

PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 1
R-PLAN 268160
917 MERIVALE ROAD
CITY OF OTTAWA

SERVICEABILITY REPORT
REPORT No. R-823-102A

T.L. MAK ENGINEERING CONSULTANTS LTD.

AUGUST 2024

REFERENCE FILE NUMBER 823-102

Introduction

The developer of this property is proposing to redevelop the existing residential lot described as Lot 1 Registered Plan 268160 City of Ottawa by constructing a six (6) storey residential apartment building plus a basement consisting of twenty (20)-units, including ten (1)-bedroom units and ten (10) bachelor units.

The municipal address of this property is referenced as 917 Merivale Road and it is located in the City Ward (16 - River). The site is situated on the east side of Merivale Road, south of Crerar Avenue and north of Anna Avenue, see site plan and legal survey plan in Appendix A for details.

The area of this property is ±0.0482 hectares. In addition to the six (6) storey residential building, the other development features will comprise of an interlock paver access to the front entrance plus a concrete sidewalk access along the south side yard to the waste storage and one vehicular entrance asphalt access road, parking areas and an amenity area is also located in the rear yard including landscaped areas along the north side of the site, etc., to meet the City of Ottawa's site plan requirements.

A site geotechnical report was prepared by the owner's soils engineer Yuri Mendez Engineering entitled "Subsurface Investigation Report" 917 Merivale Road (Report No. 63-SPD-R0) dated September 27, 2023 for this proposed development property.

This serviceability report will provide the City of Ottawa with our serviceability brief to address the proposed servicing scheme for this site.

Existing Site Conditions and Servicing

This property is presently occupied by one and one half (1 ½) storey wooden sided residential building. The existing house is located near the front centre on this property with existing gazebo structures located at the rear yard, asphalt surface along the south side yard and gravel plus interlock walkway located along the north side of the property limit which currently provides pedestrian access to the gazebos. For additional details of the site's pre-development conditions, refer to the coloured Google Image (2023) and aerial photography from (GeoOttawa 2022) in Appendix B.

Approximately one half of this site is currently permeable surface covered and consisting of grass/landscaped areas with the remaining areas being roof area, concrete steps, gravel and asphalt surfaces. Presently, most of the landscape areas are concentrated at the rear of lot and at the front yard.

The topography of the land is found to be graded primarily to drain from rear to the front of the lot (east to west). The existing gradient of the property is sloping approximately 1.9% from back to front.

Water and sanitary service lateral currently servicing the existing dwelling on 917 Merivale Road will be removed. The existing water services shall be blanked at the main and the existing house laterals shall be capped at the front property line for re-development of this lot.

As for the availability of underground municipal services, there are existing municipal services along Merivale Road in front of this property consisting of a 450mm diameter storm sewer, a 375mm diameter sanitary sewer, and a 300mm diameter watermain for development of this property. Refer to the City of Ottawa Merivale Road UCC drawing and As-Built plan and profile drawing included in Appendix C for details.

Because the site will be connecting to and outletting into the separated Merivale Road storm sewer located within the Merivale Road road right of way in the City of Ottawa, therefore, the approval exemption under Ontario Regulations 525/98 would apply since storm water discharges from this site will outlet flow into a downstream storm sewer. Thus, an Environmental Compliance Approval (ECA) application will not be required to be submitted to the Ministry.

Proposed Residential Apartment Building Site

A vehicle entrance located at the northwest corner of the lot is proposed to provide vehicular access to this property along with an access roadway from this entranceway to direct vehicular traffic in and out of the site. Vehicular parking will be available at the rear of the site, east of the proposed building.

A. Water Supply

The proposed building located within Pressure Zone 2W at 917 Merivale Road is a 6-storey residential multiunit building with a basement. The building contains twenty (20) total units, ten (10) 1-bedroom, and ten (10) bachelor units. Each floor covers an average area of around 218 m², for a gross floor area of 1,310 m² (excluding the basement).

The building is to be serviced by the 300 mm diameter watermain along Merivale Road. The ground elevation along Merivale Road is approximately 77.9 m.

Demand Projections

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines, where the residential consumption rate of 280 L/cap/d was used to estimate average day

demands (AVDY). Persons per unit (PPU) for each unit were estimated based on the City of Ottawa’s Water Design Guidelines.

Following discussions with the City, peaking factors are to be estimated from Table 3-3 of the MECP Design Guidelines for Drinking-Water Systems, given that the proposed development population is less than 500 people. Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 9.5. Peak hour (PKHR) demands were calculated by multiplying AVDY by a factor of 14.3. **Table 1** shows the estimated domestic demands of the proposed building.

Table 1: Estimated Domestic Demand

Unit Type	Unit Count	PPU	Consumption	AVDY		MXDY		PKHR	
				L/d	L/s	L/d	L/s	L/d	L/s
Apartment, 1-Bedroom	10	1.4	280	3,920	0.05	37,240	0.43	56,056	0.65
Apartment, Bachelor	10	1.4		3,920	0.05	37,240	0.43	56,056	0.65
Total	20			7,840	0.09	74,480	0.86	112,112	1.30

The fire flow required was determined following the Fire Underwriter Survey (FUS) method and is provided in the attached worksheet. The proposed building will be of wood frame construction. It is understood that the building will be equipped with sprinklers, and that the basement is more than 50% below ground level. The resulting required fire flow is 11,000 L/min (183 L/s) for a duration of 2.25 hours.

Details are provided in the attached **Fire Flow Calculations** (See Appendix D). Furthermore, **Figure 1** found in Appendix D provides separation distances for the FUS calculations. The proposed **Site Plan** attached in Appendix D was used to determine distances from the proposed building to the property lines.

In summary, the estimated water demands for the proposed building are as follows:

- AVDY = 7,840 L/d (0.09 L/s)
- MXDY = 74,480 L/d (0.86 L/s);
- PKHR = 112,112 L/d (1.30 L/s); and,
- Fire Flow (FUS) = 11,000 L/min (183 L/s).

Boundary Conditions

The hydraulic gradeline (HGL) boundary conditions for 917 Merivale Road, as presented in **Table 2**, were provided by the City on May 2, 2024 (see attached **Water Boundary Conditions Email** in Appendix D). Note that slight architectural changes to the buildings were made

following the reception of the boundary conditions, resulting in slightly less water demands. As such, the received boundary conditions are considered conservative and applicable).

Table 2: Boundary Conditions

Demand Scenario	Head (m)
Minimum HGL (Peak Hour)	124.4
Maximum HGL (Average Day)	132.8
Available Fire Flow @ Residual 20 psi	118.0

Hydraulic Analysis

Peak Hour & Average Day

During peak hour demands, the resulting minimum hydraulic gradeline of 124.4 m corresponds to a peak hour pressure of 456 kPa (66 psi). This value is above the minimum pressure objective of 276 kPa (40 psi) for residential buildings up to two storeys. Adding 5 psi per floor above two stories, to account for headloss due to elevation and pipe losses, a minimum pressure of 413 kPa (60 psi) would be required for the sixth floor. The peak hour pressure at ground level is above this objective and therefore considered acceptable.

During average day demands, the resulting maximum hydraulic gradeline of 132.8 m corresponds to a maximum pressure of 538 kPa (78 psi). This value is less than the maximum pressure objective of 552 kPa (80 psi) and therefore considered acceptable.

Supporting hydraulic calculations are attached in Appendix D.

Maximum Day + Fire Flow

A maximum day plus fire flow (11,000 L/min) hydraulic gradeline of 118.0 m corresponds to a residual pressure of 393 kPa (57 psi) at this location, which is above the minimal residual pressure requirement of 140 kPa (20 psi).

Based on Table 1 of Appendix I of the City of Ottawa Technical Bulletin ISTB-2018-02 and a desktop review (i.e., Google Street View) to confirm hydrant class, four (4) hydrants are located in the vicinity of the proposed building. One (1) Class AA hydrants is within 75 m, both with a capacity contribution of up to 5,700 L/min. Three (3) other Class AA hydrants are within 150 m from the site, both with a capacity contribution of up to 3,800 L/min. The combined hydrant flow coverage for 917 Merivale Road is therefore 17,100 L/min, which is above the RFF obtained from the FUS (11,000 L/min) method.

The hydrant coverage is illustrated in **Figure 2** attached in Appendix D. A breakdown of the hydrant coverage is summarized in **Table 3** below.

Table 3: Fire Hydrant Coverage

Building	Fire Flow Demand (L/min)	Fire Hydrants					Combined Hydrant Flow Coverage (L/min)
		Hydrant Class	Within 75 m		Between 76 m and 150 m		
			Quantity	Contribution to RFF	Quantity	Contribution to RFF	
917 Merivale Road	11,000	AA	1	5,700	3	3,800	17,100
		A					
		B					
		C					

Conclusion

In conclusion, based on the boundary condition provided, 300 mm diameter watermain along Merivale Road provides adequate fire flow capacity, as per the Fire Underwriters Survey (FUS) method, to the proposed development 917 Merivale Road. Resulting pressures during anticipated demand flows meet the pressure objectives during average and peak demand conditions, as per the City of Ottawa’s Drinking Water Design Guidelines.

B. Sanitary Flow

The peak sanitary flow for the 20 units, which comprise of ten (1)-bedroom and ten (10) bachelor apartment units, is estimated at $Q = 0.34$ L/s with an infiltration rate of 0.02 L/s. Refer to Appendix E sheet 1 of 1 regarding sanitary flow calculations. This flow will enter the existing 375 mm diameter sanitary sewer on Merivale Road via the proposed 150 mm diameter PVC sanitary service lateral from the six (6)-storey residential apartment building.

The existing peak sanitary flow of the site for single detached dwelling unit is $Q = 0.06$ L/s with an infiltration rate of 0.02 L/s. The net increase in flow from this proposed development is 0.28 L/s which is not expected to negatively impact the existing 375 mm dia. sanitary sewer.

Waste water from this site outlets into the Merivale Road 375 mm dia. sanitary sewer then continues to outlet north along Merivale Road into the existing downstream 1050 mm dia. concrete sanitary collector sewer located north of Carling Avenue which further direct sewage flow northward crossing Island Park Drive into the Geneva Street 1050 mm dia. sanitary collector sewer.

C. Storm Flow

Stormwater outlet for this proposed development property will be the existing 450 mm dia. storm sewer located on Merivale Road. The proposed residential apartment building has a rooftop that is partially flat and will be able to provide on-site stormwater management (SWM)

storage. Roof water from the building will be drained and controlled by two (2) roof drains which then outlets directly into the existing Merivale Road 450 mm dia. storm sewer via the proposed 300 mm dia. PVC storm sewer from the site.

On-site drainage shall be graded and drained into (2) catch basin manholes where they are interconnected by oversized underground 600 mm dia. storm sewer stormwater flow here will be controlled by the specified ICD located in CB/MH#1 to a rate of 6.0 L/s (min.) permitted by the City.

The building foundation weeping tile drainage system shall have its own separate gravity flow pipe where weeping tile water is outletted via a 150 mm dia. storm pipe to the existing Merivale Road storm sewer. The stormwater outlet for the rooftop water from roof drains will also be a separately designated proposed 150 mm dia. PVC pipe that will also be outletted into the existing Merivale Road storm sewer.

Two (2) roof drains are proposed for this residential building to restrict flow to a rate of 2.84 L/s (1.26 L/s + 1.58 L/s) into the existing municipal storm sewer.

Based on the residential site plan from the owner's architect, the average post-development runoff coefficient is estimated at $C = 0.82$ and $A = 0.0482$ hectares.

An estimation of the pre-development flow condition was carried out using the criteria accepted by the City of Ottawa. If post-development C value exceeds the lesser of the $C_{pre} = 0.59$ then SWM is required. Because $C_{post} = 0.82$ for this site exceeds $C_{pre} = 0.59$ then SWM measures are required.

Therefore, based on our calculation, on-site retention is required for this proposed residential development site, because the site post-development C value of 0.82 is greater than the $C_{pre} = 0.59$.

The storage volume for the five (5)-Year and up to the 100-Year storm event attenuation will be stored by means of flat rooftop of the proposed residential building and by the oversized underground storm drainage pipe and structures on-site. Also refer to the site storm drainage report (Report No. R-823-102) for further details.

Conclusion

To develop this proposed residential site (± 0.0482 ha. in size) and in controlling the 5-Year stormwater release rate off-site to an allowable rate of 5.17 L/s, a calculated site storage volume of approximately 5.26 m^3 (min.) is required during the 5-Year event. We estimate that the required storage volume is 1.69 m^3 (min.) from rooftop storage and 3.57 m^3 (min.) from the

site underground drainage system are necessary to attenuate the 5-Year storm event. Refer to the Storm Drainage Report (Report No. R-823-102) for details.

During the 5-Year storm event for the flat rooftop storage, the ponding depth of rooftop is estimated at 120 mm at Roof Drain No. 1 and Roof Drain No. 2 and 0 mm at the roof perimeter, assuming a 2.0% (min.) roof pitch to the roof drains. The rooftop storage available at Roof Area No. 1 is 1.17 m³ and Roof Area No. 2 is 1.38 m³, for a total of 2.55 m³, which is greater than the required volume of 1.69 m³.

As for the remaining storage volume of 3.57 m³ (min.) required from the site development area for the 5-Year storm event, the estimated H.W.L. of 76.48 m will provide a total available underground storage volume of 4.0 m³ consisting of the proposed underground storm piping and drainage structures. In total, the 5-Year available site storage volume is approximately 4.0 m³ which is greater than the required site storage volume of 3.57 m³. See Appendix E for details.

In order to control the 100-Year stormwater release rate off-site to an allowable rate of 5.17 L/s, a calculated site storage volume of approximately 13.37 m³ (min.) is required during the 100-Year event. We estimate that the required storage volume of 4.65 m³ (min.) of rooftop storage and 8.72 m³ (min.) from the site underground drainage system are necessary to attenuate the 5-Year storm event. See Table 4 to 6 inclusive

During the 100-year storm event for the flat rooftop storage, the ponding depth on this rooftop is estimated at 150 mm at Roof Drain No. 1 and Roof Drain No. 2 and 0 mm at the roof perimeter assuming a 2.0% (min.) roof pitch to the drains. The rooftop storage available at Roof Area No. 1 is 2.35 m³ and Roof Area No. 2 is 2.69 m³ for a total of 5.04 m³ which is greater than the required volume of 4.65 m³.

As for the remaining storage volume of 8.72 m³ (min.) required from the site development area for the 100-Year storm event, the estimated H.W.L. of 77.18 m will provide a total available underground storage volume of 8.90 m³ consisting of the proposed underground storm piping and drainage structures. In total, the 100-Year available site storage volume is approximately 8.90 m³ which is greater than the required site storage volume of 8.72 m³. See Appendix E for details.

Therefore, by means of flat building rooftop storage, grading the site to the proposed grades and constructing the proposed underground storm piping and drainage system as shown on the Proposed Site Grading and Servicing Plan (Dwg. No. 823-102, G-1) the desirable 5-Year and 100-Year storm event attenuation volume of 4.0 m³ and 8.90 m³ respectively will be available on-site.

In order to control the release flow rate off-site from the controlled drainage area of the lot, an inlet control device (ICD) will be installed at the outlet of CB/MH#1 in the 300 mm diameter storm pipe (outlet pipe) with $Q = 6.0$ L/s under a head of 1.0 m. A rooftop drain with a maximum release rate of 1.26 L/s will be installed at Roof Drain No. 1 and a maximum release rate of 1.58 L/s will be installed at Roof Drain No. 2 under a head of 150 mm at the proposed apartment building flat rooftop as depicted on (Dwg. No. 823-102, G-1).

A specified inlet control device (ICD) will be installed at the outlet of CB/MH#1 in the 300 mm diameter storm pipe (outlet pipe) with $Q = 6.0$ L/s under a head of 1.0 m. The ICD type recommended is a Hydrovex Regulator (75-VHV-1) or equivalent. The specified ICD at 6.0 L/s is the lowest flow ICD permitted by the City of Ottawa for maintenance purposes.

The building weeping tile drainage will outlet via its separate 150mm diameter PVC storm lateral. The roof drains will be outletted also via a separate 150mm PVC storm lateral from the apartment building whereupon both laterals “wye” into the proposed 300 mm dia. storm pipe due to one (1) connection to the existing Merivale Road storm sewer is permitted. Therefore, both storm laterals and the site storm sewer all outlets to the existing Merivale Road 450 mm diameter storm sewer with only one (1) storm pipe connection. The City of Ottawa recommends that pressurized drain pipe material be used in the building for the roof drain leader pipe in the event of surcharging on the City storm sewer system. Refer to the proposed site grading and servicing plan Dwg. 823-102 G-1 for details.

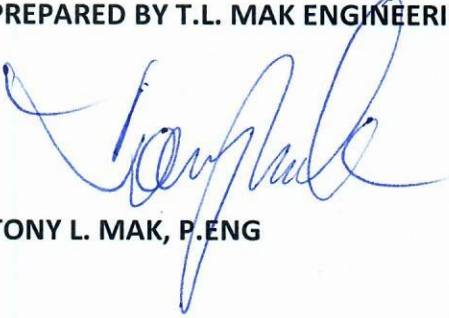
To achieve a minimum of 80 percent TSS removal, a Stormceptor structure (Model EFO-4) is proposed to be installed for the site development of this property. This Stormceptor structure shall be located downstream of the proposed CB/MH#1, which houses the site’s inlet control device (ICD). Based on the Stormceptor system that is proposed for this site, size of the lot, and impervious ratio, a greater than 80 percent TSS removal is estimated for all rainfall events including large storms. (See Appendix D of the Storm Drainage Report {Report No. R-823-102} for details).

Erosion and Sediment Control

The contractor shall implement Best Management Practices to provide for protection of the receiving storm sewer during construction activities. These practices are required to ensure no sediment and/or associated pollutants are released to the receiving watercourse. These practices include installation of a “siltsack” catch basin sediment control device or equal in catch basins as recommended by manufacturer on-site and off-site within the Merivale Road road right of way adjacent to this property. Siltsack shall be inspected every 2 to 3 weeks and after major storm. The deposits will be disposed of as per the requirements of the contract. See Dwg. #823-102 ESC-1 for details.

Refer to Appendix F for the summary of the Development Servicing Study Checklist that is applicable to this development.

PREPARED BY T.L. MAK ENGINEERING CONSULTANTS LTD.



TONY L. MAK, P.ENG



PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 1
R-PLAN 268160
917 MERIVALE ROAD
CITY OF OTTAWA

APPENDIX A
SITE PLAN
AND
LEGAL SURVEY PLAN

ZONING REQUIREMENTS - TM (2196)

EXISTING ZONING: TM(2196) Traditional Main Street Zone
 PROPOSED ZONING: TM(2196) Traditional Main Street Zone

	PARKING REQUIRED	PROVIDED	COMPLIES Y/N
OFF-STREET PARKING	4	4	Y
VEHICLE PARKING	1	1	Y
BARBER FIRE PARKING	6	6	Y
BICYCLE PARKING	10	22 (STANDARD)	Y

	FRONT YARD	REAR YARD	SIDE YARD
FRONT YARD	3M	3M	3M
FRONT YARD MIN. DIM.	3M	3M	3M
REAR YARD	3M	3M	3M
REAR YARD MIN. DIM.	3M	3M	3M
SIDE YARD	3M	3M	3M
SIDE YARD MIN. DIM.	3M	3M	3M
MINIMUM LOT WIDTH	13.2M	13.2M	13.2M
BUILDING COVERAGE AREA	NA	33.5%	Y
TOTAL SPA	NA	1.5M SPA	Y
MINIMUM SPACE AREA	2.5M (MIN. 1.5M) (MIN. 1.5M)	2.5M (MIN. 1.5M) (MIN. 1.5M)	Y
TOTAL NUMBER OF UNITS	NA	05	Y
MIN. LANDSCAPE BUFFER	0 - NOT APPLICABLE	NA	Y

SITE DATA

ITEM	AREA	% TOTAL
LOT AREA	4613 SM (103 SFT)	100%
BUILDING AREA	243 SM (5252 SFT)	52.7%
GROSS FLOOR AREA	1341 SM (14,434 SFT)	29%
ASPHALT/CONC. AREA	1643 SM (1,813 SFT)	14.5%
LANDSCAPED AREA	REAR LOT AREA: 79 SM (792 SFT)	

UNIT DATA

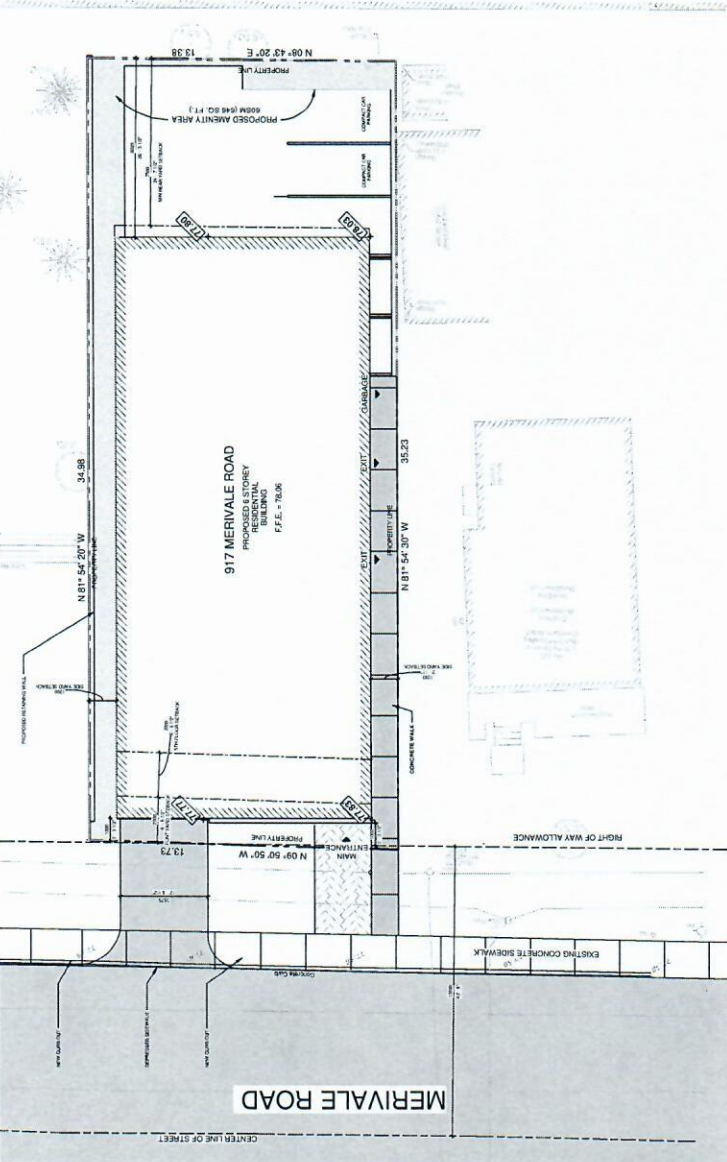
LEVEL	STUDIO	1 BED	2 BED	TOTAL	FLOOR AREA
BASEMENT	0	0	0	0	119 SM (1290 SFT)
MAIN FLOOR	0	0	0	0	103 SM (1116 SFT)
2ND FLOOR	2	2	0	4	243 SM (2622 SFT)
3RD FLOOR	2	2	0	4	243 SM (2622 SFT)
4TH FLOOR	2	2	0	4	243 SM (2622 SFT)
5TH FLOOR	2	2	0	4	243 SM (2622 SFT)
TOTAL	10	10	0	20	1341 SM (14,434 SFT)

PARKING COUNT

USAGE	COUNT	PARKING RATIO	PARKING REQUIRED (PROVIDED)	COMPLIES (Y/N)
STUDIOS	10	1.0	0	Y
1 BED	10	1.0	4	Y
2 BED	0	0.0	0	Y
BF SPOTS	0	0.0	0	Y
VISITOR	0	0.0	1	Y
TOTAL	20	2.0	5	Y

ESTABLISHED GRADE ELEVATION DATA

BUILDING CORNER	ELEVATION	CALCULATION
NORTH WEST CORNER	77.77	77.77 + 77.80 = 76.03
NORTH EAST CORNER	77.80	+ 77.83
SOUTH EAST CORNER	76.03	= 311.43 / 4
SOUTH WEST CORNER	77.83	= 77.8575
AVERAGE GRADE ELEVATION		AVG ELEV. = 77.86



1. PROPOSED SITE PLAN
 1/8" = 1' - 0"

NO.	DATE	BY	DESCRIPTION
1.	2024.08.28	DS	ISSUED FOR PERMITS
2.	2024.08.28	DS	ISSUED FOR PERMITS

NO.	DATE	BY	DESCRIPTION
1.	2024.08.28	DS	ISSUED FOR PERMITS
2.	2024.08.28	DS	ISSUED FOR PERMITS



BIOSIS DESIGNS
 E: info@biosisdesigns.com
 W: 613.259.9642

MERVALE APARTMENT
 917 MERVALE RD
 OTTAWA ON
 BY: MARC AMYOT

2301 As indicated
 DS, SH
 SH
 REVIEWED

PROPOSED SITE PLAN
A101

PLEASE NOTE:
 THE GRADIENTS AND ELEVATIONS OF THE GROUND AND THE PROPOSED BUILDING ARE FOR INFORMATION ONLY. THE CLIENT IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AUTHORITIES. THE CLIENT IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AUTHORITIES. THE CLIENT IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AUTHORITIES.

SURVEYOR'S REAL PROPERTY REPORT
PART 1 Plan of
LOT 1
REGISTERED PLAN 268160
CITY OF OTTAWA

Surveyed by Annis, O'Sullivan, Vollebek Ltd.
 Scale 1:150
 Date: June 7, 2023

Surveyor's Certificate

I, the undersigned, being duly qualified and sworn as a Surveyor under the Survey Act and the Regulations made under thereunder, do hereby certify that the above described plan is a true and correct copy of the original plan as shown to me by the Surveyor's Office of the City of Ottawa.

Annis, O'Sullivan, Vollebek Ltd.
 Chartered Surveyors

PART 2
REVISIONS TO THE PLAN
DATE

NO. 1
 DATE: 07/20/2023
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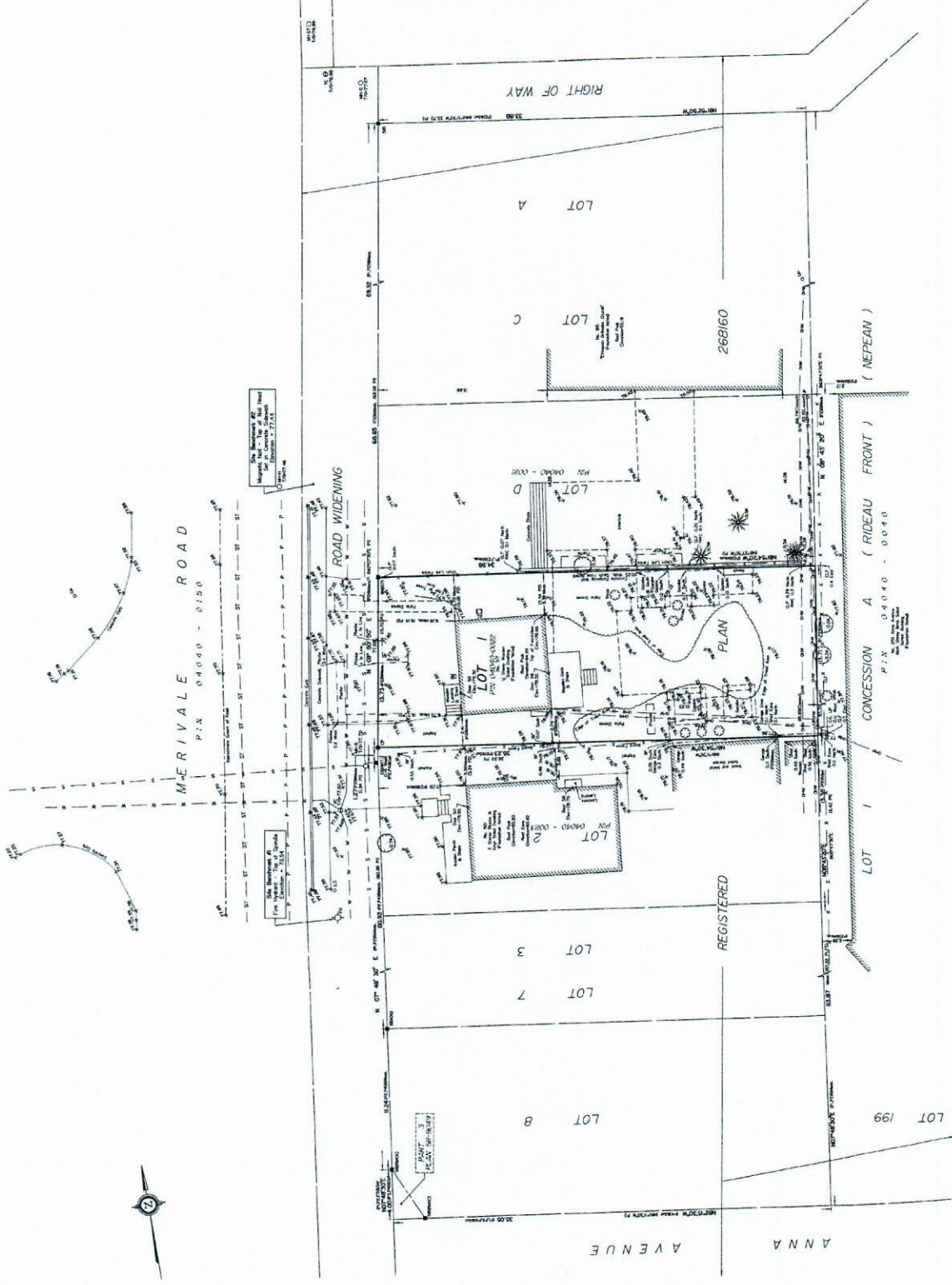
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NO. 18
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NO. 19
 DATE: 07/20/2023
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NO. 20
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 REASON: [Text]



Notes & Legend

- Survey Measurement (Plotted)
- Standard Line (Horizontal)
- Short Standard Line (Horizontal)
- Concrete Pile
- Wellhead
- Manhole
- Valve Chamber (Plotted)
- Valve Chamber (Not Plotted)
- Underground Water
- Underground Utility
- Overhead Water
- Overhead Utility
- Fire Hydrant
- Meter Pit
- Light Standard
- AC Condenser
- Gas Meter
- Gas Valve
- Gate
- Disturbance Tree
- Condition Tree
- Diameter
- Location of Easements
- Location of Right of Way
- Location of Top of Wall Extension

ELEVATION NOTES

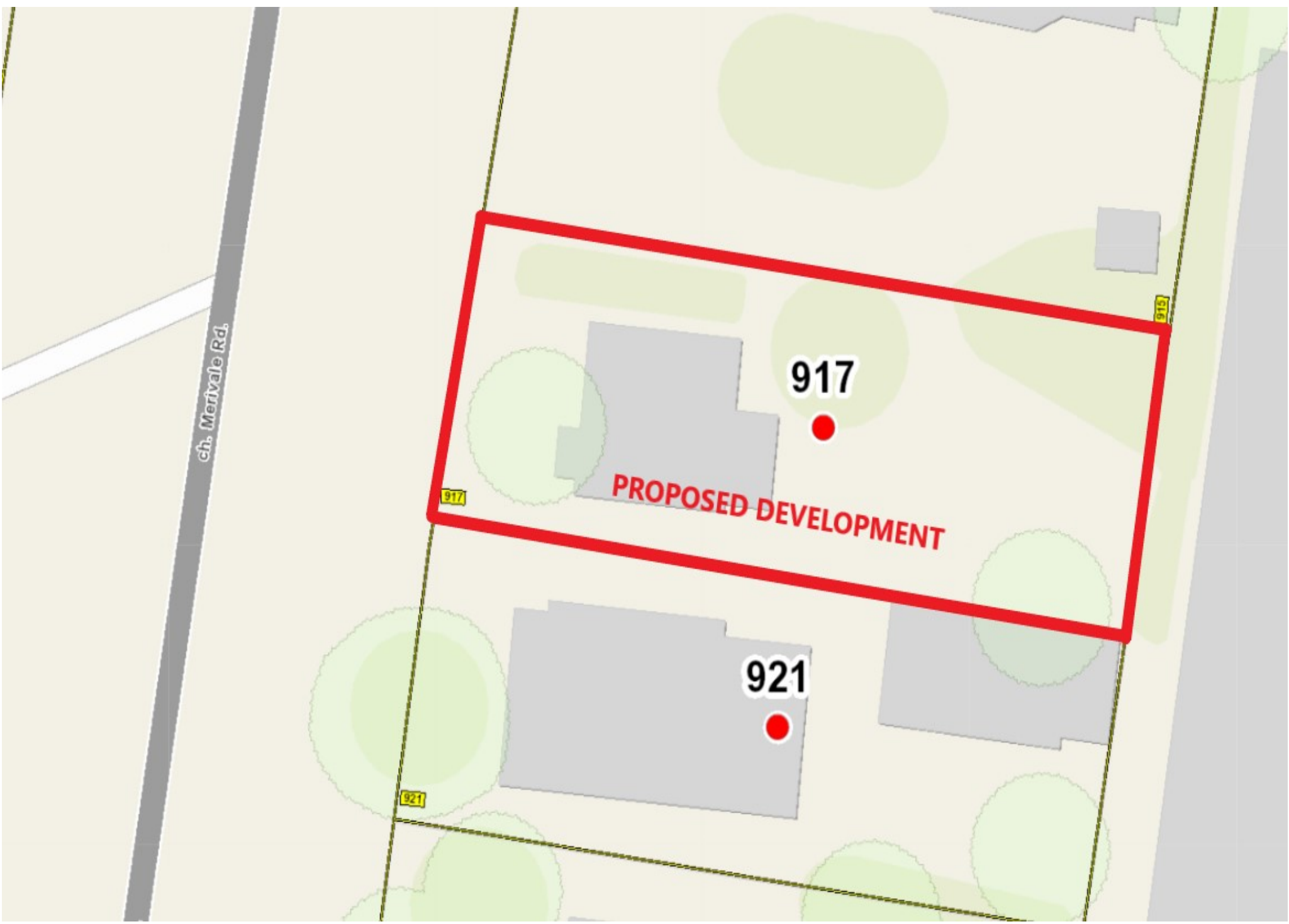
1. Elevations are in meters and are referred to the CVD2013 geoid.
2. Elevation of the top of the wall extension is 100.00 m.
3. Elevation of the top of the wall extension is 100.00 m.
4. Elevation of the top of the wall extension is 100.00 m.

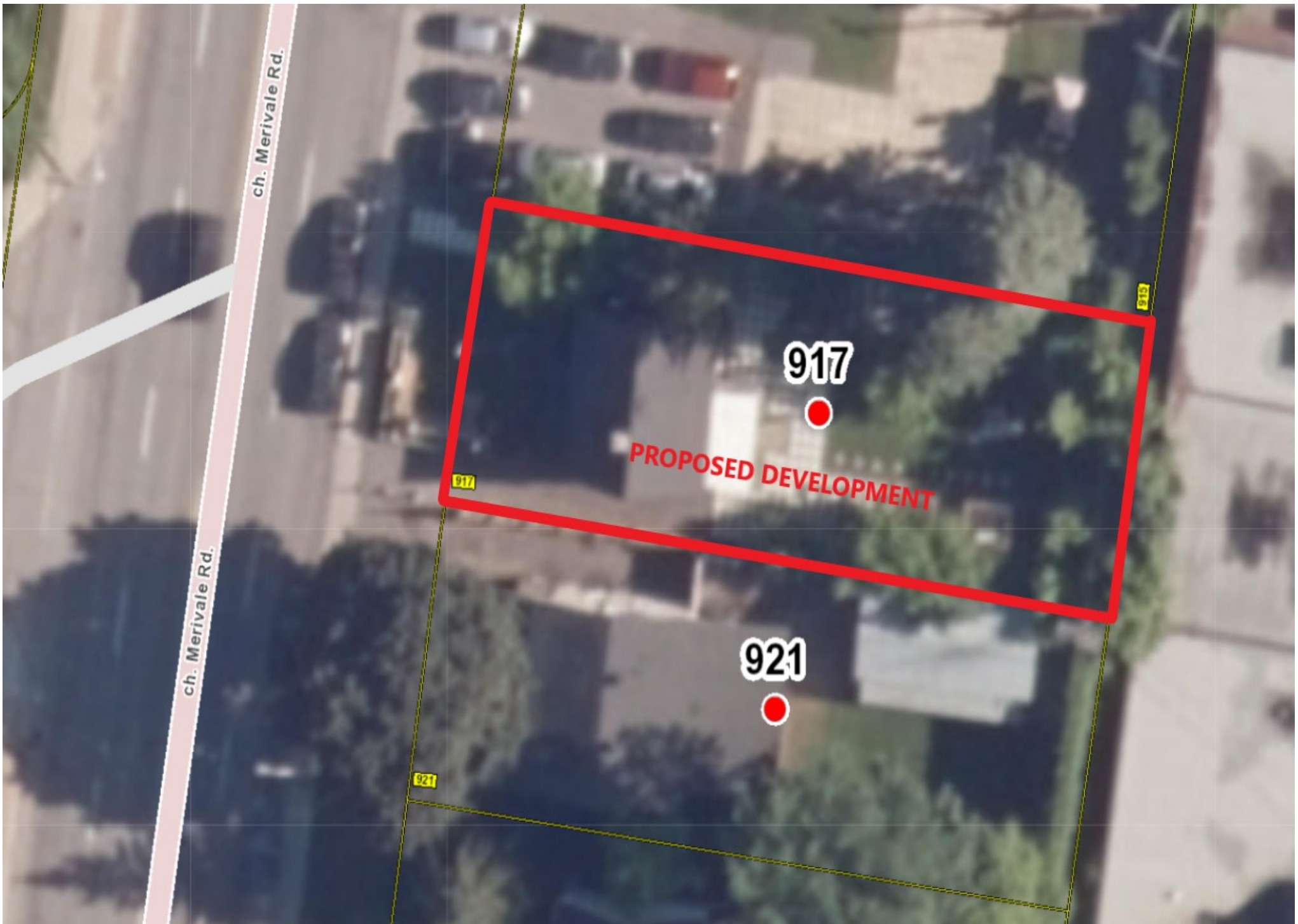
ANNIS, O'SULLIVAN, VOLLEBEK LTD.
 Chartered Surveyors
 1000 Avenue Road, Suite 1000
 Ottawa, Ontario K1R 7V7
 Phone: 613-277-2800 Fax: 613-277-1079

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**APPENDIX B
SITE PRE-DEVELOPMENT CONDITION
GOOGLE IMAGE (2023)
AND
AERIAL PHOTOGRAPHY 2022 (GEOOTTAWA)**



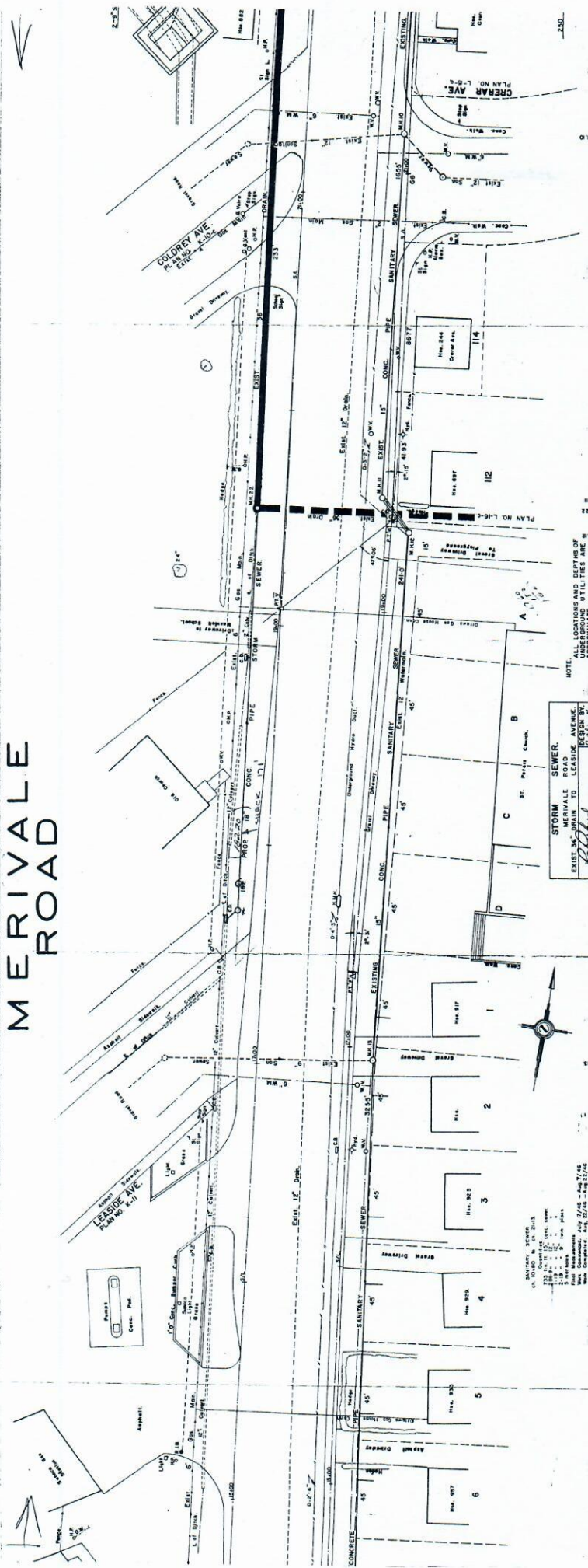




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LOT 1
R-PLAN 268160
917 MERIVALE ROAD
CITY OF OTTAWA**

**APPENDIX C
MERIVALE ROAD
CITY OF OTTAWA
PLAN AND PROFILE
AND
UCC DRAWINGS**

MERIVALE ROAD



STORM SEWER
 MERIVALE ROAD
 EXIST. S.W. TO LESLIE AVENUE BY
 12" DIA. ST. PIPE
 12" DIA. ST. PIPE
 12" DIA. ST. PIPE

NOTE: ALL LOCATIONS AND DEPTHS OF UNDERGROUND UTILITIES ARE TO BE VERIFIED BY THE CONTRACTOR. CONSTRUCTION IS REQUESTED WITHIN THE SPECIFIED COURTESY.

STATION	VERT. CURVE DATA	GRADE	DEPTH	PIPE SIZE	PIPE MATERIAL	CONCRETE	MANHOLE	INVERT	OUTLET	REMARKS
15+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
15+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
16+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
16+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
17+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
17+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
18+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
18+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
19+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
19+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
20+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
20+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
21+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
21+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
22+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
22+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
23+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
23+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
24+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
24+50		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	
25+00		1.3%	1.00	12"	CONC.	12"	1.00	1.00	1.00	

695

DATE: 12/15/50
 BY: [Signature]
 CHECKED: [Signature]

SCALE: 1" = 40'

12.5%
 12.5%
 12.5%

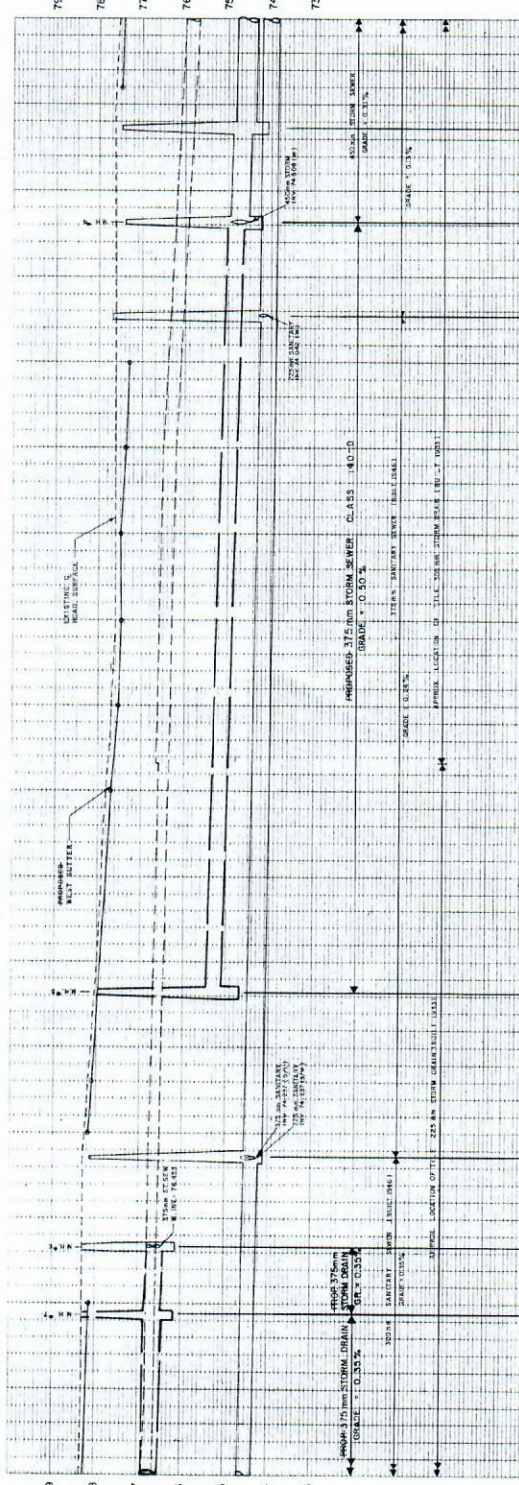
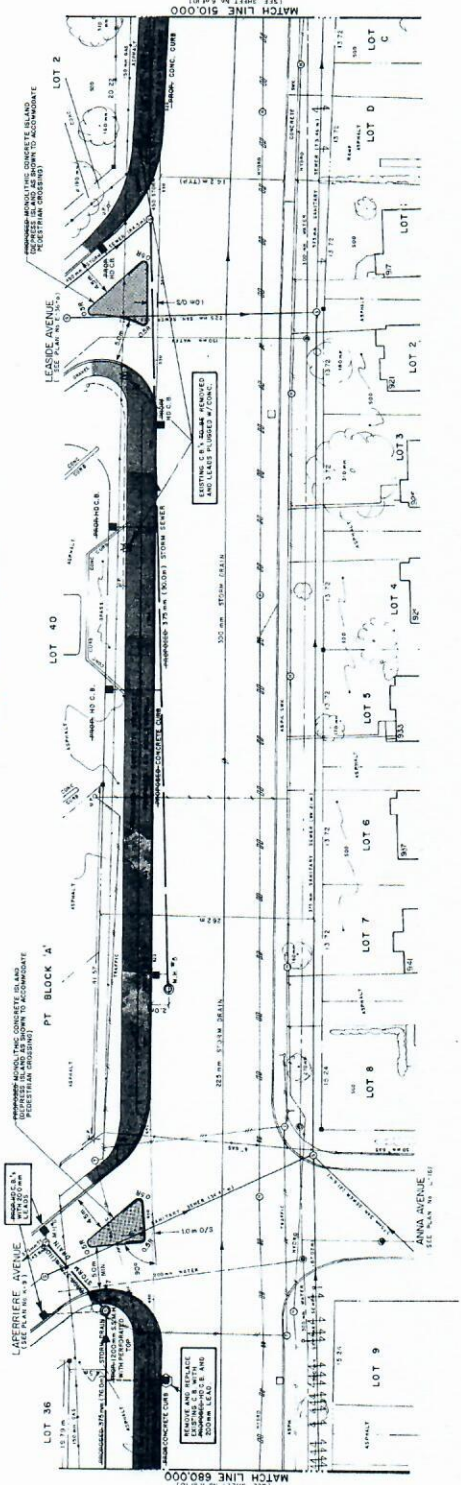
12.5%
 12.5%
 12.5%

2268 MERIVALE ROAD (7 of 10)

2700 SLAVY

* ALSO SEE 2268

MERIVALE ROAD



Station	Elevation Surface	Proposed Sewer (Elev.)	Proposed Storm Sewer (Elev.)
58000	78.22	78.22	78.22
58010	78.22	78.22	78.22
58020	78.22	78.22	78.22
58030	78.22	78.22	78.22
58040	78.22	78.22	78.22
58050	78.22	78.22	78.22
58060	78.22	78.22	78.22
58070	78.22	78.22	78.22
58080	78.22	78.22	78.22
58090	78.22	78.22	78.22
58100	78.22	78.22	78.22
58110	78.22	78.22	78.22
58120	78.22	78.22	78.22
58130	78.22	78.22	78.22
58140	78.22	78.22	78.22
58150	78.22	78.22	78.22
58160	78.22	78.22	78.22
58170	78.22	78.22	78.22
58180	78.22	78.22	78.22
58190	78.22	78.22	78.22
58200	78.22	78.22	78.22
58210	78.22	78.22	78.22
58220	78.22	78.22	78.22
58230	78.22	78.22	78.22
58240	78.22	78.22	78.22
58250	78.22	78.22	78.22
58260	78.22	78.22	78.22
58270	78.22	78.22	78.22
58280	78.22	78.22	78.22
58290	78.22	78.22	78.22
58300	78.22	78.22	78.22
58310	78.22	78.22	78.22
58320	78.22	78.22	78.22
58330	78.22	78.22	78.22
58340	78.22	78.22	78.22
58350	78.22	78.22	78.22
58360	78.22	78.22	78.22
58370	78.22	78.22	78.22
58380	78.22	78.22	78.22
58390	78.22	78.22	78.22
58400	78.22	78.22	78.22
58410	78.22	78.22	78.22
58420	78.22	78.22	78.22
58430	78.22	78.22	78.22
58440	78.22	78.22	78.22
58450	78.22	78.22	78.22
58460	78.22	78.22	78.22
58470	78.22	78.22	78.22
58480	78.22	78.22	78.22
58490	78.22	78.22	78.22
58500	78.22	78.22	78.22

REVISIONS:

NO.	DATE	DESCRIPTION	DRAWN BY	APPROVED BY
1				

FIELD MEASUREMENTS:

Contractor: **BLAKE ASSOCIATES INC.** (SEE PLAN FOR ADDRESS)
 Date: **10/21/14**
 Surveyed by: **M. J. COLE**
 Checked by: **M. J. COLE**

NOTES:

1. All elevations are taken from last available records. Contractor is responsible for verifying all elevations before digging.

2. All elevations shown in this drawing are based on the datum of mean sea level.

3. Proposed storm and sanitary sewers may be constructed in a trench.

4. All elevations shown in this drawing are based on the datum of mean sea level.

5. All elevations shown in this drawing are based on the datum of mean sea level.

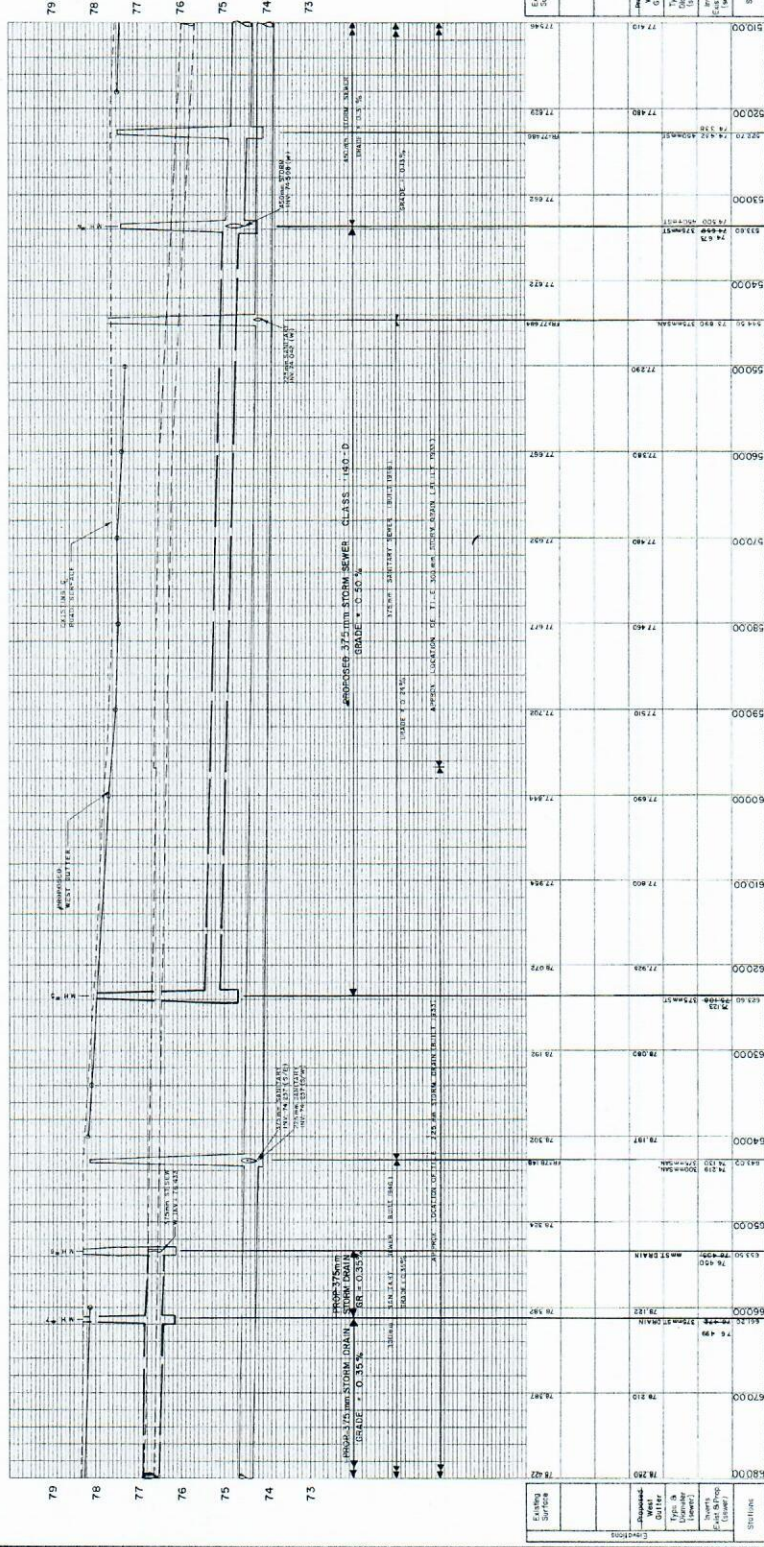
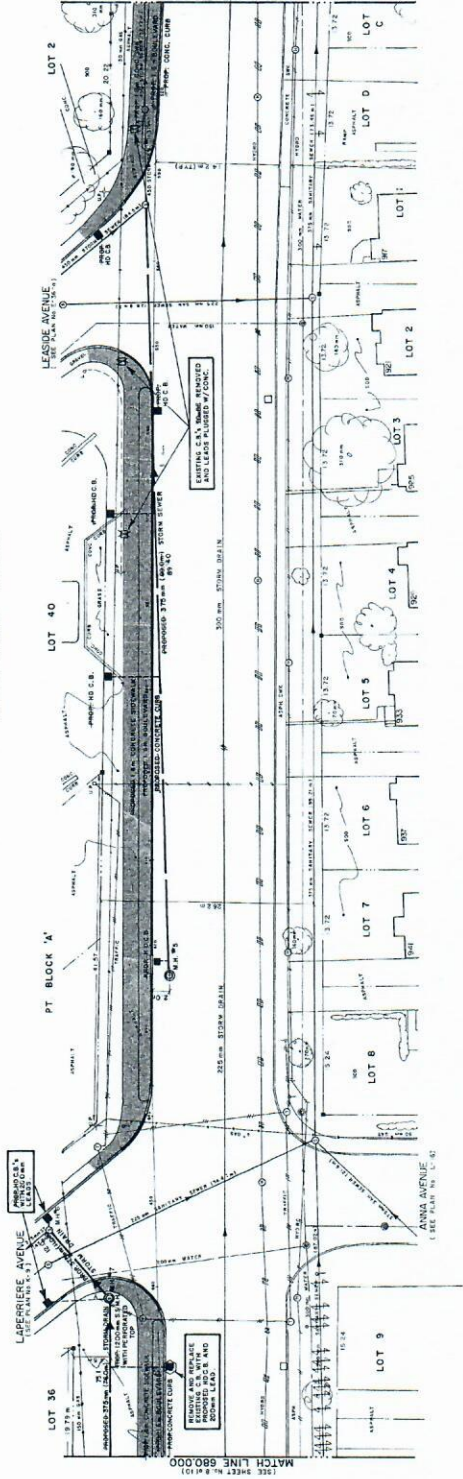
City of Ottawa
 Department of Engineering And Works
 Engineering Branch
 Design And Construction Division

Author: **D. G. Curry**
 Date: **10/21/14**
 Checked by: **W. R. COLE**

PROJ. NO. 2268
VERT. 50
2700SER

NOTE:
ALSO SEE PLAN # 2700

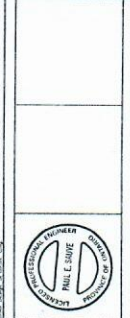
MERIVALE ROAD



Revisions:

No.	Date	Description	Drawn By	App'd By

Final Measurements:
 Surveyed by: J. BRADLEY
 Checked by: J. BRADLEY
 Date: APR 27/16



Notes:
 1. This plan is based on field notes and other data furnished to the Engineer by the contractor.
 2. The contractor is responsible for obtaining all necessary permits and approvals from the appropriate authorities.
 3. The contractor is responsible for ensuring that all work is done in accordance with the applicable codes and standards.
 4. The contractor is responsible for ensuring that all work is done in a safe and sound manner.

City of Ottawa
 Department Of Engineering And Works
 Design And Construction Division
 400 Laurier Street - Ottawa, Ontario K1P 1B7
 D.G. Curry P.Eng. W.R. Cole P.Eng.

Contract No.	92P2268	Sheet No.	2268
Project No.	NCR 1220	Sheet No.	VERTICAL
MERIVALE ROAD			

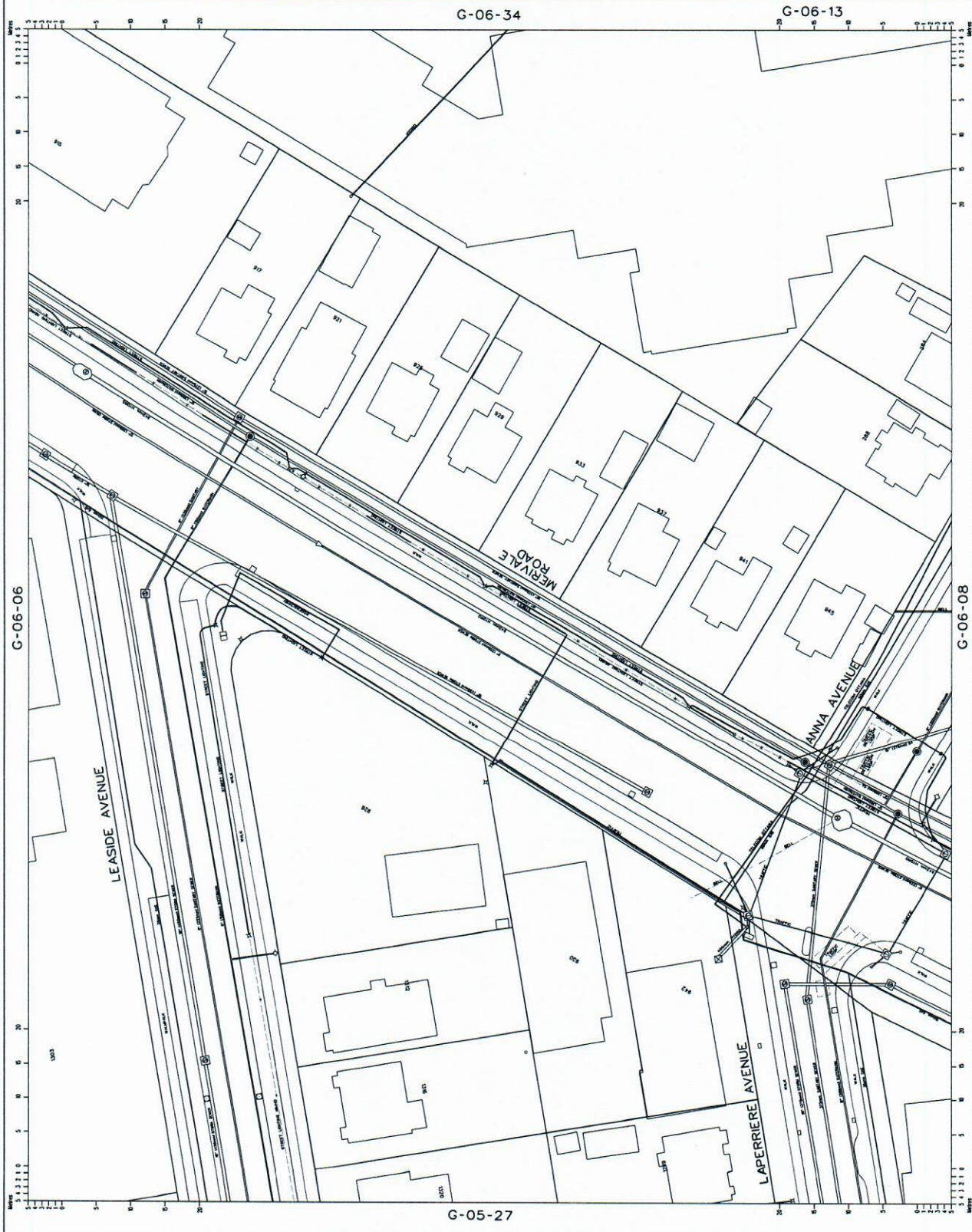
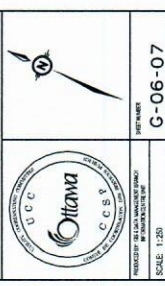
REVISIONS / REVISIONS	DATE	BY
REVISED FOR 2019 BUDGET / 2019	AUG 2019	CM
ADDED 15' WIDE TRAFFIC & BIWAY STRIP /	MAY 2009	LZ
TRAFFIC SIGNAL	NOV 2008	LZ
CONTRACT MODIFICATION ON MERVALE /	JUL 2017	JM
PROPOSED 15' WIDE TRAFFIC BIWAY STRIP /	AUG 2012	CM
MODIFIED FROM 15' WIDE TRAFFIC BIWAY STRIP /	NOV 2012	PM

LEGEND	
Water Main, Utility Corridor, Fire Hydrant	●
Street Address, Catch Basin Manhole	○
Catch Basin (Existing, With Tank, New) (M)	○
Flow Supply, Power, Potential, Transformer, Tower, Regulator	○
Any, Hand Hole, Valve, Gas Valve	○
CC Transformer, Sub-Transformer, Transformer, Substation	○
Transformer Pad, Box, Core, Switch, Enc, Sub	○
Tank, Covered Box, Transformer Enc, S.L., Enclosed	○
Reel (Light Tower) Hole, Flat Light Cabinet	○
Source: Meter Hole, Metering Point	○
Receptor	○
Prop. Dist. Corridor, Limit	○
Abandoned	○
Copyright	○
Priority Lane	○
Hand Post	○

TELECOM GLOSSARY	
A	Abandon
AT	At-Risk
B	Bid
C	Contract
CT	Contractor
H	Hand Post
L	Limit
M	Manhole
P	Proposed
R	Receptor
S	Source
T	Transformer
U	Utility
V	Valve
Z	Zone

GLOSSARY - OTHER	
CC	Contractor
CT	Contractor
H	Hand Post
L	Limit
M	Manhole
P	Proposed
R	Receptor
S	Source
T	Transformer
U	Utility
V	Valve
Z	Zone

CAUTION/ATTENTION
 Always refer to the drawings for details.
 This drawing is for informational purposes only.
 It is not intended to be used for construction or other purposes.
 It is not intended to be used for legal purposes.
 It is not intended to be used for insurance purposes.
 It is not intended to be used for any other purposes.



C-06-06

C-06-34

C-06-13

C-06-08

SCALE: 1:250
 SHEET NUMBER: G-06-07

PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 1
R-PLAN 268160
917 MERIVALE ROAD
CITY OF OTTAWA

APPENDIX D
CITY OF OTTAWA

- **SITE PLAN AND ARCHITECTURAL DRAWINGS**
- **WATER BOUNDARY CONDITIONS E-MAIL**
- **FUS FIRE FLOW CALCULATION**
- **FUS EXPOSURE DISTANCES – FIGURE 1**
- **SUPPORTING HYDRAULIC CALCULATIONS**
- **HYDRANT SPACING – FIGURE 2**

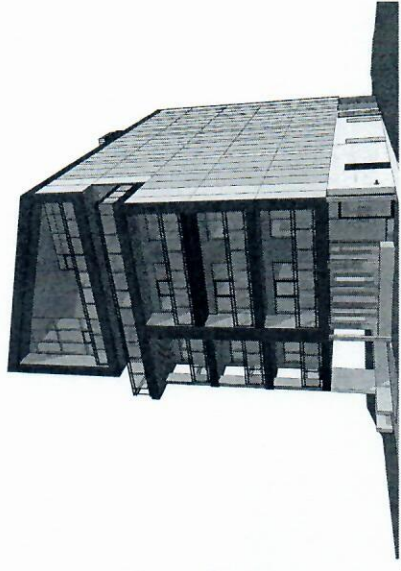
ATTACHMENT 1 : SITE PLAN AND ARCHITECTURAL DRAWINGS



NO.	DATE	BY	DESCRIPTION
1	22.03.2024	DS	ISSUED FOR IFA

NO.	DATE	BY	DESCRIPTION
1	22.03.2024	DS	ISSUED FOR IFA

MARC AMYOT | 917 MERIVALE ROAD, OTTAWA, ON



PROJECT DESCRIPTION:
6 storey apartment building with basement storage

Project No. 2301
Date Issued For 24.05.17
Internal Review

- 1. ARCHITECT: Marc Amyot Inc., 917 Merivale Road, Ottawa, ON K1H 8M2, Canada. Phone: (613) 837-8642, Email: info@marc amyot.com
- 2. STRUCTURAL ENGINEER: CH2M Hill, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@ch2m.com
- 3. MECHANICAL ENGINEER: Mott MacDonald, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@mottmac.com
- 4. ELECTRICAL ENGINEER: Mott MacDonald, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@mottmac.com
- 5. CONTRACTOR: Mott MacDonald, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@mottmac.com
- 6. OWNER: Mott MacDonald, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@mottmac.com
- 7. PROJECT MANAGER: Mott MacDonald, 1000 Bank Street, Ottawa, ON K1P 6L7, Canada. Phone: (613) 837-8642, Email: info@mottmac.com

NO.	DATE	BY	DESCRIPTION
1	22.03.2024	DS	ISSUED FOR IFA

NO.	DATE	BY	DESCRIPTION
1	22.03.2024	DS	ISSUED FOR IFA



BIOSIS DESIGNS
E: info@biosisdesigns.com
P: 613-837-8642

MERIVALE APARTMENT
917 MERIVALE RD
OTTAWA, ON
MARC AMYOT

2301 1:1
PROJECT SCALE
DS, SH
DATE
SH
REVISED

COVER PAGE

A001

PLEASE NOTE:
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ARCHITECTURAL PARTITION ASSEMBLIES

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

ARCHITECTURAL WALL ASSEMBLIES

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

ARCHITECTURAL FLOOR ASSEMBLIES

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

ARCHITECTURAL ROOF ASSEMBLIES

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS



REVISION RECORD

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

ISSUE RECORD

NO.	DATE	BY	DESCRIPTION
1	2011.05.10	DS	ISSUE FOR PERMITS

ARCHITECTURAL ASSEMBLIES
1:20

BIOSIS DESIGNS

E: info@biosisdesigns.com
P: 613-529-9642

MERVILLE APARTMENT
917 MERVILLE RD
OTTAWA ON
M5C 1A7

MARC AMYOT

2301 1:20
PROJECT SCALE
DS, SH
DATE
SH
REVISED

ARCHITECTURAL ASSEMBLIES

A005

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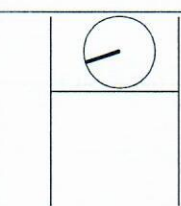


REVISION RECORD

NO.	DATE	BY	DESCRIPTION
1	24.11.2024	SH	ISSUED FOR PERMITS

ISSUE RECORD

NO.	DATE	BY	DESCRIPTION
1	24.11.2024	SH	ISSUED FOR PERMITS



B
BIOISIS DESIGNS
 E: info@biosisdesigns.com
 P: 613-929-8432

MERVILLE APARTMENT
 817 MERVILLE RD
 OTTAWA ON
 M4C 1V8

MARC AMYOT

2301 As Indicated
 DS / SH
 SH
 PROJECT

LIFE & SAFETY PLAN

A006

PLANNED PERMITS
 THIS PLAN IS A PRELIMINARY DESIGN AND IS NOT TO BE USED FOR CONSTRUCTION. THE DESIGNER ACCEPTS NO LIABILITY FOR ANY ERRORS OR OMISSIONS. THE CLIENT IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS. THE DESIGNER'S LIABILITY IS LIMITED TO THE DESIGN SERVICES PROVIDED. THIS PLAN IS TO BE USED IN CONJUNCTION WITH THE CONTRACT DOCUMENTS.

2012 ONTARIO BUILDING CODE - DATA MATRIX

NO.	ITEM	DESCRIPTION	CODE REFERENCE
1	PERMITS DESCRIPTION	<input checked="" type="checkbox"/> NEW GARAGE <input checked="" type="checkbox"/> ADDITION <input checked="" type="checkbox"/> CHANGE OF USE <input type="checkbox"/> OTHER	3.2.2.00 A, 3.1.1.01, 3.1.1.02
2	MAJOR COMPONENTS PROVIDED	GROUP C UP TO 3 STOREYS, SPRINKLERED COMBUSTIBLE CONSTRUCTION	3.2.2.00 A, 3.1.1.01, 3.1.1.02
3	EXISTS AREA	TOTAL COVERED AREA: 114.00 m ²	14.1.1.01, 14.1.1.02
4	EXISTS AREA	TOTAL COVERED AREA: 114.00 m ²	14.1.1.01, 14.1.1.02
5	NUMBER OF STOREYS (GROSS HEIGHT)	1	3.2.2.00 A, 3.1.1.01, 3.1.1.02
6	NEIGHBOURHOOD ZONING	STREET: MERVILLE RD	3.2.2.00 A, 3.1.1.01, 3.1.1.02
7	NEIGHBOURHOOD ZONING	GROUP C UP TO 3 STOREYS, SPRINKLERED COMBUSTIBLE CONSTRUCTION	3.2.2.00 A, 3.1.1.01, 3.1.1.02
8	PROTECTED SYSTEM	YES	3.2.2.00 A, 3.1.1.01, 3.1.1.02
9	STAIRWELL REQUIRED	YES	3.1.1.01, 3.1.1.02
10	FIRE ALARM REQUIRED	YES	3.2.2.00 A, 3.1.1.01, 3.1.1.02
11	WATER SUPPLY	750 BY EXISTING	3.2.2.00 A, 3.1.1.01, 3.1.1.02
12	INSULATION	NO, TOP FLOOR FLOOR SLAB 150 MM RIGID GRADE	3.2.2.00 A, 3.1.1.01, 3.1.1.02
13	CONSTRUCTION	PERMITTED CONSTRUCTION COMBUSTIBLE IN NON-CONFORMING CLASSES	3.2.2.00 A
14	MEZZANINE AREA (M ²)	NA	3.1.1.01, 3.1.1.02
15	MEZZANINE AREA (M ²)	NA	3.1.1.01, 3.1.1.02
16	MEZZANINE DESIGN	YES	3.1.1.01, 3.1.1.02
17	MEZZANINE DESIGN	NO	3.1.1.01, 3.1.1.02
18	MEZZANINE DESIGN	REINFORCED CONCRETE WALLS 150	3.1.1.01, 3.1.1.02
19	MEZZANINE DESIGN	REINFORCED CONCRETE WALLS 150	3.1.1.01, 3.1.1.02
20	MEZZANINE DESIGN	MEZZANINE DESIGN	3.1.1.01, 3.1.1.02
21	MEZZANINE DESIGN	MEZZANINE DESIGN	3.1.1.01, 3.1.1.02
22	MEZZANINE DESIGN	MEZZANINE DESIGN	3.1.1.01, 3.1.1.02

FLOOR SAFETY LEVELS

FLOOR	DESCRIPTION	LEVEL
FLOOR 1	MEZZANINE DESIGN	0.00
FLOOR 2	MEZZANINE DESIGN	0.00

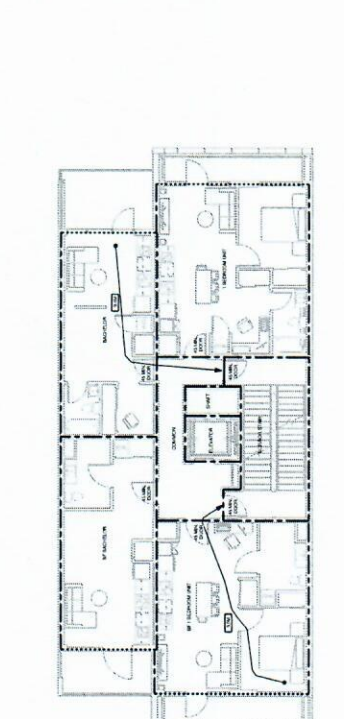
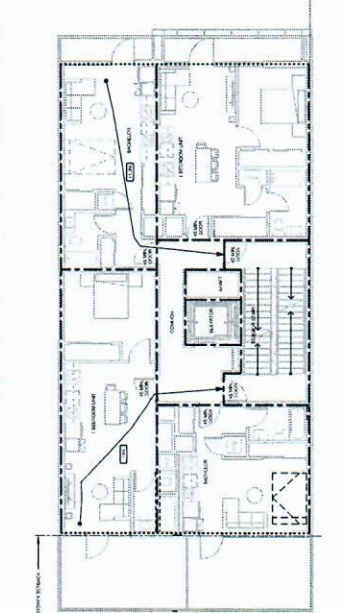
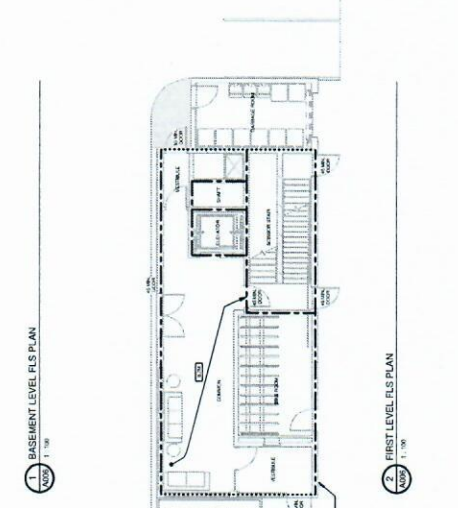
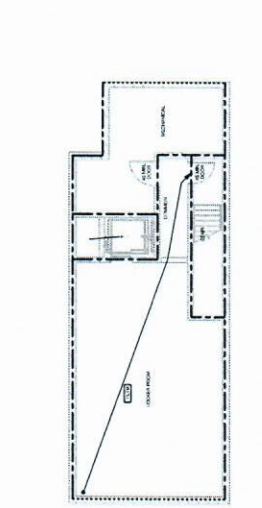
NOTES:

- ALL LEVELS ARE TO FINISH FLOORING.
- MEZZANINE DESIGN IS TO BE USED IN CONJUNCTION WITH THE CONTRACT DOCUMENTS.

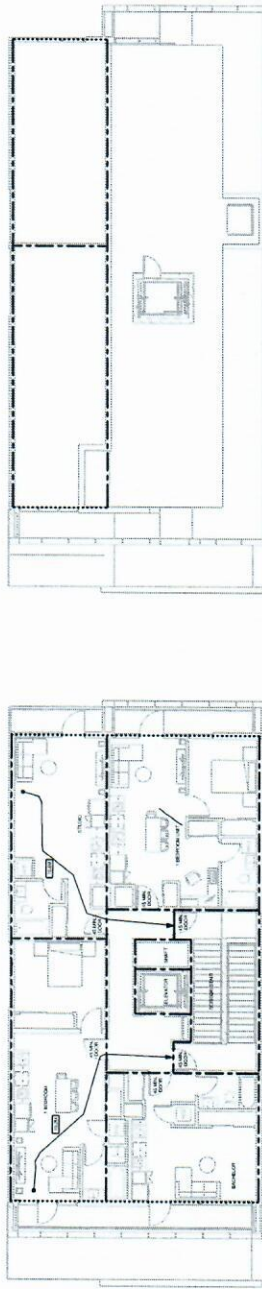
FIRE ALARM LOGS

SYMBOLS:

- MEZZANINE DESIGN
- MEZZANINE DESIGN
- MEZZANINE DESIGN

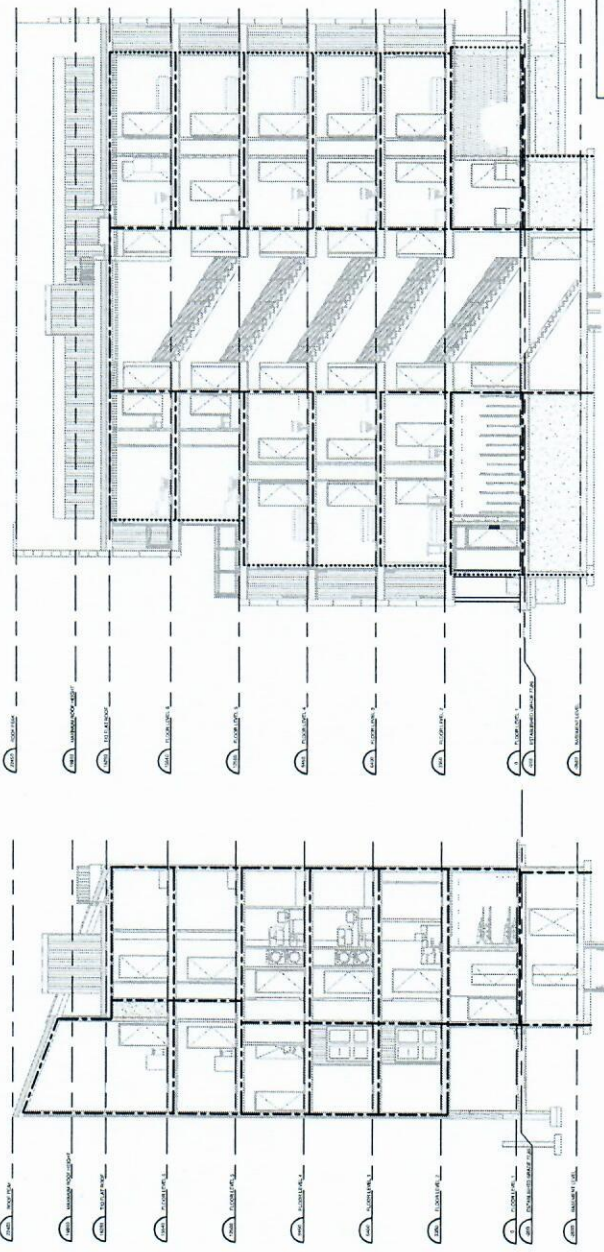


PLANNED PERMITS
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2 SIXTH LEVEL FLS PLAN
1:100

1 FLAT ROOF FLS PLAN
1:100



3 NORTH-SOUTH FLS SECTION
1:100

4 EAST/WEST FLS SECTION
1:100

FLOOR SAFETY LEGEND	
[Symbol]	DESCRIPTION
[Symbol]	REVISIONS TO THIS DRAWING
[Symbol]	EXISTING CONSTRUCTION
[Symbol]	NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING
[Symbol]	PROPOSED EXISTING CONSTRUCTION
[Symbol]	PROPOSED NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING

REVISION RECORD			
NO.	DATE	BY	DESCRIPTION

ISSUE RECORD			
NO.	DATE	BY	DESCRIPTION

LIFE SAFETY LEGEND	
[Symbol]	REVISIONS TO THIS DRAWING
[Symbol]	EXISTING CONSTRUCTION
[Symbol]	NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING
[Symbol]	PROPOSED EXISTING CONSTRUCTION
[Symbol]	PROPOSED NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING

FIRE ALARM LEGEND	
[Symbol]	DESCRIPTION
[Symbol]	EXISTING CONSTRUCTION
[Symbol]	NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING

PARTIAL PANEL SCHEDULE		
NO.	DESCRIPTION	QTY



LIFE SAFETY LEGEND	
[Symbol]	DESCRIPTION
[Symbol]	EXISTING CONSTRUCTION
[Symbol]	NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING
[Symbol]	PROPOSED EXISTING CONSTRUCTION
[Symbol]	PROPOSED NEW CONSTRUCTION
[Symbol]	REMOVE EXISTING CONSTRUCTION
[Symbol]	REVISIONS TO THIS DRAWING

REVISION RECORD			
NO.	DATE	BY	DESCRIPTION

ISSUE RECORD			
NO.	DATE	BY	DESCRIPTION



BIOSIS DESIGNS
 E: info@biosisdesigns.com
 P: (416) 299-9642

MERVILLE APARTMENT
 817 MERVILLE RD
 OTTAWA ON
 for
MARC AMYOT

2301 1:100
 PROJECT SCALE
 Author
 Checker
 IN CHARGE

LIFE & SAFETY PLAN
A007

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MERVALE APARTMENT
817 MERVALE RD
OTTAWA ON
MARC AMYOT

2301 PROJECT NO.
1:100 SCALE
DS
SH

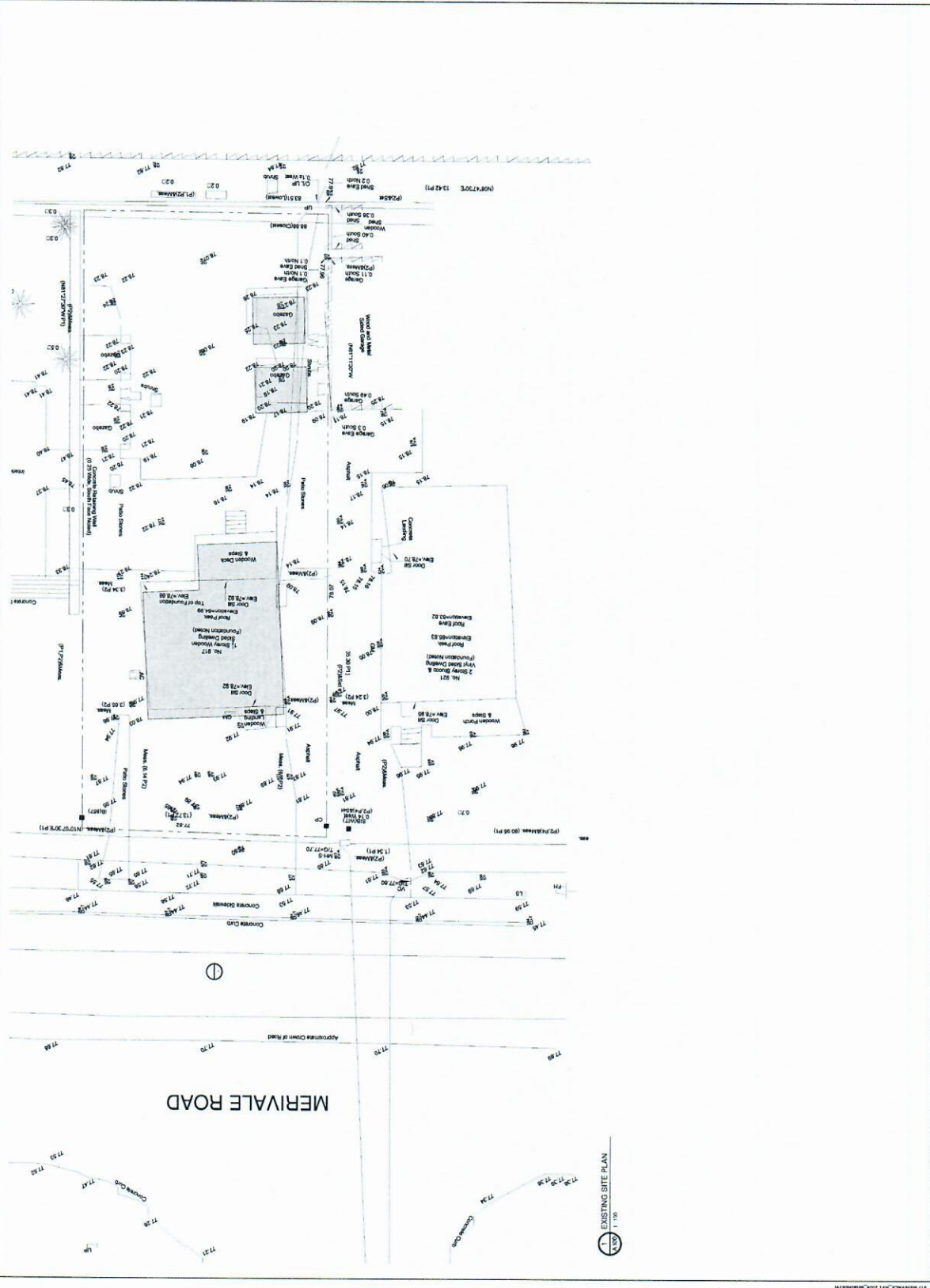
EXISTING SITE PLAN

A100

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REVISION RECORD	
NO.	DATE BY DESCRIPTION

ISSUE RECORD	
NO.	DATE BY DESCRIPTION



ZONING REQUIREMENTS - TM (2196)

EXISTING ZONING: TM(2196) Traditional Main Street Zone
 PROPOSED ZONING: TM(2196) Traditional Main Street Zone

	REQUIRED	PROVIDED	COMPLIES
OFF-STREET PARKING	4	4	Y
VEHICLE PARKING	1	1	Y
BARBER POLE PARKING	0	0	Y
BI-CYCLE PARKING	10	22 (RETRACTED)	Y

FRONT YARD	MAX 2M	MIN 2M	Y
FRONT YARD SETBACK	2.0M	2.0M	Y
REAR YARD	1.5M	1.5M	Y
REAR YARD SETBACK	1.5M	1.5M	Y
REAR YARD FENCE	1.2M	1.2M	Y
MINIMUM LOT WIDTH	0	13.2M	Y
BUILDING FOOTPRINT AREA	NA	93.1%	Y
TOTAL AREA	NA	124.1M	Y
AMOUNT SPACE AREA	NA	CONVERTED TO 1.2M	Y
TOTAL NUMBER OF UNITS	NA	20	Y
MIN. LANDSCAPE BUFFER	1.1M (NOT APPLICABLE)	NA	Y

SITE DATA

ITEM	AREA	% TOTAL
LOT AREA	461.5 SM (5,163 SFT)	100%
BUILDING AREA	245 SM (2,652 SFT)	53.1%
GROSS FLOOR AREA	1,341 SM (14,428 SFT)	291%
ASPHALT/PAVEMENT AREA	196.5 SM (2,137 SFT)	43%
LANDSCAPED AREA	REAR LOT AREA: 70 SM (759 SFT)	14.5%

UNIT DATA

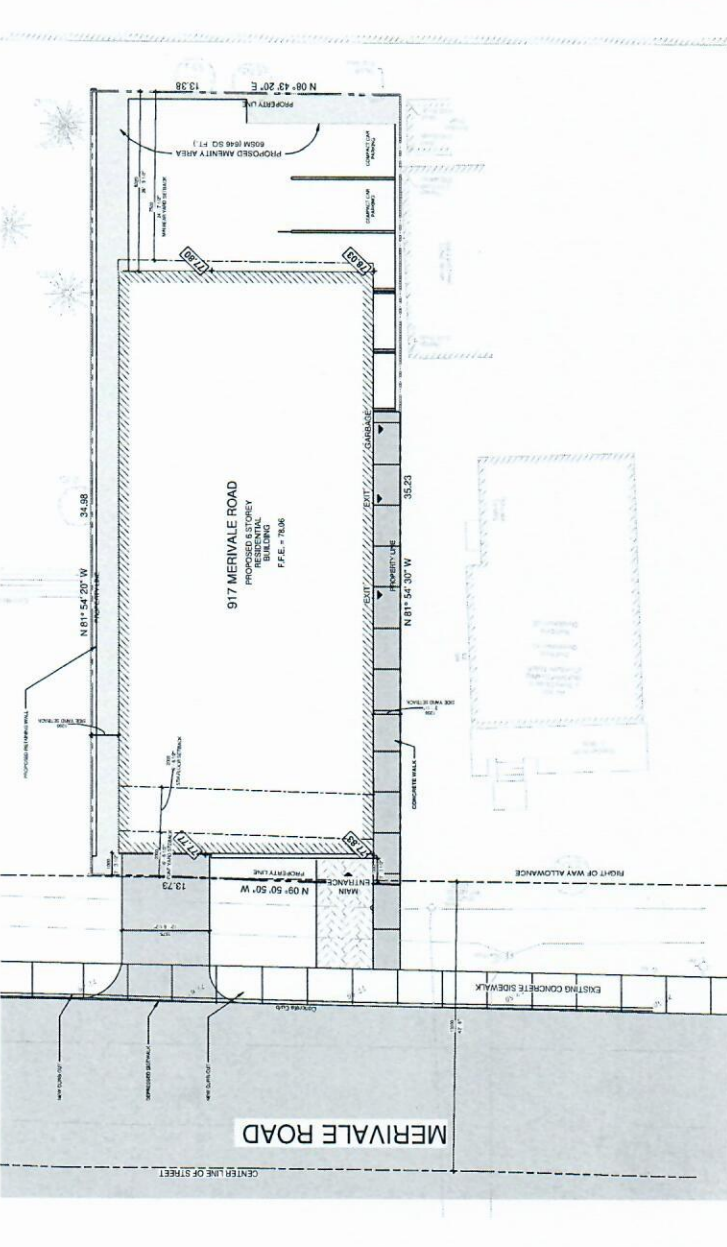
LEVEL	STUDIO	1 BED	2 BED	TOTAL	FLOOR AREA
BASEMENT	0	0	0	0	119 SM (1,280 SFT)
MAIN FLOOR	0	0	0	0	105 SM (1,116 SFT)
2ND FLOOR	2	2	0	4	245 SM (2,652 SFT)
3RD FLOOR	2	2	0	4	245 SM (2,652 SFT)
4TH FLOOR	2	2	0	4	245 SM (2,652 SFT)
5TH FLOOR	2	2	0	4	245 SM (2,652 SFT)
6TH FLOOR	2	2	0	4	239 SM (2,570 SFT)
TOTAL	10	10	0	20	1,341 SM (14,428 SFT) A.S.

PARKING COUNT

USAGE	COUNT	PARKING RATIO	PARKING REQUIRED	COMPLIES
STUDIOS	10	1.0	0	Y
1 BED	10	1.0	4	Y
2 BED	0	0.0	0	Y
BF SPOTS	0	0.0	0	Y
VISITOR	0	0.0	1	Y
TOTAL	20	0.4	5	Y

ESTABLISHED GRADE ELEVATION DATA

BUILDING CORNER	ELEVATION	CALCULATION
NORTH WEST CORNER	77.77	77.77 + 77.80 + 76.03
NORTH EAST CORNER	77.80	= 311.43
SOUTH WEST CORNER	76.03	311.43 / 4
SOUTH EAST CORNER	77.83	= 77.8275
AVERAGE GRADE ELEVATION		AVG ELEV. = 77.86



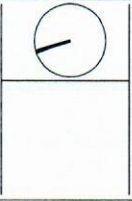
PROPOSED SITE PLAN
 1:100

REVISION RECORD

NO.	DATE	BY	DESCRIPTION
1	21.04.2018	DA	ISSUED FOR PERMITS

ISSUE RECORD

NO.	DATE	BY	DESCRIPTION
1	21.04.2018	DA	ISSUED FOR PERMITS



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 M4RC AMYOT

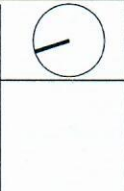
2301 As indicated
 PROJECT SCALE
 DS, SH
 1:100
 SH
 1:100

PROPOSED SITE PLAN
A101

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1	2011.03.01	DS	ISSUE FOR PERMITS
2	2011.03.01	DS	ISSUE FOR PERMITS
3	2011.03.01	DS	ISSUE FOR PERMITS
4	2011.03.01	DS	ISSUE FOR PERMITS
5	2011.03.01	DS	ISSUE FOR PERMITS
6	2011.03.01	DS	ISSUE FOR PERMITS
7	2011.03.01	DS	ISSUE FOR PERMITS
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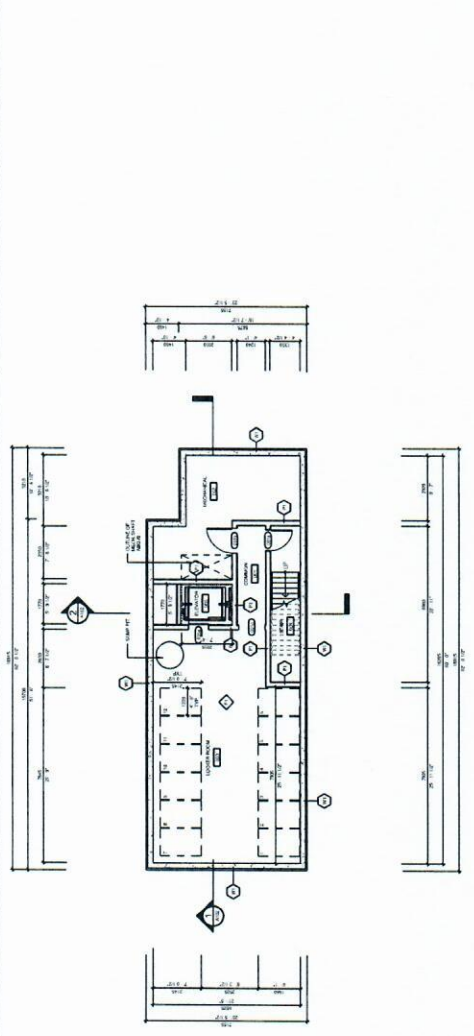


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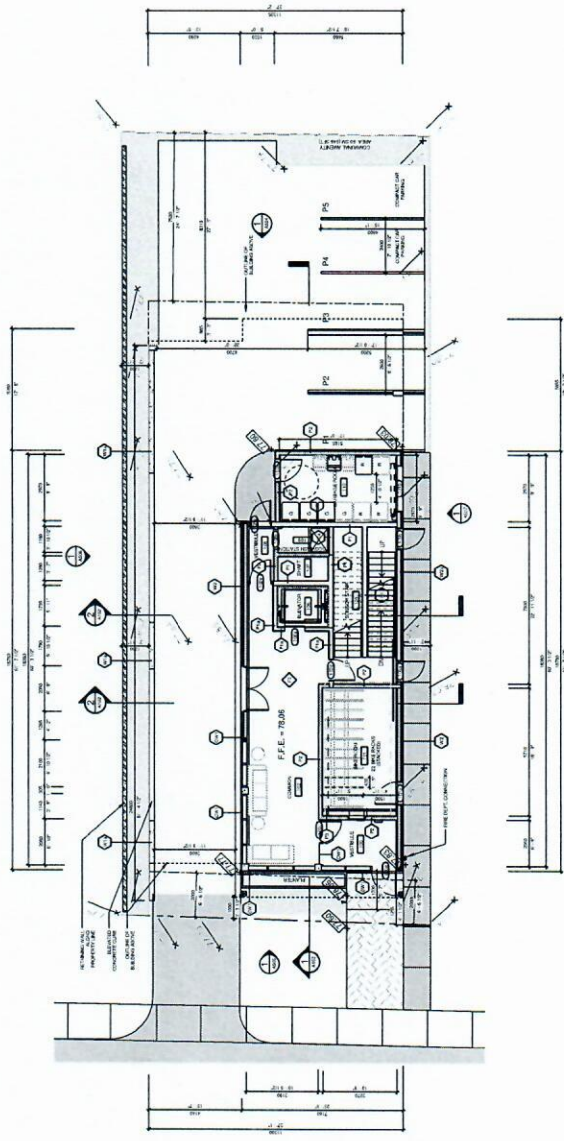
MERVALE APARTMENT
 917 MERVALE RD
 OTTAWA ON
 PROJECT NO. 1100
 CLIENT: DS, SH
 ARCHITECT: BIOSIS DESIGNS
 SCALE: 1/8" = 1'-0"

FLOOR PLANS
A102

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3 BASEMENT LEVEL FLOOR PLAN
 A.102a 1/8" = 1'-0"



1 FIRST LEVEL FLOOR PLAN
 A.102b 1/8" = 1'-0"



NO.	DATE	BY	DESCRIPTION
PERMITS RECORD			
1	27.11.2018	DS	ISSUED FOR PERMITS
2	27.11.2018	DS	ISSUED FOR PERMITS

NO.	DATE	BY	DESCRIPTION
ISSUE RECORD			
1	27.11.2018	DS	ISSUED FOR PERMITS
2	27.11.2018	DS	ISSUED FOR PERMITS



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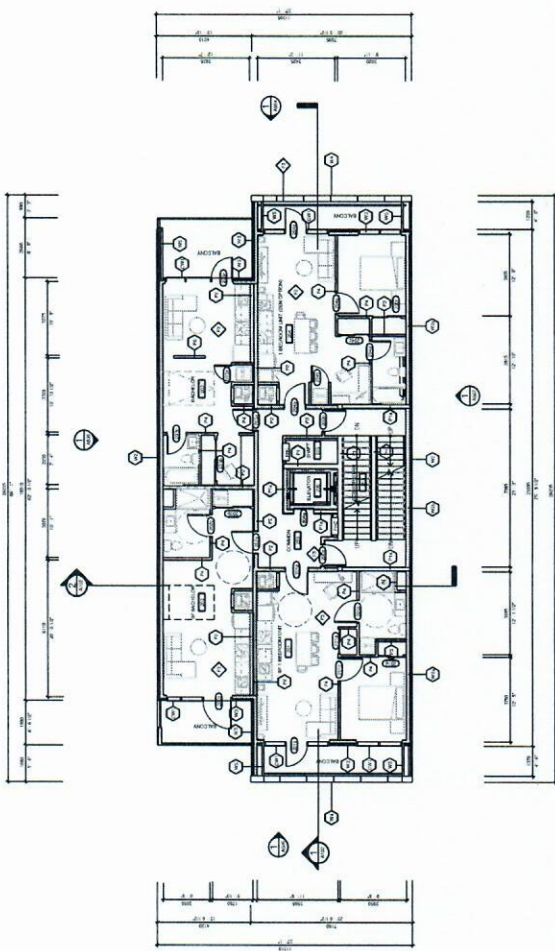
MERVILLE APARTMENT
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 For
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2301
 PROJECT: 61-93-0202
 DATE: 27.11.18
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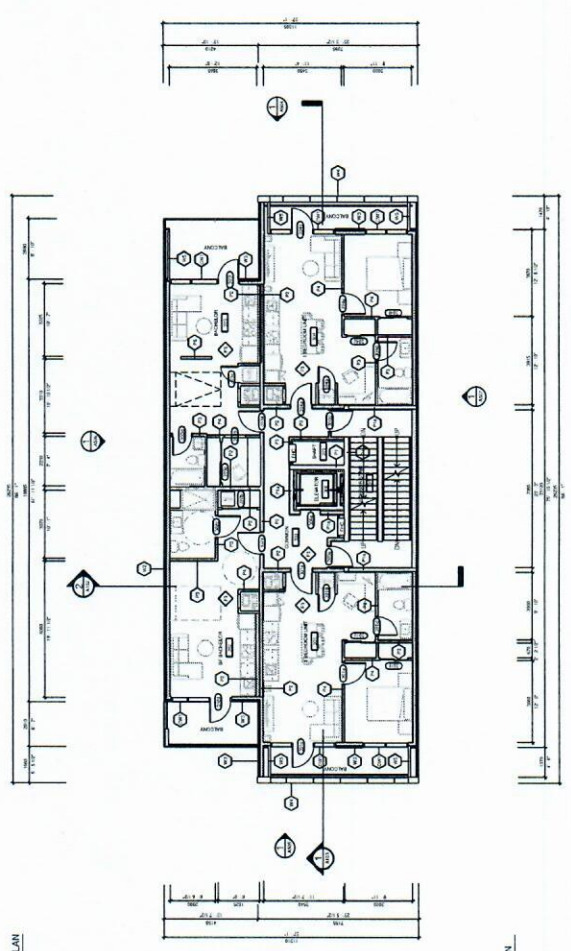
FLOOR PLANS

A103

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2 SECOND LEVEL FLOOR PLAN
 1:100



1 THIRD LEVEL FLOOR PLAN
 1:100



REVISION RECORD		
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NO.	DATE	DESCRIPTION



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MARC AMYOT

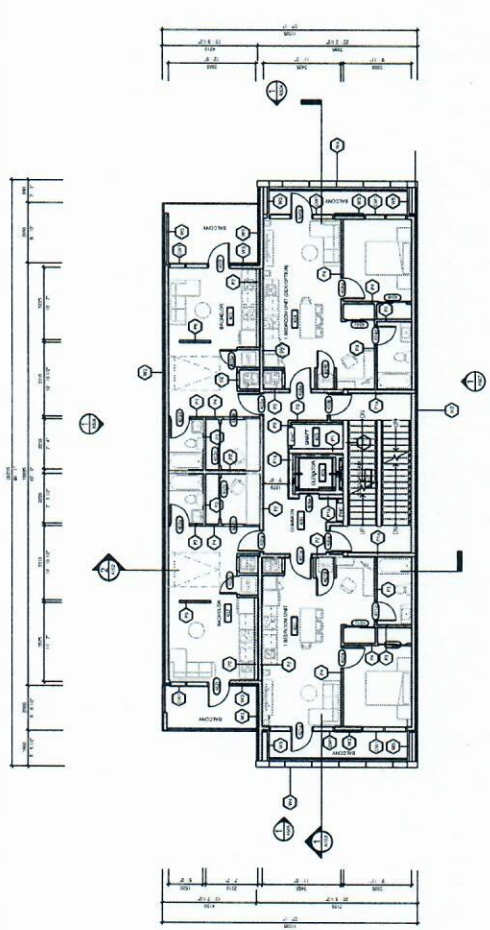
2301
 1:100

DS SH
 MARS
 SH
 REVISED

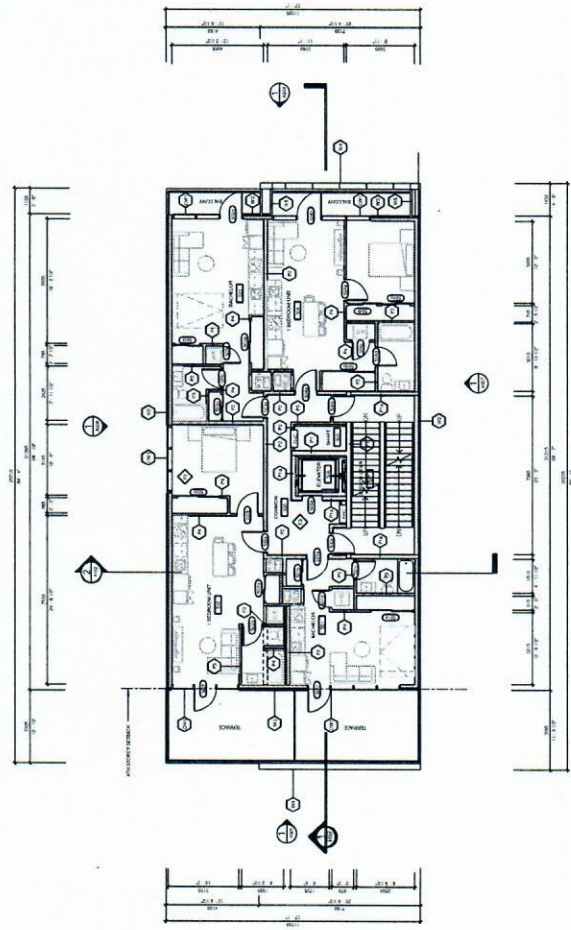
FLOOR PLANS

A104

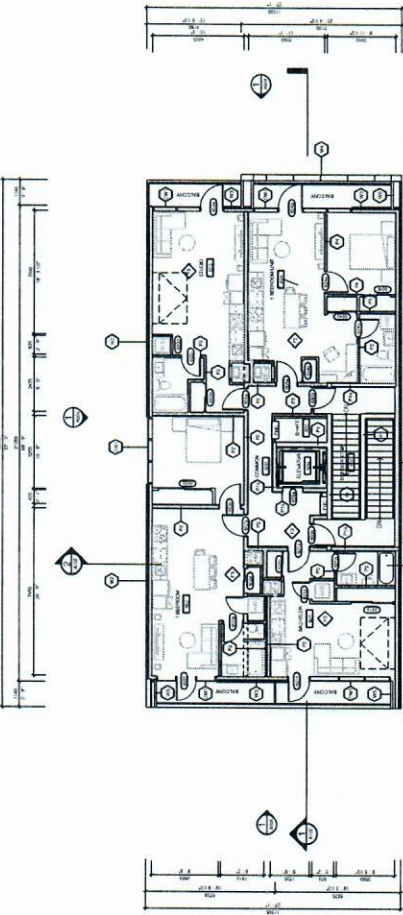
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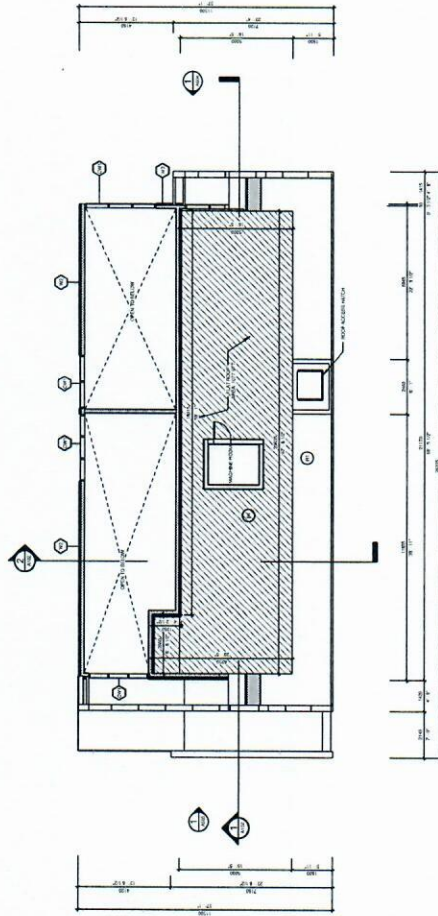
1
 FOURTH LEVEL FLOOR PLAN
 1:100



2
 FIFTH LEVEL FLOOR PLAN
 1:100



1 SIXTH LEVEL FLOOR PLAN
1:100



2 FLAT ROOF PLAN
1:100



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 917 MERVALE RD
 OTTAWA ON
 MRC AMYOT

2301 PROJECT #244
 DS, SH
 SH
 SH
 SH

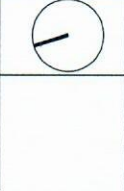
FLOOR PLANS

A105

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NO.	DATE	DESCRIPTION



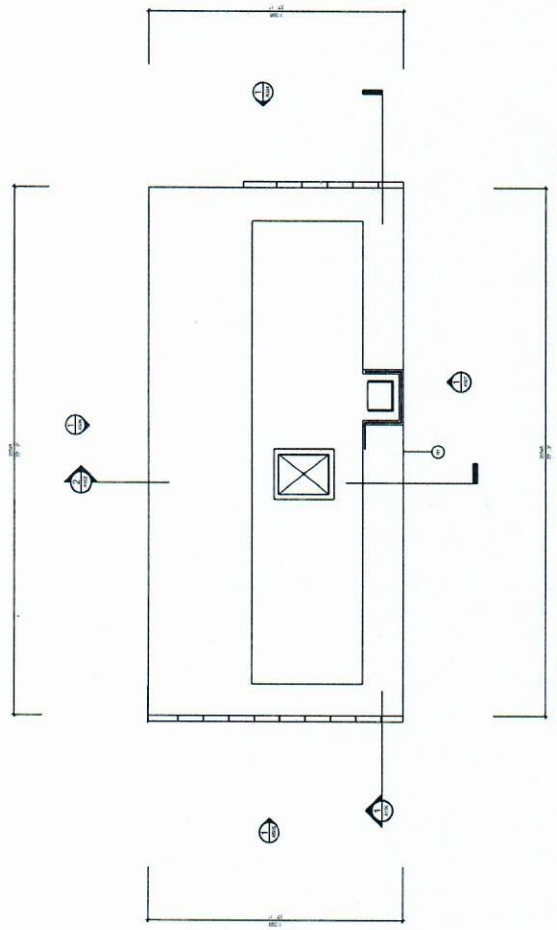
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MERVILLE APARTMENT
 917 MERVILLE RD
 OTTAWA ON
 M: MARC AMYOT

2301 1:100
 PROJECT SCALE
 Author
 Checker
 DRAWER

FLOOR PLANS
A106

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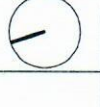


ROOF PLAN
 1:100



REVISION RECORD		
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NO.	DATE	DESCRIPTION

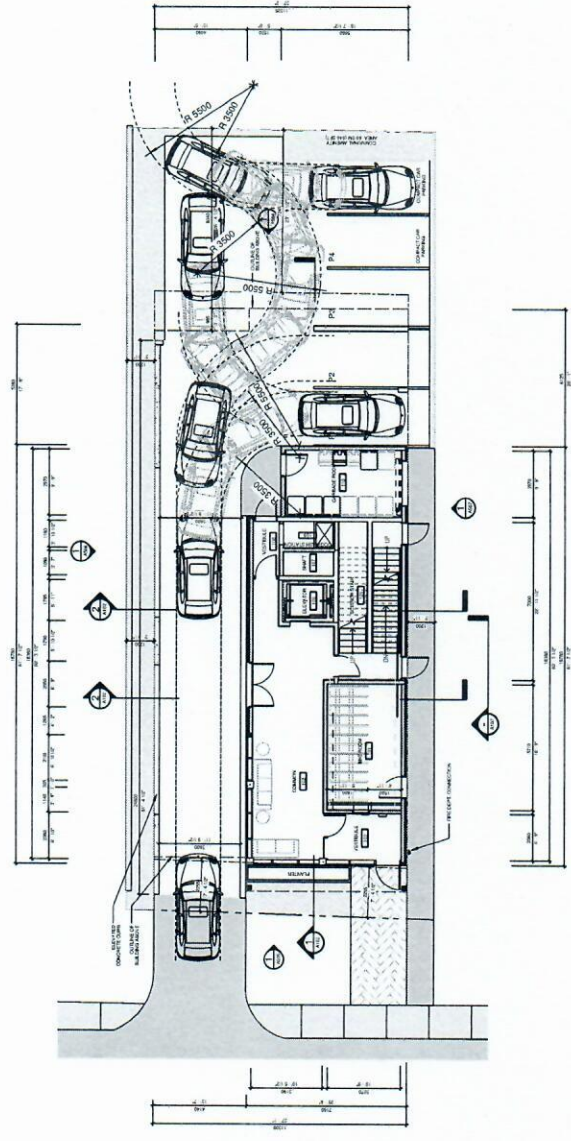


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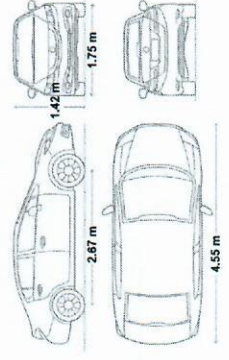
MERVALE APARTMENT
 917 MERVALE RD
 OTTAWA ON
 M4C 1M7
 MARC AMYOT
 PROJECT #1411
 DS SH
 SH
 MARCH 2012

VEHICLE TURNING PLAN
A107

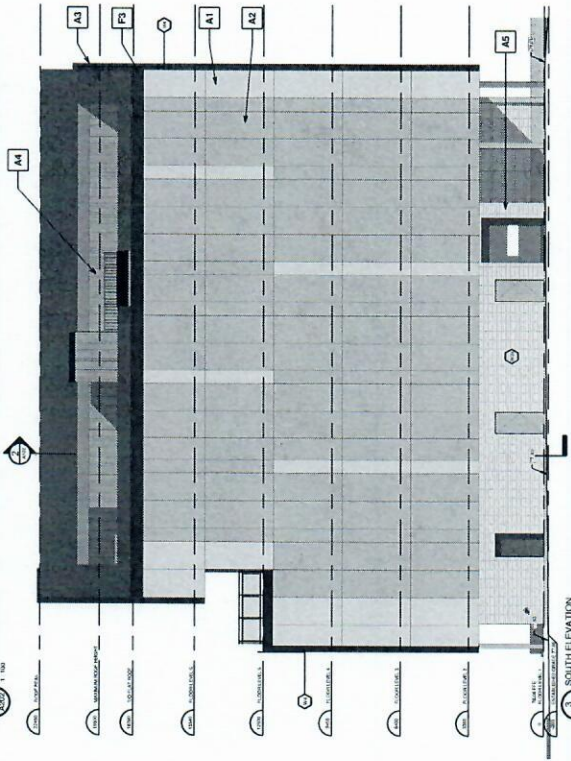
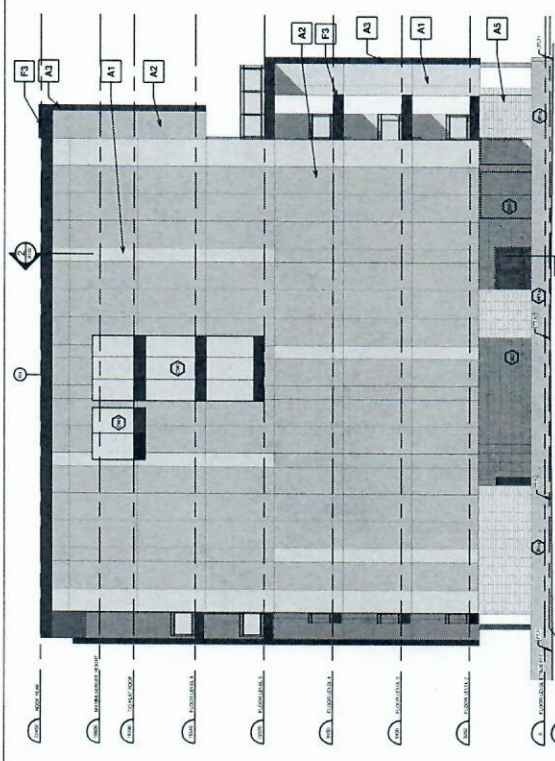
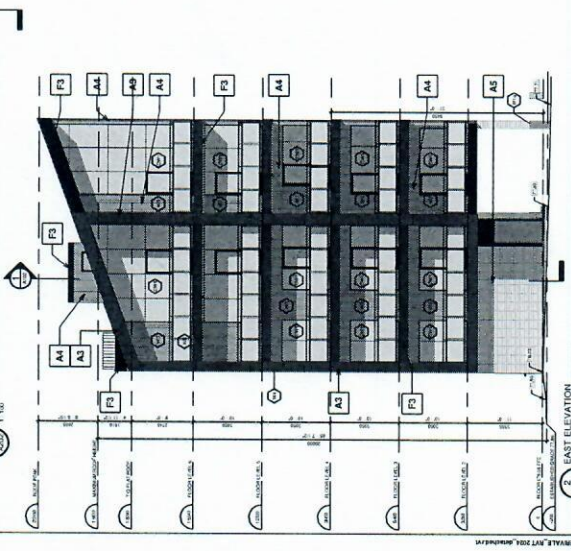
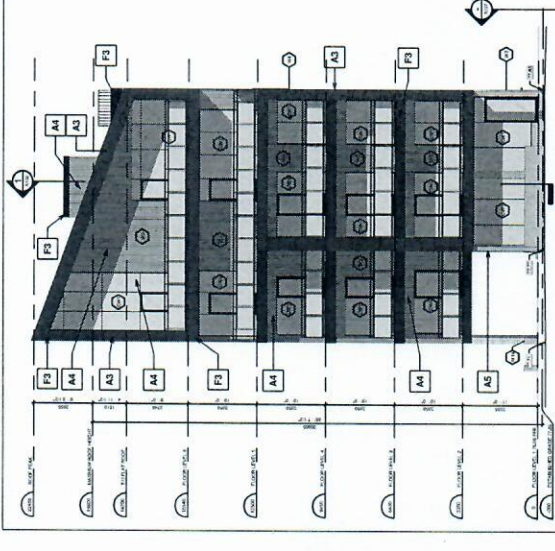
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1 FIRST LEVEL VEHICLE TURNING - MID SIZED SEDAN
 1/200



2 TYP. VEHICLE DIMENSIONS
 1/20



EXTERIOR FINISH LEGEND

A1 - METAL CLADDING PANEL	A2 - METAL CLADDING PANEL	A3 - METAL CLADDING PANEL	A4 - METAL CLADDING PANEL	A5 - METAL CLADDING PANEL
COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE
STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH
PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED
A6 - METAL CLADDING PANEL	A7 - METAL CLADDING PANEL	A8 - METAL CLADDING PANEL	A9 - METAL CLADDING PANEL	A10 - METAL CLADDING PANEL
COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE
STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH
PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED
A11 - METAL CLADDING PANEL	A12 - METAL CLADDING PANEL	A13 - METAL CLADDING PANEL	A14 - METAL CLADDING PANEL	A15 - METAL CLADDING PANEL
COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE	COLOR: BRUSH FREE
STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH	STYLE: SMOOTH
PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED	PATTERN: STACKED

MATERIAL LEGEND

M1	WALNUT
M2	VERTICAL CLADDING
M3	BRUSH FREE BRICK
M4	CONTEMPORARY
M5	HORIZONTAL BRICK



NO.	DATE	BY	DESCRIPTION
1			

NO.	DATE	BY	DESCRIPTION
1			

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MERIVALE APARTMENT
917 MERIVALE RD
OTTAWA ON
FOR
MARC AMYOT

2301 As indicated
PROJECT: 15141
DS,SH
SI
SH
SH
SH

BUILDING ELEVATIONS
A202

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2	2014.03.04	DS	ISSUED FOR I.P.A.
3	2014.03.04	DS	ISSUED FOR I.P.A.
4	2014.03.04	DS	ISSUED FOR I.P.A.
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10	2014.03.04	DS	ISSUED FOR I.P.A.

ISSUE RECORD			
NO.	DATE	BY	DESCRIPTION
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2	2014.03.04	DS	ISSUED FOR I.P.A.
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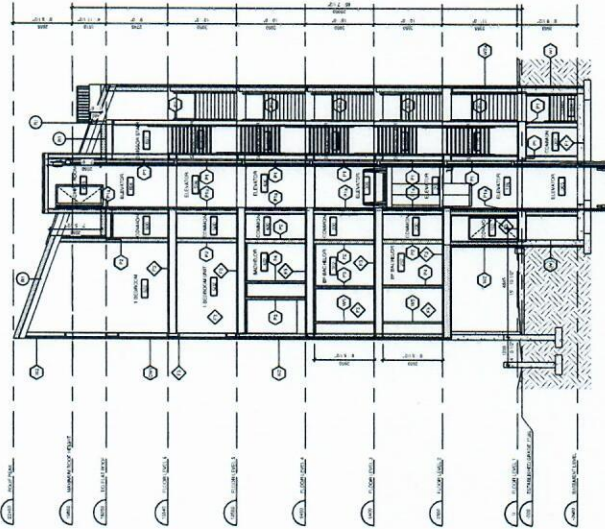
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MERIVALE APARTMENT
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 OTTAWA ON
 For
 MARC AMYOT

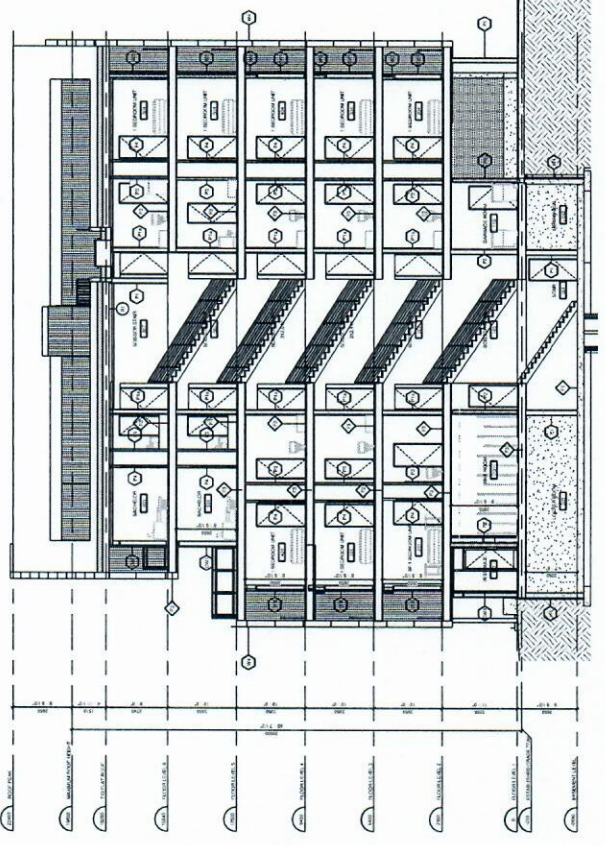
2301 PROJECT NO.
 1:100 SCALE
 SHL DS
 DS
 DS
 DS
 DS
 DS

BUILDING SECTIONS
A301

PLANNING
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2 NORTH-SOUTH SECTION THRU STAIRS
 1:100



1 EAST-WEST SECTION THRU STAIRS
 1:100



NO.	DATE	BY	DESCRIPTION

NO.	DATE	BY	DESCRIPTION

NO.	DATE	BY	DESCRIPTION

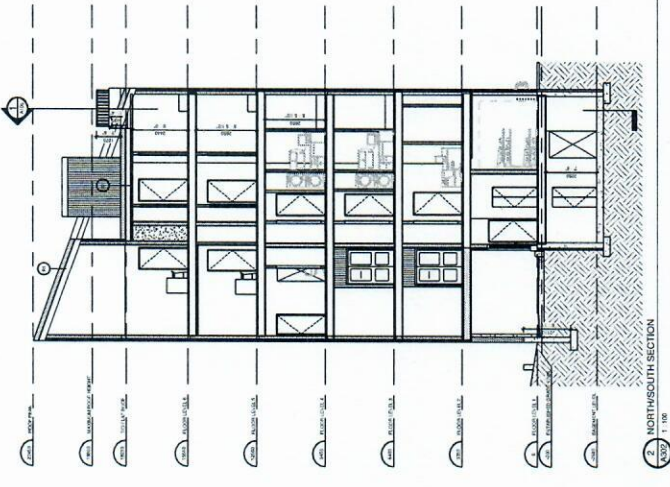
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 M4C 1M7
MARC AMYOT

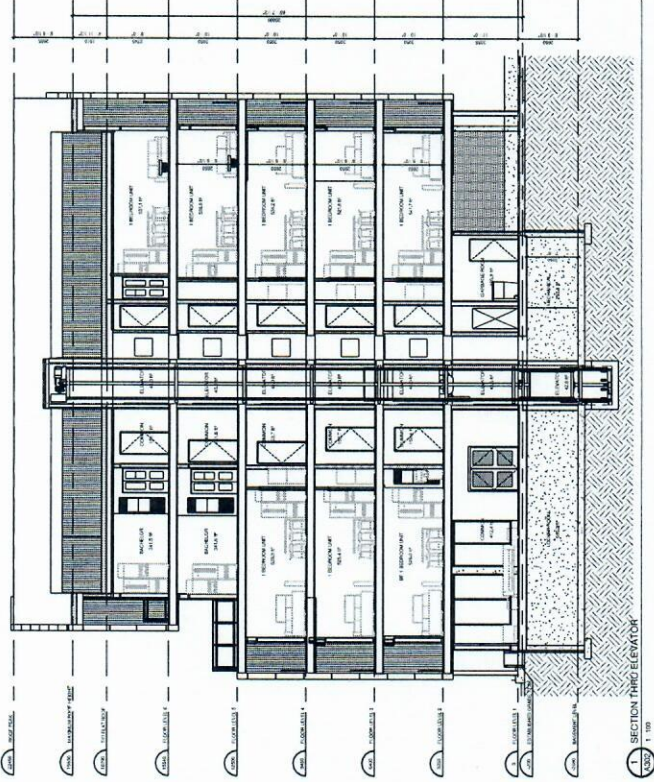
2301 **1:100**
 PROJECT **1:100**
 DS, SH
 DATE
 SHEET
 NUMBER

BUILDING SECTIONS
A302

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2 **NORTH/SOUTH SECTION**
 2020 1:100



1 **SECTION THROUGH ELEVATION**
 2020 1:100



NO.	DATE	BY	DESCRIPTION
REVISION RECORD			
1			
2			
3			
4			
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8			
9			
10			

NO.	DATE	BY	DESCRIPTION
ISSUE RECORD			
1			
2			
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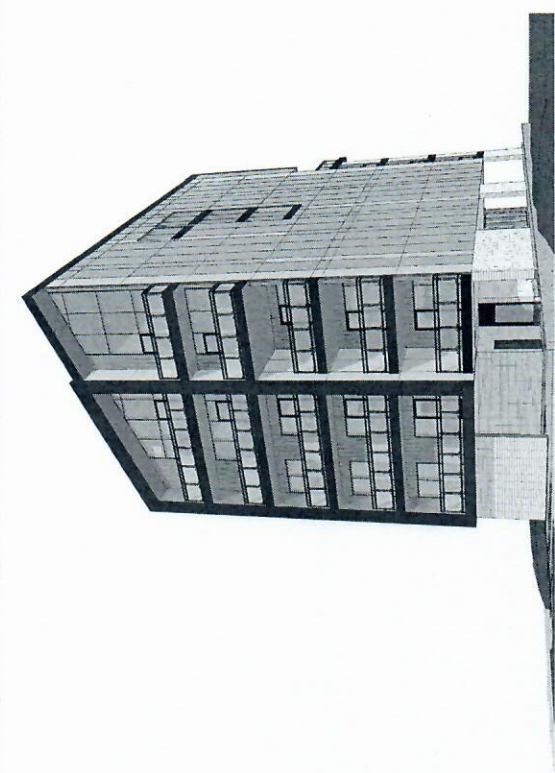
MERVALE APARTMENT
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 M4C 1P8
 M: MARC AMYOT

2301 SCALE
 PROJECT: DS, SH
 DRAWING: SH, SH
 DRAWING: SH, SH

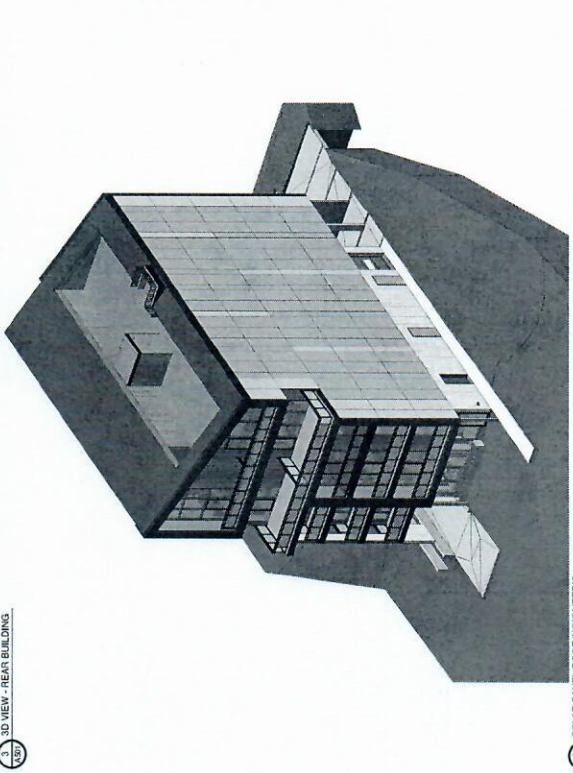
3D VIEWS

A501

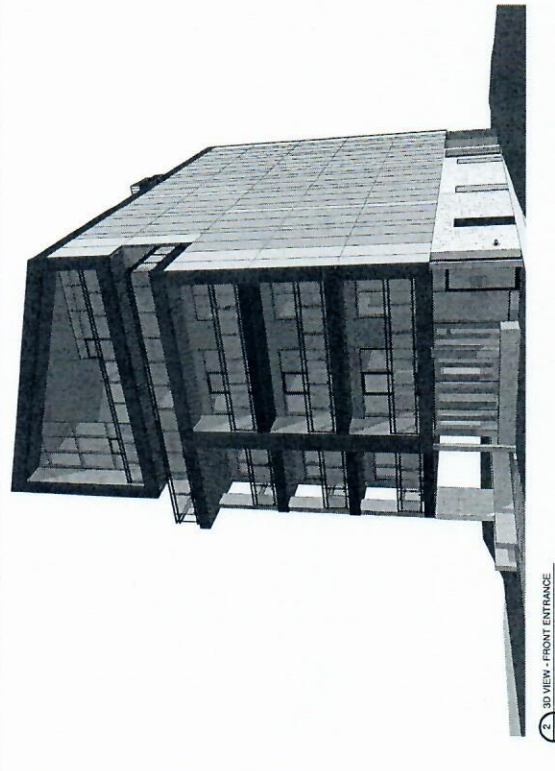
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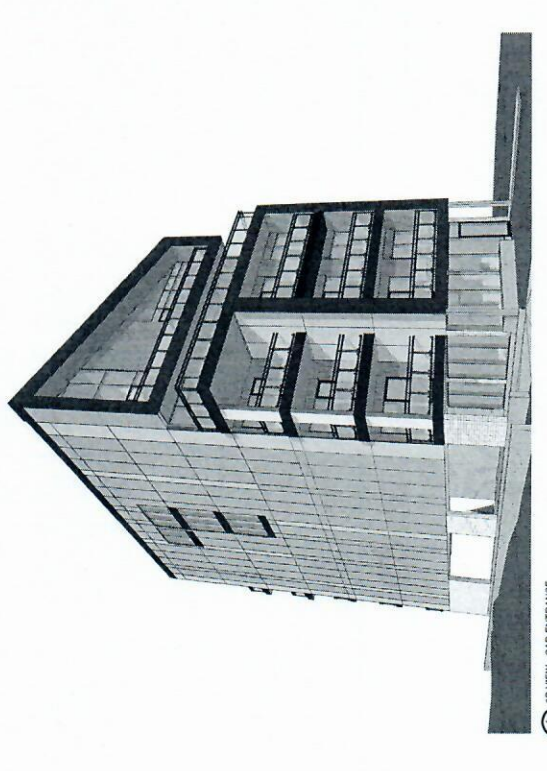
3 3D VIEW - REAR BUILDING



4 SOLAR PANEL ROOF ANIMETRIC



2 3D VIEW - FRONT ENTRANCE



1 3D VIEW - CAR ENTRANCE

ATTACHMENT 2 : WATER BOUNDARY CONDITIONS E-MAIL

Mineault-Guitard, Alexandre

From: TL MaK <tlmakecl@bellnet.ca>
Sent: Friday, May 3, 2024 11:01 AM
To: Mineault-Guitard, Alexandre
Cc: Alemany, Kevin
Subject: RE: 917 Merivale Road - Water Boundary Conditions Request
Attachments: 917 Merivale Road April 2024.pdf

Hi Alex,

Attached please find the Water Boundary Conditions received from the City on May 2, 2024 for your calculation use.

Have a good weekend.

Thank you,

Tony Mak

T.L. Mak Engineering Consultants Ltd.
1455 Youville Drive, Suite 218
Ottawa, ON. K1C 6Z7
Tel. 613-837-5516 | Fax: 613-837-5277
E-mail: tlmakecl@bellnet.ca

From: Adams, Reed [<mailto:reed.adams@ottawa.ca>]
Sent: May 2, 2024 2:50 PM
To: TL MaK
Subject: RE: 917 Merivale Road - Water Boundary Conditions Request

Hi Tony,

Here are the boundary conditions for 917 Merivale:

The following are boundary conditions, HGL, for hydraulic analysis at 917 Merivale Road (zone 2W2C) assumed to be connected to the 305 mm watermain on Merivale Road (see attached PDF for location).

Min HGL: 124.4 m
Max HGL: 132.8 m
Max Day + Fire Flow (183 L/s): 118.0 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Thanks,

Reed

From: Adams, Reed
Sent: April 18, 2024 1:59 PM
To: TL MaK <tlmakecl@bellnet.ca>
Subject: RE: 917 Merivale Road - Water Boundary Conditions Request

Hi Tony,

I've sent the request to our water resources group and should get a response back in two weeks max.

Thanks,

Reed

From: TL MaK <tlmakecl@bellnet.ca>
Sent: April 18, 2024 1:41 PM
To: Adams, Reed <reed.adams@ottawa.ca>
Subject: 917 Merivale Road - Water Boundary Conditions Request

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Hi Reed,

Regarding this site, we are requesting for water boundary conditions from the City of Ottawa to be provided for our hydraulic analysis. The particulars are as follows:

The proposed building located within Pressure Zone 2W at 917 Merivale Road is a 6-storey residential multi-unit building with a basement. The building contains twenty (20) total units, one (1) 2-bedroom, nine (9) 1-bedroom, and ten (10) bachelor units. Each floor covers an area of around 224 m², for a gross floor area of 1,341 m². The building is to be serviced by the 300 mm diameter watermain along Merivale Road.

The domestic demands were calculated using the City of Ottawa's Water Design Guidelines, where the residential consumption rate of 280 L/cap/d was used to estimate average day demands (AVDY). Persons per unit (PPU) for each unit were estimated based on the City of Ottawa's Water Design Guidelines. Following discussions with the City, peaking factors are to be estimated from Table 3-3 of the MECP Design Guidelines for Drinking-Water Systems, given that the proposed development population is less than 500 people. Maximum day (MXDY) demands were calculated by multiplying AVDY demands by a factor of 9.5. Peak hour (PKHR) demands were calculated by multiplying AVDY by a factor of 14.3. Table 1 shows the estimated domestic demands of the existing building.

Table 1: Estimated Domestic Demand

Unit Type	Unit Count	PPU	Consumption	AVDY		MXDY		PKHR	
				L/d	L/s	L/d	L/s	L/d	L/s

Apartment, 2-Bedroom	1	2.1	280	588	0.01	5,586	0.06	8,408	0.10
Apartment, 1-Bedroom	9	1.4		3,528	0.04	33,516	0.39	50,450	0.58
Apartment, Bachelor	10	1.4		3,920	0.05	37,240	0.43	56,056	0.65
Total	20			8,036	0.09	76,342	0.88	114,915	1.33

The fire flow required was determined following the Fire Underwriter Survey (FUS) method and is provided in the attached worksheet. The proposed building will be of wood frame construction. It is understood that the building will be equipped with sprinklers, and that the basement is more than 50% below ground level. The resulting required fire flow is 11,000 L/min (183 L/s) for a duration of 2.25 hours.

In summary:

- AVDY = 8,036 L/d (0.09 L/s);
- MXDY = 76,342 L/d (0.88 L/s);
- PKHR = 114,915 L/d (1.33 L/s); and,
- Fire Flow = 11,000 L/min (183 L/s).

The City is requested to provide boundary conditions for the Average Day, Maximum Day, Peak Hour and Fire Flow conditions indicated above.

Thank you for your prompt attention to this matter. Please forward the boundary conditions as soon as possible.

Have a good day.

Regards,

Tony Mak

T.L. Mak Engineering Consultants Ltd.
 1455 Youville Drive, Suite 218
 Ottawa, ON. K1C 6Z7
 Tel. 613-837-5516 | Fax: 613-837-5277
 E-mail: tlmakecl@bellnet.ca

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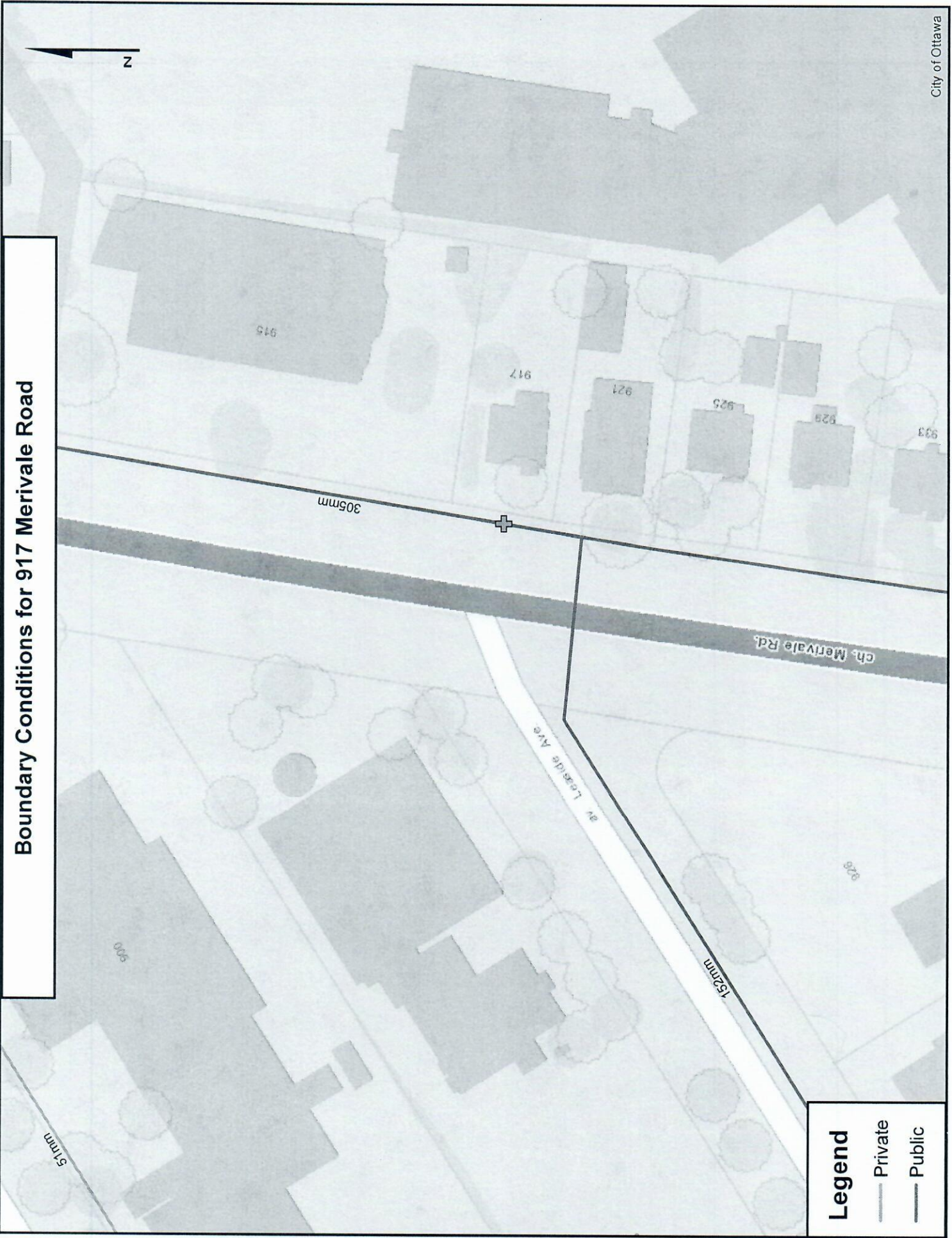
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Boundary Conditions for 917 Merivale Road



ATTACHMENT 3 : FUS FIRE FLOW CALCULATION



FUS Fire Flow Calculation - Long Method

Calculations based on: "Water Supply for Public Fire Protection" by Fire Underwriters' Survey, 2020

Stantec Project #: 163401084
 Project Name: 917 Merivale Road
 Date: August 14, 2024
 Data inputted by: Melissa Nelson, EIT
 Data reviewed by: Alexandre Mineault-Guitard, P.Eng

Fire Flow Calculation #: 1
 Building Type/Description/Name: Residential

Multi-unit building
 Wood frame, with sprinklers. 6-storey residential multi-unit building with a basement that is more than 50% below grade. The building contains twenty (20) total units, ten (10) 1-bedroom, and ten (10) bachelor units.
 Notes: The average floor area is 218 sq.meters, for a gross floor area of 1,310 sq.meters.

Fire Underwriters Survey Determination of Required Fire Flow - Long Method									
Step	Task	Term	Options	Multiplier Associated with Option	Choose:	Value Used	Unit	Total Fire Flow (L/min)	
1	Choose Frame Used for Construction of Unit	Coefficient related to type of construction (C)	Framing Material						
			Type V - Wood Frame	1.5	Type V - Wood Frame	1.5	m		
			Type IV-A - Mass Timber	0.8					
			Type IV-B - Mass Timber	0.9					
			Type IV-C - Mass Timber	1					
			Type IV-D - Mass Timber	1.5					
			Type III - Ordinary construction	1					
			Type II - Non-combustible construction	0.8					
Type I - Fire resistive construction	0.6								
2	Choose Type of Housing (if TH, Enter Number of Units Per TH Block)	Type of Housing	Floor Space Area						
			Single Family	1	Other (Comm, Ind, Apt etc.)	20	Units		
			Townhouse - indicate # of units	0					
Other (Comm, Ind, Apt etc.)	20								
2.2	# of Storeys	Number of Floors/Storeys in the Unit (do not include basement if 50% below grade):			6	6	Storeys		
3	Enter Ground Floor Area of One Unit	Average Floor Area (A) based on total floor area of all floors for one unit (non-fire resistive construction):			218	218	Area in Square Metres (m ²)		
					Square Metres (m ²)				
3.1	Obtain Total Effective Building Area	Total Effective Building Area (# of Storeys x # of Units (if single family or townhouse) x Average Floor Area):			1,310	1310			
4	Obtain Required Fire Flow without Reductions	Required Fire Flow (without reductions or increases per FUS) ($F = 220 \cdot C \cdot \sqrt{A}$) Round to nearest 1,000 L/min						12,000	
5	Apply Factors Affecting Burning	Reductions/Increases Due to Factors Affecting Burning							
5.1	Choose Combustibility of Building Contents	Occupancy Content Hazard Reduction or Surcharge	Non-combustible	-0.25	Limited combustible	-0.15	N/A	10,200	
			Limited combustible	-0.15					
			Combustible	0					
			Free burning	0.15					
			Rapid burning	0.25					
5.2	Choose Reduction Due to Presence of Sprinklers	Sprinkler Reduction	Adequate Sprinkler conforms to NFPA13	-0.3	Adequate Sprinkler conforms to NFPA13	-0.3	N/A	-3,060	
			None	0					
		Water Supply Credit	Water supply is standard for sprinkler and fire dept. hose line	-0.1	Water supply is standard for sprinkler and fire dept. hose line	-0.1	N/A	-1,020	
			Water supply is not standard or N/A	0					
Sprinkler Supervision Credit	Sprinkler system is fully supervised	-0.1	Sprinkler system is fully supervised	-0.1	N/A	-1,020			
	Sprinkler not fully supervised or N/A	0							
5.3	Choose Presence of Sprinklers for Exposures within 30m	Sprinkler Conforms to NFPA13	Adequate sprinkler for exposures conforms to NFPA13		None for exposures		N/A		
			None for exposures						
		Water Supply	Water supply is standard for sprinkler and fire dept. hose line of exposures		Water supply is not standard or N/A for exposures	0	N/A	0	
			Water supply is not standard or N/A for exposures						
		Sprinkler Supervision	Sprinkler system of exposures is fully supervised		Sprinkler not fully supervised or N/A for exposures		N/A		
Sprinkler not fully supervised or N/A for exposures									
5.4	Choose Separation Distance Between Units	Exposure Distance Between Units	Front Yard	30.1m or greater	0	0.6	m	6,120	
			Right Side	3.1 to 10.0m	0.2				
			Rear Yard	10.1 to 20.0m	0.15				
			Left Side	0 to 3.0m	0.25				
6	Obtain Required Fire Flow, Duration & Volume	<i>Total Required Fire Flow, rounded to nearest 1,000 L/min, with max/min limits applied:</i>						11,000	
		<i>Total Required Fire Flow (above) in L/s:</i>						183	
		<i>Required Duration of Fire Flow (hrs)</i>						2.25	
		<i>Required Volume of Fire Flow (m³)</i>						1,485	

ATTACHMENT 4 : FIGURE 1 – FUS EXPOSURE DISTANCES

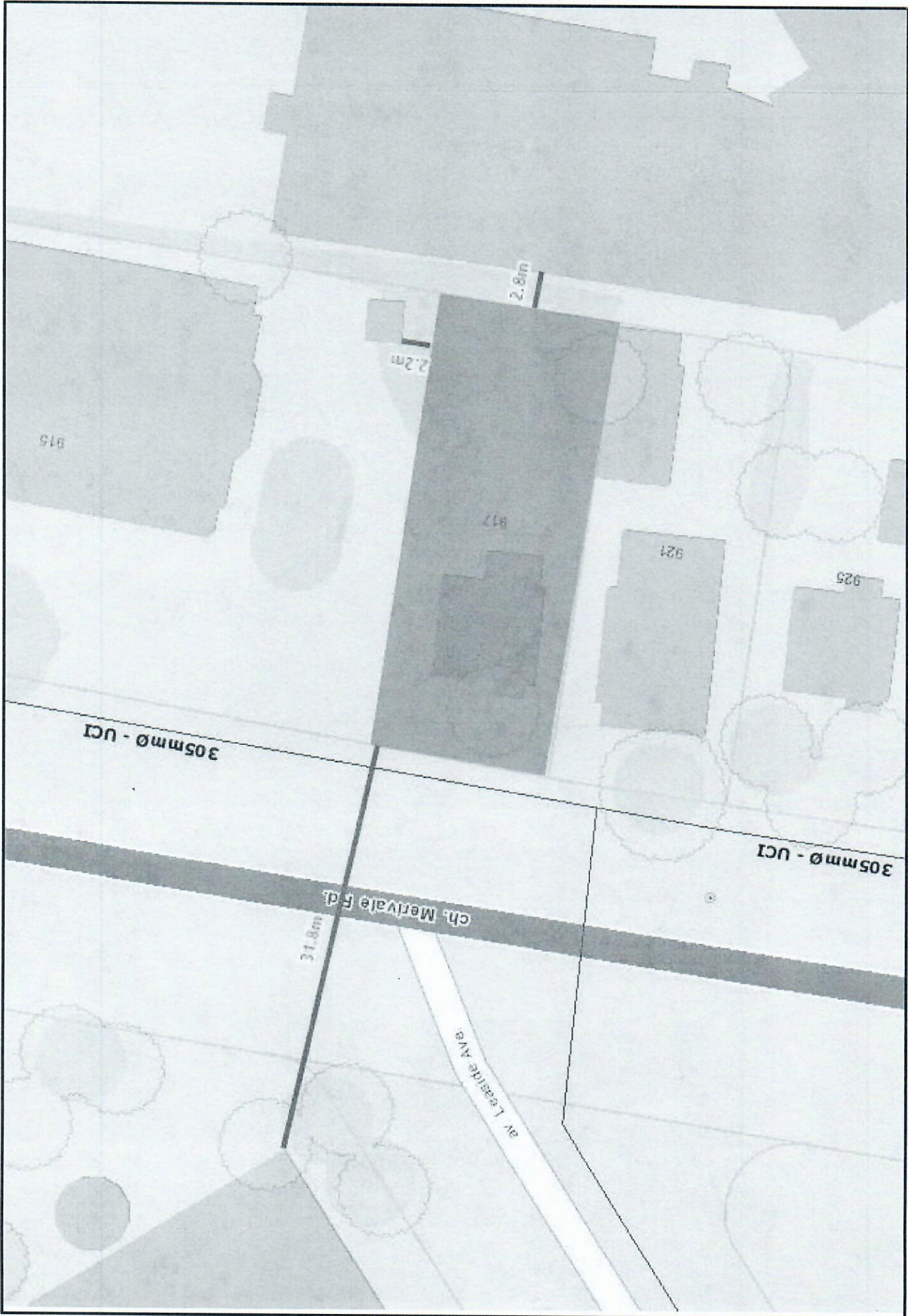


Figure 1: FUS Exposure Distances (Property Line to Adjacent Buildings)

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ATTACHMENT 5 : SUPPORTING HYDRAULIC CALCULATIONS



Supporting Hydraulic Calculations

Stantec Project #: 163401084

Project Name: 917 Merivale Road

Date: May 9, 2024

Data inputted by: Alexandre Mineault-G, P.Eng.

Data reviewed by: Alexandre Mineault-G, P.Eng.

Boundary Conditions provided by the City:

Scenario 1: Peak Hour (Min HGL): 124.4 m;

Scenario 2: Average Day (Max HGL): 132.8 m; and

Scenario 3: Maximum Day plus Fire Flow: 118.0 m.

Sample Calculations

$$HGL (m) = hp + hz \quad (1)$$

where: hp = Pressure Head (m); and hz = Elevation Head (m), estimated from topography.

For Scenario 1, we have:

$$HGL(m) = 124.4 \text{ and } hz (m) = 77.9.$$

Rearranging Equation 1, we can calculate the Pressure Head (hp) as follow:

$$\begin{aligned} hp (m) &= HGL - hz \\ \therefore hp &= 124.4 - 77.9 \text{ m} = 46.5 \text{ m}. \end{aligned}$$

To convert from Pressure Head (m) to a pressure value (kPa), the following equation can be used:

$$P (kPa) = (\rho * g * hp) / 1000 \quad (2)$$

where: ρ = density of water = 1000 kg/m³; and g = gravitational acceleration = 9.81 m/s².

Using Equation 2, we can calculate the Pressure Head (hp) as follow:

$$\begin{aligned} P (kPa) &= (1000 * 9.81 * 46.5) / 1000 \\ \therefore P &= 456 \text{ kPa}. \end{aligned}$$

Considering that 1 kPa = 0.145 psi, the pressure under Scenario 1 is equal to:

$$P = 66 \text{ psi}.$$

Applying the same procedures, the pressures under Scenario 2 and Scenario 3 are calculated as follows:

Scenario 2: P = 78 psi; and Scenario 3: P = 57 psi.

To summarize:

Scenario 1: Minimum Pressure under Peak Hour Demand: 456 kPa (66 psi)
Scenario 2: Maximum Pressure under Average Day Demand: 538 kPa (78 psi)
Scenario 3: Minimum Pressure under Maximum Day + Fire Flow Demand: 393 kPa (57 psi)

ATTACHMENT 6 : FIGURE 2 – HYDRANT SPACING

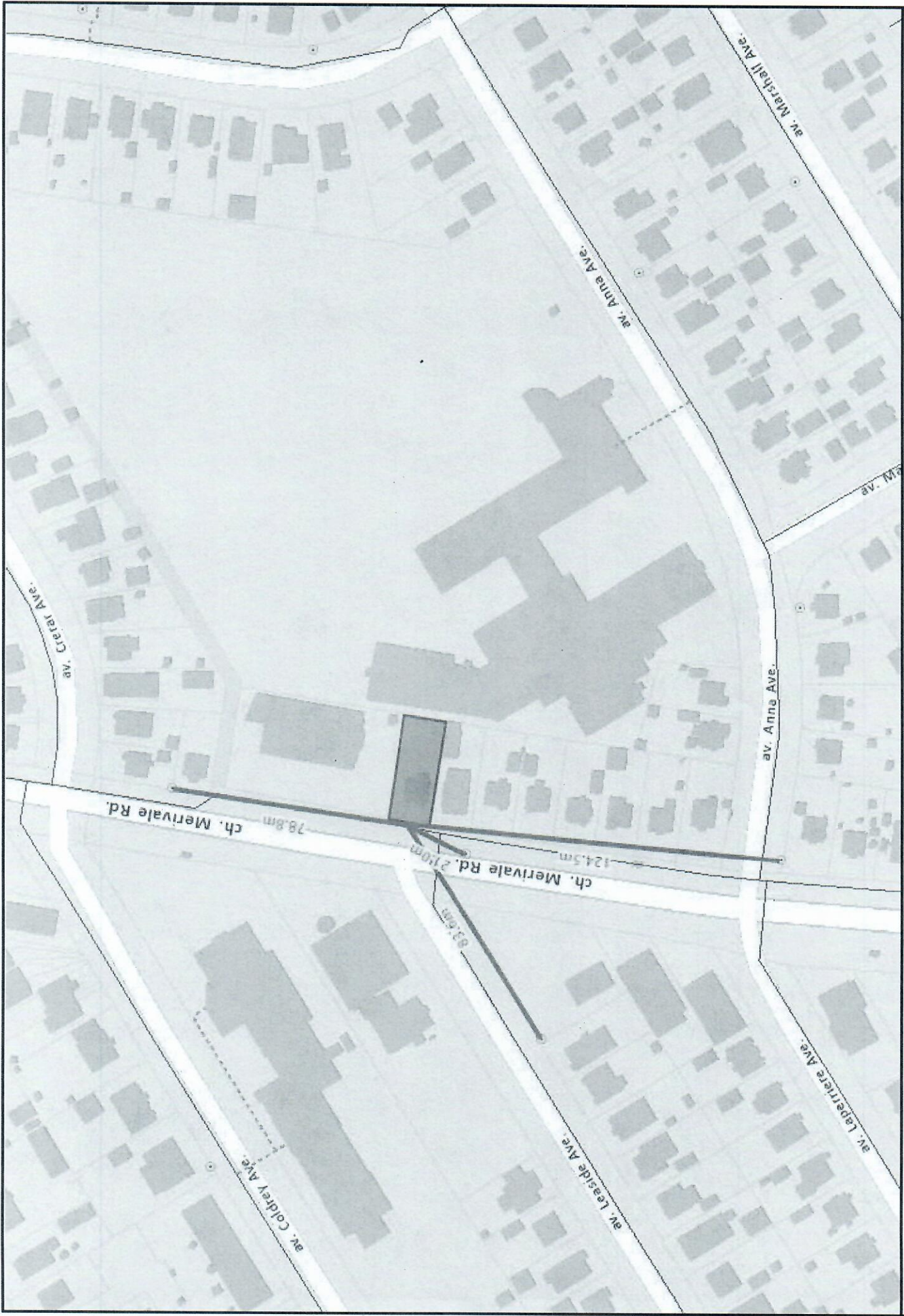


Figure 2: Hydrant Spacing

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PROPOSED
SIX (6) STOREY APARTMENT BUILDING SITE
LOT 1
R-PLAN 268160
917 MERIVALE ROAD
CITY OF OTTAWA

APPENDIX E
CITY OF OTTAWA
SANITARY SEWER DESIGN SHEET
SHEET No. 1 OF 1

SANITARY SEWER DESIGN SHEET

$M = 1 + \frac{14}{4 + \sqrt{P}}$ where P = population in 1000's
 $K = 0.8$
 $Q(p) = \frac{P \cdot q \cdot M}{86.4}$ (L/s)
 $Q(l) = IA$ (L/s) where A = area in hectares
 $Q(d) = Q(p) + Q(l)$ (L/s)

q = average daily per capita flow (L/cap. d)
 I = unit of peak extraneous flow (L/ha. s)
 M = peaking factor
 Q (p) = peak population flow (L/s)
 Q (l) = peak extraneous flow (L/s)
 Q (d) = peak design flow

DENSITY
 • 1 BEDROOM
 • BACHELOR

LOCATION			INDIVIDUAL		CUMULATIVE		PROPOSED SEWER											
STREET	FROM	TO	Area A	Pop.	Area A	Pop.	Peaking factor M	Pop. flow Q(p) (L/s)	Peak extraneous flow Q(l) (L/s)	Peak design flow Q(d) (L/s)	Length (m)	Pipe size (mm)	Type of pipe	Grade %	Capacity (L/s) n=0.012	Full flow velocity (m/s)	Actual velocity at Q(d)	
			(ha.)		(ha.)													
917 MERIVALE ROAD	SITE	EX-3750 SAN-SWR	0.048	28	0.048	28	3.49	0.32	0.02	0.34	3.5	150	PVC	1.0	19.8	1.12		



SHEET No. 1 of 1
 DESIGN: TLM
 CHECKED: TLM
 DATE: AUGUST 2024
 PROJECT: 917 MERIVALE ROAD
 PROPOSED SIX (6) STOREY APARTMENT BUILDING SITE - CITY OF OTTAWA
 (FILE# 823-102)

**PROPOSED
THREE (3) STOREY APARTMENT BUILDING SITE
LOT 75
R-PLAN 263
370 ATHLONE AVENUE
CITY OF OTTAWA**

**APPENDIX F
DEVELOPMENT SERVICING STUDY CHECKLIST SUMMARY**

Servicing study guidelines for development applications

4. Development Servicing Study Checklist

The following section describes the checklist of the required content of servicing studies. It is expected that the proponent will address each one of the following items for the study to be deemed complete and ready for review by City of Ottawa Infrastructure Approvals staff.

The level of required detail in the Servicing Study will increase depending on the type of application. For example, for Official Plan amendments and re-zoning applications, the main issues will be to determine the capacity requirements for the proposed change in land use and confirm this against the existing capacity constraint, and to define the solutions, phasing of works and the financing of works to address the capacity constraint. For subdivisions and site plans, the above will be required with additional detailed information supporting the servicing within the development boundary.

4.1 General Content

- Executive Summary (for larger reports only).
- Date and revision number of the report.
- Location map and plan showing municipal address, boundary, and layout of proposed development.
- Plan showing the site and location of all existing services.
- Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.
- Summary of Pre-consultation Meetings with City and other approval agencies.
- Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.
- Statement of objectives and servicing criteria.
- Identification of existing and proposed infrastructure available in the immediate area.
- Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).
- Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.
- Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.
- Proposed phasing of the development, if applicable.

- Reference to geotechnical studies and recommendations concerning servicing.

- All preliminary and formal site plan submissions should have the following information:
 - Metric scale
 - North arrow (including construction North)
 - Key plan
 - Name and contact information of applicant and property owner
 - Property limits including bearings and dimensions
 - Existing and proposed structures and parking areas
 - Easements, road widening and rights-of-way
 - Adjacent street names

4.2 Development Servicing Report: Water

- Confirm consistency with Master Servicing Study, if available
- Availability of public infrastructure to service proposed development
- Identification of system constraints
- Identify boundary conditions
- Confirmation of adequate domestic supply and pressure
- Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.
- Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.
- Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design
- Address reliability requirements such as appropriate location of shut-off valves
- Check on the necessity of a pressure zone boundary modification.
- Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range

- Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.
- Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.
- Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.
- Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.

4.3 Development Servicing Report: Wastewater

- Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).
- Confirm consistency with Master Servicing Study and/or justifications for deviations.
- Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.
- Description of existing sanitary sewer available for discharge of wastewater from proposed development.
- Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)
- Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.
- Description of proposed sewer network including sewers, pumping stations, and forcemains.
- Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).
- Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.
- Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.
- Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.
- Special considerations such as contamination, corrosive environment etc.

4.4 Development Servicing Report: Stormwater Checklist

- Description of drainage outlets and downstream constraints including legality of outlets (i.e. municipal drain, right-of-way, watercourse, or private property)
- Analysis of available capacity in existing public infrastructure.
- A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.
- Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.
- Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.
- Description of the stormwater management concept with facility locations and descriptions with references and supporting information.
- Set-back from private sewage disposal systems.
- Watercourse and hazard lands setbacks.
- Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.
- Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.
- Storage requirements (complete with calculations) and conveyance capacity for minor events (1:5 year return period) and major events (1:100 year return period).
- Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.
- Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.
- Any proposed diversion of drainage catchment areas from one outlet to another.
- Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and stormwater management facilities.
- If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100 year return period storm event.
- Identification of potential impacts to receiving watercourses
- Identification of municipal drains and related approval requirements.
- Descriptions of how the conveyance and storage capacity will be achieved for the development.
- 100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.

- Inclusion of hydraulic analysis including hydraulic grade line elevations.
- Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.
- Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.
- Identification of fill constraints related to floodplain and geotechnical investigation.

4.5 Approval and Permit Requirements: Checklist

The Servicing Study shall provide a list of applicable permits and regulatory approvals necessary for the proposed development as well as the relevant issues affecting each approval. The approval and permitting shall include but not be limited to the following:

- Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.
- Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.
- Changes to Municipal Drains.
- Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)

4.6 Conclusion Checklist

- Clearly stated conclusions and recommendations
- Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.
- All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario