

LRL File No. 220536-03 Project: Commercial Development Location: 5254 Bank Street, Ottawa Date: April 2, 2024 Designed: K.Herold

Water Demand (Based on City of Ottawa Design Guidelines - Water Distribution, 2010)

Unit Type		Unit Rate	Area (ft ²)	Area (ha)	Demand (L/d)
Service/Repair Shop (Industrial-Light)	35000	L/(grossha)/d	18331.0	0.17	5960.3
· · · · · · · · · · · · · · · · · · ·				0.17	5960.3
ercial / Industrial Consumption Rates					
Unit Type	Value	Units	Value	Units	
Average Daily Demand	5,960	L/d	0.069	L/s	
Maximum Daily Factor	1.5	(Design guidelines - w	vater distribution Ta	ble 4.2)	
Maximum Daily Demand	8,940	L/d	0.103	L/s	
Peak Hour Factor	1.8	(Design guidelines - w	vater distribution Ta	ble 4.2)	
Maximum Hour Demand	16,093	L/d	0.186	L/s	
emand					
Demand	Value	Units	Value	Units	
Average Daily Demand	5,960	L/d	0.069	L/s	
Maximum Daily Demand	8,940	L/d	0.103	L/s	
Maximum Hourly Demand	16,093	L/d	0.186	L/s	
Service Pipe Sizing					
Q = VA	Q = Flow Rate	V = Velocity	A = Area of pi	ne	
Assumed maximum velocity =		m/s	A = Alcu of pl		
Q =	= 0.19	L/s			
		m ³ /s			
Q =	= 0.00019	m ⁻ /s			
Minimum pipe diameter (d) =	= (4Q/πV) ^{1/2}				
	= 0.011	m			
=		mm			



Pipe Pressure Losses Calculations

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Piezometric Head Equation (Derived from Bernoulli's Equation)

$$h = \frac{p}{\gamma} + z$$

Where:

h = HGL (m)

- p = Pressure (Pa)
- γ = Specific weight (N/m3) = z = Ground Elevation (m) =

9810 112.93

Water Pressure on Huron Street				
HGL (m)		Pressure		
		kPa	psi	
Minimum =	159.9	460.78	66.83	
Maximum =	165.2	512.77	74.37	
Max. Day + Fire =	155.2	414.67	60.14	

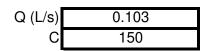
Hazen Williams Equation

$$h_f = \frac{10.67 \times Q^{1.85} \times L}{C^{1.85} \times d^{4.87}}$$

Where:

- h_f = Head loss over the length of pipe (m)
- Q = Volumetric flow rate (m³/s)
- L = Length of pipe (m)
- C = Pipe roughness coefficient
- d = Pipe diameter (m)

Scenario 1: maximum daily demand



L (m.)	39.5	
I.D. (mm)	19	
V (m/s)	0.36	
h _f (m)	0.41	
Head Loss (psi)	0.58	
Min. Pressure (psi)	66.25	
Max. Pressure (psi)	73.79	_
Service Obv. @ Street Connection (m)	110.53	
Service Obv. @ Building Connection (m)	111.60	
Pressure Adjustment (psi)	-1.52	(due to service elev. Diff. from street to building)
Adjusted Min. Pressure (psi)	64.73	(must not be less than 50psi)
Adjusted Max. Pressure (psi)	72.27	(must not be more than 80psi)

Scenario 2: maximum hourly demand

Q (L/s)	0.186	
C	150	
L (m.)	39.5	
I.D. (mm)	19	
V (m/s)	0.66	
h _f (m)	1.21	
Head Loss (psi)	1.71	
Min. Pressure (psi)	65.12	
Max. Pressure (psi)	72.66	_
Service Obv. @ Street Connection (m)	110.53	
Service Obv. @ Building Connection (m)	111.60	
Pressure Adjustment (psi)	-1.52	(due to service elev. Diff. from street to building)
Adjusted Min. Pressure (psi)	63.59	(must not be less than 40psi)
Adjusted Max. Pressure (psi)	71.14	(must not be more than 80psi)

Boundary Conditions 5254 Bank Street

Provided Information

	Demand		
Scenario	L/min	L/s	
Average Daily Demand	4.14	0.069	
Maximum Daily Demand	6.18	0.103	
Peak Hour	11.16	0.186	
Fire Flow Demand # 1	6000	100.0	

Location



Results

Connection 1 – Bank Street

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	165.2	74.3
Peak Hour	159.9	66.8
Max Day plus Fire #1	155.2	60.1

¹ Ground Elevation = 112.93 m

<u>Notes</u>

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

