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Site Servicing Report

2983 Navan Road – Block 16 Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash



Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

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1.0 INTRODUCTION

1.1 General

In 2023, J.L. Richards & Associates Limited (JLR) was retained by 12714001 Canada Inc. (the Owner) to prepare the detailed design of municipal infrastructure for Site Plan Approval (SPA) of their parcel located at 2983 Navan Road. The mixed-use site known as “Block 16 - Gas Station, Commercial Building, Drive-Thru Restaurant and Car Wash” will be referred to herein as Block 16. This Site Servicing Report (SSR) presents the servicing constraints and strategies for water, wastewater, stormwater servicing, and stormwater management in accordance with the City of Ottawa Design Guidelines, the associated technical bulletins and relevant design excerpts. This SSR also includes strategies for implementing erosion and sedimentation control measures throughout the construction phase of the project.

1.2 Site Description

The Block 16 is located within the City of Ottawa’s Official Plan boundary and consists of a 0.77 ha parcel bounded by Navan Road to the south and existing residential properties to the east, the proposed East Ridge Orleans Subdivision and Future Mixed-Use Block 15 to the north, and Brian Coburn Boulevard to the west. The legal description of the subject property can be found in the Draft Plan of Subdivision attached to Appendix A.

A topographical survey was completed by Stantec Inc. in August 2023 (Appendix A). The survey indicates that the existing ground surface generally slopes downwards in a southeasterly direction towards Navan Road.

1.3 Proposed Development

The proposed commercial development will consist of a gas station including a car wash, a commercial retail space along with a Drive-Thru Restaurant. The Concept Plan for the Block 16 is attached to Appendix A.

1.4 Proposed Connections to Existing Infrastructure

The proposed site plan will be serviced via the future East Ridge Orleans Subdivision and via existing infrastructure on Navan Road as follows and as shown on the servicing drawings:

Watermain

- Connection to the proposed existing 204 mm diameter watermain along Navan Road

Sanitary

- Connection to the future East Ridge Orleans Subdivision. A 200mm sanitary sewer stub will be dropped to service Block 16 as part of this future subdivision.

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Storm

- Connection to the future East Ridge Orleans Subdivision. A 450mm storm sewer stub will be dropped to service Block 16 as part of this future subdivision.

1.5 Consultation and Permits

An initial pre-consultation meeting was held on July 6, 2022, followed by a Phase 2 Pre-consultation help on September 13, 2023, each to discuss the proposed site plan, the planning approval process requirements, provide clarifications on design criteria, and high level discussion on servicing constraints. A copy of the pre-consultation meeting notes and the site servicing checklist has been provided in Appendix B.

2.0 WATER SERVICING

2.1 Water Supply Design Criteria

A Hydraulic Network Analysis (HNA) was completed as part of the detailed design for the East Ridge Orleans subdivision to confirm that the proposed watermains could provide adequate supply while complying with both the Ottawa Design Guidelines for Water Distribution (July 2010) and Technical Bulletins ISDTB-2014-02, ISTB-2018-02 and ISTB-2021-03. These documents are herein referred to as the Design Guidelines and TB-2014-02, TB-2018-02, and TB-2021-03, respectively.

The HNA completed as part of the East Ridge Orleans Subdivision design included water demands for the Commercial Site Plan (Block 16). The HNA has since been updated to reflect the proposed water service lateral for Block 16 but is based on the same demands and the boundary conditions used in the original East Ridge Orleans Subdivision HNA (refer to Appendix C for a copy of City correspondence for boundary conditions).

Section 4.2.2 of the Design Guidelines states the following criteria for development additions to the public water distribution system:

- Under maximum hourly demand conditions (peak hour), the residual pressures shall not be less than 276 kPa (40 psi);
- During periods of maximum day and fire flow demand, the residual pressure at any point in the distribution system shall not be less than 140 kPa (20 psi);
- In accordance with the Ontario Building Code (OBC) in areas that may be occupied, the static pressure at any fixture shall not exceed 552 kPa (80 psi);
- The maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi); and
- Feeder mains, which have been provided primarily for the purpose of redundancy, shall meet, at a minimum, the basic day plus fire flow demand.

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2.2 Domestic Water Demands

The estimated commercial water demands presented in this section are based on the site layout proposed in the Site Plan (Appendix A). A plug flow of 3.60 L/s was added to each demand scenario to consider the demands required for the car wash (refer to Appendix D for a confirmation letter from the mechanical engineer). Table 1 summarizes the water demands projected for this site.

Table 1: Water Demands

Demand Scenario	Commercial Water Consumption or Peaking Factor	Commercial Water Demands (L/s)	Car Wash Demands (L/s)	Total Demands (L/s)
Average Day Demand	28,000 L/ha/d	0.25	3.60 L/s	3.85
Maximum Day Demand	1.5 x Avg Day	0.37	3.60 L/s	3.97
Peak Hour Demand	1.8 x Max Day	0.67	3.60 L/s	4.27

2.3 Fire Flow Requirements

The City has specified that the Fire Underwriters Survey (FUS) method shall be used for any public or private site where new fire hydrants are being designed. Specifically, the required fire flow (RFF) for each structure was calculated in accordance with TB-2018-02.

The required fire flow for the Commercial Site Plan (Block 16) was calculated to be 83 L/s. Refer to Appendix C for the detailed RFF calculations for the critical fire area.

2.4 Proposed Water Servicing, Boundary Conditions and Water Model

2.4.1 Proposed Water Servicing

Water will be supplied to the Commercial Site Plan (Block 16) by a 150 mm diameter water service that will connect to the existing 305 mm watermain on Navan Road, located east of the intersection between Navan Road and Brian Coburn Blvd. Fire protection will be provided by a new proposed hydrant within the site. As shown in the servicing plan, the car wash is serviced directly by the gas station. Design of the service will be verified by Owner's mechanical engineer however, as noted in Section 2.2, the demand from the car wash has been included in this HNA.

Watermain roughness coefficients were determined using friction factors presented in Section 4.2.12 of the Design Guidelines and the internal pipe diameters were modelled based on Section 4.3.5 of the design Guidelines.

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2.4.2 Boundary Conditions

Hydraulic boundary conditions were provided by the City at the proposed connection location listed in Section 2.4.1 above. Table 2 summarizes the hydraulic boundary conditions received by the City (refer to Appendix C for a copy of the City correspondence). The boundary condition for maximum day plus fire flow corresponds to a required fire flow of 100 L/s. It is noted that the fire flow demand for the City boundary condition is more conservative than the calculated fire flow requirement of 5000 L/min (83 L/s).

Table 2: Hydraulic Boundary Conditions

Demand Scenarios	Connection 3 Head (m)
Maximum HGL	130.7
Peak Hour	126.8
Max Day plus Fire Flow 6,000 L/min (100.00 L/s)	127.3

2.4.3 Water Model

A hydraulic water model within the WaterCAD® software platform was used to carry out the HNA (refer to the overall schematics presented in Appendix C). The water demands from Table 1 and the boundary conditions from Table 2 were input into the model for each demand scenario. Table 3 summarizes the watermain diameters and roughness coefficients used in the model, based on Sections 4.2.12 and 4.3.5 of the Design Guidelines.

Table 3: Watermain Internal Diameters and C-Factors

Nominal Diameter	Inside Diameter	C-Factor
150 mm	155 mm	100
200 mm	204 mm	110
300 mm	297 mm	120

2.5 Simulation Results

The HNA was carried out under steady-state peak hour, maximum day plus fire flow, and maximum pressure conditions to confirm that the proposed water servicing can meet the design criteria outlined in Section 2.1.

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2.5.1 Peak Hour

The simulation results found the minimum pressure at the site during the peak hour condition to be 399 kPa (57.9psi) (refer to Appendix C), which exceeds the minimum pressure criterion of 276 kPa (40 psi) per the Design Guidelines.

2.5.2 Maximum Day Plus Fire Flow

Fire water supply will be provided by a proposed hydrant off the 150 mm diameter water service for Block 16. Hydrant spacing was carried out in accordance with the Design Guidelines.

To ensure adequate fire protection, the maximum day demand shown in Table 1 was analyzed simultaneously with the fire flow requirements. The fire flow simulation was carried out by allowing WaterCAD® to calculate the maximum fire flow that can be drawn from the hydrant without allowing any part of the system to experience pressures less than 140 kPa (20 psi). Using the 6,000 L/min (100 L/s) boundary condition provided by the City (refer to Table 2), the system is expected to deliver a minimum of 6,000 L/min (100 L/s) within the site. Per Appendix I of TB-2018-02, adequate water supply can be provided by the hydrant to the proposed site.

2.5.3 Maximum Pressure

Based on a zero (0 L/s) demand condition, the simulation results found the maximum pressure at the site to be 437 kPa (63.4). This value is below the maximum pressure constraint of 552 kPa (80 psi), therefore pressure reducing valves (PRVs) are not anticipated to be required.

2.6 Summary and Conclusions

Based on the water simulation results, the proposed development can be serviced by the proposed 150 mm water service lateral connected to the 305 mm diameter watermain on Navan Road. Furthermore, adequate fire water supply can be achieved with the proposed hydrant off the 150 mm water lateral servicing Block 16.

3.0 WASTEWATER SERVICING

3.1 Design Criteria

The sanitary sewer system within the Block 16 is designed in accordance with the Ottawa Sewer Design Guidelines and subsequent technical bulletins. The design parameters are applied under two scenarios as per ISTB Technical Bulletin 2018-01. The key design parameters have been summarized in Table 4.

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Table 4: Wastewater Key Design Parameters

Design Parameter	Design Value
Commercial Average Flow	28,000 L/gross ha/Day
Residential Average Flow	280 L/Cap/Day
Residential Peaking Factor	Harmon's Formula
Commercial Peaking Factor	1.5
Harmon's Correction Factor (K)	0.8
Infiltration Allowance	0.33 L/s/ha
Manning's Roughness Coefficient (n)	0.013
Allowable Slopes	Varies (Refer to Section 6.1.2.2 of ODSG)
Allowable Velocities	0.6 m/s – 3.0 m/s
Allowable Freeboard	-

3.2 Proposed Sanitary Servicing and Design Flows

Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer system. Wastewater will then discharge into the East Ridge Orleans Subdivision via a 200mm sanitary sewer stub proposed as part of the subdivision as shown on the Servicing Plan.

Wastewater flows from the proposed development are presented in the Block 16 Sanitary Design Sheet (refer to Appendix D). Based on the design criteria presented in Table 3-1 the total design peak flow of 4.23 L/s is calculated for the development which is based on the site area of 0.77ha. Table 5 summarizes the results from the sanitary design sheet.

Table 5: Sanitary Design Flow Summary

Commercial Type	Site Area	Average Flow	Com. Peak Flow	Infilt. Flow	Total Flow
Commercial Flows	0.77 ha	28,000 L/gross ha/Day	0.374 L/s	0.254 L/s	0.628 L/s
Car Wash Flows	-	-	-	-	3.6 L/s
Total Wastewater Flows – Block 16					4.23 L/s

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The flows from Block 16 (4.23 L/s) were incorporated in the detailed design of the sanitary sewer within the future East Ridge Orleans Subdivision thus there is sufficient downstream capacity. It is proposed to adopt the sanitary servicing strategy described in this section.

3.3 Summary and Conclusions

Wastewater servicing for Block 16 will be designed in accordance with the City of Ottawa Sewer Design Guidelines, the associated technical bulletins, and various background documents as highlighted throughout this section. Wastewater generated from the Block 16 will be conveyed via a proposed 200 mm diameter sanitary sewer outletting to the East Ridge Orleans Subdivision to the north of the site. It is recommended that this wastewater servicing plan be implemented to provide adequate sanitary servicing for the proposed development.

4.0 STORM SERVICING AND STORMWATER MANAGEMENT

4.1 Design Criteria

Storm and stormwater management servicing for the Gas Bar Site Plan (Block 16) was developed in accordance with the City of Ottawa 2012 Sewer Design Guidelines (OSDG) and the more recent Technical Bulletin PIEDTB-2016-01 (September 6, 2016). These two documents are herein referred to as the Design Guidelines in this section. A summary of the key storm and stormwater management criteria follows:

- Control minor system flows to the allowable release rates of 68 L/s as identified in Table 5-4 Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2024);
- The runoff coefficients (C-factors) to be calculated based on the ratio of pervious and impervious surfaces depicted on proposed site plans;
- Minimum roadway slope of 0.1% from crest-to-crest for overland flow route;
- Maximum parking ponding depth of 350 mm (static and dynamic) as per the Design Guidelines and maximum depth of surface flow to be 300 mm;
- Minimum vertical clearance of 0.15 m between the spill elevation on the street and the finished grade;
- Major system flows, up to and including the 1:100-year design storm event, are contained within the site.
- Quality control will be accommodated by Pond #3 to meet an MECP Enhanced Level of Protection (80% TSS removal) as identified in Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).
- Ponding in landscaped areas to enhance groundwater recharge in accordance with the City of Ottawa Urban Design Guidelines for Gas Stations, Guideline 30.
- Provide measures to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

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4.2 Proposed Stormwater Management Approach

In order to achieve the allowable release rates, the stormwater management of the site will include:

- Surface storage within the site in greater than a 1:2-year event with captured flows conveyed to the minor system;
- Controlled release of the flows captured in the minor system for the entire site using Inlet Control Devices (ICDs).
- Flows stored in oversized storage pipes underground. Flows will accumulate in the storage pipes and be released from the site via an OGS and orifice control into the minor system on Paleo Drive.

4.3 Proposed Minor System Servicing

Internal to the gas station site, two minor system sewer runs will be provided on either side of the main building structure. Both of these sewer runs will be oversized and will connect into a single manhole upstream of the connection upstream of the easement stub, upstream of Paleo Drive. The connection manhole will have a 127mm diameter orifice plate on the downstream outlet sewer to control to the allowable release rate, 68 L/s, as defined in the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023). The upsized storm sewers will be 525mm diameter.

Downstream of the orifice an Stormceptor EFO4 model, or equivalent, will provide 80% TSS removal as well as capture of oils and spills.

The runoff coefficient is based on the ratio of impervious surfaces and areas. A design sheet for sizing of the sewers to confirm capacity for the 1:2-year rational method flow is provided in Appendix E.

The gas station roof structure is uncontrolled and drains directly to the minor system.

There are no basements and therefore no HGL constraints in the system.

4.4 Stormwater Management Modelling Approach

4.4.1 Dual Drainage Model

The analysis of both major and minor drainage systems was carried out to demonstrate their compliance with respect to the design criteria described in Section 4.6. The performance of the major overland system and minor storm sewer system was analyzed with PCSWMM. This software is a dynamic model which allows both hydrologic and hydraulic components to be simulated in the same platform and also allows the simulation of the interaction between the major and minor systems. The PCSWMM software platform was used to:

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- Generate the surface runoff hydrograph for each sub-area under various recurrences.
- Subdivide each inflow hydrograph into its minor and major system components based on the proposed inlet capture rates and roadway sag storage.
- Assess cascading, if any, and carry out dynamic routing of storm flows to determine flow depths along the roadways. As previously stated, the maximum major overland flow depths within the parking lot areas are to be limited to 350 mm or less, as per Technical Bulletin PIEDTB-2016-01.

PCSWMM was set-up to evaluate the proposed servicing as detailed on Drawing C01 and C02. Subcatchments were delineated for the structure roof areas, parking lot low points and landscaped low points. Model schematics are prepared in Appendix F.

4.4.2 Boundary Conditions

Boundary conditions are taken from the downstream subdivision model issued as part of the Site Servicing Report 2983, 3053 and 3079 Navan Road & 2690 Pagé Road, Ottawa, Ontario (JLR 2023).

The downstream 1:100-year HGL at the connection to the Subdivision at MH518A is identified as 81.28 m. The boundary condition in the model was set at this elevation as a constant during the storm simulations.

4.5 Modelling Parameters

4.5.1 Hydrological Modelling Parameters

The following parameters were used in the hydrologic component of PCSWMM:

- **Areas and Imperviousness:** Catchment ID and drainage areas used by PCSWMM match those shown on either Drawing DST or Figure E-1 (Appendix E1). Sealed and roof areas are set at 100% impervious and other grassed or landscaped areas are pervious.
- **Catchment Width:** The catchment width is estimated at the width of overland sheet flow based on the grading of the catchment and slope direction.
- **Manning's Roughness Coefficient:** Manning's Roughness Coefficients of 0.013 and 0.25 were used for the impervious and pervious surfaces, respectively.
- **Horton Infiltration parameters:** City of Ottawa OSDG Horton Infiltration Parameters have been used in the modelling.

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- **Initial Abstraction:** Initial abstraction of 4.67 mm and 1.57 mm was used for the pervious and impervious surfaces respectively, consistent with the OSDG.

4.5.2 Simulation of Storm Distributions

To assess peak flow rates and peak volume storage requirements the 3-hour Chicago storm has been simulated for the site for the 1:2-year event and 1:100-year event and the 24-hour SCS storms for the 1:100-year event.

4.6 Simulation Results

4.6.1 Low Point Ponding Analysis

Ponding depths in the low points in the parking area and landscaped areas are shown in Table 6.

Table 6: Catchbasin Ponding Depths

Low Point ID	Top of Grate (m)	Maximum Static Depth (mm)	3-hour Chicago 1:2 year Depth (mm)	3-hour Chicago 1:100 year Depth (mm)	24-hour SCS 1:100 year Depth (mm)
1	85.25	190/300	0	260	230
2A	85.35	250	0	0	0
2B	85.40	220	0	0	0
3	85.35	300	0	60	30
4	84.75	150	0	100	0
5	85.35	150	0	100	80
6	85.37	80	0	100	90
7	84.55	300	0	300	160
8	85.04	270	0	290	250
9	84.80	300	0	200	130
10	85.18	240	0	150	90
11	85.12	300	0	210	170
12	85.45	150	0	90	70
13	85.50	150	0	160	150

The simulation results compiled in Table 6 shows that:

- No ponding nor dynamic flow will occur in the 1:2-year event;

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- Maximum ponding depth of 300 mm during the 1:100-year event; and,
- There is no spill from the site in the 1:100-year event.

4.6.2 Site Release Rate

Table 8 below shows the release rates from the site via the 127mm diameter orifice plate. All release rates are below the 68 L/s allowable release rate.

Table 7: Release Rates

	3-hour Chicago 1:2 year Release Rate (L/s)	3-hour Chicago 1:100 year Release Rate (L/s)	24-hour SCS 1:100 year Release Rate (L/s)
Flow at MH 514	47	66	65

4.7 Water Quality

An OGS unit is proposed for the site to provide site specific water quality to 80% TSS removal and capture of oils and spills. The sizing details for the unit are contained in Appendix F. The unit sized for the site is an Stormceptor EFO4, or equivalent.

4.8 Summary and Conclusions

The stormwater servicing achieves a release rate from the site to the minor system limited to the allowable release rate and contains up to the 1:100 year on site.

5.0 Erosion and Sediment Control

Erosion and sediment control measures, as outlined in the Ontario Ministry of Natural Resources (MNR) Guidelines on Erosion and Sediment Control for Urban Construction Sites, will be implemented to trap sediment on site. The following erosion and sediment control measures can be implemented during construction as shown on the Erosion and Sediment Control Plan (Drawing ESC):

- Supply and installation of a silt fence barrier, as per OPSD 219.110.
- Supply and installation of siltsack or sentinel CB inserts between the frame and cover of catch basins and maintenance holes adjacent to the project area during construction, to prevent sediment from entering the sewer system.
- Stockpiling of material during construction is to be located along flat areas away from drainage paths. For material placed on sloped areas, stockpiles are to be enclosed with a silt fence to protect watercourses.

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- All catch basins are to be equipped with sumps, inspected frequently, and cleaned as required.
- Temporary ICDs are to be placed blocking part of the sewer pipe in the connecting storm maintenance holes to eliminate construction debris from entering the existing storm sewer system. The ICDs are to be removed after the proposed storm sewers have been fully cleaned.
- A mud mat is to be built at each of the site entranceways to prevent the transport of sediment onto paved surfaces. The mud mat shall be:
 - Minimum of 20 m in length for the full width of the entrance way (10 m wide minimum).
 - Minimum of 400 mm thick underlain with a geotextile (or graded aggregate filter); and
 - Constructed with 50 mm diameter clear stone for the first 10 m (extending from the paved street) and the remainder of the length with 150 mm diameter clear stone.

The proposed removal and reinstatement measures as well as the erosion control measures shall conform to the following documents:

- “Guidelines on Erosion and Sediment Control for Urban Construction Sites” published by Ontario Ministries of Natural Resources, Environment, Municipal Affairs, and Transportation & Communication, Association of Construction Authorities of Ontario and Urban Development Institute, Ontario, May 1987.
- “MTO Drainage Manual”, Chapter F: “Erosion of Materials and Sediment Control”, Ministry of Transportation & Communications, 1985.
- “Erosion and Sediment Control” Training Manual by Ministry of Environment, Spring 1998.
- Applicable Regulations and Guidelines of the Ministry of Natural Resources.

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6.0 CONCLUSIONS

Block 16 will be serviced as follows:

- Water servicing will be provided by connection to the proposed watermain along the Navan Road.
- Wastewater servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Storm servicing will be provided by a connection to the future East Ridge Orleans Subdivision
- Flows exceeding the allowable peak flow for Block 16, will be held on-site, using a combination of both, above ground and underground storage.

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This report has been prepared by J.L. Richards & Associates Limited for 12714001 CANADA INC.'s exclusive use. Its discussions and conclusions are summary in nature and cannot properly be used, interpreted or extended to other purposes without a detailed understanding and discussions with the client as to its mandated purpose, scope and limitations. This report is based on information, drawings, data, or reports provided by the named client, its agents, and certain other suppliers or third parties, as applicable, and relies upon the accuracy and completeness of such information. Any inaccuracy or omissions in information provided, or changes to applications, designs, or materials may have a significant impact on the accuracy, reliability, findings, or conclusions of this report.

This report was prepared for the sole benefit and use of the named client and may not be used or relied on by any other party without the express written consent of J.L. Richards & Associates Limited, and anyone intending to rely upon this report is advised to contact J.L. Richards & Associates Limited in order to obtain permission and to ensure that the report is suitable for their purpose.

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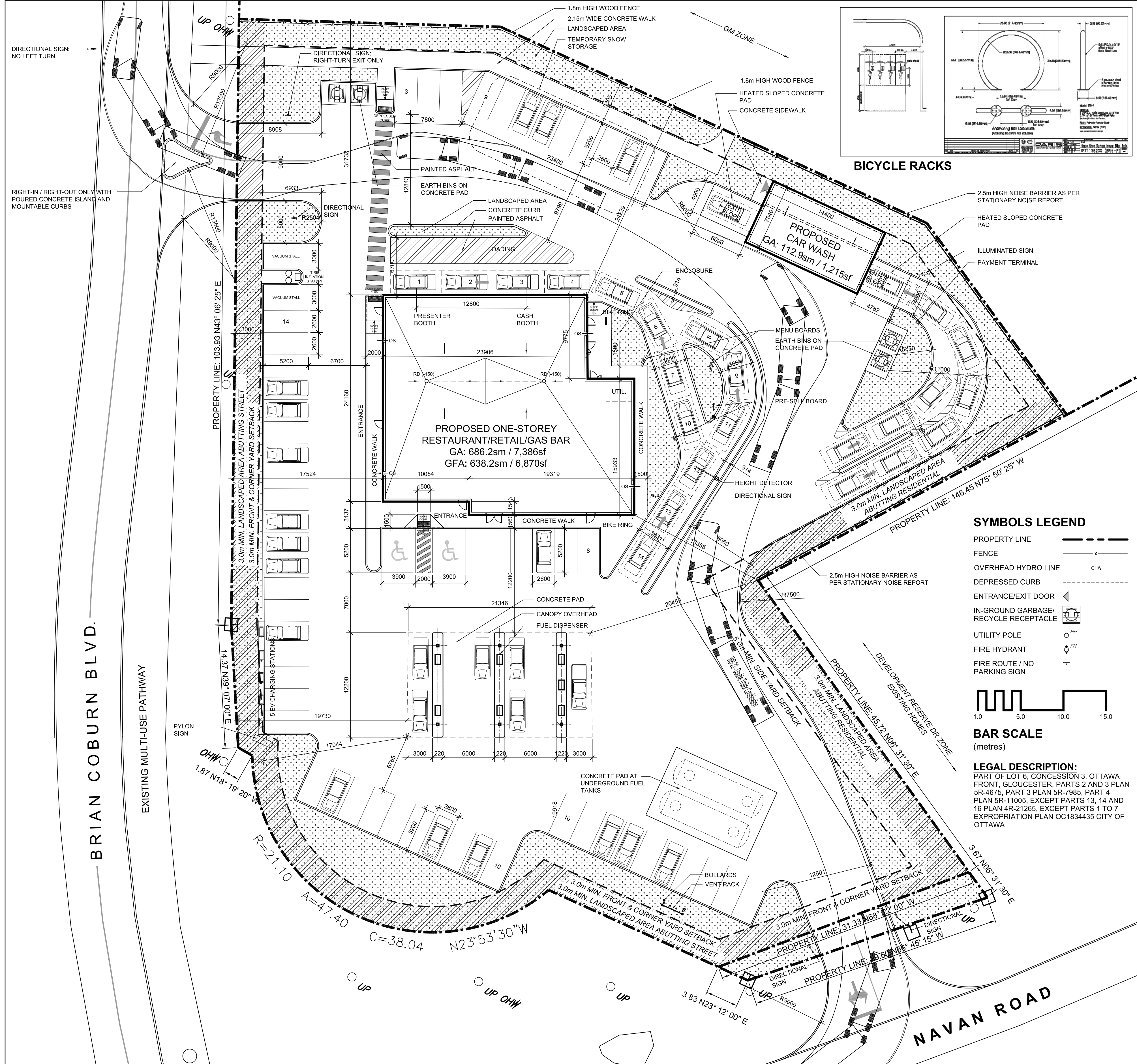


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Appendix A

Concept Plan, Draft Plan of
Subdivision and Topographical
Survey



SITE INFORMATION

SITE AREA: 7,717sm / 1.93 acres

NOTE THAT PROPERTY BOUNDARY INFORMATION HAS BEEN TAKEN FROM SURVEY PREPARED BY STANTEC, DATED 12 OCTOBER 2023.

BUILDING DATA:

AREA CALCULATIONS:

Gross Area (by Ontario Building Code definition):
The total area of all floors above grade measured between the outside surfaces of exterior walls is:

Retail Building: 686.2sm / 7,386sf
Carwash Building: 112.9sm / 1,215sf
Total Gross Area: 799.1sm / 8,600sf

Gross Floor Area (City of Ottawa Zoning Bylaw definition for the purpose of determining maximum building area and parking requirements): The total floor area measured from the interior of outside walls excluding mechanical/electrical service rooms, stairwells, elevator shafts, parking/loading facilities, washrooms and storage areas:

GFA (Restaurant/Retail Bldg): 602sm / 6,480sf

ZONING

DESIGNATION: GM[2546] H(14.5)
General Mixed use, Exception 2546

PERMITTED NON RESIDENTIAL USES:

Section 187: Convenience Store
Drive-through Facility
Restaurant
Retail Store
Car Wash
Gas Bar

Exception 2546:

MINIMUM SETBACKS:

Table 187(c): Front & Corner Yard: 3.0m
Table 187(d): Interior Side Yard: 5.0m (abutting res. zone)
Table 187(e.iii): Rear Side Yard: 7.5m (abutting res. bldg)

BUILDING HEIGHT:

Exception 2546: 14.5m maximum permitted
5.5m proposed

FSI:

Table 187(g): 2.0 times coverage (15,000sm) maximum permitted
0.1 times coverage (799.1sm) proposed

MINIMUM WIDTH OF LANDSCAPED AREA:

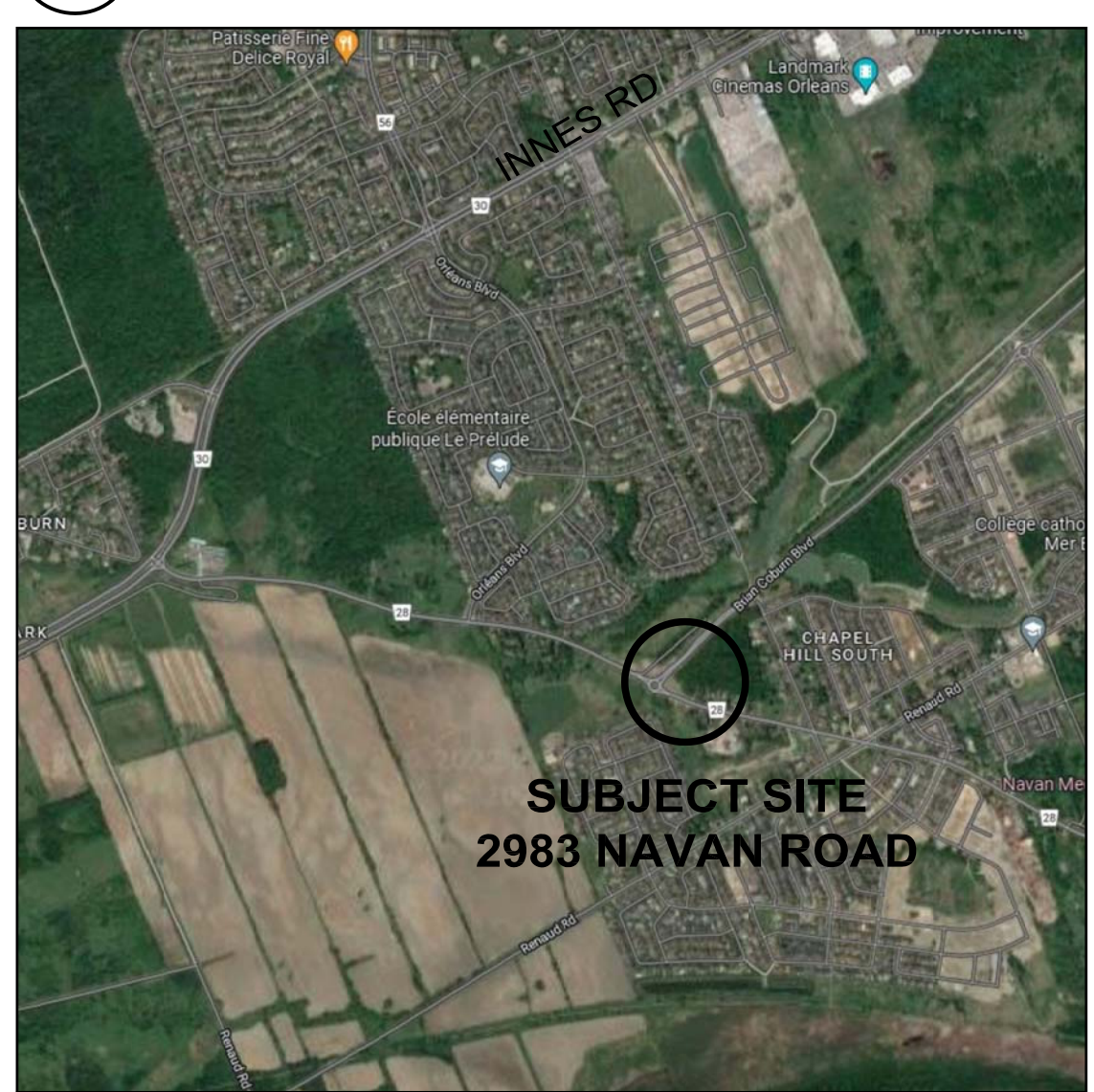
Table 187(h.i): Abutting a Street: 3.0m
Table 187(h.ii): Abutting a Res.Zone: 3.0m

PARKING:

Table 101: Convenience Store: 3.4 cars per 100sm of GFA
Fast-Food Restaurant: 10 cars per 100sm of GFA
9 for Convenience Store (266sm/100x3.4)
34 for Restaurant (336sm/100x10)
Provided: 61 cars (not incl. fuel dispensers & drive-through)

03 SITE PLAN
SP-A01 SCALE: 1:250

02 SITE & BUILDING DATA and ZONING REVIEW
SP-A01 SCALE: NTS

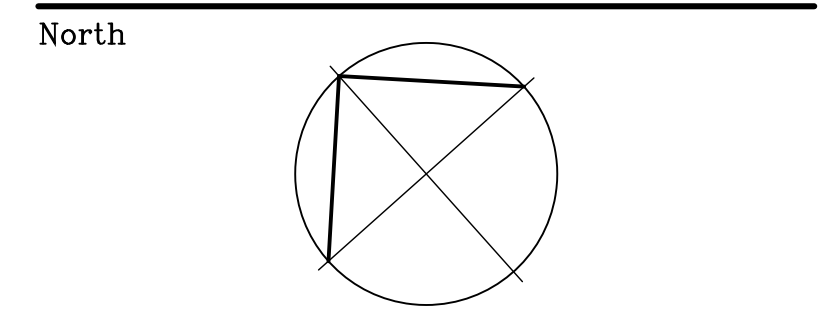


01 LOCATION PLAN
SP-A01 SCALE: NTS

OWNER:
1274001 CANADA INC.
100-768 Boulevard St Joseph
Gatineau, QC J8Y 4B8

PLANNING, CIVIL & TRAFFIC CONSULTANT:
J.L.RICHARDS & ASSOCIATES LTD.
1000-343 Preston Street
Ottawa, ON K1A 1N4

LANDSCAPE ARCHITECT:
JAMES B. LENNOX & ASSOCIATES INC.
3332 Carling Avenue
Ottawa, ON K2H 5A8



Revisions

No.	By	Description	Date
08	IW	SITE PLAN APPLICATION	08 DEC 2023
09	IW	REVISED FOR SPA	01 MAR 2024
10	IW	REVISED FOR SPA	14 AUG 2024

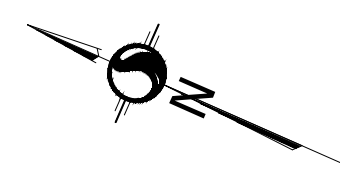
Project
NEW RESTAURANT, CONVENIENCE STORE & GAS BAR

2130 BRIAN COBURN BLVD.
Drawing
SITE PLAN

Scale AS SHOWN
Drawn AK / KE
Checked

Project No. 22-127
Date 12 MAY 2022
Drawing No. **SP-A01**
PLAN NO.

44-20-2033



PAGE ROAD
ROAD ALLOWANCE BETWEEN LOTS 5 & 6 (AS WIDENED)
PIN 04404-0409

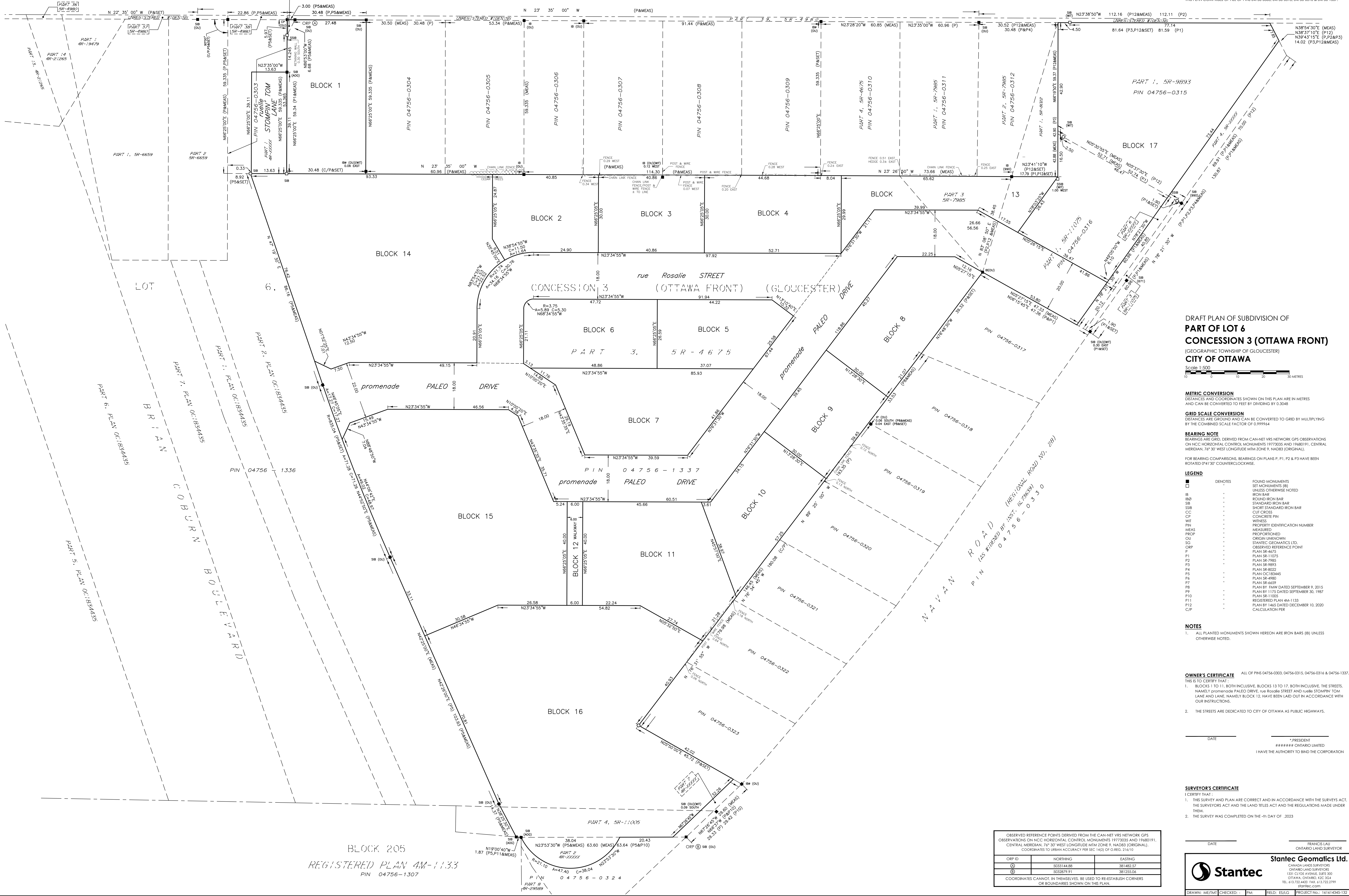
APPROVED UNDER SECTION 51 OF THE PLANNING ACT BY THE
CITY OF OTTAWA THIS ____ DAY OF _____ 20____

DON HERWEYER, M.C.P., R.P.P., ACTING GENERAL
MANAGER PLANNING, REAL ESTATE AND
ECONOMIC DEVELOPMENT DEPARTMENT,
CITY OF OTTAWA

PLAN 4M-

I HEREBY CERTIFY THAT THIS PLAN 4M-_____ IS REGISTERED IN THE
LAND REGISTRY OFFICE FOR THE LAND TITLES DIVISION OF
OTTAWA-CARLETON (No.4) AT _____ O'CLOCK ON THE _____ DAY OF
_____ 2023 AND ENTERED IN THE REGISTER FOR P.L.N.'s
04756-0303, 04756-0315, 04756-0316 & 04756-1337, AND THE REQUIRED
CONSENTS ARE REGISTERED AS PLAN DOCUMENT NUMBER
OC-_____
REPRESENTATIVE FOR LAND REGISTRAR

THIS PLAN COMPRISES OF ALL OF PINS 04756-0303, 04756-0315, 04756-0316 & 04756-1337.



DRAFT PLAN OF SUBDIVISION OF
**PART OF LOT 6
CONCESSION 3 (OTTAWA FRONT)**
(GEOGRAPHIC TOWNSHIP OF GLOUCESTER)
CITY OF OTTAWA

Scale 1:500
0 5 10 15 20 30 METRES

METRIC CONVERSION
DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES
AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

GRID SCALE CONVERSION
DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING
BY THE COMBINED SCALE FACTOR OF 0.999964

BEARING NOTE
BEARINGS ARE GRID, DERIVED FROM CAN-NET VRS NETWORK GPS OBSERVATIONS
ON NCC HORIZONTAL CONTROL MONUMENTS 1977035 AND 1980191, CENTRAL
MERIDIAN, 76° 30' WEST LONGITUDE MAM ZONE 9, NAD83 (ORIGINAL).

FOR BEARING COMPARISONS, BEARINGS ON PLANS P. 1, P. 2 & P. 3 HAVE BEEN
ROTATED 0°41'30" COUNTERCLOCKWISE.

LEGEND

SYMBOL	DENOTES	FOUND MONUMENTS
□	SET MONUMENTS (B)	UNLESS OTHERWISE NOTED
IB	IRON BAR	ROUND IRON BAR
SB	STANDARD IRON BAR	SHORT STANDARD IRON BAR
CC	CUT CROSS	CONCRETE PIN
WIT	WITNESS	
FIN	PROPERTY IDENTIFICATION NUMBER	
MEAS	MEASURED	
PROP	PROPORTIONED	
CU	CORNER UNDERNOWN	
SG	STANTEC GEOMATICS LTD.	
CRP	OBSERVED REFERENCE POINT	
P1	PLAN SR-4675	
P2	PLAN SR-7995	
P3	PLAN SR-8893	
P4	PLAN SR-8822	
P5	PLAN OC18345	
P6	PLAN SR-4993	
P7	PLAN SR-6659	
P8	PLAN BY FIRM DATED SEPTEMBER 9, 2015	
P9	PLAN BY 1175 DATED SEPTEMBER 30, 1987	
P10	PLAN SR-11025	
P11	REGISTERED PLAN 4M-1133	
P12	PLAN BY 1465 DATED DECEMBER 10, 2020	
C/P	CALCULATION PIN	

NOTES

1. ALL PLANTED MONUMENTS SHOWN HEREON ARE IRON BARS (B) UNLESS OTHERWISE NOTED.

OWNER'S CERTIFICATE

ALL OF PINS 04756-0303, 04756-0315, 04756-0316 & 04756-1337.

THIS IS TO CERTIFY THAT

1. BLOCKS 1 TO 11, BOTH INCLUSIVE, BLOCKS 13 TO 17, BOTH INCLUSIVE, THE STREETS, NAMELY PROMENADE PALEO DRIVE, rue Rosalie STREET and rue STONPIN TOM LANE AND LANE, NAMELY BLOCK 12, HAVE BEEN Laid OUT IN ACCORDANCE WITH OUR INSTRUCTIONS.

2. THE STREETS ARE DEDICATED TO CITY OF OTTAWA AS PUBLIC HIGHWAYS.

DATE _____ PRESIDENT
FRANCOIS LAU
ONTARIO LAND SURVEYOR
I HAVE THE AUTHORITY TO BIND THE CORPORATION

SURVEYOR'S CERTIFICATE

I CERTIFY THAT

1. THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEYS ACT, THE SURVEYS ACT AND THE LAND TITLES ACT AND THE REGULATIONS MADE UNDER THEM.

2. THE SURVEY WAS COMPLETED ON THE ____ DAY OF _____ 2023.

DATE _____

FRANCOIS LAU
ONTARIO LAND SURVEYOR

Stantec
CANADA LAND SURVEYORS
ONTARIO LAND SURVEYORS
1331 COLLE AVENUE, SUITE 300
OTTAWA, ONTARIO, CANADA K1G 3G4
TEL: 416-752-4400 FAX: 416-752-2799
dgn@stn.com

DRAWN: ME/MTM CHECKED: _____ PLOT: _____ FIELD: ES/LG PROJECT NO.: 16161436-132

BLOCK 205
REGISTERED PLAN 4M-1133
PIN 04756-1307

OBSERVED REFERENCE POINTS DERIVED FROM THE CAN-NET VRS NETWORK GPS OBSERVATIONS ON NCC HORIZONTAL CONTROL MONUMENTS 1977035 AND 1980191, CENTRAL MERIDIAN, 76° 30' WEST LONGITUDE MAM ZONE 9, NAD83 (ORIGINAL). COORDINATES TO UTM ACCURACY PER SEC 1403.03 REG. 314115

CRP ID	NORTHING	EASTING
(1)	9333144.68	381482.57
(2)	9332879.91	381255.06

COORDINATES CANNOT, IN THEMSELVES, BE USED TO RE-ESTABLISH CORNERS OR BOUNDARIES SHOWN ON THIS PLAN.

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix B

Pre Consultation Meeting Notes
and Site Servicing Report
Checklist

Carmine Zayoun
12714001 Canada Inc (Zayoun Group)
Via email: carmine@zayoungroup.com

**Subject: Pre-Consultation: Meeting Feedback
Proposed Site Plan Application – 2983 Navan Road ‘
Gas Station and Commercial Building – PC2023-0227**

Please find below information regarding next steps as well as consolidated comments from the above-noted pre-consultation meeting held on September 13, 2023.

Pre-Consultation Preliminary Assessment

1 <input type="checkbox"/>	2 <input type="checkbox"/>	3 <input type="checkbox"/>	4 <input checked="" type="checkbox"/>	5 <input type="checkbox"/>
----------------------------	----------------------------	----------------------------	---------------------------------------	----------------------------

One (1) indicates that considerable major revisions are required while five (5) suggests that the proposal appears to meet the City’s key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

Next Steps

1. A review of the proposal and materials submitted for the above-noted pre-consultation has been undertaken. Please proceed to complete a Phase 2 / Phase 3 Pre-consultation Application Form and submit it together with the necessary studies and/or plans to planningcirculations@ottawa.ca.
2. In your subsequent pre-consultation submission, please ensure that all comments or issues detailed herein are addressed. A detailed cover letter stating how each issue has been addressed must be included with the submission materials. Please coordinate the numbering of your responses within the cover letter with the comment number(s) herein.
3. Please note, if your development proposal changes significantly in scope, design, or density before the Phase 3 pre-consultation, you may be required to complete or repeat the Phase 2 pre-consultation process.

Supporting Information and Material Requirements

1. The attached **Study and Plan Identification List** outlines the information and material that has been identified, during this phase of pre-consultation, as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City’s Terms of Reference (ToR) and/or Guidelines, as available on Ottawa.ca. These ToR and Guidelines outline



the specific requirements that must be met for each plan or study to be deemed adequate.

Consultation with Technical Agencies

1. You are encouraged to consult with technical agencies early in the development process and throughout the development of your project concept. A list of technical agencies and their contact information is enclosed.

Planning

Comments:

1. In the Official Plan the subject site is designated as Neighbourhood is modified with the Evolving Neighbourhood overlay. Brian Coburn Boulevard is also designated as a Minor Corridor. The property is further identified as Low-density residential in the EUC – Phases 1 Community Design Plans (CDP). The subject lands are currently zoned GM[2546]H(14.5) General Mixed-Use, Exception.

2. Committee of Adjustment

No variances have been identified at this point. Staff will set up a meeting with a Committee of Adjustment Plan if any required.

3. Design guidelines

[Urban Design Guidelines for Drive-Through Facilities](#)

[Urban Design Guidelines for Gas Stations](#)

4. Landscape requirements

Landscape buffers will need to comply with Section 110 of the Zoning By-law and are consistent with the design guidelines.

The turning radius encroached into the landscape buffers on the demonstration plan. Ensure all landscaping is protected by barrier curbs

5. Parking requirements

Parking should comply with Sections 100, 101, 106, 109 and 111

Vehicle and bicycle parking should be situated with easily access while minimizing pavement. There seems to be a lot of pavement in front of this building.

6. Easements

Are there service easements required over the pedestrian walkway connecting this property with the subdivision?

7. Confirm that the location of the Car Wash will not become a noise problem for future residents of the townhouses behind it. Noise study will need to identify any issues and provide remediation measures.
8. Provide locations of signage and ensure that space is made available for tree planting

Urban Design

9. Relevant guidelines – The City’s Urban Design Guidelines for Gas Stations and Urban Design Guidelines for Drive-Through Facilities are both applicable to this site. The applicant should ensure their submission meets the direction of these guidelines.
10. Design Brief - A Design Brief is required. Please refer to the attached Terms of Reference for details.
11. Public realm – Please refer to the attached PDF for Urban Design comments related to the location of sidewalks, walkways and pedestrian movements on the site
12. Landscaping - Extensive tree and shrub planting is need on this site, in particular to soften the interface with the existing and future residential and to enhance the ROW.

Feel free to contact the Urban Design Planner, Christopher Moise, at Christopher.Moise@ottawa.ca , for follow-up questions

Engineering

Comments:

13. General Comments:

- a. Review of the Phase 3 submission for this application will not occur until the detailed design of the subdivision that it is within (D07-16-21-0027) is approved.
- b. At the stage of site plan approval, a condition will be imposed detailing that a commence work notification will not be issued until the subdivision’s infrastructure is in-service.

14. Engineering Studies:

- a. All engineering studies (detailed in the Study and Plan Identification List form) are to follow the to be approved draft plan of subdivision D07-16-21-0027.

- b. An interceptor pit is required with the provision of the car wash.
15. An MECP Environmental Compliance Approval **Industrial Sewage Works** will be required for the proposed development. Please contact the Ministry of the Environment, Conservation and Parks, Ottawa District Office to arrange a pre-submission consultation:

b. Emily Diamond at (613) 521-3450, ext. 238 or
Emily.Diamond@ontario.ca

Note: this site does not meet the City's requirements for ToR. To have the ECA application reviewed under ToR, a request will need to be sent to Charles Warnock (charles.warnock@ottawa.ca).

Feel free to contact Reed Adams (reed.adams@ottawa.ca), Infrastructure Project Manager, for follow-up questions.

Noise

Comments:

16. A stationary noise report for the car wash is required because of the adjacent residential.

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca , for follow-up questions.

Transportation

Comments:

17. A full TIA is not required as this site is covered under the recent subdivision TIA.
18. A memo including the pertinent subdivision trips is sufficient.
19. The right of way protection on Brian Coburn and Navan Roads is 40m and 37.5m, respectively

Feel free to contact the Senior Transportation Engineer, Mike Giampa, at Mike.Giampa@ottawa.ca , for follow-up questions

Planning Forestry

Comments:

20. A Tree Conservation Report and Landscape Plan must be submitted with both SPC applications

21. A permit is required prior to any tree removal on site. The tree permit will be released upon site plan approval. Please contact the File Lead or the Planning Forester, Hayley Murray (hayley.murray@ottawa.ca) for information on obtaining the tree permit.
22. If marine clay soils are present, setbacks on City properties must adhere to the 2017 SMC guidelines (attached). The Geotechnical report must address the implications of these soils, if present, on tree planting in relation to private land.
23. If underground parking is planned, a design must be provided for the site to support tree planting
24. We expect a very strong landscape plan to re-establish canopy cover across the properties. Tree planting and protecting existing urban forest canopy is imperative to reach the City's target of 40% canopy cover.

Feel free to contact Hayley Murray, Planning Forester, for follow-up questions.

Parkland

Comments:

25. Parkland contributions were made through the Subdivision process.

Feel free to contact Jessica Button, Parks Planner, for follow-up questions.

Conservation Authority

Comments:

26. The Rideau Valley Conservation authority will be commenting on this application

Feel free to contact RVCA, for follow-up questions.

Other

27. The High Performance Development Standard (HPDS) is a collection of voluntary and required standards that raise the performance of new building projects to achieve sustainable and resilient design. The HPDS was passed by Council on April 13, 2022.
 - a. At this time, the HPDS is not in effect and Council has referred the 2023 HPDS Update Report back to staff with direction to bring forward an updated report to Committee with recommendations for revised phasing timelines, resource requirements and associated amendments to the Site Plan Control By-law by no later than Q1 2024.



- b. Please refer to the HPDS information attached and ottawa.ca/HPDS for more information.

Submission Requirements and Fees

1. Outlines the application type/subtype required and the associated fees
 - a. Additional information regarding fees related to planning applications can be found [here](#).
2. The attached **Study and Plan Identification List** outlines the information and material that has been identified as either required (R) or advised (A) as part of a future complete application submission.
 - a. The required plans and studies must meet the City's Terms of Reference (ToR) and/or Guidelines, as available on Ottawa.ca. These ToR and Guidelines outline the specific requirements that must be met for each plan or study to be deemed adequate.
3. All of the above comments or issues should be addressed to ensure the effectiveness of the application submission review.

Should there be any questions, please do not hesitate to contact myself or the contact identified for the above areas / disciplines.

Yours Truly,
Steve Belan

cc.

Tim Chadder
Raad Akrawi
Madelen Fellows
Karla Ferrey
Tatyana Roumie
Christopher Moise
Adam Reed
Mike Giampa
Haley Murray
Jessica Button

12714001 Canada Inc – Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash

2983 Navan Road

SITE SERVICING REPORT CHECKLIST

REFERENCED STUDIES AND REPORTS	REFERENCE
Site Servicing Report for 12714001 Canada Inc, Block 16 – Gas Bar, Commercial Building/Drive-Through Restaurant and Car Wash, 2983 Navan Road Road (J.L. Richards & Associates Limited, December 8, 2023)	Site Servicing Report

4.1	GENERAL CONTENT	REFERENCE
<input type="checkbox"/>	Executive Summary (for larger reports only).	N/A
<input checked="" type="checkbox"/>	Date and revision number of the report.	Site Servicing Report
<input checked="" type="checkbox"/>	Location map and plan showing municipal address, boundary, and layout of proposed development.	Site Servicing Report (Appendix A) All Drawings
<input checked="" type="checkbox"/>	Plan showing the site and location of all existing services.	Servicing Plan
<input checked="" type="checkbox"/>	Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Site Servicing Report
<input checked="" type="checkbox"/>	Summary of Pre-consultation Meetings with City and other approval agencies.	Site Servicing Report (Appendix 'A')
<input checked="" type="checkbox"/>	Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defensible design criteria.	Reference made to Stantec 2005 EUC ISSU
<input checked="" type="checkbox"/>	Statement of objectives and servicing criteria.	Site Servicing Report
<input checked="" type="checkbox"/>	Identification of existing and proposed infrastructure available in the immediate area.	Site Servicing Report Servicing Plan
<input type="checkbox"/>	Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	N/A

<input checked="" type="checkbox"/>	Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighbouring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Grading Plan
<input type="checkbox"/>	Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A
<input type="checkbox"/>	Proposed phasing of the development, if applicable.	N/A
<input checked="" type="checkbox"/>	Reference to geotechnical studies and recommendations concerning servicing.	Site Servicing Report and Drawings
<input checked="" type="checkbox"/>	All preliminary and formal site plan submissions should have the following information: <ul style="list-style-type: none"> ▪ Metric scale ▪ North arrow (including construction North) ▪ Key plan ▪ Name and contact information of applicant and property owner ▪ Property limits, including bearings and dimensions ▪ Existing and proposed structures and parking areas ▪ Easements, road widening and rights-of-way ▪ Adjacent street names 	All Drawings

4.2	SITE SERVICING REPORT: WATER	REFERENCE
<input type="checkbox"/>	Confirm consistency with Master Servicing Study, if available.	N/A
<input checked="" type="checkbox"/>	Availability of public infrastructure to service proposed development.	Site Servicing Report (Section 2.0) Servicing Plan
<input checked="" type="checkbox"/>	Identification of system constraints.	Site Servicing Report (Section 2.0) Servicing Plan
<input checked="" type="checkbox"/>	Identify boundary conditions.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Confirmation of adequate domestic supply and pressure.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Site Servicing Report (Section 2.0)

<input type="checkbox"/>	Definition of phasing constraints. Hydraulic modelling is required to confirm servicing for all defined phases of the project, including the ultimate design.	N/A
<input checked="" type="checkbox"/>	Address reliability requirements, such as appropriate location of shutoff valves.	Site Servicing Report (Section 2.0)
<input type="checkbox"/>	Check on the necessity of a pressure zone boundary modification.	N/A
<input checked="" type="checkbox"/>	Reference to water supply analysis to show that major infrastructure can deliver sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants), including special metering provisions.	Site Servicing Report (Section 2.0) Servicing Plan
<input type="checkbox"/>	Description of off-site required feeder mains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A
<input checked="" type="checkbox"/>	Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Site Servicing Report (Section 2.0)
<input checked="" type="checkbox"/>	Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	Site Servicing Report (Section 2.0)

4.3	SITE SERVICING REPORT: WASTEWATER	REFERENCE
<input checked="" type="checkbox"/>	Summary of proposed design criteria (Note: Wet weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Site Servicing Report (Section 3.0,
<input type="checkbox"/>	Confirm consistency with Master Servicing Study and/or justifications for deviations.	Stantec 2005 EUC ISSU
<input type="checkbox"/>	Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the Guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A
<input checked="" type="checkbox"/>	Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Site Servicing Report (Section 3.0) Servicing Plan

<input checked="" type="checkbox"/>	Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable.)	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Calculations related to dry weather and wet weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Site Servicing Report (Section 3.0)
<input checked="" type="checkbox"/>	Description of proposed sewer network, including sewers, pumping stations and forcemains.	Site Servicing Report (Section 3.0) Servicing Plan
<input type="checkbox"/>	Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A
<input type="checkbox"/>	Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A
<input type="checkbox"/>	Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A
<input type="checkbox"/>	Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A
<input type="checkbox"/>	Special considerations, such as contamination, corrosive environment, etc.	N/A

4.4	SITE SERVICING REPORT: STORMWATER	REFERENCE
<input checked="" type="checkbox"/>	Description of drainage outlets and downstream constraints, including legality of outlets (i.e., municipal drain, right-of-way, watercourse, or private property).	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Analysis of available capacity in existing public infrastructure.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns, and proposed drainage pattern.	Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Water quantity control objective (e.g. controlling post-development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of the stormwater management concept with facility locations and descriptions with references and supporting information.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
<input type="checkbox"/>	Setback from private sewage disposal systems.	N/A
<input type="checkbox"/>	Watercourse and hazard lands setbacks.	N/A
<input checked="" type="checkbox"/>	Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Site Servicing Report (Appendix 'A')
<input type="checkbox"/>	Confirm consistency with subwatershed and Master Servicing Study, if applicable study exists.	Stantec 2005 EUC ISSU
<input checked="" type="checkbox"/>	Storage requirements (complete with calculations) and conveyance capacity for minor events (1:2 year return period) and major events (1:100 year return period).	Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of watercourses within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	N/A
<input checked="" type="checkbox"/>	Calculate pre- and post-development peak flow rates, including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Any proposed diversion of drainage catchment areas from one outlet to another.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Proposed minor and major systems, including locations and sizes of stormwater trunk sewers, and stormwater management facilities.	Servicing, Grading and Drainage Plans
<input type="checkbox"/>	If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post-development flows up to and including the 100-year return period storm event.	Quantity control proposed per Site Servicing Report (Section 4.0)
<input type="checkbox"/>	Identification of potential impacts to receiving watercourses.	N/A
<input type="checkbox"/>	Identification of municipal drains and related approval requirements.	N/A
<input checked="" type="checkbox"/>	Description of how the conveyance and storage capacity will be achieved for the development.	Site Servicing Report (Section 4.0)

<input checked="" type="checkbox"/>	100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Site Servicing Report (Section 4.0) Servicing, Grading and Drainage Plans
<input checked="" type="checkbox"/>	Inclusion of hydraulic analysis, including hydraulic grade line elevations.	Site Servicing Report (Section 4.0)
<input checked="" type="checkbox"/>	Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Site Servicing Report (Section 5.0) Servicing Plan
<input type="checkbox"/>	Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	N/A
<input type="checkbox"/>	Identification of fill constraints related to floodplain and geotechnical investigation.	N/A

4.5	APPROVAL AND PERMIT REQUIREMENTS	REFERENCE
The Site Servicing Report shall provide a list of applicable permits and regulatory approvals necessary for the proposed development, as well as the relevant issues affecting such approval. The approval and permitting shall include but not be limited to the following:		
<input type="checkbox"/>	Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams, as defined in the Act.	N/A
<input type="checkbox"/>	Application for Environmental Compliance Approval (ECA) under the Ontario Water Resources Act.	As part of future submission
<input type="checkbox"/>	Changes to Municipal Drains.	N/A
<input type="checkbox"/>	Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation, etc.).	N/A

4.6	CONCLUSION CHECKLIST	REFERENCE
<input checked="" type="checkbox"/>	Clearly stated conclusions and recommendations.	Site Servicing Report
<input checked="" type="checkbox"/>	Comments received from review agencies, including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	Not yet applicable

<input checked="" type="checkbox"/>	All draft and final reports shall be signed and stamped by a Professional Engineer registered in Ontario.	Site Servicing Report All Drawings

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix C

Water Servicing

WATERMAIN DEMAND CALCULATION SHEET

PROJECT : NAVAN ROAD DEVELOPMENT PROJECT - GAS BAR
LOCATION : CITY OF OTTAWA
DEVELOPER : 12714001 Canada Inc.

NODE	RESIDENTIAL			NON-RESIDENTIAL	AVERAGE DAILY DEMAND (l/s)				MAXIMUM DAILY DEMAND (l/s)				PEAK HOUR DEMAND (l/s)			
	UNITS		POP'N	COMM (ha.)	Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total	Res.	Non-res.	Plug flow	Total
	Townhouses (TH)	Condo Units (CU)														
J-14	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27
TOTALS	0	0	0	0.77	0.00	0.25	3.60	3.85	0.00	0.37	3.60	3.97	0.00	0.67	3.60	4.27

ASSUMPTIONS			
RESIDENTIAL DENSITIES			AVG. DAILY DEMAND
- Townhouse (TH)	2.7	p / p / u	- Residential 280 l / cap / day
			- Institutional 28,000 l / ha / day
- Condo Units (CU)	1.8	p / p / u	- Commercial 28,000 l / ha / day
			MAX. DAILY DEMAND
			- Residential 700 l / cap / day
			- Institutional 42,000 l / ha / day
			- Commercial 42,000 l / ha / day
			MAX. HOURLY DEMAND
			- Residential 1,540 l / cap / day
			- Institutional 75,600 l / ha / day
			- Commercial 75,600 l / ha / day

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	686 m ²	Commercial area consisting of a Gas Retail and Drive Thru
C	Height in storeys	1 storeys	Basements are excluded.
	Total Floor Area	686 m ²	
D	Fire Flow Formula	F=220C√A	
	Fire Flow	4610 L/min	
	Rounded Fire Flow	5000 L/min	Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	5000 L/min	No rounding applied.
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
G	<i>North Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	4 Storey Condo Unit
	Length of Exposed Wall:	32.2 m	
	Height of Exposed Wall:	4 storeys	
	Length-Height Factor	128.6 m-storeys	
	Separation Distance	38.96 m	
	North Side Exposure Charge	5%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	
East Side Exposure Charge	0%		
<i>South Side Exposure</i>			
Exposing Wall:	Non-combustible		
Exposed Wall:	Wood Frame		
Length of Exposed Wall:	0.0 m		
Height of Exposed Wall:	0 storeys		
Length-Height Factor	0.0 m-storeys		
Separation Distance	46 m	Over 45 m to next structure	
South Side Exposure Charge	0%		
<i>West Side Exposure</i>			
Exposing Wall:	Non-combustible	Gas Retail/Drive Thru	
Exposed Wall:	Wood Frame		
Length of Exposed Wall:	0.0 m		
Height of Exposed Wall:	0 storeys		
Length-Height Factor	0.0 m-storeys		
Separation Distance	46 m	Over 200 m to next structure	
West Side Exposure Charge	0%		
Total Exposure Charge	5%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	250 L/min		
H	Fire Flow	5250 L/min	
	Rounded Fire Flow	5000 L/min	Flow rounded to nearest 1000 L/min.
City Cap (RFF)	Required Fire Flow	5000 L/min	
		83 L/s	

Fire Underwriters Survey (FUS) Fire Flow Calculations
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

FUS Fire Flow Calculations

NAVAN ROAD DEVELOPMENT PROJECT - Commercial Building
(JLR 29899-002)

Step	Parameter	Value	Note
A	Type of Construction	Non-combustible	
	Coefficient (C)	0.8	
B	Ground Floor Area	107 m ²	Commercial Buildign - Car Wash
C	Height in storeys	1 storeys	Basements are excluded.
	Total Floor Area	107 m ²	
D	Fire Flow Formula	F=220C ^{1/2} A	
	Fire Flow	1821 L/min	
	Rounded Fire Flow	2000 L/min	Flow rounded to nearest 1000 L/min.
E	Occupancy Class	Combustible	
	Occupancy Charge	0%	
	Occupancy Increase or Decrease	0	
	Fire Flow	2000 L/min	No rounding applied.
F	Sprinkler Protection	None	
	Sprinkler Credit	0%	
	Decrease for Sprinkler	0 L/min	
G	<i>North Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	Townhomes
	Length of Exposed Wall:	14.4 m	
	Height of Exposed Wall:	2 storeys	
	Length-Height Factor	28.9 m-storeys	
	Separation Distance	18.81 m	
	North Side Exposure Charge	12%	
	<i>East Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Wood Frame	
	Length of Exposed Wall:	0.0 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	46 m	
	East Side Exposure Charge	0%	
	<i>South Side Exposure</i>		
	Exposing Wall:	Non-combustible	Car Wash
	Exposed Wall:	Non-combustible	Gas Retail/Drive Thru
	Length of Exposed Wall:	5.4 m	
	Height of Exposed Wall:	0 storeys	
	Length-Height Factor	0.0 m-storeys	
	Separation Distance	32.22 m	Over 45 m to next structure
	South Side Exposure Charge	5%	
	<i>West Side Exposure</i>		
	Exposing Wall:	Non-combustible	Gas Retail/Drive Thru
	Exposed Wall:	Wood Frame	
Length of Exposed Wall:	0.0 m		
Height of Exposed Wall:	0 storeys		
Length-Height Factor	0.0 m-storeys		
Separation Distance	46 m	Over 200 m to next structure	
West Side Exposure Charge	0%		
Total Exposure Charge	17%	The total exposure charge is below the maximum value of 75%.	
Increase for Exposures	340 L/min		
H	Fire Flow	2340 L/min	
	Rounded Fire Flow	2000 L/min	Flow rounded to nearest 1000 L/min.
City Cap (RFF)	Required Fire Flow (RFF)	2000 L/min	
		33 L/s	

Fire Underwriters Survey (FUS) Fire Flow Calculations
In accordance with City of Ottawa Technical Bulletin ISTB-2018-02 dated March 21, 2018

William Rugamba

From: William Rugamba
Sent: July 15, 2024 4:00 PM
To: William Rugamba
Subject: FW: Navan Subdivision - Boundary Condition Request
Attachments: NavanSubdivision_Boundary Condition(4july2024).docx

William Rugamba, M.Eng., B.A.Sc., EIT
Civil Engineering Graduate
Ottawa, ON
Work: [343-804-4374](tel:343-804-4374)

From: Polyak, Alex <alex.polyak@ottawa.ca>
Sent: Monday, July 15, 2024 10:12 AM
To: Mahad Musse <mmusse@jlrichards.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Mahad,

Please find the boundary conditions attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., C.E.T., P.Eng. 

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Development and Building Services Department (PDBS) | Direction générale des services de la planification, de l'aménagement et du bâtiment (DGSPAB)

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: Mahad Musse <mmusse@jlrichards.ca>

Sent: July 12, 2024 1:31 PM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

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Hi Alex,

Just wondering if you have a status update for the boundary conditions for Navan.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca

From: Mahad Musse <mmusse@jlrichards.ca>

Sent: Wednesday, July 3, 2024 11:02 AM

To: Polyak, Alex <alex.polyak@ottawa.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Armstrong, Justin <justin.armstrong@ottawa.ca>; Tatyana Roumie <troumie@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

Good morning Alex,

As we discussed last week our Client is looking into the option of converting the row townhouse units into duplex units (townhouse units with apartments in the basement). As a result, this will increase the total demand on the site and we will therefore require new water boundary conditions. We'd like to note that the footprint of the blocks will not change and neither will their layout or any of the offsets.

As a summary:

- Domestic demands were calculated based on a daily consumption rate of 280 L/cap/day with peaking factors consistent with City of Ottawa Guidelines
- Required Fire Flow (RFF) was calculated in accordance to the Fire Underwriters Survey (FUS) Water Supply for Public Fire Protection and the City of Ottawa FUS protocol (Bulletin ISDTB-2014-02 & Bulletin ISDTB-2018-02), which considers material, expose distance & height. We have attached the calculation spreadsheet and the figure.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.74 L/s

Maximum Day Demand: 10.53 L/s

Peak Hour Demand: 18.17 L/s

Required Fire Flow (per FUS): 6,000 L/min (100 L/s)

Required Fire Flow (per FUS): 10,000 L/min (167 L/s)

Required Fire Flow (per FUS): 14,000 L/min (233 L/s)

Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

For your reference, the previous boundary condition received from the City is attached and below is the email chain.

If you have any questions or comments please let us know.

Thanks
Mahad



Mahad Musse, B.Eng., EIT
Civil Engineering Graduate

1000-343 Preston Street
Ottawa, ON, K1S 1N4

Work: [343-633-1501](tel:343-633-1501)
mmusse@jlrichards.ca

From: Polyak, Alex <alex.polyak@ottawa.ca>

Sent: Thursday, August 17, 2023 3:01 PM

To: William Rugamba <wrugamba@jlrichards.ca>

Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>

Subject: RE: Navan Subdivision - Boundary Condition Request

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Hello William,

Sorry that I missed your call, I was in a meeting. The boundary conditions are attached.

Regards,

Oleksandr (Alex) Polyak, B.Eng., P.Eng

Project Manager, Infrastructure Approvals, Development Review East Branch | Gestionnaire de projet, Direction de l'examen des projets d'aménagement – Est.
Planning, Real Estate and Economic Development Department | Direction générale de la planification, des biens immobiliers et du développement économique

City of Ottawa | Ville d'Ottawa
110 Laurier Ave., 4th Fl East, Ottawa ON K1P 1J1
Email: alex.polyak@ottawa.ca
Cell : 613-857-4380
www.Ottawa.ca



From: William Rugamba <wrugamba@jlrichards.ca>
Sent: August 15, 2023 9:26 AM
To: Polyak, Alex <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; Carmine Zayoun <carmine@zayoungroup.com>; Shahira Jalal <sjalal@jlrichards.ca>
Subject: RE: Navan Subdivision - Boundary Condition Request

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Good morning Alex,

Just wanted to follow up on the status of this boundary request. Please let me know if you need anything else from us.

Thanks,
William

William Rugamba, M.Eng.
Civil Engineering Intern

J.L. Richards & Associates Limited
1000-343 Preston Street, Ottawa, ON K1S 1N4
Direct: 343-804-4374



From: Tatyana Roumie
Sent: Tuesday, July 25, 2023 3:53 PM
To: 'alex.polyak@ottawa.ca' <alex.polyak@ottawa.ca>
Cc: Karla Ferrey <kferrey@jlrichards.ca>; Raad Akrawi <rakrawi@groupeheafey.com>; carmine@zayoungroup.com; Shahira Jalal <sjalal@jlrichards.ca>
Subject: Navan Subdivision - Boundary Condition Request

Hello Alex.

To support our upcoming detailed design for the site, we are requesting updated boundary conditions for the 3079 Navan Road Development.

As a brief history, we received boundary conditions from the City in July 2021 (attached, but with incorrect connection locations) and again in April 2022 (also attached) in support of the functional servicing design. We understand from the April 2022 boundary conditions that the maximum available fire flow for the site is 250 L/s.

We are currently requesting updated boundary conditions for this site as we are commencing the detailed servicing design and this request will accommodate the recent site plan changes and proposed connection points. This request is also applicable to the upcoming site plan designs which will be submitted as separate applications.

We request boundary conditions under high pressure, peak hour, and maximum day + fire flow conditions (for each of the below fire flows). Domestic demand and fire flow calculations are attached. Please provide the boundary conditions at the proposed connection locations as shown in the attached figure.

Average Day Demand: 6.44 L/s
Maximum Day Demand: 9.77 L/s
Peak Hour Demand: 16.50 L/s
Required Fire Flow (per FUS): 6,000 L/min (100 L/s)
Required Fire Flow (per FUS): 10,000 L/min (167 L/s)
Required Fire Flow (per FUS): 14,000 L/min (233 L/s)
Required Fire Flow (per FUS): 15,000 L/min (250 L/s)

Thanks,
Tatyana

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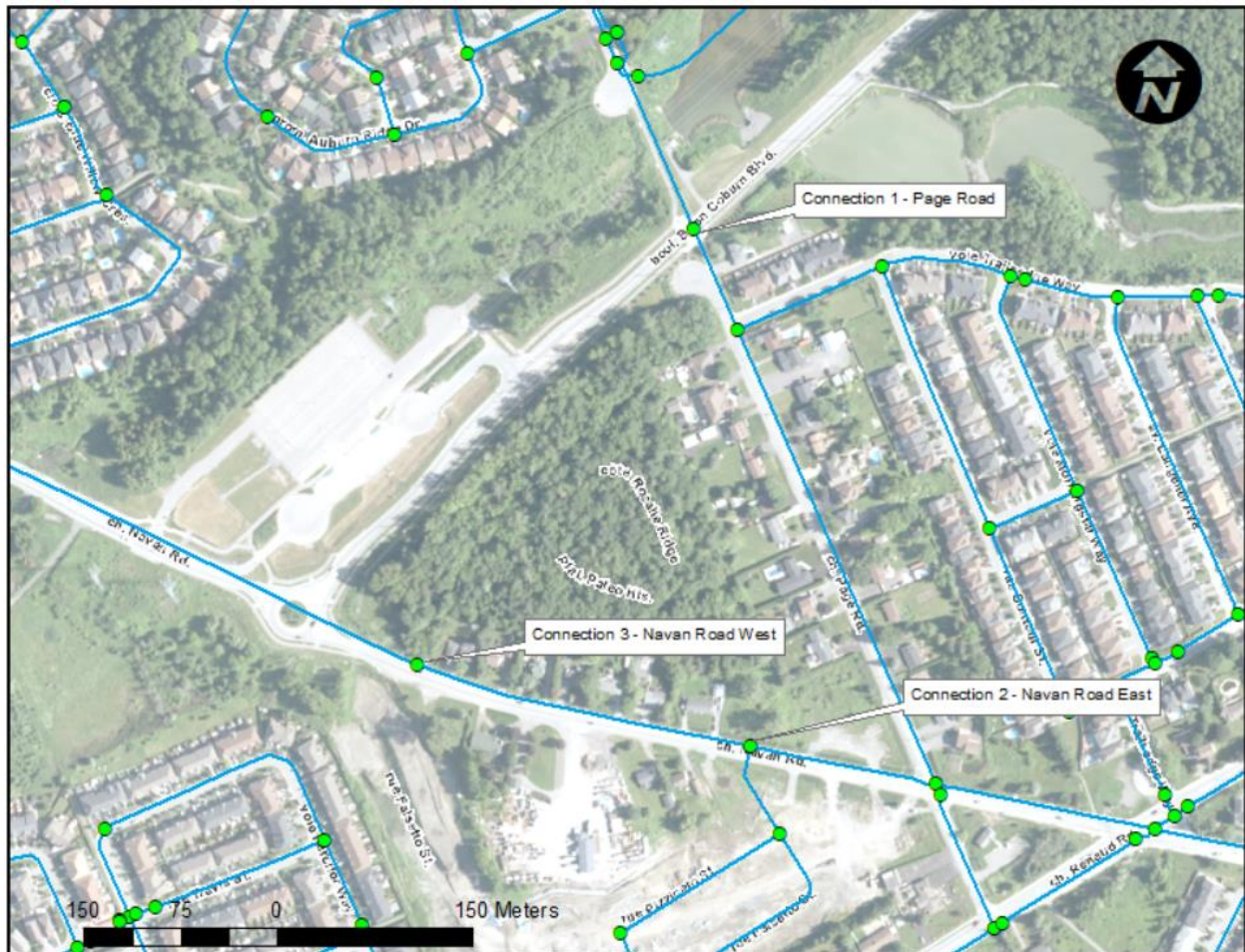
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Boundary Conditions Navan Subdivision

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	404	6.74
Maximum Daily Demand	632	10.53
Peak Hour	1,090	18.17
Fire Flow Demand #1	6,000	100.00
Fire Flow Demand #2	10,000	166.67
Fire Flow Demand #3	14,000	233.33
Fire Flow Demand #4	15,000	250.00

Location



Results

Connection 1 - Page Road

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	64.0
Peak Hour	127.0	58.6
Max Day plus Fire Flow #1	128.2	60.4
Max Day plus Fire Flow #2	126.8	58.3
Max Day plus Fire Flow #3	124.9	55.7
Max Day plus Fire Flow #4	124.4	55.0

¹ Ground Elevation = 85.7 m

Connection 2 - Navan Road East

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	71.4
Peak Hour	126.8	65.9
Max Day plus Fire Flow #1	127.7	67.1
Max Day plus Fire Flow #2	125.5	64.1
Max Day plus Fire Flow #3	122.7	60.1
Max Day plus Fire Flow #4	121.9	58.9

¹ Ground Elevation = 80.5 m

Connection 3 - Navan Road West

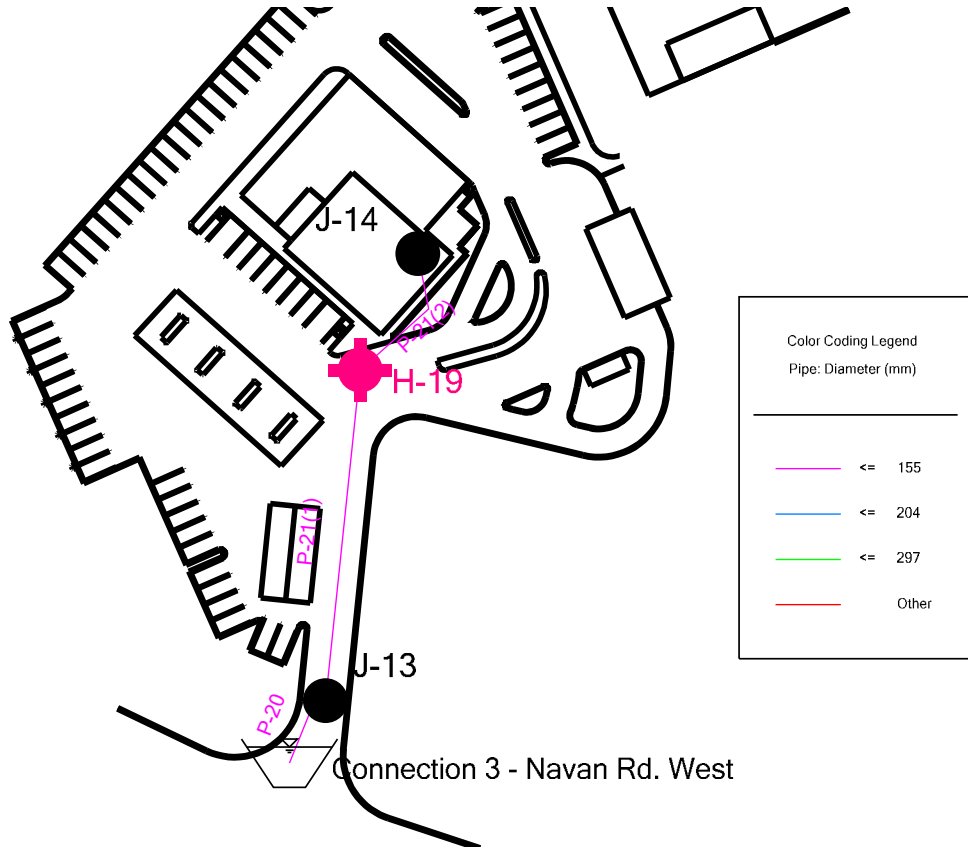
Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	130.7	69.3
Peak Hour	126.8	63.8
Max Day plus Fire Flow #1	127.3	64.5
Max Day plus Fire Flow #2	124.6	60.6
Max Day plus Fire Flow #3	120.9	55.3
Max Day plus Fire Flow #4	119.8	53.8

¹ Ground Elevation = 81.9 m

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

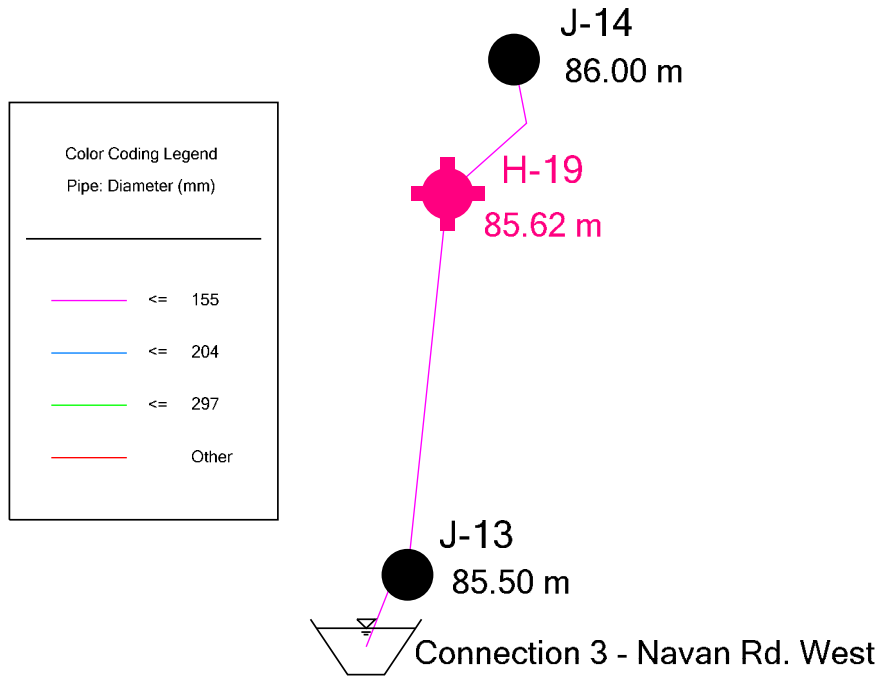
Gas Bar, Commercial Building, and Car Wash (Block 16) Model Schematic



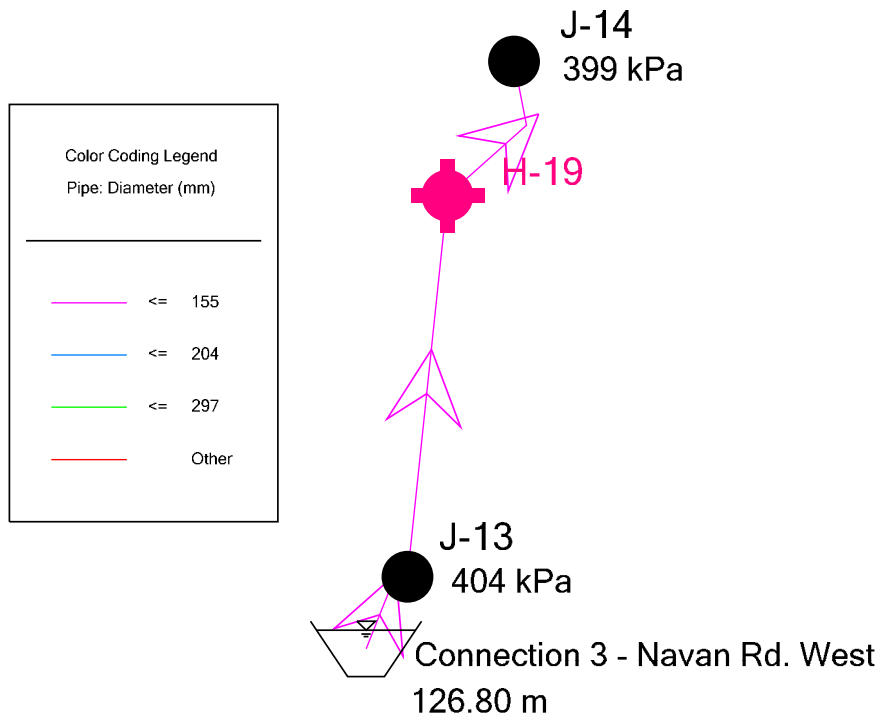
Gas Bar, Commercial Building, and Car Wash (Block 16)

Model Schematic

Elevation Model



Gas Bar, Commercial Building, and Car Wash (Block 16) Peak Hour Demand



Gas Bar, Commercial Building, and Car Wash (Block 16)

Peak Hour Demand

Junction Table

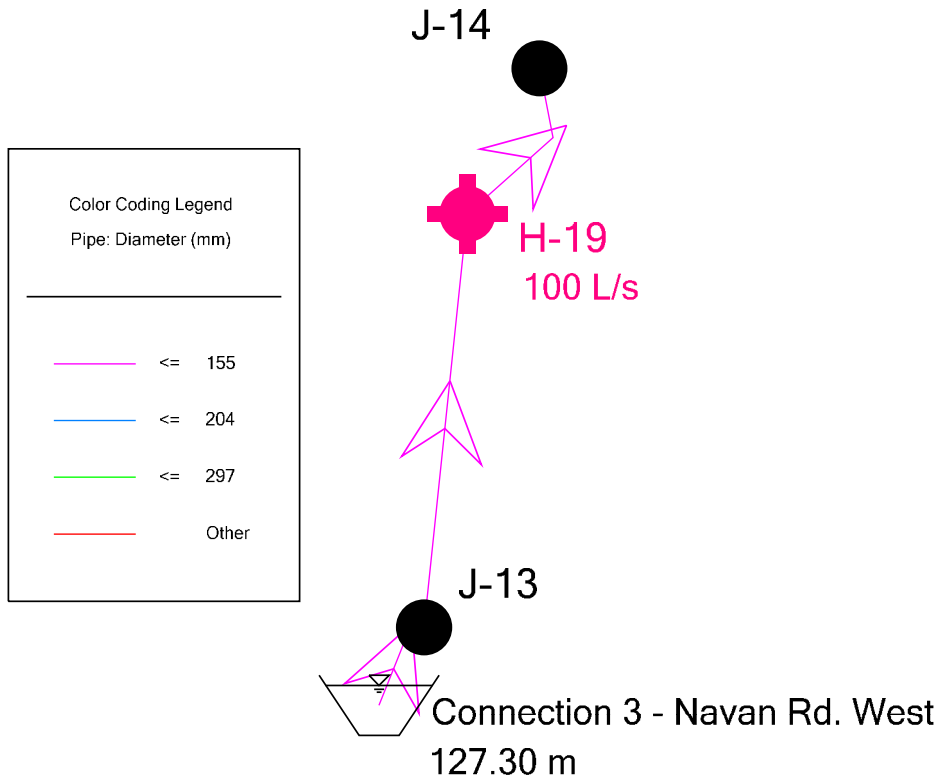
Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	4.27	126.74	399
J-13	85.50	0.00	126.79	404

Gas Bar, Commercial Building, and Car Wash (Block 16)
Peak Hour Demand
Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	4.27	0.23
175	P-21(1)	49	155	PVC	100.0	4.27	0.23
176	P-21(2)	22	155	PVC	100.0	4.27	0.23

Gas Bar, Commercial Building, and Car Wash (Block 16)

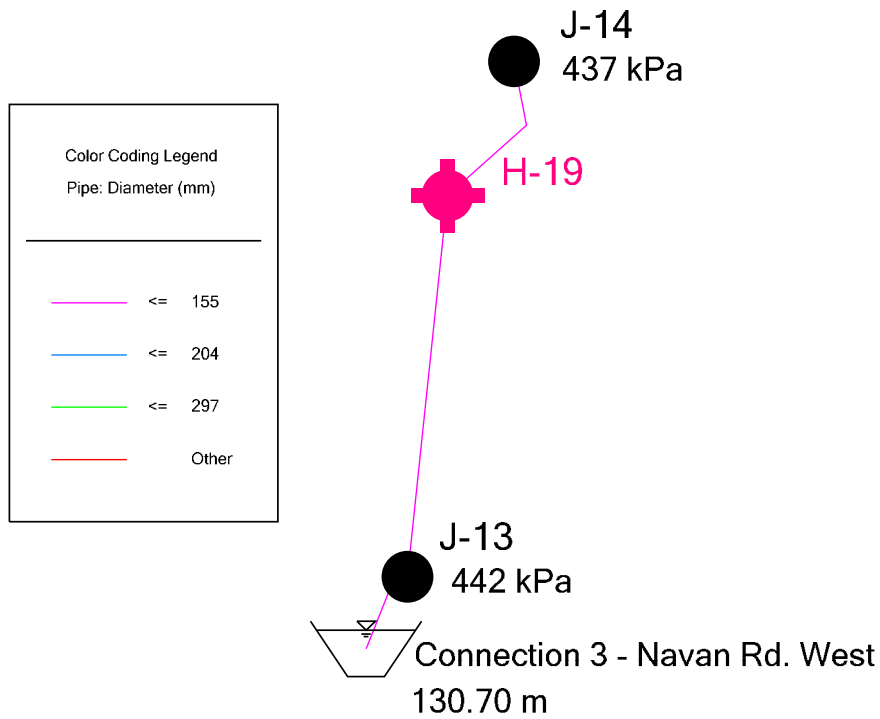
Max Day + Fire Flow Requirement



Gas Bar, Commercial Building, and Car Wash (Block 16)
Max Day + Fire Flow Requirement

Label	Satisfies Fire Flow Constraints?	Fire Flow (Available) (L/s)	Flow (Total Available) (L/s)	Pressure (Residual Lower Limit) (kPa)	Pressure (Calculated Residual) (kPa)	Pressure (Calculated System Lower Limit) (kPa)	Junction w/ Minimum Pressure (System)
H-19	True	100	100	140	224	238	J-14

Gas Bar, Commercial Building, and Car Wash (Block 16) Maximum Pressure Analysis



Gas Bar, Commercial Building, and Car Wash (Block 16)

Maximum Pressure Analysis

Junction Table

Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (kPa)
J-14	86.00	0	130.70	437
J-13	85.50	0	130.70	442

Gas Bar, Commercial Building, and Car Wash (Block 16)

Maximum Pressure Analysis

Pipe Table

ID	Label	Length (Scaled) (m)	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
61	P-20	11	155	PVC	100.0	0	0.00
175	P-21(1)	49	155	PVC	100.0	0	0.00
176	P-21(2)	22	155	PVC	100.0	0	0.00

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix D

Sanitary Design Sheet

Street Name	MH No.		Residential							Commercial/Institutional				Infiltration			Peak Design Flow L/s	Pipe Data							Upstream Geometry				Downstream Geometry								
	From	To	Multiples	Apartments	Area (ha)	Pop.	Cum. Pop.	Cum. Area (ha)	Peaking Factor	Residential Flow (L/s)	Area (ha)	Cum. Area (ha)	Peaking Factor	Inst. Flow (L/s)	Plug Flow (L/s)	Area (ha)		Cum. Area (ha)	Peak Extr. Flow L/s	Dia	Type	Actual Diameter	Slope	Q Full (L/s)	V Full	Length	Residual Capacity	% Full	TG From	Obvert	Invert	Cover	TG TO	Drop	Obvert	Invert	Cover
GAS STATION	GAS BAR	22			0.00	0	0	0.00	3.80	0.00	0.77	0.77	1.50	0.37		0.77	0.77	0.25	0.63	200	Circular	203.20	1.00%	34.22	1.06	27.26	33.59	2%	86.000	83.369	83.165	2.631	85.590	0.510	83.096	82.893	2.494
GAS STATION	CAR WASH	22			0.00	0	0	0.00	3.80	0.00		0.00	1.50	0.00	3.60	0.00	0.00	3.60	200	Circular	203.20	1.00%	34.22	1.06	5.91	30.62	11%	85.770	82.705	82.502	3.065	85.590	0.060	82.646	82.443	2.944	
GAS STATION TO EXISTING STUB	22	STUB 16			0.00	0	0	0.00	3.80	0.00		0.77	1.50	0.37	3.60	0.00	0.77	0.25	4.23	200	Circular	203.20	0.65%	27.59	0.85	2.02	23.36	15%	85.590	82.586	82.383	3.004	85.350		82.573	82.370	2.777
EXISTING STUB TO PALEO DRIVE	STUB 16	21	0	0	0.04	0	0	0.04	3.80	0.00		0.77	1.50	0.37	3.60	0.04	0.81	0.27	4.24	200	Circular	203.20	0.65%	27.59	0.85	38.75	23.34	15%	85.350	82.573	82.370	2.777	85.041		82.321	82.118	2.720

Design Parameters	
Single Family Population	3.4 Cap/Unit
Semi-Detached/Townhouse Population	2.7 Cap/Unit
Apartments Population	1.8 Cap/Unit
Residential Flows	280 L/Cap/Day
Infiltration Flows	0.33 L/s/ha
Correction Factor	0.8
Commercial Peak Factor	1.5
Institutional/Commercial Average Flow	28000 L/gross ha/d
Manning Coefficient	0.013



Friday, August 16, 2024

J. L. Richards
1000-343 Preston St.
Ottawa, Ontario, K1S 1N4

Attn: W. Rugamba

Re: 2983 Navan Road, Ontario - Gas Bar-Commercial-QM&E Project # 23-047
Confirmations

Dear William,

This letter serves as confirmation of items you sent by e-mail August 12, 2024.

1. Maximum domestic water flow rate is 57 gpm (3.60 L/s).
2. That quantity was arrived at by referring to data provided by the client's car wash specialist.
3. The design of the oil interceptor has been completed, it will be located entirely within the walls of the car wash building.

Please do not hesitate to contact us should you have any questions or comments.

Yours truly,

A handwritten signature in black ink, appearing to read 'C. W. Clark', with a stylized flourish at the end.

C. W. Clark, P.Eng
QM&E Engineering
CC: by you to whom it may concern

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix E

Storm Design Sheet

PIPE REACH			Peak Flow Estimation										Sewer Data								Upstream Geometry					Downstream Geometry					Self Cleansing Velocities						
LOCATION	From MH	To MH	C-Factor (1:2)		Total Area (ha)	Cum. Total Area (ha)	Inlet Time (min.)	In Pipe Flow Time (min)	Total Time	1:2 Year Storm (RATIONAL METHOD)				Plug Flows Orifice Flow ⁽¹⁾ MH518 (L/s)	Total Peak Flow ⁽⁵⁾ (L/s)	Type	Nominal Dia. (mm)	Actual Dia. (mm)	Slope	Length (m)	Q Full (L/s)	V Full (m/s)	Residual Capacity ⁽⁶⁾ (L/s)	% Full	TG From	Obvert	Invert	Cover	TG To	Drop	Obvert	Invert	Cover	Q/Qf Ratio	Flow Depth (mm)	Actual Velocity ⁽⁷⁾ (m/s)	Flow Depth to Dia. Ratio (d/D)
			0.20	0.90						2.78AR	Cum. 2.78AR	1:2 Year Intensity (mm/hr)	1:2 Year Peak Flow (L/s)																								
GAS STATION	MH520	MH519	0.055	0.265	0.32	0.32	10.00	0.75	10.75	0.69	0.69	76.81	53.33	53.33	CONCRETE	625	533.40	0.40%	57.15	284.46	1.27	231.14	19%	85.374	83.320	82.787	2.05	85.709	0.060	83.090	82.557	2.62	0.19	156.29	0.98	0.29	
GAS STATION	MH519	MH518			0.00	0.32	10.75	0.38	11.12	0.00	0.69	74.05	51.41	51.41	CONCRETE	625	533.40	0.50%	31.97	317.25	1.42	265.64	16%	85.709	83.030	82.497	2.68	85.468	1.052	82.871	82.337	2.69	0.16	145.08	1.04	0.27	
GAS STATION	MH522	MH521	0.066	0.168	0.23	0.23	10.00	0.53	10.53	0.46	0.46	76.81	35.15	35.15	CONCRETE	525	533.40	0.50%	45.09	317.25	1.42	282.10	11%	85.505	83.016	82.482	2.49	85.630		82.790	82.257	2.84	0.11	119.48	0.93	0.22	
GAS STATION	MH521	MH518	0.030	0.149	0.18	0.41	10.53	0.38	10.91	0.39	0.85	74.83	63.33	63.33	CONCRETE	625	533.40	0.35%	27.21	265.81	1.19	202.48	24%	85.630	82.790	82.257	2.84	85.468	0.876	82.695	82.161	2.77	0.24	177.09	0.98	0.33	
GAS STATION	MH518	MH518A	0.021	0.016	0.04	0.77	11.12	0.29	11.41	0.05	1.59	72.74	115.83	66.14	66.14	CONCRETE	450	457.20	0.25%	15.62	148.72	0.91	32.89	78%	85.468	81.819	81.361	3.65	85.591	0.300	81.780	81.322	3.81	0.44	213.51	0.88	0.47
EAST ORLEANS RIDGE SUBDIVISION	EXST MH518A	EXST MH514 ⁽⁸⁾	Refer to Note 8		1.07		11.41	0.83	12.24	2.29	2.29	71.78	164.59	Refer to Note 5	CONCRETE	625	533.40	0.25%	50.21	224.33	1.00	99.74	73%	85.400	81.480	80.946	3.92	84.620		81.354	80.821	3.27				Refer to Note 8	

Design Parameters (Per OSDG)	
Manning's Coefficient =	0.013
1:2 Year Intensity =	732.951 / (Tc + 6.199) ^{0.810}
Note: Tc is the time of concentration in minutes	

Drainage Areas Breakdown	
Total Site Area:	0.77 ha
Controlled Area Within Site Property Line:	0.77 ha -->
Existing Navan Rd Rear-Yard Area Captured Within Site:	0.05 ha
Block 15 Rear-Yard area captured in 518A-514:	0.07 ha
Subdivision Block 11 Rear-Yard Area in 518A-514:	0.14
Existing Navan Rd Rear-Yard Area Captured in 518A-514:	0.04
Total Captured Areas:	1.07
Uncontrolled Area - Outlet to Subdivision:	0.00 ha -->

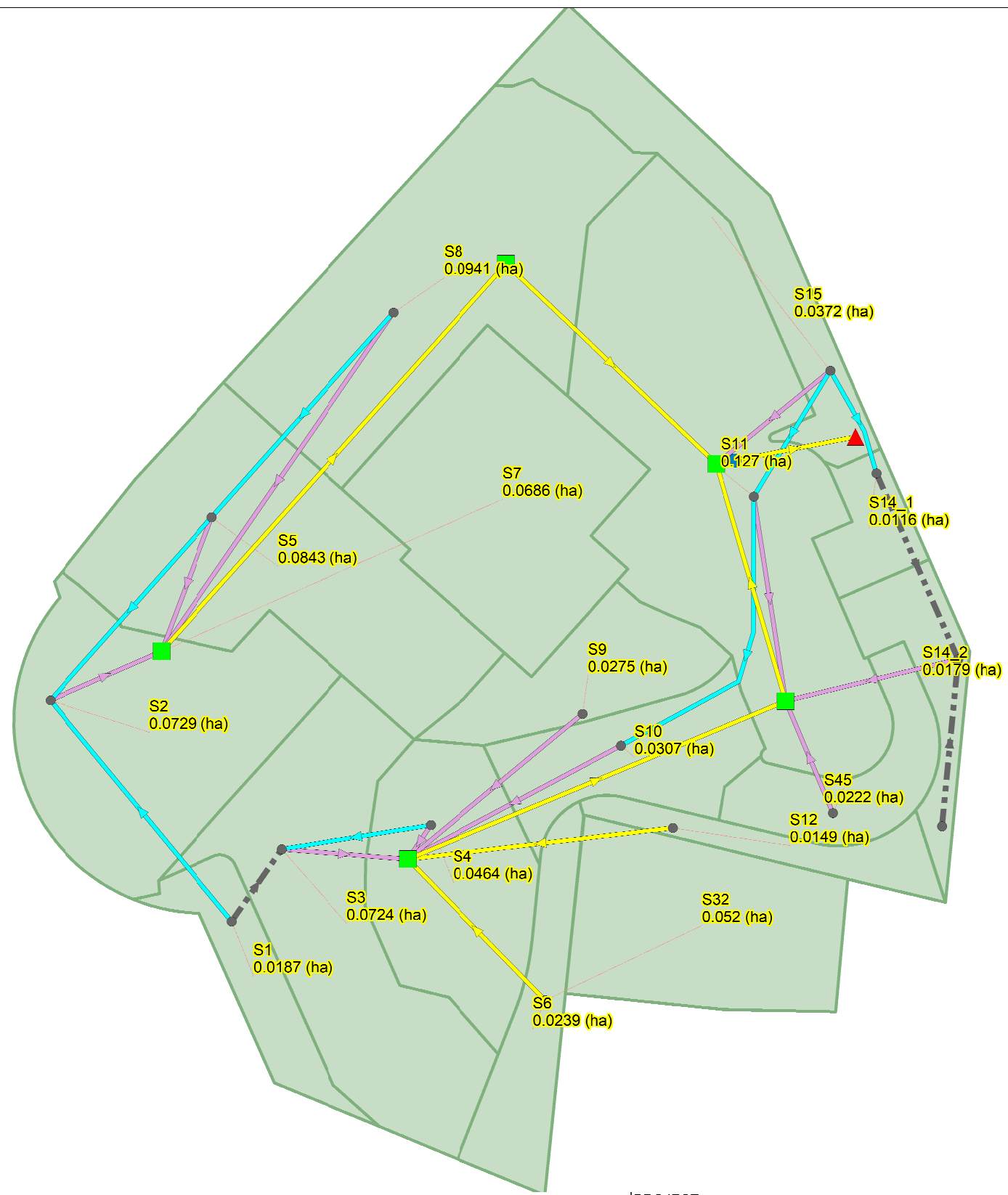
Notes on Plug Flows	
(1) Orifice Flow Rate from Block 16	

Notes on Peak Flow and Pipe Sizing	
(5) Peak flows are lower than the allowable release rate for Block 16 (68 L/s)	
(6) Pipes are conservatively sized for 1:2 Year Peak Flow Rate	
(7) Actual Velocities based on actual peak flows from Note 5	
(8) Details from Existing Sewers Downstream of EXST MH518A (referred to as STUB 16) can be found within East Orleans Ridge Subdivision Design Sheet. This line is only used to carry over the downstream values for time of concentration and 1:2 year peak flow	

Site Servicing Report
2983 Navan Road – Block 16
Gas Station, Commercial Building, Drive-Thru Restaurant & Car Wash

Appendix F

Stormwater Management



Legend

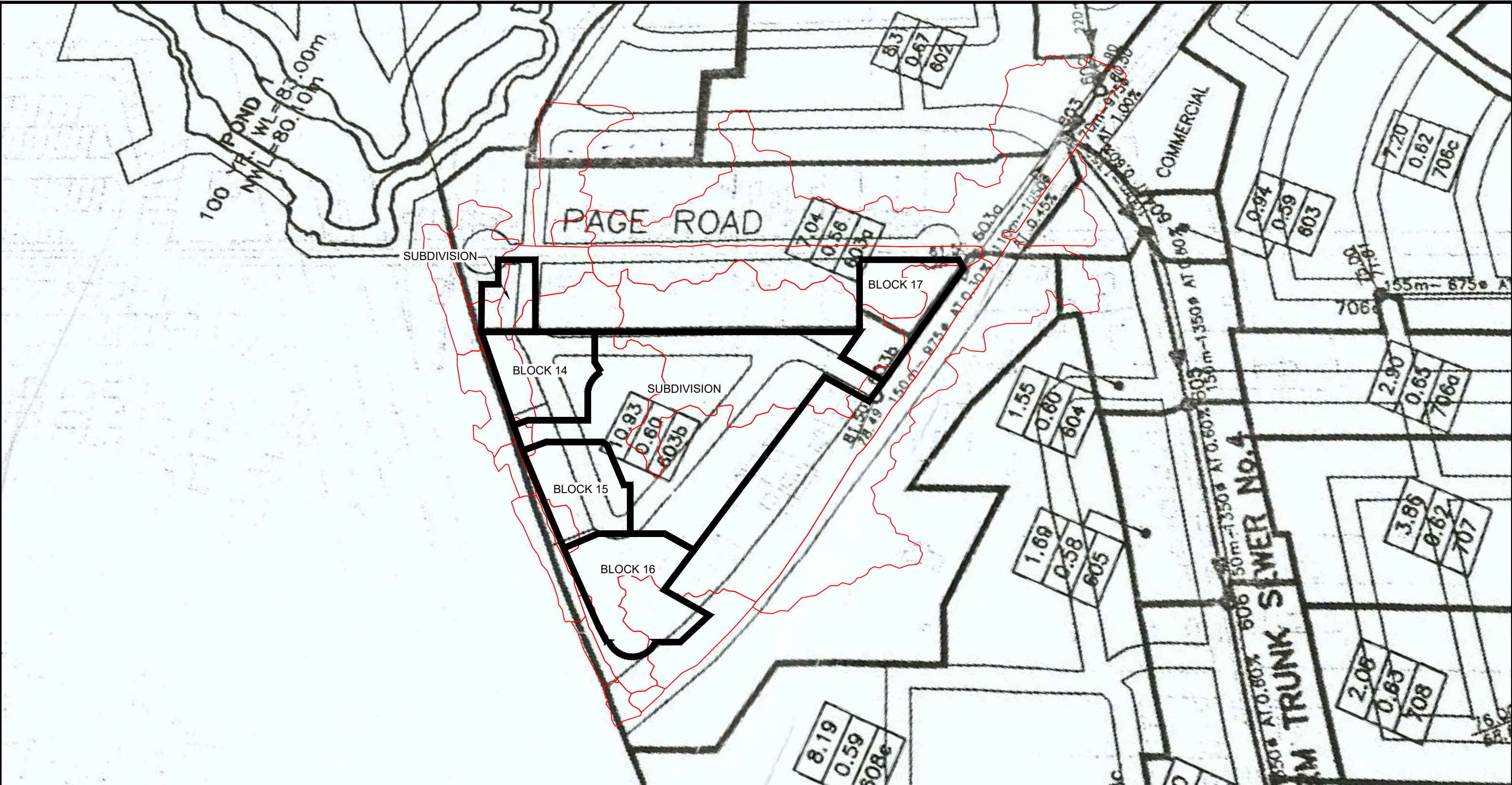
● Junctions	Conduits
▲ Outfalls	— Storm Sewers
■ Manholes	--- Storm Sewers
● Manholes	— Weirs
	— Outlets
	■ Subcatchments



150 m

PROJECT:	2983 Navan Road - Block 16 Ottawa, ON		
DRAWING:	Overall System Model Schematic		
	DESIGN:	ML	JLR NO.: 29899-002
	DRAWN:	ML	DRAWING NO.: Figure 3
	CHECKED:	BP	
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File Location: P:\29000\29899-002 - Navan Subdivision\05-Production\01-Civil\29899-002 C PRE-DST.dwg



LEGEND:

- PRE-DEVELOPMENT DRAINAGE AREAS
- NAVAN SITE PLAN AND SUBDIVISION BOUNDARY

NOTE:
 UNDERLYING CATCHMENT DELINEATION FROM GLOUCESTER EUC INFRASTRUCTURE SERVICING UPDATE 2005 WHICH INFORMED THE ALLOWABLE RELEASE RATES FROM THE SITE AND NO PRE-DEVELOPMENT MODELLING WAS REQUIRED

PROJECT:	2983, 3053 AND 3079 NAVAN RD & 2690 PAGE RD		
DRAWING:	PRE-DEVELOPMENT DRAINAGE PLAN		
 <small>www.jrichards.ca</small> J.L. Richards <small>ENGINEERS-ARCHITECTS-PLANNERS</small>	<small>DESIGN:</small> BP	DRAWING #: FIGURE	
	<small>DRAWN:</small> KC		
<small>CHECKED:</small> BP			
<small>JLR #:</small> 29899-002			
<small>This drawing is copyright protected and may not be reproduced or used for purposes other than execution of the described work without the express written consent of J.L. Richards & Associates Limited.</small>			

PLOT DATE: July 26, 2024 2:27:18 PM

Post-Development 3-hour Chicago 1:2 year Event

```

[ TITLE
;;Project Title/Notes

[ OPTIONS
;;Option Value
FLOW_UNITS LFS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 01/01/2000
START_TIME 00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE 01/01/2000
END_TIME 06:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 1
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 8
MAX_TRIALS 0
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 12

[ EVAPORATION
;;Data Source Parameters
CONSTANT 0.0
DRY_ONLY NO

[ RAINGAGES
;;Name Format Interval SCF Source
3CHI002 INTENSITY 0:10 1.0 TIMESERIES 3CHI002
3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100

[ SUBCATCHMENTS
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen
;;Subcatchment
S10 3CHI002 CB208 0.0187 0 23.61 2.2 0
S11 3CHI002 CB204 0.0307 99.617 55.32 2.2 0
S12 3CHI002 CB202 0.127 90.325 65.66 2.2 0
S14_1 3CHI002 CB213 0.0149 0.369 6.44 2.2 0
S14_2 3CHI002 CB211 0.0116 52.676 22.78 2.2 0
S15 3CHI002 CB212 0.0179 31.654 58.12 2.2 0
S2 3CHI002 CB210B 0.0372 42.751 108.74 2.2 0
S3 3CHI002 CB209 0.0729 62.937 33.61 2.2 0
S32 3CHI002 CB207 0.0724 96.692 45.79 2.2 0
S4 3CHI002 CB214 0.052 14.286 29.7 2.2 0
S45 3CHI002 CB206 0.0464 99.975 24.67 2.2 0
S5 3CHI002 CB203 0.0222 99.962 14.32 2.2 0
S6 3CHI002 CB200 0.0843 89.589 82.96 2.2 0
S7 3CHI002 520_(P-Stm) 0.0239 0 40.3 2.2 0
S8 3CHI002 CB201 0.0941 80.117 35.06 2.2 0
S9 3CHI002 CB205 0.0275 76.83 19.08 2.2 0

[ SUBAREAS
;;Subcatchment N-Imperv N-Periv S-Imperv S-Periv PctZero RouteTo PctRouted
S10 0.013 0.25 1.57 4.67 0 OUTLET 0
S11 0.013 0.25 1.57 4.67 0 OUTLET 0
S12 0.013 0.25 1.57 4.67 0 OUTLET 100
S14_1 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S14_2 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S15 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S2 0.013 0.25 1.57 4.67 0 OUTLET 0
S3 0.013 0.25 1.57 4.67 0 OUTLET 80
S4 0.013 0.25 1.57 4.67 0 OUTLET 0
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S6 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S7 0.013 0.25 1.57 4.67 0 OUTLET 0
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S9 0.013 0.25 1.57 4.67 0 OUTLET 0

[ INFILTRATION
;;Subcatchment Param1 Param2 Param3 Param4 Param5
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S12 76.2 13.2 4.14 7 0
S14_1 76.2 13.2 4.14 7 0
S14_2 76.2 13.2 4.14 7 0
S15 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S32 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S45 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[ JUNCTIONS
;;Name Elevation MaxDepth InitDepth SurDepth Aponded
518_Orifice 81.361 4.106 0 0 0

[ OUTFALLS
;;Name Elevation Type Stage Data Gated Route To
;Cylindrical Structure Slab Top Circular Frame SI
518A_(P-Stm) 80.646 FIXED 81.2 NO

[ STORAGE
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params
SurDepth Evap Psi Ksat IMD
;Cylindrical Structure Slab Top Circular Frame SI
518_(P-Stm) 81.061 4.406 0.139 FUNCTIONAL 0 0 1.13 0
;Cylindrical Structure Slab Top Circular Frame SI
519_(P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0
;Cylindrical Structure Slab Top Circular Frame SI
520_(P-Stm) 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0
;Cylindrical Structure Slab Top Circular Frame SI

[ CONDUITS
;;Name From Node To Node Length Roughness InOffset OutOffset
InitFlow MaxFlow
;Cylindrical Structure Slab Top Circular Frame SI
522_(P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0
;85.1800000001676
CB200 83.18 2.32 0 TABULAR CB200 0
;85.5000000001435
CB201 83.09 2.66 0 TABULAR CB201 0
;85.36
CB202 82.76 2.89 0 TABULAR CB202 0
;85.3500000000931
CB203 82.75 2.9 0 TABULAR CB203 0
;85.35000000022731
CB204 82.75 2.75 0 TABULAR CB204 0
;85.45000000023662
CB205 82.85 2.75 0 TABULAR CB205 0
;85.48
CB206 82.77 2.73 0 TABULAR CB206 0
;85.05
CB207 83.15 2.3 0 TABULAR CB207 0
;84.90000000021652
CB208 83.3 2.04 0 TABULAR CB208 0
;84.8
CB209 83.2 1.9 0 TABULAR CB209 0
;85.25
CB210B 82.65 3 0 TABULAR CB210B 0
;85.35
CB211 83.7 1.9 0 TABULAR CB211 0
;85.4
CB212 83.2 2.42 0 TABULAR CB212 0
;85.5000000000554
CB212A 83.62 2 0 TABULAR CB212A 0
;84.75
CB213 83.15 1.75 0 TABULAR CB213 0
;84.55
CB214 82.95 1.9 0 TABULAR CB214 0

[ PVC Pipes
Pipe_-(111)_(P-Stm) 522_(P-Stm) 521_(P-Stm) 45.094 0.013 82.482 82.257
Pipe_-(104)_(P-Stm)_2 518_Orifice 518A_(P-Stm) 15.623 0.013 81.361 81.322
Pipe_-(71)_(P-Stm) 520_(P-Stm) 519_(P-Stm) 57.146 0.013 82.787 82.557
Pipe_-(72)_(P-Stm) 519_(P-Stm) 518_(P-Stm) 31.967 0.013 82.497 82.337

[ ORIFICES
;;Name From Node To Node Type Offset Qcoeff Gated
CloseTime
OR1 518_(P-Stm) 518_Orifice SIDE 81.361 0.65 NO

[ WEIRS
;;Name From Node To Node Type CrestHt Qcoeff Gated
EndCoeff SurchARGE RoadWidth RoadSurf Coeff. Curve
W1 0 CB208 CB209 TRANSVERSE 85.31 1.84 NO
W2 0 CB210B CB211 TRANSVERSE 85.6 1.84 NO
W3 0 CB210B CB202 TRANSVERSE 85.45 1.84 NO
W4 0 CB202 CB204 TRANSVERSE 85.55 1.84 NO
W5 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO
W6 0 CB201 CB200 TRANSVERSE 85.65 1.84 NO
W7 0 CB200 CB209 TRANSVERSE 85.42 1.84 NO

[ OUTLETS
;;Name From Node To Node Offset Type QTable/Qcoeff
Qexpon Gated
CB200 CB200 520_(P-Stm) 83.18 TABULAR/HEAD IPEx_Type_A
;Critical
CB201 CB201 520_(P-Stm) 83.09 TABULAR/HEAD Vortex_ICD_100
NO
;Critical
CB202 CB202 521_(P-Stm) 82.76 TABULAR/HEAD IPEx_Type_A
NO
CB203 CB203 521_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_70
NO
CB204 CB204 522_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_65
NO
CB205 CB205 522_(P-Stm) 82.85 TABULAR/HEAD Vortex_ICD_65
NO
CB206 CB206 522_(P-Stm) 82.77 TABULAR/HEAD Vortex_ICD_70
NO
CB207 CB207 522_(P-Stm) 83.15 TABULAR/HEAD Vortex_ICD_100
NO
CB209 CB209 520_(P-Stm) 83.2 TABULAR/HEAD IPEx_Type_A
NO
CB210B CB210B 518_(P-Stm) 82.65 TABULAR/HEAD Vortex_ICD_65
NO
CB212 CB212 521_(P-Stm) 83.2 TABULAR/HEAD Vortex_ICD_100
NO

[ XSECTIONS

```

Link Culvert	Shape	Geom1	Geom2	Geom3	Geom4	Barrels	Vortex_ICD_100	1	8.9
;Link							Vortex_ICD_100	1	8.9
;Link							Vortex_ICD_100	1.2	9.8
;Link							Vortex_ICD_100	1.4	10.6
;Link							Vortex_ICD_100	1.6	11.3
C2	CIRCULAR	0.25	0	0	0	1	Vortex_ICD_100	1.8	12
C4	CIRCULAR	0.25	0	0	0	1	Vortex_ICD_100	2	12.6
C5	CIRCULAR	0.25	0	0	0	1	Vortex_ICD_100	2.5	14.1
CB213	CIRCULAR	0.25	0	0	0	1	Vortex_ICD_100	3	15.5
CB214	CIRCULAR	0.25	0	0	0	1	Vortex_ICD_100	3	15.5
Pipe_--(104)_-(P-Stm)_2	CIRCULAR	0.45	0	0	0	1	;Tempest Rating Curve for Vortex ICD 105, No grate allowance		
Pipe_--(110)_-(P-Stm)	CIRCULAR	0.525	0	0	0	1	Vortex_ICD_105	0	0
Pipe_--(111)_-(P-Stm)	CIRCULAR	0.525	0	0	0	1	Vortex_ICD_105	0.1	3.1
Pipe_--(71)_-(P-Stm)	CIRCULAR	0.525	0	0	0	1	Vortex_ICD_105	0.2	4.4
Pipe_--(72)_-(P-Stm)	CIRCULAR	0.525	0	0	0	1	Vortex_ICD_105	0.3	5.4
OR1	CIRCULAR	0.127	0	0	0	0	Vortex_ICD_105	0.4	6.2
W1	RECT_OPEN	0.03	1.5	0	0	0	Vortex_ICD_105	0.5	6.9
W2	RECT_OPEN	0.05	2.59	0	0	0	Vortex_ICD_105	0.6	7.6
W3	RECT_OPEN	0.2	1.34	0	0	0	Vortex_ICD_105	0.7	8.2
W4	RECT_OPEN	0.1	4.9	0	0	0	Vortex_ICD_105	0.8	8.8
W5	RECT_OPEN	0.05	4.725	0	0	0	Vortex_ICD_105	0.9	9.3
W6	RECT_OPEN	0.1	6	0	0	0	Vortex_ICD_105	1	9.8
W7	RECT_OPEN	0.08	10.5	0	0	0	Vortex_ICD_105	1.2	10.7
[LOSSES]							Vortex_ICD_105	1.4	11.6
;Link							Vortex_ICD_105	1.6	12.4
;Link							Vortex_ICD_105	1.8	13.1
;Link							Vortex_ICD_105	2	13.9
;Link							Vortex_ICD_105	2.5	15.5
;Link							Vortex_ICD_105	3	17
[CURVES]							;Tempest Rating Curve for Vortex ICD 40, No grate allowance		
;Name							Vortex_ICD_40	0	0
;Name							Vortex_ICD_40	0.1	0.4
;Name							Vortex_ICD_40	0.2	0.6
;Name							Vortex_ICD_40	0.3	0.7
;Name							Vortex_ICD_40	0.4	0.9
;Name							Vortex_ICD_40	0.5	1
;Name							Vortex_ICD_40	0.6	1.1
;Name							Vortex_ICD_40	0.7	1.1
;Name							Vortex_ICD_40	0.8	1.2
;Name							Vortex_ICD_40	0.9	1.3
;Name							Vortex_ICD_40	1	1.4
;Name							Vortex_ICD_40	1.2	1.5
;Name							Vortex_ICD_40	1.4	1.6
;Name							Vortex_ICD_40	1.6	1.7
;Name							Vortex_ICD_40	1.8	1.8
;Name							Vortex_ICD_40	2	1.9
;Name							Vortex_ICD_40	2.5	2.2
;Name							Vortex_ICD_40	3	2.4
;Name							;Tempest Rating Curve for Vortex ICD 45, No grate allowance		
;Name							Vortex_ICD_45	0	0
;Name							Vortex_ICD_45	0.1	0.6
;Name							Vortex_ICD_45	0.2	0.8
;Name							Vortex_ICD_45	0.3	1
;Name							Vortex_ICD_45	0.4	1.1
;Name							Vortex_ICD_45	0.5	1.3
;Name							Vortex_ICD_45	0.6	1.4
;Name							Vortex_ICD_45	0.7	1.5
;Name							Vortex_ICD_45	0.8	1.6
;Name							Vortex_ICD_45	0.9	1.7
;Name							Vortex_ICD_45	1	1.8
;Name							Vortex_ICD_45	1.2	2
;Name							Vortex_ICD_45	1.4	2.1
;Name							Vortex_ICD_45	1.6	2.3
;Name							Vortex_ICD_45	1.8	2.4
;Name							Vortex_ICD_45	2	2.6
;Name							Vortex_ICD_45	2.5	2.9
;Name							Vortex_ICD_45	3	3.1
;Name							;Tempest Rating Curve for Vortex ICD 50, No grate allowance		
;Name							Vortex_ICD_50	0	0
;Name							Vortex_ICD_50	0.1	0.7
;Name							Vortex_ICD_50	0.2	1
;Name							Vortex_ICD_50	0.3	1.2
;Name							Vortex_ICD_50	0.4	1.4
;Name							Vortex_ICD_50	0.5	1.6
;Name							Vortex_ICD_50	0.6	1.8
;Name							Vortex_ICD_50	0.7	1.9
;Name							Vortex_ICD_50	0.8	2
;Name							Vortex_ICD_50	0.9	2.1
;Name							Vortex_ICD_50	1	2.3
;Name							Vortex_ICD_50	1.2	2.5
;Name							Vortex_ICD_50	1.4	2.7
;Name							Vortex_ICD_50	1.6	2.9
;Name							Vortex_ICD_50	1.8	3
;Name							Vortex_ICD_50	2	3.2
;Name							Vortex_ICD_50	2.5	3.6
;Name							Vortex_ICD_50	3	3.9
;Name							;Tempest Rating Curve for Vortex ICD 55, No grate allowance		
;Name							Vortex_ICD_55	0	0
;Name							Vortex_ICD_55	0.1	0.9
;Name							Vortex_ICD_55	0.2	1.2
;Name							Vortex_ICD_55	0.3	1.5
;Name							Vortex_ICD_55	0.4	1.7
;Name							Vortex_ICD_55	0.5	1.9
;Name							Vortex_ICD_55	0.6	2.1
;Name							Vortex_ICD_55	0.7	2.3
;Name							Vortex_ICD_55	0.8	2.4
;Name							Vortex_ICD_55	0.9	2.6
;Name							Vortex_ICD_55	1	2.7
;Name							Vortex_ICD_55	1.2	3
;Name							Vortex_ICD_55	1.4	3.2
;Name							Vortex_ICD_55	1.6	3.4
;Name							Vortex_ICD_55	1.8	3.6
;Name							Vortex_ICD_55	2	3.8
;Name							Vortex_ICD_55	2.5	4.3
;Name							Vortex_ICD_55	3	4.7
;Name							;Tempest Rating Curve for Vortex ICD 60, No grate allowance		
;Name							Vortex_ICD_60	0	0
;Name							Vortex_ICD_60	0.1	1.1
;Name							Vortex_ICD_60	0.2	1.5
;Name							Vortex_ICD_60	0.3	1.8
;Name							Vortex_ICD_60	0.4	2.1
;Name							Vortex_ICD_60	0.5	2.3
;Name							Vortex_ICD_60	0.6	2.5
;Name							Vortex_ICD_60	0.7	2.7
;Name							Vortex_ICD_60	0.8	2.9
;Name							Vortex_ICD_60	0.9	3.1
;Name							Vortex_ICD_60	1	3.2
;Name							Vortex_ICD_60	1.2	3.6
;Name							Vortex_ICD_60	1.4	3.8
;Name							Vortex_ICD_60	1.6	4.1
;Name							Vortex_ICD_60	1.8	4.3
;Name							Vortex_ICD_60	2	4.6
;Name							Vortex_ICD_60	2.5	5.1
;Name							Vortex_ICD_60	3	5.6
;Name							;Tempest Rating Curve for Vortex ICD 65, No grate allowance		
;Name							Vortex_ICD_65	0	0
;Name							Vortex_ICD_65	0.1	1.2
;Name							Vortex_ICD_65	0.2	1.6
;Name							Vortex_ICD_65	0.3	2
;Name							Vortex_ICD_65	0.4	2.3
;Name							Vortex_ICD_65	0.5	2.5
;Name							Vortex_ICD_65	0.6	2.8
;Name							Vortex_ICD_65	0.7	3
;Name							Vortex_ICD_65	0.8	3.2
;Name							Vortex_ICD_65	0.9	3.4
;Name							Vortex_ICD_65	1	3.6
;Name							Vortex_ICD_65	1.2	4
;Name							Vortex_ICD_65	1.4	4.3
;Name							Vortex_ICD_65	1.6	4.6
;Name							Vortex_ICD_65	1.8	4.9
;Name							Vortex_ICD_65	2	5.1
;Name							Vortex_ICD_65	2.5	5.7
;Name							Vortex_ICD_65	3	6.3
;Name							;Tempest Rating Curve for Vortex ICD 100, No grate allowance		
;Name							Vortex_ICD_100	0	0
;Name							Vortex_ICD_100	0.1	2.8
;Name							Vortex_ICD_100	0.2	4
;Name							Vortex_ICD_100	0.3	4.9
;Name							Vortex_ICD_100	0.4	5.6
;Name							Vortex_ICD_100	0.5	6.3
;Name							Vortex_ICD_100	0.6	6.9
;Name							Vortex_ICD_100	0.7	7.5
;Name							Vortex_ICD_100	0.8	8
;Name							Vortex_ICD_100	0.9	8.5

```

;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70 Rating 0 0
Vortex_ICD_70 0.1 1.3
Vortex_ICD_70 0.2 1.9
Vortex_ICD_70 0.3 2.3
Vortex_ICD_70 0.4 2.7
Vortex_ICD_70 0.5 3
Vortex_ICD_70 0.6 3.3
Vortex_ICD_70 0.7 3.6
Vortex_ICD_70 0.8 3.8
Vortex_ICD_70 0.9 4.1
Vortex_ICD_70 1 4.4
Vortex_ICD_70 1.2 4.7
Vortex_ICD_70 1.4 5.1
Vortex_ICD_70 1.6 5.5
Vortex_ICD_70 1.8 5.8
Vortex_ICD_70 2 6.1
Vortex_ICD_70 2.5 6.8
Vortex_ICD_70 3 7.5
    
```

```

;Tempest Rating Curve for Vortex ICD 75, No grate allowance
Vortex_ICD_75 Rating 0 0
Vortex_ICD_75 0.1 1.6
Vortex_ICD_75 0.2 2.2
Vortex_ICD_75 0.3 2.7
Vortex_ICD_75 0.4 3.2
Vortex_ICD_75 0.5 3.5
Vortex_ICD_75 0.6 3.9
Vortex_ICD_75 0.7 4.2
Vortex_ICD_75 0.8 4.5
Vortex_ICD_75 0.9 4.8
Vortex_ICD_75 1 5
Vortex_ICD_75 1.2 5.5
Vortex_ICD_75 1.4 5.9
Vortex_ICD_75 1.6 6.3
Vortex_ICD_75 1.8 6.7
Vortex_ICD_75 2 7.1
Vortex_ICD_75 2.5 7.9
Vortex_ICD_75 3 8.7
    
```

```

;Tempest Rating Curve for Vortex ICD 80, No grate allowance
Vortex_ICD_80 Rating 0 0
Vortex_ICD_80 0.1 1.8
Vortex_ICD_80 0.2 2.6
Vortex_ICD_80 0.3 3.1
Vortex_ICD_80 0.4 3.6
Vortex_ICD_80 0.5 4
Vortex_ICD_80 0.6 4.4
Vortex_ICD_80 0.7 4.8
Vortex_ICD_80 0.8 5.1
Vortex_ICD_80 0.9 5.4
Vortex_ICD_80 1 5.7
Vortex_ICD_80 1.2 6.3
Vortex_ICD_80 1.4 6.8
Vortex_ICD_80 1.6 7.2
Vortex_ICD_80 1.8 7.7
Vortex_ICD_80 2 8.1
Vortex_ICD_80 2.5 9
Vortex_ICD_80 3 9.9
    
```

```

;Tempest Rating Curve for Vortex ICD 85, No grate allowance
Vortex_ICD_85 Rating 0 0
Vortex_ICD_85 0.1 2
Vortex_ICD_85 0.2 2.9
Vortex_ICD_85 0.3 3.5
Vortex_ICD_85 0.4 4.1
Vortex_ICD_85 0.5 4.5
Vortex_ICD_85 0.6 5
Vortex_ICD_85 0.7 5.4
Vortex_ICD_85 0.8 5.7
Vortex_ICD_85 0.9 6.1
Vortex_ICD_85 1 6.4
Vortex_ICD_85 1.2 7
Vortex_ICD_85 1.4 7.6
Vortex_ICD_85 1.6 8.1
Vortex_ICD_85 1.8 8.6
Vortex_ICD_85 2 9.1
Vortex_ICD_85 2.5 10.1
Vortex_ICD_85 3 11.1
    
```

```

;Tempest Rating Curve for Vortex ICD 90, No grate allowance
Vortex_ICD_90 Rating 0 0
Vortex_ICD_90 0.1 2.2
Vortex_ICD_90 0.2 3.2
Vortex_ICD_90 0.3 3.9
Vortex_ICD_90 0.4 4.5
Vortex_ICD_90 0.5 5.1
Vortex_ICD_90 0.6 5.5
Vortex_ICD_90 0.7 6
Vortex_ICD_90 0.8 6.4
Vortex_ICD_90 0.9 6.8
Vortex_ICD_90 1 7.2
Vortex_ICD_90 1.2 7.9
Vortex_ICD_90 1.4 8.5
Vortex_ICD_90 1.6 9.1
Vortex_ICD_90 1.8 9.6
Vortex_ICD_90 2 10.2
Vortex_ICD_90 2.5 11.4
Vortex_ICD_90 3 12.5
    
```

```

;Tempest Rating Curve for Vortex ICD 95, No grate allowance
Vortex_ICD_95 Rating 0 0
Vortex_ICD_95 0.1 2.6
Vortex_ICD_95 0.2 3.6
Vortex_ICD_95 0.3 4.4
Vortex_ICD_95 0.4 5.1
Vortex_ICD_95 0.5 5.7
Vortex_ICD_95 0.6 6.2
Vortex_ICD_95 0.7 6.7
Vortex_ICD_95 0.8 7.1
Vortex_ICD_95 0.9 7.6
Vortex_ICD_95 1 8
Vortex_ICD_95 1.2 8.7
Vortex_ICD_95 1.4 9.4
Vortex_ICD_95 1.6 10.1
Vortex_ICD_95 1.8 10.7
Vortex_ICD_95 2 11.3
Vortex_ICD_95 2.5 12.6
Vortex_ICD_95 3 13.8
    
```

CB200	Storage	0	0.36
CB200		2.24	210.73
CB200		2.32	210.73
CB201	Storage	0	0.36
CB201		2.41	169.64
CB201		2.56	169.64
CB201		2.66	169.64
CB202	Storage	0	0.36
CB202		2.6	0.36
CB202		2.79	283.64
CB202		2.89	283.64
CB203	Storage	0	0.36
CB203		2.6	0.36
CB203		2.9	178.09
CB204	Storage	0	0.36
CB204		2.6	0.36
CB204		2.75	146.08
CB205	Storage	0	0.36
CB205		2.6	0.36
CB205		2.75	86.52
CB206	Storage	0	0.36
CB206		2.6	0.36

CB206		2.68	61.65
CB206		2.73	61.65
CB207	Storage	0	0.36
CB207		1.97	0.36
CB207		2.3	182.21
CB208	Storage	0	0.073
CB208		1.74	0.073
CB208		2.01	67.88
CB208		2.04	67.88
CB209	Storage	0	0.073
CB209		1.6	0.073
CB209		1.9	84.87
CB210B	Storage	0	0.073
CB210B		2.6	0.073
CB210B		2.9	60.53
CB210B		3	60.53
CB211	Storage	0	0.073
CB211		1.65	0.073
CB211		1.9	21.99
CB212	Storage	0	0.36
CB212		2.2	0.36
CB212		2.42	55.12
CB212A	Storage	0	0.073
CB212A		2	0.073
CB213	Storage	0	0.073
CB213		1.6	0.073
CB213		1.75	9.05
CB214	Storage	0	0.073
CB214		1.6	0.073
CB214		1.9	34.75

```

[TIMESERIES]
;Name Date Time Value
;Rainfall (mm/hr)
3CHI002 01/01/2000 00:00:00 2.491
3CHI002 01/01/2000 00:10:00 2.366
3CHI002 01/01/2000 00:20:00 3.696
3CHI002 01/01/2000 00:30:00 4.976
3CHI002 01/01/2000 00:40:00 7.828
3CHI002 01/01/2000 00:50:00 19.966
3CHI002 01/01/2000 01:00:00 76.805
3CHI002 01/01/2000 01:10:00 22.777
3CHI002 01/01/2000 01:20:00 11.852
3CHI002 01/01/2000 01:30:00 8.025
3CHI002 01/01/2000 01:40:00 6.096
3CHI002 01/01/2000 01:50:00 4.938
3CHI002 01/01/2000 02:00:00 4.165
3CHI002 01/01/2000 02:10:00 3.613
3CHI002 01/01/2000 02:20:00 3.197
3CHI002 01/01/2000 02:30:00 2.873
3CHI002 01/01/2000 02:40:00 2.613
3CHI002 01/01/2000 02:50:00 2.4
3CHI002 01/01/2000 03:00:00 0
    
```

```

;Rainfall (mm/hr)
3CHI100 01/01/2000 00:00:00 5.339
3CHI100 01/01/2000 00:10:00 6.376
3CHI100 01/01/2000 00:20:00 7.977
3CHI100 01/01/2000 00:30:00 10.797
3CHI100 01/01/2000 00:40:00 17.136
3CHI100 01/01/2000 00:50:00 45.128
3CHI100 01/01/2000 01:00:00 178.107
3CHI100 01/01/2000 01:10:00 51.056
3CHI100 01/01/2000 01:20:00 26.163
3CHI100 01/01/2000 01:30:00 17.571
3CHI100 01/01/2000 01:40:00 13.277
3CHI100 01/01/2000 01:50:00 10.712
3CHI100 01/01/2000 02:00:00 9.008
3CHI100 01/01/2000 02:10:00 7.793
3CHI100 01/01/2000 02:20:00 6.883
3CHI100 01/01/2000 02:30:00 6.174
3CHI100 01/01/2000 02:40:00 5.607
3CHI100 01/01/2000 02:50:00 5.142
3CHI100 01/01/2000 03:00:00 0
    
```

```

[REPORT]
;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
    
```

```

[TAGS]
Node 518_Orifice RY_Manhole
Node CB200 RY_Manhole
Node CB201 RY_Manhole
Node CB202 RY_Manhole
Node CB203 RY_Manhole
Node CB204 RY_Manhole
Node CB205 RY_Manhole
Node CB206 RY_Manhole
Node CB207 RY_Manhole
Node CB208 RY_Manhole
Node CB209 RY_Manhole
Node CB210B RY_Manhole
Node CB211 RY_Manhole
Node CB212 RY_Manhole
Node CB212A RY_Manhole
Node CB213 RY_Manhole
Node CB214 RY_Manhole
Link C2 RY_Sewer
Link C4 RY_Sewer
Link C5 RY_Sewer
    
```

```

[MAP]
DIMENSIONS 381219.0294 5032865.6782 381334.8286 5033009.9278
UNITS Meters
    
```

```

[COORDINATES]
;Node X-Coord Y-Coord
518_Orifice 381303.7 5032953.188
518A_(P-Stm) 381317.029 5032955.797
518_(P-Stm) 381301.695 5032952.807
519_(P-Stm) 381278.558 5032974.866
520_(P-Stm) 381240.562 5032932.182
521_(P-Stm) 381309.324 5032926.69
522_(P-Stm) 381267.739 5032909.252
CB200 381246.119 5032946.933
CB201 381266.142 5032969.504
CB202 381305.816 5032949.229
CB203 381314.519 5032914.342
CB204 381291.187 5032921.779
CB205 381286.939 5032925.282
CB206 381270.233 5032913.017
CB207 381253.811 5032910.356
CB208 381248.314 5032902.406
CB209 381228.288 5032926.78
CB210B 381314.246 5032963.095
CB211 381319.351 5032951.771
CB212 381328.283 5032931.288
CB212A 381326.507 5032912.9
CB213 381296.875 5032912.718
CB214 381282.831 5032893.727
    
```

```

[VERTICES]
;Link X-Coord Y-Coord
;
    
```

W2	381318.013	5032956.452	S10	381287.827	5032916.061
W4	381305.808	5032934.267	S10	381287.58	5032915.993
W4	381304.151	5032928.843	S10	381287.336	5032915.916
[POLYGONS]			S10	381287.095	5032915.831
;; Subcatchment	X-Coord	Y-Coord	S10	381286.857	5032915.737
;;	-----464	-----825	S10	381286.622	5032915.635
S1	381240.329	5032905.467	S10	381286.391	5032915.525
S1	381240.224	5032906.884	S10	381286.163	5032915.407
S1	381246.264	5032909.575	S10	381285.94	5032915.281
S1	381246.323	5032909.6	S10	381285.722	5032915.148
S1	381246.383	5032909.624	S10	381285.508	5032915.007
S1	381246.444	5032909.645	S10	381285.3	5032914.859
S1	381246.506	5032909.663	S10	381285.096	5032914.703
S1	381246.568	5032909.68	S10	381284.899	5032914.541
S1	381246.631	5032909.695	S10	381284.707	5032914.371
S1	381246.695	5032909.707	S10	381284.521	5032914.196
S1	381246.758	5032909.717	S10	381284.341	5032914.013
S1	381246.823	5032909.725	S10	381284.168	5032913.825
S1	381246.887	5032909.731	S10	381284.001	5032913.63
S1	381246.951	5032909.734	S10	381283.841	5032913.43
S1	381247.016	5032909.735	S10	381283.689	5032913.225
S1	381247.08	5032909.734	S10	381283.543	5032913.014
S1	381247.145	5032909.732	S10	381283.405	5032912.799
S1	381247.209	5032909.725	S10	381283.275	5032912.578
S1	381247.273	5032909.717	S10	381283.152	5032912.354
S1	381247.337	5032909.707	S10	381283.037	5032912.125
S1	381247.401	5032909.695	S10	381282.93	5032911.892
S1	381247.463	5032909.681	S10	381282.832	5032911.656
S1	381247.526	5032909.664	S10	381282.742	5032911.416
S1	381247.588	5032909.645	S10	381282.66	5032911.174
S1	381247.649	5032909.624	S10	381282.586	5032910.929
S1	381247.709	5032909.601	S10	381282.521	5032910.681
S1	381247.768	5032909.576	S10	381282.465	5032910.431
S1	381247.827	5032909.548	S10	381282.418	5032910.18
S1	381247.885	5032909.519	S10	381282.379	5032909.927
S1	381247.941	5032909.488	S11	381282.349	5032909.672
S1	381247.996	5032909.455	S11	381282.247	5032909.62
S1	381248.051	5032909.419	S11	381282.152	5032909.07
S1	381248.103	5032909.382	S11	381295.209	5032987.02
S1	381248.155	5032909.344	S11	381303.22	5032979.472
S1	381248.205	5032909.303	S11	381312.567	5032959.198
S1	381248.254	5032909.261	S11	381307.857	5032956.13
S1	381248.301	5032909.217	S11	381307.843	5032956.124
S1	381248.347	5032909.171	S11	381307.829	5032956.117
S1	381248.391	5032909.124	S11	381307.815	5032956.111
S1	381248.433	5032909.075	S11	381307.801	5032956.103
S1	381248.474	5032909.025	S11	381307.788	5032956.095
S1	381248.513	5032908.973	S11	381307.774	5032956.087
S1	381248.55	5032908.92	S11	381307.762	5032956.078
S1	381248.585	5032908.866	S11	381307.749	5032956.069
S1	381248.618	5032908.811	S11	381307.737	5032956.06
S1	381248.65	5032908.754	S11	381307.725	5032956.05
S1	381248.679	5032908.697	S11	381307.713	5032956.039
S1	381248.706	5032908.638	S11	381307.702	5032956.028
S1	381259.547	5032884.338	S11	381307.691	5032956.017
S1	381264.625	5032879.885	S11	381307.681	5032956.006
S1	381253.013	5032884.608	S11	381307.67	5032955.994
S1	381244.74	5032903.289	S11	381307.662	5032955.981
S1	381244.061	5032903.318	S11	381307.653	5032955.969
S1	381243.383	5032903.37	S11	381307.644	5032955.956
S1	381242.707	5032903.444	S11	381307.635	5032955.943
S1	381242.035	5032903.54	S11	381307.628	5032955.929
S1	381241.365	5032903.658	S11	381307.621	5032955.915
S1	381240.7	5032903.799	S11	381307.614	5032955.901
S1	381240.04	5032903.961	S11	381307.608	5032955.887
S1	381239.386	5032904.145	S11	381307.602	5032955.873
S1	381238.738	5032904.35	S11	381307.596	5032955.858
S1	381238.097	5032904.577	S11	381307.592	5032955.843
S1	381237.464	5032904.825	S11	381307.587	5032955.828
S10	381282.247	5032908.62	S11	381307.58	5032955.813
S10	381276.098	5032921.781	S11	381307.578	5032955.798
S10	381287.174	5032924.398	S11	381307.57	5032955.783
S10	381297.866	5032927.382	S11	381307.562	5032955.767
S10	381298.206	5032927.496	S11	381307.576	5032955.752
S10	381298.542	5032927.621	S11	381307.573	5032955.737
S10	381298.873	5032927.758	S11	381307.572	5032955.721
S10	381299.199	5032927.907	S11	381307.57	5032955.705
S10	381299.52	5032928.067	S11	381307.573	5032955.69
S10	381299.835	5032928.238	S11	381307.574	5032955.674
S10	381300.144	5032928.42	S11	381307.576	5032955.659
S10	381300.446	5032928.612	S11	381307.578	5032955.644
S10	381300.742	5032928.815	S11	381307.581	5032955.628
S10	381301.03	5032929.029	S11	381307.584	5032955.613
S10	381301.31	5032929.252	S11	381307.588	5032955.598
S10	381301.583	5032929.485	S11	381307.592	5032955.583
S10	381301.847	5032929.727	S11	381307.59	5032955.568
S10	381302.103	5032929.979	S11	381307.602	5032955.554
S10	381302.349	5032930.239	S11	381307.608	5032955.539
S10	381302.586	5032930.508	S11	381307.614	5032955.525
S10	381302.814	5032930.785	S11	381307.62	5032955.511
S10	381303.032	5032931.069	S11	381307.628	5032955.497
S10	381303.24	5032931.362	S11	381307.636	5032955.484
S10	381303.437	5032931.661	S11	381307.644	5032955.471
S10	381304.434	5032927.758	S11	381307.653	5032955.458
S10	381307.137	5032921.453	S11	381307.662	5032955.445
S10	381307.111	5032921.441	S11	381307.672	5032955.433
S10	381307.084	5032921.427	S11	381307.682	5032955.421
S10	381307.058	5032921.413	S11	381307.69	5032955.41
S10	381307.033	5032921.398	S11	381307.703	5032955.398
S10	381307.008	5032921.382	S11	381307.714	5032955.388
S10	381306.984	5032921.366	S11	381307.726	5032955.377
S10	381306.96	5032921.348	S11	381307.738	5032955.367
S10	381306.937	5032921.33	S11	381307.75	5032955.358
S10	381306.915	5032921.311	S11	381307.762	5032955.348
S10	381306.893	5032921.291	S11	381307.775	5032955.34
S10	381306.872	5032921.27	S11	381307.788	5032955.331
S10	381306.852	5032921.249	S11	381307.802	5032955.324
S10	381306.833	5032921.227	S11	381307.816	5032955.316
S10	381306.814	5032921.204	S11	381307.83	5032955.309
S10	381306.796	5032921.181	S11	381307.844	5032955.303
S10	381306.779	5032921.157	S11	381307.858	5032955.294
S10	381306.763	5032921.132	S11	381307.873	5032955.292
S10	381306.747	5032921.107	S11	381307.888	5032955.287
S10	381306.733	5032921.082	S11	381307.902	5032955.283
S10	381306.719	5032921.056	S11	381307.918	5032955.279
S10	381306.706	5032921.029	S11	381307.933	5032955.276
S10	381306.695	5032921.002	S11	381307.948	5032955.273
S10	381306.684	5032920.975	S11	381307.963	5032955.271
S10	381306.674	5032920.947	S11	381307.979	5032955.269
S10	381306.665	5032920.919	S11	381307.994	5032955.268
S10	381306.657	5032920.891	S11	381308.01	5032955.268
S10	381306.65	5032920.862	S11	381308.025	5032955.267
S10	381306.644	5032920.833	S11	381308.041	5032955.268
S10	381306.639	5032920.804	S11	381308.056	5032955.269
S10	381306.635	5032920.775	S11	381308.072	5032955.271
S10	381306.632	5032920.746	S11	381308.087	5032955.273
S10	381306.63	5032920.716	S11	381308.098	5032955.293
S10	381306.629	5032920.687	S11	381308.107	5032955.306
S10	381306.629	5032920.658	S11	381308.121	5032955.311
S10	381306.63	5032920.628	S11	381308.134	5032955.31
S10	381306.632	5032920.599	S11	381308.148	5032955.301
S10	381302.601	5032917.181	S11	381309.355	5032955.284
S10	381301.944	5032913.56	S11	381309.566	5032955.261
S10	381291.376	5032916.092	S11	381309.775	5032955.23
S10	381291.126	5032916.147	S11	381309.983	5032955.192
S10	381290.874	5032916.193	S11	381310.19	5032955.147
S10	381290.621	5032916.231	S11	381310.395	5032955.095
S10	381290.366	5032916.26	S11	381310.599	5032955.036
S10	381290.111	5032916.28	S11	381310.8	5032955.969
S10	381289.856	5032916.291	S11	38131	

S11	381312.803	5032953.81	S11	381306.915	5032921.311
S11	381312.961	5032953.669	S11	381306.937	5032921.33
S11	381313.114	5032953.522	S11	381306.96	5032921.348
S11	381313.261	5032953.371	S11	381306.984	5032921.366
S11	381313.403	5032953.214	S11	381307.008	5032921.382
S11	381313.54	5032953.052	S11	381307.033	5032921.398
S11	381313.671	5032952.886	S11	381307.058	5032921.413
S11	381313.796	5032952.715	S11	381307.084	5032921.427
S11	381313.916	5032952.54	S11	381307.111	5032921.441
S11	381314.029	5032952.362	S11	381307.137	5032921.453
S11	381314.136	5032952.179	S11	381304.434	5032927.758
S11	381314.237	5032951.993	S11	381303.437	5032931.661
S11	381314.331	5032951.803	S11	381299.932	5032934.777
S11	381314.418	5032951.61	S11	381296.614	5032936.126
S11	381316.055	5032947.841	S11	381293.269	5032937.63
S11	381316.645	5032946.459	S11	381290.74	5032939.886
S11	381312.262	5032944.586	S11	381291.953	5032941.245
S11	381315.128	5032937.954	S11	381287.876	5032944.882
S11	381317.996	5032931.347	S11	381294.345	5032952.211
S11	381318.388	5032931.518	S11	381285.292	5032960.286
S11	381320.605	5032926.434	S11	381286.244	5032968.414
S11	381320.687	5032926.249	S11	381286.461	5032970.262
S11	381320.763	5032926.062	S11	381287.493	5032979.07
S11	381320.832	5032925.872	S12	381323.653	5032914.693
S11	381320.895	5032925.68	S11	381326.899	5032904.508
S11	381320.951	5032925.486	S12	381316.177	5032907.046
S11	381321.001	5032925.29	S12	381287.051	5032914.024
S11	381321.043	5032925.092	S12	381286.59	5032915.621
S11	381321.079	5032924.893	S11	381286.826	5032917.724
S11	381321.108	5032924.693	S12	381287.066	5032915.82
S11	381321.13	5032924.492	S12	381287.309	5032915.907
S11	381321.145	5032924.291	S12	381287.554	5032915.985
S11	381321.153	5032924.089	S11	381287.803	5032916.058
S11	381321.154	5032923.887	S12	381288.053	5032916.115
S11	381321.149	5032923.685	S12	381288.306	5032916.167
S11	381321.136	5032923.483	S12	381288.56	5032916.211
S11	381321.116	5032923.282	S11	381288.815	5032916.245
S11	381321.089	5032923.082	S12	381289.072	5032916.27
S11	381321.056	5032922.882	S12	381289.329	5032916.286
S11	381321.016	5032922.684	S12	381289.587	5032916.293
S11	381320.968	5032922.488	S11	381289.845	5032916.299
S11	381320.914	5032922.293	S12	381290.102	5032916.281
S11	381320.854	5032922.1	S12	381290.359	5032916.261
S11	381320.787	5032921.909	S12	381290.615	5032916.232
S11	381320.713	5032921.721	S12	381290.87	5032916.194
S11	381320.633	5032921.536	S12	381291.124	5032916.147
S11	381320.546	5032921.353	S12	381291.376	5032916.092
S11	381320.453	5032921.174	S12	381301.944	5032913.56
S11	381320.354	5032920.997	S12	381312.521	5032911.026
S11	381320.249	5032920.825	S12	381312.897	5032910.942
S11	381320.138	5032920.656	S12	381313.277	5032910.872
S11	381320.022	5032920.491	S12	381313.658	5032910.815
S11	381319.899	5032920.33	S12	381314.041	5032910.771
S11	381319.772	5032920.173	S12	381314.426	5032910.74
S11	381319.639	5032920.021	S12	381314.811	5032910.723
S11	381319.5	5032919.873	S12	381315.196	5032910.719
S11	381319.357	5032919.731	S11	381315.582	5032910.728
S11	381319.209	5032919.583	S12	381315.967	5032910.751
S11	381319.056	5032919.461	S12	381316.351	5032910.787
S11	381318.899	5032919.334	S12	381316.733	5032910.836
S11	381318.738	5032919.212	S12	381317.114	5032910.899
S11	381318.572	5032919.096	S12	381317.492	5032910.974
S11	381318.403	5032918.986	S12	381317.867	5032911.063
S11	381318.23	5032918.882	S12	381318.239	5032911.164
S11	381318.053	5032918.784	S11	381318.608	5032911.279
S11	381317.873	5032918.692	S12	381318.972	5032911.406
S11	381317.69	5032918.606	S12	381319.331	5032911.545
S11	381317.504	5032918.526	S12	381319.686	5032911.697
S11	381317.316	5032918.454	S12	381320.035	5032911.861
S11	381317.125	5032918.387	S12	381320.379	5032911.937
S11	381316.932	5032918.327	S12	381320.715	5032912.225
S11	381316.737	5032918.274	S12	381321.045	5032912.424
S11	381316.54	5032918.228	S12	381321.368	5032912.635
S11	381316.342	5032918.188	S12	381321.681	5032912.856
S11	381316.142	5032918.156	S12	381321.992	5032913.089
S11	381315.942	5032918.13	S12	381322.291	5032913.332
S11	381315.741	5032918.111	S12	381322.582	5032913.585
S11	381315.539	5032918.099	S12	381322.864	5032913.849
S11	381315.337	5032918.094	S12	381323.137	5032914.121
S11	381315.135	5032918.096	S12	381323.4	5032914.402
S11	381314.933	5032918.105	S12	381323.653	5032914.693
S11	381314.731	5032918.121	S14-1	381325.118	5032942.295
S11	381314.531	5032918.144	S14-1	381325.119	5032942.291
S11	381314.331	5032918.174	S14-1	381315.128	5032937.954
S11	381314.132	5032918.21	S14-1	381312.262	5032944.556
S11	381313.935	5032918.254	S14-1	381316.645	5032946.459
S11	381307.281	5032919.848	S14-1	381316.055	5032947.841
S11	381307.252	5032919.855	S14-1	381314.418	5032951.61
S11	381307.224	5032919.864	S14-1	381314.331	5032951.803
S11	381307.196	5032919.873	S11	381314.236	5032951.993
S11	381307.169	5032919.883	S14-1	381314.136	5032952.179
S11	381307.142	5032919.895	S14-1	381314.029	5032952.362
S11	381307.115	5032919.907	S14-1	381313.915	5032952.541
S11	381307.088	5032919.92	S14-1	381313.796	5032952.716
S11	381307.062	5032919.934	S14-1	381313.67	5032952.887
S11	381307.037	5032919.949	S14-1	381313.539	5032953.053
S11	381307.012	5032919.964	S14-1	381315.106	5032952.425
S11	381306.988	5032919.981	S14-1	381319.796	5032954.473
S11	381306.964	5032919.998	S14-1	381325.118	5032942.295
S11	381306.941	5032920.017	S14-2	381315.128	5032937.954
S11	381306.919	5032920.036	S14-2	381325.119	5032942.291
S11	381306.897	5032920.055	S14-2	381329.565	5032932.118
S11	381306.876	5032920.076	S14-2	381326.899	5032904.508
S11	381306.855	5032920.097	S14-2	381323.653	5032914.693
S11	381306.836	5032920.119	S14-2	381323.897	5032914.993
S11	381306.817	5032920.142	S14-2	381324.13	5032915.201
S11	381306.799	5032920.165	S14-2	381324.362	5032915.617
S11	381306.782	5032920.189	S14-2	381324.563	5032915.94
S11	381306.765	5032920.213	S14-2	381324.763	5032916.27
S11	381306.75	5032920.238	S14-2	381324.951	5032916.607
S11	381306.735	5032920.264	S14-2	381325.127	5032916.951
S11	381306.721	5032920.29	S14-2	381325.291	5032917.3
S11	381306.708	5032920.316	S14-2	381325.443	5032917.655
S11	381306.696	5032920.343	S14-2	381325.583	5032918.015
S11	381306.685	5032920.37	S14-2	381325.77	5032918.38
S11	381306.675	5032920.398	S14-2	381325.824	5032918.749
S11	381306.666	5032920.426	S14-2	381325.926	5032919.121
S11	381306.658	5032920.454	S14-2	381326.014	5032919.497
S11	381306.651	5032920.483	S14-2	381326.099	5032919.876
S11	381306.645	5032920.511	S14-2	381326.152	5032920.257
S11	381306.64	5032920.54	S14-2	381326.2	5032920.64
S11	381306.635	5032920.57	S14-2	381326.236	5032921.024
S11	381306.632	5032920.599	S14-2	381326.258	5032921.41
S11	381306.63	5032920.628	S14-2	381326.267	5032921.796
S11	381306.629	5032920.658	S14-2	381326.262	5032922.182
S11	381306.625	5032920.687	S14-2	381326.244	5032922.567
S11	381306.63	5032920.716	S14-2	381326.213	5032922.952
S11	381306.632	5032920.746	S14-2	381326.168	5032923.336
S11	381306.635	5032920.775	S14-2	381326.11	5032923.717
S11	381306.639	5032920.804	S14-2	381326.039	5032924.097
S11	381306.644	5032920.833	S14-2	381325.959	5032924.474
S11	381306.65	5032920.862	S14-2	381325.857	5032924.847
S11	381306.657	5032920.891	S14-2	381325.747	5032925.217
S11	381306.665	5032920.919	S14-2	381325.624	5032925.583
S11	381306.674	5032920.947	S14-2	381325.488	5032925.945
S11	381306.684	5032920.975	S14-2	381325.34	5032926.301
S11	381306.695	5032921.002	S14-2	381322.339	5032933.237
S11	381306.706	5032921.029	S14-2	381317.996	5032931.347
S11	381306.719	5032921.056	S14-2	381315.128	5032927.954
S11	381306.733	5032921.082	S15	381285.409	5033003.364
S11	381306.747	5032921.107	S15	381285.416	5033003.371
S11	381306.763	5032921.132	S15	381307.633	5032982.361
S11	381306.779	5032921.157	S15	381319.796	5032954.473
S11	381306.796	5032921.181	S15	381315.106	5032952.425
S11	381306.814	5032921.204	S15	381313.539	5032953.053
S11	381306.833	5032921.227	S15	381313.403	5032953.214
S11	381306.852	5032921.249	S15	381313.26	5032953.371
S11	381306.872	5032921.27	S15	381313.113	5032953.523
S11	381306.893	5032921.291	S15	381312.96	5032953.67


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S9      381299.199    5032927.907
S9      381298.873    5032927.758
S9      381298.542    5032927.621
S9      381298.206    5032927.496
S9      381297.866    5032927.382
S9      381287.164    5032924.395
S9      381276.098    5032921.781
S9      381268.429    5032922.45
S9      381273.896    5032926.903
S9      381277.469    5032930.913
S9      381278.399    5032931.986
S9      381281.347    5032929.355
S9      381290.74     5032939.886
```

```
;;Storage Node X-Coord Y-Coord
;;-----
```

```
[SYMBOLS]
;;Gage X-Coord Y-Coord
;;-----
```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

```
*****
Element Count
*****
Number of rain gages ..... 2
Number of subcatchments ... 17
Number of nodes ..... 23
Number of links ..... 29
Number of pollutants ..... 0
Number of land uses ..... 0
```

```
*****
Raingage Summary
*****
```

Name	Data Source	Data Type	Recording Interval
3CHI002	3CHI002	INTENSITY	10 min.
3CHI100	3CHI100	INTENSITY	10 min.

```
*****
Subcatchment Summary
*****
```

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
S1	0.02	29.61	0.00	2.2000	3CHI002	CB208
S10	0.03	55.32	99.62	2.2000	3CHI002	CB204
S11	0.13	65.66	90.33	2.2000	3CHI002	CB202
S12	0.01	6.44	0.37	2.2000	3CHI002	CB213
S14_1	0.01	22.78	52.68	2.2000	3CHI002	CB211
S14_2	0.02	58.12	31.65	2.2000	3CHI002	CB212
S15	0.04	108.74	42.75	2.2000	3CHI002	CB210B
S2	0.07	33.61	62.94	2.2000	3CHI002	CB209
S3	0.07	45.79	96.69	2.2000	3CHI002	CB207
S32	0.05	29.70	14.29	2.2000	3CHI002	CB214
S4	0.05	24.67	99.97	2.2000	3CHI002	CB206
S45	0.02	14.32	99.96	2.2000	3CHI002	CB203
S5	0.08	82.96	89.52	2.2000	3CHI002	CB200
S6	0.02	40.30	0.00	2.2000	3CHI002	CB214
S7	0.07	25.95	100.00	2.2000	3CHI002	520_(P-
Stm)						
S8	0.09	35.06	80.12	2.2000	3CHI002	CB201
S9	0.03	19.08	76.83	2.2000	3CHI002	CB205

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*****
Node Summary
*****
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Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
518_Orifice	JUNCTION	81.36	4.11	0.0	
518A_(P-Stm)	OUTFALL	80.65	1.13	0.0	
518_(P-Stm)	STORAGE	81.06	4.41	0.0	
519_(P-Stm)	STORAGE	82.20	3.51	0.0	
520_(P-Stm)	STORAGE	82.49	2.89	0.0	
521_(P-Stm)	STORAGE	81.96	3.67	0.0	
522_(P-Stm)	STORAGE	82.18	3.32	0.0	
CB200	STORAGE	83.18	2.32	0.0	
CB201	STORAGE	83.09	2.66	0.0	
CB202	STORAGE	82.76	2.89	0.0	
CB203	STORAGE	82.75	2.90	0.0	
CB204	STORAGE	82.75	2.75	0.0	
CB205	STORAGE	82.85	2.75	0.0	
CB206	STORAGE	82.77	2.73	0.0	
CB207	STORAGE	83.15	2.30	0.0	
CB208	STORAGE	83.30	2.04	0.0	
CB209	STORAGE	83.20	1.90	0.0	
CB210B	STORAGE	82.65	3.00	0.0	
CB211	STORAGE	83.70	1.90	0.0	
CB212	STORAGE	83.20	2.42	0.0	
CB212A	STORAGE	83.62	2.00	0.0	
CB213	STORAGE	83.15	1.75	0.0	
CB214	STORAGE	82.95	1.90	0.0	

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*****
Link Summary
*****
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Name	From Node	To Node	Type	Length	%Slope
-					
C2	CB208	CB207	CONDUIT	9.7	1.0346
C4	CB212A	CB212	CONDUIT	18.5	2.0031
C5	CB211	CB212	CONDUIT	22.3	2.0142
CB213	CB213	522_(P-Stm)	CONDUIT	29.3	1.3396
CB214	CB214	522_(P-Stm)	CONDUIT	21.6	0.8915
Pipe_-(104)_(P-Stm)_2	518_Orifice	518A_(P-Stm)	CONDUIT	15.6	0.2496
Pipe_-(110)_(P-Stm)	521_(P-Stm)	518_(P-Stm)	CONDUIT	27.2	0.3528
Pipe_-(111)_(P-Stm)	522_(P-Stm)	521_(P-Stm)	CONDUIT	45.1	0.4990
Pipe_-(71)_(P-Stm)	520_(P-Stm)	519_(P-Stm)	CONDUIT	57.1	0.4025
Pipe_-(72)_(P-Stm)	519_(P-Stm)	518_(P-Stm)	CONDUIT	32.0	0.5005
OR1	518_(P-Stm)	518_Orifice	ORIFICE		
W1	CB208	CB209	WEIR		
W2	CB210B	CB211	WEIR		
W3	CB210B	CB202	WEIR		
W4	CB202	CB204	WEIR		
W5	CB206	CB207	WEIR		
W6	CB201	CB200	WEIR		
W7	CB200	CB209	WEIR		
CB200	CB200	520_(P-Stm)	OUTLET		
CB201	CB201	520_(P-Stm)	OUTLET		
CB202	CB202	521_(P-Stm)	OUTLET		
CB203	CB203	521_(P-Stm)	OUTLET		
CB204	CB204	522_(P-Stm)	OUTLET		
CB205	CB205	522_(P-Stm)	OUTLET		
CB206	CB206	522_(P-Stm)	OUTLET		
CB207	CB207	522_(P-Stm)	OUTLET		
CB209	CB209	520_(P-Stm)	OUTLET		
CB210B	CB210B	518_(P-Stm)	OUTLET		
CB212	CB212	521_(P-Stm)	OUTLET		

```
*****
Cross Section Summary
*****
```

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C2	CIRCULAR	0.25	0.05	0.06	0.25	1	60.49
C4	CIRCULAR	0.25	0.05	0.06	0.25	1	84.17
C5	CIRCULAR	0.25	0.05	0.06	0.25	1	84.40
CB213	CIRCULAR	0.25	0.05	0.06	0.25	1	68.83
CB214	CIRCULAR	0.25	0.05	0.06	0.25	1	56.15
Pipe_-(104)_(P-Stm)_2	CIRCULAR	142.46	0.45	0.16	0.11	0.45	1
Pipe_-(110)_(P-Stm)	CIRCULAR		0.53	0.22	0.13	0.53	1 255.47
Pipe_-(111)_(P-Stm)	CIRCULAR		0.53	0.22	0.13	0.53	1 303.80
Pipe_-(71)_(P-Stm)	CIRCULAR		0.53	0.22	0.13	0.53	1 272.85
Pipe_-(72)_(P-Stm)	CIRCULAR		0.53	0.22	0.13	0.53	1 304.28

CB207	0.000	0.1	0.0	0.0	0.001	1.9	0	01:10
13.08								
CB208	0.000	0.0	0.0	0.0	0.000	0.9	0	01:10
2.80								
CB209	0.000	0.0	0.0	0.0	0.000	0.2	0	01:10
9.88								
CB210B	0.000	0.0	0.0	0.0	0.000	0.5	0	01:12
2.89								
CB211	0.000	0.0	0.0	0.0	0.000	0.1	0	01:10
2.03								
CB212	0.000	0.0	0.0	0.0	0.000	0.8	0	01:11
3.35								
CB212A	0.000	0.0	0.0	0.0	0.000	0.0	0	00:00
0.00								
CB213	0.000	0.0	0.0	0.0	0.000	0.3	0	01:19
1.47								
CB214	0.000	0.0	0.0	0.0	0.000	0.3	0	01:16
8.94								

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10 ⁶ ltr
518A_(P-Stm)	62.73	13.16	46.70	0.178
System	62.73	13.16	46.70	0.178

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	3.99	0 01:02	0.10	0.07	1.00
C4	CONDUIT	0.00	0 00:00	0.00	0.00	0.19
C5	CONDUIT	2.03	0 01:10	0.67	0.02	0.24
CB213	CONDUIT	1.47	0 01:19	0.11	0.02	0.56
CB214	CONDUIT	8.94	0 01:11	0.33	0.16	0.97
Pipe_-(104)_(P-Stm)_2	CONDUIT	46.70	0 01:18	0.94	0.33	0.35
Pipe_-(110)_(P-Stm)	CONDUIT	57.49	0 01:07	0.72	0.23	1.00
Pipe_-(111)_(P-Stm)	CONDUIT	31.05	0 01:11	0.50	0.10	1.00
Pipe_-(71)_(P-Stm)	CONDUIT	54.18	0 01:10	0.99	0.20	0.87
Pipe_-(72)_(P-Stm)	CONDUIT	50.98	0 01:06	0.97	0.17	1.00
OR1	ORIFICE	46.73	0 01:19			1.00
W1	WEIR	0.00	0 00:00			0.00
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	0.00	0 00:00			0.00
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	0.00	0 00:00			0.00
W6	WEIR	0.00	0 00:00			0.00
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	16.08	0 01:10			
CB201	DUMMY	13.84	0 01:10			
CB202	DUMMY	23.39	0 01:08			
CB203	DUMMY	3.64	0 01:08			
CB204	DUMMY	4.37	0 01:10			
CB205	DUMMY	3.33	0 01:08			
CB206	DUMMY	6.50	0 01:10			
CB207	DUMMY	11.33	0 01:10			
CB209	DUMMY	9.88	0 01:10			
CB210B	DUMMY	2.89	0 01:11			
CB212	DUMMY	3.35	0 01:11			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C2	1.00	0.12	0.03	0.00	0.13	0.00	0.00	0.72	0.04	0.00
C4	1.00	0.97	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00
C5	1.00	0.17	0.00	0.00	0.02	0.00	0.00	0.80	0.03	0.00
CB213	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.82	0.08	0.00
CB214	1.00	0.09	0.00	0.00	0.09	0.00	0.00	0.82	0.03	0.00
Pipe_-(104)_(P-Stm)_2	1.00	0.11	0.00	0.00	0.00	0.00	0.00	0.89	0.00	0.00
Pipe_-(110)_(P-Stm)	1.00	0.11	0.00	0.00	0.15	0.00	0.00	0.75	0.00	0.00
Pipe_-(111)_(P-Stm)	1.00	0.10	0.00	0.00	0.89	0.00	0.00	0.00	0.77	0.00
Pipe_-(71)_(P-Stm)	1.00	0.10	0.00	0.00	0.11	0.00	0.00	0.79	0.04	0.00
Pipe_-(72)_(P-Stm)	1.00	0.11	0.00	0.00	0.13	0.00	0.00	0.75	0.01	0.00

Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full Capacity	
	Both Ends	Upstream	Dnstream	Normal Flow	Limited
C2	0.35	0.35	0.38	0.01	0.01
CB213	0.01	0.01	0.32	0.01	0.01
CB214	0.01	0.01	0.32	0.01	0.01
Pipe_-(110)_(P-Stm)	0.53	0.53	0.62	0.01	0.01
Pipe_-(111)_(P-Stm)	0.32	0.32	0.53	0.01	0.01
Pipe_-(71)_(P-Stm)	0.01	0.01	0.23	0.01	0.01
Pipe_-(72)_(P-Stm)	0.30	0.30	0.46	0.01	0.01

Analysis begun on: Thu Aug 29 09:25:38 2024
Analysis ended on: Thu Aug 29 09:25:38 2024
Total elapsed time: < 1 sec

Post-Development 3-hour Chicago 1:100 year Event

```

[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option Value
FLOW_UNITS LPS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 01/01/2000
START_TIME 00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE 01/01/2000
END_TIME 06:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 1
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 12

[EVAPORATION]
;;Data Source Parameters
CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;;Name Format Interval SCF Source
3CHI100 INTENSITY 0:10 1.0 TIMESERIES 3CHI100

[SUBCATCHMENTS]
Rain Gage Outlet Area %Imperv Width %Slope CurbLen
;;Name
SnowPack

S1 3CHI100 CB208 0.0187 0 29.61 2.2 0
S10 3CHI100 CB204 0.0307 99.617 55.32 2.2 0
S11 3CHI100 CB202 0.127 90.325 65.66 2.2 0
S12 3CHI100 CB213 0.0149 0.369 6.44 2.2 0
S14_1 3CHI100 CB211 0.0116 52.676 22.78 2.2 0
S14_2 3CHI100 CB212 0.0179 31.654 58.12 2.2 0
S15 3CHI100 CB210B 0.0372 42.751 108.74 2.2 0
S2 3CHI100 CB209 0.0729 62.937 33.61 2.2 0
S3 3CHI100 CB207 0.0724 96.692 45.79 2.2 0
S32 3CHI100 CB214 0.052 14.286 29.7 2.2 0
S4 3CHI100 CB206 0.0464 99.975 24.67 2.2 0
S45 3CHI100 CB203 0.0222 99.962 14.32 2.2 0
S5 3CHI100 CB200 0.0843 89.589 82.96 2.2 0
S6 3CHI100 CB214 0.0239 0 40.3 2.2 0
S7 3CHI100 520_(P-Stm) 0.0686 100 25.95 2.2 0
S8 3CHI100 CB201 0.0941 80.117 35.06 2.2 0
S9 3CHI100 CB205 0.0275 76.83 19.08 2.2 0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
S1 0.013 0.25 1.57 4.67 0 OUTLET 0
S10 0.013 0.25 1.57 4.67 0 OUTLET 0
S11 0.013 0.25 1.57 4.67 0 OUTLET 0
S12 0.013 0.25 1.57 4.67 0 OUTLET 0
S14_1 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S14_2 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S15 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S2 0.013 0.25 1.57 4.67 0 OUTLET 0
S3 0.013 0.25 1.57 4.67 0 OUTLET 0
S32 0.013 0.25 1.57 4.67 0 PERVIOUS 80
S4 0.013 0.25 1.57 4.67 0 OUTLET 0
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S6 0.013 0.25 1.57 4.67 0 OUTLET 0
S7 0.013 0.25 1.57 4.67 0 OUTLET 0
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S9 0.013 0.25 1.57 4.67 0 OUTLET 0

[INFILTRATION]
;;Subcatchment Param1 Param2 Param3 Param4 Param5
S1 76.2 13.2 4.14 7 0
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S12 76.2 13.2 4.14 7 0
S14_1 76.2 13.2 4.14 7 0
S14_2 76.2 13.2 4.14 7 0
S15 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S32 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S45 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[JUNCTIONS]
;;Name Elevation MaxDepth InitDepth SurDepth Aponded
518_Orifice 81.361 4.106 0 0 0

[OUTFALLS]
;;Name Elevation Type Stage Data Gated Route To
;;Cylindrical Structure Slab Top Circular Frame SI
518A_(P-Stm) 80.646 FIXED 81.28 NO

[STORAGE]
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params
;;Depth Fevap Psi Ksat IMD
;;Cylindrical Structure Slab Top Circular Frame SI
518_(P-Stm) 81.061 4.406 0.219 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI
519_(P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI

520_(P-Stm) 82.487 2.888 0 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI
521_(P-Stm) 81.957 3.673 0 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI
522_(P-Stm) 82.182 3.323 0 FUNCTIONAL 0 0 1.13 0
;;85.1800000001676
CB200 83.18 2.32 0 TABULAR CB200 0
;;85.5000000001435
CB201 83.09 2.66 0 TABULAR CB201 0
;;85.36
CB202 82.76 2.89 0 TABULAR CB202 0
;;85.3500000000931
CB203 82.75 2.9 0 TABULAR CB203 0
;;85.3500080822731
CB204 82.75 2.75 0 TABULAR CB204 0
;;85.4500080823662
CB205 82.85 2.75 0 TABULAR CB205 0
;;85.48
CB206 82.77 2.73 0 TABULAR CB206 0
;;85.05
CB207 83.15 2.3 0 TABULAR CB207 0
;;84.9000080821652
CB208 83.3 2.04 0 TABULAR CB208 0
;;84.8
CB209 83.2 1.9 0 TABULAR CB209 0
;;85.25
CB210B 82.65 3 0 TABULAR CB210B 0
;;85.35
CB211 83.7 1.9 0 TABULAR CB211 0
;;85.4
CB212 83.2 2.42 0 TABULAR CB212 0
;;85.500000000554
CB212A 83.62 2 0 TABULAR CB212A 0
;;84.75
CB213 83.15 1.75 0 TABULAR CB213 0
;;84.55
CB214 82.95 1.9 0 TABULAR CB214 0

[CONDUITS]
From Node To Node Length Roughness InOffset OutOffset
InitFlow MaxFlow
;;
C2 0 CB208 CB207 9.666 0.013 83.3 83.2
C4 0 CB212A CB212 18.475 0.013 83.62 83.25
C5 0 CB211 CB212 22.346 0.013 83.7 83.25
CB213 0 CB213 522_(P-Stm) 29.34 0.013 83.15 82.757
CB214 0 CB214 522_(P-Stm) 21.65 0.013 82.95 82.757
;;PVC Pipes
Pipe_ (104)_ (P-Stm)_2 518_Orifice 518A_(P-Stm) 15.623 0.013 81.361 81.322
;;PVC Pipes
Pipe_ (110)_ (P-Stm) 521_(P-Stm) 518_(P-Stm) 27.209 0.013 82.257 82.161
;;PVC Pipes
Pipe_ (111)_ (P-Stm) 522_(P-Stm) 521_(P-Stm) 45.094 0.013 82.482 82.257
;;Concrete Pipes 100-D
Pipe_ (71)_ (P-Stm) 520_(P-Stm) 519_(P-Stm) 57.146 0.013 82.787 82.557
;;Concrete Pipes 100-D
Pipe_ (72)_ (P-Stm) 519_(P-Stm) 518_(P-Stm) 31.967 0.013 82.497 82.337

[ORIFICES]
From Node To Node Type Offset Qcoeff Gated
OR1 518_(P-Stm) 518_Orifice SIDE 81.361 0.65 NO

[WEIRS]
From Node To Node Type CrestHt Qcoeff Gated
EndCon EndCoeff EndSurch RoadWidth RoadSurf Coeff. Curve
W1 0 CB208 CB209 TRANSVERSE 85.31 1.84 NO
W2 0 CB210B CB211 TRANSVERSE 85.6 1.84 NO
W3 0 CB210B CB202 TRANSVERSE 85.45 1.84 NO
W4 0 CB202 CB204 TRANSVERSE 85.55 1.84 NO
W5 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO
W6 0 CB201 CB200 TRANSVERSE 85.65 1.84 NO
W7 0 CB200 NO TRANSVERSE 85.42 1.84 NO
W8 0 CB209 TRANSVERSE 85.42 1.84 NO

[OUTLETS]
From Node To Node Offset Type QTable/Qcoeff
Gexpon Gated
CB200 CB200 520_(P-Stm) 83.18 TABULAR/HEAD IPEX_Type_A
NO
CB201 CB201 520_(P-Stm) 83.09 TABULAR/HEAD Vortex_ICD_100
NO
CB202 CB202 521_(P-Stm) 82.76 TABULAR/HEAD IPEX_Type_A
NO
CB203 CB203 521_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_70
NO
CB204 CB204 522_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_65
NO
CB205 CB205 522_(P-Stm) 82.85 TABULAR/HEAD Vortex_ICD_65
NO
CB206 CB206 522_(P-Stm) 82.77 TABULAR/HEAD Vortex_ICD_70
NO
CB207 CB207 522_(P-Stm) 83.15 TABULAR/HEAD Vortex_ICD_100
NO
CB209 CB209 520_(P-Stm) 83.2 TABULAR/HEAD IPEX_Type_A
NO

```



```
;;-----
[SYMBOLS]
;;Gage      X-Coord      Y-Coord
;;-----
```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

```
*****
Element Count
*****
Number of rain gages ..... 1
Number of subcatchments ... 17
Number of nodes ..... 23
Number of links ..... 29
Number of pollutants ..... 0
Number of land uses ..... 0
```

```
*****
Raingage Summary
*****
Name          Data Source          Data Type          Recording Interval
-----
3CHI100       3CHI100              INTENSITY         10 min.
```

```
*****
Subcatchment Summary
*****
Name          Area      Width  %Imperv  %Slope Rain Gage          Outlet
-----
S1             0.02     29.61   0.00    2.2000 3CHI100             CB208
S10            0.03     55.32  99.62   2.2000 3CHI100             CB204
S11            0.13     65.66  90.33   2.2000 3CHI100             CB202
S12            0.01     6.44   0.37    2.2000 3CHI100             CB213
S14_1          0.01     22.78  52.68   2.2000 3CHI100             CB211
S14_2          0.02     58.12  31.65   2.2000 3CHI100             CB212
S15            0.04     108.74 42.75   2.2000 3CHI100             CB210B
S2             0.07     33.61  62.94   2.2000 3CHI100             CB209
S3             0.07     45.79  96.69   2.2000 3CHI100             CB207
S32            0.05     29.70  14.29   2.2000 3CHI100             CB214
S4             0.05     24.67  99.97   2.2000 3CHI100             CB206
S45            0.02     14.32  99.96   2.2000 3CHI100             CB203
S5             0.08     82.96  89.59   2.2000 3CHI100             CB200
S6             0.02     40.30  0.00    2.2000 3CHI100             CB214
S7             0.07     25.95  100.00  2.2000 3CHI100             520_(P-
Stm)
S8             0.09     35.06  80.12   2.2000 3CHI100             CB201
S9             0.03     19.08  76.83   2.2000 3CHI100             CB205
```

```
*****
Node Summary
*****
Name          Type          Invert Elev.  Max. Depth  Ponded Area  External Inflow
-----
518_Orifice   JUNCTION      81.36         4.11        0.0           0.0
518A_(P-Stm)  OUTFALL       80.65         1.13        0.0           0.0
518_(P-Stm)   STORAGE       81.06         4.41        0.0           0.0
519_(P-Stm)   STORAGE       82.20         3.51        0.0           0.0
520_(P-Stm)   STORAGE       82.49         2.89        0.0           0.0
521_(P-Stm)   STORAGE       81.96         3.67        0.0           0.0
522_(P-Stm)   STORAGE       82.18         3.32        0.0           0.0
CB200         STORAGE       83.18         2.32        0.0           0.0
CB201         STORAGE       83.09         2.66        0.0           0.0
CB202         STORAGE       82.76         2.89        0.0           0.0
CB203         STORAGE       82.75         2.90        0.0           0.0
CB204         STORAGE       82.75         2.75        0.0           0.0
CB205         STORAGE       82.85         2.75        0.0           0.0
CB206         STORAGE       82.77         2.73        0.0           0.0
CB207         STORAGE       83.15         2.30        0.0           0.0
CB208         STORAGE       83.30         2.04        0.0           0.0
CB209         STORAGE       83.20         1.90        0.0           0.0
CB210B        STORAGE       82.65         3.00        0.0           0.0
CB211         STORAGE       83.70         1.90        0.0           0.0
CB212         STORAGE       83.20         2.42        0.0           0.0
CB212A        STORAGE       83.62         2.00        0.0           0.0
CB213         STORAGE       83.15         1.75        0.0           0.0
CB214         STORAGE       82.95         1.90        0.0           0.0
```

```
*****
Link Summary
*****
Name          From Node      To Node      Type          Length  %Slope
-----
C2            CB208          CB207        CONDUIT       9.7     1.0346
C4            CB212A         CB212        CONDUIT       18.5    2.0031
C5            CB211          CB212        CONDUIT       22.3    2.0142
CB213         CB213          522_(P-Stm)  CONDUIT       29.3    1.3396
CB214         CB214          522_(P-Stm)  CONDUIT       21.6    0.8915
Pipe_-(104)_(P-Stm)_2 518_Orifice   518A_(P-Stm)  CONDUIT       15.6    0.2496
Pipe_-(110)_(P-Stm)  521_(P-Stm)   518_(P-Stm)   CONDUIT       27.2    0.3528
Pipe_-(111)_(P-Stm)  522_(P-Stm)   521_(P-Stm)   CONDUIT       45.1    0.4990
Pipe_-(71)_(P-Stm)  520_(P-Stm)   519_(P-Stm)   CONDUIT       57.1    0.4025
Pipe_-(72)_(P-Stm)  519_(P-Stm)   518_(P-Stm)   CONDUIT       32.0    0.5005
OR1          518_(P-Stm)   518_Orifice   ORIFICE
W1           CB208         CB209         WEIR
W2           CB210B        CB211         WEIR
W3           CB210B        CB202         WEIR
W4           CB202         CB204         WEIR
W5           CB206         CB207         WEIR
W6           CB201         CB200         WEIR
W7           CB200         CB209         WEIR
CB200        CB200         520_(P-Stm)   OUTLET
CB201        CB201         520_(P-Stm)   OUTLET
CB202        CB202         521_(P-Stm)   OUTLET
CB203        CB203         521_(P-Stm)   OUTLET
CB204        CB204         522_(P-Stm)   OUTLET
CB205        CB205         522_(P-Stm)   OUTLET
CB206        CB206         522_(P-Stm)   OUTLET
Cb207        CB207         522_(P-Stm)   OUTLET
CB209        CB209         520_(P-Stm)   OUTLET
CB210B       CB210B        518_(P-Stm)   OUTLET
CB212        CB212         521_(P-Stm)   OUTLET
```

```
*****
Cross Section Summary
*****
Conduit      Shape          Full Depth  Full Area  Hyd. Rad.  Max. Width  No. of Barrels  Full Flow
-----
C2           CIRCULAR       0.25       0.05      0.06       0.25        1        60.49
C4           CIRCULAR       0.25       0.05      0.06       0.25        1        84.17
C5           CIRCULAR       0.25       0.05      0.06       0.25        1        84.40
CB213        CIRCULAR       0.25       0.05      0.06       0.25        1        68.83
CB214        CIRCULAR       0.25       0.05      0.06       0.25        1        56.15
Pipe_-(104)_(P-Stm)_2  CIRCULAR     0.45       0.16      0.11       0.45        1        1
Pipe_-(110)_(P-Stm)  CIRCULAR     0.53       0.22      0.13       0.53        1        255.47
Pipe_-(111)_(P-Stm)  CIRCULAR     0.53       0.22      0.13       0.53        1        303.80
Pipe_-(71)_(P-Stm)  CIRCULAR     0.53       0.22      0.13       0.53        1        272.85
Pipe_-(72)_(P-Stm)  CIRCULAR     0.53       0.22      0.13       0.53        1        304.28
```

Analysis Options
Flow Units LPS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed NO
Water Quality NO
Infiltration Method HORTON
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 01/01/2000 00:00:00
Ending Date 01/01/2000 06:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 1.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001500 m

Runoff Quantity Continuity
Volume hectare-m Depth mm
Total Precipitation 0.059 71.708
Evaporation Loss 0.000 0.000
Infiltration Loss 0.010 11.940
Surface Runoff 0.048 58.708
Final Storage 0.001 1.158
Continuity Error (%) -0.137

Flow Routing Continuity
Volume hectare-m Volume 10^6 ltr
Dry Weather Inflow 0.000 0.000
Wet Weather Inflow 0.048 0.483
Groundwater Inflow 0.000 0.000
RDII Inflow 0.000 0.000
External Inflow 0.000 0.000
External Outflow 0.048 0.482
Flooding Loss 0.000 0.000
Evaporation Loss 0.000 0.000
Exfiltration Loss 0.000 0.000
Initial Stored Volume 0.000 0.000
Final Stored Volume 0.000 0.002
Continuity Error (%) -0.048

Time-Step Critical Elements
None

Highest Flow Instability Indexes
Link W3 (5)

Most Frequent Nonconverging Nodes
Convergence obtained at all time steps.

Routing Time Step Summary
Minimum Time Step : 0.50 sec
Average Time Step : 1.00 sec
Maximum Time Step : 1.00 sec
% of Time in Steady State : 0.00
Average Iterations per Step : 2.00
% of Steps Not Converging : 0.00
Time Step Frequencies
1.000 - 0.871 sec : 100.00 %
0.871 - 0.758 sec : 0.00 %
0.758 - 0.660 sec : 0.00 %
0.660 - 0.574 sec : 0.00 %
0.574 - 0.500 sec : 0.00 %

Subcatchment Runoff Summary

Table with columns: Total Runoff, Total Runoff, Peak Runoff, Total Runoff Precip Coeff, Total Runoff, Total Evap, Total Infil, Imperv Runoff, Perv Runoff. Rows include subcatchments S1 through S9 and various nodes like S14_1, S14_2, S5, S6, S7, S8, S9.

Node Depth Summary

Summary table with columns: Node, Type, Average Depth Meters, Maximum Depth Meters, Maximum HGL Meters, Time of Max Occurrence days hr:min, Reported Max Depth Meters.

Table showing node inflow summary for nodes 518_Orifice through CB214, including Junction, Outfall, and Storage types with flow volumes and times of max.

Node Inflow Summary

Table with columns: Flow, Balance, Error Node Percent, Maximum Inflow, Maximum Inflow, Time of Max Occurrence, Lateral Inflow, Total Inflow. Lists nodes like 518_Orifice, 518A(P-Stm), 519(P-Stm), etc.

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Table with columns: Maximum, Average, Avg, Evap, Exfil, Maximum, Max, Time of Max, Occurrence. Lists storage nodes like 518(P-Stm), 519(P-Stm), 520(P-Stm), etc.

Post-Development 3-hour Chicago 1:100-year Event

July 2024

CB210B	0.001	5.3	0.0	0.0	0.007	45.6	0	01:20
8.84								
CB211	0.000	0.5	0.0	0.0	0.000	3.0	0	01:36
5.42								
CB212	0.000	1.7	0.0	0.0	0.001	8.8	0	01:35
10.13								
CB212A	0.000	10.8	0.0	0.0	0.000	63.0	0	01:36
1.31								
CB213	0.000	4.8	0.0	0.0	0.000	52.4	0	01:36
7.61								
CB214	0.000	8.4	0.0	0.0	0.005	99.8	0	01:37
25.84								

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10 ⁶ ltr
518A_(P-Stm)	68.00	32.78	66.14	0.482
System	68.00	32.78	66.14	0.482

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	25.25	0 01:02	0.51	0.42	1.00
C4	CONDUIT	5.35	0 01:09	0.15	0.06	1.00
C5	CONDUIT	5.42	0 01:06	0.66	0.06	1.00
CB213	CONDUIT	19.74	0 01:06	0.38	0.27	1.00
CB214	CONDUIT	25.84	0 01:09	0.53	0.46	1.00
Pipe_-(104)_(P-Stm)_2	CONDUIT	66.14	0 01:37	1.05	0.46	0.42
Pipe_-(110)_(P-Stm)	CONDUIT	65.44	0 01:05	0.73	0.26	1.00
Pipe_-(111)_(P-Stm)	CONDUIT	40.12	0 01:05	0.49	0.13	1.00
Pipe_-(71)_(P-Stm)	CONDUIT	92.08	0 01:03	1.03	0.34	1.00
Pipe_-(72)_(P-Stm)	CONDUIT	66.89	0 01:06	0.88	0.22	1.00
OR1	ORIFICE	66.14	0 01:37			1.00
W1	WEIR	5.83	0 01:21			0.55
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	16.84	0 01:08			0.30
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	17.54	0 01:10			0.32
W6	WEIR	10.38	0 01:11			0.10
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	26.00	0 01:05			
CB201	DUMMY	14.07	0 01:04			
CB202	DUMMY	29.10	0 01:02			
CB203	DUMMY	5.69	0 01:07			
CB204	DUMMY	5.50	0 01:03			
CB205	DUMMY	5.15	0 01:06			
CB206	DUMMY	6.87	0 01:02			
CB207	DUMMY	12.67	0 01:04			
CB209	DUMMY	22.99	0 01:05			
CB210B	DUMMY	5.18	0 01:05			
CB212	DUMMY	5.89	0 01:05			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class						Norm Ltd	Inlet Ctrl	
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit			
C2	1.00	0.07	0.07	0.00	0.32	0.00	0.00	0.54	0.01	0.00
C4	1.00	0.17	0.01	0.00	0.28	0.00	0.00	0.54	0.03	0.00
C5	1.00	0.15	0.00	0.00	0.29	0.00	0.00	0.56	0.05	0.00
CB213	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.05	0.00
CB214	1.00	0.05	0.00	0.00	0.32	0.00	0.00	0.63	0.03	0.00
Pipe_-(104)_(P-Stm)_2	1.00	0.07	0.00	0.00	0.00	0.00	0.00	0.93	0.00	0.00
Pipe_-(110)_(P-Stm)	1.00	0.06	0.00	0.00	0.38	0.00	0.00	0.56	0.00	0.00
Pipe_-(111)_(P-Stm)	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.58	0.00
Pipe_-(71)_(P-Stm)	1.00	0.06	0.00	0.00	0.34	0.00	0.00	0.60	0.03	0.00
Pipe_-(72)_(P-Stm)	1.00	0.07	0.00	0.00	0.37	0.00	0.00	0.56	0.02	0.00

Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full Normal Flow	Hours Capacity Limited
	Both Ends	Upstream	Dnstream		
C2	1.79	1.79	1.83	0.01	0.01
C4	1.31	1.31	1.55	0.01	0.01
C5	1.25	1.25	1.57	0.01	0.01
CB213	1.52	1.52	1.76	0.01	0.01
CB214	1.65	1.65	1.76	0.01	0.01
Pipe_-(110)_(P-Stm)	1.92	1.92	1.99	0.01	0.01
Pipe_-(111)_(P-Stm)	1.76	1.76	1.92	0.01	0.01
Pipe_-(71)_(P-Stm)	1.58	1.58	1.71	0.01	0.01
Pipe_-(72)_(P-Stm)	1.75	1.75	1.86	0.01	0.01

Analysis begun on: Thu Aug 29 09:25:36 2024
Analysis ended on: Thu Aug 29 09:25:36 2024
Total elapsed time: < 1 sec

Post-Development 24-hour SCS 1:100-year Event

```

[TITLE]
;;Project Title/Notes

[OPTIONS]
;;Option Value
FLOW_UNITS LPS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 01/01/2000
START_TIME 00:00:00
REPORT_START_DATE 01/01/2000
REPORT_START_TIME 00:00:00
END_DATE 01/02/2000
END_TIME 00:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 1
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 12

[EVAPORATION]
;;Data Source Parameters
CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;;Name Format Interval SCF Source
24SCS100 INTENSITY 0:15 1.0 TIMESERIES 24SCS100
3CHI1100 INTENSITY 0:10 1.0 TIMESERIES 3CHI1100

[SUBCATCHMENTS]
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen
SnowPack
S1 24SCS100 CB208 0.0187 0 29.61 2.2 0
S10 24SCS100 CB204 0.0307 99.617 55.32 2.2 0
S11 24SCS100 CB202 0.127 90.325 65.66 2.2 0
S12 24SCS100 CB213 0.0149 0.369 6.44 2.2 0
S14_1 24SCS100 CB211 0.0116 52.676 22.78 2.2 0
S14_2 24SCS100 CB212 0.0179 31.654 58.12 2.2 0
S15 24SCS100 CB210B 0.0372 42.751 108.74 2.2 0
S2 24SCS100 CB209 0.0729 62.937 33.61 2.2 0
S3 24SCS100 CB207 0.0724 96.692 45.79 2.2 0
S32 24SCS100 CB214 0.052 14.286 29.7 2.2 0
S4 24SCS100 CB206 0.0464 99.975 24.67 2.2 0
S45 24SCS100 CB203 0.0222 99.962 14.32 2.2 0
S5 24SCS100 CB200 0.0843 89.589 82.96 2.2 0
S6 24SCS100 CB214 0.0239 0 40.3 2.2 0
S7 24SCS100 520_(P-Stm) 0.0686 100 25.95 2.2 0
S8 24SCS100 CB201 0.0941 80.117 35.06 2.2 0
S9 24SCS100 CB205 0.0275 76.83 19.08 2.2 0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perov S-Imperv S-Perov PctZero RouteTo PctRouted
S1 0.013 0.25 1.57 4.67 0 OUTLET
S10 0.013 0.25 1.57 4.67 0 OUTLET
S11 0.013 0.25 1.57 4.67 0 OUTLET
S12 0.013 0.25 1.57 4.67 0 OUTLET
S14_1 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S14_2 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S15 0.013 0.25 1.57 4.67 0 PERVIOUS 100
S2 0.013 0.25 1.57 4.67 0 OUTLET
S3 0.013 0.25 1.57 4.67 0 OUTLET
S32 0.013 0.25 1.57 4.67 0 PERVIOUS 80
S4 0.013 0.25 1.57 4.67 0 OUTLET
S45 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S5 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S6 0.013 0.25 1.57 4.67 0 OUTLET
S7 0.013 0.25 1.57 4.67 0 OUTLET
S8 0.013 0.25 1.57 4.67 0 IMPERVIOUS 100
S9 0.013 0.25 1.57 4.67 0 OUTLET

[INFILTRATION]
;;Subcatchment Param1 Param2 Param3 Param4 Param5
S1 76.2 13.2 4.14 7 0
S10 76.2 13.2 4.14 7 0
S11 76.2 13.2 4.14 7 0
S12 76.2 13.2 4.14 7 0
S14_1 76.2 13.2 4.14 7 0
S14_2 76.2 13.2 4.14 7 0
S15 76.2 13.2 4.14 7 0
S2 76.2 13.2 4.14 7 0
S3 76.2 13.2 4.14 7 0
S32 76.2 13.2 4.14 7 0
S4 76.2 13.2 4.14 7 0
S45 76.2 13.2 4.14 7 0
S5 76.2 13.2 4.14 7 0
S6 76.2 13.2 4.14 7 0
S7 76.2 13.2 4.14 7 0
S8 76.2 13.2 4.14 7 0
S9 76.2 13.2 4.14 7 0

[JUNCTIONS]
;;Name Elevation MaxDepth InitDepth SurDepth Aponded
518_Orifice 81.361 4.106 0 0 0

[OUTFALLS]
;;Name Elevation Type Stage Data Gated Route To
;;Cylindrical Structure Slab Top Circular Frame SI
518A_(P-Stm) 80.646 FIXED 81.27 NO

[STORAGE]
;;Name Elev. MaxDepth InitDepth Shape Curve Name/Params
SurDepth Fevap Psi Ksat IMD
;;Cylindrical Structure Slab Top Circular Frame SI
518_(P-Stm) 81.061 4.406 0.209 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI
519_(P-Stm) 82.197 3.512 0 FUNCTIONAL 0 0 1.13 0
;;Cylindrical Structure Slab Top Circular Frame SI

[CONDUITS]
;;Name From Node To Node Length Roughness InOffset OutOffset
InitFlow MaxFlow
C2 0 CB208 CB207 9.666 0.013 83.3 83.2
C4 0 CB212A CB212 18.475 0.013 83.62 83.25
C5 0 CB211 CB212 22.346 0.013 83.7 83.25
CB213 0 CB213 522_(P-Stm) 29.34 0.013 83.15 82.757
CB214 0 CB214 522_(P-Stm) 21.65 0.013 82.95 82.757
PVC Pipes
Pipe_-(104)_(P-Stm)_2 518_Orifice 518A_(P-Stm) 15.623 0.013 81.361 81.322
PVC Pipes
Pipe_-(110)_(P-Stm) 521_(P-Stm) 518_(P-Stm) 27.209 0.013 82.257 82.161
PVC Pipes
Pipe_-(111)_(P-Stm) 522_(P-Stm) 521_(P-Stm) 45.094 0.013 82.482 82.257
Concrete Pipes 100-D
Pipe_-(71)_(P-Stm) 520_(P-Stm) 519_(P-Stm) 57.146 0.013 82.787 82.557
Concrete Pipes 100-D
Pipe_-(72)_(P-Stm) 519_(P-Stm) 518_(P-Stm) 31.967 0.013 82.497 82.337

[ORIFICES]
;;Name From Node To Node Type Offset Qcoeff Gated
CloseTime
OR1 518_(P-Stm) 518_Orifice SIDE 81.361 0.65 NO

[WEIRS]
;;Name From Node To Node Type CrestHt Qcoeff Gated
EndCon EndCoeff SurchARGE RoadWidth RoadSurf Coeff. Curve
W1 0 CB208 CB209 TRANSVERSE 85.31 1.84 NO
W2 0 CB210B CB211 TRANSVERSE 85.6 1.84 NO
W3 0 CB210B CB202 TRANSVERSE 85.45 1.84 NO
W4 0 CB202 CB204 TRANSVERSE 85.55 1.84 NO
W5 0 CB206 CB207 TRANSVERSE 85.45 1.84 NO
W6 0 CB201 CB200 TRANSVERSE 85.65 1.84 NO
W7 0 CB200 CB209 TRANSVERSE 85.42 1.84 NO

[OUTLETS]
;;Name From Node To Node Offset Type QTable/Qcoeff
Qexpon Gated
CB200 CB200 520_(P-Stm) 83.18 TABULAR/HEAD IPEX_Type_A
NO
CB201 CB201 520_(P-Stm) 83.09 TABULAR/HEAD Vortex_ICD_100
NO
CB202 CB202 521_(P-Stm) 82.76 TABULAR/HEAD IPEX_Type_A
NO
CB203 CB203 521_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_70
NO
CB204 CB204 522_(P-Stm) 82.75 TABULAR/HEAD Vortex_ICD_65
NO
CB205 CB205 522_(P-Stm) 82.85 TABULAR/HEAD Vortex_ICD_65
NO
CB206 CB206 522_(P-Stm) 82.77 TABULAR/HEAD Vortex_ICD_70
NO
CB207 CB207 522_(P-Stm) 83.15 TABULAR/HEAD Vortex_ICD_100
NO
CB209 CB209 520_(P-Stm) 83.2 TABULAR/HEAD IPEX_Type_A
NO
CB210B CB210B 518_(P-Stm) 82.65 TABULAR/HEAD Vortex_ICD_65
NO
CB212 CB212 521_(P-Stm) 83.2 TABULAR/HEAD Vortex_ICD_100

[XSECTIONS]
;;Link Shape Geom1 Geom2 Geom3 Geom4 Barrels
Culvert

```



```
;;-----
C2          CIRCULAR      0.25      0      0      0      1
C4          CIRCULAR      0.25      0      0      0      1
C5          CIRCULAR      0.25      0      0      0      1
CB213      CIRCULAR      0.25      0      0      0      1
CB214      CIRCULAR      0.25      0      0      0      1
Pipe_--(104)_(P-Stm)_2_CIRCULAR 0.45  0      0      0      1
Pipe_--(110)_(P-Stm)_2_CIRCULAR 0.525 0      0      0      1
Pipe_--(111)_(P-Stm)_2_CIRCULAR 0.525 0      0      0      1
Pipe_--(71)_(P-Stm)_2_CIRCULAR 0.525 0      0      0      1
Pipe_--(72)_(P-Stm)_2_CIRCULAR 0.525 0      0      0      1
OR1        CIRCULAR      0.127   0      0      0      0
W1         RECT_OPEN     0.03    1.5    0      0      0
W2         RECT_OPEN     0.05    2.59   0      0      0
W3         RECT_OPEN     0.2     1.34   0      0      0
W4         RECT_OPEN     0.1     4.9    0      0      0
W5         RECT_OPEN     0.05    4.725  0      0      0
W6         RECT_OPEN     0.1     6      0      0      0
W7         RECT_OPEN     0.08    10.5   0      0      0

[LOSSES]
;;Link      Kentry      Kexit      Kavg      Flap Gate      Seepage
;;-----
[CURVES]
;;Name      Type      X-Value      Y-Value
;;-----
CB210-OUT85.5 Rating 0 0
CB210-OUT85.5 Rating 85.5 0.04
CB210-OUT85.5 Rating 100 0.04
CB210-OUT-85.6 Rating 0 0
CB210-OUT-85.6 Rating 85.6 0.08
CB210-OUT-85.6 Rating 100 0.08

;IPEX Type A ICD Rating Curve
IPEX_Type_A Rating 0 0
IPEX_Type_A Rating 0.1 5.7
IPEX_Type_A Rating 0.2 8.1
IPEX_Type_A Rating 0.3 9.9
IPEX_Type_A Rating 0.4 11.4
IPEX_Type_A Rating 0.5 12.8
IPEX_Type_A Rating 0.6 14
IPEX_Type_A Rating 0.7 15.1
IPEX_Type_A Rating 0.8 16.2
IPEX_Type_A Rating 0.9 17.2
IPEX_Type_A Rating 1 18.1
IPEX_Type_A Rating 1.2 19.8
IPEX_Type_A Rating 1.4 21.4
IPEX_Type_A Rating 1.6 22.9
IPEX_Type_A Rating 1.8 24.3
IPEX_Type_A Rating 2 25.6
IPEX_Type_A Rating 2.5 28.6
IPEX_Type_A Rating 3 31.3

;IPEX Type B ICD Rating Curve
IPEX_Type_B Rating 0 0
IPEX_Type_B Rating 0.1 8.1
IPEX_Type_B Rating 0.2 11.5
IPEX_Type_B Rating 0.3 14.1
IPEX_Type_B Rating 0.4 16.2
IPEX_Type_B Rating 0.5 18.2
IPEX_Type_B Rating 0.6 19.9
IPEX_Type_B Rating 0.7 21.5
IPEX_Type_B Rating 0.8 23
IPEX_Type_B Rating 0.9 24.4
IPEX_Type_B Rating 1 25.7
IPEX_Type_B Rating 1.2 28.1
IPEX_Type_B Rating 1.4 30.4
IPEX_Type_B Rating 1.6 32.5
IPEX_Type_B Rating 1.8 34.4
IPEX_Type_B Rating 2 36.3
IPEX_Type_B Rating 2.5 40.6
IPEX_Type_B Rating 3 44.5

;IPEX Type C ICD Rating Curve
IPEX_Type_C Rating 0 0
IPEX_Type_C Rating 0.1 10.6
IPEX_Type_C Rating 0.2 15
IPEX_Type_C Rating 0.3 18.3
IPEX_Type_C Rating 0.4 21.2
IPEX_Type_C Rating 0.5 23.7
IPEX_Type_C Rating 0.6 25.9
IPEX_Type_C Rating 0.7 28
IPEX_Type_C Rating 0.8 29.9
IPEX_Type_C Rating 0.9 31.7
IPEX_Type_C Rating 1 33.5
IPEX_Type_C Rating 1.2 36.6
IPEX_Type_C Rating 1.4 39.6
IPEX_Type_C Rating 1.6 42.3
IPEX_Type_C Rating 1.8 44.9
IPEX_Type_C Rating 2 47.3
IPEX_Type_C Rating 2.5 52.9
IPEX_Type_C Rating 3 57.9

;IPEX Type AA ICD Rating Curve
IPEX_Type_D Rating 0 0
IPEX_Type_D Rating 0.1 15.4
IPEX_Type_D Rating 0.2 21.7
IPEX_Type_D Rating 0.3 26.6
IPEX_Type_D Rating 0.4 30.7
IPEX_Type_D Rating 0.5 34.3
IPEX_Type_D Rating 0.6 37.6
IPEX_Type_D Rating 0.7 40.6
IPEX_Type_D Rating 0.8 43.4
IPEX_Type_D Rating 0.9 46.1
IPEX_Type_D Rating 1 48.5
IPEX_Type_D Rating 1.2 53.2
IPEX_Type_D Rating 1.4 57.4
IPEX_Type_D Rating 1.6 61.4
IPEX_Type_D Rating 1.8 65.1
IPEX_Type_D Rating 2 68.7
IPEX_Type_D Rating 2.5 76.8
IPEX_Type_D Rating 3 84.1

;IPEX Type E ICD Rating Curve
IPEX_Type_E Rating 0 0
IPEX_Type_E Rating 0.1 20.5
IPEX_Type_E Rating 0.2 28.9
IPEX_Type_E Rating 0.3 35.5
IPEX_Type_E Rating 0.4 40.9
IPEX_Type_E Rating 0.5 45.8
IPEX_Type_E Rating 0.6 50.1
IPEX_Type_E Rating 0.7 54.2
IPEX_Type_E Rating 0.8 57.9
IPEX_Type_E Rating 0.9 61.4
IPEX_Type_E Rating 1 64.7
IPEX_Type_E Rating 1.2 70.9
IPEX_Type_E Rating 1.4 76.6
IPEX_Type_E Rating 1.6 81.9
IPEX_Type_E Rating 1.8 86.8
IPEX_Type_E Rating 2 91.5
IPEX_Type_E Rating 2.5 102.3
IPEX_Type_E Rating 3 112.1

;Tempest Rating Curve for Vortex ICD 100, No grate allowance
Vortex_ICD_100 Rating 0 0
Vortex_ICD_100 Rating 0.1 2.8
Vortex_ICD_100 Rating 0.2 4
Vortex_ICD_100 Rating 0.3 4.9
Vortex_ICD_100 Rating 0.4 5.6
Vortex_ICD_100 Rating 0.5 6.3
Vortex_ICD_100 Rating 0.6 6.9
Vortex_ICD_100 Rating 0.7 7.5
Vortex_ICD_100 Rating 0.8 8
Vortex_ICD_100 Rating 0.9 8.5
Vortex_ICD_100 Rating 1 8.9
Vortex_ICD_100 Rating 1.2 9.8

;Tempest Rating Curve for Vortex ICD 105, No grate allowance
Vortex_ICD_105 Rating 0 0
Vortex_ICD_105 Rating 0.1 3.1
Vortex_ICD_105 Rating 0.2 3.4
Vortex_ICD_105 Rating 0.3 3.7
Vortex_ICD_105 Rating 0.4 4
Vortex_ICD_105 Rating 0.5 4.2
Vortex_ICD_105 Rating 0.6 4.4
Vortex_ICD_105 Rating 0.7 4.6
Vortex_ICD_105 Rating 0.8 4.8
Vortex_ICD_105 Rating 0.9 5
Vortex_ICD_105 Rating 1 5.2
Vortex_ICD_105 Rating 1.2 5.6

;Tempest Rating Curve for Vortex ICD 40, No grate allowance
Vortex_ICD_40 Rating 0 0
Vortex_ICD_40 Rating 0.1 0.4
Vortex_ICD_40 Rating 0.2 0.6
Vortex_ICD_40 Rating 0.3 0.7
Vortex_ICD_40 Rating 0.4 0.9
Vortex_ICD_40 Rating 0.5 1
Vortex_ICD_40 Rating 0.6 1
Vortex_ICD_40 Rating 0.7 1.1
Vortex_ICD_40 Rating 0.8 1.2
Vortex_ICD_40 Rating 0.9 1.3
Vortex_ICD_40 Rating 1 1.4
Vortex_ICD_40 Rating 1.2 1.5
Vortex_ICD_40 Rating 1.4 1.6
Vortex_ICD_40 Rating 1.6 1.7
Vortex_ICD_40 Rating 1.8 1.8
Vortex_ICD_40 Rating 2 1.9
Vortex_ICD_40 Rating 2.5 2.2
Vortex_ICD_40 Rating 3 2.4

;Tempest Rating Curve for Vortex ICD 45, No grate allowance
Vortex_ICD_45 Rating 0 0
Vortex_ICD_45 Rating 0.1 0.6
Vortex_ICD_45 Rating 0.2 0.8
Vortex_ICD_45 Rating 0.3 1
Vortex_ICD_45 Rating 0.4 1.1
Vortex_ICD_45 Rating 0.5 1.3
Vortex_ICD_45 Rating 0.6 1.4
Vortex_ICD_45 Rating 0.7 1.5
Vortex_ICD_45 Rating 0.8 1.6
Vortex_ICD_45 Rating 0.9 1.7
Vortex_ICD_45 Rating 1 1.8
Vortex_ICD_45 Rating 1.2 2
Vortex_ICD_45 Rating 1.4 2.1
Vortex_ICD_45 Rating 1.6 2.3
Vortex_ICD_45 Rating 1.8 2.4
Vortex_ICD_45 Rating 2 2.6
Vortex_ICD_45 Rating 2.5 2.9
Vortex_ICD_45 Rating 3 3.1

;Tempest Rating Curve for Vortex ICD 50, No grate allowance
Vortex_ICD_50 Rating 0 0
Vortex_ICD_50 Rating 0.1 0.7
Vortex_ICD_50 Rating 0.2 1
Vortex_ICD_50 Rating 0.3 1.2
Vortex_ICD_50 Rating 0.4 1.4
Vortex_ICD_50 Rating 0.5 1.6
Vortex_ICD_50 Rating 0.6 1.8
Vortex_ICD_50 Rating 0.7 1.9
Vortex_ICD_50 Rating 0.8 2
Vortex_ICD_50 Rating 0.9 2.1
Vortex_ICD_50 Rating 1 2.3
Vortex_ICD_50 Rating 1.2 2.5
Vortex_ICD_50 Rating 1.4 2.7
Vortex_ICD_50 Rating 1.6 2.9
Vortex_ICD_50 Rating 1.8 3
Vortex_ICD_50 Rating 2 3.2
Vortex_ICD_50 Rating 2.5 3.6
Vortex_ICD_50 Rating 3 3.9

;Tempest Rating Curve for Vortex ICD 55, No grate allowance
Vortex_ICD_55 Rating 0 0
Vortex_ICD_55 Rating 0.1 0.9
Vortex_ICD_55 Rating 0.2 1.2
Vortex_ICD_55 Rating 0.3 1.5
Vortex_ICD_55 Rating 0.4 1.7
Vortex_ICD_55 Rating 0.5 1.9
Vortex_ICD_55 Rating 0.6 2.1
Vortex_ICD_55 Rating 0.7 2.3
Vortex_ICD_55 Rating 0.8 2.4
Vortex_ICD_55 Rating 0.9 2.6
Vortex_ICD_55 Rating 1 2.7
Vortex_ICD_55 Rating 1.2 3
Vortex_ICD_55 Rating 1.4 3.2
Vortex_ICD_55 Rating 1.6 3.4
Vortex_ICD_55 Rating 1.8 3.6
Vortex_ICD_55 Rating 2 3.8
Vortex_ICD_55 Rating 2.5 4.3
Vortex_ICD_55 Rating 3 4.7

;Tempest Rating Curve for Vortex ICD 60, No grate allowance
Vortex_ICD_60 Rating 0 0
Vortex_ICD_60 Rating 0.1 1.1
Vortex_ICD_60 Rating 0.2 1.5
Vortex_ICD_60 Rating 0.3 1.8
Vortex_ICD_60 Rating 0.4 2.1
Vortex_ICD_60 Rating 0.5 2.3
Vortex_ICD_60 Rating 0.6 2.5
Vortex_ICD_60 Rating 0.7 2.7
Vortex_ICD_60 Rating 0.8 2.9
Vortex_ICD_60 Rating 0.9 3.1
Vortex_ICD_60 Rating 1 3.2
Vortex_ICD_60 Rating 1.2 3.6
Vortex_ICD_60 Rating 1.4 3.8
Vortex_ICD_60 Rating 1.6 4
Vortex_ICD_60 Rating 1.8 4.3
Vortex_ICD_60 Rating 2 4.6
Vortex_ICD_60 Rating 2.5 5.1
Vortex_ICD_60 Rating 3 5.6

;Tempest Rating Curve for Vortex ICD 65, No grate allowance
Vortex_ICD_65 Rating 0 0
Vortex_ICD_65 Rating 0.1 1.2
Vortex_ICD_65 Rating 0.2 1.6
Vortex_ICD_65 Rating 0.3 2
Vortex_ICD_65 Rating 0.4 2.3
Vortex_ICD_65 Rating 0.5 2.5
Vortex_ICD_65 Rating 0.6 2.8
Vortex_ICD_65 Rating 0.7 3
Vortex_ICD_65 Rating 0.8 3.2
Vortex_ICD_65 Rating 0.9 3.4
Vortex_ICD_65 Rating 1 3.6
Vortex_ICD_65 Rating 1.2 4
Vortex_ICD_65 Rating 1.4 4.3
Vortex_ICD_65 Rating 1.6 4.6
Vortex_ICD_65 Rating 1.8 4.9
Vortex_ICD_65 Rating 2 5.1
Vortex_ICD_65 Rating 2.5 5.7
Vortex_ICD_65 Rating 3 6.3

;Tempest Rating Curve for Vortex ICD 70, No grate allowance
Vortex_ICD_70 Rating 0 0
```


Post-Development 24-hour SCS 1:100-year Event

August 2024

CB210B	0.000	0.6	0.0	0.0	0.005	35.5	0	12:01
6.26								
CB211	0.000	0.1	0.0	0.0	0.000	2.6	0	12:12
3.47								
CB212	0.000	0.3	0.0	0.0	0.001	8.0	0	12:12
5.94								
CB212A	0.000	1.7	0.0	0.0	0.000	55.3	0	12:12
1.36								
CB213	0.000	0.5	0.0	0.0	0.000	14.2	0	12:09
8.24								
CB214	0.000	0.3	0.0	0.0	0.002	30.3	0	12:08
18.95								

Outfall Loading Summary

Outfall Node	Flow Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10 ⁶ ltr
518A_(P-Stm)	93.05	8.15	64.71	0.655
System	93.05	8.15	64.71	0.655

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C2	CONDUIT	12.10	0 11:49	0.25	0.20	1.00
C4	CONDUIT	4.74	0 11:56	0.11	0.06	1.00
C5	CONDUIT	3.89	0 11:55	0.60	0.05	1.00
CB213	CONDUIT	16.59	0 11:50	0.39	0.24	1.00
CB214	CONDUIT	18.95	0 11:59	0.59	0.34	1.00
Pipe_-(104)_(P-Stm)_2	CONDUIT	64.71	0 12:09	1.04	0.45	0.41
Pipe_-(110)_(P-Stm)	CONDUIT	53.57	0 11:49	0.73	0.21	1.00
Pipe_-(111)_(P-Stm)	CONDUIT	34.44	0 11:49	0.48	0.11	1.00
Pipe_-(71)_(P-Stm)	CONDUIT	66.91	0 11:51	0.86	0.25	1.00
Pipe_-(72)_(P-Stm)	CONDUIT	43.59	0 11:50	0.88	0.14	1.00
OR1	ORIFICE	64.71	0 12:09			1.00
W1	WEIR	0.00	0 00:00			0.00
W2	WEIR	0.00	0 00:00			0.00
W3	WEIR	9.31	0 11:59			0.14
W4	WEIR	0.00	0 00:00			0.00
W5	WEIR	10.18	0 12:00			0.22
W6	WEIR	0.00	0 00:00			0.00
W7	WEIR	0.00	0 00:00			0.00
CB200	DUMMY	23.50	0 11:51			
CB201	DUMMY	13.93	0 11:49			
CB202	DUMMY	27.55	0 11:48			
CB203	DUMMY	4.32	0 11:59			
CB204	DUMMY	4.98	0 11:52			
CB205	DUMMY	4.51	0 11:55			
CB206	DUMMY	6.50	0 11:49			
CB207	DUMMY	12.51	0 11:49			
CB209	DUMMY	18.18	0 11:54			
CB210B	DUMMY	4.87	0 11:52			
CB212	DUMMY	4.68	0 11:50			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class				Norm Ltd	Inlet Ctrl			
		Up Dry	Down Dry	Sub Sup Crit	Up Down Crit					
C2	1.00	0.46	0.02	0.00	0.07	0.00	0.00	0.45	0.00	0.00
C4	1.00	0.48	0.01	0.00	0.05	0.00	0.00	0.45	0.01	0.00
C5	1.00	0.48	0.00	0.00	0.06	0.00	0.00	0.46	0.02	0.00
CB213	1.00	0.05	0.00	0.00	0.07	0.00	0.00	0.88	0.01	0.00
CB214	1.00	0.04	0.00	0.00	0.07	0.00	0.00	0.89	0.01	0.00
Pipe_-(104)_(P-Stm)_2	1.00	0.07	0.00	0.00	0.00	0.00	0.00	0.93	0.00	0.00
Pipe_-(110)_(P-Stm)	1.00	0.06	0.00	0.00	0.08	0.00	0.00	0.85	0.00	0.00
Pipe_-(111)_(P-Stm)	1.00	0.06	0.00	0.00	0.94	0.00	0.00	0.00	0.86	0.00
Pipe_-(71)_(P-Stm)	1.00	0.06	0.00	0.00	0.07	0.00	0.00	0.87	0.01	0.00
Pipe_-(72)_(P-Stm)	1.00	0.07	0.00	0.00	0.08	0.00	0.00	0.85	0.01	0.00

Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full Normal Flow	Hours Capacity Limited
	Both Ends	Upstream	Dnstream		
C2	1.48	1.48	1.53	0.01	0.01
C4	0.95	0.95	1.19	0.01	0.01
C5	0.89	0.89	1.22	0.01	0.01
CB213	1.19	1.19	1.44	0.01	0.01
CB214	1.31	1.31	1.44	0.01	0.01
Pipe_-(110)_(P-Stm)	1.62	1.62	1.73	0.01	0.01
Pipe_-(111)_(P-Stm)	1.44	1.44	1.62	0.01	0.01
Pipe_-(71)_(P-Stm)	1.24	1.24	1.38	0.01	0.01
Pipe_-(72)_(P-Stm)	1.43	1.43	1.55	0.01	0.01

Analysis begun on: Thu Aug 29 09:25:39 2024
Analysis ended on: Thu Aug 29 09:25:41 2024
Total elapsed time: 00:00:02

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

08/28/2024

Province:	Ontario
City:	Ottawa
Nearest Rainfall Station:	OTTAWA CDA RCS
Climate Station Id:	6105978
Years of Rainfall Data:	20

Project Name:	Navan Gas Bar
Project Number:	29899-002
Designer Name:	Bobby Pettigrew
Designer Company:	J.L. Richards & Associates Ltd
Designer Email:	bpettigrew@jlrichards.ca
Designer Phone:	343-804-5381
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:

Drainage Area (ha):	0.82
% Imperviousness:	73.64

Runoff Coefficient 'c': 0.74

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	19.63
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	66.00
Peak Conveyance (maximum) Flow Rate (L/s):	66.00
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	657
Estimated Average Annual Sediment Volume (L/yr):	534

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	80
EFO6	90
EFO8	95
EFO10	97
EFO12	99

Recommended Stormceptor EFO Model: EFO4

Estimated Net Annual Sediment (TSS) Load Reduction (%): 80

Water Quality Runoff Volume Capture (%): > 90

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

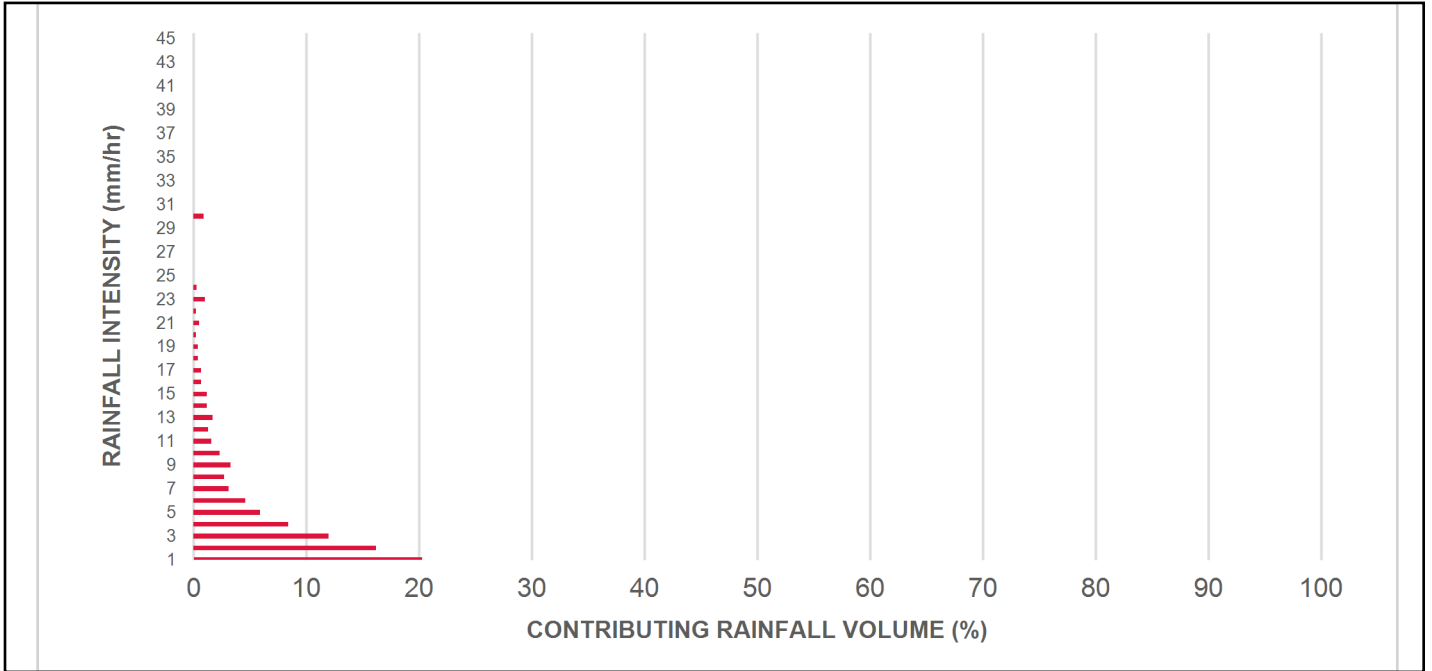
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.6	8.6	0.85	51.0	42.0	100	8.6	8.6
1.00	20.3	29.0	1.69	101.0	85.0	98	20.0	28.6
2.00	16.2	45.2	3.38	203.0	169.0	87	14.1	42.7
3.00	12.0	57.2	5.07	304.0	254.0	81	9.7	52.4
4.00	8.4	65.6	6.76	406.0	338.0	77	6.5	58.9
5.00	5.9	71.6	8.46	507.0	423.0	73	4.3	63.3
6.00	4.6	76.2	10.15	609.0	507.0	69	3.2	66.5
7.00	3.1	79.3	11.84	710.0	592.0	65	2.0	68.5
8.00	2.7	82.0	13.53	812.0	676.0	64	1.8	70.2
9.00	3.3	85.3	15.22	913.0	761.0	63	2.1	72.3
10.00	2.3	87.6	16.91	1015.0	846.0	63	1.4	73.8
11.00	1.6	89.2	18.60	1116.0	930.0	62	1.0	74.8
12.00	1.3	90.5	20.29	1218.0	1015.0	61	0.8	75.6
13.00	1.7	92.2	21.98	1319.0	1099.0	59	1.0	76.6
14.00	1.2	93.5	23.68	1421.0	1184.0	57	0.7	77.3
15.00	1.2	94.6	25.37	1522.0	1268.0	56	0.6	77.9
16.00	0.7	95.3	27.06	1623.0	1353.0	53	0.4	78.3
17.00	0.7	96.1	28.75	1725.0	1437.0	51	0.4	78.7
18.00	0.4	96.5	30.44	1826.0	1522.0	48	0.2	78.9
19.00	0.4	96.9	32.13	1928.0	1607.0	46	0.2	79.1
20.00	0.2	97.1	33.82	2029.0	1691.0	43	0.1	79.2
21.00	0.5	97.5	35.51	2131.0	1776.0	41	0.2	79.4
22.00	0.2	97.8	37.20	2232.0	1860.0	39	0.1	79.4
23.00	1.0	98.8	38.90	2334.0	1945.0	38	0.4	79.8
24.00	0.3	99.1	40.59	2435.0	2029.0	36	0.1	79.9
25.00	0.9	100.0	42.28	2537.0	2114.0	35	0.3	80.3
30.00	0.9	100.9	50.73	3044.0	2537.0	29	0.3	80.5
35.00	-0.9	100.0	59.19	3551.0	2959.0	25	N/A	80.3
40.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
45.00	0.0	100.0	66.00	3960.0	3300.0	22	0.0	80.3
Estimated Net Annual Sediment (TSS) Load Reduction =								80 %

Climate Station ID: 6105978 Years of Rainfall Data: 20

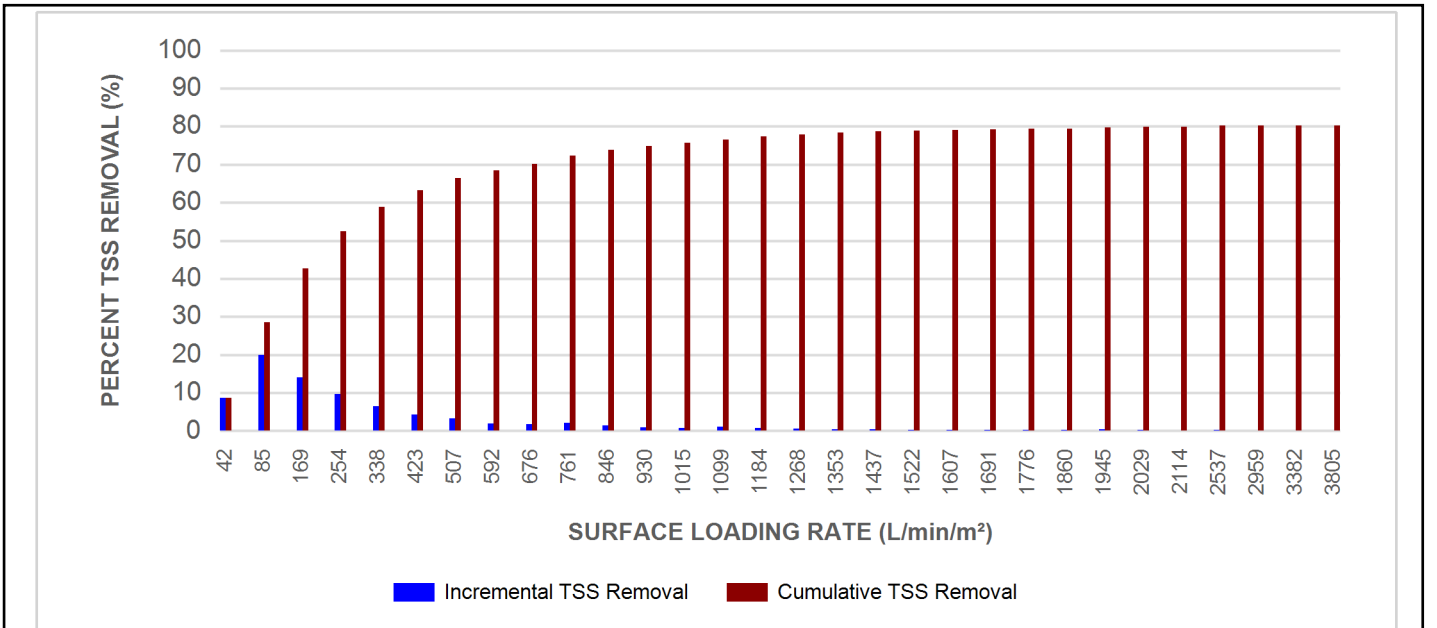


Stormceptor® EF Sizing Report

RAINFALL DATA FROM OTTAWA CDA RCS RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

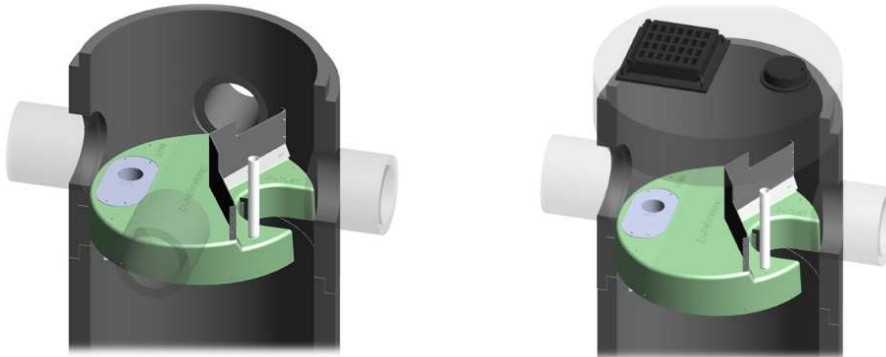
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

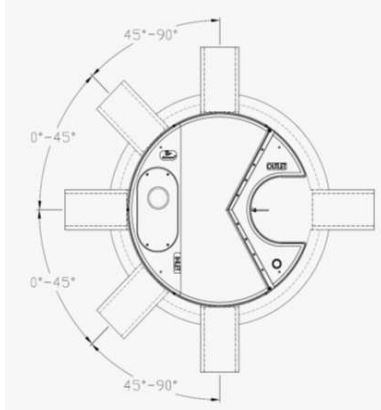
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall

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remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

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assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.



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