PROPOSED KOREAN COMMUNITY CHURCH, 3555 BORRISOKANE ROAD CITY OF OTTAWA

TRAFFIC IMPACT ASSESSMENT REPORT

Presented to:

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CASTLEGLENN CONSULTANTS LTD.

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INTRODUCTION

The 2017 City of Ottawa "*Transportation Impact Assessment Guidelines*" set out a multi-step preapplication process where the scope, assumptions, study area and methodology to conduct a transportation impact assessment (TIA) are detailed and each sequential stage approved by City of Ottawa staff.

This report provides a summary of the following reports which have already been submitted to City staff:

- the Screening Report (Step 1) that was submitted to the City on Wednesday, January 18th, 2023; and
- the Scoping Report (Step 2) that was submitted to the City on Tuesday, February 28th, 2023

Transportation comments from City staff, as well as consultant responses, concerning both the pre-consultation phase (March 26th, 2021) and the various TIA submissions are documented within Appendix "G".

This report represents a combined Forecasting Report and Strategy Report (representing Steps 3 and 4) of the multi-step TIA process.

1.0 SCREENING REPORT (Submitted to the City on Jan. 18th, 2023)

1.1 SUMMARY OF DEVELOPMENT

Exhibit 2-2 illustrates proposed site plan (July, 2023) for the Korean Community Church which is proposed as a single storey building with a gross area of 2,913.5 m² to be located at 3555 Borrisokane Road. The development is proposed to have two entrances:

- An access onto Borrisokane Road: and
- A secondary access through the adjacent Halo Car Wash that would connect to Flagstaff Drive.

1.2 SCREENING RESULTS

The City of Ottawa's initial screening process is used to determine...:

- if the number of trips generated by the development makes it desirable to assess the development design and transportation system performance of one or more modes;
- if the development's location makes it desirable to assess development design; or
- if the development and/or boundary street conditions yield a high potential for safety concerns.

1.2.1 Screening: Trip Generation Triggers

The City of Ottawa TIA guidelines set the threshold for the trip generation trigger at 60 person-trips-or-more during the weekday peak hours. If the proposed development meets the trip threshold, both the Design Review and Network Impact components of the TIA are to be considered.

The threshold to meet a trip generation warrant¹ for the existing industrial land use zoning (which is acceptable zoning for "a place of assembly") is established at 5,000 m². Since the proposed Korean

^{1. &}quot;City of Ottawa Transportation Impact Assessment Guidelines". Dillon Consulting, June 2017

Community Church is approximately 2,913.5 m² (or 31,360 ft²), the Trip Generation Trigger was found NOT to be satisfied.

1.2.2 Screening: Location Triggers

The development site is not proposing any new driveways onto boundary streets that are part of the City of Ottawa's Transit Priority, Rapid Transit or Spine Bicycle Networks. The parcel is currently zoned "IL(304)" which is defined as a "Light Industrial Zone". As well the location is not designated as a Design Priority Area; therefore, the Location Trigger was found NOT to be satisfied.

1.2.3 Screening: Safety Triggers

The proposed development is located on Borrisokane Road, which has a posted speed limit of 80km/h. The entrance onto Borrisokane Road is located on a horizontal curve, raising the possibility of limited sight lines; therefore, the "Safety Trigger" was found to be satisfied.

1.2.4 Screening: Conclusions

The screening results indicate that only the Safety Trigger was found to be satisfied, the TIA is required to address the "Design Review" component of the TIA process.

The completed stamped and signed screening forms can be found within Appendix "B".

2.0 SCOPING REPORT (Submitted to the City on Feb. 28th, 2023)

2.1 EXISTING AND PLANNED CONDITIONS

2.1.1 The Proposed Development

Exhibit 2-1 conceptually illustrates the location of the proposed Korean Community Church. The frontage of the lot faces Borrisokane Road and the lot is situated south of Flagstaff Drive and North of Cambrian Road.

The development lands are currently zoned "IL[304] – Light industrial Zone", which accommodates acceptable zoning for "a place of assembly" such as a church. The lot is currently undeveloped.

The development of the Korean Community Church:



Exhibit 2-1: Location of Proposed Development

- would provide for a building structure of approximately 31,360 ft²;
- the assembly area, (on which the City of Ottawa's parking requirement is based), sums to 541 m² [Chapel No. 1: 383 m² 4,123 ft² : Chapel No. 2: 158 m² 1,700 ft²]
- would be developed within a single construction phase.;
- has an anticipated estimated timing of occupancy of the Fall of 2024; and
- would accommodate 136 on-site parking stalls. [Based on the City of Ottawa Zoning By-Law which indicated for N66 Place-of-Worship a parking rate of 10 stalls-per-100m² of assembly area, a total of 54 parking stalls would be required.]

During the pre-consultation meeting (held March 26, 2021), it was noted that the church may choose in the future to use some of its facilities as a daycare during the week as current zoning permits this use. However, it was later clarified that there are currently no formal plans in place for this land use. The traffic forecasting component of the TIA study was therefore assumed to include a maximum 30 child daycare facility.

Exhibit 2-2 illustrates the proposed site plan upon which this traffic impact assessment (TIA) is based. (See Appendix "C" for the full site plan.)

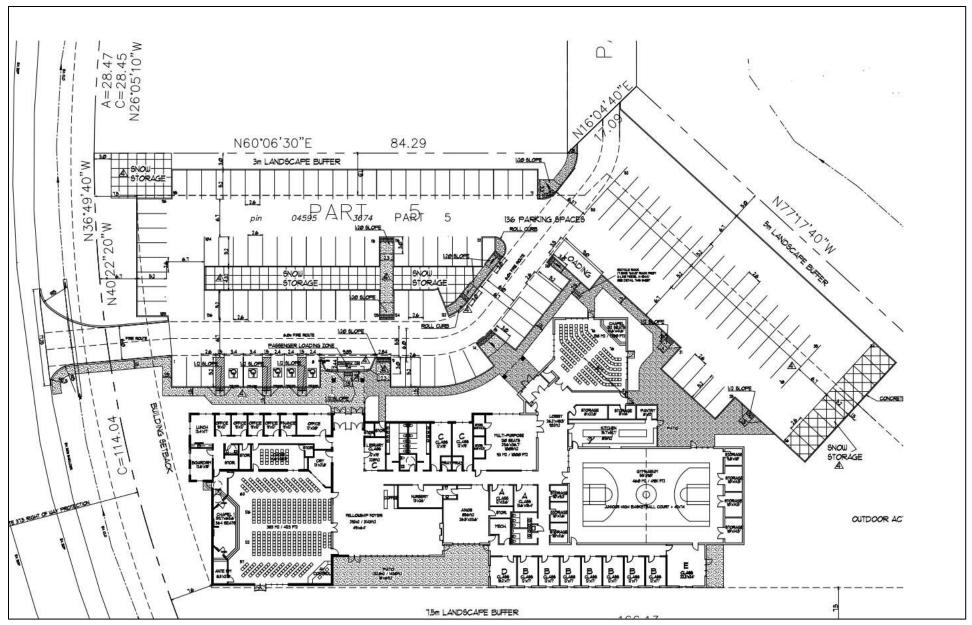


Exhibit 2-2: Site Plan (January 2024)

2.1.2 Existing Conditions

2.1.2.1 Study Area Roadways

The City of Ottawa's Transportation Master Plan (2013 TMP), a desktop review of aerial photography, the GeoOttawa web site and available TIA's / community plans of adjacent sites, the City of Ottawa Official Plan Schedule C16 and related documents were referenced to document the existing roadways that would serve the proposed church site and the surrounding area.

The municipal-owned roadways in the vicinity of the proposed development include the following:

Borrisokane Road

- is an existing 2-lane arterial roadway;
- posted speed limit of 80 km/hr;
- has a 37.5m ROW protection between Strandherd Drive and Cambrian Road (as per Schedule C16);

Cambrian Road

- is an existing 2-lane arterial roadway;
- posted speed limit of 70 km/hr;
- has a 37.5m ROW protection between Borrisokane Road and Longfields Drive (as per Schedule C16);

Flagstaff Drive

- is an existing 2-lane collector roadway;
- unposted speed limit of 50 km/hr;
- has a 24m ROW (as per GeoOttawa measurement);

2.1.2.2 Study Area Intersections

1. Borrisokane Road and Flagstaff Drive

This existing intersection is a 3-leg traffic minor east leg STOP-controlled intersection.

- All approaches currently provide for a single lane of shared traffic;
- There are currently no sidewalks or bicycle provisions along either corridor.



Exhibit 2-3: Borrisokane Road and Flagstaff
Drive Intersection



Exhibit 2-4: Borrisokane Road and Cambrian Road Intersection

The following adjacent existing accesses/driveways were identified along the three boundary streets surrounding the development (Borrisokane Road, Cambrian Road and Flagstaff Drive):

Borrisokane Road Accesses:

- Borrisokane Road has 2 existing farmland acreage accesses:
- One access is located directly west of the Borrisokane Road / Flagstaff Drive intersection; and
- The other is across the street from the southern limit of the development.

2. Borrisokane Road and Cambrian Road

This existing intersection is a 3-leg intersection. The westbound approach from Cambrian Road is currently STOP-controlled.

- All approaches provide for a single allmovement shared lane;
- There are no sidewalks or bicycle provisions along either corridor.

2.1.2.3 Existing Surrounding Driveways

Exhibit 2-5 illustrates the adjacent existing driveways within the immediate proximity of the site. (Within 200 meters from the proposed 3555 Borrisokane Road development access along each boundary street).

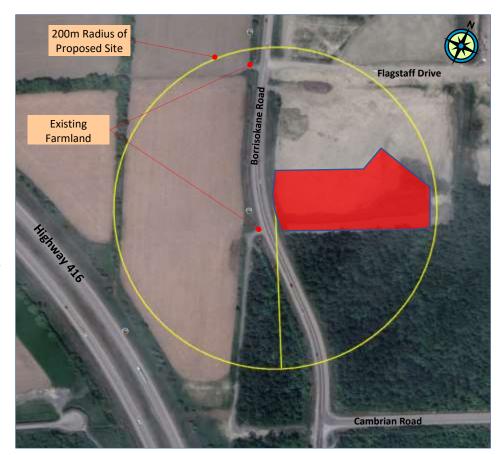


Exhibit 2-5: Existing Accesses & Driveways within 200m Radius of Proposed Site

2.1.2.4 Pedestrian Facilities

No existing pedestrian facilities exist within the immediate proximity of the development.

2.1.2.5 Cycling Facilities

No existing cycling facilities exist within the immediate proximity of the development.

2.1.2.6 Area Traffic Management

No area traffic management strategies were found to be in place within the immediate proximity of the development.

2.1.2.7 Existing Transit Provisions

There are no existing transit operational service along roadways within the immediate proximity of the proposed development. (As of December 2022.) The Half Moon Bay subdivision to the east has several routes, which have the potential to be extended towards the development as the community continues to grow. These routes include 671 and 675, which are special high school and middle school buses, and route 75 from Barrhaven Centre to Tunney's Pasture.

2.1.2.8 Existing Peak Hour Travel Demand by Mode

Pedestrian and Cyclist Travel Demand

There were no recorded current pedestrian or cyclist traffic volumes recorded at the time of the traffic count provided by the City of Ottawa (November 9, 2018).

Vehicular Travel Demand

Exhibit 2-6 illustrates the existing (unbalanced) morning and afternoon peak hour traffic volumes within the study area as referenced from a recent traffic impact study². The referenced traffic count information for the Borrisokane Road/Cambrian Road intersection was derived from a City of Ottawa traffic count undertaken on November 9, 2018. The study also provided forecast 2018 traffic for the Borrisokane Road/Flagstaff intersection as illustrated in the exhibit.

Exhibit 2-7 illustrates the existing balanced morning and afternoon peak hour traffic volumes. The following steps were completed in order to bring the traffic to a balanced 2023 horizon year:

- Traffic volumes were balanced along Borrisokane Road; and
- A 2% annual growth rate was applied to traffic along Borrisokane Road to update the counts to a 2022 horizon year.

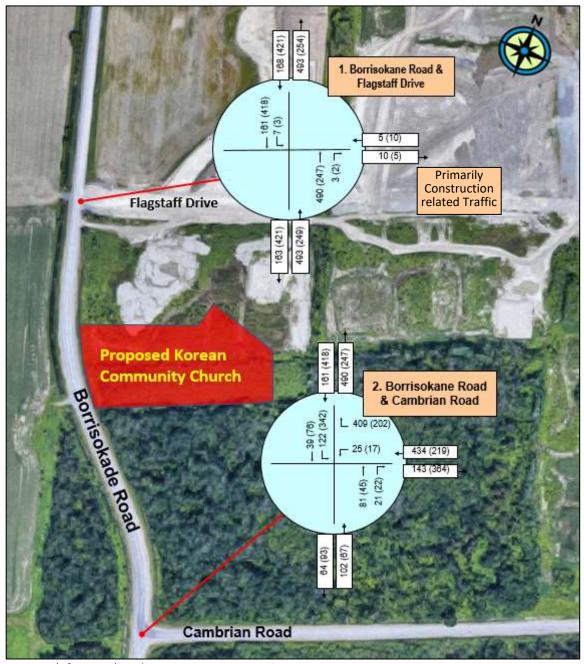
Existing Traffic Volumes Intersection Capacity Analysis

Table 2-1 summarizes the existing (2022) intersection capacity analysis undertaken with SynchroTM 10 traffic software assuming STOP controlled intersections. City of Ottawa's MMLOS Guidelines were consulted to determine appropriate level of service metrics for each intersection. The levels

^{2. &}quot;Halo Car Wash, 3555 Borrisokane Road, TIA Strategy Report (Revised)" Sept. 23, 2022 D. J. Halpenny & Assoc Ltd.

of service are based on the volume-to-capacity (v/c) ratios. The City of Ottawa's target level of service for motor vehicle traffic for "All Other Designations" is LOS "D"³.

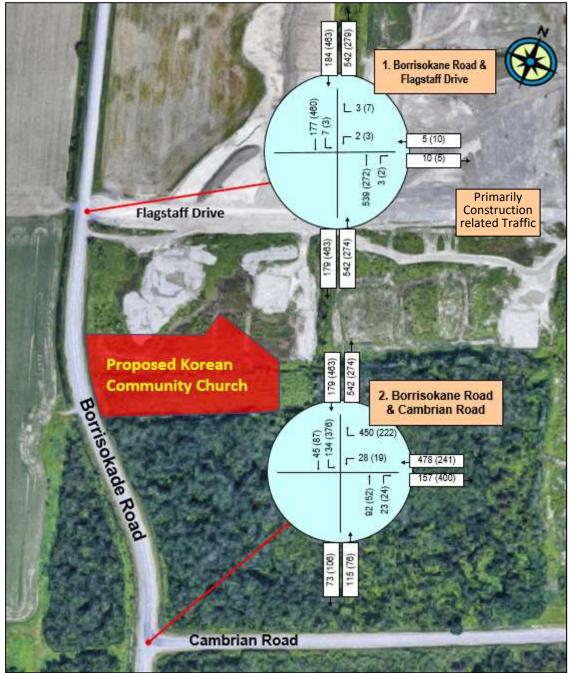
This analysis assumes existing conditions (without the development in place) and only considers the effects of background growth to a 2023 horizon year. The Synchro analysis output sheets can be found in Appendix "F". [The digital Synchro files were forwarded along with this report for review.]



Morning (Afternoon) Peak Hour

Exhibit 2-6: 2018 Non-Balanced Morning and Afternoon Peak Hour Traffic Volumes

^{3. &}quot;City of Ottawa MMLOS Guidelines, Draft Report", September 2015. Pages 21-24



Morning (Afternoon) Peak Hour

Exhibit 2-7: (2022) Balanced Morning and Afternoon Peak Hour Traffic Volumes

			Weekday	Morning Peak Ho	ur (Afternoon	Peak Ho	our)
Borris	sokane Road Intersection	Control Type		Critical M	ovement		
with		Comiron Type	Approach / Movement	95 th Percentile Queue (m)	Delay (seconds)	LOS	v/c 0.01 (0.02) 0.01 (0.02) 0.57 (0.35)
1	Flogstoff Drive*	Minor Leg-STOP	WB (WB)	0 (0.75)	13.2 (11.4)	B (B)	
1.	Flagstaff Drive*	control	SB-LT (SB-LT)	0 (0)	8.6 (7.8)	A (A)	
2.	2. Cambrian Road Minor Leg-STOP	Minor Leg-STOP	WB (WB)	27.75 (11.25)	14.3 (12.6)	B (B)	
2.	Cambridii Kodu	control	SB-LT (SB-LT)	2.25 (8.25)	7.8 (8.4)	A (A)	0.01 (0.27)

Table 2-1: Existing (2022) Traffic Operational Analysis Results

Table 2-1 indicates that the above intersections presently operate at LOS "B" or higher in their current configuration and STOP controlled operation.

2.1.2.9 Existing Road Safety Information

The City of Ottawa's five-year (2016-through-2020) historical collision information was reviewed for each of the study area intersections and roadway segments. The collision information provided included:

- the date and time of each collision;
- the type of collision (e.g., angle collision, rear-end); and
- vehicle details (truck, passenger vehicle, etc.);

Table 2-2 provides:

- a summary of the recorded intersection and mid-block collisions for the locations within the study area in terms of the type of collision and collision severity.
- the calculated collision rate [as measured in number of collisions-per-million-vehicles that travelled either through the intersection or along the corridor.] A rate greater than 1.0 collisions/MEV was considered to indicate a potential area of concern.

Table 2-2: Five-Year Collision History (January 1st 2016 – December 31st 2020)

Intersection / Mid-block Segment		Cambrian Road and Borrisokane Road Intersection	Borrisokane Road between Cambrian Road and Jock River	Cambrian Road between Borrisokane Road and Apolune St.
Tota	al Collisions	12	13	1
	Rear End	6	1	-
	Single Vehicle	-	-	-
Collision	Sideswipe	-	1	-
	Turning Movement	1	-	-
Type	Angle	1	-	-
	Pedestrian	-	1	-
	Other	4	12	1
Average Annual Daily Traffic		14,000	11,400	5,800
Collision Rate [per-Millions of- Entering-Vehicles] (MEV)]		0.46	0.63	0.1

^{*} Source: AADT estimated from Borrisokane Road/Cambrian Road City of Ottawa Traffic Count, November 9, 2018 with Peak-Hr-to-AADT conversion factor of 10.

^{*} Construction was taking place along Flagstaff Road at the time of this study.

Table 2-2 indicates the following:

- The Cambrian Road and Borrisokane Road 3-leg intersection was determined to have 12 collisions over a five-year period and, due to its low AADT, exhibited an overall collision rate of 0.46 collisions/MEV, which is above the acceptable range. As well, approximately 50% of the collisions were rear-end collisions:
- The Mid-block on Borrisokane Road [between Cambrian Road and the Jock River] experienced 13 collisions.
- The Mid-block on Cambrian Road [between Borrisokane Road and Apolune Street] experienced a single collision.

Conclusions:

- The mid-block along Borrisokane Road [between Cambrian Road and the Jock River] and the
 midblock along Cambrian Road [between Borrisokane Road and Apolune Street] exhibited
 acceptable levels of safety; and
- The Cambrian Road / Borrisokane Road intersection exhibits a rate of 0.46 which is less than 1.0 collisions/MEV. This was therefore considered to have an acceptable levels of safety.

2.1.3 Planned Conditions

The following documents and sources of information were referenced to document the planned conditions of roadways that would serve the proposed church site and the surrounding area. These include:

- City of Ottawa's Transportation Master Plan (2013 TMP);
- a desktop review of aerial photography;
- the GeoOttawa web site;
- available TIA's of adjacent sites;
- "Barrhaven South Urban Expansion Study Area Community Design Plan Transportation Master Study" (2018); and
- Mattamy Homes Half Moon Bay West Subdivision draft plan.

2.1.3.1 Changes to the Study Area Transportation Network

The development is within the Barrhaven South Community Design Plan (CDP) Area. This plan call for a protected 37.5m wide right-of-way to accommodate a future expansion to four-lane arterials on the following roadways:

- Borrisokane Road north of Cambrian Road; and
- Cambrian Road between Borrisokane Road and Jockvale Road.

The "Barrhaven South Urban Expansion Study" also indicated the need for the installation of traffic signals with left turn auxiliary lanes at the intersection of Borrisokane Road and Cambrian Road by 2031. The City of Ottawa also noted that the intersection of Borrisokane Road and Flagstaff Drive is expected to be upgraded to include a southbound left turn lane and associated widening in 2023 at, complete with a gored area south of the intersection, and sidewalk/separated cycle track on the northeast and southeast corners.

2.1.3.2 Other Study Area Developments

A review of the City of Ottawa's development applications website indicated 2 major development proposals as "active applications" adjacent to the study area that are anticipated to have an impact on the traffic volumes in the study area's transportation network.

Exhibit 2-8 illustrates the location of the adjacent development initiatives:

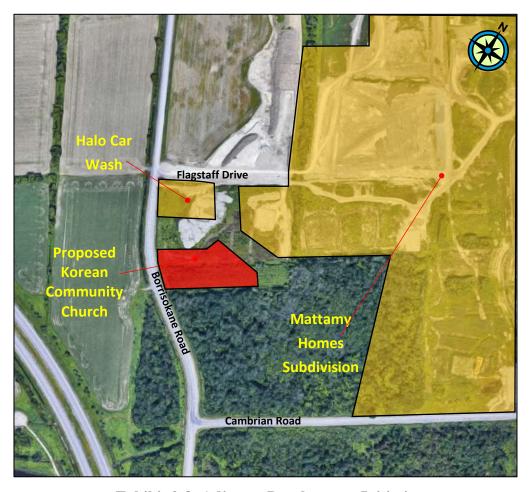


Exhibit 2-8: Adjacent Development Initiatives

a. "Mattamy Homes: Half Moon Bay West" 3345 Borrisokane Road, 3640 Greenbank Road, 3853 & 3900 Cambrian Road

This is a proposal for a planned subdivision which will include 973 residential units, consisting of 447 detached dwellings, 300 townhouse dwellings, 154 back-to-back townhouse dwellings, and 72 apartment units.

Additionally, the subject properties will contain a school (6.07 hectares), 4.58 hectares of parkland, 4.07 hectares reserved for a stormwater management (two blocks) facility, a 2.11 hectare employment block, and several new streets and pathways.

Upon completion of the subdivision, Flagstaff Drive will be urbanized to include sidewalks.

b. "Halo Car Wash" 3555 Borrisokane Road

The Halo Car Wash facility is a proposal for an automated car wash that would accommodate a single storey $476~\text{m}^2~(5,125~\text{ft}^2]$ building on-site and 21 parking stalls. It is expected to be completed by the end of 2023.

2.2 STUDY AREA AND TIME PERIODS

2.2.1 Study Area

The study area around the proposed development includes the following boundary streets:

• Borrisokane Road;

Cambrian Road; and

• Flagstaff Drive.

A total of two intersections along the above boundary streets reside within the study area:

- Borrisokane Road/Flagstaff Drive;
- Borrisokane Road/Cambrian Road

2.2.2 Time Periods

In accordance with TIA Guidelines, the forecasting report will conduct its operational analysis of the weekday morning and afternoon peak hours of travel demand.

The Client has indicated that there are currently no formal plans in place to support a daycare facility. However, this study conservatively assumes that a daycare facility with a capacity for 30 children would be in place contributing to weekday morning drop-offs and afternoon pick-ups.

As regards weekend traffic, City staff responded on May 19th, 2023 that "since the main driver of trips is Sunday and Flagstaff is not yet constructed, no analysis of Sunday traffic will be required."

2.2.3 Horizon Years

The proposed development, at this point in time, is anticipated to be achieved by the Fall of 2024. The analysis to be undertaken for the forecasting report also include a period anticipated to be five years beyond buildout, which would be 2029.

2.3 EXEMPTION REQUEST

Table 2-3 reflects exemptions/reductions in scope of work that were requested subsequent to the submission of the Screening Report. These exemption requests are normally addressed within the Design Review and Network Impact components of the TIA.

Table 2-3: Requested Exemptions as per TIA Guidelines

Module	Module Element Exemption Considerations		Include Module in TIA
	Design	Review Component	
4.1 Development Design	4.1.3 New Street Networks	Only required for plans of subdivision	No
4.2 Parking	4.2.2 Spillover Parking	The 136 parking stalls being supplied exceeds the City of Ottawa Zoning By-Law requirement of 55 stalls.	No
	Networ	k Impact Component	
4.5 through 4.9 All Elements		The development is not expected to generate more than 60 vehicle-trips during the peak hours of travel demand. Therefore, the "Network Impact" component of the TIA is not required	No

3.0 FORECASTING REPORT

3.1 DEVELOPMENT GENERATED TRAVEL DEMAND

This section of the report describes the projected traffic generation by mode, as well as vehicle trip distribution and trip assignment associated with the full build-out and occupancy of the proposed development planned for 2024.

3.1.1 Trip Generation and Mode Shares

3.1.1.1 Trip Generation Rate and Split

The City of Ottawa TIA guidelines indicate that the source for forecasting traffic generation for employment generators is the ITE Trip Generation Manual, 11th Edition⁴.

- The proposed expansion is classified as a "Institutional (500-599)"
- The Church component is analyzed as ITE Land Use "560 Church"
- An optional daycare maybe in operation in the future, with a maximum capacity of 30 students, analyzed as ITE Land Use "565 Daycare Center"

Table 3-1 outlines the average number of trip-ends as referenced from the ITE Trip Generation Manual, 11th Edition for the proposed Korean Church and the potential day care facility

Morning Peak Hour | Afternoon Peak Hour ITE Land Use Variable **Exit** Entry **Exit** Entry 31,360 Sq. Ft. GFA 7 8 560 - Church 6 4 565 - Day Care Center 30 Students 12 11 11 13 18 15 18 21 Total

Table 3-1: Vehicle Trip Generation per Peak Period (ITE Trip Generation Manual, 11th Edition)

Table 3-1 outlines the inbound/outbound directional split characteristics that were applied to forecast the number of inbound and outbound vehicle trips. The development is forecast to generate:

33

- 18 inbound and 15 outbound trips during the morning peak hour; and
- 18 inbound and 21 outbound trips during the afternoon peak hour.

3.1.1.2 Mode Shares

Table 3-2 outlines the mode shares for an employment generator within the South Nepean district, as referenced from the 2020 TRANS Trip Generation Manual⁵. Other than school land uses the document does not provide similar information for the intended place-of-worship/daycare land use.

In lue of not having other reliable information available Table 3-3 presents the resulting peak hour person trips for the proposed church that were derived from applying the values in Table 3-2.

^{4. &}quot;TIA Guidelines", City of Ottawa, Page 27

^{5. &}quot;TRANS Trip Generation Manual" Summary Report, Page 14, Table 12: Employment Generator Mode Share by District

Table 3-2: Mode Shares Employment Generator, South Nepean (TRANS 2020)

Peak Period Mode Share Split (TRANS 2020 Table 12)				
Mode	Mode Share, AM	Mode Share, PM*		
Auto Driver	80%	80%		
Auto Passenger	10%	10%		
Transit	5%	5%		
Cycling	1%	1%		
Walking	4%	4%		

^{*} Employees are likely using the same mode of transportation when leaving work, it is fair to equivocate the PM peak period employment generator mode with the AM peak period.

Table 3-3: Peak Hour Person Trips* by Mode

Mode	Morning	Afternoon
Auto Driver	33	39
Auto Passenger	4	5
Transit	2	2
Cycling	1	1
Walking	2	2
Total Person Trips	42	49

^{*} Values Rounded Up

3.1.1.3 Future Mode Shares

Table 3-4 outlines the future mode share targets for this development, along with justifications for each target. The <u>existing</u> mode shares indicated in Table 3-2 were used for trip forecasting.

Table 3-4: Future (2029) Assumed Weekday Mode Share Targets for Community Church

Travel Mode	Target Mode Share	Rationale
Auto-driver	80%	Borrisokane Road is expected to be widened to 4 lanes based on the Barrhaven South Community Design Plan (CDP), but since the development horizon year is 2024, the auto-driver mode share is anticipated to remain the same.
Auto Passenger 10% Auto passenger mode share is expected to remain the same.		Auto passenger mode share is expected to remain the same.
Transit	5%	There are no nearby transit facilities, with no plans currently in place for any such provisions. Transit mode share should be decreased.
Cycling	2%	No current pedestrian or cycling facilities are present, however the intersection of
Walking 3%		Borriskane / Flagstaff is expected to be upgraded with a sidewalk/separated cycle track on the northeast and southeast corners. Flagstaff Drive is to be urbanized, and will also have sidewalks. An increase to walking and/or cycling mode shares are expected.

3.1.2 Trip Distribution

A review of the 2022 traffic flows along the roadways near the proposed site was used as the basis for the following site generated traffic distribution trends being adopted:

 Table 3-5
 Traffic Distribution Assumptions

	Origin and Destination of Traffic	2024	2029
•	Traffic coming to/from Borrisokane Road north of Flagstaff Drive;	50 percent	40 percent
•	Traffic coming to/from the Borrisokane Road south of Cambrian Road; and	20 percent	20 percent
•	Traffic coming to/from the East along Cambrian Road.	30 percent	20 percent
•	Traffic coming to/from the east along Flagstaff Drive	0 Percent	20 percent

3.1.3 Trip Assignment

Exhibit 3-1 displays site-generated vehicle trips upon full occupanc"y of the development (in 2024). For the purpose of this TIA, it was assumed that by 2024 the Halo Car Wash development and the corresponding access to Flagstaff Drive would NOT be in place. All site traffic would use the Borrisokane Road access, however by 2029, it is assumed that the access to Flagstaff Drive would exist.



Exhibit 3-1: Site Generated Traffic 2024

Exhibit 3-2 shows site-generated vehicle trips for the 2029 horizon year, assuming the Flagstaff Drive access is in place as a result of the development of the Halo Car Wash to the north.

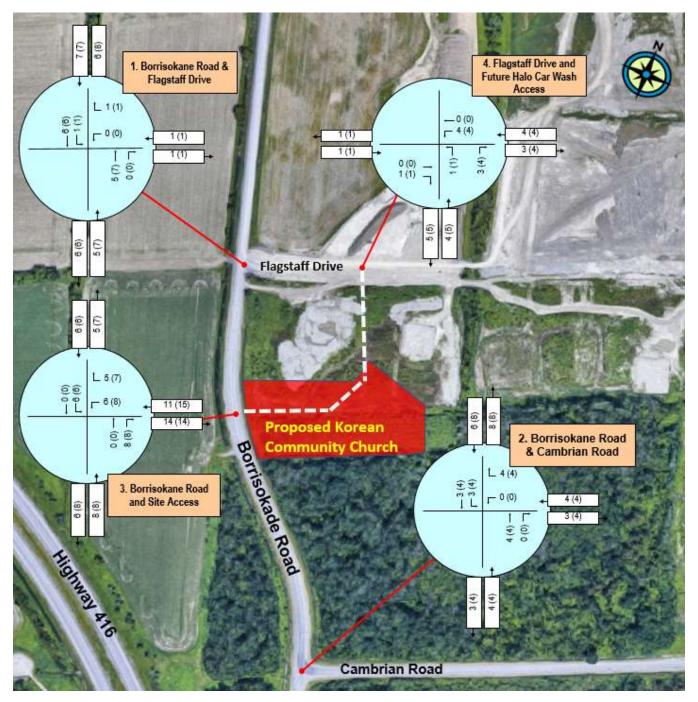


Exhibit 3-2: Site Generated Traffic 2029

3.2 BACKGROUND NETWORK TRAVEL DEMAND

3.2.1 Transportation Network Plans

Section 2.1.3.1 indicated the following changes are anticipated to occur to the transportation network:

- As part of the "Barrhaven South Community Development Plan"⁶, the following two roads are anticipated to be widened to accommodate a four-lane cross-section:
 - Borrisokane Road north of Cambrian Road (A 37.5m right-of-way was noted at the pre-consultation meeting.); and
 - Cambrian Road between Borrisokane Road and Jockvale Road
- As part of the "Barrhaven South Urban Expansion Study", the installation of traffic signals with left turn auxiliary lanes at the Borrisokane Road / Cambrian Road intersection is expected by 2031.

3.2.2 Background Growth

The traffic growth from the proposed subdivision developments in the area would account for the majority of growth in the area. Any additional growth would be attributed to further development beyond the study area.

This study has utilized a 2 percent average annual growth that was applied to the 2018 traffic counts to produce estimates for the base year (2022), the year of occupancy of the Church (2024), and a period 5 years after occupancy (2029). Over the 11-year period between 2018 and 2029 the growth was assumed to be 22% along the corridors surround the development.

All of the traffic along Flagstaff Drive would likely originate from the proposed subdivision developments east of the site.

3.2.3 Other Developments

There are two developments in the study area with active development applications and completed TIA studies (previously summarized in Section 2.1.3.2). The traffic generation and distribution for these developments is summarized below.

- 3555 Borrisokane Road "Halo Car Wash" and
- Mattamy Homes: Half Moon Bay West 3345 Borrisokane Road, 3640 Greenbank Road, 3853 & 3900 Cambrian Road

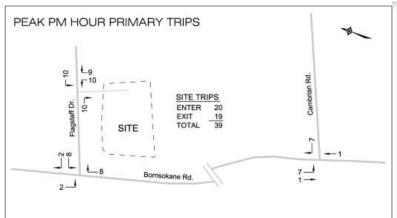
A previous TIA for 3555 Borrisokane Road "Halo Car Wash" was prepared by D. J. Halpenny & Associates Ltd. utilized the trips generated at the completion of the "Half Moon Bay West" development for its forecasted background traffic.

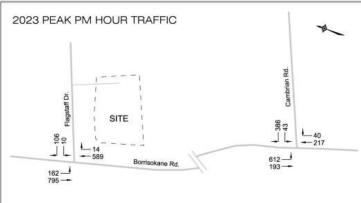
Exhibit 3-2 (adopted from the Halpenny TIA) illustrates the afternoon peak hour vehicle traffic generated by both of the above developments assuming full occupancy. It was assumed for the purposes of this study that the morning peak hour volumes would be inversely proportional to the afternoon peak hour volumes

^{6. &}quot;Barrhaven South Urban Expansion Area Community Design Plan (CDP)" - May 2018

^{7. &}quot;Barrhaven South Urban Expansion Study Area Community Design Plan Transportation Master Study" (2018)

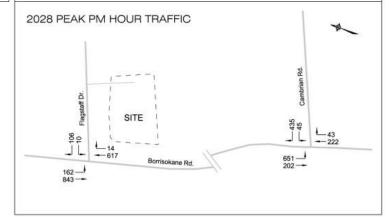
with the vast majority of traffic leaving the residential community in the morning and returning in the afternoon.





Halo Car Wash Site Generated Traffic

Exhibit 3-3: Other Development Traffic



Mattamy Homes Subdivision and Background Traffic

3.3 DEMAND RATIONALIZATION

The following section contains forecast traffic volumes for the forecast time of build-out (2024) and 5-years after build-out (2029), both with, and without, the proposed Korean Community Church development.

3.3.1 Background Traffic Forecasts - Without Korean Church Development

Exhibit 3-4 and Exhibit 3-5 illustrate background traffic forecasts for the forecast years 2024 and 2029, which combine the existing traffic network and the traffic generated by future developments in the study area.

These exhibits present forecast traffic without the Korean Community Church in place, for comparison purposes.

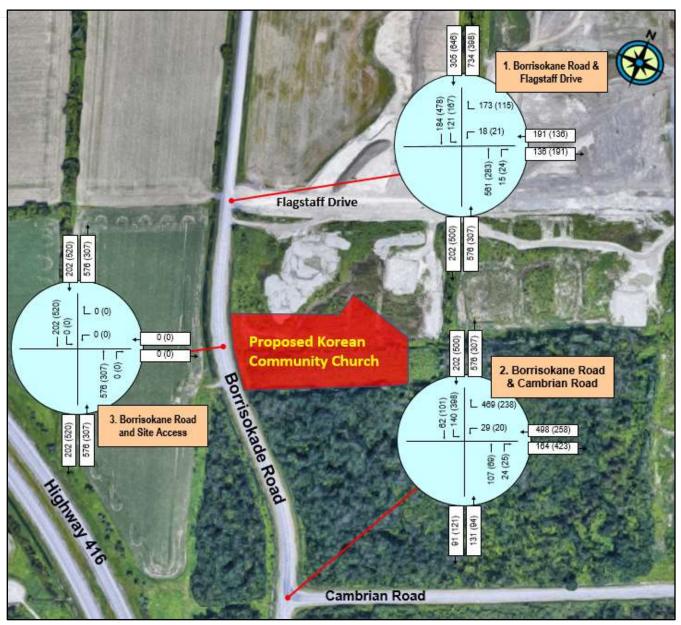


Exhibit 3-4: Forecast 2024 Background Morning and Afternoon Peak Hour Vehicle Traffic Volumes - Without Church Development

Morning (Afternoon) Peak Hour



Exhibit 3-5: Forecast 2029
Background Morning and
Afternoon Peak Hour Vehicle
Traffic Volumes - Without
Church Development

3.3.2 Total Traffic Forecasts – With Korean Church Development

Exhibit 3-6 and Exhibit 3-7 illustrate the total traffic forecasts for the 2024 and 2029 horizon years. The forecasts were developed by combining the forecast background network traffic and Korean Church development-generated traffic demands.

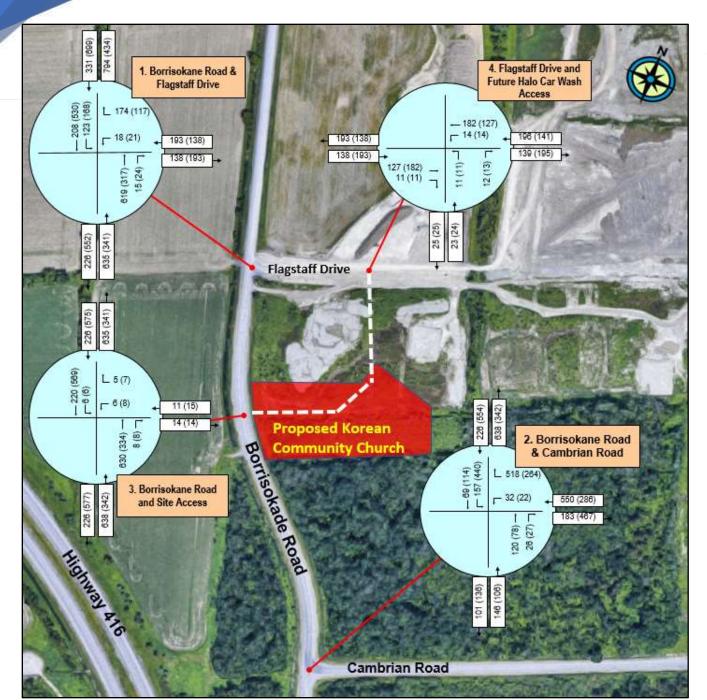
Development-Generated Traffic Impacts

The advent of development is expected to cause minor impact on the surrounding transportation network, with a total addition of:

- 33 two-way auto-vehicle trips (18 inbound and 15 outbound) during the morning peak hour; and
- 39 two-way auto-vehicle trips (18 inbound and 21 outbound) during the afternoon peak hour.



Morning (Afternoon) Peak Hour



Transportation Impact Assessment Forecasting and Strategy Report

Exhibit 3-7: Total 2029 Forecast Morning and Afternoon Peak Hour Vehicle Traffic Volumes - With Church Development

Morning (Afternoon) Peak Hour

4.0 STRATEGY REPORT

4.1 DEVELOPMENT DESIGN

4.1.1 Design for Sustainable Modes

Travel Demand Management (TDM): The City of Ottawa's "TDM Supportive Development Design and Infrastructure" checklist was reviewed as part of this strategy report and it was determined that proposed development meets all of the required TDM infrastructure measures (See Appendix "F").

Pedestrian/Cycling Infrastructure: The proposed site plan provides convenient and safe facilities such as sidewalks/paths that accommodate both pedestrian and cycling modes that are consistent with the requirements under the City's Accessibility Design Standards (2015).

Parking: All parking requirements for motor-vehicles and bicycles were found to be satisfied. (See Sections 4.1.3)

Other TDM Measures: The following TDM measures were considered in the evaluation process:

- *Pick-up & Drop-off Area*. The architect is encouraged to consider the viability of incorporating a pick-up & drop-off area fronting the building entrance to accommodate deliveries, and pick-up/drop offs of employees and visitors. This would be particularly useful in the case of the potential day-care centre land use should it materialize;
- Carpool/Rideshare: This TDM measure is useful for high employment land uses where preferential parking locations are reserved for those employees who actively and consistently share vehicle rides to/from a site. However, in the case of a church/daycare land this measure is thought to not be useful, nor applicable. Visitors to a church already by tradition ride share in that young families and spouses travel together to attend services. During the week the need for carpooling/ride sharing is not thought to be critical as parking demand is expected to be low, and motorist have a plentiful supply of nearby stalls that would be utilized;
- *Bicycle Facilities*: Secure bicycle parking and repair station. The mode share for cyclist is expected to be low, as such a simple bike rack would suffice. Other services such as bicycle repair stations would be inconsistent with the proposed land use;
- On-Site Amenities: This generally refers to much larger employment generators that offer convenience amenities that accommodate activities such as going to lunch, exercising, games rooms etc. that can take place without having to leave the site. In general, churches cater to services and special events for their congregants that include kitchen's, study halls, reception areas, wedding halls and sanctuaries that represent common amenities; and
- *Transit:* Currently, there are no transit facilities that exist within the vicinity of the proposed site. OC Transpo is encouraged to coordinate future service to the community as whole that would provide extended convenient transit services to the proposed church.

Active Transportation Infrastructure: A review of nearby active transportation infrastructure indicates that a sidewalk has recently been constructed along the south side of Flagstaff Drive extending from the residential community on the east to Borrisokane Road. In addition, City staff have confirmed that future plan are being prepared that would include sidewalks and separated cycle tracks on the NE and SE corners of the Borrisokane Road/Flagstaff Drive intersection. As well, the proposed Halo Car Wash

development provides for a sidewalk on the east side of the connecting private driveway that would provide good pedestrian connectivity to the church and cycling activity along Borrisokane Road.

4.1.2 Circulation and Access

The proposed development will have access onto Borrisokane Road, and eventually Flagstaff Drive through the Halo Car Wash Development. The Borrisokane access road fronting the Church is designed with a width of 6.7m. The north-south accessway connecting to Flagstaff Road will have a width of 7.5 m.

4.1.2.1 Turning Movement Analysis

Turning movement analysis was completed at both accesses to the property. Analysis was done using the following design vehicles:

- Fire Truck;
- Garbage Truck; and
- Delivery Vehicle (Medium Single Unit Truck or MSU).

The access roadway into the proposed church from Borrisokane Road is 6m wide. The width of the access at the road edge is 17.25 meters which exceeds the City of Ottawa's by-law requirement for a maximum of 9, however this was necessary to accommodate the narrow roadway width along Borrisokane Road and avoid heavy vehicles leaving the site conflicting with the southbound traffic stream. The access provides for a throat length of 15 meters. The access onto Flagstaff has been designed as per the Halo Car Wash Landscape plan, completed in April 2022.

Appendix "J" contains the turning movement diagrams for all 3 design vehicles at both accesses. While the MSU's can complete all the turns without encroaching onto opposing lanes, the garbage truck and fire truck require to cross the centre line. As garbage truck collection occurs only once a week, and in the case of an emergency all other vehicles give way to fire trucks, these turning movements are still deemed acceptable.

4.1.3 Motor Vehicle Parking

Table 4-1 summarizes the supply of parking required by the City of Ottawa's by-laws compared to the planned parking provisions associated with the proposed development.

• *Place of Worship Parking:* The City of Ottawa's minimum parking requirements⁸ indicate that 10 parking spaces-per-100 m² of gross floor area is required for a place of worship. The assembly area sums to 541 m², and therefore 54 vehicle parking stalls would be required.

ggg				
Land Use	Development Size	City Parking Requirement Rate	City Parking Requirement	Parking Provisions
Place of Worship	541.0 m2	10 per 100 m ²	54 stalls	
Daycare	106.6 m2	2 per 100 m ²	3 stalls	
	Total		57 stalls	136 stalls

Table 4-1: Auto Parking Provisions Summary

^{8. &}quot;City of Ottawa By-Law 2016-249", Table 101, Row N59, area D on Schedule A1 3555 Borrisokane Road – Korean Community Church

• Daycare Parking: The advent of a potential daycare requires 2 parking spaces-per-100 m² of gross floor area. The daycare sums to 106.6 m^2 (4 classrooms: $12 \times 17 * 3 = 612\text{SF} \& 22.3 \times 24 = 535.2\text{SF}$, for a total of 1147.2SF or 106.6 m^2), and therefore 3 vehicle parking stalls would be required.

Conservatively assuming both of these land uses to be in place, the Zoning bylaw requirements for the site when combined require 57 motor-vehicle parking stalls. The current site plan would provide for 136 parking stalls.

In summary, Table 4-1 indicates that the planned Korean Church development's supply of parking (136 stalls) fully satisfies the City of Ottawa's parking by-law requirements.

4.1.4 Bicycle Parking

Table 4-2 summarizes the bicycle parking stall requirement of the City of Ottawa's By-law.

The City of Ottawa's bicycle parking By-law⁹ indicates that 1 bicycle stall is required-per-1,500 m² of gross floor area for all other non-residential land uses.

Table 4-2: Bicycle Parking Provisions Summary

Land Use	City Requirement	Parking Provisions
All other non- residential uses	2 stalls	7 stalls
Total	2 stalls	2 234113

The gross church area of 2,913.5 m² would

require a total of two bicycle stalls. A small outdoor bike rack with space for 7 bicycles will be provided on-site. The bike rack is located at the front of the building by the eastern entrance. (See Appendix "C" for the full site plan.)

4.2 BOUNDARY STREET DESIGN

The City of Ottawa's Multi-Modal Level of Service (MMLOS) guidelines¹⁰ along with the associated addendum¹¹ document was used to evaluate the multi-modal operational characteristics of the individual roadway segments in the vicinity of the proposed church site.

Exhibit 4-1 illustrates the location of the following three boundary street segments near the proposed Korean Church development analyzed for MMLOS:

- Cambrian Road east of Borrisokane Road
- Flagstaff Drive east of Borrisokane Road
- Borrisokane Road between Cambrain and Flagstaff Drive

The City of Ottawa's Multi-Modal Level of Service Guidelines outline the following level of service measures for various non-automotive transportation modes in the city based on existing conditions:

- Pedestrian Level of Service (PLOS);
- Transit Level of Service (TLOS); and
- Bicycle Level of Service (BLOS);
- Truck Level of Service (TkLOS).

^{9. &}quot;City of Ottawa By-Law 2016-249", Section 111, Table 11A, (b) and (g)

^{10. &}quot;Multi-Modal Level of Service (MMLOS) Guidelines", IBI Group, September 2015

^{11. &}quot;Document 5: Addendum to the City's Multi-Modal Level of Service Guidelines", December 2016



Exhibit 4-1: Boundary Street Segments for MMLOS Analysis

Table 4-3 provides the results of a segment MMLOS analysis in each direction of travel. The worst-scoring direction is used as an overall segment score.

Table 4-3: Segment MMLOS Analysis Results

Location				Level of Service and Targets							
Roadway Segment		Policy Area/ Land Use Designation	PLOS	Target PLOS	BLOS	Target BLOS	TLOS	Target TLOS	TkLOS	Target TkLOS	
Cambrian Road east	WB		F	D	F	D	N/A	N/A	N/A	N/A	
of Borrisokane Road	EB		F	D	F	D	N/A	N/A	N/A	N/A	
Flagstaff Drive east	WB	"All other	F	D	E	D	N/A	N/A	N/A	N/A	
of Borrisokane Road	EB	Designations"	F	D	E	D	N/A	N/A	N/A	N/A	
Borrisokane Road between Cambrain and Flagstaff Drive	NB	Designations	F	D	F	D	N/A	N/A	N/A	N/A	
	SB		F	D	F	D	N/A	N/A	N/A	N/A	

Note – Levels of Service highlighted in bold font fail to meet the respective target LOS

NA – not applicable: The roadways do not have transit service and are not a City assigned truck route.

[&]quot;All other Designations" is a catch-all category referred to within the MMLOS Guidelines under Section 7.1 – *Modal Targets by Official Plan Designation/Policy Area*.

The table indicates the following:

All segments fail to meet their PLOS target due to the absence of

Pedestrian Level of Service (PLOS): existing sidewalks and operating speeds on the roadways

exceeding 60km/h.

All segments fail to meets their BLOS target due to the absence of *Bicycle Level of Service (BLOS):*

cycling facilities and the operating speed of the roadways

exceeding 70km/h.

Transit Level of Service (TLOS): None of the study area segments have any existing transit service.

Truck Level of Service (TkLOS): There were no target TkLOS in the study area.

Detailed segment MMLOS analysis calculations are provided within Appendix "I".

TLOS analysis was not performed on segments without existing transit service.

TkLOS analysis was not performed on segments which are not designated truck routes.

The poor existing levels of service related to pedestrian and cycling modes will likely be addressed through the anticipated ultimate upgrades to the corridors.

4.3 **ACCESS INTERSECTION DESIGN**

The Flagstaff Drive access will form a shared two-lane private roadway with a width of 7.5 m that would be operational only when the Halo Car Wash development to the north is completed. For the purpose of this TIA, it was assumed that the Halo Car Wash development and the private connecting roadway would not be in place by 2024, but will be completed by the 2029 horizon.

4.3.1 Location and Design of Access

The proposed development will have two accesses – one onto Borrisokane Road and the other via a private roadway through the Halo Car Wash onto Flagstaff Drive which was for the purposes of this TIA conservatively assumed to be available during the 2029-time horizon.

4.3.1.1 Access Evaluation

The City of Ottawa Private approach bylaw and TAC Geometric design were used to evaluate the accesses on the proposed development's property. The minimum widths, throat lengths and other access features were determined and applied to the intersection designs. The Flagstaff access was found to comply with City of Ottawa 9m maximum private approach bylaw. The Borrisokane Road access was found to require a larger access width of 17.25m needed to accommodate heavy vehicle truck turning movements that would avoid crossing over into the southbound traffic stream. (See Appendix "J")

A visual evaluation of the private approach onto Borrisokane Road showed potential sight line issues. Sight distance and stopping sight distance analysis for this access was completed to check these concerns. Following TAC guidelines and tables, the time gaps for a single unit truck were found for both left and right turns. It was determined using equation 9.9.1 that for a single unit truck:

- 213 meters are required to make a right turn; and
- 240 meters are required to make a left turn.

Appendix "K" provides a detailed illustration of the intersection sight distances.

3555 Borrisokane Road - Korean Community Church

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4.3.2 Access Control

For the purposes of this TIA document the two intersections along Borrisokane Road were assumed to be initial configured as follows:

- The Borrisokane Road access was assumed to be operational in 2024 and be configured as a 2-lane corridor and operate as "T" intersection with minor leg-STOP control;
- The Flagstaff Dive accesses to the site through the Halo Car Wash lands was assumed to be operational by 2029 and would also be configured as a 2-lane corridor and operate as "T" intersection with minor leg-STOP control.

4.3.3 Access MMLOS Analysis

The City of Ottawa's MMLOS guidelines state that the LOS analysis is applicable to signalized intersections only¹². Since the accesses to the proposed Korean Church were assumed to operate using minor leg-STOP control, no MMLOS analysis (including the auto LOS intersection capacity analysis) is required to be performed for the proposed access to the Korean Church.

4.4 LEVEL OF SERVICE ANALYSIS

4.4.1 Intersection Analysis (2024 Horizon Year)

Table 4-4 and Table 4-5 summarize the intersection capacity analysis results for the 2024 horizon year, both with, and without, the proposed Korean Community Church development.

As indicated in Section 4.3.2 the Flagstaff Drive access was not considered to be operational by 2024.

The analysis was undertaken with Synchro™ 10 traffic software. The Synchro analysis output sheets can be found within Appendix "E".

Table 4-4: 2024 Intersection Capacity Analysis (Without the Korean Community Church)

Borrisokane Road Intersection with			Weekday	Weekday Morning Peak Hour (Afternoon Peak Hour)						
		Control Type		Critical I	Movement	ovement				
		Comrot Type	Approach / Movement	95 th Percentile Queue (m)	Delay (seconds)	LOS	v/c			
1.	Flagstaff Drive	Minor Leg- STOP control	WB	17.25	19.5	С	0.45			
			(WB)	(9.0)	(15.5)	(C)	(0.30)			
			SB-LT	3.75	9.3	Α	0.13			
			(SB-LT)	(3.75)	(8.4)	(A)	(0.14)			
2.	Cambrian Road	Minor Leg- STOP control	WB	31.5	15.4	С	0.61			
			(WB)	(14.25)	(13.6)	(B)	(0.40)			
			SB-LT	3.0	7.9	Α	0.11			
			(SB-LT)	(9.0)	(8.5)	(A)	(0.29)			

^{12. &}quot;Multi-Modal Level of Service (MMLOS) Guidelines", IBI Group, September 2015. Page 4

Table 4-5: Total 2024 Intersection Capacity Analysis (With the Korean Community Church)

Borrisokane Road Intersection with			Weekday Morning Peak Hour (Afternoon Peak Hou					
		Control Type	Critical Movement					
		Control Type	Approach /	95th Percentile	Delay			
			Movement	Queue (m)	(seconds)	LOS	v/c	
			WB	17.25	19.9	С	0.46	
1	Flagstoff Drive	Minor Leg-STOP	(WB)	(9.75)	(15.8)	(C)	(0.30)	
1.	Flagstaff Drive	control	SB-LT	3.75	9.3	Α	0.13	
			(SB-LT)	(3.75)	(8.4)	(A)	(0.14)	
		Minor Leg-STOP	WB	33.0	15.8	С	0.62	
2.	Cambrian Road		(WB)	(15)	(13.9)	(B)	(0.41)	
۷.	Cambrian Road	control	SB-LT	3.0	7.9	Α	0.11	
			(SB-LT)	(9.0)	(8.5)	(A)	(0.30)	
	Korean Church Access		WB	0.75	14.5	В	0.04	
3.		Minor Leg-STOP	(WB)	(1.50)	(13.6)	(B)	(0.05)	
3.		control	SB-LT	0.0	8.8	Α	0.01	
			(SB-LT)	(0.0)	(8.0)	(A)	(0.01)	

The City of Ottawa's MMLOS Guidelines were referenced to determine the appropriate level of service metrics for each intersection. The levels of service are based on the volume-to-capacity (v/c) ratios. The target auto level-of-service for the "All Other Designations" is LOS "D"¹³.

A comparison of the result of Table 4-4 (<u>without</u> the proposed Korean Community Church) to Table 4-5 (with the proposed Korean Community Church) indicate the following:

• All critical intersections turning movements are anticipated to operate at acceptable levels of service "C"-or-better with delays being less than 20 seconds and queues along Borrisokane Road at the proposed access being less than a single car length.

4.4.2 Intersection Analysis (2029 Horizon Year)

Table 4-6 and Table 4-7 summarize the intersection capacity analysis results for the 2029 horizon year both with, and without the proposed Korean Community Church development in place.

Table 4-6: 2029 Intersection Capacity Analysis (Without the Korean Church)

Borrisokane Road Intersection with			Weekday Morning Peak Hour (Afternoon Peak Hour)					
		Control Type		Critical M	Iovement			
		Comroi Type	Approach / Movement	95 th Percentile Queue (m)	Delay (seconds)	LOS	v/c	
1.	. Flagstaff Drive	Minor Leg-STOP control	WB	19.5	21.9	С	0.49	
1.			(WB)	(10.50)	(16.7)	(C)	(0.32)	
			SB-LT	3.75	9.5	Α	0.14	
			(SB-LT)	(3.75)	(8.5)	(A)	(0.15)	
	Cambrian Road	Minor Leg-STOP control	WB	41.25	17.9	С	0.69	
2.			(WB)	(18.75)	(15.4)	(C)	(0.47)	
2.			SB-LT	3.0	7.9	Α	0.12	
			(SB-LT)	(10.5)	(8.7)	(A)	(0.32)	
4.	Flagstaff Drive / Halo Car Wash	Minor Leg-STOP control	NB	0.75	10.0	В	0.03	
			(NB)	(0.75)	(10.1)	(B)	(0.03)	
			WB-LT	0.0	7.5	A	0.01	
			(WB-LT)	(0.0)	(7.7)	(A)	(0.01)	

^{13. &}quot;City of Ottawa MMLOS Guidelines, Draft Report", September 2015. Pages 21-24

Table 4-7: 2029 Intersection Capacity Analysis (With the Korean Community Church)

			Weekda	y Morning Peak Ho	our (Afterno	on Peak H	(our)
	Intersection	Control Type		Critical M	Iovement		
	Intersection	Сошгоі Туре	Approach / Movement	95 th Percentile Queue (m)	Delay (seconds)	LOS	v/c
	Borrisokane Road /	Minor Leg-STOP	WB (WB)	20.25 (10.5)	22.3 (17.0)	C (C)	0.50 (0.33)
1.	Flagstaff Drive	control	SB-LT (SB-LT)	3.75 (3.75)	9.5 (8.5)	A (A)	0.14 (0.15)
2.	Borrisokane Road /	Minor Leg-STOP	WB (WB)	42.75 (19.5)	18.3 (15.8)	C (C)	0.69 (0.48)
	Cambrian Road	control	SB-LT (SB-LT)	3.0 (10.5)	8.0 (8.7)	A (A)	0.12 (0.32)
2	Borrisokane Road / Site	Minor Leg-STOP	WB (WB)	0.75 (0.75)	15.4 (14.3)	C (B)	0.03 (0.04)
3.	Access	control	SB-LT (SB-LT)	0.0 (0.0)	8.9 (8.0)	A (A)	0.01 (0.01)
4.	Flagstaff Drive / Halo	Minor Leg-STOP	NB (NB)	0.75 (0.75)	10.0 (10.1)	B (B)	0.03 (0.04)
4.	Car Wash	control	WB-LT (WB-LT)	0.0 (0.0)	7.5 (7.7)	A (A)	0.01 (0.01)

A comparison of the result of Table 4-6 (without the proposed Korean Community Church) to Table 4-7 (with the Korean Community Church) for the 2029 Horizon year indicates the following:

- All critical intersections turning movements are anticipated to operate at acceptable levels of service "C"-or-better with delays less than 23 seconds; and
- The forecast SB-LT queue turning from Borrisokane Road into the development is anticipated to operate at LOS "A" with a 9 second delay and an average queue length of less than a single car length.
- Similarly, the forecast SB-LT queue at the Borrisokane Road / Flagstaff Drive intersection is anticipated to operate at LOS "A" with an 10 second delay and an average queue length of less than a single car length.

4.4.3 Intersection Analysis Conclusions

The 2024 and 2029 results both indicate that the effect of the proposed Korean Community Church can be considered to be essentially negligible, as the adjacent intersections operates well, with or without, the development in place for both time horizons.

• Based on the results of the Synchro analysis, it was determined that the low peak hour traffic volumes of generated traffic were found not to warrant traffic signals nor additional auxiliary turning lanes at the proposed Korean Church access on Borrisokane Road or the Flagstaff Drive/Halo Car Wash by the 2029 horizon year.

4.5 Intersection Design

4.5.1 Left Turn Lane Warrants

As stated in Section 2.1.3.1, future plans for the Borrisokane Road corridor inclusive of the Flagstaff Drive (Intersections 1) and Cambrian Road (Intersections 2) intersection include widening of the corridors and auxiliary turning lanes.

It is anticipated that the Flagstaff Drive intersection is to be upgraded by the end of 2023 and the Cambrian Road intersection by 2031. Requests were made to the City of Ottawa staff to review these plans, but unfortunately the plans could not be provided until they had been finalized.

An auxiliary left turn lane warrant analysis was undertaken for the Borrisokane Road /Site access and the Flagstaff/Halo Car Wash intersections following MTO geometric design standards ¹⁴ for Ontario highways. The warrants for an auxiliary left turn lanes are based on:

- the left turn volume: (LTvol);
- the volume of opposing vehicles: (Vo); and
- the volume of advancing vehicles: (Va).

The analysis assumed the 5-Year Horizon (2029) morning and afternoon peak hours of travel demand as illustrated within Exhibit 3-7. It was found that southbound left turn auxiliary lanes are not warranted at either intersection based on the traffic forecasts contained within this document.

The complete analysis tables can be found within Appendix "I".

The Borrisokane Road / Site Access intersection can continue to operate as a minor leg STOP-controlled intersection with no auxiliary lanes.

^{14 &}quot;Geometric Design Guide for Canadian Roads, Chapter 9: Intersections" TAC, June 2017, MTO Design Supplement, Appendix 9

5.0 CONCLUSION

The following traffic operation related conclusions have been found relating to the proposed 3555 Borrisokane Road Korean Community Church Development:

- The net effect of the site generated traffic associated with the proposed Korean Community Church Development upon the adjacent roadway network is anticipated to result in negligible impacts to traffic operational characteristics such as levels-of-service, volume-to-capacity ratios and delays at the adjacent intersections;
- Although the existing levels of service related to pedestrian and cycling modes along the area
 corridors can be considered poor, it is anticipated that the future proposed improvements along the
 Borrisokane Road, Flagstaff Drive and Cambrian Road corridors will represent significant
 improvements to the local community.
- A review of current parking demand and supply indicates that the development site has sufficient
 parking supply to accommodate the proposed Korean Community Church and the site plans are in
 full compliance with the City's current municipal parking by-laws for motor vehicles. The bicycle
 parking provisions will be confirmed upon site plan submission.
- A review of the warrants for left-turn auxiliary lanes at the proposed access into the site from Borrisokane Road and Flagstaff Drive indicated that there is no need, nor justification, for auxiliary left turn lanes that would provide access into the site.
- The Borrisokane Road Access is required to be wider at the edge of the roadway (Bylaw maximum is 9 meters, 17.25 meters needed) than the City's Private approach bylaw allows. This is in order to accommodate the turning movements of trucks, without encroaching over the median line or impeding oncoming traffic along Borrisokane Road.

6.0 SIGN-OFF

Should you have any questions or comments, please do not hesitate to contact us.

We await your feedback prior to finalizing the Traffic Impact Assessment report.

Yours truly,

Mr. Arthur Gordon B.A. P.Eng Principal Engineer

Castleglenn Consultants Inc.

Mr. Konstantin Joulanov BASc., M. Eng

Transportation Planner

Castleglenn Consultants Inc.



APPENDIX A: CERTIFICATION FORM FOR TIA STUDY PROJECT MANAGER



Certification Form for TIA Study PM

TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

✓	I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
/	I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
✓	I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
✓	I am either a licensed¹ or registered² professional in good standing, whose field of expertise is either transportation engineering or transportation planning.

^{1,2} License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

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City Of Ottawa Infrastructure Services and Community Sustainability Planning and Growth Management 110 Laurier Avenue West, 4th fl. Ottawa, ON K1P 1J1

Tel.: 613-580-2424 Fax: 613-560-6006

Revision Date: October, 2020

Dated at	Ottawa	this 18	_{day of} January	, 20 23 .
	(City)			
Name :	Arthur Gordon			
Professio	onal title: Principal,	Chairman Boa	rd of Directors	
	John -			
Signatur	e of individual certifier	that s/he meets	the above criteria	

Office Co	ntact Information (Please Print)
Address:	2460 Lancaster Road, Suite 200
City / Posta	al Code: K1B 4S5
E-Mail Add	ress: agordon@castleglenn.ca

Stamp



APPENDIX B: SCREENING FORM



City of Ottawa 2017 TIA Guidelines Screening Form

1. Description of Proposed Development

Municipal Address	3555 Borrisokane Road
Description of Location	Ottawa Korean Community Church
Land Use Classification	Light Industrial
Development Size (units)	0
Development Size (m²)	2913.5 m2 GFA
Number of Accesses and Locations	2
Phase of Development	1 phase
Buildout Year	Fall of 2024

If available, please attach a sketch of the development or site plan to this form.

2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size
Single-family homes	40 units
Townhomes or apartments	90 units
Office	3,500 m ²
Industrial	5,000 m ²
Fast-food restaurant or coffee shop	100 m ²
Destination retail	1,000 m ²
Gas station or convenience market	75 m²

^{*} If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, <u>the Trip Generation</u> <u>Trigger is satisfied.</u>

The average trip generation for a church during peak weekday hours based on ITE Trip Generation Manual is estimated to be under 20 trips, way below the 60 trips required for the Trip Generation Trigger.

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3. Location Triggers

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		

^{*}DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?		
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?		
Is the proposed driveway within auxiliary lanes of an intersection?		
Does the proposed driveway make use of an existing median break that serves an existing site?		
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		
Does the development include a drive-thru facility?		

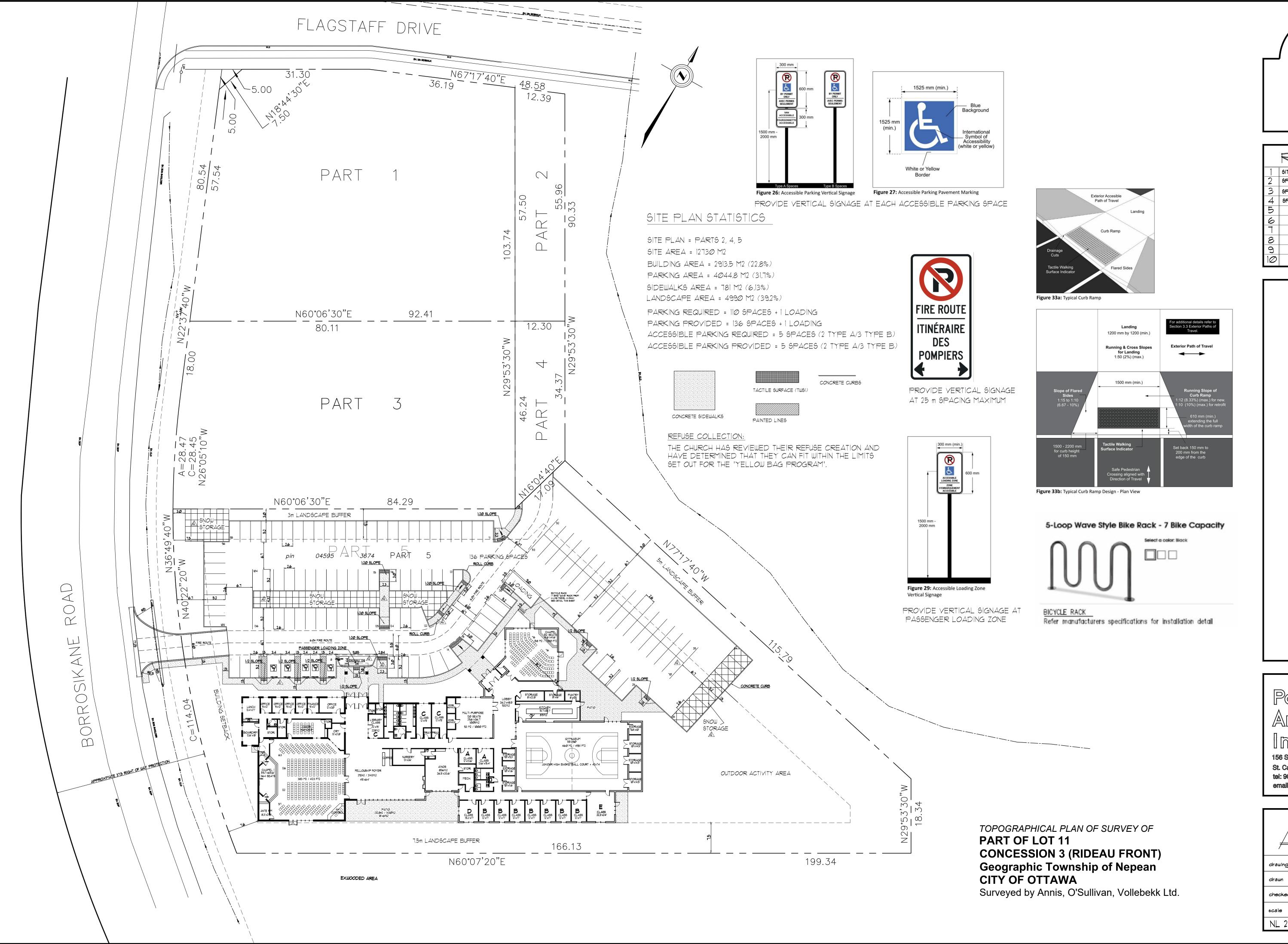
If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

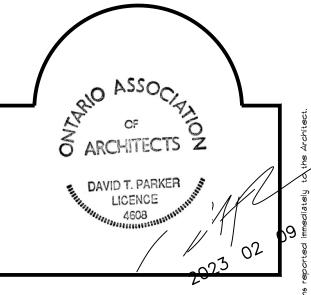
5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?		
Does the development satisfy the Location Trigger?		
Does the development satisfy the Safety Trigger?		

If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, <u>the TIA Study must continue into the next stage</u> (Screening and Scoping).

APPENDIX C: SITE PLAN





SITE PLAN APPLICATION JUNE 28/23
2 SPA REVIEW COMMENTS OCT 04/23
3 SPA REVIEW COMMENTS OCT 30/23
4 SPA REVIEW COMMENTS JAN 02/24
5
6
7
8
9

OTTAWA KOREAN COMMUNITY CHURCH 3555 BORRISOKANE ROAD OTTAWA, ONTARIO

Parker Architects Inc.

156 St. Paul Street
St. Catharines, ON, L2R 3M2
tel: 905-687-6681
email: info@parkerarchitects.ca

drawing SITE PLAN
drawn DTP

checked -

ecale 1:400 NL 22 - 106 date JUNE 28, 2023 APPENDIX D: EXISTING TRAFFIC COUNTS



Transportation Services - Traffic Services

Work Order

38148

Turning Movement Count - Full Study Summary (No AADT) Report

BORRISOKANE RD @ CAMBRIAN RD

Survey Date: Friday, November 09, 2018

Total Observed U-Turns

Northbound: ()

Southbound ()

Eastbound: ()

Westbound:

Full Study

			BOR	RISO	CANE	RD						CA	MBRU	AN RD					
	N	lorthbo	bnuc		S	outhbo	bnuc	770		1	Eastbo	und		1	Nestb	ound			
Pariod	LT	SŢ	RT	NB TOT	LT	ST	RT	SB	STR	LT	ST	RT	TOT	LT	51	RT	TOT	STR	Grand Total
06:30 07:30	0	30	13	43	103	26	D	131	174	0	0	0	0	- 5	0	308	313	313	487
07:30 OH:30	0	81	21	102	122	39	D	161	263	0	0	D	.0	25	0	409	434	434	697
06:30 09:30	0	66	18	84	140	34	Ď	174	258	0	0	o	0	19	0	332	351	351	609
15:30 16:80	D.	43	13	56	295	63	Ď	358	414	0	0	4	0	51	0	202	213	213	627
16:30: 17:30	D	39	38	57	328	65	n	393	450	Ü.	0	0	0	11	0	191	202	202	652
17:30. 18:30	0	23	2	25	369	49	D	418	443	ů.	0	0	0	5	0	190	195	195	638
18:30 19:30	0	6	2	8	147	18	n	165	173	0.	O.	0	0	1.4	0	96	97	97	270
Sub Total	0	288	87	375	1504	296	D	1800	2175	a-	0	D	.0	77	0	1728	1805	1805	3980
U Turns	D			0	.0			0	0	0			0	2			2	2	2
Total	0	288	87	375	1504	296	0	1800	2175	0	0	0	0	79	0	1728	1807	1807	3982

Comments

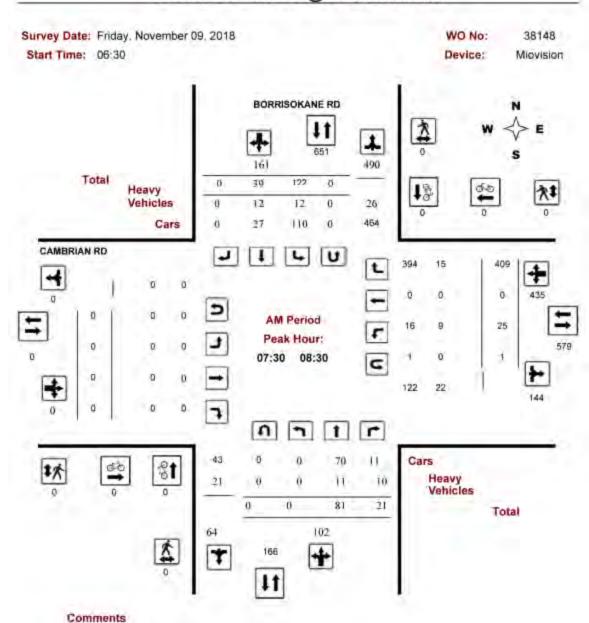
Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.



Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

BORRISOKANE RD @ CAMBRIAN RD

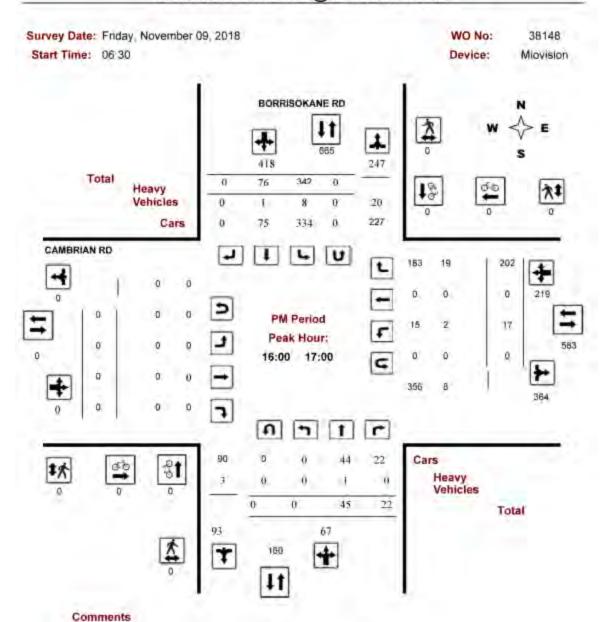


Ottawa

Transportation Services - Traffic Services

Turning Movement Count - Full Study Peak Hour Diagram

BORRISOKANE RD @ CAMBRIAN RD



APPENDIX E: SYNCHRO ANALYSIS

Intersection						
Int Delay, s/veh	10.2					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	450	}	00	40.4	4
Traffic Vol, veh/h	28	450	92	23	134	45
Future Vol, veh/h	28	450	92	23	134	45
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	29	474	97	24	141	47
Major/Minor I	Minor1	N	Major1	ı	Major2	
Conflicting Flow All	438	109	0	0	121	0
	109	109				
Stage 1		-	-	-	-	-
Stage 2	329		-	-		-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	517	939	-	-	1418	-
Stage 1	837	-	-	-	-	-
Stage 2	659	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	464	939	-	-	1418	-
Mov Cap-2 Maneuver	464	-	-	-	-	-
Stage 1	837	-	-	-	-	-
Stage 2	592	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	14.3		0		5.9	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_		1418	-
HCM Lane V/C Ratio		-	_	0.568		-
HCM Control Delay (s)		-	-		7.8	0
HCM Lane LOS		-	_	В	Α	A
HCM 95th %tile Q(veh))	-	-	3.7	0.3	-
				J	5.5	

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N.		1			र्स
Traffic Vol, veh/h	2	3	539	3	7	177
Future Vol, veh/h	2	3	539	3	7	177
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	-	-
Veh in Median Storage		-	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mymt Flow	2	3	567	3	7	186
IVIVIIIL FIOW		J	507	J	1	100
Major/Minor	Minor1	N	/lajor1		Major2	
Conflicting Flow All	769	569	0	0	570	0
Stage 1	569	-	-	-	-	-
Stage 2	200	<u>-</u>	<u>-</u>	<u>-</u>	_	<u>-</u>
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	0.22	_		4.12	_
, ,	5.42	-		-	-	-
Critical Hdwy Stg 2			-		2 240	
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	369	522	-	-	1002	-
Stage 1	566	-	-	-	-	-
Stage 2	834	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	366	522	-	-	1002	-
Mov Cap-2 Maneuver	366	-	-	-	-	-
Stage 1	566	-	-	-	-	-
Stage 2	827	-	-	_	-	-
y -						
Approach	WB		NB		SB	
HCM Control Delay, s	13.2		0		0.3	
HCM LOS	В					
Minor Long/Major Maria	. •	NDT	NDDV	VDL 4	CDI	CDT
Minor Lane/Major Mvn	Ι	NBT	NRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	446	1002	-
HCM Lane V/C Ratio		-	-	0.012		-
HCM Control Delay (s)		-	-	13.2	8.6	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)	-	-	0	0	-

Intersection						
Int Delay, s/veh	7.9					
		WDD	NDT	NDD	ODI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	000	₽	•	0=0	4
Traffic Vol, veh/h	19	222	52	24	376	87
Future Vol, veh/h	19	222	52	24	376	87
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	20	234	55	25	396	92
NA = 1 = =/NA1== .	N A : 4		1-1-4		4-1-0	
	Minor1		Major1		Major2	
Conflicting Flow All	952	68	0	0	80	0
Stage 1	68	-	-	-	-	-
Stage 2	884	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	250	990	-	-	1469	-
Stage 1	875	-	_	_	_	-
Stage 2	353	-	-	-	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	179	990	_	_	1469	_
Mov Cap-1 Maneuver	179	-	<u>-</u>		- 100	_
Stage 1	875	_				
•	253	-	_	_	_	
Stage 2	203	-	_	_	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	12.6		0		6.8	
HCM LOS	В				3.0	
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-		1469	-
HCM Lane V/C Ratio		_		0.348		_
HCM Control Delay (s)		_		12.6	8.4	0
HCM Lane LOS			_	12.0 B	Α	A
HCM 95th %tile Q(veh	١	-	-	1.6	1.1	- A
HOM SOM WHILE CALACH)	_	-	1.0	1.1	_

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	<u></u>	1	, LOIK	UDL	4
Traffic Vol, veh/h	3	7	272	2	3	460
Future Vol, veh/h	3	7	272	2	3	460
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	3	7	286	2	3	484
NA = : = = /NA:== =	A! 4		1-:- 4		4-1-0	
	Minor1		//ajor1		Major2	
Conflicting Flow All	777	287	0	0	288	0
Stage 1	287	-	-	-	-	-
Stage 2	490	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-	2.218	-
Pot Cap-1 Maneuver	365	752	-	-	1274	-
Stage 1	762	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	364	752	-	-	1274	-
Mov Cap-2 Maneuver	364	-	-	-	-	-
Stage 1	762	-	-	-	-	-
Stage 2	614	-	-	-	-	-
Approach	WB		NB		SB	
Approach HCM Control Delay s	WB		NB 0		SB 0.1	
HCM Control Delay, s	11.4		NB 0		SB 0.1	
HCM Control Delay, s HCM LOS	11.4 B		0		0.1	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	11.4 B	NBT	0 NBRV	VBLn1	0.1 SBL	SBT
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	11.4 B	NBT -	0 NBRV	570	0.1 SBL 1274	SBT_
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	11.4 B	NBT - -	0 NBRV -	570 0.018	0.1 SBL 1274 0.002	-
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	11.4 B	-	0 NBRV	570 0.018 11.4	0.1 SBL 1274 0.002 7.8	- - 0
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	11.4 B	-	0 NBRV -	570 0.018	0.1 SBL 1274 0.002	-

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	אטול	1\D1	TOIL	ODL	- 6 1
Traffic Vol, veh/h	0	0	630	0	0	220
Future Vol, veh/h	0	0	630	0	0	220
Conflicting Peds, #/hr	0	0	030	0	0	0
Sign Control		Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	riee -		riee -	None
Storage Length	0	NOHE -	-	NONE -	-	None
		-	0		-	0
Veh in Median Storage	•					
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	663	0	0	232
Major/Minor	Minor1	N	/lajor1	ı	Major2	
Conflicting Flow All	895	663	0	0	663	0
Stage 1	663	-	-	-	-	_
Stage 2	232	_	_	_	-	_
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	-	_	_		_
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518	3 318	_	_	2.218	_
Pot Cap-1 Maneuver	311	461	_	_	926	_
Stage 1	512	-	_	_	-	_
Stage 2	807	_	_	_	_	_
Platoon blocked, %	001		_	_		_
Mov Cap-1 Maneuver	311	461	_	_	926	_
Mov Cap-2 Maneuver	311	-	_	_	-	_
Stage 1	512	_	_	_	_	_
Stage 2	807	_	_		_	_
Stage 2	007		_		-	
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBT	NIPDV	VBLn1	SBL	SBT
	IL		INDEA			
Capacity (veh/h)		-	-	-	926	-
HCM Cantral Dalay (a)		-	-	-	-	-
HCM Long LOS		-	-	0	0	-
HCM Lane LOS	\	-	-	Α	A	-
HCM 95th %tile Q(veh		-	-	-	0	-

Intersection						
Int Delay, s/veh	12.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		1			4
Traffic Vol, veh/h	32	514	116	26	154	66
Future Vol, veh/h	32	514	116	26	154	66
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mymt Flow	34	541	122	27	162	69
IVIVIII(I IOW	04	J+1	122	LI	102	03
Major/Minor	Minor1	N	Major1	<u> </u>	Major2	
Conflicting Flow All	529	136	0	0	149	0
Stage 1	136	-	-	-	-	-
Stage 2	393	_	-	-	-	-
Critical Hdwy	6.76	6.24	_	_	4.2	_
Critical Hdwy Stg 1	5.76	-	_	_		_
Critical Hdwy Stg 2	5.76	_		_	_	_
Follow-up Hdwy	3.824		_	_	2.29	_
Pot Cap-1 Maneuver	456	907	_	_	1385	-
	813			-		
Stage 1		-	-	-	-	-
Stage 2	614	-	-	-	-	-
Platoon blocked, %	400		-	-	100-	-
Mov Cap-1 Maneuver	400	907	-	-	1385	-
Mov Cap-2 Maneuver	400	-	-	-	-	-
Stage 1	813	-	-	-	-	-
Stage 2	539	-	-	-	-	-
Approach	WB		NB		SB	
	17.9				5.6	
HCM Control Delay, s			0		5.0	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-		844	1385	
HCM Lane V/C Ratio		_	_	0.681		_
HCM Control Delay (s	۱	_	_	17.9	7.9	0
HCM Lane LOS				17.9 C	7.9 A	A
	1	-	-	5.5	0.4	
HCM 95th %tile Q(veh)	-	-	ე.ე	0.4	-

Intersection						
Int Delay, s/veh	4.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1	11511	UDL	4
Traffic Vol, veh/h	18	173	614	15	122	202
Future Vol, veh/h	18	173	614	15	122	202
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None		None	-	
Storage Length	0	-	_	-	-	-
Veh in Median Storage		_	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	19	182	646	16	128	213
WWITCH	10	102	0-10	10	120	210
Major/Minor I	Minor1		Major1		Major2	
Conflicting Flow All	1123	654	0	0	662	0
Stage 1	654	-	-	-	-	-
Stage 2	469	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	227	467	-	-	927	-
Stage 1	517	-	-	-	-	-
Stage 2	630	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	191	467	-	-	927	-
Mov Cap-2 Maneuver	191	-	-	-	-	-
Stage 1	517	-	-	-	-	-
Stage 2	531	-	_	-	-	-
3 11 3 1						
	14/5				25	
Approach	WB		NB		SB	
HCM Control Delay, s	21.9		0		3.6	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	411	927	
HCM Lane V/C Ratio		_	_	0.489		_
HCM Control Delay (s)		_	_	21.9	9.5	0
HCM Lane LOS		_	_	C C	Α	A
HCM 95th %tile Q(veh))	_	_	2.6	0.5	-
. Tom Jour Jours Q Vor				2.0	0.0	

Intersection						
Int Delay, s/veh	0.8					
		EDD	VA/PI	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	107	40	40	4100	Y	
Traffic Vol, veh/h	127	10	10	182	10	9
Future Vol, veh/h	127	10	10	182	10	9
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	134	11	11	192	11	9
Major/Mirror	ni = =4		1c:- 0		Aire - 4	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	145	0	354	140
Stage 1	-	-	-	-	140	-
Stage 2	-	-	-	-	214	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	_	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1437	-	644	908
Stage 1	-	-	-	-	887	-
Stage 2	_	_	_	_	822	-
Platoon blocked, %	_	_		_	VLL	
Mov Cap-1 Maneuver		_	1437		638	908
Mov Cap-1 Maneuver	-	_	1437	-	638	900
•	-	-			887	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	_	815	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		10	
HCM LOS	U		U. 4		В	
I IOIVI LOO					D	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		743	-	-	1437	-
HCM Lane V/C Ratio		0.027	-		0.007	-
HCM Control Delay (s)		10	-	-	7.5	0
HCM Lane LOS		В	-	-	A	A
HCM 95th %tile Q(veh)		0.1	-	_	0	-
JULI /VIII Q(VOII)		V. 1			J	

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL W	אופאי	1\D1	NDIX	ODL	- 1
Traffic Vol, veh/h	0	0	334	0	0	559
Future Vol, veh/h	0	0	334	0	0	559
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
Storage Length	0	NOITE	_	NONE -	_	NOHE
Veh in Median Storage		_	0	_	_	0
Grade, %	s, # 0 0	-	0	_	-	0
Peak Hour Factor	95	95	95	95	95	95
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	0	0	352	0	0	588
Major/Minor	Minor1	N	/lajor1	ı	Major2	
Conflicting Flow All	940	352	0	0	352	0
Stage 1	352	-	_	-	_	-
Stage 2	588	-	-	-	-	-
Critical Hdwy	6.42	6.22	_	-	4.12	-
Critical Hdwy Stg 1	5.42	-	_	_	-	_
Critical Hdwy Stg 2	5.42	_	-	_	-	_
Follow-up Hdwy	3.518	3.318	-	_	2.218	_
Pot Cap-1 Maneuver	293	692	-	_	1207	-
Stage 1	712	-	-	_	_	_
Stage 2	555	_	-	_	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	293	692	_	_	1207	_
Mov Cap-2 Maneuver	293	-	_	_	-	_
Stage 1	712	_	_	_	_	_
Stage 2	555	_	_	_	_	_
Glage 2	555					
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	\ +	NBT	NIDDV	VBLn1	SBL	SBT
	IL		NDRV			
Capacity (veh/h)		-	-		1207	-
HCM Cantrol Delay (a)		-	-	-	-	-
HCM Long LOS		-	-	0	0	-
HCM Lane LOS	\	-	-	A	A	-
HCM 95th %tile Q(veh		-	-	-	0	-

Intersection						
Int Delay, s/veh	8.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		WDK		NDK	ODL	
Lane Configurations	** 22	260	74	27	126	ન
Traffic Vol, veh/h	22		74 74	27	436 436	110
Future Vol, veh/h	0	260 0	74 0	27 0	436	110
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	23	274	78	28	459	116
Major/Minor I	Minor1	N	Major1	ı	Major2	
		92			106	0
Conflicting Flow All	1126		0	0		
Stage 1	92	-	-	-	-	-
Stage 2	1034	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	195	960	-	-	1437	-
Stage 1	853	-	-	-	-	-
Stage 2	297	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	129	960	-	-	1437	-
Mov Cap-2 Maneuver	129	-	-	-	-	-
Stage 1	853	_	-	-	-	-
Stage 2	196	_	_	_	_	_
Jugo L	.00					
Approach	WB		NB		SB	
HCM Control Delay, s	15.4		0		6.9	
HCM LOS	С					
Minor Lang/Major My	+	NDT	NDDV	MDI 51	CDI	CDT
Minor Lane/Major Mvm	l .	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1437	-
HCM Lane V/C Ratio		_	_	0.465	U.319	-
HCM Control Delay (s)		-	-	15.4	8.7	0
		-			8.7 A 1.4	0 A

Intersection						
Int Delay, s/veh	3.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	וטייי	1\U	TADIA	ODL	- 6 1
Traffic Vol, veh/h	21	116	310	24	167	524
Future Vol, veh/h	21	116	310	24	167	524
-	0	0	0	0	0	0
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None		None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	22	122	326	25	176	552
Major/Minor	Minor1		Aniar1		Majara	
	Minor1		Major1		Major2	
Conflicting Flow All	1243	339	0	0	351	0
Stage 1	339	-	-	-	-	-
Stage 2	904	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-		-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	193	703	-	_	1208	-
Stage 1	722	-	-	_	-	-
Stage 2	395	-	_	-	-	-
Platoon blocked, %	300		_	_		_
Mov Cap-1 Maneuver	152	703	_	_	1208	
Mov Cap-1 Maneuver	152	100	_		1200	
	722	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	312	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	16.7		0		2.1	
HCM LOS	C		- 0		۷.۱	
I IOWI LOG	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	452	1208	-
HCM Lane V/C Ratio		-	_	0.319		-
HCM Control Delay (s)		_	_		8.5	0
HCM Lane LOS		_	_	C	Α	A
HCM 95th %tile Q(veh)	\		_	1.4	0.5	
HOW JOHN JOHN WINE WINE	,			1.7	0.0	

Intersection						
Int Delay, s/veh	0.8					
		EDD	14/51	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			र्स	A	
Traffic Vol, veh/h	182	10	10	127	10	9
Future Vol, veh/h	182	10	10	127	10	9
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	192	11	11	134	11	9
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	203	0	354	198
Stage 1	-	-	-	-	198	-
Stage 2	-	-	-	-	156	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	_	-	2.218	_	3.518	3.318
Pot Cap-1 Maneuver	_	_	1369	_	644	843
Stage 1	_	_	-	_	835	-
Stage 2				_	872	_
Platoon blocked, %	_	_	_	_	012	_
Mov Cap-1 Maneuver	_	-	1369		638	843
	-	-	1309	-		
Mov Cap-2 Maneuver	-	-	-	-	638	-
Stage 1	-	-	-	-	835	-
Stage 2	-	-	-	-	864	-
Approach	EB		WB		NB	
	0		0.6		10.1	
HCM LOS	U		0.0			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		721			1369	
HCM Lane V/C Ratio		0.028	_		0.008	_
HCM Control Delay (s)		10.1	_	_	7.7	0
HCM Lane LOS		В			Α	A
HCM 95th %tile Q(veh)		0.1	-	-	0	- -
HOW Sour Mule Q(ven)		U. I	-	-	U	_

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		1			4
Traffic Vol, veh/h	6	5	630	8	6	220
Future Vol, veh/h	6	5	630	8	6	220
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	5	663	8	6	232
IVIVIIIL FIOW	U	ວ	003	0	U	232
Major/Minor	Minor1	N	/lajor1	ı	Major2	
Conflicting Flow All	911	667	0	0	671	0
Stage 1	667	-	_	-	_	-
Stage 2	244	-	-	_	_	_
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	-	_	_		_
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518		_		2.218	_
Pot Cap-1 Maneuver	304	459			919	
Stage 1	510	455	_	_	919	_
Stage 2	797	-	-	-	-	-
	191	-	-	-	-	
Platoon blocked, %	200	450	-	-	040	-
Mov Cap-1 Maneuver	302	459	-	-	919	-
Mov Cap-2 Maneuver	302	-	-	-	-	-
Stage 1	510	-	-	-	-	-
Stage 2	791	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	15.4		0		0.2	
HCM LOS	13.4 C		U		0.2	
I IOIVI LUO	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	358	919	-
HCM Lane V/C Ratio		-	-	0.032		_
HCM Control Delay (s)		_	_	15.4	8.9	0
HCM Lane LOS		-	_	С	А	A
HCM 95th %tile Q(veh))	-	-	0.1	0	-
TOTAL OCUT TOUTO SE(VOIT)	1			0.1	9	

Intersection						
Int Delay, s/veh	12.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	WDR		NDR	ODL	<u> २०।</u>
Traffic Vol, veh/h	32	518	1 20	26	157	6 9
Future Vol, veh/h	32	518	120	26	157	69
Conflicting Peds, #/hr	0	0	0	0	0	09
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	
Storage Length	0	-	_	-	_	-
			0			
Veh in Median Storage		-	~	-	-	0
Grade, %	0	05	0	05	05	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	34	545	126	27	165	73
Major/Minor	Minor1	N	Major1		//ajor2	
Conflicting Flow All	543	140	0	0	153	0
Stage 1	140	-	-	-	-	-
Stage 2	403	_	_	_	_	_
Critical Hdwy	6.76	6.24	_	_	4.2	_
Critical Hdwy Stg 1	5.76	- 0.27	_	_	-7.2	_
Critical Hdwy Stg 1	5.76	_		_		
Follow-up Hdwy	3.824	3.336		_	2.29	
Pot Cap-1 Maneuver	447	903	_		1380	_
Stage 1	810	300	_		1000	
Stage 2	607			-		
Platoon blocked, %	007	-	_	_	_	-
Mov Cap-1 Maneuver	391	903	-	-	1380	
Mov Cap-1 Maneuver	391	303	-	-	1300	-
	810	-		-	-	-
Stage 1		-	-	-	-	-
Stage 2	531	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	18.3		0		5.5	
HCM LOS	С					
		NAT	NES	VDL 4	0.51	007
Minor Lane/Major Mvm	ıt .	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	000	1380	-
HCM Lane V/C Ratio		-	-	0.69	0.12	-
HCM Control Delay (s)		-	-	18.3	8	0
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh)		-	-	5.7	0.4	-

Intersection						
Int Delay, s/veh	4.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	וטוי	1\D1	אטוז	ODL	<u>₽</u>
Traffic Vol, veh/h	18	174	619	15	123	208
Future Vol, veh/h	18	174	619	15	123	208
Conflicting Peds, #/hr	0	0	019	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	s, # 0 0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
	95	2	95 5	95	95	15
Heavy Vehicles, %						
Mvmt Flow	19	183	652	16	129	219
Major/Minor	Minor1	N	Major1	ľ	Major2	
Conflicting Flow All	1137	660	0	0	668	0
Stage 1	660	-	-	-	-	-
Stage 2	477	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	_	_	-	-
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy		3.318	_	_	2.218	_
Pot Cap-1 Maneuver	223	463	_	_	922	_
Stage 1	514	-100		_	- JLL	_
Stage 2	624	_	-	-	-	-
Platoon blocked, %	024	-	_	_	-	
-	100	463		-	922	
Mov Cap-1 Maneuver	188	403	-	-	922	-
Mov Cap-2 Maneuver	188	-	-	-	-	-
Stage 1	514	-	-	-	-	-
Stage 2	525	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	22.3		0		3.5	
HCM LOS	C				3.0	
					0	
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		922	-
HCM Lane V/C Ratio		-	-	0.497	0.14	-
HCM Control Delay (s)		-	-	22.3	9.5	0
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh))	-	-	2.7	0.5	-
, -						

Intersection						
Int Delay, s/veh	0.9					
			14/5	14/5-		
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7			ન	Y	
Traffic Vol, veh/h	127	11	14	182	11	12
Future Vol, veh/h	127	11	14	182	11	12
Conflicting Peds, #/hr	0	0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	† 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	134	12	15	192	12	13
NA . ' . /NA'			1		A'	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	146	0	362	140
Stage 1	-	-	-	-	140	-
Stage 2	-	-	-	-	222	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1436	-	637	908
Stage 1	_	_	-	_	887	-
Stage 2	_	_	_	_	815	_
Platoon blocked, %	_	_		_	310	
Mov Cap-1 Maneuver			1436	_	629	908
Mov Cap-1 Maneuver	_	_	1430	_	629	300
	_	-			887	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	805	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.5		10	
HCM LOS	U		0.0		В	
TIOWI LOO					U	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		749	-	-	1436	-
HCM Lane V/C Ratio		0.032	_	-	0.01	_
HCM Control Delay (s)		10	-	-	7.5	0
HCM Lane LOS		В	-	-	Α	A
HCM 95th %tile Q(veh)		0.1	_	_	0	-
		J. 1			J	

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>			4
Traffic Vol, veh/h	8	7	334	8	6	559
Future Vol, veh/h	8	7	334	8	6	559
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	8	7	352	8	6	588
IVIVIII(I IOW	U	1	332	U	U	300
Major/Minor I	Minor1	N	Major1	ľ	Major2	
Conflicting Flow All	956	356	0	0	360	0
Stage 1	356	-	-	-	-	-
Stage 2	600	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	_
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	286	688	-	-	1199	-
Stage 1	709	-	-	-	-	-
Stage 2	548	-	-	-	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	284	688	-	-	1199	_
Mov Cap-2 Maneuver	284	-	_	_	-	_
Stage 1	709	_	_	_	_	_
Stage 2	544	_	_	_	_	_
Olago 2	011					
Approach	WB		NB		SB	
HCM Control Delay, s	14.6		0		0.1	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VRI n1	SBL	SBT
Capacity (veh/h)	IC .	INDI	אוטויי	391	1199	ו מט
HCM Lane V/C Ratio		-	-			-
		-	-	14.6	0.005	-
HCM Control Delay (s) HCM Lane LOS		-	-	14.6 B	8 A	0 A
	\	-	-	0.1	0	
HCM 95th %tile Q(veh))	-	-	0.1	U	-

Intersection						
Int Delay, s/veh	8.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1			4
Traffic Vol, veh/h	22	264	78	27	440	114
Future Vol, veh/h	22	264	78	27	440	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	23	278	82	28	463	120
NA = : = = /NA:= =	N 41:		1-1-4		4-1-0	
	Minor1		Major1		Major2	
Conflicting Flow All	1142	96	0	0	110	0
Stage 1	96	-	-	-	-	-
Stage 2	1046	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy		3.336	-	-	2.29	-
Pot Cap-1 Maneuver	190	955	-	-	1432	-
Stage 1	849	-	-	-	-	-
Stage 2	292	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	124	955	-	-	1432	-
Mov Cap-2 Maneuver	124	-	-	-	-	-
Stage 1	849	-	-	-	-	-
Stage 2	191	-	-	-	-	-
Approach	WB		NB		SB	
HCM LOS	15.8 C		0		6.9	
HCM LOS	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	630	1432	-
HCM Lane V/C Ratio		-	-	0.478		-
HCM Control Delay (s)		_	-	15.8	8.7	0
HCM Lane LOS		-	-	С	Α	A
HCM 95th %tile Q(veh))	-	-	2.6	1.4	-
2111 22 21 70 21 2 2 (1011)						

Intersection						
Int Delay, s/veh	3.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	וטייי	1\U	TIDIT	ODL	- 6 1
Traffic Vol, veh/h	21	117	317	24	168	530
Future Vol, veh/h	21	117	317	24	168	530
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
Storage Length	0	-	_	-	_	NONE
Veh in Median Storage			0		_	0
Grade, %	0, # 0	<u>-</u>	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	22	123	334	25	177	558
WIVITIT FIOW	22	123	334	20	177	220
Major/Minor	Minor1	N	Major1	ı	Major2	
Conflicting Flow All	1259	347	0	0	359	0
Stage 1	347	-	-	-	-	-
Stage 2	912	-	-	_	-	_
Critical Hdwy	6.42	6.22	_	-	4.12	-
Critical Hdwy Stg 1	5.42	_	_	_	_	_
Critical Hdwy Stg 2	5.42	_	_	-	_	-
Follow-up Hdwy	3.518	3.318	-	_	2.218	_
Pot Cap-1 Maneuver	188	696	_	-	1200	-
Stage 1	716	-	-	_	-	_
Stage 2	392	_	_	_	_	_
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	148	696	_	_	1200	_
Mov Cap-2 Maneuver		-	_	_	-	_
Stage 1	716	_	_	_	_	_
Stage 2	308	_	_	_	_	_
Olaye Z	300					
Approach	WB		NB		SB	
HCM Control Delay, s	17		0		2.1	
HCM LOS	С					
Minor Long/Major M.		NDT	MDDV	MDL 4	CDI	CDT
Minor Lane/Major Mvr	iit	NBT		WBLn1	SBL	SBT
Capacity (veh/h)		-	-		1200	-
HCM Lane V/C Ratio		-		0.326		-
HCM Control Delay (s)	-	-		8.5	0
110141 100	,			_		
HCM Lane LOS HCM 95th %tile Q(veh	,	-	-	C 1.4	A 0.5	A -

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK	VVDL			NDK
Lane Configurations	182	11	1.1	127	11	13
Traffic Vol. veh/h	182 182	11	14	127 127	11	
Future Vol, veh/h		11	14		11	13
Conflicting Peds, #/hr	0 Eroo	0 Eroo	0 Eroo	0 Eroo	0 Stop	0 Stop
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	- # 0	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	05	05	0	0	- 05
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	102	2	2	124	2	2
Mvmt Flow	192	12	15	134	12	14
Major/Minor N	/lajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	204	0	362	198
Stage 1	-	-	-	-	198	-
Stage 2	_	-	_	_	164	_
Critical Hdwy		_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	_	-	-	_	5.42	-
Critical Hdwy Stg 2	_		_	_	5.42	_
Follow-up Hdwy	_	_	2.218	_	3.518	
Pot Cap-1 Maneuver		_	1368	-	637	843
Stage 1	_	-	- 330	_	835	-
Stage 2	_	_	_	_	865	_
Platoon blocked, %	_	_	-	_	000	_
Mov Cap-1 Maneuver		_	1368	_	629	843
Mov Cap-1 Maneuver	-	-	1000	_	629	- 043
Stage 1	-	-		-	835	
_	_	-	-	-	855	-
Stage 2	-	-	_	-	000	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		10.1	
HCM LOS					В	
Minor Lane/Major Mvmt	·	NBLn1	EBT	EBR	WBL	WBT
	·					
Capacity (veh/h)		729	-		1368	-
HCM Control Doloy (a)		0.035	-		0.011	-
HCM Long LOS		10.1	-	-	7.7	0
HCM C5th 9/tile O(vah)		B	-	-	A	Α
HCM 95th %tile Q(veh)		0.1	-	-	0	-

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	אטוע	1dVI	NOIN	ODL	<u>₩</u>
Traffic Vol, veh/h	0	0	576	0	0	202
Future Vol, veh/h	0	0	576	0	0	202
Conflicting Peds, #/hr	0	0	0	0	0	202
			Free		Free	Free
Sign Control RT Channelized	Stop -	Stop None		Free None	Free -	
			-			
Storage Length	0	-	-	-	-	-
Veh in Median Storage	-	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	606	0	0	213
Major/Minor I	Minor1	N	//ajor1	I	Major2	
Conflicting Flow All	819	606	0	0	606	0
Stage 1	606	-	-	-	-	-
Stage 2	213	_	_	_		_
Critical Hdwy	6.42	6.22	_	_	4.12	-
Critical Hdwy Stg 1	5.42	U.ZZ		-	4.12	_
Critical Hdwy Stg 2	5.42	-	-	-	-	
, ,		3.318	-	-	2.218	-
Follow-up Hdwy			-	-		-
Pot Cap-1 Maneuver	345	497	-	-	972	-
Stage 1	545	-	-	-	-	-
Stage 2	823	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	345	497	-	-	972	-
Mov Cap-2 Maneuver	345	-	-	-	-	-
Stage 1	545	-	-	-	-	-
Stage 2	823	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	_	972	-
HCM Lane V/C Ratio		_	_	_		_
HCM Control Delay (s)		_	_	0	0	_
HCM Lane LOS		_	_	A	A	_
HCM 95th %tile Q(veh)			_	-	0	_
HOW JOHN /OHIE Q(VEH)					U	

Intersection						
Int Delay, s/veh	10.6					
		14/5-5			0-:-	05-
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	A		1			4
Traffic Vol, veh/h	29	469	107	24	140	62
Future Vol, veh/h	29	469	107	24	140	62
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	31	494	113	25	147	65
Majay/Minay	\		1-1-1		AnineO	
	Minor1		Major1		Major2	
Conflicting Flow All	485	126	0	0	138	0
Stage 1	126	-	-	-	-	-
Stage 2	359	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	485	919	-	-	1398	-
Stage 1	822	-	-	-	-	-
Stage 2	638	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	432	919	-	_	1398	-
Mov Cap-2 Maneuver	432	-	-	-	-	-
Stage 1	822	_	-	_	_	-
Stage 2	568	_	_	_	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	15.4		0		5.5	
HCM LOS	С					
Minor Lane/Major Mvm	ıt	NBT	NRRV	VBLn1	SBL	SBT
		INDI	INDIN	862	1398	ODI
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.608		-
HCM Control Delay (s)		-		15.4	7.9	0
HCM Lane LOS		-	-	15.4 C		
		-	-	4.2	0.4	Α
HCM 95th %tile Q(veh)		-	-	4.2	0.4	-

Intersection						
Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f _a		<u> </u>	4
Traffic Vol, veh/h	18	173	561	15	121	184
Future Vol, veh/h	18	173	561	15	121	184
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mymt Flow	19	182	591	16	127	194
WWITETIOW	10	102	001	10	121	134
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	1047	599	0	0	607	0
Stage 1	599	-	-	-	-	-
Stage 2	448	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	253	502	-	-	971	-
Stage 1	549	-	-	-	-	-
Stage 2	644	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	216	502	-	-	971	_
Mov Cap-2 Maneuver	216	-	-	_	-	_
Stage 1	549	_	-	_	_	_
Stage 2	549	_	_	_	_	_
Olago 2	0.10					
Approach	WB		NB		SB	
LIONA O LE LIDEL	19.5		0		3.7	
HCM Control Delay, s						
HCM Control Delay, s HCM LOS	С					
	С					
HCM LOS		NRT	NRR	NRI n1	SRI	SRT
HCM LOS Minor Lane/Major Mvn		NBT		WBLn1	SBL 971	SBT
Minor Lane/Major Mvn Capacity (veh/h)		-	-	446	971	-
Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	nt	NBT - -	-	446 0.451	971 0.131	<u>-</u> -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	nt	- -	- - -	446 0.451 19.5	971 0.131 9.3	- - 0
Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	<u>nt</u>	-	-	446 0.451	971 0.131	<u>-</u> -

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	11511	1	TTDIT.	ODL	4
Traffic Vol, veh/h	0	0	307	0	0	520
Future Vol, veh/h	0	0	307	0	0	520
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	-	-
Veh in Median Storage,		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	0	323	0	0	547
WWITTIOW	U	U	020	U	U	J-1
Major/Minor N	/linor1	N	Major1	N	Major2	
Conflicting Flow All	870	323	0	0	323	0
Stage 1	323	-	-	-	-	-
Stage 2	547	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	_
	3.518	3.318	_	_	2.218	-
Pot Cap-1 Maneuver	322	718	_	_	1237	-
Stage 1	734	-	-	_	_	_
Stage 2	580	_	_	_	_	_
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	322	718	_	_	1237	_
Mov Cap-1 Maneuver	322	- 10	_	_	1201	_
Stage 1	734		_			
Stage 2	580	_		_	-	-
		_	-	-		-
Olugo Z	500					
Olago Z	300					
	WB		NB		SB	
Approach	WB					
Approach HCM Control Delay, s	WB 0		NB 0		SB 0	
Approach	WB					
Approach HCM Control Delay, s HCM LOS	WB 0 A		0	A/DL 4	0	ODT
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	WB 0 A	NBT	0	VBLn1	0 SBL	SBT
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	WB 0 A		0	-	0	-
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	WB 0 A	NBT -	0 NBRV -	-	0 SBL 1237	-
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	WB 0 A	NBT -	0 NBRV	- - 0	0 SBL 1237 - 0	-
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	WB 0 A	NBT -	0 NBRV -	-	0 SBL 1237	-

Intersection						
Int Delay, s/veh	8.1					
		14/5-5			05:	0==
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	A		1€			ની
Traffic Vol, veh/h	20	238	69	25	398	101
Future Vol, veh/h	20	238	69	25	398	101
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	21	251	73	26	419	106
manici ion		201	7.0	20	110	100
Major/Minor	Minor1	N	Major1	N	//ajor2	
Conflicting Flow All	1030	86	0	0	99	0
Stage 1	86	-	-	-	-	-
Stage 2	944	-	-	_	_	_
Critical Hdwy	6.76	6.24	_	_	4.2	_
Critical Hdwy Stg 1	5.76	- V.L.	_	_	- '	_
Critical Hdwy Stg 1	5.76	_			_	
Follow-up Hdwy			_	_	2.29	_
Pot Cap-1 Maneuver	224	967	-	-	1445	
		907	-	-	1443	-
Stage 1	858	-	-	-	-	-
Stage 2	329	-	-	-	-	-
Platoon blocked, %			-	-	44	-
Mov Cap-1 Maneuver	155	967	-	-	1445	-
Mov Cap-2 Maneuver	155	-	-	-	-	-
Stage 1	858	-	-	-	-	-
Stage 2	228	-	-	-	-	-
Annress	MA		MD		OD	
Approach	WB		NB		SB	
HCM Control Delay, s	13.6		0		6.8	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NRDV	VBLn1	SBL	SBT
	IL.	וטוו				ODT
Capacity (veh/h)		-	-		1445	-
HCM Cantrol Dalay (a)		-		0.395	0.29	-
HCM Control Delay (s)		-	-	13.6	8.5	0
HCM Lane LOS		-	-	В	A	Α
HCM 95th %tile Q(veh)		-	-	1.9	1.2	-

Intersection						
Int Delay, s/veh	3.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1			4
Traffic Vol, veh/h	21	115	283	24	167	478
Future Vol, veh/h	21	115	283	24	167	478
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage		_	0	-	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	22	121	298	25	176	503
IVIVIIIL FIOW	22	121	230	25	170	505
Major/Minor I	Minor1		Major1	1	Major2	
Conflicting Flow All	1166	311	0	0	323	0
Stage 1	311	-	-	-	-	-
Stage 2	855	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	_	_	_	_
Follow-up Hdwy	3.518	3.318	_	_	2.218	-
Pot Cap-1 Maneuver	214	729	_	_	1237	_
Stage 1	743	-	_	_	0,	_
Stage 2	417	_	_	_	_	_
Platoon blocked, %	711		_	_		_
Mov Cap-1 Maneuver	172	729	_	_	1237	
Mov Cap-1 Maneuver	172	129	-	_	1231	-
	743		-	-	-	-
Stage 1		-		-	-	-
Stage 2	335	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	15.5		0		2.2	
HCM LOS	С					
Minor Long/Maior M		NDT	MDD	MDI 4	ODI	CDT
Minor Lane/Major Mvm	IT	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	486	1237	-
HCM Lane V/C Ratio		-	-	0.295		-
HCM Control Delay (s)		-	-	15.5	8.4	0
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh)		-	-	1.2	0.5	-

Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	11511	\$	11211	UDL	4
Traffic Vol, veh/h	8	8	576	9	9	202
Future Vol, veh/h	8	8	576	9	9	202
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	8	606	9	9	213
NA . ' (NA'	\ A' \ \ \ 4		1.1.4		4	
	Minor1		Major1		Major2	
Conflicting Flow All	842	611	0	0	615	0
Stage 1	611	-	-	-	-	-
Stage 2	231	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	334	494	-	-	965	-
Stage 1	542	-	-	-	-	-
Stage 2	807	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	330	494	-	-	965	-
Mov Cap-2 Maneuver	330	-	-	-	-	-
Stage 1	542	-	-	-	-	-
Stage 2	798	-	-	-	-	-
, and the second						
Approach	\A/D		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	14.5		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	396	965	-
HCM Lane V/C Ratio		-	_	0.043	0.01	-
HCM Control Delay (s)		_	-	14.5	8.8	0
HCM Lane LOS		-	_	В	А	A
HCM 95th %tile Q(veh))	_	_	0.1	0	_
7000 00 00				J .,		

Intersection						
Int Delay, s/veh	10.7					
		14/5-5			0-:-	05-
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1			4
Traffic Vol, veh/h	29	474	111	24	145	65
Future Vol, veh/h	29	474	111	24	145	65
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	31	499	117	25	153	68
Major/Mina-	Minera		Anic 1		Ania no	
	Minor1		Major1		Major2	
Conflicting Flow All	504	130	0	0	142	0
Stage 1	130	-	-	-	-	-
Stage 2	374	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	472	914	-	-	1393	-
Stage 1	819	-	-	-	-	-
Stage 2	627	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	418	914	-	_	1393	_
Mov Cap-2 Maneuver	418	-	_	_	-	_
Stage 1	819	_	_	_	_	_
Stage 2	556	<u>-</u>	_	_	_	_
Glaye Z	550	_	-	_	_	
Approach	WB		NB		SB	
HCM Control Delay, s	15.8		0		5.5	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NRPV	VBLn1	SBL	SBT
	IC .	INDI	אאטאו			ODT
Capacity (veh/h)		-	-	855	1393	-
HCM Control Doloy (a)		-		0.619	0.11	-
HCM Control Delay (s)		-	-	15.8	7.9	0
HCM Lane LOS HCM 95th %tile Q(veh)		-	-	C 4.4	0.4	Α
			-	44	() 4	_

Intersection						
Int Delay, s/veh	4.5					
		MDD	NET	NDD	ODI	OPT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	470	1	4-	404	4
Traffic Vol, veh/h	18	173	569	15	121	193
Future Vol, veh/h	18	173	569	15	121	193
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	19	182	599	16	127	203
Major/Minor I	Minor1	N	Major1		Major2	
Conflicting Flow All	1064	607	0	0	615	0
	607	- 007				
Stage 1	457	-	-	-	-	-
Stage 2			-	-	4 40	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-		2.218	-
Pot Cap-1 Maneuver	247	496	-	-	965	-
Stage 1	544	-	-	-	-	-
Stage 2	638	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	210	496	-	-	965	-
Mov Cap-2 Maneuver	210	-	-	-	-	-
Stage 1	544	-	-	-	-	-
Stage 2	544	-	-	-	-	-
Approach	WB		NB		SB	
					3.6	
HCM Control Delay, s HCM LOS	19.9 C		0		3.0	
HCIVI LUS	U					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	440	965	-
HCM Lane V/C Ratio		-	-	0.457		-
HCM Control Delay (s)		-	-		9.3	0
HCM Lane LOS		-	-	С	Α	A
HCM 95th %tile Q(veh)		-	-	2.3	0.5	-
(1011)						

Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	WEIT	1	HOIL	ODL	4
Traffic Vol, veh/h	10	11	307	9	9	520
Future Vol, veh/h	10	11	307	9	9	520
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_		-	None
Storage Length	0	-	_	-	-	-
Veh in Median Storage		-	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	12	323	9	9	547
				•	•	
NA - 1 /NA1	M		1		M.'. O	
	Minor1		Major1		Major2	
Conflicting Flow All	893	328	0	0	332	0
Stage 1	328	-	-	-	-	-
Stage 2	565	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		-	-	2.218	-
Pot Cap-1 Maneuver	312	713	-	-	1227	-
Stage 1	730	-	-	-	-	-
Stage 2	569	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	309	713	-	-	1227	-
Mov Cap-2 Maneuver	309	-	-	-	-	-
Stage 1	730	-	-	-	-	-
Stage 2	563	-	-	-	-	-
Approach	WB		NB		SB	
	13.6		0		0.1	
HCM Control Delay, s HCM LOS			U		0.1	
HOW LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	439	1227	-
HCM Lane V/C Ratio		-	-	0.05	0.008	-
HCM Control Delay (s)		-	-	13.6	8	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0.2	0	-

Intersection						
Int Delay, s/veh	8.2					
		WDD	NDT	NDD	ODI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	0.40	1€	^-	40.4	4
Traffic Vol, veh/h	20	243	73	25	404	105
Future Vol, veh/h	20	243	73	25	404	105
Conflicting Peds, #/hr	0	0	_ 0	_ 0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	36	4	14	47	10	30
Mvmt Flow	21	256	77	26	425	111
Major/Minor	Minor1		Major1		Major2	
						^
Conflicting Flow All	1051	90	0	0	103	0
Stage 1	90	-	-	-	-	-
Stage 2	961	-	-	-	-	-
Critical Hdwy	6.76	6.24	-	-	4.2	-
Critical Hdwy Stg 1	5.76	-	-	-	-	-
Critical Hdwy Stg 2	5.76	-	-	-	-	-
Follow-up Hdwy	3.824	3.336	-	-	2.29	-
Pot Cap-1 Maneuver	217	962	-	-	1440	-
Stage 1	855	-	-	-	-	-
Stage 2	323	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	149	962	-	-	1440	-
Mov Cap-2 Maneuver	149	-	-	-	-	-
Stage 1	855	-	-	-	-	-
Stage 2	222	-	-	-	-	-
	,					
Approach	WB		NB		SB	
HCM Control Delay, s	13.9		0		6.8	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NRRV	VBLn1	SBL	SBT
	IC .	NDT	- INDIX		1440	- 301
Capacity (veh/h)		-				
HCM Control Dolay (c)		-		0.407		0
HCM Long LOS		-	-		8.5	
HCM Lane LOS	\	-	-	В	A	Α
HCM 95th %tile Q(veh))	-	-	2	1.2	-

Intersection						
Int Delay, s/veh	3.2					
		14/5-5				05-
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	**		1			4
Traffic Vol, veh/h	21	115	294	24	167	487
Future Vol, veh/h	21	115	294	24	167	487
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	5	2	2	15
Mvmt Flow	22	121	309	25	176	513
Majan/Minar	Mineral		1-1-1		Mais =0	
	Minor1		Major1		Major2	
Conflicting Flow All	1187	322	0	0	334	0
Stage 1	322	-	-	-	-	-
Stage 2	865	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	208	719	-	-	1225	-
Stage 1	735	-	-	-	-	-
Stage 2	412	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	166	719	-	-	1225	_
Mov Cap-2 Maneuver	166	-	_	_	-	_
Stage 1	735	-	_	_	-	-
Stage 2	329	_	_	_	_	_
Olago Z	020					
Approach	WB		NB		SB	
HCM Control Delay, s	15.8		0		2.2	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NRPV	VBLn1	SBL	SBT
	IL .	INDI	אאטאו			ODT
Capacity (veh/h)		-	-	475	1225	-
HCM Control Polov (a)		-		0.301		-
HCM Control Delay (s)		-	-	15.8	8.4	0
HCM Lane LOS		-	-	C	A	Α
HCM 95th %tile Q(veh))	-	-	1.3	0.5	-

Appendix F: TDM Supportive Development" Checklist	
3555 Borrisokane Road – Korean Community Church	Page - F -

TDM-Supportive Development Design and Infrastructure Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

The Official Plan or Zoning By-law provides related guidance that must be followed The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance

	TDN	M-supportive design & infrastructure measures: Non-residential developments		Check if completed & descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES		
	1.1	Building location & access points		
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances		
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations		
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort		
	1.2	Facilities for walking & cycling		
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (See Official Plan policy 4.3.3)	X	No current transit provisions are provided in the area of the development
REQUIRED	1.2.21	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (See Official Plan policy 4.3.12)	☑	Sidewalks are provided from the building entrances to Borrisokane Road

	TDN	A-supportive design & infrastructure measures: Non-residential developments		Check if completed & descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (See Official Plan policy 4.3.10)	\square	Concrete slab sidewalks are provided to and from the building entrances
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (See Official Plan policy 4.3.10)	$\overline{\checkmark}$	Sidewalks are easily accessible with gradual grade transition
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (See Official Plan policy 4.3.11)	X	No current public sidewalks or cycling paths are located in the area. The sidewalk from the building entrance connects to Borrisokane Road, which will have sidewalk and cycling facilities in the future.
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	X	No transit provisions
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	X	are available in the area.
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	X	Borrisokane Road has an operating speed of 80 km/h, with no bike lanes
	1.3	Amenities for walking & cycling		
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	$\overline{\checkmark}$	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	V	Wayfinding not required

	TDN	M-supportive design & infrastructure measures: Non-residential developments	add	Check if completed & descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACIL	LITI	ES
	2.1	Bicycle parking		
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (See Official Plan policy 4.3.6)	Ø	Bicycle racks are provided near the main entrance
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well- used areas (See Zoning By-law Section 111)	\square	7 bicycle parking spaces are provided based zoning By-law regulations
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (See Zoning By-law Section 111)	$\overline{\mathbf{V}}$	Bicycle spaces meet minimum dimensions, all are horizontal spaces, racks anchored to concrete
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	V	Expected number of cyclists based on mode share is 1, 7 parking spaces are provided
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	V	The 7 provided stalls provides a massive buffer over the expected number of cyclists
	2.2	Secure bicycle parking		
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (See Zoning By-law Section 111)	V	The City bylaw requires 2 stalls and provision is being made for 7outdoor bicycle parking spaces on racks.
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	X	There is no secure bicycle parking provision in the bylaw for less than 50 stalls.
	2.3	Shower & change facilities		
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	×	
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	X	
	2.4	Bicycle repair station		
BETTER	2.4.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	×	No repair station is provided

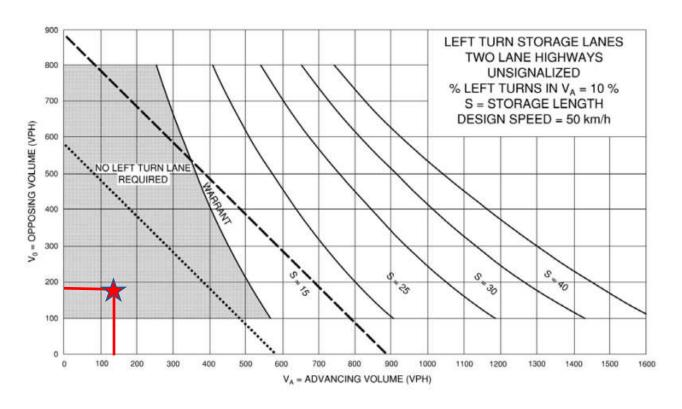
	TDN	M-supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references		
	3.	TRANSIT			
	3.1	Customer amenities			
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	×	There are no on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	×	There are no off-site transit stops abutting the site	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	×	There are no on-site transit stops	
	4.	RIDESHARING			
	4.1	Pick-up & drop-off facilities			
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	X	There is no dedicated area for carpool motorists as stalls are available on a 1 st come basis.	
	4.2	Carpool parking			
BASIC	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools	×	There are no signed parking spaces for carpools	
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement	\boxtimes	There are no signed parking spaces for carpools	
	5.	CARSHARING & BIKESHARING			
	5.1	Carshare parking spaces			
BETTER	5.1.1	Provide carshare parking spaces in permitted non- residential zones, occupying either required or provided parking spaces (See Zoning By-law Section 94)	X	There are no carsharing spaces	
	5.2	Bikeshare station location			
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	X	There are no bike- sharing spaces	

	TDN	M-supportive design & infrastructure measures: Non-residential developments	add	Check if completed & descriptions, explanations or plan/drawing references
	6.	PARKING		
	6.1	Number of parking spaces		
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	\checkmark	Sufficient parking is provided based on zoning By-laws
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	$\overline{\mathbf{V}}$	136 stalls are provided, while only 57 are necessary based on zoning by-laws
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (See Zoning By-law Section 104)	×	Does not apply to site land uses
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (See Zoning By-law Section 111)	X	No reduction has been made
	6.2	Separate long-term & short-term parking areas		
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	X	All parking spaces are shared
	7.	OTHER		
	7.1	On-site amenities to minimize off-site trips		
BETTER	7.1.1	Provide on-site amenities to minimize mid-day or mid- commute errands	X	No amenities are provided in this office building

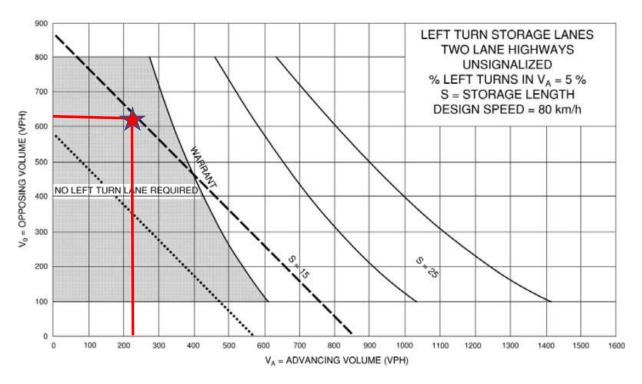
APPENDIX G: MMLOS ANALYSIS WORKSHEET

Roadway Segments Adjacent to the Development								
Performance Measure	Cambrian Road	Flagstaff Drive	Borrisokane Road					
	East of Borrisokane Road	East of Borrisokane Road	b/w Cambrian Road and Flagstaff Drive					
	Pedestrian LC	OS (PLOS)						
Sidewalk Width (m)	No sidewalk	No sidewalk	No sidewalk					
Boulevard Width (m)	N/A	N/A	N/A					
Average Daily Curb Lane Traffic Volume	N/A	N/A	N/A					
Presence of On-Street Parking	N/A	N/A	N/A					
Operating Speed (km/h)	80	60	90					
Posted +10 km/hr	Б	Б	T.					
Segment PLOS Target PLOS	F D	F D	F D					
Target FLOS	Bicycle LOS		D					
Bikeway Type	Mixed Traffic	Mixed Traffic	Mixed Traffic					
Number of Lanes per direction	1	1	1					
Bike Lane Width (m)	N/A	N/A	N/A					
Operating Speed (km/h)	80	60	90					
Posted +10 km/hr	DT/A	NT/A	DT/A					
Bike Lane Blockage Segment BLOS	N/A F	N/A F	N/A F					
Target BLOS	D D	D D	D D					
Target DLOS	Transit LOS		D					
Facility Type	11411511 202	(1200)						
Level/Exposure to Parking/Driveway Friction Average Transit Travel		N/A						
Speed (km/h) Posted Speed Limit (km/h) Segment TLOS								
Target TLOS	Truck LOS (TkLOS)							
Number of lanes (in each direction) Curb Lane Width (m) Segment TkLOS								
Target TkLOS								

APPENDIX H: LEFT TURN LANE WARRANTS

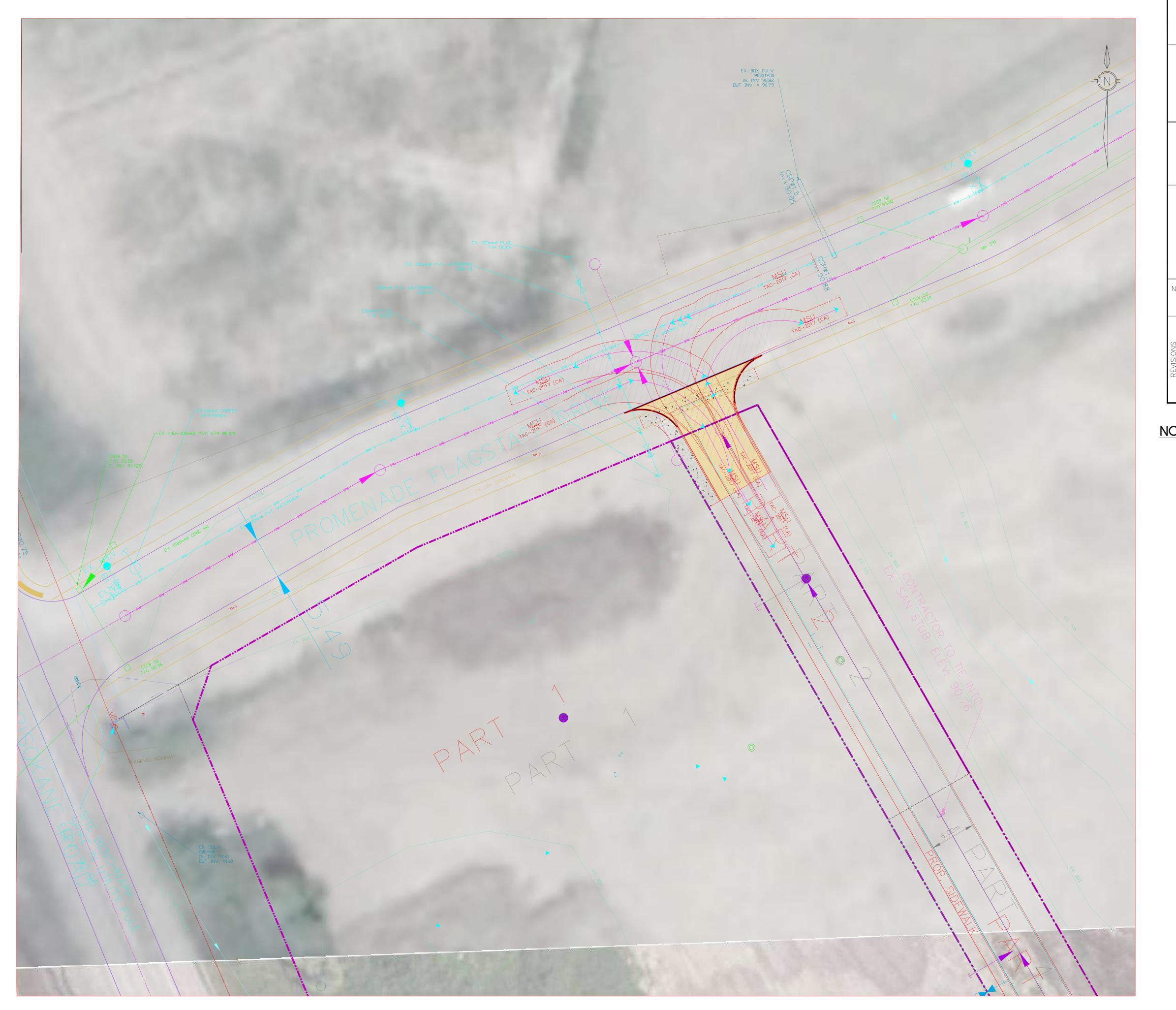


Flagstaff Drive and Halo Car Wash Intersection Results



Borrisokane Road and Site Access Intersection Results

APPENDIX I: TURNING MOVEMENT ANALYSIS



CITY OF OTTAWA FUNCTIONAL PLANNING DESIGN 3555 BORRISOKANE ROAD

ACCESS ON BORRISOKANE ROAD DESIGN VEHICLE TURNING MANEUVERS

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T320 03

Sheet 3 of 5

Asset Group

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RM AEG

Dwn. Chk'd.

RM AK

Utility Circ. No. Index No.

PROJECT REF. NUMBER: 7320

Cost. Inspector

Scale:

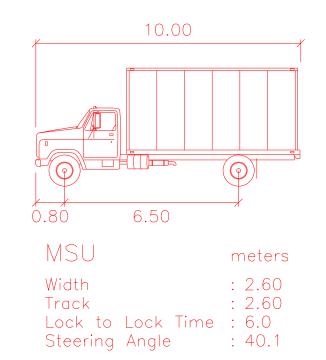
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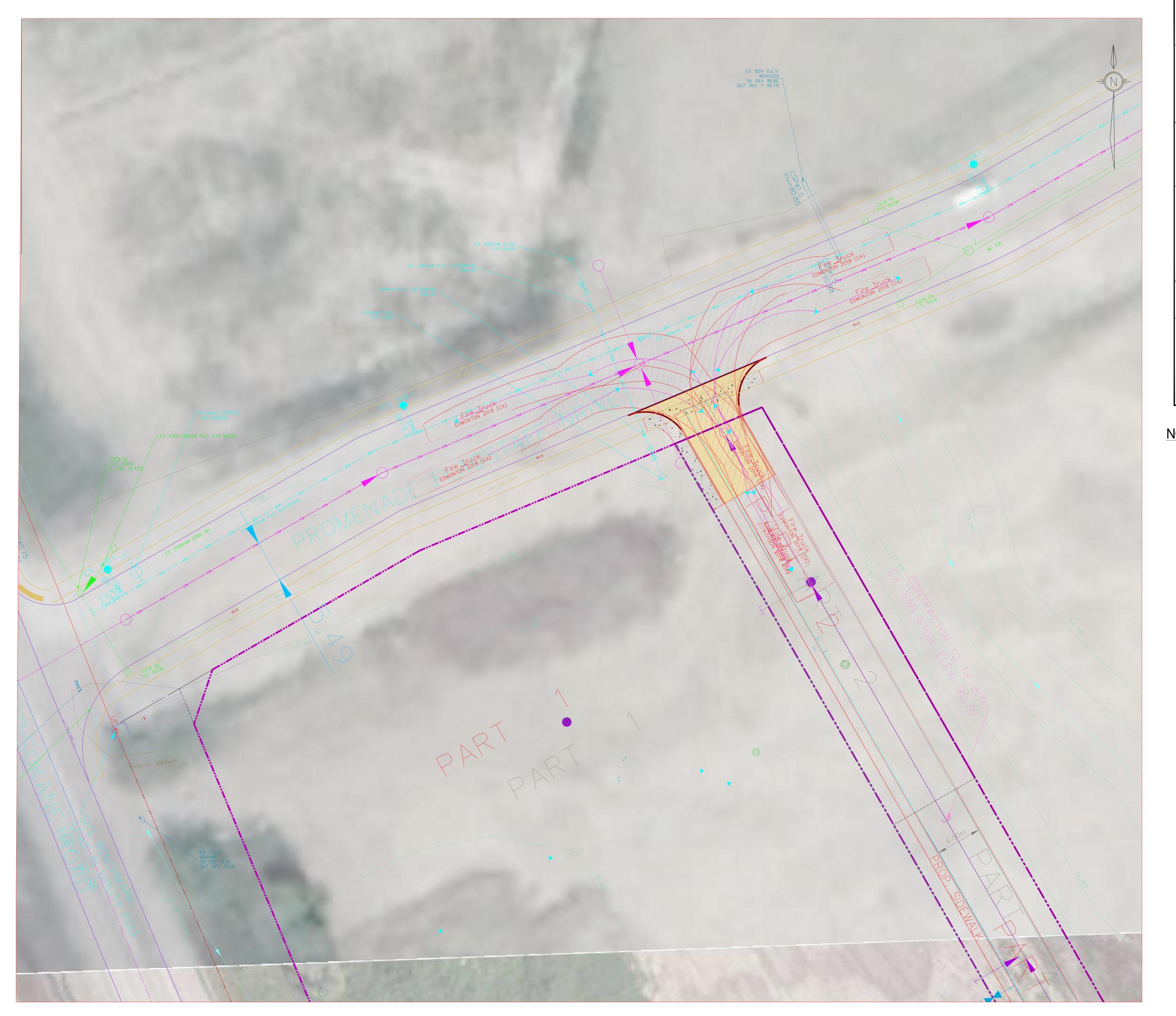
HORIZ 1: 250

NOTE: The location of utilities is approximate only, the exact location should be determined by consulting the municipal authorities and utility companies concerned. The contractor shall prove the location of utilities and shall be responsible for adequate protection from damage.

	No.	Description	Ву	Date (dd/mm/yy)
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CITY OF OTTAWA FUNCTIONAL PLANNING DESIGN 3555 BORRISOKANE ROAD

ACCESS ON BORRISOKANE ROAD DESIGN VEHICLE TURNING MANEUVERS

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Sheet 3 of 5

Asset Group

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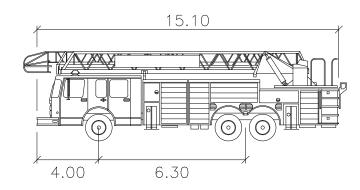
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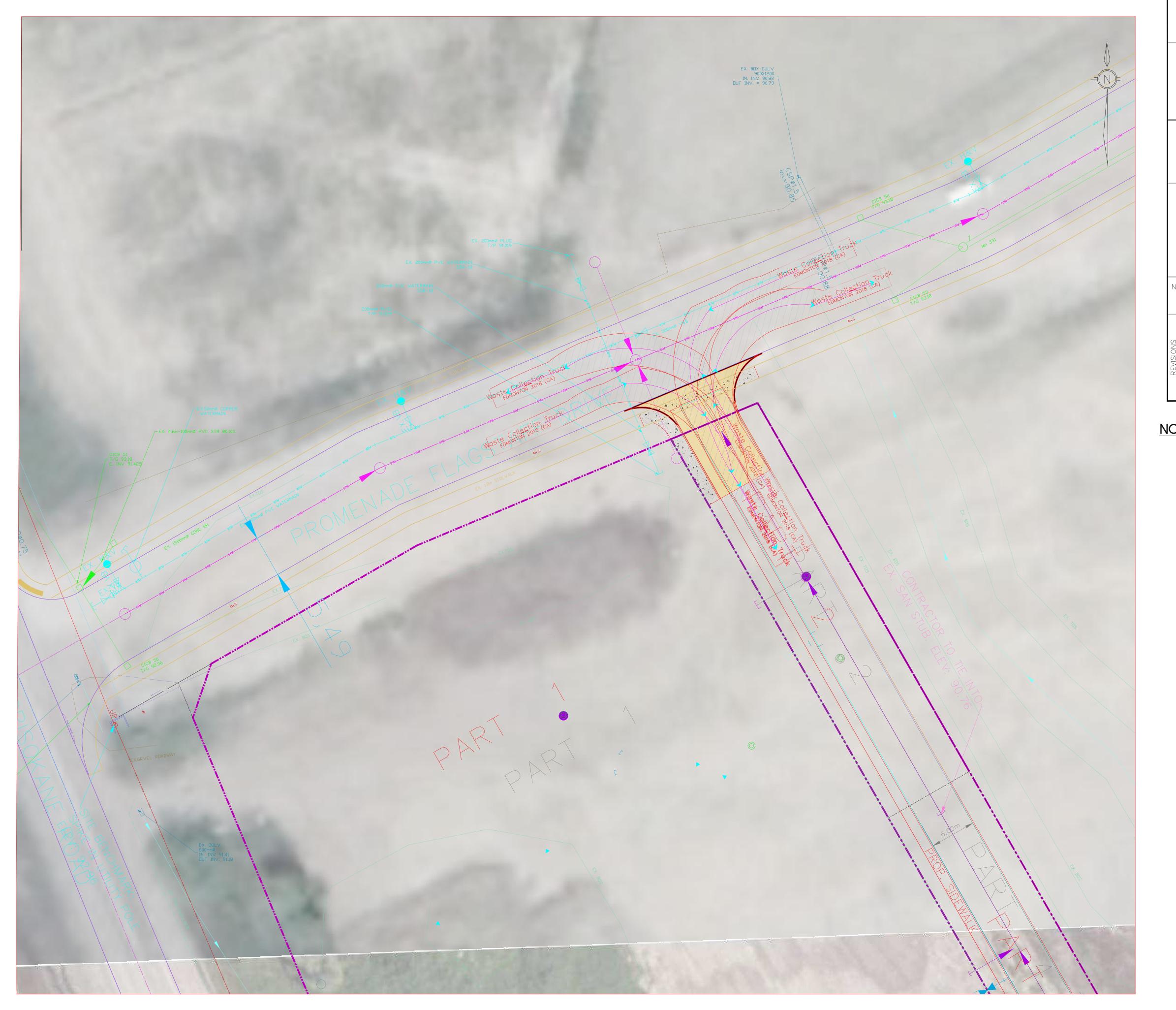
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NOTES



Fire Truck

		m	neters
Width Track Lock to L Steering A		:	2.60 2.60 6.0 32.6



CITY OF OTTAWA FUNCTIONAL PLANNING DESIGN 3555 BORRISOKANE ROAD

ACCESS ON BORRISOKANE ROAD DESIGN VEHICLE TURNING MANEUVERS

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Sheet 3 of 5

Asset Group

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Utility Circ. No. Index No.

PROJECT REF. NUMBER: 7320

Cost. Inspector

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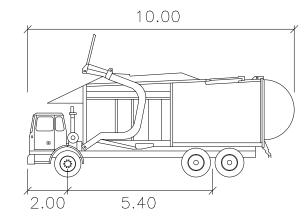
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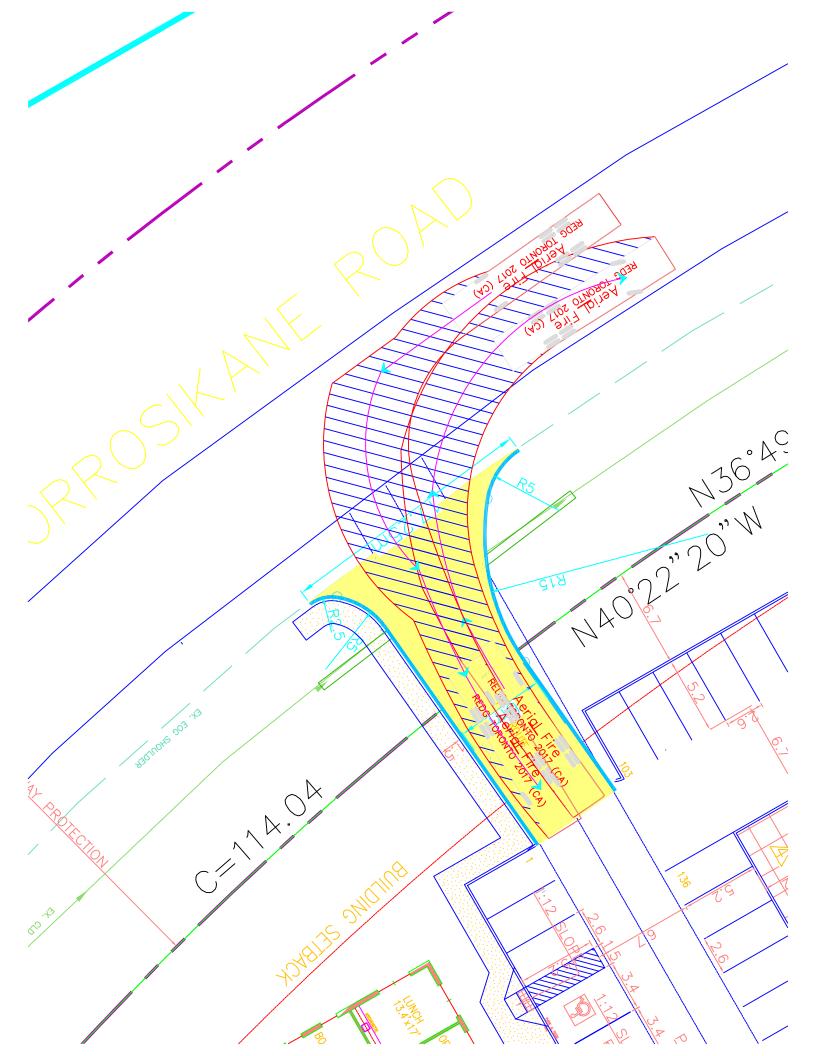
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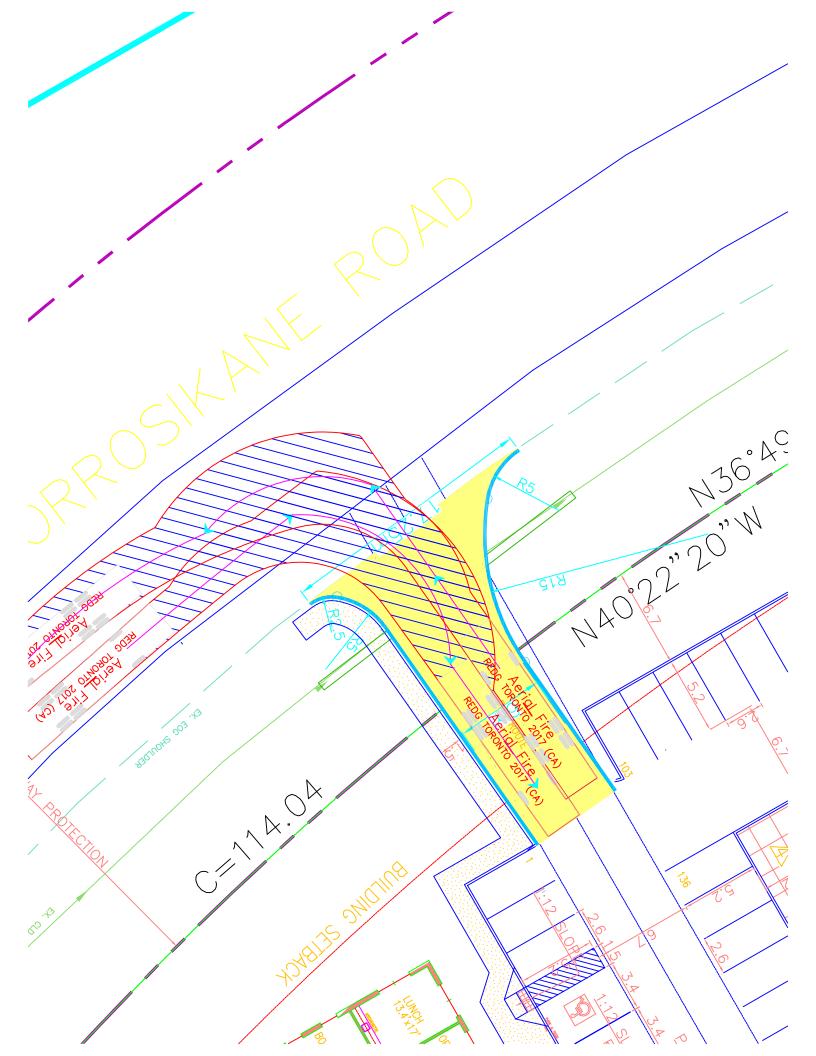
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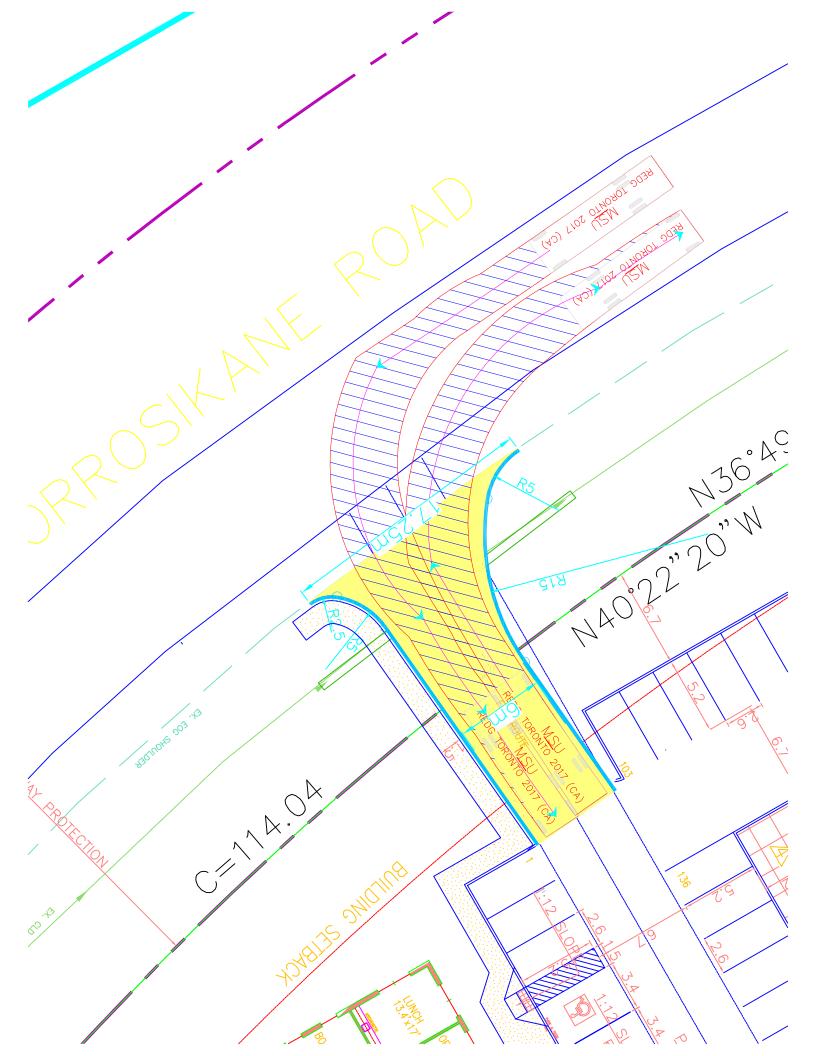


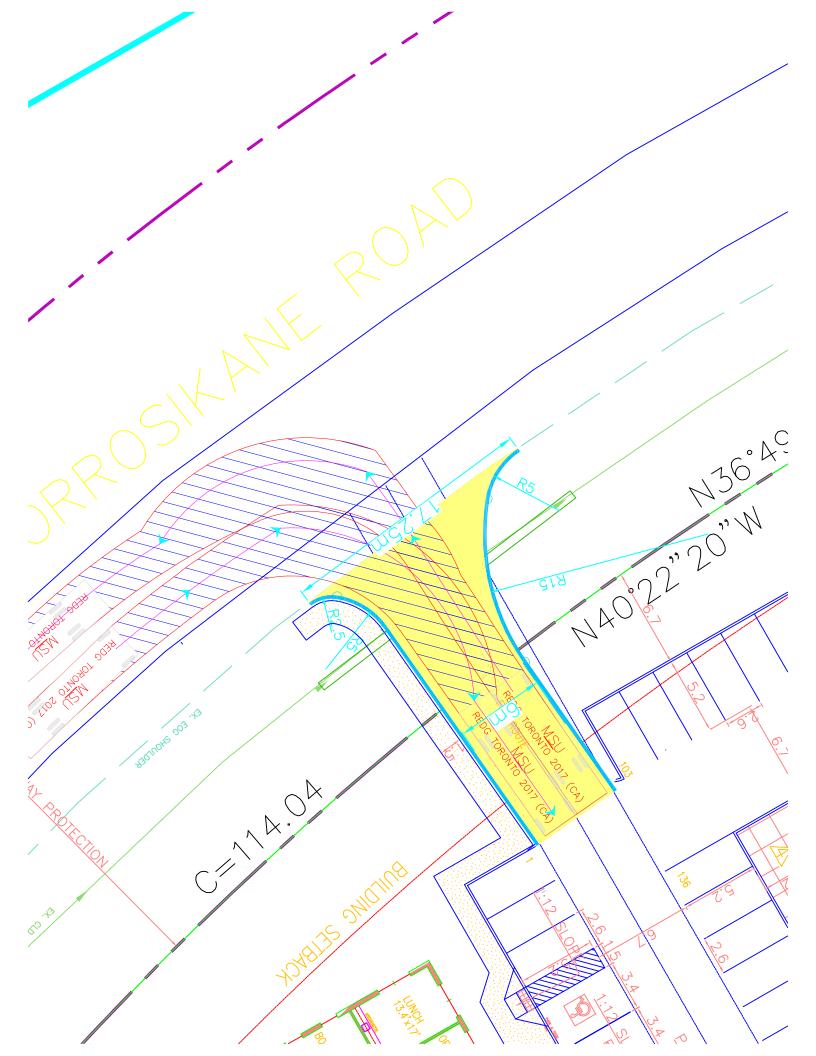
Waste Collection Truck

Width : 2.60
Track : 2.60
Lock to Lock Time : 6.0
Steering Angle : 27.7

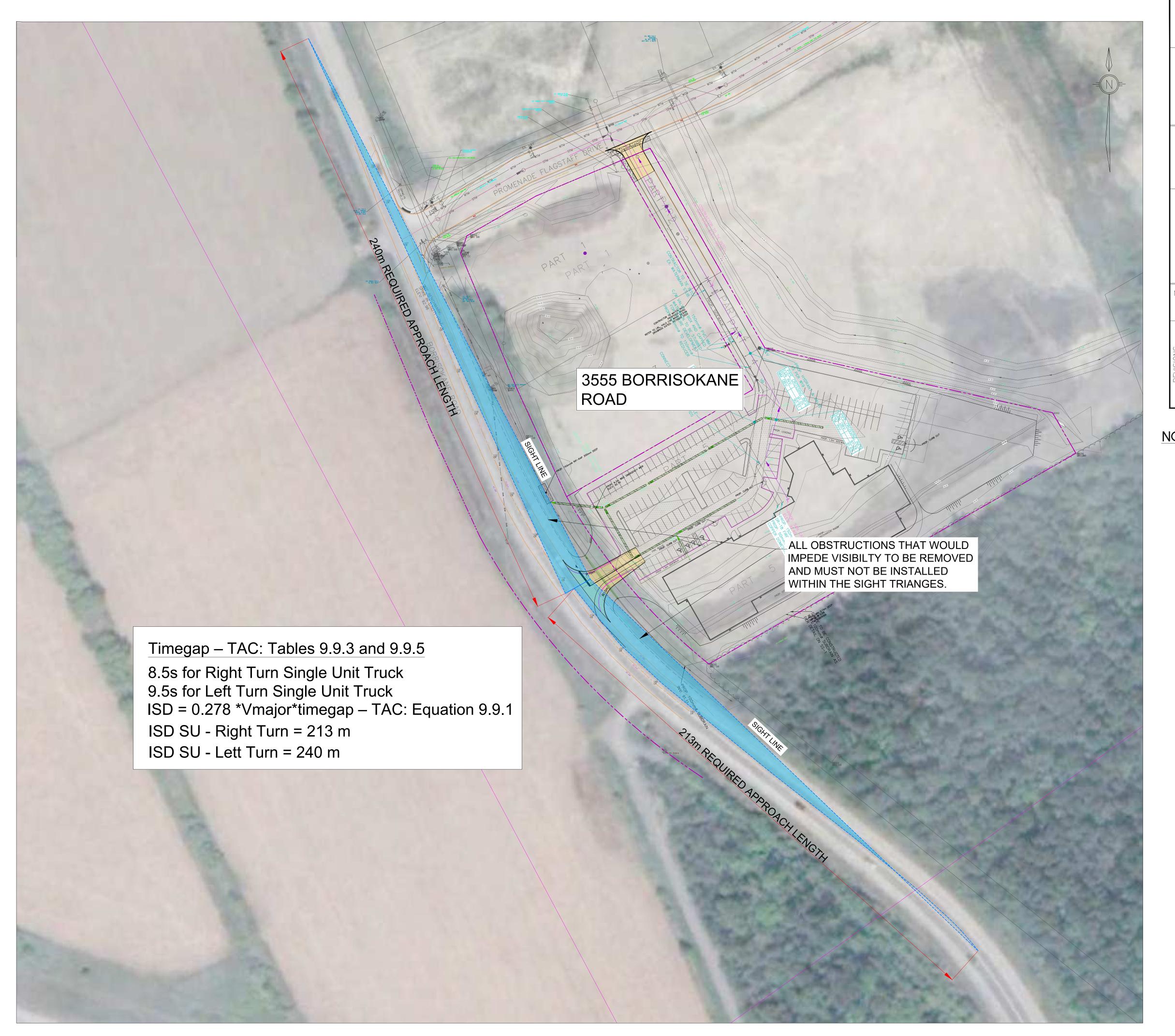


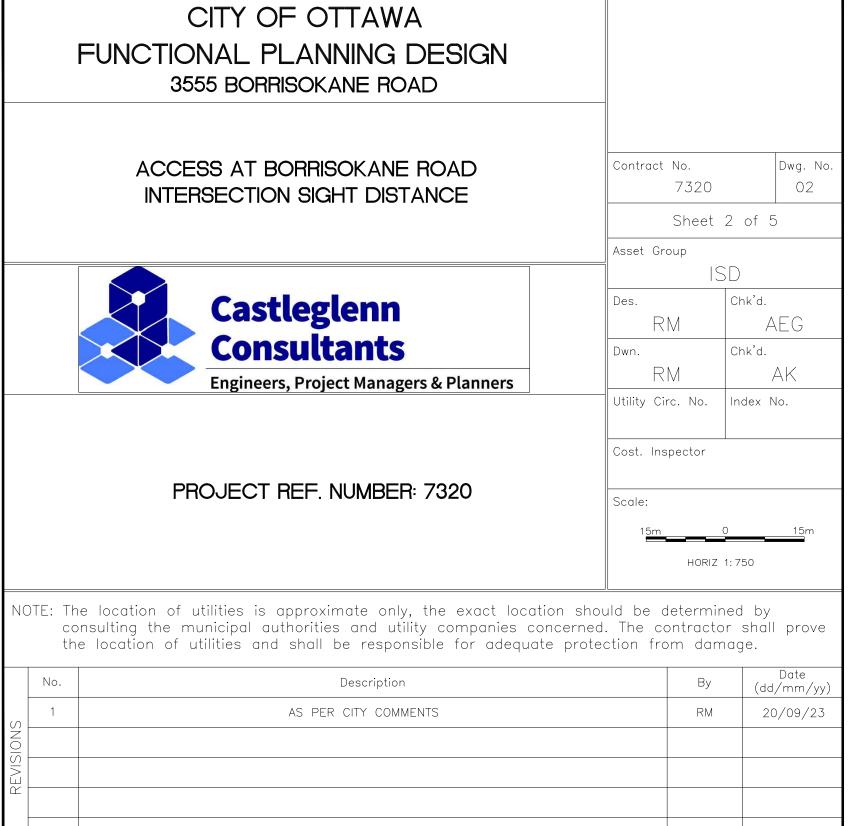






APPENDIX J: SIGHT DISTANCE ANALYSIS





NOTES

APPENDIX K: Access Designs



CITY OF OTTAWA FUNCTIONAL PLANNING DESIGN

3555 BORRISOKANE ROAD

ACCESS ON BORRISOKANE ROAD DESIGN VEHICLE TURNING MANEUVERS

Castleglenn Consultants

Engineers, Project Managers & Planners

PROJECT REF. NUMBER: 7320

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Cost. Inspector	
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HORIZ 1: 250

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Asset Group

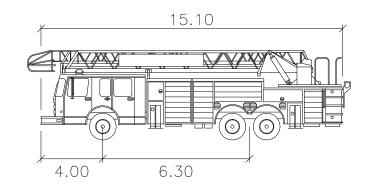
Sheet 3 of 5

ISD

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REVISIONS	No.	Description	Ву	(dd/mm/yy)
	1	AS PER CITY COMMENTS	RM	20/09/23
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	Y			



Fire Truck

meters : 2.60 : 2.60 : 6.0 : 32.6 Width Track Lock to Lock Time Steering Angle