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Site Servicing Report

2983 Navan Road – Block 16 Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

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1.0 INTRODUCTION

1.1 General

In 2023, J.L. Richards & Associates Limited (JLR) was retained by 12714001 Canada Inc. (the Owner) to prepare the detailed design of municipal infrastructure for Site Plan Approval (SPA) of their parcel located at 2983 Navan Road. The mixed-use site known as “Block 16 - Gas Station, Commercial Building, Drive-Thru Restaurant and Car Wash” will be referred to herein as Block 16. This Site Servicing Report (SSR) presents the servicing constraints and strategies for water, wastewater, stormwater servicing, and stormwater management in accordance with the City of Ottawa Design Guidelines, the associated technical bulletins and relevant design excerpts. This SSR also includes strategies for implementing erosion and sedimentation control measures throughout the construction phase of the project.

1.2 Site Description

The Block 16 is located within the City of Ottawa’s Official Plan boundary and consists of a 0.77 ha parcel bounded by Navan Road to the south and existing residential properties to the east, the proposed East Ridge Orleans Subdivision and Future Mixed-Use Block 15 to the north, and Brian Coburn Boulevard to the west. The legal description of the subject property can be found in the Draft Plan of Subdivision attached to Appendix A.

A topographical survey was completed by Stantec Inc. in August 2023 (Appendix A). The survey indicates that the existing ground surface generally slopes downwards in a southeasterly direction towards Navan Road.

1.3 Proposed Development

The proposed commercial development will consist of a gas station including a car wash, a commercial retail space along with a Drive-Thru Restaurant. The Concept Plan for the Block 16 is attached to Appendix A.

1.4 Proposed Connections to Existing Infrastructure

The proposed site plan will be serviced via the future East Ridge Orleans Subdivision and via existing infrastructure on Navan Road as follows and as shown on the servicing drawings:

Watermain

- Connection to the proposed existing 204 mm diameter watermain along Navan Road

Sanitary

- Connection to the future East Ridge Orleans Subdivision. A 200mm sanitary sewer stub will be dropped to service Block 16 as part of this future subdivision.

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Storm

- Connection to the future East Ridge Orleans Subdivision. A 525 mm storm sewer stub will be dropped to service Block 16 as part of this future subdivision.

1.5 Consultation and Permits

An initial pre-consultation meeting was held on July 6, 2022, followed by a Phase 2 Pre-consultation help on September 13, 2023, each to discuss the proposed site plan, the planning approval process requirements, provide clarifications on design criteria, and high level discussion on servicing constraints. A copy of the pre-consultation meeting notes and the site servicing checklist has been provided in Appendix B.

2.0 WATER SERVICING

2.1 Water Supply Design Criteria

A Hydraulic Network Analysis (HNA) was completed as part of the detailed design for the East Ridge Orleans subdivision to confirm that the proposed watermains could provide adequate supply while complying with both the Ottawa Design Guidelines for Water Distribution (July 2010) and Technical Bulletins ISDTB-2014-02, ISTB-2018-02 and ISTB-2021-03. These documents are herein referred to as the Design Guidelines and TB-2014-02, TB-2018-02, and TB-2021-03, respectively.

The HNA completed as part of the East Ridge Orleans Subdivision design included water demands for the Commercial Site Plan (Block 16). The HNA has since been updated to reflect the proposed water service lateral for Block 16 but is based on the same demands and the boundary conditions used in the original East Ridge Orleans Subdivision HNA (refer to Appendix C for a copy of City correspondence for boundary conditions).

Section 4.2.2 of the Design Guidelines states the following criteria for development additions to the public water distribution system:

- Under maximum hourly demand conditions (peak hour), the residual pressures shall not be less than 276 kPa (40 psi);
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 140 kPa (20 psi);
- In accordance with the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi);
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi); and
- Feeder mains, which have been provided primarily for the purpose of redundancy, shall meet, at a minimum, the basic day plus fire flow demand.

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2.2 Domestic Water Demands

The estimated commercial water demands presented in this section are based on the site layout proposed in the Site Plan (Appendix A). A plug flow of 3.60 L/s was added to each demand scenario to consider the demands required for the car wash (refer to Appendix D for a confirmation letter from the mechanical engineer). Table 1 summarizes the water demands projected for this site.

Table 1: Water Demands

Demand Scenario	Commercial Water Consumption or Peaking Factor	Commercial Water Demands (L/s)	Car Wash Demands (L/s)	Total Demands (L/s)
Average Day Demand	28,000 L/ha/d	0.25	3.60 L/s	3.85
Maximum Day Demand	1.5 x Avg Day	0.37	3.60 L/s	3.97
Peak Hour Demand	1.8 x Max Day	0.67	3.60 L/s	4.27

2.3 Fire Flow Requirements

The City has specified that the Fire Underwriters Survey (FUS) method shall be used for any public or private site where new fire hydrants are being designed. Specifically, the required fire flow (RFF) for each structure was calculated in accordance with TB-2018-02.

The required fire flow for the Commercial Site Plan (Block 16) was calculated to be 83 L/s. Refer to Appendix C for the detailed RFF calculations for the critical fire area.

2.4 Proposed Water Servicing, Boundary Conditions and Water Model

2.4.1 Proposed Water Servicing

Water will be supplied to the Commercial Site Plan (Block 16) by a 150 mm diameter water service that will connect to the existing 305 mm watermain on Navan Road, located east of the intersection between Navan Road and Brian Coburn Blvd. Fire protection will be provided by a new proposed hydrant within the site. As shown in the servicing plan, the car wash is serviced directly by the gas station. Design of the service will be verified by Owner's mechanical engineer however, as noted in Section 2.2, the demand from the car wash has been included in this HNA.

Watermain roughness coefficients were determined using friction factors presented in Section 4.2.12 of the Design Guidelines and the internal pipe diameters were modelled based on Section 4.3.5 of the design Guidelines.

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2.4.2 Boundary Conditions

Hydraulic boundary conditions were provided by the City at the proposed connection location listed in Section 2.4.1 above. Table 2 summarizes the hydraulic boundary conditions received by the City (refer to Appendix C for a copy of the City correspondence). The boundary condition for maximum day plus fire flow corresponds to a required fire flow of 100 L/s. It is noted that the fire flow demand for the City boundary condition is more conservative than the calculated fire flow requirement of 5000 L/min (83 L/s).

Table 2: Hydraulic Boundary Conditions

Demand Scenarios	Connection 3 Head (m)
Maximum HGL	130.7
Peak Hour	126.8
Max Day plus Fire Flow 6,000 L/min (100.00 L/s)	127.3

2.4.3 Water Model

A hydraulic water model within the WaterCAD® software platform was used to carry out the HNA (refer to the overall schematics presented in Appendix C). The water demands from Table 1 and the boundary conditions from Table 2 were input into the model for each demand scenario. Table 3 summarizes the watermain diameters and roughness coefficients used in the model, based on Sections 4.2.12 and 4.3.5 of the Design Guidelines.

Table 3: Watermain Internal Diameters and C-Factors

Nominal Diameter	Inside Diameter	C-Factor
150 mm	155 mm	100
200 mm	204 mm	110
300 mm	297 mm	120

2.5 Simulation Results

The HNA was carried out under steady-state peak hour, maximum day plus fire flow, and maximum pressure conditions to confirm that the proposed water servicing can meet the design criteria outlined in Section 2.1.

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2.5.1 Peak Hour

The simulation results found the minimum pressure at the site during the peak hour condition to be 399 kPa (57.9psi) (refer to Appendix C), which exceeds the minimum pressure criterion of 276 kPa (40 psi) per the Design Guidelines.

2.5.2 Maximum Day Plus Fire Flow

Fire water supply will be provided by a proposed hydrant off the 150 mm diameter water service for Block 16. Hydrant spacing was carried out in accordance with the Design Guidelines.

To ensure adequate fire protection, the maximum day demand shown in Table 1 was analyzed simultaneously with the fire flow requirements. The fire flow simulation was carried out by allowing WaterCAD® to calculate the maximum fire flow that can be drawn from the hydrant without allowing any part of the system to experience pressures less than 140 kPa (20 psi). Using the 6,000 L/min (100 L/s) boundary condition provided by the City (refer to Table 2), the system is expected to deliver a minimum of 6,000 L/min (100 L/s) within the site. Per Appendix I of TB-2018-02, adequate water supply can be provided by the hydrant to the proposed site.

2.5.3 Maximum Pressure

Based on a zero (0 L/s) demand condition, the simulation results found the maximum pressure at the site to be 437 kPa (63.4). This value is below the maximum pressure constraint of 552 kPa (80 psi), therefore pressure reducing valves (PRVs) are not anticipated to be required.

2.6 Summary and Conclusions

Based on the water simulation results, the proposed development can be serviced by the proposed 150 mm water service lateral connected to the 305 mm diameter watermain on Navan Road. Furthermore, adequate fire water supply can be achieved with the proposed hydrant off the 150 mm water lateral servicing Block 16.

3.0 WASTEWATER SERVICING

3.1 Design Criteria

The sanitary sewer system within the Block 16 is designed in accordance with the Ottawa Sewer Design Guidelines and subsequent technical bulletins. The design parameters are applied under two scenarios as per ISTB Technical Bulletin 2018-01. The key design parameters have been summarized in Table 4.

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Table 4: Wastewater Key Design Parameters

Design Parameter	Design Value
Commercial Average Flow	28,000 L/gross ha/Day
Residential Average Flow	280 L/Cap/Day
Residential Peaking Factor	Harmon's Formula
Commercial Peaking Factor	1.5
Harmon's Correction Factor (K)	0.8
Infiltration Allowance	0.33 L/s/ha
Manning's Roughness Coefficient (n)	0.013
Allowable Slopes	Varies (Refer to Section 6.1.2.2 of ODSG)
Allowable Velocities	0.6 m/s – 3.0 m/s
Allowable Freeboard	-

3.2 Proposed Sanitary Servicing and Design Flows

Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer system. Wastewater will then discharge into the East Ridge Orleans Subdivision via a 200mm sanitary sewer stub proposed as part of the subdivision as shown on the Servicing Plan.

Wastewater flows from the proposed development are presented in the Block 16 Sanitary Design Sheet (refer to Appendix D). Based on the design criteria presented in Table 3-1 the total design peak flow of 4.23 L/s is calculated for the development which is based on the site area of 0.77ha. Table 5 summarizes the results from the sanitary design sheet.

Table 5: Sanitary Design Flow Summary

Commercial Type	Site Area	Average Flow	Com. Peak Flow	Infilt. Flow	Total Flow
Commercial Flows	0.77 ha	28,000 L/gross ha/Day	0.374 L/s	0.254 L/s	0.628 L/s
Car Wash Flows	-	-	-	-	3.6 L/s
Total Wastewater Flows – Block 16					4.23 L/s

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The flows from Block 16 (4.23 L/s) were incorporated in the detailed design of the sanitary sewer within the future East Ridge Orleans Subdivision thus there is sufficient downstream capacity. It is proposed to adopt the sanitary servicing strategy described in this section.

3.3 Summary and Conclusions

Wastewater servicing for Block 16 will be designed in accordance with the City of Ottawa Sewer Design Guidelines, the associated technical bulletins, and various background documents as highlighted throughout this section. Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer outletting to the East Ridge Orleans Subdivision to the north of the site. It is recommended that this wastewater servicing plan be implemented to provide adequate sanitary servicing for the proposed development.

4.0 STORM SERVICING AND STORMWATER MANAGEMENT

4.1 Design Criteria

Storm and stormwater management servicing for the Gas Bar Site Plan (Block 16) was developed in accordance with the City of Ottawa 2012 Sewer Design Guidelines (OSDG) and the more recent Technical Bulletin PIEDTB-2016-01 (September 6, 2016). These two documents are herein referred to as the Design Guidelines in this section. A summary of the key storm and stormwater management criteria follows:

- Control minor system flows to the allowable release rates of 68 L/s as identified in Table 5-4 Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024);
- The runoff coefficients (C-factors) to be calculated based on the ratio of pervious and impervious surfaces depicted on proposed site plans;
- Minimum roadway slope of 0.1% from crest-to-crest for overland flow route;
- Maximum parking ponding depth of 350 mm (static and dynamic) as per the Design Guidelines and maximum depth of surface flow to be 300 mm;
- Minimum vertical clearance of 0.15 m between the spill elevation on the street and the finished grade;
- Major system flows, up to and including the 1:100-year design storm event, are contained within the site.
- Quality control will be accommodated by Pond #3 to meet an MECP Enhanced Level of Protection (80% TSS removal) as identified in Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).
- Ponding in landscaped areas to enhance groundwater recharge in accordance with the City of Ottawa Urban Design Guidelines for Gas Stations, Guideline 30.
- Provide measures to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

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4.2 Proposed Stormwater Management Approach

In order to achieve the allowable release rates, the stormwater management of the site will include:

- Surface storage within the site in greater than a 1:2-year event with captured flows conveyed to the minor system;
- Controlled release of the flows captured in the minor system for the entire site using Inlet Control Devices (ICDs).
- Flows stored in oversized storage pipes underground. Flows will accumulate in the storage pipes and be released from the site via an OGS and orifice control into the minor system on Paleo Drive.

4.3 Proposed Minor System Servicing

Internal to the gas station site, two minor system sewer runs will be provided on either side of the main building structure. Both of these sewer runs will be oversized and will connect into a single manhole upstream of the connection upstream of the easement stub, upstream of Paleo Drive. The connection manhole will have a 127mm diameter orifice plate on the downstream outlet sewer to control to the allowable release rate, 68 L/s, as defined in the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023). The upsized storm sewers will be 525mm diameter.

Downstream of the orifice an Stormceptor EFO4 model, or equivalent, will provide 80% TSS removal as well as capture of oils and spills.

The runoff coefficient is based on the ratio of impervious surfaces and areas. A design sheet for sizing of the sewers to confirm capacity for the 1:2-year rational method flow is provided in Appendix E.

The gas station roof structure is uncontrolled and drains directly to the minor system.

There are no basements and therefore no HGL constraints in the system.

4.4 Stormwater Management Modelling Approach

4.4.1 Dual Drainage Model

The analysis of both major and minor drainage systems was carried out to demonstrate their compliance with respect to the design criteria described in Section 4.6. The performance of the major overland system and minor storm sewer system was analyzed with PCSWMM. This software is a dynamic model which allows both hydrologic and hydraulic components to be simulated in the same platform and also allows the simulation of the interaction between the major and minor systems. The PCSWMM software platform was used to:

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- Generate the surface runoff hydrograph for each sub-area under various recurrences.
- Subdivide each inflow hydrograph into its minor and major system components based on the proposed inlet capture rates and roadway sag storage.
- Assess cascading, if any, and carry out dynamic routing of storm flows to determine flow depths along the roadways. As previously stated, the maximum major overland flow depths within the parking lot areas are to be limited to 350 mm or less, as per Technical Bulletin PIEDTB-2016-01.

PCSWMM was set-up to evaluate the proposed servicing as detailed on Drawing C01 and C02. Subcatchments were delineated for the structure roof areas, parking lot low points and landscaped low points. Model schematics are prepared in Appendix F.

4.4.2 Boundary Conditions

Boundary conditions are taken from the downstream subdivision model issued as part of the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).

The downstream 1:100-year HGL at the connection to the Subdivision at MH518A is identified as 81.28 m. The boundary condition in the model was set at this elevation as a constant during the storm simulations.

4.4.3 Ditch Drainage

The ditch drainage system surrounding the site currently contains a single 375mm diameter culvert at the proposed site access on Navan Road. Due to access requirements the culvert will have to be extended. Since the culvert is not to current Ottawa City design standards for the minimum driveway culvert, it is proposed to replace the undersized culvert with a new 600mm diameter culvert.

The outlet of the proposed access culvert will be such that a 1% grade on the existing ditch will be maintained to the downstream conveyance culvert under Navan Road. The Navan Road culvert inlet is lower than the proposed culvert outlet to facilitate conveyance of the stormwater and mitigate potential ponding in this ditch section.

4.5 Modelling Parameters

4.5.1 Hydrological Modelling Parameters

The following parameters were used in the hydrologic component of PCSWMM:

- **Areas and Imperviousness:** Catchment ID and drainage areas used by PCSWMM match those shown on either Drawing DST or Figure E-1 (Appendix

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E1). Sealed and roof areas are set at 100% impervious and other grassed or landscaped areas are pervious.

- **Catchment Width:** The catchment width is estimated at the width of overland sheet flow based on the grading of the catchment and slope direction.
- **Manning's Roughness Coefficient:** Manning's Roughness Coefficients of 0.013 and 0.25 were used for the impervious and pervious surfaces, respectively.
- **Horton Infiltration parameters:** City of Ottawa OSDG Horton Infiltration Parameters have been used in the modelling.
- **Initial Abstraction:** Initial abstraction of 4.67 mm and 1.57 mm was used for the pervious and impervious surfaces respectively, consistent with the OSDG.

4.5.2 Simulation of Storm Distributions

To assess peak flow rates and peak volume storage requirements the 3-hour Chicago storm has been simulated for the site for the 1:2-year event and 1:100-year event and the 24-hour SCS storms for the 1:100-year event.

4.6 Simulation Results

4.6.1 Low Point Ponding Analysis

Ponding depths in the low points in the parking area and landscaped areas are shown in Table 6.

Table 6: Catchbasin Ponding Depths

Low Point ID	Top of Grate (m)	Maximum Static Depth (mm)	3-hour Chicago 1:2 year Depth (mm)	3-hour Chicago 1:100 year Depth (mm)	24-hour SCS 1:100 year Depth (mm)
1	85.25	190/300	-	250	230
2A	85.35	250	-	-	-
2B	85.40	220	-	-	-
3	85.35	300	-	60	30
4	84.75	150	-	100	-
5	85.35	150	-	100	70
6	85.37	80	-	100	90

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Low Point ID	Top of Grate (m)	Maximum Static Depth (mm)	3-hour Chicago 1:2 year Depth (mm)	3-hour Chicago 1:100 year Depth (mm)	24-hour SCS 1:100 year Depth (mm)
7	84.55	300	-	300	160
8	85.04	270	-	290	250
9	84.80	300	-	190	130
10	85.18	240	-	150	90
11	85.12	300	-	210	170
12	85.45	150	-	90	70
13	85.50	150	-	160	150

The simulation results compiled in Table 6 shows that:

- No ponding nor dynamic flow will occur in the 1:2-year event;
- Maximum ponding depth of 300 mm during the 1:100-year event; and,
- There is no spill from the site in the 1:100-year event.

4.6.2 Site Release Rate

Table 8 below shows the release rates from the site via the 127mm diameter orifice plate. All release rates are below the 68 L/s allowable release rate.

Table 7: Release Rates

	3-hour Chicago 1:2 year Release Rate (L/s)	3-hour Chicago 1:100 year Release Rate (L/s)	24-hour SCS 1:100 year Release Rate (L/s)
Flow at MH 514	46	66	65

4.7 Water Quality

An OGS unit is proposed for the site to provide site specific water quality to 80% TSS removal and capture of oils and spills. The sizing details for the unit are contained in Appendix F. The unit sized for the site is an Stormceptor EFO4, or equivalent.

4.8 Summary and Conclusions

The stormwater servicing achieves a release rate from the site to the minor system limited to the allowable release rate and contains up to the 1:100 year on site.

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5.0 Erosion and Sediment Control

Erosion and sediment control measures, as outlined in the Ontario Ministry of Natural Resources (MNR) Guidelines on Erosion and Sediment Control for Urban Construction Sites, will be implemented to trap sediment on site. The following erosion and sediment control measures can be implemented during construction as shown on the Erosion and Sediment Control Plan (Drawing ESC):

- Supply and installation of a silt fence barrier, as per OPSD 219.110.
- Supply and installation of siltsack or sentinel CB inserts between the frame and cover of catch basins and maintenance holes adjacent to the project area during construction, to prevent sediment from entering the sewer system.
- Stockpiling of material during construction is to be located along flat areas away from drainage paths. For material placed on sloped areas, stockpiles are to be enclosed with a silt fence to protect watercourses.
- All catch basins are to be equipped with sumps, inspected frequently, and cleaned as required.
- Temporary ICDs are to be placed blocking part of the sewer pipe in the connecting storm maintenance holes to eliminate construction debris from entering the existing storm sewer system. The ICDs are to be removed after the proposed storm sewers have been fully cleaned.
- A mud mat is to be built at each of the site entranceways to prevent the transport of sediment onto paved surfaces. The mud mat shall be:
 - Minimum of 20 m in length for the full width of the entrance way (10 m wide minimum).
 - Minimum of 400 mm thick underlain with a geotextile (or graded aggregate filter); and
 - Constructed with 50 mm diameter clear stone for the first 10 m (extending from the paved street) and the remainder of the length with 150 mm diameter clear stone.

The proposed removal and reinstatement measures as well as the erosion control measures shall conform to the following documents:

- “Guidelines on Erosion and Sediment Control for Urban Construction Sites” published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- “MTO Drainage Manual”, Chapter F: “Erosion of Materials and Sediment Control”, Ministry of Transportation & Communications, 1985.
- “Erosion and Sediment Control” Training Manual by Ministry of Environment, Spring 1998.
- Applicable Regulations and Guidelines of the Ministry of Natural Resources.

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6.0 CONCLUSIONS

Block 16 will be serviced as follows:

- Water servicing will be provided by connection to the proposed watermain along the Navan Road.
- Wastewater servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Storm servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Flows exceeding the allowable peak flow for Block 16, will be held on-site, using a combination of both, above ground and underground storage.

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This report has been prepared by J.L. Richards & Associates Limited for 12714001 CANADA INC.'s exclusive use. Its discussions and conclusions are summary in nature and cannot properly be used, interpreted or extended to other purposes without a detailed understanding and discussions with the client as to its mandated purpose, scope and limitations. This report is based on information, drawings, data, or reports provided by the named client, its agents, and certain other suppliers or third parties, as applicable, and relies upon the accuracy and completeness of such information. Any inaccuracy or omissions in information provided, or changes to applications, designs, or materials may have a significant impact on the accuracy, reliability, findings, or conclusions of this report.

This report was prepared for the sole benefit and use of the named client and may not be used or relied on by any other party without the express written consent of J.L. Richards & Associates Limited, and anyone intending to rely upon this report is advised to contact J.L. Richards & Associates Limited in order to obtain permission and to ensure that the report is suitable for their purpose.

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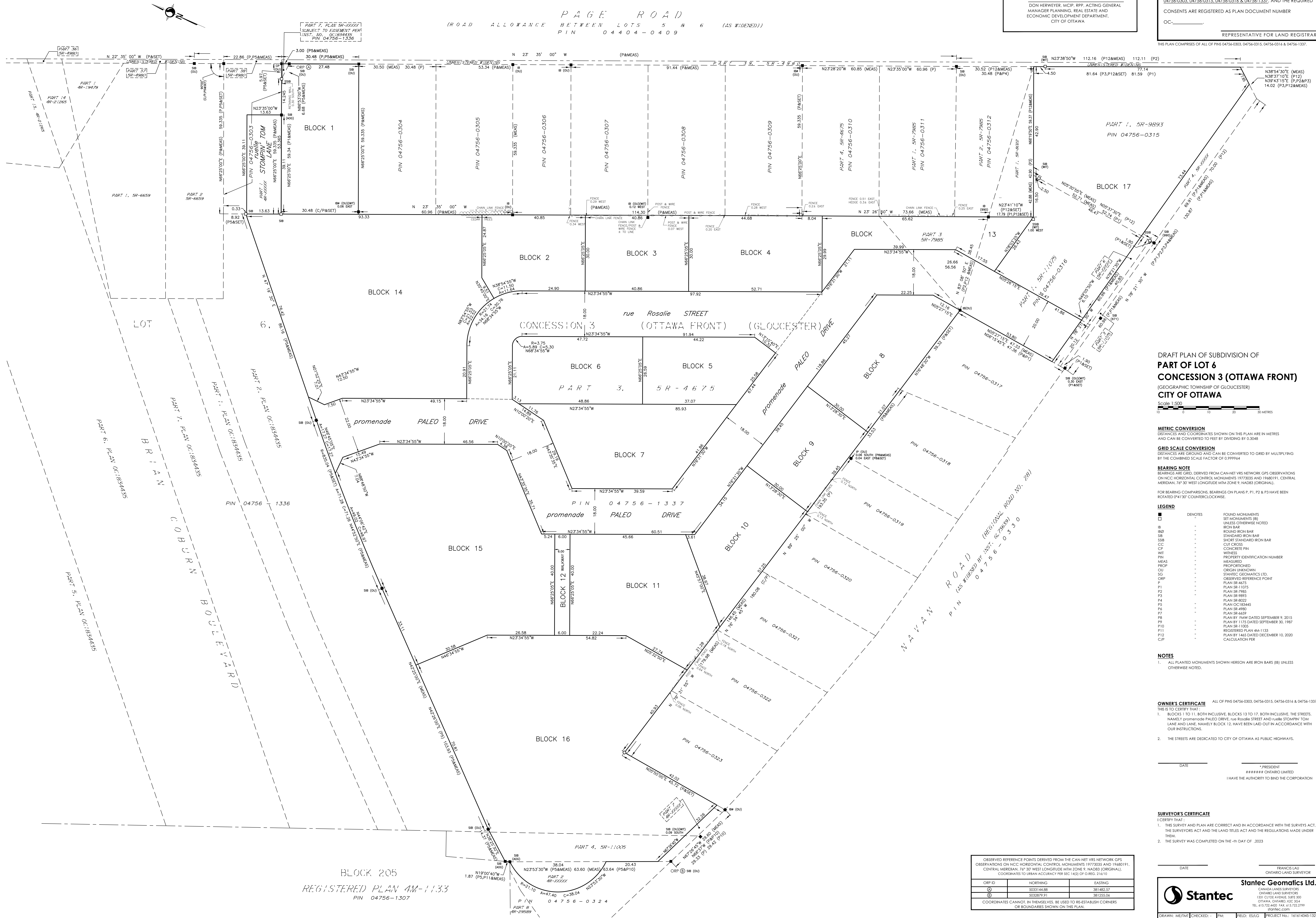
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Appendix A

Concept Plan, Draft Plan of
Subdivision and Topographical
Survey



I HEREBY CERTIFY THAT THIS PLAN 4M-_____ IS REGISTERED IN THE
 LAND REGISTRY OFFICE FOR THE LAND TITLES DIVISION OF
 OTTAWA-CARLETON (No.4) AT _____ O'CLOCK ON THE _____ DAY OF
 _____, 2023 AND ENTERED IN THE REGISTER FOR P.I.N.'S
 04756-0303, 04756-0315, 04756-0316 & 04756-1337, AND THE REQUIRED
 CONSENTS ARE REGISTERED AS PLAN DOCUMENT NUMBER
 QC-_____.

 REPRESENTATIVE FOR LAND REGISTRAR

DRAFT PLAN OF SUBDIVISION OF
PART OF LOT 6
CONCESSION 3 (OTTAWA FRONT)
(GEOGRAPHIC TOWNSHIP OF GLOUCESTER)
CITY OF OTTAWA

METRIC CONVERSION
DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES
AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

GRID SCALE CONVERSION

BEARING NOTE
 BEARINGS ARE GRID, DERIVED FROM CAN-NET VRS NETWORK GPS OBSERVATION
 ON NCC HORIZONTAL CONTROL MONUMENTS 19773035 AND 19680191, CENTRAL
 MERIDIAN, 76° 30' WEST LONGITUDE MTM ZONE 9, NAD83 (ORIGINAL).
 FOR BEARING COMPARISONS, BEARINGS ON PLANS P, P1, P2 & P3 HAVE BEEN
 ROTATED 0°41'30" COUNTERCLOCKWISE.

LEGEND	
DE	DENOTES
FO	FOUND MONUMENTS
SE	SEE MONUMENTS (B)
UN	UNLESS OTHERWISE NOTED
IB	IRON BAR
RO	ROUND IRON BAR
ST	STANDARD IRON BAR
SC	SHORT STANDARD IRON BAR
CI	CUT CROSS
CS	CROSSING PIN
WT	WITNESS
PI	PROPERTY IDENTIFICATION NUMBER
ME	MEASURED
PR	PROPORTIONED
OR	ORIGIN UNKNOWN
ST	STATIST GEOMATICS LTD.
OS	OBSERVED REFERENCE POINT
PI	PLAN 55-1075
PI	PLAN 56-475
PI	PLAN 56-785
PI	PLAN 56-895
PI	PLAN 56-922
PI	PLAN OC18345
PI	PLAN 56-490
PI	PLAN 56-459
PI	PLAN 56-144W DATED SEPTEMBER 2 1987
PI	PLAN 56-175 DATED SEPTEMBER 30 1987
PI	PLAN 52-005
PI	REGISTERED PLAN AM-1133
PI	PLAN 56-146S DATED DECEMBER 10 2020
PI	CALCULATION PER

NOTES

ALL PLANTED MONUMENTS SHOWN HEREON ARE IRON BARS (IB) UNLESS OTHERWISE NOTED.

OWNER'S CERTIFICATE ALL OF PINS 04756-0303, 04756-0315, 04756-0316 & 04756-1337

BLOCKS 1 TO 11, BOTH INCLUSIVE, BLOCKS 13 TO 17, BOTH INCLUSIVE, THE STREETS, NAMELY PROMENADE PALEO DRIVE, RUE ROSOLIE STREET AND RUE LIE STOMPIN' TOM LANE AND LANE, NAMELY BLOCK 12, HAVE BEEN LAID OUT IN ACCORDANCE WITH OUR INSTRUCTIONS.

THE STREETS ARE DEDICATED TO CITY OF OTTAWA AS PUBLIC HIGHWAYS.

DATE _____ *,PRESIDENT
 ##### ONTARIO LIMITED
 I HAVE THE AUTHORITY TO BIND THE CORPORATION

SURVEYOR'S CERTIFICATE

THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEYS ACT,
THE SURVEYORS ACT AND THE LAND TITLES ACT AND THE REGULATIONS MADE UNDER
THEM.

THE SURVEY WAS COMPLETED ON THE -1th DAY OF ,2023

DATE _____

FRANCIS LAU
ONTARIO LAND SURVEYOR

 **Stantec**

Stantec Geomatics Ltd.
 CANADA LANDS SURVEYORS
 ONTARIO LAND SURVEYORS
 1331 CLYDE AVENUE, SUITE 300
 OTTAWA, ONTARIO, K2C 3G4
 TEL. 613.722.4420 FAX. 613.722.2799
stantec.com

DRAWN: ME/TMT	CHECKED: -	PM:	FIELD: ES/LG	PROJECT No.: 161614345-132
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Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix B

Pre Consultation Meeting Notes
and Site Servicing Report
Checklist



File No.: PC2023-0227

Carmine Zayoun
12714001 Canada Inc (Zayoun Group)
Via email: carmine@zayoungroup.com

**Subject: Pre-Consultation: Meeting Feedback
Proposed Site Plan Application – 2983 Navan Road ‘
Gas Station and Commercial Building – PC2023-0227**

Please find below information regarding next steps as well as consolidated comments from the above-noted pre-consultation meeting held on September 13, 2023.

Pre-Consultation Preliminary Assessment

1 ☐	2 ☐	3 ☐	4 ☒	5 ☐
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One (1) indicates that considerable major revisions are required while five (5) suggests that the proposal appears to meet the City’s key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

Next Steps

1. A review of the proposal and materials submitted for the above-noted pre-consultation has been undertaken. Please proceed to complete a Phase 2 / Phase 3 Pre-consultation Application Form and submit it together with the necessary studies and/or plans to planningcirculations@ottawa.ca.
2. In your subsequent pre-consultation submission, please ensure that all comments or issues detailed herein are addressed. A detailed cover letter stating how each issue has been addressed must be included with the submission materials. Please coordinate the numbering of your responses within the cover letter with the comment number(s) herein.
3. Please note, if your development proposal changes significantly in scope, design, or density before the Phase 3 pre-consultation, you may be required to complete or repeat the Phase 2 pre-consultation process.

Supporting Information and Material Requirements

1. The attached **Study and Plan Identification List** outlines the information and material that has been identified, during this phase of pre-consultation, as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City’s Terms of Reference (ToR) and/or Guidelines, as available on Ottawa.ca. These ToR and Guidelines outline

the specific requirements that must be met for each plan or study to be deemed adequate.

Consultation with Technical Agencies

1. You are encouraged to consult with technical agencies early in the development process and throughout the development of your project concept. A list of technical agencies and their contact information is enclosed.

Planning

Comments:

1. In the Official Plan the subject site is designated as Neighbourhood is modified with the Evolving Neighbourhood overlay. Brian Coburn Boulevard is also designated as a Minor Corridor. The property is further identified as Low-density residential in the EUC – Phases 1 Community Design Plans (CDP). The subject lands are currently zoned GM[2546]H(14.5) General Mixed-Use, Exception.

2. Committee of Adjustment

No variances have been identified at this point. Staff will set up a meeting with a Committee of Adjustment Plan if any required.

3. Design guidelines

[Urban Design Guidelines for Drive-Through Facilities](#)

[Urban Design Guidelines for Gas Stations](#)

4. Landscape requirements

Landscape buffers will need to comply with Section 110 of the Zoning By-law and are consistent with the design guidelines.

The turning radius encroached into the landscape buffers on the demonstration plan. Ensure all landscaping is protected by barrier curbs

5. Parking requirements

Parking should comply with Sections 100, 101, 106, 109 and 111

Vehicle and bicycle parking should be situated with easily access while minimizing pavement. There seems to be a lot of pavement in front of this building.

6. Easements

Are there service easements required over the pedestrian walkway connecting this property with the subdivision?

7. Confirm that the location of the Car Wash will not become a noise problem for future residents of the townhouses behind it. Noise study will need to identify any issues and provide remediation measures.
8. Provide locations of signage and ensure that space is made available for tree planting

Urban Design

9. Relevant guidelines – The City's Urban Design Guidelines for Gas Stations and Urban Design Guidelines for Drive-Through Facilities are both applicable to this site. The applicant should ensure their submission meets the direction of these guidelines.
10. Design Brief - A Design Brief is required. Please refer to the attached Terms of Reference for details.
11. Public realm – Please refer to the attached PDF for Urban Design comments related to the location of sidewalks, walkways and pedestrian movements on the site
12. Landscaping - Extensive tree and shrub planting is need on this site, in particular to soften the interface with the existing and future residential and to enhance the ROW.

Feel free to contact the Urban Design Planner, Christopher Moise, at Christopher.Moise@ottawa.ca , for follow-up questions

Engineering

Comments:

13. General Comments:

- a. Review of the Phase 3 submission for this application will not occur until the detailed design of the subdivision that it is within (D07-16-21-0027) is approved.
- b. At the stage of site plan approval, a condition will be imposed detailing that a commence work notification will not be issued until the subdivision's infrastructure is in-service.

14. Engineering Studies:

- a. All engineering studies (detailed in the Study and Plan Identification List form) are to follow the to be approved draft plan of subdivision D07-16-21-0027.



- b. An interceptor pit is required with the provision of the car wash.
15. An MECP Environmental Compliance Approval **Industrial Sewage Works** will be required for the proposed development. Please contact the Ministry of the Environment, Conservation and Parks, Ottawa District Office to arrange a pre-submission consultation:

b. Emily Diamond at (613) 521-3450, ext. 238 or
Emily.Diamond@ontario.ca

Note: this site does not meet the City's requirements for ToR. To have the ECA application reviewed under ToR, a request will need to be sent to Charles Warnock (charles.warnock@ottawa.ca).

Feel free to contact Reed Adams (reed.adams@ottawa.ca), Infrastructure Project Manager, for follow-up questions.

Noise

Comments:

16. A stationary noise report for the car wash is required because of the adjacent residential.

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca, for follow-up questions.

Transportation

Comments:

17. A full TIA is not required as this site is covered under the recent subdivision TIA.
18. A memo including the pertinent subdivision trips is sufficient.
19. The right of way protection on Brian Coburn and Navan Roads is 40m and 37.5m, respectively

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca, for follow-up questions

Planning Forestry

Comments:

20. A Tree Conservation Report and Landscape Plan must be submitted with both SPC applications

21. A permit is required prior to any tree removal on site. The tree permit will be released upon site plan approval. Please contact the File Lead or the Planning Forester, Hayley Murray (hayley.murray@ottawa.ca) for information on obtaining the tree permit.
22. If marine clay soils are present, setbacks on City properties must adhere to the 2017 SMC guidelines (attached). The Geotechnical report must address the implications of these soils, if present, on tree planting in relation to private land.
23. If underground parking is planned, a design must be provided for the site to support tree planting
24. We expect a very strong landscape plan to re-establish canopy cover across the properties. Tree planting and protecting existing urban forest canopy is imperative to reach the City's target of 40% canopy cover.

Feel free to contact Hayley Murray, Planning Forester, for follow-up questions.

Parkland

Comments:

25. Parkland contributions were made through the Subdivision process.

Feel free to contact Jessica Button, Parks Planner, for follow-up questions.

Conservation Authority

Comments:

26. The Rideau Valley Conservation authority will be commenting on this application

Feel free to contact RVCA, for follow-up questions.

Other

27. The High Performance Development Standard (HPDS) is a collection of voluntary and required standards that raise the performance of new building projects to achieve sustainable and resilient design. The HPDS was passed by Council on April 13, 2022.
 - a. At this time, the HPDS is not in effect and Council has referred the 2023 HPDS Update Report back to staff with direction to bring forward an updated report to Committee with recommendations for revised phasing timelines, resource requirements and associated amendments to the Site Plan Control By-law by no later than Q1 2024.



- b. Please refer to the HPDS information attached and ottawa.ca/HPDS for more information.

Submission Requirements and Fees

1. Outlines the application type/subtype required and the associated fees
 - a. Additional information regarding fees related to planning applications can be found [here](#).
2. The attached **Study and Plan Identification List** outlines the information and material that has been identified as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on Ottawa.ca. These ToR and Guidelines outline the specific requirements that must be met for each plan or study to be deemed adequate.
3. All of the above comments or issues should be addressed to ensure the effectiveness of the application submission review.

Should there be any questions, please do not hesitate to contact myself or the contact identified for the above areas / disciplines.

Yours Truly,
Steve Belan

cc.

Tim Chadder
Raad Akrawi
Madelen Fellows
Karla Ferrey
Tatyana Roumie
Christopher Moise
Adam Reed
Mike Giampa
Haley Murray
Jessica Button

12714001 Canada Inc – Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash

2983 Navan Road

SITE SERVICING REPORT CHECKLIST

REFERENCED STUDIES AND REPORTS	REFERENCE
Site Servicing Report for 12714001 Canada Inc, Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash, 2983 Navan Road Road (J.L. Richards & Associates Limited, December 8, 2023)	Site Servicing Report

4.1	GENERAL CONTENT	REFERENCE
☐	Executive Summary (for larger reports only).	N/A
☒	Date and revision number of the report.	Site Servicing Report
☒	Location map and plan showing municipal address, boundary, and layout of proposed development.	Site Servicing Report (Appendix A) All Drawings
☒	Plan showing the site and location of all existing services.	Servicing Plan
☒	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Site Servicing Report
☒	Summary of Pre-consultation Meetings with City and other approval agencies.	Site Servicing Report (Appendix 'A')
☒	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Reference made to Stantec 2005 EUC ISSU
☒	Statement of objectives and servicing criteria.	Site Servicing Report
☒	Identification of existing and proposed infrastructure available in the immediate area.	Site Servicing Report Servicing Plan
☐	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	N/A

☒	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Grading Plan
☐	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
☐	Proposed phasing of the development, if applicable.	N/A
☒	Reference to geotechnical studies and recommendations concerning servicing.	Site Servicing Report and Drawings
☒	All preliminary and formal site plan submissions should have the following information: ▪ Metric scale ▪ North arrow (including construction North) ▪ Key plan ▪ Name and contact information of applicant and property owner ▪ Property limits, including bearings and dimensions ▪ Existing and proposed structures and parking areas ▪ Easements, road widening and rights-of-way ▪ Adjacent street names	All Drawings

4.2	SITE SERVICING REPORT: WATER	REFERENCE
☐	Confirm consistency with Master Servicing Study, if available.	N/A
☒	Availability of public infrastructure to service proposed development.	Site Servicing Report (Section 2.0) Servicing Plan
☒	Identification of system constraints.	Site Servicing Report (Section 2.0) Servicing Plan
☒	Identify boundary conditions.	Site Servicing Report (Section 2.0)
☒	Confirmation of adequate domestic supply and pressure.	Site Servicing Report (Section 2.0)
☒	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Site Servicing Report (Section 2.0)
☒	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Site Servicing Report (Section 2.0)

☐	Definition of phasing constraints. Hydraulic modelling is required to confirm servicing for all defined phases of the project, including the ultimate design.	N/A
☒	Address reliability requirements, such as appropriate location of shutoff valves.	Site Servicing Report (Section 2.0)
☐	Check on the necessity of a pressure zone boundary modification.	N/A
☒	Reference to water supply analysis to show that major infrastructure can deliver sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Site Servicing Report (Section 2.0)
☒	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants), including special metering provisions.	Site Servicing Report (Section 2.0) Servicing Plan
☐	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
☒	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Site Servicing Report (Section 2.0)
☒	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Site Servicing Report (Section 2.0)

4.3	SITE SERVICING REPORT: WASTEWATER	REFERENCE
☒	Summary of proposed design criteria (Note: Wet weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Site Servicing Report (Section 3.0,
☐	Confirm consistency with Master Servicing Study and/or justifications for deviations.	Stantec 2005 EUC ISSU
☐	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the Guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
☒	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Site Servicing Report (Section 3.0) Servicing Plan

☒	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable.)	Site Servicing Report (Section 3.0)
☒	Calculations related to dry weather and wet weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Site Servicing Report (Section 3.0)
☒	Description of proposed sewer network, including sewers, pumping stations and forcemains.	Site Servicing Report (Section 3.0) Servicing Plan
☐	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
☐	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
☐	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
☐	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
☐	Special considerations, such as contamination, corrosive environment, etc.	N/A

4.4	SITE SERVICING REPORT: STORMWATER	REFERENCE
☒	Description of drainage outlets and downstream constraints, including legality of outlets (i.e., municipal drain, right-of-way, watercourse, or private property).	Site Servicing Report (Section 4.0)
☒	Analysis of available capacity in existing public infrastructure.	Site Servicing Report (Section 4.0)
☒	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Servicing, Grading and Drainage Plans
☒	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Site Servicing Report (Section 4.0)

☒	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Site Servicing Report (Section 4.0)
☒	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
☐	Setback from private sewage disposal systems.	N/A
☐	Watercourse and hazard lands setbacks.	N/A
☒	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Site Servicing Report (Appendix 'A')
☐	Confirm consistency with subwatershed and Master Servicing Study, if applicable study exists.	Stantec 2005 EUC ISSU
☒	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:2 year return period) and major events (1:100 year return period).	Site Servicing Report (Section 4.0)
☐	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
☒	Calculate pre- and post-development peak flow rates, including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Site Servicing Report (Section 4.0)
☒	Any proposed diversion of drainage catchment areas from one outlet to another.	Site Servicing Report (Section 4.0)
☒	Proposed minor and major systems, including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Servicing, Grading and Drainage Plans
☐	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Quantity control proposed per Site Servicing Report (Section 4.0)
☐	Identification of potential impacts to receiving watercourses.	N/A
☐	Identification of municipal drains and related approval requirements.	N/A
☒	Description of how the conveyance and storage capacity will be achieved for the development.	Site Servicing Report (Section 4.0)

☒	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
☒	Inclusion of hydraulic analysis, including hydraulic grade line elevations.	Site Servicing Report (Section 4.0)
☒	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Site Servicing Report (Section 5.0) Servicing Plan
☐	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
☐	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5	APPROVAL AND PERMIT REQUIREMENTS	REFERENCE
The Site Servicing Report shall provide a list of applicable permits and regulatory approvals necessary for the proposed development, as well as the relevant issues affecting such approval. The approval and permitting shall include but not be limited to the following:		
☐	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams, as defined in the Act.	N/A
☐	Application for Environmental Compliance Approval (ECA) under the Ontario Water Resources Act.	As part of future submission
☐	Changes to Municipal Drains.	N/A
☐	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation, etc.).	N/A

4.6	CONCLUSION CHECKLIST	REFERENCE
☒	Clearly stated conclusions and recommendations.	Site Servicing Report
☒	Comments received from review agencies, including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	Not yet applicable

☒	All draft and final reports shall be signed and stamped by a Professional Engineer registered in Ontario.	Site Servicing Report All Drawings

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix C

Water Servicing

WATERMAIN DEMAND CALCULATION SHEET

PROJECT : NAVAN ROAD DEVELOPMENT PROJECT - GAS BAR
LOCATION : CITY OF OTTAWA
DEVELOPER : 12714001 Canada Inc.

NODE	RESIDENTIAL			NON-RESIDENTIAL	AVERAGE DAILY DEMAND (l/s)				MAXIMUM DAILY DEMAND (l/s)				PEAK HOUR DEMAND (l/s)			
	UNITS		POP'N	COMM (ha.)	Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total
	Townhouses (TH)	Condo Units (CU)														
J-14	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27
TOTALS	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27

ASSUMPTIONS	
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RESIDENTIAL DENSITIES		AVG. DAILY DEMAND		MAX. HOURLY DEMAND	
- Townhouse (TH)	2.7 p / p / u	- Residential	280 l / cap / day	- Residential	1,540 l / cap / day
		- Institutional	28,000 l / ha / day	- Institutional	75,600 l / ha / day
- Condo Units (CU)	1.8 p / p / u	- Commercial	28,000 l / ha / day	- Commercial	75,600 l / ha / day
	p / p / u	MAX. DAILY DEMAND			
		- Residential	700 l / cap / day		
	p / p / u	- Institutional	42,000 l / ha / day		
		- Commercial	42,000 l / ha / day		

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building (JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	686 m ²	Commercial area consisting of a Gas Retail and Drive Thru
C	Height in storeys	1 storeys	Basements are excluded.
	Total Floor Area	686 m ²	
D	Fire Flow Formula	F=220C√A	
	Fire Flow	4610 L/min	
	Rounded Fire Flow	5000 L/min	Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	5000 L/min	No rounding applied.
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
G	<i>North Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	4 Storey Condo Unit
	Length of Exposed Wall:	32.2 m	
	Height of Exposed Wall:	4 storeys	
	Length-Height Factor	128.6 m-storeys	
	Separation Distance	38.96 m	
	North Side Exposure Charge	5%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Non-combustible	
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	Over 45 m to next structure
	South Side Exposure Charge	0%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	Over 200 m to next structure
	West Side Exposure Charge	0%	
	Total Exposure Charge	5%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	250 L/min	
H	Fire Flow	5250 L/min	
	Rounded Fire Flow	5000 L/min	Flow rounded to nearest 1000 L/min.
City Cap (RFF)	Required Fire Flow	5000 L/min	
		83 L/s	

Fire Underwriters Survey (FUS) Fire Flow Calculations

In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	107 m ²	Commercial Buildign - Car Wash
C	Height in storeys	1 storeys	Basements are excluded.
	Total Floor Area	107 m ²	
D	Fire Flow Formula	F=220C√A	
	Fire Flow	1821 L/min	
	Rounded Fire Flow	2000 L/min	Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	2000 L/min	No rounding applied.
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
G	<i>North Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	Townhomes
	Length of Exposed Wall:	14.4 m	
	Height of Exposed Wall:	2 storeys	
	Length-Height Factor	28.9 m-storeys	
	Separation Distance	18.81 m	
	North Side Exposure Charge	12%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Non-combustible	Gas Retail/Drive Thru
	Length of Exposed Wall:	5.4 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	32.22 m	Over 45 m to next structure
	South Side Exposure Charge	5%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	Over 200 m to next structure
	West Side Exposure Charge	0%	
	Total Exposure Charge	17%	The total exposure charge is below the maximum value of 75%.
	Increase for Exposures	340 L/min	
H	Fire Flow	2340 L/min	
	Rounded Fire Flow	2000 L/min	Flow rounded to nearest 1000 L/min.
City Cap	Required Fire Flow (RFF)	2000 L/min	
		33 L/s	

Fire Underwriters Survey (FUS) Fire Flow Calculations

In accordance with City of Ottawa Technical Bulletin ITB-2018-02 dated March 21, 2018

William Rugamba

From: William Rugamba
Sent: July 15, 2024 4:00 PM
To: William Rugamba
Subject: FW: Navan Subdivision - Boundary Condition Request
Attachments: NavanSubdivision_Boundary Condition(4july2024).docx

William Rugamba, M.Eng., B.A.Sc., EIT
Civil Engineering Graduate
Ottawa, ON
Work: [343-804-4374](tel:343-804-4374)

From: Polyak, Alex <alex.polyak@ottawa.ca>
Sent: Monday, July 15, 2024 10:12 AM
To: Mahad Musse <mmusse@jlrichards.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Mahad,

Please find the boundary conditions attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., C.E.T., P.Eng. 

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Development and Building Services Department (PDBS) | Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB)

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: Mahad Musse <mmusse@jlrichards.ca>

Sent: July 12, 2024 1:31 PM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

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Hi Alex,

Just wondering if you have a status update for the boundary conditions for Navan.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca

From: Mahad Musse <mmusse@jlrichards.ca>

Sent: Wednesday, July 3, 2024 11:02 AM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

Good morning Alex,

As we discussed last week our Client is looking into the option of converting the row townhouse units into duplex units (townhouse units with apartments in the basement). As a result, this will increase the total demand on the site and we will therefore require new water boundary conditions. We'd like to note that the footprint of the blocks will not change and neither will their layout or any of the offsets.

As a summary:

- Domestic demands were calculated based on a daily consumption rate of 280 L/cap/day with peaking factors consistent with City of Ottawa Guidelines
- Required Fire Flow (RFF) was calculated in accordance to the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection and the City of Ottawa FUS protocol (Bulletin ISDTB-2014-02 & Bulletin ISDTB-2018-02), which considers material, expose distance & height. We have attached the calculation spreadsheet and the figure.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.74 L/s

Maximum Day Demand: 10.53 L/s

Peak Hour Demand: 18.17 L/s

Required Fire Flow (per FUS): 6,000 L/min (100 L/s)

Required Fire Flow (per FUS): 10,000 L/min (167 L/s)

Required Fire Flow (per FUS): 14,000 L/min (233 L/s)

Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

For your reference, the previous boundary condition received from the City is attached and below is the email chain.

If you have any questions or comments please let us know.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca

From: Polyak, Alex <alex.polyak@ottawa.ca>

Sent: Thursday, August 17, 2023 3:01 PM

To: William Rugamba <wrugamba@jlrichards.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

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Hello William,

Sorry that I missed your call, I was in a meeting. The boundary conditions are attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., P.Eng

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: William Rugamba <wrugamba@jlrichards.ca>

Sent: August 15, 2023 9:26 AM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Alex,

Just wanted to follow up on the status of this boundary request. Please let me know if you need anything else from us.

Thanks,
William

William Rugamba, M.Eng.
Civil Engineering Intern

J.L. Richards & Associates Limited
1000-343 Preston Street, Ottawa, ON K1S 1N4
Direct: 343-804-4374



From: Tatyana Roumie
Sent: Tuesday, July 25, 2023 3:53 PM
To: 'alex.polyak@ottawa.ca' <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; carmine@zayoungroup.com; Shahira Jalal <sjalal@jlrichards.ca>
Subject: Navan Subdivision - Boundary Condition Request

Hello Alex.

To support our upcoming detailed design for the site, we are requesting updated boundary conditions for the 3079 Navan Road Development.

As a brief history, we received boundary conditions from the City in July 2021 (attached, but with incorrect connection locations) and again in April 2022 (also attached) in support of the functional servicing design. We understand from the April 2022 boundary conditions that the maximum available fire flow for the site is 250 L/s.

We are currently requesting updated boundary conditions for this site as we are commencing the detailed servicing design and this request will accommodate the recent site plan changes and proposed connection points. This request is also applicable to the upcoming site plan designs which will be submitted as separate applications.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.44 L/s
Maximum Day Demand: 9.77 L/s
Peak Hour Demand: 16.50 L/s
Required Fire Flow (per FUS): 6,000 L/min (100 L/s)
Required Fire Flow (per FUS): 10,000 L/min (167 L/s)
Required Fire Flow (per FUS): 14,000 L/min (233 L/s)
Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

Thanks,
Tatyana

This e-mail originates from the City of Ottawa e-mail system. Any distribution, use or copying of this e-mail or the information it contains by other than the intended recipient(s) is unauthorized. Thank you.

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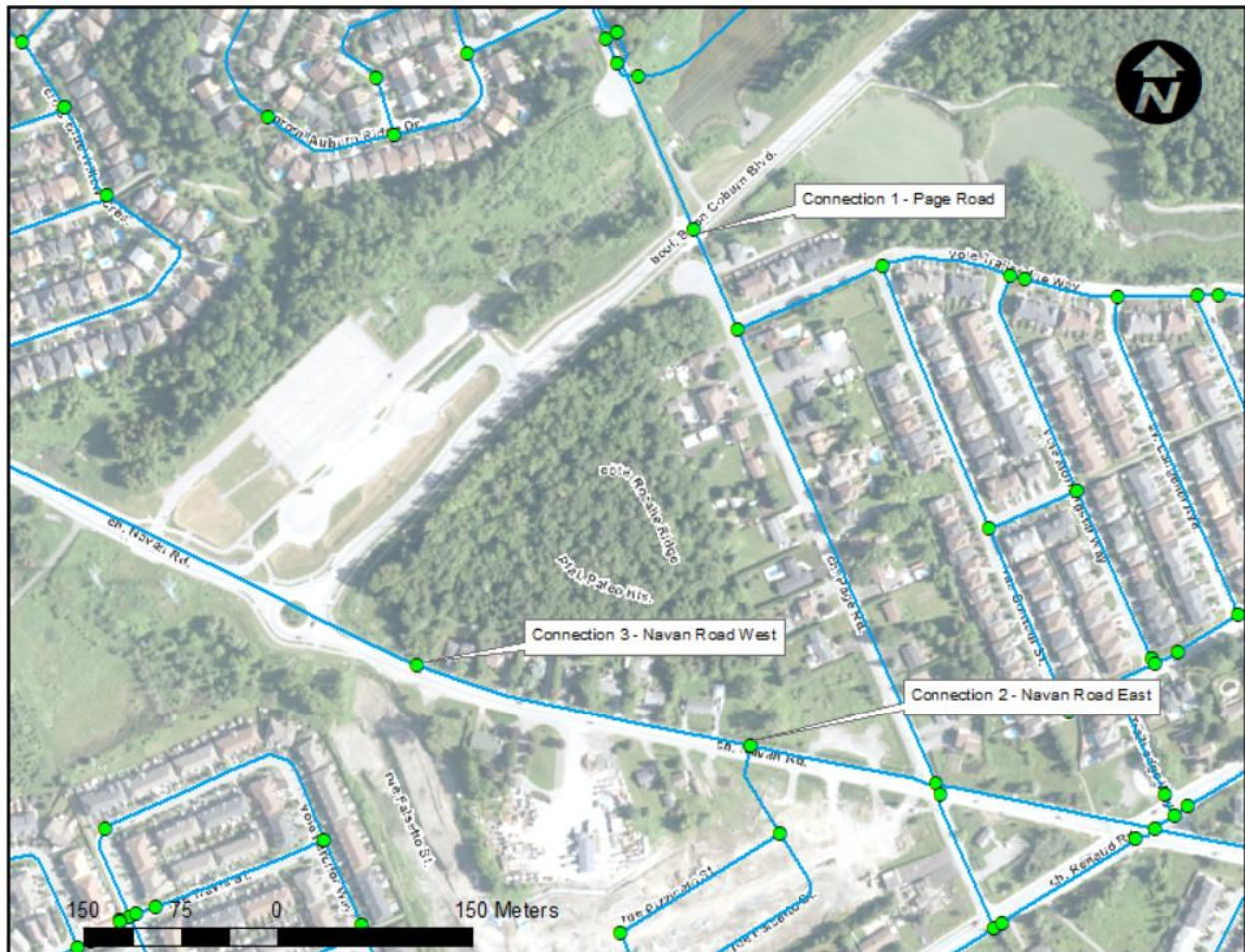
'

Boundary Conditions Navan Subdivision

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	404	6.74
Maximum Daily Demand	632	10.53
Peak Hour	1,090	18.17
Fire Flow Demand #1	6,000	100.00
Fire Flow Demand #2	10,000	166.67
Fire Flow Demand #3	14,000	233.33
Fire Flow Demand #4	15,000	250.00

Location



Results

Connection 1 - Page Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	64.0
Peak Hour	127.0	58.6
Max Day plus Fire Flow #1	128.2	60.4
Max Day plus Fire Flow #2	126.8	58.3
Max Day plus Fire Flow #3	124.9	55.7
Max Day plus Fire Flow #4	124.4	55.0

¹ Ground Elevation = 85.7 m

Connection 2 - Navan Road East

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	71.4
Peak Hour	126.8	65.9
Max Day plus Fire Flow #1	127.7	67.1
Max Day plus Fire Flow #2	125.5	64.1
Max Day plus Fire Flow #3	122.7	60.1
Max Day plus Fire Flow #4	121.9	58.9

¹ Ground Elevation = 80.5 m

Connection 3 - Navan Road West

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	69.3
Peak Hour	126.8	63.8
Max Day plus Fire Flow #1	127.3	64.5
Max Day plus Fire Flow #2	124.6	60.6
Max Day plus Fire Flow #3	120.9	55.3
Max Day plus Fire Flow #4	119.8	53.8

¹ Ground Elevation = 81.9 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermain deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

The map shows a street layout with several buildings and a proposed water main line. The main line is shown in pink, running from the bottom left towards the center. A hydrant, labeled 'H-19', is marked with a pink cross at the intersection of the main line and a side street. Other hydrants are marked with black dots: 'J-14' is located near a building at the top, and 'J-13' is located near a building at the bottom. The map also shows a parking lot with several spaces, a building with a ramp, and a street labeled 'Connection 3 - Navan Rd. West'. A legend on the right side of the map indicates that pink lines represent 'Proposed Water Main' and blue lines represent 'Existing Water Main'. The map is titled 'Map of Connection 3 - Navan Rd. West'.

Color Coding Legend

Pipe: Diameter (mm)

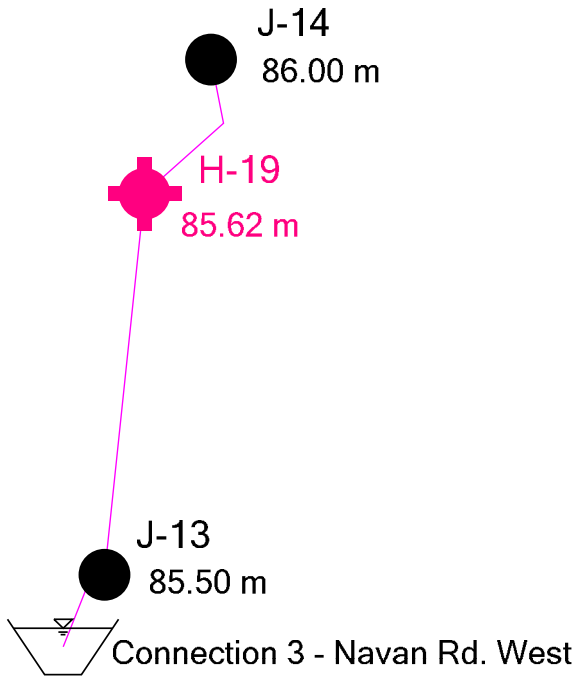
	<=	155
	<=	204
	<=	297
		Other

Gas Bar, Commercial Building, and Car Wash (Block 16)

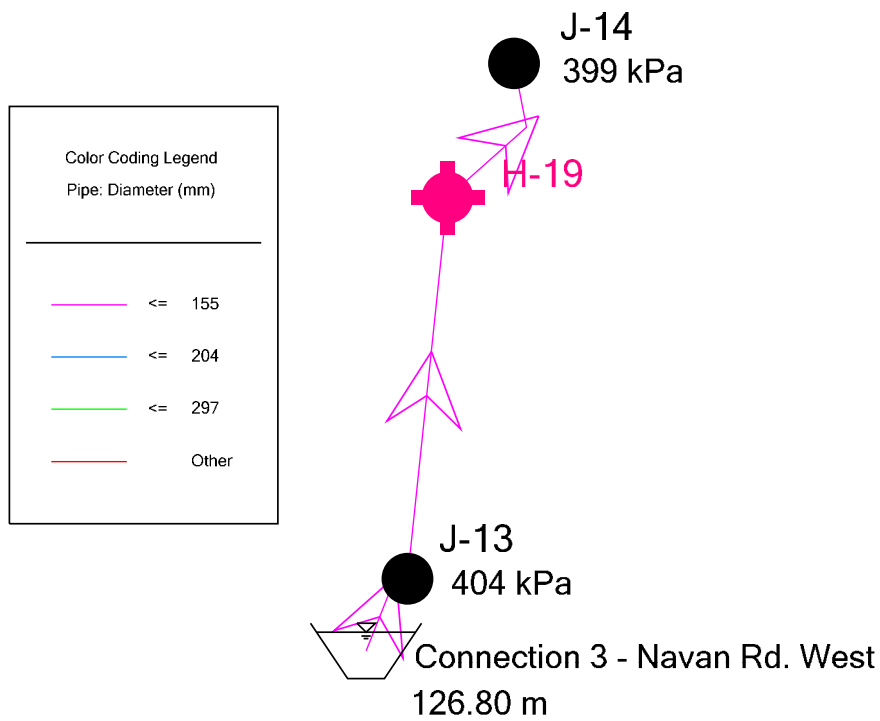
Model Schematic

Elevation Model

Color Coding Legend	
Pipe: Diameter (mm)	
	<= 155
	<= 204
	<= 297
	Other



Gas Bar, Commercial Building, and Car Wash (Block 16) Peak Hour Demand



Gas Bar, Commercial Building, and Car Wash (Block 16)

Peak Hour Demand

Junction Table

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	4.27	126.74	399
J-13	85.50	0.00	126.79	404

Gas Bar, Commercial Building, and Car Wash (Block 16)

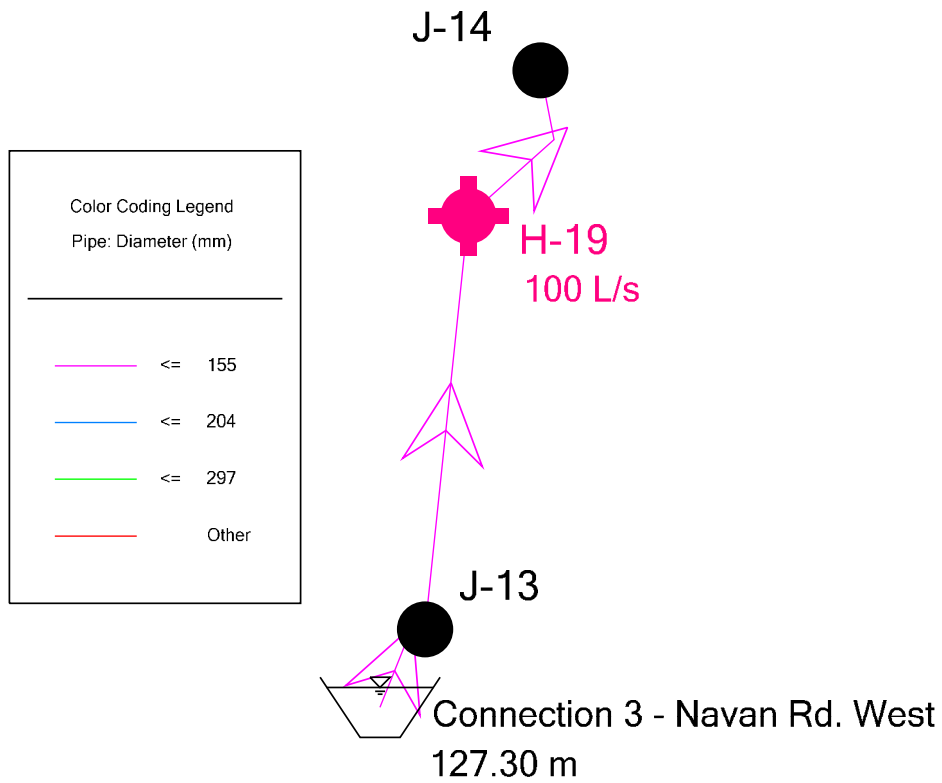
Peak Hour Demand

Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	4.27	0.23
175	P-21(1)	49	155	PVC	100.0	4.27	0.23
176	P-21(2)	22	155	PVC	100.0	4.27	0.23

Gas Bar, Commercial Building, and Car Wash (Block 16)

Max Day + Fire Flow Requirement



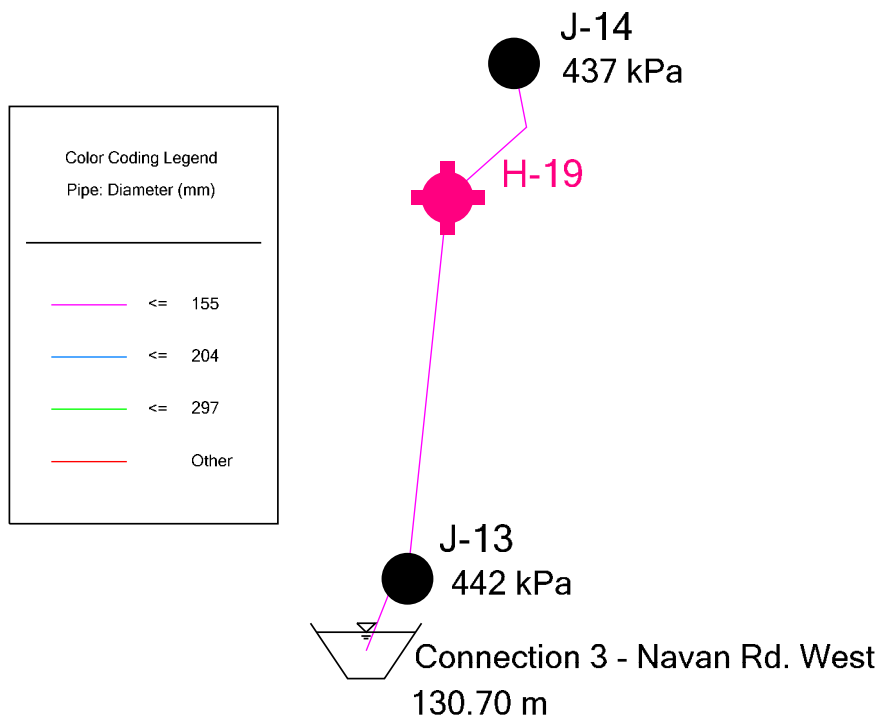
Gas Bar, Commercial Building, and Car Wash (Block 16)

Max Day + Fire Flow Requirement

Label	Satisfies Fire Flow Constraints?	Fire Flow (Available) (L/s)	Flow (Total Available) (L/s)	Pressure (Residual Lower Limit) (kPa)	Pressure (Calculated Residual) (kPa)	Pressure (Calculated System Lower Limit) (kPa)	Junction w/ Minimum Pressure (System)
H-19	True	100	100	140	224	238	J-14

Gas Bar, Commercial Building, and Car Wash (Block 16)

Maximum Pressure Analysis



Gas Bar, Commercial Building, and Car Wash (Block 16)

Maximum Pressure Analysis

Junction Table

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	0	130.70	437
J-13	85.50	0	130.70	442

Gas Bar, Commercial Building, and Car Wash (Block 16)

Maximum Pressure Analysis

Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	0	0.00
175	P-21(1)	49	155	PVC	100.0	0	0.00
176	P-21(2)	22	155	PVC	100.0	0	0.00

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix D

Sanitary Design Sheet

Street Name	MH No.		Residential								Commercial/Institutional					Infiltration			Peak Design Flow L/s	Pipe Data								Upstream Geometry				Downstream Geometry					
	From	To	Multiples	Apartments	Area (ha)	Pop.	Cum. Pop.	Cum. Area (ha)	Peaking Factor	Residential Flow (L/s)	Area (ha)	Cum. Area (ha)	Peaking Factor	Inst. Flow (L/s)	Plug Flow (L/s)	Area (ha)	Cum. Area (ha)	Peak Extr. Flow L/s		Dia	Type	Actual Diameter	Slope	Q Full (L/s)	V Full	Length	Residual Capacity	% Full	TG From	Obvert	Invert	Cover	TG TO	Drop	Obvert	Invert	Cover
GAS STATION	GAS BAR	22			0.00	0	0	0.00	3.80	0.00	0.77	0.77	1.50	0.37		0.77	0.77	0.25	0.63	200	Circular	203.20	1.00%	34.22	1.06	27.26	33.59	2%	86.000	83.369	83.165	2.631	85.590	0.510	83.096	82.893	2.494
GAS STATION	CAR WASH	22			0.00	0	0	0.00	3.80	0.00		0.00	1.50	0.00	3.60	0.00	0.00	0.00	3.60	200	Circular	203.20	1.00%	34.22	1.06	5.91	30.62	11%	85.770	82.705	82.502	3.065	85.590	0.060	82.646	82.443	2.944
GAS STATION TO EXISTING STUB	22	STUB 16			0.00	0	0	0.00	3.80	0.00		0.77	1.50	0.37	3.60	0.00	0.77	0.25	4.23	200	Circular	203.20	0.65%	27.59	0.85	2.02	23.36	15%	85.590	82.586	82.383	3.004	85.350		82.573	82.370	2.777
EXISTING STUB TO PALEO DRIVE	STUB 16	21			0.04	0	0	0.04	3.80	0.00		0.77	1.50	0.37	3.60	0.04	0.81	0.27	4.24	200	Circular	203.20	0.65%	27.59	0.85	38.75	23.34	15%	85.350	82.573	82.370	2.777	85.041		82.321	82.118	2.720

Design Parameters	
Single Family Population	3.4 Cap/Unit
Semi-Detached/Townhouse Population	2.7 Cap/Unit
Apartments Population	1.8 Cap/Unit
Residential Flows	280 L/Cap/Day
Infiltration Flows	0.33 L/s/ha
Correction Factor	0.8
Commercial Peak Factor	1.5
Institutional/Commercial Average Flow	28000 L/gross ha/d
Manning Coefficient	0.013

Street Name	MH No.		Residential								Commercial/Institutional					Park/Roads			Infiltration			Peak Design Flow L/s	Pipe Data							Upstream Geometry				Downstream Geometry				Self-Cleansing Velocities							
	From	To	Multiples	Apartments	Area (ha)	Pop.	Cum. Pop.	Cum. Area (ha)	Peaking Factor	Residential Flow (L/s)	Area (ha)	Cum. Area (ha)	Peaking Factor	Inst. Flow (L/s)	Plug Flow (L/s)	Area (ha)	Cum. Area (ha)	Plug Flow (L/s)	Area (ha)	Cum. Area (ha)	Peak Extr. Flow L/s		Dia	Type	Actual Diameter	Slope	Q Full (L/s)	V Full	Length	Residual Capacity	% Full	TG From	Obvert	Invert	Cover	TG TO	Drop	Obvert	Invert	Cover	Op/Qf Ratio	Flow Depth (mm)	Actual Velocity (m/s)	Flow Depth to Diameter Ratio (d/D)	
OUTLET TO PAGE (CUL-DU-SAC @ Brian Coburn)																																													
STOMPIN' TOM LANE	24	23A	8	8	0.25	36	36	0.25	3.67	0.43	0.00	1.50	0.00			0.00			0.25	0.25	0.08	0.59	200	Circular	203.20	2.00%	48.39	1.49	54.95	47.80	1%	85.698	83.493	83.290	2.205	85.346		82.394	82.191	2.952	0.01	15.65	0.51	0.08	
STOMPIN' TOM LANE	23A	23				0	36	0.25	3.67	0.43	0.00	1.50	0.00			0.00			0.00	0.25	0.08	0.59	200	Circular	203.20	2.00%	48.39	1.49	11.18	47.80	1%	85.346	82.394	82.191	2.952	85.350	0.300	82.170	81.967	3.179	0.01	15.65	0.51	0.08	
Page Road (2690 to Navan Road)	23	EX10			5.94	81	117	6.19	3.55	1.36		0.00	1.50	0.00		0.00			5.94	6.19	2.04	3.40																							
OUTLET TO PAGE @ NAVAN																																													
Site Plan - BLOCK 15	SAN STUB 15	13		83	0.45	149	149	0.45	3.55	1.72	0.09	0.09	1.50	0.05		0.00			0.54	0.54	0.18	1.94	250	Circular	254.00	1.50%	75.98	1.50	13.47	74.04	3%	85.000	82.366	82.112	2.634	85.065	0.259	82.164	81.910	2.901	0.03	27.94	0.64	0.11	
PALEO DRIVE	13	12			0.14	0	149	0.59	3.55	1.72		0.09	1.50	0.05		0.00			0.14	0.68	0.22	1.99	200	Circular	203.20	1.20%	37.48	1.16	5.15	35.50	5%	85.065	81.905	81.702	3.160	84.982		81.843	81.640	3.138	0.05	31.70	0.61	0.16	
Easement - BLOCK 12	SAN STUB 16	21			0.04	0	0	0.04	3.80	0.00	0.77	0.77	1.50	0.37	3.60		0.00		0.81	0.81	0.27	4.24	200	Circular	203.20	0.65%	27.59	0.85	38.75	23.34	15%	85.380	82.574	82.370	2.776	85.041		82.322	82.118	2.719	0.15	53.85	0.62	0.27	
Easement- BLOCK 12	21	12				0	0	0.00	3.80	0.00	0.77	0.77	1.50	0.37	3.60		0.00		0.00	0.81	0.27	4.24	200	Circular	203.20	0.60%	26.50	0.82	13.09	22.26	16%	85.041	82.322	82.118	2.719	84.982	0.400	82.243	82.040	2.738	0.16	54.98	0.60	0.27	
PALEO DRIVE	12	11	6	6	0.23	27	176	0.82	3.53	2.02		0.86	1.50	0.42	3.60		0.00		0.23	1.72	0.57	6.60	200	Circular	203.20	0.50%	24.19	0.75	37.41	17.59	27%	84.982	81.843	81.640	3.138	84.909		81.656	81.453	3.253	0.27	72.34	0.63	0.36	
PALEO DRIVE	11	10	3	3	0.16	14	190	0.98	3.52	2.17		0.86	1.50	0.42	3.60		0.00		0.16	1.88	0.62	6.81	200	Circular	203.20	0.50%	24.19	0.75	10.53	17.38	28%	84.909	81.656	81.453	3.253	84.935		81.603	81.400	3.332	0.28	73.56	0.64	0.36	
PALEO DRIVE	10	09	12	12	0.39	54	244	1.37	3.49	2.76		0.86	1.50	0.42	3.60	0.20	0.20		0.59	2.47	0.82	7.60	200	Circular	203.20	0.55%	25.40	0.78	81.88	17.80	30%	84.935	81.603	81.400	3.332	84.248	0.321	81.152	80.949	3.096	0.30	76.00	0.68	0.37	
Site Plan - BLOCK 14	SAN STUB 14	17		84	0.48	151	151	0.48	3.55	1.74	0.09	0.09	1.50	0.05		0.00			0.57	0.57	0.19	1.97	200	Circular	203.20	1.50%	41.91	1.29	8.26	39.94	5%	84.600	82.437	82.234	2.163	84.945	0.060	82.313	82.110	2.632	0.05	29.87	0.66	0.15	
ROSALIA STREET	17	16	31	31	0.95	140	291	1.43	3.47	3.27		0.09	1.50	0.05		0.00			0.95	1.52	0.50	3.82	200	Circular	203.20	0.86%	31.79	0.98	111.68	27.97	12%	84.945	82.253	82.050	2.692	84.030	0.201	81.289	81.086	2.741	0.12	47.35	0.66	0.23	
ROSALIA STREET	16	09	2	2	0.07	9	300	1.50	3.46	3.37		0.09	1.50	0.05		0.00			0.07	1.59	0.52	3.94	200	Circular	203.20	0.66%	27.80	0.86	16.49	23.86	14%	84.030	81.088	80.885	2.942	84.248	0.148	80.979	80.776	3.269	0.14	51.61	0.61	0.25	
PALEO DRIVE	09	08	5	5	0.15	23	567	3.01	3.36	6.17		0.96	1.50	0.46	3.60		0.20		0.15	4.21	1.39	11.62	200	Circular	203.20	0.35%	20.24	0.62	25.46	8.62	57%	84.248	80.831	80.628	3.417	83.267	0.100	80.742	80.539	2.525	0.57	110.34	0.65	0.54	
PALEO DRIVE	08	07			0.25	0	567	3.26	3.67	6.75		0.96	1.50	0.46	3.60		0.20		0.25	4.46	1.47	12.29	200	Circular	203.20	0.35%	20.24	0.62	14.54	7.96	61%	83.267	80.642	80.439	2.625	82.688		80.591	80.388	2.097	0.61	114.20	0.65	0.56	
PALEO DRIVE	07	06				0	567	3.26	3.36	6.17		0.96	1.50	0.46	3.60		0.20		0.00	4.46	1.47	11.70	200	Circular	203.20	0.35%	20.10	0.62	19.95	8.39	56%	82.688	80.591	80.388	2.097	82.496	0.300	80.522	80.319	1.974	0.58	111.35	0.64	0.55	
PALEO DRIVE	06	03				0	567	3.26	3.36	6.17		0.96	1.50	0.46	3.60		0.20		0.00	4.46	1.47	11.70	200	Circular	203.20	0.36%	20.39	0.63	22.88	8.68	57%	82.496	80.222	80.019	2.274	82.456	0.404	80.141	79.938	2.315	0.57	110.34	0.65	0.54	
Site Plan - BLOCK 17	SAN STUB 17	03		96	0.56	173	173	0.56	3.54	1.98		0.00	1.50	0.00		0.00			0.56	0.56	0.18	2.17	200	Circular	203.20	1.50%	41.91	1.29	10.30	39.74	5%	82.800	79.952	79.749	2.848	82.456	0.060	79.797	79.594	2.659	0.05	31.29	0.68	0.15	
PALEO DRIVE	03	02			0.07	0	740	3.89	3.30	7.92		0.96	1.50	0.46	3.60		0.20		0.07	5.09	1.68	13.67	200	Circular	203.20	0.33%	19.75	0.61	57.98	6.08	69%	82.456	79.737	79.534	2.719	81.831	0.070	79.544	79.341	2.287	0.69	124.16	0.66	0.61	
NAVAN ROAD	02	01				0	740	3.89	3.30	7.92		0.96	1.50	0.46	3.60		0.20		0.00	5.09	1.68	13.67	200	Circular	203.20	0.35%	20.24	0.62	114.96	6.57	68%	81.831	79.474	79.271	2.357	81.523		79.072	78.868	2.451	0.68	122.12	0.67	0.60	
NAVAN ROAD	01	EX10			5.82	174	914	9.71	3.26	9.66		0.96	1.50	0.46	3.60	0.20	0.20		5.82	10.81	3.60	17.32	200	Circular	203.20	0.55%	25.38	0.78	45.18	8.05	68%	81.523	79.072	78.868	2.451	81.585		78.823	78.620	2.761	0.68	123.14	0.64	0.61	
Page Road (Navan to Renaud)	EX10	Renaud					1031	15.90	3.23	10.80		0.96	1.50	0.46	4.00		0.20		0.00	17.10	5.64	20.91	250	Circular	254.00	0.74%	63.19	1.05	106.80	32.28	39%														
			67	330	5.34	776					0.96								17.10	16.86																									

Design Parameters		
Single Family Population	3.4	Cap/Unit
Semi-Detached/Townhouse Population	2.7	Cap/Unit
Apartments Population	1.8	Cap/Unit
Residential Flows	280	L/Cap/Day
Infiltration Flows	0.33	L/s/ha
Correction Factor	0.8	-
Commercial Peak Factor	1.5	-
Institutional/Commercial Average Flow	28000	L/gross ha/d
Manning Coefficient	0.013	

PER EUC Area 13A: 6.60
Area 13B: 10.50
Total: 17.10

Subdivision Area = 3.86 ha
Site Plans Areas = 1.48 ha
Subdivision + Site Plan Areas = 5.34 ha
EUC Areas = 11.76 ha
Total Area = 17.10 ha

Note: 5.34 ha is total draft plan area + additional flows on stompin' tom lane

Sanitary Inv Ex MH 13 Page Rd	81.13
Sanitary Inv Ex MH 14 Page Rd	81.83
Sanitary Inv at Ex MH 10 Page @ Navan	78.620



Friday, August 16, 2024

J. L. Richards
1000-343 Preston St.
Ottawa, Ontario, K1S 1N4

Attn: W. Rugamba

Re: 2983 Navan Road, Ontario - Gas Bar-Commercial-QM&E Project # 23-047
Confirmations

Dear William,

This letter serves as confirmation of items you sent by e-mail August 12, 2024.

1. Maximum domestic water flow rate is 57 gpm (3.60 L/s).
2. That quantity was arrived at by referring to data provided by the client's car wash specialist.
3. The design of the oil interceptor has been completed, it will be located entirely withing the walls of the car wash building.

Please do not hesitate to contact us should you have any questions or comments.

Yours truly,

A handwritten signature in black ink, appearing to read 'CW Clark'.

C. W. Clark, P.Eng
QM&E Engineering
CC: by you to whom it may concern

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

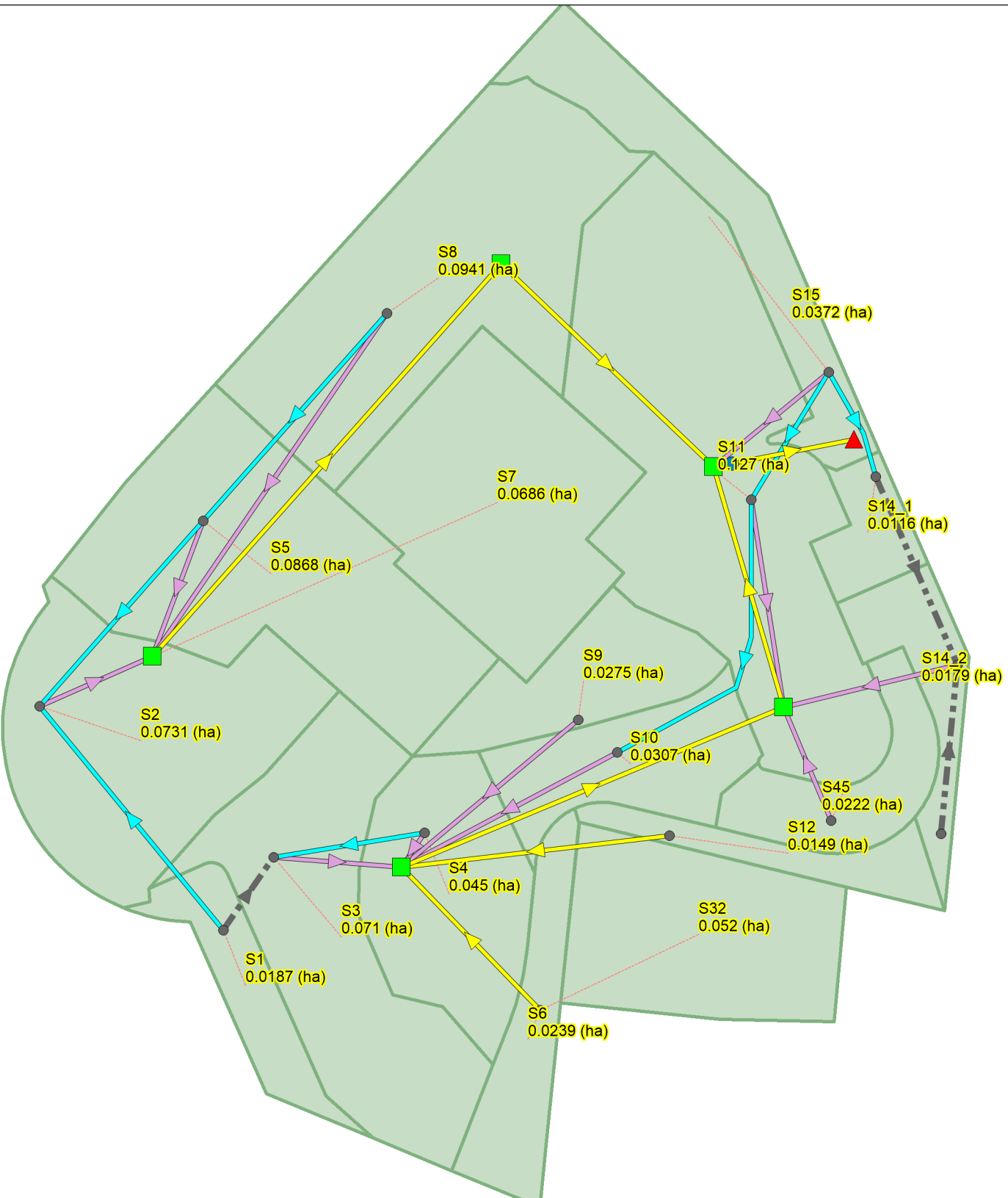
Appendix E

Storm Design Sheet

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix F

Stormwater Management



Legend

- Junctions

▲ Outfalls

Storages

■ Manholes

● Catchbasins
- Conduits

— Storm Sewers

— Storm Sewers

— Weirs

— Outlets

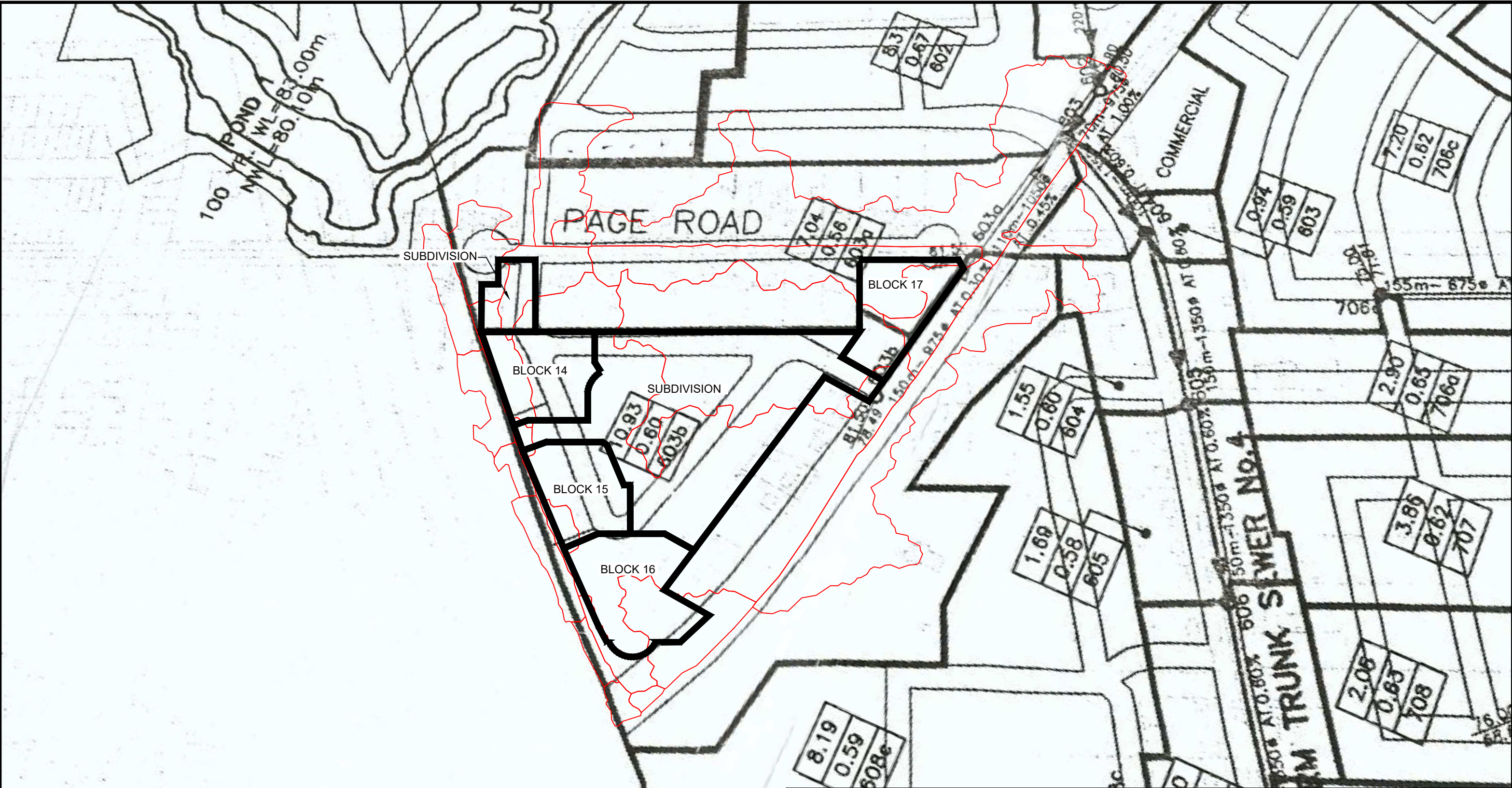
■ Subcatchments



35 m

PROJECT:				2983 Navan Road - Block 16 Ottawa ON			
DRAWING:				Overall System Model Schematic			
 J.L.Richards ENGINEERS · ARCHITECTS · PLANNERS		This drawing is copyright protected and may not be reproduced or use for purposes other than execution of the described work without the express written consent of J.L. Richards & Associates Limited.		DESIGN:	ML	JLR NO.: 29899-002	
				DRAWN:	ML	DRAWING NO.: Figure 3	
				CHECKED:	BP		

File Location: P:\29000\29899-002 - Navan Subdivision\05-Production\01-Civil\29899-002 C PRE-DST.dwg



LEGEND:

- PRE-DEVELOPMENT DRAINAGE AREAS
- NAVAN SITE PLAN AND SUBDIVISION BOUNDARY

NOTE:
UNDERLYING CATCHMENT DELINEATION FROM GLOUCESTER EUC INFRASTRUCTURE
SERVICING UPDATE 2005 WHICH INFORMED THE ALLOWABLE RELEASE RATES FROM
THE SITE AND NO PRE-DEVELOPMENT MODELLING WAS REQUIRED

PROJECT: 2983, 3053 AND 3079 NAVAN RD & 2690 PAGE RD

DRAWING: PRE-DEVELOPMENT DRAINAGE PLAN



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DESIGN: BP
DRAWN: KC
CHECKED: BP
JLR #: 29899-002

DRAWING #:
FIGURE

PLOT DATE: July 26, 2024 2:27:18 PM

Post-Development 3-hour Chicago 1:2 year Event

[TITLE] ;;Project Title/Notes														
[OPTIONS] ;;Option Value FLOW UNITS LPS INFILTRATION HORTON FLOW ROUTING DYNWAVE LINK OFFSETS ELEVATION MIN SLOPE 0 ALLOW FLOODING NO SKIP STEADY_STATE NO														
START DATE 01/01/2000 START TIME 00:00:00 REPORT START DATE 01/01/2000 REPORT START TIME 00:00:00 END DATE 01/01/2000 END TIME 06:00:00 SWEEP START 01/01 SWEEP END 12/31 DRY DAYS 0 REPORT STEP 00:01:00 WET STEP 00:01:00 DRY STEP 00:01:00 ROUTING STEP 1 RULE STEP 00:00:00														
INERTIAL DAMPING PARTIAL NORMAL FLOW LIMITED BOTH FORCE MAIN EQUATION H-W VARIABLE STEP 0.75 LENGTHENING STEP 0 MIN SURFAREA 0 MAX TRIALS 8 HEAD TOLERANCE 0.0015 SYS_FLOW_TOL 5 LAT_FLOW_TOL 5 MINIMUM_STEP 0.5 THREADS 12														
[EVAPORATION] ;;Data Source Parameters ;;----- CONSTANT 0.0 DRY_ONLY NO														
[RAINGAGES] ;;Name Format Interval SCF Source ;;----- 3CHI002 INTENSITY 0:10 1.0 TIMESERIES 3CHI002 3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100														
[SUBCATCHMENTS] ;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen ;;Subcatchment SubArea ;;-----														
S1 3CHI002 CB208 0.0187 0.061 23.61 2.2 0 S10 3CHI002 CB204 0.0307 99.617 55.32 2.2 0 S11 3CHI002 CB202 0.127 86.66 65.66 2.2 0 S12 3CHI002 CB213 0.0149 0.253 6.44 2.2 0 S14_1 3CHI002 CB211 0.0116 52.747 22.78 2.2 0 S14_2 3CHI002 CB212 0.0179 31.754 58.12 2.2 0 S15 3CHI002 CB210B 0.0372 25.766 108.74 2.2 0 S2 3CHI002 CB209 0.0731 63.05 33.61 2.2 0 S3 3CHI002 CB207 0.071 96.627 45.79 2.2 0 S32 3CHI002 CB214 0.052 14.286 29.7 2.2 0 S4 3CHI002 CB206 0.045 99.974 24.67 2.2 0 S45 3CHI002 CB203 0.0222 99.961 14.32 2.2 0 S5 3CHI002 CB200 0.0868 89.897 82.96 2.2 0 S6 3CHI002 CB214 0.0239 0.002 40.3 2.2 0 S7 3CHI002 520_ (P-Stm) 0.0686 100 25.95 2.2 0 S8 3CHI002 CB201 0.0941 80.117 35.06 2.2 0 S9 3CHI002 CB205 0.0275 76.829 19.08 2.2 0														
[SUBAREAS] ;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted ;;-----														
S1 0.013 0.25 1.57 4.67 0 OUTLET S10 0.013 0.25 1.57 4.67 0 OUTLET S11 0.013 0.25 1.57 4.67 0 OUTLET S12 0.013 0.25 1.57 4.67 0 OUTLET S14_1 0.013 0.25 1.57 4.67 0 PERVIOUS 100 S14_2 0.013 0.25 1.57 4.67 0 PERVIOUS 100 S15 0.013 0.25 1.57 4.67 0 PERVIOUS 100 S2 0.013 0.25 1.57 4.67 0 OUTLET S3 0.013 0.25 1.57 4.67 0 OUTLET S32 0.013 0.25 1.57 4.67 0 PERVIOUS 80 S4 0.013 0.25 1.57 4.67 0 OUTLET S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 S6 0.013 0.25 1.57 4.67 0 OUTLET S7 0.013 0.25 1.57 4.67 0 OUTLET S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100 S9 0.013 0.25 1.57 4.67 0 OUTLET														
[INFILTRATION] ;;Subcatchment Param1 Param2 Param3 Param4 Param5 ;;-----														
S1 76.2 13.2 4.14 7 0 S10 76.2 13.2 4.14 7 0 S11 76.2 13.2 4.14 7 0 S12 76.2 13.2 4.14 7 0 S14_1 76.2 13.2 4.14 7 0 S14_2 76.2 13.2 4.14 7 0 S15 76.2 13.2 4.14 7 0 S2 76.2 13.2 4.14 7 0 S3 76.2 13.2 4.14 7 0 S32 76.2 13.2 4.14 7 0 S4 76.2 13.2 4.14 7 0 S45 76.2 13.2 4.14 7 0 S5 76.2 13.2 4.14 7 0 S6 76.2 13.2 4.14 7 0 S7 76.2 13.2 4.14 7 0 S8 76.2 13.2 4.14 7 0 S9 76.2 13.2 4.14 7 0														
[JUNCTIONS] ;;Name Elevation MaxDepth InitDepth SurDepth Aponded 518_Orifice 81.361 4.106 0 0 0														
[OUTFALLS] ;;Name Elevation Type Stage Data Gated Route To ;Cylindrical Structure Slab Top Circular Frame SI 518A_ (P-Stm) 80.646 FIXED 81.2 NO														
[STORAGE] ;;Name Elev. Psi MaxDepth Ksat InitDepth Shape Curve Name/Params SurDepth Evap ;;-----														
;Cylindrical Structure Slab Top Circular Frame SI 518_ (P-Stm) 81.061 4.406 0.139 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI 519_ (P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI 520_ (P-Stm) 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI														
521_ (P-Stm) 81.957 3.673 0 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI 522_ (P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0 0 ;85.18000000001676 CB200 83.18 2.32 0 TABULAR CB200 0 0 ;85.50000000001435 CB201 83.09 2.66 0 TABULAR CB201 0 0 ;85.36 CB202 82.76 2.89 0 TABULAR CB202 0 0 ;85.35000000000931 CB203 82.75 2.9 0 TABULAR CB203 0 0 ;85.3500080822731 CB204 82.75 2.75 0 TABULAR CB204 0 0 ;85.4500080823662 CB205 82.85 2.75 0 TABULAR CB205 0 0 ;85.48 CB206 82.77 2.73 0 TABULAR CB206 0 0 ;85.05 CB207 83.15 2.3 0 TABULAR CB207 0 0 ;84.9000080821652 CB208 83.3 2.04 0 TABULAR CB208 0 0 ;84.8 CB209 83.2 1.9 0 TABULAR CB209 0 0 ;85.25 CB210B 82.65 3 0 TABULAR CB210B 0 0 ;85.35 CB211 83.7 1.9 0 TABULAR CB211 0 0 ;85.4 CB212 83.2 2.42 0 TABULAR CB212 0 0 ;85.50000000000554 CB212A 83.62 2 0 TABULAR CB212A 0 0 ;84.75 CB213 83.15 1.75 0 TABULAR CB213 0 0 ;84.55 CB214 82.95 1.9 0 TABULAR CB214 0 0														
[CONDUITS] ;;Name From Node To Node Length Roughness InOffset OutOffset InitFlow MaxFlow ;;-----														
C2 CB208 CB207 9.666 0.013 83.3 83.2 0 0 CB212A CB212 18.475 0.013 83.62 83.25 C4 0 CB211 CB212 22.346 0.013 83.7 83.25 C5 0 CB213 522_ (P-Stm) 29.34 0.013 83.15 82.757 0 CB214 522_ (P-Stm) 21.65 0.013 82.95 82.757 0 0 ;PVC Pipes Pipe_-_ (104)_ (P-Stm)_2 518_Orifice 518A_ (P-Stm) 15.623 0.013 81.361 81.322 0 0 ;PVC Pipes Pipe_-_ (110)_ (P-Stm) 521_ (P-Stm) 518_ (P-Stm) 27.209 0.013 82.257 82.161 0 0 ;PVC Pipes Pipe_-_ (111)_ (P-Stm) 522_ (P-Stm) 521_ (P-Stm) 45.094 0.013 82.482 82.257 0 0 ;Concrete Pipes 100-D Pipe_-_ (71)_ (P-Stm) 520_ (P-Stm) 519_ (P-Stm) 57.146 0.013 82.787 82.557 0 0 ;Concrete Pipes 100-D Pipe_-_ (72)_ (P-Stm) 519_ (P-Stm) 518_ (P-Stm) 31.967 0.013 82.497 82.337 0 0														
[ORIFICES] ;;Name From Node To Node Type Offset Qcoeff Gated CloseTime ;;-----														
OR1 518_ (P-Stm) 518_Orifice SIDE 81.361 0.65 NO														
[WEIRS] ;;Name From Node To Node Type CrestHt Qcoeff Gated EndCon EndCoeff Surcharge RoadWidth RoadSurf Coeff. Curve ;;-----														
W1 0 CB208 YES CB209 TRANSVERSE 85.31 1.84 NO 0 0 CB210B CB211 TRANSVERSE 85.6 1.84 NO W2 0 YES CB210B CB202 TRANSVERSE 85.45 1.84 NO W3 0 YES CB202 TRANSVERSE 85.55 1.84 NO W4 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO 0 0 YES W5 0 CB201 YES CB200 TRANSVERSE 85.65 1.84 NO W6 0 CB200 YES W7 0 CB200 TRANSVERSE 85.42 1.84 NO 0 0 YES														
[OUTLETS] ;;Name From Node To Node Offset Type QTable/Qcoeff Qexpon Gated ;;-----														
CB200 CB200 520_ (P-Stm) 83.18 TABULAR/HEAD IPEx_Type_A NO ;Critical CB201 CB201 520_ (P-Stm) 83.09 TABULAR/HEAD Vortex_ICD_100 NO ;Critical CB202 CB202 521_ (P-Stm) 82.76 TABULAR/HEAD IPEx_Type_A NO CB203 CB203 521_ (P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_70 NO CB204 CB204 522_ (P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_65 NO CB205 CB205 522_ (P-Stm) 82.85 TABULAR/HEAD Vortex_ICD_65 NO CB206 CB206 522_ (P-Stm) 82.77 TABULAR/HEAD Vortex_ICD_70 NO CB207 CB207 522_ (P-Stm) 83.15 TABULAR/HEAD Vortex_ICD_100 NO CB209 CB209 520_ (P-Stm) 83.2 TABULAR/HEAD IPEx_Type_A NO CB210B CB210B 518_ (P-Stm) 82.65 TABULAR/HEAD Vortex_ICD_65 NO CB212 CB212 521_ (P-Stm) 83.2 TABULAR/HEAD Vortex_ICD_100 NO														
[XSECTIONS]														

;;Link Culvert ;;-----	Shape	Geom1	Geom2	Geom3	Geom4	Barrels	Vortex ICD 100	1	8.9
							Vortex ICD 100	1.2	9.8
							Vortex ICD 100	1.4	10.6
							Vortex ICD 100	1.6	11.3
							Vortex ICD 100	1.8	12
							Vortex ICD 100	2	12.6
							Vortex ICD 100	2.5	14.1
							Vortex ICD 100	3	15.5
C2	CIRCULAR	0.25	0	0	0	1	;Tempest Rating Curve for Vortex ICD 105, No grate allowance		
C4	CIRCULAR	0.25	0	0	0	1	Vortex ICD 105	0	0
C5	CIRCULAR	0.25	0	0	0	1	Vortex ICD 105	0.1	3.1
CB213	CIRCULAR	0.25	0	0	0	1	Vortex ICD 105	0.2	4.4
CB214	CIRCULAR	0.25	0	0	0	1	Vortex ICD 105	0.3	5.4
Pipe _ (104) (P-Stm) 2 CIRCULAR 0.45			0	0	0	1	Vortex ICD 105	0.4	6.2
Pipe _ (110) (P-Stm) CIRCULAR 0.525			0	0	0	1	Vortex ICD 105	0.5	6.9
Pipe _ (111) (P-Stm) CIRCULAR 0.525			0	0	0	1	Vortex ICD 105	0.6	7.6
Pipe _ (71) (P-Stm) CIRCULAR 0.525			0	0	0	1	Vortex ICD 105	0.7	8.2
Pipe _ (72) (P-Stm) CIRCULAR 0.525			0	0	0	1	Vortex ICD 105	0.8	8.8
OR1	CIRCULAR	0.127	0	0	0	0	Vortex ICD 105	0.9	9.3
W1	RECT_OPEN	0.03	1.5	0	0	0	Vortex ICD 105	1	9.8
W2	RECT_OPEN	0.05	2.59	0	0	0	Vortex ICD 105	1.2	10.7
W3	RECT_OPEN	0.2	1.34	0	0	0	Vortex ICD 105	1.4	11.6
W4	RECT_OPEN	0.1	4.9	0	0	0	Vortex ICD 105	1.6	12.4
W5	RECT_OPEN	0.05	4.725	0	0	0	Vortex ICD 105	1.8	13.1
W6	RECT_OPEN	0.1	6	0	0	0	Vortex ICD 105	2	13.9
W7	RECT_OPEN	0.08	10.5	0	0	0	Vortex ICD 105	2.5	15.5
							Vortex ICD 105	3	17
[LOSSES]									
;;Link ;;-----	Kentry	Kexit	Kavg	Flap Gate	Seepage				
[CURVES]									
;;Name ;;-----	Type	X-Value	Y-Value				;Tempest Rating Curve for Vortex ICD 40, No grate allowance		
CB210-OUT85.5	Rating	0	0				Vortex ICD 40	0	0
CB210-OUT85.5		85.5	0.04				Vortex ICD 40	0.1	0.4
CB210-OUT85.5		100	0.04				Vortex ICD 40	0.2	0.6
							Vortex ICD 40	0.3	0.7
CB210-OUT-85.6	Rating	0	0				Vortex ICD 40	0.4	0.9
CB210-OUT-85.6		85.6	0.08				Vortex ICD 40	0.5	1
CB210-OUT-85.6		100	0.08				Vortex ICD 40	0.6	1.1
							Vortex ICD 40	0.7	1.1
							Vortex ICD 40	0.8	1.2
							Vortex ICD 40	0.9	1.3
							Vortex ICD 40	1	1.4
							Vortex ICD 40	1.2	1.5
							Vortex ICD 40	1.4	1.6
							Vortex ICD 40	1.6	1.7
							Vortex ICD 40	1.8	1.8
							Vortex ICD 40	2	1.9
							Vortex ICD 40	2.5	2.2
							Vortex ICD 40	3	2.4
;IPEX Type A ICD Rating Curve									
IPEX Type A	Rating	0	0				;Tempest Rating Curve for Vortex ICD 45, No grate allowance		
IPEX Type A		0.1	5.7				Vortex ICD 45	0	0
IPEX Type A		0.2	8.1				Vortex ICD 45	0.1	0.6
IPEX Type A		0.3	9.9				Vortex ICD 45	0.2	0.8
IPEX Type A		0.4	11.4				Vortex ICD 45	0.3	1
IPEX Type A		0.5	12.8				Vortex ICD 45	0.4	1.1
IPEX Type A		0.6	14				Vortex ICD 45	0.5	1.3
IPEX Type A		0.7	15.1				Vortex ICD 45	0.6	1.4
IPEX Type A		0.8	16.2				Vortex ICD 45	0.7	1.5
IPEX Type A		0.9	17.2				Vortex ICD 45	0.8	1.6
IPEX Type A		1	18.1				Vortex ICD 45	0.9	1.7
IPEX Type A		1.2	19.8				Vortex ICD 45	1	1.8
IPEX Type A		1.4	21.4				Vortex ICD 45	1.2	2.1
IPEX Type A		1.6	22.9				Vortex ICD 45	1.4	2.3
IPEX Type A		1.8	24.3				Vortex ICD 45	1.6	2.3
IPEX Type A		2	25.6				Vortex ICD 45	1.8	2.4
IPEX Type A		2.5	28.6				Vortex ICD 45	2	2.6
IPEX Type A		3	31.3				Vortex ICD 45	2.5	2.9
							Vortex ICD 45	3	3.1
;IPEX Type B ICD Rating Curve									
IPEX Type B	Rating	0	0				;Tempest Rating Curve for Vortex ICD 50, No grate allowance		
IPEX Type B		0.1	11.1				Vortex ICD 50	0	0
IPEX Type B		0.2	11.5				Vortex ICD 50	0.1	0.7
IPEX Type B		0.3	14.1				Vortex ICD 50	0.2	1
IPEX Type B		0.4	16.2				Vortex ICD 50	0.3	1.2
IPEX Type B		0.5	18.2				Vortex ICD 50	0.4	1.4
IPEX Type B		0.6	19.9				Vortex ICD 50	0.5	1.6
IPEX Type B		0.7	21.5				Vortex ICD 50	0.6	1.8
IPEX Type B		0.8	23				Vortex ICD 50	0.7	1.9
IPEX Type B		0.9	24.4				Vortex ICD 50	0.8	2.1
IPEX Type B		1	25.7				Vortex ICD 50	0.9	2.3
IPEX Type B		1.2	28.1				Vortex ICD 50	1	2.6
IPEX Type B		1.4	30.4				Vortex ICD 50	1.2	2.9
IPEX Type B		1.6	32.5				Vortex ICD 50	1.4	3.2
IPEX Type B		1.8	34.4				Vortex ICD 50	1.6	3.6
IPEX Type B		2	36.3				Vortex ICD 50	1.8	4.3
IPEX Type B		2.5	40.6				Vortex ICD 50	2.5	5.6
IPEX Type B		3	44.5				Vortex ICD 50	3	7.7
;IPEX Type C ICD Rating Curve									
IPEX Type C	Rating	0	0				;Tempest Rating Curve for Vortex ICD 55, No grate allowance		
IPEX Type C		0.1	10.6				Vortex ICD 55	0	0
IPEX Type C		0.2	15				Vortex ICD 55	0.1	0.9
IPEX Type C		0.3	18.3				Vortex ICD 55	0.2	1.2
IPEX Type C		0.4	21.2				Vortex ICD 55	0.3	1.5
IPEX Type C		0.5	23.7				Vortex ICD 55	0.4	1.7
IPEX Type C		0.6	25.6				Vortex ICD 55	0.5	1.9
IPEX Type C		0.7	28				Vortex ICD 55	0.6	2.1
IPEX Type C		0.8	29.9				Vortex ICD 55	0.7	2.3
IPEX Type C		0.9	31.7				Vortex ICD 55	0.8	2.6
IPEX Type C		1	33.5				Vortex ICD 55	0.9	2.7
IPEX Type C		1.2	36.6				Vortex ICD 55	1	3
IPEX Type C		1.4	39.6				Vortex ICD 55	1.2	3.2
IPEX Type C		1.6	42.3				Vortex ICD 55	1.4	3.4
IPEX Type C		1.8	44.9				Vortex ICD 55	1.6	3.6
IPEX Type C		2	47.3				Vortex ICD 55	1.8	3.8
IPEX Type C		2.5	52.9				Vortex ICD 55	2.5	4.3
IPEX Type C		3	57.9				Vortex ICD 55	3	4.7
;IPEX Type AA ICD Rating Curve									
IPEX Type D	Rating	0	0				;Tempest Rating Curve for Vortex ICD 60, No grate allowance		
IPEX Type D		0.1	15.4				Vortex ICD 60	0	0
IPEX Type D		0.2	21.7				Vortex ICD 60	0.1	1.1
IPEX Type D		0.3	26.6				Vortex ICD 60	0.2	1.5
IPEX Type D		0.4	30.7				Vortex ICD 60	0.3	1.8
IPEX Type D		0.5	34.3				Vortex ICD 60	0.4	2.1
IPEX Type D		0.6	37.6				Vortex ICD 60	0.5	2.3
IPEX Type D		0.7	40.6				Vortex ICD 60	0.6	2.5
IPEX Type D		0.8	43.4				Vortex ICD 60	0.7	2.7
IPEX Type D		0.9	46.1				Vortex ICD 60	0.8	2.9
IPEX Type D		1	48.5				Vortex ICD 60	0.9	3.1
IPEX Type D		1.2	53.2				Vortex ICD 60	1	3.2
IPEX Type D		1.4	57.4				Vortex ICD 60	1.2	3.6
IPEX Type D		1.6	61.4				Vortex ICD 60	1.4	4.1
IPEX Type D		1.8	65.1				Vortex ICD 60	1.6	4.3
IPEX Type D		2	68.7				Vortex ICD 60	1.8	4.6
IPEX Type D		2.5	76.8				Vortex ICD 60	2.5	5.1
IPEX Type D		3	84.1				Vortex ICD 60	3	5.6
;IPEX Type E ICD Rating Curve									
IPEX Type E	Rating	0	0				;Tempest Rating Curve for Vortex ICD 65, No grate allowance		
IPEX Type E		0.1	20.5				Vortex ICD 65	0	0
IPEX Type E		0.2	28.9				Vortex ICD 65	0.1	1.2
IPEX Type E		0.3	35.5				Vortex ICD 65	0.2	1.6
IPEX Type E		0.4	40.9				Vortex ICD 65	0.3	2
IPEX Type E		0.5	45.8				Vortex ICD 65	0.4	2.3
IPEX Type E		0.6	50.1				Vortex ICD 65	0.5	2.5
IPEX Type E		0.7	54.2				Vortex ICD 65	0.6	2.8
IPEX Type E		0.8	57.9				Vortex ICD 65	0.7	3
IPEX Type E		0.9	61.4				Vortex ICD 65	0.8	3.2
IPEX Type E		1	64.7				Vortex ICD 65	0.9	3.4
IPEX Type E		1.2	70.9				Vortex ICD 65	1	3.6
IPEX Type E		1.4	76.6				Vortex ICD 65	1.2	4
IPEX Type E		1.6	81.9				Vortex ICD 65	1.4	4.3
IPEX Type E		1.8	86.8				Vortex ICD 65	1.6	4.6
IPEX Type E		2	91.5				Vortex ICD 65	1.8	4.9
IPEX Type E		2.5	102.3				Vortex ICD 65	2	5.1
IPEX Type E		3	112.1				Vortex ICD 65	2.5	5.7
							Vortex ICD 65	3	6.3
;Tempest Rating Curve for Vortex ICD 100, No grate allowance									
Vortex ICD 100	Rating	0	0						
Vortex ICD 100		0.1	2.8						
Vortex ICD 100		0.2	4						
Vortex ICD 100		0.3	4.9						
Vortex ICD 100		0.4	5.6						
Vortex ICD 100		0.5	6.3						
Vortex ICD 100		0.6	6.9						
Vortex ICD 100		0.7	7.5						
Vortex ICD 100		0.8	8						
Vortex ICD 100		0.9	8.5						

```
;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex ICD 70 Rating 0 0
Vortex ICD 70 0.1 1.3
Vortex ICD 70 0.2 1.9
Vortex ICD 70 0.3 2.3
Vortex ICD 70 0.4 2.7
Vortex ICD 70 0.5 3.3
Vortex ICD 70 0.6 3.3
Vortex ICD 70 0.7 3.6
Vortex ICD 70 0.8 3.8
Vortex ICD 70 0.9 4.1
Vortex ICD 70 1 4.3
Vortex ICD 70 1.2 4.7
Vortex ICD 70 1.4 5.1
Vortex ICD 70 1.6 5.5
Vortex ICD 70 1.8 5.8
Vortex ICD 70 2 6.1
Vortex ICD 70 2.5 6.8
Vortex ICD 70 3 7.5
```

```
;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex ICD 75 Rating 0 0
Vortex ICD 75 0.1 1.6
Vortex ICD 75 0.2 2.2
Vortex ICD 75 0.3 2.7
Vortex ICD 75 0.4 3.2
Vortex ICD 75 0.5 3.5
Vortex ICD 75 0.6 3.9
Vortex ICD 75 0.7 4.2
Vortex ICD 75 0.8 4.5
Vortex ICD 75 0.9 4.8
Vortex ICD 75 1 5.5
Vortex ICD 75 1.2 5.5
Vortex ICD 75 1.4 5.9
Vortex ICD 75 1.6 6.3
Vortex ICD 75 1.8 6.7
Vortex ICD 75 2 7.1
Vortex ICD 75 2.5 7.9
Vortex ICD 75 3 8.7
```

```
;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex ICD 80 Rating 0 0
Vortex ICD 80 0.1 1.8
Vortex ICD 80 0.2 2.6
Vortex ICD 80 0.3 3.1
Vortex ICD 80 0.4 3.6
Vortex ICD 80 0.5 4
Vortex ICD 80 0.6 4.4
Vortex ICD 80 0.7 4.8
Vortex ICD 80 0.8 5.1
Vortex ICD 80 0.9 5.4
Vortex ICD 80 1 5.7
Vortex ICD 80 1.2 6.3
Vortex ICD 80 1.4 6.8
Vortex ICD 80 1.6 7.2
Vortex ICD 80 1.8 7.7
Vortex ICD 80 2 8.1
Vortex ICD 80 2.5 9
Vortex ICD 80 3 9.9
```

```
;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex ICD 85 Rating 0 0
Vortex ICD 85 0.1 2
Vortex ICD 85 0.2 2.9
Vortex ICD 85 0.3 3.5
Vortex ICD 85 0.4 4.1
Vortex ICD 85 0.5 4.5
Vortex ICD 85 0.6 5
Vortex ICD 85 0.7 5.4
Vortex ICD 85 0.8 5.7
Vortex ICD 85 0.9 6.1
Vortex ICD 85 1 6.4
Vortex ICD 85 1.2 7
Vortex ICD 85 1.4 7.6
Vortex ICD 85 1.6 8.1
Vortex ICD 85 1.8 8.6
Vortex ICD 85 2 9.1
Vortex ICD 85 2.5 10.1
Vortex ICD 85 3 11.1
```

```
;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex ICD 90 Rating 0 0
Vortex ICD 90 0.1 2.2
Vortex ICD 90 0.2 3.2
Vortex ICD 90 0.3 3.9
Vortex ICD 90 0.4 4.5
Vortex ICD 90 0.5 5.1
Vortex ICD 90 0.6 5.5
Vortex ICD 90 0.7 6
Vortex ICD 90 0.8 6.4
Vortex ICD 90 0.9 6.8
Vortex ICD 90 1 7.2
Vortex ICD 90 1.2 7.9
Vortex ICD 90 1.4 8.5
Vortex ICD 90 1.6 9.1
Vortex ICD 90 1.8 9.6
Vortex ICD 90 2 10.2
Vortex ICD 90 2.5 11.4
Vortex ICD 90 3 12.5
```

```
;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex ICD 95 Rating 0 0
Vortex ICD 95 0.1 2.6
Vortex ICD 95 0.2 3.6
Vortex ICD 95 0.3 4.4
Vortex ICD 95 0.4 5.1
Vortex ICD 95 0.5 5.7
Vortex ICD 95 0.6 6.2
Vortex ICD 95 0.7 6.7
Vortex ICD 95 0.8 7.1
Vortex ICD 95 0.9 7.6
Vortex ICD 95 1 8
Vortex ICD 95 1.2 8.7
Vortex ICD 95 1.4 9.4
Vortex ICD 95 1.6 10.1
Vortex ICD 95 1.8 10.7
Vortex ICD 95 2 11.3
Vortex ICD 95 2.5 12.6
Vortex ICD 95 3 13.8
```

```
CB200 Storage 0 0.36
CB200 2.24 217.87
CB200 2.32 217.87
CB201 Storage 0 0.36
CB201 2.41 0.36
CB201 2.56 169.64
CB201 2.66 169.64
CB202 Storage 0 0.36
CB202 2.6 0.36
CB202 2.79 273.92
CB202 2.89 273.92
CB203 Storage 0 0.36
CB203 2.6 0.36
CB203 2.9 178.09
CB204 Storage 0 0.36
CB204 2.6 0.36
CB204 2.75 146.08
CB205 Storage 0 0.36
CB205 2.6 0.36
CB205 2.75 86.52
CB206 Storage 0 0.36
CB206 2.6 0.36
```

```
CB206 2.68 57.95
CB206 2.73 57.95
CB207 Storage 0 0.36
CB207 1.97 0.36
CB207 2.3 178.5
CB208 Storage 0 0.073
CB208 1.74 0.073
CB208 2.01 67.88
CB208 2.04 67.88
CB209 Storage 0 0.073
CB209 1.6 0.073
CB209 1.9 84.87
CB210B Storage 0 0.073
CB210B 2.6 0.073
CB210B 2.9 70.98
CB210B 3 70.98
CB211 Storage 0 0.073
CB211 1.65 0.073
CB211 1.9 21.99
CB212 Storage 0 0.36
CB212 2.2 0.36
CB212 2.42 55.12
CB212A Storage 0 0.073
CB212A 2 0.073
CB213 Storage 0 0.073
CB213 1.6 0.073
CB213 1.75 9.05
CB214 Storage 0 0.073
CB214 1.6 0.073
CB214 1.9 34.75
```

```
[TIMESERIES]
;;Name Date Time Value
;;
;Rainfall (mm/hr)
3CHI002 01/01/2000 00:00:00 2.491
3CHI002 01/01/2000 00:10:00 2.966
3CHI002 01/01/2000 00:20:00 3.696
3CHI002 01/01/2000 00:30:00 4.976
3CHI002 01/01/2000 00:40:00 7.828
3CHI002 01/01/2000 00:50:00 19.966
3CHI002 01/01/2000 01:00:00 76.805
3CHI002 01/01/2000 01:10:00 22.777
3CHI002 01/01/2000 01:20:00 11.852
3CHI002 01/01/2000 01:30:00 8.025
3CHI002 01/01/2000 01:40:00 6.096
3CHI002 01/01/2000 01:50:00 4.938
3CHI002 01/01/2000 02:00:00 4.165
3CHI002 01/01/2000 02:10:00 3.613
3CHI002 01/01/2000 02:20:00 3.197
3CHI002 01/01/2000 02:30:00 2.873
3CHI002 01/01/2000 02:40:00 2.613
3CHI002 01/01/2000 02:50:00 2.4
3CHI002 01/01/2000 03:00:00 0
;Rainfall (mm/hr)
3CHI100 01/01/2000 00:00:00 5.339
3CHI100 01/01/2000 00:10:00 6.376
3CHI100 01/01/2000 00:20:00 7.977
3CHI100 01/01/2000 00:30:00 10.797
3CHI100 01/01/2000 00:40:00 17.136
3CHI100 01/01/2000 00:50:00 45.128
3CHI100 01/01/2000 01:00:00 178.107
3CHI100 01/01/2000 01:10:00 51.056
3CHI100 01/01/2000 01:20:00 26.163
3CHI100 01/01/2000 01:30:00 17.571
3CHI100 01/01/2000 01:40:00 13.277
3CHI100 01/01/2000 01:50:00 10.712
3CHI100 01/01/2000 02:00:00 9.008
3CHI100 01/01/2000 02:10:00 7.793
3CHI100 01/01/2000 02:20:00 6.883
3CHI100 01/01/2000 02:30:00 6.174
3CHI100 01/01/2000 02:40:00 5.607
3CHI100 01/01/2000 02:50:00 5.142
3CHI100 01/01/2000 03:00:00 0
```

```
[REPORT]
;;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

```
[TAGS]
Node 518 Orifice RY Manhole
Node CB200 RY Manhole
Node CB201 RY Manhole
Node CB202 RY Manhole
Node CB203 RY Manhole
Node CB204 RY Manhole
Node CB205 RY Manhole
Node CB206 RY Manhole
Node CB207 RY Manhole
Node CB208 RY Manhole
Node CB209 RY Manhole
Node CB210B RY Manhole
Node CB211 RY Manhole
Node CB212 RY Manhole
Node CB212A RY Manhole
Node CB213 RY Manhole
Node CB214 RY Manhole
Link C2 RY Sewer
Link C4 RY Sewer
Link C5 RY Sewer
```

```
[MAP]
DIMENSIONS 381219.0294 5032865.6782 381334.8286 5033009.9278
UNITS Meters
```

```
[COORDINATES]
;;Node X-Coord Y-Coord
;;
518 Orifice 381303.7 5032953.188
518A (P-Stm) 381317.029 5032955.797
518 (P-Stm) 381301.695 5032952.807
519 (P-Stm) 381278.558 5032974.866
520 (P-Stm) 381240.562 5032932.182
521 (P-Stm) 381309.324 5032926.69
522 (P-Stm) 381267.739 5032909.252
CB200 381246.119 5032946.933
CB201 381266.142 5032969.504
CB202 381305.816 5032949.229
CB203 381314.519 5032914.342
CB204 381291.187 5032921.779
CB205 381286.939 5032925.282
CB206 381270.233 5032913.017
CB207 381253.811 5032910.356
CB208 381248.314 5032902.406
CB209 381228.288 5032926.78
CB210B 381314.246 5032963.095
CB211 381319.351 5032951.771
CB212 381328.283 5032931.288
CB212A 381326.507 5032912.9
CB213 381296.875 5032912.718
CB214 381282.831 5032893.727
```

```
[VERTICES]
;;Link X-Coord Y-Coord
;;
```

W2	381318.013	5032956.452
W4	381305.808	5032934.267
W4	381304.151	5032928.843
[POLYGONS]		
;;Subcatchment		
;;-----		
S1	381237.464	5032904.825
S1	381240.329	5032905.467
...		
S9	381290.74	5032939.886
;;Storage Node		
;;-----		
[SYMBOLS]		
;;Gage		
;;-----		

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 2
Number of subcatchments ... 17
Number of nodes 23
Number of links 29
Number of pollutants 0
Number of land uses 0

Rainage Summary

Name	Data Source	Data Type	Recording Interval
3CHI002	3CHI002	INTENSITY	10 min.
3CHI100	3CHI100	INTENSITY	10 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S1	0.02	29.61	0.06	2.2000	3CHI002	CB208
S10	0.03	55.32	99.62	2.2000	3CHI002	CB204
S11	0.13	65.66	86.66	2.2000	3CHI002	CB202
S12	0.01	6.44	0.25	2.2000	3CHI002	CB213
S14_1	0.01	22.78	52.75	2.2000	3CHI002	CB211
S14_2	0.02	58.12	31.75	2.2000	3CHI002	CB212
S15	0.04	108.74	25.77	2.2000	3CHI002	CB210B
S2	0.07	33.61	63.05	2.2000	3CHI002	CB209
S3	0.07	45.79	96.63	2.2000	3CHI002	CB207
S32	0.05	29.70	14.29	2.2000	3CHI002	CB214
S4	0.04	24.67	99.97	2.2000	3CHI002	CB206
S45	0.02	14.32	99.96	2.2000	3CHI002	CB203
S5	0.09	82.96	89.99	2.2000	3CHI002	CB200
S6	0.02	40.30	0.00	2.2000	3CHI002	CB214
S7	0.07	25.95	100.00	2.2000	3CHI002	520_(P-
S8	0.09	35.06	80.12	2.2000	3CHI002	CB201
S9	0.03	19.08	76.83	2.2000	3CHI002	CB205

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
518 Orifice	JUNCTION	81.36	4.11	0.0	
518A_(P-Stm)	OUTFALL	80.65	1.13	0.0	
518_(P-Stm)	STORAGE	81.06	4.41	0.0	
519_(P-Stm)	STORAGE	82.20	3.51	0.0	
520_(P-Stm)	STORAGE	82.49	2.89	0.0	
521_(P-Stm)	STORAGE	81.96	3.67	0.0	
522_(P-Stm)	STORAGE	82.18	3.32	0.0	
CB207	STORAGE	83.18	2.32	0.0	
CB201	STORAGE	83.09	2.66	0.0	
CB202	STORAGE	82.76	2.89	0.0	
CB203	STORAGE	82.75	2.90	0.0	
CB204	STORAGE	82.75	2.75	0.0	
CB205	STORAGE	82.85	2.75	0.0	
CB206	STORAGE	82.77	2.73	0.0	
CB207	STORAGE	83.15	2.30	0.0	
CB208	STORAGE	83.30	2.04	0.0	
CB209	STORAGE	83.20	1.90	0.0	
CB210B	STORAGE	82.65	3.00	0.0	
CB211	STORAGE	83.70	1.90	0.0	
CB212	STORAGE	83.20	2.42	0.0	
CB212A	STORAGE	83.62	2.00	0.0	
CB213	STORAGE	83.15	1.75	0.0	
CB214	STORAGE	82.95	1.90	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope
Roughness					
-					
C2	CB208	CB207	CONDUIT	9.7	1.0346
C4	CB212A	CB212	CONDUIT	18.5	2.0031
C5	CB211	CB212	CONDUIT	22.3	2.0142
0.0130	CB213	522_(P-Stm)	CONDUIT	29.3	1.3396
0.0130	CB214	522_(P-Stm)	CONDUIT	21.6	0.8915
0.0130	Pipe_-(104)_(P-Stm)_2	518_Orifice	CONDUIT	15.6	0.2496
0.0130	Pipe_-(110)_(P-Stm)	521_(P-Stm)	CONDUIT	27.2	0.3528
0.0130	Pipe_-(111)_(P-Stm)	522_(P-Stm)	CONDUIT	45.1	0.4990
0.0130	Pipe_-(71)_(P-Stm)	520_(P-Stm)	CONDUIT	57.1	0.4025
0.0130	Pipe_-(72)_(P-Stm)	519_(P-Stm)	CONDUIT	32.0	0.5005
0.0130	OR1	518 Orifice	ORIFICE		
W1	CB208	CB209	WEIR		
W2	CB210B	CB211	WEIR		
W3	CB210B	CB202	WEIR		
W4	CB202	CB204	WEIR		
W5	CB206	CB207	WEIR		
W6	CB201	CB200	WEIR		
W7	CB200	CB209	WEIR		
CB200	CB200	520_(P-Stm)	OUTLET		
CB201	CB201	520_(P-Stm)	OUTLET		
CB202	CB202	521_(P-Stm)	OUTLET		
CB203	CB203	521_(P-Stm)	OUTLET		
CB204	CB204	522_(P-Stm)	OUTLET		
CB205	CB205	522_(P-Stm)	OUTLET		
CB206	CB206	522_(P-Stm)	OUTLET		
CB207	CB207	522_(P-Stm)	OUTLET		
CB209	CB209	520_(P-Stm)	OUTLET		
CB210B	CB210B	518_(P-Stm)	OUTLET		
CB212	CB212	521_(P-Stm)	OUTLET		

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C2	CIRCULAR	0.25	0.05	0.06	0.25	1	60.49
C4	CIRCULAR	0.25	0.05	0.06	0.25	1	84.17
C5	CIRCULAR	0.25	0.05	0.06	0.25	1	84.40
CB213	CIRCULAR	0.25	0.05	0.06	0.25	1	68.83
CB214	CIRCULAR	0.25	0.05	0.06	0.25	1	56.15
Pipe_-(104)_(P-Stm)_2	CIRCULAR	0.45	0.16	0.11	0.45	1	142.46
Pipe_-(110)_(P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	255.47
Pipe_-(111)_(P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	303.80
Pipe_-(71)_(P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	272.85
Pipe_-(72)_(P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	304.28

Analysis Options									

Flow Units LPS									
Process Models:									
Rainfall/Runoff YES									
RDII NO									
Snowmelt NO									
Groundwater NO									
Flow Routing YES									
Fonding Allowed NO									
Water Quality NO									
Infiltration Method HORTON									
Flow Routing Method DYNWAVE									
Surcharge Method EXTRAN									
Starting Date 01/01/2000 00:00:00									
Ending Date 01/01/2000 06:00:00									
Antecedent Dry Days 0.0									
Report Time Step 00:01:00									
Wet Time Step 00:01:00									
Dry Time Step 00:01:00									
Routing Time Step 1.00 sec									
Variable Time Step YES									
Maximum Trials 8									
Number of Threads 1									
Head Tolerance 0.001500 m									

CB207	0.000	0.1	0.0	0.0	0.001	1.9	0	01:10
12.84								
CB208	0.000	0.0	0.0	0.0	0.000	0.9	0	01:10
2.78								
CB209	0.000	0.0	0.0	0.0	0.000	0.2	0	01:10
9.92								
CB210B	0.000	0.0	0.0	0.0	0.000	0.3	0	01:12
1.86								
CB211	0.000	0.0	0.0	0.0	0.000	0.1	0	01:10
2.03								
CB212	0.000	0.0	0.0	0.0	0.000	0.8	0	01:11
3.35								
CB212A	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00
0.00								
CB213	0.000	0.0	0.0	0.0	0.000	0.0	0	01:12
0.03								
CB214	0.000	0.0	0.0	0.0	0.000	0.3	0	01:16
8.59								

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LFS	Max Flow LFS	Total Volume 10 ⁶ ltr
518A (P-Stm)	62.49	13.01	46.39	0.176
System	62.49	13.01	46.39	0.176

Link Flow Summary

Link	Type	Maximum Flow LFS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	3.97	0 01:02	0.10	0.07	1.00
C4	CONDUIT	0.00	0 00:00	0.00	0.00	0.19
C5	CONDUIT	2.03	0 01:10	0.67	0.02	0.24
CB213	CONDUIT	0.03	0 01:12	0.08	0.00	0.51
CB214	CONDUIT	8.59	0 01:12	0.33	0.15	0.92
Pipe _ (104) _ (P-Stm)	2 CONDUIT	46.39	0 01:19	0.94	0.33	0.35
Pipe _ (110) _ (P-Stm)	CONDUIT	55.22	0 01:07	0.71	0.22	1.00
Pipe _ (111) _ (P-Stm)	CONDUIT	30.05	0 01:11	0.51	0.10	1.00
Pipe _ (71) _ (P-Stm)	CONDUIT	54.76	0 01:10	1.00	0.20	0.85
Pipe _ (72) _ (P-Stm)	CONDUIT	51.62	0 01:06	0.97	0.17	1.00
OR1	ORIFICE	46.41	0 01:18			1.00
W1	WEIR	0.00	0 00:00			0.00
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	0.00	0 00:00			0.00
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	0.00	0 00:00			0.00
W6	WEIR	0.00	0 00:00			0.00
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	16.60	0 01:10			
CB201	DUMMY	13.84	0 01:10			
CB202	DUMMY	22.58	0 01:08			
CB203	DUMMY	3.65	0 01:08			
CB204	DUMMY	4.39	0 01:10			
CB205	DUMMY	3.34	0 01:09			
CB206	DUMMY	6.37	0 01:10			
CB207	DUMMY	11.17	0 01:10			
CB209	DUMMY	9.92	0 01:10			
CB210B	DUMMY	1.86	0 01:11			
CB212	DUMMY	3.35	0 01:11			

Flow Classification Summary

Conduit	Adjusted /Actual Length	----- Dry	Up Dry	Fraction of Down Dry	Sub Crit	Time in Flow Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C2	1.00	0.10	0.00	0.00	0.16	0.00	0.00	0.75	0.07	0.00
C4	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00
C5	1.00	0.17	0.00	0.00	0.02	0.00	0.00	0.80	0.03	0.00
CB213	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.82	0.09	0.00
CB214	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.83	0.03	0.00
Pipe _ (104) _ (P-Stm)	2 1.00	0.11	0.00	0.00	0.00	0.00	0.00	0.89	0.00	0.00
Pipe _ (110) _ (P-Stm)	1.00	0.11	0.00	0.00	0.14	0.00	0.00	0.75	0.00	0.00
Pipe _ (111) _ (P-Stm)	1.00	0.10	0.00	0.00	0.89	0.00	0.00	0.00	0.77	0.00
Pipe _ (71) _ (P-Stm)	1.00	0.10	0.00	0.00	0.11	0.00	0.00	0.79	0.04	0.00
Pipe _ (72) _ (P-Stm)	1.00	0.11	0.00	0.00	0.13	0.00	0.00	0.76	0.01	0.00

Conduit Surcharge Summary

Conduit	----- Both Ends	Hours Full Upstream	----- Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
C2	0.34	0.34	0.38	0.01	0.01
CB213	0.01	0.01	0.30	0.01	0.01
CB214	0.01	0.01	0.30	0.01	0.01
Pipe _ (110) _ (P-Stm)	0.51	0.51	0.60	0.01	0.01
Pipe _ (111) _ (P-Stm)	0.30	0.30	0.51	0.01	0.01
Pipe _ (71) _ (P-Stm)	0.01	0.01	0.19	0.01	0.01
Pipe _ (72) _ (P-Stm)	0.27	0.27	0.44	0.01	0.01

Analysis begun on: Tue Jan 28 12:44:04 2025
Analysis ended on: Tue Jan 28 12:44:05 2025
Total elapsed time: 00:00:01

Post-Development 3-hour Chicago 1:100 year Event

[TITLE] ;;Project Title/Notes										520_ (P-Stm)	82.487	2.888	0	FUNCTIONAL	0	0	1.13	0
[OPTIONS] ;;Option Value FLOW UNITS LPS INFILTRATION HORTON FLOW ROUTING DYNWAVE LINK OFFSETS ELEVATION MIN SLOPE 0 ALLOW PONDING NO SKIP STEADY_STATE NO										0 ;Cylindrical Structure Slab Top Circular Frame SI 521_ (P-Stm) 81.957 3.673 0 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI 522_ (P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0 0 ;85.1800000001676 CB200 83.18 2.32 0 TABULAR CB200 0 ;85.5000000001435 CB201 83.09 2.66 0 TABULAR CB201 0 0 ;85.36 CB202 82.76 2.89 0 TABULAR CB202 0 0 ;85.3500000000931 CB203 82.75 2.9 0 TABULAR CB203 0 0 ;85.3500080822731 CB204 82.75 2.75 0 TABULAR CB204 0 0 ;85.4500080823662 CB205 82.85 2.75 0 TABULAR CB205 0 0 ;85.48 CB206 82.77 2.73 0 TABULAR CB206 0 0 ;85.05 CB207 83.15 2.3 0 TABULAR CB207 0 0 ;84.9000080821652 CB208 83.3 2.04 0 TABULAR CB208 0 0 ;84.8 CB209 83.2 1.9 0 TABULAR CB209 0 0 ;85.25 CB210B 82.65 3 0 TABULAR CB210B 0 0 ;85.35 CB211 83.7 1.9 0 TABULAR CB211 0 0 ;85.4 CB212 83.2 2.42 0 TABULAR CB212 0 0 ;85.5000000000554 CB212A 83.62 2 0 TABULAR CB212A 0 0 ;84.75 CB213 83.15 1.75 0 TABULAR CB213 0 0 ;84.55 CB214 82.95 1.9 0 TABULAR CB214 0								
[EVAPORATION] ;;Data Source Parameters ;;----- CONSTANT 0.0 DRY_ONLY NO										[CONDUITS] ;;Name From Node To Node Length Roughness InOffset OutOffset ;;InitFlow MaxFlow ;;----- C2 CB208 CB207 9.666 0.013 83.3 83.2 C4 0 CB212A CB212 18.475 0.013 83.62 83.25 C5 0 CB211 CB212 22.346 0.013 83.7 83.25 CB213 0 CB213 522_ (P-Stm) 29.34 0.013 83.15 82.757 0 0 CB214 522_ (P-Stm) 21.65 0.013 82.95 82.757 0 ;PVC Pipes Pipe_ (104) (P-Stm)_2 518_Orifice 518A_ (P-Stm) 15.623 0.013 81.361 81.322 0 ;PVC Pipes Pipe_ (110) (P-Stm) 521_ (P-Stm) 518_ (P-Stm) 27.209 0.013 82.257 82.161 0 ;PVC Pipes Pipe_ (111) (P-Stm) 522_ (P-Stm) 521_ (P-Stm) 45.094 0.013 82.482 82.257 0 ;Concrete Pipes 100-D Pipe_ (71) (P-Stm) 520_ (P-Stm) 519_ (P-Stm) 57.146 0.013 82.787 82.557 0 ;Concrete Pipes 100-D Pipe_ (72) (P-Stm) 519_ (P-Stm) 518_ (P-Stm) 31.967 0.013 82.497 82.337 0								
[RAINGAGES] ;;Name Format Interval SCF Source ;;----- 3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100										[ORIFICES] ;;Name From Node To Node Type Offset Qcoeff Gated ;;CloseTime ;;----- OR1 518_ (P-Stm) 518_Orifice SIDE 81.361 0.65 NO								
[SUBCATCHMENTS] ;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen ;;SnowPack ;;-----										[WEIRS] ;;Name From Node To Node Type CrestHt Qcoeff Gated ;;EndCon EndCoeff SurchARGE RoadWidth RoadSurf Coeff. Curve ;;----- W1 CB208 CB209 TRANSVERSE 85.31 1.84 NO W2 0 CB210B CB211 TRANSVERSE 85.6 1.84 NO W3 0 YES CB202 TRANSVERSE 85.45 1.84 NO W4 0 CB202 CB204 TRANSVERSE 85.55 1.84 NO W5 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO W6 0 CB201 CB200 TRANSVERSE 85.65 1.84 NO W7 0 YES CB200 0 0 CB209 TRANSVERSE 85.42 1.84 NO								
[SUBAREAS] ;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted ;;-----										[OUTLETS] ;;Name From Node To Node Offset Type QTable/Qcoeff ;;Gated ;;----- CB200 CB200 520_ (P-Stm) 83.18 TABULAR/HEAD IPEX_Type_A NO ;Critical CB201 CB201 520_ (P-Stm) 83.09 TABULAR/HEAD Vortex_ICD_100 NO ;Critical CB202 CB202 521_ (P-Stm) 82.76 TABULAR/HEAD IPEX_Type_A NO CB203 CB203 521_ (P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_70 NO CB204 CB204 522_ (P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_65 NO CB205 CB205 522_ (P-Stm) 82.85 TABULAR/HEAD Vortex_ICD_65 NO CB206 CB206 522_ (P-Stm) 82.77 TABULAR/HEAD Vortex_ICD_70 NO CB207 CB207 522_ (P-Stm) 83.15 TABULAR/HEAD Vortex_ICD_100 NO CB209 CB209 520_ (P-Stm) 83.2 TABULAR/HEAD IPEX_Type_A NO								
[INFILTRATION] ;;Subcatchment Param1 Param2 Param3 Param4 Param5 ;;-----										[JUNCTIONS] ;;Name Elevation MaxDepth InitDepth SurDepth Aponded ;;----- 518_Orifice 81.361 4.106 0 0 0								
[OUTFALLS] ;;Name Elevation Type Stage Data Gated Route To ;;----- ;Cylindrical Structure Slab Top Circular Frame SI 518A_ (P-Stm) 80.646 FIXED 81.28 NO										[STORAGE] ;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params ;;Ksat Psf IMD ;;----- ;Cylindrical Structure Slab Top Circular Frame SI 518_ (P-Stm) 81.061 4.406 0.219 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI 519_ (P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0 0 ;Cylindrical Structure Slab Top Circular Frame SI								

CB210B	CB210B	518_ (P-Stm)	82.65	TABULAR/HEAD	Vortex_ICD_65	Vortex ICD 100	0.4	5.6
NO						Vortex_ICD_100	0.5	6.3
CB212	CB212	521_ (P-Stm)	83.2	TABULAR/HEAD	Vortex_ICD_100	Vortex_ICD_100	0.6	6.9
NO						Vortex_ICD_100	0.7	7.5
						Vortex_ICD_100	0.8	8
						Vortex_ICD_100	0.9	8.5
						Vortex_ICD_100	1	8.9
						Vortex_ICD_100	1.2	9.8
						Vortex_ICD_100	1.4	10.6
						Vortex_ICD_100	1.6	11.3
						Vortex_ICD_100	1.8	12
						Vortex_ICD_100	2	12.6
						Vortex_ICD_100	2.5	14.1
						Vortex_ICD_100	3	15.5
[XSECTIONS]								
;;Link								
Culvert								
;-----								
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C2	CIRCULAR	0.25	0	0	0	1		
C4	CIRCULAR	0.25	0	0	0	1		
C5	CIRCULAR	0.25	0	0	0	1		
CB213	CIRCULAR	0.25	0	0	0	1		
CB214	CIRCULAR	0.25	0	0	0	1		
Pipe -- (104) (P-Stm) 2	CIRCULAR	0.45	0	0	0	1		
Pipe -- (110) (P-Stm)	CIRCULAR	0.525	0	0	0	1		
Pipe -- (111) (P-Stm)	CIRCULAR	0.525	0	0	0	1		
Pipe -- (71) (P-Stm)	CIRCULAR	0.525	0	0	0	1		
Pipe -- (72) (P-Stm)	CIRCULAR	0.525	0	0	0	1		
OR1	CIRCULAR	0.127	0	0	0			
W1	RECT_OPEN	0.03	1.5	0	0			
W2	RECT_OPEN	0.05	2.59	0	0			
W3	RECT_OPEN	0.2	1.34	0	0			
W4	RECT_OPEN	0.1	4.9	0	0			
W5	RECT_OPEN	0.05	4.725	0	0			
W6	RECT_OPEN	0.1	6	0	0			
W7	RECT_OPEN	0.08	10.5	0	0			
[LOSSES]								
;;Link								
;-----								
;-----								
[CURVES]								
;;Name								
;-----								
;-----								
CB210-OUT85.5	Rating	0	0					
CB210-OUT85.5		85.5	0.04					
CB210-OUT85.5		100	0.04					
CB210-OUT-85.6	Rating	0	0					
CB210-OUT-85.6		85.6	0.08					
CB210-OUT-85.6		100	0.08					
;IPEX Type A ICD Rating Curve								
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Vortex ICD 65	1.6	4.6	CB205	Storage	0	0.36
Vortex ICD 65	1.8	4.9	CB205		2.6	0.36
Vortex ICD 65	2	5.1	CB205		2.75	86.52
Vortex ICD 65	2.5	5.7				
Vortex ICD 65	3	6.3	CB206	Storage	0	0.36
			CB206		2.6	0.36
			CB206		2.68	57.95
			CB206		2.73	57.95
;Tempest Rating Curve for Vortex ICD 70, No grate allowance			CB207	Storage	0	0.36
Vortex ICD 70	0	1.3	CB207		1.97	0.36
Vortex ICD 70	0.1	1.9	CB207		2.3	178.5
Vortex ICD 70	0.2	2.3				
Vortex ICD 70	0.3	2.7				
Vortex ICD 70	0.4	3				
Vortex ICD 70	0.5	3.3				
Vortex ICD 70	0.6	3.6	CB208	Storage	0	0.073
Vortex ICD 70	0.7	3.6	CB208		1.74	0.073
Vortex ICD 70	0.8	2.8	CB208		2.01	67.88
Vortex ICD 70	0.9	4.1	CB208		2.04	67.88
Vortex ICD 70	1	4.3				
Vortex ICD 70	1.2	4.7	CB209	Storage	0	0.073
Vortex ICD 70	1.4	5.1	CB209		1.6	0.073
Vortex ICD 70	1.6	5.5	CB209		1.9	84.87
Vortex ICD 70	1.8	5.8				
Vortex ICD 70	2	6.1	CB210B	Storage	0	0.073
Vortex ICD 70	2.5	6.8	CB210B		2.6	0.073
Vortex ICD 70	3	7.5	CB210B		2.9	70.98
			CB210B		3	70.98
;Tempest Rating Curve for Vortex ICD 75, No grate allowance			CB211	Storage	0	0.073
Vortex ICD 75	0	1.6	CB211		1.65	0.073
Vortex ICD 75	0.1	2.2	CB211		1.9	21.99
Vortex ICD 75	0.2	2.7				
Vortex ICD 75	0.3	3.2				
Vortex ICD 75	0.4	3.5	CB212	Storage	0	0.36
Vortex ICD 75	0.5	3.9	CB212		2.2	0.36
Vortex ICD 75	0.6	3.9	CB212		2.42	55.12
Vortex ICD 75	0.7	4.2				
Vortex ICD 75	0.8	4.5	CB212A	Storage	0	0.073
Vortex ICD 75	0.9	4.8	CB212A		2	0.073
Vortex ICD 75	1	5				
Vortex ICD 75	1.2	5.5	CB213	Storage	0	0.073
Vortex ICD 75	1.4	5.9	CB213		1.6	0.073
Vortex ICD 75	1.6	6.3	CB213		1.75	9.05
Vortex ICD 75	1.8	6.7				
Vortex ICD 75	2	7.1	CB214	Storage	0	0.073
Vortex ICD 75	2.5	7.9	CB214		1.6	0.073
Vortex ICD 75	3	8.7	CB214		1.9	34.75
;Tempest Rating Curve for Vortex ICD 80, No grate allowance			[TIMESERIES]			
Vortex ICD 80	0	1.8	;;Name			
Vortex ICD 80	0.1	2.6	Date			
Vortex ICD 80	0.2	3.1	Time			
Vortex ICD 80	0.3	3.6	Value			
Vortex ICD 80	0.4	4	;Rainfall (mm/hr)			
Vortex ICD 80	0.5	4.4	3CHI100	01/01/2000	00:00:00	5.339
Vortex ICD 80	0.6	4.8	3CHI100	01/01/2000	00:10:00	6.376
Vortex ICD 80	0.7	5.1	3CHI100	01/01/2000	00:20:00	7.977
Vortex ICD 80	0.8	5.4	3CHI100	01/01/2000	00:30:00	10.797
Vortex ICD 80	0.9	5.7	3CHI100	01/01/2000	00:40:00	17.136
Vortex ICD 80	1	6.3	3CHI100	01/01/2000	00:50:00	45.128
Vortex ICD 80	1.2	6.8	3CHI100	01/01/2000	01:00:00	178.107
Vortex ICD 80	1.4	7.2	3CHI100	01/01/2000	01:10:00	51.056
Vortex ICD 80	1.6	7.7	3CHI100	01/01/2000	01:20:00	26.163
Vortex ICD 80	1.8	8.1	3CHI100	01/01/2000	01:30:00	17.571
Vortex ICD 80	2	8.7	3CHI100	01/01/2000	01:40:00	13.277
Vortex ICD 80	2.5	9	3CHI100	01/01/2000	01:50:00	10.712
Vortex ICD 80	3	9.9	3CHI100	01/01/2000	02:00:00	9.008
			3CHI100	01/01/2000	02:10:00	7.793
			3CHI100	01/01/2000	02:20:00	6.883
			3CHI100	01/01/2000	02:30:00	6.174
			3CHI100	01/01/2000	02:40:00	5.607
			3CHI100	01/01/2000	02:50:00	5.142
			3CHI100	01/01/2000	03:00:00	0
			[REPORT]			
			;;Reporting Options			
			INPUT YES			
			CONTROLS NO			
			SUBCATCHMENTS ALL			
			NODES ALL			
			LINKS ALL			
			[TAGS]			
			Node	518 Orifice	RY Manhole	
			Node	CB200	RY Manhole	
			Node	CB201	RY Manhole	
			Node	CB202	RY Manhole	
			Node	CB203	RY Manhole	
			Node	CB204	RY Manhole	
			Node	CB205	RY Manhole	
			Node	CB206	RY Manhole	
			Node	CB207	RY Manhole	
			Node	CB208	RY Manhole	
			Node	CB209	RY Manhole	
			Node	CB210B	RY Manhole	
			Node	CB211	RY Manhole	
			Node	CB212	RY Manhole	
			Node	CB212A	RY Manhole	
			Node	CB213	RY Manhole	
			Node	CB214	RY Manhole	
			Link	C2	RY Sewer	
			Link	C4	RY Sewer	
			Link	C5	RY Sewer	
			[MAP]			
			DIMENSIONS	381219.0294	5032865.6782	381334.8286
			UNITS	Meters		5033009.9278
			[COORDINATES]			
			;;Node			
			X-Coord			
			Y-Coord			
			518 Orifice			
			381303.7			
			518A (P-Stm)			
			381317.029			
			518 (P-Stm)			
			381301.695			
			519 (P-Stm)			
			381278.558			
			520 (P-Stm)			
			381240.562			
			521 (P-Stm)			
			381309.324			
			522 (P-Stm)			
			381267.739			
			CB200			
			381246.119			
			CB201			
			381266.142			
			CB202			
			381305.816			
			CB203			
			381314.519			
			CB204			
			381291.187			
			CB205			
			381286.939			
			CB206			
			381270.233			
			CB207			
			381253.811			
			CB208			
			381248.314			
			CB209			
			381228.288			
			CB210B			
			381314.246			
			CB211			
			381319.351			
			CB212			
			381328.283			
			CB212A			
			381326.507			
			CB213			
			381296.875			
			CB214			
			381282.831			
			[VERTICES]			
			;;Link			
			X-Coord			
			Y-Coord			
			W2			
			381318.013			
			W4			
			381305.808			
			W4			
			381304.151			
			[POLYGONS]			
			;;Subcatchment			
			X-Coord			
			Y-Coord			
			S1			
			381237.464			
			S1			
			381240.329			
			S9			
			381273.896			
			S9			
			381277.469			
			S9			
			381278.399			
			S9			
			381281.347			
			S9			
			381290.74			
CB200	Storage	0	0.36			
CB200		2.24	217.87			
CB200		2.32	217.87			
CB201	Storage	0	0.36			
CB201		2.41	0.36			
CB201		2.56	169.64			
CB201		2.66	169.64			
CB202	Storage	0	0.36			
CB202		2.6	0.36			
CB202		2.79	273.92			
CB202		2.89	273.92			
CB203	Storage	0	0.36			
CB203		2.6	0.36			
CB203		2.9	178.09			
CB204	Storage	0	0.36			
CB204		2.6	0.36			
CB204		2.75	146.08			

;;Storage Node	X-Coord	Y-Coord
;;		
[SYMBOLS]		
;;Gage	X-Coord	Y-Coord
;;		

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 1
Number of subcatchments ... 17
Number of nodes 23
Number of links 29
Number of pollutants 0
Number of land uses 0

***** Raingage Summary *****			
Name	Data Source	Data Type	Recording Interval
3CHI100	3CHI100	INTENSITY	10 min.

***** Subcatchment Summary *****						
Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S1	0.02	29.61	0.06	2.2000	3CHI100	CB208
S10	0.03	35.32	99.62	2.2000	3CHI100	CB204
S11	0.13	65.66	86.66	2.2000	3CHI100	CB202
S12	0.01	6.44	0.25	2.2000	3CHI100	CB213
S14_1	0.01	22.78	52.75	2.2000	3CHI100	CB211
S14_2	0.02	58.12	31.75	2.2000	3CHI100	CB212
S15	0.04	108.74	25.77	2.2000	3CHI100	CB210B
S2	0.07	33.61	63.05	2.2000	3CHI100	CB209
S3	0.07	45.79	96.63	2.2000	3CHI100	CB207
S32	0.05	29.70	14.29	2.2000	3CHI100	CB214
S4	0.04	24.67	99.97	2.2000	3CHI100	CB206
S45	0.02	14.32	99.96	2.2000	3CHI100	CB203
S5	0.09	82.96	89.90	2.2000	3CHI100	CB200
S6	0.02	40.30	0.00	2.2000	3CHI100	CB214
S7	0.07	25.95	100.00	2.2000	3CHI100	520_ (P-
Stm)						
S8	0.09	35.06	80.12	2.2000	3CHI100	CB201
S9	0.03	19.08	76.83	2.2000	3CHI100	CB205

***** Node Summary *****						
Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow	
518 Orifice	JUNCTION	81.36	4.11	0.0		
518A (P-Stm)	OUTFALL	80.65	1.13	0.0		
518 (P-Stm)	STORAGE	81.06	4.41	0.0		
519 (P-Stm)	STORAGE	82.20	3.51	0.0		
520 (P-Stm)	STORAGE	82.49	2.89	0.0		
521 (P-Stm)	STORAGE	81.96	3.67	0.0		
522 (P-Stm)	STORAGE	82.18	3.32	0.0		
CB200	STORAGE	83.18	2.32	0.0		
CB201	STORAGE	83.09	2.66	0.0		
CB202	STORAGE	82.76	2.89	0.0		
CB203	STORAGE	82.75	2.90	0.0		
CB204	STORAGE	82.75	2.75	0.0		
CB205	STORAGE	82.85	2.75	0.0		
CB206	STORAGE	82.77	2.73	0.0		
CB207	STORAGE	83.15	2.30	0.0		
CB208	STORAGE	83.30	2.04	0.0		
CB209	STORAGE	83.20	1.90	0.0		
CB210B	STORAGE	82.65	3.00	0.0		
CB211	STORAGE	83.70	1.90	0.0		
CB212	STORAGE	83.20	2.42	0.0		
CB212A	STORAGE	83.62	2.00	0.0		
CB213	STORAGE	83.15	1.75	0.0		
CB214	STORAGE	82.95	1.90	0.0		

***** Link Summary *****						
Name	From Node	To Node	Type	Length	%Slope	
Roughness						
-						
C2	CB208	CB207	CONDUIT	9.7	1.0346	
C4	CB212A	CB212	CONDUIT	18.5	2.0031	
C5	CB211	CB212	CONDUIT	22.3	2.0142	
0.0130	CB213	522_ (P-Stm)	CONDUIT	29.3	1.3396	
0.0130	CB214	522_ (P-Stm)	CONDUIT	21.6	0.8915	
0.0130	Pipe _ (104)_ (P-Stm)_ 2 518_Orifice	518A_ (P-Stm)	CONDUIT	15.6	0.2496	
0.0130	Pipe _ (110)_ (P-Stm) 521_ (P-Stm)	518_ (P-Stm)	CONDUIT	27.2	0.3528	
0.0130	Pipe _ (111)_ (P-Stm) 522_ (P-Stm)	521_ (P-Stm)	CONDUIT	45.1	0.4990	
0.0130	Pipe _ (71)_ (P-Stm) 520_ (P-Stm)	519_ (P-Stm)	CONDUIT	57.1	0.4025	
0.0130	Pipe _ (72)_ (P-Stm) 519_ (P-Stm)	518_ (P-Stm)	CONDUIT	32.0	0.5005	
0.0130	OR1	518 (P-Stm)	518 Orifice			
W1	CB208	CB209	WEIR			
W2	CB210B	CB211	WEIR			
W3	CB210B	CB202	WEIR			
W4	CB202	CB204	WEIR			
W5	CB206	CB207	WEIR			
W6	CB201	CB200	WEIR			
W7	CB200	CB209	WEIR			
CB200	CB200	520_ (P-Stm)	OUTLET			
CB201	CB201	520_ (P-Stm)	OUTLET			
CB202	CB202	521_ (P-Stm)	OUTLET			
CB203	CB203	521_ (P-Stm)	OUTLET			
CB204	CB204	522_ (P-Stm)	OUTLET			
CB205	CB205	522_ (P-Stm)	OUTLET			
CB206	CB206	522_ (P-Stm)	OUTLET			
CB207	CB207	522_ (P-Stm)	OUTLET			
CB209	CB209	520_ (P-Stm)	OUTLET			
CB210B	CB210B	518_ (P-Stm)	OUTLET			
CB212	CB212	521_ (P-Stm)	OUTLET			

***** Cross Section Summary *****							
Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C2	CIRCULAR	0.25	0.05	0.06	0.25	1	60.49
C4	CIRCULAR	0.25	0.05	0.06	0.25	1	84.17
C5	CIRCULAR	0.25	0.05	0.06	0.25	1	84.40
CB213	CIRCULAR	0.25	0.05	0.06	0.25	1	68.83
CB214	CIRCULAR	0.25	0.05	0.06	0.25	1	56.15
Pipe _ (104)_ (P-Stm)_ 2	CIRCULAR	0.45	0.16	0.11	0.45	1	142.46
Pipe _ (110)_ (P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	255.47
Pipe _ (111)_ (P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	303.80
Pipe _ (71)_ (P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	272.85
Pipe _ (72)_ (P-Stm)	CIRCULAR	0.53	0.22	0.13	0.53	1	304.28

January 2025

CB210B	0.001	4.8	0.0	0.0	0.008	42.5	0	01:20
7.22								
CB211	0.000	0.5	0.0	0.0	0.000	3.0	0	01:36
5.42								
CB212	0.000	1.7	0.0	0.0	0.001	8.7	0	01:35
10.15								
CB212A	0.000	10.6	0.0	0.0	0.000	62.8	0	01:35
1.32								
CB213	0.000	4.6	0.0	0.0	0.000	49.4	0	01:38
7.52								
CB214	0.000	8.1	0.0	0.0	0.005	97.2	0	01:36
25.87								

Outfall Loading Summary

Outfall Node	Flow	Avg	Max	Total
	Frq	Flow	Flow	Volume
	Pcnt	LPS	LPS	10^6 ltr
518A (P-Stm)	67.75	32.60	66.10	0.477
System	67.75	32.60	66.10	0.477

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	24.70	0 01:03	0.50	0.41	1.00
C4	CONDUIT	5.30	0 01:09	0.15	0.06	1.00
C5	CONDUIT	5.42	0 01:06	0.66	0.06	1.00
CB213	CONDUIT	18.57	0 01:06	0.38	0.27	1.00
CB214	CONDUIT	25.87	0 01:09	0.53	0.46	1.00
Pipe _ (104) (P-Stm)	2 CONDUIT	66.10	0 01:37	1.05	0.46	0.42
Pipe _ (110) (P-Stm)	CONDUIT	64.79	0 01:05	0.72	0.25	1.00
Pipe _ (111) (P-Stm)	CONDUIT	40.32	0 01:05	0.49	0.13	1.00
Pipe _ (71) (P-Stm)	CONDUIT	92.47	0 01:03	1.04	0.34	1.00
Pipe _ (72) (P-Stm)	CONDUIT	65.69	0 01:06	0.89	0.22	1.00
OR1	ORIFICE	66.10	0 01:37			1.00
W1	WEIR	5.11	0 01:21			0.50
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	20.72	0 01:09			0.25
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	16.85	0 01:10			0.31
W6	WEIR	10.31	0 01:11			0.10
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	26.02	0 01:05			
CB201	DUMMY	14.08	0 01:04			
CB202	DUMMY	29.09	0 01:02			
CB203	DUMMY	5.70	0 01:07			
CB204	DUMMY	5.52	0 01:03			
CB205	DUMMY	5.18	0 01:06			
CB206	DUMMY	6.86	0 01:02			
CB207	DUMMY	12.67	0 01:04			
CB209	DUMMY	23.04	0 01:05			
CB210B	DUMMY	5.01	0 01:06			
CB212	DUMMY	5.95	0 01:05			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd
C2	1.00	0.05	0.00	0.00	0.39	0.00	0.00	0.56	0.07
C4	1.00	0.17	0.01	0.00	0.27	0.00	0.00	0.55	0.03
C5	1.00	0.15	0.00	0.00	0.28	0.00	0.00	0.56	0.05
CB213	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.05
CB214	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.03
Pipe _ (104) (P-Stm)	2	1.00	0.07	0.00	0.00	0.00	0.00	0.93	0.00
Pipe _ (110) (P-Stm)	1.00	0.06	0.00	0.00	0.38	0.00	0.00	0.56	0.00
Pipe _ (111) (P-Stm)	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.58
Pipe _ (71) (P-Stm)	1.00	0.06	0.00	0.00	0.34	0.00	0.00	0.60	0.03
Pipe _ (72) (P-Stm)	1.00	0.07	0.00	0.00	0.37	0.00	0.00	0.57	0.02

Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full Normal Flow	Hours Capacity Limited
	Both Ends	Upstream	Dnstream		
C2	1.76	1.76	1.80	0.01	0.01
C4	1.29	1.29	1.54	0.01	0.01
C5	1.23	1.23	1.55	0.01	0.01
CB213	1.50	1.50	1.74	0.01	0.01
CB214	1.63	1.63	1.74	0.01	0.01
Pipe _ (110) (P-Stm)	1.90	1.90	1.98	0.01	0.01
Pipe _ (111) (P-Stm)	1.74	1.74	1.90	0.01	0.01
Pipe _ (71) (P-Stm)	1.56	1.56	1.69	0.01	0.01
Pipe _ (72) (P-Stm)	1.73	1.73	1.85	0.01	0.01

Analysis begun on: Tue Jan 28 12:44:03 2025
Analysis ended on: Tue Jan 28 12:44:04 2025
Total elapsed time: 00:00:01

Post-Development 24-hour SCS 1:100-year Event

[TITLE]									
;;Project Title/Notes									
[OPTIONS]									
;;Option		Value							
FLOW UNITS		LPS							
INFILTRATION		HORTON							
FLOW ROUTING		DYNWAVE							
LINK OFFSETS		ELEVATION							
MIN SLOPE		0							
ALLOW PONDING		NO							
SKIP STEADY STATE		NO							
START DATE		01/01/2000							
START TIME		00:00:00							
REPORT START DATE		01/01/2000							
REPORT START TIME		00:00:00							
END DATE		01/01/2000							
END TIME		06:00:00							
SWEEP START		01/01							
SWEEP END		12/31							
DRY DAYS		0							
REPORT STEP		00:01:00							
WET STEP		00:01:00							
DRY STEP		00:01:00							
ROUTING STEP		1							
RULE STEP		00:00:00							
INERTIAL DAMPING		PARTIAL							
NORMAL FLOW LIMITED		BOTH							
FORCE MAIN EQUATION		H-W							
VARIABLE STEP		0.75							
LENGTHENING STEP		0							
MIN SURFAREA		0							
MAX TRIALS		8							
HEAD TOLERANCE		0.0015							
SVS FLOW TOL		5							
LAT FLOW TOL		5							
MINIMUM STEP		0.5							
THREADS		12							
[EVAPORATION]									
;;Data Source		Parameters							
;;-----									
CONSTANT		0.0							
DRY ONLY		NO							
[RAINGAGES]									
;;Name		Format	Interval	SCF	Source				
;;-----									
3CHI100		INTENSITY	0:10	1.0	TIMESERIES 3CHI100				
[SUBCATCHMENTS]									
;;Name		Rain Gage	Outlet	Area	%Imperv	Width	%Slope	CurbLen	
;;-----									
SnowPack									
;;-----									
S1	3CHI100	CB208	0.0187	0.061	29.61	2.2	0		
S10	3CHI100	CB204	0.0307	99.617	55.32	2.2	0		
S11	3CHI100	CB202	0.127	86.66	65.66	2.2	0		
S12	3CHI100	CB213	0.0149	0.253	6.44	2.2	0		
S14_1	3CHI100	CB211	0.0116	52.747	22.78	2.2	0		
S14_2	3CHI100	CB212	0.0179	31.754	58.12	2.2	0		
S15	3CHI100	CB210B	0.0372	25.766	108.74	2.2	0		
S2	3CHI100	CB209	0.0731	63.05	33.61	2.2	0		
S3	3CHI100	CB207	0.071	96.627	45.79	2.2	0		
S32	3CHI100	CB214	0.052	14.286	29.7	2.2	0		
S4	3CHI100	CB206	0.045	99.974	24.67	2.2	0		
S45	3CHI100	CB203	0.0222	99.961	14.32	2.2	0		
S5	3CHI100	CB200	0.0868	89.897	82.96	2.2	0		
S6	3CHI100	CB214	0.0239	0.002	40.3	2.2	0		
S7	3CHI100	520 (P-Stm)	0.0686	100	25.95	2.2	0		
S8	3CHI100	CB21	0.0941	80.117	35.06	2.2	0		
S9	3CHI100	CB205	0.0275	76.829	19.08	2.2	0		
[SUBAREAS]									
;;Subcatchment		N-Imperv	N-Perv	S-Imperv	S-Perv	PctZero	RouteTo	PctRouted	
;;-----									
S1	0.013	0.25	1.57	4.67	0	OUTLET			
S10	0.013	0.25	1.57	4.67	0	OUTLET			
S11	0.013	0.25	1.57	4.67	0	OUTLET			
S12	0.013	0.25	1.57	4.67	0	OUTLET			
S14_1	0.013	0.25	1.57	4.67	0	PERVIOUS	100		
S14_2	0.013	0.25	1.57	4.67	0	PERVIOUS	100		
S15	0.013	0.25	1.57	4.67	0	PERVIOUS	100		
S2	0.013	0.25	1.57	4.67	0	OUTLET			
S3	0.013	0.25	1.57	4.67	0	OUTLET			
S32	0.013	0.25	1.57	4.67	0	PERVIOUS	80		
S4	0.013	0.25	1.57	4.67	0	OUTLET			
S45	0.013	0.25	1.57	4.67	0	IMPERVIOUS	100		
S5	0.013	0.25	1.57	4.67	0	IMPERVIOUS	100		
S6	0.013	0.25	1.57	4.67	0	OUTLET			
S7	0.013	0.25	1.57	4.67	0	OUTLET			
S8	0.013	0.25	1.57	4.67	0	IMPERVIOUS	100		
S9	0.013	0.25	1.57	4.67	0	OUTLET			
[INFILTRATION]									
;;Subcatchment		Param1	Param2	Param3	Param4	Param5			
;;-----									
S1	76.2	13.2	4.14	7	4.14	0			
S10	76.2	13.2	4.14	7	0	0			
S11	76.2	13.2	4.14	7	0	0			
S12	76.2	13.2	4.14	7	0	0			
S14_1	76.2	13.2	4.14	7	0	0			
S14_2	76.2	13.2	4.14	7	0	0			
S15	76.2	13.2	4.14	7	0	0			
S2	76.2	13.2	4.14	7	0	0			
S3	76.2	13.2	4.14	7	0	0			
S32	76.2	13.2	4.14	7	0	0			
S4	76.2	13.2	4.14	7	0	0			
S45	76.2	13.2	4.14	7	0	0			
S5	76.2	13.2	4.14	7	0	0			
S6	76.2	13.2	4.14	7	0	0			
S7	76.2	13.2	4.14	7	0	0			
S8	76.2	13.2	4.14	7	0	0			
S9	76.2	13.2	4.14	7	0	0			
[JUNCTIONS]									
;;Name		Elevation	MaxDepth	InitDepth	SurDepth	Aponded			
;;-----									
518_Orifice		81.361	4.106	0	0	0			
[OUTFALLS]									
;;Name		Elevation	Type	Stage Data		Gated	Route To		
;;-----									
;Cylindrical Structure Slab Top Circular Frame SI		80.646	FIXED	81.28		NO			
[STORAGE]									
;;Name		Elev.	MaxDepth	InitDepth	Shape	Curve Name/Params			
;;Subcatchment		Psi	Fevap	IMD					
;;-----									
;Cylindrical Structure Slab Top Circular Frame SI		81.061	4.406	0.219	FUNCTIONAL	0	0	1.13	0
[XSECTIONS]									
;Cylindrical Structure Slab Top Circular Frame SI		82.197	3.512	0	FUNCTIONAL	0	0	1.13	0
[CONDUITS]									

										Vortex ICD 100	1.4	10.6	
C2	CIRCULAR	0.25	0	0	0	1	Vortex ICD 100	1.6	11.3				
C4	CIRCULAR	0.25	0	0	0	1	Vortex ICD 100	1.8	12				
C5	CIRCULAR	0.25	0	0	0	1	Vortex ICD 100	2	12.6				
CB213	CIRCULAR	0.25	0	0	0	1	Vortex ICD 100	2.5	14.1				
CB214	CIRCULAR	0.25	0	0	0	1	Vortex ICD 100	3	15.5				
Pipe_--(104)--(P-Stm)_2 CIRCULAR 0.45										;Tempest Rating Curve for Vortex ICD 105, No grate allowance			
Pipe_--(110)--(P-Stm)_CIRCULAR 0.525			0	0	0	1	Vortex ICD 105	Rating	0	0			
Pipe_--(111)--(P-Stm)_CIRCULAR 0.525			0	0	0	1	Vortex ICD 105		0.1	3.1			
Pipe_--(71)--(P-Stm)_CIRCULAR 0.525			0	0	0	1	Vortex ICD 105		0.2	4.4			
Pipe_--(72)--(P-Stm)_CIRCULAR 0.525			0	0	0	1	Vortex ICD 105		0.3	5.4			
OR1	CIRCULAR	0.127	0	0	0		Vortex ICD 105		0.4	6.2			
W1	RECT OPEN	0.03	1.5	0	0		Vortex ICD 105		0.5	6.9			
W2	RECT OPEN	0.05	2.59	0	0		Vortex ICD 105		0.6	7.6			
W3	RECT OPEN	0.2	1.34	0	0		Vortex ICD 105		0.7	8.2			
W4	RECT OPEN	0.1	4.9	0	0		Vortex ICD 105		0.8	8.8			
W5	RECT OPEN	0.05	4.725	0	0		Vortex ICD 105		0.9	9.3			
W6	RECT OPEN	0.1	6	0	0		Vortex ICD 105		1	9.8			
W7	RECT OPEN	1.08	10.5	0	0		Vortex ICD 105		1.2	10.7			
[LOSSES]										Vortex ICD 105	1.4	11.6	
;;Link	Kentry	Kexit	Kavg	Flap	Gate	Seepage	Vortex ICD 105		1.6	12.4			
;-----										Vortex ICD 105	1.8	13.1	
[CURVES]										Vortex ICD 105	2	13.9	
;;Name	Type	X-Value	Y-Value								Vortex ICD 105	2.5	15.5
;;-----											Vortex ICD 105	3	17
CB210-OUT85.5										;Tempest Rating Curve for Vortex ICD 40, No grate allowance			
CB210-OUT85.5	Rating	0	0							Vortex ICD 40	Rating	0	0
CB210-OUT85.5	85.5	0.04							Vortex ICD 40		0.1	0.4	
CB210-OUT85.5	100	0.04							Vortex ICD 40		0.2	0.6	
CB210-OUT-85.6										;Tempest Rating Curve for Vortex ICD 40, No grate allowance			
CB210-OUT-85.6	Rating	0	0							Vortex ICD 40		0.3	0.7
CB210-OUT-85.6	85.6	0.08							Vortex ICD 40		0.4	0.9	
CB210-OUT-85.6	100	0.08							Vortex ICD 40		0.5	1	
;IPEX Type A ICD Rating Curve										Vortex ICD 40		0.6	1
IPEX Type A	Rating	0	0							Vortex ICD 40		0.7	1.1
IPEX Type A	0.1	5.7							Vortex ICD 40		0.8	1.2	
IPEX Type A	0.2	8.1							Vortex ICD 40		0.9	1.3	
IPEX Type A	0.3	9.9							Vortex ICD 40		1	1.4	
IPEX Type A	0.4	11.4							Vortex ICD 40		1.2	1.5	
IPEX Type A	0.5	12.8							Vortex ICD 40		1.4	1.6	
IPEX Type A	0.6	14							Vortex ICD 40		1.6	1.7	
IPEX Type A	0.7	15.1							Vortex ICD 40		1.8	1.8	
IPEX Type A	0.8	16.2							Vortex ICD 40		2	1.9	
IPEX Type A	0.9	17.2							Vortex ICD 40		2.5	2.2	
IPEX Type A	1	18.1							Vortex ICD 40		3	2.4	
IPEX Type A	1.2	19.8							;Tempest Rating Curve for Vortex ICD 45, No grate allowance				
IPEX Type A	1.4	21.4							Vortex ICD 45	Rating	0	0	
IPEX Type A	1.6	22.9							Vortex ICD 45		0.1	0.6	
IPEX Type A	1.8	24.3							Vortex ICD 45		0.2	0.8	
IPEX Type A	2	25.6							Vortex ICD 45		0.3	1	
IPEX Type A	2.5	28.6							Vortex ICD 45		0.4	1.1	
IPEX Type A	3	31.3							Vortex ICD 45		0.5	1.3	
;IPEX Type B ICD Rating Curve										Vortex ICD 45		0.6	1.4
IPEX Type B	Rating	0	0							Vortex ICD 45		0.7	1.5
IPEX Type B	0.1	8.1							Vortex ICD 45		0.8	1.6	
IPEX Type B	0.2	11.5							Vortex ICD 45		0.9	1.7	
IPEX Type B	0.3	14.1							Vortex ICD 45		1	1.8	
IPEX Type B	0.4	16.2							Vortex ICD 45		1.2	2	
IPEX Type B	0.5	18.2							Vortex ICD 45		1.4	2.1	
IPEX Type B	0.6	19.9							Vortex ICD 45		1.6	2.3	
IPEX Type B	0.7	21.5							Vortex ICD 45		1.8	2.4	
IPEX Type B	0.8	23							Vortex ICD 45		2	2.6	
IPEX Type B	0.9	24.7							Vortex ICD 45		2.5	2.9	
IPEX Type B	1	25.4							Vortex ICD 45		3	3.1	
IPEX Type B	1.2	28.1							;Tempest Rating Curve for Vortex ICD 50, No grate allowance				
IPEX Type B	1.4	30.4							Vortex ICD 50	Rating	0	0	
IPEX Type B	1.6	32.5							Vortex ICD 50		0.1	0.7	
IPEX Type B	1.8	34.4							Vortex ICD 50		0.2	1	
IPEX Type B	2	36.3							Vortex ICD 50		0.3	1.2	
IPEX Type B	2.5	40.6							Vortex ICD 50		0.4	1.4	
IPEX Type B	3	44.5							Vortex ICD 50		0.5	1.6	
;IPEX Type C ICD Rating Curve										Vortex ICD 50		0.6	1.8
IPEX Type C	Rating	0	0							Vortex ICD 50		0.7	1.9
IPEX Type C	0.1	10.6							Vortex ICD 50		0.8	2	
IPEX Type C	0.2	15							Vortex ICD 50		0.9	2.1	
IPEX Type C	0.3	18.3							Vortex ICD 50		1	2.3	
IPEX Type C	0.4	21.2							Vortex ICD 50		1.2	2.5	
IPEX Type C	0.5	23.7							Vortex ICD 50		1.4	2.7	
IPEX Type C	0.6	25.9							Vortex ICD 50		1.6	2.9	
IPEX Type C	0.7	28							Vortex ICD 50		1.8	3	
IPEX Type C	0.8	29.9							Vortex ICD 50		2	3.2	
IPEX Type C	0.9	31.7							Vortex ICD 50		2.5	3.6	
IPEX Type C	1	33.5							Vortex ICD 50		3	3.9	
IPEX Type C	1.2	36.6							;Tempest Rating Curve for Vortex ICD 55, No grate allowance				
IPEX Type C	1.4	39.6							Vortex ICD 55	Rating	0	0	
IPEX Type C	1.6	42.3							Vortex ICD 55		0.1	0.9	
IPEX Type C	1.8	44.9							Vortex ICD 55		0.2	1.2	
IPEX Type C	2	47.3							Vortex ICD 55		0.3	1.5	
IPEX Type C	2.5	52.9							Vortex ICD 55		0.4	1.7	
IPEX Type C	3	57.9							Vortex ICD 55		0.5	1.9	
;IPEX Type AA ICD Rating Curve										Vortex ICD 55		0.6	2.1
IPEX Type D	Rating	0	0							Vortex ICD 55		0.7	2.3
IPEX Type D	0.1	15.4							Vortex ICD 55		0.8	2.4	
IPEX Type D	0.2	11.7							Vortex ICD 55		0.9	2.6	
IPEX Type D	0.3	26.6							Vortex ICD 55		1	2.7	
IPEX Type D	0.4	30.7							Vortex ICD 55		1.2	3	
IPEX Type D	0.5	34.3							Vortex ICD 55		1.4	3.2	
IPEX Type D	0.6	37.6							Vortex ICD 55		1.6	3.4	
IPEX Type D	0.7	40.6							Vortex ICD 55		1.8	3.6	
IPEX Type D	0.8	43.4							Vortex ICD 55		2	3.8	
IPEX Type D	0.9	46.1							Vortex ICD 55		2.5	4.3	
IPEX Type D	1	48.5							Vortex ICD 55		3	4.7	
IPEX Type D	1.2	53.2							;Tempest Rating Curve for Vortex ICD 60, No grate allowance				
IPEX Type D	1.4	57.4							Vortex ICD 60	Rating	0	0	
IPEX Type D	1.6	61.4							Vortex ICD 60		0.1	1.1	
IPEX Type D	1.8	65.1							Vortex ICD 60		0.2	1.5	
IPEX Type D	2	68.7							Vortex ICD 60		0.3	1.8	
IPEX Type D	2.5	76.8							Vortex ICD 60		0.4	2.1	
IPEX Type D	3	84.1							Vortex ICD 60		0.5	2.3	
;IPEX Type E ICD Rating Curve										Vortex ICD 60		0.6	2.5
IPEX Type E	Rating	0	0							Vortex ICD 60		0.7	2.7
IPEX Type E	0.1	20.5							Vortex ICD 60		0.8	2.9	
IPEX Type E	0.2	28.9							Vortex ICD 60		0.9	3.1	
IPEX Type E	0.3	35.5							Vortex ICD 60		1	3.2	
IPEX Type E	0.4	40.9							Vortex ICD 60		1.2	3.6	
IPEX Type E	0.5	45.8							Vortex ICD 60		1.4	3.8	
IPEX Type E	0.6	50.1							Vortex ICD 60		1.6	4.1	
IPEX Type E	0.7	54.2							Vortex ICD 60		1.8	4.3	
IPEX Type E	0.8	57.9							Vortex ICD 60		2	4.6	
IPEX Type E	0.9	61.4							Vortex ICD 60		2.5	5.1	
IPEX Type E	1	64.7							Vortex ICD 60		3	5.6	
IPEX Type E	1.2	70.9							;Tempest Rating Curve for Vortex ICD 65, No grate allowance				
IPEX Type E	1.4	76.6							Vortex ICD 65	Rating	0	0	
IPEX Type E	1.6	81.9							Vortex ICD 65		0.1	1.2	
IPEX Type E	1.8	86.8							Vortex ICD 65		0.2	1.6	
IPEX Type E	2	91.5							Vortex ICD 65		0.3	2	
IPEX Type E	2.5	102.3							Vortex ICD 65		0.4	2.3	
IPEX Type E	3	112.1							Vortex ICD 65		0.5	2.5	
;Tempest Rating Curve for Vortex ICD 100, No grate allowance										Vortex ICD 65		0.6	2.8
Vortex ICD 100	Rating	0	0							Vortex ICD 65		0.7	3.2
Vortex ICD 100	0.1	2.8							Vortex ICD 65		0.8	3.4	
Vortex ICD 100	0.2	4.9							Vortex ICD 65		0.9	3.6	
Vortex ICD 100	0.3	4.9							Vortex ICD 65		1	4	
Vortex ICD 100	0.4	5.6							Vortex ICD 65		1.2	4.3	
Vortex ICD 100	0.5	6.3							Vortex ICD 65		1.4	4.6	
Vortex ICD 100	0.6	6.9							Vortex ICD 65		1.6	4.9	
Vortex ICD 100	0.7	7.5							Vortex ICD 65		1.8	5.1	
Vortex ICD 100	0.8	8							Vortex ICD 65		2	5.7	
Vortex ICD 100	0.9	8.5							Vortex ICD 65		2.5	6.3	
Vortex ICD 100	1	8.9							;Tempest Rating Curve for Vortex ICD 70, No grate allowance				
Vortex ICD 100	1.2	9.8							Vortex ICD 70	Rating	0	0	

CB207	Storage	0	0.36
CB207		1.97	0.36
CB207		2.3	178.5
CB208	Storage	0	0.073
CB208		1.74	0.073
CB208		2.01	67.88
CB208		2.04	67.88
CB209	Storage	0	0.073
CB209		1.6	0.073
CB209		1.9	84.87
CB210B	Storage	0	0.073
CB210B		2.6	0.073
CB210B		2.9	70.98
CB210B		3	70.98
CB211	Storage	0	0.073
CB211		1.65	0.073
CB211		1.9	21.99
CB212	Storage	0	0.36
CB212		2.2	0.36
CB212		2.42	55.12
CB212A	Storage	0	0.073
CB212A		2	0.073
CB213	Storage	0	0.073
CB213		1.6	0.073
CB213		1.75	9.05
CB214	Storage	0	0.073
CB214		1.6	0.073
CB214		1.9	34.75

```

TIMESERIES
;;Name      Date      Time      Value
;Rainfall (mm/hr)
3CH1100    01/01/2000  00:00:00  5.339
3CH1100    01/01/2000  00:10:00  6.376
3CH1100    01/01/2000  00:20:00  7.977
3CH1100    01/01/2000  00:30:00  10.797
3CH1100    01/01/2000  00:40:00  17.136
3CH1100    01/01/2000  00:50:00  45.128
3CH1100    01/01/2000  01:00:00  178.107
3CH1100    01/01/2000  01:10:00  51.056
3CH1100    01/01/2000  01:20:00  17.136
3CH1100    01/01/2000  01:30:00  17.571
3CH1100    01/01/2000  01:40:00  13.277
3CH1100    01/01/2000  01:50:00  10.712
3CH1100    01/01/2000  02:00:00  9.008
3CH1100    01/01/2000  02:10:00  6.883
3CH1100    01/01/2000  02:20:00  6.883
3CH1100    01/01/2000  02:30:00  6.174
3CH1100    01/01/2000  02:40:00  5.607
3CH1100    01/01/2000  02:50:00  0.142
3CH1100    01/01/2000  03:00:00  0

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```
[REPORT]
;;Reporting Options
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

```

[Tags]
Node      518 Orifice      RY Manhole
Node      CB200           RY Manhole
Node      CB201           RY Manhole
Node      CB202           RY Manhole
Node      CB203           RY Manhole
Node      CB204           RY Manhole
Node      CB205           RY Manhole
Node      CB206           RY Manhole
Node      CB207           RY Manhole
Node      CB208           RY Manhole
Node      CB209           RY Manhole
Node      CB210B          RY Manhole
Node      CB211           RY Manhole
Node      CB212           RY Manhole
Node      CB212A          RY Manhole
Node      CB213           RY Manhole
Node      CB214           RY Manhole
Link      C2              RY Sewer
Link      C4              RY Sewer
Link      C5              RY Sewer

```

[MAP]				
DIMENSIONS	381219.0294	5032865.6782	381334.8286	5033009.9278
UNITS	Meters			

[COORDINATES]			
:::Node	X-Coord		Y-Coord
518 Orifice	381303.7		5032953.188
518 (P-Stm)	381277.029		5032955.797
518 (P-Stm)	381301.695		5032952.807
519 (P-Stm)	381278.558		5032974.866
520 (P-Stm)	381240.562		5032932.182
520 (P-Stm)	381242.324		5032926.69
522*	381267.739		5032909.252
CB200	381246.119		5032946.933
CB201	381266.142		5032969.504
CB202	381305.816		5032949.229
CB203	381314.519		5032914.342
CB204	381291.187		5032921.779
CB205	381286.939		5032925.282
CB206	381270.233		5032913.017
CB207	381253.811		5032910.356
CB208	381248.314		5032902.406
CB209	381228.288		5032926.78
CB210	381314.246		5032963.095
CB211	381319.351		5032951.771
CB212	381328.283		5032931.288
CB212A	381326.507		5032912.9
CB213	381296.875		5032912.718
CB214	381282.831		5032893.727

[VERTICES]		
;;Link	X-Coord	Y-Coord
;;		
W2	381318.013	5032956.452
W4	381305.808	5032934.267
W4	381304.151	5032928.843

[POLYGONS]		
;;Subcatchment	X-Coord	Y-Coord
S1	381237.464	5032904.825
S1	381240.329	5032905.467
S1	381240.224	5032906.884
...		
S9	381278.399	5032931.986
S9	381281.347	5032929.355
S9	381290.74	5032939.886

```
[SYMBOLS]
;;Gage      X-Coord      Y-Coord
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```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)							Analysis Options						
***** Element Count ***** Number of rain gages 1 Number of subcatchments ... 17 Number of nodes 23 Number of links 29 Number of pollutants 0 Number of land uses 0							***** Flow Units LPS Process Models Rainfall/Runoff YES RDII NO Snowmelt NO Groundwater NO Flow Routing YES Ponding Allowed NO Water Quality NO Infiltration Method HORTON Flow Routing Method DYNWAVE Surcharge Method EXTRAN Starting Date 01/01/2000 00:00:00 Ending Date 01/01/2000 06:00:00 Antecedent Dry Days 0.0 Report Time Step 00:01:00 Wet Time Step 00:01:00 Dry Time Step 00:01:00 Routing Time Step 1.00 sec Variable Time Step YES Maximum Trials 8 Number of Threads 1 Head Tolerance 0.001500 m						
***** Raingage Summary *****							***** Runoff Quantity Continuity ***** Total Precipitation 0.059 Evaporation Loss 0.000 Infiltration Loss 0.010 Surface Runoff 0.048 Final Storage 0.001 Continuity Error (%) -0.141						
***** Subcatchment Summary *****							***** Flow Routing Continuity ***** Dry Weather Inflow 0.000 Wet Weather Inflow 0.048 Groundwater Inflow 0.000 RDII Inflow 0.000 External Inflow 0.000 External Outflow 0.048 Flooding Loss 0.000 Evaporation Loss 0.000 Exfiltration Loss 0.000 Initial Stored Volume 0.000 Final Stored Volume 0.000 Continuity Error (%) -0.030						
***** Node Summary *****							***** Time-Step Critical Elements ***** None						
***** Link Summary *****							***** Highest Flow Instability Indexes ***** Link W3 (1)						
***** Link Summary *****							***** Most Frequent Nonconverging Nodes ***** Convergence obtained at all time steps.						
***** Link Summary *****							***** Routing Time Step Summary ***** Minimum Time Step : 0.50 sec Average Time Step : 1.00 sec Maximum Time Step : 1.00 sec % of Time in Steady State : 0.00 Average Iterations per Step : 2.00 % of Steps Not Converging : 0.00 Time Step Frequencies : 1.000 - 0.871 sec : 100.00 % 0.871 - 0.758 sec : 0.00 % 0.758 - 0.660 sec : 0.00 % 0.660 - 0.574 sec : 0.00 % 0.574 - 0.500 sec : 0.00 %						
***** Link Summary *****							***** Subcatchment Runoff Summary *****						
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***** Link Summary *****							***** Subcatchment Runoff Summary *****						

518_Orifice	JUNCTION	0.08	0.20	81.56	0	01:37	0.20
518A_ (P-Stm)	OUTFALL	0.63	0.63	81.28	0	00:00	0.63
518_ (P-Stm)	STORAGE	1.27	3.78	84.84	0	01:37	3.78
519_ (P-Stm)	STORAGE	0.81	2.65	84.84	0	01:37	2.65
520_ (P-Stm)	STORAGE	0.72	2.36	84.85	0	01:37	2.36
521_ (P-Stm)	STORAGE	0.90	2.89	84.84	0	01:37	2.89
522_ (P-Stm)	STORAGE	0.81	2.66	84.85	0	01:37	2.66
CB200	STORAGE	0.43	2.15	85.33	0	01:20	2.15
CB201	STORAGE	0.58	2.57	85.66	0	01:11	2.57
CB202	STORAGE	0.60	2.74	85.50	0	01:20	2.74
CB203	STORAGE	0.54	2.66	85.41	0	01:11	2.65
CB204	STORAGE	0.65	2.70	85.45	0	01:20	2.70
CB205	STORAGE	0.56	2.69	85.54	0	01:20	2.69
CB206	STORAGE	0.63	2.70	85.47	0	01:10	2.70
CB207	STORAGE	0.58	2.18	85.33	0	01:20	2.17
CB208	STORAGE	0.52	2.03	85.33	0	01:21	2.03
CB209	STORAGE	0.37	1.79	84.99	0	01:25	1.79
CB210B	STORAGE	0.64	2.85	85.50	0	01:20	2.85
CB211	STORAGE	0.19	1.18	84.88	0	01:36	1.18
CB212	STORAGE	0.32	1.67	84.87	0	01:35	1.67
CB212A	STORAGE	0.21	1.26	84.88	0	01:35	1.25
CB213	STORAGE	0.32	1.70	84.85	0	01:38	1.70
CB214	STORAGE	0.37	1.90	84.85	0	01:36	1.90

Node Inflow Summary

Flow		Maximum	Maximum		Lateral	Total	
Balance		Lateral	Total	Time of Max	Inflow	Inflow	
Error		Inflow	Inflow	Occurrence	Volume	Volume	
Node	Type	LPS	LPS	days hr:min	10^6 ltr	10^6 ltr	
Percent							
518_Orifice	JUNCTION	0.00	66.10	0 01:37	0	0.477	-
0.014_							
518A_ (P-Stm)	OUTFALL	0.00	66.10	0 01:37	0	0.477	
0.000_							
518_ (P-Stm)	STORAGE	0.00	84.12	0 01:07	0	0.477	-
0.111_							
519_ (P-Stm)	STORAGE	0.00	92.47	0 01:03	0	0.206	-
0.377_							
520_ (P-Stm)	STORAGE	33.94	96.71	0 01:05	0.0482	0.207	
0.489_							
521_ (P-Stm)	STORAGE	0.00	78.54	0 01:05	0	0.256	
0.083_							
522_ (P-Stm)	STORAGE	0.00	60.78	0 01:06	0	0.15	-
0.029_							
CB200	STORAGE	42.36	43.45	0 01:10	0.0573	0.0621	
0.008_							
CB201	STORAGE	44.54	44.54	0 01:10	0.0582	0.0582	
0.013_							
CB202	STORAGE	61.68	61.68	0 01:10	0.0821	0.0834	
0.024_							
CB203	STORAGE	10.98	10.98	0 01:10	0.0156	0.0156	
0.003_							
CB204	STORAGE	15.18	15.18	0 01:10	0.0215	0.0215	
0.014_							
CB205	STORAGE	13.15	13.15	0 01:10	0.0167	0.0167	
0.009_							
CB206	STORAGE	22.26	22.26	0 01:10	0.0316	0.0316	
0.029_							
CB207	STORAGE	34.97	51.81	0 01:10	0.0489	0.068	-
0.003_							
CB208	STORAGE	7.47	26.09	0 01:03	0.00525	0.0156	
0.021_							
CB209	STORAGE	33.11	33.11	0 01:10	0.0399	0.0434	-
0.001_							
CB210B	STORAGE	16.73	37.44	0 01:09	0.0136	0.0176	
0.083_							
CB211	STORAGE	5.46	7.13	0 01:08	0.00538	0.00544	
0.063_							
CB212	STORAGE	8.14	13.45	0 01:06	0.00691	0.0143	-
0.123_							
CB212A	STORAGE	0.00	5.30	0 01:09	0	0.00175	
0.096_							
CB213	STORAGE	3.78	20.64	0 01:06	0.00397	0.00517	-
0.056_							
CB214	STORAGE	27.81	27.81	0 01:10	0.0234	0.0235	
0.059_							

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Maximum	Average	Avg	Evap	Exfil	Maximum	Max	Time of Max
Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	Occurrence	
Outflow							
Storage Unit	1000 m³	Full	Loss	Loss	1000 m³	Full	days hr:min
518_ (P-Stm)	0.001	28.9	0.0	0.0	0.004	85.8	0 01:37
66.10_							
519_ (P-Stm)	0.001	23.0	0.0	0.0	0.003	75.4	0 01:37
65.69_							
520_ (P-Stm)	0.001	24.9	0.0	0.0	0.003	81.7	0 01:37
92.47_							
521_ (P-Stm)	0.001	24.5	0.0	0.0	0.003	78.6	0 01:37
64.79_							
522_ (P-Stm)	0.001	24.5	0.0	0.0	0.003	80.1	0 01:37
43.77_							
CB200	0.001	2.0	0.0	0.0	0.011	25.0	0 01:20
26.02_							
CB201	0.002	6.1	0.0	0.0	0.015	49.8	0 01:11
21.05_							
CB202	0.001	2.7	0.0	0.0	0.015	28.0	0 01:20
43.18_							
CB203	0.000	0.8	0.0	0.0	0.002	6.7	0 01:11
5.70_							
CB204	0.001	6.4	0.0	0.0	0.006	46.3	0 01:20
5.52_							
CB205	0.000	4.7	0.0	0.0	0.003	42.4	0 01:20
5.18_							
CB206	0.001	8.4	0.0	0.0	0.004	67.6	0 01:10
22.10_							
CB207	0.002	6.2	0.0	0.0	0.012	40.2	0 01:20
37.16_							
CB208	0.002	15.2	0.0	0.0	0.010	91.1	0 01:21
9.56_							
CB209	0.000	3.6	0.0	0.0	0.005	39.9	0 01:25
23.04_							
CB210B	0.001	4.8	0.0	0.0	0.008	42.5	0 01:20
7.22_							

CB211	0.000	0.5	0.0	0.0	0.000	3.0	0 01:36
5.42_							
CB212	0.000	1.7	0.0	0.0	0.001	8.7	0 01:35
10.15_							
CB212A	0.000	10.6	0.0	0.0	0.000	62.8	0 01:35
1.32_							
CB213	0.000	4.6	0.0	0.0	0.000	49.4	0 01:38
7.52_							
CB214	0.000	8.1	0.0	0.0	0.005	97.2	0 01:36
25.87_							

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
518A_ (P-Stm)	67.75	32.60	66.10	0.477
System	67.75	32.60	66.10	0.477

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/Full Flow	Max/Full Depth
C2	CONDUIT	24.70	0 01:03	0.50	0.41	1.00
C4	CONDUIT	5.30	0 01:09	0.15	0.06	1.00
C5	CONDUIT	5.42	0 01:06	0.66	0.06	1.00
CB213	CONDUIT	18.57	0 01:06	0.38	0.27	1.00
CB214	CONDUIT	25.87	0 01:09	0.53	0.46	1.00
Pipe_ (104)_ (P-Stm)	2 CONDUIT	66.10	0 01:37	1.05	0.46	0.42
Pipe_ (110)_ (P-Stm)	CONDUIT	64.79	0 01:05	0.72	0.25	1.00
Pipe_ (111)_ (P-Stm)	CONDUIT	40.32	0 01:05	0.49	0.13	1.00
Pipe_ (71)_ (P-Stm)	CONDUIT	92.47	0 01:03	1.04	0.34	1.00
Pipe_ (72)_ (P-Stm)	CONDUIT	65.69	0 01:06	0.89	0.22	1.00
OR1	ORIFICE	66.10	0 01:37			1.00
W1	WEIR	5.11	0 01:21			0.50
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	20.72	0 01:09			0.25
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	16.85	0 01:10			0.31
W6	WEIR	10.31	0 01:11			0.10
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	26.02	0 01:05			
CB201	DUMMY	14.08	0 01:04			
CB202	DUMMY	29.09	0 01:02			
CB203	DUMMY	5.70	0 01:07			
CB204	DUMMY	5.52	0 01:03			
CB205	DUMMY	5.18	0 01:06			
CB206	DUMMY	6.86	0 01:02			
CB207	DUMMY	12.67	0 01:04			
CB209	DUMMY	23.04	0 01:05			
CB210B	DUMMY	5.01	0 01:06			
CB212	DUMMY	5.95	0 01:05			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Dry	Up Dry	Fraction of Down Dry	Sub Crit	Time in Flow Class Sup Crit	Up Crit	Down Crit	Flow Class Norm	Inlet Ctrl
C2	1.00	0.05	0.00	0.00	0.39	0.00	0.00	0.56	0.07	0.00
C4	1.00	0.17	0.01	0.00	0.27	0.00	0.00	0.55	0.03	0.00
C5	1.00	0.15	0.00	0.00	0.28	0.00	0.00	0.56	0.05	0.00
CB213	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.05	0.00
CB214	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.03	0.00
Pipe_ (104)_ (P-Stm)	2 1.00	0.07	0.00	0.00	0.00	0.00	0.00	0.93	0.00	0.00
Pipe_ (110)_ (P-Stm)	1.00	0.06	0.00	0.00	0.38	0.00	0.00	0.56	0.00	0.00
Pipe_ (111)_ (P-Stm)	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.58	0.00
Pipe_ (71)_ (P-Stm)	1.00	0.06	0.00	0.00	0.34	0.00	0.00	0.60	0.03	0.00
Pipe_ (72)_ (P-Stm)	1.00	0.07	0.00	0.00	0.37	0.00	0.00	0.57	0.02	0.00

Conduit Surcharge Summary

Conduit	Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Full Normal Flow	Hours Capacity Limited
C2	1.76	1.76	1.80	0.01	0.01
C4	1.29	1.29	1.54	0.01	0.01
C5	1.23	1.23	1.55	0.01	0.01
CB213	1.50	1.50	1.74	0.01	0.01
CB214	1.63	1.63	1.74	0.01	0.01
Pipe_ (110)_ (P-Stm)	1.90	1.90	1.98	0.01	0.01
Pipe_ (111)_ (P-Stm)	1.74	1.74	1.90	0.01	0.01
Pipe_ (71)_ (P-Stm)	1.56	1.56	1.69	0.01	0.01
Pipe_ (72)_ (P-Stm)	1.73	1.73	1.85	0.01	0.01

Analysis begun on: Tue Jan 28 12:44:03 2025
Analysis ended on: Tue Jan 28 12:44:04 2025
Total elapsed time: 00:00:01

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

08/28/2024

Province:	Ontario	Project Name:	Navan Gas Bar
City:	Ottawa	Project Number:	29899-002
Nearest Rainfall Station:	OTTAWA CDA RCS	Designer Name:	Bobby Pettigrew
Climate Station Id:	6105978	Designer Company:	J.L. Richards & Associates Ltd
Years of Rainfall Data:	20	Designer Email:	bpettigrew@jlrchards.ca
		Designer Phone:	343-804-5381
Site Name:		EOR Name:	
		EOR Company:	
Drainage Area (ha):	0.82	EOR Email:	
% Imperviousness:	73.64	EOR Phone:	

Runoff Coefficient 'c': 0.74

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	19.63
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	66.00
Peak Conveyance (maximum) Flow Rate (L/s):	66.00
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	657
Estimated Average Annual Sediment Volume (L/yr):	534

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EFO4	80
EFO6	90
EFO8	95
EFO10	97
EFO12	99

Recommended Stormceptor EFO Model: **EFO4**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **80**

Water Quality Runoff Volume Capture (%): **> 90**

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor® EF Sizing Report

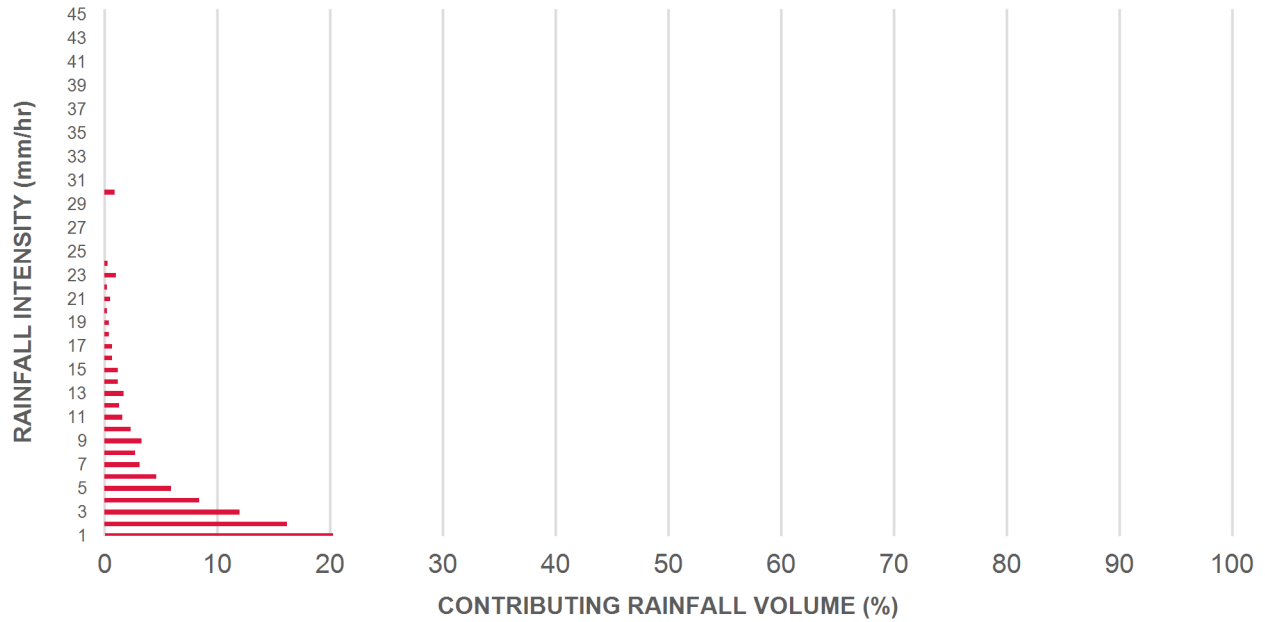
Upstream Flow Controlled Results

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.6	8.6	0.85	51.0	42.0	100	8.6	8.6
1.00	20.3	29.0	1.69	101.0	85.0	98	20.0	28.6
2.00	16.2	45.2	3.38	203.0	169.0	87	14.1	42.7
3.00	12.0	57.2	5.07	304.0	254.0	81	9.7	52.4
4.00	8.4	65.6	6.76	406.0	338.0	77	6.5	58.9
5.00	5.9	71.6	8.46	507.0	423.0	73	4.3	63.3
6.00	4.6	76.2	10.15	609.0	507.0	69	3.2	66.5
7.00	3.1	79.3	11.84	710.0	592.0	65	2.0	68.5
8.00	2.7	82.0	13.53	812.0	676.0	64	1.8	70.2
9.00	3.3	85.3	15.22	913.0	761.0	63	2.1	72.3
10.00	2.3	87.6	16.91	1015.0	846.0	63	1.4	73.8
11.00	1.6	89.2	18.60	1116.0	930.0	62	1.0	74.8
12.00	1.3	90.5	20.29	1218.0	1015.0	61	0.8	75.6
13.00	1.7	92.2	21.98	1319.0	1099.0	59	1.0	76.6
14.00	1.2	93.5	23.68	1421.0	1184.0	57	0.7	77.3
15.00	1.2	94.6	25.37	1522.0	1268.0	56	0.6	77.9
16.00	0.7	95.3	27.06	1623.0	1353.0	53	0.4	78.3
17.00	0.7	96.1	28.75	1725.0	1437.0	51	0.4	78.7
18.00	0.4	96.5	30.44	1826.0	1522.0	48	0.2	78.9
19.00	0.4	96.9	32.13	1928.0	1607.0	46	0.2	79.1
20.00	0.2	97.1	33.82	2029.0	1691.0	43	0.1	79.2
21.00	0.5	97.5	35.51	2131.0	1776.0	41	0.2	79.4
22.00	0.2	97.8	37.20	2232.0	1860.0	39	0.1	79.4
23.00	1.0	98.8	38.90	2334.0	1945.0	38	0.4	79.8
24.00	0.3	99.1	40.59	2435.0	2029.0	36	0.1	79.9
25.00	0.9	100.0	42.28	2537.0	2114.0	35	0.3	80.3
30.00	0.9	100.9	50.73	3044.0	2537.0	29	0.3	80.5
35.00	-0.9	100.0	59.19	3551.0	2959.0	25	N/A	80.3
40.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
45.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
Estimated Net Annual Sediment (TSS) Load Reduction =								80 %

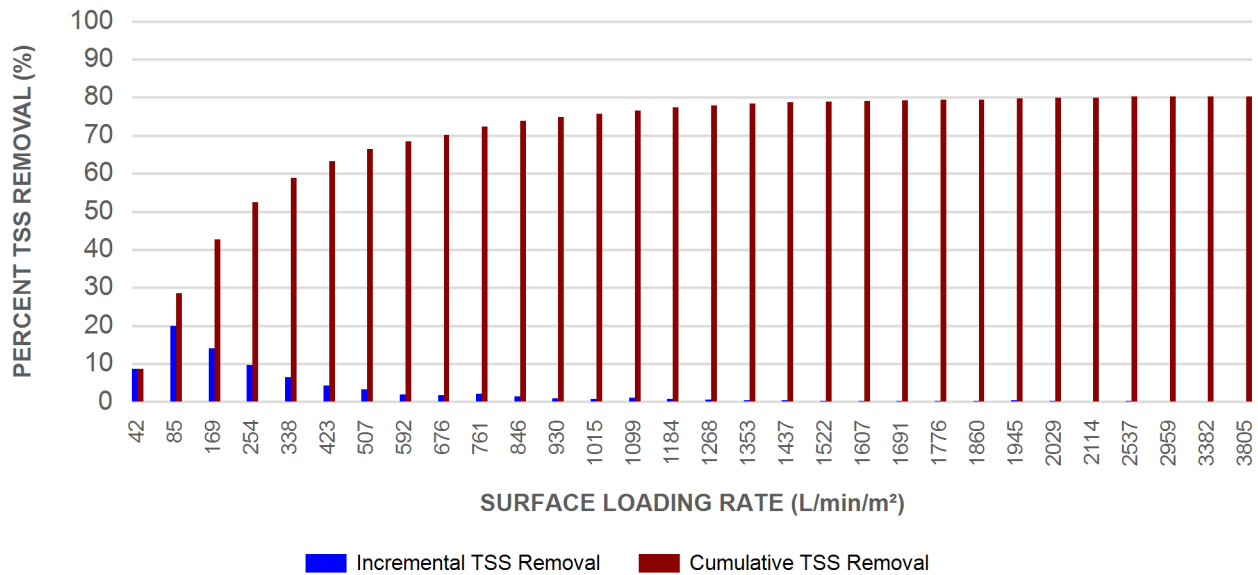
Climate Station ID: 6105978 Years of Rainfall Data: 20

Stormceptor® EF Sizing Report

RAINFALL DATA FROM OTTAWA CDA RCS RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

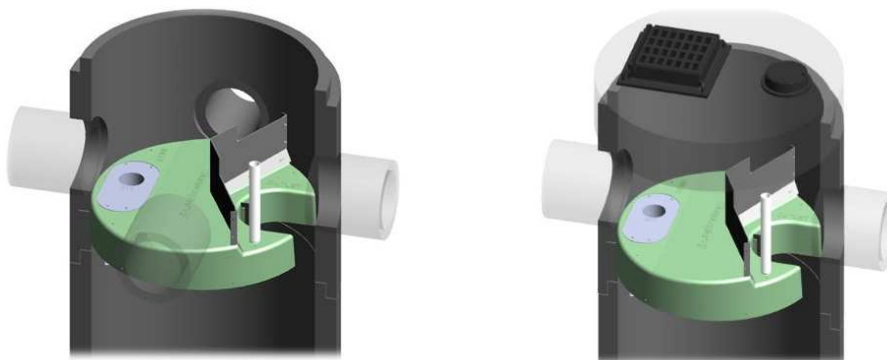
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

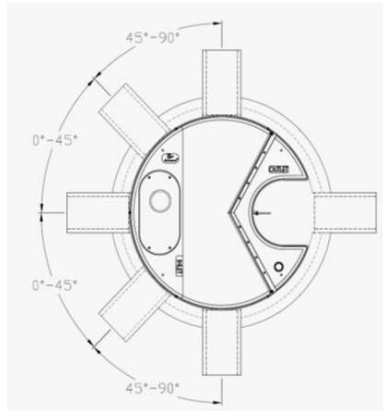
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

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STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall

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remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

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assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.



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