

Report  
Project: 142609-6.4.3

# 1050 TAWADINA ROAD SERVICING BRIEF

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IBI GROUP

Prepared for West Urban Developments  
by ARCADIS/IBI GROUP

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# 1 INTRODUCTION

In 2011, Canada Lands Company (CLC), bought and took ownership of about 125 ha of the former CFB Rockcliffe air base site. The acquisition of the decommissioned base by CLC offers the opportunity today to reconnect this site back into the urban fabric of the City and create a highly desirable mixed-use community for approximately 10,000 residents. CLC completed a Community Design Plan (CDP) in 2015. In support of the CDP, there were numerous supporting documents including the “Former CFB Rockcliffe Master Servicing Study” (MSS), August 2015, prepared by IBI Group. That report provided a plan for provision of major infrastructure needed to support the proposed development of the Wateridge Village.

CLC plans to develop the Wateridge Village property in several phases. Phases 1A, 1B and 2B have already been constructed, which cover about 45 ha. The Phase 2B registered 4M plan is provided in **Appendix A**. This phase covers about 10 ha and includes 12 blocks. Block 11 is located in the West portion of the Wateridge Village Phase 2B and has been severed into 2 parcels. The plan showing the severed parcels is included in **Appendix A**. ARCADIS/IBI Group Professional Services Inc. (ARCADIS/IBI Group) has been retained by West Urban Developments to provide professional engineering services for Block 11, Parcel 1. The subject site is approximately 0.72 ha and consists of two 9- storey residential buildings and an amenity building, with a total of 254 units. The site also consists of below grade parking facilities. Additionally, the 1050 Tawadina M-plan and Architectural Site Plan have also been provided in **Appendix A**.

Block 11, Parcel 1 is bounded by Tawadina Road to the North, Parcel 2 to the South, Bareille-Snow Street to the West and Michael Stoqua Street to the East. Its Civic Address is 1050 Tawadina Road. Refer to key plan on **Figure 1.1** for Site location.

**Figure 1.1 Site Location**



The proposed servicing design conforms to current City of Ottawa and MECP design criteria, and no pre-consultation meetings were requested from the Rideau Valley Conservation Authority (RVCA) or the Ontario Ministry of Environment, Conservation and Parks (MECP).

## 1.1 Guidelines and Standards

This evaluation takes into consideration the City of Ottawa Sewer Design Guidelines (OSDG) (October 2012), and the February 2014 Technical Bulletin ISDTB-2014-01, the September 2016 Technical Bulletin PIEDTB-2016-01, the June 2018 Technical Bulletin ISTB-2018-04, October 2019 Technical Bulletin 2019-01, and the July Technical Bulletin 2019-02.

It also considers the City of Ottawa Water Distribution Design Guidelines (OWDDG), and the 2010 Technical Bulletin 2010-02, the 2014 Technical Bulletin 2014-02, the 2018 Technical Bulletin 2018-02 and the 2020 Technical Bulletin 2020-02.

All specifications are as per current City of Ottawa standards and specifications, and Province of Ontario (OPSS/D) standards, specifications and drawings.

## 1.2 Pre-Consultation Meeting

The City of Ottawa hosted a virtual pre-consultation meeting on August 15th, 2022. Notes of the meeting are provided in **Appendix A**. There were no major engineering concerns flagged in this meeting. The City of Ottawa Servicing Study Checklist has also been included in **Appendix A**.

## 1.3 Environmental Issues

There are no environmental issues related to this site, as all environmental concerns were dealt with as part of the CLC's Wateridge Phase 2B subdivision approval.

The Wateridge Phase 2B Development has previously cleared and pre-graded the subject lands. There are no existing watercourses or drainage features associated with this site.

## 1.4 Geotechnical Concerns

Englobe Corporation was retained to prepare a geotechnical investigation for the proposed mixed use development for the 1050 Tawadina Road. The objectives of the investigation were to prepare a report to:

- Determine the subsoil and groundwater conditions at the site by means of test pits and boreholes and;
- To provide geotechnical recommendations pertaining to design of the proposed development including construction considerations.

The geotechnical report 02203079.000 was prepared by Englobe Corporation in November 2022. The report contains recommendations which include but are not limited to the following:

- Site grading;
- Foundation Design;
- Pavement Structure;
- Sewer and Watermain Construction;
- Groundwater Control;
- Grade raises

In general the grading plan for 1050 Tawadina Road adheres to the grade raise constraints noted above. A copy of the grading plans is included in **Appendix E**. The site does not pose any significant grade raise; thus a grading plan review letter is not required for this development.

## 2 WATER DISTRIBUTION

### 2.1 Existing Conditions

Phase 2B of Wateridge Village at Rockcliffe will be serviced with potable water from the City of Ottawa's Montreal Road Pressure Zone (Zone MONT). An existing 400 mm diameter watermain on Montreal Road will supply Phase 2B with connections at Codd's Road and Burma Road. As part of the Phase 1 water plan, two 400 mm mains were extended northward along Codd's Road and Wanaki Road. A copy of the existing watermain plan for Phase 2B is included in **Appendix B**.

There is an existing 400mm watermain in Tawadina Road to the north of Block 11, an existing 200mm watermain in Bareille-Snow Street to the west of the site, and an existing 200mm watermain in Michael Stoqua Street to the east of the site. In order to provide a redundant water supply to the subject site, two watermain connections are propose, one from Tawadina Road and the second at Michael Stoqua Street. Refer to the General Plan of Services included in **Appendix A** for the detailed water distribution plan for the site.

### 2.2 Design Criteria

#### 2.2.1 Water Demands

The proposed development consists of 254 apartment units. In order to calculate water demand rates, the per unit population density and consumption rates are taken from Tables 4.1 and 4.2 of the Ottawa Design Guidelines – Water Distribution were used and are summarized as follows:

- Apartment 1.8 person per unit
- Average Day Demand 280 l/cap/day
- Peak Daily Demand 700 l/cap/day
- Peak Hour Demand 1,540 l/cap/day

A water demand calculation sheet is included in **Appendix B** and the total water demands are summarized as follows:

- Average Day 1.48 l/s
- Maximum Day 3.70 l/s
- Peak Hour 8.15 l/s

#### 2.2.2 System Pressures

The 2010 City of Ottawa Water Distribution Guidelines states that the preferred practice for the design of a new distribution system is to have normal operating pressures range between 345 kPa (50 psi) and 552 kPa (80 psi) under maximum daily flow conditions. Other pressure criteria identified in the guidelines are as follows:

- Minimum Pressure Minimum system pressure under peak hour demand conditions shall not be less than 276 kPa (40 psi).
- Fire Flow During the period of maximum day demand, the system pressure shall not be less than 140 kPa (20 psi) during a fire flow event.

**Maximum Pressure** Maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi). In accordance with the Ontario Building/Plumbing Code the maximum pressure should not exceed 552 kPa (80 psi) in occupied areas. Pressure reduction controls may be required for buildings when it is not possible/feasible to maintain the system pressure below 552 kPa.

### 2.2.3 Fire Flow Rate

The Fire Underwriters Survey was used to determine the fire flow for the site. The calculations result in a fire flow of 10,000 l/min; a copy of the FUS calculation is included in **Appendix B**.

### 2.2.4 Boundary Conditions

According to the Master Servicing Study completed by IBI dated June 2020, Nodes N046 and N048 as shown in **Appendix B – Water Distribution System: Hydraulic Modeling Results** indicates the hydrant closest to the proposed connections for the site. The available fire flow for these two hydrants is also tabulated in the report. The available flow for nodes N046 and N048 at 20 psi is 26,690 L/min and 27,290 L/min as shown in Table 3-2, included in **Appendix B**, which is greater than the required domestic and fire demand of 10,000 L/min. Therefore, adequate water supply and pressure are available to serve the proposed development.

### 2.2.5 Hydraulic Model

A computer model for the 1050 Tawadina Road water distribution system has been developed using the InfoWater SA program. The model includes the boundary conditions from the Master Servicing Study completed by IBI dated June 2020.

## 2.3 Proposed Water Plan

The proposed development consists of 146 one-bedroom units and 108 two-bedroom units, equating to an estimated occupancy of 432. Two new 200 mm diameter connections will be installed to service both buildings, one connecting to the existing 400 mm diameter watermain on Tawadina Road and another connecting to the existing 200 mm diameter watermain on Michael Stoqua Street.

The site is surrounded by four existing fire hydrants, one located on Bareille-Snow Street, two on Tawadina Road, and one on Michael Stoqua Street. The hydrants are spaced less than 90 m apart, meeting the requirement of Table 4.9 of the City of Ottawa - Design Guidelines – Water Distribution, July 2010.

Calculations for fire flows using the Fire Underwriters Survey (FUS) indicate a maximum required fire flow of approximately 166.67 L/s (10,000 L/min) for Building A and approximately 150.00 L/s (9,000 L/min) for Building B, based on a non-combustible construction with a sprinkler system designed to NFPA. Since the fire flow calculation for the Building A yields a higher demand, the required fire flow for Building A will be used in subsequent calculations. Refer to **Appendix B** for detailed water demand calculations.

As per Section 2.2.1, the water demand for the proposed development is determined by the greater of the maximum day demand plus fire flow or the peak hour demand. In this instance, the maximum day demand plus fire flow demand ( $3.70 \text{ L/s} + 166.67 \text{ L/s} = 170.37 \text{ L/s} = 10,222 \text{ L/min}$ ) is the governing requirement. Refer to Section 2.2.1 for the summarized water demand requirement.

According to the Master Servicing Study completed by IBI dated June 2020, Nodes N046 and N048 as shown in **Appendix B – Water Distribution System: Hydraulic Modeling Results** indicate the hydrants closest to the proposed connections for the site. The available fire flow for these two hydrants is also tabulated in the report. The available flow for nodes N046 and N048 at 20 psi is 26,690 L/min and 27,290 L/min as shown in Table 3-2, are both greater than the



required domestic and fire demand of 10,000 L/min. Therefore, adequate water supply and pressure are available to serve the proposed development.

Moreover, based on the Block 11 – Parcel 1 Site Plan Submission Technical Memorandum prepared by IBI group dated November 23, 2022, the basic day pressures range from 551.6 kPa to 555.0 kPa on Tawadina Road; the peak hour pressures range between 498.8 kPa and 508.1 kPa; and the fire flows available during maximum day demand range between 462.6 L/s and 850.5 L/s. A copy of the Block 11 – Parcel 1 Site Plan Submission Technical Memorandum is included in **Appendix A**. Since the peak hour pressure exceed 276 kPa as per City's criteria and the available fire flow exceeds the required fire flow rate of 320.17 L/s, the water distribution system surrounding the proposed development is adequate to support the proposed development

## 3 WASTEWATER

### 3.1 Existing Conditions

Canada Lands Company completed a Community Design Plan (CDP) in 2015. To support that plan, a number of technical reports were prepared including the 'Former CFB Rockcliffe Master Servicing Study, August 2015 (MSS), which was subsequently updated in June 2020. That report recommended that the existing combined sewers on the subject site be abandoned in favour of dedicated sanitary and storm sewer systems.

In particular, the MSS recommended that future wastewater flow from Phase 2B be directed to the Codd's Road Shaft. Accordingly, wastewater flows from the subject site will be designed to outlet to that location. The previous Phase 1A design included the new connection to that shaft and the proposed Phase 2B sanitary sewers will connect to the Phase 1B system. The sanitary sewers in Phase 2B were oversized to provide capacity for Future Phase 2C and 2D connection. A copy of Phase 2B sanitary drainage area plan and design sheet are included in **Appendix C**.

#### 3.1.1 Verification of Existing Sanitary Sewer Capacity

An analysis was completed by IBI Group to determine the ability of the existing sanitary sewer system to accommodate the proposed development. The results of the analysis are included in the Block 11 – Parcel 1 Site Plan Submission Technical Memorandum dated November 23, 2022. Based on the analysis, the wastewater flows in the Tawadina Road sewer from MH303A to MH304A is 7.96 L/s, with a spare capacity of 23.06 L/s. The sewer downstream of the Tawadina Road sewer, along Bareille-Snow Street, from MH304A to MH308A has a wastewater flow of 26.80 L/s, with a spare capacity of 12.93 L/s. As such, it is IBI Group's opinion that the existing sanitary sewers in Tawadina Road and Bareille-Snow Street can accommodate the sanitary flow from the proposed development.

### 3.2 Proposed Sewers

All on-site sewers have been designed to City of Ottawa and MECP design criteria which include but are not limited to the below listed criteria. The detailed sanitary sewer design sheets which are included in **Appendix C** illustrate the population densities and sewers which provide the necessary outlets. The design wastewater criteria for this analysis area:

#### 3.2.1 Design Flow:

Average Residential Flow	-	280 l/cap/day
Peak Residential Factor	-	Modified Harmon Formula
Infiltration Allowance	-	0.33 l/sec/Ha
Minimum Pipe Size	-	200mm diameter

#### 3.2.2 Population Density:

Apartment Units	-	1.8 person/unit
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## 4 SITE STORMWATER MANAGEMENT

### 4.1 Objective

The purpose of this evaluation is to prepare the dual drainage design, including the minor and major system, for the 1050 Tawadina Road development. The design includes the assignment of inlet control devices, on-site storage, maximum depth of surface ponding and hydraulic grade line analysis. The evaluation takes into consideration the City of Ottawa Sewer Design Guidelines (OSDG) (October 2012), the February 2014 Technical Bulletin ISDTB-2014-01, the September 2016 Technical Bulletin PIEDTB-2016-01 and the June 2018 Technical Bulletin ISTB-2018-04.

### 4.2 Existing Conditions

CLC completed an update to the servicing report, "Former CFB Rockcliffe Master Servicing Study" in 2020. That report recommended a preferred Stormwater Management Plan for the Wateridge Village at Rockcliffe site. The report recommended construction of two stormwater ponds and related appurtenances to service the CLC property; the Western Stormwater Management Facility and the Eastern Stormwater Management Facility. The Eastern Pond is proposed to provide management of flows from most of Phase 1 and 2 of the CLC property, including the subject site. The Eastern pond was constructed and put into service in 2017.

The MSS Report also recommends a series of local and trunk storm sewers to collect runoff from Phases 1 and 2 and route those flows to the Eastern Facility. The Phase 1 design followed the recommendations of the MSS report, including construction of the large diameter sewers, which outlet to the Eastern Stormwater Management Facility; the Eastern Stormwater Management Facility and outlet to the Ottawa River. The Phase 2B storm sewers connect to the downstream Phase 1 sewer system. A copy of the storm drainage area plan and the storm sewer design sheet for Phase 2B are included in **Appendix D**.

### 4.3 Design Criteria

The stormwater system for the subdivision was designed following the principles of dual drainage, making accommodations for both major and minor flow.

Some of the key criteria include the following:

- Design Storm 1:2-year return (Ottawa)
- Rational Method Sewer Sizing
- Initial Time of Concentration 10 minutes
- Runoff Coefficients
  - Landscaped Areas C = 0.25
  - Landscaped Area with Pathway/Roof C = 0.50 - 0.65
  - Building and Roof Area C = 0.90
  - Parking Area and Driveway C = 0.90
- Pipe Velocities 0.80 m/s to 3.0 m/s
- Minimum Pipe Size 250 mm diameter  
(200 mm CB Leads)

## 4.4 System Concept

According to the Wateridge Phase 2B report prepared by IBI Group dated April 2019, the development of the adjacent downstream properties included the expected stormwater servicing needs of the subject property. The existing storm sewers constructed adjacent to the site were oversized to provide the needed capacity for minor storm runoff from the subject site. Minor storm runoff from the subject site is proposed to connect to the existing 525 mmØ sewer in Bareille-Snow Street.

### 4.4.1 Dual Drainage Design

The dual drainage system proposed for the subject site will accommodate both major and minor stormwater runoff. Minor flow from the subject site will be conveyed through the storm sewer network and discharge into the existing 525 mmØ sewer in Bareille-Snow Street.

The balance of the surface flow not captured by the minor system will be conveyed via the major system. Where possible, storage will be provided in surface sags or low points within the roadway. Once the maximum storage is utilized, the excess flow will cascade to the next downstream street sag. Major flow up to 100-year storm event will be restricted and detained on-site. Emergency overflow will be directed towards the south-west corner of the site at Bareille-Snow Street.

### 4.4.2 Proposed Minor System

Using the criteria identified in Section 4.3, the proposed on-site storm sewers were sized accordingly. A detailed storm sewer design sheet and the associated storm sewer drainage area plan are included in **Appendix D**. The general plan of services, depicting all on-site storm sewers can be found in **Appendix A**.

## 4.5 Stormwater Management

Wateridge Phase 2B is part of the larger development referred to as the Former CFB Rockcliffe. The stormwater management strategy was outlined in the “Former CFB Rockcliffe Master Servicing Study” (MSS) (IBI Group, August 2020). Phase 2B is located between Hemlock Road and Tawadina Road (refer to Figure 1.1). As part of the Phase 2B development, the design of downstream Phase 2A has been completed.

The subject site is part of the drainage area that ultimately discharges to the Eastern SWM Facility. The trunk storm sewer to the pond and the pond itself were constructed as part of Wateridge Phase 1A.

### 4.5.1 Water Quality Control

The design takes into consideration the August 2020 MSS, the “Design Brief Wateridge Village at Rockcliffe Phase 1B” (IBI Group, June 2017), the “Design Brief Wateridge Village at Rockcliffe Phase 1A” (IBI Group, April 2016), the City of Ottawa Sewer Design Guidelines (OSDG) (October 2012), and the February 2014 Technical Bulletin ISDTB-2014-01.

Any runoff from the site, as with all future developments in Wateridge Village at Rockcliffe, will have end of pipe quality treatment. Any impacts to receiving watercourses will therefore be mitigated. There are no municipal drains in the vicinity of the subject development and there are no drainage catchment diversions proposed by the current development.

## 4.5.2 Water Quantity Control

The subject site will be limited to a maximum minor system release rate of 195 L/s according to Wateridge Phase 2B Design Brief dated April 2019. In the Phase 2B subdivision stormwater management system design, the development blocks are subjected to minor system inflow restriction with major flow cascading to a street segment. The restricted rates were provided in Table 2-2, taken from the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing, which is included in **Appendix A**. This will be achieved through a combination of inlet control devices (ICD's) at inlet locations, surface storage where possible and underground storage where required.

Surface flows in excess of the site's allowable release rate will be stored on site in a proposed cistern and gradually released into the minor system to respect the site's allowable release rate. The maximum surface retention depth located within the developed areas will be limited to 300mm during a 1:100 year event as show on the ponding plan located in **Appendix D** and grading plans located in **Appendix E**. Overland flow routes will be designed to permit emergency overland flow.

Along the perimeter of the site, the opportunity to capture and store runoff is limited due to grading constraints and building geometry. These areas will discharge uncontrolled to Tawadina Road, Michael Stoqua Street and Bareille-Snow Street. These areas are located at the perimeter of the site where it is necessary to tie into public boulevards and adjacent properties or in areas where ponding stormwater is undesirable.

Based on the proposed site plan, the total uncontrolled area has been calculated to be (0.09+0.05) 0.14 ha. For the detailed storm drainage area plan for the site, refer to Drawing 500 in **Appendix D**.

Based on a 1:100 year event, the flow from the 0.14 ha uncontrolled area can be determined as:

$$\begin{aligned}
 Q_{\text{uncontrolled}} &= 2.78 \times C \times i_{100\text{yr}} \times A \quad \text{where:} \\
 C &= \text{Average runoff coefficient} = 0.58 \times 1.25 = 0.725 \text{ (100 year C-value)} \\
 i_{100\text{yr}} &= \text{Intensity of 100-year storm event (mm/hr)} \\
 &= 1735.688 \times (T_c + 6.014)^{0.820} = 178.56 \text{ mm/hr; where } T_c = 10 \text{ minutes} \\
 A &= \text{Uncontrolled Area} = 0.14 \text{ Ha}
 \end{aligned}$$

Therefore, the uncontrolled release rate can be determined as:

$$\begin{aligned}
 Q_{\text{uncontrolled}} &= 2.78 \times C \times i_{100\text{yr}} \times A \\
 &= 2.78 \times 0.725 \times 178.56 \times 0.14 \\
 &= 50.38 \text{ L/s}
 \end{aligned}$$

The Maximum allowable release rate from the site can be determined by subtracting the Uncontrolled release rate from the minor system restricted flow rate.

$$\begin{aligned}
 Q_{\text{max}} &= Q_{\text{restricted}} - Q_{\text{uncontrolled}} \\
 Q_{\text{max}} &= 195 \text{ L/s} - 50.38 \text{ L/s} \\
 Q_{\text{max}} &= \mathbf{144.62 \text{ L/s}}
 \end{aligned}$$

Surface flows in excess of the site's allowable release rate will be stored on site in the proposed underground cistern and gradually released into the minor system to respect the site's allowable release rate. There will be no surface retention located within the developed site plan. Overland flow routes will be detailed on the grading to permit emergency overland flow.

The modified rational method was used to evaluate the on-site stormwater management. There are two uncontrolled areas on this site. The flows are calculated above. Therefore, the total restricted flow rate through the minor system will be the design flow rate of **144.62 l/s**. This will be achieved using an Inlet Control Device placed in the storm control manhole on-site. A summary of the ICD's, their corresponding storage requirements, storage availability, and associated drainage areas has been provided below.

DRAINAGE AREA	ICD RESTRICTED FLOW (L/s)	100 YEAR STORAGE REQUIRED (m <sup>3</sup> )	SURFACE STORAGE PROVIDED (m <sup>3</sup> )
SC#3	144.00	140	0
<b>TOTAL</b>	<b>144.00</b>	<b>140</b>	<b>0</b>

#### 4.5.3 2 Year Ponding

A review of the 2-year ponding has been completed using the modified rational method. A minimum Tc of 3min has been used. Where volumes are calculated as a negative value, 13.85m<sup>3</sup> has been shown. A summary of each drainage area has been provided below.

DRAINAGE AREA	Total 2-Year Ponding Volume (m <sup>3</sup> )	Comment
SC#3	13.85	This area is controlled at CTRL MH1, and there is 140m <sup>3</sup> of sub- surface storage provided in this area. The required ponding is provided underground. A 50% reduction to the release rate was considered for this area.

Based on the above, there will be no surface ponding in the 2-year event.

#### 4.5.4 100 year + 20% Stress Test

A cursory review of the 100yr event + 20% has been performed using the modified rational method. The Peak flow from each area during a 100-year event has been increased by 20%. The calculations have been included in **Appendix D**.

A summary of the require storage volumes, and overflow balances is provided below.

DRAINAGE AREA	ICD RESTRICTED FLOW (L/s)	100yr20 STORAGE REQUIRED (m <sup>3</sup> )	SURFACE STORAGE PROVIDED (m <sup>3</sup> )	100yr20 OVERFLOW (m <sup>3</sup> )
SC#3	144.00	183.08	140	43.08
<b>TOTAL</b>	<b>144</b>			<b>43.08</b>

The stress test overflow from SC#3 will follow the intended overflow route as identified in the Phase 2B grading design drawings. The volume of overflow is 43.08m<sup>3</sup>. Based on the Tc of 19minutes, this volume can be reverse calculated to 37.79 L/s.

## 5 LOW IMPACT DEVELOPMENT

### 5.1 Introduction

Aquafor Beech was retained by Arcadis on behalf of WestUrban Developments Ltd. to complete the design of an infiltration-based Stormwater Management (SWM) facility in support of the development at 1050 Tawadina Road, Ottawa. The facility is to serve as an integral part of the site's ability to achieve erosion control, water balance, and water quality targets in accordance with the Stormwater Management Existing Conditions Report & LID Pilot Project Scoping (Aquafor Beech (2015)).

The site is encompassed by Tawadina Road to the North, Michael Stoqua Street to the East, a future development and Hemlock Road to the South, and Rue Bareille-Snow Street to the West. Presently, the site is vacant and located on the former CFB Rockcliffe air base site. The surrounding roads and underground services for the site have been constructed. The site has been zoned for a Mid-Rise Mixed Use.

The proposed development block consists of two 9-storey residential buildings with one level of underground parkade. The buildings located northwest and southeast are labelled as Building A and Building B respectively. The site also features a central plaza area with a small amenity building, and a small surface parking lot with access from Rue Bareille-Snow Street.

### 5.2 Background Information

A review of both existing site conditions and relevant design standards was completed to support the development of the infiltration facility. The following subsections outline relevant information from both review exercises.

#### 5.2.1 Relevant Design Standards

The following design standards were referenced in the design development process for the proposed infiltration facility:

1. City of Ottawa Sewer Design Guidelines (Second Edition, October 2012)
2. Stormwater Management Planning and Design Manual (Ministry of Environment, Conservation, and Parks, March 2003)
3. City of Ottawa Low Impact Development (LID) Technical Guidance Report: Implementation in Areas with Potential Hydrogeological Constraints (February, 2021)
4. Low Impact Development Stormwater Management Guidance Manual – Draft for Consultation (Ministry of Environment, Conservation, and Parks, January 2022)
5. Low Impact Development Stormwater Management Practice Inspection and Maintenance Guide – Version 1.0 (Toronto Region Conservation Authority, 2016)

#### 5.2.2 Subsurface Conditions

Two onsite investigations were completed within the 1050 Tawadina Road development block area:

1. Geotechnical Investigation: Proposed Two New Apartment Buildings – 1050 Tawadina Road, Ottawa, ON (Englobe, November 2022); and,
2. Permeability Testing and Monitoring Well Installations – 1050 Tawadina Road, Ottawa (McIntosh Perry, August 2023)

The Geotechnical Investigation was completed in 2022 by Englobe, involving installation of three boreholes and one monitoring well across the site. These features were used to classify subsurface soil physical and chemical properties, groundwater depth, and bedrock conditions. With this information, a number of design recommendations were developed including but not limited to subgrade preparation, engineered shoring, temporary dewatering, and foundation design.

In-situ infiltration testing was completed at a number of test pits and holes to various depths across the site in the summer of 2023. Testing was completed using a Guelph permeameter. Each test consisted of a 5-15cm head test, based on the level of saturation and subsurface materials encountered at the test location. Changes in reservoir water levels were monitored and recorded over time until a steady state was reached between three consecutive readings.

The relevant findings from both investigations in regards to design of the infiltration facility are outlined below:

1. Infiltration Facility Setbacks
  - a. Infiltration and any other LID practices must be located on site such that a minimum horizontal setback of 2.0m is provided between the LID footprint and edge of building foundations.
2. Bedrock and Groundwater
  - a. Bedrock elevation in the approximate infiltration facility excavation area was observed at 87.7m per data collected at BH22-2, or a depth of approximately 2.1m below finished design grade. No groundwater was observed in the monitoring well adjacent to the excavation area (MW22-4) during the single reading on June 3<sup>rd</sup>, 2022, thus groundwater is not expected to restrict design depth of the facility.
3. Infiltration Rate
  - a. In-situ infiltration rates in test pits or cores dug to a 1m depth (TP1 and TP4), approximately the depth of the infiltration facility invert, averaged to 17.3mm/hr. The design infiltration rate adopts a safety factor in accordance with the LID Stormwater Management Guidance Manual, producing an average design infiltration rate of 4.96mm/hr.

## 5.3 Infiltration Facility Sizing

The following subsections outline the design development process used in sizing the infiltration facility.

### 5.3.1 Stormwater Management Design Targets

To aid in the development of the infiltration facility, several design targets were identified from the various guidance documents outlined in Section 5.2.2 above. Table 1 below summarizes the design targets applied and source of information.



Table 1: Various Design Targets Applicable to the Infiltration Facility.

Design Target Category	Target Value or Range	Source
<b>Clearance to bedrock or groundwater</b>	Minimum 1.0m	City of Ottawa LID Technical Guidance Report: Implementation in Areas with Potential Hydrogeological Constraints
<b>Erosion Control Storage</b>	4mm rainfall depth across entire site impervious area	Wateridge Phase 2B LID Developer's Checklist
<b>Water Balance Storage</b>	4mm rainfall depth across entire site impervious area	Wateridge Phase 2B LID Developer's Checklist
<b>Water Quality Storage</b>	15mm rainfall depth across entire site impervious area	Wateridge Phase 2B LID Developer's Checklist
<b>Drawdown Time</b>	48-92 hours	City of Ottawa LID Technical Guidance Report: Implementation in Areas with Potential Hydrogeological Constraints
<b>Average Release Rate from Site</b>	Maximum 50% of the peak allowable rate (97.5 L/s)	City of Ottawa Pre-Application Consultation Meeting (July 21, 2022)

### 5.3.2 Proposed Hydrologic Conditions

Intensity-duration-frequency (IDF) data was referenced from the City of Ottawa Sewer Design Guidelines, adopting rainfall intensities for the 2-year to 100-year design storm event under a 10-minute time of concentration. Given that the infiltration facility has been designed to only accept inflows from rooftop areas, catchment area was delineated based upon total combined rooftop area from Building 'A' and Building 'B', with a standard impervious surface runoff coefficient of 0.9 adopted for the hydrological analysis. Additionally, the Draft LID SWM Guidance Manual was referenced to identify the recommended Runoff Volume Control Target for achieving Level 1 or 80% annual total suspended sediment (TSS) removal. Table 2 through Table 4 below summarizes the catchment characteristics, peak design storm flows, and required runoff storage volumes relevant to the design.

Table 2: Site Runoff Coefficient Calculation.

Site Runoff Coefficient		
Site Area (ha)		0.72
	RC*	Area (ha)
<b>Pavement/Concrete</b>	0.9	0.17
<b>Building</b>	0.9	0.34
<b>Landscaping</b>	0.25	0.16
<b>Pavers</b>	0.9	0.05
<b>Total</b>	<b>0.76</b>	<b>0.72</b>

Table 3: Design Storm Peak Flows from Building Rooftops.

Return Period	Rainfall Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	
		Building A	Building B
2-year	77.1	0.04	0.03
5-year	104.4	0.05	0.04
10-year	122.5	0.061	0.044
25-year	145.3	0.08	0.06
50-year	162.2	0.10	0.07
100-year	179	0.11	0.08

Table 4: Runoff Volume Storage Requirements for Site.

SWM Category	Target Value	Required Volume (m <sup>3</sup> )
Erosion Control	4mm rainfall depth across entire site impervious area	22m <sup>3</sup>
Water Balance Storage	4mm rainfall depth across entire site impervious area	22m <sup>3</sup>
Water Quality Storage	15mm rainfall depth across entire site impervious area	83m <sup>3</sup>

To achieve all three stormwater management category targets, the infiltration facility was thus designed to ensure 83m<sup>3</sup> of storage is provided.

### 5.3.3 Infiltration Facility Summary

With design targets and site constraints established, a design for the infiltration facility was developed. The facility consists of a plastic chamber system complete with inlet debris settling rows, inspection ports, inlet and outlet connections, and an open bottom stone base for infiltration of stored water below the outlet invert. A summary of key design information for the infiltration facility is provided in Table 5 below.

Table 5: Key Design Parameters of Proposed Infiltration Facility.

Design Parameter	Value
Maximum Storage Volume (m <sup>3</sup> )	83m <sup>3</sup>
Excavation Footprint Area (m <sup>2</sup> )	165m <sup>2</sup>
Total Facility Depth (m)	0.81m
Minimum Cover (m)	0.6m
Minimum Clearance to Bedrock (m)	1.0m
Drawdown Time (hrs)	61hrs*
Inlet Pipe Diameter(s) (mm)	250mm – x2
Outlet Pipe Diameter (mm)	150mm
Structural Loading Capacity	HS-25 Rated

\*Note: drawdown time based off water level reduction from outlet pipe invert to bottom of levelling course 19mm stone.

In addition to the design information in the above table, various other design aspects were incorporated to enhance the function of the system and allow for greater ease of operation and maintenance. These additional design aspects are outlined and described below:

1. Overflow bypass system
  - a. Two standard OPSD 705.010 catchbasins are proposed to be installed along the inlet pipes from each building such that in major storm events when the infiltration facility has reached maximum capacity, overflow can exit the system and drain to CB1 or overland through the site entrance onto Rue Bareille-Snow Street. Additionally, the catchbasins allow for bypass should the infiltration facility inlets or outlet become blocked.
2. Inlet Debris Rows
  - a. Inlet debris rows are included at each inlet location as part of the Aquabox Cube infiltration chamber design such that sediment and other fine debris has the opportunity to settle in a small forebay area before runoff spills over the internal weir wall and into the main chamber area. The debris rows concentrate sediments entering the system to a small area for ease of maintenance.
3. Inspection Ports
  - a. Three inspection ports are provided in the design featuring 375mm diameter riser pipes. These ports can be used for visual inspection inside the chamber or cleanout of sediments via vac truck.

## 5.4 Operation and Maintenance Considerations

A number of operation and maintenance (O&M) practices should be considered by the site owner to ensure the infiltration facility can maintain its as-designed function in future years. The following considerations are summarized from previous industry experience of Aquafor Beech and the TRCAs' Low Impact Development Stormwater Management Practice Inspection and Maintenance Guide.

Design Component	O & M Description	Frequency
<b>Contributing Catchment</b>	Inspect Contributing rooftop area and paved surfaces near inlet CB2 and CB2 to ensure no significant leaf litter, sediment, leaking contaminated substances, or other garbage debris may enter the system and cause partial or full blockage of the inlet system.	Biannual visual inspections.
<b>Inlet Conveyance System</b>	Inlets should remain unobstructed to ensure runoff enters infiltration facility unimpeded. Visual inspection of inlet catchbasins CB2 and CB3 should be completed. CCTV and flushing of pipe segments should occur when pipe segments are or suspected to be clogged.	Visual Inspection – biannual Flushing & CCTV – when clogging/damage suspected.
<b>Debris Row/Pretreatment</b>	For effective debris row function, these areas should be inspected visually via the inspection ports for sediment or other debris accumulation limiting storage capacity or conveyance of inlet flows into the main chamber area. Inlet flushing and vac truck cleanout of the debris row shall be adopted to remove debris and sediment when required.	Biannual visual inspections. Flushing & Vac Truck – when sediment accumulation reaches half the height of the debris row geotextile wall.
<b>Main Filter Bed Area</b>	Visual inspection in dry weather to quantify sediment accumulation and inspections following storm events to monitor draw down time. Should facility draw down exceed 92 hours or sediment accumulation limit inlet/outlet function of facility, flushing and vac truck sediment removal shall be adopted.	Annual visual inspections. Flushing & Vac Truck – when drawdown exceeds 92hrs <b>OR</b> sediment accumulation impeding inlet/outlet function.
<b>Outlet Conveyance System</b>	Outlet should remain unobstructed to ensure discharged water enters underground cistern unimpeded. Visual inspection of outlet catchbasins CB1 and monitoring of Cistern water levels can help identify any conveyance problems in the outlet system. Where clogging is suspected, CCTV and flushing of pipe sediments should occur.	Visual Inspection – biannual Flushing & CCTV – when clogging/damage suspected.
<b>Emergency Overflow Outlets</b>	Grate openings of CB2 and CB3 along inlet pipes should remain unobstructed and free of debris such that surcharge of excess runoff to the surface in major storm events can occur.	Biannual visual inspections.
<b>Inspection Ports</b>	As a vital component to maintenance access, inspection of the inspection ports to ensure proper function and access is maintained via the surface grates.	Biannual access function inspections.

## 6 SEDIMENT AND EROSION CONTROL PLAN

### 6.1 General

During construction, existing stream and conveyance systems can be exposed to significant sediment loadings. Although construction is only a temporary situation, it is proposed to introduce a number of mitigative construction techniques to reduce unnecessary construction sediment loadings. These will include:

- groundwater in trench will be pumped into a filter mechanism prior to release to the environment;
- bulkhead barriers will be installed at the nearest downstream manhole in each sewer which connects to an existing downstream sewer;
- seepage barriers will be constructed in any temporary drainage ditches; and
- silt sacks will remain on open surface structure such as manholes and catchbasins until these structures are commissioned and put into use.

### 6.2 Trench Dewatering

During construction of municipal services, any trench dewatering using pumps will be discharged into a filter trap made up of geotextile filters and straw bales similar in design to the OPSD 219.240 Dewatering Trap. These will be constructed in a bowl shape with the fabric forming the bottom and the straw bales forming the sides. Any pumped groundwater will be filtered prior to release to the existing surface runoff. The contractor will inspect and maintain the filters as needed including sediment removal and disposal and material replacement as needed.

### 6.3 Bulkhead Barriers

At the first manhole constructed immediately upstream of an existing sewer, a ½ diameter bulkhead will be constructed over the lower half of the outletting sewer. This bulkhead will trap any sediment carrying flows, thus preventing any construction –related contamination of existing sewers. The bulkheads will be inspected and maintained including periodic sediment removal as needed.

### 6.4 Seepage Barriers

These barriers will consist of both the Light Duty Straw Bale Barrier as per OPSD 219.100 or the Light Duty Silt Fence Barrier as per OPSD 219.110 and will be installed in accordance with the sediment and erosion control drawing. The barriers are typically made of layers of straw bales or geotextile fabric staked in place. All seepage barriers will be inspected and maintained as needed.

### 6.5 Surface Structure Filters

All catchbasins, and to a lesser degree manholes, convey surface water to sewers. However, until the surrounding surface has been completed these structures will be covered to prevent sediment from entering the minor storm sewer system. Until rear yards are sodded or until streets are asphalted and curbed, all catchbasins and manholes will be equipped with geotextile filter socks. These will stay in place and be maintained during construction and build until it is appropriate to remove them.

## 7 APPROVALS AND PERMIT REQUIREMENTS

### 7.1 City of Ottawa

The City of Ottawa reviews all development documents including this report and working drawings. Upon completion, the City will approve the local watermains, submit the sewer ECA application to the province, and eventually issue a Commence Work Notification.

### 7.2 Province of Ontario

The Ministry of Environment, Conservation and Parks (MECP) Environmental Compliance Approval is not required for the subject development. A Permit To Take Water for the subject site has been provided by the MECP. The permit, number 0565-A5AMP8, expires on December 31, 2025.

### 7.3 Conservation Authority

Since no watercourses are impacted by the proposed development, no permits will be required from the local Conservation Authority (Rideau Valley Conservation Authority).

### 7.4 Federal Government

There are no federal permits, authorizations or approvals needed for this development.

## 8 CONCLUSIONS & RECOMMENDATIONS

### 8.1 Conclusions

This report and the accompanying working drawings clearly indicate that the proposed development meets the requirements of the stakeholder regulators, including the City of Ottawa, provincial MECP and SNC. The proposed development is also in general conformance with the Master Servicing Study completed by IBI dated June 2020.

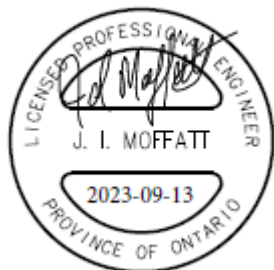
Downstream sanitary and storm sewers were designed with the proposed development area included. There is a reliable water supply available adjacent to the proposed development.

### 8.2 Recommendations

It is recommended that the regulators review this submission with an aim of providing the requisite approvals to permit the owners to proceed to the construction stage of the subject site.

Report prepared by:

**ARCADIS/IBI GROUP**



James I. Moffatt, P.Eng.  
Associate



Anton Chettrar, P. Eng.  
Civil Engineer

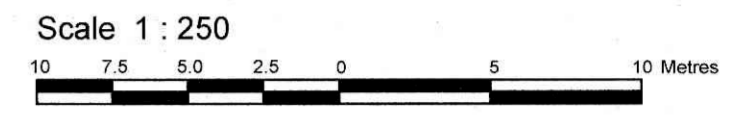
# APPENDIX A

AOV Part of Block 11 Registered Plan 4M-1651  
Site Plan for 1050 Tawadina Road  
142609-001 – Site Servicing Plan  
City of Ottawa Pre-Consultation Meeting Notes  
Assessment of Revised Block 11 and 12 Storm and Sanitary  
Servicing  
McIntosh Perry – Permeability Testing and Monitoring Well  
Installations – 1050 Tawadina Road, Ottawa  
Development Servicing Study Checklist



PART OF BLOCK 11  
REGISTERED PLAN 4M-1651  
CITY OF OTTAWA

Surveyed by Annis, O'Sullivan, Vollebakk Ltd.



Metric  
DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND  
CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

Surveyor's Certificate

I CERTIFY THAT:  
1. This survey and plan are correct and in accordance with the Surveys Act, the  
Surveyors Act and the regulations made under them.  
2. The survey was completed on the 5th day of April, 2022.

April 13, 2022  
Date

*A.J. Broxham*  
Andrew J. Broxham  
Ontario Land Surveyor

ASSOCIATION OF ONTARIO  
LAND SURVEYORS  
PLAN SUBMISSION FORM  
V-25453  
THIS PLAN IS NOT VALID UNLESS  
IT IS AN EMBOSSED ORIGINAL  
COPY ISSUED BY THE SURVEYOR  
In accordance with  
Regulation 1026, Section 29 (3).

Notes & Legend

Symbol	Denotes
□	Survey Monument Planted
■	Survey Monument Found
SIB	Standard Iron Bar
SSIB	Short Standard Iron Bar
IB	Iron Bar
(WIT)	Witness
Meas.	Measured
(AOG)	Annis, O'Sullivan, Vollebakk Ltd.
(PI)	Plan 4R-34408
○ FH	Fire Hydrant
○ LS	Light Standard
○ WV	Water Valve
○ M+ST	Maintenance Hole (Storm Sewer)
○ M+S	Maintenance Hole (Sanitary)
○ M+H	Maintenance Hole (Hydro)
○ M	Maintenance Hole (Unidentified)
□ CB	Catch Basin
T/G	Top of Grade
— P —	Underground Power
— G —	Underground Gas
— — —	Underground Shared Conduit
B/C	Shared Bell and Cable
B/C/G	Shared Bell, Cable and Gas
B/C/P	Shared Bell, Cable and Power
H-T	Hydro Transformer Pad
+ 65.00	Location of Elevations

Bearings are grid, derived from the westerly limit of rue Michael Stouqua Street, shown to be N0°00'00"E on Plan 4R-34408, and are referred to the central meridian of MTM Zone 9 (76°30' West Longitude) NAD-83 (original).

Dimensions illustrated hereon are consistent with Plan 4R-34408 unless otherwise noted. All found survey monuments are (AOG) unless otherwise noted.

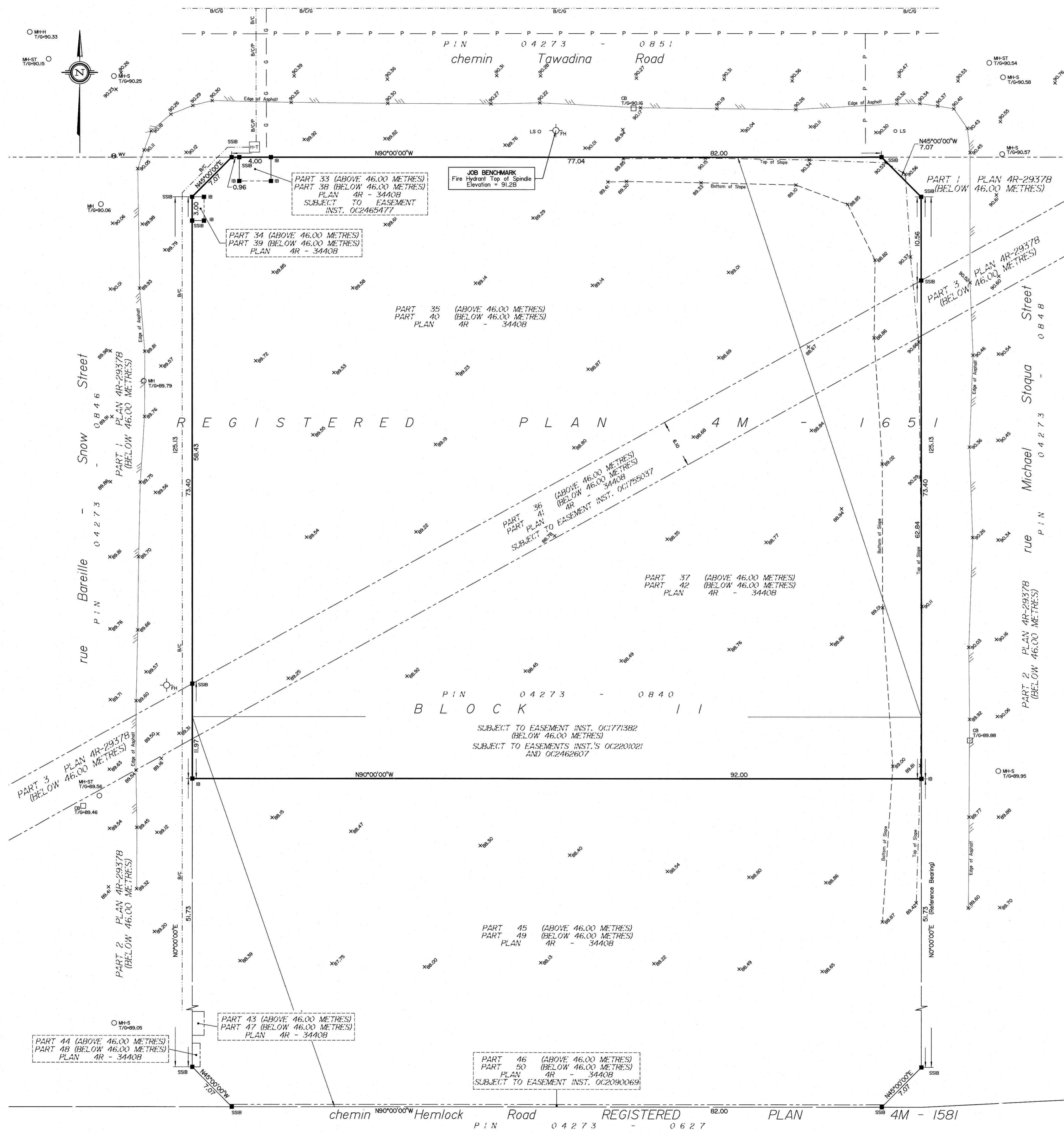
ELEVATION NOTES

- Elevations shown are geodetic and are referred to City of Ottawa Vertical Benchmark No. 396 (01919680138), having an elevation of 95.06 metres (CGVD28 geodetic datum).
- It is the responsibility of the user of this information to verify that the job benchmark has not been altered or disturbed and that its relative elevation and description agrees with the information shown on this drawing.

UTILITY NOTES

- This drawing cannot be accepted as acknowledging all of the utilities and it will be the responsibility of the user to contact the respective utility authorities for confirmation.
- Only visible surface utilities were located.
- A field location of underground plant by the pertinent utility authority is mandatory before any work involving breaking ground, probing, excavating etc.
- Location of underground services derived from composite utility plan drawing No. 301 (IB) project 118863, Revision 7 dated April 11, 2021.

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Email: Nepean@avolt.com  
Job No. 22726-22, West Urban 5811 4M-1651 POS DI | SW



PART 33 (ABOVE 46.00 METRES)  
PART 38 (BELOW 46.00 METRES)  
PLAN 4R - 34408  
SUBJECT TO EASEMENT  
INST. OC2465477

PART 34 (ABOVE 46.00 METRES)  
PART 39 (BELOW 46.00 METRES)  
PLAN 4R - 34408

PART 35 (ABOVE 46.00 METRES)  
PART 40 (BELOW 46.00 METRES)  
PLAN 4R - 34408

PART 36 (ABOVE 46.00 METRES)  
PART 41 (BELOW 46.00 METRES)  
PLAN 4R - 34408  
SUBJECT TO EASEMENT INST. OC1755037

PART 37 (ABOVE 46.00 METRES)  
PART 42 (BELOW 46.00 METRES)  
PLAN 4R - 34408

SUBJECT TO EASEMENT INST. OC1771382  
(BELOW 46.00 METRES)  
SUBJECT TO EASEMENTS INST.'S OC2201021  
AND OC2462607

PART 45 (ABOVE 46.00 METRES)  
PART 49 (BELOW 46.00 METRES)  
PLAN 4R - 34408

PART 44 (ABOVE 46.00 METRES)  
PART 48 (BELOW 46.00 METRES)  
PLAN 4R - 34408

PART 43 (ABOVE 46.00 METRES)  
PART 47 (BELOW 46.00 METRES)  
PLAN 4R - 34408

PART 46 (ABOVE 46.00 METRES)  
PART 50 (BELOW 46.00 METRES)  
PLAN 4R - 34408  
SUBJECT TO EASEMENT INST. OC2090069

REGISTERED PLAN 4M - 1581  
chemin Hemlock Road  
PIN 04273 - 0627

LOCATION PLAN



GENERAL NOTES

- ALL EXISTING STRUCTURES, RETAINING WALLS AND LANDSCAPING TO BE REMOVED WITHIN COMBINED DEVELOPMENT PARCELS.
- REFER TO LANDSCAPE PLANS FOR ALL PLANTING AND GROUND COVER INFORMATION & DETAILS.
- REFER TO WATERIDGE VILLAGE AT ROCKCLIFFE PHASE 2B ISSUED FOR CONSTRUCTION DRAWINGS AS PREPARED BY IB GROUP 2019.05.10 FOR ALL DESIGN GEOMETRIC ELEVATIONS ADJACENT TO DEVELOPMENT PERIMETER.
- ALL EXISTING SITE INFORMATION AS PER TOPOGRAPHICAL SURVEY PLAN DATED APRIL 5th, 2022 PREPARED BY ANNIS, O'SULLIVAN, VOLLEBEKK LTD.
- ALL SITE REHABILITATION OF SIDEWALKS, BUS ZONE APRONS, AND PAVED LANES ARE TO BE COMPLETED AT THE OWNER'S EXPENSE.
- ANY SNOW ACCUMULATED IN SURFACE PARKING AREAS IS TO BE TRUCKED OFF-SITE.
- WASTE & RECYCLING BINS TO BE ROLLED OUT TO BARRIELE-SNOW STREET FOR CURBSIDE COLLECTION.

SHEET NOTES

- N.01** PROPOSED ELECTRICAL TRANSFORMER LOCATION
- N.02** PARKADE ENTRY RAMP
- N.03** MAIN BUILDING ENTRY
- N.04** GARBAGE AND RECYCLING ACCESS
- N.05** FIRE DEPARTMENT CONNECTION
- N.06** TRANSFORMER
- N.07** BIKE PARKING STALLS
- N.08** VISITOR PARKING STALLS
- N.09** DEPRESSED CURB

ZONING NOTES

CURRENT ZONING: GM1 H(30)

REQUIRED	PROPOSED
5m	5m
5m	5m
5m	5m
3m	5m
30m	29.5m
2	9
750sm	677sm-742sm
20,171sm	254 UNITS

BUILDING A - DEVELOPMENT STATS		BUILDING B - DEVELOPMENT STATS	
NUMBER OF STOREYS	TOTAL UNITS	NUMBER OF STOREYS	TOTAL UNITS
9	144	9	110
FLOOR	GROSS AREA	FLOOR	GROSS AREA
MAIN	1,922sm	MAIN	1,423sm
2 FLR	1,977sm	2 FLR	1,331sm
3 FLR	1,977sm	3 FLR	1,331sm
4 FLR	1,201sm	4 FLR	900sm
5 FLR	1,105sm	5 FLR	900sm
6 FLR	1,105sm	6 FLR	742sm
7 FLR	677sm	7 FLR	742sm
8 FLR	677sm	8 FLR	742sm
9 FLR	677sm	9 FLR	742sm
<b>TOTAL</b>	<b>11,318sm</b>	<b>TOTAL</b>	<b>8,853sm</b>

**VEHICULAR PARKING**  
 MIN. 0.5 RESIDENTIAL STALLS/ RESIDENCE UNIT  
 - FIRST 12 SPACES/ BUILDING (254-4(2\*12))/0.5 = 115 REQUIRED

MAX. 40% COMPACT  
 MAX. 5% MOTORCYCLE  
 STANDARD  
**TOTAL RESIDENTIAL STALLS\*** = 170 PROPOSED\*

MIN. 0.1 VISITOR STALLS/ RESIDENCE UNIT  
 - FIRST 12 SPACES/ LOT (254-12)/0.1 = 25 REQUIRED  
**+25 PROPOSED\*\***

\*\* 5 Stalls provided at-grade and 20 in underground parking garage

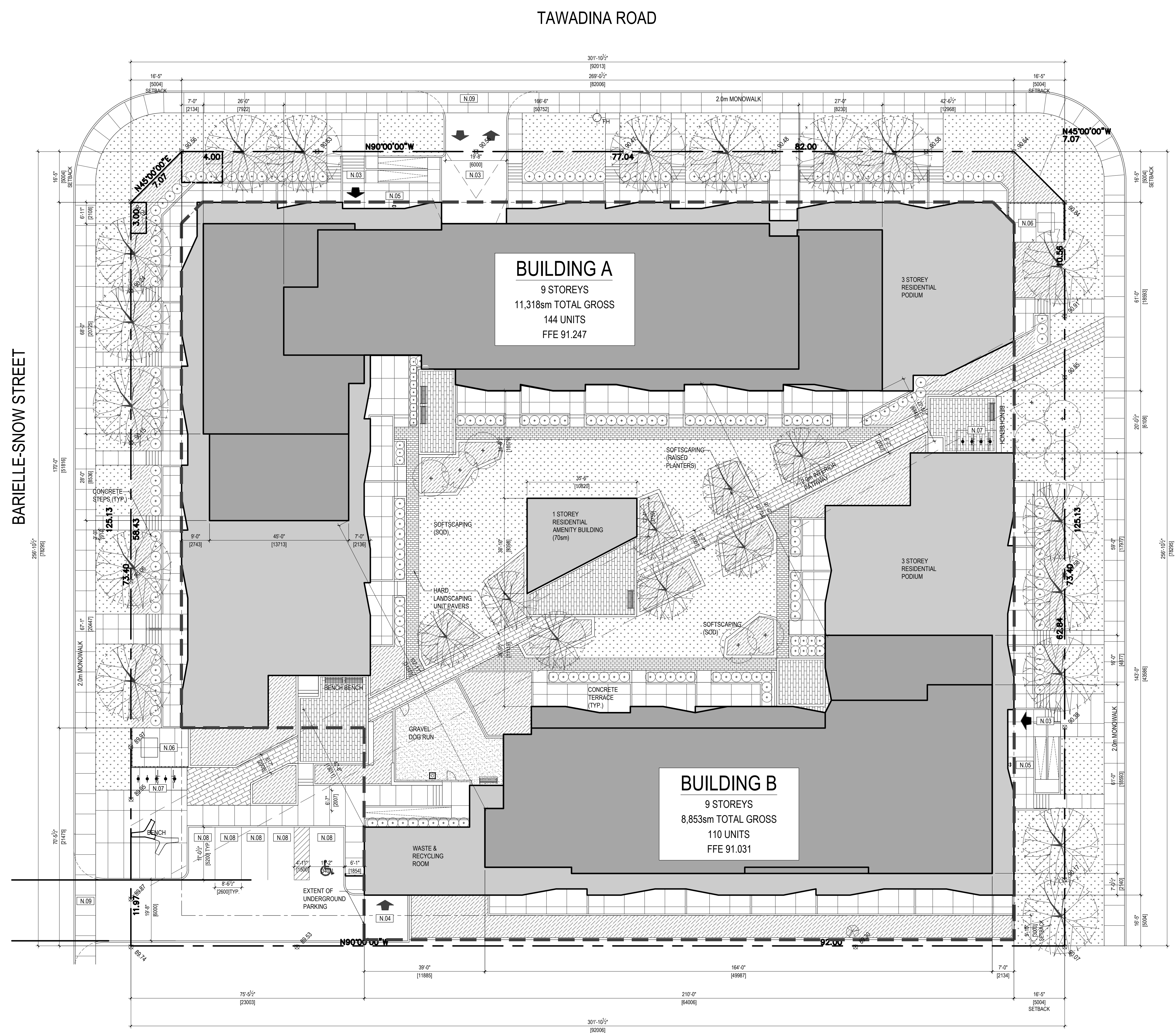
**TOTAL PARKING PROVIDED = 195 STALLS**

\* Note 5 of the 195 stalls are proposed as barrier free

**BICYCLE PARKING**  
 REQUIRED: UNDERGROUND  
 MIN. 0.5 STALLS/ RESIDENCE UNIT = 127 SPACES

PROPOSED:  
 UNDERGROUND = 138 SPACES  
 EXTERIOR = 18 SPACES  
**TOTAL = 156 SPACES**

**AMENITY SPACE REQUIREMENTS:**  
 REQUIRED AMENITY SPACE = 6sm/ RESIDENCE UNIT  
 254 UNITS x 6sm = 1524sm TOTAL AMENITY REQUIRED  
 MIN. 50% REQUIRED TO BE COMMUNAL = 762sm  
 PROVIDED COMMUNAL AMENITY SPACE = 1830sm  
 PROVIDED PRIVATE AMENITY = 654sm  
**TOTAL PROVIDED AMENITY SPACE = 2484sm**



MICHAEL STOQUA STREET

BARRIELE-SNOW STREET

TAWADINA ROAD

RELEASES

NO.	DESCRIPTION	DATE
02	ISSUED FOR SPC	02.28.23
01	ISSUED FOR CLC	10.24.22

1050 TAWADINA RD  
 WATERIDGE

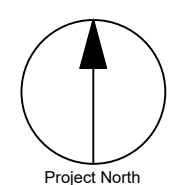
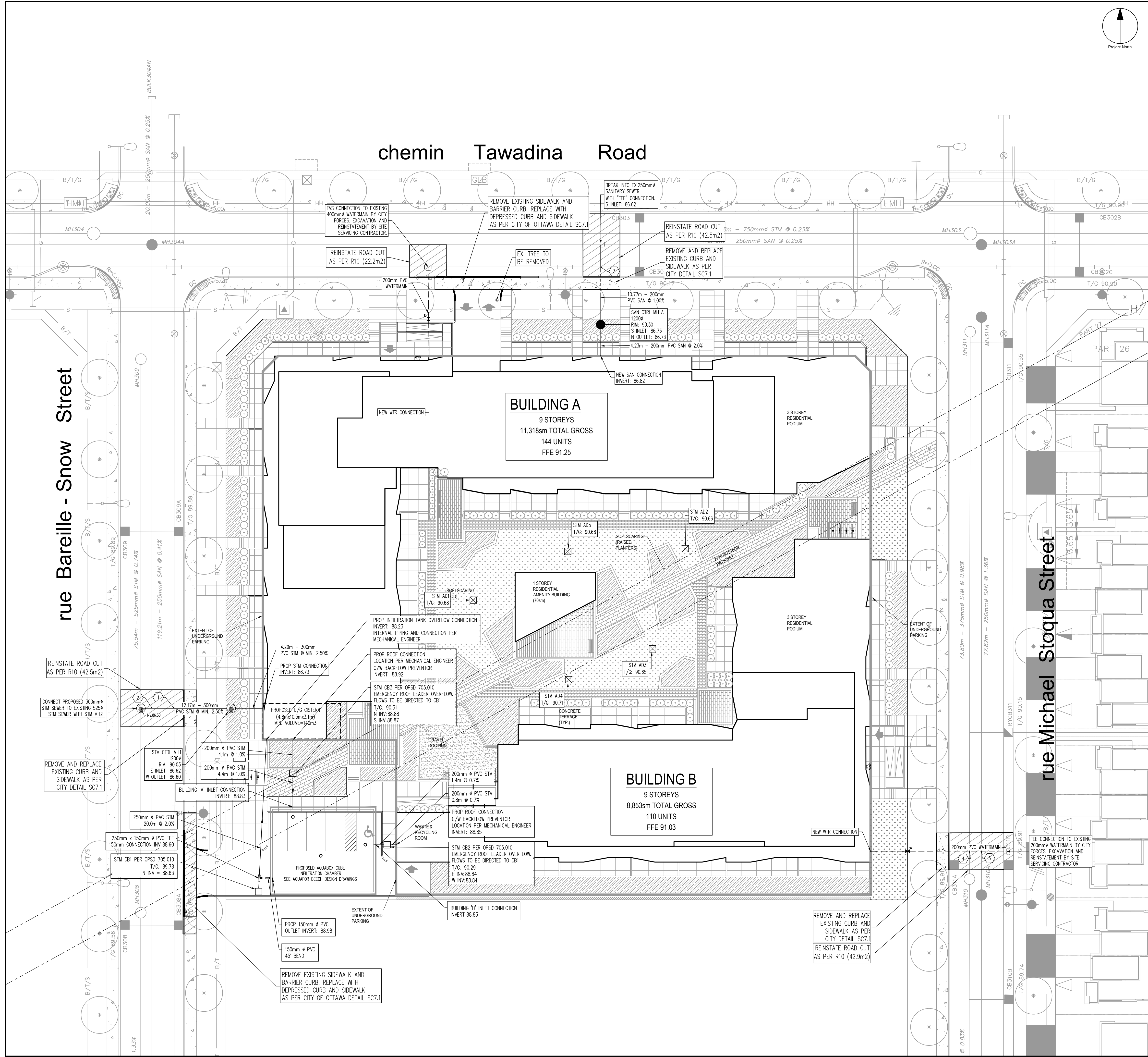
PROJECT NAME: 22.01.W.U.  
 DATE: 22.01.24  
 SCALE: AS NOTED

SITE PLAN

DRAWING NUMBER

SPC.100

THIS DRAWING AND DESIGN ARE AT ALL TIMES TO REMAIN THE EXCLUSIVE PROPERTY OF THE ARCHITECT AND MAY NOT BE USED OR REPRODUCED WITHOUT PRIOR WRITTEN CONSENT.



**LEGEND:**

	PROPOSED	EXISTING
STORM SEWER		
WATER LINE		
SANITARY SEWER		
PIPE FLOW DIRECTION		
STORM MANHOLE		
AREA DRAIN		
CATCH BASIN		
DOUBLE CATCH BASIN		
TRENCH DRAIN		
PIPE INSULATION		
SANITARY MANHOLE		
OBSERVATION PORT		
WATER HYDRANT		
WATER VALVE		
90° BEND		
45° BEND		
TEE FITTING		
PIPE CROSSING NUMBER		
BUILDING		
RETAINING WALL		
CONSTRUCTION LIMIT		
PROPERTY LINE		
R/W PLAN		

**LABEL LEGEND:**

T/G = TOP OF GRATE	EX. TRANSFORMER
CB = CATCH BASIN	EX. HYDRO MANHOLE
DCB = DOUBLE CATCH BASIN	EX. BELL PEDESTAL
MH = MANHOLE	EX. BELL GRADE LEVEL BOX
AD = AREA DRAIN	EX. ROGERS PEDESTAL
	EX. STREET LIGHT
	EX. STREET LIGHT GROUNDING
	EX. JOINT UTILITY TRENCH
	EX. HYDRO CABLE AND DUCTS
	EX. BELL CABLE
	EX. ROGERS CABLE
	EX. GAS
	EX. STREET LIGHT CABLE
	EX. UTILITY DROP LOCATIONS
	EX. CONCRETE ENCASED DUCT BANK
	EX. TREE LOCATION

- NOTES:**
- IT IS THE RESPONSIBILITY OF THE APPLICANT/CONSULTANT TO ENSURE COMPLIANCE WITH ALL APPLICABLE PROVINCIAL STANDARDS AND TO OBTAIN ALL PROVINCIAL APPROVALS, INCLUDING BUT NOT LIMITED ENVIRONMENTAL COMPLIANCE APPROVALS.
  - THE CONTRACTOR SHALL ENSURE ALL MATERIAL AND CONSTRUCTION IS IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARD DRAWINGS AND SPECIFICATIONS. DRAWINGS AND SPECIFICATIONS FROM OPSS & OPSD, SHALL BE USED WHERE THERE IS NO EQUIVALENT FROM CITY STANDARDS.
  - THE CONTRACTOR SHALL VERIFY ALL SURVEY STAKE LAYOUT POINTS AND DIMENSIONS AND ANY DISCREPANCIES SHALL BE REPORTED TO THE CONSULTANT PRIOR TO CONSTRUCTION. ANY DISCREPANCIES ARE TO BE IMMEDIATELY REPORTED TO THE CONSULTANT.
  - ALL EXISTING UTILITIES SHOWN ON DRAWINGS (PLAN AND PROFILE) ARE FOR REFERENCE PURPOSES ONLY. THE CONTRACTOR SHALL SATISFY THEMSELVES AS TO THE ACTUAL LOCATION AND DEPTH OF ANY UTILITY AND SHALL BE LIABLE FOR ALL OR ANY DAMAGES.
  - THE POSITION OF ALL STATIC STRUCTURES, SMALL UTILITIES, DEEP UTILITIES, AND ANY OTHER ABOVEGROUND OR UNDERGROUND UTILITIES STRUCTURES AND APPURTENANCES SHALL BE FIELD VERIFIED BY THE CONTRACTOR TO SATISFY HIMSELF AND SHALL ASSUME LIABILITY FOR ANY DAMAGES TO THEM DURING CONSTRUCTION. ANY DAMAGES ARE TO BE RECTIFIED TO THE SATISFACTION OF THE CITY INSPECTOR OR FRANCHISE UTILITY OWNERS.
  - REFER TO THE PRELIMINARY GEOTECHNICAL INVESTIGATION REPORT COMPLETED BY ENGBLOE ON OCTOBER 11TH, 2022 FOR GEOTECHNICAL INFORMATION.
  - ALL SEWER AND WATERMAIN PIPES SHALL BE INSTALLED AT A MINIMUM FROST COVER OF 2.4m.
  - ALL WATERMAIN CONNECTIONS WITHIN MUNICIPAL R.O.W.'S TO BE INSTALLED BY CITY FORCES AT THE OWNER'S EXPENSE.

**Pipe Clearance Table**

Crossing No.	Bottom of Pipe	Top of Pipe	Clearance
1	87.50 WTR	87.00 STM	0.50
2	86.62 STM	85.24 SAN	1.38
3	87.40 WTR	86.87 SAN	0.53
4	87.40 WTR	86.87 STM	0.53
5	87.40 WTR	85.77 SAN	0.63

CLIENT

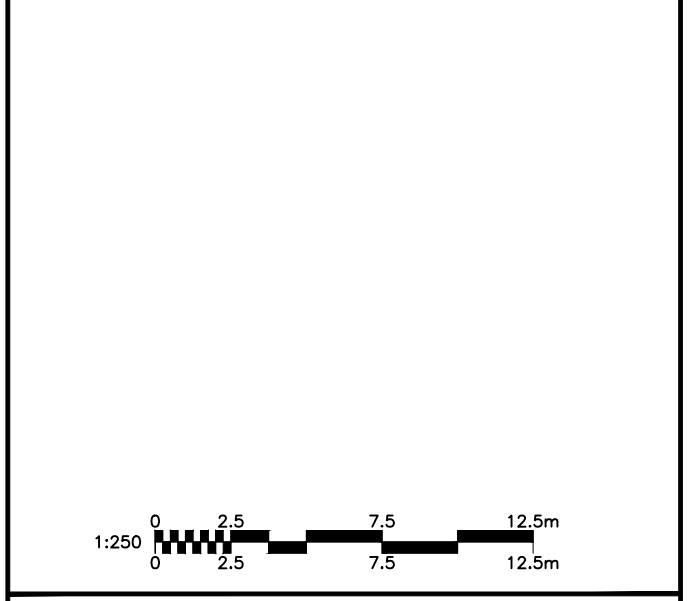
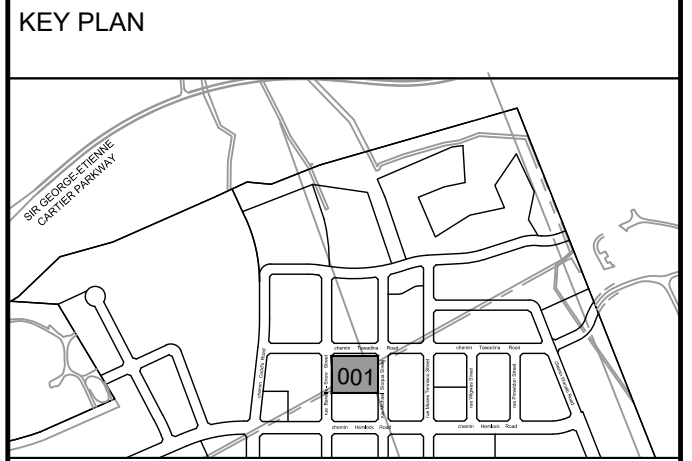
**WestUrban**  
Developments Ltd.

**COPYRIGHT**

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**ISSUES**

No.	DESCRIPTION	DATE
1	ISSUED FOR CLC SUBMISSION	2023-01-11
2	SUBMISSION NO 2 FOR CITY REVIEW	2023-09-13
3		
4		
5		
6		
7		
8		



**SEAL**

**ARCADIS**

333 Preston Street - Suite 500  
Ottawa ON K1S 5N4 Canada  
tel 613 225 1311  
www.arcadis.com

**PROJECT**

1050 Tawadina Road

WATERIDGE VILLAGE PHASE 2

**PROJECT NO:** 142609

**DRAWN BY:** M.M.

**PROJECT MGR:** S.L.

**CHECKED BY:** J.I.M.

**APPROVED BY:** A.C.

**SHEET TITLE**

SITE SERVICING PLAN

**SHEET NUMBER** C-001

**ISSUE** 2

**Pre-Application Consultation Meeting Notes**

**1050 Tawadina Road**

File Number: D07-01-22-0178

Thursday July 21, 2022, Microsoft Teams

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**Attendees:**

*City of Ottawa:*

Jean-Charles Renaud, File Lead  
Joyce Tshiyoyo, Student Planner  
Reza Bakhit, Project Manager  
Selma Hassan, Urban Design

*Applicant Team:*

Cameron Salisbury, WestUrban Developments Ltd. (Owner)  
Christine McGuaig, Q9 Planning + Design  
James Andalis, FAAS Architect  
Dorothy Poon, Design Works Engineering  
Courtney Clarke  
Matthew Fitzgerald  
Robert Pringle

*Community Association Representatives:*

Jane Thompson  
Roxanne Field

*Regret(s):*

Neetie Paudel, Transportation (City)

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**Subject: 1050 Tawadina Road**

**Meeting Notes:**

**Opening & attendee introduction**

- Introduction of meeting attendees

**Proposal Overview**

- Maintain a strong at grade development for the public
- Giving access to the public courtyard
- Balconies added to provide privacy and a sense of connection to the courtyard space
- This is intended to be a rental development
- Working with the team to design this

### Questions:

- JC: Is there any particular reason why this development is strictly residential?
  - Cameron: We did consider but looking at the area, which is already mixed-use, we decided to stick to residential
  - Christine: Yes, and the CDP allows for residential only, so this is no problem
- JC: What is the front yard setback?
  - Christine: We will add address component if needed
  - JC: This would be a corner lot – rear yard setback

### Preliminary Comments from Related Discipline:

#### Planning (JC)

- Southern property line
  - Build too close to the southern property line at just 0 metres. If that is to remain, that could be problematic with adjacency to the proposal of the other lot on the southern side.
  - You have units facing south and how they would interact with each other which might be problematic interaction.
- GM31 zone includes floorplate max for buildings over seven storeys
- Two buildings above four storeys need to be at least 23 metres apart **(including to buildings on other properties. Will need to accommodate for half that distance on your property)**
- Surface parking spaces are in a prominent location. Why is so much surface parking needed when parking minimum is exceeded by 60 spaces? Area could be better used as soft landscaping.
  - Response (Christine): We would like to make it easier and still have some spaces
  - JC: it would still be good to maybe decrease some spaces. Maybe add only surface parking spaces for accessibility and drop off but move the rest underground
- Trees: Must allow sufficient soil volumes, particularly those on top of the parking structure - at least a metre of soil depth
- A surface access easement would be required for the pedestrian connection.
- Please consider designing the site to allow a shared entrance with the future development to the south.
- When a site plan is filed, the applicant should show that their plan aligns with the CLC approved subdivision plan for street treatment. Please show this information on the site/landscape plans greyed out. Ensure adequate space for street trees.

#### Urban Design (Selma)

### UDRP, Design Brief and CDP Design Guidelines

- The site is a mixed use block in the Core Area and is required to go before the City's Urban Design Review Panel. The following link should take the applicant to the information page on the UDRP [Urban Design Review Panel | City of Ottawa](#). If they have any questions, they can contact the UDRP co-ordinator Sole Carvajal [sole.carvajal@ottawa.ca](mailto:sole.carvajal@ottawa.ca)
- A Design Brief is required with the application submission. The Terms of Reference for the Design Brief is attached. All items highlighted in yellow must be clearly addressed in written and / or graphic format as appropriate.

- The CDP includes a number of guidelines that are relevant to this site. There are at least four around the theme of maximum length of straight, continuous, building frontages (40m) and variations in setbacks to break up long facades. As presented, at eye level a pedestrian would be looking at very long, straight, solid brick facades. This does not meet the direction of the guidelines or contribute to animation of the street. The applicant is asked to reconsider this edge treatment.
- The CDP includes a guideline stating “Although the maximum building height in mid-rise mixed-use sections of the Core (blocks 31-33, 35-37) is 30 metres, at least half of the total land area of each of these blocks will have a maximum building height of 20 metres”. As presented, the proposal meets this guideline.

### **Zoning**

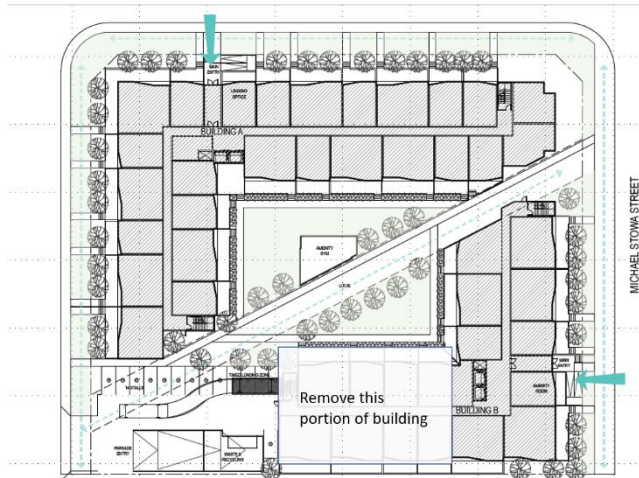
- Please provide drawings that dimension:
  - The setbacks from all property lines
  - The 23m separation distance after the 4<sup>th</sup> floor
  - The width of landscaped area / landscape buffers as noted in zoning
  - The depths of all projections into the ROW
- Please show and label the footprint of the underground parking garage
- For buildings over 20 storeys in height, zoning requires the maximum building area of each floor plate over 20 metres to be 750 square metres of gross floor area. On building A, the gross floor area of the 8<sup>th</sup> and 9<sup>th</sup> floor appear to be 1212.6m<sup>2</sup> and 839.6m<sup>2</sup>, respectively. The applicant is asked to re-examine their building design to address this zoning requirement.

### **Landscape**

- I will reserve the landscape comments until a landscape plan is provided. However, street tree planting is important, as is general planting on site. The landscape drawings need to show and detail that trees planted above the parking garage will have the soil volumes and growing conditions necessary to achieve optimal growth.

### **Built Form**

- It is suggested that the applicant simplify the overall architectural expression, in particular on the upper floors.
- As noted in point #3, the proposal presents a solid wall to the street on all facades, at eye level. This is not an acceptable interface to the public realm. The building needs to be closer to grade to reduce the expanse of this wall. The patio guard railing should be transparent or translucent and not a solid brick material.
- If the southern property line is considered in interior side yard, then the required setback is 3m. Given the 7 storey building height and the potential for the abutting site to build to 30m in height also with a 3m setback, then the 3m setback is not adequate despite what is permitted by the zoning. An optimal configuration for both sites would be a U-shaped development, so that the two sites together create a perimeter block development. It is suggested that the applicant remove the ‘southern wing’ of the building as shown.



### Transportation (Neetie)

- Reduced scope of TIA is accepted. Design review component should be included (already discussed with the transportation consultant). Additionally, Module 4.5- Transportation Demand Management should also be included.
- Post-Development Monitoring Plan (monitoring cut-through, transit shares and network constraints) was prepared as part of the TIA for Wateridge Phase 2A/2. The study shall commence one year after 80% occupancy of each phase of subdivision.
- Turning templates will be required for all accesses showing the largest vehicle to access the site (loading trucks, garbage etc.); required for internal movements and at all access (entering and exiting and going in both directions). Ensure they are no conflicts with the loading zone and surface parking.
- Internal walkways should be a minimum of 1.5m.
- Ensure the access is 3m away from the property line (measured at the highway line and at the curb line or edge of the roadway).
- Site triangles at the following locations on the final plan will be required:
  - Local Road to Local Road: 3 metre x 3 metres
- As the site proposed is residential, AODA legislation applies for all areas accessible to the public (i.e. **outdoor pathways**, parking, etc.).
  - Please consider using the City's Accessibility Design Standards, which provide a summary of AODA requirements. <https://ottawa.ca/en/city-hall/creating-equal-inclusive-and-diverse-city/accessibility-services/accessibility-design-standards-features#accessibility-design-standards>

### Civil Engineer (Reza)

#### **General:**

- It is the sole responsibility of the consultant to investigate the location of existing underground utilities in the proposed servicing area and submit a request for locates to avoid conflict(s). The location of existing utilities and services shall be documented on an **Existing Conditions Plan**.
- Reference documents for information purposes:
  - Ottawa Sewer Design Guidelines (October 2012)
  - Technical Bulletin PIEDTB-2016-01

- Technical Bulletins ISTB-2018-01, ISTB-2018-02 and ISTB-2018-03.
- Ottawa Design Guidelines - Water Distribution (2010)
- Technical Bulletin ISTB-2021-03
- Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
- City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
- City of Ottawa Environmental Noise Control Guidelines (January 2016)
- City of Ottawa Accessibility Design Standards (2012) (City recommends development be in accordance with these standards on private property)
- Ottawa Standard Tender Documents (latest version)
- Ontario Provincial Standards for Roads & Public Works (2013)
- Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at [InformationCentre@ottawa.ca](mailto:InformationCentre@ottawa.ca) or by phone at (613) 580-424 x.44455).

### **Stormwater Management Criteria and Information:**

- The subject site located in the new Water ridge development. Therefore, the designer need to follow the requirements of the master plan and control to the storm sewer level of serving using the design runoff coefficient for the site.
- The designer should make sure that the entrance to the parking garage is higher than the major system overflow. This should be discussed in the SWM report and reflect on the site grading plan.
- **Underground Storage:** Please note that the Modified Rational Method for storage computation in the Sewer Design Guidelines was originally intended to be used for above ground storage (i.e. parking lot) where the change in head over the orifice varied from 1.5 m to 1.2 m (assuming a 1.2 m deep CB and a max ponding depth of 0.3 m). This change in head was small and hence the release rate fluctuated little, therefore there was no need to use an average release rate.

When underground storage is used, the release rate fluctuates from a maximum peak flow based on maximum head down to a release rate of zero. This difference is large and has a significant impact on storage requirements. **We therefore require that an average release rate equal to 50% of the peak allowable rate shall be applied to estimate the required volume. Alternatively, the consultant may choose to use a submersible pump in the design to ensure a constant release rate.**

In the event that there is a disagreement from the designer regarding the required storage, The City will require that the designer demonstrate their rationale utilizing dynamic modelling, that will then be reviewed by City modellers in the Water Resources Group.

Please provide information on UG storage pipe. Provide required cover over pipe and details, chart of storage values, capacity etc. How will this pipe be cleaned of sediment and debris? Provide information on type of underground storage system including product name and model, number of chambers, chamber configuration, confirm invert of chamber system, top of chamber system, required cover over system and details, interior bottom slope (for self-cleansing), chart of storage values, length, width and height, capacity, entry ports (maintenance) etc.



Provide a cross section of underground chamber system showing invert and obvert/top, major and minor HWLs, top of ground, system volume provided during major and minor events. UG storage to provide actual 2- and 100-year event storage requirements.

In regard to all proposed UG storage, ground water levels (and in particular HGW levels) will need to be reviewed to ensure that the proposed system does not become surcharged and thereby ineffective.

Modeling can be provided to ensure capacity for both storm and sanitary sewers for the proposed development by City's Water Distribution Dept. – Modeling Group, through PM and upon request.

- Please note that the minimum orifice dia. for a plug style **ICD is 83mm and the minimum flow rate from a vortex ICD is 6 L/s** in order to reduce the likelihood of plugging.

**Storm Sewer:**

- A 525mm dia. CONC storm sewer (2020) is available within Bareille Snow street.
- A 750mm dia CONC storm sewer ( 2020) is available within Tawadina road
- A 375mm dia CONC storm sewer ( 2020) is available within Michael Stoqua Street.
- A 1200mm dia. CONC Storm sewer (2018) is available within Hemlock road.

**Sanitary Sewer:**

- A 250mm dia. PVC SAN sewer (2020) is available within Bareille Snow street.
- A 250mm dia PVC SAN sewer ( 2020) is available within Tawadina road
- A 250mm dia PVC SAN sewer ( 2020) is available within Michael Stoqua Street.
- A 250mm dia. PVC SAN sewer (2018) is available within Hemlock road

**Note: A 2400mm dia CONC SAN trunk sewer ( 1964) runs through the subject property. The City AMB will be circulated on all the submissions for their comments. A protection plans may be required to be submitted for the review. Please make sure to include building footprint plan in the submission and confirm all the proposed structures are outside the easement. The proposal should be satisfactory to the AMB and the maintenance team as well as the development review.**

- Please provide the new Sanitary sewer discharge and we confirm if sanitary sewer main has the capacity. An analysis and demonstration that there is sufficient/adequate residual capacity to accommodate any increase in wastewater flows in the receiving and downstream wastewater system is required to be provided. Needs to be demonstrated that there is adequate capacity to support any increase in wastewater flow.
- Please apply the wastewater design flow parameters *in Technical Bulletin PIEDTB-2018-01*.
- Sanitary sewer monitoring maintenance hole is required to be installed at the property line (on the private side of the property) as per City of Ottawa Sewer-Use By-Law 2003-514 (14) *Monitoring Devices*.
- A backwater valve is required on the sanitary service for protection.

**Water:**

- A 203mm dia. PVC watermain (2021) is available within Bareille Snow street.
- A 406mm dia PVC watermain ( 2021) is available within Tawadina road
- A 203mm dia PVC watermain ( 2021) is available within Michael Stoqua Street.
- A 305mm dia. PVC watermain (2018) is available within Hemlock road
  
- Existing residential service to be blanked at the main.
- **Water Supply Redundancy:** Residential buildings with a basic day demand greater than 50m<sup>3</sup>/day (0.57 L/s) are required to be connected to a minimum of two water services separated by an isolation valve to avoid a vulnerable service area as per the *Ottawa Design Guidelines - Water Distribution, WDG001, July 2010 Clause 4.3.1 Configuration*.
- Please **review Technical Bulletin ISTB-2018-0**, maximum fire flow hydrant capacity is provided in Section 3 Table 1 of Appendix I. A **hydrant coverage figure** shall be provided and **demonstrate there is adequate fire protection for the proposal**. Two or more public hydrants are anticipated to be required to handle fire flow.
- Boundary conditions are required to confirm that the require fire flows can be achieved as well as availability of the domestic water pressure on the City street in front of the development. Use Table 3-3 of the MOE Design Guidelines for Drinking-Water System to determine Maximum Day and Maximum Hour peaking factors for 0 to 500 persons and use Table 4.2 of the Ottawa Design Guidelines, Water Distribution for 501 to 3,000 persons. Please provide the following information to the City of Ottawa via email to request water distribution network boundary conditions for the subject site. Please note that once this information has been provided to the City of Ottawa it takes approximately 5-10 business days to receive boundary conditions.
  - Type of Development and Units
  - Site Address
  - A plan showing the proposed water service connection location.
  - **Average Daily Demand** (L/s)
  - **Maximum Daily Demand** (L/s)
  - **Peak Hour Demand** (L/s)
  - **Fire Flow** (L/min)

*[Fire flow demand requirements shall be based on **Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection 1999**]*

*[Fire flow demand requirements shall be based on ISTB-2021-03]*

*Note: The OBC method can be used if the fire demand for the private property is less than 9,000 L/min. If the OBC fire demand reaches 9000 L/min, then the FUS method is to be used.*

*Exposure separation distances shall be defined on a figure to support the FUS calculation and required fore flow (RFF).*

- **Hydrant capacity shall be assessed to demonstrate the RFF can be achieved.** Please identify which hydrants are being considered to meet the RFF on a fire hydrant coverage plan as part of the boundary conditions request.

**Snow Storage:**

- Any portion of the subject property which is intended to be used for permanent or temporary snow storage shall be as shown on the approved site plan and grading plan. Snow storage shall not interfere with approved grading and drainage patters or servicing. Snow storage areas shall be setback from the property lines, foundations, fencing or landscaping a minimum of 1.5m.

Snow storage areas shall not occupy driveways, aisles, required parking spaces or any portion of a road allowance. If snow is to be removed from the site please indicate this on the plan(s).

**Gas pressure regulating station:**

A gas pressure regulating station may be required depending on HVAC needs (typically for 12+ units). Be sure to include this on the Grading, Site Servicing, SWM and Landscape plans. This is to ensure that there are no barriers for overland flow routes (SWM) or conflicts with any proposed grading or landscape features with installed structures and has nothing to do with supply and demand of any product.



Gas Pressure  
Regulating Station.pdf

**Regarding Quantity Estimates:**

Please note that external Garbage and/or bicycle storage structures are to be added to QE under Landscaping as it is subject to securities. In addition, sump pumps for Sanitary and Storm laterals and/or cisterns are to be added to QE under Hard items as it is subject to securities, even though it is internal and is spoken to under SWM and Site Servicing Report and Plan.

**CCTV sewer inspection**

CCTV sewer inspection required for pre and post construction conditions to ensure no damage to City Assets surrounding site. Conditions for Pre-Construction/ Pre-Blast Survey & Use of Explosives will be applied to agreements. Refer to City's Standard S.P. No. F-1201 entitled Use of Explosives, as amended.

**Required Engineering Plans and Studies:**

**PLANS:**

- Existing Conditions and Removals Plan
- Site Servicing Plan
- Grade Control and Drainage Plan
- Erosion and Sediment Control Plan
- Roof Drainage Plan ( If roof utilized for the SWM)
- Topographical survey

**REPORTS:**

- Site Servicing and Stormwater Management Report
- Geotechnical Study/Investigation
- Slope Stability Assessment Reports ( if required, please see requirements below)
- Noise Control Study
- Phase I ESA 4)
- Phase II ESA (Depending on recommendations of Phase I ESA)

- Wind analysis
- Shadow Study

Please refer to the **City of Ottawa Guide to Preparing Studies and Plans [Engineering]:**

Specific information has been incorporated into both the [Guide to Preparing Studies and Plans](#) for a site plan. The guide outlines the requirement for a statement to be provided on the plan about where the property boundaries have been derived from.

Added to the general information for servicing and grading plans is a note that an **O.L.S.** should be engaged when reporting on or relating information to property boundaries or existing conditions. The importance of engaging an **O.L.S.** for development projects is emphasized.

#### **Phase One Environmental Site Assessment:**

- A Phase I ESA is required to be completed in accordance with Ontario Regulation 153/04 in support of this development proposal to determine the potential for site contamination. Depending on the Phase I recommendations a Phase II ESA may be required.
- The Phase I ESA shall provide all the required Environmental Source Information as required by O. Reg. 153/04. ERIS records are available to public at a reasonable cost and need to be included in the ESA report to comply with O.Reg. 153/04 and the Official Plan. The City will not be in a position to approve the Phase I ESA without the inclusion of the ERIS reports.
- Official Plan Section 4.8.4:
- <https://ottawa.ca/en/city-hall/planning-and-development/official-plan-and-master-plans/official-plan/volume-1-official-plan/section-4-review-development-applications#4-8-protection-health-and-safety>

#### **Geotechnical Investigation:**

- A Geotechnical Study/Investigation shall be prepared in support of this development proposal.
- Reducing the groundwater level in this area can lead to potential damages to surrounding structures due to excessive differential settlements of the ground. The impact of groundwater lowering on adjacent properties needs to be discussed and investigated to ensure there will be no short term and long term damages associated with lowering the groundwater in this area.
- Geotechnical Study shall be consistent with the **Geotechnical Investigation and Reporting Guidelines for Development Applications**.

[https://documents.ottawa.ca/sites/documents/files/geotech\\_report\\_en.pdf](https://documents.ottawa.ca/sites/documents/files/geotech_report_en.pdf)

#### **Slope Stability Assessment Reports**

- A report addressing the stability of slopes, prepared by a qualified geotechnical engineer licensed in the Province of Ontario, should be provided wherever a site has slopes (existing or proposed) steeper than 5 horizontal to 1 vertical (i.e., 11 degree inclination from horizontal) and/or more than 2 metres in height.
- A report is also required for sites having retaining walls greater than 1 metre high, that addresses the global stability of the proposed retaining walls.
- <https://documents.ottawa.ca/en/document/slope-stability-guidelines-development-applications>

**Noise Study:**

- A **Transportation Noise Assessment** is required as the subject development is located within 100m proximity of an Arterial Road
- A **Stationary Noise Assessment** is required in order to assess the noise impact of the proposed sources of stationary noise (mechanical HVAC system/equipment) of the development onto the surrounding residential area to ensure the noise levels do not exceed allowable limits specified in the City Environmental Noise Control Guidelines.

[https://documents.ottawa.ca/sites/default/files/documents/enviro\\_noise\\_guide\\_en.pdf](https://documents.ottawa.ca/sites/default/files/documents/enviro_noise_guide_en.pdf)

**Wind analysis:**

When greater than 8-storey in height Wind Study for all buildings/dwellings.

- A wind analysis must be prepared, signed and stamped by an engineer who specializes in pedestrian level wind evaluation. Where a wind analysis is prepared by a company which do not have extensive experience in pedestrian level wind evaluation, an independent peer review may be required at the expense of the proponent.
- [Terms of Reference: Wind Analysis \(ottawa.ca\)](#)

**Shadow Study**

When greater than 8-storey in height, a Shadow Study required for all buildings/dwellings.

**Exterior Site Lighting:**

- Any proposed light fixtures (both pole-mounted and wall mounted) must be part of the approved Site Plan. All external light fixtures must meet the criteria for Full Cut-off Classification as recognized by the Illuminating Engineering Society of North America (IESNA or IES), and must result in minimal light spillage onto adjacent properties (as a guideline, 0.5 fc is normally the maximum allowable spillage). In order to satisfy these criteria, the please provide the City with a **Certification (Statement) Letter** from an acceptable professional engineer stating that the design is compliant.

**Fourth (4<sup>th</sup>) Review Charge:**

Please be advised that additional charges for each review, after the 3<sup>rd</sup> review, will be applicable to each file. There will be no exceptions.

**Construction approach** – Please contact the Right-of-Ways Permit Office

[TMconstruction@ottawa.ca](mailto:TMconstruction@ottawa.ca) early in the Site Plan process to determine the ability to construct site **and copy File Lead on this request.**

Please note that these comments are considered preliminary based on the information available to date and therefore maybe amended as additional details become available and presented to the City. It is the responsibility of the applicant to verify the above information. The applicant may contact me for follow-up questions related to engineering/infrastructure prior to submission of an application if necessary.

If you have any questions or require any clarification, please let me know.

### **Community Association Comments:**

#### Roxanne

- It was good having a comprehensive package

#### Jane Thompson

- We like the general approach to the development especially the underground and a storage is great
  - It fits the goals of the centre of the community
- The biggest issue: has to do with transportation and transit
  - Very far from transit stops and not on the main bus line
  - it is a very car-centric community and there needs to be more transit
  - Even though there is parking offered, there is still more demand on parking than the surplus or what is provided
  - Advocating more transport
    - Afraid to be limited in space with the development coming in
  - They feel isolated from other modes of transport so parking needs to be more sufficient
  - Would not be comfortable with less than the minimum requirement for visitor parking
  - Happy to see the bicycle parking as there are bicycle lanes near the area
  - TIA screening needed to see how many units is being provided
- Multi-use
  - The community is encouraging commercial uses to accommodate more services and it would please the residents
  - Something like adding little shops and offices
  - More transparency and connection to the streets especially from the corridors
- Parking entry adjacent to the neighboring site
  - Discussing how they would get along and if both sides are comfortable with that
- Because of rental and high density – need to consider the effects of the drop off
  - May cause issues for the winter
  - Also adding space for garbage and space for collection
  - It's a dense development but be careful as more people means more cars
  - Need for more amenity space
- Colours
  - Predominance of dull colours in the neighborhood but some liveliness and fun colours added would be great for the area

### **Next Steps:**

- Follow up email that will include meeting notes and the plans and studies list required for SPC submission
- Book some time to approach community association to discuss proposal, as well as with the ward Councillor



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# Memorandum

<b>To/Attention</b>	John Bernier, City of Ottawa Shawn Wessel, City of Ottawa	<b>Date</b>	April 26, 2022
<b>From</b>	Meghan Black Jim Moffatt	<b>Project No</b>	118863-5.3.1.5
<b>cc</b>	Mary Jarvis, Canada Lands Company		
<b>Subject</b>	Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing		

## 1. Background

Blocks 11 and 12 are located within Phase 2B of the Wateridge development and are indicated in **Figure 1**. The municipal servicing of the two blocks was addressed in, “Design Brief, Wateridge Village at Rockcliffe Phase 2B,” prepared by IBI Group in April 2019. Subsequent to the approval of the Phase 2B detailed design, Canada Lands Company has sub-divided the subject blocks into five parcels for development. The parcels, identified as Parcels 1-5, are being considered for purchase by various parties. IBI has been engaged to assess the impact of this change on adjacent existing storm and sanitary sewers. Enclosed **Figure 1** depicts Blocks 11 and 12 and the respective five parcels.

## 2. Stormwater Management

### 2.1 Objective

The objective of the evaluation is to assess the impact on the dual drainage system of discretizing Blocks 11 and 12 into Parcels 1-5 and the associated impacts to the storm servicing. The detailed design of Parcels 1-5 will be carried out by others.

### 2.2 Dual Drainage Design

Per the Phase 2B design brief, minor storm runoff from Block 11 (identified as drainage area B309) drains to Bareille-Snow Street, with major flow tipping to Bareille-Snow Street at Hemlock Road. Minor flow from Block 12 (identified as drainage area B340) drains to Codd’s Road with major flow draining to Hemlock Road. The minor system restriction for the two development blocks corresponds to between the 5 and 100 year storm event, and no on-site storage was proposed. The storm drainage area plan (Drawing 750) from the Phase 2B submission is enclosed in **Appendix A** for reference. With the proposed adjustments to the storm servicing for the sub-divided or discretized parcels, minor system capture and on-site storage has been re-assessed.

## 2.3 Hydrological Analysis

Hydrological analysis of the dual drainage system of the subject site has been conducted using DDSWMM, consistent with the simulations completed for the Phase 2B design brief.

### 2.3.1 Storm and Design Parameters

The following storms and design parameters have been used in the evaluation. The main hydrological parameters are summarized in **Table 2.1**, with a comparison of what was included in the Phase 2B evaluation.

- **Design Storms:** The subject site has been evaluated with the following storms, consistent with the Phase 2B evaluation:
  - 5 and 100 year 3 hour Chicago storm events, and associated stress test; applied for the evaluation of the trunk storm sewers;
  - 100 year 24 hour SCS Type II storm event, applied for the evaluation of the trunk storm sewers;
  - July 1979, August 1988, August 1996 historical storms per the OSDG.
- **Area and Imperviousness:** Block 11 (identified as drainage area B309) and Block 12 (identified as drainage area B340) have been discretized into Parcels 1 through 5. An imperviousness value of 86% has been applied to the parcels, consistent with the values applied for B309 and B340 in the Phase 2B design brief.
- **Infiltration:** Infiltration losses were selected to be consistent with the OSDG. The Horton values are as follows:  $f_0 = 76.2$  mm/h,  $f_c = 13.2$  mm/h,  $k = 0.00115$  s<sup>-1</sup>.
- **Subcatchment Width:** The catchment width for the parcels was based on 225 m/ha.
- **Slope:** The ground slope was based upon the average slope for both impervious and pervious area. Generally, the slope is approximately 2% (0.02 m/m). This assumes a slope of approximately 1% for impervious or road surfaces and 3% for pervious surfaces (lot grading).
- **Initial Abstraction (Detention Storage):** Detention storage depths of 1.5 mm and 4.67 mm were used for impervious and pervious areas, respectively. These values are consistent with the OSDG.
- **Manning's roughness:** Manning's roughness coefficients of 0.013 and 0.25 were used for impervious and pervious areas, respectively.
- **Baseflow:** No baseflow components were assumed for any of the areas contributing runoff to the minor system within the DDSWMM model.
- **Minor System Capture:** The minor system capture for the parcels ranges from the 5 year to the 100 year, with three parcels capturing between the 5 and 100 year simulated flow.
- **Major System Storage and Routing:** In order to continue to satisfy City design guidelines, on-site storage has been introduced on four of the parcels, as noted below.

A summary of parameters and minor system and on-site storage is presented in the following tables. A summary from the Phase 2B detailed design is included to facilitate review. Refer to



**Figure 2** for the overall storm sewer network and to **Figure 3** for a depiction of the minor and major system connectivity for the five parcels.

**Table 2.1 Hydrological Parameters**

Block	Phase 2B Design Brief							Current Evaluation							
	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)	Parcel	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)
11	B309	1.24	S308A on Bareille-Snow	MH309 on Bareille-Snow	0.86	135.1	270.2	1	B309_1	0.72	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	81	162
								2	B309_2	0.52	S308A on Bareille-Snow	MH310 on Michael Stoqua	0.86	58.5	117
12	B340	1.24	S207 on Hemlock	MH305 on Codd's Road	0.86	173.1	346.3	3	B340_3	0.34	S308A on Bareille-Snow	MH308 on Bareille-Snow	0.86	38.25	76.5
								4	B340_4	0.53	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	59.63	119.25
								5	B340_5	0.37	S340 on Codd's	MH305 on Codd's Road	0.86	41.63	83.25

**Table 2.2 Minor System Restriction and On-site Storage**

Block	Phase 2B Design Brief				Current Evaluation					
	Drainage Area ID	Minor System Capture		Required On-Site Storage (cu-m)	Parcel	Drainage Area ID	Minor System Capture		Major System	
		Simulated Flow (l/s)	Corresponding Design Storm				Simulated Flow (l/s)	Corresponding Design Storm	Required On-Site Storage (cu-m)	Comment
11	B309	370	Between 5 and 100	None	1	B309_1	195	Between 5 and 100 year	43	Control up to the 100 year event
					2	B309_2	105	5 year	64	Control up to the 100 year event
12	B340	366	Between 5 and 100	None	3	B340_3	95	Between 5 and 100 year	18	Control up to the 100 year event
					4	B340_4	150	Between 5 and 100 year	21	Control up to the 100 year event
					5	B340_5	139	100 year	None	N/A

## 2.4 Results of Hydrological Modeling

### 2.4.1 Minor System

The minor system hydrographs generated by the hydrological model were exported to the hydraulic model for analysis, discussed in **Section 2.5**.

### 2.4.2 Major System

Due to the adjustment in major system connectivity, the major system has been reassessed. Refer to drainage areas on Drawing 750 from the Phase 2B submission in **Appendix A**.

#### 2.4.2.1 Street Segment Storage

The available and utilized street sag storage is summarized in the below table for street segments in affected by the revised storm servicing of Parcels 1-5.

**Table 2.3 Summary of On-site Street Storage (Available and Utilized) During Target Minor System Design Storm in Vicinity of Parcels 1-5**

Street	Drainage Area ID	Minor System Design Storm	Available Static Storage (cu-m)	Total Storage Utilized During Minor System Design Storm (cu-m)	Overflow During Minor System Design Storm (l/s)
Michael Stocqua	S310A	5	61.39	0	0
Bareille-Snow	S308A	5	40.38	0	0
Hemlock	S176C	5	1.14	0	0

The results indicate that there is no ponding on the street segments during the minor system design storm.

#### 2.4.2.2 Velocity x Depth

According to the City of Ottawa Sewer Design Guidelines (October 2012), the maximum depth of flow should not exceed 350 mm and the product of velocity and depth on all the street segments should not exceed 0.6 m<sup>2</sup>/s during the 100 year storm event.

The cascading overflow is the flow exiting a drainage area when maximum minor system inflow and maximum available ponding has been utilized. To determine velocity of the cascading overflow, a SWMHYMO file was created (118863VD.dat).

To determine velocity of the cascading overflow at critical locations, SWMHYMO was used. The ROW sections were entered into the model with the appropriate longitudinal slopes to obtain the maximum velocity of flow using the Route Channel routine. The overflow is obtained from the respective DDSWMM output file and is noted in the footnotes of the below tables.

To determine depth of the cascading overflow, the *Calculation Sheet: Overflow From Typical Road Ponding Area* provided at the February 2014 Technical Bulletin ISDTB-2014-01 was used. The

exception to this is where the road is on grade in which case the depths were obtained from the SWMHYMO model.

The results are presented in **Table 2.4** and **Table 2.5** and the supporting calculations are included in **Appendix A**.

**Table 2.4 Summary of Cascading Flow during the 100 year 3 hour Chicago storm**

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Michael Stoqua	S311A	N/A	49	0.73	N/A	0.04	0.04	0.03
Michael Stoqua	S310A	D14	0	0	0.29	0	0.29	0
Bareille-Snow	S309	N/A	43	0.50	N/A	0.05	0.05	0.03
Bareille-Snow	S308	N/A	65	0.84	N/A	0.05	0.05	0.04
Bareille-Snow	S308A	D18	26	0.47	0.26	0.05	0.31	0.03
Codd's	S340	N/A	50	0.88	N/A	0.04	0.04	0.04
Codd's	S231	N/A	100	0.62	N/A	0.07	0.07	0.04
Hemlock	S205C	N/A	37	0.48	N/A	0.05	0.05	0.02
Hemlock	S207	N/A	61	0.55	N/A	0.06	0.06	0.03

(1) Overflow from DDSWMM output 118863-3CHI100.out

(2) Velocity from SWMHYMO output 118863VD.out

(3) Depth of the cascading overflow was determined from the Calculation Sheet: Overflow From Typical Road Ponding Area provided in the February 2014 Technical Bulletin ISDTB-2014-01. For those areas which have a continuous road grade (or no dummy segment), the depth was taken from SWMHYMO VxD simulation.

**Table 2.5 Summary of Cascading Flow during the 100 year 3 hour Chicago storm + 20%**

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Michael Stoqua	S311A	N/A	66	0.79	N/A	0.05	0.05	0.04
Michael Stoqua	S310A	D14	33	0.61	0.29	0.06	0.35	0.04
Bareille-Snow	S309	N/A	71	0.57	N/A	0.06	0.06	0.03
Bareille-Snow	S308	N/A	216	1.15	N/A	0.08	0.08	0.09
Bareille-Snow	S308A	D18	268	1.29	0.26	0.13	<b>0.39</b>	0.17
Codd's	S340	N/A	98	1.04	N/A	0.05	0.05	0.06
Codd's	S231	N/A	165	0.71	N/A	0.08	0.08	0.06
Hemlock	S205C	N/A	46	0.51	N/A	0.05	0.05	0.03

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Hemlock	S207	N/A	89	0.60	N/A	0.07	0.07	0.04

(1) Overflow from DDSWMM output 118863-3CHI120.out

(2) Velocity from SWMHYMO output 118863VD.out

(3) Depth of the cascading overflow was determined from the Calculation Sheet: Overflow From Typical Road Ponding Area provided in the February 2014 Technical Bulletin ISDTB-2014-01. For those areas which have a continuous road grade (or no dummy segment), the depth was taken from SWMHYMO VxD simulation.

During the 100 year 3 hour Chicago storm, the summation of depth of ponding and depth of cascading flow for all street segments is less than the City guideline of 0.35 m. The product of depth and velocity is also less than the City guideline of 0.6 m<sup>2</sup>/s.

During the sensitivity analysis applying the 100 year 3 hour Chicago storm increased by 20%, the summation of depth of ponding and depth of cascading flow for all street segments is less than the City guideline of 0.35 m, with the exception of S308A, noted in the above table in bold red type. At all locations, the product of depth and velocity is less than the City guideline of 0.6 m<sup>2</sup>/s.

These results are consistent with those of the Phase 2B detailed design. It should be noted that major flow from the above-noted affected areas is at or below that accounted for in the Phase 2B model.

The area at which total depth of ponding and cascading flow exceeds 0.35 m during the stress test is noted in the below table with the critical adjacent property elevation.

**Table 2.6 Critical Ponding Locations during the Stress Test and Adjacent Property Elevations**

Drainage Area ID	Low Point Elevation (m)	Max. Depth (Static + Dynamic) (m)	(1) Corresponding Elevation (m)	(2) Adjacent Property Line (m)	Difference (2) – (1)
S308A	88.74	0.39	89.13	89.01	-0.12

The corresponding stress test ponding elevation is greater than the adjacent block grading at the boulevard. At the detailed design stage of the blocks, house openings must be greater than the ponding elevation.

## 2.5 Storm Hydraulic Grade Line Analysis

The hydraulic grade line (HGL) was evaluated using the XPSWMM hydraulic model. The existing overall model for the Wateridge site, most recently revised as part of the Phase 4 submission (December 2021), was revised to include the revised servicing of Parcels 1-5.

XPSWMM simulations were conducted for the 100 year 3 hour Chicago storm to ensure that the HGL is at least 0.3 m below the underside of footing elevations. A sensitivity analysis was also performed using the 100 year Chicago storm with a 20% increase in intensity to ensure that there is no severe flooding to properties. Hydraulic grade line elevations along the existing downstream Phase 1A trunk storm sewer and relevant Phase 2B storm sewers are presented in the below table for these storms, along with a comparison of underside of footing (USF) elevations. Results

for the overall development area are presented in the enclosed **Appendix A**, including for the three historical storms per OSDG. Refer to **Figure 1** for the location of storm maintenance holes.

**Table 2.7 Storm Hydraulic Grade Line – Phase 1A Trunk and Relevant Phase 2B Storm Sewers**

MH ID	Street	Proposed Ground Elev. (m)	USF (m)	100 year 3 hour Chicago		100 year 3 hour Chicago + 20%	
				HGL (m)	USF – HGL (m)	HGL (m)	USF – HGL (m)
MH194	<i>Top of the escarpment</i>	82.05	N/A	80.47	N/A	80.55	N/A
MH193	OSHEDINAA	84.68	82.68	81.12	1.56	81.28	1.40
MH192	OSHEDINAA	84.99	82.99	81.46	1.53	81.64	1.35
MH191	OSHEDINAA	85.76	83.76	81.72	2.04	81.93	1.83
MH190	OSHEDINAA	86.36	84.36	81.96	2.40	82.19	2.17
MH180	OSHEDINAA	86.96	84.96	82.27	2.69	82.77	2.19
MH178	HEMLOCK	89.00	86.60	83.41	3.19	83.47	3.13
MH176	HEMLOCK	88.03	85.63	83.77	1.86	83.85	1.78
MH231	CODD'S	89.81	87.41	85.61	1.79	85.64	1.77
MH305	CODD'S	91.00	88.60	86.54	2.06	86.56	2.04
MH207	HEMLOCK	88.53	86.13	84.65	1.48	84.65	1.48
MH206	HEMLOCK	89.10	86.70	85.65	1.05	85.65	1.05
MH308	BAREILLE-SNOW	89.68	87.28	86.88	0.40	86.69	0.59
MH309	BAREILLE-SNOW	90.15	87.75	87.44	0.31	87.08	0.67
MH205	HEMLOCK	89.35	86.95	85.86	1.09	85.88	1.07
MH310	MICHAEL STOCQUA	90.04	87.64	87.28	0.36	87.42	0.22
MH311	MICHAEL STOCQUA	90.69	88.29	87.44	0.85	87.56	0.73

Along the Phase 1A trunk and Phase 2B storm sewers presented above, a minimum 0.3 m clearance between the USF and HGL is maintained during the 100 year 3 hour Chicago storm and the HGL elevations remain below USF elevations during the sensitivity analysis. This is also true for the results for the remainder of the development area for additional storm simulations (enclosed in **Appendix A**).

## 2.6 Conclusion

The storm servicing of Blocks 11 and 12 was addressed during the detailed design of Phase 2B. The purpose of this evaluation is to assess the impact on the dual drainage system of discretizing Blocks 11 and 12 into Parcels 1-5 and the associated revisions to the storm servicing. The proposed minor and major connectivity of the five parcels is presented on **Figure 3** and minor system capture and required on-site storage is summarized in **Table 2.2**.

In terms of major flow, the depth and velocity of flow on streets adjacent to the five parcels was evaluated. City guidelines with respect to ponding during the minor system design storm, as well as maximum depth and velocity of flow are maintained. Major flow from the adjacent street segments is at or below that accounted for in the Phase 2B model.

With respect to minor flow, the hydraulic grade line evaluation was updated with the revised inflow hydrographs from the five parcels. Results indicate that a minimum 0.3 m clearance between the USF and HGL is maintained during the 100 year 3 hour Chicago storm and the HGL elevations remain below USF elevations during the sensitivity analysis.

It is therefore concluded that the proposed storm servicing to support Parcels 1-5 can be accommodated by the existing storm infrastructure.

## 3. Wastewater Outlet

### 3.1 Objective

The objective of this evaluation is to assess the impact on the existing wastewater system by the sub-division of Blocks 11 and 12 into five parcels. **Figure 4** shows the location of the subject site and the existing sanitary sewers which will be impacted by this change.

### 3.2 Existing Conditions

Development of Phase 2B included the construction of sanitary sewers in Codd's Road from MH231A to the MH340A and Bareille-Snow Street from BLK308A to MH304A. The sanitary sewer on Codd's Road was designed to capture wastewater flows from Block 12 and the sanitary sewer on Bareille-Snow Street was designed to capture wastewater flows from Block 11. The Bareille-Snow sewer outlets to a sanitary sewer in Hemlock Road. The latter sewer was designed in 2017, using the City's wastewater flow criteria in effect at that time and predicted a flow of 28.49 l/s tributary from the Bareille-Snow sewer. The Bareille-Snow sanitary sewer was designed in 2019 based on flow calculation criteria in effect at that time and predicted a slightly less flow of 25.17 l/s. A highlighted copy of the Phase 2B sanitary sewer design sheet is included in **Appendix B**. The spreadsheet has been highlighted to indicate the immediate downstream sewers on Codd's Road and Bareille-Snow Street. The flow calculations in the Phase 2B spreadsheet were based on the City of Ottawa's wastewater criteria in effect of that time (2019) and the block population densities noted in the Master Servicing Study.

### 3.3 Proposed Condition

Because of the sub-division of Blocks 11 and 12 into five parcels, less wastewater flow is now proposed to outlet to the Codd's Road sanitary sewer. The Phase 2B sewer designed assumed all Block 12 would outlet to that sewer but now only parcel 5 is proposed to outlet in that direction. No further analysis is therefore needed for the Codd's Road sewer.

Parcels 3 and 4, which represent the balance of Block 12, are now proposed to outlet to the existing sanitary sewer in Bareille-Snow Street and not the Codd's Road sewer. There is no

proposed change to the wastewater outlet for parcels 1 and 2. The Phase 2B design assumed all Block 11 would outlet to the Bareille-Snow sewer. Consequently, the expected wastewater flows to the latter pipe will likely increase.

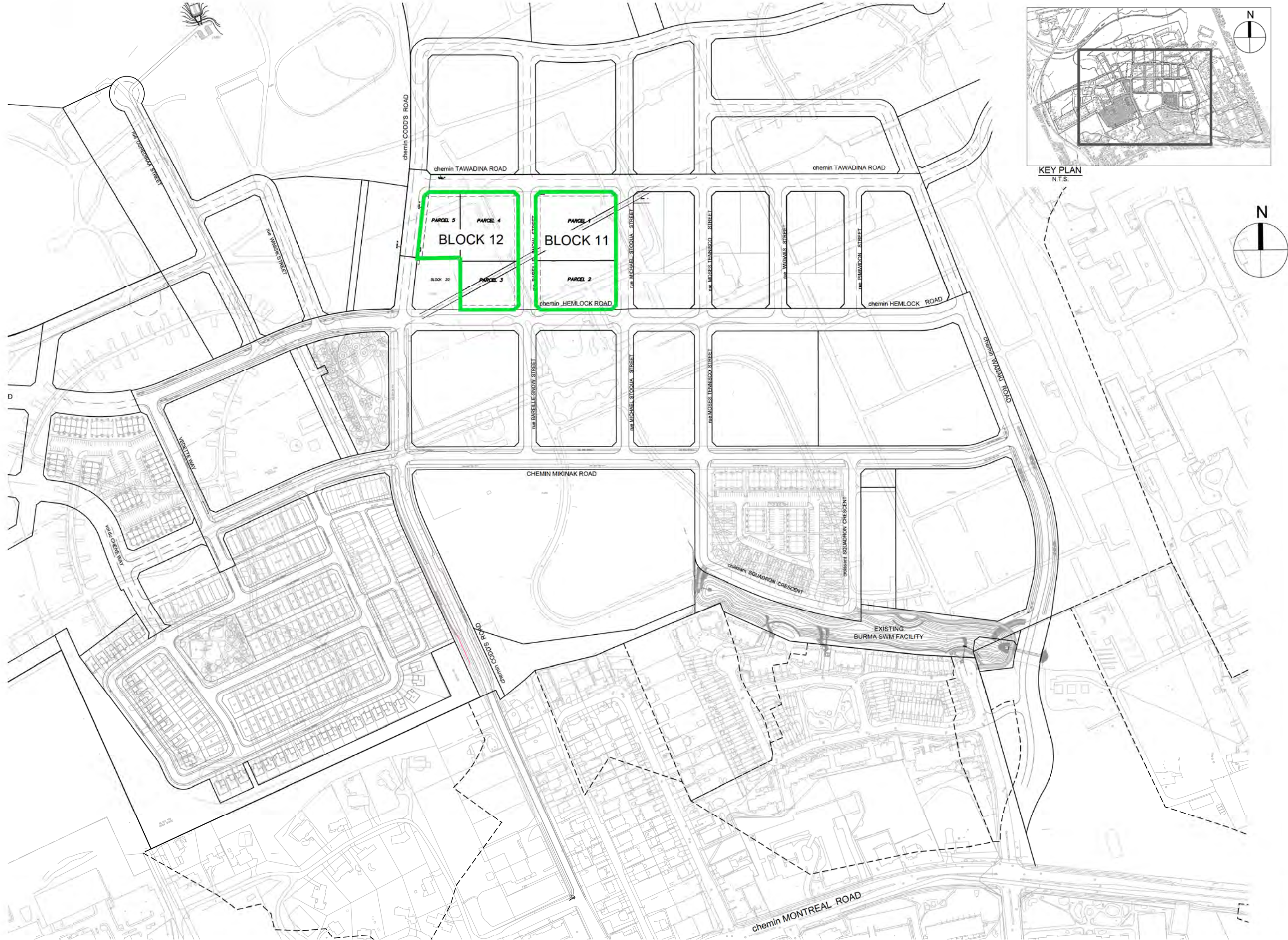
An analysis of the ability of the existing sanitary sewer system in Bareille-Snow Street to accommodate the flows from both Block 11 and 12 was completed. This analysis is included on the updated sanitary sewer spreadsheet included in **Appendix B**. The updated spreadsheet was based not only on the current City of Ottawa wastewater criteria, which came into effect in 2018 but also on the most current concept plans for the various parcels which are also included in **Appendix B**. The updated analysis includes the existing sewer system highlighted on the Phase 2B design sheet.

Based on the updated analysis, the calculated wastewater flows tributary to the Hemlock Road sewer from Bareille-Snow Street is 30.31 l/s. This shows a wastewater flow increase of 1.82 l/s as a result of re-directing wastewater flows from parcels 3 and 4 in Block 12. The capacity of that sewer is 88.83 l/s. The Phase 1B design of the sanitary sewer in Hemlock Road between Bareille-Snow Street and Codd's Road indicated a spare capacity in that sewer of about 58 l/s. For reference, a highlighted copy of the Phase 1B sanitary sewer design sheet is included in **Appendix B**.

### **3.4 Conclusion**

The impact of re-directing wastewater flows from Block 12 to the Bareille-Snow Street sanitary sewer has been completed. Based on the analysis noted above, the existing wastewater system in Wateridge Village Phase 1B and 2B has sufficient available capacity to carry the re-directed flows from Block 12. It is therefore concluded that the existing sanitary sewers in Bareille-Snow Street, Codd's Road and Hemlock Road adjacent to the subject property can accommodate the re-direction of flows from Block 12.





Sheet No.

Drawing Title

Project Title

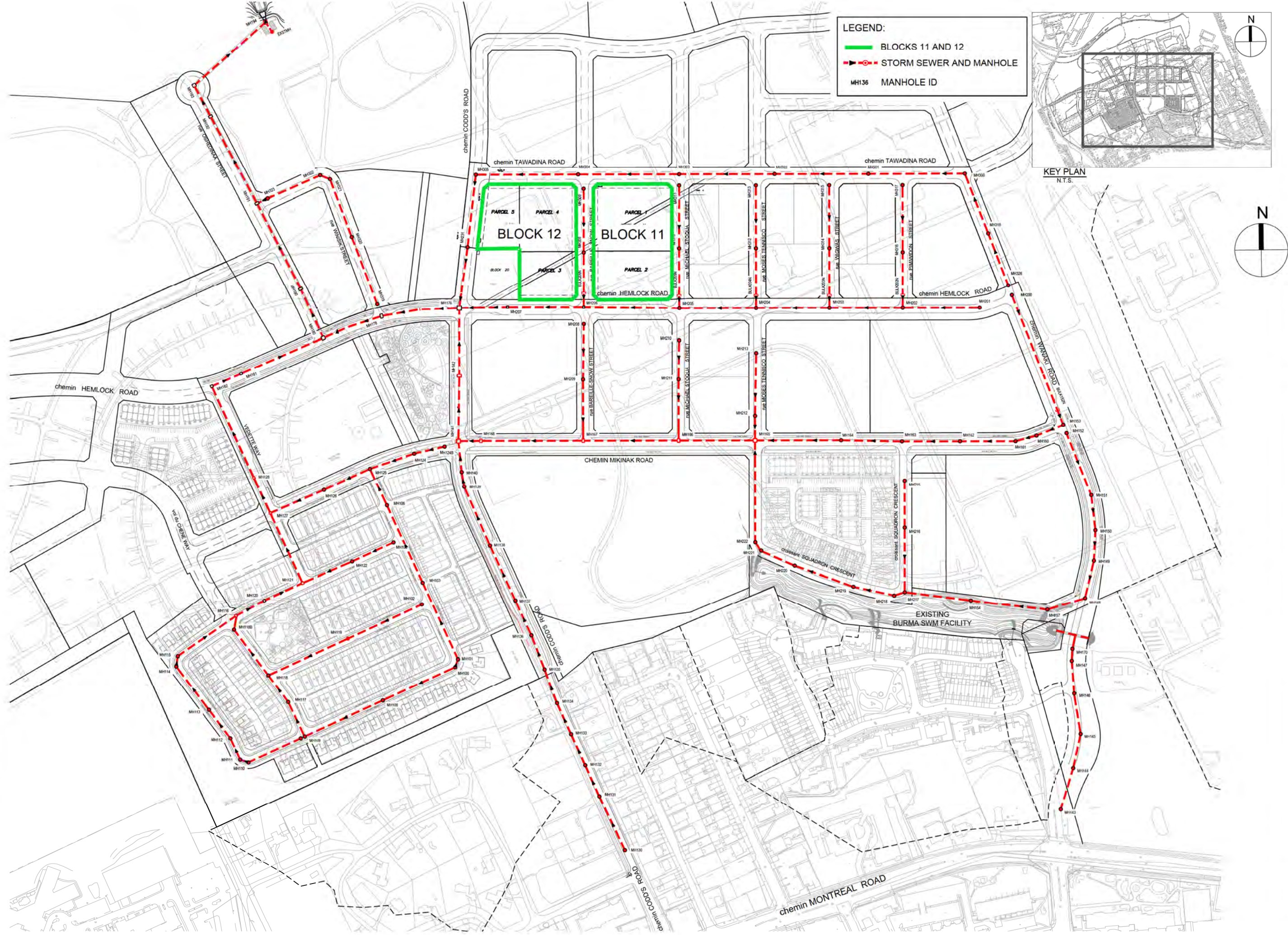
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**FIGURE 1**

**LOCATION PLAN**  
**STORM AND SANITARY SERVICING**  
**ASSESSMENT OF BLOCK 11 AND 12**  
**WATERIDGE VILLAGE PHASE 2B**



N.T.S.



Sheet No.

Drawing Title  
**LOCATION PLAN  
AND STORM SEWER  
NETWORK**

Project Title  
**STORM AND SANITARY SERVICING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B**

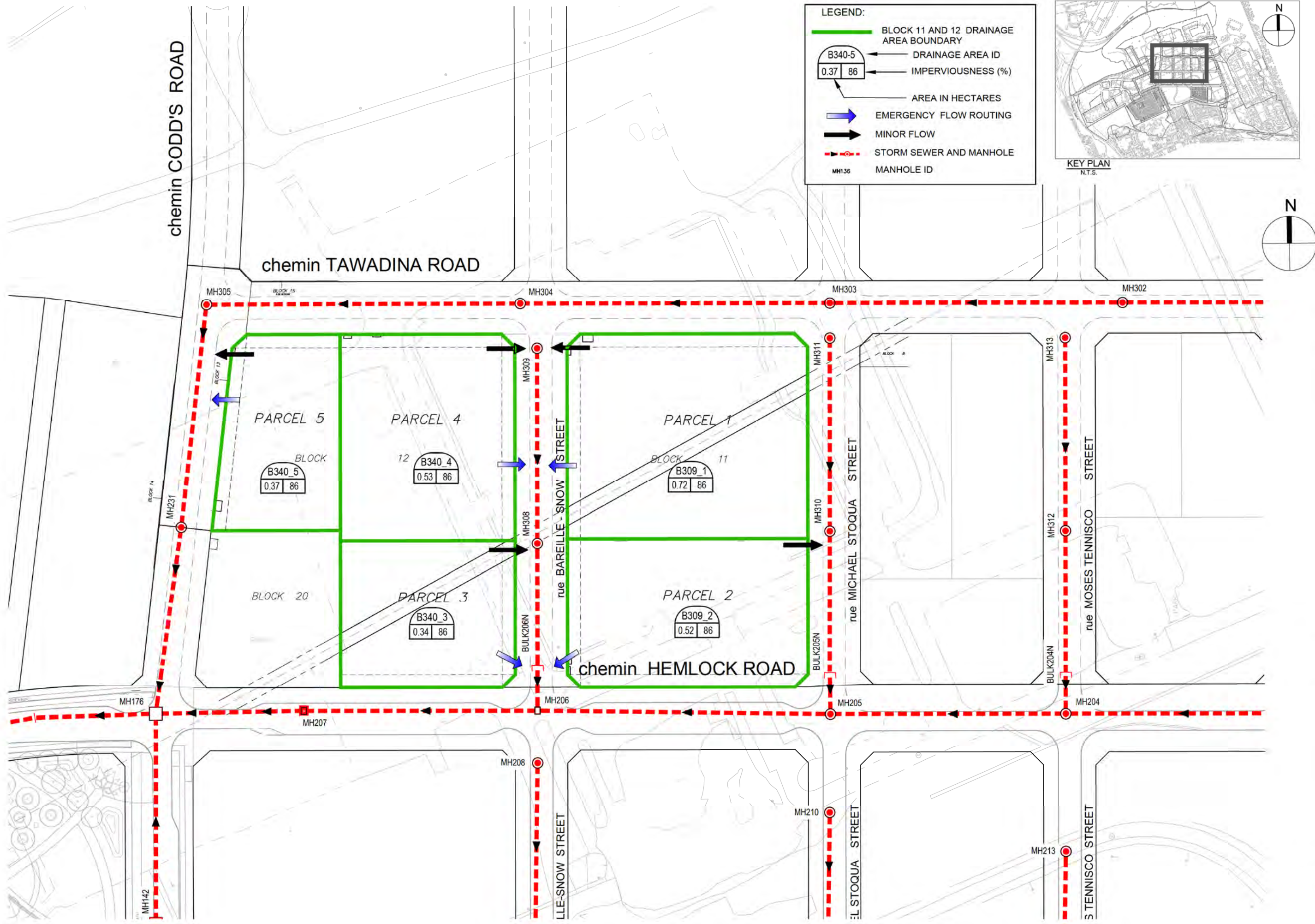
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**FIGURE 2**

N.T.S.

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J:\38298-CFBRockville\5.9 Drawings\55civi\SWM\FIGURES\BLOCK 11,12\FIGURE 3.dwg Layout Name: FIG3 Plot Style: AIA STANDARD COLOR-HALF-CTB Plotted At: 4/25/2022 5:22 PM Last Saved By: svolkig, Apr. 25, 22



Sheet No.

FIGURE 3

MINOR AND MAJOR SYSTEM  
CONNECTIVITY

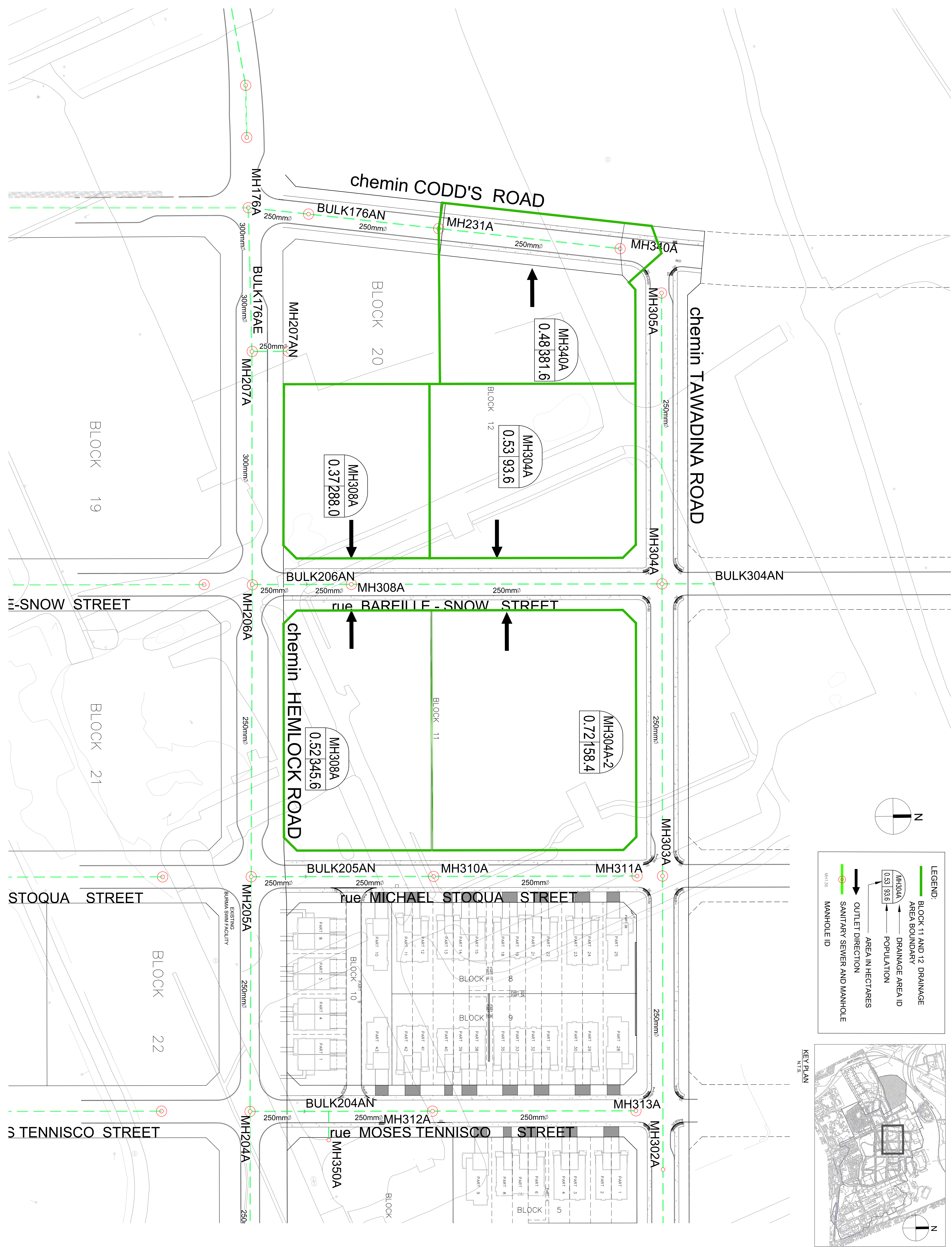
STORM AND SANITARY SERVICING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B

Project Title

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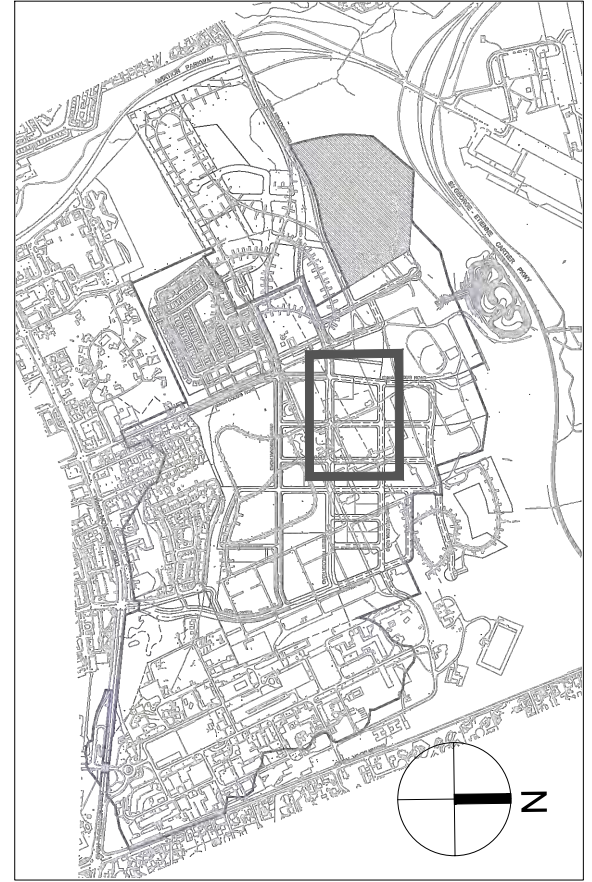
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**LEGEND:**

- BLOCK 11 AND 12 DRAINAGE AREA BOUNDARY
- DRAINAGE AREA ID
- POPULATION
- AREA IN HECTARES
- SANITARY SEWER AND MANHOLE
- MANHOLE ID



## **Appendix A**

Supporting Storm Information

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## **Summary of Model Files**

### **DDSWMM:**

5 year 3 hour Chicago: 118863-3CHI5.DAT  
100 year 3 hour Chicago: 118863-3CHI100.DAT  
100 year 3 hour Chicago + 20%: 118863-3CHI120.DAT

100 year 24 hour SCS Type II: 118863-24SCS100.DAT  
100 year 24 hour SCS Type II + 20%: 118863-24SCS120.DAT

July 1979: 118863-JUL79.DAT  
August 1988: 118863-AUG88.DAT  
August 1996: 118863-Aug96.DAT

### **SWMHYMO VxD:**

118863VD.dat

### **XPSWMM:**

5 year 3 hour Chicago: 118863-3CHI5\_BLK1112\_V08\_2022-03-15.XP  
100 year 3 hour Chicago: 118863-3CHI100\_BLK1112\_V08\_2022-02-28.XP  
100 year 3 hour Chicago + 20%: 118863-3CHI120\_BLK1112\_V08\_2022-02-28.XP

100 year 24 hour SCS Type II: 118863-24SCS100\_BLK1112\_V08\_2022-03-15.XP  
100 year 24 hour SCS Type II + 20%: 118863-24SCS120\_BLK1112\_V08\_2022-03-15.XP

July 1979: 118863-JUL1979\_BLK1112\_V08\_2022-03-15.XP  
August 1988: 118863-AUG1988\_BLK1112\_V08\_2022-03-15.XP  
August 1996: 118863-AUG1996\_BLK1112\_V08\_2022-03-15.XP



# Velocity x Depth Calculation

Iteration equation:

Velocity:

$$v_x = v_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (v_{\max} - v_{\min})$$

Depth:

$$d_x = d_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (d_{\max} - d_{\min})$$

100 Year 3 Hour Chicago Storm																				
			SWMHYMO (118863VD.OUT)							Calculation Sheet: Overflow for Typical Road Ponding Area					SWMHYMO (118863VD.OUT)			Velocity x Depth	Maximum Static Ponding Depth	Total Depth (Static + Dynamic)
Area ID (Dummy Segment, if applicable)	Road ROW Section	Longitudinal Slope (%)	Overflow Flowrate		Flowrate (cms)		Velocity (m/s)			Flowrate (cms)		Depth (m)			Depth (m)			(m <sup>2</sup> /s)	(m)	(m)
			Qx (l/s)	Qx (cms)	Qmin	Qmax	vmin	vmax	vx	Qmin	Qmax	dmin	dmax	dx	dmin	dmax	dx			
S311A	20	1.52	<b>49</b>	0.049	0.039	0.084	0.699	0.847	<b>0.73</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.044</b>	<b>0.03</b>	0.00	0.04
S310A	20	1.22	<b>0</b>	0.000	0.000	0.002	0.000	0.301	<b>0.00</b>	0.000	0.001	0.000	0.001	<b>0.000</b>	N/A	N/A	<b>N/A</b>	<b>0.00</b>	0.29	0.29
S309	20	0.60	<b>43</b>	0.043	0.024	0.053	0.439	0.532	<b>0.50</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.050</b>	<b>0.03</b>	0.00	0.05
S308	20	1.84	<b>65</b>	0.065	0.043	0.092	0.769	0.932	<b>0.84</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.047</b>	<b>0.04</b>	0.00	0.05
S308A	20	0.71	<b>26</b>	0.026	0.009	0.027	0.365	0.478	<b>0.47</b>	0.021	0.027	0.050	0.055	<b>0.054</b>	N/A	N/A	<b>N/A</b>	<b>0.03</b>	0.26	0.31
S340	20	2.40	<b>50</b>	0.050	0.049	0.105	0.878	1.064	<b>0.88</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.041</b>	<b>0.04</b>	0.00	0.04
S205C	24	0.71	<b>37</b>	0.037	0.024	0.053	0.439	0.532	<b>0.48</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.047</b>	<b>0.02</b>	0.00	0.05
S231	20	0.53	<b>100</b>	0.100	0.096	0.155	0.617	0.697	<b>0.62</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.068	0.082	<b>0.069</b>	<b>0.04</b>	0.00	0.07
S207	24	0.51	<b>61</b>	0.061	0.053	0.096	0.532	0.617	<b>0.55</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.055	0.068	<b>0.057</b>	<b>0.03</b>	0.00	0.06



# Velocity x Depth Calculation

Iteration equation:

Velocity:

$$v_x = v_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (v_{\max} - v_{\min})$$

Depth:

$$d_x = d_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (d_{\max} - d_{\min})$$

100 Year 3 Hour Chicago Storm + 20%																				
				SWMHYMO (118863VD.OUT)						Calculation Sheet: Overflow for Typical Road Ponding Area					SWMHYMO (118863VD.OUT)			Velocity x Depth	Maximum Static Ponding Depth	Total Depth (Static + Dynamic)
Area ID (Dummy Segment, if applicable)	Road ROW Section	Longitudinal Slope (%)	Overflow Flowrate		Flowrate (cms)		Velocity (m/s)			Flowrate (cms)		Depth (m)			Depth (m)			(m <sup>2</sup> /s)	(m)	(m)
			Qx (l/s)	Qx (cms)	Qmin	Qmax	vmin	vmax	vx	Qmin	Qmax	dmin	dmax	dx	dmin	dmax	dx			
S311A	20	1.52	66	0.066	0.039	0.084	0.699	0.847	0.79	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.049	0.04	0.00	0.05
S310A	20	1.22	33	0.033	0.012	0.035	0.478	0.626	0.61	0.028	0.035	0.055	0.060	0.059	N/A	N/A	N/A	0.04	0.29	0.35
S309	20	0.60	71	0.071	0.053	0.096	0.532	0.617	0.57	N/A	N/A	N/A	N/A	N/A	0.055	0.068	0.060	0.03	0.00	0.06
S308	20	1.84	216	0.216	0.167	0.272	1.081	1.221	1.15	N/A	N/A	N/A	N/A	N/A	0.068	0.082	0.075	0.09	0.00	0.07
S308A	20	0.71	268	0.268	0.255	0.364	0.841	0.919	1.29	0.240	0.269	0.125	0.130	0.130	N/A	N/A	N/A	0.17	0.26	0.39
S340	20	2.40	98	0.098	0.049	0.105	0.878	1.064	1.04	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.053	0.06	0.00	0.05
S205C	24	0.71	46	0.046	0.024	0.053	0.439	0.532	0.51	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.052	0.03	0.00	0.05
S231	20	0.53	165	0.165	0.155	0.234	0.697	0.773	0.71	N/A	N/A	N/A	N/A	N/A	0.082	0.095	0.084	0.06	0.00	0.08
S207	24	0.51	89	0.089	0.053	0.096	0.532	0.617	0.60	N/A	N/A	N/A	N/A	N/A	0.055	0.068	0.066	0.04	0.00	0.07

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
<b>Phase 1B</b>																	
S143	143	102.40	100.00	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84
S144	144	99.41	97.01	95.79	1.22	95.79	1.22	95.78	1.23	95.79	1.22	95.78	1.23	95.79	1.22	95.78	1.23
S145	145	97.64	95.24	93.01	2.23	93.01	2.23	93.01	2.23	93.01	2.23	93.00	2.24	93.01	2.23	93.00	2.24
S146	146	95.28	92.88	90.96	1.92	91.82	1.06	90.77	2.11	91.26	1.62	90.91	1.97	91.01	1.87	90.63	2.25
S147	147	93.27	N/A	90.93	N/A	91.78	N/A	90.72	N/A	91.23	N/A	90.88	N/A	90.98	N/A	90.60	N/A
USBRM	N/A	N/A	N/A	90.88	N/A	91.72	N/A	90.67	N/A	91.17	N/A	90.83	N/A	90.93	N/A	90.56	N/A
BURMA	N/A	N/A	N/A	89.41	N/A	89.87	N/A	89.24	N/A	89.53	N/A	89.43	N/A	89.31	N/A	89.04	N/A
OUTLET	N/A	N/A	N/A	89.26	N/A	89.75	N/A	89.07	N/A	89.39	N/A	89.29	N/A	89.15	N/A	88.65	N/A
S152	152	92.73	90.33	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62
S151	151	92.50	90.10	89.58	0.52	89.57	0.53	89.58	0.52	89.58	0.52	89.58	0.52	89.58	0.52	89.57	0.53
S150	150	92.32	89.92	89.49	0.43	89.48	0.44	89.49	0.43	89.49	0.43	89.49	0.43	89.49	0.43	89.49	0.43
S149	149	92.34	89.94	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52
S148	148	92.14	89.74	89.30	0.44	89.29	0.45	89.30	0.44	89.30	0.44	89.30	0.44	89.30	0.44	89.30	0.44
S157	157	91.24	N/A	89.21	N/A	89.20	N/A	89.21	N/A	89.21	N/A	89.21	N/A	89.21	N/A	89.21	N/A
S154	154	91.02	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A
S215	215	90.77	88.37	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79
S216	216	90.85	88.45	87.30	1.15	87.30	1.15	87.30	1.15	87.30	1.15	87.30	1.15	87.31	1.14	87.30	1.15
S217	217	90.66	88.26	87.13	1.13	87.18	1.08	87.12	1.14	87.15	1.11	87.14	1.12	87.13	1.13	87.12	1.14
S218	218	90.40	88.00	87.04	0.96	87.10	0.90	87.02	0.98	87.06	0.94	87.05	0.95	87.04	0.96	87.02	0.98
S219	219	90.08	87.68	86.85	0.83	86.94	0.74	86.82	0.86	86.88	0.80	86.86	0.82	86.84	0.84	86.81	0.87
S220	220	89.86	87.46	86.74	0.72	86.84	0.62	86.70	0.76	86.78	0.68	86.75	0.71	86.72	0.74	86.68	0.78
S221	221	89.88	87.48	86.57	0.91	86.72	0.76	86.51	0.97	86.63	0.85	86.59	0.89	86.54	0.94	86.36	1.12
S222	222	89.86	87.46	86.38	1.08	86.51	0.95	86.32	1.14	86.43	1.03	86.39	1.07	86.35	1.11	86.19	1.27
S200	200	94.71	92.31	90.73	1.58	90.74	1.57	90.73	1.58	90.72	1.59	90.73	1.58	90.72	1.59	90.73	1.58
S214	214	93.52	91.12	90.26	0.86	90.28	0.84	90.26	0.86	90.27	0.85	90.26	0.86	90.26	0.86	90.26	0.86
MH201	201	94.29	91.89	90.72	1.17	90.73	1.16	90.72	1.17	90.72	1.17	90.72	1.17	90.72	1.17	90.71	1.18
MH202	202	93.91	91.51	90.42	1.09	90.43	1.08	90.41	1.10	90.42	1.09	90.41	1.10	90.41	1.10	90.40	1.11
MH203	203	92.38	89.98	88.66	1.32	88.68	1.30	88.63	1.35	88.66	1.32	88.63	1.35	88.64	1.34	88.61	1.37
MH204	204	90.40	88.00	87.08	0.92	87.10	0.90	87.06	0.94	87.08	0.92	87.06	0.94	87.07	0.93	87.02	0.98
MH205	205	89.35	86.95	85.86	1.09	85.88	1.07	85.83	1.12	85.86	1.09	85.84	1.11	85.84	1.11	85.77	1.18
MH206	206	89.10	86.70	85.65	1.05	85.65	1.05	85.62	1.08	85.65	1.05	85.63	1.07	85.63	1.07	85.57	1.13
MH207	207	88.53	86.13	84.65	1.48	84.65	1.48	84.62	1.51	84.65	1.48	84.63	1.50	84.64	1.49	84.58	1.55
S212	212	90.25	87.85	86.86	0.99	86.87	0.98	86.83	1.02	86.85	1.00	86.83	1.02	86.84	1.01	86.82	1.03
S213	213	89.74	87.34	86.45	0.89	86.45	0.89	86.43	0.91	86.45	0.89	86.44	0.90	86.44	0.90	86.42	0.92
S210	210	89.14	86.74	86.43	0.31	86.43	0.31	86.42	0.32	86.43	0.31	86.42	0.32	86.43	0.31	86.41	0.33
S211	211	89.15	86.75	85.94	0.81	85.93	0.82	85.93	0.82	85.94	0.81	85.93	0.82	85.93	0.82	85.92	0.83
S208	208	88.77	86.37	85.92	0.45	85.91	0.46	85.78	0.59	85.91	0.46	85.81	0.56	85.88	0.49	85.70	0.67
S209	209	88.75	86.35	85.46	0.89	85.45	0.90	85.41	0.94	85.46	0.89	85.42	0.93	85.45	0.90	85.38	0.97
MH231	231	89.81	87.41	85.61	1.79	85.64	1.77	85.73	1.67	85.78	1.63	85.84	1.57	85.77	1.63	85.71	1.69

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996		
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	
<b>Wateridge Village Phase 1A</b>																		
S153	153	92.78	90.38	89.45	0.93	89.46	0.92	89.44	0.94	89.45	0.93	89.44	0.94	89.45	0.93	89.44	0.94	
S160	160	92.27	89.87	89.01	0.86	89.02	0.85	89.01	0.86	89.01	0.86	89.01	0.86	89.01	0.86	89.00	0.87	
S161	161	91.94	89.54	88.57	0.97	88.58	0.96	88.57	0.97	88.57	0.97	88.57	0.97	88.57	0.97	88.57	0.97	
S162	162	91.34	88.94	88.26	0.68	88.26	0.68	88.25	0.69	88.26	0.68	88.25	0.69	88.26	0.68	88.25	0.69	
S163	163	90.94	88.54	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	
S164	164	90.22	87.82	87.00	0.82	87.01	0.81	86.99	0.83	87.00	0.82	87.00	0.82	87.00	0.82	86.99	0.83	
S165B	165	89.61	87.21	86.45	0.76	86.45	0.76	86.44	0.77	86.44	0.77	86.44	0.77	86.44	0.77	86.44	0.77	
S165	165	89.30	86.90	85.98	0.92	86.05	0.85	85.93	0.97	86.01	0.89	85.99	0.91	85.96	0.94	85.83	1.07	
S166	166	88.90	86.50	84.88	1.62	85.03	1.47	84.78	1.72	84.93	1.57	84.88	1.62	84.85	1.65	84.59	1.91	
S167	167	88.40	86.00	84.71	1.29	84.86	1.14	84.60	1.40	84.76	1.24	84.71	1.29	84.67	1.33	84.39	1.61	
S168	168	87.70	85.30	84.54	0.76	84.66	0.64	84.43	0.87	84.58	0.72	84.54	0.76	84.50	0.80	84.22	1.08	
S141	141	87.32	84.92	84.28	0.64	84.39	0.53	84.18	0.74	84.32	0.60	84.28	0.64	84.25	0.67	83.97	0.95	
S142	142	87.52	85.12	84.02	1.10	84.12	1.00	83.94	1.18	84.06	1.06	84.03	1.09	84.00	1.12	83.74	1.38	
MH176	176	88.03	85.63	83.77	1.86	83.85	1.78	83.69	1.94	83.80	1.83	83.77	1.86	83.75	1.88	83.49	2.14	
MH178	178	89.00	86.60	83.41	3.19	83.47	3.13	83.34	3.26	83.44	3.16	83.41	3.19	83.39	3.21	83.18	3.42	
MH180	180	88.23	85.83	82.20	3.62	82.44	3.38	81.98	3.84	82.27	3.56	82.21	3.62	82.10	3.73	81.49	4.34	
MH190	190	88.10	85.70	81.90	3.80	82.12	3.58	81.65	4.05	81.97	3.73	81.91	3.79	81.80	3.90	81.23	4.47	
MH191	191	86.36	83.96	81.66	2.30	81.86	2.10	81.44	2.52	81.73	2.23	81.67	2.29	81.56	2.40	81.06	2.91	
MH192	192	85.92	83.52	81.41	2.11	81.59	1.93	81.21	2.31	81.47	2.05	81.41	2.11	81.31	2.21	80.89	2.63	
MH193	193	84.85	82.45	81.09	1.36	81.24	1.21	80.92	1.53	81.14	1.31	81.09	1.36	81.00	1.45	80.60	1.85	
MH194	194	82.44	N/A	80.45	N/A	80.53	N/A	80.35	N/A	80.48	N/A	80.46	N/A	80.40	N/A	80.13	N/A	
S130	130		N/A	101.25	N/A	101.25	N/A	101.24	N/A	101.25	N/A	101.24	N/A	101.24	N/A	101.23	N/A	
S131	131		N/A	101.05	N/A	101.05	N/A	101.04	N/A	101.05	N/A	101.04	N/A	101.04	N/A	101.03	N/A	
S132	132		N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.63	N/A	
S133	133		N/A	96.52	N/A	96.52	N/A	96.51	N/A	96.52	N/A	96.51	N/A	96.51	N/A	96.50	N/A	
S134	134		N/A	93.01	N/A	93.01	N/A	93.00	N/A	93.01	N/A	93.00	N/A	93.00	N/A	92.99	N/A	
S135	135		N/A	90.11	N/A	90.11	N/A	90.10	N/A	90.11	N/A	90.10	N/A	90.10	N/A	90.09	N/A	
S136	136		N/A	87.38	N/A	87.38	N/A	87.37	N/A	87.38	N/A	87.37	N/A	87.37	N/A	87.37	N/A	
S137	137			86.91	85.77	1.14	85.77	1.14	85.76	1.15	85.77	1.14	85.76	1.15	85.77	1.14	85.76	1.15
S138	138			86.31	84.96	1.35	84.96	1.35	84.95	1.36	84.96	1.35	84.95	1.36	84.95	1.36	84.94	1.37
S139	139			85.66	84.46	1.20	84.48	1.18	84.46	1.20	84.46	1.20	84.46	1.20	84.46	1.20	84.45	1.21
S140	140			N/A	84.35	N/A	84.42	N/A	84.34	N/A	84.37	N/A	84.35	N/A	84.34	N/A	84.34	N/A
S100	100			87.16	85.70	1.46	85.69	1.47	85.70	1.46	85.70	1.46	85.70	1.46	85.70	1.46	85.70	1.46
S108	108			86.66	85.24	1.43	85.23	1.43	85.23	1.43	85.24	1.42	85.23	1.43	85.23	1.43	85.23	1.43
S109	109			85.36	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31
S117	117			85.06	83.54	1.52	83.58	1.48	83.53	1.53	83.54	1.52	83.53	1.53	83.54	1.52	83.53	1.53
S118	118			84.71	83.21	1.50	83.48	1.23	83.20	1.51	83.25	1.46	83.22	1.49	83.21	1.50	83.20	1.51
S101	101			87.16	85.55	1.61	85.55	1.61	85.54	1.62	85.55	1.61	85.54	1.62	85.54	1.62	85.54	1.62
S102	102			86.46	84.72	1.74	84.72	1.74	84.71	1.75	84.72	1.74	84.71	1.75	84.71	1.75	84.70	1.76
S119	119			85.46	83.95	1.51	83.95	1.51	83.95	1.51	83.95	1.51	83.94	1.52	83.95	1.51	83.95	1.51
S104	104			N/A	85.90	N/A	85.89	N/A	85.89	N/A	85.90	N/A	85.89	N/A	85.89	N/A	85.88	N/A

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
S103	103		86.46	84.36	2.10	84.36	2.10	84.34	2.12	84.36	2.10	84.35	2.11	84.35	2.11	84.34	2.12
S105	105		85.71	83.90	1.81	83.91	1.80	83.89	1.82	83.90	1.81	83.89	1.82	83.90	1.81	83.89	1.82
S122	122		84.86	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33
S121	121		84.26	82.80	1.46	83.03	1.23	82.43	1.83	82.82	1.44	82.77	1.49	82.61	1.65	81.98	2.28
S127	127		84.36	82.67	1.69	82.92	1.44	82.34	2.02	82.71	1.65	82.66	1.70	82.51	1.85	81.85	2.51
S128	128		N/A	82.61	N/A	82.86	N/A	82.30	N/A	82.67	N/A	82.61	N/A	82.47	N/A	81.81	N/A
S107	107		N/A	85.29	N/A	85.29	N/A	85.28	N/A	85.29	N/A	85.28	N/A	85.28	N/A	85.27	N/A
S106	106		85.61	83.76	1.85	83.75	1.86	83.73	1.88	83.76	1.85	83.74	1.87	83.75	1.86	83.73	1.88
S124	124		85.69	83.94	1.75	83.94	1.75	83.93	1.76	83.94	1.75	83.93	1.76	83.93	1.76	83.92	1.77
S125	125		85.34	83.37	1.97	83.38	1.96	83.35	1.99	83.37	1.97	83.36	1.98	83.36	1.98	83.35	1.99
S126	126		84.96	82.87	2.09	83.14	1.82	82.85	2.11	82.89	2.07	82.85	2.11	82.86	2.10	82.84	2.12
S182	182		N/A	82.46	N/A	82.70	N/A	82.18	N/A	82.52	N/A	82.46	N/A	82.32	N/A	81.68	N/A
S181	181		N/A	82.36	N/A	82.61	N/A	82.11	N/A	82.43	N/A	82.37	N/A	82.24	N/A	81.61	N/A
S110	110		85.56	83.59	1.97	83.80	1.76	83.59	1.97	83.59	1.97	83.59	1.97	83.59	1.97	83.59	1.97
S111	111		84.96	83.59	1.37	83.80	1.16	83.58	1.38	83.59	1.37	83.58	1.38	83.59	1.37	83.58	1.38
S112	112		84.91	83.40	1.52	83.77	1.14	83.18	1.73	83.50	1.41	83.42	1.49	83.22	1.69	83.22	1.69
S113	113		84.51	83.41	1.10	83.74	0.77	83.06	1.45	83.48	1.03	83.40	1.11	83.08	1.43	83.05	1.46
S114	114		83.91	83.06	0.85	83.31	0.60	82.66	1.25	83.11	0.80	83.04	0.87	82.85	1.06	82.49	1.42
S115	115		83.56	83.04	0.52	83.33	0.23	82.64	0.92	83.13	0.43	83.01	0.55	82.83	0.73	82.45	1.11
S116	116		83.71	82.88	0.83	83.16	0.55	82.51	1.20	82.92	0.79	82.85	0.86	82.70	1.01	82.10	1.61
S120	120		83.96	82.86	1.10	83.08	0.88	82.48	1.48	82.88	1.08	82.83	1.13	82.67	1.29	82.06	1.90

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
<b>Phase 2B, 4</b>																	
MH317	317	94.08	91.68	91.17	0.51	91.18	0.50	91.14	0.54	91.15	0.53	91.15	0.53	91.14	0.54	91.11	0.57
MH316	316	94.09	91.69	90.96	0.73	90.96	0.73	90.95	0.74	90.95	0.74	90.95	0.74	90.95	0.74	90.92	0.77
MH315	315	93.39	91.36	90.28	1.08	90.29	1.07	90.25	1.11	90.26	1.10	90.27	1.09	90.27	1.09	90.26	1.10
MH314	314	93.00	91.16	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.89	1.27
MH313	313	92.62	90.71	89.35	1.36	89.34	1.37	89.35	1.36	89.35	1.36	89.35	1.36	89.35	1.36	89.34	1.37
MH312	312	91.36	89.68	88.42	1.26	88.42	1.26	88.41	1.27	88.42	1.26	88.42	1.26	88.42	1.26	88.38	1.30
MH311	311	90.69	88.29	87.44	0.85	87.56	0.73	87.40	0.89	87.48	0.81	87.45	0.84	87.47	0.82	87.38	0.91
MH310	310	90.04	87.64	87.28	0.36	87.42	0.22	87.25	0.39	87.35	0.29	87.30	0.34	87.33	0.31	87.06	0.58
MH309	309	90.15	87.75	87.44	0.31	87.08	0.67	87.33	0.42	87.44	0.31	87.41	0.34	87.43	0.32	87.22	0.53
MH308	308	89.68	87.28	86.88	0.40	86.69	0.59	86.81	0.47	86.88	0.40	86.87	0.41	86.88	0.40	86.76	0.52
MH326	326	94.76	92.36	91.33	1.03	91.33	1.03	91.32	1.04	91.32	1.04	91.32	1.04	91.32	1.04	91.33	1.03
MH318	318	94.40	92.00	91.03	0.97	91.03	0.97	91.00	1.00	91.03	0.97	91.00	1.00	91.00	1.00	91.00	1.00
MH300	300	94.00	91.60	90.71	0.89	90.70	0.90	90.67	0.93	90.70	0.90	90.68	0.92	90.68	0.92	90.68	0.92
MH301	301	93.73	91.33	90.21	1.12	90.21	1.12	90.20	1.13	90.20	1.13	90.21	1.12	90.20	1.13	90.20	1.13
MH302	302	92.80	90.40	88.64	1.76	88.64	1.76	88.63	1.77	88.63	1.77	88.64	1.76	88.63	1.77	88.63	1.77
MH303	303	90.67	88.27	87.80	0.47	87.81	0.46	87.63	0.64	87.65	0.62	87.79	0.48	87.72	0.55	87.64	0.63
MH304	304	90.30	87.90	87.39	0.51	87.38	0.52	87.30	0.60	87.31	0.59	87.38	0.52	87.34	0.56	87.30	0.60
MH305	305	91.00	88.60	86.54	2.06	86.56	2.04	86.61	1.99	86.64	1.96	86.69	1.91	86.65	1.95	86.60	2.00
MH319	319	88.81	86.61	86.13	0.48	86.12	0.49	86.12	0.49	86.13	0.48	86.12	0.49	86.12	0.49	86.12	0.49
MH320	320	89.12	86.92	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43
MH321	321	87.67	85.47	84.18	1.29	84.39	1.08	84.10	1.37	84.15	1.32	84.11	1.36	84.13	1.34	84.09	1.38
MH322	322	87.50	85.30	84.18	1.12	84.39	0.91	84.10	1.20	84.15	1.15	84.10	1.20	84.12	1.18	84.09	1.21
MH323	323	86.57	84.37	83.40	0.97	83.48	0.89	83.31	1.06	83.37	1.00	83.32	1.05	83.34	1.03	83.30	1.07

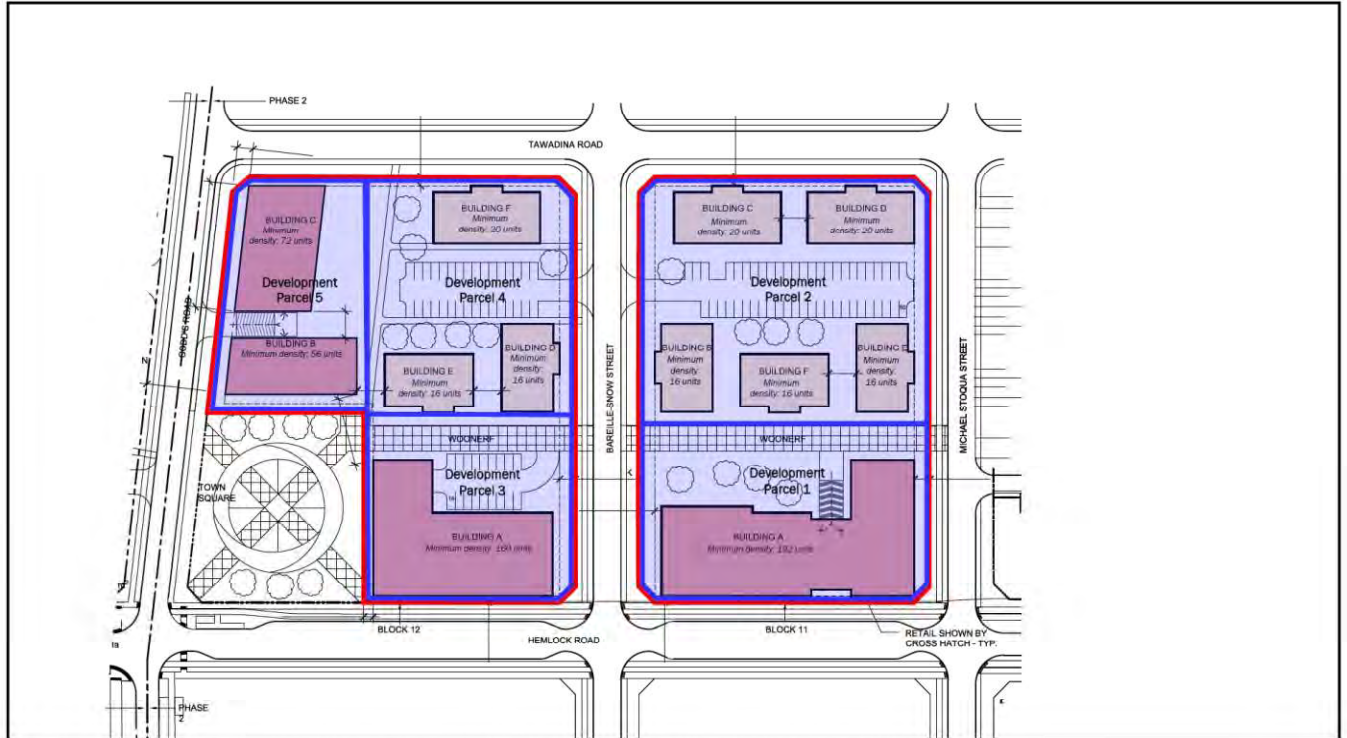
## **Appendix B**

Supporting Sanitary Information

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# SCHEDULE "A"

## PARCEL IDENTIFICATION, DESCRIPTION, AND MINIMUM DENSITY<sup>1</sup>



\*\*Boundaries of the development parcels are estimated. Purchasers to provide dimensioned sketch or electronic survey to confirm these boundaries

<sup>1</sup> This image if provided for demonstration purposes only



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Block 11&12 Proposed Conditions  
 Old Criteria being used

AS-BUILT SANITARY SEWER DESIGN SHEET

Former CFB Rockcliffe  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL								ICI AREAS						INFILTRATION ALLOWANCE		FIXED FLOW	TOTAL FLOW	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA Phase 1B (Ha)	UNIT TYPES				AREA EXTERNAL (Ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)			PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY		
					SF	SD	TH	APT		IND	CUM			IND	COMMERCIAL	INDUSTRIAL		IND	CUM									IND	CUM	L/s
Phase 1B																														
Phase 1B																														
Phase 1B	EX205A	BULK205AN	MH205A					0.66	33.1	33.1	4.00	0.54		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.72	66.24	21.00	250	1.14	1.307	65.52	98.91%	
Hemlock Road	205A	MH205A	MH206A	0.25					0.0	186.6	4.00	3.02		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.73	31.02	111.90	250	0.25	0.612	27.29	87.99%	
Phase 1B	EX206A-B	BULK206AN	MH206A					9.79	2598.3	2598.3	3.49	36.78		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	39.52	88.83	21.00	250	2.05	1.753	49.30	55.50%	
Hemlock Road	206A	MH206A	MH207A	0.20					0.0	2784.9	3.47	39.14		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	42.64	100.88	89.30	300	1.00	1.383	58.24	57.73%	
Block 20	PARK1	MH207AN	MH207A	0.32					0.0	0.0	4.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.09	50.02	13.80	250	0.65	0.987	49.93	99.82%		
Hemlock Road	PARK1, 207A	MH207A	BULK176AE	0.12					0.0	2784.9	3.47	39.14		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	42.77	134.59	33.70	300	1.78	1.845	91.83	68.23%	
Phase 1A																														
Phase 1A																														
Phase 1A	BULK176AE	MH176A							0.0	2784.9	3.47	39.14		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	42.77	65.38	21.97	300	0.42	0.896	22.61	34.59%	
Phase 1B																														
Phase 1B																														
chemin Wanaki Road	200A, COM1	MH200A	MH214A	0.25					0.0	0.0	4.00	0.00		0.00	0.90	0.90	0.00	0.78	1.15	1.15	0.32	0.00	1.10	71.01	98.50	250	1.31	1.401	69.90	98.45%
chemin Wanaki Road	214A, COM2	MH214A	BULK153AN	0.16					0.0	0.0	4.00	0.00		0.00	0.65	1.55	0.00	1.35	0.81	1.96	0.55	0.00	1.89	57.20	44.60	250	0.85	1.129	55.30	96.69%
Phase 1A																														
Phase 1A																														
chemin Wanaki Road	COM2	BULK153AN	MH153A						0.0	0.0	4.00	0.00		0.00	1.55	0.00	0.00	1.35	0.00	1.96	0.55	0.00	1.89	51.91	20.13	250	0.70	1.024	50.01	96.35%
chemin Wanaki Road	153A, COM3	MH153A	MH151A	0.21					0.0	0.0	4.00	0.00		0.00	0.88	2.43	0.00	2.11	1.09	3.05	0.85	0.00	2.96	36.70	85.04	250	0.35	0.724	33.74	91.93%
chemin Wanaki Road	151A, COM4	MH151A	MH150A	0.11					0.0	0.0	4.00	0.00		0.00	0.45	2.88	0.00	2.50	0.56	3.61	1.01	0.00	3.51	36.70	40.97	250	0.35	0.724	33.19	90.43%
chemin Wanaki Road	150A, COM5	MH150A	MH149A	0.11					0.0	0.0	4.00	0.00		0.00	0.95	3.83	0.00	3.32	1.06	4.67	1.31	0.00	4.63	36.70	41.34	250	0.35	0.724	32.07	87.38%
chemin Wanaki Road	149A	MH149A	MH148A	0.10					0.0	0.0	4.00	0.00		0.00	3.83	0.00	0.00	3.32	0.10	4.77	1.34	0.00	4.66	36.70	40.04	250	0.35	0.724	32.04	87.30%
chemin Wanaki Road	148A	MH148A	MH157A	0.04					0.0	0.0	4.00	0.00		0.00	3.83	0.00	0.00	3.32	0.04	4.81	1.35	0.00	4.67	36.70	20.58	250	0.35	0.724	32.03	87.27%
Phase 1B																														
Phase 1B																														
chemin Wanaki Road	143B	BULK143AE	MH143A	0.31					104.0	104.0	4.00	1.69		0.00	0.00	0.00	0.00	0.00	0.31	0.31	0.09	0.00	1.77	43.87	21.50	250	0.50	0.866	42.10	95.96%
chemin Wanaki Road	143A	MH143A	MH144A	0.27					0.0	104.0	4.00	1.69		0.00	0.00	0.00	0.00	0.00	0.27	0.58	0.16	0.00	1.85	83.69	34.70	250	1.82	1.652	81.85	97.79%
chemin Wanaki Road	144A, 144B	MH144A	MH145A	0.72					0.0	104.0	4.00	1.69		0.00	0.00	0.00	0.00	0.00	0.72	1.30	0.36	0.00	2.05	88.61	41.10	250	2.04	1.749	86.56	97.69%
chemin Wanaki Road	145A, 145B, 145C	MH145A	MH146A	2.77					835.6	939.6	3.82	14.53		0.00	0.00	0.00	0.00	0.00	2.77	4.07	1.14	0.00	15.67	105.83	53.30	250	2.91	2.089	90.16	85.19%
chemin Wanaki Road	146A	MH146A	MH147A	0.14					0.0	939.6	3.82	14.53		0.00	0.00	0.00	0.00	0.00	0.14	4.21	1.18	0.00	15.71	43.54	37.30	250	0.97	1.206	27.83	63.92%
chemin Wanaki Road	PARK2	BLK147AE	MH147A	0.55					0.0	0.0	4.00	0.00		0.00	0.00	0.00	0.00	0.00	0.55	0.55	0.15	0.00	0.15	39.24	17.70	250	0.40	0.774	39.08	99.61%
chemin Wanaki Road	147C	BLK147AW	MH147A	0.10					33.6	33.6	4.00	0.54		0.00	0.00	0.00	0.00	0.00	0.10	0.10	0.03	0.00	0.57	41.62	17.70	250	0.45	0.821	41.04	98.62%
chemin Wanaki Road	147A	MH147A	MH170A	0.03					0.0	973.2	3.81	15.01		0.00	0.00	0.00	0.00	0.00	0.03	4.89	1.37	0.00	16.38	38.74	10.30	250	0.39	0.765	22.36	57.72%
chemin Wanaki Road	147B	MH170A	MH147C	0.16					0.0	973.2	3.81	15.01		0.00	0.00	0.00	0.00	0.00	0.16	5.05	1.41	0.00	16.42	31.63	38.20	250	0.26	0.624	15.21	48.08%
chemin Wanaki Road		MH147C	BLK148AW						0.0	973.2	3.81	15.01		0.00	0.00	0.00	0.00	0.00	0.00	5.05	1.41	0.00	16.42	46.01	11.80	250	0.55	0.908	29.58	64.30%
Phase 1A																														
Phase 1A																														
chemin Wanaki Road	BULK148AW	MH157A							0.0	973.2	3.81	15.01		0.00	0.00	0.00	0.00	0.00	0.00	5.05	1.41	0.00	16.42	62.04	8.00	250	1.00	1.224	45.61	73.52%
chemin Wanaki Road	157A	MH157A	MH158A	0.05					0.0	973.2	3.81	15.01		0.00	3.83	0.00	0.00	3.32	0.05	9.91	2.77	0.00	21.11	31.02	25.68	250	0.25	0.612	9.91	31.94%
Street No. 2	INST1	BULK158AN	MH158A						0.0	0.0	4.00	0.00		2.62	2.62	0.00	0.00	0.00	2.27	2.62	0.73	0.00	3.01	39.24	15.10	250	0.40	0.774	36.23	92.33%
chemin Wanaki Road	158A	MH158A	MH154A	0.22					0.0	973.2	3.81	15.01		2.62	3.83	0.00	0.00	5.60	0.22	12.75	3.57	0.00	24.18	31.02	68.91	250	0.25	0.612	6.84	22.05%





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Block 11&12 Proposed Conditions  
 Old Criteria being used

AS-BUILT SANITARY SEWER DESIGN SHEET

Former CFB Rockcliffe  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE		FIXED FLOW	TOTAL FLOW	PROPOSED SEWER DESIGN														
STREET	AREA ID	FROM MH	TO MH	AREA Phase 1B (Ha)	UNIT TYPES				AREA EXTERNAL (Ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY							
					SF	SD	TH	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM		IND	CUM									L/s	(%)						
Phase 1B																																						
Block 9	154A	Ex. BULK	MH217Aa	0.19						0.0	973.2	3.81	15.01		2.62		3.83		0.00	5.60	0.19	12.94	3.62	0.00	24.23	104.37	24.40	250	2.83	2.060	80.13	76.78%						
Block 9		MH217Aa	MH217A							0.0	973.2	3.81	15.01		2.62		3.83		0.00	5.60	0.00	12.94	3.62	0.00	24.23	62.66	78.50	250	1.02	1.237	38.42	61.32%						
croissant Squadron Crescent	215Aa-b	MH215A	MH216A	0.79	3	4				117.8	117.8	4.00	1.91		0.00		0.00		0.00	0.00	0.79	0.79	0.22	0.00	2.13	55.49	56.10	250	0.80	1.095	53.36	96.16%						
croissant Squadron Crescent	216Aa-b	MH216A	MH217A	0.67	2	6				94.5	212.3	4.00	3.44		0.00		0.00		0.00	0.00	0.67	1.46	0.41	0.00	3.85	46.01	70.80	250	0.55	0.908	42.16	91.63%						
croissant Squadron Crescent	217A	MH217A	MH218A	0.02						0.0	1185.5	3.75	18.01		2.62		3.83		0.00	5.60	0.02	14.42	4.04	0.00	27.65	39.72	9.70	250	0.41	0.784	12.07	30.39%						
croissant Squadron Crescent	218A	MH218A	MH218B	0.02						0.0	1185.5	3.75	18.01		2.62		3.83		0.00	5.60	0.02	14.44	4.04	0.00	27.66	39.24	9.90	250	0.40	0.774	11.58	29.51%						
Thorncliffe Village	THORN1	MH600A	MH601A							5.55	1574.0	3.66	23.36		0.00		0.00		0.00	0.00	5.55	19.99	5.60	0.00	28.96	69.16	21.40	300	0.47	0.948	40.20	58.12%						
Thorncliffe Village		MH601A	MH218B							0.0	1574.0	3.66	23.36		0.00		0.00		0.00	0.00	0.00	19.99	5.60	0.00	28.96	108.18	46.90	300	1.15	1.483	79.22	73.23%						
croissant Squadron Crescent	218B	MH218B	MH219A	0.07						0.0	2759.5	3.47	38.82		2.62		3.83		0.00	5.60	0.07	34.50	9.66	0.00	54.08	96.76	40.20	300	0.92	1.326	42.68	44.11%						
croissant Squadron Crescent	219A	MH219A	MH220A	0.15						0.0	2759.5	3.47	38.82		2.62		3.83		0.00	5.60	0.15	34.65	9.70	0.00	54.12	66.92	72.40	300	0.44	0.917	12.79	19.12%						
croissant Squadron Crescent	220A, 220B	MH220A	MH221A	1.46						319.0	3078.5	3.43	42.81		2.62		3.83		0.00	5.60	1.46	36.11	10.11	0.00	58.52	74.82	43.30	300	0.55	1.025	16.30	21.78%						
croissant Squadron Crescent	221A	MH221A	MH222A	0.02						0.0	3078.5	3.43	42.81		2.62		3.83		0.00	5.60	0.02	36.13	10.12	0.00	58.53	64.60	7.40	300	0.41	0.885	6.07	9.40%						
croissant Squadron Crescent		MH222A	MH223A							0.0	3078.5	3.43	42.81		2.62		3.83		0.00	5.60	0.00	36.13	10.12	0.00	58.53	58.82	81.60	300	0.34	0.806	0.30	0.51%						
croissant Squadron Crescent	BLOCK 15	BLK223AE	MH223A											Design by Others																								
croissant Squadron Crescent	222A	MH223A	MH165A	0.22						0.0	3078.5	3.43	42.81		2.62		3.83		0.00	5.60	0.22	36.35	10.18	0.00	58.59	96.24	36.10	300	0.91	1.319	37.65	39.12%						
<b>Design Parameters:</b>				<b>Notes:</b>										<b>Designed:</b>						<b>No.</b>		<b>Revision</b>						<b>Date</b>										
Residential				1. Mannings coefficient (n) = 0.013										WY						1.		City submission No. 1						2016-07-08										
SF 3.4 p/p/u				2. Demand (per capita): 350 L/day										JIM						2.		City submission No. 2						2016-11-04										
TH/SD 2.7 p/p/u				3. Infiltration allowance: 0.28 L/s/Ha										Dwg. Reference: 38298-501						3.		City submission No. 3						2017-01-25										
APT 1.8 p/p/u				4. Residential Peaking Factor: Harmon Formula = 1+(4+P^0.5) where P = population in thousands																4.		Revised as per Mattamy's Design						2017-12-08										
Other 60 p/p/Ha																				5.		As-Built Submission						2018-01-29										
																				6.		Block 11 & 12 Study						2022-03-15										
																				File Reference:		Date:						Sheet No:										
																				38298.5.7.1		2016-07-08						1 of 2										



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**LEGEND**  
 MH231A Existing infrastructure (shown for information only)  
 Block 11, 12 Existing Conditions

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS								INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN						
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	L/s
Pimiwidon Street	MH317-1, MH317-2	MH317A	MH316A	1.50	1	104			284.2	284.2	3.47	3.20	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.50	1.50	0.50	0.00	0.00	3.69	40.68	83.00	250	0.43	0.803	36.99	90.93%	
Pimiwidon Street	MH316A	MH316A	BULK202AN	0.16		1			2.7	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.16	1.66	0.55	0.00	0.00	3.77	37.74	43.10	250	0.37	0.745	33.96	90.00%	
Pimiwidon Street	-	BULK202AN	MH202A						0.0	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.66	0.55	0.00	0.00	3.77	40.68	21.00	250	0.43	0.803	36.91	90.72%	
Wigwas Street	MH315A	MH315A	MH314A	0.79	2	18			55.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.79	0.79	0.26	0.00	0.00	0.92	49.63	111.64	250	0.64	0.979	48.72	98.16%	
Wigwas Street	MH314A	MH314A	BULK203AN	0.06					0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.06	0.85	0.28	0.00	0.00	0.93	83.46	14.37	250	1.81	1.647	82.53	98.88%	
Wigwas Street	-	BULK203AN	MH203A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.85	0.28	0.00	0.00	0.93	80.17	21.00	250	1.67	1.582	79.24	98.83%	
Moses Tennisco Street	MH313A	MH313A	MH312A	0.66	2	16			50.0	50.0	3.65	0.59	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.66	0.66	0.22	0.00	0.00	0.81	75.73	77.20	250	1.49	1.495	74.92	98.93%	
Moses Tennisco Street	MH312A, PARK	MH312A	BULK204AN	0.21		2			5.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.87	0.29	0.00	0.00	0.94	94.29	49.70	250	2.31	1.861	93.35	99.00%	
Park	PARK	MH350A	pipe	0.42					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.42	0.42	0.14	0.00	0.00	0.14	48.39	11.00	200	2.00	1.492	48.25	99.71%	
Moses Tennisco Street	-	BULK204AN	MH204A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.87	0.29	0.00	0.00	0.94	89.90	21.00	250	2.10	1.774	88.96	98.95%	
Michael Stoqua Street	MH311A	MH311A	MH310A	0.44	1	9			27.7	27.7	3.69	0.33	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.44	0.44	0.15	0.00	0.00	0.48	72.35	77.82	250	1.36	1.428	71.87	99.34%	
Michael Stoqua Street	MH310A	MH310A	BULK205AN	0.21		2			5.4	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.65	0.21	0.00	0.00	0.61	65.66	49.19	250	1.12	1.296	65.05	99.07%	
Michael Stoqua Street	-	BULK205AN	MH205A						0.0	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.65	0.21	0.00	0.00	0.61	66.24	21.00	250	1.14	1.307	65.63	99.08%	
Wanaki Road	MH200A	MH200A	MH318A						0.0	0.0	3.80	0.00	0.00	0.00	1.01	1.01	0.00	0.00	1.50	0.49	1.01	1.01	0.33	0.00	0.00	0.82	42.53	63.35	250	0.47	0.839	41.71	98.06%	
Wanaki Road	MH318A	MH318A	MH300A						0.0	0.0	3.80	0.00	0.00	0.95	1.96	0.00	0.00	1.50	0.95	0.95	1.96	0.85	0.00	0.00	1.60	42.53	77.11	250	0.47	0.839	40.93	96.24%		
Tawadina Road	MH300A	MH300A	MH301A	0.47		15			40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%	
Tawadina Road	MH301A	MH301A	MH302A	0.54		14			37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%	
Tawadina Road	MH302A	MH302A	MH303A	0.26		2			5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%	
Tawadina Road	MH303A	MH303A	MH304A	0.21					0.0	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.21	3.44	1.14	0.00	0.00	3.07	31.02	112.10	250	0.25	0.612	27.95	90.11%	
Tawadina Road	MH305A	MH305A	MH304A	0.24					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Barelle-Snow Street	EXT-1	BULK304AN	MH304A	7.35				905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%	
Barelle-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	1.47				190	342.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	1.47	12.50	4.13	0.00	0.00	25.14	39.72	119.21	250	0.41	0.784	14.58	36.70%	
Barelle-Snow Street	MH308A	MH308A	BULK206AN	0.07					0.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.07	12.57	4.15	0.00	0.00	25.17	84.15	16.82	250	1.84	1.661	58.99	70.09%	
Barelle-Snow Street	-	BULK206AN	MH206A						0.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	12.57	4.15	0.00	0.00	25.17	88.83	21.00	250	2.05	1.753	63.66	71.67%	
Codd's Road	MH340A	MH340A	BLK231AN	1.78				278	500.4	500.4	3.38	5.48	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.78	1.78	0.59	0.00	0.00	6.07	75.98	70.00	250	1.50	1.500	69.91	92.01%	
Codd's Road	-	MH231A	BULK176AN						0.0	500.4	3.38	5.48	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.78	0.59	0.00	0.00	6.07	83.92	50.22	250	1.83	1.656	77.86	92.77%	

<b>Design Parameters:</b>				<b>Notes:</b>				<b>Designed:</b>				<b>Revision</b>				<b>Date</b>			
Residential		ICI Areas		1. Mannings coefficient (n) = 0.013		280 L/day		200 L/day		KH		1		Submission No. 1 for City Review		2018-12-20			
SF	3.4 p/p/u			3. Infiltration allowance: 0.33 L/s/Ha						Checked: JIM		2		Submission No. 2 for City Review		2019-03-15			
TH/F/SD	2.7 p/p/u	INST	28,000 L/Ha/day	4. Residential Peaking Factor:		Harmon Formula = 1+(14/(4+(P/1000)^0.5))0.8		where K = 0.8 Correction Factor		Dwg. Reference: 118863-400		3		MECP Submission		2019-04-17			
TH/S	2.3 p/p/u	COM	28,000 L/Ha/day	5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0								4		Record information Added (No.1)		2020-10-08			
APT	1.8 p/p/u	IND	35,000 L/Ha/day									5		Record information Added (No.2)		2021-03-23			
Other	60 p/p/Ha		17000 L/Ha/day									6		Block 11 & 12 Study		2022-03-15			
												<b>File Reference:</b> 118863.5.7.1		<b>Date:</b> 2021-03-31		<b>Sheet No:</b> 1 of 1			



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**LEGEND**  
 MH231A Existing infrastructure (shown for information only)  
 Block 11&12 Proposed Conditions

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		AREA (Ha)		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	(%)
Pimiwidon Street	MH317-1, MH317-2	MH317A	MH316A	1.50	1	104			284.2	284.2	3.47	3.20	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.50	1.50	0.50	0.00	0.00	3.69	40.68	83.00	250	0.43	0.803	36.99	90.93%	
Pimiwidon Street	MH316A	MH316A	BULK202AN	0.16		1			2.7	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.16	1.66	0.55	0.00	0.00	3.77	37.74	43.10	250	0.37	0.745	33.96	90.00%	
Pimiwidon Street	-	BULK202AN	MH202A						0.0	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.66	0.55	0.00	0.00	3.77	40.68	21.00	250	0.43	0.803	36.91	90.72%	
Wigwas Street	MH315A	MH315A	MH314A	0.79	2	18			55.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.79	0.79	0.26	0.00	0.00	0.92	49.63	111.64	250	0.64	0.979	48.72	98.16%	
Wigwas Street	MH314A	MH314A	BULK203AN	0.06					0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.06	0.85	0.28	0.00	0.00	0.93	83.46	14.37	250	1.81	1.647	82.53	98.88%	
Wigwas Street	-	BULK203AN	MH203A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.85	0.28	0.00	0.00	0.93	80.17	21.00	250	1.67	1.582	79.24	98.83%	
Moses Tennisco Street	MH313A	MH313A	MH312A	0.66	2	16			50.0	50.0	3.65	0.59	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.66	0.66	0.22	0.00	0.00	0.81	75.73	77.20	250	1.49	1.495	74.92	98.93%	
Moses Tennisco Street	MH312A, PARK	MH312A	BULK204AN	0.21		2			5.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.87	0.29	0.00	0.00	0.94	94.29	49.70	250	2.31	1.861	93.35	99.00%	
Park	PARK	MH350A	pipe	0.42					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.42	0.42	0.14	0.00	0.00	0.14	48.39	11.00	200	2.00	1.492	48.25	99.71%	
Moses Tennisco Street	-	BULK204AN	MH204A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.87	0.29	0.00	0.00	0.94	89.90	21.00	250	2.10	1.774	88.96	98.95%	
Michael Stoqua Street	MH311A	MH311A	MH310A	0.44	1	9			27.7	27.7	3.69	0.33	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.44	0.44	0.15	0.00	0.00	0.48	72.35	77.82	250	1.36	1.428	71.87	99.34%	
Michael Stoqua Street	MH310A	MH310A	BULK205AN	0.21		2			5.4	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.65	0.21	0.00	0.00	0.61	65.66	49.19	250	1.12	1.296	65.05	99.07%	
Michael Stoqua Street	-	BULK205AN	MH205A						0.0	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.65	0.21	0.00	0.00	0.61	66.24	21.00	250	1.14	1.307	65.63	99.08%	
Wanaki Road	MH200A	MH200A	MH318A						0.0	0.0	3.80	0.00	0.00	0.00	1.01	1.01	0.00	0.00	1.50	0.49	1.01	1.01	0.33	0.00	0.00	0.82	42.53	63.35	250	0.47	0.839	41.71	98.06%	
Tawadina Road	MH300A	MH300A	MH301A	0.47		15			40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%	
Tawadina Road	MH301A	MH301A	MH302A	0.54		14			37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%	
Tawadina Road	MH302A	MH302A	MH303A	0.26		2			5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%	
Tawadina Road	MH303A	MH303A	MH304A	0.21					0.0	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.21	3.44	1.14	0.00	0.00	3.07	31.02	112.10	250	0.25	0.612	27.95	90.11%	
Tawadina Road	MH305A	MH305A	MH304A	0.24					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Bareille-Snow Street	EXT-1	BULK304AN	MH304A	7.35				905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%	
Bareille-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	1.48				140	252.0	1964.7	3.07	19.57	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	1.48	12.51	4.13	0.00	0.00	24.33	39.72	119.21	250	0.41	0.784	15.39	38.75%	
Bareille-Snow Street	MH308A	MH308A	BULK206AN	0.96				352	633.6	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.96	13.47	4.45	0.00	0.00	30.31	84.15	16.82	250	1.84	1.661	53.85	63.99%	
Bareille-Snow Street	-	BULK206AN	MH206A						0.0	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	13.47	4.45	0.00	0.00	30.31	88.83	21.00	250	2.05	1.753	58.52	65.88%	
Codd's Road	MH340A	MH340A	BLK231AN	0.88				212	381.6	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	75.98	70.00	250	1.50	1.500	71.46	94.04%	
Codd's Road	MH231A	MH231A	BULK176AN						0.0	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.88	0.29	0.00	0.00	4.53	83.92	50.22	250	1.83	1.656	79.40	94.61%	

**Design Parameters:**

Residential	ICI Areas	
SF 3.4 p/p/u		
TH/F/SD 2.7 p/p/u	INST 28,000 L/Ha/day	
TH/S 2.3 p/p/u	COM 28,000 L/Ha/day	
APT 1.8 p/p/u	IND 35,000 L/Ha/day	MOE Chart
Other 60 p/p/Ha	17000 L/Ha/day	

**Notes:**

- Mannings coefficient (n) = 0.013
- Demand (per capita): 280 L/day
- Infiltration allowance: 0.33 L/s/Ha
- Residential Peaking Factor: Harmon Formula =  $1 + (14 / (4 + (P/1000)^{0.5})) \cdot 0.8$  where K = 0.8 Correction Factor
- Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0

**Designed:** KH

**Checked:** JIM

**Dwg. Reference:** 118863-400

No.	Revision	Date
1	Submission No. 1 for City Review	2018-12-20
2	Submission No. 2 for City Review	2019-03-15
3	MECP Submission	2019-04-17
4	Record information Added (No.1)	2020-10-08
5	Record information Added (No.2)	2021-03-23
6	Block 11 & 12 Study	2022-03-15

**File Reference:** 118863.5.7.1

**Date:** 2021-03-31

**Sheet No:** 1 of 1



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# Technical Memorandum

**To/Attention** Mary Jarvis - Canada Lands Company **Date** November 23, 2022  
**From** Jim Moffatt – IBI Group **Project No** 118863-2.0  
**cc** Krisendat Sewgoolam - Canada Lands Company  
Meghan Black - IBI Group  
Anton Chetrar - IBI Group  
**Subject** Block 11 - Parcel 1 Site Plan Submission  
Wateridge Village Phase 2B

## Introduction

This technical memorandum has been prepared for Canada Lands Company and includes a review of the proposed site plan for Parcel 1 at Block 11 in Phase 2B of the Wateridge Village community. The review is based on the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing prepared by IBI Group dated April 26, 2022, also included in **Appendix A**.

**Figure 1**, in the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing, shows the location site plan for Parcel 1 at Block 11 for which DesignWorks Engineering is seeking approvals. Parcel 1 in Block 11 is surrounded by Tawadina Street to the north, Bareille-Snow Street to the west, Parcel 2 to the south and Michael Stoqua Street to the east. The plan consists of two 9-storey residential buildings with one level of underground parking.

The DesignWorks Engineering site plan shows different storm and sanitary servicing outlets than the ones provided by the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing dated April 26, 2022. This memorandum will outline the impacts on wastewater disposal and a review of the water supply and low impact development for the proposed development. In terms of management of stormwater, the proposed design was compared to the aforementioned April 2022 IBI memo.

## Sanitary Servicing

As stated previously, our review will be based on the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing prepared by IBI Group dated April 26, 2022.

In the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing, Parcel 1 in Block 11 is proposed to outlet into the sanitary sewer system on Barreille-Snow Street, north of MH308A. On the site plan submitted by DesignWorks Engineering for parcel, the sanitary sewer is proposed to outlet on Tawadina Street, west of MH304A.

An analysis of the ability of the existing sanitary sewer system in Tawadina Street to accommodate the flows from Parcel 1 in Block 11 was also completed. This analysis is included on the updated sanitary sewer spreadsheet included in **Appendix B**. The updated spreadsheet was based not only on the current City of Ottawa wastewater criteria, which came into effect in 2018 but also on the proposed site plan as submitted by DesignWorks Engineering. The following **Table 1** provides a review of the impacts of this change and the ability of the sanitary sewers to accept and convey any changes in flows.

Mary Jarvis – November 23, 2022

**Table 1: Sanitary Flow vs Sewer Capacity Analysis**

Street Location	Original Plan			Final DesignWorks Plan			Sewer Design		
	Units	Total Popn	Flows(l/s)	Units	TotalPopn	Flows(l/s)	Size(mm)	Spare Capacity(l/s)	
								Flow	%
<u>Tawadina</u>									
MH303A – MH304A	0	83.7	3.07	240	515.7	7.96	250	23.06	74.33
<u>Bareille-Snow</u>									
MH304A – MH308A	140	1964.7	24.33	0	2238.3	26.80	250	12.93	32.54

The updated analysis includes the existing sewer system highlighted on the Phase 2B design sheet. It is noted that the proposed site plan has new population of 432.0 people. This shows an increase of 273.6 people from the results of the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing. The new calculated wastewater flows in the Tawadina Road sewer from MH303A to MH304A from Parcel 1 is 7.96 l/s. This shows a wastewater flow increase of 4.89 l/s as a result of re-directing wastewater flow of Parcels 1 from Barreille-Snow Street to Tawadina Road. The spare capacity of that sewer is 23.06 l/s. The capacity of the sanitary sewer in Barreille-Snow Street was analyzed as well. The wastewater flow between MH304A and MH308A is 26.80 l/s. This shows an increase of 2.47 l/s in wastewater flow with an available capacity of 12.93 l/s. For reference, a highlighted copy of the Phase 2B sanitary sewer design sheet is included in **Appendix B**.

The impact of re-directing wastewater flows from Parcel 1 in Block 11 to the Tawadina Road sanitary sewer has been completed. Based on the analysis noted above, the existing wastewater system in Wateridge Village Phase 2B has sufficient available capacity to carry the re-directed flows from Parcel 1 in Block 11. It is therefore concluded that the existing sanitary sewers in Tawadina Road, Bareille-Snow Street adjacent to the subject property can accommodate the re-direction of flows from Parcel 1 in Block 11.

### **Stormwater Servicing**

The stormwater servicing is not consistent with the servicing presented in the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing prepared by IBI Group dated April 26, 2022. For example, the minor storm connection proposed by DesignWorks Engineering is to Tawadina Road to the north, while it was concluded in the IBI memo that the connection is to be to Bareille-Snow Street to the west. IBI cannot at this time comment on the implication of such a change. It should be noted that in addition to minor system connectivity, the April 2022 memo also outlined major system connectivity as well as minor and major system requirements.

Mary Jarvis – November 23, 2022

### **Water Servicing**

The objective of this evaluation is to review the water distribution of the submitted site plan by DesignWorks Engineering. A watermain model for the site plan area was included in the phase 2B Design Brief. For reference, the modeling results for Phase 2B are included in **Appendix C**.

The site plan shows a new 200mm diameter watermain connection at the existing 400mm watermain on Tawadina Road. This connection is expected to service both buildings on the site plan. The water design criteria used in calculating the water demands and system pressures for the site plan in Block 11 submitted by DesignWorks Engineering is based on the latest City of Ottawa Water Distribution Guidelines. It is also confirmed that the fire flow demand was calculated on the latest Fire Underwriters Survey (FUS) 2020.

The Wateridge Phase 2B figure shows four nodes around the subject site (I14, I16, I18 and I20). The basic day pressures range from 551.6 kPa to 555.0 kPa on Tawadina Road. The City of Ottawa criteria for pressure reduction during basic day demand is 552 kPa. Therefore, based on our analysis the building along Tawadina Road will not require pressure reducing valves on internal plumbing. The peak hour pressures range between 498.8 kPa and 508.1 kPa. The City criteria is that peak hour pressures must exceed 276 kPa so there is no issue with this criterial. The fire flows available during maximum day demand range between 462.6 l/s and 850.5 l/s which greatly exceeds the required fire flow rate of 320.17 l/s for the proposed buildings on the site plan.

The results of the average day demand for the site shows a demand of 1.4 L/s or 120,960 L/day. The City of Ottawa requires that a minimum 2 feeds be provided to a service area with a demand above 50,000 L/day, to avoid service disruptions. Therefore, an additional watermain connection to service the site is required.

### **Low Impact Development**

A review of the proposed site plan, located at Wateridge Village Phase 2B – Block 11, low impact development (LID) requirements was completed and included in **Appendix D**.

### **Conclusion**

In summary, a review of the proposed site plan for which DesignWorks Engineering is seeking approvals was completed. In terms of wastewater disposal impacts, although the proposed sanitary servicing outlet is not consistent the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing, we can conclude that the existing sanitary sewer in Tawadina Road can accommodate the re-direction of flows from Parcel 1 in Block 11. Based on the analysis above of the water distribution, an additional watermain connection is required at the proposed site plan to meet City of Ottawa Design Guidelines.

In terms of management of stormwater, the stormwater servicing is not consistent with the servicing presented in the Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing prepared by IBI Group dated April 26, 2022. Therefore, IBI cannot at this time comment on the implication of such a change.

Mary Jarvis – November 23, 2022

We trust our conclusions are satisfactory for your purposes. We are, of course, available to review and discuss the information contained within this document.

Regards,

**IBI GROUP**

A handwritten signature in black ink, appearing to read "Jim Moffatt". The signature is written in a cursive, flowing style.

Jim Moffatt, P. Eng.  
Associate

Mary Jarvis – November 23, 2022

## **APPENDIX A**

- Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing





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# Memorandum

<b>To/Attention</b>	John Bernier, City of Ottawa Shawn Wessel, City of Ottawa	<b>Date</b>	April 26, 2022
<b>From</b>	Meghan Black Jim Moffatt	<b>Project No</b>	118863-5.3.1.5
<b>cc</b>	Mary Jarvis, Canada Lands Company		
<b>Subject</b>	Assessment of Revised Block 11 and 12 Storm and Sanitary Servicing		

## 1. Background

Blocks 11 and 12 are located within Phase 2B of the Wateridge development and are indicated in **Figure 1**. The municipal servicing of the two blocks was addressed in, “Design Brief, Wateridge Village at Rockcliffe Phase 2B,” prepared by IBI Group in April 2019. Subsequent to the approval of the Phase 2B detailed design, Canada Lands Company has sub-divided the subject blocks into five parcels for development. The parcels, identified as Parcels 1-5, are being considered for purchase by various parties. IBI has been engaged to assess the impact of this change on adjacent existing storm and sanitary sewers. Enclosed **Figure 1** depicts Blocks 11 and 12 and the respective five parcels.

## 2. Stormwater Management

### 2.1 Objective

The objective of the evaluation is to assess the impact on the dual drainage system of discretizing Blocks 11 and 12 into Parcels 1-5 and the associated impacts to the storm servicing. The detailed design of Parcels 1-5 will be carried out by others.

### 2.2 Dual Drainage Design

Per the Phase 2B design brief, minor storm runoff from Block 11 (identified as drainage area B309) drains to Bareille-Snow Street, with major flow tipping to Bareille-Snow Street at Hemlock Road. Minor flow from Block 12 (identified as drainage area B340) drains to Codd’s Road with major flow draining to Hemlock Road. The minor system restriction for the two development blocks corresponds to between the 5 and 100 year storm event, and no on-site storage was proposed. The storm drainage area plan (Drawing 750) from the Phase 2B submission is enclosed in **Appendix A** for reference. With the proposed adjustments to the storm servicing for the sub-divided or discretized parcels, minor system capture and on-site storage has been re-assessed.

## 2.3 Hydrological Analysis

Hydrological analysis of the dual drainage system of the subject site has been conducted using DDSWMM, consistent with the simulations completed for the Phase 2B design brief.

### 2.3.1 Storm and Design Parameters

The following storms and design parameters have been used in the evaluation. The main hydrological parameters are summarized in **Table 2.1**, with a comparison of what was included in the Phase 2B evaluation.

- **Design Storms:** The subject site has been evaluated with the following storms, consistent with the Phase 2B evaluation:
  - 5 and 100 year 3 hour Chicago storm events, and associated stress test; applied for the evaluation of the trunk storm sewers;
  - 100 year 24 hour SCS Type II storm event, applied for the evaluation of the trunk storm sewers;
  - July 1979, August 1988, August 1996 historical storms per the OSDG.
- **Area and Imperviousness:** Block 11 (identified as drainage area B309) and Block 12 (identified as drainage area B340) have been discretized into Parcels 1 through 5. An imperviousness value of 86% has been applied to the parcels, consistent with the values applied for B309 and B340 in the Phase 2B design brief.
- **Infiltration:** Infiltration losses were selected to be consistent with the OSDG. The Horton values are as follows:  $f_0 = 76.2 \text{ mm/h}$ ,  $f_c = 13.2 \text{ mm/h}$ ,  $k = 0.00115 \text{ s}^{-1}$ .
- **Subcatchment Width:** The catchment width for the parcels was based on 225 m/ha.
- **Slope:** The ground slope was based upon the average slope for both impervious and pervious area. Generally, the slope is approximately 2% (0.02 m/m). This assumes a slope of approximately 1% for impervious or road surfaces and 3% for pervious surfaces (lot grading).
- **Initial Abstraction (Detention Storage):** Detention storage depths of 1.5 mm and 4.67 mm were used for impervious and pervious areas, respectively. These values are consistent with the OSDG.
- **Manning's roughness:** Manning's roughness coefficients of 0.013 and 0.25 were used for impervious and pervious areas, respectively.
- **Baseflow:** No baseflow components were assumed for any of the areas contributing runoff to the minor system within the DDSWMM model.
- **Minor System Capture:** The minor system capture for the parcels ranges from the 5 year to the 100 year, with three parcels capturing between the 5 and 100 year simulated flow.
- **Major System Storage and Routing:** In order to continue to satisfy City design guidelines, on-site storage has been introduced on four of the parcels, as noted below.

A summary of parameters and minor system and on-site storage is presented in the following tables. A summary from the Phase 2B detailed design is included to facilitate review. Refer to

**Figure 2** for the overall storm sewer network and to **Figure 3** for a depiction of the minor and major system connectivity for the five parcels.

**Table 2.1 Hydrological Parameters**

Block	Phase 2B Design Brief							Current Evaluation							
	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)	Parcel	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)
11	B309	1.24	S308A on Bareille-Snow	MH309 on Bareille-Snow	0.86	135.1	270.2	1	B309_1	0.72	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	81	162
								2	B309_2	0.52	S308A on Bareille-Snow	MH310 on Michael Stoqua	0.86	58.5	117
12	B340	1.24	S207 on Hemlock	MH305 on Codd's Road	0.86	173.1	346.3	3	B340_3	0.34	S308A on Bareille-Snow	MH308 on Bareille-Snow	0.86	38.25	76.5
								4	B340_4	0.53	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	59.63	119.25
								5	B340_5	0.37	S340 on Codd's	MH305 on Codd's Road	0.86	41.63	83.25

**Table 2.2 Minor System Restriction and On-site Storage**

Block	Phase 2B Design Brief				Current Evaluation					
	Drainage Area ID	Minor System Capture		Required On-Site Storage (cu-m)	Parcel	Drainage Area ID	Minor System Capture		Major System	
		Simulated Flow (l/s)	Corresponding Design Storm				Simulated Flow (l/s)	Corresponding Design Storm	Required On-Site Storage (cu-m)	Comment
11	B309	370	Between 5 and 100	None	1	B309_1	195	Between 5 and 100 year	43	Control up to the 100 year event
					2	B309_2	105	5 year	64	Control up to the 100 year event
12	B340	366	Between 5 and 100	None	3	B340_3	95	Between 5 and 100 year	18	Control up to the 100 year event
					4	B340_4	150	Between 5 and 100 year	21	Control up to the 100 year event
					5	B340_5	139	100 year	None	N/A

## 2.4 Results of Hydrological Modeling

### 2.4.1 Minor System

The minor system hydrographs generated by the hydrological model were exported to the hydraulic model for analysis, discussed in **Section 2.5**.

### 2.4.2 Major System

Due to the adjustment in major system connectivity, the major system has been reassessed. Refer to drainage areas on Drawing 750 from the Phase 2B submission in **Appendix A**.

#### 2.4.2.1 Street Segment Storage

The available and utilized street sag storage is summarized in the below table for street segments in affected by the revised storm servicing of Parcels 1-5.

**Table 2.3 Summary of On-site Street Storage (Available and Utilized) During Target Minor System Design Storm in Vicinity of Parcels 1-5**

Street	Drainage Area ID	Minor System Design Storm	Available Static Storage (cu-m)	Total Storage Utilized During Minor System Design Storm (cu-m)	Overflow During Minor System Design Storm (l/s)
Michael Stocqua	S310A	5	61.39	0	0
Bareille-Snow	S308A	5	40.38	0	0
Hemlock	S176C	5	1.14	0	0

The results indicate that there is no ponding on the street segments during the minor system design storm.

#### 2.4.2.2 Velocity x Depth

According to the City of Ottawa Sewer Design Guidelines (October 2012), the maximum depth of flow should not exceed 350 mm and the product of velocity and depth on all the street segments should not exceed 0.6 m<sup>2</sup>/s during the 100 year storm event.

The cascading overflow is the flow exiting a drainage area when maximum minor system inflow and maximum available ponding has been utilized. To determine velocity of the cascading overflow, a SWMHYMO file was created (118863VD.dat).

To determine velocity of the cascading overflow at critical locations, SWMHYMO was used. The ROW sections were entered into the model with the appropriate longitudinal slopes to obtain the maximum velocity of flow using the Route Channel routine. The overflow is obtained from the respective DDSWMM output file and is noted in the footnotes of the below tables.

To determine depth of the cascading overflow, the *Calculation Sheet: Overflow From Typical Road Ponding Area* provided at the February 2014 Technical Bulletin ISDTB-2014-01 was used. The

exception to this is where the road is on grade in which case the depths were obtained from the SWMHYMO model.

The results are presented in **Table 2.4** and **Table 2.5** and the supporting calculations are included in **Appendix A**.

**Table 2.4 Summary of Cascading Flow during the 100 year 3 hour Chicago storm**

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Michael Stoqua	S311A	N/A	49	0.73	N/A	0.04	0.04	0.03
Michael Stoqua	S310A	D14	0	0	0.29	0	0.29	0
Bareille-Snow	S309	N/A	43	0.50	N/A	0.05	0.05	0.03
Bareille-Snow	S308	N/A	65	0.84	N/A	0.05	0.05	0.04
Bareille-Snow	S308A	D18	26	0.47	0.26	0.05	0.31	0.03
Codd's	S340	N/A	50	0.88	N/A	0.04	0.04	0.04
Codd's	S231	N/A	100	0.62	N/A	0.07	0.07	0.04
Hemlock	S205C	N/A	37	0.48	N/A	0.05	0.05	0.02
Hemlock	S207	N/A	61	0.55	N/A	0.06	0.06	0.03

(1) Overflow from DDSWMM output 118863-3CHI100.out

(2) Velocity from SWMHYMO output 118863VD.out

(3) Depth of the cascading overflow was determined from the Calculation Sheet: Overflow From Typical Road Ponding Area provided in the February 2014 Technical Bulletin ISDTB-2014-01. For those areas which have a continuous road grade (or no dummy segment), the depth was taken from SWMHYMO VxD simulation.

**Table 2.5 Summary of Cascading Flow during the 100 year 3 hour Chicago storm + 20%**

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Michael Stoqua	S311A	N/A	66	0.79	N/A	0.05	0.05	0.04
Michael Stoqua	S310A	D14	33	0.61	0.29	0.06	0.35	0.04
Bareille-Snow	S309	N/A	71	0.57	N/A	0.06	0.06	0.03
Bareille-Snow	S308	N/A	216	1.15	N/A	0.08	0.08	0.09
Bareille-Snow	S308A	D18	268	1.29	0.26	0.13	<b>0.39</b>	0.17
Codd's	S340	N/A	98	1.04	N/A	0.05	0.05	0.06
Codd's	S231	N/A	165	0.71	N/A	0.08	0.08	0.06
Hemlock	S205C	N/A	46	0.51	N/A	0.05	0.05	0.03

Street	Drainage Area ID	Dummy Segment ID	Overflow (l/s) <sup>1</sup>	Velocity (m/s) <sup>2</sup>	Max. Static Ponding Depth (m)	Depth of Dynamic Flow (m) <sup>3</sup>	Max. Depth (Static + Dynamic) (m)	Velocity x Depth (m <sup>2</sup> /s)
Hemlock	S207	N/A	89	0.60	N/A	0.07	0.07	0.04

(1) Overflow from DDSWMM output 118863-3CHI120.out

(2) Velocity from SWMHYMO output 118863VD.out

(3) Depth of the cascading overflow was determined from the Calculation Sheet: Overflow From Typical Road Ponding Area provided in the February 2014 Technical Bulletin ISDTB-2014-01. For those areas which have a continuous road grade (or no dummy segment), the depth was taken from SWMHYMO VxD simulation.

During the 100 year 3 hour Chicago storm, the summation of depth of ponding and depth of cascading flow for all street segments is less than the City guideline of 0.35 m. The product of depth and velocity is also less than the City guideline of 0.6 m<sup>2</sup>/s.

During the sensitivity analysis applying the 100 year 3 hour Chicago storm increased by 20%, the summation of depth of ponding and depth of cascading flow for all street segments is less than the City guideline of 0.35 m, with the exception of S308A, noted in the above table in bold red type. At all locations, the product of depth and velocity is less than the City guideline of 0.6 m<sup>2</sup>/s.

These results are consistent with those of the Phase 2B detailed design. It should be noted that major flow from the above-noted affected areas is at or below that accounted for in the Phase 2B model.

The area at which total depth of ponding and cascading flow exceeds 0.35 m during the stress test is noted in the below table with the critical adjacent property elevation.

**Table 2.6 Critical Ponding Locations during the Stress Test and Adjacent Property Elevations**

Drainage Area ID	Low Point Elevation (m)	Max. Depth (Static + Dynamic) (m)	(1) Corresponding Elevation (m)	(2) Adjacent Property Line (m)	Difference (2) – (1)
S308A	88.74	0.39	89.13	89.01	-0.12

The corresponding stress test ponding elevation is greater than the adjacent block grading at the boulevard. At the detailed design stage of the blocks, house openings must be greater than the ponding elevation.

## 2.5 Storm Hydraulic Grade Line Analysis

The hydraulic grade line (HGL) was evaluated using the XPSWMM hydraulic model. The existing overall model for the Wateridge site, most recently revised as part of the Phase 4 submission (December 2021), was revised to include the revised servicing of Parcels 1-5.

XPSWMM simulations were conducted for the 100 year 3 hour Chicago storm to ensure that the HGL is at least 0.3 m below the underside of footing elevations. A sensitivity analysis was also performed using the 100 year Chicago storm with a 20% increase in intensity to ensure that there is no severe flooding to properties. Hydraulic grade line elevations along the existing downstream Phase 1A trunk storm sewer and relevant Phase 2B storm sewers are presented in the below table for these storms, along with a comparison of underside of footing (USF) elevations. Results

for the overall development area are presented in the enclosed **Appendix A**, including for the three historical storms per OSDG. Refer to **Figure 1** for the location of storm maintenance holes.

**Table 2.7 Storm Hydraulic Grade Line – Phase 1A Trunk and Relevant Phase 2B Storm Sewers**

MH ID	Street	Proposed Ground Elev. (m)	USF (m)	100 year 3 hour Chicago		100 year 3 hour Chicago + 20%	
				HGL (m)	USF – HGL (m)	HGL (m)	USF – HGL (m)
MH194	<i>Top of the escarpment</i>	82.05	N/A	80.47	N/A	80.55	N/A
MH193	OSHEDINAA	84.68	82.68	81.12	1.56	81.28	1.40
MH192	OSHEDINAA	84.99	82.99	81.46	1.53	81.64	1.35
MH191	OSHEDINAA	85.76	83.76	81.72	2.04	81.93	1.83
MH190	OSHEDINAA	86.36	84.36	81.96	2.40	82.19	2.17
MH180	OSHEDINAA	86.96	84.96	82.27	2.69	82.77	2.19
MH178	HEMLOCK	89.00	86.60	83.41	3.19	83.47	3.13
MH176	HEMLOCK	88.03	85.63	83.77	1.86	83.85	1.78
MH231	CODD'S	89.81	87.41	85.61	1.79	85.64	1.77
MH305	CODD'S	91.00	88.60	86.54	2.06	86.56	2.04
MH207	HEMLOCK	88.53	86.13	84.65	1.48	84.65	1.48
MH206	HEMLOCK	89.10	86.70	85.65	1.05	85.65	1.05
MH308	BAREILLE-SNOW	89.68	87.28	86.88	0.40	86.69	0.59
MH309	BAREILLE-SNOW	90.15	87.75	87.44	0.31	87.08	0.67
MH205	HEMLOCK	89.35	86.95	85.86	1.09	85.88	1.07
MH310	MICHAEL STOCQUA	90.04	87.64	87.28	0.36	87.42	0.22
MH311	MICHAEL STOCQUA	90.69	88.29	87.44	0.85	87.56	0.73

Along the Phase 1A trunk and Phase 2B storm sewers presented above, a minimum 0.3 m clearance between the USF and HGL is maintained during the 100 year 3 hour Chicago storm and the HGL elevations remain below USF elevations during the sensitivity analysis. This is also true for the results for the remainder of the development area for additional storm simulations (enclosed in **Appendix A**).



## 2.6 Conclusion

The storm servicing of Blocks 11 and 12 was addressed during the detailed design of Phase 2B. The purpose of this evaluation is to assess the impact on the dual drainage system of discretizing Blocks 11 and 12 into Parcels 1-5 and the associated revisions to the storm servicing. The proposed minor and major connectivity of the five parcels is presented on **Figure 3** and minor system capture and required on-site storage is summarized in **Table 2.2**.

In terms of major flow, the depth and velocity of flow on streets adjacent to the five parcels was evaluated. City guidelines with respect to ponding during the minor system design storm, as well as maximum depth and velocity of flow are maintained. Major flow from the adjacent street segments is at or below that accounted for in the Phase 2B model.

With respect to minor flow, the hydraulic grade line evaluation was updated with the revised inflow hydrographs from the five parcels. Results indicate that a minimum 0.3 m clearance between the USF and HGL is maintained during the 100 year 3 hour Chicago storm and the HGL elevations remain below USF elevations during the sensitivity analysis.

It is therefore concluded that the proposed storm servicing to support Parcels 1-5 can be accommodated by the existing storm infrastructure.

## 3. Wastewater Outlet

### 3.1 Objective

The objective of this evaluation is to assess the impact on the existing wastewater system by the sub-division of Blocks 11 and 12 into five parcels. **Figure 4** shows the location of the subject site and the existing sanitary sewers which will be impacted by this change.

### 3.2 Existing Conditions

Development of Phase 2B included the construction of sanitary sewers in Codd's Road from MH231A to the MH340A and Bareille-Snow Street from BLK308A to MH304A. The sanitary sewer on Codd's Road was designed to capture wastewater flows from Block 12 and the sanitary sewer on Bareille-Snow Street was designed to capture wastewater flows from Block 11. The Bareille-Snow sewer outlets to a sanitary sewer in Hemlock Road. The latter sewer was designed in 2017, using the City's wastewater flow criteria in effect at that time and predicted a flow of 28.49 l/s tributary from the Bareille-Snow sewer. The Bareille-Snow sanitary sewer was designed in 2019 based on flow calculation criteria in effect at that time and predicted a slightly less flow of 25.17 l/s. A highlighted copy of the Phase 2B sanitary sewer design sheet is included in **Appendix B**. The spreadsheet has been highlighted to indicate the immediate downstream sewers on Codd's Road and Bareille-Snow Street. The flow calculations in the Phase 2B spreadsheet were based on the City of Ottawa's wastewater criteria in effect of that time (2019) and the block population densities noted in the Master Servicing Study.

### 3.3 Proposed Condition

Because of the sub-division of Blocks 11 and 12 into five parcels, less wastewater flow is now proposed to outlet to the Codd's Road sanitary sewer. The Phase 2B sewer designed assumed all Block 12 would outlet to that sewer but now only parcel 5 is proposed to outlet in that direction. No further analysis is therefore needed for the Codd's Road sewer.

Parcels 3 and 4, which represent the balance of Block 12, are now proposed to outlet to the existing sanitary sewer in Bareille-Snow Street and not the Codd's Road sewer. There is no

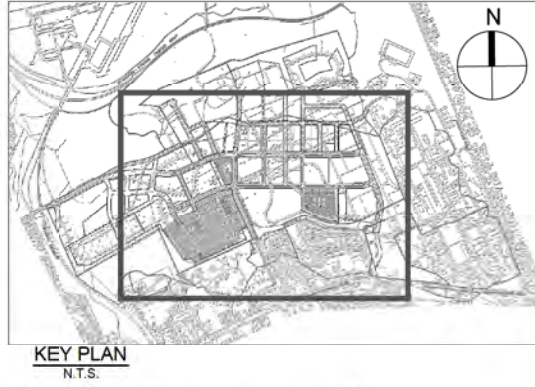
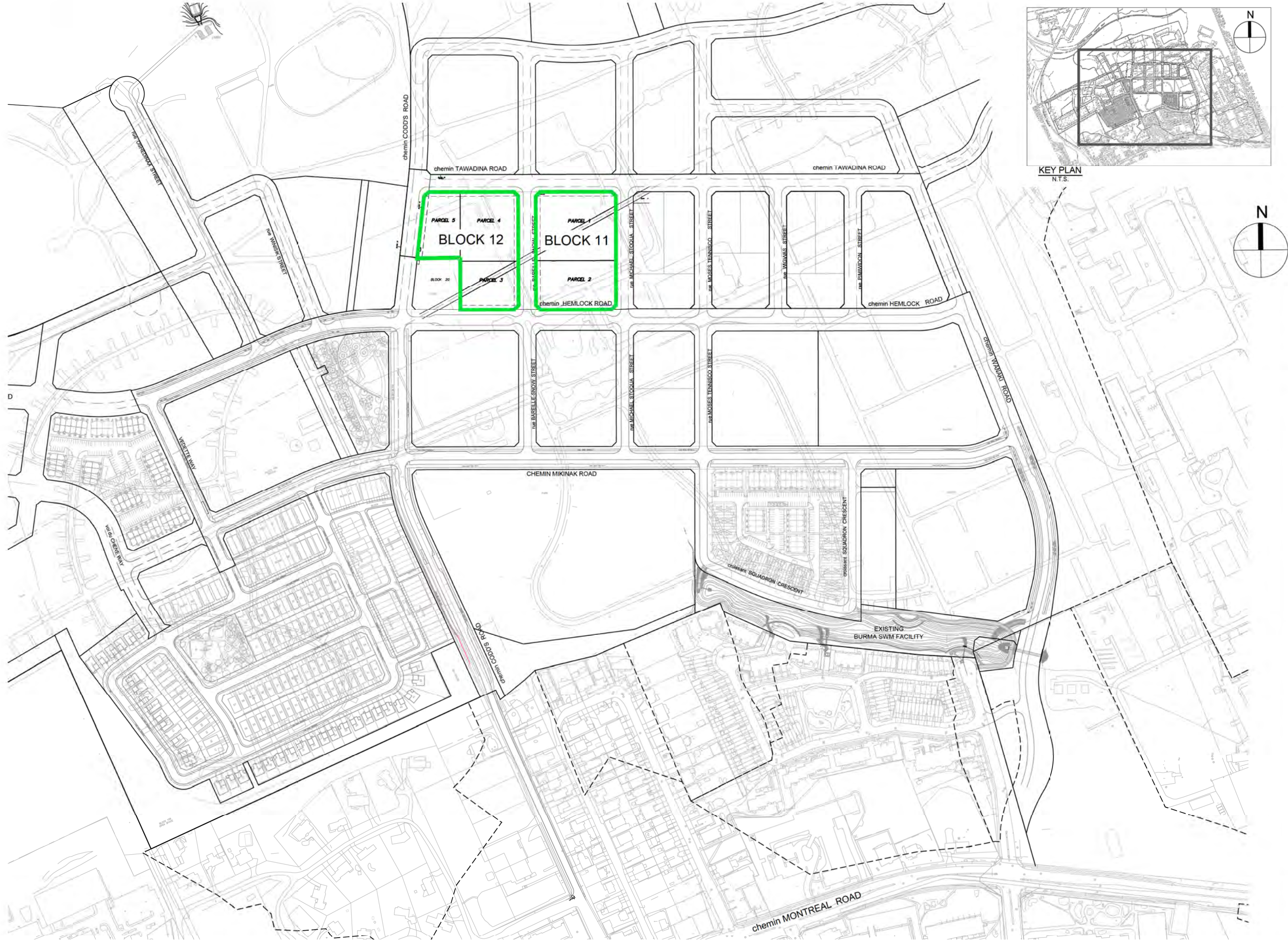
proposed change to the wastewater outlet for parcels 1 and 2. The Phase 2B design assumed all Block 11 would outlet to the Bareille-Snow sewer. Consequently, the expected wastewater flows to the latter pipe will likely increase.

An analysis of the ability of the existing sanitary sewer system in Bareille-Snow Street to accommodate the flows from both Block 11 and 12 was completed. This analysis is included on the updated sanitary sewer spreadsheet included in **Appendix B**. The updated spreadsheet was based not only on the current City of Ottawa wastewater criteria, which came into effect in 2018 but also on the most current concept plans for the various parcels which are also included in **Appendix B**. The updated analysis includes the existing sewer system highlighted on the Phase 2B design sheet.

Based on the updated analysis, the calculated wastewater flows tributary to the Hemlock Road sewer from Bareille-Snow Street is 30.31 l/s. This shows a wastewater flow increase of 1.82 l/s as a result of re-directing wastewater flows from parcels 3 and 4 in Block 12. The capacity of that sewer is 88.83 l/s. The Phase 1B design of the sanitary sewer in Hemlock Road between Bareille-Snow Street and Codd's Road indicated a spare capacity in that sewer of about 58 l/s. For reference, a highlighted copy of the Phase 1B sanitary sewer design sheet is included in **Appendix B**.

### **3.4 Conclusion**

The impact of re-directing wastewater flows from Block 12 to the Bareille-Snow Street sanitary sewer has been completed. Based on the analysis noted above, the existing wastewater system in Wateridge Village Phase 1B and 2B has sufficient available capacity to carry the re-directed flows from Block 12. It is therefore concluded that the existing sanitary sewers in Bareille-Snow Street, Codd's Road and Hemlock Road adjacent to the subject property can accommodate the re-direction of flows from Block 12.



Sheet No.

Drawing Title

LOCATION PLAN

Project Title  
STORM AND SANITARY SERVICING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B

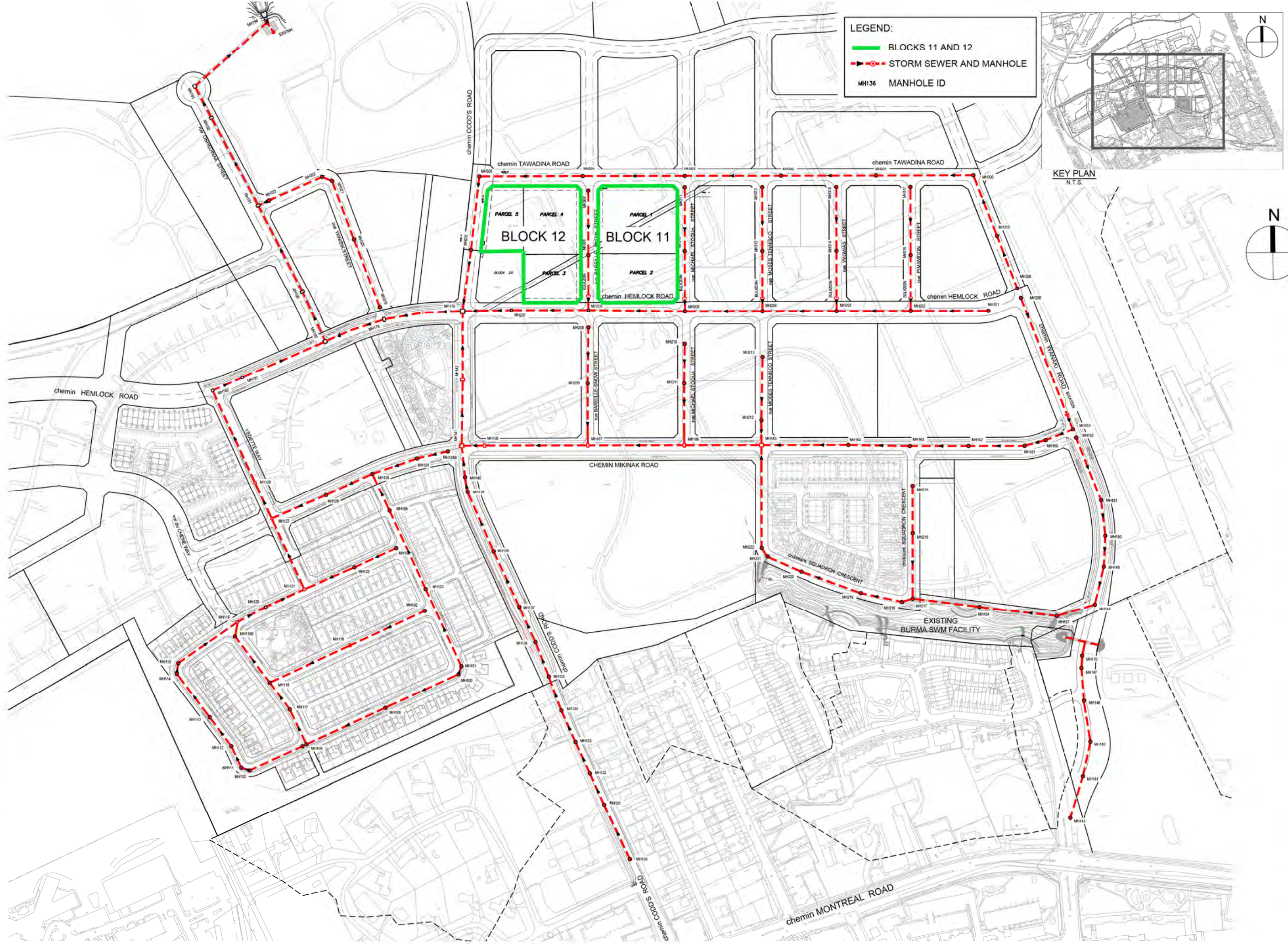
Scale

FIGURE 1



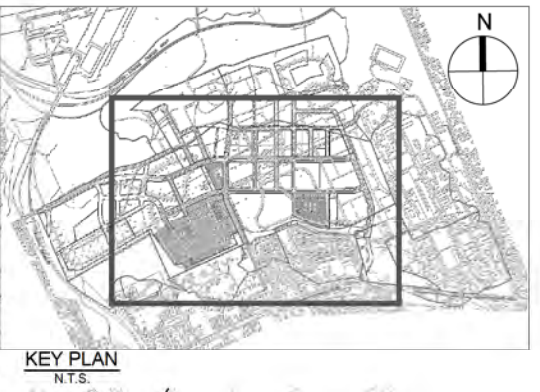
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d:\38298-CBRockville\5.9 Drawings\5ch\SWM\FIGURES\BLOCK 11,12\FIGURE 2.dwg Layout Name: FIG2 Plot Style: AIA STANDARD COLOR-HALF.CTB Plotted At: 4/25/2022 5:45 PM Last Saved By: swukic, Apr. 25, 22



**LEGEND:**

- BLOCKS 11 AND 12
- - - STORM SEWER AND MANHOLE
- MH136 MANHOLE ID



Sheet No. Project Title

**LOCATION PLAN  
AND STORM SEWER  
NETWORK**

**STORM AND SANITARY SERVING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B**

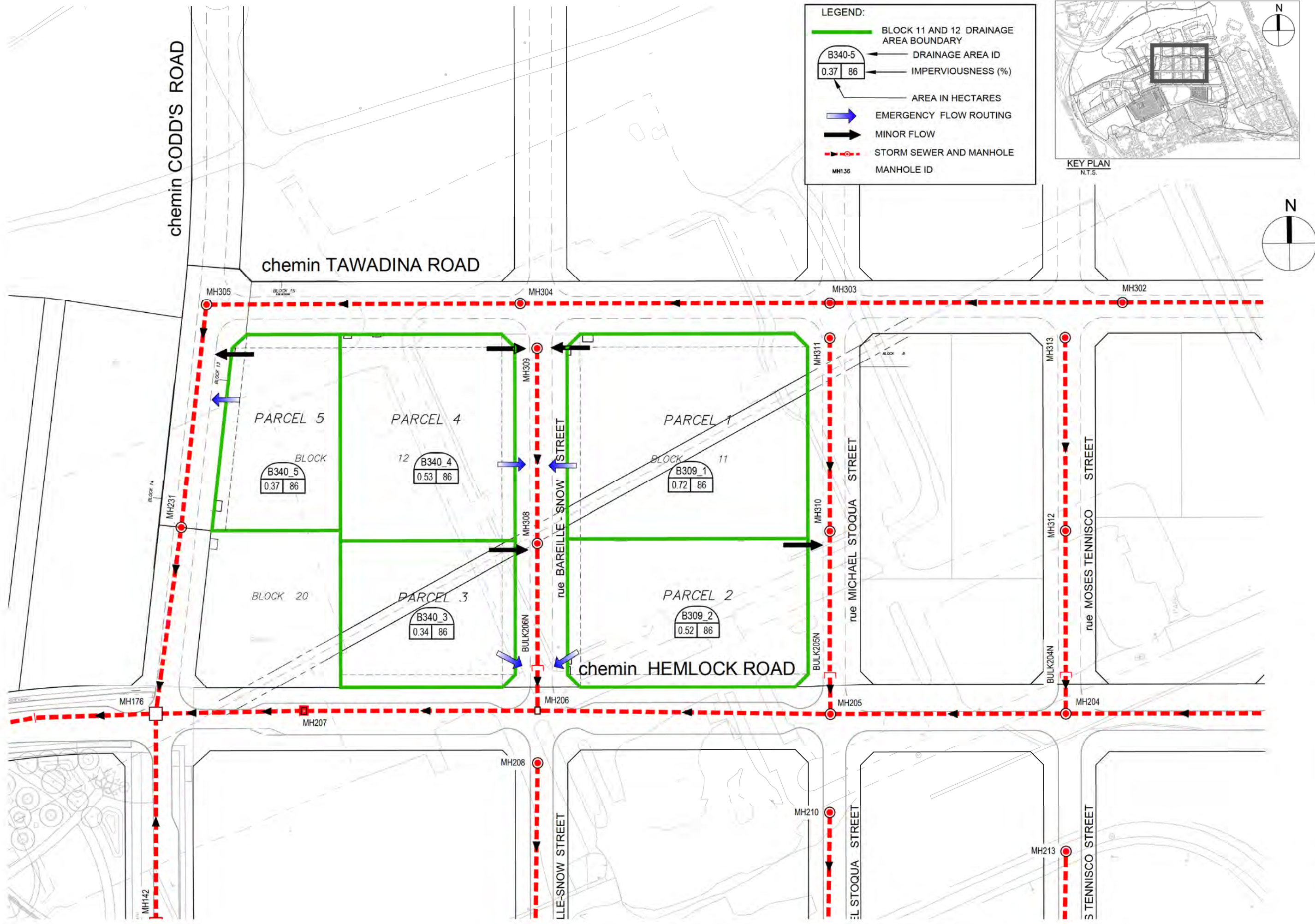
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**FIGURE 2**

N.T.S.



J:\38298-CFBRockville\5.9 Drawings\55civi\SWM\FIGURES\BLOCK 11,12\FIGURE 3.dwg Layout Name: FIG3 Plot Style: AIA STANDARD COLOR-HALF-CTB Plotted At: 4/25/2022 5:22 PM Last Saved By: svolkig, Apr. 25, 22



Sheet No.

Drawing Title

MINOR AND MAJOR SYSTEM  
CONNECTIVITY

Project Title

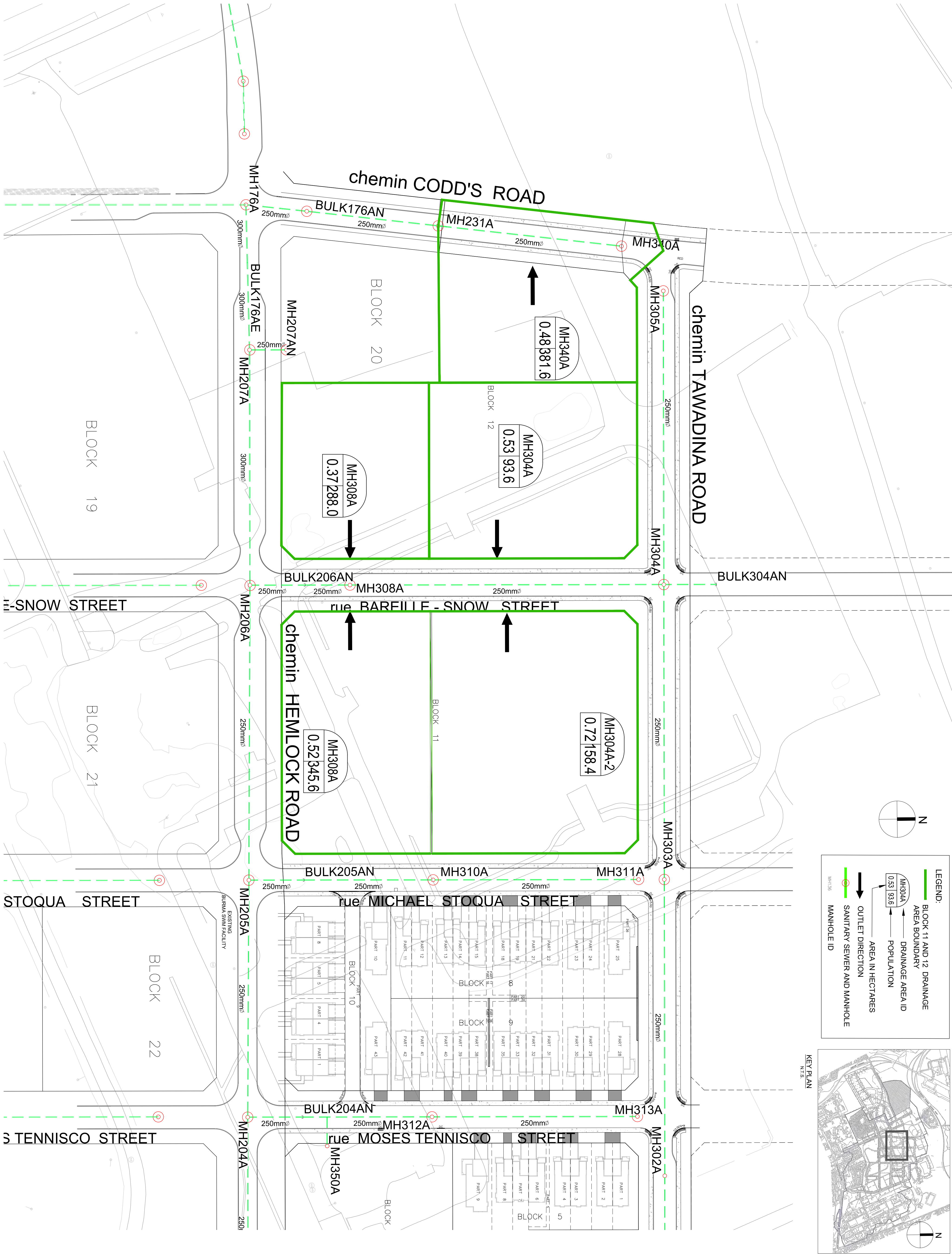
STORM AND SANITARY SERVICING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B

Scale

FIGURE 3

IBI

N.T.S.



Sheet No.

Drawing Title

Project Title

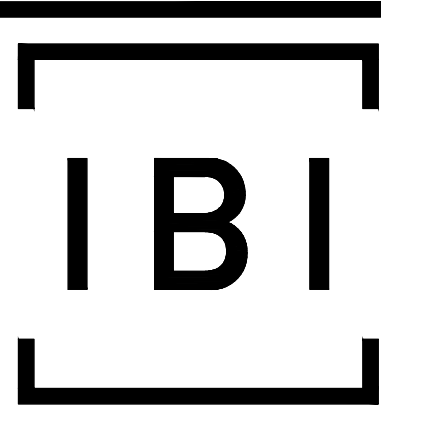
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FIGURE 4

LOCATION PLAN  
AND SANITARY SEWER  
NETWORK

STORM AND SANITARY SERVICING  
ASSESSMENT OF BLOCK 11 AND 12  
WATERIDGE VILLAGE PHASE 2B

N.T.S.



## **Appendix A**

Supporting Storm Information

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## **Summary of Model Files**

### **DDSWMM:**

5 year 3 hour Chicago: 118863-3CHI5.DAT  
100 year 3 hour Chicago: 118863-3CHI100.DAT  
100 year 3 hour Chicago + 20%: 118863-3CHI120.DAT

100 year 24 hour SCS Type II: 118863-24SCS100.DAT  
100 year 24 hour SCS Type II + 20%: 118863-24SCS120.DAT

July 1979: 118863-JUL79.DAT  
August 1988: 118863-AUG88.DAT  
August 1996: 118863-Aug96.DAT

### **SWMHYMO VxD:**

118863VD.dat

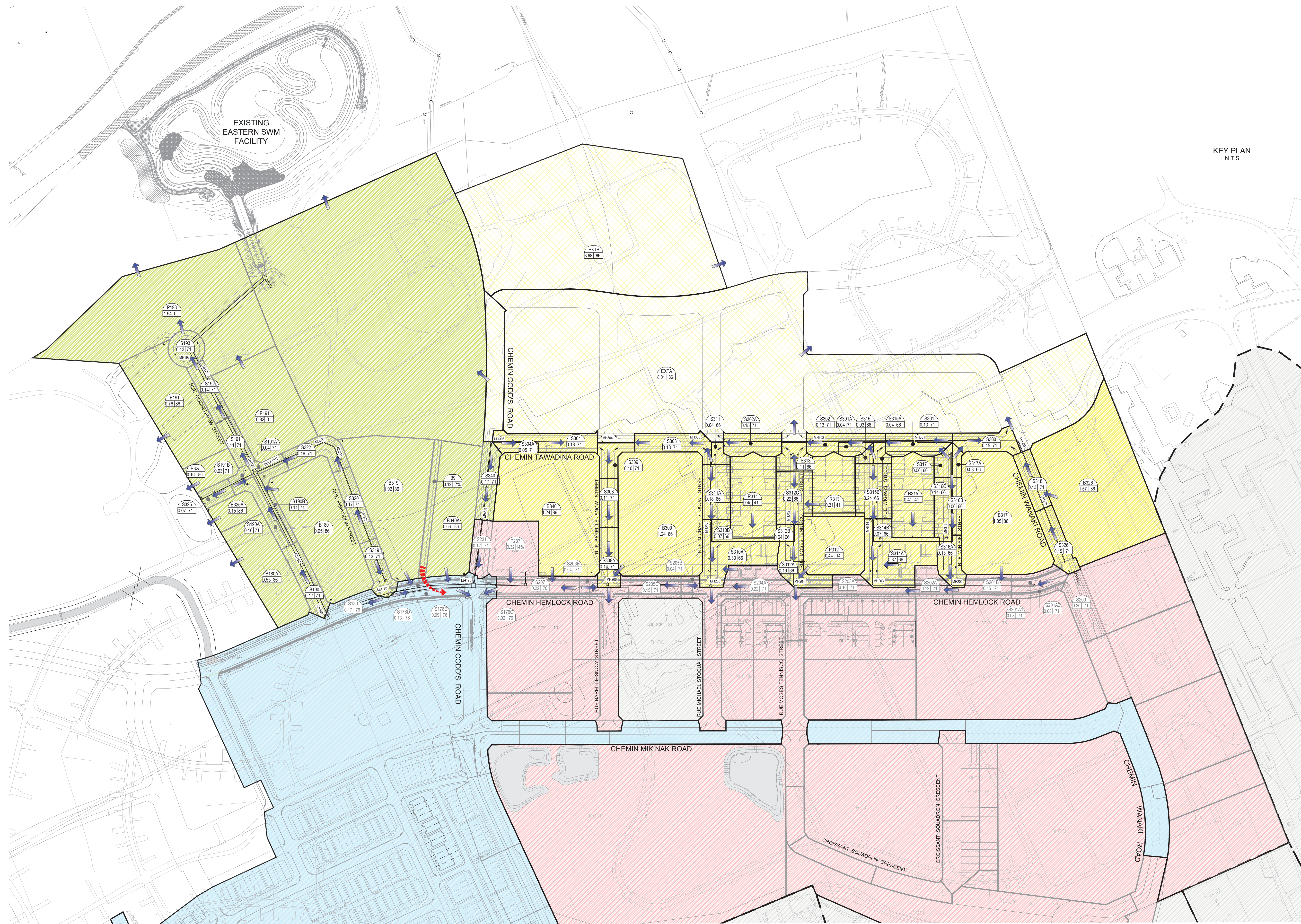
### **XPSWMM:**

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100 year 3 hour Chicago: 118863-3CHI100\_BLK1112\_V08\_2022-02-28.XP  
100 year 3 hour Chicago + 20%: 118863-3CHI120\_BLK1112\_V08\_2022-02-28.XP

100 year 24 hour SCS Type II: 118863-24SCS100\_BLK1112\_V08\_2022-03-15.XP  
100 year 24 hour SCS Type II + 20%: 118863-24SCS120\_BLK1112\_V08\_2022-03-15.XP

July 1979: 118863-JUL1979\_BLK1112\_V08\_2022-03-15.XP  
August 1988: 118863-AUG1988\_BLK1112\_V08\_2022-03-15.XP  
August 1996: 118863-AUG1996\_BLK1112\_V08\_2022-03-15.XP





KEY PLAN  
N.T.S.

**LEGEND:**

- PHASE 2B DRAINAGE AREA
- PHASE 2A DRAINAGE AREA (FUTURE)
- PHASE 2C 2D DRAINAGE AREA (FUTURE)
- PHASE 1B DRAINAGE AREA (EXISTING)
- PHASE 1A DRAINAGE AREA (EXISTING)
- EXTERNAL DRAINAGE AREA

AREA ID  
 Imp. (%)  
 AREA (ha)

MAJOR FLOW  
 TOTAL FLOW  
 MANHOLE ID

14			
13			
12			
11			
10			
9			
8			
7			
6			
5			
4			
3			
2			
1	SUBMISSION No.1 FOR CITY REVIEW	P.S.	2018; 12; 20
No.	REVISIONS	By	Date

**CANADA LANDS COMPANY**  
 SOCIÉTÉ IMMOBILIÈRE DU CANADA  
 30 Metcalfe Street Suite 601  
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 613 998 7777

**IBI GROUP**  
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 Ottawa ON K1S 5N4 Canada  
 tel 613 225 1311 fax 613 225 9868  
 ibigroup.com

Project Title  
**WATERIDGE VILLAGE AT ROCKCLIFFE**  
 PHASE 2B

Drawing Title  
**DDSWMM  
 MODEL SCHEMATIC**

Scale  
 1:2000

Design	M.B	Date	DEC. 2018
Drawn	S.V.	Checked	P.S.
Project No.	118863	Drawing No.	750

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# Velocity x Depth Calculation

Iteration equation:

Velocity:

$$v_x = v_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (v_{\max} - v_{\min})$$

Depth:

$$d_x = d_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (d_{\max} - d_{\min})$$

100 Year 3 Hour Chicago Storm																				
			SWMHYMO (118863VD.OUT)							Calculation Sheet: Overflow for Typical Road Ponding Area					SWMHYMO (118863VD.OUT)			Velocity x Depth	Maximum Static Ponding Depth	Total Depth (Static + Dynamic)
Area ID (Dummy Segment, if applicable)	Road ROW Section	Longitudinal Slope (%)	Overflow Flowrate		Flowrate (cms)		Velocity (m/s)			Flowrate (cms)		Depth (m)			Depth (m)			(m <sup>2</sup> /s)	(m)	(m)
			Qx (l/s)	Qx (cms)	Qmin	Qmax	vmin	vmax	vx	Qmin	Qmax	dmin	dmax	dx	dmin	dmax	dx			
S311A	20	1.52	<b>49</b>	0.049	0.039	0.084	0.699	0.847	<b>0.73</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.044</b>	<b>0.03</b>	0.00	0.04
S310A	20	1.22	<b>0</b>	0.000	0.000	0.002	0.000	0.301	<b>0.00</b>	0.000	0.001	0.000	0.001	<b>0.000</b>	N/A	N/A	<b>N/A</b>	<b>0.00</b>	0.29	0.29
S309	20	0.60	<b>43</b>	0.043	0.024	0.053	0.439	0.532	<b>0.50</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.050</b>	<b>0.03</b>	0.00	0.05
S308	20	1.84	<b>65</b>	0.065	0.043	0.092	0.769	0.932	<b>0.84</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.047</b>	<b>0.04</b>	0.00	0.05
S308A	20	0.71	<b>26</b>	0.026	0.009	0.027	0.365	0.478	<b>0.47</b>	0.021	0.027	0.050	0.055	<b>0.054</b>	N/A	N/A	<b>N/A</b>	<b>0.03</b>	0.26	0.31
S340	20	2.40	<b>50</b>	0.050	0.049	0.105	0.878	1.064	<b>0.88</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.041</b>	<b>0.04</b>	0.00	0.04
S205C	24	0.71	<b>37</b>	0.037	0.024	0.053	0.439	0.532	<b>0.48</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.041	0.055	<b>0.047</b>	<b>0.02</b>	0.00	0.05
S231	20	0.53	<b>100</b>	0.100	0.096	0.155	0.617	0.697	<b>0.62</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.068	0.082	<b>0.069</b>	<b>0.04</b>	0.00	0.07
S207	24	0.51	<b>61</b>	0.061	0.053	0.096	0.532	0.617	<b>0.55</b>	N/A	N/A	N/A	N/A	<b>N/A</b>	0.055	0.068	<b>0.057</b>	<b>0.03</b>	0.00	0.06

# Velocity x Depth Calculation

Iteration equation:

Velocity:

$$v_x = v_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (v_{\max} - v_{\min})$$

Depth:

$$d_x = d_{\min} + \frac{Q_x - Q_{\min}}{Q_{\max} - Q_{\min}} (d_{\max} - d_{\min})$$

100 Year 3 Hour Chicago Storm + 20%																				
				SWMHYMO (118863VD.OUT)						Calculation Sheet: Overflow for Typical Road Ponding Area					SWMHYMO (118863VD.OUT)			Velocity x Depth	Maximum Static Ponding Depth	Total Depth (Static + Dynamic)
Area ID (Dummy Segment, if applicable)	Road ROW Section	Longitudinal Slope (%)	Overflow Flowrate		Flowrate (cms)		Velocity (m/s)			Flowrate (cms)		Depth (m)			Depth (m)			(m <sup>2</sup> /s)	(m)	(m)
			Qx (l/s)	Qx (cms)	Qmin	Qmax	vmin	vmax	vx	Qmin	Qmax	dmin	dmax	dx	dmin	dmax	dx			
S311A	20	1.52	66	0.066	0.039	0.084	0.699	0.847	0.79	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.049	0.04	0.00	0.05
S310A	20	1.22	33	0.033	0.012	0.035	0.478	0.626	0.61	0.028	0.035	0.055	0.060	0.059	N/A	N/A	N/A	0.04	0.29	0.35
S309	20	0.60	71	0.071	0.053	0.096	0.532	0.617	0.57	N/A	N/A	N/A	N/A	N/A	0.055	0.068	0.060	0.03	0.00	0.06
S308	20	1.84	216	0.216	0.167	0.272	1.081	1.221	1.15	N/A	N/A	N/A	N/A	N/A	0.068	0.082	0.075	0.09	0.00	0.07
S308A	20	0.71	268	0.268	0.255	0.364	0.841	0.919	1.29	0.240	0.269	0.125	0.130	0.130	N/A	N/A	N/A	0.17	0.26	0.39
S340	20	2.40	98	0.098	0.049	0.105	0.878	1.064	1.04	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.053	0.06	0.00	0.05
S205C	24	0.71	46	0.046	0.024	0.053	0.439	0.532	0.51	N/A	N/A	N/A	N/A	N/A	0.041	0.055	0.052	0.03	0.00	0.05
S231	20	0.53	165	0.165	0.155	0.234	0.697	0.773	0.71	N/A	N/A	N/A	N/A	N/A	0.082	0.095	0.084	0.06	0.00	0.08
S207	24	0.51	89	0.089	0.053	0.096	0.532	0.617	0.60	N/A	N/A	N/A	N/A	N/A	0.055	0.068	0.066	0.04	0.00	0.07

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
<b>Phase 1B</b>																	
S143	143	102.40	100.00	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84	98.16	1.84
S144	144	99.41	97.01	95.79	1.22	95.79	1.22	95.78	1.23	95.79	1.22	95.78	1.23	95.79	1.22	95.78	1.23
S145	145	97.64	95.24	93.01	2.23	93.01	2.23	93.01	2.23	93.01	2.23	93.00	2.24	93.01	2.23	93.00	2.24
S146	146	95.28	92.88	90.96	1.92	91.82	1.06	90.77	2.11	91.26	1.62	90.91	1.97	91.01	1.87	90.63	2.25
S147	147	93.27	N/A	90.93	N/A	91.78	N/A	90.72	N/A	91.23	N/A	90.88	N/A	90.98	N/A	90.60	N/A
USBRM	N/A	N/A	N/A	90.88	N/A	91.72	N/A	90.67	N/A	91.17	N/A	90.83	N/A	90.93	N/A	90.56	N/A
BURMA	N/A	N/A	N/A	89.41	N/A	89.87	N/A	89.24	N/A	89.53	N/A	89.43	N/A	89.31	N/A	89.04	N/A
OUTLET	N/A	N/A	N/A	89.26	N/A	89.75	N/A	89.07	N/A	89.39	N/A	89.29	N/A	89.15	N/A	88.65	N/A
S152	152	92.73	90.33	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62	89.71	0.62
S151	151	92.50	90.10	89.58	0.52	89.57	0.53	89.58	0.52	89.58	0.52	89.58	0.52	89.58	0.52	89.57	0.53
S150	150	92.32	89.92	89.49	0.43	89.48	0.44	89.49	0.43	89.49	0.43	89.49	0.43	89.49	0.43	89.49	0.43
S149	149	92.34	89.94	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52	89.42	0.52
S148	148	92.14	89.74	89.30	0.44	89.29	0.45	89.30	0.44	89.30	0.44	89.30	0.44	89.30	0.44	89.30	0.44
S157	157	91.24	N/A	89.21	N/A	89.20	N/A	89.21	N/A	89.21	N/A	89.21	N/A	89.21	N/A	89.21	N/A
S154	154	91.02	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A	87.68	N/A
S215	215	90.77	88.37	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79	87.58	0.79
S216	216	90.85	88.45	87.30	1.15	87.30	1.15	87.30	1.15	87.30	1.15	87.30	1.15	87.31	1.14	87.30	1.15
S217	217	90.66	88.26	87.13	1.13	87.18	1.08	87.12	1.14	87.15	1.11	87.14	1.12	87.13	1.13	87.12	1.14
S218	218	90.40	88.00	87.04	0.96	87.10	0.90	87.02	0.98	87.06	0.94	87.05	0.95	87.04	0.96	87.02	0.98
S219	219	90.08	87.68	86.85	0.83	86.94	0.74	86.82	0.86	86.88	0.80	86.86	0.82	86.84	0.84	86.81	0.87
S220	220	89.86	87.46	86.74	0.72	86.84	0.62	86.70	0.76	86.78	0.68	86.75	0.71	86.72	0.74	86.68	0.78
S221	221	89.88	87.48	86.57	0.91	86.72	0.76	86.51	0.97	86.63	0.85	86.59	0.89	86.54	0.94	86.36	1.12
S222	222	89.86	87.46	86.38	1.08	86.51	0.95	86.32	1.14	86.43	1.03	86.39	1.07	86.35	1.11	86.19	1.27
S200	200	94.71	92.31	90.73	1.58	90.74	1.57	90.73	1.58	90.72	1.59	90.73	1.58	90.72	1.59	90.73	1.58
S214	214	93.52	91.12	90.26	0.86	90.28	0.84	90.26	0.86	90.27	0.85	90.26	0.86	90.26	0.86	90.26	0.86
MH201	201	94.29	91.89	90.72	1.17	90.73	1.16	90.72	1.17	90.72	1.17	90.72	1.17	90.72	1.17	90.71	1.18
MH202	202	93.91	91.51	90.42	1.09	90.43	1.08	90.41	1.10	90.42	1.09	90.41	1.10	90.41	1.10	90.40	1.11
MH203	203	92.38	89.98	88.66	1.32	88.68	1.30	88.63	1.35	88.66	1.32	88.63	1.35	88.64	1.34	88.61	1.37
MH204	204	90.40	88.00	87.08	0.92	87.10	0.90	87.06	0.94	87.08	0.92	87.06	0.94	87.07	0.93	87.02	0.98
MH205	205	89.35	86.95	85.86	1.09	85.88	1.07	85.83	1.12	85.86	1.09	85.84	1.11	85.84	1.11	85.77	1.18
MH206	206	89.10	86.70	85.65	1.05	85.65	1.05	85.62	1.08	85.65	1.05	85.63	1.07	85.63	1.07	85.57	1.13
MH207	207	88.53	86.13	84.65	1.48	84.65	1.48	84.62	1.51	84.65	1.48	84.63	1.50	84.64	1.49	84.58	1.55
S212	212	90.25	87.85	86.86	0.99	86.87	0.98	86.83	1.02	86.85	1.00	86.83	1.02	86.84	1.01	86.82	1.03
S213	213	89.74	87.34	86.45	0.89	86.45	0.89	86.43	0.91	86.45	0.89	86.44	0.90	86.44	0.90	86.42	0.92
S210	210	89.14	86.74	86.43	0.31	86.43	0.31	86.42	0.32	86.43	0.31	86.42	0.32	86.43	0.31	86.41	0.33
S211	211	89.15	86.75	85.94	0.81	85.93	0.82	85.93	0.82	85.94	0.81	85.93	0.82	85.93	0.82	85.92	0.83
S208	208	88.77	86.37	85.92	0.45	85.91	0.46	85.78	0.59	85.91	0.46	85.81	0.56	85.88	0.49	85.70	0.67
S209	209	88.75	86.35	85.46	0.89	85.45	0.90	85.41	0.94	85.46	0.89	85.42	0.93	85.45	0.90	85.38	0.97
MH231	231	89.81	87.41	85.61	1.79	85.64	1.77	85.73	1.67	85.78	1.63	85.84	1.57	85.77	1.63	85.71	1.69

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996		
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	
<b>Wateridge Village Phase 1A</b>																		
S153	153	92.78	90.38	89.45	0.93	89.46	0.92	89.44	0.94	89.45	0.93	89.44	0.94	89.45	0.93	89.44	0.94	
S160	160	92.27	89.87	89.01	0.86	89.02	0.85	89.01	0.86	89.01	0.86	89.01	0.86	89.01	0.86	89.00	0.87	
S161	161	91.94	89.54	88.57	0.97	88.58	0.96	88.57	0.97	88.57	0.97	88.57	0.97	88.57	0.97	88.57	0.97	
S162	162	91.34	88.94	88.26	0.68	88.26	0.68	88.25	0.69	88.26	0.68	88.25	0.69	88.26	0.68	88.25	0.69	
S163	163	90.94	88.54	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	87.68	0.86	
S164	164	90.22	87.82	87.00	0.82	87.01	0.81	86.99	0.83	87.00	0.82	87.00	0.82	87.00	0.82	86.99	0.83	
S165B	165	89.61	87.21	86.45	0.76	86.45	0.76	86.44	0.77	86.44	0.77	86.44	0.77	86.44	0.77	86.44	0.77	
S165	165	89.30	86.90	85.98	0.92	86.05	0.85	85.93	0.97	86.01	0.89	85.99	0.91	85.96	0.94	85.83	1.07	
S166	166	88.90	86.50	84.88	1.62	85.03	1.47	84.78	1.72	84.93	1.57	84.88	1.62	84.85	1.65	84.59	1.91	
S167	167	88.40	86.00	84.71	1.29	84.86	1.14	84.60	1.40	84.76	1.24	84.71	1.29	84.67	1.33	84.39	1.61	
S168	168	87.70	85.30	84.54	0.76	84.66	0.64	84.43	0.87	84.58	0.72	84.54	0.76	84.50	0.80	84.22	1.08	
S141	141	87.32	84.92	84.28	0.64	84.39	0.53	84.18	0.74	84.32	0.60	84.28	0.64	84.25	0.67	83.97	0.95	
S142	142	87.52	85.12	84.02	1.10	84.12	1.00	83.94	1.18	84.06	1.06	84.03	1.09	84.00	1.12	83.74	1.38	
MH176	176	88.03	85.63	83.77	1.86	83.85	1.78	83.69	1.94	83.80	1.83	83.77	1.86	83.75	1.88	83.49	2.14	
MH178	178	89.00	86.60	83.41	3.19	83.47	3.13	83.34	3.26	83.44	3.16	83.41	3.19	83.39	3.21	83.18	3.42	
MH180	180	88.23	85.83	82.20	3.62	82.44	3.38	81.98	3.84	82.27	3.56	82.21	3.62	82.10	3.73	81.49	4.34	
MH190	190	88.10	85.70	81.90	3.80	82.12	3.58	81.65	4.05	81.97	3.73	81.91	3.79	81.80	3.90	81.23	4.47	
MH191	191	86.36	83.96	81.66	2.30	81.86	2.10	81.44	2.52	81.73	2.23	81.67	2.29	81.56	2.40	81.06	2.91	
MH192	192	85.92	83.52	81.41	2.11	81.59	1.93	81.21	2.31	81.47	2.05	81.41	2.11	81.31	2.21	80.89	2.63	
MH193	193	84.85	82.45	81.09	1.36	81.24	1.21	80.92	1.53	81.14	1.31	81.09	1.36	81.00	1.45	80.60	1.85	
MH194	194	82.44	N/A	80.45	N/A	80.53	N/A	80.35	N/A	80.48	N/A	80.46	N/A	80.40	N/A	80.13	N/A	
S130	130		N/A	101.25	N/A	101.25	N/A	101.24	N/A	101.25	N/A	101.24	N/A	101.24	N/A	101.23	N/A	
S131	131		N/A	101.05	N/A	101.05	N/A	101.04	N/A	101.05	N/A	101.04	N/A	101.04	N/A	101.03	N/A	
S132	132		N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.64	N/A	99.63	N/A	
S133	133		N/A	96.52	N/A	96.52	N/A	96.51	N/A	96.52	N/A	96.51	N/A	96.51	N/A	96.50	N/A	
S134	134		N/A	93.01	N/A	93.01	N/A	93.00	N/A	93.01	N/A	93.00	N/A	93.00	N/A	92.99	N/A	
S135	135		N/A	90.11	N/A	90.11	N/A	90.10	N/A	90.11	N/A	90.10	N/A	90.10	N/A	90.09	N/A	
S136	136		N/A	87.38	N/A	87.38	N/A	87.37	N/A	87.38	N/A	87.37	N/A	87.37	N/A	87.37	N/A	
S137	137			86.91	85.77	1.14	85.77	1.14	85.76	1.15	85.77	1.14	85.76	1.15	85.77	1.14	85.76	1.15
S138	138			86.31	84.96	1.35	84.96	1.35	84.95	1.36	84.96	1.35	84.95	1.36	84.95	1.36	84.94	1.37
S139	139			85.66	84.46	1.20	84.48	1.18	84.46	1.20	84.46	1.20	84.46	1.20	84.46	1.20	84.45	1.21
S140	140			N/A	84.35	N/A	84.42	N/A	84.34	N/A	84.37	N/A	84.35	N/A	84.34	N/A	84.34	N/A
S100	100			87.16	85.70	1.46	85.69	1.47	85.70	1.46	85.70	1.46	85.70	1.46	85.70	1.46	85.70	1.46
S108	108			86.66	85.24	1.43	85.23	1.43	85.23	1.43	85.24	1.42	85.23	1.43	85.23	1.43	85.23	1.43
S109	109			85.36	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31	84.05	1.31
S117	117			85.06	83.54	1.52	83.58	1.48	83.53	1.53	83.54	1.52	83.53	1.53	83.54	1.52	83.53	1.53
S118	118			84.71	83.21	1.50	83.48	1.23	83.20	1.51	83.25	1.46	83.22	1.49	83.21	1.50	83.20	1.51
S101	101			87.16	85.55	1.61	85.55	1.61	85.54	1.62	85.55	1.61	85.54	1.62	85.54	1.62	85.54	1.62
S102	102			86.46	84.72	1.74	84.72	1.74	84.71	1.75	84.72	1.74	84.71	1.75	84.71	1.75	84.70	1.76
S119	119			85.46	83.95	1.51	83.95	1.51	83.95	1.51	83.95	1.51	83.94	1.52	83.95	1.51	83.95	1.51
S104	104			N/A	85.90	N/A	85.89	N/A	85.89	N/A	85.90	N/A	85.89	N/A	85.89	N/A	85.88	N/A

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
S103	103		86.46	84.36	2.10	84.36	2.10	84.34	2.12	84.36	2.10	84.35	2.11	84.35	2.11	84.34	2.12
S105	105		85.71	83.90	1.81	83.91	1.80	83.89	1.82	83.90	1.81	83.89	1.82	83.90	1.81	83.89	1.82
S122	122		84.86	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33	83.53	1.33
S121	121		84.26	82.80	1.46	83.03	1.23	82.43	1.83	82.82	1.44	82.77	1.49	82.61	1.65	81.98	2.28
S127	127		84.36	82.67	1.69	82.92	1.44	82.34	2.02	82.71	1.65	82.66	1.70	82.51	1.85	81.85	2.51
S128	128		N/A	82.61	N/A	82.86	N/A	82.30	N/A	82.67	N/A	82.61	N/A	82.47	N/A	81.81	N/A
S107	107		N/A	85.29	N/A	85.29	N/A	85.28	N/A	85.29	N/A	85.28	N/A	85.28	N/A	85.27	N/A
S106	106		85.61	83.76	1.85	83.75	1.86	83.73	1.88	83.76	1.85	83.74	1.87	83.75	1.86	83.73	1.88
S124	124		85.69	83.94	1.75	83.94	1.75	83.93	1.76	83.94	1.75	83.93	1.76	83.93	1.76	83.92	1.77
S125	125		85.34	83.37	1.97	83.38	1.96	83.35	1.99	83.37	1.97	83.36	1.98	83.36	1.98	83.35	1.99
S126	126		84.96	82.87	2.09	83.14	1.82	82.85	2.11	82.89	2.07	82.85	2.11	82.86	2.10	82.84	2.12
S182	182		N/A	82.46	N/A	82.70	N/A	82.18	N/A	82.52	N/A	82.46	N/A	82.32	N/A	81.68	N/A
S181	181		N/A	82.36	N/A	82.61	N/A	82.11	N/A	82.43	N/A	82.37	N/A	82.24	N/A	81.61	N/A
S110	110		85.56	83.59	1.97	83.80	1.76	83.59	1.97	83.59	1.97	83.59	1.97	83.59	1.97	83.59	1.97
S111	111		84.96	83.59	1.37	83.80	1.16	83.58	1.38	83.59	1.37	83.58	1.38	83.59	1.37	83.58	1.38
S112	112		84.91	83.40	1.52	83.77	1.14	83.18	1.73	83.50	1.41	83.42	1.49	83.22	1.69	83.22	1.69
S113	113		84.51	83.41	1.10	83.74	0.77	83.06	1.45	83.48	1.03	83.40	1.11	83.08	1.43	83.05	1.46
S114	114		83.91	83.06	0.85	83.31	0.60	82.66	1.25	83.11	0.80	83.04	0.87	82.85	1.06	82.49	1.42
S115	115		83.56	83.04	0.52	83.33	0.23	82.64	0.92	83.13	0.43	83.01	0.55	82.83	0.73	82.45	1.11
S116	116		83.71	82.88	0.83	83.16	0.55	82.51	1.20	82.92	0.79	82.85	0.86	82.70	1.01	82.10	1.61
S120	120		83.96	82.86	1.10	83.08	0.88	82.48	1.48	82.88	1.08	82.83	1.13	82.67	1.29	82.06	1.90

Storm Hydraulic Grade Line Elevations

XPSWMM NODE ID	MH NO.	PROPOSED GROUND ELEVATION (M)	USF (M)	100 YEAR 3 HOUR CHICAGO		100 YEAR 3 HOUR CHICAGO INCREASED BY 20%		100 YEAR 24 HOUR SCS TYPE II		100 YEAR 24 HOUR SCS TYPE II + 20%		JULY 1 1979		AUGUST 1988		AUGUST 1996	
				HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)	HGL (M)	USF - HGL (M)
<b>Phase 2B, 4</b>																	
MH317	317	94.08	91.68	91.17	0.51	91.18	0.50	91.14	0.54	91.15	0.53	91.15	0.53	91.14	0.54	91.11	0.57
MH316	316	94.09	91.69	90.96	0.73	90.96	0.73	90.95	0.74	90.95	0.74	90.95	0.74	90.95	0.74	90.92	0.77
MH315	315	93.39	91.36	90.28	1.08	90.29	1.07	90.25	1.11	90.26	1.10	90.27	1.09	90.27	1.09	90.26	1.10
MH314	314	93.00	91.16	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.91	1.25	89.89	1.27
MH313	313	92.62	90.71	89.35	1.36	89.34	1.37	89.35	1.36	89.35	1.36	89.35	1.36	89.35	1.36	89.34	1.37
MH312	312	91.36	89.68	88.42	1.26	88.42	1.26	88.41	1.27	88.42	1.26	88.42	1.26	88.42	1.26	88.38	1.30
MH311	311	90.69	88.29	87.44	0.85	87.56	0.73	87.40	0.89	87.48	0.81	87.45	0.84	87.47	0.82	87.38	0.91
MH310	310	90.04	87.64	87.28	0.36	87.42	0.22	87.25	0.39	87.35	0.29	87.30	0.34	87.33	0.31	87.06	0.58
MH309	309	90.15	87.75	87.44	0.31	87.08	0.67	87.33	0.42	87.44	0.31	87.41	0.34	87.43	0.32	87.22	0.53
MH308	308	89.68	87.28	86.88	0.40	86.69	0.59	86.81	0.47	86.88	0.40	86.87	0.41	86.88	0.40	86.76	0.52
MH326	326	94.76	92.36	91.33	1.03	91.33	1.03	91.32	1.04	91.32	1.04	91.32	1.04	91.32	1.04	91.33	1.03
MH318	318	94.40	92.00	91.03	0.97	91.03	0.97	91.00	1.00	91.03	0.97	91.00	1.00	91.00	1.00	91.00	1.00
MH300	300	94.00	91.60	90.71	0.89	90.70	0.90	90.67	0.93	90.70	0.90	90.68	0.92	90.68	0.92	90.68	0.92
MH301	301	93.73	91.33	90.21	1.12	90.21	1.12	90.20	1.13	90.20	1.13	90.21	1.12	90.20	1.13	90.20	1.13
MH302	302	92.80	90.40	88.64	1.76	88.64	1.76	88.63	1.77	88.63	1.77	88.64	1.76	88.63	1.77	88.63	1.77
MH303	303	90.67	88.27	87.80	0.47	87.81	0.46	87.63	0.64	87.65	0.62	87.79	0.48	87.72	0.55	87.64	0.63
MH304	304	90.30	87.90	87.39	0.51	87.38	0.52	87.30	0.60	87.31	0.59	87.38	0.52	87.34	0.56	87.30	0.60
MH305	305	91.00	88.60	86.54	2.06	86.56	2.04	86.61	1.99	86.64	1.96	86.69	1.91	86.65	1.95	86.60	2.00
MH319	319	88.81	86.61	86.13	0.48	86.12	0.49	86.12	0.49	86.13	0.48	86.12	0.49	86.12	0.49	86.12	0.49
MH320	320	89.12	86.92	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43	85.49	1.43
MH321	321	87.67	85.47	84.18	1.29	84.39	1.08	84.10	1.37	84.15	1.32	84.11	1.36	84.13	1.34	84.09	1.38
MH322	322	87.50	85.30	84.18	1.12	84.39	0.91	84.10	1.20	84.15	1.15	84.10	1.20	84.12	1.18	84.09	1.21
MH323	323	86.57	84.37	83.40	0.97	83.48	0.89	83.31	1.06	83.37	1.00	83.32	1.05	83.34	1.03	83.30	1.07

## **Appendix B**

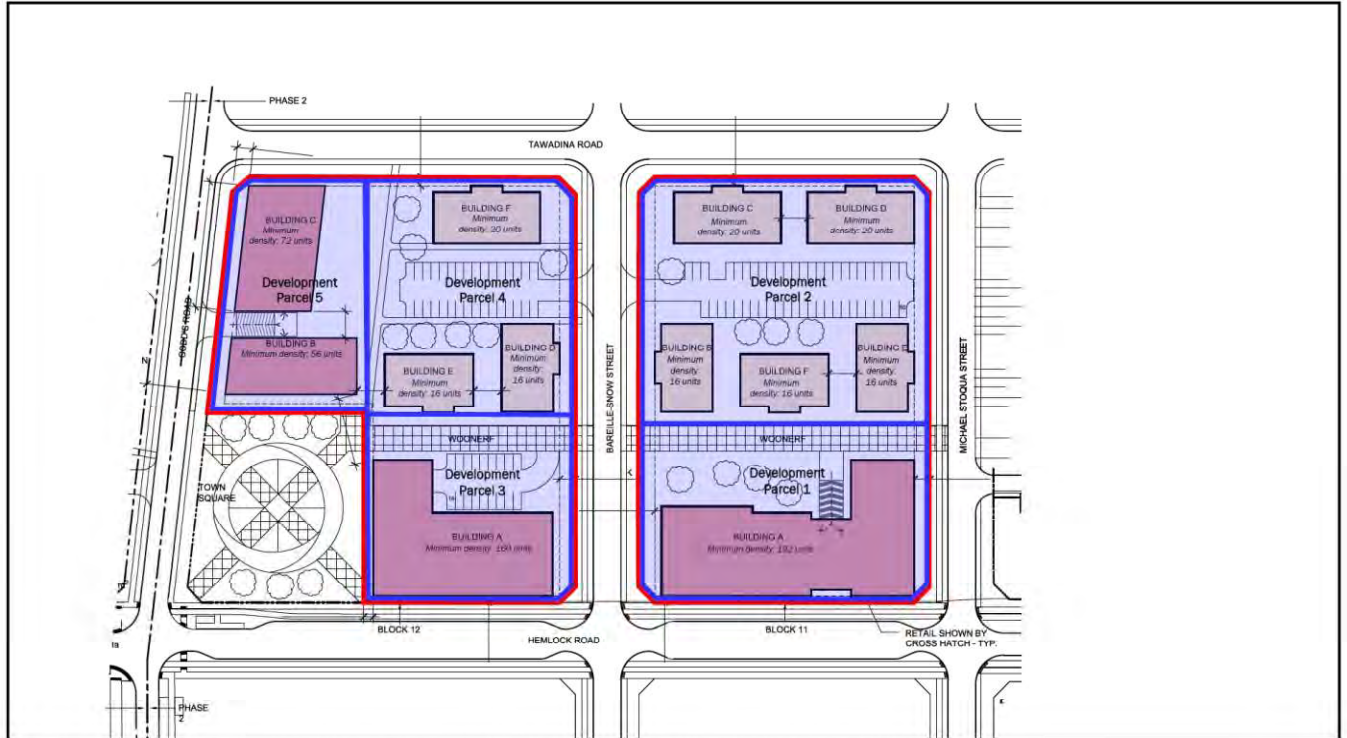
Supporting Sanitary Information

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# SCHEDULE "A"

## PARCEL IDENTIFICATION, DESCRIPTION, AND MINIMUM DENSITY<sup>1</sup>



\*\*Boundaries of the development parcels are estimated. Purchasers to provide dimensioned sketch or electronic survey to confirm these boundaries

<sup>1</sup> This image if provided for demonstration purposes only





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LEGEND

Block 11&12 Proposed Conditions  
 Old Criteria being used

AS-BUILT SANITARY SEWER DESIGN SHEET

Former CFB Rockcliffe  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE		FIXED FLOW	TOTAL FLOW	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA Phase 1B (Ha)	UNIT TYPES				AREA EXTERNAL (Ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD	TH	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM		IND	CUM									L/s	(%)
Phase 1B																																
Block 9	154A	Ex. BULK	MH217Aa	0.19																												
Block 9		MH217Aa	MH217A																													
croissant Squadron Crescent	215Aa-b	MH215A	MH216A	0.79	3	4																										
croissant Squadron Crescent	216Aa-b	MH216A	MH217A	0.67	2	6																										
croissant Squadron Crescent	217A	MH217A	MH218A	0.02																												
croissant Squadron Crescent	218A	MH218A	MH218B	0.02																												
Thorncliffe Village	THORN1	MH600A	MH601A																													
Thorncliffe Village		MH601A	MH218B																													
croissant Squadron Crescent	218B	MH218B	MH219A	0.07																												
croissant Squadron Crescent	219A	MH219A	MH220A	0.15																												
croissant Squadron Crescent	220A, 220B	MH220A	MH221A	1.46																												
croissant Squadron Crescent	221A	MH221A	MH222A	0.02																												
croissant Squadron Crescent		MH222A	MH223A																													
croissant Squadron Crescent	BLOCK 15	BLK223AE	MH223A																													
Design by Others																																
croissant Squadron Crescent	222A	MH223A	MH165A	0.22																												
<b>Design Parameters:</b>				<b>Notes:</b>										<b>Designed:</b>						<b>No.</b>		<b>Revision</b>						<b>Date</b>				
Residential				1. Mannings coefficient (n) = 0.013										WY						1.		City submission No. 1						2016-07-08				
SF 3.4 p/p/u				2. Demand (per capita): 350 L/day										JIM						2.		City submission No. 2						2016-11-04				
TH/SD 2.7 p/p/u				3. Infiltration allowance: 0.28 L/s/Ha										38298-501						3.		City submission No. 3						2017-01-25				
APT 1.8 p/p/u				4. Residential Peaking Factor: Harmon Formula = 1+(4+P^0.5) where P = population in thousands																4.		Revised as per Mattamy's Design						2017-12-08				
Other 60 p/p/Ha																				5.		As-Built Submission						2018-01-29				
																				6.		Block 11 & 12 Study						2022-03-15				
																				File Reference:		Date:						Sheet No:				
																				38298.5.7.1		2016-07-08						1 of 2				



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**LEGEND**  
 MH231A Existing infrastructure (shown for information only)  
 Block 11, 12 Existing Conditions

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										IND	CUM
Pimiwidon Street	MH317-1, MH317-2	MH317A	MH316A	1.50	1	104			284.2	284.2	3.47	3.20	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.50	1.50	0.50	0.00	0.00	3.69	40.68	83.00	250	0.43	0.803	36.99	90.93%	
Pimiwidon Street	MH316A	MH316A	BULK202AN	0.16		1			2.7	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.16	1.66	0.55	0.00	0.00	3.77	37.74	43.10	250	0.37	0.745	33.96	90.00%	
Pimiwidon Street	-	BULK202AN	MH202A						0.0	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.66	0.55	0.00	0.00	3.77	40.68	21.00	250	0.43	0.803	36.91	90.72%	
Wigwas Street	MH315A	MH315A	MH314A	0.79	2	18			55.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.79	0.79	0.26	0.00	0.00	0.92	49.63	111.64	250	0.64	0.979	48.72	98.16%	
Wigwas Street	MH314A	MH314A	BULK203AN	0.06					0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.06	0.85	0.28	0.00	0.00	0.93	83.46	14.37	250	1.81	1.647	82.53	98.88%	
Wigwas Street	-	BULK203AN	MH203A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.85	0.28	0.00	0.00	0.93	80.17	21.00	250	1.67	1.582	79.24	98.83%	
Moses Tennisco Street	MH313A	MH313A	MH312A	0.66	2	16			50.0	50.0	3.65	0.59	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.66	0.66	0.22	0.00	0.00	0.81	75.73	77.20	250	1.49	1.495	74.92	98.93%	
Moses Tennisco Street	MH312A, PARK	MH312A	BULK204AN	0.21		2			5.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.87	0.29	0.00	0.00	0.94	94.29	49.70	250	2.31	1.861	93.35	99.00%	
Park	PARK	MH350A	pipe	0.42					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.42	0.42	0.14	0.00	0.00	0.14	48.39	11.00	200	2.00	1.492	48.25	99.71%	
Moses Tennisco Street	-	BULK204AN	MH204A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.87	0.29	0.00	0.00	0.94	89.90	21.00	250	2.10	1.774	88.96	98.95%	
Michael Stoqua Street	MH311A	MH311A	MH310A	0.44	1	9			27.7	27.7	3.69	0.33	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.44	0.44	0.15	0.00	0.00	0.48	72.35	77.82	250	1.36	1.428	71.87	99.34%	
Michael Stoqua Street	MH310A	MH310A	BULK205AN	0.21		2			5.4	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.65	0.21	0.00	0.00	0.61	65.66	49.19	250	1.12	1.296	65.05	99.07%	
Michael Stoqua Street	-	BULK205AN	MH205A						0.0	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.65	0.21	0.00	0.00	0.61	66.24	21.00	250	1.14	1.307	65.63	99.08%	
Wanaki Road	MH200A	MH200A	MH318A						0.0	0.0	3.80	0.00	0.00	0.00	1.01	1.01	0.00	0.00	1.50	0.49	1.01	1.01	0.33	0.00	0.00	0.82	42.53	63.35	250	0.47	0.839	41.71	98.06%	
Wanaki Road	MH318A	MH318A	MH300A						0.0	0.0	3.80	0.00	0.00	0.95	1.96	0.00	0.00	1.50	0.95	0.95	1.96	0.85	0.00	0.00	1.60	42.53	77.11	250	0.47	0.839	40.93	96.24%		
Tawadina Road	MH300A	MH300A	MH301A	0.47		15			40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%	
Tawadina Road	MH301A	MH301A	MH302A	0.54		14			37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%	
Tawadina Road	MH302A	MH302A	MH303A	0.26		2			5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%	
Tawadina Road	MH303A	MH303A	MH304A	0.21					0.0	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.21	3.44	1.14	0.00	0.00	3.07	31.02	112.10	250	0.25	0.612	27.95	90.11%	
Tawadina Road	MH305A	MH305A	MH304A	0.24					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Barelle-Snow Street	EXT-1	BULK304AN	MH304A	7.35				905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%	
Barelle-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	1.47				190	342.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	1.47	12.50	4.13	0.00	0.00	25.14	39.72	119.21	250	0.41	0.784	14.58	36.70%	
Barelle-Snow Street	MH308A	MH308A	BULK206AN	0.07					0.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.07	12.57	4.15	0.00	0.00	25.17	84.15	16.82	250	1.84	1.661	58.99	70.09%	
Barelle-Snow Street	-	BULK206AN	MH206A						0.0	2054.7	3.06	20.38	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	12.57	4.15	0.00	0.00	25.17	88.83	21.00	250	2.05	1.753	63.66	71.67%	
Codd's Road	MH340A	MH340A	BLK231AN	1.78				278	500.4	500.4	3.38	5.48	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.78	1.78	0.59	0.00	0.00	6.07	75.98	70.00	250	1.50	1.500	69.91	92.01%	
Codd's Road	-	MH231A	BULK176AN						0.0	500.4	3.38	5.48	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.78	0.59	0.00	0.00	6.07	83.92	50.22	250	1.83	1.656	77.86	92.77%	

**Design Parameters:**

Residential	ICI Areas
SF 3.4 p/p/u	
TH/F/SD 2.7 p/p/u	INST 28,000 L/Ha/day
TH/S 2.3 p/p/u	COM 28,000 L/Ha/day
APT 1.8 p/p/u	IND 35,000 L/Ha/day
Other 60 p/p/Ha	17000 L/Ha/day

MOE Chart

**Notes:**

- Mannings coefficient (n) = 0.013
- Demand (per capita): 280 L/day 200 L/day
- Infiltration allowance: 0.33 L/s/Ha
- Residential Peaking Factor:  
 Harmon Formula =  $1 + (14 / (4 + (P / 1000)^{0.5})) \cdot 0.8$   
 where K = 0.8 Correction Factor
- Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0

**Designed:** KH

**Checked:** JIM

**Dwg. Reference:** 118863-400

No.	Revision	Date
1	Submission No. 1 for City Review	2018-12-20
2	Submission No. 2 for City Review	2019-03-15
3	MECP Submission	2019-04-17
4	Record information Added (No.1)	2020-10-08
5	Record information Added (No.2)	2021-03-23
6	Block 11 & 12 Study	2022-03-15

**File Reference:** 118863.5.7.1

**Date:** 2021-03-31

**Sheet No:** 1 of 1



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**LEGEND**  
 MH231A Existing infrastructure (shown for information only)  
 Block 11&12 Proposed Conditions

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		AREA (Ha)		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	(%)
Pimiwidon Street	MH317-1, MH317-2	MH317A	MH316A	1.50	1	104			284.2	284.2	3.47	3.20	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.50	1.50	0.50	0.00	0.00	3.69	40.68	83.00	250	0.43	0.803	36.99	90.93%	
Pimiwidon Street	MH316A	MH316A	BULK202AN	0.16		1			2.7	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.16	1.66	0.55	0.00	0.00	3.77	37.74	43.10	250	0.37	0.745	33.96	90.00%	
Pimiwidon Street	-	BULK202AN	MH202A						0.0	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.66	0.55	0.00	0.00	3.77	40.68	21.00	250	0.43	0.803	36.91	90.72%	
Wigwas Street	MH315A	MH315A	MH314A	0.79	2	18			55.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.79	0.79	0.26	0.00	0.00	0.92	49.63	111.64	250	0.64	0.979	48.72	98.16%	
Wigwas Street	MH314A	MH314A	BULK203AN	0.06					0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.06	0.85	0.28	0.00	0.00	0.93	83.46	14.37	250	1.81	1.647	82.53	98.88%	
Wigwas Street	-	BULK203AN	MH203A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.85	0.28	0.00	0.00	0.93	80.17	21.00	250	1.67	1.582	79.24	98.83%	
Moses Tennisco Street	MH313A	MH313A	MH312A	0.66	2	16			50.0	50.0	3.65	0.59	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.66	0.66	0.22	0.00	0.00	0.81	75.73	77.20	250	1.49	1.495	74.92	98.93%	
Moses Tennisco Street	MH312A, PARK	MH312A	BULK204AN	0.21		2			5.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.87	0.29	0.00	0.00	0.94	94.29	49.70	250	2.31	1.861	93.35	99.00%	
Park	PARK	MH350A	pipe	0.42					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.42	0.42	0.14	0.00	0.00	0.14	48.39	11.00	200	2.00	1.492	48.25	99.71%	
Moses Tennisco Street	-	BULK204AN	MH204A						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.87	0.29	0.00	0.00	0.94	89.90	21.00	250	2.10	1.774	88.96	98.95%	
Michael Stoqua Street	MH311A	MH311A	MH310A	0.44	1	9			27.7	27.7	3.69	0.33	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.44	0.44	0.15	0.00	0.00	0.48	72.35	77.82	250	1.36	1.428	71.87	99.34%	
Michael Stoqua Street	MH310A	MH310A	BULK205AN	0.21		2			5.4	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.65	0.21	0.00	0.00	0.61	65.66	49.19	250	1.12	1.296	65.05	99.07%	
Michael Stoqua Street	-	BULK205AN	MH205A						0.0	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.65	0.21	0.00	0.00	0.61	66.24	21.00	250	1.14	1.307	65.63	99.08%	
Wanaki Road	MH200A	MH200A	MH318A						0.0	0.0	3.80	0.00	0.00	0.00	1.01	1.01	0.00	0.00	1.50	0.49	1.01	1.01	0.33	0.00	0.00	0.82	42.53	63.35	250	0.47	0.839	41.71	98.06%	
Tawadina Road	MH300A	MH300A	MH301A	0.47		15			40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%	
Tawadina Road	MH301A	MH301A	MH302A	0.54		14			37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%	
Tawadina Road	MH302A	MH302A	MH303A	0.26		2			5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%	
Tawadina Road	MH303A	MH303A	MH304A	0.21					0.0	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.21	3.44	1.14	0.00	0.00	3.07	31.02	112.10	250	0.25	0.612	27.95	90.11%	
Tawadina Road	MH305A	MH305A	MH304A	0.24					0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Bareille-Snow Street	EXT-1	BULK304AN	MH304A	7.35				905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%	
Bareille-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	1.48				140	252.0	1964.7	3.07	19.57	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	1.48	12.51	4.13	0.00	0.00	24.33	39.72	119.21	250	0.41	0.784	15.39	38.75%	
Bareille-Snow Street	MH308A	MH308A	BULK206AN	0.96				352	633.6	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.96	13.47	4.45	0.00	0.00	30.31	84.15	16.82	250	1.84	1.661	53.85	63.99%	
Bareille-Snow Street	-	BULK206AN	MH206A						0.0	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	13.47	4.45	0.00	0.00	30.31	88.83	21.00	250	2.05	1.753	58.52	65.88%	
Codd's Road	MH340A	MH340A	BLK231AN	0.88				212	381.6	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	75.98	70.00	250	1.50	1.500	71.46	94.04%	
Codd's Road	MH231A	MH231A	BULK176AN						0.0	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.88	0.29	0.00	0.00	4.53	83.92	50.22	250	1.83	1.656	79.40	94.61%	

**Design Parameters:**

Residential	ICI Areas	
SF 3.4 p/p/u		
TH/F/SD 2.7 p/p/u	INST 28,000 L/Ha/day	
TH/S 2.3 p/p/u	COM 28,000 L/Ha/day	
APT 1.8 p/p/u	IND 35,000 L/Ha/day	MOE Chart
Other 60 p/p/ha	17000 L/Ha/day	

**Notes:**

- Mannings coefficient (n) = 0.013
- Demand (per capita): 280 L/day
- Infiltration allowance: 0.33 L/s/ha
- Residential Peaking Factor: Harmon Formula =  $1 + (14 / (4 + (P/1000)^{0.5})) \cdot 0.8$  where K = 0.8 Correction Factor
- Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0

**Designed:** KH

**Checked:** JIM

**Dwg. Reference:** 118863-400

No.	Revision	Date
1	Submission No. 1 for City Review	2018-12-20
2	Submission No. 2 for City Review	2019-03-15
3	MECP Submission	2019-04-17
4	Record information Added (No.1)	2020-10-08
5	Record information Added (No.2)	2021-03-23
6	Block 11 & 12 Study	2022-03-15

**File Reference:** 118863.5.7.1

**Date:** 2021-03-31

**Sheet No:** 1 of 1

Mary Jarvis – November 23, 2022

## **APPENDIX B**

- Sanitary Sewer Spreadsheet – Original Concept Site Plan
- Sanitary Sewer Spreadsheet – DesignWorks Engineering Site Plan



**IBI GROUP**  
 400-333 Preston Street  
 Ottawa, Ontario K1S 5N4 Canada  
 tel 613 225 1311 fax 613 225 9868  
 ibigroup.com

**LEGEND**  
 MH231A Existing infrastructure (shown for information only)  
 Block 11&12 Proposed Conditions

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		AREA (Ha)		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	(%)
Pimiwidon Street	MH317-1, MH317-2	MH317A	MH316A	1.50	1	104				284.2	284.2	3.47	3.20	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	1.50	1.50	0.50	0.00	0.00	3.69	40.68	83.00	250	0.43	0.803	36.99	90.93%
Pimiwidon Street	MH316A	MH316A	BULK202AN	0.16		1				2.7	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.16	1.66	0.55	0.00	0.00	3.77	37.74	43.10	250	0.37	0.745	33.96	90.00%
Pimiwidon Street	-	BULK202AN	MH202A							0.0	286.9	3.47	3.23	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	1.66	0.55	0.00	0.00	3.77	40.68	21.00	250	0.43	0.803	36.91	90.72%	
Wigwas Street	MH315A	MH315A	MH314A	0.79	2	18				55.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.79	0.79	0.26	0.00	0.00	0.92	49.63	111.64	250	0.64	0.979	48.72	98.16%	
Wigwas Street	MH314A	MH314A	BULK203AN	0.06						0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.06	0.85	0.28	0.00	0.00	0.93	83.46	14.37	250	1.81	1.647	82.53	98.88%	
Wigwas Street	-	BULK203AN	MH203A							0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.85	0.28	0.00	0.00	0.93	80.17	21.00	250	1.67	1.582	79.24	98.83%	
Moses Tennisco Street	MH313A	MH313A	MH312A	0.66	2	16				50.0	50.0	3.65	0.59	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.66	0.66	0.22	0.00	0.00	0.81	75.73	77.20	250	1.49	1.495	74.92	98.93%	
Moses Tennisco Street	MH312A, PARK	MH312A	BULK204AN	0.21		2				5.4	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.87	0.29	0.00	0.00	0.94	94.29	49.70	250	2.31	1.861	93.35	99.00%	
Park	PARK	MH350A	pipe	0.42						0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.42	0.42	0.14	0.00	0.00	0.14	48.39	11.00	200	2.00	1.492	48.25	99.71%	
Moses Tennisco Street	-	BULK204AN	MH204A							0.0	55.4	3.64	0.65	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.87	0.29	0.00	0.00	0.94	89.90	21.00	250	2.10	1.774	88.96	98.95%	
Michael Stoqua Street	MH311A	MH311A	MH310A	0.44	1	9				27.7	27.7	3.69	0.33	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.44	0.44	0.15	0.00	0.00	0.48	72.35	77.82	250	1.36	1.428	71.87	99.34%	
Michael Stoqua Street	MH310A	MH310A	BULK205AN	0.21		2				5.4	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.21	0.65	0.21	0.00	0.00	0.61	65.66	49.19	250	1.12	1.296	65.05	99.07%	
Michael Stoqua Street	-	BULK205AN	MH205A							0.0	33.1	3.68	0.39	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.65	0.21	0.00	0.00	0.61	66.24	21.00	250	1.14	1.307	65.63	99.08%	
Wanaki Road	MH200A	MH200A	MH318A							0.0	0.0	3.80	0.00	0.00	0.00	1.01	1.01	0.00	0.00	1.50	0.49	1.01	1.01	0.33	0.00	0.82	42.53	63.35	250	0.47	0.839	41.71	98.06%	
Tawadina Road	MH300A	MH300A	MH301A	0.47		15				40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%	
Tawadina Road	MH301A	MH301A	MH302A	0.54		14				37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%	
Tawadina Road	MH302A	MH302A	MH303A	0.26		2				5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%	
Tawadina Road	MH303A	MH303A	MH304A	0.21						0.0	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.21	3.44	1.14	0.00	3.07	31.02	112.10	250	0.25	0.612	27.95	90.11%	
Tawadina Road	MH305A	MH305A	MH304A	0.24						0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Bareille-Snow Street	EXT-1	BULK304AN	MH304A	7.35					905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%
Bareille-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	1.48					140	252.0	1964.7	3.07	19.57	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	1.48	12.51	4.13	0.00	24.33	39.72	119.21	250	0.41	0.784	15.39	38.75%	
Bareille-Snow Street	MH308A	MH308A	BULK206AN	0.96					352	633.6	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.96	13.47	4.45	0.00	30.31	84.15	16.82	250	1.84	1.661	53.85	63.99%	
Bareille-Snow Street	-	BULK206AN	MH206A							0.0	2598.3	3.00	25.23	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	13.47	4.45	0.00	30.31	88.83	21.00	250	2.05	1.753	58.52	65.88%	
Codd's Road	MH340A	MH340A	BLK231AN	0.88					212	381.6	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	75.98	70.00	250	1.50	1.500	71.46	94.04%
Codd's Road	MH231A	MH231A	BULK176AN							0.0	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.88	0.29	0.00	0.00	4.53	83.92	50.22	250	1.83	1.656	79.40	94.61%

**Design Parameters:**

Residential	ICI Areas	
SF 3.4 p/p/u		
TH/F/SD 2.7 p/p/u	INST 28,000 L/Ha/day	
TH/S 2.3 p/p/u	COM 28,000 L/Ha/day	
APT 1.8 p/p/u	IND 35,000 L/Ha/day	MOE Chart
Other 60 p/p/Ha	17,000 L/Ha/day	

**Notes:**

- Mannings coefficient (n) = 0.013
- Demand (per capita): 280 L/day
- Infiltration allowance: 0.33 L/s/Ha
- Residential Peaking Factor:  
 Harmon Formula =  $1 + (14 / (4 + (P / 1000)^{0.5})) \cdot 0.8$   
 where K = 0.8 Correction Factor
- Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0

**Designed:** KH

**Checked:** JIM

**Dwg. Reference:** 118863-400

No.	Revision	Date
1	Submission No. 1 for City Review	2018-12-20
2	Submission No. 2 for City Review	2019-03-15
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6	Block 11 & 12 Study	2022-03-15

**File Reference:** 118863.5.7.1

**Date:** 2021-03-31

**Sheet No:** 1 of 1



**IBI GROUP**  
 400-333 Preston Street  
 Ottawa, Ontario K1S 5N4 Canada  
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 ibigroup.com

LEGEND  
 MH231A Existing infrastructure (shown for information only)  
 Block 11 Proposed Conditions (DesignWorks Engineering)

**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	(%)
Tawadina Road	MH300A	MH300A	MH301A	0.47		15				40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%
Tawadina Road	MH301A	MH301A	MH302A	0.54		14				37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%
Tawadina Road	MH302A	MH302A	MH303A	0.26		2				5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%
Tawadina Road	MH303A	MH303A	MH304A	0.93					240	432.0	515.7	3.37	5.64	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.93	4.16	1.37	0.00	0.00	7.96	31.02	112.10	250	0.25	0.612	23.06	74.33%
Tawadina Road	MH305A	MH305A	MH304A	0.24						0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%
Bareille-Snow Street	EXT-1	BULK304AN	MH304A	7.35					905	1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%
Bareille-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	0.76					52	93.6	2238.3	3.04	22.04	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.76	12.51	4.13	0.00	0.00	26.80	39.72	119.21	250	0.41	0.784	12.93	32.54%
Bareille-Snow Street	MH308A	MH308A	BULK206AN	0.96					352	633.6	2871.9	2.97	27.61	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.96	13.47	4.45	0.00	0.00	32.69	84.15	16.82	250	1.84	1.661	51.46	61.15%
Bareille-Snow Street		BULK206AN	MH206A							0.0	2871.9	2.97	27.61	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	13.47	4.45	0.00	0.00	32.69	88.83	21.00	250	2.05	1.753	56.13	63.20%
Codd's Road	MH340A	MH340A	BLK231AN	0.88					212	381.6	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	75.98	70.00	250	1.50	1.500	71.46	94.04%
Codd's Road		MH231A	BULK176AN							0.0	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.00	0.88	0.29	0.00	0.00	4.53	83.92	50.22	250	1.83	1.656	79.40	94.61%
<b>Design Parameters:</b>				<b>Notes:</b>										<b>Designed:</b> KH						<b>No.</b>			<b>Revision</b>						<b>Date</b>					
Residential				1. Mannings coefficient (n) = 0.013										2. Demand (per capita): 280 L/day						1			Submission No. 1 for City Review						2018-12-20					
SF 3.4 p/p/u				3. Infiltration allowance: 0.33 L/s/Ha										4. Residential Peaking Factor: Harmon Formula = 1+(14/(4+(P/1000)^0.5))0.8 where K = 0.8 Correction Factor						2			Submission No. 2 for City Review						2019-03-15					
TH/F/SD 2.7 p/p/u				5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0										Checked: JIM						3			MECP Submission						2019-04-17					
TH/S 2.3 p/p/u														Dwg. Reference: 118863-400						4			Record information Added (No.1)						2020-10-08					
APT 1.8 p/p/u																				5			Record information Added (No.2)						2021-03-23					
Other 60 p/p/Ha																				File Reference: 118863.5.7.1						Date: 2021-03-31						Sheet No: 1 of 1		

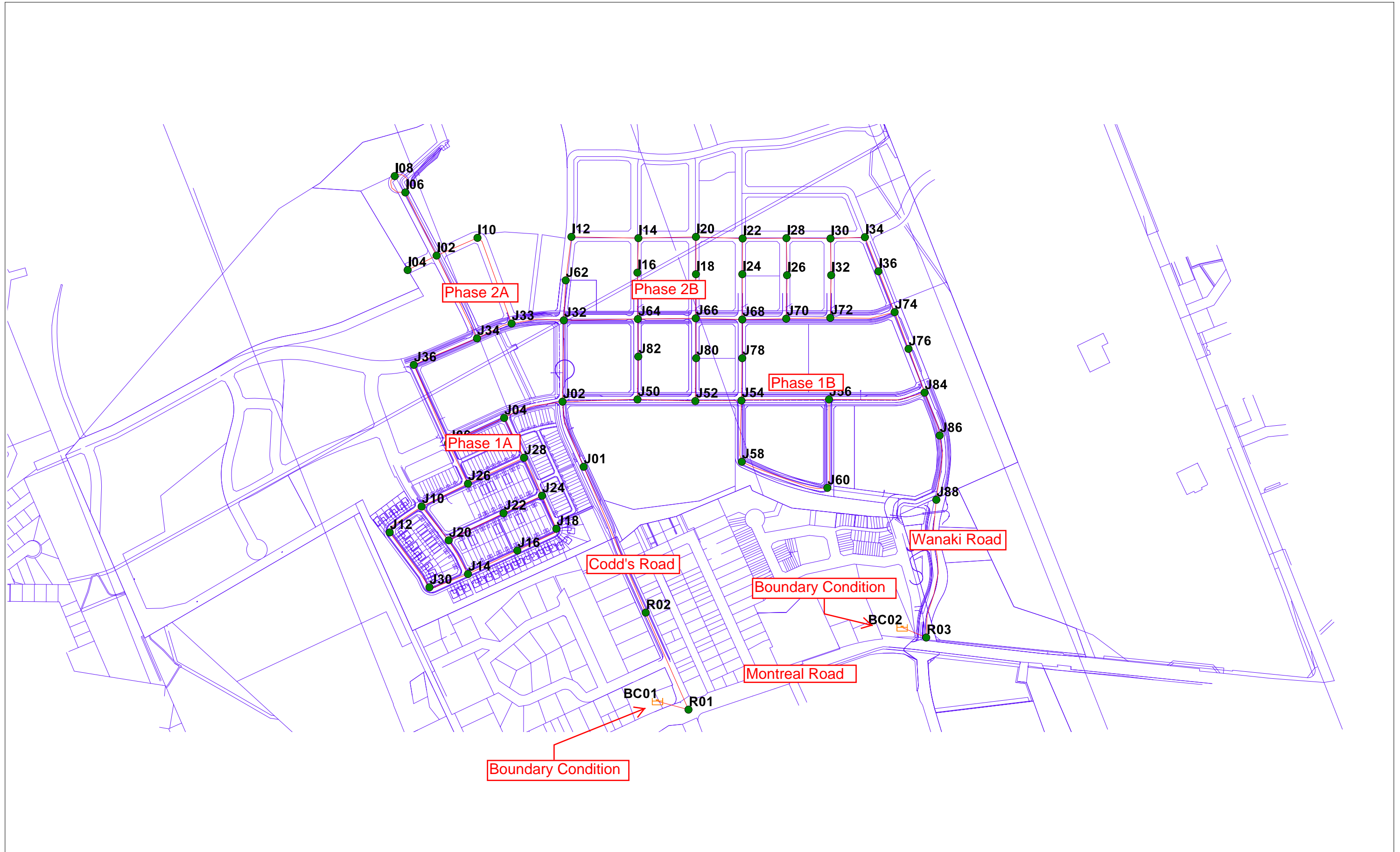


Mary Jarvis – November 23, 2022

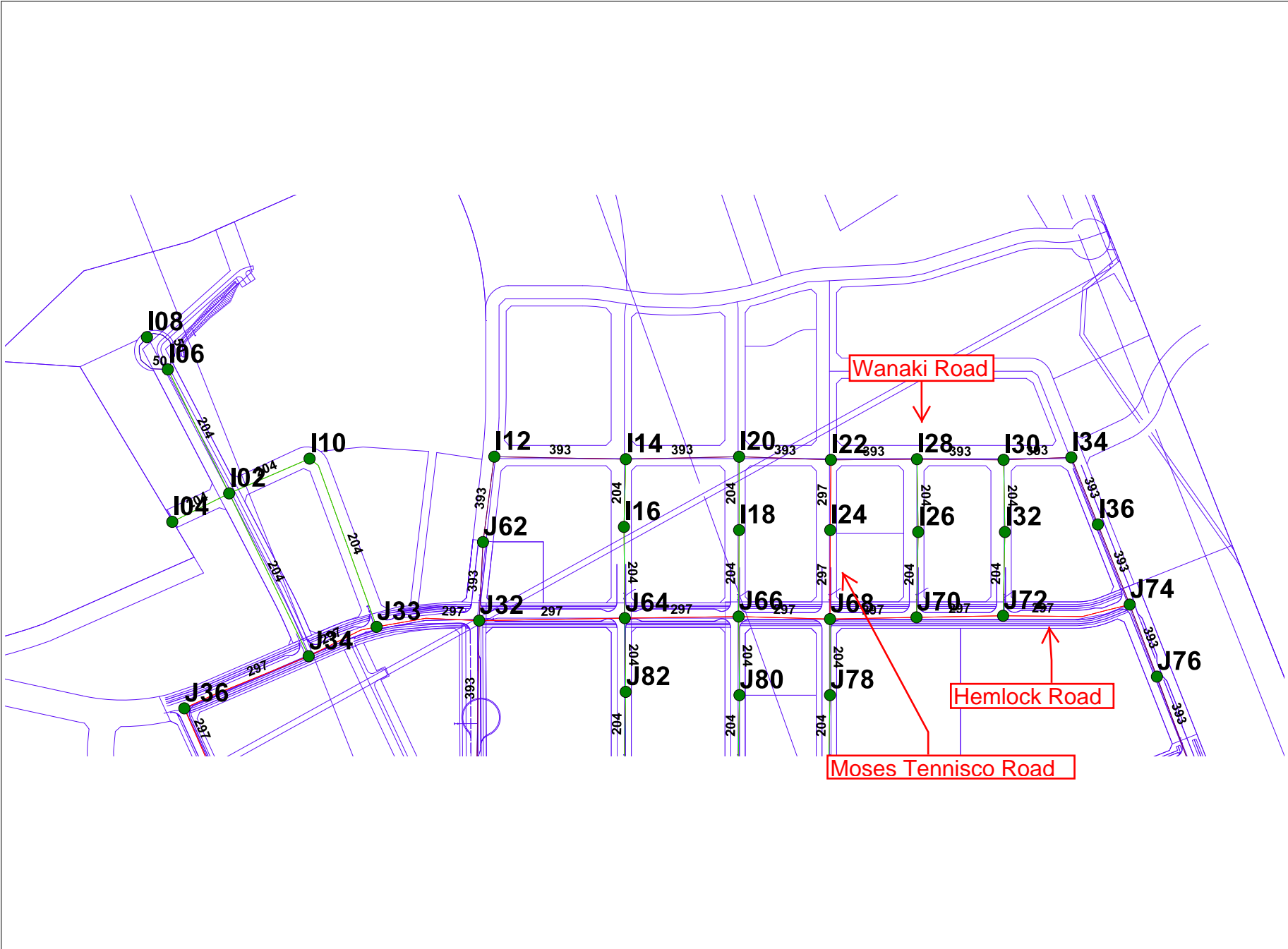
## APPENDIX C

- Water Modeling Results – Phase 2B Design Brief

# Wateridge Overall Model



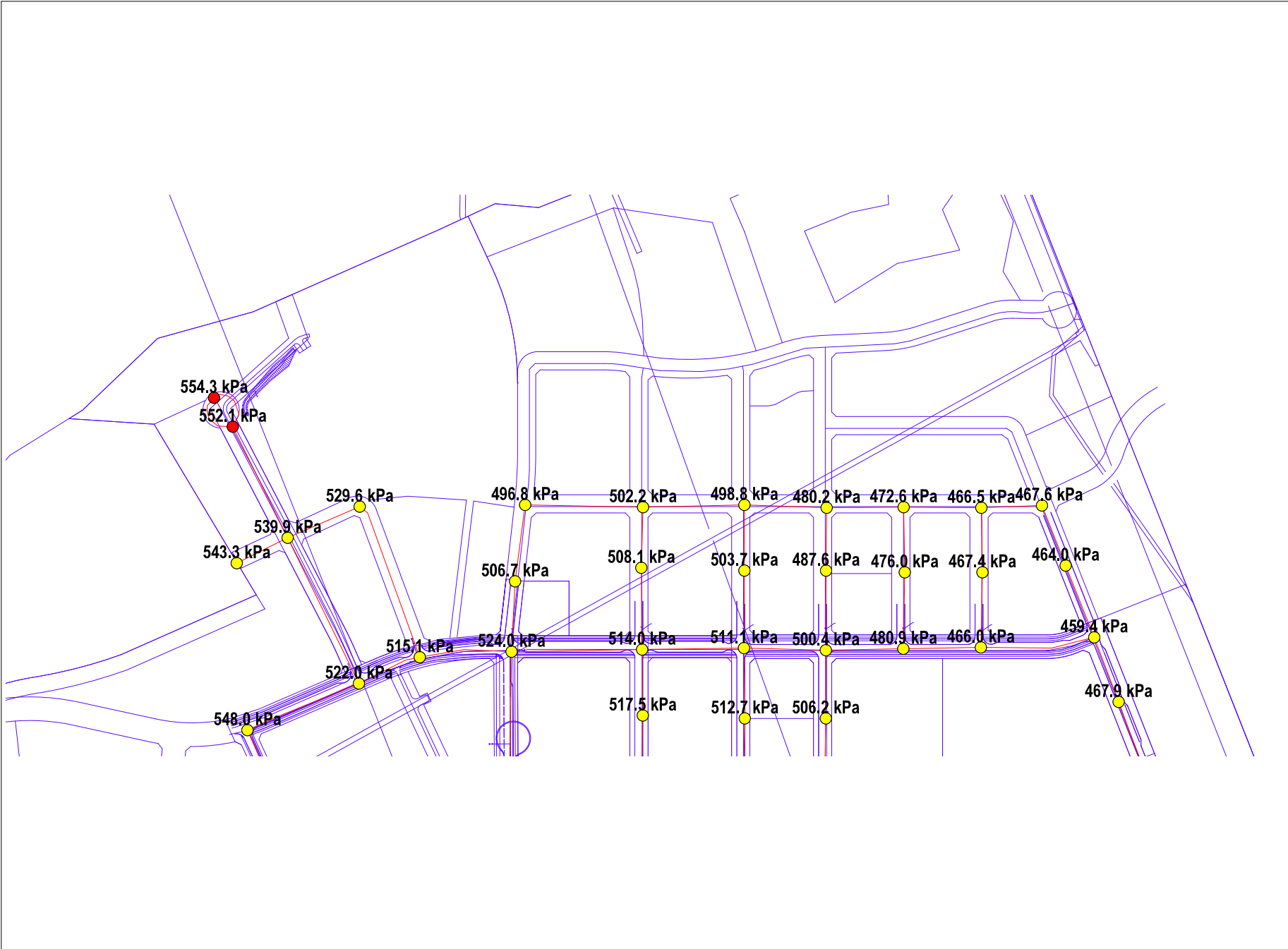
# Phase 2 Node ID's and Pipe Sizes



# Phase 2 Basic Day (Max HGL) Pressures



# Phase 2 Peak Hour Pressures



# Phase 2 Max Day + Fire Design Fireflows



Mary Jarvis – November 23, 2022

## APPENDIX D

- Low Impact Development (LID) Review

**To:** Anton Chettrar & Jim Moffatt  
IBI  
400-333 Preston Street, Ottawa, ON K1S 5N4

Krisendat Sewgoolam & Mary Jarvis  
Canada Lands Corporation (CLC)  
30 Metcalfe Street, Suite 601, Ottawa, Ontario, K1P 5L4

**From:** Chris Denich, M.Sc. P.Eng., Aquafor Beech Ltd.  
55 Regal Road, Guelph, ON, N1K 1B6

**Re:** Submission 1: Site Plan Package Submission to Canada Lands Company; 1050 Tawadina Road, Ottawa

---

At the request of CLC, we have completed a review of submission 1 for 1050 Tawadina Road, Ottawa (Block 11) in regards to the Low Impact Development (LID) requirements. The review has been based on the designs as detailed in the relevant reports and site drawings prepared by Westurban Developments and offer the following advisory comments, without prejudice. The following documents, reports and drawings were reviewed:

1. Wateridge Village Municipal Servicing and Stormwater Management Feasibility Study Report (October 21, 2022) – Prepared by Design Works Engineering Ltd.;
2. Civil Drawings (Issued for CLC Submission) – October 25, 2022 – Prepared by Design Works Engineering Ltd:
  - a. Site Grading Plan;
  - b. Site Servicing Plan;
  - c. Site Erosion and Sediment Control Plan;
  - d. Utility Plan;
3. Geotechnical Investigation – Proposed Two New Apartments Buildings 1050 Tawadina Road, Ottawa, ON (November 3, 2022) – Prepared by Englobe.
4. Architectural Drawings (undated) – Prepared By Formed Alliance Architects Studio (FAAS)
5. Landscape Drawings (October 24, 2022) – Prepared by CSW

### **General Comments**

1. In regards to submission 1, it is noted that CLC's goal for this overall development is for the Wateridge Village development (Former CFB Rockcliffe) to be a model community for LID. In general, the proposed design is not in keeping with CLC's design vision nor the LID Demonstration Project goals and objectives, including overall aesthetic enhancement and synergies using LIDs. The current site plan does not demonstrate LID technologies to the full extent.
2. It is acknowledged that per Section 5.3 Wateridge Village Municipal Servicing and Stormwater Management Feasibility Study Report that reference has been appropriately made to Wateridge Phase 2B LID Developer's Checklist, which was include as Appendix D. It is further noted that notwithstanding the comments below, the design calculations demonstrates that proposed LID achieves the required 4mm LID Infiltration target and 4mm LID Erosion Target, but does not achieve the required Minimum Water Quality Target of the 15mm event as specified in Table 2.1.

#### **Guelph Office:**

55 Regal Road, Unit 3, Guelph, Ontario, N1K 1B6  
Tel: 519-224-3740 • Fax: 519-224-3750

#### **Head Office:**

2600 Skymark Ave, Mississauga, Ontario, L4W 5B2  
Tel: 905-629-0099 • Fax: 905-629-0089



3. It is acknowledged that a series two (2) Soleno Underground Infiltration Systems (Solo Max Perforated Subdrain) been included with the intent of infiltrating runoff from the respective roof drainage area. The following is noted:
  - a. Sufficient design details have not been provided for the proposed Underground Infiltration Systems. No design details and/or cross-sections are provided within the civil drawings and no product specifications/ technical documents. Trench widths, bedding materials, filter fabrics, founding elevations, backfill and compaction requirements etc. should be detailed.
  - b. Per the TRCA/CVC LID Planning and Design Guide (2010), Wiki Document ([wiki.sustainabletechnologies.ca](http://wiki.sustainabletechnologies.ca)) or most current, infiltration galleries (soakaways, trenches and chambers), should be set back at least four (4) metres from building foundations (specifically where liveable spaces, mechanical rooms, parking or other are located sub-surface) unless infiltration facility inverts are located below the lowest finished floor elevation. As such the following is recommended:
    - i. Show offset from the respective Building A proximal to the infiltration gallery and increase to 4m if feasible.
    - ii. Please confirm if the infiltration system inverts are located below the lowest finished floor elevation of Building A proximal to the infiltration system.
    - iii. If 4m cannot be accommodated or infiltration systems cannot be located below the lowest finished floor elevation, it is recommended that inclusion of impermeable barriers proximal to the building side of the infiltration system or additional building waterproofing be included.
  - c. It is understood that the infiltration systems will accept roof runoff. Pre-treatment devices (leaf screens and/or filters) are recommended to prevent debris from entering the infiltration systems.
  - d. The Wateridge Village Municipal Servicing and Stormwater Management Feasibility Study Report should include a discussion of winter operation/ functionality of the infiltration systems
  - e. Per the TRCA/CVC LID Planning and Design Guide (2010), Wiki Document ([wiki.sustainabletechnologies.ca](http://wiki.sustainabletechnologies.ca)) or most current, please confirm that the impervious drainage area to the areas of each infiltration systems is between 5:1 and 20:1.
  - f. LID specific Erosion and Sediment Controls and Construction Staging for Section 5.21 of the Stormwater Management Existing Conditions Report & LID Pilot Project Scoping (Aquafor Beech (2015) have not been provided. LID controls that rely on infiltration require specific ESC controls to be in place during construction to prevent contamination/ clogging during construction.
  - g. LID designs should reference the requirements of the City of Ottawa, Low Impact Development Technical Guidance Report – Implementation in Areas with Potential Hydrogeological Constraints (February 2021) for design, analysis and in-situ testing requirements.
4. No discussion or details are provided with the Wateridge Village Municipal Servicing and Stormwater Management Feasibility Study Report or the Geotechnical Investigation in regards to the site context as it relates to the Underground Infiltration Systems specifically:
  - a. In-situ Infiltration rates of the native soils within the proposed footprint of the Underground Infiltration Systems
  - b. the seasonally high groundwater elevation,

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- c. bedrock elevation, and
  - d. the soil stratigraphy that proposed Underground Infiltration Systems would be founded
5. As an advisory comment, opportunities for additional LID integration into the site include but are not limited to:
- a. Raised planter areas: opportunity to design as bioretention planters
  - b. Tree plantings: opportunity to design tree pits or cluster plantings
  - c. Area drains: opportunity to design as bioretention areas
  - d. Unit paver areas: opportunity to design as permeable pavements

The above noted comments should be considered preliminary in nature and limited to the information provided. Additional information shall be required prior to Aquafor Beech completing a thorough and complete review.

August 17, 2023

Cameron Salisbury, MEdes., RPP., MCIP  
Directory of Development  
West Urban Developments Ltd.  
111-2036 Island Highway South  
Campbell River, BC  
V9W 0E8

Re: Permeability Testing and Monitoring Well Installations – 1050 Tawadina Road, Ottawa

McIntosh Perry ('MP') was retained by Cameron Salisbury of West Urban Developments Ltd. ('Client') to conduct permeability investigations within an undeveloped parcel of land located at 1050 Tawadina Road in Ottawa, Ontario ('the Site'). The scope of work included the completion of in-situ permeability testing at two locations in the northwest and southeast corner of the Site, at varying depths (0.5, 1.0., and 1.5 m below ground surface (bgs)). Additionally, McIntosh Perry advanced two boreholes equipped with monitoring wells within these same areas.

### **Permeability Testing – Infiltration Values**

McIntosh Perry completed permeability testing in the northwest and southeast corners of the Site at 0.5, 1.0., and 1.5 m bgs. To complete these tests, MP utilized a Guelph Permeameter (a constant head permeameter used to measure in-situ saturated hydraulic conductivities of soil). Holes were dug using either a hand auger or mechanized equipment (backhoe). A total of six (6) holes/test pits were advanced, three (3) within the northwest portion of the Site (Hole 1A, TP1, and TP2), and three (3) within the southeast portion of the Site (TP3, TP4, and TP5). The locations of these holes are indicated on Figure 1 below. This work was completed on July 17, 2023 (Hole 1A) and August 2, 2023 (TP1, TP2, TP3, TP4, TP5).



**Figure 1. Infiltration Testing and Monitoring Well Locations**

Each infiltration test consisted of a 5-15 cm head test, based on the level of saturation and subsurface materials encountered where testing was attempted. Water was added to the Guelph Permeameter reservoir and allowed to infiltrate into the soil at the specified head pressure. Changes in reservoir water level (h) were recorded at regular intervals and normalized for change in time (t). Each test was considered complete when  $dh/dt$  (change in head/change in time) reached a steady-state for at least three consecutive measurements.

Appendix C.2 of the Toronto Region Conservation Authority's (TRCA) Stormwater Management Criteria (August 2012) provides guidance on the calculation of infiltration rates using field saturated hydraulic conductivity ( $K_s$ ). The recommended calculation is as follows:

$$K_s = (6 \times 10^{-11}) (I)^{3.7363}$$

Where:

- $K_s$  is the field saturated hydraulic conductivity (in cm/s), as measured by a Guelph Permeameter, double-ring infiltrometer, single-ring infiltrometer, or other accepted method
- $I$  is the infiltration rate (in mm/hr)

Based on the above calculation, the estimated soil infiltration rate ( $I$ ) from the data collected at all locations is shown in the table below.

Table 1: Infiltration Rates

Borehole ID	$K_{fs}$ cm/s	Infiltration Rate (mm/ hour)	Corrected I* (mm/ hr)	Subsurface Materials	Depth of Hole (m bgs)
Hole 1A	$4.07 \times 10^{-8}$	5.7	1.64	Clay	0.5
TP1	$2.80 \times 10^{-6}$	17.7	5.08	Silty sand	1.0
TP2	$1.48 \times 10^{-6}$	14.9	4.28	Silty sand	1.5
TP3	$2.95 \times 10^{-6}$	18.02	5.15	Medium to fine- grained sand	0.5
TP4	$2.34 \times 10^{-6}$	16.9	4.84	Silty sand	1.0
TP5	$1.32 \times 10^{-6}$	14.5	4.15	Fine-grained sand with silt and clay	1.5

\* Includes a safety factor calculated per TRCA guidance. Safety factors are chosen based on the ratio of highest to lowest permeability rates measured at the same test location, within unique strata.

As shown, the highest infiltration rate was observed in TP5 at a depth of approximately 1.5 m bgs. The lowest infiltration rate was observed in Hole 1A at a depth of approximately 0.5 m bgs. These values are generally consistent with the observed stratigraphy, in that fine-grain materials will typically have lower hydraulic conductivity rates.

### Monitoring Well Installations

McIntosh Perry installed two (2) boreholes (equipped with monitoring wells) on August 4, 2023. Boreholes were advanced by Strata Drilling under the supervision of McIntosh Perry personnel. One borehole was installed within overburden materials at bedrock refusal (1.9 m bgs), and one was drilled through bedrock materials until interception with groundwater occurred (8.3 m bgs). In addition, groundwater level measurements were obtained from each monitoring well after installation (approximately 10-15 after installation).

Monitoring well BH23-1 (MW) was installed within the southeast portion of the Site to a final depth of 1.9 m bgs. Monitoring well BH23-2 (MW) was installed within the northwest portion of the Site, to a final depth of 8.3 m bgs. Based on test pits dug as part of the infiltration testing, overburden encountered within the area of BH23-1 (MW) and BH23-2 (MW) included cobbles/debris followed by silty sand with trace gravel and clay until refusal on bedrock. Bedrock was encountered at 2 m bgs at BH23-2 (MW), after which time Strata employed the use of an air hammer to advance the borehole to a final depth of 8.3 m bgs. Groundwater was encountered in bedrock between 6-7 m bgs.

The newly installed monitoring wells were constructed using 2" (51 mm) Schedule 40 polyvinyl chloride (PVC) well screen (10 slot), flush-threaded to Schedule 40 PVC riser pipe. A silica sand filter pack was installed from the base of each well screen to 0.3 m above the top of the screen. A bentonite clay seal was installed above the silica sand filter pack to prevent infiltration of surface water into the groundwater monitoring well. The screened interval was positioned to intersect the water table.

### Water Level Measurements

Water levels were measured immediately after the installation of both wells, on August 4, 2023. No groundwater was observed in BH23-1 (MW). Details of groundwater level measurements are described below:

Monitoring Well	Water Level (m bgs)	Well Depth (m bgs)
BH23-1 (MW)	(no water observed)	1.9 (overburden)
BH23-2 (MW)	5.9	8.3 (bedrock)

It should be noted that the above water levels may not be representative of long-term, stabilized groundwater table. Part of the rationale for installing the monitoring wells is to partially provide infrastructure for future measurements of the groundwater table.

We trust that this information is satisfactory for your present requirements. Should you have any questions or require additional information, please do not hesitate to contact the undersigned.

Respectfully submitted,

McIntosh Perry Consulting Engineers Ltd.



Rebecca Leduc, M.Sc.  
Environmental Scientist  
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## Servicing study guidelines for development applications

### 4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

#### 4.1 General Content

- Executive Summary (for larger reports only).
- Date and revision number of the report.
- Location map and plan showing municipal address, boundary, and layout of proposed development.
- Plan showing the site and location of all existing services.
- Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
- Summary of Pre-consultation Meetings with City and other approval agencies.
- Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.
- Statement of objectives and servicing criteria.
- Identification of existing and proposed infrastructure available in the immediate area.
- Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).
- Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
- Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
- Proposed phasing of the development, if applicable.

- Reference to geotechnical studies and recommendations concerning servicing.
  
- All preliminary and formal site plan submissions should have the following information:
  - Metric scale
  
  - North arrow (including construction North)
  
  - Key plan
  
  - Name and contact information of applicant and property owner
  
  - Property limits including bearings and dimensions
  
  - Existing and proposed structures and parking areas
  
  - Easements, road widening and rights-of-way
  
  - Adjacent street names

#### **4.2 Development Servicing Report: Water**

- Confirm consistency with Master Servicing Study, if available
- Availability of public infrastructure to service proposed development
- Identification of system constraints
- Identify boundary conditions
- Confirmation of adequate domestic supply and pressure
- Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
- Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
- Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
- Address reliability requirements such as appropriate location of shut-off valves
- Check on the necessity of a pressure zone boundary modification.
- Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range



- Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
- Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
- Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
- Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.

### 4.3 Development Servicing Report: Wastewater

- Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
- Confirm consistency with Master Servicing Study and/or justifications for deviations.
- Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
- Description of existing sanitary sewer available for discharge of wastewater from proposed development.
- Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
- Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
- Description of proposed sewer network including sewers, pumping stations, and forcemains.
- Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).
- Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
- Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
- Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
- Special considerations such as contamination, corrosive environment etc.

#### 4.4 Development Servicing Report: Stormwater Checklist

- Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)
- Analysis of available capacity in existing public infrastructure.
- A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
- Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
- Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
- Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
- Set-back from private sewage disposal systems.
- Watercourse and hazard lands setbacks.
- Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
- Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.
- Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
- Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
- Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
- Any proposed diversion of drainage catchment areas from one outlet to another.
- Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
- If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.
- Identification of potential impacts to receiving watercourses
- Identification of municipal drains and related approval requirements.
- Descriptions of how the conveyance and storage capacity will be achieved for the development.
- 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.

- Inclusion of hydraulic analysis including hydraulic grade line elevations.
- Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
- Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
- Identification of fill constraints related to floodplain and geotechnical investigation.

#### **4.5 Approval and Permit Requirements: Checklist**

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

- Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.
- Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
- Changes to Municipal Drains.
- Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)

#### **4.6 Conclusion Checklist**

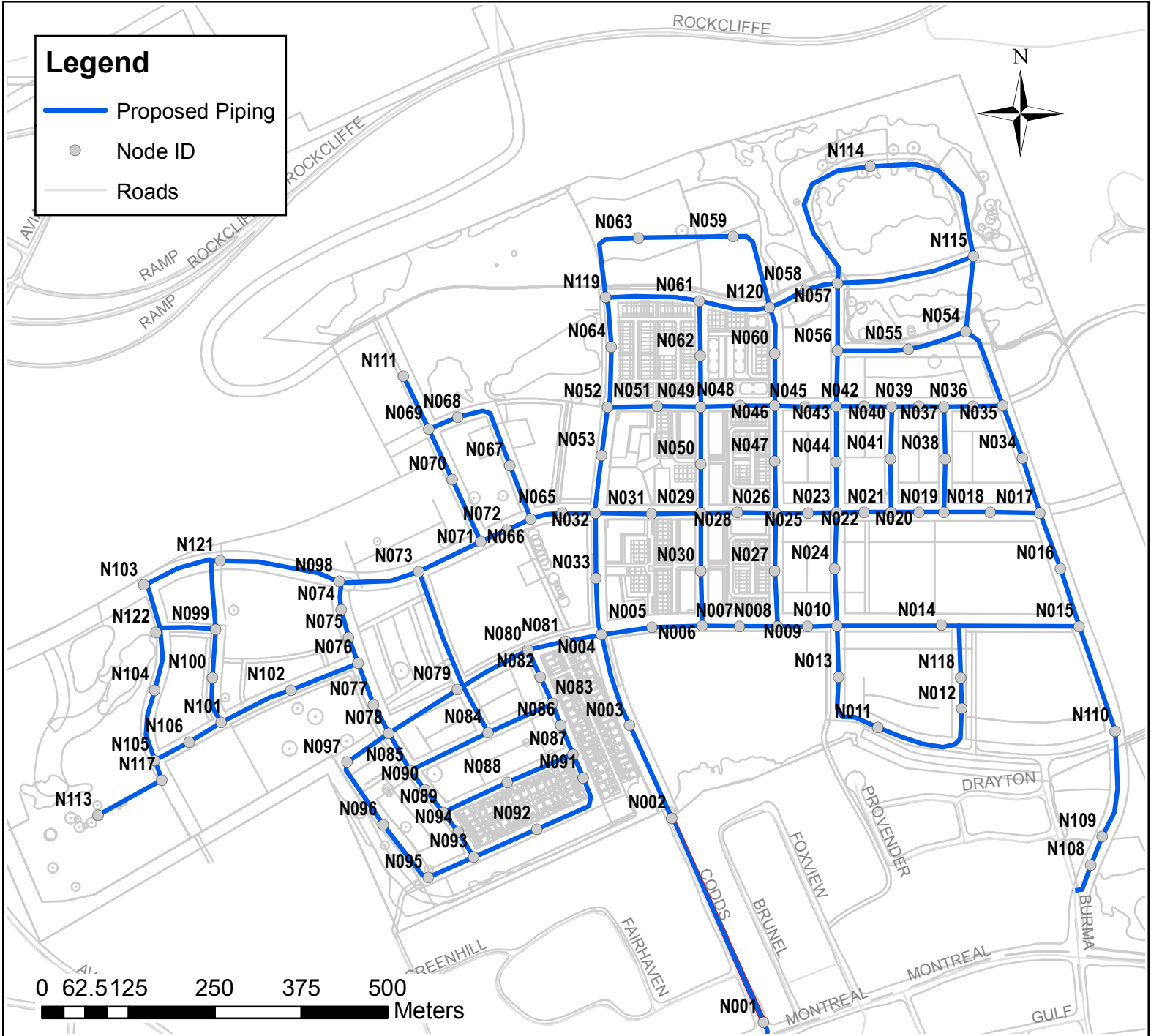
- Clearly stated conclusions and recommendations
- Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
- All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario

# **APPENDIX B**

Water Demand Calculations  
Fire flow Calculations  
MSS Update 2020 Water Model Results

# Legend

- Proposed Piping
- Node ID
- Roads



**BSDY & MXDY Analysis with MRPS at Max. Discharge HGL = 147m**

ID	BSDY		MXDY	
	Max Pressure (psi)	Min Pressure (psi)	Max Pressure (psi)	Min Pressure (psi)
N001	62.99	55.65	62.98	56.39
N002	72.31	64.97	72.31	65.71
N003	82.44	75.08	82.43	75.82
N004	84.74	77.37	84.74	78.11
N005	84.90	77.53	84.90	78.27
N006	83.19	75.82	83.19	76.56
N007	83.19	75.82	83.19	76.55
N008	82.60	75.23	82.60	75.96
N009	83.18	75.81	83.18	76.55
N010	81.92	74.55	81.92	75.29
N011	81.21	73.84	81.21	74.58
N012	81.21	73.85	81.21	74.58
N013	81.93	74.56	81.93	75.30
N014	79.65	72.29	79.65	73.03
N015	78.06	70.72	78.06	71.46
N016	76.52	69.17	76.52	69.91
N017	75.83	68.46	75.83	69.20
N018	75.66	68.29	75.66	69.03
N019	76.21	68.83	76.20	69.57
N020	77.39	70.01	77.39	70.75
N021	77.85	70.47	77.85	71.21
N022	78.92	71.55	78.92	72.28
N023	80.58	73.20	80.58	73.94
N024	82.20	74.82	82.20	75.56
N025	82.28	74.90	82.27	75.63
N026	83.14	75.76	83.14	76.50
N027	84.13	76.76	84.13	77.49
N028	83.52	76.14	83.51	76.87
N029	83.61	76.23	83.60	76.96
N030	84.36	76.98	84.36	77.72
N031	84.15	76.77	84.15	77.51
N032	84.17	76.79	84.17	77.53
N033	84.68	77.30	84.68	78.04
N034	75.97	68.60	75.97	69.34
N035	76.76	69.39	76.76	70.13
N036	76.81	69.43	76.81	70.17
N037	76.88	69.50	76.88	70.24
N038	76.47	69.09	76.47	69.83
N039	77.31	69.93	77.31	70.67
N040	77.85	70.47	77.85	71.21
N041	78.20	70.82	78.20	71.56
N042	78.09	70.71	78.09	71.45
N043	78.22	70.85	78.22	71.58
N044	80.52	73.14	80.52	73.88
N045	79.72	72.34	79.72	73.08
<b>N046</b>	82.48	75.10	82.48	75.84
<b>N047</b>	81.99	74.61	81.99	75.35
<b>N048</b>	81.87	74.49	81.87	75.23
<b>N049</b>	82.03	74.65	82.03	75.38

ID	Static Demand (Lpm)	Static Pressure (psi)	Static Head (m)	Fire-Flow Demand (Lpm)	Residual Pressure (psi)	Available Flow at Hydrant (Lpm)	Available Flow Pressure (psi)
N043	0.20	78.2	147.0	13,000	56.9	24,887	20
N044	0.20	80.5	147.0	13,000	58.2	24,323	20
N045	0.20	79.7	147.0	13,000	58.3	25,466	20
N046	0.20	82.5	147.0	13,000	61.1	26,690	20
N047	0.20	82.0	147.0	13,000	53.4	19,004	20
N048	0.20	81.9	147.0	13,000	60.4	26,290	20
N049	0.20	82.0	147.0	13,000	60.5	26,368	20
N050	0.20	81.4	147.0	13,000	52.9	18,940	20
N051	0.20	82.1	147.0	13,000	60.6	26,315	20
N052	0.17	80.6	147.0	13,000	59.2	25,799	20
N053	0.17	82.8	147.0	13,000	61.4	26,838	20
N054	0.20	80.0	147.0	13,000	56.8	22,758	20
N055	0.20	80.1	147.0	13,000	49.1	18,127	20
N056	0.20	81.6	147.0	13,000	59.1	24,553	20
N057	0.20	86.8	147.0	13,000	64.2	26,500	20
N058	0.20	85.1	147.0	13,000	62.0	25,281	20
N059	0.20	79.6	147.0	13,000	39.9	15,951	20
N060	0.20	82.8	147.0	13,000	54.5	19,377	20
N061	0.20	82.3	147.0	13,000	58.8	23,498	20
N062	0.20	80.8	147.0	13,000	51.9	18,751	20
N063	0.20	83.7	147.0	13,000	43.5	16,420	20
N064	0.17	83.8	147.0	13,000	60.6	24,487	20
N065	0.17	84.1	147.0	13,000	61.4	25,452	20
N066	0.17	84.2	147.0	13,000	60.7	24,282	20
N067	0.17	82.5	147.0	13,000	44.8	16,782	20
N068	0.17	86.3	147.0	13,000	42.0	15,966	20
N069	0.17	87.4	147.0	13,000	43.6	16,196	20
N070	0.17	83.1	147.0	13,000	43.8	16,519	20
N071	0.17	85.8	147.0	13,000	61.5	24,009	20
N072	0.17	83.9	147.0	13,000	59.8	23,443	20
N073	0.17	88.3	147.0	13,000	63.7	24,562	20
N074	0.17	90.1	147.0	13,000	62.9	22,377	20
N075	0.17	90.4	147.0	13,000	47.0	16,638	20
N076	0.17	89.9	147.0	13,000	62.6	22,252	20
N077	0.17	88.0	147.0	13,000	61.8	22,691	20
N078	0.05	88.1	147.0	13,000	63.0	24,005	20
N079	0.05	87.3	147.0	13,000	63.0	24,596	20
N080	0.05	86.1	147.0	13,000	62.4	24,832	20
N081	0.05	85.5	147.0	13,000	62.8	25,988	20
N082	0.05	85.6	147.0	13,000	61.0	23,529	20
N083	0.05	85.6	147.0	13,000	60.8	23,337	20
N084	0.05	86.9	147.0	13,000	62.1	23,782	20
N085	0.05	87.7	147.0	13,000	62.6	23,773	20



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**WATERMAIN DEMAND CALCULATION SHEET**

1050 Tawadina Road | WestUrban Developments Ltd.  
 142609-6.0 | Rev #1 | 2023-06-15  
 Prepared By: AB | Checked By: AC

NODE	RESIDENTIAL				NON-RESIDENTIAL (ICI)			AVERAGE DAILY DEMAND (l/s)			MAXIMUM DAILY DEMAND (l/s)			MAXIMUM HOURLY DEMAND (l/s)			FIRE DEMAND (l/min)
	SINGLE FAMILY UNITS	TOWNHOUSE	APARTMENT	POPULATION	INDUST. (ha)	COMM. (ha)	INSTIT. (ha)	RESIDENTIAL	ICI	TOTAL	RESIDENTIAL	ICI	TOTAL	RESIDENTIAL	ICI	TOTAL	
<b>BUILDING A</b>			144	259.20				0.84		0.84	2.10		2.10	4.62		4.62	15,000
<b>BUILDING B</b>			110	198.00				0.64		0.64	1.60		1.60	3.53		3.53	15,000
<b>TOTAL</b>			<b>254</b>	<b>457.20</b>						<b>1.48</b>			<b>3.70</b>			<b>8.15</b>	

ASSUMPTIONS						
POPULATION DENSITY	WATER DEMAND RATES		PEAKING FACTORS FOR POP. OF 501 TO 3000		FIRE DEMANDS	
Single Family	3.4 persons/unit	Residential	280 l/cap/day	Maximum Daily	Residential 2.5 x avg. day	Single Family 10,000 l/min (166.7 l/s)
Townhouse	2.7 persons/unit	Commercial Shopping Center	2,500 L/(1000m2)/day	Maximum Hourly	Commercial 1.5 x avg. day	Semi Detached & Townhouse 10,000 l/min (166.7 l/s)
Apartment	1.8 persons/unit				Residential 2.2 x max. day	Medium Density 15,000 l/min (250 l/s)
					Commercial 1.8 x max. day	



STEP	Contents	Description	Adjustment Factor	Result
1	Floor Area	Building A		m2
	Total Storey			9 storey
	<b>Total Effective Floor Area</b>			<b>8853 m2</b>
2	<b>Type of Construction</b>	Type V Wood Frame	1.5	Type II Noncombustible Construction 0.8
		Type III Ordinary Construction	1.0	
		Type II Noncombustible Construction	0.8	
		Type I Fire Resistive Construction	0.6	
3	<b>Required Fire Flow</b>	RFF = 220C $\sqrt$ A		<b>17000 L/min</b>
4	<b>Occupancy and Contents</b>	Noncombustible Contents	-25%	Noncombustible Contents. -25%
		Limited Combustible Contents	-15%	
		Combustible Contents	0%	
		Free Burning Contents	15%	
		Rapid Burning Contents	25%	
	<b>Fire Flow</b>			<b>12750 L/min</b>
5	<b>Automatic Sprinkler Protection</b>	Automatic Sprinkler Conforming to NFPA 13	-30%	Yes -30%
		Standard Water Supply for both the system and Fire Department Hose Lines	-10%	Yes -10%
		Fully Supervised System	-10%	No
		<b>Fire Flow</b>		
6	<b>Exposure Adjustment</b>	Based on <b>Table 6</b> Exposure Adjustment Charges for Subject Building		
	<b>North</b>	Separation (m)	6.2	With unprotected opening 20%
		Length X Height Factor (m.storeys)		
		Construction Type	Type II	
	<b>South</b>	Separation (m)	>30	With unprotected opening 0%
		Length X Height Factor (m.storeys)	0	
Construction Type		Type II		
<b>East</b>	Separation (m)	>30	With unprotected opening 0%	
	Length X Height Factor (m.storeys)	0		
	Construction Type	Type II		
<b>West</b>	Separation (m)	>30	With unprotected opening 0%	
	Length X Height Factor (m.storeys)	0		
	Construction Type	Type II		
	<b>Fire Flow</b>			<b>9180 L/min</b>
7	<b>Total Required Fire Flow</b>	Rounded to Nearest 1000 L/min		<b>9000 L/min</b>

Notes 1. Fire flow calculation are based on Fire Underwriters Survey version 2020.

# **APPENDIX C**

Sanitary Sewer Design Sheet  
Wateridge Phase 2B Sanitary Design Sheet  
Wateridge Phase 2B Sanitary Drainage Area Plan

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN					
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES			AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)			ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY		
					SF	TYP. APT	1 Bed APT		2 Bed APT	IND			CUM	IND	CUM			IND	CUM										IND	CUM	L/s
Tawadina Road		BLDG A/B	CTRL MH1A	0.72		254			457.2	457.2	3.40	5.03	0.00	0.0	0.00	0.0	0.00	0.00	0.00	0.00	0.00	0.00	5.27	22.47	4.23	150	2.00	1.232	17.20	76.56%	
		CTRL MH1A	TEE						0.0	457.2	3.40	5.03	0.00	0.0	0.00	0.0	0.00	0.00	0.00	0.00	0.00	5.27	15.89	4.23	150	1.00	0.871	10.62	66.84%		
<b>Design Parameters:</b>				<b>Notes:</b>										<b>Designed:</b>						<b>No.</b>			<b>Revision</b>		<b>Date</b>						
Residential				1. Mannings coefficient (n) = 0.013										AC						1			Servicing Brief - Submission No. 1		2023-07-04						
SF 3.4 p/p/u				2. Demand (per capita): 280 L/day 200 L/day										Checked: JIM																	
APT 1.8 p/p/u				3. Infiltration allowance: 0.33 L/s/Ha										Dwg. Reference: 142609																	
1 Bed 1.4 p/p/u				4. Residential Peaking Factor: Harmon Formula = $1 + \frac{14}{4 + (P/1000)^{0.5}}$ 0.8										File Reference: 142609-6.04.04																	
2 Bed 2.1 p/p/u				where K = 0.8 Correction Factor										Date: 2023-07-04																	
Other 60 p/p/Ha				5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0										Sheet No: 1 of 1																	



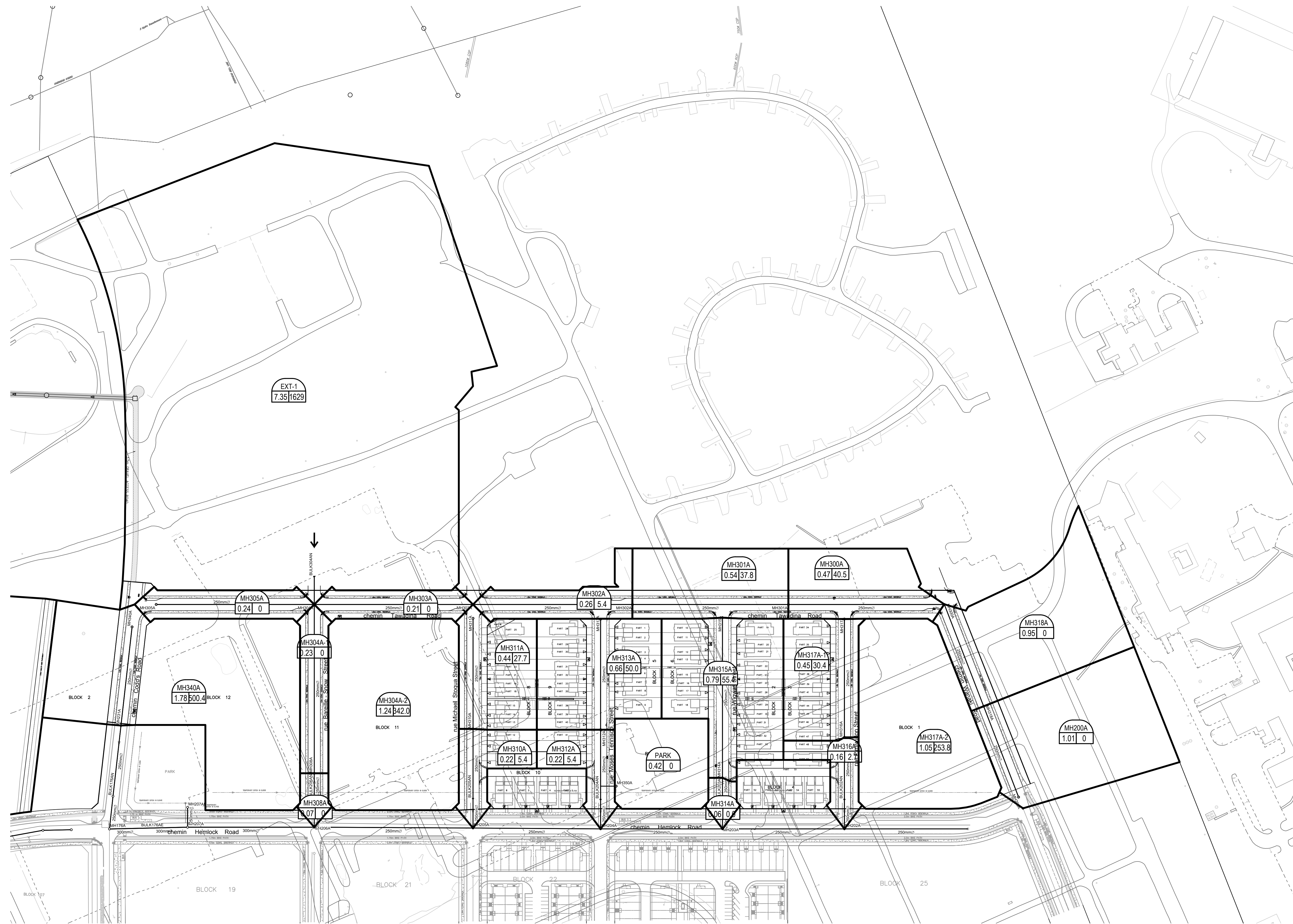
**IBI GROUP**  
 400-333 Preston Street  
 Ottawa, Ontario K1S 5N4 Canada  
 tel 613 225 1311 fax 613 225 9868  
 ibigroup.com

LEGEND  
 MH231A Existing infrastructure (shown for information only)  
 Block 11 Proposed Conditions (DesignWorks Engineering)

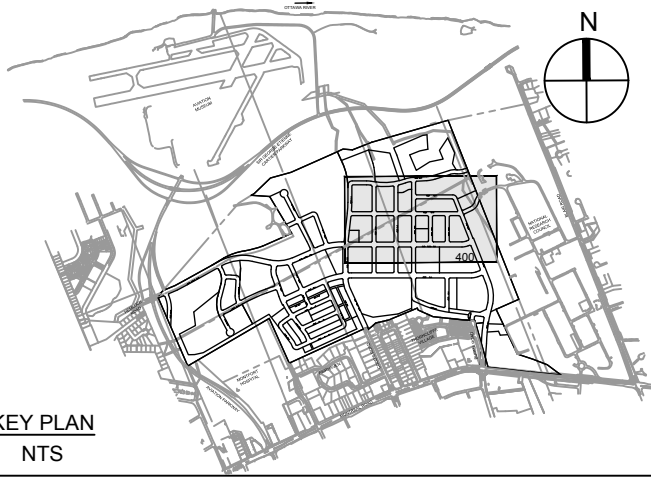
**SANITARY SEWER DESIGN SHEET**

Wateridge at Rockcliffe - Phase 2B  
 City of Ottawa  
 Canada Lands Company

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN								
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW (L/s)	IND	CUM	TOTAL FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD / TH/F	TH/S	APT		IND	CUM			IND	CUM	IND	CUM	IND	CUM			IND	CUM										L/s	(%)
Tawadina Road	MH300A	MH300A	MH301A	0.47		15				40.5	40.5	3.67	0.48	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.47	2.43	0.80	0.00	0.00	2.24	31.02	109.85	250	0.25	0.612	28.78	92.79%
Tawadina Road	MH301A	MH301A	MH302A	0.54		14				37.8	78.3	3.62	0.92	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.54	2.97	0.98	0.00	0.00	2.85	59.18	110.39	250	0.91	1.168	56.33	95.18%
Tawadina Road	MH302A	MH302A	MH303A	0.26		2				5.4	83.7	3.61	0.98	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.26	3.23	1.07	0.00	0.00	3.00	72.61	111.69	250	1.37	1.433	69.62	95.87%
Tawadina Road	MH303A	MH303A	MH304A	0.93				240		432.0	515.7	3.37	5.64	0.00	0.00	0.00	1.96	0.00	0.00	1.50	0.95	0.93	4.16	1.37	0.00	0.00	7.96	31.02	112.10	250	0.25	0.612	23.06	74.33%
Tawadina Road	MH305A	MH305A	MH304A	0.24						0.0	0.0	3.80	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.24	0.24	0.08	0.00	0.00	0.08	49.63	111.61	250	0.64	0.979	49.55	99.84%	
Bareille-Snow Street	EXT-1	BULK304AN	MH304A	7.35				905		1629.0	1629.0	3.12	16.49	0.00	0.00	0.00	0.00	0.00	1.00	0.00	7.35	7.35	2.43	0.00	0.00	18.91	31.02	20.00	250	0.25	0.612	12.11	39.04%	
Bareille-Snow Street	MH304A-1, MH304A-2	MH304A	MH308A	0.76				52		93.6	2238.3	3.04	22.04	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.76	12.51	4.13	0.00	0.00	26.80	39.72	119.21	250	0.41	0.784	12.93	32.54%
Bareille-Snow Street	MH308A	MH308A	BULK206AN	0.96				352		633.6	2871.9	2.97	27.61	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.96	13.47	4.45	0.00	0.00	32.69	84.15	16.82	250	1.84	1.661	51.46	61.15%
Bareille-Snow Street		BULK206AN	MH206A							0.0	2871.9	2.97	27.61	0.00	0.00	0.00	1.96	0.00	0.00	1.00	0.64	0.00	13.47	4.45	0.00	0.00	32.69	88.83	21.00	250	2.05	1.753	56.13	63.20%
Codd's Road	MH340A	MH340A	BLK231AN	0.88				212		381.6	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	75.98	70.00	250	1.50	1.500	71.46	94.04%	
Codd's Road		MH231A	BULK176AN							0.0	381.6	3.43	4.24	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00	0.88	0.88	0.29	0.00	0.00	4.53	83.92	50.22	250	1.83	1.656	79.40	94.61%
<b>Design Parameters:</b>				<b>Notes:</b>										<b>Designed:</b>						<b>Revision</b>			<b>Date</b>											
Residential				1. Mannings coefficient (n) = 0.013										KH						1			Submission No. 1 for City Review											
SF 3.4 p/p/u				2. Demand (per capita): 280 L/day										JIM						2			Submission No. 2 for City Review											
TH/F/SD 2.7 p/p/u				3. Infiltration allowance: 0.33 L/s/Ha										Checked:						3			MECP Submission											
TH/S 2.3 p/p/u				4. Residential Peaking Factor: Harmon Formula = 1+(14/(4+(P/1000)^0.5))0.8										Dwg. Reference: 118863-400						4			Record information Added (No.1)											
APT 1.8 p/p/u				5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0										File Reference: 118863.5.7.1						5			Record information Added (No.2)											
Other 60 p/p/Ha														Date: 2021-03-31						Date: 2021-03-31			Sheet No: 1 of 1											
INST 28,000 L/Ha/day																																		
COM 28,000 L/Ha/day																																		
IND 35,000 L/Ha/day																																		
MOE Chart 17000 L/Ha/day																																		



SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



KEY PLAN

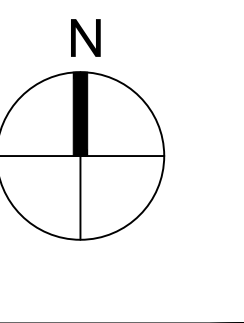
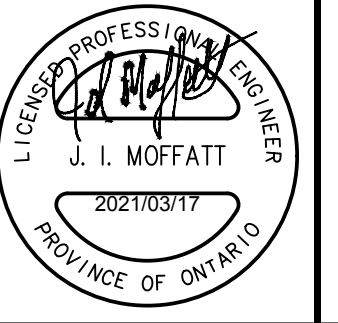
NTS

14			
13			
12			
11			
10			
9			
8			
7	REVISED PER SERVICING CHANGES FOR BLOCKS 8, 9, 10	J.I.M.	2021-03-17
6	ISSUED FOR CONSTRUCTION	J.I.M.	2019-09-10
5	RE-ISSUED FOR RFP	J.I.M.	2019-07-02
4	ISSUED FOR RFP	J.I.M.	2019-05-17
3	MECP SUBMISSION	J.I.M.	2019-04-17
2	SUBMISSION NO. 2 FOR CITY REVIEW	J.I.M.	2019-03-15
1	SUBMISSION NO. 1 FOR CITY REVIEW	J.I.M.	2018-12-20
No.	REVISIONS	By	Date

**CANADA LANDS COMPANY**  
SOCIÉTÉ IMMOBILIÈRE DU CANADA  
100 Queen Street  
Ottawa, On  
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Project Title  
**WATERIDGE VILLAGE AT ROCKCLIFFE**  
PHASE 2B



Drawing Title  
**SANITARY DRAINAGE**  
AREA PLAN

Scale  
1:1500

Design	K.H.	Date	DECEMBER 2018
Drawn	M.M.	Checked	J.I.M.
Project No.	118863	Drawing No.	400

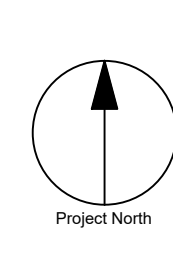
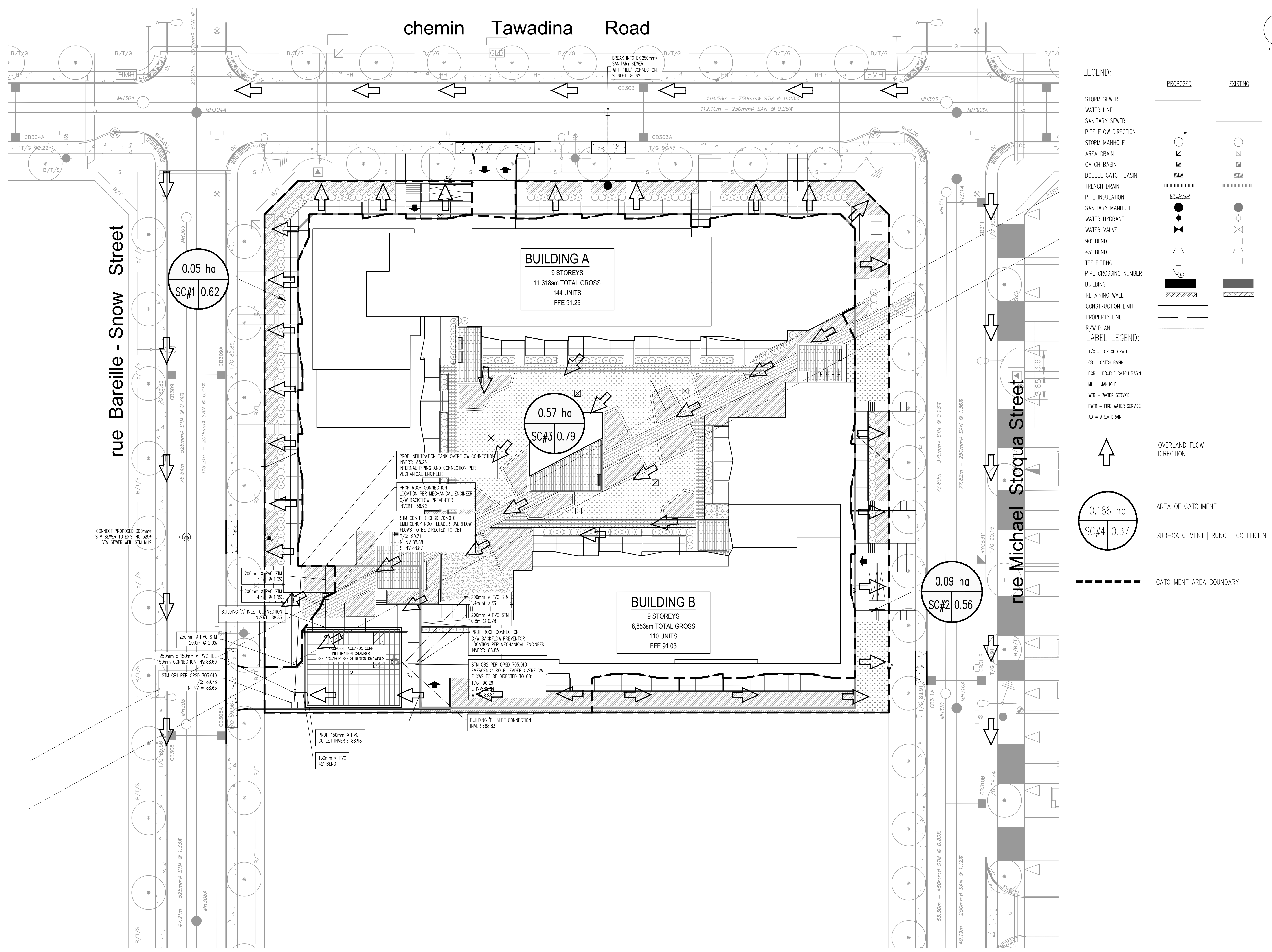
# APPENDIX D

Storm Sewer Design Sheet  
142609-500 - Storm Drainage Area Plan  
Wateridge Phase 2B Storm Design Sheet  
Wateridge Phase 2B Storm Drainage Area Plan  
Modified Rational Method on-site SWM calculations  
Temporary Orifice Sizing  
Sample Runoff Coefficient Calculations  
Minor system release rate (Wateridge Phase 2B)

LOCATION				AREA (Ha)														RATIONAL DESIGN FLOW													SEWER DATA										
STREET	AREA ID	FROM	TO	C=	C=	C=	C=	C=	C=	C=	C=	C=	C=	IND	CUM	INLET	TIME	TOTAL	I(2)	I(5)	I(10)	I(15)	I(30)	5yr PEAK	5yr PEAK	10yr PEAK	100yr PEAK	FIXED FLOW		DESIGN	CAPACITY	LENGTH	PIPE SIZE (mm)			SLOPE	VELOCITY	AVAIL CAP (5yr)			
				0.20	0.25	0.40	0.50	0.57	0.65	0.70	0.79	0.80	2.78AC	2.78AC	(min)	IN PIPE	(min)	(mm/hr)	(mm/hr)	(mm/hr)	(mm/hr)	(mm/hr)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	IND	CUM	FLOW (L/s)	(L/s)	(m)	DIA	W	H	(%)	(m/s)	(L/s)	(%)			
BUILDING A	SC3	BUILDING A	CTRL MH1										0.57	125	125	10.00	0.04	10.04	76.81						96.15				0.00	0.00	96.15	142.67	4.29	300				2.00	1.955	46.52	32.6%
		CTRL MH1	MH1										0.00	125	10.04	0.10	10.14	76.66						95.97				1.00	1.00	95.97	142.67	12.17	300				2.00	1.955	46.70	32.73%	

<b>Definitions:</b> Q = 2.78QA, where: Q = Peak Flow in Litres per Second (L/s) A = Area in Hectares (Ha) i = Rainfall intensity in millimeters per hour (mm/hr) [i = 732.951 / (TC+6.199)^0.810]      2 YEAR [i = 998.071 / (TC+6.053)^0.814]      5 YEAR [i = 1174.184 / (TC+6.014)^0.816]      10 YEAR [i = 1735.688 / (TC+6.014)^0.820]      100 YEAR	<b>Notes:</b> 1. Manning's coefficient (n) = 0.013	<b>Designed:</b> AC	<b>No.</b> 1	<b>Revision</b> Servicing Brief - Submission No. 2	<b>Date</b> 2023-07-04
		<b>Checked:</b> JIM			
		<b>Dwg. Reference:</b> 142609-500			
			<b>File Reference:</b> 142609-6.04.04	<b>Date:</b> 2023-07-04	<b>Sheet No.:</b> 1 of 1

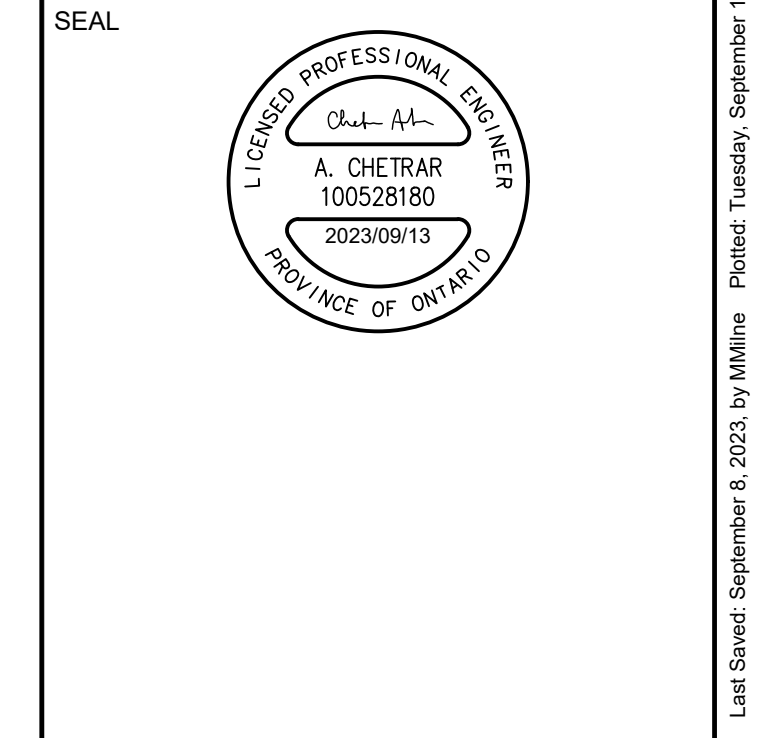
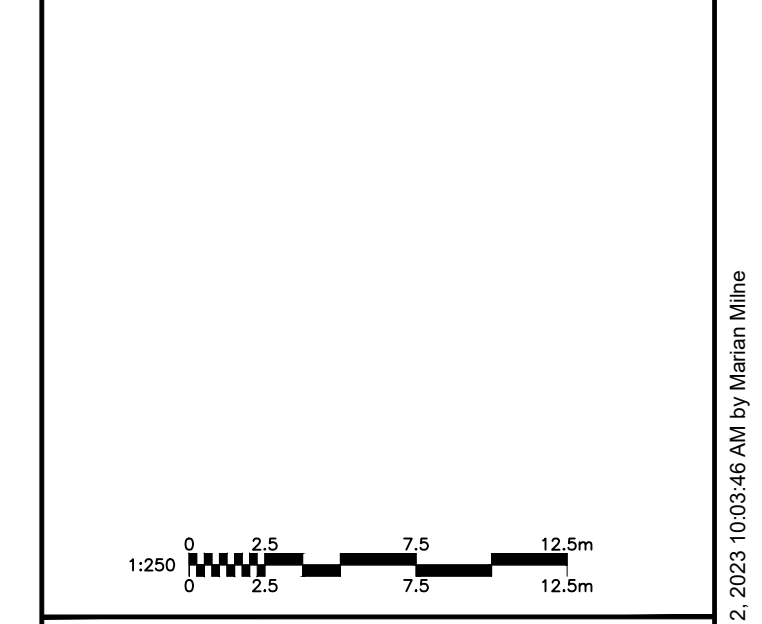
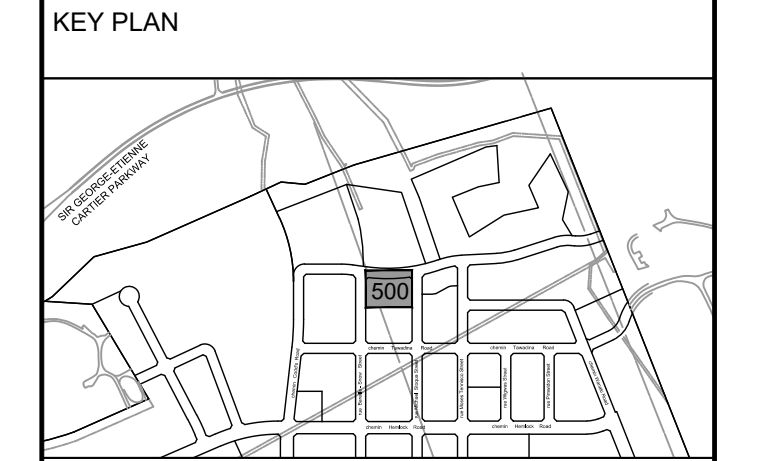


CLIENT  
**WestUrban**  
 Developments Ltd.

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ISSUES

No.	DESCRIPTION	DATE
1	ISSUED FOR CLC SUBMISSION	2023-01-11
2	SUBMISSION NO 2 FOR CITY REVIEW	2023-09-13
3		
4		
5		
6		
7		
8		



LEGEND:

- STORM SEWER
- WATER LINE
- SANITARY SEWER
- PIPE FLOW DIRECTION
- STORM MANHOLE
- AREA DRAIN
- CATCH BASIN
- DOUBLE CATCH BASIN
- TRENCH DRAIN
- PIPE INSULATION
- SANITARY MANHOLE
- WATER HYDRANT
- WATER VALVE
- 90° BEND
- 45° BEND
- TEE FITTING
- PIPE CROSSING NUMBER

OVERLAND FLOW DIRECTION

AREA OF CATCHMENT

SUB-CATCHMENT | RUNOFF COEFFICIENT

CATCHMENT AREA BOUNDARY

**ARCADIS**  
 333 Preston Street - Suite 500  
 Ottawa ON K1S 5N4 Canada  
 Tel 613 225 1311  
 www.arcadis.com

PROJECT  
**1050 Tawadina Road**  
 WATERIDGE VILLAGE PHASE 2

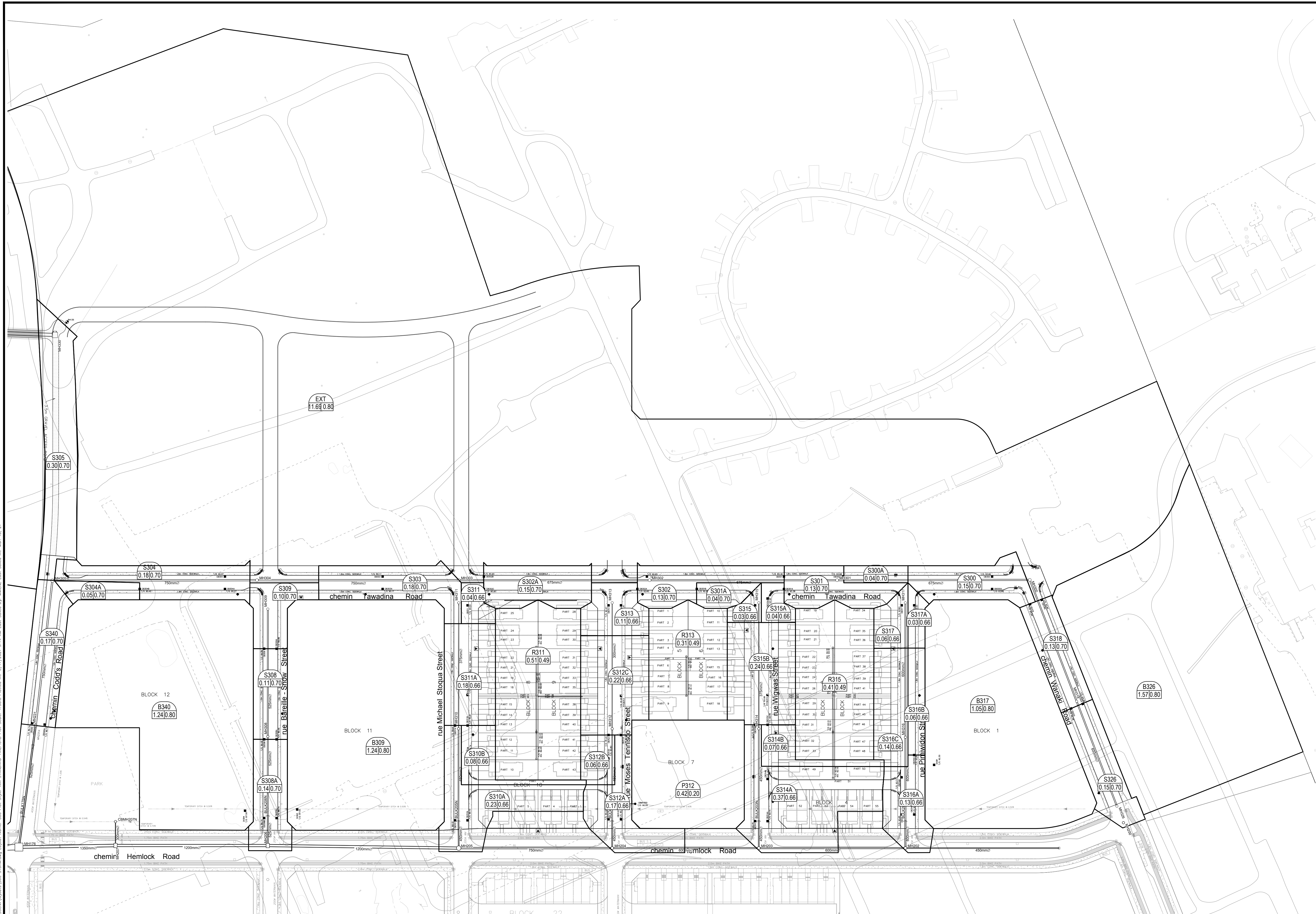
PROJECT NO: 142609  
 DRAWN BY: M.M.  
 PROJECT MGR: S.L.

CHECKED BY: J.I.M.  
 APPROVED BY: A.C.

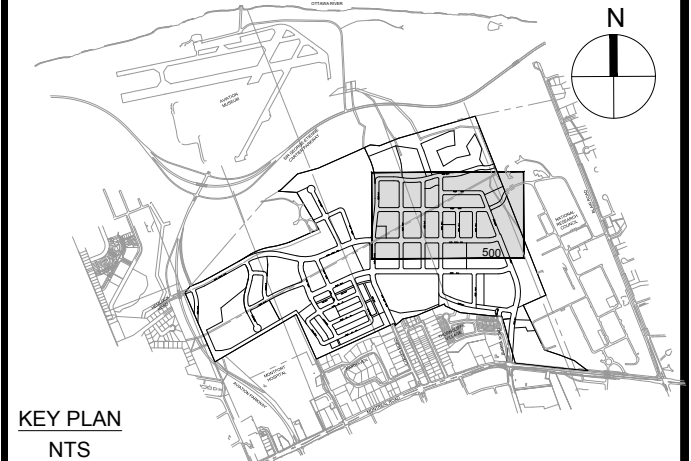
SHEET TITLE  
**STORM DRAINAGE AREA PLAN**

SHEET NUMBER **C-500** ISSUE **2**





SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



KEY PLAN

NTS

14			
13			
12			
11			
10			
9			
8			
7	REVISED PER SERVICING CHANGES FOR BLOCKS 8, 9, 10	J.I.M.	2021-03-17
6	ISSUED FOR CONSTRUCTION	J.I.M.	2019-09-10
5	RE-ISSUED FOR RFP	J.I.M.	2019-07-02
4	ISSUED FOR RFP	J.I.M.	2019-05-17
3	MECP SUBMISSION	J.I.M.	2019-04-17
2	SUBMISSION NO. 2 FOR CITY REVIEW	J.I.M.	2019-03-15
1	SUBMISSION NO. 1 FOR CITY REVIEW	J.I.M.	2018-12-20
No.	REVISIONS	By	Date

**CANADA LANDS COMPANY**  
SOCIÉTÉ IMMOBILIÈRE DU CANADA

100 Queen Street  
Ottawa, On  
K1P 1J9

**IBI GROUP**  
400 - 333 Preston Street  
Ottawa ON K1S 5N4 Canada  
tel 613 225 1311 fax 613 225 9868  
ibigroup.com

Project Title  
**WATERIDGE VILLAGE AT ROCKCLIFFE**  
PHASE 2B

LICENSÉ PROFESSIONNEL EN INGÉNIEUR  
J. I. MOFFATT  
202103317  
PROVINCE OF ONTARIO

Drawing Title  
**STORM DRAINAGE**  
AREA PLAN

Scale  
1:1000

Design K.H.	Date DECEMBER 2018
Drawn M.M.	Checked J.I.M.
Project No. 118863	Drawing No. 500

J:\118863\_Wtridge2A2B\5.9 Drawings\59civil\layouts\500 STORM.dwg, 2021-03-17 9:14:53 AM, AutoCAD PDF (High Quality Print).pc3

D07-16-15-0003\_P2B



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tel 613 225 1311 fax 613 225 9868  
ibigroup.com

LEGEND

Black text 5 year event curve design  
Blue text 100 year event curve design  
MH206 Existing infrastructure (shown for information only)  
MH311 Record Information No. 2

STORM SEWER DESIGN SHEET

Wateridge at Rockcliffe - Phase 2B  
City of Ottawa  
Canada Lands Company

LOCATION				AREA (Ha)													RATIONAL DESIGN FLOW												SEWER DATA												
STREET	AREA ID	FROM	TO	C= 0.20	C= 0.30	C= 0.40	C= 0.49	C= 0.57	C= 0.65	C= 0.66	C= 0.70	C= 0.73	C= 0.80	IND 2.78AC	CUM 2.78AC	INLET (min)	TIME IN PIPE	TOTAL (min)	i (2) (mm/hr)	i (5) (mm/hr)	i (10) (mm/hr)	i (100) (mm/hr)	2yr PEAK FLOW (L/s)	5yr PEAK FLOW (L/s)	10yr PEAK FLOW (L/s)	100yr PEAK FLOW (L/s)	FIXED FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	PIPE SIZE (mm)			SLOPE (%)	VELOCITY (m/s)	AVAIL CAP (2yr)					
Pimiwidon Street	S317A, B317	MH317	MH316							0.09				1.05	2.50	2.50	10.00	0.88	10.88	76.81	104.19	122.14	178.56						260.52	439.15	79.15	600			0.47	1.505	178.63	40.68%			
Pimiwidon Street	S316A-B	MH316	BLK202N							0.33					0.61	3.11	10.88	0.76	11.64	73.59	99.78	116.94	170.92						309.90	313.81	49.32	600			0.24	1.075	3.91	1.25%			
Pimiwidon Street		BULK202N	MH202												0.00	3.11	11.64	0.24	11.88	71.03	96.26	112.79	164.83						298.95	320.28	16.00	600			0.25	1.097	21.33	6.66%			
Wigwas Street	S315, S315A-B, R315	MH315	MH314				0.41			0.31					1.13	1.13	10.00	0.98	10.98	76.81	104.19	122.14	178.56						117.46	142.86	73.50	375			0.61	1.253	25.40	17.78%			
Wigwas Street	S314A-B	MH314	BLK203N							0.44					0.81	1.93	10.98	0.50	11.48	73.24	99.30	116.38	170.09						192.10	294.44	54.27	450			0.98	1.793	102.34	34.76%			
Wigwas Street		BULK203N	MH203												0.00	1.93	11.48	0.18	11.66	71.55	96.97	113.63	166.06						187.59	247.07	16.00	450			0.69	1.505	59.47	24.07%			
Moses Tennisco St	S313, R313	MH313	MH312				0.31			0.11					0.62	0.62	10.00	0.81	10.81	76.81	104.19	122.14	178.56						65.03	111.88	74.68	300			1.23	1.533	46.86	41.88%			
Moses Tennisco St	S312A-C	MH312	BULK204N							0.45					0.83	1.45	10.81	0.37	11.18	73.82	100.09	117.31	171.46						145.11	400.16	54.25	450			1.81	2.437	255.05	63.74%			
Park Block 7	P312	CBMH350	pipe	0.42											0.23	0.23	10.00	0.13	10.13	76.81	104.19	122.14	178.56						24.33	87.74	13.50	250			2.00	1.731	63.40	72.27%			
Moses Tennisco St		BULK204N	MH204												0.00	1.68	11.18	0.11	11.29	72.54	98.34	115.24	168.43						165.53	400.16	16.00	450			1.81	2.437	234.63	58.63%			
Michael Stoqua St	S311, S311A, R311	MH311	MH310				0.45			0.22					1.02	1.02	10.00	0.77	10.77	76.81	104.19	122.14	178.56						105.93	181.07	73.30	375			0.98	1.588	75.15	41.50%			
Michael Stoqua St	S310A-B	MH310	BLK205N							0.37					0.68	1.70	10.77	0.56	11.33	73.97	100.30	117.55	171.82						170.06	270.97	55.30	450			0.83	1.651	100.92	37.24%			
Michael Stoqua St		BLK205N	MH205												0.00	1.70	11.33	0.16	11.48	72.06	97.67	114.46	167.27						165.60	279.02	16.00	450			0.88	1.700	113.42	40.65%			
Bareille-Snow St	S309, B309	MH309	MH308							0.10			1.24		2.95	2.95	10.00	0.72	10.72	76.81	104.19	122.14	178.56						307.62	385.95	74.54	525			0.74	1.727	78.33	20.30%			
Bareille-Snow St	S308, S308A	MH308	BULK206N							0.25					0.49	3.44	10.72	0.34	11.06	74.15	100.54	117.84	172.24						345.75	517.42	47.21	525			1.33	2.315	171.67	33.18%			
Bareille-Snow St		BULK206N	MH206												0.00	3.44	11.06	0.12	11.18	72.96	98.91	115.92	169.42						340.15	536.52	17.50	525			1.43	2.401	196.37	36.60%			
Wanaki Road	B200, S200A	MH326	MH318							0.15			1.57		3.78	3.78	10.00	0.69	10.69	76.81	104.19	122.14	178.56						394.22	457.45	65.27	600			0.51	1.567	63.23	13.82%			
Wanaki Road	S318	MH318	MH300							0.13					0.25	4.04	10.69	0.83	11.52	74.24	100.66	117.99	172.45						406.34	443.79	75.72	600			0.48	1.521	37.46	8.44%			
Tawadina Road	S300, S300A	MH300	MH301							0.19					0.37	4.41	11.52	1.59	13.12	71.41	96.78	113.41	165.73						426.43	438.47	113.36	675			0.25	1.187	12.03	2.74%			
Tawadina Road	S301, S301A	MH301	MH302							0.17					0.33	4.74	13.12	0.88	14.00	66.60	90.18	105.65	154.34						427.21	769.51	110.30	675			0.77	2.083	342.29	44.48%			
Tawadina Road	S302, S302A	MH302	MH303							0.28					0.54	5.28	14.00	0.69	14.69	64.24	86.94	101.83	148.73						459.22	996.00	111.13	675			1.29	2.696	536.79	53.89%			
Tawadina Road	S303	MH303	MH304							0.18					0.35	5.63	14.69	1.62	16.30	62.52	84.59	99.06	144.67						476.43	556.99	118.58	750			0.23	1.221	80.57	14.46%			
Tawadina Road	S304, S304A	MH304	MH305							0.23					0.45	6.08	16.30	1.51	17.82	58.85	79.57	93.16	136.02						483.78	603.49	120.08	750			0.27	1.323	119.71	19.84%			
Codd's Road	S340, B340A	MH305	MH231							0.17			2.02		4.82	10.90	17.82	0.50	18.32	55.83	75.44	88.31	128.90						822.55	1,308.85	86.40	750			1.27	2.870	486.29	37.15%			
Codd's Road	S231	MH231	MH176							0.12					0.23	11.14	18.32	0.43	18.75	54.91	74.17	86.82	126.71						826.06	1,240.05	70.90	750			1.14	2.719	413.99	33.38%			
Block 1	-	DICB1	Pipe	1.05											0.58	0.58	61.68	0.20	61.88	24.06	32.28	37.67	54.75																		
Block 11	-	DICB3	Pipe	1.24											0.69	0.69	81.62	0.19	81.81	19.53	26.16	30.52	44.31																		
Block 12	-	DICB4	Pipe	1.24											0.69	0.69	80.96	0.23	81.19	19.65	26.32	30.70	44.58																		
Block 8	-	DICB5	Pipe	0.66											0.37	0.37	28.47	0.15	28.62	41.47	55.87	65.32	95.20																		

<b>Definitions:</b> Q = 2.78CiA, where: Q = Peak Flow in Litres per Second (L/s) A = Area in Hectares (Ha) i = Rainfall intensity in millimeters per hour (mm/hr) [i = 732.951 / (TC+6.199)^0.810] 2 YEAR [i = 998.071 / (TC+6.053)^0.814] 5 YEAR [i = 1174.184 / (TC+6.014)^0.816] 10 YEAR [i = 1735.688 / (TC+6.014)^0.820] 100 YEAR	<b>Notes:</b> 1. Mannings coefficient (n) = 0.013	<b>Designed:</b> KH	<b>No.</b> 1	<b>Revision</b> Submission No. 1 for City Review	<b>Date</b> 2018-12-20
		<b>Checked:</b> JIM	2	Submission No. 2 for City Review	2019-03-15
			3	MECP Submission	2019-04-17
			4	Record Information Added (No.1)	2020-10-08
			5	Record Information Added (No.2)	2021-03-31
<b>Dwg. Reference:</b> 118863-500			<b>File Reference:</b> 118863.5.7.1		<b>Date:</b> 2021-03-31
					<b>Sheet No:</b> 1 of 1

**Formulas and Descriptions**

$i_{2yr} = 1.2 \text{ year Intensity} = 732.951 / (T_c + 6.199)^{0.810}$   
 $i_{5yr} = 1.5 \text{ year Intensity} = 998.071 / (T_c + 6.053)^{0.814}$   
 $i_{100yr} = 1.100 \text{ year Intensity} = 1735.688 / (T_c + 6.014)^{0.820}$   
 $T_c = \text{Time of Concentration (min)}$   
 $C = \text{Average Runoff Coefficient}$   
 $A = \text{Area (Ha)}$   
 $Q = \text{Flow} = 2.78CI A \text{ (L/s)}$

**Maximum Allowable Release Rate**

**Restricted Flowrate** ( $Q_{restricted} = 2.78 \cdot C \cdot i_{5yr} \cdot A_{site}$  based on  $C=0.50, T_c=20min$ )

Based on Wateridge Village report

$Q_{restricted} =$	195.00 L/s
--------------------	------------

**Uncontrolled Release** ( $Q_{uncontrolled} = 2.78 \cdot C \cdot i_{100yr} \cdot A_{uncontrolled}$ )

$C = 0.725$  (0.58 x 1.25 for 100 year)  
 $T_c = 10 \text{ min}$   
 $i_{100yr} = 178.56 \text{ mm/hr}$   
 $A_{uncontrolled} = 0.14 \text{ Ha}$

$Q_{uncontrolled} =$	50.38 L/s
----------------------	-----------

**Maximum Allowable Release Rate** ( $Q_{max\ allowable} = Q_{restricted} - Q_{uncontrolled}$ )

$Q_{max\ allowable} =$	144.62 L/s
------------------------	------------

**MODIFIED RATIONAL METHOD (100-Year, 5-Year & 2-Year Ponding)**

Drainage Area		SC3							
Area (Ha)	0.570	Restricted Flow ICD Actual (L/s)=	144.00						
C =	0.99	Restricted Flow $Q_r$ for swm.calc (L/s)=	72.00						
50% reduction for sub-surface storage									
100-Year Ponding					100-Year +20% Ponding				
$T_c$ Variable (min)	$i_{100yr}$ (mm/hour)	Peak Flow $Q_p = 2.78 \cdot C \cdot i_{100yr} \cdot A$ (L/s)	$Q_r$ (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m <sup>3</sup> )	100YR $Q_p$ 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m <sup>3</sup> )	
9	188.25	294.58	72.00	222.58	120.19				
14	148.72	232.72	72.00	160.72	135.01				
19	123.87	193.83	72.00	121.83	<b>138.88</b>	232.59	160.59	<b>183.08</b>	
24	106.68	166.93	72.00	94.93	136.69				
29	94.01	147.11	72.00	75.11	130.70				

Storage (m <sup>3</sup> )					100+20				
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Surface	Sub-surface	Balance
0.00	138.88	0.00	140	0.00	0.00	183.08	43.08		
					convert to flow with peak $T_c$ (L/s)				
					37.79				

overflows to: off site

SWM Statistics of Modified Site Areas		
Controlled	Area	ICD Flow
SC3	0.570	144.000
<b>Sum</b>	<b>0.57</b>	<b>144.00</b>
Uncontrolled	Area	Flow
SC1	0.050	0.98
SC2	0.090	1.76
<b>Sum</b>	<b>0.14</b>	<b>50.38</b>
Total Sum	<b>0.710</b>	<b>194.384</b>
Allowable		<b>195.00</b>
		TRUE

Drainage Area		SC3			
Area (Ha)	0.570	Restricted Flow $Q_r$ (L/s)=	72.00		
C =	0.72				
2-Year Ponding					
$T_c$ Variable (min)	$i_{2yr}$ (mm/hour)	Peak Flow $Q_p = 2.78 \cdot C \cdot i_{2yr} \cdot A$ (L/s)	$Q_r$ (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m <sup>3</sup> )
3	121.46	138.58	72.00	66.58	11.98
4	111.72	127.47	72.00	55.47	13.31
5	103.57	118.17	72.00	46.17	<b>13.85</b>
6	96.64	110.26	72.00	38.26	13.77
7	90.66	103.44	72.00	31.44	13.20

Storage (m <sup>3</sup> )				
Overflow	Required	Surface	Sub-surface	Balance
0.00	13.85	0.00	140	0.00

overflows to: off site



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**IBI GROUP**

**ORIFICE SIZING**

1050 Tawadina Road | WestUrban Developments  
 142609-6.0 | Rev #1 | 2023-06-30  
 Prepared By: AC | Checked By: JIM

Orifice coefficients	
Cv =	0.60

	Invert	Diameter	Centre ICD	Max. Pond Elevation	Hydraulic Slope	Target Flow	Theoretical		Recommended	
							Orifice	Actual Flow	Orifice	Actual Flow
							(m)	(l/s)	(m)	(l/s)
STM MH1	86.86	300	87.010	89.010	2.000	144.00	0.1958	144.09	0.195	142.92
						144.00				142.92



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**RUN-OFF COEFFICIENTS**

Development Name | Name of Client/Developer  
 142609-6.0 | Rev #1 | 2023-06-29  
 Prepared By: AC | Checked By: JIM

	SC1			SC2			SC3		
	GRASS	ROOF	ASPHALT	GRASS	ROOF	ASPHALT	GRASS	ROOF	ASPHALT
	202.32		307.68	432.72		467.28	1016.76		4763.24
<b>TOTAL (m<sup>2</sup>)</b>	202.32	0.00	307.68	432.72	0.00	467.28	1016.76	0.00	4763.24
	510.00			900.00			5780.00		

Runoff Coefficient (C):	0.2	0.9	0.9	0.2	0.9	0.9	0.2	0.9	0.9
Ave. Runoff Coefficient (C):	0.62			0.56			0.78		

<b>Runoff Coefficient Used(C):</b>	<b>0.62</b>	<b>0.56</b>	<b>0.79</b>
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**Table 2.1 Hydrological Parameters**

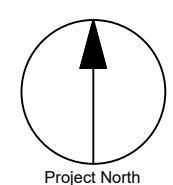
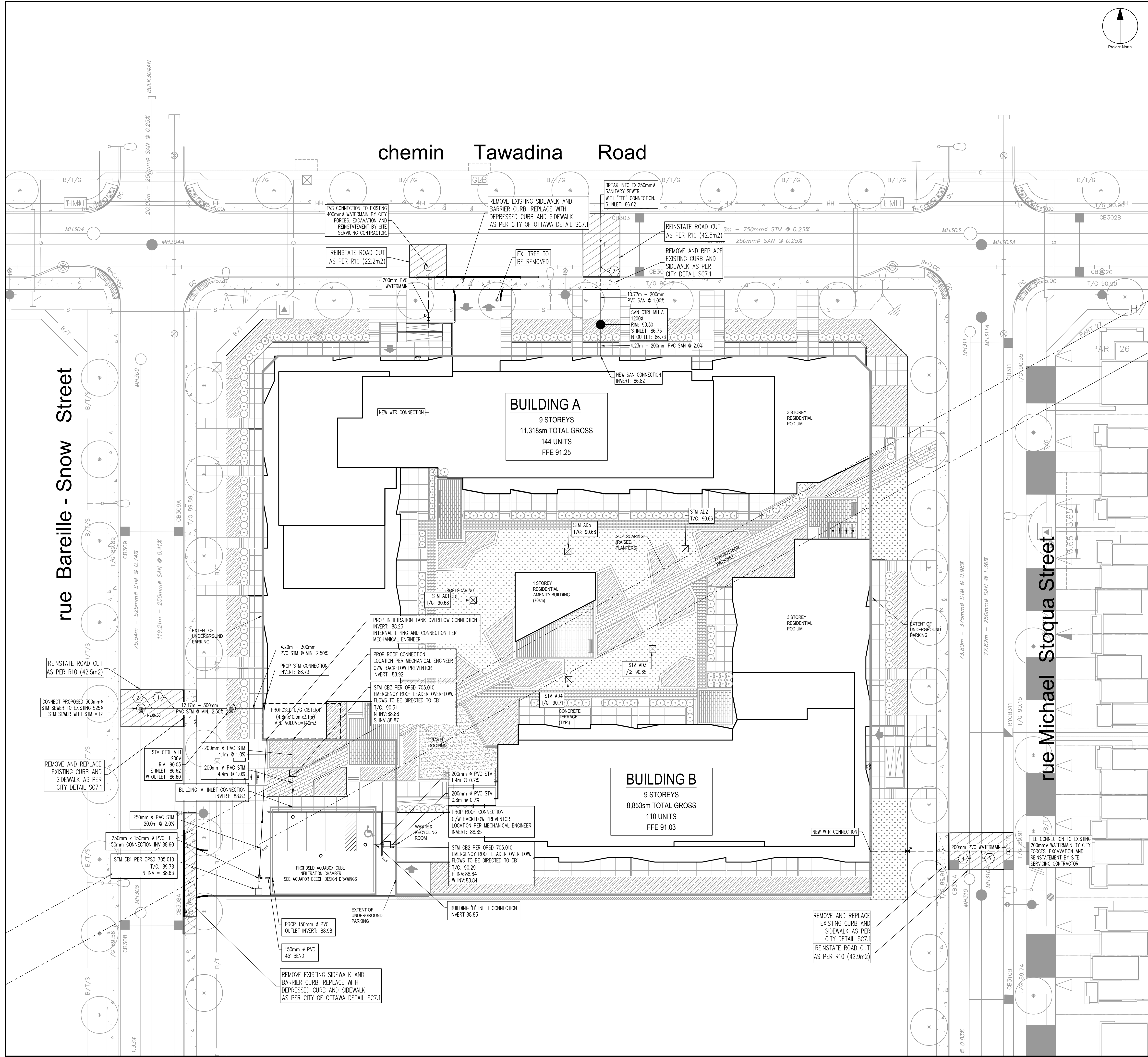
Block	Phase 2B Design Brief							Current Evaluation							
	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)	Parcel	Drainage Area ID	Area (ha)	Major System: D/S Segment ID	Minor System: MH ID	IMP Ratio	Segment Length (m)	Sub-catchment Width (m)
11	B309	1.24	S308A on Bareille-Snow	MH309 on Bareille-Snow	0.86	135.1	270.2	1	B309_1	0.72	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	81	162
								2	B309_2	0.52	S308A on Bareille-Snow	MH310 on Michael Stoqua	0.86	58.5	117
12	B340	1.24	S207 on Hemlock	MH305 on Codd's Road	0.86	173.1	346.3	3	B340_3	0.34	S308A on Bareille-Snow	MH308 on Bareille-Snow	0.86	38.25	76.5
								4	B340_4	0.53	S308 on Bareille-Snow	MH309 on Bareille-Snow	0.86	59.63	119.25
								5	B340_5	0.37	S340 on Codd's	MH305 on Codd's Road	0.86	41.63	83.25

**Table 2.2 Minor System Restriction and On-site Storage**

Block	Phase 2B Design Brief				Current Evaluation					
	Drainage Area ID	Minor System Capture		Required On-Site Storage (cu-m)	Parcel	Drainage Area ID	Minor System Capture		Major System	
		Simulated Flow (l/s)	Corresponding Design Storm				Simulated Flow (l/s)	Corresponding Design Storm	Required On-Site Storage (cu-m)	Comment
11	B309	370	Between 5 and 100	None	1	B309_1	195	Between 5 and 100 year	43	Control up to the 100 year event
					2	B309_2	105	5 year	64	Control up to the 100 year event
12	B340	366	Between 5 and 100	None	3	B340_3	95	Between 5 and 100 year	18	Control up to the 100 year event
					4	B340_4	150	Between 5 and 100 year	21	Control up to the 100 year event
					5	B340_5	139	100 year	None	N/A

# **APPENDIX E**

142609-900 – Site Erosion and Sediment Control Plan  
142609-200 – Site Grading Plan



**LEGEND:**

	PROPOSED	EXISTING
STORM SEWER		
WATER LINE		
SANITARY SEWER		
PIPE FLOW DIRECTION		
STORM MANHOLE		
AREA DRAIN		
CATCH BASIN		
DOUBLE CATCH BASIN		
TRENCH DRAIN		
PIPE INSULATION		
SANITARY MANHOLE		
OBSERVATION PORT		
WATER HYDRANT		
WATER VALVE		
90° BEND		
45° BEND		
TEE FITTING		
PIPE CROSSING NUMBER		
BUILDING		
RETAINING WALL		
CONSTRUCTION LIMIT		
PROPERTY LINE		
R/W PLAN		

**LABEL LEGEND:**

T/G = TOP OF GRATE  
 CB = CATCH BASIN  
 DCB = DOUBLE CATCH BASIN  
 MH = MANHOLE  
 AD = AREA DRAIN

**UTILITY LEGEND:**

	EX. TRANSFORMER
	EX. HYDRO MANHOLE
	EX. BELL PEDESTAL
	EX. BELL GRADE LEVEL BOX
	EX. ROGERS PEDESTAL
	EX. STREET LIGHT
	EX. STREET LIGHT GROUNDING
	EX. JOINT UTILITY TRENCH
	EX. HYDRO CABLE AND DUCTS
	EX. BELL CABLE
	EX. ROGERS CABLE
	EX. GAS
	EX. STREET LIGHT CABLE
	EX. UTILITY DROP LOCATIONS
	EX. CONCRETE ENCASED DUCT BANK
	EX. TREE LOCATION

- NOTES:**
- IT IS THE RESPONSIBILITY OF THE APPLICANT/CONSULTANT TO ENSURE COMPLIANCE WITH ALL APPLICABLE PROVINCIAL STANDARDS AND TO OBTAIN ALL PROVINCIAL APPROVALS, INCLUDING BUT NOT LIMITED ENVIRONMENTAL COMPLIANCE APPROVALS.
  - THE CONTRACTOR SHALL ENSURE ALL MATERIAL AND CONSTRUCTION IS IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARD DRAWINGS AND SPECIFICATIONS. DRAWINGS AND SPECIFICATIONS FROM OPSS & OPSD, SHALL BE USED WHERE THERE IS NO EQUIVALENT FROM CITY STANDARDS.
  - THE CONTRACTOR SHALL VERIFY ALL SURVEY STAKE LAYOUT POINTS AND DIMENSIONS AND ANY DISCREPANCIES SHALL BE REPORTED TO THE CONSULTANT PRIOR TO CONSTRUCTION. ANY DISCREPANCIES ARE TO BE IMMEDIATELY REPORTED TO THE CONSULTANT.
  - ALL EXISTING UTILITIES SHOWN ON DRAWINGS (PLAN AND PROFILE) ARE FOR REFERENCE PURPOSES ONLY. THE CONTRACTOR SHALL SATISFY THEMSELVES AS TO THE ACTUAL LOCATION AND DEPTH OF ANY UTILITY AND SHALL BE LIABLE FOR ALL OR ANY DAMAGES.
  - THE POSITION OF ALL STATIC STRUCTURES, SMALL UTILITIES, DEEP UTILITIES, AND ANY OTHER ABOVEGROUND OR UNDERGROUND UTILITIES STRUCTURES AND APPURTENANCES SHALL BE FIELD VERIFIED BY THE CONTRACTOR TO SATISFY HIMSELF AND SHALL ASSUME LIABILITY FOR ANY DAMAGES TO THEM DURING CONSTRUCTION. ANY DAMAGES ARE TO BE RECTIFIED TO THE SATISFACTION OF THE CITY INSPECTOR OR FRANCHISE UTILITY OWNERS.
  - REFER TO THE PRELIMINARY GEOTECHNICAL INVESTIGATION REPORT COMPLETED BY ENGLOBE ON OCTOBER 11TH, 2022 FOR GEOTECHNICAL INFORMATION.
  - ALL SEWER AND WATERMAIN PIPES SHALL BE INSTALLED AT A MINIMUM FROST COVER OF 2.4m.
  - ALL WATERMAIN CONNECTIONS WITHIN MUNICIPAL R.O.W.'S TO BE INSTALLED BY CITY FORCES AT THE OWNER'S EXPENSE.

**Pipe Clearance Table**

Crossing No.	Bottom of Pipe	Top of Pipe	Clearance
1	87.50 WTR	87.00 STM	0.50
2	86.62 STM	85.24 SAN	1.38
3	87.40 WTR	86.87 SAN	0.53
4	87.40 WTR	86.87 STM	0.53
5	87.40 WTR	85.77 SAN	0.63

CLIENT

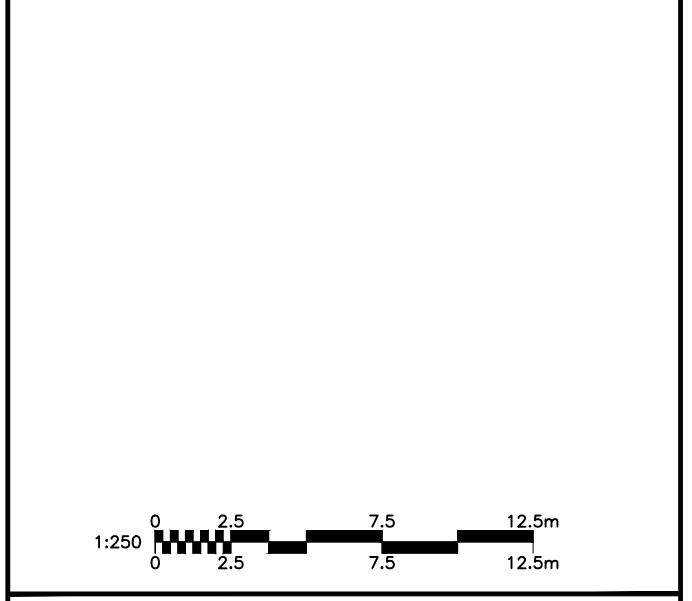
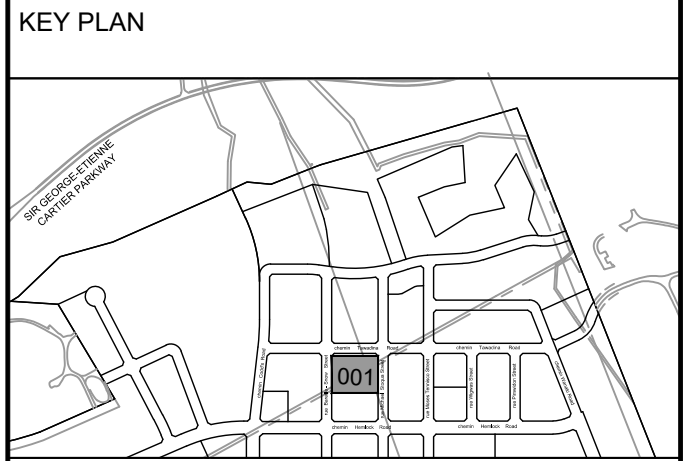
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**ISSUES**

No.	DESCRIPTION	DATE
1	ISSUED FOR CLC SUBMISSION	2023-01-11
2	SUBMISSION NO 2 FOR CITY REVIEW	2023-09-13
3		
4		
5		
6		
7		
8		



**SEAL**

ARCADIS

333 Preston Street - Suite 500  
 Ottawa ON K1S 5N4 Canada  
 Tel 613 225 1311  
 www.arcadis.com

**PROJECT**

1050 Tawadina Road

WATERIDGE VILLAGE PHASE 2

**PROJECT NO:** 142609

**DRAWN BY:** M.M.

**PROJECT MGR:** S.L.

**CHECKED BY:** J.I.M.

**APPROVED BY:** A.C.

**SHEET TITLE**

SITE SERVICING PLAN

**SHEET NUMBER** C-001

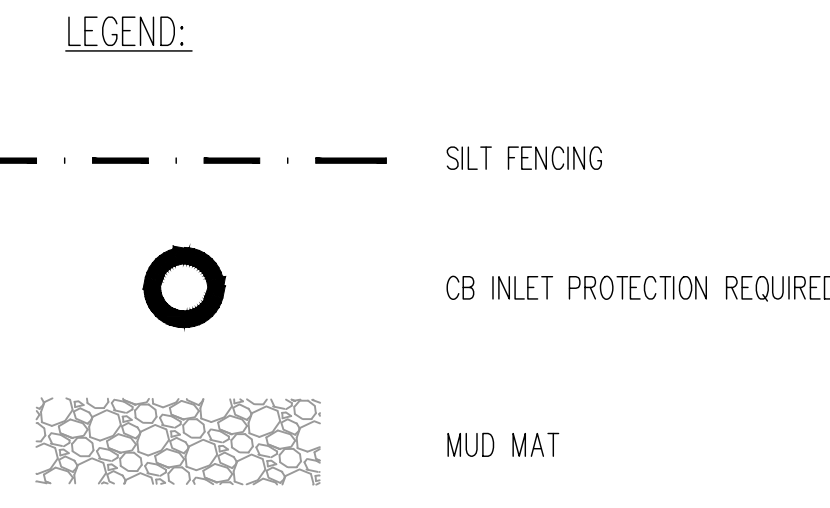
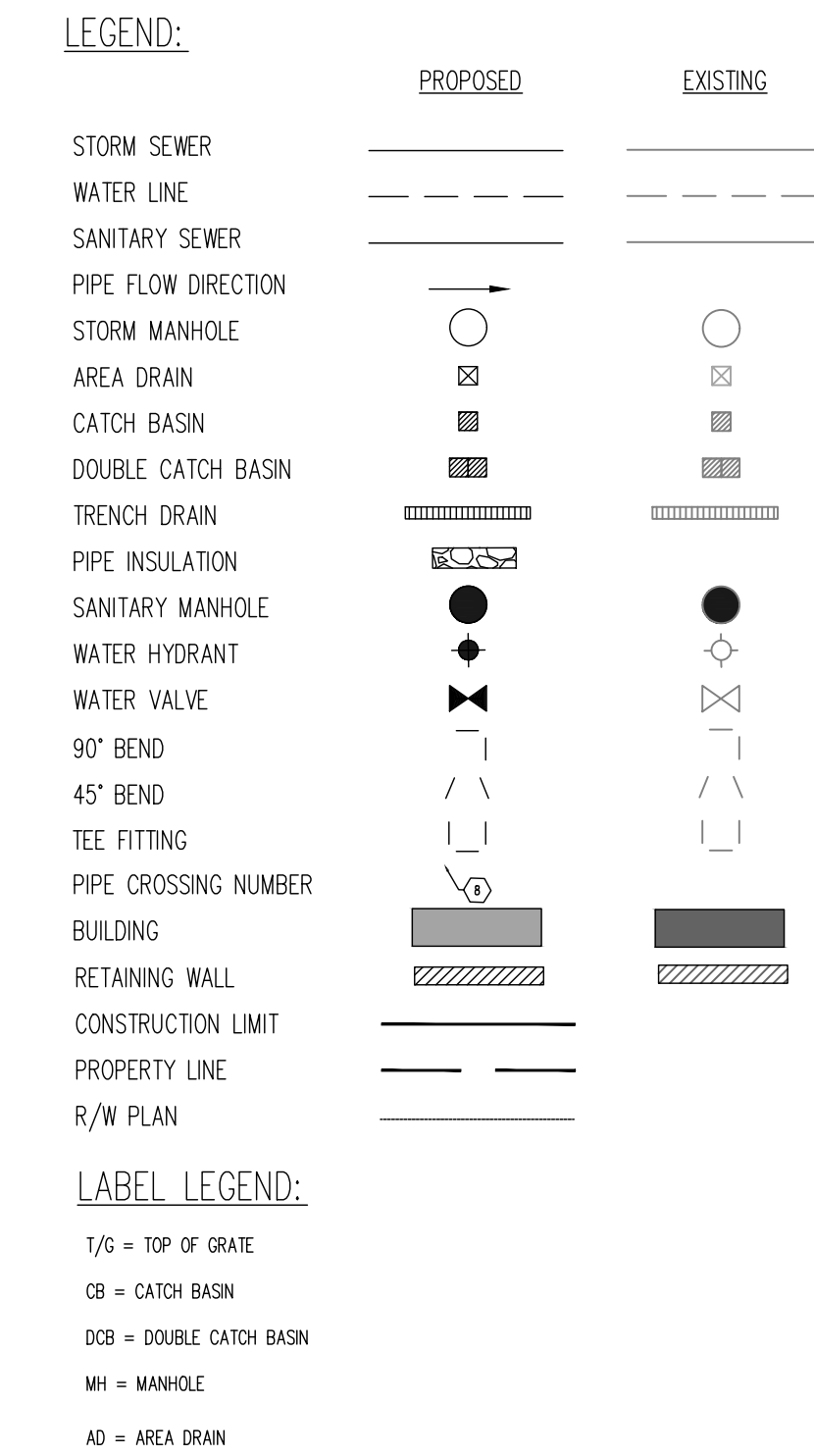
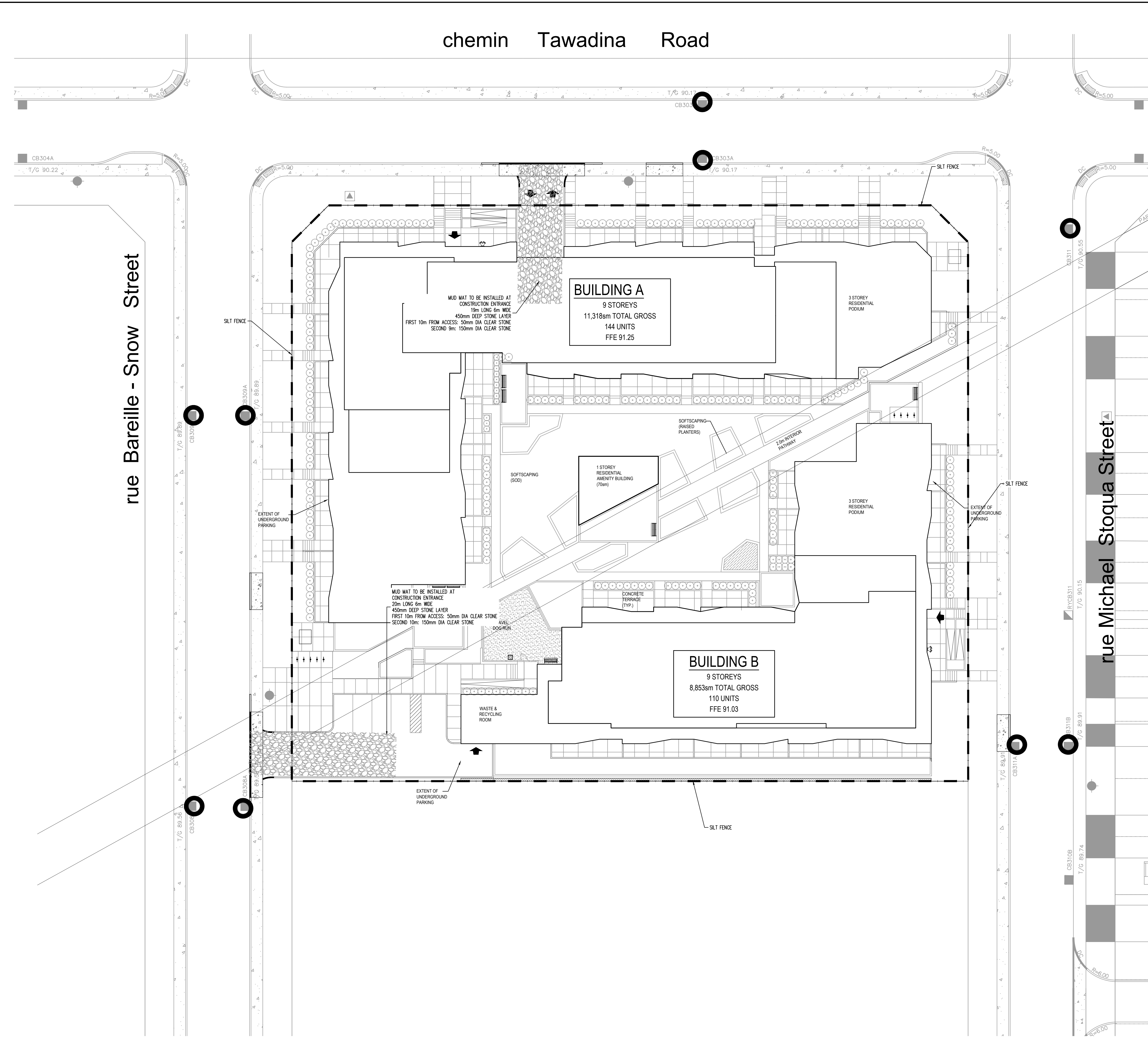
**ISSUE** 2



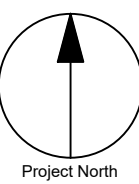
chemin Tawadina Road

rue Bareille - Snow Street

rue Michael Stogya Street



- NOTES:**
- IT IS THE RESPONSIBILITY OF THE APPLICANT/CONSULTANT TO ENSURE COMPLIANCE WITH ALL APPLICABLE PROVINCIAL STANDARDS AND TO OBTAIN ALL PROVINCIAL APPROVALS, INCLUDING BUT NOT LIMITED ENVIRONMENTAL COMPLIANCE APPROVALS.
  - EROSION AND SEDIMENT CONTROL (ESC) MEASURES WILL BE IMPLEMENTED PRIOR TO, AND MAINTAINED, DURING CONSTRUCTION PHASES TO PREVENT ENTRY OF SEDIMENT INTO THE WATER. ALL DAMAGED ESC MEASURES SHOULD BE REPAIRED AND/OR REPLACED WITHIN 48 HOURS OF INSPECTION.
  - ALL DISTURBED AREAS WILL BE MINIMIZED TO THE EXTENT POSSIBLE AND TEMPORARILY OR PERMANENTLY STABILIZED OR RESTORED AS WORK PROGRESSES. THE ESC STRATEGIES OUTLINED IN THE REPORTS AND IN THESE NOTES ARE NOT STATIC AND MAY NEED TO BE UPGRADED/AMENDED AS SITE CONDITIONS CHANGE TO MINIMIZE SEDIMENT LADEN RUNOFF FROM LEAVING THE WORK AREA. IF THE PRESCRIBED MEASURES IN THE REPORTS AND NOTES ARE NOT EFFECTIVE IN PREVENTING THE RELEASE OF DELETERIOUS SUBSTANCE, THEN ALTERNATIVE MEASURES MUST BE IMPLEMENTED IMMEDIATELY TO MINIMIZE POTENTIAL ECOLOGICAL IMPACTS AND A TORONTO REGION CONSERVATION AUTHORITY ENFORCEMENT OFFICE SHOULD BE IMMEDIATELY CONTACTED. ADDITIONAL ESC MEASURES TO BE KEPT ON SITE AND USED AS NECESSARY.
  - ALL ACTIVITIES, INCLUDING MAINTENANCE PROCEDURES, WILL BE CONTROLLED TO PREVENT THE ENTRY OF PETROLEUM PRODUCTS, DEBRIS, RUBBLE, CONCRETE, OR OTHER DELETERIOUS SUBSTANCES INTO THE WATER. VEHICULAR REFUELING AND MAINTENANCE WILL BE CONDUCTED A MINIMUM OF 30m FROM ANY ONSITE WATER BODY OR DRAINAGE WAY.
  - FILTER FABRIC TO BE PLACED UNDER GRATES ON ALL CATCH BASINS SURROUNDING THE SITE, TO TRAP SEDIMENT. SILT TRAPS ARE TO BE CLEANED REGULARLY AND ARE NOT TO BE REMOVED UNTIL SUCH TIME AS THE CONSTRUCTION IS COMPLETE AND THE LANDSCAPING IS GRADED AND SODED. FILTER FABRIC FOR SILT CONTROL TO BE TERRA FIX 270R OR APPROVED EQUIVALENT.
  - ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL MEET THE REQUIREMENTS SET OUT IN OPSS 804, OPSS 805, CITY OF OTTAWA CONSTRUCTION SPECIFICATION 01 57 13, CITY OF OTTAWA STANDARD SPECIFICATION SP F-1004 AND CITY OF OTTAWA STANDARD SPECIFICATION SP F-804.



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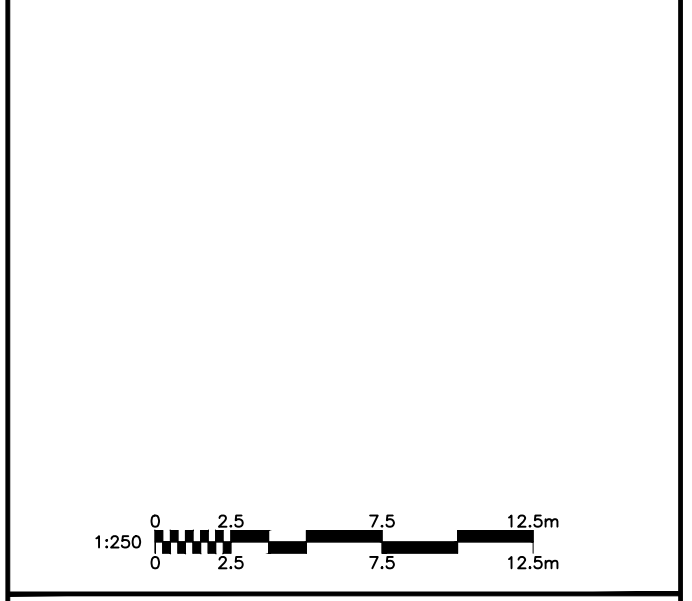
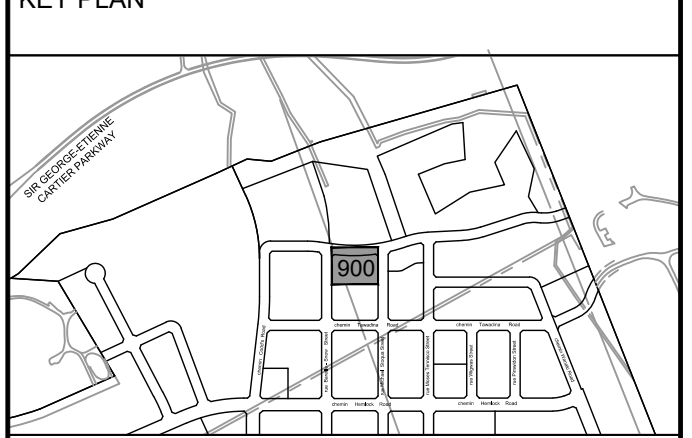
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**PROJECT**

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WATERIDGE VILLAGE PHASE 2

**PROJECT NO:**  
142609

**DRAWN BY:**  
M.M.

**CHECKED BY:**  
J.I.M.

**PROJECT MGR:**  
S.L.

**APPROVED BY:**  
A.C.

**SHEET TITLE**

SITE EROSION AND SEDIMENT CONTROL PLAN

**SHEET NUMBER**  
C-900

**ISSUE**  
2