

LRJ

ENGINEERING | INGÉNIERIE

Geotechnical Investigation

Proposed Nine-Storey Mixed-Use Apartment Building
725 Somerset Street West
Ottawa, Ontario

Prepared for:

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LRL File No.: 220463

May 2023



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1 INTRODUCTION

LRL Associates Ltd. (LRL) was retained by Upstay Canada Inc. to perform a geotechnical investigation for a proposed nine-storey mixed-use apartment building, located at 725 Somerset Street West, Ottawa, Ontario.

The purpose of the investigation was to identify the subsurface conditions across the site by the completion of a borehole drilling program. Based on the visual and factual information obtained, this report will provide guidelines on the geotechnical engineering aspects of the design of the project, including construction considerations.

This report has been prepared in consideration of the terms and conditions noted above. Should there be any changes in the design features, which may relate to the geotechnical recommendations provided in the report, LRL should be advised in order to review the report recommendations.

2 SITE AND PROJECT DESCRIPTION

The site under investigation currently encompasses a two-storey multi-unit commercial building. There is on grade asphalt parking to the west and north of the existing building. The site is bound by 118 Empress Avenue to the north, 721 Somerset Street West to the east, Somerset Street West to the south, and Empress Avenue to the west. Topography of the site is flat, and has a surface area of about 1,000 m². Vehicular site access comes by way of Empress Avenue, and is civically located at 725 Somerset Street West. The location is presented in Figure 1 included in **Appendix A**.

This development will consist of construction of a nine-story mixed-use residential apartment building, having a footprint of about 790 m². At the time of generating this report, one (1) level of underground parking is being considered.

3 PROCEDURE

The fieldwork for this investigation was carried out on May 15 and 16, 2023; in conjunction with the Phase II Environmental Site Assessment (ESA). Prior to the fieldwork, the site was cleared for the presence of any underground services and utilities. A total of seven (7) boreholes were drilled onsite and labelled BH23-1 through BH23-7 to get a general understanding of the underlying soil conditions. The approximate locations of the boreholes are shown in Figure 2 included in **Appendix A**.

The boreholes were advanced using a truck mounted CME 55 drill rig equipped with 200 mm diameter continuous flight hollow stem auger supplied and operated by George Downing Estate Drilling Ltd. A “two man” crew experienced with geotechnical drilling operated the drill rig and equipment.

Sampling of the overburden materials encountered in the boreholes was carried out at regular depth intervals using a 50.8 mm diameter drive open conventional spoon sampler in conjunction with standard penetration testing (SPT) “N” values. The SPT were conducted following the method **ASTM D1586** and the results of SPT, in terms of the number of blows per 0.3 m of split-spoon sampler penetration after first 0.15 m designated as “N” value.



All boreholes were advanced until practical auger refusal over bedrock, four (4) of the boreholes consisted of HQ-size ($\text{Ø}63.5$ mm) rock coring. The boreholes were terminated at depths ranging from 0.60 to 4.65 m below ground surface (bgs). Upon completion, select boreholes were converted into monitoring wells for the Phase II ESA. The remaining boreholes were compacted and backfilled using the overburden cuttings, and topped with asphalt cold patch.

The fieldwork was supervised throughout by a member of our engineering staff who oversaw the drilling activities, cared for the samples obtained and logged the subsurface conditions encountered within each of the boreholes. All soil samples were transported back to our office for further evaluation. The recovered soil samples collected from the boreholes were classified based on visual examination of the materials recovered and the results of the in-situ testing.

Furthermore, all boreholes were located using a Garmin Etrex Legend GPS (Global Positioning System) receiver using NAD 83 datum (North American Datum). LRL's field personnel determined the existing grade elevations at the borehole locations through a topographic survey carried out using a temporary site bench mark (top of fire hydrant near south western corner of the site), and given an elevation of 100.00 m. Ground surface elevations of the boring locations are shown on their respective borehole logs.

4 SUBSURFACE SOIL AND GROUNDWATER CONDITIONS

4.1 General

A review of local surficial geology maps provided by the Department of Energy, Mines and Resources Canada suggest that the site's subsurface conditions consist of bedrock. The bedrock can be classified as "Ottawa Formation"; consisting of limestone with some shaly partings.

The subsurface conditions encountered in the boreholes were classified based on visual and tactile examination of the materials recovered from the boreholes. The soil descriptions presented in this report are based on commonly accepted methods of classification and identification employed in geotechnical practice. Classification and identification of soil were conducted according to the procedure **ASTM D2487** and judgement, and LRL does not guarantee descriptions as exact, but infers accuracy to the extent that is common in current geotechnical practice.

The subsurface soil conditions encountered are given in their respective borehole logs presented in **Appendix B**. A greater explanation of the information presented in the borehole logs can be found in **Appendix C** of this report. These logs indicate the subsurface conditions encountered at a specific test location only. Boundaries between zones on the logs are often not distinct, but are rather transitional and have been interpreted as such.

4.2 Pavement Structure

A pavement structure was encountered at the surface of all boring locations, and consisted of asphalt overlying fill material consisting of a sand and gravel material. The extended to depths ranging between 0.55 and 0.85 m bgs.



4.3 Bedrock

Underlying the pavement structure in all boring locations, bedrock was encountered. The bedrock was cored in BH23-1, BH23-2, BH23-6, and BH23-7.

The bedrock formation in this area can be described as consisting of limestone, with shaly partings, and grey to dark grey in colour.

The Rock Quality Designation (RQD) was determined after the rock was cored, this is done by summing the lengths of the intact recovered cores which are greater than 100 mm in length, and dividing by the total length of the core run. The RQD values, expressed as a percent, ranged from 38 to 98%, with the majority of the values being in the good to excellent range.

Three (3) rock core samples were selected to determine the unconfined compressive strengths at various depths. The results are summarized below in **Table 1**.

Table 1: Unconfined Compressive Strength of Select Rock Cores

Sample			Bedrock Type	Strength (MPa)
Borehole	Core ID	Depth (m)		
BH23-1	RC1	1.52 – 1.68	Limestone	94.0
BH23-2	RC2	2.44 – 2.69	Limestone	76.6
BH23-7	RC3	3.71 – 4.01	Limestone	107.2

The laboratory reports can be found in **Appendix D** of this report.

4.4 Groundwater Conditions

As part of the Phase II ESA, monitoring wells were installed to collect groundwater samples. A summary of the water levels in the wells are shown below in **Table 2**.

Table 2: Groundwater Monitoring Data

Boring Location	Existing Grade Elevation (m)	Date of Observation	Water Level Data	
			Depth Below Existing Grade (m)	Elevation (m)
BH23-1	99.93	May 29, 2023	1.00	98.93
BH23-2	99.49	May 29, 2023	1.11	98.38
BH23-6	99.29	May 29, 2023	0.63	98.66
BH23-7	98.90	May 29, 2023	2.50	96.40

It should be noted that groundwater levels could fluctuate with seasonal weather conditions, (i.e.: rainfall, droughts, spring thawing) and due to construction activities at or near the vicinity of the site.



5 GEOTECHNICAL CONSIDERATIONS

This section of the report provides general geotechnical recommendations for the design aspect of the proposed development based on our interpretation of the information gathered from the borehole data performed at this site and from the project requirements.

5.1 Foundations

Based on the subsurface soil conditions established at this site, it is recommended that the footings for the proposed apartment building be founded over bedrock, or structural fill overlying the bedrock. Therefore, all material shall be removed from the proposed building footprint down to the required founding depth.

5.2 Shallow Foundation on Bedrock

Conventional strip and column footings set over sound bedrock may be designed using a maximum allowable bearing pressure of **1,000 kPa** for Ultimate Limit State (**ULS**) factored bearing resistance. Serviceability Limit State (**SLS**) does not apply for footings founded on bedrock since failure of the concrete would occur before unacceptable settlement of the foundation. For footings founded on sound bedrock, there are no restrictions for maximum footing sizes and grade raise fill thickness. Prior to pouring the footings, the rock should be free of any soil, debris or deleterious substances and should be inspected by a geotechnical engineer.

In order to limit the potential of differential settlement, the footings should rest entirely on either bedrock, or a minimum 300 mm thick structural fill pad; consisting of Granular B Type II, and not a combination of these materials.

The footings should be constructed on a relatively flat bedrock surface (10 degrees or less from the horizontal). If the footings will be founded on bedrock that is sloped greater than 10 degrees, and less than 30 degrees, rock anchors should be considered. For angles greater than 30 degrees, the bedrock must be levelled and step footings should be constructed.

Any excavations below the underside of footing for the proposed building to be founded on bedrock should be backfilled using lean concrete only, having a minimum compressive strength of 10 MPa at 28 days.

5.2.1 Rock Anchors

If the need for rock anchors is required, they should be designed by a structural engineer. The engineer will design the rock anchors based on the type of bedrock and strength parameters.

Grouted rock anchor may fail in one or more of the following modes:

- Failure within the rock mass;
- Failure of the rock/grout bond;
- Failure of the grout/tendon bond; or
- Failure of the steel tendon, or top anchorage.



The capacity of rock anchors is dependent on the bond between the rock and grout. The method of installation will also affect the capacity of the bond between the rock and the grout. An invert cone angle of 90° may be used in the design of the anchors. Pull out testing should be carried out on the anchors to verify installations and to design load capacities. If bedrock is removed through mechanical hydraulic hammers (i.e. for levelling and installation of anchors), it is not expected to affect the contribution of the upper level of rock in the calculation of anchors capacity.

The bond length (grouted portion of the dowel) should be a minimum of 3.0 m. Generally, the bond between the grout and dowel are twice the bond developed between the grout and the bedrock. Therefore, the design should be based on failure between the grout and the bedrock.

Straight-shafted dowels anchor force is dependent on the ultimate bond stress of the bedrock or the grout. Typically, the ultimate bond force is taken as 10% of the average unconfined compressive strength of the bedrock, or the compressive strength of the grout, whichever is less (but not more than 3.1 MPa). The allowable bond stress is taken as 50% of the ultimate bond stress.

The required bond length can be determined using the following equation:

$$L(m) = P/(\pi \times d \times T_b)$$

Where;

P = Working Capacity of anchor (kg);

T_b = working bond stress (kg/m²);

d = Core hole diameter (m).

5.3 Shallow Foundation on Structural Fill

Conventional strip and column footings set over properly compacted and approved structural fill (having a minimum thickness of 300 mm) conforming to OPSS Granular B Type II or approved equivalent may be designed for a maximum allowable bearing pressure of **150** kPa for Serviceability Limit State (SLS) and **225** kPa for Ultimate Limit State (ULS) factored bearing resistance. The factored ULS value includes the geotechnical resistance factor of 0.5. For footings founded on properly compacted structural fill overlying bedrock, there are no restrictions for maximum footing sizes and grade raise fill thickness.

The structural fill should be compacted to a minimum of 98% of its Standard Proctor Maximum Dry Density (SPMDD)

In order to allow the spread of load beneath the footings and to prevent undermining during construction, the structural fill should extend minimum 1.0 m beyond the outside edges of the footings and then outward and downward at 1 horizontal to 1 vertical profile (or flatter) over a distance equal to the depth of the structural fill below the footing.

5.4 Bedrock Excavation

It is expected that some bedrock excavation will be required as part of this development. It is anticipated that bedrock removal will be possible with the use of heavy excavation equipment, but that removal of most of the bedrock could be facilitated by means of a hoe



ramming operation. Both horizontal and vertical overbreak of the bedrock excavation face/bottom can be expected due to the hoe ramming operation. If control of potential bedrock overbreak is required, line drilling at the proposed excavation face is recommended. The smaller the distance between the drill holes, the fewer overbreaks is expected. It is generally considered that the drilling at 150 mm horizontal spacing to the full depth of the excavation should control overbreak to an acceptable level. Considering the proximity of the existing structures adjacent to the site and the potential for vibration during excavating and removal of the bedrock, monitoring of the hoe ramming shall be carried out throughout the operation on nearby buildings to ensure that the vibration limit is not exceeded. As outlined in **OPSS 120, Table 3** below summarizes the following vibration limits for the nearest existing structures.

In addition, a pre and post construction excavation condition survey of nearby structures is required to be carried out.

Table 3: Vibration Frequency and Limit

Frequency of Vibration (HZ)	Vibration Limit, PPV (Peak Particle Velocity) mm/sec
≤ 40	20
> 40	50

5.5 Lateral Earth Pressure

The following equation should be used to estimate the intensity of the lateral earth pressure against any earth retaining structure/foundation walls.

$$P = K (\gamma h + q)$$

Where;

P = Earth pressure at depth h;

K = Appropriate coefficient of earth pressure;

γ = Unit weight of compacted backfill, adjacent to the wall;

h = Depth (below adjacent to the highest grade) at which P is calculated;

q = Intensity of any surcharge distributed uniformly over the backfill surface (usually surcharge from traffic, equipment or soil stockpiled and typically considered 10 kPa).

The coefficient of earth pressure at rest (K_0) should be used in the calculation of the earth pressure on the storm water manhole/basement walls, which are expected to be rather rigid and not to deflect.

The above expression assumes that perimeter drainage system prevents the build-up of any hydrostatic pressure behind the foundation wall.

Table 4 below provides various material types and their respective earth pressure properties.



Table 4: Material and Earth Pressure Properties

Type of Material	Bulk Density (kN/m ³)	Friction Angle (Φ)	Pressure Coefficient		
			At Rest (K ₀)	Active (K _A)	Passive (K _P)
Granular A	23.0	34	0.44	0.28	3.53
Granular Type I B	20.0	31	0.49	0.32	3.12
Granular Type II B	23.0	32	0.47	0.31	3.25

5.6 Settlement

The estimated total settlement of the shallow foundations, designed using the recommended serviceability limit state capacity value, as well as other recommendations given above, will be less than 25 mm. The differential settlement between adjacent column footings is anticipated to be 15 mm or less.

5.7 Liquefaction

For buildings founded over bedrock or structural fill, the potential of soil liquefaction is not considered to be a concern.

5.8 Seismic

Based on the results of this geotechnical investigation and in accordance with the Ontario Building Code 2012 (table 4.1.8.4.A.) and Canadian Foundation Engineering Manual (4th edition), the site can be classified as Class “B” as per the Site Classification for Seismic Site Response. It should be noted that a greater seismic site response class may be obtained by conducting seismic velocity testing using a multichannel analysis of surface waves (MASW).

The above classifications were recommended based on conventional method exercised for Site Classification for Seismic Site Response and in accordance with the generally accepted geotechnical engineering practice.

5.9 Frost Protection

All exterior footings for any heated structure exposed to frost conditions should have a minimum of 1.5 m of earth cover. Footings for any unheated structures, signage or lighting, and where snow will be cleared, 1.8 m of earth cover is required. Alternatively, the required frost protection could be provided using a combination of earth cover and extruded polystyrene insulation. Detailed guidelines for footing insulation frost protection can be provided upon request.

In the event that foundations are to be constructed during winter months, the foundation soils are required to be protected from freezing temperatures using suitable construction techniques. The base of all excavations should be insulated from freezing temperatures immediately upon exposure, until heat can be supplied to the building interior and the footings have sufficient soil cover to prevent freezing of the subgrade soils.



5.10 Foundation Walls Backfill

To prevent possible lateral loading, the backfill material against any foundation walls, grade beams, isolated walls, or piers should consist of free draining, non-frost susceptible material such as sand or sand and gravel meeting OPSS Granular B Type I, II or Select Subgrade Material (SSM).

The foundation wall backfill should be compacted to minimum 95% of its SPMDD using light compaction equipment, where no loads will be set over top. The compaction shall be increased to 98% of its SPMDD under walkways, slabs or paved areas close to the foundation or retaining walls. Backfilling against foundation walls should be carried out on both sides of the wall at the same time where applicable.

5.11 Slab-on-grade Construction

Concrete slab-on-grade should rest over compacted, free draining and well graded structural fill only. Therefore, all overburden soils should be removed from the proposed building's footprint down to the bedrock surface. The exposed undisturbed bedrock should then be inspected and approved by qualified geotechnical personnel.

Any underfloor fill needed to raise the general floor grade shall consist of OPSS Granular B Type I, II, or SSM, compacted to 95% of its SPMDD. The final lift shall Granular B Type II, and compacted to 98% of its SPMDD. A 200 mm Granular A meeting the **OPSS 1010** shall be placed underneath the slab and compacted to 100% of its SPMDD.

It is also recommended that the area of extensive exterior slab-on-grade (sidewalks, ramp etc.) shall be constructed using Granular A base of thickness 150 mm. The modulus of subgrade reaction (ks) for the design of the slabs set over structural fill is **24 MPa/m**.

In order to further minimize and control cracking, the floor slab shall be provided with wire or fibre mesh reinforcement and construction or control joints. The construction or control joints should be spaced equal distance in both directions and should not exceed 4.5 m. The wire or fibre mesh reinforcement shall be carried out through the joints.

If any areas of the proposed building area are to remain unheated during the winter period, thermal protection of the slab on grade may be required. The "Guide for Concrete Floor and Slab Construction", **ACI 302.1R-04** is recommended to follow for the design and construction of vapour retarders below the floor slab. Further details on the insulation requirements could be provided, if necessary.

5.12 Corrosion Potential and Cement Type

A soil sample was submitted to Paracel Laboratories Ltd. for chemical testing. The following **Table 5** below summarizes the results.

Table 5: Results of Chemical Analysis

Sample Location	Depth (m)	pH	Sulphate (µg/g)	Chloride (µg/g)	Resistivity (Ohm.cm)
BH23-1	0.0 – 0.6	7.86	5610	71	460

The above results revealed a measured sulphate concentration of 5610 µg/g in the sample. Based on the CAN/CSA-A23.1 standards (Concrete Materials and Methods of



Concrete Construction), a sulphate concentration of less than 1000 µg/g falls within the negligible category for sulphate attack on buried concrete. The test results from soil samples were above the noted threshold. As such, a sulphate resistant cement is recommended for this site for buried concrete (ie: footings and foundation walls).

The pH, resistivity and chloride concentration provide an indication of the degree of corrosiveness of the sub-surface environment. The soil resistivity was measured to be 460 ohm.cm, which falls between severe range for soil resistivity, indicating an extremely corrosive environment.

6 EXCAVATION AND BACKFILLING REQUIREMENTS

6.1 Excavation

It is anticipated that the depth of excavation for the building or any proposed services will not extend below 2.4 m. Excavation must be carried-out in accordance with the Occupational Health and Safety Act and Regulations for Construction Projects.

According to the Ontario's Occupational Health and Safety Act (OHS), O. Reg. 213/91 and its amendments, the surficial overburden expected to be excavated into at this site can be classified as Type 3 for fully drained excavations. Therefore, shallow temporary excavations in the overburden soil can be cut at 1 horizontal to 1 vertical, for a fully drained excavation starting from the base of the excavation and as per requirements of the OHS regulations. When excavating into bedrock, the side of the excavation does not need to be sloped, and can be cut vertically from the base of excavation. It is recommended to install a protective meshing over the side walls of the bedrock to ensure loose rock debris will not fall into excavation.

Any excavated material stockpiled near an excavation or trench should be stored at a distance equal to or greater than the depth of the excavation/trench and construction equipment traffic should be limited near open excavation.

6.2 Groundwater Control

Based on the subsurface conditions encountered at this site, groundwater seepage or infiltration into the temporary excavations during construction is expected. This will be able to be controlled by pumping with sump pumps. Surface water runoff into the excavation should be minimized and diverted away from the excavation.

A permit to take water (PTTW) is required from Ministry of Environment and Climate Change (MOECC), Ontario Reg. 387/04, if more than 400,000 litres per day of groundwater will be pumped during a construction period less than 30 days. Registration in the Environmental Activity and Sector Registry (EASR) is required when water takings range between 50,000 and 400,000 litres per day.

The actual amount of groundwater inflow into open excavations will depend on several factors such as the contractor's schedule, rate of excavation, the size of excavation, depth below the groundwater level, and at the time of year which the excavation is executed. It is expected that pumping rates will be less than 50,000 litres per day. As such, EASR registration is not required for the construction at this site. However, a hydrological study may be carried out to better determine the amount of water to be pumped.



6.3 Pipe Bedding Requirements

It is anticipated that any underground services required as part of this project will be founded over properly prepared and approved structural fill. Consequently all organic material should be removed down to a suitable bearing layer. Any sub-excavation of disturbed soil should be removed and replaced with a Granular B Type II or I, or an approved equivalent, laid in loose lifts of thickness not exceeding 300 mm and compacted to 95% of its SPMDD. Bedding, thickness of cover material and compaction requirements for watermains and sewer pipes should conform to the manufacturer's design requirements and to the detailed installations outlined in the Ontario Provincial Standard Specifications (OPSS) or any other applicable standards.

6.4 Trench Backfill

All service trenches should be backfilled using compactable material, free of organics, debris and large cobbles or boulders. Acceptable native materials (if encountered and where possible) should be used as backfill between the roadway subgrade level and the depth of seasonal frost penetrations (i.e. 1.8 m below finished grade) in order to reduce the potential for differential frost heaving between the new excavated trench and the adjacent section of roadway. Where native backfill is used, it should match the native materials exposed on the trench walls. Backfill below the zone of seasonal frost penetration could consist of either acceptable native material or imported granular material conforming to OPSS Granular B Type II. Any boulders larger than 150 mm in size should not be used as trench backfill.

To minimize future settlement of the backfill and achieve an acceptable subgrade for the roadway, the trench should be compacted in maximum 300 mm thick lifts to at least 95% of its SPMDD. The specified density may be reduced where the trench backfill is not located within or in close proximity to existing roadways or any other structures.

For trenches carried out in existing paved areas, transitions should be constructed to ensure that proper compaction is achieved between any new pavement structure and the existing pavement structure to minimize potential future differential settlement between the existing and new pavement structure. The transition should start at the subgrade level and extend to the underside of the asphaltic concrete level (if any) at a 1 horizontal to 1 vertical slope. This is especially important where trench boxes are used and where no side slopes is provided to the excavation. Where asphaltic concrete is present, it should be cut back to a minimum of 150 mm from the edge of the excavation to allow for proper compaction between the new and existing pavement structures.

7 REUSE OF ON-SITE SOILS

The existing surficial overburden materials consists of fill material. This material could be reused as general backfill material (service trenches, general landscaping/backfilling) if it can be compacted according to the specifications outlined herein at the time of construction and found free from any waste, organics and debris.

It should be noted that the adequacy of any material for reuse as backfill will depend on its water content at the time of its use and on the weather conditions prevailing prior to and during that time. Therefore, all excavated materials to be reused shall be stockpiled in a manner that will prevent any significant changes in their moisture content, especially during wet conditions, and approved for reuse by a geotechnical engineer.



8

8 PAVEMENT REINSTATEMENT

There are no access roads or municipal streets proposed to be constructed as part of this project. However, there may be some street reinstatement from connecting to the municipal services.

The reinstatement of any pavement structure within the existing street should be conducted as recommended in **Section 6.4** and the pavement structure should be reinstated to match at minimum what already exists.

Where the existing asphaltic concrete surface of a roadway is affected by the excavating process, the damaged zones should be saw cut and any damaged or loose pieces of asphaltic concrete should be removed down to the binder course or its entire depth, where only one layer exist. The existing base should be scarified and proof-rolled with any soft areas excavated and replaced to the proper level with OPSS Granular A. Where two layers of asphalt exist on an access lane, the surface course should be grinded over a width of 150mm to allow the new surface course to overlap the binder layer and not create one straight vertical joint. On existing streets, the overlap should be increased to 300 mm.

9 INSPECTION SERVICES

The engagement of the services of the geotechnical consultant during construction is recommended to confirm that the subsurface conditions throughout the proposed site do not materially differ from those given in the report and that the construction activities do not adversely affect the intent of the design.

All footing areas and any structural fill areas for the proposed building should be inspected by LRL to ensure that a suitable subgrade has been reached and properly prepared. The placing and compaction of any granular materials beneath the foundations and slab-on-grade should be inspected to ensure that the materials used conform to the required gradation and compaction specifications.

If the footings are to be constructed during winter season, the footing subgrade should be protected from freezing temperatures using suitable construction techniques.

10 REPORT CONDITIONS AND LIMITATIONS

It is stressed that the information presented in this report is provided for the guidance of the designers and is intended for this project only. The use of this report as a construction document or its use by a third party beyond the client specifically listed in the report is neither intended nor authorized by LRL Associates Ltd. Contractors bidding on or undertaking the works should examine the factual results of the investigation, satisfy themselves as to the adequacy of the information for construction, and make their own interpretation of the factual data as it affects their construction techniques, schedule, safety and equipment capabilities.

The professional services for this project include only the geotechnical aspects of the subsurface conditions at this site. The presence or implications of possible contamination resulting from previous uses or activities at this site or adjacent properties, and/or resulting from the introduction onto the site of materials from off-site sources are outside the terms of reference for this report.



The recommendations provided in this report are based on subsurface data obtained at the specific test pit locations only. Boundaries between zones presented on the test pit logs are often not distinct but transitional and were interpreted. Experience indicates that the subsurface soil and groundwater conditions can vary significantly between and beyond the test locations. For this reason, the recommendations given in this report are subject to a field verification of the subsurface soil conditions at the time of construction.

The recommendations are applicable only to the project described in this report. Any changes to the project will require a review by LRL Associates Ltd., to ensure compatibility with the recommendations contained in this project.

We trust this report provides sufficient information for your present purposes. If you have any questions concerning this report or if we may be of further services to you, please do not hesitate to contact the undersigned.

Yours truly,
LRL Associates Ltd.



Brad Johnson, P. Eng.
Geotechnical Engineer



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APPENDIX A
Site and Borehole Location Plan



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5430 Canotek Road | Ottawa, ON, K1J 9G2
www.lri.ca | (613) 842-3434

PROJECT

GEOTECHNICAL INVESTIGATION
SITE PLAN APPLICATION
725 SOMERSET STREET WEST
OTTAWA, ONTARIO

DRAWING TITLE

SITE LOCATION
(NOT TO SCALE)
Source: GeoOttawa

CLIENT

UPSTAY CANADA INC.

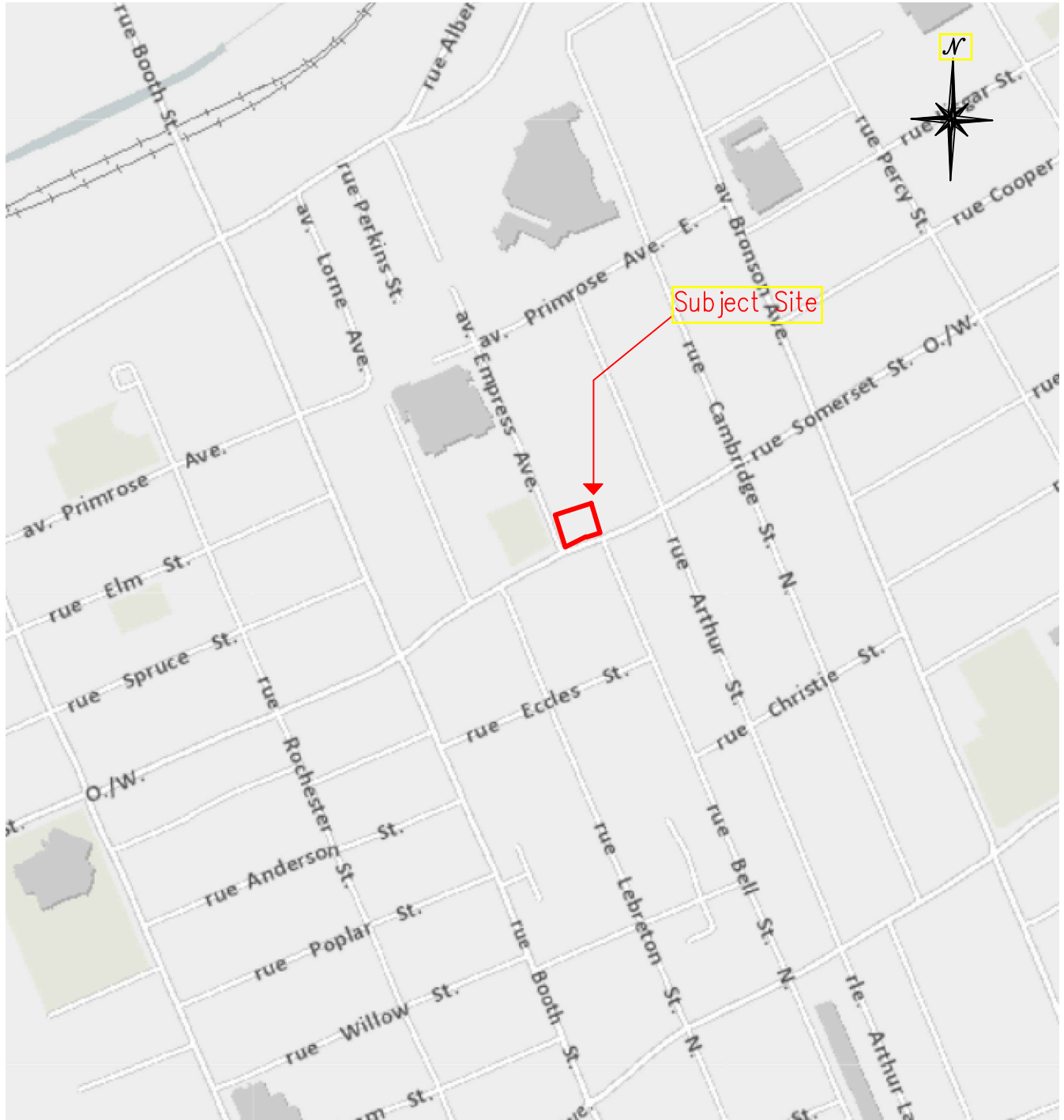
DATE

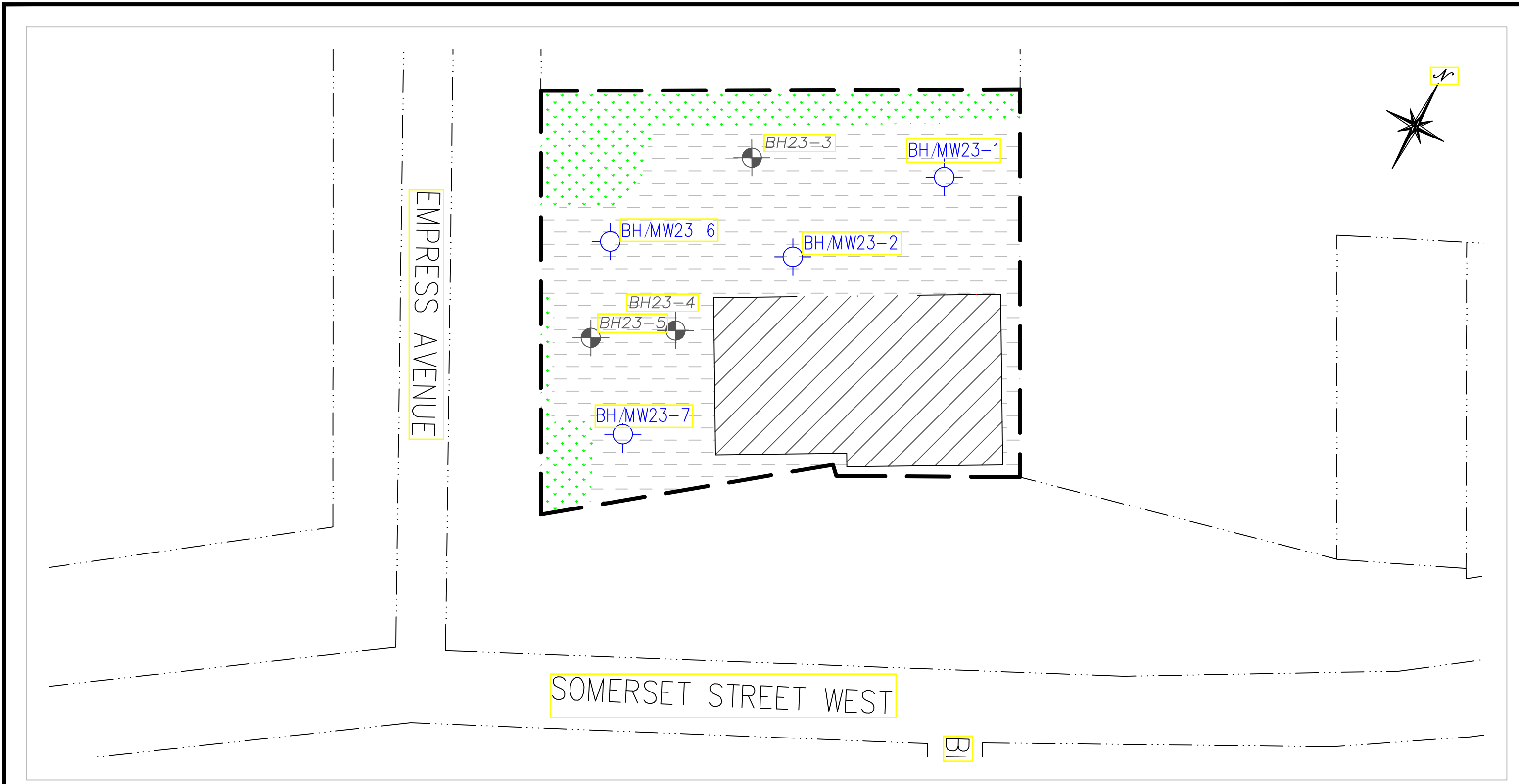
MAY 2023

PROJECT

220463

FIGURE 1





LEGEND

- SUBJECT PROPERTY BOUNDARY
- NEIGHBOURING PROPERTY BOUNDARY
- EXISTING BUILDING
- GRASSED AREA
- ASPHALT AREA
- MONITORING WELL AND GROUND SURFACE ELEVATION
- BOREHOLE AND GROUND SURFACE ELEVATION

10m 5 0 10 20m

SCALE: 1:600

No.	REVISIONS	BY	DATE
01	FINAL	A.K.	26/05/2023

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5430 Canotek Road | Ottawa, ON, K1J 9G2
www.lri.ca | (613) 842-3434

CLIENT
Upstay Canada Inc.

DESIGNED BY: A.K. DRAWN BY: A.K. APPROVED BY: J.A.

PROJECT
**GEOTECHNICAL INVESTIGATION
SITE PLAN APPLICATION
725 SOMERSET STREET WEST
OTTAWA, ONTARIO**

DRAWING TITLE
**SITE PLAN
SCALE 1:600**

PROJECT NO.
220463

DATE
MAY 2023

FIGURE 2

APPENDIX B
Borehole Logs



LRJ

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5430 Caroleek Road | Ottawa, ON, K1J 9G2
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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 15, 2023

BOREHOLE LOG: BH/MW23-1

PROJECT: GEOTECHNICAL INVESTIGATION AND
PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	SPT N Values (Blows/0.3 m)		Vane Shear Strength (KPa)		Water Content %		MONITORING WELL DETAILS
									20 40 60 80	20 40 60 80	20 40 60 80	25 50 75	25 50 75		
0.0	ASPHALT	99.93													
0.0		0.00													
0.0		99.78													
1.0	FILL: Sand and gravel, grey, becoming brown at (0.6 m bgs), dry.	0.15			SS1	70	100						4.0		
2.0					SS2	50 / 5'	50						4.3		
3.0		99.18													
3.0	BEDROCK Grey sandstone, fractured.	0.60													
4.0					R1	50	98								
5.0															
6.0															
7.0															
8.0															
9.0															
10.0					R2	95	95								
11.0															
12.0															
13.0															
14.0					R3	78	100								
15.0															
15.0		94.53													
16.0	End of Borehole	4.65													
17.0															
18.0															
19.0															
20.0															

EASTING: 18T 0444569

NORTHING: 5028830

SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).

GROUND SURFACE ELEVATION: 99.93 m

TOP OF RISER ELEVATION: 99.87 m

HOLE DIAMETER: 91 mm

MONITORING WELL DIAMETER: 51 mm

NOTES:

bgs: Below Ground Surface
N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 15, 2023

BOREHOLE LOG: BH/MW23-2

PROJECT: GEOTECHNICAL INVESTIGATION AND
PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	Vane Shear Strength (KPa)		Water Content (%)		MONITORING WELL DETAILS
									20	40	60	80	
0.0	ASPHALT	99.49											
0.0		0.00											
0.0		99.34											
0.15	FILL: Sand and gravel, grey, becoming brown at (0.2 m bgs), moist become dray at (0.2-0.3m bgs).	0.15		SS1	7	70			7		4.8		
0.15		98.79											
0.55	BEDROCK Grey sandstone, fractured.	0.55											
0.55													
1.0					R1	64	90						
1.0													
2.0													
3.0													
4.0													
5.0													
6.0													
7.0													
8.0													
9.0					R2	98	100						
9.0													
10.0													
11.0													
12.0													
13.0													
14.0													
15.0					R3	78	100						
15.0													
15.0	End of Borehole	94.34											
15.0		4.45											
16.0													
17.0													
18.0													
19.0													
20.0													

EASTING: 18T 0444567 **NORTHING:** 5028821
SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).
GROUND SURFACE ELEVATION: 99.49 m **TOP OF RISER ELEVATION:** 99.35 m
HOLE DIAMETER: 91 mm **MONITORING WELL DIAMETER:** 51 mm

NOTES:
 bgs: Below Ground Surface
 N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 15, 2023

BOREHOLE LOG: BH23-3

PROJECT: GEOTECHNICAL INVESTIGATION AND
PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	Vane Shear Strength (KPa)		Water Content (%)		MONITORING WELL DETAILS
									SPT N Values (Blows/0.3 m)	Vane Remold Shear Strength (KPa)	Liquid Limit (%)		
0.0	ASPHALT	99.61											
0.0	FILL:	99.51											
0.10	Sand and gravel, grey, becoming brown at (0.75 m bgs), dry.	0.10			SS1	32	60		32		3.0		
2.0	End of Borehole	98.76			SS2	50 / 4"	67		0		5.0		
0.75		0.75											

EASTING: 18T 0444554 **NORTHING:** 5028826
SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).
GROUND SURFACE ELEVATION: 99.61m **TOP OF RISER ELEVATION:** N/A
HOLE DIAMETER: 91 mm **MONITORING WELL DIAMETER:** N/A

NOTES:
 bgs: Below Ground Surface
 N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 15, 2023

BOREHOLE LOG: BH23-4

PROJECT: GEOTECHNICAL INVESTIGATION AND
PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR ROD (%)	RECOVERY (%)	LABORATORY ANALYSIS	SPT N Values (Blows/0.3 m)		Vane Shear Strength (KPa)	Water Content (%)	MONITORING WELL DETAILS
									20 40 60 80	20 40 60 80	25 50 75		
0.0	ASPHALT	99.17											
0.0	FILL:	99.07											
0.10	Sand and gravel, brown, dry.	0.10			SS1	9	53					4.5	
0.60	End of Borehole	98.47											

EASTING: 18T 044454

NORTHING: 5028814

SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).

GROUND SURFACE ELEVATION: 99.17 m

TOP OF RISER ELEVATION: N/A

HOLE DIAMETER: 91 mm

MONITORING WELL DIAMETER: N/A

NOTES:

bgs: Below Ground Surface
N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 16, 2023

BOREHOLE LOG: BH23-5

PROJECT: GEOTECHNICAL INVESTIGATION AND PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	SPT N Values (Blows/0.3 m)		Vane Shear Strength (KPa)		Water Content (%)		MONITORING WELL DETAILS
									20 40 60 80	20 40 60 80	20 40 60 80	25 50 75			
0.0	ASPHALT	99.09													
0.13	FILL: Sand and gravel, brown, dry.	99.96			SS1	8	29		8				4.1		
0.75	End of Borehole	98.21													

EASTING: 18T 0444547

NORTHING: 5028809

SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).

GROUND SURFACE ELEVATION: 99.09 m

TOP OF RISER ELEVATION: N/A

HOLE DIAMETER: 91 mm

MONITORING WELL DIAMETER: N/A

NOTES:

bgs: Below Ground Surface
N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 16, 2023

BOREHOLE LOG: BH/MW23-6

PROJECT: GEOTECHNICAL INVESTIGATION AND PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	Vane Shear Strength (KPa)		Water Content (%)		MONITORING WELL DETAILS
									20	40	60	80	
0.0	ASPHALT FILL: Sand and gravel, grey, becoming brown at (0 - 0.05m bgs), moist at (0-0.1 m bgs), dry.	99.29 0.00 99.19											
0.10				SS1	16	52					4.1		
0.65	BEDROCK Grey sandstone, fractured.	98.54 0.65											
1.0					R1	38	100						
2.0													
3.0													
4.0													
5.0													
6.0													
7.0													
8.0					R2	66	100						
9.0													
10.0													
11.0													
12.0													
13.0													
14.0													
15.0					R3	85	100						
16.0	End of Borehole	94.09 4.45											
17.0													
18.0													
19.0													
20.0													

EASTING: 18T 0444543 **NORTHING:** 5028826
SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).
GROUND SURFACE ELEVATION: 99.29 m **TOP OF RISER ELEVATION:** 99.18 m
HOLE DIAMETER: 91 mm **MONITORING WELL DIAMETER:** 51 mm

NOTES:
bgs: Below Ground Surface
N/A: Not applicable



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PROJECT NO.: 220463

CLIENT: UPSTAY CANADA INC.

DATE: MAY 16, 2023

BOREHOLE LOG: BH/MW23-7

PROJECT: GEOTECHNICAL INVESTIGATION AND PHASE II ENVIRONMENTAL SITE ASSESSMENT
LOCATION: 725 SOMERSET STREET, OTTAWA, ONTARIO

FIELD PERSONNEL: ABDUL KADER / RAED

DRILLER: GEORGE DOWNING ESTATE DRILLING LTD.

DRILLING EQUIPMENT: TRACK MOUNTED CME-55

DRILLING METHOD: Hollow Stem Auger

DEPTH	SOIL DESCRIPTION	ELEV./DEPTH (m)	LITHOLOGY	TYPE	SAMPLE NUMBER	N OR RQD (%)	RECOVERY (%)	LABORATORY ANALYSIS	SPT N Values (Blows/0.3 m)		Vane Shear Strength (KPa)		Water Content (%)		MONITORING WELL DETAILS
									20	40	60	80	20	40	
0.0	ASPHALT FILL: Sand and gravel, grey, brown, become stained at (0.10-0.85m bgs) dry.	98.90 0.00 98.80 0.10													
1.0					SS1	14	42		14				4.0		
2.0					SS2	50 / 2"	83						8.9		
3.0	BEDROCK Grey sandstone, fractured.	97.95 0.85													
4.0					R1	81	99								
5.0															
6.0					R2	94	100								
7.0															
8.0															
9.0															
10.0															
11.0															
12.0															
13.0															
14.0					R3	71	100								
15.0															
16.0	End of Borehole	93.35 4.60													
17.0															
18.0															
19.0															
20.0															

EASTING: 18T 0444539 **NORTHING:** 5028803
SITE DATUM: Elevations measured from temporary benchmark established at the top of the fire hydrant near the south western corner of the Site (100.00 m).
GROUND SURFACE ELEVATION: 98.90 m **TOP OF RISER ELEVATION:** 98.80 m
HOLE DIAMETER: 91 mm **MONITORING WELL DIAMETER:** 51 mm

NOTES:
 bgs: Below Ground Surface
 N/A: Not applicable

APPENDIX C
Symbols and Terms used in Borehole Logs

Symbols and Terms Used on Borehole and Test Pit Logs

1. Soil Description

The soil descriptions presented in this report are based on commonly accepted methods of classification and identification employed in geotechnical practice. Classification and identification of soil involves some judgement and LRL Associates Ltd. does not guarantee descriptions as exact, but infers accuracy to the extent that is common in current geotechnical practice. Boundaries between zones on the logs are often not distinct but transitional and were interpreted.

a. Proportion

The proportion of each constituent part, as defined by the grain size distribution, is denoted by the following terms:

Term	Proportions
“trace”	1% to 10%
“some”	10% to 20%
prefix (i.e. “sandy” silt)	20% to 35%
“and” (i.e. sand “and” gravel)	35% to 50%

b. Compactness and Consistency

The state of compactness of granular soils is defined on the basis of the Standard Penetration Number (N) as per ASTM D-1586. It corresponds to the number of blows required to drive 300 mm of the split spoon sampler using a metal drop hammer that has a weight of 62.5 kg and free fall distance of 760 mm. For a 600 mm long split spoon, the blow counts are recorded for every 150 mm. The “N” value is obtained by adding the number of blows from the 2nd and 3rd count. Technical refusal indicates a number of blows greater than 50.

The consistency of clayey or cohesive soils is based on the shear strength of the soil, as determined by field vane tests and by a visual and tactile assessment of the soil strength.

The state of compactness of granular soils is defined by the following terms:

State of Compactness Granular Soils	Standard Penetration Number “N”	Relative Density (%)
Very loose	0 – 4	<15
Loose	4 – 10	15 – 35
Compact	10 - 30	35 – 65
Dense	30 - 50	65 - 85
Very dense	> 50	> 85

The consistency of cohesive soils is defined by the following terms:

Consistency Cohesive Soils	Undrained Shear Strength (C_u) (kPa)	Standard Penetration Number “N”
Very soft	<12.5	<2
Soft	12.5 - 25	2 - 4
Firm	25 - 50	4 - 8
Stiff	50 - 100	8 - 15
Very stiff	100 - 200	15 - 30
Hard	>200	>30

c. Field Moisture Condition

Description (ASTM D2488)	Criteria
Dry	Absence of moisture, dusty, dry to touch.
Moist	Damp, but not visible water.
Wet	Visible, free water, usually soil is below water table.

2. Sample Data

a. Elevation depth

This is a reference to the geodesic elevation of the soil or to a benchmark of an arbitrary elevation at the location of the borehole or test pit. The depth of geological boundaries is measured from ground surface.

b. Type

Symbol	Type	Letter Code
⋮	Auger	AU
⚡	Split Spoon	SS
	Shelby Tube	ST
	Rock Core	RC

c. Sample Number

Each sample taken from the borehole is numbered in the field as shown in this column.

LETTER CODE (as above) – Sample Number.

d. Recovery (%)

For soil samples this is the percentage of the recovered sample obtained versus the length sampled. In the case of rock, the percentage is the length of rock core recovered compared to the length of the drill run.

3. Rock Description

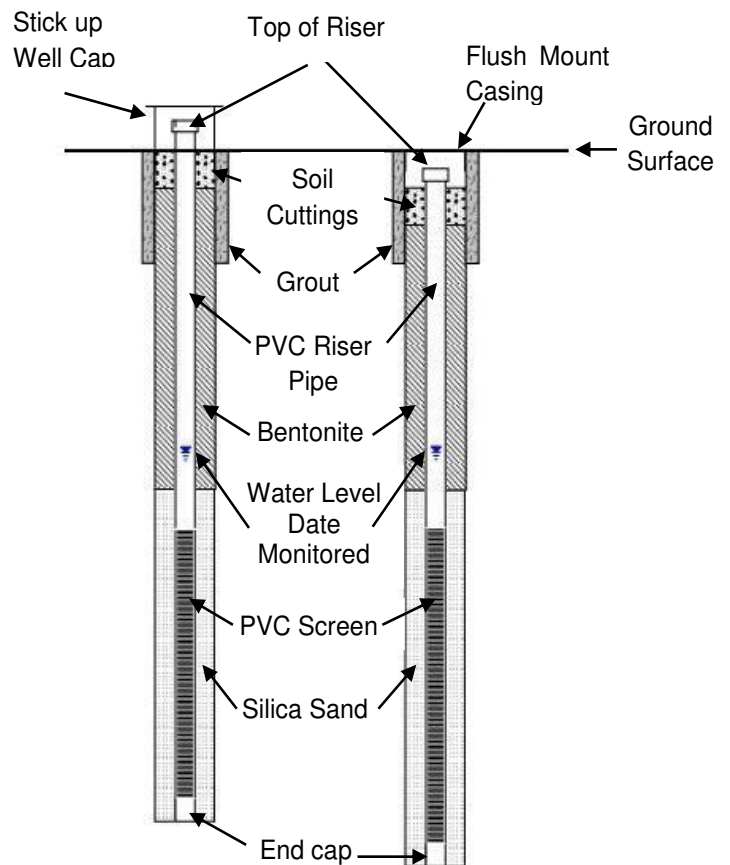
Rock Quality Designation (RQD) is a rough measure of the degree of jointing or fracture in a rock mass. The RQD is calculated as the cumulative length of rock pieces recovered having lengths of 100 mm or more divided by the length of coring. The qualitative description of the bedrock based on RQD is given below.

Rock Quality Designation (RQD) (%)	Description of Rock Quality
0 – 25	Very poor
25 – 50	Poor
50 – 75	Fair
75 – 90	Good
90 – 100	Excellent

Strength classification of rock is presented below.

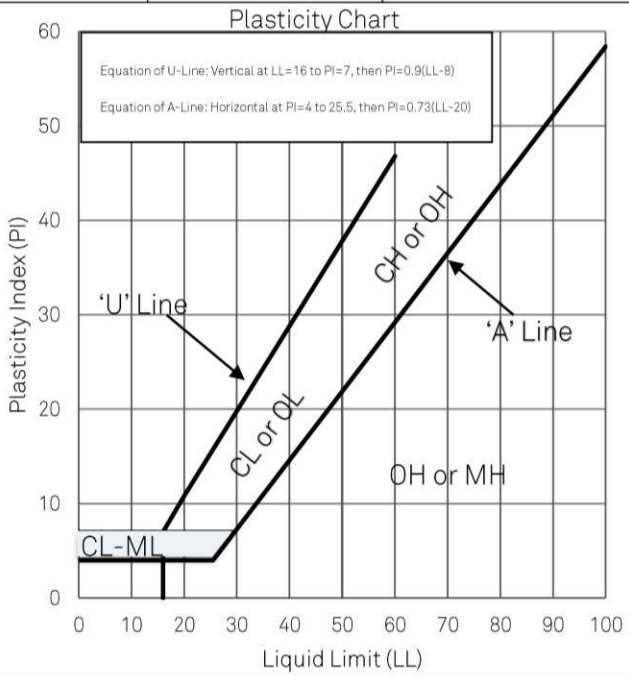
Strength Classification	Range of Unconfined Compressive Strength (MPa)
Extremely weak	< 1
Very weak	1 – 5
Weak	5 – 25
Medium strong	25 – 50
Strong	50 – 100
Very strong	100 – 250
Extremely strong	> 250

4. General Monitoring Well Data



**5. Classification of Soils for Engineering Purposes (ASTM D2487)
(United Soil Classification System)**

Major divisions		Group Symbol	Typical Names	Classification Criteria
Coarse-grained soils More than 50% retained on No. 200 sieve* (>0.075 mm)	Gravels More than 50% of coarse fraction retained on No. 4 sieve(4.75 mm)	Clean gravels <5% fines	GW	Well-graded gravel
			GP	Poorly graded gravel
		Gravels with >12% fines	GM	Silty gravel
			GC	Clayey gravel
	Sands 50% or more of coarse fraction passes No. 4 sieve(<4.75 mm)	Clean sands <5% fines	SW	Well-graded sand
			SP	Poorly graded sand
		Sands with >12% fines	SM	Silty sand
			SC	Clayey sand
				<p>Classification on basis of percentage of fines: Less than 5% pass No. 200 sieve - GW, GP, SW, SP More than 12% pass No. 200 sieve - GM, GC, SM, SC 5 to 12% pass No. 200 sieve - Borderline classifications, use of dual symbols</p> <p>$C_u = \frac{D_{60}}{D_{10}} \geq 4$; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3</p> <p>Not meeting either C_u or C_c criteria for GW</p> <p>Atterberg limits below "A" line or PI less than 4</p> <p>Atterberg limits on or above "A" line and PI > 7</p> <p>Atterberg limits plotting in hatched area are borderline classifications requiring use of dual symbols</p> <p>If fines are organic add "with organic fines" to group name</p> <p>$C_u = \frac{D_{60}}{D_{10}} \geq 6$; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3</p> <p>Not meeting either C_u or C_c criteria for SW</p> <p>Atterberg limits below "A" line or PI less than 4</p> <p>Atterberg limits plotting in hatched area are borderline classifications requiring use of dual symbols</p> <p>If fines are organic add "with organic fines" to group name</p>
Fine-grained soils 50% or more passes No. 200 sieve* (<0.075 mm)	Silts and Clays Liquid Limit <50%	Inorganic	ML	Silt
			CL	Lean Clay -low plasticity
	Silts and Clays Liquid Limit >50%	Organic	OL	Organic clay or silt (Clay plots above 'A' Line)
		Inorganic	MH	Elastic silt
			CH	Fat Clay -high plasticity
	Highly Organic Soils	Organic	OH	Organic clay or silt (Clay plots above 'A' Line)
		PT		Peat, muck and other highly organic soils
				<p>If 15 to 29% coarse-grained, add "with sand" or "with gravel" as appropriate. If > 30% coarse-grained, add "sandy" or "gravelly" as appropriate. Class as organic when oven dried liquid limit is < 75% of undried liquid limit.</p>



APPENDIX D
Laboratory Results



Unconfined Compressive Strength of Intact Rock Core

ASTM D 7012: Method C

Client: Upstay Canada Inc.
 Project: Geotechnical Investigation
 Location: 725 Somerset St. W., Ottawa, ON.

File No.: 220463
 Report No.: 1
 Date: May 24, 2023

Drill Core Information

Date(s) Sampled: May 15 & 16, 2023
 Sampled By: LRL Engineering
 Date Received: May 16, 2023

Laboratory Identification	Core No.	Field Identification	Borehole	Run	Depth, m	Location / Description
C01754	1	n/a	BH 23-1	RC 1	1.52 - 1.68	n/a
C01755	2	n/a	BH 23-2	RC 2	2.44 - 2.69	n/a
C01756	3	n/a	BH 23-7	RC 3	3.71 - 4.01	n/a

Rock Core Unconfined Compressive Strength Test Data

Laboratory Identification	Core No.	Conditioning	Length, mm	Diameter, mm	Density, kg/m ³	MPa	Description of Failure
C01754	1	As received	130.0	63.0	2709	94.0	Columnar, relatively well formed cone on one end
C01755	2	As received	133.2	63.0	2654	76.6	Columnar, relatively well formed cone on one end
C01756	3	As received	131.2	63.0	2703	107.2	Columnar, relatively well formed cone on one end

Comments: _____

Date Issued: May 25, 2023

Reviewed By: 

W.A.M. Laughlin, Geo. Tech.

Certificate of Analysis

LRL Associates Ltd.

5430 Canotek Road
Ottawa, ON K1J 9G2
Attn: Brad Johnson

Client PO:
Project: 220463
Custody: 71523

Report Date: 26-May-2023
Order Date: 24-May-2023

Order #: 2321129

This Certificate of Analysis contains analytical data applicable to the following samples as submitted :

Paracel ID	Client ID
2321129-01	BH 23-1 SS1

Approved By:



Mark Foto, M.Sc.
Lab Supervisor

Certificate of Analysis
Client: **LRL Associates Ltd.**
Client PO:

Report Date: 26-May-2023
Order Date: 24-May-2023
Project Description: **220463**

Analysis Summary Table

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC, water extraction	25-May-23	26-May-23
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	26-May-23	26-May-23
Resistivity	EPA 120.1 - probe, water extraction	25-May-23	26-May-23
Solids, %	CWS Tier 1 - Gravimetric	25-May-23	26-May-23

Certificate of Analysis

Report Date: 26-May-2023

Client: LRL Associates Ltd.

Order Date: 24-May-2023

Client PO:

Project Description: 220463

Client ID:	BH 23-1 SS1	-	-	-
Sample Date:	15-May-23 00:00	-	-	-
Sample ID:	2321129-01	-	-	-
MDL/Units	Soil	-	-	-

Physical Characteristics

% Solids	0.1 % by Wt.	96.9	-	-	-
----------	--------------	------	---	---	---

General Inorganics

pH	0.05 pH Units	7.86	-	-	-
Resistivity	0.1 Ohm.m	4.6	-	-	-

Anions

Chloride	10 ug/g dry	71	-	-	-
Sulphate	10 ug/g dry	5610	-	-	-

Certificate of Analysis

Report Date: 26-May-2023

Client: LRL Associates Ltd.

Order Date: 24-May-2023

Client PO:

Project Description: 220463

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	10	ug/g						
Sulphate	ND	10	ug/g						
General Inorganics									
Resistivity	ND	0.1	Ohm.m						

Certificate of Analysis
 Client: LRL Associates Ltd.
 Client PO:

Report Date: 26-May-2023
 Order Date: 24-May-2023
 Project Description: 220463

Method Quality Control: Duplicate

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	ND	10	ug/g	ND			NC	35	
Sulphate	ND	10	ug/g	ND			NC	35	
General Inorganics									
pH	6.83	0.05	pH Units	6.85			0.3	2.3	
Resistivity	189	0.1	Ohm.m	193			1.9	20	
Physical Characteristics									
% Solids	89.9	0.1	% by Wt.	89.4			0.6	25	

Certificate of Analysis
 Client: LRL Associates Ltd.
 Client PO:

Report Date: 26-May-2023
 Order Date: 24-May-2023
 Project Description: 220463

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	98.9	10	ug/g	ND	98.9	82-118			
Sulphate	102	10	ug/g	ND	102	80-120			

Certificate of Analysis

Report Date: 26-May-2023

Client: LRL Associates Ltd.

Order Date: 24-May-2023

Client PO:

Project Description: 220463

Qualifier Notes:

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable

ND: Not Detected

MDL: Method Detection Limit

Source Result: Data used as source for matrix and duplicate samples

%REC: Percent recovery.

RPD: Relative percent difference.

NC: Not Calculated

Soil results are reported on a dry weight basis when the units are denoted with 'dry'.
Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.