	0	0	0	1	1
12:00	1	0	2	0	3	2	5
12:15	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0
12:45	0	1	1	0	2	1	3
13:00	0	0	0	0	0	0	0
13:15	1	0	1	0	2	1	3
13:30	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0
15:15	0	2	2	0	4	1	5
15:30	0	11	11	0	22	0	33
15:45	0	4	4	7	15	0	20
16:00	0	0	0	8	8	0	16
16:15	0	4	4	0	8	4	12
16:30	0	1	1	0	2	1	3
16:45	0	2	1	1	3	2	5
17:00	0	3	0	0	3	0	3
17:15	0	0	0	3	3	0	3
17:30	2	3	5	4	14	1	15
17:45	1	3	4	3	11	3	14
<b>Total</b>	<b>28</b>	<b>48</b>	<b>48</b>	<b>34</b>	<b>76</b>	<b>82</b>	<b>158</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ ESPRIT DR**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38373  
**Device:** Miovision

**Full Study Heavy Vehicles**  
**BRIAN COBURN BLVD**

ESPRIT DR

Time Period	Northbound			Southbound			Eastbound			Westbound			W STR TOT	STR TOT	Grand Total				
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT							
07:00	0	2	1	3	0	0	0	0	0	2	0	2	0	5	1	6	8	11	
07:15	0	2	0	2	1	1	1	3	5	1	3	0	4	4	2	0	6	10	15
07:30	0	2	2	5	0	1	0	6	1	4	0	5	0	4	0	4	9	15	
07:45	0	2	0	2	0	3	0	3	5	1	3	0	4	0	5	1	6	10	15
08:00	1	2	1	4	0	0	0	4	0	3	1	4	2	4	0	6	10	14	
08:15	0	1	0	1	1	1	0	2	3	0	2	0	4	0	4	0	4	6	9
08:30	0	3	0	3	1	0	0	4	0	3	0	3	1	3	0	4	7	11	
08:45	0	0	1	1	1	0	0	2	3	1	3	1	5	0	1	2	7	10	
09:00	0	3	0	3	0	0	0	3	0	4	0	4	0	2	0	2	6	9	
09:15	0	1	0	1	2	0	2	1	3	5	0	2	0	1	0	1	3	8	
09:30	0	1	0	1	0	0	1	2	0	3	1	4	0	0	1	1	5	7	
09:45	0	0	0	0	0	2	2	1	2	0	3	1	0	0	1	1	2	4	
11:30	0	2	0	2	0	0	0	2	1	1	0	2	0	4	0	4	6	8	
11:45	0	0	0	0	1	1	1	3	3	0	0	0	0	1	0	1	1	4	
12:00	0	1	0	1	0	0	0	1	0	3	0	3	0	2	0	2	5	6	
12:15	0	0	0	0	0	1	0	1	0	2	0	2	0	2	0	2	4	5	
12:30	0	1	0	1	0	0	0	1	0	2	0	2	0	3	0	3	5	6	
12:45	0	0	0	0	0	2	2	0	2	0	1	0	1	0	4	0	5	7	
13:00	0	1	1	2	0	0	0	2	0	2	1	3	0	0	0	2	3	5	
13:15	0	1	0	1	0	1	0	2	0	4	1	5	0	0	0	3	7	9	
15:00	0	1	0	1	0	0	0	1	0	4	0	4	0	3	0	3	7	8	
15:15	0	1	0	1	0	0	0	1	0	4	0	4	0	3	0	3	7	8	
15:30	1	1	1	3	0	0	2	2	5	0	3	0	3	2	3	0	5	8	13
15:45	0	1	0	1	0	1	0	1	2	0	4	0	4	0	4	0	9	13	15
16:00	1	1	0	2	0	4	1	5	7	0	3	0	3	0	0	1	4	11	14
16:15	0	0	0	0	0	2	0	2	0	2	0	2	0	0	0	0	2	4	6
16:30	0	2	0	2	0	2	0	2	4	0	3	0	3	0	2	0	2	5	9
16:45	0	0	0	0	0	1	0	1	1	0	3	1	4	0	1	0	1	5	6
17:00	1	2	0	3	0	2	0	2	5	0	0	1	1	0	3	0	3	4	9
17:15	0	0	0	0	0	1	0	1	1	0	3	0	3	0	1	0	1	4	5
17:30	0	1	0	1	0	2	0	2	3	0	3	0	3	0	0	1	4	7	11
17:45	0	0	0	0	0	0	0	0	0	0	1	1	2	0	1	1	2	4	7
<b>Total</b>	<b>8</b>	<b>32</b>	<b>8</b>	<b>48</b>	<b>5</b>	<b>32</b>	<b>8</b>	<b>45</b>	<b>93</b>	<b>7</b>	<b>78</b>	<b>8</b>	<b>93</b>	<b>9</b>	<b>72</b>	<b>9</b>	<b>90</b>	<b>183</b>	<b>276</b>



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**BRIAN COBURN BLVD @ ESPRIT DR**

**Survey Date:** Tuesday, February 26, 2019  
**Start Time:** 07:00

**WO No:** 38373  
**Device:** Miovision

**Full Study 15 Minute U-Turn Total**  
**ESPRIT DR**

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn Total	U-Turn Total	U-Turn Total	U-Turn Total	U-Turn Total	U-Turn Total	U-Turn Total		
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	1	0	0	0	1	1
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	1	0	0	0	1	1
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>2</b>

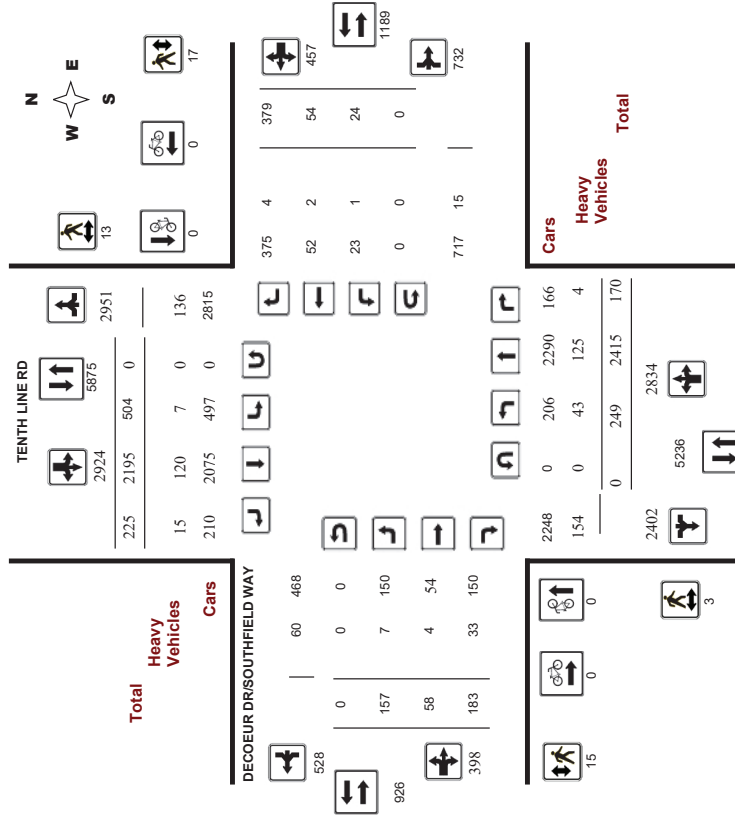


**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**  
**DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD**

**Survey Date:** Thursday, February 09, 2017  
**Start Time:** 07:00

**WO No:** 36678  
**Device:** Miovision

**Full Study Diagram**





# Transportation Services - Traffic Services

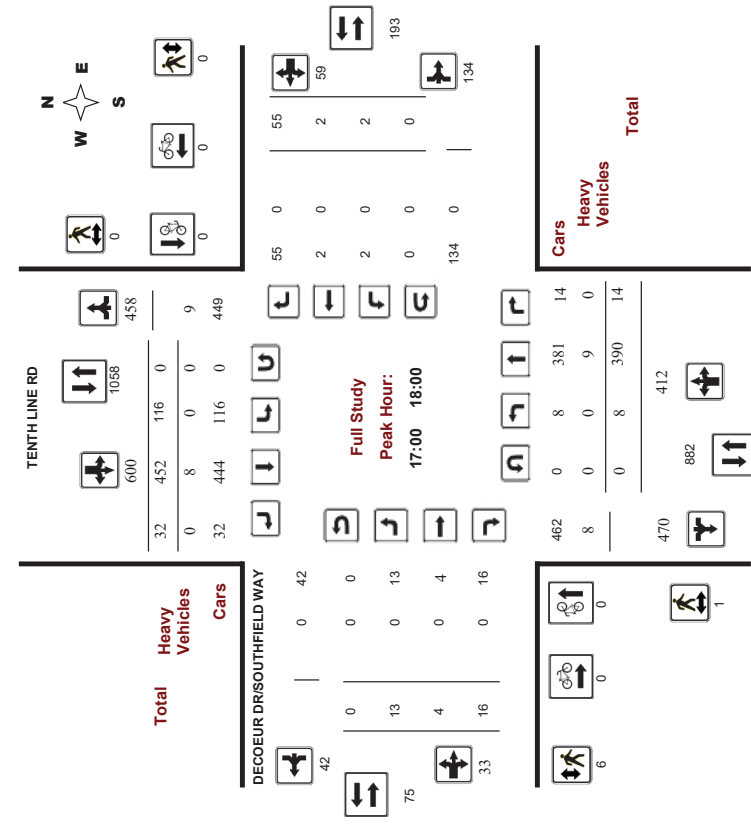
## Turning Movement Count - Study Results

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision

#### Full Study Peak Hour Diagram



Comments



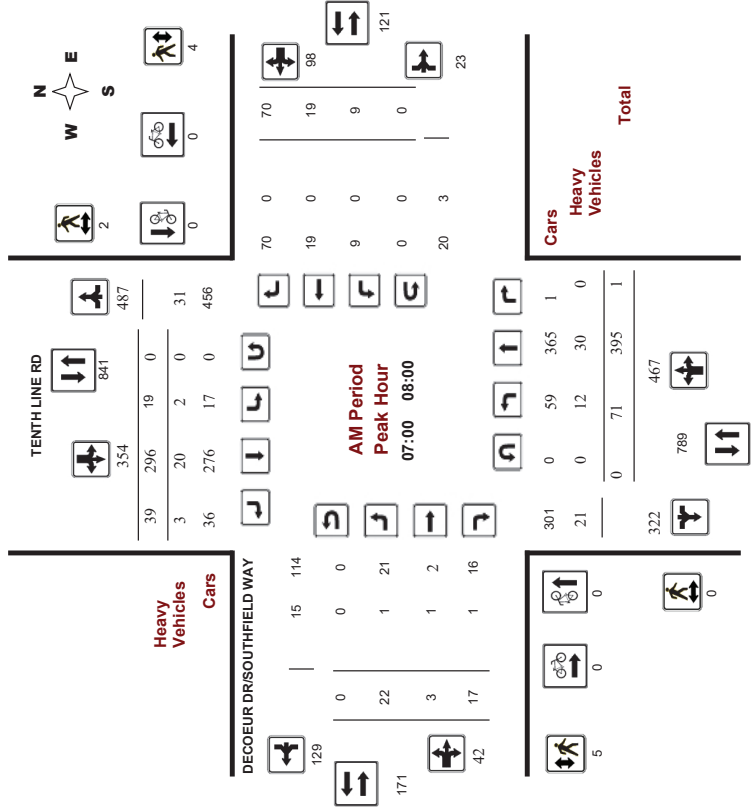
# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision



Comments



### Transportation Services - Traffic Services

### Turning Movement Count - Peak Hour Diagram

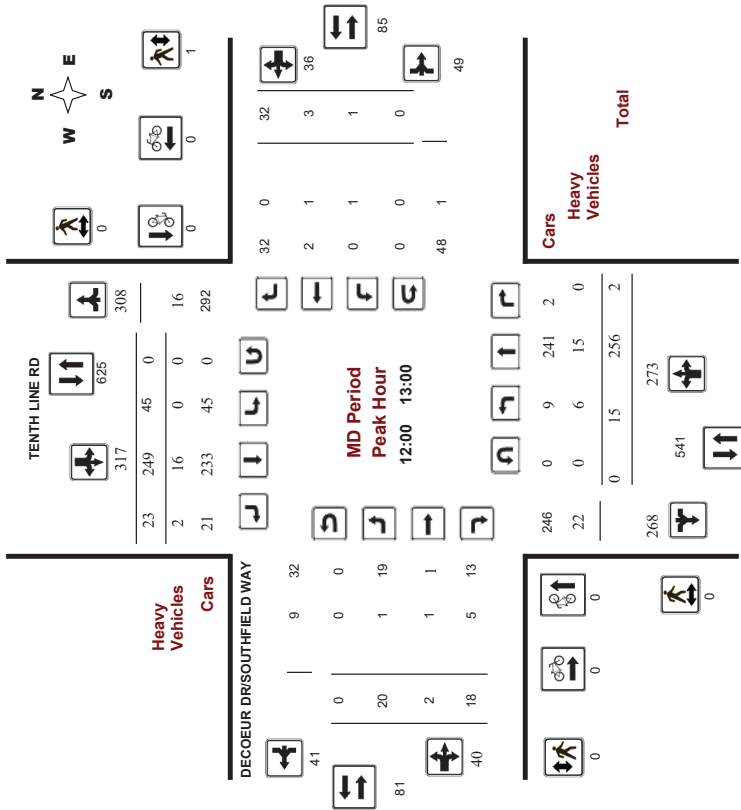
### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017

WO No: 36678

Device: Miovision

Start Time: 07:00



Comments



### Transportation Services - Traffic Services

### Turning Movement Count - Peak Hour Diagram

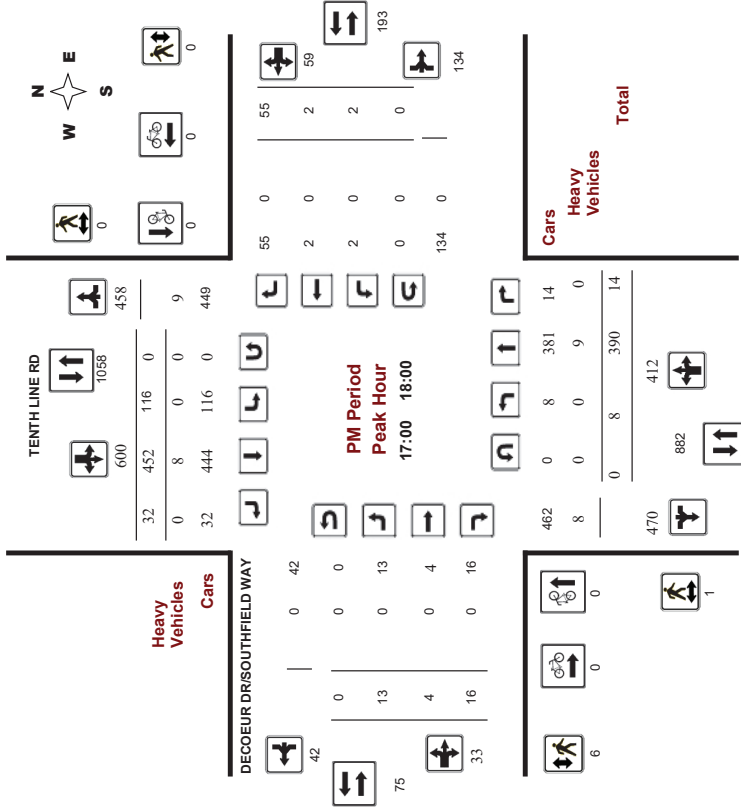
### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017

WO No: 36678

Device: Miovision

Start Time: 07:00



Comments





**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD**

**Survey Date:** Thursday, February 09, 2017      **WO No:** 36678  
**Start Time:** 07:00      **Device:** Miovision

**Full Study Summary (8 HR Standard)**

**Survey Date:** Thursday, February 09, 2017      **Total Observed U-Turns**      **AAADT Factor**  
Northbound: 0      Southbound: 0      90  
Eastbound: 0      Westbound: 0

Period	Northbound				Southbound				Eastbound				Westbound				WB TOT	STR TOT	Grand Total
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00-08:00	71	385	1	467	19	296	39	354	821	22	3	17	42	9	19	70	88	140	961
08:00-09:00	30	381	1	412	31	232	40	303	715	25	6	22	53	6	7	70	83	136	851
09:00-10:00	20	266	1	287	33	181	15	229	516	18	0	13	31	4	2	52	58	89	605
11:30-12:30	14	247	2	263	40	245	26	311	574	28	0	18	46	1	2	41	44	90	664
12:30-13:30	21	243	2	266	55	232	20	307	573	19	2	18	39	1	4	31	36	75	648
15:00-16:00	15	301	4	320	95	353	28	476	796	19	7	51	77	1	0	35	36	113	909
16:00-17:00	70	182	145	407	115	204	25	344	751	13	36	28	77	0	18	25	43	120	871
17:00-18:00	8	390	14	412	116	452	32	600	1012	13	4	16	33	2	2	55	59	92	1104
<b>Sub Total</b>	<b>249</b>	<b>2415</b>	<b>170</b>	<b>2834</b>	<b>504</b>	<b>2195</b>	<b>225</b>	<b>2924</b>	<b>5788</b>	<b>157</b>	<b>58</b>	<b>183</b>	<b>398</b>	<b>24</b>	<b>54</b>	<b>379</b>	<b>457</b>	<b>855</b>	<b>6613</b>
<b>U-Turns</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Total</b>	<b>249</b>	<b>2415</b>	<b>170</b>	<b>2834</b>	<b>504</b>	<b>2195</b>	<b>225</b>	<b>2924</b>	<b>5788</b>	<b>157</b>	<b>58</b>	<b>183</b>	<b>398</b>	<b>24</b>	<b>54</b>	<b>379</b>	<b>457</b>	<b>855</b>	<b>6613</b>

Note: These values are calculated by multiplying the totals by the appropriate expansion factor.

**EQ 12hr**      346      3357      236      3939      701      3051      313      4065      8004      218      81      254      553      33      75      527      635      1188      9192

**AVG 12hr**      311      3021      212      3544      631      2746      282      3659      7203      196      73      229      498      30      68      474      572      1070      8273

Note: These volumes are calculated by multiplying the Equivalent 12 hr. totals by the AADT factor.

**AVG 24hr**      407      3958      278      4643      827      3597      369      4793      9436      257      96      300      653      39      89      621      749      1402      10838

Note: These volumes are calculated by multiplying the Average Daily 12 hr. totals by 12 to 24 expansion factor.

Note: U-Turns provided for approach totals. Refer to "U-Turn" Report for specific breakdown.



**Transportation Services - Traffic Services**  
**Turning Movement Count - Study Results**

**DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD**

**Survey Date:** Thursday, February 09, 2017      **WO No:** 36678  
**Start Time:** 07:00      **Device:** Miovision

**Full Study 15 Minute Increments**

**Survey Date:** Thursday, February 09, 2017      **Total Observed U-Turns**      **AAADT Factor**  
Northbound: 0      Southbound: 0      90  
Eastbound: 0      Westbound: 0

Time Period	Northbound				Southbound				Eastbound				Westbound				W TOT	STR TOT	Grand Total	
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT				
07:00	11	97	0	108	7	72	14	93	201	6	0	3	9	4	6	27	37	46	247	
07:15	07:30	21	94	0	115	3	90	9	102	217	2	0	4	6	0	3	13	16	22	239
07:30	07:45	24	106	0	130	4	72	10	86	216	3	1	6	10	3	6	13	22	32	248
07:45	08:00	15	98	1	114	5	62	6	73	187	11	2	4	17	2	4	17	23	40	227
08:00	08:15	8	93	0	101	5	77	12	94	196	4	1	5	10	3	3	19	25	35	230
08:15	08:30	4	111	0	115	14	55	13	82	197	6	2	5	13	1	2	23	26	39	236
08:30	08:45	12	91	1	104	5	57	12	74	178	12	2	5	19	1	2	19	22	41	219
08:45	09:00	6	86	0	92	7	43	3	53	145	3	1	7	11	1	0	9	10	21	166
09:00	09:15	3	75	0	78	7	48	3	58	136	7	0	2	9	0	0	19	19	28	164
09:15	09:30	6	59	0	65	8	49	2	59	124	3	0	3	6	1	2	14	17	23	147
09:30	09:45	5	76	1	82	7	41	5	53	135	4	0	5	9	3	0	7	10	19	154
09:45	10:00	6	56	0	62	11	43	5	59	121	4	0	3	7	0	0	12	12	19	140
10:00	10:15	7	59	0	66	15	51	5	71	137	5	0	3	8	0	0	12	12	20	157
10:15	10:30	3	60	1	64	7	58	9	74	138	11	0	8	19	0	1	8	9	28	165
10:30	10:45	3	60	0	63	9	66	6	81	144	7	0	5	12	1	1	7	9	21	165
10:45	11:00	1	68	1	70	9	70	6	85	155	5	0	2	7	0	0	14	14	21	176
11:00	11:15	3	66	1	70	18	53	6	77	147	2	1	4	7	0	0	3	3	10	157
11:15	11:30	8	62	0	70	9	60	5	74	144	6	1	7	14	0	2	8	10	24	168
11:30	11:45	5	65	0	70	17	57	3	77	147	6	0	4	9	1	1	11	13	22	166
11:45	12:00	3	68	0	75	16	65	8	89	164	5	2	7	14	0	0	8	8	22	186
12:00	12:15	7	68	0	75	16	65	8	89	164	5	2	7	14	0	0	8	8	22	186
12:15	12:30	3	61	1	95	25	86	7	118	213	11	3	13	27	1	0	8	9	36	249
12:30	12:45	2	69	1	72	27	94	6	127	199	1	1	9	11	0	0	9	9	20	219
12:45	13:00	3	73	2	78	27	108	7	142	220	2	1	22	25	0	0	10	10	35	255
13:00	13:15	11	76	11	98	14	77	7	98	196	10	5	8	23	0	3	8	11	34	230
13:15	13:30	14	40	102	31	30	8	69	171	3	12	10	25	0	2	7	9	34	205	
13:30	13:45	0	65	95	44	21	5	70	165	0	12	8	20	0	10	0	10	30	195	
13:45	14:00	15	68	29	112	26	76	5	107	219	0	7	2	9	0	3	10	13	22	241
14:00	14:15	1	111	2	114	35	122	6	163	277	2	1	6	9	0	2	14	16	25	302
14:15	14:30	2	103	4	109	21	111	11	143	252	3	2	4	9	1	0	15	16	25	277
14:30	14:45	3	87	5	95	26	104	7	137	232	3	1	1	5	0	0	15	15	20	252
14:45	15:00	2	89	3	94	34	115	8	157	251	5	0	5	10	1	0	11	12	22	273
<b>Total:</b>	<b>249</b>	<b>2415</b>	<b>170</b>	<b>2834</b>	<b>504</b>	<b>2195</b>	<b>225</b>	<b>2924</b>	<b>5788</b>	<b>157</b>	<b>58</b>	<b>183</b>	<b>398</b>	<b>24</b>	<b>54</b>	<b>379</b>	<b>457</b>	<b>855</b>	<b>6613</b>	

Note: U-Turns are included in Totals.



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision

### Full Study Cyclist Volume

#### DECOEUR DR/SOUTHFIELD WAY

Time Period	TENTH LINE RD		DECOEUR DR/SOUTHFIELD WAY		Grand Total
	Northbound	Southbound	Eastbound	Westbound	
07:00 07:15	0	0	0	0	0
07:15 07:30	0	0	0	0	0
07:30 07:45	0	0	0	0	0
07:45 08:00	0	0	0	0	0
08:00 08:15	0	0	0	0	0
08:15 08:30	0	0	0	0	0
08:30 08:45	0	0	0	0	0
08:45 09:00	0	0	0	0	0
09:00 09:15	0	0	0	0	0
09:15 09:30	0	0	0	0	0
09:30 09:45	0	0	0	0	0
09:45 10:00	0	0	0	0	0
10:00 10:15	0	0	0	0	0
10:15 10:30	0	0	0	0	0
10:30 10:45	0	0	0	0	0
10:45 11:00	0	0	0	0	0
11:00 11:15	0	0	0	0	0
11:15 11:30	0	0	0	0	0
11:30 11:45	0	0	0	0	0
11:45 12:00	0	0	0	0	0
12:00 12:15	0	0	0	0	0
12:15 12:30	0	0	0	0	0
12:30 12:45	0	0	0	0	0
12:45 13:00	0	0	0	0	0
13:00 13:15	0	0	0	0	0
13:15 13:30	0	0	0	0	0
13:30 13:45	0	0	0	0	0
13:45 14:00	0	0	0	0	0
14:00 14:15	0	0	0	0	0
14:15 14:30	0	0	0	0	0
14:30 14:45	0	0	0	0	0
14:45 15:00	0	0	0	0	0
15:00 15:15	0	0	0	0	0
15:15 15:30	0	0	0	0	0
15:30 15:45	0	0	0	0	0
15:45 16:00	0	0	0	0	0
16:00 16:15	0	0	0	0	0
16:15 16:30	0	0	0	0	0
16:30 16:45	0	0	0	0	0
16:45 17:00	0	0	0	0	0
17:00 17:15	0	0	0	0	0
17:15 17:30	0	0	0	0	0
17:30 17:45	0	0	0	0	0
17:45 18:00	0	0	0	0	0
Total	0	0	0	0	0



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision

### Full Study Pedestrian Volume

#### DECOEUR DR/SOUTHFIELD WAY

Time Period	TENTH LINE RD		DECOEUR DR/SOUTHFIELD WAY		Grand Total
	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	
07:00 07:15	0	1	1	3	4
07:15 07:30	0	0	0	2	3
07:30 07:45	0	0	0	1	1
07:45 08:00	0	1	1	1	2
08:00 08:15	0	0	0	0	0
08:15 08:30	0	0	0	0	0
08:30 08:45	0	1	1	2	3
08:45 09:00	0	1	1	1	2
09:00 09:15	0	0	0	0	1
09:15 09:30	0	0	0	0	0
09:30 09:45	0	0	0	0	0
09:45 10:00	0	1	1	0	1
10:00 10:15	0	1	1	1	2
10:15 10:30	0	0	0	0	0
10:30 10:45	0	0	0	0	0
10:45 11:00	0	0	0	0	0
11:00 11:15	0	0	0	0	0
11:15 11:30	0	0	0	0	0
11:30 11:45	0	0	0	0	0
11:45 12:00	0	0	0	0	0
12:00 12:15	0	0	0	0	0
12:15 12:30	0	0	0	0	0
12:30 12:45	0	0	0	0	0
12:45 13:00	0	0	0	0	0
13:00 13:15	0	2	2	0	2
13:15 13:30	0	0	0	0	0
13:30 13:45	0	0	0	0	0
13:45 14:00	0	0	0	0	0
14:00 14:15	0	0	0	0	0
14:15 14:30	0	0	0	0	0
14:30 14:45	0	0	0	0	0
14:45 15:00	0	0	0	0	0
15:00 15:15	0	0	0	0	0
15:15 15:30	0	0	0	0	0
15:30 15:45	0	0	0	0	0
15:45 16:00	0	0	0	0	0
16:00 16:15	2	1	3	0	3
16:15 16:30	0	0	0	0	0
16:30 16:45	0	1	1	0	1
16:45 17:00	0	3	3	0	4
17:00 17:15	1	0	1	0	2
17:15 17:30	0	0	0	0	0
17:30 17:45	0	0	0	0	0
17:45 18:00	0	0	0	0	0
Total	3	13	16	15	32



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision

#### Full Study Heavy Vehicles

##### DECOEUR DR/SOUTHFIELD WAY

Time Period	Northbound				Southbound				Eastbound				Westbound				Grand Total		
	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT	LT	ST	RT	TOT			
07:00	1	8	0	9	0	4	0	4	13	0	0	0	0	0	0	0	13		
07:15	1	6	0	7	1	4	0	5	12	0	0	0	0	0	0	0	12		
07:30	6	8	0	14	1	6	2	9	23	1	1	0	2	0	0	0	25		
07:45	4	8	0	12	0	6	1	7	19	0	0	1	1	0	0	0	20		
08:00	2	2	0	4	0	9	0	9	11	1	0	3	4	0	0	0	15		
08:15	2	3	0	5	0	5	2	7	12	1	0	0	1	0	0	1	14		
08:30	2	6	0	8	0	6	2	8	16	1	0	3	4	0	0	0	20		
08:45	2	5	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0		
09:00	2	2	0	4	0	9	0	9	13	0	0	2	3	0	0	0	10		
09:15	2	2	0	4	0	9	0	9	13	0	0	0	0	0	0	0	13		
09:30	1	4	0	5	0	2	0	2	7	0	0	0	0	0	0	0	7		
09:45	1	7	0	8	0	5	1	6	14	0	0	0	0	0	0	0	14		
10:00	0	4	0	4	0	2	1	3	7	0	0	0	0	0	0	0	7		
10:15	3	4	0	7	0	3	0	3	10	0	0	0	0	0	0	0	10		
10:30	1	2	0	3	0	0	1	1	4	0	0	1	1	0	0	0	5		
10:45	3	3	0	6	0	3	0	3	9	0	0	0	0	0	0	0	9		
11:00	4	4	0	8	0	5	1	6	10	1	0	0	1	0	0	0	11		
11:15	4	4	0	8	0	4	0	4	10	0	0	0	0	0	0	0	10		
11:30	4	4	0	8	0	4	0	4	10	0	0	0	0	0	0	0	10		
11:45	1	4	0	5	0	4	1	5	10	0	1	3	4	0	0	0	14		
12:00	2	4	0	6	0	4	0	4	10	0	0	2	2	0	0	0	13		
12:15	2	4	0	6	0	4	0	4	10	0	0	2	2	0	0	0	13		
12:30	1	4	0	5	0	4	0	4	10	0	0	0	0	0	0	0	10		
12:45	2	4	0	6	0	4	0	4	10	0	0	2	2	0	0	0	13		
13:00	0	0	0	0	0	5	0	5	0	0	2	2	0	0	0	0	7		
13:15	2	6	0	8	0	4	0	4	12	0	0	1	0	0	0	0	13		
13:30	1	4	0	5	0	5	2	7	12	1	0	1	2	0	0	0	14		
13:45	1	6	0	7	0	4	0	4	11	0	0	4	4	0	0	0	16		
14:00	1	2	0	3	0	2	4	1	7	10	0	0	1	0	0	0	12		
14:15	1	7	0	8	0	7	0	7	15	0	0	5	5	0	0	0	20		
14:30	2	3	0	5	0	3	0	3	8	0	0	0	0	0	0	0	8		
14:45	1	2	1	4	1	1	0	2	6	0	0	2	2	0	0	0	8		
15:00	1	0	2	3	2	1	0	3	6	0	2	4	0	1	0	0	11		
15:15	1	2	1	4	0	1	0	1	5	0	0	0	0	0	0	0	5		
15:30	0	1	0	1	0	2	0	2	3	0	0	0	0	0	0	0	3		
15:45	0	4	0	4	0	2	0	2	6	0	0	0	0	0	0	0	6		
16:00	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	2		
16:15	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	6		
16:30	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	6		
16:45	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	6		
17:00	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	6		
17:15	0	4	0	4	0	2	0	2	6	0	0	0	0	0	0	0	6		
17:30	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	2		
17:45	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	6		
Total	43	125	4	172	7	120	15	142	314	7	4	33	44	1	2	4	7	51	365



# Transportation Services - Traffic Services

## Turning Movement Count - Study Results

### DECOEUR DR/SOUTHFIELD WAY @ TENTH LINE RD

Survey Date: Thursday, February 09, 2017  
Start Time: 07:00

WO No: 36678  
Device: Miovision

#### Full Study 15 Minute U-Turn Total

##### DECOEUR DR/SOUTHFIELD WAY

Time Period	Northbound		Southbound		Eastbound		Westbound		Total
	U-Turn	Total	U-Turn	Total	U-Turn	Total	U-Turn	Total	
07:00	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0



# Transportation Services - Traffic Services

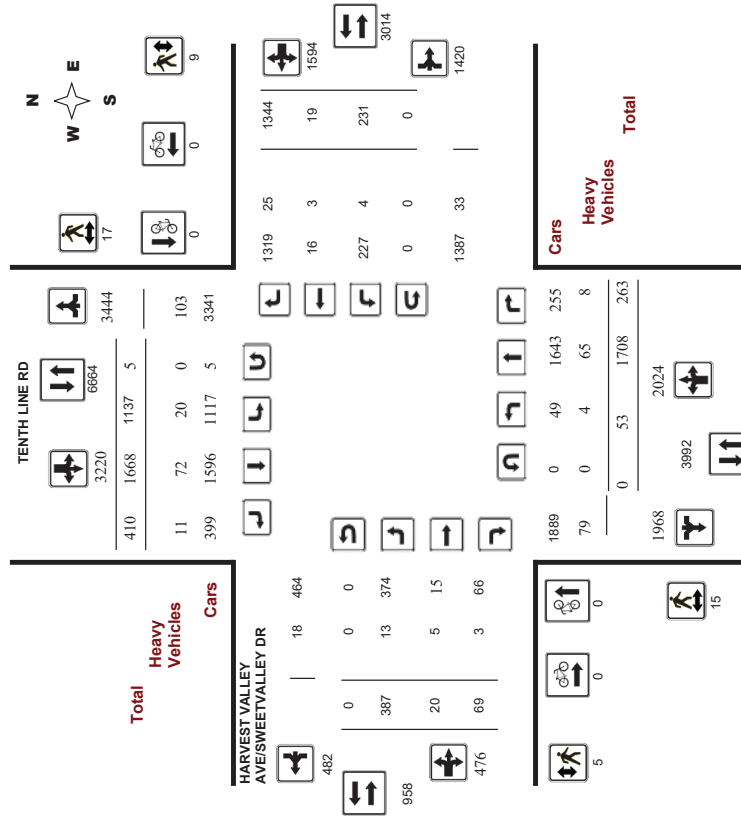
## Turning Movement Count - Study Results

### HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37740  
Device: Miovision

#### Full Study Diagram



# Transportation Services - Traffic Services

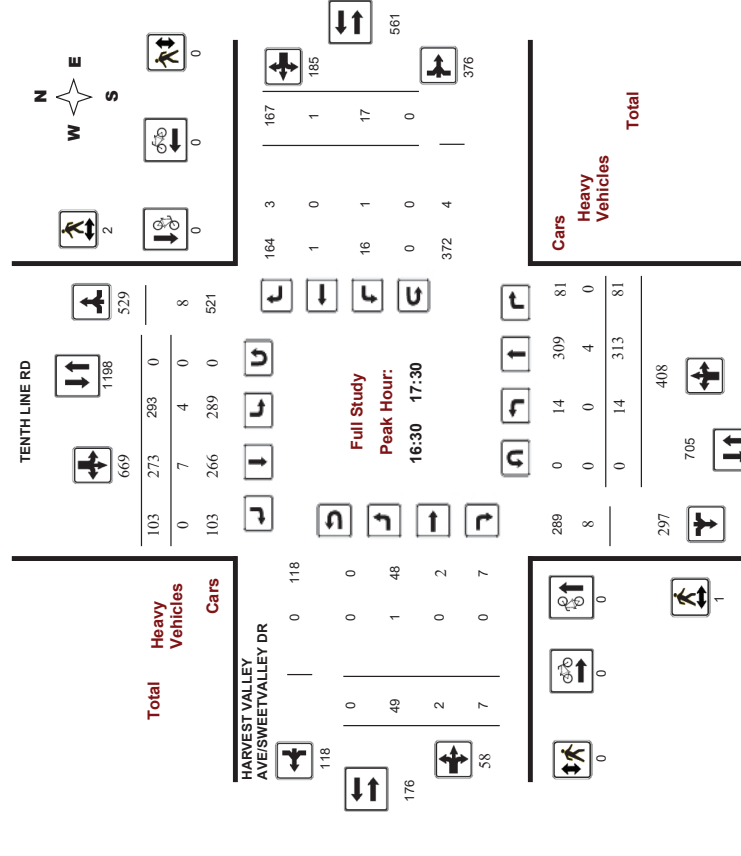
## Turning Movement Count - Study Results

### HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37740  
Device: Miovision

#### Full Study Peak Hour Diagram





# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

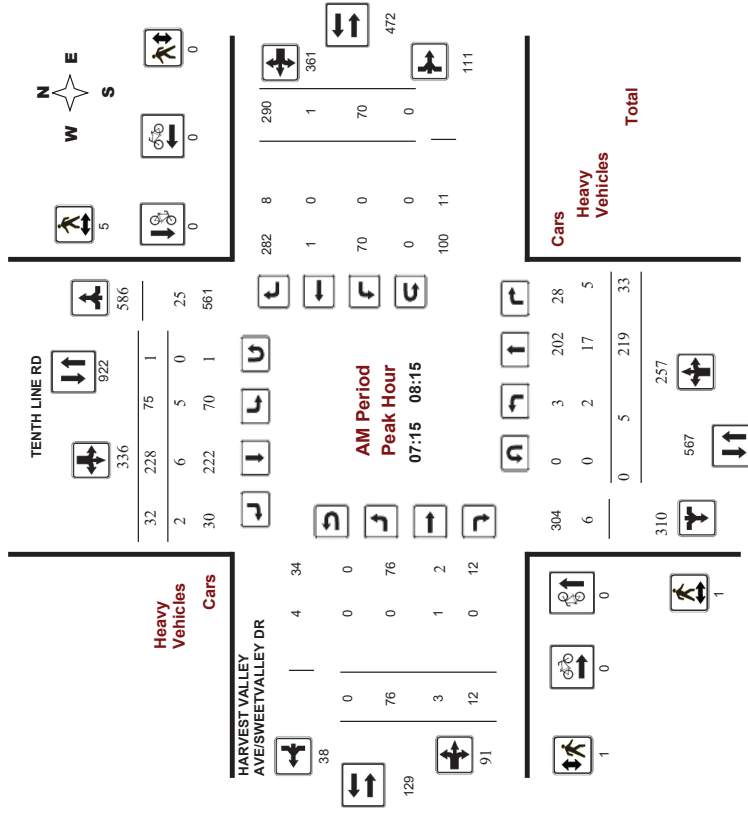
### HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018

WO No: 37740

Start Time: 07:00

Device: Miovision



Comments



# Transportation Services - Traffic Services

## Turning Movement Count - Peak Hour Diagram

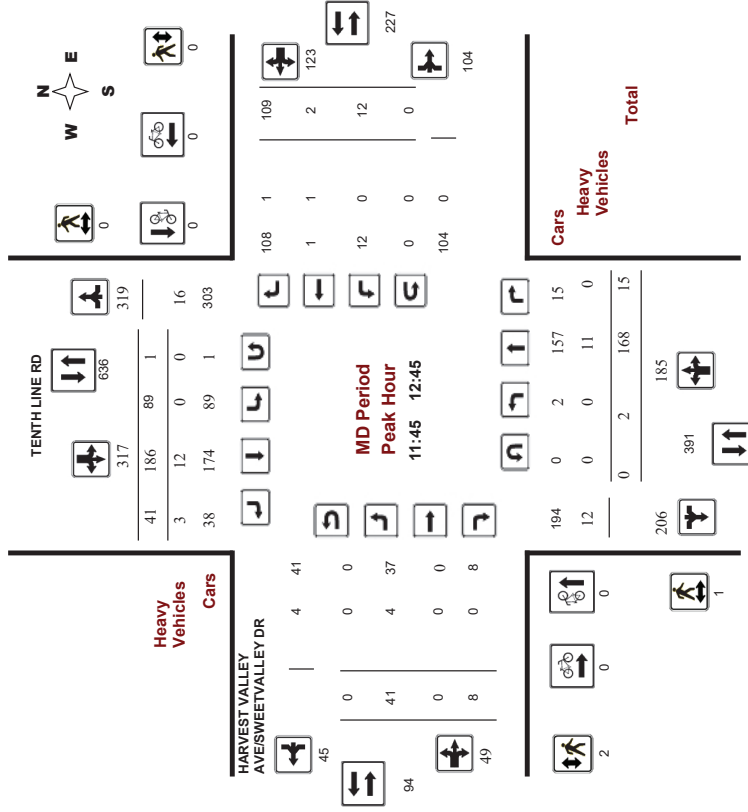
### HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018

WO No: 37740

Start Time: 07:00

Device: Miovision



Comments





Transportation Services - Traffic Services  
Turning Movement Count - Study Results

HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37740  
Device: Miovision

Full Study 15 Minute Increments

HARVEST VALLEY AVE/SWEETVALLEY DR

Time Period	Northbound			Southbound			Eastbound			Westbound			W	STR	Grand Total						
	LT	ST	RT	LT	ST	RT	LT	ST	RT	LT	ST	RT									
07:00	2	53	3	58	10	47	3	60	118	15	1	6	22	27	0	69	96	118	236		
07:15	07:30	2	44	11	57	14	61	5	80	137	14	2	6	22	22	0	83	105	127	284	
07:45	08:00	0	65	8	74	22	58	6	86	160	20	0	3	23	17	0	77	94	117	277	
08:00	08:15	0	52	5	57	16	48	11	75	132	24	0	1	25	16	0	66	82	107	239	
08:15	08:30	2	58	9	69	24	61	10	85	164	18	1	2	21	15	1	64	80	101	265	
08:30	08:45	1	52	6	59	27	34	9	70	129	19	0	4	23	21	1	57	79	102	231	
08:45	09:00	2	40	5	47	17	28	8	53	100	18	2	6	26	23	0	51	74	100	200	
09:00	09:15	3	47	5	55	14	27	6	47	102	20	1	2	23	8	0	41	49	72	174	
09:15	09:30	0	37	4	45	12	35	7	54	99	14	0	2	16	3	0	42	45	61	160	
09:30	09:45	0	43	4	43	15	31	11	57	100	11	2	2	15	4	0	43	47	62	162	
09:45	10:00	0	50	4	54	18	39	7	64	118	11	1	0	12	1	1	38	40	52	170	
10:00	10:15	0	36	3	39	18	18	8	44	83	11	0	3	14	7	1	38	46	60	143	
10:15	10:30	1	43	3	47	26	48	13	87	134	14	0	1	15	2	2	21	25	40	174	
10:30	10:45	0	46	6	52	26	44	8	78	130	8	0	4	12	2	0	25	27	39	169	
10:45	11:00	1	32	4	37	20	54	9	83	120	7	0	1	8	4	0	32	36	44	164	
11:00	11:15	0	47	2	49	18	40	11	69	118	12	0	2	14	4	0	31	35	49	167	
11:15	11:30	0	34	2	36	22	50	9	81	117	2	0	1	3	1	0	29	30	33	150	
11:30	11:45	3	48	3	54	22	60	7	89	143	9	0	0	9	1	0	24	25	34	177	
11:45	12:00	0	33	2	35	29	39	7	75	110	7	1	1	9	3	0	20	23	32	142	
12:00	12:15	2	51	3	56	39	58	22	119	175	15	0	0	15	5	0	34	39	54	229	
12:15	12:30	1	58	7	66	51	77	16	144	210	6	3	4	13	3	2	40	45	58	268	
12:30	12:45	3	62	11	76	55	61	15	131	207	7	0	1	10	2	1	34	37	47	283	
12:45	13:00	5	47	12	64	61	73	12	146	210	8	1	3	12	4	3	31	38	50	269	
13:00	13:15	4	79	21	104	82	64	19	165	269	14	1	1	16	6	1	42	49	65	334	
13:15	13:30	2	82	23	107	66	66	27	159	266	13	0	3	16	2	0	52	54	70	336	
13:30	13:45	3	74	19	96	70	73	27	170	266	8	0	2	10	3	0	36	39	49	315	
13:45	14:00	5	78	18	101	75	70	30	175	276	14	1	1	16	6	0	37	43	59	335	
14:00	14:15	0	67	17	84	68	62	24	154	238	7	0	1	8	8	0	49	57	65	303	
14:15	14:30	1	49	11	61	76	45	17	138	199	12	0	0	12	1	0	44	45	57	266	
14:30	14:45	0	53	1708	263	2024	1142	1668	410	3220	5244	387	20	69	476	231	19	1344	1594	5244	7,314

Note: U-Turns are included in Totals.



Transportation Services - Traffic Services  
Turning Movement Count - Study Results

HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00

WO No: 37740  
Device: Miovision

Full Study Cyclist Volume

HARVEST VALLEY AVE/SWEETVALLEY DR

Time Period	TENTH LINE RD		Southbound		Eastbound		Westbound		Street Total		Grand Total
	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total					
07:00	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0
09:00	0	0	0	0	0	0	0	0	0	0	0
09:15	0	0	0	0	0	0	0	0	0	0	0
09:30	0	0	0	0	0	0	0	0	0	0	0
09:45	0	0	0	0	0	0	0	0	0	0	0
10:00	0	0	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0
12:00	0	0	0	0	0	0	0	0	0	0	0
12:15	0	0	0	0	0	0	0	0	0	0	0
12:30	0	0	0	0	0	0	0	0	0	0	0
12:45	0	0	0	0	0	0	0	0	0	0	0
13:00	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	0	0	0	0	0	0	0	0	0
13:30	0	0	0	0	0	0	0	0	0	0	0
13:45	0	0	0	0	0	0	0	0	0	0	0
14:00	0	0	0	0	0	0	0	0	0	0	0
14:15	0	0	0	0	0	0	0	0	0	0	0
14:30	0	0	0	0	0	0	0	0	0	0	0
14:45	0	0	0	0	0	0	0	0	0	0	0
15:00	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	0	0	0	0	0	0	0	0	0
15:30	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	0	0	0	0	0	0	0	0	0
16:00	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	0	0	0	0	0	0	0	0	0
16:45	0	0	0	0	0	0	0	0	0	0	0
17:00	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	0	0	0	0	0	0	0	0	0
18:00	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0



Transportation Services - Traffic Services  
Turning Movement Count - Study Results

HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00  
WO No: 37740  
Miovision Device:

Full Study Pedestrian Volume  
HARVEST VALLEY AVE/SWEETVALLEY DR

Time Period	NB Approach (E or W Crossing)		SB Approach (E or W Crossing)		EB Approach (N or S Crossing)		WB Approach (N or S Crossing)		Total	Grand Total
	NB	SB	EB	WB	NB	SB	EB	WB		
07:00 07:15	1	0	0	0	0	0	0	0	1	2
07:15 07:30	0	3	0	0	0	0	0	0	3	3
07:30 07:45	0	2	1	0	0	0	0	0	1	3
07:45 08:00	1	0	0	0	0	0	0	0	0	1
08:00 08:15	0	0	0	0	0	0	0	0	0	0
08:15 08:30	0	1	0	0	0	0	0	0	1	2
08:30 08:45	0	0	0	0	0	0	0	0	0	0
08:45 09:00	0	1	0	0	0	0	0	0	1	1
09:00 09:15	0	0	0	0	0	0	0	0	0	0
09:15 09:30	0	0	0	0	0	0	0	0	0	0
09:30 09:45	0	0	0	0	0	0	0	0	0	0
09:45 10:00	1	1	0	0	0	0	0	0	2	2
11:30 11:45	0	0	0	0	0	0	0	0	0	0
11:45 12:00	1	0	0	0	2	0	0	0	1	3
12:00 12:15	0	0	0	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0	0	0	0
12:45 13:00	2	1	0	0	0	0	0	0	3	4
13:00 13:15	0	0	0	0	0	0	0	0	0	0
13:15 13:30	1	0	0	0	0	0	0	0	1	1
15:00 15:15	0	0	0	0	0	0	0	0	0	0
15:15 15:30	0	1	0	0	0	0	0	0	1	2
15:30 15:45	0	2	0	0	0	0	0	0	2	3
15:45 16:00	1	3	0	0	0	0	0	0	4	5
16:00 16:15	0	0	0	0	0	0	0	0	0	1
16:15 16:30	1	0	0	0	0	0	0	0	1	1
16:30 16:45	0	2	0	0	0	0	0	0	2	2
16:45 17:00	0	0	0	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0	0	0	0
17:15 17:30	1	0	0	0	0	0	0	0	1	1
17:30 17:45	5	0	0	0	0	0	0	0	5	5
17:45 18:00	0	0	0	0	0	0	0	0	0	0
Total	15	17	5	5	9	9	14	14	32	46



Transportation Services - Traffic Services  
Turning Movement Count - Study Results

HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
Start Time: 07:00  
WO No: 37740  
Miovision Device:

Full Study Heavy Vehicles  
HARVEST VALLEY AVE/SWEETVALLEY DR

Time Period	Northbound			Southbound			Eastbound			Westbound			W TOT	STR TOT	Grand Total				
	LT	ST	RT	N TOT	LT	ST	RT	S TOT	LT	ST	RT	E TOT				LT	ST	RT	
07:00 07:15	0	6	0	6	1	0	0	1	7	1	0	0	1	0	0	1	2	9	
07:15 07:30	1	7	2	10	2	1	0	3	13	0	1	0	1	0	0	2	3	16	
07:30 07:45	0	6	1	7	2	2	1	5	12	0	0	0	0	0	0	3	3	15	
07:45 08:00	0	1	1	2	1	0	0	1	3	0	0	0	0	0	0	3	3	6	
08:00 08:15	1	3	1	5	0	3	1	4	9	0	0	0	0	0	0	0	0	9	
08:15 08:30	0	3	1	4	1	2	2	5	9	1	0	1	2	0	0	2	4	13	
08:30 08:45	1	3	0	4	1	3	1	5	9	1	1	0	2	0	0	0	2	11	
08:45 09:00	0	2	0	2	0	3	1	4	6	0	1	0	1	0	0	0	1	7	
09:00 09:15	0	6	1	7	0	5	0	5	12	0	0	0	0	0	0	2	3	15	
09:15 09:30	0	0	0	0	0	4	0	4	4	0	1	0	0	0	0	2	3	7	
09:30 09:45	0	2	1	3	0	3	0	3	6	1	0	0	1	0	0	0	1	7	
09:45 10:00	0	2	0	2	0	1	0	1	3	0	0	0	0	0	0	0	1	4	
11:30 11:45	0	0	0	0	0	1	0	1	1	1	1	0	0	0	0	0	1	2	
11:45 12:00	0	2	0	2	0	4	0	4	6	1	0	0	1	0	0	1	2	8	
12:00 12:15	0	2	0	2	0	0	1	1	3	1	0	0	0	0	0	0	1	4	
12:15 12:30	0	3	0	3	0	3	1	4	7	0	0	0	0	0	0	0	1	8	
12:30 12:45	0	4	0	4	0	5	1	6	10	2	0	0	2	0	0	0	2	12	
12:45 13:00	0	4	0	4	0	3	0	3	7	0	0	0	0	0	0	0	0	7	
13:00 13:15	0	0	0	0	1	4	0	5	5	1	0	0	1	0	0	0	1	6	
13:15 13:30	0	2	0	2	0	1	0	1	3	0	0	0	0	0	0	0	0	3	
15:00 15:15	0	1	0	1	0	5	0	5	6	1	0	0	1	0	0	1	2	8	
15:15 15:30	0	1	0	1	0	4	1	5	6	0	1	1	2	0	1	2	4	10	
15:30 15:45	0	0	0	0	0	3	0	3	3	0	0	0	0	0	0	0	1	4	
15:45 16:00	0	0	0	0	2	2	0	4	4	0	0	0	0	0	0	1	2	6	
16:00 16:15	1	0	0	1	1	2	1	4	5	1	0	1	2	0	1	0	1	8	
16:15 16:30	0	1	0	1	0	1	0	1	2	0	0	0	0	0	0	0	1	3	
16:30 16:45	0	2	0	2	1	5	0	6	8	1	0	0	1	0	0	1	1	10	
16:45 17:00	0	2	0	2	1	0	0	1	3	0	0	0	0	0	0	0	1	4	
17:00 17:15	0	0	0	0	1	2	0	3	3	0	0	0	0	0	0	0	1	4	
17:15 17:30	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	1	2	
17:30 17:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
17:45 18:00	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	2	
Total	4	65	8	77	20	72	11	103	180	13	5	3	21	4	3	25	32	53	233





Transportation Services - Traffic Services

Turning Movement Count - Study Results

HARVEST VALLEY AVE/SWEETVALLEY DR @ TENTH LINE RD

Survey Date: Thursday, April 19, 2018  
 Start Time: 07:00

WO No: 37740  
 Division: Miovision

Full Study 15 Minute U-Turn Total

Time Period	TENTH LINE RD		HARVEST VALLEY AVE/SWEETVALLEY DR		Total
	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	
07:00	0	0	0	0	0
07:15	0	0	0	0	0
07:30	0	1	0	0	1
07:45	0	0	0	0	0
08:00	0	0	0	0	0
08:15	0	0	0	0	0
08:30	0	0	0	0	0
08:45	0	0	0	0	0
09:00	0	0	0	0	0
09:15	0	0	0	0	0
09:30	0	0	0	0	0
09:45	0	1	0	0	1
10:00	0	0	0	0	0
11:30	0	0	0	0	0
11:45	0	1	0	0	1
12:00	0	0	0	0	0
12:15	0	0	0	0	0
12:30	0	0	0	0	0
12:45	0	0	0	0	0
13:00	0	1	0	0	1
13:15	0	0	0	0	0
13:30	0	0	0	0	0
15:00	0	0	0	0	0
15:15	0	0	0	0	0
15:30	0	0	0	0	0
15:45	0	0	0	0	0
16:00	0	0	0	0	0
16:15	0	0	0	0	0
16:30	0	0	0	0	0
16:45	0	0	0	0	0
17:00	0	0	0	0	0
17:15	0	0	0	0	0
17:30	0	0	0	0	0
17:45	0	0	0	0	0
18:00	0	0	0	0	0
Total	0	5	0	0	5

# Appendix C

Synchro Intersection Worksheets – Existing Conditions

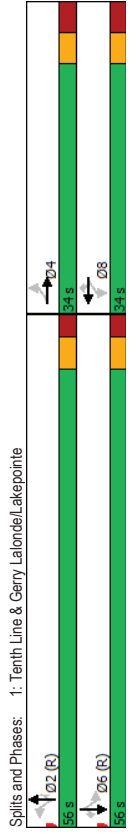
Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	101	15	30	56	170	8	894	7	48	502
Traffic Volume (vph)	101	15	30	56	170	8	894	7	48	502
Future Volume (vph)	101	15	30	56	170	8	894	7	48	502
Lane Group Flow (vph)	112	41	33	62	189	9	993	8	53	588
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	2	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	2	6	6
Detector Phase	4	4	8	8	2	2	2	2	6	6
Switch Phase										
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Total Split (s)	34.0	34.0	34.0	34.0	56.0	56.0	56.0	56.0	56.0	56.0
Total Split (%)	37.8%	37.8%	37.8%	37.8%	62.2%	62.2%	62.2%	62.2%	62.2%	62.2%
Maximum Green (s)	27.2	27.2	27.2	27.2	49.8	49.8	49.8	49.8	49.8	49.8
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag										
Lead-Lag Optimize?										
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	2.0	2.0	2.0	2.0	14.0	14.0	14.0	14.0	14.0	14.0
Pedestrian Calls (#/hr)	2	2	4	4	3	3	3	3	4	4
Act Effr Green (s)	15.7	15.7	15.7	15.7	61.3	61.3	61.3	61.3	61.3	61.3
Actuated G/C Ratio	0.17	0.17	0.17	0.17	0.68	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.55	0.14	0.15	0.20	0.56	0.02	0.44	0.01	0.17	0.26
Control Delay	42.3	16.4	29.9	30.7	21.7	4.2	4.8	0.0	9.0	6.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.3	16.4	29.9	30.7	21.7	4.2	4.8	0.0	9.0	6.8
LOS	D	B	C	C	A	A	A	A	A	A
Approach Delay	35.4		24.6		4.7				6.7	
Approach LOS	D		C		A				A	
Queue Length 50th (m)	18.3	2.5	5.0	9.5	14.0	0.2	16.8	0.0	2.7	15.7
Queue Length 95th (m)	29.0	9.2	10.9	16.9	28.5	m0.9	46.3	m0.0	11.2	35.3
Internal Link Dist (m)	372.5		134.8		154.1				468.1	
Turn Bay Length (m)	30.0		50.0		35.0		55.0		70.0	
Base Capacity (vph)	355	475	380	527	511	473	2235	966	306	2172
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.09	0.09	0.12	0.37	0.02	0.44	0.01	0.17	0.26

Intersection Summary  
 Cycle Length: 90  
 Actuated Cycle Length: 90  
 Offset: 61 (68%), Referenced to phase 2:NBT and 6:SBTL, Start of Green  
 Natural Cycle: 65

Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.56	
Intersection Signal Delay: 10.3	Intersection LOS: B
Intersection Capacity Utilization 63.8%	IOU Level of Service B
Analysis Period (min) 15	
m Volume for 95th percentile queue is metered by upstream signal.	

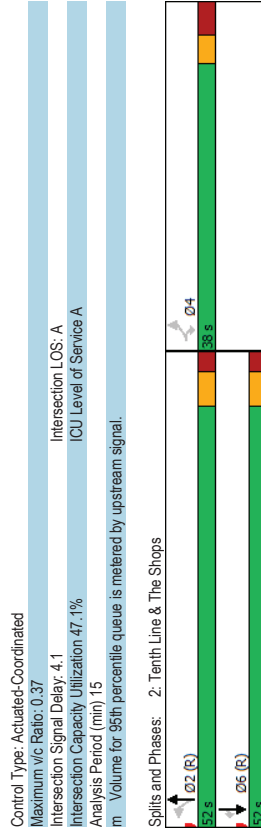


Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	54	13	70	851	493	61
Traffic Volume (vph)	54	13	70	851	493	61
Future Volume (vph)	60	14	78	946	548	68
Lane Group Flow (vph)	Perm	Perm	Perm	NA	NA	Perm
Turn Type						
Protected Phases	4	4	2	2	6	6
Permitted Phases	4	4	2	2	6	6
Detector Phase						
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	52.0	52.0	52.0	52.0
Total Split (%)	42.2%	42.2%	57.8%	57.8%	57.8%	57.8%
Maximum Green (s)	31.2	31.2	45.8	45.8	45.8	45.8
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	24.0	24.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)	0	0	4	4	4	4
Act Effr Green (s)	10.4	10.4	71.2	71.2	71.2	71.2
Actuated G/C Ratio	0.12	0.12	0.79	0.79	0.79	0.79
v/c Ratio	0.31	0.08	0.13	0.37	0.22	0.06
Control Delay	41.1	17.8	2.9	3.1	2.1	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.1	17.8	2.9	3.1	2.1	0.4
LOS	D	B	A	A	A	A
Approach Delay	36.7		3.1	1.9		
Approach LOS	D		A	A		
Queue Length 50th (m)	9.8	0.0	2.4	16.6	7.4	0.0
Queue Length 95th (m)	20.9	5.3	m=4.4	21.2	9.6	0.1
Internal Link Dist (m)	33.9		222.1	154.1		
Turn Bay Length (m)			75.0			60.0
Base Capacity (vph)	574	523	613	2572	2500	1151
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.03	0.13	0.37	0.22	0.06

Intersection Summary	
Cycle Length: 90	
Actuated Cycle Length: 90	
Offset: 69 (77%), Referenced to phase 2:NBT and 6:SBT, Start of Green	
Natural Cycle: 65	



Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Existing AM Peak Hour  
2370 Tenth Line Rd

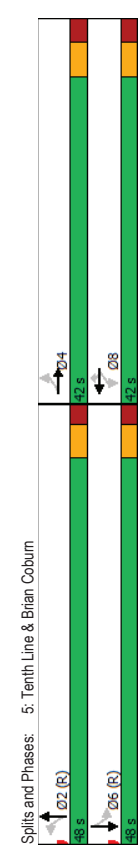
Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	150	178	52	373	220	214	539	115	309
Future Volume (vph)	150	178	52	373	220	214	539	115	309
Lane Group Flow (vph)	167	265	58	414	244	238	641	128	462
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	8	2	2	6	6
Detector Phase	4	4	8	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	31.4	31.4	31.4	31.4	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	42.0	42.0	48.0	48.0	48.0	48.0	48.0
Total Split (%)	46.7%	46.7%	46.7%	46.7%	53.3%	53.3%	53.3%	53.3%	53.3%
Maximum Green (s)	35.6	35.6	35.6	35.6	42.0	42.0	42.0	42.0	42.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.7	2.7	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time (s)	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0
Total Lost Time (s)	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	18.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	1	5	5	3	3	3	3	3
Act Effr Green (s)	28.3	28.3	28.3	28.3	49.3	49.3	49.3	49.3	49.3
Actuated G/C Ratio	0.31	0.31	0.31	0.31	0.55	0.55	0.55	0.55	0.55
v/c Ratio	1.04	0.52	0.22	0.76	0.41	0.52	0.36	0.37	0.27
Control Delay	111.4	25.7	22.1	36.6	7.0	16.0	10.3	23.5	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	111.4	25.7	22.1	36.6	7.0	16.0	10.3	23.5	14.5
LOS	F	C	C	D	A	B	B	C	B
Approach Delay	58.9			25.3		11.8		16.4	
Approach LOS	E			C		B		B	
Queue Length 50th (m)	28.6	33.5	7.2	62.9	5.2	20.1	32.2	11.2	16.2
Queue Length 95th (m)	#61.5	49.9	14.7	85.3	19.3	21.1	23.5	40.8	47.7
Internal Link Dist (m)	392.1			351.9		301.3		222.1	
Turn Bay Length (m)	45.0	50.0	50.0	45.0	105.0	110.0			
Base Capacity (vph)	203	631	338	683	686	460	1763	342	1716
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.42	0.17	0.61	0.35	0.52	0.36	0.37	0.27

Intersection Summary	
Cycle Length: 90	
Actuated Cycle Length: 90	
Offset: 43 (48%), Referenced to phase 2:NBT and 6:SBTL, Start of Green	
Natural Cycle: 65	

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Existing AM Peak Hour  
2370 Tenth Line Rd

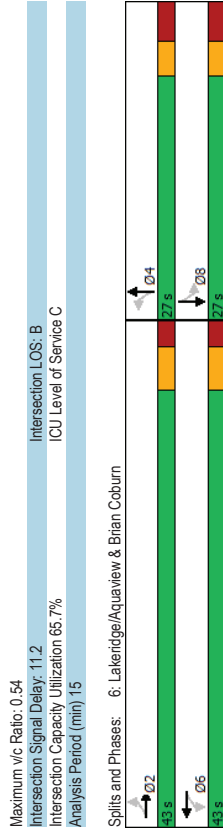
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.04
Intersection Signal Delay: 24.3
Intersection LOS: C
Intersection Capacity Utilization: 61.9%
Analysis Period (min): 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
10	296	50	480	123	23	11	10
11	296	50	480	123	23	11	10
11	363	66	560	137	57	12	51
Perm	NA	Perm	NA	Perm	NA	Perm	NA
2	2	6	6	4	4	8	8
2	2	6	6	4	4	8	8
10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4
43.0	43.0	43.0	43.0	27.0	27.0	27.0	27.0
61.4%	61.4%	61.4%	61.4%	38.6%	38.6%	38.6%	38.6%
37.0	37.0	37.0	20.6	20.6	20.6	20.6	20.6
3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0
2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4
3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
13.0	13.0	13.0	8.0	8.0	8.0	8.0	8.0
0	0	2	2	15	15	0	0
42.2	42.2	42.2	42.2	12.7	12.7	12.7	12.7
0.68	0.68	0.68	0.20	0.20	0.20	0.20	0.20
0.02	0.33	0.09	0.48	0.54	0.17	0.05	0.15
6.5	7.5	6.8	9.3	30.4	12.8	19.7	10.1
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6.5	7.5	6.8	9.3	30.4	12.8	19.7	10.1
A	A	A	A	C	B	B	B
7.4	7.4	9.1	25.3	11.9			
A	A	A	C	B			
0.4	17.2	2.3	31.6	14.1	2.4	1.1	1.0
2.5	39.4	7.8	69.6	28.5	10.0	4.7	8.1
351.9		379.2	249.4				312.2
65.0	65.0	30.0	30.0	30.0	30.0	30.0	30.0
478	1112	618	1161	417	534	376	531
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0.02	0.33	0.09	0.48	0.33	0.11	0.03	0.10



Maximum v/c Ratio: 0.54  
Intersection Signal Delay: 11.2  
Intersection Capacity Utilization 65.7%  
Analysis Period (min) 15  
Intersection LOS: B  
ICU Level of Service C  
Splits and Phases: 6: Lakeridge/Aquaview & Brian Coburn

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Existing AM Peak Hour  
2370 Tenth Line Rd

Existing AM Peak Hour  
2370 Tenth Line Rd

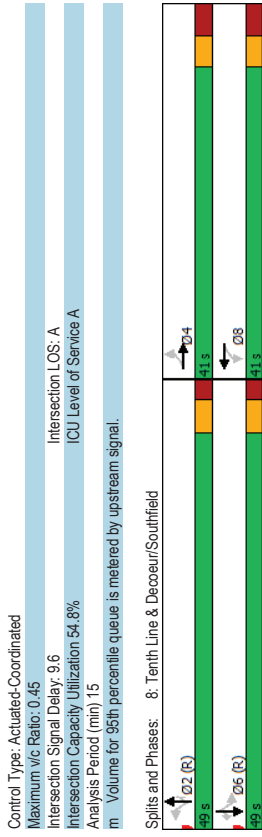
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	30	248	34	410	96	50	25	47
Future Volume (vph)	30	248	34	410	96	50	25	47
Lane Group Flow (vph)	33	333	38	483	107	97	28	102
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	6	4	4	8	8
Permitted Phases	2	2	6	6	4	4	8	8
Detector Phase								
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2
Yellow Time (s)	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	13.0	13.0	13.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	23	23	3	15	15	15	5	5
Act Effr Green (s)	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2
Actuated g/C Ratio	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.09	0.39	0.09	0.54	0.27	0.19	0.08	0.19
Control Delay	10.4	12.2	10.2	15.3	22.3	13.2	19.4	12.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.4	12.2	10.2	15.3	22.3	13.2	19.4	12.0
LOS	B	B	B	B	C	B	B	B
Approach Delay	12.0	12.0	14.9	18.0	13.6	13.6	13.6	13.6
Approach LOS	B	B	B	B	B	B	B	B
Queue Length 50th (m)	2.4	26.3	2.7	44.9	11.8	5.9	2.9	5.4
Queue Length 95th (m)	6.7	43.8	7.2	71.0	24.1	16.1	8.4	15.8
Internal Link Dist (m)	379.2	379.2	585.6	585.6	222.2	222.2	382.8	382.8
Turn Bay Length (m)	65.0	65.0	65.0	30.0	30.0	30.0	30.0	30.0
Base Capacity (vph)	358	864	437	892	391	510	363	537
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.39	0.09	0.54	0.27	0.19	0.08	0.19

Intersection Summary	
Cycle Length: 80	
Actuated Cycle Length: 80	
Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green	
Natural Cycle: 50	

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	86	27	9	29	79	627	1	19	346	59
Future Volume (vph)	86	27	9	29	79	627	1	19	346	59
Lane Group Flow (vph)	96	79	10	110	88	697	1	21	384	66
Turn Type	Perm	NA	Perm	NA	Perm	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	8	8	2	2	2	6	6	6
Detector Phase	4	4	8	8	2	2	2	6	6	6
Switch Phase										
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	49.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%	45.6%	45.6%	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%
Maximum Green (s)	34.1	34.1	34.1	34.1	43.1	43.1	43.1	43.1	43.1	43.1
Yellow Time (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag										
Lead-Lag Optimize?										
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	27.0	27.0	27.0	27.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	2	4	4	4	4	5	5	5
Act Effr Green (s)	16.5	16.5	16.5	16.5	65.3	65.3	65.3	65.3	65.3	65.3
Actuated g/C Ratio	0.18	0.18	0.18	0.18	0.73	0.73	0.73	0.73	0.73	0.73
v/c Ratio	0.45	0.27	0.04	0.32	0.15	0.31	0.00	0.05	0.17	0.07
Control Delay	37.0	14.6	24.7	12.3	8.9	7.7	0.0	6.3	5.5	2.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.0	14.6	24.7	12.3	8.9	7.7	0.0	6.3	5.5	2.2
LOS	D	B	C	B	A	A	A	A	A	A
Approach Delay	26.9		13.4		7.9					5.1
Approach LOS	C		B		A					A
Queue Length 50th (m)	15.8	4.6	1.5	4.9	4.1	19.5	0.0	1.4	14.2	0.0
Queue Length 95th (m)	22.2	12.0	4.3	13.6	18.2	55.1	0.0	m3.4	17.1	2.8
Internal Link Dist (m)	344.3		315.6		346.2				301.3	
Turn Bay Length (m)	45.0	20.0	20.0	90.0	90.0	60.0	60.0	60.0	70.0	70.0
Base Capacity (vph)	439	556	466	633	575	2270	1058	441	2292	1003
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.14	0.02	0.17	0.15	0.31	0.00	0.05	0.17	0.07
Intersection Summary										
Cycle Length: 90										
Actuated Cycle Length: 90										
Offset: 36 (40%), Referenced to phase 2:NBT and 6:SBTL, Start of Green										
Natural Cycle: 70										





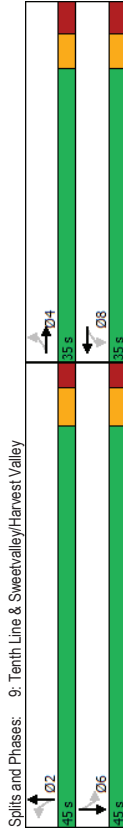
Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	135	3	70	1	5	270	76	274
Future Volume (vph)	135	3	70	1	5	270	76	274
Lane Group Flow (vph)	150	16	78	323	6	337	84	368
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	2	2	6	6
Permitted Phases	4	4	8	8	2	2	6	6
Detector Phase								
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%
Maximum Green (s)	28.5	28.5	28.5	38.8	38.8	38.8	38.8	38.8
Yellow Time (s)	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	2.10	2.10	2.10	2.10	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	1	5	5	0	0	1	1
Act Effr Green (s)	17.9	17.9	17.9	39.1	39.1	39.1	39.1	39.1
Actuated G/C Ratio	0.26	0.26	0.26	0.56	0.56	0.56	0.56	0.56
v/c Ratio	0.82	0.04	0.23	0.53	0.01	0.20	0.16	0.21
Control Delay	56.2	10.6	21.3	6.0	9.6	8.6	10.6	8.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.2	10.6	21.3	6.0	9.6	8.6	10.6	8.2
LOS	E	B	C	A	A	A	B	A
Approach Delay	51.8		9.0		8.6		8.6	
Approach LOS	D		A		A		A	
Queue Length 50th (m)	18.2	0.3	8.0	0.1	0.3	9.4	4.7	9.7
Queue Length 95th (m)	#38.7	4.1	17.3	15.7	2.3	21.3	15.2	22.0
Internal Link Dist (m)	180.2		318.8		263.5		346.2	
Turn Bay Length (m)	38.0		60.0		54.0		65.0	
Base Capacity (vph)	295		597		784		1724	
Starvation Cap Reductn	0		0		0		0	
Spillback Cap Reductn	0		0		0		0	
Storage Cap Reductn	0		0		0		0	
Reduced v/c Ratio	0.51	0.03	0.15	0.41	0.01	0.20	0.16	0.21

Intersection Summary	
Cycle Length: 80	
Actuated Cycle Length: 69.8	
Natural Cycle: 65	
Control Type: Actuated-Uncoordinated	

Maximum v/c Ratio: 0.82
Intersection Signal Delay: 14.0
Intersection LOS: B
Intersection Capacity Utilization 77.1%
ICU Level of Service D
Analysis Period (min) 15
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



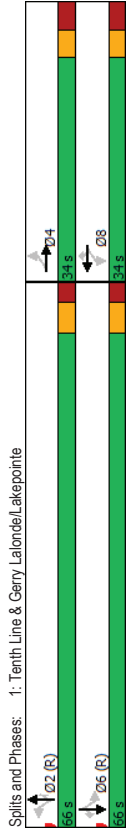
Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	120	98	17	20	117	10	942	48	197	1125
Traffic Volume (vph)	120	98	17	20	117	10	942	48	197	1125
Future Volume (vph)	133	162	19	22	130	11	1047	53	219	1250
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	8	8	8	2	2	6	6	6	6
Protected Phases	4	8	8	8	2	2	6	6	6	6
Detector Phase	4	8	8	8	2	2	6	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	34.0%	34.0%	34.0%	34.0%	66.0%	66.0%	66.0%	66.0%	66.0%	66.0%
Total Split (%)	27.2	27.2	27.2	27.2	59.8	59.8	59.8	59.8	59.8	59.8
Maximum Green (s)	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
Yellow Time (s)	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Total Lost Time (s)	6.2	6.2	6.2	6.2	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag										
Lead-Lag Optimize?	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Recall Mode	None	None	None	None	7.0	7.0	7.0	7.0	7.0	7.0
Walk Time (s)	20.0	20.0	20.0	20.0	14.0	14.0	14.0	14.0	14.0	14.0
Flash Dont Walk (s)	5	5	1	1	5	5	5	4	4	4
Pedestrian Calls (#/hr)	16.7	16.7	16.7	16.7	70.3	70.3	70.3	70.3	70.3	70.3
Act Effr Green (s)	0.17	0.17	0.17	0.17	0.70	0.70	0.70	0.70	0.70	0.70
Actuated G/C Ratio	0.62	0.55	0.11	0.08	0.39	0.05	0.45	0.05	0.73	0.54
v/c Ratio	49.6	38.0	33.1	32.1	13.2	3.8	4.4	0.7	28.9	9.1
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	49.6	38.0	33.1	32.1	13.2	3.8	4.4	0.7	28.9	9.1
Total Delay	49.6	38.0	33.1	32.1	13.2	3.8	4.4	0.7	28.9	9.1
LOS	D	D	C	C	B	A	A	A	C	A
Approach Delay	43.2	17.8			4.2				11.2	
Approach LOS	D	B			A				B	
Queue Length 50th (m)	24.5	25.0	3.2	3.7	4.2	0.4	18.5	0.2	21.3	50.4
Queue Length 95th (m)	38.0	39.1	8.4	9.0	17.5	m0.9	20.8	0.8	#81.1	96.1
Internal Link Dist (m)	372.5		134.8		154.1				468.1	
Turn Bay Length (m)	30.0	50.0	50.0	35.0	55.0		70.0	50.0	50.0	75.0
Base Capacity (vph)	352	466	282	474	474	231	2329	1024	302	2329
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.38	0.35	0.07	0.05	0.27	0.05	0.45	0.05	0.73	0.54

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	90 (90%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	90

Control Type: Actuated-Coordinated	
Maximum v/c Ratio:	0.73
Intersection Signal Delay:	12.1
Intersection LOS:	B
Intersection Capacity Utilization:	72.3%
IOU Level of Service C	
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	





Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Existing PM Peak Hour  
2370 Tenth Line Rd

Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	188	367	58	187	211	143	524	255	744
Future Volume (vph)	188	367	58	187	211	143	524	255	744
Lane Group Flow (vph)	209	646	64	208	234	159	633	283	1017
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA
Protected Phases	4	4	8	8	8	2	2	6	6
Permitted Phase	4	4	8	8	8	2	2	6	6
Detector Phase	4	4	8	8	8	2	2	6	6
Switch Phase									
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	31.4	31.4	31.4	31.4	29.0	29.0	29.0	29.0	29.0
Total Split (s)	47.0	47.0	47.0	47.0	53.0	53.0	53.0	53.0	53.0
Total Split (%)	47.0%	47.0%	47.0%	47.0%	53.0%	53.0%	53.0%	53.0%	53.0%
Maximum Green (s)	40.6	40.6	40.6	40.6	47.0	47.0	47.0	47.0	47.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.7	2.7	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time (s)	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0
Total Lost Time (s)	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0
Lead/Lag									
Lead-Lag Optimize?									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	18.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	1	13	13	7	7	9	9	9
Act Effr Green (s)	39.7	39.7	39.7	39.7	47.9	47.9	47.9	47.9	47.9
Actuated G/C Ratio	0.40	0.40	0.40	0.40	0.48	0.48	0.48	0.48	0.48
v/c Ratio	0.51	0.96	0.71	0.30	0.34	1.08	0.40	0.94	0.66
Control Delay	27.6	55.5	68.9	21.8	5.4	128.7	17.5	56.9	12.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.6	55.5	68.9	21.8	5.4	128.7	17.5	56.9	12.3
LOS	C	E	E	C	A	F	B	E	B
Approach Delay	48.7			20.2			39.8		22.0
Approach LOS	D			C			D		C
Queue Length 50th (m)	29.5	113.1	10.1	26.6	3.0	-35.4	33.4	29.6	74.5
Queue Length 95th (m)	51.4	#184.7	#33.6	43.3	17.4	#70.9	53.7	#96.3	43.8
Internal Link Dist (m)	392.1			351.9			301.3		222.1
Turn Bay Length (m)	45.0	50.0	50.0	45.0	105.0	110.0	110.0	110.0	110.0
Base Capacity (vph)	423	686	82	708	706	147	1570	300	1549
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.94	0.70	0.29	0.33	1.08	0.40	0.94	0.66

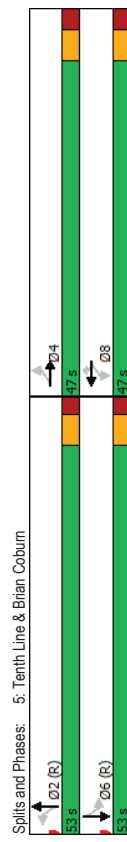
Intersection Summary

Cycle Length: 100  
Actuated Cycle Length: 100  
Offset: 4 (4%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green  
Natural Cycle: 75

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Existing PM Peak Hour  
2370 Tenth Line Rd

Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.08
Intersection Signal Delay: 32.4
Intersection LOS: C
Intersection Capacity Utilization 99.2%
IOU Level of Service F
Analysis Period (min) 15
Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
36	547	31	373	71	19	27	13
36	547	31	373	71	19	27	13
40	689	34	435	79	51	30	30
Perm	NA	Perm	NA	Perm	NA	Perm	NA
2	2	6	6	4	4	8	8
2	2	6	6	4	4	8	8
10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4
54.0	54.0	54.0	54.0	26.0	26.0	26.0	26.0
67.5%	67.5%	67.5%	67.5%	32.5%	32.5%	32.5%	32.5%
48.0	48.0	48.0	19.6	19.6	19.6	19.6	19.6
3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0
2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4
3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Max	Max	Max	None	None	None	None	None
7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
13.0	13.0	13.0	13.0	8.0	8.0	8.0	8.0
5	5	7	7	11	11	2	2
54.8	54.8	54.8	11.3	11.3	11.3	11.3	11.3
0.74	0.74	0.74	0.15	0.15	0.15	0.15	0.15
0.06	0.54	0.08	0.34	0.41	0.19	0.17	0.12
4.8	8.1	5.1	5.8	34.7	16.4	28.9	17.6
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4.8	8.1	5.1	5.8	34.7	16.4	28.9	17.6
A	A	A	A	C	B	C	B
7.9	5.8	5.8	27.5	23.2	23.2	23.2	23.2
A	A	A	A	C	C	C	C
1.4	38.9	1.3	19.9	9.7	2.4	3.5	1.6
5.2	81.9	4.8	41.7	21.4	10.9	10.3	8.0
351.9	379.2	379.2	249.4	312.2	312.2	312.2	312.2
65.0	65.0	65.0	30.0	30.0	30.0	30.0	30.0
644	1272	448	1275	334	434	300	433
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0
0.06	0.54	0.08	0.34	0.24	0.12	0.10	0.07

Intersection Summary  
Cycle Length: 80  
Actuated Cycle Length: 73.8  
Natural Cycle: 60  
Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.54  
Intersection Signal Delay: 9.7  
Intersection LOS: A  
ICU Level of Service B  
Intersection Capacity Utilization 56.8%  
Analysis Period (min) 15  
Splits and Phases: 6: Lakeridge/Aquaview & Brian Coburn  
54.1 s ← D2  
54.1 s → D6  
26.5 s ← D4  
26.5 s → D8

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Existing PM Peak Hour  
2370 Tenth Line Rd

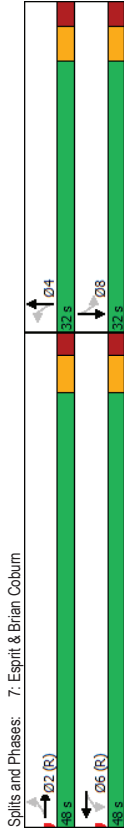
Existing PM Peak Hour  
2370 Tenth Line Rd

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	52	426	27	315	70	38	23	38
Future Volume (vph)	52	426	27	315	70	38	23	38
Lane Group Flow (vph)	58	609	30	370	78	70	26	84
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	6	4	4	8	8
Detector Phase	2	2	6	6	4	4	8	8
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2
Yellow Time (s)	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	13.0	13.0	13.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	6	6	1	1	1	7	7
Act Effr Green (s)	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2
Actuated g/C Ratio	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.13	0.68	0.11	0.41	0.20	0.13	0.06	0.16
Control Delay	10.7	18.1	11.0	13.1	21.1	13.6	19.2	12.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.7	18.1	11.0	13.1	21.1	13.6	19.2	12.0
LOS	B	B	B	C	B	B	B	B
Approach Delay	17.4	12.9		17.5			13.7	
Approach LOS	B	B		B			B	
Queue Length 50th (m)	4.2	61.0	2.2	31.4	8.4	4.4	2.7	4.4
Queue Length 95th (m)	10.2	97.2	6.5	50.6	18.3	12.9	8.0	13.8
Internal Link Dist (m)	379.2		585.6		222.2		382.8	
Turn Bay Length (m)	65.0	65.0	270	901	395	524	405	514
Base Capacity (vph)	447	899	270	901	395	524	405	514
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.68	0.11	0.41	0.20	0.13	0.06	0.16

Intersection Summary

Cycle Length: 80  
Actuated Cycle Length: 80  
Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green  
Natural Cycle: 60

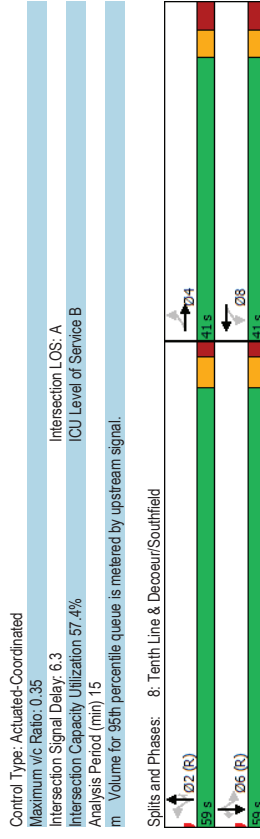
Control Type	Actuated-Coordinated
Maximum v/c Ratio	0.68
Intersection Signal Delay	15.8
Intersection LOS	B
IOU Level of Service C	
Intersection Capacity Utilization	69.7%
Analysis Period (min)	15



Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	47	16	2	24	34	600	14	116	804	94
Future Volume (vph)	47	16	2	24	34	600	14	116	804	94
Lane Group Flow (vph)	52	51	2	88	38	667	16	129	893	104
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	2	2	2	2	6	6
Permitted Phases	4	4	8	8	2	2	2	2	6	6
Detector Phase										
Switch Phase										
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	59.0	59.0	59.0	59.0	59.0	59.0
Total Split (%)	41.0%	41.0%	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%	59.0%	59.0%
Maximum Green (s)	34.1	34.1	34.1	34.1	53.1	53.1	53.1	53.1	53.1	53.1
All-Red Time (s)	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag										
Lead-Lag Optimize?										
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	27.0	27.0	27.0	27.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	1	0	0	0	0	0	0	0	0
Act Effr Green (s)	15.1	15.1	15.1	15.1	76.7	76.7	76.7	76.7	76.7	76.7
Actuated G/C Ratio	0.15	0.15	0.15	0.15	0.77	0.77	0.77	0.77	0.77	0.77
v/c Ratio	0.28	0.19	0.01	0.31	0.09	0.26	0.01	0.24	0.35	0.09
Control Delay	38.3	17.1	29.0	15.8	7.8	6.2	0.5	4.8	3.8	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.3	17.1	29.0	15.8	7.8	6.2	0.5	4.8	3.8	0.4
LOS	D	B	C	B	A	A	A	A	A	A
Approach Delay		27.8		16.1		6.1				3.6
Approach LOS		C		B		A				A
Queue Length 50th (m)	9.6	3.2	0.4	4.9	1.5	15.6	0.0	3.5	14.0	0.0
Queue Length 95th (m)	15.5	10.1	1.8	13.8	9.2	50.2	0.6	m10.5	m32.0	m0.0
Internal Link Dist (m)		344.3		315.6		346.2				301.3
Turn Bay Length (m)	45.0	20.0	20.0	90.0	90.0	60.0	60.0	60.0	70.0	70.0
Base Capacity (vph)	416	554	430	573	406	2542	1147	529	2542	1124
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.09	0.00	0.15	0.09	0.26	0.01	0.24	0.35	0.09
<b>Intersection Summary</b>										
Cycle Length: 100										
Actuated Cycle Length: 100										
Offset: 21 (21%), Referenced to phase 2:NBT and 6:SBTL, Start of Green										
Natural Cycle: 70										



Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	95	2	17	1	14	373	293	374
Traffic Volume (vph)	95	2	17	1	14	373	293	374
Future Volume (vph)	106	10	19	187	16	504	326	603
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	8	8	2	2	6	6	6
Protected Phases	4	8	8	2	2	6	6	6
Permitted Phases	4	8	8	2	2	6	6	6
Detector Phase	4	8	8	2	2	6	6	6
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	65.0	65.0	65.0	65.0	65.0
Total Split (%)	35.0%	35.0%	35.0%	65.0%	65.0%	65.0%	65.0%	65.0%
Maximum Green (s)	28.5	28.5	28.5	58.8	58.8	58.8	58.8	58.8
Yellow Time (s)	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	2.10	2.10	2.10	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	2	2	0	0	0	0	0
Act Effr Green (s)	15.7	15.7	15.7	60.8	60.8	60.8	60.8	60.8
Actuated G/C Ratio	0.18	0.18	0.18	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.62	0.04	0.09	0.46	0.03	0.23	0.59	0.28
Control Delay	49.3	17.4	29.2	8.4	6.9	5.9	14.9	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	17.4	29.2	8.4	6.9	5.9	14.9	5.4
LOS	D	B	C	A	A	B	A	A
Approach Delay	46.5	10.3	10.3	5.9	8.7	8.7	8.7	8.7
Approach LOS	D	B	B	A	A	A	A	A
Queue Length 50th (m)	16.5	0.3	2.7	0.1	0.7	12.2	23.6	12.7
Queue Length 95th (m)	32.0	4.1	8.1	15.5	3.9	29.0	75.6	31.2
Internal Link Dist (m)	180.2			318.8		263.5		346.2
Turn Bay Length (m)	38.0	60.0	60.0	54.0	65.0	65.0	65.0	65.0
Base Capacity (vph)	311	493	404	595	504	2212	555	2178
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.02	0.05	0.31	0.03	0.23	0.59	0.28
Intersection Summary								
Cycle Length: 100								
Actuated Cycle Length: 89.2								
Natural Cycle: 80								
Control Type: Actuated-Uncoordinated								

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	95	2	17	1	14	373	293	374
Traffic Volume (vph)	95	2	17	1	14	373	293	374
Future Volume (vph)	106	10	19	187	16	504	326	603
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	8	8	2	2	6	6	6
Protected Phases	4	8	8	2	2	6	6	6
Permitted Phases	4	8	8	2	2	6	6	6
Detector Phase	4	8	8	2	2	6	6	6
Switch Phase								
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	65.0	65.0	65.0	65.0	65.0
Total Split (%)	35.0%	35.0%	35.0%	65.0%	65.0%	65.0%	65.0%	65.0%
Maximum Green (s)	28.5	28.5	28.5	58.8	58.8	58.8	58.8	58.8
Yellow Time (s)	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2
Lead/Lag								
Lead-Lag Optimize?								
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	2.10	2.10	2.10	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	1	2	2	0	0	0	0	0
Act Effr Green (s)	15.7	15.7	15.7	60.8	60.8	60.8	60.8	60.8
Actuated G/C Ratio	0.18	0.18	0.18	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.62	0.04	0.09	0.46	0.03	0.23	0.59	0.28
Control Delay	49.3	17.4	29.2	8.4	6.9	5.9	14.9	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	17.4	29.2	8.4	6.9	5.9	14.9	5.4
LOS	D	B	C	A	A	B	A	A
Approach Delay	46.5	10.3	10.3	5.9	8.7	8.7	8.7	8.7
Approach LOS	D	B	B	A	A	A	A	A
Queue Length 50th (m)	16.5	0.3	2.7	0.1	0.7	12.2	23.6	12.7
Queue Length 95th (m)	32.0	4.1	8.1	15.5	3.9	29.0	75.6	31.2
Internal Link Dist (m)	180.2			318.8		263.5		346.2
Turn Bay Length (m)	38.0	60.0	60.0	54.0	65.0	65.0	65.0	65.0
Base Capacity (vph)	311	493	404	595	504	2212	555	2178
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.02	0.05	0.31	0.03	0.23	0.59	0.28
Intersection Summary								
Cycle Length: 100								
Actuated Cycle Length: 89.2								
Natural Cycle: 80								
Control Type: Actuated-Uncoordinated								



## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Gerry Lalonde AM Existing]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	92	2.0	0.195	9.2	LOSA	1.1	7.8	0.51	0.62	0.51	50.2	
2	T1	23	2.0	0.195	4.0	LOSA	1.1	7.8	0.51	0.62	0.51	47.0	
3	R2	87	2.0	0.195	4.4	LOSA	1.1	7.8	0.51	0.62	0.51	49.0	
Approach													
		202	2.0	0.195	6.5	LOSA	1.1	7.8	0.51	0.62	0.51	49.3	
East: Brian Coburn													
4	L2	49	2.0	0.712	10.4	LOSB	8.0	57.2	0.65	0.54	0.65	51.0	
5	T1	871	2.0	0.712	5.1	LOSA	8.0	57.2	0.65	0.54	0.65	54.3	
6	R2	14	2.0	0.712	5.2	LOSA	8.0	57.2	0.65	0.54	0.65	49.4	
Approach													
		934	2.0	0.712	5.3	LOSA	8.0	57.2	0.65	0.54	0.65	54.1	
North: Gerry Lalonde													
7	L2	8	2.0	0.414	17.9	LOSB	3.3	23.4	0.97	1.02	1.07	46.1	
8	T1	9	2.0	0.414	12.7	LOSB	3.3	23.4	0.97	1.02	1.07	43.3	
9	R2	182	2.0	0.414	13.1	LOSB	3.3	23.4	0.97	1.02	1.07	46.0	
Approach													
		199	2.0	0.414	13.2	LOSB	3.3	23.4	0.97	1.02	1.07	45.0	
West: Brian Coburn													
10	L2	32	2.0	0.272	9.2	LOSA	1.8	12.8	0.25	0.42	0.25	52.6	
11	T1	297	2.0	0.272	3.9	LOSA	1.8	12.8	0.25	0.42	0.25	56.1	
12	R2	53	2.0	0.272	4.0	LOSA	1.8	12.8	0.25	0.42	0.25	50.8	
Approach													
		382	2.0	0.272	4.3	LOSA	1.8	12.8	0.25	0.42	0.25	55.0	
All Vehicles		1718	2.0	0.712	6.2	LOSA	8.0	57.2	0.58	0.58	0.59	52.4	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Aqgelik, M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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2021-052 Sidra 2021-10-05.sp8

## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg AM Existing]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Aubepines													
1	L2	121	2.0	0.197	9.4	LOSA	1.1	7.9	0.53	0.65	0.53	49.6	
2	T1	17	2.0	0.197	4.2	LOSA	1.1	7.9	0.53	0.65	0.53	46.5	
3	R2	61	2.0	0.197	4.6	LOSA	1.1	7.9	0.53	0.65	0.53	48.4	
Approach													
		199	2.0	0.197	7.5	LOSA	1.1	7.9	0.53	0.65	0.53	49.0	
East: Brian Coburn													
4	L2	36	2.0	0.595	10.1	LOSB	5.6	40.2	0.55	0.51	0.55	51.4	
5	T1	722	2.0	0.595	4.7	LOSA	5.6	40.2	0.55	0.51	0.55	54.8	
6	R2	13	2.0	0.595	4.8	LOSA	5.6	40.2	0.55	0.51	0.55	49.8	
Approach													
		771	2.0	0.595	5.0	LOSA	5.6	40.2	0.55	0.51	0.55	54.5	
North: Strasbourg													
7	L2	28	2.0	0.227	13.9	LOSB	1.5	10.9	0.85	0.84	0.85	48.1	
8	T1	24	2.0	0.227	8.7	LOSA	1.5	10.9	0.85	0.84	0.85	45.1	
9	R2	84	2.0	0.227	9.1	LOSA	1.5	10.9	0.85	0.84	0.85	47.0	
Approach													
		137	2.0	0.227	10.0	LOSA	1.5	10.9	0.85	0.84	0.85	46.9	
West: Brian Coburn													
10	L2	8	2.0	0.281	9.3	LOSA	1.9	13.2	0.30	0.42	0.30	52.6	
11	T1	330	2.0	0.281	4.0	LOSA	1.9	13.2	0.30	0.42	0.30	56.1	
12	R2	42	2.0	0.281	4.1	LOSA	1.9	13.2	0.30	0.42	0.30	50.8	
Approach													
		380	2.0	0.281	4.1	LOSA	1.9	13.2	0.30	0.42	0.30	55.4	
All Vehicles		1487	2.0	0.595	5.6	LOSA	5.6	40.2	0.51	0.54	0.51	53.1	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Aqgelik, M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Gerry Lalonde PM Existing]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	41	2.0	0.240	18.0	1.8	12.7	0.98	0.95	0.98	45.0		
2	T1	11	2.0	0.240	12.8	1.8	12.7	0.98	0.95	0.98	42.4		
3	R2	40	2.0	0.240	13.2	1.8	12.7	0.98	0.95	0.98	44.1		
Approach		92	2.0	0.240	15.3	1.8	12.7	0.98	0.95	0.98	44.3		
East: Brian Coburn													
4	L2	69	2.0	0.495	10.7	3.9	27.7	0.62	0.60	0.62	51.0		
5	T1	468	2.0	0.495	5.4	3.9	27.7	0.62	0.60	0.62	54.2		
6	R2	13	2.0	0.495	5.5	3.9	27.7	0.62	0.60	0.62	49.3		
Approach		550	2.0	0.495	6.1	3.9	27.7	0.62	0.60	0.62	53.7		
North: Gerry Lalonde													
7	L2	4	2.0	0.138	10.6	0.8	6.0	0.68	0.67	0.68	50.7		
8	T1	20	2.0	0.138	5.4	0.8	6.0	0.68	0.67	0.68	47.4		
9	R2	87	2.0	0.138	5.8	0.8	6.0	0.68	0.67	0.68	49.4		
Approach		111	2.0	0.138	5.9	0.8	6.0	0.68	0.67	0.68	49.1		
West: Brian Coburn													
10	L2	208	2.0	0.827	10.3	14.0	99.8	0.72	0.51	0.72	50.5		
11	T1	900	2.0	0.827	4.9	14.0	99.8	0.72	0.51	0.72	53.8		
12	R2	72	2.0	0.827	5.0	14.0	99.8	0.72	0.51	0.72	48.9		
Approach		1180	2.0	0.827	5.9	14.0	99.8	0.72	0.51	0.72	52.9		
All Vehicles		1933	2.0	0.827	6.4	14.0	99.8	0.70	0.56	0.70	52.4		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg PM Existing]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Aubepines													
1	L2	64	2.0	0.194	13.2	1.3	9.1	0.82	0.83	0.82	47.5		
2	T1	20	2.0	0.194	8.1	1.3	9.1	0.82	0.83	0.82	44.7		
3	R2	38	2.0	0.194	8.5	1.3	9.1	0.82	0.83	0.82	46.5		
Approach		122	2.0	0.194	10.9	1.3	9.1	0.82	0.83	0.82	46.7		
East: Brian Coburn													
4	L2	60	2.0	0.427	9.7	3.3	23.6	0.42	0.48	0.42	51.8		
5	T1	458	2.0	0.427	4.3	3.3	23.6	0.42	0.48	0.42	55.2		
6	R2	40	2.0	0.427	4.4	3.3	23.6	0.42	0.48	0.42	50.1		
Approach		558	2.0	0.427	4.9	3.3	23.6	0.42	0.48	0.42	54.5		
North: Strasbourg													
7	L2	26	2.0	0.078	10.5	0.4	3.2	0.63	0.66	0.63	49.6		
8	T1	14	2.0	0.078	5.3	0.4	3.2	0.63	0.66	0.63	46.5		
9	R2	26	2.0	0.078	5.7	0.4	3.2	0.63	0.66	0.63	48.4		
Approach		66	2.0	0.078	7.5	0.4	3.2	0.63	0.66	0.63	48.4		
West: Brian Coburn													
10	L2	37	2.0	0.664	9.8	7.0	49.9	0.49	0.47	0.49	51.7		
11	T1	778	2.0	0.664	4.5	7.0	49.9	0.49	0.47	0.49	55.1		
12	R2	118	2.0	0.664	4.6	7.0	49.9	0.49	0.47	0.49	50.0		
Approach		932	2.0	0.664	4.7	7.0	49.9	0.49	0.47	0.49	54.3		
All Vehicles		1678	2.0	0.664	5.3	7.0	49.9	0.50	0.50	0.50	53.5		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Appendix D

Existing Background Development Volumes

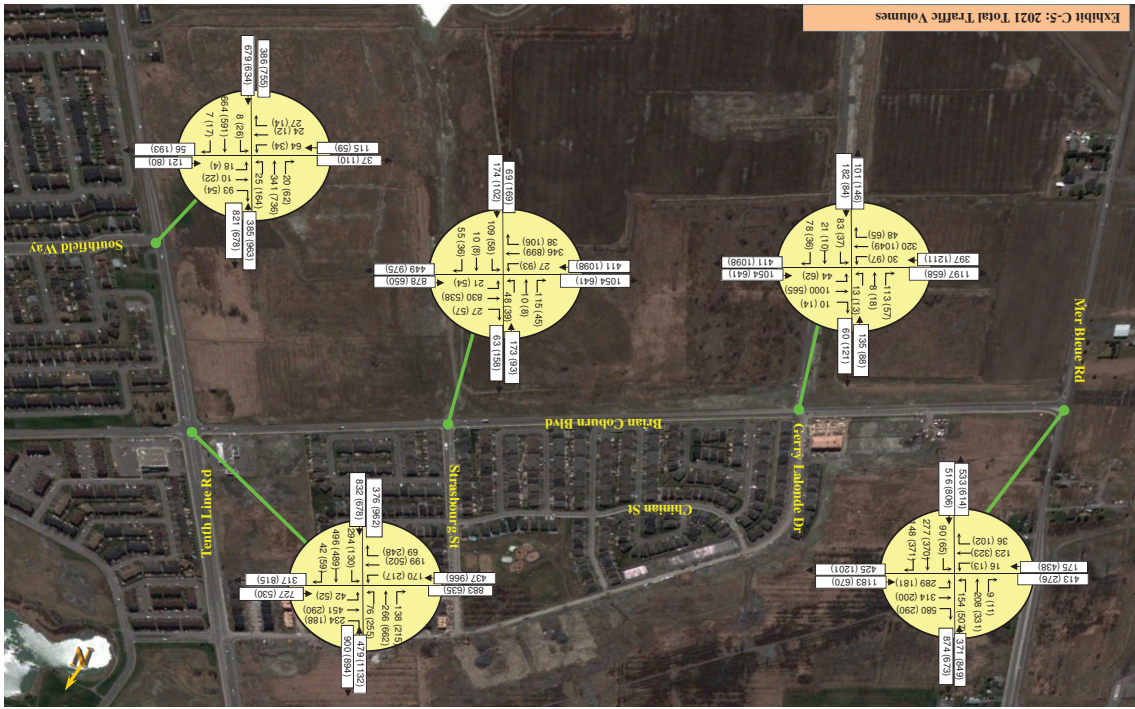
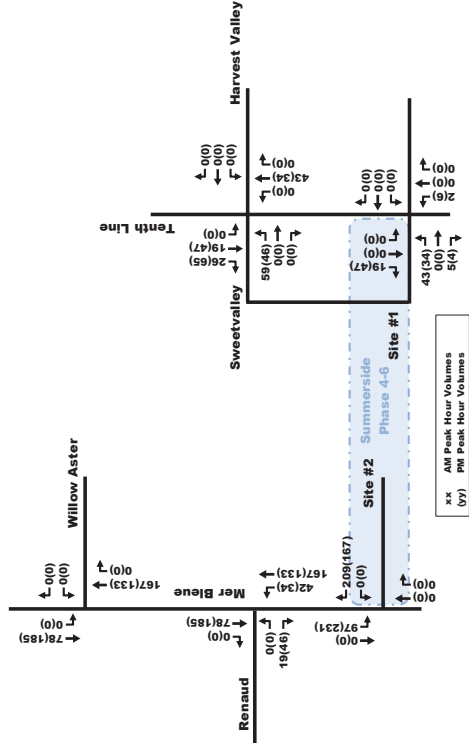


Figure 7: Total 'New' 2024 Site-Generated Traffic Volumes



# Appendix E

Collision Data

Accident Date	Accident Year	Accident Time	Location	Environment Condition	Light	Traffic Control	Traffic Control Condition	Classification Of Accident	Initial Impact Type	Road Surface Condition
2015-02-02	2015	14:19	BRIAN COBURN BLVD @ STRASBOURG ST	01 - Clear	01 - Daylight	02 - Stop sign		03 - P.D. only	03 - Rear end	03 - Loose snow
2015-09-29	2016	16:11	BRIAN COBURN BLVD @ STRASBOURG ST	01 - Clear	01 - Daylight	02 - Stop sign		03 - P.D. only	03 - Rear end	01 - Dry
2017-02-24	2017	14:28	BRIAN COBURN BLVD @ STRASBOURG ST	01 - Clear	01 - Daylight	02 - Stop sign		03 - P.D. only	03 - Rear end	02 - Wet
2017-04-03	2017	7:23	BRIAN COBURN BLVD @ STRASBOURG ST	01 - Clear	01 - Daylight	02 - Stop sign		02 - Non-fatal injury	02 - Angle	01 - Dry
2019-01-12	2019	18:25	BRIAN COBURN BLVD @ STRASBOURG ST (0013501)	01 - Clear	07 - Dark	02 - Stop sign		03 - P.D. only	02 - Angle	01 - Dry
2019-10-18	2019	19:28	BRIAN COBURN BLVD @ STRASBOURG ST (0013501)	01 - Clear	07 - Dark	11 - Roundabout		03 - P.D. only	02 - Angle	01 - Dry
2019-12-11	2019	18:14	BRIAN COBURN BLVD @ STRASBOURG ST (0013501)	01 - Clear	07 - Dark	02 - Stop sign		03 - P.D. only	03 - Rear end	03 - Loose snow
2015-08-07	2015	9:50	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-11-17	2015	7:36	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-12-28	2015	14:25	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-09-20	2015	14:25	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2015-10-09	2015	7:10	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	03 - Dawn	01 - Traffic signal		03 - P.D. only	05 - Turning movement	02 - Wet
2015-11-27	2015	19:20	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	02 - Wet
2016-05-01	2016	15:44	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	02 - Wet
2016-12-01	2016	18:36	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	02 - Wet
2016-12-01	2016	19:34	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	02 - Wet
2016-12-01	2016	19:34	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	02 - Wet
2016-12-31	2016	15:11	BRIAN COBURN BLVD @ TEMPLE LINE RD	03 - Snow	05 - Dusk	01 - Traffic signal		03 - P.D. only	05 - Turning movement	03 - Loose snow
2017-02-28	2017	13:21	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2017-02-29	2017	19:08	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-03-04	2017	3:40	BRIAN COBURN BLVD @ TEMPLE LINE RD	04 - Freezing Rain	07 - Dark	01 - Traffic signal		03 - P.D. only	07 - SMV other	02 - Wet
2017-04-13	2017	10:19	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-04-22	2017	18:55	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	03 - Rear end	01 - Dry
2017-04-29	2017	14:30	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2017-04-30	2017	20:45	BRIAN COBURN BLVD @ TEMPLE LINE RD	02 - Rain	07 - Dark	01 - Traffic signal		03 - P.D. only	05 - Turning movement	02 - Wet
2017-05-18	2017	14:58	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-07-26	2017	13:21	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-07-30	2017	15:08	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-09-01	2017	8:22	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	01 - Dry
2017-10-18	2017	7:15	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	07 - SMV other	01 - Dry
2017-10-28	2017	8:30	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	05 - Dusk	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	06 - Ice
2018-02-15	2018	17:20	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	06 - Ice
2018-02-15	2018	10:27	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	02 - Rain	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2018-04-17	2018	11:57	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	06 - Ice
2018-06-24	2018	21:24	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	07 - SMV other	01 - Dry
2018-07-25	2018	11:42	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	02 - Rain	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	07 - SMV other	02 - Wet
2018-10-15	2018	15:40	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	01 - Dry
2018-10-19	2018	10:05	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-11-17	2018	14:50	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	02 - Wet
2019-01-06	2019	5:24	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	02 - Angle	01 - Dry
2019-01-06	2019	10:11	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	02 - Wet
2019-02-32	2019	15:50	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2019-04-01	2019	16:50	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2019-04-05	2019	16:29	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	99 - Other	01 - Dry
2019-04-18	2019	6:10	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	02 - Rain	03 - Dawn	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	02 - Wet
2019-05-16	2019	13:57	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2019-06-16	2019	15:00	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2019-07-15	2019	7:15	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	02 - Rain	07 - Dark	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	02 - Wet
2019-07-27	2019	21:45	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		02 - Non-fatal injury	05 - Turning movement	01 - Dry
2019-08-09	2019	12:00	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	04 - Sideswipe	01 - Dry
2019-12-19	2019	8:25	BRIAN COBURN BLVD @ TEMPLE LINE RD (0013966)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	03 - Rear end	01 - Dry
2017-06-26	2017	11:06	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		02 - Non-fatal injury	03 - Rear end	01 - Dry
2018-01-02	2018	14:30	BRIAN COBURN BLVD @ TEMPLE LINE RD	03 - Snow	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	03 - Loose snow
2019-01-15	2019	7:30	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry
2019-05-23	2019	8:23	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry
2019-05-23	2019	8:23	BRIAN COBURN BLVD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry
2019-07-25	2019	13:32	DECOEUR DR @ SOUTHFIELD WAY @ TEMPLE LINE RD (0033551)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2019-07-25	2019	13:32	DECOEUR DR @ SOUTHFIELD WAY @ TEMPLE LINE RD (0033551)	01 - Clear	01 - Daylight	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2019-10-18	2019	21:00	DECOEUR DR @ SOUTHFIELD WAY @ TEMPLE LINE RD (0033551)	01 - Clear	07 - Dark	01 - Traffic signal		03 - P.D. only	05 - Turning movement	01 - Dry
2018-07-13	2018	11:55	DECOEUR DR @ SOUTHFIELD WAY @ TEMPLE LINE RD (0033551)	01 - Clear	07 - Dark	10 - No control		03 - P.D. only	07 - SMV other	01 - Dry
2016-02-25	2016	17:57	TEMPLE LINE RD @ TEMPLE LINE RD	03 - Snow	07 - Dark	10 - No control		03 - P.D. only	07 - SMV other	06 - Ice
2017-02-09	2017	16:16	TEMPLE LINE RD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	01 - Approaching	01 - Dry
2017-04-24	2017	13:42	TEMPLE LINE RD @ TEMPLE LINE RD	01 - Clear	01 - Daylight	10 - No control		03 - P.D. only	03 - Rear end	01 - Dry

# Appendix F

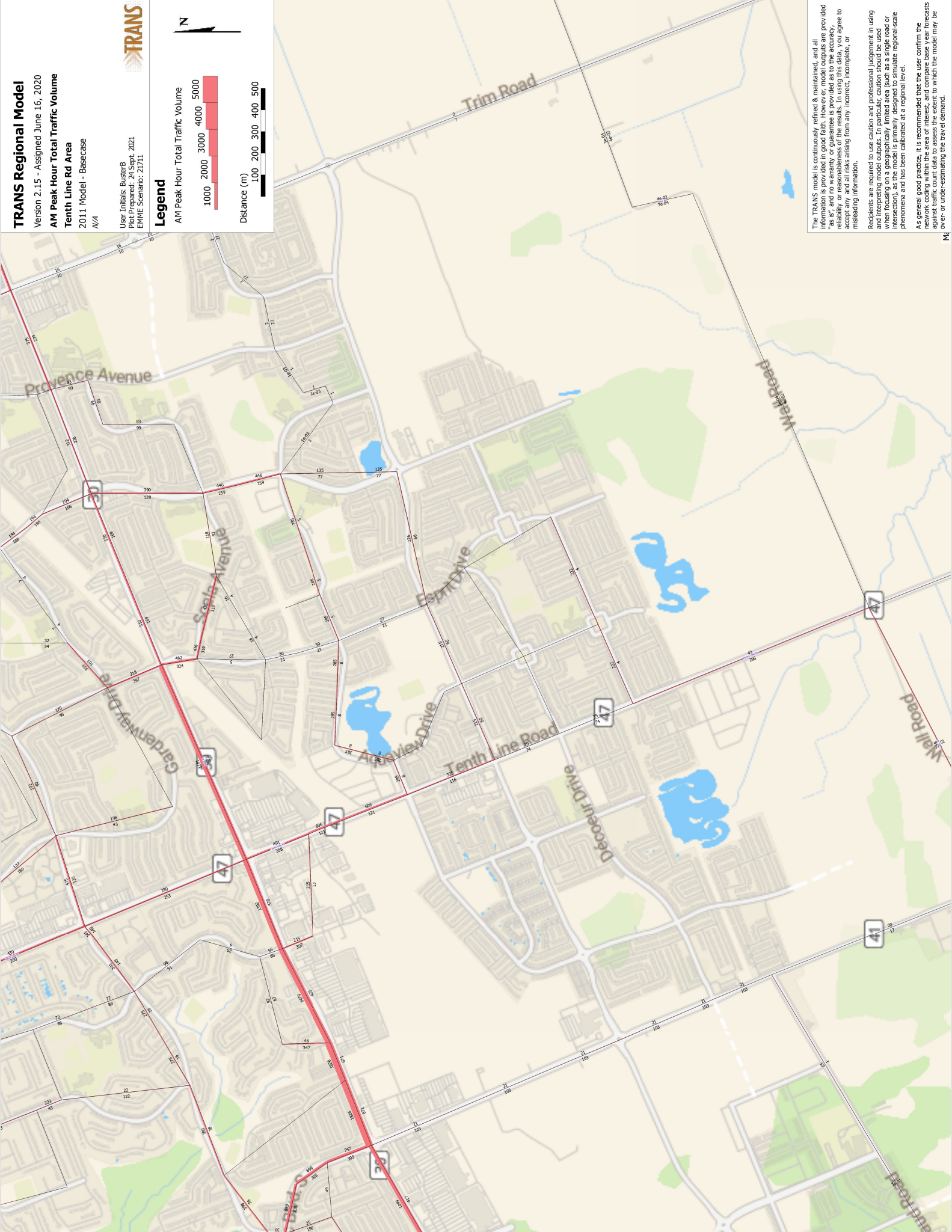
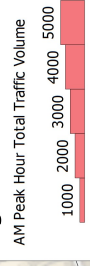
TRANS Model Plots

**TRANS Regional Model**  
 Version 2.15 - Assigned June 16, 2020  
**AM Peak Hour Total Traffic Volume**  
**Tenth Line Rd Area**  
 2011 Model - Basecase  
 N/A

User Initial: BuserB  
 Plot Prepared: 24 Sept. 2021  
 Exhibit Scenario: 21711



**Legend**



The TRANS model is continuously refined & maintained, and all model outputs are provided as advisory information only. The model is provided "as is" and no warranty, guarantee is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.

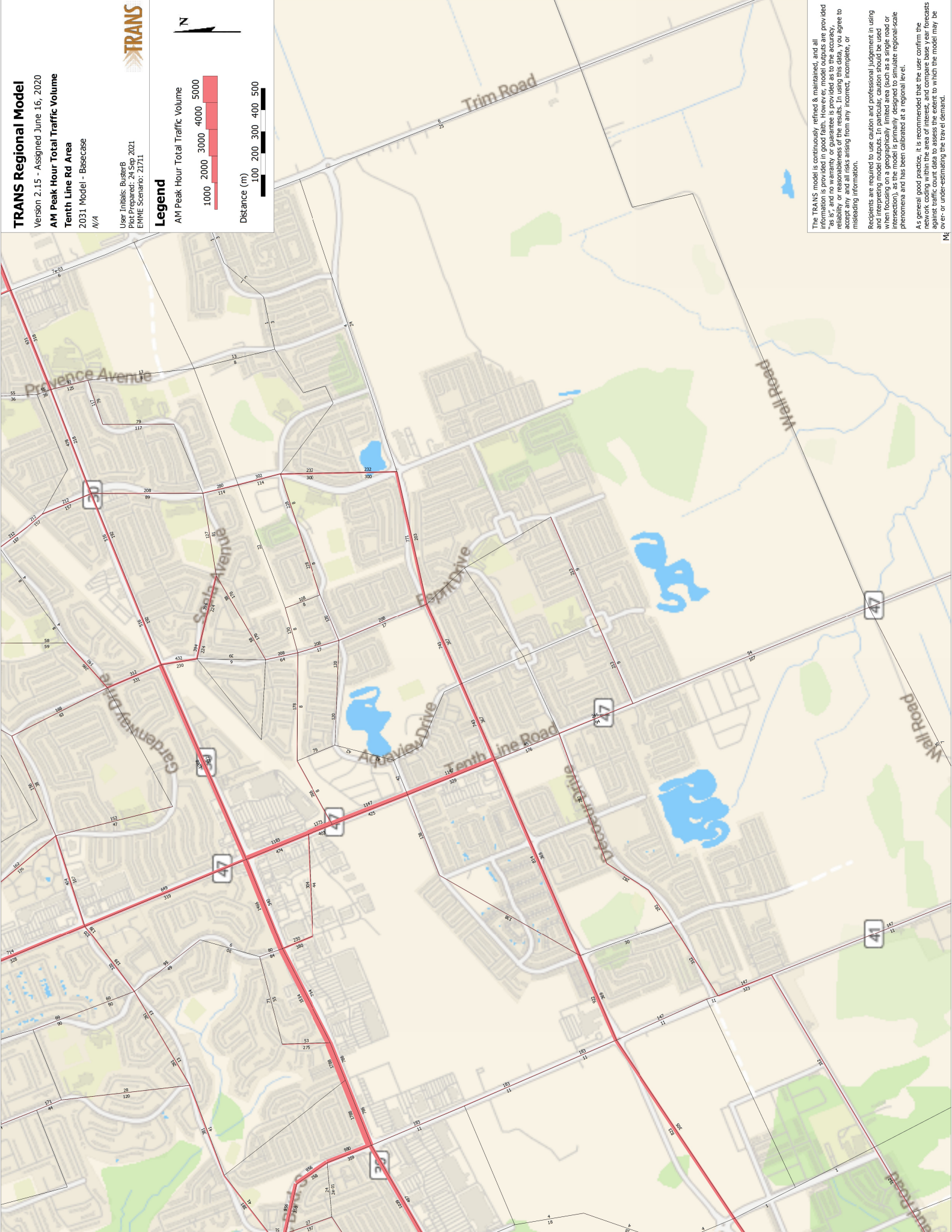
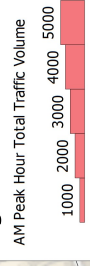


**TRANS Regional Model**  
 Version 2.15 - Assigned June 16, 2020  
**AM Peak Hour Total Traffic Volume**  
**Tenth Line Rd Area**  
 2031 Model - Basecase  
 N/A

User: Inhibe, BusterB  
 Plot Prepared: 24 Sep 2021  
 EIR/IEC Scenario: 21711



**Legend**



The TRANS model is continuously refined & maintained, and all model outputs are provided as advisory information only. No warranty, express or implied, is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As a general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be over- or under-estimating the travel demand.

# Appendix G

Future Background Development Volumes



Table 3: Revised Modified Person Trip Generation (ITE)

Land Use	Units	AM Peak (persons/h)		PM Peak (persons/h)	
		In	Out	In	Out
Single Family Home	372 du	108	243	294	152
Townhouse/Stacked Townhouse	194 du	19	95	90	45
<b>Total Person Trips</b>		<b>127</b>	<b>338</b>	<b>384</b>	<b>197</b>
			<b>465</b>	<b>581</b>	<b>581</b>

Table 4: Revised Site Trip Generation (ITE)

Travel Mode	Mode Share	AM Peak (persons/h)		PM Peak (persons/h)	
		In	Out	In	Out
Auto Driver	55%	70	186	212	109
Auto Passenger	15%	20	51	58	30
Transit	20%	25	68	93	39
Non-motorized	10%	12	33	45	19
Total Person Trips	100%	127	338	465	197
<b>Total 'New' Auto Trips</b>		<b>70</b>	<b>186</b>	<b>212</b>	<b>109</b>
			<b>256</b>	<b>321</b>	<b>321</b>

The trip generation for the revised site plan forecasts an additional 67 two-way people trips during the AM peak and 84 two-way people trips during the PM peak. Of these trips, the updated modal splits will see an additional 20 inbound auto trips and a reduction of 4 outbound auto trips during the AM peak, and 18 additional inbound auto trips and 4 outbound auto trips during the PM peak.

Transit trips are noted to increase during the AM Peak for inbound service, and reduce for the AM outbound trips and both inbound and outbound trips during the PM peak.

The following Figure 7 depicts 'new' site-generated trips assigned to the study area network, based on the above distribution.

Figure 7: 'New' Site-Generated Traffic Volumes

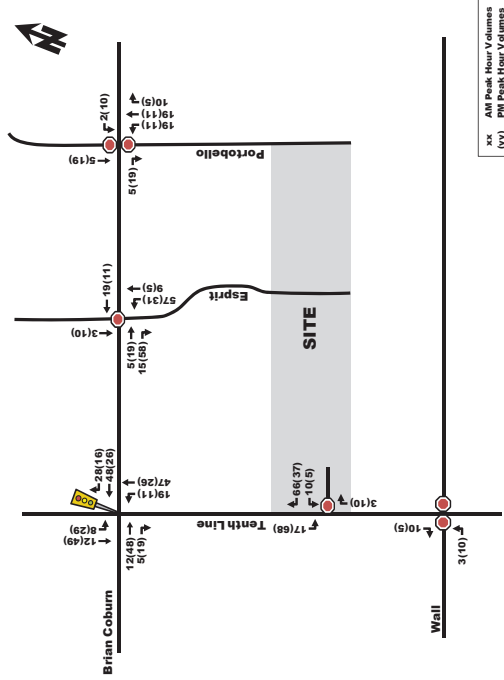
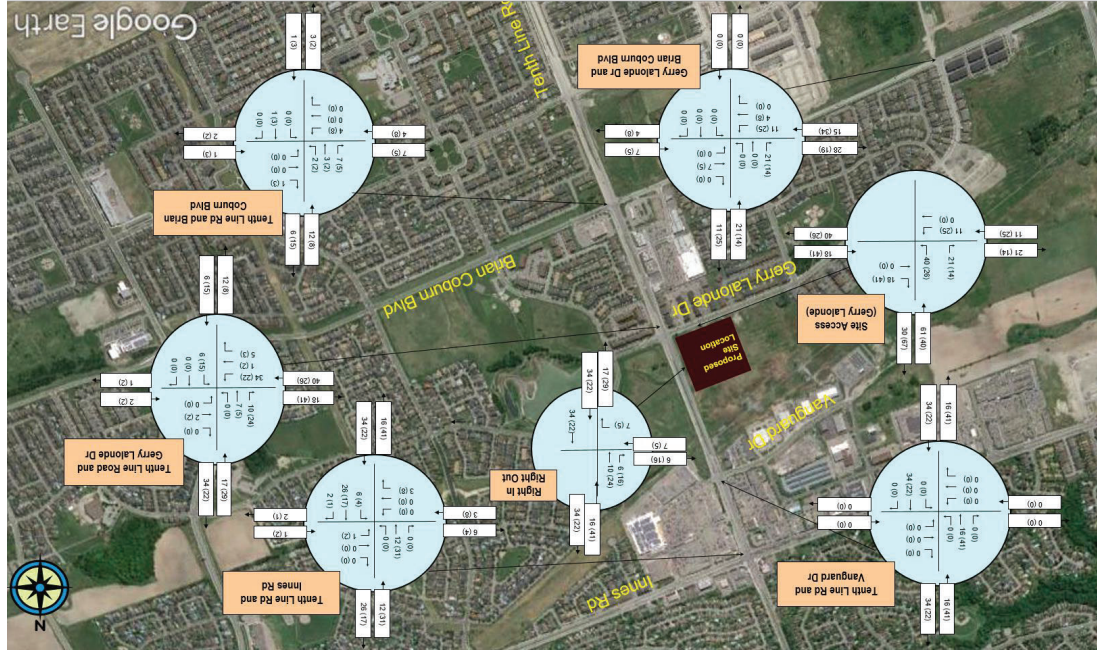
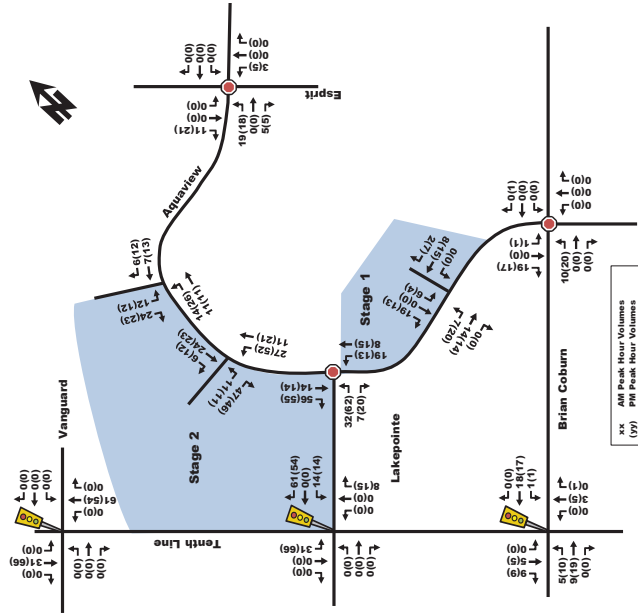


Figure 7: New Total Site-Generated Traffic Volumes



Transportation Impact Assessment Analysis and Strategy Report

Exhibit 4-4: Net Site Auto Generated Travel Demand - Full Build Out (AM (PM))

# Appendix H

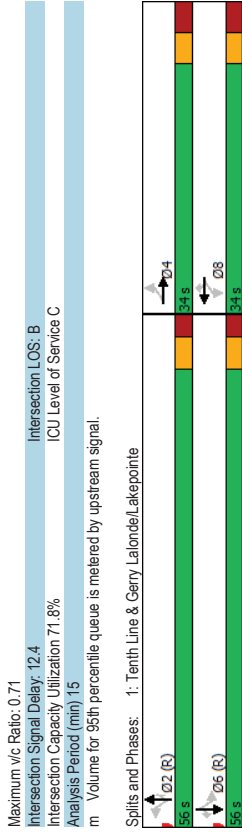
Synchro and Sidra Worksheets – 2026 Future Background Conditions

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	170	19	40	44	60	231	22	992	15	79	541	73
Traffic Volume (vph)	170	19	40	44	60	231	22	992	15	79	541	73
Future Volume (vph)	1566	1483	0	1642	1745	1483	1496	3283	1483	1626	3191	1441
Satd. Flow (prot)	0.718			0.719			0.450			0.256		
FI Permitted	1179	1483	0	1240	1745	1460	706	3283	1442	438	3191	1400
Satd. Flow (perm)	40			100			46					
Satd. Flow (RTOR)	170	59	0	44	60	231	22	992	15	79	541	73
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	8	8	8	8	2	2	2	6	6	6
Permitted Phases	4	4	8	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	8	8	8	8	2	2	2	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	56.0	56.0	56.0	56.0	56.0	56.0
Total Split (%)	37.8%	37.8%	37.8%	37.8%	37.8%	37.8%	62.2%	62.2%	62.2%	62.2%	62.2%	62.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.4	18.4	18.4	18.4	18.4	18.4	58.6	58.6	58.6	58.6	58.6	58.6
Actuated v/c Ratio	0.20	0.20	0.20	0.20	0.20	0.20	0.65	0.65	0.65	0.65	0.65	0.65
v/c Ratio	0.71	0.18	0.17	0.17	0.17	0.17	0.05	0.46	0.02	0.28	0.26	0.08
Control Delay	48.1	13.4	28.3	27.8	24.0	4.6	5.3	0.1	12.0	7.9	2.5	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.1	13.4	28.3	27.8	24.0	4.6	5.3	0.1	12.0	7.9	2.5	2.5
LOS	D	B	C	C	C	C	A	A	A	B	A	A
Approach Delay	39.2			25.3			5.2			7.8		
Approach LOS	D			C			A			A		
Queue Length 50th (m)	27.5	2.7	6.3	8.6	20.1	0.7	16.3	0.0	5.1	18.3	0.0	0.0
Queue Length 95th (m)	43.2	10.9	13.4	16.5	37.9	m2.1	46.4	m0.1	17.1	34.2	5.5	5.5
Internal Link Dist (m)	372.5			134.8			154.1			468.1		
Turn Bay Length (m)	30.0			50.0			35.0			70.0		
Base Capacity (vph)	356	476	374	527	511	459	2137	954	285	2077	937	750
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.12	0.12	0.11	0.45	0.05	0.46	0.02	0.28	0.26	0.08	0.08

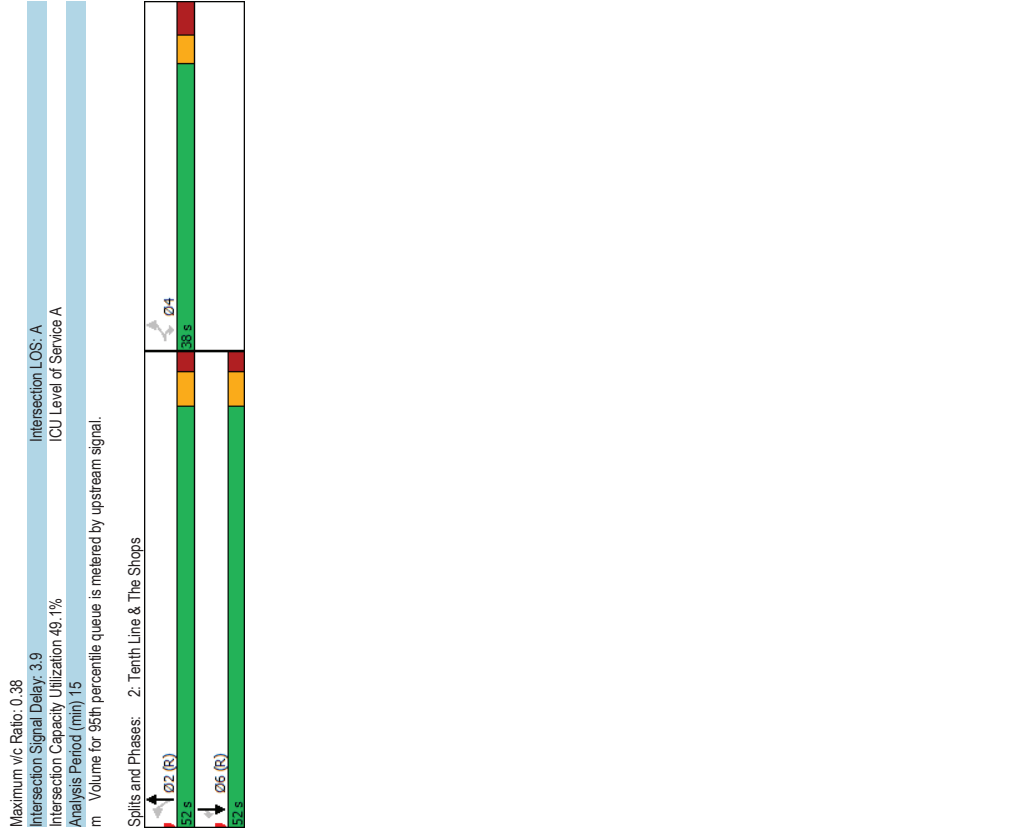
Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 61 (68%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 65
Control Type: Actuated-Coordinated



Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	54	13	70	970	563	61
Traffic Volume (vph)	54	13	70	970	563	61
Future Volume (vph)	1658	1483	1658	3252	3161	1483
Satd. Flow (prot)	0.950	0.441				
Flt Permitted						
Satd. Flow (perm)	1656	1483	766	3252	3161	1437
Satd. Flow (RTOR)	13					61
Lane Group Flow (vph)	54	13	70	970	563	61
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	52.0	52.0	52.0	52.0
Total Split (%)	42.2%	42.2%	57.8%	57.8%	57.8%	57.8%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	10.3	10.3	71.3	71.3	71.3	71.3
Actuated G/C Ratio	0.11	0.11	0.79	0.79	0.79	0.79
v/c Ratio	0.29	0.07	0.12	0.38	0.22	0.05
Control Delay	40.6	18.5	2.8	2.8	2.3	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.6	18.5	2.8	2.8	2.3	0.5
LOS	D	B	A	A	A	A
Approach Delay	36.3		2.8	2.1		
Approach LOS	D		A	A		
Queue Length 50th (m)	8.7	0.0	1.9	16.7	8.6	0.0
Queue Length 95th (m)	19.3	5.2	m4.0	m21.3	11.7	0.1
Inlet Link Dist (m)	33.9		222.1	154.1		
Turn Bay Length (m)			75.0		60.0	
Base Capacity (vph)	574	522	607	2576	2504	1151
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.02	0.12	0.38	0.22	0.05
<b>Intersection Summary</b>						
Cycle Length: 90						
Actuated Cycle Length: 90						
Offset: 69 (77%), Referenced to phase 2:NBLT and 6:SBT, Start of Green						
Natural Cycle: 65						
Control Type: Actuated-Coordinated						





Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

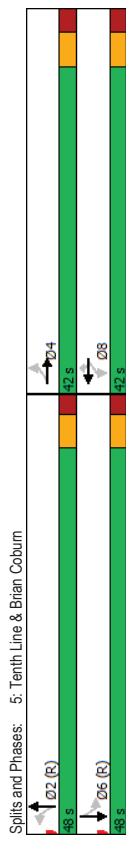
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	167	213	71	53	465	253	236	602	38	132	342	124
Traffic Volume (vph)	167	213	71	53	465	253	236	602	38	132	342	124
Future Volume (vph)	1642	1562	0	1610	1728	1483	1658	3216	0	1566	3071	0
Satd. Flow (prot)	0.268			0.497			0.476					
FI Permitted												
Satd. Flow (perm)	462	1562	0	842	1728	1455	828	3216	0	615	3071	0
Satd. Flow (RTOR)	22			198			10			77		
Lane Group Flow (vph)	167	284	0	53	465	253	236	640	0	132	466	0
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA
Protected Phases	4	4		8	8	8	2	2		6		6
Detector Phase	4	4		8	8	8	2	2		6		6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	31.4	31.4	31.4	31.4	31.4	31.4	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	42.0	42.0	42.0	42.0	48.0	48.0	48.0	48.0	48.0	48.0
Total Split (%)	46.7%	46.7%	46.7%	46.7%	46.7%	46.7%	53.3%	53.3%	53.3%	53.3%	53.3%	53.3%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.4	6.4	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	30.9	30.9	30.9	30.9	30.9	30.9	46.7	46.7	46.7	46.7	46.7	46.7
Actuated G/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.52	0.52	0.52	0.52	0.52	0.52
v/c Ratio	1.06	0.52	0.18	0.79	0.40	0.55	0.38	0.41	0.29	0.41	0.29	0.29
Control Delay	118.5	24.3	20.3	36.1	7.1	18.2	11.6	27.7	16.7	27.7	16.7	16.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	118.5	24.3	20.3	36.1	7.1	18.2	11.6	27.7	16.7	27.7	16.7	16.7
LOS	F	C	C	D	A	B	B	B	C	C	B	B
Approach Delay	59.1			25.5			13.4			19.1		
Approach LOS	E			C			B			B		
Queue Length 50th (m)	27.4	33.2	5.9	66.6	6.0	31.1	39.7	11.6	17.0	11.6	17.0	17.0
Queue Length 95th (m)	465.3	54.0	13.8	98.9	21.2	19.8	22.4	42.2	46.2	42.2	46.2	46.2
Internal Link Dist (m)	392.1			351.9			301.3			222.1		
Turn Bay Length (m)	45.0			50.0			105.0			110.0		
Base Capacity (vph)	182	631	333	683	695	430	1674	319	1632	319	1632	1632
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.92	0.45	0.16	0.68	0.36	0.55	0.38	0.41	0.29	0.41	0.29	0.29

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	43 (48%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

Maximum v/c Ratio:	1.06
Intersection Signal Delay:	25.8
Intersection LOS:	C
Intersection Capacity Utilization:	89.2%
ICU Level of Service:	E
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

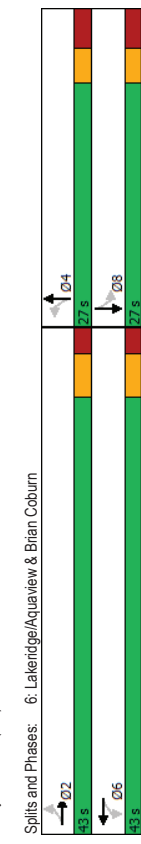
AM Peak Hour  
11-12-2021

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
→	→	→	←	←	←	←	←	←	←	←	←
EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
20	348	31	50	593	24	123	23	28	12	10	55
Future Volume (vph)											
20	348	31	50	593	24	123	23	28	12	10	55
Future Volume (vph)											
1658	1646	0	1595	1717	0	1658	1554	0	1551	1511	0
Satd. Flow (prot)											
0.371	0.537		0.715						0.724		
FI Permitted											
647	1646	0	902	1717	0	1248	1554	0	1144	1511	0
Satd. Flow (perm)											
10			4			28					55
Satd. Flow (RTOR)											
20	379	0	50	617	0	123	51	0	12	65	0
Lane Group Flow (vph)											
Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type											
2	2	6	6	6	4	4	4	8	8	8	8
Permitted Phases											
2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase											
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4
Minimum Split (%)	43.0	43.0	43.0	43.0	43.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (%)	61.4%	61.4%	61.4%	61.4%	61.4%	38.6%	38.6%	38.6%	38.6%	38.6%	38.6%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0
Yellow Time (%)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4
Lead/Lag											
Lead/Lag Optimize?											
Recall Mode	Max	Max	Max	Max	Max	None	None	None	None	None	None
Act Effct Green (s)	42.4	42.4	42.4	42.4	42.4	12.2	12.2	12.2	12.2	12.2	12.2
Actuated G/C Ratio	0.68	0.68	0.68	0.68	0.68	0.20	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.05	0.34	0.08	0.53	0.08	0.50	0.16	0.05	0.19	0.05	0.19
Control Delay	6.4	7.3	6.4	9.7	6.4	29.8	13.0	20.0	20.0	9.4	6.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.4	7.3	6.4	9.7	6.4	29.8	13.0	20.0	20.0	9.4	6.4
LOS	A	A	A	A	A	C	B	B	B	A	A
Approach Delay	7.3	7.3	9.5	9.5	9.5	24.9	11.0	11.0	11.0	11.0	11.0
Approach LOS	A	A	A	A	A	C	B	B	B	A	A
Queue Length 50th (m)	0.8	17.6	2.0	35.3	2.0	12.5	2.2	1.1	1.1	0.9	0.9
Queue Length 95th (m)	3.7	40.2	7.0	78.0	7.0	26.0	9.4	4.7	4.7	9.0	9.0
Internal Link Dist (m)	351.9		379.2		379.2	249.4		312.2		312.2	
Turn Bay Length (m)	65.0		65.0		65.0	30.0		30.0		30.0	
Base Capacity (vph)	440	1124	614	1170	614	414	534	379	538	379	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.34	0.08	0.53	0.08	0.30	0.10	0.03	0.12	0.03	0.12
Intersection Summary											
Cycle Length: 70											
Actuated Cycle Length: 62.2											
Natural Cycle: 60											
Control Type: Semi Act-Uncoord											
Maximum v/c Ratio: 0.53											

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

AM Peak Hour  
11-12-2021

Intersection Signal Delay: 10.9  
Intersection Capacity Utilization 68.1%  
Analysis Period (min) 15  
Intersection LOS: B  
ICU Level of Service C



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
11-12-2021

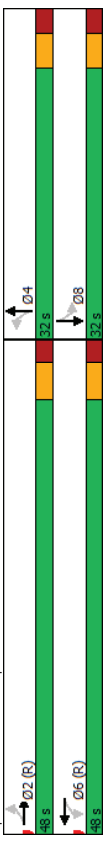
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	30	277	75	34	464	24	151	59	37	25	50	45
Traffic Volume (vph)	30	277	75	34	464	24	151	59	37	25	50	45
Future Volume (vph)	1642	1616	0	1851	1697	0	1688	1480	0	1566	1542	0
Satd. Flow (prot)	0.391		0.508			0.695			0.695			
FIIPermitted	674	1616	0	808	1697	0	1201	1480	0	1112	1542	0
Satd. Flow (perm)	26		5			37			45			
Satd. Flow (RTOR)	30	352	0	34	488	0	151	96	0	25	95	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	2	6	6	6	4	4	4	8	8	8	8
Protected Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	48.0	48.0	48.0	48.0	48.0	48.0	48.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	42.0	42.0	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2	26.2	26.2
Actuated G/C Ratio	0.52	0.52	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.08	0.41	0.08	0.55	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18
Control Delay	10.3	12.3	10.1	15.4	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.3	12.3	10.1	15.4	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
LOS	B	B	B	B	B	C	B	B	B	B	B	B
Approach Delay	12.2		15.1		20.2		13.7					
Approach LOS	B		B		C		B					
Queue Length 50th (m)	2.1	27.8	2.4	45.7	17.5	6.2	2.6	5.2	2.6	5.2	5.2	5.2
Queue Length 95th (m)	6.2	46.3	6.7	72.2	33.2	16.5	7.8	15.3	7.8	15.3	15.3	15.3
Internal Link Dist (m)	379.2		585.6		222.2		382.8					
Turn Bay Length (m)	65.0		65.0		30.0		30.0					
Base Capacity (vph)	353	860	424	893	393	509	364	535	364	535	535	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.41	0.08	0.55	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18

Intersection Summary	
Cycle Length:	80
Actuated Cycle Length:	80
Offset:	0 (0%), Referenced to phase 2EBTL and 6:WBTL, Start of Green
Natural Cycle:	50
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
11-12-2021

Maximum v/c Ratio:	0.55
Intersection Signal Delay:	15.1
Intersection Capacity Utilization:	55.2%
Analysis Period (min):	15
Intersection LOS:	B
ICU Level of Service:	B

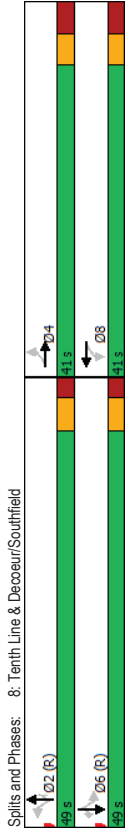


Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	86	27	44	9	29	70	79	715	1	19	400	59
Traffic Volume (vph)	86	27	44	9	29	70	79	715	1	19	400	59
Future Volume (vph)	1610	1389	0	1658	1545	0	1445	3131	1483	1523	3161	1401
Satd. Flow (prot)	0.693			0.711			0.516			0.373		
FI Permitted	1173	1389	0	1241	1545	0	781	3131	1442	597	3161	1359
Satd. Flow (perm)	44			70			47			59		
Satd. Flow (RTOR)	86	71	0	9	99	0	79	715	1	19	400	59
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase	4	4	4	8	8	8	2	2	2	6	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	49.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9

Maximum v/c Ratio: 0.41  
 Intersection Signal Delay: 9.1  
 Intersection LOS: A  
 ICU Level of Service B  
 Intersection Capacity Utilization 56.5%  
 Analysis Period (min) 15  
 Volume for 95th percentile queue is metered by upstream signal.



Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.1	16.1	16.1	65.7	65.7	65.7	65.7	65.7	65.7
Actuated g/C Ratio	0.18	0.18	0.18	0.73	0.73	0.73	0.73	0.73	0.73
v/c Ratio	0.41	0.25	0.04	0.30	0.14	0.31	0.00	0.04	0.17
Control Delay	36.0	14.9	25.1	12.6	8.7	7.6	0.0	6.1	5.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.0	14.9	25.1	12.6	8.7	7.6	0.0	6.1	5.1
LOS	D	B	C	B	A	A	A	A	A
Approach Delay	26.4		13.7		7.7			4.8	
Approach LOS	C		B		A			A	
Queue Length 50th (m)	14.1	4.2	1.4	4.5	3.5	19.4	0.0	0.7	12.1
Queue Length 95th (m)	20.2	11.3	3.9	12.9	16.5	56.7	0.0	m3.3	19.0
Internal Link Dist (m)	344.3		315.6		346.2			301.3	
Turn Bay Length (m)	45.0		20.0		90.0			60.0	
Base Capacity (vph)	444	553	470	628	569	2284	1065	485	2306
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.19	0.13	0.02	0.16	0.14	0.31	0.00	0.04	0.17

Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 36 (40%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 70
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	351	33	76	326	58
Traffic Volume (vph)	135	3	12	70	1	290	5	351	33	76	326	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3183	0
Satd. Flow (prot)	0.457			0.748			0.524			0.524		
Flt Permitted	795	1433	0	1304	1447	0	776	3074	0	872	3183	0
Satd. Flow (RTOR)	12			290			17			35		
Lane Group Flow (vph)	135	15	0	70	291	0	5	384	0	76	384	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	8	2	2	6	6	6	6	6
Permitted Phases	4	4	8	8	8	2	2	6	6	6	6	6
Detector Phase	4	4	8	8	8	2	2	6	6	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	351	33	76	326	58
Traffic Volume (vph)	135	3	12	70	1	290	5	351	33	76	326	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3183	0
Satd. Flow (prot)	0.457			0.748			0.524			0.524		
Flt Permitted	795	1433	0	1304	1447	0	776	3074	0	872	3183	0
Satd. Flow (RTOR)	12			290			17			35		
Lane Group Flow (vph)	135	15	0	70	291	0	5	384	0	76	384	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	8	2	2	6	6	6	6	6
Permitted Phases	4	4	8	8	8	2	2	6	6	6	6	6
Detector Phase	4	4	8	8	8	2	2	6	6	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2

Lead/Lag Optimize?

Intersection LOS: B  
ICU Level of Service D

Recall Mode: None

Intersection LOS: B  
ICU Level of Service D

Act Effct Green (s): 16.3

Analysis Period (min): 15

Actuated G/C Ratio: 0.24

Splits and Phases: 9: Tenth Line & Sweetvalley/Harvest Valley

v/c Ratio: 0.71

Intersection Signal Delay: 12.1

Control Delay: 44.1

Intersection Capacity Utilization: 77.1%

Queue Delay: 0.0

Analysis Period (min): 15

Total Delay: 44.1

Intersection LOS: B

LOS: D

Intersection LOS: B

Approach Delay: 40.9

Intersection LOS: B

Approach LOS: D

Intersection LOS: B

Queue Length 50th (m): 15.6

Intersection LOS: B

Queue Length 95th (m): 32.7

Intersection LOS: B

Internal Link Dist (m): 180.2

Intersection LOS: B

Turn Bay Length (m): 38.0

Intersection LOS: B

Base Capacity (vph): 334

Intersection LOS: B

Starvation Cap Reductn: 0

Intersection LOS: B

Spillback Cap Reductn: 0

Intersection LOS: B

Storage Cap Reductn: 0

Intersection LOS: B

Reduced v/c Ratio: 0.40

Intersection LOS: B

Intersection Summary

Intersection LOS: B

Cycle Length: 80

Intersection LOS: B

Actuated Cycle Length: 68.2

Intersection LOS: B

Natural Cycle: 65

Intersection LOS: B

Control Type: Actuated-Uncoordinated

Intersection LOS: B

Maximum v/c Ratio: 0.71

Intersection LOS: B

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

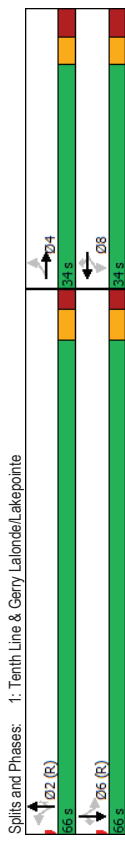
Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	173	103	62	31	25	171	38	1013	63	263	1238	187
Traffic Volume (vph)	173	103	62	31	25	171	38	1013	63	263	1238	187
Future Volume (vph)	1658	1637	0	1658	1745	1483	1658	3316	1483	1658	3316	1455
Satd. Flow (prot)	0.741			0.604			0.187			0.253		
FI Permitted	1292	1637	0	1049	1745	1464	326	3316	1436	441	3316	1411
Satd. Flow (perm)	30			113			63			63		187
Satd. Flow (RTOR)	173	165	0	31	25	171	38	1013	63	263	1238	187
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	8	8	8	8	2	2	2	2	6	6
Permitted Phases	4	4	8	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	8	8	8	8	2	2	2	2	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	34.0%	34.0%	34.0%	34.0%	34.0%	34.0%	66.0%	66.0%	66.0%	66.0%	66.0%	66.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2

Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.8	18.8	18.8	18.8	68.2	68.2	68.2	68.2	68.2	68.2	68.2
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.68	0.68	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.71	0.50	0.16	0.08	0.47	0.17	0.45	0.06	0.88	0.55	0.18
Control Delay	53.6	33.5	33.0	30.8	16.7	5.2	4.9	0.6	46.9	10.1	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.6	33.5	33.0	30.8	16.7	5.2	4.9	0.6	46.9	10.1	1.7
LOS	D	C	C	C	B	A	A	A	D	B	A
Approach Delay	43.7	20.5			4.7				14.9		
Approach LOS	D	C			A				B		
Queue Length 50th (m)	31.8	23.4	5.1	4.0	9.6	1.2	41.5	0.2	35.4	56.4	0.0
Queue Length 95th (m)	49.0	38.8	11.8	9.9	25.5	2.6	16.2	0.7	#100.9	94.7	8.0
Internal Link Dist (m)	30.0	372.5			134.8		154.1		468.1		
Turn Bay Length (m)	351	467	285	474	480	222	2262	999	300	2262	1022
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.35	0.11	0.05	0.36	0.17	0.45	0.06	0.88	0.55	0.18

Intersection Summary
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 90 (90%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 110
Control Type: Actuated-Coordinated

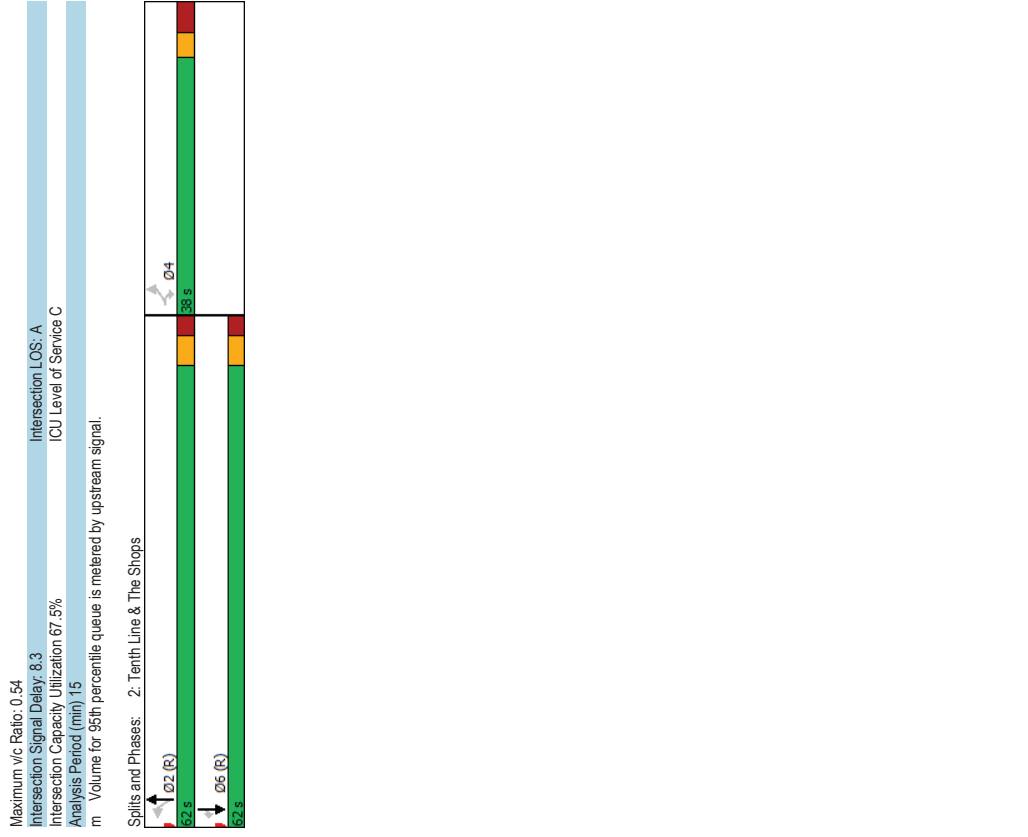
Maximum v/c Ratio: 0.88	Intersection LOS: B
Intersection Signal Delay: 14.8	ICU Level of Service E
Intersection Capacity Utilization 66.8%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	111	54	966	1168	160
Future Volume (vph)	149	111	54	966	1168	160
Satd. Flow (prot)	1658	1483	1658	3316	3316	1483
Flt Permitted	0.950	0.210				
Satd. Flow (perm)	1653	1464	366	3316	3316	1431
Satd. Flow (RTOR)	65					160
Lane Group Flow (vph)	149	111	54	966	1168	160
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	62.0	62.0	62.0	62.0
Total Split (%)	38.0%	38.0%	62.0%	62.0%	62.0%	62.0%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	70.2	70.2	70.2	70.2
Actuated G/C Ratio	0.17	0.17	0.70	0.70	0.70	0.70
v/c Ratio	0.54	0.37	0.21	0.42	0.50	0.15
Control Delay	43.4	18.7	8.1	5.9	5.7	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	18.7	8.1	5.9	5.8	0.7
LOS	D	B	A	A	A	A
Approach Delay	32.9		6.0	5.2		
Approach LOS	C		A	A		
Queue Length 50th (m)	27.5	8.0	2.0	19.8	28.6	0.1
Queue Length 95th (m)	37.7	18.9	11.6	66.3	37.2	2.5
Internal Link Dist (m)	33.9		222.1	154.1		
Turn Bay Length (m)			75.0		60.0	
Base Capacity (vph)	515	501	256	2326	2326	1051
Starvation Cap Reductn	0	0	0	0	93	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.21	0.42	0.52	0.15
Intersection Summary						
Cycle Length: 100						
Actuated Cycle Length: 100						
Offset: 85 (85%), Referenced to phase 2:NBLT and 6:SBT, Start of Green						
Natural Cycle: 70						
Control Type: Actuated-Coordinated						



Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Future Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Satd. Flow (prot)	1658	1647	0	1658	1745	1483	1566	3269	0	1658	3193	0
Flt Permitted	0.568			0.099			0.185			0.363		
Satd. Flow (perm)	981	1647	0	173	1745	1436	304	3269	0	630	3193	0
Satd. Flow (RTOR)	31			210			11			38		
Lane Group Flow (vph)	212	709	0	59	240	236	161	625	0	291	1012	0
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA
Protected Phases	4	4		8	8	8	2	2		6		6
Permitted Phases	4	4		8	8	8	2	2		6		6
Detector Phase	4	4		8	8	8	2	2		6		6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0		10.0
Minimum Split (s)	31.4	31.4		31.4	31.4	29.0	29.0	29.0		29.0		29.0
Total Split (s)	47.0	47.0		47.0	47.0	53.0	53.0	53.0		53.0		53.0
Total Split (%)	47.0%	47.0%		47.0%	47.0%	53.0%	53.0%	53.0%		53.0%		53.0%
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7		3.7		3.7
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3		2.3		2.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0		6.0		6.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None	None	None	None		None		None
Act Effct Green (s)	40.6	40.6		40.6	40.6	40.6	47.0	47.0		47.0		47.0
Actuated G/C Ratio	0.41	0.41		0.41	0.41	0.41	0.47	0.47		0.47		0.47
v/c Ratio	0.53	1.03		0.84	0.34	0.33	1.13	0.41		0.98		0.67
Control Delay	28.6	72.4		104.0	22.2	5.3	145.6	18.0		65.4		11.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0		0.0
Total Delay	28.6	72.4		104.0	22.2	5.3	145.6	18.0		65.4		11.9
LOS	C	E		F	C	A	F	B		E		B
Approach Delay		62.3			23.7		44.2			23.8		
Approach LOS		E			C		D			C		
Queue Length 50th (m)	30.5	~144.5		10.1	31.4	3.0	~36.6	32.6		55.0		46.1
Queue Length 95th (m)	53.5	#213.2		#35.2	50.1	17.4	#71.0	53.3		#99.1		43.7
Internal Link Dist (m)		392.1			351.9		301.3			222.1		
Turn Bay Length (m)	45.0			50.0		45.0	105.0			110.0		
Base Capacity (vph)	398	687		70	708	707	142	1542		296		1520
Starvation Cap Reductn	0	0		0	0	0	0	0		0		0
Spillback Cap Reductn	0	0		0	0	0	0	0		0		0
Storage Cap Reductn	0	0		0	0	0	0	0		0		0
Reduced v/c Ratio	0.53	1.03		0.84	0.34	0.33	1.13	0.41		0.98		0.67
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 4 (4%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green												
Natural Cycle: 75												
Control Type: Actuated-Coordinated												

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

Maximum v/c Ratio: 1.13	Intersection LOS: D
Intersection Signal Delay: 38.3	ICU Level of Service H
Intersection Capacity Utilization 110.5%	
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
# Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
Splits and Phases: 5: Tenth Line & Brian Coburn	



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	56	685	73	31	434	20	71	19	27	28	13	31
Traffic Volume (vph)	56	685	73	31	434	20	71	19	27	28	13	31
Future Volume (vph)	1658	1716	0	1658	1714	0	1626	1546	0	1523	1532	0
Satd. Flow (prot)	0.489	0.310			0.728					0.727		
Flt Permitted	848	1716	0	540	1714	0	1240	1546	0	1134	1532	0
Satd. Flow (RTOR)	12			5			27			31		
Lane Group Flow (vph)	56	758	0	31	454	0	71	46	0	28	44	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	6	6	6	4	4	8	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	8	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	54.0	54.0	54.0	54.0	54.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Total Split (%)	67.5%	67.5%	67.5%	67.5%	67.5%	32.5%	32.5%	32.5%	32.5%	32.5%	32.5%	32.5%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4

Lead/Lag Optimize?	Max	Max	Max	None	None	None	None
Recall Mode	55.0	55.0	55.0	11.2	11.2	11.2	11.2
Act Effct Green (s)	0.74	0.74	0.74	0.15	0.15	0.15	0.15
Actuated G/C Ratio	0.09	0.09	0.08	0.36	0.38	0.18	0.16
v/c Ratio	5.0	8.9	5.2	5.9	33.9	16.4	28.7
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	5.0	8.9	5.2	5.9	33.9	16.4	28.7
Total Delay	5.0	8.9	5.2	5.9	33.9	16.4	28.7
LOS	A	A	A	C	B	C	B
Approach Delay	8.6	5.8	5.8	27.0	27.0	20.2	20.2
Approach LOS	A	A	A	C	C	C	C
Queue Length 50th (m)	2.1	46.1	1.1	21.1	8.6	2.2	3.3
Queue Length 95th (m)	6.8	97.9	4.6	44.2	19.6	10.2	9.8
Internal Link Dist (m)	351.9		379.2		249.4		312.2
Turn Bay Length (m)	65.0		65.0		30.0		30.0
Base Capacity (vph)	631	1280	402	1276	330	431	302
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.59	0.08	0.36	0.22	0.11	0.09

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 73.9

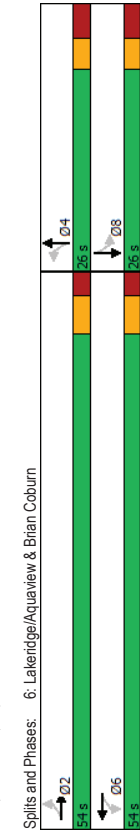
Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.59

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Intersection Signal Delay: 9.7  
Intersection Capacity Utilization: 70.8%  
Analysis Period (min): 15  
Intersection LOS: A  
ICU Level of Service: C

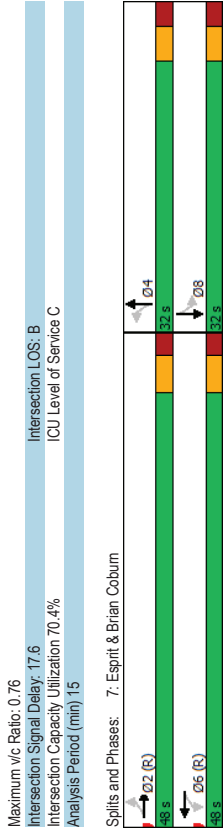


Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	52	491	188	27	344	18	103	43	25	23	48	38
Traffic Volume (vph)	52	491	188	27	344	18	103	43	25	23	48	38
Future Volume (vph)	1658	1672	0	1658	1714	0	1658	1549	0	1658	1490	0
Satd. Flow (prot)	0.499			0.242			0.701			0.713		
Flt Permitted	865	1672	0	422	1714	0	1206	1549	0	1242	1490	0
Satd. Flow (perm)	36			5			25			38		
Satd. Flow (RTOR)	52	679	0	27	362	0	103	68	0	23	86	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	2	6	6	6	4	4	4	8	8	8	8
Permitted Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C	C	C	C	B	B	B
Act Effct Green (s)	42.0	42.0	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2	26.2	26.2
Actuated G/C Ratio	0.52	0.52	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.11	0.76	0.12	0.40	0.26	0.13	0.06	0.17	0.06	0.17	0.06	0.17
Control Delay	10.5	21.0	11.4	12.9	22.1	14.1	19.0	12.9	14.1	19.0	12.9	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.5	21.0	11.4	12.9	22.1	14.1	19.0	12.9	14.1	19.0	12.9	12.9
LOS	B	C	B	B	B	C	B	B	C	B	B	B
Approach Delay	20.3		12.8		18.9		14.2					
Approach LOS	C		B		B		B					
Queue Length 50th (m)	3.7	72.4	1.9	30.5	11.4	4.5	2.4	5.0	2.4	5.0	2.4	5.0
Queue Length 95th (m)	9.3	116.6	6.3	49.1	23.4	12.9	7.3	14.5	7.3	14.5	7.3	14.5
Internal Link Dist (m)	379.2		585.6		222.2		382.8					
Turn Bay Length (m)	65.0		65.0		30.0		30.0					
Base Capacity (vph)	454	894	221	902	394	524	406	513	406	513	406	513
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.76	0.12	0.40	0.26	0.13	0.06	0.17	0.06	0.17	0.06	0.17

Scenario 1 2370 Tenth Line Rd 11:59 pm 09-07-2021 2026 Future Background  
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Scenario 1 2370 Tenth Line Rd 11:59 pm 09-07-2021 2026 Future Background  
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Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

PM Peak Hour  
11-12-2021

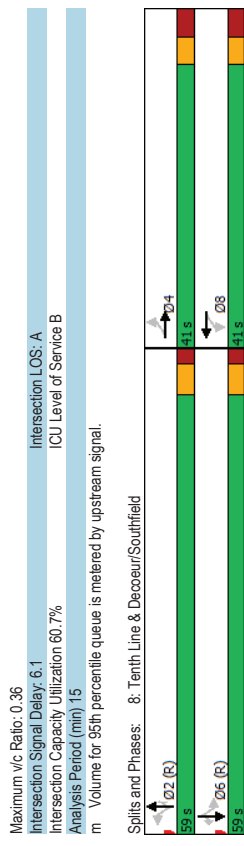
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	47	16	30	2	24	55	34	674	14	116	917	94
Future Volume (vph)	47	16	30	2	24	55	34	674	14	116	917	94
Satd. Flow (prot)	1658	1561	0	1658	1564	0	1658	3316	1483	1658	3316	1483
Flt Permitted	0.706			0.727			0.296			0.393		
Satd. Flow (perm)	1232	1561	0	1268	1564	0	515	3316	1483	686	3316	1435
Satd. Flow (RTOR)	30			55			43			43		94
Lane Group Flow (vph)	47	46	0	2	79	0	34	674	14	116	917	94
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4		8	8		2	2	2	2	6	6
Detector Phase	4	4		8	8		2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	59.0	59.0	59.0	59.0	59.0	59.0
Total Split (%)	41.0%	41.0%	41.0%	41.0%	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%	59.0%	59.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9

Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.0	15.0	15.0	76.8	76.8	76.8	76.8	76.8	76.8	76.8
Actuated g/C Ratio	0.15	0.15	0.15	0.77	0.77	0.77	0.77	0.77	0.77	0.77
v/c Ratio	0.26	0.18	0.01	0.28	0.09	0.26	0.01	0.22	0.36	0.08
Control Delay	37.7	17.2	29.0	15.8	7.7	6.1	0.0	4.6	3.7	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.7	17.2	29.0	15.8	7.7	6.1	0.0	4.6	3.7	0.5
LOS	D	B	C	B	A	A	A	A	A	A
Approach Delay	27.5		16.1		6.1			3.6		
Approach LOS	C		B		A			A		
Queue Length 50th (m)	8.7	2.9	0.4	4.3	1.3	15.8	0.0	2.8	12.9	0.0
Queue Length 95th (m)	14.4	9.6	1.8	12.8	8.5	50.8	0.3	m8.3	m32.8	m0.0
Internal Link Dist (m)	344.3			315.6		346.2				301.3
Turn Bay Length (m)	45.0		20.0		90.0		60.0	60.0		70.0
Base Capacity (vph)	420	552	432	569	395	2546	1148	526	2546	1123
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.08	0.00	0.14	0.09	0.26	0.01	0.22	0.36	0.08

Intersection Summary
Cycle Length: 100
Actuated Cycle Length: 100
Offset: 21 (21%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 70
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

PM Peak Hour  
11-12-2021



Split and Phases	8: Tenth Line & Decoeur/Southfield
Phase 1	0.2 (R)
Phase 2	0.2 (R)
Phase 3	0.2 (R)
Phase 4	0.2 (R)
Phase 5	0.2 (R)
Phase 6	0.2 (R)
Phase 7	0.2 (R)
Phase 8	0.2 (R)
Phase 9	0.2 (R)
Phase 10	0.2 (R)
Phase 11	0.2 (R)
Phase 12	0.2 (R)
Phase 13	0.2 (R)
Phase 14	0.2 (R)
Phase 15	0.2 (R)

Maximum v/c Ratio	0.36
Intersection Signal Delay	6.1
Intersection LOS	A
ICU Level of Service	B
Intersection Capacity Utilization	60.7%
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

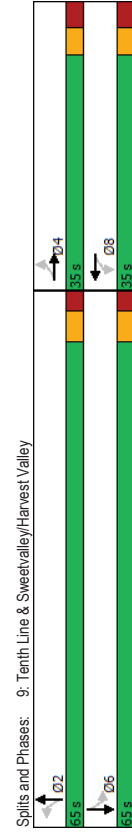


Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
95	2	7	17	1	167	14	442	81	293	478	168
95	2	7	17	1	167	14	442	81	293	478	168
1658	1925	0	1595	1464	0	1658	3239	0	1658	3163	0
0.600			0.752			0.405			0.458		
1045	1525	0	1261	1464	0	707	3239	0	799	3163	0
7			167			36			85		
95	9	0	17	188	0	14	523	0	293	646	0
Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
4	4	8	8	8	2	2	2	2	6	6	6
4	4	8	8	8	2	2	2	2	6	6	6
10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
35.0	35.0	35.0	35.0	35.0	65.0	65.0	65.0	65.0	65.0	65.0	65.0
35.0%	35.0%	35.0%	35.0%	35.0%	65.0%	65.0%	65.0%	65.0%	65.0%	65.0%	65.0%
3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2
None	None	None	None	None	Max	Max	Max	Max	Max	Max	Max
14.9	14.9	14.9	14.9	14.9	61.8	61.8	61.8	61.8	61.8	61.8	61.8
0.17	0.17	0.17	0.17	0.17	0.69	0.69	0.69	0.69	0.69	0.69	0.69
0.55	0.03	0.08	0.44	0.03	0.03	0.23	0.53	0.29	0.53	0.29	0.29
44.6	18.1	29.4	8.7	6.7	5.8	13.0	5.7	5.7	13.0	5.7	5.7
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
44.6	18.1	29.4	8.7	6.7	5.8	13.0	5.7	5.7	13.0	5.7	5.7
D	B	C	A	A	A	A	B	A	B	A	A
42.3	10.6	5.8	5.8	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
14.5	0.3	2.4	0.1	0.6	12.2	18.8	14.5	14.5	18.8	14.5	14.5
28.5	4.0	7.5	14.7	3.6	30.7	63.7	36.6	36.6	63.7	36.6	36.6
180.2	318.8	263.5	346.2	263.5	346.2	346.2	346.2	346.2	346.2	346.2	346.2
38.0	60.0	54.0	65.0	65.0	65.0	65.0	65.0	65.0	65.0	65.0	65.0
335	493	404	582	488	2249	552	2211	2211	552	2211	2211
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0
0.28	0.02	0.04	0.29	0.03	0.23	0.63	0.29	0.29	0.63	0.29	0.29
Intersection Summary											
Cycle Length: 100											
Actuated Cycle Length: 89.4											
Natural Cycle: 75											
Control Type: Actuated-Uncoordinated											
Maximum v/c Ratio: 0.55											

Intersection Signal Delay: 9.6  
Intersection Capacity Utilization 74.2%  
Analysis Period (min) 15  
Intersection LOS: A  
ICU Level of Service D



## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde AM FB2026]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	83	2.0	0.186	9.6 LOS A	1.0	7.4	0.55	0.65	0.55	50.0		
2	T1	21	2.0	0.186	4.4 LOS A	1.0	7.4	0.55	0.65	0.55	46.8		
3	R2	78	2.0	0.186	4.8 LOS A	1.0	7.4	0.55	0.65	0.55	48.8		
Approach		182	2.0	0.186	7.0 LOS A	1.0	7.4	0.55	0.65	0.55	49.1		
East: Brian Coburn													
4	L2	44	2.0	0.780	11.6 LOS B	10.5	74.9	0.77	0.62	0.80	50.5		
5	T1	939	2.0	0.780	6.2 LOS A	10.5	74.9	0.77	0.62	0.80	53.8		
6	R2	13	2.0	0.780	6.3 LOS A	10.5	74.9	0.77	0.62	0.80	48.9		
Approach		996	2.0	0.780	6.5 LOS A	10.5	74.9	0.77	0.62	0.80	53.6		
North: Gerry Lalonde													
7	L2	7	2.0	0.499	23.2 LOS C	4.4	31.7	1.00	1.10	1.24	43.2		
8	T1	8	2.0	0.499	18.1 LOS B	4.4	31.7	1.00	1.10	1.24	40.8		
9	R2	185	2.0	0.499	18.5 LOS B	4.4	31.7	1.00	1.10	1.24	42.3		
Approach		200	2.0	0.499	18.6 LOS B	4.4	31.7	1.00	1.10	1.24	42.2		
West: Brian Coburn													
10u	U	32	2.0	0.314	11.3 LOS B	2.2	15.9	0.25	0.44	0.25	56.9		
10	L2	40	2.0	0.314	9.2 LOS A	2.2	15.9	0.25	0.44	0.25	52.2		
11	T1	330	2.0	0.314	3.8 LOS A	2.2	15.9	0.25	0.44	0.25	55.7		
12	R2	48	2.0	0.314	3.9 LOS A	2.2	15.9	0.25	0.44	0.25	50.5		
Approach		450	2.0	0.314	4.9 LOS A	2.2	15.9	0.25	0.44	0.25	54.8		
All Vehicles		1828	2.0	0.780	7.5 LOS A	10.5	74.9	0.65	0.63	0.69	51.9		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: CGH TRANSPORTATION | Processor: November 15, 2021 15:00:23 PM  
Project: CGH TRANSPORTATION/CGH Working - Documents/Projects/2021-052 Mattamy 2370 Tenth Line/DATA/Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde PM FB2026]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	37	2.0	0.295	21.5 LOS C	2.3	16.3	1.00	0.98	1.00	43.2		
2	T1	10	2.0	0.295	16.3 LOS B	2.3	16.3	1.00	0.98	1.00	40.8		
3	R2	36	2.0	0.295	16.7 LOS B	2.3	16.3	1.00	0.98	1.00	42.3		
Approach		83	2.0	0.295	18.8 LOS B	2.3	16.3	1.00	0.98	1.00	42.5		
East: Brian Coburn													
4	L2	62	2.0	0.547	11.0 LOS B	4.6	32.7	0.70	0.63	0.70	50.7		
5	T1	512	2.0	0.547	5.7 LOS A	4.6	32.7	0.70	0.63	0.70	54.0		
6	R2	12	2.0	0.547	5.8 LOS A	4.6	32.7	0.70	0.63	0.70	49.1		
Approach		586	2.0	0.547	6.3 LOS A	4.6	32.7	0.70	0.63	0.70	53.5		
North: Gerry Lalonde													
7	L2	4	2.0	0.152	11.1 LOS B	1.0	6.8	0.73	0.71	0.73	50.4		
8	T1	18	2.0	0.152	5.9 LOS A	1.0	6.8	0.73	0.71	0.73	47.1		
9	R2	92	2.0	0.152	6.3 LOS A	1.0	6.8	0.73	0.71	0.73	48.1		
Approach		114	2.0	0.152	6.4 LOS A	1.0	6.8	0.73	0.71	0.73	48.8		
West: Brian Coburn													
10u	U	27	2.0	0.888	12.7 LOS B	20.4	145.4	0.87	0.50	0.87	54.2		
10	L2	212	2.0	0.888	10.5 LOS B	20.4	145.4	0.87	0.50	0.87	50.0		
11	T1	986	2.0	0.888	5.2 LOS A	20.4	145.4	0.87	0.50	0.87	53.1		
12	R2	65	2.0	0.888	5.3 LOS A	20.4	145.4	0.87	0.50	0.87	48.4		
Approach		1290	2.0	0.888	6.2 LOS A	20.4	145.4	0.87	0.50	0.87	52.4		
All Vehicles		2073	2.0	0.888	6.7 LOS A	20.4	145.4	0.82	0.57	0.82	52.0		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: CGH TRANSPORTATION | Processor: November 15, 2021 15:00:24 PM  
Project: CGH TRANSPORTATION/CGH Working - Documents/Projects/2021-052 Mattamy 2370 Tenth Line/DATA/Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Strasbourg AM FB2026]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate Cycles	Aver. No. Cycles	Average Speed km/h	
South: des Subespines													
1	L2	109	2.0	0.181	9.5	LOSA	1.0	7.2	0.54	0.66	0.54	49.6	
2	T1	15	2.0	0.181	4.3	LOSA	1.0	7.2	0.54	0.66	0.54	46.5	
3	R2	55	2.0	0.181	4.7	LOSA	1.0	7.2	0.54	0.66	0.54	48.4	
Approach													
		179	2.0	0.181	7.6	LOSA	1.0	7.2	0.54	0.66	0.54	48.9	
East: Brian Coburn													
4	L2	32	2.0	0.633	10.0	LOSB	6.5	46.1	0.56	0.50	0.56	51.4	
5	T1	795	2.0	0.633	4.7	LOSA	6.5	46.1	0.56	0.50	0.56	54.8	
6	R2	12	2.0	0.633	4.8	LOSA	6.5	46.1	0.56	0.50	0.56	49.7	
Approach													
		839	2.0	0.633	4.9	LOSA	6.5	46.1	0.56	0.50	0.56	54.6	
North: Strasbourg													
7	L2	25	2.0	0.221	14.6	LOSB	1.5	10.8	0.87	0.86	0.87	47.6	
8	T1	22	2.0	0.221	9.5	LOSA	1.5	10.8	0.87	0.86	0.87	44.7	
9	R2	76	2.0	0.221	9.8	LOSA	1.5	10.8	0.87	0.86	0.87	46.5	
Approach													
		123	2.0	0.221	10.7	LOSB	1.5	10.8	0.87	0.86	0.87	46.4	
West: Brian Coburn													
10	L2	7	2.0	0.295	9.3	LOSA	2.0	14.2	0.29	0.41	0.29	52.6	
11	T1	360	2.0	0.295	3.9	LOSA	2.0	14.2	0.29	0.41	0.29	56.1	
12	R2	38	2.0	0.295	4.1	LOSA	2.0	14.2	0.29	0.41	0.29	50.9	
Approach													
		405	2.0	0.295	4.1	LOSA	2.0	14.2	0.29	0.41	0.29	55.5	
All Vehicles													
		1546	2.0	0.633	5.5	LOSA	6.5	46.1	0.51	0.52	0.51	53.4	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Modal Designation.

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Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 5:00:22 PM  
Project: C:\Users\AndrewHarte\CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.sp8

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Strasbourg PM FB2026]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate Cycles	Aver. No. Cycles	Average Speed km/h	
South: des Subespines													
1	L2	58	2.0	0.195	14.4	LOSB	1.3	9.4	0.86	0.86	0.86	46.9	
2	T1	18	2.0	0.195	9.2	LOSA	1.3	9.4	0.86	0.86	0.86	44.1	
3	R2	34	2.0	0.195	9.6	LOSA	1.3	9.4	0.86	0.86	0.86	45.8	
Approach													
		110	2.0	0.195	12.1	LOSB	1.3	9.4	0.86	0.86	0.86	46.1	
East: Brian Coburn													
4	L2	54	2.0	0.445	9.6	LOSA	3.6	25.5	0.41	0.46	0.41	51.9	
5	T1	503	2.0	0.445	4.2	LOSA	3.6	25.5	0.41	0.46	0.41	55.3	
6	R2	36	2.0	0.445	4.3	LOSA	3.6	25.5	0.41	0.46	0.41	50.2	
Approach													
		593	2.0	0.445	4.7	LOSA	3.6	25.5	0.41	0.46	0.41	54.7	
North: Strasbourg													
7	L2	23	2.0	0.073	10.7	LOSB	0.4	2.9	0.65	0.66	0.65	49.5	
8	T1	13	2.0	0.073	5.5	LOSA	0.4	2.9	0.65	0.66	0.65	46.4	
9	R2	23	2.0	0.073	5.9	LOSA	0.4	2.9	0.65	0.66	0.65	48.3	
Approach													
		59	2.0	0.073	7.7	LOSA	0.4	2.9	0.65	0.66	0.65	48.3	
West: Brian Coburn													
10	L2	33	2.0	0.706	9.8	LOSA	8.2	58.6	0.51	0.46	0.51	51.7	
11	T1	870	2.0	0.706	4.5	LOSA	8.2	58.6	0.51	0.46	0.51	55.1	
12	R2	106	2.0	0.706	4.6	LOSA	8.2	58.6	0.51	0.46	0.51	50.0	
Approach													
		1009	2.0	0.706	4.7	LOSA	8.2	58.6	0.51	0.46	0.51	54.4	
All Vehicles													
		1771	2.0	0.706	5.2	LOSA	8.2	58.6	0.50	0.49	0.50	53.6	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Modal Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com  
Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 5:00:22 PM  
Project: C:\Users\AndrewHarte\CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.sp8

# Appendix I

Synchro Worksheets – 2026 Future Background Conditions with Phase/Cycle Changes

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

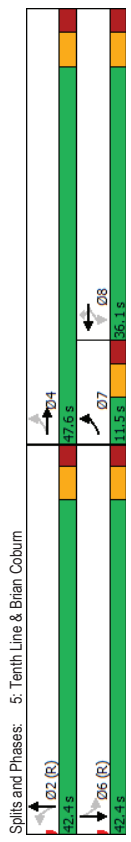
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	167	213	71	53	465	253	236	602	38	132	342	124
Traffic Volume (vph)	167	213	71	53	465	253	236	602	38	132	342	124
Future Volume (vph)	1642	1562	0	1610	1728	1483	1658	3216	0	1566	3071	0
Satd. Flow (prot)	0.176			0.586			0.458					
FI/Permitted	304	1562	0	993	1728	1455	797	3216	0	571	3071	0
Satd. Flow (perm)	25			158			9					
Satd. Flow (RTOR)	167	284	0	53	465	253	236	640	0	132	466	0
Lane Group Flow (vph)	pm-pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7	4		8	8	2	2		6		6	
Permitted Phases	4			8	8	2	2		6		6	
Detector Phase	7	4		8	8	2	2		6		6	
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.4	31.4		31.4	31.4	31.4	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	11.5	47.6		36.1	36.1	36.1	42.4	42.4	42.4	42.4	42.4	42.4
Total Split (%)	12.8%	52.9%		40.1%	40.1%	40.1%	47.1%	47.1%	47.1%	47.1%	47.1%	47.1%
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7		2.7	2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag		Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	39.9	38.9		27.4	27.4	27.4	38.7	38.7	38.7	38.7	38.7	38.7
Actuated G/C Ratio	0.43	0.43		0.30	0.30	0.30	0.43	0.43	0.43	0.43	0.43	0.43
v/c Ratio	0.81	0.41		0.18	0.89	0.46	0.69	0.46	0.54	0.34	0.34	0.34
Control Delay	47.8	17.5		23.5	49.6	12.1	40.2	24.4	23.7	9.5	9.5	9.5
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.8	17.5		23.5	49.6	12.1	40.2	24.4	23.7	9.5	9.5	9.5
LOS	D	B		C	D	B	D	C	C	A	A	A
Approach Delay	28.8			35.5			28.7			12.6		
Approach LOS	C			D			C			B		
Queue Length 50th (m)	16.8	28.2		6.4	73.0	11.6	33.8	45.8	18.1	25.9		
Queue Length 95th (m)	#40.6	47.3		15.1	#121.7	31.0	#73.6	69.7	29.1	21.1		
Internal Link Dist (m)	392.1			351.9			301.3			222.1		
Turn Bay Length (m)	45.0			50.0			45.0	105.0		110.0		
Base Capacity (vph)	206	728		327	570	586	342	1389	245	1360		
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0		
Storage Cap Reductn	0	0		0	0	0	0	0	0	0		
Reduced v/c Ratio	0.81	0.39		0.16	0.82	0.43	0.69	0.46	0.54	0.34		

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

Maximum v/c Ratio:	0.89	Intersection LOS:	C
Intersection Signal Delay:	27.1	ICU Level of Service E	
Intersection Capacity Utilization:	89.2%		
Analysis Period (min):	15		
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			





Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

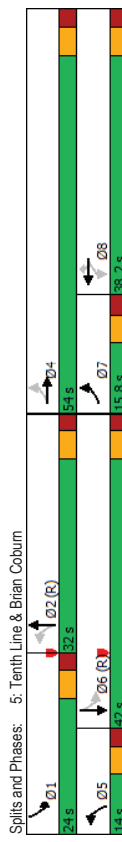
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Future Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Satd. Flow (prot)	1658	1647	0	1658	1745	1483	1566	3266	0	1658	3190	0
Flt Permitted	0.415			0.161			0.144				0.188	
Satd. Flow (perm)	717	1647	0	281	1745	1431	236	3266	0	327	3190	0
Satd. Flow (RTOR)	30			236			7			27		
Lane Group Flow (vph)	212	709	0	59	240	236	161	625	0	291	1012	0
Turn Type	pm-pt	NA	Perm	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	
Protected Phases	7	4		8	8	2	5	2	1	6		
Permitted Phases	4			8	8	2	5	2	6			
Detector Phase	7	4		8	8	2	5	2	1	6		
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	10.0
Minimum Split (s)	11.4	31.4	31.4	31.4	31.4	11.0	29.0	11.0	29.0	11.0	29.0	11.0
Total Split (s)	15.8	54.0	38.2	38.2	38.2	14.0	32.0	24.0	42.0	21.8%	38.2%	
Total Split (%)	14.4%	49.1%	34.7%	34.7%	34.7%	12.7%	29.1%	21.8%	38.2%			
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.4	6.4	6.4	6.4	6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	C-Max	None	None	None	C-Max	None
Act Effct Green (s)	47.2	47.2	31.4	31.4	31.4	36.0	27.6	49.8	36.0	49.8	36.0	36.0
Actuated G/C Ratio	0.43	0.43	0.29	0.29	0.29	0.33	0.25	0.45	0.33	0.45	0.33	0.33
v/c Ratio	0.55	0.98	0.74	0.48	0.41	0.90	0.76	0.83	0.95	0.83	0.95	0.95
Control Delay	26.7	59.5	74.4	26.9	4.9	80.3	42.8	53.7	45.5	53.7	45.5	45.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.7	59.5	74.4	26.9	4.9	80.3	42.8	53.7	45.5	53.7	45.5	45.5
LOS	C	E	E	C	A	F	D	D	D	D	D	D
Approach Delay	51.9		22.4			50.5		47.3				
Approach LOS	D		C			D		D				
Queue Length 50th (m)	28.9	141.3	7.2	29.3	0.0	24.2	66.4	37.2	99.3			
Queue Length 95th (m)	46.1	#220.7	#34.1	52.7	8.0	#64.3	54.8	#81.5	#146.9			
Internal Link Dist (m)	392.1		351.9			301.3		222.1				
Turn Bay Length (m)	45.0		50.0		45.0	105.0		110.0				
Base Capacity (vph)	388	729	81	504	581	178	825	367	1062			
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.97	0.73	0.48	0.41	0.90	0.76	0.79	0.95			

Intersection Summary
Cycle Length: 110
Actuated Cycle Length: 110
Offset: 3 (3%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle: 85
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

Maximum v/c Ratio: 0.98	Intersection LOS: D
Intersection Signal Delay: 45.5	ICU Level of Service H
Intersection Capacity Utilization 110.5%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



# Appendix J

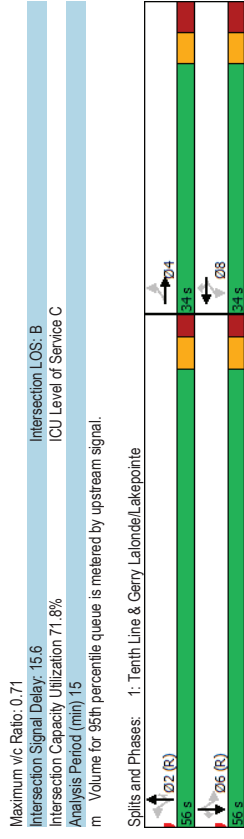
Synchro and Sidra Worksheets – 2031 Future Background Conditions

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	170	19	40	44	60	231	22	992	15	79	541	73
Traffic Volume (vph)	170	19	40	44	60	231	22	992	15	79	541	73
Future Volume (vph)	1566	1483	0	1642	1745	1483	1496	3283	1483	1626	3191	1441
Satd. Flow (prot)	0.718			0.719			0.450			0.256		
FI Permitted	1179	1483	0	1240	1745	1460	706	3283	1442	438	3191	1400
Satd. Flow (perm)	40			100			46					
Satd. Flow (RTOR)	170	59	0	44	60	231	22	992	15	79	541	73
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	8	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	8	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	8	8	8	8	2	2	2	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	56.0	56.0	56.0	56.0	56.0	56.0
Total Split (%)	37.8%	37.8%	37.8%	37.8%	37.8%	37.8%	62.2%	62.2%	62.2%	62.2%	62.2%	62.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.4	18.4	18.4	18.4	18.4	18.4	58.6	58.6	58.6	58.6	58.6	58.6
Actuated v/c Ratio	0.20	0.20	0.20	0.20	0.20	0.20	0.65	0.65	0.65	0.65	0.65	0.65
v/c Ratio	0.71	0.18	0.17	0.17	0.17	0.17	0.05	0.46	0.02	0.28	0.26	0.08
Control Delay	48.1	13.4	28.3	27.8	24.0	8.9	12.7	1.2	12.0	7.9	2.5	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.1	13.4	28.3	27.8	24.0	8.9	12.7	1.2	12.0	7.9	2.5	0.0
LOS	D	B	C	C	C	C	A	B	A	B	A	A
Approach Delay	39.2			25.3			12.4			7.8		
Approach LOS	D			C			B			A		
Queue Length 50th (m)	27.5	2.7	6.3	8.6	20.1	1.6	37.2	0.0	5.1	18.3	0.0	0.0
Queue Length 95th (m)	43.2	10.9	13.4	16.5	37.9	m3.4	90.6	m0.4	17.1	34.2	5.5	0.0
Internal Link Dist (m)	372.5			134.8			154.1			468.1		
Turn Bay Length (m)	30.0			50.0			35.0			70.0		
Base Capacity (vph)	356	476	374	527	511	459	2137	954	285	2077	937	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.12	0.12	0.11	0.45	0.05	0.46	0.02	0.28	0.26	0.08	0.08

Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 61 (68%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 65
Control Type: Actuated-Coordinated



Maximum v/c Ratio: 0.71
Intersection Signal Delay: 15.6
Intersection LOS: B
ICU Level of Service C
Intersection Capacity Utilization 71.8%
Analysis Period (min): 15
m Volume for 95th percentile queue is metered by upstream signal.



Maximum v/c Ratio: 0.71
Intersection Signal Delay: 15.6
Intersection LOS: B
ICU Level of Service C
Intersection Capacity Utilization 71.8%
Analysis Period (min): 15
m Volume for 95th percentile queue is metered by upstream signal.

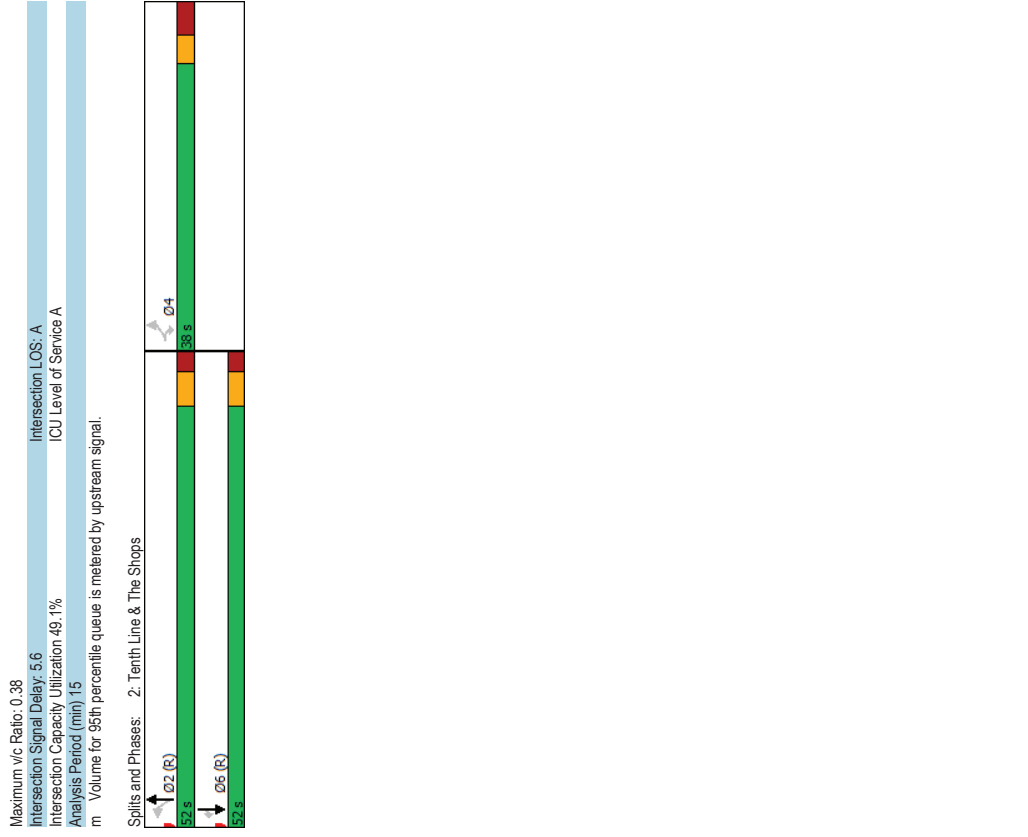


Maximum v/c Ratio: 0.71
Intersection Signal Delay: 15.6
Intersection LOS: B
ICU Level of Service C
Intersection Capacity Utilization 71.8%
Analysis Period (min): 15
m Volume for 95th percentile queue is metered by upstream signal.

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	54	13	70	970	563	61
Traffic Volume (vph)	54	13	70	970	563	61
Future Volume (vph)	54	13	70	970	563	61
Satd. Flow (prot)	1658	1483	1658	3252	3161	1483
Flt Permitted	0.950		0.441			
Satd. Flow (perm)	1656	1483	766	3252	3161	1437
Satd. Flow (RTOR)	13					61
Lane Group Flow (vph)	54	13	70	970	563	61
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	52.0	52.0	52.0	52.0
Total Split (%)	42.2%	42.2%	57.8%	57.8%	57.8%	57.8%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	10.3	10.3	71.3	71.3	71.3	71.3
Actuated g/C Ratio	0.11	0.11	0.79	0.79	0.79	0.79
v/c Ratio	0.29	0.07	0.12	0.38	0.22	0.05
Control Delay	40.6	18.5	6.0	5.6	2.3	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.6	18.5	6.0	5.6	2.3	0.5
LOS	D	B	A	A	A	A
Approach Delay	36.3		5.7	2.1		
Approach LOS	D		A	A		
Queue Length 50th (m)	8.7	0.0	3.0	23.0	8.6	0.0
Queue Length 95th (m)	19.3	5.2	m8.1	43.2	11.7	0.1
Internal Link Dist (m)	33.9		222.1	154.1		
Turn Bay Length (m)			75.0		60.0	
Base Capacity (vph)	574	522	607	2576	2504	1151
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.02	0.12	0.38	0.22	0.05
<b>Intersection Summary</b>						
Cycle Length: 90						
Actuated Cycle Length: 90						
Offset: 69 (77%), Referenced to phase 2:NBLT and 6:SBT, Start of Green						
Natural Cycle: 65						
Control Type: Actuated-Coordinated						



Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

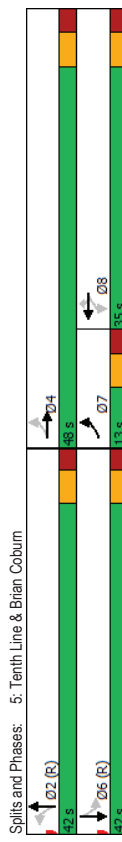
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	167	213	71	53	465	253	236	602	38	132	342	124
Traffic Volume (vph)	167	213	71	53	465	253	236	602	38	132	342	124
Future Volume (vph)	1642	1562	0	1610	1728	1483	1658	3216	0	1566	3071	0
Satd. Flow (prot)	0.169			0.586			0.455					
Flt Permitted	292	1562	0	993	1728	1455	792	3216	0	564	3071	0
Satd. Flow (perm)	25			161			8					
Satd. Flow (RTOR)	167	284	0	53	465	253	236	640	0	132	466	0
Lane Group Flow (vph)	pm-pt	NA	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	
Turn Type	7	4		8	8	2	2		6			
Protected Phases	4			8	8	2	2		6			
Permitted Phases	7	4		8	8	2	2		6			
Detector Phase	7	4		8	8	2	2		6			
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	
Minimum Split (s)	11.4	31.4		31.4	31.4	29.0	29.0	29.0	29.0	29.0	29.0	
Total Split (s)	13.0	48.0		35.0	35.0	42.0	42.0	42.0	42.0	42.0	42.0	
Total Split (%)	14.4%	53.3%		38.9%	38.9%	46.7%	46.7%	46.7%	46.7%	46.7%	46.7%	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	
Lead/Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	
Act Effct Green (s)	39.9	39.9		26.9	26.9	37.7	37.7	37.7	37.7	37.7	37.7	
Actuated G/C Ratio	0.44	0.44		0.30	0.30	0.42	0.42	0.42	0.42	0.42	0.42	
v/c Ratio	0.73	0.40		0.18	0.90	0.46	0.71	0.47	0.56	0.35	0.35	
Control Delay	36.5	17.0		24.3	52.7	12.3	42.7	25.5	25.4	10.0	10.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	36.5	17.0		24.3	52.7	12.3	42.7	25.5	25.4	10.0	10.0	
LOS	D	B		C	D	B	D	C	C	A	A	
Approach Delay	24.2			37.5			30.1			13.4		
Approach LOS	C			D			C			B		
Queue Length 50th (m)	16.6	28.0		6.6	74.5	11.5	35.4	47.2	18.3	26.2	26.2	
Queue Length 95th (m)	#36.5	46.8		15.4	#125.5	31.2	#74.7	69.8	#30.2	22.1	22.1	
Internal Link Dist (m)	392.1			351.9			301.3			222.1		
Turn Bay Length (m)	45.0			50.0			45.0	105.0		110.0		
Base Capacity (vph)	228	735		315	549	572	331	1353	236	1327	1327	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.73	0.39		0.17	0.85	0.44	0.71	0.47	0.56	0.35	0.35	

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
11-12-2021

Maximum v/c Ratio:	0.90	Intersection LOS:	C
Intersection Signal Delay:	27.5	ICU Level of Service E	
Intersection Capacity Utilization:	89.2%		
Analysis Period (min):	15		
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
20	348	31	50	593	24	123	23	28	12	10	55
Future Volume (vph)	20	348	31	50	593	24	123	23	28	12	55
Satd. Flow (prot)	1658	1646	0	1595	1717	0	1658	1554	0	1551	1511
Flt Permitted	0.371			0.537		0.715				0.724	
Satd. Flow (perm)	647	1646	0	902	1717	0	1248	1554	0	1144	1511
Satd. Flow (RTOR)	10			4		28				55	
Lane Group Flow (vph)	20	379	0	50	617	0	123	51	0	12	65
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2	2	6	6	6	4	4	8	8	8	
Permitted Phases	2	2	6	6	6	4	4	8	8	8	
Detector Phase											
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	43.0	43.0	43.0	43.0	43.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (%)	61.4%	61.4%	61.4%	61.4%	61.4%	38.6%	38.6%	38.6%	38.6%	38.6%	38.6%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4

Lead/Lag Optimize?

Max	Max	Max	None	None	None	None	None
Act Effct Green (s)	42.4	42.4	42.4	12.2	12.2	12.2	12.2
Actuated G/C Ratio	0.68	0.68	0.68	0.20	0.20	0.20	0.20
v/c Ratio	0.05	0.34	0.08	0.53	0.50	0.16	0.05
Control Delay	6.4	7.3	6.4	9.7	29.8	13.0	20.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.4	7.3	6.4	9.7	29.8	13.0	20.0
LOS	A	A	A	C	B	B	A
Approach Delay	7.3		9.5		24.9		11.0
Approach LOS	A		A		C		B
Queue Length 50th (m)	0.8	17.6	2.0	35.3	12.5	2.2	1.1
Queue Length 95th (m)	3.7	40.2	7.0	78.0	26.0	9.4	4.7
Internal Link Dist (m)				379.2		249.4	
Turn Bay Length (m)	65.0		65.0		30.0		30.0
Base Capacity (vph)	440	1124	614	1170	414	534	379
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.34	0.08	0.53	0.30	0.10	0.03

Intersection Summary

Cycle Length: 70
Actuated Cycle Length: 62.2
Natural Cycle: 60
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.53

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
20	348	31	50	593	24	123	23	28	12	10	55
Future Volume (vph)	20	348	31	50	593	24	123	23	28	12	55
Satd. Flow (prot)	1658	1646	0	1595	1717	0	1658	1554	0	1551	1511
Flt Permitted	0.371			0.537		0.715				0.724	
Satd. Flow (perm)	647	1646	0	902	1717	0	1248	1554	0	1144	1511
Satd. Flow (RTOR)	10			4		28				55	
Lane Group Flow (vph)	20	379	0	50	617	0	123	51	0	12	65
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2	2	6	6	6	4	4	8	8	8	
Permitted Phases	2	2	6	6	6	4	4	8	8	8	
Detector Phase											
Switch Phase											
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	43.0	43.0	43.0	43.0	43.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (%)	61.4%	61.4%	61.4%	61.4%	61.4%	38.6%	38.6%	38.6%	38.6%	38.6%	38.6%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4

Lead/Lag Optimize?

Max	Max	Max	None	None	None	None	None
Act Effct Green (s)	42.4	42.4	42.4	12.2	12.2	12.2	12.2
Actuated G/C Ratio	0.68	0.68	0.68	0.20	0.20	0.20	0.20
v/c Ratio	0.05	0.34	0.08	0.53	0.50	0.16	0.05
Control Delay	6.4	7.3	6.4	9.7	29.8	13.0	20.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.4	7.3	6.4	9.7	29.8	13.0	20.0
LOS	A	A	A	C	B	B	A
Approach Delay	7.3		9.5		24.9		11.0
Approach LOS	A		A		C		B
Queue Length 50th (m)	0.8	17.6	2.0	35.3	12.5	2.2	1.1
Queue Length 95th (m)	3.7	40.2	7.0	78.0	26.0	9.4	4.7
Internal Link Dist (m)				379.2		249.4	
Turn Bay Length (m)	65.0		65.0		30.0		30.0
Base Capacity (vph)	440	1124	614	1170	414	534	379
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.34	0.08	0.53	0.30	0.10	0.03

Intersection Summary

Cycle Length: 70
Actuated Cycle Length: 62.2
Natural Cycle: 60
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.53

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
11-12-2021

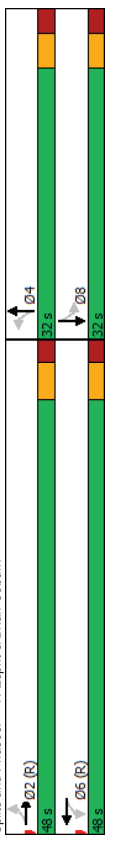
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	30	277	75	34	464	24	151	59	37	25	50	45
Traffic Volume (vph)	30	277	75	34	464	24	151	59	37	25	50	45
Future Volume (vph)	1642	1616	0	1851	1697	0	1688	1480	0	1566	1542	0
Satd. Flow (prot)	0.391	0.508		0.695			0.695			0.695		
FIIPermitted	674	1616	0	808	1697	0	1201	1480	0	1112	1542	0
Satd. Flow (perm)	26	26		5			37			45		
Satd. Flow (RTOR)	30	352	0	34	488	0	151	96	0	25	95	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	2		6	6		4		4		8	
Protected Phases	2	2		6	6		4		4		8	
Detector Phase	2	2		6	6		4		4		8	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0		10.0		10.0	
Minimum Split (s)	26.0	26.0		26.0	23.8		23.8		23.8		23.8	
Total Split (s)	48.0	48.0		48.0	48.0		32.0		32.0		32.0	
Total Split (%)	60.0%	60.0%		60.0%	60.0%		40.0%		40.0%		40.0%	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.3		3.3		3.3	
All-Red Time (s)	2.3	2.3		2.3	2.3		2.5		2.5		2.5	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0		0.0		0.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0		5.8		5.8		5.8	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		Max		Max		Max	
Act Effct Green (s)	42.0	42.0		42.0	42.0		26.2		26.2		26.2	
Actuated G/C Ratio	0.52	0.52		0.52	0.52		0.33		0.33		0.33	
v/c Ratio	0.08	0.41		0.08	0.55		0.38		0.19		0.07	
Control Delay	10.3	12.3		10.1	15.4		24.3		13.8		19.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0		0.0		0.0	
Total Delay	10.3	12.3		10.1	15.4		24.3		13.8		19.3	
LOS	B	B		B	B		C		B		B	
Approach Delay	12.2	12.2		15.1	15.1		20.2		13.7		13.7	
Approach LOS	B	B		B	B		C		B		B	
Queue Length 50th (m)	2.1	27.8		2.4	45.7		17.5		6.2		2.6	
Queue Length 95th (m)	6.2	46.3		6.7	72.2		33.2		16.5		7.8	
Internal Link Dist (m)	379.2	379.2		585.6	585.6		222.2		382.8		382.8	
Turn Bay Length (m)	65.0	65.0		65.0	65.0		30.0		30.0		30.0	
Base Capacity (vph)	353	860		424	893		393		509		364	
Starvation Cap Reductn	0	0		0	0		0		0		0	
Spillback Cap Reductn	0	0		0	0		0		0		0	
Storage Cap Reductn	0	0		0	0		0		0		0	
Reduced v/c Ratio	0.08	0.41		0.08	0.55		0.38		0.19		0.07	

Intersection Summary	
Cycle Length:	80
Actuated Cycle Length:	80
Offset:	0 (0%), Referenced to phase 2EBTL and 6:WBTL, Start of Green
Natural Cycle:	50
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
11-12-2021

Maximum v/c Ratio:	0.55
Intersection Signal Delay:	15.1
Intersection Capacity Utilization:	55.2%
Analysis Period (min):	15
Intersection LOS:	B
ICU Level of Service:	B

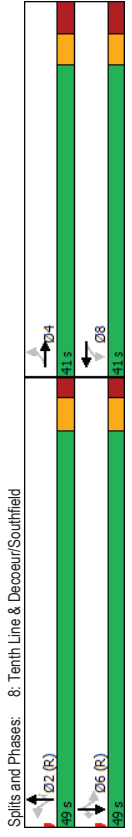


Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	86	27	44	9	29	70	79	715	1	19	400	59
Traffic Volume (vph)	86	27	44	9	29	70	79	715	1	19	400	59
Future Volume (vph)	1610	1389	0	1658	1545	0	1445	3131	1483	1523	3161	1401
Satd. Flow (prot)	0.693			0.711			0.516			0.373		
FI Permitted	1173	1389	0	1241	1545	0	781	3131	1442	597	3161	1359
Satd. Flow (perm)	44			70			47			59		
Satd. Flow (RTOR)	86	71	0	9	99	0	79	715	1	19	400	59
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase	4	4	4	8	8	8	2	2	2	6	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	49.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												

Maximum v/c Ratio: 0.41  
 Intersection Signal Delay: 9.1  
 Intersection LOS: A  
 ICU Level of Service B  
 Intersection Capacity Utilization 56.5%  
 Analysis Period (min) 15  
 Volume for 95th percentile queue is metered by upstream signal.



Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.1	16.1	16.1	65.7	65.7	65.7	65.7	65.7	65.7
Actuated G/C Ratio	0.18	0.18	0.18	0.73	0.73	0.73	0.73	0.73	0.73
v/c Ratio	0.41	0.25	0.04	0.30	0.14	0.31	0.00	0.04	0.17
Control Delay	36.0	14.9	25.1	12.6	8.7	7.6	0.0	6.8	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.0	14.9	25.1	12.6	8.7	7.6	0.0	6.8	5.0
LOS	D	B	C	B	A	A	A	A	A
Approach Delay	26.4		13.7		7.7			4.8	
Approach LOS	C		B		A			A	
Queue Length 50th (m)	14.1	4.2	1.4	4.5	3.5	19.4	0.0	0.3	3.4
Queue Length 95th (m)	20.2	11.3	3.9	12.9	16.5	56.7	0.0	mb.7	38.9
Internal Link Dist (m)	344.3		315.6		346.2			301.3	
Turn Bay Length (m)	45.0		20.0		90.0			60.0	
Base Capacity (vph)	444	553	470	628	569	2284	1065	485	2306
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.19	0.13	0.02	0.16	0.14	0.31	0.00	0.04	0.17

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	36 (40%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated



Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	351	33	76	326	58
Traffic Volume (vph)	135	3	12	70	1	290	5	351	33	76	326	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3183	0
Satd. Flow (prot)	0.457			0.748			0.524			0.524		
Flt Permitted	795	1433	0	1304	1447	0	776	3074	0	872	3183	0
Satd. Flow (perm)	12			290			17			35		
Satd. Flow (RTOR)	135	15	0	70	291	0	5	384	0	76	384	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	6	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	6	6	6	6
Permitted Phases	4	4	4	8	8	8	2	2	6	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	6	6	6	6
Switch Phase	4	4	4	8	8	8	2	2	6	6	6	6

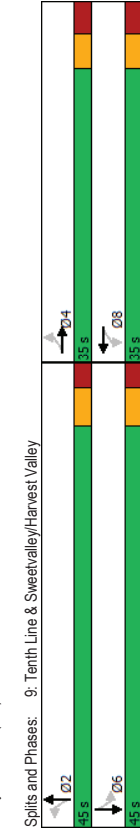
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2

Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	16.3	16.3	16.3	16.3	16.3	16.3	39.1	39.1	39.1	39.1	39.1	39.1
Actuated G/C Ratio	0.24	0.24	0.24	0.24	0.24	0.24	0.57	0.57	0.57	0.57	0.57	0.57
v/c Ratio	0.71	0.04	0.23	0.51	0.01	0.22	0.01	0.22	0.15	0.21	0.15	0.21
Control Delay	44.1	11.2	21.6	6.2	9.0	8.2	9.8	7.8	9.8	7.8	9.8	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.1	11.2	21.6	6.2	9.0	8.2	9.8	7.8	9.8	7.8	9.8	7.8
LOS	D	B	C	A	A	A	A	A	A	A	A	A

Approach Delay	40.9	9.2	8.3	8.3	8.3	8.3	8.1	8.1	8.1	8.1	8.1	8.1
Approach LOS	D	A	A	A	A	A	A	A	A	A	A	A
Queue Length 50th (m)	15.6	0.3	7.1	0.1	0.3	10.1	3.8	9.4	3.8	9.4	3.8	9.4
Queue Length 95th (m)	32.7	4.0	15.9	15.2	2.1	24.3	13.9	23.2	13.9	23.2	13.9	23.2
Internal Link Dist (m)	180.2		318.8		263.5		346.2		346.2		346.2	
Turn Bay Length (m)	38.0		60.0		54.0		65.0		65.0		65.0	
Base Capacity (vph)	334	610	549	777	444	1769	500	1840	500	1840	500	1840
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.40	0.02	0.13	0.37	0.01	0.22	0.15	0.21	0.15	0.21	0.15	0.21

Intersection Summary												
Cycle Length: 80												
Actuated Cycle Length: 68.2												
Natural Cycle: 65												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.71												

Intersection Signal Delay: 12.1	Intersection LOS: B
Intersection Capacity Utilization 77.1%	ICU Level of Service D
Analysis Period (min) 15	



Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

PM Peak Hour  
11-12-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	173	103	62	31	25	171	38	1013	63	263	1238	187
Traffic Volume (vph)	173	103	62	31	25	171	38	1013	63	263	1238	187
Future Volume (vph)	1658	1636	0	1658	1745	1483	1658	3316	1483	1658	3316	1455
Satd. Flow (prot)	0.741			0.577			0.190					
FI Permitted	1292	1636	0	1002	1745	1464	331	3316	1435	444	3316	1410
Satd. Flow (perm)	26			131			63					187
Satd. Flow (RTOR)	173	165	0	31	25	171	38	1013	63	263	1238	187
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	2	2	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase	4	4	4	8	8	8	2	2	2	2	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Total Split (s)	33.8	33.8	33.8	33.8	33.8	33.8	76.2	76.2	76.2	76.2	76.2	76.2
Total Split (%)	30.7%	30.7%	30.7%	30.7%	30.7%	30.7%	69.3%	69.3%	69.3%	69.3%	69.3%	69.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2

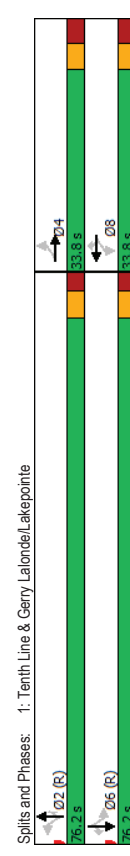
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	19.8	19.8	19.8	19.8	19.8	77.2	77.2	77.2	77.2	77.2	77.2
Actuated G/C Ratio	0.18	0.18	0.18	0.18	0.18	0.70	0.70	0.70	0.70	0.70	0.70
v/c Ratio	0.75	0.52	0.17	0.08	0.46	0.16	0.44	0.06	0.85	0.53	0.18
Control Delay	61.5	39.1	37.8	35.1	14.8	5.5	4.3	0.7	41.3	9.6	1.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0
Total Delay	61.5	39.1	37.8	35.1	14.8	5.5	4.3	0.7	41.3	9.6	1.5
LOS	E	D	D	D	B	A	A	A	D	A	A
Approach Delay	50.5	20.2				4.2			13.6		
Approach LOS	D	C				A			B		
Queue Length 50th (m)	35.5	27.1	5.7	4.5	7.3	1.5	21.4	0.2	36.8	59.2	0.0
Queue Length 95th (m)	54.8	44.2	13.2	11.0	24.6	4.0	29.3	0.9	#104.2	92.9	7.5
Internal Link Dist (m)	372.5		134.8			154.1			468.1		
Turn Bay Length (m)	30.0		50.0		35.0	55.0		70.0	50.0		75.0
Base Capacity (vph)	317	421	245	428	458	232	2328	1026	311	2328	1045
Starvation Cap Reductn	0	0	0	0	0	0	0	225	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.39	0.13	0.06	0.37	0.16	0.48	0.06	0.85	0.53	0.18

Intersection Summary
Cycle Length: 110
Actuated Cycle Length: 110
Offset: 92 (84%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle: 110
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

PM Peak Hour  
11-12-2021

Maximum v/c Ratio: 0.85	Intersection LOS: B
Intersection Signal Delay: 14.6	ICU Level of Service E
Intersection Capacity Utilization 66.8%	
Analysis Period (min) 15	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	





Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

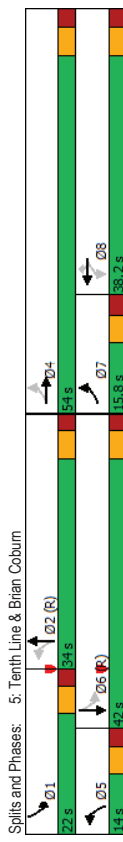
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Future Volume (vph)	212	469	240	59	240	236	161	578	47	291	821	191
Satd. Flow (prot)	1658	1647	0	1658	1745	1483	1566	3267	0	1658	3190	0
Flt Permitted	0.415			0.161			0.138			0.199		
Satd. Flow (perm)	717	1647	0	281	1745	1431	227	3267	0	346	3190	0
Satd. Flow (RTOR)	30			236			7			27		
Lane Group Flow (vph)	212	709	0	59	240	236	161	625	0	291	1012	0
Turn Type	pm-pt	NA	Perm	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	
Protected Phases	7	4		8	8	2	5	2	1	6		
Permitted Phases	4			8	8	2	5	2	6			
Detector Phase	7	4		8	8	2	5	2	1	6		
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	5.0	10.0	10.0		5.0	10.0	
Minimum Split (s)	11.4	31.4		31.4	31.4	11.0	29.0	11.0		29.0		
Total Split (s)	15.8	54.0		38.2	38.2	14.0	34.0	22.0		42.0		
Total Split (%)	14.4%	49.1%		34.7%	34.7%	12.7%	30.9%	20.0%		38.2%		
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7		3.7	3.7	
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3		2.3	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0		6.0	6.0	
Lead/Lag												
Lead/Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	C-Max	None		None	C-Max	
Act Effct Green (s)	47.2	47.2		31.4	31.4	37.2	28.8	49.9		36.0		
Actuated G/C Ratio	0.43	0.43		0.29	0.29	0.34	0.26	0.45		0.33		
v/c Ratio	0.55	0.98		0.74	0.48	0.41	0.90	0.73		0.85	0.95	
Control Delay	26.7	59.5		73.9	26.2	4.3	80.7	40.3		54.4	45.3	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	26.7	59.5		73.9	26.2	4.3	80.7	40.3		54.4	45.3	
LOS	C	E		E	C	A	F	D		D	D	
Approach Delay		51.9		21.8		48.5		47.3				
Approach LOS		D		C		D		D				
Queue Length 50th (m)	28.9	141.3		7.1	28.8	0.0	23.8	64.3		37.4	110.5	
Queue Length 95th (m)	46.1	#220.7		#34.1	49.4	6.3	#63.4	53.1		#63.0	#146.9	
Internal Link Dist (m)		392.1			351.9		301.3			222.1		
Turn Bay Length (m)	45.0			50.0		45.0	105.0			110.0		
Base Capacity (vph)	388	729		81	504	581	178	861		349	1062	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.65	0.97		0.73	0.48	0.41	0.90	0.73		0.83	0.95	

Intersection Summary	
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	3 (3%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
11-12-2021

Maximum v/c Ratio:	0.98
Intersection Signal Delay:	44.9
Intersection LOS:	D
Intersection Capacity Utilization:	110.5%
ICU Level of Service:	H
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

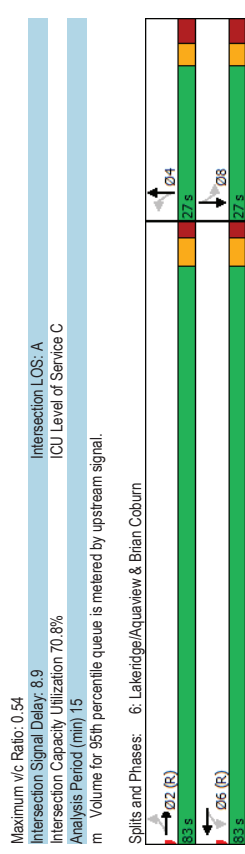
PM Peak Hour  
11-12-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	56	685	73	31	434	20	71	19	27	28	13	31
Traffic Volume (vph)	56	685	73	31	434	20	71	19	27	28	13	31
Future Volume (vph)	1658	1715	0	1658	1714	0	1626	1536	0	1523	1530	0
Satd. Flow (prot)	0.491	0.332	0.728									
Flt Permitted	849	1715	0	578	1714	0	1237	1536	0	1122	1530	0
Satd. Flow (perm)	12	5	27									
Satd. Flow (RTOR)	56	758	0	31	454	0	71	46	0	28	44	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	2	6	6	6	4	4	4	8	8	8	8
Permitted Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase	2	2	6	6	6	4	4	4	8	8	8	8
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	83.0	83.0	83.0	83.0	83.0	27.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (%)	75.5%	75.5%	75.5%	75.5%	75.5%	24.5%	24.5%	24.5%	24.5%	24.5%	24.5%	24.5%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None	None	None
Act Effct Green (s)	89.6	89.6	89.6	89.6	89.6	12.5	12.5	12.5	12.5	12.5	12.5	12.5
Actuated G/C Ratio	0.81	0.81	0.81	0.81	0.81	0.11	0.11	0.11	0.11	0.11	0.11	0.11
v/c Ratio	0.08	0.54	0.07	0.32	0.51	0.23	0.22	0.22	0.22	0.22	0.22	0.22
Control Delay	3.1	3.7	4.9	5.6	57.9	25.7	47.3	22.7	47.3	22.7	47.3	22.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	3.1	3.7	4.9	5.6	57.9	25.7	47.3	22.7	47.3	22.7	47.3	22.7
LOS	A	A	A	A	A	E	C	D	C	D	C	C
Approach Delay	3.7	5.6	5.6	5.6	45.3							
Approach LOS	A	A	A	A	D							
Queue Length 50th (m)	1.4	26.8	1.8	27.6	14.7	3.8	5.6	2.6	5.6	2.6	2.6	2.6
Queue Length 95th (m)	m2.7	m48.0	4.9	38.3	27.8	13.9	13.7	12.3	13.7	12.3	12.3	12.3
Internal Link Dist (m)	351.9		379.2		249.4							
Turn Bay Length (m)	65.0		65.0		30.0							
Base Capacity (vph)	691	1399	470	1397	231	309	210	311	210	311	210	311
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.54	0.07	0.32	0.31	0.15	0.13	0.14	0.13	0.14	0.13	0.14

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Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

PM Peak Hour  
11-12-2021



Scenario 1 2370 Tenth Line Rd 11:59 pm 09-07-2021 2026 Future Background  
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Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

PM Peak Hour  
11-12-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	52	491	188	27	344	18	103	43	25	23	48	38
Traffic Volume (vph)	52	491	188	27	344	18	103	43	25	23	48	38
Future Volume (vph)	1658	1672	0	1658	1714	0	1658	1549	0	1658	1487	0
Satd. Flow (prot)	0.525	0.324				0.701				0.713		
FI Permitted												
Satd. Flow (perm)	39	679	0	27	362	0	103	68	0	23	86	0
Satd. Flow (RTOR)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Lane Group Flow (vph)	2	2	2	6	6	6	4	4	4	8	8	8
Turn Type	2	2	2	6	6	6	4	4	4	8	8	8
Permitted Phases	2	2	2	6	6	6	4	4	4	8	8	8
Detector Phase	2	2	2	6	6	6	4	4	4	8	8	8
Switch Phase	2	2	2	6	6	6	4	4	4	8	8	8
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	81.0	81.0	81.0	81.0	81.0	81.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	73.6%	73.6%	73.6%	73.6%	73.6%	73.6%	26.4%	26.4%	26.4%	26.4%	26.4%	26.4%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8

Lead-Lag Optimize?

Recall Mode	C-Max	C-Max	C-Max	C-Max	Max	Max
Act Effct Green (s)	75.0	75.0	75.0	75.0	23.2	23.2
Actuated G/C Ratio	0.68	0.68	0.68	0.68	0.21	0.21
v/c Ratio	0.08	0.59	0.07	0.31	0.41	0.20
Control Delay	7.8	13.3	6.4	7.8	43.2	26.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	7.8	13.3	6.4	7.8	43.2	26.3
LOS	A	B	A	A	D	C
Approach Delay	12.9	7.7	7.7	36.5	27.8	27.8
Approach LOS	B	B	A	D	C	C
Queue Length 50th (m)	4.3	83.4	1.8	27.6	19.2	7.7
Queue Length 95th (m)	m6.2	97.2	4.7	41.1	35.9	19.6
Internal Link Dist (m)	379.2			585.6	222.2	382.8
Turn Bay Length (m)	65.0		65.0		30.0	30.0
Base Capacity (vph)	619	1152	385	1170	253	345
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.59	0.07	0.31	0.41	0.20

Intersection Summary

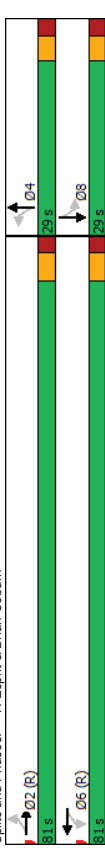
Cycle Length: 110
Actuated Cycle Length: 110
Offset: 24 (22%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

PM Peak Hour  
11-12-2021

Maximum v/c Ratio: 0.59
Intersection Signal Delay: 15.5
Intersection LOS: B
Intersection Capacity Utilization 70.4%
ICU Level of Service C
Analysis Period (min): 15
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Esprit & Brian Coburn



Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

PM Peak Hour  
11-12-2021

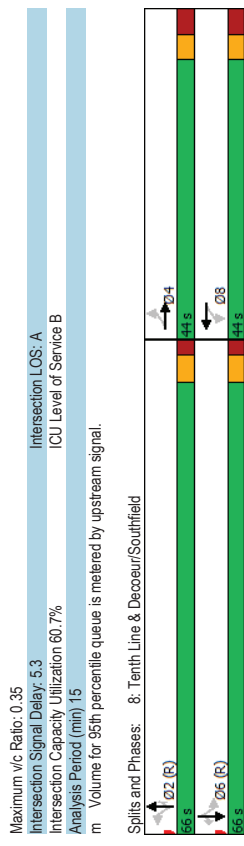
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	47	16	30	2	24	55	34	674	14	116	917	94
Future Volume (vph)	47	16	30	2	24	55	34	674	14	116	917	94
Satd. Flow (prot)	1658	1561	0	1658	1564	0	1658	3316	1483	1658	3316	1483
Flt Permitted	0.706			0.727			0.298			0.393		
Satd. Flow (perm)	1232	1561	0	1268	1564	0	519	3316	1483	686	3316	1433
Satd. Flow (RTOR)	30			55			39			39		94
Lane Group Flow (vph)	47	46	0	2	79	0	34	674	14	116	917	94
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4		8	8	8	2	2	2	2	6	6
Detector Phase	4	4		8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	44.0	44.0	44.0	44.0	44.0	44.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	60.0%	60.0%	60.0%	60.0%	60.0%	60.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9

Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.1	15.1	15.1	86.7	86.7	86.7	86.7	86.7	86.7	86.7
Actuated g/C Ratio	0.14	0.14	0.14	0.79	0.79	0.79	0.79	0.79	0.79	0.79
v/c Ratio	0.28	0.19	0.01	0.30	0.08	0.26	0.01	0.21	0.35	0.08
Control Delay	43.4	19.6	34.0	17.9	5.5	4.3	0.0	3.7	3.0	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	19.6	34.0	17.9	5.5	4.3	0.0	3.7	3.0	0.3
LOS	D	B	C	B	A	A	A	A	A	A
Approach Delay				18.3	4.3					
Approach LOS				B	A					
Queue Length 50th (m)	9.7	3.2	0.4	4.8	1.1	13.3	0.0	2.7	12.5	0.0
Queue Length 95th (m)	16.3	10.7	2.1	14.4	6.2	34.0	m0.0	m7.4	m30.8	m0.0
Internal Link Dist (m)				344.3	315.6	346.2			301.3	
Turn Bay Length (m)	45.0		20.0		90.0	60.0	60.0	60.0	70.0	
Base Capacity (vph)	415	546	427	563	409	2613	1176	540	2613	1149
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.08	0.00	0.14	0.08	0.26	0.01	0.21	0.35	0.08

Intersection Summary
Cycle Length: 110
Actuated Cycle Length: 110
Offset: 109 (99%), Referenced to phase 2: NBTL and 6: SBT, Start of Green
Natural Cycle: 70
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

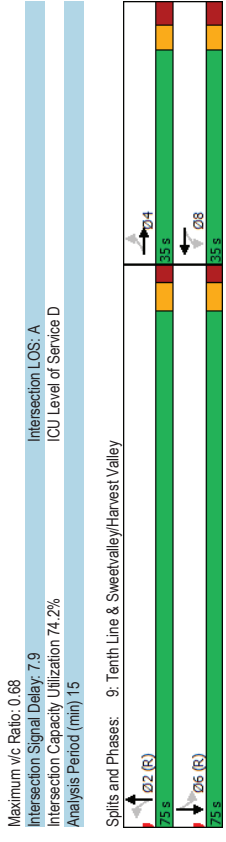
PM Peak Hour  
11-12-2021



Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR	SBT
Lane Configurations	95	2	7	17	1	167	14	442	81	293	478	168
Traffic Volume (vph)	95	2	7	17	1	167	14	442	81	293	478	168
Future Volume (vph)	1658	1925	0	1595	1464	0	1658	3239	0	1658	3163	0
Satd. Flow (prot)	0.530			0.752			0.403			0.458		
Flt Permitted												
Satd. Flow (perm)	923	1525	0	1261	1464	0	703	3239	0	799	3163	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	95	9	0	17	168	0	14	523	0	293	646	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	4	4	8	8	8	2	2	2	6	6	6	6
Detector Phase	4	4	8	8	8	2	2	2	6	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	75.0	75.0	75.0	75.0	75.0	75.0	75.0
Total Split (%)	31.8%	31.8%	31.8%	31.8%	31.8%	68.2%	68.2%	68.2%	68.2%	68.2%	68.2%	68.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.7	16.7	16.7	16.7	16.7	80.6	80.6	80.6	80.6	80.6	80.6	80.6
Actuated G/C Ratio	0.15	0.15	0.15	0.15	0.15	0.73	0.73	0.73	0.73	0.73	0.73	0.73
v/c Ratio	0.68	0.04	0.09	0.46	0.03	0.03	0.22	0.50	0.28	0.50	0.28	0.28
Control Delay	66.1	21.9	37.1	9.9	6.1	5.3	5.2	5.2	5.2	5.2	1.2	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.1	21.9	37.1	9.9	6.1	5.3	5.2	5.2	5.2	5.2	1.2	1.2
LOS	E	C	D	A	A	A	A	A	A	A	A	A
Approach Delay	62.3		12.4			5.3						
Approach LOS	E		B			A						
Queue Length 50th (m)	19.8	0.4	3.2	0.2	0.7	14.0				13.5		5.0
Queue Length 95th (m)	33.0	4.3	8.4	16.1	3.5	30.2				5.5		0.0
Internal Link Dist (m)	180.2		318.8			263.5				346.2		
Turn Bay Length (m)	38.0		60.0			54.0				65.0		
Base Capacity (vph)	239	400	326	503	514	2382				585		2339
Starvation Cap Reductn	0	0	0	0	0	0				0		0
Spillback Cap Reductn	0	0	0	0	0	0				0		0
Storage Cap Reductn	0	0	0	0	0	0				0		0
Reduced v/c Ratio	0.40	0.02	0.05	0.33	0.03	0.22				0.50		0.28
Intersection Summary												
Cycle Length	110											
Actuated Cycle Length	110											
Offset	14 (13%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green											
Natural Cycle	75											
Control Type	Actuated-Coordinated											





## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde AM FB2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	83	2.0	0.186	9.6 LOS A	1.0	7.4	0.55	0.65	0.55	50.0		
2	T1	21	2.0	0.186	4.4 LOS A	1.0	7.4	0.55	0.65	0.55	46.8		
3	R2	78	2.0	0.186	4.8 LOS A	1.0	7.4	0.55	0.65	0.55	48.8		
Approach													
		182	2.0	0.186	6.9 LOS A	1.0	7.4	0.55	0.65	0.55	49.1		
East: Brian Coburn													
4	L2	44	2.0	0.817	12.2 LOS B	12.6	89.6	0.83	0.65	0.88	50.3		
5	T1	989	2.0	0.817	6.9 LOS A	12.6	89.6	0.83	0.65	0.88	53.5		
6	R2	13	2.0	0.817	7.0 LOS A	12.6	89.6	0.83	0.65	0.88	48.7		
Approach													
		1046	2.0	0.817	7.1 LOS A	12.6	89.6	0.83	0.65	0.88	53.3		
North: Gerry Lalonde													
7	L2	7	2.0	0.559	28.2 LOS C	5.3	38.0	1.00	1.15	1.35	40.8		
8	T1	8	2.0	0.559	23.0 LOS C	5.3	38.0	1.00	1.15	1.35	38.6		
9	R2	185	2.0	0.559	23.4 LOS C	5.3	38.0	1.00	1.15	1.35	40.0		
Approach													
		200	2.0	0.559	23.6 LOS C	5.3	38.0	1.00	1.15	1.35	39.9		
West: Brian Coburn													
10u	U	30	2.0	0.314	11.3 LOS B	2.3	16.1	0.26	0.44	0.26	56.9		
10	L2	40	2.0	0.314	9.2 LOS A	2.3	16.1	0.26	0.44	0.26	52.2		
11	T1	330	2.0	0.314	3.8 LOS A	2.3	16.1	0.26	0.44	0.26	55.7		
12	R2	48	2.0	0.314	3.9 LOS A	2.3	16.1	0.26	0.44	0.26	50.5		
Approach													
		448	2.0	0.314	4.8 LOS A	2.3	16.1	0.26	0.44	0.26	54.8		
All Vehicles													
		1876	2.0	0.817	8.3 LOS A	12.6	89.6	0.88	0.65	0.75	51.4		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).  
Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 15:00:24 PM  
Project: CGH TRANSPORTATION/CGH Working - Documents/Projects/2021-052 Mattamy 2370 Tenth Line/DATA/Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde PM FB2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	37	2.0	0.365	27.2 LOS C	3.0	21.1	1.00	1.02	1.04	40.5		
2	T1	10	2.0	0.365	22.0 LOS C	3.0	21.1	1.00	1.02	1.04	38.4		
3	R2	36	2.0	0.365	22.4 LOS C	3.0	21.1	1.00	1.02	1.04	39.7		
Approach													
		83	2.0	0.365	24.5 LOS C	3.0	21.1	1.00	1.02	1.04	39.9		
East: Brian Coburn													
4	L2	62	2.0	0.552	11.0 LOS B	4.7	33.7	0.71	0.63	0.71	50.7		
5	T1	512	2.0	0.552	5.7 LOS A	4.7	33.7	0.71	0.63	0.71	53.9		
6	R2	12	2.0	0.552	5.8 LOS A	4.7	33.7	0.71	0.63	0.71	49.0		
Approach													
		586	2.0	0.552	6.3 LOS A	4.7	33.7	0.71	0.63	0.71	53.4		
North: Gerry Lalonde													
7	L2	4	2.0	0.153	11.1 LOS B	1.0	6.8	0.73	0.71	0.73	50.4		
8	T1	18	2.0	0.153	5.9 LOS A	1.0	6.8	0.73	0.71	0.73	47.1		
9	R2	92	2.0	0.153	6.3 LOS A	1.0	6.8	0.73	0.71	0.73	49.1		
Approach													
		114	2.0	0.153	6.4 LOS A	1.0	6.8	0.73	0.71	0.73	48.8		
West: Brian Coburn													
10u	U	27	2.0	0.923	13.0 LOS B	25.4	180.7	1.00	0.51	1.00	53.7		
10	L2	212	2.0	0.923	10.8 LOS B	25.4	180.7	1.00	0.51	1.00	49.5		
11	T1	1037	2.0	0.923	5.5 LOS A	25.4	180.7	1.00	0.51	1.00	52.6		
12	R2	65	2.0	0.923	5.6 LOS A	25.4	180.7	1.00	0.51	1.00	48.0		
Approach													
		1341	2.0	0.923	6.5 LOS A	25.4	180.7	1.00	0.51	1.00	51.9		
All Vehicles													
		2124	2.0	0.923	7.1 LOS A	25.4	180.7	0.91	0.57	0.91	51.5		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).  
Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 15:00:25 PM  
Project: CGH TRANSPORTATION/CGH Working - Documents/Projects/2021-052 Mattamy 2370 Tenth Line/DATA/Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Strasbourg AM FB2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	HV %	Deg. Sat	Average Delay sec	Level of Service	95% Back of Queue Vehicles	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South: des Subespines													
1	L2	109	2.0	0.181	9.5	LOSA	1.0	7.2	0.54	0.66	0.54	49.6	
2	T1	15	2.0	0.181	4.3	LOSA	1.0	7.2	0.54	0.66	0.54	46.5	
3	R2	55	2.0	0.181	4.7	LOSA	1.0	7.2	0.54	0.66	0.54	48.4	
Approach													
		179	2.0	0.181	7.6	LOSA	1.0	7.2	0.54	0.66	0.54	48.9	
East: Brian Coburn													
4	L2	32	2.0	0.661	10.1	LOSB	7.1	50.5	0.58	0.51	0.58	51.3	
5	T1	834	2.0	0.661	4.8	LOSA	7.1	50.5	0.58	0.51	0.58	54.7	
6	R2	12	2.0	0.661	4.9	LOSA	7.1	50.5	0.58	0.51	0.58	49.7	
Approach													
		878	2.0	0.661	5.0	LOSA	7.1	50.5	0.58	0.51	0.58	54.5	
North: Strasbourg													
7	L2	25	2.0	0.234	15.3	LOSB	1.6	11.7	0.90	0.88	0.90	47.2	
8	T1	22	2.0	0.234	10.1	LOSB	1.6	11.7	0.90	0.88	0.90	44.4	
9	R2	76	2.0	0.234	10.5	LOSB	1.6	11.7	0.90	0.88	0.90	46.1	
Approach													
		123	2.0	0.234	11.4	LOSB	1.6	11.7	0.90	0.88	0.90	46.0	
West: Brian Coburn													
10	L2	7	2.0	0.295	9.3	LOSA	2.0	14.3	0.29	0.41	0.29	52.6	
11	T1	360	2.0	0.295	3.9	LOSA	2.0	14.3	0.29	0.41	0.29	56.1	
12	R2	38	2.0	0.295	4.1	LOSA	2.0	14.3	0.29	0.41	0.29	50.9	
Approach													
		405	2.0	0.295	4.1	LOSA	2.0	14.3	0.29	0.41	0.29	55.5	
All Vehicles													
		1585	2.0	0.661	5.5	LOSA	7.1	50.5	0.53	0.53	0.53	53.3	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Modal Designation.

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Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 5:00:23 PM  
Project: C:\Users\AndrewHarte\CGH\_TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.sp8

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Strasbourg PM FB2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	HV %	Deg. Sat	Average Delay sec	Level of Service	95% Back of Queue Vehicles	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South: des Subespines													
1	L2	58	2.0	0.209	15.1	LOSB	1.4	10.3	0.89	0.88	0.89	46.4	
2	T1	18	2.0	0.209	9.9	LOSA	1.4	10.3	0.89	0.88	0.89	43.7	
3	R2	34	2.0	0.209	10.3	LOSB	1.4	10.3	0.89	0.88	0.89	45.4	
Approach													
		110	2.0	0.209	12.8	LOSB	1.4	10.3	0.89	0.88	0.89	45.7	
East: Brian Coburn													
4	L2	54	2.0	0.446	9.6	LOSA	3.6	25.7	0.41	0.47	0.41	51.9	
5	T1	503	2.0	0.446	4.2	LOSA	3.6	25.7	0.41	0.47	0.41	55.3	
6	R2	36	2.0	0.446	4.3	LOSA	3.6	25.7	0.41	0.47	0.41	50.2	
Approach													
		593	2.0	0.446	4.7	LOSA	3.6	25.7	0.41	0.47	0.41	54.7	
North: Strasbourg													
7	L2	23	2.0	0.073	10.7	LOSB	0.4	2.9	0.65	0.66	0.65	49.5	
8	T1	13	2.0	0.073	5.5	LOSA	0.4	2.9	0.65	0.66	0.65	46.4	
9	R2	23	2.0	0.073	5.9	LOSA	0.4	2.9	0.65	0.66	0.65	48.3	
Approach													
		59	2.0	0.073	7.7	LOSA	0.4	2.9	0.65	0.66	0.65	48.3	
West: Brian Coburn													
10	L2	33	2.0	0.734	9.9	LOSA	9.1	65.0	0.54	0.46	0.54	51.6	
11	T1	913	2.0	0.734	4.6	LOSA	9.1	65.0	0.54	0.46	0.54	54.9	
12	R2	106	2.0	0.734	4.7	LOSA	9.1	65.0	0.54	0.46	0.54	49.9	
Approach													
		1052	2.0	0.734	4.7	LOSA	9.1	65.0	0.54	0.46	0.54	54.3	
All Vehicles													
		1814	2.0	0.734	5.3	LOSA	9.1	65.0	0.52	0.49	0.52	53.6	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Modal Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com  
Organisation: CGH TRANSPORTATION | Processed: November 15, 2021 5:00:23 PM  
Project: C:\Users\AndrewHarte\CGH\_TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.sp8

# Appendix K

MMLOS Analysis

# Multi-Modal Level of Service - Segments Form

Consultant Scenario Comments	CGH Transportation Inc.	Project Date	2370 Tenth Line Road
	Existing/Future		2021-11-30

SEGMENTS			Section	Section	Section
			Brian Coburn Blvd	Tenth Line Rd	Decoeur Dr
<b>Pedestrian</b>	Sidewalk Width	<b>-</b>	no sidewalk	≥ 2 m	1.8 m
	Boulevard Width		n/a	> 2 m	0.5 - 2 m
	Avg Daily Curb Lane Traffic Volume		> 3000	> 3000	≤ 3000
	Operating Speed		> 60 km/h	> 60 km/h	> 50 to 60 km/h
	On-Street Parking		no	no	no
	<b>Exposure to Traffic PLoS</b>		<b>F</b>	<b>D</b>	<b>B</b>
	Effective Sidewalk Width				
	Pedestrian Volume				
<b>Crowding PLoS</b>	<b>-</b>	<b>-</b>	<b>-</b>		
<b>Level of Service</b>	<b>-</b>	<b>-</b>	<b>-</b>		
<b>Bicycle</b>	Type of Cycling Facility	<b>F</b>	Mixed Traffic	Curbside Bike Lane	Mixed Traffic
	Number of Travel Lanes		2-3 lanes total	2 ea. dir. (w median)	≤ 2 (no centreline)
	Operating Speed		≥ 60 km/h	>50 to 70 km/h	≥ 50 to 60 km/h
	<b># of Lanes &amp; Operating Speed LoS</b>		<b>F</b>	<b>C</b>	<b>D</b>
	Bike Lane (+ Parking Lane) Width			≥ 1.8 m	
	<b>Bike Lane Width LoS</b>		<b>-</b>	<b>A</b>	<b>-</b>
	Bike Lane Blockages			Rare	
	<b>Blockage LoS</b>		<b>-</b>	<b>A</b>	<b>-</b>
	Median Refuge Width (no median = < 1.8 m)		< 1.8 m refuge	< 1.8 m refuge	< 1.8 m refuge
	No. of Lanes at Unsignalized Crossing		≤ 3 lanes	≤ 3 lanes	≤ 3 lanes
	Sidestreet Operating Speed		≤ 40 km/h	≤ 40 km/h	≤ 40 km/h
	<b>Unsignalized Crossing - Lowest LoS</b>		<b>A</b>	<b>A</b>	<b>A</b>
<b>Level of Service</b>	<b>F</b>	<b>C</b>	<b>D</b>		
<b>Transit</b>	Facility Type	<b>D</b>	Mixed Traffic		Mixed Traffic
	Friction or Ratio Transit:Posted Speed		Vt/Vp ≥ 0.8		Vt/Vp ≥ 0.8
	<b>Level of Service</b>		<b>D</b>	<b>-</b>	<b>D</b>
<b>Truck</b>	Truck Lane Width	<b>B</b>	> 3.7 m	≤ 3.5 m	
	Travel Lanes per Direction		1	> 1	
	<b>Level of Service</b>		<b>B</b>	<b>A</b>	<b>-</b>

**Multi-Modal Level of Service - Intersections Form**

Consultant	CGH Transportation Inc.
Scenario	2370 Tenth Line Road
Comments	2021-11-30

Project	2370 Tenth Line Road
Date	2021-11-30

		Gerry Lalonde Drive / Lakepointe Drive at Tenth Line Road				The Shops of Tenth Line Access at Tenth Line Road				Brian Coburn Boulevard at Tenth Line Road			
		NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
Pedestrian	Crossing Side	8	8	7	7	7	7	7	5	7	5	6	
	Lanes	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	
	Median	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	
	Conflicting Left Turns	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	
	Conflicting Right Turns	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	
	Right Turns on Red (RTOR)?	No	No	No	No	No	No	No	No	No	No	No	
	Ped Signal Leading Interval?	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	
	Right Turn Channel	10-15m	10-15m	10-15m	10-15m	5-10m	5-10m	5-10m	10-15m	10-15m	10-15m	10-15m	
	Corner Radius	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	
	Crosswalk Type	-12	-12	4	4	19	13	-	38	-6	-12	37	20
PETSI Score	F	F	F	F	F	F	-	E	F	F	E	F	
Ped. Exposure to Traffic LoS	90	90	100	100	90	90	-	90	100	100	100	100	
Cycle Length	36	36	7	7	37	46	-	7	31	31	18	18	
Effective Walk Time	16	16	43	43	16	11	-	38	24	24	34	34	
Average Pedestrian Delay	B	B	E	E	B	B	-	D	C	C	D	D	
Pedestrian Delay LoS	F	F	F	F	F	F	-	E	F	F	E	F	
Level of Service	F												
Approach From		NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
Bicycle	Bicycle Lane Arrangement on Approach	Pocket Bike Lane	Pocket Bike Lane	Mixed Traffic	Mixed Traffic	Pocket Bike Lane	Pocket Bike Lane	Mixed Traffic	Mixed Traffic	Pocket Bike Lane	Pocket Bike Lane	Mixed Traffic	Mixed Traffic
	Right Turn Lane Configuration	> 50 m Introduced right turn lane	> 50 m Introduced right turn lane	≤ 50 m	≤ 50 m	> 50 m Introduced right turn lane	> 50 m Introduced right turn lane	≤ 50 m	≤ 50 m	≤ 50 m	≤ 50 m	≤ 50 m	≤ 50 m
	Right Turning Speed	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h	≤ 25 km/h
	Cyclist relative to RT motorists	D	D	D	D	D	D	D	D	D	D	D	D
	Separated or Mixed Traffic	Separated	Separated	Mixed Traffic	Mixed Traffic	Separated	Separated	Mixed Traffic	Mixed Traffic	Separated	Separated	Mixed Traffic	Mixed Traffic
	Left Turn Approach	≥ 2 lanes crossed	≥ 2 lanes crossed	One lane crossed	No lane crossed	≥ 2 lanes crossed	≥ 2 lanes crossed	One lane crossed	No lane crossed	≥ 2 lanes crossed	≥ 2 lanes crossed	One lane crossed	One lane crossed
	Operating Speed	≥ 60 km/h	≥ 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	≥ 60 km/h	≥ 60 km/h	> 40 to ≤ 50 km/h	> 40 to ≤ 50 km/h	≥ 60 km/h	≥ 60 km/h	≥ 60 km/h	≥ 60 km/h
	Left Turning Cyclist	F	F	E	C	F	F	E	B	F	F	F	F
	Level of Service	F	F	E	C	D	F	E	B	F	F	F	F
Transit	Average Signal Delay	≤ 10 sec	-	-	> 40 sec	-	-	-	-	-	-	> 40 sec	> 40 sec
	Level of Service	B	-	-	F	-	-	-	-	-	-	F	F
Truck	Effective Corner Radius	-	-	-	-	-	-	-	-	-	-	-	-
	Number of Receiving Lanes on Departure from Intersection	-	-	-	-	-	-	-	-	-	-	-	-
	Level of Service	-	-	-	-	-	-	-	-	-	-	-	-
Auto	Volume to Capacity Ratio	0.81 - 0.90	0.81 - 0.90	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60
	Level of Service	D	D	A	A	D	A	A	A	D	A	F	F

Brian Coburn Boulevard at Aquaview Drive / Lakeridge Drive				Brian Coburn Boulevard at Esprit Drive				Decoeur Drive / Southfield Way at Tenth Line Road				Sweetvalley Drive / Harvest Valley Avenue at Tenth Line Road			
NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
5	4	5	4	5	5	5	5	8	8	5	4	7	7	5	4
No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m	No Median - 2.4 m
Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive
Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control	Permissive or yield control
RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed	RTOR allowed
No	No	No	No	No	No	No	No	No	No	No	No	No	No	No	No
No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel
10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m	10-15m
Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings	Std transverse markings
37	53	37	53	37	37	37	37	-12	-12	37	53	4	4	37	53
E	D	E	D	E	E	E	E	F	F	E	D	F	F	E	D
80	80	70	70	80	80	80	80	90	90	100	100	80	80	100	100
12	12	24	24	15	15	29	29	27	27	7	7	23	23	8	8
29	29	15	15	26	26	16	16	22	22	43	43	20	20	42	42
C	C	B	B	C	C	B	B	C	C	E	E	C	C	E	E
E	D	E	D	E	E	E	E	F	F	E	E	F	F	E	E
E	E	E	E	E	E	E	E	F	F	E	E	F	F	E	E
E	E	E	E	E	E	E	E	F	F	E	E	F	F	E	E
NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST	NORTH	SOUTH	EAST	WEST
Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Pocket Bike Lane	Pocket Bike Lane	Mixed Traffic	Mixed Traffic	Pocket Bike Lane	Pocket Bike Lane	Mixed Traffic	Mixed Traffic
-	-	-	-	-	-	-	-	D	D	-	-	-	-	-	-
Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic	Separated	Separated	Mixed Traffic	Mixed Traffic	Separated	Separated	Mixed Traffic	Mixed Traffic
One lane crossed	One lane crossed	One lane crossed	One lane crossed	One lane crossed	One lane crossed	One lane crossed	One lane crossed	≥ 2 lanes crossed	≥ 2 lanes crossed	One lane crossed	One lane crossed	≥ 2 lanes crossed	≥ 2 lanes crossed	One lane crossed	One lane crossed
> 50 to < 60 km/h	> 50 to < 60 km/h	≥ 60 km/h	≥ 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	≥ 60 km/h	≥ 60 km/h	≥ 60 km/h	≥ 60 km/h	> 50 to < 60 km/h	> 50 to < 60 km/h	≥ 60 km/h	≥ 60 km/h	> 50 to < 60 km/h	> 40 to ≤ 50 km/h
E	E	F	F	E	E	F	F	F	F	E	E	F	F	E	D
E	E	F	F	E	E	F	F	F	F	E	E	F	F	E	D
F	F	F	F	F	F	F	F	F	F	F	F	F	F	F	F
≤ 20 sec	≤ 20 sec	≤ 20 sec	≤ 10 sec	≤ 20 sec	≤ 20 sec	≤ 20 sec	≤ 40 sec	≤ 10 sec	≤ 10 sec	≤ 10 sec	≤ 10 sec	≤ 20 sec	≤ 20 sec	≤ 10 sec	≤ 10 sec
-	-	C	B	C	C	-	E	B	B	-	E	C	-	B	-
C	C	C	C	C	C	C	E	E	E	E	E	C	C	C	C
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
0.61 - 0.70	0.61 - 0.70	0.61 - 0.70	0.61 - 0.70	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60	0.0 - 0.60
B	B	B	B	A	A	A	A	A	A	A	A	A	A	A	A

# Appendix L

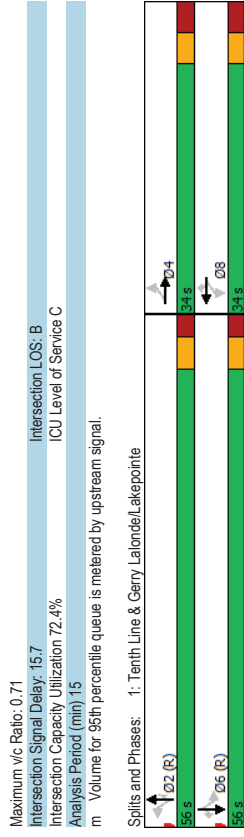
Synchro and Sidra Worksheets – 2026 Future Total Conditions

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	170	19	40	44	60	231	22	1012	15	79	552	73
Traffic Volume (vph)	170	19	40	44	60	231	22	1012	15	79	552	73
Future Volume (vph)	1566	1483	0	1642	1745	1483	1496	3283	1483	1626	3191	1441
Satd. Flow (prot)	0.718			0.446			0.249					
Flt Permitted	1179	1483	0	1240	1745	1460	700	3283	1442	426	3191	1400
Satd. Flow (perm)	40			95			46					
Satd. Flow (RTOR)	170	59	0	44	60	231	22	1012	15	79	552	73
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	8	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	8	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	8	8	8	8	2	2	2	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	56.0	56.0	56.0	56.0	56.0	56.0
Total Split (%)	37.8%	37.8%	37.8%	37.8%	37.8%	37.8%	62.2%	62.2%	62.2%	62.2%	62.2%	62.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.4	18.4	18.4	18.4	18.4	18.4	58.6	58.6	58.6	58.6	58.6	58.6
Actuated v/c Ratio	0.20	0.20	0.20	0.20	0.20	0.20	0.65	0.65	0.65	0.65	0.65	0.65
v/c Ratio	0.71	0.18	0.17	0.17	0.17	0.62	0.05	0.47	0.02	0.29	0.27	0.08
Control Delay	48.1	13.4	28.3	27.8	25.1	8.8	12.8	1.1	12.3	7.9	2.5	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.1	13.4	28.3	27.8	25.1	8.8	12.8	1.1	12.3	7.9	2.5	2.5
LOS	D	B	C	C	C	C	A	B	A	B	A	A
Approach Delay	39.2			26.0			12.6			7.9		
Approach LOS	D			C			B			A		
Queue Length 50th (m)	27.5	2.7	6.3	8.6	21.0	1.6	38.1	0.0	5.2	18.7	0.0	0.0
Queue Length 95th (m)	43.2	10.9	13.4	16.5	38.8	m3.2	92.4	m0.4	17.3	34.8	5.5	5.5
Internal Link Dist (m)	372.5			134.8			154.1			468.1		
Turn Bay Length (m)	30.0			50.0			35.0			70.0		
Base Capacity (vph)	356	476	374	527	507	455	2137	954	277	2077	937	750
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.12	0.12	0.11	0.46	0.05	0.47	0.02	0.29	0.27	0.08	0.08

Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 61 (68%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 65
Control Type: Actuated-Coordinated



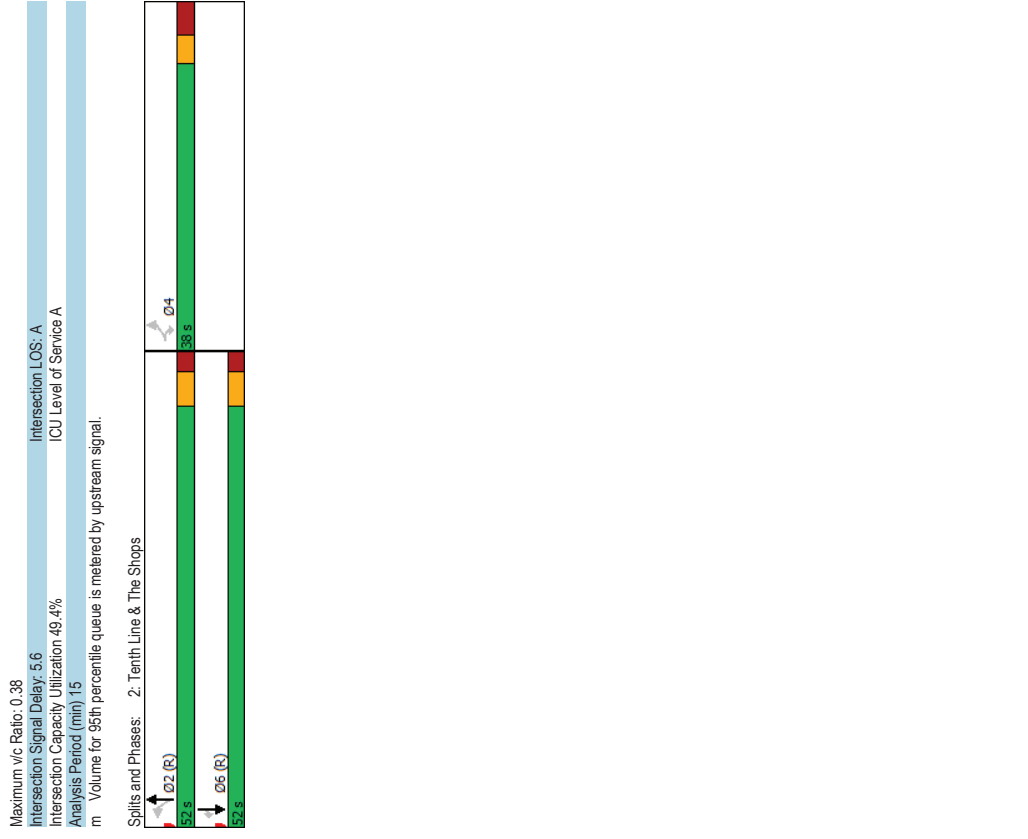
Maximum v/c Ratio: 0.71  
Intersection Signal Delay: 15.7  
Intersection LOS: B  
ICU Level of Service C  
Intersection Capacity Utilization 72.4%  
Analysis Period (min) 15  
Volume for 95th percentile queue is metered by upstream signal.  
m  
Splits and Phases: 1: Tenth Line & Gerry Lalonde/Lakepointe



Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	54	13	70	990	574	61
Traffic Volume (vph)	54	13	70	990	574	61
Future Volume (vph)	54	13	70	990	574	61
Satd. Flow (prot)	1658	1483	1658	3252	3161	1483
Flt Permitted	0.950		0.436			
Satd. Flow (perm)	1656	1483	757	3252	3161	1437
Satd. Flow (RTOR)	13					61
Lane Group Flow (vph)	54	13	70	990	574	61
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	52.0	52.0	52.0	52.0
Total Split (%)	42.2%	42.2%	57.8%	57.8%	57.8%	57.8%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	10.3	10.3	71.3	71.3	71.3	71.3
Actuated g/C Ratio	0.11	0.11	0.79	0.79	0.79	0.79
v/c Ratio	0.29	0.07	0.12	0.38	0.23	0.05
Control Delay	40.6	18.5	6.1	5.7	2.3	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.6	18.5	6.1	5.7	2.3	0.5
LOS	D	B	A	A	A	A
Approach Delay	36.3		5.7	2.1		
Approach LOS	D		A	A		
Queue Length 50th (m)	8.7	0.0	3.3	29.1	8.7	0.0
Queue Length 95th (m)	19.3	5.2	m8.9	43.3	11.7	0.1
Internal Link Dist (m)	33.9		222.1	154.1		
Turn Bay Length (m)			75.0		60.0	
Base Capacity (vph)	574	522	599	2576	2504	1151
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.02	0.12	0.38	0.23	0.05
<b>Intersection Summary</b>						
Cycle Length: 90						
Actuated Cycle Length: 90						
Offset: 69 (77%), Referenced to phase 2:NBLT and 6:SBT, Start of Green						
Natural Cycle: 65						
Control Type: Actuated-Coordinated						



Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
12-02-2021

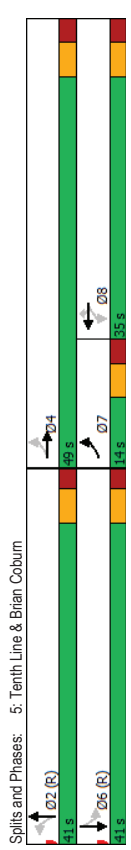
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	184	213	71	53	465	253	236	605	38	132	353	124
Future Volume (vph)	184	213	71	53	465	253	236	605	38	132	353	124
Satd. Flow (prot)	1642	1562	0	1610	1728	1483	1658	3216	0	1566	3075	0
Flt Permitted	0.169			0.586			0.445			0.337		
Satd. Flow (perm)	292	1562	0	993	1728	1455	774	3216	0	554	3075	0
Satd. Flow (RTOR)	25			153			8			64		
Lane Group Flow (vph)	184	284	0	53	465	253	236	643	0	132	477	0
Turn Type	pm-pt	NA	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	7	4		8	8	2	2			6		
Permitted Phases	4			8	8	2	2			6		
Detector Phase	7	4		8	8	2	2			6		
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	11.4	31.4		31.4	31.4	29.0	29.0	29.0		29.0	29.0	
Total Split (s)	14.0	49.0		35.0	35.0	41.0	41.0	41.0		41.0	41.0	
Total Split (%)	15.6%	54.4%		38.9%	38.9%	45.6%	45.6%	45.6%		45.6%	45.6%	
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7		3.7	3.7	
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3		2.3	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0		6.0	6.0	
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead/Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	40.9	40.9		26.9	26.9	26.9	36.7	36.7		36.7	36.7	
Actuated G/C Ratio	0.45	0.45		0.30	0.30	0.30	0.41	0.41		0.41	0.41	
v/c Ratio	0.75	0.39		0.18	0.90	0.47	0.75	0.49		0.58	0.37	
Control Delay	35.7	16.2		24.3	52.7	13.0	47.1	26.9		28.0	10.9	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	35.7	16.2		24.3	52.7	13.0	47.1	26.9		28.0	10.9	
LOS	D	B		C	D	B	D	C		C	B	
Approach Delay		23.9			37.7			32.3			14.6	
Approach LOS		C			D			C			B	
Queue Length 50th (m)	18.1	27.3		6.6	74.5	12.6	40.4	51.2		18.8	27.7	
Queue Length 95th (m)	#39.4	45.8		15.4	#125.5	32.5	#77.5	70.1		#34.0	25.6	
Internal Link Dist (m)		117.2			351.9			157.1			222.1	
Turn Bay Length (m)	45.0			50.0		45.0	105.0			110.0		
Base Capacity (vph)	246	752		315	549	566	316	1317		226	1292	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.75	0.38		0.17	0.85	0.45	0.75	0.49		0.58	0.37	

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

AM Peak Hour  
12-02-2021

Maximum v/c Ratio:	0.90
Intersection Signal Delay:	28.4
Intersection LOS:	C
Intersection Capacity Utilization:	90.2%
ICU Level of Service:	E
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	





Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
12-02-2021

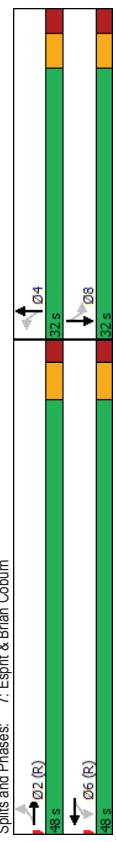
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	30	277	75	34	464	24	151	59	37	25	50	45
Traffic Volume (vph)	30	277	75	34	464	24	151	59	37	25	50	45
Future Volume (vph)	1642	1616	0	1851	1697	0	1688	1480	0	1566	1542	0
Satd. Flow (prot)	0.391		0.508			0.695			0.695			
Flt Permitted	674	1616	0	808	1697	0	1201	1480	0	1112	1542	0
Satd. Flow (perm)	26		5			37			45			
Satd. Flow (RTOR)	30	352	0	34	488	0	151	96	0	25	95	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	2	6	6	6	4	4	4	8	8	8	8
Protected Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C	C	C	C	C	C	C
Act Effct Green (s)	42.0	42.0	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2	26.2	26.2
Actuated G/C Ratio	0.52	0.52	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.08	0.41	0.08	0.55	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18
Control Delay	10.3	12.3	10.1	15.4	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.3	12.3	10.1	15.4	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
LOS	B	B	B	B	B	C	B	B	B	B	B	B
Approach Delay	12.2		15.1		20.2		13.7		13.7		13.7	
Approach LOS	B		B		C		B		B		B	
Queue Length 50th (m)	2.1	27.8	2.4	45.7	17.5	6.2	2.6	6.2	2.6	6.2	5.2	5.2
Queue Length 95th (m)	6.2	46.3	6.7	72.2	33.2	16.5	7.8	16.5	7.8	16.5	15.3	15.3
Internal Link Dist (m)	379.2		585.6		222.2		382.8		382.8		382.8	
Turn Bay Length (m)	65.0		65.0		30.0		30.0		30.0		30.0	
Base Capacity (vph)	353	860	424	893	393	509	364	535	364	535	535	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.41	0.08	0.55	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18

Intersection Summary	
Cycle Length:	80
Actuated Cycle Length:	80
Offset:	0 (0%), Referenced to phase 2EBTL and 6:WBTL, Start of Green
Natural Cycle:	50
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
12-02-2021

Maximum v/c Ratio:	0.55
Intersection LOS:	B
ICU Level of Service B	
Intersection Signal Delay:	15.1
Intersection Capacity Utilization:	55.2%
Analysis Period (min):	15



Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

AM Peak Hour  
12-02-2021

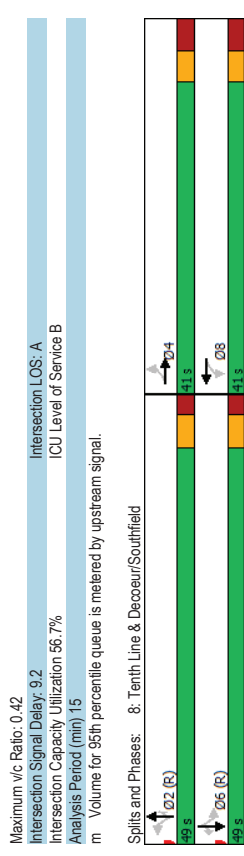
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	89	27	44	9	29	70	81	715	1	19	403	59
Traffic Volume (vph)	89	27	44	9	29	70	81	715	1	19	403	59
Future Volume (vph)	1610	1389	0	1658	1545	0	1445	3131	1483	1523	3161	1401
Satd. Flow (prot)	0.693			0.711			0.515			0.372		
FI Permitted	1173	1389	0	1241	1545	0	779	3131	1442	595	3161	1359
Satd. Flow (perm)	70			70			47			59		
Lane Group Flow (vph)	89	71	0	9	99	0	81	715	1	19	403	59
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	4	4		8	8	8	2	2	2	6	6	6
Detector Phase	4	4		8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	49.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												

Recall Mode	None	None	None	C	B	A	A	A	A	A	A	A
Act Effct Green (s)	16.2	16.2	16.2	16.2	16.2	16.2	65.6	65.6	65.6	65.6	65.6	65.6
Actuated G/C Ratio	0.18	0.18	0.18	0.18	0.18	0.18	0.73	0.73	0.73	0.73	0.73	0.73
v/c Ratio	0.42	0.25	0.04	0.30	0.14	0.31	0.00	0.04	0.17	0.06	0.00	0.00
Control Delay	36.3	14.8	25.0	12.6	8.8	7.7	0.0	6.9	5.0	2.3	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.3	14.8	25.0	12.6	8.8	7.7	0.0	6.9	5.0	2.3	0.0	0.0
LOS	D	B	C	B	A	A	A	A	A	A	A	A
Approach Delay	26.8		13.6		7.8							
Approach LOS	C		B		A							
Queue Length 50th (m)	14.6	4.2	1.4	4.5	3.7	19.5	0.0	0.3	3.4	0.0	0.0	0.0
Queue Length 95th (m)	20.7	11.3	3.9	12.9	17.1	56.7	0.0	0.7	39.8	10.7	0.0	0.0
Internal Link Dist (m)	95.2		315.6		346.2							
Turn Bay Length (m)	44.0		20.0		90.0		60.0	60.0	70.0			
Base Capacity (vph)	454	553	470	628	567	2282	1063	433	2304	1006	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.13	0.02	0.16	0.14	0.31	0.00	0.04	0.17	0.06	0.00	0.00

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	36 (40%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

AM Peak Hour  
12-02-2021



Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
135	3	12	70	1	290	5	353	33	76	329	58
135	3	12	70	1	290	5	353	33	76	329	58
1658	1433	0	1658	1447	0	1409	3074	0	1580	3187	0
0.457			0.748			0.523			0.523		
795	1433	0	1304	1447	0	775	3074	0	870	3187	0
12			290			17			35		
135	15	0	70	291	0	5	366	0	76	387	0
Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
4	4	4	8	8	8	2	2	2	6	6	6
4	4	4	8	8	8	2	2	2	6	6	6

Intersection Signal Delay: 12.0  
Intersection Capacity Utilization 77.1%  
Analysis Period (min) 15

Intersection LOS: B  
ICU Level of Service D

Splits and Phases: 9: Tenth Line & Sweetvalley/Harvest Valley



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	353	33	76	329	58
Traffic Volume (vph)	135	3	12	70	1	290	5	353	33	76	329	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3187	0
Satd. Flow (prot)	0.457			0.748			0.523			0.523		
Flt Permitted	795	1433	0	1304	1447	0	775	3074	0	870	3187	0
Satd. Flow (perm)	12			290			17			35		
Satd. Flow (RTOR)	135	15	0	70	291	0	5	366	0	76	387	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase	4	4	4	8	8	8	2	2	2	6	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	Max	Max	Max	Max	Max	Max	Max
Act Effct Green (s)	16.3	16.3	16.3	16.3	16.3	39.1	39.1	39.1	39.1	39.1	39.1	39.1
Actuated G/C Ratio	0.24	0.24	0.24	0.24	0.24	0.57	0.57	0.57	0.57	0.57	0.57	0.57
v/c Ratio	0.71	0.04	0.23	0.51	0.01	0.01	0.22	0.15	0.21	0.15	0.21	0.21
Control Delay	44.1	11.2	21.6	6.2	6.2	9.0	8.3	9.8	7.8	9.8	7.8	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.1	11.2	21.6	6.2	6.2	9.0	8.3	9.8	7.8	9.8	7.8	7.8
LOS	D	B	C	A	A	A	A	A	A	A	A	A
Approach Delay	40.9	9.2	9.2	8.3	8.3	8.3	8.3	8.1	8.1	8.1	8.1	8.1
Approach LOS	D	D	A	A	A	A	A	A	A	A	A	A
Queue Length 50th (m)	15.6	0.3	7.1	0.1	0.3	10.2	10.2	3.8	3.8	9.5	9.5	9.5
Queue Length 95th (m)	32.7	4.0	15.9	15.2	2.1	24.4	24.4	13.9	13.9	23.4	23.4	23.4
Internal Link Dist (m)	180.2		318.8		263.5	263.5	346.2			346.2		
Turn Bay Length (m)	38.0		60.0		54.0	54.0	65.0			65.0		
Base Capacity (vph)	334	610	549	777	444	1769	498	1842	498	1842	498	1842
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.40	0.02	0.13	0.37	0.01	0.22	0.15	0.21	0.15	0.21	0.15	0.21

Intersection Summary

Cycle Length: 80  
Actuated Cycle Length: 68.2  
Natural Cycle: 65  
Control Type: Actuated-Uncoordinated  
Maximum v/c Ratio: 0.71

Intersection	Int Delay, s/veh							
	EBL	EBR	NBL	NBT	SBT	SBR		
Int Delay, s/veh	0							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations								
Traffic Vol, veh/h	0	5	0	879	473	13		
Future Vol, veh/h	0	5	0	879	473	13		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	0	-	-	-	-		
Veh in Median Storage, #	-	0	-	0	0	-		
Grade, %	-	0	-	0	0	-		
Peak Hour Factor	100	100	100	100	100	100		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	0	5	0	879	473	13		
Major/Minor	Minor2	Major1	Major1	Major2				
Conflicting Flow All	-	243	-	0	-	0		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Critical Hdwy	-	6.94	-	-	-	-		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	3.32	-	-	-	-		
Pot Cap-1 Maneuver	0	758	0	-	-	-		
Stage 1	0	0	0	-	-	-		
Stage 2	0	0	0	-	-	-		
Platoon blocked, %	-	-	-	-	-	-		
Mov Cap-1 Maneuver	-	758	-	-	-	-		
Mov Cap-2 Maneuver	-	-	-	-	-	-		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Approach	EB	NB	SB					
HCM Control Delay, s	9.8	0	0					
HCM LOS	A							
Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR				
Capacity (veh/h)	-	758	-	-				
HCM Lane V/C Ratio	-	0.007	-	-				
HCM Control Delay (s)	-	9.8	-	-				
HCM Lane LOS	-	A	-	-				
HCM 95th %tile Q(veh)	-	0	-	-				

Intersection	Int Delay, s/veh							
	EBL	EBT	WBT	WBR	SBL	SBR		
Int Delay, s/veh	0.3							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations								
Traffic Vol, veh/h	1	156	149	2	4	6		
Future Vol, veh/h	1	156	149	2	4	6		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	-	-	-	0	-		
Veh in Median Storage, #	-	0	0	0	-	0		
Grade, %	-	0	0	0	-	0		
Peak Hour Factor	100	100	100	100	100	100		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	1	156	149	2	4	6		
Major/Minor	Major1	Major2	Minor2					
Conflicting Flow All	151	0	-	0	308	150		
Stage 1	-	-	-	-	150	-		
Stage 2	-	-	-	-	158	-		
Critical Hdwy	4.12	-	-	-	6.42	6.22		
Critical Hdwy Stg 1	-	-	-	-	5.42	-		
Critical Hdwy Stg 2	-	-	-	-	5.42	-		
Follow-up Hdwy	2.218	-	-	-	3.518	3.318		
Pot Cap-1 Maneuver	1430	-	-	-	684	896		
Stage 1	-	-	-	-	878	-		
Stage 2	-	-	-	-	871	-		
Platoon blocked, %	-	-	-	-	-	-		
Mov Cap-1 Maneuver	1430	-	-	-	683	896		
Mov Cap-2 Maneuver	-	-	-	-	683	-		
Stage 1	-	-	-	-	877	-		
Stage 2	-	-	-	-	871	-		
Approach	EB	WB	SB					
HCM Control Delay, s	0	0	9.6					
HCM LOS								A
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1			
Capacity (veh/h)	1430	-	-	-	797			
HCM Lane V/C Ratio	0.001	-	-	-	0.013			
HCM Control Delay (s)	7.5	0	-	-	9.6			
HCM Lane LOS	A	A	-	-	A			
HCM 95th %tile Q(veh)	0	-	-	-	0			

12: Site Access & Brian Coburn

AM Peak Hour  
12-02-2021

12: Tenth Line & Gerry Lalonde/Lakepointe

PM Peak Hour  
12-02-2021

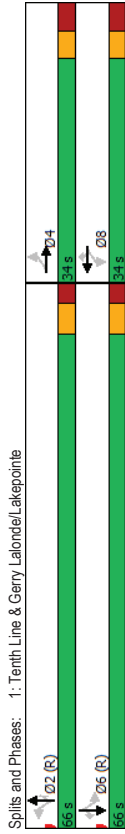
Intersection	EBT	EBR	WBL	WBT	NBL	NBR
Int Delay, s/veh	0.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4/9	2/1	3/822	3/2	19	19
Traffic Vol, veh/h	449	21	3	822	32	19
Future Vol, veh/h	449	21	3	822	32	19
Conflicting Peds, #/hr	0					
Sign Control	Free Free Free Stop Stop					
RT Channelized	- None - None - None					
Storage Length	-					
Veh in Median Storage, #	0					
Grade, %	0					
Peak Hour Factor	100 100 100 100 100 100					
Heavy Vehicles, %	2 2 2 2 2 2					
Mvmt Flow	449	21	3	822	32	19
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	470	0	1288	460
Stage 1	-	-	-	-	460	-
Stage 2	-	-	-	-	828	-
Critical Hwy	-	-	4.12	-	6.42	6.22
Critical Hwy Stg 1	-	-	-	-	5.42	-
Critical Hwy Stg 2	-	-	-	-	5.42	-
Follow-up Hwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1092	-	181	601
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	429	-
Platoon blocked, %	-					
Mov Cap-1 Maneuver	-	-	1092	-	180	601
Mov Cap-2 Maneuver	-	-	-	-	180	-
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	427	-
Approach	EB	WB	NB			
HCM Control Delay, s	0	0	23.6			
HCM LOS	C					
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	244	-	-	1092	-	
HCM Lane V/C Ratio	0.209	-	-	0.003	-	
HCM Control Delay (s)	23.6	-	-	8.3	0	
HCM Lane LOS	C	-	-	A	A	
HCM 95th %ile Q(veh)	0.8	-	-	0	-	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	173	103	62	31	25	171	38	1036	63	263	1266	187
Future Volume (vph)	173	103	62	31	25	171	38	1036	63	263	1266	187
Satd. Flow (prot)	1658	1637	0	1658	1745	1483	1658	3316	1483	1658	3316	1455
Flt Permitted	0.741			0.604			0.179			0.246		
Satd. Flow (perm)	1292	1637	0	1049	1745	1464	312	3316	1436	428	3316	1411
Satd. Flow (RTOR)	30			108			63			63		187
Lane Group Flow (vph)	173	165	0	31	25	171	38	1036	63	263	1266	187
Turn Type	Perm	NA	NA	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	2	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase	4	4	4	8	8	8	2	2	2	2	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Total Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	34.0%	34.0%	34.0%	34.0%	34.0%	34.0%	66.0%	66.0%	66.0%	66.0%	66.0%	66.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.8	18.8	18.8	18.8	18.8	18.8	68.2	68.2	68.2	68.2	68.2	68.2
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.19	0.19	0.68	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.71	0.50	0.16	0.08	0.47	0.18	0.46	0.06	0.90	0.56	0.18	0.7
Control Delay	53.6	33.5	33.0	30.8	17.7	5.6	5.6	0.6	52.1	10.3	1.7	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.6	33.5	33.0	30.8	17.7	5.6	5.6	0.6	52.1	10.3	1.7	1.7
LOS	D	C	C	C	B	A	A	A	A	D	B	A
Approach Delay	43.7	21.2	5.3									
Approach LOS	D	C	C									
Queue Length 50th (m)	31.8	23.4	5.1	4.0	10.5	1.2	56.1	0.2	37.1	58.4	0.0	0.0
Queue Length 95th (m)	49.0	38.8	11.8	9.9	26.5	2.8	19.7	0.7	102.5	98.0	8.0	8.0
Internal Link Dist (m)	372.5		134.8				154.1			468.1		
Turn Bay Length (m)	30.0		50.0		35.0	55.0	70.0		50.0	75.0		
Base Capacity (vph)	351	467	285	474	476	213	2262	999	292	2262	1022	750
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.35	0.11	0.05	0.36	0.18	0.46	0.06	0.90	0.56	0.18	0.18
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 90 (90%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green												
Natural Cycle: 110												
Control Type: Actuated-Coordinated												



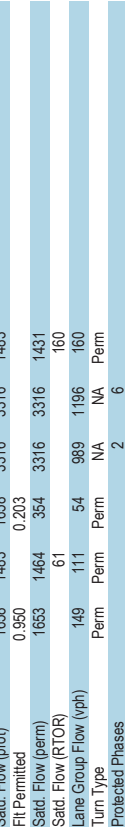
Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Maximum v/c Ratio: 0.90  
 Intersection Signal Delay: 15.4  
 Intersection LOS: B  
 ICU Level of Service E  
 Intersection Capacity Utilization 87.5%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Maximum v/c Ratio: 0.90  
 Intersection Signal Delay: 15.4  
 Intersection LOS: B  
 ICU Level of Service E  
 Intersection Capacity Utilization 87.5%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

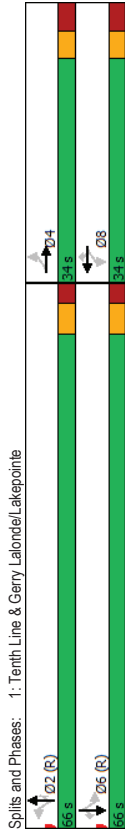


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	111	54	989	1196	160
Future Volume (vph)	149	111	54	989	1196	160
Satd. Flow (prot)	1658	1483	1658	3316	3316	1483
Flt Permitted	0.950		0.203			
Satd. Flow (perm)	1653	1464	354	3316	3316	1431
Satd. Flow (RTOR)	61					160
Lane Group Flow (vph)	149	111	54	989	1196	160
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	62.0	62.0	62.0	62.0
Total Split (%)	38.0%	38.0%	62.0%	62.0%	62.0%	62.0%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	70.2	70.2	70.2	70.2
Actuated g/C Ratio	0.17	0.17	0.70	0.70	0.70	0.70
v/c Ratio	0.54	0.37	0.22	0.43	0.51	0.15
Control Delay	43.4	19.9	10.6	8.2	5.8	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	19.9	10.6	8.2	5.8	0.7
LOS	D	B	B	A	A	A
Approach Delay	33.4		8.3	5.2		
Approach LOS	C		A	A		
Queue Length 50th (m)	27.5	8.7	2.9	33.5	29.4	0.1
Queue Length 95th (m)	37.7	19.5	13.5	76.1	37.9	2.5
Internal Link Dist (m)	33.9			222.1	154.1	
Turn Bay Length (m)			75.0			60.0
Base Capacity (vph)	515	498	248	2326	2326	1051
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.22	0.43	0.54	0.15

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	85 (85%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Maximum v/c Ratio: 0.90  
 Intersection Signal Delay: 15.4  
 Intersection LOS: B  
 ICU Level of Service E  
 Intersection Capacity Utilization 87.5%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



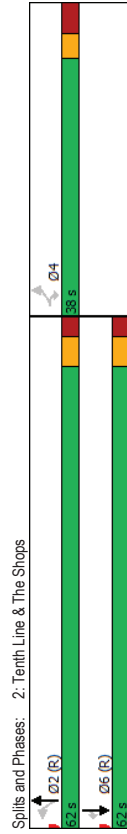
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	111	54	989	1196	160
Future Volume (vph)	149	111	54	989	1196	160
Satd. Flow (prot)	1658	1483	1658	3316	3316	1483
Flt Permitted	0.950		0.203			
Satd. Flow (perm)	1653	1464	354	3316	3316	1431
Satd. Flow (RTOR)	61					160
Lane Group Flow (vph)	149	111	54	989	1196	160
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	37.8	37.8	24.2	24.2	24.2	24.2
Total Split (s)	38.0	38.0	62.0	62.0	62.0	62.0
Total Split (%)	38.0%	38.0%	62.0%	62.0%	62.0%	62.0%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	70.2	70.2	70.2	70.2
Actuated g/C Ratio	0.17	0.17	0.70	0.70	0.70	0.70
v/c Ratio	0.54	0.37	0.22	0.43	0.51	0.15
Control Delay	43.4	19.9	10.6	8.2	5.8	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	19.9	10.6	8.2	5.8	0.7
LOS	D	B	B	A	A	A
Approach Delay	33.4		8.3	5.2		
Approach LOS	C		A	A		
Queue Length 50th (m)	27.5	8.7	2.9	33.5	29.4	0.1
Queue Length 95th (m)	37.7	19.5	13.5	76.1	37.9	2.5
Internal Link Dist (m)	33.9			222.1	154.1	
Turn Bay Length (m)			75.0			60.0
Base Capacity (vph)	515	498	248	2326	2326	1051
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.22	0.43	0.54	0.15

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	85 (85%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

PM Peak Hour  
12-02-2021

Maximum v/c Ratio: 0.54  
Intersection Signal Delay: 9.2  
Intersection Capacity Utilization 67.5%  
Analysis Period (min) 15



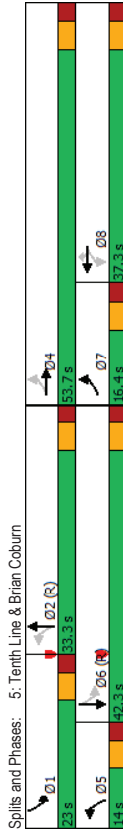
Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

PM Peak Hour  
12-02-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	232	469	240	59	240	236	161	581	47	291	849	191
Future Volume (vph)	232	469	240	59	240	236	161	581	47	291	849	191
Satd. Flow (prot)	1658	1647	0	1658	1745	1483	1656	3267	0	1658	3191	0
Flt/Permitted	0.410			0.164			0.142				0.190	
Satd. Flow (perm)	709	1647	0	286	1745	1430	233	3267	0	330	3191	0
Satd. Flow (RTOR)	29			236			7			26		
Lane Group Flow (vph)	232	709	0	59	240	236	161	628	0	291	1040	0
Turn Type	pm-pt	NA	NA	Perm	NA	Perm	pm-pt	NA	pm-pt	NA	NA	NA
Permitted Phases	7	4		8	8	5	2	1	6			
Detector Phase	7	4		8	8	5	2	1	6			
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	
Minimum Split (s)	11.4	31.4		31.4	31.4	11.0	29.0	11.0	29.0	11.0	29.0	
Total Split (s)	16.4	53.7		37.3	37.3	14.0	33.3	23.0	42.3	20.9	38.5%	
Total Split (%)	14.9%	48.8%		33.9%	33.9%	12.7%	30.3%	20.9%	38.5%			
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0	
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lead	Lag	Lead	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	None	None	None	None	None	C-Max	None	None	None	C-Max	
Act Effct Green (s)	47.2	47.2		30.8	30.8	36.2	28.1	50.2	36.3	50.2	36.3	
Actuated g/C Ratio	0.43	0.43		0.28	0.28	0.28	0.33	0.26	0.46	0.33	0.46	
v/c Ratio	0.59	0.98		0.74	0.49	0.41	0.93	0.75	0.84	0.97	0.84	
Control Delay	28.4	59.9		86.3	37.2	6.4	78.6	44.0	43.1	57.6	43.1	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	28.4	59.9		86.3	37.2	6.4	78.6	44.0	43.1	57.6	43.1	
LOS	C	E		F	D	A	E	D	D	D	E	
Approach Delay	52.1			29.0			51.0			54.4		
Approach LOS	D			C			D			D		
Queue Length 50th (m)	32.3	142.3		11.5	43.0	0.0	21.3	65.4	40.0	112.8	40.0	
Queue Length 95th (m)	50.9	#221.9		#34.9	66.9	17.9	#61.6	86.2	#79.7	#157.2	#79.7	
Internal Link Dist (m)	117.2			351.9			157.1			222.1		
Turn Bay Length (m)	45.0			50.0		45.0	105.0			110.0		
Base Capacity (vph)	390	724		80	490	571	174	840	366	1070	366	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.59	0.98		0.74	0.49	0.41	0.93	0.75	0.82	0.97	0.82	
Intersection Summary												
Cycle Length: 110												
Actuated Cycle Length: 110												
Offset: 0 (0%), Referenced to phase 2:NBL and 6:SBTL, Start of Green												
Natural Cycle: 95												
Control Type: Actuated-Coordinated												

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Maximum v/c Ratio: 0.98  
 Intersection Signal Delay: 49.3  
 Intersection Capacity Utilization 111.3%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

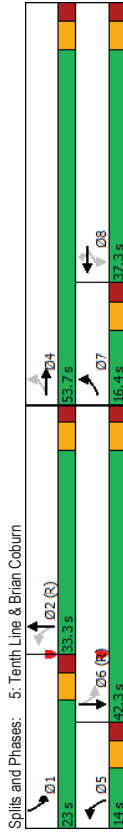
Intersection LOS: D  
 ICU Level of Service H

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	56	685	73	31	434	20	71	19	27	28	13	31
Future Volume (vph)	56	685	73	31	434	20	71	19	27	28	13	31
Satd. Flow (prot)	1688	1716	0	1658	1714	0	1626	1546	0	1523	1532	0
Flt Permitted	0.489			0.310			0.728			0.727		
Satd. Flow (perm)	848	1716	0	540	1714	0	1240	1546	0	1134	1532	0
Satd. Flow (RTOR)	12			5			27			31		
Lane Group Flow (vph)	56	758	0	31	454	0	71	46	0	28	44	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	6	6	6	4	4	4	8	8	8
Detector Phase	2	2	6	6	6	6	4	4	4	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	54.0	54.0	54.0	54.0	54.0	54.0	26.0	26.0	26.0	26.0	26.0	26.0
Total Split (%)	67.5%	67.5%	67.5%	67.5%	67.5%	67.5%	32.5%	32.5%	32.5%	32.5%	32.5%	32.5%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	Max	Max	Max	Max	Max	Max	None	None	None	None	None	None
Act Effct Green (s)	55.0	55.0	55.0	55.0	55.0	55.0	11.2	11.2	11.2	11.2	11.2	11.2
Actuated g/C Ratio	0.74	0.74	0.74	0.74	0.74	0.74	0.15	0.15	0.15	0.15	0.15	0.15
v/c Ratio	0.09	0.59	0.08	0.36	0.38	0.38	0.18	0.18	0.16	0.16	0.17	0.17
Control Delay	5.0	8.9	5.2	5.9	33.9	16.4	28.7	14.8	28.7	14.8	14.8	14.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.0	8.9	5.2	5.9	33.9	16.4	28.7	14.8	28.7	14.8	14.8	14.8
LOS	A	A	A	A	A	A	C	B	C	C	B	B
Approach Delay	8.6	5.8	5.8	27.0	27.0	27.0	20.2	20.2	20.2	20.2	20.2	20.2
Approach LOS	A	A	A	C	C	C	C	C	C	C	C	C
Queue Length 50th (m)	2.1	46.1	1.1	21.1	8.6	2.2	3.3	1.5	3.3	1.5	1.5	1.5
Queue Length 95th (m)	6.8	97.9	4.6	44.2	19.6	10.2	9.8	9.2	9.8	9.2	9.2	9.2
Internal Link Dist (m)	351.9		379.2	249.4	249.4	249.4	312.2	312.2	312.2	312.2	312.2	312.2
Turn Bay Length (m)	65.0		65.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0
Base Capacity (vph)	631	1280	402	1276	330	431	302	431	302	431	431	431
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.59	0.08	0.36	0.22	0.11	0.09	0.10	0.09	0.10	0.10	0.10

Intersection Summary	Cycle Length: 80	Actuated Cycle Length: 73.9	Natural Cycle: 60	Control Type: Semi Act-Uncoord	Maximum v/c Ratio: 0.59
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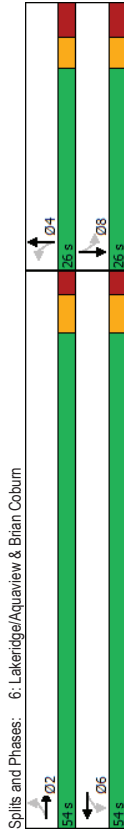
Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Maximum v/c Ratio: 0.98  
 Intersection Signal Delay: 49.3  
 Intersection Capacity Utilization 111.3%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Intersection Signal Delay: 9.7  
Intersection Capacity Utilization 70.8%  
Analysis Period (min) 15



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Intersection LOS: A  
ICU Level of Service C

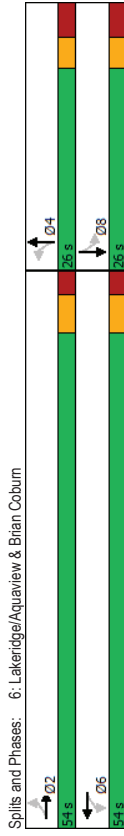
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	52	491	188	27	344	18	103	43	25	23	48	38
Future Volume (vph)	52	491	188	27	344	18	103	43	25	23	48	38
Satd. Flow (prot)	1658	1672	0	1658	1714	0	1658	1549	0	1658	1490	0
Flt Permitted	0.499			0.242			0.701			0.713		
Satd. Flow (perm)	865	1672	0	422	1714	0	1206	1549	0	1242	1490	0
Satd. Flow (RTOR)	36			5			25			38		
Lane Group Flow (vph)	52	679	0	27	362	0	103	68	0	23	86	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												

Recall Mode	C-Max	C-Max	C-Max	Max	Max
Act Effct Green (s)	42.0	42.0	42.0	26.2	26.2
Actuated g/C Ratio	0.52	0.52	0.52	0.33	0.33
v/c Ratio	0.11	0.76	0.12	0.26	0.13
Control Delay	10.5	21.0	11.4	22.1	14.1
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	10.5	21.0	11.4	22.1	14.1
LOS	B	C	B	C	B
Approach Delay	20.3	12.8	18.9	14.2	14.2
Approach LOS	C	B	B	B	B
Queue Length 50th (m)	3.7	72.4	1.9	30.5	11.4
Queue Length 95th (m)	9.3	116.6	6.3	49.1	23.4
Internal Link Dist (m)	379.2	585.6	222.2	382.8	382.8
Turn Bay Length (m)	65.0	65.0	30.0	30.0	30.0
Base Capacity (vph)	454	894	221	902	394
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.11	0.76	0.12	0.40	0.26

Intersection Summary  
Cycle Length: 80  
Actuated Cycle Length: 80  
Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green  
Natural Cycle: 60  
Control Type: Actuated-Coordinated

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

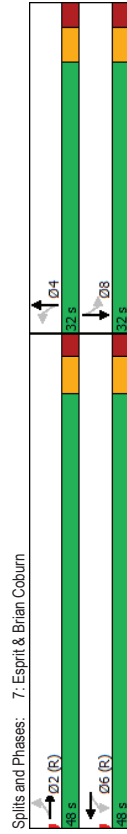
Intersection LOS: A  
ICU Level of Service C



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

PM Peak Hour  
12-02-2021

Maximum v/c Ratio: 0.76  
Intersection Signal Delay: 17.6  
Intersection Capacity Utilization 70.4%  
Analysis Period (min) 15



Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

PM Peak Hour  
12-02-2021

Intersection LOS: B  
ICU Level of Service C



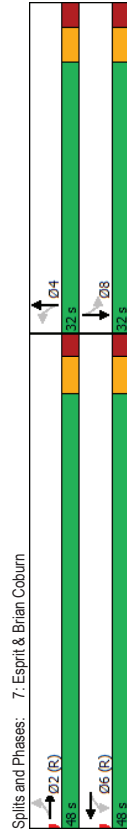
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	50	16	30	2	24	55	38	674	14	116	920	94
Future Volume (vph)	50	16	30	2	24	55	38	674	14	116	920	94
Satd. Flow (prot)	1688	1561	0	1658	1564	0	1658	3316	1483	1658	3316	1483
Flt/Permitted	0.706			0.727			0.295			0.393		
Satd. Flow (perm)	1232	1561	0	1268	1564	0	1268	3316	1483	1658	3316	1435
Satd. Flow (RTOR)	30			55			43			43		94
Lane Group Flow (vph)	50	46	0	2	79	0	38	674	14	116	920	94
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	59.0	59.0	59.0	59.0	59.0	59.0
Total Split (%)	41.0%	41.0%	41.0%	41.0%	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%	59.0%	59.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.0	15.0	15.0	15.0	15.0	15.0	76.7	76.7	76.7	76.7	76.7	76.7
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.15	0.77	0.77	0.77	0.77	0.77	0.77
v/c Ratio	0.27	0.18	0.01	0.28	0.01	0.28	0.10	0.26	0.01	0.22	0.36	0.08
Control Delay	38.0	17.1	29.0	15.7	7.8	6.1	0.0	8.2	6.9	2.2	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	17.1	29.0	15.7	7.8	6.1	0.0	8.2	6.9	2.2	0.0	0.0
LOS	D	B	C	B	B	B	A	A	A	A	A	A
Approach Delay	28.0			16.1			6.1					6.6
Approach LOS	C			B			A					A
Queue Length 50th (m)	9.2	2.9	0.4	4.3	1.5	15.8	0.0	5.0	23.7	0.0	0.0	0.0
Queue Length 95th (m)	15.0	9.6	1.8	12.8	9.3	50.8	0.3	24.3	74.3	7.0	0.0	0.0
Internal Link Dist (m)	45.0			315.6			346.2					120.2
Turn Bay Length (m)	420	552	432	569	393	2544	1147	526	2544	1123	0	0
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.08	0.00	0.14	0.10	0.26	0.01	0.22	0.36	0.08	0.00	0.00

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	21 (21%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

PM Peak Hour  
12-02-2021

Maximum v/c Ratio: 0.76  
Intersection Signal Delay: 17.6  
Intersection Capacity Utilization 70.4%  
Analysis Period (min) 15

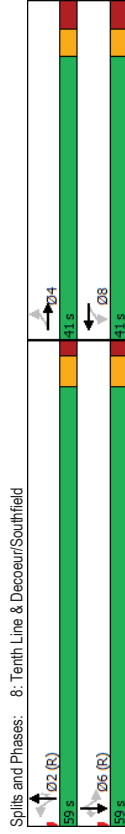


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	50	16	30	2	24	55	38	674	14	116	920	94
Future Volume (vph)	50	16	30	2	24	55	38	674	14	116	920	94
Satd. Flow (prot)	1688	1561	0	1658	1564	0	1658	3316	1483	1658	3316	1483
Flt/Permitted	0.706			0.727			0.295			0.393		
Satd. Flow (perm)	1232	1561	0	1268	1564	0	1268	3316	1483	1658	3316	1435
Satd. Flow (RTOR)	30			55			43			43		94
Lane Group Flow (vph)	50	46	0	2	79	0	38	674	14	116	920	94
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	59.0	59.0	59.0	59.0	59.0	59.0
Total Split (%)	41.0%	41.0%	41.0%	41.0%	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%	59.0%	59.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.0	15.0	15.0	15.0	15.0	15.0	76.7	76.7	76.7	76.7	76.7	76.7
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.15	0.77	0.77	0.77	0.77	0.77	0.77
v/c Ratio	0.27	0.18	0.01	0.28	0.01	0.28	0.10	0.26	0.01	0.22	0.36	0.08
Control Delay	38.0	17.1	29.0	15.7	7.8	6.1	0.0	8.2	6.9	2.2	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	17.1	29.0	15.7	7.8	6.1	0.0	8.2	6.9	2.2	0.0	0.0
LOS	D	B	C	B	B	B	A	A	A	A	A	A
Approach Delay	28.0			16.1			6.1					6.6
Approach LOS	C			B			A					A
Queue Length 50th (m)	9.2	2.9	0.4	4.3	1.5	15.8	0.0	5.0	23.7	0.0	0.0	0.0
Queue Length 95th (m)	15.0	9.6	1.8	12.8	9.3	50.8	0.3	24.3	74.3	7.0	0.0	0.0
Internal Link Dist (m)	45.0			315.6			346.2					120.2
Turn Bay Length (m)	420	552	432	569	393	2544	1147	526	2544	1123	0	0
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.08	0.00	0.14	0.10	0.26	0.01	0.22	0.36	0.08	0.00	0.00

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	21 (21%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Maximum v/c Ratio: 0.36  
 Intersection Signal Delay: 7.8  
 Intersection Capacity Utilization 61.0%  
 Analysis Period (min) 15



Lanes, Volumes, Timings  
9: Tenth Line & SweetValley/HarvestValley

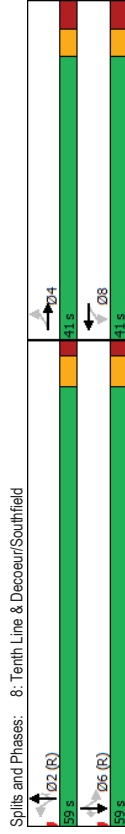
Intersection LOS: A  
 ICU Level of Service B



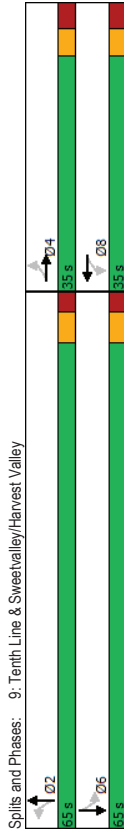
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	95	2	7	17	1	167	14	446	81	293	481	168
Traffic Volume (vph)	95	2	7	17	1	167	14	446	81	293	481	168
Future Volume (vph)	1688	1625	0	1595	1464	0	1658	3239	0	1658	3163	0
Satd. Flow (prot)	0.600			0.752			0.403				0.456	
Flt Permitted	1045	1525	0	1261	1464	0	703	3239	0	796	3163	0
Satd. Flow (perm)	95	9	0	17	167	0	14	527	0	293	649	0
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	34.5	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2
Minimum Split (s)	35.0	35.0	35.0	35.0	35.0	35.0	65.0	65.0	65.0	65.0	65.0	65.0
Total Split (s)	35.0%	35.0%	35.0%	35.0%	35.0%	35.0%	65.0%	65.0%	65.0%	65.0%	65.0%	65.0%
Total Split (%)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
Yellow Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2
Total Lost Time (s)	Lead/Lag											
Lead/Lag Optimize?	None											
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Act Effct Green (s)	14.9	14.9	14.9	14.9	14.9	14.9	61.8	61.8	61.8	61.8	61.8	61.8
Actuated g/C Ratio	0.17	0.17	0.17	0.17	0.17	0.17	0.69	0.69	0.69	0.69	0.69	0.69
v/c Ratio	0.55	0.03	0.08	0.44	0.03	0.23	0.53	0.29	0.53	0.29	0.53	0.29
Control Delay	44.6	18.1	29.4	8.7	5.8	13.1	5.8	13.1	5.8	13.1	5.8	13.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.6	18.1	29.4	8.7	5.8	13.1	5.8	13.1	5.8	13.1	5.8	13.1
LOS	D	B	C	A	A	A	A	A	A	B	A	A
Approach Delay	42.3		10.6		5.9							8.0
Approach LOS	D		B		A							A
Queue Length 50th (m)	14.5	0.3	2.4	0.1	0.6	12.3	18.9	14.6	18.9	14.6	18.9	14.6
Queue Length 95th (m)	28.5	4.0	7.5	14.7	3.6	31.1	63.9	37.0	63.9	37.0	63.9	37.0
Internal Link Dist (m)	180.2		318.8		263.5							346.2
Turn Bay Length (m)	38.0		60.0		54.0							65.0
Base Capacity (vph)	335	493	404	582	485	2249	550	2211	550	2211	550	2211
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.02	0.04	0.29	0.03	0.23	0.63	0.29	0.63	0.29	0.63	0.29
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 89.4												
Natural Cycle: 75												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.55												

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Maximum v/c Ratio: 0.36  
 Intersection Signal Delay: 7.8  
 Intersection Capacity Utilization 61.0%  
 Analysis Period (min) 15



Intersection Signal Delay: 9.6  
 Intersection Capacity Utilization 74.3%  
 Analysis Period (min) 15  
 Intersection LOS: A  
 ICU Level of Service D



Splits and Phases: 9: Tenth Line & Sweetvalley/Harvest Valley

Intersection	10: Tenth Line & Site Access																				
Int Delay, s/veh	0.1																				
Movement	EBL	EBR	NBL	NBT	SBT	SBR															
Lane Configurations	<table border="0"> <tr> <td></td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> <td>↑</td> </tr> </table>											↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑											
Traffic Vol, veh/h	0	14	0	788	1114	39															
Future Vol, veh/h	0	14	0	788	1114	39															
Conflicting Peds, #/hr	0	0	0	0	0	0															
Sign Control	Stop	Stop	Free	Free	Free	Free															
RT Channelized	-	None	-	None	-	None															
Storage Length	-	0	-	-	-	-															
Veh in Median Storage, #	0	-	-	0	0	-															
Grade, %	0	-	-	0	0	-															
Peak Hour Factor	100	100	100	100	100	100															
Heavy Vehicles, %	2	2	2	2	2	2															
Mvmt Flow	0	14	0	788	1114	39															

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	577	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.94	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.32	-
Pot Cap-1 Maneuver	0	460	0
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	460	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	13.1	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	460	-	-
HCM Lane V/C Ratio	-	0.03	-	-
HCM Control Delay (s)	-	13.1	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

HCM 2010 TWSC  
 11: Decoeur & Site Access  
 PM Peak Hour  
 12-02-2021

Intersection	Int Delay, s/veh										
Movement	EBL	EBT	WBT	WBR	SBL	SBR					
Lane Configurations	4 1 4 1 4 1										
Traffic Vol, veh/h	1	92	103	5	4	8					W
Future Vol, veh/h	1	92	103	5	4	8					
Conflicting Peds, #/hr	0	0	0	0	0	0					
Sign Control	Free	Free	Free	Free	Stop	Stop					
RT Channelized	-	None	-	None	-	None					
Storage Length	-	-	-	-	-	-					
Veh in Median Storage, #	-	0	0	-	0	-					
Grade, %	-	0	0	-	0	-					
Peak Hour Factor	100	100	100	100	100	100					
Heavy Vehicles, %	2	2	2	2	2	2					
Mvmt Flow	1	92	103	5	4	8					
Major/Minor	Major1	Major2	Minor2								
Conflicting Flow All	108	0	-	0	200	106					
Stage 1	-	-	-	-	106	-					
Stage 2	-	-	-	-	94	-					
Critical Hdwy	4.12	-	-	-	6.42	6.22					
Critical Hdwy Stg 1	-	-	-	-	5.42	-					
Critical Hdwy Stg 2	-	-	-	-	5.42	-					
Follow-up Hdwy	2.218	-	-	-	3.518	3.318					
Pot Cap-1 Maneuver	1483	-	-	-	789	948					
Stage 1	-	-	-	-	918	-					
Stage 2	-	-	-	-	930	-					
Platoon blocked, %	-	-	-	-	-	-					
Mov Cap-1 Maneuver	1483	-	-	-	788	948					
Mov Cap-2 Maneuver	-	-	-	-	788	-					
Stage 1	-	-	-	-	917	-					
Stage 2	-	-	-	-	930	-					
Approach	EB	WB	SB								
HCM Control Delay, s	0.1	0	9.1								
HCM LOS	A										
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1						
Capacity (veh/h)	1483	-	-	-	888						
HCM Lane V/C Ratio	0.001	-	-	-	0.014						
HCM Control Delay (s)	7.4	0	-	-	9.1						
HCM Lane LOS	A	A	-	-	A						
HCM 95th %tile Q(veh)	0	-	-	-	0						

HCM 2010 TWSC  
 12: Site Access & Brian Coburn  
 PM Peak Hour  
 12-02-2021

Intersection	Int Delay, s/veh										
Movement	EBT	EBR	WBT	WBR	NBL	NBR					
Lane Configurations	1 4 1 4 1 4										
Traffic Vol, veh/h	910	59	6	586	39	30					W
Future Vol, veh/h	910	59	6	586	39	30					
Conflicting Peds, #/hr	0	0	0	0	0	0					
Sign Control	Free	Free	Free	Free	Stop	Stop					
RT Channelized	-	None	-	None	-	None					
Storage Length	-	-	-	-	-	-					
Veh in Median Storage, #	0	-	-	0	0	-					
Grade, %	0	-	-	0	0	-					
Peak Hour Factor	100	100	100	100	100	100					
Heavy Vehicles, %	2	2	2	2	2	2					
Mvmt Flow	910	59	6	586	39	30					
Major/Minor	Major1	Major2	Minor1								
Conflicting Flow All	0	0	969	0	1538	940					
Stage 1	-	-	-	-	940	-					
Stage 2	-	-	-	-	598	-					
Critical Hdwy	-	-	4.12	-	6.42	6.22					
Critical Hdwy Stg 1	-	-	5.42	-	5.42	-					
Critical Hdwy Stg 2	-	-	5.42	-	5.42	-					
Follow-up Hdwy	-	-	2.218	-	3.518	3.318					
Pot Cap-1 Maneuver	-	-	711	-	127	320					
Stage 1	-	-	380	-	549	-					
Stage 2	-	-	711	-	125	320					
Platoon blocked, %	-	-	-	-	-	-					
Mov Cap-1 Maneuver	-	-	711	-	125	320					
Mov Cap-2 Maneuver	-	-	-	-	380	-					
Stage 1	-	-	-	-	542	-					
Stage 2	-	-	-	-	-	-					
Approach	EB	WB	NB								
HCM Control Delay, s	0	0.1	39.9								
HCM LOS	E										
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT						
Capacity (veh/h)	170	-	-	711	-						
HCM Lane V/C Ratio	0.406	-	-	0.008	-						
HCM Control Delay (s)	39.9	-	-	10.1	0						
HCM Lane LOS	E	-	-	B	A						
HCM 95th %tile Q(veh)	1.8	-	-	0	-						



## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Gerry Lalonde AM FT2026]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jobbin													
1	L2	83	2.0	0.189	9.7 LOS A	1.1	7.6	0.57	0.66	0.57	49.9		
2	T1	21	2.0	0.189	4.6 LOS A	1.1	7.6	0.57	0.66	0.57	46.8		
3	R2	78	2.0	0.189	4.9 LOS A	1.1	7.6	0.57	0.66	0.57	48.7		
Approach 182 2.0 0.189 7.1 LOS A 1.1 7.6 0.57 0.66 0.57 49.0													
East: Brian Coburn													
4	L2	44	2.0	0.807	12.1 LOS B	12.0	85.6	0.81	0.65	0.86	50.4		
5	T1	974	2.0	0.807	6.7 LOS A	12.0	85.6	0.81	0.65	0.86	53.6		
6	R2	13	2.0	0.807	6.8 LOS A	12.0	85.6	0.81	0.65	0.86	48.8		
Approach 1031 2.0 0.807 7.0 LOS A 12.0 85.6 0.81 0.65 0.86 53.4													
North: Gerry Lalonde													
7	L2	7	2.0	0.541	26.6 LOS C	5.1	36.1	1.00	1.14	1.32	41.5		
8	T1	8	2.0	0.541	21.5 LOS C	5.1	36.1	1.00	1.14	1.32	39.3		
9	R2	185	2.0	0.541	21.8 LOS C	5.1	36.1	1.00	1.14	1.32	40.7		
Approach 200 2.0 0.541 22.0 LOS C 5.1 36.1 1.00 1.14 1.32 40.6													
West: Brian Coburn													
10u	U	32	2.0	0.327	11.4 LOS B	2.4	17.0	0.26	0.44	0.26	56.9		
10	L2	40	2.0	0.327	9.2 LOS A	2.4	17.0	0.26	0.44	0.26	52.2		
11	T1	349	2.0	0.327	3.8 LOS A	2.4	17.0	0.26	0.44	0.26	55.7		
12	R2	48	2.0	0.327	3.9 LOS A	2.4	17.0	0.26	0.44	0.26	50.5		
Approach 469 2.0 0.327 4.8 LOS A 2.4 17.0 0.26 0.44 0.26 54.9													
All Vehicles 1882 2.0 0.807 8.0 LOS A 12.0 85.6 0.67 0.65 0.73 51.6													

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Gerry Lalonde PM FT2026]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jobbin													
1	L2	37	2.0	0.364	27.0 LOS C	3.0	21.0	1.00	1.01	1.03	40.6		
2	T1	10	2.0	0.364	21.8 LOS C	3.0	21.0	1.00	1.01	1.03	38.5		
3	R2	36	2.0	0.364	22.2 LOS C	3.0	21.0	1.00	1.01	1.03	39.8		
Approach 83 2.0 0.364 24.3 LOS C 3.0 21.0 1.00 1.01 1.03 40.0													
East: Brian Coburn													
4	L2	62	2.0	0.587	11.4 LOS B	5.4	38.7	0.74	0.66	0.76	50.6		
5	T1	552	2.0	0.587	6.0 LOS A	5.4	38.7	0.74	0.66	0.76	53.8		
6	R2	12	2.0	0.587	6.1 LOS A	5.4	38.7	0.74	0.66	0.76	48.9		
Approach 626 2.0 0.587 6.5 LOS A 5.4 38.7 0.74 0.66 0.76 53.3													
North: Gerry Lalonde													
7	L2	4	2.0	0.160	11.5 LOS B	1.0	7.3	0.76	0.73	0.76	50.1		
8	T1	18	2.0	0.160	6.3 LOS A	1.0	7.3	0.76	0.73	0.76	46.9		
9	R2	92	2.0	0.160	6.7 LOS A	1.0	7.3	0.76	0.73	0.76	48.9		
Approach 114 2.0 0.160 6.8 LOS A 1.0 7.3 0.76 0.73 0.76 48.6													
West: Brian Coburn													
10u	U	27	2.0	0.921	13.0 LOS B	25.3	180.2	1.00	0.51	1.00	53.7		
10	L2	212	2.0	0.921	10.8 LOS B	25.3	180.2	1.00	0.51	1.00	49.5		
11	T1	1035	2.0	0.921	5.5 LOS A	25.3	180.2	1.00	0.51	1.00	52.6		
12	R2	65	2.0	0.921	5.6 LOS A	25.3	180.2	1.00	0.51	1.00	48.0		
Approach 1339 2.0 0.921 6.5 LOS A 25.3 180.2 1.00 0.51 1.00 51.9													
All Vehicles 2162 2.0 0.921 7.2 LOS A 25.3 180.2 0.91 0.59 0.92 51.5													

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra  
2021-052 Sidra 2021-10-05.s88

## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg AM FT2026]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Subepines													
1	L2	115	2.0	0.190	9.6 LOS A	1.1	7.6	0.56	0.67	0.56	49.4		
2	T1	15	2.0	0.190	4.4 LOS A	1.1	7.6	0.56	0.67	0.56	46.4		
3	R2	55	2.0	0.190	4.8 LOS A	1.1	7.6	0.56	0.67	0.56	48.3		
Approach		185	2.0	0.190	7.8 LOS A	1.1	7.6	0.56	0.67	0.56	48.8		
East: Brian Coburn													
4	L2	32	2.0	0.659	10.1 LOS B	7.0	50.0	0.59	0.52	0.59	51.3		
5	T1	824	2.0	0.659	4.8 LOS A	7.0	50.0	0.59	0.52	0.59	54.6		
6	R2	12	2.0	0.659	4.9 LOS A	7.0	50.0	0.59	0.52	0.59	49.6		
Approach		868	2.0	0.659	5.0 LOS A	7.0	50.0	0.59	0.52	0.59	54.4		
North: Strasbourg													
7	L2	25	2.0	0.234	15.2 LOS B	1.6	11.6	0.90	0.88	0.90	47.3		
8	T1	22	2.0	0.234	10.0 LOS B	1.6	11.6	0.90	0.88	0.90	44.4		
9	R2	76	2.0	0.234	10.4 LOS B	1.6	11.6	0.90	0.88	0.90	46.2		
Approach		123	2.0	0.234	11.3 LOS B	1.6	11.6	0.90	0.88	0.90	46.1		
West: Brian Coburn													
10	L2	7	2.0	0.308	9.3 LOS A	2.1	15.1	0.29	0.41	0.29	52.6		
11	T1	379	2.0	0.308	4.0 LOS A	2.1	15.1	0.29	0.41	0.29	56.1		
12	R2	38	2.0	0.308	4.1 LOS A	2.1	15.1	0.29	0.41	0.29	50.8		
Approach		424	2.0	0.308	4.1 LOS A	2.1	15.1	0.29	0.41	0.29	55.5		
All Vehicles		1600	2.0	0.659	5.6 LOS A	7.0	50.0	0.53	0.53	0.53	53.3		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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 Organisation: CGH TRANSPORTATION | Processed: December 3, 2021 1:16:53 PM  
 Project: C:\Users\AndrewHarte\CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra 2021-052 Sidra 2021-10-05.sp8

## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg PM FT2026]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Subepines													
1	L2	65	2.0	0.224	15.2 LOS B	1.6	11.2	0.90	0.89	0.90	46.3		
2	T1	18	2.0	0.224	10.1 LOS B	1.6	11.2	0.90	0.89	0.90	43.6		
3	R2	34	2.0	0.224	10.5 LOS B	1.6	11.2	0.90	0.89	0.90	45.3		
Approach		117	2.0	0.224	13.1 LOS B	1.6	11.2	0.90	0.89	0.90	45.5		
East: Brian Coburn													
4	L2	54	2.0	0.474	9.7 LOS A	4.0	28.3	0.44	0.47	0.44	51.8		
5	T1	536	2.0	0.474	4.3 LOS A	4.0	28.3	0.44	0.47	0.44	55.2		
6	R2	36	2.0	0.474	4.4 LOS A	4.0	28.3	0.44	0.47	0.44	50.1		
Approach		626	2.0	0.474	4.8 LOS A	4.0	28.3	0.44	0.47	0.44	54.6		
North: Strasbourg													
7	L2	23	2.0	0.076	11.0 LOS B	0.4	3.1	0.68	0.68	0.68	49.3		
8	T1	13	2.0	0.076	5.8 LOS A	0.4	3.1	0.68	0.68	0.68	46.2		
9	R2	23	2.0	0.076	6.2 LOS A	0.4	3.1	0.68	0.68	0.68	48.1		
Approach		59	2.0	0.076	8.0 LOS A	0.4	3.1	0.68	0.68	0.68	48.1		
West: Brian Coburn													
10	L2	33	2.0	0.738	9.9 LOS A	9.3	66.2	0.54	0.46	0.54	51.5		
11	T1	919	2.0	0.738	4.6 LOS A	9.3	66.2	0.54	0.46	0.54	54.9		
12	R2	106	2.0	0.738	4.7 LOS A	9.3	66.2	0.54	0.46	0.54	49.8		
Approach		1058	2.0	0.738	4.7 LOS A	9.3	66.2	0.54	0.46	0.54	54.2		
All Vehicles		1860	2.0	0.738	5.4 LOS A	9.3	66.2	0.53	0.50	0.53	53.5		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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 Project: C:\Users\AndrewHarte\CGH TRANSPORTATION\CGH Working - Documents\Projects\2021-052 Mattamy 2370 Tenth Line\DATA\Sidra 2021-052 Sidra 2021-10-05.sp8

# Appendix M

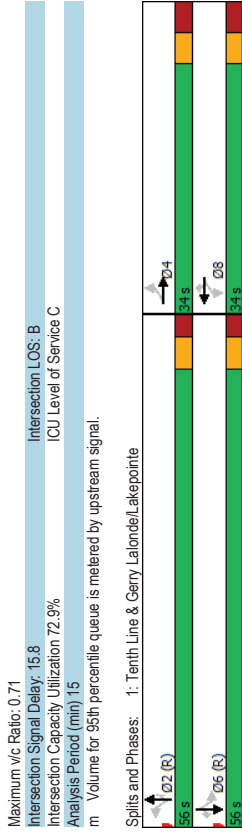
Synchro and Sidra Worksheets – 2031 Future Total Conditions

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	170	19	40	44	60	231	22	1030	15	79	563	73
Traffic Volume (vph)	170	19	40	44	60	231	22	1030	15	79	563	73
Future Volume (vph)	1566	1483	0	1642	1745	1483	1496	3283	1483	1626	3191	1441
Satd. Flow (prot)	0.718			0.441				0.243				
FI Permitted	1179	1483	0	1240	1745	1460	692	3283	1442	415	3191	1400
Satd. Flow (perm)	40			91				46				
Satd. Flow (RTOR)	170	59	0	44	60	231	22	1030	15	79	563	73
Lane Group Flow (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	4	4	8	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	8	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	8	8	8	8	2	2	2	6	6	6
Switch Phase	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Initial (s)	33.8	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	34.0	56.0	56.0	56.0	56.0	56.0	56.0
Total Split (%)	37.8%	37.8%	37.8%	37.8%	37.8%	37.8%	62.2%	62.2%	62.2%	62.2%	62.2%	62.2%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	18.4	18.4	18.4	18.4	18.4	18.4	58.6	58.6	58.6	58.6	58.6	58.6
Actuated v/c Ratio	0.20	0.20	0.20	0.20	0.20	0.20	0.65	0.65	0.65	0.65	0.65	0.65
v/c Ratio	0.71	0.18	0.17	0.17	0.17	0.62	0.05	0.48	0.02	0.29	0.27	0.08
Control Delay	48.1	13.4	28.3	27.8	25.9	8.7	12.8	1.1	12.6	8.0	2.5	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.1	13.4	28.3	27.8	25.9	8.7	12.8	1.1	12.6	8.0	2.5	0.0
LOS	D	B	C	C	C	C	A	B	A	B	A	A
Approach Delay	39.2			26.5			12.6		7.9			
Approach LOS	D			C			B		A			
Queue Length 50th (m)	27.5	2.7	6.3	8.6	21.7	1.5	38.3	0.0	5.2	19.2	0.0	0.0
Queue Length 95th (m)	43.2	10.9	13.4	16.5	39.5	m3.1	96.1	m0.3	17.6	35.7	5.5	0.0
Internal Link Dist (m)	372.5			134.8			154.1		468.1			
Turn Bay Length (m)	30.0			50.0			35.0		70.0			
Base Capacity (vph)	366	476	374	527	504	450	2137	954	270	2077	937	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.12	0.12	0.11	0.46	0.05	0.48	0.02	0.29	0.27	0.08	0.08

Intersection Summary
Cycle Length: 90
Actuated Cycle Length: 90
Offset: 61 (68%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle: 65
Control Type: Actuated-Coordinated



Maximum v/c Ratio: 0.71
Intersection Signal Delay: 15.8
Intersection LOS: B
ICU Level of Service C
Intersection Capacity Utilization 72.9%
Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.
Splits and Phases: 1: Tenth Line & Gerry Lalonde/Lakepointe
← D2 (R) 58 s
← D6 (R) 56 s
← D4 34 s
← D6 (R) 56 s



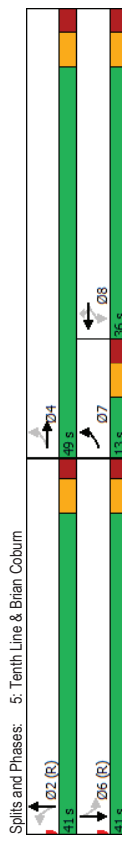
Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	184	213	71	53	489	253	236	615	38	132	359	124
Traffic Volume (vph)	184	213	71	53	489	253	236	615	38	132	359	124
Future Volume (vph)	1642	1562	0	1610	1728	1483	1658	3216	0	1566	3075	0
Satd. Flow (prot)	0.159			0.586			0.440					
FI Permitted	274	1562	0	993	1728	1455	766	3216	0	544	3075	0
Satd. Flow (perm)	25			143			8			62		
Satd. Flow (RTOR)	184	284	0	53	489	253	236	663	0	132	483	0
Lane Group Flow (vph)	pm-pt	NA	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	
Turn Type	7	4		8	8	2	2		6			
Protected Phases	4			8	8	2	2		6			
Permitted Phases	7	4		8	8	2	2		6			
Detector Phase	7	4		8	8	2	2		6			
Switch Phase	7	4		8	8	2	2		6			
Minimum Initial (s)	5.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.4	31.4		31.4	31.4	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	13.0	49.0		36.0	36.0	41.0	41.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	14.4%	54.4%		40.0%	40.0%	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%
Yellow Time (s)	3.7	3.7		3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.7	2.7		2.7	2.7	2.3	2.3	2.3	2.3	2.3	2.3	2.3
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.4	6.4		6.4	6.4	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead/Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	41.0	41.0		28.0	28.0	36.6	36.6	36.6	36.6	36.6	36.6	36.6
Actuated G/C Ratio	0.46	0.46		0.31	0.31	0.41	0.41	0.41	0.41	0.41	0.41	0.41
v/c Ratio	0.82	0.39		0.17	0.91	0.46	0.76	0.50	0.60	0.38	0.38	0.38
Control Delay	45.5	16.2		23.4	52.6	13.3	48.4	27.3	29.0	11.0	11.0	11.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	45.5	16.2		23.4	52.6	13.3	48.4	27.3	29.0	11.0	11.0	11.0
LOS	D	B		C	D	B	D	C	C	B	B	B
Approach Delay	27.7			38.2			32.9		14.9			
Approach LOS	C			D			C		B			
Queue Length 50th (m)	18.1	27.3		6.4	78.4	13.7	40.8	52.4	18.9	28.3	28.3	28.3
Queue Length 95th (m)	#43.9	45.8		15.1	#131.8	33.7	#78.1	71.2	#34.6	26.3	26.3	26.3
Internal Link Dist (m)	117.2			351.9			157.1		222.1			
Turn Bay Length (m)	45.0			50.0			45.0		105.0			
Base Capacity (vph)	225	752		326	568	574	311	1312	221	1287	1287	1287
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.38		0.16	0.86	0.44	0.76	0.50	0.60	0.38	0.38	0.38

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	0 (0%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Maximum v/c Ratio:	0.91
Intersection Signal Delay:	29.5
Intersection LOS:	C
Intersection Capacity Utilization:	91.6%
ICU Level of Service:	F
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

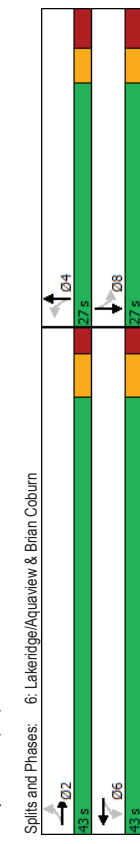
AM Peak Hour  
12-02-2021

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	→	→	→	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	20	348	31	50	624	24	123	23	28	12	10	55
Future Volume (vph)	20	348	31	50	624	24	123	23	28	12	10	55
Satd. Flow (prot)	1658	1646	0	1595	1717	0	1658	1554	0	1551	1511	0
Flt Permitted	0.351			0.537			0.715			0.724		
Satd. Flow (perm)	612	1646	0	902	1717	0	1248	1554	0	1144	1511	0
Satd. Flow (RTOR)	10			4			28			55		
Lane Group Flow (vph)	20	379	0	50	648	0	123	51	0	12	65	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	6	6	6	4	4	8	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	8	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	43.0	43.0	43.0	43.0	43.0	27.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (%)	61.4%	61.4%	61.4%	61.4%	61.4%	38.6%	38.6%	38.6%	38.6%	38.6%	38.6%	38.6%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	Max	Max	Max	Max	Max	None	None	None	None	None	None	None
Act Effct Green (s)	42.4	42.4	42.4	42.4	42.4	12.2	12.2	12.2	12.2	12.2	12.2	12.2
Actuated G/C Ratio	0.68	0.68	0.68	0.68	0.68	0.20	0.20	0.20	0.20	0.20	0.20	0.20
v/c Ratio	0.05	0.34	0.08	0.55	0.08	0.50	0.16	0.05	0.19	0.05	0.19	0.05
Control Delay	6.5	7.3	6.4	10.2	29.8	13.0	20.0	9.4	20.0	9.4	20.0	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.5	7.3	6.4	10.2	29.8	13.0	20.0	9.4	20.0	9.4	20.0	9.4
LOS	A	A	A	B	B	C	B	B	B	B	A	A
Approach Delay	7.3			9.9		24.9					11.0	
Approach LOS	A			A		C					B	
Queue Length 50th (m)	0.8	17.6	2.0	38.2	12.5	2.2	2.2	1.1	1.1	0.9	0.9	0.9
Queue Length 95th (m)	3.7	40.2	7.0	84.6	26.0	9.4	4.7	4.7	4.7	4.7	4.7	4.7
Internal Link Dist (m)				351.9		379.2		249.4		312.2		
Turn Bay Length (m)	65.0			65.0		30.0		30.0		30.0		
Base Capacity (vph)	416	1124	614	1170	414	534	379	538	379	538	379	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.34	0.08	0.55	0.08	0.30	0.10	0.03	0.12	0.03	0.12	0.03
Intersection Summary												
Cycle Length: 70												
Actuated Cycle Length: 62.2												
Natural Cycle: 60												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 0.55												

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

AM Peak Hour  
12-02-2021

Intersection Signal Delay: 11.1  
Intersection Capacity Utilization 68.1%  
Analysis Period (min) 15  
Intersection LOS: B  
ICU Level of Service C



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
12-02-2021

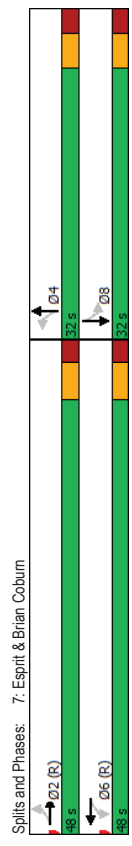
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	3	3	3	3	3	3	3	3	3	3	3	3
Traffic Volume (vph)	30	277	75	34	490	24	151	59	37	25	50	45
Future Volume (vph)	30	277	75	34	490	24	151	59	37	25	50	45
Satd. Flow (prot)	1642	1616	0	1551	1697	0	1658	1480	0	1566	1542	0
Flt Permitted	0.370			0.508			0.695			0.695		
Satd. Flow (perm)	638	1616	0	808	1697	0	1201	1480	0	1112	1542	0
Satd. Flow (RTOR)	26			5			37			45		
Lane Group Flow (vph)	30	352	0	34	514	0	151	96	0	25	95	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	6	6	6	6	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	6	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C	C	C	C	C	C
Act Effct Green (s)	42.0	42.0	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2	26.2	26.2
Actuated G/C Ratio	0.52	0.52	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.09	0.41	0.08	0.58	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18
Control Delay	10.4	12.3	10.1	16.0	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.4	12.3	10.1	16.0	24.3	13.8	19.3	12.3	19.3	12.3	12.3	12.3
LOS	B	B	B	B	B	C	B	B	B	B	B	B
Approach Delay	12.2	15.7	15.7	20.2	20.2	13.7	13.7	13.7	13.7	13.7	13.7	13.7
Approach LOS	B	B	B	C	C	B	B	B	B	B	B	B
Queue Length 50th (m)	2.1	27.8	2.4	49.3	17.5	6.2	2.6	5.2	2.6	5.2	5.2	5.2
Queue Length 95th (m)	6.3	46.3	6.7	77.7	33.2	16.5	7.8	15.3	7.8	15.3	15.3	15.3
Internal Link Dist (m)	379.2			585.6		222.2		382.8				
Turn Bay Length (m)	65.0			65.0		30.0		30.0				
Base Capacity (vph)	334	860	424	893	393	509	364	535	364	535	535	535
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.41	0.08	0.58	0.38	0.19	0.07	0.18	0.07	0.18	0.07	0.18

Intersection Summary	
Cycle Length:	80
Actuated Cycle Length:	80
Offset:	0 (0%), Referenced to phase 2EBTL and 6:WBTL, Start of Green
Natural Cycle:	55
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

AM Peak Hour  
12-02-2021

Maximum v/c Ratio:	0.58
Intersection LOS:	B
Intersection Signal Delay:	15.3
ICU Level of Service B	
Intersection Capacity Utilization:	55.2%
Analysis Period (min):	15



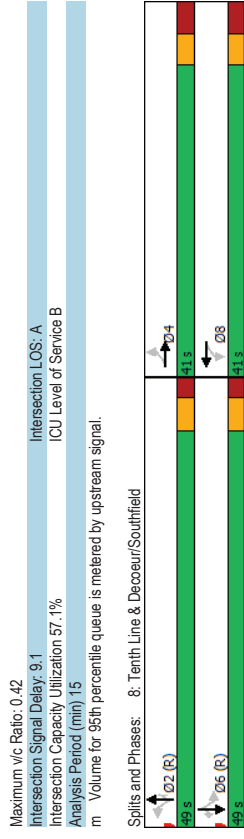


Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	88	27	44	9	29	70	81	729	1	19	410	59
Traffic Volume (vph)	89	27	44	9	29	70	81	729	1	19	410	59
Future Volume (vph)	1610	1389	0	1658	1545	0	1445	3131	1483	1523	3161	1401
Satd. Flow (prot)	0.693			0.711			0.511			0.366		
FI Permitted	1173	1389	0	1241	1545	0	773	3131	1442	585	3161	1359
Satd. Flow (perm)	44			70			47			59		
Lane Group Flow (vph)	89	71	0	9	99	0	81	729	1	19	410	59
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	4	4		8	8	8	2	2	2	6	6	6
Detector Phase	4	4		8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9	40.9	40.9	40.9	40.9	28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	49.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6	3.6	3.6	3.6	3.6	2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9	6.9	6.9	6.9	6.9	5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.2	16.2	16.2	16.2	16.2	16.2	65.6	65.6	65.6	65.6	65.6	65.6
Actuated v/c Ratio	0.18	0.18	0.18	0.18	0.18	0.18	0.73	0.73	0.73	0.73	0.73	0.73
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.3	14.8	36.3	14.8	36.3	14.8	8.8	7.7	0.0	6.6	4.8	2.2
LOS	D	B	D	B	D	B	A	A	A	A	A	A
Approach Delay	26.8			13.6			7.8			4.5		
Approach LOS	C			B			A			A		
Queue Length 50th (m)	14.6	4.2	1.4	4.5	3.7	20.0	0.0	0.0	0.3	3.4	0.0	0.0
Queue Length 95th (m)	20.7	11.3	3.9	12.9	17.1	58.1	0.0	0.0	0.4	39.8	11.0	0.0
Internal Link Dist (m)	95.2			315.6			346.2			120.2		
Turn Bay Length (m)	44.0	55.3	47.0	628	563	2282	1063	426	2304	1006	0	0
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.13	0.02	0.16	0.14	0.32	0.00	0.04	0.18	0.06		

Intersection Summary	
Cycle Length:	90
Actuated Cycle Length:	90
Offset:	36 (40%), Referenced to phase 2/NBTL and 6/SBTL, Start of Green
Natural Cycle:	70
Control Type:	Actuated-Coordinated



Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lanes, Volumes, Timings  
9: Tenth Line & Sweetvalley/Harvest Valley

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	368	33	76	335	58
Traffic Volume (vph)	135	3	12	70	1	290	5	368	33	76	335	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3187	0
Satd. Flow (prot)	0.457			0.748			0.520			0.521		
Flt Permitted	795	1433	0	1304	1447	0	771	3074	0	867	3187	0
Satd. Flow (perm)	12			290			17			34		
Lane Group Flow (vph)	135	15	0	70	291	0	5	391	0	76	393	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	8	2	2	6	6			
Permitted Phases	4	4	8	8	8	2	2	6	6			
Detector Phase	4	4	8	8	8	2	2	6	6			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	135	3	12	70	1	290	5	368	33	76	335	58
Traffic Volume (vph)	135	3	12	70	1	290	5	368	33	76	335	58
Future Volume (vph)	1658	1433	0	1658	1447	0	1409	3074	0	1580	3187	0
Satd. Flow (prot)	0.457			0.748			0.520			0.521		
Flt Permitted	795	1433	0	1304	1447	0	771	3074	0	867	3187	0
Satd. Flow (perm)	12			290			17			34		
Lane Group Flow (vph)	135	15	0	70	291	0	5	391	0	76	393	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	8	2	2	6	6			
Permitted Phases	4	4	8	8	8	2	2	6	6			
Detector Phase	4	4	8	8	8	2	2	6	6			
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	43.8%	43.8%	43.8%	43.8%	43.8%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%	56.3%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2	6.2

Lead/Lag Optimize?

Intersection LOS: B  
ICU Level of Service D

Recall Mode: None

Intersection Signal Delay: 12.0  
Analysis Period (min) 15

Act Effct Green (s): 16.3

Splits and Phases: 9: Tenth Line & Sweetvalley/Harvest Valley

Actuated g/C Ratio: 0.24



v/c Ratio: 0.71

Intersection Capacity Utilization 77.1%

Control Delay: 44.1

Intersection LOS: B

Queue Delay: 0.0

ICU Level of Service D

Total Delay: 44.1

Analysis Period (min) 15

LOS: D

Intersection LOS: B

Approach Delay: 40.9

ICU Level of Service D

Approach LOS: D

Analysis Period (min) 15

Queue Length 50th (m): 15.6

Intersection LOS: B

Queue Length 95th (m): 32.7

ICU Level of Service D

Internal Link Dist (m): 180.2

Analysis Period (min) 15

Turn Bay Length (m): 38.0

Intersection LOS: B

Base Capacity (vph): 334

ICU Level of Service D

Starvation Cap Reductn: 0

Analysis Period (min) 15

Spillback Cap Reductn: 0

Intersection LOS: B

Storage Cap Reductn: 0

ICU Level of Service D

Reduced v/c Ratio: 0.40

Analysis Period (min) 15

Intersection Summary

Intersection LOS: B

Cycle Length: 80

ICU Level of Service D

Actuated Cycle Length: 68.2

Analysis Period (min) 15

Natural Cycle: 65

Intersection LOS: B

Control Type: Actuated-Uncoordinated

ICU Level of Service D

Maximum v/c Ratio: 0.71

Analysis Period (min) 15

Intersection	Int Delay, s/veh							
	EBL	EBR	NBL	NBT	SBT	SBR		
Int Delay, s/veh	0							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations								
Traffic Vol, veh/h	0	5	0	889	480	13		
Future Vol, veh/h	0	5	0	889	480	13		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	0	-	-	-	-		
Veh in Median Storage, #	0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	100	100	100	100	100	100		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	0	5	0	889	480	13		
Major/Minor	Minor2	Major1	Major1	Major2				
Conflicting Flow All	-	247	-	0	-	0		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Critical Hdwy	-	6.94	-	-	-	-		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	3.32	-	-	-	-		
Pot Cap-1 Maneuver	0	753	0	-	-	-		
Stage 1	0	0	0	-	-	-		
Stage 2	0	0	0	-	-	-		
Platoon blocked, %	-	-	-	-	-	-		
Mov Cap-1 Maneuver	-	753	-	-	-	-		
Mov Cap-2 Maneuver	-	-	-	-	-	-		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Approach	EB	NB	SB					
HCM Control Delay, s	9.8	0	0					
HCM LOS	A							
Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR				
Capacity (veh/h)	-	753	-	-				
HCM Lane V/C Ratio	-	0.007	-	-				
HCM Control Delay (s)	-	9.8	-	-				
HCM Lane LOS	-	A	-	-				
HCM 95th %tile Q(veh)	-	0	-	-				

Intersection	Int Delay, s/veh							
	EBL	EBT	WBT	WBR	SBL	SBR		
Int Delay, s/veh	0.3							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations								
Traffic Vol, veh/h	1	156	149	2	4	6		
Future Vol, veh/h	1	156	149	2	4	6		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	-	-	-	0	-		
Veh in Median Storage, #	-	0	0	0	-	0		
Grade, %	-	0	0	0	-	0		
Peak Hour Factor	100	100	100	100	100	100		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	1	156	149	2	4	6		
Major/Minor	Major1	Major2	Minor2					
Conflicting Flow All	151	0	-	0	308	150		
Stage 1	-	-	-	-	150	-		
Stage 2	-	-	-	-	158	-		
Critical Hdwy	4.12	-	-	-	6.42	6.22		
Critical Hdwy Stg 1	-	-	-	-	5.42	-		
Critical Hdwy Stg 2	-	-	-	-	5.42	-		
Follow-up Hdwy	2.218	-	-	-	3.518	3.318		
Pot Cap-1 Maneuver	1430	-	-	-	684	896		
Stage 1	-	-	-	-	878	-		
Stage 2	-	-	-	-	871	-		
Platoon blocked, %	-	-	-	-	-	-		
Mov Cap-1 Maneuver	1430	-	-	-	683	896		
Mov Cap-2 Maneuver	-	-	-	-	683	-		
Stage 1	-	-	-	-	877	-		
Stage 2	-	-	-	-	871	-		
Approach	EB	WB	SB					
HCM Control Delay, s	0	0	9.6					
HCM LOS								A
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1			
Capacity (veh/h)	1430	-	-	-	797			
HCM Lane V/C Ratio	0.001	-	-	-	0.013			
HCM Control Delay (s)	7.5	0	-	-	9.6			
HCM Lane LOS	A	A	-	-	A			
HCM 95th %tile Q(veh)	0	-	-	-	0			

12-02-2021  
AM Peak Hour  
12: Site Access & Brian Coburn

HCM 2010 TWSC

Intersection	EBT	EBR	WBL	WBT	NBL	NBR
Int Delay, s/veh	0.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4/9	2/1	3/846	32/19		
Traffic Vol, veh/h	449	21	3/846	32/19		
Future Vol, veh/h	449	21	3/846	32/19		
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	449	21	3/846	32/19		
Major/Minor	Major1	Major2	Minor1	Minor2		
Conflicting Flow All	0	0	470	0	1312	460
Stage 1	-	-	-	-	460	-
Stage 2	-	-	-	-	852	-
Critical Hwy	-	-	4.12	-	6.42	6.22
Critical Hwy Stg 1	-	-	-	-	5.42	-
Critical Hwy Stg 2	-	-	-	-	5.42	-
Follow-up Hwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1092	-	175	601
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	418	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1092	-	174	601
Mov Cap-2 Maneuver	-	-	-	-	174	-
Stage 1	-	-	-	-	636	-
Stage 2	-	-	-	-	416	-
Approach	EB	WB	NB			
HCM Control Delay, s	0	0	24.3			
HCM LOS			C			
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	237	-	-	1092	-	
HCM Lane V/C Ratio	0.215	-	-	0.003	-	
HCM Control Delay (s)	24.3	-	-	8.3	0	
HCM Lane LOS	C	-	-	A	A	
HCM 95th %ile Q(veh)	0.8	-	-	0	-	

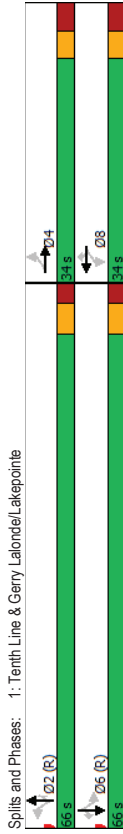
12-02-2021  
PM Peak Hour  
1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	173	103	62	31	25	171	38	1057	63	263	1290	187
Future Volume (vph)	173	103	62	31	25	171	38	1057	63	263	1290	187
Satd. Flow (prot)	1658	1637	0	1658	1745	1483	1658	3316	1483	1658	3316	1455
Flt Permitted	0.741			0.604			0.173			0.239		
Satd. Flow (perm)	1292	1637	0	1049	1745	1464	302	3316	1436	416	3316	1411
Satd. Flow (RTOR)	30			103			63			63		187
Lane Group Flow (vph)	173	165	0	31	25	171	38	1057	63	263	1290	187
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	8	8	8	2	2	2	2	6	6	6
Permitted Phases	4	4	8	8	8	2	2	2	2	6	6	6
Detector Phase	4	4	8	8	8	2	2	2	2	6	6	6
Switch Phase	4	4	8	8	8	2	2	2	2	6	6	6
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	33.8	33.8	33.8	33.8	33.8	27.2	27.2	27.2	27.2	27.2	27.2	27.2
Total Split (s)	34.0	34.0	34.0	34.0	34.0	66.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	34.0%	34.0%	34.0%	34.0%	34.0%	66.0%	66.0%	66.0%	66.0%	66.0%	66.0%	66.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.5	3.5	3.5	3.5	3.5	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.8	6.8	6.8	6.2	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	18.8	18.8	18.8	18.8	18.8	68.2	68.2	68.2	68.2	68.2	68.2	68.2
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.19	0.68	0.68	0.68	0.68	0.68	0.68	0.68
v/c Ratio	0.71	0.50	0.16	0.08	0.48	0.18	0.47	0.06	0.93	0.57	0.18	0.18
Control Delay	53.6	33.5	33.0	30.8	18.7	5.7	5.6	0.5	57.9	10.4	1.7	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.6	33.5	33.0	30.8	18.7	5.7	5.6	0.5	57.9	10.4	1.7	1.7
LOS	D	C	C	C	B	A	A	A	A	E	B	A
Approach Delay	43.7	22.0	5.3									16.7
Approach LOS	D	C	C									B
Queue Length 50th (m)	31.8	23.4	5.1	4.0	11.3	1.2	57.3	0.2	38.9	60.4	0.0	0.0
Queue Length 95th (m)	49.0	38.8	11.8	9.9	27.4	m2.6	19.8	0.6	#103.9	101.0	8.0	8.0
Internal Link Dist (m)	372.5		134.8			154.1				468.1		
Turn Bay Length (m)	30.0		50.0		35.0	55.0		70.0	50.0	75.0		
Base Capacity (vph)	351	467	285	474	473	206	2262	999	283	2262	1022	750
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.35	0.11	0.05	0.36	0.18	0.47	0.06	0.93	0.57	0.18	0.18
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 90 (90%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green												
Natural Cycle: 110												
Control Type: Actuated-Coordinated												

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

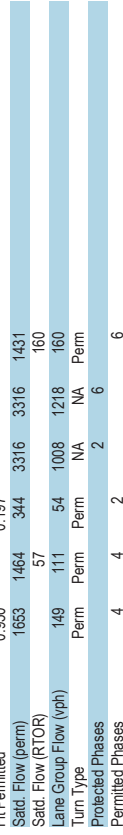
Maximum v/c Ratio: 0.93  
 Intersection Signal Delay: 15.9  
 Intersection Capacity Utilization 88.1%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Maximum v/c Ratio: 0.93  
 Intersection Signal Delay: 15.9  
 Intersection Capacity Utilization 88.1%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 Volume for 95th percentile queue is metered by upstream signal.



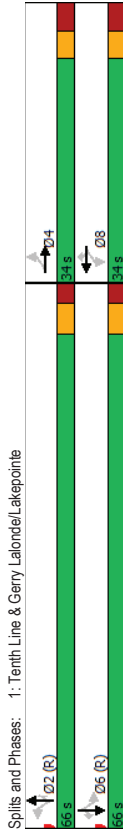
Splits and Phases: 2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	111	54	1008	1218	160
Future Volume (vph)	149	111	54	1008	1218	160
Satd. Flow (prot)	1688	1483	1688	3316	3316	1483
Flt Permitted	0.950	0.197				
Satd. Flow (perm)	1653	1464	344	3316	3316	1431
Satd. Flow (RTOR)	57					160
Lane Group Flow (vph)	149	111	54	1008	1218	160
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Green (s)	37.8	37.8	24.2	24.2	24.2	24.2
Minimum Split (s)	38.0	38.0	62.0	62.0	62.0	62.0
Total Split (%)	38.0%	38.0%	62.0%	62.0%	62.0%	62.0%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	70.2	70.2	70.2	70.2
Actuated g/C Ratio	0.17	0.17	0.70	0.70	0.70	0.70
v/c Ratio	0.54	0.38	0.22	0.43	0.52	0.15
Control Delay	43.4	21.1	10.8	8.3	5.8	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	21.1	10.8	8.3	5.9	0.7
LOS	D	C	B	A	A	A
Approach Delay	33.9		8.4	5.3		
Approach LOS	C		A	A		
Queue Length 50th (m)	27.5	9.4	2.9	34.4	30.0	0.1
Queue Length 95th (m)	37.7	20.2	13.7	78.2	38.4	2.5
Internal Link Dist (m)	33.9			222.1	154.1	
Turn Bay Length (m)			75.0			60.0
Base Capacity (vph)	515	495	241	2326	2326	1051
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.22	0.43	0.54	0.15

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	85 (85%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

Lanes, Volumes, Timings  
1: Tenth Line & Gerry Lalonde/Lakepointe

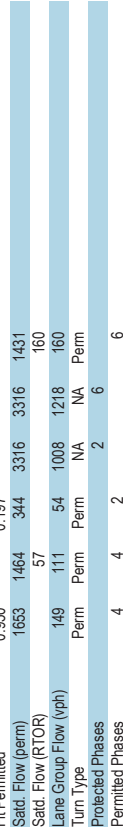
Maximum v/c Ratio: 0.93  
 Intersection Signal Delay: 15.9  
 Intersection Capacity Utilization 88.1%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 1: Tenth Line & Gerry Lalonde/Lakepointe

Lanes, Volumes, Timings  
2: Tenth Line & The Shops

Maximum v/c Ratio: 0.93  
 Intersection Signal Delay: 15.9  
 Intersection Capacity Utilization 88.1%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 2: Tenth Line & The Shops

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	149	111	54	1008	1218	160
Future Volume (vph)	149	111	54	1008	1218	160
Satd. Flow (prot)	1688	1483	1688	3316	3316	1483
Flt Permitted	0.950	0.197				
Satd. Flow (perm)	1653	1464	344	3316	3316	1431
Satd. Flow (RTOR)	57					160
Lane Group Flow (vph)	149	111	54	1008	1218	160
Turn Type	Perm	Perm	Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4	4	2	2	6	6
Detector Phase	4	4	2	2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Green (s)	37.8	37.8	24.2	24.2	24.2	24.2
Minimum Split (s)	38.0	38.0	62.0	62.0	62.0	62.0
Total Split (%)	38.0%	38.0%	62.0%	62.0%	62.0%	62.0%
Yellow Time (s)	3.0	3.0	3.7	3.7	3.7	3.7
All-Red Time (s)	3.8	3.8	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.8	6.8	6.2	6.2	6.2	6.2
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	16.8	16.8	70.2	70.2	70.2	70.2
Actuated g/C Ratio	0.17	0.17	0.70	0.70	0.70	0.70
v/c Ratio	0.54	0.38	0.22	0.43	0.52	0.15
Control Delay	43.4	21.1	10.8	8.3	5.8	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.4	21.1	10.8	8.3	5.9	0.7
LOS	D	C	B	A	A	A
Approach Delay	33.9		8.4	5.3		
Approach LOS	C		A	A		
Queue Length 50th (m)	27.5	9.4	2.9	34.4	30.0	0.1
Queue Length 95th (m)	37.7	20.2	13.7	78.2	38.4	2.5
Internal Link Dist (m)	33.9			222.1	154.1	
Turn Bay Length (m)			75.0			60.0
Base Capacity (vph)	515	495	241	2326	2326	1051
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.22	0.22	0.43	0.54	0.15

Intersection Summary	
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	85 (85%), Referenced to phase 2:NBLT and 6:SBT, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated

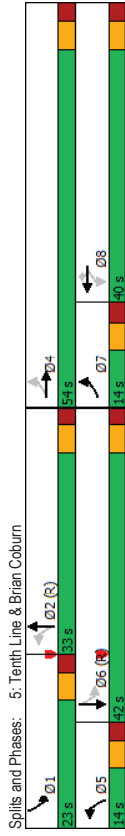


Lanes, Volumes, Timings  
5: Tenth Line & Brian Coburn

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Maximum v/c Ratio: 1.00  
 Intersection Signal Delay: 52.8  
 Intersection Capacity Utilization 113.1%  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Lane Configurations  
 Traffic Volume (vph)  
 Future Volume (vph)  
 Satd. Flow (prot)  
 Flt Permitted  
 Satd. Flow (perm)  
 Satd. Flow (RTOR)  
 Lane Group Flow (vph)  
 Turn Type  
 Protected Phases  
 Permitted Phases  
 Detector Phase  
 Switch Phase



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	56	720	73	31	434	20	71	19	27	28	13	31
Future Volume (vph)	56	720	73	31	434	20	71	19	27	28	13	31
Satd. Flow (prot)	1688	1716	0	1658	1714	0	1626	1546	0	1523	1532	0
Flt Permitted	0.489			0.291			0.728			0.727		
Satd. Flow (perm)	848	1716	0	507	1714	0	1240	1546	0	1134	1532	0
Satd. Flow (RTOR)	11			5			27			31		
Lane Group Flow (vph)	56	793	0	31	454	0	71	46	0	28	44	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2			6			4			8		
Permitted Phases	2			6			4			8		
Detector Phase	2			6			4			8		
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	24.4	24.4	24.4	24.4	24.4	24.4
Total Split (s)	54.0	54.0	54.0	54.0	54.0	54.0	26.0	26.0	26.0	26.0	26.0	26.0
Total Split (%)	67.5%	67.5%	67.5%	67.5%	67.5%	67.5%	32.5%	32.5%	32.5%	32.5%	32.5%	32.5%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	3.4	3.4	3.4	3.4	3.4	3.4	3.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4

Lead-Lag Optimize?

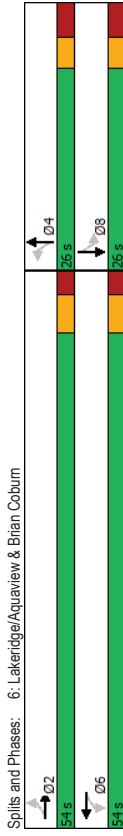
Recall Mode	Max	Max	Max	None	None	None
Act Effct Green (s)	55.0	55.0	55.0	11.2	11.2	11.2
Actuated g/C Ratio	0.74	0.74	0.74	0.15	0.15	0.15
v/c Ratio	0.09	0.62	0.08	0.36	0.38	0.18
Control Delay	5.0	9.5	5.3	5.9	33.9	16.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.0	9.5	5.3	5.9	33.9	16.4
LOS	A	A	A	A	C	B
Approach Delay	9.2	5.9	27.0	27.0	20.2	20.2
Approach LOS	A	A	C	C	C	C
Queue Length 50th (m)	2.1	50.1	1.2	21.1	8.6	2.2
Queue Length 95th (m)	6.8	107.6	4.6	44.2	19.6	10.2
Internal Link Dist (m)	351.9		379.2	249.4		
Turn Bay Length (m)	65.0		65.0	30.0		30.0
Base Capacity (vph)	631	1280	377	1276	330	431
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.62	0.08	0.36	0.22	0.11

Intersection Summary

Cycle Length: 80
Actuated Cycle Length: 73.9
Natural Cycle: 60
Control Type: Semi Act-Uncoord
Maximum v/c Ratio: 0.62

Lanes, Volumes, Timings  
6: Lakeridge/Aquaview & Brian Coburn

Intersection Signal Delay: 10.0  
Intersection Capacity Utilization 70.8%  
Analysis Period (min) 15



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Intersection LOS: B  
ICU Level of Service C

PM Peak Hour  
12-02-2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	52	518	188	27	344	18	103	43	25	23	48	38
Future Volume (vph)	52	518	188	27	344	18	103	43	25	23	48	38
Satd. Flow (prot)	1658	1675	0	1658	1714	0	1658	1549	0	1658	1490	0
Flt Permitted	0.499			0.222			0.701			0.713		
Satd. Flow (perm)	865	1675	0	387	1714	0	1206	1549	0	1242	1490	0
Satd. Flow (RTOR)	34			5			25			38		
Lane Group Flow (vph)	52	706	0	27	362	0	103	68	0	23	86	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	6	6	6	4	4	4	8	8	8	8
Detector Phase	2	2	6	6	6	4	4	4	8	8	8	8
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	23.8	23.8	23.8	23.8	23.8	23.8	23.8
Total Split (s)	48.0	48.0	48.0	48.0	48.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	60.0%	60.0%	60.0%	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.3	3.3	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.3	2.3	2.3	2.3	2.3	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0	5.8	5.8	5.8	5.8	5.8	5.8	5.8
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	42.0	42.0	42.0	42.0	42.0	26.2	26.2	26.2	26.2	26.2	26.2	26.2
Actuated g/C Ratio	0.52	0.52	0.52	0.52	0.52	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.11	0.79	0.13	0.40	0.26	0.13	0.26	0.13	0.06	0.06	0.17	0.17
Control Delay	10.5	22.7	11.8	12.9	22.1	14.1	19.0	12.9	0.0	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.5	22.7	11.8	12.9	22.1	14.1	19.0	12.9	0.0	0.0	0.0	0.0
LOS	B	C	B	B	B	C	B	B	B	B	B	B
Approach Delay	21.9	12.9	12.9	18.9	14.2							
Approach LOS	C	B	B	B	B							
Queue Length 50th (m)	3.7	77.8	2.0	30.5	11.4	4.5	2.4	5.0				
Queue Length 95th (m)	9.3	#126.8	6.4	49.1	23.4	12.9	7.3	14.5				
Internal Link Dist (m)	379.2		585.6	222.2								
Turn Bay Length (m)	65.0		65.0	30.0								
Base Capacity (vph)	454	895	203	902	394	524	406	513				
Starvation Cap Reductn	0	0	0	0	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0	0	0	0				
Reduced v/c Ratio	0.11	0.79	0.13	0.40	0.26	0.13	0.06	0.17				

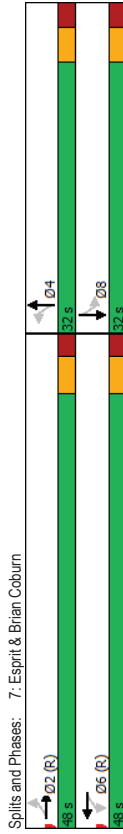
Intersection Summary	
Cycle Length: 80	
Actuated Cycle Length: 80	
Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green	
Natural Cycle: 60	
Control Type: Actuated-Coordinated	



Lanes, Volumes, Timings  
7: Esprit & Brian Coburn

Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Maximum v/c Ratio: 0.79  
 Intersection Signal Delay: 18.5  
 Intersection LOS: B  
 Intersection Capacity Utilization 70.4%  
 ICU Level of Service C  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

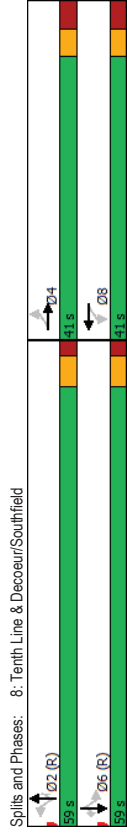


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Traffic Volume (vph)	50	16	30	2	24	55	38	688	14	116	938	94
Future Volume (vph)	50	16	30	2	24	55	38	688	14	116	938	94
Satd. Flow (prot)	1688	1561	0	1658	1564	0	1658	3316	1483	1658	3316	1483
Flt Permitted							0.727	0.289				0.387
Satd. Flow (perm)	1232	1561	0	1268	1564	0	503	3316	1483	675	3316	1435
Satd. Flow (RTOR)	30			55			55		43			94
Lane Group Flow (vph)	50	46	0	2	79	0	38	688	14	116	938	94
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4		8	8		8	8	2	2	6	6
Detector Phase	4	4		8	8		8	8	2	2	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	40.9	40.9		40.9	40.9		28.9	28.9	28.9	28.9	28.9	28.9
Total Split (s)	41.0	41.0		41.0	41.0		59.0	59.0	59.0	59.0	59.0	59.0
Total Split (%)	41.0%	41.0%		41.0%	41.0%		59.0%	59.0%	59.0%	59.0%	59.0%	59.0%
Yellow Time (s)	3.3	3.3		3.3	3.3		3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.6	3.6		3.6	3.6		2.2	2.2	2.2	2.2	2.2	2.2
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.9	6.9		6.9	6.9		5.9	5.9	5.9	5.9	5.9	5.9
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	15.0	15.0		15.0	15.0		76.7	76.7	76.7	76.7	76.7	76.7
Actuated g/C Ratio	0.15	0.15		0.15	0.15		0.77	0.77	0.77	0.77	0.77	0.77
v/c Ratio	0.27	0.18		0.01	0.28		0.10	0.27	0.01	0.22	0.37	0.08
Control Delay	38.0	17.1		29.0	15.7		7.9	6.2	0.0	8.3	6.9	2.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.0	17.1		29.0	15.7		7.9	6.2	0.0	8.3	6.9	2.2
LOS	D	B		C	B		A	A	A	A	A	A
Approach Delay		28.0		16.1			6.2			6.7		
Approach LOS		C		B			A			A		
Queue Length 50th (m)	9.2	2.9		0.4	4.3		1.5	16.2	0.0	5.0	24.4	0.0
Queue Length 95th (m)	15.0	9.6		1.8	12.8		9.3	52.0	0.3	24.5	76.3	7.0
Internal Link Dist (m)		95.2			315.6			346.2			120.2	
Turn Bay Length (m)	45.0			20.0			90.0		60.0		60.0	70.0
Base Capacity (vph)	420	552		432	569		386	2544	1147	518	2544	1123
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.08		0.00	0.14		0.10	0.27	0.01	0.22	0.37	0.08

Intersection Summary												
Cycle Length:	100											
Actuated Cycle Length:	100											
Offset:	21 (21%), Referenced to phase 2:NBLT and 6:SBTL, Start of Green											
Natural Cycle:	70											
Control Type:	Actuated-Coordinated											

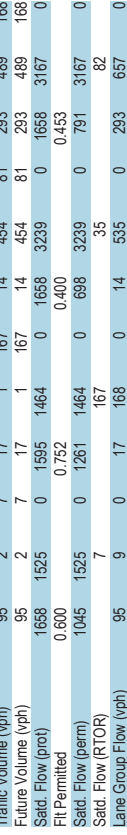
Lanes, Volumes, Timings  
8: Tenth Line & Decoeur/Southfield

Maximum v/c Ratio: 0.37  
 Intersection Signal Delay: 7.8  
 Intersection Capacity Utilization 61.5%  
 Analysis Period (min) 15



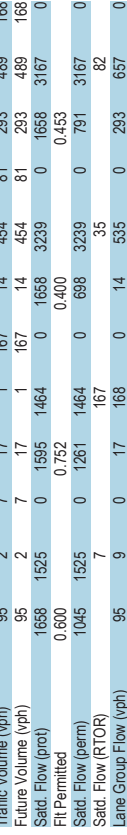
Lanes, Volumes, Timings  
9: Tenth Line & SweetValley/HarvestValley

Intersection LOS: A  
 ICU Level of Service B



Lanes, Volumes, Timings  
PM Peak Hour  
12-02-2021

Intersection LOS: A  
 ICU Level of Service B



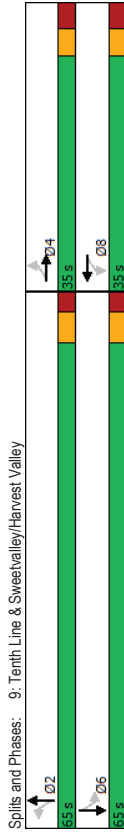
Lanes, Volumes, Timings  
PM Peak Hour  
12-02-2021

Intersection LOS: A  
 ICU Level of Service B



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	95	2	7	17	1	167	14	464	81	293	489	168
Traffic Volume (vph)	95	2	7	17	1	167	14	464	81	293	489	168
Future Volume (vph)	1688	1625	0	1595	1464	0	1658	3239	0	1658	3167	0
Satd. Flow (prot)	0.600			0.752			0.400				0.453	
Flt Permitted	1045	1525	0	1261	1464	0	698	3239	0	791	3167	0
Satd. Flow (RTOR)	7			167			35			82		
Lane Group Flow (vph)	95	9	0	17	168	0	14	535	0	293	657	0
Lane Type	Perm	NA	0	Perm	NA	0	Perm	NA	0	Perm	NA	0
Protected Phases	4			8			2			6		
Permitted Phases	4			8			2			6		
Detector Phase	4			8			2			6		
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	34.5	34.5	34.5	34.5	34.5	34.5	29.2	29.2	29.2	29.2	29.2	29.2
Total Split (s)	35.0	35.0	35.0	35.0	35.0	35.0	65.0	65.0	65.0	65.0	65.0	65.0
Total Split (%)	35.0%	35.0%	35.0%	35.0%	35.0%	35.0%	65.0%	65.0%	65.0%	65.0%	65.0%	65.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	3.2	3.2	3.2	3.2	3.2	3.2	2.5	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.2	6.2	6.2	6.2	6.2	6.2
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Act Effct Green (s)	14.9	14.9	14.9	14.9	14.9	14.9	61.8	61.8	61.8	61.8	61.8	61.8
Actuated g/C Ratio	0.17	0.17	0.17	0.17	0.17	0.17	0.69	0.69	0.69	0.69	0.69	0.69
v/c Ratio	0.55	0.03	0.08	0.44	0.08	0.44	0.03	0.24	0.54	0.30	0.54	0.30
Control Delay	44.6	18.1	29.4	8.7	6.7	5.9	13.2	5.8	13.2	5.8	13.2	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.6	18.1	29.4	8.7	6.7	5.9	13.2	5.8	13.2	5.8	13.2	5.8
LOS	D	B	C	A	A	A	A	A	B	A	B	A
Approach Delay	42.3			10.6			5.9			8.1		
Approach LOS	D			B			A			A		
Queue Length 50th (m)	14.5	0.3	2.4	0.1	0.6	12.6	18.9	15.0	18.9	15.0	18.9	15.0
Queue Length 95th (m)	28.5	4.0	7.5	14.7	3.6	31.7	64.3	37.6	64.3	37.6	64.3	37.6
Internal Link Dist (m)	180.2			318.8			263.5			346.2		
Turn Bay Length (m)	38.0			60.0			54.0			65.0		
Base Capacity (vph)	335	493	404	582	482	2248	546	2213	546	2213	546	2213
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.02	0.04	0.29	0.03	0.24	0.54	0.30	0.54	0.30	0.54	0.30
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 89.4												
Natural Cycle: 75												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.55												

Intersection Signal Delay: 9.7  
 Intersection Capacity Utilization 74.6%  
 Analysis Period (min) 15  
 Intersection LOS: A  
 ICU Level of Service D



Intersection	EBL	EBR	NBL	NBT	SBT	SBR
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	14	0	799	1132	39
Future Vol, veh/h	0	14	0	799	1132	39
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- None	- None	- None	- None	- None	- None
Storage Length	- 0	- 0	- 0	- 0	- 0	- 0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	14	0	799	1132	39
Major/Minor	Minor2	Major1	Major1	Major2		
Conflicting Flow All	- 586	- 0	- 0	- 0	- 0	- 0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	454	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	454	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	13.2	0	0	0	0	0
HCM LOS	B					
Minor Lane/Major Mvmt	NB	EB	NB	SB	SBR	
Capacity (veh/h)	-	454	-	-	-	-
HCM Lane V/C Ratio	-	0.031	-	-	-	-
HCM Control Delay (s)	-	13.2	-	-	-	-
HCM Lane LOS	-	B	-	-	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-	-	-

HCM 2010 TWSC  
11: Decoour & Site Access

PM Peak Hour  
12-02-2021

Intersection	Int Delay, s/veh									
	EBL	EBT	WBT	WBR	SBL	SBR				
Int Delay, s/veh	0.6									
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations							W			
Traffic Vol, veh/h	1	92	103	5	4	8				
Future Vol, veh/h	1	92	103	5	4	8				
Conflicting Peds, #/hr	0						0			
Sign Control	Free						Free			
RT Channelized	- None						- None			
Storage Length	-						-			
Veh in Median Storage, #	-						-			
Grade, %	-						-			
Peak Hour Factor	100	100	100	100	100	100				
Heavy Vehicles, %	2	2	2	2	2	2				
Mvmt Flow	1	92	103	5	4	8				
Major/Minor	Major1	Major2	Minor2							
Conflicting Flow All	108	0	0	200	106					
Stage 1	-	-	-	106	-					
Stage 2	-	-	-	94	-					
Critical Hdwy	4.12	-	-	6.42	6.22					
Critical Hdwy Stg 1	-	-	-	5.42	-					
Critical Hdwy Stg 2	-	-	-	5.42	-					
Follow-up Hdwy	2.218	-	-	3.518	3.318					
Pot Cap-1 Maneuver	1483	-	-	788	948					
Stage 1	-	-	-	917	-					
Stage 2	-	-	-	930	-					
Platoon blocked, %										
Mov Cap-1 Maneuver	1483	-	-	788	948					
Mov Cap-2 Maneuver	-	-	-	788	-					
Stage 1	-	-	-	917	-					
Stage 2	-	-	-	930	-					
Approach	EB	WB	SB							
HCM Control Delay, s	0.1	0	9.1							
HCM LOS	A									
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1					
Capacity (veh/h)	1483	-	-	-	888					
HCM Lane V/C Ratio	0.001	-	-	-	0.014					
HCM Control Delay (s)	7.4	0	-	-	9.1					
HCM Lane LOS	A	A	-	-	A					
HCM 95th %tile Q(veh)	0	-	-	-	0					

HCM 2010 TWSC  
12: Site Access & Brian Coburn

PM Peak Hour  
12-02-2021

Intersection	Int Delay, s/veh									
	EBT	EBR	WBT	WBR	NBL	NBR				
Int Delay, s/veh	1.8									
Movement	EBT	EBR	WBT	WBR	NBL	NBR				
Lane Configurations							W			
Traffic Vol, veh/h	934	59	6	586	39	30				
Future Vol, veh/h	934	59	6	586	39	30				
Conflicting Peds, #/hr	0						0			
Sign Control	Free						Free			
RT Channelized	- None						- None			
Storage Length	-						-			
Veh in Median Storage, #	-						-			
Grade, %	-						-			
Peak Hour Factor	100	100	100	100	100	100				
Heavy Vehicles, %	2	2	2	2	2	2				
Mvmt Flow	934	59	6	586	39	30				
Major/Minor	Major1	Major2	Minor1							
Conflicting Flow All	0	0	993	0	1562	964				
Stage 1	-	-	-	-	964	-				
Stage 2	-	-	-	-	598	-				
Critical Hdwy	-	-	4.12	-	6.42	6.22				
Critical Hdwy Stg 1	-	-	5.42	-	-	-				
Critical Hdwy Stg 2	-	-	5.42	-	-	-				
Follow-up Hdwy	-	-	2.218	-	3.518	3.318				
Pot Cap-1 Maneuver	-	-	696	-	123	310				
Stage 1	-	-	370	-	-	-				
Stage 2	-	-	549	-	-	-				
Platoon blocked, %										
Mov Cap-1 Maneuver	-	-	696	-	121	310				
Mov Cap-2 Maneuver	-	-	-	-	121	-				
Stage 1	-	-	-	-	370	-				
Stage 2	-	-	-	-	542	-				
Approach	EB	WB	NB							
HCM Control Delay, s	0	0.1	41.7							
HCM LOS	E									
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT					
Capacity (veh/h)	165	-	-	696	-					
HCM Lane V/C Ratio	0.418	-	-	0.009	-					
HCM Control Delay (s)	41.7	-	-	10.2	0					
HCM Lane LOS	E	-	-	B	A					
HCM 95th %tile Q(veh)	1.9	-	-	0	-					

## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde AM FT2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	83	2.0	0.189	9.7	LOSA	1.1	7.6	0.57	0.66	0.57	49.9	
2	T1	21	2.0	0.189	4.5	LOSA	1.1	7.6	0.57	0.66	0.57	46.8	
3	R2	78	2.0	0.189	4.9	LOSA	1.1	7.6	0.57	0.66	0.57	48.7	
Approach													
		182	2.0	0.189	7.1	LOSA	1.1	7.6	0.57	0.66	0.57	49.0	
East: Brian Coburn													
4	L2	44	2.0	0.843	12.9	LOSB	14.5	103.2	0.88	0.68	0.96	50.1	
5	T1	1024	2.0	0.843	7.5	LOSA	14.5	103.2	0.88	0.68	0.96	53.3	
6	R2	13	2.0	0.843	7.6	LOSA	14.5	103.2	0.88	0.68	0.96	48.5	
Approach													
		1081	2.0	0.843	7.7	LOSA	14.5	103.2	0.88	0.68	0.96	53.1	
North: Gerry Lalonde													
7	L2	7	2.0	0.613	33.7	LOSC	6.2	44.1	1.00	1.19	1.45	38.4	
8	T1	8	2.0	0.613	28.5	LOSC	6.2	44.1	1.00	1.19	1.45	36.5	
9	R2	185	2.0	0.613	28.9	LOSC	6.2	44.1	1.00	1.19	1.45	37.7	
Approach													
		200	2.0	0.613	29.0	LOSC	6.2	44.1	1.00	1.19	1.45	37.7	
West: Brian Coburn													
10u	U	30	2.0	0.326	11.4	LOSB	2.4	17.1	0.26	0.44	0.26	56.9	
10	L2	40	2.0	0.326	9.2	LOSA	2.4	17.1	0.26	0.44	0.26	52.2	
11	T1	349	2.0	0.326	3.8	LOSA	2.4	17.1	0.26	0.44	0.26	55.7	
12	R2	48	2.0	0.326	3.9	LOSA	2.4	17.1	0.26	0.44	0.26	50.5	
Approach													
		467	2.0	0.326	4.8	LOSA	2.4	17.1	0.26	0.44	0.26	54.9	
All Vehicles													
		1930	2.0	0.843	9.2	LOSA	14.5	103.2	0.71	0.67	0.80	51.0	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

Site: 101 [Brian Coburn Gerry Lalonde PM FT2031]

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Satn %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: Jerome Jodoin													
1	L2	37	2.0	0.408	33.2	LOSC	3.4	24.4	1.00	1.05	1.11	36.0	
2	T1	10	2.0	0.408	28.0	LOSC	3.4	24.4	1.00	1.05	1.11	36.2	
3	R2	36	2.0	0.408	28.4	LOSC	3.4	24.4	1.00	1.05	1.11	37.3	
Approach													
		83	2.0	0.408	30.5	LOSC	3.4	24.4	1.00	1.05	1.11	37.5	
East: Brian Coburn													
4	L2	62	2.0	0.568	11.4	LOSB	5.4	38.7	0.74	0.66	0.76	50.6	
5	T1	552	2.0	0.568	6.0	LOSA	5.4	38.7	0.74	0.66	0.76	53.8	
6	R2	12	2.0	0.568	6.1	LOSA	5.4	38.7	0.74	0.66	0.76	48.9	
Approach													
		626	2.0	0.568	6.5	LOSA	5.4	38.7	0.74	0.66	0.76	53.3	
North: Gerry Lalonde													
7	L2	4	2.0	0.161	11.5	LOSB	1.0	7.3	0.76	0.73	0.76	50.1	
8	T1	18	2.0	0.161	6.3	LOSA	1.0	7.3	0.76	0.73	0.76	46.9	
9	R2	92	2.0	0.161	6.7	LOSA	1.0	7.3	0.76	0.73	0.76	48.9	
Approach													
		114	2.0	0.161	6.8	LOSA	1.0	7.3	0.76	0.73	0.76	48.6	
West: Brian Coburn													
10u	U	27	2.0	0.955	13.6	LOSB	32.9	234.4	1.00	0.52	1.00	53.7	
10	L2	212	2.0	0.955	11.5	LOSB	32.9	234.4	1.00	0.52	1.00	49.5	
11	T1	1086	2.0	0.955	6.1	LOSA	32.9	234.4	1.00	0.52	1.00	52.6	
12	R2	65	2.0	0.955	6.2	LOSA	32.9	234.4	1.00	0.52	1.00	48.0	
Approach													
		1390	2.0	0.955	7.1	LOSA	32.9	234.4	1.00	0.52	1.00	51.9	
All Vehicles													
		2213	2.0	0.955	7.8	LOSA	32.9	234.4	0.91	0.59	0.93	51.4	

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg AM FT2031]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Sat %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Subroutines													
1	L2	115	2.0	0.190	9.6 LOS A	1.1	7.6	0.56	0.67	0.56	49.4		
2	T1	15	2.0	0.190	4.4 LOS A	1.1	7.6	0.56	0.67	0.56	46.4		
3	R2	55	2.0	0.190	4.8 LOS A	1.1	7.6	0.56	0.67	0.56	48.3		
Approach		185	2.0	0.190	7.8 LOS A	1.1	7.6	0.56	0.67	0.56	48.8		
East: Brian Coburn													
4	L2	32	2.0	0.687	10.2 LOS B	7.7	54.8	0.62	0.52	0.62	51.2		
5	T1	863	2.0	0.687	4.9 LOS A	7.7	54.8	0.62	0.52	0.62	54.5		
6	R2	12	2.0	0.687	5.0 LOS A	7.7	54.8	0.62	0.52	0.62	49.5		
Approach		907	2.0	0.687	5.1 LOS A	7.7	54.8	0.62	0.52	0.62	54.3		
North: Strasbourg													
7	L2	25	2.0	0.249	15.9 LOS B	1.8	12.6	0.92	0.90	0.92	46.8		
8	T1	22	2.0	0.249	10.8 LOS B	1.8	12.6	0.92	0.90	0.92	44.0		
9	R2	76	2.0	0.249	11.2 LOS B	1.8	12.6	0.92	0.90	0.92	45.8		
Approach		123	2.0	0.249	12.1 LOS B	1.8	12.6	0.92	0.90	0.92	45.6		
West: Brian Coburn													
10	L2	7	2.0	0.308	9.3 LOS A	2.1	15.2	0.29	0.41	0.29	52.6		
11	T1	379	2.0	0.308	4.0 LOS A	2.1	15.2	0.29	0.41	0.29	56.1		
12	R2	38	2.0	0.308	4.1 LOS A	2.1	15.2	0.29	0.41	0.29	50.8		
Approach		424	2.0	0.308	4.1 LOS A	2.1	15.2	0.29	0.41	0.29	55.5		
All Vehicles		1639	2.0	0.687	5.6 LOS A	7.7	54.8	0.55	0.54	0.55	53.2		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

**Site: 101 [Brian Coburn Strasbourg PM FT2031]**

Mattamy 2370 Tenth Line  
Site Category: (None)  
Roundabout

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flows Total veh/h	Deg. Sat %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South: des Subroutines													
1	L2	65	2.0	0.242	16.1 LOS B	1.7	12.3	0.93	0.91	0.93	45.8		
2	T1	18	2.0	0.242	10.9 LOS B	1.7	12.3	0.93	0.91	0.93	43.2		
3	R2	34	2.0	0.242	11.3 LOS B	1.7	12.3	0.93	0.91	0.93	44.8		
Approach		117	2.0	0.242	13.9 LOS B	1.7	12.3	0.93	0.91	0.93	45.1		
East: Brian Coburn													
4	L2	54	2.0	0.475	9.7 LOS A	4.0	28.5	0.44	0.47	0.44	51.8		
5	T1	536	2.0	0.475	4.3 LOS A	4.0	28.5	0.44	0.47	0.44	55.2		
6	R2	36	2.0	0.475	4.4 LOS A	4.0	28.5	0.44	0.47	0.44	50.1		
Approach		626	2.0	0.475	4.8 LOS A	4.0	28.5	0.44	0.47	0.44	54.6		
North: Strasbourg													
7	L2	23	2.0	0.076	11.0 LOS B	0.4	3.1	0.68	0.68	0.68	49.3		
8	T1	13	2.0	0.076	5.8 LOS A	0.4	3.1	0.68	0.68	0.68	46.2		
9	R2	23	2.0	0.076	6.2 LOS A	0.4	3.1	0.68	0.68	0.68	48.1		
Approach		59	2.0	0.076	8.0 LOS A	0.4	3.1	0.68	0.68	0.68	48.1		
West: Brian Coburn													
10	L2	33	2.0	0.767	10.0 LOS B	10.4	73.8	0.56	0.47	0.58	51.4		
11	T1	962	2.0	0.767	4.7 LOS A	10.4	73.8	0.58	0.47	0.58	54.7		
12	R2	106	2.0	0.767	4.8 LOS A	10.4	73.8	0.58	0.47	0.58	49.7		
Approach		1101	2.0	0.767	4.8 LOS A	10.4	73.8	0.58	0.47	0.58	54.1		
All Vehicles		1903	2.0	0.767	5.5 LOS A	10.4	73.8	0.56	0.50	0.56	53.4		

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Appendix N

TDM Checklist

**TDM Measures Checklist:**  
*Non-Residential Developments (office, institutional, retail or industrial)*

**Legend**

**BASIC** The measure is generally feasible and effective, and in most cases would benefit the development and its users

**BETTER** The measure could maximize support for users of sustainable modes, and optimize development performance

**★** The measure is one of the most dependably effective tools to encourage the use of sustainable modes

TDM measures: <i>Non-residential developments</i>		Check if proposed & add descriptions
<b>1. TDM PROGRAM MANAGEMENT</b>		
<b>1.1 Program coordinator</b>		
BASIC ★	1.1.1 Designate an internal coordinator, or contract with an external coordinator	<input type="checkbox"/>
<b>1.2 Travel surveys</b>		
BETTER	1.2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	<input type="checkbox"/>
<b>2. WALKING AND CYCLING</b>		
<b>2.1 Information on walking/cycling routes &amp; destinations</b>		
BASIC	2.1.1 Display local area maps with walking/cycling access routes and key destinations at major entrances	<input type="checkbox"/>
<b>2.2 Bicycle skills training</b>		
<i>Commuter travel</i>		
BETTER ★	2.2.1 Offer on-site cycling courses for commuters, or subsidize off-site courses	<input type="checkbox"/>
<b>2.3 Valet bike parking</b>		
<i>Visitor travel</i>		
BETTER	2.3.1 Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	<input type="checkbox"/>

TDM measures: <i>Non-residential developments</i>		Check if proposed & add descriptions
<b>3. TRANSIT</b>		
<b>3.1 Transit information</b>		
BASIC	3.1.1 Display relevant transit schedules and route maps at entrances	<input checked="" type="checkbox"/>
BASIC	3.1.2 Provide online links to OC Transpo and STO information	<input checked="" type="checkbox"/>
BETTER	3.1.3 Provide real-time arrival information display at entrances	<input type="checkbox"/>
<b>3.2 Transit fare incentives</b>		
<i>Commuter travel</i>		
BETTER	3.2.1 Offer preloaded PRESTO cards to encourage commuters to use transit	<input type="checkbox"/>
BETTER ★	3.2.2 Subsidize or reimburse monthly transit pass purchases by employees	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.2.3 Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	<input type="checkbox"/>
<b>3.3 Enhanced public transit service</b>		
<i>Commuter travel</i>		
BETTER	3.3.1 Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.3.2 Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	<input type="checkbox"/>
<b>3.4 Private transit service</b>		
<i>Commuter travel</i>		
BETTER	3.4.1 Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	<input type="checkbox"/>
<i>Visitor travel</i>		
BETTER	3.4.2 Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	<input type="checkbox"/>



TDM measures: Non-residential developments		Check if proposed & add descriptions
<b>4. RIDESHARING</b>		
<i>Commuter travel</i>		
<b>4.1</b>	<b>Ridematching service</b>	
<i>BASIC</i>	4.1.1 Provide a dedicated ridematching portal at OttawaRideMatch.com	<input type="checkbox"/>
<b>4.2 Carpool parking price incentives</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	4.2.1 Provide discounts on parking costs for registered carpools	<input type="checkbox"/>
<b>4.3 Vanpool service</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	4.3.1 Provide a vanpooling service for long-distance commuters	<input type="checkbox"/>
<b>5. CARSHARING &amp; BIKESHARING</b>		
<b>5.1 Bikeshare stations &amp; memberships</b>		
<i>BETTER</i>	5.1.1 Contract with provider to install on-site bikeshare station for use by commuters and visitors	<input type="checkbox"/>
<i>Commuter travel</i>		
<i>BETTER</i>	5.1.2 Provide employees with bikeshare memberships for local business travel	<input type="checkbox"/>
<b>5.2 Carshare vehicles &amp; memberships</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	5.2.1 Contract with provider to install on-site carshare vehicles and promote their use by tenants	<input type="checkbox"/>
<i>BETTER</i>	5.2.2 Provide employees with carshare memberships for local business travel	<input type="checkbox"/>
<b>6. PARKING</b>		
<b>6.1 Priced parking</b>		
<i>Commuter travel</i>		
<i>BASIC</i>	6.1.1 Charge for long-term parking (daily, weekly, monthly)	<input checked="" type="checkbox"/>
<i>BASIC</i>	6.1.2 Unbundle parking cost from lease rates at multi-tenant sites	<input type="checkbox"/>
<i>Visitor travel</i>		
<i>BETTER</i>	6.1.3 Charge for short-term parking (hourly)	<input type="checkbox"/>

TDM measures: Non-residential developments		Check if proposed & add descriptions
<b>7. TDM MARKETING &amp; COMMUNICATIONS</b>		
<b>7.1 Multimodal travel information</b>		
<i>Commuter travel</i>		
<i>BASIC</i>	7.1.1 Provide a multimodal travel option information package to new/relocating employees and students	<input checked="" type="checkbox"/>
<i>Visitor travel</i>		
<i>BETTER</i>	7.1.2 Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	<input type="checkbox"/>
<b>7.2 Personalized trip planning</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	7.2.1 Offer personalized trip planning to new/relocating employees	<input type="checkbox"/>
<b>7.3 Promotions</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	7.3.1 Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	<input type="checkbox"/>
<b>8. OTHER INCENTIVES &amp; AMENITIES</b>		
<b>8.1 Emergency ride home</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	8.1.1 Provide emergency ride home service to non-driving commuters	<input type="checkbox"/>
<b>8.2 Alternative work arrangements</b>		
<i>Commuter travel</i>		
<i>BASIC</i>	8.2.1 Encourage flexible work hours	<input type="checkbox"/>
<i>BETTER</i>	8.2.2 Encourage compressed workweeks	<input type="checkbox"/>
<i>BETTER</i>	8.2.3 Encourage telework	<input type="checkbox"/>
<b>8.3 Local business travel options</b>		
<i>Commuter travel</i>		
<i>BASIC</i>	8.3.1 Provide local business travel options that minimize the need for employees to bring a personal car to work	<input type="checkbox"/>
<b>8.4 Commuter incentives</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	8.4.1 Offer employees a taxable, mode-neutral commuting allowance	<input type="checkbox"/>
<b>8.5 On-site amenities</b>		
<i>Commuter travel</i>		
<i>BETTER</i>	8.5.1 Provide on-site amenities/services to minimize mid-day or mid-commute errands	<input type="checkbox"/>

**TDM Measures Checklist:**  
*Residential Developments (multi-family, condominium or subdivision)*

**Legend**

**BASIC** The measure is generally feasible and effective, and in most cases would benefit the development and its users

**BETTER** The measure could maximize support for users of sustainable modes, and optimize development performance

**★** The measure is one of the most dependably effective tools to encourage the use of sustainable modes

TDM measures: Residential developments		Check if proposed & add descriptions
<b>1. TDM PROGRAM MANAGEMENT</b>		
<b>1.1 Program coordinator</b>		
BASIC ★	1.1.1 Designate an internal coordinator, or contract with an external coordinator	<input type="checkbox"/>
<b>1.2 Travel surveys</b>		
BETTER	1.2.1 Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	<input type="checkbox"/>
<b>2. WALKING AND CYCLING</b>		
<b>2.1 Information on walking/cycling routes &amp; destinations</b>		
BASIC	2.1.1 Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	<input type="checkbox"/>
<b>2.2 Bicycle skills training</b>		
BETTER	2.2.1 Offer on-site cycling courses for residents, or subsidize off-site courses	<input type="checkbox"/>

TDM measures: Residential developments		Check if proposed & add descriptions
<b>3. TRANSIT</b>		
<b>3.1 Transit information</b>		
BASIC	3.1.1 Display relevant transit schedules and route maps at entrances (multi-family, condominium)	<input type="checkbox"/>
BETTER	3.1.2 Provide real-time arrival information display at entrances (multi-family, condominium)	<input type="checkbox"/>
<b>3.2 Transit fare incentives</b>		
BASIC ★	3.2.1 Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	<input checked="" type="checkbox"/>
BETTER	3.2.2 Offer at least one year of free monthly transit passes on residence purchase/move-in	<input checked="" type="checkbox"/>
<b>3.3 Enhanced public transit service</b>		
BETTER ★	3.3.1 Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	<input type="checkbox"/>
<b>3.4 Private transit service</b>		
BETTER	3.4.1 Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	<input type="checkbox"/>
<b>4. CARSHARING &amp; BIKESHARING</b>		
<b>4.1 Bikeshare stations &amp; memberships</b>		
BETTER	4.1.1 Contract with provider to install on-site bikeshare station (multi-family)	<input type="checkbox"/>
BETTER	4.1.2 Provide residents with bikeshare memberships, either free or subsidized (multi-family)	<input type="checkbox"/>
<b>4.2 Carshare vehicles &amp; memberships</b>		
BETTER	4.2.1 Contract with provider to install on-site carshare vehicles and promote their use by residents	<input type="checkbox"/>
BETTER	4.2.2 Provide residents with carshare memberships, either free or subsidized	<input type="checkbox"/>
<b>5. PARKING</b>		
<b>5.1 Priced parking</b>		
BASIC ★	5.1.1 Unbundle parking cost from purchase price (condominium)	<input checked="" type="checkbox"/>
BASIC ★	5.1.2 Unbundle parking cost from monthly rent (multi-family)	<input checked="" type="checkbox"/>

TDM measures: Residential developments		Check if proposed & add descriptions
<b>6. TDM MARKETING &amp; COMMUNICATIONS</b>		
<b>6.1 Multimodal travel information</b>		
BASIC ★	6.1.1 Provide a multimodal travel option information package to new residents	<input checked="" type="checkbox"/>
<b>6.2 Personalized trip planning</b>		
BETTER ★	6.2.1 Offer personalized trip planning to new residents	<input type="checkbox"/>

**TDM-Supportive Development Design and Infrastructure Checklist:  
Non-Residential Developments (office, institutional, retail or industrial)**

Legend	
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
<b>1. WALKING &amp; CYCLING: ROUTES</b>		
<b>1.1 Building location &amp; access points</b>		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	<input checked="" type="checkbox"/>
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	<input checked="" type="checkbox"/>
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	<input checked="" type="checkbox"/>
<b>1.2 Facilities for walking &amp; cycling</b>		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see <i>Official Plan policy 4.3.3</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see <i>Official Plan policy 4.3.12</i> )	<input checked="" type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see <i>Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see <i>Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see <i>Official Plan policy 4.3.11</i> )	<input checked="" type="checkbox"/>
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	<input checked="" type="checkbox"/>
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	<input type="checkbox"/>
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	<input type="checkbox"/>
<b>1.3 Amenities for walking &amp; cycling</b>		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	<input type="checkbox"/>
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
<b>2. WALKING &amp; CYCLING: END-OF-TRIP FACILITIES</b>		
<b>2.1 Bicycle parking</b>		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions, that no more than 50% of spaces are vertical spaces, and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	<input type="checkbox"/>
BETTER	2.1.5 Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	<input type="checkbox"/>
<b>2.2 Secure bicycle parking</b>		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i> )	<input type="checkbox"/>
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	<input type="checkbox"/>
<b>2.3 Shower &amp; change facilities</b>		
BASIC	2.3.1 Provide shower and change facilities for the use of active commuters	<input type="checkbox"/>
BETTER	2.3.2 In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	<input type="checkbox"/>
<b>2.4 Bicycle repair station</b>		
BETTER	2.4.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
<b>3. TRANSIT</b>		
<b>3.1 Customer amenities</b>		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	<input type="checkbox"/>
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	<input type="checkbox"/>
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	<input type="checkbox"/>
<b>4. RIDESHARING</b>		
<b>4.1 Pick-up &amp; drop-off facilities</b>		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	<input type="checkbox"/>
<b>4.2 Carpool parking</b>		
BASIC	4.2.1 Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	<input type="checkbox"/>
BETTER	4.2.2 At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	<input type="checkbox"/>
<b>5. CARSHARING &amp; BIKESHARING</b>		
<b>5.1 Carshare parking spaces</b>		
BETTER	5.1.1 Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (see <i>Zoning By-law Section 94</i> )	<input type="checkbox"/>
<b>5.2 Bikeshare station location</b>		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
<b>6. PARKING</b>		
<b>6.1 Number of parking spaces</b>		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	<input checked="" type="checkbox"/>
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	<input type="checkbox"/>
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see <i>Zoning By-law Section 704</i> )	<input checked="" type="checkbox"/>
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see <i>Zoning By-law Section 111</i> )	<input type="checkbox"/>
<b>6.2 Separate long-term &amp; short-term parking areas</b>		
BETTER	6.2.1 Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	<input type="checkbox"/>
<b>7. OTHER</b>		
<b>7.1 On-site amenities to minimize off-site trips</b>		
BETTER	7.1.1 Provide on-site amenities to minimize mid-day or mid-commute errands	<input type="checkbox"/>

**TDM-Supportive Development Design and Infrastructure Checklist:  
Residential Developments (multi-family or condominium)**

**Legend**

**REQUIRED** The Official Plan or Zoning By-law provides related guidance that must be followed

**BASIC** The measure is generally feasible and effective, and in most cases would benefit the development and its users

**BETTER** The measure could maximize support for users of sustainable modes, and optimize development performance

TDM-supportive design & infrastructure measures: Residential developments		Check if completed & add descriptions, explanations or plan/drawing references
<b>1. WALKING &amp; CYCLING: ROUTES</b>		
<b>1.1 Building location &amp; access points</b>		
BASIC	1.1.1 Locate building close to the street, and do not locate parking areas between the street and building entrances	<input checked="" type="checkbox"/>
BASIC	1.1.2 Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	<input checked="" type="checkbox"/>
BASIC	1.1.3 Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	<input checked="" type="checkbox"/>
<b>1.2 Facilities for walking &amp; cycling</b>		
REQUIRED	1.2.1 Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see <i>Official Plan policy 4.3.3</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.2 Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings; between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see <i>Official Plan policy 4.3.12</i> )	<input checked="" type="checkbox"/>

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3 Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see <i>Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.4 Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see <i>Official Plan policy 4.3.10</i> )	<input checked="" type="checkbox"/>
REQUIRED	1.2.5 Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on-road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see <i>Official Plan policy 4.3.11</i> )	<input checked="" type="checkbox"/>
BASIC	1.2.6 Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	<input checked="" type="checkbox"/>
BASIC	1.2.7 Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	<input type="checkbox"/>
BASIC	1.2.8 Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	<input type="checkbox"/>
<b>1.3 Amenities for walking &amp; cycling</b>		
BASIC	1.3.1 Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	<input type="checkbox"/>
BASIC	1.3.2 Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
<b>2. WALKING &amp; CYCLING: END-OF-TRIP FACILITIES</b>		
<b>2.1 Bicycle parking</b>		
REQUIRED	2.1.1 Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see <i>Official Plan policy 4.3.6</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.2 Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
REQUIRED	2.1.3 Ensure that bicycle parking spaces and access aisles meet minimum dimensions, that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see <i>Zoning By-law Section 111</i> )	<input checked="" type="checkbox"/>
BASIC	2.1.4 Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	<input type="checkbox"/>
<b>2.2 Secure bicycle parking</b>		
REQUIRED	2.2.1 Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see <i>Zoning By-law Section 111</i> )	<input type="checkbox"/>
BETTER	2.2.2 Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multi-family residential developments	<input type="checkbox"/>
<b>2.3 Bicycle repair station</b>		
BETTER	2.3.1 Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	<input type="checkbox"/>
<b>3. TRANSIT</b>		
<b>3.1 Customer amenities</b>		
BASIC	3.1.1 Provide shelters, lighting and benches at any on-site transit stops	<input type="checkbox"/>
BASIC	3.1.2 Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	<input type="checkbox"/>
BETTER	3.1.3 Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	<input type="checkbox"/>

TDM-supportive design & infrastructure measures: <i>Residential developments</i>		Check if completed & add descriptions, explanations or plan/drawing references
<b>4. RIDESHARING</b>		
<b>4.1 Pick-up &amp; drop-off facilities</b>		
BASIC	4.1.1 Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	<input type="checkbox"/>
<b>5. CARSHARING &amp; BIKESHARING</b>		
<b>5.1 Carshare parking spaces</b>		
BETTER	5.1.1 Provide up to three carshare parking spaces in an R3, R4 or RS Zone for specified residential uses (see <i>Zoning By-law Section 94</i> )	<input type="checkbox"/>
<b>5.2 Bikeshare station location</b>		
BETTER	5.2.1 Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	<input type="checkbox"/>
<b>6. PARKING</b>		
<b>6.1 Number of parking spaces</b>		
REQUIRED	6.1.1 Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	<input checked="" type="checkbox"/>
BASIC	6.1.2 Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	<input type="checkbox"/>
BASIC	6.1.3 Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see <i>Zoning By-law Section 104</i> )	<input checked="" type="checkbox"/>
BETTER	6.1.4 Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see <i>Zoning By-law Section 111</i> )	<input type="checkbox"/>
<b>6.2 Separate long-term &amp; short-term parking areas</b>		
BETTER	6.2.1 Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	<input type="checkbox"/>