

265 CATHERINE STREET SERVICING AND STORMWATER MANAGEMENT REPORT

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Prepared for: 11034936 Canada Inc.

Prepared by: Stantec Consulting Ltd.

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Mirchaelms

Signature

Prepared by:

Reviewed by:

Michael Wu, EIT

Printed Name



Signature

Dustin Thiffault, P.Eng

Printed Name

mile

Approved by:

Signature

Kris Kilborn

Printed Name



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1 Introduction

Stantec Consulting Ltd. has been commissioned by 11034936 Canada Inc. to prepare the following Servicing and Stormwater Management Report in support of a Site Plan Control application for the proposed development located at 265 Catherine Street in the City of Ottawa.

The site is 1.0 ha in area and is situated along the north side of Catherine Street, the south side of Arlington Avenue, the west side of Kent Street, and the east side of Lyon Street North. The site is currently zoned GM [1875] S271 and consists of the former Greyhound bus terminal that has now been demolished. The site is bounded by Catherine Street to the south, Kent Street to the east, Lyon Street North to the west, and Arlington Avenue to the north, as shown in **Figure 1.1** below.

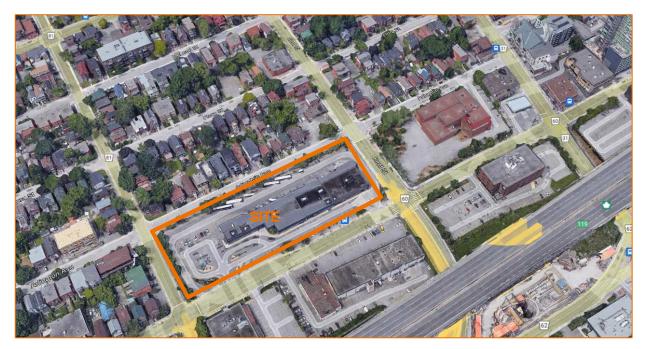


Figure 1.1: Key Plan of Site

The proposed 1.0 ha site will be developed in two phases, with the first phase consisting of a 26-storey residential high-rise with a six-storey podium, while the second phase will consist of seven 3-storey townhouses and two residential high-rises, one 40-storey and the other 36-storey, with a six-storey podium. A 0.1 ha park is proposed at the northeast of the site. Quadrangle Architects Ltd. has prepared a site plan dated February 12th, 2024, as shown in **Appendix A.1**, while the buildings and unit type breakdown are listed in **Table 1.1** below.



Unit Type	Building A	Building B	Building C	Total
Bachelor	80	87	-	167
One-bedroom	91	169	-	260
One-bedroom with den	69	189	-	258
Two-bedroom	125	245	-	370
Two-bedroom with den	30	-	-	30
Three-bedroom	5	37	-	42
Townhouses	-	-	7	7
Residential Total	400	727	7	1134
Commercial (m ²)	1299.2	1123.8	-	2423

Table 1.1: Unit Type Breakdown

1.1 Objective

This site servicing and stormwater management (SWM) report presents a servicing scheme that is free of conflicts, provides on-site servicing in accordance with City of Ottawa Design Guidelines, and uses the existing municipal infrastructure in accordance with any limitations communicated during consultation with the City of Ottawa staff. Details of the existing infrastructure located within the Catherine Street, Lyon Street North, Kent Street, and Arlington Avenue right of ways (ROW) were obtained from available asbuilt drawings and site topographic survey.

Criteria and constraints provided by the City of Ottawa have been used as a basis for the detailed servicing design of the proposed development. Specific and potential development constraints to be addressed are as follows:

- Potable Water Servicing
 - Estimated water demands to characterize the proposed feed(s) for the proposed development which will be serviced from either the existing 127 mm diameter watermains within the Catherine Street and Kent Street ROWs, or the existing 203 mm diameter watermains within the Lyon Street North and Arlington Avenue ROWs.
 - Watermain servicing for the development is to be able to provide average day and maximum day (including peak hour) demands (i.e., non-emergency conditions) at pressures within the acceptable range of 345 to 552 kPa (50 to 80 psi)
 - Under fire flow (emergency) conditions, the water distribution system is to maintain a minimum pressure greater than 140 kPa (20 psi)
- Wastewater (Sanitary) Servicing
 - Define and size the sanitary service lateral which will be connected to the existing 300 mm diameter combined sewers within the Catherine Street ROW, the 450 mm diameter combined sewer within the Lyon Street North ROW, or the 375 mm diameter combined sewer within the Kent Street ROW.
- Storm Sewer Servicing

- Define major and minor conveyance systems in conjunction with the proposed grading plan.
- Determine the stormwater management storage requirements to meet the allowable release rate for the site.
- Define and size the proposed storm service lateral that will be connected to the existing 525 mm and 600 mm diameter municipal storm sewer within the Arlington Avenue ROW.
- Prepare a grading plan in accordance with the proposed site plan and existing grades.

Drawing SSP-1 illustrate the proposed internal servicing scheme for the site.



2 Background

Documents referenced in preparing of this stormwater and servicing report for the 265 Catherine Street development include:

- *City of Ottawa Sewer Design Guidelines* (SDG), City of Ottawa, October 2012, including all subsequent technical bulletins
- *City of Ottawa Design Guidelines Water Distribution*, City of Ottawa, July 2010, including all subsequent technical bulletins
- Design Guidelines for Drinking Water Systems, Ministry of the Environment, Conservation, and Parks (MECP), 2008
- *Fire Protection Water Supply Guideline* for Part 3 in the Ontario Building Code, Office of the Fire Marshal (OFM), October 2020
- Water Supply for Public Fire Protection, Fire Underwriters Survey (FUS), 2020
- Geotechnical Investigation Proposed Mixed-Use Development, 265 Catherine Street, Ottawa, Ontario, Paterson Group Inc., August 13, 2021



3 Water Servicing

3.1 Background

The proposed building is in Pressure Zone 1W of the City of Ottawa's Water Distribution System. The existing watermains along the boundaries of the site consist of a 203 mm diameter ductile iron watermain within Arlington Avenue, 203 mm diameter UCI watermain within Lyon Street North, and 127 mm diameter UCI watermains within Catherine and Kent Streets. There are existing fire hydrants on Arlington Avenue and Catherine Street. According to the Catherine Street as-builts dated 1999 provided by the City, there were plans to upsize the 127 mm diameter watermain to 203 mm, though there are no indications from the infrastructure maps on GeoOttawa or the provided UCC plans that the upsizing has taken place.

3.2 Water Demands

3.2.1 POTABLE (DOMESTIC) WATER DEMANDS

The City of Ottawa Water Distribution Guidelines (July 2010) and ISTB 2021-03 Technical Bulletin were used to determine water demands based on projected population densities for residential areas and associated peaking factors. The population was estimated using an occupancy of 1.4 persons per unit for bachelor and one-bedroom apartments, 2.1 persons per unit for one-bedroom with den and two-bedroom apartments, 3.1 persons per unit for two-bedroom with den and three-bedroom apartments, and 2.7 persons per unit for the townhouses. Based on the unit type breakdown in **Table 1.1**, the proposed building is estimated to have a total population of 2159 persons.

A daily rate of 280 L/cap/day has been used to estimate average daily (AVDY) potable water demand for the residential units, and 28,000 L/ha/day for the commercial areas. Maximum day (MXDY) demands were determined by multiplying the AVDY demands by a factor of 2.5 for residential areas and 1.5 for commercial areas. Peak hourly (PKHR) demands were determined by multiplying the MXDY by a factor of 2.2 for residential areas and 1.8 for commercial areas. The estimated demands for each tower are summarized in **Table 3.1** below and detailed in **Appendix B.1**.

Block/ Building	Comm. Area (m²)	Total Apartment Units	Total Townhome Units	Population	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Α	1299.2	400	0	755	2.5	6.2	13.6
В	1123.8	727	0	1385	4.5	11.3	24.8
С	0	0	7	19	0.1	0.2	0.3
Total (Res)					7.0	17.5	38.5
Total (Com)					0.1	0.1	0.2

Table 3.1: Estimated Water Demands



Total 2423.0 1127 7 2159 7.1 17.6	38.7	17.6	2159	7	1121	2423.0	Total
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3.2.2 FIRE FLOW DEMANDS

Based on the site plan, the fire flow requirement was calculated in accordance with Fire Underwriters Survey (FUS) methodology. Through correspondence with the architect (see **Appendix B.4**), Buildings A and B will be constructed out of cast-in-place concrete, sprinklered, and the vertical separations between the floors will be equipped with all the necessary fire separations required by code.

As such, they were estimated based on a building of non-combustible construction type with two-hour fire rated structural members, and full protections of all vertical openings (one hour fire rating), and the final sprinkler design to conform to the NFPA 13 standard. The gross floor area of the largest floor + 25 % of the gross floor area of two additional floors were used in the FUS calculation for the two high-rises, as per Page 22 of the *Fire Underwriters Survey's Water Supply for Public Fire Protection* (2020).

As for Building C, through correspondence with the architect, it is confirmed that the set of three-storey townhouses would likely be of wood frame construction type and not sprinklered. Thus, the gross floor area of all three floors of the townhouses were used in the FUS calculation.

The worst-case scenario for the fire flow was at Buildings B and C, in which their required fire flows were both determined to be 166.7 L/s (10,000 L/min). Detailed fire flow calculations per the FUS methodology and the supporting FUS exposure sketch are provided in **Appendix B.2**.

3.3 Level of Servicing

3.3.1 BOUNDARY CONDITIONS

The estimated domestic potable water demands, and fire flow demands, were used to define the level of servicing required for the proposed development from the municipal watermain and hydrants within the Catherine Street, Lyon Street North, Kent Street, and Arlington Avenue ROWs.

Discussions with the mechanical consultant and architect indicate that as the towers are to exceed 84 metres in height, the site will be required to be serviced from two separate watermains located on opposite sides of the site for adequate servicing. However, the initial boundary conditions provided in April 2023 indicated that the 127 mm diameter watermains on Catherine Street and Kent Street would not be able to provide the required fire flow. Spurred by discussions with the mechanical consultant and by recently revised boundary conditions (April 2024), it is proposed that the watermain along Kent Street between Arlington and Catherine will need to be upsized to 203 mm to meet the required fire flow, which was met as shown in the revised boundary conditions (See **Appendix B.3** for correspondence).

Table 3.2 below outlines the boundary conditions for the two proposed connections servicing the site in consideration of upsizing of the watermain on Lyon Street. The available fire flow is adequate in meeting the site's worst-case fire flow demands.



Connection	Arlington Street	Kent Street
Min. HGL (m)	106.3	106.3
Max. HGL (m)	115.3	115.3
MXDY+FF (166.7 L/s) (m)	96.7	94.2

Table 3.2: Catherine and Kent Boundary Conditions (200mm Kent Upsize)

3.3.2 ALLOWABLE DOMESTIC PRESSURES

The desired normal operating pressure range in occupied areas as per the City of Ottawa 2010 Water Distribution Design Guidelines is 345 kPa to 552 kPa (50 psi to 80 psi) under a condition of maximum daily flow and no less than 276 kPa (40 psi) under a condition of maximum hourly demand. Furthermore, the maximum pressure at any point in the water distribution should not exceed 689 kPa (100 psi) as per the Ontario Building/Plumbing Code; pressure reducing measures are required to service areas where pressures greater than 552 kPa (80 psi) are anticipated in occupied areas.

The proposed finished floor elevation at the ground floor of 69.0m will serve as the floor elevation for the calculation of residual pressures at ground level. As per the boundary conditions, the on-site pressures are expected to range from 365.7 kPa to 453.9 kPa (53.0 psi to 65.8 psi) under normal operating conditions, which are within the normal operating pressure range defined by the City of Ottawa design guidelines as within 276 kPa to 552 kPa (40 psi to 80 psi). It is anticipated that booster pumps will be required to service the upper floors of the towers.

3.3.3 ALLOWABLE FIRE FLOW PRESSURES

The boundary conditions provided by the City of Ottawa indicate that the upsized watermain within Kent Street is expected to maintain a residual pressure of 25.2m equivalent to 247 kPa (35.8 psi) under the worst-case fire flow conditions. Similarly, the main within Arlington Street is expected to maintain a residual pressure of 27.7m equivalent to 272 kPa (39.4 psi). This demonstrates that with the upsizing and complete looping the watermains and nearby hydrants can provide the required fire flows while maintaining a residual pressure of 20 psi.

3.3.4 FIRE HYDRANT COVERAGE

The building will be sprinklered and two Siamese (fire department) connections are to be provided, one each at the main entrances along Catherine Street for each phase. There are five hydrants in the proximity of the proposed development site, as shown in **Figure 3.1**. The distance of each hydrant from the proposed building is less than 115 m. As the Siamese connections are fronting Catherine Street, only the two hydrants along Catherine Street were considered for the hydrant coverage calculations.

According to the NFPA 1 Table 18.5.4.3 in Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02, a hydrant situated less than 76 m away from a building can supply a maximum capacity of 5,678 L/min. Hence, the required fire flow for this site (10,000 L/min) can be achieved with the two hydrants





along Catherine Street alone. See **Appendix B.5** for fire hydrant coverage table calculations and NFPA Table 18.5.4.3.

Figure 3.1: Fire Hydrant Coverage Sketch

As per Section 3.2.5.16 of the Ontario Building Code, the distance between the fire department connection and hydrant cannot be obstructed or more than 45 m. As HYD-02 is located across Kent Street from the site and is more than 45 m from the Phase 1 fire department connection, a new fire hydrant is proposed on site to provide an unobstructed distance less than 45 m to the Phase 1 fire department connection and meet OBC requirements.

3.4 Proposed Water Servicing

The development will be serviced by two 150 mm building service connections. One service will be connected to the 203 mm diameter watermain on Arlington Avenue, and the other to the watermain on Kent Street. As the site is required to be serviced by two service laterals on opposite sides of the site, the existing 127 mm diameter watermain on Kent Street will need to be upsized between Arlington and Catherine Street, where a new fire hydrant will be installed on Kent Street near Catherine. The sizing of the service connections is to be confirmed by the mechanical consultant.

The proposed water servicing is shown on Drawing SSP-1.

Thermal insulation is required on the water service lateral to Arlington Avenue, as there is less than 2.4 m cover provided per W22. Booster pumps are required for both towers. The mechanical consultant or plumbing contractor will ultimately be responsible to confirm building pressures are adequate to meet building code requirements.



4 Wastewater Servicing

The subject site at 265 Catherine Street is located within a City of Ottawa combined sewage area. The existing sewers adjacent to the development site consist of a pair of combined sewers, one 300mm in diameter and the other 1800mm in diameter within Catherine Street, a pair of combined sewers, one 450 mm in diameter and the other 1350 mm in diameter within Lyon Street North, a 1200mm diameter combined sewer within Arlington Avenue, and a 375mm diameter combined sewer and the 3000mm diameter Combined Sewage Storage Tunnel (CSST) within Kent Street.

4.1 Design Criteria

As outlined in the City of Ottawa Sewer Design Guidelines and the MECP Design Guidelines for Sewage Works, the following criteria were used to calculate the estimated wastewater flow rates and to determine the size and location of the sanitary service lateral:

- Minimum velocity = 0.6 m/s (0.8 m/s for upstream sections)
- Maximum velocity = 3.0 m/s
- Manning roughness coefficient for all smooth wall pipes = 0.013
- Minimum size of sanitary sewer service = 135 mm
- Minimum grade of sanitary sewer service = 1.0 % (2.0 % preferred)
- Average wastewater generation = 280 L/person/day (per City Design Guidelines)
- Peak Factor = based on Harmon Equation; maximum of 4.0 (residential)
- Harmon correction factor = 0.8
- Infiltration allowance = 0.33 L/s/ha (per City Design Guidelines)
- Minimum cover for sewer service connections 2.0 m
- Population density for one-bedroom apartments 1.4 persons/apartment
- Population density for one-bedroom with den apartments 2.1 persons/apartment
- Population density for two-bedroom with den and three-bedroom apartments 3.1 persons/apartment
- Population density for general townhome 2.7 persons/unit
- Average commercial wastewater generation 28,000 L/ha/day of building space

4.2 Wastewater Generation and Servicing Design

The estimated peak wastewater flow generated are based on the current site plan and unit breakdown as shown in **Table 1.1**. The anticipated wastewater peak flow generated from the proposed development is summarized in **Table 4.1** below.

The lands to be conveyed to the proposed park do not form part of the site plan development and were not considered as part of the sanitary sewage calculations.



	Residential Units Commercial Areas			Commercial Areas				
Number of Units	Population	Peak Factor	Peak Flow (L/s)	Area (ha)	Peak Factor	Peak Flow (L/s)	Infiltration Flow (L/s)	Total Peak Flow (L/s)
1134	2159	2.848	19.9	0.242	1.50	0.1	0.4	20.4
	Total Estimated Wastewater Peak Flow (L/s):							

Table 4.1 - Estimated Total Wastewater Peak Flow

1. Design residential flow based on 280 L/p/day and design commercial flow based on 28,000 L/ha/day.

2. Peak factor for residential units calculated using Harmon's formula and taken as 1.50 for commercial areas.

3. Residential population estimated based on 1.4 persons/unit for one-bedroom apartments, 2.1 persons/unit for onebedroom units with dens, 3.1 persons/unit for two-bedroom units with dens and three-bedroom units, and 2.7 persons/unit for townhouses.

4. Infiltration design flow equals 0.33 L/s/ha.

Detailed sanitary sewage calculations are included in **Appendix C.1**. A full port backwater valve will be required for the proposed building in accordance with the Sewer Design Guidelines and will be coordinated with the building mechanical engineers.

The peak sanitary wastewater design flow is well within the target release rate calculated in **Section 5.4.1**, and serves as a constraint for stormwater release rate in the separate system. The anticipated peak wastewater flows for the proposed development were provided to the City of Ottawa staff to evaluate the adequacy of the receiving municipal combined sewer system in the vicinity of the site and downstream network, and the City has provided the go-ahead for the proposed sanitary discharge into the 1200 mm combined sewer in Arlington Avenue.

4.3 Proposed Sanitary Servicing

A 250 mm diameter sanitary building service for building A and a 300mm service for Buildings B and C, complete with full port backwater valve as per City standard S14.1, are recommended to service the proposed development. The sanitary laterals are be equipped with a sanitary monitoring sample port per City standard S18.1 before connecting to the sewer main as per City standard S12.2. The proposed sanitary servicing is shown on **Drawing SSP-1** and **Drawing SA-1**.

Existing connections are to be abandoned and full port backwater valves installed on the proposed sanitary services within the site to prevent any surcharge from the downstream sewer main from impacting the proposed property. A sump pump will be required for sewage discharge from the mechanical room. Sizing of the service laterals, sump pit, sump pump, and design of the internal plumbing and associated mechanical systems are to be confirmed by the mechanical consultant.



5 Stormwater Management and Servicing

5.1 Objectives

The goal of this stormwater servicing and stormwater management (SWM) plan is to determine the measures necessary to control the quantity and quality of stormwater released from the proposed development to meet the criteria established during the consultation process with City of Ottawa staff, and to provide sufficient details required for approval.

5.2 Stormwater Management (SWM) Criteria

The Stormwater Management (SWM) criteria were established by combining current design practices outlined by the City of Ottawa Sewer Design Guidelines (SDG) (October 2012), review of project preconsultation notes with the City of Ottawa, and through consultation with City of Ottawa staff. The following summarizes the criteria, with the source of each criterion indicated in brackets:

General

- Use of the dual drainage principle (City of Ottawa SDG)
- Wherever feasible and practical, site-level measures should be used to reduce and control the volume and rate of runoff (City of Ottawa SDG)
- Assess impact of 100-year event outlined in the City of Ottawa Sewer Design Guidelines on the major and minor drainage systems (City of Ottawa SDG)

Storm Sewer & Inlet Controls

- Discharge for each storm event to be restricted to a 2-year storm event pre-development rate with a maximum pre-development C coefficient of 0.4 (City of Ottawa pre-consultation, **Appendix A.2**)
- Peak flows generated from events greater than the 2-year and including the 100-year storm must be detained on site (City of Ottawa pre-consultation, **Appendix A.2**)
- The preferred stormwater system outlet for this site is the 525 mm and 600 mm diameter storm sewer within the Arlington Avenue ROW. (City of Ottawa pre-consultation, **Appendix A.2**)
- The foundation drainage system is to be independently connected to sewer main unless being pumped with appropriate back up power, sufficient sized pump, and backflow prevention. (City of Ottawa pre-consultation, **Appendix A.2**)
- T_c should be not less than 10 minutes since IDF curves become unrealistic at less than 10 min (City of Ottawa SDG).

Surface Storage & Overland Flow

- Building openings to be a minimum of 0.30 m above the 100-year water level (City of Ottawa SDG)
- Maximum depth of flow under either static or dynamic conditions shall be less than 0.30 m (City of Ottawa SDG)

• Provide adequate emergency overflow conveyance off-site with a minimum vertical clearance of 15 cm between the spill elevation and the ground elevation at the building envelope in the proximity of the flow route or ponding area (City of Ottawa SDG)

5.3 Existing Conditions

The existing site (0.75 ha, not including road widening and the proposed park) is dominated by roofs and asphalt pavement with only around 395.5 m² in soft area. As such the overall site pre-development runoff coefficient was established to be C=0.87, in which the hard surface areas use a coefficient of 0.90 while soft surface areas have a coefficient of 0.20. This exceeds the maximum permissible pre-development runoff coefficient of C=0.4 identified in the City of Ottawa pre-consultation for this site. Therefore, the pre-development runoff coefficient of 0.4 was used for the site analysis.

The pre-development release rates for the site have been determined using the rational method and the drainage characteristics identified above. A time of concentration for the pre-development area (12 minutes) was assigned based on the relatively large site area, its high impervious area, and its proximity to the stormwater outfall. The peak pre-development flow rates shown in **Table 5.1** have been calculated using the rational method as follows:

$$Q = 2.78 (C)(I)(A)$$

Where:

Q = peak flow rate, L/s C = site runoff coefficient I = rainfall intensity, mm/hr (per City of Ottawa IDF curves) A = drainage area, ha

Given the outer perimeters of the site will continue to drain uncontrolled to the surrounding rights of way as per existing conditions, only the roofs and controlled portions of the site were included in the target release rate. Furthermore, the site's location in the combined sewer area, as summarized in **Section 4**, has provided for a restrictive target release for the site, in which the stormwater contributions to the sewers are to be restricted to the 2-year pre-development release rate less contribution from sanitary sewer discharge.

As such, the target release rate for the site is summarized in **Table 5.1**: Target Release Rate below:

Table 5.1: Target Release Rate

	Area (ha)	Target Flow Rate to Combined Sewer (L/s)	Less Peak Sanitary Discharge (L/s)
Controlled Site	0.75	58.2	37.8

A target release rate of 58 L/s was obtained using a C of 0.4 and a 2-year storm event for the roof and controlled areas of the site, with the design focusing on measures adopted to provide a storm servicing approach that restricts a 100-year peak storm run-off to the target release rate calculated as 38 L/s as

shown in **Table 5.1**: Target Release Rate. To meet the stormwater quantity control criteria, two stormwater cisterns are proposed to attenuate peak run-off, one for each of the two development phases.

5.4 Stormwater Management Design

The site is to be serviced by two proposed 300 mm diameter storm sewers, which will collect stormwater discharge from the cisterns and connect to the existing 525 mm diameter and 600 mm diameter storm sewers on Arlington Avenue. The site has been subdivided into catchment areas to effectively collect, store, and convey runoff at flowrates not exceeding the target release rate established by consultation with the City of Ottawa (refer to **Drawing SD-1** for drainage areas).

Two stormwater cisterns located in the underground parking area are proposed to attenuate peak flows from the rooftop areas from the towers and the townhouses and the common areas. Area drains will convey stormwater runoff from the surface to the stormwater cisterns via the internal plumbing of the buildings. The stormwater cisterns will be pumped at controlled rates to monitor manholes which outlets to the 525 mm and 600 mm diameter storm sewer on Arlington Avenue via 300 mm diameter pipes. The stormwater cisterns' locations will be coordinated by building's architect in conjunction with mechanical and structural engineers.

Footing drainage will be independent of the internal stormwater cistern quantity control system while sharing the same outlet. The mechanical design for the weeping tile system will include dedicated storm pits and duplex pumps to pump the weeping tile drainage to the storm main downstream of the cistern.

The proposed site plan, drainage areas and proposed storm sewer infrastructure are shown on **Drawing SD-1** and **SSP-1**.

5.4.1 QUANTITY CONTROL: STORAGE REQUIREMENTS

The Modified Rational Method (MRM) was used to assess the flow rate and volume of runoff generated under post-development conditions. The site was subdivided into sub-catchments tributary to separate quantity control measures and subject to different inlet controls. **Drawing SD-1** shows the delineated sub-catchment areas. The MRM spreadsheet is included in **Appendix D.1**.

The following assumptions were made in the creation of the storm drainage plan and accompanying MRM spreadsheet:

- Restricted combined release rate is 58.2 L/s. Upon deducting the sanitary peak flow of 20.4 L/s, the storm target release is 37.8 L/s.
- Excess run-off that cannot be captured as surface storage due to grading constraints is to sheet flow uncontrolled to the adjacent roadways (areas UNC-1 and UNC-2).
- Stormwater cistern equipped with mechanical pump to attenuate peak flows from the cistern will be used to manage stormwater flows from the site.



5.4.1.1 Uncontrolled Areas

Uncontrolled areas represent drainage areas that cannot be graded to enter the storm sewer system and are not captured by the proposed storm cistern. As such, they will sheet drain off the site to the adjacent roadways (see **Drawing SD-1**).

Area IDs	Area (ha)	2-Yr uncontrolled peak flow (L/s)	100-Yr uncontrolled peak flow (L/s)
UNC-1	0.04	6.9	18.4
UNC-2	0.11	17.1	49.8

Table 5.2: Peak Uncontrolled 2- and 100-Year run-off

Based on discussions with the City of Ottawa staff, overland flow from all uncontrolled drainage areas will not be considered in the overall development peak discharge rate as it contributes to the controlled system within the public roadways, which are equipped with ICDs.

5.4.1.2 Stormwater Cisterns

As part of the stormwater management design of the site development, two stormwater cisterns located in the underground parking area and equipped with mechanical pumps are proposed to attenuate peak flows from drainage areas CIST 1-1 to CIST 1-4 for Cistern 1 and CIST 2-1 to CIST 2-4 for Cistern 2. The final location of the cisterns within the proposed building is to be coordinated by the architect with mechanical and structural engineers.

Cistern 1 for Phase 1 is to be designed to provide a minimum active storage volume of 110 m³ with a maximum controlled release rate of 14.3 L/s, while Cistern 2 for Phase 2 is to be designed to provide a minimum active storage volume of 170 m³ with a maximum controlled release rate of 23.6 L/s. The stormwater cisterns are to discharge at the specified controlled release rate using a pump.**Table 5.3** summarizes the respective flow rates and volume of retained stormwater in the 2-year and 100-year storm events.

Cistern	Storm Return Period	Area IDs	Drainage Area (ha)	Q _{release} (L/s)	V _{required} (m ³)	V _{available} (m ³)	Total V (m ³)
1	2-year	CIST 1-1 to	0.28	14.3	26.8	110	
	100-year	CIST 1-4	0.20	14.5	107.1	110	
2	2-year	CIST 2-1 to	0.46	23.6	38.6	170	280
	100-year	CIST 2-4	0.40	23.0	163.2	170	

Table 5.3: Proposed Cistern 2 and 100-Year Storage Requirement

5.4.1.3 Results

The proposed stormwater management plan provides adequate attenuation to meet the target release rate for the 2 and 100-year storm events as shown in **Table 5.4** below.

Area Type	2-Year (L/s)	100-Year (L/s)	Target (L/s)
Uncontrolled	24.0	68.1	-
Controlled Areas	37.8		
Sanitary Contributions	20.4		58.2
Total Flow to Sewer	58.2		

 Table 5.4: Estimated Post-Development Discharge

Flows from the uncontrolled areas are not considered in the overall release rate for the site as detailed in **Section 5.4.1.1**.

5.4.2 QUALITY CONTROL

Through correspondence with the City of Ottawa, it was confirmed that no stormwater quality control measures apply, as the 525 mm and 600 mm diameter storm sewer in Arlington Avenue ultimately discharges into the 1350 mm diameter combined sewer in Lyon Street North.

5.5 Proposed Stormwater Servicing

Two 300 mm diameter stormwater building services, complete with full port backwater valve as per City standard S14.1, are proposed for the storm service discharge, as per **Drawing SSP-1** and **Drawing SD-1**. A stormwater sump and pump are required for the proposed foundation drain, and the roof drains are to be connected to the cisterns.

The combined foundation drain, roof drain, and subdrain flows will outlet to the cistern, which then pumps the discharge at a controlled rate and to the existing 525 mm and 600 mm diameter storm sewer within the Arlington Avenue ROW. The laterals are to connect to the main as detailed on the servicing drawings via monitoring sample ports. The proposed stormwater servicing is shown on **Drawing SSP-1** and **SD-1**.



6 Site Grading

The proposed re-development site, excluding the park, measures approximately 0.90 ha in area and consists of the former Greyhound bus terminal and asphalt area, with small patches of grassland. The topography across the site generally slopes from the middle towards the Arlington Avenue ROW at the north and the Catherine Street ROW at the south.

A detailed grading plan (see **Drawing GP-1**) has been provided to satisfy the stormwater management requirements as detailed in **Section 5**, adhere to any grade raise restrictions for the site, and provide for minimum cover requirements for storm and sanitary sewers where possible. Site grading has been established to provide emergency overland flow routes required for stormwater management.

7 Utilities

Overhead (OH) hydro-wires run east-west on the north side of Catherine Street and south side of Arlington Avenue, and north-south on the east side of Lyon Street North and on the west side of Kent Street, terminating halfway along the east property line of the site. All utilities within the work area will require relocation during construction. The existing utility poles within the public right of way are to be protected during construction.

As the site is surrounded by existing residential and commercial development, Hydro Ottawa, Bell, Rogers, and Enbridge servicing is readily available through existing infrastructure to service this site. The exact size, location, and routing of utilities will be finalized after design circulation. Existing overhead wires and utility plants may need to be temporarily moved/reconfigured to allow sufficient clearance for the movement of heavy machinery required for construction. The relocation of existing utilities will be coordinated with the individual utility providers upon design circulation.

8 Approvals

The proposed development lies on a private site under singular ownership and the storm discharge drains to an existing storm sewer outlet, therefore, the site will not require an Environmental Compliance Approval (ECA) from the Ministry of the Environment, Conservation and Parks (MECP) under O.Reg. 525/98.

For ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). It is possible that groundwater may be encountered during the foundation excavation on this site. A minimum of two to four weeks should be allotted for completion of the EASR registration and the preparation of the Water Taking and Discharge Plan by a Qualified Person as stipulated under O.Reg. 63/16. An MECP Permit to Take Water (PTTW), which is required for dewatering volumes exceeding 400,000L/day, is not anticipated for the site.



9 Erosion and Sediment Control During Construction

To protect downstream water quality and prevent sediment build-up in catch basins and storm sewers, erosion and sediment control measures must be implemented during construction. The following recommendations will be included in the contract documents and communicated to the Contractor.

- 1. Implement best management practices to provide appropriate protection of the existing and proposed drainage system and the receiving water course(s).
- 2. Limit the extent of the exposed soils at any given time.
- 3. Re-vegetate exposed areas as soon as possible.
- 4. Minimize the area to be cleared and grubbed.
- 5. Protect exposed slopes with geotextiles, geogrid, or synthetic mulches.
- 6. Install silt barriers/fencing around the perimeter of the site as indicated in **Drawing ECDS-1** to prevent the migration of sediment offsite.
- 7. Install trackout control mats (mud mats) at the entrance/egress to prevent migration of sediment into the public ROW.
- 8. Provide sediment traps and basins during dewatering works.
- 9. Install sediment traps (such as SiltSack® by Terrafix) between catch basins and frames.
- 10. Schedule the construction works at times which avoid flooding due to seasonal rains.

The Contractor will also be required to complete inspections and guarantee the proper performance of their erosion and sediment control measures at least after every rainfall. The inspections are to include:

- Verification that water is not flowing under silt barriers.
- Cleaning and changing the sediment traps placed on catch basins.

Refer to **Drawing ECDS-1** for the proposed location of silt fences, sediment traps, and other erosion control measures.



10 Geotechnical Investigation

A geotechnical investigation report was prepared by Paterson Group on August 13, 2021 to provide an assessment of the subsurface conditions found at the site. Three (3) boreholes, numbered BH 1-20 to BH 3-20, were advanced to a maximum depth of 14.7 metres below the existing ground surface in the investigation carried out on August 19, 2020. The information obtained from the field investigation will guide the detailed design of the site and identify development constraints. Excerpts from the geotechnical investigation report are attached in **Appendix E**.

The subsurface profile encountered at the test hole locations are characterized primarily by a layer of concrete or asphaltic concrete underlain by fill extending to an approximate depth of 0.6 m to 2.3 m below the existing ground surface. The fill material was observed to generally consist of brown silty sand with crushed stone and occasional brick, metal, and plastic fragments and underlain by a silty clay deposit and silty clay layer. In addition, the silty clay deposit generally consisted of brown silty clay, with the glacial till deposit underlain the silty clay deposit consisting of a grey sandy silt, clayey silt or silty clay with gravel, cobbles, and boulders.

From available geological mapping, the bedrock consists of interbedded limestone and shale of the Verulam formation and shale of the Billings formation at depths ranging from 10 m to 15 m. Groundwater levels were measured from monitoring wells at all three boreholes in the August 2020 investigation and are expected to be 4 metres to 5 metres below the existing ground surface within the silty clay layer, though as groundwater levels are subject to seasonal fluctuations, they could vary at the time of construction.

Based on Paterson's recommendations, the site is suitable for the proposed development. It is recommended that the foundation support for the proposed mixed-use high-rise buildings consist of either a raft foundation bearing on the stiff silty clay and compact glacial till deposit, or a deep foundation extending to the bedrock surface. Due to the presence of the silty clay deposit, grading is subject to a permissible grade raise restriction of 2.0 m.

The recommended rigid pavement structure is further presented in **Table 10.1** below.

Material	Car-only Parking Areas	Access Lanes, Ramp and Heavy Truck Parking Areas	
Wear Course – HL-3 or Superpave 12.5 Asphaltic Concrete	50 mm	40 mm	
Binder Course – HL-8 or Superpave 19.0 Asphaltic Concrete	-	50 mm	
BASE – OPSS Granular A Crushed Stone		50 mm	
SUBBASE – OPSS Granular B Type II	300 mm	450 mm	

Table 10.1: Recommended Pavement Structure



11 Conclusions

11.1 Water Servicing

Based on the supplied boundary conditions for existing watermains and calculated domestic and fire flow demands for the subject site, the adjacent watermain on Arlington Avenue has sufficient capacity to sustain both the required domestic and emergency fire flow demands for the development. As the site is required to be serviced by two service laterals on opposite sides of the site, the existing 127 mm diameter watermain on Kent Street will need to be upsized to meet required fire flows. Booster pump(s) are required to provide adequate pressures to the towers' upper stories. One building water service will be connected to the existing 203 mm diameter watermain in Arlington Avenue and the other to the upsized watermain in Kent Street, where a new fire hydrant will be located adjacent to the Catherine Street intersection. Sizing of the water service and requirements for booster pump(s) are to be confirmed by the mechanical consultant.

11.2 Sanitary Servicing

The proposed sanitary sewer service will consist of two sanitary service laterals, a sanitary sump pit, monitoring ports, and sump pump(s) directing wastewater to the existing 1200 mm diameter combined sewer on Arlington Avenue. Existing connections are to be abandoned and full port backwater valves installed on the proposed sanitary service within the site to prevent any surcharge from the downstream sewer main from impacting the proposed property. A sump pump will be required for sewage discharge from the mechanical room. Sizing of the service lateral, sump pit, and sump pump are to be confirmed by the mechanical consultant.

11.3 Stormwater Servicing and Management

Cisterns in the underground parking has been proposed to limit the stormwater discharge rate for all rainfall events up to and including the 100-year event to a peak 2-year predevelopment release rate. The remaining site area drains uncontrolled to the adjacent surrounding ROWs as per existing conditions.

Two 300 mm diameter storm service laterals are proposed for the building's foundation drain and internal storm sewer system, which is to be mechanically pumped and include a full port backwater valve. The roof drains and ramp drain are to be connected through internal plumbing to the cistern, which will pump discharge at a controlled rate through the service lateral and the backwater valve to the 525 mm and 600 mm diameter municipal storm sewer in the Arlington Avenue ROW. Sizing of the service lateral, cistern, and foundation drain pump are to be confirmed by the mechanical consultant.

11.4 Grading

Site grading has been designed to provide an adequate emergency overland flow route. All four sides drain uncontrolled to the adjacent right-of-ways as per existing conditions.



11.5 Erosion and Sediment Control During Construction

Erosion and sediment control measures and best management practices outlined in this report and included in the drawing set, will be implemented during construction to reduce the impact on adjacent properties, the public ROW, and existing facilities.

11.6 Geotechnical Investigation

Based on the geotechnical investigation, the site is considered suitable for the proposed building, and it is recommended that the foundation support for the proposed mixed-use high-rise buildings consist of either a raft foundation bearing on the stiff silty clay and compact glacial till deposit, or a deep foundation extending to the bedrock surface. Due to the presence of the silty clay deposit, grading is subject to a permissible grade raise restriction of 2.0 m.

11.7 Utilities

The site is situated within an established neighbourhood, hence existing utility infrastructure is readily available to service the proposed development. Overhead wires along all boundaries of the site will need to be accommodated during construction. It is anticipated that existing infrastructure will be sufficient to provide a means of distribution for the proposed site. Exact size, location and routing of utilities will be finalized after design circulation.

11.8 Approvals

This site will not be subjected to the Ministry of the Environment, Conservation and Parks (MECP) Environmental Compliance Approval (ECA) process under O.Reg. 525/98. For the expected dewatering needs of 50,000 to 400,000 L/day, the proponent will need to register on the MECP's Environmental Activity and Sector Registry (EASR). A Permit to Take Water, for dewatering needs in excess of 400,000 L/day, is not anticipated for this site.

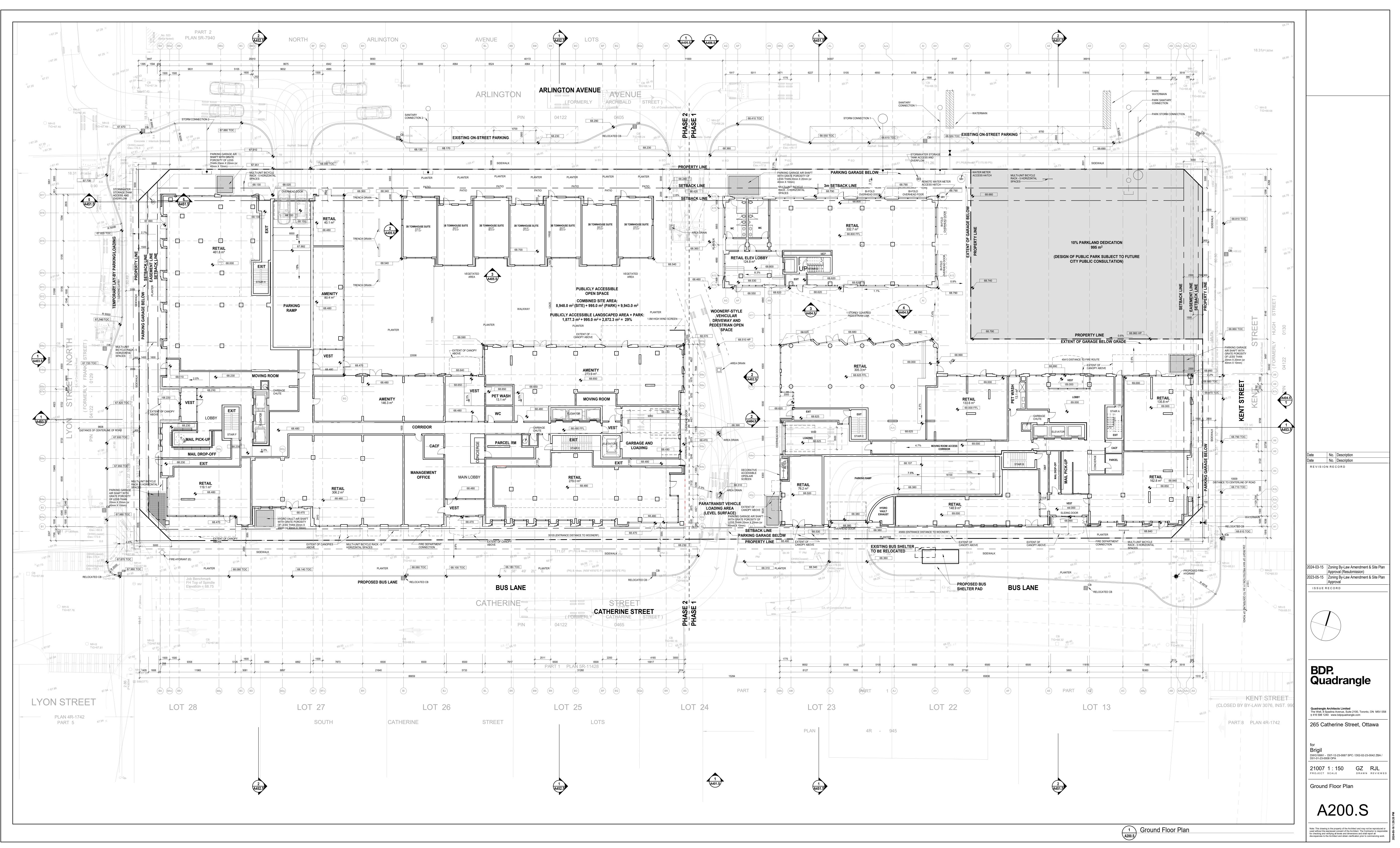


APPENDICES



Appendix A Background

A.1 Site Plan by Quadrangle Architects Ltd. (February 12, 2024)



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265 Catherine Street Servicing and Stormwater Management Report Background

A.2 Pre-Consultation

Kilborn, Kris

From: Sent:	Bernier, John <john.bernier@ottawa.ca> Monday, May 2, 2022 2:51 PM</john.bernier@ottawa.ca>			
То:	John Moser			
Cc:	Dubyk, Wally; Jean-Luc Rivard; Wang, Randolph; Patel, Parthvi; McCreight, Andrew; Fawzi,			
	Mohammed; Walker, Max; 'Kirsten Beale'; Philip Thibert; 'Mary Huang'; 'Nakanishi, Alice'; Russett,			
	Mike; Richardson, Mark; Ryan Lupien; Kilborn, Kris; Heather Rolleston; Jessy Desjardins			
Subject:	265 Catherine (Greyhound site) - PC2022-0057 - Follow-up			
Attachments:	19M-01044-00cm Trip Generation Manual Summary Report FINAL.pdf; TRANS Trip Calculator-			
	PM.xlsx; NCHRP_Report_684_estimator_update_073113.xlsx; 19M-01044-00 Trip Generation Manual			
	Background Report_FINAL.pdf; design_brief_TOR_265 Catherine.pdf; 265 Catherine - PRE-			
	APPLICATION Consultation form.pdf; 265 Presentation - 2022-04-01 - BDPQ Revised.pdf; Pre-con			
	Applicant's Study and Plan Identification List.docx			

Good afternoon John,

Sorry for the delay. Please refer to the below [and/or attached notes] regarding the Pre-Application Consultation (pre-con) Meeting held on April 6th, 2022 for the property at 265 Catherine Street for a mixed-use development consisting of 40, 36, and 28 storey towers. The proposal included 6-storey podiums with commercial, retail, and amenity space. A portion of the site is dedicated to a on-site public park which complimented semi-private landscaped areas. I have attached the required Plans & Study List for application submission.

Below [or attached] are staff's preliminary comments based on the information available at the time of pre-con meeting:

Planning Comments – John Bernier:

- Relevant Policies:
 - **Old OP:** 3.6.1 General Urban Area
 - New OP: Downtown Core Transect/ Minor Corridor
 - Consult the <u>New Official Plan</u>
 - Secondary Plan: Centertown Secondary Plan will become the amalgamated 'Central and East Downtown Core Secondary Plan'
 - **Zoning:** General Mixed-use, with except 1875, height schedule 271
- Comments:

0

- **Public consultation**:
 - I see that three community workshops done please provide detail of this in your planning rationale and prepare to have a further public meeting once the application has been submitted.
 - Looking for the dates of previous meetings, what you heard from the community, and the progression of the design.
 - Please provide a public consultation strategy, this may be included in the Planning Rationale.
- Height:

- We would like to see good policy argument for the replacement heights proposed and how this impacts the city views of parliament.
- You've acknowledged in your policy context slide that the heights within this development need to achieve appropriate transition to the mid-rise and low density areas to the north. How is this able to be achieved when low-rise immediately to the north?
- If we allow increased height it would open it up for more heights south of the site and potentially south of the highway. We would like to see this considered in your Planning Rationale.

• Site and Building Design

- New OP: Floorplate size should generally be limited to 750 square metres for residential building
- Min 23m separation between towers.
- Original OPA/Rezoning required Privately-Owned Public Spaces (min 25% of the site), we would like for that to carry forward into future designs.
- Corner site triangles on all four sides will be required as well as ROW protections.
 - I note that your underground parking garage does not consider this.

• Application Details:

• Until there is a replacement of Section 37 (Community Benefits Charges By-law), additional heights and densities will require community benefits. Please include an as-of-right GFA calculation vs. proposed.

• Phasing

- Please provide us with a clear phasing plan and strategy in the Planning Rationale.
- I would suggest that the first phase conform to the current policies, i.e. Maximum heights of 25-storeys.

• Future Meeting

• As your project evolves we are open to having another meeting.

Policy Planning – Max Walker:

- Generally, the proposal seems to conform with the direction provided in the Secondary Plan and new OP, which encourages high-rise development to buffer the highway, varied building heights, and quality architecture design to positively contribute to the sense of place. The project's success rest, in my opinion, principally on providing good transition and varied and articulated built forms.
- To minimize impacts on neighbouring properties and on the public realm, transition in building heights shall be designed in accordance with applicable design guidelines. Please consider policies in section 4.6.6 and pay special attention to policies 5, 8 and 9.
- It is also important to note that the site is located within a Tier 2 design priority area. This is an area of national and regional importance to defining Ottawa's image. This area should support moderate pedestrian volumes and is characterized by its regional attractions related to leisure, entertainment, nature and culture. In this regard, it is especially important to consider Policy 4.2.6(1). This policy notes that the visual integrity and symbolic primacy of the Parliament Buildings and other national symbols, as seen from Confederation Boulevard, the main approach routes to the Parliamentary Precinct and from other key viewpoints and view

sequences is protected. The area to which view protection applies can be extended through development or supplementary planning processes, to apply to lands where the City determines that height and foreground controls are necessary in accordance with the intent of Schedule C6A, Schedule C6B, Schedule C6C and the National Capital Commission's Canada's Capital Views Protection, or its successor document.

- The site is located within the Downtown transect along a Minor Corridor. Please ensure that the policy objectives of 5.1.4 are achieved. On Downtown Core Minor Corridors, all buildings shall have active entrances facing the Minor Corridor, regardless of use. While permission for a higher building is permitted through a secondary plan, the height of such buildings shall, with respect to the wall heights directly adjacent to a street, be proportionate to the width of the abutting right of way and consistent with the objectives in the urban design section on Mid-rise and High-rise built form in Subsection 4.6.6, Policies 7), 8) and 9).
- The porosity of the site is also another important feature of the proposed development; excellent. Providing for safe, direct and convenient pedestrian and cycling networks and crossings, including along desire lines is an important direction of the new Official as well as the Secondary Plan. Policy 20 of the latter document states that any future development of 265 Catherine Street within the height limit of the zoning in place on January 23rd, 2014 will include a minimum of 25 percent of the lot area as a POPS. It is encouraged to maintain and enhance the provision of strong public realm elements that also enhance the local mobility network.
- The important part for the development proponent will ensure that the proposal is consistent with both the existing and new OP. It is important to note that the New OP does not have legal status, and while approved in principle by Council and does represent Council's intent, it does not preclude an application to amend the existing OP. An incoming application should continue to be "tested" against the new OP, but the document in force is what is existing. However, suppose an amendment is not in line with the new Official Plan; in that case, we should seriously consider whether there would be a recommendation for an amendment authorized to the new Official Plan.
- Please let me know if you have any questions regarding the foregoing and I would be happy to schedule a short meeting with you and the applicant to discuss the appropriate process.

Urban Design Comments – Randolph Wang:

- 1. A Design Brief is required as part of the submission. The Terms of Reference of the Design Brief is attached for convenience. Please note:
 - a. Both a wind study and a shadow study is required.
 - b. The context study should include a broader area.
 - c. The site is subject to the <u>Centretown Community Design Plan</u> and the <u>Centretown</u> <u>Secondary Plan</u>, which recently has been amalgamated into the Central and East Downtown Core Secondary Plan. The CDP and the Secondary Plan provide specific vision for the community and the area where the property is situated. The context study should include images and renderings that show the proposed development in both the existing and planned context. The context study should examine the potential impacts of the proposed development on the CDP and Secondary Plan vision for the area.
 - d. Please study views of the proposed buildings from the various vantage points identified in Schedule C6-A - Views, Viewsheds, and View Sequences of the Parliament Buildings and other National Symbols of the new Official Plan. The intent of the study will be to ensure absolute no impact on the Parliamentary Buildings and National Symbols.
 - e. Please study views of the proposed buildings from adjacent heritage sites, important institutions, such as the Museum of Nature, and open spaces, to make sure no adverse impacts.

- f. Please study the views of the proposed building from various vantage points within the neibourhood, along the adjacent streets, and on the highway.
- 2. The site is within a Design Priority Area. The proposed development will be subject to formal review by the City's Urban Design Review Panel. Information on scheduling and submission can be found on the <u>UDRP website</u>. Given the complexity of the development, UDRP informal review prior to the submission is also recommended.
- 3. A few high-level design comments on the concept presented at the meeting:
 - a. The overall approach to built form and public realm design, including site porosity, is appreciated.
 - b. The concept displays considerable merits with respect to the public realm vision, which includes a series of connected yet differentiated privately owned public spaces -- the NS mid-block connection, the animated urban plaza on the east side of the site, and the quieter residential courtyard on the west side of the site. However, the requirements for a public park on site will result in significant changes to the concept for the better.
 - c. The ground floor plan of these buildings, including the location and design of the garage ramp, should support the vision and intent of the public spaces. In this regard, the propose to locate the ramp under Tower 2 is more favorable comparing with the proposal to locate the ramp under Tower 3.
 - d. The concept displays a clear intent to provide built form transition. However, the rationale should be future developed and clarified and evaluated based on their impactions. The 3-storey towns and 6-storey mixed-use building are appropriate for Arlington. The 6-storey podium is also appropriate for Catherine Street. Consideration should be given to lowering the podium of Tower 1 from 6-storey to a maximum 4 storeys. Considerations should also be given to lowering the podium of Tower 2 to allow for more light into the public spaces and opening up sky views of the courtyard.
 - e. The tower floor plates should respect policies of the CDP and Secondary Plan as well as the City's guidelines for high-rise buildings and be reduced to a maximum of 750m².
 - f. While the reasoning behind the pursuit of a rectangular shape floor plate was well explained and appreciated, the design of these towers should take into consideration other factors. Tower 1 is particularly concerning and should be designed with a more compact floor plate. Strategic differentiation of tower floor plates would also make sense with respect to the overall massing composition of towers (three towers of the same floor plate but different heights may be awkward).
 - g. Please be mindful of the hydro wires available on the streets. The applicants are highly encouraged to consider burring the hydro wires.

Parks Comments – Mike Russet:

- Site generates +/- 1034.56m2 parkland dedication @ 10% of site calculation;
- PRCS (Parks Planning) requests consideration for full dedication of the required parkland;
- PRCS requests location of the future park block dedication fronting onto Arlington Ave;
- parkland dedication must be free-&-clear of all encumbrances no strata park considerations, therefore underground garage within/under future parkland dedication not permitted, no Limiting Distance (LDA);
- future parkland connectivity to and design considerations for landscaped open space of importance;
- consider any trigger of Section 37 and/or community benefits requirements to include design & construction of the future park block.

Engineering Comments – Mohammad Fawzi:

Water Boundary Conditions:

Will be provided at request of consultant. Requests must include the location of the service and the expected loads required by the proposed development. Please provide the following and <u>submit Fire</u> <u>Flow Calculation Sheet</u> per FUS method with the request:

- Location of service
- Type of development and amount of required fire flow (per FUS method <u>include FUS</u> <u>calculation sheet with request</u>)
- Average Daily Demand (I/s)
- Maximum Hourly Demand (I/s)
- Maximum Daily Demand (I/s)
- Water Supply Redundancy Fire Flow: Applicant to ensure that a second service with an inline valve chamber be provided where the average daily demand exceeds 50 m³ / day (0.5787 l/s per day)

Water services larger than 19 mm require a Water Data Card. Please complete card and submit.

Stormwater Management:

- Coefficient (C) of runoff determined as per existing conditions but in no case more than 0.4
- TC = To be calculated, minimum 10 minutes
- Any storm events greater than 2 year, up to 100 year, and including 100-year storm event must be detained on site.
- Foundation drains are to be independently connected to sewer main unless being pumped with appropriate back up power, sufficient sized pump and back flow prevention.
- Roof drains are to be connected downstream of any incorporated ICD within the SWM system.

Stormwater management criteria (Quality Control)

Include a section in the SWM report concerning quality control requirements. It is the consultant's responsibility to check with the relevant Conservation Authority for quality control issues and include this information in the SWM report.

Phase I and Phase II ESA:

- Phase I ESA is required; Phase II ESA may be required depending on the results of the Phase I ESA. Phase I ESA must include an EcoLog ERIS Report.
- Phase I ESA and Phase II ESAs must conform to clause 4.8.4 of the Official Plan that requires that development applications conform to Ontario Regulation 153/04.

Required Studies

- Servicing and Stormwater Management Report
- Geotechnical Study
- Phase I ESA
- Phase II ESA (depends on outcome of Phase I)
- Noise Study (proximity to Highway 417)

Required Plans

- Site Servicing Plan
- Grade Control and Drainage Plan
- Erosion and Sediment Control Plan (Can be combined with grading plan)

Relevant information

- 1. The Servicing Study Guidelines for Development Applications are available at the following address: <u>https://ottawa.ca/en/city-hall/planning-and-development/information-development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#servicing-study-guidelines-development-applications</u>
- 2. Servicing and site works shall be in accordance with the following documents:
 - ⇒ Ottawa Sewer Design Guidelines (October 2012)
 - \Rightarrow Ottawa Design Guidelines Water Distribution (2010)
 - ➡ Geotechnical Investigation and Reporting Guidelines for Development Applications in the City of Ottawa (2007)
 - ⇒ City of Ottawa Slope Stability Guidelines for Development Applications (revised 2012)
 - ⇒ City of Ottawa Environmental Noise Control Guidelines (January, 2016)
 - ⇒ City of Ottawa Park and Pathway Development Manual (2012)
 - ⇒ City of Ottawa Accessibility Design Standards (2012)
 - ⇒ Ottawa Standard Tender Documents (latest version)
 - ⇒ Ontario Provincial Standards for Roads & Public Works (2013)
- 3. Record drawings and utility plans are also available for purchase from the City (Contact the City's Information Centre by email at <u>InformationCentre@ottawa.ca</u> or by phone at (613) 580-2424 x.44455).
- 4. Any proposed work in utility easements requires written consent of easement owner.

<u>Forestry Comments – Mark Richardson</u> TCR requirements:

- 1. a Tree Conservation Report (TCR) must be supplied for review along with the suite of other plans/reports required by the City
 - a. an approved TCR is a requirement of Site Plan approval.
 - b. The TCR may be combined with the LP provided all information is supplied
- 2. Any removal of privately-owned trees 10cm or larger in diameter, or city-owned trees of any diameter requires a tree permit issued under the Tree Protection Bylaw (Bylaw 2020 340); the permit will be based on an approved TCR and made available at or near plan approval.
- 3. The Planning Forester from Planning and Growth Management as well as foresters from Forestry Services will review the submitted TCR
 - a. If tree removal is required, both municipal and privately-owned trees will be addressed in a single permit issued through the Planning Forester
 - b. Compensation may be required for city owned trees if so, it will need to be paid prior to the release of the tree permit
- 4. the TCR must list all trees on site, as well as off-site trees if the CRZ extends into the developed area, by species, diameter and health condition
- 5. please identify trees by ownership private onsite, private on adjoining site, city owned, coowned (trees on a property line)
- 6. If trees are to be removed, the TCR must clearly show where they are, and document the reason they cannot be retained

- All retained trees must be shown, and all retained trees within the area impacted by the development process must be protected as per City guidelines available at <u>Tree Protection</u> <u>Specification</u> or by searching Ottawa.ca
- 8. the City encourages the retention of healthy trees; if possible, please seek opportunities for retention of trees that will contribute to the design/function of the site.
- 9. For more information on the process or help with tree retention options, contact Mark Richardson <u>mark.richardson@ottawa.ca</u> or on <u>City of Ottawa</u>

LP tree planting requirements:

For additional information on the following please contact tracy.smith@Ottawa.ca

Minimum Setbacks

- Maintain 1.5m from sidewalk or MUP/cycle track.
- Maintain 2.5m from curb
- Coniferous species require a minimum 4.5m setback from curb, sidewalk or MUP/cycle track/pathway.
- Maintain 7.5m between large growing trees, and 4m between small growing trees. Park or open space planting should consider 10m spacing, except where otherwise approved in naturalization / afforestation areas. Adhere to Ottawa Hydro's planting guidelines (species and setbacks) when planting around overhead primary conductors.

Tree specifications

- Minimum stock size: 50mm tree caliper for deciduous, 200cm height for coniferous.
- Maximize the use of large deciduous species wherever possible to maximize future canopy coverage
- Tree planting on city property shall be in accordance with the City of Ottawa's Tree Planting Specification; and include watering and warranty as described in the specification (can be provided by Forestry Services).
- Plant native trees whenever possible
- No root barriers, dead-man anchor systems, or planters are permitted.

• No tree stakes unless necessary (and only 1 on the prevailing winds side of the tree) Hard surface planting

- Curb style planter is highly recommended
- No grates are to be used and if guards are required, City of Ottawa standard (which can be provided) shall be used.
- Trees are to be planted at grade

Soil Volume

• Please ensure adequate soil volumes are met:

Tree Type/Size	Single Tree Soil Volume (m3)	Multiple Tree Soil Volume
. , , , , , , , , , , , , , , , , , , ,		(m3/tree)
Ornamental	15	9
Columnar	15	9
Small	20	12
Medium	25	15
Large	30	18
Conifer	25	15

Please note that these soil volumes are not applicable in cases with Sensitive Marine Clay.

Sensitive Marine Clay

• Please follow the City's 2017 Tree Planting in Sensitive Marine Clay guidelines

Tree Canopy Cover

- The landscape plan shall show how the proposed tree planting will replace and increase canopy cover on the site over time, to support the City's 40% urban forest canopy cover target.
- At a site level, efforts shall be made to provide as much canopy cover as possible, through tree planting and tree retention, with an aim of 40% canopy cover at 40 years, as appropriate.
- Indicate on the plan the projected future canopy cover at 40 years for the site.

Transportation Comments – Wally Dubyk:

Road Resurfacing along Catherine Street is targeted to start this season.

Sewer Lining along Kent Street is targeted to start 1-2 years.

Update to the TIA Guideline Forecasting Report

- We would like to inform all consultants making TIA Forecasting Report submissions to the City of Ottawa as part of a development application, that all new applications (pre-consultation meetings dated after March 3, 2021) must use the NEW TRANS Trip Generation Manual when forecasting site generated trips using this manual (see attached).
- The TRANS committee (a joint transportation planning committee serving the National Capital region) finalized a new manual early in March 2021. The document will be available in French and English on the TRANS website http://www.ncr-trans-rcn.ca/surveys/2009-trip-generation.
- The new manual has simplified the conversion from vehicle trips to person trips and then trips by modal share. The City has also developed a spreadsheet that will apply the factors of location and building type to quickly provide the existing trip numbers by mode share.

A Screening Form is to be submitted to determine if a transportation study is required. Consultants should fill in the form in Appendix 'B'. Click on the website: <u>www.ottawa.ca/TIA</u>

Catherine Street is designated as an Arterial road within the City's Official Plan with a ROW protection limit of 23.0 metres. The ROW protection limit and the offset distance (11.5 metres) are to be dimensioned from the existing centerline of pavement and shown on the drawings. The Certified Ontario Land Surveyor is to confirm the ROW protected limits and any portion that may fall within the private property to be conveyed to the City.

ROW interpretation – Land for a road widening will be taken equally from both sides of a road, measured from the centreline in existence at the time of the widening if required by the City. The centreline is a line running down the middle of a road surface, equidistant from both edges of the pavement. In determining the centreline, paved shoulders, bus lay-bys, auxiliary lanes, turning lanes and other special circumstances are not included in the road surface.

Kent Street is designated as an Arterial road within the City's Official Plan with a ROW protection limit of 20.0 metres. Maximum land requirement from property abutting existing ROW (0.90 m). Subject to

widening/easement policy. The ROW protection limit is to be dimensioned on the drawings. The Certified Ontario Land Surveyor is to confirm the ROW protected limits and any portion that may fall within the private property to be conveyed to the City.

Lyon Street is designated as an Arterial road within the City's Official Plan with a ROW protection limit of 20.0 metres. Maximum land requirement from property abutting existing ROW (0.90 m). Subject to widening/easement policy. The ROW protection limit is to be dimensioned on the drawings. The Certified Ontario Land Surveyor is to confirm the ROW protected limits and any portion that may fall within the private property to be conveyed to the City.

A 5.0 metres x 5.0 metres sight triangle would be required at the intersection of Kent Street Arlington Avenue. The sight triangle area is to be conveyed to the City and is to be shown on all drawings. The sight triangle dimensions are to be measured from the ROW protected limits.

A 5.0 metres x 5.0 metres sight triangle would be required at the intersection of Lyon Street and Catherine Street. The sight triangle area is to be conveyed to the City and is to be shown on all drawings. The sight triangle dimensions are to be measured from the ROW protected

All underground and above ground building footprints and permanent walls need to be shown on the plan to confirm that any permanent structure does not extend either above or below into the sight triangles and/or future road widening protection limits.

Permanent structures such as curbing, stairs, retaining walls, and underground parking foundation also bicycle parking racks are not to extend into the City's right-of-way limits.

The concrete sidewalks should be 2.0 metres in width and be continuous and depressed through the proposed accesses.

The closure of an existing private approach shall reinstate the sidewalk, shoulder, curb and boulevard to City standards.

The Owner acknowledges and agrees that all private accesses to Roads shall comply with the City's Private Approach By-Law being By-Law No. 2003-447 as amended <u>https://ottawa.ca/en/living-ottawa/laws-licences-and-permits/laws/law-z/private-approach-law-no-2003-447</u> or as approved through the Site Plan control process.

The City does not recommend a lay-by along Catherine Street within the City's ROW.

The Owner shall be required to enter into maintenance and liability agreement for all pavers, plant and landscaping material placed in the City right-of-way and the Owner shall assume all maintenance and replacement responsibilities in perpetuity.

Bicycle parking spaces are required as per Section 111 of the Ottawa Comprehensive Zoning By-law. Bicycle parking spaces should be located in safe, secure places near main entrances and preferably protected from the weather.

Relocating an existing roadway curbing by 30 cm will require a RMA report and approval by the delegated authority. Please confirm if you are triggering an RMA.

A construction Traffic Management Plan is to be provided for approval by the Senior Engineer, Traffic Management, Transportation Services Dept.

Comments from Centretown Community Association:

Mary Huang:

I would also like to thank the developer for doing consultation early in the planning process and including open public spaces in the development. I agree with a lot of the points Alice brought up and bring up some of my own points.

1. There have been very few family sized units being built in Centretown. I think with such a large piece of land and a school next door and other schools nearby, it might be nice to get 15-20% 3 bedrooms. I am pretty sure there will be a lot of interest in the larger units.

2. In the visioning workshop, a question was posed about accessibility standards. It was indicated at the time they plan to build to minimum Ontario Building Code which is 15% and more visitable. With the growing seniors population which is expected to be 25% of the city of Ottawa in 2035, I think the Ontario Building Code is behind the needs from the changing demographic. In view that housing is likely to be built for 50+ years, I hope the applicant would give serious consideration to build to universal standards and be more accessible with wider corridors and wider doors upfront. The additional cost is small if planned at the beginning whereas trying to retrofit afterwards would be very expensive and difficult. I think it would be good if all units are at least visitable with a portion of the units with accessible features such as roll in / walk in shower. Some people consider barrier free showers a high end feature and it would be attractive for aging seniors with mobility issues or people with disability.

https://www.accessiblehousingnetwork.org/accessible-housing

3. I understand it is hard to create deeply affordable units without government subsidies. With the applicant asking for significant density increases over zoning for the land. The City of Ottawa's declared a housing and homelessness emergency and plans to introduce Inclusionary Zoning, I hope the project could figure out a way to do some moderately affordable housing units or like LeBreton Flats Library parcel find a non profit partner for some of the units. Another example from Toronto is Daniel Corp with WoodGreen and the City of Toronto.

4. I live in a market rental of CCOC at Arlington & Lyon and I suspect a 40 story tower and a 36 story tower would significantly shadow the smaller houses and lowrises in the area. If the plan is to ask for quite a significant increase in density, I hope it includes plans for a combination of offers on the previous 3 points. Arlington and especially Lyon are narrower streets than others that have 6 storey buildings. Have the appropriate studies been done for sun/shadow and wind?

5. There are apparently underground waterways not far from there and soil conditions may need additional measures to support the weight of such tall buildings. Were there some engineering studies on that topic?

6. Before the pandemic, Lyon onramp to the Queensway are often backed up during rush hour since the Catherine to Arlington block is a very short block and there is a traffic light at Catherine and Lyon. With the additional cars from the development that would be more of a bottleneck. Has a traffic study been done?

Alice Nakanishi:

1. I think there will be concerns from the neighbours about a 36-storey tower and a 40-storey tower with a 6-storey podium when the nearby newer developments on Catherine Street have a number

of storeys in the 20s. We recently had a meeting for another development where the City planner suggested that a 4-storey podium would provide a better experience at the pedestrian level. Pedestrians would find a 6-storey podium more overpowering than a 4-storey podium.

- 2. I am concerned with any blasting that may be used to build the underground parking lot. The rest of the neighbourhood has older heritage houses. Owners would be concerned about any damage to their property due to vibrations from blasting.
- 3. We ask for more family sized units. Glashan Public School for Grades 7 & 8 is across the street from this site. Glebe C.I. and Lisgar C. I high schools are within walking distance.
- 4. We ask for more bike parking and visitor parking spots, as well as electrical vehicle charging stations.
- 5. Appreciate the plans showing that nothing, e.g., parking lot, would be built underneath where the trees would be planted. We have been fooled by another developer who had shown locations of trees on their site plan; however, it wasn't pointed out the walls of the underground parking lot would be a couple of metres from the surface. The trees had no chance of surviving.
- 6. Appreciate the plans showing open space for the public to walk through, as well as to sit and enjoy. Opportunities for art performances and gardens. Opportunities for retail businesses and professional services.
- 7. The entrance to the underground parking should probably be closer to Lyon Street instead of Kent Street. With vehicles coming off the Queensway there would be a bottleneck of traffic flow if the parking entrance is closer to Kent & Catherine Streets.
- 8. We ask that some units will be more affordable.
- 9. We thank the developer for engaging the community much earlier in the planning process and allow us the opportunity to provide feedback. Since this site has at least 3 sides facing a street I understood that it can qualify and be developed as a landmark site (or something like that). I had suggested to the developer at one of the earlier meetings that since people, especially tourists, coming off the Queensway would probably see this site first then maybe it can be developed and viewed as a gateway to the downtown core. A positive experience as they head downtown.

<u>Other</u>

You are encouraged to contact the Ward Councillor, Catherine McKenney

Please refer to the links to "<u>Guide to preparing studies and plans</u>" and <u>fees</u> for general information. Additional information is available related to <u>building permits</u>, <u>development charges</u>, and the <u>Accessibility Design Standards</u>. Be aware that other fees and permits may be required, outside of the development review process. You may obtain background drawings by contacting <u>informationcentre@ottawa.ca</u>.

These pre-con comments are valid for one year. If you submit a development application(s) after this time, you may be required to meet for another pre-consultation meeting and/or the submission requirements may change. You are as well encouraged to contact us for a follow-up meeting if the plan/concept will be further refined.

Please do not hesitate to contact me if you have any questions.

Regards,

John Bernier, MCIP, RPP

Planner II | **Urbaniste II** Development Review, Central | *Examen des projets d'aménagement, Central* Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement City of Ottawa | *Ville d'Ottawa* 110 Laurier Avenue West. Ottawa, ON | *110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1* 613.580.2424 ext./poste 21576 <u>ottawa.ca/planning</u> / <u>ottawa.ca/urbanisme</u>

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Appendix B Water Demands

B.1 Domestic Water Demands

265 Catherine Street, Ottawa, ON - Domestic Water Demand Estimates

Site Plan provided by Quadrangle Architects Ltd. (2024-02-12) Project No. 160401663

Guid	elines:		
1 Bedroom	1.4	ppu	
2 Bedroom	2.1	ppu	
3 Bedroom	3.1	ppu	
Townhouses	2.7	рри	
Townhouses Demand conversion factors pr Water Design Guidelines and " Residential	r Table 4.2 of th	ppu e City of Ottawa	

Building ID	Commercial (m²)	No. of Units	Population	Avg Day Demand		Max Day	1 2 Demand	Peak Hour	1 2 Demand
	(11-)	Units		(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Building A									
Bachelor		80	112	21.8	0.36	54.4	0.91	119.8	2.00
1 Bedroom		91	127	24.8	0.41	61.9	1.03	136.2	2.27
1 Bedroom + Den ³		69	145	28.2	0.47	70.4	1.17	155.0	2.58
2 Bedroom		125	263	51.0	0.85	127.6	2.13	280.7	4.68
2 Bedroom + Den ³		30	93	18.1	0.30	45.2	0.75	99.5	1.66
3 Bedroom		5	16	3.0	0.05	7.5	0.13	16.6	0.28
Commercial	1299.2			2.5	0.04	3.8	0.06	6.8	0.11
Building A Residential Total		400	755	146.9	2.45	367.2	6.12	807.8	13.46
Phase 1 Subtotal	1299.2	400	755	149.39	2.49	370.95	6.18	814.57	13.58
Building B									
Bachelor		87	122	23.7	0.39	59.2	0.99	130.3	2.17
1 Bedroom		169	237	46.0	0.77	115.0	1.92	253.0	4.22
1 Bedroom + Den ³		189	397	77.2	1.29	192.9	3.22	424.5	7.07
2 Bedroom		245	515	100.0	1.67	250.1	4.17	550.2	9.17
3 Bedroom		37	115	22.3	0.37	55.8	0.93	122.7	2.04
Commercial	1123.8			2.2	0.04	3.3	0.05	5.9	0.10
Building B Residential Total		727	1385	269.2	4.49	673.0	11.22	1480.6	24.68
Building B Subtotal	1123.8	727	1385	209.2	4.49	676.3	11.22	1480.0	24.08
Building B Subiolai	1123.0	121	1365	271.4	4.52	070.3	11.27	1400.5	24.70
Building C									
Townhouse		7	19	3.7	0.06	9.2	0.15	20.2	0.34
Phase 2 Subtotal	1123.8	734	1403	275.07	4.58	685.49	11.42	1506.76	25.11
Total Site :	2423	1134	2159	424.5	7.1	1056.4	17.6	2321.3	38.7

1 The City of Ottawa water demand criteria used to estimate peak demand rates for residential areas are as follows:

maximum day demand rate = 2.5 x average day demand rate

peak hour demand rate = 2.2 x maximum day demand rate (as per Technical Bulletin ISD-2010-02)

2 Water demand criteria used to estimate peak demand rates for long-term care units based on commercial areas and are as follows:

maximum daily demand rate = 1.5 x average day demand rate

peak hour demand rate = 1.8 x maximum day demand rate (as per Technical Bulletin ISD-2010-02)

3 Assumption that "1 bedroom with den" has density of 2.1 ppu, "2-bedroom with den" has density of 3.1 ppu

265 Catherine Street Servicing and Stormwater Management Report Water Demands

B.2 Fire Flow Demands (FUS 2020)

FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines

Stantec Project #: 160401663 Project Name: 265 Catherine Street Date: 2023-03-22 Fire Flow Calculation #: 1

Description: Building A, 6-Storey Podium and 26-Storey High-Rise Tower Podium Footprint: 2455.9 m². Tower Footprint: 750 m².

Notes: Footprint areas as per Quadrangle Architects Limited Site Plan provided March 17, 2023.

Step	Task			Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction								-
2	Determine Effective	Sum of	Sum of Largest Floor + 25% of Two Additional Floors Vertical Openings Protected?						YES	-
2	Floor Area	2455.9 2455.9 2195.8							3618.825	-
3	Determine Required Fire Flow				(F = 220 x C	x A ^{1/2}). Rour	nd to nearest 1000 L/mi	n	-	11000
4	Determine Occupancy Charae					Limited Co	mbustible		-15%	9350
						Conforms	to NFPA 13		-30%	
5	Determine Sprinkler					Standard W	ater Supply		-10%	-4675
Ĵ	Reduction					Fully Sup	pervised		-10%	
					% C	0	Sprinkler System		100%	
		Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinklered ?	-	-
	Determine Increase	North	20.1 to 30	75.74	3	> 100	Type V	NO	10%	
6	for Exposures (Max. 75%)	East	> 30	0	0	0-20	Type V	NO	0%	3273
		South	20.1 to 30	75.82	2	> 100	Type V	NO	10%	5275
		West	10.1 to 20	52.59	3	> 100	Type V	NO	15%	
					Total Requi	red Fire Flow	in L/min, Rounded to N	learest 1000L/min		8000
7	Determine Final					Total R	equired Fire Flow in L/s			133.3
Í	Required Fire Flow					Required	Duration of Fire Flow (h	nrs)		2.00
						Required	l Volume of Fire Flow (n	n³)		960



FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines Stantec

Stantec Project #: 160401663 Project Name: 265 Catherine Street Date: 2023-03-22 Fire Flow Calculation #: 2

Description: Building B, 6-Storey Podium with 36-Storey and 40-Storey High-Rise Towers Podium Footprint: 2968.9 m². Tower Footprint: 750 m² each.

Notes: Footprint areas as per Quadrangle Architects Limited Site Plan provided March 17, 2023.

Step	Task				Value Used	Req'd Fire Flow (L/min)					
1	Determine Type of Construction	Type II - Noncombustible Construction / Type IV-A - Mass Timber Construction									-
2	Determine Effective	Sum of	Sum of Largest Floor + 25% of Two Additional Floors Vertical Openings Protected?							YES	-
2	Floor Area	2968.9 2665.5 2665.5							4301.65	-	
3	Determine Required Fire Flow				(F = 220 x C	x A ^{1/2}). Rour	nd to nearest 1000 L/min			-	12000
4	Determine Occupancy Charae					Limited Co	ombustible			-15%	10200
						Conforms	to NFPA 13			-30%	
5	Determine Sprinkler					Standard W	ater Supply			-10%	-5100
Ĵ	Reduction					Fully Su	pervised			-10%	
					% C	· · ·	Sprinkler System			100%	
		Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of Adjacent Wall	Firewall / Sprinkler	red ?	-	-
	Determine Increase	North	10.1 to 20	83.57	3	> 100	Туре V	NO		15%	
6	for Exposures (Max. 75%)	East	3.1 to 10	52.96	3	> 100	Type V	NO		20%	4590
		South	20.1 to 30	83.57	2	> 100	Type I-II - Unprotected Openings	YES		0%	4370
		West	20.1 to 30	52.96	3	> 100	Type V	NO		10%	
					Total Requi	ed Fire Flow	in L/min, Rounded to Ne	arest 1000L/min			10000
7	Determine Final					Total F	Required Fire Flow in L/s				166.7
	Required Fire Flow					Required	Duration of Fire Flow (hrs	5)			2.00
						Required	l Volume of Fire Flow (m ³)			1200

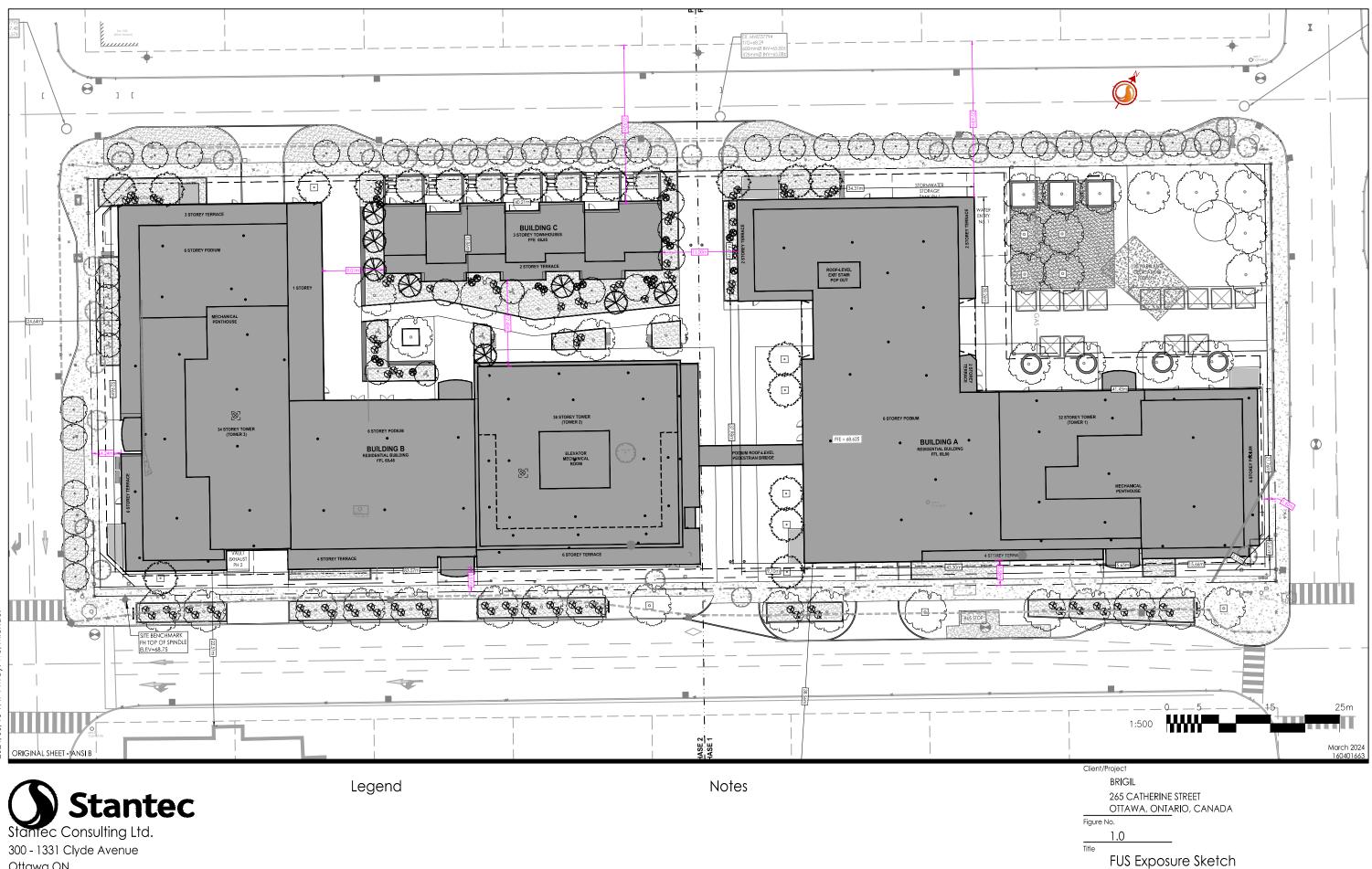
FUS Fire Flow Calculation Sheet - 2020 FUS Guidelines Stantec

Stantec Project #: 160401663 Project Name: 265 Catherine Street Date: 2023-03-22 Fire Flow Calculation #: 3

Description: Townhouses Footprint: 404.4 m².

Notes: Footprint areas as per Quadrangle Architects Limited Site Plan provided March 17, 2023.

Step	Task	Notes										Value Used	Req'd Fire Flow (L/min)
1	Determine Type of Construction		Type V - Wood Frame / Type IV-D - Mass Timber Construction								1.5	-	
2	Determine Effective		Sum of All Floor Areas							-	-		
2	Floor Area	404.4	404.4 404.4 284.1							1092.9	-		
3	Determine Required Fire Flow Determine				(F = 220 x C	x A ^{1/2}). Rour	nd to nearest 100	00 L/min				-	11000
4	Determine Occupancy Charae					Limited Co	ombustible					-15%	9350
						No	ne					0%	
5	Determine Sprinkler				Non-	Standard Wo	ater Supply or N/	Ά				0%	0
Ŭ	Reduction				N	lot Fully Supe	ervised or N/A					0%	0
					% C		Sprinkler System					0%	
		Direction	Exposure Distance (m)	Exposed Length (m)	Exposed Height (Stories)	Length-Height Factor (m x stories)	Construction of A Wall	Adjacent	Fire	wall / Sprinkle	ered ?	-	-
	Determine Increase	North	20.1 to 30	40.21	3	> 100	Туре V			NO		10%	
6	for Exposures (Max. 75%)	East	10.1 to 20	10.86	3	21-49	Type I-II - Unprotected	d Openings		YES		0%	935
		South	10.1 to 20	40.21	2	81-100	Type I-II - Unprotected	d Openings		YES		0%	/35
		West	3.1 to 10	10.86	3	21-49	Type I-II - Unprotected	d Openings		YES		0%	
					Total Requi	red Fire Flow	in L/min, Round	ed to Near	est 1000L/	min			10000
7	Determine Final		Total Required Fire Flow in L/s								166.7		
Ĺ	Required Fire Flow					Required	Duration of Fire	Flow (hrs)					2.00
						Required	Volume of Fire	Flow (m ³)					1200



Ottawa ON Tel. 613.722.4420 www.stantec.com

B.3 Boundary Conditions

From:	Fawzi, Mohammed
То:	Wu, Michael
Cc:	Kilborn, Kris; Thiffault, Dustin; Sharp, Mike
Subject:	RE: Updated Boundary Conditions Request - 265 Catherine Street
Date:	Tuesday, April 30, 2024 10:00:53 AM
Attachments:	~WRD0004.jpg
	265 Catherine Street REVISED April 2024.pdf

Hi Michael,

The following are boundary conditions, HGL, for hydraulic analysis at 265 Catherine Street (zone 1W) assumed to be connected via two connections. One connection is from Kent Street (with watermain upgrade to a 203 mm from Arlington Avenue to Catherine Street) and the second connection is from the 203mm watermain on Arlington Avenue (see attached PDF for location).

Note:

• Boundary conditions are provided with a watermain upgrade on Kent Street to a 203 mm, from Arlington Avenue to Catherine Street.

Both Connections:

Minimum HGL: 106.3 m Maximum HGL: 115.3 m

Connection 1 (Kent):

Max Day + Fire Flow (166.7 L/s): 94.2 m

<u>Connection 2 (Arlington):</u> Max Day + Fire Flow (166.7 L/s): 96.7 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager, Infrastructure - Gestionnaire de projet, Projets d'infrastructure Development Review All Wards (DRAW) | Direction de l'examen des projets d'aménagement - Tous les quartiers (EPATQ) Planning, Development and Building Services Department (PDBS)| Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB) City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West | 110 Avenue Laurier Ouest Ottawa, ON K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

From: Wu, Michael <Michael.Wu@stantec.com>
Sent: Wednesday, April 10, 2024 8:58 AM
To: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>
Cc: Kilborn, Kris <kris.kilborn@stantec.com>; Thiffault, Dustin <dustin.thiffault@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>
Subject: Updated Boundary Conditions Request - 265 Catherine Street

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Good morning, Mohammed:

As per the request yesterday afternoon, we are requesting updated boundary conditions for the proposed development at 265 Catherine Street, which is projected to service a total population of 2159 persons with 2423 m² of commercial space and will be served by the watermains on Arlington Avenue and Kent Street.

The request is for the scenario in which **only** the Kent Street watermain is upsized to 200 mm diameter while the Catherine Street watermain **remains unchanged**.

The updated water demands for the proposed development are as follows:

- Average Day Demand: 7.1 L/s (424.5 L/min)
- Maximum Day Demand: 17.6 L/s (1,056.4 L/min)
- Peak Hour Demand: 38.7 L/s (2,321.3 L/min)
- Fire Flow Demand: 166.7 L/s (10,000 L/min)

Attached are the calculation sheets for your reference.

We appreciate your time looking into this for us, and please feel free to reach out if you have any questions or comments.

Thanks,

Michael Wu EIT Civil Engineering Intern, Community Development

Direct: 1 (613) 738-6033 Michael.Wu@stantec.com

Stantec 300-1331 Clyde Avenue Ottawa ON K2C 3G4





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265 Catherine Street

No. 160401663

SITE PLAN HYDRAULIC ANALYSIS

Revision: Revision Date:

Project:

01 14-May-2024 Prepared By: MW Checked By: DT

BOUNDARY CONDITIONS (BC)							
Connection at Kent Street							
Site Plan Revision Date	12-Feb-2024						
Min. HGL (m)	106.3						
Max. HGL (m)	115.3						
Max. Day + Fire Flow (166.67 L/s)	94.2						
Max. Day + Fire Flow (166.67 L/s)	94.2						

Ground Floor Elevation (GFE) (Level 01) (m)

	GROUND FLOOR (GF) PRESSURE RANGE										
	GF HGL GF Pressure GF Pressure (m) (kPa) (psi) Outcome										
	= BC HGL (m) - FFE (m)	= GF HGL (m) x 9.804 (kPa/m)	= GF Pressure (kPA) x 0.145 (psi/kPa)	If min <50 psi: booster pump If max >100 psi: pressure reducer							
Minimum Normal	37.3	365.7	53.0	No Booster Pump Required							
Maximum Normal	46.3	453.9	65.8	No Pressure Reducer Required							

69

Number of Floors Above Ground	40
Approximate Height of One Storey (m)	3
Pressure Drop Per Floor (kPa)	29.4
Pressure Drop Per Floor (psi)	4.3

R	RESIDUAL PRESSURE RANGE IN MULTI-LEVEL BUILDINGS									
	Residual Pressure (kPa)	Residual Pressure (psi)	Outcome							
Top Floor Min	-781.4	-113.3								
Top Floor Max	-693.2	-100.5								
Maximum Number of Floors Above Ground at Minimum Pressure	3		Booster Pump Required							

RESIDUAL PRESSURE UNDER FIRE FLOW CONDITIONS							
	Residual HGL (m)	Residual Pressure	Residual Pressure				
	Residual HGL (III)	(kPa)	(psi)				
Ground Floor	25.2	247.1	35.8				
Top Floor	-91.8	-900.0	-130.5				

PRESSURE	PRESSURE CHECK													
	Pressure	Pressure												
	(kPa)	(psi)												
UNDER NORMAL OPER	ATING CONDITION	IS												
Pressure Below Minimum	<276	<40												
Pressure Below Normal	276-345	40-50												
Pressure Within Normal Range	345-552	50-80												
Pressure Above Normal Range	552-690	80-100												
Pressure Above Maximum	>690	>100												
UNDER FIRE FLOW	V CONDITIONS													
Pressure Below Minimum	<140	<20												
Acceptable Pressure	≥140	≥20												



265 Catherine Street

No. 160401663

SITE PLAN HYDRAULIC ANALYSIS

Revision: Revision Date:

Project:

01 14-May-2024 Prepared By: MW Checked By: DT

BOUNDARY CONDITIONS (BC)
Connection at Arlington Avenu	e
Site Plan Revision Date	12-Feb-2024
Min. HGL (m)	106.3
Max. HGL (m)	115.3
Max. Day + Fire Flow (166.67 L/s)	96.7

Ground Floor Elevation (GFE) (Level 01) (m)

	GROUND FLOOR (GF) PRESSURE RANGE													
	GF HGL (m)	GF Pressure (kPa)	GF Pressure (psi)	Outcome										
	= BC HGL (m) - FFE (m)	= GF HGL (m) x 9.804 (kPa/m)	= GF Pressure (kPA) x 0.145 (psi/kPa)	If min <50 psi: booster pump If max >100 psi: pressure reducer										
Minimum Normal	37.3	365.7	53.0	No Booster Pump Required										
Maximum Normal	46.3	453.9	65.8	No Pressure Reducer Required										

69

Number of Floors Above Ground	40
Approximate Height of One Storey (m)	3
Pressure Drop Per Floor (kPa)	29.4
Pressure Drop Per Floor (psi)	4.3

R	ESIDUAL PRESSURE R	RANGE IN MULTI-LE	EVEL BUILDINGS
	Residual Pressure (kPa)	Residual Pressure (psi)	Outcome
Top Floor Min	-781.4	-113.3	
Top Floor Max	-693.2	-100.5	
Maximum Number of Floors Above Ground at Minimum Pressure	3		Booster Pump Required

RESIDUAL PRESSURE UNDER FIRE FLOW CONDITIONS												
	Residual HGL (m)	Residual Pressure	Residual Pressure									
	Residual HGL (m)	(kPa)	(psi)									
Ground Floor	27.7	271.6	39.4									
Top Floor	-89.3	-875.5	-126.9									

PRESSURE	PRESSURE CHECK												
	Pressure	Pressure											
	(kPa)	(psi)											
UNDER NORMAL OPER	ATING CONDITION	IS											
Pressure Below Minimum	<276	<40											
Pressure Below Normal	276-345	40-50											
Pressure Within Normal Range	345-552	50-80											
Pressure Above Normal Range	552-690	80-100											
Pressure Above Maximum	>690	>100											
UNDER FIRE FLO	V CONDITIONS												
Pressure Below Minimum	<140	<20											
Acceptable Pressure	≥140	≥20											

B.4 Correspondence With Architect on Construction Type and Vertical Opening Protections

Wu, Michael

From:	Ryan Lupien <rlupien@bdpquadrangle.com></rlupien@bdpquadrangle.com>
Sent:	Wednesday, 19 April, 2023 13:42
То:	Wu, Michael
Cc:	Kilborn, Kris; Sharp, Mike; Ford, Matthew; Ghazi Ziben
Subject:	RE: 265 Catherine Street Roof Plan Question

Hi,

Yes, we will maintain all necessary fire-separations required by code. Fresh air shafts will be enclosed in a fire-rated assembly and will likely have fire-dampers behind the vents in the walls, not at the floors. The mechanical engineer will specify fire-dampers where they are required between fire compartments.

Does this answer your question? I'm not quite sure I understand the reason for asking.

Regards,

BDP Quadrangle Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Wu, Michael <Michael.Wu@stantec.com>
Sent: Wednesday, April 19, 2023 1:14 PM
To: Ryan Lupien <rlupien@bdpquadrangle.com>
Cc: Kilborn, Kris <kris.kilborn@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>; Ford, Matthew
<Matthew.Ford@stantec.com>; Ghazi Ziben <GZiben@bdpquadrangle.com>
Subject: RE: 265 Catherine Street Roof Plan Question

Good afternoon, Ryan, one quick follow-up:

Can you confirm that the vertical openings (between floors) for Buildings A and B are going to be **protected** per the fire code requirements outlined in the Ontario and National Building Codes?

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Wu, Michael
Sent: Wednesday, 19 April, 2023 09:38
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>
Cc: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>
Subject: RE: 265 Catherine Street Roof Plan Question

Perfect, thanks for the update.

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>
Sent: Wednesday, 19 April, 2023 09:36
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>
Subject: RE: 265 Catherine Street Roof Plan Question

Hi Michael,

Yes, we will include bi-level roof drains on the roof of the podium – we are just waiting to receive the landscape design to place into our drawings. Generally, we will slope the concrete under the roof topping to a maximum of 100 mm; therefore, a drain will be placed roughly 5 m in from the perimeter and be spaced roughly 10 m apart internally to achieve a 2% slope. Additionally, we will be required by code to include for emergency overflow scuppers on all roofs and terraces every 30 m which we will add in during detailed design at building permit stage. We will also add in drains in our model to the site to match the coordinated landscape/civil design locations.

Regards,

BDP Quadrangle

Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Wu, Michael <<u>Michael.Wu@stantec.com</u>> Sent: Wednesday, April 19, 2023 9:29 AM To: Ryan Lupien <rlupien@bdpquadrangle.com> Cc: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>> Subject: 265 Catherine Street Roof Plan Question

Good morning, Ryan:

I have a quick question on the site plan provided on April 17th. In the roof plan, it seems that only the three towers are shown to be equipped with roof drains, whereas the seventh-floor podiums, including the pedestrian bridge, and the townhouses are not shown to have any drains.

As such, I was wondering if you could confirm that there would be drains for the seventh-floor podiums, the pedestrian bridge and for the townhouses or if updated roof plans can be provided.

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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Wu, Michael

From:Ryan Lupien <rlupien@bdpquadrangle.com>Sent:Thursday, 23 March, 2023 09:52To:Wu, Michael; Kilborn, KrisCc:Ghazi Ziben; Ford, Matthew; Sharp, MikeSubject:RE: 265 Catherine Street - Grading

Hi Michael,

Buildings A and B are high-rise residential and will be cast-in-place concrete construction and they will be required to be sprinklered including the parking garage and possibly the pedestrian link between the market and the art space. We are hoping that the townhouse Building C will also be constructed out of cast-in-place concrete, but it is possible that this building could be constructed as a Part 9 building and be constructed out of wood and it may not need to be sprinklered as a result.

We will be locating a fire department connection at each of the main entrances along Catherine Street – we don't know if we'll need additional connections at this point. We would prefer free-standing connections, so we don't have to interrupt the building façade. We still need to locate them on our drawing, but we also haven't been able to identify the locations of all the fire hydrants – it looks like there is one at the southwest corner of the site. Perhaps you could highlight them for us and propose new ones if needed? The fire department connection/main entrances need to be within 45 m of the nearest fire hydrant.

Thank you,

BDP Quadrangle

Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Wu, Michael <Michael.Wu@stantec.com>
Sent: Thursday, March 23, 2023 9:28 AM
To: Ryan Lupien <rlupien@bdpquadrangle.com>; Kilborn, Kris <kris.kilborn@stantec.com>
Cc: Ghazi Ziben <GZiben@bdpquadrangle.com>; Ford, Matthew <Matthew.Ford@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>
Subject: RE: 265 Catherine Street - Grading

Good morning, Ryan:

I was wondering if you could provide us the construction type for the three buildings and whether they will be sprinklered, we would need them for when we request the hydraulic boundary conditions from the City.

As well, could you also provide us the fire department connection locations?

Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Thursday, 23 March, 2023 07:09
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Jennifer Hemmings <<u>jhemmings@nak-design.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street - Grading

Thanks Ryan

I reviewed the pre-consult documents and there was no mention of services to be provided. The city usually requests dedicated stm, san and water services be provided for parks. I will propose them for SPA and we can get feedback.

Thanks Mike

Mike Sharp C.E.T.

Civil Engineering Technologist

Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4 Office: (613) 722-4420



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From: Ryan Lupien <rlupien@bdpquadrangle.com>
Sent: Wednesday, March 22, 2023 4:49 PM
To: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com></u>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Jennifer Hemmings <<u>jhemmings@nak-design.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Shannon, Mike,

Mike, I'm not sure the answer to your question about the park area being included in the calculations or not, but we had discussed that Landscape will propose a design for the park area for the submission. We adjusted the grade of the exit and moving room on the west side of Building B to suit your markup. We have not yet done a review of each of the grade points in your drawing yet.

Please find attached a slightly updated CAD plan. There were two minimal building footprint changes at the main residential entrances along Catherine Street. The client requested that we provide a narrower entrance design. This will affect the two planting elements in those locations unfortunately. We removed all our outdated landscape background elements except the retractable bollards. We'll replace with a new landscape CAD background once it is ready. We will eventually have to show some type of area boundary to prove that there is at least 25% pedestrian open space as per the zoning bylaw.

The air shafts are highlighted. I think most of them will be fine, however the one along at the north of the market shifts the transformer over to the east a small amount. Additionally, we would like to maintain an entrance into the market from the middle window segment so that we can keep all the windows on the east end as overhead doors to allow the market to open. This will affect the trees there. Will you be able to work with this? Could you locate a separate tree in front of each of the building piers instead?

The air shaft at the southeast corner of the park will affect the paving pattern you have planned there. This shaft could shift south to be closer to the building if that helps. Let me know what is preferred.

We could have a call between us if you like tomorrow.

Thank you,

BDP Quadrangle

Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Wednesday, March 22, 2023 10:41 AM
To: Shannon Card <<u>scard@nak-design.com</u>>; Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Jennifer Hemmings <<u>jhemmings@nak-design.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Ryan, Shannon

Are we designing the park servicing and grading as well? Some projects I've been on we do and others we don't. The landscaping shows design features in the park and I just need to know if I'm including this area in the storm sewer design.

Thanks Mike

Mike Sharp C.E.T. Civil Engineering Technologist Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

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From: Shannon Card <<u>scard@nak-design.com</u>>
Sent: Tuesday, March 21, 2023 4:35 PM
To: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Jennifer Hemmings <<u>jhemmings@nak-design.com</u>>; Ford, Matthew
<<u>Matthew.Ford@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Mike, Ryan,

Please see attached landscape ground floor for coordination. Note that I will be issuing a full design update tomorrow morning that will aid in describing the updated concept – but wanted to get the CAD out to start coordinating grading, services etc.

Mike, we're proposing the amenity space on the west end of the site have some level of exposed stormwater management incorporated. Essentially, we'd like to explore incorporating permeable pavements or even elevated pavements with sunken planting beds – like a concrete boardwalk. We will share some precedent imagery with the design update tomorrow morning that will aid in illustrating the design intent on this side, but happy to have a call to discuss this in more detail and review design alternatives.

Thank you, Shannon

From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Tuesday, March 21, 2023 11:51 AM
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Jennifer Hemmings
<<u>ihemmings@nak-design.com</u>>; Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Wu, Michael
<<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street - Grading

Good morning, Ryan, please see latest grading for coordination. Please review the secondary exit and moving entrance for building B. We had to adjust the grades slightly to accommodate existing tie ins. I think all other access points are working well.

We are currently working on SPA design drawings and have been coordinating with Shannon with the landscape development. Once we have landscape, we will be able to provide you with a cistern volume required.

As the plans develop, we will have area drains (AD) and trench drains (TD) to coordinate with mechanical, structural, architectural, landscape. Preliminary locations have been indicated on the grading plan. These will be required to connect through the building and into the stormwater cistern.

Take care, Mike

Mike Sharp C.E.T. Civil Engineering Technologist

Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

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From: Ryan Lupien <rlupien@bdpquadrangle.com>
Sent: Monday, February 27, 2023 9:16 AM
To: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Jennifer Hemmings
<<u>ihemmings@nak-design.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Mike,

Okay, we will work with the proposed grades. Thank you for letting me know.

Regards,

BDP Quadrangle Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Monday, February 27, 2023 9:13 AM
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Jennifer Hemmings
<<u>jhemmings@nak-design.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Ryan, I really can't lower that entrance, I'm already at min slope out to the road. If I add low points with catchbasins, there will be a ponding elevation. I then must set any floors in that area 0.3m above the ponding area. If you want that to use the 68.65 as the FFE everywhere then you will have to accommodate a ramp at the front entrance and adjust the floor elevations at the loading doors, and make additional ramps/stairs that have access to Lyon Street.

Mike Sharp C.E.T. Civil Engineering Technologist

Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4 Office: (613) 722-4420



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From: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>
Sent: Friday, February 24, 2023 1:38 PM
To: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>; Shannon Card <<u>scard@nak-design.com</u>>; Jennifer Hemmings
<<u>jhemmings@nak-design.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Mike,

Thanks for the update. Things are looking good. One question though, if you refer to the attached, would it be possible to get the secondary residential entrance at the back to be the same elevation as the main lobby? We can work with keeping the amenity to the right at 68.65, but we will have to work out some location to make this transition. We can make the steps in the Artspace and market work.

We'll now use these lobby grades to start adjusting our model.

Thank you,

BDP Quadrangle Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Friday, February 24, 2023 11:07 AM
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>
Subject: RE: 265 Catherine Street - Grading

Good morning, Ryan. Please see attached the revised grading as discussed. Please review and let me know if you have any comments. Thx

Mike

Mike Sharp C.E.T.

Civil Engineering Technologist

Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

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From: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>
Sent: Wednesday, February 22, 2023 3:12 PM
To: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi,

I am available for the rest of today up until about 5:00. Tomorrow is a bit busy with meetings and I'm free most of the day on Friday. If you are able today, it would be preferable to allow us to get going on the modelling work on our end.

Let me know.

Thanks,

BDP Quadrangle Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

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From: Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Sent: Wednesday, February 22, 2023 2:44 PM
To: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>
Subject: RE: 265 Catherine Street - Grading

Hi Ryan, I have reviewed the mark ups, and I was wondering if we could have a quick meeting to discuss. I think in general all comments can be accommodated with a few minor grading changes and with the addition of ramps in the building as shown on the markup. I want to ensure I understand where all the access points are etc. I hope to be able to provide something back shortly, using the FFE's that I have started with and the ones that have been added on the markup.

Thanks

Mike Sharp C.E.T.

Civil Engineering Technologist

Direct: 613 784-2208 Cell:613 558-5204 Fax: 613 722-2799 Mike.Sharp@stantec.com

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From: Ryan Lupien <<u>rlupien@bdpquadrangle.com</u>>
Sent: Wednesday, February 22, 2023 2:14 PM
To: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Cc: Ghazi Ziben <<u>GZiben@bdpquadrangle.com</u>>
Subject: 265 Catherine Street - Grading

Hi Kris, Mike,

We are wondering if you have had a chance to look at the grading mark-ups we sent. We would really like to be able to begin establishing the grades in the Revit model based on this coordination.

Please let us know what you think about our markups. We can have quick call to discuss as well.

Thanks,

BDP Quadrangle

Ryan Lupien (he/him/his) Senior Associate, Senior Architect t 416 598 1240 x 249

bdpquadrangle.com subscribe | twitter | instagram | linkedin

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B.5 Fire Hydrant Coverage Calculations

	Project:	265 Catheri	ine Street	160401663				
Stantec	TABLE 1: FIRE HYDRANT COVERAGE TABLE							
	Revision:	1	Prepared By:	MW				
	Revision Date:	2023-	05-10 Checked By:					

		Hydrants ¹		Total Available	Total Required									
Description	HYD-01	HYD-02	Proposed	Fire Flow (L/min)	Fire Flow ² (L/min)									
265 Catherine Street														
Distance from fire department connection (m)	41.7	49.1	21.7	-	-									
Maximum fire flow capacity ³ (L/min)	5,678	5,678	5,678	17,034	10,000									

le 18.5.4.3
Maximum
Capacity
(L/min)
5,678
3,785
2,839

Notes:

1. Hydrant locations as per GeoOttawa accessed May 10, 2023. Refer to fire hydrant coverage sketch (Figure 3-1).

2. See OBC Calculations, Appendix A.2 for fire flow requirements.

3. See NFPA 1 Table 18.5.4.3 in Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 for maxiumim fire flow capacity of hydrants by distance to building.

Appendix C Sanitary

C.1 Sanitary Calculation Sheet

			SUBDIVIS		erine Stree	ət	SANITARY SEWER DESIGN SHEET										DESIGN PARAMETERS																			
	Sta	ntec									(City of C	Ottawa)					MAX PEAK F	CTOR (RES.)	-	4.0		AVG. DAILY F	LOW / PERS	ON	280	l/p/day		MINIMUM VE	LOCITY		0.60	m/s				
	Jua	nicec	DATE:		2024-	-03-07											MIN PEAK FA	CTOR (RES.)=		2.0		COMMERCIA	L		28,000	l/ha/day		MAXIMUM VE			3.00	m/s				
			REVISIO			1											PEAKING FA	CTOR (INDUST	(RIAL):	2.4		INDUSTRIAL	(HEAVY)		55,000	l/ha/day		MANNINGS r	n		0.013					
			DESIGN		M	W	FILE NUMB	BER:		160401663								CTOR (ICI >209	%):	1.5		INDUSTRIAL	. ,			l/ha/day		BEDDING CL	LASS		В					
			CHECK	ED BY:		-											PERSONS / 1			1.4		INSTITUTION			28,000	l/ha/day		MINIMUM CC	OVER		2.50	m				
																	PERSONS / 2	BEDROOM		2.1		INFILTRATIO	N		0.33	l/s/Ha		HARMON CC	ORRECTION	N FACTOR	0.8					
																	PERSONS / 3	BEDROOM		3.1																
																	PERSONS / T	OWNHOME		2.7																
	LOC	CATION					RESIDE	ENTIAL AREA	AND POPUL	ATION				COMMERCIAL INDUS			RIAL (L)	INDUST	RIAL (H)	INSTITU	TIONAL	GREEN /	UNUSED	C+I+I		INFILTRATION	1	TOTAL				PII	IPE			
	REA ID	FROM	то	AREA					POP.	CUMUL		PEAK	PEAK	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	PEAK	TOTAL	ACCU.	INFILT.	FLOW	LENGT	H DIA	MATERIAL	CLASS	SLOPE	CAP.	CAP. V	VEL.
1	UMBER	M.H.	M.H.		1 BEDOOM	2 BEDROOM	3 BEDROOM	TOWN		AREA	POP.	FACT.	FLOW		AREA		AREA		AREA		AREA		AREA	FLOW	AREA	AREA	FLOW									1
				(ha)						(ha)			(I/S)	(na)	(ha)	(na)	(ha)	(ha)	(ha)	(ha)	(ha)	(na)	(ha)	(I/S)	(ha)	(ha)	(l/s)	(I/S)	(m)	(mm)			(%)	(l/s)	(%)	(m/s)
P2	(BLDG C)	BLDG C	LATERAL	2 0.04		0	0	7	10	0.040	19	3 507	0.2	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.086	0.086	0.0	0.126	0.126	0.0	0.3	10.1	300	PVC	SDR 35	2.00	135.8	0.19%	1.93
	(BLDG B)	BLDG B		0.30		434	37	0	1385	0.340	1403	2.960	13.5	0.112	0.112	0.000	0.000	0.000	0.000	0.000	0.000	0.086	0.000	0.0	0.120	0.624	0.0	13.7	10.1	300	PVC	SDR 35	2.00	135.8	10.11%	1.93
	(BLDG A)	BLDG A		1 0.24	171	194	35	0	755	0.580	2159	2.848	19.9	0.130	0.242	0.000	0.000	0.000	0.000	0.000	0.000	0.143	0.314	0.1	0.513	1.137	0.4	20.4	10.0	250	PVC	SDR 35	2.00	85.7	23.81%	1.73

265 Catherine Street Servicing and Stormwater Management Report Sanitary

C.2 Correspondence with City on Sanitary Sewer Capacity

Wu, Michael

From:	Kilborn, Kris
Sent:	Thursday, 27 April, 2023 09:54
То:	Fawzi, Mohammed
Cc:	Ford, Matthew; Sharp, Mike; Thiffault, Dustin; Wu, Michael
Subject:	RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Good morning Mohammed and thanks for the chat about 265 Catherine Project.

As discussed, our submission will include san and storm connections to the Arlington Street Sewer. The Development will have two sets of san/storm connections to the Arlington

Sewer to coincide with phasing and to ensure mechanical can design the site. As this is a large site spanning an entire City block.

As we have minimal room for onsite monitoring manholes we will be showing these within the municipal right of way. City can comment and the owner may have to enter into an encroachment agreement with the City. For now, we will show W3 Chambers on the water services (which will also be within the right of way) which you could review if chambers are required on all.

Thanks for checking in on the boundary conditions. It appears we will need to install new watermain along Catherine and Kent which we will show and submit in plan view only for this submission.

Sincerely

Kris Kilborn

Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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Please note our reception is on the 3rd floor.

From: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>
Sent: Wednesday, April 26, 2023 10:54 AM
To: Kilborn, Kris <kris.kilborn@stantec.com>
Cc: Ford, Matthew <Matthew.Ford@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>; Thiffault, Dustin <Dustin.Thiffault@stantec.com>; Wu, Michael <Michael.Wu@stantec.com>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Kris,

I'm available today until 4:00pm – so feel free to send an invite. I would recommend we include Asset Management though as I mentioned they are the ones who would approve a connection to a trunk sewer.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Sent: April 25, 2023 2:56 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>dustin.thiffault@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Hey Mohammed do you have time for a quick call this afternoon. Would just like to review our plans with you and show you our drawings and why We are requesting to connect to Catherine.

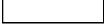
I could send out a team's meeting request. Thanks, and let me know.

Sincerely

Kris Kilborn

Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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Please note our reception is on the 3rd floor.

From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Tuesday, April 25, 2023 1:35 PM
To: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>Dustin.Thiffault@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Kris,

I can definitely get the City's Asset Management Branch involved in our discussion as they would be the ones ultimately making the decision as to whether or not we can connect to the trunk sewer. Prior to doing so, could you confirm why we cannot connect to the 450mm dia. pipe on Lyon Street?

Thanks Kris.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Sent: April 25, 2023 11:18 AM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>dustin.thiffault@stantec.com</u>>

Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Mohammed

We have design and installed connections to deep Trunk Sewers on other high rise projects in the City and we require this connection for our phase 1 development.

As you can appreciate this development is a full City block which will be constructed in phases and require connection to the 1800dia. In previous emails you mentioned that

We should not connect to the existing brick sewer on Arlington and our hands are a bit tied now.

Could we set up a call to discuss.

Sincerely

Kris Kilborn

Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4

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The Ottawa office is open however many staff are working remotely. To contact me please use email, or my mobile and leave a message.

Please note our reception is on the 3rd floor.

From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Tuesday, April 25, 2023 10:50 AM
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Unfortunately the City would not permit a connection to such a large deep trunk sewer.

Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 24, 2023 12:14 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>; Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Hi Mohammed, thanks for the response.

And as a quick follow-up, does the City have any objections to using the 1800 mm diameter combined sewer on Catherine Street for the sanitary and storm discharge from the site? As a refresher, we anticipate around 26.88 L/s of sanitary discharge from the site.

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Monday, 24 April, 2023 08:32
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

```
Hi Michael,
```

Thank you for the revised boundary conditions.

I can confirm there are no current scheduled City projects in the vicinity of the site.

Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 21, 2023 3:21 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>;
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

As a quick follow-up, we would like to submit revised boundary conditions for the 265 Catherine Street site with updated fire flow demands at the same connection points.

While the revised worst case fire flow demands from the site has been reduced to 166.7 L/s (10000 L/min), connecting to the Lyon Street North and Arlington Avenue 203 mm diameter watermains would be a challenge, not least by the site's servicing be consolidated at Catherine Street, where the building's main entrance will be at.

As such, as part of the updated boundary condition request, we would like to obtain the hydraulic boundary conditions for the site under the following scenarios:

1. Upsizing the Catherine Street watermain to a 203 mm diameter watermain from Kent Street to Lyon Street North only

2. Upsizing both the Catherine Street and Kent Street watermains to 203 mm diameter within the vicinity of the site

Attached are the revised fire flow calculations and sketches of the two proposed upsizing options detailing the range of the proposed upsizing.

In addition, please advise if there are other design considerations for other ongoing City projects in the vicinity that could impact the site.

Please let me know if you have any further questions or comments.

Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Wu, Michael
Sent: Wednesday, 19 April, 2023 15:35
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Good afternoon, Mohammed:

Just a quick question, is it possible for you to provide the HGL at the four connections under the max day + fire flow conditions with the fire flow demand of 166.7 L/s?

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Wednesday, 19 April, 2023 11:50
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Thank you for confirming.

As a heads up, please note that a letter or email confirmation by the architect confirming the parameters used in the fire flow calculations is required. Parameters to be confirmed applicable are confirming that the vertical openings are protected, type of construction, occupancy charge and sprinkler reductions. This can be appended to the Servicing Report.

Thanks Michael.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 18, 2023 4:37 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

As a follow-up, we have decided to update the fire flow demand calculation approach.

Specifically, for the FUS "protected vertical openings" classification for the two high-rise buildings on site, as the main openings between the floors are the elevator shafts and emergency exit stairwell, they would have already been subjected to the strictest fire protection measures outlined in the Ontario Building Code and the National Building Code, therefore it is reasoned that the two high-rises be classified as having protected vertical openings.

Under this approach, Building B's fire flow demand is reduced to 166.7 L/s (10,000 L/min), which is adequate for the watermains on Arlington Avenue and Lyon Street North to provide their respective fire flows while maintaining a residual pressure of 20 psi.

Please let me know if you have any questions or comments to this new approach.

Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Wu, Michael
Sent: Thursday, 13 April, 2023 13:23
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Perfect, thanks for the information.

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>> Sent: Thursday, 13 April, 2023 13:05 To: Wu, Michael <<u>Michael.Wu@stantec.com</u>> Hi Michael,

Please note that the Fire demand request of 316.7 L/s was not met. Next steps: Take measures to lower the fire demand, identify hydrants to request for a multi-hydrant analysis.

The following are boundary conditions, HGL, for hydraulic analysis at 265 Catherine Street (zone 1W) assumed to be connected to either the 127 mm watermain on Catherine Street, OR the 203 mm watermain on Lyon Street, OR the 203 mm watermain on Arlington Avenue, OR the 127 mm on Kent Street (see attached PDF for location).

Connection	Min HGL (m)	Maximum HGL (m)					
Catherine Street	80.4	115.3					
Lyon Street	104.9	115.3					
Arlington Avenue	105.9	115.3					
Kent Street	97.8	115.2					

Fire Flow:

Available Fire flow at 20 psi: 46 L/s assuming ground elevation of 68.2 m (Catherine Connection) Available Fire flow at 20 psi: 187 L/s assuming ground elevation of 67.6 m (Lyon Connection) Available Fire flow at 20 psi: 270 L/s assuming ground elevation of 68.0 m (Arlington Connection) Available Fire flow at 20 psi: 64 L/s assuming ground elevation of 68.6 m (Kent Connection)

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 11, 2023 11:45 AM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good morning, Mohammed, hope you are well. Just checking in to see when we could expect to receive the boundary conditions, the combined sewer capacity confirmation and the existing water consumption data for the site (if possible).

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Wednesday, 29 March, 2023 14:50
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Received.

I will get back to you as soon as possible. Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: March 29, 2023 1:18 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

In addition to the hydraulic boundary conditions, as part of the servicing for the proposed development on 265 Catherine Street, we would like to confirm if there is sufficient capacity downstream of the 450 mm diameter combined sewers in Lyon Street North, 300 mm diameter combined sewers in Catherine Street, 375 mm diameter combined sewers in Kent Street, and the 1200 mm diameter trunk combined sewers in Arlington Avenue to receive an additional peak flow of 26.8 L/s from the proposed development.

Please find our sanitary design sheet and location map attached for your information. Furthermore, we were wondering if there are any existing water consumption data for the site during its use as a Greyhound bus terminal.

Thank you,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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Appendix D Stormwater Servicing

D.1 Modified Rational Method Sheet

 File No:
 160401663

 Project:
 265 Catherine Street

 Date:
 14-Mar-24

SWM Approach: Post-development to Pre-development flows

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

0h. 2			Coefficient Table		Duncti			0
Sub-catchn Area	nent		Area (ha)		Runoff Coefficient			Overall Runoff
Catchment Type	ID / Description		"A"	·	"C"	"A	x C"	Coefficient
Uncontrolled - Park	UNC-PARK	Hard	0.002		0.9	0.002		
		Soft	0.010		0.2	0.002		
	Su	btotal		0.012			0.00396	0.330
Controlled - Park	PARK	Hard	0.056		0.9	0.050		
	e	Soft btotal	0.031	0.087	0.2	0.006	0.05655	0.650
		Diolai		0.067			0.05055	0.650
Uncontrolled - Non-Tributary	UNC-2	Hard	0.082		0.9	0.074		
	Su	Soft btotal	0.031	0.113	0.2	0.006	0.08023	0.710
		biolai		0.115			0.00025	0.710
Uncontrolled - Non-Tributary	UNC-1	Hard	0.035		0.9	0.032		
	Su	Soft btotal	0.002	0.037	0.2	0.000	0.03219	0.870
	Su	biolai		0.007			0.05219	0.070
Uncontrolled - Non-Tributary	CIST 2-4	Hard	0.041		0.9	0.037		
	e	Soft btotal	0.021	0.062	0.2	0.004	0.04092	0.660
	Su	DIOIAI		0.062			0.04092	0.000
Uncontrolled - Non-Tributary	CIST 2-3	Hard	0.032		0.9	0.029		
	e	Soft btotal	0.025	0.057	0.2	0.005	0.03363	0.590
	Su	DIOIAI		0.057			0.03303	0.590
Uncontrolled - Non-Tributary	CIST 2-2	Hard	0.041		0.9	0.037		
	0	Soft	0.000	0.044	0.2	0.000	0.0000	0.000
	Su	btotal		0.041			0.0369	0.900
Controlled - Tributary	CIST 2-1	Hard	0.304		0.9	0.274		
	0	Soft	0.000	0.004	0.2	0.000	0.0700	0.000
	Su	btotal		0.304			0.2736	0.900
Uncontrolled - Non-Tributary	CIST 1-4	Hard	0.007		0.9	0.006		
		Soft	0.000		0.2	0.000		
	Su	btotal		0.007			0.0063	0.900
Uncontrolled - Non-Tributary	CIST 1-3	Hard	0.011		0.9	0.010		
	_	Soft	0.000		0.2	0.000		
	Su	btotal		0.011			0.0099	0.900
Uncontrolled - Non-Tributary	CIST 1-2	Hard	0.022		0.9	0.020		
		Soft	0.000		0.2	0.000		
	Su	btotal		0.022			0.0198	0.900
Controlled - Tributary	CIST 1-1	Hard	0.241		0.9	0.217		
-		Soft	0.000		0.2	0.000		
	Su	btotal		0.241			0.2169	0.900
Total (Exclude Park) Overall Runoff Coefficient= C:				0.895			0.750	0.84
								0.04
Γotal Roof Areas Γotal Tributary Surface Areas (Con	trolled and Uncontrol	ed)	0.000 h 0.745 h					
otal Tributary Area to Outlet		,	0.745 h					
			. /					
otal Uncontrolled Areas (Non-Trib	outary)		0.150 h	а				

0.895 ha

Total Site (Exclude Park)

Date: 2024-03-14, 08:42 Stantec Consulting Ltd.

Stormwater Management Calculations

Project #160401663, 265 Catherine Street Modified Rational Method Calculations for Storage

a = 1735.688

l (mm/hr)

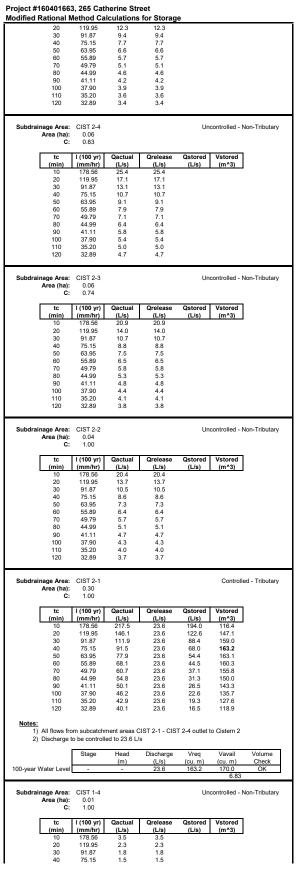
	2 yr Intensi	itv	$I = a/(t + b)^{c}$	a =	732.951	t (min)	l (mm/hr)
	2 yr intens City of Otta			a = b =	6.199	t (min) 10	76.81
	, 0. 010			c =	0.81	20	52.03
					-	30	40.04 32.86
						40 50	32.86 28.04
						60	24.56
						70 80	21.91 19.83
						80 90	19.83 18.14
						100	16.75
						110 120	15.57 14.56
					·		11.00
	2 YEA	AR Predeve	elopment I a	arget Releas	e from Por	tion of Site	
Subdrai	inage Area: Area (ha):	Predevelop 0.75	ment Tributar	y Area to Outl	et		
	Area (na): C:	0.40					
	Typical Tim	e of Concer	tration				
				l			
	tc (min)	l (2 yr) (mm/hr)	Qtarget (L/s)		Sanitary	20.4	L/s
	12	69.89	58.2		Target	37.8	L/s
	2 YEAR M	Aodified R	ational Metl	hod for Enti	re Site		
Subdrai	inage Area: Area (ha):	UNC-PARK 0.01				Uncont	olled - Park
	Area (na): C:	0.33					
	tc	l (5 yr)	Qactual	Qrelease	Qstored	Vstored	
	(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m^3)	
	10 20	76.81 52.03	0.8 0.6	0.8 0.6			
	30	40.04	0.4	0.4			
	40 50	32.86 28.04	0.4 0.3	0.4 0.3			
	60	24.56	0.3	0.3			
	70 80	21.91	0.2	0.2 0.2			
	90	19.83 18.14	0.2 0.2	0.2			
	100	16.75	0.2	0.2			
	110 120	15.57 14.56	0.2 0.2	0.2 0.2			
Subdrai	inage Area:	PARK				Cont	rolled - Park
	Area (ha): C:	0.09 0.65					
			Operiod	Orolaaar	Octored	Vetored	
	tc (min)	l (5 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m ³)	
	10	76.81	12.1	12.1	0.0	0.0	
	20 30	52.03 40.04	8.2 6.3	8.2 6.3	0.0 0.0	0.0 0.0	
	40 50	32.86	5.2	5.2	0.0	0.0	
	50	28.04 24.56	4.4 3.9	4.4 3.9	0.0 0.0	0.0 0.0	
	60						
	70	21.91	3.4	3.4	0.0	0.0	
					0.0 0.0 0.0	0.0 0.0 0.0	
	70 80 90 100	21.91 19.83 18.14 16.75	3.4 3.1 2.9 2.6	3.4 3.1 2.9 2.6	0.0 0.0 0.0	0.0 0.0 0.0	
	70 80 90 100 110	21.91 19.83 18.14 16.75 15.57	3.4 3.1 2.9 2.6 2.4	3.4 3.1 2.9 2.6 2.4	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
	70 80 90 100 110 120	21.91 19.83 18.14 16.75	3.4 3.1 2.9 2.6	3.4 3.1 2.9 2.6	0.0 0.0 0.0	0.0 0.0 0.0	
-	70 80 90 100 110 120 2 Above CB	21.91 19.83 18.14 16.75 15.57 14.56	3.4 3.1 2.9 2.6 2.4 2.3	3.4 3.1 2.9 2.6 2.4 2.3	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Orific	70 80 90 100 110 120 • Above CB	21.91 19.83 18.14 16.75 15.57 14.56	3.4 3.1 2.9 2.6 2.4 2.3	3.4 3.1 2.9 2.6 2.4	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Orific Orific Inve	70 80 90 100 110 120 Above CB ce Equation: ce Equation: ce Equation: ce Equation:	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^A 102.00 66.83	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm	3.4 3.1 2.9 2.6 2.4 2.3	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Orific Orific Inve	70 80 90 100 110 120 * Above CB ce Equation: ce Diameter: ert Elevation /G Elevation	21.91 19.83 18.14 16.75 15.57 14.56 • CdA(2gh) ^A 102.00 66.83 68.60	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m	3.4 3.1 2.9 2.6 2.4 2.3	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Orific Orific Inve T/ Max Po	70 80 90 100 110 120 Above CB ce Equation: ce Equation: ce Equation: ce Equation:	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^A 102.00 66.83	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm	3.4 3.1 2.9 2.6 2.4 2.3	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Orific Orific Inve T/ Max Po	70 80 90 100 110 120 • Above CB ce Equation: ce Equation: ce Equation: ce Elevation rd Elevation rd G Elevation	21.91 19.83 18.14 16.75 15.57 14.56 • CdA(2gh)^ 102.00 66.83 68.60 0.00 0.00	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m	3.4 3.1 2.9 2.6 2.4 2.3 Where C =	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	Volume
Orific Orific Inve T/ Max Po Down	70 80 90 100 110 120 • Above CB ce Equation: ce Diameter: ert Elevation nding Depth istream W/L	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^ 102.00 66.83 68.60 0.00 0.00 Stage	3.4 3.1 2.9 2.6 2.4 2.3 0.5 m m m m m Head (m)	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s)	0.0 0.0 0.0 0.0 0.0 0.57 Vreq (cu. m)	0.0 0.0 0.0 0.0 0.0 Vavail (cu. m)	Volume Check OK
Orific Orific Inve T/ Max Po Down	70 80 90 100 110 120 • Above CB ce Equation: ce Equation: ce Equation: ce Elevation rd Elevation rd G Elevation	21.91 19.83 18.14 16.75 15.57 14.56 • CdA(2gh)^ 102.00 66.83 68.60 0.00 0.00	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m m Head	3.4 3.1 2.9 2.6 2.4 2.3 Where C =	0.0 0.0 0.0 0.0 0.0 0.57	0.0 0.0 0.0 0.0 0.0 Vavail	
Orific Orific Inve T/ Max Po Down	70 80 90 100 110 120 * Above CB ce Equation: ee Diameter: art Elevation of G Elevation ding Depth histream W/L Water Level	21.91 19.83 18.14 16.75 15.57 14.56 • CdA(2gh)^ 102.00 66.83 68.60 0.00 0.00 0.00 Stage 68.60 UNC-2	3.4 3.1 2.9 2.6 2.4 2.3 0.5 m m m m m Head (m)	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.57	0.0 0.0 0.0 0.0 0.0 Vavail (cu. m)	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 100 110 120 * Above CB be Equation: the Diameter: art Elevation of G Elevation ding Depth histream W/L	21,91 19,83 18,14 16,75 15,57 14,56 = CdA(2gh)^ 102,00 66,83 68,60 0,00 Stage 68,60	3.4 3.1 2.9 2.6 2.4 2.3 0.5 m m m m m Head (m)	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.57	0.0 0.0 0.0 0.0 0.0 0.0 0.0 Vavail (cu. m) 4.7	Check OK
Orific Orific Inve T/ Max Po Down	70 80 90 110 120 * Above CB De Equation: * Diameter: et Elevation G Elevation G Elevation Multip Cepth Uster Level inage Area: Area (ha): C:	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^A 102.00 66.83 68.60 0.00 Stage 68.60 0.00 Stage UNC-2 0.11 0.71	3.4 3.1 2.9 2.6 2.4 2.3 mm m m Head (m) 1.77	3.4 3.1 2.9 2.6 2.4 2.3 Where C =	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un	0.0 0.0 0.0 0.0 0.0 0.0 0.0 4.7 controlled - N	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB ce Equation: exe Diameter: art Elevation onding Depth stream W/L Water Level inage Area: Area (ha): C: tc (min)	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^ 102.00 66.83 68.60 0.00 Stage 68.60 0.00 UNC-2 0.11 0.71 0.71 0.71	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m Head (m) 1.77 Qactual (L/s)	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) Qrelease (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.57	0.0 0.0 0.0 0.0 0.0 0.0 0.0 Vavail (cu. m) 4.7	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 3 Above CB es Equation: se Equation: se Equation: se Diameter: art Elevation G Elevation G Elevation G Elevation Ming Depth sstream W/L Water Level inage Area: Area (ha): C: tc (min) 10	21.91 19.83 18.14 16.75 15.57 14.56 = CdA(2gh)^{A} 102.00 66.83 68.60 0.00 0.00 0.00 Stage 68.60 UNC-2 0.11 0.71 I (2 yr) (mm/hr) 76.81	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m m m M Head (m) 1.77 Qactual (L/S) 17.1	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Uischarge (U/s) 27.5 Qrelease (U/s) 17.1	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 3 Above CB ∞ Equation: ex Equation: ex Diameter: ert Elevation GG Elevation GG Elevation GG Elevation Maing Depth sstream W/L Water Level inage Area: Area (ha): C: (min) 10 20 30	21,91 19,83 18,14 16,75 15,57 14,56 = CdA(2gh)^1 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 UNC-2 0.11 0.71 I (2yr) (mm/h) 76,81 52,03 40,04	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m m Head (m) 1.77 1.1 1.6 8.9	3.4 3.1 2.9 2.6 2.4 2.3 Where C =	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB ce Equation: ce Equation: ce Equation: e Diameter: art Elevation drig Depth ustream W/L Water Level inage Area: Area (ha): C: (min) 10 20 30 40	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0,00 0,00 0,00 0,00 Stage 68,60 0 8,60 0 0,00 0,00 0,00 0,00 0,00	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m Head (m) 1.77 1.71 11.6 8.9 7.3	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L(s) 27.5 27.5 27.5 27.5 27.5 27.5 27.5 27.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB cre Equation: e e Diameter: art Elevation of G Elevation of G Elevation of G Elevation of G Elevation ding Depth instream W/L Water Level inage Area: Area (ha): C: (min) 10 20 30 40 50 60	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0,00 0,00 0,00 0,00 0,00 0,00 0,0	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m Head (m) 1.77 1.71 11.6 8.9 7.3 6.3 5.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 17.1 11.6 8.9 7.3 6.3 5.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB • Equation: • ce Equati	21,91 19,83 19,83 14,14 16,75 15,57 14,56 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 I (2 yr) (mm/hr) 76,81 52,03 40,04 32,80 428,04 28,04 24,56 21,91	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m m Head (m) 1.77 1.1.6 8.9 7.3 6.3 5.5 4.9	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5 7.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB cre Equation: e e Diameter: art Elevation of G Elevation of G Elevation of G Elevation of G Elevation ding Depth instream W/L Water Level inage Area: Area (ha): C: (min) 10 20 30 40 50 60	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 I (2 yr) (mm/hy) 76,81 52,03 28,04 22,80 4,24,56 28,04 21,91 19,83 18,14	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m Head (m) 1.77 1.71 11.6 8.9 7.3 6.3 5.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 17.1 11.6 8.9 7.3 6.3 5.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 3 Above CB es Equation: er Equation: er Elevation GG Elevation GG Elevation GG Elevation GG Elevation GG Elevation C El	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0,00 0,00 0,00 0,00 0,00 0,00 0,0	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m Head (m) 1.77 17.1 11.6 8.9 7.3 6.3 5.5 4.9 4.4 4.0 3.7	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L(s) 27.5 Qrelease (L(s) 17.1 11.6 8.9 7.3 6.3 5.5 4.4 4.4 4.0 3.7	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB • e Equation: • e Equation: • e Diameter: • et levation nding Depth stream W/L Water Level • G Elevation • G Elevation • G Elevation • G Elevation • C E E E E E E E E E E E E E E E E E E	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 I (2 yr) (mm/hy) 76,81 52,03 28,04 22,80 4,24,56 28,04 21,91 19,83 18,14	3.4 3.1 2.9 2.6 2.4 2.3 0.5 mm m m m Head (m) 1.77 1.1 17.1 17.1 17.1 17.1 17.1 17.1	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 Qrelease (L/s) 17.1 17.1 17.1 17.1 17.1 17.1 17.1 17.	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve T/ Max Po Down 5-year	70 80 90 110 120 • Above CB De Equation: pe Equation: pe Equation: ref Elevation dig Depth stream W/L Water Level inage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 60 70 80 90 110	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0,00 0,00 Stage 68,60 UNC-2 0,11 0,71 I (2 yr) (mm/hr) 76,81 52,03 40,04 32,86 28,04 24,56 21,91 19,83 81,81 41,67 57 75,71 76,81 52,03 40,04 32,86 28,04 24,56 21,91 19,83 81,81 4 16,75 7 57 7 7 8,81 52,03 40,04 32,86 21,91 7 7 8,81 52,03 40,04 10,75 7 7 7 8,81 52,05 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,81 7 7 7 8,91 7 7 8 7 7 8 7 8 8 8 8 8 9 7 7 7 7 8 7 7 8 7 7 8 7 7 7 8 7 7 7 7	3.4 3.1 2.9 2.6 2.4 2.3 .5 mm m m Head (m) 1.77 1.1 1.77 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.3 6.3 5.5 4.9 4.4 0.3,7 3.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 4.9 4.4 4.0 3.7 3.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 Un Qstored	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Subdrai	70 80 90 110 120 • Above CB cre Equation: exe Equation: exe Equation: exe Equation: exe Equation: dig Depth ustream W/L Water Level inage Area: Area (ha): C: C: C: C: C: C: C: C: C: C: C: C: C:	21,91 19,83 18,14 16,75 15,57 14,56 102,00 66,83 68,60 0,00 0,00 0,00 Stage 0,86,60 0,00	3.4 3.1 2.9 2.6 2.4 2.3 .5 mm m m Head (m) 1.77 1.1 1.77 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.3 6.3 5.5 4.9 4.4 0.3,7 3.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 4.9 4.4 4.0 3.7 3.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 0.0 Un Un (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Check OK
Orific Orific Inve Tr Max Po Down 5-year Subdrai	70 80 90 110 120 3 Above CB 20 Equation: 20 Equation: 20 Equation: 4 Elevation (G Elevation 4 C Elevation 4 C Elevation 4 C Elevation 10 10 20 40 50 60 70 80 90 100 110 120 100 100 100 100 10	21,91 19,83 19,83 19,83 14,56 15,57 14,56 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 1(2yr) (mm/hr) 76,81 52,03 40,04 32,86 40,04 32,86 40,04 32,86 UNC-2 0.11 0.71 14,56 UNC-2 0.11 0.71 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.04 15,57 14,56 UNC-2 0.04 15,57 14,56 15,57 14,56 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 UNC-1 0.04	3.4 3.1 2.9 2.6 2.4 2.3 .5 mm m m Head (m) 1.77 1.1 1.77 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.3 6.3 5.5 4.9 4.4 0.3,7 3.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 4.9 4.4 4.0 3.7 3.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 0.0 Un Un (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.0 Vavail (cu. m) 4.7 controlled - N Vstored (m^3)	Check OK
Subdrai	70 80 90 110 120 • Above CB • Equation: exe Equation: exe Equation: exe Equation: exe Equation: exe Equation: exe Equation: Marge Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 100 110 120	21,91 19,83 18,14 16,75 15,57 14,56 = CdA(2gh)^A 102,00 66,83 68,60 0,00 0,00 Stage 68,60 UNC-2 0,11 0,71 1(2 yr) (mm/hr) 76,81 52,03 40,04 32,86 24,96 21,91 19,83 18,14 16,75 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,11 0,71 16,27 14,56 UNC-2 0,04 0,04 24,56 UNC-1 0,04 0,87 0,04 0,87 0,04 0,87 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,04 0,057 14,56 0,04 0,04 0,04 0,04 0,057 14,56 0,04 0,04 0,04 0,04 0,04 0,057 14,56 0,04 0,04 0,04 0,04 0,04 0,057 14,56 0,04 0,04 0,04 0,057 14,56 0,04 0,05 0,04 0,04 0,05 0,00	3.4 3.1 2.9 2.6 2.4 2.3 mm m m m Head (m) 1.77 Qactual (L/s) 1.77 1.71 1.77 1.77 1.73 5.5 5.3 5.5 4.9 4.4 4.0 3.7 3.5 3.2	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Uischarge (L/s) 27.5 7.3 7.5 7.3 6.3 5.5 6.3 5.5 4.9 4.4 4.0 3.7 3.2	0.0 0.0 0.0 0.0 0.57 Vreq (cu.m) 0.0 Un Qestored (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Vstored (m^3)	Check OK
Orific Inver Ti Max Po Down 5-year Subdrai	70 80 90 110 120 3 Above CB 20 Equation: 20 Equation: 20 Equation: 4 Elevation (G Elevation 4 C Elevation 4 C Elevation 4 C Elevation 10 10 20 40 50 60 70 80 90 100 110 120 100 100 100 100 10	21,91 19,83 19,83 19,83 14,56 15,57 14,56 102,00 66,83 68,60 0.00 0.00 0.00 Stage 68,60 UNC-2 0.11 0.71 1(2yr) (mm/hr) 76,81 52,03 40,04 32,86 40,04 32,86 40,04 32,86 UNC-2 0.11 0.71 14,56 UNC-2 0.11 0.71 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.11 0.71 15,57 14,56 UNC-2 0.04 15,57 14,56 UNC-2 0.04 15,57 14,56 UNC-2 0.04 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 15,57 14,56 16,75 16,57	3.4 3.1 2.9 2.6 2.4 2.3 .5 mm m m Head (m) 1.77 1.1 1.77 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.1 1.7.3 6.3 5.5 4.9 4.4 0.3,7 3.5	3.4 3.1 2.9 2.6 2.4 2.3 Where C = Discharge (L/s) 27.5 27.5 27.5 27.5 4.9 4.4 4.0 3.7 3.5	0.0 0.0 0.0 0.0 0.57 Vreq (cu. m) 0.0 0.0 Un Un (L/s)	0.0 0.0 0.0 0.0 0.0 0.0 0.0 Vavail (cu. m) 4.7 controlled - N Vstored (m^3)	Check OK

Project #160401663, 265 Catherine Street

100 yr Intensity City of Ottawa I = a/(t + b)t (min) b = 10 20 30 40 50 60 70 80 90 100 110 120 178.56 119.95 91.87 75.15 75.15 63.95 55.89 49.79 44.99 41.11 37.90 35.20 32.89 100 YEAR Modified Rational Method for Entire Site Subdrainage Area: UNC-PARK Area (ha): 0.01 C: 0.41 Uncontrolled - Park tc l (100 yr) Qactual Qrelease Qstored Vstored (min) 10 20 30 40 50 60 70 80 90 100 110 120 (mm/hr) 178.56 (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.6 0.6 0.5 0.5 0.5 (L/s) (L/s) (m^3) 119.95 1.7 1.3 1.0 0.9 0.8 0.7 0.6 119.95 91.87 75.15 63.95 55.89 49.79 44.99 41.11 37.90 25.20 0.6 0.5 0.5 0.5 35.20 32.89 Subdrainage Area: Area (ha): C: Controlled - Park PARK 0.09 l (100 yr) Qrelease Qstored Vstored tc Qactual (min) (mm/hr) 178.56 (L/s) (L/s) (L/s) (m^3) 35.1 28.4 4.0 20 30 40 50 60 70 80 90 100 110 120 119.95 23.6 18.1 14.8 12.6 11.0 9.8 23.6 18.1 14.8 12.6 11.0 9.8 8.8 8.1 7.4 6.9 6.5 0.0 0.0 0.0 0.0 0.0 0.0 119.95 91.87 75.15 63.95 55.89 49.79 44.99 41.11 8.8 8.1 7.4 6.9 0.0 0.0 0.0 0.0 0.0 37.90 35.20 32.89 6.5 Surface Storage Above CB storage: Orifice Equation: Q = CdA(2gh)^0.5 Where C = 0.57 CdA(2gh)*0 102.00 mm 66.83 m 68.60 m 0.11 m 66.67 m Orifice Diameter: Invert Elevation T/G Elevation Max Ponding Depth Downstream W/L Discharge Stage Vrea Vavail Volume Head (m) 1.88 (L/s) (cu. m) 4.0 (cu. m) Check OK 100-year Water Level 68.71 Subdrainage Area: Area (ha): C: Uncontrolled - Non-Tributary UNC-2 0.11 0.89 Qactua (L/s) 49.8 l (100 yr) Qrelease Qstored Vstored (L/s) (m^3) tc (min) 10 20 30 40 50 60 70 80 90 100 110 120 (mm/hr) 178.56 119.95 91.87 (L/s) 49.8 33.4 25.6 21.0 17.8 15.6 13.9 12.5 11.5 10.6 33.4 25.6 21.0 17.8 15.6 13.9 12.5 11.5 10.6 75.15 63.95 55.89 49.79 44.99 41.11 37.90 35.20 9.8 9.8 9.2 32.89 9.2 Uncontrolled - Non-Tributary Subdrainage Area: UNC-1 Area (ha): C: 0.04 1.00 l (100 yr) tc (min) Qstored Vstored (L/s) (m^3) Qactual Qrelease (L/s) 18.4 (L/s) 18.4 (mm/hr) 178.56

Stormwater Management Calculations

unieu K		lethod Ca		for Storage	9		
	20 30	52.03 40.04	4.7 3.6	4.7 3.6			
	40 50	32.86 28.04	2.9 2.5	2.9 2.5			
	60 70	24.56 21.91	2.2 2.0	2.2 2.0			
	80	19.83	1.8	1.8			
	90 100	18.14 16.75	1.6 1.5	1.6 1.5			
	110 120	15.57 14.56	1.4 1.3	1.4 1.3			
Subdrain	age Area:	CIST 2-4			Unc	ontrolled - Nor	n-Tributary
	Area (ha): C:	0.06			one		, modaly
Г	tc	l (2 yr)	Qactual	Qrelease	Qstored	Vstored	
L	(min) 10	(mm/hr) 76.81	(L/s) 8.7	(L/s) 8.7	(L/s)	(m^3)	
	20 30	52.03 40.04	5.9 4.6	5.9 4.6			
	40 50	32.86 28.04	3.7 3.2	3.7 3.2			
	60	24.56	2.8	2.8			
	70 80	21.91 19.83	2.5 2.3	2.5 2.3			
	90 100	18.14 16.75	2.1 1.9	2.1 1.9			
	110 120	15.57 14.56	1.8	1.9 1.8 1.7			
Suba''						entrolle d. M	Telburt
Subdrain /	age Area: Area (ha): C:	CIST 2-3 0.06 0.59			Und	ontrolled - Nor	I- I FIDULARY
Г	tc	l (2 yr)	Qactual	Qrelease	Qstored	Vstored	
L	(min) 10	(mm/hr) 76.81	(L/s) 7.2	(L/s) 7.2	(L/s)	(m^3)	
	20 30	52.03 40.04	4.9 3.7	4.9 3.7			
	40	32.86	3.1	3.1			
	50 60	28.04 24.56	2.6 2.3	2.6 2.3			
	70 80	21.91 19.83	2.0 1.9	2.0 1.9			
	90	18.14	1.7	1.7			
	100 110	16.75 15.57	1.6 1.5	1.6 1.5			
	120	14.56	1.4	1.4			
	age Area:	CIST 2-2 0.04			Unc	ontrolled - Nor	n-Tributary
,	Area (ha): C:	0.90					
	tc (min)	l (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m ³)	
	10 20	76.81 52.03	7.9 5.3	7.9 5.3			
	30	40.04 32.86	4.1 3.4	4.1 3.4			
	40	28.04	2.9	2.9			
	40 50	04.50	2.5	2.5 2.2			
	50 60 70	24.56 21.91	2.2				
	50 60 70 80	21.91 19.83	2.0	2.0 1.9			
	50 60 70 80 90 100	21.91 19.83 18.14 16.75	2.0 1.9 1.7	1.9 1.7			
	50 60 70 80 90	21.91 19.83 18.14	2.0 1.9	1.9			
Subdrain	50 60 70 80 90 100 110 120 age Area:	21.91 19.83 18.14 16.75 15.57 14.56	2.0 1.9 1.7 1.6	1.9 1.7 1.6		Controlled	- Tributary
Subdrain	50 60 70 80 90 100 110 120 age Area: Area (ha): C:	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90	2.0 1.9 1.7 1.6 1.5	1.9 1.7 1.6 1.5	Onterrat		- Tributary
Subdrain:	50 60 70 80 90 100 110 120 age Area: Area (ha): C: tc (min)	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) (mm/hr)	2.0 1.9 1.7 1.6 1.5 Qactual (L/s)	1.9 1.7 1.6 1.5 Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	- Tributary
Subdrain:	50 60 70 80 90 100 110 120 Area (ha): C: tc (min) 10 20	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) (mm/hr) 76.81 52.03	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7	1.9 1.7 1.6 1.5 Qrelease (L/s) 23.6 23.6	(L/s) 58.7 32.1	Vstored (m^3) 35.2 38.6	- Tributary
Subdrain.	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6	Vstored (m^3) 35.2 38.6 34.8 27.9	- Tributary
Subdrain:	50 60 70 80 90 110 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04	2.0 1.9 1.7 1.6 1.5	1.9 1.7 1.6 1.5 Qrelease (L/s) 23.6 23.6 23.6 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6 6.5	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4	- Tributary
Subdrain:	50 60 70 80 90 100 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91	2.0 1.9 1.7 1.6 1.5 Qactual (L's) 82.2 55.7 42.9 35.2 30.0 26.3 23.5	1.9 1.7 1.6 1.5 Qrelease (L/s) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0	- Tributary
Subdrain:	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60	21,91 19,83 18,14 16,75 15,57 14,56 CIST 2-1 0,30 0,90 I (2 yr) (mm/hr) 76,81 52,03 40,04 32,86 28,04 24,56 21,91 19,83 18,14	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3	1.9 1.7 1.6 1.5 Qrelease (L's) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9	- Tributary
Subdrain:	50 60 70 80 90 100 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 120	21,91 19,83 18,14 16,75 15,57 14,56 CIST 2-1 0.30 0.90 (mm/hr) 76,81 52,03 40,04 32,86 28,04 24,56 28,04 24,56 21,91 19,83 18,14 16,75	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.4 17.9	1.9 1.7 1.6 1.5 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0 0.0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0	- Tributary
Subdrain.	50 60 70 80 90 100 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90	21,91 19,83 18,14 16,75 15,57 14,56 CIST 2-1 0,30 0,90 I (2 yr) (mm/hr) 76,81 52,03 40,04 32,86 28,04 24,56 21,91 19,83 18,14	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 35 .2 30.0 26.3 23.5 21.2 19.4	1.9 1.7 1.6 1.5 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0	- Tributary
Notes:	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 110 120 120	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 I (2 yr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.4 17.9 16.7 15.6	1.9 1.7 1.6 1.5 Qrelease (L/s) 23.6 24.6 24.	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	- Tributary
Notes: 1) /	50 60 70 80 90 110 120 age Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 110 120 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 50 50 50 50 50 50 50 50 5	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.30 (mm/hr) 76.81 52.03 40.04 32.86 21.91 19.83 18.14 16.75 15.57 14.56 m subcatchho b e controll	2.0 1.9 1.7 1.6 1.5 Qactual (Us) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.4 17.9 16.7 15.6 mment areas C ed to 23.6 L/s	1.9 1.7 1.6 1.5 Orrelease (Us) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 IST 2-1 - CIST	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vstored (m^3) 35.2 33.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Notes: 1) / 2) [50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 110 120 100 110 120 100 110 120 100 110 120 100 110 120 100 110 120 100 10	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 1(2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 14.56 m subcatchi	2.0 1.9 1.7 1.6 1.5 Qactual (L's) 62.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.4 17.9 16.7 15.6 ment areas C	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 23.5 21.2 19.4 17.7 15.6 IST (Strutcher) (Strut	(L/s) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	- Tributary Volume <u>Check</u> OK
Notes: 1) / 2) [5-year W	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 100 120 20 20 20 20 20 20 20 20 20	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 1(2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 71.456 m subcatch o be controll Stage -	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.3 23.5 21.2 19.6 15.6 ment areas C ed to 23.6 L/s Head	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 IST 2-1 - CIST 3 Discharge (Us)	(L/s) 58.7 32.1 32.1 19.3 31.6 6.5 2.7 0.0 12.4 vtreq (cu. m) 38.6	Vstored (m^3) 38.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Volume Check OK
Notes: 1) / 2) [5-year W Subdrain:	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 100 120 20 20 20 20 20 20 20 20 20	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.30 (mm/hr) 76.81 52.03 40.04 32.86 21.91 19.83 18.14 16.75 15.57 14.56 m subcatchho b e controll	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.3 23.5 21.2 19.6 15.6 ment areas C ed to 23.6 L/s Head	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 IST 2-1 - CIST 3 Discharge (Us)	(L/s) 58.7 32.1 32.1 19.3 31.6 6.5 2.7 0.0 12.4 vtreq (cu. m) 38.6	Vstored (m^3) 35.2 33.6 33.8 27.9 19.4 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Volume Check OK
Notes: 1) / 2) [5-year W Subdrain:	50 60 70 80 90 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 40 40 50 60 60 60 70 80 90 110 120 20 20 20 20 20 20 20 20 20	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 1(2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 14.55 m subcatchi o be controll Stage - CIST 1-4 0.90 1(2 yr) (1.2	2.0 1.9 1.7 1.6 1.5 2.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.3 15.6 ment areas C dt to 23.6 L/i Head (m) - - - - - - - - - - - - -	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 IST 2-1 - CIST 3 Discharge (Us)	(LLS) 58.7 32.1 19.3 21.1 9.3 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Vstored (m^3) 33.2 33.6 34.8 34.8 34.8 27.9 19.4 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Volume Check OK
Notes: 1) / 2) [5-year W Subdrain:	50 60 70 80 90 100 110 120 age Area: Area (ha): C: tc (min) 10 20 30 40 50 60 60 80 90 110 120 20 20 20 20 20 20 20 20 20	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.90 1 (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 14.55 14.56 m subcatch o be controll Stage - CIST 1-4 0.90 1 (2 yr) (mm/hr) 76.81 5.27 14.56 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.57 14.56 15.20 15.27 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 14.56 15.77 17.77 17.77 17.77 17.78 17.76 17.776 17.7776 17.7776 17.77777777777777777777777777777777777	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 15.6 ment areas C ed to 23.6 L/s Head (m) - Qactual (L/s) 1.3	1.9 1.7 1.6 1.5 Qrelease (Us) 23.6 24.6	(LLS) 58.7 32.1 19.3 11.6 6.5 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Vstored (m^3) 35.2 38.6 34.8 27.9 19.4 9.9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Cistern 2 Vavail (cu. m) 170.0	Volume Check OK
Notes: 1) A 2) [5-year W ubdrain	50 60 70 80 90 100 110 120 age Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 110 120 20 30 40 50 60 70 80 90 110 120 20 30 40 50 60 70 80 90 110 120 20 30 40 50 60 70 80 90 110 120 20 30 40 50 60 70 80 90 100 110 120 20 30 40 50 60 110 110 120 20 30 40 50 60 110 110 20 30 40 50 60 110 110 20 30 40 50 60 110 110 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 60 110 120 20 30 40 50 60 100 110 120 20 30 40 50 60 50 60 100 110 120 20 30 40 50 60 20 20 20 20 20 20 20 20 20 2	21.91 19.83 18.14 16.75 15.57 14.56 CIST 2-1 0.30 0.30 0.30 0.30 0.12 yr) (mm/hr) 76.81 22.03 40.04 32.86 21.91 19.83 18.14 16.75 14.56 m subcatchh o be controll Stage - CIST 1-4 0.01 0.90 1(2 yr) (mm/hr) 0.90 1(2 yr) (mm/hr) 0.90 1(2 yr) 0.90 0	2.0 1.9 1.7 1.6 1.5 Qactual (L/s) Qactual 82.2 55.7 42.9 35.2 30.0 26.3 23.5 21.2 19.4 17.9 16.7 15.6 ment areas C ed to 23.6 L/s Head (m) - Qactual (L/s)	1.9 1.7 1.6 1.5 23.6 23.6 23.6 23.6 23.6 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 IST 2-1 - CISI S Discharge (L/s) 23.6 23.6 23.5 21.2 23.6 23.6 23.5 21.2 19.4 17.9 16.7 15.6 Complete State S	(LLS) 58.7 32.1 19.3 21.1 9.3 2.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Vstored (m^3) 33.2 33.6 34.8 34.8 34.8 27.9 19.4 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Volume Check OK



Stormwater Management Calculations

	Rational M 50	28.04	0.5	0.5	7		
	60	24.56	0.5	0.5			
	70 80	21.91 19.83	0.4 0.3	0.4 0.3			
	80 90	19.83	0.3	0.3			
	100 110	16.75 15.57	0.3 0.3	0.3 0.3			
	110	15.57 14.56	0.3	0.3			
ubdra	inage Area: Area (ha):	CIST 1-3 0.01			Un	controlled - I	Non-Tributary
	C:	0.90					l.
	tc (min)	l (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m ³)	
	10 20	76.81 52.03	2.1 1.4	2.1 1.4			
	30	40.04	1.1	1.1			
	40 50	32.86 28.04	0.9 0.8	0.9 0.8			
	60	24.56	0.7	0.7			
	70 80	21.91 19.83	0.6 0.5	0.6 0.5			
	90	18.14	0.5	0.5			
	100 110	16.75 15.57	0.5 0.4	0.5 0.4			
	120	14.56	0.4	0.4			
ubdra	inage Area: Area (ha):	CIST 1-2 0.02			Un	controlled - I	Non-Tributary
	C:	0.90					
	tc (min)	l (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m ³)	
	10 20	76.81 52.03	4.2 2.9	4.2 2.9			
	30	40.04	2.2	2.2			
	40 50	32.86 28.04	1.8 1.5	1.8 1.5			
	60	24.56	1.4	1.4			
	70 80	21.91 19.83	1.2 1.1	1.2 1.1			
	90	18.14	1.0	1.0			
	100 110	16.75 15.57	0.9 0.9	0.9 0.9			
	120	14.56	0.8	0.8			
ubdra	inage Area: Area (ha): C:	CIST 1-1 0.24 0.90				Controll	ed - Tributary
	tc (min)	l (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	
	10	76.81	54.0	14.3	39.7	23.8	
	20 30	52.03 40.04	36.6 28.2	14.3 14.3	22.3 13.9	26.8 25.0	
	40	32.86	23.1	14.3	8.8	21.2	
	50 60	28.04 24.56	19.7 17.3	14.3 14.3	5.5 3.0	16.4 10.8	
	70	21.91	15.4	14.3	1.1	4.8	
	80 90	19.83 18.14	13.9 12.8	13.9 12.8	0.0 0.0	0.0 0.0	
	100	16.75	11.8	11.8	0.0	0.0	
	110 120	15.57 14.56	10.9 10.2	10.9 10.2	0.0	0.0	
Notes:							
		to be controlle	ed to 14.3 L/s				
		Stage	Head (m)	Discharge (L/s)	Vreq (cu. m)	Vavail (cu. m)	Volume Check OK
2)	Water Level	_	-				
2)	Water Level	-		14.3	26.8	110.0	
2)	Water Level	-	-	14.3	20.0	110.0	
2) 2-year	Water Level		-	14.3	20.0		Vavailahle*
2) 2-year	TO OUTLET	T Tributary t	- to Cistern 1	0.281	ha	Vrequired	
2) 2-year	TO OUTLET	Tributary t al 2yr Flow t	to Cistern 1	0.281 54.0	ha L/s		
2) 2-year	TO OUTLET Tot Total	Tributary t al 2yr Flow t 2yr Flow fro Tributary t	to Cistern 1 m Cistern 1 to Cistern 2	0.281 54.0 14.3 0.464	ha L/s L/s ha	Vrequired	
2) 2-year	TO OUTLET Tot Total Total	Tributary t al 2yr Flow t 2yr Flow frou Tributary t al 2yr Flow t	to Cistern 1 m Cistern 1 to Cistern 2 to Cistern 2	0.281 54.0 14.3 0.464 82.2	ha L/s L/s ha L/s	Vrequired	
2) 2-year	TO OUTLET Tot Total Total	Tributary (tal 2yr Flow (2yr Flow frou Tributary (tal 2yr Flow frou 2yr Flow frou	to Cistern 1 m Cistern 1 to Cistern 2 to Cistern 2	0.281 54.0 14.3 0.464 82.2 23.6	ha L/s L/s ha L/s L/s	Vrequired	
2) 22-year	TO OUTLET Tot Total Total tal 2yr Uncc	Tributary (tal 2yr Flow (2yr Flow frou Tributary (tal 2yr Flow frou 2yr Flow frou	to Cistern 1 m Cistern 1 to Cistern 2 to Cistern 2 m Cistern 2 m Cistern 2 trolled Area w from Site	0.281 54.0 14.3 0.464 82.2 23.6 0.745 24.0	ha L/s L/s ha L/s L/s ha L/s	Vrequired	
2) 22-year	TO OUTLE1 Tot Total Total tal 2yr Uncc	Tributary 1 al 2yr Flow for Tributary 1 al 2yr Flow for Total Cont Total Cont Introlled Flor Fotal Uncont otal 2yr Flow	to Cistern 1 m Cistern 1 to Cistern 2 to Cistern 2 m Cistern 2 m Cistern 2 trolled Area w from Site	0.281 54.0 14.3 0.464 82.2 23.6 0.745 24.0 0.150 28.4	ha L/s L/s ha L/s ha L/s ha L/s	Vrequired	
2) P-year	TO OUTLET Total Total tal 2yr Uncc	Tributary 1 al 2yr Flow for Tributary 1 al 2yr Flow for Total Cont Total Cont Introlled Flor Fotal Uncont otal 2yr Flow	to Cistern 1 m Cistern 1 to Cistern 2 to Cistern 2 m Cistern 2 m Cistern 2 rrolled Area w from Site rrolled Area v from Park il Park Area	0.281 54.0 14.3 0.464 82.2 23.6 0.745 24.0 0.150 28.4 0.099 37.8	ha L/s L/s L/s L/s L/s ha L/s ha L/s ha L/s	Vrequired	

odified Rational M 50	63.95	1.2	1.2	e		
60	55.89	1.1	1.1			
70 80	49.79 44.99	1.0 0.9	1.0 0.9			
90	41.11	0.8	0.8			
100 110	37.90 35.20	0.7 0.7	0.7 0.7			
120	32.89	0.6	0.6			
Subdrainage Area:	CIST 1-3			Un	controlled - N	on-Tributary
Area (ha): C:	0.01 1.00					
tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	
10 20	178.56 119.95	5.5 3.7	5.5 3.7			
30	91.87	2.8	2.8			
40 50	75.15 63.95	2.3 2.0	2.3 2.0			
60	55.89	1.7	1.7			
70 80	49.79 44.99	1.5 1.4	1.5 1.4			
90	41.11	1.3	1.3			
100 110	37.90 35.20	1.2 1.1	1.2 1.1			
120	32.89	1.0	1.0			
Subdrainage Area:	CIST 1-2 0.02			Un	controlled - N	on-Tributary
Area (ha): C:	0.02 1.00					
tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	
10	178.56	10.9	10.9 7.3			
30	119.95 91.87	7.3 5.6	5.6			
40	75.15	4.6	4.6			
50 60	63.95 55.89	3.9 3.4	3.9 3.4			
70	49.79	3.0	3.0			
80 90	44.99 41.11	2.8 2.5	2.8 2.5			
100	37.90	2.3	2.3			
110 120	35.20 32.89	2.2 2.0	2.2 2.0			
Subdrainage Area: Area (ha): C:	CIST 1-1 0.24 1.00				Controlle	d - Tributary
tc (min)	l (100 yr)	Qactual	Qrelease	Qstored	Vstored	
(min) 10	(mm/hr) 178.56	(L/s) 139.5	(L/s) 14.3	(L/s) 125.2	(m^3) 75.1	
20	119.95	93.7	14.3	79.4	95.3	
30 40	91.87 75.15	71.8 58.7	14.3 14.3	57.5 44.4	103.5 106.7	
50	63.95	50.0	14.3	35.7	107.1	
60 70	55.89 49.79	43.7 38.9	14.3 14.3	29.4 24.6	105.8 103.5	
80	44.99	35.1	14.3	20.9	100.2	
90 100	41.11 37.90	32.1 29.6	14.3 14.3	17.9 15.3	96.4 92.1	
110	35.20	27.5	14.3	13.2	87.4	
120	32.89	25.7	14.3	11.4	82.3	
Notes: 1) All flows from 2) Discharge to				ST 1-4 outlet	to Cistern 1	
٦	Stage	Head	Discharge	Vreq	Vavail	Volume
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Ok

D.2 Storm Sewer Design Sheet

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			_																		10.03																		_

D.3 SWM Quality Control Measures

Wu, Michael

From:	Fawzi, Mohammed <mohammed.fawzi@ottawa.ca></mohammed.fawzi@ottawa.ca>
Sent:	Wednesday, 3 May, 2023 13:35
То:	Wu, Michael
Cc:	Ford, Matthew; Sharp, Mike; Thiffault, Dustin; Kilborn, Kris
Subject:	RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

There are no stormwater quality controls for the Arlington Sewers. The 600mm storm sewer on Arlington leads to a combined sewer.

Thanks for confirming Michael.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <Michael.Wu@stantec.com>
Sent: May 02, 2023 2:25 PM
To: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>
Cc: Ford, Matthew <Matthew.Ford@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>; Thiffault, Dustin <dustin.thiffault@stantec.com>; Kilborn, Kris <kris.kilborn@stantec.com>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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On a side note, Mohammed, since we are now proposing to have the stormwater discharge to the 525 mm and 600 mm storm sewer in Arlington Avenue, does that impact what stormwater quality control criteria the site would be subject to?

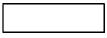
Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Wu, Michael
Sent: Tuesday, 2 May, 2023 13:41
To: Fawzi, Mohammed <mohammed.fawzi@ottawa.ca>
Cc: Ford, Matthew <Matthew.Ford@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>; Thiffault, Dustin <Dustin.Thiffault@stantec.com>; Kilborn, Kris <kris.kilborn@stantec.com>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Mohammed, understood. Thanks for the update anyways.

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Tuesday, 2 May, 2023 13:40
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>Dustin.Thiffault@stantec.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>;
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

My apologies for the wait but the Water Resources Group is quite busy at the moment and as such boundary conditions may take up to two weeks. As for the SWM quality control criteria, this would not be applicable given that we are proposing to discharge to a combined sewer.

Thanks Michael.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: May 01, 2023 4:22 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>dustin.thiffault@stantec.com</u>>; Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

I just want to do a quick follow up on the revised boundary conditions for the site. Since we are proposing to upsize the Catherine Street and Kent Street watermains to service the site, is there a timeline on when we can expect the revised boundary conditions that account for the upsizing at those two streets?

On a side note, it is my understanding that the City now provides the SWM quality control criteria as a result of Bill 23. As such, given we are to control the stormwater discharge from the site to a 2-year predevelopment release rate with a C no more than 0.50, what quality control measures are the site subjected to?

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>

Sent: Thursday, 27 April, 2023 09:54

To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>

Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>

Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Good morning Mohammed and thanks for the chat about 265 Catherine Project.

As discussed, our submission will include san and storm connections to the Arlington Street Sewer. The Development will have two sets of san/storm connections to the Arlington

Sewer to coincide with phasing and to ensure mechanical can design the site. As this is a large site spanning an entire City block.

As we have minimal room for onsite monitoring manholes we will be showing these within the municipal right of way. City can comment and the owner may have to enter into an encroachment agreement with the City.

For now, we will show W3 Chambers on the water services (which will also be within the right of way) which you could review if chambers are required on all.

Thanks for checking in on the boundary conditions. It appears we will need to install new watermain along Catherine and Kent which we will show and submit in plan view only for this submission.

Sincerely

Kris Kilborn

Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4

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Please note our reception is on the 3rd floor.

From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>

Sent: Wednesday, April 26, 2023 10:54 AM

To: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>

Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>

Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

I'm available today until 4:00pm – so feel free to send an invite. I would recommend we include Asset Management though as I mentioned they are the ones who would approve a connection to a trunk sewer.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>

Sent: April 25, 2023 2:56 PM

To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>

Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin <dustin.thiffault@stantec.com>; Wu, Michael <<u>Michael.Wu@stantec.com></u>

Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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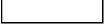
Hey Mohammed do you have time for a quick call this afternoon. Would just like to review our plans with you and show you our drawings and why We are requesting to connect to Catherine.

I could send out a team's meeting request. Thanks, and let me know.

Sincerely

Kris Kilborn Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Tuesday, April 25, 2023 1:35 PM
To: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>Dustin.Thiffault@stantec.com</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Kris,

I can definitely get the City's Asset Management Branch involved in our discussion as they would be the ones ultimately making the decision as to whether or not we can connect to the trunk sewer. Prior to doing so, could you confirm why we cannot connect to the 450mm dia. pipe on Lyon Street?

Thanks Kris.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>
Sent: April 25, 2023 11:18 AM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>; Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Thiffault, Dustin
<<u>dustin.thiffault@stantec.com</u>>

Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Mohammed

We have design and installed connections to deep Trunk Sewers on other high rise projects in the City and we require this connection for our phase 1 development.

As you can appreciate this development is a full City block which will be constructed in phases and require connection to the 1800dia. In previous emails you mentioned that

We should not connect to the existing brick sewer on Arlington and our hands are a bit tied now.

Could we set up a call to discuss.

Sincerely

Kris Kilborn

Principal, Community Development Business Center Practice Leader

Mobile: 613 297-0571 Fax: 613 722-2799 kris.kilborn@stantec.com Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Tuesday, April 25, 2023 10:50 AM
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Unfortunately the City would not permit a connection to such a large deep trunk sewer.

Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 24, 2023 12:14 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>; Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Hi Mohammed, thanks for the response.

And as a quick follow-up, does the City have any objections to using the 1800 mm diameter combined sewer on Catherine Street for the sanitary and storm discharge from the site? As a refresher, we anticipate around 26.88 L/s of sanitary discharge from the site.

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Monday, 24 April, 2023 08:32
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

```
Hi Michael,
```

Thank you for the revised boundary conditions.

I can confirm there are no current scheduled City projects in the vicinity of the site.

Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 21, 2023 3:21 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>;
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

As a quick follow-up, we would like to submit revised boundary conditions for the 265 Catherine Street site with updated fire flow demands at the same connection points.

While the revised worst case fire flow demands from the site has been reduced to 166.7 L/s (10000 L/min), connecting to the Lyon Street North and Arlington Avenue 203 mm diameter watermains would be a challenge, not least by the site's servicing be consolidated at Catherine Street, where the building's main entrance will be at.

As such, as part of the updated boundary condition request, we would like to obtain the hydraulic boundary conditions for the site under the following scenarios:

1. Upsizing the Catherine Street watermain to a 203 mm diameter watermain from Kent Street to Lyon Street North only

2. Upsizing both the Catherine Street and Kent Street watermains to 203 mm diameter within the vicinity of the site

Attached are the revised fire flow calculations and sketches of the two proposed upsizing options detailing the range of the proposed upsizing.

In addition, please advise if there are other design considerations for other ongoing City projects in the vicinity that could impact the site.

Please let me know if you have any further questions or comments.

Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

Stantec 300 - 1331 Clyde Avenue Ottawa ON K2C 3G4

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From: Wu, Michael
Sent: Wednesday, 19 April, 2023 15:35
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>; Kilborn, Kris
<<u>kris.kilborn@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Good afternoon, Mohammed:

Just a quick question, is it possible for you to provide the HGL at the four connections under the max day + fire flow conditions with the fire flow demand of 166.7 L/s?

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Wednesday, 19 April, 2023 11:50
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Thiffault, Dustin <<u>Dustin.Thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Thank you for confirming.

As a heads up, please note that a letter or email confirmation by the architect confirming the parameters used in the fire flow calculations is required. Parameters to be confirmed applicable are confirming that the vertical openings are protected, type of construction, occupancy charge and sprinkler reductions. This can be appended to the Servicing Report.

Thanks Michael.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, Mohammed.Fawzi@ottawa.ca

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 18, 2023 4:37 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>; Thiffault, Dustin <<u>dustin.thiffault@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

As a follow-up, we have decided to update the fire flow demand calculation approach.

Specifically, for the FUS "protected vertical openings" classification for the two high-rise buildings on site, as the main openings between the floors are the elevator shafts and emergency exit stairwell, they would have already been subjected to the strictest fire protection measures outlined in the Ontario Building Code and the National Building Code, therefore it is reasoned that the two high-rises be classified as having protected vertical openings.

Under this approach, Building B's fire flow demand is reduced to 166.7 L/s (10,000 L/min), which is adequate for the watermains on Arlington Avenue and Lyon Street North to provide their respective fire flows while maintaining a residual pressure of 20 psi.

Please let me know if you have any questions or comments to this new approach.

Thanks,

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Wu, Michael
Sent: Thursday, 13 April, 2023 13:23
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Perfect, thanks for the information.

Michael Wu, EIT

Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>> Sent: Thursday, 13 April, 2023 13:05 To: Wu, Michael <<u>Michael.Wu@stantec.com</u>> Hi Michael,

Please note that the Fire demand request of 316.7 L/s was not met. Next steps: Take measures to lower the fire demand, identify hydrants to request for a multi-hydrant analysis.

The following are boundary conditions, HGL, for hydraulic analysis at 265 Catherine Street (zone 1W) assumed to be connected to either the 127 mm watermain on Catherine Street, OR the 203 mm watermain on Lyon Street, OR the 203 mm watermain on Arlington Avenue, OR the 127 mm on Kent Street (see attached PDF for location).

Connection	Min HGL (m)	Maximum HGL (m)
Catherine Street	80.4	115.3
Lyon Street	104.9	115.3
Arlington Avenue	105.9	115.3
Kent Street	97.8	115.2

Fire Flow:

Available Fire flow at 20 psi: 46 L/s assuming ground elevation of 68.2 m (Catherine Connection) Available Fire flow at 20 psi: 187 L/s assuming ground elevation of 67.6 m (Lyon Connection) Available Fire flow at 20 psi: 270 L/s assuming ground elevation of 68.0 m (Arlington Connection) Available Fire flow at 20 psi: 64 L/s assuming ground elevation of 68.6 m (Kent Connection)

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: April 11, 2023 11:45 AM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good morning, Mohammed, hope you are well. Just checking in to see when we could expect to receive the boundary conditions, the combined sewer capacity confirmation and the existing water consumption data for the site (if possible).

Thanks,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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From: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Sent: Wednesday, 29 March, 2023 14:50
To: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: RE: 265 Catherine Street Combined Sewer Capacity Confirmation

Hi Michael,

Received.

I will get back to you as soon as possible. Thank you.

Best Regards,

Mohammed Fawzi, P.Eng.

Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review - Central Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue. Laurier Ouest. Ottawa (Ontario) K1P 1J1 613.580.2424 ext./poste 20120, <u>Mohammed.Fawzi@ottawa.ca</u>

Please note that due to the current situation, I am working remotely. Email is currently the best way to contact me

From: Wu, Michael <<u>Michael.Wu@stantec.com</u>>
Sent: March 29, 2023 1:18 PM
To: Fawzi, Mohammed <<u>mohammed.fawzi@ottawa.ca</u>>
Cc: Ford, Matthew <<u>Matthew.Ford@stantec.com</u>>
Subject: 265 Catherine Street Combined Sewer Capacity Confirmation

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Good afternoon, Mohammed:

In addition to the hydraulic boundary conditions, as part of the servicing for the proposed development on 265 Catherine Street, we would like to confirm if there is sufficient capacity downstream of the 450 mm diameter combined sewers in Lyon Street North, 300 mm diameter combined sewers in Catherine Street, 375 mm diameter combined sewers in Kent Street, and the 1200 mm diameter trunk combined sewers in Arlington Avenue to receive an additional peak flow of 26.8 L/s from the proposed development.

Please find our sanitary design sheet and location map attached for your information. Furthermore, we were wondering if there are any existing water consumption data for the site during its use as a Greyhound bus terminal.

Thank you,

Michael Wu, EIT Civil Engineering Intern, Community Development

Work: (613) 738-6033 Mobile: (613) 858-0548 michael.wu@stantec.com

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Appendix E Geotechnical Investigation Report Excerpts (Paterson, August 2021)

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Geotechnical Engineering

Environmental Engineering

Hydrogeology

Geological Engineering

Materials Testing

Building Science

Noise and Vibration Studies

Geotechnical Investigation

Proposed Mixed-Use Development 265 Catherine Street Ottawa, Ontario

Prepared For

Brigil

Paterson Group Inc.

Consulting Engineers 154 Colonnade Road Ottawa (Nepean), Ontario Canada K2E 7J5

Tel: (613) 226-7381 Fax: (613) 226-6344 www.patersongroup.ca August 13, 2021

Report PG5933-1



4.0 Observations

4.1 Surface Conditions

Currently, the subject site is occupied by a bus terminal building with associated asphalt covered parking areas and access lanes. The subject site is approximately at grade with surrounding streets.

The site is bordered by Catherine Street to the south, Lyon Street to the west, Arlington Avenue and further by residential dwellings to the north and Kent Street to the east. The existing ground surface across the subject site is relatively flat and at grade with adjacent properties.

4.2 Subsurface Profile

Overburden

Generally, the subsurface profile at the test hole locations consists of concrete or asphaltic concrete underlain by fill extending to an approximate depth of 0.6 to 2.3 m below the existing ground surface. The fill was generally observed to consist of a compact brown silty sand with crushed stone and occasional brick, metal, and plastic fragments.

A native silty sand layer and/or silty clay deposit was encountered underlying the fill. The silty clay deposit was observed to consist of a very stiff to stiff, brown silty clay, becoming a stiff grey silty clay below an approximate depths ranging between 3.0 to 7.6 m below the existing ground surface.

Underlying the silty clay deposit below approximate depths ranging between 4.4 to 9.7 m, a glacial till layer was encountered. The glacial till deposit was observed to consist of a grey sandy silt, clayey silt or silty clay with gravel, cobbles and boulders.

Practical refusal to augering or the DCPT was encountered at depths ranging from 7.4 to 11.7 m below the existing ground surface.

Bedrock

Based on available geological mapping, the subject site is located in an area where the bedrock consists of interbedded limestone and shale of the Verulam Formation and shale of the Billings Formation at depths ranging from 10 to 15 m.



5.0 Discussion

5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is considered suitable for the proposed development. Based on the subsurface conditions encountered in the test holes and the anticipated building loads, it is recommended that foundation support for the proposed high-rise buildings consist of:

- a raft foundation bearing on the undisturbed, stiff silty clay and compact glacial till deposit, or
- a deep foundation, such as end-bearing piles, which extends to the bedrock surface

Due to the presence of the silty clay deposit, a permissible grade raise restriction will be required for the proposed grading.

The above and other considerations are further discussed in the following sections.

5.2 Site Grading and Preparation

Stripping Depth

Topsoil and deleterious fill, such as those containing organic materials, should be stripped from under any buildings, paved areas and other settlement sensitive structures. Existing construction debris should be entirely removed from within the perimeter of all buildings.

Protection of Subgrade (Raft Foundation)

Where a raft foundation is utilized, it is recommended that a minimum 50 mm thick lean concrete mud slab be placed on the undisturbed, silty clay or glacial till subgrade shortly after the completion of the excavation. The main purpose of the mud slab is to reduce the risk of disturbance of the subgrade under the traffic of workers and equipment.

The final excavation to the raft bearing surface level and the placing of the mud slab should be done in smaller sections to avoid exposing large areas of the silty clay or glacial till to potential disturbance due to drying.



Spread Footing Foundations

Foundations may consist of strip footings, up to 4 m wide, and pad footings, up to 8 m wide, placed over an undisturbed, very stiff to stiff silty clay bearing surface using bearing resistance values at Serviceability Limit State (SLS) of **160 kPa** and factored bearing resistance values at Ultimate Limit States (ULS) of **250 kPa**. A geotechnical resistance factor of 0.5 was applied to the bearing resistance value at ULS.

An undisturbed soil bearing surface consists of one from which all topsoil and deleterious materials, such as loose, frozen or disturbed soil, whether in situ or not, have been removed, in the dry, prior to the placement of concrete for footings.

The bearing resistance values at SLS for conventional style footings will be subjected to potential post-construction total and differential settlements of 25 and 20 mm, respectively.

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to a silty clay or glacial till bearing medium when a plane extending down and out from the bottom edge of the footing at a minimum of 1.5H:1V, passes only through in situ soil or engineered fill of the same or higher capacity as the soil.

Permissible Grade Raise

Due to the presence of the silty clay deposit, a permissible grade raise restriction of **2.0 m** is recommended for grading at the subject site.

If higher than permissible grade raises are required, preloading with or without a surcharge, lightweight fill, and/or other measures should be investigated to reduce the risks of unacceptable long-term post construction total and differential settlements.

The earth forces calculated are unfactored. For the ULS case, the earth loads should be factored as live loads, as per OBC 2012.

5.7 Pavement Structure

Car only parking areas, heavy truck parking areas and access lanes are anticipated at this site. The proposed pavement structures are presented in Tables 3 and 4.

Table 3 - Recommended Pavement Structure - Car Only Parking Areas		
Thickness (mm)	Material Description	
50	Wear Course - HL-3 or Superpave 12.5 Asphaltic Concrete	
150	BASE - OPSS Granular A Crushed Stone	
300	SUBBASE - OPSS Granular B Type II	
SUBGRADE - In situ soil, or OPSS Granular B Type I or II material placed over in situ soil		

Table 4 - Recommended Pavement Structure Access Lanes, Ramp and Heavy Truck Parking Areas		
Thickness (mm)	Material Description	
40	Wear Course - HL-3 or Superpave 12.5 Asphaltic Concrete	
50	Binder Course - HL-8 or Superpave 19.0 Asphaltic Concrete	
150	BASE - OPSS Granular A Crushed Stone	
450	SUBBASE - OPSS Granular B Type II	
SUBGRADE - In situ soil, or OPSS Granular B Type I or II material placed over in situ soil		

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for this project.

If soft spots develop in the subgrade during compaction or due to construction traffic, the affected areas should be excavated and replaced with OPSS Granular B Type II material.

The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 99% of the material's SPMDD using suitable vibratory equipment.



Pavement Structure Drainage

Satisfactory performance of the pavement structure is largely dependent on maintaining the contact zone between the subgrade material and the base stone in a dry condition. Failure to provide adequate drainage under conditions of heavy wheel loading can result in the fine subgrade soil being pumped into the voids in the stone subbase, thereby reducing load carrying capacity.

Due to the low permeability of the subgrade materials consideration should be given to installing subdrains during the pavement construction as per City of Ottawa standards. The subdrain inverts should be approximately 300 mm below subgrade level. The subgrade surface should be crowned to promote water flow to the drainage lines.

7.0 Recommendations

A materials testing and observation services program is a requirement for the provided foundation design data to be applicable. The following aspects of the program should be performed by the geotechnical consultant:

- **Q** Review of the grading plan from a geotechnical perspective.
- **Q** Review the Contractor's design of the temporary shoring system.
- **Q** Review of waterproofing details for elevator shaft and building sump pits.
- Review and inspection of the foundation waterproofing system and all foundation drainage systems.
- Observation of all bearing surfaces prior to the placement of concrete.
- Sampling and testing of the concrete and fill materials used.
- Periodic observation of the condition of unsupported excavation side slopes in excess of 3 m in height, if applicable.
- Observation of all subgrades prior to backfilling.
- **G** Field density tests to determine the level of compaction achieved.
- Sampling and testing of the bituminous concrete including mix design reviews.

A report confirming that these works have been conducted in general accordance with our recommendations could be issued, upon request, following the completion of a satisfactory materials testing and observation program by the geotechnical consultant.

8.0 Statement of Limitations

North Bay

patersondroup

Ottawa

The recommendations provided in this report are in accordance with our present understanding of the project. We request permission to review our recommendations when the drawings and specifications are completed.

A geotechnical investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, we request immediate notification to permit reassessment of our recommendations.

The recommendations provided herein should only be used by the design professionals associated with this project. They are not intended for contractors bidding on or undertaking the work. The latter should evaluate the factual information provided in this report and determine its suitability and completeness for their intended construction schedule and methods. Additional testing may be required for their purposes.

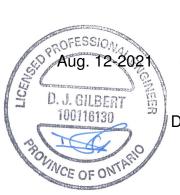
The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Brigil or their agents is not authorized without review by Paterson for the applicability of our recommendations to the altered use of the report.

Paterson Group Inc.

Joey R. Villeneuve, M.A.Sc., P.Eng..

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David J. Gllbert, P.Eng.