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# **39** Years

# Hillside Commons Residential Apartments 3277 St. Joseph Boulevard Ottawa, Ontario

**Transportation Impact Assessment** 

# Hillside Commons Residential Apartments 3277 St. Joseph Boulevard

**Transportation Impact Assessment** 

Prepared By:

NOVATECH Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario K2M 1P6

December 2021

Novatech File: 120237 Ref: R-2021-075



December 15, 2021

City of Ottawa Planning and Growth Management Department 110 Laurier Ave. W., 4<sup>th</sup> Floor, Ottawa, Ontario K1P 1J1

#### Attention: Ms. Neeti Paudel Project Manager, Infrastructure Approvals

Dear Ms. Paudel:

#### Reference: 3277 St. Joseph Boulevard Transportation Impact Assessment Novatech File No. 120237

We are pleased to submit the following Transportation Impact Assessment (TIA), in support of a Site Plan Control application at 3277 St. Joseph Boulevard, for your review and signoff. The structure and format of this report is in accordance with the City of Ottawa Transportation Impact Assessment Guidelines (June 2017).

If you have any questions or comments regarding this report, please feel free to contact Jennifer Luong, or the undersigned.

Yours truly,

NOVATECH

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Joshua Audia, B.Sc. E.I.T. | Transportation/Traffic

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# **TIA Plan Reports**

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

## CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- I am either a licensed<sup>1</sup> or registered<sup>2</sup> professional in good standing, whose field of expertise [check √ appropriate field(s)] is either transportation engineering or transportation planning □.

<sup>1,2</sup> License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

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Dated at	<u>Ottawa</u>	this _	<u>15th</u>	_ day of _	December	, 2021.
	(City)			-		

Name:

Jennifer Luong, P.Eng. (Please Print)

Professional Title:

Senior Project Manager, Transportation/Traffic\_\_\_\_\_

Geninger Lung

Signature of Individual certifier that s/he meets the above four criteria

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## EXECUTIVE SUMMARY

This Transportation Impact Assessment (TIA) has been prepared for the property located at 3277 St. Joseph Boulevard, in support of a Site Plan Control application. The subject site is approximately 1.2 acres in area, and is currently vacant.

The subject site is surrounded by the following:

- Residential uses, followed by Eric Czapnik Way and Ottawa Road 174 to the north,
- St. Joseph Boulevard, followed by residential uses to the south,
- Tenth Line Road, followed by an Ottawa Police Station to the east, and
- Residential uses, followed by Vieux-Silo Street to the west.

On Schedule B of the City of Ottawa's Official Plan, the subject site is designated as 'Mixed Use Centre.' The subject site is also located within the Orléans Town Centre Secondary Plan area. The implemented zoning for the property is the Residential Fifth-Density, Subzone Z (R5Z[1363] and R5Z[1415]), which permits the proposed land uses.

The proposed development will consist of two apartment buildings of nine-storeys each. The easterly building (referred to as 'Building A') will step down to five storeys at the north end to comply with the zoning requirements. It will include 172 dwellings, and the westerly building (referred to as 'Building B') will include 102 dwellings. In total, the proposed development will therefore include 274 dwellings, as well as 184 vehicle parking spaces within a multi-level parking garage.

Access to the proposed development will be provided via one driveway to St. Joseph Boulevard and one driveway to Lionel-Rhéo Private. Vehicles entering the site via one driveway will be able to exit via the other driveway, as the parking garages below Buildings A and B will connect. The proposed development is anticipated to be completed in a single phase, with a buildout year of 2024.

The study area for this report includes the boundary roadways St. Joseph Boulevard, Tenth Line Road, and Lionel-Rhéo Private, as well as the following intersections:

#### Signalized Intersections

- St. Joseph Boulevard/Tenth Line Road
- St. Joseph Boulevard/Old Tenth Line Road/ Ottawa Road 174 EB Off-Ramp

#### **Unsignalized Intersections**

- St. Joseph Boulevard/Vieux-Silo Street
- St. Joseph Boulevard/Eric Czapnik Way

The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2024 build-out year and the 2029 horizon year.

The conclusions and recommendations of the TIA can be summarized as follows:

#### **Forecasting**

• The proposed development is estimated to generate 111 person trips (including 58 vehicle trips) during the AM peak hour and 112 person trips (including 60 vehicle trips) during the PM peak hour.

#### <u>Development Design</u>

- New pedestrian walkways will connect the main and secondary entrances of Buildings A and B to the existing sidewalk on St. Joseph Boulevard. The main entrance of Building A is located at the southeastern corner of the subject site and the main entrance of Building B is located east of the proposed RIRO access, while the secondary entrances to both buildings will be accessed in the service easement between the two buildings, and are located approximately 35m north of St. Joseph Boulevard. New pedestrian walkways will also connect the main entrance of Building A and the northern end of the subject site to the existing sidewalk on Tenth Line Road.
- Bicycle parking will be provided on the upper parking garage level within Building B.
- Measuring from the main entrance, the bus stops within 400m are stops #1794, #6763, #7843, #7846, #8596, and #8761, which are served by OC Routes 33, 236, and 302.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.
- Pick-ups and drop-offs will be facilitated in a designated loop at the northwest corner of Building A, which will be accessed via Lionel-Rhéo Private. Access to the underground parking garage via Building A will be located directly adjacent to this pick-up/drop-off loop.
- Garbage collection and loading/deliveries will take place at the service entries to Buildings A and B, via the access along the service easement between the buildings. The fire route for the proposed development will be located curbside on St. Joseph Boulevard.

#### <u>Parking</u>

• The proposed number of vehicle parking spaces meets the minimum requirements, as outlined in the City's ZBL. The proposed number of bicycle parking spaces also meets the minimum requirements.

#### Boundary Streets

- The results of the segment multi-modal level of service (MMLOS) analysis can be summarized as follows:
  - Both St. Joseph Boulevard and Tenth Line Road do not meet the target pedestrian level of service (PLOS) C;
  - Both St. Joseph Boulevard and Tenth Line Road do not meet the target bicycle level of service (BLOS) C;
  - Tenth Line Road achieves the target transit level of service (TLOS) D;
  - Both St. Joseph Boulevard and Tenth Line Road meet the target truck level of service (TkLOS) D.
- Based on Exhibit 4 of the *MMLOS Guidelines*, the best possible PLOS for roadways with curb lane volumes greater than 3,000 vehicles per day and an operating speed greater than 60 km/h is a PLOS D. This can be achieved by providing sidewalks with a minimum width of 2.0m and a minimum boulevard width greater than 2.0m. This applies to both sides of St. Joseph Boulevard and the east side of Tenth Line Road, and is identified for the City's consideration. For the west side of Tenth Line Road, the target PLOS C can be achieved by providing a sidewalk with a minimum width of 2.0m and a minimum width of 2.0m and a minimum boulevard width of 0.5m. This is identified for the City's consideration.

• Based on Exhibit 11 of the *MMLOS Guidelines*, a physically separated bikeway (such as cycle tracks or multi-use pathways) are required to achieve the target BLOS A for St. Joseph Boulevard or BLOS C for Tenth Line Road, given the current operating speed of both roadways. This is identified for the City's consideration.

## Access Design

- The proposed access to St. Joseph Boulevard meets all required provisions of the *Private Approach By-Law* (PABL), except for Section 25(u). Measuring from the property line, the grade of the access is approximately 2% (descending toward the roadway) for the first 4m inside the property line, and 6% (descending toward the parking garage) for the next 5m. By limiting the maximum grade to 6% within the first 9m of the property line, it is anticipated that drivers exiting the parking garage will have adequate sightlines to pedestrians walking along St. Joseph Boulevard. Therefore, it is requested that the requirement of Section 25(u) of the PABL be waived.
- The proposed access to St. Joseph Boulevard does not meet the Transportation Association of Canada (TAC)'s clear throat length requirement of 40m, as the underground parking garage door is located within this distance. However, the potential for queueing back onto St. Joseph Boulevard is mitigated by the access being restricted to right-in/right-out, and there is another approximately 40m of clear throat before the first parking spaces within the parking garage. Queueing onto St. Joseph Boulevard is not anticipated.
- TAC's *Geometric Design Guide* identifies a minimum corner clearance requirement of 70m for arterial roadways, measuring between the private approach and the nearest edge of the roadway. While it is acknowledged that the proposed access to St. Joseph Boulevard does not meet this requirement, it is located as far from the intersection at St. Joseph Boulevard/ Tenth Line Road as possible.

## Transportation Demand Management

- The proponent has committed to providing the following TDM measures:
  - Display local area maps with walking/cycling access routes and key destinations at major entrances;
  - Display relevant transit schedules and route maps at entrances;
  - Unbundle parking cost from monthly rent.

## Neighbourhood Traffic Management

• Eric Czapnik Way exceeds the City's threshold for considering neighbourhood traffic management measures. Eric Czapnik Way is not anticipated to operate at or near capacity in the 2029 total traffic conditions. Further, the function of Eric Czapnik Way as a local roadway is not anticipated to change as a result of the proposed development, and no neighbourhood traffic management measures are required.

#### <u>Transit</u>

• The proposed development is projected to generate 30 transit trips during the AM peak hour and 29 transit trips during the PM peak hour. No capacity issues are anticipated for OC Transpo Routes 33, 236, or 302, based on the above transit trip estimates.

#### Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
  - Neither signalized intersection meets the target PLOS;
  - Neither signalized intersection meets the target BLOS;
  - St. Joseph Boulevard/Tenth Line Road does not meet the target TLOS;
  - Both signalized intersections meet the target TkLOS.
- All approaches of St. Joseph Boulevard/Tenth Line Road have a divided cross-section with a width equivalent to ten lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements. No approaches meet the City's vehicle/pedestrian conflict threshold for zebrastriped crosswalks. There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.
- All approaches of St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp have a divided cross-section with a width equivalent to eight to ten lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements further. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.
- All approaches of St. Joseph Boulevard/Tenth Line Road do not meet the target BLOS, based on both left and right turn characteristics. Achieving the target BLOS A would require physically-separated bikeways (such as cycle tracks or multi-use pathways) and off-road facilities for cyclists to turn left. Therefore, the implementation of a protected intersection would be required to meet the target BLOS A, and would involve the removal of all right turn channels. This is identified for the City's consideration.
- The south and east approaches of St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp do not meet the target BLOS based on left turn characteristics, and the south approach does not meet the target based on right turn characteristics. Achieving the BLOS C would require two-stage left-turn bike boxes for northbound and westbound cyclists. Implementing bike boxes at the north approach would not require right turn on red (RTOR) restrictions, and is therefore identified for the City's consideration. There is no through phase for northbound cyclists to get to a left-turn bike box, and northbound cyclists could use Tenth Line Road to turn left onto St. Joseph Boulevard instead. The south approach can achieve the BLOS C based on right turn characteristics by implementing a pocket bike lane across the channelized northbound right turn lane. This is identified for the City's consideration.
- The south and east approaches of St. Joseph Boulevard/Tenth Line Road do not meet the target TLOS D. The east approach does not have a target TLOS, but the approach delays of approximately 35 seconds during the AM peak hour is noted. The City's RTTP Affordable Network includes transit priority signals and queue jump lanes on Tenth Line Road, and would be expected to improve the delays for transit vehicles to the target TLOS D or better.

## Existing Intersection Operations

All study area intersections currently operate at an Auto LOS C or better during the AM and PM peak hours. For all auxiliary lanes at the study area intersections, the Synchro analysis does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection (i.e. on St. Joseph Boulevard, westbound queues at Tenth Line Road and eastbound queues at Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp do not extend through the intersection at Eric Czapnik Way).

## Background Intersection Operations

 All study area intersections in the 2029 background conditions are projected to operate at an Auto LOS D or better during the AM and PM peak hours. The Synchro analysis identifies that, in the AM peak hour, the 95<sup>th</sup>-percentile northbound queue length at St. Joseph Boulevard/Tenth Line Road is approximately 190m, which extends past the auxiliary northbound left turn lane. For all auxiliary lanes within the study area, Synchro does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection.

#### Total Intersection Operations

- The addition of site-generated traffic is anticipated to have little impact on the operations of the study area intersections. The proposed RIRO access to St. Joseph Boulevard is anticipated to operate at an Auto LOS A during the peak hours.
- Based on the foregoing, the proposed development is recommended from a transportation perspective.

#### 1.0 SCREENING

#### 1.1 Introduction

This Transportation Impact Assessment (TIA) has been prepared for the property located at 3277 St. Joseph Boulevard, in support of a Site Plan Control application. The subject site is approximately 1.2 acres in area, and is currently vacant.

The subject site is surrounded by the following:

- Residential uses, followed by Eric Czapnik Way and Ottawa Road 174 to the north,
- St. Joseph Boulevard, followed by residential uses to the south,
- Tenth Line Road, followed by an Ottawa Police Station to the east, and
- Residential uses, followed by Vieux-Silo Street to the west.

An aerial of the vicinity around the subject site is provided in Figure 1.

Figure 1: Site Location



Note: The above aerial does not show the recent construction of Vieux-Silo Street, Récolte Private, or Lionel-Rhéo Private. An approximate alignment of these roadways have been added to the figure.

## 1.2 Proposed Development

On Schedule B of the City of Ottawa's Official Plan, the subject site is designated as 'Mixed Use Centre.' The subject site is also located within the Orléans Town Centre Secondary Plan area. The implemented zoning for the property is the Residential Fifth-Density, Subzone Z (R5Z[1363] and R5Z[1415]), which permits the proposed land uses.

The proposed development will consist of two apartment buildings of nine-storeys each. The easterly building (referred to as 'Building A') will step down to five storeys at the north end to comply with the zoning requirements. It will include 172 dwellings, and the westerly building (referred to as 'Building B') will include 102 dwellings. In total, the proposed development will therefore include 274 dwellings, as well as 184 vehicle parking spaces within a multi-level parking garage.

Access to the proposed development will be provided via one driveway to St. Joseph Boulevard and one driveway to Lionel-Rhéo Private. Vehicles entering the site via one driveway will be able to exit via the other driveway, as the parking garages below Buildings A and B will connect. The proposed development is anticipated to be completed in a single phase, with a buildout year of 2024.

A copy of the proposed site plan is included in **Appendix A**.

## 1.3 Screening Form

The City's *2017 TIA Guidelines* identify three triggers for completing a TIA report, including trip generation, location, and safety. The criteria for each trigger are outlined in the City's TIA Screening Form, which is included in **Appendix B**. The trigger results are as follows:

- Trip Generation Trigger The development is expected to generate over 60 person trips/peak hour; further assessment **is required** based on this trigger.
- Location Triggers The development proposes new driveways to a Spine Cycling Route (St. Joseph Boulevard) and is located in a Design Priority Area (Orléans Town Centre); further assessment is required based on this trigger.
- Safety Triggers A proposed driveway within 150m of a signalized intersection, and there is a documented history of traffic operations/safety concerns on the boundary streets within 500m of the development; further assessment **is required** based on this trigger.

#### 2.0 SCOPING

## 2.1 Existing Conditions

#### 2.1.1 Roadways

All roadways within the study area fall under the jurisdiction of the City of Ottawa.

St. Joseph Boulevard is an arterial roadway that generally runs on an east-west alignment between Ottawa Road 174 and Trim Road. West of Ottawa Road 174, the roadway continues as Montreal Road, Rideau Street, and Wellington Street. East of Trim Road, the roadway continues as Old Montreal Road. Within the study area, St. Joseph Boulevard has a four-lane divided urban cross-section, asphalt sidewalks on both sides, and a posted speed limit of 60 km/h. St. Joseph Boulevard

is classified as a truck route, allowing full loads. On-street parking is not permitted. The City's Official Plan identifies a right-of-way (ROW) protection of 37.5m between 130m west of Duford Drive/Place d'Orléans Drive and Trim Road. A widening is not anticipated as part of this application.

Tenth Line Road is an arterial roadway that runs on a north-south alignment between Jeanne d'Arc Boulevard North and Smith Road. The roadway has a four-lane divided urban cross-section, concrete sidewalks on both sides, and a posted speed limit of 60 km/h. Tenth Line Road is classified as a truck route, allowing full loads. On-street parking is not permitted. The City's Official Plan does not identify a ROW protection on Tenth Line Road north of Tompkins Avenue. Therefore, a widening is not required as part of this application.

Old Tenth Line Road is an arterial roadway that generally runs on a north-south alignment between St. Joseph Boulevard and Tenth Line Road. The roadway has a three-lane undivided urban cross-section, concrete sidewalks on both sides, and a posted speed limit of 60 km/h. Old Tenth Line Road is not classified as a truck route. On-street parking is not permitted.

Ottawa Road 174 is a City-owned freeway that generally runs on an east-west alignment between Highway 417 and 600m east of Trim Road, and continues as an arterial roadway from 600m east of Trim Road to Canaan Road. The roadway then continues as Highway 17 east of Canaan Road. Within the study area, the roadway has a four- or six-lane divided rural cross-section, no sidewalks, and a posted speed limit of 100 km/h. Ottawa Road 174 is classified as a truck route, allowing full loads. On-street parking is not permitted.

Vieux-Silo Street is a local roadway that runs on a north-south alignment between St. Joseph Boulevard and Eric Czapnik Way. The roadway has a two-lane undivided urban cross-section, a concrete sidewalk on the west side, and a regulatory speed limit of 50 km/h per the Highway Traffic Act. Vieux-Silo Street is not classified as a truck route.

Eric Czapnik Way is a local roadway that generally runs on an east-west alignment starting at Vieux-Silo Street before curving to a north-south alignment and terminating at St. Joseph Boulevard between Tenth Line Road and Old Tenth Line Road. The roadway has a two-lane undivided urban cross-section, concrete sidewalks on the south side between Vieux-Silo Street and 205 Eric Czapnik Way and on the east side along the entire frontage of 3363 St. Joseph Boulevard, and an unposted speed limit of 50 km/h. Eric Czapnik Way is not classified as a truck route. On-street parking is permitted.

Récolte Private is a local roadway that generally runs on an east-west alignment starting at Vieux-Silo Street before curving to a north-south alignment and terminating at Eric Czapnik Way. The roadway has a two-lane undivided urban cross-section, concrete sidewalks on the south side of the roadway between Vieux-Silo Street and Lionel-Rhéo Private, and a regulatory speed limit of 50 km/h. Récolte Private is not classified as a truck route. On-street parking is permitted.

Lionel-Rhéo Private is a private roadway that runs on an east-west alignment for approximately 80m east of Récolte Private. The roadway has a two-lane undivided urban cross-section, and asphalt sidewalks on the south side. Lionel-Rhéo Private is not classified as a truck route. Perpendicular parking is provided on the north side of the roadway.

The roadway network of the greater area surrounding the subject site is illustrated in Figure 2.

## Figure 2: Roadway Network



## 2.1.2 Intersections

## St. Joseph Boulevard/Vieux-Silo Street

- Unsignalized three-legged intersection
- Southbound Approach (Vieux-Silo Street): one shared left turn/right turn lane
- Eastbound Approach (St. Joseph Boulevard): one left turn lane and two through lanes
- Westbound Approach (St. Joseph Boulevard): one through lane and one shared through/right turn lane
- Standard crosswalk is provided at southbound approach



## St. Joseph Boulevard/Tenth Line Road

- Signalized four-legged intersection
- Northbound Approach (Tenth Line Road): one left turn lane, one shared left turn/through lane, one through lane, and one channelized right turn lane
- Southbound Approach (Tenth Line Road): one left turn lane, two through lanes, and one channelized right turn lane
- Eastbound Approach (St. Joseph Boulevard): one left turn lane, two through lanes, and one channelized right turn lane
- Westbound Approach (St. Joseph Boulevard): one left turn lane, two through lanes, and one channelized right turn lane
- Standard crosswalks are provided on all approaches
- Pocket bike lane provided at westbound approach

## St. Joseph Boulevard/Eric Czapnik Way

- Unsignalized three-legged intersection
- Southbound Approach (Eric Czapnik Way): one shared left turn/right turn lane
- Eastbound Approach (St. Joseph Boulevard): one left turn lane and two through lanes
- Westbound Approach (St. Joseph Boulevard): two through lanes and one shared through/right turn lane





#### <u>St. Joseph Boulevard/Old Tenth Line Road/</u> Ottawa Road 174 EB Off-Ramp

- Signalized four-legged intersection
- Northbound Approach (Old Tenth Line Road): one left turn lane and one channelized right turn lane
- Southbound Approach (OR 174 Off-Ramp): two left turn lanes, two through lanes, and one channelized right turn lane
- Eastbound Approach (St. Joseph Boulevard): one through lane and one shared through/right turn lane
- Westbound Approach (St. Joseph Boulevard): one left turn lane and two through lanes
- Standard crosswalks are provided at northbound, southbound, and eastbound approaches



## 2.1.3 Driveways

The City's *2017 TIA Guidelines* requires a review of existing driveways on the boundary streets within 200m of any proposed accesses. This can be summarized as follows:

## St. Joseph Boulevard, North Side

• Four driveways to residences at 3245, 3251, 3259, and 3265 St. Joseph Boulevard

## Récolte Private, North Side

• Eighteen driveways to residences at 524-558 Récolte Private

## 2.1.4 Pedestrian and Cycling Facilities

## St. Joseph Boulevard, South Side

No driveways

## Récolte Private, West Side

• Eight driveways to residences at 500-514 Récolte Private

Sidewalks are provided on both sides of St. Joseph Boulevard, Tenth Line Road, and Old Tenth Line Road, and on one side of Vieux-Silo Street, Eric Czapnik Way, Récolte Private, and Lionel-Rhéo Private in select locations.

In the City of Ottawa's primary cycling network, St. Joseph Boulevard and Tenth Line Road north of St. Joseph Boulevard are classified as Spine Cycling Routes, and Tenth Line Road south of St. Joseph Boulevard is classified as a Major Pathway. Crosstown Bikeway Route #9 runs through the study area, and includes St. Joseph Boulevard between Notre Dame Street and Tenth Line Road, and Tenth Line Road between St. Joseph Boulevard and Innes Road. A pocket bike lane is provided on westbound St. Joseph Boulevard between Tenth Line Road and Eric Czapnik Way.

## 2.1.5 Area Traffic Management

There are no Area Traffic Management (ATM) studies within the study area that have been completed or are currently in progress.

## 2.1.6 Transit

The nearest transit stops to the subject site are as follows:

St. Joseph/Ad. 3227

 Stop #7846 – for routes 33 and 302 (located on south side of St. Joseph Boulevard, approximately opposite Vieux-Silo Street)

St. Joseph/Ad. 3245

 Stop #1794 – for routes 33 and 302 (located at northeast corner of St. Joseph Boulevard/Vieux-Silo Street)

St. Joseph/Tenth Line Road

- Stop #6763 for routes 33, 233, 235, 236, and 302 (located on west side of Tenth Line Road, approximately 125m south of St. Joseph Boulevard)
- Stop #7843 for routes 33 and 302 (located on north side of St. Joseph Boulevard, approximately 75m west of Tenth Line Road)
- Stop #8596 for routes 33, 233, 235, and 236 (located on east side of Tenth Line Road, approximately 50m north of St. Joseph Boulevard)
- Stop #8761 for routes 33, 233, 235, and 236 (located on north side of St. Joseph Boulevard, approximately 200m east of Tenth Line Road)

OC Transpo Route 33 is a local route, travelling between Blair LRT Station and Portobello/Summer Sky. The route operates every 15 to 30 minutes from 5:30am to 7:00pm, with select trips after 7:00pm. Route 33 operates on weekdays only.

OC Transpo Route 233 is a connexion route, travelling between Blair LRT Station and Portobello/ Summer Sky. The route operates in the peak direction every 20 to 30 minutes from 6:30am to 7:30am, and every 30 to 40 minutes from 4:30pm to 5:30pm. Route 233 operates on weekdays only. As of June 20, 2021, Route 233 has been suspended, as other routes serve the study area.

OC Transpo Route 235 is a connexion route, travelling between Blair LRT Station and Gardenway/ Portobello. The route operates in the peak direction every 15 minutes from 6:00am to 9:00am, and 15 minutes from 3:30pm to 6:30pm. Route 235 operates on weekdays only. As of June 20, 2021, Route 235 has been suspended, as other routes serve the study area.

OC Transpo Route 236 is a connexion route, travelling between Blair LRT Station and Lakeridge/ Vista Park. The route operates in the peak direction every 15 minutes from 5:30am to 9:00am, and every 15 minutes from 3:30pm to 6:30pm. Route 236 operates on weekdays only.

OC Transpo Route 302 is a free shopping route for residents of rural communities, travelling between St. Laurent LRT Station and the communities of Cumberland, Sarsfield, and Navan. The route is scheduled to arrive in the study area at 9:41am (toward St. Laurent and Gloucester Shopping Centres) and 2:49pm (toward the communities). Route 302 operates on Tuesdays only.

Locations of the transit stops described above are shown in **Figure 3**. OC Transpo maps for the routes outlined above and a copy of the system map is included in **Appendix C**.



Figure 3: OC Transpo Bus Stop Locations

## 2.1.7 Existing Traffic Volumes

Weekday traffic counts completed by the City of Ottawa were used to determine the existing pedestrian, cyclist, and vehicular traffic volumes at study area intersections where data is available. These counts were completed on the dates listed below:

•	St. Joseph Boulevard/Tenth Line Road	Mar 20, 2018
•	St. Joseph Boulevard/Eric Czapnik Way	Apr 23, 2015
•	St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp	Jan 25, 2018

Eastbound/westbound through volumes at St. Joseph Boulevard/Vieux-Silo Street and St. Joseph Boulevard/Eric Czapnik Way have been carried from the observed eastbound/westbound volumes at St. Joseph Boulevard/Tenth Line Road. Turning movement volumes onto or from Vieux-Silo Street have been estimated using projections from the previously completed Hillside Vista traffic studies, which are referenced in Section 2.2.2. Turning movement volumes onto or from Eric Czapnik Way have been estimated by adding the observed 2015 volumes and projections from the previously completed Hillside Vista traffic studies.

Weekday AM and PM peak hour traffic volumes at the study area intersections are shown in **Figure 4**. Peak hour summary sheets of the above traffic counts are included in **Appendix D**.





## 2.1.8 Collision Records

Historical collision data from the last five years were obtained from the City's Public Works and Service Department for the study area intersections and road segments between intersections. Copies of the collision summary reports are included in **Appendix E**.

The collision data have been evaluated to identify collision patterns, defined in the *2017 TIA Guidelines* as 'more than six collisions in five years' for any one movement. The number of collisions at each intersection from January 1, 2015 to December 31, 2019 is summarized in **Table 1**. During the five-year period, there were no reported fatal collisions in the analyzed area.

#### Table 1: Reported Collisions

Intersection/		_				
Street Segment	Angle	Rear End	Sideswipe	SMV <sup>(1)</sup> / Other	Turning Movement	Total
St. Joseph Boulevard/ Tenth Line Road	10	64	12	7	4	97
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp	10	5	1	6	1	23
St. Joseph Boulevard between Prestone Drive and Tenth Line Road	-	-	3	3	-	6
St. Joseph Boulevard between Tenth Line Road and Old Tenth Line Road	-	-	-	3	-	3

1. SMV = Single Motor Vehicle

## St. Joseph Boulevard/Tenth Line Road

A total of 97 collisions were reported at this intersection over the last five years, of which there were ten angle impacts, 64 rear-end impacts, 12 sideswipe impacts, seven single vehicle/other impacts, and four turning movement impacts. Seventeen collisions resulted in injuries. Thirty-nine of the 97 collisions occurred in poor driving conditions.

Of the ten angle impacts, one involved a northbound vehicle and an eastbound vehicle, five involved a northbound vehicle and a westbound vehicle, and four involved a southbound vehicle and an eastbound vehicle.

Of the 64 rear-end impacts, 50 involved northbound vehicles, one involved southbound vehicles, ten involved eastbound vehicles, and three involved westbound vehicles. For the northbound and eastbound approaches, high traffic volumes and downhill grades may play a factor in these collisions. It should be noted that a red light camera is installed for eastbound vehicles on St. Joseph Boulevard at this intersection.

Of the 12 sideswipe impacts, seven involved northbound vehicles, three involved southbound vehicles, one involved eastbound vehicles, and one involved westbound vehicles. High traffic volumes are likely a factor in these collisions.

Of the seven single vehicle/other impacts, three involved a northbound vehicle, one involved a southbound vehicle, and three involved an eastbound vehicle.

#### St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp

A total of 23 collisions were reported at this intersection over the last five years, of which there were ten angle impacts, five rear-end impacts, one sideswipe impact, six single vehicle/other impacts, and one turning movement impact. Seven collisions resulted in injuries. Twelve of the 23 collisions occurred in poor driving conditions.

Of the ten angle impacts, one involved a northbound vehicle and an eastbound vehicle, seven involved a southbound vehicle and an eastbound vehicle, and two involved a southbound vehicle and a westbound vehicle. High traffic volumes exiting Ottawa Road 174 are likely a factor in these collisions, which typically involved one driver failing to observe the traffic signal. Additionally, a red light camera has been installed for eastbound vehicles on St. Joseph Boulevard at this intersection.

## St. Joseph Boulevard between Prestone Drive and Tenth Line Road

In this road segment overall, a total of six collisions have been identified in the last five years, consisting of three sideswipe impacts and three single vehicle impacts. One of the six collisions resulted in injuries. Two of the six collisions occurred in poor driving conditions.

## St. Joseph Boulevard between Tenth Line Road and Old Tenth Line Road

No collisions were identified at St. Joseph Boulevard/Eric Czapnik Way, which is located midblock between Tenth Line Road and Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp. In this road segment overall, a total of three collisions have been identified in the last five years, all involving a single vehicle. No collisions resulted in injuries, and one collision occurred in poor driving conditions.

## 2.2 Planned Conditions

## 2.2.1 Planned Transportation Projects

The City's 2013 Transportation Master Plan (TMP) identifies multiple Rapid Transit or Transit Priority (RTTP) projects in proximity of the subject site. Consistent with the 2031 Network Concept, extension of the Confederation Line LRT from Blair Station to Trim Station is currently under construction, and revenue service is planned for 2024. Additionally, the 2031 Affordable Network and 2031 Network Concept identifies transit signal priority and queue jump lanes on Tenth Line Road between Ottawa Road 174 and Charlemagne Boulevard.

The City's 2013 TMP does not identify any roadway projects within the study area in its Affordable Road Network. The 2013 TMP identifies a widening of Ottawa Road 174 in its 2031 Network Concept, which would include widening the roadway from four lanes to six lanes between Highway 417 and Trim Road, and widening the roadway from two lanes to four lanes between Trim Road and the urban boundary.

The City's 2013 Pedestrian Plan does not identify any new pedestrian infrastructure projects within the study area.

The City's 2013 Cycling Plan includes the implementation of a multi-use pathway on Tenth Line Road between St. Joseph Boulevard and Innes Road, as a Phase 3 (2026-2031) project.

## 2.2.2 Other Area Developments

In proximity of the proposed development, there are multiple other developments that are approved or are in the approval process, and are listed below. Relevant excerpts of the traffic studies in support of these developments are included in **Appendix F**.

## <u>Hillside Vista</u>

The Hillside Vista developments consist of multiple phases, some of which have been completed. The subject site is adjacent to the Hillside Vista developments. A total of 44 townhome dwellings have been built (considered 'Phase 1A'), and a total of 90 walk-up condominium dwellings are proposed as part of 'Phase 1B.' It is assumed that buildout of Phase 1B will be completed by the time the subject site is complete (i.e. the buildout year of 2024). Phase 1B is approved and now under construction.

# 211 Centrum Boulevard

This development is planned to include 17-storey and 9-storey buildings, consisting of a total of 397 retirement home units. The anticipated buildout year for this development is 2024. At the time of writing, the Official Plan Amendment, Zoning By-Law Amendment, and Site Plan Control applications associated with this development are in the review process.

## 3030 St. Joseph Boulevard

This development is planned to include a 16-storey building, consisting of 165 residential units and 426 m<sup>2</sup> of ground floor retail space. The development is anticipated to be built out by 2024. At the time of writing, the Zoning By-Law Amendment application associated with this development has been recommended for approval.

## Petrie's Landing (8466, 8600, 8700, and 8900 Jeanne d'Arc Boulevard)

This development is divided into three phases, and can be summarized as follows:

- Petrie's Landing I: 806 dwellings and 1,500 m<sup>2</sup> of commercial space;
- Petrie's Landing II: 113 dwellings;
- Petrie's Landing III: 790 dwellings, 23,000 ft<sup>2</sup> of retail space, and 370,000 ft<sup>2</sup> of office space.

The first two phases are anticipated to be built out by 2024, and the third phase is assumed to be 50% built out in 2024, and fully built out by 2027. At the time of writing, the Site Plan Control and Zoning By-Law Amendment applications associated with the second phase of the development are in the review process.

## 2.3 Study Area and Time Periods

The study area for this report includes the boundary roadways St. Joseph Boulevard, Tenth Line Road, and Lionel-Rhéo Private, as well as the following intersections:

## **Signalized Intersections**

- St. Joseph Boulevard/Tenth Line Road
- St. Joseph Boulevard/Old Tenth Line Road/ Ottawa Road 174 EB Off-Ramp

#### **Unsignalized Intersections**

- St. Joseph Boulevard/Vieux-Silo Street
- St. Joseph Boulevard/Eric Czapnik Way

Per discussions with City staff, the intersection at St. Joseph Boulevard/Place d'Orléans Drive/ Duford Drive has not been considered in the study area, as the number of site-generated trips that would travel through this intersection to access Ottawa Road 174 is anticipated to be minimal during the peak hours. Further discussion of the trip distribution and assignment assumptions are included in Sections 3.1.2 and 3.1.3.

The selected time periods for the analysis are the weekday AM and PM peak hours, as they represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2024 build-out year and the 2029 horizon year.

## 2.4 Exemptions Review

This module reviews possible exemptions from the final Transportation Impact Assessment, as outlined in the *2017 TIA Guidelines*. The applicable exemptions for this site are shown in **Table 2**.

Module	Element	Exemption Criteria	Status
<b>Design Review</b>	Component		
<b>4.1</b> Development Design	<i>4.1.2</i> Circulation and Access	Only required for site plans	Not Exempt
	<i>4.1.3</i> New Street Networks	<ul> <li>Only required for plans of subdivision</li> </ul>	Exempt
4.2	<i>4.2.1</i> Parking Supply	<ul> <li>Only required for site plans</li> </ul>	Not Exempt
Parking	<i>4.2.2</i> Spillover Parking	<ul> <li>Only required for site plans where parking supply is 15% below unconstrained demand</li> </ul>	Exempt
Network Impact	Component		
<b>4.5</b> Transportation Demand Management	All elements	<ul> <li>Not required for non-residential site plans expected to have fewer than 60 employees and/or students on location at any given time</li> </ul>	Not Exempt
<b>4.6</b> Neighbourhood Traffic Management	<i>4.6.1</i> Adjacent Neighbourhoods	<ul> <li>Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds</li> </ul>	Not Exempt
<b>4.8</b> Network Concept	All elements	<ul> <li>Only required when proposed development generates more than 200 person-trips during the peak hour in excess of the equivalent volume permitted by the established zoning</li> </ul>	Exempt

## Table 2: TIA Exemptions

Based on the foregoing, the following modules will be included in the TIA report:

## **Design Review Component**

- Module 4.1: Development Design
- Module 4.2: Parking
- Module 4.3: Boundary Streets
- Module 4.4: Access Design

## Network Impact Component

- Module 4.5: Transportation Demand Management
- Module 4.6: Neighbourhood Traffic Management
- Module 4.7: Transit
- Module 4.9: Intersection Design

## 3.0 FORECASTING

## 3.1 Development-Generated Travel Demand

## 3.1.1 Trip Generation

The proposed development will include a total of 274 mid-rise apartment dwellings. The *TRANS Trip Generation Manual Summary Report*, prepared in October 2020 by WSP, includes data to estimate the trip generation and mode shares for residential uses, divided into single-family detached housing, low-rise multifamily housing (one or two storeys), and high-rise multifamily housing (three or more storeys). Relevant excerpts of the *TRANS Trip Generation Manual* are included in **Appendix G**.

The *TRANS Trip Generation Manual* identifies the subject site as being located within the Orléans district, which has the following observed mode shares during the peak hours:

#### AM Peak Hour

- Auto Driver: 54%
- Auto Passenger: 7%
- Transit: 29%
- Cyclist: 0%
- Pedestrian: 10%

## PM Peak Hour

- Auto Driver: 60%
- Auto Passenger: 13%
- Transit: 21%
- Cyclist: 0%
- Pedestrian: 6%

For the proposed development, one set of mode shares have been assumed for both peak hours, based on the above shares (i.e. 55% driver, 10% passenger, 25% transit, 10% pedestrian).

For the High-Rise Multifamily Housing land use, the process of converting the trip generation estimates from peak period to peak hour is shown in the following tables. The estimated number of person trips generated by the proposed dwellings for the AM and PM peak periods are shown in **Table 3**. A breakdown of these trips by modal share is shown in **Table 4**.

#### Table 3: Proposed Residential – Peak Period Trip Generation

	TRANS	llaite	AM Pea	ak Period	(ppp <sup>(1)</sup> )	PM Peak Period (ppp)			
Land Use	Rate	Units	IN	OUT	тот	IN	OUT	тот	
High-Rise Multifamily Hous	AM: 0.80 sing PM: 0.90	274	68	151	219	143	104	247	

1. ppp: Person Trips per Peak Period

#### Table 4: Proposed Residential – Peak Period Trips by Mode Share

Travel Mode	Mode Share	A	I Peak Peri	od	PM Peak Period			
	Mode Share	IN	OUT	тот	IN	OUT	ТОТ	
Peak Period	68	151	219	143	104	247		
Auto Driver	55%	37	83	120	79	57	136	
Auto Passenger	10%	7	15	22	14	11	25	
Transit	25%	17	38	55	36	26	62	
Cyclist	0%	0	0	0	0	0	0	
Pedestrian	10%	7	15	22	14	10	24	

Table 4 of the *TRANS Trip Generation Manual* includes adjustment factors to convert the estimated number of trips generated for each mode from peak period to peak hour. A breakdown of the peak hour trips by mode is shown in **Table 5**.

#### Table 5: Proposed Residential – Peak Hour Trips by Mode Share

Travel Mode	Adj. Factor		Α	M Peak Ho	ur	PM Peak Hour			
	AM	PM	IN	OUT	тот	IN	OUT	тот	
Auto Driver	0.48	0.44	18	40	58	35	25	60	
Auto Passenger	0.48	0.44	3	7	10	6	5	11	
Transit	0.55	0.47	9	21	30	17	12	29	
Cyclist	0.58	0.48	0	0	0	0	0	0	
Pedestrian	0.58	0.52	4	9	13	7	5	12	
Peak Hour Person Trips			34	77	111	65	47	112	

From the previous table, the proposed high-rise dwellings are estimated to generate 111 person trips (including 58 vehicle trips) during the AM peak hour and 112 person trips (including 60 vehicle trips) during the PM peak hour.

## 3.1.2 Trip Distribution

The assumed distribution of trips generated by the proposed development have been derived from the initial trip distribution assumptions of the *Orléans Town Centre (OTC) West Transportation Impact Study*, prepared by Novatech in July 2007, and existing traffic patterns. The assumed distribution of site-generated trips can be described as follows:

- 5% to/from the north via Tenth Line Road;
- 25% to/from the south via Tenth Line Road;
- 10% to/from the east via St. Joseph Boulevard;
- 15% to/from the west via St. Joseph Boulevard;
- 45% to/from the west via Ottawa Road 174.

Trips arriving from the west are anticipated to enter the study area via St. Joseph Boulevard or the Ottawa Road 174 EB Off-Ramp. All trips departing to the west via Ottawa Road 174 have been equally distributed to the on-ramps at Tenth Line Road and Champlain Street (via Place d'Orléans Drive, west of the study area). Relative to the study area, this adds another 22.5% of outbound trips to the north via Tenth Line Road and 22.5% of outbound trips to the west via St. Joseph Boulevard.

#### 3.1.3 Trip Assignment

It is anticipated that, based on the layout of the proposed development, trips to/from the subject site may utilize the proposed right-in/right-out (RIRO) access to St. Joseph Boulevard when arriving and the Lionel-Rhéo Private access when departing, or vice versa. The assumed assignment of site-generated trips can be described as follows:

#### St. Joseph Boulevard RIRO Access

- 100% of trips arriving from the north via Tenth Line Road;
- 100% of trips arriving from the south via Tenth Line Road;
- 25% of trips arriving from the east via St. Joseph Boulevard;
- 25% of trips arriving from the west via Ottawa Road 174;
- 50% of trips departing to the west via St. Joseph Boulevard;
- 50% of trips departing to the west via Ottawa Road 174.

#### Lionel-Rhéo Private Access via Vieux-Silo Street

- 100% of trips arriving from the west via St. Joseph Boulevard;
- 100% of trips departing to the south via Tenth Line Road;
- 50% of trips departing to the west via St. Joseph Boulevard.

#### Lionel-Rhéo Private Access via Eric Czapnik Way

- 75% of trips arriving from the east via St. Joseph Boulevard;
- 75% of trips arriving from the west via Ottawa Road 174;
- 100% of trips departing to the north via Tenth Line Road;
- 100% of trips departing to the east via St. Joseph Boulevard;
- 50% of trips departing to the west via Ottawa Road 174.

## 3.2 Background Traffic

## 3.2.1 Other Area Developments

As first discussed in Section 2.2.2, traffic generated by the following developments in proximity of the subject site have been considered in the 2024 and 2029 background volumes. Relevant excerpts of the traffic studies in support of these developments are included in **Appendix F**.

#### <u>Hillside Vista</u>

A total of 44 townhome dwellings have been built (considered 'Phase 1A'), and a total of 90 walkup condominium dwellings are proposed as part of 'Phase 1B.' A Transportation Brief was prepared by Novatech in December 2017 in support of Phase 1B, and traffic volumes generated by this phase have been added to the 2024 and 2029 background traffic volumes.

## 211 Centrum Boulevard

This development is planned to include 17-storey and 9-storey buildings, consisting of a total of 397 retirement home units. A TIA was prepared by CGH in April 2021 in support of this development, and traffic volumes generated by this development have been added to the 2024 and 2029 background traffic volumes.

# 3030 St. Joseph Boulevard

This development is planned to include a 16-storey building, consisting of 165 residential units and 426 m<sup>2</sup> of ground floor retail space. A Transportation Brief was prepared by Parsons in September 2017 in support of this development, and traffic volumes generated by this development have been added to the 2024 and 2029 background traffic volumes.

#### Petrie's Landing (8466, 8600, 8700, and 8900 Jeanne d'Arc Boulevard)

This development is divided into three phases, and can be summarized as follows:

- Petrie's Landing I: 806 dwellings and 1,500 m<sup>2</sup> of commercial space;
- Petrie's Landing II: 113 dwellings;
- Petrie's Landing III: 790 dwellings, 23,000 ft<sup>2</sup> of retail space, and 370,000 ft<sup>2</sup> of office space.

A TIA was prepared by Parsons in February 2021 in support of Phase 2, and identified that the first two phases are anticipated to be built out by 2024, while the third phase is assumed to be 50% built out in 2024 and fully built out by 2027. Therefore, traffic volumes generated by 100% of Phase 1, 100% of Phase 2, and 50% of Phase 3 have been added to the 2024 background traffic volumes, and traffic volumes generated by 100% of all phases have been added to the 2029 background traffic volumes.

## 3.2.2 General Background Growth Rate

A review of snapshots of the City's *Strategic Long-Range Model* and *Intersection Traffic Growth Rates (2000-2016)* has been conducted. Both resources are included in **Appendix H**. Comparing snapshots of the 2011 and 2031 AM peak hour traffic volumes, the *Strategic Long-Range Model* generally suggests positive growth on the arterial roadways, ranging from approximately 0.5% on Tenth Line Road and approximately 4% on St. Joseph Boulevard. The *Intersection Traffic Growth Rates* figures, which determine growth rates based on total vehicular volumes entering the intersection, identify the following growth rates at St. Joseph Boulevard/Tenth Line Road between 2000 and 2016.

- AM Peak Hour: positive growth between +4.0% and +8.0% per annum;
- PM Peak Hour: positive growth between +2.0% and +4.0% per annum.

In addition to the above resources, Exhibit 2.10 of the City's 2013 TMP projects population and employment growth rates of approximately 1.6% to 3.0% per year, respectively. Therefore, an annual background growth rate of 3% has been assumed for traffic volumes on St. Joseph Boulevard, Tenth Line Road, Old Tenth Line Road/Ottawa Road 174.

## 3.3 Future Traffic Conditions

The figures below present the following future traffic conditions:

- Proposed site-generated traffic volumes are shown in Figure 5;
- Other area development-generated traffic volumes in 2024 are shown in Figure 6;
- Other area development-generated traffic volumes in 2029 are shown in Figure 7;
- Background traffic volumes in 2024 are shown in Figure 8;
- Background traffic volumes in 2029 are shown in Figure 9;
- Total traffic volumes in 2024 are shown in Figure 10;
- Total traffic volumes in 2029 are shown in Figure 11.

## Figure 5: Site-Generated Traffic Volumes







## Figure 7: 2029 Other Area Development-Generated Traffic Volumes







## Figure 9: 2029 Background Traffic Volumes



## Figure 10: 2024 Total Traffic Volumes



## Figure 11: 2029 Total Traffic Volumes



## 3.4 Demand Rationalization

A review of the existing and background intersection operations has been conducted to determine if and when traffic volumes exceed capacity within the study area. The intersection parameters used in the analysis are consistent with the *2017 TIA Guidelines* (Saturated Flow Rate: 1,800 vphpl, Peak Hour Factor: 0.9 in existing conditions and 1.0 in future conditions).

Per Exhibit 22 of the *Multi-Modal Level of Service (MMLOS) Guidelines*, the target vehicular level of service (Auto LOS) at all study area intersections is an Auto LOS D, which equates to a vehicle-to-capacity (v/c) ratio of 0.90 at signalized intersections, and a maximum delay of 35 seconds at unsignalized intersections. Signal timing plans were obtained from the City, and are included in **Appendix I**.

## 3.4.1 Existing Intersection Operations

Intersection capacity analysis has been conducted for the existing traffic conditions. The results of the analysis are summarized in **Table 6** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix J**.

Intersection	AM Peak			PM Peak		
	Max v/c or Delay	LOS	Mvmt	Max v/c or Delay	LOS	Mvmt
St. Joseph Boulevard/ Vieux-Silo Street <sup>(1)</sup>	16 sec	С	SBL/R	18 sec	С	SBL/R
St. Joseph Boulevard/ Tenth Line Road <sup>(2)</sup>	0.73	С	NBT	0.79	С	EBR
St. Joseph Boulevard/ Eric Czapnik Way <sup>(1)</sup>	10 sec	А	SBL/R	11 sec	В	SBL/R
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp <sup>(2)</sup>	0.37	A	WBT	0.66	В	SBT

#### **Table 6: Existing Traffic Operations**

1. Unsignalized intersection

2. Signalized intersection

From the previous table, all study area intersections currently operate at an Auto LOS C or better during the AM and PM peak hours.

For all auxiliary lanes at the study area intersections, the Synchro analysis does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection (i.e. on St. Joseph Boulevard, westbound queues at Tenth Line Road and eastbound queues at Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp do not extend through the intersection at Eric Czapnik Way).

## 3.4.2 2024 Background Intersection Operations

Intersection capacity analysis has been conducted for the 2024 background traffic conditions. The results of the analysis are summarized in **Table 7** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

## Table 7: 2024 Background Traffic Operations

	AM Peak			PM Peak		
Intersection	Max v/c or Delay	LOS	Mvmt	Max v/c or Delay	LOS	Mvmt
St. Joseph Boulevard/ Vieux-Silo Street <sup>(1)</sup>	16 sec	С	SBL/R	18 sec	С	SBL/R
St. Joseph Boulevard/ Tenth Line Road <sup>(2)</sup>	0.74	С	NBT	0.79	С	EBR
St. Joseph Boulevard/ Eric Czapnik Way <sup>(1)</sup>	10 sec	А	SBL/R	11 sec	В	SBL/R
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp <sup>(2)</sup>	0.35	A	WBT	0.66	В	SBT

1. Unsignalized intersection

2. Signalized intersection

From the previous table, all study area intersections in the 2024 background conditions are projected to continue operating at an Auto LOS C or better during the AM and PM peak hours. The max v/c ratios and delays at some intersections appear to improve when compared to the existing conditions, due to differences in the Peak Hour Factor parameter (0.9 in existing conditions versus 1.0 in future conditions).

For all auxiliary lanes at the study area intersections, the Synchro analysis does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection.

## 3.4.3 2029 Background Intersection Operations

Intersection capacity analysis has been conducted for the 2029 background traffic conditions. The results of the analysis are summarized in **Table 8** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix K**.

#### Table 8: 2029 Background Traffic Operations

	AM Peak			PM Peak		
Intersection	Max v/c or Delay	LOS	Mvmt	Max v/c or Delay	LOS	Mvmt
St. Joseph Boulevard/ Vieux-Silo Street <sup>(1)</sup>	17 sec	С	SBL/R	21 sec	С	SBL/R
St. Joseph Boulevard/ Tenth Line Road <sup>(2)</sup>	0.84	D	NBT	0.82	D	EBR
St. Joseph Boulevard/ Eric Czapnik Way <sup>(1)</sup>	10 sec	А	SBL/R	11 sec	В	SBL/R
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp <sup>(2)</sup>	0.40	А	WBT	0.71	С	SBT

1. Unsignalized intersection

2. Signalized intersection

From the previous table, all study area intersections in the 2029 background conditions are projected to operate at an Auto LOS D or better during the AM and PM peak hours. Compared to the 2024 background conditions, the max v/c ratios and delays at the study area intersections increase due to background traffic growth.

The Synchro analysis identifies that, in the AM peak hour, the 95<sup>th</sup>-percentile northbound queue length at St. Joseph Boulevard/Tenth Line Road is approximately 190m, which extends past the auxiliary northbound left turn lane. For all auxiliary lanes within the study area, Synchro does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection.

## 4.0 ANALYSIS

## 4.1 Development Design

## 4.1.1 Design for Sustainable Modes

New pedestrian walkways will connect the main and secondary entrances of Buildings A and B to the existing sidewalk on St. Joseph Boulevard. The main entrance of Building A is located at the southeastern corner of the subject site and the main entrance of Building B is located east of the proposed RIRO access, while the secondary entrances to both buildings will be accessed in the service easement between the two buildings, and are located approximately 35m north of St. Joseph Boulevard. New pedestrian walkways will also connect the main entrance of Building A and the northern end of the subject site to the existing sidewalk on Tenth Line Road.

bicycle parking will be provided in the upper parking garage level within Building B. The number of bicycle parking spaces, as well as the minimum bicycle parking requirements per the City's *Zoning By-Law* (ZBL), are reviewed further in Section 4.2.

The nearest bus stops to the subject site are shown in Section 2.1.6 and **Figure 3**. OC Transpo's service guideline for peak period services is to provide service within a five-minute (400m) walk of home, work, or school for 95% of urban residents. Measuring from the main entrance, the bus stops within 400m are stops #1794, #6763, #7843, #7846, #8596, and #8761, which are served by OC Routes 33, 236, and 302.

A review of the *Transportation Demand Management (TDM)-Supportive Development Design and Infrastructure Checklist* has been conducted. All required TDM-supportive design and infrastructure measures in the TDM checklist are met. A copy of the checklist is included in **Appendix L**. In addition to the required measures, the proposed development also meets the following 'basic' or 'better' measures as defined in the *TDM-Supportive Development Design and Infrastructure Checklist*:

- Locate building close to the street, and do not locate parking areas between the street and building entrances;
- Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations;
- Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort;
- Provide safe, direct and attractive walking routes from building entrances to nearby transit stops.
### 4.1.2 Circulation and Access

Pick-ups and drop-offs will be facilitated in a designated loop at the northwest corner of Building A, which will be accessed via Lionel-Rhéo Private. Access to the underground parking garage via Building A will be located directly adjacent to this pick-up/drop-off loop. Garbage collection and loading/deliveries will take place at the service entries to Buildings A and B, via the access along the service easement between the buildings. The fire route for the proposed development will be located curbside on St. Joseph Boulevard.

Turning movement figures have been prepared for a Medium Single Unit (MSU) design vehicle, which represents garbage or delivery trucks, and is anticipated to be the largest vehicle to traverse the site. These figures are included in **Figure 12** and **Figure 13**.

### 4.2 Parking

The subject site is located in Area C of Schedule 1 and Area Z of Schedule 1A of the City's ZBL. Minimum vehicle parking rates, and minimum bicycle parking rates for the proposed development are identified in Sections 101, 102, and 111 of the ZBL, and are summarized in **Table 9**.

### Table 9: Required and Proposed Parking

Land Use	Rate	Units	Required	Provided
Minimum Vehicle	Parking			
Dwelling,	No minimum rate for resident parking; 0.1	minimum rate for resident parking; 0.1		157 (resident)
Mid-Rise	spaces per unit for visitors after the first 12 units		20	27 (visitor)
Minimum Bicycle	Parking			
Dwelling,	0 E angege per dwelling	074 unite	127	127
Mid-Rise	0.5 spaces per dwelling	274 units	137	137

Based on the previous table, the proposed number of vehicle parking spaces meets both of the minimum requirements, as outlined in the City's ZBL. Of the 184 proposed parking spaces, three will be allocated as accessible parking spaces.

Section 111(12) of the ZBL identifies that, where the number of bicycle parking spaces required for a single residential building exceeds 50 spaces, a minimum of 25% of the required total must be located within a building or structure, a secure area, or bicycle lockers. All bicycle parking will be provided in the upper parking garage level within Building B. Therefore, this requirement is met.

### 4.3 Boundary Streets

This section provides a review of the boundary streets St. Joseph Boulevard and Tenth Line Road. The *MMLOS Guidelines*, produced by IBI Group, in October 2015 were used to evaluate the levels of service for each alternative mode of transportation on the boundary streets. The roadways have been evaluated against the MMLOS targets associated with the 'Mixed Use Centre' land use designation, and are based on existing conditions.

A detailed segment MMLOS review of the boundary streets is included in **Appendix M**. A summary of the segment MMLOS analysis are provided in **Table 10**.



SHT8X11.DWG - 216mmx279mm



SHT8X11.DWG - 216mmx279mm

Table To. Segment www.cos Summary								
Seament	PL	OS	BL	OS	TL	OS	TkL	.OS
Segment	Actual	Target	Actual	Target	Actual	Target	Actual	Target
St. Joseph Boulevard	F	С	F	Α	D	-	А	D
Tenth Line Road	F	С	F	С	D	D	A	D

### Table 10: Segment MMLOS Summary

The results of the segment MMLOS analysis can be summarized as follows:

- Both St. Joseph Boulevard and Tenth Line Road do not meet the target pedestrian level of service (PLOS) C;
- Both St. Joseph Boulevard and Tenth Line Road do not meet the target bicycle level of service (BLOS) A or C;
- Tenth Line Road achieves the target transit level of service (TLOS) D;
- Both St. Joseph Boulevard and Tenth Line Road meet the target truck level of service (TkLOS) D.

Based on Exhibit 4 of the *MMLOS Guidelines*, the best possible PLOS for roadways with curb lane volumes greater than 3,000 vehicles per day and an operating speed greater than 60 km/h is a PLOS D. This can be achieved by providing sidewalks with a minimum width of 2.0m and a minimum boulevard width greater than 2.0m. This applies to both sides of St. Joseph Boulevard and the east side of Tenth Line Road, and is identified for the City's consideration. For the west side of Tenth Line Road, the target PLOS C can be achieved by providing a sidewalk with a minimum width of 2.0m and a minimum width of 0.5m. This is identified for the City's consideration.

Based on Exhibit 11 of the *MMLOS Guidelines*, a physically separated bikeway (such as cycle tracks or multi-use pathways) are required to achieve the target BLOS A for St. Joseph Boulevard or BLOS C for Tenth Line Road, given the current operating speed of both roadways. The *Ontario Traffic Manual (OTM) – Book 18* includes a desirable cycling facility pre-selection tool, based on the operating speed and AADT of a roadway. For roadways with an operating speed of 70 km/h and AADT volumes of 11,000 to 16,000 vehicles per day, OTM Book 18 identifies that separated facilities are appropriate. This is identified for the City's consideration. The pre-selection tool is included in **Figure 14**.

### 4.4 Access Design

The proposed connection to Lionel-Rhéo Private will be approximately 6.0m in width. Per Section 107(1)(a) of the City's ZBL, any driveway providing access to a parking lot or garage must have a minimum width of 6.0m for a double traffic lane. Per Section 107(1)(aa) of the ZBL, the maximum permitted width for a double traffic lane leading to 20 or more parking spaces is 6.7m. The proposed connection to Lionel-Rhéo Private meets both of these provisions.

The design of the proposed RIRO access to St. Joseph Boulevard has been evaluated based on the relevant provisions of the City's ZBL and *Private Approach By-Law* (PABL), and the Transportation Association of Canada (TAC)'s *Geometric Design Guide for Canadian Roads*.

Section 25(a) of the PABL identifies that, for sites with 45m to 150m of frontage on a roadway, a maximum of two two-way private approaches to that roadway are permitted. As only one access to St. Joseph Boulevard is proposed, this requirement is met.





1 Operating speeds are assumed to be similar to posted speeds. If evidence suggests this is not the case,

- Operating speeds are assumed to be similar to posted speeds. If evidence suggests this is not the case, practitioners may consider using 85th percentile speeds or implementing measures to reduce operating speeds.
- 2 Physically separated bikeways may always be considered in the designated operating space area of the nomograph.
- 3 On roadways with two or more lanes per direction (including multi-lane one-way roadways), a buffered bicycle lane should be considered the minimum with a typical facility being a physically separated bikeway.

Section 25(c) of the PABL identifies a maximum width requirement of 9.0m for any two-way private approach, as measured at the street line. Since the proposed RIRO access is approximately 6.1m in width at the street line, this requirement is met. Section 107(1)(a) and 107(1)(aa) of the City's ZBL identify that any driveway providing access to a parking lot or garage must have a minimum width of 6.0m and a maximum width of 6.7m, for double traffic lanes leading to 20 or more parking spaces. The proposed RIRO access also meets both of these provisions.

Section 25(m)(ii) of the PABL identifies that, for a property that abuts or is within 46m of an arterial roadway, there is a minimum distance requirement between a private approach and the nearest intersecting street line, based on the land use and the number of parking space provided. For apartment buildings with 100 to 199 parking spaces, a minimum distance of 30m is required. Measuring along the street line, the nearest edge of the proposed RIRO access is approximately 57m to the extension of the Tenth Line Road ROW, and therefore this requirement is met.

Section 25(p) of the PABL identifies a minimum separation requirement of 3m between a private approach and the nearest property line, as measured at the street line. The western edge of the proposed RIRO access is approximately 5m from the westerly property line, and therefore this requirement is met.

Section 25(u) of the PABL identifies a requirement that any private approach serving a parking area with more than 50 parking spaces shall not have a grade exceeding 2% for the first 9m inside the property line. Measuring from the property line, the grade of the access is approximately 2% (descending toward the roadway) for the first 4m inside the property line, and 6% (descending toward the parking garage) for the next 5m. By limiting the maximum grade to 6% within the first 9m of the property line, it is anticipated that drivers exiting the parking garage will have adequate sightlines to pedestrians walking along St. Joseph Boulevard. Therefore, it is requested that the requirement of Section 25(u) of the PABL be waived.

TAC's *Geometric Design Guide* identifies minimum clear throat length requirements for accesses based on land use, development size, and class of roadway. For apartment developments with more than 200 dwellings accessing arterial roadways, a minimum clear throat length of 40m is required. The proposed RIRO access has approximately 13.7m of clear throat before the garage door. While the clear throat length is not met, queueing back onto St. Joseph Boulevard is mitigated by the access being restricted to right-in/right-out. Approximately 15 inbound vehicles per hour are anticipated for the PM peak hour, which is expected to result in queueing of less than one vehicle. There is another approximately 40m of clear throat before the first parking spaces within the parking garage. Therefore, queueing onto St. Joseph Boulevard is not anticipated.

TAC's *Geometric Design Guide* identifies a minimum corner clearance requirement of 70m for arterial roadways, measuring between the private approach and the edge of the roadway. The proposed RIRO access to St. Joseph Boulevard is approximately 65m from the curbline of Tenth Line Road, and does not meet this requirement. However, it is located as far from the intersection at St. Joseph Boulevard/Tenth Line Road as possible.

Dedicated right turn lanes are typically considered when the peak hour turning volumes meet or exceed 60 vehicles per hour, or 10% of the adjacent through volume. As shown in the total traffic volume figures (**Figures 10** and **11**), these thresholds are not met by the westbound right turn movements into the proposed RIRO access to St. Joseph Boulevard. Therefore, no auxiliary westbound right turn lane at the RIRO access is recommended or required.

### 4.5 Transportation Demand Management

### 4.5.1 Context for TDM

Broken down by dwelling type, the proposed development will include nine studio units, 111 onebedroom units, 83 one-bedroom plus den units, 69 two-bedroom units, and two three-bedroom units.

### 4.5.2 Need and Opportunity

The subject site is designated as 'Mixed Use Centre' on Schedule B of the City's Official Plan. The implemented zoning for the property is 'Residential Fifth-Density, Subzone Z' (R5Z). As first discussed in Section 3.1.1, the mode shares for the proposed development are assumed to be generally consistent with the surveyed mode shares of the Orléans district, as outlined in the *TRANS Trip Generation Manual*.

As discussed in Section 2.2.1, the Confederation Line LRT will be extended to Trim Road along Ottawa Road 174, and isolated transit priority measures are proposed on Tenth Line Road between Ottawa Road 174 and Charlemagne Boulevard. These measures are anticipated to improve the transit share of the proposed development and the Orléans district, and therefore failure to meet the assumed driver share target of 55% during the peak hours is not anticipated.

### 4.5.3 TDM Program

A review of the City's *TDM Measures Checklist* has been conducted by the proponent, who has committed to providing the following TDM measures.

- Display local area maps with walking/cycling access routes and key destinations at major entrances;
- Display relevant transit schedules and route maps at entrances;
- Unbundle parking cost from monthly rent.

A copy of the checklist is included in **Appendix L**.

### 4.6 Neighbourhood Traffic Management

The *2017 TIA Guidelines* identify two-way peak hour traffic volume thresholds for considering when a Neighbourhood Traffic Management (NTM) plan should be developed, when the site relies on local or collector roadways for access. The NTM two-way volume thresholds (in vehicles per hour, or vph) are as follows:

- 120 vph for local roadways;
- 300 vph for collector roadways;
- 600 vph for major collector roadways.

The proposed development can be accessed via the local roadways Vieux-Silo Street or Eric Czapnik Way, as well as the private roadways Récolte Private and Lionel-Rhéo Private. As shown in **Figure 11**, only Eric Czapnik Way is anticipated to exceed the NTM volume threshold of 120 vph (by less than 5 vph), and therefore only Eric Czapnik Way is reviewed below.

The typical lane capacities shown in the City's TRANS Long-Range Transportation Model have been used to estimate the directional capacity of this roadway, based on roadway classification and general characteristics (for example, suburban with limited access, urban with on-street parking, etc.). To compare the directional capacity with the NTM thresholds, the two-way NTM thresholds have been halved to represent a one-way volume threshold. The assumed directional capacities (in vehicles per hour per lane, or vphpl) and NTM one-way volume thresholds (in vph) for Eric Czapnik Way can be summarized as follows.

- Directional capacity: 400 vphpl capacity in each direction;
- NTM threshold: 60 vph threshold in each direction (15% of capacity).

It should be noted that any roadway operating at 60% capacity or less (i.e. a v/c ratio of 0.60 or better) is considered to be operating at the best possible Auto LOS A. Therefore, the NTM thresholds are considered to be extremely low.

The 2029 total traffic peak hour volumes and corresponding v/c ratios for Eric Czapnik Way are summarized as follows:

### AM Peak Hour

- Northbound: 45 vph (v/c: 0.11)
- Southbound: 76 vph (v/c: 0.19)
- PM Peak Hour
- Northbound: 71 vph (v/c: 0.18)
- Southbound: 53 vph (v/c: 0.13)

From the above, Eric Czapnik Way is not anticipated to operate at or near capacity in the 2029 total traffic conditions. Detailed intersection analysis for total traffic conditions is included in Sections 4.8.2 and 4.8.3, and identify no operational concerns on Eric Czapnik Way. Further, the function of Eric Czapnik Way as a local roadway is not anticipated to change as a result of the proposed development, and no neighbourhood traffic management measures are required.

### 4.7 Transit

Based on the trip generation estimates presented in Section 3.1.1, the proposed development is projected to generate the following number of transit trips:

- 30 transit trips (9 inbound trips and 21 outbound trips) during the AM peak hour;
- 29 transit trips (17 inbound trips and 12 outbound trips) during the PM peak hour.

All site-generated transit trips are anticipated to board and alight buses at the stops listed in Section 2.1.6, which includes stops on St. Joseph Boulevard and Tenth Line Road. No capacity issues are anticipated for OC Transpo Routes 33, 236, or 302, based on the above transit trip estimates.

### 4.8 Intersection Design

### 4.8.1 Intersection MMLOS Review

This section provides a review of the signalized study area intersections using complete streets principles. The signalized intersections within the study area have been evaluated for PLOS, BLOS, TLOS, and TkLOS.

Based on Schedule B of the City's Official Plan, the MMLOS targets associated with the 'Mixed Use Centre' designation have been used to evaluate St. Joseph Boulevard/Vieux-Silo Street and St. Joseph Boulevard/Tenth Line Road, and the targets associated with the 'General Urban Area' designation have been used to evaluate St. Joseph Boulevard/Eric Czapnik Way and St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp.

The full intersection MMLOS analysis is included in **Appendix M**. A summary of the results is shown in **Table 11**.

### Table 11: Intersection MMLOS Summary

Intersection	PL	OS	BL	OS	TLOS		TkLOS	
Intersection	Actual	Target	Actual	Target	Actual	Target	Actual	Target
St. Joseph Boulevard/ Tenth Line Road	F	С	F	А	Е	D	А	D
St. Joseph Boulevard/Old Tenth Line Road/OR 174 EB Off-Ramp	F	С	F	С	С	-	D	D

The results of the intersection MMLOS analysis can be summarized as follows:

- Neither signalized intersection meets the target PLOS;
- Neither signalized intersection meets the target BLOS;
- St. Joseph Boulevard/Tenth Line Road does not meet the target TLOS;
- Both signalized intersections meet the target TkLOS.

A discussion of St. Joseph Boulevard/Tenth Line Road and St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp in further detail is included below.

### St. Joseph Boulevard/Tenth Line Road

The intersection does not meet the target PLOS C, BLOS A, or TLOS E.

All approaches have a divided cross-section with a width equivalent to ten lanes crossed or more (assuming a lane width equals 3.5m, per the *MMLOS Guidelines*). There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks (greater than 400,000 vehicle/pedestrian conflicts over an eight-hour period). There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.

All approaches do not meet the target BLOS, based on both left and right turn characteristics. Based on Exhibit 12 of the *MMLOS Guidelines*, achieving the target BLOS A would require physicallyseparated bikeways (such as cycle tracks or multi-use pathways) and off-road facilities for cyclists to turn left. Therefore, the implementation of a protected intersection would be required to meet the target BLOS A, and would involve the removal of all right turn channels. This is identified for the City's consideration.

The south and east approaches do not meet the target TLOS D. The east approach does not have a target TLOS, but the approach delays of approximately 35 seconds during the AM peak hour is noted. The City's RTTP Affordable Network includes transit priority signals and queue jump lanes on Tenth Line Road, and would be expected to improve the delays for transit vehicles to the target TLOS D or better.

<u>St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp</u> The intersection does not meet the target PLOS C or BLOS C.

All approaches have a divided cross-section with a width equivalent to eight to ten lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements further. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.

The south and east approaches do not meet the target BLOS based on left turn characteristics, and the south approach does not meet the target based on right turn characteristics. Per Exhibit 12 of the *MMLOS Guidelines*, achieving the BLOS C would require two-stage left-turn bike boxes for northbound and westbound cyclists. Implementing bike boxes at the north approach would not require right turn on red (RTOR) restrictions, and is therefore identified for the City's consideration. There is no through phase for northbound cyclists to get to a left-turn bike box, and northbound cyclists could use Tenth Line Road to turn left onto St. Joseph Boulevard instead. Exhibit 12 of the *MMLOS Guidelines* identifies that the south approach can achieve the BLOS C based on right turn characteristics by implementing a pocket bike lane across the channelized northbound right turn lane. This is identified for the City's consideration.

### 4.8.2 2024 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2024 total traffic conditions. The results of the analysis are summarized in **Table 12** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

	AM Peak			PM Peak		
Intersection	Max v/c or Delay	LOS	Mvmt	Max v/c or Delay	LOS	Mvmt
St. Joseph Boulevard/ Vieux-Silo Street <sup>(1)</sup>	17 sec	С	SBL/R	20 sec	С	SBL/R
St. Joseph Boulevard/ Tenth Line Road <sup>(2)</sup>	0.74	С	NBT	0.79	С	EBR
St. Joseph Boulevard/ Eric Czapnik Way <sup>(1)</sup>	10 sec	А	SBL/R	11 sec	В	SBL/R
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp <sup>(2)</sup>	0.36	А	WBT	0.66	В	SBT
St. Joseph Boulevard/ RIRO Site Access <sup>(1)</sup>	10 sec	A	SBR	10 sec	А	SBR

### Table 12: 2024 Total Traffic Operations

1. Unsignalized intersection

2. Signalized intersection

Comparing the previous table and the 2024 background conditions, the addition of site-generated traffic is anticipated to have little impact on the operations of the study area intersections.

### 4.8.3 2029 Total Intersection Operations

Intersection capacity analysis has been conducted for the 2029 total traffic conditions. The results of the analysis are summarized in **Table 13** for the weekday AM and PM peak hours. Detailed reports are included in **Appendix N**.

### Table 13: 2029 Total Traffic Operations

	AM Peak			PM Peak		
Intersection	Max v/c or Delay	LOS	Mvmt	Max v/c or Delay	LOS	Mvmt
St. Joseph Boulevard/ Vieux-Silo Street <sup>(1)</sup>	19 sec	С	SBL/R	24 sec	С	SBL/R
St. Joseph Boulevard/ Tenth Line Road <sup>(2)</sup>	0.84	D	NBT	0.82	D	EBR
St. Joseph Boulevard/ Eric Czapnik Way <sup>(1)</sup>	10 sec	А	SBL/R	11 sec	В	SBL/R
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp <sup>(2)</sup>	0.40	А	WBT	0.71	С	SBT
St. Joseph Boulevard/ RIRO Site Access <sup>(1)</sup>	10 sec	А	SBR	10 sec	А	SBR

1. Unsignalized intersection

2. Signalized intersection

Comparing the previous table and the 2029 background conditions, the addition of site-generated traffic is anticipated to have little impact on the operations of the study area intersections.

### 5.0 CONCLUSIONS AND RECOMMENDATIONS

The conclusions and recommendations of this TIA can be summarized as follows:

### **Forecasting**

• The proposed development is estimated to generate 111 person trips (including 58 vehicle trips) during the AM peak hour and 112 person trips (including 60 vehicle trips) during the PM peak hour.

### Development Design

- New pedestrian walkways will connect the main and secondary entrances of Buildings A and B to the existing sidewalk on St. Joseph Boulevard. The main entrance of Building A is located at the southeastern corner of the subject site and the main entrance of Building B is located east of the proposed RIRO access, while the secondary entrances to both buildings will be accessed in the service easement between the two buildings, and are located approximately 35m north of St. Joseph Boulevard. New pedestrian walkways will also connect the main entrance of Building A and the northern end of the subject site to the existing sidewalk on Tenth Line Road.
- Bicycle parking will be provided on the upper parking garage level within Building B.
- Measuring from the main entrance, the bus stops within 400m are stops #1794, #6763, #7843, #7846, #8596, and #8761, which are served by OC Routes 33, 236, and 302.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.
- Pick-ups and drop-offs will be facilitated in a designated loop at the northwest corner of Building A, which will be accessed via Lionel-Rhéo Private. Access to the underground parking garage via Building A will be located directly adjacent to this pick-up/drop-off loop.

• Garbage collection and loading/deliveries will take place at the service entries to Buildings A and B, via the access along the service easement between the buildings. The fire route for the proposed development will be located curbside on St. Joseph Boulevard.

### <u>Parking</u>

• The proposed number of vehicle parking spaces meets the minimum requirements, as outlined in the City's ZBL. The proposed number of bicycle parking spaces also meets the minimum requirements.

### Boundary Streets

- The results of the segment multi-modal level of service (MMLOS) analysis can be summarized as follows:
  - Both St. Joseph Boulevard and Tenth Line Road do not meet the target pedestrian level of service (PLOS) C;
  - Both St. Joseph Boulevard and Tenth Line Road do not meet the target bicycle level of service (BLOS) C;
  - Tenth Line Road achieves the target transit level of service (TLOS) D;
  - Both St. Joseph Boulevard and Tenth Line Road meet the target truck level of service (TkLOS) D.
- Based on Exhibit 4 of the *MMLOS Guidelines*, the best possible PLOS for roadways with curb lane volumes greater than 3,000 vehicles per day and an operating speed greater than 60 km/h is a PLOS D. This can be achieved by providing sidewalks with a minimum width of 2.0m and a minimum boulevard width greater than 2.0m. This applies to both sides of St. Joseph Boulevard and the east side of Tenth Line Road, and is identified for the City's consideration. For the west side of Tenth Line Road, the target PLOS C can be achieved by providing a sidewalk with a minimum width of 2.0m and a minimum width of 2.0m and a minimum boulevard width of 0.5m. This is identified for the City's consideration.
- Based on Exhibit 11 of the *MMLOS Guidelines*, a physically separated bikeway (such as cycle tracks or multi-use pathways) are required to achieve the target BLOS A for St. Joseph Boulevard or BLOS C for Tenth Line Road, given the current operating speed of both roadways. This is identified for the City's consideration.

### Access Design

- The proposed access to St. Joseph Boulevard meets all required provisions of the *Private Approach By-Law* (PABL), except for Section 25(u). Measuring from the property line, the grade of the access is approximately 2% (descending toward the roadway) for the first 4m inside the property line, and 6% (descending toward the parking garage) for the next 5m. By limiting the maximum grade to 6% within the first 9m of the property line, it is anticipated that drivers exiting the parking garage will have adequate sightlines to pedestrians walking along St. Joseph Boulevard. Therefore, it is requested that the requirement of Section 25(u) of the PABL be waived.
- The proposed access to St. Joseph Boulevard does not meet the Transportation Association
  of Canada (TAC)'s clear throat length requirement of 40m, as the underground parking
  garage door is located within this distance. However, the potential for queueing back onto
  St. Joseph Boulevard is mitigated by the access being restricted to right-in/right-out, and
  there is another approximately 40m of clear throat before the first parking spaces within the
  parking garage. Queueing onto St. Joseph Boulevard is not anticipated.

• TAC's *Geometric Design Guide* identifies a minimum corner clearance requirement of 70m for arterial roadways, measuring between the private approach and the nearest edge of the roadway. While it is acknowledged that the proposed access to St. Joseph Boulevard does not meet this requirement, it is located as far from the intersection at St. Joseph Boulevard/ Tenth Line Road as possible.

### Transportation Demand Management

- The proponent has committed to providing the following TDM measures:
  - Display local area maps with walking/cycling access routes and key destinations at major entrances;
  - Display relevant transit schedules and route maps at entrances;
  - Unbundle parking cost from monthly rent.

### Neighbourhood Traffic Management

• Eric Czapnik Way exceeds the City's threshold for considering neighbourhood traffic management measures. Eric Czapnik Way is not anticipated to operate at or near capacity in the 2029 total traffic conditions. Further, the function of Eric Czapnik Way as a local roadway is not anticipated to change as a result of the proposed development, and no neighbourhood traffic management measures are required.

### <u>Transit</u>

• The proposed development is projected to generate 30 transit trips during the AM peak hour and 29 transit trips during the PM peak hour. No capacity issues are anticipated for OC Transpo Routes 33, 236, or 302, based on the above transit trip estimates.

### Intersection MMLOS

- The results of the intersection MMLOS analysis can be summarized as follows:
  - Neither signalized intersection meets the target PLOS;
  - Neither signalized intersection meets the target BLOS;
  - St. Joseph Boulevard/Tenth Line Road does not meet the target TLOS;
  - Both signalized intersections meet the target TkLOS.
- All approaches of St. Joseph Boulevard/Tenth Line Road have a divided cross-section with a width equivalent to ten lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements. No approaches meet the City's vehicle/pedestrian conflict threshold for zebrastriped crosswalks. There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.
- All approaches of St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp have a divided cross-section with a width equivalent to eight to ten lanes crossed or more. There is limited opportunity in improving the PLOS at each approach without reducing the number of travel lanes or restricting turning movements further. No approaches meet the City's vehicle/pedestrian conflict threshold for zebra-striped crosswalks. There is limited opportunity in improving the delay score for pedestrians without incurring major delays for vehicles.

- All approaches of St. Joseph Boulevard/Tenth Line Road do not meet the target BLOS, based on both left and right turn characteristics. Achieving the target BLOS A would require physically-separated bikeways (such as cycle tracks or multi-use pathways) and off-road facilities for cyclists to turn left. Therefore, the implementation of a protected intersection would be required to meet the target BLOS A, and would involve the removal of all right turn channels. This is identified for the City's consideration.
- The south and east approaches of St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp do not meet the target BLOS based on left turn characteristics, and the south approach does not meet the target based on right turn characteristics. Achieving the BLOS C would require two-stage left-turn bike boxes for northbound and westbound cyclists. Implementing bike boxes at the north approach would not require right turn on red (RTOR) restrictions, and is therefore identified for the City's consideration. There is no through phase for northbound cyclists to get to a left-turn bike box, and northbound cyclists could use Tenth Line Road to turn left onto St. Joseph Boulevard instead. The south approach can achieve the BLOS C based on right turn characteristics by implementing a pocket bike lane across the channelized northbound right turn lane. This is identified for the City's consideration.
- The south and east approaches of St. Joseph Boulevard/Tenth Line Road do not meet the target TLOS D. The east approach does not have a target TLOS, but the approach delays of approximately 35 seconds during the AM peak hour is noted. The City's RTTP Affordable Network includes transit priority signals and queue jump lanes on Tenth Line Road, and would be expected to improve the delays for transit vehicles to the target TLOS D or better.

### Existing Intersection Operations

 All study area intersections currently operate at an Auto LOS C or better during the AM and PM peak hours. For all auxiliary lanes at the study area intersections, the Synchro analysis does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection (i.e. on St. Joseph Boulevard, westbound queues at Tenth Line Road and eastbound queues at Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp do not extend through the intersection at Eric Czapnik Way).

### Background Intersection Operations

 All study area intersections in the 2029 background conditions are projected to operate at an Auto LOS D or better during the AM and PM peak hours. The Synchro analysis identifies that, in the AM peak hour, the 95<sup>th</sup>-percentile northbound queue length at St. Joseph Boulevard/Tenth Line Road is approximately 190m, which extends past the auxiliary northbound left turn lane. For all auxiliary lanes within the study area, Synchro does not identify any 50<sup>th</sup>-percentile or 95<sup>th</sup>-percentile queue lengths that exceed the storage lengths provided. Similarly, Synchro does not identify any queues that result in blocking at an upstream intersection.

### Total Intersection Operations

• The addition of site-generated traffic is anticipated to have little impact on the operations of the study area intersections. The proposed RIRO access to St. Joseph Boulevard is anticipated to operate at an Auto LOS A during the peak hours.

Based on the foregoing, the proposed development is recommended from a transportation perspective.

### **NOVATECH**

Prepared by:

Kudia

Joshua Audia, B.Sc. E.I.T., Transportation/Traffic

Reviewed by:



Jennifer Luong, P.Eng. Senior Project Manager, Transportation/Traffic

# **APPENDIX A**

Proposed Site Plan



### SITE PLAN LEGEND

	NEW CONSTRUCTION - BUILDING
	NEW CONSTRUCTION - RETAINING WALL
Ψ Ψ Ψ	GRASS
	INTERLOCK PATHWAY
	CONCRETE PATHWAY
	BUILDING ACCESS / EXI
<b>_</b> · <b>_</b>	PROPRETY LINE
	SETBACK LINE
	ZONING LINE
	TREE/BUSH

# GENERAL NOTES

NOTE A: NO PERMANENT BUILDING OR STRUCTURE SHALL BE PLACED WITHIN 5000MMMEASURED RADIALLY FROM ANY PRIMARY VOLTAGE CONDUCTOR OR EQUIPMENT.MEASURED FROM THE CLOSEST PRIMAR CONDUCTOR (AT REST) TO THE CLOSEST POINT OF THE BUILDING OR STRUCTURE

NOTE B: ALL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL OTHER DRAWINGS AND SPECIFICATIONS. ANY DISCEPANCIES BETWEEN DRAWINGS WILL BE REPORTED TO THE PROJECT LEAD IMMEDIATELY FOR CLARIFICATION PRIOR COMMENCE ANY CONSTRUCTION

NOTE C: ALL GENERAL SITE INFORMATION AND CONDITIONS HAVE BEEN COMPLIED FROM EXISTING PLANS AND SURVEY

NOTE D: CONTRACTOR IS RESPONSIBLE TO CHECK AND VERIFY ALL DIMENSIONS ON SITE AND REPORT ALL ERRORS AND / OR OMISSIONS TO THE ARCHITECT.

NOTE E: ALL CONTRACTORS MUST COMPLY WITH ALL CURRENT APPLICABLE CODES, REGULATIONS AND BYLAWS

NOTE F: DO NOT SCALE DRAWINGS.

# **PROJECT INFORMATION STATISTICS**

			100
SITE SUMMARY			
ADDRESS:	3277 ST-JOS	EPH-BOULEV.	ARD
ZONING: SITE AREA: PROPOSED USE:	R5Z (1415), 4965.04 m2 RESIDENTIA	R5Z (1363), R8 L APARTMEN	5Z(2168) TS (274 UNITS)
BUILDING AREA:	3045.07 m2	UNDERGRO	
ZONING SUMMARY			
MIN LOT AREA: MIN LOT WIDTH: BUILDING HEIGHT:		REQUIRED 1000 m <sup>2</sup> 25 m 10 storeys	PROPOSED 4965.04 m <sup>2</sup> 68.63 m 9 storeys
MIN YARD SETBACKS			
FRONT YARD: CORNER SIDE YARD: REAR YARD: INTERIOR SIDE YARD:	ACE:	3 m 3 m 7.5 m 1.5-6 m	3m 3m 7.5 m 3-6m
SOFT LANDSCAPING: HARD LANDSCAPING:	10L.	-	TBD TBD TBD
VEHICULAR PARKING			
RESIDENTIAL APARTM (274) UNITS EXEMPT UNDER ZONE 0 SPACES PER DWELL	ENTS 'Z' ING:	REQUIRED	157
VISITOR PARKING (274 AS PER TABLE 102 0.1 SPACES PER DWEL	-12)UNITS .LING:	27	27
TOTAL VEHICULAR PAI	RKING:	27	184
ACCESSIBLE PARKING (INCLUDED IN TOTAL P	: PARKING COUI	1 NT)	3
BICYCLE PARKING			
RESIDENTIAL APARTM	ENTS (274)UN	REQUIRED	PROPOSED
0.5 SPACES PER DWEL	LING :	137	137
WASTE MANAGEMENT	CONTAINER	3	
BUILDING 'A' - 172 UNI	rs	REQUIRED	
GARBAGE (172 X 0.11Y RECYCLING (172 X 0.03 ORGANICS (240L per 50	7 = 18.92Y3) 88Y = 6.54Y3) 0 UNITS = 4)	4Y <sup>3</sup> 4Y <sup>3</sup> 240L	5 2 4
BUILDING 'B' - 102 UNI	rs	REQUIRED	AMOUNT
GARBAGE (102 X 0.11Y RECYCLING (102 X 0.03 ORGANICS (240L per 50	7 = 11.22Y3) 88Y = 3.88Y3) 9 UNITS = 3)	4Y <sup>3</sup> 4Y <sup>3</sup> 240L	3 2 3
BUILDING SUMMARY			
LEVEL P3 PARKING:	GROSS 19	FLOOR AREA 72.82 m2	UNIT COUNT 47



BREAKDOWN:PRIVATE TERRACES / BALCONIES 'A'1483.5m²PRIVATE TERRACES / BALCONIES 'B650.6m²COMMUNAL ROOF TERRACE:602m²COMMUNAL GYM:86.86m²COMMUNAL GROUND FLOOR:113.81m²COMMUNAL EXTERIOR GRADE:130.91m²

### UNIT STATISTICS

1BEDROOM STUDIO
1 BEDROOM:
1 BEDROOM + DEN:
2 BEDROOM:
3 BEDROOM:

PROPOSED 9 units (3.28%) 111 units (40.51%) 83 units (30.29%) 69 units (25.18%) 2 units (0.72%)



#### 88 Saint-Joseph Boulevard, Gatineau QC J8Y 3W5 Tel : 819-600-1555



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PROJECT TEAM / ÉQUIPE DU PROJET : Le droit d'auteur est retenu par Rossmann Architects.



1.4	ISSUED FOR SPCA	2021-12-22
1.3	FOR COORDINATION	2021-12-16
1.1	<b>COORDINATION 33%</b>	2021-11-10
revisions	description	date

PROJECT NAME / NOME DU PROJET :

# PHOENIX HILLSIDE

DRAWING NAME / NOM DU DESSIN :

# GENERAL SITE PLAN CONTROL

PROJECT NO. / NO. DE PROJET :	20030
DATE :	2021-10-27
DRAWN BY / DESSINÉ PAR :	IG
REVIEWED BY / VÉRIFIÉ PAR :	LaG
SCALE / ÉCHELLE :	As indicated
PROJECT PHASE / PHASE DU PROJET :	1
DWG NO. / NO. DESSIN :	

# A003

REVISION NO. / NO. DE RÉVISION :

# **APPENDIX B**

**TIA Screening Form** 



#### Transportation Impact Assessment Screening Form

### City of Ottawa 2017 TIA Guidelines Screening Form

### 1. Description of Proposed Development

Municipal Address	3277 St. Joseph Boulevard
Description of Location	Located at the northwest corner of St. Joseph Boulevard and Tenth Line Road
Land Use Classification	Mid-Rise Apartments
Development Size (units)	274 dwellings
Development Size (m <sup>2</sup> )	-
Number of Accesses and Locations	One full-movement access to Lionel-Rhéo Private; One right-in/right-out access to St. Joseph Boulevard
Phase of Development	1
Buildout Year	2024

If available, please attach a sketch of the development or site plan to this form.

### 2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
Townhomes or apartments	90 units
Office	3,500 m <sup>2</sup>
Industrial	5,000 m <sup>2</sup>
Fast-food restaurant or coffee shop	100 m <sup>2</sup>
Destination retail	1,000 m <sup>2</sup>
Gas station or convenience market	75 m²

\* If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

#### If the proposed development size is greater than the sizes identified above, <u>the Trip Generation</u> <u>Trigger is satisfied.</u>



#### Transportation Impact Assessment Screening Form

### 3. Location Triggers

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?	$\checkmark$	
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*	$\checkmark$	

\*DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

#### If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

#### 4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		$\checkmark$
Are there any horizontal/vertical curvatures on a boundary street limiting sight lines at a proposed driveway?		$\checkmark$
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/suburban conditions)?	$\checkmark$	
Is the proposed driveway within auxiliary lanes of an intersection?		$\checkmark$
Does the proposed driveway make use of an existing median break that serves an existing site?		$\checkmark$
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?	$\checkmark$	
Does the development include a drive-thru facility?		$\checkmark$

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

### 5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?	$\checkmark$	
Does the development satisfy the Location Trigger?	$\checkmark$	
Does the development satisfy the Safety Trigger?	$\checkmark$	

If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, <u>the TIA Study must continue into the next stage</u> (Screening and Scoping).

# **APPENDIX C**

OC Transpo Route Maps



Peak periods with selected trips midday and evening / Périodes de pointe et service limité en mi-journée et soirée





Peak periods only Périodes de pointe seulement



2019.07





Peak periods only Périodes de pointe seulement



Future route after O-Train Line 1 is open Trajet du circuit après l'ouverture de la Ligne 1 de l'O-Train

Lost and Found / Objets perdus......613-563-4011 Security / Sécurité ......613-741-2478

CC Transpo INFO 613-741-4390 octranspo.com



Peak periods only Périodes de pointe seulement







### Tuesday only / Mardi seulement

Selected time periods Périodes sélectionnées



2019.08

Schedule / Horaire613-560-1000 Text / Texto											
Customer Relations Service à la clientèle											
Lost and Found / Objets perdus 613-563-4011 Security / Sécurité613-741-2478											
Effective December 25, 2016 En vigueur 25 décembre 2016											
CC Transpo INFO 613-741-4390 octranspo.com											



# **APPENDIX D**

Traffic Count Data



## Turning Movement Count - Peak Hour Diagram ST. JOSEPH BLVD @ TENTH LINE RD



Comments



## Turning Movement Count - Peak Hour Diagram ST. JOSEPH BLVD @ TENTH LINE RD



Comments



## Turning Movement Count - Study Results ST. JOSEPH BLVD @ TENTH LINE RD

Survey D	ate: 7	luesda	y, Ma	rch 20,	2018							wo	No:			37	613		
Start IIr	ne: (	07:00		_			-			–		Dev	ice:			Mio	/ision		
				F	ull S	Stud	y Sı	Imm	ary (8	B HF	R Sta	nda	rd)						
Survey Da	ate:	Tuesda	ay, Ma	arch 20	), 2018	8			Total O	bser	/ed U-	Turns	5				AAD	T Facto	or
							Ν	lorthbou	nd: 2		Sout	nbound	: 1				1.00		
								Eastbou	nd: 24	4	Wes	tbound:	5						
			TEN	TH LIN	E RD							ST. JO	OSEPI	H BLV	D				
	No	orthbou	nd		So	Southbour				Eastbo		bound		Westbound					
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	452	756	19	1227	9	104	33	146	1373	9	65	91	165	29	281	53	363	528	1901
08:00 09:00	378	772	16	1166	3	122	76	201	1367	17	78	135	230	28	233	52	313	543	1910
09:00 10:00	349	772	30	1151	12	125	35	172	1323	13	89	181	283	30	181	60	271	554	1877
11:30 12:30	328	571	28	927	6	121	26	153	1080	15	173	324	512	43	206	81	330	842	1922
12:30 13:30	372	534	33	939	11	103	38	152	1091	31	172	322	525	37	162	77	276	801	1892
15:00 16:00	307	580	27	914	8	120	33	161	1075	29	237	429	695	54	200	119	373	1068	2143
16:00 17:00	336	547	27	910	10	161	29	200	1110	46	315	518	879	54	162	131	347	1226	2336
17:00 18:00	332	614	17	963	7	135	45	187	1150	53	280	508	841	62	194	136	392	1233	2383
Sub Total	2854	5146	197	8197	66	991	315	1372	9569	213	1409	2508	4130	337	1619	709	2665	6795	16364
U Turns				2				1	3				24				5	29	32
Total	2854	5146	197	8199	66	991	315	1373	9572	213	1409	2508	4154	337	1619	709	2670	6824	16396
EQ 12Hr	3967	7153	274	11397	92	1377	438	1908	13305	296	1959	3486	5774	468	2250	986	3711	9485	22790
Note: These	values a	are calcu	lated b	y multipl	ying the	totais d	y the a	ppropriat	e expans	sion fac	tor.			1.39					
AVG 12Hr	3739	6741	258	10741	86	1298	413	1799	13305	279	1846	3285	5442	441	2121	929	3498	9485	22790
Note: These	volumes	s are calo	culated	by multi	plying th	ne Equiv	alent 1	2 hr. tota	als by the	AADT	factor.			1					
AVG 24Hr	4898	8831	338	14070	113	1701	541	2356	16426	366	2418	4304	7129	578	2778	1217	4582	11711	28137
Note: These	volumes	s are calo	culated	by multi	plying th	ne Avera	age Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



## Turning Movement Count - Study Results ERIC CZAPNIK WAY @ ST. JOSEPH BLVD





# Turning Movement Count - Study Results ERIC CZAPNIK WAY @ ST. JOSEPH BLVD

Survey Date: Thursday, April 23, 2015												WO	No:		35086								
Start Time: 07:00													Dev	ice:		Miovision							
Full Study 15 Minute Increments														5									
		FF		ΖΡΝ	IK WA	۰ Y		, tuu	y i.	/ 14111	S		SEPH		ק ח								
Northbound Southbound Eastbound														Westbound									
				N				s	STR		asibuu		Е				w	Grand					
Time Period	LT	ST	RT	тот	LT	ST	RT	тот	тот	LT	ST	RT	тот	LT	ST	RT	тот	тот	Total				
07:00 07:15	0	0	0	0	5	0	13	18	18	1	5	0	6	0	86	8	94	100	118				
07:15 07:30	0	0	0	0	1	0	11	12	12	7	28	0	35	0	90	6	96	131	143				
07:30 07:45	0	0	0	0	2	0	5	7	7	3	21	0	24	0	85	2	87	111	118				
07:45 08:00	0	0	0	0	2	0	5	7	7	5	23	0	28	0	83	0	83	111	118				
08:00 08:15	0	0	0	0	2	0	12	14	14	4	28	0	32	0	79	3	82	114	128				
08:15 08:30	0	0	0	0	1	0	4	5	5	3	23	0	26	0	82	0	82	108	113				
08:30 08:45	0	0	0	0	2	0	3	5	5	1	28	0	29	0	65	5	70	99	104				
08:45 09:00	0	0	0	0	3	0	8	11	11	4	40	0	44	0	58	3	61	105	116				
09:00 09:15	0	0	0	0	1	0	4	5	5	2	38	0	40	0	80	2	82	122	127				
09:15 09:30	0	0	0	0	1	0	6	7	7	5	35	0	40	0	71	5	76	116	123				
09:30 09:45	0	0	0	0	0	0	5	5	5	2	42	0	44	0	51	4	55	99	104				
09:45 10:00	0	0	0	0	2	0	6	8	8	6	46	0	52	0	72	5	77	129	137				
10:00 10:15	0	0	0	0	1	0	6	7	7	3	55	0	58	0	68	4	72	130	137				
10:15 10:30	0	0	0	0	2	0	4	6	6	5	41	0	46	0	57	2	59	105	111				
10:30 10:45	0	0	0	0	4	0	1	5	5	3	48	0	51	0	56	1	57	108	113				
10:45 11:00	0	0	0	0	1	0	4	5	5	6	68	0	/4	0	76	6	82	156	161				
11:00 11:15	0	0	0	0	1	0	3	4	4	/	36	0	43	0	79	5	84	127	131				
11:15 11:30	0	0	0	0	2	0	9	11	11	5	47	0	52	0	/2	4	76	128	139				
11:30 11:45	0	0	0	0	2	0	11	13	13	4	46	0	50	0	92	4	96	146	159				
11:45 12:00	0	0	0	0	2	0	13	15	15	5	48	0	53	1	//	5	83	136	151				
12:00 12:15	0	0	0	0	1	0	6	1	7	4	43	0	47	0	66	3	69	116	123				
12:15 12:30	0	0	0	0	3	0	2	5	5	3	55	0	58	0	74	4	78	136	141				
12:30 12:45	0	0	0	0	0	0	8	8	8	4	47	0	51	0	91	1	92	143	151				
12:45 13:00	0	0	0	0	1	0	/	8	8	8	52	0	60	0	78	4	82	142	150				
13:00 13:15	0	0	0	0	1	0	8	9	9	3	60	0	63	0	79	5	84	147	156				
13:15 13:30	0	0	0	0	0	0	5	5	5	2	47	0	49	0	68	5	73	122	127				
13.30 13:45	0	0	0	0	0	0	5 F	0 F	0 F	4	13	0	<i>11</i>	0	00 70	ঠ 7	11	140	154				
13.45 14:00	0	0	0	0	0	0	о 5	о 7	3 7	4	00 61	0	59 65	0	19	1	00	140	100				
14.00 14.10	0	0	0	0	2	0	5	<i>1</i> 5	/ 5	4	01 //7	0	52	0	76	0	76	12/	134				
14.13 14.30	0	0	0		2	0	5	2 2	9 2	J 1	41 61	0	5Z	0	70	1	21 21	146	154				
14.40	0	0	0	0	2	0	10	12	12	5	48	0	53	0	00	4	02	140	159				
15:00 15:15	0	0	0	0	0	0	5	5	5	1	40 66	0	70	0	86	2	88	158	163				
15:15 15:30	0	0	0	0	3	0	11	14	14	4	66	0	69	0	71	2 	75	144	158				
15:30 15:45	0	0	0	0	4	0	6	10	10	5	50	0	64	0	74	- 0	83	147	157				
15:45 16:00	0	0	0	0	- <del>1</del>	0	8	9	9	6	92	0	98	0	85	4	89	187	196				
16:00 16:15	0	0	0	0	5	0	12	17	17	4	70	0	74	0	110	+ 8	127	201	218				
16:15 16:30	0	0	0	0	4	0	13	17	17	+ 8	71	0	70	0	100	5	105	184	201				
16:30 16:45	0	0	0	0	7	0	7	14	14	3	72	0	75	0	92	2	94	169	183				
16:45 17:00	0	0	0	0	4	0	Δ	8	8	3	87	0	90	0	107	2	110	200	208				
17:00 17:15	0	0	0	0	- 3	0	- <del>-</del>	12	12	5	ga	0	104	0	98	3	101	205	217				
17:15 17:30	0	0	0	0	1	0	4	5	5	7	100	0	107	0	89	4	93	200	205				



# Turning Movement Count - Study Results ERIC CZAPNIK WAY @ ST. JOSEPH BLVD

Survey Date: Thursday, April 23, 2015														wo	No:		35086						
Start Time: 07:00														Dev	ice:			Miovision					
17:30	17:45	0	0	0	0	1	0	5	6	6	6	72	0	78	0	105	2	107	185	191			
17:45	18:00	0	0	0	0	1	0	4	5	5	2	83	0	85	0	101	4	105	190	195			
18:00	18:15	0	0	0	0	0	0	10	10	10	1	54	0	55	0	113	2	115	170	180			
18:15	18:30	0	0	0	0	1	0	3	4	4	0	54	0	54	0	83	2	85	139	143			
18:30	18:45	0	0	0	0	1	0	7	8	8	4	39	0	43	0	81	2	83	126	134			
18:45	19:00	0	0	0	0	0	0	9	9	9	5	65	0	70	0	59	3	62	132	141			
Total:		0	0	0	0	87	0	321	408	408	197	2544	0	2741	1	3844	178	4023	408	7,172			

Note: U-Turns are included in Totals.



### Turning Movement Count - Peak Hour Diagram OLD TENTH LINE RD/OR174 IC101 RAMP63 @ ST. JOS



Comments



### Turning Movement Count - Peak Hour Diagram OLD TENTH LINE RD/OR174 IC101 RAMP63 @ ST. JOS



Comments
## **APPENDIX E**

**Collision Records** 



Location: OLD TI	ENTH LINE R	D/OR174 IC101	RAMP63 @ ST. JOS						
Traffic Control: Tra	ffic signal						Total Collisions:	23	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	r Vehicle type	First Event	No. Ped
2016-Jan-05, Tue,13:58	Clear	Angle	Non-fatal injury	Packed snow	East	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Feb-18, Thu,08:03	Clear	Angle	P.D. only	Wet	East	Going ahead	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Aug-14, Sun,20:16	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Going ahead	Passenger van	Other motor vehicle	
2016-Oct-14, Fri,11:44	Clear	Sideswipe	P.D. only	Dry	East	Changing lanes	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Oct-14, Fri,22:18	Clear	Rear end	Non-fatal injury	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2017-Feb-12, Sun,16:23	Snow	Angle	P.D. only	Slush	East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Apr-05, Wed, 12:01	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Pick-up truck	Other motor vehicle	
2017-May-16, Tue,21:56	Clear	SMV other	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Ran off road	0
2017-May-28, Sun,20:30	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Pick-up truck	Other motor vehicle	
2017-Jun-23, Fri,08:51	Rain	Angle	Non-fatal injury	Wet	East	Going ahead	Passenger van	Other motor vehicle	0
					South	Going ahead	Pick-up truck	Other motor vehicle	
2017-Jun-30, Fri,08:30	Clear	Angle	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Sep-29, Fri,15:00	Clear	SMV other	P.D. only	Dry	East	Turning right	Automobile, station wagon	Debris falling off vehicle	0



Location: OLD T	ENTH LINE RE	D/OR174 IC101 RA	MP63 @ ST. JO	S					
Traffic Control: Tra	ffic signal						Total Collisions:	23	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2017-Oct-30, Mon,16:40	Clear	Angle	Non-fatal injury	Dry	South	Turning left	Passenger van	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
2017-Nov-30, Thu,17:27	Snow	Turning movement	P.D. only	Wet	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Pick-up truck	Other motor vehicle	
2018-Jan-08, Mon,14:52	Snow	SMV other	P.D. only	Loose snow	South	Slowing or stoppin	g Automobile, station wagon	Skidding/sliding	0
2018-Jan-08, Mon,17:41	Snow	Angle	P.D. only	Loose snow	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jan-22, Mon,16:30	Snow	SMV other	P.D. only	Ice	South	Slowing or stoppin	g Automobile, station wagon	Pole (sign, parking met	er) 0
2018-Feb-08, Thu,18:10	Clear	Rear end	P.D. only	Wet	East	Stopped	Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stoppin	g Pick-up truck	Other motor vehicle	
2018-Nov-12, Mon,07:08	Snow	SMV other	P.D. only	lce	North	Going ahead	Automobile, station wagon	Skidding/sliding	0
2018-Nov-27, Tue, 15:15	Clear	Rear end	P.D. only	Drv	East	Turnina riaht	Automobile, station wagon	Other motor vehicle	0
, - , - ,				,	East	Turning right	Automobile, station wagon	Other motor vehicle	
2018-Dec-14, Fri,14:17	Freezing Rain	Angle	Non-fatal injury	lce	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
	Ū	3	, ,		South	Going ahead	Pick-up truck	Other motor vehicle	
2019-Feb-22, Fri,10:56	Clear	SMV other	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Debris on road	0
2019-Apr-09, Tue,15:02	Snow	Rear end	P.D. only	Loose snow	South	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					South	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	
Location: ST. JO	SEPH BLVD @	D TENTH LINE RD	)						
Traffic Control: Tra	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped



Location: ST. JO	SEPH BLVD (	@ TENTH LINE	RD							
Traffic Control: Tra	ffic signal				Total Collisions: 97					
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped		
2015-Jan-14, Wed, 17:27	Clear	Rear end	P.D. only	Ice	East	Slowing or stopping Automobile, station wage	on Other motor vehicle	0		
					East	Stopped Pick-up truck	Other motor vehicle			
2015-Jan-27, Tue,08:12	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping Passenger van	Other motor vehicle	0		
					North	Slowing or stopping Passenger van	Other motor vehicle			
					North	Slowing or stopping Automobile, station wage	n Other motor vehicle			
					North	Slowing or stopping Automobile, station wage	n Other motor vehicle			
					North	Slowing or stopping Pick-up truck	Other motor vehicle			
					North	Slowing or stopping Automobile, station wage	n Other motor vehicle			
2015-Feb-08, Sun,12:49	Snow	Angle	P.D. only	Ice	East	Unknown Unknown	Other motor vehicle	0		
					South	Going ahead Pick-up truck	Other motor vehicle			
2015-Feb-17, Tue,05:54	Clear	Rear end	P.D. only	lce	North	Slowing or stopping Automobile, station wage	on Other motor vehicle	0		
					North	Stopped Pick-up truck	Other motor vehicle			
2015-Feb-17, Tue,06:37	Clear	Sideswipe	P.D. only	Ice	North	Changing lanes Automobile, station wage	n Other motor vehicle	0		
					North	Going ahead Pick-up truck	Other motor vehicle			
2015-Feb-17, Tue,16:22	Clear	Rear end	P.D. only	Dry	North	Going ahead Automobile, station wage	n Other motor vehicle	0		
					North	Slowing or stopping Pick-up truck	Other motor vehicle			
2015-Feb-18, Wed, 17:18	Clear	Rear end	Non-fatal injury	Wet	North	Going ahead Pick-up truck	Other motor vehicle	0		
					North	Slowing or stopping Automobile, station wage	n Other motor vehicle			
					North	Stopped Automobile, station wage	n Other motor vehicle			
					North	Stopped Automobile, station wage	n Other motor vehicle			
					North	Stopped Automobile, station wage	on Other motor vehicle			
2015-Feb-24, Tue,08:25	Clear	SMV other	P.D. only	lce	North	Slowing or stopping Passenger van	Skidding/sliding	0		
2015-Mar-01, Sun,13:55	Clear	Rear end	P.D. only	Dry	North	Going ahead Automobile, station wage	n Other motor vehicle	0		
					North	Stopped Pick-up truck	Other motor vehicle			



Location: ST. JO	SEPH BLVD (	@ TENTH LINE	RD					
Traffic Control: Tra	ffic signal					Total Collisions	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped
2015-Jun-28, Sun,14:47	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					North	Stopped Automobile, station wagon	Other motor vehicle	
2015-Aug-31, Mon,08:00	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					North	Stopped Automobile, station wagon	Other motor vehicle	
2015-Sep-01, Tue,10:31	Clear	Rear end	Non-fatal injury	Dry	North	Slowing or stopping Pick-up truck	Other motor vehicle	0
					North	Stopped Pick-up truck	Other motor vehicle	
2015-Sep-24, Thu,12:11	Clear	Sideswipe	P.D. only	Dry	South	Changing lanes Automobile, station wagon	Other motor vehicle	0
					South	Turning right Pick-up truck	Other motor vehicle	
2015-Oct-04, Sun,07:59	Clear	Rear end	P.D. only	Dry	North	Going ahead Automobile, station wagon	Other motor vehicle	0
					North	Stopped Automobile, station wagon	Other motor vehicle	
2015-Oct-04, Sun,12:55	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping Pick-up truck	Other motor vehicle	0
					North	Stopped Automobile, station wagon	Other motor vehicle	
2015-Oct-09, Fri,11:05	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stopping Automobile, station wagon	Other motor vehicle	
2015-Oct-15, Thu,06:52	Clear	Rear end	P.D. only	Dry	North	Going ahead Pick-up truck	Other motor vehicle	0
					North	Stopped Automobile, station wagon	Other motor vehicle	
2015-Oct-30, Fri,06:37	Clear	Rear end	P.D. only	Wet	North	Slowing or stopping Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stopping Automobile, station wagon	Other motor vehicle	
2015-Nov-27, Fri,10:50	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping Passenger van	Other motor vehicle	0
					North	Stopped Pick-up truck	Other motor vehicle	
2015-Nov-27, Fri,19:46	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping Pick-up truck	Other motor vehicle	0
					North	Stopped Pick-up truck	Other motor vehicle	
2015-Dec-17, Thu,07:40	Rain	Rear end	P.D. only	Wet	North	Stopped Pick-up truck	Other motor vehicle	0
					North	Slowing or stopping Automobile, station wagon	Other motor vehicle	



Location: ST. JO	SEPH BLVD (	② TENTH LINE RD	)						
Traffic Control: Tra	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2015-Dec-23, Wed, 18:41	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2015-Dec-24, Thu,20:13	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Jan-20, Wed,17:04	Clear	Angle	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Passenger van	Other motor vehicle	
2016-Jan-31, Sun,10:19	Clear	Rear end	P.D. only	Wet	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Feb-05, Fri,09:50	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Feb-12, Fri,14:55	Clear	Rear end	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Pick-up truck	Other motor vehicle	
2016-Feb-20, Sat,15:30	Rain	Rear end	P.D. only	Slush	South	Going ahead	Police vehicle	Other motor vehicle	0
					South	Slowing or stopping	Passenger van	Other motor vehicle	
2016-Feb-25, Thu,19:00	Snow	SMV other	P.D. only	lce	East	Turning right	Pick-up truck	Skidding/sliding	0
2016-Mar-07, Mon,17:53	Clear	Rear end	Non-fatal injury	Wet	North	Slowing or stopping	Pick-up truck	Other motor vehicle	0
					North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Pick-up truck	Other motor vehicle	
					North	Stopped	Pick-up truck	Other motor vehicle	
2016-Mar-10, Thu,13:25	Clear	SMV other	Non-fatal injury	Wet	North	Slowing or stopping	Delivery van	Other	0
2016-Mar-12, Sat,15:20	Clear	Turning movement	P.D. only	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	



Location: ST. JOS	SEPH BLVD @	D TENTH LINE	RD						
Traffic Control: Traf	fic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2016-Apr-22, Fri,15:26	Clear	Rear end	P.D. only	Dry	East	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Turning right	Automobile, station wagon	Other motor vehicle	
2016-May-07, Sat,11:56	Clear	Rear end	P.D. only	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2016-Jun-30, Thu,12:34	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	j Truck - open	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jul-09, Sat,11:02	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping	JPick-up truck	Other motor vehicle	0
					North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2016-Jul-11, Mon,15:47	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Aug-26, Fri,12:28	Clear	Rear end	Non-fatal injury	Dry	North	Turning right	Delivery van	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
					North	Turning right	Pick-up truck	Other motor vehicle	
2016-Sep-10, Sat,11:03	Clear	Rear end	Non-fatal injury	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Sep-28, Wed,17:50	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2016-Oct-05, Wed,09:17	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Dec-31, Sat,21:19	Snow	Angle	P.D. only	Loose snow	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	



Location: ST. JO	SEPH BLVD (	@ TENTH LINE	RD						
Traffic Control: Tra	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2017-Jan-04, Wed, 15:15	Snow	Sideswipe	P.D. only	Slush	North	Going ahead	Automobile, station wagon	Skidding/sliding	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jan-16, Mon,11:41	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Passenger van	Other motor vehicle	
2017-Feb-10, Fri,10:33	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Mar-15, Wed, 17:17	Clear	Angle	Non-fatal injury	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Pick-up truck	Other motor vehicle	
2017-Mar-17, Fri,14:18	Clear	Rear end	P.D. only	Dry	North	Changing lanes	Passenger van	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Apr-26, Wed,06:19	Rain	Rear end	P.D. only	Wet	North	Turning left	Pick-up truck	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2017-May-27, Sat,17:48	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2017-May-29, Mon,15:33	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping	Truck - dump	Other motor vehicle	0
					North	Slowing or stopping	Pick-up truck	Other motor vehicle	
2017-Jun-23, Fri,17:40	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping	Pick-up truck	Skidding/sliding	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2017-Jun-25, Sun,13:20	Rain	Rear end	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2017-Jul-07, Fri,08:25	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	



Location: ST. JOS	SEPH BLVD @	① TENTH LINE RD	)						
Traffic Control: Trat	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2017-Jul-19, Wed,21:00	Clear	Sideswipe	P.D. only	Dry	East	Turning right	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Aug-03, Thu,11:25	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Oct-12, Thu,19:35	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Dec-02, Sat,18:13	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Delivery van	Other motor vehicle	
2017-Dec-06, Wed,11:15	Clear	Rear end	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Pick-up truck	Other motor vehicle	
2017-Dec-14, Thu,21:44	Clear	Rear end	Non-fatal injury	Loose snow	North	Slowing or stopping	Automobile, station wagon	Skidding/sliding	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Dec-16, Sat,03:57	Snow	SMV other	P.D. only	Loose snow	South	Turning right	Automobile, station wagon	Skidding/sliding	0
2017-Dec-18, Mon,17:38	Snow	Rear end	Non-fatal injury	Loose snow	East	Slowing or stopping	Pick-up truck	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Dec-23, Sat,15:55	Snow	Rear end	P.D. only	Loose snow	East	Slowing or stopping	J Truck - open	Skidding/sliding	0
					East	Stopped	Passenger van	Other motor vehicle	
2017-Dec-23, Sat,16:09	Snow	Rear end	P.D. only	Loose snow	West	Going ahead	Automobile, station wagon	Skidding/sliding	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Dec-26, Tue,18:45	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Dec-28, Thu,10:30	Clear	Rear end	P.D. only	Ice	West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	



Location: ST. JO	SEPH BLVD @	D TENTH LINE	RD						
Traffic Control: Tra	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Dec-28, Thu,11:27	Clear	Rear end	P.D. only	Ice	North	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-02, Tue,13:29	Snow	Rear end	P.D. only	Loose snow	East	Going ahead	Passenger van	Skidding/sliding	0
					East	Stopped	Pick-up truck	Other motor vehicle	
2018-Jan-10, Wed,14:30	Clear	Rear end	P.D. only	lce	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-12, Fri,17:12	Freezing Rain	Rear end	P.D. only	Packed snow	East	Slowing or stopping	g Automobile, station wagon	Skidding/sliding	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jan-14, Sun,10:20	Clear	Rear end	P.D. only	lce	North	Slowing or stopping	g Automobile, station wagon	Skidding/sliding	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Mar-10, Sat, 10:52	Clear	Angle	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Apr-15, Sun,12:35	Clear	Rear end	Non-fatal injury	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-07, Thu,09:15	Clear	Rear end	P.D. only	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-13, Wed,08:13	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
2018-Jun-19, Tue,11:03	Clear	Rear end	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Slowing or stopping	g Automobile, station wagon	Other motor vehicle	
					East	Stopped	Pick-up truck	Other motor vehicle	



Location: ST. JO	SEPH BLVD (	① TENTH LINE RD	)						
Traffic Control: Tra	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2018-Jun-22, Fri,13:15	Clear	Rear end	P.D. only	Dry	North	Slowing or stoppin	g Truck - closed	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Jun-27, Wed,13:12	Clear	SMV other	Non-fatal injury	Dry	East	Slowing or stoppin	g Motorcycle	Other	0
2018-Jul-24, Tue,09:35	Rain	Rear end	P.D. only	Wet	North	Turning left	Passenger van	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Sep-06, Thu,20:22	Clear	Turning movement	Non-fatal injury	Dry	West	Turning left	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Sep-29, Sat,08:40	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Unknown	Pick-up truck	Other motor vehicle	
2018-Oct-11, Thu,18:40	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Oct-26, Fri,21:19	Clear	Turning movement	P.D. only	Dry	East	Turning left	Passenger van	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Dec-13, Thu,23:39	Clear	SMV other	P.D. only	Dry	East	Turning right	Automobile, station wagon	Curb	0
2018-Dec-20, Thu,13:55	Clear	Angle	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jan-20, Sun,22:00	Snow	SMV other	P.D. only	Packed snow	North	Turning right	Automobile, station wagon	Skidding/sliding	0
2019-Jan-29, Tue,08:20	Snow	Sideswipe	P.D. only	Slush	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Feb-27, Wed,08:24	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	



From: January 1, 2015 To: December 31, 2019

**Total Collisions: 6** 

Location: ST. JO	SEPH BLVD (	@ TENTH LINE	RD						
Traffic Control: Trat	ffic signal						Total Collisions:	97	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2019-Mar-06, Wed, 18:12	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Mar-22, Fri,07:45	Rain	Rear end	P.D. only	Wet	West	Going ahead	Delivery van	Other motor vehicle	0
					West	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2019-May-03, Fri,14:40	Rain	Rear end	Non-fatal injury	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-22, Sat,14:25	Clear	Rear end	P.D. only	Dry	North	Going ahead	Passenger van	Other motor vehicle	0
					North	Stopped	Passenger van	Other motor vehicle	
2019-Sep-20, Fri,06:24	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Sep-25, Wed,06:30	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Oct-06, Sun,14:20	Clear	Sideswipe	P.D. only	Dry	South	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					South	Unknown	Unknown	Other motor vehicle	
2019-Oct-25, Fri,18:32	Clear	Rear end	P.D. only	Dry	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Nov-03, Sun,11:30	Clear	Sideswipe	P.D. only	Dry	South	Changing lanes	Pick-up truck	Other motor vehicle	0
					South	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Dec-04, Wed,09:38	Snow	Sideswipe	P.D. only	Wet	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
ocation: ST JO		otwn PRESTON	DR & TENTH LINE	RD					

Date/Day/Time Environment Impact Type Classification Surface Veh. Dir Vehicle Manoeuver Vehicle type First Event No. Ped Cond'n

Traffic Control: No control



Location: ST. JO	SEPH BLVD b	twn PRESTONE	E DR & TENTH LINE	E RD					
Traffic Control: No	control						Total Collisions:	6	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Feb-15, Mon,07:17	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Pick-up truck	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2016-Jun-11, Sat,09:47	Rain	SMV other	Non-fatal injury	Wet	West	Slowing or stopping	g Motorcycle	Curb	0
2017-Oct-17, Tue,14:20	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jan-06, Sat,17:00	Clear	SMV other	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Snowbank/drift	0
2018-Apr-10, Tue,19:03	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Unknown	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jun-05, Wed,07:36	Clear	SMV other	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Ran off road	0
Location: ST. JO	SEPH BLVD b	twn TENTH LIN	E RD & OR174 IC1	01 RAMP63					
Traffic Control: No	control						Total Collisions:	3	
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Feb-11, Wed,09:50	Clear	SMV other	P.D. only	Loose snow	East	Going ahead	Automobile, station wagon	Ditch	0
2015-Jun-26, Fri,14:45	Clear	SMV other	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Ran off road	0
2017-Aug-28, Mon,17:16	Clear	SMV other	P.D. only	Dry	East	Going ahead	Truck - dump	Pole (utility, power)	0

## **APPENDIX F**

Other Area Developments

## **OTHER AREA DEVELOPMENTS**

Hillside Vista – Phase One







## **OTHER AREA DEVELOPMENTS**

Hillside Vista Walk-Up Condos



cycle to process vehicles in queue. All other movements at the St. Joseph/Tenth Line intersection currently operate with acceptable queues.

### 3.0 DEMAND FORECASTING

### 3.1 Background Traffic

As mentioned previously, a TB was prepared by Novatech, dated November 2013, for Phase 1A of the OTC East Residential Development lands (City file # D07-12-13-0208). Given the most recent traffic volumes at the St. Joseph/Tenth Line intersection do not capture the projected Phase 1A site-generated traffic volumes summarized in the November 2013 TB, the subsequent future background traffic projections will include a revised site trip generation (assuming the same trip generation rates, modal share values, trip distribution and assignment outlined in Sections 3.2 and 3.3) for 44 townhome dwelling units, currently being constructed.

In addition to the revised site trip generation, it is assumed that approximately half the projected Phase 1A site-generated traffic will take advantage of the new full-movement connection to St. Joseph Boulevard. For analysis purposes, this is considered a reasonable assumption, given the proximity of the new St. Joseph Boulevard connection to Phase 1A, the proximity/location of the OR174 on/off-ramps, etc. The new full-movement St. Joseph Boulevard connection (west of Tenth Line Road) is depicted as Vieux-Silo Street on the proposed Site Plan (**Figure 2**).

With regard to general background traffic growth, the November 2013 TB included a 1% annual growth rate, applied to existing traffic volumes along St. Joseph Boulevard, to account for area development. As such, the subsequent analysis will also include a 1% annual growth rate applied to existing background traffic volumes along St. Joseph Boulevard. For the purpose of this assessment, the expected date of full occupancy is assumed to be 2018, and the resultant future background traffic volumes (i.e. 1% annual background traffic growth and projected site-generated traffic for Phase 1A) at study area intersections are depicted on **Figure 5**.

### Figure 5: Future Background Peak Hour Traffic Volumes



### 3.2 Projected Site-Generated Traffic

The following **Table 2** summarizes the projected peak hour vehicle trip generation for 90 residential condominium dwelling units, proposed for a portion of Phase 1B of the OTC East

### 3.3 Projected Site-Generated Traffic Distribution and Assignment

Consistent with the assumptions of the November 2013 TB, the following distribution of sitegenerated traffic to/from the subject site was assumed:

55%	to/from the west via OR174;
18%	to/from the west via St. Joseph Boulevard;
5%	to/from the east via St. Joseph Boulevard;
5%	to/from the east via OR174;
2%	to/from the north via Tenth Line Road; and
15%	to/from the south via Tenth Line Road.
100%	

Based on the foregoing assumed distribution, the following **Figure 6** depicts the projected additional site-generated traffic (i.e. traffic generated by the currently proposed Phase 1B, comprised of 90 condominium units) assigned to the study area network. It should be noted, it is assumed that approximately half the projected traffic generated by Phase 1B will take advantage of the existing full-movement connection to St. Joseph Boulevard (i.e. residents will use the existing St. Joseph/Eric Czapnik intersection), similar to the background traffic assumptions made Phase 1A.

A description of the trip assignment assumptions is provided as follows. The assumptions are based on the adjacent roadway pattern, existing operating conditions at the Tenth Line Road/St. Joseph Boulevard intersection, and the principles of logical trip routing.

- Trips to/from the north via Tenth Line Road or to/from the west via OR174 assigned equally between the Vieux Silo Street and Eric Czapnik Way,
- Trips to/from the west via St. Joseph Boulevard assigned to Vieux-Silo Street,
- Trips to/from the east via St. Joseph Boulevard assigned to Eric Czapnik Way,
- Trips to the east via OR174 assigned to Eric Czapnik Way; trips from the east via OR174 assigned to Vieux-Silo Street, and
- Trips to the south via Tenth Line Road assigned to 2/3 to Vieux-Silo Street and 1/3 to Eric Czapnik Way; trips from the south via Tenth Line Road assigned to Eric Czapnik Way.

### Figure 6: Projected Site-Generated Traffic



## **OTHER AREA DEVELOPMENTS**

211 Centrum Boulevard

## 1 Screening

This study has been prepared according to the City of Ottawa's 2017 Transportation Impact Assessment (TIA) Guidelines. Accordingly, a Step 1 Screening Form has been prepared and is included as Appendix A, along with the Certification Form for the TIA Study PM. As shown in the Screening Form, the trip generation, location and safety triggers were met and a TIA is required including the Design Review component and the Network Impact Component.

## 2 Existing and Planned Conditions

### 2.1 Proposed Development

The proposed development located at 211 Centrum Boulevard, currently zoned as Mixed-Use Centre (MC), is planned to include 397 retirement home units across one nine- and one 17-storey building connected by a fourstorey podium to be built in a single phase for occupancy by 2024. The proposed design includes 282 underground parking spaces and 21 space surface lot accommodating visitor parking. Access to the underground garage will be via an access to Brisebois Crescent and an access to the surface lot will be via a drop-off loop on Brisebois Crescent. Figure 1 illustrates the Study Area Context and Figure 2 illustrates the proposed concept plan.



Source: http://maps.ottawa.ca/geoOttawa/ Accessed: December 4, 2019



### 5.3 Trip Assignment

Using the distribution outlined above, turning movement splits, and access to major transportation infrastructure, the trips generated by the site have been assigned to the Study Area road network. Figure 9 illustrates the new site generated volumes.



## 6 Background Network Travel Demand

### 6.1 Transportation Network Plans

The transportation network plans were discussed in Section 2.3.1 and are not anticipated to impact to site, trip generation, or distribution.

### 6.2 Background Growth

A review of the background projections from the City's TRANS Regional Model for the 2011 and 2031 horizons was completed to determine the background growth for each of the study area roadways. Table 13 summarizes the results of the model and the projections are provided in Appendix E.



## **OTHER AREA DEVELOPMENTS**

3030 St. Joseph Boulevard





#### Table 5: Mid-Rise Apartment Trip Generation

Travel Mede	Mode Share	AM Peak (Person Trips/h)			PM Peak (Person Trips/h)		
		In	Out	Total	In	Out	Total
Auto Driver	50%	9	21	30	22	16	38
Auto Passenger	15%	3	7	10	7	5	12
Transit	25%	5	10	15	10	8	18
Non-motorized	10%	1	4	5	4	3	7
Total Person Trips	100%	18	42	60	43	32	75
Total 'New' Auto Trips		9	21	30	22	16	38

**Table 6: Total Site Vehicle Trip Generation** 

Land Lise	AM Peak (veh/h)			PM Peak (veh/h)		
	In	Out	Total	In	Out	Total
Specialty Retail	7	6	13	11	14	25
Mid-Rise Apartments	9	21	30	22	16	38
Less Retail Pass-by (25%)	-2	-2	-4	-3	-3	-6
Total Site-Generated Auto Trips	14	25	39	30	27	57

As shown in Table 6, the resulting number of potential 'new' two-way vehicle trips for the proposed development is approximately 40 and 60 veh/h during the weekday morning and afternoon peak hours, respectively.

### 5.2. ASSIGNMENT OF SITE-GENERATED TRAFFIC

As the only vehicular connection to the proposed development and its garage is via the proposed right-in/right-out driveway to St. Joseph Boulevard, the assignment of peak hour site-generated traffic is very straight forward, as depicted in Figure 4. At the two Place d'Orleans intersections east and west of the site driveway, site-generated traffic was distributed based on a combination of the current distribution proportions and OR174 access.



Figure 4: Assignment of Site-Generated Traffic

## **OTHER AREA DEVELOPMENTS**

Petrie's Landing

#### Petrie's Landing I

Brigil is proposing the construction of the remainder 4 of 6 total residential Towers, consisting of approximately 806 additional residential units and 1,500 sq. meters of commercial. The proposed Petrie's Landing I is located on Jeanne D'Arc Boulevard, approximately 1.5 km east of the subject site, as illustrated in **Figure 5**. Currently, Tower I has been built and Tower II is nearing completion. The projected two-way vehicle trips for this proposed residential development are approximately 70 to 65 veh/h for Tower II and 210 to 180 veh/h for Towers III-VI during the AM and PM peak hours respectively.

#### Petrie's Landing III

Brigil is proposing the construction of a mixed-use development consisting of approximately 370,000 ft<sup>2</sup> of office, 23,000 ft<sup>2</sup> of retail and up to 790 residential units. The proposed Petrie's Landing III is located on Jeanne D'Arc Boulevard, approximately 500 meters east of the subject site, as illustrated in **Figure 5**. The projected two-way vehicle trips for this proposed mixed-use development is approximately 660 and 685 veh/h during the morning and afternoon peak hours, respectively.



#### Figure 5: Petrie's Landing I, II and III Concept Plan

#### Cardinal Creek Village

Tamarack Homes is currently constructing a 1,446-unit subdivision and a 430,000 ft<sup>2</sup> shopping centre, south of OR-174 and east of Cardinal Creek, as illustrated in **Figure 6**. The Transportation Impact Study (prepared by IBI Group) projected approximately 1,460 veh/h and 2,619 veh/h by horizon year 2031 (full build-out) during the morning and afternoon peak hours, respectively.

Based on these distributions, 'new' site-generated trips to/from the proposed development are assigned to study area intersections and are illustrated as **Figure 8**.



Figure 8: Site-Generated Traffic (Block 8)

### 3.2. BACKGROUND NETWORK TRAVEL DEMAND

#### 3.2.1. TRANSPORTATION NETWORK PLANS

The transportation network changes have been discussed within Section 2.1.3., and none were anticipated to impact the transportation analysis for this development.

#### 3.2.2. BACKGROUND GROWTH

A 2% annual background traffic growth has been added along the Jeanne D'Arc Boulevard through movements to anticipate future development growth along the corridor. Given that Jeanne D'Arc Boulevard between Tenth Line Road and Trim Road (arterials on each side of the study area) are bound by OR 174 and the Ottawa River, a 2% background growth is conservative. Known future developments were superimposed on top of the 2% annual growth and are described in section 3.2.3.

#### 3.2.3. OTHER AREA DEVELOPMENTS

Other area developments were outlined in **Section 2.1.3**. Trips generated by these developments have been summarized in **Table 6**.

	AM Peak (persons/h)			PM Peak (persons/h)		
	In	Out	Total	In	Out	Total
Petrie's Landing I	72	210	282	144	101	245
Petrie's Landing II – Blocks 6 & 7	11	35	46	48	30	78
Petrie's Landing III	422	237	659	254	430	584
Cardinal Creek (External Only)	412	940	1,352	1,246	980	2,226
Total	917	1,422	2,339	1,692	1,541	3,233

#### Table 6: Other Area Developments Vehicle Trip Generation

#### Petrie's Landing I – Tower II to VI

Petrie's Landing I – Tower II to VI are expected to be fully occupied by 2024. For a more conservative analysis, all Towers were superimposed to background 2022 and forward. The projected traffic volumes are illustrated in **Figure 9**.



Figure 9: Petrie's Landing I Tower II - IV Projected Traffic Volumes

#### Petrie's Landing III

**Figure 10** illustrates the projected traffic volumes for Petrie's Landing III at full build-out, obtained from the 2013 Petrie's Landing I TIS. Considering assumed time horizons, 30% of build-out volumes will be applied in year 2022, and 100% in year 2027.

Figure 10: Petrie's Landing III Projected Traffic Volumes - Full Build-Out



## **APPENDIX G**

Relevant Excerpts of TRANS Trip Generation Manual (WSP, 2020)

to make use of this resource while considering the local land use context and trip characteristics for all travel modes through local and regional data.

Factor	Application	Apply To	Period	Value
Person-Trip Conversion Factor	Vehicle to person-trip conversion, to normalize the measure of trip rates to account for all modes. Applicable to the ITE trip generation rates, which are mainly reported as vehicle trip rates.	Vehicle trip rates	All	1.28

### Table 2: Person-Trip Conversion Factor

# **3 RESIDENTIAL TRIP GENERATION RATES**

## 3.1 Development of Residential Trip Rates

The residential trip generation rates in this manual are reflect the number of **person-trips per household** during the **peak period**. The morning peak period is from 7:00 AM to 9:30 AM, while the afternoon peak period is from 3:30 PM to 6:00 PM.

A geographic review of trip generation rates found that rates varied by dwelling type but not significantly by the geographic sectors and districts used in the 2009 TRANS Trip Generation Study<sup>1</sup>. As such, residential trip generation rates in this manual are defined for the following three dwelling types:

- Single-Family Detached Housing
- Multifamily Housing (Low-Rise)
- Multifamily Housing (High-Rise)

Low-rise housing refers to any building that houses multiple families that is two storeys or less (e.g. semi-detached homes, townhouses). High-rise housing refers to any building that houses multiple families that is three or more storeys (e.g. apartments and condo buildings). These dwelling types are from the TRANS Origin-Destination Survey but are organized to be equivalent to the categories of the ITE *Trip Generation Manual* and local generator surveys.

<sup>&</sup>lt;sup>1</sup> While person trip rates were not found to vary significantly with geographic area, location does have an impact on mode share as discussed in Section 4.2. As a result, vehicular trip rates do vary by geography as reflected in previous versions of the manual. The variation by dwelling type, in part, reflects differences in the number of persons per dwelling.

## 3.2 Recommended Residential Trip Generation Rates

A blended trip rate was developed from the three data sources through application of a rank-sum weighting process, considering the strengths and weaknesses of each dataset for the dwelling type in question. The recommended blended **residential person-trip rates** are presented in **Table 3**. All rates represent person-trips per dwelling unit and are to be applied to the **AM or PM peak period**.

ITE Land Use Code	Dwelling Unit Type	Period	Person-Trip Rate
210	Single detected	AM	2.05
210	Single-detached	PM	2.48
220	Multi I Ipit (Low Pice)	AM	1.35
220	Multi-Offit (LOW-Rise)	PM	1.58
221 8 222	Multi I Init (High Diso)	AM	0.80
221 & 222	Multi-Offic (High-Rise)	PM	0.90

Table 3: Recommended Residential Person-trip Rates

## 3.3 Adjustment Factors – Peak Period to Peak Hour

The various trip generation data sources require some adjustment to standardize the data for developing robust blended trip rates. The peak period conversion factor in **Table 4** may be used where applicable to develop trip generation rate estimates in the desired format.

### Table 4: Adjustment Factors for Residential Trip Generation Rates

Factor	Application	Apply To	Period	Value
	Peak period to peak hour conversion. Because the 2020 TRANS Trip Generation Study reports trip generation rates by peak period, factors must be applied if the practitioner requires peak hour rates. In practice, the conversion to peak hour trip rates should occur <b>after</b> the application of modal shares.	Person-trip	AM	0.50
		rates per peak period	PM	0.44
		Vehicle trip	AM	0.48
		rates per peak period	PM	0.44
Peak Period		Transit trip	AM	0.55
Factor		rates per peak period	PM	0.47
		Cycling trip	AM	0.58
		period	PM	0.48
		Walking trip	AM	0.58
		rates per peak period	PM	0.52



Figure 1: National Capital Region by Sector
### Table 8: Residential Mode Share for High-Rise Multifamily Housing

		Mode									
District	Period	Auto	Auto		0						
		Driver	Pass.	Iransit	Cycling	Walking					
	AM	18%	2%	26%	1%	52%					
Ottawa Centre	PM	17%	9%	21%	1%	52%					
Ottown Inc	AM	26%	6%	28%	5%	34%					
Ottawa Inner Area	PM	25%	8%	21%	6%	39%					
	AM	27%	3%	37%	12%	21%					
lie de Hull	PM	26%	8%	27%	11%	28%					
Ottowe Feet	AM	39%	7%	38%	2%	13%					
Ollawa Easi	PM	40%	14%	28%	3%	15%					
Pagaan Hill	AM	48%	9%	30%	3%	10%					
	PM	52%	16%	28%	0%	4%					
	AM	38%	12%	42%	2%	7%					
Alla VISIa	PM	45%	16%	28%	2%	9%					
	AM	39%	6%	44%	1%	9%					
	PM	44%	11%	35%	2%	9%					
Maringla	AM	41%	6%	42%	2%	8%					
wenvale	PM	41%	11%	33%	2%	13%					
	AM	28%	11%	41%	3%	16%					
Ollawa Wesi	PM	33%	11%	26%	7%	23%					
Bayabara/Cadaryiayy	AM	40%	12%	38%	2%	8%					
Bayshore/Cedarview	PM	40%	15%	33%	1%	11%					
Hull Dárinhária	AM	48%	11%	30%	1%	10%					
Hull Periphene	PM	47%	15%	23%	3%	13%					
Orloopo	AM	54%	7%	29%	0%	10%					
Oneans	PM	61%	13%	21%	0%	6%					
South Gloucester /	AM	50%	15%	25%	1%	9%					
Leitrim	PM	53%	17%	21%	1%	9%					
South Noncon	AM	58%	6%	30%	2%	4%					
South Nepean	PM	54%	15%	25%	0%	21% $28%$ $13%$ $15%$ $10%$ $4%$ $7%$ $9%$ $9%$ $9%$ $9%$ $9%$ $13%$ $16%$ $23%$ $8%$ $11%$ $10%$ $13%$ $10%$ $9%$ $9%$ $4%$ $7%$ $4%$ $5%$ $1%$ $1%$ $1%$ $10%$ $13%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $1%$ $0$ $0$ $0$ $0$ $0$					
Kapata Stittavilla	AM	43%	26%	28%	0%	4%					
	PM	55%	19%	21%	0%	5%					
Plataau	AM	53%	9%	35%	3%	1%					
Flateau	PM	65%	7%	25%	2%	1%					
Avlmer	AM	45%	17%	25%	0%	13%					
Ayimei	PM	31%	21%	23%	4%	20%					
Pointe Catineau	AM	44%	15%	24%	3%	14%					
	PM	52%	15%	20%	2%	11%					
Gatineau Est	AM	53%	10%	25%	0%	12%					
	PM	61%	10%	25%	0%	4%					
Masson-Angers	AM	63%	15%	19%	0%	3%					
	PM	64%	18%	16%	0%	1%					
Other Rural Districts	AM	63%	15%	19%	0%	3%					
	PM	64%	18%	16%	0%	1%					

# **5 RESIDENTIAL DIRECTIONAL SPLITS**

After calculating the total person trips generated by the development and applying the appropriate modal shares, directional factors can be applied to estimate the number of inbound and outbound trips by vehicle. The vehicle trip directional splits were developed for both the AM and PM peak periods<sup>2</sup>. The vehicle trip directional splits, as shown in **Table 9**, have been developed for the NCR based on a review of the local trip generator surveys as well as the latest published data in the ITE *Trip Generation Manual* (10<sup>th</sup> Edition).

ITE Land Use Code	Dwelling Unit Type	Period	Inbound	Outbound
210	Single detached	AM	30%	70%
210	Single-detached	PM	62%	38%
220	Multi Llpit (Low Bicc)	AM	30%	70%
220		PM	56%	44%
221 & 222	Multi Unit (High Dicc)	AM	31%	69%
		PM	58%	42%

Table 9: Recommended Vehicle Trip Directional Splits (Peak Period)

## 6 NON-RESIDENTIAL MODE SHARE

Mode shares were developed for three types of non-residential development: schools (elementary and high school); employment generators; and commercial (retail) generators. These mode shares were developed through data provided by the Ville de Gatineau from local school surveys as well as the TRANS Origin-Destination Survey. The non-residential mode shares presented below are limited and do not capture all development types. For data on the travel characteristics associated with colleges and universities, transportation terminals, and sports and entertainment venues in the National Capital Region, practitioners should refer to the various reports for the TRANS *Special Generators Survey* (2013), which are posted on the TRANS website. For other development types, practitioners may need to carry out their own local generator data collection where necessary.

<sup>&</sup>lt;sup>2</sup> A directional split for active transportation was calculated based on the local generator surveys for low-rise and mid-rise land uses. The splits are mostly in-line with the vehicle directional splits, which could be used as a rough assumption for areas with lower vehicle mode share.

### **APPENDIX H**

Strategic Long-Range Model and Intersection Growth Rate Figures





The TRANS model is continuously refined & maintained, and all information is provided in good faith. However, model outputs are provided "as is", and no warranty or guarantee is provided as to the accuracy, reliability or reasonableness of the results. In using this data, you agree to accept any and all risks arising from any incorrect, incomplete, or misleading information.

Recipients are required to use caution and professional judgement in using and interpreting model outputs. In particular, caution should be used when focusing on a geographically limited area (such as a single road or intersection), as the model is primarily designed to simulate regional-scale phenomena and has been calibrated at a regional level.

As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be Ma over- or under-estimating the travel demand.





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As general good practice, it is recommended that the user confirm the network coding within the area of interest, and compare base year forecasts against traffic count data to assess the extent to which the model may be Ma over- or under-estimating the travel demand.

### INTERSECTION TRAFFIC GROWTH RATE, AM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



### INTERSECTION TRAFFIC GROWTH RATE, PM PEAK PERIOD

Total Vehicular Volume Entering the Intersection, 2000 to 2016



## **APPENDIX I**

Signal Timing Plans

### **Traffic Signal Timing**

City of Ottawa, Transportation Services Department											
	Traffic Signal Operations Unit										
Intersection:	Main:	Tenth Line	Side:	St. Joseph							
Controller:	ATC 3		TSD:	6126							
Author:	Matthew	w Anderson	Date:	26-May-2021							

### Existing Timing Plans<sup>+</sup>

	Plan			Ped Minimum Time							
	AM Peak	Off Peak 2	PM Peak 3	Night 4	Walk	DW	A+R				
Cycle	Free	Free	Free	Free							
Offset	-	-	-	-							
NB Thru/Left	max=59.3	max=49.3	max=59.3	max=49.3	7	19	3.7+2.6				
EB Left	max=14	max=14	max=14	-	-	-	3.7+2.3				
WB Left	max=14	max=14	max=14	-	-	-	3.7+2.3				
EB Thru	max=28.1	max=28.1	max=28.1	max=25.1	7	16	3.7+2.4				
WB Thru	max=28.1	max=28.1	max=28.1	max=25.1	7	16	3.7+2.4				
SB Thru/Left	max=28.3	max=28.3	max=28.3	max=28.3	7	19	3.7+2.6				

### Phasing Sequence<sup>‡</sup>



 $\underline{\text{Note:}}$  1) All Plans have a minimum recall on the NB movement of 22 seconds green

Plan

4

2

4

Saturday Time

0:15

7:00

20:00

#### Schedule

Weekday	
Time	Plan
0:15	4
6:30	1
9:30	2
15:00	3
18:30	2
23:30	4

Sunday	
Time	Plan
0:15	4
7:00	2
19:00	4

#### Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn ...

••••• Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

### **Traffic Signal Timing**

City of Ottawa, Transportation Services Department

#### Traffic Signal Operations Unit

Intersection:	Main:	Old Tenth Line / Off-F	lamp	Side:	St. Joseph
Controller:	MS-3200			TSD:	5910
Author:	Matthew /	Anderson		Date:	26-May-2021

### **Existing Timing Plans<sup>†</sup>**

	Plan			Ped Minimum Time							
	AM Peak	Off Peak 2	PM Peak 3	Night 4	Walk	DW	A+R				
Cycle	Free	Free	Free	Free							
Offset	-	-	-	-							
SB Thru	34	30	35	24	7	16	3.7+3.3				
EB Thru	31.6	27.6	26.6	26.6	7	12	3.7+2.9				
WB Thru	31.6	27.6	26.6	26.6	7	12	3.7+2.9				
NB Left (fp)	16.3	16.3	16.3	11.3	-	-	3.7+2.6				
SB Left (fp)	45	40	45	35	-	-	3.7+3.3				

### Phasing Sequence<sup>‡</sup>



Notes: 1) The SB Thru and SB Left have min recalls of 15s and 5s green respectively

### Schedule

Weekday	Weekday			y	Sunday				
Time	Plan		Time	Plan		Time	Plan		
0:15	4	_	0:15	4		0:15	4		
6:30	1	-	7:00	2	-	7:00	2		
9:30	2	-	20:00	4	-	19:00	4		
15:00	3	-							
18:30	2	_							
23:30	4	-							

### Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

◄····· Pedestrian signal

Cost is \$59.96 (\$53.06 + HST)

## **APPENDIX J**

Existing Synchro Analysis

### 1: St. Joseph & Vieux-Silo AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	1	203	741	1	7	3
Future Volume (vph)	1	203	741	1	7	3
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt					0.963	
Flt Protected	0.950				0.965	
Satd. Flow (prot)	1674	3316	3316	0	1638	0
Flt Permitted	0.950			-	0.965	-
Satd, Flow (perm)	1674	3316	3316	0	1638	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	3			3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	1	226	823	1	8	3
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	226	824	0	11	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.0	3.5	5.	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	0 0 0 0					
laters after Oan after Utilized	1 04 70/			10		

Intersection Capacity Utilization 31.7% Analysis Period (min) 15

ICU Level of Service A

### 2: Tenth Line & St. Joseph AM Peak Hour

	٦	-	$\mathbf{F}$	4	+	×	•	t	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<b>X</b>	**	1	<b>N</b>	**	1	<b>X</b>	<b>⊿</b> ↑♠	1	× 1	**	1
Traffic Volume (vph)	16	75	119	37	272	42	416	796	20	7	124	54
Future Volume (vph)	16	75	110	37	272	42	416	796	20	7	124	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	70.0	1000	0.0	160.0	1000	55.0	105.0	1000	60.0
Storage Lanes	0.0		0.0	10.0		0.0	100.0		1	100.0		00.0
Taper Length (m)	30.0		1	25.0		1	25.0		I	35.0		1
	1 00	0.05	1 00	20.0	0.05	1 00	20.0	0.01	1 00	1.00	0.05	1 00
Lane Ulli. Factor	0.00	0.95	0.00	1.00	0.95	0.00	0.91	0.91	1.00	1.00	0.95	1.00
	0.99		0.99	1.00		0.90			0.950			0 050
FIL Fit Drotootod	0.050		0.000	0.050		0.000	0.050	0.000	0.000	0.050		0.000
Fil Protected	0.950	2240	4455	0.950	2240	1111	0.950	0.990	1 1 1 1	0.950	2002	1400
Sato. Flow (prot)	1674	3310	1455	1470	3316	1441	1509	3112	1441	1674	3283	1483
Fit Permitted	0.568	0040	4405	0.630	0040	4.400	0.950	0.998		0.950	0000	4 4 9 9
Satd. Flow (perm)	994	3316	1435	974	3316	1406	1509	3112	1441	1674	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			132			126			125			125
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	7		1	1		7						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	18	83	132	41	302	47	462	884	22	8	138	60
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	18	83	132	41	302	47	416	930	22	8	138	60
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0	Ŭ		7.0	Ū		5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	l eft	Thru	Right	Left	Thru	Right
Leading Detector (m)	61	30.5	61	61	30.5	6 1	61	30.5	6 1	61	30.5	6 1
Trailing Detector (m)	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0	0.1
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.0	6.1	6.1	1.0	0.0	0.0	1.0	6.1	6.1	1.0	6.1
Detector 1 Type												
Detector 1 Channel												
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8		8			6			2
Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

J.Audia, Novatech

Synchro 10 Report

### 2: Tenth Line & St. Joseph AM Peak Hour

	۶	-	$\rightarrow$	4	+	•	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	Min	Min	Min
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		7	7	1	1	1	1	1	1
Act Effct Green (s)	17.8	14.1	14.1	19.6	17.0	17.0	37.7	37.7	37.7	13.2	13.2	13.2
Actuated g/C Ratio	0.19	0.15	0.15	0.21	0.18	0.18	0.41	0.41	0.41	0.14	0.14	0.14
v/c Ratio	0.07	0.16	0.40	0.17	0.49	0.13	0.68	0.73	0.03	0.03	0.29	0.19
Control Delay	31.8	40.8	11.9	32.5	40.4	0.7	31.0	28.6	0.1	42.7	41.8	1.3
Queue Delav	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.8	40.8	11.9	32.5	40.4	0.7	31.0	28.6	0.1	42.7	41.8	1.3
LOS	С	D	В	С	D	A	С	С	A	D	D	A
Approach Delay		23.7			34.8			28.9			30.1	
Approach LOS		С			С			С			С	
Queue Length 50th (m)	2.1	6.2	0.0	4.8	19.9	0.0	56.4	65.2	0.0	1.1	10.6	0.0
Queue Length 95th (m)	9.0	16.2	16.3	16.3	49.9	0.0	132.1	131.9	0.0	5.9	24.0	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	261	891	482	253	893	470	934	1927	940	508	997	538
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.09	0.27	0.16	0.34	0.10	0.45	0.48	0.02	0.02	0.14	0.11
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 92.3												
Natural Cycle: 105												
Control Type: Actuated-Uncoc	ordinated											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 29.5	5			In	tersection	LOS: C						
Intersection Capacity Utilization	on 62.1%			IC	CU Level o	f Service E	3					
Analysis Period (min) 15	Analysis Period (min) 15											
Splits and Phases: 2. Tenth	line & St. Jo	oseph										
										4		

Ø2	<b>₩</b> ø6	Ø	3	Ø4
32.3 s	59.3 s	14 s		29.1s
		▶ 07	,	<b>4</b> <b>Ø</b> 8
		14 s		29.1s

### 3: St. Joseph & Eric Czapnik AM Peak Hour

Lane Group         EBL         EBT         WBT         WBR         SBL         SBR           Lane Configurations         1
Lane Configurations         Image: Configuration for the state of the state o
Traffic Volume (vph)         20         82         333         13         9         40           Future Volume (vph)         20         82         333         13         9         40           Ideal Flow (vphpl)         1800         1800         1800         1800         1800         1800         1800           Storage Length (m)         40.0         0.0         0.0         0.0         0.0           Storage Lanes         1         0         1         0         1         0           Taper Length (m)         30.0         1.00         0.91         1.00         1.00           Lane Util. Factor         1.00         0.95         0.91         0.91         1.00         1.00           Ped Bike Factor         7         0.995         0.890         154         0           Fit Protected         0.950         0.991         0.991         0.911         0         1554         0           Stat. Flow (prot)         1674         3316         4698         0         1554         0           Link Speed (k/h)         60         60         50         1514         0         1514         0
Future Volume (vph)         20         82         333         13         9         40           Ideal Flow (vphpl)         1800         100         100         Taper Length (m)         30.0         10.0         1.00         <
Ideal Flow (vph)         1800         100           Storage Lanes         1         0         0.95         0.91         0.91         1.00
Storage Length (m)         40.0         0.0         0.0         0.0           Storage Lanes         1         0         1         0           Taper Length (m)         30.0         10.0         1.00           Lane Util. Factor         1.00         0.95         0.91         0.91         1.00           Ped Bike Factor         -         <
Storage Lanes         1         0         1         0           Taper Length (m)         30.0         10.0         10.0           Lane Util. Factor         1.00         0.95         0.91         0.91         1.00         1.00           Ped Bike Factor         -         <
Taper Length (m)         30.0         10.0           Lane Util. Factor         1.00         0.95         0.91         0.91         1.00         1.00           Ped Bike Factor
Lane Util. Factor         1.00         0.95         0.91         0.91         1.00         1.00           Ped Bike Factor
Ped Bike Factor         0.995         0.890           Frt         0.950         0.991           Satd. Flow (prot)         1674         3316         4698         0         1554         0           Flt Permitted         0.950         0.991         0.991         0.991         0.951         0.991           Satd. Flow (perm)         1674         3316         4698         0         1554         0           Link Speed (k/h)         60         60         50         0.91         0         0.91         0
Frt         0.995         0.890           Fit Protected         0.950         0.991           Satd. Flow (prot)         1674         3316         4698         0         1554         0           Fit Permitted         0.950         0.991         0.991         0         0         0         0         0           Satd. Flow (perm)         1674         3316         4698         0         1554         0           Link Speed (k/h)         60         60         50         0
Fit Protected         0.950         0.991           Satd. Flow (prot)         1674         3316         4698         0         1554         0           Fit Permitted         0.950         0.991         0.991         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         1674         3316         4698         0         1554         0         0         Link Speed (k/h)         60         60         50         0         1614
Satd. Flow (prot)         1674         3316         4698         0         1554         0           Fit Permitted         0.950         0.991
Fit Permitted         0.950         0.991           Satd. Flow (perm)         1674         3316         4698         0         1554         0           Link Speed (k/h)         60         60         50         50
Satd. Flow (perm)         1674         3316         4698         0         1554         0           Link Speed (k/h)         60         60         50
Link Speed (k/h) 60 60 50
Link Distance (m) 144.1 123.1 184.3
Travel Time (s) 8.6 7.4 13.3
Confl. Peds. (#/hr) 3 3
Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90
Heavy Vehicles (%) 1% 2% 3% 1% 1% 1%
Adi, Flow (vph) 22 91 370 14 10 44
Shared Lane Traffic (%)
Lane Group Flow (vph) 22 91 384 0 54 0
Enter Blocked Intersection No No No No No No
Lane Alignment Left Left Right L NA R NA
Median Width(m) 5.5 5.0 3.5
Link Offset(m) 0.0 0.0 0.0
Crosswalk Width(m) 5.0 10.0 5.0
Two way left Turn Lane
Headway Factor 1.09 1.09 1.09 1.09 1.09 1.09
Turning Speed (k/h) 24 14 14 14
Sign Control Free Free Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized

Intersection Capacity Utilization 24.4%

ICU Level of Service A

Analysis Period (min) 15

### 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

	۶	-	$\mathbf{F}$	4	←	*	•	Ť	۲	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>A1</b>		×.	**		<b>X</b>		1	88	**	1
Traffic Volume (vph)	0	76	0	31	289	0	0	0	101	49	285	50
Future Volume (vph)	0	76	0	31	289	0	0	0	101	49	285	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Storage Lanes	0.0		0.0	1		0.0	1		10.0	2		100.0
Taper Length (m)	10.0		U	35.0		U	10.0			60.0		I
Lane Litil Eactor	1 00	0.05	0.05	1 00	0.05	1 00	1 00	1 00	1 00	00.0	0.05	1 00
Pod Piko Eastor	1.00	0.55	0.35	1.00	0.55	1.00	1.00	1.00	1.00	0.37	0.35	0.00
									0.850			0.99
Fit Protocted				0.050					0.000	0.050		0.000
Fil Fiolecieu	٥	2004	٥	1640	2216	٥	1760	٥	1/02	0.900	2216	1010
Salu. Flow (prot)	U	3221	U	0 700	3310	U	1/02	U	1403	2902	3310	1210
	0	2004	0	0.700	2240	0	1700	0	1100	0.950	2240	1105
Sata. Flow (perm)	U	3221	U	1210	3310	U	1/02	U	1483	2982	3310	1195
Right Turn on Red			res			res			Yes			Yes 420
Satd. Flow (RTOR)		00			00				882		00	132
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	5%	1%	3%	2%	1%	1%	1%	2%	10%	2%	25%
Adj. Flow (vph)	0	84	0	34	321	0	0	0	112	54	317	56
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	84	0	34	321	0	0	0	112	54	317	56
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
Median Width(m)		5.0			4.5			7.0			7.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		Left	Thru		Left		Right	Left	Thru	Riaht
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		61	1.8		6.1		61	61	1.8	6.1
Detector 1 Type		CI+Fx		CI+Fx	CI+Ex		CI+Fx		CI+Ex	CI+Fx	CI+Fx	CI+Fx
Detector 1 Channel		OI LA		OI LA	OI LA		OI' EX			OI LA	OI LA	OI LA
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7		0.0	28.7		0.0		0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Size(iii)												
Detector 2 Channel		CITEX										
Detector 2 Extend (a)		0.0			0.0						0.0	
		0.0		Dorre	0.0		Dret		<b>Free</b>	Dret	0.0	Derry
Protected Decore		NA A		Perm	INA 0		Prot		Free	Prot	NA	Perm
Protected Phases		4		0	ŏ		5		<b>F</b>	1	б	
Permitted Phases		4		8	^		_		Free	4		6
Detector Phase		4		8	8		5			1	6	6
Switch Phase												

J.Audia, Novatech

Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

	≯ →	$\rightarrow$	-	•	1	1	۲	1	Ŧ	-
Lane Group	EBL EBT	EBR WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)	31.6	31.6	31.6		16.3			50.3	34.0	34.0
Total Split (%)	38.6%	38.6%	38.6%		19.9%			61.4%	41.5%	41.5%
Maximum Green (s)	25.0	25.0	25.0		10.0			43.3	27.0	27.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag					Lead				Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	None	None	None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	7.0						7.0	7.0
Flash Dont Walk (s)	12.0	12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)	0	0	0						1	1
Act Effct Green (s)	10.6	10.6	10.6				40.8	16.5	16.5	16.5
Actuated g/C Ratio	0.26	0.26	0.26				1.00	0.40	0.40	0.40
v/c Ratio	0.10	0.11	0.37				0.08	0.04	0.24	0.10
Control Delay	12.6	13.5	14.3				0.1	7.5	8.5	0.5
Queue Delay	0.0	0.0	0.0				0.0	0.0	0.0	0.0
Total Delay	12.6	13.5	14.3				0.1	7.5	8.5	0.5
LOS	В	В	В				А	А	А	А
Approach Delay	12.6		14.3			0.1			7.3	
Approach LOS	В		В			А			А	
Queue Length 50th (m)	2.0	1.6	8.2				0.0	0.9	6.3	0.0
Queue Length 95th (m)	6.3	6.8	19.1				0.0	3.0	12.7	0.5
Internal Link Dist (m)	99.1		222.7			251.2			211.3	
Turn Bay Length (m)		60.0					15.0	110.0		130.0
Base Capacity (vph)	1993	748	2052				1483	2906	2216	842
Starvation Cap Reductn	0	0	0				0	0	0	0
Spillback Cap Reductn	0	0	0				0	0	0	0
Storage Cap Reductn	0	0	0				0	0	0	0
Reduced v/c Ratio	0.04	0.05	0.16				0.08	0.02	0.14	0.07
Intersection Summary										
Area Type: Othe	r									
Cycle Length: 81.9										
Actuated Cycle Length: 40.8										
Natural Cycle: 70										
Control Type: Actuated-Uncoordina	ted									
Maximum v/c Ratio: 0.37										
Intersection Signal Delay: 9.5		I	ntersection	LOS: A						
Intersection Capacity Utilization 32.	5%	I	CU Level o	of Service A						
Analysis Period (min) 15										
Splits and Phases: 4: Old Tenth L	_ine/OR 174 EB F	Ramp & St. Joseph								

Ø1		<b>→</b> Ø4	
50.3 s		31.6 s	
<b>▲</b> Ø5	🕈 Ø6	<b>€</b> Ø8	
16.3 s	34 s	31.6 s	

J.Audia, Novatech

### 1: St. Joseph & Vieux-Silo PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	4	874	556	5	3	2
Future Volume (vph)	4	874	556	5	3	2
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util, Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor		0.00	0.00	0.00		
Frt			0.999		0.946	
Flt Protected	0.950				0.971	
Satd, Flow (prot)	1674	3349	3345	0	1619	0
Flt Permitted	0.950				0.971	
Satd, Flow (perm)	1674	3349	3345	0	1619	0
Link Speed (k/h)	10/1	60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	4	25	2.5	4	0.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%
Adi, Flow (vph)	4	971	618	6	3	2
Shared Lane Traffic (%)	· ·		0.0	3	3	_
Lane Group Flow (vph)	4	971	624	0	5	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	LNA	RNA
Median Width(m)	2011	5.0	3.5	i ugitt	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sign Control	<u> </u>	Free	Free	17	Stop	17
		1100	1100		otop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
				10		· · · ·

Intersection Capacity Utilization 35.5%

ICU Level of Service A

Analysis Period (min) 15

### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	×	**	1	×.	**	1	8	<b>≜1</b> ⊾	1	*	**	1
Traffic Volume (vph)	54	298	525	62	199	148	320	627	13	8	137	42
Future Volume (vph)	54	298	525	62	100	148	320	627	13	8	137	42
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	70.0	1000	0.0	160.0	1000	55.0	105.0	1000	60.0
Storage Lanes	0.0		0.0	10.0		0.0	100.0		1	105.0		00.0
Tapor Longth (m)	30.0		1	25.0		I	25.0		1	35.0		1
	1 00	0.05	1 00	20.0	0.05	1 00	20.0	0.01	1 00	1 00	0.05	1 00
Lane Ulli. Factor	0.00	0.95	0.00	1.00	0.95	0.07	0.91	0.91	0.09	1.00	0.95	1.00
	0.99		0.99	1.00		0.97			0.90	1.00		0.050
FIL Filt Drote stord	0.050		0.000	0.050		0.000	0.050	0.000	0.000	0.050		0.000
Fit Protected	0.950	2240	4455	0.950	2240	4 4 4 4	0.950	0.998	4 4 4 4	0.950	2002	4 4 0 0
Sato. Flow (prot)	1674	3310	1455	1470	3316	1441	1509	3112	1441	1674	3283	1483
Fit Permitted	0.614	0040	4405	0.484	0040	4404	0.950	0.998	4440	0.950	0000	4.400
Satd. Flow (perm)	1070	3316	1435	748	3316	1401	1509	3112	1419	1672	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			583			164			125			125
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	9		1	1		9			2	2		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	60	331	583	69	221	164	356	697	14	9	152	47
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	60	331	583	69	221	164	320	733	14	9	152	47
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0	Ŭ		7.0	Ū		5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	l eft	Thru	Right	Left	Thru	Right	l eft	Thru	Right	l eft	Thru	Right
Leading Detector (m)	61	30.5	61	61	30.5	6.1	61	30.5	6.1	61	30.5	6 1
Trailing Detector (m)	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0	0.1
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type												
Detector 1 Channel												
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector I Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8		8			6			2
Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

J.Audia, Novatech

### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		9	9	1	1	1	1	1	1
Act Effct Green (s)	23.1	17.1	17.1	23.2	17.2	17.2	32.0	32.0	32.0	13.2	13.2	13.2
Actuated g/C Ratio	0.25	0.18	0.18	0.25	0.19	0.19	0.35	0.35	0.35	0.14	0.14	0.14
v/c Ratio	0.19	0.54	0.79	0.28	0.36	0.42	0.61	0.68	0.02	0.04	0.33	0.15
Control Delay	27.9	40.5	12.3	29.9	37.9	10.2	33.1	31.0	0.1	40.8	41.4	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.9	40.5	12.3	29.9	37.9	10.2	33.1	31.0	0.1	40.8	41.4	1.0
LOS	С	D	В	С	D	В	С	С	А	D	D	А
Approach Delay		22.9			26.7			31.2			32.2	
Approach LOS		С			С			С			С	
Queue Length 50th (m)	6.4	25.1	0.0	7.4	16.2	0.0	47.1	55.5	0.0	1.3	12.1	0.0
Queue Length 95th (m)	20.7	52.3	37.4	23.5	36.0	17.8	96.5	97.7	0.0	6.2	25.1	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	324	875	808	255	875	490	918	1894	912	499	980	530
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.19	0.38	0.72	0.27	0.25	0.33	0.35	0.39	0.02	0.02	0.16	0.09
Intersection Summary												
Area Type: Oth	er											
Cycle Length: 134.7												
Actuated Cycle Length: 92.6												
Natural Cycle: 105												
Control Type: Actuated-Uncoordina	ated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 27.5				In	tersection	LOS: C						
Intersection Capacity Utilization 63	8.2%			IC	U Level of	f Service E	}					
Analysis Period (min) 15												
Onlite and Discourse Or Tauth Line	0.01.1											

	<b>★</b> Ø6	<b>√</b> ø3	3	Ø4
32.3 s	59.3 s	14 s		29.1 s
		ة الأ	,	<b>4</b> Ø8
		14 s		29.1s

### 3: St. Joseph & Eric Czapnik PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>ቀ</b> ቶሴ		W.	
Traffic Volume (vph)	24	295	394	18	10	25
Future Volume (vph)	24	295	394	18	10	25
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.993		0.903	
Flt Protected	0.950				0.986	
Satd. Flow (prot)	1674	3349	4689	0	1569	0
Flt Permitted	0.950				0.986	
Satd. Flow (perm)	1674	3349	4689	0	1569	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	4			4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	1%	3%	1%	1%	1%
Adj. Flow (vph)	27	328	438	20	11	28
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	328	458	0	39	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 25.7% Analysis Period (min) 15

ICU Level of Service A

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>≜1</b> ⊾		×	**		×		1	88	**	1
Traffic Volume (vph)	0	324	3	96	276	0	4	0	82	55	859	136
Future Volume (vph)	0	324	3	96	276	0	4	0	82	55	859	136
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Storage Lanes	0.0		0.0	00.0		0.0	0.0		10.0	2		100.0
Taper Length (m)	10.0		U	35.0		0	10.0		1	60.0		1
	1 00	0.05	0.05	1 00	0.05	1 00	1 00	1 00	1.00	0.07	0.05	1 00
Larie Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	0.00
		0.000					1.00		0.050			0.99
FIL Fit Drotostad		0.999		0.050			0.050		0.000	0.050		0.000
Fil Piolecieu	0	2242	0	0.950	2240	٥	1252	0	1400	0.950	2240	1401
Sato. Flow (prot)	0	3313	U	16/4	3310	0	1353	U	1498	3248	3349	1401
	0	0040	0	0.535	0040	•	0.950	0	4.400	0.950	0040	4000
Satd. Flow (perm)	0	3313	0	943	3316	U	1352	0	1498	3248	3349	1383
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1							237			151
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	1%	2%	1%	1%	2%	1%	25%	1%	1%	1%	1%	8%
Adj. Flow (vph)	0	360	3	107	307	0	4	0	91	61	954	151
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	363	0	107	307	0	4	0	91	61	954	151
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
Median Width(m)		5.0			4.5	Ŭ		7.0			7.0	Ŭ
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		l eft	Thru		l eft		Right	l eft	 Thru	Right
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.0	1.8		6.1		6.1	6.0	1.8	6.0
Detector 1 Type		CI+Ev			CI+Ev		CI+Ev		CI+Ev	CI+Ev	CI+Ev	
Detector 1 Channel												
Detector 1 Extend (c)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (a)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Deley (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (S)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		20.7			20.7						20.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+EX			CI+EX						CI+EX	
Detector 2 Channel												
Detector 2 Extend (s)		0.0		_	0.0		-		_	-	0.0	_
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4			8		5			1	6	
Permitted Phases				8					Free			6
Detector Phase		4		8	8		5			1	6	6
Switch Phase												

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Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL EBT	EBR WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)	26.6	26.6	26.6		16.3			51.3	35.0	35.0
Total Split (%)	34.1%	34.1%	34.1%		20.9%			65.9%	44.9%	44.9%
Maximum Green (s)	20.0	20.0	20.0		10.0			44.3	28.0	28.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag					Lead				Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	None	None	None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	7.0						7.0	7.0
Flash Dont Walk (s)	12.0	12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)	0	0	0						1	1
Act Effct Green (s)	12.9	12.9	12.9		6.1		50.9	23.7	21.9	21.9
Actuated g/C Ratio	0.25	0.25	0.25		0.12		1.00	0.47	0.43	0.43
v/c Ratio	0.43	0.45	0.37		0.03		0.06	0.04	0.66	0.22
Control Delay	19.2	25.6	18.7		27.0		0.1	7.4	15.3	3.8
Queue Delay	0.0	0.0	0.0		0.0		0.0	0.0	0.0	0.0
Total Delay	19.2	25.6	18.7		27.0		0.1	7.4	15.3	3.8
LOS	В	С	В		С		А	А	В	A
Approach Delay	19.2		20.5			1.2			13.4	
Approach LOS	В		С			А			В	
Queue Length 50th (m)	12.0	6.8	10.0		0.3		0.0	1.1	25.7	0.0
Queue Length 95th (m)	30.7	24.4	26.2		2.9		0.0	3.9	75.1	9.6
Internal Link Dist (m)	99.1		222.7			251.2			211.3	
Turn Bay Length (m)		60.0					15.0	110.0		130.0
Base Capacity (vph)	1375	391	1375		280		1498	2843	1945	866
Starvation Cap Reductn	0	0	0		0		0	0	0	0
Spillback Cap Reductn	0	0	0		0		0	0	0	0
Storage Cap Reductn	0	0	0		0		0	0	0	0
Reduced v/c Ratio	0.26	0.27	0.22		0.01		0.06	0.02	0.49	0.17
Intersection Summary										
Area Type: Oth	ier									
Cycle Length: 77.9										
Actuated Cycle Length: 50.9										
Natural Cycle: 70										
Control Type: Actuated-Uncoordin	ated									
Maximum v/c Ratio: 0.66										
Intersection Signal Delay: 15.3			Intersectior	ILOS: B						
Intersection Capacity Utilization 59 Analysis Period (min) 15	9.8%		ICU Level o	of Service B						
Splits and Phases: 4: Old Tenth	Line/OR 174 EB	Ramp & St. Joseph								

Ø1	·	<b>→</b> Ø4	
i1.3 s		26.6 s	
<b>▲</b> Ø5	<b>♦</b> Ø6	₩Ø8	
6.2 -	25 -	26.6 a	

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## **APPENDIX K**

Background Synchro Analysis

### 1: St. Joseph & Vieux-Silo AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	2	232	811	4	16	8
Future Volume (vph)	2	232	811	4	16	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.999		0.955	
Flt Protected	0.950				0.968	
Satd. Flow (prot)	1674	3316	3313	0	1629	0
Flt Permitted	0.950				0.968	
Satd. Flow (perm)	1674	3316	3313	0	1629	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	8			8		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	2	232	811	4	16	8
Shared Lane Traffic (%)	_					-
Lane Group Flow (vph)	2	232	815	0	24	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Riaht	L NA	R NA
Median Width(m)		5.0	3.5	3	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sian Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Othor					
Alea Type.	Uner					
Intersection Conscitution	ion 22 00/			10	ه امیدا ا	Contine A

Intersection Capacity Utilization 33.8% Analysis Period (min) 15 ICU Level of Service A

### 2: Tenth Line & St. Joseph AM Peak Hour

	≯	+	$\mathbf{F}$	4	+	•	•	1	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	**	1	ሻ	<b>*</b> *	1	۲.	<b>≜</b> 12	1	ň	**	1
Traffic Volume (vph)	23	93	133	41	301	55	453	929	23	8	189	60
Future Volume (vph)	23	93	133	41	301	55	453	929	23	8	189	60
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	70.0		0.0	160.0		55.0	105.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	30.0			25.0			25.0			35.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.91	0.91	1.00	1.00	0.95	1.00
Ped Bike Factor	0.99		0.99	1.00		0.97						
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950	0.998		0.950		
Satd, Flow (prot)	1674	3316	1455	1470	3316	1441	1509	3112	1441	1674	3283	1483
Flt Permitted	0.562			0.629			0.950	0.998		0.950		
Satd, Flow (perm)	980	3316	1435	972	3316	1399	1509	3112	1441	1674	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd, Flow (RTOR)			133			126			125			125
Link Speed (k/h)		60			60			60	-		60	-
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	10		1	1	0.0	10		_0.0				
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adi Flow (vph)	23	93	133	41	301	55	453	929	23	8	189	60
Shared Lane Traffic (%)							10%	010	•	•		
Lane Group Flow (vph)	23	93	133	41	301	55	408	974	23	8	189	60
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	I NA	Left	Right	I NA	Left	Right	I NA	Left	R NA	I NA	l eft	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												-
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8	v	8	Ű.	Ŭ	6	-	-	2
Detector Phase	. 7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase				-	-			-		_	_	_

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### 2: Tenth Line & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		7	7	1	1	1	1	1	1
Act Effct Green (s)	18.1	14.3	14.3	19.7	17.1	17.1	40.4	40.4	40.4	14.0	14.0	14.0
Actuated g/C Ratio	0.19	0.15	0.15	0.21	0.18	0.18	0.42	0.42	0.42	0.15	0.15	0.15
v/c Ratio	0.10	0.19	0.41	0.17	0.51	0.16	0.64	0.74	0.03	0.03	0.40	0.19
Control Delay	33.5	42.8	12.1	34.3	42.5	0.9	29.9	29.2	0.1	43.6	43.8	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.5	42.8	12.1	34.3	42.5	0.9	29.9	29.2	0.1	43.6	43.8	1.3
LOS	С	D	В	С	D	А	С	С	А	D	D	A
Approach Delay		25.5			35.9			28.9			33.8	
Approach LOS		С			D			С			С	
Queue Length 50th (m)	3.0	7.8	0.0	5.4	22.5	0.0	57.2	72.5	0.0	1.2	16.0	0.0
Queue Length 95th (m)	10.7	17.9	16.3	16.3	49.7	0.0	129.2	140.6	0.0	5.9	31.5	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	251	859	470	244	862	456	900	1857	910	490	961	522
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.11	0.28	0.17	0.35	0.12	0.45	0.52	0.03	0.02	0.20	0.11
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 95.9												
Natural Cycle: 115												
Control Type: Actuated-Uncoo	rdinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 30.3	}			In	tersection	LOS: C						
Intersection Capacity Utilization	n 71.4%			IC	CU Level o	f Service C	;					
Analysis Period (min) 15												
Colite and Disease Or Tradi		aant										
Spins and Phases: 2: renth	Splits and Phases: 2: Tenth Line & St. Joseph											

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32.3 s	59.3 s	14 s		29.1 s
		≯ø	7	<b>₽</b> Ø8
		14 s		29.1 s

### 3: St. Joseph & Eric Czapnik AM Peak Hour

	٦	-	←	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>ቀ</b> ቶሴ		W.	
Traffic Volume (vph)	21	100	368	17	11	50
Future Volume (vph)	21	100	368	17	11	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.993		0.889	
Flt Protected	0.950				0.991	
Satd, Flow (prot)	1674	3316	4689	0	1553	0
Flt Permitted	0.950			-	0.991	
Satd, Flow (perm)	1674	3316	4689	0	1553	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	3%	1%	1%	1%
Adi, Flow (vph)	21	100	368	17	11	50
Shared Lane Traffic (%)						
Lane Group Flow (vph)	21	100	385	0	61	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	LNA	R NA
Median Width(m)	20.1	5.5	5.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane		0.0	10.0		0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summany						
	Other					
Area Type:	Uther					
Control Type: Unsignalized	i 00 00/					0

Intersection Capacity Utilization 26.0% Analysis Period (min) 15

ICU Level of Service A

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

	≯	<b>→</b>	$\mathbf{F}$	4	+	•	•	Ť	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>≜1</b> 2		<u>م</u>	<b>^</b>		۲. ۲		1	ሻሻ	<b>^</b>	1
Traffic Volume (vph)	0	96	0	34	324	0	0	0	110	53	311	55
Future Volume (vph)	0	96	0	34	324	0	0	0	110	53	311	55
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	60.0		0.0	0.0		15.0	110.0		130.0
Storage Lanes	0		0	1		0	1		1	2		1
Taper Length (m)	10.0			35.0			10.0			60.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	1.00
Ped Bike Factor												0.99
Frt									0.850			0.850
Flt Protected				0.950						0.950		
Satd. Flow (prot)	0	3221	0	1642	3316	0	1762	0	1483	2982	3316	1210
Flt Permitted				0.692						0.950		
Satd, Flow (perm)	0	3221	0	1196	3316	0	1762	0	1483	2982	3316	1195
Right Turn on Red	-		Yes			Yes		-	Yes			Yes
Satd, Flow (RTOR)									868			132
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1				11.0	1	1	10.0				1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	5%	1%	3%	2%	1%	1%	1%	2%	10%	2%	25%
Adi Flow (vph)	0	96	0	34	324	0	0	0	110	53	311	55
Shared Lane Traffic (%)	Ŭ	00	Ū	01	021	Ŭ	Ū	Ŭ	110	00	011	00
Lane Group Flow (vph)	0	96	0	34	324	0	0	0	110	53	311	55
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	I NA	Left	Right	Left	l eft	R NA	Left	l eft	Right
Median Width(m)	Lon	5.0	IXIWX		4.5	rugin	Lon	7.0	11101	Lon	7.0	rtigitt
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane		0.0			10.0			0.0			0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	14	24	1.00	14	24	1.00	14	24	1.00	1.00
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		l eft	Thru		l eft		Right	Left	Thru	Right
Leading Detector (m)		30.5		61	30.5		61		6 1	61	30.5	6 1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.0	1.8		6.1		6.0	6.0	1.8	6.1
Detector 1 Type		CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Fx
Detector 1 Channel		OFER					ONEX			OI' EX		
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7		0.0	28.7		0.0		0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel		OIL									OFEX	
Detector 2 Extend (s)		0.0			0.0						0.0	
		0.0 ΝΔ		Perm	0.0 ΝΔ		Prot		Froo	Prot	0.0 ΝΔ	Parm
Protected Phases		1			۹. و		5		1166	1	8	
Permitted Phases		4		Q	U		0		Free	I	U	6
Detector Phase		Λ		Q Q	Q		5		TIEE	1	6	6
Switch Phase		4		0	U		5			1	U	0

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Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

	≯ →	$\rightarrow$	-	•	1	1	۲	1	ŧ	-
Lane Group	EBL EBT	EBR WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)	31.6	31.6	31.6		16.3			50.3	34.0	34.0
Total Split (%)	38.6%	38.6%	38.6%		19.9%			61.4%	41.5%	41.5%
Maximum Green (s)	25.0	25.0	25.0		10.0			43.3	27.0	27.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
l ead/l ag	0.0	0.0	0.0		l ead			1.0	Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	30	30		3.0			30	3.0	3.0
Recall Mode	None	None	None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	7.0		None			IVIIII	7.0	7.0
Flash Dont Walk (s)	12.0	12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)	12.0	0	0						10.0	10.0
Act Effct Green (s)	11.6	11.6	11.6				42.0	16.5	16.5	16.5
Actuated g/C Ratio	0.28	0.28	0.28				1 00	0.39	0.39	0.39
v/c Ratio	0.20	0.20	0.20				0.07	0.05	0.00	0.00
Control Delay	12.0	12.6	13.6				0.07	8.6	9.24	0.10
Queue Delay	0.0	0.0	0.0				0.1	0.0	0.4	0.4
Total Delay	12.0	12.6	13.6				0.0	8.6	9.0 9.4	0.0
	12.0 R	12.0 R	R				Δ	Δ	Δ	0.4
Approach Delay	12.0	U	13.5			0.1	7	7	8.1	Л
Approach LOS	12.0 R		10.0 R			Δ			Δ	
Oueue Length 50th (m)	22	16	83			~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	0.0	٥Q	62	0.0
Queue Length 95th (m)	6.7	6.5	18.6				0.0	3.7	15.6	0.0
Internal Link Dist (m)	0.7	0.5	222.7			251.2	0.0	5.7	211.3	0.0
Turn Bay Length (m)	55.1	60.0	222.1			201.2	15.0	110.0	211.5	130.0
Base Canacity (vnh)	1058	707	2016				1/83	2850	2177	830
Starvation Can Reduct	1950	0	2010				0	2000	2177	000
Spillback Cap Reducts	0	0	0				0	0	0	0
Storage Cap Reductin	0	0	0				0	0	0	0
Bodupod v/o Potio	0.05	0.05	0 16				0.07	0 02	0 14	0.07
Intersection Summany	0.05	0.05	0.10				0.07	0.02	0.14	0.07
	~									
Area Type: Other	ſ									
Astusted Cuele Length 42										
Actuated Cycle Length. 42										
Natural Cycle: 70	a d									
Control Type: Actuated-Uncoordinat	ea									
Maximum V/c Ratio: 0.35		1.	- <b>t t</b> <sup>2</sup>							
Intersection Signal Delay: 9.6	70/		CLUC	LUS: A						
Analysis Period (min) 15	70	](	U Level o	T SERVICE A						
Splits and Phases: 4: Old Tenth L	ine/OR 174 EB R	amp & St. Joseph								

Ø1		<b>→</b> Ø4					
50.3 s			31.6 s				
<b>▲</b> Ø5	<b>♦</b> Ø6		<b>₩</b> Ø8				
16.3 s	34 s		31.6 s				

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### 1: St. Joseph & Vieux-Silo PM Peak Hour

	≯	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	9	963	612	14	8	5
Future Volume (vph)	9	963	612	14	8	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.997		0.948	
Flt Protected	0.950				0.970	
Satd, Flow (prot)	1674	3349	3338	0	1621	0
Flt Permitted	0.950				0.970	
Satd, Flow (perm)	1674	3349	3338	0	1621	0
Link Speed (k/h)		60	60	Ť	50	- V
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	7	_ 1.0	0.0	7	0.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%
Adi Flow (vph)	9	963	612	14	8	5
Shared Lane Traffic (%)	5	000	012		5	J
Lane Group Flow (vph)	Q	963	626	0	13	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	l eft	Left	Left	Right		RNA
Median Width(m)	Loit	50	3.5	rugiit	3.5	11101
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0 5.0	5.0		5.0	
		5.0	5.0		5.0	
Headway Eactor	1 00	1 09	1 00	1 00	1 00	1 00
Turning Speed (k/h)	24	1.03	1.03	1.03	2/	1.03
Sign Control	24	Froo	Froo	14	Stop	14
		riee	riee		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Interpretion Consoit / Utilized	lion 20 10/			10		Convice A

Intersection Capacity Utilization 38.1% Analysis Period (min) 15

ICU Level of Service A

### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	**	1	7	**	1	ľ	41	1	μ.	**	1
Traffic Volume (vph)	62	335	574	68	230	166	349	741	18	9	218	48
Future Volume (vph)	62	335	574	68	230	166	349	741	18	9	218	48
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	70.0		0.0	160.0		55.0	105.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	30.0			25.0			25.0			35.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.91	0.91	1.00	1.00	0.95	1.00
Ped Bike Factor	0.99		0.99	1.00		0.97			0.98	1.00		
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950	0.998		0.950		
Satd. Flow (prot)	1674	3316	1455	1470	3316	1441	1509	3111	1441	1674	3283	1483
Flt Permitted	0.608			0.470			0.950	0.998		0.950		
Satd. Flow (perm)	1058	3316	1435	727	3316	1399	1509	3111	1417	1672	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			574			166			125			125
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	10		1	1		10			3	3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	62	335	574	68	230	166	349	741	18	9	218	48
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	62	335	574	68	230	166	314	776	18	9	218	48
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		20.7			20.7			20.7			20.7	
Detector 2 Size(III)												
Detector 2 Type		CI+EX			CI+EX			CI+EX			CI+EX	
Detector 2 Channel		0.0			0.0			0.0			0.0	
	nmint	0.0	Dorm	nmint	0.0	Dorm	Calit	0.0	Dorm	Colit	0.0	Dorm
Protected Phases	pili+pi 7	NA A	Pelli	pin+pt 2	N/A Q	Feilli	Split	NA 6	Peilli	Spill	NA 2	Felli
Permitted Phases	1	4	1	J Q	0	Q	U	U	6	2	2	0
Detector Phase	4	1	4	2	Q	۵ ۵	6	6	6	2	2	2
Switch Phase	1	Ŧ	-	5	U	U	U	U	U	2	2	2

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### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		9	9	1	1	1	1	1	1
Act Effct Green (s)	23.5	17.6	17.6	23.6	17.6	17.6	34.1	34.1	34.1	14.3	14.3	14.3
Actuated g/C Ratio	0.24	0.18	0.18	0.25	0.18	0.18	0.35	0.35	0.35	0.15	0.15	0.15
v/c Ratio	0.20	0.55	0.79	0.29	0.38	0.42	0.59	0.70	0.03	0.04	0.45	0.15
Control Delay	30.0	42.7	12.5	32.0	40.0	10.5	32.5	32.0	0.1	42.0	43.9	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	30.0	42.7	12.5	32.0	40.0	10.5	32.5	32.0	0.1	42.0	43.9	1.0
LOS	С	D	В	С	D	В	С	С	А	D	D	А
Approach Delay		24.0			28.2			31.6			36.4	
Approach LOS		С			С			С			D	
Queue Length 50th (m)	7.2	27.2	0.0	8.0	18.1	0.0	48.2	62.9	0.0	1.4	18.5	0.0
Queue Length 95th (m)	21.8	54.3	37.1	23.8	38.1	17.8	93.8	104.6	0.0	6.4	35.3	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	315	846	793	245	846	480	887	1829	884	482	946	516
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.40	0.72	0.28	0.27	0.35	0.35	0.42	0.02	0.02	0.23	0.09
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 96.3												
Natural Cycle: 105												
Control Type: Actuated-Uncoc	ordinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 28.9	)			In	tersection	LOS: C						
Intersection Capacity Utilization	on 66.1%			IC	CU Level o	f Service C	;					
Analysis Period (min) 15												
Splits and Phases: 2: Tenth	Line & St. Jo	oseph										
		8						Q	- 2	Å.		

₽ø2	<b>★</b> <i>ø</i> 6	<b>1</b> 03	<b>4</b> 04	
32.3 s	59.3 s	14 s	29.1 s	
		♪ <sub>07</sub>	₹ø8	
		14 s	29.1 s	

### 3: St. Joseph & Eric Czapnik PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>ቀ</b> ቶሴ		W.	
Traffic Volume (vph)	28	332	442	28	12	31
Future Volume (vph)	28	332	442	28	12	31
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor		0.00	0.01	0.01		
Frt			0.991		0.903	
Flt Protected	0.950		0.001		0.986	
Satd Flow (prot)	1674	3349	4681	0	1569	0
Flt Permitted	0.950	0010	1001	0	0.986	0
Satd Flow (perm)	1674	3349	4681	0	1569	0
Link Speed (k/h)	10/1	60	60	0	50	0
Link Distance (m)		144 1	123.1		184.3	
Travel Time (s)		86	7.4		13.3	
Confl Peds (#/hr)	5	0.0	7.1	5	.0.0	
Peak Hour Factor	1 00	1 00	1 00	1 00	1 00	1 00
Heavy Vehicles (%)	1%	1%	3%	1%	1%	1%
Adi Flow (vnh)	28	332	442	28	12	31
Shared Lane Traffic (%)	20	002	772	20	12	01
Lane Group Flow (vph)	28	332	470	0	43	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment		l off	l eft	Right		RNA
Median Width(m)	Leit	5.5	50	right	2.5	111/1
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
		5.0	10.0		5.0	
Headway Eactor	1.00	1 00	1.00	1 00	1 00	1 00
Turning Speed (k/b)	1.09	1.09	1.09	1.09	1.09	1.09
Sign Control	24	Eroo	Eroo	14	24 Stop	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
				10		· · · ·

Intersection Capacity Utilization 26.9%

ICU Level of Service A

Analysis Period (min) 15

### 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>≜1</b> 6		1	**		1		1	ሻሻ	<b>*</b> *	1
Traffic Volume (vph)	0	365	3	105	324	0	4	0	89	60	936	148
Future Volume (vph)	0	365	3	105	324	0	4	0	89	60	936	148
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	60.0		0.0	0.0		15.0	110.0		130.0
Storage Lanes	0		0	1		0	1		1	2		1
Taper Length (m)	10.0			35.0			10.0			60.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	1.00
Ped Bike Factor							1.00					0.99
Frt		0.999							0.850			0.850
Flt Protected				0.950			0.950			0.950		
Satd, Flow (prot)	0	3313	0	1674	3316	0	1353	0	1498	3248	3349	1401
Flt Permitted				0.533			0.950			0.950		
Satd. Flow (perm)	0	3313	0	939	3316	0	1352	0	1498	3248	3349	1383
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1							237			148
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1					1	1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	1%	1%	2%	1%	25%	1%	1%	1%	1%	8%
Adj. Flow (vph)	0	365	3	105	324	0	4	0	89	60	936	148
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	368	0	105	324	0	4	0	89	60	936	148
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
Median Width(m)		5.0			4.5	Ŭ		7.0			7.0	Ŭ
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		Left	Thru		Left		Right	Left	Thru	Right
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		CI+Ex		Cl+Ex	CI+Ex		CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7						28.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4			8		5			1	6	
Permitted Phases				8					Free			6
Detector Phase		4		8	8		5			1	6	6
Switch Phase												

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Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL I	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)		10.0		10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)		25.6		25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)		26.6		26.6	26.6		16.3			51.3	35.0	35.0
Total Split (%)	34	.1%		34.1%	34.1%		20.9%			65.9%	44.9%	44.9%
Maximum Green (s)		20.0		20.0	20.0		10.0			44.3	28.0	28.0
Yellow Time (s)		3.7		3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)		2.9		2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)		0.0		0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)		6.6		6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag							Lead				Lag	Lag
Lead-Lag Optimize?							Yes				Yes	Yes
Vehicle Extension (s)		3.0		3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	N	lone		None	None		None			Min	Min	Min
Walk Time (s)		7.0		7.0	7.0						7.0	7.0
Flash Dont Walk (s)		12.0		12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)		1		0	0						1	1
Act Effct Green (s)		13.0		13.0	13.0		6.1		50.9	23.5	21.7	21.7
Actuated g/C Ratio	(	0.26		0.26	0.26		0.12		1.00	0.46	0.43	0.43
v/c Ratio	(	0.43		0.44	0.38		0.02		0.06	0.04	0.66	0.22
Control Delay		19.1		25.1	18.6		27.2		0.1	7.5	15.3	3.8
Queue Delay		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Total Delay		19.1		25.1	18.6		27.2		0.1	7.5	15.3	3.8
LOS		В		С	В		С		А	А	В	А
Approach Delay		19.1			20.2			1.2			13.4	
Approach LOS		В			С			А			В	
Queue Length 50th (m)		12.0		6.6	10.4		0.3		0.0	1.1	24.8	0.0
Queue Length 95th (m)		31.1		24.0	27.6		2.9		0.0	3.8	73.3	9.4
Internal Link Dist (m)	(	99.1			222.7			251.2			211.3	
Turn Bay Length (m)				60.0					15.0	110.0		130.0
Base Capacity (vph)	1	377		390	1378		281		1498	2843	1948	866
Starvation Cap Reductn		0		0	0		0		0	0	0	0
Spillback Cap Reductn		0		0	0		0		0	0	0	0
Storage Cap Reductn		0		0	0		0		0	0	0	0
Reduced v/c Ratio	(	0.27		0.27	0.24		0.01		0.06	0.02	0.48	0.17
Intersection Summary												
Area Type: Othe	ər											
Cycle Length: 77.9												
Actuated Cycle Length: 50.9												
Natural Cycle: 70												
Control Type: Actuated-Uncoordina	ited											
Maximum v/c Ratio: 0.66												
Intersection Signal Delay: 15.3				In	tersection	LOS: B						
Intersection Capacity Utilization 63.2%			IC	CU Level of	Service B							
Analysis Period (min) 15												

Splits and Phases: 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph

Ø1		→04	
51.3 s		26.6 s	
ØS	Ø6	<b>√</b> Ø8	
16.3 s	35.s	26.6 s	
#### 1: St. Joseph & Vieux-Silo AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	2	263	922	4	16	8
Future Volume (vph)	2	263	922	4	16	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor		0.00	0.00	0.00		
Frt			0.999		0.955	
Flt Protected	0.950		0.000		0.968	
Satd, Flow (prot)	1674	3316	3313	0	1629	0
Flt Permitted	0.950	0010	0010		0.968	J
Satd Flow (perm)	1674	3316	3313	0	1629	0
Link Speed (k/h)	1011	60	60		50	J
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	8	21.5	0.0	8	0.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adi, Flow (vph)	2	263	922	4	16	8
Shared Lane Traffic (%)	2	200	022			J
Lane Group Flow (vph)	2	263	926	0	24	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	l eft	Right	I NA	RNA
Median Width(m)	2011	5.0	3.5	, agint	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sign Control	27	Free	Free	17	Stop	17
		1166	1166		Otop	
Intersection Summary	0.11					
Area Type:	Other					
Control Type: Unsignalized						
Interception Canacity Litilizat	ion 37 0%				'III ovol of	Sonvice A

Intersection Capacity Utilization 37.0% Analysis Period (min) 15 ICU Level of Service A

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#### 2: Tenth Line & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	**	1	7	<b>*</b> *	1	7	<b>₹</b> ∱	1	2	<b>*</b> *	1
Traffic Volume (vph)	26	104	151	47	342	61	516	1106	26	9	253	68
Future Volume (vph)	26	104	151	47	342	61	516	1106	26	9	253	68
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	70.0		0.0	160.0		55.0	105.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	30.0			25.0			25.0			35.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.91	0.91	1.00	1.00	0.95	1.00
Ped Bike Factor	0.99		0.99	1.00		0.97						
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950	0.998		0.950		
Satd. Flow (prot)	1674	3316	1455	1470	3316	1441	1509	3111	1441	1674	3283	1483
Flt Permitted	0.468			0.621			0.950	0.998		0.950		
Satd. Flow (perm)	817	3316	1435	960	3316	1399	1509	3111	1441	1674	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			151			126			125			125
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	10		1	1		10						
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	26	104	151	47	342	61	516	1106	26	9	253	68
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	26	104	151	47	342	61	464	1158	26	9	253	68
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	Cl+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0	-		0.0	-	• "	0.0	5	<b>A</b>	0.0	-
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	1	4		3	8	^	6	6	•	2	2	^
Permitted Phases	4	4	4	8		8	^	^	6			2
Detector Phase	1	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

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#### 2: Tenth Line & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		7	7	1	1	1	1	1	1
Act Effct Green (s)	21.2	15.7	15.7	23.1	18.8	18.8	48.9	48.9	48.9	15.4	15.4	15.4
Actuated g/C Ratio	0.19	0.14	0.14	0.21	0.17	0.17	0.45	0.45	0.45	0.14	0.14	0.14
v/c Ratio	0.12	0.22	0.45	0.20	0.60	0.18	0.69	0.84	0.04	0.04	0.55	0.22
Control Delay	35.5	46.3	12.0	36.7	49.7	1.1	33.6	35.3	0.1	45.3	50.9	1.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.5	46.3	12.0	36.7	49.7	1.1	33.6	35.3	0.1	45.3	50.9	1.6
LOS	D	D	В	D	D	А	С	D	А	D	D	A
Approach Delay		26.9			41.8			34.3			40.6	
Approach LOS		С			D			С			D	
Queue Length 50th (m)	4.1	10.3	0.0	7.6	36.2	0.0	81.6	112.4	0.0	1.7	26.8	0.0
Queue Length 95th (m)	11.5	19.6	17.3	18.1	56.1	0.0	153.2	#192.7	0.0	6.5	41.0	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	227	724	431	240	724	403	759	1565	787	413	810	460
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.14	0.35	0.20	0.47	0.15	0.61	0.74	0.03	0.02	0.31	0.15
Intersection Summary												
Area Type: Cycle Length: 134 7	Other											
Actuated Cycle Length: 109.8												
Natural Cycle: 125												
Control Type: Actuated I Incoo	rdinated											
Maximum v/c Patio: 0.84	iuinaleu											
Intersection Signal Delay: 35 5	;			In	tersection							
Intersection Capacity Utilizatio	, n 77 9%					f Service D						
Analysis Period (min) 15	11 7 7.0 70											
# 95th percentile volume exc	reeds canaci	tv queue r	nav he lon	ner								
Queue shown is maximum	after two cyc	les.		gor.								
Splits and Phases: 2: Tenth	Line & St. Jo	oseph										
1 10 000 L 10000		06						603	4	<b>1</b> 04		
7 102	1	20						+ <u>w</u> s		- 12 T		_

	<b>√</b> ø <sub>6</sub>	<b></b>	03	- Ø4	
32.3 s	59.3 s	14 s		29.1 s	
		∕∘	07	<b>₽</b> Ø8	
		14 s		29.1 s	

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#### 3: St. Joseph & Eric Czapnik AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>**1</b>		W.	
Traffic Volume (vph)	21	113	418	17	11	50
Future Volume (vph)	21	113	418	17	11	50
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.994		0.889	
Flt Protected	0.950				0.991	
Satd. Flow (prot)	1674	3316	4693	0	1553	0
Flt Permitted	0.950				0.991	
Satd. Flow (perm)	1674	3316	4693	0	1553	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	3%	1%	1%	1%
Adj. Flow (vph)	21	113	418	17	11	50
Shared Lane Traffic (%)		-				
Lane Group Flow (vph)	21	113	435	0	61	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0	3	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane		0.0			0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summarv						
Area Type:	Other					
Control Type: Unsignalized	00101					
Control Type. Onoignalized	. 00.00/			10		· · · ·

Intersection Capacity Utilization 26.9% Analysis Period (min) 15

ICU Level of Service A

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>≜1</b> 6		5	**		5		1	ሻሻ	**	1
Traffic Volume (vph)	0	107	0	38	367	0	0	0	125	61	353	62
Future Volume (vph)	0	107	0	38	367	0	0	0	125	61	353	62
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	60.0		0.0	0.0		15.0	110.0		130.0
Storage Lanes	0		0	1		0	1		1	2		1
Taper Length (m)	10.0			35.0			10.0			60.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	1.00
Ped Bike Factor												0.99
Frt									0.850			0.850
Flt Protected				0.950						0.950		
Satd, Flow (prot)	0	3221	0	1642	3316	0	1762	0	1483	2982	3316	1210
Flt Permitted				0.684						0.950		
Satd, Flow (perm)	0	3221	0	1182	3316	0	1762	0	1483	2982	3316	1195
Right Turn on Red	-		Yes			Yes		-	Yes			Yes
Satd, Flow (RTOR)									844			132
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1				11.0	1	1	10.0				1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	5%	1%	3%	2%	1%	1%	1%	2%	10%	2%	25%
Adi Flow (vph)	0	107	0	38	367	0	0	0	125	61	353	62
Shared Lane Traffic (%)	Ŭ	101	Ū	00	001	Ŭ	Ŭ	Ŭ	120	01	000	02
Lane Group Flow (yph)	0	107	0	38	367	0	0	0	125	61	353	62
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	I NA	Left	Right	Left	Left	R NA	Left	l eft	Right
Median Width(m)	Lon	5.0	11101	2101	4.5	rugitu	Lon	7.0	11101	Lon	7.0	rugni
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane		0.0			10.0			0.0			0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		l eft	Thru		l eft		Riaht	l eft	Thru	Riaht
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		Cl+Fx		CI+Ex	CI+Ex		CI+Fx		CI+Ex	CI+Fx	CI+Ex	CI+Fx
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7		0.0	28.7				0.0	0.0	28.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		Cl+Fx			CI+Ex						CI+Ex	
Detector 2 Channel		OI LA			OI LA						OT EX	
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4		. •	8		5		1,00	1	6	, 0111
Permitted Phases				8	<u> </u>		Ŭ		Free		<b>v</b>	6
Detector Phase		4		8	8		5		1100	1	6	6
Switch Phase				Ŭ	Ŭ		Ű				Ŭ	0

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Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

	≯ →	$\rightarrow$	-	•	1	1	۲	1	ŧ	-
Lane Group	EBL EBT	EBR WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)	31.6	31.6	31.6		16.3			50.3	34.0	34.0
Total Split (%)	38.6%	38.6%	38.6%		19.9%			61.4%	41.5%	41.5%
Maximum Green (s)	25.0	25.0	25.0		10.0			43.3	27.0	27.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag					Lead				Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	None	None	None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	7.0						7.0	7.0
Flash Dont Walk (s)	12.0	12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)	1	0	0						1	1
Act Effct Green (s)	11.6	11.6	11.6				42.0	16.5	16.5	16.5
Actuated g/C Ratio	0.28	0.28	0.28				1.00	0.39	0.39	0.39
v/c Ratio	0.12	0.12	0.40				0.08	0.05	0.27	0.11
Control Delay	12.0	12.8	14.0				0.1	8.6	9.6	0.7
Queue Delay	0.0	0.0	0.0				0.0	0.0	0.0	0.0
Total Delay	12.0	12.8	14.0				0.1	8.6	9.6	0.7
LOS	В	В	В				А	А	А	А
Approach Delay	12.0		13.9			0.1			8.4	
Approach LOS	В		В			А			А	
Queue Length 50th (m)	2.5	1.7	9.5				0.0	1.0	7.1	0.0
Queue Length 95th (m)	7.4	7.1	21.1				0.0	4.1	17.6	0.9
Internal Link Dist (m)	99.1		222.7			251.2			211.3	
Turn Bay Length (m)		60.0					15.0	110.0		130.0
Base Capacity (vph)	1957	718	2015				1483	2850	2176	829
Starvation Cap Reductn	0	0	0				0	0	0	0
Spillback Cap Reductn	0	0	0				0	0	0	0
Storage Cap Reductn	0	0	0				0	0	0	0
Reduced v/c Ratio	0.05	0.05	0.18				0.08	0.02	0.16	0.07
Intersection Summary										
Area Type: Othe	er									
Cycle Length: 81.9										
Actuated Cycle Length: 42										
Natural Cycle: 70										
Control Type: Actuated-Uncoordina	ated									
Maximum v/c Ratio: 0.40										
Intersection Signal Delay: 9.8		li	ntersection	LOS: A						
Intersection Capacity Utilization 34	.9%		CU Level o	f Service A						
Analysis Fendu (11111) 13										
Splits and Phases: 4: Old Tenth	Line/OR 174 EB R	amp & St. Joseph								

Ø1		<b>→</b> Ø4	
50.3 s		31.6 s	
▲ Ø5	<b>♦</b> Ø6	<b>↓</b> Ø8	
16.3 s	34 s	31.6 s	

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#### 1: St. Joseph & Vieux-Silo PM Peak Hour

	٦	-	-	•	1	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>A</b> 1.		W.	
Traffic Volume (vph)	9	1094	695	14	8	5
Future Volume (vph)	9	1094	695	14	8	5
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.997		0.948	
Flt Protected	0.950				0.970	
Satd. Flow (prot)	1674	3349	3338	0	1621	0
Flt Permitted	0.950				0.970	
Satd. Flow (perm)	1674	3349	3338	0	1621	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	7			7		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%
Adj. Flow (vph)	9	1094	695	14	8	5
Shared Lane Traffic (%)						
Lane Group Flow (vph)	9	1094	709	0	13	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.0	3.5	Ŭ	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 41.9%			IC	U Level of	Service A

Analysis Period (min) 15

#### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<b>N</b>	**	1	5	**	1	5	<b>≜</b> 12	1	<b>N</b>	**	1
Traffic Volume (vph)	70	380	653	77	260	189	397	884	20	10	303	54
Future Volume (vph)	70	380	653	77	260	189	397	884	20	10	303	54
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	70.0		0.0	160.0		55.0	105.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	30.0			25.0			25.0			35.0		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.91	0.91	1.00	1.00	0.95	1.00
Ped Bike Factor	0.99		0.99	1.00		0.97			0.98	1.00		
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950	0.998		0.950		
Satd. Flow (prot)	1674	3316	1455	1470	3316	1441	1509	3111	1441	1674	3283	1483
Flt Permitted	0.560			0.395			0.950	0.998		0.950		
Satd. Flow (perm)	976	3316	1435	611	3316	1399	1509	3111	1417	1672	3283	1483
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			653			189			125			125
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		82.1			144.1			338.4			235.5	
Travel Time (s)		4.9			8.6			20.3			14.1	
Confl. Peds. (#/hr)	10		1	1		10			3	3		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	70	380	653	77	260	189	397	884	20	10	303	54
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	70	380	653	77	260	189	357	924	20	10	303	54
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		20.7			20.7			20.7			20.7	
Detector 2 Size(III)												
Detector 2 Type		CI+EX			CI+EX			CI+EX			CI+EX	
Detector 2 Channel		0.0			0.0			0.0			0.0	
	nmint	0.0	Dorm	nmint	0.0	Dorm	Calit	0.0	Dorm	Colit	0.0	Dorm
Protected Phases	pin+pi 7	NA A	Feilli	pin+pt 2	N/A Q	Feilli	Split	NA 6	Peilli	Spill	NA 2	Fein
Permitted Diases	1	4	1	J Q	0	Q	U	U	6	2	2	0
Notactor Phase	4	Λ	4	2	Q	۵ ۵	6	6	6	2	2	2
Switch Phase	1	т Т	т Т	0	0	0	U	U	U	2	2	2

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#### 2: Tenth Line & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		9	9	1	1	1	1	1	1
Act Effct Green (s)	26.1	20.1	20.1	26.2	20.2	20.2	41.0	41.0	41.0	16.7	16.7	16.7
Actuated g/C Ratio	0.24	0.19	0.19	0.24	0.19	0.19	0.38	0.38	0.38	0.15	0.15	0.15
v/c Ratio	0.24	0.62	0.82	0.37	0.42	0.46	0.62	0.78	0.03	0.04	0.60	0.16
Control Delay	34.7	48.5	13.2	38.5	44.8	10.5	34.5	36.1	0.1	44.8	50.7	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.7	48.5	13.2	38.5	44.8	10.5	34.5	36.1	0.1	44.8	50.7	1.0
LOS	С	D	В	D	D	В	С	D	А	D	D	A
Approach Delay		26.7			31.5			35.1			43.2	
Approach LOS		С			С			D			D	
Queue Length 50th (m)	10.2	37.6	0.0	11.4	24.8	0.0	65.0	91.4	0.0	1.8	31.4	0.0
Queue Length 95th (m)	24.5	62.3	42.2	26.9	43.2	19.3	109.1	131.0	0.0	6.7	48.7	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	291	752	830	215	752	463	789	1627	800	429	842	473
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.51	0.79	0.36	0.35	0.41	0.45	0.57	0.03	0.02	0.36	0.11
Intersection Summary												
Area Type: 0	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 108.1												
Natural Cycle: 105												
Control Type: Actuated-Uncoor	dinated											
Maximum v/c Ratio: 0.82												
Intersection Signal Delay: 32.6				In	tersection	LOS: C						
Intersection Capacity Utilization	71.5%			IC	CU Level o	f Service C	;					
Analysis Period (min) 15												
Splits and Dhasses D. Tanth I	inc & Ct L	acanh										
	Splits and Phases: 2: Tenth Line & St. Joseph											

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		♪ <sub>Ø7</sub>	₹ø8
		14 s	29.1 s

#### 3: St. Joseph & Eric Czapnik PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>ቀ</b> ቶሴ		W.	
Traffic Volume (vph)	28	376	502	28	12	31
Future Volume (vph)	28	376	502	28	12	31
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor		0.00	0.01	0.01		
Frt			0.992		0.903	
Flt Protected	0.950		0.002		0.986	
Satd Flow (prot)	1674	3349	4685	0	1569	0
Flt Permitted	0.950	0010	1000	0	0.986	0
Satd Flow (nerm)	1674	3349	4685	0	1569	0
Link Speed (k/h)	10/1	60	60	0	50	0
Link Distance (m)		144 1	123.1		184.3	
Travel Time (s)		86	7.4		13.3	
Confl Peds (#/hr)	5	0.0	7.1	5	.0.0	
Peak Hour Factor	1 00	1 00	1 00	1 00	1 00	1 00
Heavy Vehicles (%)	1%	1%	3%	1%	1%	1%
Adi Flow (vnh)	28	376	502	28	12	31
Shared Lane Traffic (%)	20	010	002	20	12	01
Lane Group Flow (vph)	28	376	530	0	13	0
Enter Blocked Intersection	20 No	No	No	No	40 No	No
Lane Alianment				Right		R NA
Median Width(m)	Leit	5.5	5.0	right	2.5	17.1974
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
		5.0	10.0		5.0	
Headway Eactor	1.00	1 00	1.00	1 00	1 00	1 00
Turning Speed (k/h)	1.09	1.09	1.09	1.09	1.09	1.09
Sign Control	24	Eroo	Eroo	14	Ctop	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
				10		· · · ·

Intersection Capacity Utilization 28.0%

ICU Level of Service A

Analysis Period (min) 15

#### 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>≜1</b> 6		1	**		1		1	ሻሻ	<b>*</b> *	1
Traffic Volume (vph)	0	414	4	119	365	0	5	0	102	68	1065	169
Future Volume (vph)	0	414	4	119	365	0	5	0	102	68	1065	169
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	60.0		0.0	0.0		15.0	110.0		130.0
Storage Lanes	0		0	1		0	1		1	2		1
Taper Length (m)	10.0			35.0			10.0			60.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	1.00
Ped Bike Factor							1.00					0.99
Frt		0.999							0.850			0.850
Flt Protected				0.950			0.950			0.950		
Satd, Flow (prot)	0	3313	0	1674	3316	0	1353	0	1498	3248	3349	1401
Flt Permitted				0.507			0.950			0.950		
Satd, Flow (perm)	0	3313	0	894	3316	0	1352	0	1498	3248	3349	1383
Right Turn on Red			Yes			Yes			Yes			Yes
Satd, Flow (RTOR)		1							237			169
Link Speed (k/h)		60			60			60	-		60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1					1	1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	1%	1%	2%	1%	25%	1%	1%	1%	1%	8%
Adi, Flow (vph)	0	414	4	119	365	0	5	0	102	68	1065	169
Shared Lane Traffic (%)	Ť					Ŭ	Ŭ	Ť				
Lane Group Flow (vph)	0	418	0	119	365	0	5	0	102	68	1065	169
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	I NA	Left	Right	Left	l eft	R NA	Left	l eft	Right
Median Width(m)		5.0			4.5	g.i.t	2011	7.0			7.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane		0.0						0.0			0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		Left	Thru		Left		Right	Left	Thru	Riaht
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7						28.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4			8		5			1	6	
Permitted Phases				8					Free			6
Detector Phase		4		8	8		5			1	6	6
Switch Phase												

J.Audia, Novatech

Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph PM Peak Hour

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Lane Group	EBL EBT	EBR WBL	. WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	) 10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	5 25.6		11.3			12.0	30.0	30.0
Total Split (s)	26.6	26.6	6 26.6		16.3			51.3	35.0	35.0
Total Split (%)	34.1%	34.1%	34.1%		20.9%			65.9%	44.9%	44.9%
Maximum Green (s)	20.0	20.0	) 20.0		10.0			44.3	28.0	28.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	) 2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag					Lead				Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	3.0	) 3.0		3.0			3.0	3.0	3.0
Recall Mode	None	None	e None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	) 7.0						7.0	7.0
Flash Dont Walk (s)	12.0	12.0	) 12.0						16.0	16.0
Pedestrian Calls (#/hr)	1	C	) 0						1	1
Act Effct Green (s)	13.9	13.9	) 13.9		6.1		54.8	26.6	24.7	24.7
Actuated g/C Ratio	0.25	0.25	5 0.25		0.11		1.00	0.49	0.45	0.45
v/c Ratio	0.50	0.52	2 0.43		0.03		0.07	0.04	0.71	0.24
Control Delay	20.8	29.2	2 20.1		28.4		0.1	7.6	17.0	3.7
Queue Delay	0.0	0.0	) 0.0		0.0		0.0	0.0	0.0	0.0
Total Delay	20.8	29.2	2 20.1		28.4		0.1	7.6	17.0	3.7
LOS	C	C	C C		С		A	A	В	A
Approach Delay	20.8		22.4			1.4			14.8	
Approach LOS	C	0.4	C		0.4	A	0.0		B	0.0
Queue Length 50th (m)	17.1	9.4	i 14.7		0.4		0.0	1.4	32.7	0.0
Queue Length 95th (m)	35.4	27.0	000 7		3.5	054.0	0.0	4.Z	#96.2	10.1
Internal Link Dist (m)	99.1	<u> </u>	222.1			251.2	15.0	110.0	211.3	120.0
Furn Bay Length (III)	1064	00.0	1065		250		1409	2690	1700	130.0
Storyotion Con Bodystn	1204	34 I			200		1490	2000	1/00	017
Stal Valion Cap Reductin	0				0		0	0	0	0
Storage Can Reductin	0	(			0		0	0	0	0
Reduced v/c Ratio	0 33	0.34	5 0.29		0.02		0.07	0 03	0 60	0 21
	0.00	0.00	0.25		0.02		0.07	0.00	0.00	0.21
Intersection Summary										
Area Type: Oth	er									
Cycle Length. 77.9										
Natural Cycle: 70										
Control Type: Actuated Uncoordin	atad									
Maximum v/c Patio: 0.71	aleu									
Intersection Signal Delay: 16.8			Intersection							
Intersection Capacity Utilization 68	5%			of Service C	<u>.</u>					
Analysis Period (min) 15			100 Leven		,					
# 95th percentile volume exceed	s capacity queue	may be longer								
Queue shown is maximum afte	r two cycles.	anay be longer.								
	1: /OD /7/ 55									
plits and Phases: 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph										



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## **APPENDIX L**

Transportation Demand Management Checklists

## TRANSPORTATION DEMAND MANAGEMENT

TDM-Supportive Development Design and Infrastructure Checklist

## **TDM-Supportive Development Design and Infrastructure Checklist:**

Residential Developments (multi-family or condominium)

Legend					
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed				
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users				
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance				

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	□ - N/A
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible <i>(see Official</i> <i>Plan policy 4.3.12)</i>	

	TDM-s	upportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well- used areas ( <i>see Zoning By-law Section 111</i> )	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored <i>(see Zoning By-law Section 111)</i>	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multi-family residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses <i>(see Zoning By-law Section 94)</i>	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly <i>(see Zoning By-law</i> <i>Section 104)</i>	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking <i>(see Zoning By-law Section 111)</i>	
	6.2	Separate long-term & short-term parking areas	:
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

## TRANSPORTATION DEMAND MANAGEMENT

**TDM Measures Checklist** 

### **TDM Measures Checklist:**

Residential Developments (multi-family, condominium or subdivision)

	Legend
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance
*	The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	* 1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & dest	tinations
			linations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	
BASIC	2.1.1 <b>2.2</b>	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium) Bicycle skills training	

		TDM	measures: Residential developments	Check if proposed & add descriptions
		3.	TRANSIT	
		3.1	Transit information	
BASIC		3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	
BETTER		3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
		3.2	Transit fare incentives	
BASIC	*	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER		3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
		3.3	Enhanced public transit service	
BETTER	*	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels <i>(subdivision)</i>	
		3.4	Private transit service	
BETTER		3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
		4.	CARSHARING & BIKESHARING	
		4.1	Bikeshare stations & memberships	
BETTER		4.1.1	Contract with provider to install on-site bikeshare station ( <i>multi-family</i> )	
BETTER		4.1.2	Provide residents with bikeshare memberships, either free or subsidized <i>(multi-family)</i>	
		4.2	Carshare vehicles & memberships	
BETTER		4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER		4.2.2	Provide residents with carshare memberships, either free or subsidized	
		5.	PARKING	
		5.1	Priced parking	
BASIC	*	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC	*	5.1.2	Unbundle parking cost from monthly rent (multi-family)	$\checkmark$

	TDM	measures: Residential developments	Check if proposed & add descriptions
	6.	TDM MARKETING & COMMUNICATION	IS
	6.1	Multimodal travel information	
BASIC	★ 6.1.1	Provide a multimodal travel option information package to new residents	
	6.2	Personalized trip planning	
BETTER	★ 6.2.1	Offer personalized trip planning to new residents	

## **APPENDIX M**

MMLOS Analysis

#### Segment MMLOS Analysis

This section provides a review of the boundary streets St. Joseph Boulevard and Tenth Line Road, using complete streets principles. The *Multi-Modal Level of Service (MMLOS) Guidelines*, produced by IBI Group in October 2015, were used to evaluate the levels of service for each alternative mode of transportation on the boundary streets, based on the targets for the 'Mixed Use Centre' designation.

Exhibit 4 of the MMLOS Guidelines has been used to evaluate the segment pedestrian level of service (PLOS) of the boundary streets. Exhibit 22 of the MMLOS Guidelines suggest a target PLOS C for all roadways within Mixed Use Centres. The results of the segment PLOS analysis are summarized in **Table 1**.

Exhibit 11 of the MMLOS Guidelines has been used to evaluate the segment bicycle level of service (BLOS) of the boundary streets. Within Mixed Use Centres, Exhibit 22 of the MMLOS Guidelines suggest a target BLOS A for Crosstown Bikeways (St. Joseph Boulevard west of Tenth Line Road), a target BLOS C for arterial Spine Routes (Tenth Line Road north of St. Joseph Boulevard). The results of the segment BLOS analysis are summarized in **Table 2**.

Exhibit 15 of the MMLOS Guidelines has been used to evaluate the segment transit level of service (TLOS) of the boundary streets. While only Tenth Line Road has a target TLOS D (since it is identified as a Transit Priority Corridor with Isolated Measures), St. Joseph Boulevard has also been evaluated for TLOS, as there is currently transit service on both roadways. The results of the segment TLOS analysis are summarized in **Table 3**.

Exhibit 20 of the MMLOS Guidelines has been used to evaluate the segment truck level of service (TkLOS) of the boundary streets. Within Mixed Use Centres, Exhibit 22 of the MMLOS Guidelines suggest a target TkLOS D for arterial roadways with a truck route designation (St. Joseph Boulevard, Tenth Line Road). The results of the segment TkLOS analysis are summarized in **Table 4**.

#### Table 1: PLOS Segment Analysis

Sidewalk Width	Boulevard Width	Avg. Daily Curb Lane Traffic Volume	Presence of On- Street Parking	Operating Speed <sup>(1)</sup>	PLOS						
St. Joseph Boulevard (Vieux-Silo Street to Tenth Line Road, north side)											
1.5m	0m	> 3,000 vpd	No	70 km/h	F						
St. Joseph Boulevard (Vieux-Silo Street to Tenth Line Road, south side)											
1.5m	0m	> 3,000 vpd	No	70 km/h	F						
<b>Tenth Line R</b>	oad (Ottawa I	Road 174 to St. Joseph	Boulevard, east si	de)							
<u>&gt;</u> 2.0m	0m	> 3,000 vpd	No	70 km/h	F						
<b>Tenth Line R</b>	Tenth Line Road (Ottawa Road 174 to St. Joseph Boulevard, west side)										
<u>&gt;</u> 2.0m	0m	<u>&lt;</u> 3,000 vpd	No	70 km/h	D						

1. Operating speed taken as the speed limit plus 10 km/h for St. Joseph Boulevard and Tenth Line Road

#### Table 2: BLOS Segment Analysis

<b>Road Class</b>	Type of Route	Type of Bikeway	<b>Travel Lanes</b>	<b>Operating Speed</b>	BLOS					
St. Joseph	Boulevard (Vieux-Sil	o Street to Tenth	Line Road)							
Arterial	Crosstown Bikeway	Mixed Traffic	4	70 km/h	F					
Tenth Line Road (Ottawa Road 174 to St. Joseph Boulevard)										
Arterial	Spine Route	Mixed Traffic	4	70 km/h	F					

#### **Table 3: TLOS Segment Analysis**

Eccility Type	Exposure to Cong										
Гасшиу гуре	Congestion	Friction	Incident Potential								
St. Joseph Boulevard (Vieux-Silo Street to Tenth Line Road)											
Mixed Traffic – Limited Parking/Driveway Friction	Yes	Low	Medium	D							
Tenth Line Road (Ottawa F	Tenth Line Road (Ottawa Road 174 to St. Joseph Boulevard)										
Mixed Traffic – Limited Parking/Driveway Friction	Yes	Low	Medium	D							

#### **Table 4: TkLOS Segment Analysis**

Curb Lane Width	Number of Travel Lanes Per Direction	TkLOS								
St. Joseph Boulevard (Vieux-	St. Joseph Boulevard (Vieux-Silo Street to Tenth Line Road)									
> 3.7m	> 3.7m 2									
Tenth Line Road (Ottawa Road 174 to St. Joseph Boulevard)										
> 3.7m	2	А								

#### Intersection MMLOS Analysis

The following is a review of the MMLOS of the signalized intersections within the study area, using complete streets principles. St. Joseph Boulevard/Tenth Line Road has been evaluated using the MMLOS targets for intersections within a Mixed Use Centre, while St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp has been evaluated using the MMLOS targets for intersections within the General Urban Area, and are based on existing conditions.

Exhibit 5 of the Addendum to the MMLOS Guidelines has been used to evaluate the existing PLOS at the intersections listed above. Exhibit 22 of the MMLOS Guidelines identifies a target PLOS C for all roadways within the Mixed Use Centre and General Urban Area designations. The results of the intersection PLOS analysis are summarized in **Table 5** through **Table 6**.

Exhibit 12 of the MMLOS Guidelines has been used to evaluate the existing BLOS at the intersections listed above. Within the Mixed Use Centre, Exhibit 22 of the MMLOS Guidelines identifies a target BLOS A for Crosstown Bikeways (St. Joseph Boulevard west of Tenth Line Road, Tenth Line Road south of St. Joseph Boulevard) and a target BLOS C for arterial Spine Routes (St. Joseph Boulevard east of Tenth Line Road, Tenth Line Road north of St. Joseph Boulevard). Within the General Urban Area, Exhibit 22 suggests a target BLOS C for Spine Routes (St. Joseph Boulevard), and a target BLOS D for roadways with no cycling route designation (Old Tenth Line Road). The Ottawa Road 174 EB Off-Ramp has not been evaluated for BLOS, as it is a freeway exit. The results of the intersection BLOS analysis are summarized in **Table 7**.

Exhibit 16 of the MMLOS Guidelines has been used to evaluate the existing TLOS at the intersections listed above. Exhibit 22 of the MMLOS Guidelines identifies a target TLOS D for Transit Priority Corridors with Isolated Measures (Tenth Line Road), and does not identify a target TLOS for roadways without a Rapid Transit or Transit Priority designation (St. Joseph Boulevard, Old Tenth Line Road, Ottawa Road 174 EB Off-Ramp). The TLOS has been evaluated for every approach that is currently used by transit. The results of the intersection TLOS analysis are summarized in **Table 8**.

Exhibit 21 of the MMLOS Guidelines has been used to evaluate the existing TkLOS at the intersections listed above. Exhibit 22 of the MMLOS Guidelines identifies a target TkLOS D for arterial truck routes (St. Joseph Boulevard, Tenth Line Road, Ottawa Road 174 EB Off-Ramp), and a target TkLOS E for arterial roadways with no truck route designation (Old Tenth Line Road). The results of the intersection TkLOS analysis are summarized in **Table 9**.

CRITERIA	North Approach	South Approach		East Approach		West Approach		
			PETSI SCORE					
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	10	No	10	No	10	No	10
Lanes Crossed (3.5m Lane Width)	10 +	-10	10 +	-10	10 +	-10	10 +	-10
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Perm + Prot	-8	Perm + Prot	-8	Protected	0	Protected	0
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5	Permissive or Yield	-5
Right Turn on Red	N/A	0	N/A	0	N/A	0	N/A	0
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS	•							
Parallel Radius	> 15m to 25m	-8	> 25m	-9	> 15m to 25m	-8	> 15m to 25m	-8
Parallel Right Turn Channel	Conventional with Receiving	-3	Conventional with Receiving	-3	Conventional without Receiving	0	Conventional without Receiving	0
Perpendicular Radius	> 15m to 25m	-8	> 15m to 25m	-8	> 15m to 25m	-8	> 25m	-9
Perpendicular Right Turn Channel	Conventional without Receiving	0	Conventional without Receiving	0	Conventional with Receiving	-3	Conventional with Receiving	-3
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	Standard	-7	Standard	-7
	PETSI SCORE	-51		-52		-43		-44
	LOS	F		F		F		F
			DELAY SCORE					
Cycle Length		129.7		129.7		129.7		129.7
Pedestrian Walk Time		6.0		6.0		34.0		3.0
	DELAY SCORE	59.0		59.0		35.3		61.9
	LOS	E		E		D		F
	OVERALL	F		F		F		F

#### Table 5: PLOS Intersection Analysis – St. Joseph Boulevard/Tenth Line Road

#### Table 6: PLOS Intersection Analysis – St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp

CRITERIA	North Approach		South Approach		East Approach		West Approach	
			PETSI SCORE					
CROSSING DISTANCE CONDITIONS								
Median > 2.4m in Width	No	6	No	22	N/A	0	No	10
Lanes Crossed (3.5m Lane Width)	9	0	8	23	N/A	0	10 +	-10
SIGNAL PHASING AND TIMING								
Left Turn Conflict	No Left Turn/Prohibited	0	Permissive	-8	N/A	0	Protected	0
Right Turn Conflict	Permissive or Yield	-5	Permissive or Yield	-5	N/A	0	Permissive or Yield	-5
Right Turn on Red	N/A	0	N/A	0	N/A	0	RTOR Allowed	-3
Leading Pedestrian Interval	No	-2	No	-2	N/A	0	No	-2
CORNER RADIUS								
Parallel Radius	No Right Turn	0	> 3m to 5m	-4	N/A	0	> 25m	-9
Parallel Right Turn Channel	No Right Turn	0	No Right Turn Channel	-4	N/A	0	Conventional with Receiving	-3
Perpendicular Radius	> 25m	-9	> 15m to 25m	-8	N/A	0	N/A	0
Perpendicular Right Turn Channel	Conventional with Receiving	-3	Smart Channel	2	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Standard	-7	Standard	-7	N/A	0	Standard	-7
	PETSI SCORE	-20		-13		-		-39
	LOS	F		F		-		F
			DELAY SCORE					
Cycle Length		77.9		77.9		0		81.9
Pedestrian Walk Time		8.0		8.0		0.0		11.0
	DELAY SCORE	31.4		31.4		-		30.7
	LOS	D		D		-		D
	OVERALL	F		F		-		F

Approach	Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
St. Joseph Boul	evard/Tenth Li	ne Road		
North Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane < 50m; turning speed <u>&lt;</u> 25 km/h	D
	Wixed Hame	Left Turn Accommodation	Dual left turn lanes	F
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane < 50m; turning speed <u>&lt;</u> 25 km/h	D
		Left Turn Accommodation	Dual left turn lanes	F
East Approach	Pocket	Right Turn Lane Characteristics	Right turn lane > 50m and introduced to the right; turning speed < 30 km/h	D
	Bike Lane	Left Turn Accommodation	2 lanes crossed, $\geq$ 50 km/h	F
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane > 50m	F
	Mixed Hame	Left Turn Accommodation	2 lanes crossed, $\geq$ 50 km/h	F
St. Joseph Boul	evard/Old Tent	h Line Road/Ot	tawa Road 174 EB Off-Ramp	
North Approach	Mixed Traffic	Right Turn Lane Characteristics Left Turn Accommodation	Approach is a freeway exit; cyclists prohibited	N/A
South Approach	Mixed Traffic	Right Turn Lane Characteristics	Right turn lane < 50m; turning speed <u>&lt;</u> 25 km/h	D
		Left Turn Accommodation	0 lanes crossed, $\geq$ 60 km/h	D
East Approach	Mixed Traffic	Right Turn Lane Characteristics	No right turns	-
	Mixed Hame	Left Turn Accommodation	2 lanes crossed, $\geq$ 60 km/h	F
West Approach	Mixed Traffic	Right Turn Lane Characteristics	Shared through/right turn lane	А
WEST Approach		Left Turn Accommodation	No left turns	-

### Table 7: BLOS Intersection Analysis

### **Table 8: TLOS Intersection Analysis**

Approach	Dela								
Арргоасн	AM Peak	PM Peak	ILU3						
St. Joseph Boulevard/Tent	h Line Road								
South Approach	29 sec	31 sec	E						
East Approach	35 sec	27 sec	E						
St. Joseph Boulevard/Old Tenth Line Road/Ottawa Road 174 EB Off-Ramp									
North Approach	7 sec	13 sec	Ċ						

1. Delay based on outputs from Synchro analysis of existing conditions

Approach	Effective Corner Radius	Number of Receiving Lanes Departing Intersection	TkLOS		
St. Joseph Boulevard	I/Tenth Line Road				
North Approach	> 15m	2	А		
South Approach	> 15m	2	А		
East Approach	> 15m	3	А		
West Approach	> 15m	3	А		
St. Joseph Boulevard	I/Old Tenth Line Road/Otta	awa Road 174 EB Off-Ramp			
North Approach	> 15m	3	А		
South Approach	> 15m	2	А		
East Approach	No right turns	-	N/A		
West Approach	< 10m	2	D		

### Table 9: TkLOS Intersection Analysis

## **APPENDIX N**

Total Synchro Analysis

#### 1: St. Joseph & Vieux-Silo AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	5	232	823	4	26	11
Future Volume (vph)	5	232	823	4	26	11
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.999		0.960	
Flt Protected	0.950				0.966	
Satd. Flow (prot)	1674	3316	3313	0	1634	0
Flt Permitted	0.950				0.966	
Satd. Flow (perm)	1674	3316	3313	0	1634	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	8			8		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adj. Flow (vph)	5	232	823	4	26	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	5	232	827	0	37	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.0	3.5	U ·	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
				10		

Intersection Capacity Utilization 34.2% Analysis Period (min) 15 ICU Level of Service A

#### 2: Tenth Line & St. Joseph AM Peak Hour

Lane Conjuguitors         EB         EBT         EBR         WBL         WBR         NBL         NBT         NBF         SBL         SBT         SBR           Lane Conjuguitors         N         4         A         7         N         4         7         7         4         7         7         4         A         7         7         4         A         7         7         4         A         7         7         4         A         7         7         4         A         8         183         61           Full         0.0         10		≯	-	$\mathbf{r}$	4	+	*	1	Ť	1	1	ŧ	~
Lane Configurations         Y         AA         F         Y         AA         F         Y         AA         F           Traffer Volume (ph)         23         93         143         41         303         66         458         929         23         8         199         61           Future Volume (ph)         23         93         143         41         303         66         458         929         23         8         199         61           Storage Larget (n)         0.0         0.0         70.0         0.0         160.0         150.0         10.0         95         1.00         0.95         1.00         0.95         1.00         0.95         1.00         0.95         0.050         0.85	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (oph)         23         93         143         41         303         66         458         929         23         8         189         61           isiaal Flow (vphp)         1800	Lane Configurations	5	**	1	5	**	1	<b>X</b>	<b>≜1</b> ⊾	1	5	**	1
Fulue Volume (oph)         23         33         143         41         303         66         458         929         23         8         199         61           isteal Flow (vphp)         1800	Traffic Volume (vph)	23	93	143	41	303	66	458	929	23	8	189	61
issue I Eveny heigh         1800<	Future Volume (vph)	23	93	143	41	303	66	458	929	23	8	189	61
Storage Length (m)         100         100         700         000         1000         1000         1000         6600           Storage Length (m)         30         0         700         0.00         100         0.95         100         6600           Storage Length (m)         30.0         95         1.00         0.95         1.00         0.91         0.91         0.01         1.00 <td< td=""><td>Ideal Flow (vphpl)</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td><td>1800</td></td<>	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Lange (m)         Cite         Cite <thcite< th=""> <thcite< th="">         Cite</thcite<></thcite<>	Storage Length (m)	0.0	1000	0.0	70.0	1000	0.0	160.0	1000	55.0	105.0	1000	60.0
Trave Length (m)         30.0         25.0         25.0         35.0         30.0           Lane Uli Factor         1.00         0.95         1.00         0.91         0.91         0.01         0.00         0.97           Fri         0.850         0.950         0.950         0.860         0.850         0.850           Sald, Flow (port)         1674         3316         1455         1470         3316         1451         1470         3316         1441         1509         0.950         0.9	Storage Lanes	1		1	1 1		1	100.0		1	1		1
Lane Util, Factor         100         0.95         1.00         0.95         1.00         0.97         0.91         1.00         0.95         1.00           Ped Bke Factor         0.99         0.99         1.00         0.97         0.950         0.850         <	Taper Length (m)	30.0		•	25.0			25.0		•	35.0		•
Land Sum 1 data         Los         Los <thlos< th=""></thlos<>	Lane Litil Factor	1.00	0 95	1 00	1 00	0 95	1 00	0.01	0.91	1 00	1 00	0.95	1 00
Part No.         0.350         0.360         0.950         1125         1125 <t< td=""><td>Ped Bike Factor</td><td>0.00</td><td>0.55</td><td>0.00</td><td>1.00</td><td>0.55</td><td>0.07</td><td>0.51</td><td>0.51</td><td>1.00</td><td>1.00</td><td>0.55</td><td>1.00</td></t<>	Ped Bike Factor	0.00	0.55	0.00	1.00	0.55	0.07	0.51	0.51	1.00	1.00	0.55	1.00
Intervented         0.950	Frt	0.55		0.55	1.00		0.57			0.850			0.850
Introduction         0.333         0.331         0.453         0.336         0.441         0.539         0.3112         1441         1674         3316         1451         1474         1339         1039         1039         3112         1441         1674         3316         1451         1432         124         1441         1674         3316         1451         1339         1039         112         1441         1674         3316         1435         1435         1435         126         125         2336         1438         1433         1433         116         11674         3384         235.5         125           Link Distance (m)         82.1         1441.1         336.4         20.3         14.1         235.5         125         125         125         145         144.1         366         20.3         14.1         100         100         1.00         1	Fit Protected	0.050		0.000	0.050		0.000	0.050	0 008	0.000	0.950		0.000
Satur         Oscol         1470         Oscol         0.990	Sate Flow (prot)	1674	2216	1/55	1/70	2216	1//1	1500	2112	1//1	167/	2002	1/93
In Partitude       0.333       0.323       0.330	Salu. Flow (plot)	0.550	3310	1455	0.620	3310	1441	0.050	0.000	1441	0.050	5205	1405
Satul Providenting         973         S310         H33         S72         S310         H33         S72         S310         H33	Fit Fermilled	0.559	2216	1/25	0.029	2216	1200	1500	0.990	1111	1674	2002	1/02
Nglin fund in eu         res	Bight Turp on Rod	975	3310	1455 Voo	912	3310	1399 Voo	1509	3112	144 I Voo	1074	3203	1403 Voo
Sadu. Prov (K1 OK)         143         129         129         129         129         129           Link Speed (kh)         60         60         60         60         60         141         158         203         141         1         144         1         144         1         143         141         1<	Cotd. Flow (PTOP)			142			106			105			105
Link Speed (M)         OU	Salu. Flow (RTOR)		00	145		00	120		<u> </u>	120		00	120
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $			60			60			00			60	
Iravel Inne (s)       4.9       8.6       20.3       14.1         Confl. Peds, (#hr)       10       1       1       100         Peak Hour Factor       1.00	Link Distance (m)		82.1			144.1			338.4			235.5	
Cont. Preds. (kmr)         10         1         1         10           Peak Hour Factor         1.00 <td>I ravel I ime (s)</td> <td>10</td> <td>4.9</td> <td>4</td> <td></td> <td>8.6</td> <td>10</td> <td></td> <td>20.3</td> <td></td> <td></td> <td>14.1</td> <td></td>	I ravel I ime (s)	10	4.9	4		8.6	10		20.3			14.1	
Peak Hour Factor       1.00       1.0	Confl. Peds. (#/hr)	10		1	1		10	4.00					
Heavy Vehicles (%)       1%       2%       4%       15%       2%       5%       2%       4%       5%       1%       3%       2%         Adj. Flow (vph)       23       93       143       41       303       66       458       929       23       8       189       61         Lane Group Flow (vph)       23       93       143       41       303       66       458       929       23       8       189       61         Lane Group Flow (vph)       23       93       143       41       303       66       442       975       23       8       189       61         Lane Alignment       L NA       Left       Right       L NA       Left       Right       L NA       Left       R NA       L NA       Left       N No       <	Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)       23       93       143       41       303       66       458       929       23       8       189       61         Shared Lane Traffic (%)       10%       No       No <td>Heavy Vehicles (%)</td> <td>1%</td> <td>2%</td> <td>4%</td> <td>15%</td> <td>2%</td> <td>5%</td> <td>2%</td> <td>4%</td> <td>5%</td> <td>1%</td> <td>3%</td> <td>2%</td>	Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Shared Lane Traffic (%)       23       93       143       41       303       66       412       975       23       8       189       61         Enter Blocked Intersection       No       No<	Adj. Flow (vph)	23	93	143	41	303	66	458	929	23	8	189	61
Lane Group Flow (vph)         23         93         143         41         303         66         412         975         23         8         189         61           Enter Blocked Intersection         No         <	Shared Lane Traffic (%)							10%					
Enter Blocked Intersection         No         Left         Right	Lane Group Flow (vph)	23	93	143	41	303	66	412	975	23	8	189	61
Lane Alignment         LNA         Left         Right         LNA         Left         Right         LNA         Left         R NA         LAN         Left         R NA           Median Width(m)         7.0         7.0         5.0         <	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Median Width(m)         7.0         7.0         5.0         5.0           Link Offset(m)         0.0         0.0         0.0         0.0         0.0           Crosswalk Width(m)         5.0         5.0         5.0         5.0           Two way Left Turn Lane	Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Link Offset(m)         0.0         0.0         0.0         0.0           Crosswalk Width(m)         5.0         5.0         5.0         5.0           Two way Left Turn Lane         Headway Factor         1.09<	Median Width(m)		7.0			7.0			5.0			5.0	
Crosswalk Width(m)         5.0         5.0         5.0         5.0           Two way Left Turn Lane         1.09	Link Offset(m)		0.0			0.0			0.0			0.0	
Two way Left Turn Lane         Headway Factor       1.09 <td>Crosswalk Width(m)</td> <td></td> <td>5.0</td> <td></td> <td></td> <td>5.0</td> <td></td> <td></td> <td>5.0</td> <td></td> <td></td> <td>5.0</td> <td></td>	Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Headway Factor       1.09<	Two way Left Turn Lane												
Turning Speed (k/h)       24       14	Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Number of Detectors         1         2         1	Turning Speed (k/h)	24		14	24		14	24		14	24		14
Detector Template         Left         Thru         Right         Left         Glassistion         Right         Left         Thru         Right         Left         Thru         Right         Left         Classistion         Glassistion         Glassistion         Glassistion         Glassistion         Glassistion	Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Leading Detector (m)         6.1         30.5         6.1         6.1         30.5         6.1         6.1         30.5         6.1         6.1         30.5         6.1         6.1         30.5         6.1         6.1         30.5         6.1         6.1         30.5         6.1         1.1         30.5         6.1         1.1         30.5         6.1         6.1         8.5         6.1         6.1         8.5         7.5         7.5	Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Trailing Detector (m)         0.0	Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Detector 1 Position(m)         0.0	Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         6.1         1.8         6.1         Cl+Ex         Cl-Ex         Cl-Ex         Detector         Dot         0.0	Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Type         CI+Ex         CI         CI+Ex	Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Channel           Detector 1 Extend (s)         0.0         <	Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Extend (s)         0.0	Detector 1 Channel												
Detector 1 Queue (s)         0.0	Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)         0.0	Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)         28.7         28.7         28.7         28.7           Detector 2 Size(m)         1.8         1.8         1.8         1.8         1.8           Detector 2 Size(m)         1.8         1.8         1.8         1.8         1.8           Detector 2 Type         Cl+Ex         Cl+Ex         Cl+Ex         Cl+Ex         Cl+Ex           Detector 2 Channel         0.0         0.0         0.0         0.0         0.0           Turn Type         pm+pt         NA         Perm         pm+pt         NA         Perm         Split         NA         Perm           Protected Phases         7         4         3         8         6         6         2         2           Permitted Phases         4         4         3         8         6         6         2         2           Detector Phase         7         4         3         8         6         6         2         2         2	Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Size(m)         1.8         1.8         1.8         1.8           Detector 2 Type         CI+Ex         CI+Ex         CI+Ex         CI+Ex           Detector 2 Channel         0.0         0.0         0.0         0.0           Detector 2 Extend (s)         0.0         0.0         0.0         0.0           Turn Type         pm+pt         NA         Perm         Split         NA         Perm           Protected Phases         7         4         3         8         6         6         2         2           Permitted Phases         4         4         3         8         6         6         2         2           Detector Phase         7         4         3         8         6         6         2         2         2	Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Type         CI+Ex         CI+Ex         CI+Ex         CI+Ex           Detector 2 Channel         0.0         0.0         0.0         0.0           Detector 2 Extend (s)         0.0         0.0         0.0         0.0           Turn Type         pm+pt         NA         Perm         Split         NA         Perm           Protected Phases         7         4         3         8         6         6         2         2           Permitted Phases         4         4         3         8         6         6         2         2           Detector Phase         7         4         3         8         6         6         2         2	Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Channel         0.0         0.0         0.0         0.0           Detector 2 Extend (s)         0.0         0.0         0.0         0.0           Turn Type         pm+pt         NA         Perm         Split         NA         Perm         Split         NA         Perm           Protected Phases         7         4         3         8         6         6         2         2           Permitted Phases         4         4         8         8         6         6         2         2           Detector Phase         7         4         3         8         8         6         6         2         2	Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Extend (s)0.00.00.00.0Turn Typepm+ptNAPermSplitNAPermProtected Phases74386622Permitted Phases44886622Detector Phase7443886622Switch Phase7443886622	Detector 2 Channel												
Turn Typepm+ptNAPermpm+ptNAPermSplitNAPermSplitNAPermProtected Phases74386622Permitted Phases4488622Detector Phase7443886622Switch Phase	Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Protected Phases         7         4         3         8         6         6         2         2           Permitted Phases         4         4         8         8         6         2         2           Detector Phase         7         4         4         3         8         6         6         2         2           Switch Phase         7         4         4         3         8         6         6         2         2         2	Turn Type	pm+pt	NA	Perm	pm+nt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Permitted Phases         4         4         8         6         2         2         2           Detector Phase         7         4         3         8         6         6         2         2         2           Switch Phase         7         4         4         3         8         6         6         2         2         2	Protected Phases	7	4	1 0111	3	8	1 3111	6	6	. 0111	2	2	
Detector Phase         7         4         3         8         6         6         2         2         2           Switch Phase         7         4         3         8         6         6         2         2         2	Permitted Phases	4	т	4	8	U	8	V	U	6	2	2	2
Switch Phase	Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
	Switch Phase	1	т	T	0	U	U	U	0	J	2	2	2

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Synchro 10 Report

#### 2: Tenth Line & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		7	7	1	1	1	1	1	1
Act Effct Green (s)	18.2	14.3	14.3	19.7	17.1	17.1	40.4	40.4	40.4	14.0	14.0	14.0
Actuated g/C Ratio	0.19	0.15	0.15	0.21	0.18	0.18	0.42	0.42	0.42	0.15	0.15	0.15
v/c Ratio	0.10	0.19	0.43	0.17	0.51	0.19	0.65	0.74	0.03	0.03	0.40	0.19
Control Delay	33.5	42.8	12.0	34.3	42.6	1.2	30.1	29.2	0.1	43.6	43.8	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.5	42.8	12.0	34.3	42.6	1.2	30.1	29.2	0.1	43.6	43.8	1.3
LOS	С	D	В	С	D	А	С	С	А	D	D	А
Approach Delay		24.9			35.1			29.0			33.8	
Approach LOS		С			D			С			С	
Queue Length 50th (m)	3.0	7.8	0.0	5.4	22.6	0.0	58.0	72.6	0.0	1.2	16.0	0.0
Queue Length 95th (m)	10.7	17.9	17.0	16.3	50.0	0.0	130.9	140.9	0.0	5.9	31.5	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	251	857	477	244	861	456	899	1854	909	489	959	522
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.11	0.30	0.17	0.35	0.14	0.46	0.53	0.03	0.02	0.20	0.12
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 96												
Natural Cycle: 115												
Control Type: Actuated-Uncoo	rdinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 30.1				In	tersection	LOS: C						
Intersection Capacity Utilizatio	n 71.5%			IC	CU Level o	f Service C	)					
Analysis Period (min) 15												
Splits and Phases: 2: Tenth	Line & St. Jo	oseph										
	1									1		

	<b>★</b> ø6	<b>√</b> ø∶	3	<b>₩</b> Ø4
32.3 s	59.3 s	14 s		29.1 s
		> ø	7	<b>4</b> <b>Ø</b> 8
		14 s		29.1 s

#### 3: St. Joseph & Eric Czapnik AM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>**1</b>		M	
Traffic Volume (vph)	21	100	370	24	15	61
Future Volume (vph)	21	100	370	24	15	61
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util, Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor			0.0.			
Frt			0.991		0.892	
Flt Protected	0.950				0.990	
Satd, Flow (prot)	1674	3316	4681	0	1556	0
Flt Permitted	0.950				0.990	-
Satd. Flow (perm)	1674	3316	4681	0	1556	0
Link Speed (k/h)		60	60		50	·
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	3%	1%	1%	1%
Adj. Flow (vph)	21	100	370	24	15	61
Shared Lane Traffic (%)						•
Lane Group Flow (vph)	21	100	394	0	76	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	Outor					
	ion 07 10/					Convior A

Intersection Capacity Utilization 27.1% Analysis Period (min) 15 ICU Level of Service A

#### 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>A1</b>		8	**		×		1	88	**	1
Traffic Volume (vph)	0	100	0	34	325	0	0	0	110	53	311	63
Future Volume (vph)	0	100	0	34	325	0	0	0	110	53	311	63
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Storage Lanes	0.0		0.0	1		0.0	1		10.0	2		100.0
Taper Length (m)	10.0		Ū	35.0		v	10.0			60.0		
Lane Litil Factor	1 00	0 95	0 95	1 00	0.95	1 00	1 00	1 00	1 00	00.0	0.95	1 00
Ped Bike Factor	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.01	0.00	0.99
Frt									0.850			0.55
Fit Protected				0.950					0.000	0.950		0.000
Satd Flow (prot)	٥	3221	٥	16/12	3316	٥	1762	٥	1/83	2082	3316	1210
Elt Permitted	U	JZZ I	U	0.680	5510	U	1702	U	1405	0.050	5510	1210
Sata Elow (porm)	٥	2001	٥	1101	3316	٥	1760	٥	1/92	0.900	2216	1105
Bight Turn on Pod	0	JZZ I	Voc	1191	3310	Voc	1702	0	1400 Voc	2902	5510	Voc
Sate Flow (PTOP)			165			165			863			120
Link Speed (k/b)		60			60			60	003		60	132
Link Opeed (k/n)		102.1			00			00			00	
		123.1			240.7			210.Z			200.0	
Confl Dada (#/hr)	1	7.4			14.0	1	1	10.D			14.1	1
Comil. Peas. (#/m)	1 00	1 00	1 00	1.00	1 00	1 00	1 00	1 00	1 00	1 00	1.00	1 00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy vehicles (%)	1%	5%	1%	3%	2%	1%	1%	1%	2%	10%	2%	25%
Adj. Flow (vpn)	0	100	0	34	325	0	0	0	110	53	311	63
Shared Lane Traffic (%)	0	400	•	24	205	0	0	•	440	50	044	60
Lane Group Flow (Vpn)	U	100	U	34	325	U	U	U	110	53	311	63 No
Enter Blocked Intersection	INO	INO	INO D NIA	INO	INO	INO	INO	INO	INO D NIA	INO	INO	INO
	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
		5.0			4.5			7.0			7.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
		5.0			10.0			5.0			5.0	
I wo way Left Turn Lane	4.00	4.00	4.00	1.00	4.00	4.00	4.00	4.00	4.00	4 00	4.00	4.00
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24	•	14	24	•	14	24		14	24	•	14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Inru		Left	I nru		Left		Right	Left	Inru	Right
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		CI+EX		CI+EX	CI+EX		CI+EX		CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Channel		0.0			• •		0.0		0.0			0.0
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7						28.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			CI+Ex						CI+Ex	_
Detector 2 Channel												
Detector 2 Extend (s)		0.0		_	0.0		_		_	-	0.0	_
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4		_	8		5			1	6	
Permitted Phases				8					Free			6
Detector Phase		4		8	8		5			1	6	6
Switch Phase												

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Synchro 10 Report

# 4: Old Tenth Line/OR 174 EB Ramp & St. Joseph AM Peak Hour

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)		10.0		10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)		25.6		25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)		31.6		31.6	31.6		16.3			50.3	34.0	34.0
Total Split (%)		38.6%		38.6%	38.6%		19.9%			61.4%	41.5%	41.5%
Maximum Green (s)		25.0		25.0	25.0		10.0			43.3	27.0	27.0
Yellow Time (s)		3.7		3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)		2.9		2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)		0.0		0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)		6.6		6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag							Lead				Lag	Lag
Lead-Lag Optimize?							Yes				Yes	Yes
Vehicle Extension (s)		3.0		3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode		None		None	None		None			Min	Min	Min
Walk Time (s)		7.0		7.0	7.0						7.0	7.0
Flash Dont Walk (s)		12.0		12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)		1		0	0						1	1
Act Effct Green (s)		11.6		11.6	11.6				42.0	16.5	16.5	16.5
Actuated g/C Ratio		0.28		0.28	0.28				1.00	0.39	0.39	0.39
v/c Ratio		0.11		0.10	0.36				0.07	0.05	0.24	0.11
Control Delay		12.0		12.6	13.6				0.1	8.6	9.4	0.8
Queue Delay		0.0		0.0	0.0				0.0	0.0	0.0	0.0
Total Delay		12.0		12.6	13.6				0.1	8.6	9.4	0.8
LOS		В		В	В				А	А	А	A
Approach Delay		12.0			13.5			0.1			8.1	
Approach LOS		В			В			А			А	
Queue Length 50th (m)		2.4		1.6	8.3				0.0	0.9	6.2	0.0
Queue Length 95th (m)		7.0		6.5	18.7				0.0	3.7	15.6	1.0
Internal Link Dist (m)		99.1			222.7			251.2			211.3	
Turn Bay Length (m)				60.0					15.0	110.0		130.0
Base Capacity (vph)		1958		724	2016				1483	2850	2177	830
Starvation Cap Reductn		0		0	0				0	0	0	0
Spillback Cap Reductn		0		0	0				0	0	0	0
Storage Cap Reductn		0		0	0				0	0	0	0
Reduced v/c Ratio		0.05		0.05	0.16				0.07	0.02	0.14	0.08
Intersection Summary												
Area Type: O	ther											
Cycle Length: 81.9												
Actuated Cycle Length: 42												
Natural Cycle: 70												
Control Type: Actuated-Uncoord	inated											
Maximum v/c Ratio: 0.36												
Intersection Signal Delay: 9.5	on Signal Delay: 9.5 Intersection LOS: A											
Intersection Capacity Utilization 3	33.7%			IC	CU Level of	Service A						
Analysis Period (min) 15												
Splits and Phases: 4: Old Tent	th Line/OR	174 FR R	amp & St	Joseph								

Ø1		<b>—●</b> Ø4					
50.3 s			31.6 s				
<b>▲</b> Ø5			<b>↓</b> Ø8				
16.3 s	34 s		31.6 s				

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	≯	-	-	•	1	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		1111	<b>4</b> 1a			1
Traffic Volume (vph)	0	259	814	8	0	12
Future Volume (vph)	0	259	814	8	0	12
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	25.0			0.0	0.0	0.0
Storage Lanes	1			0	0	1
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.86	0.95	0.95	1.00	1.00
Frt			0.999			0.865
Flt Protected						
Satd. Flow (prot)	0	6003	3312	0	0	1510
Flt Permitted						
Satd. Flow (perm)	0	6003	3312	0	0	1510
Link Speed (k/h)		60	60		50	
Link Distance (m)		55.5	82.1		81.0	
Travel Time (s)		3.3	4.9		5.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	0	259	814	8	0	12
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	259	822	0	0	12
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.5	3.5		0.0	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summarv						
Area Type:	Other					
Control Type: Unsignalized	0 0101					
Intersection Capacity Utilizati	ion 34.0%			ICI	J Level of	Service A
Analysis Period (min) 15				.0.		20.10071

## 1: St. Joseph & Vieux-Silo PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> 5		W.	
Traffic Volume (vph)	14	963	620	14	14	6
Future Volume (vph)	14	963	620	14	14	6
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.997		0.959	
Flt Protected	0.950				0.966	
Satd. Flow (prot)	1674	3349	3338	0	1633	0
Flt Permitted	0.950				0.966	
Satd. Flow (perm)	1674	3349	3338	0	1633	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	7			7		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%
Adj. Flow (vph)	14	963	620	14	14	6
Shared Lane Traffic (%)						
Lane Group Flow (vph)	14	963	634	0	20	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.0	3.5	Ŭ	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
	Cara 00 40/			10	111	

Intersection Capacity Utilization 38.1% Analysis Period (min) 15

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	×.	**	1	5	**	1	×.	_ <b>∆†</b> ♠	1	*	**	1
Traffic Volume (vph)	62	335	580	68	235	173	358	741	18	9	218	49
Future Volume (vph)	62	335	580	68	235	173	358	741	18	9	218	49
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	70.0	1000	0.0	160.0	1000	55.0	105.0	1000	60.0
Storage Lanes	1		1	1 1		1	100.0		1	100.0		1
Taper Length (m)	30.0			25.0			25.0		I.	35.0		
Lane Litil Factor	1 00	0 95	1 00	1 00	0.95	1 00	0.01	0.91	1 00	1 00	0.95	1 00
Ped Bike Factor	0.00	0.55	0.00	1.00	0.55	0.07	0.51	0.51	0.08	1.00	0.55	1.00
Frt	0.55		0.55	1.00		0.57			0.50	1.00		0 850
Elt Protected	0.950		0.000	0.950		0.000	0.950	0 008	0.000	0.950		0.000
Satd Flow (prot)	167/	3316	1/55	1/70	3316	1//1	1500	3112	1//1	1674	3083	1/83
Elt Dormittod	0.605	5510	1400	0.460	5510	1441	0.050	0.008	1441	0.050	5205	1405
Sold Flow (porm)	1052	2216	1/25	0.405	2216	1200	1500	2112	1/17	1670	2202	1/02
Dight Turn on Pod	1055	3310	1400 Voc	125	3310	1099 Voc	1509	3112	1417 Voc	1072	5205	140J
Sata Elow (PTOP)			F00			105			105			105
Link Speed (k/b)		60	560		60	175		60	120		60	120
Link Speed (k/n)		00			144 1			220 4			00 025 5	
		02.1			144.1			338.4			235.5	
Confl Dada (#/br)	10	4.9	1	1	0.0	10		20.3	2	C	14.1	
Confi. Peas. (#/nr)	10	4.00	1 00	1 00	4.00	10	4.00	4 00	3	3	4.00	4 00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vpn)	62	335	580	68	235	173	358	741	18	9	218	49
Shared Lane Traffic (%)	00	005	-00	00	005	470	10%		10	•	040	10
Lane Group Flow (vph)	62	335	580	68	235	1/3	322	///	18	9	218	49
Enter Blocked Intersection	No	No	No	No	NO	No	No	NO	No	No	NO	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			Cl+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8		8			6			2
Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		9	9	1	1	1	1	1	1
Act Effct Green (s)	23.5	17.6	17.6	23.6	17.6	17.6	34.4	34.4	34.4	14.3	14.3	14.3
Actuated g/C Ratio	0.24	0.18	0.18	0.24	0.18	0.18	0.36	0.36	0.36	0.15	0.15	0.15
v/c Ratio	0.20	0.56	0.79	0.29	0.39	0.44	0.60	0.70	0.03	0.04	0.45	0.15
Control Delay	30.1	42.8	12.5	32.1	40.2	10.5	32.8	31.9	0.1	42.0	44.1	1.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	30.1	42.8	12.5	32.1	40.2	10.5	32.8	31.9	0.1	42.0	44.1	1.0
LOS	С	D	В	С	D	В	С	С	А	D	D	Α
Approach Delay		24.0			28.2			31.7			36.3	
Approach LOS		С			С			С			D	
Queue Length 50th (m)	7.3	27.6	0.0	8.1	18.7	0.0	50.0	63.1	0.0	1.4	18.7	0.0
Queue Length 95th (m)	21.8	54.3	37.4	23.8	38.7	18.3	96.5	104.6	0.0	6.4	35.3	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	313	843	797	244	843	485	884	1824	882	481	944	515
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.40	0.73	0.28	0.28	0.36	0.36	0.43	0.02	0.02	0.23	0.10
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 96.6												
Natural Cycle: 105												
Control Type: Actuated-Uncoo	rdinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 28.9				In	tersection	LOS: C						
Intersection Capacity Utilization	n 66.2%			IC	CU Level o	f Service C	)					
Analysis Period (min) 15												
Colite and Discoss Or Trade		aaab										
Spins and Phases: 2: Tenth		JSeph						0	23	x		23

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32.3 s	59.3 s	14 s	29,1 s
		▲ Ø7	<b>∲</b> Ø8
		14 s	29.1 s

## 3: St. Joseph & Eric Czapnik PM Peak Hour

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>ቀ</b> ቶሴ		W.	
Traffic Volume (vph)	28	332	447	43	15	38
Future Volume (vph)	28	332	447	43	15	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.987		0.903	
Flt Protected	0.950				0.986	
Satd. Flow (prot)	1674	3349	4664	0	1569	0
Flt Permitted	0.950				0.986	
Satd. Flow (perm)	1674	3349	4664	0	1569	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	3%	1%	1%	1%
Adj. Flow (vph)	28	332	447	43	15	38
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	332	490	0	53	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0	5.	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane		0.0			0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
	Other					
Control Type: Unsignalized	Ourer					
				10		

Intersection Capacity Utilization 27.3% Analysis Period (min) 15

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Lane Configurations		۸t.		*	**		*		1	**	**	1
Traffic Volume (vnh)	0	368	3	105	328	0	4	0	89	60	936	164
Future Volume (vph)	0	368	3	105	328	0	4	0	80	60	936	16/
I deal Flow (vphpl)	1800	1800	1800	1800	1900	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Storage Lange	0.0		0.0	00.0		0.0	0.0		10.0	110.0		130.0
Topor Longth (m)	10.0		U	25.0		0	10.0		1	60.0		1
	1 00	0.05	0.05	1 00	0.05	1 00	1 00	1 00	1 00	0.07	0.05	1 00
Lane Ulli. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	0.00
		0.000					1.00		0.950			0.99
Fit Brotostad		0.999		0.050			0.050		0.000	0.050		0.000
Fil Fiolecieu	٥	2212	٥	1674	2216	٥	1252	٥	1100	0.950	2240	1401
Salu. Flow (prot)	U	3313	U	1074	3310	U	1000	U	1490	3240	5549	1401
Fit Permitted	0	2242	0	0.531	2240	0	0.950	0	1400	0.950	2240	1000
Sato. Flow (perm)	U	3313	U	930	3310	U	1352	U	1498	3248	3349	1383
		4	res			res			res			res
Sato. Flow (RTOR)		1			00			00	237		00	164
Link Speed (k/n)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
I ravel lime (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1				4.00	1	1				4.00	1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	1%	1%	2%	1%	25%	1%	1%	1%	1%	8%
Adj. Flow (vph)	0	368	3	105	328	0	4	0	89	60	936	164
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	371	0	105	328	0	4	0	89	60	936	164
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
Median Width(m)		5.0			4.5			7.0			7.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		Left	Thru		Left		Right	Left	Thru	Right
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		CI+Ex		CI+Ex	Cl+Ex		CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7						28.7	
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			Cl+Ex						CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0						0.0	
Turn Type		NA		Perm	NA		Prot		Free	Prot	NA	Perm
Protected Phases		4			8		5			1	6	
Permitted Phases				8					Free			6
Detector Phase		4		8	8		5			1	6	6
Switch Phase				-			-				-	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)		10.0		10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)		25.6		25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)		26.6		26.6	26.6		16.3			51.3	35.0	35.0
Total Split (%)		34.1%		34.1%	34.1%		20.9%			65.9%	44.9%	44.9%
Maximum Green (s)		20.0		20.0	20.0		10.0			44.3	28.0	28.0
Yellow Time (s)		3.7		3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)		2.9		2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)		0.0		0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)		6.6		6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag							Lead				Lag	Lag
Lead-Lag Optimize?							Yes				Yes	Yes
Vehicle Extension (s)		3.0		3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode		None		None	None		None			Min	Min	Min
Walk Time (s)		7.0		7.0	7.0						7.0	7.0
Flash Dont Walk (s)		12.0		12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)		1		0	0						1	1
Act Effct Green (s)		13.1		13.1	13.1		6.1		51.0	23.5	21.8	21.8
Actuated g/C Ratio		0.26		0.26	0.26		0.12		1.00	0.46	0.43	0.43
v/c Ratio		0.44		0.44	0.39		0.03		0.06	0.04	0.66	0.24
Control Delay		19.1		25.1	18.7		27.2		0.1	7.5	15.4	3.7
Queue Delay		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Total Delay		19.1		25.1	18.7		27.2		0.1	7.5	15.4	3.7
LOS		В		С	В		С		А	А	В	А
Approach Delay		19.1			20.2			1.2			13.3	
Approach LOS		В			С			А			В	
Queue Length 50th (m)		12.1		6.6	10.6		0.3		0.0	1.1	25.0	0.0
Queue Length 95th (m)		31.4		24.0	28.0		2.9		0.0	3.8	73.3	9.9
Internal Link Dist (m)		99.1			222.7			251.2			211.3	
Turn Bay Length (m)				60.0					15.0	110.0		130.0
Base Capacity (vph)		1375		388	1376		280		1498	2839	1945	872
Starvation Cap Reductn		0		0	0		0		0	0	0	0
Spillback Cap Reductn		0		0	0		0		0	0	0	0
Storage Cap Reductn		0		0	0		0		0	0	0	0
Reduced v/c Ratio		0.27		0.27	0.24		0.01		0.06	0.02	0.48	0.19
Intersection Summary												
Area Type: Of	ther											
Cycle Length: 77.9												
Actuated Cycle Length: 51												
Natural Cycle: 70												
Control Type: Actuated-Uncoordi	nated											
Maximum v/c Ratio: 0.66												
Intersection Signal Delay: 15.3				In	tersection	LOS: B						
Intersection Capacity Utilization 6	3.3%			IC	U Level of	Service B						
Analysis Period (min) 15												
Splits and Phases: 4: Old Tent	h Line/OR 1	74 EB R	amp & St.	Joseph								

₩ <sub>Ø1</sub>		→04	22
51.3 s		26.6 s	
↑ø5	Ø6	₹ Ø8	N. N.
16.3 s	35.s	26.6.s	

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		1111	<b>A</b> 12			1
Traffic Volume (vph)	0	977	627	15	0	8
Future Volume (vph)	0	977	627	15	0	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	25.0			0.0	0.0	0.0
Storage Lanes	1			0	0	1
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.86	0.95	0.95	1.00	1.00
Frt			0.996			0.865
Flt Protected						
Satd. Flow (prot)	0	6063	3334	0	0	1510
Flt Permitted						
Satd. Flow (perm)	0	6063	3334	0	0	1510
Link Speed (k/h)		60	60		50	
Link Distance (m)		55.5	82.1		81.0	
Travel Time (s)		3.3	4.9		5.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	1%	1%	2%	2%	2%
Adi, Flow (vph)	0	977	627	15	0	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	977	642	0	0	8
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.5	3.5		0.0	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	1.00	1.00	24	14
Sign Control	27	Free	Free	17	Stop	17
Intersection Summarv						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	on 28.8%			ICI	U Level of	Service A

Analysis Period (min) 15

## 1: St. Joseph & Vieux-Silo AM Peak Hour

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	8	**	<b>≜1</b> ⊾		M	
Traffic Volume (vph)	5	263	934	4	26	11
Future Volume (vph)	5	263	934	4	26	11
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0			-	10.0	-
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.999		0.960	
Flt Protected	0.950				0.966	
Satd, Flow (prot)	1674	3316	3313	0	1634	0
Flt Permitted	0.950				0.966	
Satd, Flow (perm)	1674	3316	3313	0	1634	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	8			8		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	2%	1%	1%	1%
Adi, Flow (vph)	5	263	934	4	26	11
Shared Lane Traffic (%)	5					
Lane Group Flow (vph)	5	263	938	0	37	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	LNA	R NA
Median Width(m)	20.1	5.0	3.5		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane		0.0			0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
later estime Orgenite Utilized	07 A0/			10		0 A

Intersection Capacity Utilization 37.4% Analysis Period (min) 15

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Lane Configurations	*	**	1	*	**	1	*	<u>.</u>	1	*	**	1
Traffic Volume (vph)	26	104	161	47	344	72	521	1106	26	9	253	69
Future Volume (vph)	26	104	161	/7	3//	72	521	1106	26	q	253	60
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0	1000	0.0	70.0	1000	0.0	160.0	1000	55.0	105.0	1000	60.0
Storage Lanes	0.0		0.0	10.0		0.0	100.0		1	100.0		00.0
Taper Length (m)	30.0		1	25.0		1	25.0		I	35.0		1
	1 00	0.05	1 00	1.00	0.05	1.00	20.0	0.01	1 00	1 00	0.05	1 00
Pod Piko Factor	0.00	0.35	0.00	1.00	0.55	0.07	0.31	0.31	1.00	1.00	0.55	1.00
	0.99		0.99	1.00		0.97			0.850			0.850
Fit Protocted	0.050		0.000	0.050		0.000	0.050	0 008	0.000	0.050		0.000
Fil Fiolected	1674	2216	1/55	1470	2216	1111	1500	2111	1111	1674	2202	1/00
Salu. Flow (plot)	0.465	3310	1400	0.601	3310	1441	0.050	0.000	1441	0.050	3203	1403
	0.400	2240	1105	0.021	2240	1200	0.950	0.990	1 1 1 1	0.950	2002	1400
Salu. Flow (perm)	012	3310	1430 Vaa	900	3310	1399 Vee	1009	3111	144 I Voo	10/4	3203	1403
			101			100			105			105
Sato. Flow (RTOR)		00	101		00	120		00	125		00	125
Link Speed (k/n)		60			60			60			60	
		82.1			144.1			338.4			235.5	
I ravel Time (s)	40	4.9	4		8.6	40		20.3			14.1	
Confl. Peds. (#/nr)	10	4.00	1	1	4.00	10	4.00	4.00	4.00	4.00	4.00	4.00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vph)	26	104	161	47	344	72	521	1106	26	9	253	69
Shared Lane Traffic (%)							10%					
Lane Group Flow (vph)	26	104	161	47	344	72	469	1158	26	9	253	69
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8		8			6			2
Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		7	7	1	1	1	1	1	1
Act Effct Green (s)	21.3	15.8	15.8	23.2	18.8	18.8	48.8	48.8	48.8	15.4	15.4	15.4
Actuated g/C Ratio	0.19	0.14	0.14	0.21	0.17	0.17	0.44	0.44	0.44	0.14	0.14	0.14
v/c Ratio	0.12	0.22	0.47	0.20	0.61	0.21	0.70	0.84	0.04	0.04	0.55	0.22
Control Delay	35.5	46.3	12.0	36.7	49.7	1.7	34.0	35.4	0.1	45.4	50.9	1.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.5	46.3	12.0	36.7	49.7	1.7	34.0	35.4	0.1	45.4	50.9	1.6
LOS	D	D	В	D	D	А	С	D	А	D	D	A
Approach Delay		26.3			40.9			34.4			40.5	
Approach LOS		С			D			С			D	
Queue Length 50th (m)	4.1	10.3	0.0	7.6	36.4	0.0	83.0	112.6	0.0	1.7	26.8	0.0
Queue Length 95th (m)	11.5	19.6	17.8	18.1	56.6	0.8	155.8	#192.7	0.0	6.5	41.0	0.2
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	227	723	438	241	723	403	758	1564	786	412	809	460
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.14	0.37	0.20	0.48	0.18	0.62	0.74	0.03	0.02	0.31	0.15
Intersection Summary												
Area Type: C	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 109.8												
Natural Cycle: 125												
Control Type: Actuated-Uncoord	linated											
Maximum v/c Ratio: 0.84					, e							
Intersection Signal Delay: 35.4	70.00/			In	tersection	LUS: D						
Intersection Capacity Utilization	78.0%			IC	U Level o	f Service D						
Analysis Period (min) 15												
# 95th percentile volume excended Queue shown is maximum af	eds capacil ter two cyc	ty, queue r les.	nay be lon	ger.								
Splits and Phases: 2. Tenth I	ine & St. Jo	seph										
		06						603	4	<b>1</b> 04		

Ø2	<b>√</b> ø <sub>6</sub>	Ø3	;	<i>4</i> ∕04
32.3 s	59.3 s	14 s		29.1 s
		▶ 07	,	<b>₩</b> Ø8
		14 s		29.1 s

## 3: St. Joseph & Eric Czapnik AM Peak Hour

	٦	-	-	•	1	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>**1</b>		W.	
Traffic Volume (vph)	21	113	420	24	15	61
Future Volume (vph)	21	113	420	24	15	61
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.992		0.892	
Flt Protected	0.950				0.990	
Satd. Flow (prot)	1674	3316	4685	0	1556	0
Flt Permitted	0.950				0.990	
Satd, Flow (perm)	1674	3316	4685	0	1556	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	3%	1%	1%	1%
Adj. Flow (vph)	21	113	420	24	15	61
Shared Lane Traffic (%)		-				
Lane Group Flow (vph)	21	113	444	0	76	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane		0.0			0.0	
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	0					
				10		· · · ·

Intersection Capacity Utilization 28.0%

ICU Level of Service A

Analysis Period (min) 15

Lane Configurations     EBL     EBT     SER     WBL     WBR     NBL     NBT     NBT     NBT     NBT     SER     SER       Lane Configurations     +		≯	<b>→</b>	$\mathbf{i}$	4	←	•	1	Ť	1	1	ŧ	~
Lane Configurations     Ap     N     Ap     N     Ap     N     Ap	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Tartific Volume (oph)     0     117     0     38     388     0     0     0     125     61     333     70       Future Volume (oph)     0     1110     1800 <t< td=""><td>Lane Configurations</td><td></td><td><b>A1</b></td><td></td><td>×</td><td>**</td><td></td><td>*</td><td></td><td>1</td><td>88</td><td>**</td><td>1</td></t<>	Lane Configurations		<b>A1</b>		×	**		*		1	88	**	1
Future Volume (wph)     0     111     0     38     388     0     0     0     1205     61     333     70       ideal Flow (wphp)     1800     100     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00     1.00 <td< td=""><td>Traffic Volume (vph)</td><td>0</td><td>111</td><td>0</td><td>38</td><td>368</td><td>0</td><td>0</td><td>0</td><td>125</td><td>61</td><td>353</td><td>70</td></td<>	Traffic Volume (vph)	0	111	0	38	368	0	0	0	125	61	353	70
isea     Fixed     1800 <t< td=""><td>Future Volume (vph)</td><td>0</td><td>111</td><td>0</td><td>38</td><td>368</td><td>0</td><td>0</td><td>0</td><td>125</td><td>61</td><td>353</td><td>70</td></t<>	Future Volume (vph)	0	111	0	38	368	0	0	0	125	61	353	70
Storage Length (m)     0.0     0.0     0.0     0.0     0.0     15.0     110.0     130.0       Storage Length (m)     10.0     0     1     0     1     1     2     1       Taper Length (m)     10.0     0.95     1.00     1.00     1.00     0.97     0.95     1.00       Lane UIR Factor     1.00     0.95     1.00     1.00     1.00     0.97     0.95       FH     0.950     0.950     0.950     0.950     0.850     0.850       Stall, Flow (prot)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1210       Stall, Flow (prot)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1210       Link Speed (khh)     60     50     60     60     60     121     131     246.7     275.2     235.3     122     131     141     14     141     141	Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Lanes     0     1     0     1     1     2     1       Taper Length (m)     100     35.0     100     100     60.0     60.0       Lane UIL Factor     100     0.95     0.95     1.00     0.950     0.850       FI     Detoted     0.950     0.850     0.850     0.850     0.850       FIP rotected     0.950     0.852     0.850     0.850     0.850     0.850       Stad, Flow (pront)     0.321     0.1642     3316     0.1762     0.14433     2982     3316     1115       Stad, Flow (pront)     0.321     0.1179     3316     0.1762     0.14433     2982     3316     1115       Stad, Flow (pront)     60     60     60     60     60     111     <	Storage Length (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Tape Length (m)     10.0     35.0     10.0     10.0     60.0       Lane Ull, Factor     1.00     0.95     0.95     1.00     0.95     1.00     0.97     0.95     1.00       FIP office     0.950     0.950     0.950     0.950     0.850     0.850       Said, Flow (prot)     0     3221     0     1642     3316     0     1762     0     1483     2982     3316     1210       Said, Flow (prot)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1195       Said, Flow (prot)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1195       Said, Flow (prot)     121     246.7     2752     285.3     114     1	Storage Lanes	0.0		0.0	1		0	1		10.0	2		100.0
Lane Util, Factor     1.00     0.95     0.95     1.00     0.95     1.00     1.00     1.00     0.07     0.95     0.09       Ped Bike Factor     0.950     0.850     0.850     0.850     0.850       FIP Potected     0.950     0.850     0.950     0.950     0.950       Satid, Flow (port)     0     321     0     1642     3316     0     1762     0     1483     2822     3316     11179     3316     0     1762     0     1483     2822     3316     11179     3316     0     1762     0     1483     2822     3316     11179     3316     0     1762     1483     2822     3316     11179     131     246.7     275.2     235.3     1121     111     1	Taper Length (m)	10.0		Ŭ	35.0		v	10.0		•	60 0		•
Ded Bike Factor     No     Out     No	Lane Litil Factor	1 00	0.95	0.95	1 00	0.95	1 00	1 00	1 00	1 00	0 97	0.95	1 00
Fit     0.950     0.850     0.850     0.850       FIP rotected     0.950     0.950     0.950     0.950       Stid. Flow (prot)     0.3221     0.1179     3316     0.1762     0.1483     2982     3316     1210       Stid. Flow (perm)     0.3221     0.1179     3316     0.1762     0.1483     2982     3316     1195       Right Turn on Red     Yes     Yes     Yes     Yes     Yes     Yes     Yes     System     132       Link Speed (k1)     60     50     70     73     73     70     70     73     73     70     70     73     73     70     70     70     70     70     70     70     70	Ped Bike Factor	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	0.07	0.00	0.99
Pic Protected     0.950     0.050     0.950     0.950       Satd, Flow (prot)     0     3221     0     1642     3316     0     1762     0     1483     2982     3316     1210       Satd, Flow (perm)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1150       Satd, Flow (perm)     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1155       Satd, Flow (Port)     1     <	Frt									0 850			0.850
Satid, Flow (prot)     0     3221     0     1642     3316     0     1762     0     1443     2982     3316     1210       FI Permitted     0.682     0     1762     0     1443     2982     3316     11210       Satid, Flow (perm)     0     3221     0     1179     3316     0     1762     0     1443     2982     3316     1210       Satid, Flow (perm)     0     3221     0     1179     3316     0     1762     0     1443     2982     3316     1210       Satid, Flow (pth)     60     60     60     60     60     141     1 </td <td>Fit Protected</td> <td></td> <td></td> <td></td> <td>0.950</td> <td></td> <td></td> <td></td> <td></td> <td>0.000</td> <td>0.950</td> <td></td> <td>0.000</td>	Fit Protected				0.950					0.000	0.950		0.000
Permitted   0   0.21   0   1179   3316   0   1762   0   1483   2982   3316   1195     Satd. Flow (perm)   0   3221   0   1179   3316   0   1762   0   1483   2982   3316   1195     Satd. Flow (PtrOR)   ****   ***	Satd Flow (prot)	0	3221	0	1642	3316	0	1762	0	1483	2982	3316	1210
Intrinuities     0     3221     0     1179     3316     0     1762     0     1483     2982     3316     1195       Righ Turn on Red     Yes	Elt Permitted	0	0221	0	0.682	0010	U	1102	0	1400	0.950	0010	1210
Dealer form (pchn)     O     DEaler     Fes	Satd Flow (perm)	0	3221	0	1179	3316	0	1762	0	1483	2982	3316	1195
Ball Flow (RTOR)     Bot     Flow     Bot	Right Turn on Red	0	0221	Yes	1175	0010	Yes	1102	0	Yes	2002	0010	Yes
Data Hole     Bodd	Satd Flow (RTOR)			100			100			839			132
Link Distance (m)     123.1     246.7     275.2     285.3       Travel Time (s)     7.4     14.8     16.5     14.1       Confl. Peds. (#hr)     1     1     1     1       Peak Hour Factor     1.00     1.05     1.05     1.05     1.05     1.05     1.05     1.05     1.06     1.05     1.06     1.06     1.05     1.07     1.00     1.09     1.09     1.09     1.09     1.09     1.09	Link Speed (k/b)		60			60			60	000		60	102
Link Distance (in)     Lev.1     Lev.1 <td>Link Distance (m)</td> <td></td> <td>123.1</td> <td></td> <td></td> <td>2/6 7</td> <td></td> <td></td> <td>275.2</td> <td></td> <td></td> <td>235.3</td> <td></td>	Link Distance (m)		123.1			2/6 7			275.2			235.3	
Induct mine (s)   I.Y   I.Y <thi.y< th="">   I.Y   <thi.y< th=""></thi.y<></thi.y<>	Travel Time (s)		7 /			1/1 8			16.5			1/ 1	
Deak Hour Factor     1.00	Confl Peds (#/hr)	1	· . <del>-</del>			14.0	1	1	10.5			17.1	1
Cath Noth Tack     Tool	Peak Hour Factor	1 00	1 00	1 00	1 00	1.00	1 00	1 00	1 00	1.00	1 00	1.00	1 00
Native Values (vs)     No     No <td></td> <td>1.00</td> <td>5%</td> <td>1.00</td> <td>3%</td> <td>2%</td> <td>1.00</td> <td>1%</td> <td>1.00</td> <td>2%</td> <td>1.00</td> <td>2%</td> <td>25%</td>		1.00	5%	1.00	3%	2%	1.00	1%	1.00	2%	1.00	2%	25%
No. Finder Varier   0   11   0   30   500   0   0   123   01   303   10     Lane Traffic (%)   111   0   38   368   0   0   0   125   61   353   70     Enter Blocked Intersection   No   No <td< td=""><td>Adi Flow (vph)</td><td>170</td><td>111</td><td>170</td><td>38</td><td>2 /0</td><td>170</td><td>170</td><td>170</td><td>125</td><td>61</td><td>2.70</td><td>2370</td></td<>	Adi Flow (vph)	170	111	170	38	2 /0	170	170	170	125	61	2.70	2370
Diale Claim Friew (vpl)     0     111     0     38     368     0     0     125     61     353     70       Enter Blocked Intersection     No     No <td< td=""><td>Shared Lane Traffic (%)</td><td>0</td><td>111</td><td>0</td><td>50</td><td>500</td><td>0</td><td>U</td><td>0</td><td>125</td><td>01</td><td>555</td><td>70</td></td<>	Shared Lane Traffic (%)	0	111	0	50	500	0	U	0	125	01	555	70
Lance Order Prof (vpr)     O     N1     No     No </td <td>Lane Group Flow (vph)</td> <td>0</td> <td>111</td> <td>0</td> <td>38</td> <td>368</td> <td>0</td> <td>0</td> <td>0</td> <td>125</td> <td>61</td> <td>353</td> <td>70</td>	Lane Group Flow (vph)	0	111	0	38	368	0	0	0	125	61	353	70
Line Alignment     Left     Left     Left     Right     Left     Right     Left     Right     Left     Right     Left     Right     Right     Left     Left     Right     Right     Left     Left     Right     Left     Right     Left     Right     Left     Right     Left	Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Later Angument     Left     KNA     Left     KNA     Left     Num       Width(m)     5.0     1.09	Lano Alignment	Loft	Loft			Loft	Diabt	Loft	Loft		Loft	Loft	Diaht
Median Holium     3.0     4.0     1.0     1.0     1.0     1.0       Link Offset(m)     0.0     0.0     0.0     0.0     0.0       Crosswalk Width(m)     5.0     1.09 </td <td>Median Width(m)</td> <td>LOIL</td> <td>5.0</td> <td></td> <td></td> <td>15</td> <td>Ngn</td> <td>Leit</td> <td>7.0</td> <td></td> <td>Leit</td> <td>7.0</td> <td>Ttight</td>	Median Width(m)	LOIL	5.0			15	Ngn	Leit	7.0		Leit	7.0	Ttight
Link Onsettin/n     0.0     0.0     0.0     0.0     0.0       Crosswalk Width(m)     5.0     10.0     5.0     5.0       Two way Left Turn Lane	Link Offcot(m)		0.0			4.5			7.0			7.0	
Ordswark Hutmin     5.0     10.0     5.0     5.0     5.0       Headway Factor     1.09 <td>Crosswalk Width(m)</td> <td></td> <td>5.0</td> <td></td> <td></td> <td>10.0</td> <td></td> <td></td> <td>5.0</td> <td></td> <td></td> <td>5.0</td> <td></td>	Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way bet for the lane   1.09			5.0			10.0			5.0			5.0	
Ineadway Pactor     1.05	Hoadway Easter	1 00	1 00	1 00	1 00	1.00	1 00	1 00	1 00	1 00	1 00	1 00	1 00
Turning Speed (Nf)     24     14	Turning Speed (k/h)	1.03	1.03	1.03	1.03	1.03	1.03	24	1.03	1.03	1.03	1.03	1.03
Number of Detectors     1	Number of Detectors	24	2	14	24	2	14	24		1	24	2	14
Detector (m)     30.5     6.1     30.5     6.1     6.1     6.1     30.5     6.1       Irailing Detector (m)     0.0	Number of Detectors		∠ Thru		ا ا	Z		ا ft		Right	ا hoft	∠ Thru	Right
Leading Detector (in)     30.3     0.1     30.3     0.1     0.1     0.1     30.3     0.1       Trailing Detector (in)     0.0	Leading Detector (m)		30.5		61	30.5		61		6 1	61	30.5	6 1
Interning Detector (III)     0.0 <td>Trailing Detector (m)</td> <td></td> <td>0.0</td> <td></td> <td>0.1</td> <td>0.0</td> <td></td> <td>0.1</td> <td></td> <td>0.1</td> <td>0.1</td> <td>0.0</td> <td>0.1</td>	Trailing Detector (m)		0.0		0.1	0.0		0.1		0.1	0.1	0.0	0.1
Detector 11 Size(m)     1.8     6.1     1.8     6.1     6.1     6.1     6.1     1.8     6.1       Detector 1 Type     CI+Ex     <	Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(iii)     1.3     0.1     1.0     0.1     0.1     0.1     0.1     0.1     1.0     0.1       Detector 1 Type     Cl+Ex     Cl+Ex <t< td=""><td>Detector 1 Size(m)</td><td></td><td>1.8</td><td></td><td>6.1</td><td>1.8</td><td></td><td>6.1</td><td></td><td>6.1</td><td>6.1</td><td>1.8</td><td>6.1</td></t<>	Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Channel     OFEX     OFEX <td>Detector 1 Type</td> <td></td> <td>CI+Ev</td> <td></td> <td></td> <td>CI+Ev</td> <td></td> <td>CI+Ev</td> <td></td> <td></td> <td></td> <td>CI+Ev</td> <td></td>	Detector 1 Type		CI+Ev			CI+Ev		CI+Ev				CI+Ev	
Detector 1 Extend (s)     0.0     1.8     1.9     1.9	Detector 1 Channel												
Detector 1 Queue (s)     0.0	Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector I Delay (s)     0.0	Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector Position (m)     28.7     0.0	Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Size(m) 1.8 1.8 1.8   Detector 2 Size(m) 1.8 1.8   Detector 2 Type CI+Ex CI+Ex   Detector 2 Channel 0.0 0.0   Detector 2 Extend (s) 0.0 0.0   Turn Type NA Perm NA   Permitted Phases 4 8 5 1   Detector Phase 4 8 5 1 6	Detector 2 Position(m)		28.7		0.0	28.7		0.0		0.0	0.0	28.7	0.0
Detector 2 Size(III)1.01.01.0Detector 2 TypeCI+ExCI+ExDetector 2 ChannelDetector 2 Extend (s)0.00.0Turn TypeNAPermProtected Phases4885194898591691691691691691691691691691691691691691191 <td>Detector 2 Fosition(in)</td> <td></td> <td>1.0</td> <td></td> <td></td> <td>20.7</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>20.7</td> <td></td>	Detector 2 Fosition(in)		1.0			20.7						20.7	
Detector 2 ChannelOHEXOHEXOHEXDetector 2 ChannelDetector 2 Extend (s)0.00.00.0Turn TypeNAPermNAProtFreeProtected Phases48516Permitted Phases8Free6Detector Phase48516Detector Phase48516	Detector 2 Size(iii)												
Detector 2 Extend (s)0.00.00.0Turn TypeNAPermNAProtFreeProtNAPermProtected Phases48516Permitted Phases8516Detector Phase48516Detector Phase48516Output85166	Detector 2 Channel												
Turn TypeNAPermNAProtFreeProtNAPermProtected Phases48516Permitted Phases8Free6Detector Phase48516Output8516Detector Phase48516	Detector 2 Extend (c)		0.0			0.0						0.0	
Protected Phases48516Permitted Phases8Free6Detector Phase488516Switch Phase488516			0.0		Dorm	0.0		Drot		Froo	Prot	0.0	Dorm
Permitted Phases8Free6Detector Phase488516Switch Phase4885166	Protected Phases				Feim	INA 0		FIUL		Fiee	1	NA C	Feini
Detector Phase     4     8     8     5     1     6     6       Switch Phase     4     8     8     5     1     6     6	Pormitted Phases		4		0	0		5		Eroo	I	O	e
Detector Filase 4 0 0 0 1 0 0	Petrotor Phase		4		0	0		E		Fiee	4	C	0
	Switch Phase		4		0	0		5			I	U	0

J.Audia, Novatech

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Lane Group	EBL EBT	EBR WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)	25.6	25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)	31.6	31.6	31.6		16.3			50.3	34.0	34.0
Total Split (%)	38.6%	38.6%	38.6%		19.9%			61.4%	41.5%	41.5%
Maximum Green (s)	25.0	25.0	25.0		10.0			43.3	27.0	27.0
Yellow Time (s)	3.7	3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)	2.9	2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)	0.0	0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)	6.6	6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag					Lead				Lag	Lag
Lead-Lag Optimize?					Yes				Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	None	None	None		None			Min	Min	Min
Walk Time (s)	7.0	7.0	7.0						7.0	7.0
Flash Dont Walk (s)	12.0	12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)	1	0	0						1	1
Act Effct Green (s)	11.6	11.6	11.6				42.0	16.5	16.5	16.5
Actuated g/C Ratio	0.28	0.28	0.28				1.00	0.39	0.39	0.39
v/c Ratio	0.12	0.12	0.40				0.08	0.05	0.27	0.13
Control Delay	12.1	12.8	14.0				0.1	8.6	9.6	1.1
Queue Delay	0.0	0.0	0.0				0.0	0.0	0.0	0.0
Total Delay	12.1	12.8	14.0				0.1	8.6	9.6	1.1
LOS	В	В	В				А	А	А	А
Approach Delay	12.1		13.9			0.1			8.3	
Approach LOS	В		В			А			А	
Queue Length 50th (m)	2.6	1.7	9.5				0.0	1.0	7.1	0.0
Queue Length 95th (m)	7.5	7.1	21.1				0.0	4.1	17.6	1.7
Internal Link Dist (m)	99.1		222.7			251.2			211.3	
Turn Bay Length (m)		60.0					15.0	110.0		130.0
Base Capacity (vph)	1957	716	2015				1483	2850	2176	829
Starvation Cap Reductn	0	0	0				0	0	0	0
Spillback Cap Reductn	0	0	0				0	0	0	0
Storage Cap Reductn	0	0	0				0	0	0	0
Reduced v/c Ratio	0.06	0.05	0.18				0.08	0.02	0.16	0.08
Intersection Summary										
Area Type: Oth	ner									
Cycle Length: 81.9										
Actuated Cycle Length: 42										
Natural Cycle: 70										
Control Type: Actuated-Uncoordin	ated									
Maximum v/c Ratio: 0.40										
Intersection Signal Delay: 9.8			ntersection	LOS: A						
Intersection Capacity Utilization 35	5.0%		CU Level o	f Service A						
Analysis Period (min) 15										
Splits and Phases: 4: Old Tenth	Line/OR 174 EB F	Ramp & St. Joseph								

Ø1		<b>→</b> Ø4
50.3 s		31.6 s
Ø5		₹Ø8
16.3 s	34 s	31.6 s

## 1: St. Joseph & Vieux-Silo PM Peak Hour

	٦	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	**	<b>≜1</b> ⊾		W.	
Traffic Volume (vph)	14	1094	703	14	14	6
Future Volume (vph)	14	1094	703	14	14	6
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	85.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util, Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Frt			0.997		0.959	
Flt Protected	0.950				0.966	
Satd, Flow (prot)	1674	3349	3338	0	1633	0
Flt Permitted	0.950				0.966	
Satd, Flow (perm)	1674	3349	3338	0	1633	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		410.1	145.9		128.9	
Travel Time (s)		24.6	8.8		9.3	
Confl. Peds. (#/hr)	7			7		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%
Adi, Flow (vph)	14	1094	703	14	14	6
Shared Lane Traffic (%)						
Lane Group Flow (vph)	14	1094	717	0	20	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	LNA	R NA
Median Width(m)	2011	5.0	3.5		3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	5.0		5.0	
Two way Left Turn Lane		0.0	0.0		0.0	
Headway Factor	1 09	1 09	1 09	1 09	1 09	1 09
Turning Speed (k/h)	24	1.00	1.00	14	24	14
Sign Control		Free	Free		Stop	
Intersection Summany						
Area Tuno:	Other					
Area Type.	Other					
Lonuor Type: Unsignalized	tion 11 00/				الم امينا م	

Intersection Capacity Utilization 41.9% Analysis Period (min) 15

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<b>N</b>	**	1	<u>8</u>	**	1	<b>3</b>	<b>⊿</b> 14	1	<b>X</b>	**	1
Traffic Volume (vph)	70	380	659	77	265	196	406	884	20	10	303	55
Future Volume (vph)	70	380	659	77	265	196	406	884	20	10	303	55
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	70.0		0.0	160.0		55.0	105.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	30.0		•	25.0		•	25.0		•	35.0		•
Lane Litil Factor	1 00	0.95	1 00	1 00	0.95	1 00	0.91	0.91	1 00	1 00	0.95	1 00
Ped Bike Factor	0.99	0.00	0.99	1.00	0.00	0.97	0.01	0.01	0.98	1.00	0.00	1.00
Frt	0.00		0.850	1.00		0.850			0.850	1.00		0.850
Fit Protected	0 950		0.000	0 950		0.000	0 950	0 998	0.000	0 950		0.000
Satd Flow (prot)	167/	3316	1/155	1/70	3316	1//1	1500	3111	1//1	167/	3283	1/183
Elt Permitted	0.552	0010	1400	0 305	0010	1771	0.050	0 008	1771	0 050	5205	1-00
Sate Flow (parm)	0.002	2216	1/35	611	2216	1200	1500	2111	1/17	1672	3083	1/93
Pight Turn on Red	902	5510	1400 Voc	011	3310	1399 Voc	1009	JIII	Voc	1072	5205	1403 Voc
Sate Flow (PTOP)			650			106			105			105
Link Speed (k/b)		60	039		60	190		60	120		60	120
Link Opeed (K/II)		00			111 1			220 4			00 025 5	
		02.1			144.1			00.4 00.2			200.0	
Confl Dada (#/hr)	10	4.9	1	1	0.0	10		20.3	2	C	14.1	
Confi. Peas. (#/nr)	10	4.00	1 00	1 00	4.00	10	4.00	4.00	3	3	4.00	4 00
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Venicles (%)	1%	2%	4%	15%	2%	5%	2%	4%	5%	1%	3%	2%
Adj. Flow (vpn)	70	380	659	11	265	196	406	884	20	10	303	55
Shared Lane Traffic (%)		000	050		0.05	100	10%	005	00	40	000	
Lane Group Flow (vph)	70	380	659	11	265	196	365	925	20	10	303	55
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	Left	Right	L NA	Left	Right	L NA	Left	R NA	L NA	Left	R NA
Median Width(m)		7.0			7.0			5.0			5.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			5.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	7	4		3	8		6	6		2	2	
Permitted Phases	4		4	8		8			6			2
Detector Phase	7	4	4	3	8	8	6	6	6	2	2	2
Switch Phase												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	22.0	22.0	22.0	10.0	10.0	10.0
Minimum Split (s)	11.0	29.1	29.1	11.0	29.1	29.1	32.3	32.3	32.3	32.3	32.3	32.3
Total Split (s)	14.0	29.1	29.1	14.0	29.1	29.1	59.3	59.3	59.3	32.3	32.3	32.3
Total Split (%)	10.4%	21.6%	21.6%	10.4%	21.6%	21.6%	44.0%	44.0%	44.0%	24.0%	24.0%	24.0%
Maximum Green (s)	8.0	23.0	23.0	8.0	23.0	23.0	53.0	53.0	53.0	26.0	26.0	26.0
Yellow Time (s)	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7	3.7
All-Red Time (s)	2.3	2.4	2.4	2.3	2.4	2.4	2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.1	6.1	6.0	6.1	6.1	6.3	6.3	6.3	6.3	6.3	6.3
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	None	None
Walk Time (s)		7.0	7.0		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		16.0	16.0		16.0	16.0	19.0	19.0	19.0	19.0	19.0	19.0
Pedestrian Calls (#/hr)		1	1		9	9	1	1	1	1	1	1
Act Effct Green (s)	26.1	20.2	20.2	26.2	20.2	20.2	41.4	41.4	41.4	16.7	16.7	16.7
Actuated g/C Ratio	0.24	0.19	0.19	0.24	0.19	0.19	0.38	0.38	0.38	0.15	0.15	0.15
v/c Ratio	0.25	0.62	0.82	0.37	0.43	0.47	0.63	0.78	0.03	0.04	0.60	0.16
Control Delay	35.0	48.8	13.3	38.8	45.2	10.5	34.8	36.0	0.1	45.1	51.0	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.0	48.8	13.3	38.8	45.2	10.5	34.8	36.0	0.1	45.1	51.0	1.1
LOS	D	D	В	D	D	В	С	D	А	D	D	Α
Approach Delay		26.8			31.6			35.1			43.3	
Approach LOS		С			С			D			D	
Queue Length 50th (m)	10.4	38.3	0.0	11.6	25.6	0.0	67.2	92.0	0.0	1.8	31.8	0.0
Queue Length 95th (m)	24.5	62.3	42.4	26.9	44.1	19.9	111.8	131.1	0.0	6.7	48.7	0.0
Internal Link Dist (m)		58.1			120.1			314.4			211.5	
Turn Bay Length (m)				70.0			160.0		55.0	105.0		60.0
Base Capacity (vph)	287	749	834	215	749	468	786	1621	798	428	839	472
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.51	0.79	0.36	0.35	0.42	0.46	0.57	0.03	0.02	0.36	0.12
Intersection Summary												
Area Type:	Other											
Cycle Length: 134.7												
Actuated Cycle Length: 108.6												
Natural Cycle: 105												
Control Type: Actuated-Uncoor	dinated											
Maximum v/c Ratio: 0.82												
Intersection Signal Delay: 32.7				In	tersection	LOS: C						
Intersection Capacity Utilization	71.9%			IC	U Level o	f Service C	;					
Analysis Period (min) 15												
Splits and Phases: 2: Tenth I	_ine & St. Jo	oseph						1		1		25

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32.3 s	59.3 s	14 s	29,1 s
		▲ Ø7	<b>∲</b> Ø8
		14 s	29.1 s

## 3: St. Joseph & Eric Czapnik PM Peak Hour

	٦	-	←	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	**	<b>**%</b>		¥.	
Traffic Volume (vph)	28	376	507	43	15	38
Future Volume (vph)	28	376	507	43	15	38
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	40.0			0.0	0.0	0.0
Storage Lanes	1			0	1	0
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.95	0.91	0.91	1.00	1.00
Ped Bike Factor						
Frt			0.988		0.903	
Flt Protected	0.950				0.986	
Satd. Flow (prot)	1674	3349	4668	0	1569	0
Flt Permitted	0.950				0.986	
Satd. Flow (perm)	1674	3349	4668	0	1569	0
Link Speed (k/h)		60	60		50	
Link Distance (m)		144.1	123.1		184.3	
Travel Time (s)		8.6	7.4		13.3	
Confl. Peds. (#/hr)	5			5		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	1%	3%	1%	1%	1%
Adj. Flow (vph)	28	376	507	43	15	38
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	376	550	0	53	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	L NA	R NA
Median Width(m)		5.5	5.0	Ŭ	3.5	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		5.0	10.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 28.4% Analysis Period (min) 15

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Lane Group	FBI	FBT	FBR	WBI	WBT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Lane Configurations		۸t.		*	**		*		1	**	**	1
Traffic Volume (vph)	0	417	4	119	369	0	5	0	102	68	1065	185
Future Volume (vph)	0	/17	- 1	110	360	0	5	0	102	68	1065	185
I deal Elow (vphpl)	1800	1800	1900	1800	1800	1800	1800	1800	1800	1800	1800	1900
Storago Longth (m)	0.0	1000	0.0	60.0	1000	0.0	0.0	1000	15.0	110.0	1000	130.0
Storage Lange	0.0		0.0	00.0		0.0	0.0		10.0	110.0 2		130.0
Tapar Longth (m)	10.0		0	25.0		0	10.0		1	60.0		1
	1 00	0.05	0.05	1 00	0.05	1 00	1 00	1.00	1 00	0.07	0.05	1 00
Lane Ulli. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	0.97	0.95	0.00
		0.000					1.00		0.050			0.99
FIL Fit Drotootod		0.999		0.050			0.050		0.000	0.050		0.000
Fil Plotected	0	2242	0	0.950	2240	0	1252	0	1400	0.950	2240	1401
Sato. Flow (prot)	0	3313	U	1074	3310	U	1353	U	1498	3248	3349	1401
Fit Permitted	•	0040	0	0.506	0040	0	0.950	0	4400	0.950	0040	4000
Satd. Flow (perm)	0	3313	0	892	3316	0	1352	0	1498	3248	3349	1383
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1							237			185
Link Speed (k/h)		60			60			60			60	
Link Distance (m)		123.1			246.7			275.2			235.3	
Travel Time (s)		7.4			14.8			16.5			14.1	
Confl. Peds. (#/hr)	1					1	1					1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	1%	2%	1%	1%	2%	1%	25%	1%	1%	1%	1%	8%
Adj. Flow (vph)	0	417	4	119	369	0	5	0	102	68	1065	185
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	421	0	119	369	0	5	0	102	68	1065	185
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	R NA	L NA	Left	Right	Left	Left	R NA	Left	Left	Right
Median Width(m)		5.0			4.5			7.0			7.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		5.0			10.0			5.0			5.0	
Two way Left Turn Lane												
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors		2		1	2		1		1	1	2	1
Detector Template		Thru		Left	Thru		Left		Right	Left	Thru	Right
Leading Detector (m)		30.5		6.1	30.5		6.1		6.1	6.1	30.5	6.1
Trailing Detector (m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Position(m)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Size(m)		1.8		6.1	1.8		6.1		6.1	6.1	1.8	6.1
Detector 1 Type		CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7		0.0	28.7		0.0		0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8						1.8	
Detector 2 Type		CI+Ex			CI+Ev						CI+Ev	
Detector 2 Channel												
Detector 2 Extend (c)		0.0			0.0						0.0	
		0.0		Dorm	0.0		Drot		Froe	Prot	0.0 NA	Dorm
Protocted Disease				Feilin	INA 0				Fiee	101	NA 6	Fein
Protected Phases		4		0	Õ		5		Гтор	I	Ö	
Permitted Phases		4		8	^		-		Free	4	^	6
Detector Phase		4		ð	ð		5			1	6	6
Switch Phase												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)		10.0		10.0	10.0		5.0			5.0	15.0	15.0
Minimum Split (s)		25.6		25.6	25.6		11.3			12.0	30.0	30.0
Total Split (s)		26.6		26.6	26.6		16.3			51.3	35.0	35.0
Total Split (%)	3	4.1%		34.1%	34.1%		20.9%			65.9%	44.9%	44.9%
Maximum Green (s)		20.0		20.0	20.0		10.0			44.3	28.0	28.0
Yellow Time (s)		3.7		3.7	3.7		3.7			3.7	3.7	3.7
All-Red Time (s)		2.9		2.9	2.9		2.6			3.3	3.3	3.3
Lost Time Adjust (s)		0.0		0.0	0.0		0.0			0.0	0.0	0.0
Total Lost Time (s)		6.6		6.6	6.6		6.3			7.0	7.0	7.0
Lead/Lag							Lead				Lag	Lag
Lead-Lag Optimize?							Yes				Yes	Yes
Vehicle Extension (s)		3.0		3.0	3.0		3.0			3.0	3.0	3.0
Recall Mode	I	None		None	None		None			Min	Min	Min
Walk Time (s)		7.0		7.0	7.0						7.0	7.0
Flash Dont Walk (s)		12.0		12.0	12.0						16.0	16.0
Pedestrian Calls (#/hr)		1		0	0						1	1
Act Effct Green (s)		14.0		14.0	14.0		6.1		54.8	26.6	24.7	24.7
Actuated g/C Ratio		0.26		0.26	0.26		0.11		1.00	0.49	0.45	0.45
v/c Ratio		0.50		0.52	0.44		0.03		0.07	0.04	0.71	0.26
Control Delay		20.8		29.3	20.2		28.4		0.1	7.7	17.0	3.6
Queue Delay		0.0		0.0	0.0		0.0		0.0	0.0	0.0	0.0
Total Delay		20.8		29.3	20.2		28.4		0.1	7.7	17.0	3.6
LOS		C		С	C		С		A	A	B	A
Approach Delay		20.8			22.4			1.4			14./	
Approach LOS		47.0		0.4			0.4	A	0.0		22 O	0.0
Queue Length 50th (m)		17.3		9.4	15.0		0.4		0.0	1.4	33.0	0.0
Queue Length 95th (m)		35.0		27.0	31.3		3.5	054.0	0.0	4.Z	#90.2	10.5
Turn Day Length (m)		99.1		60.0	222.1			201.Z	15.0	110.0	211.3	120.0
Page Canacity (unb)		1062		240	1064		057		1409	2677	1707	130.0
Stanuation Can Poducta		1203		040	1204		237		1490	2011	0	024
Spillback Can Reductin		0		0	0		0		0	0	0	0
Storage Cap Reductn		0		0	0		0		0	0	0	0
Reduced v/c Ratio		0 33		0 35	0.29		0.02		0.07	0.03	0.60	0 22
		0.00		0.00	0.20		0.02		0.07	0.00	0.00	0.22
Area Turau Ot	hor											
Area Type. Or	ner											
Actuated Cycle Length: 54.8												
Natural Cycle: 70												
Control Type: Actuated Uncoordi	nated											
Maximum v/c Ratio: 0.71	nateu											
Intersection Signal Delay: 16.8				In	tersection	LOS B						
Intersection Capacity Utilization 6	8.5%					f Service C						
Analysis Period (min) 15	0.070											
# 95th percentile volume exceeds capacity, queue may be longer												
Queue shown is maximum after two cycles.												
Splits and Phases: 4: Old Tent	h Line/OR 17	4 EB R	amp & St.	Joseph								



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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		1111	<b>4</b> 16			1
Traffic Volume (vph)	0	1109	711	15	0	8
Future Volume (vph)	0	1109	711	15	0	8
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800
Storage Length (m)	25.0			0.0	0.0	0.0
Storage Lanes	1			0	0	1
Taper Length (m)	30.0				10.0	
Lane Util. Factor	1.00	0.86	0.95	0.95	1.00	1.00
Frt			0.997			0.865
Flt Protected						
Satd. Flow (prot)	0	6063	3338	0	0	1510
Flt Permitted						
Satd. Flow (perm)	0	6063	3338	0	0	1510
Link Speed (k/h)		60	60		50	
Link Distance (m)		55.5	82.1		81.0	
Travel Time (s)		3.3	4.9		5.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	2%	1%	1%	2%	2%	2%
Adj. Flow (vph)	0	1109	711	15	0	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1109	726	0	0	8
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.5	3.5	Ŭ	0.0	J
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		0.0	0.0		5.0	
Two way Left Turn Lane						
Headway Factor	1.09	1.09	1.09	1.09	1.09	1.09
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 31.2%			ICI	U Level of	Service A

Analysis Period (min) 15