

**PROPOSED
FOUR STOREY APARTMENT BUILDING SITE
LOT 15 WEST CAMBRIDGE STREET NORTH
SOUTH OF STONE BOUNDARY
R-PLAN 33
370 CAMBRIDGE STREET NORTH
CITY OF OTTAWA**

**SERVICEABILITY REPORT
REPORT No. R-821-155A**

T.L. MAK ENGINEERING CONSULTANTS LTD.

JULY 2022

REFERENCE FILE NUMBER 821-155

Introduction

The developer of this property is proposing to redevelop the existing residential lot described as Lot 15 West Cambridge Street North R-Plan 33 by constructing a four (4) storey residential apartment building consisting of two (2) bachelor apartments, ten (10) 1-bedroom apartments, six (6)-bedroom apartments and two (3)-bedroom apartments totalling 20 residential units.

The municipal address of this property is referenced as 370 Cambridge Street North and it is located in the City Ward (14 - Somerset). The site is situated on the north side of Cambridge Street North, north of Raymond Street and south of Arlington Avenue, see site plan and legal survey plan in Appendix A for details.

The area of this property is ± 0.063 hectares. In addition to the (4)-storey residential building, the other development features will comprise of an interlock paver access to the front entrance way and interlock paver access along both the north and south side yard to the side entrances of the building, an amenity area in the rear yard, as well as visitor bicycle parking, (4) asphalt vehicle parking spaces including landscaped areas throughout the site, etc., to meet the City of Ottawa's site plan requirements.

A site geotechnical report was prepared by the owner's soils engineer, Kollaard Associates, entitled Geotechnical Investigation Report (Project No. 220214) dated April 8, 2022.

This serviceability report will provide the City of Ottawa with our serviceability brief to address the proposed servicing scheme for this site.

Existing Site Conditions and Servicing

This property is currently occupied by a two storey residential building, which is a two (2)-storey brick and vinyl sided dwelling encompassed by concrete surfaces with asphalt parking area and wood sided garage located at the rear of the property. Vehicle access is via the existing Arthur Laneway along the rear of this property. For additional details of the site's pre-development conditions, refer to the coloured Google Image (2019) and aerial photography from (GeoOttawa 2021) in Appendix B.

The site is primarily hard impermeable surfaced that are covered with building roof areas, asphalt parking, concrete walkway surfaces, etc. The remainder of the lot is generally grass or landscape areas. Pre-development coefficient of runoff is estimated at $C = 0.68$.

The topography of the land is found to be gently sloping from west to east or (back to front) with an average slope of $\pm 1.6\%$.

The existing house water and sanitary service lateral currently servicing the existing dwelling on 370 Cambridge Street North will be removed. The existing water services shall be blanked at the main and the existing house laterals shall be capped at the front property line for re-development of this lot.

As for the availability of underground municipal services, there are existing municipal services along Cambridge Street North in front of this property consisting of a 375mm diameter combined sewer and a 200mm diameter watermain for development of this property. Refer to the City of Ottawa Cambridge Street North UCC drawing and As-Built plan and profile drawings included in Appendix C for details.

Because the site is located within a combined sewer shed, therefore, the approval exemption under Ontario Regulations 525/98 would not apply since storm water discharges from this site will outlet flow into a combined sewer and not a storm sewer. Thus, an Environmental Compliance Approval (ECA) application will be required to be submitted to the Ministry.

Proposed Residential Apartment Building Site

Vehicle access to this site will be via the existing Arthur Laneway located at the rear of this lot. There are (4) vehicle parking spaces provided for vehicle parking at the rear of the building. Interlock pavers are proposed at the front, as well as the north and south side of the new building for pedestrian access to the side entrances.

A. Water Supply

The proposed building located within Pressure Zone 1W at 370 Cambridge Street North is a 4-storey residential apartment building with a basement. The building contains twenty (20) total units, namely two (2) bachelor units, ten (10) 1-bedroom units, six (6) 2-bedroom units, and two (2) 3-bedroom units. The average floor area is approximately 340 m², for a total gross floor area of approximately 1,352 m² (excluding the basement). The building is to be serviced by the 200 mm diameter watermain along Cambridge Street North.

The ground elevation on the property is approximately 71.7 m, as obtained from geoOttawa elevation contours (**Figure 1** in Appendix D), and the Site Survey Plan provided to Stantec (see attached **Site Survey Plan** in Appendix D).

Demand Projections

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines, where the residential consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 2.5. Peak hour (PKHR) demands were calculated by multiplying MXDY

by a factor of 2.2. Persons per unit (PPU) for each unit were estimated based on the City of Ottawa’s Water Design Guidelines. **Table 1** shows the estimated domestic demands of the proposed building.

Table 1 : Estimated Domestic Demand

Unit Type	Unit Count	PPU	Consumption Rate (L/c/d)	AVDY		MXDY		PKHR	
				L/d	L/s	L/d	L/s	L/d	L/s
Apartment, Bachelor	2	1.4	280	784	0.01	1,960	0.02	4,312	0.05
Apartment, 1-Bedroom	10	1.4		3,920	0.05	9,800	0.11	21,560	0.25
Apartment, 2-Bedroom	6	2.1		3,528	0.04	8,820	0.10	19,404	0.22
Apartment, 3-Bedroom	2	3.1		1,736	0.02	4,340	0.05	9,548	0.11
Total	20			9,968	0.12	24,920	0.29	54,824	0.63

The City had previously indicated that the City’s Fire Marshall and various City departments are currently reviewing fire flow requirements for low- and mid-rise buildings. As per the City of Ottawa’s Water Design Guidelines, the FUS method is to be used for fire flow requirements affecting watermain sizing; with regards to fire protection on private property and not requiring new watermains, these are covered by the Ontario Building Code (OBC).

As such, the fire flow requirement was also determined following the OBC’s Office of the Fire Marshal (OFM) method. The proposed building will be of wood frame construction, where floors are fire separations, but without fire-resistance ratings. The resulting total required fire flow (RFF) for a sprinklered building is 6,300 L/min (105 L/s) for a minimum duration of 40 min. Details are provided in the attached **OFM Fire Flow Calculations** in Appendix D. The proposed **Site Plan** attached in Appendix D was used to determine distances from the proposed building to the property lines. For street facing sides, **Figure 2** in Appendix D provides separation distances from the street.

In summary, the estimated water demands for the proposed building are as follows:

- AVDY = 9,968 L/d (0.12 L/s)
- MXDY = 24,920 L/d (0.29 L/s);
- PKHR = 54,824 L/d (0.63 L/s); and,
- Fire Flow = 6,300 L/min (105 L/s).

Boundary Conditions

The hydraulic gradeline (HGL) boundary conditions for 370 Cambridge Street North, as presented in **Table 2**, were provided by the City on May 26, 2022 (see attached **Water Boundary Conditions** Email in Appendix D).

Table 2 : Boundary Conditions

Demand Scenario	Head (m)
Minimum HGL (Peak Hour)	107.0
Maximum HGL (Average Day)	115.2
Maximum Day + Fire Flow (105 L/s)	108.8

Hydraulic Analysis

Peak Hour & Average Day

During peak hour demands, the resulting minimum hydraulic gradeline of 107.0 m corresponds to a peak hour pressure of 346 kPa (50 psi). This value is above the minimum pressure objective of 276 kPa (40 psi) for residential buildings up to two storeys. The peak hour pressure exceeds this objective and is therefore considered acceptable. Given that this apartment building consists of a total of 4 storeys, further consideration will be needed to service the higher floors. Adding 5 psi per floor above two stories (i.e., 2 additional floors), a minimum pressure of 346 kPa (50 psi) would be required for the fourth floor. The peak hour pressure calculated is equal to this objective and is therefore considered acceptable.

During average day demands, the resulting maximum hydraulic gradeline of 115.2 m corresponds to a maximum pressure of 426 kPa (62 psi). This value is less than the maximum pressure objective of 552 kPa (80 psi) and is therefore considered acceptable.

Supporting hydraulic calculations are attached in Appendix D.

Maximum Day + Fire Flow

A maximum day plus fire flow hydraulic gradeline of 108.8 m corresponds to a residual pressure of 364 kPa (53 psi) at this location and is well above the minimum residual pressure requirements of 140 kPa (20 psi).

Based on Table 1 of Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 and a desktop review (i.e., Google Street View) to confirm hydrant class, four (4) Class AA hydrants are located in the vicinity of the proposed building. Two (2) Class AA hydrants within 75 m from the site, each with a capacity contribution of up to 5,700 L/min. Two (2) other Class AA hydrants are within 150 m from the site, each with a capacity contribution of up to 3,800 L/min. As such, the combined hydrant flow coverage for the building is estimated to be 19,000 L/min, which is above the OFM required fire flow (RFF) of 6,300 L/min.

Hydrant coverage and classes are illustrated in **Figure 3** attached in Appendix D. A breakdown of available hydrant flow is summarized in **Table 3**.

Table 3 : Fire Hydrant Coverage

Building	Calculated Fire Flow Demand (L/min)	Fire Hydrants					Combined Hydrant Flow Coverage (L/min)
		Hydrant Class	Within 75 m		Between 76 m and 150 m		
			Quantity	Contrib. to RFF	Quantity	Contrib. to RFF	
370 Cambridge Street N	OFM: 6,300	AA	2	5,700	2	3,800	19,000
		A					
		B					
		C					

In conclusion, based on the boundary condition provided, the 200 mm diameter watermain on Cambridge Street North (intended service line connection) provides adequate fire flow capacity as per the OBC’s Office of the Fire Marshal (OFM) method to the proposed development at 370 Cambridge Street North.

Anticipated demand flows meet the pressure objectives during average and peak demand conditions, as per the City of Ottawa’s Drinking Water Design Guidelines.

B. Sanitary Flow

The peak sanitary flow for the proposed 20 residential units, which comprise of two (2) bachelor units, ten (10) 1-bedroom units, six (2)-bedroom units and two (2) 3-bedroom units, is estimated at $Q = 0.44$ L/s with an infiltration rate of 0.02 L/s. Please refer to Appendix E regarding sanitary flow calculations. This flow will enter the existing 375mm diameter combined sewer on Cambridge Street North via the proposed 150mm diameter PVC sanitary service lateral from the four (4)-storey residential apartment building.

Peak waste water flow from water usage is typical for an apartment building of this size. No additional amenities are proposed.

The existing peak sanitary flow of the site for single detached dwelling unit is $Q = 0.06$ L/s with an infiltration rate of 0.02 L/s. The net increase in flow from this proposed development is 0.38 L/.

The peak sanitary flow of 0.44 L/s will be subtracted from the net allowable controlled release rate regarding storm-water management controlled for this site into the existing combined sewer.

Waste water from the Cambridge Street North 375mm dia. combined sewer then in turn outlets further north into the existing 900mm dia. Gladstone Avenue concrete combined

collector sewer located north of this site. Waste water flow from this collector sewer then flows further west along Gladstone Avenue.

C. Storm Flow

The storm-water outlet for the proposed development property will be the existing 375mm diameter combined sewer located on Cambridge Street North. Stormwater attenuation on site will be accomplished by means of rooftop storage with controlled roof drains that regulate flow off site.

Two (2) roof drains are proposed for this apartment building that will restrict maximum flow to a rate of 0.95 L/s each or $2 \times 0.95 \text{ L/s} = 1.90 \text{ L/s}$ under a head of 150mm into the Cambridge Street North combined sewer. The calculated net allowable controlled release rate from this site is estimated at 5.40 L/s under the 2 year pre-development event.

At this development site, for storm events up to the 100-year event the maximum post development flow draining off-site is the controlled roof top flow plus the 100-year uncontrolled flow from the site draining to the front and peak sanitary flow which totals to 10.34 L/s ($1.90 \text{ L/s} + 8.00 \text{ L/s} + 0.44 \text{ L/s}$) and is 4.94 L/s greater than the allowable flow of 5.40 L/s.

Based on the residential site plan from the owner's architect, the average post-development runoff coefficient is estimated at $C = 0.76$ and $A = 0.063$ hectares.

An estimation of the 2-year pre-development flow condition was carried out using the criteria accepted by the City of Ottawa. If post-development C value exceeds the lesser of the $C_{pre} = 0.68$ or $C_{allow} = 0.4$ (max) then SWM is required. So from our calculations, the $C_{allow} = 0.4$ value will be used at $t_c = 10$ minutes for pre-development allowable flow calculation off-site.

The pre-development flow rate calculated into a combined sewer for this residential area is the lesser of the two (2)-Year storm event where $C_{allow} = 0.4$ (max.) runoff value and $t_c = 10$ minutes or the average C_{pre} value which is 0.68 using $t_c = 10$ minutes. Because this site $C_{post} = 0.76$ and $C_{allow} = 0.4$ then SWM measures are required.

Therefore, based on our calculation, on-site retention is required for this proposed development site, because the site post-development C value of 0.76 is greater than the $C_{allow} = 0.4$.

The storage volume for the two (2)-year and up to the 100-year storm event will be stored by means of flat rooftop on the top of the fourth floor of the apartment building. Also refer to the site storm drainage report (Report No. R-821-155) for further details.

To control the two (2)-Year storm-water release rate off-site from roof top to a rate of 1.90 L/s, a site storage volume of approximately 3.95 m³ minimum is required during the two (2)-year event.

During the two (2)-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1 and 2 is estimated at 100 mm at the drain and 0mm at the roof perimeter, assuming a 1.5% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 2.67 m³ and the rooftop storage available at Roof Area 2 is 2.47 m³, for a total of 5.14 m³, which is greater than the required volume of 3.95 m³.

To control the 100-year storm-water release rate off site from roof top to a rate of 1.90 L/s, a site storage volume of approximately 14.13 m³ minimum is required during the 100-year event. During the 100-year storm event for the flat rooftop storage, the ponding depth of Roof Area 1 and 2 is estimated at 150 mm at the drain and 0 mm at the roof perimeter, assuming a 1.5% minimum roof pitch to the drain. The rooftop storage available at Roof Area 1 is 9.50 m³ and the rooftop storage available at Roof Area 2 is 8.46 m³, for a total of 17.96 m³, which is greater than the required volume of 14.13 m³.

Therefore, by means of flat building rooftop storage and grading the site to the proposed grades as shown on the Proposed Grading and Servicing Plan and Proposed Rooftop Stormwater Management Plan Dwg. 821-155 G-1 and 821-155 SWM-1 respectively, the desirable two (2)-year storm and 100-year storm event detention volume of 5.14 m³ and 17.96 m³ respectively will be available on site.

At this development site for storm events up to the 100-year event, the maximum post development flow draining off-site is the controlled roof top flow plus the 100-year uncontrolled flow from the site draining to the front and peak sanitary flow totals to 10.34 L/s (1.90 L/s + 8.00 L/s + 0.44 L/s) which is 4.94 L/s greater than the allowable flow of 5.40 L/s and considered acceptable by the City of Ottawa for a small site of this lot size.

In comparing the pre-development flow of the current site conditions to the post development flow as shown on Page No. 6 and 7 of the Storm Drainage Report (Report No. R-821-155), the SWM regulated flow plus uncontrolled flow from the proposed site under the post development conditions at the 2-year event = 5.09 L/s and the 100 year event = 10.34 L/s whereupon both of the post development flow events are less than current pre-development flow estimate for the site at 2-Year $p_{re} = 9.18$ L/s and 100-Year $p_{re} = 23.77$ L/s. Therefore with this proposed development, stormwater flow is improved from that of the existing condition.

The building weeping tile drainage will outlet via its separate 150mm diameter PVC storm lateral. The roof drains will be outletted also via a separate 150mm PVC storm lateral, where upon both laterals are connected directly to the existing Cambridge Street North 375mm

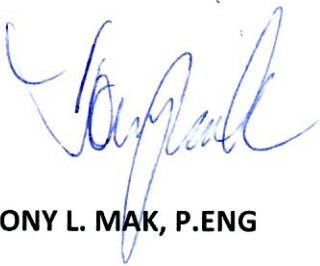
diameter storm sewer. The City of Ottawa recommends that pressurized drain pipe material be used in the building for the roof drain leader pipe in the event of surcharging in the City combined sewer system. Refer to the proposed site grading and servicing plan Dwg. 821-155 G-1 for details.

Erosion and Sediment Control

The contractor shall implement Best Management Practices to provide for protection of the receiving storm sewer during construction activities. These practices are required to ensure no sediment and/or associated pollutants are released to the receiving watercourse. These practices include installation of a "siltsack" catch basin sediment control device or equal in catch basins as recommended by manufacturer on-site and off-site within the Cambridge Street North road right of way adjacent to this property. Siltsack shall be inspected every 2 to 3 weeks and after major storm. The deposits will be disposed of as per the requirements of the contract. See Dwg. #821-155 ESC-1 for details.

Refer to Appendix F for the summary of the Development Servicing Study Checklist that is applicable to this development.

PREPARED BY T.L. MAK ENGINEERING CONSULTANTS LTD.

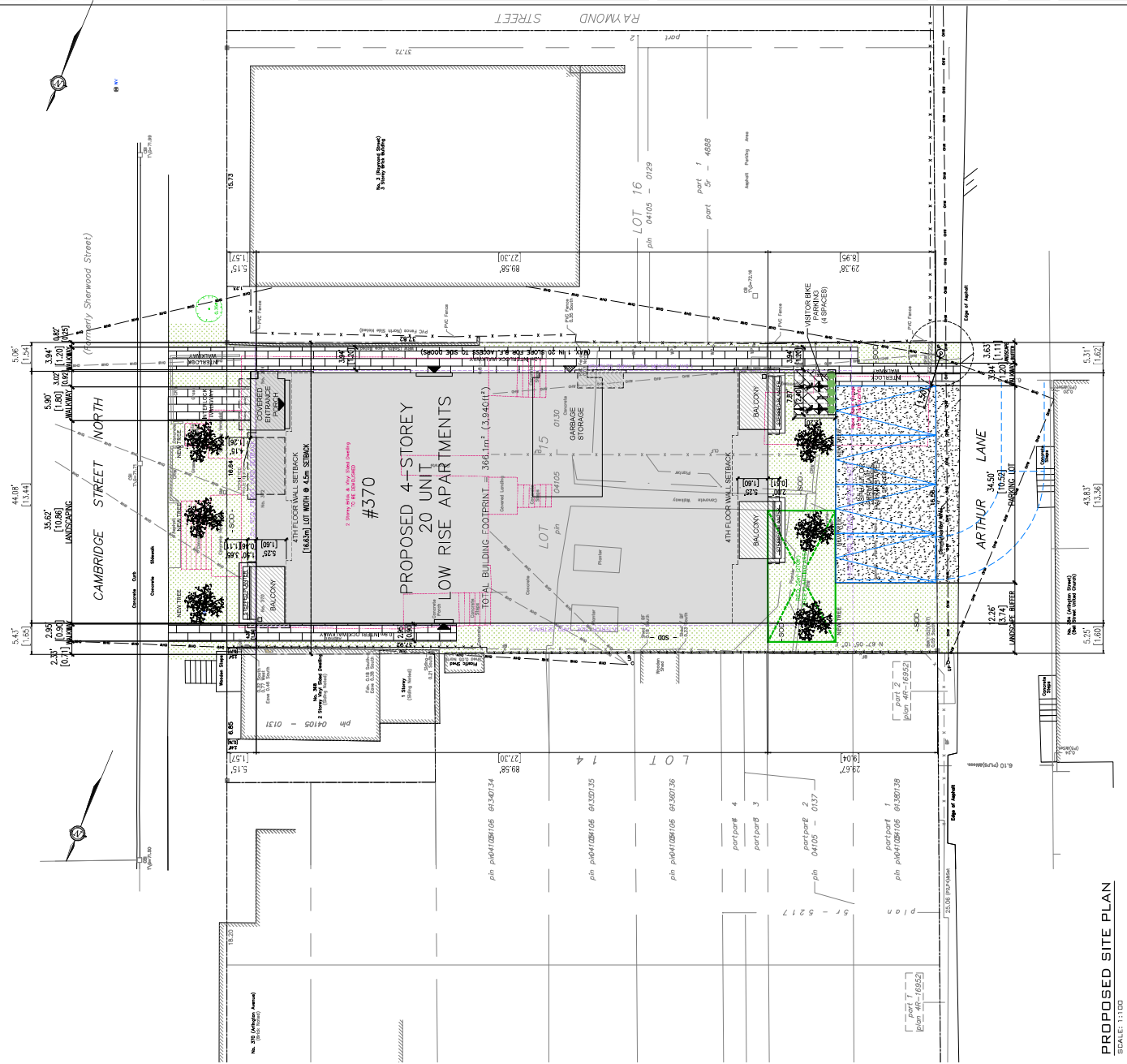


TONY L. MAK, P.ENG



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**APPENDIX A
SITE PLAN AND LEGAL SURVEY PLAN**



VEHICLE PARKING:
 16 VISITOR SPACES (4 SPACES)
 16 RESIDENT SPACES (4 SPACES)
 16 VISITOR SPACES (4 SPACES)
 16 RESIDENT SPACES (4 SPACES)

SECTION 11 - BICYCLE PARKING SPACE DATES AND DIMENSIONS:
 (1) The bicycle parking spaces shall be located within the building footprint or within the rear yard area.
 (2) The bicycle parking spaces shall be located within the building footprint or within the rear yard area.
 (3) The bicycle parking spaces shall be located within the building footprint or within the rear yard area.

SECTION 12 - MINIMUM VEHICLE PARKING SPACES:
 (1) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (3) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:

SECTION 13 - MINIMUM VEHICLE PARKING SPACES:
 (1) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (3) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:

SECTION 14 - MINIMUM VEHICLE PARKING SPACES:
 (1) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (3) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:

SECTION 15 - MINIMUM VEHICLE PARKING SPACES:
 (1) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (3) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:

SECTION 16 - MINIMUM VEHICLE PARKING SPACES:
 (1) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
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SECTION 17 - MINIMUM VEHICLE PARKING SPACES:
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SECTION 18 - MINIMUM VEHICLE PARKING SPACES:
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SECTION 19 - MINIMUM VEHICLE PARKING SPACES:
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SECTION 20 - MINIMUM VEHICLE PARKING SPACES:
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 (2) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:
 (3) The minimum number of vehicle parking spaces shall be determined in accordance with the following table:

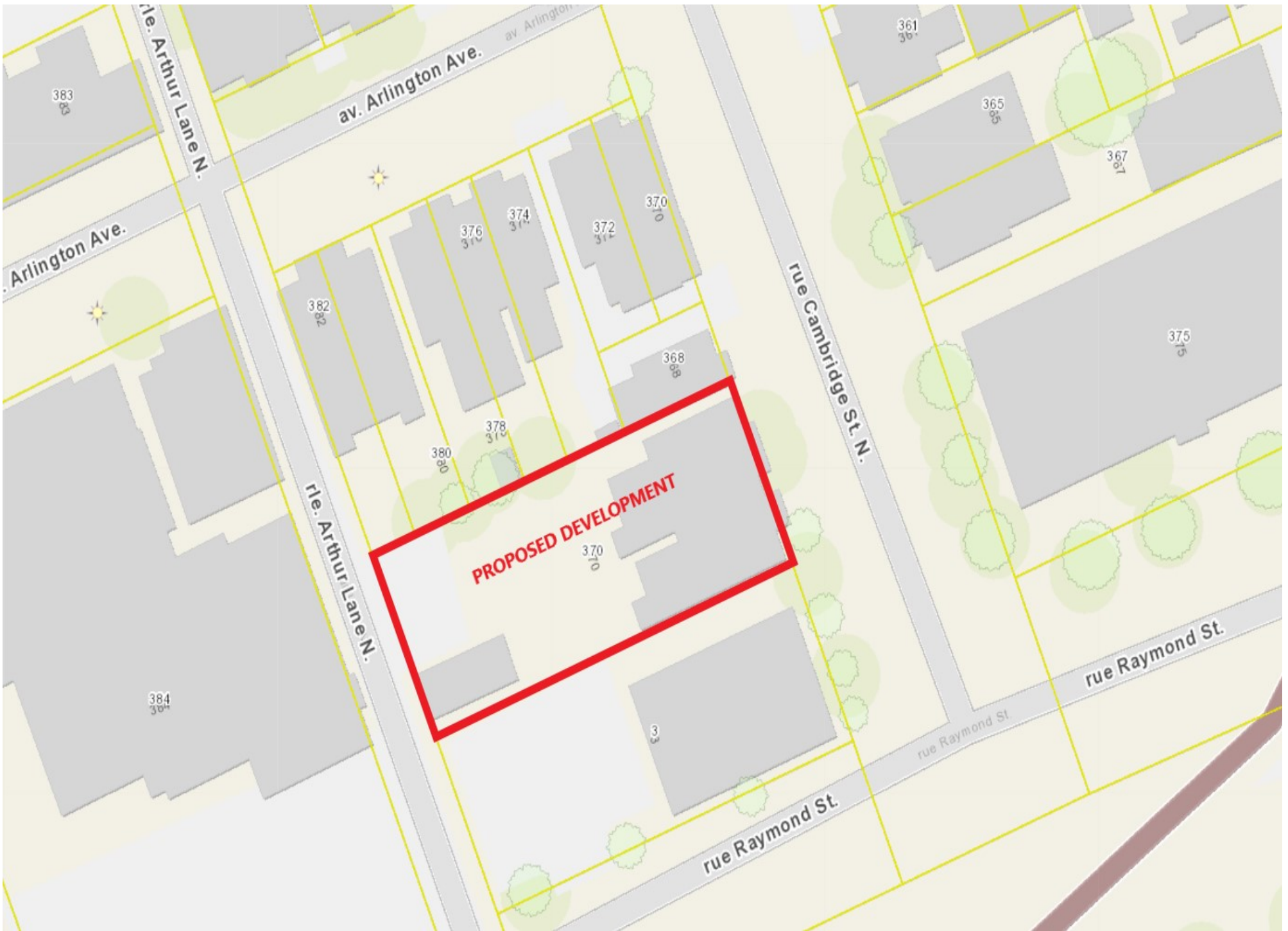
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100	REVISIONS	2024-08-15	JM

PROPOSED SITE PLAN
 SCALE: 1:100

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**APPENDIX B
SITE PRE-DEVELOPMENT CONDITION
GOOGLE IMAGE (2019)
AND
AERIAL PHOTOGRAPHY 2021 (GEOOTTAWA)**

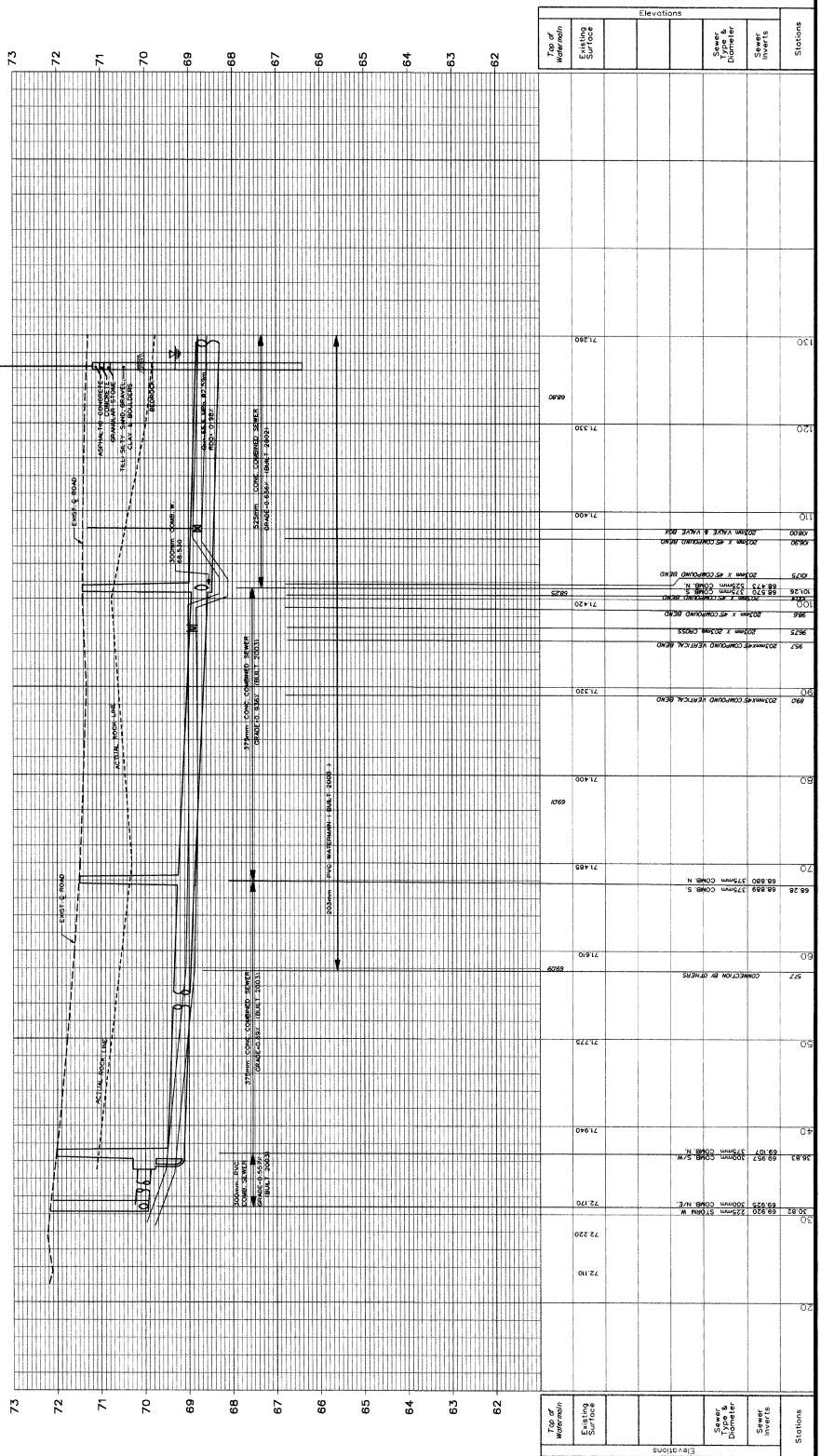
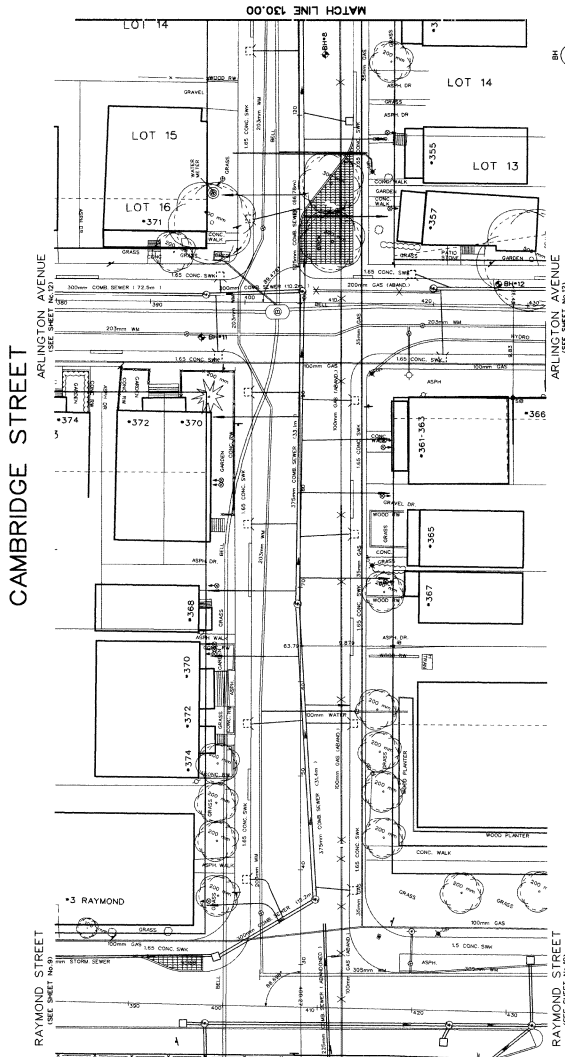






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**APPENDIX C
CAMBRIDGE STREET NORTH
CITY OF OTTAWA
PLAN AND PROFILE
AND
UCC DRAWINGS**



Revisions:

No.	Date	Description	Drawn By	Approved By

Design:

DESIGNED BY: I. BHATIA
 CHECKED BY: J. FRASER
 DATE: 2015

Drawn By: J. FRASER
Checked By: I. BHATIA
 DATE: 2015

Find Measurement & Rehabil Information

CONTRACTOR SERVICES MONITOR:
WAYNE BENNETT P.ENG.

CONTRACTOR: I. BHATIA
 PROJECT: 3335 CAMBRIDGE STREET
 DATE: 2015

- As-Built Notes:**
1. Soil information taken from: Mel Levas Engineering Ltd (2001)
 2. This plan supersedes in whole or in part plan #855-018-0102-388
 3. While illustrations and utilities shown are taken from the best available information, they should be approved by the City of Ottawa
 4. See section 10.1 for details regarding the construction of the existing sewer & watermain
 5. Benchmarks taken prior to construction.
 6. See typical cross sections for road structure material depths.
 7. Ministry of Environment Approval # Sewer: 0278-5M-0-CT Water: 5281-5M-0-0U

Legal Survey Notes:

Boundary information shown here has been completed and calculated from distances shown. It is for reference purposes only. Survey distances may not define property boundaries. No warranty is made as to the accuracy of the information shown here. THIS IS NOT A PLAN OF SURVEY. It is for reference purposes only. It is not a substitute for a legal survey. It is not a substitute for a legal survey. It is not a substitute for a legal survey.

Ottawa
 Transportation, Utilities and Public Works
 Infrastructure Services Branch
 10 LAUREL AVENUE WEST, OTTAWA, ONTARIO, K1P 0J1

Rosemarie Leclair
 Project Manager

Richard Hewitt, P.Eng.
 Professional Engineer

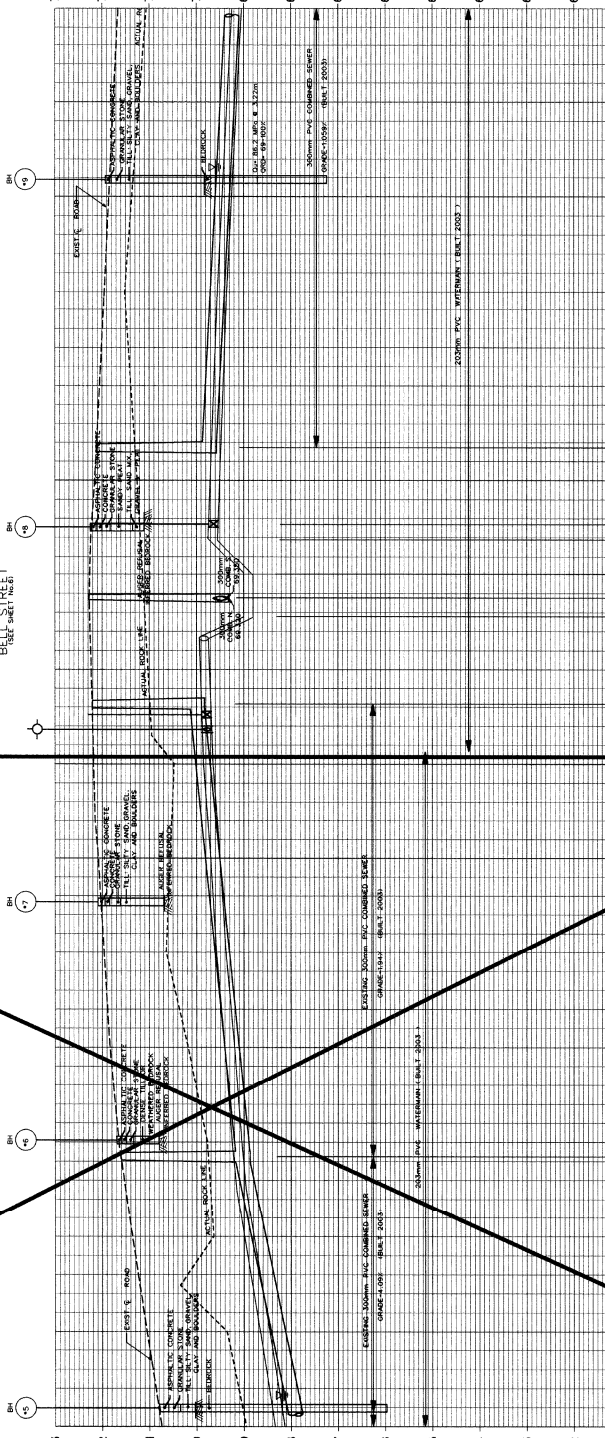
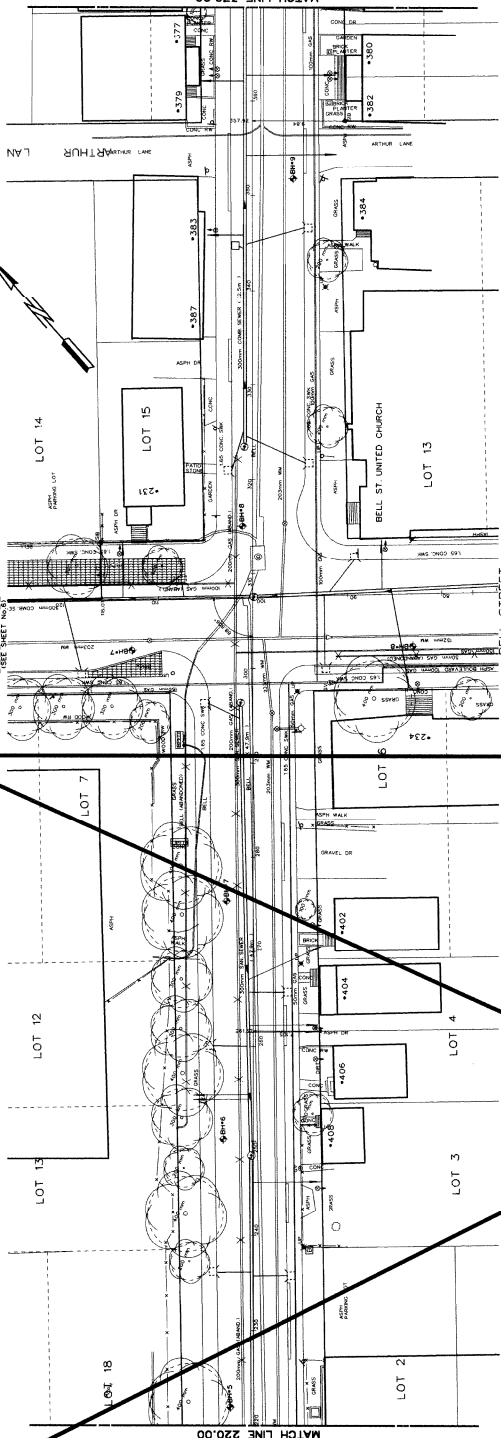
CAMBRIDGE STREET
 RAYMOND STREET TO STA. 130.00

PROJECT NO: 02-3335
 DRAWING NO: 3335-018-0102-388
 DATE: 2015
 SHEET: 3 OF 24
 TOTAL SHEETS: 24

Top of Manhole	Existing Surface	Sewer Diameter	Sewer Invert	Stations
71.90	72.220	300mm	68.880	20
72.10	72.220	300mm	68.880	30
72.10	72.220	300mm	68.880	35
72.10	72.220	300mm	68.880	40
72.10	72.220	300mm	68.880	45
72.10	72.220	300mm	68.880	50
72.10	72.220	300mm	68.880	55
72.10	72.220	300mm	68.880	60
72.10	72.220	300mm	68.880	65
72.10	72.220	300mm	68.880	70
72.10	72.220	300mm	68.880	75
72.10	72.220	300mm	68.880	80
72.10	72.220	300mm	68.880	85
72.10	72.220	300mm	68.880	90
72.10	72.220	300mm	68.880	95
72.10	72.220	300mm	68.880	100
72.10	72.220	300mm	68.880	105
72.10	72.220	300mm	68.880	110
72.10	72.220	300mm	68.880	115
72.10	72.220	300mm	68.880	120
72.10	72.220	300mm	68.880	125
72.10	72.220	300mm	68.880	130

3335 ARLINGTON AVENUE

ARLINGTON AVENUE



Top of Manhole	Existing Surface	Proposed Surface	Station
70.240	70.240	70.240	290
70.240	70.240	70.240	295
70.240	70.240	70.240	300
70.240	70.240	70.240	305
70.240	70.240	70.240	310
70.240	70.240	70.240	315
70.240	70.240	70.240	320
70.240	70.240	70.240	325
70.240	70.240	70.240	330
70.240	70.240	70.240	335
70.240	70.240	70.240	340
70.240	70.240	70.240	345
70.240	70.240	70.240	350
70.240	70.240	70.240	355
70.240	70.240	70.240	360
70.240	70.240	70.240	365
70.240	70.240	70.240	370

Revisions:

No.	Date	Description	Drawn By	Reviewed By

Design:

Checked By	Date	Checked By	Date

Final Measurement & As-Built Information

As-Built Notes:

1. See information taken from: Mel Levas Engineering Ltd. (2001)
2. This plan supersedes (in whole or in part) plan #858-89-050 and 858-89-052.
3. While illustrations and utilities shown are taken from the best available information, they cannot be guaranteed.
4. The utility lines shown are for information only.
5. Sewer & Watermain
6. See typical cross sections for road structure material depths.
7. Ministry of Environment Approval # Sewer: 0279-5AW-0CT Water: 5281-5AW-0BU

Ottawa

Transportation, Utilities and Public Works
Infrastructure Services Branch

100 LAUREL AVENUE WEST, OTTAWA, ONTARIO, K1P 1J1

Rosemarie Leclair
Project Manager

Richard Hewitt, P.Eng.
Professional Engineer

ARLINGTON AVENUE

STA. 220.00 TO STA. 370.00

PROJECT NO. 62-3335

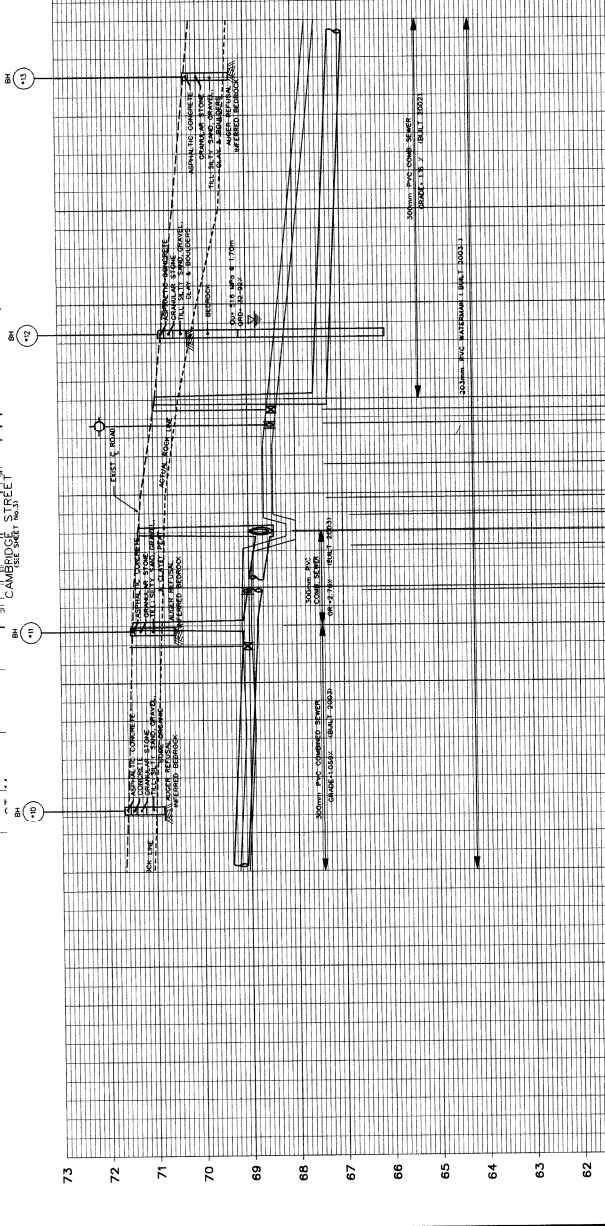
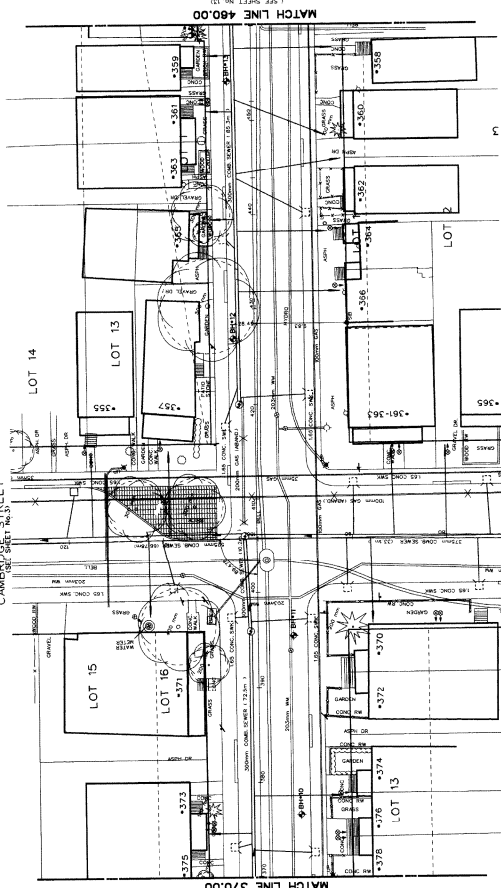
DATE: 11/25/05

SCALE: 1" = 50'

PLANNING NO. 3335

DATE: 11/24/05

REVISIONS:



Top of Watermain	Existing Surface	Elevations	Station
		73.00	370.00
		72.00	375.00
		71.00	380.00
		70.00	385.00
		69.00	390.00
		68.00	395.00
		67.00	400.00
		66.00	405.00
		65.00	410.00
		64.00	415.00
		63.00	420.00
		62.00	425.00
		61.00	430.00
		60.00	435.00
		59.00	440.00
		58.00	445.00
		57.00	450.00
		56.00	455.00
		55.00	460.00
		54.00	465.00
		53.00	470.00
		52.00	475.00
		51.00	480.00

No.	Date	Description	Drawn By	Approved By

Design:

Checked By: **P.K.K.** Date: **10/20/2011** Drawn By: Date:

Design: Date: Checked By: Date:

Checked By: Date: Checked By: Date:

Checked By: Date: Checked By: Date:

Professional Engineer

I. BHATIA

1000 UNIVERSITY AVENUE, SUITE 1000
OTTAWA, ONTARIO K1N 6N5
TEL: 453-1111 FAX: 453-1112

Professional Engineer

WAYNE BENNETT P.ENG.

1000 UNIVERSITY AVENUE, SUITE 1000
OTTAWA, ONTARIO K1N 6N5
TEL: 453-1111 FAX: 453-1112

At-Built Notes:

1. Scale information taken from: Neil Levoic Engineering Ltd (2000)
2. This plan supersedes in whole or in part plan #855,899,455,465,466,467,468,469,470,471,472,473,474,475,476,477,478,479,480,481,482,483,484,485,486,487,488,489,490,491,492,493,494,495,496,497,498,499,500,501,502,503,504,505,506,507,508,509,510,511,512,513,514,515,516,517,518,519,520,521,522,523,524,525,526,527,528,529,530,531,532,533,534,535,536,537,538,539,540,541,542,543,544,545,546,547,548,549,550,551,552,553,554,555,556,557,558,559,560,561,562,563,564,565,566,567,568,569,570,571,572,573,574,575,576,577,578,579,580,581,582,583,584,585,586,587,588,589,590,591,592,593,594,595,596,597,598,599,600,601,602,603,604,605,606,607,608,609,610,611,612,613,614,615,616,617,618,619,620,621,622,623,624,625,626,627,628,629,630,631,632,633,634,635,636,637,638,639,640,641,642,643,644,645,646,647,648,649,650,651,652,653,654,655,656,657,658,659,660,661,662,663,664,665,666,667,668,669,670,671,672,673,674,675,676,677,678,679,680,681,682,683,684,685,686,687,688,689,690,691,692,693,694,695,696,697,698,699,700,701,702,703,704,705,706,707,708,709,710,711,712,713,714,715,716,717,718,719,720,721,722,723,724,725,726,727,728,729,730,731,732,733,734,735,736,737,738,739,740,741,742,743,744,745,746,747,748,749,750,751,752,753,754,755,756,757,758,759,760,761,762,763,764,765,766,767,768,769,770,771,772,773,774,775,776,777,778,779,780,781,782,783,784,785,786,787,788,789,790,791,792,793,794,795,796,797,798,799,800,801,802,803,804,805,806,807,808,809,810,811,812,813,814,815,816,817,818,819,820,821,822,823,824,825,826,827,828,829,830,831,832,833,834,835,836,837,838,839,840,841,842,843,844,845,846,847,848,849,850,851,852,853,854,855,856,857,858,859,860,861,862,863,864,865,866,867,868,869,870,871,872,873,874,875,876,877,878,879,880,881,882,883,884,885,886,887,888,889,890,891,892,893,894,895,896,897,898,899,900,901,902,903,904,905,906,907,908,909,910,911,912,913,914,915,916,917,918,919,920,921,922,923,924,925,926,927,928,929,930,931,932,933,934,935,936,937,938,939,940,941,942,943,944,945,946,947,948,949,950,951,952,953,954,955,956,957,958,959,960,961,962,963,964,965,966,967,968,969,970,971,972,973,974,975,976,977,978,979,980,981,982,983,984,985,986,987,988,989,990,991,992,993,994,995,996,997,998,999,1000
3. While illustrations and utilities shown are taken from the existing conditions, they cannot be guaranteed.
4. The actual work line was recorded during construction of the existing sewer & watermain.
5. Boreholes taken prior to construction.
6. See typical cross sections for road structure materials depths.
7. Ministry of Environment Approval # Sewer: 0279-546-CT Water: 5281-546-JSU

Legal Survey Notes:

This plan has been prepared and calculated from the original data and not based on an actual survey. The original data was provided by the owner, and the accuracy of the information is not guaranteed. The original data was provided by the owner, and the accuracy of the information is not guaranteed. The original data was provided by the owner, and the accuracy of the information is not guaranteed.

Ottawa

Transportation, Utilities and Public Works
Infrastructure Services Branch

10 LAUREL AVENUE WEST, OTTAWA, ONTARIO K1P 1B7

Rosemarie Leclair
Branch Director

Richard Hewitt, P.Eng.
Branch Director

ARLINGTON AVENUE

STA.370.00 TO STA.480.00

Station	Top of Watermain	Existing Surface
370.00	73.00	73.00
375.00	72.00	72.00
380.00	71.00	71.00
385.00	70.00	70.00
390.00	69.00	69.00
395.00	68.00	68.00
400.00	67.00	67.00
405.00	66.00	66.00
410.00	65.00	65.00
415.00	64.00	64.00
420.00	63.00	63.00
425.00	62.00	62.00

02-3335

3335

10/20/2011

11:59

12/14/24

**PROPOSED
FOUR STOREY APARTMENT BUILDING SITE
LOT 15 WEST CAMBRIDGE STREET NORTH
SOUTH OF STONE BOUNDARY
R-PLAN 33
370 CAMBRIDGE STREET NORTH
CITY OF OTTAWA**

APPENDIX D

CITY OF OTTAWA

- **ELEVATION CONTOURS (FIGURE 1)**
- **SITE SURVEY PLAN**
- **SITE PLAN AND ARCHITECTURAL DRAWINGS**
- **OFM FIRE FLOW CALCULATIONS**
- **OFM EXPOSURE DISTANCES (FIGURE 2)**
- **WATER DATA BOUNDARY CONDITIONS**
- **SUPPORTING HYDRAULIC CALCULATIONS**
- **HYDRANT SPACING (FIGURE 3)**

ATTACHMENT 1 : FIGURE 1 – ELEVATION CONTOURS

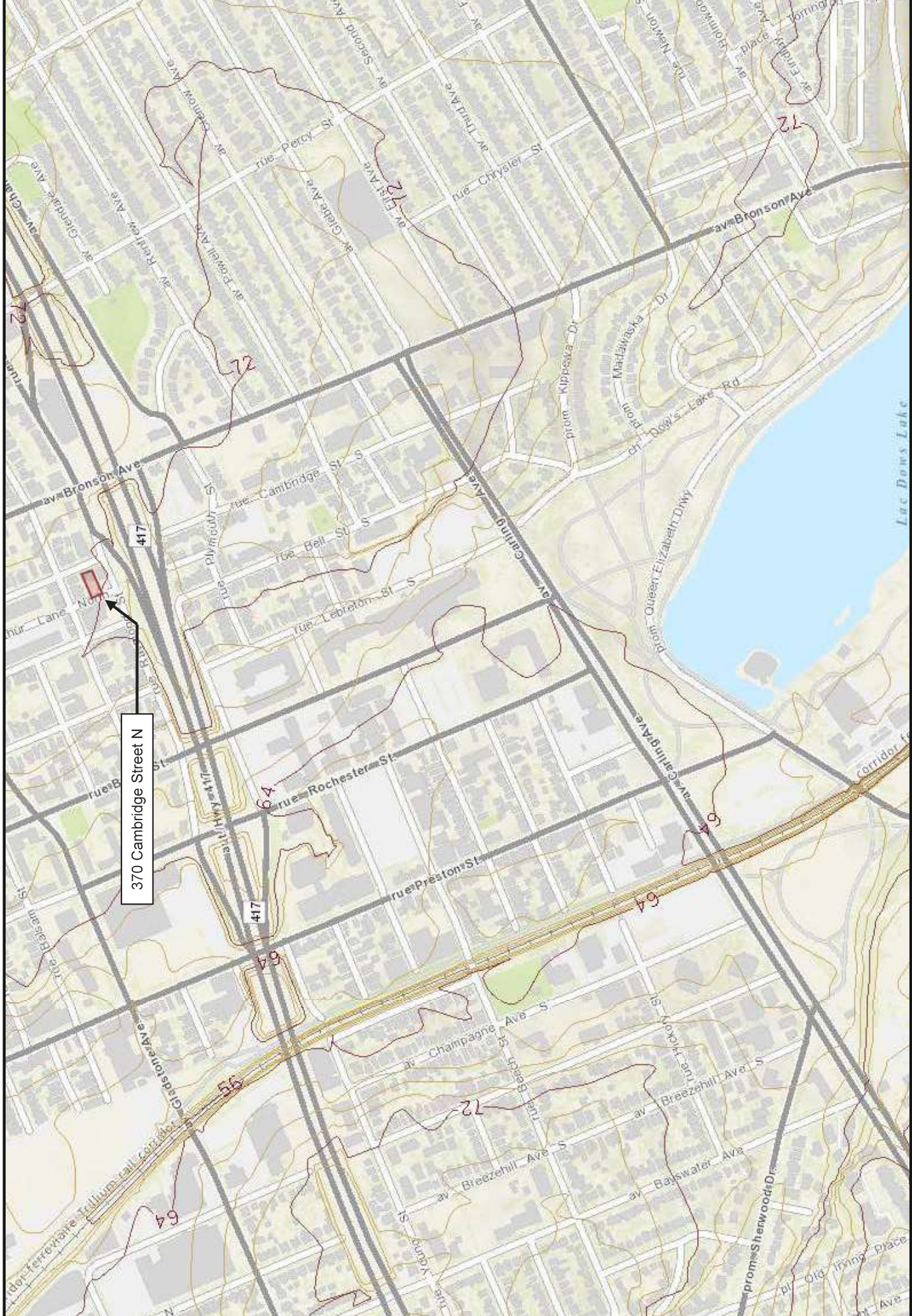


Figure 1: Elevation Contours

Source: geoOttawa 2022; Contains information licensed under the Open Government Licence – City of Ottawa.

ATTACHMENT 2 : SITE SURVEY PLAN

UTILITY NOTES

1. This drawing is based on information provided by the utility companies. The utility companies are responsible for the accuracy of the information provided.
2. The utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
3. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
4. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
5. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.

ELEVATION NOTES

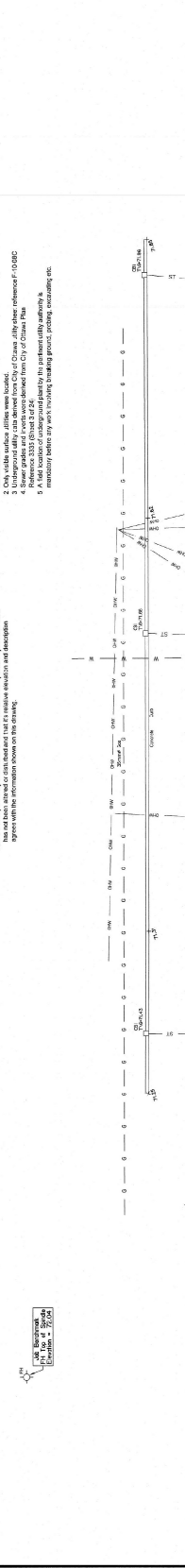
1. Elevation notes are provided for the building and are based on the existing ground elevations shown on this drawing.
2. The utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
3. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
4. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.
5. Utility lines shown on this drawing are for informational purposes only. They do not constitute a warranty of any kind.

NOTES & LEGEND

- Survey Instrument Used
- Survey Date
- Surveyor
- Scale
- Notes

NOTES & LEGEND

Survey Instrument Used
 Survey Date
 Surveyor
 Scale
 Notes



NOTES & LEGEND

Survey Instrument Used
 Survey Date
 Surveyor
 Scale
 Notes

UTILITY NOTES

1. This drawing is based on information provided by the utility companies. The utility companies are responsible for the accuracy of the information provided.

ELEVATION NOTES

1. Elevation notes are provided for the building and are based on the existing ground elevations shown on this drawing.

NOTES & LEGEND

Survey Instrument Used
 Survey Date
 Surveyor
 Scale
 Notes

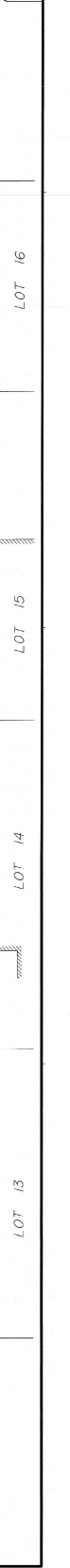


NOTES & LEGEND

Survey Instrument Used
 Survey Date
 Surveyor
 Scale
 Notes

REGISTERED PLAN

1. This drawing is based on information provided by the utility companies. The utility companies are responsible for the accuracy of the information provided.



REGISTERED PLAN

1. This drawing is based on information provided by the utility companies. The utility companies are responsible for the accuracy of the information provided.

REGISTERED PLAN

1. This drawing is based on information provided by the utility companies. The utility companies are responsible for the accuracy of the information provided.

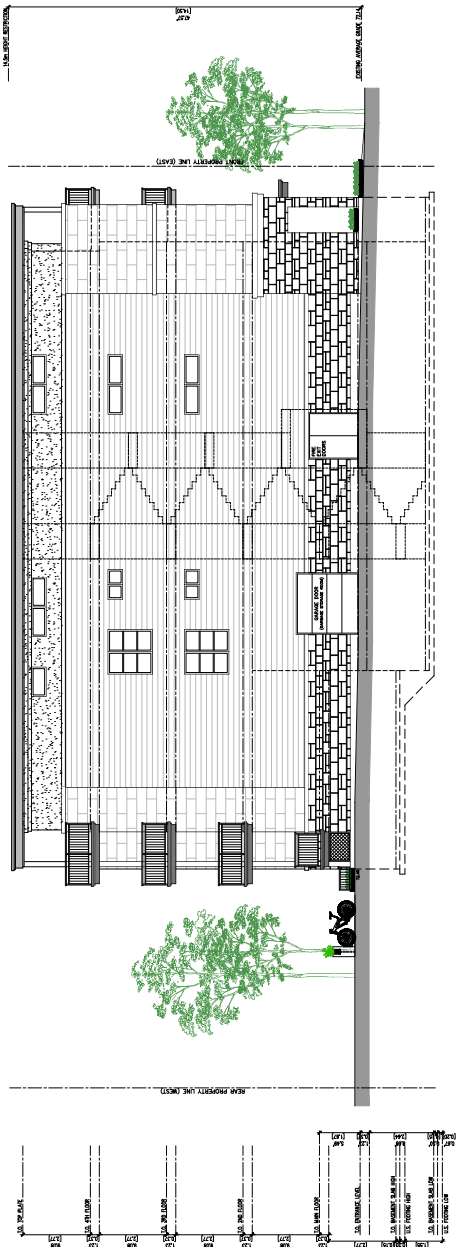
ATTACHMENT 3 : SITE PLAN AND ARCHITECTURAL DRAWINGS

PRELIMINARY CONCEPT FOR REVIEW & COMMENTS

MD
MIRROA DESIGN
 INCORPORATED, SINCE 1988
 CUSTOM HOME DESIGN
 PROJECT MANAGEMENT
 150 COURTOURSE DATE
 UNIT #27
 OTTAWA, ONTARIO
 K2E 7V7
 TEL: 613-274-2553
 FAX: 613-274-7085
 CONTACT: @MIRROA.DESIGN.COM
 WWW.MIRROA.DESIGN.COM



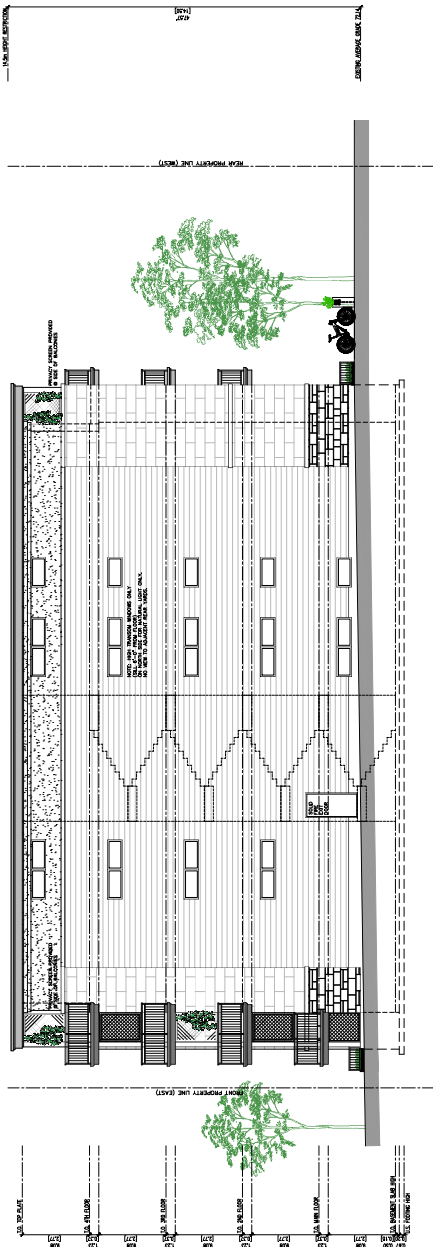
FRONT ELEVATION (EAST)
 SCALE: 1/8" = 1'-0"



LEFT SIDE ELEVATION (SOUTH)
 SCALE: 1/8" = 1'-0"



REAR ELEVATION (WEST)
 SCALE: 1/8" = 1'-0"



RIGHT SIDE ELEVATION (NORTH)
 SCALE: 1/8" = 1'-0"

GENERAL NOTES:
 1. THIS DRAWING IS A PRELIMINARY CONCEPT FOR REVIEW AND COMMENTS TO THE DEVELOPER.
 2. ALL WORK AND MATERIALS TO BE IN ACCORDANCE WITH THE CITY OF OTTAWA'S BUILDING BY-LAW AND THE NATIONAL BUILDING CODE OF CANADA.
 3. FLOOR AREAS INDICATED ARE APPROXIMATE AND SUBJECT TO VERIFICATION BY THE ARCHITECT.
 4. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED.
 5. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED.

NO.	DESCRIPTION & DATE	REVISIONS
1		

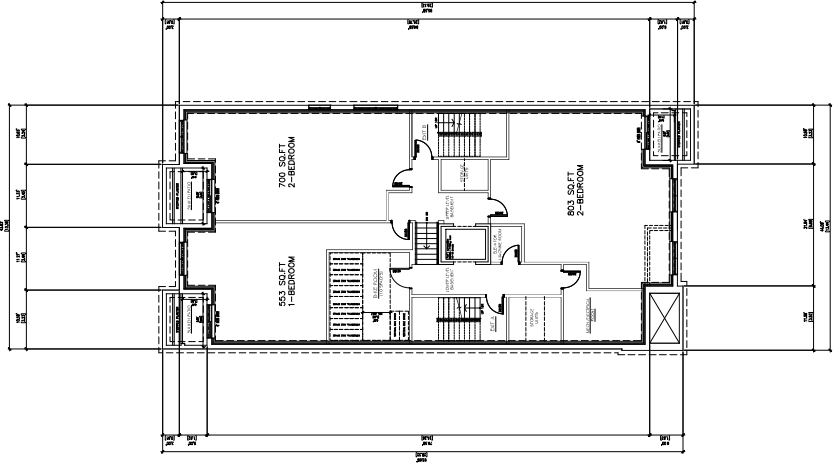
SHEET TITLE:
 150 COURTOURSE STREET NORTH
 OTTAWA, ON
 PROPOSED 4-STORY, LOW-RISE
 APARTMENT BUILDING

SHEET TITLE:
 PROPOSED ELEVATIONS

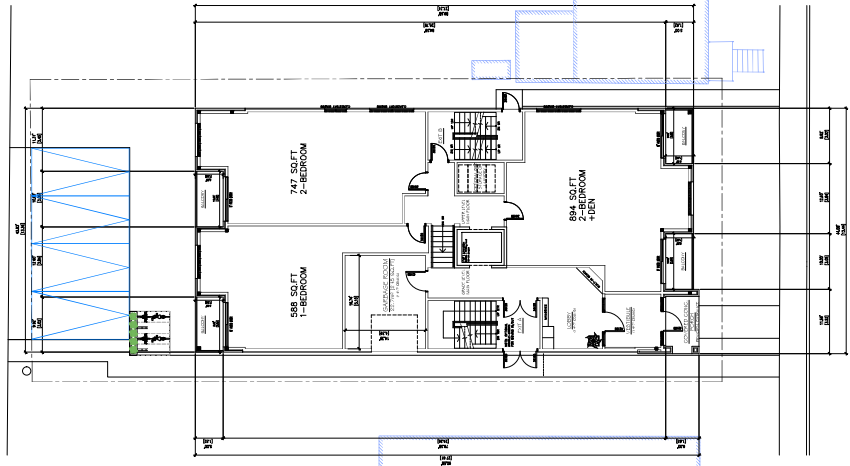
DWG. NO.	A1.2
DATE	DECEMBER 2021
DESIGNED BY	
CHECKED BY	
DATE	
PRINT DATE	

PRELIMINARY CONCEPT FOR REVIEW & COMMENTS

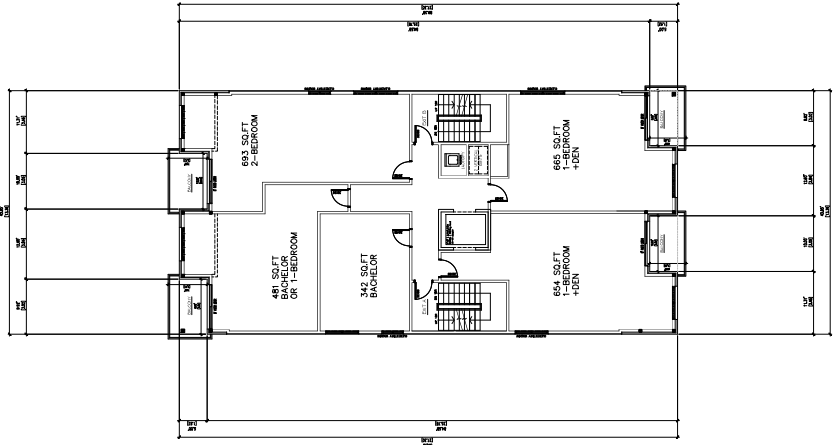
MD
MIRROA DESIGN
 INCORPORATED SINCE 1988
 CUSTOM HOME DESIGN
 PROJECT MANAGEMENT
 150 COURSE DATE
 UNIT #7
 OTTAWA, ONTARIO
 K2E 7V7
 TEL: 613-274-2553
 FAX: 613-274-7085
 CONTACT@MIRROADESIGN.COM
 WWW.MIRROADESIGN.COM



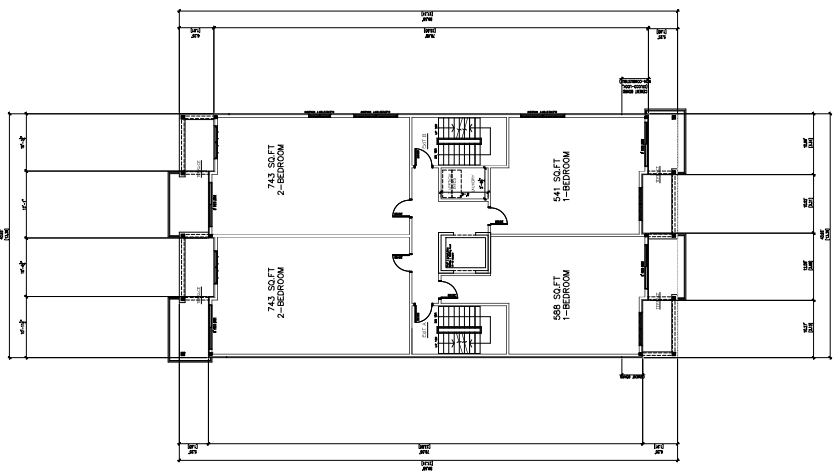
BASEMENT PLAN
 SCALE: 3/32" = 1'-0"



GROUND FLOOR PLAN
 SCALE: 3/32" = 1'-0"



2-3 FLOOR PLAN
 SCALE: 3/32" = 1'-0"



4TH FLOOR PLAN
 SCALE: 3/32" = 1'-0"

GENERAL NOTES:
 1. THIS DRAWING IS A PRELIMINARY CONCEPT FOR REVIEW ONLY. IT IS NOT TO BE USED FOR PERMITS OR CONTRACTS WITHOUT THE CONSULTANT'S APPROVAL.
 2. ALL WORK AND MATERIALS TO BE IN ACCORDANCE WITH THE 2018 NATIONAL BUILDING CODE.
 3. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED.
 4. SEE NOTES ON SHEET 3 FOR FINISHES AND MATERIALS.
 5. ALL WORK SHALL BE IN ACCORDANCE WITH THE 2018 NATIONAL BUILDING CODE.
 6. SEE NOTES ON SHEET 4 FOR FINISHES AND MATERIALS.

NO.	DESCRIPTION & DATE	REVISIONS
1		

APP TITLE:
 370 CAMBRIDGE STREET NORTH
 PROPOSED 4-STORY LEASIBLE
 APARTMENT BUILDING

SHEET TITLE:
 PROPOSED FLOOR PLANS

SCALE: AS SHOWN DWG. NO.
 DRAWN: N.D.
 CHECKED:
 DATE: DECEMBER 2021

A2.2
 PRINT DATE:

ATTACHMENT 4 : OFM FIRE FLOW CALCULATIONS



OFM Fire Flow Calculation

Calculations based on *Fire Protection Water Supply Guideline for Part 3 in the Ontario Building Code* by the Office of the Fire Marshal (OFM 1999)

Stantec Project #: 163401084
 Project Name: 370 Cambridge St. N
 Date: May 19, 2022
 Data inputted by: Alexandre Mineault-G., M.A.Sc., ing., P.Eng.
 Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

Fire Flow Calculation #: 1
 Building Type/Description/Name: Residential

Table A: Office of the Fire Marshal Determination of Required Fire Protection Water Supply

Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit		
1 General Building Details									
1.1	Enter Number of Storeys	Number of Floors/Storeys in the Unit (incl. basement):			5	5	Storeys		
1.2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Single Family	0	Other (Comm, Ind, Apt etc.)	20	Units		
			Townhouse - indicate # of units	0					
			Other (Comm, Ind, Apt etc.)	20					
1.3	Choose Presence of Sprinklers	Sprinklers?			Yes	Yes	N/A		
1.4	Choose Presence of Firewalls	Firewall separations?			None	None	N/A		
1.5	Choose Presence of Stand-Pipe System	Stand-pipe system?			None	None	N/A		
2 Determining Water Supply Coefficient K									
2.1	Choose Type of Construction	Type of Construction	Type of Construction					N/A	N/A
			Non-combustible construction + fire separations + fire-resistance ratings in accordance with Section 3.2.2 of OBC	Type I	Type IV				
			Non-combustible construction + fire separations + no fire-resistance rating	Type II					
			Combustible construction + fire separations + fire-resistance ratings in accordance with Section 3.2.2 of OBC	Type III					
			Combustible construction + fire separations + no fire-resistance rating	Type IV					
2.2	Choose Classification	Occupancy Classification (OBC)	Building Classification					N/A	
			A-2, B-1, B-2, B-3, C, D	23	C	A-2, B-1, B-2, B-3, C, D			
			A-4, F-3	28					
			A-1, A-3	32					
			E, F-2	39					
F-1	53								
2.3	Water Supply Coefficient (K)	Water Supply Coefficient K			23	23	N/A		
3 Determining Building Volume V									
3.1	Enter Ground Floor Area of One Unit	Floor Space Area					340	Area in Square Meters (m ²)	
		Average Floor Area (A) :	340	Square Metres (m ²)					
3.2	Building Height (h)	Building Height					15.7	Height in Meters (m)	
		Bottom Elevation :	0.0	Meters (m)					
		Top Elevation :	15.7	Meters (m)					
3.3	Building Volume (V)	Building Volume V = A * h			5,340	5,340	Volume in Meters Cube (m ³)		
4 Determining Spatial Coefficient S									
4.1	Choose Exposure Distances from Building to Property Line	Exposure Distance from Building to Property Line in Meters (m)	North Side		0.50	1.22	Distance in Meters (m)		
			Property Line to Street Centreline (Street Facing)	1.5					
			Total Exposure Distance	0					
			East Side		0.00				
			Property Line to Street Centreline (Street Facing)	1.5					
			Total Exposure Distance	9.8					
			South Side		0.50				
			Property Line to Street Centreline (Street Facing)	1.5					
			Total Exposure Distance	0					
West Side		0.22							
Property Line to Street Centreline (Street Facing)	4.5								
Total Exposure Distance	3.3								
4.2	Total Spatial Coefficient	Total Spatial Coefficient S _{tot} = 1 + Σ S _x			2.00	2.00	N/A		
5 Determining Required Minimum Supply of Water Q and Fire Flow									
5.1	Obtain Required Fire Volume, Flow & Duration	<i>Minimum Supply of Water, rounded to nearest 1,000 L; Q = K*V*S_{tot}</i>				246,000 L			
		<i>Required Minimum Water Supply Flow Rate (L/min)</i>				6,300 L/min			
		<i>Required Minimum Water Supply Flow Rate (L/s)</i>				105 L/s			
		<i>Required Minimum Duration of Fire Flow (min)</i>				40 min			

ATTACHMENT 5 : FIGURE 2 – OFM EXPOSURE DISTANCES

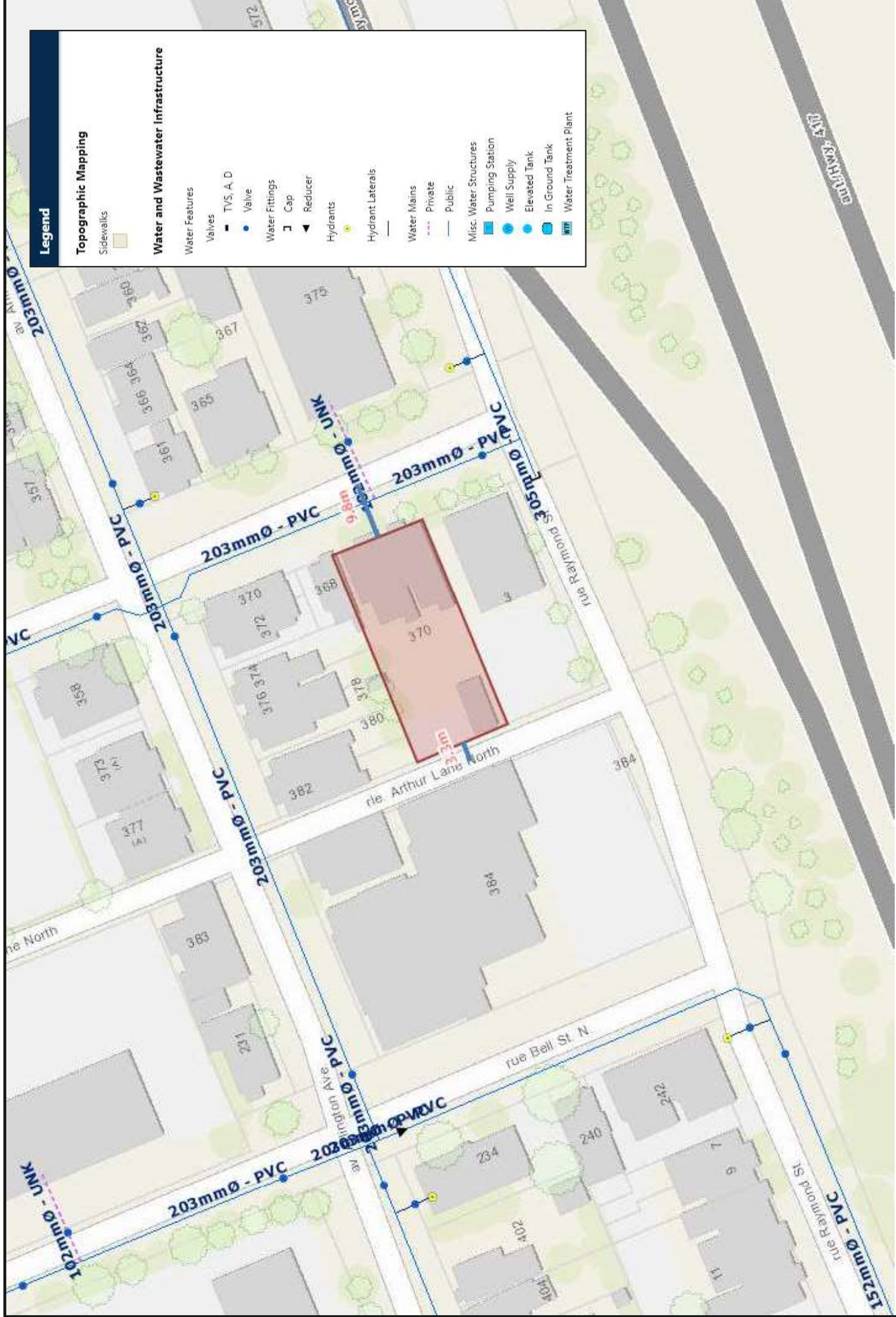


Figure 2: OFM Exposure Distances (Property Line to Street)

Source: geoOttawa 2022; Contains information licensed under the Open Government Licence – City of Ottawa.

ATTACHMENT 6 : WATER DATA BOUNDARY CONDITIONS

Mineault-Guitard, Alexandre

From: TL MaK <tlmakecl@bellnet.ca>
Sent: Thursday, May 26, 2022 10:35 AM
To: Alemany, Kevin
Cc: Mineault-Guitard, Alexandre
Subject: RE: 370 Cambridge Street North - Water Boundary Conditions Request
Attachments: 370 Cambridge Street North May 2022.pdf

Hi Kevin,

Attached please find water boundary conditions received today from the City of Ottawa regarding 370 Cambridge Street North.

Could you please proceed with your calculations at your earliest convenience for our serviceability report preparation.

Let us know if you have any questions or comments.

Regards,

Tony Mak

T.L. Mak Engineering Consultants Ltd.
1455 Youville Drive, Suite 218
Ottawa, ON. K1C 6Z7
Tel. 613-837-5516 | Fax: 613-837-5277
E-mail: tlmakecl@bellnet.ca

From: Bakhit, Reza [<mailto:reza.bakhit@ottawa.ca>]
Sent: May 26, 2022 8:46 AM
To: TL MaK
Subject: RE: 370 Cambridge Street North - Water Boundary Conditions Request

Hi Tony,

The following are boundary conditions, HGL, for hydraulic analysis at 370 Cambridge Street N (zone 1W) assumed to be connected to the 203 mm watermain on Cambridge Street North (see attached PDF for location).

Minimum HGL: 107.0 m

Maximum HGL: 115.2 m

Max Day + Fire Flow (105 L/s): 108.8 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions.

The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Regards,

Reza Bakhit, P.Eng, C.E.T

Project Manager

Planning, Real Estate and Economic Development Department / Direction générale de la planification, des biens immobiliers et du développement économique

Development Review - Central Branch

City of Ottawa | Ville d'Ottawa

110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1

613.580.2424 ext./poste 19346, reza.bakhit@ottawa.ca

Please note: Given the current pandemic, I will be working from home until further notice; reaching me by email is the easiest. I will be checking my voicemail, just not as frequently as I normally would be.

From: TL MaK <tlmakecl@bellnet.ca>

Sent: Thursday, May 19, 2022 5:28 PM

To: Bakhit, Reza <reza.bakhit@ottawa.ca>

Subject: RE: 370 Cambridge Street North - Water Boundary Conditions Request

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Hi Reza,

Regarding this site, we are requesting for water boundary conditions from the City of Ottawa to be provided for our hydraulic analysis. The particulars are as follows:

The proposed building is located within Pressure Zone 1W at 370 Cambridge Street N is a 4-storey residential apartment building with a basement. The building contains twenty (20) total units, namely two (2) bachelor units, ten (10) 1-bedroom units, six (6) 2-bedroom units, and two (2) 3-bedroom units. The average floor area is approximately 338 m², for a total gross floor area of approximately 1,352 m² (excluding the basement). The building is to be serviced by the 200 mm diameter watermain along Cambridge Street N.

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines, where the residential consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 2.5. Peak hour (PKHR) demands were calculated by multiplying MXDY by a factor of 2.2. Persons per unit (PPU) for each unit were estimated based on the City of Ottawa's Water Design Guidelines. **Table 1** shows the estimated domestic demands of the proposed building.

Table 1: Estimated Domestic Demand

Unit Type	Unit Count	PPU	Consumption Rate (L/c/d)	AVDY		MXDY		PKHR	
				L/d	L/s	L/d	L/s	L/d	L/s
Apartment, Bachelor	2	1.4	280	784	0.01	1,960	0.02	4,312	0.05

Apartment, 1-Bedroom	10	1.4	3,920	0.05	9,800	0.11	21,560	0.25
Apartment, 2-Bedroom	6	2.1	3,528	0.04	8,820	0.10	19,404	0.22
Apartment, 3-Bedroom	2	3.1	1,736	0.02	4,340	0.05	9,548	0.11
Total	20		9,968	0.12	24,920	0.29	54,824	0.63

The City had previously indicated that the City's Fire Marshall and various City departments are currently reviewing fire flow requirements for low- and mid-rise buildings. As per the City of Ottawa's Water Design Guidelines, the FUS method is to be used for fire flow requirements affecting watermain sizing; with regards to fire protection on private property and not requiring new watermains, these are covered by the Ontario Building Code (OBC).

As such, the fire flow requirement was also determined following the OBC's Office of the Fire Marshal (OFM) method. The proposed building will be of wood frame construction, where floors are fire separations, but without fire-resistance ratings. The resulting total required fire flow (RFF) for a sprinklered building is 6,300 L/min (105 L/s) for a minimum duration of 40 min.

In summary:

- AVDY = 9,968 L/d (0.12 L/s);
- MXDY = 24,920 L/d (0.29 L/s);
- PKHR = 54,824 L/d (0.63 L/s); and
- Fire Flow = 6,300 L/min (105 L/s).

The City is requested to provide boundary conditions for the Average Day, Maximum Day, Peak Hour and Fire Flow conditions indicated above.

Thank you for your prompt attention to this matter. Please forward the boundary conditions as soon as possible.

Have a good day.

Regards,

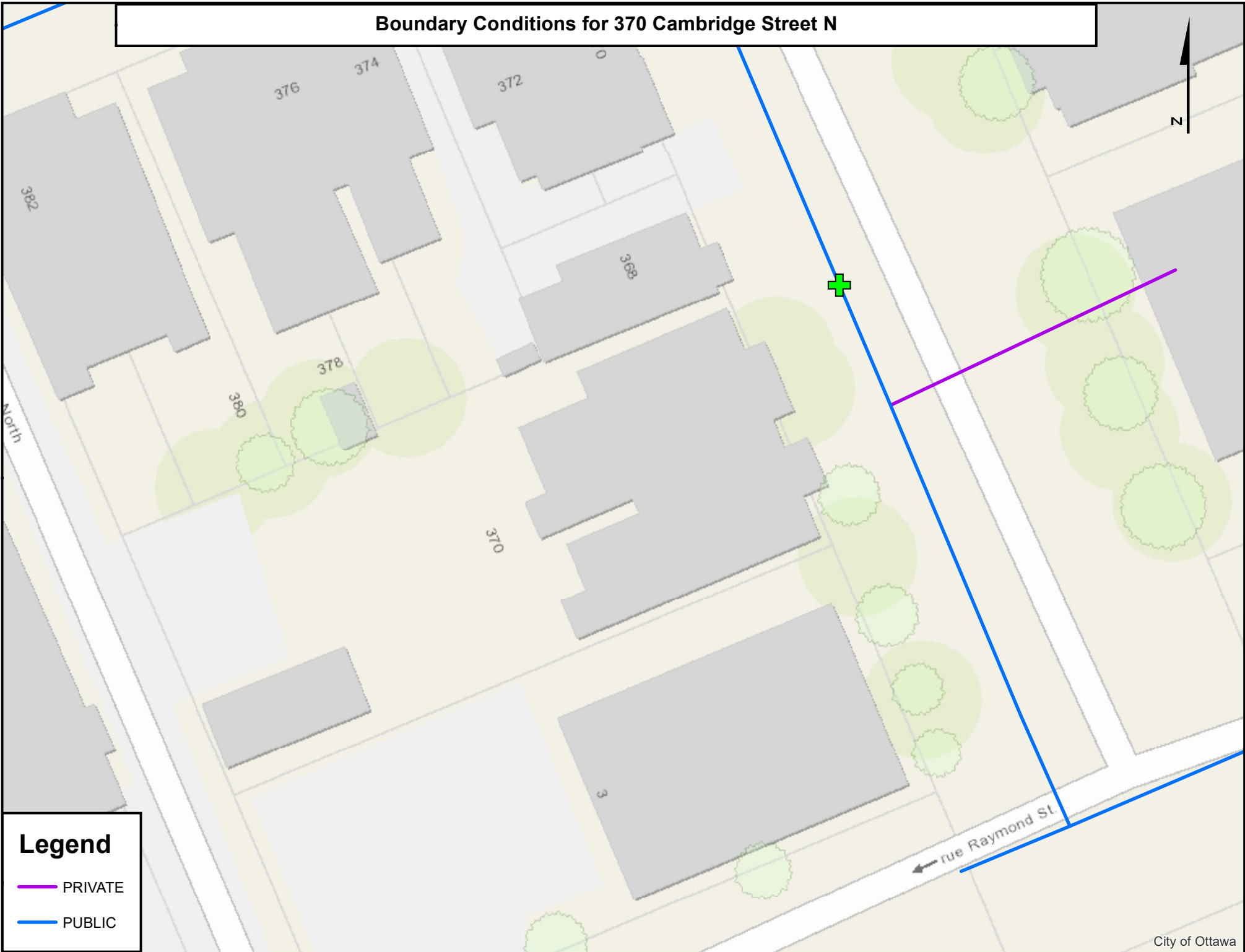
Tony Mak

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Boundary Conditions for 370 Cambridge Street N



Legend

- PRIVATE
- PUBLIC

ATTACHMENT 7 : SUPPORTING HYDRAULIC CALCULATIONS



Supporting Hydraulic Calculations

Stantec Project #: 163401084

Project Name: 370 Cambridge Street N

Date: May 27, 2022

Data inputted by: Alexandre Mineault-G, M.A.Sc., ing., P.Eng.

Data reviewed by: Kevin Alemany, M.A.Sc., P.Eng.

Boundary Conditions provided by the City:

Scenario 1: Peak Hour (Min HGL): 107.0 m;

Scenario 2: Average Day (Max HGL): 115.2 m; and

Scenario 3: Maximum Day plus Fire Flow: 108.8 m.

Sample Calculations

$$HGL (m) = hp + hz \quad (1)$$

where: hp = Pressure Head (m); and hz = Elevation Head (m), estimated from topography.

For Scenario 1, we have:

$$HGL(m) = 107 \text{ and } hz (m) = 71.7.$$

Rearranging Equation 1, we can calculate the Pressure Head (hp) as follow:

$$hp (m) = HGL - hz$$

$$\therefore hp = 107.0 - 71.7 \text{ m} = 35.3 \text{ m}.$$

To convert from Pressure Head (m) to a pressure value (kPa), the following equation can be used:

$$P (\text{kPa}) = (\rho * g * hp) / 1000 \quad (2)$$

where: ρ = density of water = 1000 kg/m³; and g = gravitational acceleration = 9.81 m/s².

Using Equation 2, we can calculate the Pressure (P) as follow:

$$P (\text{kPa}) = (1000 * 9.81 * 35.3) / 1000$$

$$\therefore P = 346 \text{ kPa}.$$

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

$$P = 50 \text{ psi}.$$

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows:

Scenario 2: P = 62 psi; and Scenario 3: P = 53 psi.

To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 346 kPa (50 psi)
Scenario 2: Maximum Pressure under Average Day Demand: 426 kPa (62 psi)
Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 364 kPa (53 psi)

ATTACHMENT 8 : FIGURE 3 – HYDRANT SPACING

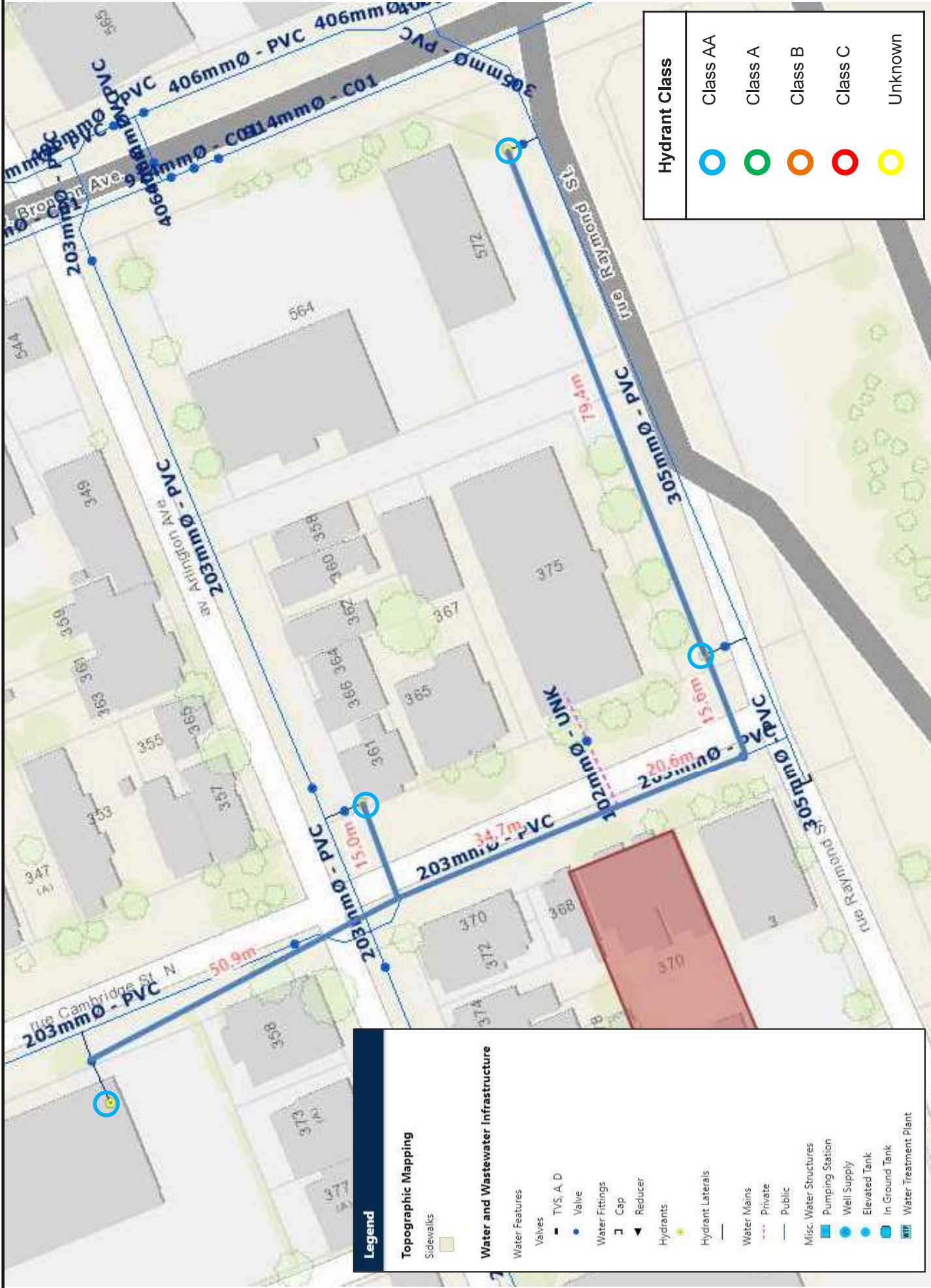


Figure 3: Hydrant Spacing

Source: geoOttawa 2022; Contains information licensed under the Open Government Licence – City of Ottawa.

PROPOSED
FOUR STOREY APARTMENT BUILDING SITE
LOT 15 WEST CAMBRIDGE STREET NORTH
SOUTH OF STONE BOUNDARY
R-PLAN 33
370 CAMBRIDGE STREET NORTH
CITY OF OTTAWA

APPENDIX E
CITY OF OTTAWA
SANITARY SEWER DESIGN SHEET
SHEET No. 1 OF 1

SANITARY SEWER DESIGN SHEET

$$M = 1 + \frac{14}{4 + \sqrt{p}} \sqrt{k} \text{ where } P = \text{population in } 1000's$$

$$K = 2.08$$

$$Q(p) = PqM \text{ (L/s)}$$

$$86.4$$

$$Q(l) = IA \text{ (L/s) where } A = \text{area in hectares}$$

$$Q(d) = Q(p) + Q(l) \text{ (L/s)}$$

q = average daily per capita flow (200 L/cap. d)
 I = unit of peak extraneous flow (0.33 L/ha. s)
 M = peaking factor 4.0 (MAX)
 $Q(p)$ = peak population flow (L/s)
 $Q(l)$ = peak extraneous flow (L/s)
 $Q(d)$ = peak design flow

LOCATION		INDIVIDUAL		CUMULATIVE		Peaking factor M	Pop. flow Q(p) (L/s)	Peak extraneous flow Q(l) (L/s)	Peak design flow Q(d) (L/s)	Length (m)	Pipe size (mm)	Type of pipe	Grade %	Capacity (L/s) n=0.013	Full flow velocity (m/s)	Actual velocity at Q(d)
		Area A (hectares)	Pop.	Area A (hectares)	Pop.											
STREET	FROM	TO														
#370 CAMBRIDGE STREET NORTH	SITE	EX-3000 COMBINED SEWER	35.6	0.063	35.6	0.063	0.42	0.02	0.44	14.5	150	PVC	1.0	19.8	1.12	



DESIGN: TLM	PROJECT: 370 CAMBRIDGE STREET NORTH	SHEET No. 1 of 1
CHECKED: TLM	PROPOSED FOUR STOREY APARTMENT	
DATE: JULY 2022	BUILDING SITE - CITY OF OTTAWA	

FILE# 821-155

**PROPOSED
FOUR STOREY APARTMENT BUILDING SITE
LOT 15 WEST CAMBRIDGE STREET NORTH
SOUTH OF STONE BOUNDARY
R-PLAN 33
370 CAMBRIDGE STREET NORTH
CITY OF OTTAWA**

**APPENDIX F
DEVELOPMENT SERVICING STUDY CHECKLIST SUMMARY**

Servicing study guidelines for development applications

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

- Executive Summary (for larger reports only).
- Date and revision number of the report.
- Location map and plan showing municipal address, boundary, and layout of proposed development.
- Plan showing the site and location of all existing services.
- Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
- Summary of Pre-consultation Meetings with City and other approval agencies.
- Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.
- Statement of objectives and servicing criteria.
- Identification of existing and proposed infrastructure available in the immediate area.
- Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).
- Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
- Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
- Proposed phasing of the development, if applicable.

- Reference to geotechnical studies and recommendations concerning servicing.

- All preliminary and formal site plan submissions should have the following information:
 - Metric scale

 - North arrow (including construction North)

 - Key plan

 - Name and contact information of applicant and property owner

 - Property limits including bearings and dimensions

 - Existing and proposed structures and parking areas

 - Easements, road widening and rights-of-way

 - Adjacent street names

4.2 Development Servicing Report: Water

- Confirm consistency with Master Servicing Study, if available
- Availability of public infrastructure to service proposed development
- Identification of system constraints
- Identify boundary conditions
- Confirmation of adequate domestic supply and pressure
- Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
- Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
- Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
- Address reliability requirements such as appropriate location of shut-off valves
- Check on the necessity of a pressure zone boundary modification.
- Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range

- Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
- Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
- Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
- Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.

4.3 Development Servicing Report: Wastewater

- Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
- Confirm consistency with Master Servicing Study and/or justifications for deviations.
- Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
- Description of existing sanitary sewer available for discharge of wastewater from proposed development.
- Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
- Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
- Description of proposed sewer network including sewers, pumping stations, and forcemains.
- Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).
- Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
- Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
- Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
- Special considerations such as contamination, corrosive environment etc.

4.4 Development Servicing Report: Stormwater Checklist

- Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)
- Analysis of available capacity in existing public infrastructure.
- A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
- Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
- Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
- Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
- Set-back from private sewage disposal systems.
- Watercourse and hazard lands setbacks.
- Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
- Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.
- Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
- Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
- Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
- Any proposed diversion of drainage catchment areas from one outlet to another.
- Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
- If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.
- Identification of potential impacts to receiving watercourses
- Identification of municipal drains and related approval requirements.
- Descriptions of how the conveyance and storage capacity will be achieved for the development.
- 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.

- Inclusion of hydraulic analysis including hydraulic grade line elevations.
- Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
- Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
- Identification of fill constraints related to floodplain and geotechnical investigation.

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

- Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.
- Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
- Changes to Municipal Drains.
- Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)

4.6 Conclusion Checklist

- Clearly stated conclusions and recommendations
- Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
- All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario