



140 Rue Lusk Street

Stormwater Management Report

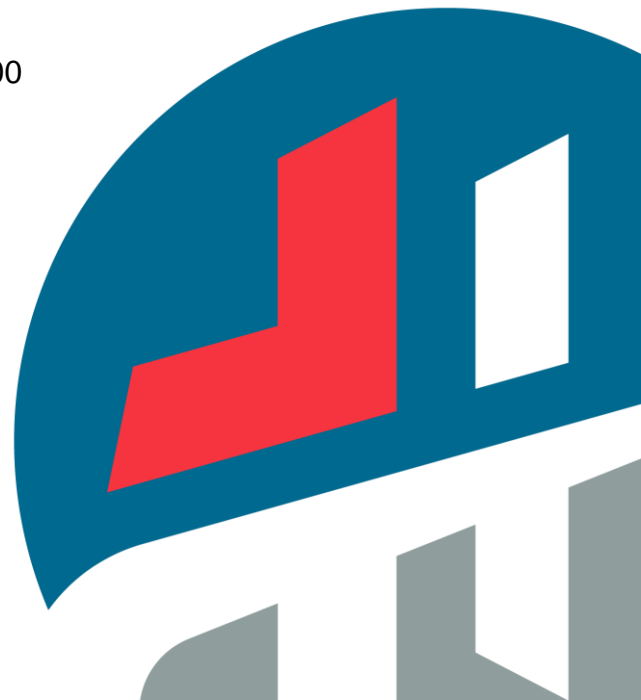
Project Location:
140 Rue Lusk Street
Ottawa, ON

Prepared for:
Troms Holdings Corp
601 Pamplona Private
Ottawa, ON K2J 5T3

Prepared by:
MTE Consultants
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December 2, 2022

MTE File No.: 52222-100





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Existing Conditions Plan	
MTE Drawing No. C1.1	Separately Appended
Site Grading, E&SC, and SWM Plan	
MTE Drawing No. C2.1	Separately Appended
Site Servicing Plan	
MTE Drawing No. C2.2	Separately Appended
Notes and Details Plan	
MTE Drawing No. C2.3	Separately Appended

1.0 Introduction

MTE Consultants Inc. was retained by Troms Holdings Corp to complete the stormwater management design for the Holiday Inn Ottawa to be constructed at 140 Lusk Street in the City of Ottawa (herein referred to as the 'Site').

The Site is 0.52ha in size and referred to as Block 14, located within the O'Keefe Court – 416 Lands, the commercial development at the northeast quadrant of the intersection of Fallowfield Road and Strandherd Drive. The Site is bounded to the north by O'Keefe Court, to the west and to the east by future commercial development, to the south by a trail easement leading to Lusk Street and the turning circle of Lusk Street. For exact location of the site refer to the key plan located on the separately appended engineering drawings.

The proposed development for the site is the construction of a four-storey hotel building with associated driveway and at grade parking areas. The overall imperviousness of the proposed development is approximately 85.3% (C =0.80).

This report addresses the stormwater management (SWM) requirements set forth by the Design Brief for O'Keefe Court Business Park – 416 Lands Commercial Development, prepared by IBI Group, dated May 2018. The design requirements for the Site conforms to the criteria set forth by the City of Ottawa and the Jock River Reach One Subwatershed Study. The site grading, servicing and stormwater management details for the site are illustrated on the separately appended MTE Engineering Drawings C1.1, C2.1, C2.2, and C2.3.

2.0 Criteria

The stormwater management design criteria for the subject Site, as established by the Design Brief for the O'Keefe Court – 416 Lands Commercial Development, prepared by IBI Group, dated May 2018, and the City of Ottawa are as follows:

- i) Attenuation of the post-development peak flows up to the 100-year storm event from the proposed development to maximum allowable release rate of 74L/s;
- ii) Providing sufficient on-site storage, a minimum of 92.4m³ as required in the O'Keefe Court – 416 Lands Commercial Development Design Brief (IBI Group, 2018), to ensure no overland flow for all storms up to and including the 100-year storm event; and
- iii) Water quality control will be provided in the downstream stormwater management facility within the O'Keefe Court – 416 Lands Commercial Development.

3.0 Methodology

In order to successfully complete the stormwater management design for this site, the following specific tasks were undertaken:

- i) Calculated the allowable runoff rates using MIDUSS NET;
- ii) Determined the percent impervious of the site and catchment parameters for inclusion in MIDUSS modelling;
- iii) Calculated post-development runoff hydrographs using MIDUSS NET; and,
- iv) Revised the site grades to attain the required storage for runoff control.

4.0 Stormwater Management

4.1 Catchment Parameters

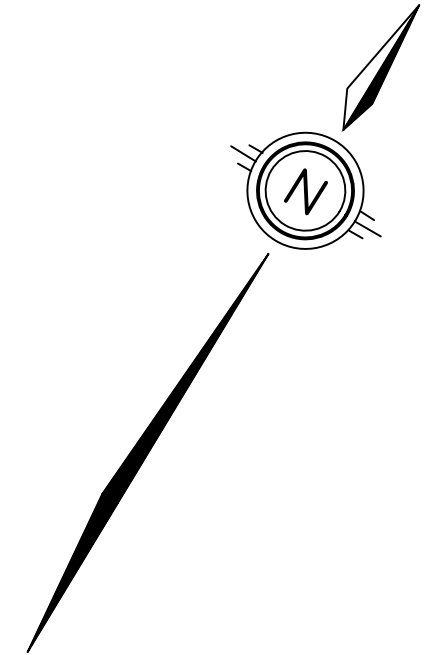
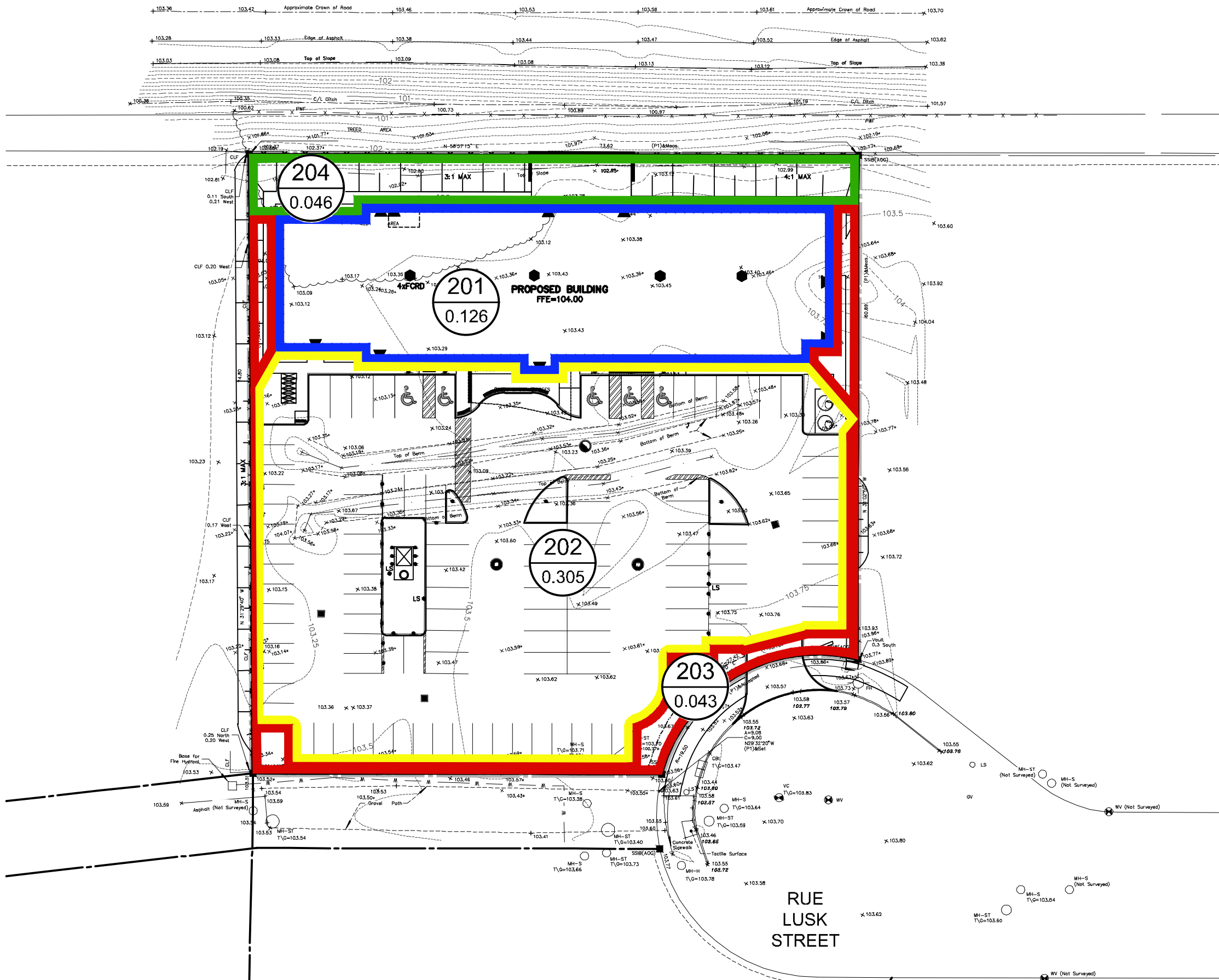
Per the Design Brief for the O’Keefe Court – 416 Lands Commercial Development, provided by IBI Group, dated May 2018, the allowable peak flow for the Site is 74L/s for all storm events up to and including the 100-year storm event. The required on-site stormwater storage is 92.4m³. Refer to Appendix A for the 416 Lands Storm Drainage Area Plan, prepared by IBI Group, dated May 18, 2022, for more detail.

The following Table 4.1 summarizes the catchments used in the modelling of the site. The post-development condition was separated into four catchment areas. The building rooftop area, the controlled driveway and parking lot area, the uncontrolled area draining towards Lusk Street, and the uncontrolled area towards O’Keefe Court. Figure 1.0 illustrates the limits of the post-development catchment areas.






Table 4.1 – Catchment Parameters


#	Catchment	Area (ha)	% Impervious	Pervious CN	Impervious CN	Slope (%)	Flow Length (m)
Post-Development Catchment Areas							
201	Building Rooftop Area (to Lusk Street)	0.126	100.0	75	98	2.0	7.5
202	Controlled Area (to Lusk Street)	0.305	96.0	75	98	1.0	15.0
203	Uncontrolled Area draining (to Lusk Street ROW)	0.043	28.6	75	98	18.0	2.0
204	Uncontrolled Area draining to O’Keefe Court Street ROW	0.046	27.0	75	98	27.0	6.0

A geotechnical investigation was not available for this development at the time this report was published. Therefore, a conservative value of 75 was used for the pervious CN.




LEGEND

-  SITE BOUNDARY
-  CATCHMENT 201
-  CATCHMENT 202
-  CATCHMENT 203
-  CATCHMENT 204

-  SUB-CATCHMENT NUMBER
AREA (ha.)

F1 Date: NOV.30/2022
Scale: 1:500

**POST-DEVELOPMENT
CATCHMENT AREAS**

 **MTE**

Engineers, Scientists, Surveyors

Project No.: 52222-100

4.2 Water Quantity – Modelling Results

It is proposed that stormwater runoff from the proposed development to be directed to the existing 450mm diameter storm stub located at the southern property line of the Site, and ultimately draining to the 975mm diameter municipal storm sewer.

In order to achieve the stormwater management requirements for the site, runoff generated from the controlled driveway and parking areas will be captured by the on-site storm sewer system and will be conveyed to catchbasin-manhole (CBMH10), wherein the flow will be controlled with the installation of a 120mm diameter on-line orifice plate on the outlet pipe. Stormwater storage volume will be provided within the driveway and parking areas, on the building rooftop storage, and within the subsurface storm sewer system. The maximum depth of ponding permitted within the parking area by grading is 0.30m.

In addition, four flow control roof drains, single notch, are proposed to be installed on the roof of the proposed building. This will help to further reduce the post-development runoff from the site.

The flow equations for the orifice and the IDF parameters for the 5 and 100-year storm events are included in Appendix B. Refer to Appendix C for the MIDUSS NET output.

A combination of the surface storage, rooftop storage, and subsurface storage provides approximately 246.0m³ of storage volume, which exceeds the required 92.4m³ by the Design Brief for O’Keefe Commercial Development (IBI Group, 2018). The following table 4.2 illustrates the stage-storage-discharge relationship of the storm system.

Table 4.2 – Stage-Storage-Discharge Information (taken From MIDUSS)

Elevation (m)	Head (m)	Orifice Flow (m ³ /s)	Volume (m ³)	Remarks
101.012	0	0	0	120mm diameter Orifice invert
103.40	2.388	0.04795	8.60	Top of grate CB3
103.45	0.05	0.04846	10.24	Top of grate CB4 and CBMH1
103.50	0.05	0.04897	15.94	Top of grate CB5. 0.10m ponding
103.55	0.05	0.04948	34.71	Top of grate CB2. 0.15m ponding
103.60	0.05	0.04998	75.43	0.20m ponding
103.65	0.05	0.05048	146.77	0.25m ponding
103.70	0.05	0.05097	246.50	0.30m ponding. Overflow elevation.

With the addition of the 120mm diameter orifice plate, the post-development runoff from the controlled portion of the site for the 5- and 100-year storm events is controlled to 0.059m³/s (or 59.0L/s) and 0.069m³/s (or 69.0L/s), respectively. The following table summarizes the flows generated by the whole site.

Table 4.3 – Summary of Flows

Modelling Condition	2-Year Storm Event (m ³ /s)	5-Year Storm Event (m ³ /s)	100-Year Storm Event (m ³ /s)
Allowable Release Rate	0.074	0.074	0.074
Post-Development	0.054	0.059	0.069

The maximum ponding elevation for the 5-year storm event is 103.509m. This represents 10.9cm of ponding depth within the parking area. For the 100-year storm event, the maximum ponding elevation is 103.579m. This represents 17.9cm of ponding depth within the parking area.

4.3 Water Quality Control

The Site is located within the Jock River Reach One Subwatershed, which requires Enhanced Level of Protection water quality control (80% TSS removal). Per the Design Report for the O'Keefe Court – 416 Lands Commercial Development (IBI Group, 2013), the end-of-pipe stormwater management facility within the O'Keefe Court Commercial Business will provide water quality control. Therefore, no additional water quality control is required on-site.

4.4 Erosion & Sediment Control

In order to minimize the effects of erosion during the grading of the Site, the following measures are proposed during construction for erosion and sedimentation control:

- i) Erosion and sedimentation facilities are to be installed prior to any area grading operations;
- ii) All erosion control measures are to be inspected and monitored by the contractor and repairs are to be completed as required;
- iii) All materials and equipment used for the purpose of site preparation and project completion should be operated and stored in a manner that prevents any deleterious substance from leaving the site;
- iv) Re-vegetation of completed areas as soon as possible after construction, including those areas not slated for construction, within 60 days of rough grading; and,
- v) To minimize the amount of mud being tracked onto the roadway, a mud mat should be installed at the primary construction entrance.

5.0 Conclusions and Recommendations

Based on the foregoing analysis, it is concluded that:

- i) the proposed SWM design provides adequate storage volume and attenuation of the 5- and 100-year storm events to allowable release rate set by the Design Brief for O'Keefe Court Commercial Development (IBI Group, 2018); and,
- ii) upon completion of construction, the site will conform to the design criteria specified by the City of Ottawa.

It is recommended that:

- i) the site grading be undertaken according to the proposed elevations, details and erosion control measures shown on the separately appended engineering drawings;
- ii) the stormwater management system be installed as detailed on the separately appended engineering drawings; and,
- iii) the stormwater management facilities be inspected by MTE Consultants Inc. during construction and certified to the City of Ottawa upon completion.

All of which is respectfully submitted,

MTE Consultants Inc.



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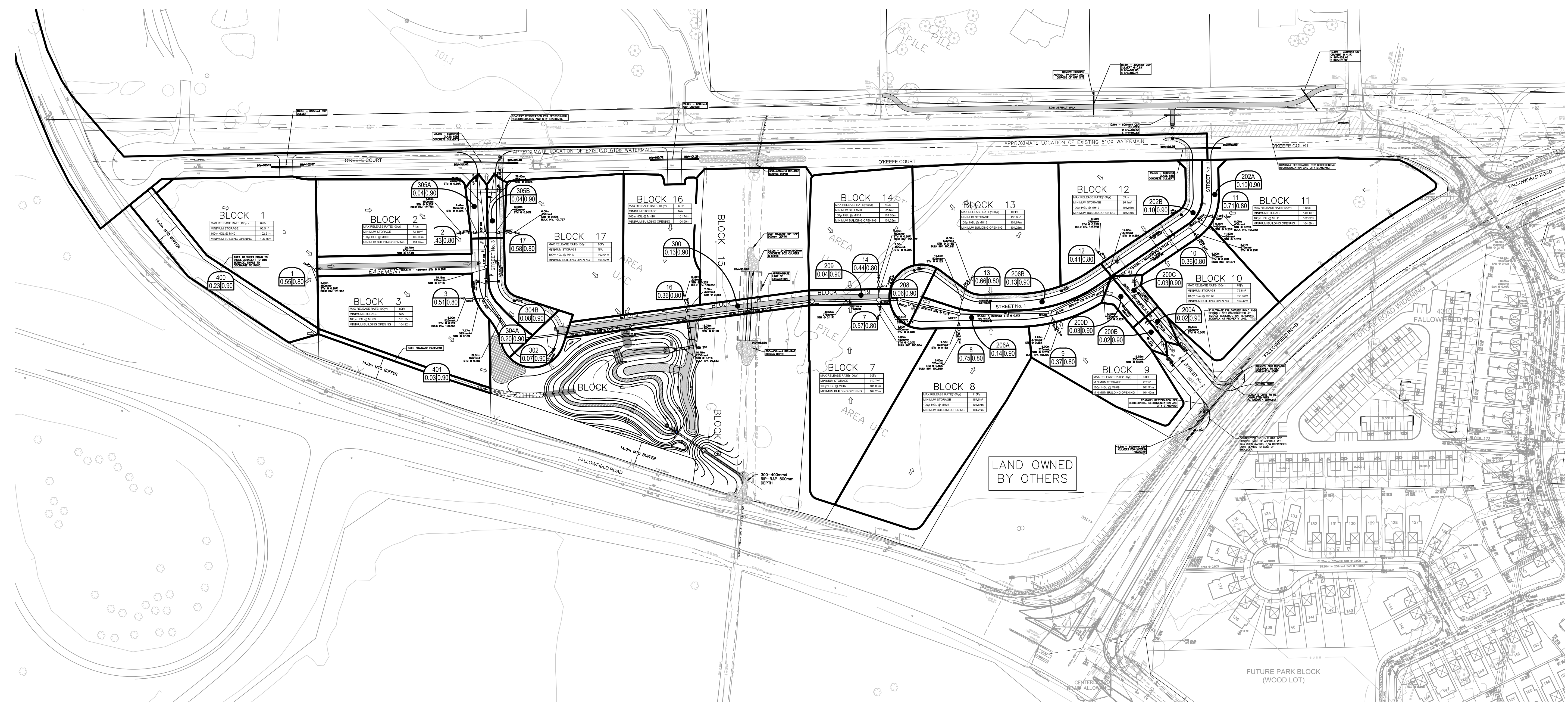
JHN:dlb

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Appendix A

416 Lands Storm Drainage Area Plan (IBI Group, 2022)

J:\39744-may16\5.9 Drawings\5.9\Drawings\Layout\5.9\5.9 Dwg Layout Name: 50051M 18B Plot Size: AIA STANDARD-FULLCTB Plot Scale: 1:25.4 Plotted At: 2/22/2018 11:12 PM Last Saved By: ehenne Last Saved At: May 22, 18



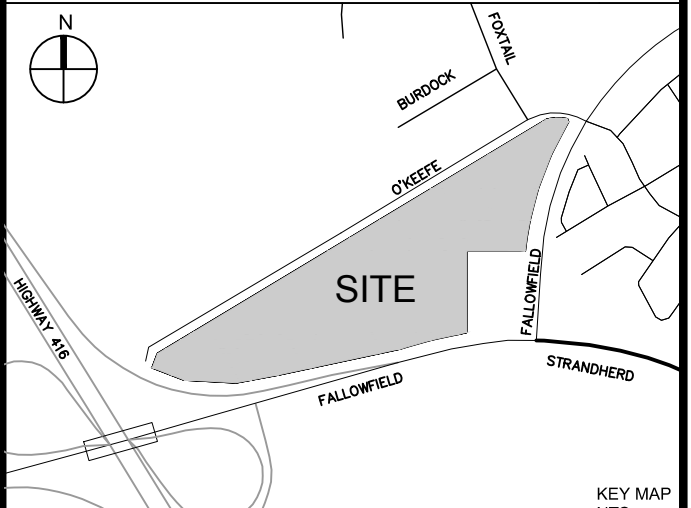
REVIEWED BY
DEVELOPMENT REVIEW SERVICES BRANCH
Signed _____
Date _____ 2017
Plan Number _____

LEGEND :

NOTE :

 MINIMUM BUILDING OPENING TO BE CONFIRMED AT SITE PLAN STAGE.

SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



No.	REVISIONS	By	Date
14			
13			
12			
11			
10			
9			
8			
7			
6			
5	REVISED PER CITY COMMENTS	DGY	18.05.22
4	REVISED PER CITY COMMENTS	DGY	18.04.06
3	REVISED PER CITY COMMENTS	DGY	18.01.10
2	REVISED PER CITY COMMENTS	DGY	17.09.26
1	ISSUED FOR CITY REVIEW	DGY	17.05.04

DCR/PHOENIX GROUP OF COMPANIES
 18A BENTLEY AVE
 OTTAWA ONT
 K2E 6T8

IBI GROUP
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 Ottawa ON K1S 5N4 Canada
 tel 613 225 1311 fax 613 225 9888
 ibigroup.com

Project Title
416 LANDS
 4401 FALLOWFIELD ROAD

Drawing Title
STORM DRAINAGE
AREA PLAN

Scale
 1:1250

Design	MB/RM	Date	MARCH 2016
Drawn	EH	Checked	DGY

Project No.	39744	Drawing No.	500
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CITY PLAN No. 17492
CITY FILE No. D07-16-13-0013

Appendix B

Calculations and IDF Parameters

CALCULATIONS

Orifice Equation (MIDUSS NET)

$$Q = C_c \frac{\pi}{4} D^2 \sqrt{2g(H - 2/3D)}$$

where

- C_c coefficient of contraction
- H head relative to the invert of the orifice
- D orifice diameter
- g gravitational acceleration

IDF Parameters

IDF Parameters	2-Year Storm Event	5-Year Storm Event	100-Year Storm Event
A	732.951	998.071	1735.688
B	6.199	6.053	6.014
C	0.810	0.814	0.820

Appendix C

MIDUSS Output

Post-Development



```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                         Q:\52222\100\SWM"
"          Output filename:                    2 Year Post a.out"
"          Licensee name:                      A"
"          Company                             "
"          Date & Time last used:              11/30/2022 at 6:31:41 PM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          732.951 Coefficient A"
"          6.199 Constant B"
"          0.810 Exponent C"
"          0.400 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                    103.571 mm/hr"
"          Total depth                          31.880 mm"
"          6 002hyd Hydrograph extension used in this file"
" 33      CATCHMENT 201"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          201 Building Rooftop"
"          100.000 % Impervious"
"          0.126 Total Area"
"          7.500 Flow length"
"          2.000 Overland Slope"
"          0.000 Pervious Area"
"          7.500 Pervious length"
"          2.000 Pervious slope"
"          0.126 Impervious Area"
"          7.500 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious SCS Curve No."
"          0.000 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          8.467 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.815 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.028 0.000 0.000 0.000 c.m/sec"

```

```

"          Catchment 201          Pervious   Impervious Total Area "
"          Surface Area          0.000     0.126     0.126     hectare"
"          Time of concentration 12.484     0.987     0.987     minutes"
"          Time to Centroid      118.364    89.244    89.244    minutes"
"          Rainfall depth        31.880    31.880    31.880    mm"
"          Rainfall volume        0.00      40.17     40.17     c.m"
"          Rainfall losses        26.828    5.912     5.912     mm"
"          Runoff depth           5.052     25.968    25.968    mm"
"          Runoff volume          0.00      32.72     32.72     c.m"
"          Runoff coefficient      0.000     0.815     0.815     "
"          Maximum flow           0.000     0.028     0.028     c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.028     0.028     0.000     0.000"
" 54      POND DESIGN"
"          0.028  Current peak flow    c.m/sec"
"          0.036  Target outflow    c.m/sec"
"          32.7   Hydrograph volume    c.m"
"          11.   Number of stages"
"          0.000  Minimum water level    metre"
"          0.150  Maximum water level    metre"
"          0.000  Starting water level    metre"
"          0     Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"              0.000     0.000     0.000"
"              0.01500    0.00090    0.04275"
"              0.03000    0.00180    0.3420"
"              0.04500    0.00270    1.154"
"              0.06000    0.00360    2.736"
"              0.07500    0.00450    5.344"
"              0.09000    0.00540    9.234"
"              0.1050    0.00630    14.663"
"              0.1200    0.00720    21.888"
"              0.1350    0.00810    31.165"
"              0.1500    0.00900    42.750"
"          1.  ROOFTOP"
"              Roof area  Store area  Area/drain  Drain flow  Roof slope"
"              hectare   hectare    sq.metre   L/min/25mm  g H:1V"
"              0.126     0.095     250.000    22.500     50.000"
"          Using 4 roofdrains on roofstorage area of 950. square metre"
"          Peak outflow          0.006     c.m/sec"
"          Maximum level          0.102     metre"
"          Maximum storage        13.647    c.m"
"          Centroidal lag         1.879     hours"
"              0.028     0.028     0.006     0.000 c.m/sec"
" 40      HYDROGRAPH  Combine  1"
"          6  Combine "
"          1  Node #"
"              To Lusk Street"
"          Maximum flow          0.006     c.m/sec"

```


"		Hydrograph volume	32.721	c.m"	
"		0.028	0.028	0.006	0.006"
" 40		HYDROGRAPH Start - New Tributary"			
"		2 Start - New Tributary"			
"		0.028	0.000	0.006	0.006"
" 33		CATCHMENT 202"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	202	Controlled Parking/Driveway Area"			
"	96.000	% Impervious"			
"	0.305	Total Area"			
"	15.000	Flow length"			
"	1.000	Overland Slope"			
"	0.012	Pervious Area"			
"	15.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.293	Impervious Area"			
"	15.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious SCS Curve No."			
"	0.159	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.467	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.833	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.061	0.000	0.006	0.006 c.m/sec"
"		Catchment 202	Pervious	Impervious	Total Area "
"		Surface Area	0.012	0.293	0.305 hectare"
"		Time of concentration	23.296	1.841	2.010 minutes"
"		Time to Centroid	131.220	90.575	90.895 minutes"
"		Rainfall depth	31.880	31.880	31.880 mm"
"		Rainfall volume	3.89	93.34	97.23 c.m"
"		Rainfall losses	26.811	5.318	6.178 mm"
"		Runoff depth	5.069	26.562	25.702 mm"
"		Runoff volume	0.62	77.77	78.39 c.m"
"		Runoff coefficient	0.159	0.833	0.806 "
"		Maximum flow	0.000	0.061	0.061 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"		4 Add Runoff "			
"		0.061	0.061	0.006	0.006"
" 54		POND DESIGN"			
"	0.061	Current peak flow	c.m/sec"		
"	0.036	Target outflow	c.m/sec"		
"	78.4	Hydrograph volume	c.m"		
"	8.	Number of stages"			

```

" 101.012 Minimum water level metre"
" 103.700 Maximum water level metre"
" 101.012 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 101.012 0.000 0.000"
" 103.400 0.04795 8.600"
" 103.450 0.04846 10.240"
" 103.500 0.04897 15.940"
" 103.550 0.04948 34.710"
" 103.600 0.04998 75.430"
" 103.650 0.05048 146.770"
" 103.700 0.05097 246.500"
" 1. ORIFICES"
" Orifice Orifice Orifice Number of"
" invert coefficie diameter orifices"
" 101.012 0.630 0.1200 1.000"
" Peak outflow 0.048 c.m/sec"
" Maximum level 103.426 metre"
" Maximum storage 9.450 c.m"
" Centroidal lag 1.566 hours"
" 0.061 0.061 0.048 0.006 c.m/sec"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" To Lusk Street"
" Maximum flow 0.054 c.m/sec"
" Hydrograph volume 110.871 c.m"
" 0.061 0.061 0.048 0.054"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.061 0.000 0.048 0.054"
" 33 CATCHMENT 203"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 203 Uncontrolled Area to Lusk Street"
" 28.600 % Impervious"
" 0.043 Total Area"
" 2.000 Flow length"
" 18.000 Overland Slope"
" 0.031 Pervious Area"
" 2.000 Pervious length"
" 18.000 Pervious slope"
" 0.012 Impervious Area"
" 2.000 Impervious length"
" 18.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 75.000 Pervious SCS Curve No."
" 0.156 Pervious Runoff coefficient"

```

"	0.100	Pervious Ia/S coefficient"			
"	8.467	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.729	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.003	0.000	0.048	0.054 c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area "
"		Surface Area	0.031	0.012	0.043 hectare"
"		Time of concentration	2.922	0.231	1.167 minutes"
"		Time to Centroid	107.060	87.909	94.573 minutes"
"		Rainfall depth	31.880	31.880	31.880 mm"
"		Rainfall volume	9.79	3.92	13.71 c.m"
"		Rainfall losses	26.915	8.655	21.693 mm"
"		Runoff depth	4.965	23.225	10.187 mm"
"		Runoff volume	1.52	2.86	4.38 c.m"
"		Runoff coefficient	0.156	0.729	0.320 "
"		Maximum flow	0.001	0.003	0.003 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.003	0.003	0.048	0.054"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.003	0.003	0.003	0.054"
" 40		HYDROGRAPH Combine 1"			
"	6	Combine "			
"	1	Node #"			
"		To Lusk Street"			
"		Maximum flow		0.056	c.m/sec"
"		Hydrograph volume		115.251	c.m"
"		0.003	0.003	0.003	0.056"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.003	0.000	0.003	0.056"
" 33		CATCHMENT 204"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	204	Uncontrolled Area to O'Keefe Court"			
"	27.000	% Impervious"			
"	0.046	Total Area"			
"	6.000	Flow length"			
"	20.000	Overland Slope"			
"	0.034	Pervious Area"			
"	6.000	Pervious length"			
"	20.000	Pervious slope"			
"	0.012	Impervious Area"			
"	6.000	Impervious length"			
"	20.000	Impervious slope"			

```

"      0.250 Pervious Manning 'n'"
"      75.000 Pervious SCS Curve No."
"      0.158 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.467 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.756 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"              0.003      0.000      0.003      0.056 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area      "
"      Surface Area      0.034      0.012      0.046      hectare"
"      Time of concentration 5.473      0.433      2.254      minutes"
"      Time to Centroid 110.034      89.119      96.676      minutes"
"      Rainfall depth      31.880      31.880      31.880      mm"
"      Rainfall volume      10.71      3.96      14.66      c.m"
"      Rainfall losses      26.839      7.789      21.695      mm"
"      Runoff depth      5.041      24.090      10.184      mm"
"      Runoff volume      1.69      2.99      4.68      c.m"
"      Runoff coefficient      0.158      0.756      0.319      "
"      Maximum flow      0.001      0.003      0.003      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.003      0.003      0.003      0.056"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"              0.003      0.003      0.003      0.056"
" 40      HYDROGRAPH Combine 2"
"      6      Combine "
"      2      Node #"
"      To O'Keefe Court"
"      Maximum flow      0.003      c.m/sec"
"      Hydrograph volume      4.685      c.m"
"              0.003      0.003      0.003      0.003"
" 38      START/RE-START TOTALS 204"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      0.520      hectare"
"      Total Impervious area      0.444      hectare"
"      Total % impervious      85.292"
" 19      EXIT"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          Q:\52222\100\SWM"
"          Output filename:                     5 Year Post a.out"
"          Licensee name:                       A"
"          Company                              "
"          Date & Time last used:               11/30/2022 at 1:31:20 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          998.071  Coefficient A"
"          6.053  Constant B"
"          0.814  Exponent C"
"          0.400  Fraction R"
"          180.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    141.178  mm/hr"
"          Total depth                          42.540  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 201"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          201  Building Rooftop"
"          100.000  % Impervious"
"          0.126  Total Area"
"          7.500  Flow length"
"          2.000  Overland Slope"
"          0.000  Pervious Area"
"          7.500  Pervious length"
"          2.000  Pervious slope"
"          0.126  Impervious Area"
"          7.500  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious SCS Curve No."
"          0.000  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.467  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.840  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.040  0.000  0.000  0.000 c.m/sec"

```

```

"          Catchment 201          Pervious   Impervious Total Area "
"          Surface Area          0.000     0.126     0.126     hectare"
"          Time of concentration 8.798     0.860     0.860     minutes"
"          Time to Centroid      111.618   88.094    88.094    minutes"
"          Rainfall depth        42.540    42.540    42.540    mm"
"          Rainfall volume        0.00     53.60     53.60     c.m"
"          Rainfall losses        32.802    6.803     6.803     mm"
"          Runoff depth           9.738    35.737    35.737    mm"
"          Runoff volume           0.00     45.03     45.03     c.m"
"          Runoff coefficient      0.000    0.840     0.840     "
"          Maximum flow           0.000    0.040     0.040     c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.040     0.040     0.000     0.000"
" 54      POND DESIGN"
"          0.040  Current peak flow    c.m/sec"
"          0.036  Target outflow    c.m/sec"
"          45.0   Hydrograph volume  c.m"
"          11.   Number of stages"
"          0.000  Minimum water level    metre"
"          0.150  Maximum water level    metre"
"          0.000  Starting water level    metre"
"          0     Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"              0.000     0.000     0.000"
"              0.01500   0.00090   0.04275"
"              0.03000   0.00180   0.3420"
"              0.04500   0.00270   1.154"
"              0.06000   0.00360   2.736"
"              0.07500   0.00450   5.344"
"              0.09000   0.00540   9.234"
"              0.1050   0.00630   14.663"
"              0.1200   0.00720   21.888"
"              0.1350   0.00810   31.165"
"              0.1500   0.00900   42.750"
"          1.  ROOFTOP"
"              Roof area  Store area  Area/drain  Drain flow  Roof slope"
"              hectare   hectare    sq.metre   L/min/25mm  g H:1V"
"              0.126     0.095     250.000    22.500     50.000"
"          Using 4 roofdrains on roofstorage area of 950. square metre"
"          Peak outflow          0.007     c.m/sec"
"          Maximum level          0.117     metre"
"          Maximum storage        20.657    c.m"
"          Centroidal lag         2.000     hours"
"              0.040     0.040     0.007     0.000 c.m/sec"
" 40      HYDROGRAPH  Combine  1"
"          6  Combine "
"          1  Node #"
"              To Lusk Street"
"          Maximum flow          0.007     c.m/sec"

```

"		Hydrograph volume	45.126	c.m"	
"		0.040	0.040	0.007	0.007"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.040	0.000	0.007	0.007"
" 33		CATCHMENT 202"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	202	Controlled Parking/Driveway Area"			
"	96.000	% Impervious"			
"	0.305	Total Area"			
"	15.000	Flow length"			
"	1.000	Overland Slope"			
"	0.012	Pervious Area"			
"	15.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.293	Impervious Area"			
"	15.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious SCS Curve No."			
"	0.229	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.467	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.866	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.088	0.000	0.007	0.007 c.m/sec"
"		Catchment 202	Pervious	Impervious	Total Area "
"		Surface Area	0.012	0.293	0.305 hectare"
"		Time of concentration	16.418	1.606	1.767 minutes"
"		Time to Centroid	121.454	89.190	89.542 minutes"
"		Rainfall depth	42.540	42.540	42.540 mm"
"		Rainfall volume	5.19	124.56	129.75 c.m"
"		Rainfall losses	32.789	5.697	6.781 mm"
"		Runoff depth	9.751	36.843	35.759 mm"
"		Runoff volume	1.19	107.88	109.07 c.m"
"		Runoff coefficient	0.229	0.866	0.841 "
"		Maximum flow	0.000	0.088	0.088 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.088	0.088	0.007	0.007"
" 54		POND DESIGN"			
"	0.088	Current peak flow	c.m/sec"		
"	0.036	Target outflow	c.m/sec"		
"	109.1	Hydrograph volume	c.m"		
"	8.	Number of stages"			

```

" 101.012 Minimum water level metre"
" 103.700 Maximum water level metre"
" 101.012 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 101.012 0.000 0.000"
" 103.400 0.04795 8.600"
" 103.450 0.04846 10.240"
" 103.500 0.04897 15.940"
" 103.550 0.04948 34.710"
" 103.600 0.04998 75.430"
" 103.650 0.05048 146.770"
" 103.700 0.05097 246.500"
" 1. ORIFICES"
" Orifice Orifice Orifice Number of"
" invert coefficie diameter orifices"
" 101.012 0.630 0.1200 1.000"
" Peak outflow 0.049 c.m/sec"
" Maximum level 103.509 metre"
" Maximum storage 19.500 c.m"
" Centroidal lag 1.560 hours"
" 0.088 0.088 0.049 0.007 c.m/sec"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" To Lusk Street"
" Maximum flow 0.056 c.m/sec"
" Hydrograph volume 154.406 c.m"
" 0.088 0.088 0.049 0.056"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.088 0.000 0.049 0.056"
" 33 CATCHMENT 203"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 203 Uncontrolled Area to Lusk Street"
" 28.600 % Impervious"
" 0.043 Total Area"
" 2.000 Flow length"
" 18.000 Overland Slope"
" 0.031 Pervious Area"
" 2.000 Pervious length"
" 18.000 Pervious slope"
" 0.012 Impervious Area"
" 2.000 Impervious length"
" 18.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 75.000 Pervious SCS Curve No."
" 0.227 Pervious Runoff coefficient"

```



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"      0.100 Pervious Ia/S coefficient"
"      8.467 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.757 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.005      0.000      0.049      0.056 c.m/sec"
"      Catchment 203 Pervious Impervious Total Area "
"      Surface Area      0.031      0.012      0.043      hectare"
"      Time of concentration 2.059      0.201      0.996      minutes"
"      Time to Centroid 102.812      86.627      93.550      minutes"
"      Rainfall depth      42.540      42.540      42.540      mm"
"      Rainfall volume      13.06      5.23      18.29      c.m"
"      Rainfall losses      32.894      10.323      26.439      mm"
"      Runoff depth      9.646      32.217      16.101      mm"
"      Runoff volume      2.96      3.96      6.92      c.m"
"      Runoff coefficient      0.227      0.757      0.378      "
"      Maximum flow      0.002      0.004      0.005      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.005      0.005      0.049      0.056"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"          0.005      0.005      0.005      0.056"
" 40 HYDROGRAPH Combine 1"
"      6 Combine "
"      1 Node #"
"      To Lusk Street"
"      Maximum flow      0.059      c.m/sec"
"      Hydrograph volume      161.329      c.m"
"          0.005      0.005      0.005      0.059"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"          0.005      0.000      0.005      0.059"
" 33 CATCHMENT 204"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      204 Uncontrolled Area to O'Keefe Court"
" 27.000 % Impervious"
"      0.046 Total Area"
"      6.000 Flow length"
" 20.000 Overland Slope"
"      0.034 Pervious Area"
"      6.000 Pervious length"
" 20.000 Pervious slope"
"      0.012 Impervious Area"
"      6.000 Impervious length"
" 20.000 Impervious slope"

```

```

"      0.250 Pervious Manning 'n'"
"    75.000 Pervious SCS Curve No."
"      0.226 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.467 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"      0.778 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.005      0.000      0.005      0.059 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area      "
"      Surface Area      0.034      0.012      0.046      hectare"
"      Time of concentration 3.857      0.377      1.908      minutes"
"      Time to Centroid 105.483      87.866      95.614      minutes"
"      Rainfall depth 42.540      42.540      42.540      mm"
"      Rainfall volume 14.28      5.28      19.57      c.m"
"      Rainfall losses 32.928      9.436      26.585      mm"
"      Runoff depth 9.612      33.104      15.955      mm"
"      Runoff volume 3.23      4.11      7.34      c.m"
"      Runoff coefficient 0.226      0.778      0.375      "
"      Maximum flow 0.002      0.004      0.005      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.005      0.005      0.005      0.059"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"          0.005      0.005      0.005      0.059"
" 40 HYDROGRAPH Combine 2"
"      6 Combine "
"      2 Node #"
"      To O'Keefe Court"
"      Maximum flow      0.005      c.m/sec"
"      Hydrograph volume 7.339      c.m"
"          0.005      0.005      0.005      0.005"
" 38 START/RE-START TOTALS 204"
"      3 Runoff Totals on EXIT"
"      Total Catchment area      0.520      hectare"
"      Total Impervious area      0.444      hectare"
"      Total % impervious      85.292"
" 19 EXIT"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          Q:\52222\100\SWM"
"          Output filename:                     100 Year Post a.out"
"          Licensee name:                       A"
"          Company                              "
"          Date & Time last used:               11/30/2022 at 1:33:39 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1735.688  Coefficient A"
"          6.014  Constant B"
"          0.820  Exponent C"
"          0.400  Fraction R"
"          180.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    242.704  mm/hr"
"          Total depth                          71.708  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 201"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          201  Building Rooftop"
"          100.000  % Impervious"
"          0.126  Total Area"
"          7.500  Flow length"
"          2.000  Overland Slope"
"          0.000  Pervious Area"
"          7.500  Pervious length"
"          2.000  Pervious slope"
"          0.126  Impervious Area"
"          7.500  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious SCS Curve No."
"          0.000  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.467  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.868  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.072  0.000  0.000  0.000 c.m/sec"

```

```

"          Catchment 201          Pervious   Impervious Total Area "
"          Surface Area          0.000     0.126     0.126     hectare"
"          Time of concentration  5.470     0.684     0.684     minutes"
"          Time to Centroid      103.771   86.695     86.695     minutes"
"          Rainfall depth        71.708    71.708     71.708     mm"
"          Rainfall volume        0.00      90.35      90.35      c.m"
"          Rainfall losses        44.863    9.485      9.485      mm"
"          Runoff depth           26.845    62.223     62.223     mm"
"          Runoff volume           0.00      78.40      78.40      c.m"
"          Runoff coefficient      0.000     0.868     0.868     "
"          Maximum flow           0.000     0.072     0.072     c.m/sec"
" 40          HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.072     0.072     0.000     0.000"
" 54          POND DESIGN"
"          0.072  Current peak flow    c.m/sec"
"          0.036  Target outflow    c.m/sec"
"          78.4   Hydrograph volume  c.m"
"          11.   Number of stages"
"          0.000  Minimum water level  metre"
"          0.150  Maximum water level  metre"
"          0.000  Starting water level  metre"
"          0     Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"              0.000     0.000     0.000"
"              0.01500   0.00090   0.04275"
"              0.03000   0.00180   0.3420"
"              0.04500   0.00270   1.154"
"              0.06000   0.00360   2.736"
"              0.07500   0.00450   5.344"
"              0.09000   0.00540   9.234"
"              0.1050    0.00630   14.663"
"              0.1200    0.00720   21.888"
"              0.1350    0.00810   31.165"
"              0.1500    0.00900   42.750"
"          1.  ROOFTOP"
"              Roof area  Store area  Area/drain  Drain flow  Roof slope"
"              hectare   hectare    sq.metre   L/min/25mm  g H:1V"
"              0.126     0.095     250.000    22.500     50.000"
"          Using 4 roofdrains on roofstorage area of 950. square metre"
"          Peak outflow                0.009    c.m/sec"
"          Maximum level                 0.148    metre"
"          Maximum storage                41.083    c.m"
"          Centroidal lag                2.301    hours"
"              0.072     0.072     0.009     0.000 c.m/sec"
" 40          HYDROGRAPH  Combine  1"
"          6  Combine "
"          1  Node #"
"              To Lusk Street"
"          Maximum flow                0.009    c.m/sec"

```

"		Hydrograph volume	78.387	c.m"	
"		0.072	0.072	0.009	0.009"
" 40		HYDROGRAPH Start - New Tributary"			
"		2 Start - New Tributary"			
"		0.072	0.000	0.009	0.009"
" 33		CATCHMENT 202"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	202	Controlled Parking/Driveway Area"			
"	96.000	% Impervious"			
"	0.305	Total Area"			
"	15.000	Flow length"			
"	1.000	Overland Slope"			
"	0.012	Pervious Area"			
"	15.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.293	Impervious Area"			
"	15.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious SCS Curve No."			
"	0.375	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.467	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.907	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.162	0.000	0.009	0.009 c.m/sec"
"		Catchment 202	Pervious	Impervious	Total Area "
"		Surface Area	0.012	0.293	0.305 hectare"
"		Time of concentration	10.207	1.276	1.427 minutes"
"		Time to Centroid	110.254	87.347	87.735 minutes"
"		Rainfall depth	71.708	71.708	71.708 mm"
"		Rainfall volume	8.75	209.96	218.71 c.m"
"		Rainfall losses	44.837	6.696	8.222 mm"
"		Runoff depth	26.871	65.011	63.486 mm"
"		Runoff volume	3.28	190.35	193.63 c.m"
"		Runoff coefficient	0.375	0.907	0.885 "
"		Maximum flow	0.002	0.162	0.162 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"		4 Add Runoff "			
"		0.162	0.162	0.009	0.009"
" 54		POND DESIGN"			
"	0.162	Current peak flow	c.m/sec"		
"	0.036	Target outflow	c.m/sec"		
"	193.6	Hydrograph volume	c.m"		
"	8.	Number of stages"			

```

" 101.012 Minimum water level metre"
" 103.700 Maximum water level metre"
" 101.012 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 101.012 0.000 0.000"
" 103.400 0.04795 8.600"
" 103.450 0.04846 10.240"
" 103.500 0.04897 15.940"
" 103.550 0.04948 34.710"
" 103.600 0.04998 75.430"
" 103.650 0.05048 146.770"
" 103.700 0.05097 246.500"
" 1. ORIFICES"
" Orifice Orifice Orifice Number of"
" invert coefficie diameter orifices"
" 101.012 0.630 0.1200 1.000"
" Peak outflow 0.050 c.m/sec"
" Maximum level 103.579 metre"
" Maximum storage 58.313 c.m"
" Centroidal lag 1.628 hours"
" 0.162 0.162 0.050 0.009 c.m/sec"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" To Lusk Street"
" Maximum flow 0.059 c.m/sec"
" Hydrograph volume 272.449 c.m"
" 0.162 0.162 0.050 0.059"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.162 0.000 0.050 0.059"
" 33 CATCHMENT 203"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 203 Uncontrolled Area to Lusk Street"
" 28.600 % Impervious"
" 0.043 Total Area"
" 2.000 Flow length"
" 18.000 Overland Slope"
" 0.031 Pervious Area"
" 2.000 Pervious length"
" 18.000 Pervious slope"
" 0.012 Impervious Area"
" 2.000 Impervious length"
" 18.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 75.000 Pervious SCS Curve No."
" 0.369 Pervious Runoff coefficient"

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"      0.100 Pervious Ia/S coefficient"
"      8.467 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
"      0.796 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.013      0.000      0.050      0.059 c.m/sec"
"      Catchment 203      Pervious      Impervious Total Area "
"      Surface Area      0.031      0.012      0.043      hectare"
"      Time of concentration 1.280      0.160      0.761      minutes"
"      Time to Centroid      97.778      84.828      91.771      minutes"
"      Rainfall depth      71.708      71.708      71.708      mm"
"      Rainfall volume      22.02      8.82      30.83      c.m"
"      Rainfall losses      45.270      14.607      36.500      mm"
"      Runoff depth      26.438      57.100      35.208      mm"
"      Runoff volume      8.12      7.02      15.14      c.m"
"      Runoff coefficient      0.369      0.796      0.491      "
"      Maximum flow      0.006      0.007      0.013      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.013      0.013      0.050      0.059"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.013      0.013      0.013      0.059"
" 40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"          To Lusk Street"
"      Maximum flow      0.069      c.m/sec"
"      Hydrograph volume      287.588      c.m"
"          0.013      0.013      0.013      0.069"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"          0.013      0.000      0.013      0.069"
" 33      CATCHMENT 204"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      204      Uncontrolled Area to O'Keefe Court"
" 27.000      % Impervious"
"      0.046      Total Area"
"      6.000      Flow length"
" 20.000      Overland Slope"
"      0.034      Pervious Area"
"      6.000      Pervious length"
" 20.000      Pervious slope"
"      0.012      Impervious Area"
"      6.000      Impervious length"
" 20.000      Impervious slope"

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"      0.250 Pervious Manning 'n'"
"      75.000 Pervious SCS Curve No."
"      0.372 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      8.467 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.809 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"              0.012      0.000      0.013      0.069 c.m/sec"
"      Catchment 204      Pervious      Impervious Total Area "
"      Surface Area      0.034      0.012      0.046      hectare"
"      Time of concentration 2.398      0.300      1.462      minutes"
"      Time to Centroid 99.506      85.952      93.463      minutes"
"      Rainfall depth 71.708      71.708      71.708      mm"
"      Rainfall volume 24.08      8.91      32.99      c.m"
"      Rainfall losses 45.047      13.721      36.589      mm"
"      Runoff depth 26.661      57.987      35.119      mm"
"      Runoff volume 8.95      7.20      16.15      c.m"
"      Runoff coefficient 0.372      0.809      0.490      "
"      Maximum flow 0.006      0.007      0.012      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"              0.012      0.012      0.013      0.069"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"              0.012      0.012      0.012      0.069"
" 40 HYDROGRAPH Combine 2"
"      6 Combine "
"      2 Node #"
"      To O'Keefe Court"
"      Maximum flow      0.012      c.m/sec"
"      Hydrograph volume 16.155      c.m"
"              0.012      0.012      0.012      0.012"
" 38 START/RE-START TOTALS 204"
"      3 Runoff Totals on EXIT"
"      Total Catchment area      0.520      hectare"
"      Total Impervious area      0.444      hectare"
"      Total % impervious      85.292"
" 19 EXIT"

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