



**St. Lawrence Testing
& Inspection Co. Ltd.**

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September 30, 2022

Mr. Anthony Orlicky
Eastern Ontario Pallets
7248 Bank St.
Metcalf, ON
K0A 2P0

**RE: New Building in South West Area of Property
Geotechnical Subsurface Investigation
Report No. 22C323**

Dear Mr. Orlicky:

In accordance with verbal instructions received from you, this report is submitted, outlining the results of a geotechnical subsurface investigation carried out South of the existing main plant and towards the West end of the main plant

A) DESCRIPTION OF FIELD WORK & STRATIGRAPHY

We arrived at the site on September 27, 2022 and met with you. We then went to the proposed building location which was marked out. You engaged a back hoe to dig the test pits. Supervision was by the undersigned geotechnical engineer. Following is the stratigraphy at the test pits.

Test Pit #1

3 m North East of North East corner.

- | | |
|-------------------------|---------------------------------------------------------------------------|
| 0 – 1.25 ft (0.38 m): | Gravel fill. |
| 1.25 – 2.5 ft (0.76 m): | Sand fill. |
| 2.5 – 3.0 ft (0.91 m): | Topsoil. |
| 3.0 – 5.3 ft (1.62 m): | Brown, moist, very stiff silty clay. |
| 5.3 – 6.0 ft (1.83 m): | Brown, moist, compact silty gravelly sand till with cobbles and boulders. |

Test Pit #2

3 m South West of South West corner.

- | | |
|------------------------|--------------------------------------------------------------------------|
| 1 – 1.0 ft (0.30 m): | Gravel fill. |
| 1.0 – 2.0 ft (0.61 m): | Sand fill. |
| 2.0 – 2.6 ft (0.79 m): | Topsoil. |
| 2.6 – 4.5 ft (1.37 m): | Brown, moist, very stiff silty clay. |
| 4.5 – 5.5 ft (1.52 m): | Brown, moist, compact silty gravelly sand till with cobble and boulders. |

Ground water was noted trickling in at 4.5 ft (1.37 m) below the surface at Test Pit #2.

B) GEOTECHNICAL DISCUSSION

It is our understanding that it is proposed to build a building at the location of the test pits. The purpose of the building is for storage. We asked you about the nature of the floor area. You indicated this would be a gravel surface.

The footings for the building should be designed in the native, very stiff silty clay using a bearing capacity of 100 KPa S.L.S. and 150 KPa U.L.S. The site seismic factor is Site Class D. The design frost depth in this area is 5.0 ft (1.5 m) below the final exterior grade.

It should be noted if the footing depth is 5.5 ft below the surface, the bearing will be 150 KPa S.L.S and 225 KPa U.L.S. The site seismic factor will be Site Class C since this depth is within the silty gravelly sand till stratum.

For the proposed interior gravel surface, any of the existing debris should be removed, including old vegetation. Where additional material is required to fill in the holes, this should be Granular "A". When the new gravel surface has been graded, it should be compacted to 100% Standard Proctor Density.

C) CONSTRUCTION CONTROL

In order to ensure that the recommendations of this report are adhered to, it is recommended that our firm be retained to inspect, test and report accordingly. For the final footing inspection after the initial footing inspection has been

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approved by us, this can also be done by the local municipal building inspector.

Respectfully submitted

ST. LAWRENCE TESTING & INSPECTION CO. LTD.



G.G. McIntee, P. Eng.

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