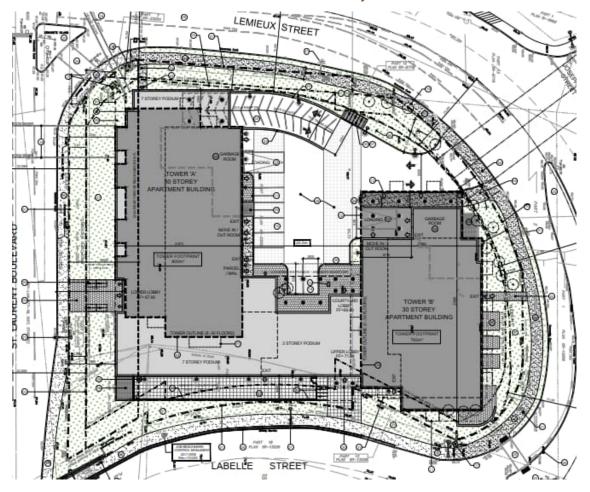
SERVICING & STORMWATER MANAGEMENT REPORT 1209 ST. LAURENT BOULEVARD, OTTAWA



Project No.: CCO-22-1215

City File No.: D07-12-22-0089

Prepared for:

1209 St. Laurent Limited Partnership.

900-2000 Peel Street, Montreal, QC H3A 2W5.

Prepared by:

McIntosh Perry Consulting Engineers Ltd. 115 Walgreen Road Carp, ON K0A 1L0

Rev 1: December 14, 2022

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1.0 PROJECT DESCRIPTION

1.1 Purpose

McIntosh Perry (MP) has been retained by 1209 St. Laurent Limited Partnership. to prepare this Servicing and Stormwater Management Report in support of the Site Plan Control application for the proposed 30-storey residential buildings, located at 1209 St. Laurent Boulevard within the City of Ottawa.

The main purpose of this report is to present a servicing design for the development in accordance with the recommendations and guidelines provided by the City of Ottawa (City), the Rideau Valley Conservation Authority (RVCA), and the Ministry of the Environment, Conservation and Parks (MECP). This report will address the water, sanitary and storm sewer servicing for the development, ensuring that existing and available services will adequately service the proposed development.

This report should be read in conjunction with the following drawings:

- COO-22-1215, C101 Site Grading and Drainage Plan, and
- COO-22-1215, C102 Ste Servicing Plan.
- CCO-22-1215, PRE Pre-Development Drainage Area Plan (Appendix E)
- CCO-22-1215, POST Post-Development Drainage Area Plan (Appendix F)

1.2 Ste Description

The property is located at 1209 St. Laurent Boulevard. It is described as Plan 4R-33004 and Plan 5R-9619, Part of Lots 4 and 14, Concession 2 (Ottawa Front), Ward 11 Beacon Hill-Cyrville, City of Ottawa. The land in question covers approximately 0.43 ha and is bounded by St. Laurent Boulevard, Lemieux Street and an existing ramp accessing St. Laurent Boulevard. The development area for the proposed works is approximately 0.43 ha. The site is zoned for Transit Oriented Development (TD3). See Site Location Plan in Appendix 'A' for more details.

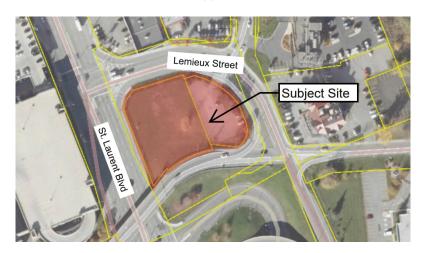


Figure 1: Site Map

1.3 Proposed Development and Statistics

The proposed development consists of two 30-storey residential towers, tower A, complete with a 7-storey podium along St. Laurent Boulevard and a 2-storey podium link to tower B. Visitor parking and drive aisles will be provided through an internal courtyard. Underground parking will be provided for residents and visitors with site access extending from Lemieux Street. Further details are provided in the Ste Plan provided by Poderick Lahey Architects. Refer to Appendix B.

1.4 Existing Conditions and Infrastructure

The existing site is currently undeveloped with a small parking lot fronting Lemieux Street. The existing site has no sanitary or water services, however, stormwater from the parking lot is captured by an existing catchbasin on site. Stormwater from the remainder of the site travels overland to the City right of ways where it is captured by existing municipal catchbasins.

Sewer and watermain mapping collected from the City of Ottawa indicate that the following services exist across the property frontages within the adjacent municipal rights-of-way(s):

St. Laurent Boulevard

- 406mm diameter unlined cast iron watermain
- o 762mm diameter transmission watermain
- 525mm diameter concrete sanitary sewer, tributary to the Cyrville Road Collector
- 600mm diameter concrete storm sewer, tributary to the Rideau River with approximately
 2.4 Km to the outlet, and
- 400-450mm diameter concrete storm sewer, tributary to the Rideau River

Lemieux Street

- o 203mm diameter unlined cast iron watermain
- 250mm diameter asbestos concrete sanitary sewer, tributary to the Cyrville Road Collector, and a
- 300mm diameter concrete storm sewer, tributary to the Rideau River with approximately
 2.4 Km to the outlet

1.5 Approvals

The proposed development is subject to the City of Ottawa site plan control process. Site plan control requires the City to review, provided concurrence and approve the engineering design package. Permits to construct can be requested once the City has issued a site plan agreement.

An Environmental Compliance Approval (ECA) through the Ministry of Environment, Conservation and Parks (MECP) is not anticipated to be required for the development since the development is contained within a single parcel of land, is not within a combined sewershed, and does not propose

industrial sewage. As a result, the stormwater management system meets the exemption requirements under O.Reg 525/90..

2.0 BACKROUND STUDIES, STANDARDS, AND REFRENCES

2.1 Background Reports / Reference Information

Background studies that have been completed for the proposed site include City of Ottawa as-built drawings, a topographical survey, a geotechnical report and a Phase I & II Environmental Ste Assessment (ESA).

As-built drawings of existing services within the vicinity of the proposed site were reviewed in order to determine accurate servicing and stormwater management schemes for the site.

A topographic survey of the site (Job No. 21851-21) was completed by Annis, O'Sullivan, Vollebekk LTD., dated March 10, 2022.

The following reports have previously been completed and are available under separate cover:

- Geotechnical Investigation completed by Paterson Group, dated December 12, 2022.
- Phase One Environmental Ste Assessment completed by GHD Group, dated January 10, 2022.
- Phase Two Environmental Site Assessment completed by GHD Group, dated January 11, 2022.

2.2 Applicable Guidelines and Standards

Oity of Ottawa:

- ♦ Ottawa Sewer Design Guidelines, City of Ottawa, SDG002, October 2012. (Ottawa Sewer Guidelines)
 - Technical Bulletin ISTB-2014-01 City of Ottawa, February 2014. (ISTB-2014-01)
 - Technical Bulletin PIEDTB-2016-01 City of Ottawa, September 2016. (PIEDTB-2016-01)
 - Technical Bulletin ISTB-2018-01 City of Ottawa, January 2018. (ISTB-2018-01)
 - Technical Bulletin ISTB-2018-03 City of Ottawa, March 2018. (ISTB-2018-03)
 - Technical Bulletin ISTB-2019-01 City of Ottawa, January 2019. (ISTB-2019-01)
 - Technical Bulletin ISTB-2019-02 City of Ottawa, February 2019. (ISTB-2019-02)
- Ottawa Design Guidelines Water Distribution City of Ottawa, July 2010. (Ottawa Water Guidelines)
 - Technical Bulletin ISD-2010-2 City of Ottawa, December 15, 2010. (ISD-2010-2)
 - Technical Bulletin ISDTB-2014-02 City of Ottawa, May 2014. (ISDTB-2014-02)
 - Technical Bulletin ISTB-2018-02 City of Ottawa, March 2018. (ISTB-2018-02)

Ministry of Environment, Conservation and Parks:

- ◆ Stormwater Planning and Design Manual, Ministry of the Environment, March 2003. (MECP Stormwater Design Manual)
- ◆ Design Guidelines for Sewage Works, Ministry of the Environment, 2008. (MEOP Sewer Design Guidelines)

Other:

Water Supply for Public Fire Protection, Fire Underwriters Survey, 2020. (FUS Guidelines)

3.0 PRE-CONSULTATION SUMMARY

A pre-consultation meeting was conducted on August 11, 2021 regarding the proposed site. Specific design parameters to be incorporated within this design include the following:

- Pre-development and post-development flows shall be calculated using a time of concentration (Tc) of 10 minutes.
- Control post-development flows to the pre-development flows with a combined C value of 0.50.
- Coordination with the RVCA is required to confirm quality control requirements.

The notes from the City of Ottawa can be found in Appendix 'B'.

WATERMAIN 4.0

4.1 Existing Watermain

The site is located within the 1E pressure zone, as per the Water Distribution System mapping included in Appendix C. There is an existing 203mm diameter UCI watermain within Lemieux Street. The watermain provides servicing to the municipal fire hydrant along the North side of Lemieux Street.

4.2 Proposed Watermain

Two new 150mm diameter PVC water services are proposed to service the development complete with water valves located at the property line. The water services are proposed to be serviced by the existing 203 mm diameter watermain within Lemieux Street. The services are designed to have a minimum of 2.4m cover. Refer to drawing C102 for a detailed servicing layout.

The Fire Underwriters Survey 2020 (FUS) method was utilized to determine the required fire flow for the site. The 'C' factor (type of construction) for the FUS calculation was determined to be 0.8 (non-combustible type). The total floor area ('A' value) for the FUS calculation was determined to be 5,214.0 m². The results of the calculations yielded a required fire flow of 7,000 L/min. The detailed calculations for the FUScan be found in Appendix 'C.

The water demands for the proposed building have been calculated to adhere to the Ottawa Design Guidelines – Water Distribution manual and can be found in Appendix 'C. The results have been summarized in Table 1, below. In accordance with Section 4.3.1 of the guidelines, service areas with a basic day demand greater than 50 m³/day require a dual connection to the municipal system. The basic day demand for the development is estimated to be 304 m³/day, therefore a dual connection is required.

Table 1: Water Supply Design Criteria and Water Demands

Ste Area	0.143 ha		
Residential	280 L/person/day		
1 Bedroom Apartment	1.4 persons/unit		
2 Bedroom Apartment 2.1 persons/unit			
Bachelor Apartment	1.4 persons/unit		
Maximum Daily Peaking Factor	2.5 x avg day		
Maximum Hour Peaking Factor	5.5 x max day		
Average Day Demand (L/s)	3.52		
Maximum Daily Demand (L/s) 8.80			
Peak Hourly Demand (L/s)	19.36		
FUS Fire How Requirement (L/s)	116.66 (7,000 L/min)		

The City provided the estimated water pressures at both for the average day scenario, peak hour scenario and the max day plus fire flow scenario for the demands indicated by the correspondence

in Appendix C. The resulting pressures for the boundary conditions results are shown in Table 2, below.

Table 2: Boundary Conditions Results

Scenario	Proposed Demands (L/s)	Connection HGL(m H₂O)*/kPa		
Average Day Demand	3.52	51.2 / 501.9		
Maximum Daily + Fire How Demand	125.47	43.1 / 422.4		
Peak Hourly Demand	19.36	42.9 / 420.5		
* Adjusted for an estimated ground elevation of 67.34m above the connection point.				

The normal operating pressure range is anticipated to be 421 kPa to 502 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi).

To confirm the adequacy of fire flow to protect the proposed development, public fire hydrants within 150 m of the proposed building were analysed per City of Ottawa ISTB 2018-02 Appendix I Table 1. Based on City guidelines (ISTB-2018-02), the existing hydrants can provide adequate fire protection to the proposed development. The results are summarized in Table 3, below.

Table 3: Fire Protection Confirmation

Building	Fire How Demand (L/ min.)	Fire Hydrant(s) within 75m	Fire Hydrant (s) within 150m	Combined Fire How (L/ min.)
1209 St. Laurent Boulevard	7,000 (FUS)	3	3	28,500

5.0 SANITARY DESIGN

5.1 Existing Sanitary Sewer

There is an existing 525mm diameter concrete sanitary sewer within St. Laurent Boulevard.

5.2 Proposed Sanitary Sewer

A new 200 mm diameter gravity sanitary is proposed be connected to the existing 525 mm diameter sanitary sewer at the existing sanitary manhole (MHSA00412) within St. Laurent Boulevard. As no permanent structures are permitted in the 8 m setback along St. Laurent Boulevard it is anticipated monitoring would occur at MHSA00412. Refer to drawing C102 for a detailed servicing layout.

The peak design flows for the proposed building were calculated using criteria from the Ottawa Sewer Guidelines and are summarized in Table 4, below. Based on the unit occupancy statistics provided by the architect, the proposed site development will generate a flow of 11.54 L/s. See Appendix 'D' of this report for more details.

Table 4: Sanitary Design Criteria

Design Parameter	Value
Ste Area	0.43 ha
Residential	280 L/ person/ day
1 Bedroom Apartment	1.4 persons/ unit
2 Bedroom Apartment	2.1 persons/unit
Bachelor Apartment	1.4 persons/ unit
Residential Peaking Factor	3.22
Extraneous How Allowance	0.33 L/ s/ ha

Table 5, below, summarizes the estimated wastewater flow from the proposed development. Refer to Appendix D for detailed calculations.

Table 5: Summary of Estimated Sanitary Flow

Design Parameter	Total How (L/s)
Total Estimated Average Dry Weather Flow	3.61
Total Estimated Peak Dry Weather Flow	11.42
Total Estimated Peak Wet Weather Flow	11.54

The proposed 200 mm diameter gravity sanitary service will be installed with a minimum full flow target velocity (cleansing velocity) of 0.6 m/s and a full flow velocity of not more than 3.0 m/s. The capacity of the service lateral is 33.74 L/s at a proposed slope of 1.00%. Due to the complexity of the downstream network, the City will need to advise of any downstream constraints. See Appendix 'D' of this report for more details.

6.0 STORM SEWER DESIGN

6.1 Existing Storm Sewers

Water runoff from the site is currently tributary to the Rideau River within the Lower Rideau subwatershed. The subject property is currently serviced by a catch basin on the east side of the site and municipal catch basins within St. Laurent Boulevard. There is an existing 600mm diameter storm sewer within St. Laurent Boulevard that is available to service the site.

6.2 Proposed Storm Sewers

A new 250 mm storm service will be extended from the existing 600 mm diameter storm sewer within St Laurent Boulevard. The sewer system will provide attenuation for the roof area and drive aisle/entrance area by an internal cistern complete with a Tempest MHF-A ICD or an approved equivalent. A cistern detail prepared by the Mechanical Engineer is available in Appendix G.

Foundation drainage is proposed to be conveyed without flow attenuation via the 250 mm diameter storm service downstream of any cistern controls.

See CCO-22-1215 - POST include in Appendix F of this report for more details. The Stormwater Management design for the subject property will be outlined in Section 7.0 of this report.

7.0 PROPOSED STORM WATER MANAGEMENT

7.1 Design Criteria and Methodology

Sormwater management for the proposed site will be maintained through the use of an internal cistern and will collect runoff from the at-grade areas within the site. The flow will be directed to the existing 600 mm diameter storm sewer within St. Laurent Boulevard.

In summary, the following design criteria have been employed in developing the stormwater management design for the site as directed by the RVCA and City:

Quality Control

 Based on the distance to the downstream storm outlet being over 2 Km, the RVCA does not require on-site water quality treatment. Refer to Appendix B for correspondence with the RVCA.

Quantity Control

- Pre-development and post-development flows shall be calculated using a time of concentration (Tc) of 10 minutes.
- Control post-development flows to the pre-development flows with a combined C value of 0.50.

Based on further coordination with City staff, the stormwater management criteria for the site are to control post-development flows to the pre-development 5-year storm event with a maximum runoff coefficient of 0.50. Refer to Appendix B.

7.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

Q = 2.78 CIA (L/s)

Where C = Runoff coefficient

= Rainfall intensity in mm/hr (City of Ottawa IDF curves)

A = Drainage area in hectares

It is recognized that the Pational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any SWM facility sized using this method is expected to function as intended.

The following coefficients were used to develop an average Cfor each area:

Roofs/Concrete/Asphalt	0.90
Gravel	0.60
Undeveloped and Grass	0.20

As per the City of Ottawa - Sewer Design Guidelines, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

As per the pre-consultation meeting with the City of Ottawa the time of concentration (Tc) used for pre-development shall be calculated using a Tc of 10 minutes and post-development flows shall be calculated using a Tc of 10 minutes.

7.3 Pre-Development Drainage

It has been assumed that the site contains no stormwater management controls for flow attenuation. The estimated pre-development peak flows for the 5, and 100-year events are summarized below in Table 6. See CCO-22-1215 - PRE in Appendix E and Appendix G for calculations.

Table 6: Pre-Development Runoff Summary

Drainage Area		Q (L/s)		
71100	Alea (IIa)		100-Year	
A1	0.428	45.52	91.17	

7.4 Post-Development Drainage

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See CCO-22-1215 - POST in Appendix 'F' of this report for more details. Based on the quantity control criteria discussed in section 7.1, post development drainage is to be limited to a maximum release rate of 45.52 L/S. A summary of the Post-Development Runoff Calculations can be found below.

Table 7: Post-Development Runoff Summary

Drainage Area	Area (ha)	5-year Peak How (L/s)	100-year Peak How (L/s)	100-year Storage Required (m³)	100-year Storage Available (m³)
B1	0.295	9.41	18.19	103.86	103.86
B2	0.133	13.60	27.33	-	-
Total	0.428	23.01	45.52	103.86	103.86

Runoff for area B1 will be collected by roof drains (uncontrolled) and surface drains and conveyed to the internal cistern. The 103.86 m3 internal cistern is anticipated to convey stormwater to the outlet at a maximum flow rate of 13.60 L/s and 27.33 L/s for the 5 and 100-year storms, respectively. Hows in excess of the 100-year storm event will need to be directed towards St. Laurent Boulevard via a cistern overflow. A cistern detail provided by the Mechanical Engineer and complete storm calculations are included in Appendix G.

Foundation drainage is proposed to be conveyed without flow attenuation via the 250 mm storm service, downstream of cistern controls.

8.0 EROSION AND SEDIMENT CONTROL

8.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catchbasins and filter fabric is to be placed under the grates of all existing catchbasins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures is to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the Ste Grading, Drainage and Sediment & Erosion Control Plan for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

8.2 Permanent Measures

Rip-rap will be placed at all locations that have the potential for concentrated flow. It is crucial that the Contractor ensure that the geotextile is keyed in properly to ensure runoff does not undermine the rip rapped area. Additional rip rap is to be placed at erosion prone locations as identified by the Contractor / Contract Administrator / City or Conservation Authority.

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the

site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

9.0 SUMMARY

- 30-storey residential towers are proposed be constructed at 1209 St. Laurent Boulevard.
- Two 150 mm diameter water services are proposed to be connected to the existing 200 mm diameter within Lemieux Street.
- A new 200 mm diameter sanitary service is proposed to service the development via the 525 mm diameter sanitary sewer within St. Laurent Boulevard tributary to the Cyrville Road collector.
- A new 250 mm storm service for rooftop, surface, and foundation drainage is proposed to service the development. The storm service will connect to the 600mm diameter storm sewer within St. Laurent Boulevard, tributary to the Ottawa River approximately 2.4 Km downstream.
- Storage for the 5- through 100-year storm events will be provided through internal cistern attenuation.
- The RVCA does not have any specific quality control requirements for the site.

10.0 RECOMMENDATION

Based on the information presented in this report, we recommend that City of Ottawa approve this Servicing and Stormwater Management Report in support of the proposed development at 1209 St. Laurent Boulevard

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



Robert D. Freel, P.Eng. Senior Project Manager, Land Development T: 613.714.6174 E: r.freel@mcintoshperry.com Rym Pol

Ryan R. Robineau, E.I.T.

Gvil Engineering Technologist, Land Development
T: 613.714.6611
E: r.robineau@mcintoshperry.com

11.0 STATEMENT OF LIMITATIONS

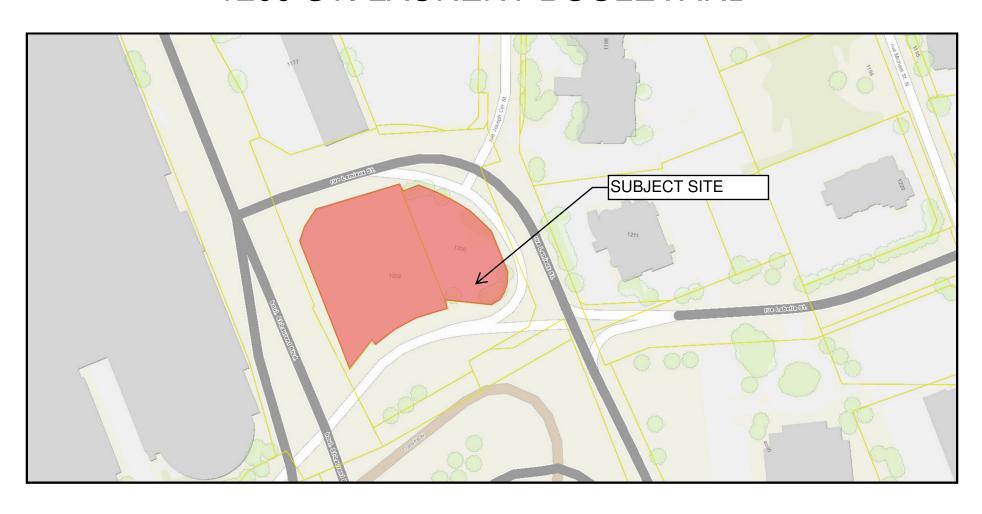
This report was produced for the exclusive use of 1209 St. Laurent Limited Partnership. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Conservation and Parks, City of Ottawa and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.

Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

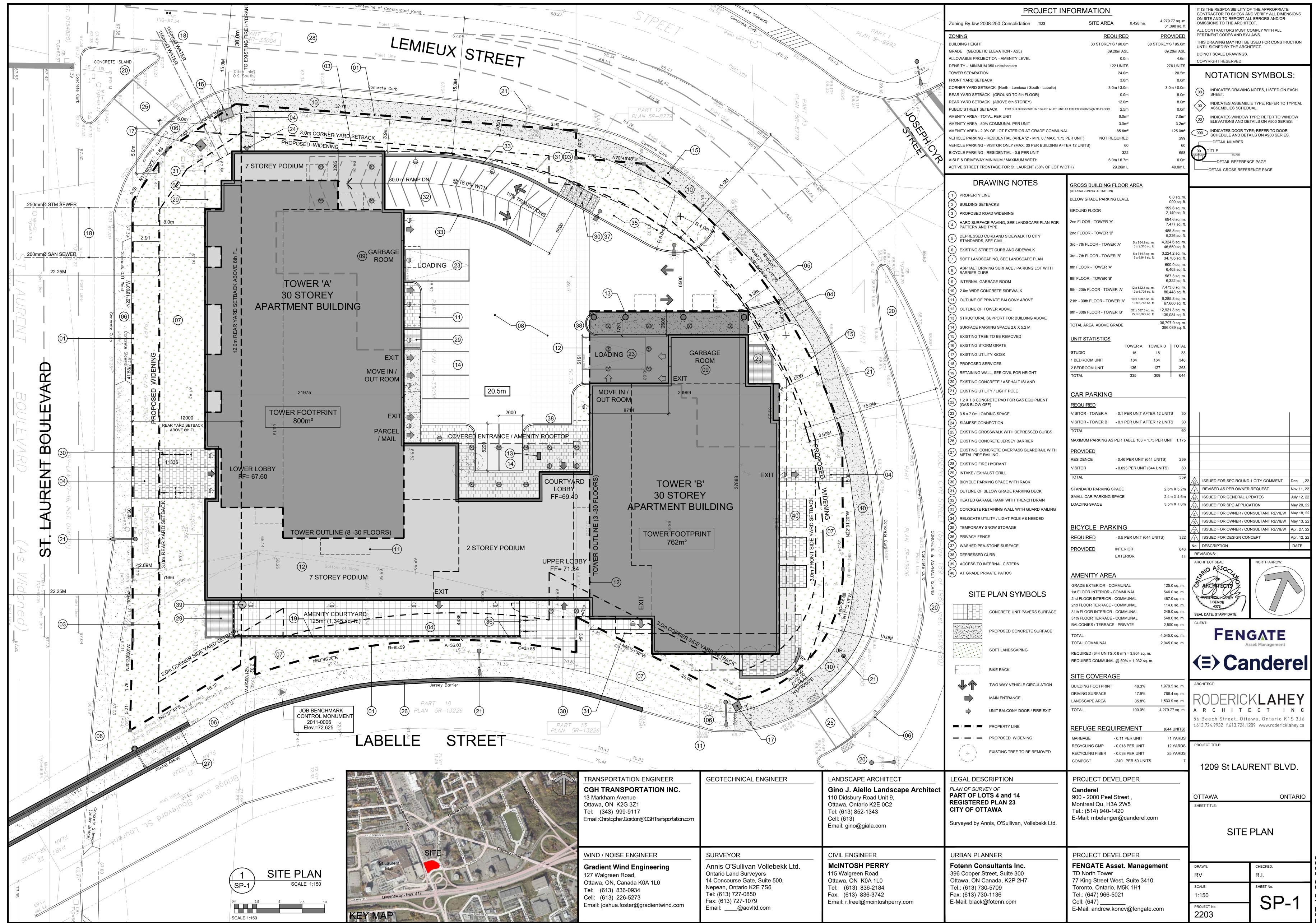
The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

APPENDIX A SITE LOCATION PLAN

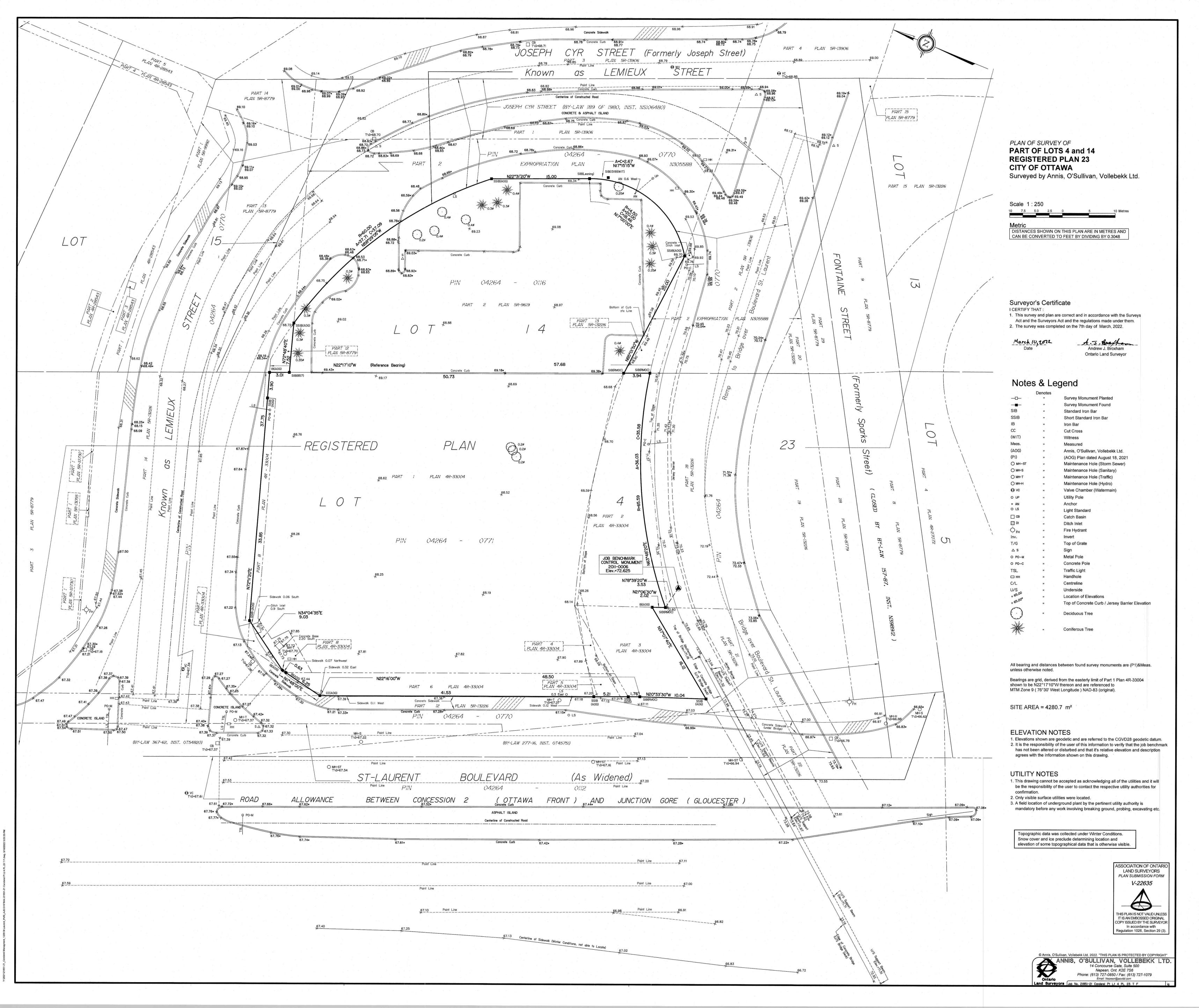
LOCATION PLAN: 1209 ST. LAURENT BOULEVARD



APPENDIX B CITY OF OTTAWA PRE-CONSULTATION NOTES



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Site Plan Pre- Application Consultation Notes

Date: August 11, 2021

Site Location: 1209 St. Laurent Blvd. & 1200 Lemieux St.

Type of Development: \boxtimes Residential (\square townhomes, \square stacked, \square singles, \boxtimes apartments), \square Office Space, \boxtimes Commercial, \square Retail, \square Institutional,

☐ Industrial, Other: N/A

Project Manager: Will Curry / Natasha Baird

Planner: Michael Boughton

Infrastructure



Water

Water District Plan No: 372-031

Existing public services:

- St-Laurent Blvd 406mm UCI
- Lemieux St (North side of site) 203mm UCI
- Lemieux St (East side of site) 152mm CI

Boundary conditions:

Civil consultant must request boundary conditions from the City's assigned Project Manager prior to first submission.

- Water boundary condition requests must include the location of the service(s) and the expected loads required by the proposed developments. Please provide all the following information:
 - Location of service(s)
 - o Type of development and the amount of fire flow required (as per FUS, 1999).
 - Average daily demand: ____ l/s.
 - Maximum daily demand: I/s.
 - Maximum hourly daily demand: ____ l/s.
- Fire protection (Fire demand, Hydrant Locations)

General comments

- At time of Site Plan Control, a water meter sizing questionnaire [water card] will have to be completed prior to receiving a water permit (water card will be provided post approval).
- Service areas with a basic demand greater than 50 m³/day shall be connected with a minimum of two water services, separated by an isolation valve, to avoid creation of vulnerable service area.

Existing public services:

- St-Laurent Blvd 525mm concrete trunk sewer, connection not permitted due to the surcharging.
- Lemieux St 250mm asbestos concrete

Is a monitoring manhole required on private property? ✓ Yes

□ No

General comments

- It was noted that the provision of sanitary services could provide a challenge for the site development. The sanitary connection should be taken from Lemieux Street. Any further questions concerning the servicing of the site should be directed to Will Curry.
- No direct sanitary service connection to the collector on St. Laurent Blvd. is permitted; an off-line MH and private sewer is permitted to connect.
- Any premise in which there is commercial or institutional food preparation shall install a grease and oil inceptor on all fixtures.
- The Environmental Site Assessment (ESA) may provide recommendations where site contamination may be present. The recommendations from the ESA need to be coordinated with the servicing report to ensure compliance with the Sewer Use By-Law.

Storm Sewer and Stormwater Management

Existing public services:

- St-Laurent Blvd 400mm and 600mm trunk sewers both concrete
- Lemieux St 300mm Concrete

General comments

- Design Criteria The following site design criteria are provide for your consideration and action.
 - o Coordination with the RVCA and Hydro Ottawa will be required.
 - o Pre to post Post C of .5
 - o Pre tc 20; post tc 10
 - Onsite design for 2-year or 5-year pipe and store up to 100-year on site.
 - No 2 year-ponding.
 - o Permissible ponding of 350mm for 100-year event.
 - At 100-year ponding elevation, you must spill to City ROW.
 - o Spill elevation must be 300mm lower than any building opening (includes ramps).
 - Require MHs just inside the property line. The site is required to accommodate the road drainage and the existing flow out-letting from the culverts towards the river.

Quality Control:

Rideau Valley Conservation Authority to confirm quality control requirements.

Ministry of Environment, Conservation and Parks (MECP)

This site will require an ECA if sewers are servicing more than one parcel through transfer of review.

NOTE: Site Plan Approval, or Draft Approval, is required before any ECA application is sent.

General Service Design Comments

- The City of Ottawa requests that all new services be located within the existing service trench to minimize necessary road cuts.
- Monitoring manholes should be located within the property near the property line in an accessible location to City forces and free from obstruction (i.e. not a parking).

- Where service length is greater than 30 m between the building and the first maintenance hole / connection, a cleanout is required.
- Manholes are required for connections to sanitary or combined trunk sewers as per City of Ottawa Standards S13.
- The City of Ottawa Standard Detail Drawings should be referenced where possible for all work within the Public Right-of-Way.
- The upstream and downstream manhole top of grate and invert elevations are required for all new sewer connections.
- Services crossing the existing watermain or sewers need to clearly provide the obvert/invert elevations to demonstration minimum separation distances. A watermain crossing table may be provided.

Geotechnical and Slope Stability Analysis

- This development requires a geotechnical analysis and a slope stability analysis prior to the OPA
 to determine the developable lands. The site is adjacent to a waterway and is on sensitive clays
 with a slope. The City will require geotechnical information to ensure that the height and type of
 building supported by the OPA and ZA is satisfied.
- Provide an updated geotechnical report and slope stability analysis certified by a qualified engineer.
- Development shall comply to the current City of Ottawa Geotechnical Guideline: https://documents.ottawa.ca/sites/documents/files/documents/cap137602.pdf
- Development shall comply to the current City of Ottawa Slope Stability Guidelines for Development Applications:

https://documents.ottawa.ca/sites/documents/files/documents/cap137604.pdf

Environmental Site Assessment

- As per the Official Plan, the environmental site assessment shall be completed as per Environmental Protection Act - O. Reg. 153/04, Part VII & VIII.
- Any reports older than 2 years shall be updated.

Other

At time of site plan control application, it will be required to verify if:

- Capital Works Projects will be within proximity to application.
- Watermain Frontage Fees are applicable.
- Minimum Drawing and File Requirements for all Plans The following plans and drawing requirements apply.
- Plans are to be submitted on standard A1 size (594mm x 841mm) sheets, utilizing an appropriate metric scale (1:200, 1:250, 1:300, 1:400, or 1:500).
- With all submitted hard copies, provide individual PDF of the DWGs and for reports please provide one PDF file of the reports. All PDF documents are to be unlocked and flattened.
- No reports submitted are to be older than 5 years.
- Existing drawings are available from informationcentre@ottawa.ca.

References and Resources

- As per section 53 of the Professional Engineers Act, O. Reg 941/40, R.S.O. 1990, all documents prepared by engineers must be signed and dated on the seal.
- All required plans are to be submitted on standard A1 size sheets (594mm x 841mm) sheets, utilizing a reasonable and appropriate metric scale as per City of Ottawa Servicing and Grading Plan Requirements: title blocks are to be placed on the right of the sheets and not along the bottom. Engineering plans may be combined, but the Site Plans must be provided separately. Plans shall include the survey monument used to confirm datum. Information shall be provided to enable a non-surveyor to locate the survey monument presented by the consultant.

- All required plans & reports are to be provided in *.pdf format (at application submission and for any, and all, re-submissions)
- Please find relevant City of Ottawa Links to Preparing Studies and Plans below:

 https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-submission/guide-preparing-studies-and-plans#standards-policies-and-guidelines
- To request City of Ottawa plan(s) or report information please contact the City of Ottawa Information Centre:
 InformationCentre@ottawa.ca<mailto:InformationCentre@ottawa.ca>
- geoOttawa <u>http://maps.ottawa.ca/geoOttawa/</u>

(613) 580-2424 ext. 44455

Ryan Robineau

From: Jamie Batchelor < jamie.batchelor@rvca.ca>

Sent: May 20, 2022 9:07 AM

To: Ryan Robineau

Subject: RE: 1200 & 1209 St Laurent Boulevard Quality Control Requirement

Good Morning Ryan,

Based on the distance to the downstream outlet being over 2km, the RVCA will not require additional on-site water quality treatment. We do recommend that you explore opportunities to incorporate LID measures into the stormwater management strategy for the site.

Jamie Batchelor, MCIP, RPP Planner, ext. 1191 Jamie.batchelor@rvca.ca



3889 Rideau Valley Drive PO Box 599, Manotick ON K4M 1A5 **T** 613-692-3571 | 1-800-267-3504 **F** 613-692-0831 | www.rvca.ca

This message may contain information that is privileged or confidential and is intended to be for the use of the individual(s) or entity n may contain confidential or personal information which may be subject to the provisions of the Municipal *Freedom of Information & I* you are not the intended recipient of this e-mail, any use, review, revision, retransmission, distribution, dissemination, copying, printing taking of any action in reliance upon this e-mail, is strictly prohibited. If you have received this e-mail in error, please contact the send and any copy of the e-mail and any printout thereof, immediately. Your cooperation is appreciated.

From: Ryan Robineau <r.robineau@mcintoshperry.com>

Sent: Tuesday, May 10, 2022 3:29 PM

To: Jamie Batchelor < jamie.batchelor@rvca.ca>

Subject: 1200 & 1209 St Laurent Boulevard Quality Control Requirement

Good afternoon Jamie,

We wanted to touch base with you regarding a proposed development at 1200 & 1209 St Laurent Boulevard.

The development involves the construction of a 30-storey apartment building with above and underground parking. Drainage will be collected and conveyed to the 600mm dia storm sewer within St Laurent Boulevard. As shown by the attached figure, water travels approximately 2.14km to the Rideau River (Outlet ID #04334). It is anticipated that drainage will be collected by roof drains, catch basins and surface drains which will be connected to the internal mechanical system.

We would like to know what SWM requirements the RVCA would have for the site.

Please let me know if you have any questions.

APPENDIX C WATERMAIN CALCULATIONS



000-22-1215 - 1209 St. Laurent Blvd - Total - Water Demands

Project: 1209 St. Laurent Blvd - Total

Project No.: COO-22-1215

Designed By: AJG
Checked By: RDF

Date: December 12, 2022

Ste Area: 0.43 gross ha

Residential NUMBER OF UNITS UNIT PATE

Single Family 3.4 homes persons/unit Semi-detached homes 2.7 persons/unit Townhouse 2.7 homes persons/unit Bachelor Apartment 33 units 1.4 persons/unit 1 Bedroom Apartment 348 units 1.4 persons/unit 2 Bedroom Apartment 263 units 2.1 persons/unit 3 Bedroom Apartment 0 units 3.1 persons/unit Average Apartment persons/unit units 1.8

Total Population 1086 persons

 Commercial
 m2

 Industrial - Light
 m2

 Industrial - Heavy
 m2

AVERAGE DAILY DEM AND

DEM AND TYPE	AMOUNT	UNITS	
Pesidential	280	L/c/d	
Industrial - Light	35,000	L/ gross ha/d	
Industrial - Heavy	55,000	L/ gross ha/d	
Shopping Centres	2,500	L/ (1000m² /d	
Hospital	900	L/ (bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/ (Space/d)	
Motels	150	L/(bed-space/d)	
Hotels	225	L/(bed-space/d)	
Tourist Commercial	28,000	L/ gross ha/ d	
Other Commercial	28,000	L/ gross ha/ d	
	Residential	3.52	L/s
AVERAGE DAILY DEMAND	Commerical/Industrial/I		
	nstitutional	0.00	L/s

MAXIMUM DAILY DEMAND

DEM AND TYPE	A	MOUNT	UNITS
Pesidential	2.5	x avg. day	L/c/d
Industrial	1.5	x avg. day	L/ gross ha/ d
Commercial	1.5	x avg. day	L/ gross ha/ d
Institutional	1.5	x avg. day	L/ gross ha/ d
	Residential	8.80	L/s
MAXIMUM DAILY DEMAND	Commerical/Industrial/I		
	nstitutional	0.00	L/s

MAXIMUM HOUR DEMAND

DEM AND TYPE	А	MOUNT	UNITS
Residential	5.5	x avg. day	L/c/d
Industrial	1.8	x max. day	L/gross ha/d
Commercial	1.8	x max. day	L/gross ha/d
Institutional	1.8	x max. day	L/ gross ha/ d
	Residential	19.36	L/s
MAXIMUM HOUR DEMAND Commerical/Industrial/I			
	nstitutional	0.00	L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	3.52	L∕s
MAXIMUM DAILY DEMAND	8.80	L/s
MAXIMUM HOUR DEMAND	19.36	L/s

000-22-1215 - 1209 St. Laurent Blvd - Fire Underwriters Survey

Project: 1209 St. Laurent Blvd

Project No.: 000-22-1215

Designed By: AJG
Checked By: RDF

Date: December 12, 2022

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:

С

Updated per City of Ottawa Technical Bulletin ISTB-2018-02

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

 $F = 220 \times C \times VA$ Where:

F = Required fire flow in liters per minute

C = Coefficient related to the type of construction.

A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type Non-Combustible Construction

0.8

Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 5,214.0 m²

* Unprotected Vertical Openings

Calculated Fire Flow

12,708.6 L/min 13,000.0 L/min

A 35,712.8 m²

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From note 2, Page 18 of the Fire Underwriter Survey:

Limited Combustible -15%

Fire How 11,050.0 L/min

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Fully Supervised Sprinklered -50%

Reduction -5,525.0 L/min

D. INCREASE FOR EXPOSURE (No Rounding)

			Length-			
	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Height Factor	
Exposure 1	30.1 to 45	Ordinary (Unprotected)	20	1	20.0	5%
Exposure 2	>45	Wood frame	33.5	2	67.0	0%
Exposure 3	>45	Wood frame	27.1	14	379.4	0%
Exposure 4	30.1 to 45	Non-Combustible	232	2	464.0	5%

%Increase* 10%

Increase* 1,105.0 L/min

E Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire How 6,630.0 L/ min
Fire How Required** 7,000.0 L/ min

* In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75% ** In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min			

000-22-1215 - 1209 St. Laurent Blvd - Boundary Condition Unit Conversion

Project: 1209 St. Laurent Blvd

Project No.: 000-22-1215

Designed By: AJG
Checked By: RDF

Date: December 12, 2022

Boundary Conditions Unit Conversion

LEMIEUX STREET

Scenario	Height (m)	Elevation (m)	m H ₂ O	PSI	kPa
Avg. DD	118.5	67.3	51.2	72.8	501.9
Fire Flow Of 116.67 L/s (7,000 L/min)	110.4	67.3	43.1	61.3	422.4
Peak Hour	110.2	67.3	42.9	61.0	420.5

From: Rasool, Rubina < Rubina. Rasool@ottawa.ca>

Sent: May 11, 2022 10:36 AM

To: Alison Gosling <a.gosling@mcintoshperry.com>

Subject: RE: 22-1215 - 1209 St. Laurent Blvd - Boundary Condition Request

Hi Alison,

Great timing, I just received the WBC.

I believe Natasha Baird was the PM for the pre-consult. I have forwarded your inquiry to her for response.

The following are boundary conditions, HGL, for hydraulic analysis at 1209 St-Laurent Boulevard (zone 1E) assumed to be a dual connection to the 203 mm on Lemieux Street (see attached PDF for location).

Minimum HGL: 110.2 m Maximum HGL: 118.5 m

Max Day + Fire Flow (116.67 L/s): 110.4 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual

1

field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Rubina

Rubina Rasool, E.I.T.

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - East Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

From: Rasool, Rubina < Rubina. Rasool@ottawa.ca>

Sent: May 9, 2022 12:13 PM

To: Alison Gosling <a.gosling@mcintoshperry.com>

Subject: RE: 22-1215 - 1209 St. Laurent Blvd - Boundary Condition Request

Hi Alison,

I have received your request for water boundary conditions. Please allow for approximately 3 weeks due to staff shortages. Note that development applications can still be submitted pending water boundary condition requests to avoid delays.

Best,

Rubina

Rubina Rasool, E.I.T.

Project Manager

Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique

Development Review - East Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

APPENDIX D SANITARY CALCULATIONS

000-22-1215 - 1209 St. Laurent Blvd - Total - Sanitary Demands

Project: 1209 St. Laurent Blvd - Total

 Project No.:
 COO-22-1215

 Designed By:
 AJG

 Checked By:
 RDF

 Date:
 Dec-22

Ste Area 0.43 Gross ha

 Bachelor
 33
 1.40
 Persons per unit

 1 Bedroom
 348
 1.40
 Persons per unit

 2 Bedroom
 263
 2.10
 Persons per unit

 Total Population
 1086 Persons

 Commercial Area
 m²

 Amenity Space
 2045.00 m²

DESIGN PARAMETERS

Institutional/Commercial Peaking Facto

Residential Peaking Factor 3.22 * Using Harmon Formula = $1+(14/(4+P^{\lambda}0.5))*0.8$

where P = population in thousands, Harmon's Correction Factor = 0.8

Mannings coefficient (n)0.013Demand (per capita)280L/dayInfiltration allowance0.33L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	How (L/s)
Dry	0.02
Wet	0.12
Total	0.14

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	How (L/s)
Residential	280	L/c/d	1086	3.52
Industrial - Light**	35,000	L/ gross ha/ d		0
Industrial - Heavy* *	55,000	L/ gross ha/ d		0
Commercial	2,800	L/(1000m ² /d)	2045.00	0.07
Hospital	900	L/ (bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/ gross ha/ d		0
Other Commercial	28,000	L/ gross ha/ d		0

AVERAGE RESIDENTIAL FLOW PEAK RESIDENTIAL FLOW	3.52 11.34	Us Us
FLAN ALGIDENTIAL I EUW	11.54	U 5
AVERAGE ICI FLOW	0.07	L/s
PEAK INSTITUTIONAL/ COMMERCIAL FLOW	0.07	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.07	L/s

TOTAL SANITARY DEMAND

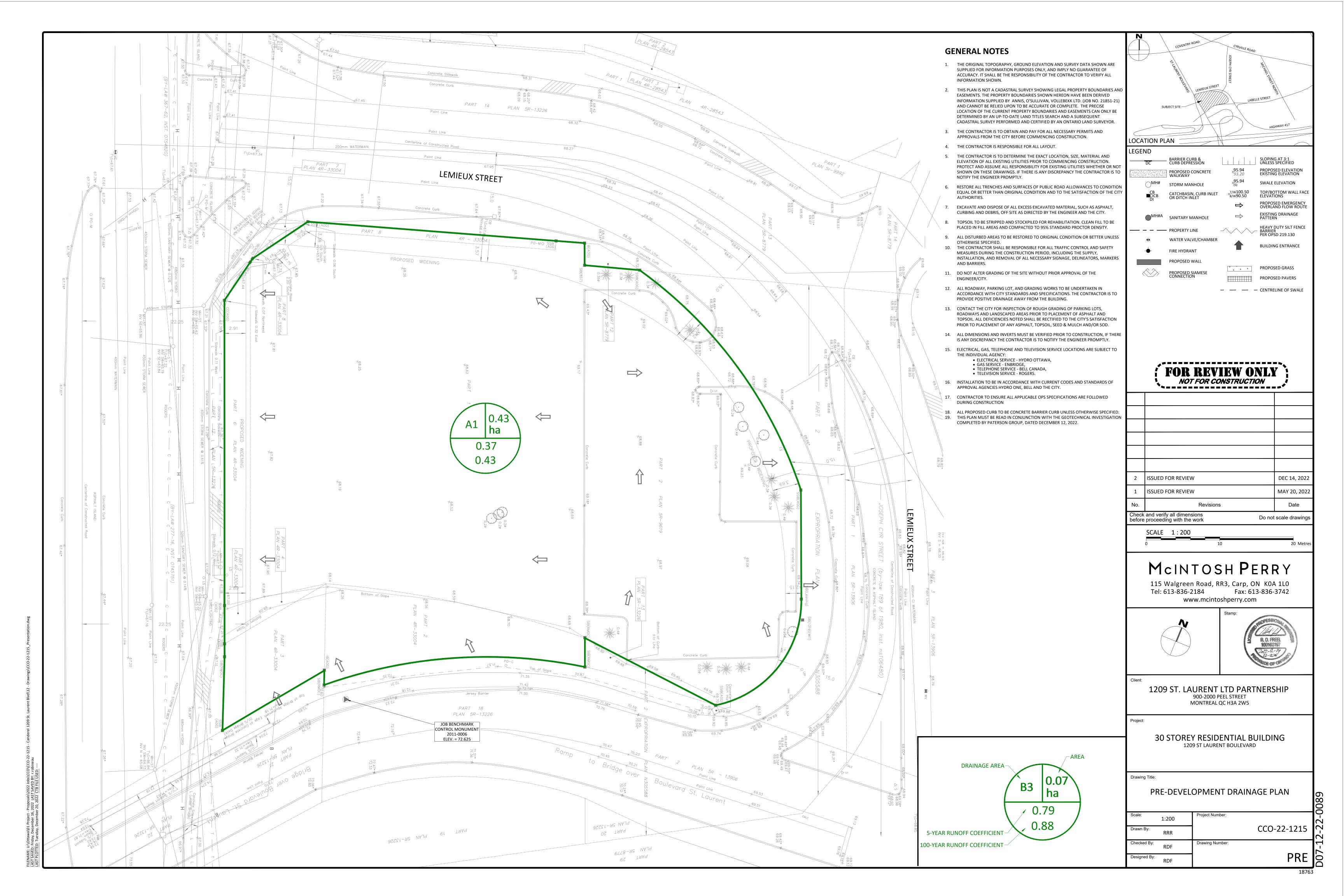
TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	3.61	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	11.42	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	11.54	L/s

SANITARY SEWER DESIGN SHEET

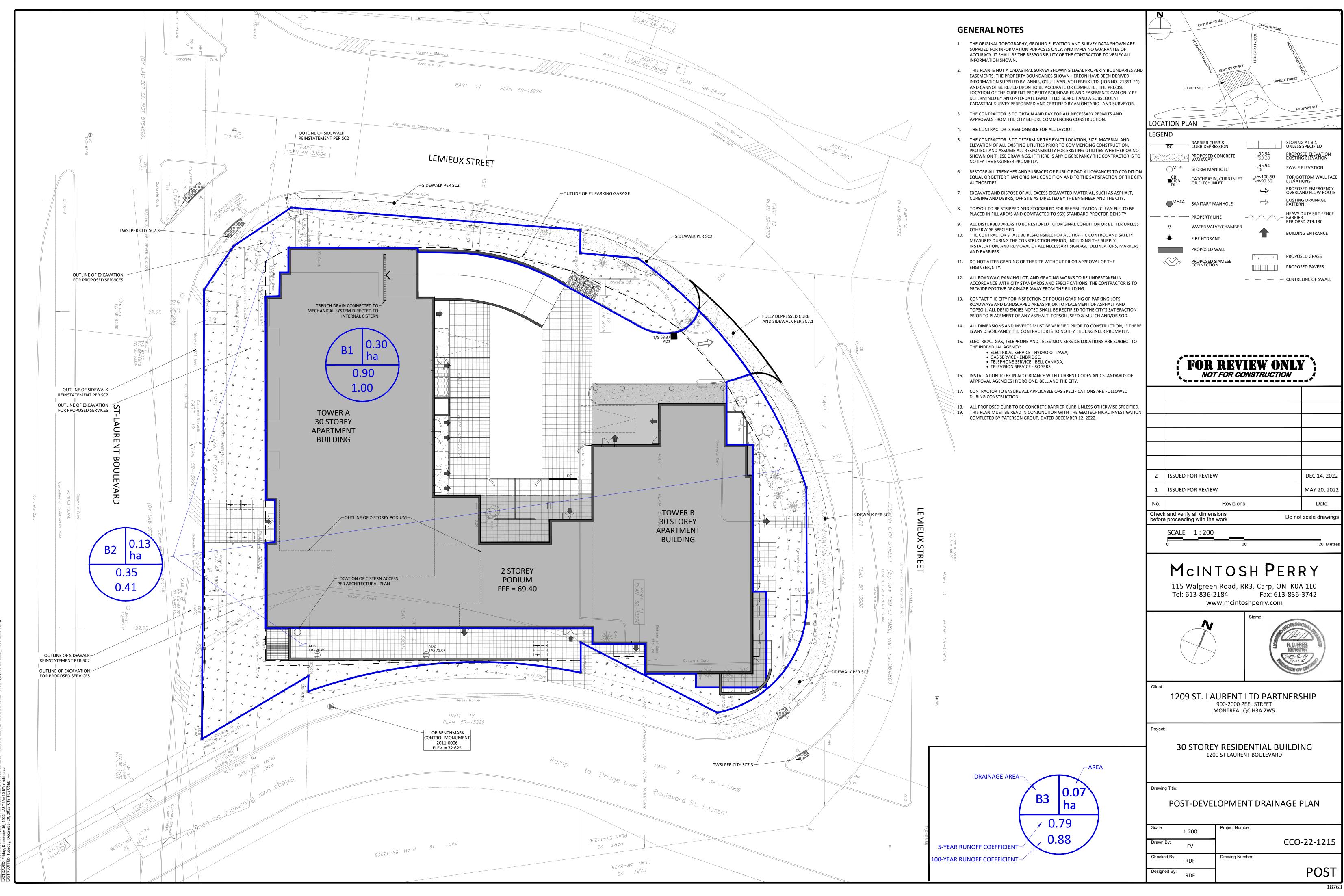
PROJECT: 30 Storey Residential Building
LOCATION: 1209 St Laurent Blvd
CLIENT: Candrel Management Inc.

	LOCATION	ı					F	RESIDENTIAL	_				1			ICI AREAS				INFILTE	RATION ALLO	OWANŒ	FLOW				SEWER DAT	A		
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31
					UNIT	TYPES		AREA	POPU	LATION		PEAK			ARE	A (ha)			PEAK	ARE	A (ha)	FLOW	DESIGN	CAPACITY	LENGTH	DIA	SLOPE	VELOCITY		LABLE
STREET	AREA ID	FROM	TO	BAC	1-BED	2-BED	APT	(ha)	IND	CUM	PEAK	FLOW		JTIONAL		ENITY		JSTRIAL	FLOW	IND	СЛМ	(L/s)	FLOW	(L/s)	(m)	(mm)	(%)	(full)	CAPA	ACITY
		MH	MH	DAC	1-60	2-000	AFI	(IIa)	טאוו	Wivi	FACTOR	(L/s)	IND	CUM	IND	CUM	IND	CUM	(L/s)	IND	COIVI	(L/S)	(L/s)	(L'S)	(111)	(111111)	(70)	(m/s)	L/s	(%)
St Laurent Blvd		BLDG	EXMH	33	348	263		0.43	1086	1086	3.22	11.33		0.00	0.20	0.20		0.00	0.07	0.43	0.43	0.14	11.54	34.22	14.24	200	1.00	1.055	22.68	66.28
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Design Parameters:				Notes:	1		L	l I			Designed:		RRR		Į.	No.					Revision	<u> </u>		<u>. </u>				Date		
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Residential		IO Areas			d (per capita)			L/day								 														
BAC 1.4 p/p/u		101/11/03	Peak Factor		ion allowano			L/s/Ha			Checked:		RDF			+ +														
1-BED 1.4 p/p/u	INST 28	,000 L/Ha/day	1.0		tial Peaking I		0.00	Lana			S lookou.		. 51			 														
APT 2.1 p/p/u		,000 L/Ha/day	1.0				14/(4+P^0.5)	* 0.8)																						
Other 60 p/p/Ha		,000 L/Ha/day	MOE Chart		where P=p			5.5)			Project No.		000-22-12	15		1														
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APPENDIX E PRE-DEVELOPMENT DRAINAGE PLAN



APPENDIX F POST-DEVELOPMENT DRAINAGE PLAN



APPENDIX G STORMWATER MANAGEMENT CALCULATIONS

CCO-22-1215 - 1209 St Laurent Boulevard - Runoff Calculations

1 of 2

Pre-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m²)	С	Gravel Area (m²)	С	Pervious Area (m²)	С	C _{AVG} 2/5-Year	C _{AVG} 100-Year
A1	0.428	1,022.61	0.90	0.00	0.60	3,256.29	0.20	0.37	0.43

Pre-Development Runoff Calculations

Drainage Area	Area (ha)	C 2/5-Year	C 100-Year	Tc (min)		l (mm/hr)			Q (L/s)	
Alea	(Ha)	2/ 5- Teal	100-1eai	(111111)	2-Year	5-Year	100-Year	2-Year	5-Year	100-Year
A1	0.428	0.37	0.43	10	76.8	104.2	178.6	33.56	45.52	91.17
Total	0.428							33.56	45.52	91.17

Post-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m²)	С	Gravel Area (m²)	С	Pervious Area (m²)	С	C _{AVG} 2/5-Year	C _{AVG} 100-Year
B1	0.295	2,939.07	0.90	0.00	0.60	11.10	0.20	0.90	1.00
B2	0.133	291.21	0.90	0.00	0.60	1,037.52	0.20	0.35	0.41

Controlled Uncontrolled

Post-Development Runoff Calculations

Drainage Area	Area (ha)	C 2/5-Year	C 100-Year	Tc (min)	(mm/hr)				Q (L/s)	
Area	(Ha)	2/ 5- Teal	100- teal	(111111)	2-Year	5-Year	100-Year	2-Year	5-Year	100-Year
B1	0.295	0.90	1.00	10	76.8	104.2	178.6	56.5	76.68	146.03
B2	0.133	0.35	0.41	10	76.8	104.2	178.6	10.0	13.60	27.33
Total	0.428							66.55	90.29	173.36

Required Restricted Flow

Drainage Area	Area (ha)	С	Tc (min)	l (mm/ hr)	Q (L/ s)
Alea	(Ha)		(111111)	5-Year	5-Year
A1	0.428	0.37	10	104.2	45.52
Total	0.428				45.52

Post-Development Restricted Runoff Calculations

Drainage Area	Unre	estricted Fic (L/s))W	R	estricted Flo (L/s)	ow	Sto	orage Requi (m³)	red	Storage Provided (m³)			
Alea	2-Year	5-Year	100-Year	2-Year	5-Year	5-Year 100-Year		5-Year	100-Year	2-Year	5-Year	100-Year	
B1	56.53	76.68	146.03	6.93	9.41	9.41 18.19		55.46	103.86	41.43	55.46	103.86	
B2	10.03	13.60	27.33	10.03	13.60	27.33							
Total	66.55	90.29	173.36	16.96	23.01	45.52	41.43	55.46	103.86	41.43	55.46	103.86	

CCO-22-1215 - 1209 St Laurent Boulevard - Runoff Calculations

Storage Requirements for Area B1

2 of 2

2-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
40	32.9	24.19	6.93	17.26	41.42
41	32.3	23.77	6.93	16.84	41.43
42	31.8	23.37	6.93	16.44	41.43
43	31.2	22.99	6.93	16.06	41.42
44	30.7	22.61	6.93	15.68	41.41

Maximum Storage Required 2-year =

41.4 m³

5-Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m³)
38	45.8	33.72	9.41	24.31	55.42
39	45.0	33.10	9.41	23.69	55.45
40	44.2	32.52	9.41	23.11	55.46
41	43.4	31.95	9.41	22.54	55.46
42	42.7	31.41	9.41	22.00	55.45

Maximum Storage Required 5-year =

5.5 m³

100Year Storm Event

Tc (min)	l (mm/hr)	Runoff (L/s) B1	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
35	82.6	67.54	18.19	49.35	103.63
36	81.0	66.21	18.19	48.02	103.73
37	79.4	64.95	18.19	46.76	103.81
38	77.9	63.74	18.19	45.55	103.85
39	76.5	62.57	18.19	44.38	103.86
40	75.1	61.46	18.19	43.27	103.84
41	73.8	60.38	18.19	42.19	103.79
42	72.6	59.35	18.19	41.16	103.72

Maximum Storage Required 100-year =

103.9 m³

2-Year Storm Event Storage Summary

Storage Available (m³) = 41.4 Storage Required (m³) = 41.4

5-Year Storm Event Storage Summary

Storage Available (m³) = 55.5 Storage Required (m³) = 55.5

100-Year Storm Event Storage Summary

Storage Available (m³) = 103.9 Storage Required (m³) = 103.9

CCO-22-1215 - 1209 St Laurent Boulevard - Runoff Calculations

4 of 3

Time of Concentration Pre-Development

Drainage Area	Sheet Flow	Sope of	Tc (min)	Tc (min)
ID	Distance (m)	Land (%)	(5-Year)	(100-Year)
A1	33	2.80	10	9

* Therefore, a Tc of 10 can be used

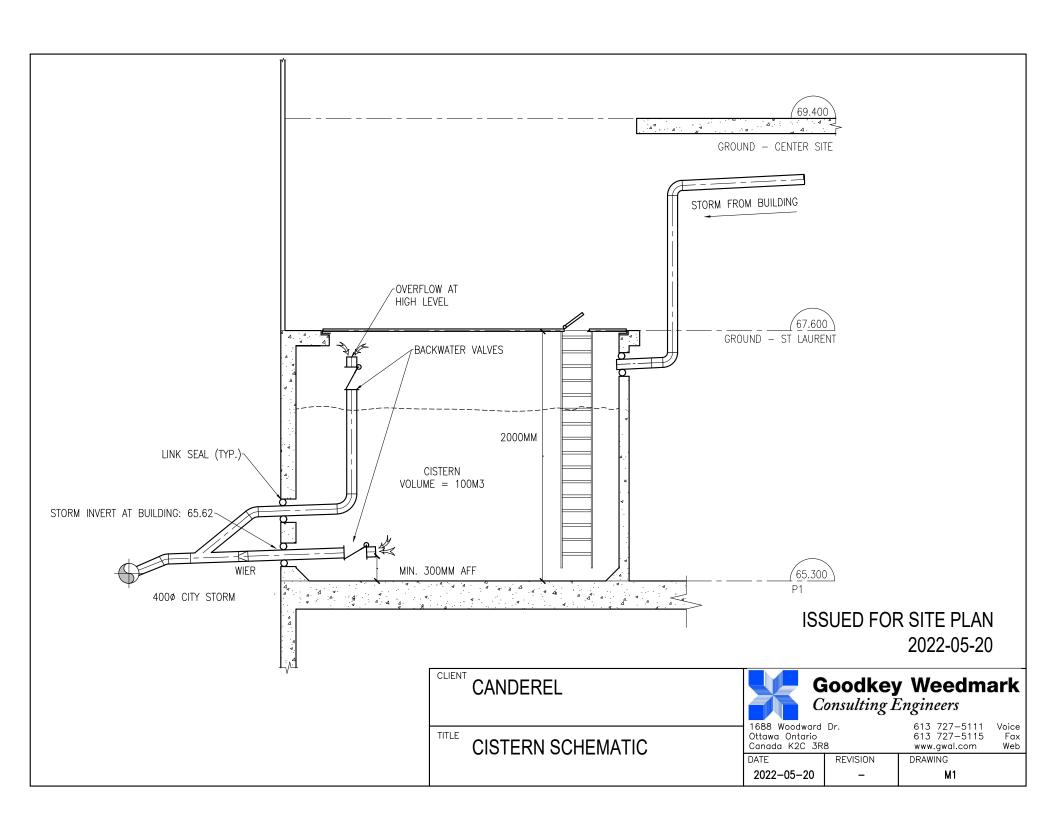
 $Tc = (3.26(1.1-c)L^0.5/S^0.33)$

c= Balanced Runoff Coefficient
 L= Length of Drainage Area
 S= Average Sope of Watershed

STORM SEWER DESIGN SHEET

PROJECT: 30-Storey Apartment
LOCATION: 1209 St.Laurent
CLIENT: Canderel Management Inc.

	LOCATION				CONTRIBUTING AREA (ha)			RATION					ONAL DESIGN	FLOW									SEWER DATA				
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
STREET	AREA ID	FROM	TO	C-VALUE	AREA	INDIV	CUMUL	INLET	TIME	TOTAL	i (5)	i (10)	i (100)	5yr PEAK	10yr PEAK	100yr PEAK	FIXED	DESIGN	CAPACITY	LENGTH		PIPE SIZE (mn	n)	SLOPE	VELOCITY	AVAILC	AP (5yr)
SINEE	ANEATO	MH	MH	GVALUE	ANCA	AC	AC	(min)	IN PIPE	(min)	(mm/hr)	(mm/hr)	(mm/hr)	FLOW (L/s)	(L/s)	(m)	DIA	W	Н	(%)	(m/s)	(L/s)	(%)				
St. Laurent Boulevard	B1	BLDG	EX.MH	0.90	0.30	0.26	0.26	10.00	0.13	10.13	104.19	122.14	178.56	76.68				47.81	62.04	9.81	250			1.00	1.224	14.23	22.94%
C. Edd on Boulovard	2.	320	De	0.00	0.00	0.20	0.20	10.00	0.10	10.10			170.00	7 0.00					02.01	0.01	200					1 1120	22.0170
																								1			
																								1			
Definitions:				Notes:		•		Designed:					No.					Revision							Date		
Q = 2.78QA, where:				1. Mannings coefficient (n) =		0.013		RRR				1.				ls	sued For Revie	W						2022-05-20		
Q = Peak Flow in Litres p	er Second (L/s)																										
A = Area in Hectares (ha	a)							Checked:																			
i = Rainfall intensity in r									RDF																		
[i = 998.071 / (TC+6.0	53)^0.814]	5 YEAR																									
[i = 1174.184 / (TC+6.	014)^0.816]	10 YEAR						Project No.:			•																
[i = 1735.688 / (TC+6.	014)^0.820]	100 YEAR							000-22-1215									ate:							Sheet No:		
																	2022	-05-20							1 of 1		



APPENDIX H CITY OF OTTAWA DESIGN CHECKLIST

City of Ottawa

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

Criteria	Location (if applicable)
Executive Summary (for larger reports only).	N/A
Date and revision number of the report.	On Cover
Location map and plan showing municipal address, boundary, and layout of proposed development.	Appendix E
Plan showing the site and location of all existing services.	Site Servicing Plan (C102)
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and	1.1 Purpose
watershed plans that provide context to which individual developments must adhere.	1.2 Site Description
	6.0 Stormwater Management
 Summary of pre-consultation meetings with City and other approval agencies. 	Appendix A
☐ Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments,	1.1 Purpose
Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and	1.2 Site Description
develop a defendable design criteria.	6.0 Stormwater Management
\square Statement of objectives and servicing criteria.	3.0 Pre-Consultation Summary



☐ Identification of existing and proposed infrastructure available in the immediate area.	N/A
☐ Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
☐ Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
☐ Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
☐ Proposed phasing of the development, if applicable.	N/A
Reference to geotechnical studies and recommendations concerning servicing.	Section 2.0 Backround Studies
 All preliminary and formal site plan submissions should have the following information: Metric scale North arrow (including construction North) Key plan Name and contact information of applicant and property owner Property limits including bearings and dimensions Existing and proposed structures and parking areas Easements, road widening and rights-of-way Adjacent street names 	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)

4.2 Development Servicing Report: Water

Criteria	Location (if applicable)
☐ Confirm consistency with Master Servicing Study, if available	N/A
Availability of public infrastructure to service proposed development	N/A
☐ Identification of system constraints	N/A
☐ Identify boundary conditions	N/A
☐ Confirmation of adequate domestic supply and pressure	N/A
 Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development. 	Appendix B
 Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves. 	N/A
 Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design 	N/A
☐ Address reliability requirements such as appropriate location of shut-off valves	N/A
☐ Check on the necessity of a pressure zone boundary modification.	N/A
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range	N/A

Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	N/A
 Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation. 	N/A
☐ Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Appendix B
 Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference. 	N/A

4.3 Development Servicing Report: Wastewater

Criteria	Location (if applicable)
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	N/A
☐ Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A
☐ Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Section 5.2 Sanitary Sewer

☐ Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	N/A
☐ Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	N/A
 Description of proposed sewer network including sewers, pumping stations, and forcemains. 	Section 5.2 Sanitary Sewer
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
 Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development. 	N/A
☐ Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
☐ Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
☐ Special considerations such as contamination, corrosive environment etc.	N/A

4.4 Development Servicing Report: Stormwater Checklist

Criteria	Location (if applicable)
 Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property) 	Section 6.0 Stormwater Management
☐ Analysis of available capacity in existing public infrastructure.	N/A
☐ A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Pre & Post-Development Plans
Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5-year event (dependent on the receiving sewer design) to 100-year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Section 6.0 Stormwater Management
☐ Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Section 6.0 Stormwater Management
 Description of the stormwater management concept with facility locations and descriptions with references and supporting information. 	Section 6.0 Stormwater Management
☐ Set-back from private sewage disposal systems.	N/A
☐ Watercourse and hazard lands setbacks.	N/A
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	N/A
☐ Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N/A
☐ Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5-year return period) and major events (1:100-year return period).	Appendix F

☐ Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	Site Grading, Drainage, Sediment & Erosion Control Plan
☐ Calculate pre-and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Section 6.0 Stormwater Management Appendix F
Any proposed diversion of drainage catchment areas from one outlet to another.	Section 6.0 Stormwater Management
 Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities. 	Section 6.0 Stormwater Management
☐ If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Appendix A
☐ Identification of potential impacts to receiving watercourses	N/A
Identification of municipal drains and related approval requirements.	N/A
Descriptions of how the conveyance and storage capacity will be achieved for the development.	Section 6.0 Stormwater Management
100-year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Grading, Drainage, Sediment & Erosion Control Plan (C101)
☐ Inclusion of hydraulic analysis including hydraulic grade line elevations.	N/A

 Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors. 	Section 7.0 Sediment & Erosion Control
☐ Identification of floodplains — proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
☐ Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

Criteria	Location (if applicable)
☐ Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/A
☐ Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A
☐ Changes to Municipal Drains.	N/A
 Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.) 	N/A

4.6 Conclusion Checklist

Criteria	Location (if applicable)
Clearly stated conclusions and recommendations	Section 8.0 Summary
	Section 9.0 Recommendations
☐ Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	All are stamped
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario	All are stamped